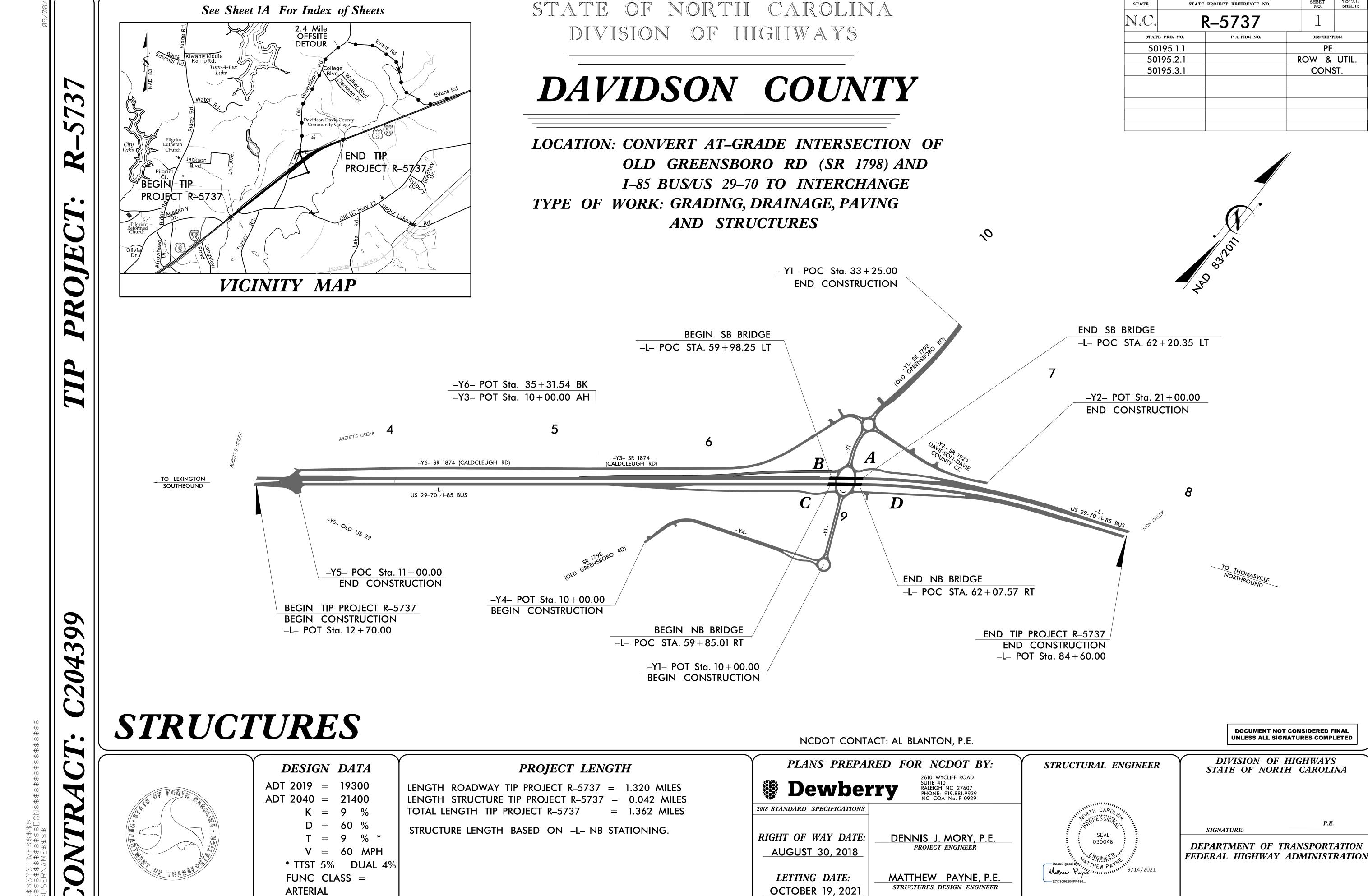
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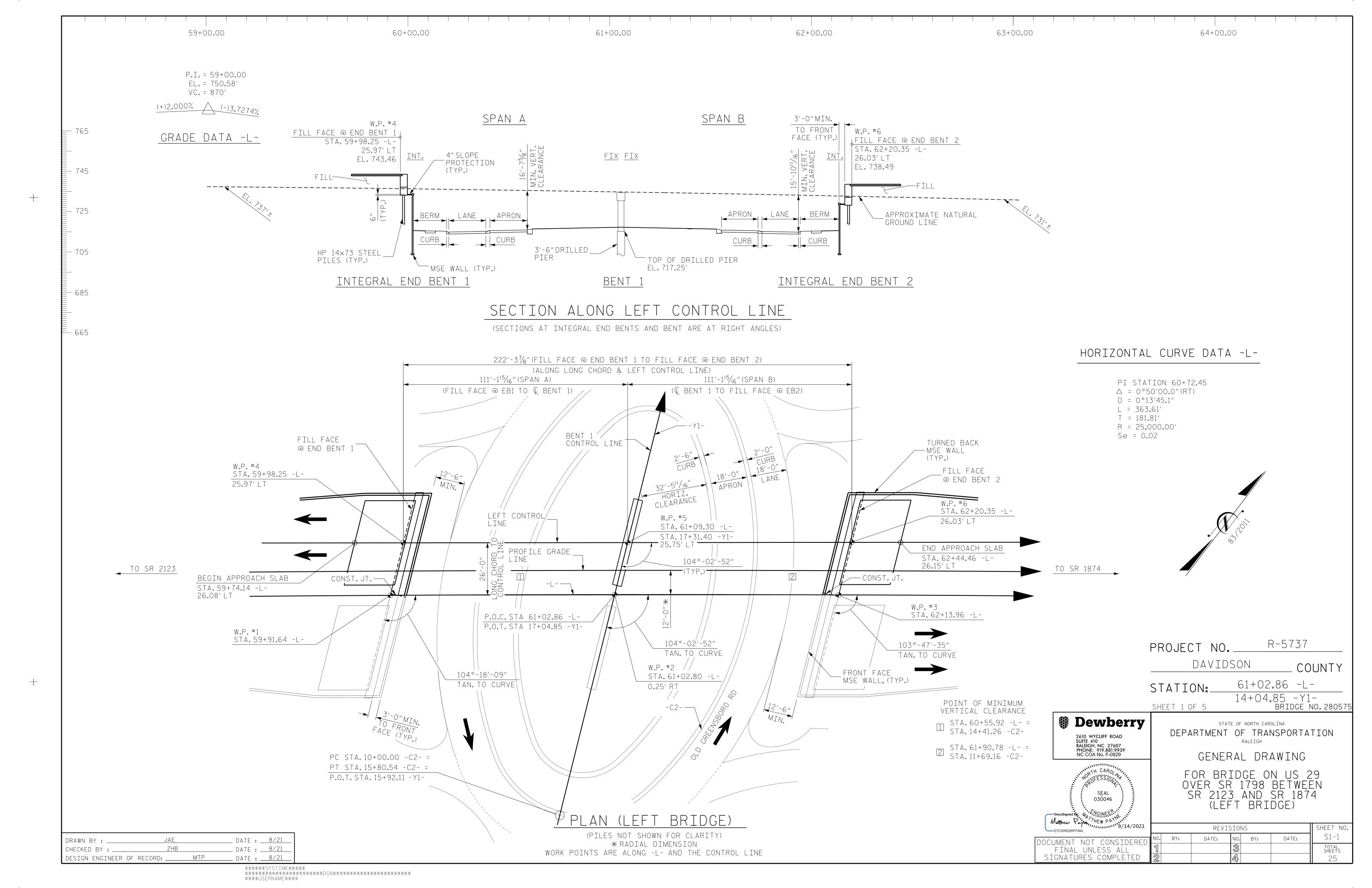
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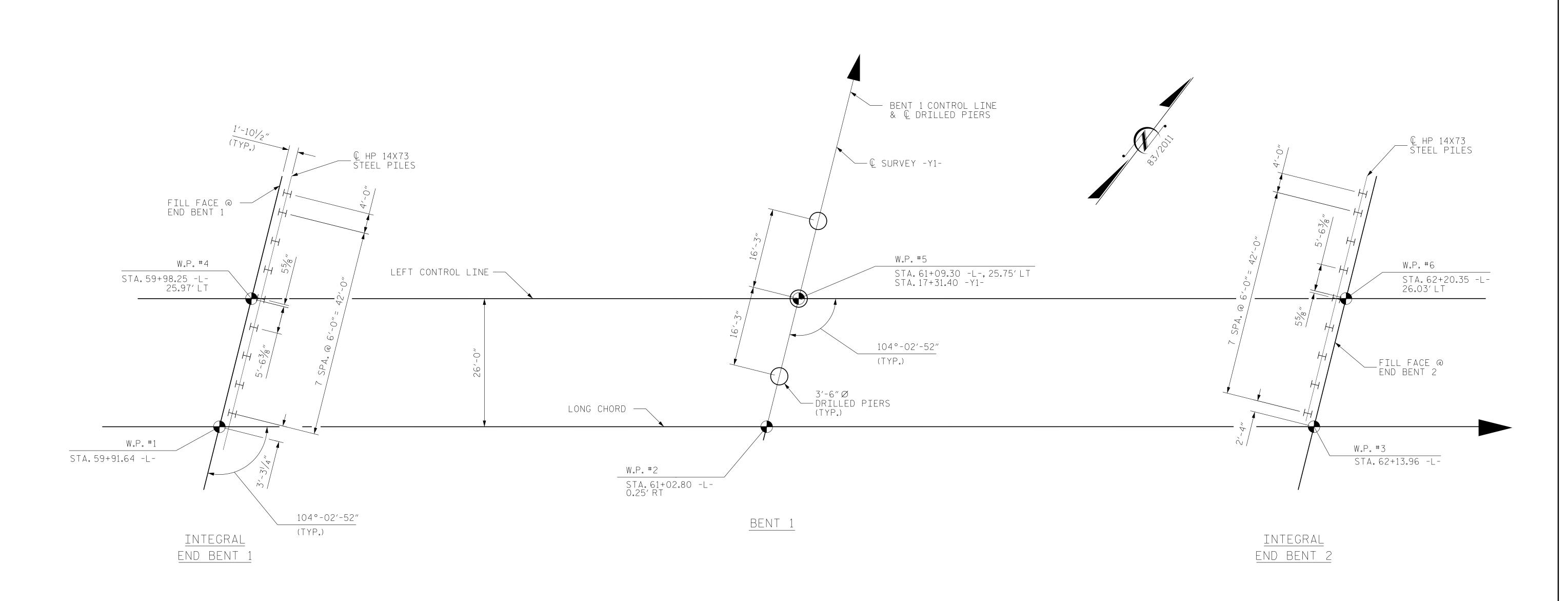
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SIGNATURE:

STATEWIDE TIER





FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO PILE CENTERLINE AT BOTTOM OF CAP

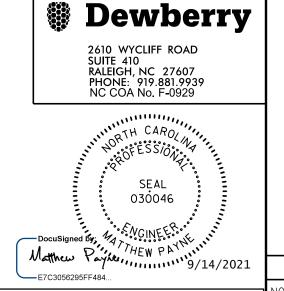
NOTES:

- 1. FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- 2. DRILLED PIERS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 538 TONS PER PIER.
- 3. INSTALL DRILLED PIERS AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 706 FT AND PENETRATION OF AT LEAST 8 FT INTO ROCK AS DEFINED BY ARTICLE 411-1 OF THE STANDARD SPECIFICATIONS.
- 4. CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- 5. FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- 6. PILES AT END BENT NO.1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 109 TONS PER PILE.
- 7. DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 185 TONS PER PILE.
- 8. DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 284 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR DOWNDRAG.
- 9. IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 60,000-80,000 FT-LBS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT NO. 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

- 10. TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING. SEE SECTION 450 OF THE STANDARD SPECIFICATIONS (AND FOR PILE DRIVING CRITERIA, SEE PILE DRIVING CRITERIA PROVISION).
- 11. DRILLED-IN PILES ARE REQUIRED FOR INTEGRAL END BENT NO.1. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 709 FT. FILL THE BOTTOM 3 FT OF HOLES FOR PILE EXCAVATION WITH CONCRETE AND THE REST OF HOLES WITH CLASS II OR III SELECT MATERIAL THAT MEETS SECTION 1016 OF THE STANDARD SPECIFICATIONS. FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

R-5737 PROJECT NO. DAVIDSON COUNTY 61+02.86 -L-STATION:

SHEET 2 OF 5

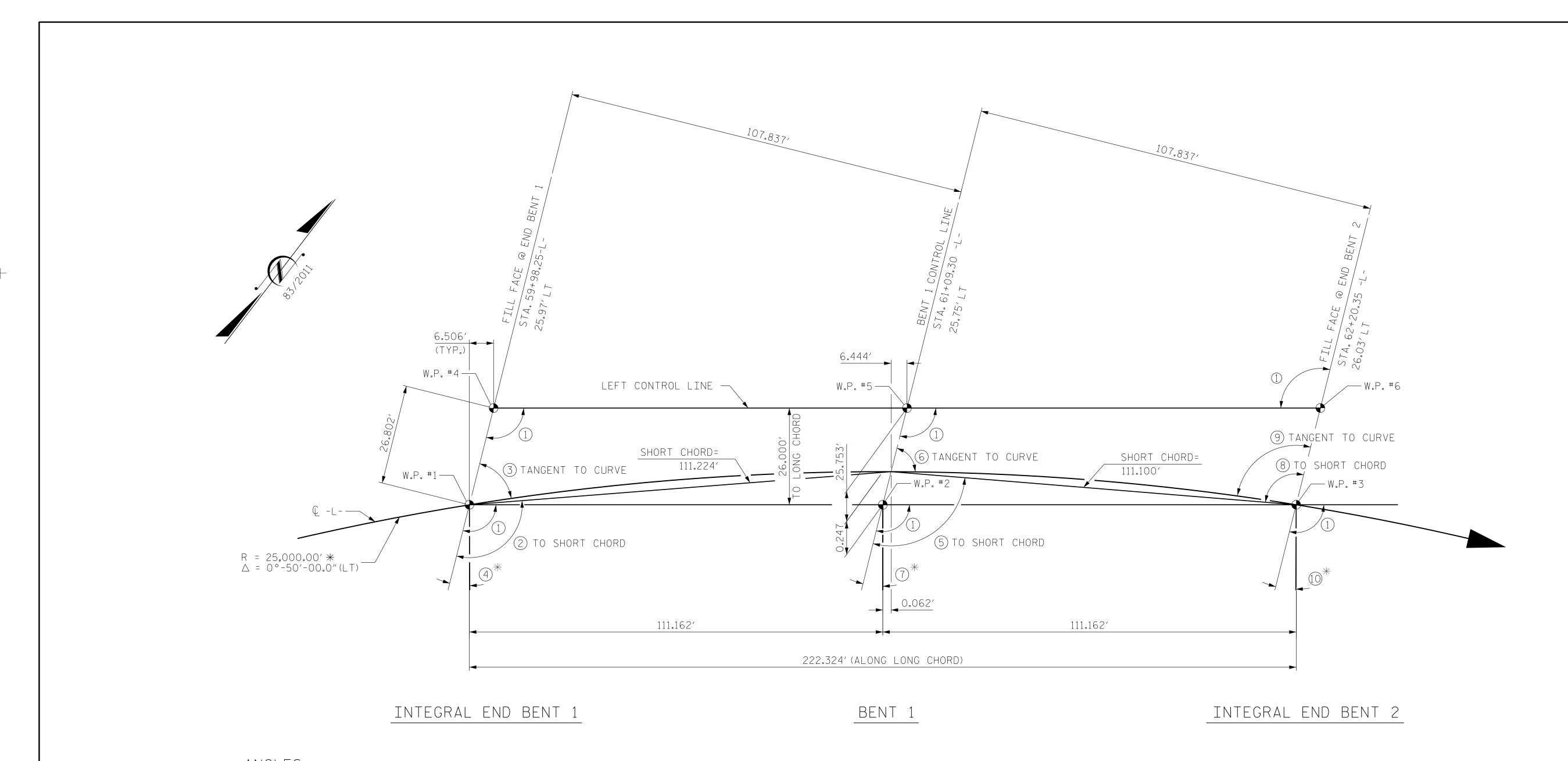


STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

> FOUNDATION LAYOUT PLAN

SHEET NO REVISIONS S1-2 DATE: DATE: BY: BY: OCUMENT NOT CONSIDEREI TOTAL SHEETS FINAL UNLESS ALL SIGNATURES COMPLETED

JAE DRAWN BY : ___ _ DATE : <u>8/21</u> _ DATE : <u>8/21</u> ZHB CHECKED BY : _ DESIGN ENGINEER OF RECORD: MTP _ DATE : <u>8/21</u>



ANGLES

1) 104°-02′-52″

2) 104°-10′-30″ SHORT CHORD

3 104°-18'-09" TANGENT TO CURVE

4) 14°-18′-09″ RADIAL

5) 103°-55′-13″ SHORT CHORD

6 104°-02′-52″ TANGENT TO CURVE

7) 14°-02′-52″ RADIAL

8 103°-55'-13" SHORT CHORD

9) 103°-47'-35" TANGENT TO CURVE

(10) 13°-47′-35″ RADIAL

LONG CHORD LAYOUT

(ALL END BENTS AND BENTS ARE PARALLEL)

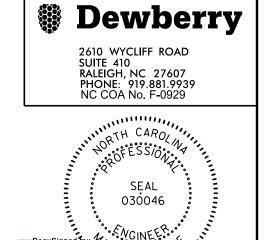
** CURVE EXAGGERATED FOR CLARITY

PROJECT NO. R-5737

DAVIDSON COUNTY

STATION: 61+02.86 -L-

SHEET 3 OF 5



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING
FOR BRIDGE ON US29
OVER SR 1798 BETWEEN
SE 2123 AND SR 1874
(LEFT BRIDGE)

Mathew Parks 19/14/2021

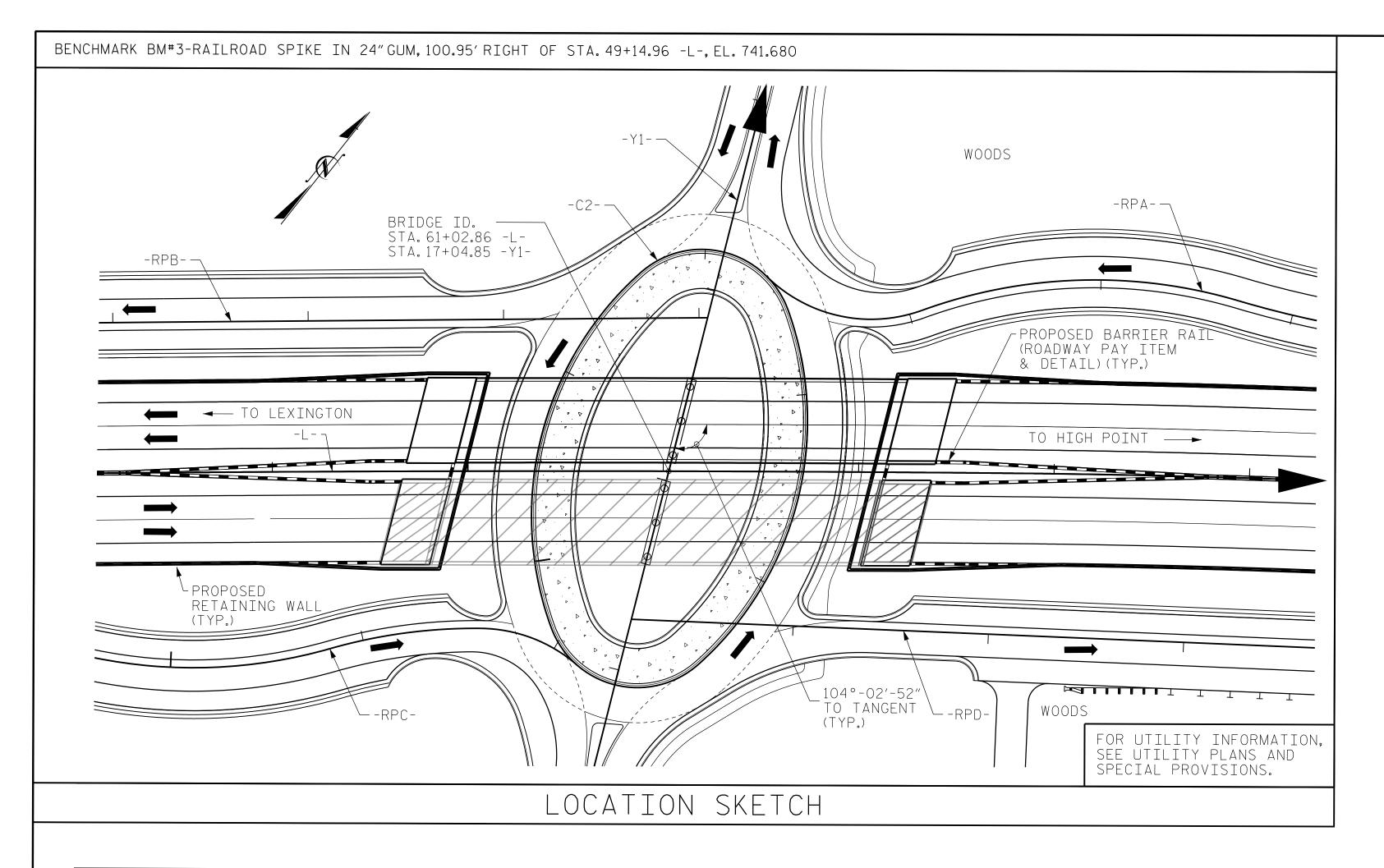
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SHEET NO. BY: DATE: NO. BY: DATE: S1-3

STOTAL SHEETS

25



			— ТОТ	AL B]	ILL OF MA	ATERIAL				
	PILE EXCAVATION NOT IN SOIL	3'-6"DIA. DRILLED PIER NOT IN SOIL	PDA TESTING	CSL TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL
	LIN. FT.	LIN.FT.	EA.	EA.	SQ.FT.	SQ.FT.	CU. YDS.	LUMP SUM	LBS.	LBS.
SUPERSTRUCTURE	_	-	-	_	9652	10,070	-	LUMP SUM		
END BENT NO.1	48	-	-	_	-	-	35.5	-	4,854	
BENT NO.1	-	33.8	-	_	-	-	40.1	-	11,089	1,676
END BENT NO.2	-	-	-	-	-	_	35.5	-	4,854	
TOTAL	48	33.8	1	1	9652	10,070	111.1	LUMP SUM	20,797	1,676

	TOTAL BILL OF MATERIAL ————————————————————————————————————								
	63" CON	PRESTRESSED CRETE DERS	PILE DRIVING EQUIPMENT SETUP FOR 14×73 STEEL PILES	HP	14 X 73 EEL PILES	CONCRETE BARRIER RAIL	ARCHITECTURAL CONCRETE SURFACE TREATMENT	4"SLOPE PROTECTION	ELASTOMERIC BEARINGS
	NO.	LIN.FT.	EA.	NO.	LIN. FT.	LIN.FT.	SQ.FT.	SQ. YDS.	LUMP SUM
SUPERSTRUCTURE	10	1097.71	-	_	_	641.2	623.2	-	LUMP SUM
END BENT NO.1	-	-	9	9	234	-	-	17	-
BENT NO.1	_	-	-	_	-	-	-	-	-
END BENT NO. 2	-	-	9	9	270	-	-	17	-
TOTAL	10	1097.71	18	18	504	641.2	623.2	34	LUMP SUM

GENERAL DRAWING NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR VERTICAL ARCHITECTURAL CONCRETE SURFACE TREATEMENT OF THE CONCRETE BARRIER RAIL, SEE SPECIAL PROVISIONS.

THE ELEVATIONS AND CLEARANCES SHOWN ON THE PLANS AT THE POINTS OF MINIMUM VERTICAL CLEARANCE ARE FROM THE BEST INFORMATION AVAILABLE.PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE ELEVATIONS ON THE EXISTING PAVEMENT AND CHECK THE CLEARANCE.REPORT ANY VARIATIONS TO THE ENGINEER.ANY PLAN REVISIONS NECESSARY TO ACHEIVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.

PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OF APPROVED BY THE ENGINEER.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

WORK SHALL NOT BE STARTED ON THIS BRIDGE (OR SPECIFIC PARTS OF BRIDGE) UNTIL ROADWAY SECTION HAS BEEN EXCAVATED.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

PROJECT NO. R-5737

DAVIDSON COL

STATION: 61+02.68 -L-

SHEET 4 OF 5

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING

COUNTY

FOR BRIDGE ON US 29 OVER SR 1798 BETWEEN SR 2123 AND SR 1874 (LEFT BRIDGE)

Dewberry

2610 WYCLIFF ROAD
SUITE 410

y	Matthew Payree, HEW PAY 9/14/2021 E7C3056295FF484
	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL
	FINAL UNLESS ALL SIGNATURES COMPLETED

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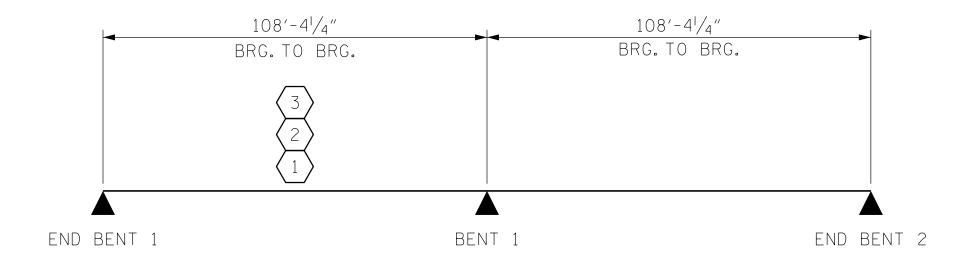
REVISIONS SHEET NO.

NO. BY: DATE: NO. BY: DATE:

3 TOTAL SHEETS
2 25

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	NGTH	I LIM	IIT ST	TATE				SE	RVICE	III	LIMI	T STA	4TE	
										MOMENT					SHEAR						MOMENT			
	LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD Factors (Y _{ll})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	LIVE-LOAD Factors (1/L)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.02		1.75	0.815	1.24	А	EL	54.18	0.847	1.41	А	EL	21.67	0.80	0.815	1.02	А	EL	54.18	
DESIG LOAD	Ν	HL-93 (OPERATING)	N/A		1.61		1.35	0.815	1.61	А	EL	54.18	0.847	2.03	А	EL	21.67	N/A						·
RATIN	G	HS-20 (INVENTORY)	36.000		1.46	52.26	1.75	0.815	1.77	А	EL	54.18	0.847	2.09	А	EL	21.67	0.80	0.815	1.46	А	EL	54.18	
		HS-20 (OPERATING)	36.000		2.30	82.80	1.35	0.815	2.30	А	EL	54.18	0.847	2.77	А	EL	21.67	N/A						
		SH	13.500		3.81	51.44	1.40	0.815	5.79	А	EL	54.18	0.847	7.08	А	I	21.67	0.80	0.815	3.81	А	EL	54.18	
		S3C	20.000		2.22	44.40	1.40	0.815	3.36	А	EL	54.18	0.847	4.07	А	I	21.67	0.80	0.815	2.22	А	EL	54.18	•
	ICLE	S3A	22.000		2.10	46.20	1.40	0.815	3.19	А	EL	54.18	0.847	3.58	А	I	21.67	0.80	0.815	2.10	А	EL	54.18	
		S4A	27.250		1.84	50.14	1.40	0.815	2.79	А	EL	54.18	0.847	3.32	А	I	21.67	0.80	0.815	1.84	А	EL	54.18	
	S) (S	S5A	34.925		1.62	56.58	1.40	0.815	2.46	А	EL	54.18	0.847	3.01	А	I	21.67	0.80	0.815	1.62	А	EL	54.18	
LEGAL	SINGL	S6A	35.550		1.46	51.90	1.40	0.815	2.22	А	EL	54.18	0.847	2.69	А	I	21.67	0.80	0.815	1.46	А	EL	54.18	
LOAD RATIN		S7B	39.950		1.32	52.73	1.40	0.815	2.01	А	EL	54.18	0.847	2.48	А	I	21.67	0.80	0.815	1.32	А	EL	54.18	
IVATIN		S7A	42.000	2	1.30	54.60	1.40	0.815	1.97	А	EL	54.18	0.847	2.52	А	I	21.67	0.80	0.815	1.30	А	EL	54.18	•
	4 ~	T4A	33.000		1.79	59.70	1.40	0.815	2.72	А	EL	54.18	0.847	3.20	А	I	21.67	0.80	0.815	1.79	А	EL	54.18	
	ACTOR ILER	T5B	33.075		1.58	52.26	1.40	0.815	2.39	А	EL	54.18	0.847	3.00	А	I	21.67	0.80	0.815	1.58	А	EL	54.18	
	TRAC1 -TRAIL TTST)	I I OA	41.600		1.43	59.49	1.40	0.815	2.18	А	EL	54.18	0.847	2.73	А	I	21.67	0.80	0.815	1.43	А	EL	54.18	•
	TRUCK SEMI-	Т7А	42.000	(3)	1.32	55.44	1.40	0.815	2.00	А	EL	54.18	0.847	2.51	А	I	21.67	0.80	0.815	1.32	А	EL	54.18	
		T7B	42.000		1.38	57.96	1.40	0.815	2.10	А	EL	54.18	0.847	2.38	А	I	21.67	0.80	0.815	1.38	А	EL	54.18	



LRFR SUMMARY

DRAWN BY: ______ JAE _____ DATE: 8/21
CHECKED BY: _____ ZHB ____ DATE: 8/21
DESIGN ENGINEER OF RECORD: _____ MTP ____ DATE: 8/21

LOAD FACTORS:

DESIGN LOAD RATING FACTORS LIMIT STATE YDC YDW

STRENGTH I 1.25 1.50

SERVICE III 1.00 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

10

2.

3.

(#) CONTROLLING LOAD RATING

(1) DESIGN LOAD RATING (HL-93)

 $\langle 2 \rangle$ DESIGN LOAD RATING (HS-20)

 $\sqrt{3}$ LEGAL LOAD RATING **

* * SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

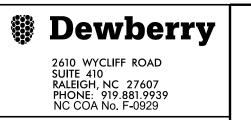
ER - EXTERIOR RIGHTGIRDER

PROJECT NO. R-5737

DAVIDSON COUNTY

STATION: 61+02.86 -L-

SHEET 5 OF 5



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

SHEET NO

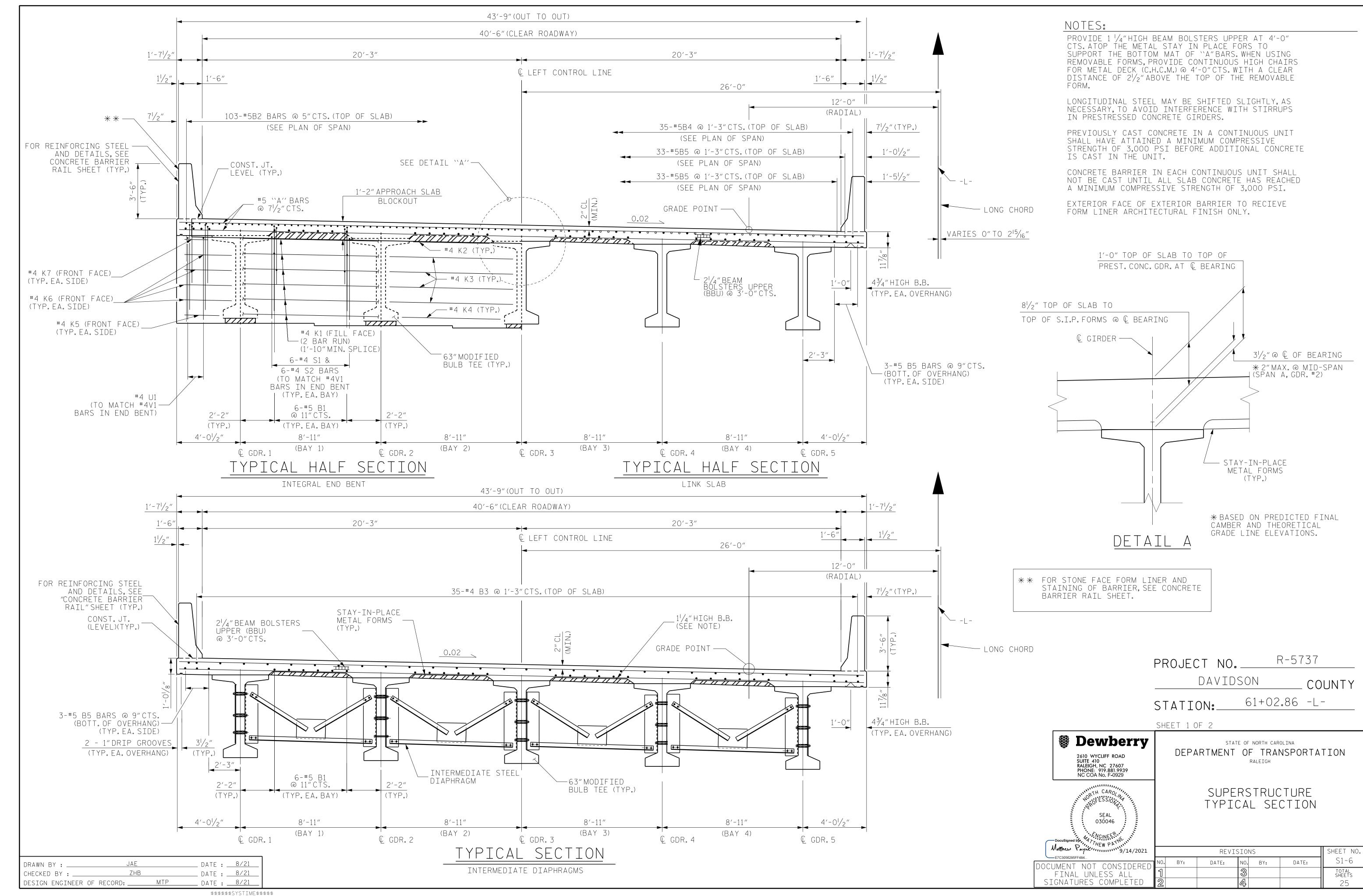
S1-5

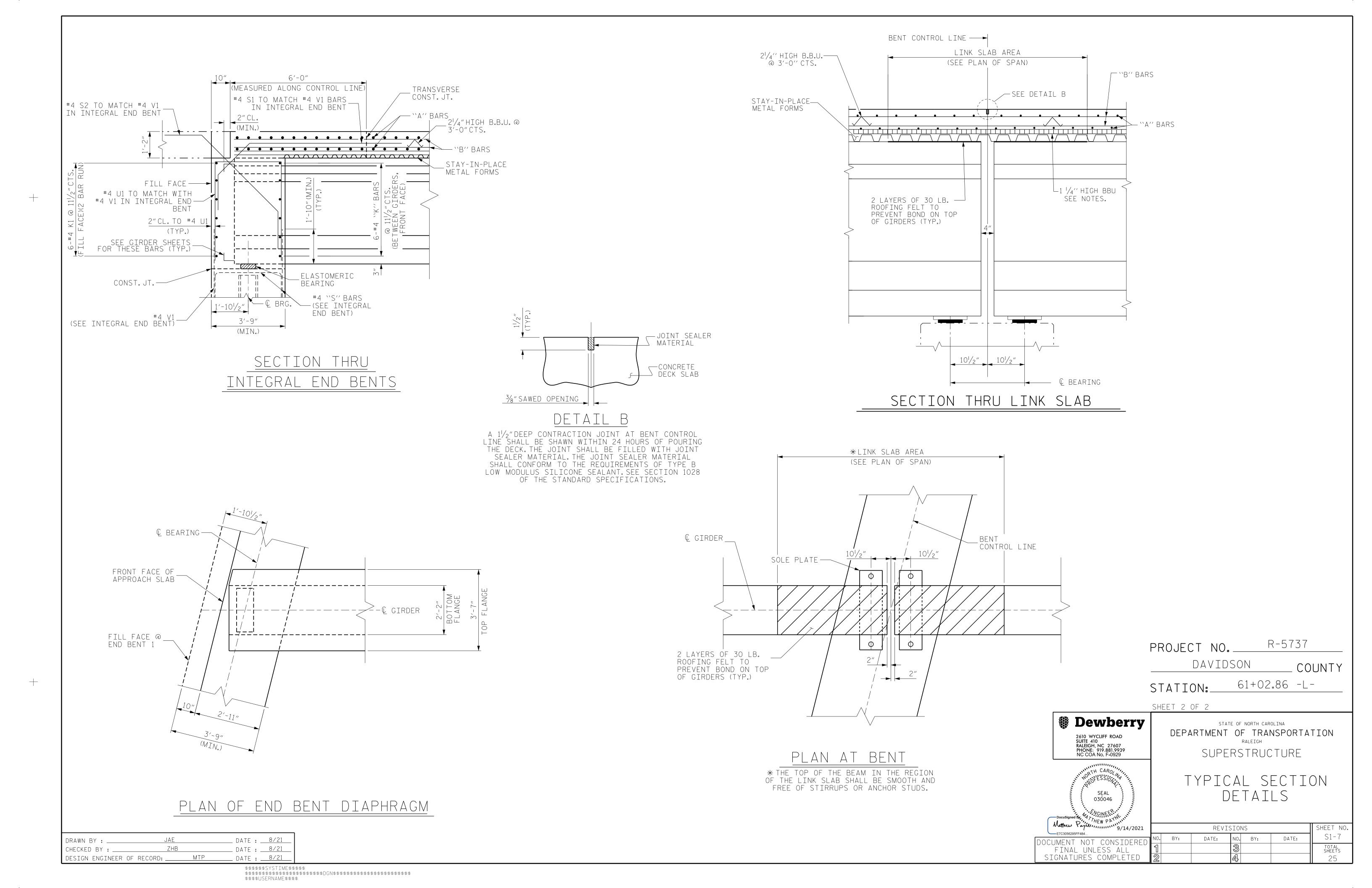
TOTAL SHEETS

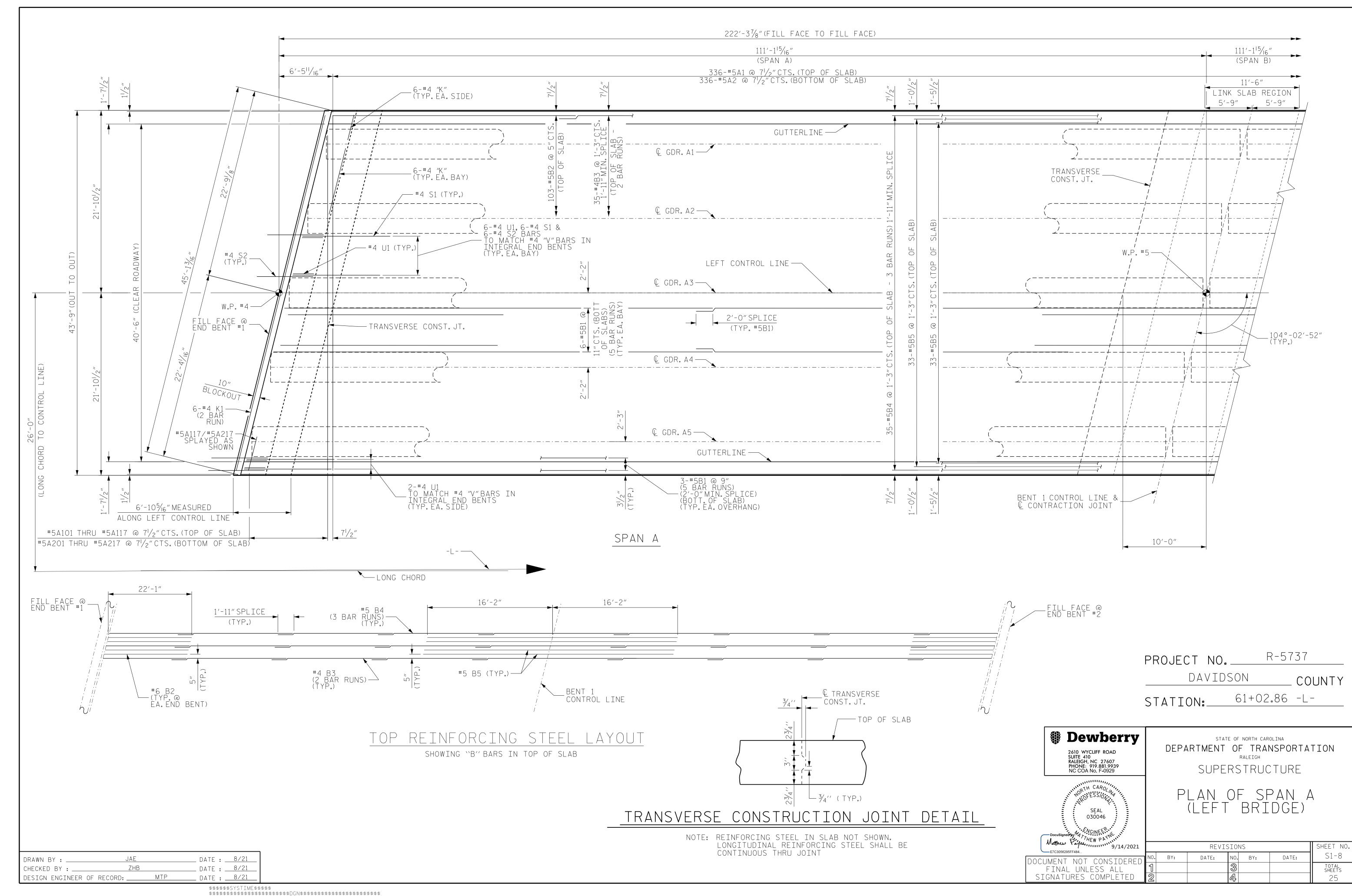
DATE:

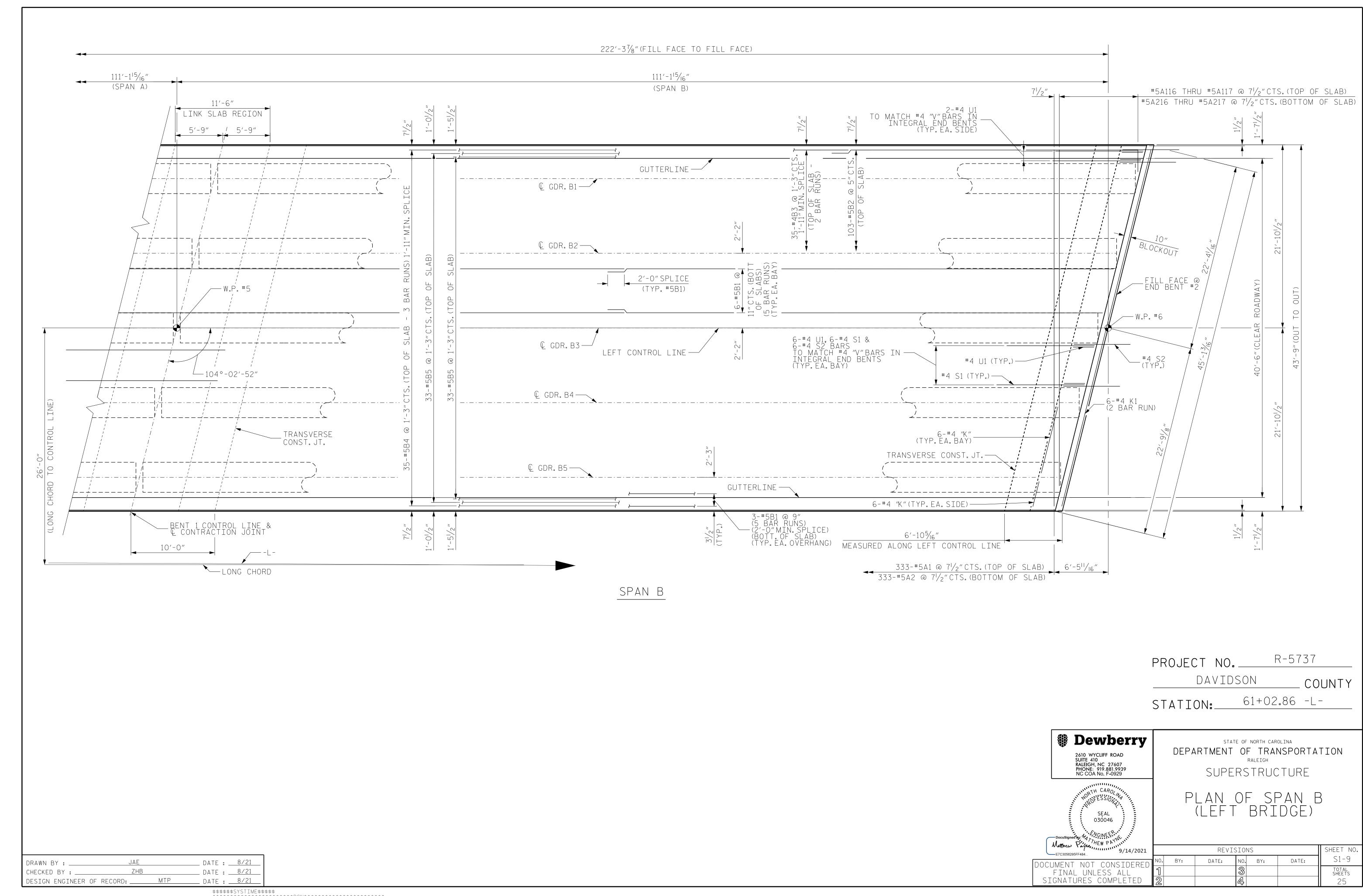
LRFR SUMMARY FOR PRESTRESSED CONCRETE GIRDERS (INTERSTATE TRAFFIC)

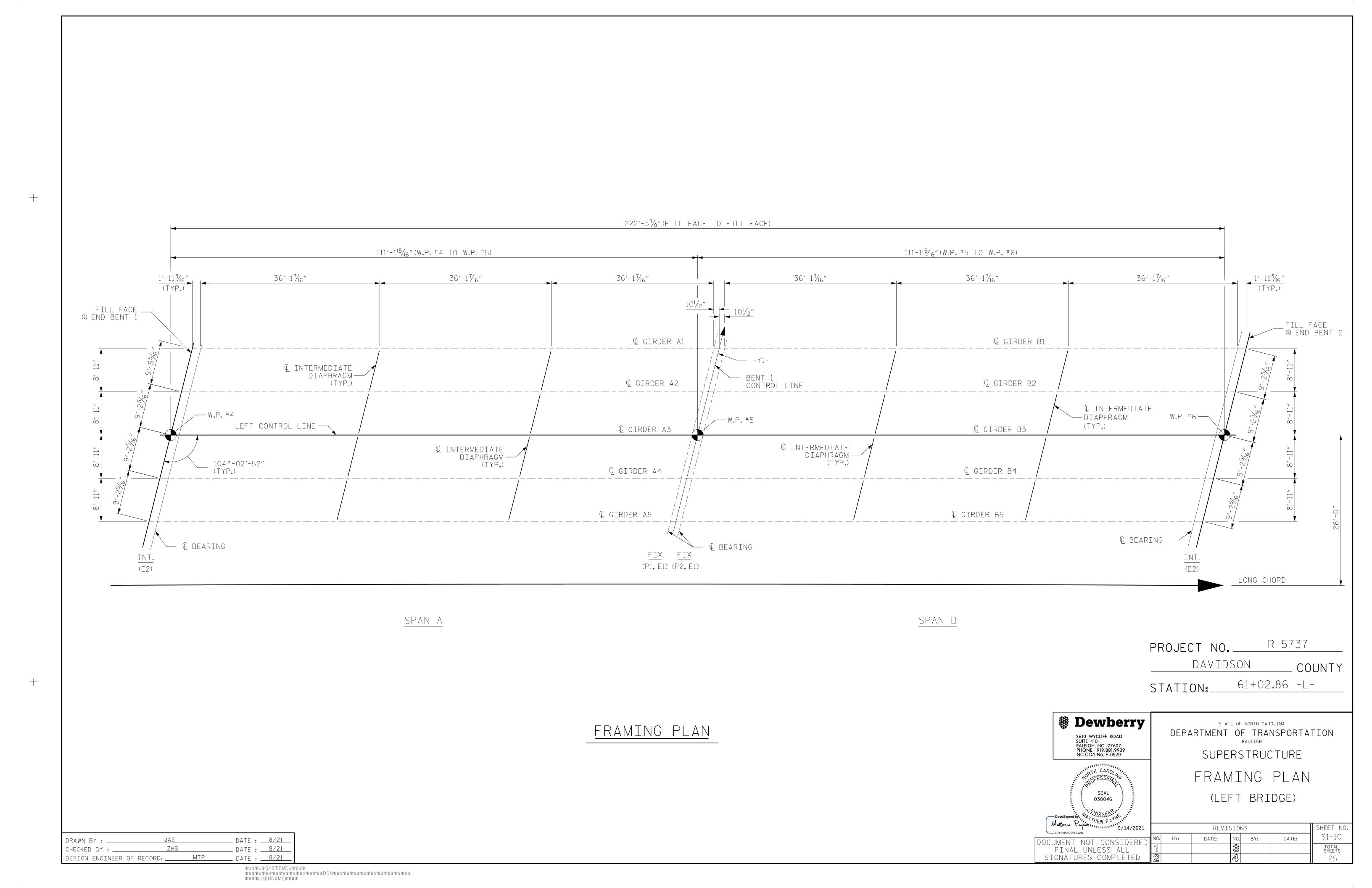
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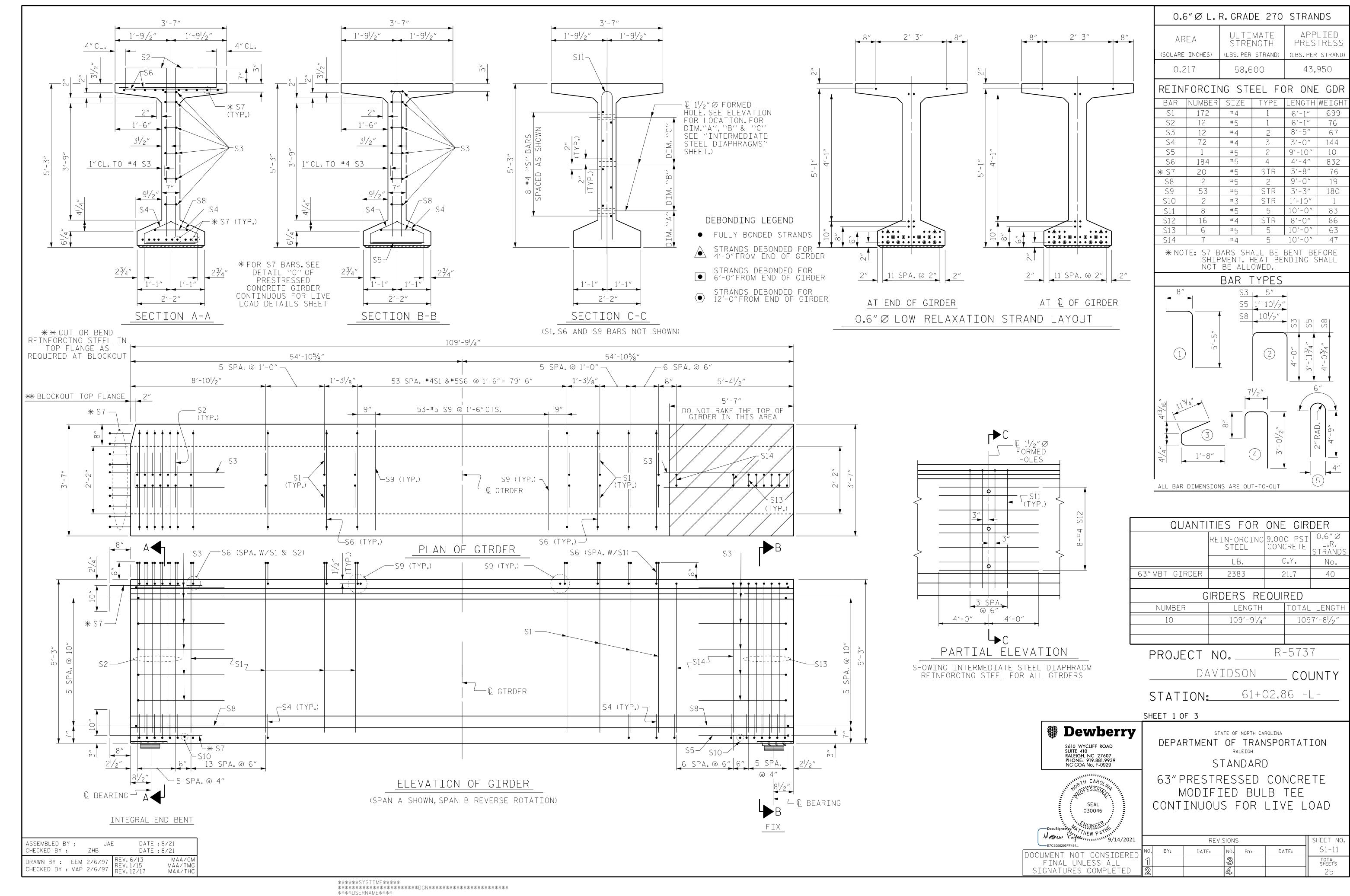


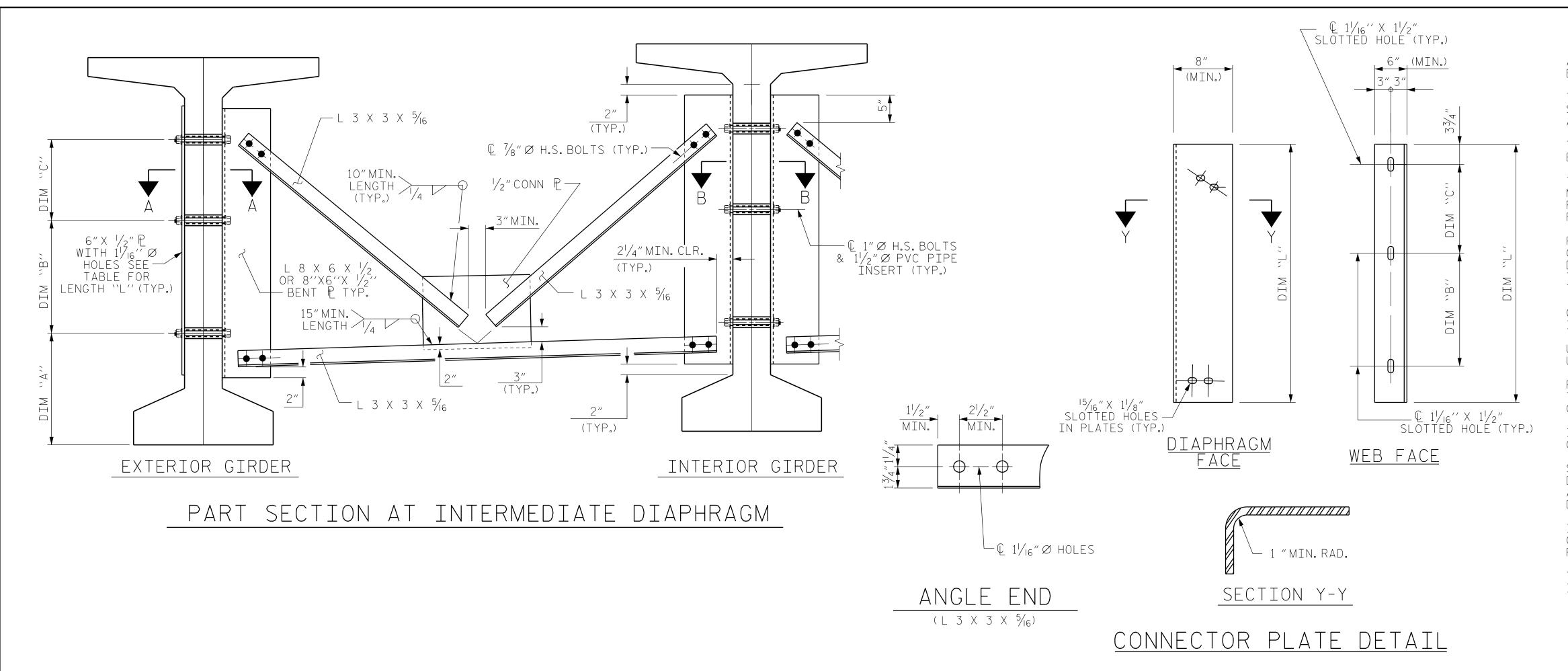












STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY A THERMAL SPRAYED COATING WITH A SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE DEPARTMENTS THERMAL SPRAYED COATINGS (METALLIZATION) PROGRAM, THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST $\frac{1}{4}$ "PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

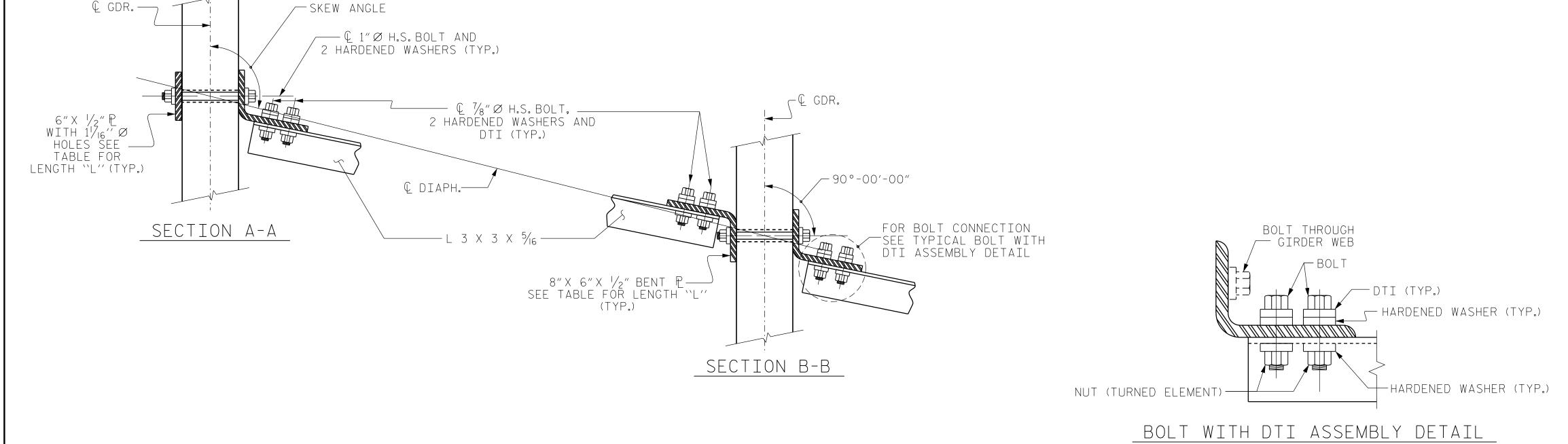
SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

TABLE

GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"	DIM "L"
63" BULB TEE	1'-07/8"	1'-51/2"	1'-51/2"	3′-5′′



Dewberry 2610 WYCLIFF ROAD SUITE 410 RALEIGH, NC 27607 PHONE: 919.881.9939 NC COA No. F-0929 030046

SHEET 2 OF 3 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR 63" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS REVISIONS

R-5737

61+02.86 -L-

COUNTY

SHEET NO S1-12 DATE: DATE: BY: BY: OCUMENT NOT CONSIDEREI TOTAL SHEETS FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT NO._

STATION:

DAVIDSON

_ DATE : <u>8/21</u> ZHB DESIGN ENGINEER OF RECORD: MTP _ DATE : <u>8/21</u> \$\$\$\$\$\$SYSTIME\$\$\$\$

_ DATE : <u>8/21</u>

JAE

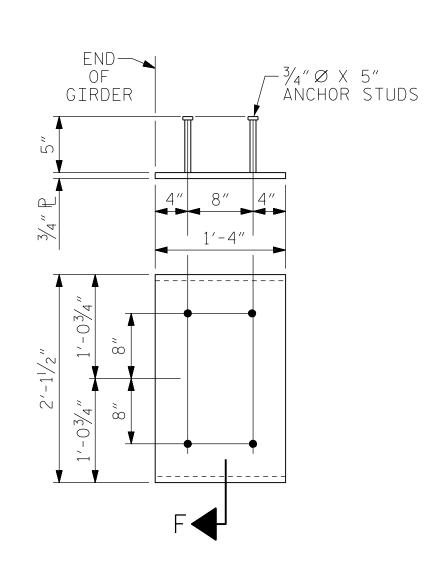
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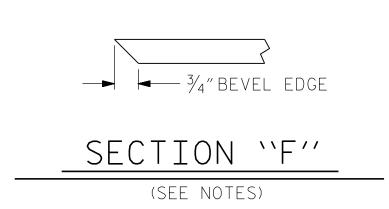
CHECKED BY : _

\$\$\$\$USERNAME\$\$\$\$

CONNECTION DETAILS

	DEAD LOAD DEFLECTION TABLE FOR GIRDERS - SPANS A & B																	
									GIRDERS 1	AND 5								
FORTIETH POINTS	0	.025 .05 .07	5 .10 .125	.15 .175	.20 .225 .25	.275 .30	.325 .35 .37	5 .40 .425	.45 .475 .50	.525 .55	.575 .60	.625 .65 .675	.70 .725	.75 .775 .80	.825 .85	.875 .90	.925 .95 .975	1.00
CAMBER (GIRDER ALONE IN PLACE)	0.00	0.089 0.131 0.16	9 0.203 0.23	0.254 0.271	0.281 0.284 0.281	0.271 0.254	0.231 0.203 0.16	59 0.131 0.131	0.089 0.045 0.045	5 0.089 0.131	0.169 0.203	0.231 0.254 0.271	0.281 0.338	0.281 0.271 0.254	1 0.231 0.203	3 0.169 0.131	0.131 0.089 0.045	5 0.00
*DEFLECTION DUE TO SUPER IMPOSED D.	L.\ 0.00	.013 0.026 0.03	9 0.052 0.06	1 0.076 0.089	0.101 0.111 0.121	0.130 0.140	0.130 0.140 0.15	59 0.165 0.167	0.174 0.171 0.174	0.171 0.169	0.167 0.165	0.159 0.153 0.147	0.141 0.147	0.131 0.121 0.102	2 0.112 0.090	0.077 0.052	0.065 0.039 0.026	0.00 ć
FINAL CAMBER	1 0	1/8" 1/4" 5/16'	7/16" 9/16"	5/8" 3/4"	13/16" 7/8" 15/16"	1" 11/16"	15/16" 19/16" 13/8	" 11/4" 15/16"	11/4" 15/16" 15/16"	15/16" 15/16"	15/16" 11/4"	11/4" 13/16" 13/16"	11/16" 11/8"	1" 15/16" 13/16"	7/8" 11/16"	5/8" 7/16"	9/16" 5/16" 1/4"	0
									GIRDERS 2	THRU 4								
FORTIETH POINTS	0	.025 .05 .07!	5 .10 .125	.15 .175	.20 .225 .25	.275 .30	.325 .35 .37	5 .40 .425	.45 .475 .50	.525 .55	.575 .60	.625 .65 .675	.70 .725	.75 .775 .80	.825 .85	.875 .90	.925 .95 .975	1.00
CAMBER (GIRDER ALONE IN PLACE)	0.00	0.076 0.111 0.14	3 0.172 0.196	0.215 0.229	0.238 0.241 0.238	0.229 0.215	0.196 0.172 0.14	13 0.111 0.131	0.076 0.038 0.038	8 0.076 0.111	0.143 0.172	0.196 0.215 0.229	0.238 0.241	0.238 0.229 0.215	0.196 0.172	0.143 0.111	0.131 0.076 0.038	3 0.00
*DEFLECTION DUE TO SUPER IMPOSED D.	L. \ 0.00	0.014 0.027 0.04	1 0.054 0.06	7 0.080 0.093	0.106 0.116 0.126	0.137 0.147	0.153 0.160 0.16	66 0.173 0.175	0.177 0.179 0.182	2 0.179 0.177	0.175 0.173	0.166 0.160 0.154	0.147 0.137	0.127 0.117 0.106	0.094 0.081	0.068 0.055	0.041 0.027 0.014	1 0.00
FINAL CAMBER	0	1/8" 3/16" 5/16'	7/16" 1/2"	9/16" 11/16"	3/4" 13/16" 7/8"	15/16" 1"	11/16" 15/16" 11/2	" 13/16" 13/8"	11/2" 111/16" 17/8"	111/16" 11/2"	13/8" 13/16"	11/8" 11/16" 11/16"	1" 15/16"	7/8" 13/16" 3/4"	11/16" 9/16"	1/2" 7/16"	5/16" 3/16" 1/8"	0





EMBEDDED PLATE 'B-1" DETAILS FOR 63" MODIFIED BULB TEES

(2 REQ'D PER GIRDER)

JAE

ZHB

DESIGN ENGINEER OF RECORD: MTP

DRAWN BY : ___

CHECKED BY : _

_ DATE : <u>8/21</u>

* \$7 (TYP.)

* \$3\frac{8}{8}'' \\
\frac{1\frac{7}{8}''}{1\frac{7}{4}''} \\
\frac{2''}{2''-2''}

* \$3\frac{8}{8}'' \\
\frac{3\frac{8}{8}''}{3\frac{8}{8}''}

\frac{1\frac{7}{4}''}{2''-2''}

\frac{2''-2''}{2''-2''}

3'-7"

DETAIL "C"

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

APPLY EPOXY PROTECTIVE COATING TO END OF GIRDER SURFACES INDICATED IN ELEVATION VIEW.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE ''B''REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2"BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 7000 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

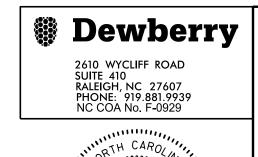
THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF $\frac{1}{4}$ ".

WHEN DRAPED STRANDS ARE DETAILED, THE LONGITUDINAL LOCATION OF THE HOLD DOWN DEVICES SHALL BE WITHIN 6" OF THE LOCATION SHOWN AND THE CENTER OF GRAVITY OF THE GROUP OF DRAPED STRANDS SHALL BE LOCATED WITHIN $\frac{1}{2}$ " OF THE THEORETICAL LOCATION SHOWN.

A 2" \times 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 63" AND 72" MODIFIED BULB TEES ONLY.

PROJECT NO. _____R-5737 _____DAVIDSON ____COUNTY STATION: ____61+02.86 -L-

SHEET 3 OF 3



DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

PRESTRESSED CONCRETE GIRDER

CONTINUOUS FOR LIVE LOAD

DETAILS

Mathew Payternin'''9/14/2021

E7C3056295FF484...

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

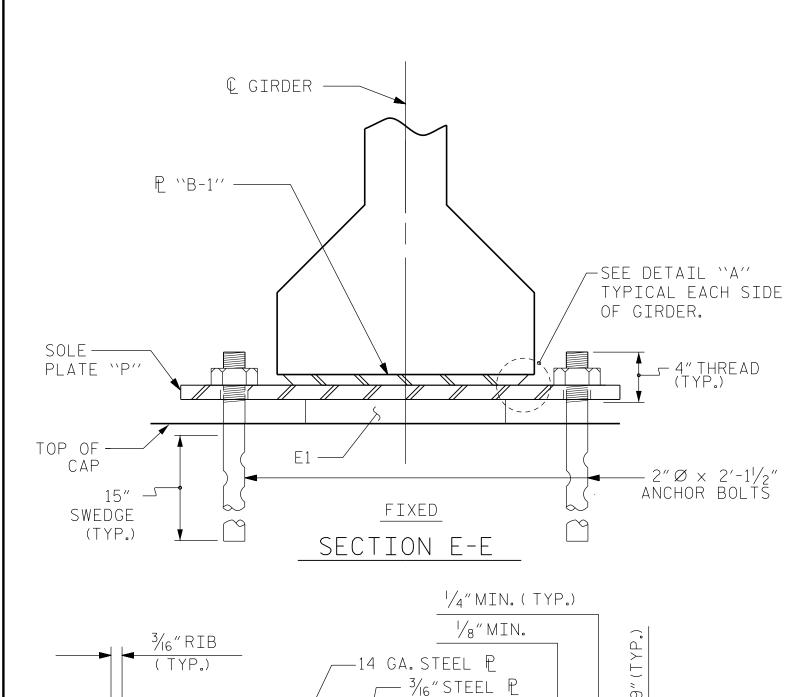
REVISIONS

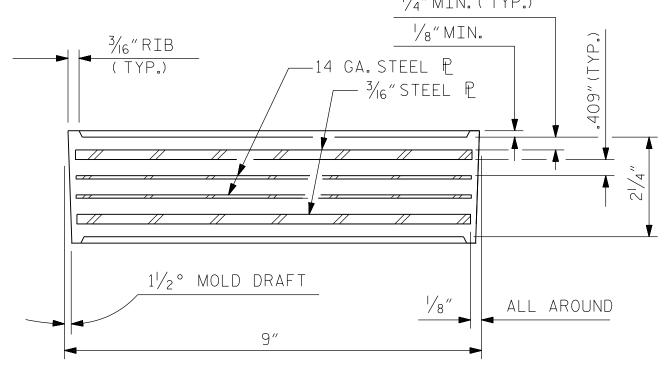
NO. BY: DATE: NO. BY: DATE: NO. BY: DATE: SIGNATURES COMPLETED

REVISIONS

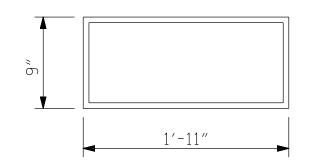
SHEET NO. S1-13

TOTAL SHEETS
25





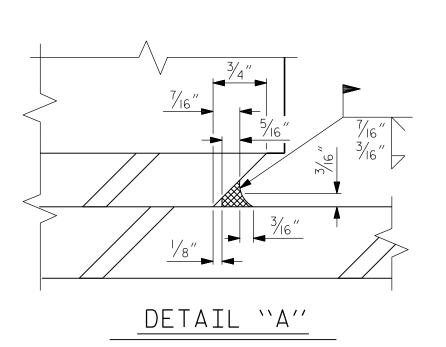
TYPICAL SECTION OF ELASTOMERIC BEARINGS

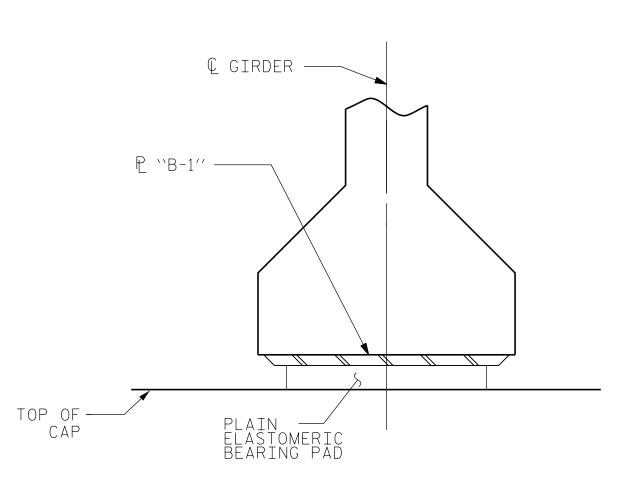


E1 (10 REQ'D)

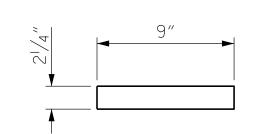
PLAN VIEW OF ELASTOMERIC BEARING

TYPE V

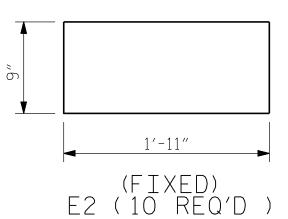




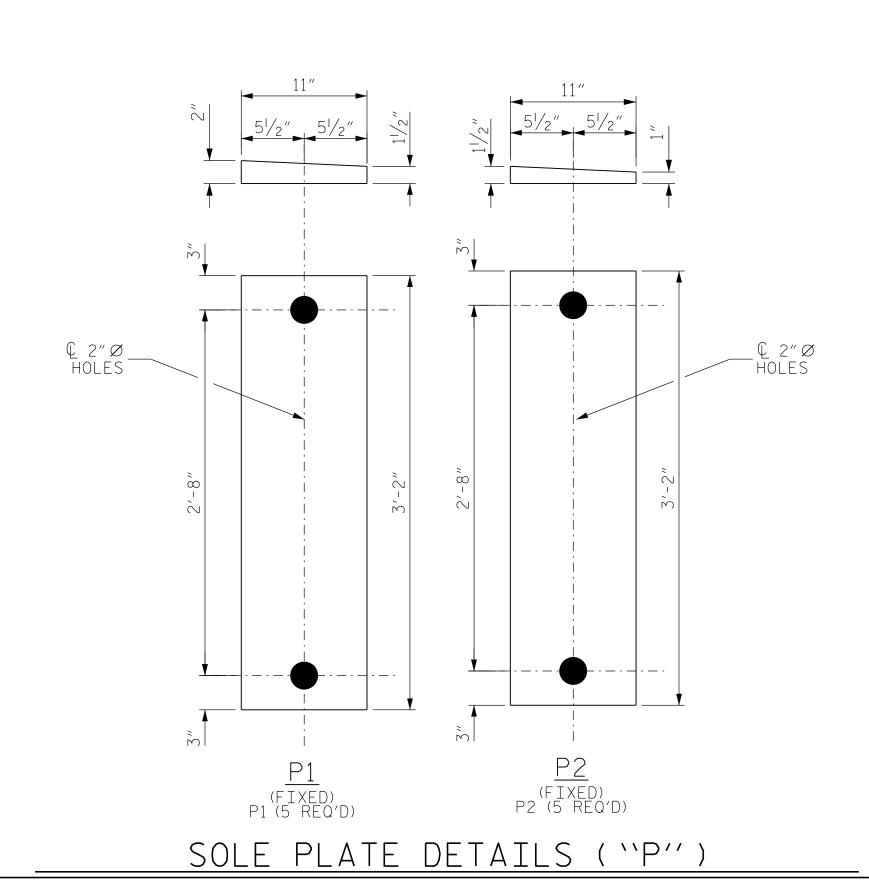
SECTION F-F

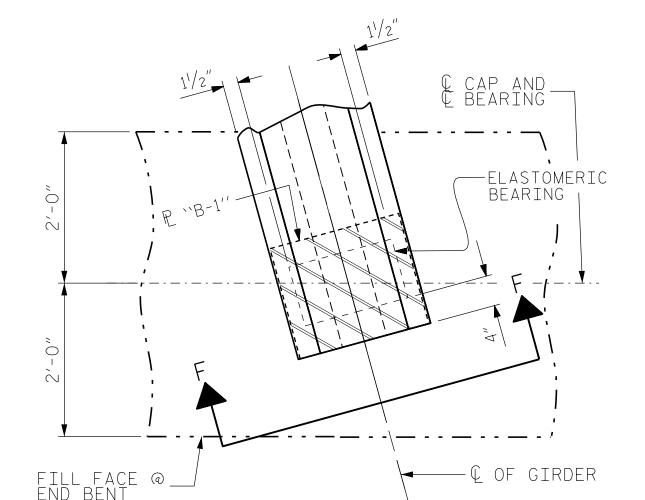


TYPICAL SECTION OF ELASTOMERIC BEARING



PLAN VIEW OF ELASTOMERIC BEARING





TYPICAL PLAN @ END BENT

NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2"Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

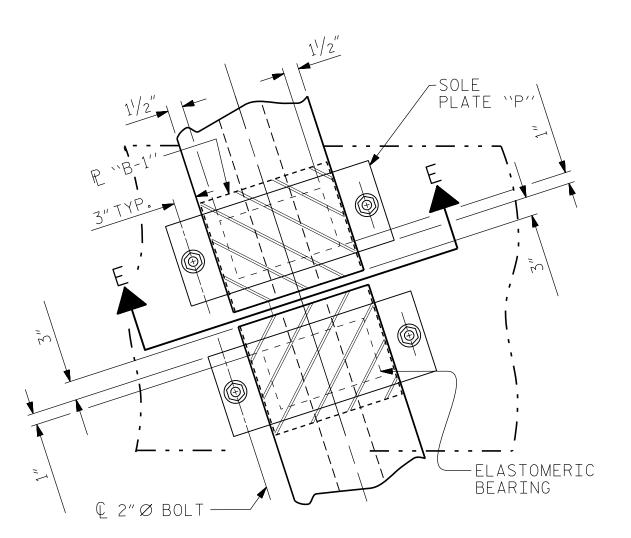
ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. NO SHOP DRAWINGS ARE REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

ALL SOLE PLATES SHALL BE AASHTO M270 GRADE 36.



MAXIMUM ALLOWABLE SERVICE LOADS

D.L.+L.L. (NO IMPACT)

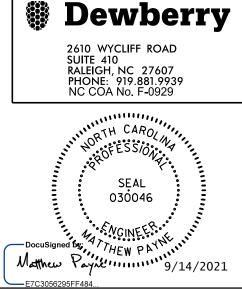
TYPE V 365 k

TYPICAL PLAN @ BENT

PROJECT NO. R-5737

DAVIDSON COUNTY

STATION: 61+02.86 -L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

ELASTOMERIC BEARING

———— DETAILS ———

PRESTRESSED CONCRETE GIRDER SUPERSTRUCTURE

Mathew Payie 9/14/2021

REVISIONS

SHEET NO

E7C3056295FF484...

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL

SIGNATURES COMPLETED 2

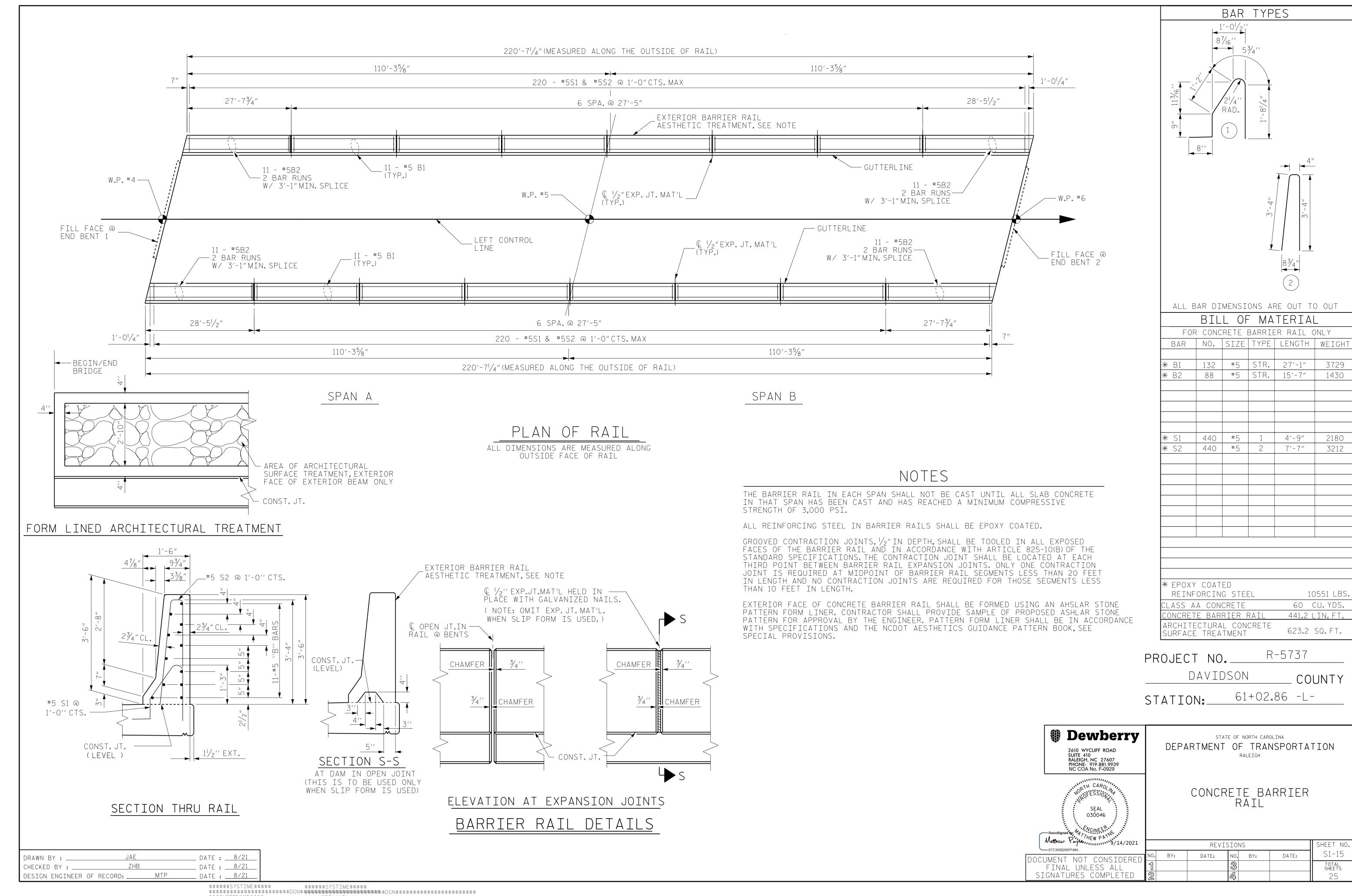
SHEET NO

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SHEET NO

STOTAL SHEETS

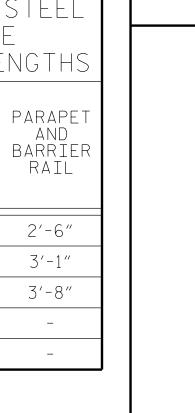
25

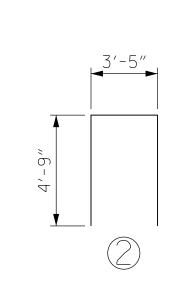


						REI	NFOR	CING	BAR	SCHED	ULE						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	336	#5	STR	43′-5″	15215	Α2	336	#5	STR	43′-5″	15215	* S1	24	#4	1	11'-11"	191
												* S2	24	#4	1	11'-0"	176
* A101	2	#5	STR	40'-11"	85	A201	2	#5	STR	40'-11"	85						
* A102	2	#5	STR	38′-5″	80	A202	2	#5	STR	38′-5″	80	U1	28	#4	2	12'-11"	242
* A103	2	#5	STR	35′-11″	75	A203	2	#5	STR	35′-11″	75	U2	24	#4	2	11'-7"	186
* A104	2	#5	STR	33′-5″	70	A204	2	#5	STR	33′-5″	70						
* A105	2	#5	STR	30′-11″	64	A205	2	#5	STR	30′-11″	64	REINFO	RCING ST	EEL		2	7,331 LBS.
* A106	2	#5	STR	28'-5"	59	A206	2	#5	STR	28'-5"	59	* EPOXY	COATED	REINF	ORCING	STEEL 30	,426 LBS.
* A107	2	#5	STR	25′-11″	54	A207	2	#5	STR	25′-11″	54						
* A108	2	#5	STR	23'-5"	49	A208	2	#5	STR	23'-5"	49						
* A109	2	#5	STR	20'-11"	44	A209	2	#5	STR	20'-11"	44						
* A110	2	#5	STR	18'-5"	38	A210	2	#5	STR	18′-5″	38						
* A111	2	#5	STR	15'-11"	33	A211	2	#5	STR	15'-11"	33						
* A112	2	#5	STR	13′-5″	28	A212	2	#5	STR	13′-5″	28						
* A113	2	#5	STR	10'-11"	23	A213	2	#5	STR	10'-11"	23						
* A114	2	#5	STR	8'-5"	18	A214	2	#5	STR	8'-5"	18						
* A115	2	#5	STR	5'-11"	12	A215	2	#5	STR	5'-11"	12						
* A116	2	#5	STR	3'-6"	7	A216	2	#5	STR	3′-6″	7						
* A117	2	#5	STR	1'-10"	4	A217	2	#5	STR	1'-10"	4						
B1	150	#6	STR	45′-8″	10289	K1	24	#4	STR	22'-11"	367						
* B2	206	#5	STR	22'-2"	4763	K2	8	#4	STR	5′-3″	28						
* B3	140	#5	STR	28'-10"	4210	K3	32	#4	STR	8'-3"	176						
* B4	105	#5	STR	26'-6"	2902	K4	8	#4	STR	6′-8″	36						
* B5	66	#5	STR	32'-4"	2226	K5	4	# 4	STR	2'-8"	7						
						K6	16	#4	STR	3′-6″	37						
						Κ7	4	#4	STR	2'-0"	5						

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE									
FOLLOWING MINIMUM SPLICE LENGTHS									
BAR SIZE	SIZE AND BARRIER RAÍL BARRIER								
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL				
#4	1'-11"	1'-7"	1'-11"	1'-7"	2'-6"				
#5	2′-5″	2'-0"	2'-5"	2'-0"	3'-1"				
#6	2'-10"	2'-5"	3'-7"	2'-5"	3′-8″				
#7	4'-2"	2'-9"	-	-	-				
#8	4'-9"	3'-2"	_	_	_				

GROOVING BRI	DGE FL	OOR
APPROACH SLABS	1,810	SQ.FT
BRIDGE DECK	8,260	SQ.FT
TOTAL	10,070	SQ.FT





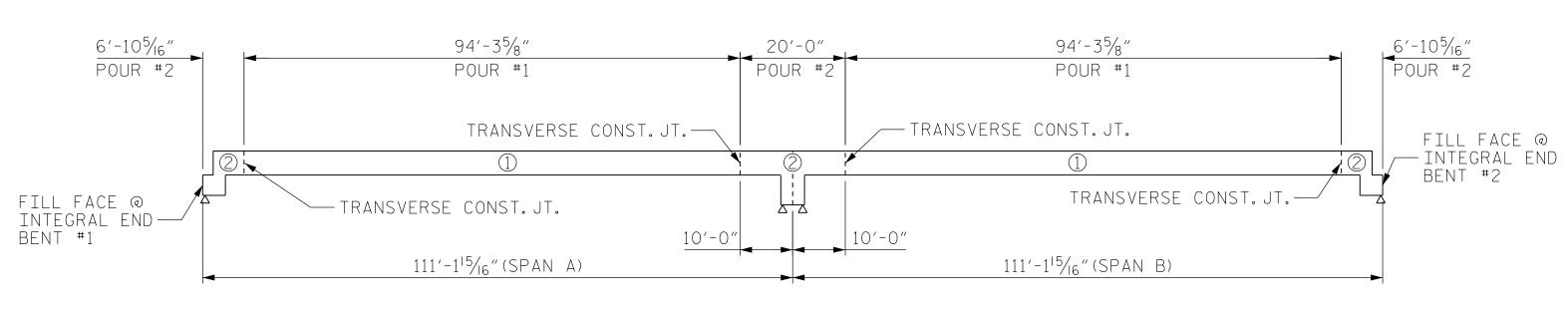
-BAR TYPES ----

ALL BAR DIMENSIONS ARE OUT TO OUT.

	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL
	(CU. YDS.)	(LBS.)	(LBS.)
POUR #1	275.1		
POUR #2	111.6		
TOTAL	386.7	27,331	30,426

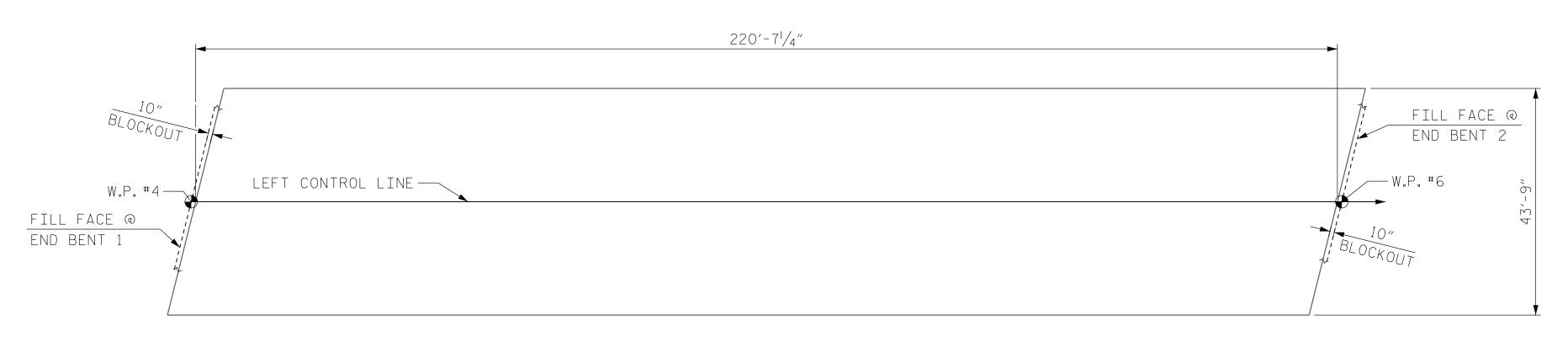
* * QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

8'-0'' $1'-8\frac{1}{2}''$



POURING SEQUENCE

POUR 2 CAN NOT BE STARTED UNTIL BOTH ADJACENT 1 POURS REACH A MINIMUM OF 3000 PSI

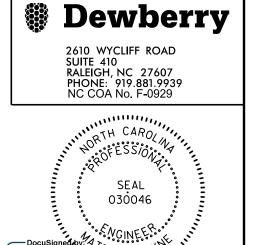


LAYOUT FOR COMPUTING AREA
REINFORCED CONCRETE DECK SLAB
(SQ.FT. = 9,652)

____ DATE : <u>8/21</u> DRAWN BY : ____ ZHB _ DATE : <u>8/21</u> _ DATE : <u>8/21</u> DESIGN ENGINEER OF RECORD: MTP

> \$\$\$\$USERNAME\$\$\$\$

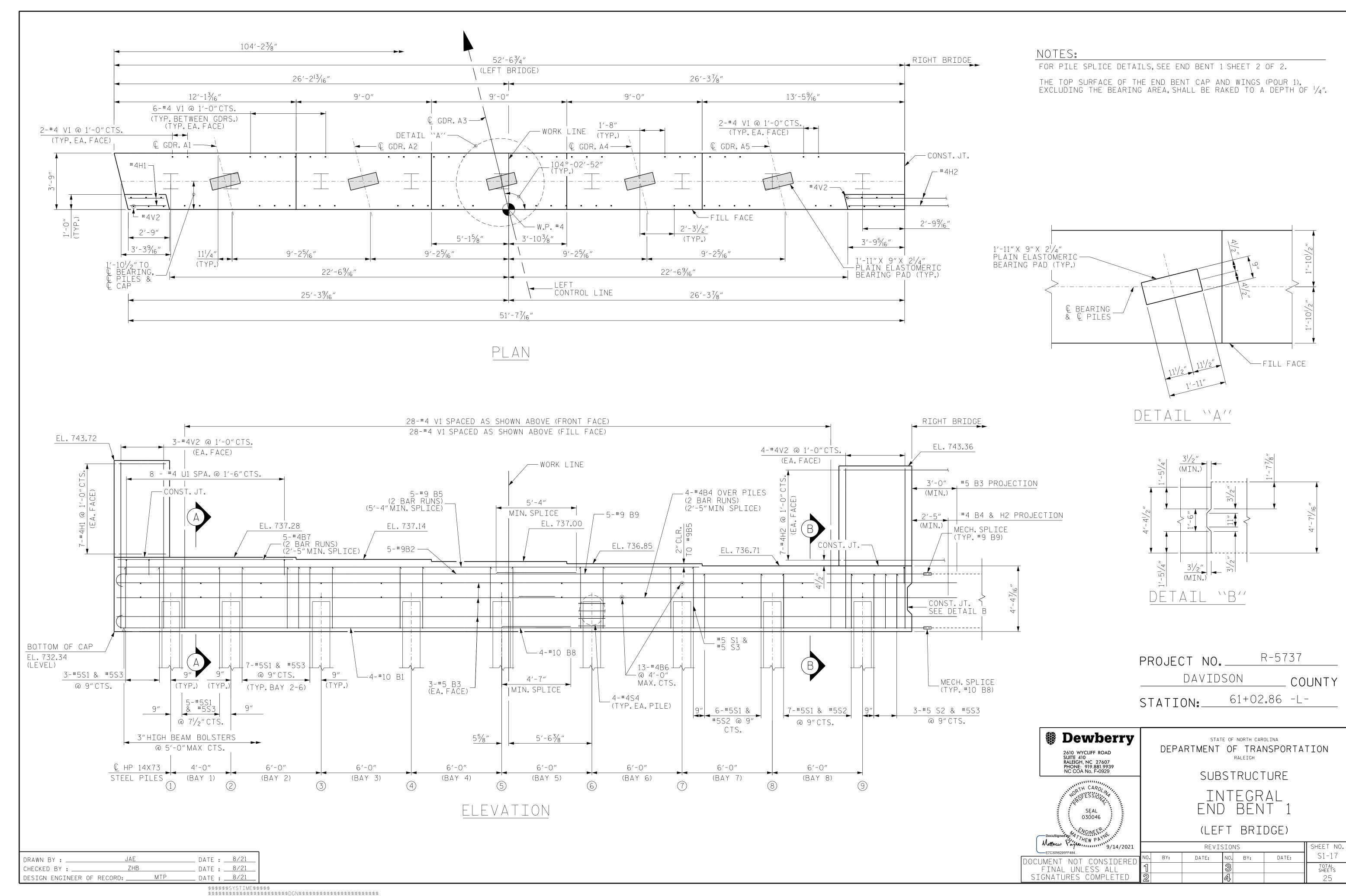
R-5737 PROJECT NO.__ DAVIDSON COUNTY 61+02.86 -L-STATION:_

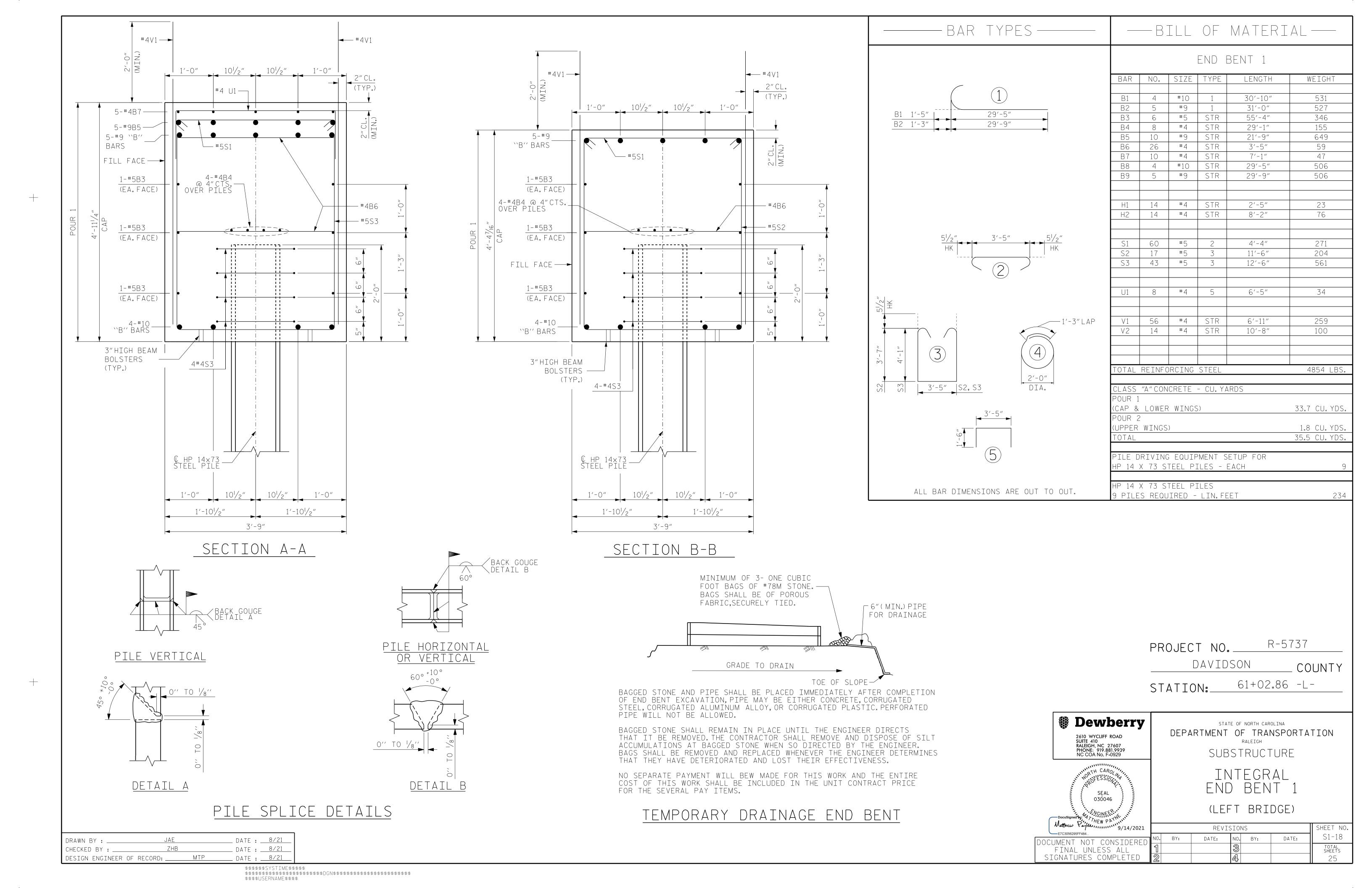


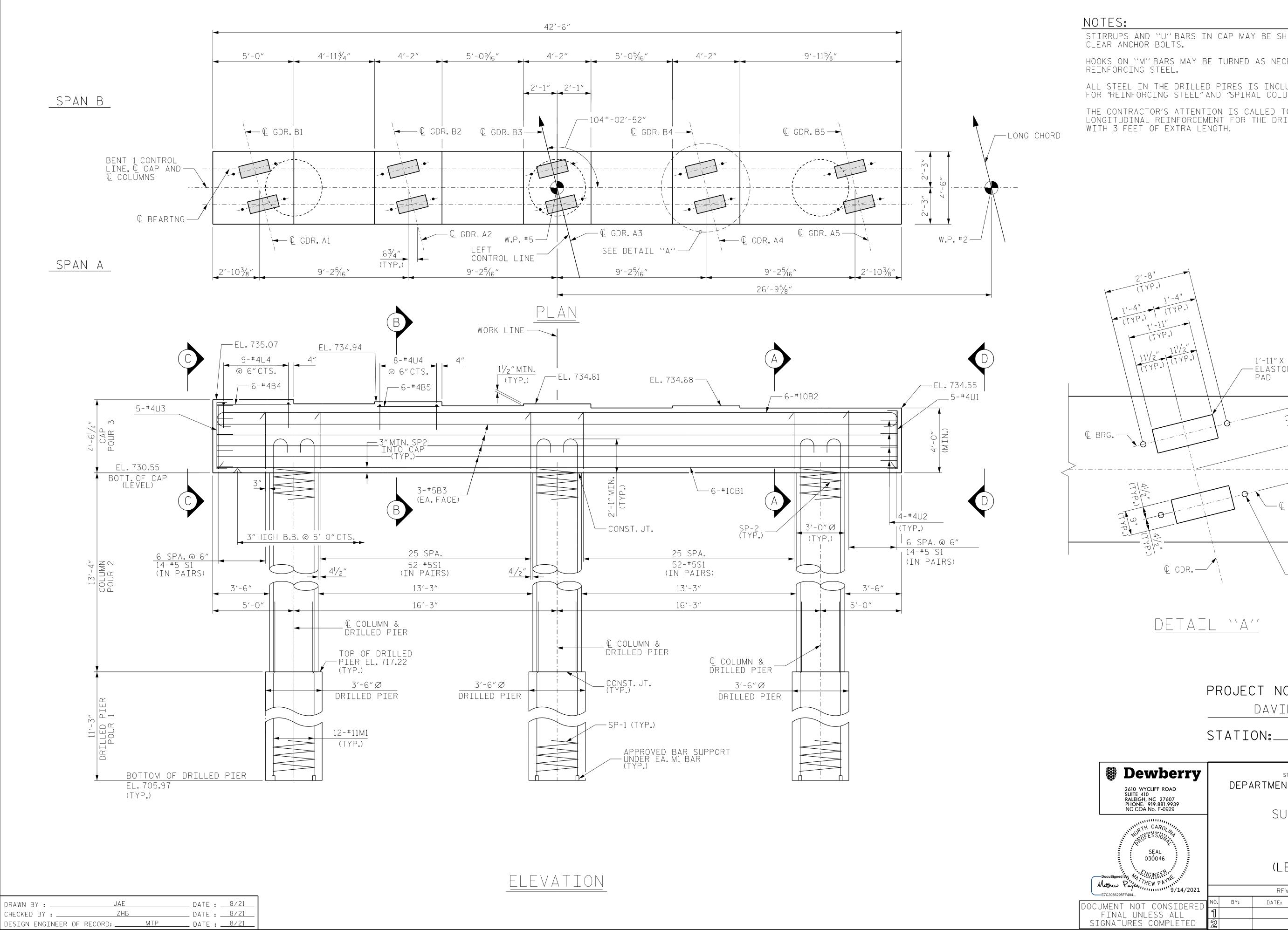
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

SUPERSTRUCTURE BILL OF MATERIAL (LEFT BRIDGE)

SHEET NO REVISIONS S1-16 DATE: DATE: BY: NO. BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS





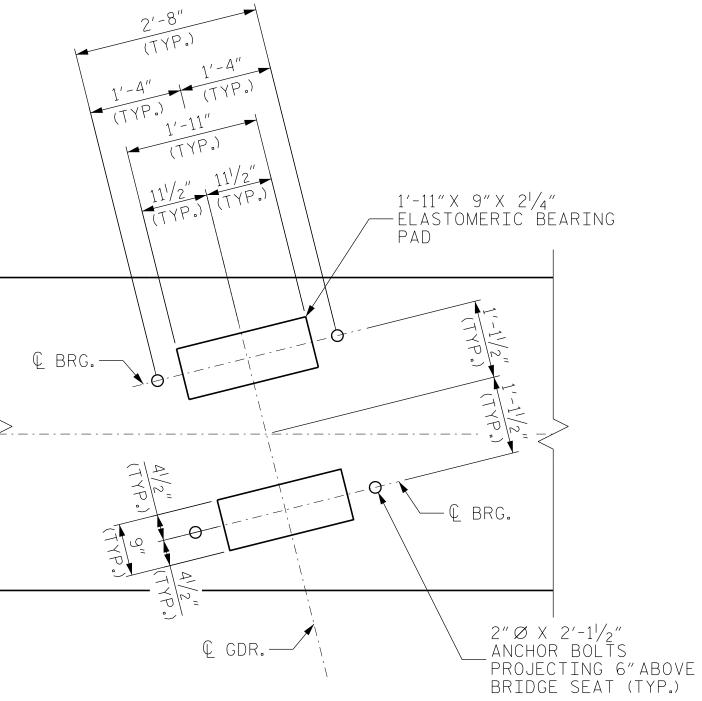


STIRRUPS AND "'U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO

HOOKS ON "M" BARS MAY BE TURNED AS NECESSARY FOR PLACING

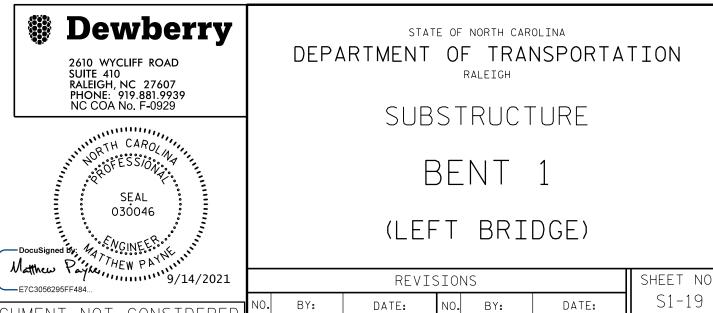
ALL STEEL IN THE DRILLED PIRES IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL".

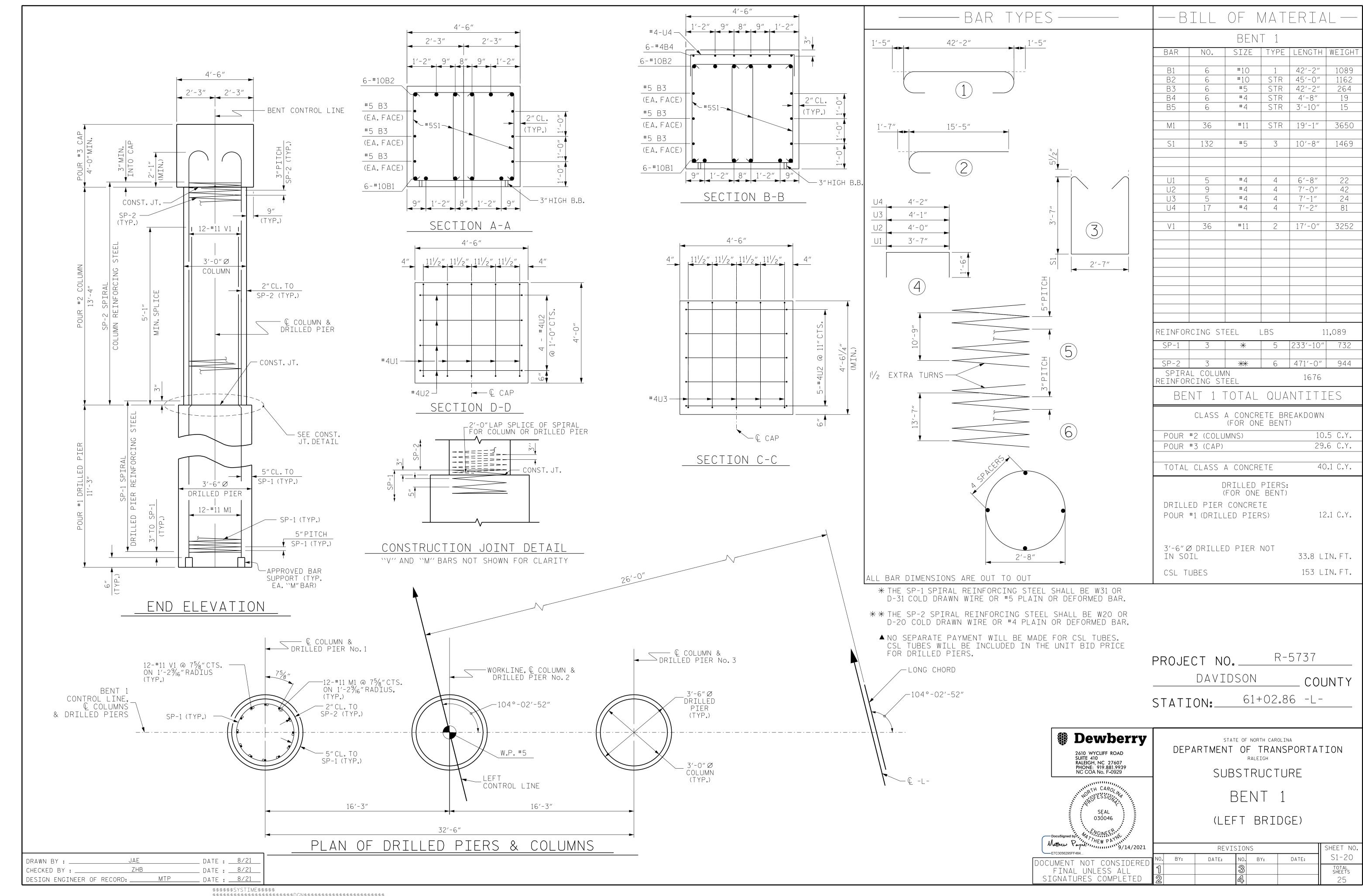
THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS ARE DETAILED

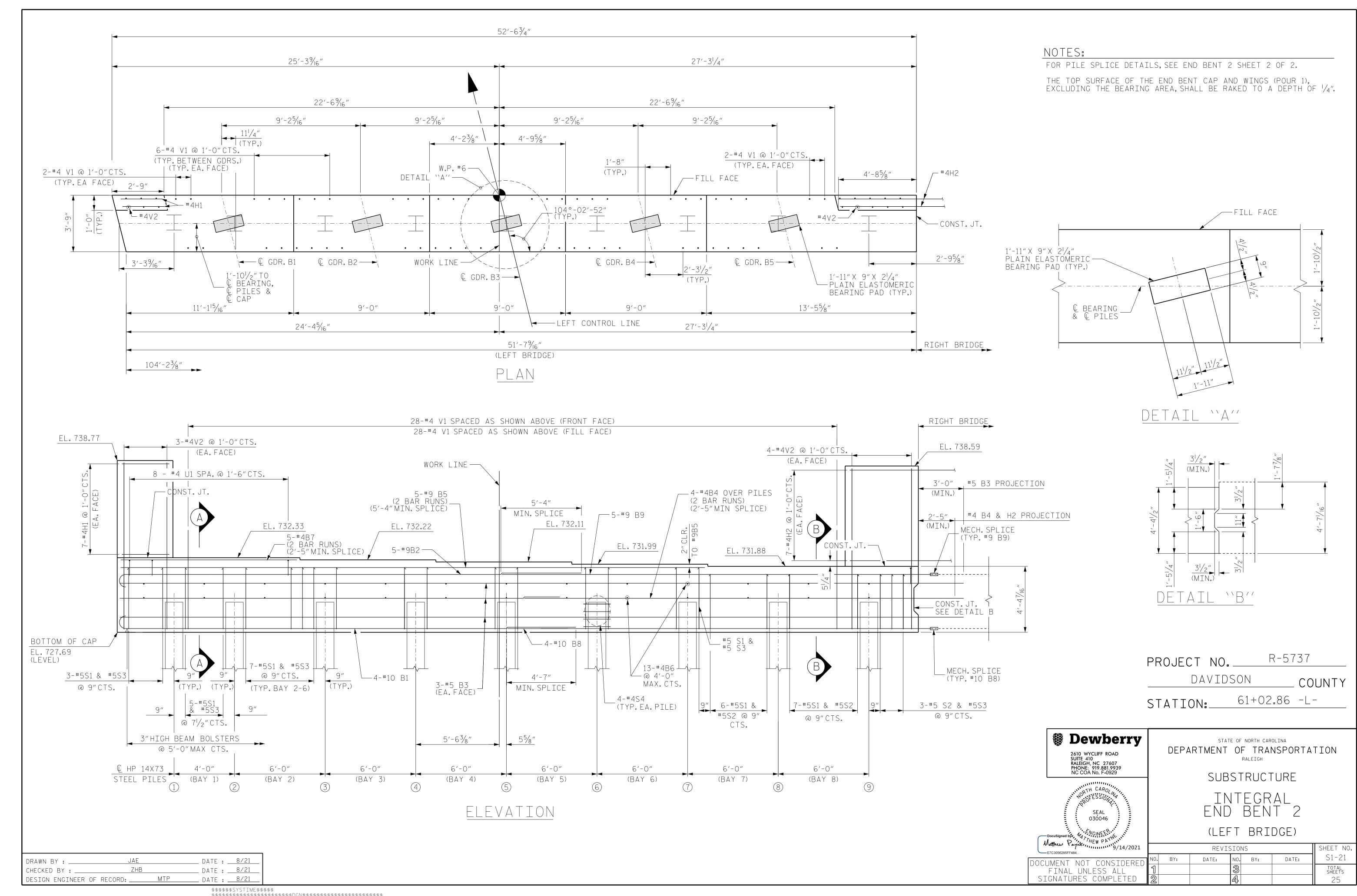


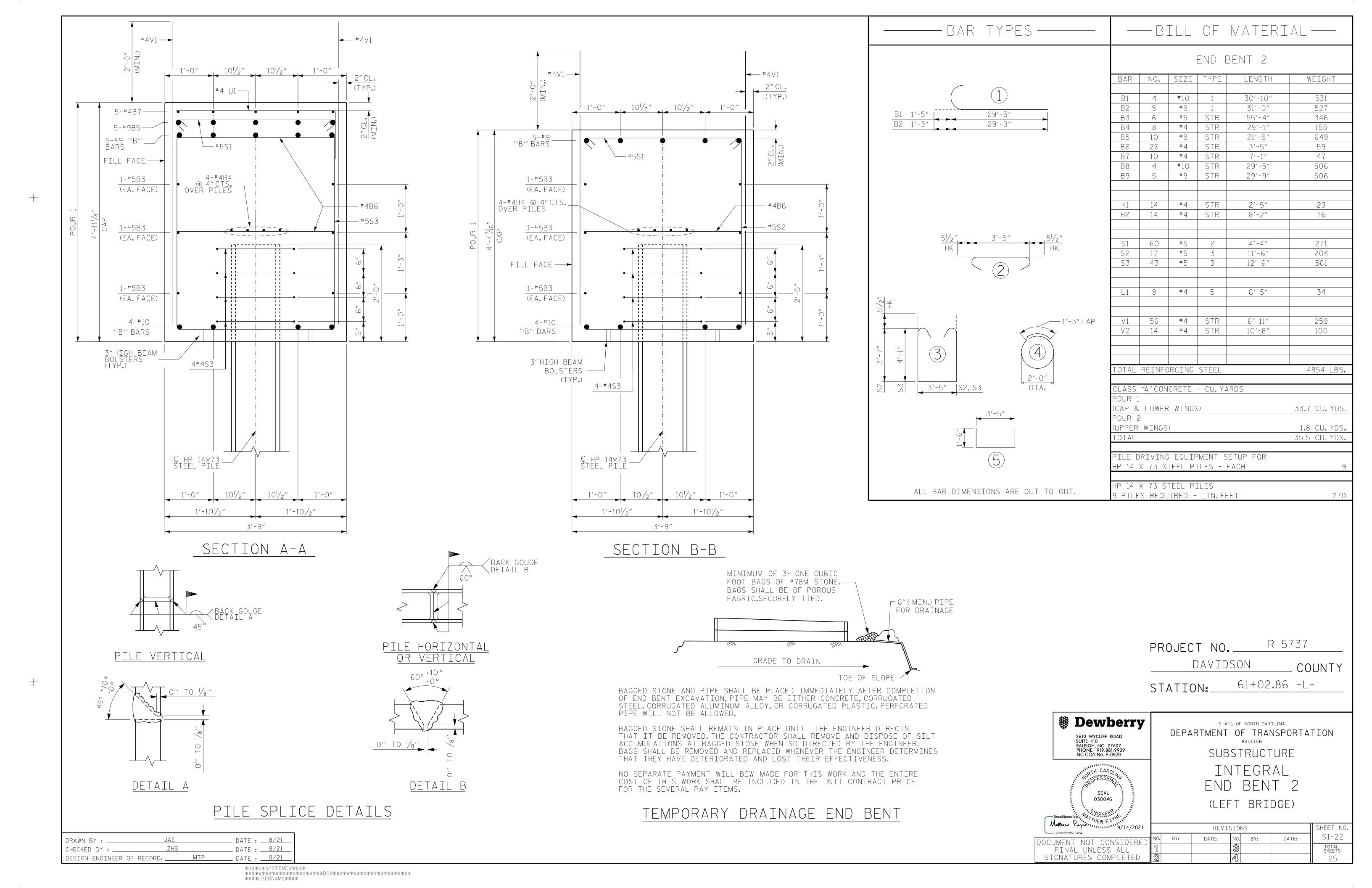
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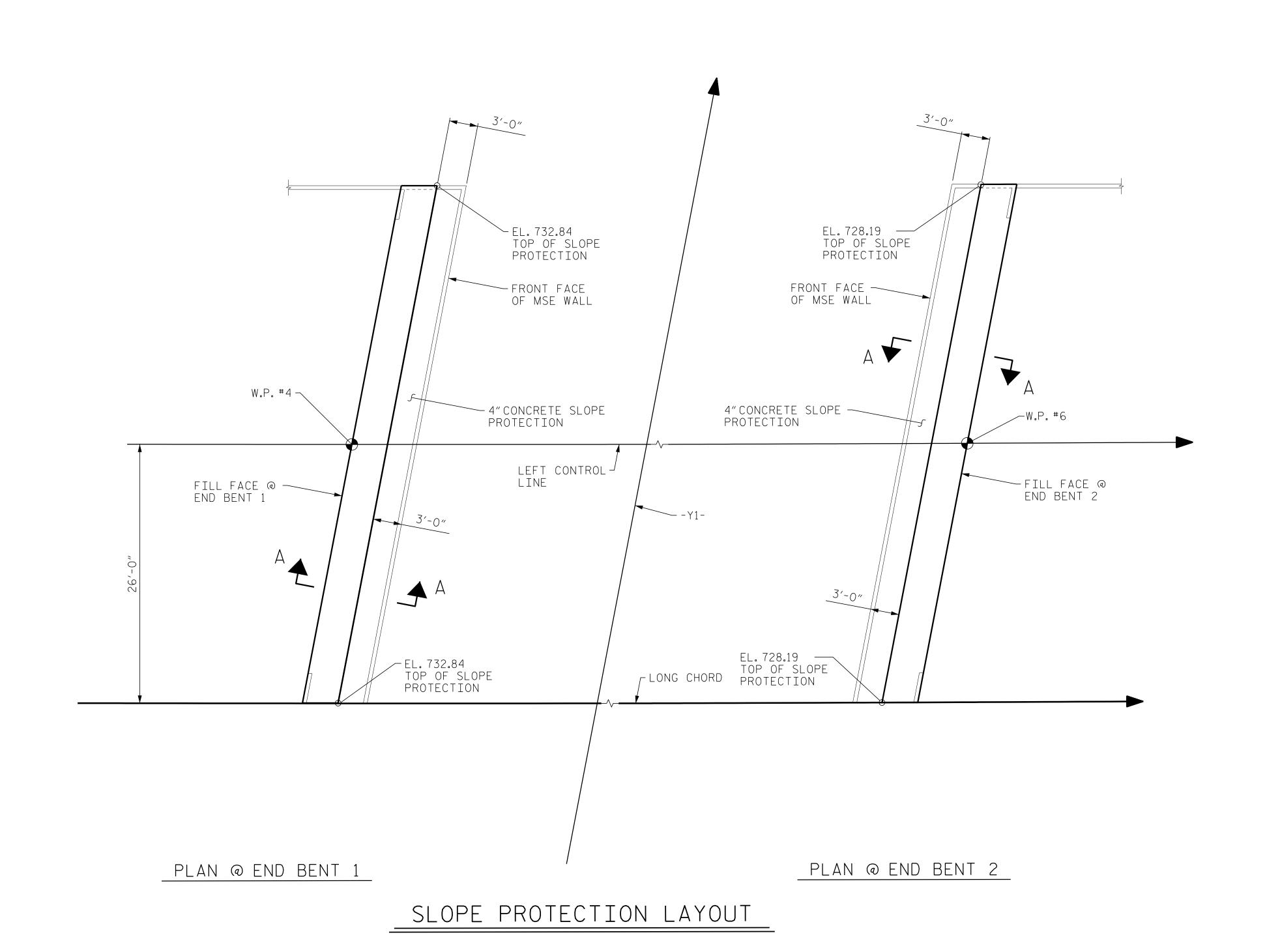
TOTAL SHEETS

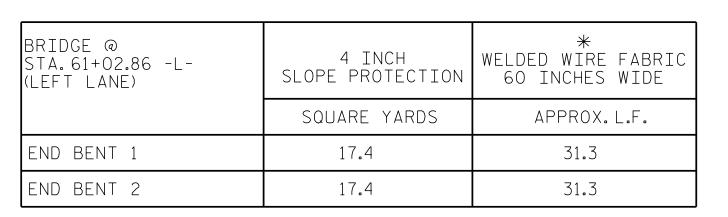




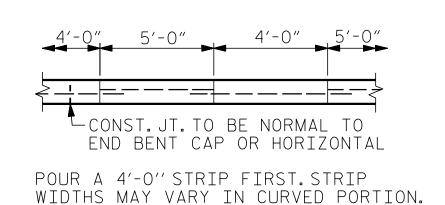




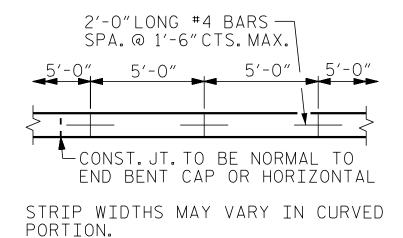




*QUANTITY SHOWN IS BASED ON 5'POURS.



OPTIONAL POURING DETAIL

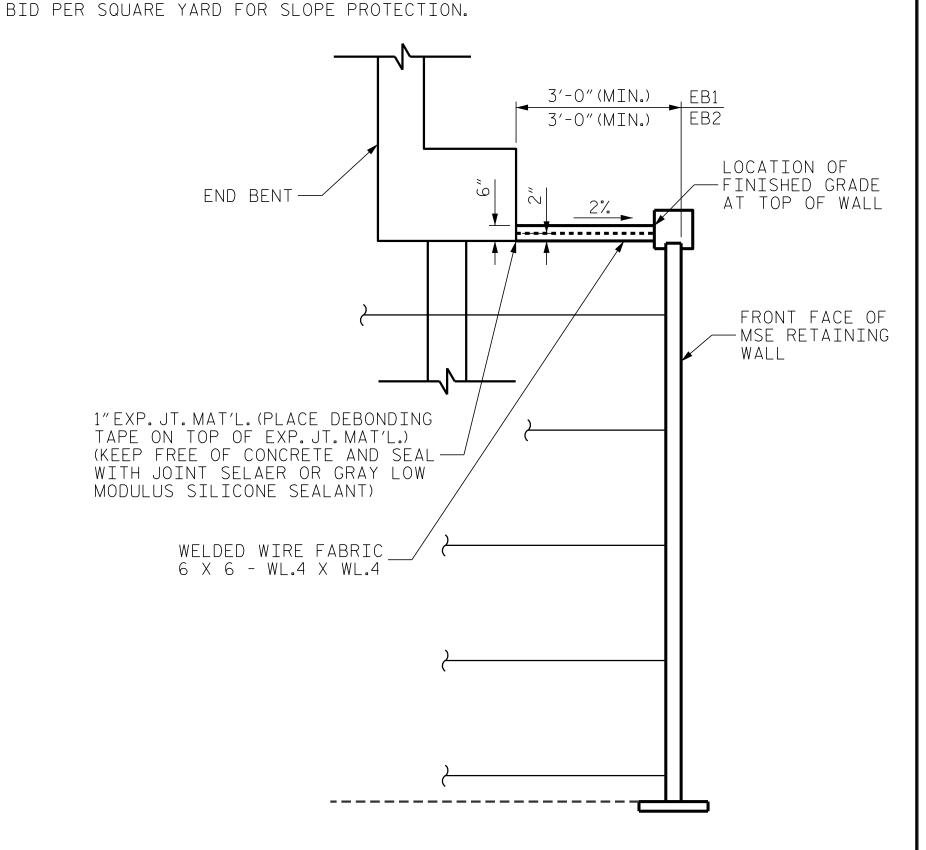


POURING DETAIL

NOTES:

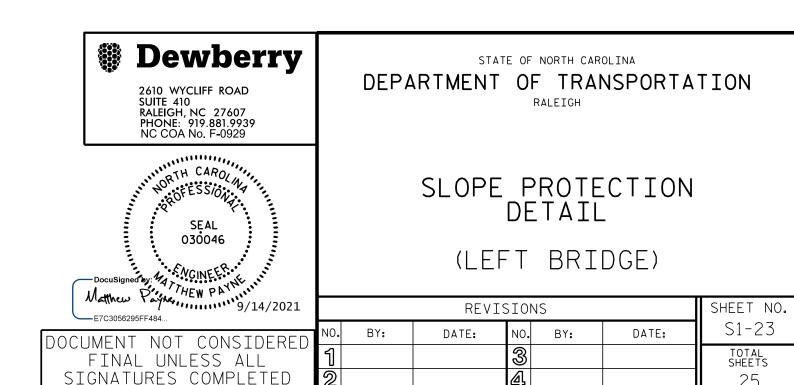
SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS. STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS. FOR BERM WIDTH, SEE GENERAL DRAWING.

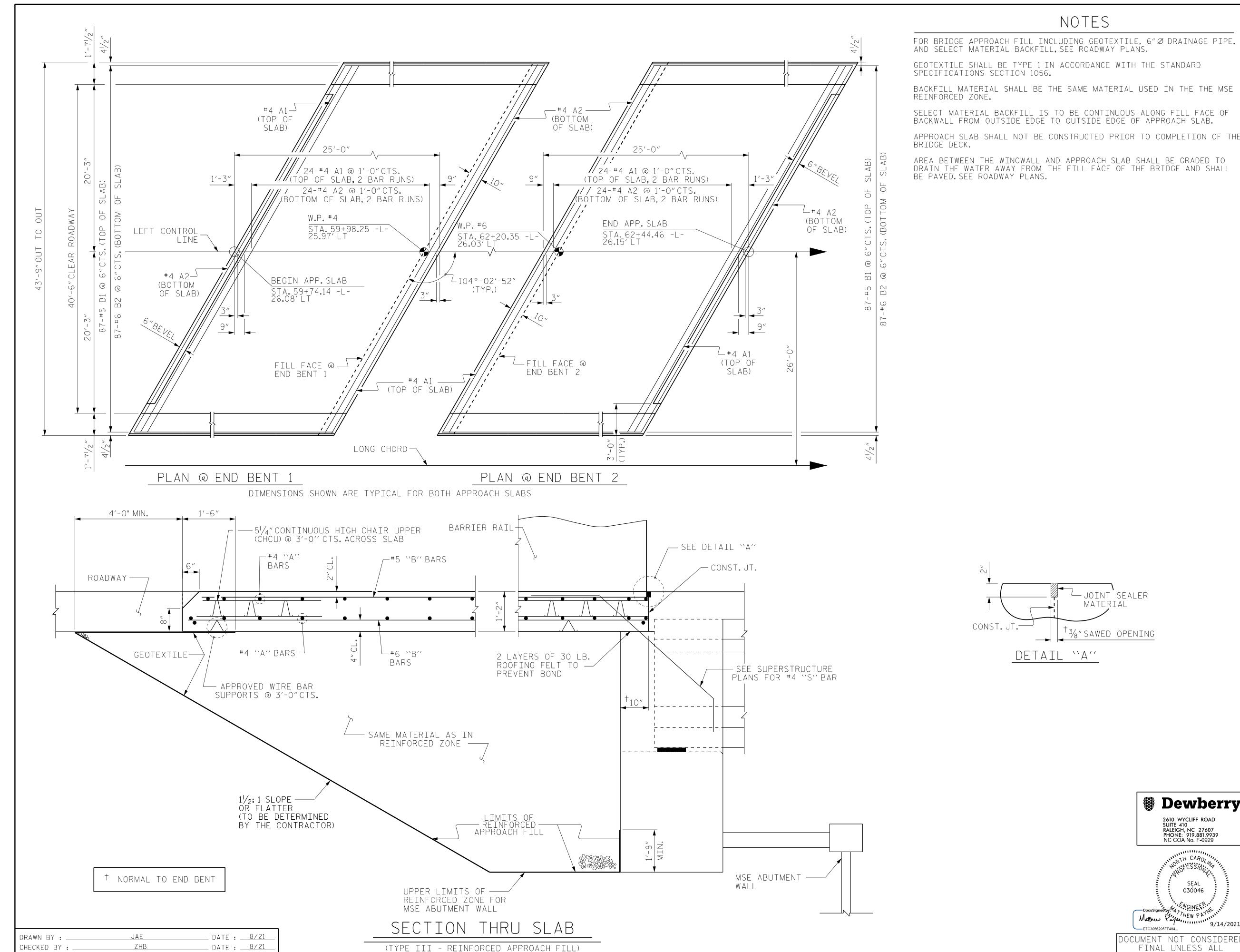
SLOPE PROTECTION SHALL CONSIST OF 4"POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - WL4.60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5'STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-O"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5'STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE



PROJECT NO. _____R-5737 _____DAVIDSON COUNTY STATION: ____61+02.86 -L-

SECTION A-A





\$\$\$\$\$\$SYSTIME\$\$\$\$ \$\$\$\$USERNAME\$\$\$\$

_ DATE : <u>8/21</u>

DESIGN ENGINEER OF RECORD: MTP

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE

	BII	_L O	F MA	BILL OF MATERIAL									
FO	R O		APPRO REQ	OACH S 'D)	LAB								
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT								
* ∆1	52	#4	STR	23'-4"	811								
Α2	54	#4	STR	23'-2"	836								
* B1	87	#5	STR	24'-2"	2193								
В2	87	#6	STR	24'-8"	3223								
REINF	ORCI	NG STE	EEL *>	k LBS.	4059								
₩EPO . REI			TEEL	** LBS.	3004.								

* * QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED. SEE SHEET 2 OF 2.

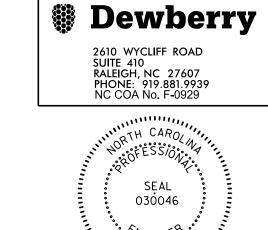
CLASS AA CONCRETE * * C.Y. 47.1

ALL BAR DIMENSIONS ARE OUT TO OUT

SPLICE LENGTHS									
BAR SIZE	EPOXY COATED	UNCOATED							
#4	1'-11"	1'-7"							
#5	2'-5"	2'-0"							
#6	3′-7″	2′-5″							

R-5737 PROJECT NO._ DAVIDSON COUNTY 61+02.86 -L-STATION:

SHEET 1 OF 2

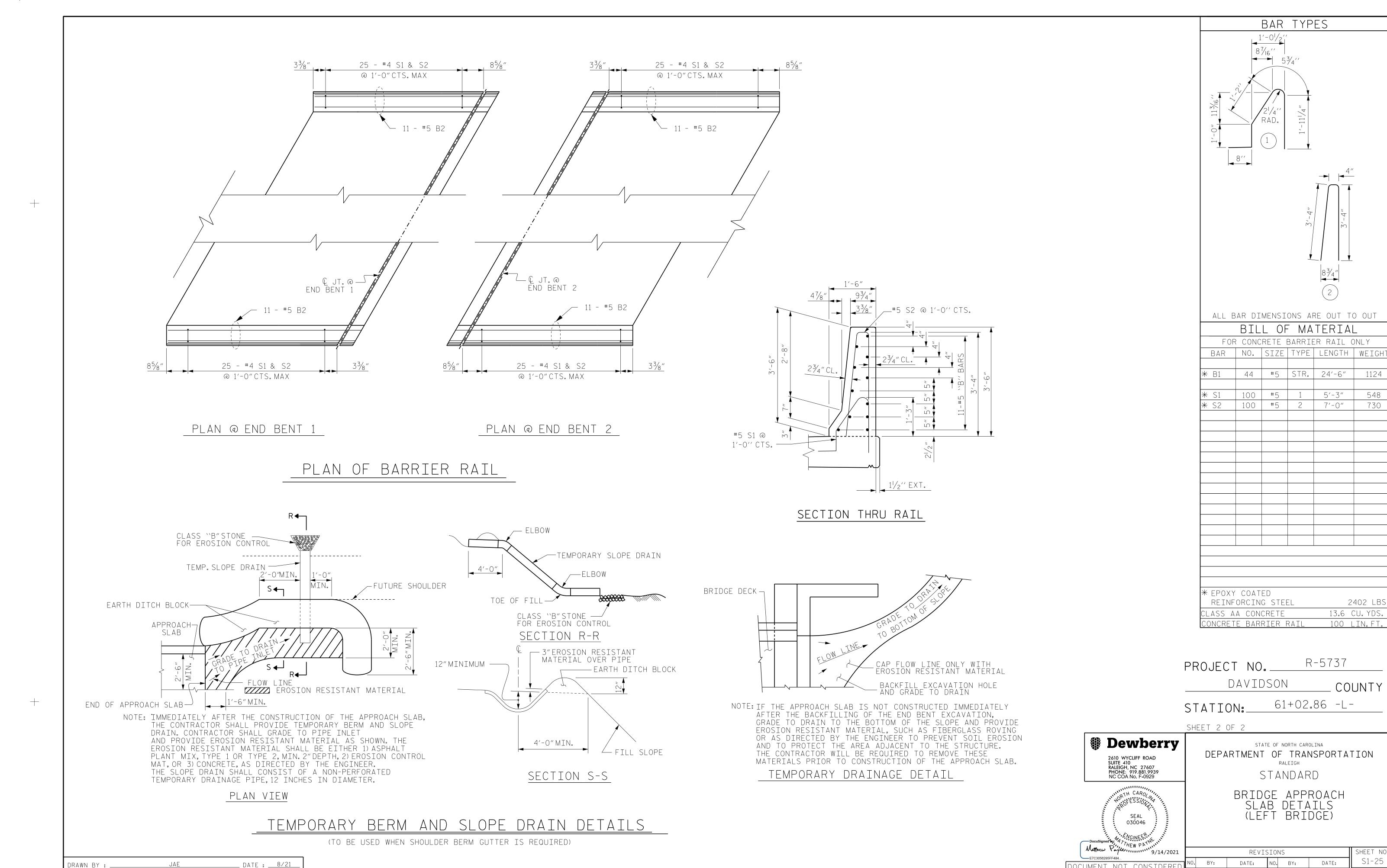


STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT

SHEET NO REVISIONS 9/14/2021 S1-24 DATE: DATE: BY: NO. BY: OCUMENT NOT CONSIDERED FINAL UNLESS ALL TOTAL SHEETS SIGNATURES COMPLETED



_ DATE : <u>8/21</u>

_ DATE : <u>8/21</u>

\$\$\$\$\$\$SYSTIME\$\$\$\$

\$\$\$\$USERNAME\$\$\$\$

ZHB

DESIGN ENGINEER OF RECORD: MTP

CHECKED BY : _

(SHT 1b)

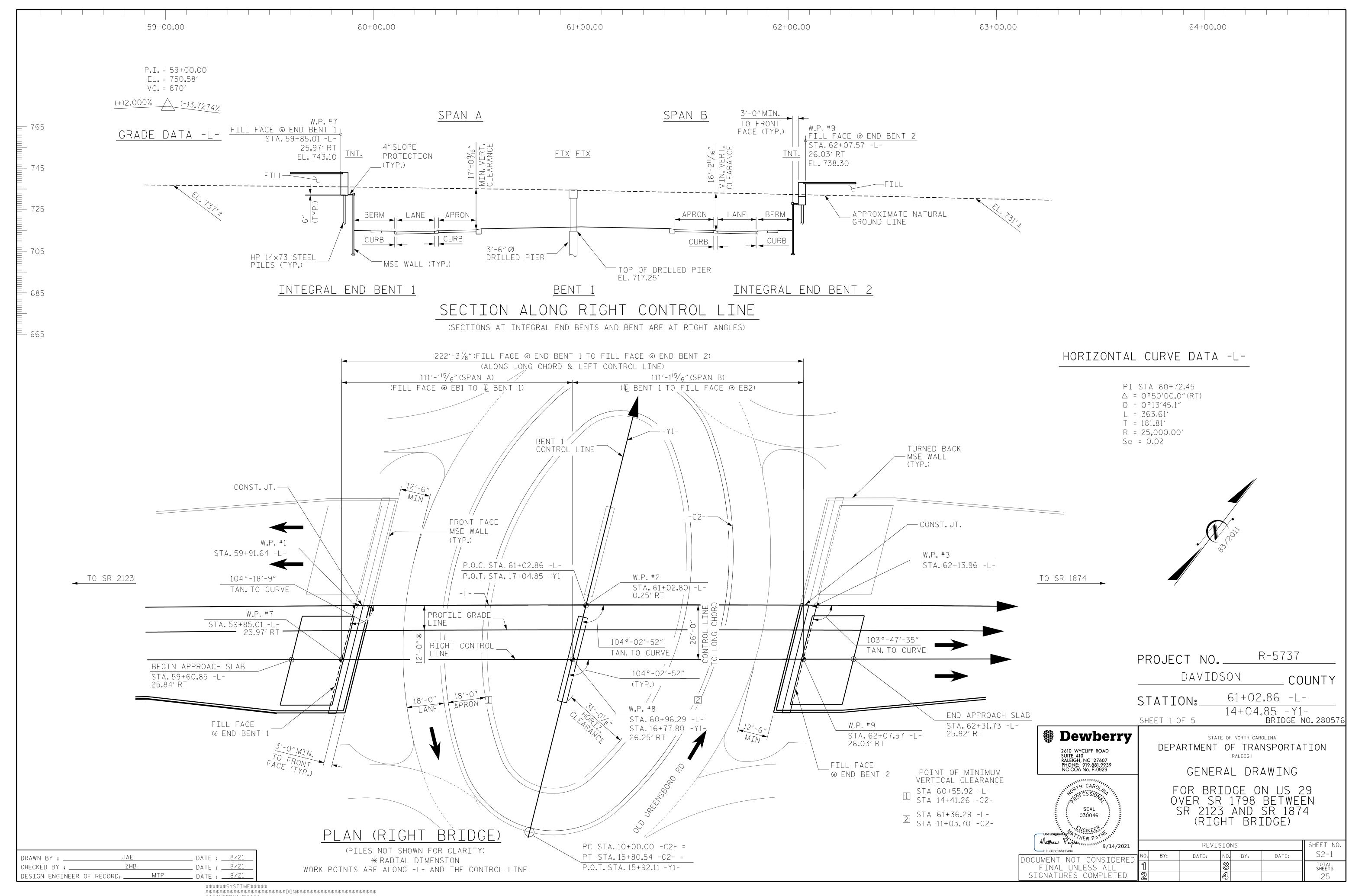
S1-25

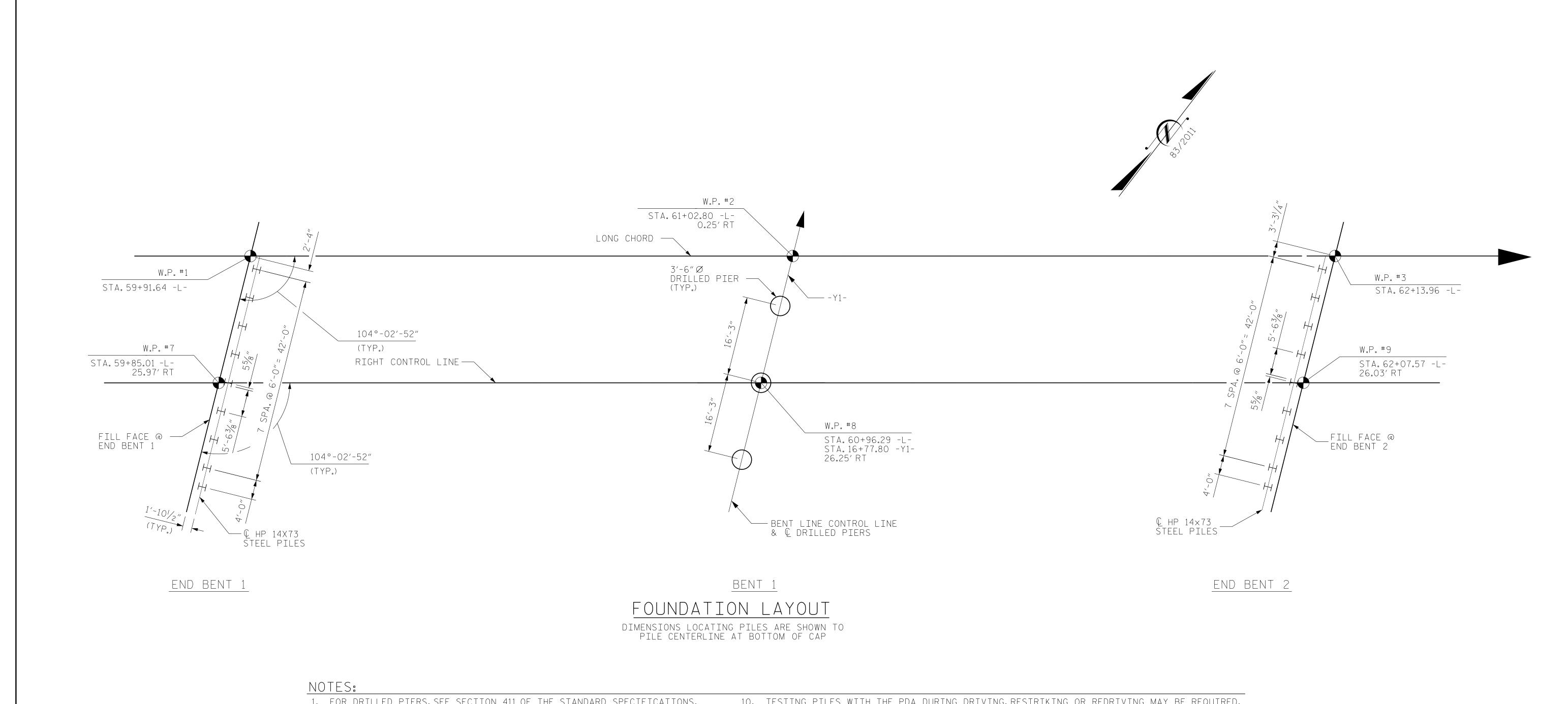
TOTAL SHEETS

OCUMENT NOT CONSIDERED

FINAL UNLESS ALL

SIGNATURES COMPLETED



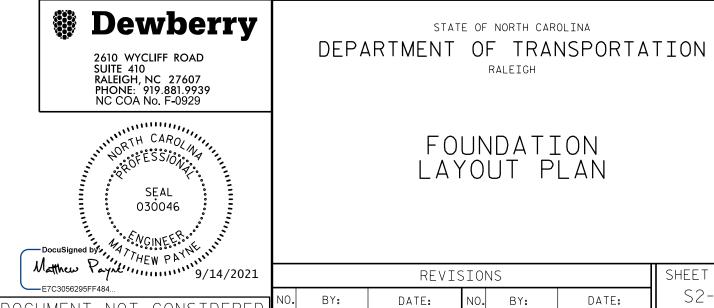


- 1. FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- 2. DRILLED PIERS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 538 TONS PER PIER.
- 3. INSTALL DRILLED PIERS AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 706 FT AND PENETRATION OF AT LEAST 8 FT INTO ROCK AS DEFINED BY ARTICLE 411-1 OF THE STANDARD SPECIFICATIONS.
- 4. CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- 5. FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- 6. PILES AT END BENT NO.1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 109 TONS PER PILE.
- 7. DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 185 TONS PER PILE.
- 8. DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 284 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR DOWNDRAG.
- 9. IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 60,000-80,000 FT-LBS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT NO. 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

- 10. TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED.
 THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING. SEE SECTION 450 OF THE STANDARD SPECIFICATIONS (AND FOR PILE DRIVING CRITERIA, SEE PILE DRIVING CRITERIA PROVISION).
- 11. DRILLED-IN PILES ARE REQUIRED FOR INTEGRAL END BENT NO.1. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 709 FT. FILL THE BOTTOM 3 FT OF HOLES FOR PILE EXCAVATION WITH CONCRETE AND THE REST OF HOLES WITH CLASS II OR III SELECT MATERIAL THAT MEETS SECTION 1016 OF THE STANDARD SPECIFICATIONS. FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

R-5737 PROJECT NO. DAVIDSON COUNTY 61+02.86 -L-STATION:

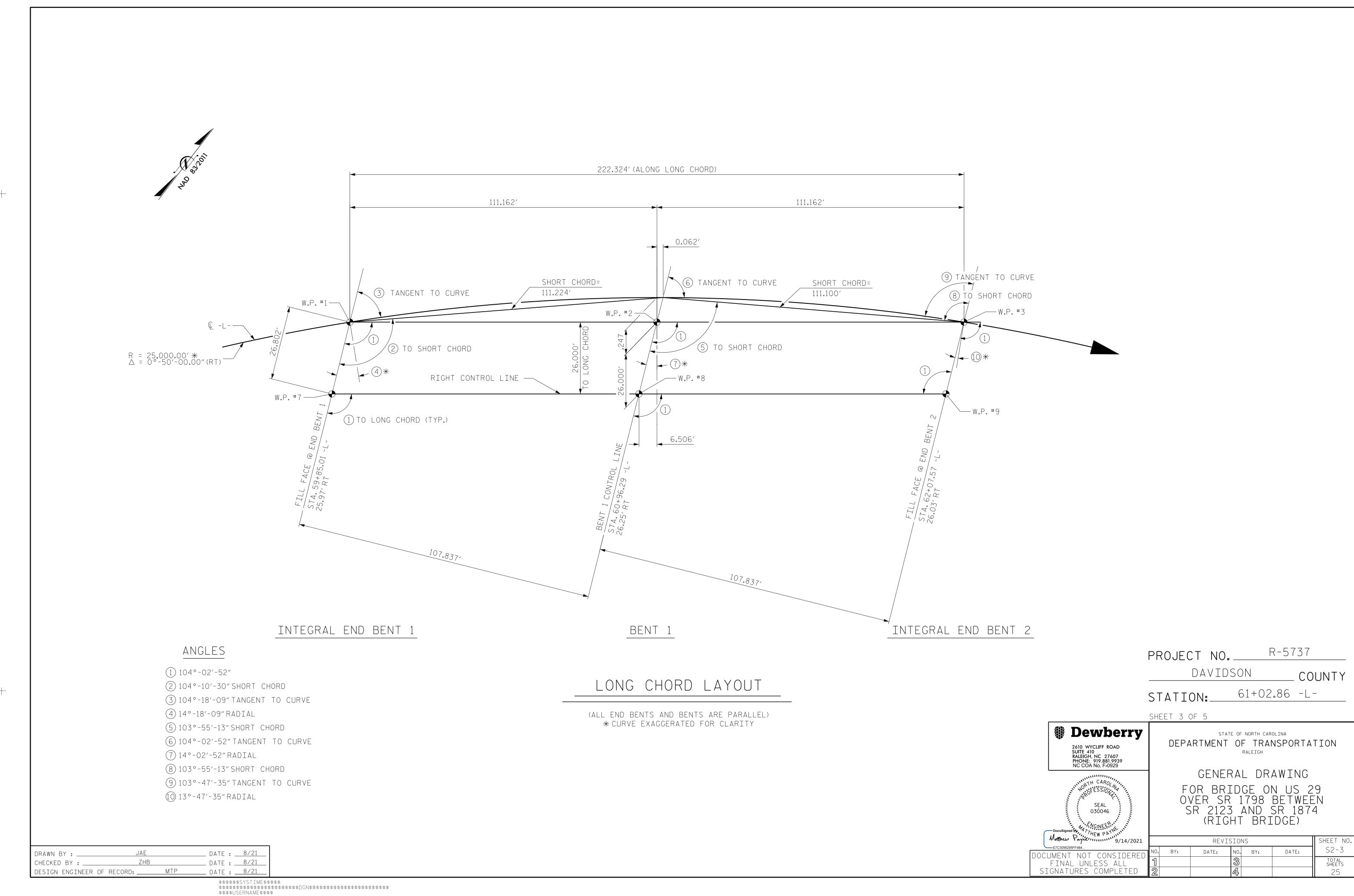
SHEET 2 OF 5

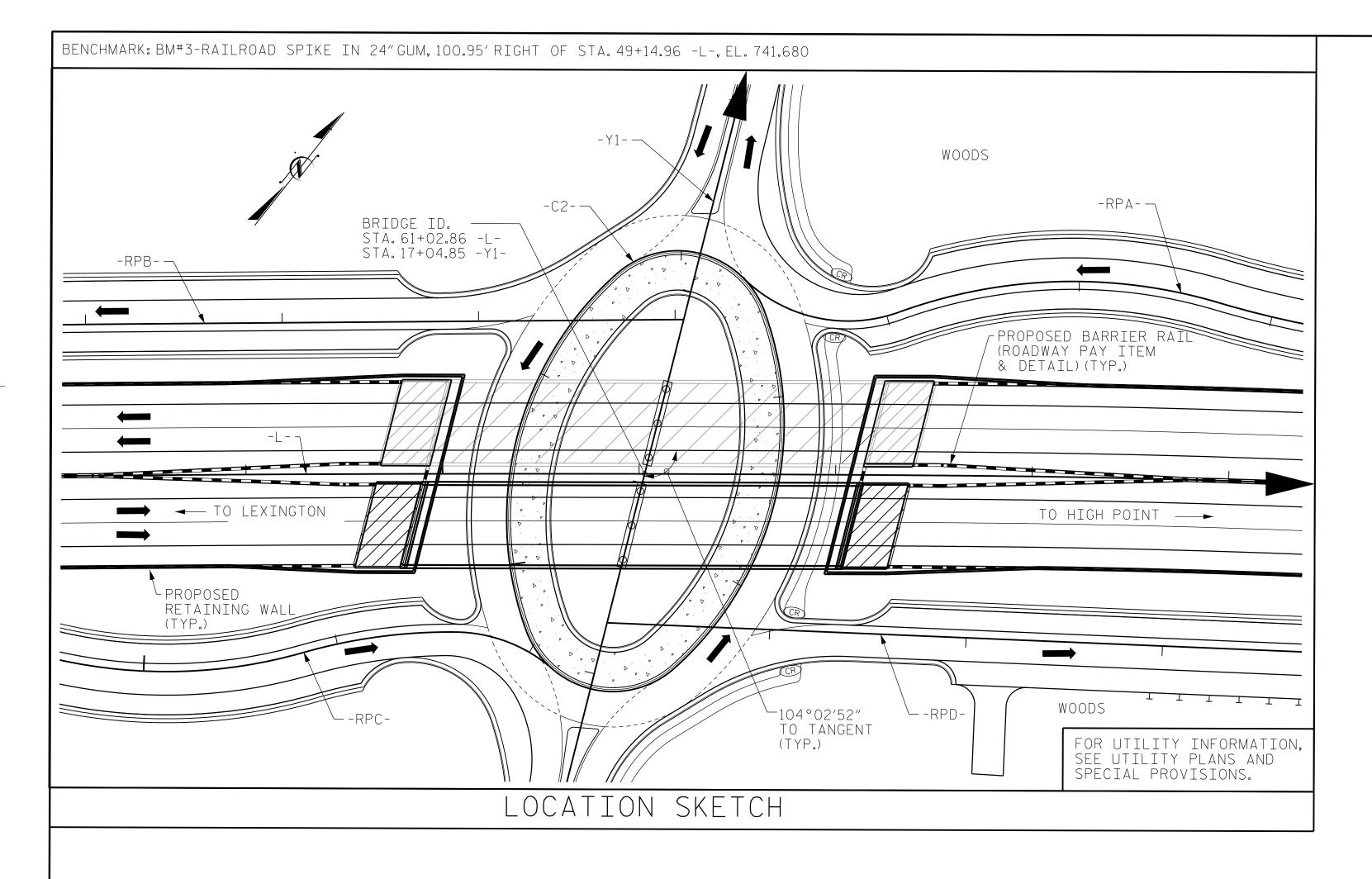


SHEET NO S2-2 DATE: DATE: BY: DOCUMENT NOT CONSIDERED TOTAL SHEETS FINAL UNLESS ALL SIGNATURES COMPLETED

JAE _ DATE : <u>8/21</u> DRAWN BY : ___ _ DATE : <u>8/21</u> ZHB CHECKED BY : _ DESIGN ENGINEER OF RECORD: MTP _ DATE : <u>8/21</u>

> \$\$\$\$\$\$SYSTIME\$\$\$\$ \$\$\$\$USERNAME\$\$\$\$





TOTAL BILL OF MATERIAL														
	PILE EXCAVATION NOT IN SOIL	3'-6"DIA. DRILLED PIER NOT IN SOIL	PDA TESTING	CSL TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL				
	LIN.FT.	LIN.FT.	EA. EA. SQ.FT.		SQ.FT.	SQ.FT.	CU. YDS.	LUMP SUM	LBS.	LBS.				
SUPERSTRUCTURE	-	-	-	-	9652	10,070	-	LUMP SUM						
END BENT NO.1	48	ı	-	-	-	-	32.9	-	4,353					
BENT NO.1	_	- 33.8		-	-	-	39.8	-	11,025	1,643				
END BENT NO.2	-	ı	-	_	-	-	32.9	-	4,353					
TOTAL	48	33.8	1	1	9652	10,070	105.6	LUMP SUM	19,731	1,643				

			TOTAL	OTAL BILL OF MATERIAL ————————————————————————————————————												
	MODIFIED 63" PRESTRESSED CONCRETE		PILE DRIVING EQUIPMENT SETUP FOR 14×73 STEEL PILES	HP	14 X 73 EEL PILES	CONCRETE BARRIER RAIL	ARCHITECTURAL CONCRETE SURFACE TREATMENT	4"SLOPE PROTECTION	ELASTOMERIC BEARINGS							
	NO.	LIN.FT.	EA.	NO.	LIN.FT.	LIN.FT.	SQ.FT.	SQ. YDS.	LUMP SUM							
SUPERSTRUCTURE	10	1097.71	-	_	-	641.2	623.2	-	LUMP SUM							
END BENT NO.1	-	_	9	9	234	-	-	17	-							
BENT NO.1	-	-	-	-	-	-	-	-	-							
END BENT NO.2	-	-	9	9	315	-	-	17	-							
TOTAL	10	1097.71	18	18	549	641.2	623.2	34	LUMP SUM							

_ DATE : <u>8/21</u> CHECKED BY : _____ _ DATE : <u>8/21</u> DESIGN ENGINEER OF RECORD: _____

Dewberry 2610 WYCLIFF ROAD SUITE 410 RALEIGH, NC 27607 PHONE: 919.881.9939 NC COA No. F-0929

GENERAL DRAWING NOTES:

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE ELEVATIONS AND CLEARANCES SHOWN ON THE PLANS AT THE POINTS OF MINIMUM VERTICAL CLEARANCE ARE FROM THE BEST Information available.Prior to beginning bridge construction, verify the elevations on the existing pavement and check THE CLEARANCE. REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHEIVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.

PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OF APPROVED BY THE ENGINEER.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

WORK SHALL NOT BE STARTED ON THIS BRIDGE (OR SPECIFIC PARTS OF BRIDGE) UNTIL ROADWAY SECTION HAS BEEN EXCAVATED. FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

> R-5737 PROJECT NO.____ DAVIDSON COUNTY

STATION: 61+02.86 -L-

SHEET 4 OF 5

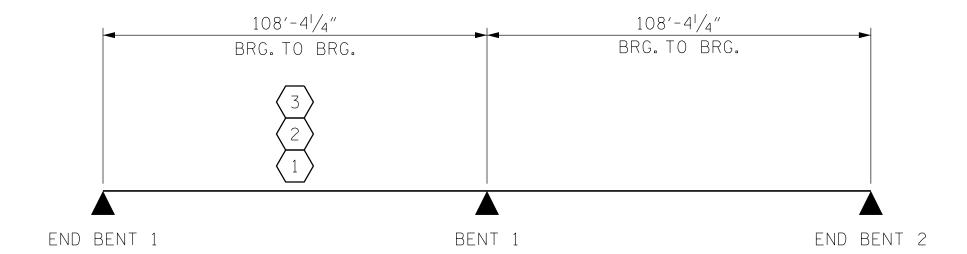
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GENERAL DRAWING

FOR BRIDGE ON US 29 OVER SR 1798 BETWEEN SR 2123 AND SR 1874 (RIGHT BRIDGE)

SHEET NO REVISIONS S2-4 DATE: BY: DATE: BY: OCUMENT NOT CONSIDERED TOTAL SHEETS FINAL UNLESS ALL SIGNATURES COMPLETER

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	NGTH	I LIM	IT ST	ГАТЕ				SE	RVICE	III	LIMI	T STA	ATE	
					ı					MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	COMMENT NUMBER
	DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	1	1.02		1.75	0.815	1.24	А	EL	54.18	0.847	1.41	А	EL	21.67	0.80	0.815	1.02	А	EL	54.18	
DESIGN		HL-93 (OPERATING)	N/A		1.61		1.35	0.815	1.61	А	EL	54.18	0.847	2.03	А	EL	21.67	N/A						
RATING		HS-20 (INVENTORY)	36.000		1.46	52.26	1.75	0.815	1.77	А	EL	54.18	0.847	2.09	А	EL	21.67	0.80	0.815	1.46	А	EL	54.18	
		HS-20 (OPERATING)	36.000		2.30	82.80	1.35	0.815	2.30	А	EL	54.18	0.847	2.77	А	EL	21.67	N/A						
		SH	13.500		3.81	51.44	1.40	0.815	5.79	А	EL	54.18	0.847	7.08	А	I	21.67	0.80	0.815	3.81	А	EL	54.18	•
	Ш	S3C	20.000		2.22	44.40	1.40	0.815	3.36	А	EL	54.18	0.847	4.07	А	I	21.67	0.80	0.815	2.22	А	EL	54.18	
		S3A	22.000		2.10	46.20	1.40	0.815	3.19	А	EL	54.18	0.847	3.58	А	I	21.67	0.80	0.815	2.10	А	EL	54.18	•
		S4A	27.250		1.84	50.14	1.40	0.815	2.79	А	EL	54.18	0.847	3.32	А	I	21.67	0.80	0.815	1.84	А	EL	54.18	
	S) (S	S5A	34.925		1.62	56.58	1.40	0.815	2.46	А	EL	54.18	0.847	3.01	А	I	21.67	0.80	0.815	1.62	А	EL	54.18	
LEGAL	SINGL	S6A	35.550		1.46	51.90	1.40	0.815	2.22	А	EL	54.18	0.847	2.69	А	I	21.67	0.80	0.815	1.46	А	EL	54.18	
LOAD RATING		S7B	39.950		1.32	52.73	1.40	0.815	2.01	А	EL	54.18	0.847	2.48	А	I	21.67	0.80	0.815	1.32	А	EL	54.18	•
IVATING		S7A	42.000	2	1.30	54.60	1.40	0.815	1.97	А	EL	54.18	0.847	2.52	А	I	21.67	0.80	0.815	1.30	А	EL	54.18	•
	4~	T4A	33.000		1.79	59.70	1.40	0.815	2.72	А	EL	54.18	0.847	3.20	А	I	21.67	0.80	0.815	1.79	А	EL	54.18	•
	ACTOR AILER T)	T5B	33.075		1.58	52.26	1.40	0.815	2.39	А	EL	54.18	0.847	3.00	А	I	21.67	0.80	0.815	1.58	А	EL	54.18	•
	1 \(\text{T} \)	Т6А	41.600		1.43	59.49	1.40	0.815	2.18	А	EL	54.18	0.847	2.73	А	I	21.67	0.80	0.815	1.43	А	EL	54.18	•
	TRUCK SEMI-	Т7А	42.000	(3)	1.32	55.44	1.40	0.815	2.00	А	EL	54.18	0.847	2.51	А	I	21.67	0.80	0.815	1.32	А	EL	54.18	
		T7B	42.000		1.38	57.96	1.40	0.815	2.10	А	EL	54.18	0.847	2.38	А	I	21.67	0.80	0.815	1.38	А	EL	54.18	



LRFR SUMMARY

LOAD FACTORS:

DESIGN LOAD RATING FACTORS LIMIT STATE YDC YDW

STRENGTH I 1.25 1.50

SERVICE III 1.00 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

1.

2.

3.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

 $\langle 2 \rangle$ DESIGN LOAD RATING (HS-20)

 $\sqrt{3}$ LEGAL LOAD RATING **

* * SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

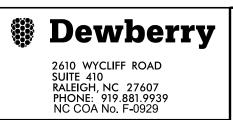
ER - EXTERIOR RIGHTGIRDER

PROJECT NO. R-5737

DAVIDSON COUNTY

STATION: 61+02.86 -L-

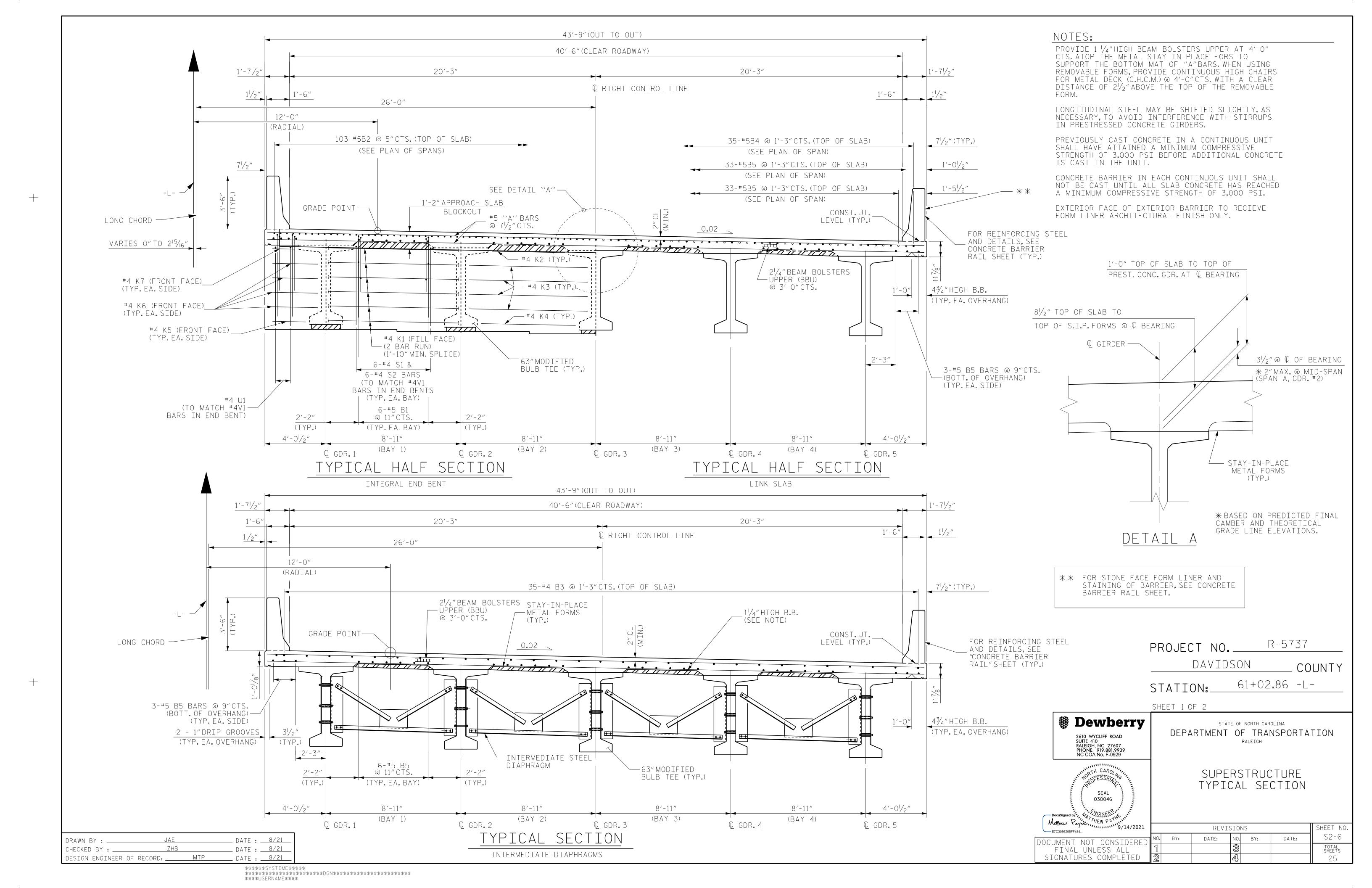
SHEET 5 OF 5

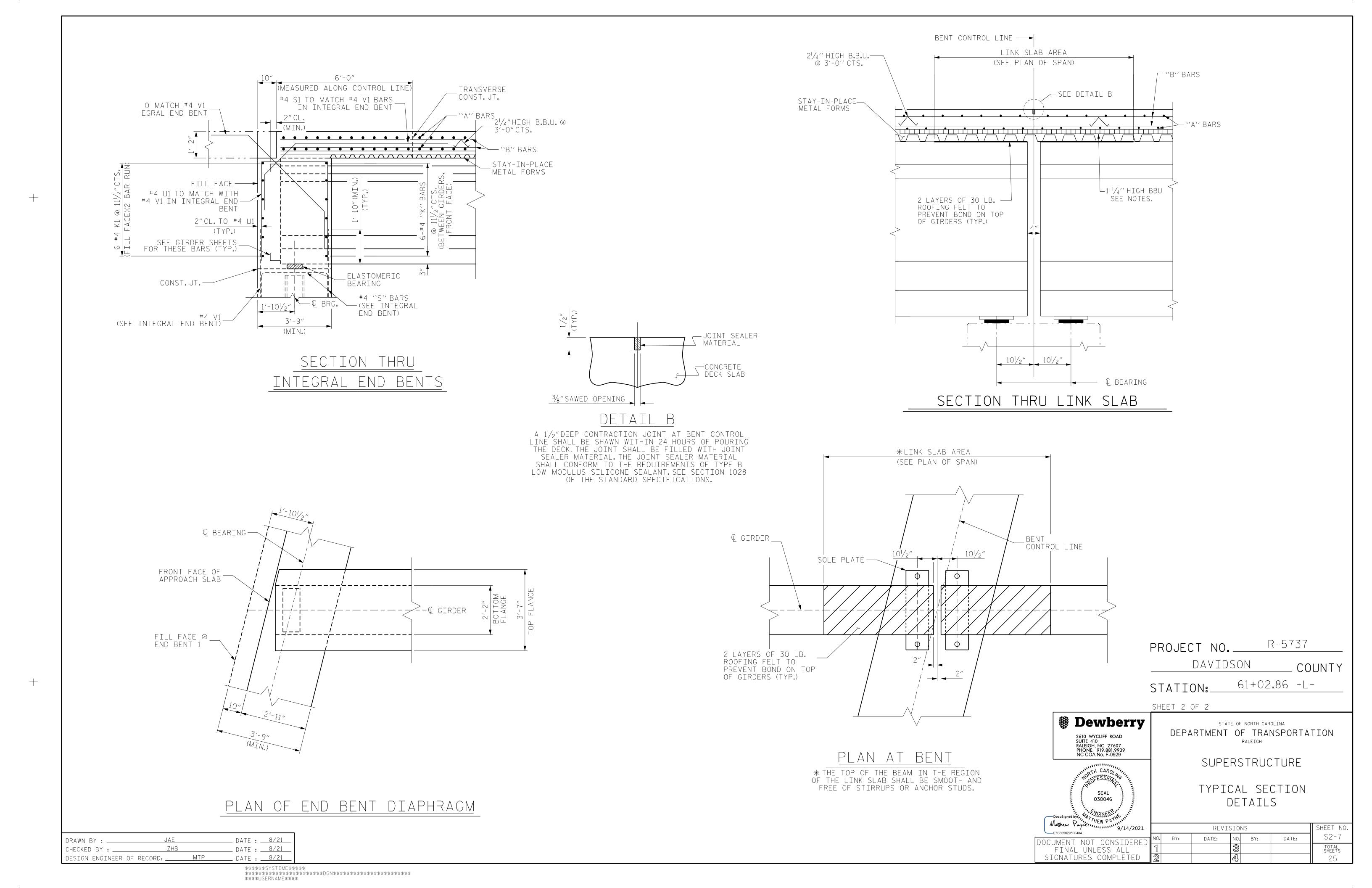


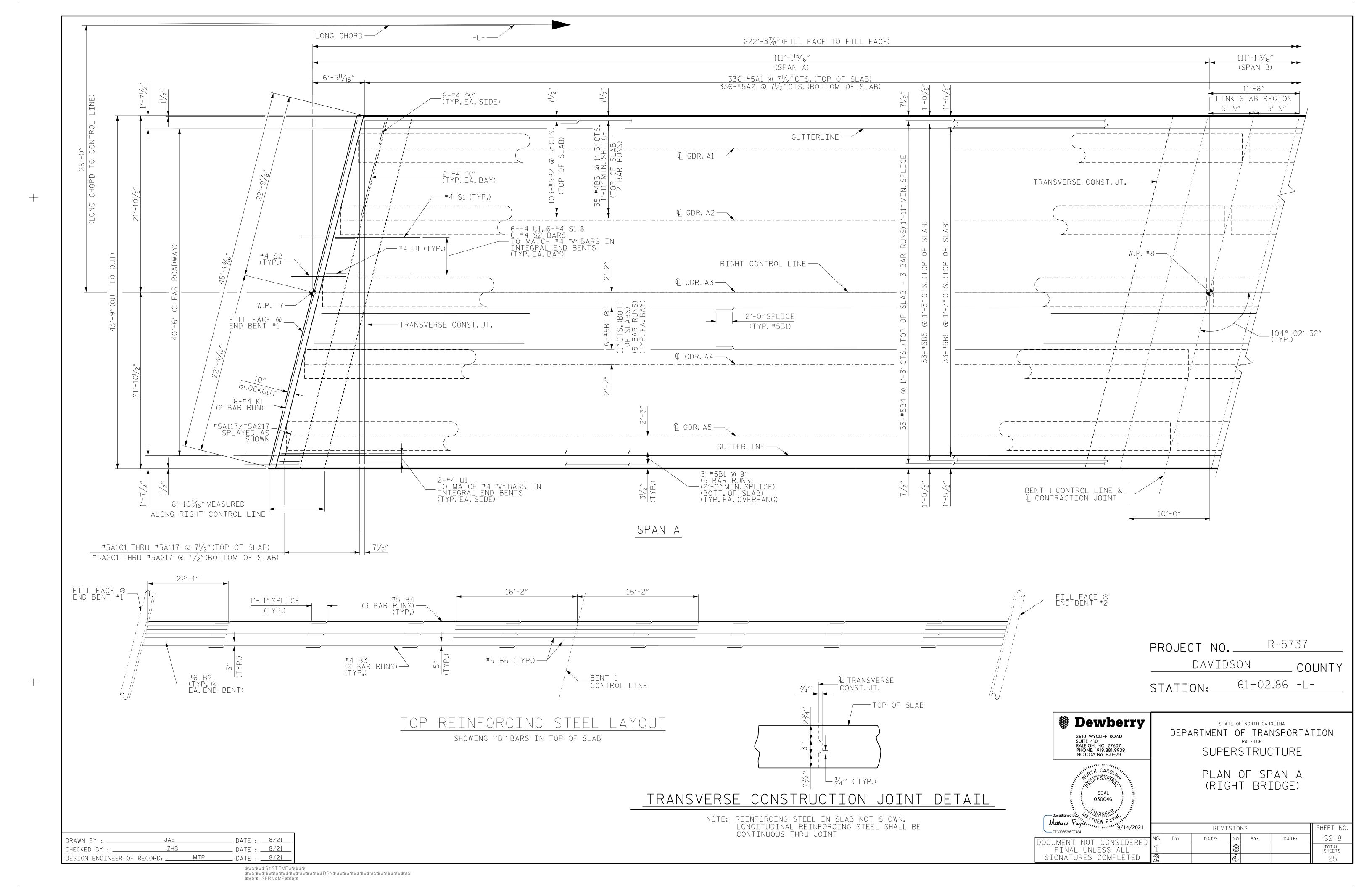
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

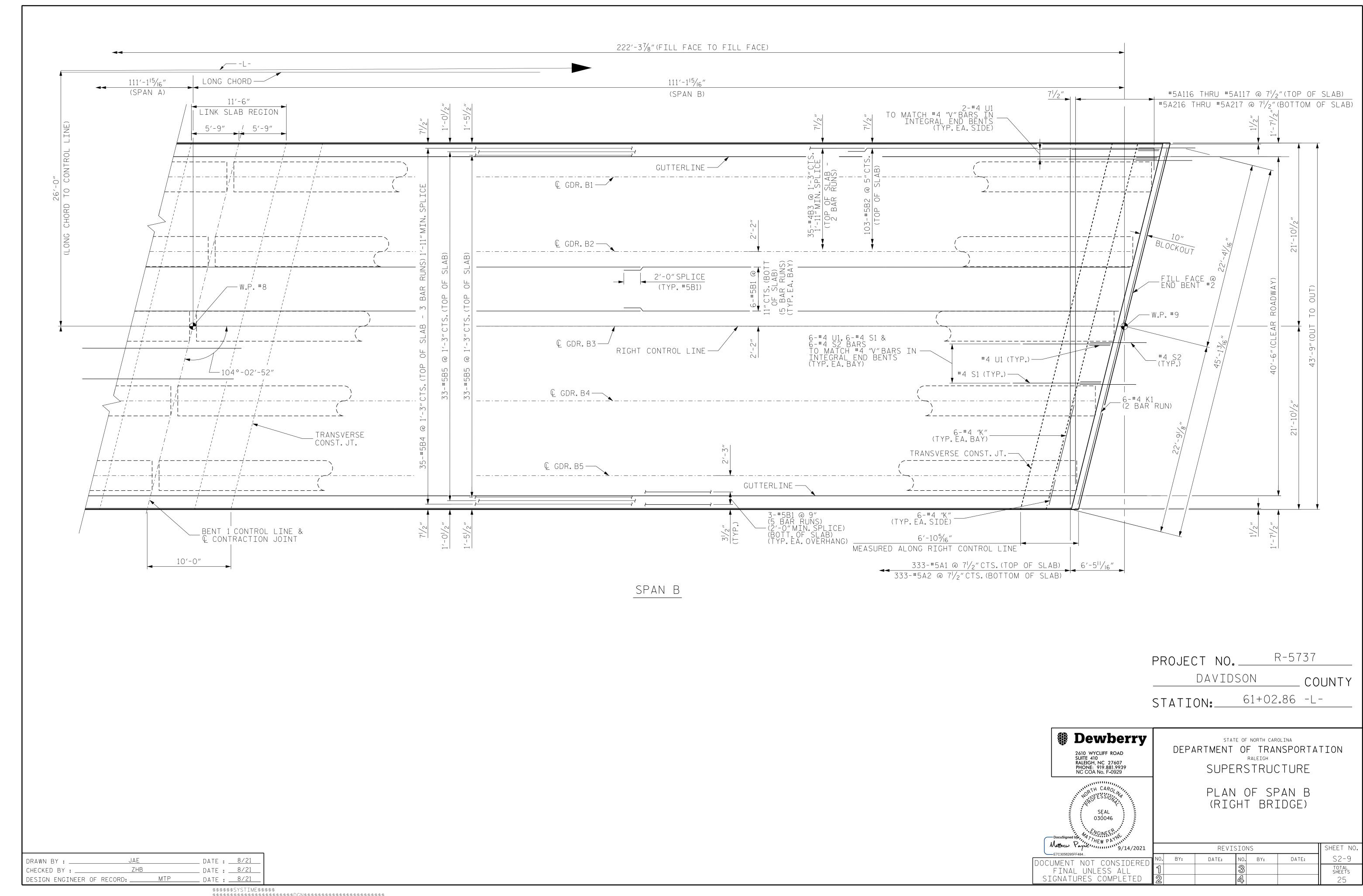
LRFR SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

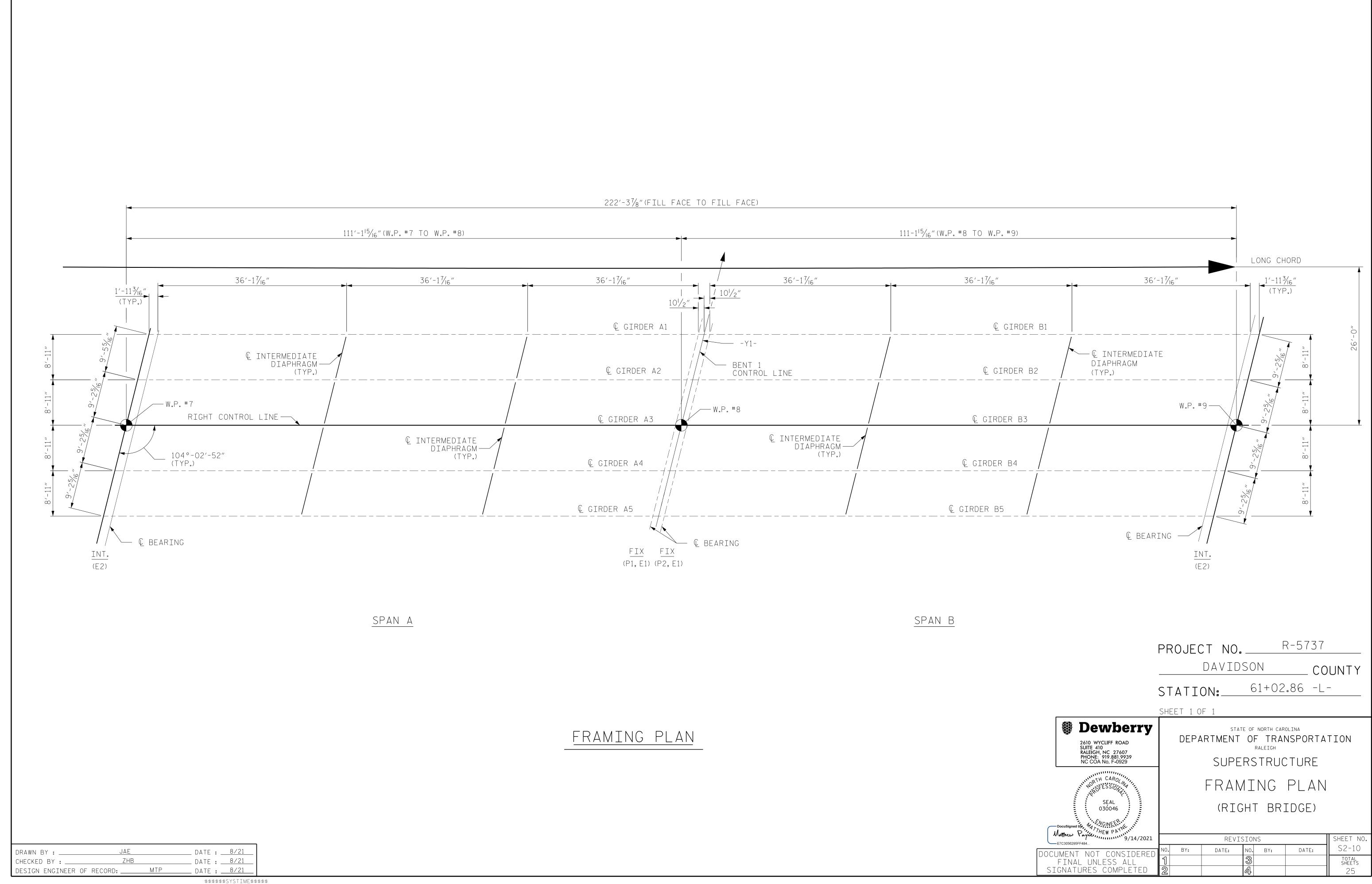
(INTERSTATE TRAFFIC)

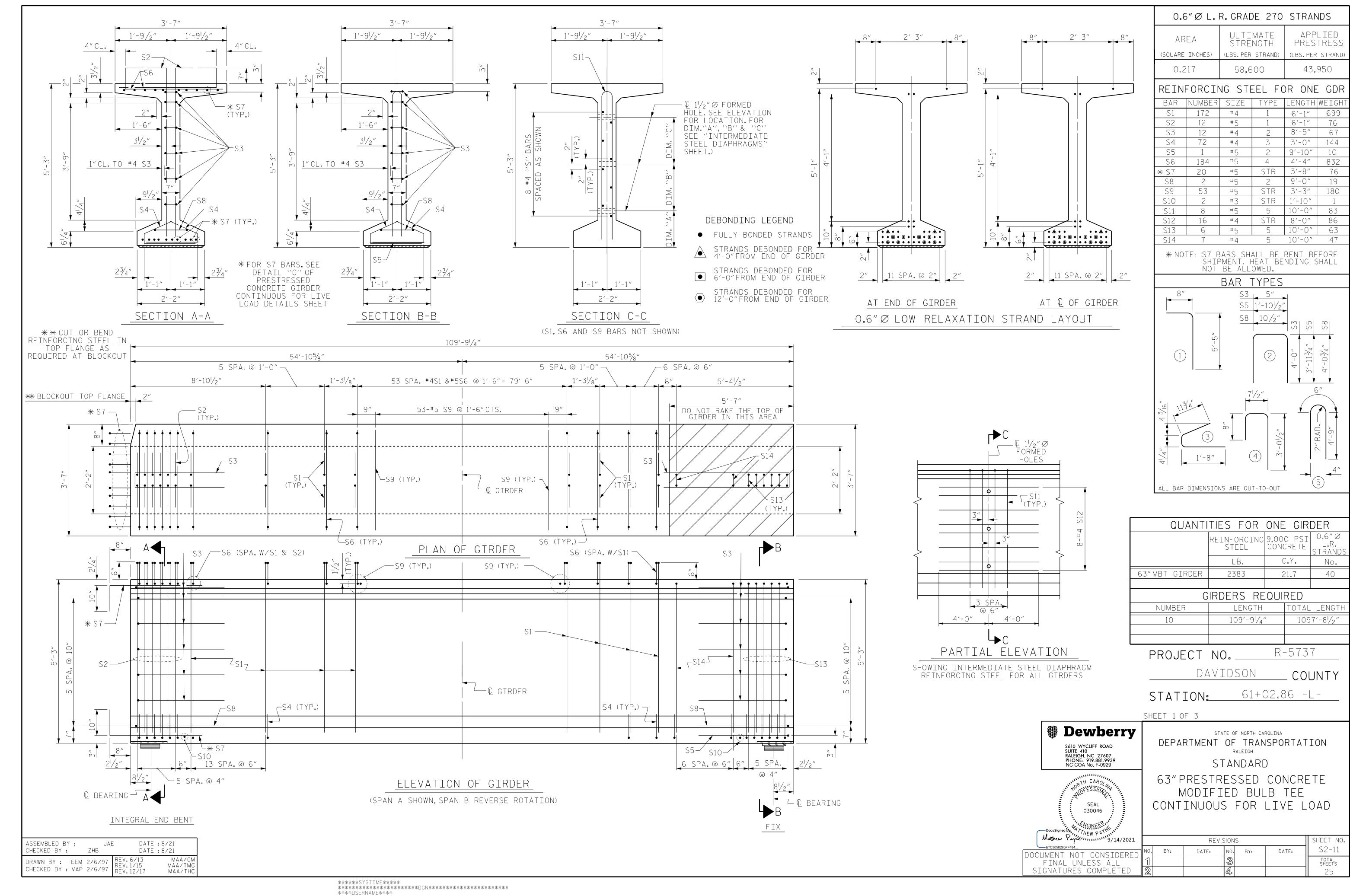


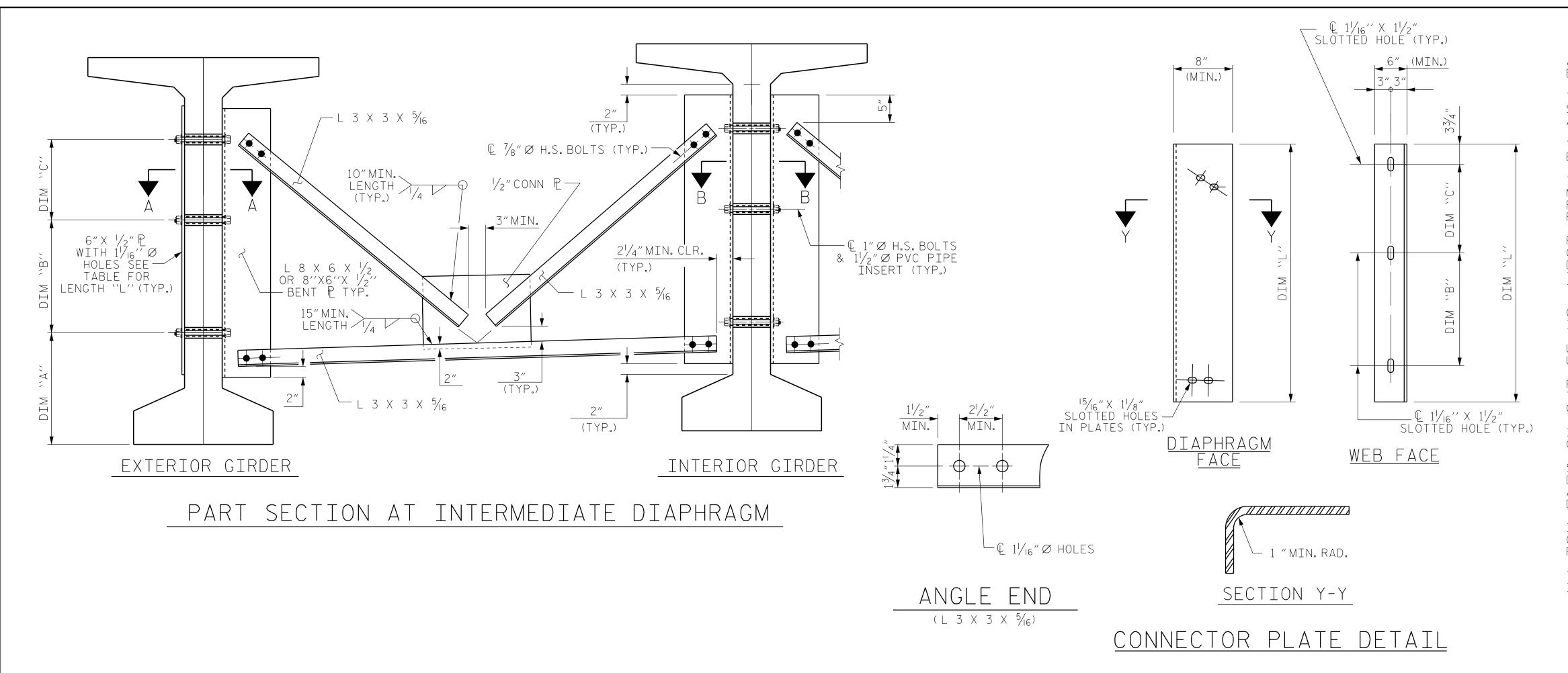












STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY A THERMAL SPRAYED COATING WITH A SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE DEPARTMENTS THERMAL SPRAYED COATINGS (METALLIZATION) PROGRAM, THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

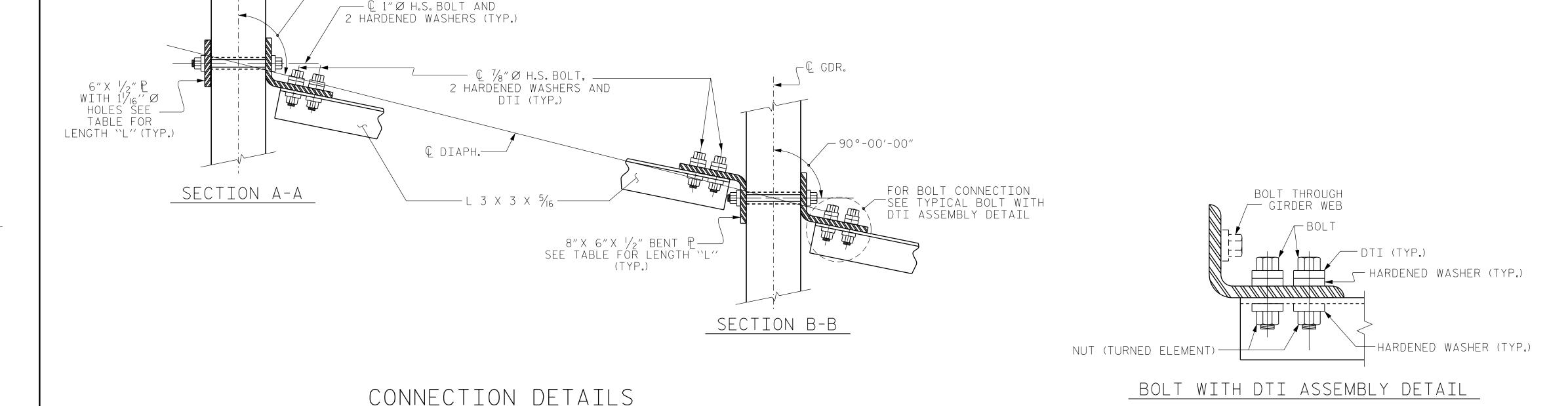
SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

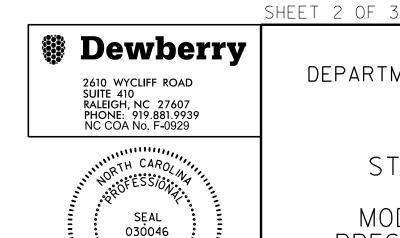
THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

TABLE

GIRDER TYPE	DIM ``A''	DIM "B"	DIM "C"	DIM ``L''
63" BULB TEE	1'-07/8"	1'-51/2"	1'-51/2"	3′-5′′



PROJECT NO. _____R-5737 _____DAVIDSON ____COUNTY STATION: ____61+02.86 -L-



DEPARTMENT OF TRANSPORTATION

STANDARD

INTERMEDIATE

STEEL DIAPHRAGMS

FOR 63''

MODIFIED BULB TEE

PRESTRESSED CONCRETE

DocuSigned by A THEW PANISH.

Mathew Payier.

9/14/2021

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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

25

SIGNATURES COMPLETED

-SKEW ANGLE

_ DATE : <u>8/21</u>

_ DATE : <u>8/21</u>

_ DATE : <u>8/21</u>

Ç GDR.─

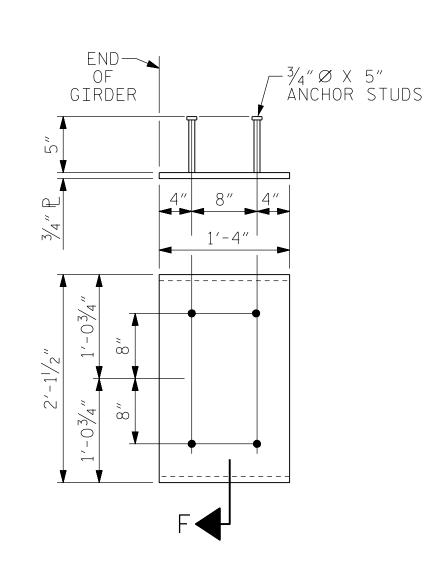
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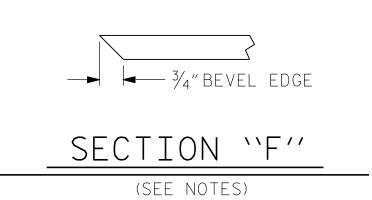
CHECKED BY : _

ZHB

DESIGN ENGINEER OF RECORD: MTP

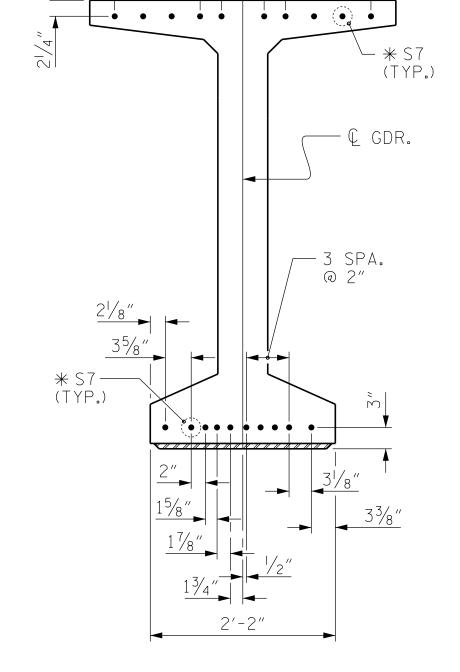
	DEAD LOAD DEFLECTION TABLE FOR GIRDERS - SPANS A & B																							
		GIRDERS 1 AND 5																						
FORTIETH POINTS	0 .025	.05 .075	.10 .125	15 .175 .20	.225	.25 .	.275 .30 .325	.35 .375	.40	.425 .45	.475 .50	.525	.55 .575	.60 .625	.65 .675	.70 .725	.75	.775 .80	.825	.85 .875	5 .90	.925	.95 .975	1.00
CAMBER (GIRDER ALONE IN PLACE)	0.00 0.089	0.131 0.169	0.203 0.231 0.	254 0.271 0.281	0.284 0	.281 C	0.271 0.254 0.23	1 0.203 0.169	0.131	0.131 0.089	0.045 0.045	0.089	0.131 0.16	9 0.203 0.231	0.254 0.271	0.281 0.338	0.281	0.271 0.254	0.231 0	.203 0.16	9 0.131	0.131	0.089 0.045	0.00
*DEFLECTION DUE TO SUPER IMPOSED D.L.↓	0.00 .013	0.026 0.039	0.052 0.064 0.	076 0.089 0.101	0.111 0	.121 C	0.130 0.140 0.130	0.140 0.159	0.165	0.167 0.174	0.171 0.174	0.171	.169 0.16	7 0.165 0.159	0.153 0.147	0.141 0.147	0.131	0.121 0.102	0.112 0	.090 0.07	7 0.052	0.065	0.039 0.026	0.00
FINAL CAMBER	0 1/8"	1/4" 5/16"	7/16" 9/16" 5	3/4" 13/16"	7/8" 15	5/16"	1" 11/16" 15/16"	19/16" 13/8"	11/4"	15/16" 11/4"	15/16" 15/16"	15/16" 1	5/ ₁₆ " 15/ ₁₆ '	1 1 1 4 " 1 1 1 4 "	13/16" 13/16"	11/16" 11/8"	1"	15/16" 13/16"	7/8"	1/16" 5/8"	7/16"	9/16"	5/16" 1/4"	0
											GIRDERS 2	THRU 4												
FORTIETH POINTS	0 .025	.05 .075	.10 .125 .	15 .175 .20	.225	.25	.275 .30 .325	.35 .375	.40	.425 .45	.475 .50	.525	.55 .575	.60 .625	.65 .675	.70 .725	.75	.775 .80	.825	.85 .875	5 .90	.925	.95 .975	1.00
CAMBER (GIRDER ALONE IN PLACE)	0.00 0.076	0.111 0.143	0.172 0.196 0.	.215 0.229 0.238	0.241 0.	.238 0	0.229 0.215 0.196	6 0.172 0.143	0.111	0.131 0.076	0.038 0.038	0.076	0.111	3 0.172 0.196	0.215 0.229	0.238 0.241	0.238	0.229 0.215	0.196 C	0.172 0.14	3 0.111	0.131	0.076 0.038	0.00
*DEFLECTION DUE TO SUPER IMPOSED D.L.↓	0.00 0.014	0.027 0.041	0.054 0.067 0.	080 0.093 0.106	0.116 0	.126 C	0.137 0.147 0.153	3 0.160 0.166	0.173	0.175 0.177	0.179 0.182	0.179	0.177 0.179	5 0.173 0.166	0.160 0.154	0.147 0.137	0.127	0.117 0.106	0.094 C	.081 0.06	8 0.055	5 0.041	0.027 0.014	0.00
FINAL CAMBER	0 1/8"	3/16" 5/16"	7/16" 1/2" 9	/16" 11/16" 3/4"	13/16"	7/8"	15/16" 1" 11/16"	15/16" 11/4"	13/16"	13/8" 11/2"	111/16" 17/8"	111/16"	1 1 3/8"	13/16" 11/8"	11/16" 11/16"	1" 15/16"	7/8"	13/16" 3/4"	11/16"	9/16" 1/2"	7/16"	5/16"	3/16" 1/8"	0





EMBEDDED PLATE 'B-1" DETAILS FOR 63" MODIFIED BULB TEES

(2 REQ'D PER GIRDER)



3'-7"

DETAIL ''C"

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

APPLY EPOXY PROTECTIVE COATING TO END OF GIRDER SURFACES INDICATED IN ELEVATION VIEW.

EMBEDDED PLATE ''B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2"BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 7000 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

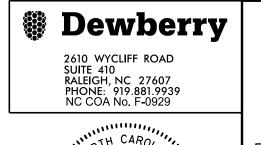
THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF $\frac{1}{4}$ ".

WHEN DRAPED STRANDS ARE DETAILED, THE LONGITUDINAL LOCATION OF THE HOLD DOWN DEVICES SHALL BE WITHIN 6" OF THE LOCATION SHOWN AND THE CENTER OF GRAVITY OF THE GROUP OF DRAPED STRANDS SHALL BE LOCATED WITHIN $\frac{1}{2}$ " OF THE THEORETICAL LOCATION SHOWN.

A 2" \times 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 63" AND 72" MODIFIED BULB TEES ONLY.

PROJECT NO. _____R-5737 _____DAVIDSON ____COUNTY STATION: ____61+02.86 -L-

SHEET 3 OF 3



SEAL 030046 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

PRESTRESSED CONCRETE GIRDER
CONTINUOUS FOR LIVE LOAD
DETAILS

Docusigned by: Marthew Parkition

Nothew Parkition

E7C3056295FF484...

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

2

REVISIONS

REVISIONS

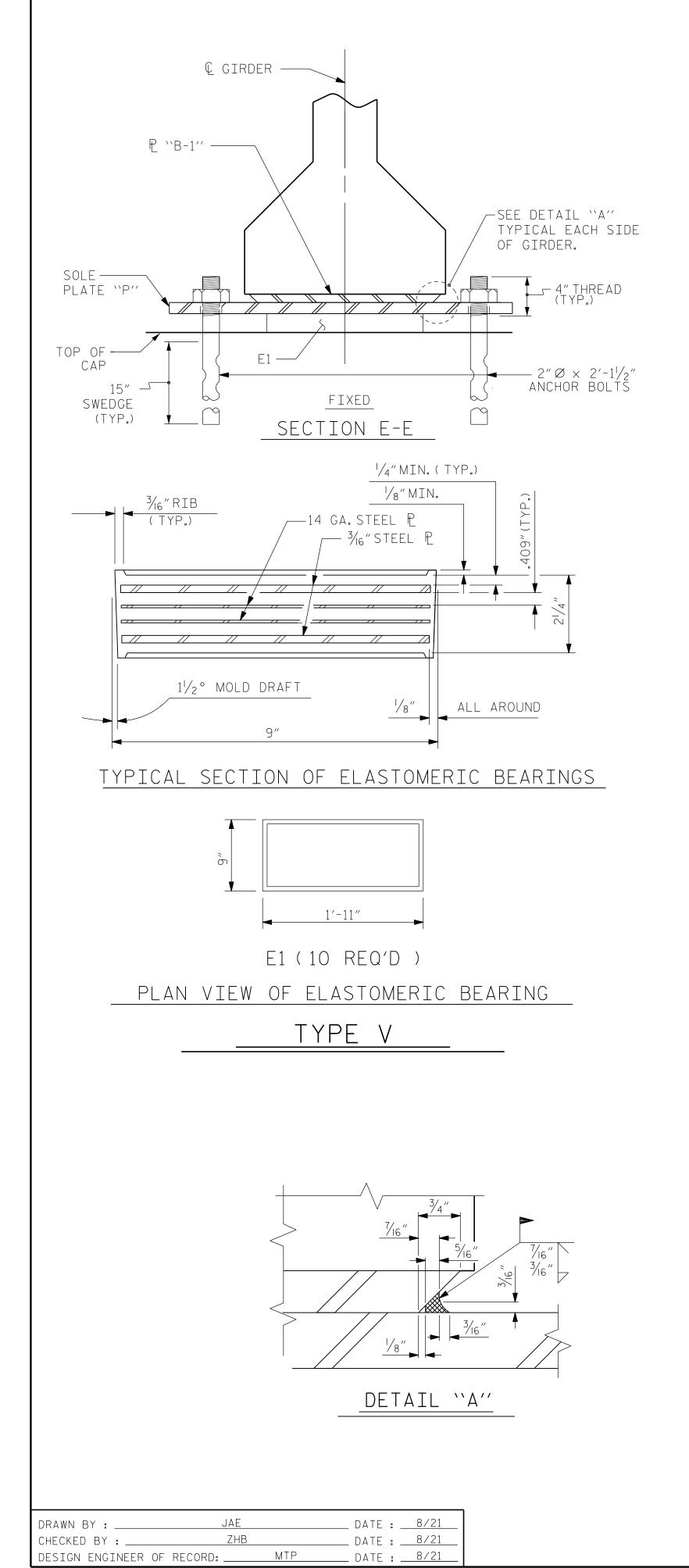
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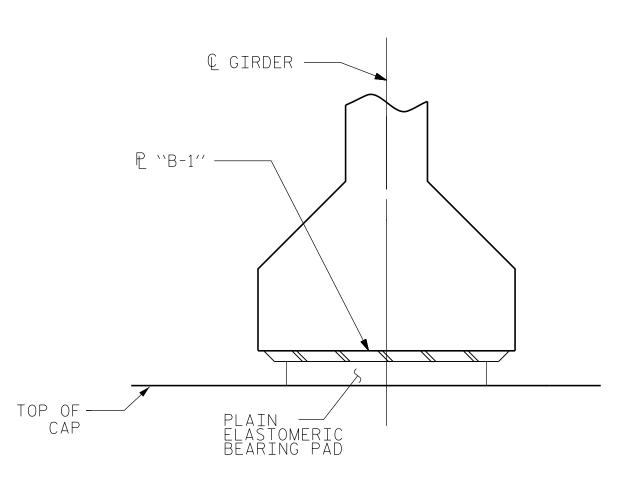
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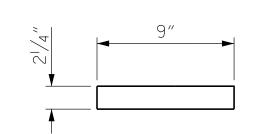
4

25

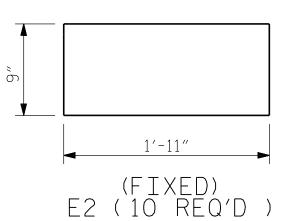




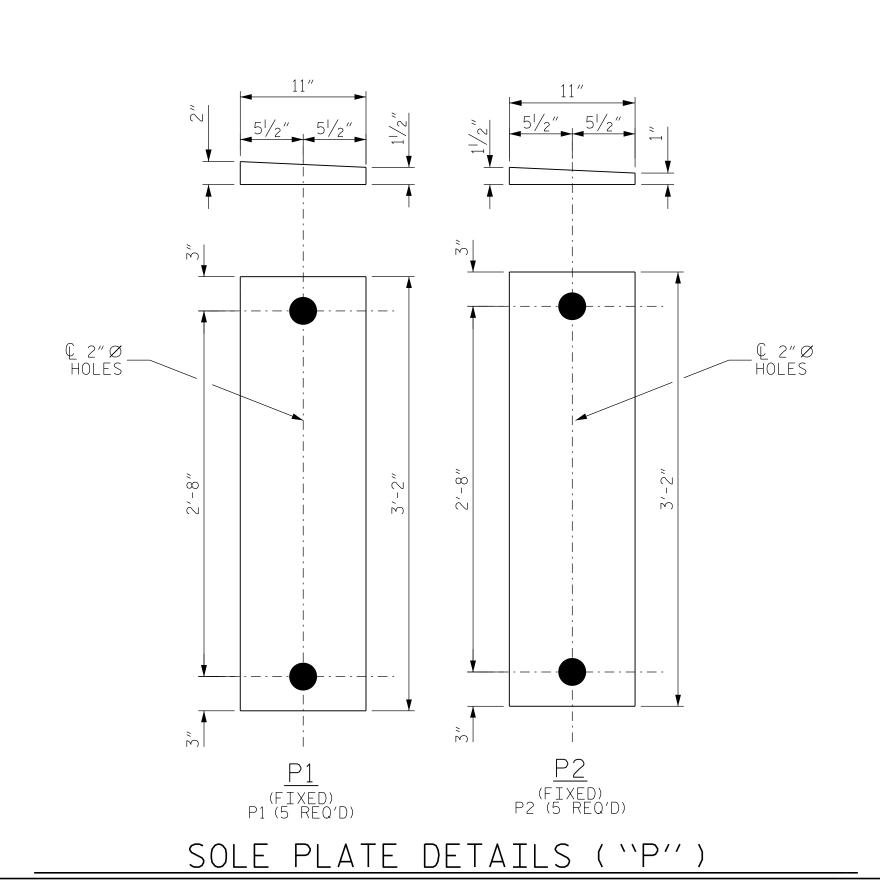
SECTION F-F



TYPICAL SECTION OF ELASTOMERIC BEARING



PLAN VIEW OF ELASTOMERIC BEARING



NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2"Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

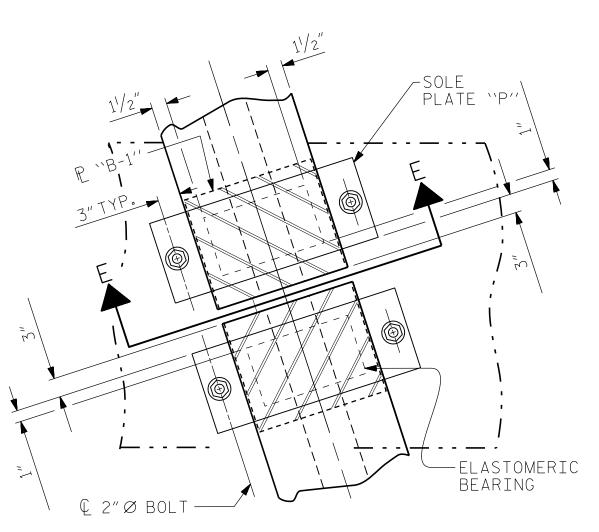
ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. NO SHOP DRAWINGS ARE REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

ALL SOLE PLATES SHALL BE AASHTO M270 GRADE 36.



MAXIMUM ALLOWABLE SERVICE LOADS D.L.+L.L.(NO IMPACT)

CAP AND BEARING

—ELASTOMERIC BEARING _I

— Ç OF GIRDER

TYPICAL PLAN @ END BENT

D.L.+L.L.(NO IMPACT)

TYPE V 365 k

_____D_E7

TYPICAL PLAN @ BENT

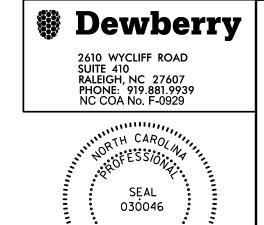
PROJECT NO. R-5737

DAVIDSON

CO

STATION: 61+02.86 -L-

SHEET 1 OF 1



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

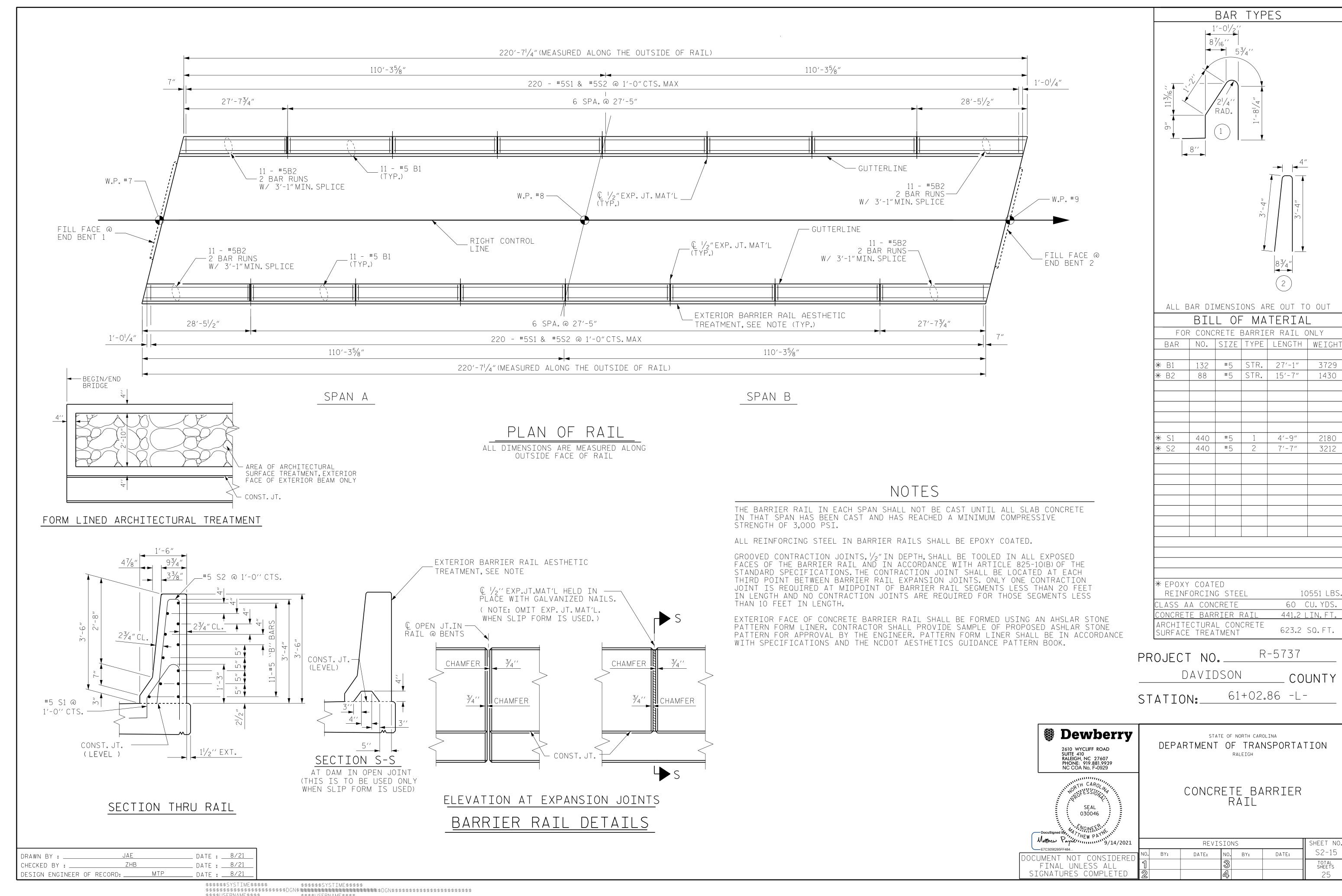
STANDARD

COUNTY

ELASTOMERIC BEARING ———— DETAILS ————

PRESTRESSED CONCRETE GIRDER SUPERSTRUCTURE

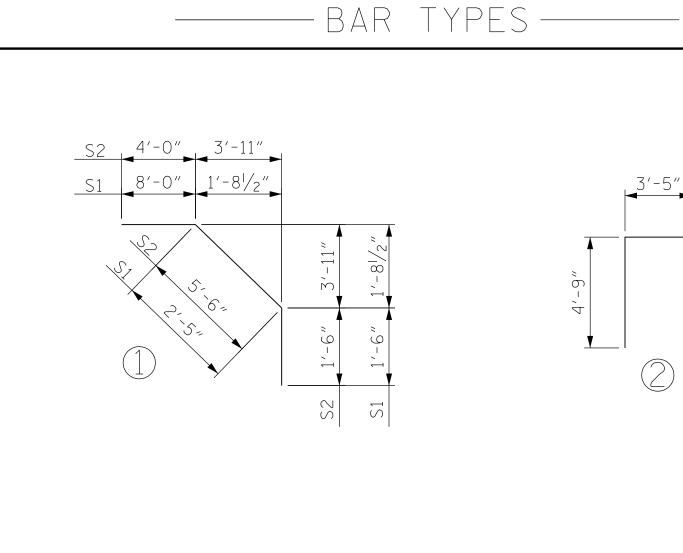
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						RFT	NFOR	CTNG	BAR	SCHED	III F							
BAR	NO.	C T 7 F	TYPE	I FNCTH	WETCHT	BAR	T		TYPE			BAR	NO	CT7F	TYPE			WE TOUT
DAK	NO.	SIZE	ITPE	LENGTH	WEIGHT	DAR	NO.	SIZE	ITTE	LENGTH	WEIGHT	DAR	NO.	SIZE	ITPE	LENG ⁻	П	WEIGHT
₩ A1	336	#5	STR	43′-5″	15215	A2	336	#5	STR	43'-5"	15215	* S1	28	# 4	1	11'-1	1"	223
,												* S2	28	# 4	1	11'-0		206
* A101	2	#5	STR	40'-11"	85	A201	2	#5	STR	40'-11"	85							
* A102	2	#5	STR	38′-5″	80	A202	2	#5	STR	38′-5″	80	U1	28	#4	2	12'-1	1"	242
* A103	2	#5	STR	35′-11″	75	A203	2	#5	STR	35′-11″	75							
* A104	2	#5	STR	33'-5"	70	A204	2	#5	STR	33′-5″	70							
* A105	2	#5	STR	30'-11"	64	A205	2	#5	STR	30'-11"	64	REINFOR	CING S	STEEL	'		27,3	331 LBS.
* A106	2	#5	STR	28'-5"	59	A206	2	#5	STR	28'-5"	59	* EPOXY	COATE	D REINF	ORCING	STEEL	30,4	26 LBS.
* A107	2	#5	STR	25′-11″	54	A207	2	#5	STR	25′-11″	54					·		
* A108	2	#5	STR	23′-5″	49	A208	2	#5	STR	23'-5"	49							
* A109	2	#5	STR	20'-11"	44	A209	2	#5	STR	20'-11"	44							
* A110	2	#5	STR	18′-5″	38	A210	2	#5	STR	18'-5"	38							
* A111	2	#5	STR	15'-11"	33	A211	2	#5	STR	15'-11"	33							
* A112	2	#5	STR	13′-5″	28	A212	2	#5	STR	13'-5"	28							
* A113	2	#5	STR	10'-11"	23	A213	2	#5	STR	10'-11"	23							
* A114	2	#5	STR	8'-5"	18	A214	2	#5	STR	8'-5"	18							
* A115	2	#5	STR	5'-11"	12	A215	2	#5	STR	5'-11"	12							
* A116	2	#5	STR	3′-6″	7	A216	2	#5	STR	3′-6″	7							
* ∆117	2	#5	STR	1'-10"	4	A217	2	#5	STR	1'-10"	4]						
B1	150	#6	STR	45′-8″	10289	K1	24	#4	STR	22'-11"	367							
 ₩ B2	206	#5	STR	22'-2"	4763	K2	8	#4	STR	5′-3″	28							
∗ B3	140	#5	STR	28'-10"	4210	К3	32	#4	STR	8'-3"	176							
 ₩ B4	105	#5	STR	26'-6"	2902	K4	8	#4	STR	6'-8"	36]						
★ B5	660	#5	STR	32'-4"	2226	K5	4	#4	STR	2'-8"	7							
						K6	16	#4	STR	3′-6″	37]						
						K7	4	#4	STR	2'-0"	5							

	SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS												
BAR SIZE	EXCEPT SLABS,	TRUCTURE APPROACH PARAPET, RIER RAIL	APPROAC	PARAPET AND BARRIER									
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL								
#4	1'-11"	1'-7"	1'-11"	1'-7"	2'-6"								
#5	2′-5″	2'-0"	2'-5"	2'-0"	3'-1"								
#6	2'-10"	2'-5"	3'-7"	2'-5"	3′-8″								
#7	4'-2"	2'-9"		-	-								
#8	4'-9"	3′-2″	_	_	_								

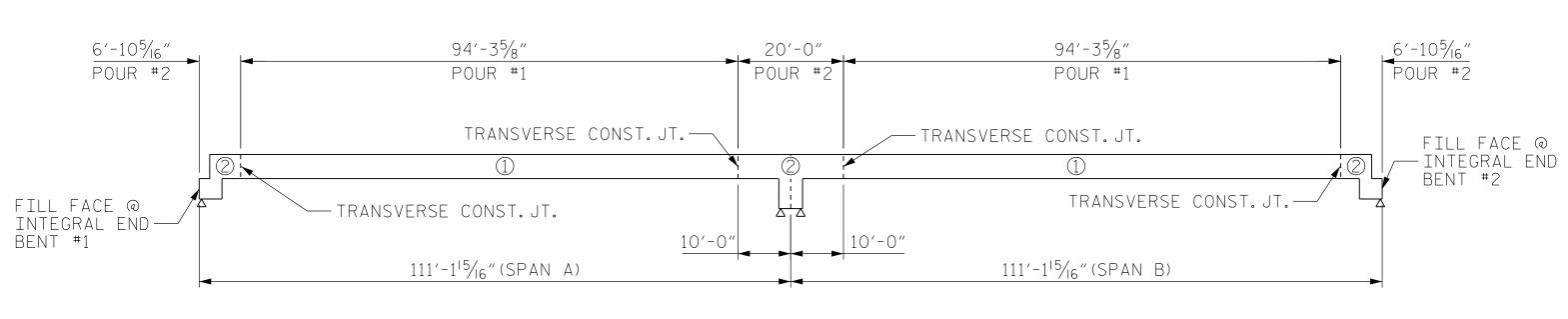
GROOVING B	BRIDGE FL	OOR
APPROACH SLABS	1,810	SQ.FT
BRIDGE DECK	8,260	.SQ.FT
OTAL	10,070	.SQ.FT



ALL BAR DIMENSIONS ARE OUT TO OUT.

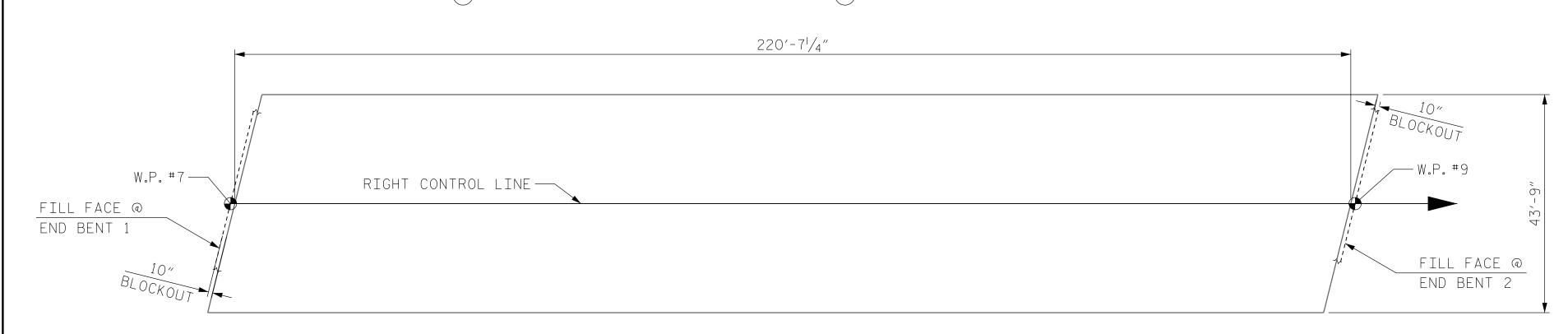
	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL
	(CU. YDS.)	(LBS.)	(LBS.)
POUR #1	275.1		
POUR #2	111.6		
TOTAL	386.7	27,331	30,426

* * QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED



POURING SEQUENCE

POUR 2 CAN NOT BE STARTED UNTIL BOTH ADJACENT 1 POURS REACH A MINIMUM OF 3000 PSI

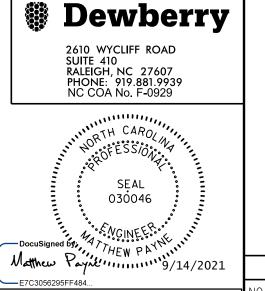


LAYOUT FOR COMPUTING AREA
REINFORCED CONCRETE DECK SLAB
(SQ.FT. = 9,652)

DRAWN BY :	JAE		DATE : _	8/21
CHECKED BY :	ZHB		DATE : _	8/21
DESTON ENGINEER	OF RECORD.	MTP	DATE .	8/21

\$\$\$\$\$\$SYSTIME\$\$\$\$ \$\$\$\$USERNAME\$\$\$\$

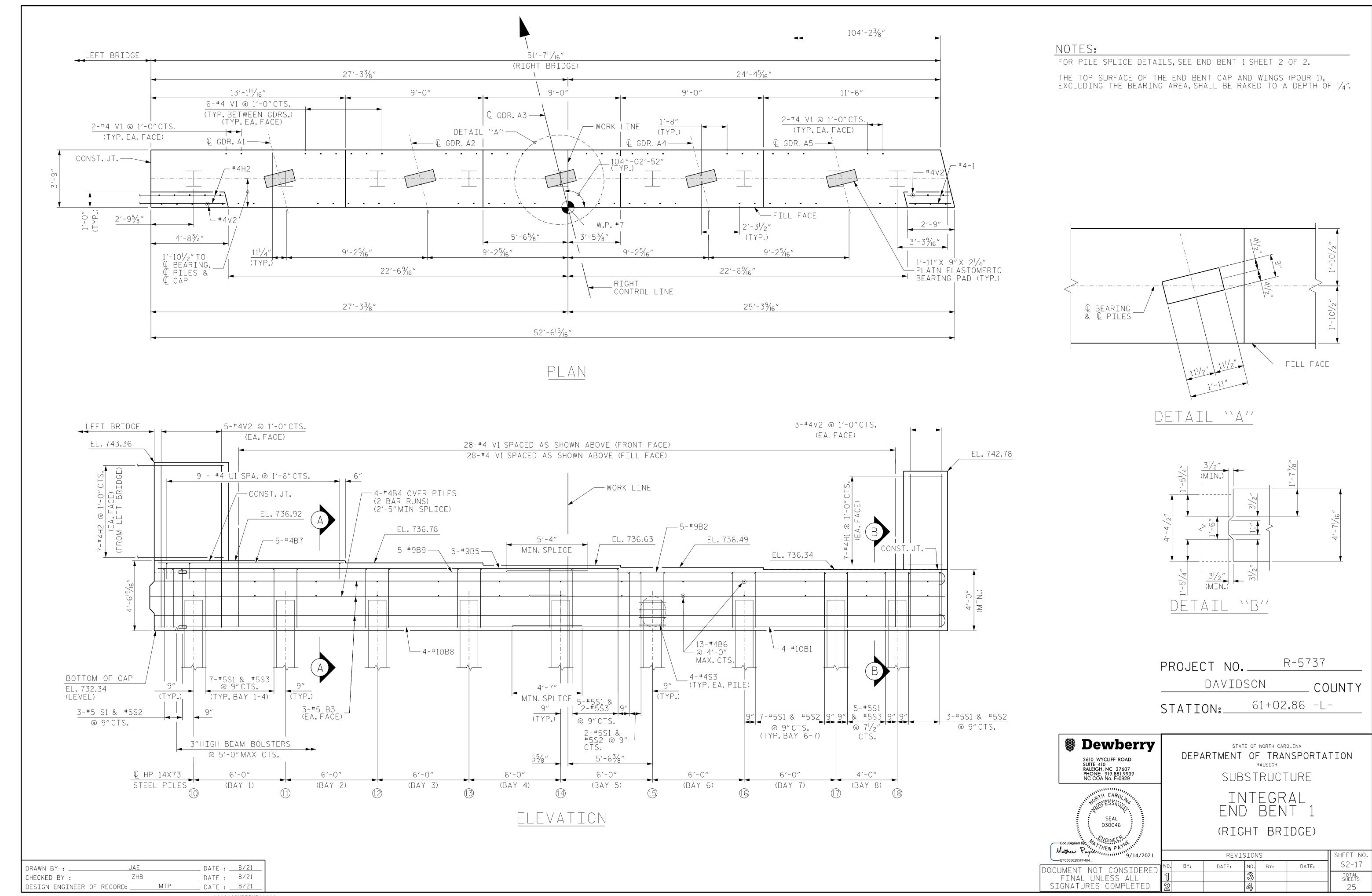
R-5737 PROJECT NO.___ DAVIDSON COUNTY 61+02.86 -L-STATION:__

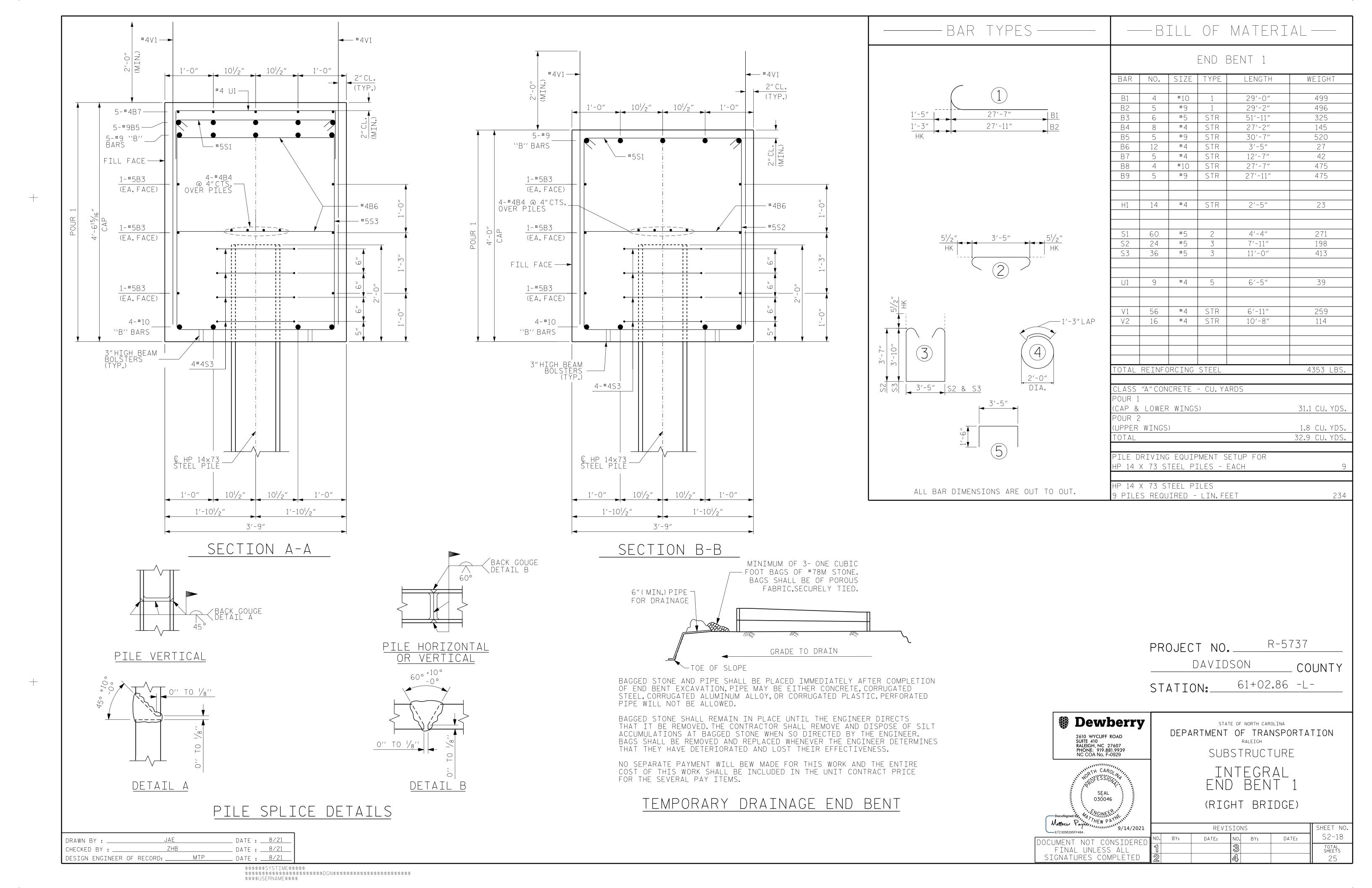


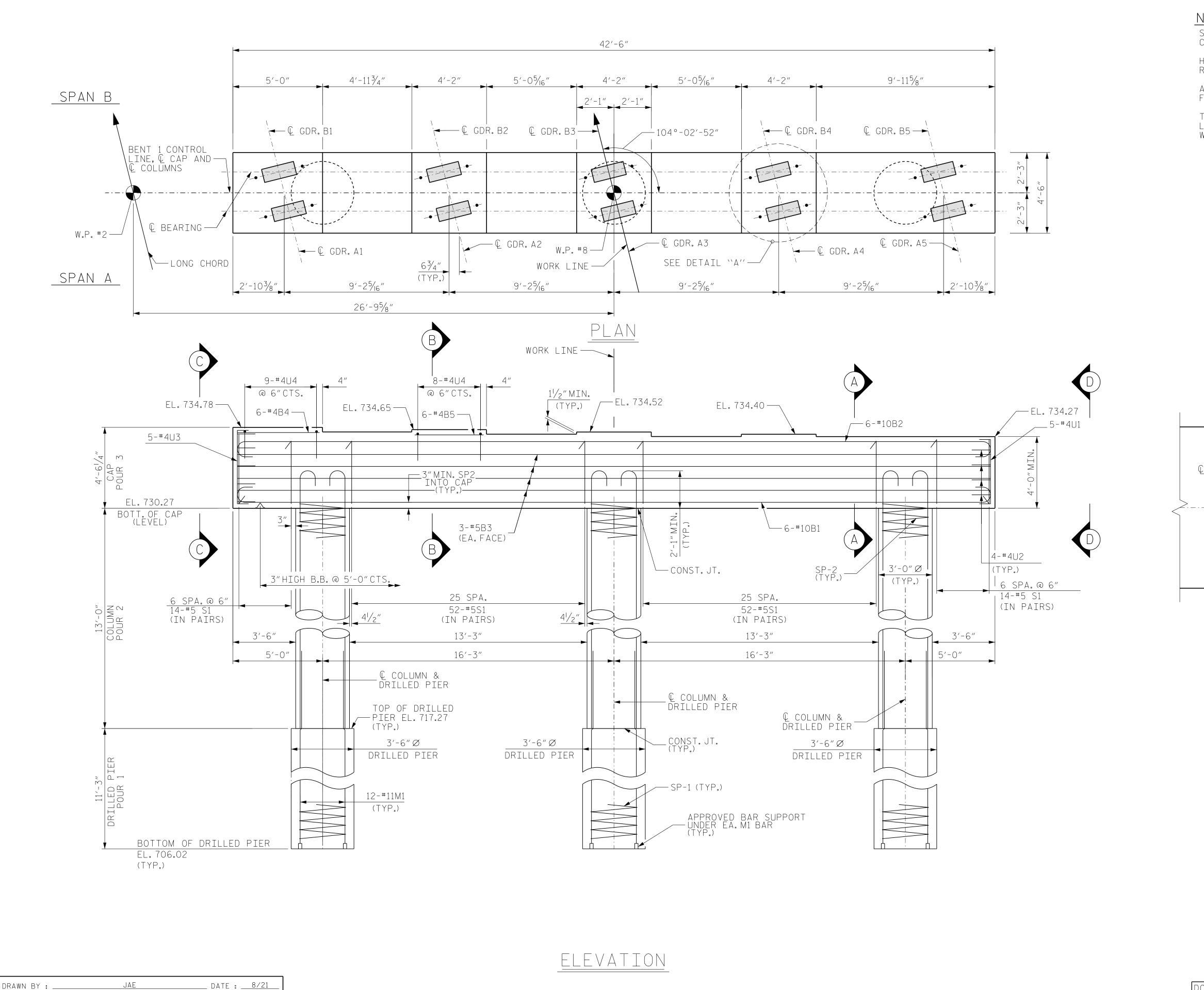
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

SUPERSTRUCTURE BILL OF MATERIAL (RIGHT BRIDGE)

SHEET NO REVISIONS S2-16 DATE: DATE: BY: NO. BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS







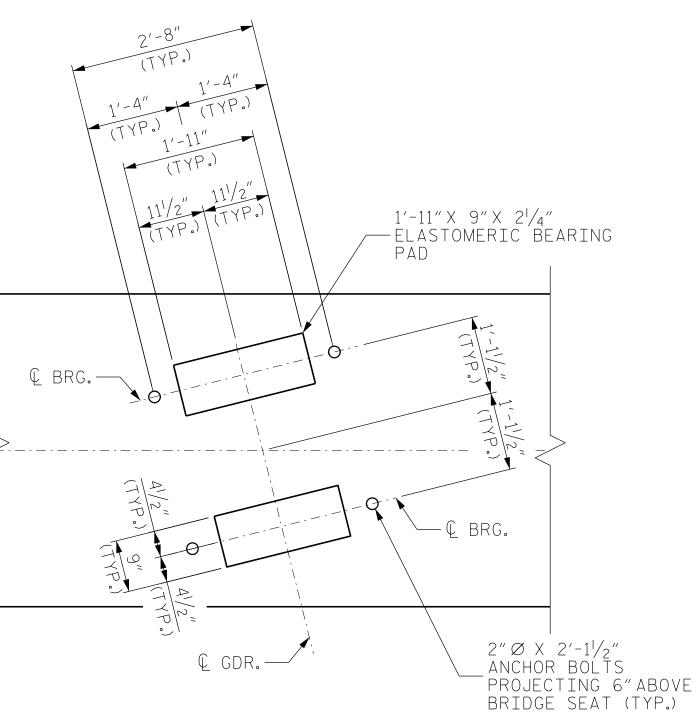
NOTES:

STIRRUPS AND "'U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

HOOKS ON "M" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

ALL STEEL IN THE DRILLED PIRES IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL".

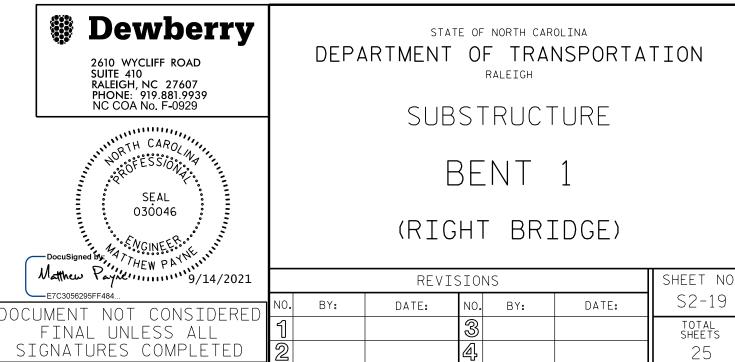
THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS ARE DETAILED WITH 3 FEET OF EXTRA LENGTH.



DETAIL ''A''

R-5737 PROJECT NO._ DAVIDSON COUNTY 61+02.86 -L-STATION:_

TOTAL SHEETS



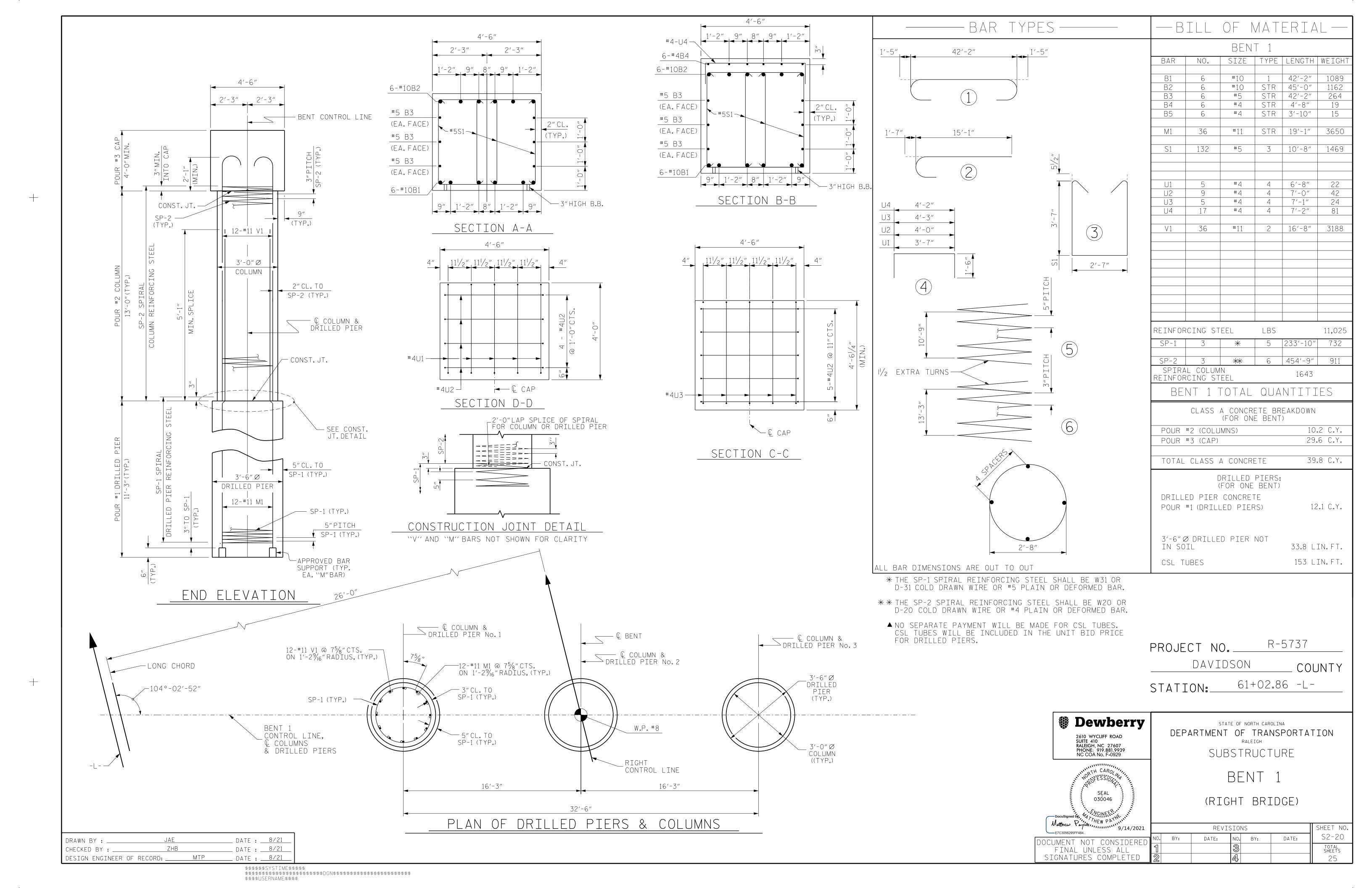
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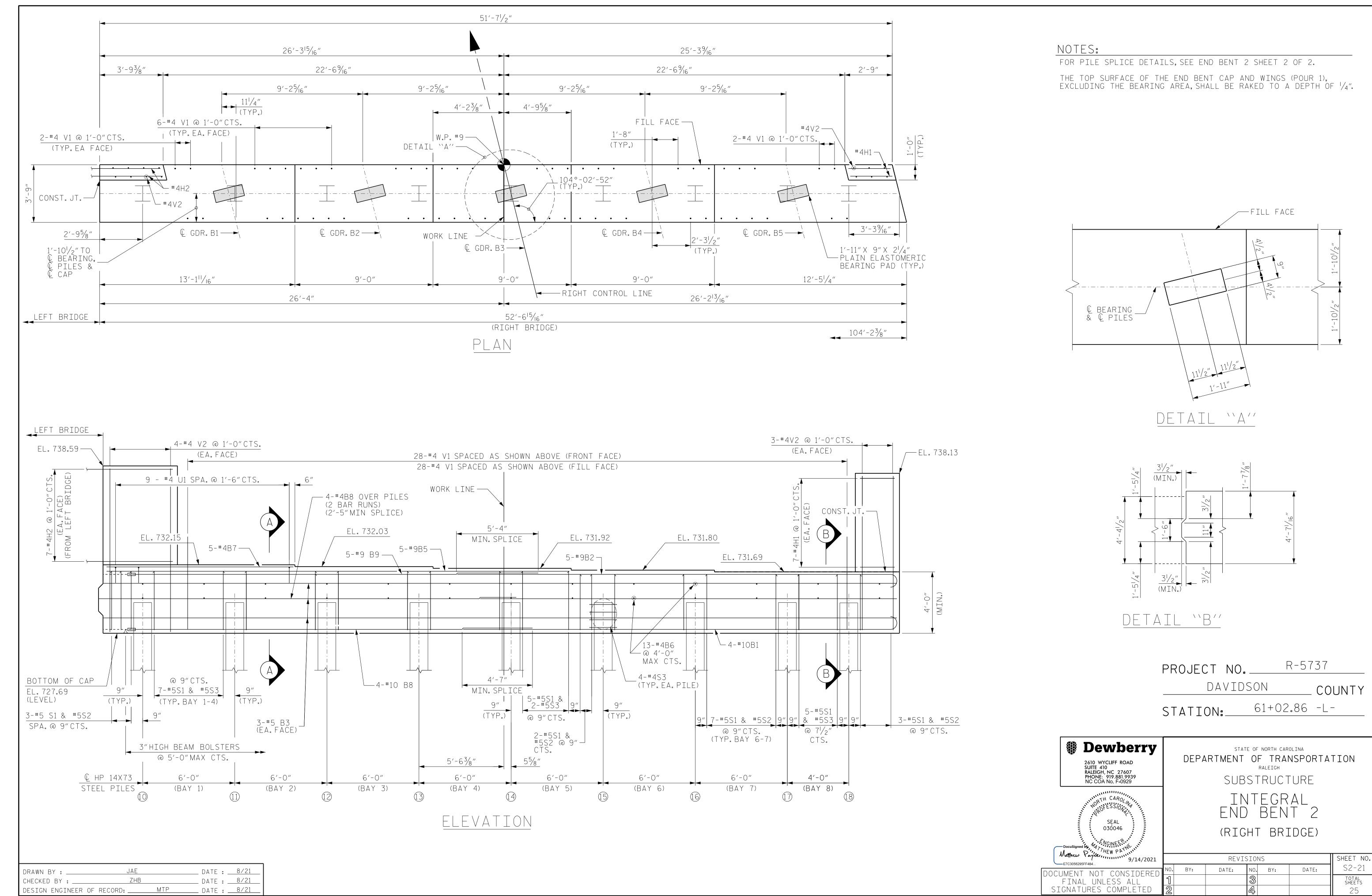
_ DATE : <u>8/21</u>

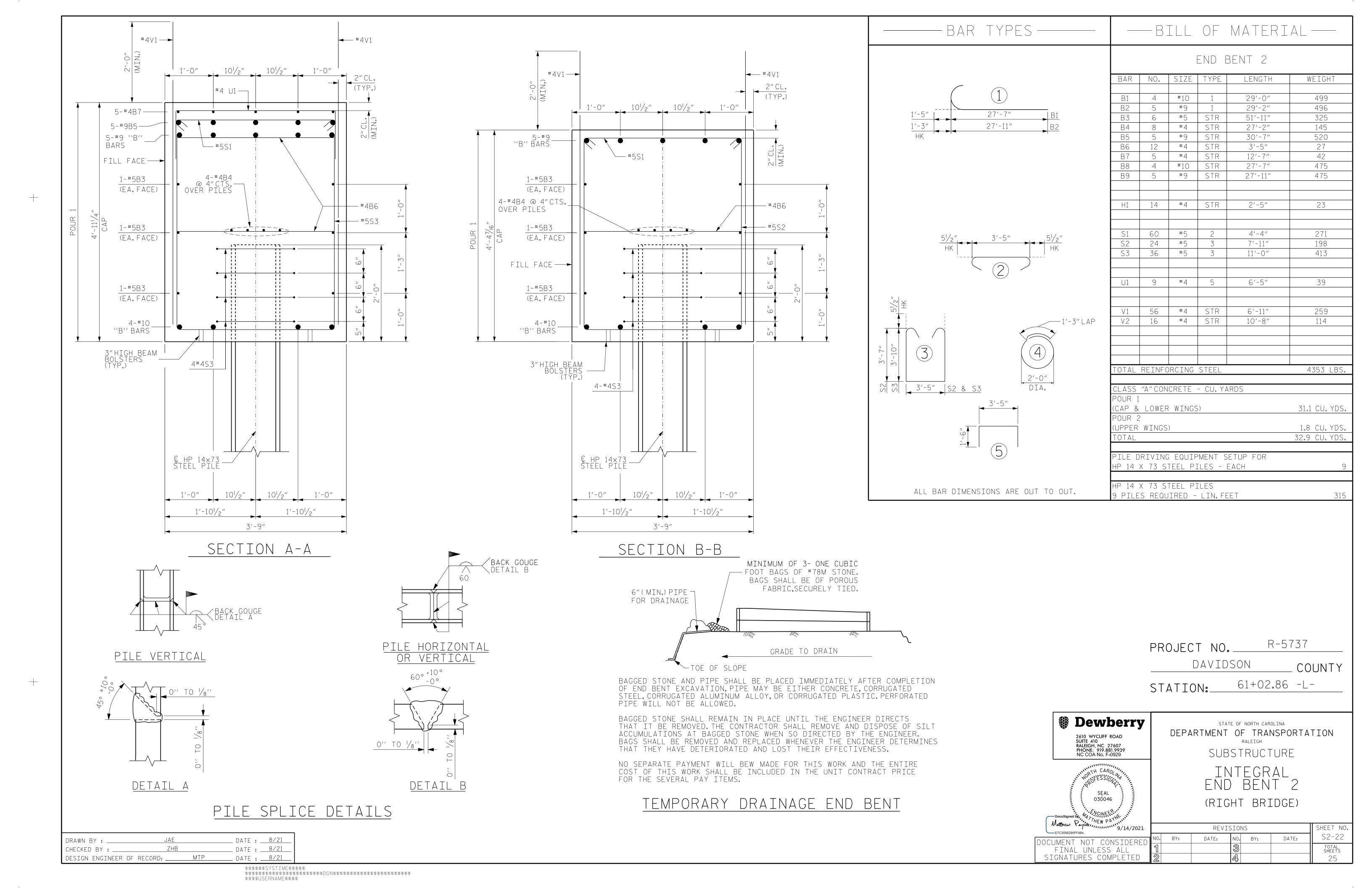
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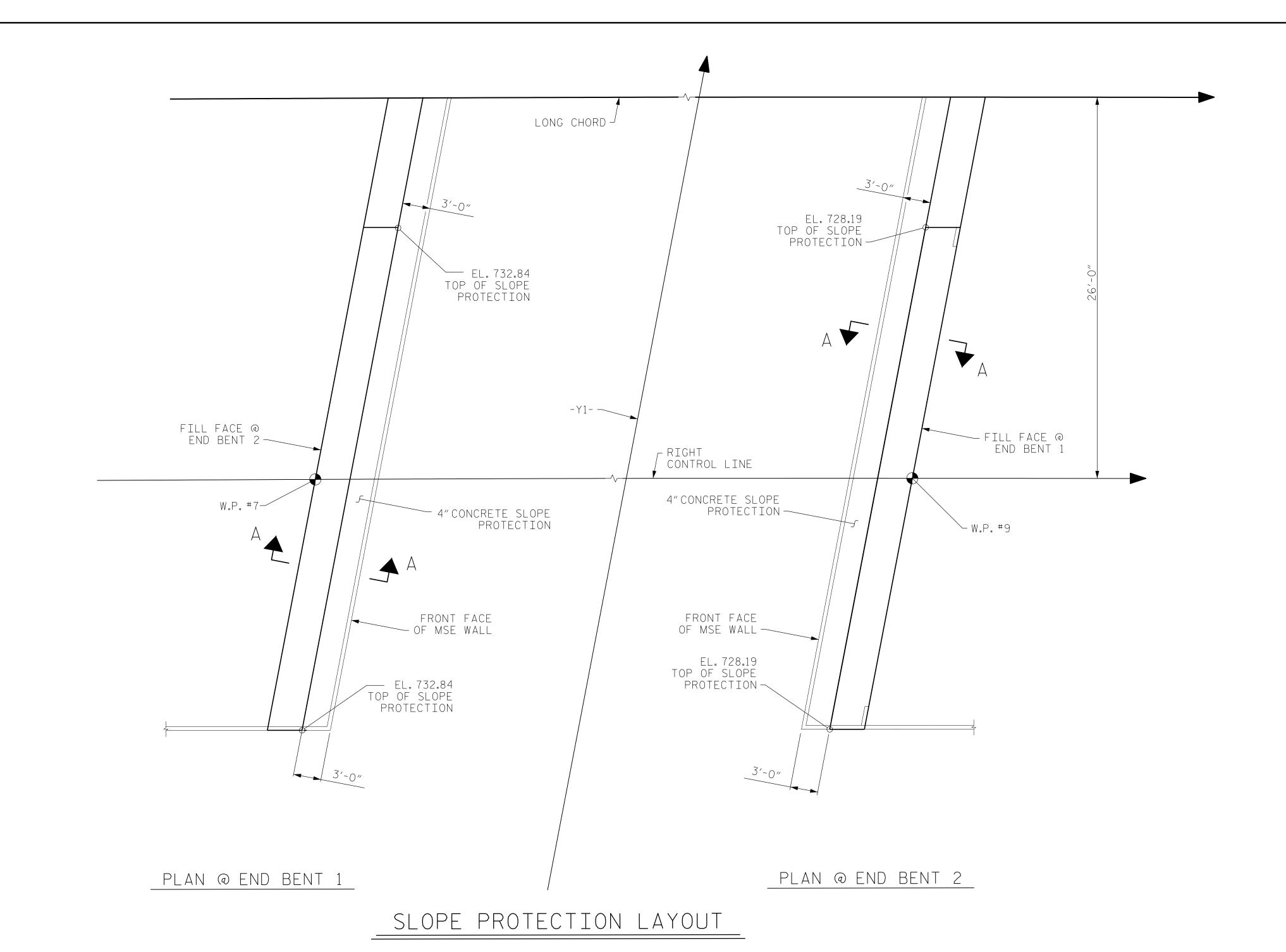
ZHB

DESIGN ENGINEER OF RECORD: MTP









* WELDED WIRE FABRIC 60 INCHES WIDE APPROX. L.F. 31

*QUANTITY SHOWN IS BASED ON 5'POURS.

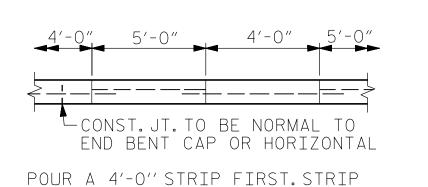
4 INCH

SLOPE PROTECTION

SQUARE YARDS

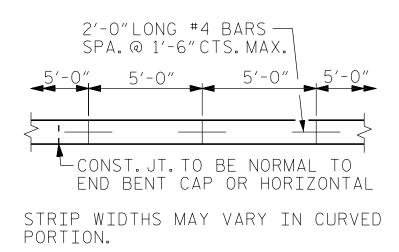
17

17



OPTIONAL POURING DETAIL

WIDTHS MAY VARY IN CURVED PORTION.

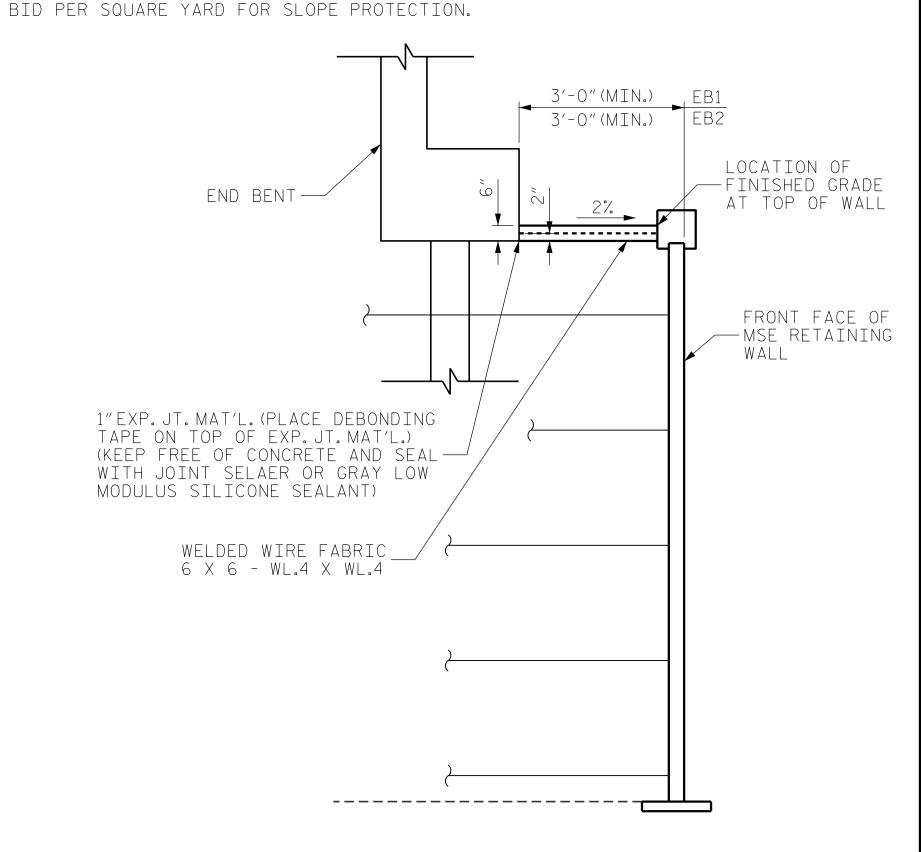


POURING DETAIL

NOTES:

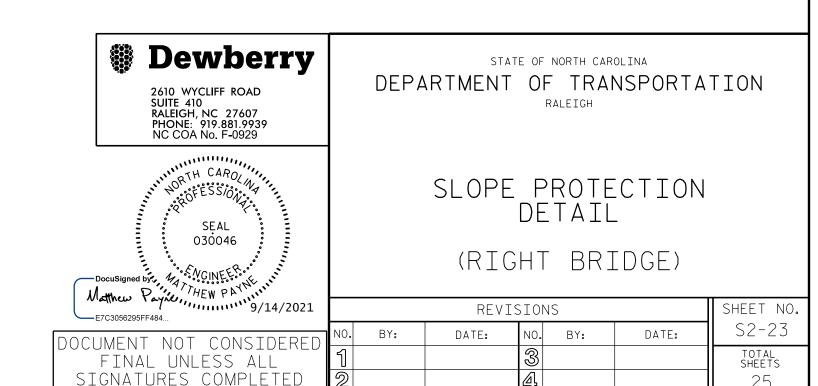
SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS.STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.FOR BERM WIDTH, SEE GENERAL DRAWING.

SLOPE PROTECTION SHALL CONSIST OF 4"POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - WL4.60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5'STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-O"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5'STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE



PROJECT NO. _____R-5737 _____DAVIDSON ____COUNTY STATION: ____61+02.86 -L-

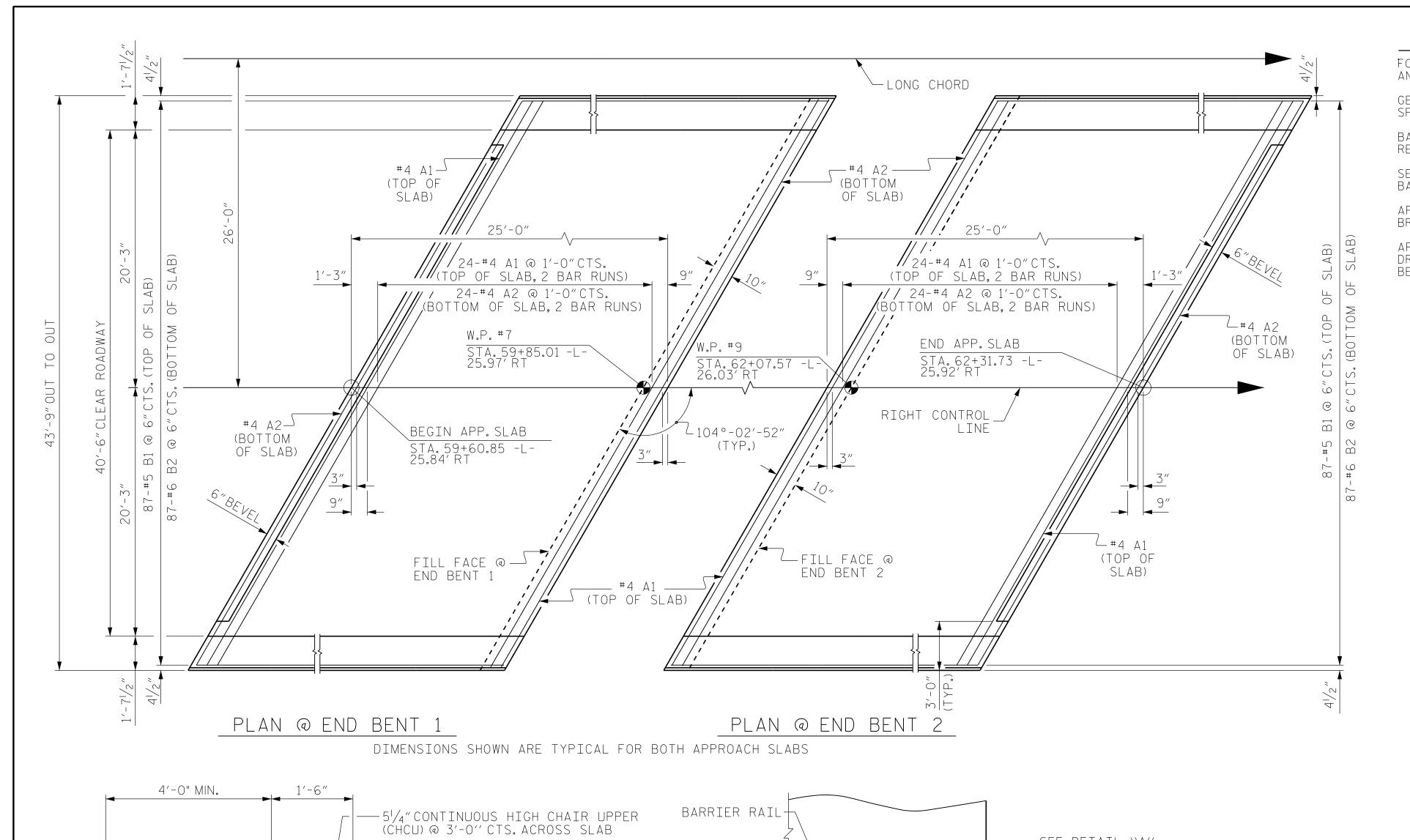
SECTION A-A

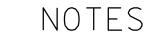


STA. 61+02.86 -L-(RIGHT LANE)

END BENT 1

END BENT 2





FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 6" Ø DRAINAGE PIPE, AND SELECT MATERIAL BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

BACKFILL MATERIAL SHALL BE THE SAME MATERIAL USED IN THE THE MSE REINFORCED ZONE.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

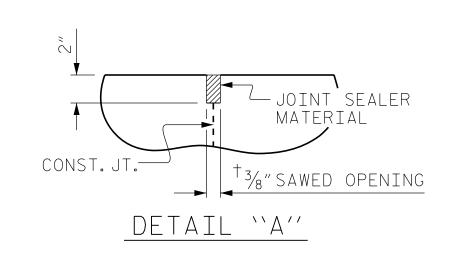
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

BILL OF MATERIAL												
FOR ONE APPROACH SLAB (2 REQ'D)												
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT							
* ∆1	52	#4	STR	23'-4"	811							
Α2	54	#4	STR	23'-2"	836							
 ₩ B1	87	#5	STR	24'-2"	2193							
B2	87	#6	STR	24'-8"	3223							
REINF	ORCIN	NG STE	EL * *	← LBS.	4059							
* EPOXY COATED . REINFORCING STEEL ** LBS. 3004.												
CLASS	AA	CONCRE	ETE * *	C. Y.	47.1 .							

** QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED. SEE SHEET 2 OF 2.

ALL BAR DIMENSIONS ARE OUT TO OUT

SPLICE LENGTHS											
BAR SIZE	EPOXY COATED	UNCOATED									
#4	1'-11"	1'-7"									
#5	2'-5"	2'-0"									
#6	3′-7″	2′-5″									



** Dewberry

2610 WYCLIFF ROAD
SUITE 410
RALEIGH, NC 27607
PHONE: 919.881.9939
NC COA No. F-0929

SHEET 1 OF 2

DEPARTN

OK : ÉŠŠ101, .

SEAL 030046 STATE OF NORTH CAROLINA

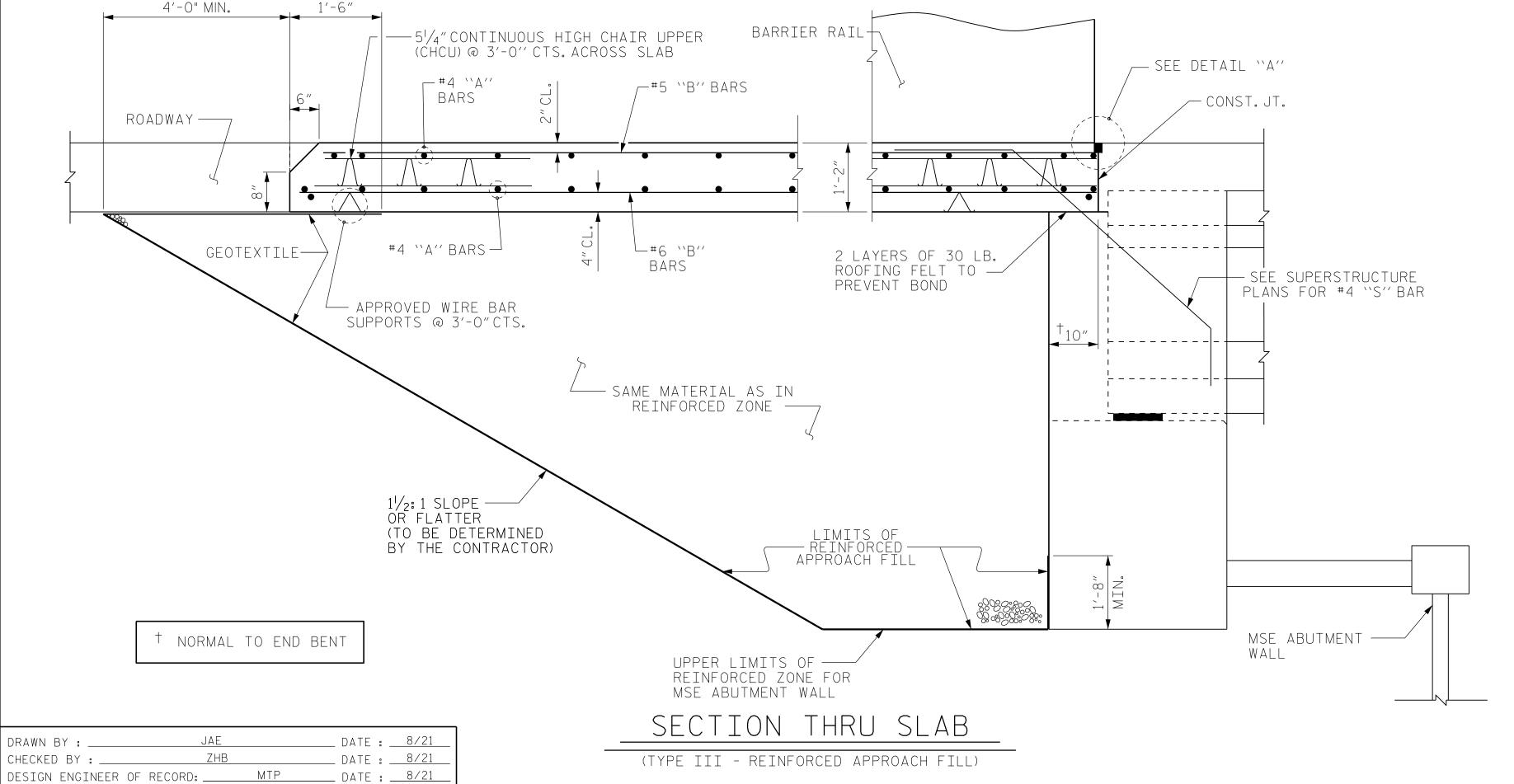
DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

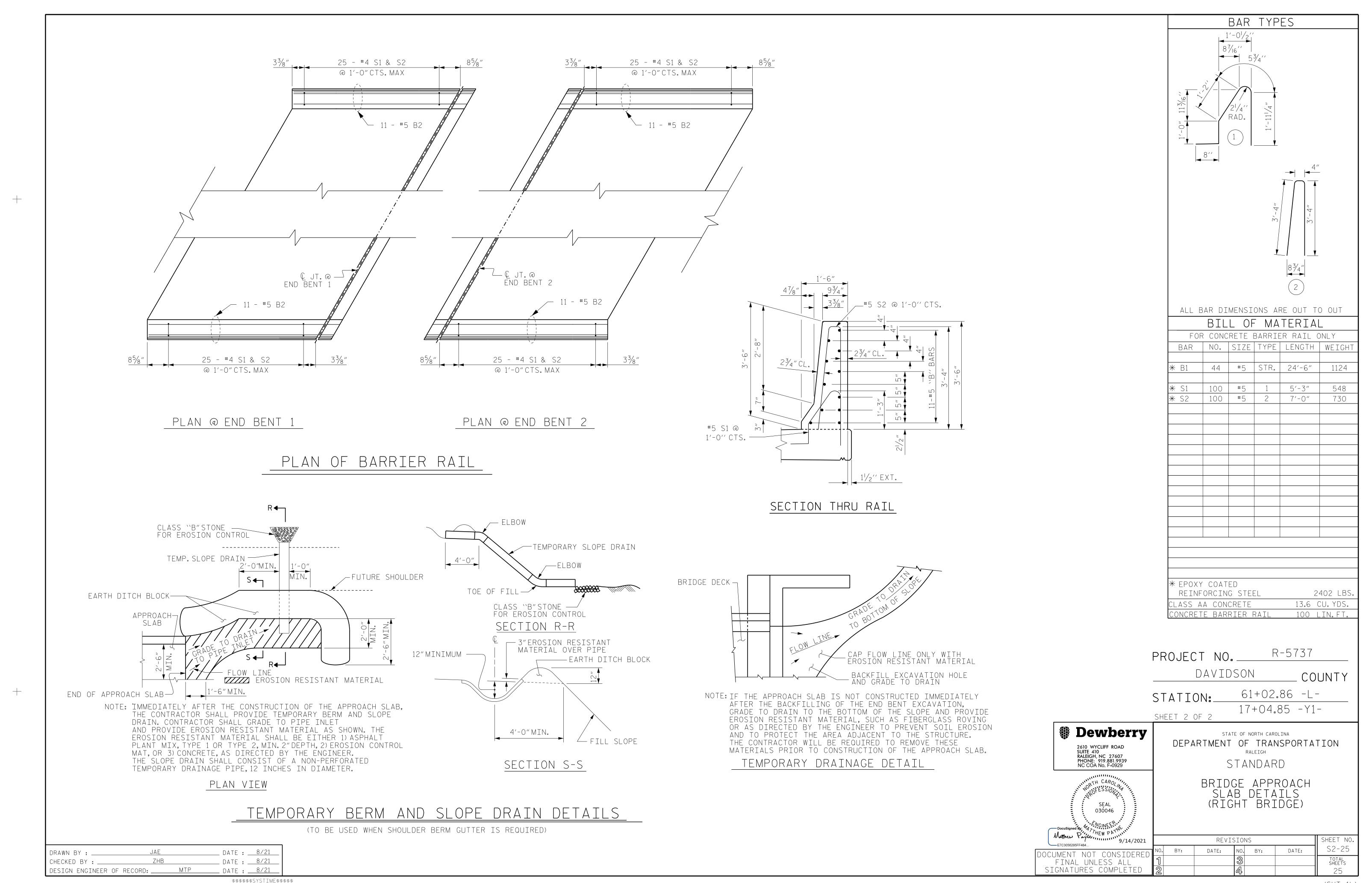
BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT

Matthew Payae, 17 HEW PAYA				SHEET NO.			
CUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S2-24
FINAL UNLESS ALL	1			33			TOTAL SHEETS
IGNATURES COMPLETED	2			4			25



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STANDARD NOTES

DESIGN DATA:

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/6 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH