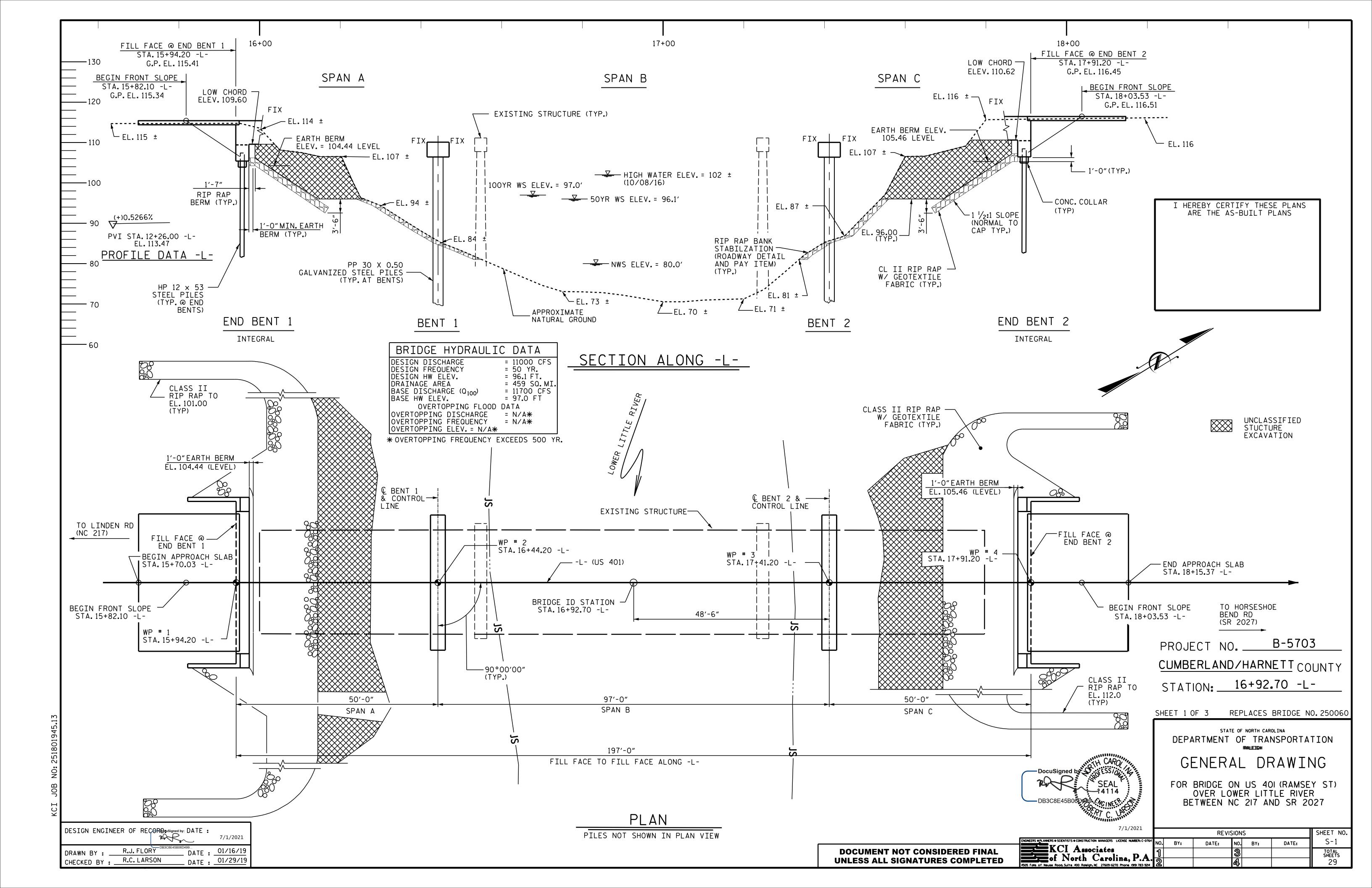
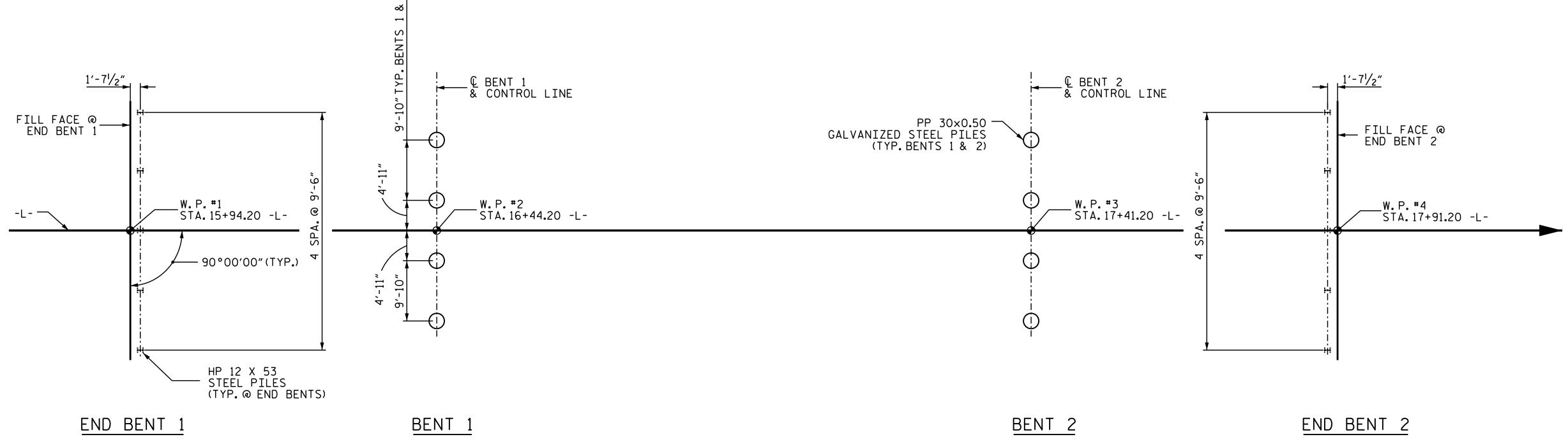




<b>PROJECT LENGTH</b>	Prepared in the Office of:KCI Associates of N.C., P.A.4505 Falls of Neuse Road, Suite 400Raleigh, NC 27609Phone (919) 783-9214Fax (919) 783-9266	Plans Prepared For: DIVISION OF HIGHWAY 1000 Birch Ridge Dr. Raleigh NC, 27610
TH OF ROADWAY TIP PROJECT B-5703 = .139 MILES TH OF STRUCTURE TIP PROJECT B-5703 = .037 MILES AL LENGTH OF TIP PROJECT B-5703 = .176 MILES	Fax (919) 783-92662018 STANDARD SPECIFICATIONSRIGHT OF WAY DATE:JUNE 26, 2019	ELIZABETH R. PHIPPS, F PROJECT ENGINEER
	<i>LETTING DATE:</i> AUGUST 17, 2021	ROBERT C. LARSON, P. PROJECT DESIGN ENGINEER
	NCDOT CONTACT:	KRISTY ALFORD, P.E.





## FOUNDATION NOTES

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS. PILES AT END BENT 1 AND END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.

PILES AT BENT 1 AND BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 250 TONS PER PILE.

DRIVE PILES AT END BENT 1 AND END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.

DRIVE PILES AT BENT 1 AND BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 425 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR SCOUR.

INSTALL PILES AT BENT 1 TO A TIP ELEVATION NO HIGHER THAN 56.0 FT (LT) AND 53.0 FT (RT).

INSTALL PILES AT BENT 2 TO A TIP ELEVATION NO HIGHER THAN 54.0 FT (LT) AND 51.0 FT (RT). THE SCOUR CRITICAL ELEVATION FOR BENT 1 AND BENT 2 IS ELEVATION 77.0 FT. SCOUR

CRITICAL ELEVATION IS USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING.FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 80-170 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT BENT 1 AND BENT 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

DESIGN ENGINE	ER OF RECC	RD: DocuSigned by:	:	7/1/2021
DRAWN BY : CHECKED BY : _	A. SAMBOY R.C. LARSO	DB3C8E45B06D499 DATE N DATE	-	<u>03/11/19</u> 09/10/20

# FOUNDATION LAYOUT

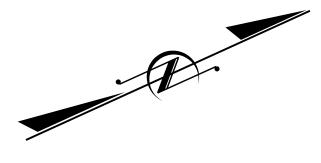
(NOTE: ALL PILES ARE VERTICAL)

## FOUNDATION RECOMMENDATION COMMENTS

AND 79.9 FT.

1.5:1 (H:V) SLOPE AT THE END BENTS ARE OK WITH SLOPE PROTECTION. INTEGRAL END BENT WILL BE USED AT END BENT 1 AND END BENT 2. USE ONLY TYPE A BRIDGE APPROACH FILL DETAILS AT END BENT 1 TO AID SLOPE STABILITY. USE TYPE I OR TYPE A BRIDGE APPROACH FILL DETAILS AT END BENT 2. THE DESIGN SCOUR ELEVATIONS FOR BENT 1 AND BENT 2 ARE 79.3 FT.

NO WAITING PERIOD IS REQUIRED BEFORE BEGINNING OF END BENT CONSTRUCTION. USE A SINGLE ROW OF PLUMB PILES AT EACH END BENT.



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DocuSianed b<sup>,</sup>

### B-5703 PROJECT NO. CUMBERLAND/HARNETT COUNTY

STATION: <u>16+92.70</u> -L-

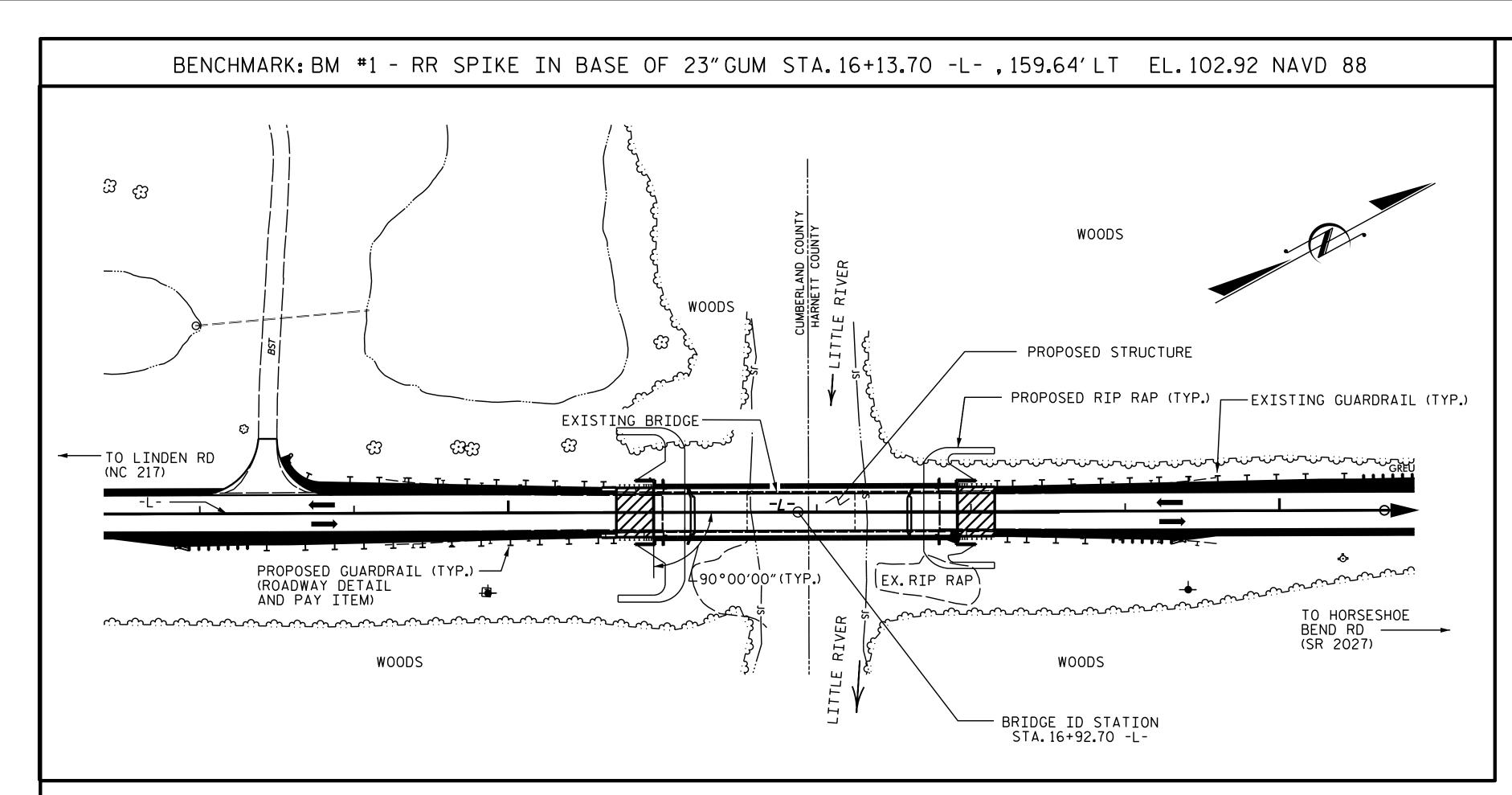
SHEET 2 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

# GENERAL DRAWING

FOR BRIDGE ON US 401 (RAMSEY ST) OVER LOWER LITTLE RIVER BETWEEN NC 217 AND SR 2027

7/1/2021		REVISI			SHEET NO.					
ENGINEERS OPLANNERS OSCIENTISTS CONSTRUCTION MANAGERS LICENSE NUMBER: C-0764	NO. BY:	DATE: I	NO. BY:	DATE:	S-2					
KCI Associates	1		3		TOTAL SHEETS					
4505 Falls of Neuse Road, Suite 400 Rateligh, NC 27609-6270 Phone 1991 783-9214	2	6	4		29					



# LOCATION SKETCH

NOTE: FOR UTILITY I	INFORMATION, SEE	UTILITY	PLAN
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	TOTAL BILL OF MATERIAL																				
	REMOVAL OF EXISTING STRUCTURE @ STA. 16+92.70 -L-	ASBESTOS ASSESSMENT	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION AT STATION 16+92.70 -L-	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE (BRIDGE)	BRIDGE APPROACH SLABS AT STA. 16+92.70 -L-	REINFORCING STEEL	PRES CON GIN	54″ TRESSED ICRETE RDERS	PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES	PILE DRIVING EQUIPMENT SETUP FOR PP 30X0.5 GALVANIZED STEEL PILES	HP	12X53 STEEL PILES	PP30 X 0.50 GALVANIZED STEEL PILES	PILE REDRIVES	CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-O" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS
	LUMP SUM	LUMP SUM	EACH	LUMP SUM	SQ.FT	SQ.FT.	CU.Y	LUMP SUM	LBS.	NO.	LIN.FT.	EACH	EACH	NO.	LIN.FT.	NO. LIN.FT.	EACH	LIN.FT.	TON	SQ.YDS.	LUMP SUM
SUPERSTRUCTURE	LUMP SUM	LUMP SUM			6885	7086		LUMP SUM		12	774.00							390.67			LUMP SUM
END BENT 1							31.1		4381			5		5	275		3		550	610	
BENT 1							15.6		2861				4			4 230	2				
BENT 2							15.6		2861				4			4 260	2				
END BENT 2							31.1		4381			5		5	275		3		305	340	
TOTAL	LUMP SUM	LUMP SUM	1	LUMP SUM	6885	7086	93.4	LUMP SUM	14,484	12	774.00	10	8	10	550	8 490	10	390.67	855	950	LUMP SUM

### NOTES (CONT'D):

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18- EVALUATING SCOUR AT BRIDGES."

FOR INTERIOR BENTS 1 AND 2, ONLY PARTIAL GALVANIZING OF THE PILES IS REQUIRED. SEE INTERIOR BENT SHEETS FOR REQUIRED GALVANIZED LENGTHS. PAYMENT FOR PARTIALLY GALVANIZED PILES WILL BE MADE UNDER THE CONTRACT UNIT PRICE FOR GALVANIZED PILES.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

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KCI	

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A K ALL AND PB3C8E45B06D499	ATE : 7/1/2021
	ATE : 03/13/19 ATE : 03/13/19

### NOTES:

ASSUMED LOADING = HL-93 OR ALERNATE LOADING. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. THIS BRIDGE IS LOCATED IN SEISMIC PERFORMANCE ZONE 1. FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN. FOR SUBMITTAL OF WORKING DRAWINGS. SEE SPECIAL PROVISIONS. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY-IN- PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING" STRUCTURE AT STATION 16+ 92.70 -L-"

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION.SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 1 @ 54.9',1 @ 69.8',1 @ 54.9' CONTINUOUS STEEL BEAMS; 25'-10" CLEAR ROADWAY WIDTH RC FLOOR ON CONCRETE POST-AND-BEAM BENTS/CONCRETE END BENTS AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

### ANS AND SPECIAL PROVISIONS.

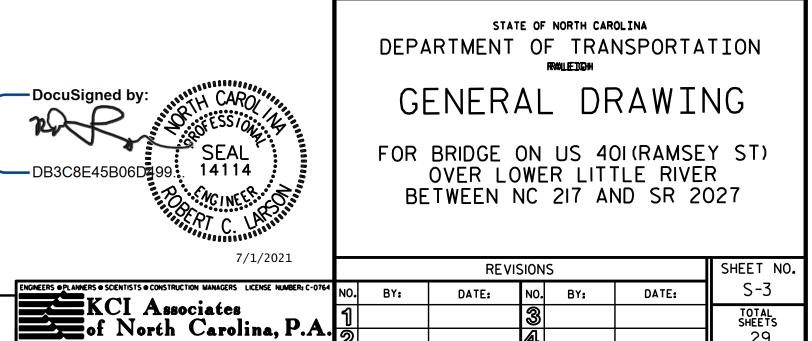
B-5703 PROJECT NO. \_\_

29

CUMBERLAND/HARNETT COUNTY

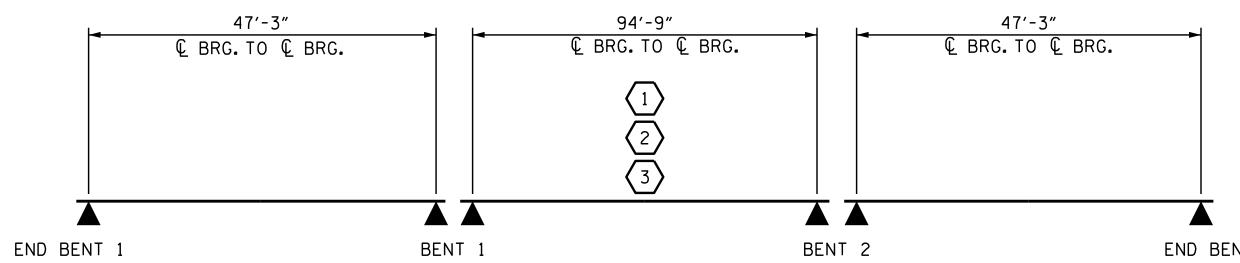
STATION: \_\_\_\_16+92.70 -L-

SHEET 3 OF 3



05 Falls of Neuse Road, Suite 400 Raleigh, NC 27609-6270 Phone (919) 783-921

										STRE	NGTH	I LIM	IT ST	ATE				SE	RVICE	III	LIMI	T STA	TE	
								1		MOMENT					SHEAR						MOMENT			
LEVEL		( M )	LING #	f ACTORS	W × RF	(کر <sub>لدل</sub> ) ۱۹۵	UTION (DF)	FACTOR		LOCATION	E FROM D OF H)	UTION (DF)	FACTOR		LOCATION	E FROM D OF H)	ORS (Y <sub>LL</sub> ). ORS (Y <sub>LL</sub> )	RIBUTION ORS (DF)	FACTOR		LOCATION	E FROM D OF H)	NUMBER	
	VEHICLE	WEIGHT (TONS)	CONTROL LOAD RA	MINIMUM RATING F. (RF)	TONS = V	LIVE-LOAD FACTORS (Y <sub>LI</sub>	DISTRIBUTION FACTORS (DF)	RATING	SPAN	GIRDER	DISTANCE LEFT END SPAN (f+)	DISTRIBUTION FACTORS (DF)	RATING	SPAN	GIRDER	DISTANCE LEFT END SPAN (ft)	LIVE-LO, FACTORS	DISTRIB FACTORS	RATING	SPAN	GIRDER	DISTANCE LEFT END SPAN (f†)	COMMENT	
		HL-93 (INVENTORY)	N⁄A	$\langle 1 \rangle$	1.32		1.75	0.810	1.65	В	E	47.38	0.940	1.34	В	I	8.91	0.80	0.760	1.32	В	I	47.38	
DESIGN LOAD		HL-93 (OPERATING)	N⁄A		1.77		1.35	0.810	2.14	В	E	47.38	0.940	1.77	В	I	8.91	N/A						
RATING		HS-20 (INVENTORY)	36.000	2	1.81	65.160	1.75	0.810	2.28	В	E	47.38	0.940	1.81	В	I	8.91	0.80	0.760	1.81	В	I	47.38	
	HS-20 (OPERATING)	36.000		2.37	85.320	1.35	0.810	2.95	В	E	47.38	0.940	2.37	В	I	8.91	N/A							
u		SNSH	13 <b>.</b> 500		4.28	57.780	1.40	0.900	6.66	А	I	23.62	0.940	5.70	А	I	9.02	0.80	0.760	4.28	В	I	47.38	
	ш	SNGARBS2	20.000		3.11	62.000	1.40	0.810	4.88	В	E	47.38	0.940	4.01	В	I	8.91	0.80	0.760	3.11	В	I	47.38	
	ICL	SNAGRIS2	22.000		2.91	64.020	1.40	0.810	4.57	В	E	47.38	0.940	3.69	В	I	8.91	0.80	0.760	2.91	В	I	47.38	
	<pre></pre>	SNCOTTS3	27 <b>.</b> 250		2.13	58.043	1.40	0.900	3.32	А	I	23.62	0.940	2.81	В	I	8.91	0.80	0.760	2.13	В	I	47.38	
	SLE (S	SNAGGRS4	34.925		1.75	61.119	1.40	0.810	2.74	В	E	47.38	0.940	2.28	В	I	8.91	0.80	0.760	1.75	В	I	47.38	
	SINGL	SNS5A	35 <b>.</b> 550		1.71	60.791	1.40	0.810	2.68	В	E	47.38	0.940	2.29	В	I	8.91	0.80	0.760	1.71	В	I	47.38	
		SNS6A	39 <b>.</b> 950		1.56	62.322	1.40	0.810	2.44	В	E	47.38	0.940	2.07	В	I	8.91	0.80	0.760	1.56	В	I	47.38	
LEGAL LOAD		SNS7B	42.000		1.48	62.160	1.40	0.810	2.33	В	E	47.38	0.940	2.01	В	I	8.91	0.80	0.760	1.48	В	I	47.38	
RATING	LER	TNAGRIT3	33.000		1.89	62.370	1.40	0.810	2.97	В	E	47.38	0.940	2.49	В	I	8.91	0.80	0.760	1.89	В	I	47.38	
	TRAI	TNT4A	33.075		1.90	62.843	1.40	0.810	2.98	В	E	47.38	0.940	2.44	В	I	8.91	0.80	0.760	1.90	В	I	47.38	
	I N	TNT6A	41.600		1.54	64.064	1.40	0.810	2.42	В	E	47.38	0.940	2.11	В	I	8.91	0.80	0.760	1.54	В	I	47.38	
	R SE	ΤΝΤ7Α	42.000		1.54	64.680	1.40	0.810	2.42	В	E	47.38	0.940	2.08	В	I	8.91	0.80	0.760	1.54	В	I	47.38	
	CTOR (TT)	TNT7B	42.000		1.58	66.360	1.40	0.810	2.48	В	E	47.38	0.940	1.97	В	I	8.91	0.80	0.760	1.58	В	I	47.38	┞──
	TRA	TNAGRIT4	43.000		1.51	64.930	1.40	0.810	2.38	В	E	47.38	0.940	1.91	В	I	8.91	0.80	0.760	1.51	В	I	47.38	┞──
	TRUCK	TNAGT5A	45.000		1.43	64.350	1.40	0.810	2.25	В	E	47.38	0.940	1.88	В	I	8.91	0.80	0.760	1.43	В	I	47.38	<b> </b>
	TR	TNAGT5B	45.000	3	1.42	63.900	1.40	0.810	2.23	В	E	47.38	0.940	1.81	В	I	8.91	0.80	0.760	1.42	В	E	47.38	1



## LRFR SUMMARY

JOB			
Н			
KCI	DESIGN ENGINEER OF RECOR	Deusigned by: DATE	:
	R	Rr.	7/1/2021
	ASSEMBLED BY : A.K.ALLAN CHECKED BY : R.C.LARSON		03/05/18 03/05/18
	DRAWN BY : MAA 1/08 CHECKED BY : GM/DI 2/08	REV. II/I2/08RR REV. I0/I/II REV. I2/I7	MAA/GM MAA/GM MAA/THC

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251801945.13

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END BENT 2

## LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{\text{DC}}$	$\gamma_{D\mathbf{W}}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

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### NOTES:

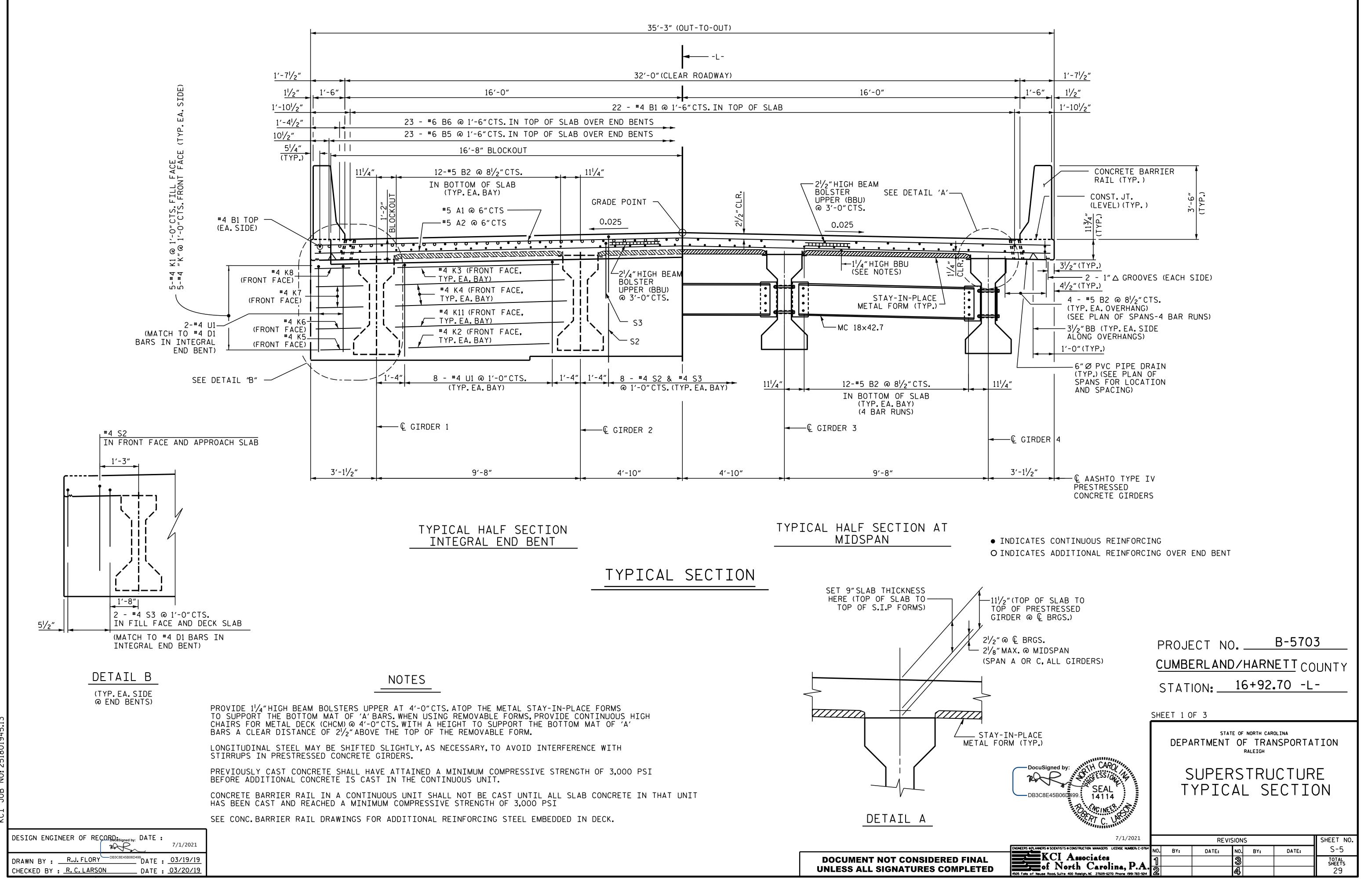
MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

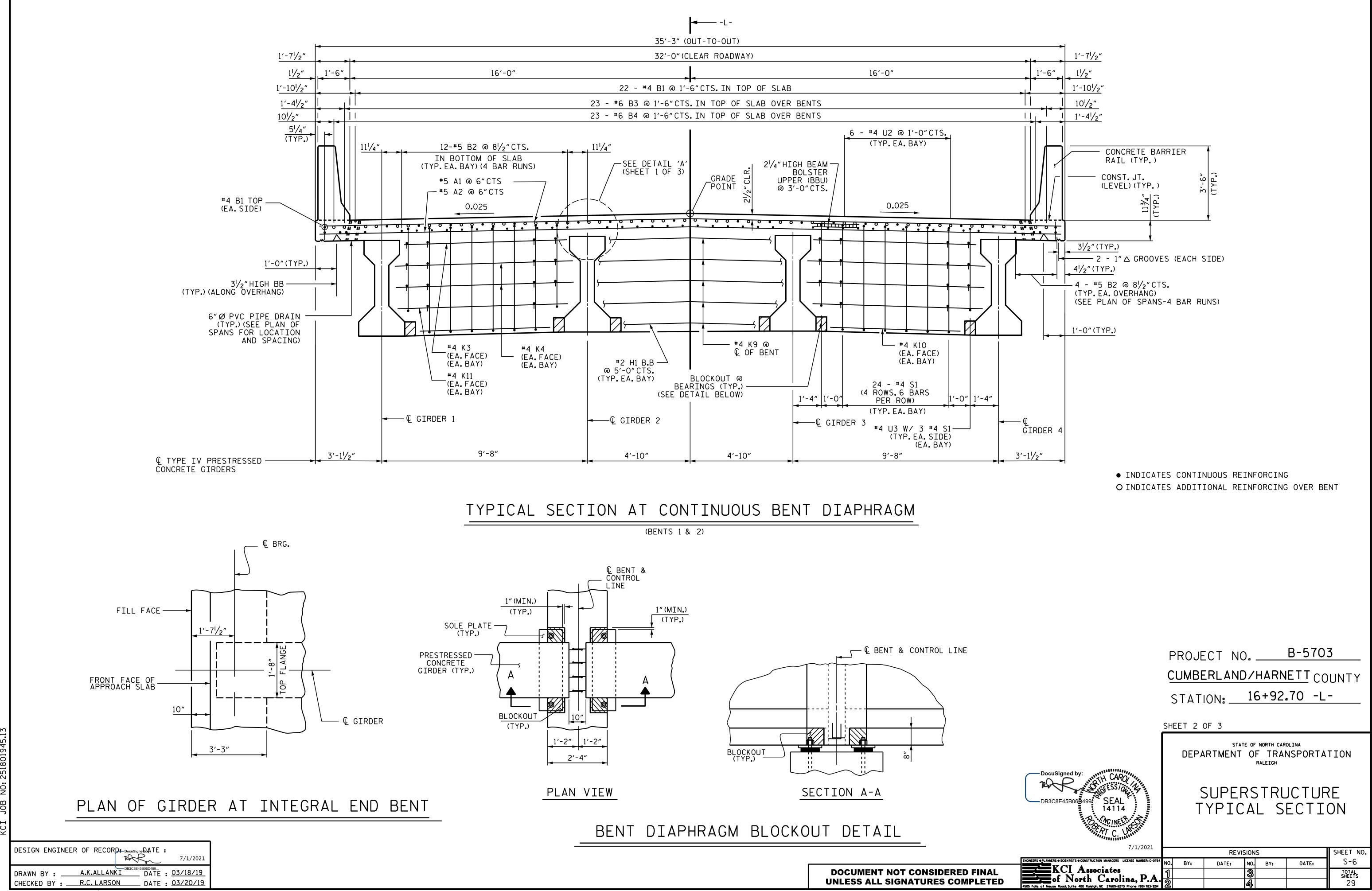
### COMMENTS:

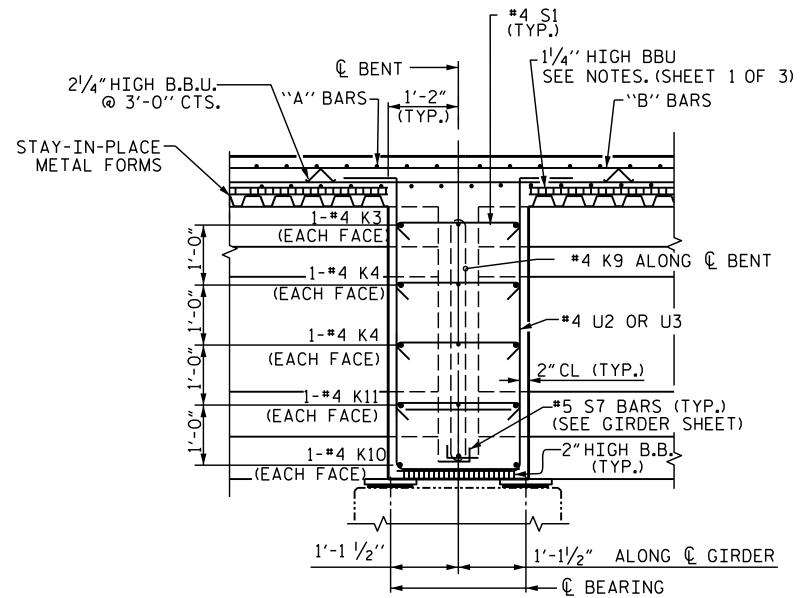
- 2.
- 3.
- 4.

(#) CONTROLLING LOAD RATING									
1 DESIGN LOAD RATING (HL-93)									
2 DESIGN LOAD RATING (HS-20)									
<pre>3 LEGAL LOAD RATING **</pre>									
** SEE CHART FOR VEHICLE TYPE									
GIRDER LOCATION									
I - INTERIOR GIRDER E - EXTERIOR GIRDER									

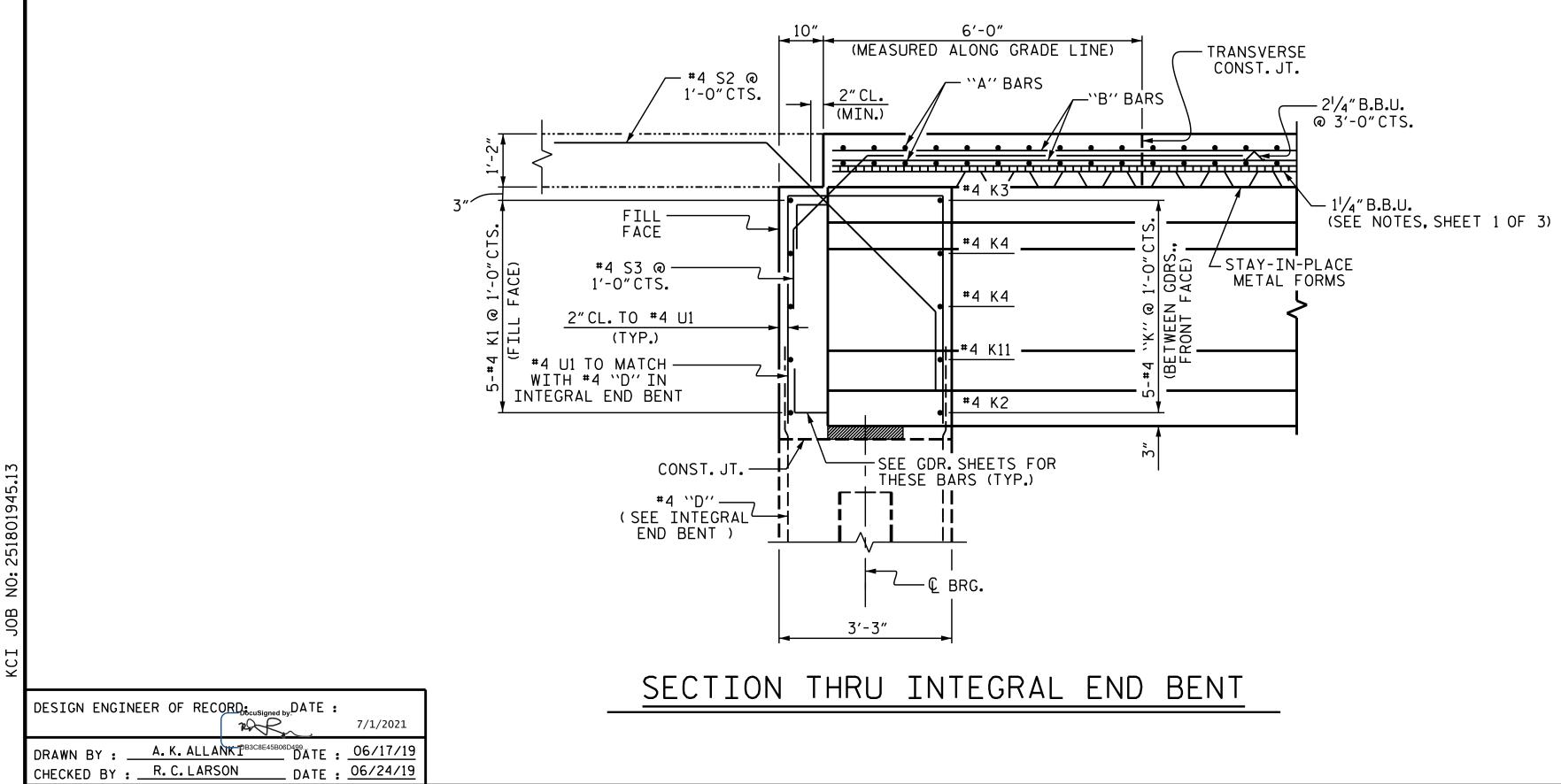
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DocuSigned by: DocuSigned by: DB3C8E45B0@D49914114 DB3C8E45B0@D49914114	LR CC	RTMENT S FR S PRE NCRE	TANDAF	NSPORTA RD RY F SSED IRDE	OR RS
7/1/2021		REVI	SIONS		SHEET NO.
ENGINEERS OPLANNERS O SCIENTISTS OCONSTRUCTION MANAGERS LICENSE NUMBER: C-0764	№. ВY: <b>1</b>	DATE:	NO. BY:	DATE:	S-4 total sheets
4505 Falls of North Carolina, P.A. 4505 Falls of Neuse Road, Suite 400 Roleign, NC 27609-6270 Phone (1919) 783-9214	2		<u>ଅ</u>		sheets 29
			STD.NO	.LRFR1	

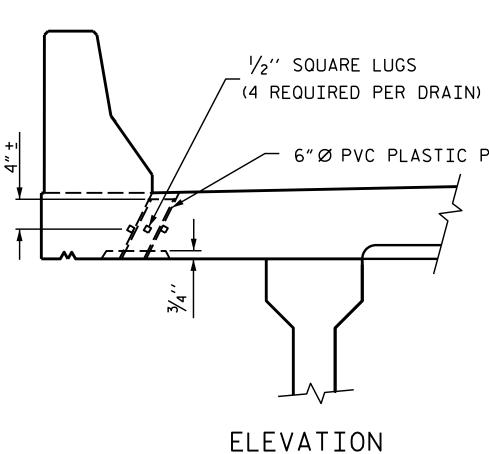


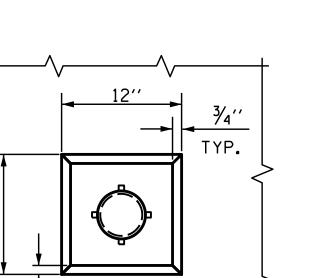




# SECTION THRU BENT DIAPHRAGM







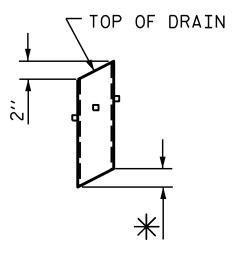
<u>3/</u>4". ΤΥΡ.

PLAN OF RECESS

**DOCUMENT NOT CONSIDERED FINAL** UNLESS ALL SIGNATURES COMPLETED

- 6"Ø PVC PLASTIC PIPE





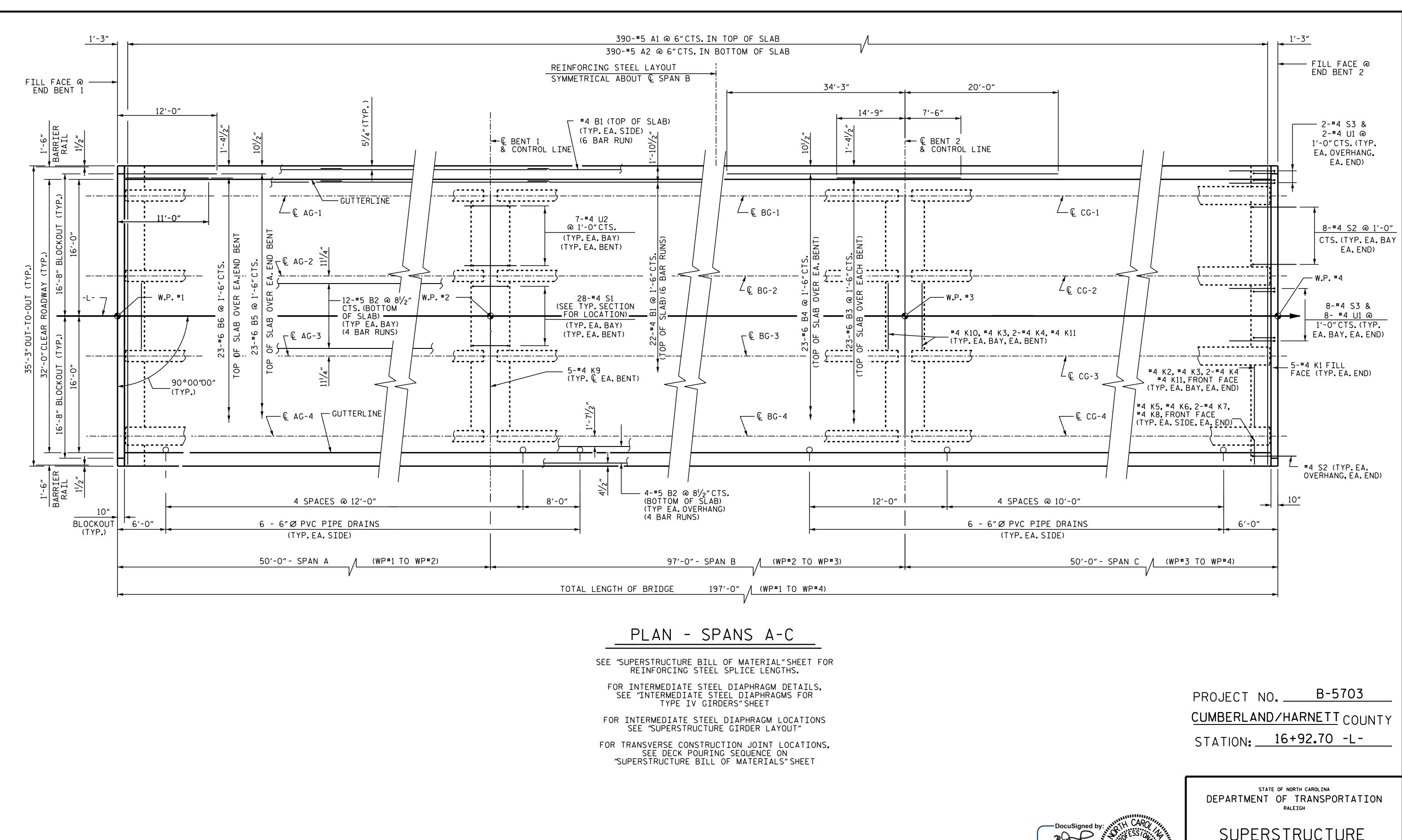
**\*** TO BE SET TO MATCH SLOPE OF BOTTOM OF OVERHANG (24 DRAINS REQUIRED)

### PIPE DETAIL

TOP OF FLOOR DRAINS TO BE SET  $\frac{3}{8}$ " BELOW SURFACE OF SLAB. 4 -  $\frac{1}{2}$ " SQUARE LUGS TO BE GLUED TO THE P.V.C. PLASTIC PIPE AT EQUAL SPACES AROUND THE PIPE DRAIN APPROXIMATELY 4" FROM THE TOP OF THE PIPE. THE 6" Ø PVC PLASTIC PIPE AND FITTINGS SHALL BE SCHEDULE 40 AND CONFORM TO ASTM D1785.

# DRAIN DETAILS

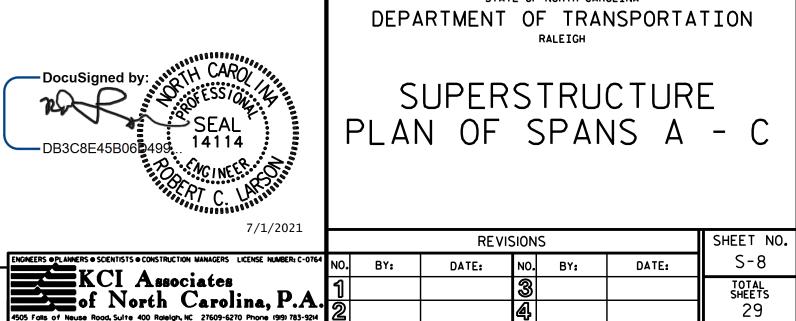
	CUMBE	ECT NO E <b>rland</b> ION:	/HARN	<u>ETT</u> CO	UNTY	
S	HEET 3 O	F 3				
DocuSigned by:	DEPA	STATE NRTMENT	OF NORTH CAR OF TRAN RALEIGH		TION	
DB3C8E45B0CD499 SEAL	SUPERSTRUCTURE TYPICAL SECTION					
7/1/2021						
T / L / 2021		REVIS	-	0475	SHEET NO. S-7	
KCI Associates	NO. ВҮ: <b>1</b>		NO. BY:	DATE:	TOTAL SHEETS	
4505 Folls of Neuse Road, Suite 400 Rateign, NC 27609-6270 Phone (99) 783-9214	2		<u> </u>		sheets 29	

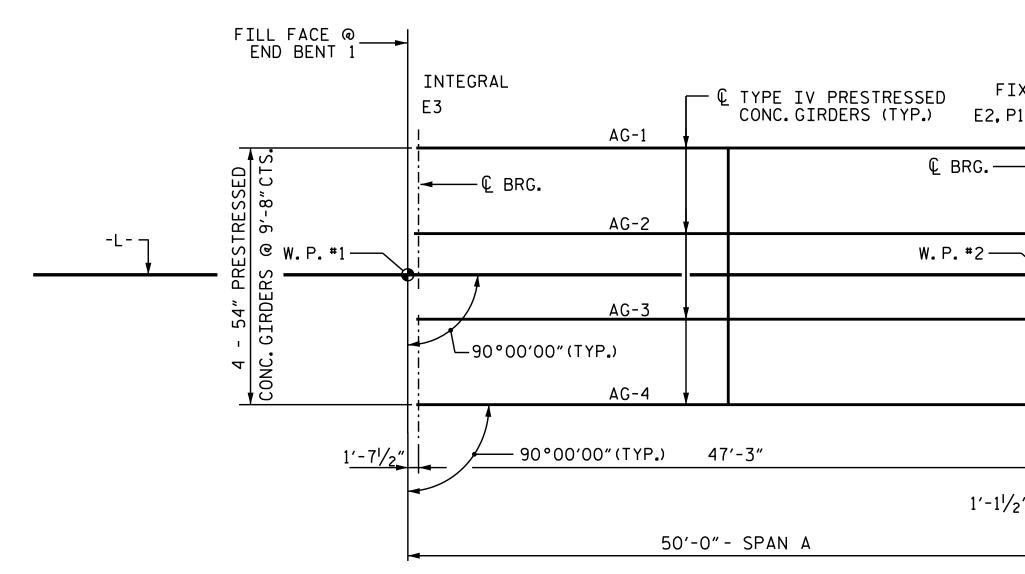


σ 801 251 -UN OB KCI

r

DESIGN ENGINEER OF RECO	RD&uSigned by: DATE:
DRAWN BY :R. J. FLORY	DB3C8E45B06D499 DATE : _09/10/20
CHECKED BY :R.C.LARSO	DATE : 09/11/20





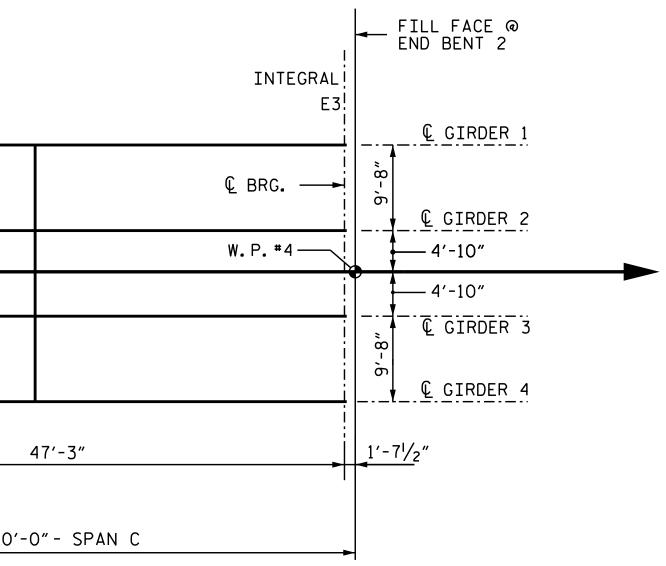


DESIGN ENGINEER OF REC		:	7/1/2021
DRAWN BY : A. SAMBOY		:	03/11/19
CHECKED BY :R.C.LARS	ON DATE	:	09/11/20

FIX I FIX	
P1     E4, P2     E2, P3     E2, P3     E2, P3	CG-1
↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓	
BG-2	CG-2
W. P. #3	
BG-3	CG-3
جـــــــــــــــــــــــــــــــــــ	
BG-4	CG-4
94'-9"	
1'-1'/2''	
97'-0"- SPAN B	50

# GIRDER LAYOUT - SPANS A, B, & C

FOR INTERMEDIATE STEEL DIAPHRAGM DETAILS SEE "INTERMEDIATE STEEL DIAPHRAGM FOR TYPE IV GIRDERS"



PROJECT NO. <u>B-5703</u> <u>CUMBERLAND/HARNETT COUNTY</u> STATION: <u>16+92.70 -L-</u>

 DocuSigned by
 State of NORTH CAROLINA

 DocuSigned by
 SEAL

 DB3C8E45B00E
 SEAL

 14114

 DB3C8E45B00E

 7/1/2021

 T/1/2021

 REVISIONS

 SHEET NO.

 Y1/2021

 REVISIONS

 SHEET NO.

 Y1/2021

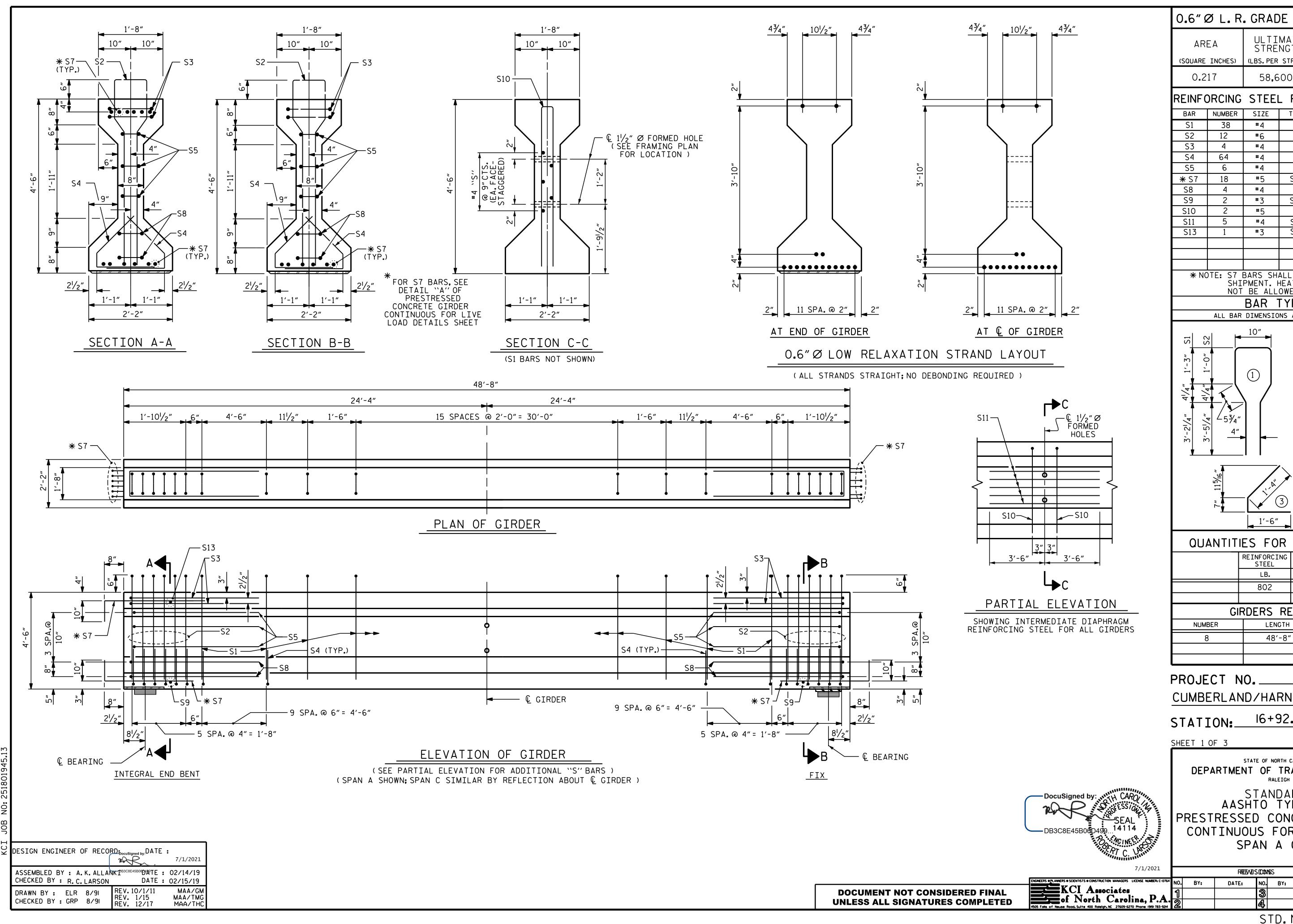
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QUARE	INCHES)	(LBS. PER	STRAND)	(LBS.PER STRAND)				
0.2	17	58,60	00		43,9	950		
		STEEL				IRDER		
bar S1	NUMBER	SIZE #4	TYPE 1			WEIGHT		
S2	38 12	#6	1	10'- 10'-		<u>271</u> 192		
S3	4	#4	2	<u> </u>		24		
S4	64	#4		<u> </u>		146		
S5	6	#4	3 2	8'-	5″	34		
€ S7	18	<b>#</b> 5	STR	3'-	8″	69		
S8	4	#4	2	8'-		23		
S9	2	#3	STR	1'-1		1		
S10	2	#5	2	<u>8'-</u>		18		
S11 S13	5 1	#4 #3	<u>STR</u> STR	7'- 1'-		23		
212	1		311		-	1		
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m ř	<u> </u>			2	=	=		
<u> </u>	•				4'-2"	4'-0"		
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		1.4"			0	•		
	₹		3)		S10	S8 S8		
	' <u>▼</u>		_			S3, S5, & S8		
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		REINFORCIN STEEL	IG 5000 CONC	PSI RETE		″ØL.R. TRANDS		
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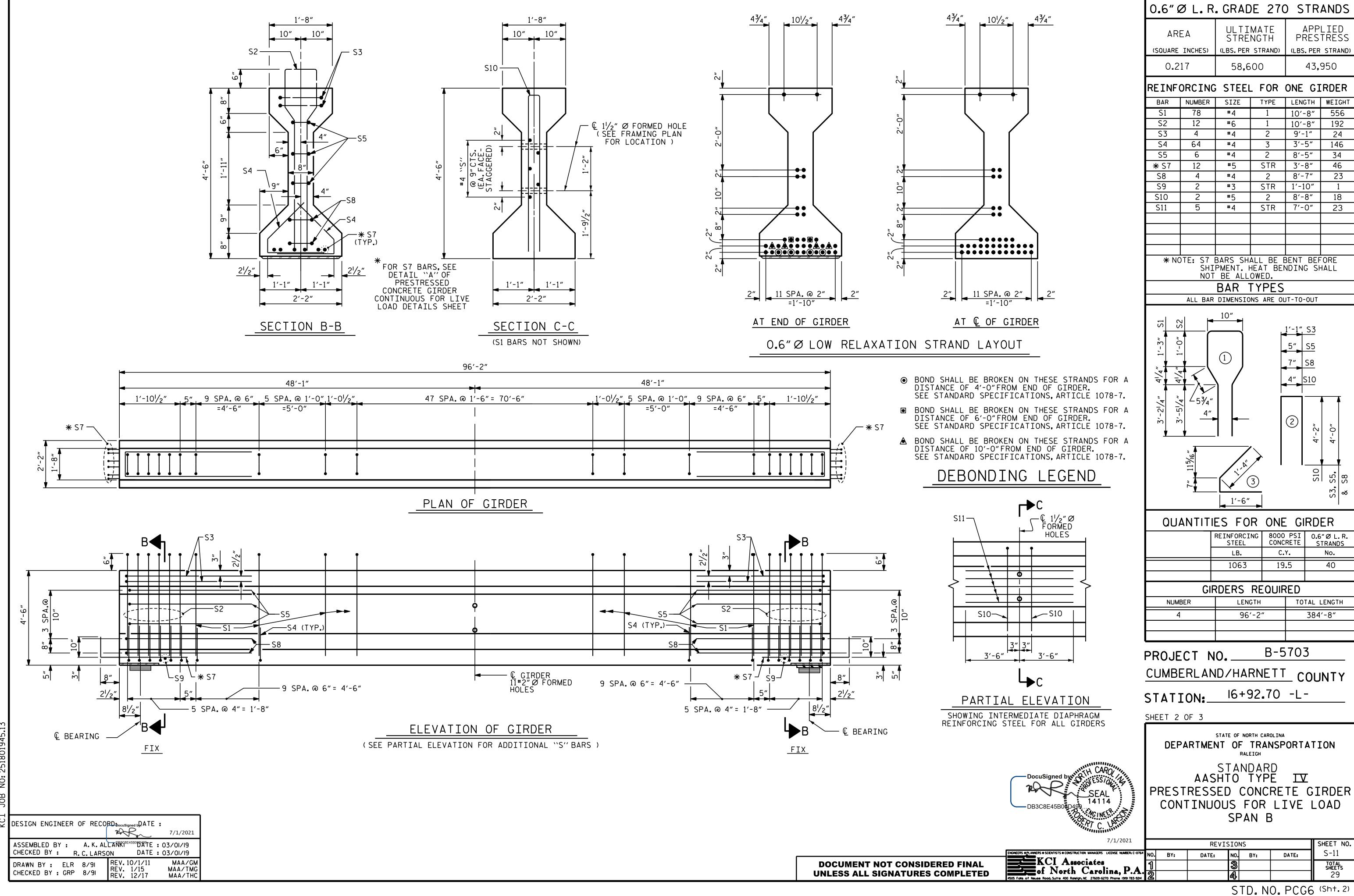
DATE:

SHEET NO.

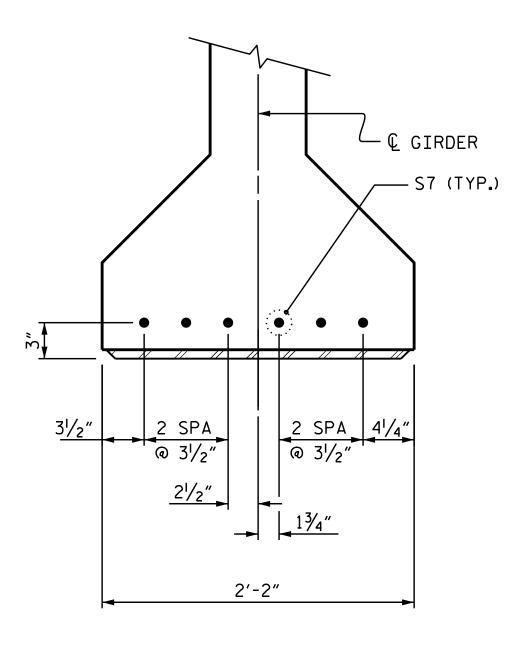
TOTAL SHEETS

29

S-10



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DETAIL ``A" (FOR AASHTO TYPE IV GIRDERS)

				DEA	D LO	AD [	DEFL	ECTI	ON 1	ABLI	E FO	R GI	RDEF	<u> </u>								
0.6″Ø LOW RELAXATION								SF	PAN A	OR C	(INTE	RIOR	OR E	XTERI	OR))							
TWENTIETH POINTS		0	.05	.10	<b>.</b> 15	.20	.25	.30	.35	.40	.45	<b>.</b> 50	<b>.</b> 55	.60	.65	.70	.75	.80	.85	.90	.95	1.00
CAMBER (GIRDER ALONE IN PLACE)	•	0.000	0.007	0.013	0.019	0.024	0.029	0.034	0.037	0.039	0.041	0.041	0.041	0.039	0.037	0.034	0.029	0.024	0.019	0.013	0.007	0
* DEFLECTION DUE TO SUPERIMPOSED D.L.	♦	0.000	0.002	0.003	0.005	0.007	0.008	0.009	0.010	0.011	0.011	0.012	0.011	0.011	0.010	0.009	0.008	0.007	0.005	0.003	0.002	0
FINAL CAMBER	ŧ	0	1/16″	1/8″	<sup>3</sup> /16″	<sup>3</sup> /16″	1/4″	5/16″	5/16″	5/16″	<sup>3</sup> ⁄8″	<sup>3</sup> ⁄8″	<sup>3</sup> /8″	5/16″	5/16″	5/16″	1/4″	3/16″	3/16″	<sup>1</sup> /8″	1/16″	0
0.6″Ø LOW RELAXATION							-				SPAN	B (IN	TERIO	ר)	-					-		
TWENTIETH POINTS		0	.05	.10	<b>.</b> 15	.20	.25	.30	.35	.40	.45	<b>.</b> 50	<b>.</b> 55	.60	.65	.70	.75	.80	.85	.90	.95	1.00
CAMBER (GIRDER ALONE IN PLACE)	•	0.000	0.034	0.067	0.097	0.126	0.151	0.172	0.189	0.202	0.209	0.212	0.209	0.202	0.189	0.172	0.151	0.126	0.097	0.067	0.034	0
* DEFLECTION DUE TO SUPERIMPOSED D.L.	♦	0.000	0.025	0.047	0.073	0.093	0.114	0.129	0.143	0.152	0.158	0.160	0.158	0.152	0.143	0.129	0.114	0.093	0.073	0.047	0.025	0
FINAL CAMBER	ŧ	0	1/8″	<sup> </sup> /4″	5/16″	<sup>3</sup> /8″	7⁄16″	1/2″	<sup>9</sup> /16″	5⁄8″	5⁄8″	5⁄8″	<sup>5</sup> /8″	5⁄8″	9/16″	1/2″	7⁄16″	3⁄8″	5⁄16″	<sup>1</sup> /4″	1/8"	0
0.6″Ø LOW RELAXATION											SPAN	B (E)	KTERI	)R)								
TWENTIETH POINTS		0	.05	.10	.15	.20	.25	.30	.35	.40	.45	<b>.</b> 50	<b>.</b> 55	.60	.65	.70	.75	.80	.85	.90	.95	1.00
CAMBER (GIRDER ALONE IN PLACE)	<b>≜</b>	0.000	0.034	0.067	0.097	0.126	0.151	0.172	0.189	0.202	0.209	0.212	0.209	0.202	0.189	0.172	0.151	0.126	0.097	0.067	0.034	0
* DEFLECTION DUE TO SUPERIMPOSED D.L.	ł	0.000	0.022	0.041	0.064	0.081	0.100	0.113	0.125	0.133	0.138	0.140	0.138	0.133	0.125	0.113	0.100	0.081	0.064	0.041	0.022	0
FINAL CAMBER	•	0	<sup> </sup> /8″	5⁄16″	<sup>3</sup> ⁄8″	°∕i6″	5⁄8″	<sup>11</sup> /16″	<sup>3</sup> ⁄4″	<sup>13</sup> /16″	7⁄8″	<sup>7</sup> ⁄8″	7⁄8″	<sup>13</sup> /16″	3⁄4″	"/16"	5⁄8″	9/16″	3⁄8″	5⁄16″	1/8"	0

\* INCLUDES FUTURE WEARING SURFACE ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM ), EXCEPT ``FINAL CAMBER '', WHICH IS GIVEN IN INCHES (FRACTION FORM ).

KC ]	DESIGN ENGINEER OF RECO	DRD: <sub>DocuSigned</sub> DATE : 7/1/2021
	ASSEMBLED BY : R.C.LAR CHECKED BY : A.SAMBO	SON <sup>DB3C8E450%DTE</sup> : 02/13/19 Y DATE: 03/04/19
	DRAWN BY : ELR 11/91 CHECKED BY : GRP 11/91	REV. 1/15 MAA/TMG REV. 2/15 MAA/TMG REV. 12/17 MAA/THC

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ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE ``B-1'' SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

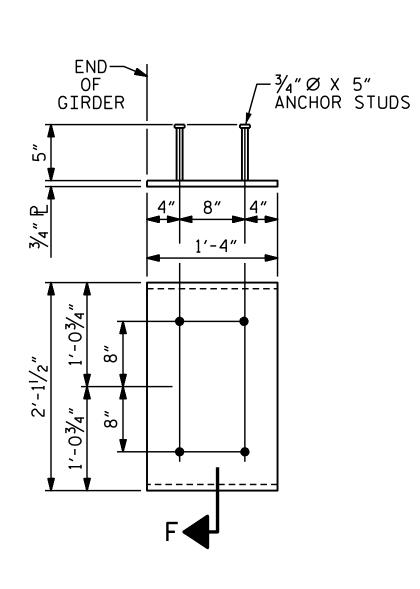
ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE ``B'' REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

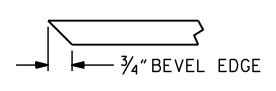
AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2"BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4000 psi FOR SPANS A AND C OR 6000 psiFOR SPAN B.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER. THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A

DEPTH OF 1/4".





SECTION "F" (SEE NOTES)

## EMBEDDED PLATE "B-1" DETAILS FOR AASHTO TYPE IV GIRDER

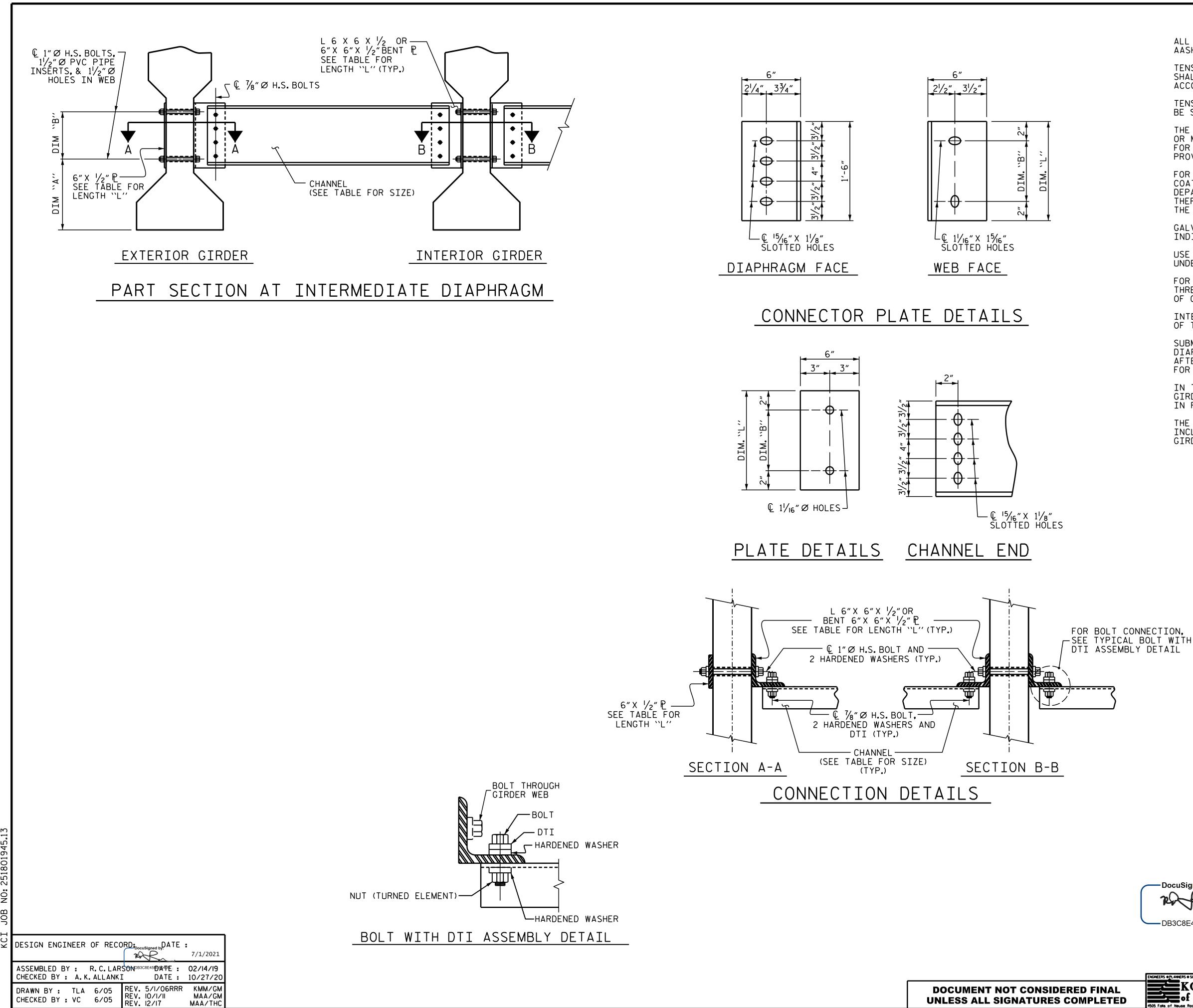
(2 REQ'D PER GIRDER)

# NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

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)	PROJECT NO. <u>B-5703</u> CUMBERLAND/HARNETT COUNTY STATION: <u>16+92.70</u> -L-								
	STATION: 10 SET 3 OF 3								
DocuSigned by DocuSigned by DB3C8E45B06D DB3C8 DC8 DC8 DC8	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD DETAILS								
T / T / ZUZI	REVISIONS SHEET NO. NO. BY: DATE: NO BY: DATE: S-12								
4505 Falls of Neuse Road, Suite 400 Rateign, NC 27609-6270 Phone (99) 783-924	NO.         BY:         DATE:         NO.         BY:         DATE:         S-12           1         3         3         TOTAL SHEETS 29								
	STD. NO. PCG9 (Sht. 3)								



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UNLESS ALL SIGNATURES COMPLETED

## STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

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TENSION ON THE ASTM A325 BOLTS THROUGH THE CHANNEL MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL  $\frac{1}{4}$  TURN.

THE PLATES, BENT PLATES, CHANNELS, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY A THERMAL SPRAYED COATING WITH A SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE DEPARTMENTS THERMAL SPRAYED COATINGS (METALLIZATION) PROGRAM, THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW. COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

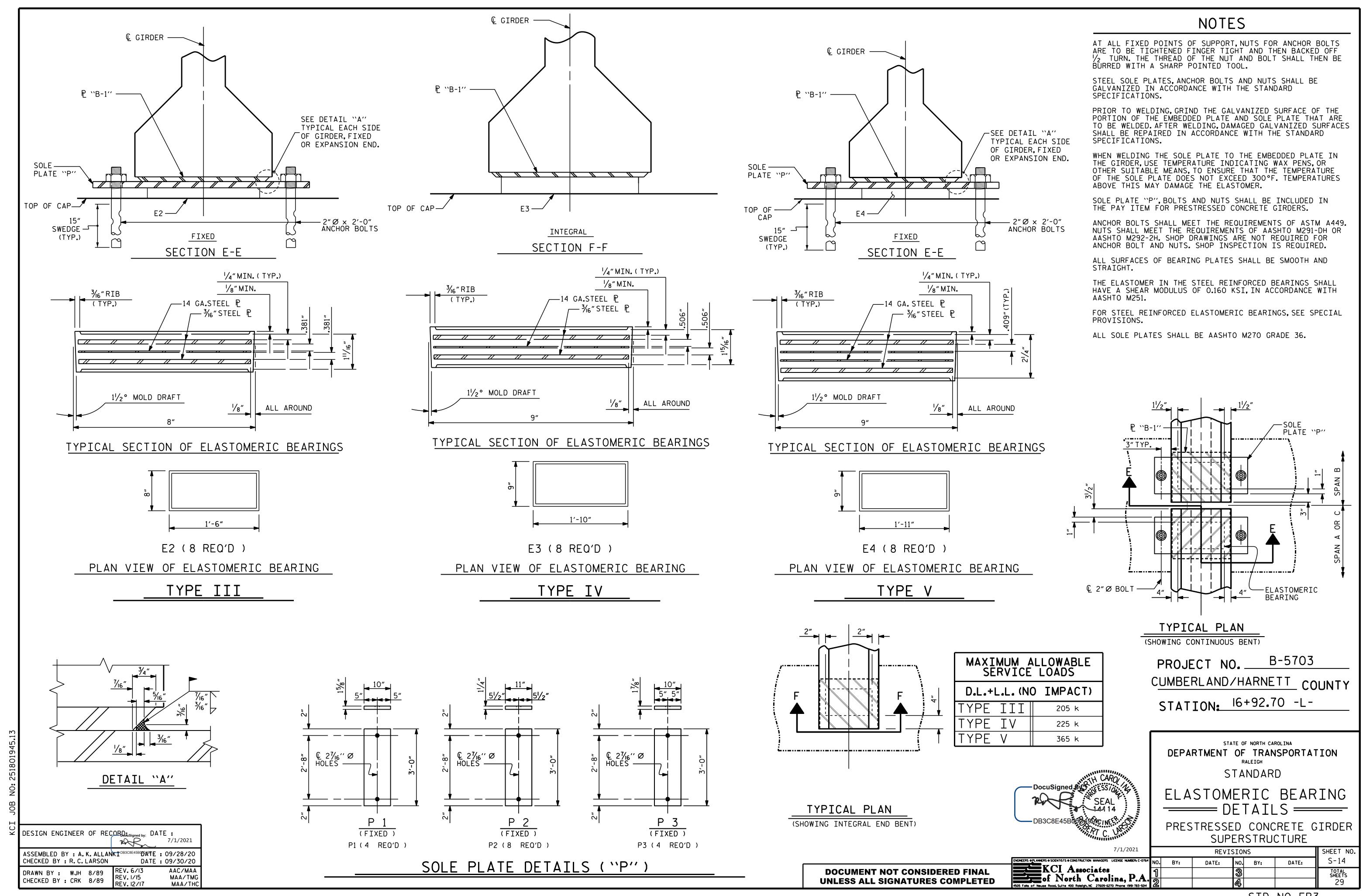
IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

GIRDER TYPE	CHANNEL SIZE	DIM ``A''	DIM ``B''	DIM ``L''
IV	MC 18 × 42.7	1′-9 <mark>'/</mark> 2″	1'-2"	1'-6"

	PROJECT NO. <u>B-5703</u> <u>CUMBERLAND/HARNETT</u> COUNTY STATION: <u>I6+92.70</u> -L-
DocuSigned by: WTH CARO BODIESSION A SEAL DB3C8E45B06D499 CINEER DF CONTRACTOR	DEPARTMENT OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR TYPE IV PRESTRESSED CONCRETE GIRDERS
7/1/2021	REVISIONS SHEET NO.
ENGINEERS OF LANNERS O SCIENTISTS O CONSTRUCTION MANAGERS LICENSE NUMBERI C-0764 KCI Associates of North Carolina, P.A. 4505 Fails of Neuse Road, Suite 400 Roleign, NC 27609-6270 Phone (919) 783-9214	NO.         BY:         DATE:         NO.         BY:         DATE:         S-13           1         3         3         TOTAL SHEETS 29
	STD.NO.PCG10 (SHT 3)

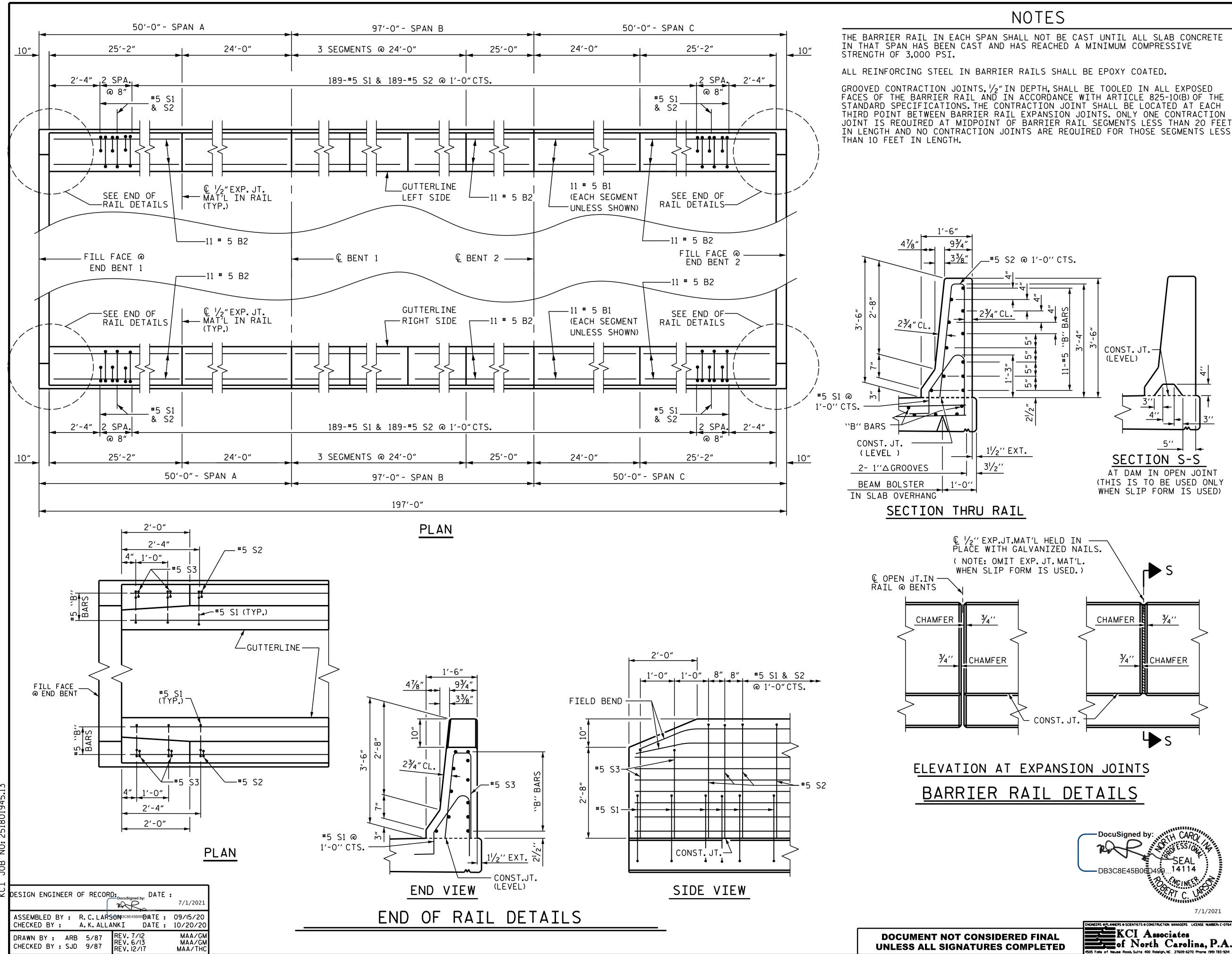
## TABLE

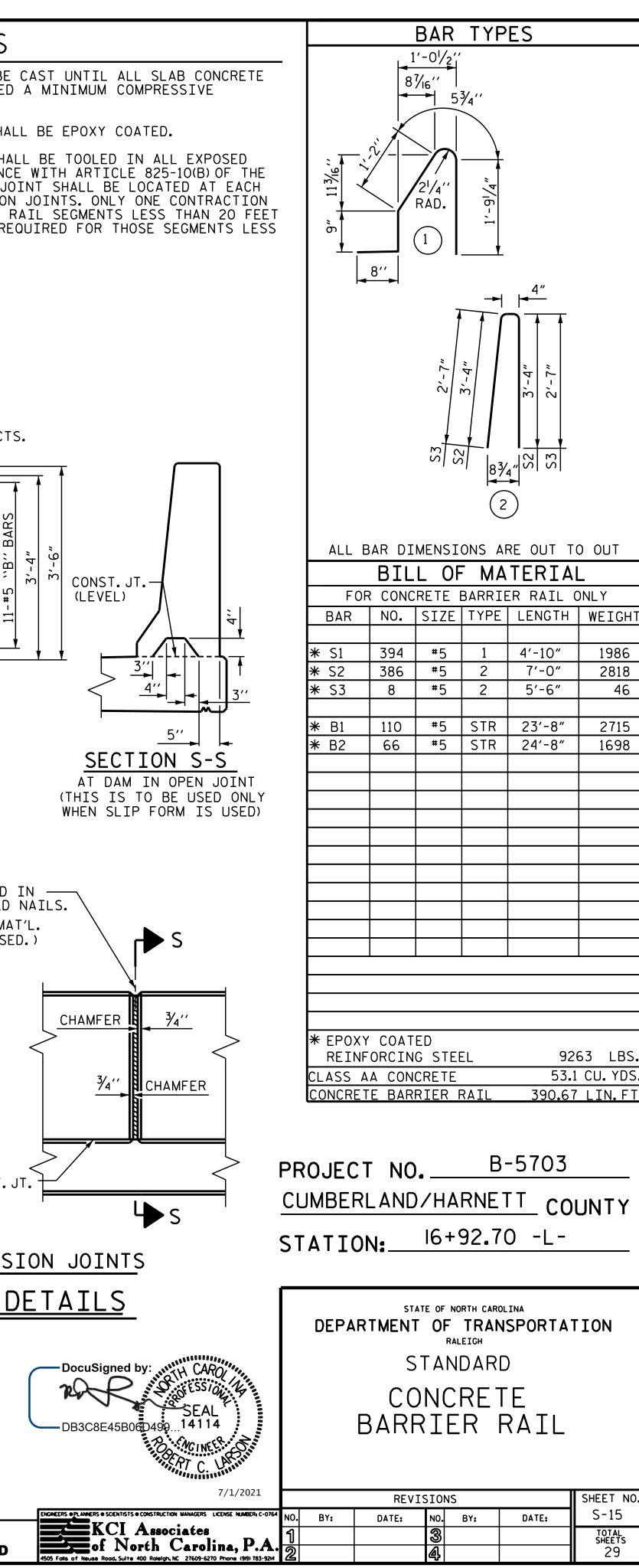


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STD. NO. EB3





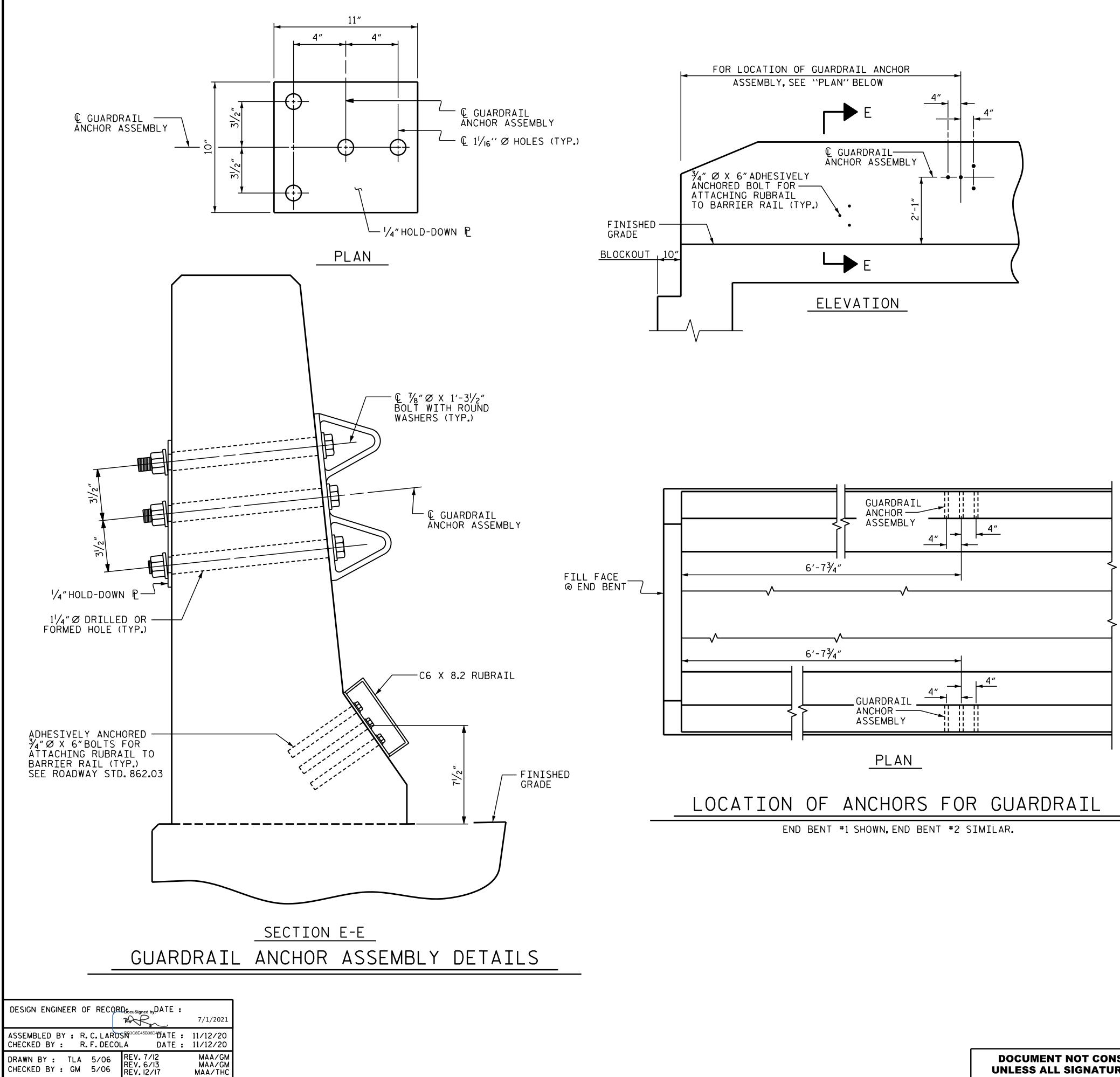
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total sheets 29

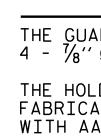
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CHECKED BY : GM 5/06



THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE ¾″∅ X 6″BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE  $\frac{3}{4}$ " Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.

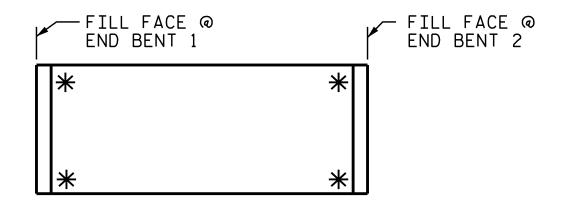
## NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $\frac{1}{4}$ " HOLD-DOWN PLATE AND 4 -  $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY

THE  $1 \frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENTS \* DENOTES GUARDRAIL ANCHOR ASSEMBLY

> B-5703 PROJECT NO. CUMBERLAND/HARNETT COUNTY

STATION: 16+92.70 -L-

	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH							
TH CARO		STANDARD						
DocuSigned by: SEAL 14114 DB3C8E45B06D4999 AT C AT	GUARDRAIL ANCHORAGE FOR BARRIER RAIL							
7/1/2021		REVIS	SIONS		SHEET NO.			
ENGINEERS OPLANNERS OSCIENTISTS CONSTRUCTION MANAGERS LICENSE NUMBER: C-0764	NO. BY:	DATE:	NO. BY:	DATE:	S- 16			
4505 Folls of Neuse Road, Suite 400 Roleigr, NC 27609-6270 Prone (919) 783-924	1 2		3 4		total sheets 29			
	(SHT	1)	STD. NO	O.GRA2				

		€ BENT : CONTROL LI	1 &	ر Cont	BENT 2 & FROL LINE
FILL FACE @ END BENT 1 		ANSVERSE ST.JOINT		TRANSVERSE CON JO	NST.
-L-	4	1		<b>←</b> 2)	
	 6′-10″ 	37'-0"	6'-2″	86'-0″	

# DECK POURING SEQUENCE

-(2) - INDICATES POUR SEQUENCE AND DIRECTION

			€ BENT 1 & _ CONTROL LINE			€ BENT 2 & _ CONTROL LINE	
FILL FAC @ END BENT	CE 1►		< TRANSVERSE► CONST.JOINT (TYP.)				
-L		2	1	2		1	
		6′-10	″ 39′-2″	8'-0"	-	89'-0"	

## OPTIONAL DECK POURING SEQUENCE

-(2) - INDICATES POUR SEQUENCE AND DIRECTION

NO POUR 2 MAY BE STARTED UNTIL BOTH ADJACENT POURS 1 HAVE REACHED A MINIMUM STRENGTH OF 3000 PSI.

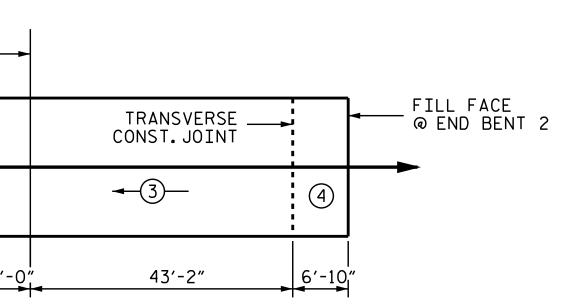
SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS							
BAR SIZE	SUPERST EXCEPT A SLABS, P AND BARR	APPROACH ARAPET,	APPROAC	PARAPET AND BARRIER			
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL		
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"		
#5	2'-6"	2'-2"	2'-6"	2'-2"	3′-5″		
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"		
#7	5'-3"	3'-6"					
#8	6'-10"	4'-7"					

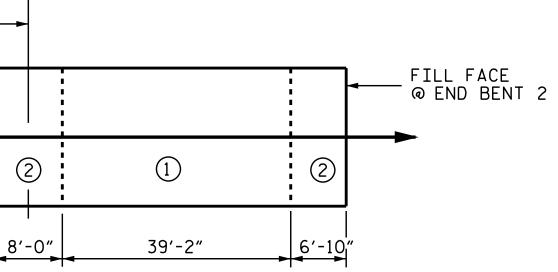
FILL FACE @ END BENT 1

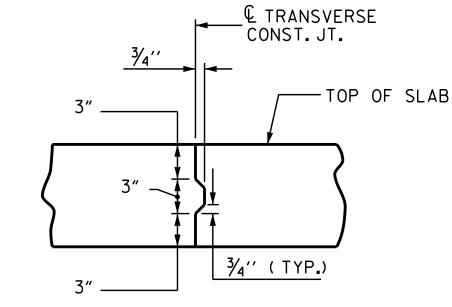
ASSEMBLED BY :		cuSigned by:	DATE :	7/1/2021
ASSEMBLED BY : CHECKED BY :	A.K.AL® R.C.LAR		DATE : DATE :	03/26/19 09/11/20
DRAWN BY : JMB CHECKED BY : SJD		REV.5/ REV.10 REV.12	/1/11	TLA/GM MAA/GM MAA/THC

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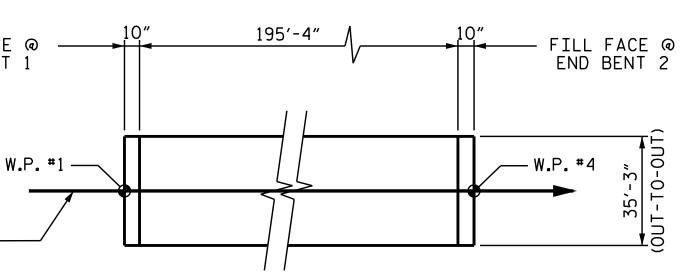






# TRANSVERSE CONSTRUCTION JOINT IN DECK SLAB

NOTE: REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THRU JOINT



LAYOUT FOR COMPUTING AREA REINFORCED CONCRETE DECK SLAB (SQ.FT.=6885)

	B	ILL (	OF M	ATERIA
BAR	NO.	SIZE	TYPE	LENGTH
<b>*</b> A1	390	5	STR.	34'-11"
Α2	390	5	STR.	34'-11"
<b>*</b> B1	144	4	STR.	34'-2″
B2	176	5	STR.	50'-5″
<b>*</b> B3	46	6	STR.	22'-3"
<b>₩</b> B4	46	6	STR.	54'-3″
<b>*</b> B5	46	6	STR.	11'-0"
<b>₩</b> B6	46	6	STR.	10'-0"
K1	10	4	STR.	34'-11"
K2	6	4	STR.	7′-2″
КЗ	18	4	STR.	7′-8″
К4	36	4	STR.	8'-8"
К5	4	4	STR.	1'-8″
К6	4	4	STR.	2'-1"
К7	8	4	STR.	2′-5″
K8	4	4	STR.	1'-11"
K9	10	4	STR.	29'-4"
K10	12	4	STR.	6'-2"
K11	18	4	STR.	7'-11″
S1	180	4	1	2′-9″
<b>*</b> S2	52	4	2	10'-3"
<b>*</b> S3	56	4	2	11'-11"
U1	56	4	3	11'-3″
U2	36	4	4	13′-10″
U3	12	4	4	11'-10″

REINFORCING STEEL

EPOXY COATED REINFORCING STEEL 25,028

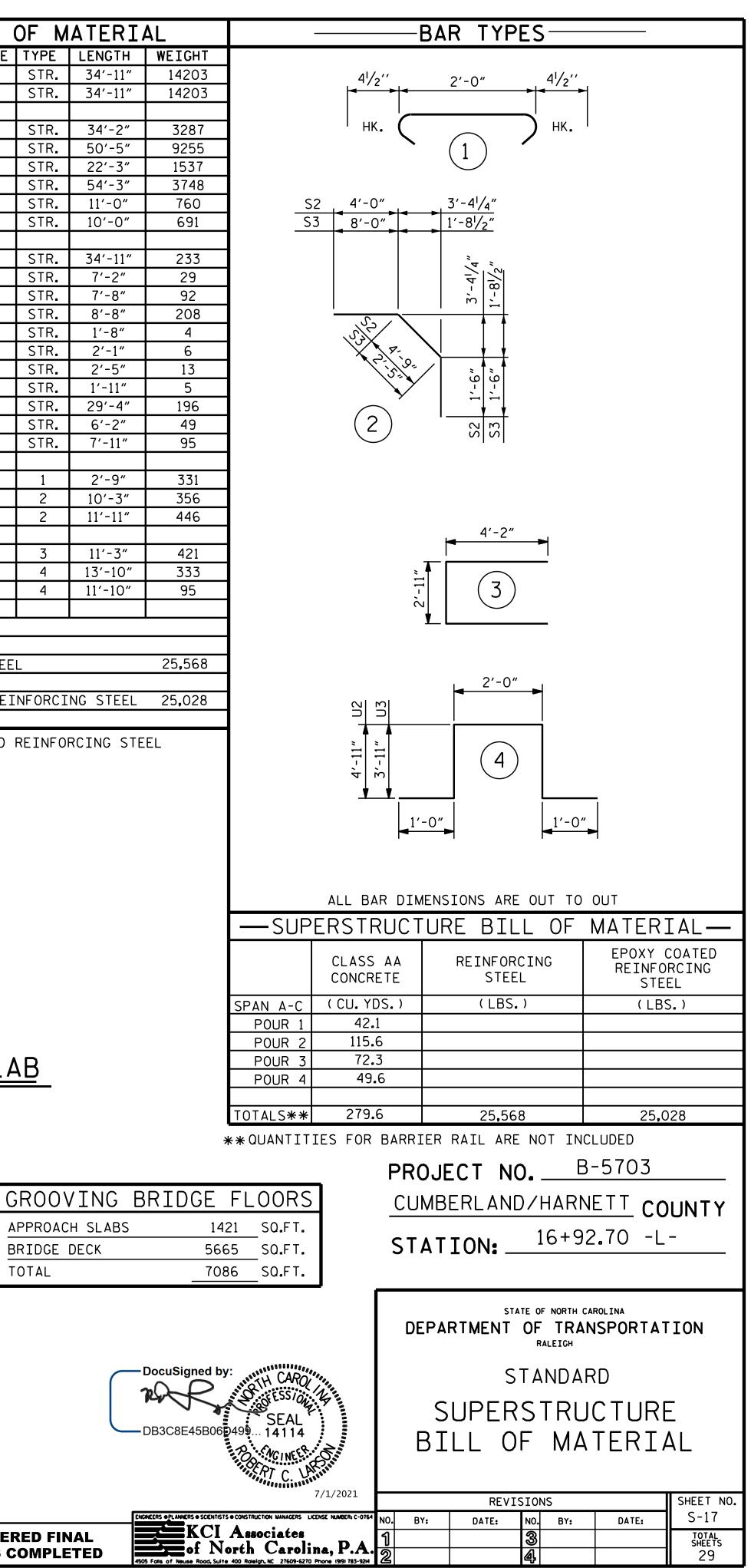
\* EPOXY COATED REINFORCING STEEL

APPROACH SLABS

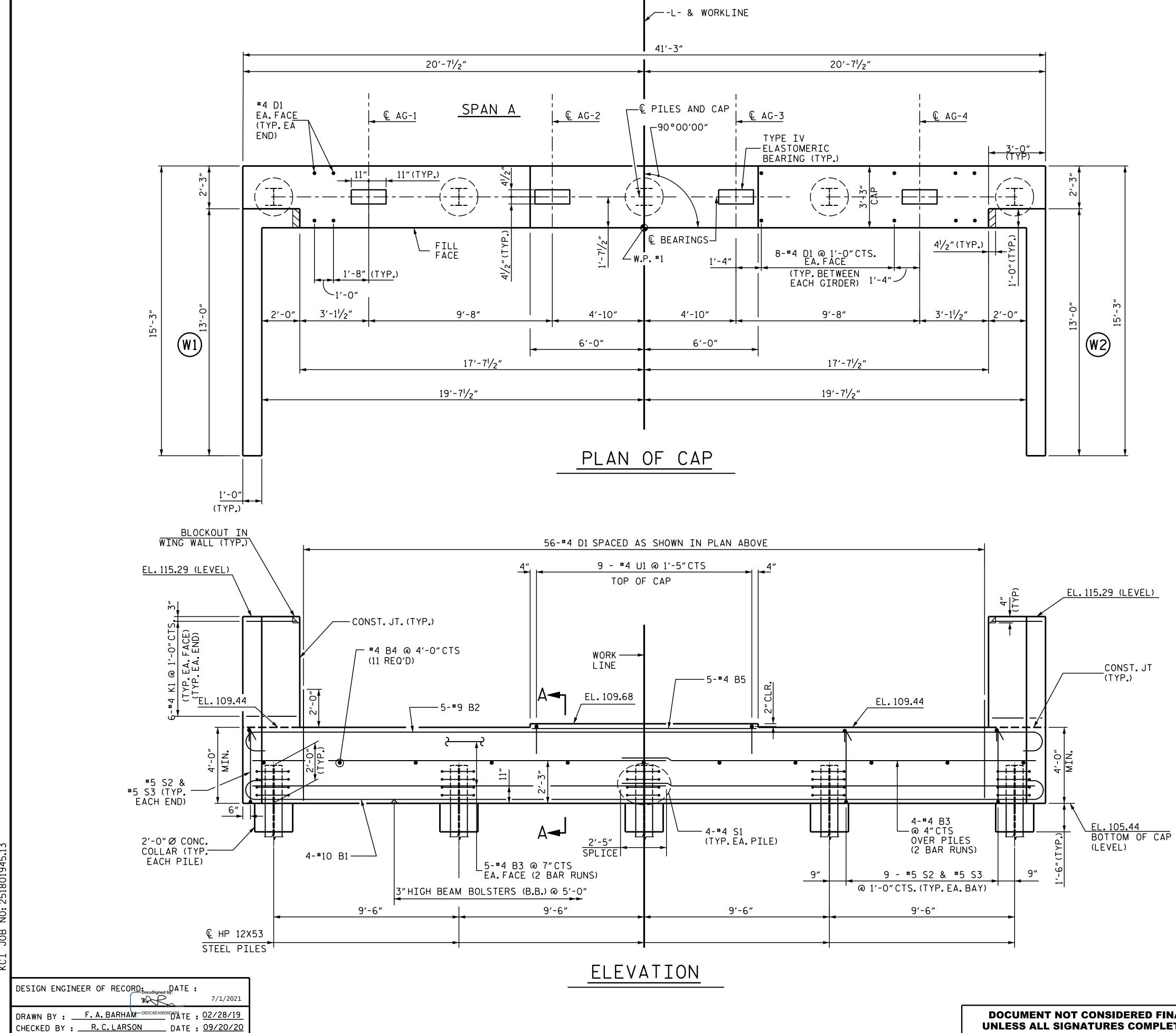
BRIDGE DECK

TOTAL

DOCUMENT NOT CONSIDERED FINAL **UNLESS ALL SIGNATURES COMPLETED** 



STD. NO. BOM2





## NOTES

THE TOP SURFACE OF THE END BENT CAP AND WINGS (POUR 1) EXCEPT THE BEARING AREA AND THE AREA OUTSIDE OF THE SUPERSTRUCTURE SHALL BE RAKED TO A DEPTH OF  $\frac{1}{4}$ "

FOR "TEMPORARY DRAINAGE AT END BENT", SEE END BENT 2.

FOR SECTION A-A SEE SHEET 3 OF 3.

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #4 D1 BARS.

THE UPPER PORTION OF THE INTEGRAL END BENT SHALL BE POURED WITH THE SUPERSTRUCTURE. SEE SUPERSTRUCTURE PLANS.

### B-5703 PROJECT NO.\_\_\_\_ CUMBERLAND/HARNETT COUNTY

STATION: <u>16+92.70</u> -L-

SHEET 1 OF 3

TH CARO

14114

SNGINEER

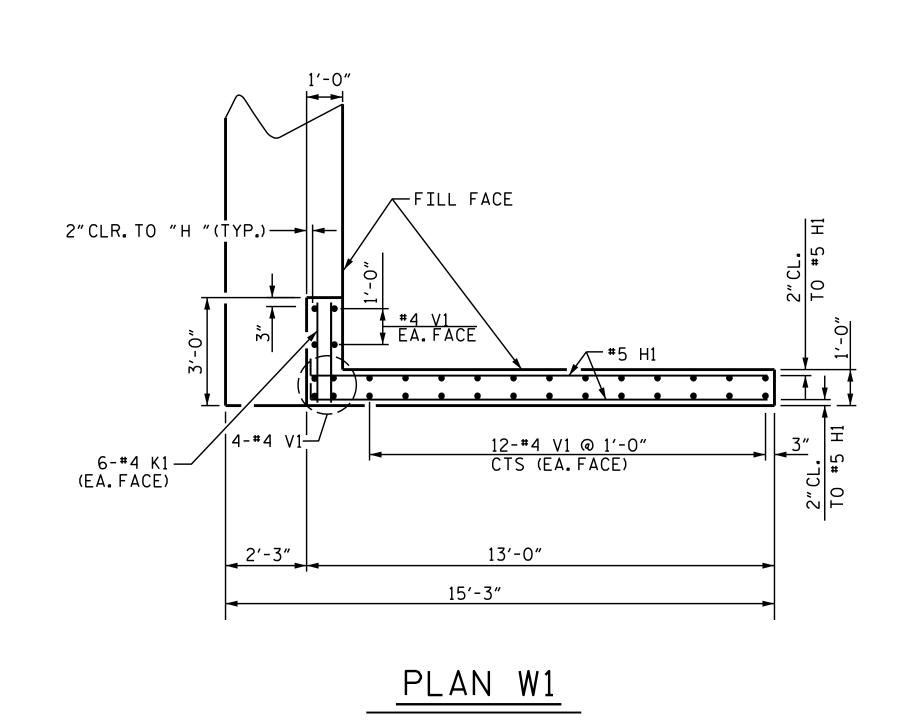
DocuSigned by

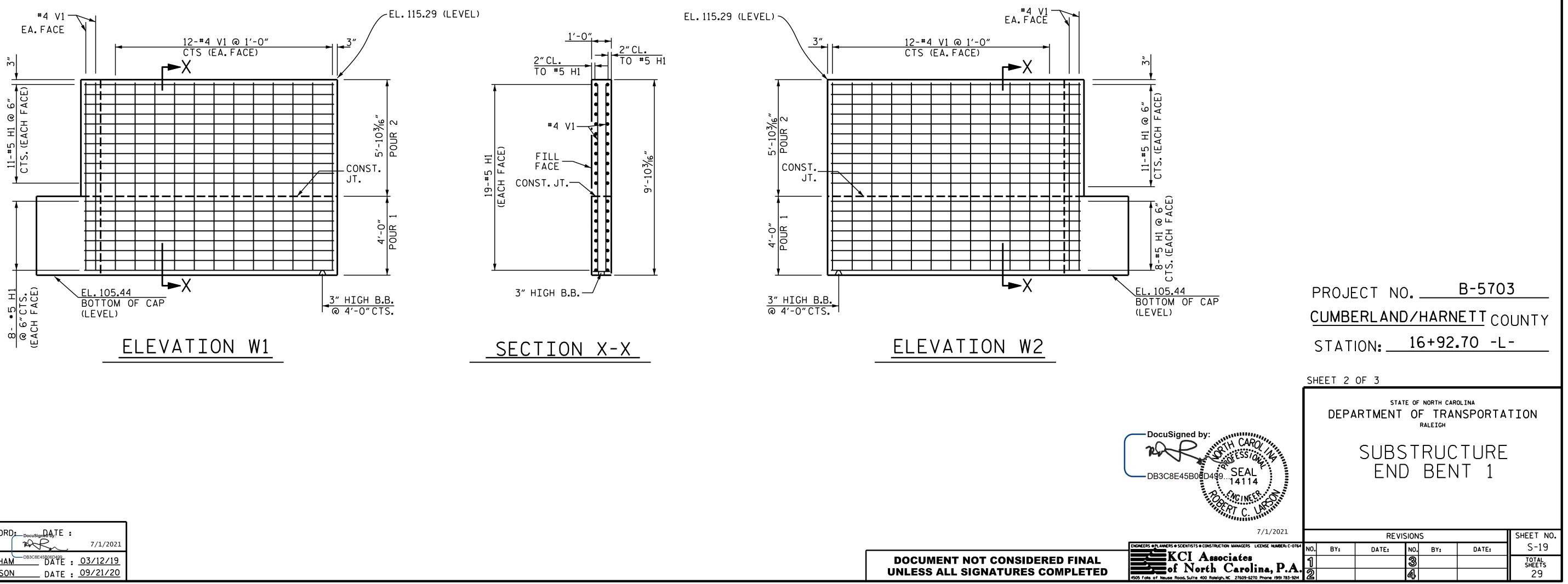
DB3C8E45B0

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

# SUBSTRUCTURE END BENT 1 (INTEGRAL)

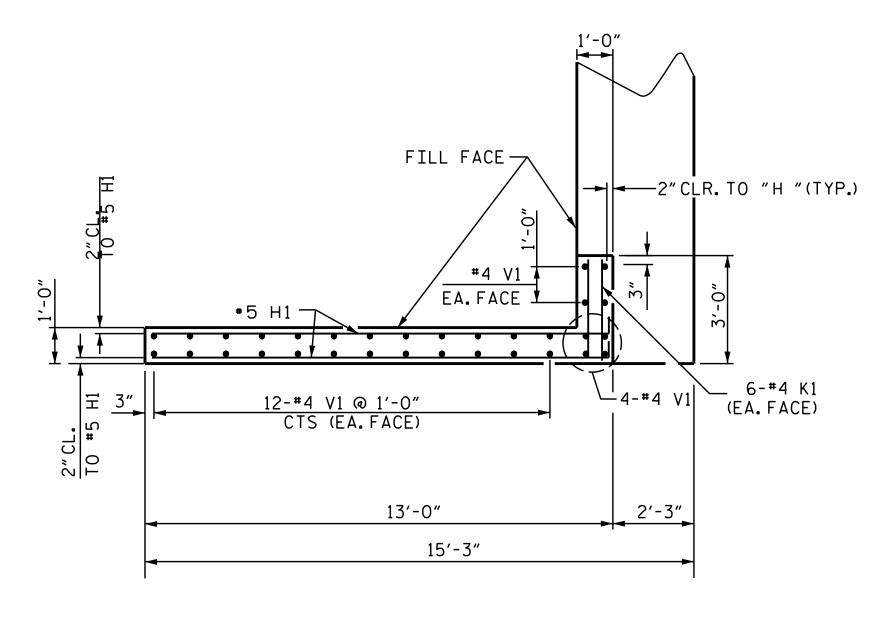
7/1/2021	REVISIONS					SHEET NO.	
ENGINEERS OPLANNERS O SCIENTISTS O CONSTRUCTION MANAGERS LICENSE NUMBER: C-0764	NO.	BY:	DATE:	NO.	BY:	DATE:	S-18
KCI Associates	1			3			TOTAL SHEETS
4505 Folls of Neuse Road, Suite 400 Roleign, NC 27609-6270 Phone 1919) 783-9214	2			4			29



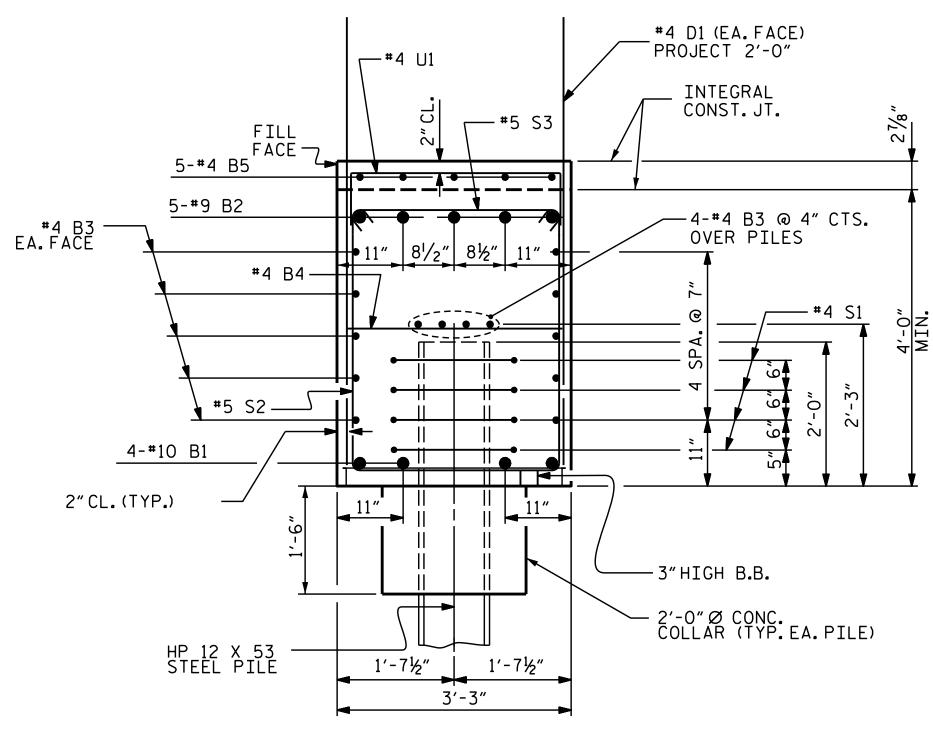


KCI

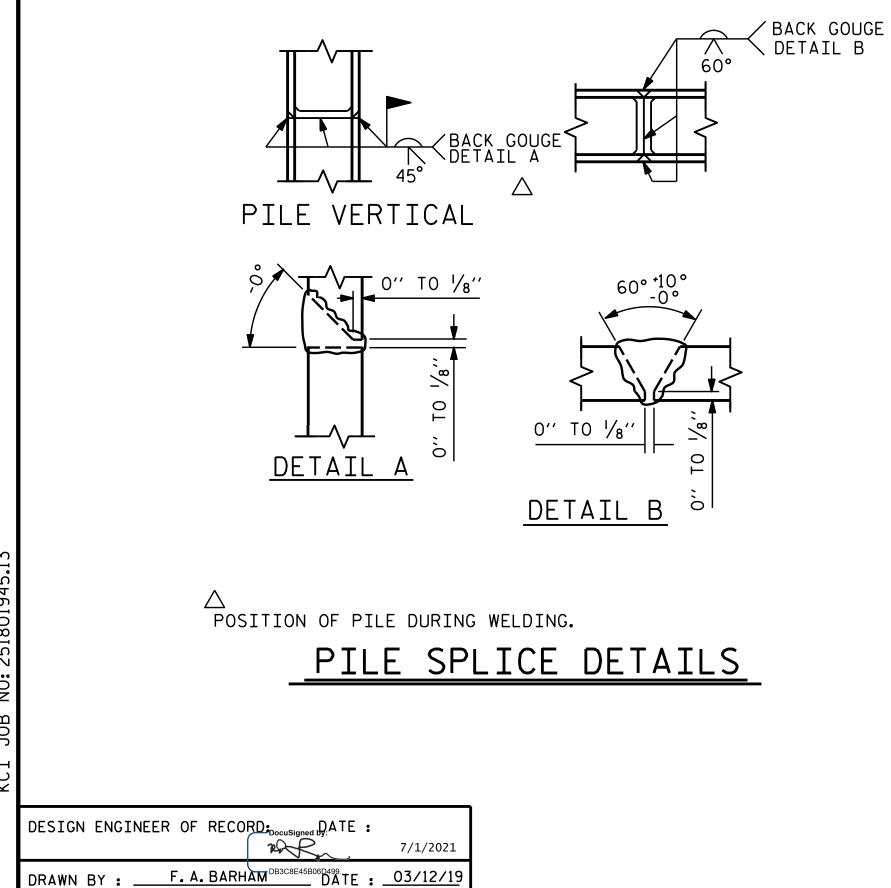
DESIGN ENGINEER OF RECORD	$D_{\text{LocuSign}} = D_{\text{LocuSign}} = D_{LocuS$
	<b>1</b> /1/2021
DRAWN BY :F.A.BARHAN	DB3C8E45B06D499 DATE : <u>03/12/19</u>
CHECKED BY :R.C.LARSON	N DATE : <u>09/21/20</u>



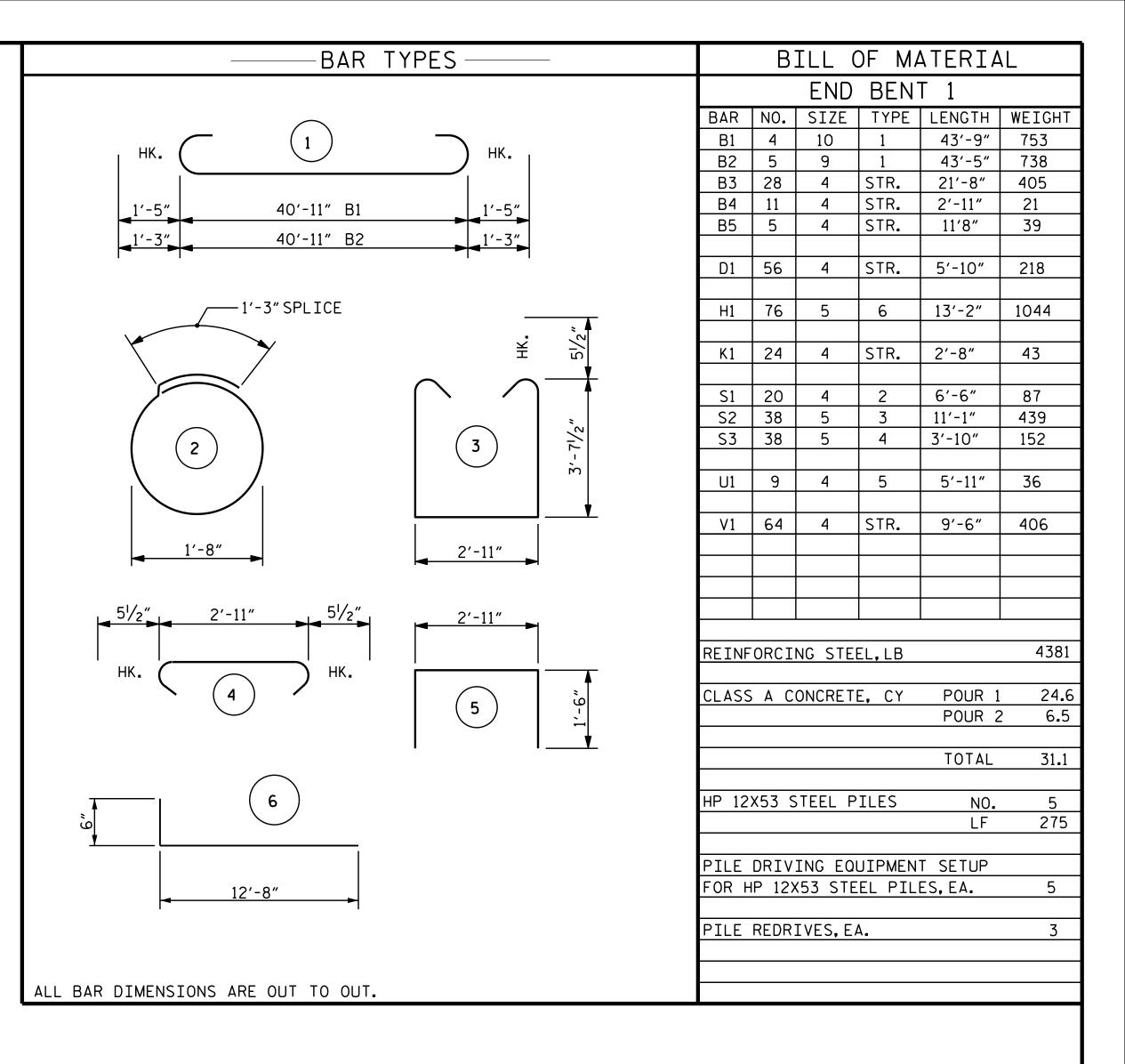
PLAN W2

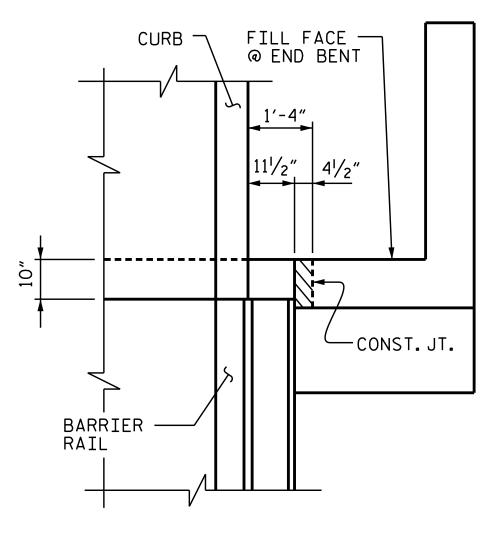


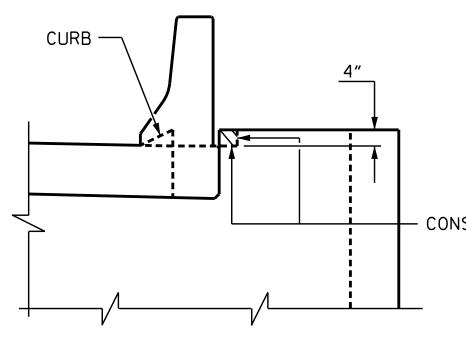
SECTION A-A



CHECKED BY : R.C.LARSON DATE : 09/21/20





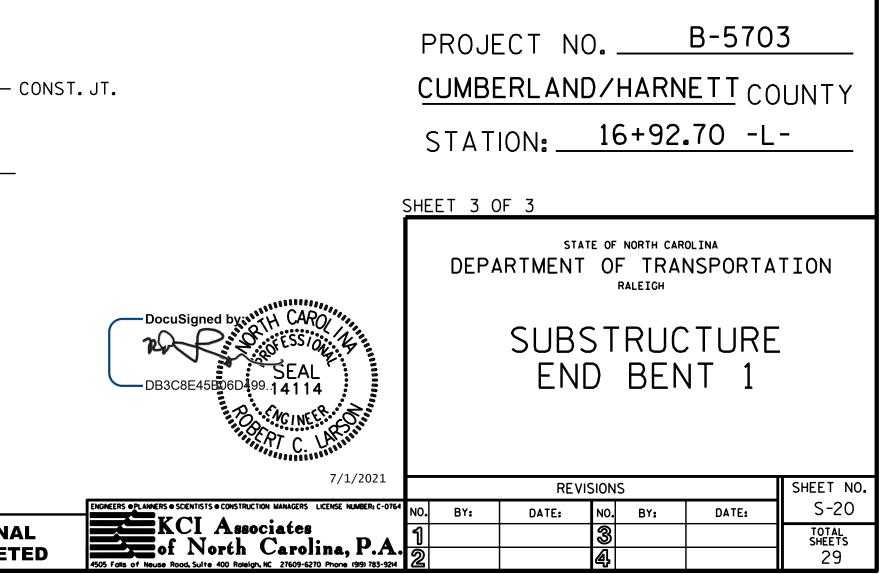


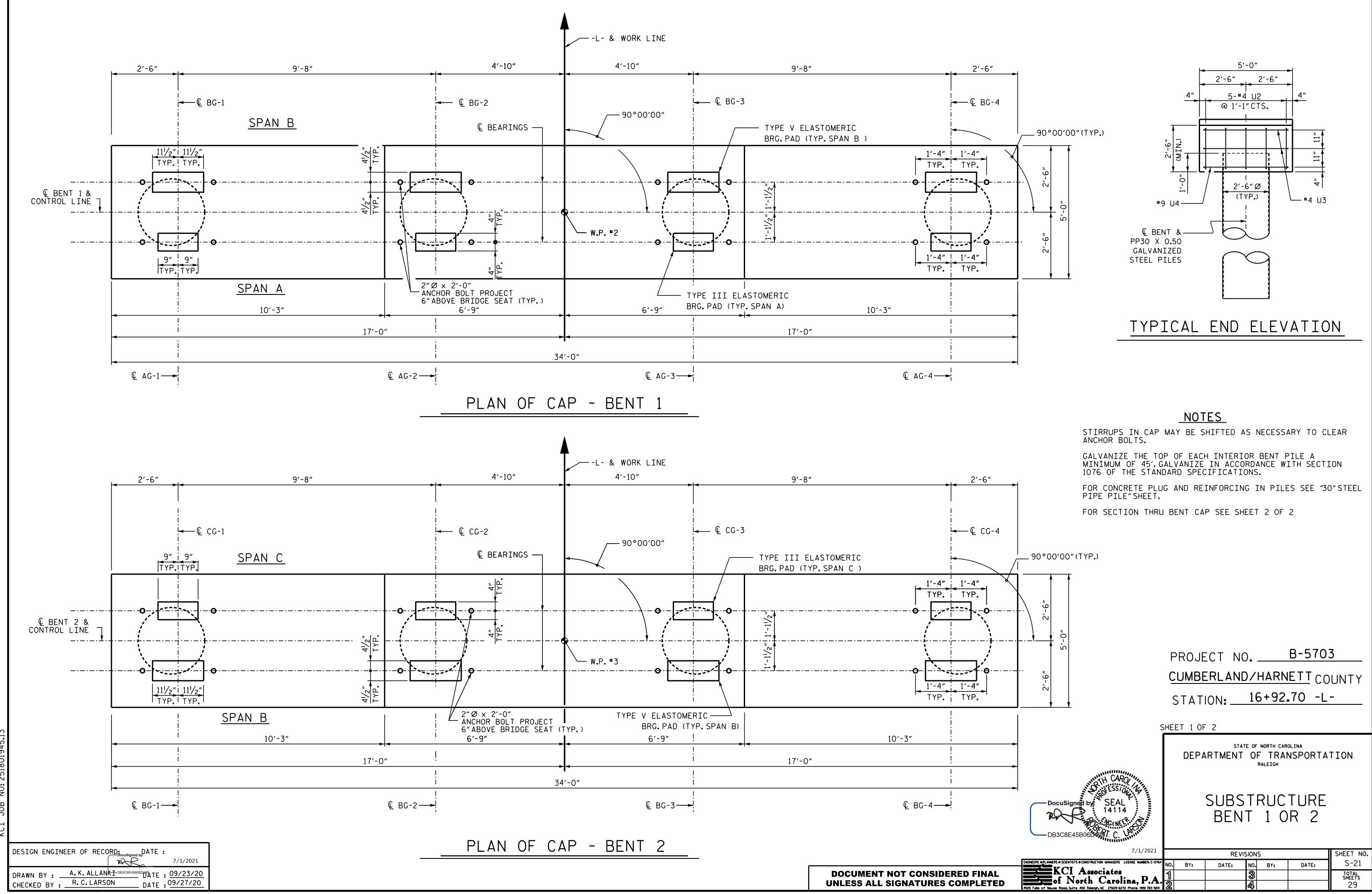
PLAN

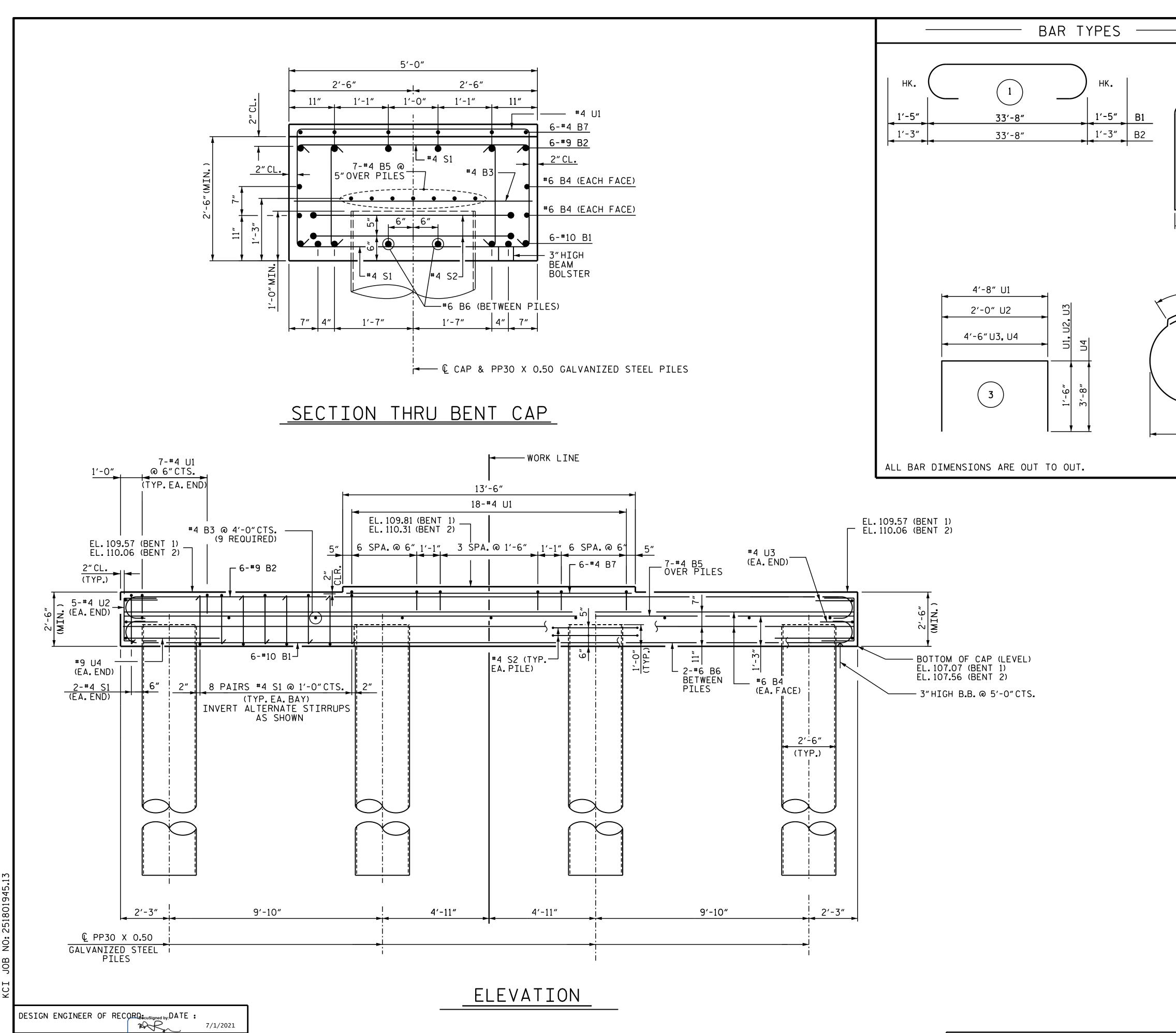
ELEVATION

# BLOCKOUT IN WING WALL

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.







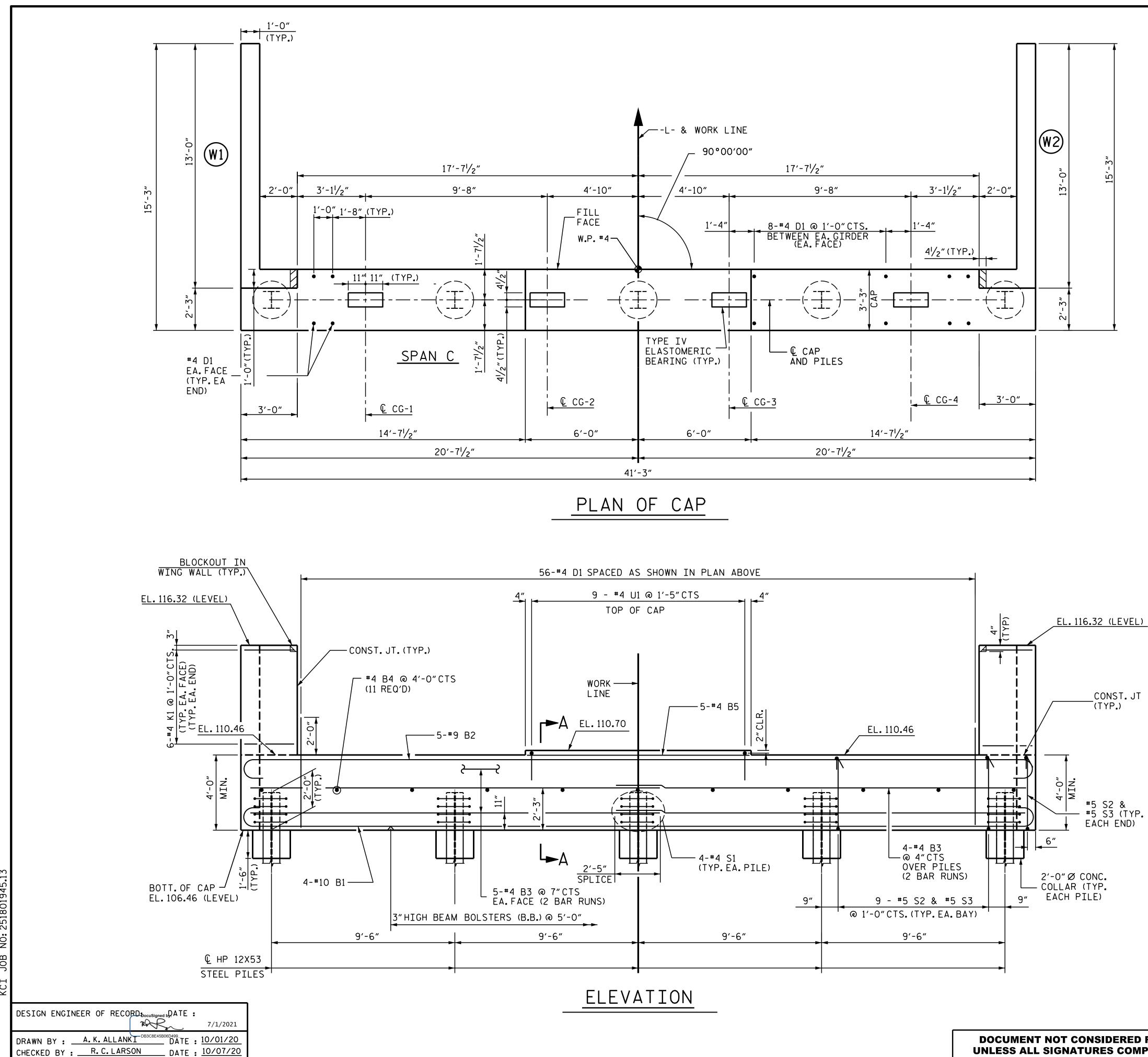
DRAWN BY : \_\_\_\_A.K.ALLANKI \_\_\_\_DB3C8E45B06D499... DATE : \_\_\_\_O9/23/20

CHECKED BY : R.C.LARSON DATE : 09/27/20

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

		BILL OF MATERIAL							
		FOR BENT 1 OR 2							
: ```	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
HK.	B1	6	10	1	36'-6″	942			
	B2	6	9	1	36'-2″	738			
$( \land \land ) \uparrow$	B3	9	4	STR.	4'-8"	28			
	B4	4	6	STR.	33'-8"	202			
	B5	7	4	STR.	33′-8″	157			
<sup>5</sup> ″	B6	6	6	STR.	7′-0″	63			
2,-11/2,	B7	6	4	STR.	13'-2″	53			
	S1	52	4	2	9'-1"	316			
	S2	8	4	4	12'-3"	65			
<u> </u>									
4'-1"	U1	32	4	3	7′-8″	164			
	U2	10	4	3	5′-0″	33			
	U3	4	4	3	7′-6″	20			
	U4	2	9	3	11'-10″	80			
┌──1′-3′′ LAP									
	REINFOR	RCING S	STEEL, LBS.			2861			
	CLASS A	CONCF	RETE, CU. YI	).		15.6			
$\mathbf{h}$	PP 30 X	0.50	GALVANIZE	D STEEL I	PILES				
$\frown$					NO.	4			
						30 - BENT 1			
				L	IN.FT. 20	50 - BENT 2			
	PILE DR	IVING	EQUIPMEN	T SETUP F	FOR				
	PP 30 X	0.50	GALVANIZE	D STEEL I	PILES,EA.	4			
3'-6"									
	PILE RE	PILE REDRIVES, EA. 2							
			ADS HAVE		JCTED				
	FROM C	LASS A	CONCRETE	)					

## B-5703 PROJECT NO.\_\_\_\_ CUMBERLAND/HARNETT COUNTY STATION: <u>16+92.70</u> -L-SHEET 2 OF 2 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE BENT 1 OR 2 7/1/2021 REVISIONS SHEET NO. ENGINEERS OPLANNERS O SCIENTISTS O CONSTRUCTION MANAGERS LICENSE NUMBER: C-0764 KCI Associates of North Carolina, P.A. 4505 Falls of Neuse Road, Suite 400 Raieligh, NC 27609-6270 Phone 1919 783-9214 S-22 NO. BY: DATE: DATE: BY: total sheets 29





## NOTES

THE TOP SURFACE OF THE END BENT CAP AND WINGS (POUR 1) EXCEPT THE BEARING AREAS AND THE AREA OUTSIDE OF THE SUPERSTRUCTURE SHALL BE RAKED TO A DEPTH OF 1/4".

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL IS CAST IF SLIPFORMING IS USED.

FOR "PILE SPLICE DETAILS", SEE END BENT 1.

FOR "BLOCKOUT IN WING WALL", SEE END BENT 1.

FOR SECTION A-A SEE SHEET 3 OF 3.

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #4 D1 BARS.

THE UPPER PORTION OF THE INTEGRAL END BENT SHALL BE POURED WITH THE SUPERSTRUCTURE. SEE SUPERSTRUCTURE PLANS.

### B-5703 PROJECT NO.\_\_\_\_ CUMBERLAND/HARNETT COUNTY

STATION: <u>16+92.70</u> -L-

SHEET 1 OF 3

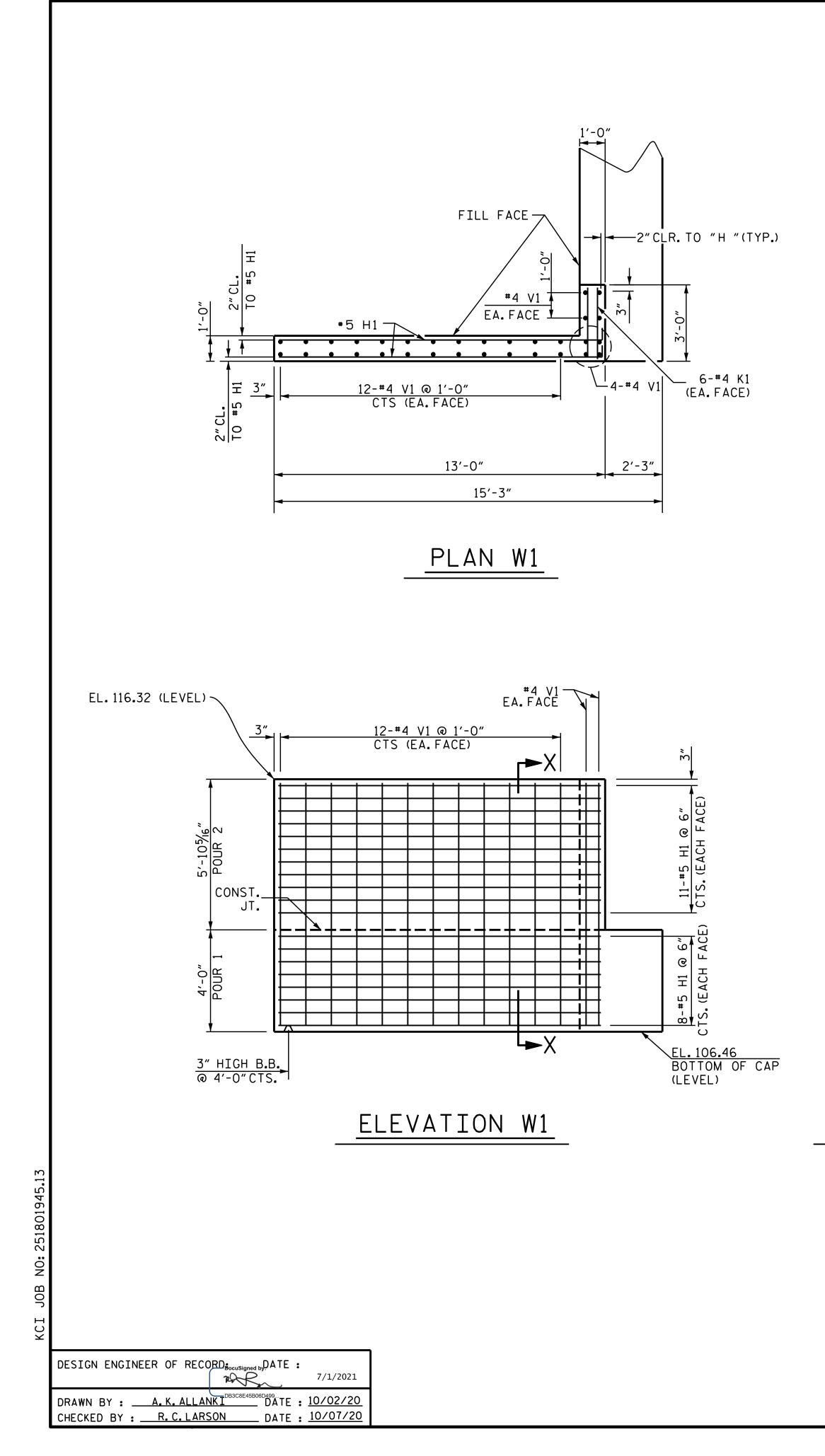
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

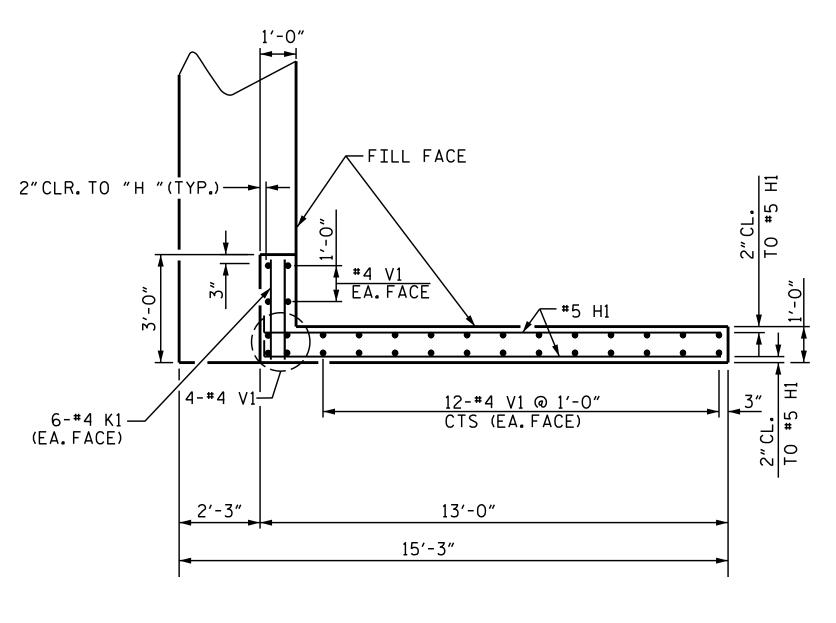
# SUBSTRUCTURE END BENT 2 (INTEGRAL)

7/1/2021					
		SHEET NO.			
ENGINEERS OPLANNERS OSCIENTISTS OCONSTRUCTION MANAGERS LICENSE NUMBER: C-0764	NO. BY:	DATE:	NO. BY:	DATE:	S-23
KCI Associates of North Carolina, P.A. 4505 Folls of Neuse Road, Sulte 400 Ratelign, NC 27609-6270 Phone 1999 783-9214	1		3		TOTAL SHEETS
4505 Fails of Neuse Road, Suite 400 Raleign, NC 27609-6270 Phone (99) 783-9214	2		4		29

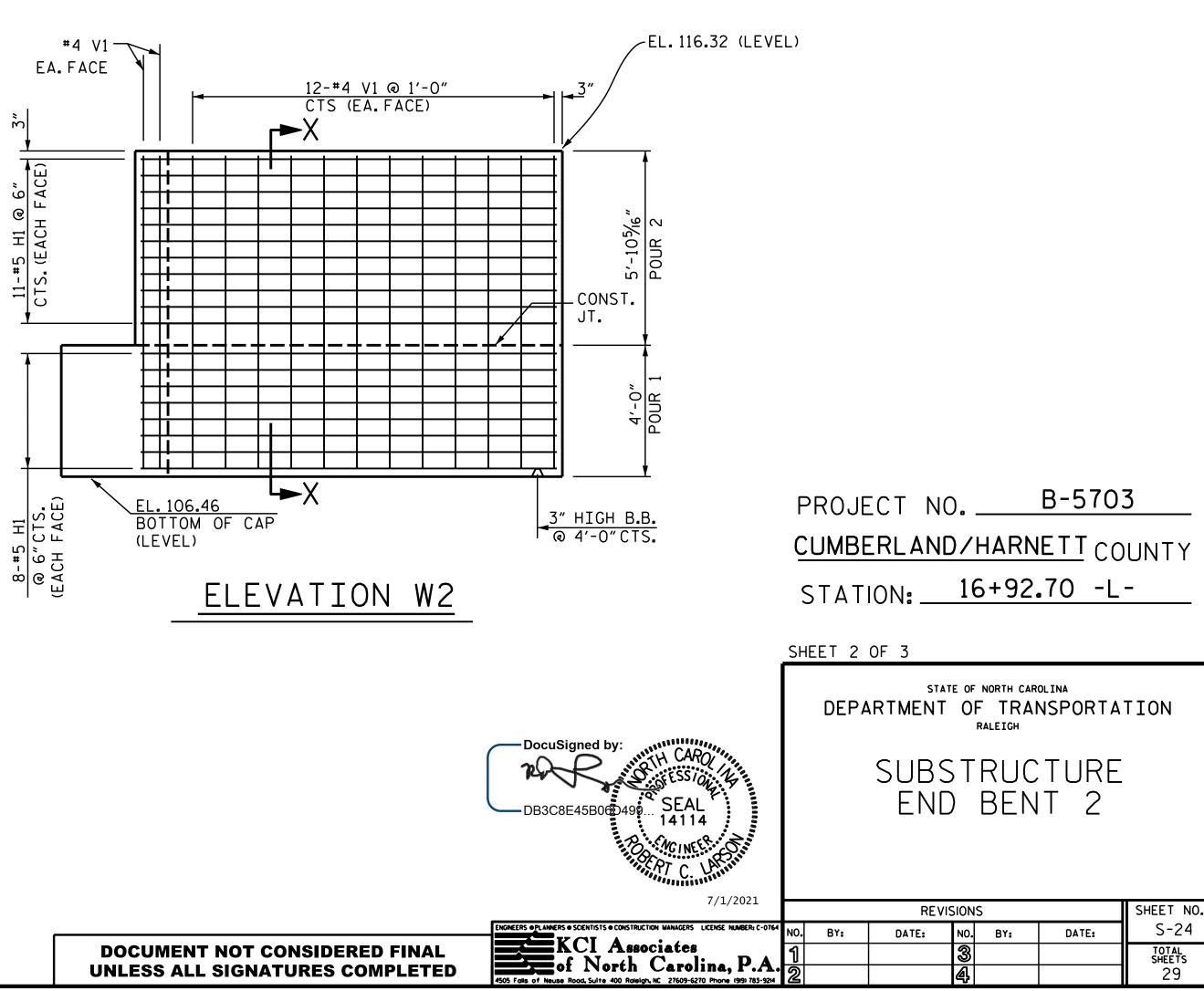
14114

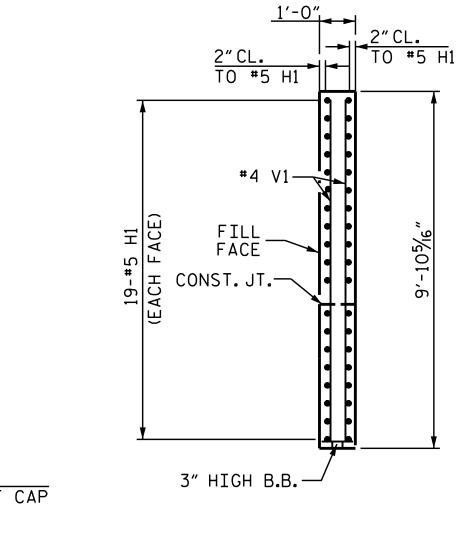
NCINE



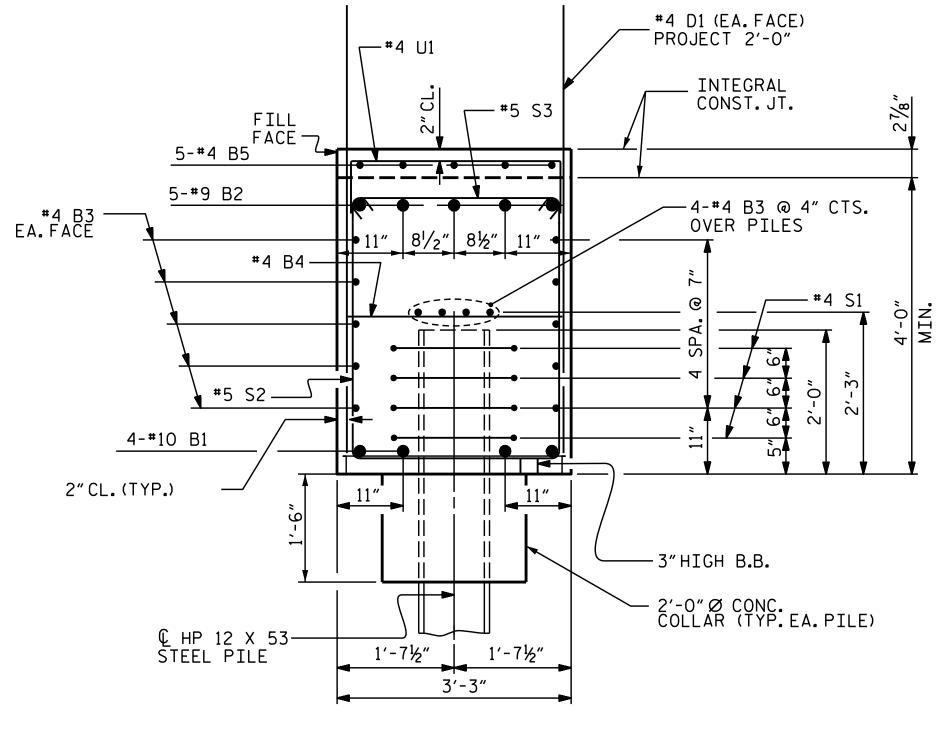


PLAN W2

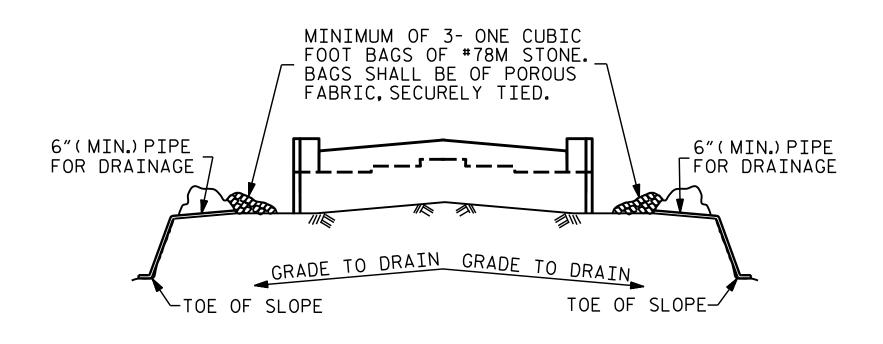




SECTION X-X



SECTION A-A



BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

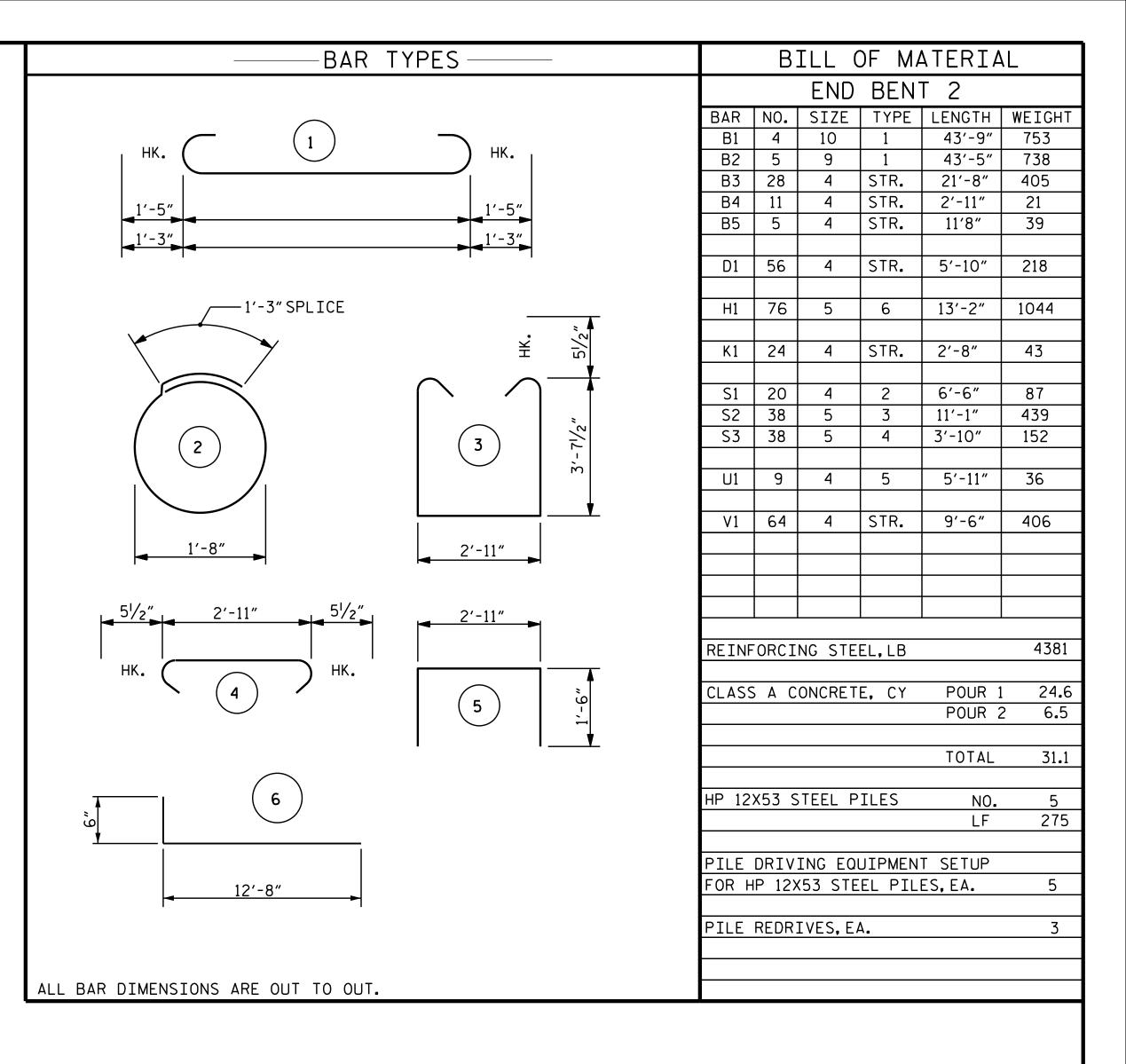
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

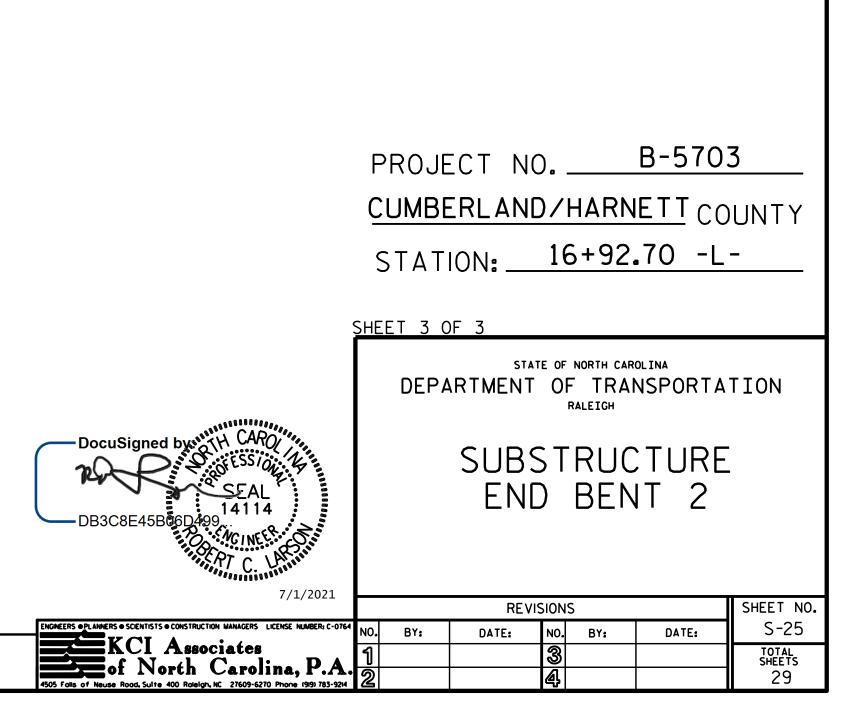
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

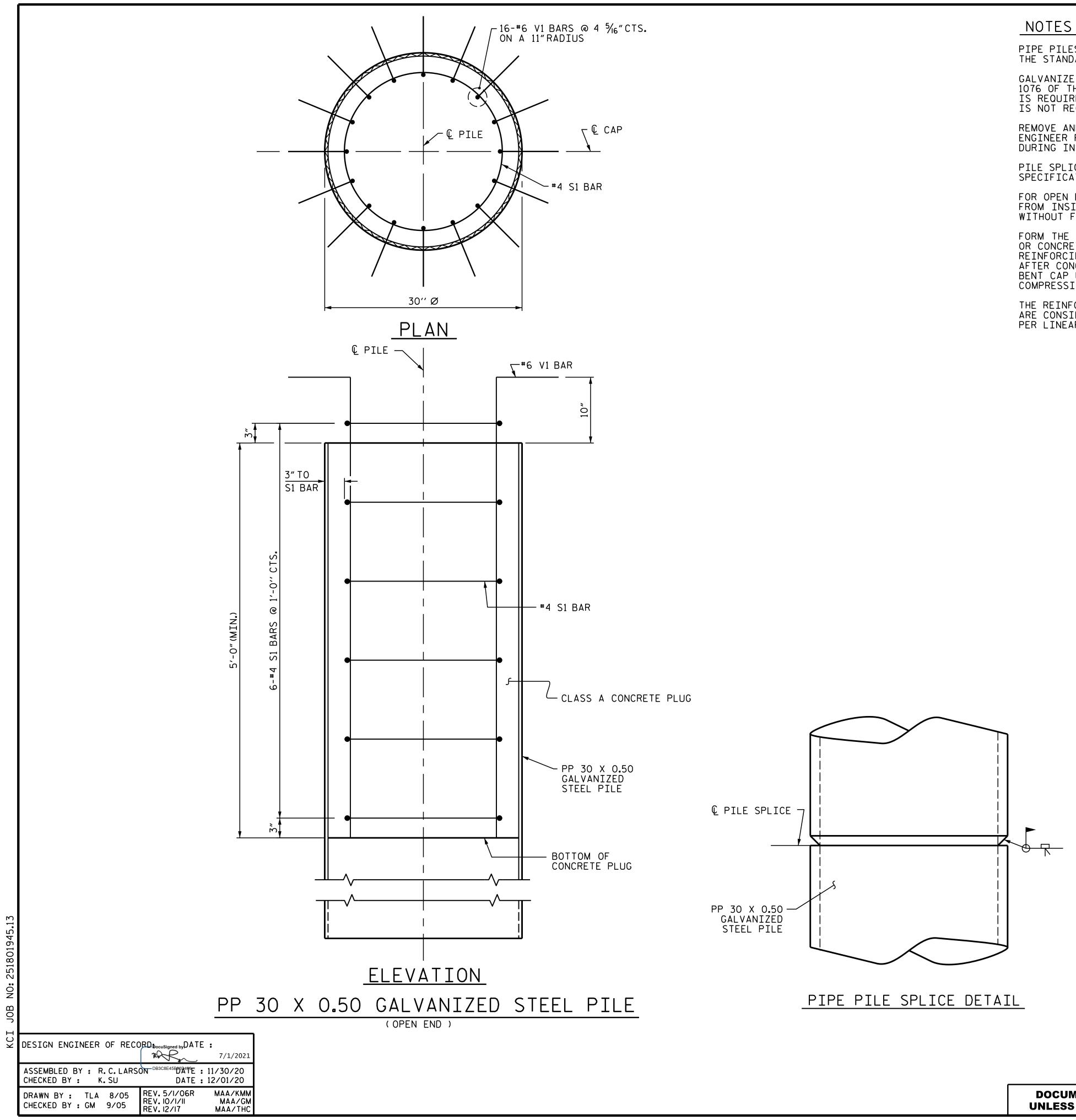
# TEMPORARY DRAINAGE AT END BENT

σ C CN ВС KCI

DESIGN ENGINEER OF RECORD	DocuSign@A:TE :	7/1/2021
DRAWN BY :A.K.ALLANKI CHECKED BY :R.C.LARSON		<u>10/02/2</u> 0 <u>10/07/2</u> 0







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PIPE PILES SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

GALVANIZE STEEL PIPE PILES IN ACCORDANCE 1076 OF THE STANDARD SPECIFICATIONS UNLE IS REQUIRED. GALVANIZING OR METALLIZING IS NOT REQUIRED.

REMOVE AND REPLACE OR REPAIR TO THE SAT ENGINEER PILES THAT ARE DAMAGED, DEFORME DURING INSTALLATION OR DRIVING.

PILE SPLICES SHALL BE IN ACCORDANCE WITH SPECIFICATIONS AND AWS D1.1.

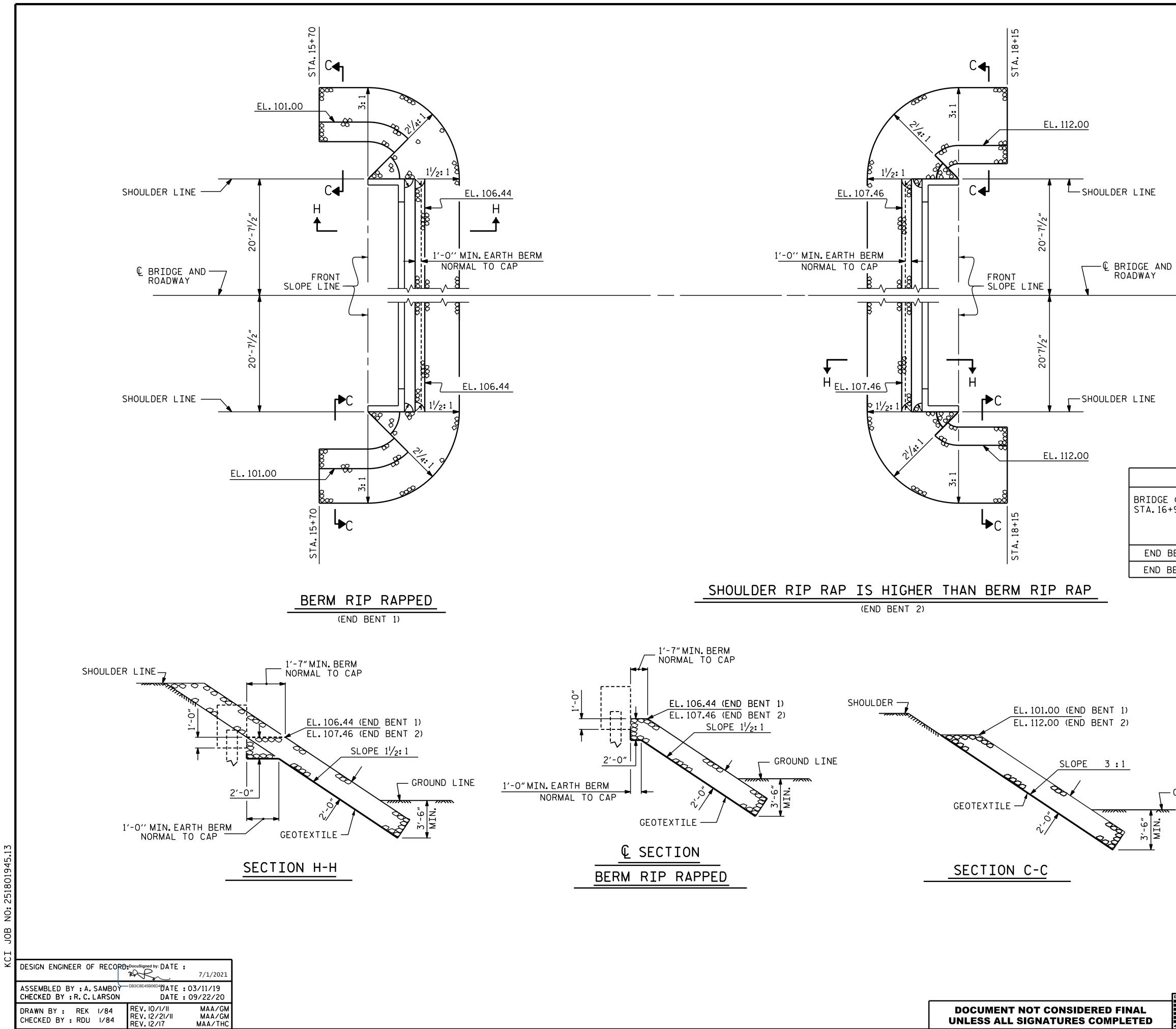
FOR OPEN END PIPE PILES, REMOVE ENOUGH S FROM INSIDE THE PILES TO CONSTRUCT THE WITHOUT FOULING THE CONCRETE.

FORM THE CONCRETE PLUG SUCH THAT THE RE OR CONCRETE DOES NOT MOVE AND THE CLEAR REINFORCING STEEL TO THE INSIDE OF THE AFTER CONCRETE PLACEMENT. DO NOT PLACE BENT CAP UNTIL THE CONCRETE PLUG HAS AT COMPRESSIVE STRENGTH OF 1500 PSI.

THE REINFORCING STEEL, CLASS A CONCRETE, ARE CONSIDERED INCIDENTAL TO THE CONTRA PER LINEAR FOOT FOR PP 30 X 0.50 GALVAN

					RIAL FOR	ONE TEEL PILE	
SECTION 1084 OF	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
	S1	6	#4	1	7'-7''	30	
ICE WITH SECTION	V1	16	<b>#</b> 6	2	6'-10''	164	
IG PIPE PILE PLATES							
	F	REINFO	RCING	STEEL =	- 19	94 Ibs	
ATISFACTION OF THE MED OR COLLAPSED							
	CLASS A CONCRETE						
TH THE STANDARD	5'-0	" MIN	emum f	PLUG		0.8 CY	
SOIL AND WATER	BAR TYPES						
E CONCRETE PLUG			1'-3	3'' LAP			
EINFORCING STEEL	×			/			
ARANCE FROM THE PILE IS MAINTAINED							
CONCRETE IN THE							
TTAINED A MINIMUM		(1)		, ,	↓		
, AND GALVANIZING		$\sim$			5	′-10′′	
ACT UNIT PRICE BID NIZED STEEL PILES.							
NIZED SIEEL FILES.		2'-0	)″				
		ALL	BAR D	IMENSION	NS ARE OUT T	0 OUT.	

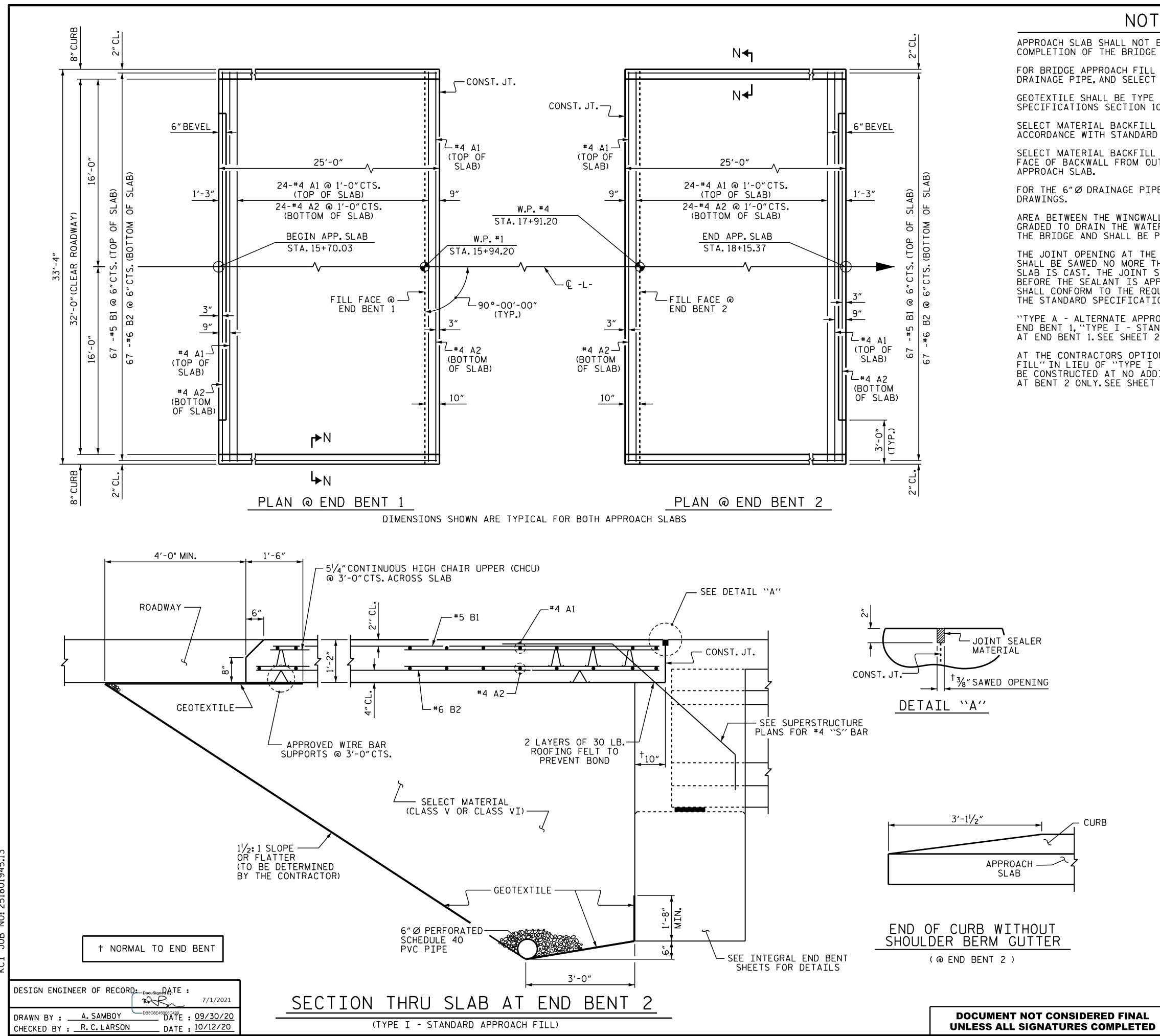
	PROJEC CUMBEI STATIC	RLAND	/HARNE		UNTY
DocuSigned by: DocuSigned by:		RTMENT	raleigh TANDAF	NSPORTA	
7/1/2021	NO. BY: 1 2	REVIS DATE:	SIONS NO. BY: 34	DATE:	SHEET NO. S-26 TOTAL SHEETS 29
			STD. N	O. SPP5	)



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— GROUND LINE	CUMBE	RLAND	E /HARNE 16+92.7	<u>TT</u> CO	UNTY
DocuSigned by OricESS/04-14 SEAL DB3C8E45B06D099 CINEER DF		RTMENT	TANDAF	NSPORTA	
7/1/2021		REVI	SIONS		SHEET NO.
ENGINEERS OPLANNERS O SCIENTISTS O CONSTRUCTION MANAGERS LICENSE NUMBER: C-0764	NO. BY:	DATE:	NO. BY:	DATE:	S-27
4505 Folls of Neuse Road, Sulte 400 Roleign, NC 27609-6270 Phone (1919) 783-9214	1		3 4		total sheets 29
			STD.	NO. RR	(Sht 2)

ESTIMATED QUANTITIES					
SE @ 6+92.70 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE			
	TONS	SQUARE YARDS			
BENT 1	550	610			
BENT 2	305	340			



## NOTES

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE,6"Ø DRAINAGE PIPE,AND SELECT MATERIAL,SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF

FOR THE 6" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

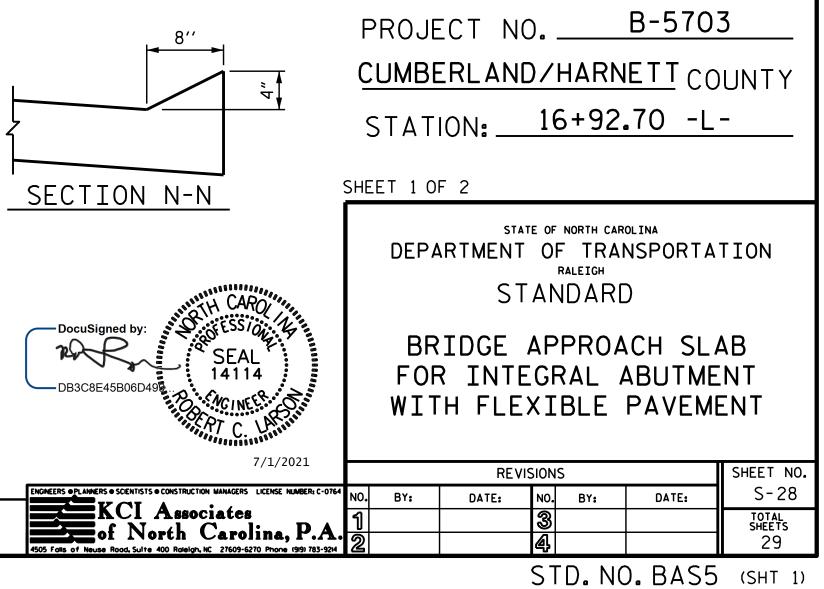
THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

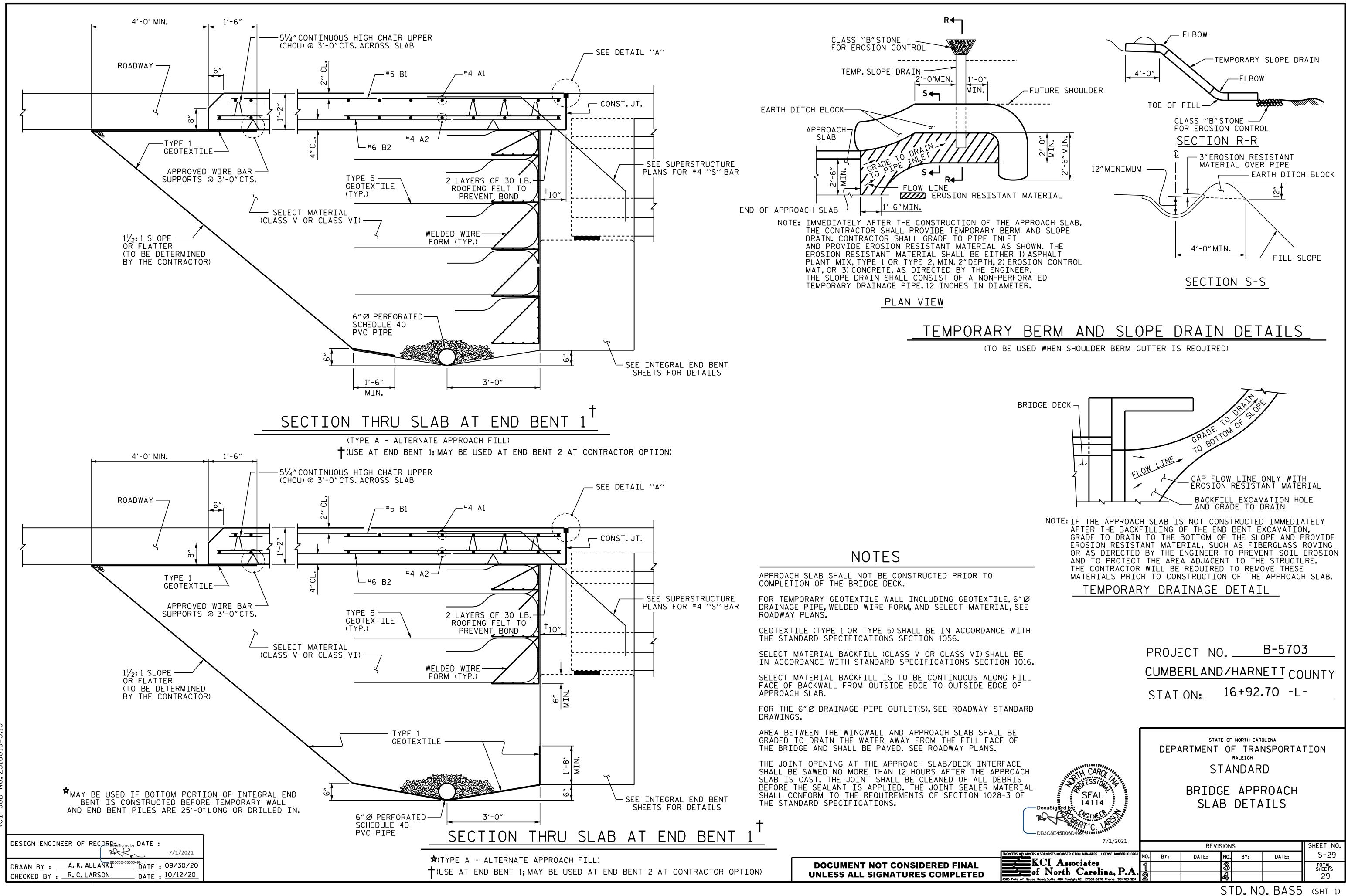
"TYPE A - ALTERNATE APPROACH FILL" SHALL BE CONSTRUCTED AT END BENT 1, "TYPE I - STANDARD APPROACH FILL" NOT ALLOWED AT END BENT 1. SEE SHEET 2 OF 2 FOR DETAILS AND NOTES.

AT THE CONTRACTORS OPTION, "TYPE A - ALTERNATE APPROACH FILL" IN LIEU OF "TYPE I - STANDARD APPROACH FILL" MAY BE CONSTRUCTED AT NO ADDITIONAL COST TO THE DEPARTMENT AT BENT 2 ONLY. SEE SHEET 2 OF 2 FOR DETAILS AND NOTES.

BILL OF MATERIAL						
FOR ONE APPROACH SLAB (2 REQ'D)						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
<b>*</b> A1	26	#4	STR	33'-0"	573	
Α2	26	#4	STR	33'-0"	573	
<b>米</b> B1	67	<b>#</b> 5	STR	24'-8"	1724	
B2	67	<b>#</b> 6	STR	24'-8"	2482	
REINFORCING STEEL				LBS.	3055	
<pre>* EPOXY COATED     REINFORCING STEEL</pre>				LBS.	2297	
CLASS	AA CC	C.Y.	36.0			

SPL	SPLICE LENGTHS					
BAR SIZE	EPOXY COATED	UNCOATED				
#4	2'-0"	1'-9"				
<b>#</b> 5	2'-6"	2'-2"				
#6	3'-10"	2'-7"				





I JOB NO: 251801945.1