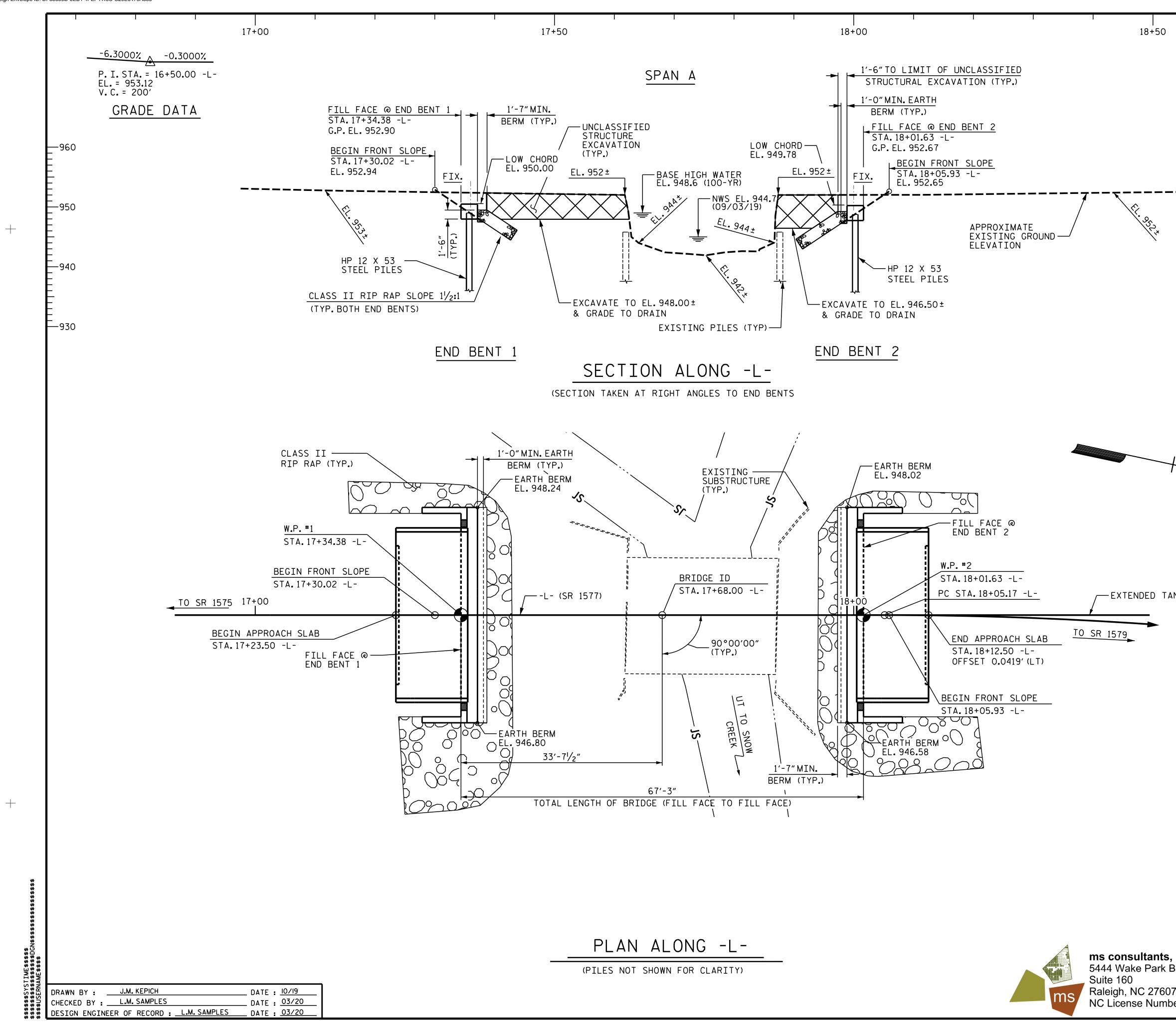
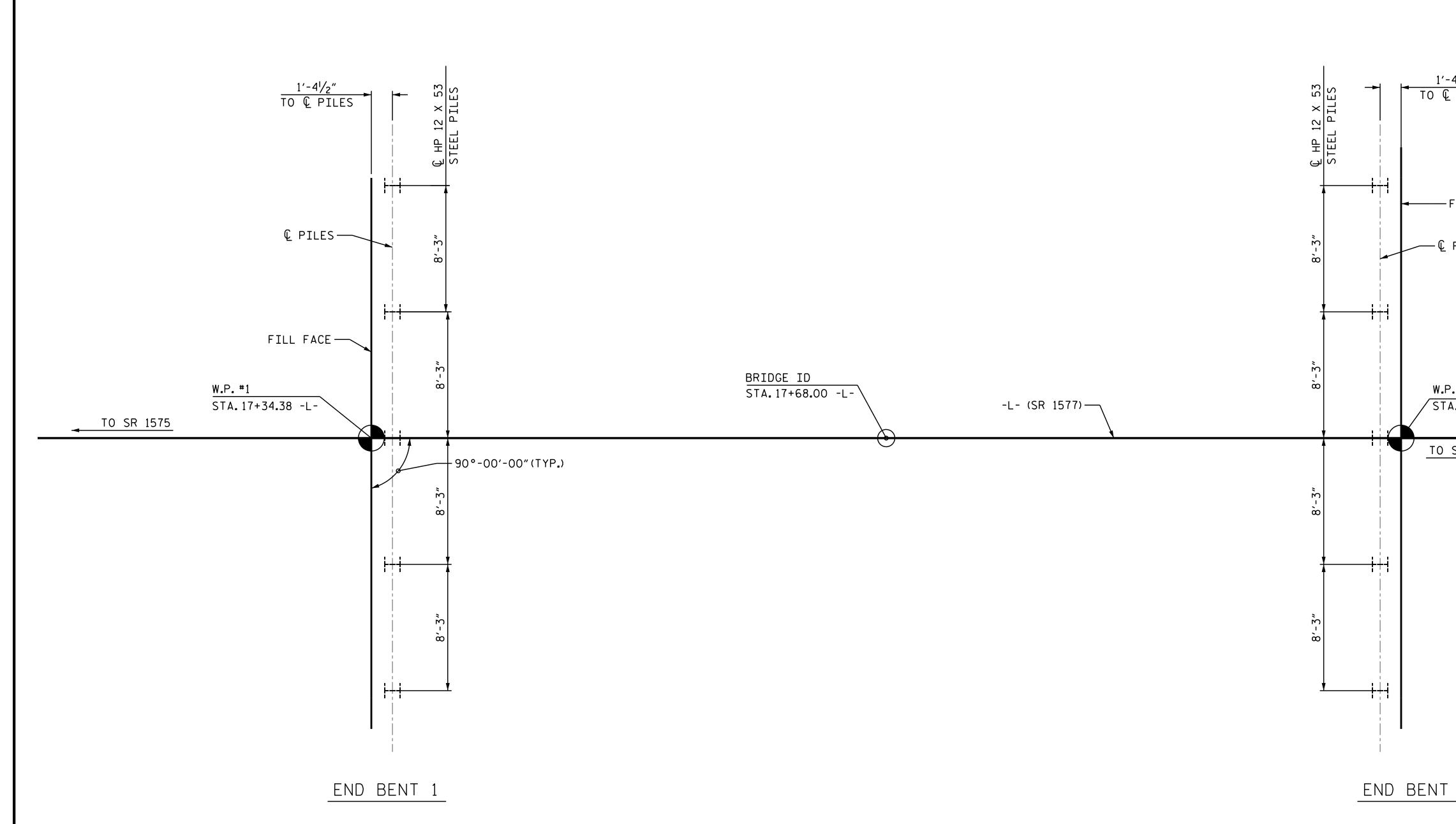


PROJECT LENGTH GTH ROADWAY PROJECT BR-0107 GTH STRUCTURE PROJECT BR-0107		MILES	Plans Prepared By: ms consultants, inc. 5444 Wake Park Blvd. Suite 160 Raleigh, NC 27607 NC License Number : C	Plans Prepared For: NORTH CAROLINA DEPARTMENT OF TRANSPORTATIO. DIVISION OF HIGHWAYS 1000 BIRCH RIDGE DRIVE RALEIGH, NC 27610
AL LENGTH PROJECT BR-0107	0.092		2018 STANDARD SPECIFICATIONS	
			LETTING DATE: JULY 20, 2021	



	-0.3000% 8.1826%
	-0.3000% 8.1826% P. I. STA. = 19+20.00 -L- EL. = 952.31
	V.C. = 230' GRADE DATA
	ONADE DATA
	HORIZONTAL CURVE DATA
	P.I.STA. = 19+38.74 -L-
	$\Delta = 23^{\circ} - 28' - 16.0'' (RT)$ D = 8° - 54' - 38.5''
	L = 263.40' T = 133.58' R = 643.00'
ANGENT	
	PROJECT NO. BR-0107
	IREDELL COUNTY
	STATION: 17+68.00 -L-
	SHEET 1 of 3 REPLACES BRIDGE NO. 131
	STATE OF NORTH CAROLINA
Lion M. Down	DEPARTMENT OF TRANSPORTATION
5663D099A9B449C	
, inc. Blvd.	GENERAL DRAWING
CEVI CEVI	BRIDGE 131 ON SR 1577 OVER UT TO SNOW CREEK
, inc.	BETWEEN SR 1575 AND SR 1579
Blvd.	REVISIONS SHEET NO.
Der: C-3239 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	2 2 4 14



FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS. PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 94 TONS PER PILE. DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 157 TONS PER PILE. PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 94 TONS PER PILE. DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 157 TONS PER PILE.

<u>NOTES</u>

\$\$\$\$\$\$\$\$YSTIME\$\$\$\$\$ \$\$\$\$\$\$\$\$\$\$\$\$\$\$0GN\$ \$\$\$\$USERNAME\$\$\$\$

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	DRAWN BY :	J.M. KEPICH L.M. SAMPLES ER OF RECORD : _		DATE : <u> / 9</u>
	CHECKED BY :	L.M. SAMPLES		DATE : <u>03/20</u>
Â	DESIGN ENGINE	ER OF RECORD :	L.M. SAMPLES	DATE : <u>03/20</u>

FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE TO CENTERLINE



ms consultants, 5444 Wake Park E Suite 160 Raleigh, NC 27607 NC License Number : C-3239

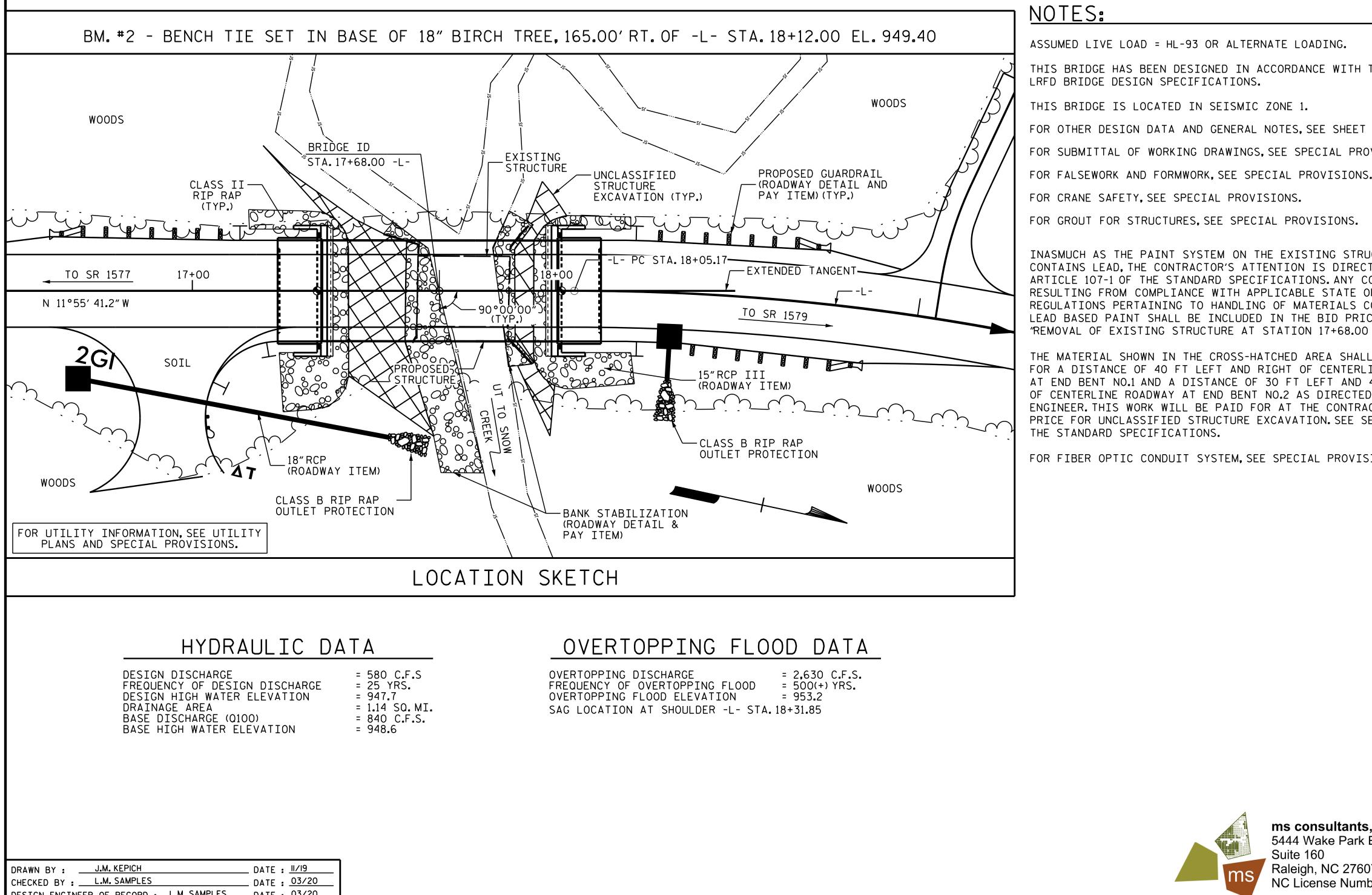
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

3

total sheets 14

4 ¹ /2″ PILES		
PILES		
FILL FACE		
PILES		
. # 2		
• 2 • 18+01.63 -L	-	
SR 1579		
2		
		PROJECT NO. <u>BR-0107</u>
		IREDELL COUNTY
		STATION: 17+68.00 -L-
		STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
	ماسىرى ٢١. مەلكى 5663D099A9B449C 5/19/2021	GENERAL DRAWING
	POFESSION SEAL	BRIDGE 131 ON SR 1577
, inc. Blvd.	SEAL 037031	OVER UT TO SNOW CREEK BETWEEN SR 1575 AND SR 1579
)7	M SAMININ	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: S-O2

					ТО	TAL BI	LL OF N	ΙΑΤ	ERIA	AL.						
	REMOVAL OF EXISTING STRUCTURE AT STATION 17+68.00	ASBESTOS ASSESSMENT	UNCLASSIFIED STRUCTURE EXCAVATION AT STATION 17+68.00	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP12X53 STEEL PILES	HP12X53 STEEL PILES		VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-O" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRES COI	″X 2'-0″ STRESSED NCRETE ED SLABS	FIBER OPTIC CONDUIT SYSTEM
	LUMP SUM	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	EA.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.	LIN.FT.
SUPERSTRUCTURE										130.25			LUMP SUM	10	650	126.25
END BENT NO.1				13.2		1965	5	5	150		73	82				
END BENT NO.2				13.2		1965	5	5	125		66	74				
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	26.4	LUMP SUM	3930	10	10	275	130.25	139	156	LUMP SUM	10	650	126.25



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A					
ERN	DRAWN BY :	J.M. KEPICH L.M. SAMPLES ER OF RECORD :		_ DATE :	11/19
\$U\$	CHECKED BY :	L.M. SAMPLES		DATE :	03/20
\$	DESIGN ENGINE	ER OF RECORD :	L.M. SAMPLES	DATE :	03/20
₩ 1					

ms consultants, 5444 Wake Park Raleigh, NC 2760 NC License Numb

THE AASHTO SN. OVISIONS.	THE EXISTING STRUCTURE CONSISTING OF A SINGLE 25'-9" SPAN WITH A CLEAR ROADWAY WIDTH OF 19.25' AND TIMBER DECK SUPPORTED BY STEEL GIRDERS ON TIMBER CAPS AND PILES LOCATED AT THE PROPOSED STRUCTURE SITE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.
S. UCTURAL STEEL CTED TO COSTS OR FEDERAL CONTAINING ICE FOR D -L"	THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.
L BE EXCAVATE INE ROADWAY 40 FT RIGHT D BY THE ACT LUMP SUM SECTION 412 OF	AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE
SIONS.	FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS. ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS. FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.
	PROJECT NO. <u>BR-0107</u> <u>IREDELL</u> COUNTY STATION: <u>17+68.00</u> -L- SHEET 3 of 3
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH GENERAL DRAWING BRIDGE 131 ON SR 1577 OVER LIT TO SNOW CREEK
	037031 BETWEEN SR 1575 AND SR 1579 MGINERED REVISIONS NO. BY: DATE: NO. NO. BY: DATE: NO. SHEET NO. SIGNATURES COMPLETED 2 4 14

		LOAD AN	ND RES	SIST	ANCE	E FAC	CTOR	RAT	ING	(LRF	D) SI	JMMA	RY F	OR F	PRES	TRES	SSED	CON	CRETI	E GI	rdef	RS	
									STRENGTH I LIMIT STATE									SERVICE III LIMIT STATE					
										MOMENT					SHEAR						MOMENT		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	L I VEL OAD F AC T ORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	L I VEL OAD F AC T ORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)
		HL-93(Inv)	N/A	1	1.018		1.75	0.274	1.05	65′	EL	32	0.513	1.2	65′	EL	6.4	0.80	0.274	1.02	65′	EL	32
DESIGN LOAD RATING		HL-93(0pr)	NZA		1.358		1.35	0.274	1.36	65′	EL	32	0.513	1.56	65′	EL	6.4	NZA					
	HS-20(Inv)	36.000	2	1.306	47.014	1.75	0.274	1.34	65′	EL	32	0.513	1.48	65′	EL	6.4	0.80	0.274	1.31	65′	EL	32	
	HS-20(0pr)	36.000		1.742	62.706	1.35	0.274	1.74	65′	EL	32	0.513	1.92	65′	EL	6.4	NZA						
	SNSH	13.500		2.868	38.725	1.4	0.274	3.69	65′	EL	32	0.513	4.33	65′	EL	6.4	0.80	0.274	2.87	65′	EL	32	
		SNGARBS2	20.000		2.171	43.424	1.4	0.274	2.79	65′	EL	32	0.513	3.11	65′	EL	6.4	0.80	0.274	2.17	65′	EL	32
		SNAGRIS2	22.000		2.071	45.552	1.4	0.274	2.66	65′	EL	32	0.513	2.89	65′	EL	6.4	0.80	0.274	2.07	65′	EL	32
		SNCOTTS3	27.250		1.428	38.924	1.4	0.274	1.84	65′	EL	32	0.513	2.17	65′	EL	6.4	0.80	0.274	1.43	65′	EL	32
	SV	SNAGGRS4	34.925		1.206	42.136	1.4	0.274	1.55	65′	EL	32	0.513	1.81	65′	EL	6.4	0.80	0.274	1.21	65′	EL	32
		SNS5A	35.550		1.179	41.911	1.4	0.274	1.52	65′	EL	32	0.513	1.85	65′	EL	6.4	0.80	0.274	1.18	65′	EL	32
		SNS6A	39.950		1.087	43.43	1.4	0.274	1.4	65′	EL	32	0.513	1.69	65′	EL	6.4	0.80	0.274	1.09	65′	EL	32
LEGAL		SNS7B	42.000		1.035	43.489	1.4	0.274	1.33	65′	EL	32	0.513	1.67	65′	EL	6.4	0.80	0.274	1.04	65′	EL	32
LOAD		TNAGRIT3	33.000		1.327	43.8	1.4	0.274	1.71	65′	EL	32	0.513	2.01	65′	EL	6.4	0.80	0.274	1.33	65′	EL	32
RATING		TNT4A	33.075		1.335	44.142	1.4	0.274	1.72	65′	EL	32	0.513	1.95	65′	EL	6.4	0.80	0.274	1.33	65′	EL	32
		TNT6A	41.600		1.096	45 . 613	1.4	0.274	1.41	65′	EL	32	0.513	1.8	65′	EL	6.4	0.80	0.274	1.10	65′	EL	32
	TIST	TNT7A	42.000		1.105	46.4	1.4	0.274	1.42	65′	EL	32	0.513	1.74	65′	EL	6.4	0.80	0.274	1.10	65′	EL	32
		TNT7B	42.000		1.15	48.298	1.4	0.274	1.48	65′	EL	32	0.513	1.62	65′	EL	6.4	0.80	0.274	1.15	65′	EL	32
		TNAGRIT4	43.000		1.089	46.815	1.4	0.274	1.4	65′	EL	32	0.513	1 . 57	65′	EL	6.4	0.80	0.274	1.09	65′	EL	32
		TNAGT5A	45.000		1.024	46.084	1.4	0.274	1.32	65′	EL	32	0.513	1 . 57	65′	EL	6.4	0.80	0.274	1.02	65′	EL	32
		TNAGT5B	45.000	3	1.01	45.431	1.4	0.274	1.3	65′	EL	32	0.513	1.49	65′	EL	6.4	0.80	0.274	1.01	65′	EL	32

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TIME\$\$\$\$\$ \$\$\$\$\$\$\$\$ } ASSEMBLED BY : J.M. KEPICH CHECKED BY : L.M. SAMPLES DATE : II/I9 DATE : 03/20 \$\$\$\$\$\$\$\$ \$\$\$\$\$\$\$ S1\$\$\$\$\$\$ DRAWN BY : CVC 6/10 CHECKED BY : DNS 6/10

STRENGTH	Ι	LIMIT	STATE	

64'-0" (BRG. TO BRG.)

FOR SPAN 'A'





LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{D\mathbf{W}}$
LOAD	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

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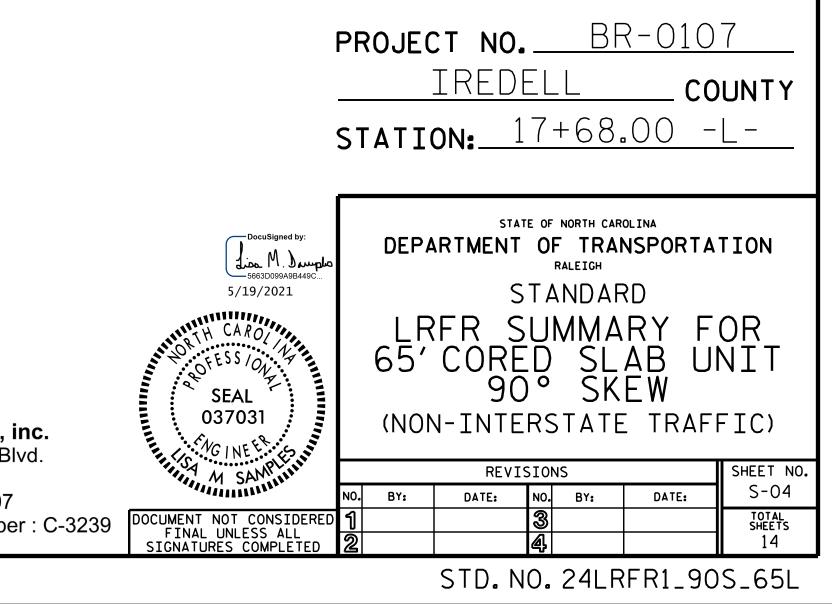
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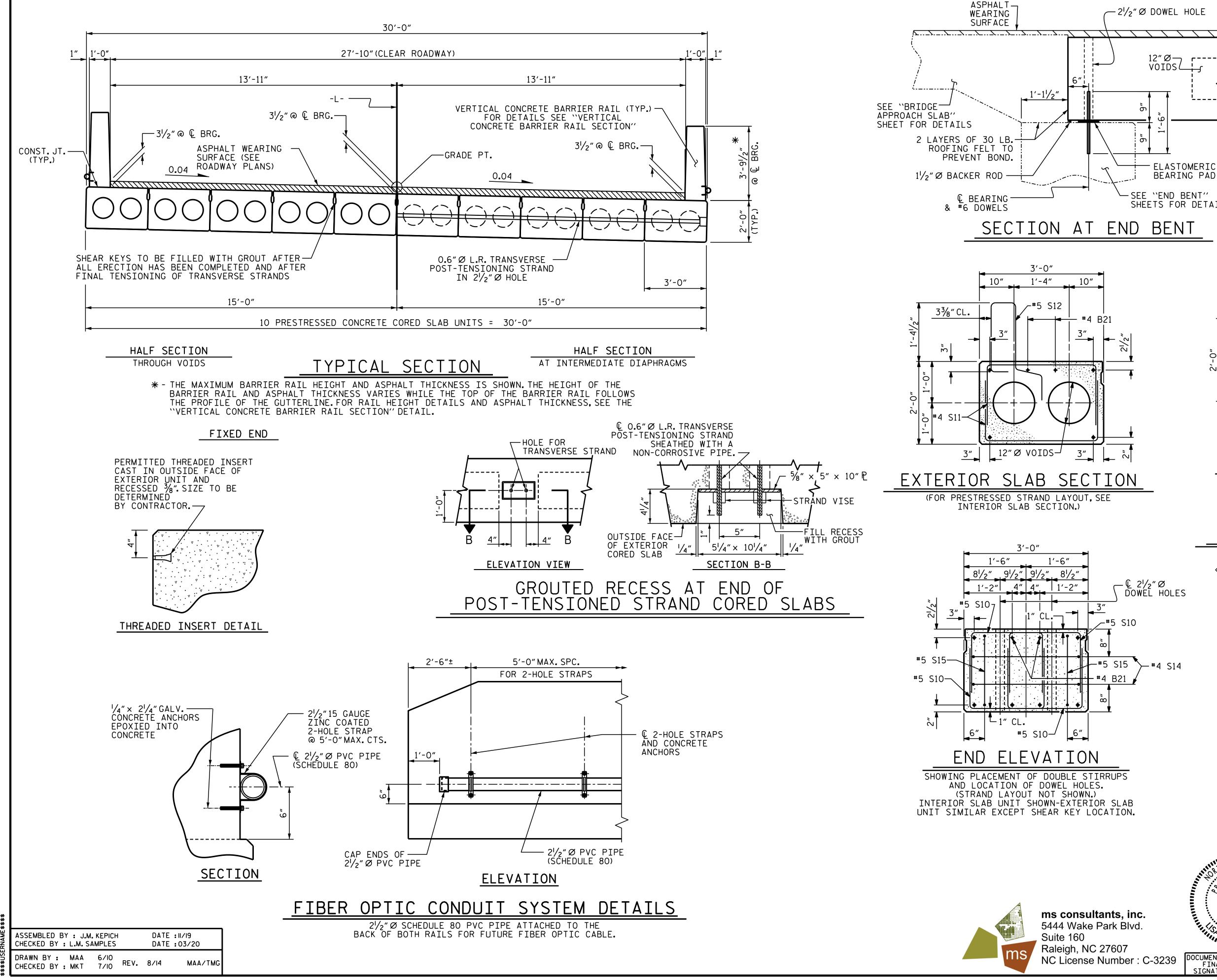
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MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENT	S:
1.	
2.	
3.	
4.	
	<pre>CONTROLLING LOAD RATING</pre>
	1 DESIGN LOAD RATING (HL-93)
	2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **
* * SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER - EXTERIOR RIGHT GIRDER





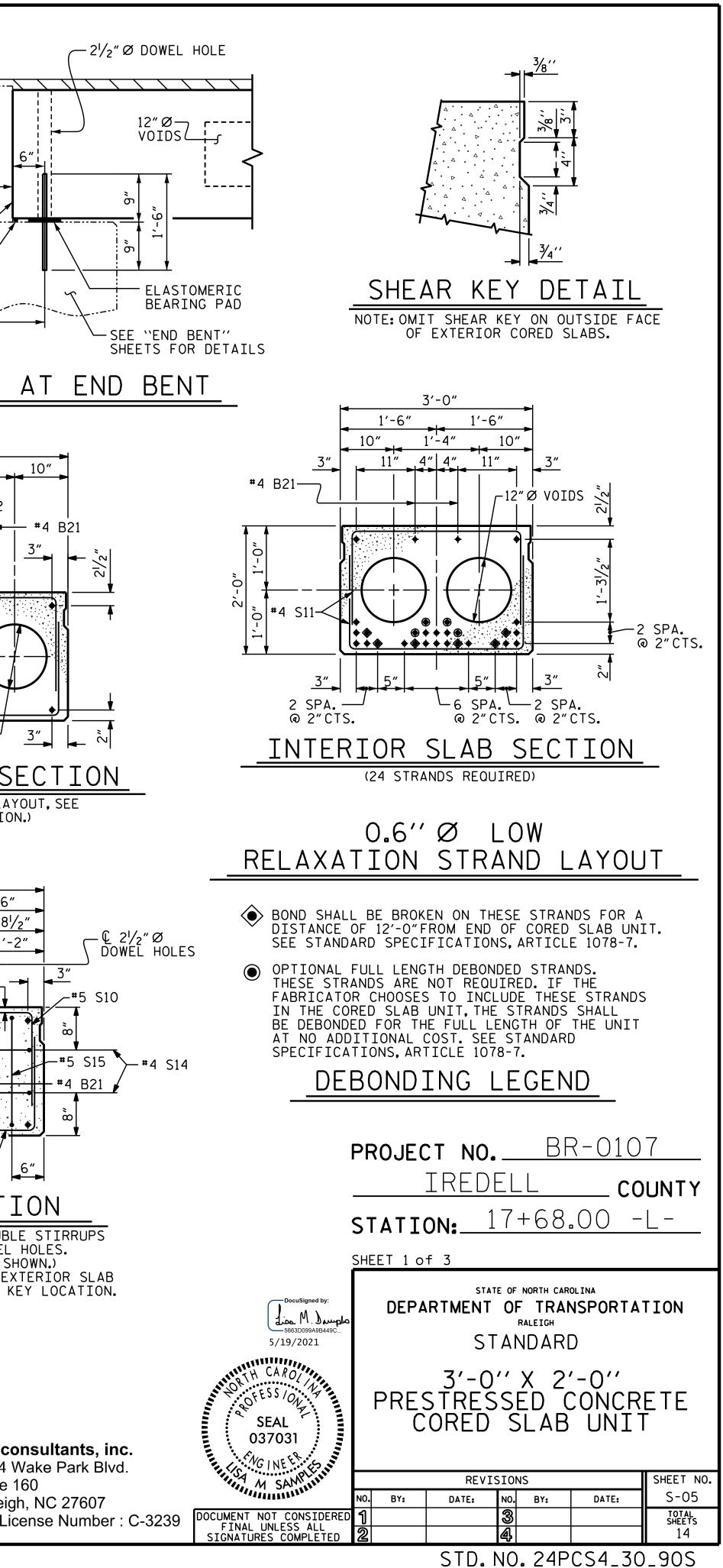
ms consultants, inc. 5444 Wake Park Blvd. Suite 160 Raleigh, NC 27607 NC License Number : C-3239

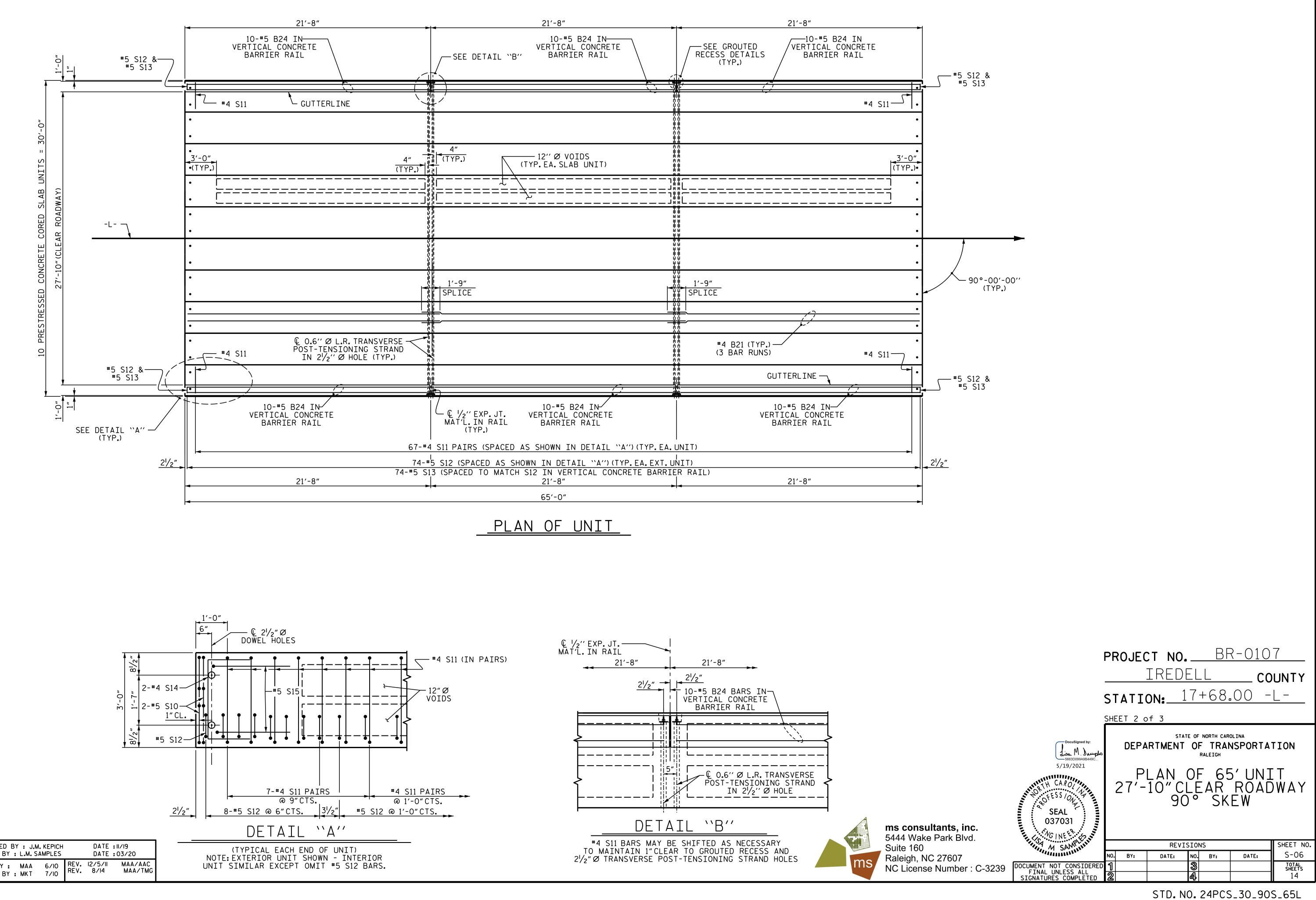
6″

10″

3″

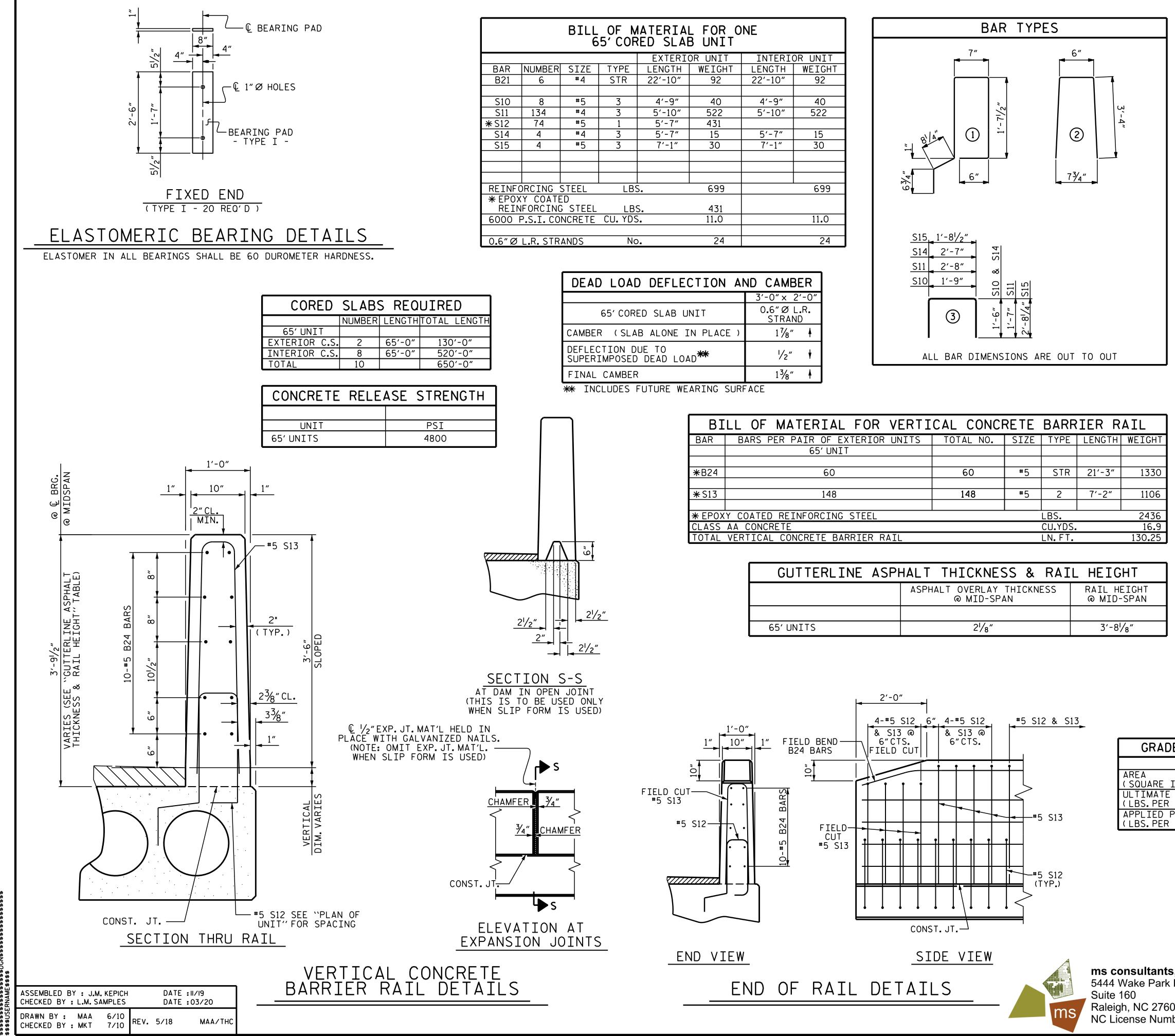
3″







° ₩					
RNAME	ASSEMBLED BY : J.M CHECKED BY : L.M.S			DATE DATE	: II/19 : 03/20
\$\$\$\$USERNAME \$\$	DRAWN BY : MAA CHECKED BY : MKT	6/10 7/10	REV. REV.	12/5/11 8/14	MAA/AAC MAA/TMG

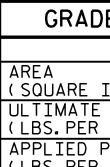


	BILL OF MATERIAL FOR ONE 65' CORED SLAB UNIT					
			EXTERI	OR UNIT	INTERI	OR UNIT
R	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
	#4	STR	22'-10"	92	22'-10"	92
	# 5	3	4′-9″	40	4′-9″	40
	#4	3	5'-10"	522	5'-10″	522
	# 5	1	5′-7″	431		
	#4	3	5′-7″	15	5′-7″	15
	# 5	3	7'-1"	30	7'-1″	30
	STEEL	LBS	5.	699		699
	ED		_			
	<u>S STEEL</u>	LBS		431		
0	NCRETE	CU. YDS	-) a	11.0		11.0
_						
2	ANDS	Nc).	24		24

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
65' CORED SLAB UNIT	0.6″ØL.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1 7∕8″ ∳
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	!∕₂″ ↓
FINAL CAMBER	1 ³ ⁄⁄8″ ↓

BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL						
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	65' UNIT					
₩ B24	60	60	#5	STR	21'-3"	1330
* S13	148	148	# 5	2	7'-2"	1106
						2436
₩ EPOX	* EPOXY COATED REINFORCING STEEL LBS. 2					
CLASS	AA CONCRETE			CU.YDS.		16.9
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN.FT.		130.25

GUTTERLINE ASPI	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
65' UNITS	21/8"	3′-8 <mark>′/</mark> 8″



ms consultants 5444 Wake Park Raleigh, NC 2760 NC License Numl

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

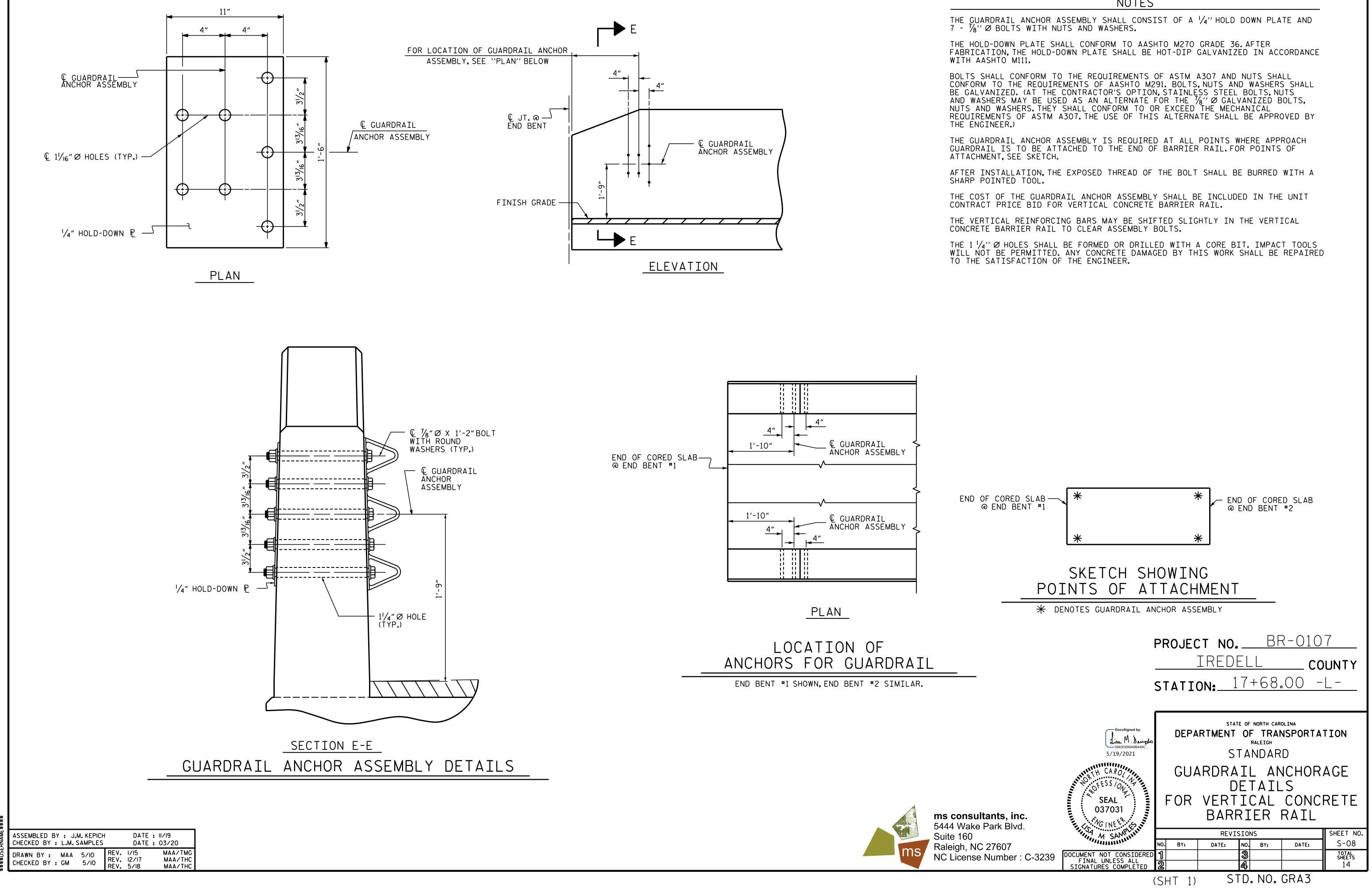
THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

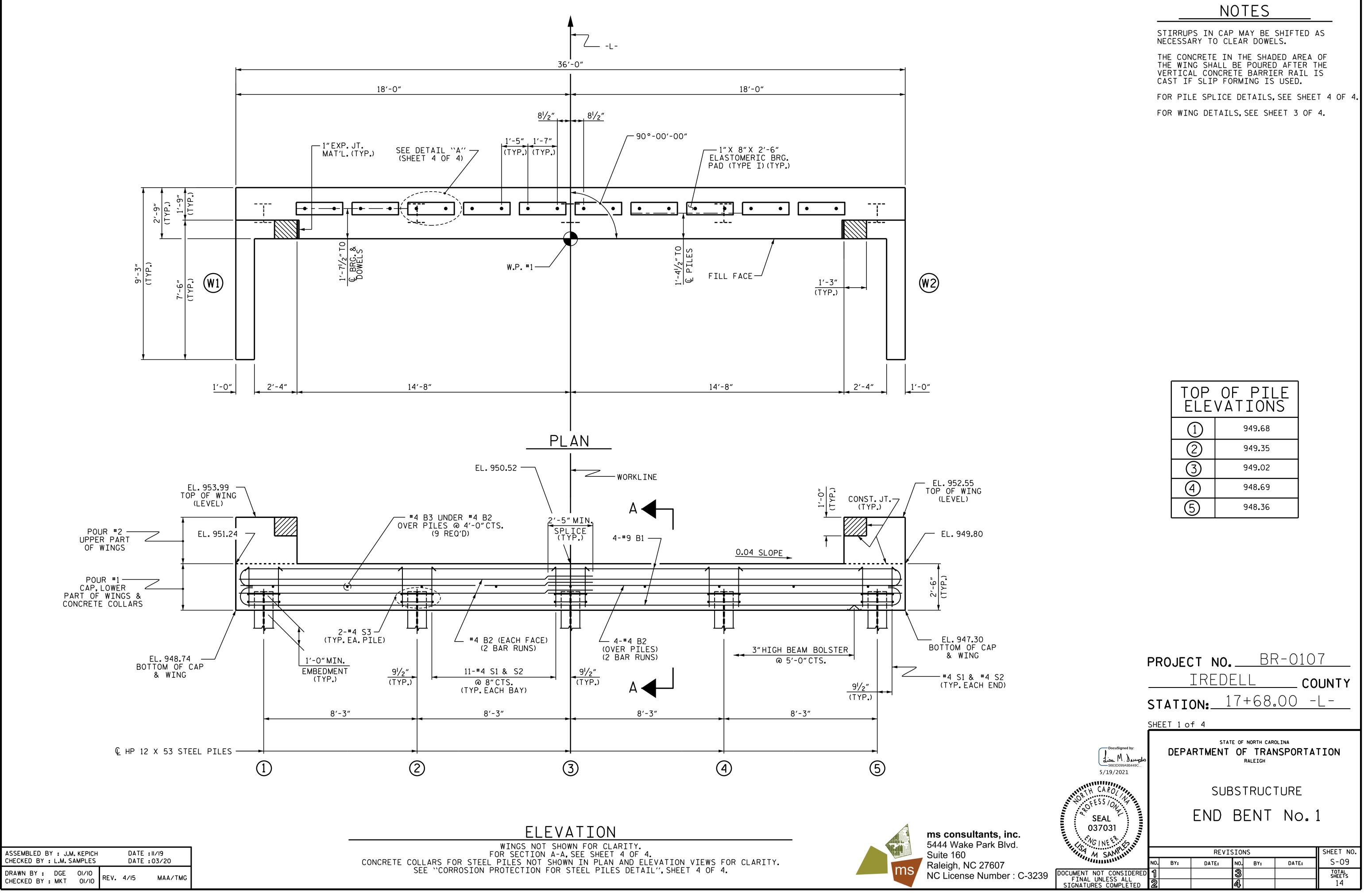
THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

E 270 S	TRANDS						
	0.6″ØL.R.				RF	R-010	7
INCHES)	0.217			T NO			
STRENGTH STRAND)	58,600	-		IREDE			UNTY
PRESTRESS STRAND)	43,950		STATI	DN:1	7+68.	.00 -	<u> </u>
	SHEET 3 of 3						
state of NORTH CAROLINA DEPARTMENT OF TRANSPORTAT RALEIGH STANDARD 3'-O"X 2'-O" PRESTRESSED CONCRE CORED SLAB UNIT					ETE		
Blvd.	MAL M	SAMPLIN		REVISI	IONS		SHEET NO.
)7			NO. BY:		NO. BY:	DATE:	S-07
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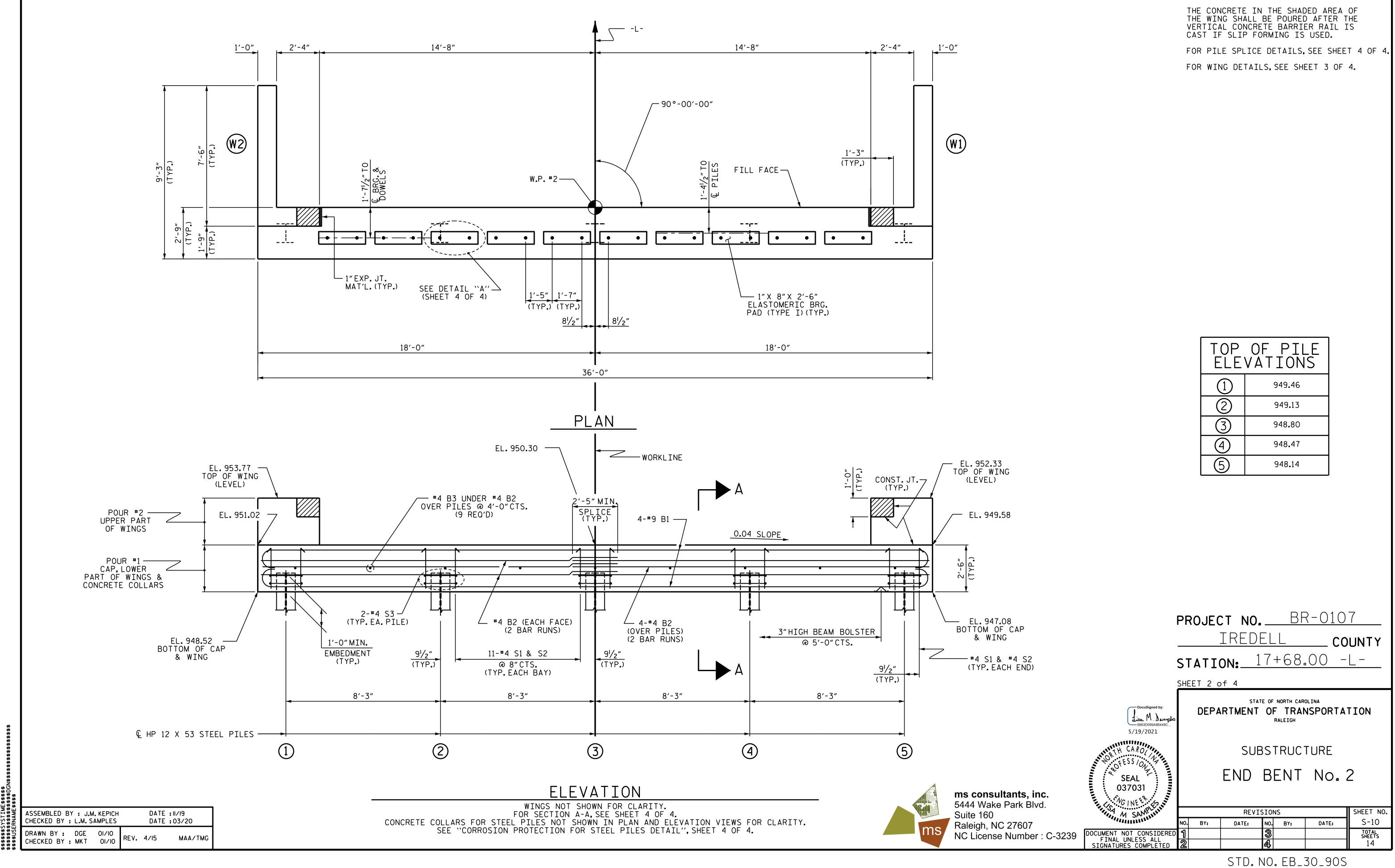


IME \$\$\$\$ \$\$\$\$\$\$[\$\$\$\$\$\$ \$\$\$\$\$\$ \$\$\$\$\$\$\$

TOP OF PILE ELEVATIONS				
	949.68			
2	949.35			
3	949.02			
4	948.69			
5	948.36			

STD. NO. EB_30_90S

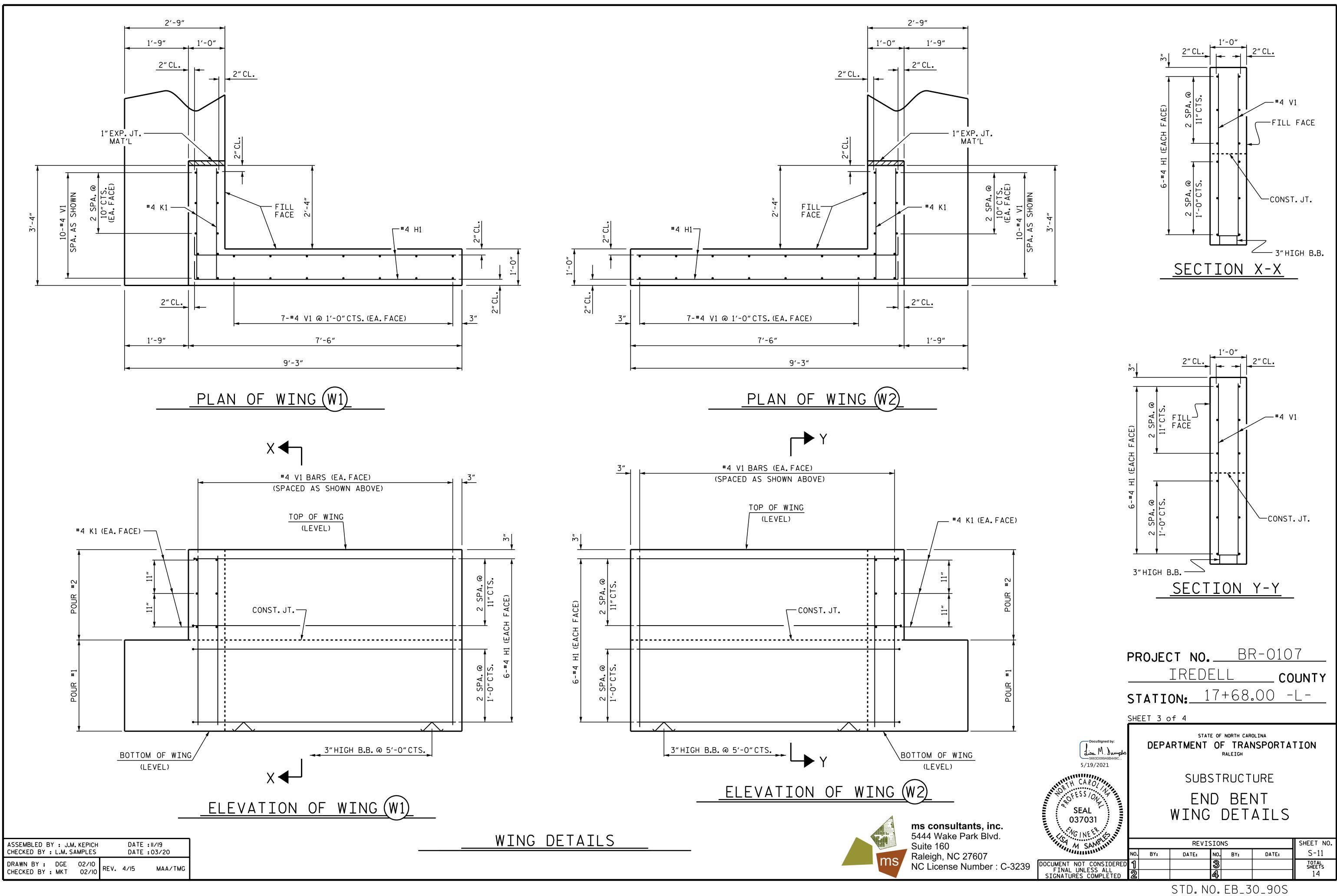
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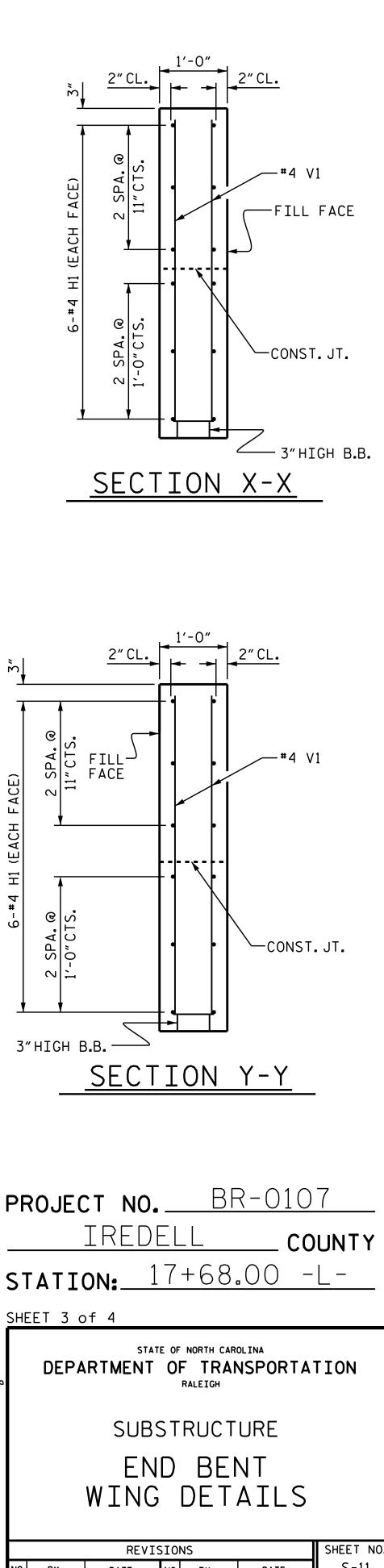
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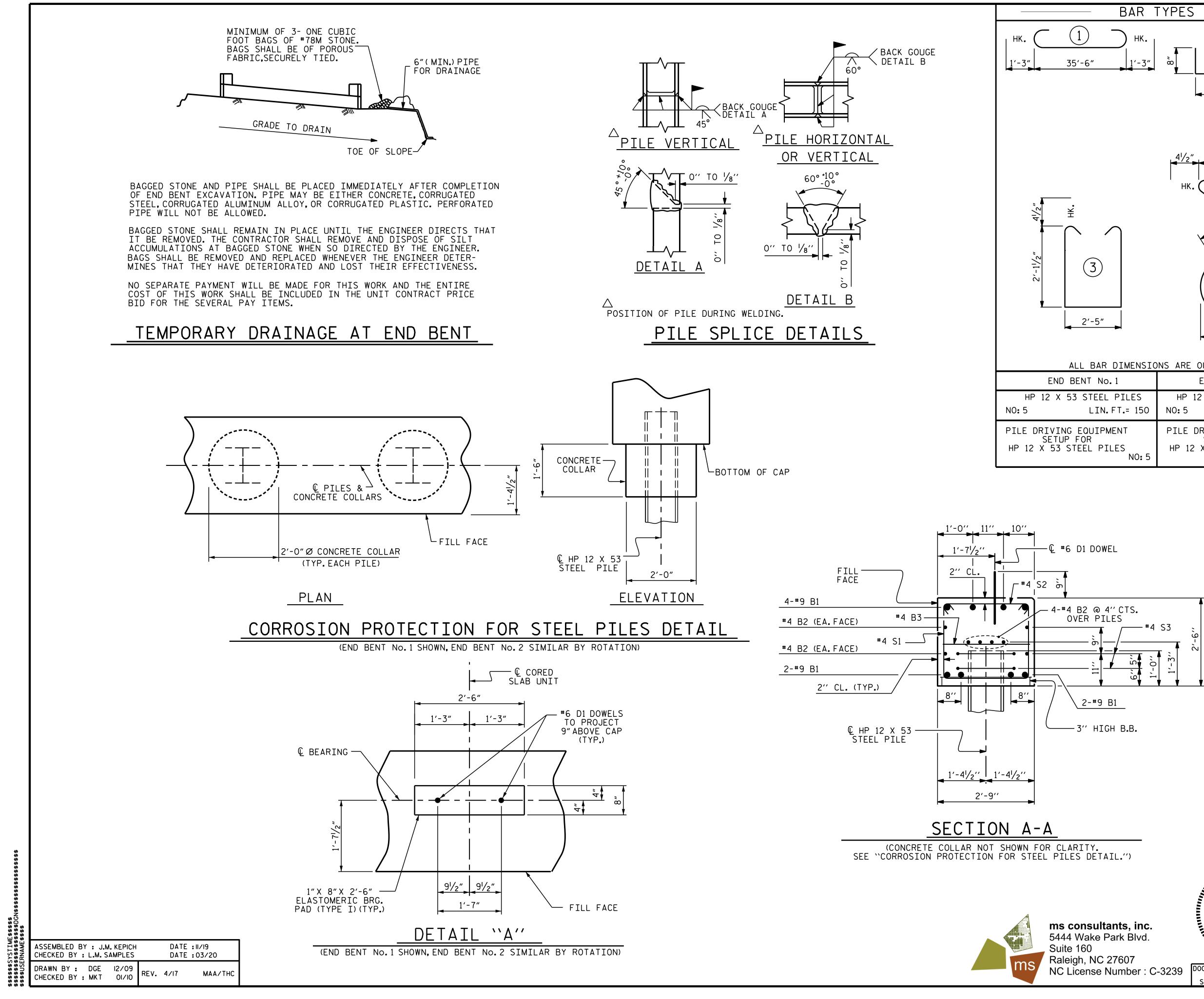
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

TOP OF PILE ELEVATIONS				
	949.46			
2	949.13			
3	948.80			
4	948.47			
5	948.14			





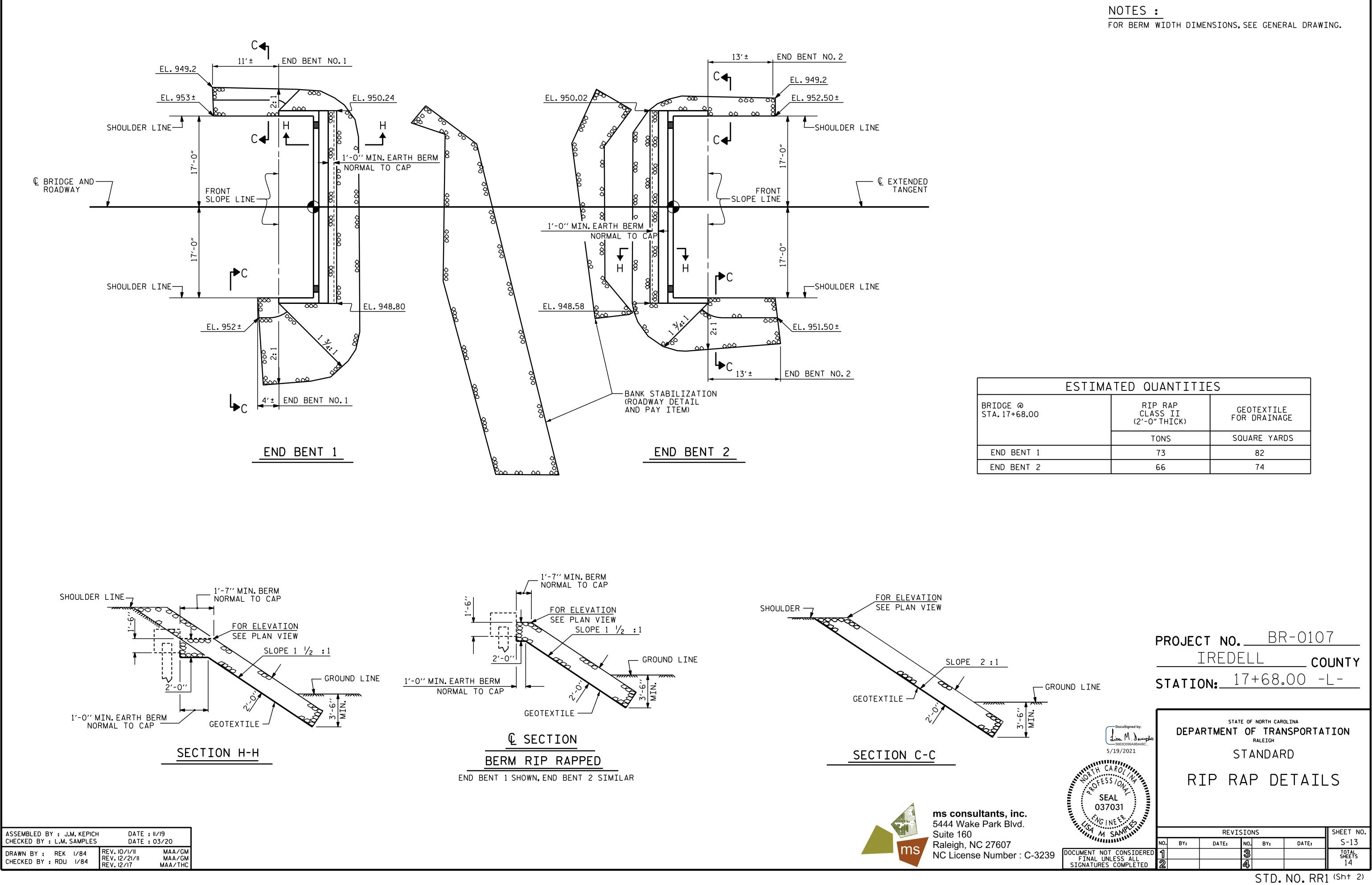




	1				ATERIA	
		FOF	R ON	IE E	ND BE	INT
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
1'-3" a (2)	B1	8	#9	1	38'-0"	1034
1'-3" œ	B2	16	#4	STR	19'-1"	204
	B3	9	#4	STR	2'-5″	15
7'-2"						
	D1	20	#6	STR	1'-6"	45
	H1	24	#4	2	7'-10"	126
				675	<u> </u>	0.7
4 ¹ /2" 2'-5" 4 ¹ /2"	К1	12	#4	STR	2'-11"	23
	<u> </u>	10	#4	7	7/ 5//	220
	S1	46 46	#4	3	7'-5" 3'-2"	228
HK. (4) (4) HK.	S2 S3	10	#4	4 5	6'-6"	97 43
(4)		10	··4	5	0-0	45
	V1	48	#4	STR	4'-8"	150
1'-3'' LAP				511	- 0	130
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) NCRETI	F BRF4	KDOWN	
			ONE ENI			
1′-8″Ø	POUR	# 1 C	AP, LOW	VER PA	RT	11.2 C.Y.
		0	F WINC	GS & C	COLLARS	
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STD.NO.EB_30_90S

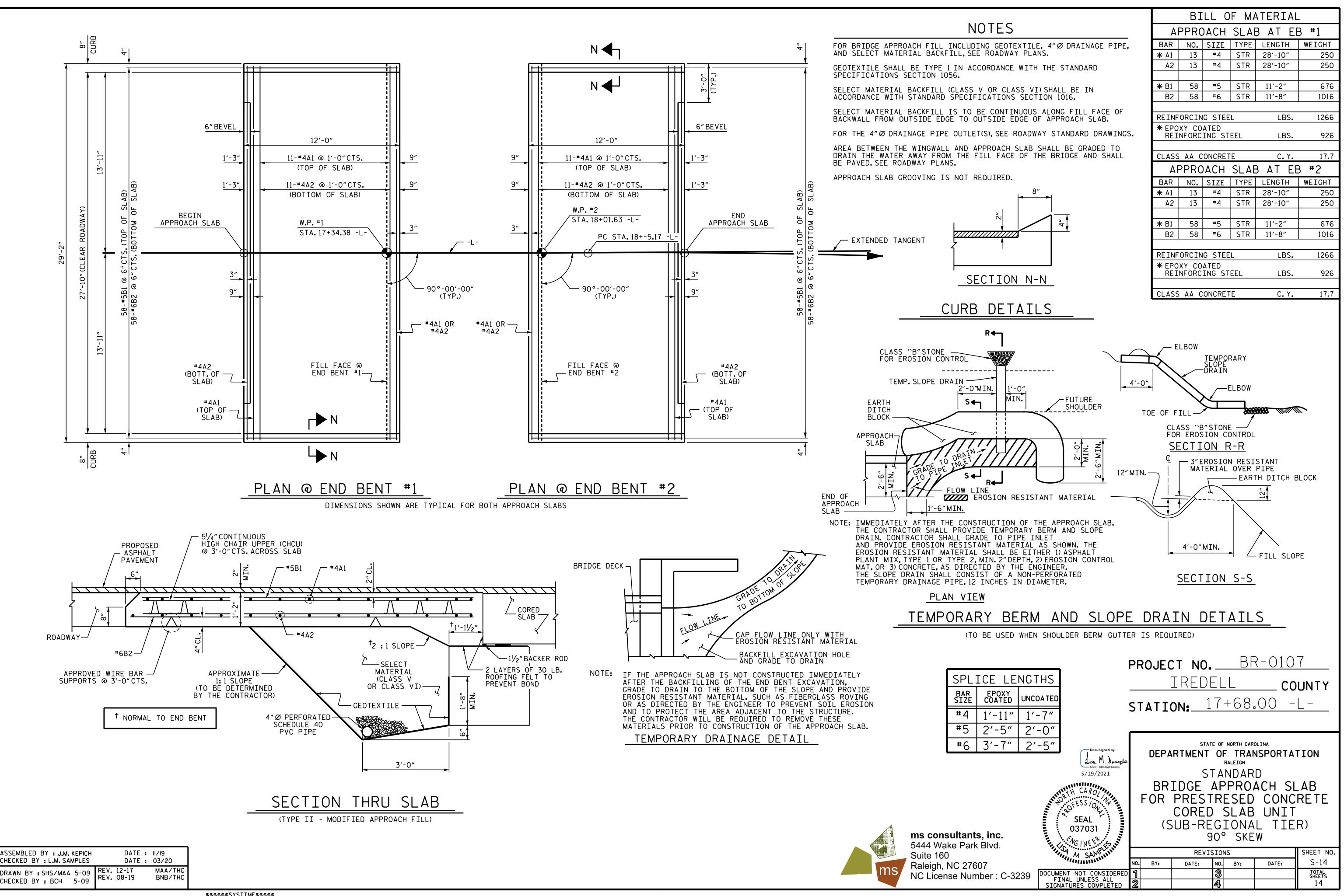


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NO.	TES	0				
FOR	BERM	WIDTH	DIMENSIONS,	SEE	GENERAL	DRAWING.

ESTIMATED QUANTITIES						
GE @ 17+68.00	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE				
	TONS	SQUARE YARDS				
D BENT 1	73	82				
) BENT 2	66	74				



\$\$\$\$\$\$\$YSTIME\$\$\$\$ \$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$ \$\$\$\$USERNAME\$\$\$

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KINAME	ASSEMBLED BY : J.M. KEPICH CHECKED BY : L.M. SAMPLES	DATE : 11/19 DATE : 03/20
	DRAWN BY :SHS/MAA 5-09 CHECKED BY : BCH 5-09	REV. 12-17 MAA/THC REV. 08-19 BNB/THC

STD. NO. BAS_30_90S

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)		
LIVE LOAD	SEE PLANS		
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.		
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.		
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.		
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.		
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.		
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.		
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.		
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS 1,800 LBS.PER SO.IN.			
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.		
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)		

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/2" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

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DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE ¾" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 1/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{16}$ " IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY V_{16} INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH JANUARY, 1990

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