5883 REFERENCE

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SHEET NO.

5-7

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3083

### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY \_HARNETT

PROJECT DESCRIPTION IMPROVE I-95 INTERCHANGES AT SR 1808 (JONESBORO RD.) AND SR 1709 (HODGES CHAPEL RD.)

SITE DESCRIPTION BRIDGE NO. 81 ON -Y2- (SR 1709) OVER -L- (I-95)

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTA SHEET
N.C.	I-5883	1	15

### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDING TO CLIMATIC CONDITIONS INCLORDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS, AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

E.G. BLONSHINE

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INVESTIGATED BY J.R. SWARTLEY

DRAWN BY \_J.R. SWARTLEY

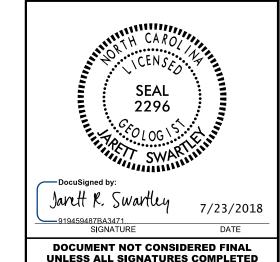
CHECKED BY \_\_S.S. LANEY

SUBMITTED BY S.S. LANEY

DATE \_FEBRUARY 2018



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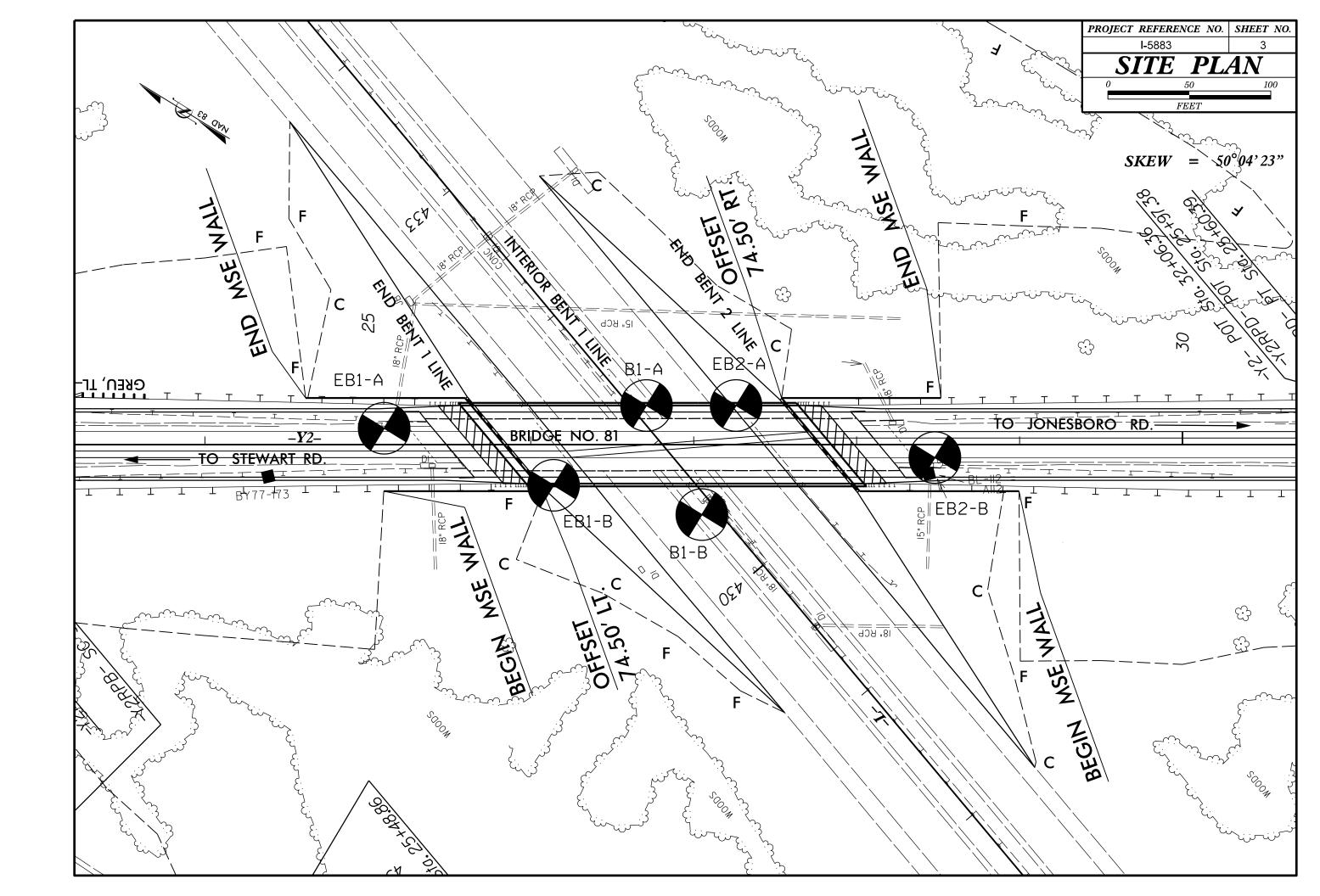
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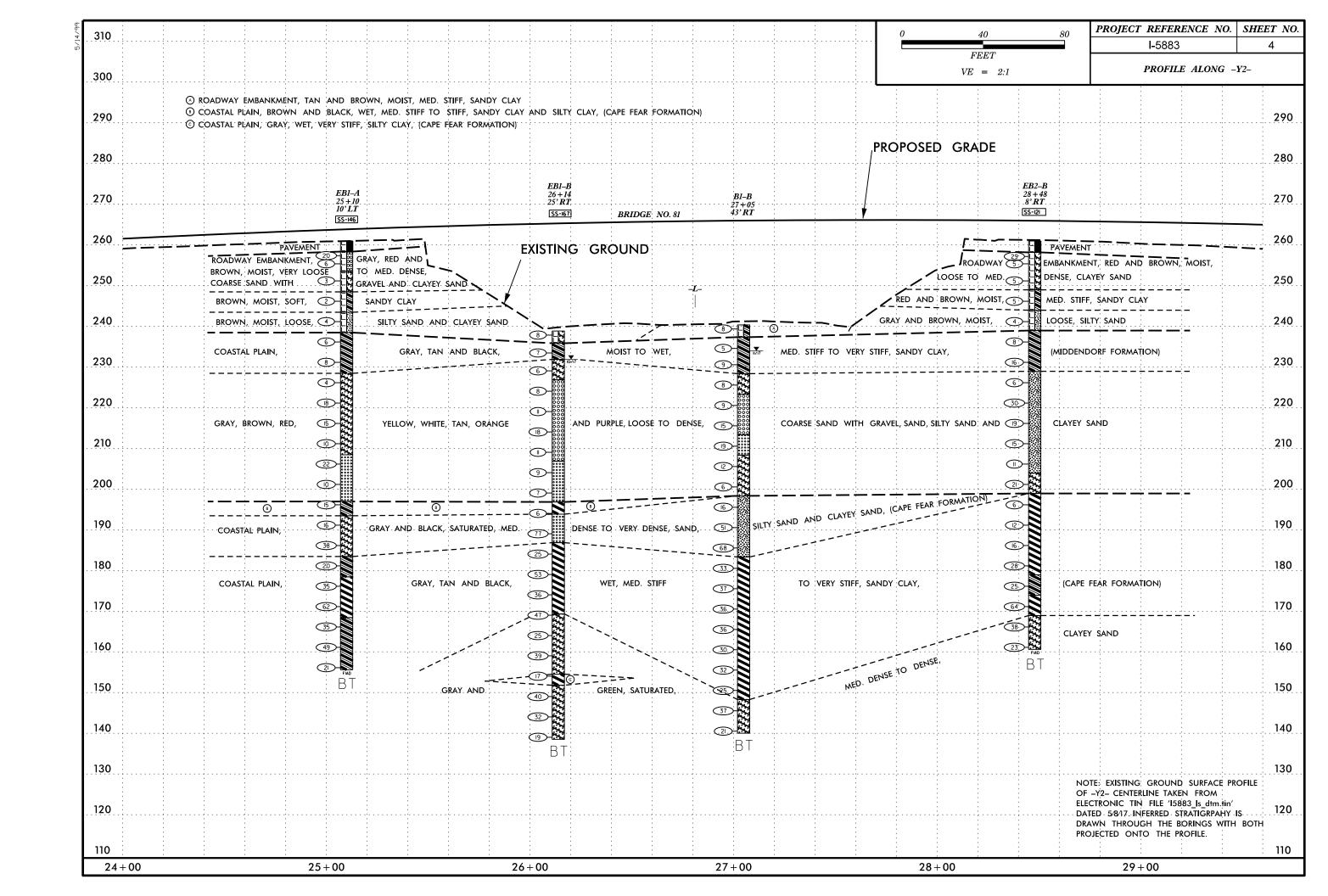
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

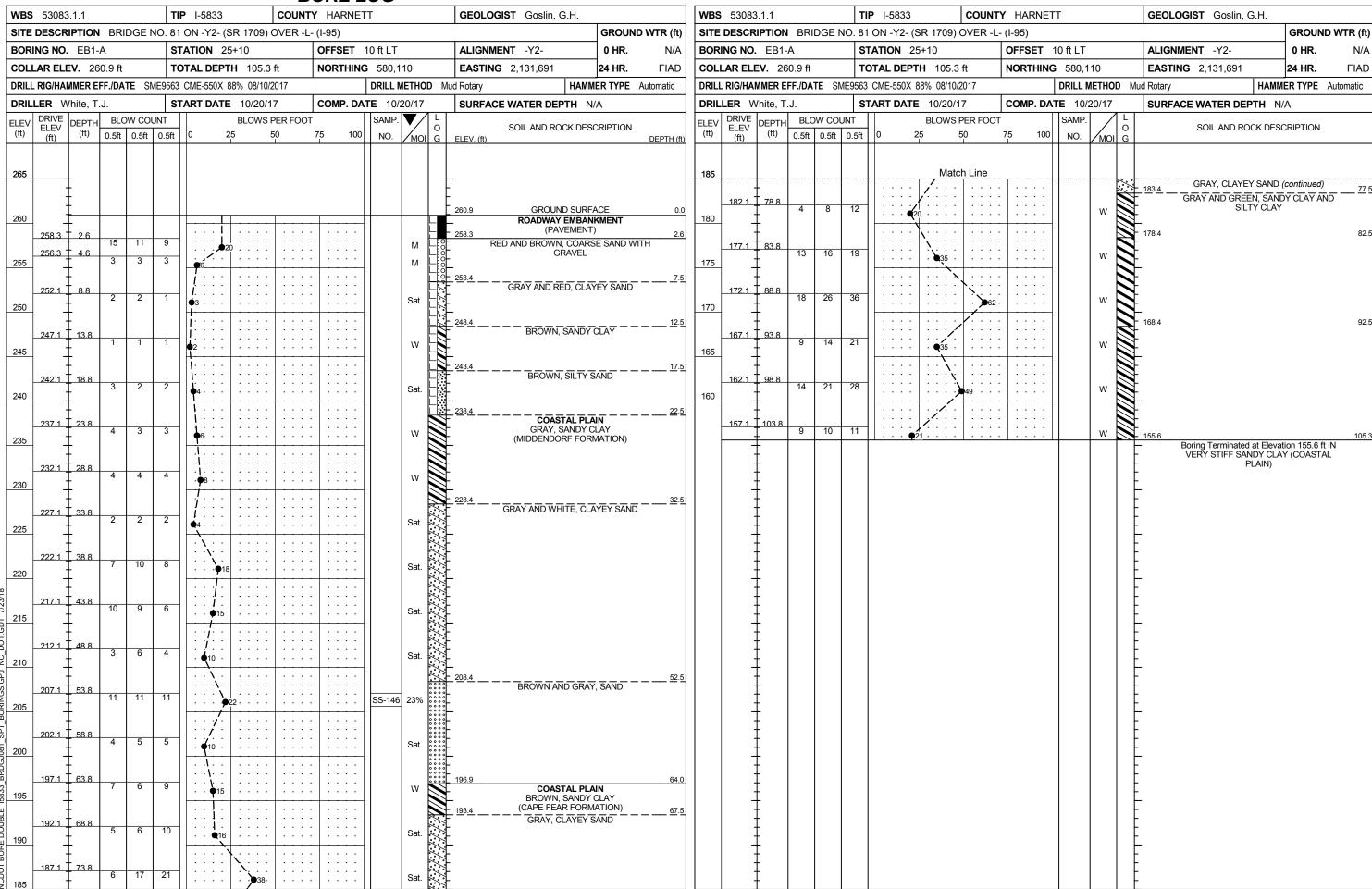
# SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

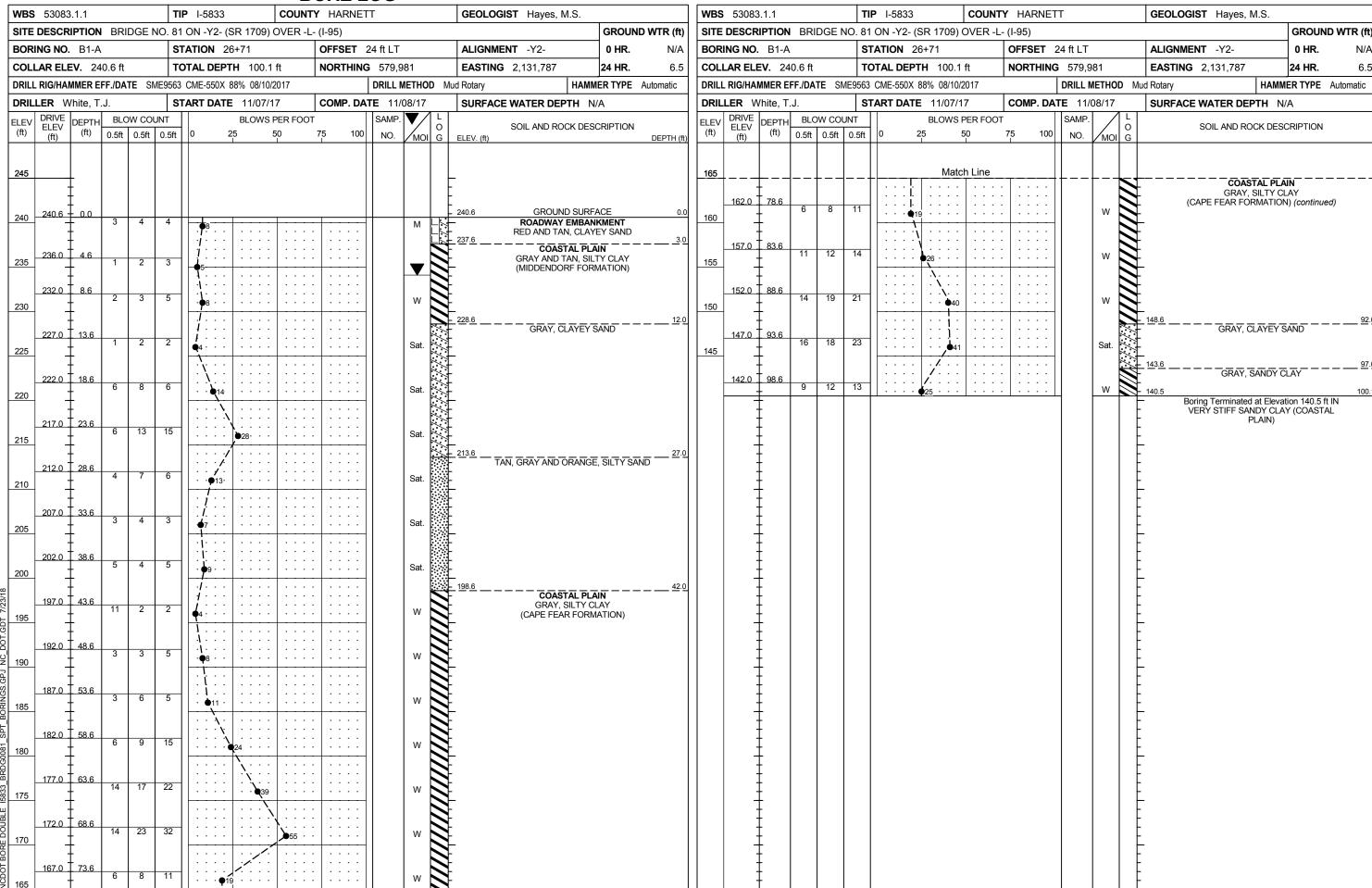
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF.GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.  ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
CENERAL CRANIII AR MATERIALS SILT-CLAY MATERIALS	MINERALOGICAL COMPOSITION	FINE TO COARSE CRAIN IGNEOUS AND METAMORPHIC ROCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (\$35% PASSING *200) (>35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	WOULD YIELD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE,	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	UNEISS, OHBBRU, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-6 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE ROCK (NCR) SEDIMENTARY ROCK THAT WOULD YELLD SET REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 0000 d00000 0000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
% PASSING SILT-	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED (CP) SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR CLAY MUCK, SOILS SOILS PEAT	PERCENTAGE OF MATERIAL	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*200   15 MX   25 MX   10 MX   35 MX   35 MX   35 MX   35 MX   35 MX   36 MN   36 MN   36 MN   36 MN   36 MN   36 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%  LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40 40 MX 41 MN	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN 11 MN MOREPATE HIGHLY	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOLIS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
OF MAJOR GRAYEL, AND SAND GRAYEL AND SAND SOILS SOILS	$lacktriangle$ STATIC WATER LEVEL AFTER $\underline{24}$ HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
CEN RATING FAIR TO		(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	PARENT MATERIAL.
AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30	-	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	FIELD.  JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
CONSISTENCY CONSISTENCY (N-VALUE) (TONS/FT <sup>2</sup> )	₩ITH SOIL DESCRIPTION → OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4	SOIL SYMBOL  SOIL SYMBOL  SPT ONT TEST BORING  SLOPE INDICATOR INSTALLATION	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR LOOSE 4 TO 10 GRANULAR MEDIUM DENSE 10 TO 30 N/A	RT .	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS
MATERIAL DENSE 30 TO 50 (NON-COHESIVE)	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT  AUGER BORING  CONE PENETROMETER TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY DENSE   > 50	INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5	MW - TEST DODING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY   MEDIUM STIFF   4 TO 8   0.5 TO 1.0   MATERIAL   STIFF   8 TO 15   1 TO 2	INFERRED ROCK LINE MONITORING WELL WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4	→▼→→→→ ALLUVIAL SOIL BOUNDARY △ PIEZOMETER INSTALLATION ── SPT N-VALUE	ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
HARD > 30 > 4		ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
TEVILLE OF CDAIN CIZE	DECOMMENDATION CAMBOLO		
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNCLASSIFIED EXCAVATION - TOTAL UNCLASSIFIED EXCAVATION -	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
U.S. STD. SIEVE SIZE 4 10 40 60 200 270  OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT 10 BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	ROCK.
U.S. STD. SIEVE SIZE	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	ROCK, <u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS, <u>SLICKENSIDE</u> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
U.S. STD. SIEVE SIZE 4 10 40 60 200 270  OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053  BOULDER CORRE GRAVEL COARSE FINE SULT CLAY	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK  ACCEPTABLE DEGRADABLE ROCK  ABBREVIATIONS  AR - AUGER REFUSAL  MED MEDIUM  VST - VANE SHEAR TEST	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BORING TEMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7- UNIT WEIGHT	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY  CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPP) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
U.S. STD. SIEVE   SIZE   4	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL BT - BORING TERMINATED MICA MICACEOUS CL CLAY MOD MODERATELY CPT - CONE PENETRATION TEST  WEA WEATHERED  7 - UNIT WEIGHT  7 - UNIT WEIGHT	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY  CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPP) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM STANDARD	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT CT CONE PENETRATION TEST NP - NON PLASTIC CSE COARSE ORG ORGANIC DMT - DULATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK SCREEN TO THE TOP	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETRACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SILCKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH DUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SPEC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM STANDARD	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETRACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
U.S. STD. SIEVE SIZE	UNDERCUT UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST WEA WEATHERED CL CLAY MOD MODERATELY CT UNIT WEIGHT CT - CONE PENETRATION TEST NP - NON PLASTIC CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC E - VOID RATIO F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  CAN BE GROOVED OR GOUGED 0.65 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SILTKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE (SPT) - NUMBER OF BLOWS IN OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
U.S. STD. SIEVE SIZE  4 10 40 60 200 270  OPENING (MM)  4.76 2.00 0.42 0.25 0.075 0.053  BOULDER (BLDR.) (COB.) (GR.) (GR.) (CSE. SD.) (FSD.) (FSD.) (SL.) (CL.)  GRAIN MM 305 75 2.0 0.25 0.05 0.005  SIZE IN. 12 3  SOIL MOISTURE - CORRELATION OF TERMS  SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION  CSATURATED - USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE  PLASTIC BRANCE - WET - (M) SEMISOLID; REQUIRES DRYING TO	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - CORPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM VST VANE SHEAR TEST WEA WEATHERED CL CLAY MOD MODERATELY CPT - COME PENETRATION TEST NP - NON PLASTIC CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAP SAPROLITIC SPICE OF THE SAP SAPROLITIC SC SULK S SPLIT SPOON F FINE SULK SULT SHELBY TUBE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY HARD  CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES I INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE.	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS,  SILTKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - CORETABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST MICA MICACEOUS MEA WEATHERD CL CLAY MOD MODERATELY CPT - CONE PENETRATION TEST NP - NON PLASTIC CSC COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK S - SPLIT SPOON F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SL SILT, SILTY RS - SHOCK PRACTURES TER - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETRACH HAND SPECIMEN.  MODERATELY HARD  EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.  FRACTURE SPACING  IERM  SPACING  IERM  THICKNESS  THICKNESS  ITEM  THICKNESS	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  JOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY77-173
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED C.L CLAY MOD MODERATELY CPT - CONE PENETRATION TEST OMC ORGANIC OMT - DILATOMETER TEST DMT - PRESSUREMETER TEST DMT - DYNAMIC PENETRATION TEST DPT - DYNAMIC PENETRATION TEST DPT - DYNAMIC PENETRATION TEST DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S SBLK S SSLIT SPOON F - FINE S SILT, SILTY FRESC FRACTURED, FRACTURES TCR - TRICONE REFUSAL READER UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  VST - VANE SHEAR TEST WEA WEATHERED Y- UNIT WEIGHT Y- DRY UNIT WEIGHT S - BULK S - BULK S - SHLIT SPOON ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI - SLIGHTLY RES - ROCK FRACTURED, FRACTURES TCR - TRICONE REFUSAL TRICOME TOWN CORR - CALIFORNIA BEARING	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY HARD  CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A CEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES I INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERWAIL.  FRACTURE SPACING  BEDDING  TERM  VERY WIDE  MORE THAN 10 FEET  VERY THICKLY BEDDED  4 FEET	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SILKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH DUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF STRATA MOS PERPRESSED AS A PERCENTAGE.  TOPSOIL (15.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY77-173  NORTHING: 580156 EASTING: 2131628
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U.S. STD. SIEVE SIZE	UNDERCUT  UNDERCUT  UNDERCUT  UNDERCUT  UNDERCUT  UNDERCUT  UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL BT - BORING TERMINATED  MICA MICACEOUS  MEA WEATHERED  CL CLAY  MOD MODERATELY  CT - CONE PENETRATION TEST  NP - NON PLASTIC CSE COARSE  ORG ORGANIC  DMT - DILATOMETER TEST  DMT - DYNAMIC PENETRATION TEST  SAP, - SAPROLITIC  S - BULK  S - SPLIT SPOON  FP - FINE  SL SILT, SILTY  FOSS FOSSILIFEROUS  SLI - SLIT, SILTY  FRAGS FRAGMENTS  W - MOISTURE CONTENT  RATIO  EQUIPMENT USED ON SUBJECT PROJECT  DRILL UNITS:  ADVANCING TOOLS:  WALL TOWARD ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  WEA WEATHERED  WEA WEATHERED  WEA WEATHERED  WEA WEATHERED  Y - UNIT WEIGHT  Y - UNIT WEIGHT  S - BULK  S - SPLIT SPOON  SS - SPLIT SPOON  SS - SPLIT SPOON  FRAC FRACTURED, FRACTURES  TCR - TRICONE REFUSAL  RT - RECOMPACTED TRIAXIAL  CBR - CALIFORNIA BEARING  HIL - HIGHLY  V - VERY  RATIO  MANUAL	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY HARD  CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.  FRACTURE SPACING  TERM  VERY WIDE  MORE THAN 10 FEET  VERY THICKLY BEDDED  1.5 - 4 FEET  MODERATELY CLOSE  1 TO 3 FEET  THINKLY BEDDED  0.03 - 0.16 FEET  VERY THINLY BEDDED  0.06 - 1.5 FEET  VERY THINLY BEDDED  0.06 - 1.5 FEET	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SILKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH DUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF STRATA MOS PERPRESSED AS A PERCENTAGE.  TOPSOIL (15.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY77-173  NORTHING: 580156 EASTING: 2131628
U.S. STD. SIEVE SIZE	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY CPT - CONE PENETRATION TEST OMT - DILATOMETER TEST DMT - PRESSURMETER TEST DMT - DILATOMETER TEST DMT - DYNAMIC PENETRATION TEST SAP SAPROLITIC E - VOID RATIO F - FINE SL SILT, SILTY FFINE SL SILT, SILTY FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS M - MOISTURE CONTENT HI HIGHLY  EQUIPMENT USED ON SUBJECT PROJECT  MANUAL  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE OPD 3 FEET OF EMBANKMENT OR BACKFILL  VST - VANE SHEAR TEST WEA WEATHERED FO SUMMER TEST SAMPLE ABBREVIATIONS S - BULK S - BULK S - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS M - MOISTURE CONTENT H HIGHLY V - VERY  DRILL UNITS: ADVANCING TOOLS:  MANUAL  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT TO TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  VST - VANE SHEAR TEST WEA WEATHERED FO SAMPLE ABBREVIATIONS SAMPLE ABBREVIATIONS S - BULK S - SPLIT SPOON S - SAMPLE ABBREVIATIONS S - SPLIT SPOON S - SENDER TO - VERT SPOON S - VI	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETRICH HAND SPECIMEN.  MODERATELY HARD  EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.  FRACTURE SPACING  IERM  SPACING  VERY WIDE  MORE THAN 10 FEET  WIDE  J THICKNESS  J LERM  THICKNESS  VERY WIDE  MORE THAN 10 FEET  THICKLY BEDDED  J LE THELE  MODERATELY CLOSE  J LE THICKLY BEDDED  J LE T LEST  THE THICK BEDSED  J LE	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  BENCH MARK: BY77-173  NORTHING: 580156 EASTING: 2131628  ELEVATION: 259.26 FEET
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U.S. STD. SIEVE SIZE	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL BT - BORING TERMINATED  MICA MICACEOUS  WEA WEATHERED  CL CLAY  MOD MODERATELY CPT - COME PENETRATION TEST  OMT - DILATOMETER TEST  DMT - SAPPOLITIC  e - VOID RATIO  F - FINE  SL SILT, SILTY  FOSS FOSSILIFEROUS  SLI SILT, SILTY  FOSS FRACTURED, FRACTURES  TCR - TRICTOME REFUSAL  FRACS FRACTURED, FRACTURES  TCR - TRICTOME REFUSAL  FRAGS FRAGMENTS  W - MOISTURE CONTENT  V - VERY  RATIO   CME-45C  CLAY BITS  WANDLE ABBREVIATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  WEA VEATHERED  WEA WEATHERED  WEA WEATHERED  WEA WEATHERET  WEA WEATHERED  WEA WEATHERET  WEA WEATHERET  WEA WEATHERET  TOP - VANCE  WEA WEATHERET  WEA WEATHERET  WEA WEATHERET  TEST  MAPL - BEBREVIATIONS  S BULK  S BULK  S BULK  S BULK  S BULK  S SHIT SPOON  SS - SPLIT SPOON  SS - SPLIT SPOON  SS - SPLIT SPOON  TO SHEED  TO SHEED  TO SHEED  TO SHEED  TO SHEED  TO TO BE MBANKMENT OR BACKFILL  WEA WEATHERET  WEA WEATHERET  WEA WEATHERET  TO BE MBANKMENT OR BACKFILL  WEA WEATHERET  WEA WEATHERET  TO SHEBANKMENT OR BACKFILL  WEA WEATHERET  WEA WEATHERET  TO SHEBANKMENT OR BACKFILL  WEA WEATHERET  SEMBANKMENT OR BACKFILL  WEA WEATHERET  SEMBANKMENT OR BACKFILL  WEA WEATHERET  TO SHEBANKMENT OR BACKFILL  WEA WEATHERET  WEA WEATHERET  TO SHEBANKMENT OR BACKFILL  WEA WEATHERET  TO SHEBANKMENT  WEA WEATHERET  TO SHEBANKMENT  WEA	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED DR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.  FRACTURE SPACING  TERM SPACING  VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 1.5 - 4 FEET THICKLY BEDDED 0.15 - 15 FEET THICKLY BEDDED 0.06 - 1.5 FEET THICKLY BEDDED 0.08 - 1.5 FEET THICKLY BEDDED 0.08 - 1.5 FEET THICKLY LAMINATED 0.0008 - 0.03 - 0.16 FEET THICKLY LAMINATED 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.000	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SILTKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPI) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  DPSOIL (TIS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY 77-173  NORTHING: 580156 EASTING: 2131628  ELEVATION: 259.26 FEET
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U.S. STD. SIEVE SIZE	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL BT - BORING TERMINATED MICA MICACCOUS WEA WEATHERED C.L CLAY MOD MEDIUM WST - VANE SHEAR TEST WEA WEATHERED C.L CLAY CPT - CONE PENETRATION TEST NP - NON PLASTIC CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST DPT - PRESSUREMETER TEST DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S SBULK S SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS S.L SILT, SILTY FRACTURED, FRACTURES TCR - TRICONE REFUSAL RAGSS FRAGMENTS W - MOISTURE CONTENT HI HIGHLY  DRILL UNITS:  CME-45C  CME-45C  CME-550  MANUAL  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT TO TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL WEA WEATHERED WEA WEATHERED WEA WEATHERED WEA WEATHERED WEA WEATHERED TO BUSED IN SHEARTLY WEA WEATHERED S WEAL - WEATHERED S SHAPLE ABBREVIATIONS S - BULK S - SPLIT SPOON S - SP	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED DR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.  FRACTURE SPACING  TERM SPACING  VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 1.5 - 4 FEET THICKLY BEDDED 0.15 - 15 FEET THICKLY BEDDED 0.06 - 1.5 FEET THICKLY BEDDED 0.08 - 1.5 FEET THICKLY BEDDED 0.08 - 1.5 FEET THICKLY LAMINATED 0.0008 - 0.03 - 0.16 FEET THICKLY LAMINATED 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.000	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SILTKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPI) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  DPSOIL (TIS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY 77-173  NORTHING: 580156 EASTING: 2131628  ELEVATION: 259.26 FEET
U.S. STD. SIEVE SIZE	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - UNDERCUT  UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL BT - BORING TERMINATED  MICA MICACEOUS  WEA WEATHERED CL CLAY  MOD MODERATELY CPT - COME PENETRATION TEST OMG ORGANIC CSE COARSE OMT - DILATOMETER TEST DPT - PRESSUREMETER TEST OPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC e - VOID RATIO F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY FRACT FRACTURED, FRACTURES TCR - TRICONE REFUSAL FRAGS FRAGMENTS HI HIGHLY  EQUIPMENT USED ON SUBJECT PROJECT  DRILL UNITS:  DRILL UNITS:  CME-45C  CLAY BITS  WAND SANDY SS - SOME TINGG-CARBUE  HAND TOOLS:  HAND TOOLS:  HAND TOOLS:  HAND TOOLS:  POST HOLE DIGGER HAND TOOLS:  HAND TOOLS:  POST HOLE DIGGER HAND AUGER  POST HOLE DIGGER HAND AUGER  TINGG-CARBUE CRUENTED CONTINUES CONTENT  WAND AUGER  POST HOLE DIGGER HAND AUGER  TRICONE TINGG-CARBUE CRUENTED CONTENT  WAND AUGER  POST HOLE DIGGER HAND AUGER  TRICONE TINGG-CARBUE CRUENTED CONTENT  CRUE-550  TRICONE TINGG-CARBUE TINGG-CARBUE CRUENTED CONTENT  CRUENTED CONTENT  ACCEPTABLE, BUT TO TO BE USED IN TOOLS:  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  SAMPLE ABBREVIATIONS  WEA WEATHERST  SAMPLE ABBREVIATIONS  VET VANE SHEAR TEST  WEA WEATHERST  SAMPLE ABBREVIATIONS  VET VANE SHEAR TEST  WEA WEATHERST  SAMPLE ABBREVIATIONS  VET VANE SHEAR TEST  WEA WEATHERST  UNCLASSIFIED  UNCLASSIFIED  VST VANE SHEAR TEST OF EMBACKFILL  WEA WEATHERST  SAMPLE ABBREVIATIONS  SET - VANE SHEAR TEST OF EMBACKFILL  WEA WEATHERST  SAMPLE ABBREVIATIONS  SET - VANE SHEAR TEST OF EMBACKFILL  WEA WEATHERST  WEA WEATHERST  SAMPLE ABBREVIATIONS  SS SPLIT SPOON  SS SPLIT SPOON  SS SPLIT SPOON  SS SPLIT SHOW  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  WEA WEA	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY  MODERATELY  CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A CEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERWAIL.  FRACTURE SPACING  TEM  VERY WIDE  MORE THAN 10 FEET  VERY WIDE  MORE THAN 10 FEET  WIDE  3 TO 10 FEET  MODERATELY CLOSE  0.16 TO 1 FOOT  VERY THICKLY BEDDED  1.5 - 4 FEET  WIDE  WERY CLOSE  0.16 TO 1 FOOT  VERY THINLY BEDDED  0.03 - 0.16 FEET  THICKLY LAMINATED  COMB - 0.03 FEET  THICKLY LAMINATED  COMB FEET  THICKLY LAMINATED  COMB - 0.03 FEET  THICKLY LAMINATED  COMB FEET  INDURATION  FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.  FRIABLE  RUBBING WITH FINGER FREES NUMEROUS GRAINS;  GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.  GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;  BREAKS EASILY WHEN HIT WITH HAMMER.  INDURATED  CRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SILTKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPI) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  DPSOIL (TIS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY 77-173  NORTHING: 580156 EASTING: 2131628  ELEVATION: 259.26 FEET
U.S. STD. SIEVE SIZE	UNDERCUT UNDERCUT UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNDERCUT UNDERCUT UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ACCEPTABLE PROCK  ACCEPTABLE PROCK  ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  VST - VANE SHEAR TEST WEA WEATHERED WEA WEATHERED Y - UNIT WEIGHT Y - UNIT WEIGHT Y - DRY UNIT WEIGHT Y - DRY UNIT WEIGHT ONG ORGANIC OMT - DILATOMETER TEST PMT - PRESSUREMETER TEST DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC OMT - DILATOMETER TEST PMT - PRESSUREMETER TEST DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC OF - FINE SL SILT, SILTY FOSS FOSSILIFEROUS SL SILT, SILTY FOSS FOSSILIFEROUS SL SILT, SILTY FRACT FRACTURED, FRACTURES TCR - TRICONE REFUSAL FRACS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING RATIO  EQUIPMENT USED ON SUBJECT  DRILL UNITS:  ADVANCING TOOLS:  WANDENSOR WANDERS  WANDERSTS  AUTOMATIC MANUAL  CME-550 HARD FACED FINGER BITS  CME-550 TUNGCARBIDE INSERTS  X CASING W ADVANCER HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETRICH HAND SPECIMEN.  MODERATELY HARD  EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD  CAN BE GROOVED OR GOUGED REDILY BY KNIFE OR PICK, CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED REDILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT  OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.  FRACTURE SPACING  IERM  SPACING  VERY WIDE  MORE THAN 10 FEET  WIDE  VERY WIDE  MODERATELY CLOSE  1 TO 3 FEET  THICKLY BEDDED  1.5 - 4 FEET  WIDE  MODERATELY CLOSE  1 TO 3 FEET  THICKLY BEDDED  2.66 - 1.5 FEET  CLOSE  CALLSE LESS THAN 0.16 FEET  THICKLY LAMINATED  FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.  RUBBING WITH FINGER FREES NUMBEROUS GRAINS;  GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.  MODERATELY INDURATED  GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;  BREAKS EASILY WHEN HIT WITH HAMMER.	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SILTKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPI) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  DPSOIL (TIS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY 77-173  NORTHING: 580156 EASTING: 2131628  ELEVATION: 259.26 FEET
U.S. STD. SIEVE SIZE	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - UNDERCUT  UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL BT - BORING TERMINATED  MICA MICACEOUS  WEA WEATHERED CL CLAY  MOD MODERATELY CPT - COME PENETRATION TEST OMG ORGANIC CSE COARSE OMT - DILATOMETER TEST DPT - PRESSUREMETER TEST OPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC e - VOID RATIO F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY FRACT FRACTURED, FRACTURES TCR - TRICONE REFUSAL FRAGS FRAGMENTS HI HIGHLY  EQUIPMENT USED ON SUBJECT PROJECT  DRILL UNITS:  DRILL UNITS:  CME-45C  CLAY BITS  WAND SANDY SS - SOME TINGG-CARBUE  HAND TOOLS:  HAND TOOLS:  HAND TOOLS:  HAND TOOLS:  POST HOLE DIGGER HAND TOOLS:  HAND TOOLS:  POST HOLE DIGGER HAND AUGER  POST HOLE DIGGER HAND AUGER  TINGG-CARBUE CRUENTED CONTINUES CONTENT  WAND AUGER  POST HOLE DIGGER HAND AUGER  TRICONE TINGG-CARBUE CRUENTED CONTENT  WAND AUGER  POST HOLE DIGGER HAND AUGER  TRICONE TINGG-CARBUE CRUENTED CONTENT  CRUE-550  TRICONE TINGG-CARBUE TINGG-CARBUE CRUENTED CONTENT  CRUENTED CONTENT  ACCEPTABLE, BUT TO TO BE USED IN TOOLS:  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  SAMPLE ABBREVIATIONS  WEA WEATHERST  SAMPLE ABBREVIATIONS  VET VANE SHEAR TEST  WEA WEATHERST  SAMPLE ABBREVIATIONS  VET VANE SHEAR TEST  WEA WEATHERST  SAMPLE ABBREVIATIONS  VET VANE SHEAR TEST  WEA WEATHERST  UNCLASSIFIED  UNCLASSIFIED  VST VANE SHEAR TEST OF EMBACKFILL  WEA WEATHERST  SAMPLE ABBREVIATIONS  SET - VANE SHEAR TEST OF EMBACKFILL  WEA WEATHERST  SAMPLE ABBREVIATIONS  SET - VANE SHEAR TEST OF EMBACKFILL  WEA WEATHERST  WEA WEATHERST  SAMPLE ABBREVIATIONS  SS SPLIT SPOON  SS SPLIT SPOON  SS SPLIT SPOON  SS SPLIT SHOW  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  WEA WEATHERST  WEA WEA	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY  MODERATELY  CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A CEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERWAIL.  FRACTURE SPACING  TEM  VERY WIDE  MORE THAN 10 FEET  VERY WIDE  MORE THAN 10 FEET  WIDE  3 TO 10 FEET  MODERATELY CLOSE  0.16 TO 1 FOOT  VERY THICKLY BEDDED  1.5 - 4 FEET  WIDE  WERY CLOSE  0.16 TO 1 FOOT  VERY THINLY BEDDED  0.03 - 0.16 FEET  THICKLY LAMINATED  COMB - 0.03 FEET  THICKLY LAMINATED  COMB FEET  THICKLY LAMINATED  COMB - 0.03 FEET  THICKLY LAMINATED  COMB FEET  INDURATION  FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.  FRIABLE  RUBBING WITH FINGER FREES NUMEROUS GRAINS;  GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.  GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;  BREAKS EASILY WHEN HIT WITH HAMMER.  INDURATED  CRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	ROCK,  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  BENCH MARK: BY77-173  NORTHING: 580156 EASTING: 2131628  ELEVATION: 259.26 FEET

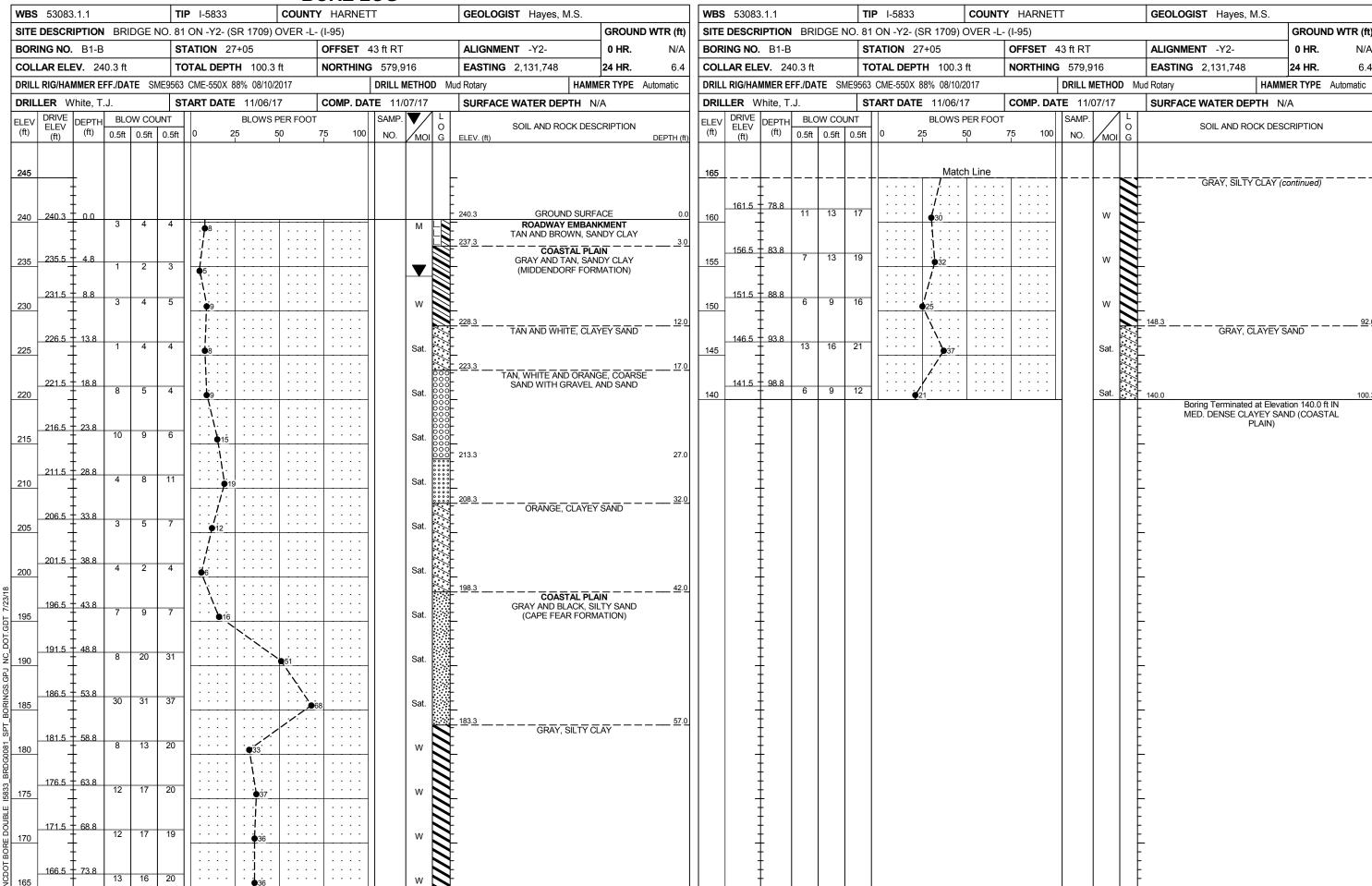




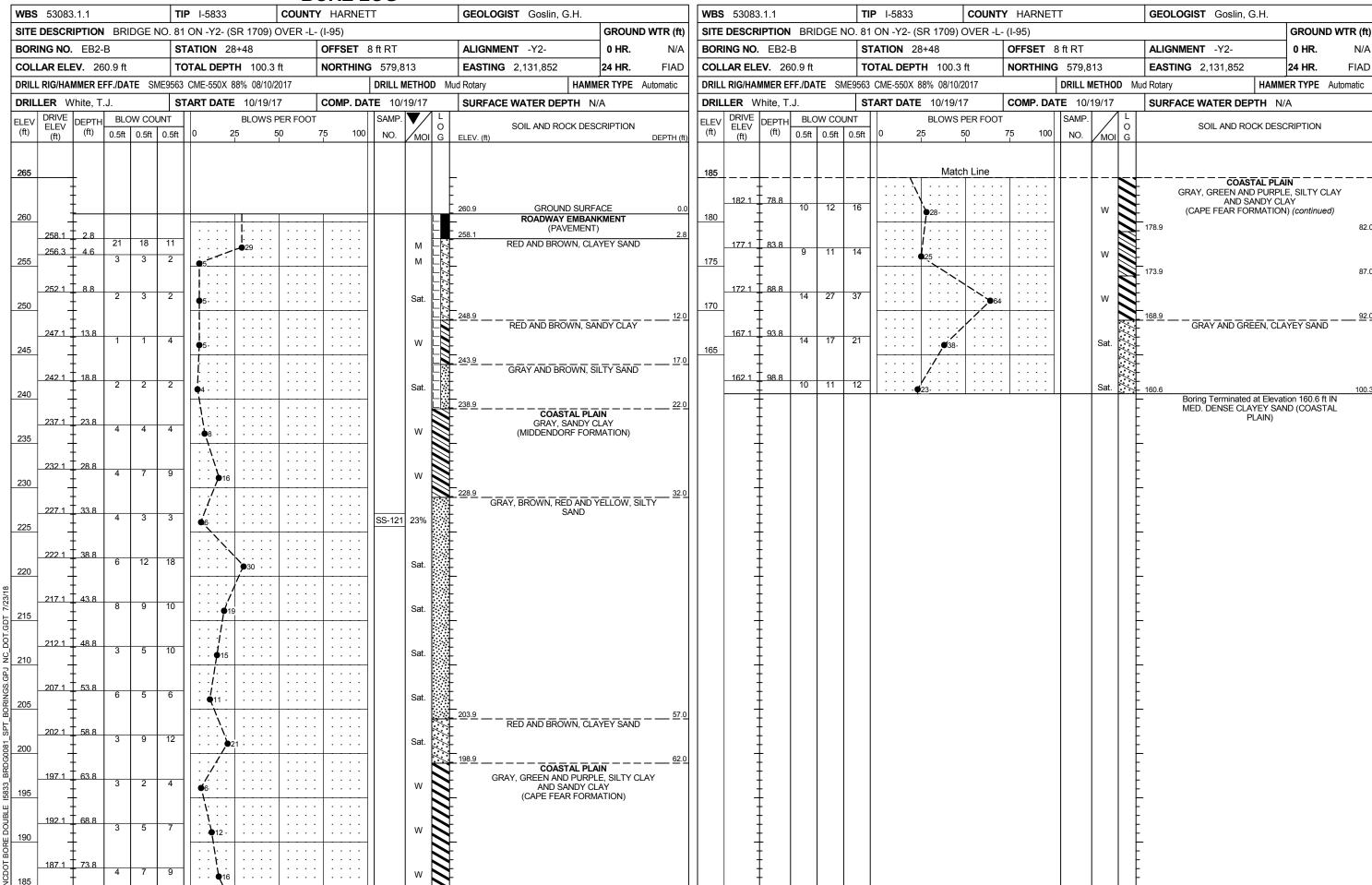


BURE LUG	DECLOSION DI LI ES	TID 1	LEGGG LIADVETT	AFOLOGIOT BL. 11 F.O.
WBS 53083.1.1 TIP I-5833 COUNTY HARNETT	GEOLOGIST Blonshine, E.G.	WBS 53083.1.1 TIP I-		GEOLOGIST Blonshine, E.G.
SITE DESCRIPTION BRIDGE NO. 81 ON -Y2- (SR 1709) OVER -L- (I-95)	GROUND WTR (ft)	SITE DESCRIPTION BRIDGE NO. 81 ON		GROUND WTR (ft)
BORING NO. EB1-B STATION 26+14 OFFSET 25 ft RT	ALIGNMENT -Y2- 0 HR. N/A		ION         26+14         OFFSET         25 ft RT	ALIGNMENT -Y2- 0 HR. N/A
COLLAR ELEV.         238.8 ft         TOTAL DEPTH         100.3 ft         NORTHING         580,003	<b>EASTING</b> 2,131,715 <b>24 HR.</b> 7.0	COLLAR ELEV. 238.8 ft TOTAL	L DEPTH 100.3 ft NORTHING 580,003	<b>EASTING</b> 2,131,715 <b>24 HR.</b> 7.0
DRILL RIG/HAMMER EFF./DATE SME9563 CME-550X 88% 08/10/2017 DRILL METHOD MIL	d Rotary HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE SME9563 CME	E-550X 88% 08/10/2017 DRILL METHOD M	Mud Rotary HAMMER TYPE Automatic
DRILLER White, T.J. START DATE 10/23/17 COMP. DATE 10/24/17	SURFACE WATER DEPTH N/A		T DATE 10/23/17 COMP. DATE 10/24/17	SURFACE WATER DEPTH N/A
ELEV (ft)   DRIVE   DEPTH   BLOW COUNT   BLOWS PER FOOT   SAMP.   V   O   O   O   O   O   O   O   O   O	SOIL AND ROCK DESCRIPTION  ELEV. (ft)  DEPTH (ft)	ELEV (ft)	BLOWS PER FOOT SAMP. CONTROL S	
2 5 3 . • 8 . · · · · · · · · · · · M	238.8 GROUND SURFACE 0.0  ROADWAY EMBANKMENT  BROWN, CLAYEY SAND  235.8 3.0	160	Match Line Sat.	GRAY AND GREEN, CLAYEY SAND (continued)
235 234.4 4.4 2 3 4 67	COASTAL PLAIN GRAY AND TAN, SANDY CLAY (MIDDENDORF FORMATION)	155 155.0 83.8 10 7 10	w	154.5 84.3 GRAY, SILTY CLAY
230 230.0 8.8 3 3 3 • 66 · · · · · · · · · · · · · · · · ·	231.8 GRAY, CLAYEY SAND	150 150.0 88.8 12 19 21 .	940 Sat.	151.8 — GRAY AND GREEN, CLAYEY SAND — 87.0
225 225.0 13.8 2 3 5		145 145.0 93.8 8 16 16 .		- - - -
220 220.0 18.8 5 4 7 . •11	-	140 140.0 98.8 7 8 11		- 138.5 100.3
215 215.0 23.8	-		<b>▼</b> 19	Boring Terminated at Elevation 138.5 ft IN MED. DENSE CLAYEY SAND (COASTAL PLAIN)
210 210.0 28.8	_			
205 205.0 33.8 3 4 5	206.8 32.0			- - - -
200 200.0 38.8	-			- - - -
195 195.0 43.8	196.8			- - -
190 190.0 48.8	193.8 (CAPE FEAR FORMATION) 45.0 BLACK, SAND			
9 10 35 42	186.8 52.0			
Tag   5   10   15				- - -
8 175 175.0 63.8 W				- - -
00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7400.2			
ш 17 22 25 W 17 165 165 0 73.8	GRAY AND GREEN, CLAYEY SAND			
Sat. 160 160.0 78.8 Sat.				- - - -





\A/D		00.4.4		- 1	TID	1.5000		1	URE			050	N COICT Discussion	- F.O		\\	5000	2.4.4		1.	TID	1.5000	TV HADNE			0501	OCIOT Disastina		
	5308		L BRIDG			I-5833	D 4700)		Y HARNE	:11		GEC	DLOGIST Blonshir		NATE (ft)	-	5308		DDID				HARNE	11		GEOL	LOGIST Blonshine		JND WTR (ft)
			BRIDG					OVER -L	<del>, `                                   </del>	04.51.T		1	ONIMENT NO		WTR (ft)							ON -Y2- (SR 1709) OVER	<del>`</del>	04.61.T		1110	NIMENIT VO		` '
-		<b>O</b> . EB2-				ION 27			OFFSET				GNMENT -Y2-	0 HR.	N/A		RING NO					TION 27+26	OFFSET				NMENT -Y2-	0 HR.	
		LEV. 23				L DEPTH			NORTHIN				STING 2,131,816		8.5		LAR EL					TAL DEPTH 100.1 ft	NORTHIN				TING 2,131,816	24 HR.	
			FF./DATE						1			Mud Rotar	<u> </u>	HAMMER TYPE	Automatic							ME-550X 88% 08/10/2017	1		METHOD	<del></del>		HAMMER TYPE	E Automatic
		White, T	ı			RT DATE			COMP. D			SUR	RFACE WATER DE	PTH N/A			LLER V					RT DATE 10/18/17	COMP. DA			SURF	ACE WATER DEPT	H N/A	
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft 0.			25		PER FOOT 50	Г 75 100	SAMP. NO.	/ (	G ELEV.		OCK DESCRIPTION	DEPTH (ft)	ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)		V COUNT 0.5ft 0.5f		BLOWS PER FO	OT 75 100	SAMP.	MOI G		SOIL AND ROC	K DESCRIPTIO	N
240	239.7	0.0	2	2 3		5			: : : : :		M L	239.7	ROADWAY	ND SURFACE Y EMBANKMENT ACK, CLAYEY SAND	0.0	160			+			Match Line	· · · · ·	 		<del> </del>	GRAY AND GREE	N, CLAYEY SA inued)	
235	235.0	4.7	2	3 5							м	236.7	TAN AND GF (MIDDENDC	STAL PLAIN RAY, SANDY CLAY ORF FORMATION)	7.0	155	156.1	83.6	11	11 13		24		-	Sat.				
230	231.1	8.6	3	3 3		6				-	Sat.	227.7		CLAYEY SAND	12.0	150	151.1	88.6	6	8 8	3	16			Sat.	<del>, , , , , , , , , , , , , , , , , , , </del>			
225	226.1	13.6	2	3 4	-  :	7 				SS-96	25% O			, TAN AND PURPLE, ND WITH GRAVEL		145	146.1	93.6	5	6 6	5	• · · · · · · · · · · · · · · · · · · ·		-	Sat.				
220	221.1	18.6	7	7 3		. I				-	Sat. 000	000- 000- 000- 000-				140	141.1	98.6	7	7 10	0	17			Sat.	139.6	Boring Terminated a MED. DENSE CLAY	t Elevation 139.	100.1 6 ft IN
215	216.1	23.6	17 1	14 13	3   .		27				Sat.	00- 00- 00- 00- 00- 00- 212.7			27.0		-	† - - - -								<u> </u>	MED. DENSE CLAY PL	EY SAND (COA AIN)	STAL
210	211.1	28.6	12	7 3		• • • • • • • • • • • • • • • • • • •					Sat.		TAN AND BRO	DWN, CLAYEY SAND	21.0		-	† - - -								[ - -			
205	206.1	33.6	5	5 5		10 10					Sat.						-	† - - - -								<u> </u>			
200	201.1	38.6	3	6 6		12-					Sat.	202.7	————— TAN, §	SILTY SAND	<u>37.0</u>		-	† - -											
195	196.1	43.6	1 1	2 6		· / · · · · /· · · · /- · ·					w	<u>  197.7</u>  -  -  -	COAS BLACK, (CAPE FEA	STAL PLAIN (, SILTY CLAY AR FORMATION)	42.0_		-	† † †											
190 190 190 190 190 190 190 190 190 190	191.1	48.6	3	3 4		· i · · · · · · · · · · · · · · · · · ·					w						-	<u> </u>											
185 185	186.1	53.6	12 1	17 18	3   .		35—				Sat.	<u>187.7</u>	GRAY AND BI	BLACK, SILTY SAND	<u> </u>		-	<u> </u>								<u>-</u>			
BORINGS.GF	181.1	58.6	11 1	19 24	1   .		\ \ \ .	3			w	<u>182.7</u> .	GRAY, SILTY CL	LAY AND SANDY CLA	<u> </u>		-	<u> </u>								<u> </u>			
175 175 175	176.1	63.6	15 1	17 27	7		l.				w						-	<u>+</u> + +								<u>-</u>			
170 170	171.1	68.6	14 2	23 31				54			w	172.7			67.0		-	<u> </u>								<u>-</u>			
JARE DOUBLE	166.1	73.6	9 2	21 28			· · · · · · · · · · · · · · · · · · ·	49			Sat.	167.7	GRAY AND GR	REEN, CLAYEY SAND	<u>72.0</u>		-	Ī - -								[ [ -			
NCDOT B 160	161.1	78.6	13 1	18 19			· · · /· · · · /· · · · /·				Sat.															-			



### SUMMARY OF LABORATOTY TEST DATA

Soil Classification and Gradation



S&ME, Inc. Raleigh, 3201 Spring Forest Road, Raleigh, North Carolina 27616											
S&ME Project #:	6235-16-015			Date Report	12/1/2017						
State Project No.:	53083.1.1	County:	Harnett	Date Tested	11/1/17-11/30/17						
Federal ID No.:	N/A	TIP No.:	I-5883								
Project Name:	Bridge No. 81 on -Y2- (SR	1709) over -L- (I-95)									

Client N	lame:				Michael B	aker Inter	rnatio	nal												
Sample No.	ו #:		:#:	Alignment	Sample	AASH			Tota	al % Pas	_			Mortar	Fraction	n (%)				
ldm	Station #:	Offset	Boring	ignr	Depth	Classific	cation			Sieve #	T .		Coarse				LL	PL	PI	Moist.
					(ft)			10	40	60	200	270	Sand	Sand	Silt	Clay				%
SS-96	27+26	24 LT	EB2-A	Y2	13.6-15.1	A-1-b		57	26	18	8.5	7.7	39	10	1	6	28	0	N.P.	25.3
SS-121			EB2-B	Y2	33.8-35.3	A-2-4		65	30	21	10.6	9.2	44	12	3	7	28	20	8	23.2
SS-146		10 LT	EB1-A	Y2	53.8-55.3	A-3		100	56	20	7.0	5.5	80	15	1	4	18	0	N.P.	23.1
SS-167	26+14	25 RT	EB1-B	Y2	48.8-50.3	A-3	(0)	99	61	17	6.1	4.8	82	12	3	2	17	0	N.P.	23.3
	as / Comm				ND-Not Dot															

References / Comments / Deviations:

ND=Not Detemined.

AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT

AASHTO T89: Determining the Liquid Limit of Soils

AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils

AASHTO T265: Laboratory Determination of Moisture Content of Soils

AASHTO M145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

Mal Krajan, ET Technician Name:

Signature

104-01-0703 Certification #

Stewart Laney, P.E. Technical Responsibility:

Project Manager Position

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# SITE PHOTOGRAPH

Bridge No. 81 on -Y2- (SR 1709) over -L- (I-95)



5883 REFERENCE

**CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

TITLE SHEET

CROSS SECTIONS

SITE PHOTOGRAPH(S)

BORE LOGS(S) SOIL TEST RESULTS

SITE PLAN

PROFILE

SHEET NO.

5-7

8-13

3083

### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY \_HARNETT

PROJECT DESCRIPTION IMPROVE I-95 INTERCHANGES AT SR 1808 (JONESBORO RD.) AND SR 1709 (HODGES CHAPEL RD.)

SITE DESCRIPTION BRIDGE NO. 80 ON -YI- (SR 1808) OVER -L- (I-95)

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTA SHEET
N.C.	I-5883	1	15

### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDING TO CLIMATIC CONDITIONS INCLORDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS, AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

E.G. BLONSHINE

M.S. HAYES

G.H. GOSLIN

T.J. WHITE

K.S. HARDEE

INVESTIGATED BY J.R. SWARTLEY

DRAWN BY \_J.R. SWARTLEY

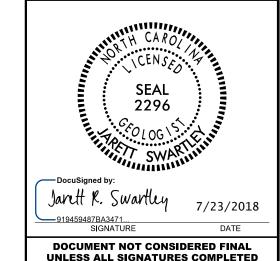
CHECKED BY \_\_S.S. LANEY

SUBMITTED BY S.S. LANEY

DATE \_FEBRUARY 2018



3201 SPRING FOREST ROAD RALEIGH, NC 27616 (919) 872-2660



PROJECT REFERENCE NO. SHEET NO.

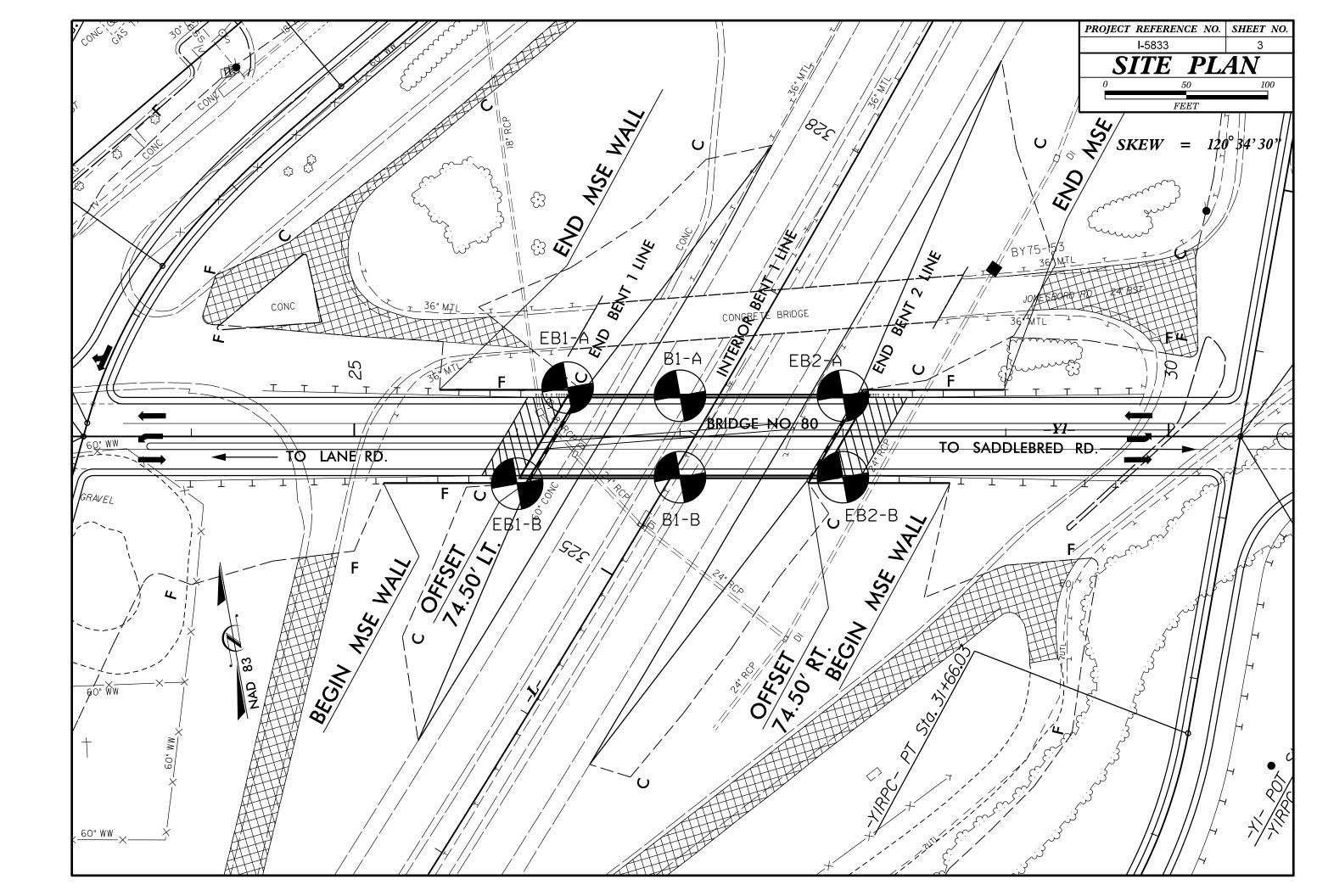
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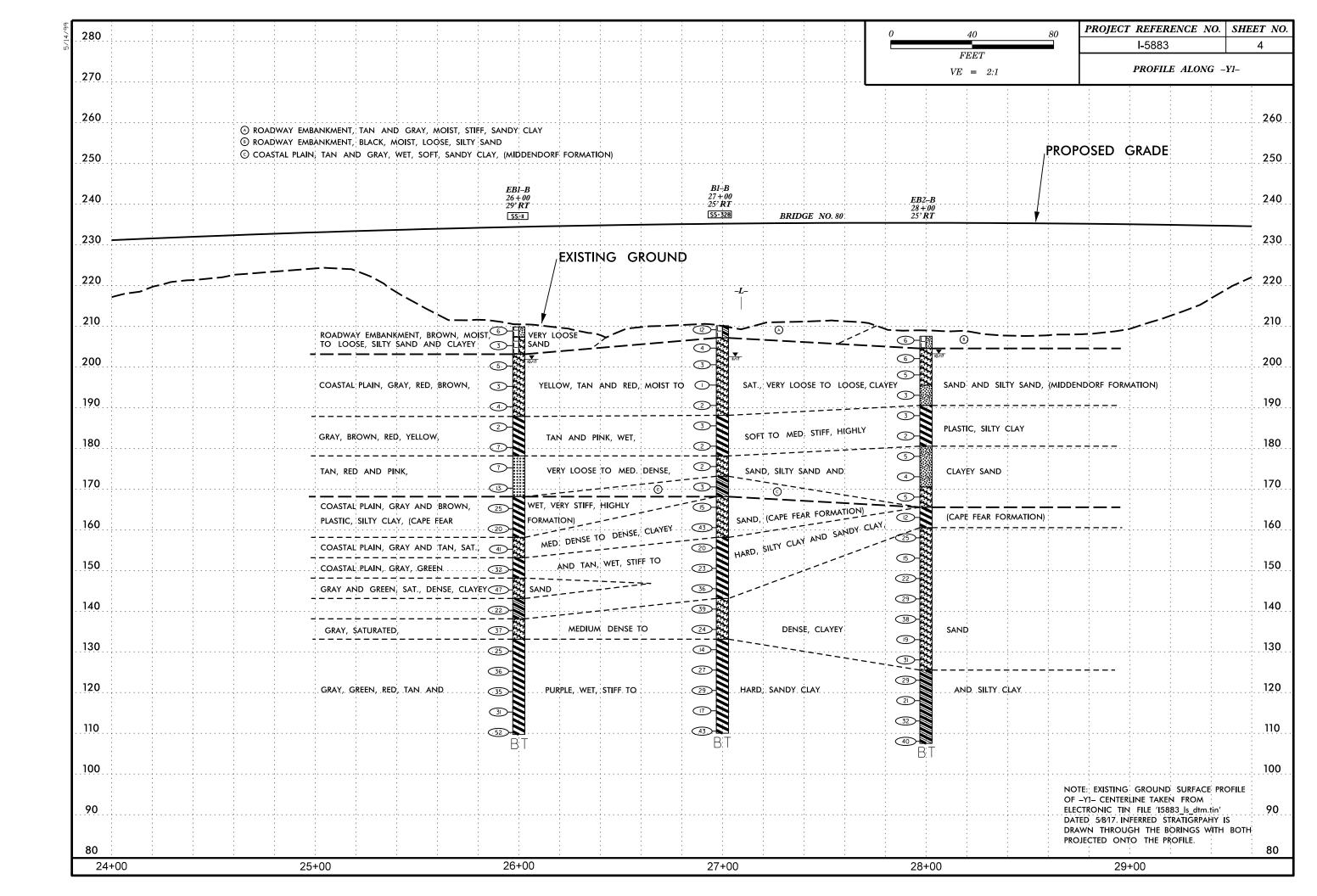
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

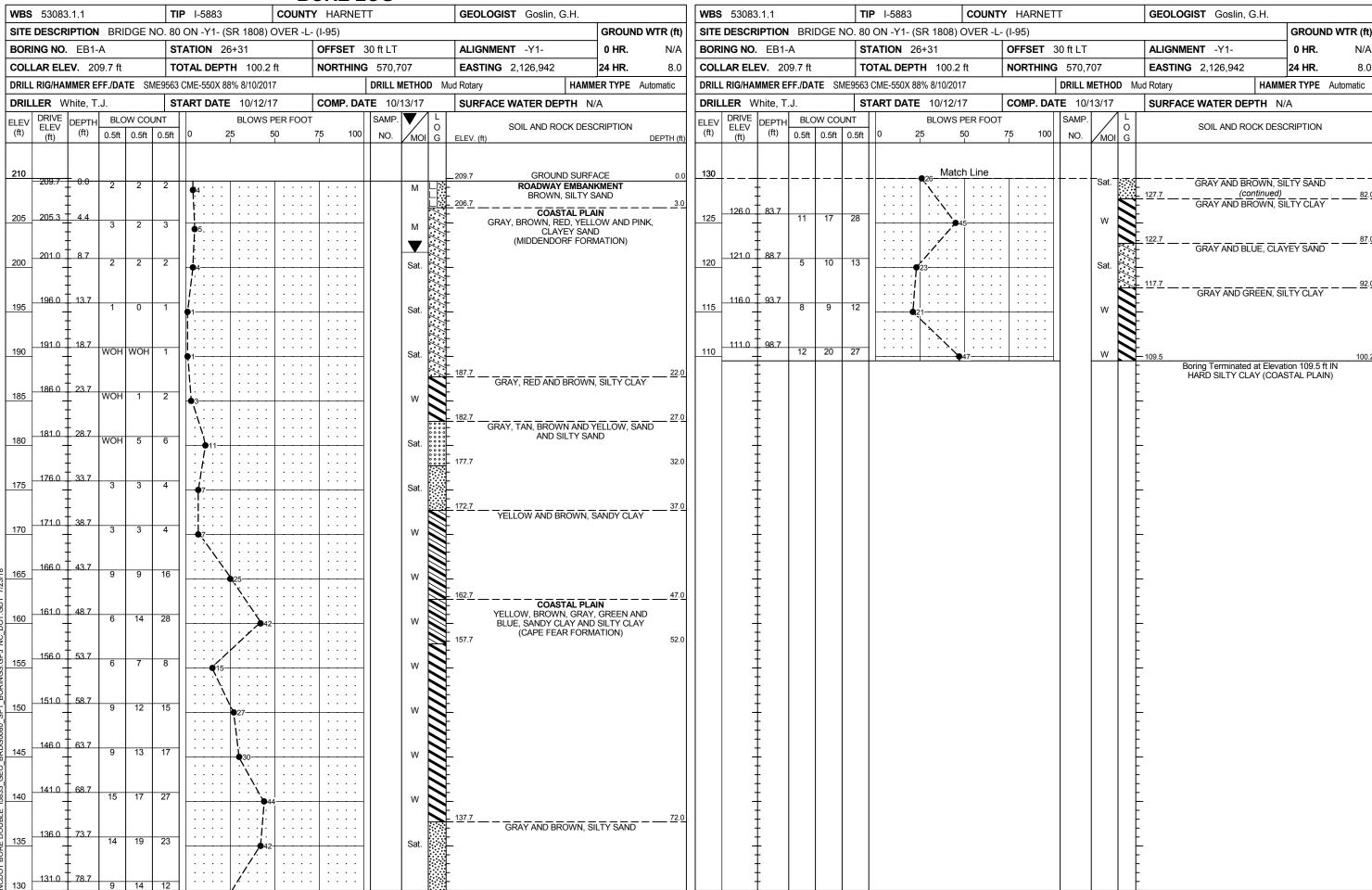
# SUBSURFACE INVESTIGATION

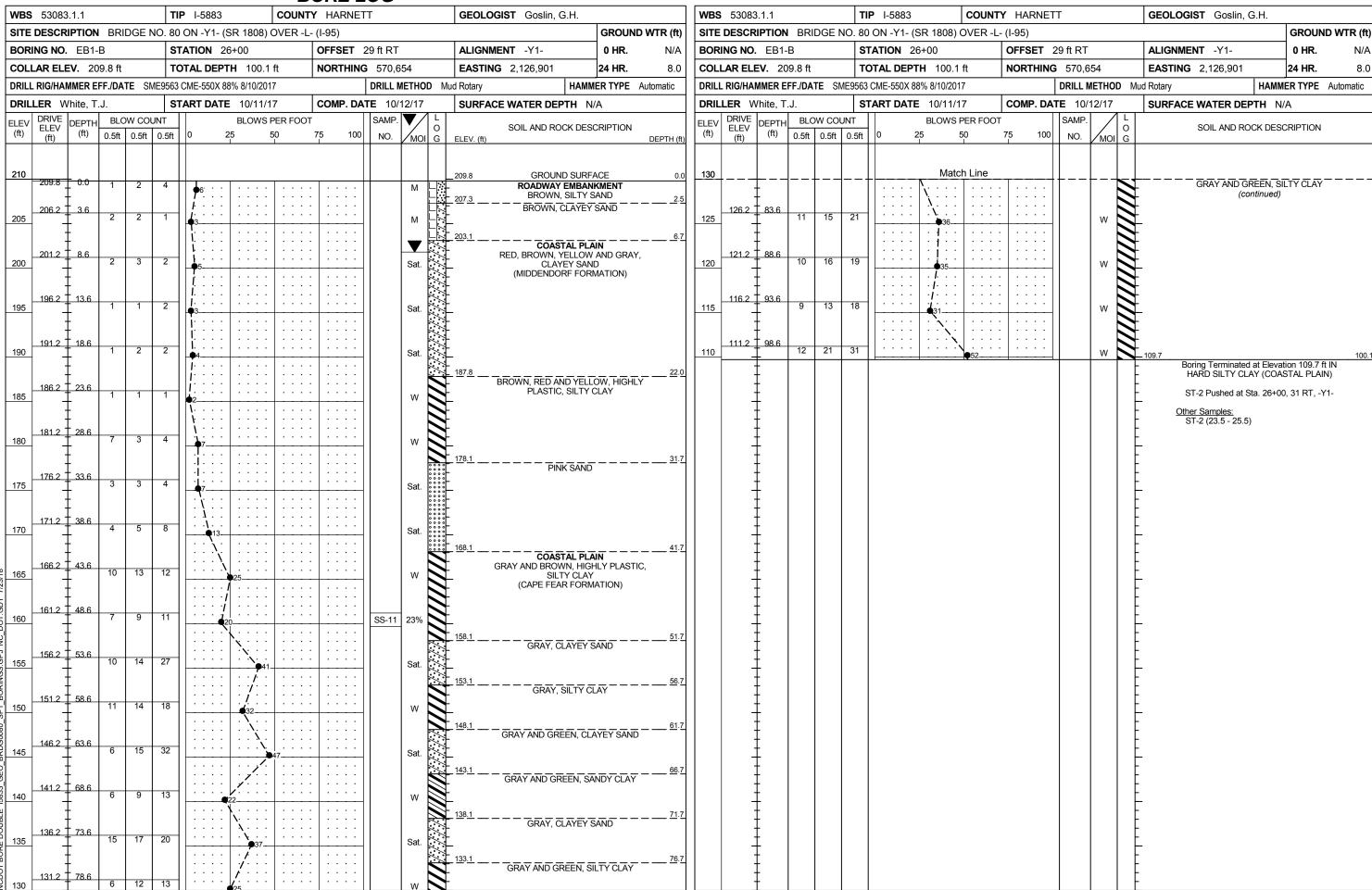
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

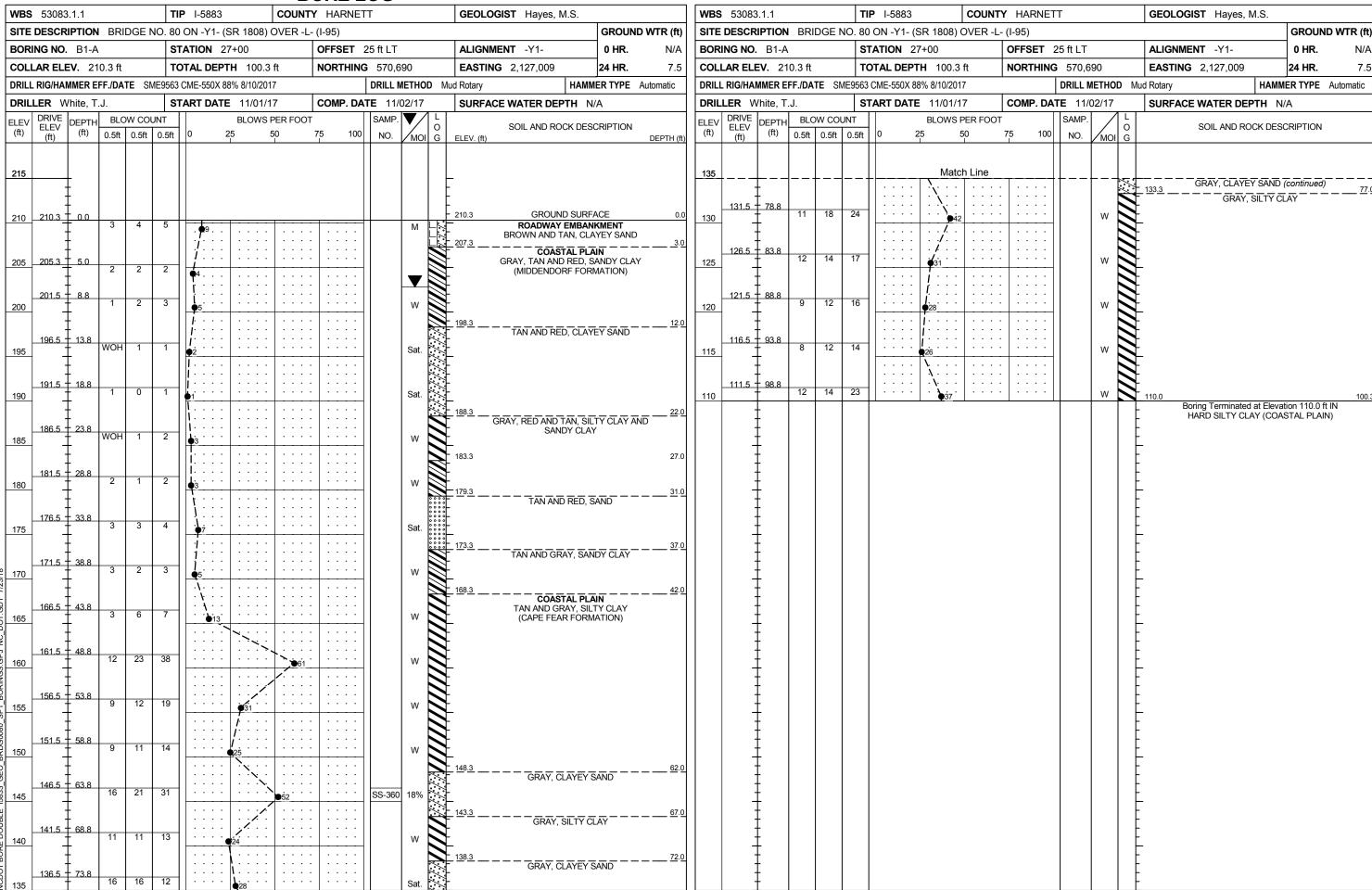
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DI586). SOIL CLASSIFICATION	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,  VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.  ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
CENERAL CRANIII AR MATERIALS SILT-CLAY MATERIALS	MINERALOGICAL COMPOSITION	FINE TO COARSE CRAIN ICNEOUS AND METAMORPHIC POCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	ROCK (CP) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-7-6 A-3 A-6, A-7	COMPRESSIBILITY  SLIGHTLY COMPRESSIBLE LL < 31	ROCK (NCR)  SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 0000 d00000 00000 00000 00000 00000 00000 0000	MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
7. PASSING SILT-	HIGHLY COMPRESSIBLE LL > 50  PERCENTAGE OF MATERIAL	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 59 MX   GRANULAR CLAY PEAT   GRANULAR CLAY PEAT   SOILS   SOILS	GRANULAR SILT - CLAY	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
25 MX 25 MX 25 MX 35 MX 35 MX 35 MX 36 M 3	ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL PASSING *40	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.  VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN.	HORIZONTAL.
LL 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 501L5 WITH	MODERATELY ORGANIC         5 - 10%         12 - 20%         SOME         20 - 35%           HIGHLY ORGANIC         > 10%         > 20%         HIGHLY         35%         AND ABOVE	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
P! 6 MX NP 18 MX 18 MX 11 MN 11 MN 18 MX 18 MX 11 MN 11 MN MODERATE OPERATE	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE  GROUND WATER	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH,  FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
GROUP INDEX W W 4 MX 8 MX 12 MX 16 MX NU MX AMUUN 15 UF SOILS		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO  (SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TITES STUME FRAUS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE		(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
AS SUBGRADE POUR	SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.  FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30  CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.
DANCE OF CTANDARD DANCE OF UNICONETNED	THIS CECENICOUS STRIBUTES	(MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (IN-VALUE) (TONS/FT <sup>2</sup> )	ROADWAY EMBANKMENT (RE)  25/025  DIP & DIP DIRECTION  OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
VERY LOOSE < 4	SPT C SLOPE INDICATOR	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY LOOSE 4 TO 10	SOIL SYMBOL  OPT ONT TEST BORING  INSTALLATION  SECTE INDICATOR  INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.  MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS
MATERIAL MEDIUM DENSE 10 10 30 N/A	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING TEST	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF  VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50	THAIN NOME WHIT EMBHINKIMEIN TO TEST	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25	- INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	(V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY   SOFT   2 TO 4   0.25 TO 0.5     SILT-CLAY   MEDIUM STIFF   4 TO 8   0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL   STIFF   8 TO 15   1 TO 2	A DIE ZOMETED	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4	TTT ALLUVIAL SOIL BOUNDARY ALLUVIAL SOIL BOUNDARY SPT N-VALUE	ALSO AN EXAMPLE.	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS  VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIF	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	USED IN THE TOP 2 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	SHALLOW UNCLASSIFIED EXCAVATION - SEED IN THE TOP 3 FEET OF ACCEPTABLE DEGRADABLE ROCK EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(BLDR.) (COB.) (GR.) (SE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{ m d}$ - DRY UNIT WEIGHT	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE (ATTERBERG LIMITS) DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC  DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO   SD SAND, SANDY   SS - SPLIT SPOON   F - FINE   SL SILT, SILTY   ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
PLASTIC   SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	
(PI) PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	BENCH MARK: BY75-I53 NORTHING: 570733 EASTING: 2127213
- MOIST - (M) COLID. AT OR NEAR ORTIMIN MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: 230.26 FEET
OM OPTIMUM MOISTURE SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	NOTES:
REQUIRES ADDITIONAL WATER TO	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE	1.0.25
- DRY - (D) ATTAIN OPTIMUM MOISTURE	CME-55 6° CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	
PLASTICITY	8* HOLLOW AUGERS	INDURATION	1
PLASTICITY INDEX (PI) DRY STRENGTH	CME-550 HARD FACED FINGER BITS	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST UNGCARBIDE INSERTS HAND TOOLS:	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM	X CASING W/ ADVANCER POST HOLE DIGGER	CDAING CAN BE CEDADATED EDOM CAMBLE WITH CIFEL BRODE.	
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST X TRICONE 2 15/16 STEEL TEETH HAND AUGER	MODERATELY INDURATED  BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE TUNGCARB. COUNTRING POR	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	X CME-550X CORE BIT VANE SHEAR TEST	DIFFICULT TO BREAK WITH HAMMER.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED  SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1-
			1

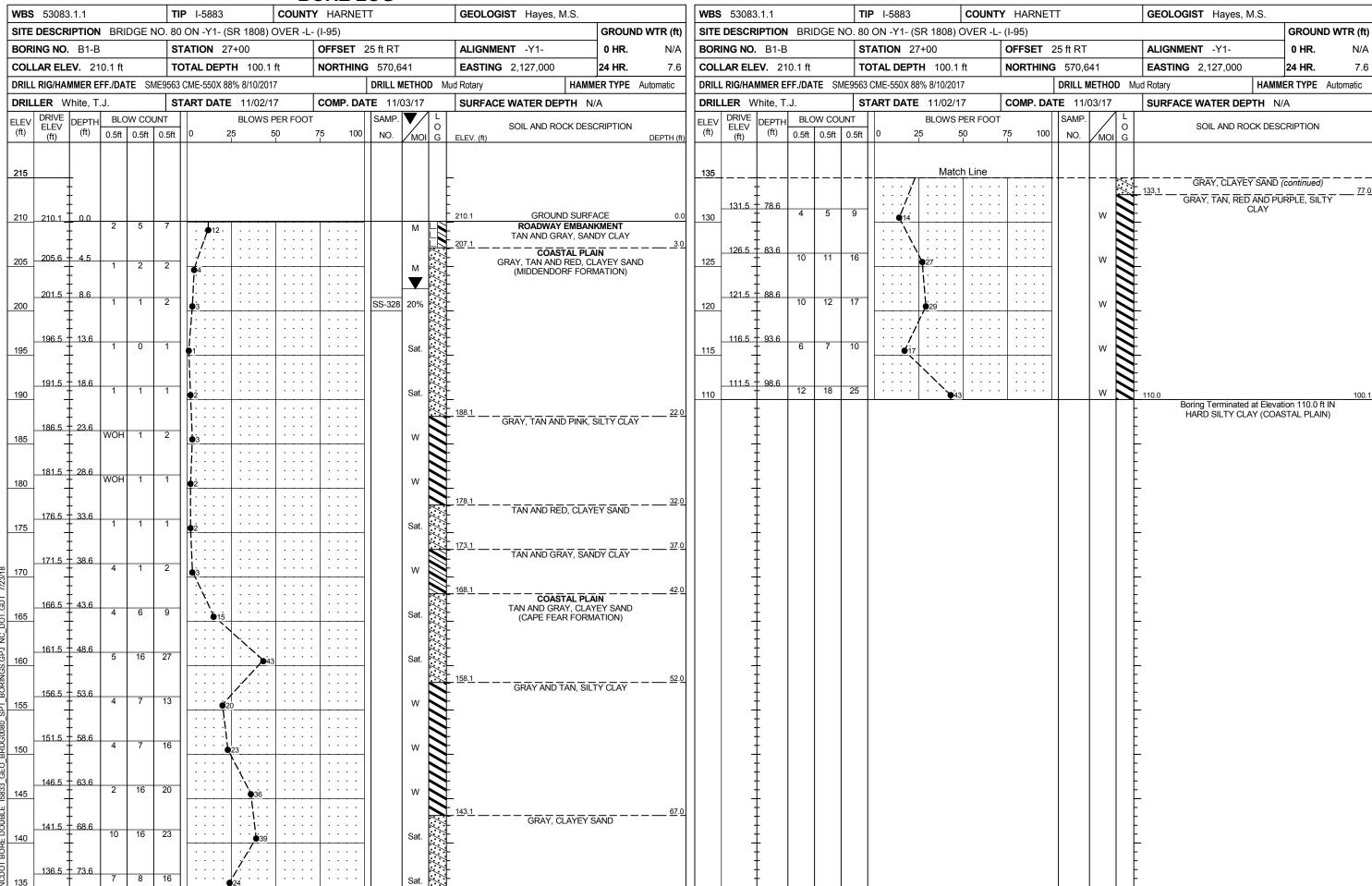




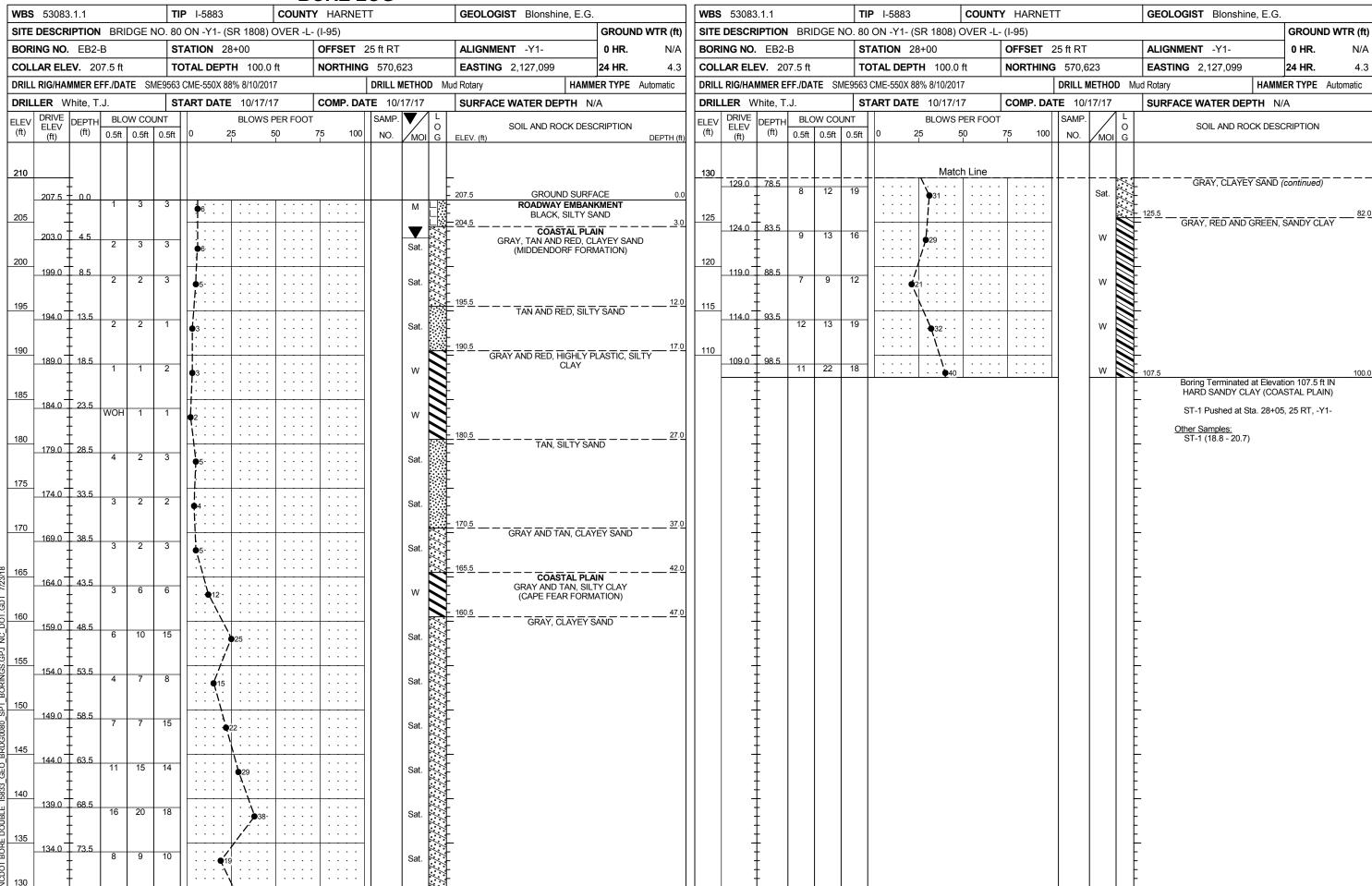








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-	53083.1.1		UDOE N			OD 4000)			11		GEUL	OGIST Blonshine,		TD (61)		53083		DDID		TIP 1-58			Y HARNET	ı		GEOLO	GIST Blonshine, E.C	1
-	DESCRIPTION		IDGE I				OVER -L	<del>, `                                   </del>			1		GROUND W	` '							/1- (SR 1808)	OVER -L	·			1		GROUND WTR (ft)
	ING NO. EE			_	ATION 2			OFFSET			_	NMENT -Y1-	0 HR.			NG NO.					<b>N</b> 28+00		OFFSET				IENT -Y1-	<b>0 HR</b> . N/A
COL	LAR ELEV.	209.2 ft	t	TC	TAL DEPT	<b>FH</b> 105.4	· ft	NORTHIN	<del>,                                    </del>			ING 2,127,108	24 HR.			AR ELE					<b>DEPTH</b> 105.4		NORTHING				<b>G</b> 2,127,108	<b>24 HR.</b> 6.2
DRIL	RIG/HAMME	R EFF./D	ATE SI	ME9563	CME-550X 8	8% 8/10/201	17	_	DRILL N	IETHOD	Mud Rotary	ŀ	HAMMER TYPE Auto	matic	DRILL	RIG/HAN	IMER EF	F./DATE	SME9563	3 CME-55	50X 88% 8/10/201	17		DRILL I	METHOD N	Mud Rotary	HAM	MER TYPE Automatic
DRII	LER White	, T.J.		ST	ART DATE	<b>■</b> 10/16/1	17	COMP. DA	TE 10/	17/17	SURF	ACE WATER DEPTI	H N/A			LER W	hite, T.	J.	S	START D	<b>DATE</b> 10/16/1	17	COMP. DA	<b>TE</b> 10/	17/17	SURFA	CE WATER DEPTH	N/A
ELEV	DRIVE DEP	***	ow co			BLOWS	PER FOOT	Γ	SAMP.	<b>V</b>		SOIL AND ROCK	C DESCRIPTION		LLLV	DRIVE ELEV	DEPTH	BLOW	COUNT		BLOWS	PER FOOT	-	SAMP.	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		SOIL AND ROCK DE	SCRIPTION
(ft)	(ft) (ft	0.5ft	0.5ft	0.5ft	0 2	25	50	75 100	NO.		ELEV. (ft)			EPTH (ft)	(ft)	(ft)	(ft)	0.5ft C	0.5ft 0.5ft	t 0	25	50	75 100	NO.	MOI G			
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	1 7	1	0	1	1				]	Sat.	189.3	GRAY, PURPLE AND	D RED. SILTY CLAY	19.9		7	.	19	24   30			●54			l w	<u> </u>		
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140	140.3 + 68.	.9	7	11	· · · · /				88.64	20%	#	J. J , O. u					.									L		
BLE	‡			``	• 18				SS-64	20 /0	107.0			72.0												<u></u>		
DOO	1056				1:::		: : :	.			137.2	GRAY AND GREEN	N, CLAYEY SAND	12.0		1										E		
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Z 13U	100.0 10.			=	1					le.°`	^\									1				1				



### SUMMARY OF LABORATOTY TEST DATA

Soil Classification and Gradation



S&ME, Inc. Raleigh, 3201 Spring Forest Road, Raleigh, North Carolina 27616											
S&ME Project #:	6235-16-015			Date Report	12/1/2017						
State Project No.:	53083.1.1	County:	Harnett	Date Tested	11/1/17-11/30/17						
Federal ID No.:	N/A	TIP No.:	I-5883								
Project Name: Bridge No. 80 on -Y1- (SR 1808) over -L- (I-95)											

Project Name:

Client N	lame:				Michael B	aker Inter	natio	nal												
No.	::		::	ent	Sample	AASH'	ТО		Tota	al % Pas	sing		Total	Mortar	Fraction	n (%)				
Sample No.	Station	set	Boring 4	Alignment	Depth	Classific	ation			Sieve #			Coarse	Fine			LL	PL	PI	Moist.
		Offset			(ft)	. = .	(0)	10	40	60	200	270	Sand	Sand	Silt	Clay				%
SS-11	26+00	29 RT	EB1-B	Y1	48.6-50.1	A-7-6 (		100	97	95	48.1	42.4	5	53	8	34	43	17	26	23.3
SS-64	28+00	25 LT	EB2-A	Y1	68.9-70.4	A-4 (		100	97	95	55.8	36.1	5	59	30	6	30	26	4	19.8
			B1-B	Y1	8.6-10.1	A-2-6 (	(0)	100	61	43	26.7	25.4	57	18	7	18	37	22	15	20.2
SS-360	27+00	25 LT	B1-A	Y1	63.8-65.3	A-2-7 (	(0)	100	48	30	18.6	15.2	70	15	10	5	41	22	19	18.4
ST-1	28+05	25 RT	EB2-B	Y1	18.8-20.7	A-7-6 (	(45)	100	100	99	98.7	96.5	1	3	36	60	64	25	39	41.0
ST-2	26+00	31 RT	EB1-B	Y1	23.5-25.5	A-7-6 (	(46)	100	99	98	97.0	94.3	2	4	34	60	65	23	42	37.8
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D . C			<u> </u>		ND Not Det															

References / Comments / Deviations:

ND=Not Detemined.

AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT

AASHTO T89: Determining the Liquid Limit of Soils

AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils

AASHTO T265: Laboratory Determination of Moisture Content of Soils

AASHTO M145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

Mal Krajan, ET Technician Name:

Signature

104-01-0703 Certification #

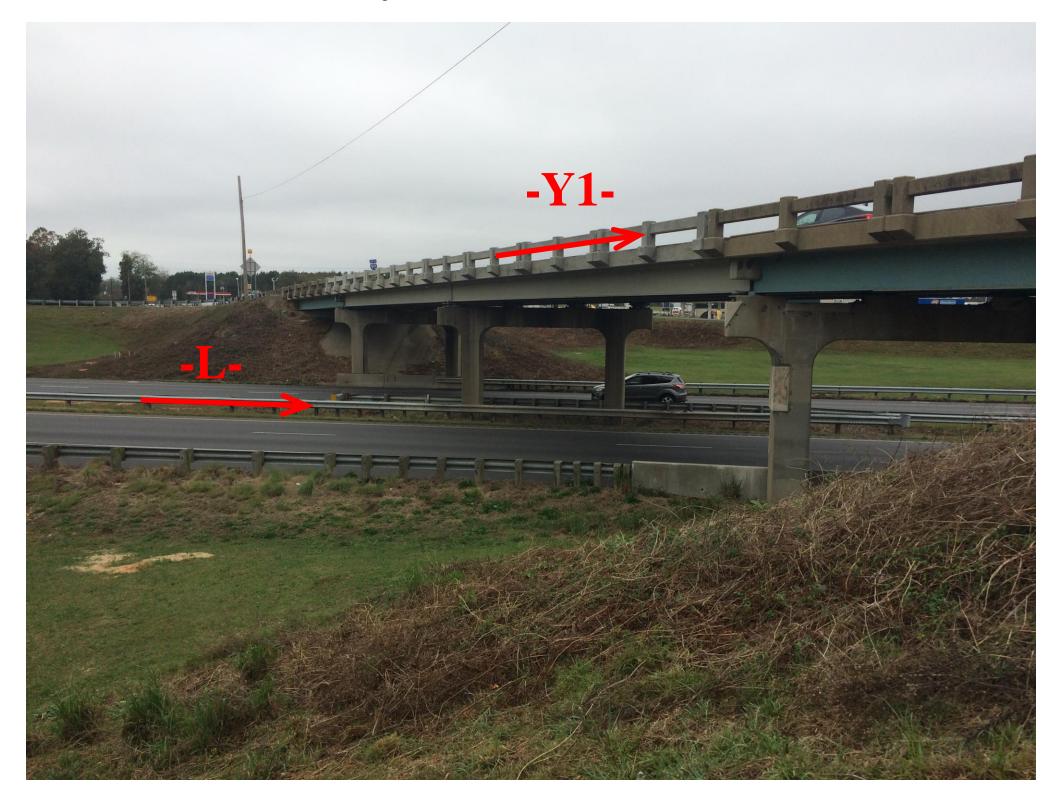
Stewart Laney, P.E. Technical Responsibility:

Project Manager Position

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# SITE PHOTOGRAPH

Bridge No. 80 on -Y1- (SR 1808) over -L- (I-95)



Looking West towards End Bent 1

5883 REFERENCE

**CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

TITLE SHEET

CROSS SECTIONS

SITE PHOTOGRAPH(S)

BORE LOGS(S) SOIL TEST RESULTS

SITE PLAN

PROFILE

SHEET NO.

5-7

8-13

3083

### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY \_HARNETT

PROJECT DESCRIPTION IMPROVE I-95 INTERCHANGES AT SR 1808 (JONESBORO RD.) AND SR 1709 (HODGES CHAPEL RD.)

SITE DESCRIPTION BRIDGE NO. 81 ON -Y2- (SR 1709) OVER -L- (I-95)

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTA SHEET
N.C.	I-5883	1	15

### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDING TO CLIMATIC CONDITIONS INCLORDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS, AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

E.G. BLONSHINE

M.S. HAYES

G.H. GOSLIN T.J. WHITE

K.S. HARDEE

INVESTIGATED BY J.R. SWARTLEY

DRAWN BY \_J.R. SWARTLEY

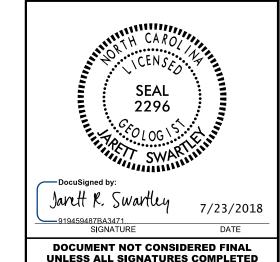
CHECKED BY \_\_S.S. LANEY

SUBMITTED BY S.S. LANEY

DATE \_FEBRUARY 2018



3201 SPRING FOREST ROAD RALEIGH, NC 27616 (919) 872-2660



PROJECT REFERENCE NO. SHEET NO.

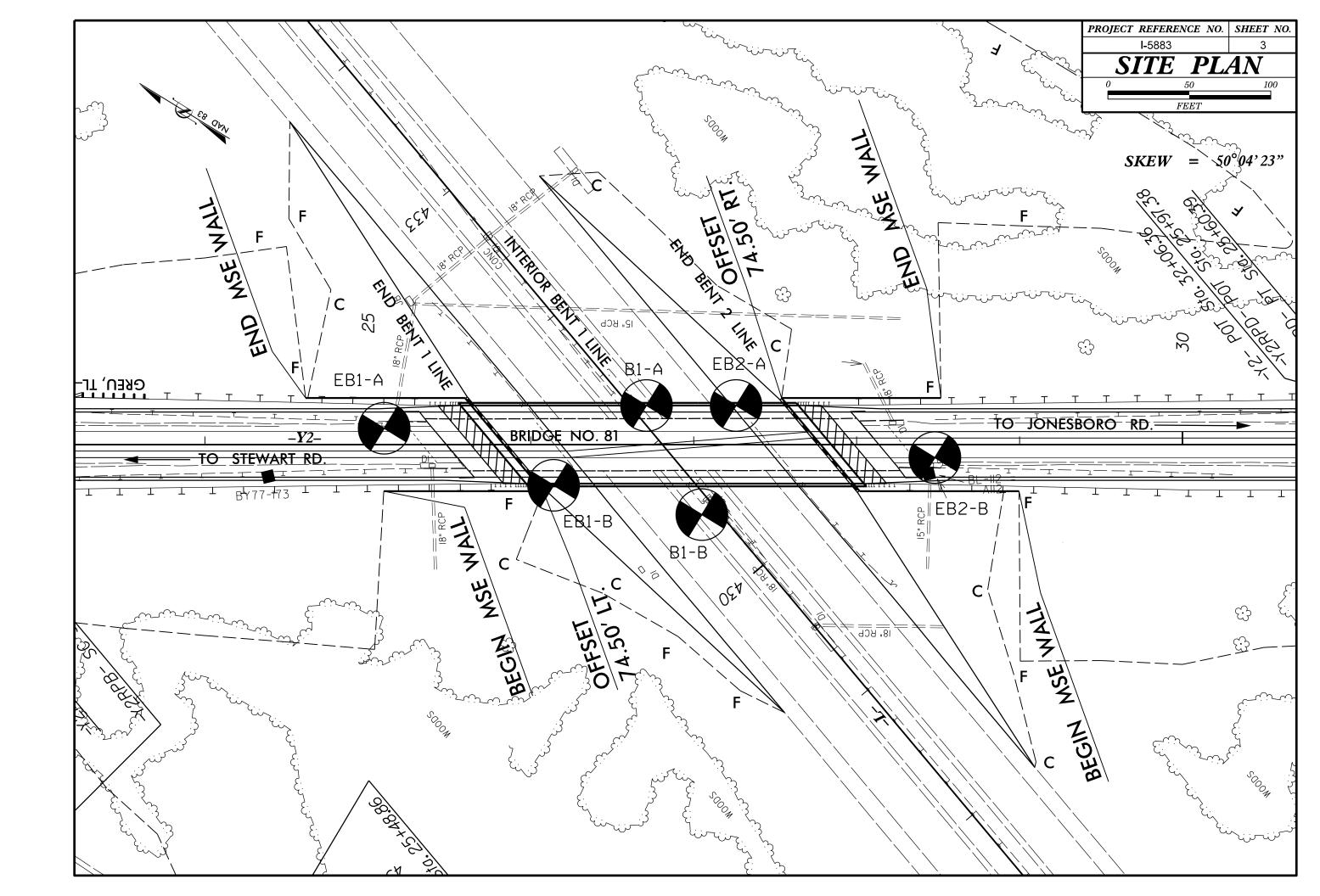
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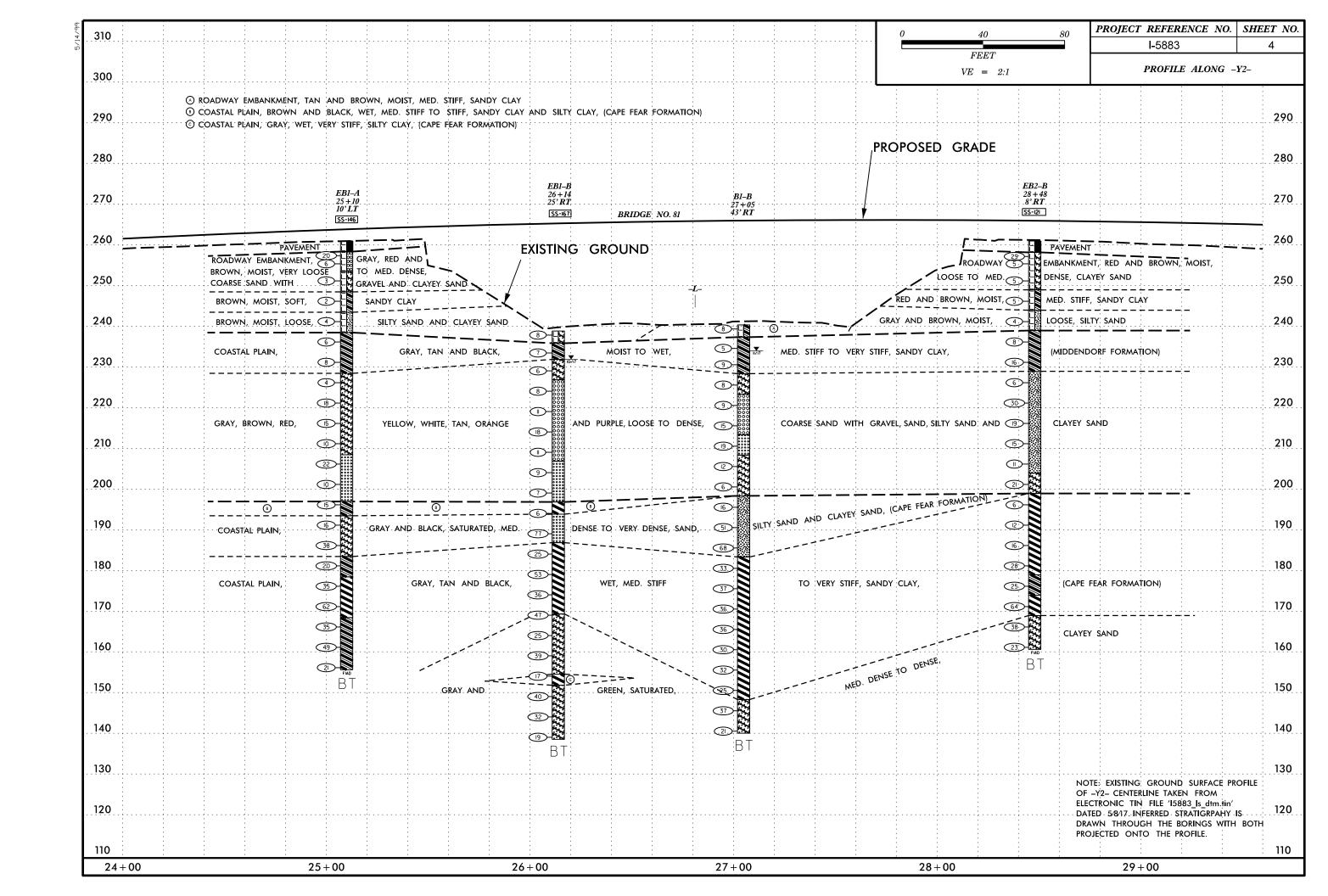
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

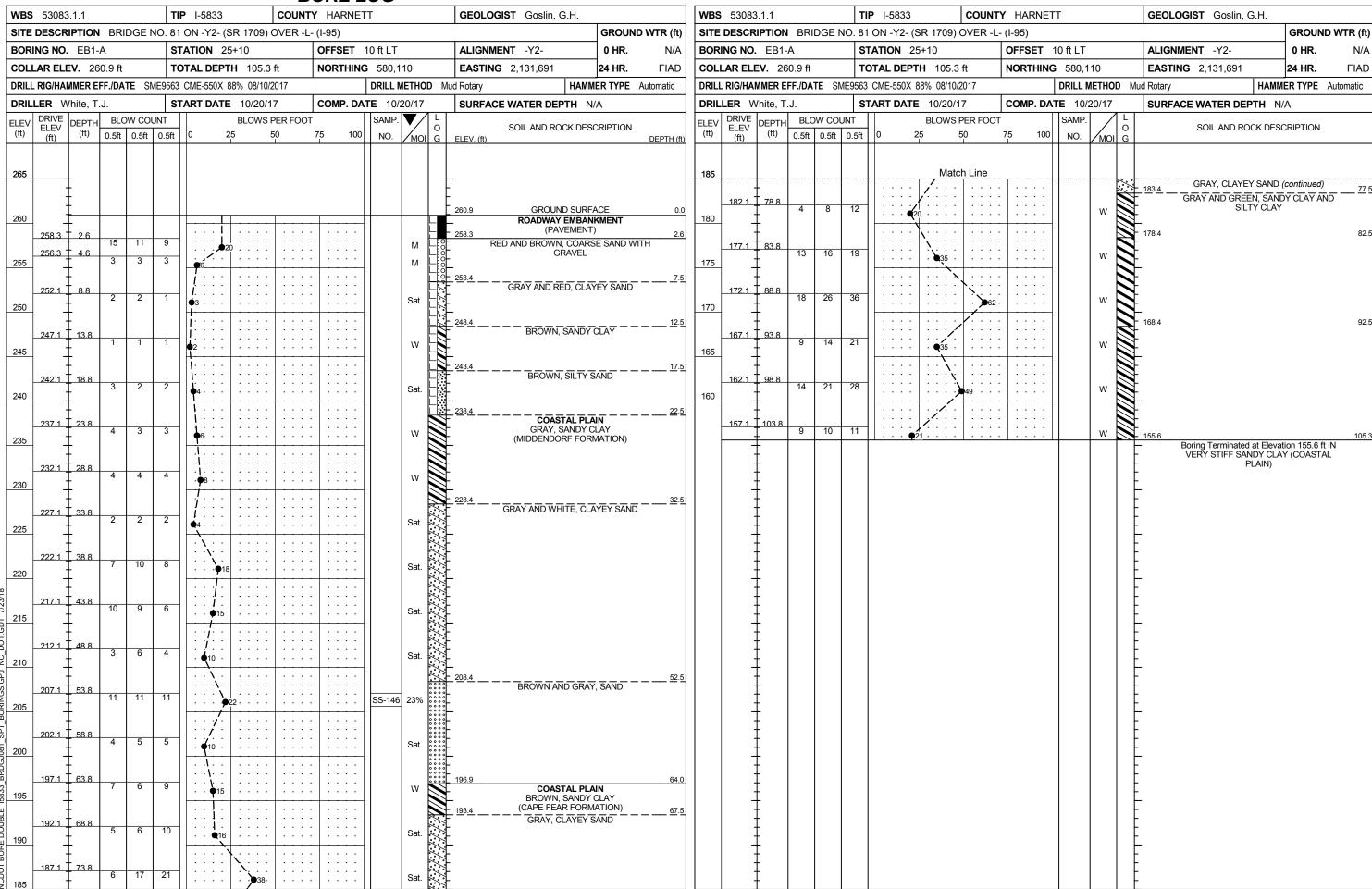
# SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

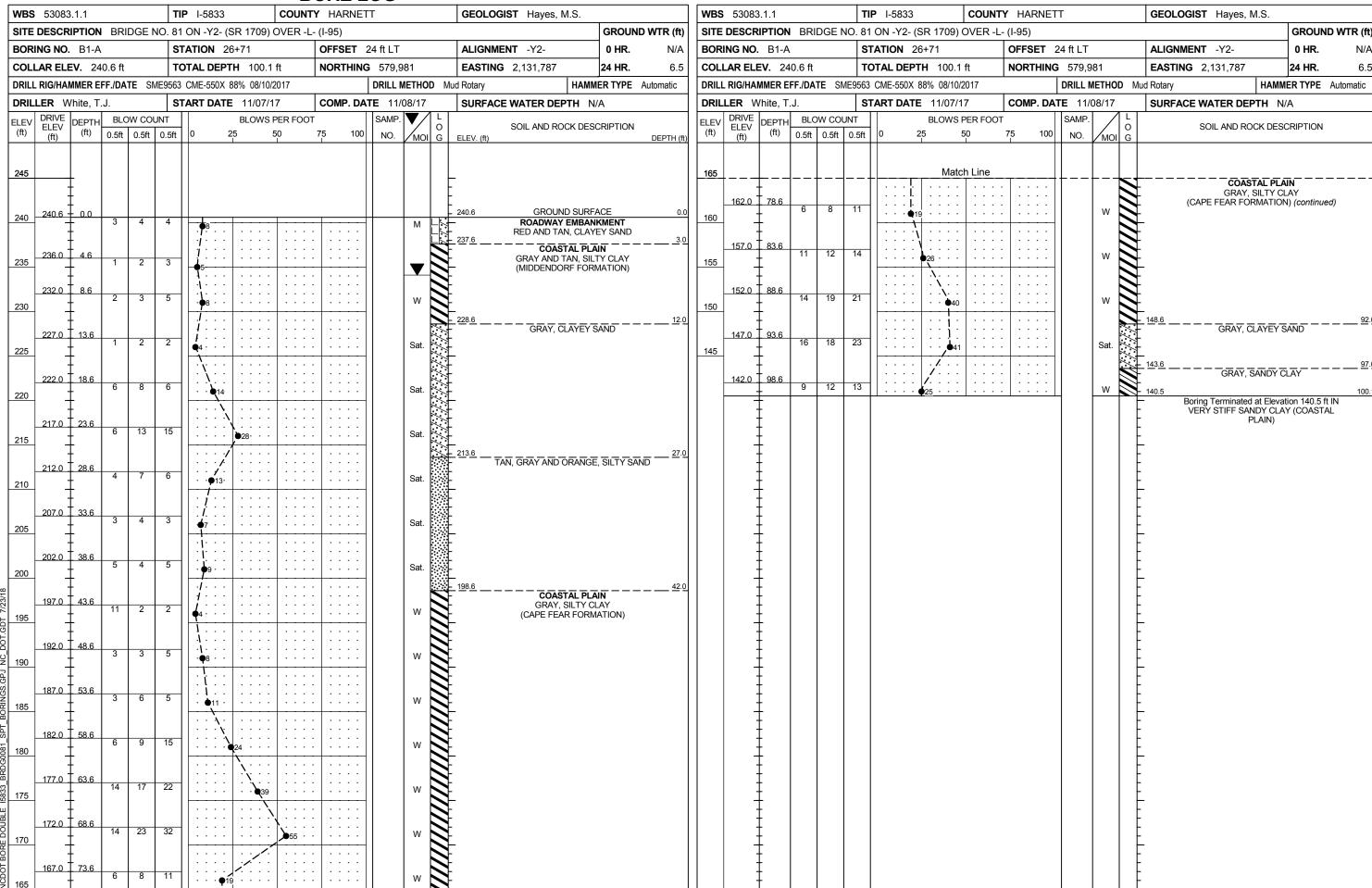
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF.GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.  ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
CENERAL CRANIILAR MATERIALS SILT-CLAY MATERIALS	MINERALOGICAL COMPOSITION	FINE TO COARSE CRAIN IGNEOUS AND METAMORPHIC ROCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	WOULD YIELD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE,	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	UNEISS, OHBBRU, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-8 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE ROCK (NCR) SEDIMENTARY ROCK THAT WOULD YELLD SET REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 0000 d00000 0000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
% PASSING SILT-	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED (CP) SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR CLAY MUCK, SOILS CLAY PEAT	PERCENTAGE OF MATERIAL	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*200   15 MX   25 MX   10 MX   35 MX   35 MX   35 MX   35 MX   36 MN   36 MN   36 MN   36 MN   36 MN   36 MN	GRANULAR SILT - CLAY  ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%  LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40 40 MX 41 MN 50ILS WITH	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN CITILE OR HIGHLY	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOLIS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
OF MAJOR GRAYEL, AND SAND GRAYEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN RATING FAIR TO		(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	PARENT MATERIAL.
AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30	-	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	FIELD.   JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
CONSISTENCY (N-VALUE) (TONS/FT <sup>2</sup> )	■ WITH SOIL DESCRIPTION ► OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4  LOOSE 4 TO 10	SOIL SYMBOL  SOIL SYMBOL  SPT DWT TEST BORING  SLOPE INDICATOR INSTALLATION	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR LOOSE 4 TO 10 GRANULAR MEDIUM DENSE 10 TO 30 MATERIAL MEDIUM DENSE 10 TO 30 N/A	M	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTILED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTILING IN SOILS
NON-COHESIVE)	ARTIFICIAL FILL (AF) OTHER HOGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING CONE PENETROMETER	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY SOFT < 2 < 0.25	INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5	MM - TECT DODING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY   MEDIUM STIFF   4 TO 8   0.5 TO 1.0     MATERIAL   STIFF   8 TO 15   1 TO 2	INFERRED ROCK LINE MONITORING WELL WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4	TTTTT ALLUVIAL SOIL BOUNDARY A PIEZOMETER INSTALLATION - SPT N-VALUE	ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
HARD > 30 > 4	INSTRUCTION —	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
TEVTUDE OD CDAIN CIZE	DECOMMENDATION CYMPOLC		
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
U.S. STD. SIEVE SIZE 4 10 40 60 200 270  OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT 10 BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	ROCK.
U.S. STD. SIEVE SIZE	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
U.S. STD. SIEVE SIZE 4 10 40 60 200 270  OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053  BOULDER CORRE CRAVEL COARSE FINE SULT CLAY	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
U.S. STD. SIEVE SIZE  4 10 40 60 200 270  OPENING (MM)  4.76 2.00 0.42 0.25 0.075 0.053  BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COES. SD.) (F SD.) (SL.) (CL.)  GRAIN MM 305 75 2.0 0.25 0.05 0.005	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT 10 BE UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK  ACCEPTABLE DEGRADABLE ROCK  ABBREVIATIONS  AR - AUGER REFUSAL  MED MEDIUM  VST - VANE SHEAR TEST	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE SHALLOW UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT BORING TEMINATED MICA MILACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7- UNIT WEIGHT	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL
U.S. STD. SIEVE   SIZE   4   10   40   60   200   270	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT 10 BE UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK  ABBREVIATIONS  AR - AUGER REFUSAL BT - BORING TERMINATED MICA MICACEOUS CL CLAY CPT - CONE PENETRATION TEST MP - NON PLASTIC  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT 10 BE USED IN THE 10P 3 FEET OF EMBANKMENT OR BACKFILL  VST - VANE SHEAR TEST WEA WEATHERED  7 - UNIT WEIGHT 7 - CONE PENETRATION TEST MP - NON PLASTIC 7 - CONE PENETRATION TEST MP - NON PLASTIC 7 - CONE PENETRATION TEST	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY  CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPP) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BORING TERMINATED MICA MICACEDUS WEA WEATHERED CL CLAY MOD MODERATELY CPT - COME PENETRATION TEST NP - NON PLASTIC CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS  DT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK STORY OF THE TOP TO THE TOP	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETRACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SILKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH DUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BORING TERMINATED MICA MICACEDUS WEA WEATHERED CL CLAY MOD MODERATELY CPT - COME PENETRATION TEST NP - NON PLASTIC CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETRACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST MICA MICACEDUS WEA WEATHERED CL CLAY MOD MODERATELY CPT - CONE PENETRATION TEST NP - NON PLASTIC CSE COARSE ORG ORGANIC DATE OF UNIT WEIGHT CSE COARSE ORG ORGANIC DATE OF UNIT WEIGHT CSE DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK S SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  CAN BE GROOVED OR GOUGED 0.65 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
U.S. STD. SIEVE SIZE  4 10 40 60 200 270  OPENING (MM)  4.76 2.00 0.42 0.25 0.075 0.053  BOULDER (BLDR.) (COB.) (GR.) (GR.) (CSE. SD.) (FSD.) (SL.) (CL.)  GRAIN MM 305 75 2.0 0.25 0.05 0.005  SIZE IN. 12 3  SOIL MOISTURE - CORRELATION OF TERMS  SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION  CSATURATED - USUALLY LIQUID: VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE  PLASTIC BRANCE - WET - (M) SEMISOLID; REQUIRES DRYING TO	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM ST VANE SHEAR TEST WEA WEATHERED CL CLAY MOD MODERATELY CPT - COME PENETRATION TEST NP - NON PLASTIC CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSURMETER TEST SAP SAPROLITIC SPINAMIC PENETRATION TEST SAP SAPROLITIC S SPLIT SPOON F FINE SULK SULT, SILTY ST SHELBY TUBE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY HARD  CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES I INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE.	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM STANDARD WEA VANE SHEAR TEST MICA MICACEOUS WEA WEATHERD WE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETRACH HAND SPECIMEN.  MODERATELY HARD  EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.  FRACTURE SPACING  IERM  SPACING  IERM  THICKNESS  THICKNESS  ITEM  THICKNESS	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  JOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY77-173
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNDERCUT UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED C.L CLAY MOD MODERATELY CPT - CONE PENETRATION TEST OMC ORGANIC OMT DILATOMETER TEST DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - SHUK S - SPLIT SPOON F - FINE FOSS FOSSILIFEROUS SL SILT, SILTY FRACT- FRACTURED, FRACTURES TCR - TRICONE REFUSAL REAGET ACCEPTABLE, BUT NOT TO BE USED IN TO TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  VST - VANE SHEAR TEST WEA WEATHERED VEATHER VE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY HARD  CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A CEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES I INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERWAIL.  FRACTURE SPACING  BEDDING  TERM  VERY WIDE  MORE THAN 10 FEET  VERY THICKLY BEDDED  4 FEET	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH DUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATAM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF STRATA MADE EXPRESSED AS A PERCENTAGE.  TOPSOIL (ITS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY77-173  NORTHING: 580156 EASTING: 2131628
U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST MED MICACEOUS MED WILL WED WED MEDIUM VST - VANE SHEAR TEST WED WED MICACEOUS WED WED WED MICACEOUS WED WED WED WILL WED WED WED MICACEOUS WED WILL WED WED WED WED WED SHEAR TEST WED W	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETRICH HAND SPECIMEN.  MODERATELY HARD  EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.  FRACTURE SPACING  IERM  SPACING  VERY WIDE  MORE THAN 10 FEET  WIDE  J THICKNESS  J LERM  THICKNESS  VERY WIDE  MORE THAN 10 FEET  THICKLY BEDDED  J LE THELE  MODERATELY CLOSE  J LE THICKLY BEDDED  J LE T LEST  THE THICK BEDSED  J LE	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SILEKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  TOPSOIL (TIS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY77-173  NORTHING: 580156 EASTING: 2131628  ELEVATION: 259.26 FEET
U.S. STD. SIEVE SIZE	UNDERCUT UND	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY HARD  CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.  FRACTURE SPACING  TERM  VERY WIDE  MORE THAN 10 FEET  VERY THICKLY BEDDED  1.5 - 4 FEET  MODERATELY CLOSE  1 TO 3 FEET  THINKLY BEDDED  0.03 - 0.16 FEET  VERY THINLY BEDDED  0.06 - 1.5 FEET  VERY THINLY BEDDED  0.06 - 1.5 FEET	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH DUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATAM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF STRATA MADE EXPRESSED AS A PERCENTAGE.  TOPSOIL (ITS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY77-173  NORTHING: 580156 EASTING: 2131628
U.S. STD. SIEVE SIZE	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL  BT - BORING TERMINATED  MCA MICACEOUS  WEA WEATHERED  CL CLAY  MOD MODERATELY  CPT - CONE PENETRATION TEST  OMC ORGANIC  COMT - DILATOMETER TEST  DPT - DYNAMIC PENETRATION TEST  SAP SAPROLITIC  C - VOID RATIO  S - SAP. SAPROLITIC  S - SULK  S - SPLIT SPOON  F - FINS  S - FOSSILIFEROUS  SL SILT, SILTY  FRACTURED, FRACTURES  TCR - TRICONE REFUSAL  RT - RECOMPACTED TRIAXIAL  FRACS FRAGMENTS  W - MOISTURE CONTENT  HI HIGHLY  V - VERY  C - CONTINUIOUS FLIGHT AUGER  WAST. VANE SHEAR TEST  WEA WEATHERED  WEA WEATHERED  Y - UNIT WEIGHT  Y - DRY UNIT WEIGHT  S - BULK  S - BULK  S - SPLIT SPOON  ST - SHELBY TUBE  RS - ROCK  RT - RECOMPACTED TRIAXIAL  FRACS FRAGMENTS  W - MOISTURE CONTENT  HI HIGHLY  V - VERY  CME-45C  MANUAL  WAST.  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT 107 TO BE  USED IN TO TO BE  USED IN TWE, ON THE TOP 3 FEET OF  EMBANKMENT OR BACKFILL  WEA WEATHERES  WEA WEATHERES  Y - UNIT WEIGHT  Y - DRY UNIT WEIGHT  S - BULK  S - BULK  S - SULK  S - SPLIT SPOON  ST - SHELBY TUBE  RS - ROCK  RT - RECOMPACTED TRIAXIAL  FRACS FRAGMENTS  W - MOISTURE CONTENT  HAMMER TYPE:  CME-45C  CME-45C  AUTOMATIC MANUAL	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETRICH HAND SPECIMEN.  MODERATELY HARD  EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.  FRACTURE SPACING  IERM  SPACING  VERY WIDE  MORE THAN 10 FEET  WIDE  J THICKNESS  J LERM  THICKNESS  VERY WIDE  MORE THAN 10 FEET  THICKLY BEDDED  J LE THELE  MODERATELY CLOSE  J LE THICKLY BEDDED  J LE T LEST  THE THICK BEDSED  J LE	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF I FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  TOPSOIL (TIS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY77-173  NORTHING: 580156 EASTING: 2131628  ELEVATION: 259.26 FEET
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U.S. STD. SIEVE SIZE	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY CPT - CONE PENETRATION TEST OFF CORE PENETRATION TEST DMT - DILATOMETER TEST DMT - DILATOMETER TEST DMT - DILATOMETER TEST DMT - DILATOMETER TEST DMT - ONDERATIO F - FINE SL SAP SAPROLITIC SS - SPLIT SPOON SS - SPLIT SPOON FRACE FRACTURED, FRACTURES TCR - TRICONE REFUSAL REAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL REACS FRAGMENTS M' - MOISTURE CONTENT HIS - HIGHLY V - VERY  BOULDMAN UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT 10 TO 8 USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL WEA WEATHERED WEAL - WEATHER	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD  CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY HARD  EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  HARD  CAN BE GROOVED OR GOUGED DEADILY BY KNIFE OR PICK, CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT  CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CAPVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERWAIL.  FRACTURE SPACING  TERM  VERY WIDE  MORE THAN 10 FEET  WIDE  3 TO 10 FEET  THICKLY BEDDED  4 FEET  WIDE  MODERATELY CLOSE  1 TO 3 FEET  THICKLY BEDDED  0.06 1- 1.5 FEET  CLOSE  0.16 TO 1 FOOT  VERY THINLY BEDDED  0.08 - 0.03 FEET  THINLY BEDDED  FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SILEKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  TOPSOIL (TIS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY77-173  NORTHING: 580156 EASTING: 2131628  ELEVATION: 259.26 FEET
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U.S. STD. SIEVE SIZE	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL  ABBREVIATIONS  AR - AUGER REFUSAL  BT - BORING TERMINATED  MCA MICACEOUS  WEA WEATHERED  MCA MICACEOUS  WEA WEATHERED  CL CLAY  MOD MODERATELY  CPT - CONE PENETRATION TEST  OMT - DILATOMETER TEST  DMT - SEMPLE ABBREVIATIONS  S - SAPOLITIC  S - BULK  S - SULK  S - SPLIT SPOON  SS - SPLIT SPOON  SS - SPLIT SPOON  ST - SHELBY TUBE  FOSS FOSSILIFEROUS  SLI SLIGHTLY  FRAC FRACTURED, FRACTURES  TCR - TRICONE REFUSAL  RATIO  EQUIPMENT USED ON SUBJECT PROJECT  DRILL UNITS:  ADVANCING TOOLS:  WEA WEATHERED  WEA WEATHERED  WEA WEATHERED  WEA WEATHERED  WEA WEATHERED  WEA WEATHERED  WEA WEATHERET  S BULK  S - BULK  S - BULK  S - SULK  S - SPLIT SPOON  ST - SHELBY TUBE  RS - ROCK  RT - RECOMPACTED TRIAXIAL  CORE - SIZE:  AUTOMATIC  MANUAL  CME-45C  CME-45C  CME-45C  CME-550  HARD FACED FINGER BITS  VANE SHEAR TEST  VANE SHEAR  VANE SHEA	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.  MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED DR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.  FRACTURE SPACING  TERM SPACING  VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 1.5 - 4 FEET THICKLY BEDDED 0.15 - 15 FEET THICKLY BEDDED 0.06 - 1.5 FEET THICKLY BEDDED 0.08 - 1.5 FEET THICKLY BEDDED 0.08 - 1.5 FEET THICKLY LAMINATED 0.0008 - 0.03 - 0.16 FEET THICKLY LAMINATED 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.0008 - 0.000	ROCK.  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  TOPSOIL (TIS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: BY77-173  NORTHING: 580156 EASTING: 2131628  ELEVATION: 259.26 FEET
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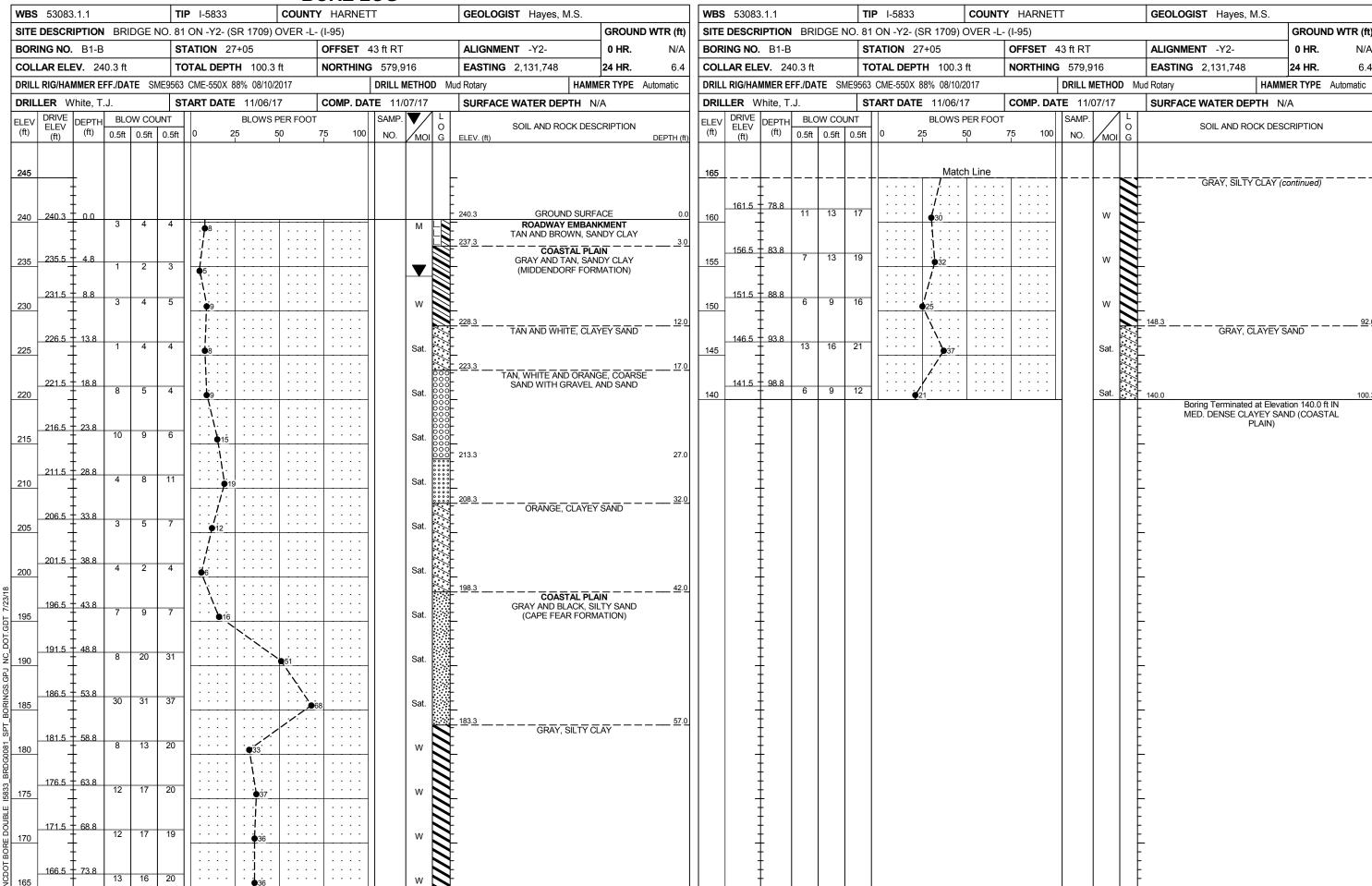




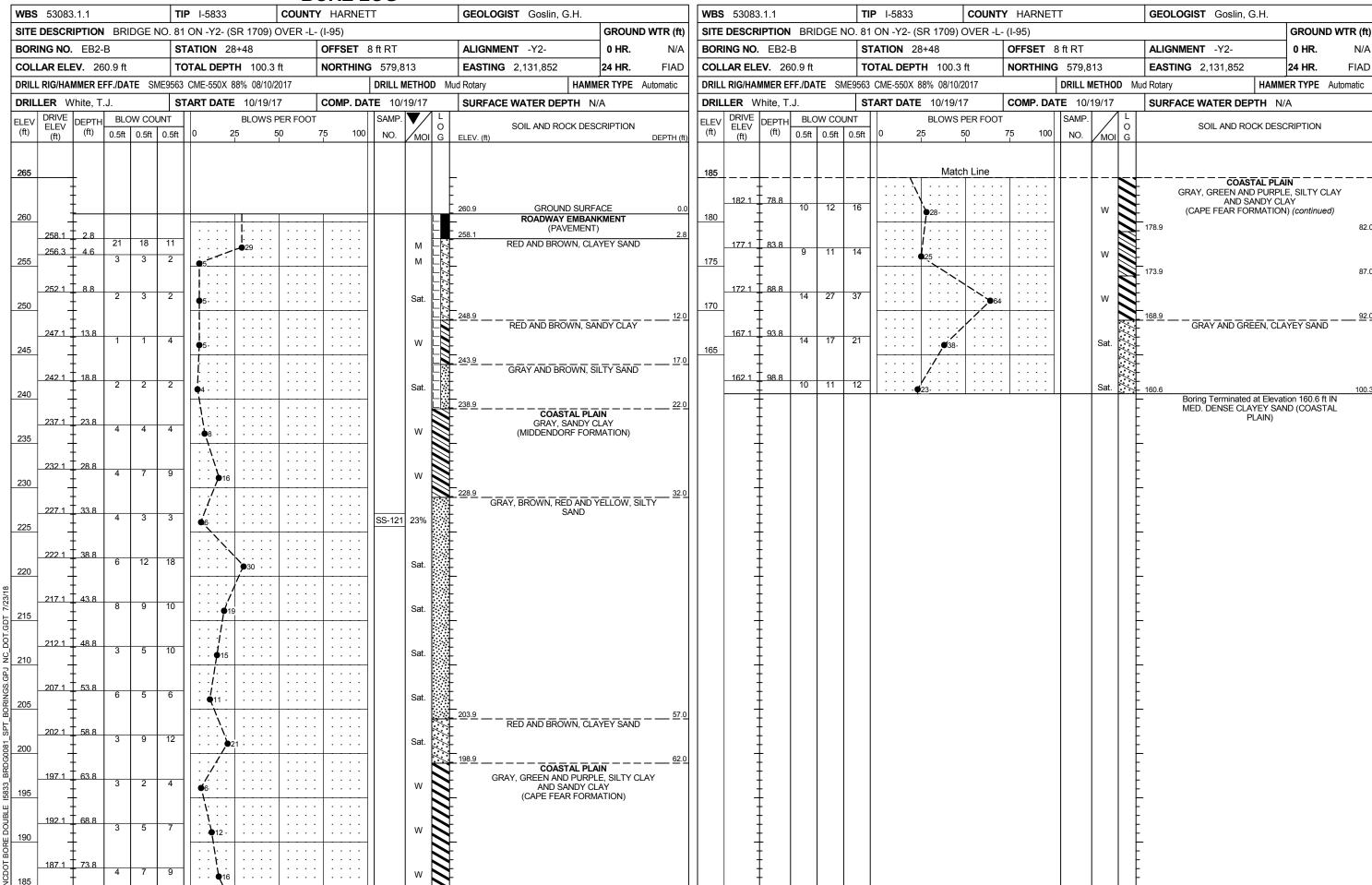


BURE LUG	DECLOSION DI LI ES	TID 1	LEGGG LIADVETT	AFOLOGIOT BL. 11 F.O.
WBS 53083.1.1   TIP I-5833   COUNTY HARNETT	GEOLOGIST Blonshine, E.G.	WBS 53083.1.1 TIP I-		GEOLOGIST Blonshine, E.G.
SITE DESCRIPTION BRIDGE NO. 81 ON -Y2- (SR 1709) OVER -L- (I-95)	GROUND WTR (ft)	SITE DESCRIPTION BRIDGE NO. 81 ON		GROUND WTR (ft)
BORING NO. EB1-B STATION 26+14 OFFSET 25 ft RT	ALIGNMENT -Y2- 0 HR. N/A		ION         26+14         OFFSET         25 ft RT	ALIGNMENT -Y2- 0 HR. N/A
COLLAR ELEV.         238.8 ft         TOTAL DEPTH         100.3 ft         NORTHING         580,003	<b>EASTING</b> 2,131,715 <b>24 HR.</b> 7.0	COLLAR ELEV. 238.8 ft TOTAL	L DEPTH 100.3 ft NORTHING 580,003	<b>EASTING</b> 2,131,715 <b>24 HR.</b> 7.0
DRILL RIG/HAMMER EFF./DATE SME9563 CME-550X 88% 08/10/2017 DRILL METHOD MIL	d Rotary HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE SME9563 CME	E-550X 88% 08/10/2017 DRILL METHOD M	Mud Rotary HAMMER TYPE Automatic
DRILLER White, T.J. START DATE 10/23/17 COMP. DATE 10/24/17	SURFACE WATER DEPTH N/A		T DATE 10/23/17 COMP. DATE 10/24/17	SURFACE WATER DEPTH N/A
ELEV (ft)   DRIVE   DEPTH   BLOW COUNT   BLOWS PER FOOT   SAMP.   V   O   O   O   O   O   O   O   O   O	SOIL AND ROCK DESCRIPTION  ELEV. (ft)  DEPTH (ft)	ELEV (ft)	BLOWS PER FOOT SAMP. CONTROL S	
2 5 3 . • 8 . · · · · · · · · · · · M	238.8 GROUND SURFACE 0.0  ROADWAY EMBANKMENT  BROWN, CLAYEY SAND  235.8 3.0	160	Match Line  Sat.	GRAY AND GREEN, CLAYEY SAND (continued)
235 234.4 4.4 2 3 4 67	COASTAL PLAIN GRAY AND TAN, SANDY CLAY (MIDDENDORF FORMATION)	155 155.0 83.8 10 7 10	w	154.5 84.3 GRAY, SILTY CLAY
230 230.0 8.8 3 3 3 • 66 · · · · · · · · · · · · · · · · ·	231.8 GRAY, CLAYEY SAND	150 150.0 88.8 12 19 21 .	940 Sat.	151.8 — GRAY AND GREEN, CLAYEY SAND — 87.0
225 225.0 13.8 2 3 5		145 145.0 93.8 8 16 16 .		- - - -
220 220.0 18.8 5 4 7 . •11	-	140 140.0 98.8 7 8 11		- 138.5 100.3
215 215.0 23.8	-		<b>▼</b> 19	Boring Terminated at Elevation 138.5 ft IN MED. DENSE CLAYEY SAND (COASTAL PLAIN)
210 210.0 28.8	_			
205 205.0 33.8 3 4 5	206.8 32.0			- - - -
200 200.0 38.8	-			- - - -
195 195.0 43.8	196.8			- - -
190 190.0 48.8	193.8 (CAPE FEAR FORMATION) 45.0 BLACK, SAND			
9 10 35 42	186.8 52.0			
TO 15 10 15 25 W				- - -
8 175 175.0 63.8 W				- - -
00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7400.2			
ш 17 22 25 W	GRAY AND GREEN, CLAYEY SAND			
Sat. 160 160.0 78.8 Sat.				- - - -





\A/D		00.4.4		- 1	TID	1.5000		1	URE			050	N COICT Discussion	- F.O.		\\	5000	2.4.4		1.	TID	1.5000	TV HADNE			0501	OCIOT Disastina		
	5308		L BRIDG			I-5833	D 4700)		Y HARNE	:11		GEC	DLOGIST Blonshir		NATE (ft)	-	5308		DDID				HARNE	11		GEOL	LOGIST Blonshine		JND WTR (ft)
			BRIDG					OVER -L	<del>, `                                   </del>	04.51.T		1	ONIMENT NO		WTR (ft)							ON -Y2- (SR 1709) OVER	<del>`</del>	04.61.T		14110	NIMENIT VO		` '
-		<b>O</b> . EB2-				ION 27			OFFSET				GNMENT -Y2-	0 HR.	N/A		RING NO					TION 27+26	OFFSET				NMENT -Y2-	0 HR.	
		LEV. 23				L DEPTH			NORTHIN				STING 2,131,816		8.5		LAR EL					TAL DEPTH 100.1 ft	NORTHIN				TING 2,131,816	24 HR.	
			FF./DATE						1			Mud Rotar	<u> </u>	HAMMER TYPE	Automatic							ME-550X 88% 08/10/2017	1		METHOD	<del></del>		HAMMER TYPE	E Automatic
		White, T	ı			RT DATE			COMP. D			SUR	RFACE WATER DE	PTH N/A			LLER V					RT DATE 10/18/17	COMP. DA			SURF	ACE WATER DEPT	H N/A	
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft 0.			25		PER FOOT 50	Γ 75 100	SAMP. NO.	/ (	G ELEV.		OCK DESCRIPTION	DEPTH (ft)	ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)		V COUNT 0.5ft 0.5f		BLOWS PER FO	OT 75 100	SAMP.	MOI G		SOIL AND ROC	K DESCRIPTIO	N
240	239.7	0.0	2	2 3		5			: : : : :		M L	239.7	ROADWAY	ND SURFACE Y EMBANKMENT ACK, CLAYEY SAND	0.0	160			+			Match Line	· · · · ·	 		<del> </del>	GRAY AND GREE	N, CLAYEY SA inued)	
235	235.0	4.7	2	3 5							м	236.7	TAN AND GF (MIDDENDC	STAL PLAIN RAY, SANDY CLAY ORF FORMATION)	7.0	155	156.1	83.6	11	11 13		24		-	Sat.				
230	231.1	8.6	3	3 3		6				-	Sat.	227.7		CLAYEY SAND	12.0	150	151.1	88.6	6	8 8	3	16			Sat.	<del>, , , , , , , , , , , , , , , , , , , </del>			
225	226.1	13.6	2	3 4	-  :	7 				SS-96	25% O			, TAN AND PURPLE, ND WITH GRAVEL		145	146.1	93.6	5	6 6	5	• · · · · · · · · · · · · · · · · · · ·		-	Sat.				
220	221.1	18.6	7	7 3		. I				-	Sat. 000	000- 000- 000- 000-				140	141.1	98.6	7	7 10	0	17			Sat.	139.6	Boring Terminated a MED. DENSE CLAY	t Elevation 139.	100.1 6 ft IN
215	216.1	23.6	17 1	14 13	3   .		27				Sat.	00- 00- 00- 00- 00- 00- 212.7			27.0		-	† - - - -								<u> </u>	MED. DENSE CLAY PL	EY SAND (COA AIN)	STAL
210	211.1	28.6	12	7 3		• • • • • • • • • • • • • • • • • • •					Sat.		TAN AND BRO	DWN, CLAYEY SAND	21.0		-	† - - -								[ - -			
205	206.1	33.6	5	5 5		10 10					Sat.						-	† - - - -								<u> </u>			
200	201.1	38.6	3	6 6		12-					Sat.	202.7	————— TAN, §	SILTY SAND	<u>37.0</u>		-	† - -											
195	196.1	43.6	1 1	2 6		· / · · · · /· · · · /- · ·					w	<u>** - 197.7</u> - -	COAS BLACK, (CAPE FEA	STAL PLAIN (, SILTY CLAY AR FORMATION)	42.0_		-	† † †											
190 190 190 190 190 190 190 190 190 190	191.1	48.6	3	3 4		· i · · · · · · · · · · · · · · · · · ·					w						-	<u> </u>											
185 185	186.1	53.6	12 1	17 18	3   .		35—				Sat.	<u>187.7</u>	GRAY AND BI	BLACK, SILTY SAND	<u> </u>		-	<u> </u>								<u>-</u>			
BORINGS.GF	181.1	58.6	11 1	19 24	1   -		\ \ \ .	3			w	182 <u>.7</u> -	GRAY, SILTY CL	LAY AND SANDY CLA	<u> </u>		-	<u> </u>								<u> </u>			
175 175	176.1	63.6	15 1	17 27	7		l.				w						-	<u>+</u> + +								<u>-</u>			
170 170	171.1	68.6	14 2	23 31				54			w	172.7			67.0		-	<u> </u>								<u>-</u>			
JARE DOUBLE	166.1	73.6	9 2	21 28			· · · · · · · · · · · · · · · · · · ·	49			Sat.	167.7	GRAY AND GR	REEN, CLAYEY SAND	<u>72.0</u>		-	Ī - -								[ [ -			
NCDOT B 160	161.1	78.6	13 1	18 19			· · · /· · · · /· · · · /·				Sat.															-			



#### SUMMARY OF LABORATOTY TEST DATA

Soil Classification and Gradation



	S&ME, Inc. Raleigh, 3201 S	Spring Forest Road, Ralei	gh, North Carolina 2761	6	
S&ME Project #:	6235-16-015			Date Report	12/1/2017
State Project No.:	53083.1.1	County:	Harnett	Date Tested	11/1/17-11/30/17
Federal ID No.:	N/A	TIP No.:	I-5883		
Project Name:	Bridge No. 81 on -Y2- (SR	1709) over -L- (I-95)			

Client N	lame:				Michael B	aker Inter	rnatio	nal												
Sample No.	ו #:		:#:	Alignment	Sample	AASH			Tota	al % Pas	_			Mortar	Fraction	n (%)				
ldm	Station #:	Offset	Boring	ignr	Depth	Classific	cation			Sieve #	T .		Coarse				LL	PL	PI	Moist.
					(ft)			10	40	60	200	270	Sand	Sand	Silt	Clay				%
SS-96	27+26	24 LT	EB2-A	Y2	13.6-15.1	A-1-b		57	26	18	8.5	7.7	39	10	1	6	28	0	N.P.	25.3
SS-121			EB2-B	Y2	33.8-35.3	A-2-4		65	30	21	10.6	9.2	44	12	3	7	28	20	8	23.2
SS-146		10 LT	EB1-A	Y2	53.8-55.3	A-3		100	56	20	7.0	5.5	80	15	1	4	18	0	N.P.	23.1
SS-167	26+14	25 RT	EB1-B	Y2	48.8-50.3	A-3	(0)	99	61	17	6.1	4.8	82	12	3	2	17	0	N.P.	23.3
	as / Comm				ND-Not Dot															

References / Comments / Deviations:

ND=Not Detemined.

AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT

AASHTO T89: Determining the Liquid Limit of Soils

AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils

AASHTO T265: Laboratory Determination of Moisture Content of Soils

AASHTO M145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

Mal Krajan, ET Technician Name:

Signature

104-01-0703 Certification #

Stewart Laney, P.E. Technical Responsibility:

Project Manager Position

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# SITE PHOTOGRAPH

Bridge No. 81 on -Y2- (SR 1709) over -L- (I-95)



5986B REFERENCE **CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

CROSS SECTION(S) BORE LOG(S) SITE PHOTOGRAPH(S)

TITLE SHEET

SITE PLAN

PROFILE(S)

SHEET NO.

5-6 7-II

4

#### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY JOHNSTON

PROJECT DESCRIPTION <u>I-95 FROM NORTH OF SR</u> 1002 (LONG BRANCH ROAD) (EXIT 71) TO I-40 (EXIT 81).

SITE DESCRIPTION BRIDGE 653 ON -Y29- (S. MARKET ST.) OVER -L- (I-95)

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
V.C.	I-5986B	1	12

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDING TO CLIMATIC CONDITIONS INCLORDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

E. BLONESHINE J. PREVATTE A. BLYTHE J. SWARTLEY INVESTIGATED BY \_\_\_\_\_S&ME, Inc. CHECKED BY S. MITCHELL

SUBMITTED BY S. MITCHELL DATE JANUARY 2020



9751 SOUTHERN PINE BLVD CHARLOTTE, NC 28273



**DOCUMENT NOT CONSIDERED FINAL** UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO. SHEET NO.

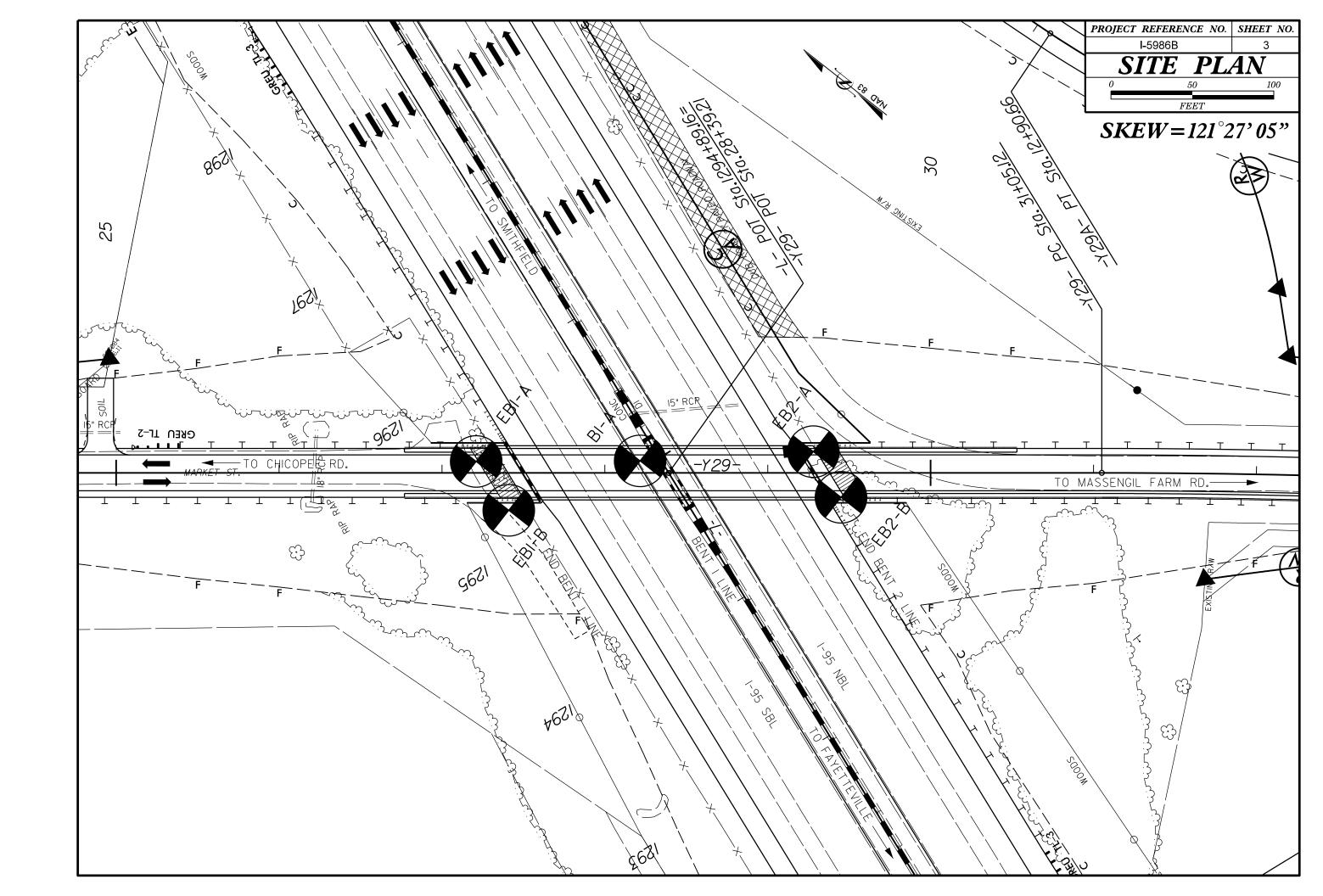
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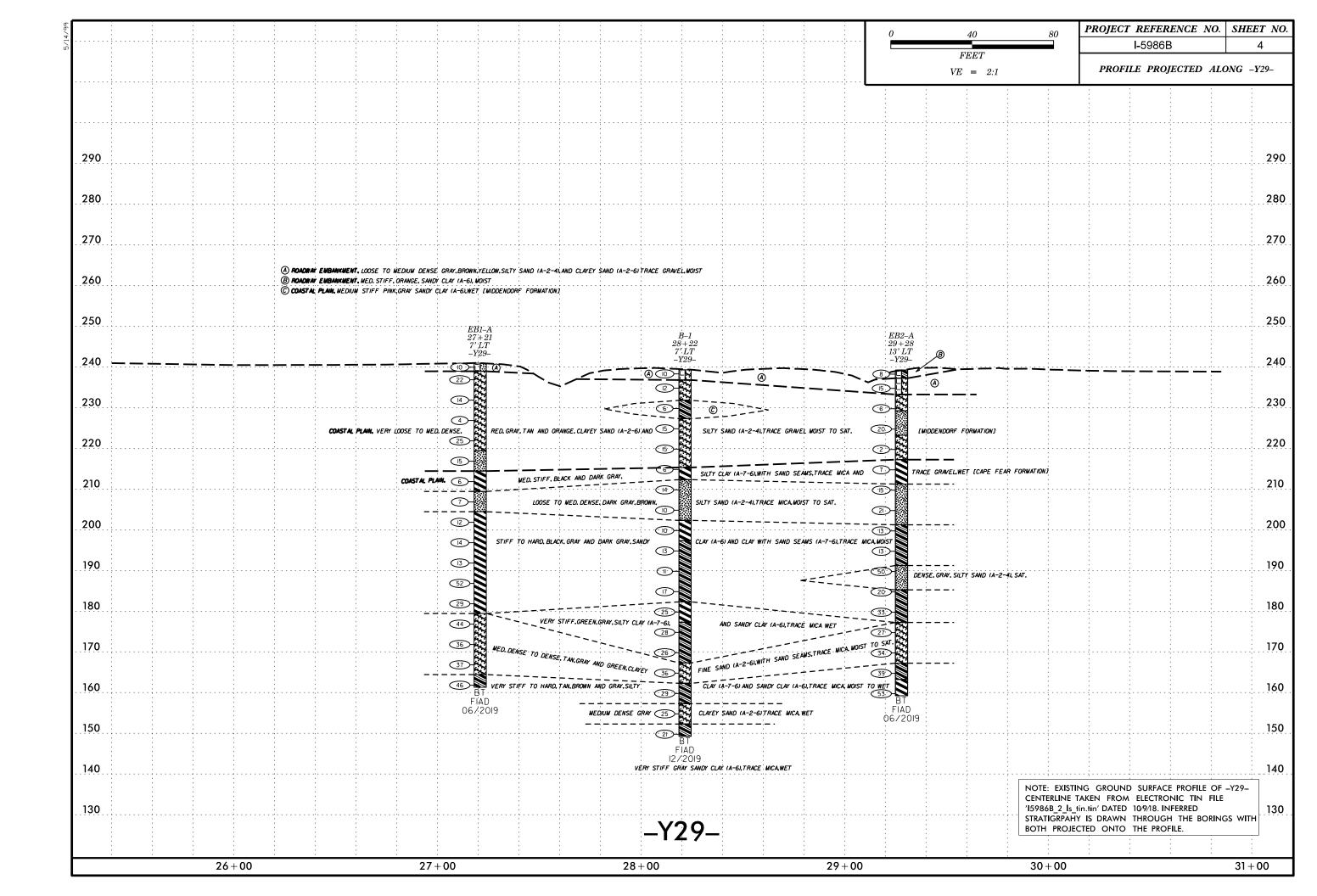
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

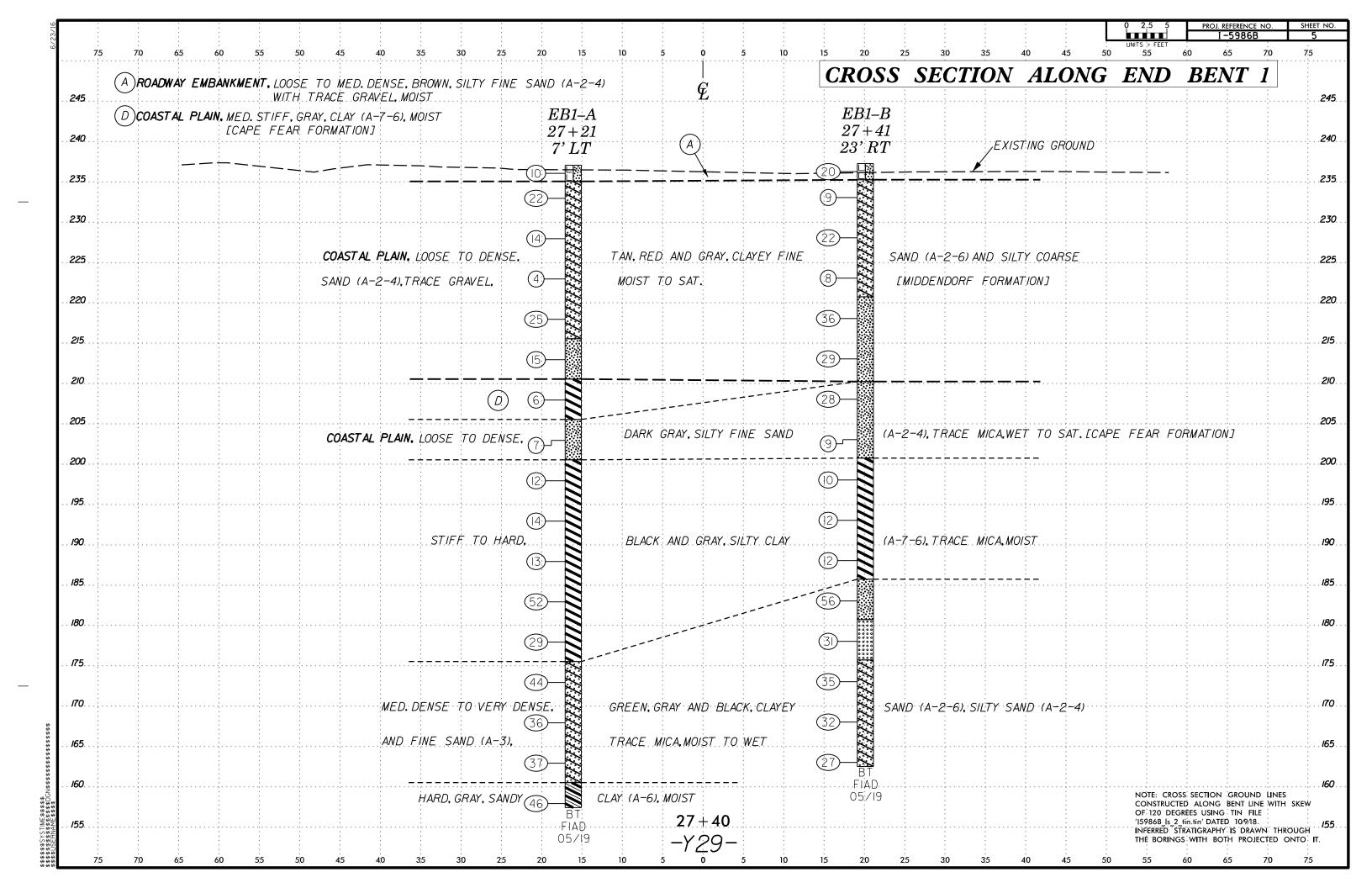
# SUBSURFACE INVESTIGATION

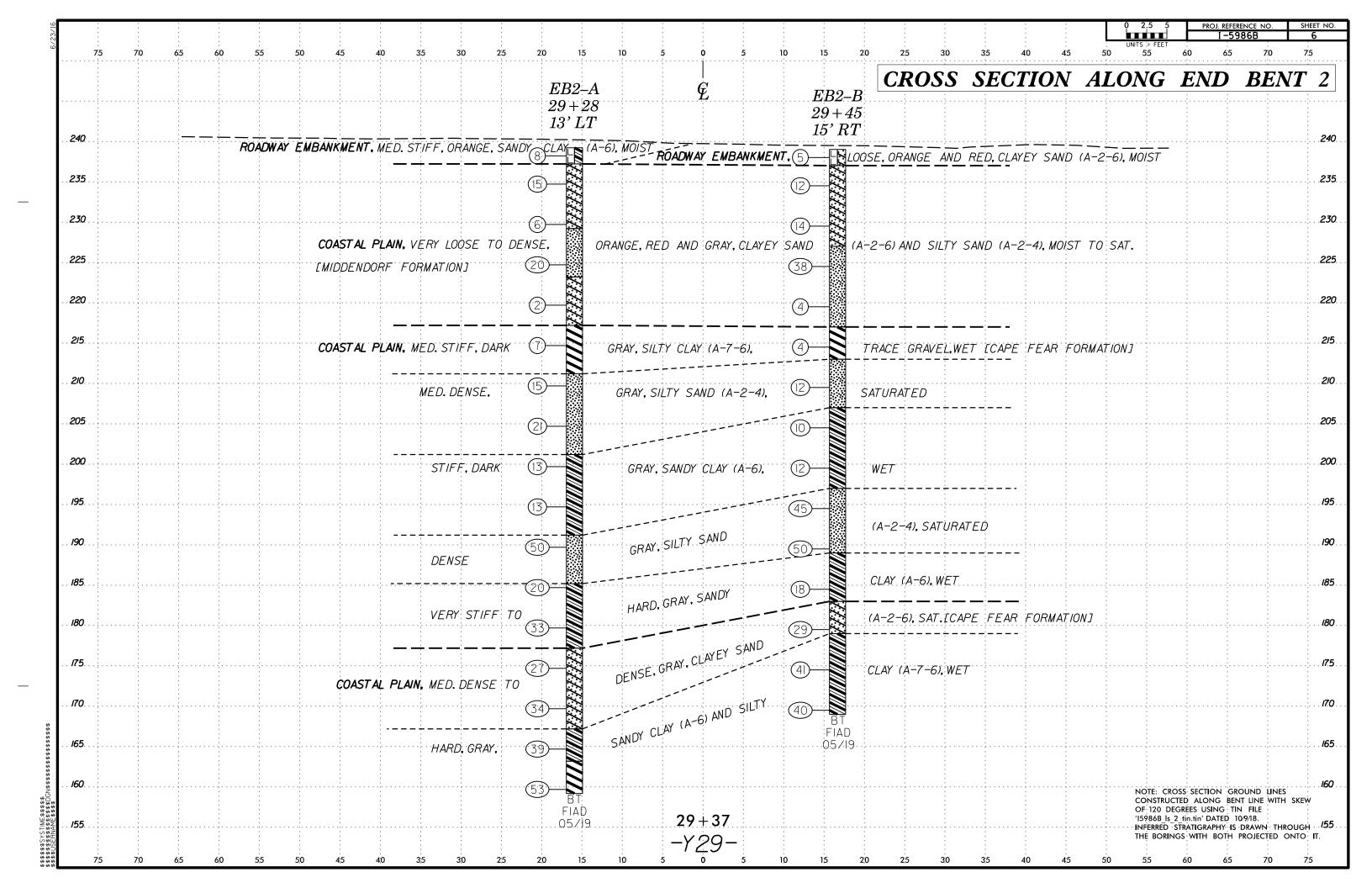
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

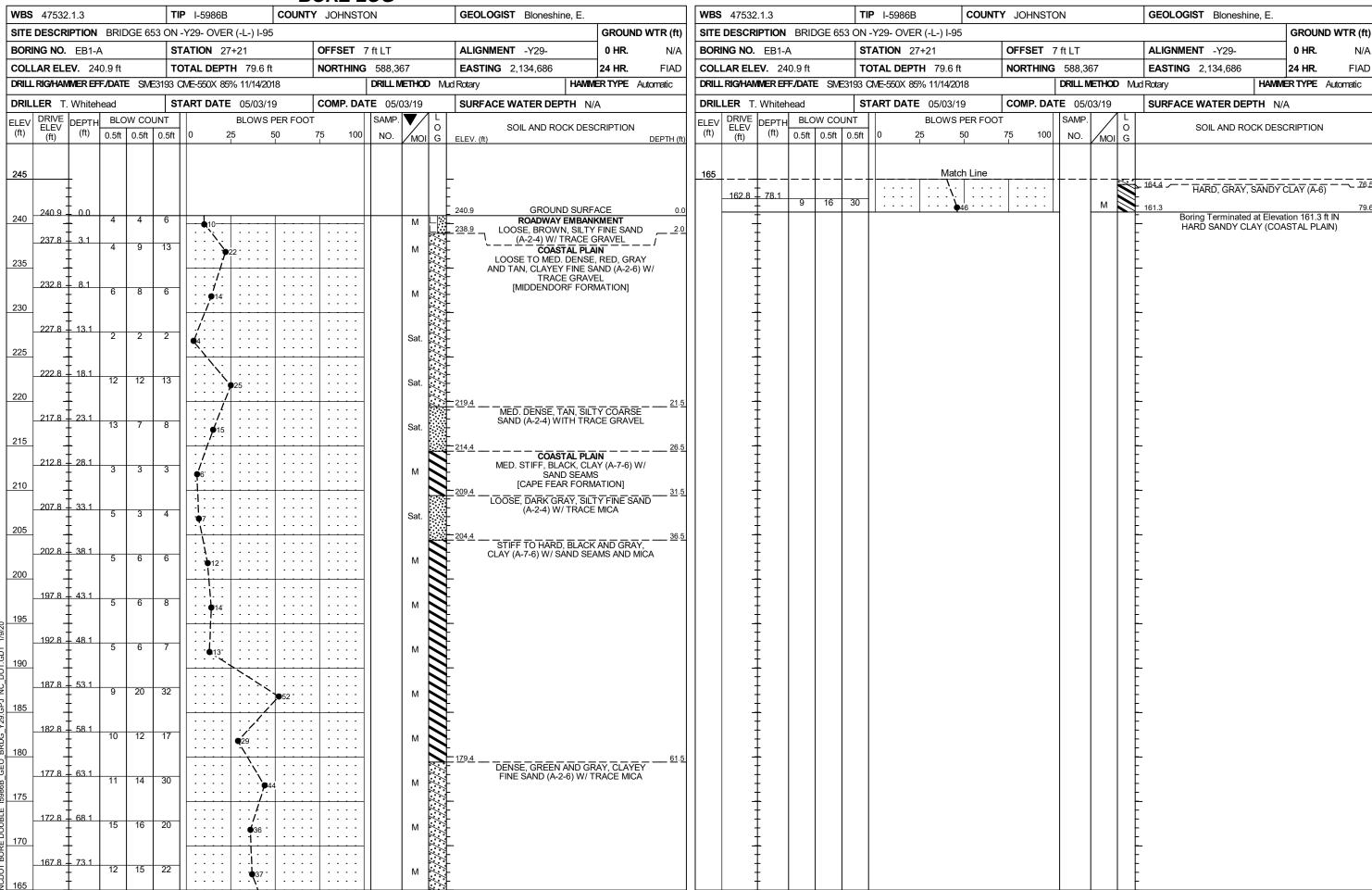
SOIL DESCRIPTION  SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGRER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (ASAPITO T 206, ASTM DISBB), SOIL CLASSIFICATION IS BASED ON THE ASAPITO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, ABSHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH			
BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586), SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586), SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
AS ANTICON COLON, COMPOSITION AND ADDRESS OF DESCRIPTION OF THE PROPERTY OF TH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF.GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
CENERAL CRANIII AR MATERIALS SILT-CLAY MATERIALS	MINERALOGICAL COMPOSITION	THE TO COARSE CRAIN ICNEOUS AND METAMORPHIC ROCK THAT	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	ROCK (CP) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	UNELSS, DABBRU, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-3 A-6, A-7	COMPRESSIBILITY  SUBJECT: V. COMPRESSIBILITY	NON-CRYSTALLINE ROCK (NCR)  SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 0000 d00000 00000 00000 00000 00000 00000 0000	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
% PASSING SILT-	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR CLAY MUCK, *40 30 MX 50 MX 51 MN FINN SOILS CLAY PEAT	PERCENTAGE OF MATERIAL	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*200   15 MX   25 MX   10 MX   35 MX   35 MX   35 MX   35 MX   36 MN   36 MN   36 MN   36 MN   36 MN   36 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%  LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN LITLE UR HIGHLY	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOUS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER		(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
OF MAJOR GRAYEL, AND MATERIALS SAND GRAYEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
CEN BATING FAIR TO	<u> </u>	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	PARENT MATERIAL.
AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	-	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	FIELD.   JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
CONSISTENCY CONSISTENCY (N-VALUE) COMPRESSIVE STRENGTH	₩ITH SOIL DESCRIPTION → OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4	SOIL SYMBOL  SOIL SYMBOL  SPT DPT DPT DPT TEST BORING  SLOPE INDICATOR INSTALLATION	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR LOOSE 4 TO 10  GRANULAR MEDIUM DENSE 10 TO 30  N/A		IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS
MATERIAL DENSE 30 TO 50 (NON-COHESIVE)	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY DENSE         > 50           VERY SOFT         < 2	- INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5	MN - TECT DODING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY   MEDIUM STIFF   4 TO 8   0.5 TO 1.0   MATERIAL   STIFF   8 TO 15   1 TO 2	INFERRED ROCK LINE "O MONITORING WELL WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4	TTTT ALLUVIAL SOIL BOUNDARY A PIEZOMETER INSTALLATION - SPT N-VALUE	ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
HARD > 30 > 4		ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNDERCUT  UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
COARCE FINE	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SAND SILT CLAY		MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(CSE. SD.) (F SD.)	ABBREVIATIONS	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN. 12 3	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
	CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS	CPT - CONE PENETRATION TEST NP - NON PLASTIC 7/d - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN Ø.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION  (ATTERBERG LIMITS) DESCRIPTION	CSE COARSE ORG ORGANIC  DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
(SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO   SD SAND, SANDY   SS - SPLIT SPOON   F - FINE   SL SILT, SILTY   ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
LL _ LIQUID LIMIT	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
	FRAC FRACTURED, FRACTURES  TCR - TRICONE REFUSAL  RT - RECOMPACTED TRIAXIAL  RT - RECOMPACTED TRIAXIAL  W - MOISTURE CONTENT  CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: BY6-202 N: 588609 E: 2134438
PLASTIC   SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTIBE	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	BENCH MHRK: B16-202 N: 388603 E: 2134438
PLASTIC PLASTIC SEMISOLID: REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE		VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	ELEVATION: 241.76 FEET
RANGE - WET - (W) SEMISULIS REQUIRES DRING TO ATTAIN OPTIMUM MOISTURE  - WET - (W) SCI.ID. AT OR NEAR ORTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT		
RANGE - WET - (W) SEMISULIS REQUIRES DRING TO ATTAIN OPTIMUM MOISTURE  - WET - (W) SEMISULIS REQUIRES DRING TO ATTAIN OPTIMUM MOISTURE  - WET - (W) SEMISULIS REQUIRES DRING TO THE OPTIMUM MOISTURE  - WET - (W) SEMISULIS REQUIRES DRING TO THE OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	
RANGE (PI) PL PLASTIC LIMIT  OM OPTIMUM MOISTURE SL SHRINKAGE LIMIT  - WET - (W) ATTAIN OPTIMUM MOISTURE SL SHRINKAGE LIMIT  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT		MODERATELY CLOSE	NOTES:
RANGE - WET - (W) SEMISULIS REQUIRES DRING TO ATTAIN OPTIMUM MOISTURE  - WET - (W) SEMISULIS REQUIRES DRING TO ATTAIN OPTIMUM MOISTURE  - WET - (W) SEMISULIS REQUIRES DRING TO THE OPTIMUM MOISTURE  - WET - (W) SEMISULIS REQUIRES DRING TO THE OPTIMUM MOISTURE	DRILL UNITS:  ADVANCING TOOLS:  HAMMER TYPE:  X CLAY BITS  X AUTOMATIC MANUAL  6' CONTINUOUS FLIGHT AUGER	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	
RANGE (PI) PL PLASTIC LIMIT  OM OPTIMUM MOISTURE SL SHRINKAGE LIMIT  - WET - (W) STATIAIN OPTIMUM MOISTURE SL SHRINKAGE LIMIT  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SR SHRINKAGE LIMIT  - DRY - (D) REQUIRES ADDITIONAL WATER TO	DRILL UNITS:  ADVANCING TOOLS:  HAMMER TYPE:  X CLAY BITS  CACCONTINUOUS FLIGHT ANDER	MODERATELY CLOSE	NOTES:
RANGE { (P1) PL	DRILL UNITS:         ADVANCING TOOLS:         HAMMER TYPE:           CME-45C         X         CLAY BITS         X         AUTOMATIC         MANUAL           CME-55         X         8* HOLLOW AUGERS         CORE SIZE:         -8        H        H	MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.16 - 1.5 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 0.008 FEET THINLY LAMINATED 0	NOTES:
RANGE (P!) PL PLASTIC LIMIT  OM OPTIMUM MOISTURE SHRINKAGE LIMIT  OM OPTIMUM MOISTURE SHRINKAGE LIMIT  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  PLASTICITY  NON PLASTIC  PLASTICITY INDEX (P!) DRY STRENGTH  NON PLASTIC  0-5 VERY LOW	DRILL UNITS:	MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.16 - 1.5 FEET VERY CLOSE LESS THAN 0.16 FEET THINLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 0.008 FEET THINLY LAMIN	NOTES:
RANGE { (P1) PL	DRILL UNITS:  DRILL UNITS:  CME-45C  ADVANCING TOOLS:  X CLAY BITS  G* CONTINUOUS FLIGHT AUGER  X 8* HOLLOW AUGERS  HARD FACED FINGER BITS  TUNG, -CARBIDE INSERTS  HAND TOOLS:  HAND TOOLS:	MODERATELY CLOSE 1 TO 3 FEET VERY CLOSE 0.16 - 1.5 FEET VERY CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.16 - 1.5 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 0.008 FEET THINLY	NOTES:
RANGE (P!) PL PLASTIC LIMIT  OM OPTIMUM MOISTURE SHRINKAGE LIMIT  OM OPTIMUM MOISTURE SHRINKAGE LIMIT  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  PLASTICITY  NON PLASTIC  PLASTICITY INDEX (P!) DRY STRENGTH  NON PLASTIC  0-5 VERY LOW	DRILL UNITS:  DRILL UNITS:  ADVANCING TOOLS:  X CLAY BITS  G'CONTINUOUS FLIGHT AUGER  X CME-55  X CME-550  HARD FACED FINGER BITS  TUNG,-CARBIDE INSERTS  YANE SHEAR TEST  CASING W/ ADVANCER  HAND TOOLS:  POST HOLE DIGGER	MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.16 - 1.5 FEET VERY CLOSE LESS THAN 0.16 FEET THINLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 0.008 FEET THINLY LAMIN	NOTES:
RANGE (PI) PL PLASTIC LIMIT  OM OPTIMUM MOISTURE SL SHRINKAGE LIMIT  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  SHRINKAGE LIMIT  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  PLASTICITY  NON PLASTIC SLIGHTLY PLASTIC SLIGHTLY PLASTIC MODERATELY PLASTIC 16-25 MEDIUM	DRILL UNITS:  ADVANCING TOOLS:  X CLAY BITS  G'CONTINUOUS FLIGHT AUGER  X CME-55  X CME-550  HARD FACED FINGER BITS  TUNG,-CARBIDE INSERTS  YANE SHEAR TEST  PORTABLE HOIST  ADVANCING TOOLS:  ADVANCING TOOLS:  ADVANCING TOOLS:  TRICONE  STEEL TEETH  HAMMER TYPE:  X AUTOMATIC MANUAL  CORE SIZE:       HAND TOOLS:  POST HOLE DIGGER  HAND AUGER	MODERATELY CLOSE  0.16 TO 1 FOOT  VERY CLOSE  0.16 TO 1 FOOT  VERY CLOSE  1 TO 3 FEET  VERY CLOSE  1 TO 3 FEET  VERY THINLY BEDDED  0.083 - 0.16 FEET  THICKLY LAMINATED  0.008 - 0.09 FEET  THINLY LAMINATED  1 NOURATION  FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.  RUBBING WITH FINGER FREES NUMEROUS GRAINS;  GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.  MODERATELY INDURATED  ROALINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;  BREAKS EASILY WHEN HIT WITH HAMMER.	NOTES:
RANGE (PI) PL PLASTIC LIMIT  OM OPTIMUM MOISTURE SHRINKAGE LIMIT  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  PLASTICITY  NON PLASTIC SLIGHTLY PLASTIC MODERATELY PLASTIC HIGHLY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH  COLOR	DRILL UNITS:  DRILL UNITS:  CME-45C  X CLAY BITS  G'CONTINUOUS FLIGHT AUGER  X CME-55  B'HOLLOW AUGERS  HARD FACED FINGER BITS  TUNGCARBIDE INSERTS  YANE SHEAR TEST  PORTABLE HOIST  TRICONE  TRICONE  TRICONE  TRICONE  TRICONE  TRICONE  TUNGCARB.  HAMMER TYPE:  X AUTOMATIC  MANUAL  CORE SIZE:  -BH	MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 - 1.5 FEET CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.16 - 1.5 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 0.008 FEET THINLY LAM	NOTES:
RANGE 2 (P1) PL PLASTIC LIMIT  OM OPTIMUM MOISTURE SL SHRINKAGE LIMIT  - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE  SHRINKAGE LIMIT  - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  PLASTICITY  NON PLASTIC SLIGHTLY PLASTIC MODERATELY PLASTIC HIGHLY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	DRILL UNITS:  ADVANCING TOOLS:  X CLAY BITS  G'CONTINUOUS FLIGHT AUGER  X 8* HOLLOW AUGERS  HARD FACED FINGER BITS  TUNGCARBIDE INSERTS  X CASING W/ ADVANCER  PORTABLE HOIST  TRICONE 'TUNGCARB.  HAMMER TYPE:  X AUTOMATIC MANUAL  CORE SIZE:	MODERATELY CLOSE  0.16 TO 1 FOOT  VERY CLOSE  VERY CLOSE  1 TO 3 FEET  VERY CLOSE  1 TO 3 FEET  VERY CLOSE  1 TO 1 FOOT  VERY THINLY BEDDED  0.16 - 1.5 FEET  VERY THINLY BEDDED  0.008 - 0.03 FEET  THINLY LAMINATED  0.008 - 0.03 FEET  THINLY LAMINATED  0.008 FEET  INDURATION  FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.  RUBBING WITH FINGER FREES NUMEROUS GRAINS;  GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.  MODERATELY INDURATED  GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;  BREAKS EASILY WHEN HIT WITH HAMMER.  CRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	NOTES:





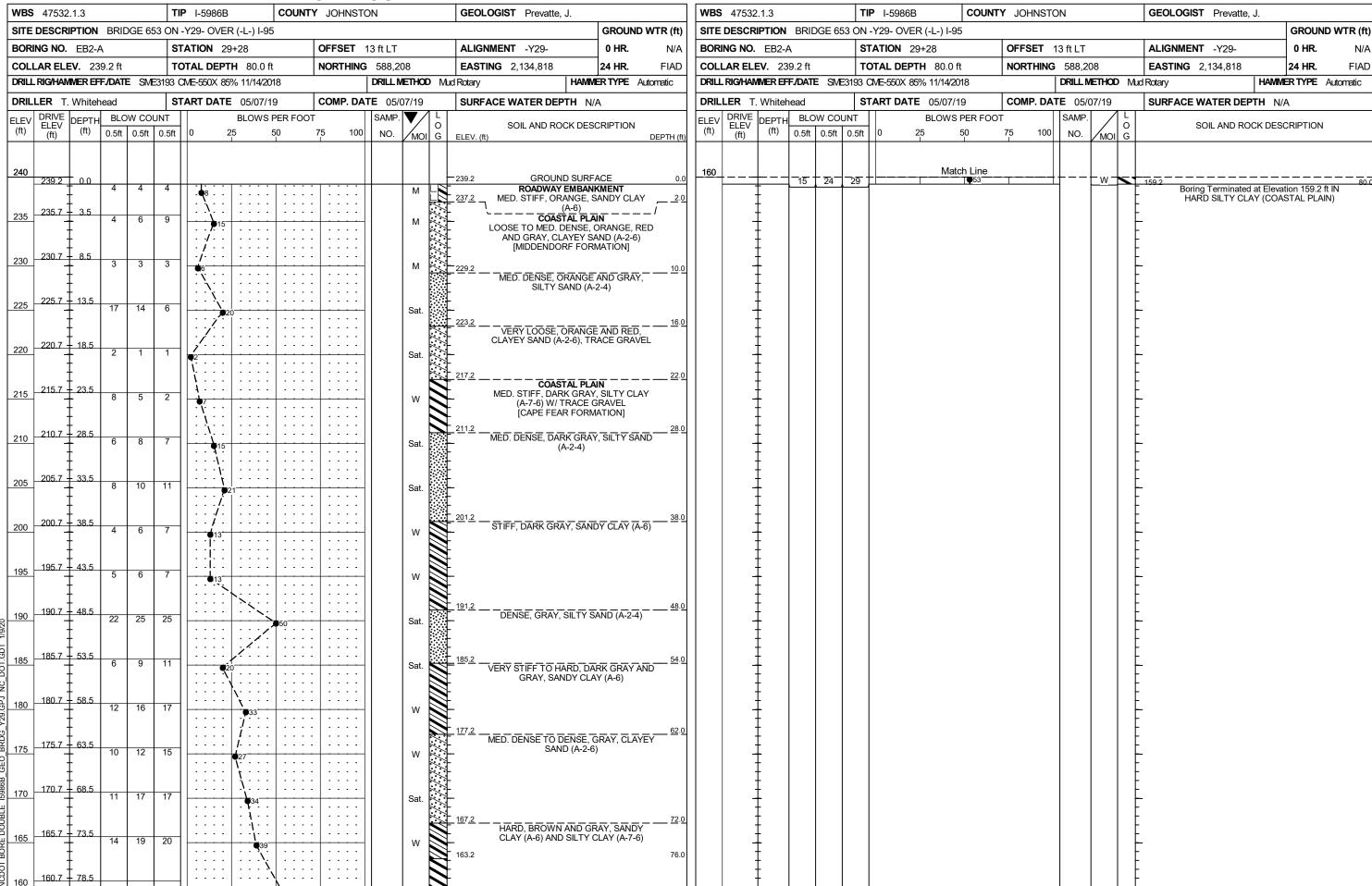






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	Ī	3	4	6		10		.			М	I°3I		EMBANKMENT BROWN, CLAYEY F	INE 2.5			Ī											(A-6) TR	ACE MICA (continued)	)
235.8	3 + 3.5	5 4	6	6	┨┟	.l _ <b></b>				-	w		1	BROWN, CLAYEY F ID (A-2-6) W/ TRACE RAVEL	=	155	155.8	83.5	6	12	13	· · · · · j	25				l w		SAND (A	SE, GRAY, CLAYEY 1-2-6), TRACE MICA	FINE
	1					7:::		.		:	<i>***</i>		MEDIUM DENSE,	<b>TAL PLAIN</b> RED, GRAY, CLAYE	Υ			<u> </u>				: : : : <b>]</b>					''		450.0		07.0
230.8	3 + 8.5	5			-	/: : :		.			<i>**</i> :	231.8	_ FINE TO COA	RSE SAND (A-2-6) RF FORMATION]	7.5		150.8	- 88.5				::::/							152.3VERY STIFF, (	RAY, SANDY CLAY	(A-6), — 87.0
230	Ŧ	2	3	3	$\exists \vdash$	6	<u> </u>				w	<b>}</b>	MEDIUM STIFF, I	PINK GRAY, COĀRS	<u>-</u> '	150			10	9	12	21			<u> </u>	Ц	W		149.3	IVACE IVIICA	90.0
	Ī					<b>\</b>		.		-		227.3		CLAY (A-6)	12.0			<u> </u>										E		ited at Elevation 149.3 SANDY CLAY (COAS	
225.8	3 13.	.5	7	8	$\frac{1}{2}$	<u>\</u>	<u> </u>				W	<del>\</del>	MEDIUM DENSE, T FINE TO COA	AN, YELLOW, CLAY	r⊨Y			<u>†</u>										E		PLAIN)	
	Ŧ					<b>▼</b> 15				:]	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,						-	1										E			
220.9	I 18.	5				111	: : : :	.			<i>`</i> ,							<del>-</del>										l F			
220	<del> </del>	3	1	14	٦Ŀ	15					W 🔆						_	<del>-</del>										ΙF			
	Ŧ					7	: : : :	.			<i>/</i> /,							Ŧ										l F			
215.8	3 <del> </del> 23.	.5 5	4	2	415	/	: : : :	.		-	\\\\	215.3			24.0			Ŧ										F			
	Ŧ	ľ	'	-		<b>/</b> ····				-	W	5	MED. STIFF, DAF	T <b>AL PLAIN</b> RK GRAY, SILTY CLA	ΑY			Ŧ										F			
	, ‡ <u>.</u> .	_				/ : :		:   : : :		-		212.3	7 (A-7-6) W/ SAND CAPE FEA	SEAMS, TRACE MIC R FORMATION]	CA <u>27.0</u>			‡													
210	3 + 28.	5 1	5	9	┧┟╌	14-				_	w	<u>.</u>	MEDIUM DENS	E TO LOOSE, DARK			_	‡										-			
	‡					-		.		:			TRA	INE SAND (A-2-4) W CE MICA	/			‡													
205.8	3 + 33.	.5		<u> </u>	_  :	1		:   : : :				<u>::</u>						‡										-			
203	‡	4	3	'		10				-	W	<b>∷</b> -					-	‡													
	‡							:   : : :				202.3	STIFF DARK GRA	AY, SILTY CLAY (A-7	37.0			‡										-			
200 200.8	3 + 38.	.5 2	4	6	┨┟╌	10					w	\$	W/ SAND SEA	AMS, TRACE MICA	٥,		-	‡													
	‡					<i>]</i> : :		:   : : :		:		197.3			42.0			‡													
195.8	3 + 43.	.5			_  :	-1		-		-		197.3 -	STIFF TO VERY	STIFF GRAY, SAND ), TRACE MICA	Y			‡													
195	‡	3	6	7		13		<del>                                     </del>		<del>.</del>	W	*	SEAT (A-0	,, 110 OE WIOA			-	‡										-			
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190 190.8	3 + 48.	.5 4	4	7	┧┟	.1 		<u> </u>		<u>.</u>	w	\$					-	‡													
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185.8	+ 3 + 53.	.5			_  :	] [		.		:		\$						‡										-			
185	‡	28	8	9		17				-	W	*					-	‡										-			
2	‡				:	<i>i, 'j</i>		.		:		182.3		EN GRAY, SILTY CL	<u>57.0</u>		:	‡													
180.8	3 + 58.	5 7	10	15	$\dashv$	\	25	<u> </u>		-	w	\$	(A-7-5) W/ SAND	SEAMS, TRACE MIC	Ä		_	‡										<u> </u>			
	‡				:	'	<u> </u>	.		:		177 9			62.0			‡													
175 8	3 + 63.	.5			] :		<u> </u>	.		:		<b>*</b> ""-	VERY STIFF GRA	Y, SANDY CLAY (A-	6),			‡													
175	+	13	9	19	$ \cdot $		28	<del></del>		<u>-</u>	w	<b>*</b>	IRA	NOE IVIIOA			-	‡										-			
5	‡				:			.		:		<b>\$</b>					:	‡										<u> </u>			
170.8	68.	5 8	12	14	٦Ŀ		j	.	-	<u>.</u>	w	<b>*</b>					-	<u>†</u>										L			
	Ī						\ \	.		:	<i>"</i>	<b>1</b>					-	<u>†</u>										l E			
165 0	3 73.	5			$ \cdot $		\	.		-	<b>,</b>	167.3	DENSE, GRAY,	CLAYEY FINE SAND	<u>72.0</u>			<u> </u>										F			
165	<del>Ŧ</del> ′³	11	14	22	<b></b> ∏-		36	+	1		W	<del>\</del>	(A-2-6),	TRACE MICA			-	<del>-</del>										F			
	Ŧ				11		: /: : :	.		:	<i>%</i> ,/,	162.3					:	Ŧ										F			
160.8	3 ‡ 78.	5	14	15	<b>  </b>		'/: : :	.		:		¥		, GRAY, SANDY CLA RACE MICA	AY ——-		:	‡													
		1 12	14				L						(,, , , ,																		



							ORE LUG		<u> </u>	
WBS	47532	.1.3			T	TIP I-5986B COUNT	/ JOHNSTON		GEOLOGIST Prevatte, J.	
SITE	DESCR	IPTION	BRID	OGE 65	3 ON	N-Y29- OVER (-L-) I-95				GROUND WTR (ft)
BORI	NG NO.	EB2-E	3		S.	<b>STATION</b> 29+45	<b>OFFSET</b> 15 ft RT		ALIGNMENT -Y29-	<b>0 HR</b> . N/A
COLI	AR ELE	<b>EV</b> . 23	9.0 ft		T	OTAL DEPTH 70.0 ft	NORTHING 588,17	77	<b>EASTING</b> 2,134,807	24 HR. FIAD
DRILL	.RIG/HAIV	MER EF	F./DAT	E SME	3193	CME-550X 85% 11/14/2018	DRILL M	ETHOD Mud	Rotary <b>HAMM</b>	ER TYPE Automatic
DRIL	LER T.	Whiteh	ead		S	START DATE 05/08/19	COMP. DATE 05/0	08/19	SURFACE WATER DEPTH N/	A
ELEV	DRIVE	DEPTH	BLC	W COL		BLOWS PER FOOT	SAMP.	V/L		ODIDTION
(ft)	ELEV (ft)	(ft)	0.5ft	0.5ft	0.5ft	0 25 50	75 100 NO.	MOI G	SOIL AND ROCK DESC ELEV. (ft)	CRIPTION DEPTH (ft)
240										
240	239.0	0.0			_				239.0 GROUND SURF	
	-	-	5	3	2	5		M	ROADWAY EMBANI 237.0 LOOSE, ORANGE AND R	
235	235.5	3.5	5	6	6	_		<del>\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</del>	\ SAND (A-2-6 COASTAL PLA	
	-	-	5	"	O	12		M	MED. DENSE, ORANGE	AND RED,
	-							<del>,,,</del>	CLAYEY SAND (A [MIDDENDORF FORI	
230	230.5	8.5	7	6	8	<u> </u>		M	•	•
	-				Ū	14		N		
	-	-						<u> </u>	227.0 LOOSE TO DENSE, ORAN	GE AND RED 12.0
225	225.5	13.5	14	17	21			Sat.	SILTY SAND (A-	
	-					38				
	-	40.5				:::: ::::				
220	220.5	18.5	5	1	3		<del>                                     </del>	Sat.		
	-								217.0	22.0
045	215.5 <sup>-</sup>	23.5							COASTAL PLA	<u>1N</u> <u>22.0</u>
215		20.0	1	2	2	7 <del>  •</del> 4 · · · ·   · · · · · · · · · · · · · ·	+	w 🛨	SOFT, DARK GRAY, SILTY [CAPE FEAR FORM	ATION1
	-	-							213.0 MED. DENSE, GRAY, SILTY	26.0
210	210.5	28.5				] :';':: :::: ::::			MEB. BENGE, GIVIT, GIET	1 3/11/2 (/ (2 1)
210	_		4	5	7	• 12. · · · · · · · · · · · · · · · · · · ·	<del>   </del>	Sat.		
	-					:			207.0	32.0
205	205.5	33.5				] :;:: :::: ::::			STIFF, DARK GRAY, SAND	OY CLAY (A-6)
200	-	-	4	5	5	10		w S		
	-									
200	200.5	38.5	4	6	6	_  : :::::				
	-	-	-		O	12		W		
	-	-							197.0	42.0
195	195.5	43.5	17	23	22			e <sub>3</sub> ,	DENSE, DARK GRAY, S (A-2-4)	BILLY SAIND
	-	<u> </u>			-	$\begin{vmatrix} \cdot \cdot \cdot \cdot \cdot & \cdot \cdot \cdot & \bullet^{45} \cdot \cdot \cdot \cdot \cdot \\ \cdot \cdot \cdot & \lambda & \cdot \cdot \cdot & \cdot \end{vmatrix}$	1::::	Sat.		
	-	L ,_				\/				
190	190.5	48.5	9	28	22			Sat.	189.0	50.0
	-	ļ							VERY STIFF, GRAY, SAND	DY CLAY (A-6)
	- 185.5 -	52 5					::::			
185	100.5	JS.5	7	8	10	18	<del>                                     </del>	w 🕽		
	-	-							183.0MED. DENSE, GRAY, CL	AYEY SAND 56.0
100	- 180.5	58.5				<u> </u>		<del>,,,,</del>	(A-2-6)	ALL CAND
180			8	11	18	29	<del>                                     </del>	Sat.	179.0	60.0
	-	<u> </u>							HARD, GRAY, SANDY	CLAY (A-6)
175	175.5 <sup>-</sup>	63.5	1.0		00					
	-	-	10	12	29	41		w S		
	-	<u> </u>				:::: ::: :::				
170	170.5	68.5	13	18	22					
			13	10		■ 40		W	Boring Terminated at Eleva	70.0 tion 169.0 ft IN
	-	-						‡	HARD SANDY CLAY (COA	ASTAL PLAIN)
	-									
	-	ţ l								
	_	-						F		
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# SITE PHOTOGRAPH

Bridge 653 on -Y29- over -L- (I-95)



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**CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

BORE LOG(S) & CORE REPORT(S)

CROSS SECTION(S)

SOIL TEST RESULTS

SITE PHOTOGRAPH(S)

TITLE SHEET

SITE PLAN

PROFILE(S)

SHEET NO.

5-8

9-18

19

20

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY JOHNSTON

PROJECT DESCRIPTION <u>I-95 WIDENING FROM SR</u> 1811 (BUD HAWKINS RD.) (EXIT 70) TO I-40 (EXIT 81) -WIDEN TO EIGHT LANES

SITE DESCRIPTION BRIDGE NO. 654 ON I-95 OVER MINGO SWAMP

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	I-5986B	1	20

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDING TO CLIMATIC CONDITIONS INCLORDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

E. BLONESHINE

M. HARTMAN T. WHITEHEAD

A. BLYTHE

J. SWARTLEY

INVESTIGATED BY \_\_\_\_\_S&ME, Inc.

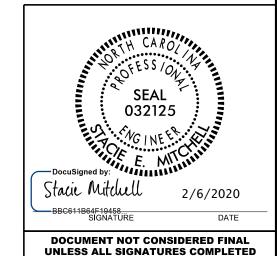
CHECKED BY S. MITCHELL

SUBMITTED BY S. MITCHELL

DATE \_\_FEBRUARY 2020



9751 SOUTHERN PINE BLVD CHARLOTTE, NC 28273



PROJECT REFERENCE NO. SHEET NO.

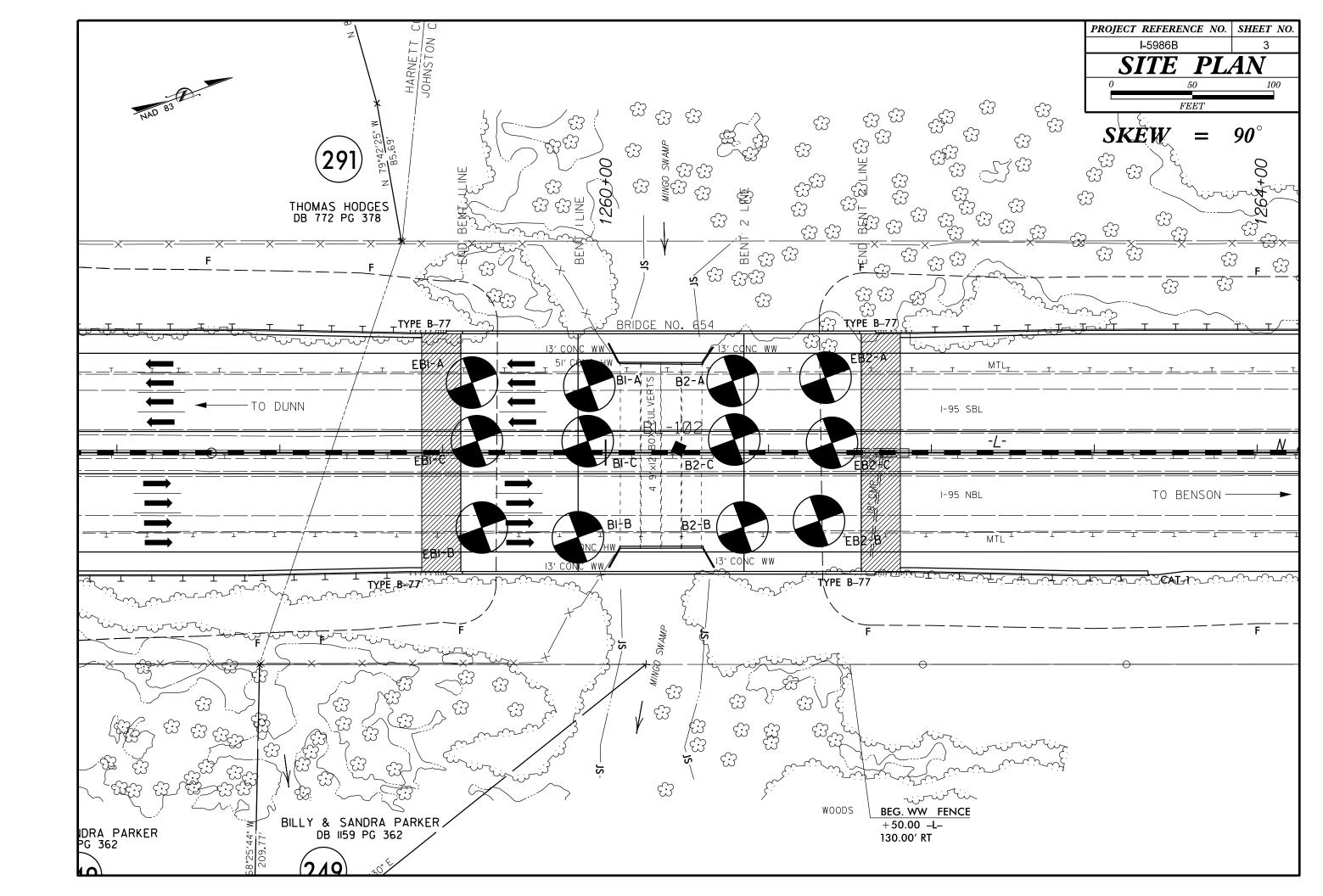
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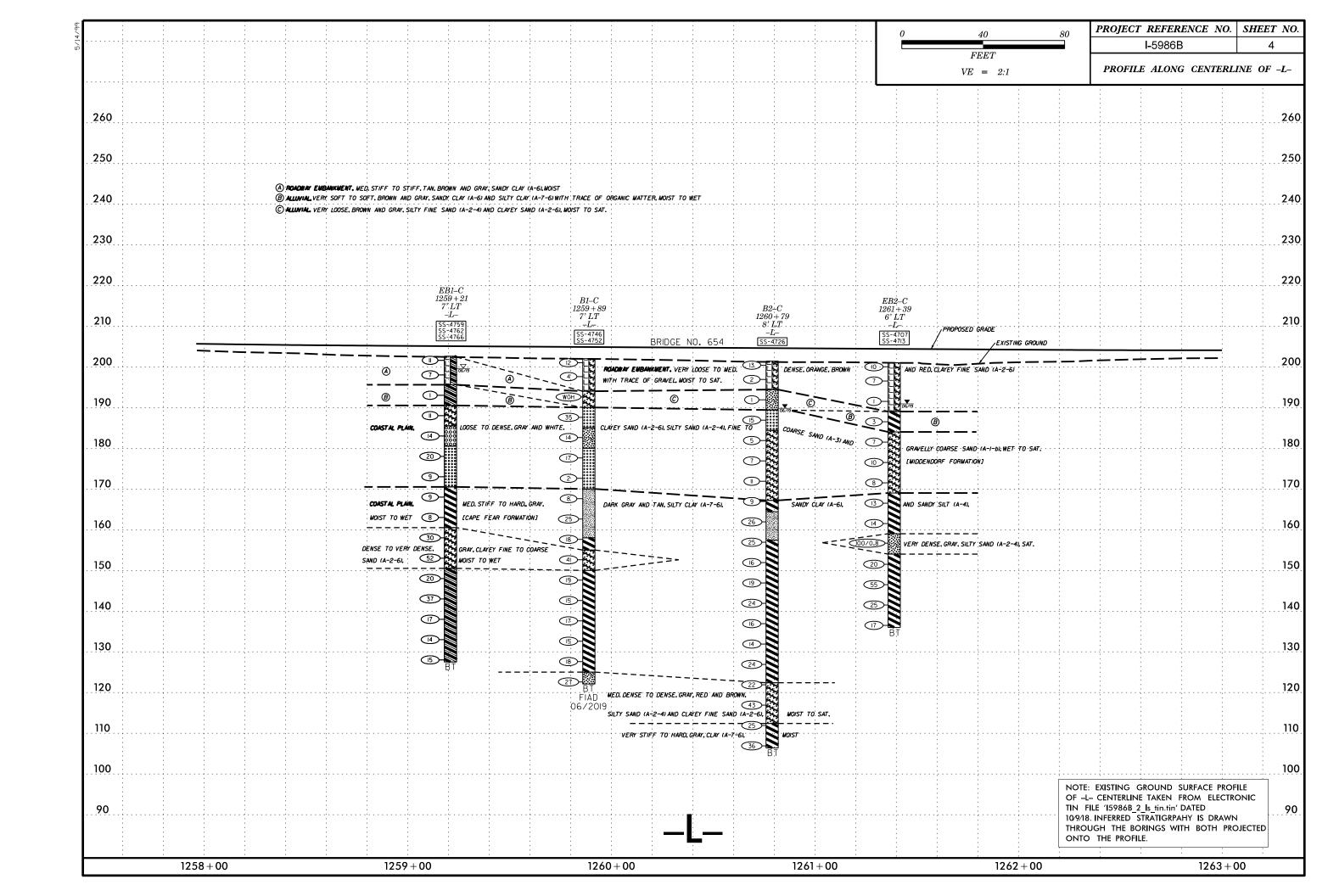
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

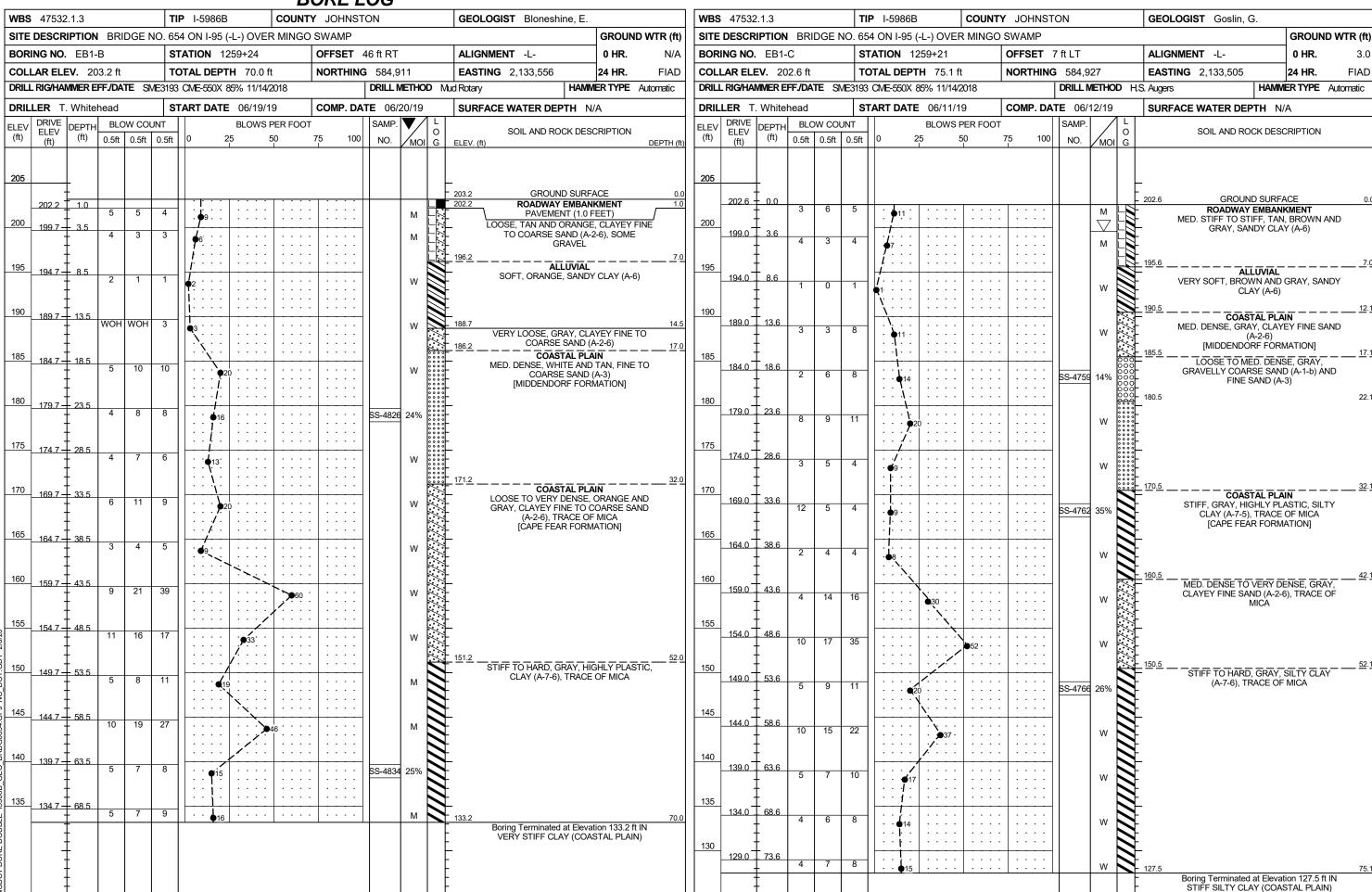
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

Part	R.  SAND OR THAT CONTAIN SAND OF CLAY MINERALS, OR HAVI HALE, SLATE, ETC.  O RISE ABOVE THE LEVEL AI TO OR ABOVE THE GROUND OF CALCIUM CARBONATE. VITY ON SLOPE OR AT BOTTO O IN THE CORE BARREL DIVI STRUCTURE OF ADJACENT OF INCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM OS DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO INCLINED FROM THE OTHER THAN STREAM OF ROCK.
Column   C	SAND OR THAT CONTAIN SAND OF CLAY MINERALS, OR HAVI HALE, SLATE, ETC.  O RISE ABOVE THE LEVEL AT TO OR ABOVE THE GROUND OF CALCIUM CARBONATE. VITY ON SLOPE OR AT BOTTO ON THE CORE BARREL DIVI STRUCTURE OF ADJACENT OF INCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM OS DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO COLORS, MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
April   Company   Compan	OF CLAY MINERALS, OR HAVII HALE, SLATE, ETC. OR RISE ABOVE THE LEVEL AT TO OR ABOVE THE GROUND OF CALCIUM CARBONATE. VITY ON SLOPE OR AT BOTTO OF THE CORE BARREL DIVII STRUCTURE OF ADJACENT SINCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM SIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS, MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
Address   Control   Cont	OF CLAY MINERALS, OR HAVII HALE, SLATE, ETC. OR RISE ABOVE THE LEVEL AT TO OR ABOVE THE GROUND OF CALCIUM CARBONATE. VITY ON SLOPE OR AT BOTTO OF THE CORE BARREL DIVII STRUCTURE OF ADJACENT SINCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM SIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS, MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
## ## ## ## ## ## ## ## ## ## ## ## ##	HALE, SLATE, ETC.  O RISE ABOVE THE LEVEL AT TO OR ABOVE THE GROUND  F CALCIUM CARBONATE.  VITY ON SLOPE OR AT BOTTO  D IN THE CORE BARREL DIVI  STRUCTURE OF ADJACENT  SINCLINED FROM THE  HORIZONTAL TRACE OF THE  BEEN DISPLACEMENT OF THE  LLEL PLANES.  ION AND DISLODGED FROM  SEDEPOSITED BY THE STREAM  MIZED AND TRACED IN THE  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  EDIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
MICHAEL OF STREET AND A SECTION OF A SECTI	O RISE ABOVE THE LEVEL AT TO OR ABOVE THE GROUND TO OR ABOVE THE GROUND OF CALCIUM CARBONATE.  VITY ON SLOPE OR AT BOTTO.  D IN THE CORE BARREL DIVID STRUCTURE OF ADJACENT OF THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LEEL PLANES.  ION AND DISLODGED FROM AN ADJACENT OF THE STREAM RIZED AND TRACED IN THE PRESENCE OF ROCK.
Mile	TO OR ABOVE THE GROUND OF CALCIUM CARBONATE. VITY ON SLOPE OR AT BOTTO D IN THE CORE BARREL DIVI STRUCTURE OF ADJACENT S INCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
Section   Control   Cont	OF CALCIUM CARBONATE.  VITY ON SLOPE OR AT BOTTO  D IN THE CORE BARREL DIVII  STRUCTURE OF ADJACENT  S INCLINED FROM THE  HORIZONTAL TRACE OF THE  BEEN DISPLACEMENT OF THE  LLEL PLANES.  ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM  NIZED AND TRACED.  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  EDIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
Part   1   1   1   1   1   1   1   1   1	VITY ON SLOPE OR AT BOTTIC  D IN THE CORE BARREL DIVII  STRUCTURE OF ADJACENT  S INCLINED FROM THE  HORIZONTAL TRACE OF THE  BEEN DISPLACEMENT OF THE  LLEL PLANES.  ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM  NIZED AND TRACED IN THE  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  COLORS, MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
Control   Cont	D IN THE CORE BARREL DIVID STRUCTURE OF ADJACENT IS INCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES.  ION AND DISLODGED FROM IN SO DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO DIRECTIONS.  COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
CONFERENCE   1.5	STRUCTURE OF ADJACENT S INCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
Section   Control   Cont	STRUCTURE OF ADJACENT S INCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
PRINCE TABLE STATE   1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	STRUCTURE OF ADJACENT  S INCLINED FROM THE  HORIZONTAL TRACE OF THE  BEEN DISPLACEMENT OF THE  LLEL PLANES.  ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM  NIZED AND TRACED IN THE  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  COLORS, MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
Part   10   10   10   10   10   10   10   1	HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
MODING ADDRESS   1	HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
PARTING   THE	BEEN DISPLACEMENT OF THE  LEL PLANES.  ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM  NIZED AND TRACED IN THE  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  DIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
MORE PART OF THE START PARKED TO STATE START PART OF THE START PARKED TO START START PART OF THE START PARKED TO START S	BEEN DISPLACEMENT OF THE  LEL PLANES.  ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM  NIZED AND TRACED IN THE  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  DIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
CROUND WATER   CROWN WITH CAPPED   CROWN WATER   CROWN W	LLEL PLANES.  ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM  NIZED AND TRACED IN THE  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  E DIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
SUBJECT   THE COMPANDED   THE STATE OF STATE   TH	ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE  IT HAS OCCURRED. KNESS IS SMALL COMPARED TO  DIRECTIONS. COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
STATIC WATER LEVEL AFTER 24 HOURS  SOUTHWAY SHOWN ISSUE SECTION PROTIONS OF SIGNAL SECTION IN SOUTH SECURITIES OF THE SOUTH SECTION IN SOUTH S	ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE  IT HAS OCCURRED. KNESS IS SMALL COMPARED TO  DIRECTIONS. COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
FRIENDE DELLIAT 10 0000 FAIL 10 POR MOUNTAINED TO STAND TO POR MOUNTAINED TO STAND THE PROPERTY OF THE STANDARD TO STANDARD TO STANDARD THE STANDARD TO STANDARD THE STANDARD	S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
DOLL SAME UNDER HORSE PRINCE OF SERVICE STATE OF	NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
FIG. R-7-5 SECROLE IS. LL -39, FIG. 6-7-6 SECROLE IS. SECRILITY. SECROLE IS. SECROLE IS. SECROLE IS. SECROLE IS. SECROLE I	NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
CONSISTENCY OF DENSEMBERS  MISCELLANEOUS SYMBOLS  MISCELANEOUS SYMBOLS  MISCELLANEOUS SYMBO	IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  DIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
PRIMARY SOIL TYPE COMPACTNESS OF CONTROL SECURITY PER COMPACTNESS OF CONTROL PER PREVAILED MANY PER PREVAILE	KNESS IS SMALL COMPARED TO E DIRECTIONS. COLORS, MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
PREVIOUS PREVIOUS PREVIOUS STREAM CORRESPOND TO STR	DIRECTIONS. COLORS, MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
GENERALLY LOSSE 4 TO 18 SULL SYMBOL SILE STEEL AND SECURED IN STRENGTH TO STRONG SOLL IN STRENGTH TO S	COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
CREMALLY CRANLAR MICHAELY CRANLAR MICHAELY CRANLAR MICHAELY CRANLAR MICHAELY CRANLAR MICHAELY CRANLAR MICHAELY CRANLAR MICHAEL MICHAELY CRANLAR MICHAEL MICHAELY CRANLAR MICHAEL MICHAELY CRANLAR MICHAEL MICH	COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
MATERIAL DELINE UNION-COHESIVE)  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENT DISTRIBUTION OF COMES STRONG BOX OF THE MEASURE OF PICK POILT.  WENT DETSE  UNION-COHES CREMENTS DISCERNIBLE  MITTOR CREMENTS DISCRIBED AND CARRIE CREMENT DISTRIBUTION OF CONTROL OF COMES OF THE MEASURE OF PICK POILT ON CARRIE CREMENT DISTRIBUTION OF CONTROL OF COMES OF COMES DEED CANADIS IN CREASE DISTRIBUTION OF CONTROL OF COMES DEED CAN	ATER LEVEL BY THE PRESEN
VERY SOFT CENSE > 50 UNDER CORE BORING SUNDING ROD  VERY SOFT 2 TO 4 0.25 TO 8.5  SILT-CLAY MEDIUM STIFF 8 TO 15 TO 1.0  MATERIAL STIFF 8 TO 50 I. BOUNDARY PART MAINTANDE WITH CORE  WITH	OF ROCK.
GENERALLY SOFT SILT-CLAY MEDIUM STIFF SILT-CLAY MEDIUM SILT-CLAY MEDIUM SILT-CLAY SCHEMED TO SORGHAM ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT. N VALUES C 108 BPF SCATTERED COCKDITAL NOT DISCERNIBLE, ORI DISCE	
SILT-CLAY MEDIUM STIFF 8 TO 15 10 30 10 10 11 TO 2 11 TO 2 10 2 TO 14 THE FERRED ROCK LINE 1 TO 2 2 TO 4 THE FERRED ROCK LINE 1 TO 2 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 30 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 30 STIFF 8 TO 30	
MATERIAL (CHESIVE) STIFF 15 TO 30 2 TO 4 30 AND EXPRESSED AS DIKES OR STRINGERS. SAPROLITE IS HARD SPECIMENS COULD TO RECEPTABLE WINDERS OF THAT RESULTS THAT AND ALLOW UNDERSOR SECRET THAN A NOR SECRET THAN A N	CCRIRED BY TOTAL LENGTH
TEXTURE OR GRAIN SIZE  U.S. STD. SIEVE SIZE 4 10 40 60 200 270 00 0.25 0.055 0.005  BOULDER (BURDA)  GRAIN SIZE  UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK  BOULDER (BURDA)  GRAIN MM 305 75 2.0 0.25 0.05 0.05 0.05 0.05 0.05 0.05	
TEXTURE OR GRAIN SIZE  U.S. STD. SIEVE SIZE  U.S. STD. SIEVE SIZE  OPENING (MM)  OPENI	THE TOTAL ELEKOTT OF CORE
U.S. STD. SIEVE SIZE  4 10 40 60 200 270  OPENING (MM)  4.76 2.00 0.42 0.25 0.055  BOULDER (BLDR.)  CRAIN MM 305 75 2.0 0.25 0.05  SILT SAND  CRAIN MM 305 75 2.0 0.25  SILT AM INTRUSIVE BODY OF IGREDUS ROCK OF APPROXIMATELY  SHALLOW  UNCLASSIFIED EXCAVATION - MCCEPTABLE EXCAV	URE OR FABRIC OF THE PAR
DEFINIC MM)  4.76 2.00 0.42 0.25 0.075 0.053  DOBLOR (COB.) (GR.) (COB.) (GR.) (COB.) (GR.) (COB.) (GR.) (COB.) (GR.) (COB.) (GR.) (COB.) (COB	ITEODM THICKNESS AND
BOULDER (BLDR.) COBBLE (GR.) LOOP (GR.) COUNTY OF THE INTOLED ROCKS.  GRAVEL (COB.) (GR.) COUNTY OF THE INTOLED ROCKS.  GRAVEL (COB.) (GR.) CLOY (CL.)  GRAVEL (COB.) (GR.) CLOY (CD.) CLOY (CD.)  GRAVEL (COB.) (GR.) CLOY (CD.)  GRAVEL (COB.) (GR.) CLOY (CD.)  GRAVEL (COBBLE (	
(BLDR.) (COB.) (GR.) (COB.) (GR.) (COB.) (GR.) (COB.) (GR.) (COB.) (GR.) (COB.)	
GRAIN MM 305 75 2.0 0.25 0.05 0.005 AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST  SIZE IN. 12 3 BY MODERATE BLOWS.  BT - BORING TERMINATED MICA, - MICACEOUS WEA WEATHERED MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A FOLLOW OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPT RESULTS OF THE WITH A 2	JM FRICTION ALONG A FAULT
SIZE IN. 12 3 BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A FOLLOW CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RECOVER. SPT REC	1DED OF DLOVE (N. OD DDE) C
CL CLAY MOD MODERATELY 7 - UNIT WEIGHT HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RE	
SOLE PIOLISTONE CONNECTATION OF TERMINATES   NP - NON PLASTIC /- DRY UNIT WEIGHT   PUINT OF A GEOLOGIST'S PICK.	SAL IS PENETRATION EQUAL
SOUL MOISTURE SCALE FIELD MOISTURE CORP. CORP. SCALE SCALE OF SCAL	AL RECOVERED DIVIDED BY
GUIDE FOR FIELD MOISTURE DESCRIPTION OF A PICK DIVINISHED TO STREAM AND EXPRESSION OF A PICK DIVINISHED TO STREAM OF THE CHARGE TEST OF THE CHARGE	
DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK PIECES CAN BE BROKEN BY FINGER PRESSURE.  - SATURATED - USUALLY LIQUID; VERY WET, USUALLY E - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON SS - SPLIT S	JALITY DESCRIBED BY TOTAL
(SAT.) FROM BELOW THE GROUND WATER TABLE F - FINE SL SILT, SILTY ST - SHELBY TUBE CONT. OF PICK, PIECES I INCH.	ER THAN 4 INCHES DIVIDED
PLASTIC TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MA	R.
PANCE - VET - (VI) SEMISULIDE REQUIRES DEFINO TO	
(*) PL PLASTIC LIMITHI HIGHLY V - VERY RATIO <u>TERM SPACING</u> <u>TERM THICKNESS</u>	
EQUIPMENT USED ON SUBJECT PROJECT VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET EL	VATION: 199.73 FEE
DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: MODERATELY CLOSE 1 TO 3 FEET THINLY REDDED 0.16 - 1.5 FEET	
PEGUIDES ADDITIONAL WATER TO CHE-45C X CLAY BITS X AUTOMATIC MANUAL CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET 10-2-0.16	
- DRY - (D)  ATTAIN OPTIMUM MOISTURE  CME-55  CME-55  CME-55  CME-55  CME-55  CME-55  CME-55	
PLASTICITY X 8' HOLLOW AUGERS	
PLASTICITY INDEX (PI)  PLASTICITY INDEX (PI)  PLASTICITY INDEX (PI)  PLASTICITY INDEX (PI)  PROBLEM OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.  PROBLEM OF MATERIAL BY CEMENTING, HEAT, PRESSURE, HEAT, PR	
NUN PLASTIC 0-5 VERY LOW   TUNG. CARBIDE INSERTS   FRIABLE ROODING WITH THOUGH THE TRANSPORT OF THE PROPERTY O	
SLIGHTLY PLASTIC 6-15 SLIGHT VANE SHEAR TEST V CASING V ADVANCER HAND TOOLS:	
POST HOLE DIGGER MODERATED A M	
COLOR  PORTABLE HOIST   TRICONESTEEL TEETH   HAND AUGER   BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR    PORTABLE HOIST   TRICONE STELL TEETH   HAND AUGER   SOUNDING ROD   INDURATED   SPECIAL TO BREAKS EASILY WHEN HIT WITH HAMMER.    COLOR   COLO	
COLOR  PORTABLE HOIST   TRICONE 'STEEL TEETH   HAND AUGER   BREAKS EASILY WHEN HIT WITH HAMMER.  COLOR   TRICONE 'TUNGCARB.   SQUINDING POR	DATE; 8-1





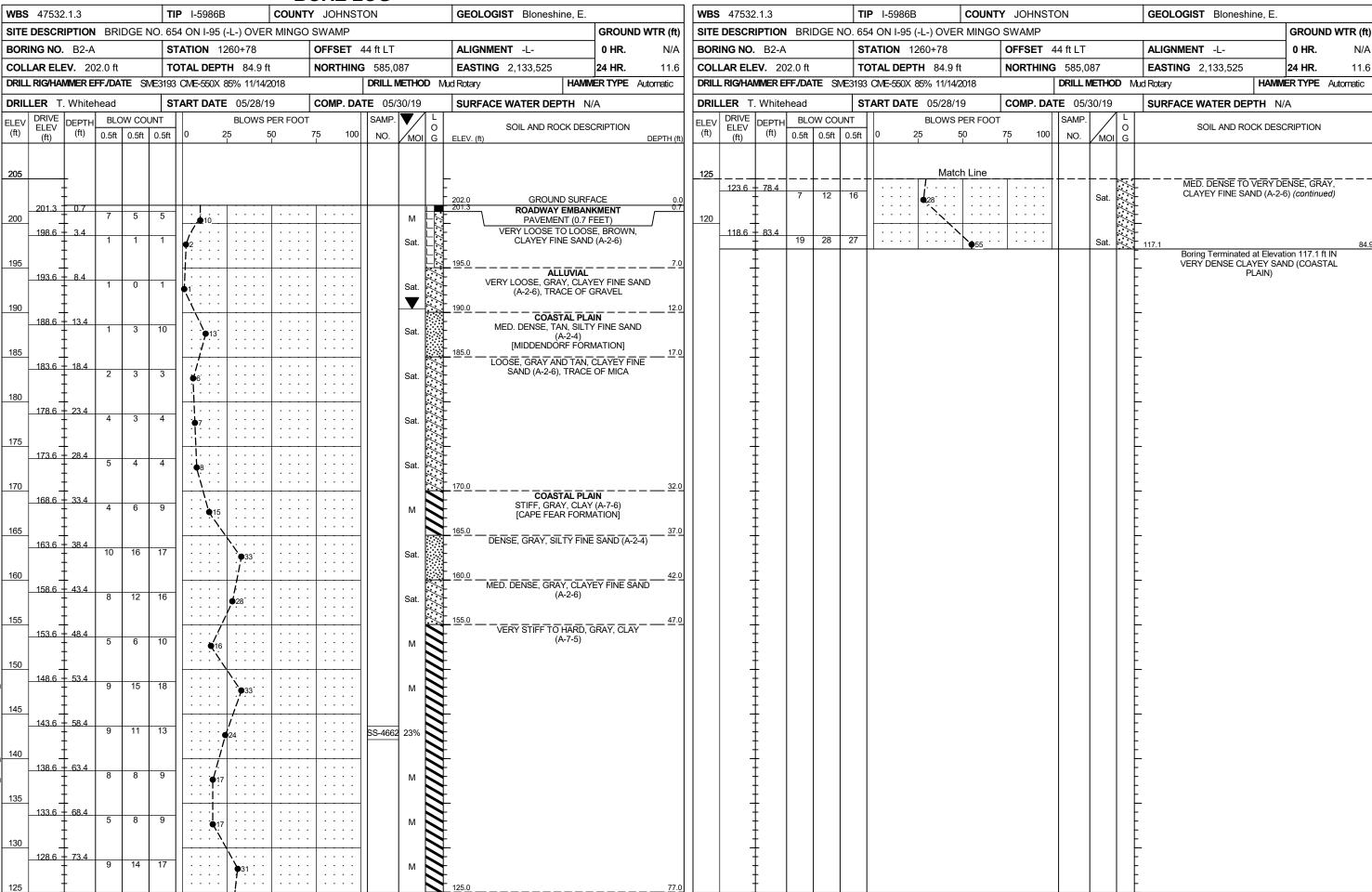
		RE LOG					
<b>WBS</b> 47532.1.3	TIP I-5986B COUNTY	JOHNSTON	GEOLOGIST Bloneshine, E.	<b>WBS</b> 47532.1.3	TIP I-5986B COUNTY	Y JOHNSTON	GEOLOGIST Bloneshine, E.
SITE DESCRIPTION BRIDGE N	D. 654 ON I-95 (-L-) OVER MINGO SV	SWAMP	GROUND WTR (ft)	SITE DESCRIPTION BRIDGE NO	O. 654 ON I-95 (-L-) OVER MINGO	SWAMP	GROUND WTR (ft)
BORING NO. EB1-A	<b>STATION</b> 1259+18 <b>O</b> I	OFFSET 43 ft LT	ALIGNMENT -L- 0 HR. N/A	BORING NO. EB1-A	<b>STATION</b> 1259+18	OFFSET 43 ft LT	ALIGNMENT -L- 0 HR. N/A
COLLAR ELEV. 203.5 ft	TOTAL DEPTH 90.0 ft No	NORTHING 584,937	<b>EASTING</b> 2,133,470 <b>24 HR.</b> FIAD	COLLAR ELEV. 203.5 ft	<b>ELEV.</b> 203.5 ft <b>TOTAL DEPTH</b> 90.0 ft		<b>EASTING</b> 2,133,470 <b>24 HR.</b> FIAD
DRILL RIG/HAMMER EFF/DATE SM	E3193 CME-550X 85% 11/14/2018	DRILL METHOD Mu	d Rotary HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF/DATE SME	E3193 CME-550X 85% 11/14/2018	DRILL METHOD M	fud Rotary HAMMER TYPE Automatic
DRILLER T. Whitehead	<b>START DATE</b> 06/03/19 <b>CG</b>	COMP. DATE 06/04/19	SURFACE WATER DEPTH N/A	DRILLER T. Whitehead	<b>START DATE</b> 06/03/19	COMP. DATE 06/04/19	SURFACE WATER DEPTH N/A
ELEV DRIVE DEPTH BLOW COU	NT BLOWS PER FOOT	SAMP. L	SOIL AND ROCK DESCRIPTION	ELEV DRIVE DEPTH BLOW COUN	NT BLOWS PER FOOT	SAMP.	SOIL AND ROCK DESCRIPTION
(ft) (ft) (ft) 0.5ft 0.5ft	0.5ft 0 25 50 75	5 100 NO. MOI G		(ft) (ft) (ft) 0.5ft 0.5ft 0	0.5ft 0 25 50	75 100 NO. MOI G	SOIL AND NOOK BESCHII TION
205				125	Match Line		
202.8 = 0.7			203.5 GROUND SURFACE 0.0 202.8 POADWAY EMBANKMENT 0.7		14 29		MED. DENSE, GRAY, SILTY FINE SAND (A-2-4) (continued)
202.3 = 0.7	•12		PAVEMENT (0.7 FEET)		$  \cdot \cdot \cdot \cdot \cdot   \cdot \cdot \cdot \cdot \cdot   \cdot \cdot \cdot \cdot \cdot $		- 121.5
200 200.0 7 3.5 3 5	6		200.5 MED. DENSE, TAN AND RED, SILTY FINE ,— 3.0 SAND (A-2-4)	120 120.0 83.5 12 16	18		MED. DENSE TO DENSE, GRAY, CLAYEY FINE TO COARSE SAND (A-2-6)
	•   •   • • • •   • • • •   • • • •	M   M   M   M   M   M   M   M	MED. DENSE, BROWN, CLAYEY COARSE	‡   '   '	10   · · · ·   · • 34 ·   · · · · · · · · · · · · · · · · ·	1 1 1 10 10 10	-
105			196.5 TO FINE SAND (A-2-6), TRACE OF GRAVEL $7.0$	145	::::  <i>j</i> ::: ::::		-
195 195.0 7 8.5 1 1	2 3	M	ALLUVIAL VERY LOOSE, BROWN, CLAYEY SAND	115 115.0 7 88.5 13 12	16 •28	   W	 - 113.5 90.0
			(A-2-6) 12.0		19-		Boring Terminated at Elevation 113.5 ft IN MED. DENSE CLAYEY SAND (COASTAL
190   190.0   13.5			COASTAL PLAIN VERY LOOSE TO MED. DENSE, LIGHT	±			PLAIN)
3 9	11 5.8	M	GRAY, GRAY AND WHITE, SILTY FINE				-
		· · · · ·	SAND (Á-2-4) AND FINE SAND (A-3) WITH GRAVEL 17.0				_
185 185.0 18.5 3 1	2		[MIDDENDORF FORMATION]				_
		W   0000-					-
180 180.0 23.5 5 7	8	W		±			_
	1   <b>Y</b> .*						
175 175.0 28.5							
175 175.0 28.5 4 6	6 . •12	W					
			171.5 32.0				
170   170.0   33.5	'		COASTAL PLAIN				-
3 1	177 1 1	W	VERY LOOSE, GRAY, CLAYEY FINE SAND (A-2-6)				-
			[CAPE FEÀR FORMATION]				
165 165.0 38.5		····   <b>S</b>	STIFF, GRAY, SILTY CLAY (A-7-6)				-
	5   . •9	SS-4693 22%					
			161.5 LOOSE, LIGHT GRAY, CLAYEY FINE SAND 42.0	±			
160 160.0 7 43.5 3 4	5		(A-2-6)	+			_
155 155.0 48.5			156.5 MED. DENSE, DARK GRAY, FINE SAND 47.0				
155 155.0 48.5 15 18 18 18 18 18 18 18 18 18 18 18 18 18	11	W	(A-3)				-
			151.5 52.0				_
150 150.0 53.5	<u> </u>		STIFF TO HARD, GRAY, SILTY CLAY (A-7-6)				
	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	SS-4696 23%	(17-17-0)				
145 145.0 T 58.5 10 19	21	🔂-					_
45   1   1   1   1   1   1   1   1   1	• • • •   • • • 40   • • • •						
							-
140 140.0 T 63.5 6 10	11	M N		±			_
(S) 135 135.0 68.5							
<u> </u>	9 • 16			‡			-
							-
130 130.0 73.5							
	7   • 13.	· · · · ·         M					
		····     <u>\$</u>	126.5 77.0				-
일 125   1							



	<i></i>	BORE LOG		,					
<b>WBS</b> 47532.1.3			GEOLOGIST Bloneshine, E.	<b>WBS</b> 47532.1.3	TIP I-5986B COUN	TY JOHNSTON	GEOLOGIST Bloneshine, E.		
SITE DESCRIPTION BRIDGE NO	D. 654 ON I-95 (-L-) OVER MING	O SWAMP	GROUND WTR (ft)	SITE DESCRIPTION BRIDGE N	NO. 654 ON I-95 (-L-) OVER MING	O SWAMP	GROUND WTR (ft)		
BORING NO. B1-A	<b>STATION</b> 1259+90	OFFSET 41 ft LT	ALIGNMENT -L- 0 HR. N/A	BORING NO. B1-A	<b>STATION</b> 1259+90	OFFSET 41 ft LT	ALIGNMENT -L- 0 HR. N/A		
COLLAR ELEV. 202.8 ft	TOTAL DEPTH 79.9 ft	<b>NORTHING</b> 585,003	<b>EASTING</b> 2,133,497 <b>24 HR</b> . 10.2	COLLAR ELEV. 202.8 ft	TOTAL DEPTH 79.9 ft	<b>NORTHING</b> 585,003	<b>EASTING</b> 2,133,497 <b>24 HR.</b> 10.2		
DRILL RIG/HAMMER EFF./DATE SME	E3193 CME-550X 85% 11/14/2018	DRILL METHOD M	ud Rotary HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE SN	VIE3193 CME-550X 85% 11/14/2018	DRILL METHOD	Vlud Rotary HAMMER TYPE Automatic		
DRILLER T. Whitehead	<b>START DATE</b> 06/02/19	COMP. DATE 06/03/19	SURFACE WATER DEPTH N/A	DRILLER T. Whitehead	<b>START DATE</b> 06/02/19	COMP. DATE 06/03/19	SURFACE WATER DEPTH N/A		
ELEV DRIVE DEPTH BLOW COUN		SAMP. L	SOIL AND ROCK DESCRIPTION	ELEV DRIVE DEPTH BLOW COL	JNT BLOWS PER FOO	OT SAMP.	SOIL AND ROCK DESCRIPTION		
(ft) (ft) (ft) 0.5ft 0.5ft	0.5ft 0 25 50	75 100 NO. MOI G	ELEV. (ft) DEPTH (ft	(ft)   LLL     (ft)   0 = 0   0 = 0	0.5ft 0 25 50	75 100 NO. MOI G			
205				125	Match Line				
			202.8 GROUND SURFACE 0.0	124.4 78.4 5 10	12	· · · · · ·	MEDIUM DENSE, GRAY, CLAYEY FINE TO 122.9 COARSE SAND (A-2-6) (continued) 79.9		
202.0 0.8 8 5	6 . 11		PAVEMENT (0.8 FEET)	4   1			Boring Terminated at Elevation 122.9 ft IN		
200 199.4 3.4			= 199.8 STIFF, ORANGE AND BROWN, SANDY ,— 3.0	4			- MED. DENSE CLAYEY SAND (COASTAL PLAIN)		
+ 4 4	3		CLAY (A-6)  LOOSE, ORANGE AND BROWN, CLAYEY				*NO RECOVERY FROM DEPTH 28.4' TO		
195			FINE SAND (A-2-6), TRACE OF GRAVEL				- 29.9'* -		
194.4 + 8.4   2   9	5		193.8 9.0				-		
			COASTAL PLAIN LOOSE TO MED. DENSE, GRAY, LIGHT	]			ļ.		
190 189.4 13.4			190.8 GRAY AND WHITE, SILTY FINE SAND 12.0 - (A-2-4) AND FINE TO COARSE SAND (A-3),	<u>                                     </u>			<u> </u>		
5 12	15	M	TRACE OF GRAVEL [MIDDENDORF FORMATION]				_		
			185.8	,			_		
185 184.4 18.4 4 6	5		-				-		
	¶ . ¶11 .						t		
180		0000	180.8 22.0				Ł		
179.4 23.4 4 6	8						-		
							F		
175 174.4 28.4			-				F		
2 4	4	.					-		
170		0000 0000 0000					-		
169.4 + 33.4   2   2	3	·   · · · · ·     W	COASTAL PLAIN  LOOSE, GRAY AND TAN, CLAYEY FINE				F		
			SAND (A-2-6) [CAPE FEAR FORMATION]				-		
165 164.4 38.4			DENSE, LIGHT GRAY AND GRAY, SILTY	4			_		
13 20	26	:   : : : :     w	FINE SAND (A-2-4)				_		
							_		
159.4 43.4 8 20	21		-				-		
	41						t		
155			_	$  \cdot   \cdot   \pm   \cdot  $			Ł		
00 154.4	11 •22	:   : : : :   M	153.8 49.0 STIFF TO VERY STIFF, GRAY, SILTY CLAY	9			Ŀ		
		: ::::	(A-7-5)				Ł		
150 149.4 53.4 9	11	- <del></del>    <b>S</b>	-				F		
	··     • • 20	SS-4679 24%					Ł		
2 145 145 1 Fo 1			_				Ł		
0 144.4 58.4 4 9	12 21	-     M					-		
							F		
140 139.4 63.4		<u> </u>	-				F		
0	ŏ						F		
98   135							F		
<u>ії 134.4 68.4 </u> щ + 5 6	7		-				F		
	· · • • · · · · · · · · · · · · · · ·						‡		
130 129.4 73.4	j	<u> </u>	-				<u> </u>		
0 1293 193 4 7	5						‡.		
		: ::::	125.8 77.0	]			‡		
Z   Z	11 3 1 1				i i	1 1 1	1		

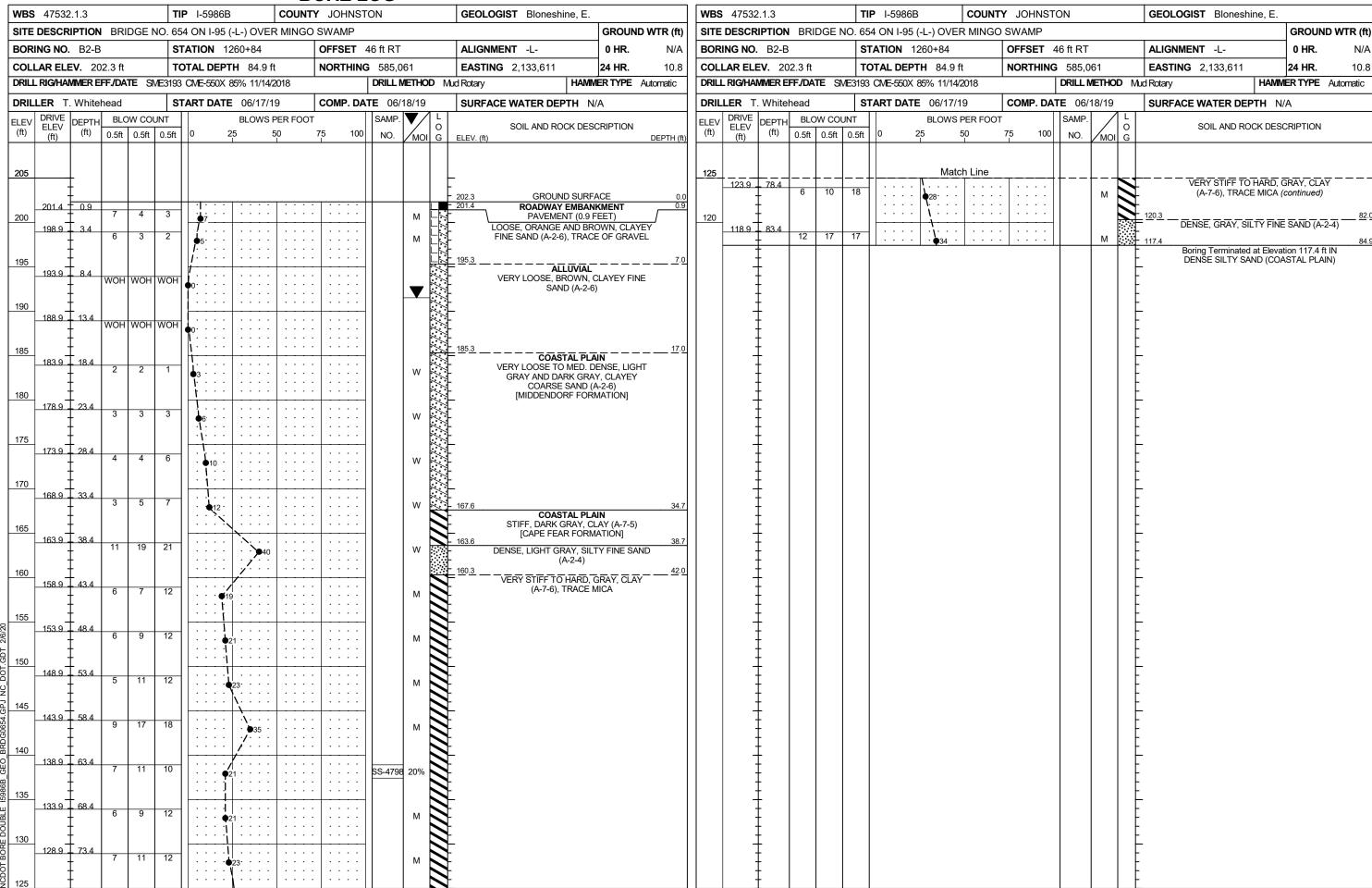
WBS 47532.1.3 TIP I-5986B COUNTY JOHNSTON GEOLOGIST Bloneshine, E.							WR	<b>S</b> 47532	) 1 3			TIP I-	5986B CC	DUNTY JOH	NSTON			GEOLOGIST Bloneshi	ne F											
			I BRIDG	SF NO		I ON I-95 (				31014			OLOLO	Dionesimin	·	UND WTR (ft	→				D. 654 ON I-95 (-L-) OVER MINGO SWAMP					GLOLOGIOT BIORCONI		ROUND WTR (ft)		
-		. B1-B				ATION 12	` ,		OFFSET	52 ft	RT		ALIGNM	MENT -L-	0 HF	•	`			ON 1259+83	OFFSE		ft RT		ALIGNMENT -L-		HR. N/A			
		<b>EV.</b> 20			+	TAL DEPT		<u> </u>	NORTHII				_	I <b>G</b> 2,133,582	24 HF		l ——	LLAR ELE				TOTAL DEPTH 84.9 ft			<b>NORTHING</b> 584,965		<b>EASTING</b> 2,133,582	24		
				SME		CME-550X 8			1.1.0.1.1.1.1.			OD	Mud Rotary			PE Automatic				ER EFF./DATE SME3193 CME-550X 85% 11/14/2018							Mud Rotary		YPE Automatic	
DRIL	ILLER T. Whitehead START DATE 06/18/19		9	COMP. DATE 06/19/19			9	SURFACE WATER DEPTH N/A			DRI	DRILLER T. Whitehead				START	<b>DATE</b> 06/18/19	СОМР	DATE	06/19/	19	SURFACE WATER DEF	TH N/A							
	DRIVE	DEPTH		COUN			BLOWS F		1		MP.	<u> </u>	T'	SOIL AND ROCK		ON.	ELE	DDI\/E			W COUNT		BLOWS PER			AMP.	/ L	T		TION
(ft)	ELEV (ft)	(ft)	0.5ft 0.	.5ft (	0.5ft	0 2	25 5	50	75 10	00 N	O. MC	O OI G	ELEV. (ft)	SOIL AND ROCK	V DESCRIPTIO	DEPTH (	ft) (ft)	(ft)	(ft)	0.5ft	0.5ft 0.5	5ft 0	25 50	75 	100	NO.	/ O MOI G		UK DESCRIP	TION
205		1															125	3 4944		l			Match Li	ne	_	L		<b>-</b>		
		‡								Ш			202.8	GROUND			.0	124.4	78.4	5	6 1	0 .	16				м	VERY STIFF, GRAY OF MICA	(continued)	b), TRACE
200	201.9	0.9	6	5	5	. 1			<b>I</b>		М		201.9	ROADWAY EI PAVEMENT	(0.9 FEET)		120	,   -	-									120.8	WE-25-W-	82.0
200	199.4	<del>+</del> 3.4 +	4	6	4	· 10 ·				-	l <sub>w</sub>			OOSE, TAN AND RE COARSE SAND (A-2			120	119.4	83.4	9	18 1	9 .	37		 	S-4820 18	3%	"I FINIT C	AND GRAY, ( AND (A-2-6)	CLAYEY 84.9
		‡				: <b>/</b> :": :				-		J	<del>}</del>					-										Boring Terminated DENSE CLAYEY SA	at Elevation 1	17.9 ft IN
195	194.4	8.4				<i>j</i> · · · ·							194.8	ALLU	— — -	8	. <u>o</u>	_	ļ									DENSE CLATET 3/	AND (COAST)	AL FLAIN)
		Ī	2	1	2	<b>ф</b> 3				1 1		7	<u>.</u>	VERY LOOSE, DARI SAND (A-2-4), TR	K GRAY, SILT				Ē									E		
190	100.4=	Ξ.,.								1 1			E	0/14D (/-2-4), 110	TIOL OF ORVE	VLL		_	E									E		
	189.4	13.4	2	2	0	1 ¶2				1 1	w							-										_		
405		‡				\\							185.8			17	0	-	_									-		
185	184.4	18.4	2	6	8	\				-	l <sub>w</sub>	000	-	MED. DENSE, LIGH	IL PLAIN IT GRAY AND	TAN,		_	-									-		
		‡				7.14					"	000		FINE TO COARSE GRA	VEL				‡									-		
180	179.4	+ + 23.4				<u>                                   </u>						000		[MIDDENDORF	FORMATION	1]			_									_		
		‡	4	6	5	. • 11 .				:	w	000						-	<u> </u>									-		
175		‡				:   : :			:   : : : :			000	175.8	OOSE, TAN AND OR		<u>27</u>	<u>o</u>	-	<u> </u>									-		
170	174.4	+ 28.4 +	2	4	5	. j				-  -	l w	://:/	\$- \$-		SAND (A-2-6)			_	<u> </u>									-		
		Ŧ								.		·//-//	170.8			32		-	F									F		
170	169.4	33.4				-				$\dashv$ $lacksquare$				COASTA MED. STIFF TO STI	L PLAIN		<u> </u>	_	ļ									-		
		Ī	3	3	5	. ♠8			.	.   55-4	4810 21%	$^{\circ}$	<del>\</del>	PLASTIC, C	LAY (A-7-6)				Ē									E		
165	404.4=	Ī 4				. 1				1 1			<u> </u>	[0/4 2 1 2/40				_										_		
	104.4	38.4	3	6	9	15				:	М		}					-	_									_		
160		‡								1 1			160.8			42	. <u>o</u>	-										-		
160	159.4	43.4	11 1	16	23					-	l <sub>w</sub>	/ <u>/</u> ///,	<del>}</del>	DENSE, GRAY, CL (A-2	LAYEY FINE S. 2-6)	SAND		-	-									-		
		‡					/				"	/ <u>/</u> //,	<del>}</del> <del>}</del>						_									-		
155 Si	154.4	48.4					- /			-		·/•//•/	<del>,</del>					-	‡									Ė		
2/6/;		Ī	9 1	15	18		33.			:	W	·//////	<del>,</del>					-	E									E		
150		Ī,,					/						150.8	ERY STIFF, GRAY,	CLAY (A-7-6)	TRACE 52	0	-	E									_		
100	149.4	53.4	5	9	10		9			:	М		}	OF N	MICA (),			-	_									-		
N N		‡				/							}						_									-		
145 145	144.4	58.4	8 1	12	14	\				-	<sub>M</sub>		}					_	_									-		
041 BRDG0654		‡				· · · · /	P <sup>26</sup>			:	l IVI		}					-	_									-		
	139 4	63.4				· · · · /							<u></u>						‡									_		
GEO		‡	5	7	10	17			:   : : : :	:	М		\$					-	‡									-		
135 135		‡				• • •   •				:			*					-	ļ.									-		
LE 15	134.4	68.4	5	7	10	17				.	М		<b>\</b>					-	F									F		
DOOUB		Ī											<b>\</b>					-	E									E		
130	129.4	73.4	5	Ω						$\exists    $			<b>}</b>					-	<u> </u>									<u>-</u>		
OT B		‡	5	8	9	17				:	M		}						_									-		
9 125		<u>†</u>				.				·			}					-	L									-		

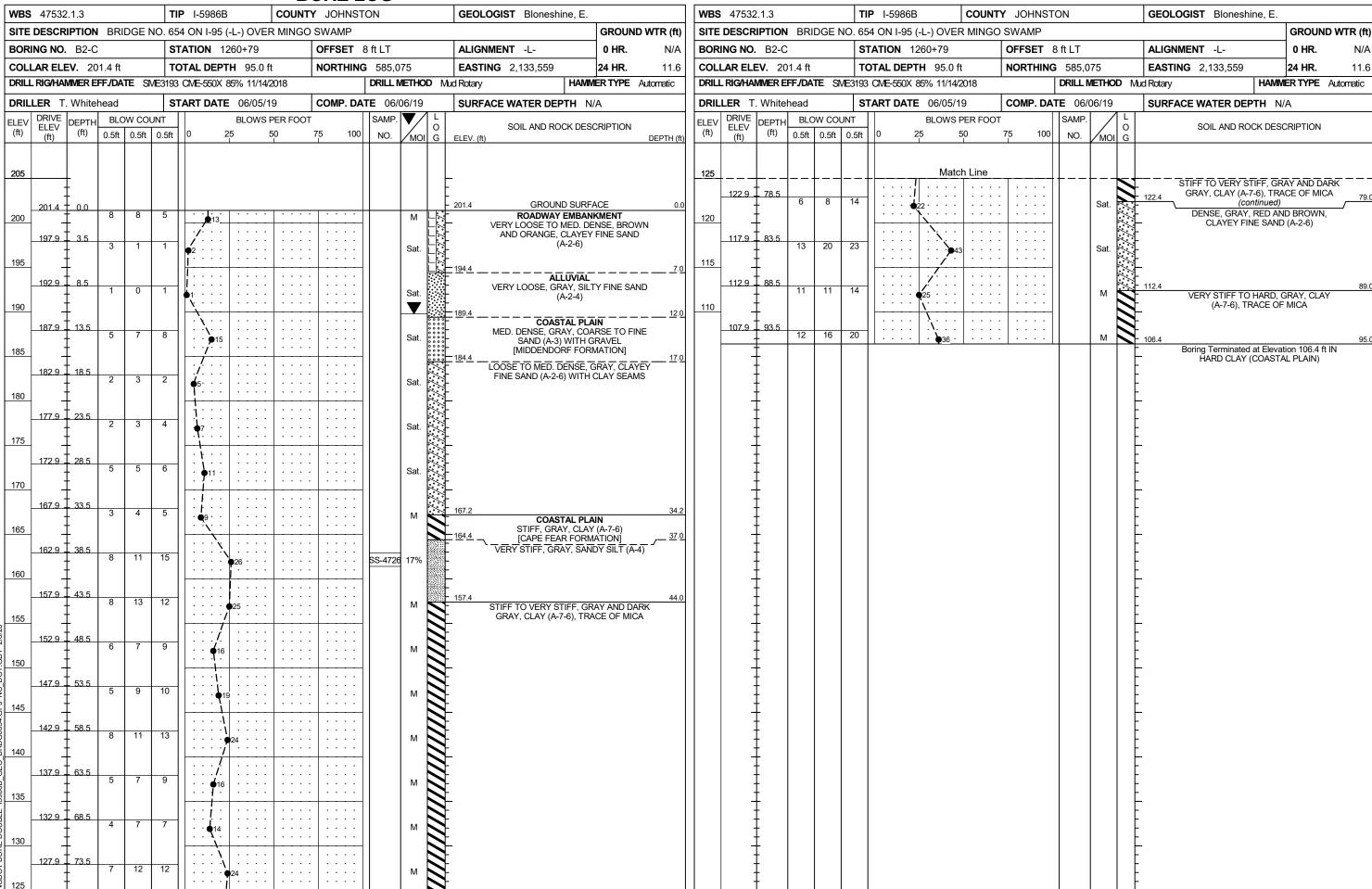
<b>WBS</b> 47532.1.3		ITY JOHNSTON	GEOLOGIST Bloneshine, E.	<b>WBS</b> 47532.1.3	TIP I-5986B COUNT	TY JOHNSTON	GEOLOGIST Bloneshine, E.
	IO. 654 ON I-95 (-L-) OVER MING		GROUND WTR (ft)	SITE DESCRIPTION BRIDGE NO			GROUND WTR (ft)
BORING NO. B1-C	<b>STATION</b> 1259+89	OFFSET 7 ft LT	ALIGNMENT -L- 0 HR. N/A	<b>BORING NO.</b> B1-C <b>STATION</b> 1259+89		OFFSET 7 ft LT	ALIGNMENT -L- 0 HR. N/A
COLLAR ELEV. 202.0 ft	TOTAL DEPTH 79.9 ft	<b>NORTHING</b> 584,991	<b>EASTING</b> 2,133,529 <b>24 HR</b> . FIAD	COLLAR ELEV. 202.0 ft	TOTAL DEPTH 79.9 ft	<b>NORTHING</b> 584,991	<b>EASTING</b> 2,133,529 <b>24 HR</b> . FIAD
DRILL RIG/HAMMER EFF/DATE SN		DRILL METHOD M.		DRILL RIG/HAMMER EFF./DATE SME		DRILL METHOD N	
DRILLER T. Whitehead	<b>START DATE</b> 06/06/19	COMP. DATE 06/07/19	SURFACE WATER DEPTH N/A	DRILLER T. Whitehead	<b>START DATE</b> 06/06/19	<b>COMP. DATE</b> 06/07/19	SURFACE WATER DEPTH N/A
ELEV DRIVE ELEV (ft) DEPTH BLOW COU	I	OT SAMP.   L O NO.   MOI G	SOIL AND ROCK DESCRIPTION  ELEV. (ft)  DEPTH (ft)	ELEV (ft) DEPTH BLOW COUNT (ft) 0.5ft 0.5ft 0	<del></del>	T SAMP. L O NO. MOI G	SOIL AND ROCK DESCRIPTION
205			_	125	Match Line		MED. DENSE, GRAY, SILTY FINE SAND
202.0 0.0 4 7	5	M L	202.0 GROUND SURFACE 0.0  ROADWAY EMBANKMENT	9 13	14	·   · · · · ·     M	(A-2-4) 79.9  Boring Terminated at Elevation 122.1 ft IN
198.6 + 3.4	2	Sat.	LOOSE TO MED. DENSE, ORANGE AND BROWN, CLAYEY FINE SAND (A-2-6), TRACE OF GRAVEL				MEĎ. DENSE SILTY SAND (COASTAL PLAIN)
195	<u> </u>						- -
190 + WOH WOH	WOH	Sat.	ALLUVIAL VERY LOOSE, BROWN, CLAYEY SAND (A-2-6) 12.0				- - -
188.6 + 13.4 4 16	19	Sat.	COASTAL PLAIN  VERY LOOSE TO DENSE, GRAY AND  WHITE, COARSE SAND (A-3) WITH				- - -
183.6 + 18.4	8	Sat.	GRAVEL, SILTY FINE SAND (A-2-4) AND  185.0 SAND (A-3) WITH CLAY SEAMS  17.0  (22.0'-32.0')  [MIDDENDORF FORMATION]				<u>-</u> -
180	14						-
175	9   17   17   10   10   10   10   10   10	Sat.					
173.6 + 28.4	1 2	Sat.					
170 168.6 + 33.4 3 4	4		170.0 32.0  COASTAL PLAIN  STIFF TO VERY STIFF, TAN, SANDY SILT				<u>-</u> -
165	8		_ (A-4) _ [CAPE FEAR FORMATION] 				- - -
163.6 + 38.4   6   10	15	SS-4746 20%					- - -
158.6 + 43.4 4 9	9 • 18		158.0 VERY STIFF, GRAY, CLAY (A-7-6), TRACE				
155	21	M	OF MICA				-
150		M M					- - -
148.6 + 53.4   5   8   5   8   5   8   5   8   5   8   5   6   6   6   6   6   6   6   6   6	11		CLAY (A-7-5), TRACE OF MICA				
143.6 + 58.4 4 7	8 15		-				
140 138.6 + 63.4 138.6 + 63.4	10		_				
898 135 135	17		- -				-
133.6 + 68.4 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	8   15   15   1   1   1   1   1   1   1	SS-4752					
130 0 128.6 - 73.4 10 - 6 9	9	м М					
125 +	\	· ·   · · · ·	125.0 77.0				<u> </u>

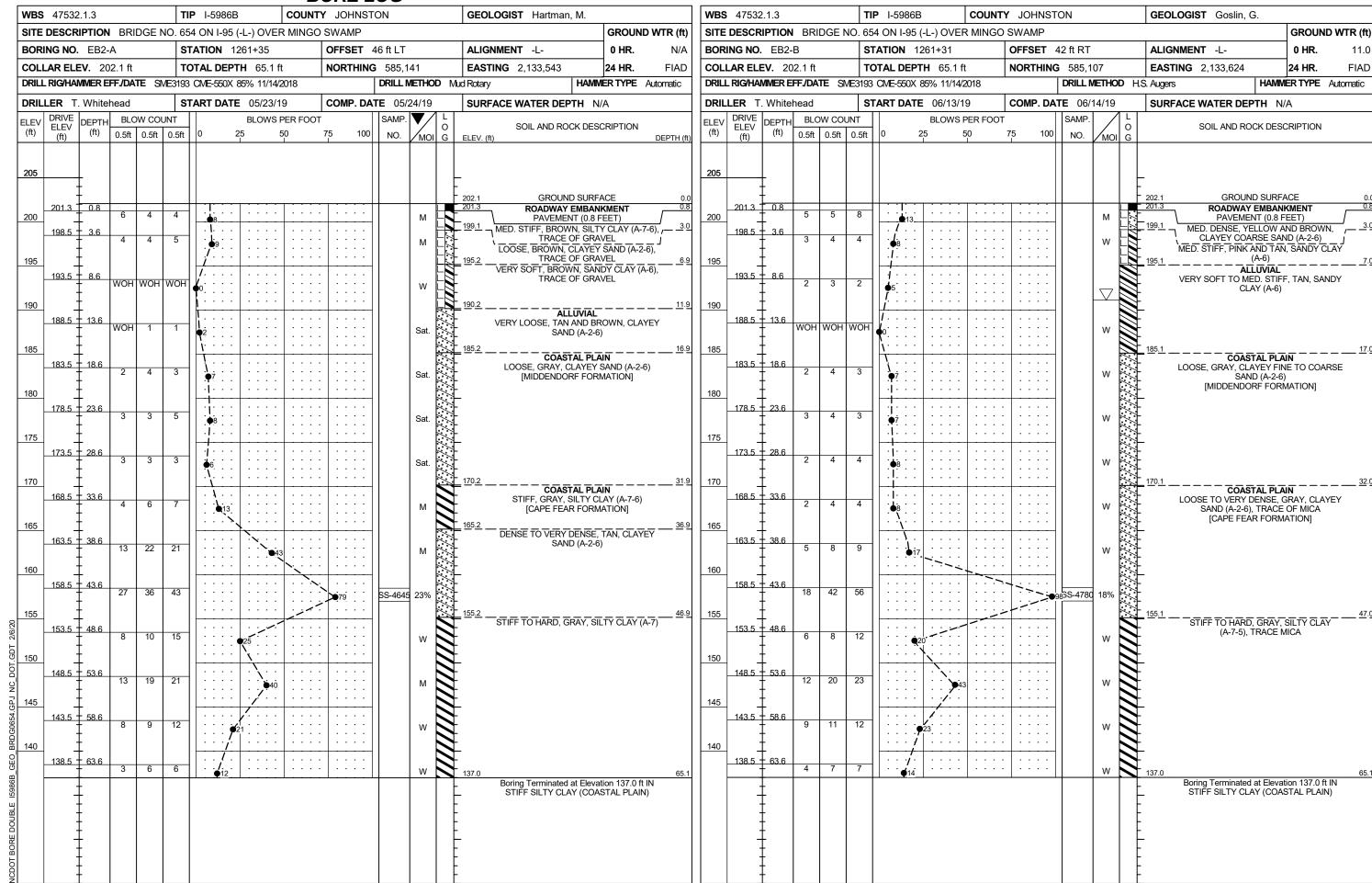


N/A

10.8







# GEOTECHNICAL BORING REPORT

SHEET 18

								ORE L				T			
	47532					<b>P</b> I-5986B		JOHNST	ON			GEOLOGIST Blonesh	ine, E.	1	
				DGE N		54 ON I-95 (-L-) OVEF						Т		1	D WTR (ft)
	ING NO.					<b>TATION</b> 1261+39	-	OFFSET (				ALIGNMENT -L-		0 HR.	N/A
	LAR ELE					OTAL DEPTH 65.0 f		NORTHING				<b>EASTING</b> 2,133,582		24 HR.	10.1
DRILL	_ RIG/HAI	VIMER E	FF./DA	TE SI	ИE3193	CME-550X 85% 11/14/2	2018		DRILL N	/IETHO	D M	ud Rotary	HAMM	ER TYPE	Automatic
DRIL	LER T.	White				TART DATE 06/04/1	9	COMP. DA	<b>FE</b> 06/	04/19	<del></del>	SURFACE WATER DEI	PTH N/	Ά	
LEV (ft)	DRIVE ELEV	DEPTH (ft)		W COL	_	<b> </b>	PER FOOT	75 400	SAMP.	▼/		SOIL AND RO	CK DESC	CRIPTION	
(11)	(ft)	(11)	0.5ft	0.5ft	0.5ft	0 25	50 T	75 100	NO.	/MOI	G	ELEV. (ft)			DEPTH (1
205		_										_			
	-											•			
200	201.0	0.0	7	5	5	10	L			М		201.0 GROUN ROADWAY	ID SURFA EMBANI		С
		[ <u></u>				7.10				'*'		VERY LOOSE TO RED, CLAYEY	LOOSE,	<b>BROWN A</b>	
	197.5	- 3.5 -	3	4	3					Sat.				(, , , _ 0)	
195	_	Ī										<del>-</del>			
	192.5	- - 8.5			0							•			
190	-		2	1	0	1						•			
		[ 									H		LUVIAL		12
	187.5	- 13.5 -	2	1	2				SS-4707	34%		SOFT, GRAY, SILT OF ORG	Y CLAY (	A-7-6), TR	ACE
185	_	_				1							4410 1411		17
	182.5	- - 18.5											TAL PLA		
180	-	-	3	3	4	7				Sat.			A-2-6)		5
	-	-				1						_ [MIDDENDC	KF FUKI	MATION	
	177.5 -	- 23.5 -	3	5	5	·   · ·   · · · · · · · · · · · · ·				Sat.		•			
175	_	-				1 - 1						· <del>-</del>			
	172.5	- - 28.5										•			
170	-	_	5	4	4	8				Sat.		•			
170	-	-				: 1 : : : : : : : : :									32
	167.5	- 33.5 -	3	6	7	\frac{7}{7}				М		STIFF, GRA	TAL PLA AY, CLAY	(A-7-6)	
165	_	-				13				'''		. [CAPÉ FEA -	R FORM	ATIONJ	
	- 162.5 -	- - 38.5													
400	-	-	4	6	8					М					
160	_	_										159.0			42
	157.5	43.5	40	60/0.3				\*\\.\_\_	SS-4713	21%		VERY DENSE, GR.	AY, SILTY	SAND (A-	·2-4)
155	_	_						100/0.8				• <del>-</del>			
	152.5	- - 48.5					-:								ARK 47
450	-102.0	- <del></del>	6	8	12	20				М		. GRAY,	CLAY (A-	7-6)	
150	_	<u>_</u> _										<del>_</del>			
	147.5	53.5	15	27	28	:::: ::::				١,,					
145	-	_					<b>9</b> 55			M		<del>-</del>			
	142.5	- - 58.5				:::: ::/:									
440	142.0	-	7	11	14	25				М					
140	_	_				<del>  / </del>						<del>-</del>			
	137.5	- 63.5 -	5	8	9	$\left \left \begin{array}{c} \dots, f_{i-1}^l \\ \dots, f_{i-1}^l \end{array}\right  \right  \dots \right $				м					
	_	_		Ť		<u> </u>	L	I I	1	IVI		136.0 Boring Terminated	at Elevat	ion 136.0 f	t IN
	-	-										. VERY STIFF CL	AY (COAS	STAL PLAII	N)
	-	-													
	_	-										<del>-</del>			
	-	-													
	-	-									1				

#### **SUMMARY OF LABORATORY TEST DATA**

Soil Classification and Gradation



	S&ME, Inc. Raleigh, 3201 S	Spring Forest Road, Raleigh	n, North Carolina 2761	6	
S&ME Project #:	6235-17-048			Date Report:	8/1/2019
State Project No.:	47532.1.3	County:	Johnston	Date Tested:	7/1/2019
Federal ID No.:	N/A	TIP No.:	I-5986B		
Project Name:	Br No 654 on 1-95 (-1-) ov	or Mingo Swamp			

Project Name:

Br. No. 654 on I-95 (-L-) over Mingo Swamp

Client Name:

Michael Baker International

Client Address: Raleigh, NC

Client Nam	ie.			Michael Ba	iker inter	nation	dl		Client F	laaress.	Raieign,	, INC						
				Sample	AASH	-TO		Total %	Passing		Tota	l Mortar	Fraction	า (%)				
Sample				Depth	Classific	cation		Siev	/e #		Coarse	Fine						Moist.
No.	Station	Offset	Alignment	(ft)			10	40	60	200	Sand	Sand	Silt	Clay	LL	PL	PI	%
SS-4645	1261+35	46' LT	-L-	43.6-45.1	A-2-6	(0)	100	89	-	24.1	32	48	10	10	36	19	17	22.5
SS-4662	1260+78	44' LT	-L-	58.4-59.9	A-7-5	(10)	100	93	-	65.5	16	28	42	14	46	31	15	22.7
SS-4679	1259+90	41' LT	-L-	53.4-54.9	A-7-6	(20)	100	99	-	81.1	2	33	44	21	51	29	22	23.8
SS-4693	1259+18	43' LT	-L-	38.5-40.0	A-7-6	(3)	99	68	-	36.7	46	19	4	31	41	17	24	22.0
SS-4696	1259+18	43' LT	-L-	53.5-55.0	A-7-6	(15)	100	95	-	68.3	10	33	40	17	50	28	22	22.6
SS-4707	1261+39	6' LT	-L-	13.5-15.0	A-7-6	(3)	87	82	-	35.8	15	47	3	35	44	19	25	33.5
SS-4713	1261+39	6' LT	-L-	43.5-45.0	A-2-4	(0)	100	87	-	33.5	28	43	16	13	23	14	9	20.5
SS-4726	1260+79	8' LT	-L-	38.5-40.0	A-4	(2)	100	99	-	17.3	2	56	24	18	31	22	9	17.3
SS-4746	1259+89	7' LT	-L-	38.4-39.9	A-4	(1)	100	100	-	47.2	3	60	17	20	25	16	9	20.0
SS-4752	1259+89	7' LT	-L-	68.4-69.9	A-7-5	(22)	100	96	-	84.5	7	17	58	18	54	32	22	30.1
SS-4759	1259+21	7' LT	-L-	18.6-20.1	A-1-b	(1)	50	43	-	4.6	64	28	3	5	N.P.	N.P.	N.P.	13.5
SS-4762	1259+21	7' LT	-L-	33.6-35.1	A-7-6	(30)	100	97	-	87.9	4	12	25	59	53	21	32	34.6
SS-4766	1259+21	7' LT	-L-	53.5-55.1	A-7-6	(16)	100	96	-	73.6	9	29	50	12	50	29	21	25.5
SS-4780	1261+31	42' RT	-L-	43.6-45.1	A-2-6	(0)	99	79	-	28.7	37	40	19	4	31	20	11	17.5
SS-4798	1260+84	46' RT	-L-	63.4-64.9	A-7-6	(16)	100	97	-	76.7	6	21	53	20	49	29	20	19.6
SS-4810	1259+83	52' RT	-L-	33.4-34.9	A-7-6	(32)	100	95	-	77.2	9	20	23	48	58	17	41	21.2
SS-4820	1259+83	52' RT	-L-	83.4-84.9	A-2-6	(0)	100	71	-	30.7	46	27	14	13	32	19	13	18.1
SS-4826	1259+24	46' RT	-L-	23.5-25.0	A-3	(1)	99	53	-	4.9	86	10	1	3	N.P.	N.P.	N.P.	23.9
SS-4834	1259+24	46' RT	-L-	63.5-65.0	A-7-6	(25)	100	99	-	85.9	2	28	49	21	52	25	27	24.7
Deferences	/ Commonto	/ Davistia		Poforoncos	/ C	-t- / D.	Defere	/	/	Daniation	Defere	/ C	Defere	/ C	/	Defere	/ C	

References / Comments / Deviations: References / Comments / Deviation References / Deviation References / Deviation References / Comments / Deviation References / Deviation Ref

AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT

AASHTO T89: Determining the Liquid Limit of Soils

AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils

AASHTO T265: Laboratory Determination of Moisture Content of Soils

AASHTO M145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

Mal Krajan, ET

Technician Name:

Technician Name:

Technician Name:

Technician Name:

Technician Name:

Technician Name:

Technician Responsibility:

Technical Responsibility:

Position

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# SITE PHOTOGRAPH

Bridge No. 654 on –L– (I-95) over Mingo Swamp



5986B REFERENCE **CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

CROSS SECTION(S) BORE LOG(S) SITE PHOTOGRAPH(S)

TITLE SHEET

SITE PLAN

PROFILE(S)

SHEET NO.

5-6 7-12

4

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY JOHNSTON

PROJECT DESCRIPTION I-95 WIDENING FROM SR 1811 (BUD HAWKINS RD.) (EXIT 70) TO I-40 (EXIT 81) -WIDEN TO EIGHT LANES

SITE DESCRIPTION BRIDGE NO. 655 ON I-95 OVER DRIVING BRANCH

STATE PROJECT REFERENCE NO. 13 I-5986B

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDING TO CLIMATIC CONDITIONS INCLORDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

H. CAMP S. HARDEE T. WHITEHEAD INVESTIGATED BY \_\_\_\_\_S&ME, Inc. CHECKED BY S. MITCHELL SUBMITTED BY S. MITCHELL DATE \_\_FEBRUARY 2020



9751 SOUTHERN PINE BLVD CHARLOTTE, NC 28273



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PROJECT REFERENCE NO. SHEET NO.

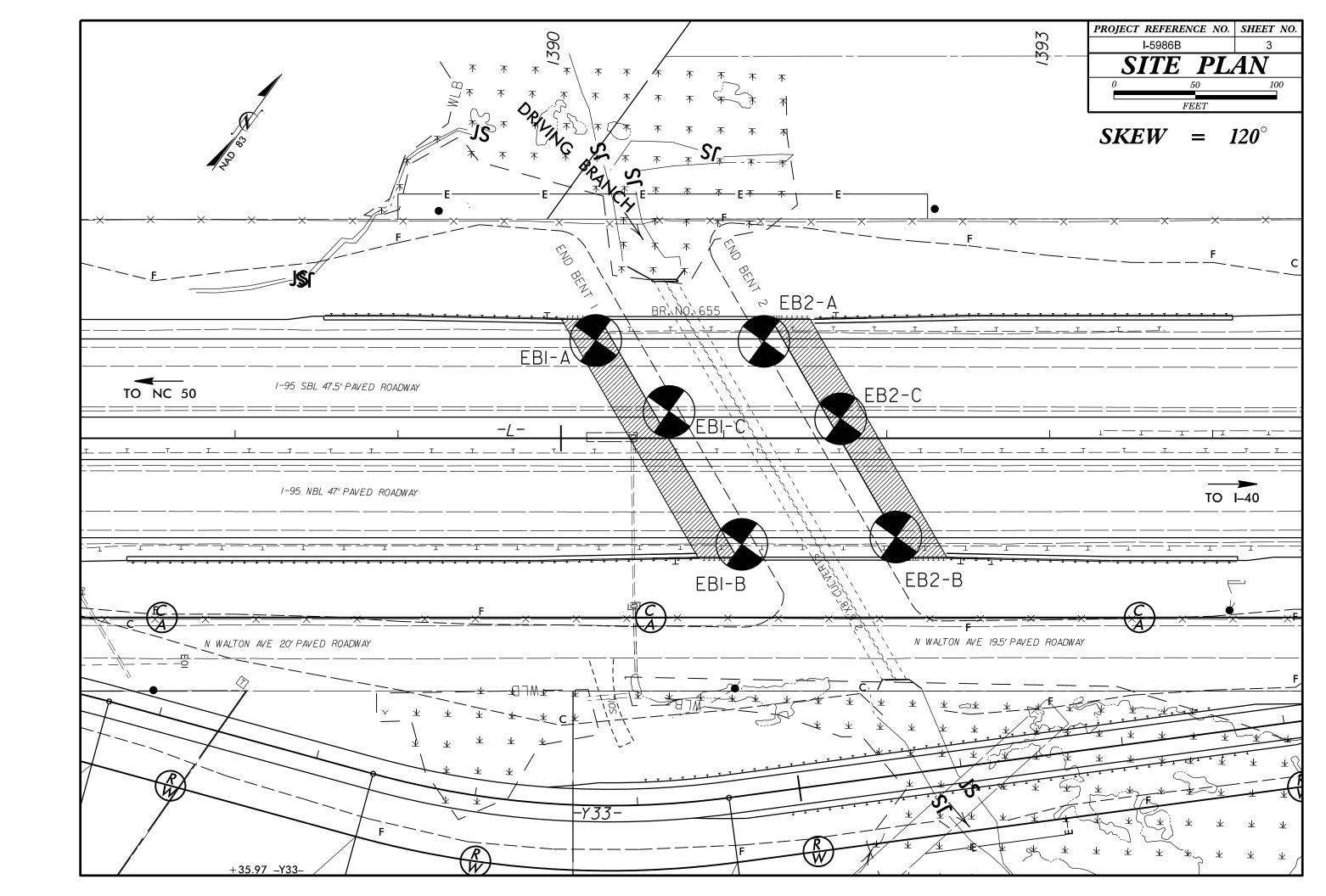
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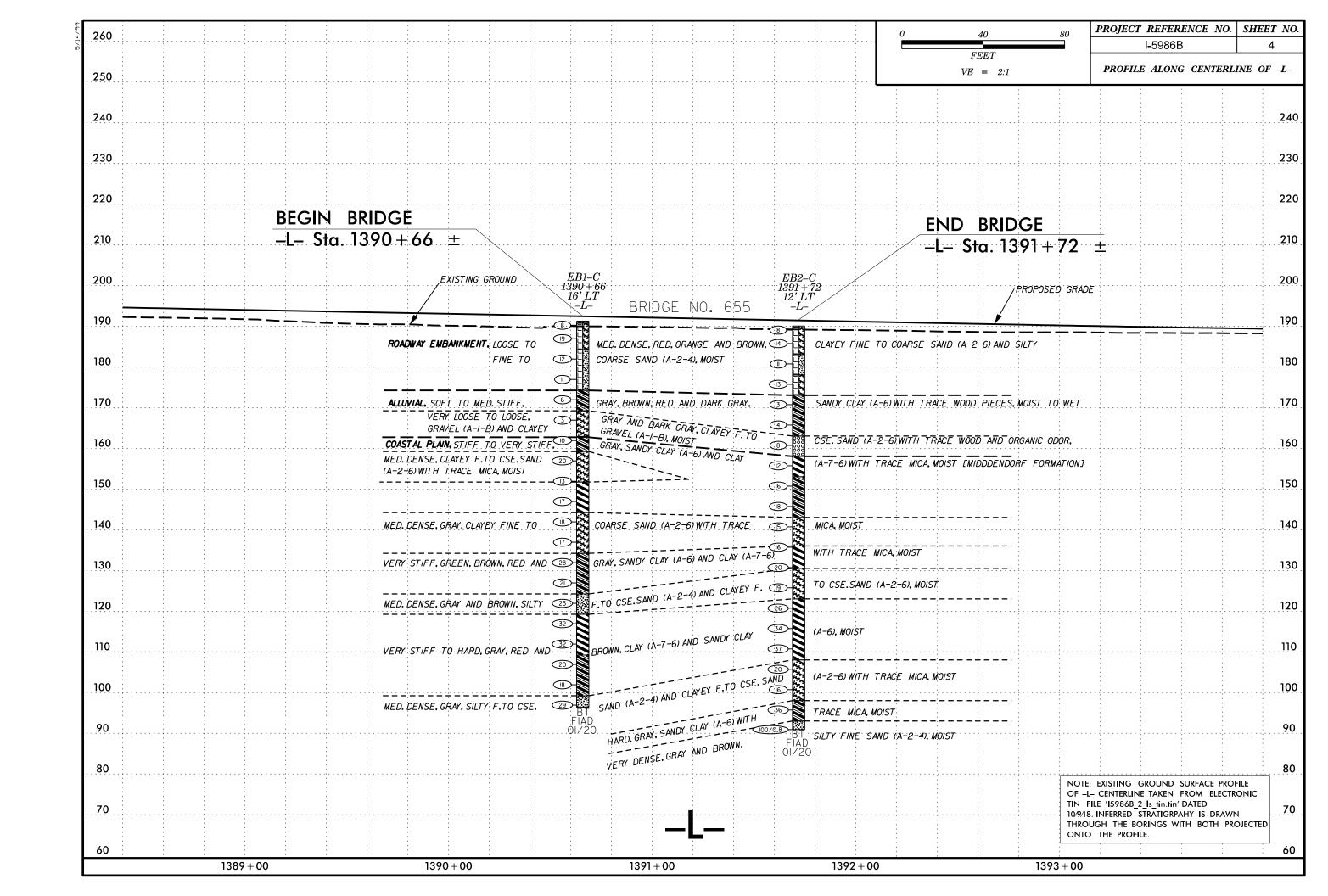
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

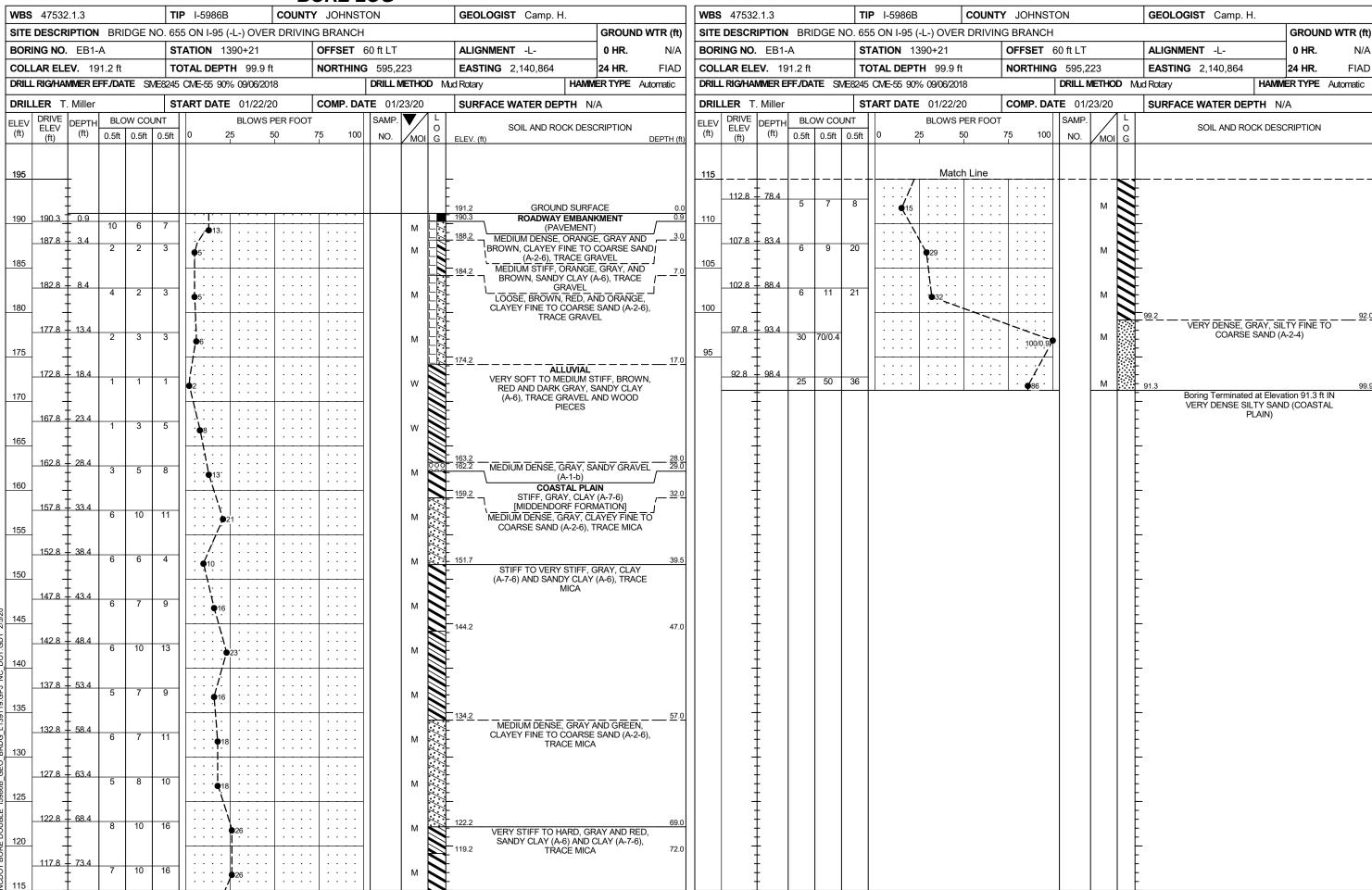
# SUBSURFACE INVESTIGATION

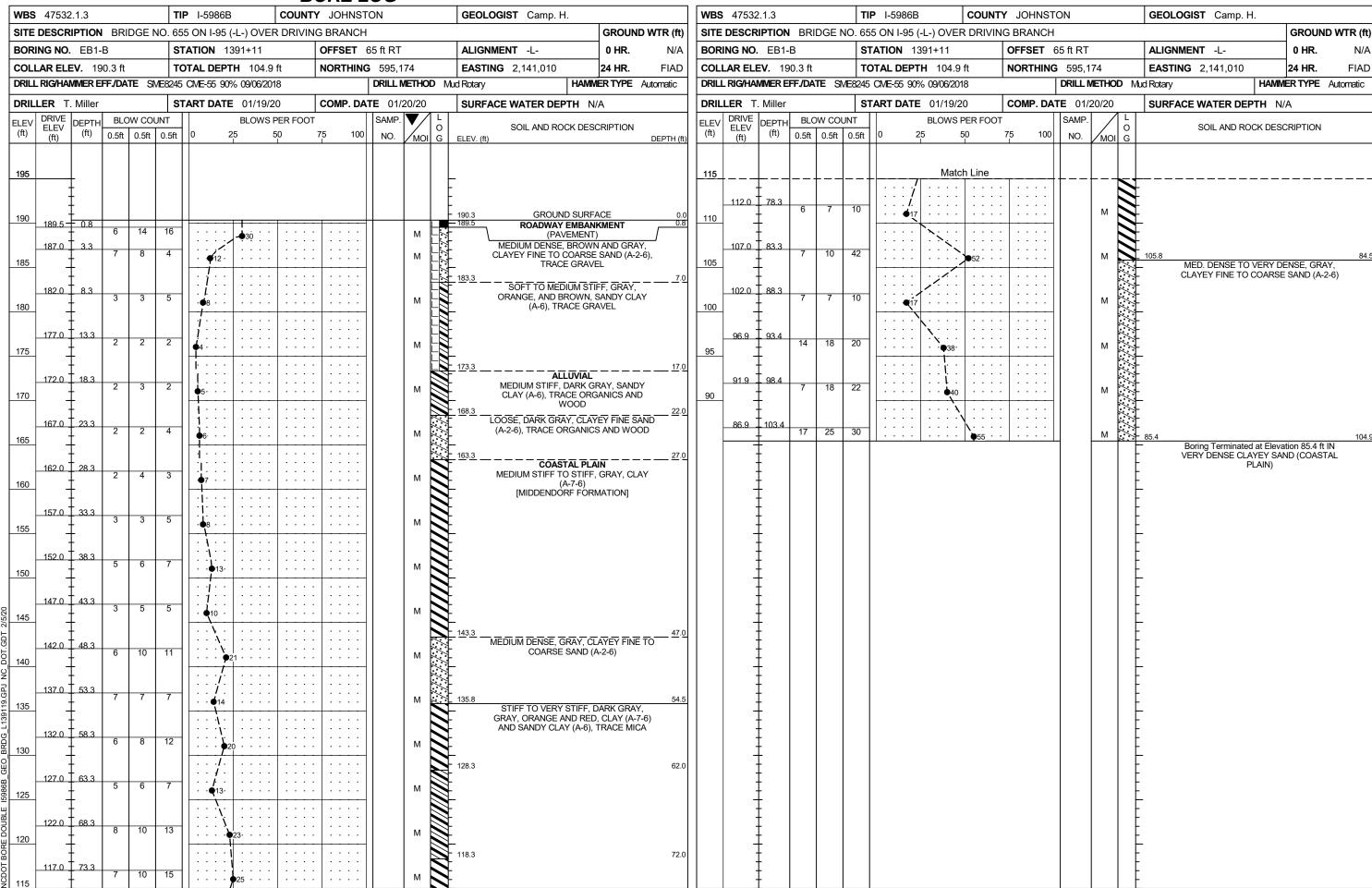
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	SI//AI//A	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED VIGORIAN NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERALOGICAL COMPOSITION	CRYSTALLINE CRYSTA	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
LLASS. (\$\(\sigma\) 39% PASSING "200) (> 39% PASSING "200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.  ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-0 A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-2-6 A-2-7 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE - FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
0000000000	SLIGHTLY COMPRESSIBLE LL < 31	ROCK (NCR)  SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED.  ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 000000000000000000000000000000000000	MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
7. PASSING	HIGHLY COMPRESSIBLE LL > 50  PERCENTAGE OF MATERIAL	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*40 30 MX 50 MX 51 MN CLAY PEAT		- WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
א מוט בי אוא מון א מוט	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3%, 3 - 5%, TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL PASSING *40	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	HORIZONTAL.
LL 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 11THE DR	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP IW MX IW MX II MN II MN IW MX IW MX II MN II MN MODERATE OPCONIC	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH,
GROUP INDEX W W 4 MX 8 MX 12 MX 16 MX NU MX AMUUNIS UF SOILS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STUNE HAUSS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND CRAVEL AND SAND SOILS SOILS	lacksquare static water level after $24$ hours	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	<u> </u>	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
AS SUBURHUE PUUN	SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30	<u> </u>	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
CONSISTENCY OR DENSENESS  RANGE OF STANDARD RANGE OF UNCONFINED	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD. SEV.)  AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTIVES OF PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	<u>IF TESTED, WOULD YIELD SPT REFUSAL</u>	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
(N-VALUE) (TUNS/FT-)	WITH SOIL DESCRIPTION OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4  CONTROL LOOSE	SOIL SYMBOL  SOIL SYMBOL  SUPPLINT TEST BORING  SLOPE INDICATOR INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIAL MEDIUM DENSE 10 TO 30 N/A	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) DENSE 30 TO 50  VERY DENSE > 50	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT  AUGER BORING  CONE PENETROMETER TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25	── INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	(V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY         MEDIUM STIFF         4 TO 8         0.5 TO 1.0           MATERIAL         STIFF         8 TO 15         1 TO 2	WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4  HARD > 30 > 4	TTTTT ALLUVIAL SOIL BOUNDARY A PIEZOMETER INSTALLATION - SPT N-VALUE	ALSO AN EXAMPLE.	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNSUITABLE WASTE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
COARSE FINE	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER		MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
	ABBRE VIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SOIL MOISTURE - CORRELATION OF TERMS	CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOUR MOISTURE SCALE FIELD MOISTURE	CPT - CONE PENETRATION TEST NP - NON PLASTIC 7 <sub>d</sub> - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS)  DESCRIPTION  GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u>	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
(SAT.) FROM BELOW THE GROUND WATER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID; REQUIRES DRYING TO	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
	FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: BL-II9 N: 595164 E: 214II24
(PI) PL PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: 176.43 FEET
SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO	CME-45C X CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	FIAD = FILLED IMMEDIATELY AFTER DRILLING
- DRY - (U) ATTAIN OPTIMUM MOISTURE	X CME-55 G* CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	A STATE OF THE STA
PLASTICITY	8" HULLUW AUGERS	INDURATION	-
PLASTICITY INDEX (PI) DRY STRENGTH	CME-550 HARD FACED FINGER BITS	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST TUNGCARBIDE INSERTS HAND TOOLS:	RUBBING WITH FINGER FREES NUMEROUS GRAINS; FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM	X CASING W/ ADVANCER HAND TOULS:	CRAINC CAN BE CERABATED FROM CAMBLE WITH CIFFL BRODE.	
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	MODERATELY INDURATED  MODERATELY INDURATED  BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE:	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN. RED. YELLOW-BROWN, BLUE-GRAY).	CORE BIT VANE SHEAR TEST	DIFFICULT TO BREAK WITH HAMMER.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED  SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;  SAMPLE BREAKS ACROSS GRAINS,	DATE: 8-15-1-
•		SHMILE DREHKS ACKUSS UKAINS.	DATE: 8-15-1

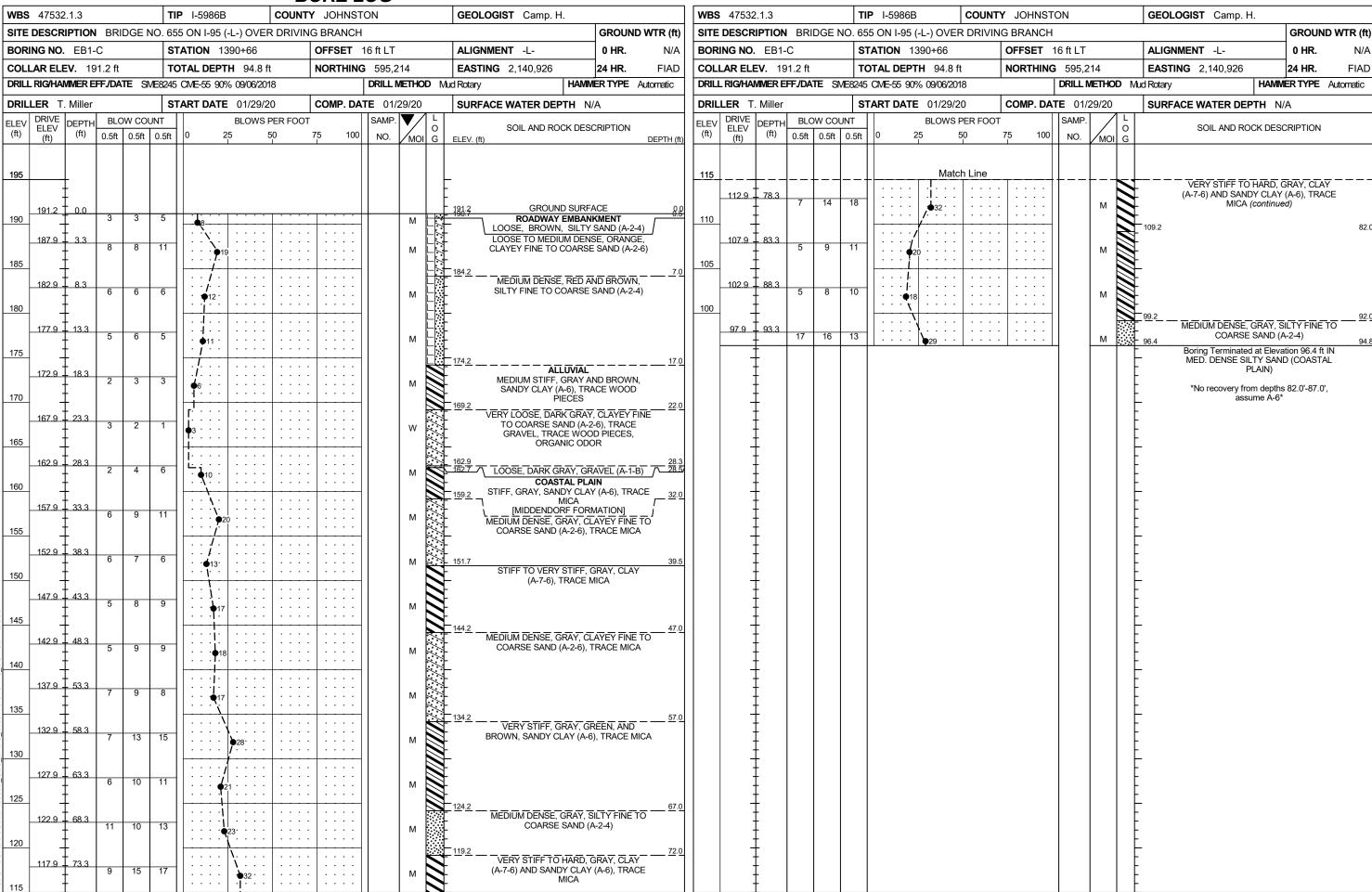


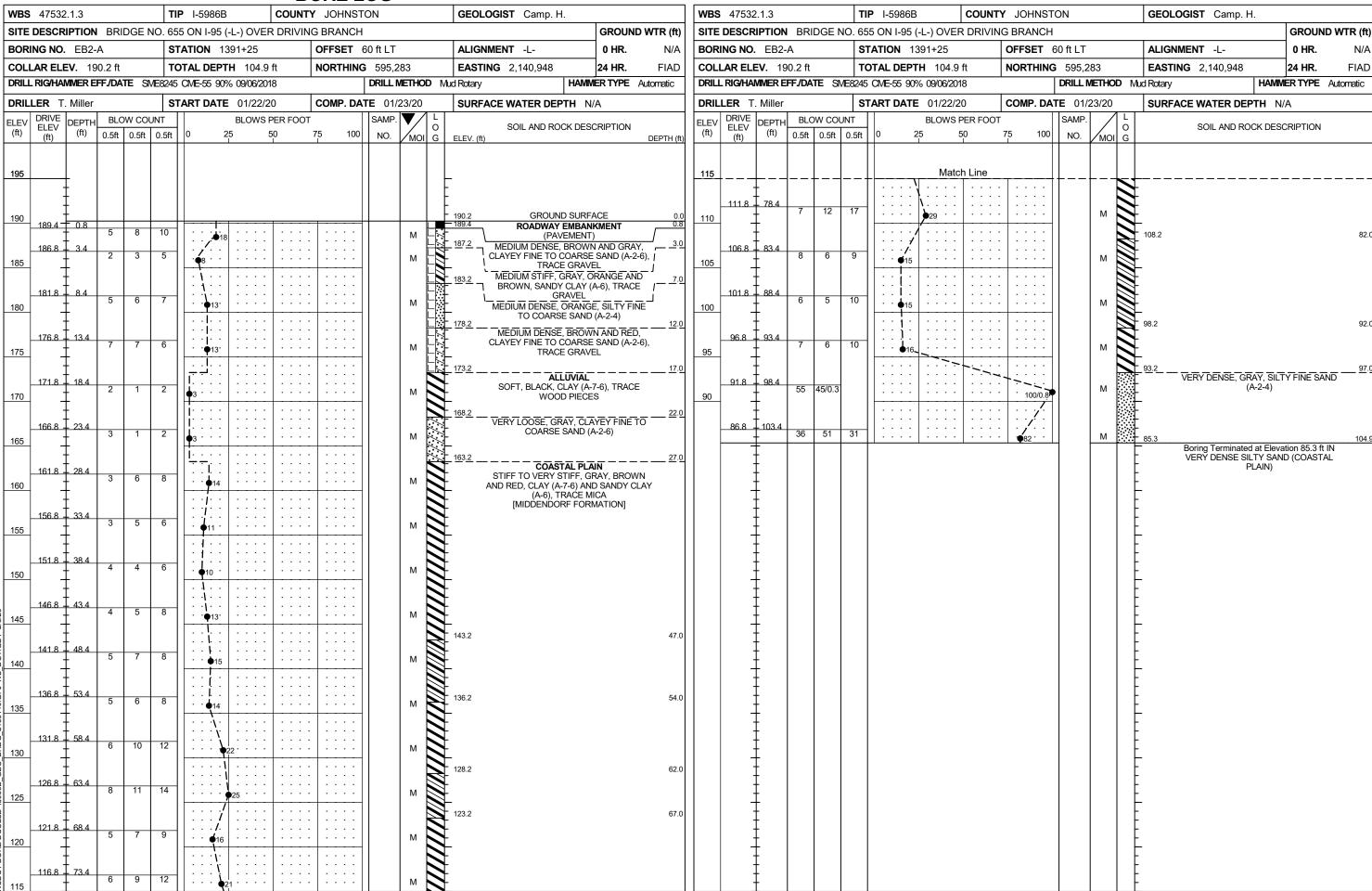


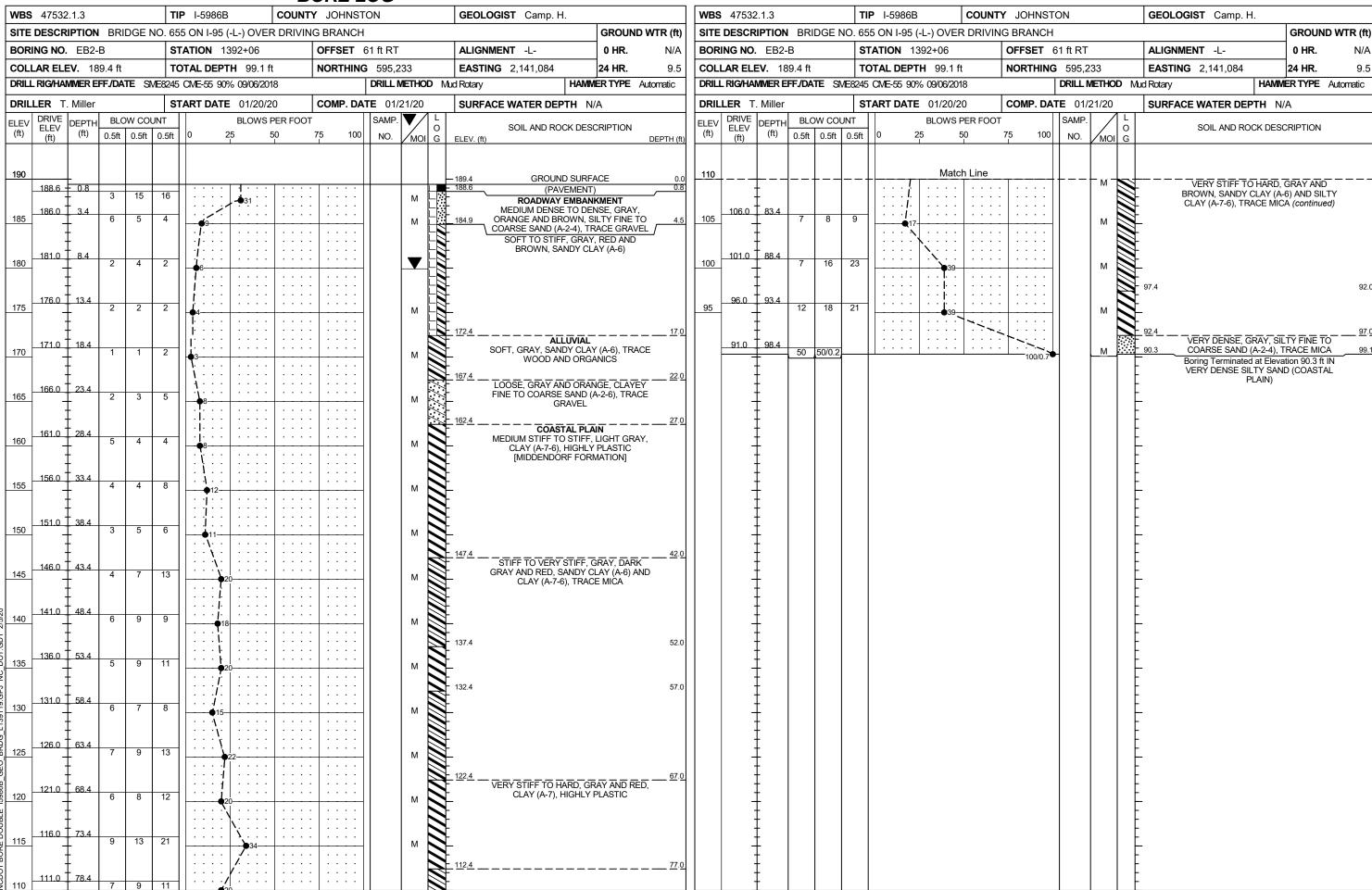


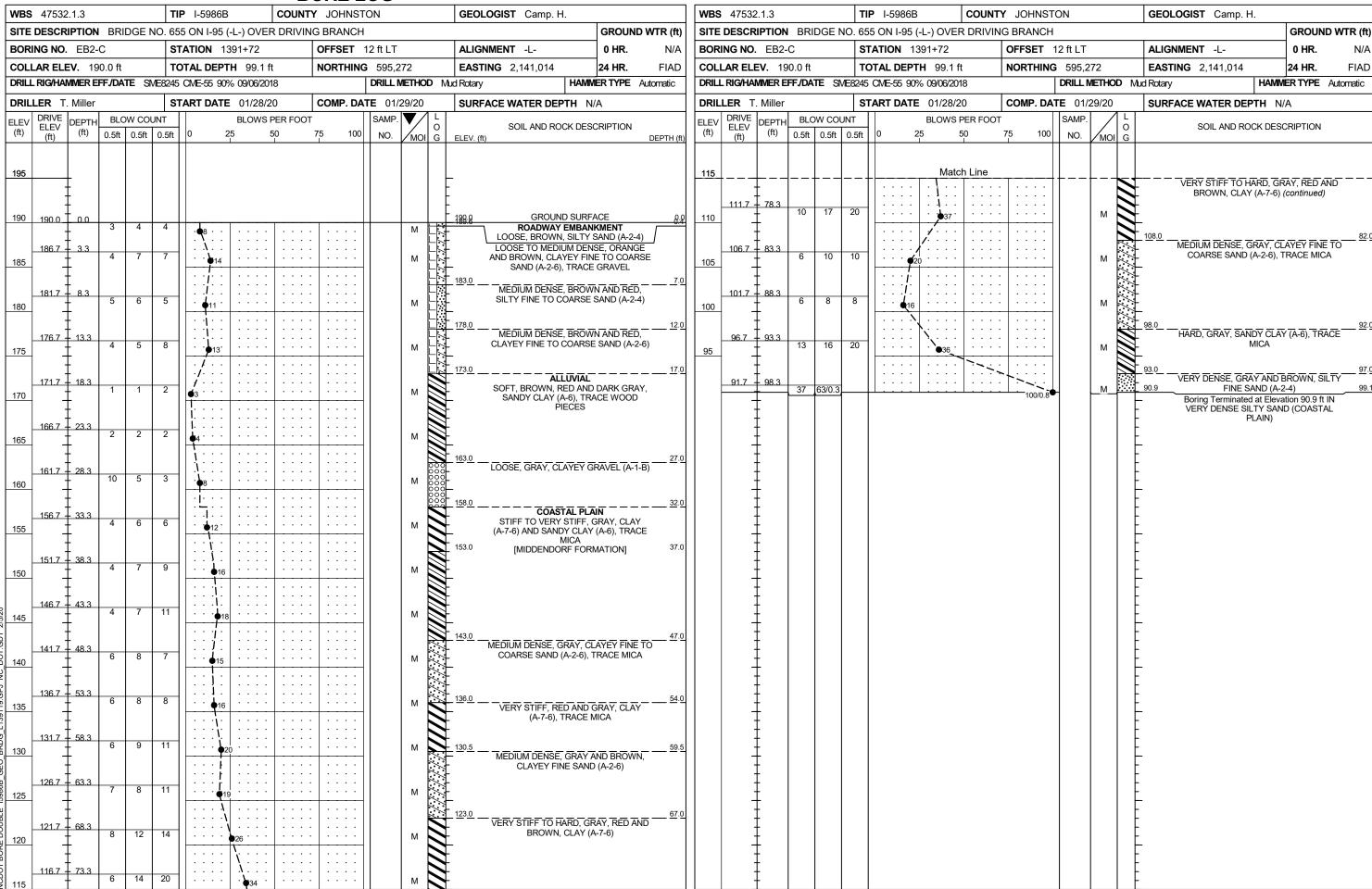


N/A









# SITE PHOTOGRAPH

Bridge No. 655 on –L– (I-95) over Driving Branch



Looking Northeast Looking West



# Culvert Foundation Recommendation Letter Box Culvert @ -Y33- Sta. 35+91 over Driving Branch Johnston County, North Carolina TIP No. I-5986B S&ME Project No. 6235-17-048

#### PREPARED FOR

Michael Baker International 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518

#### **PREPARED BY:**

S&ME, Inc. 9751 Southern Pine Boulevard Charlotte, North Carolina 28273

March 10, 2020



March 10, 2020

Michael Baker International 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518

Attention: Mr. Dwain Hathaway, P.E.

Reference: Culvert Foundation Recommendation Letter

Box Culvert @ -Y33- Sta. 35+91 over Driving Branch

Johnston County, North Carolina

NCDOT TIP No. I-5986B

S&ME Project No. 6235-17-048 NC PE Firm License No. F-0176

Dear Mr. Hathaway:

S&ME, Inc. (S&ME) has completed the authorized foundation recommendation report for the above-referenced project as part of the I-95 Widening project. Our services were performed in general accordance with the *Exhibit A - Task Order#6* of the Master Subcontract Agreement between Michael Baker International (MBI) and S&ME executed on March 24, 2017. Culvert design information was provided via the Culvert Survey & Hydraulic Design Report prepared by Sungate Design Group, P.A., dated January 13, 2020.

#### **Project and Recommendations**

We understand that the proposed culvert will consist of an approximately 62.5 feet long, 21 @ 6-foot x 9-foot RCBC with centerline bed elevation of 163.5 feet and slope of 0.3% at Station 35+91 -Y33- alignment. Based on our subsurface exploration, the box culvert is planned to bear in alluvial soils. The RCBC should be installed to bear on a minimum of 12 inches of foundation conditioning material in accordance with NCDOT Standard Specifications Section 414 due to the variability of alluvial soils. A total of 79 tons of foundation conditioning material is anticipated.

Total settlement along the proposed culvert alignment is anticipated to be less than 1 inch with differential settlement of approximately ½ inches, if constructed on suitable soils.



# Culvert Foundation Recommendation Letter Box Culvert @ -Y33- Sta. 35+91 over Driving Branch

NCDOT TIP No. I-5986B S&ME Project No. 6235-17-048

S&ME appreciates the opportunity to provide our services on this project. Please contact us if you have any questions regarding this report or if we may be of further assistance.

Sincerely,

S&ME, Inc.

State Mitchell

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Stacie E. Mitchell, P.E.

Project Manager

Senior Review By: Kristen H. Hill, P.E., P.G.

Docusigned by:

SEAL

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Alyson K. Aarons, P.E.

(née Yetman)

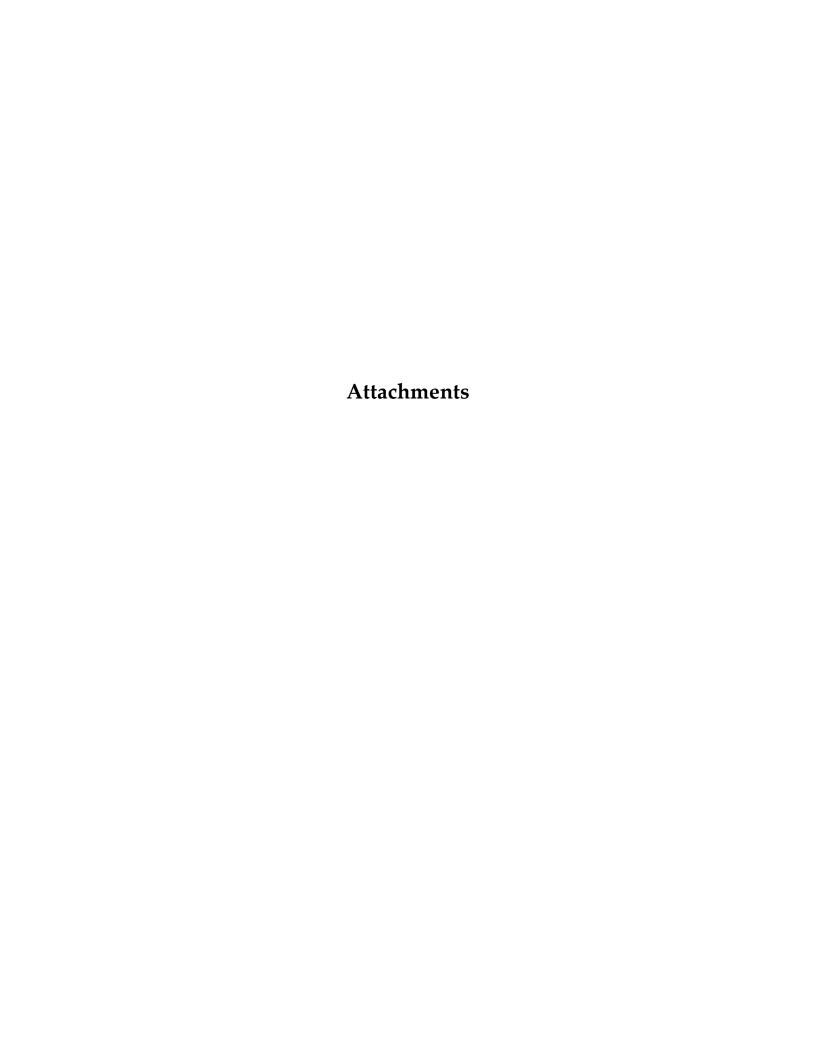
Project Engineer

NC Registration No. 046061

#### **Attachments**

Structure Subsurface Inventory Report Culvert Survey & Hydraulic Design Report FCM Quantity Calculations Settlement Calculations

March 10, 2020 2



5986B REFERENCE **CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

TITLE SHEET

SITE PLAN

PROFILE(S) BORE LOG(S)

SHEET NO.

4

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY JOHNSTON

PROJECT DESCRIPTION <u>I-95 WIDENING FROM SR</u> 1811 (BUD HAWKINS RD.) (EXIT 70) TO I-40 (EXIT 81) -WIDEN TO EIGHT LANES

SITE DESCRIPTION CULVERT ON -Y33- OVER DRIVING **BRANCH** 

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	I-5986B	1	5

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (INP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS THE ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS, AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE SUBSURFACE INFORMATIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

	H. CAMP
	T. MILLER
INVESTIGATED	BY S&ME, Inc.
DRAWN BY	J. SWARTLEY
	S. MITCHELL
SUBMITTED BY	S. MITCHELL
DATE MA.	RCH 2020

DEDCUMNET



9751 SOUTHERN PINE BLVD CHARLOTTE, NC 28273 (704) 523-4726



**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

PROJECT REFERENCE NO. SHEET NO.

I-5986B

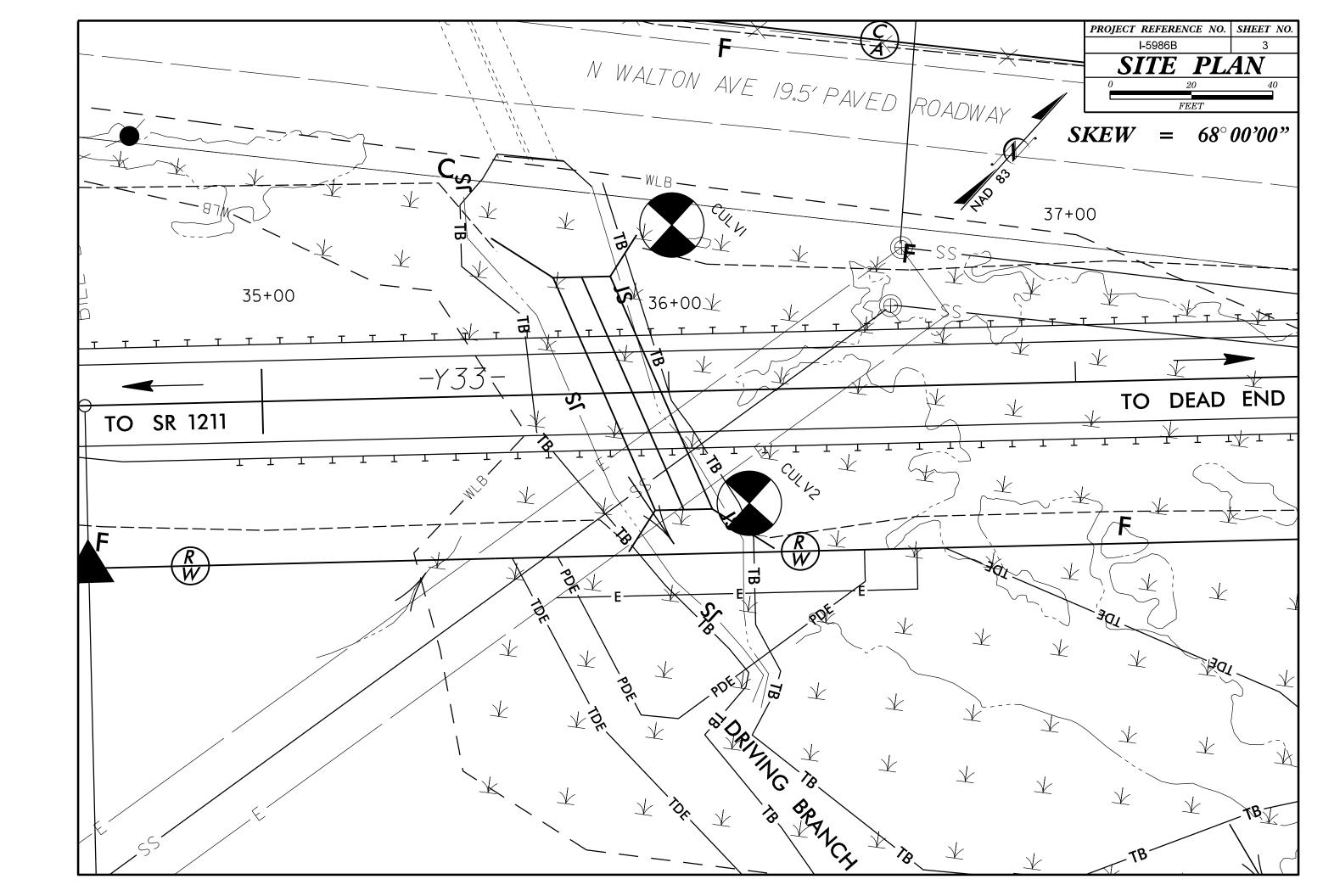
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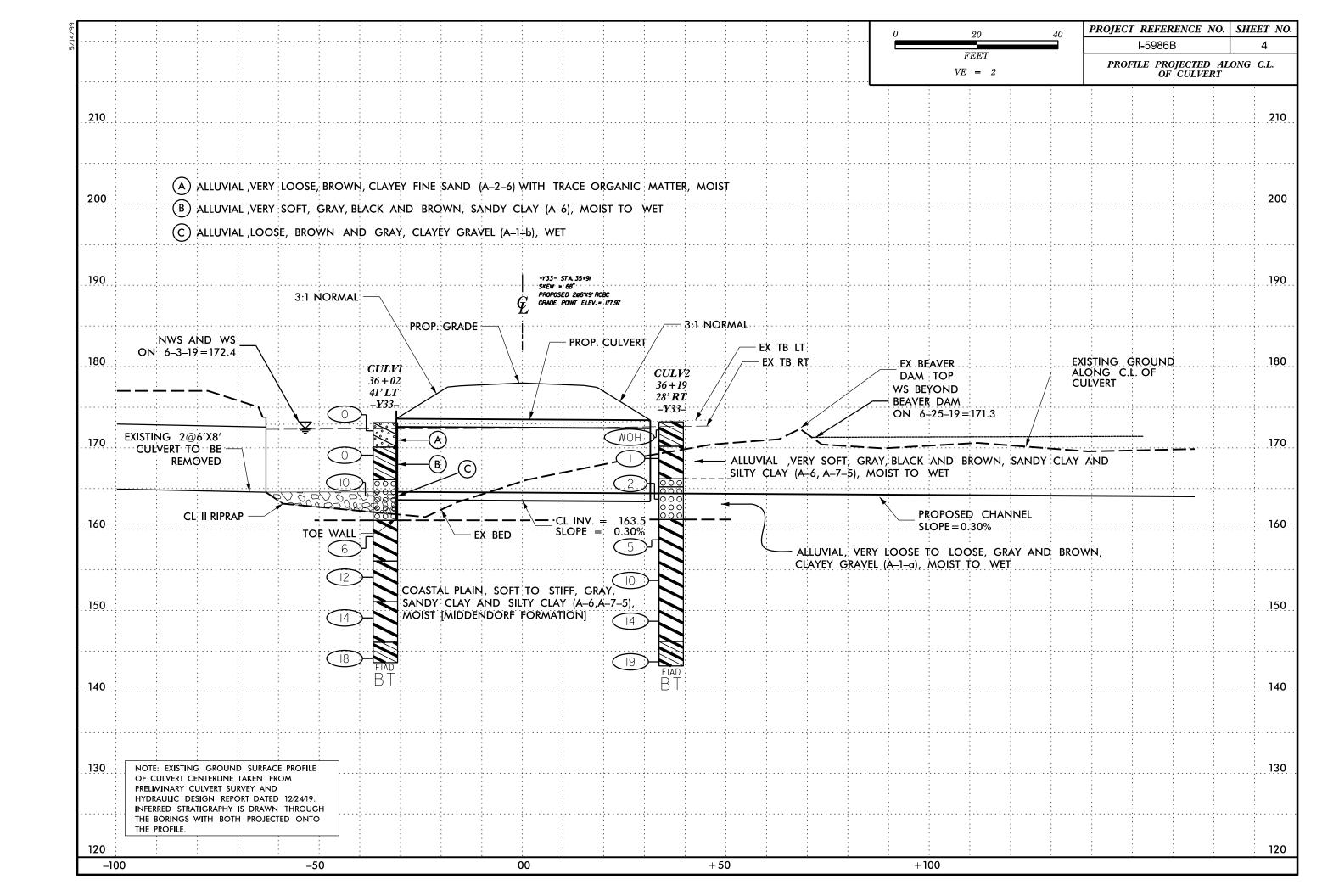
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

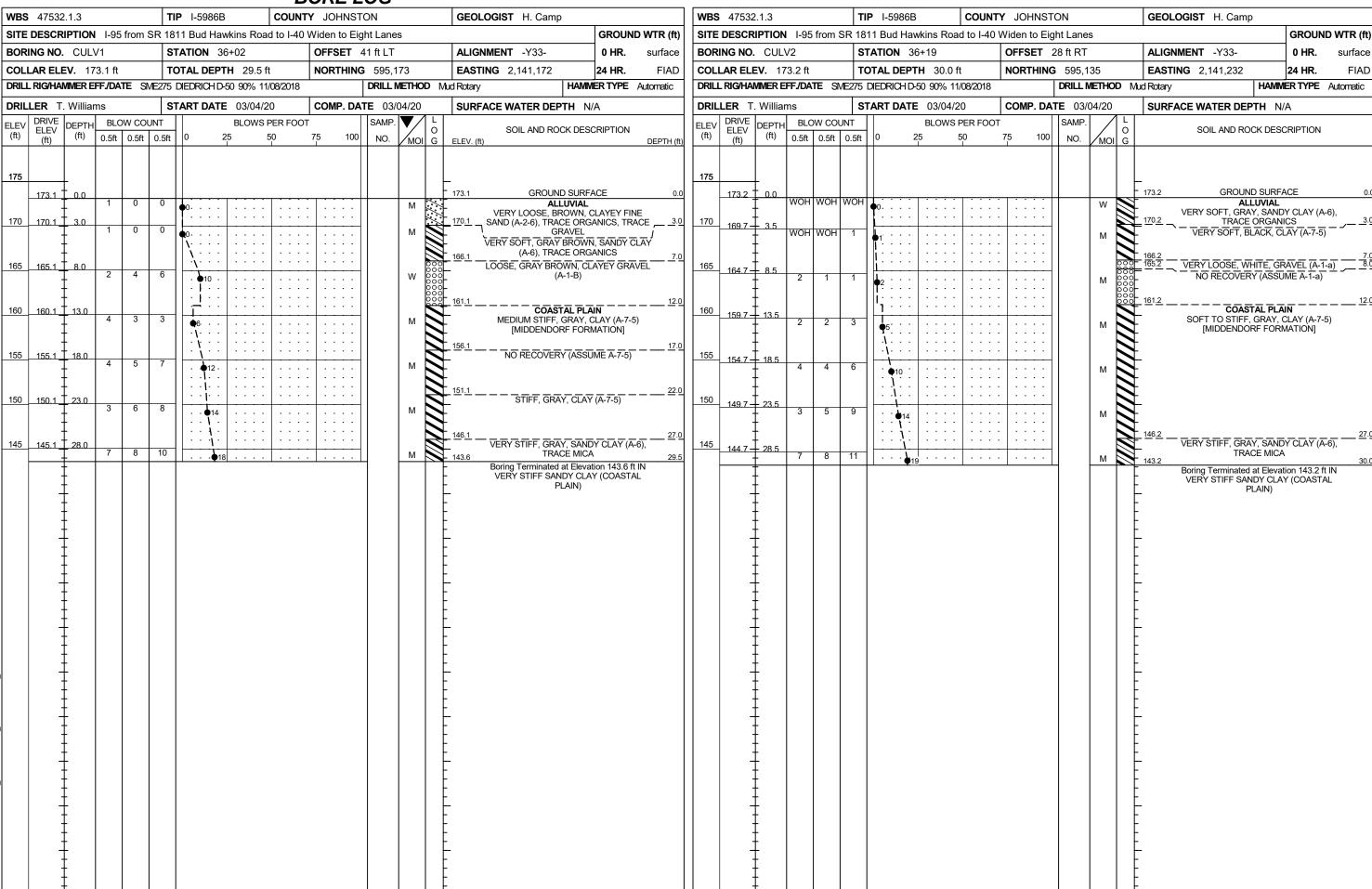
# SUBSURFACE INVESTIGATION

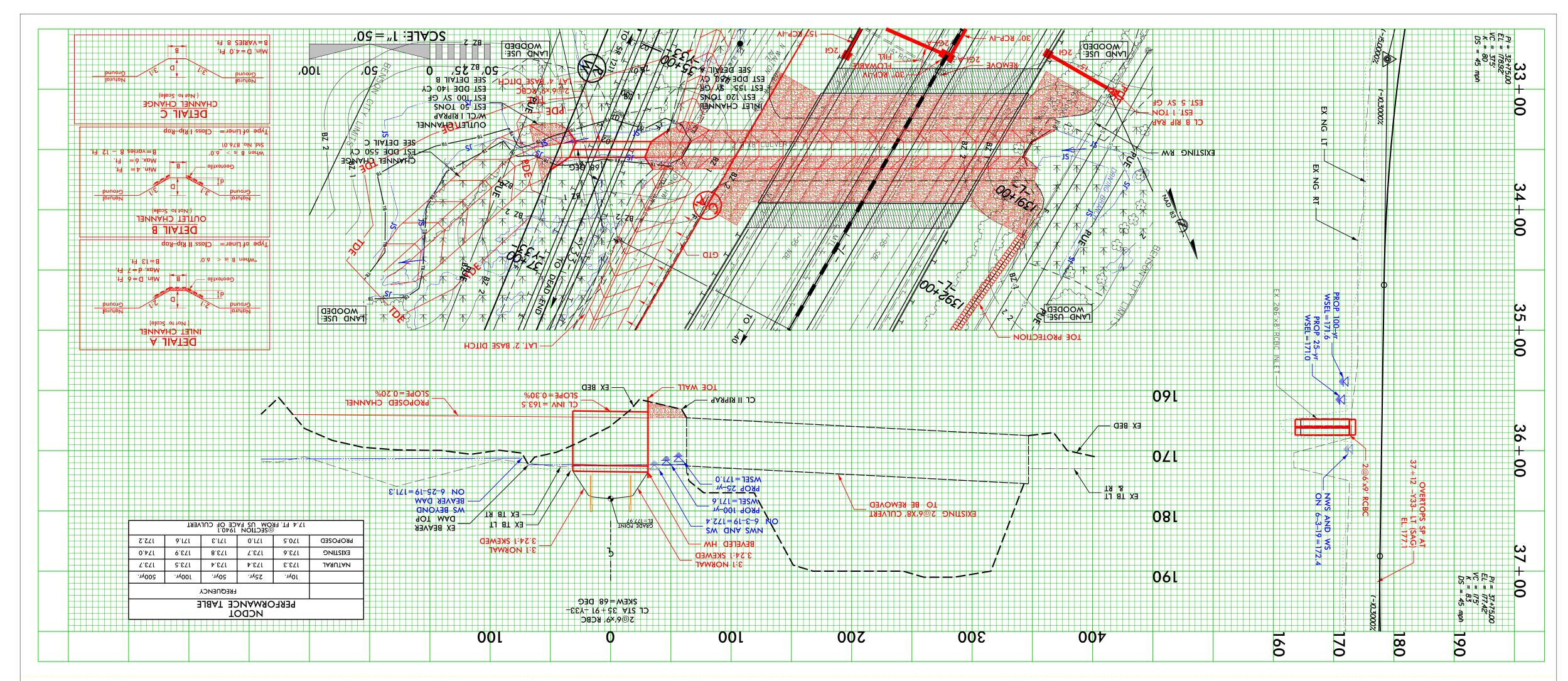
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

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SOIL DESCRIPTION  SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN	GRADATION  WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	ROCK DESCRIPTION  HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED	TERMS AND DEFINITIONS
BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	<u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	AGUIFER - A WATER BEARING FORMATION OR STRATA.  ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE.	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF,GRAY,SILTY CLAY,MOIST WITH INTERBEDDED FINE SAND LAYERS,HIGHLY PLASTIC,A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:  ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION	ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	CRYSTALLINE CRYSTALLINE WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-8 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-7-6	COMPRESSIBILITY	NON-CRYSTALLINE ROCK (NCR)  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	OF SLOPE.
7 PASSING SILT-	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED CP) SHELL BEDS, ETC.	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR CLAY MUCK,	PERCENTAGE OF MATERIAL	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*200   15 MX   25 MX   10 MX   35 MX   35 MX   35 MX   35 MX   36 MN   36 MN   36 MN   36 MN   36 MN   36 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.  DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL DAGGUE ALO	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%  LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	HORIZONTAL.
PASSING *40 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN 11 MN M MX 10 MX 11 MN 11 MN 11 MN MODERATE ORGANIC	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH,
GROUP INDEX 8 8 MX 12 MX 16 MX NU MX AMOUNTS UP SOILS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STUNE FRAUS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	√ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN, RATING EXCELLENT TO GOOD FAIR TO POOR PAIR TO POOR UNSUITABLE		(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
AS SUBURADE POUR	SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30  CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
RANGE OF STANDARD RANGE OF UNCONFINED		(MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES CLUNK SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (N-VALUE) (TONS/FT <sup>2</sup> )	ROADWAY EMBANKMENT (RE)  25/825  DIP & DIP DIRECTION  OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL  SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
VERY LOOSE ( 4	┦ 박	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GENERALLY LOOSE 4 TO 10 GRANULAR MEDIUM DENSE 10 TO 30 N/A	SOIL SYMBOL  SOIL SYMBOL  SOIL SYMBOL  SLOPE INDICATOR INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.  IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS
MATERIAL DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERT DENSE 2 30	THEEDOED CON DOWNERS CODE DODNIC CODE	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
VERY SOFT         < 2         < 0.25           GENERALLY         SOFT         2 TO 4         0.25 TO 0.5	- INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	TTTTT ALLUVIAL SOIL BOUNDARY A PIEZOMETER SPT N-VALUE	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
HARD > 30 > 4  TEXTURE OR GRAIN SIZE		ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
COARSE FINE	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER   COBBLE   GRAVEL   SAND   SAND   SILT   CLAY	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SOIL MOISTURE - CORRELATION OF TERMS	CL CLAY  MOD MODERATELY  7 - UNIT WEIGHT  CPT - CONE PENETRATION TEST  NP - NON PLASTIC  7 - DRY UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(HITEROEUR FIMITS) DESCRIFTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS  DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO   SD SAND, SANDY   SS - SPLIT SPOON   F - FINE   SL SILT, SILTY   ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
LL _ LIOUID LIMIT	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
PLASTIC   SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS \( \omega \) - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	
(PI) PL PLASTIC LIMIT	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	BENCH MARK: •SEE NOTE
ON CONTINUE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: FEET
OM OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED Ø.16 - 1.5 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO	CME-45C X CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	•ELEVATIONS DERIVED FROM GEOPAK AND .TIN FILE '15986b_2_Is_tin.tin'
ATTAIN UPTIMUM MUISTURE	CME-55  CME-55  CME-55  CME-55  CME-55	THINLY LAMINATED < 0.008 FEET	DATED 10/9/18
PLASTICITY	X 8° HOLLOW AUGERS	INDURATION	1
PLASTICITY INDEX (PI)  DRY STRENGTH	CME-550 HARD FACED FINGER BITS -N	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.  RUBBING WITH FINGER FREES NUMEROUS GRAINS;	
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST VANE SHEAR TEST HAND TOOLS:	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	CASING W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
COLOR	PORTABLE HOIST TRICONESTEEL TEETH HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
	X D-50 TRICONE TUNGCARB. SOUNDING ROD	INDURATED DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).  MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	CORE BIT VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	
IENO SOON HO ETONY, DENNY, STREENED, ETC. HILL OSED TO DESCRIBE HITEHRINGE.		SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14









# ADDITIONAL INFORMATION AND COMPUTATIONS .DRAINAGE.AREA. = .. 811 ACRES. (1.27. SQ. Ml.). IMPERVIOUS AREA .% . = . .34.8% . (FROM . FUTURE .LAND . USE .MAPS) . . 24-HR, 50-YR MAX. PRECIPITATION = 7.64 IN USGS SIR 2014-5030 REGION 4, 0.10 SQ. MI < DA < 53.5 SQ. MI. SAY $Q_{10} = 51.8(1.27)^{0.6004}10^{0.0101*34.8}10^{0.0666*7.64}$ =434 CFS 430 CFS $Q_{25} = 67.1(1.27)^{0.6067}10^{0.0075*34.8}10^{0.0708*7.64}$ =492 CFS 490 CFS $Q_{50} = 78.4(1.27)^{0.6111} 10^{0.0058*34.8} 10^{0.0738*7.64}$ =529 CFS 530 CFS $Q_{100} = 90.5(1.27)^{0.6154}10^{0.0043*34.8}10^{0.0762*7.64}$ =565 CFS 570 CFS $Q_{500} = 119(1.27)^{0.6261} 10^{0.0012*34.8} 10^{0.0813*7.64}$ =638 CFS 650 CFS -BED MATERIALS: SAND, SILT, GRAVEL -NO UPSTREAM OR DOWNSTREAM STRUCTURES THAT WERE IN PLACE AT THE TIME THIS PROJECT WAS DESIGNED WILL BE ADVERSELY IMPACTED BY THIS CULVERT PROJECT

			<u>SI</u>	TE DAT	<u>A</u>		
Drainage Area	1.27 S	Q. MI.	S	ource		QL	2 LiDAR
River Basin	NEUSE			Character		RESIDEN	NTIAL; AGRICULTURAL
Stream Classification	(Such as	Trout, Hig	gh Qual	ity Water,	etc.)		C; NSW
Data on Existing Struc	ture				2@6′X8′ F	RCBC	
Total Waterway Openin	_			'aterway (	Opening	Below 10	Oyr. WS EL
Debris Potential: Low [ Data on Structures Up		•	U/S			l SR 1173; 0.2' BED T	O CROWN
·			D.0	: 2@8′X8′		N I 40	
6 6 7							N/A
Gage Station No							N/A yrs.  Frequency N/A
istorical Flood Information NOT OT IN HURRICAN Date Elev  NOT OT IN HURRICAN Date Elev  Allowable HW Elev  Manning's n : Left O.E  Flood Study /Status  Flood Study 100 yr. District Hydrological Method  Hydraulic Design Meth	rmation: IE MATTHEft.Est. IE MATTHEft.Est. 175.6 (1.5′ P @ 37 + 3. 0.12  scharge  USG od5.0	W(10/2016) Freq. W(10/2016) Freq. BELOW 12 -Y33- Chan  N/A c  SS SIR 201  HEC-R.	VFLORENG yr. So VFLORENG yr. So SP*) LT (SAG) nel045 N/A .f.s.; WS DES 4–5030 AS 5.0.6	Elev.: Floring REGION (I–5986B	PRAYMODIV. 4 PRIDGE  ft. Nor PO.B. 0 With podway  ATA  4, 34.8%  DRIVIING	MAINT. EN ODWIN - MAINT. SI MAINT. S	BARRIER Period of GINEER Knowledge 28 yrs. DIV 4 Period of JPERVISOR Knowledge 23 yrs.  Surface Elev. 172.4 ft. Ditained From FIELD RECON Didway Established? N/A Without N/A ft.  JS AREA)
SIZE & TYPE: 2@6' X 9	' RCBC	@STATION	l 1940.1,	APPROX. 1	7.4' UPST	REAM OF	CULVERT.
FREQUENCY	Q		Inlet Contro	ol	Outlet	Control	Remarks
	(cfs)	HW/D	H.W.	WSEL	H.W.	WSEL	
10 YR	430	0.64	5.1	169.7	5.9	170.5	OUTLET CONTROL
25 YR	490	0.70	5.6	170.2	6.4	171.0	OUTLET CONTROL
100 YR 500 YR	570 650	0.78	6.2	170.8 171.4	7.0	171.6 172.2	OUTLET CONTROL OUTLET CONTROL
Outlet Velocity (V <sub>10</sub> )  Required Outlet Prote  Design: Discharg  Base Flood: Discharg	7 ction	CLASS I MATION VS EL. Tal c.f.s	f.p.s. RIPRAP (F N TO ken @ s. Fre	Natural BANKS ON BE SHO River Stat quency	Channel ILY) OWN ion 1940 25	ON PLA	Elev. 171.0 ft. Elev. 171.6 ft.
Overtopping: Dischar	ge1350	0 c.f.s	s. Fre	quency	500	).+yr.	Elev. 177.1* ft. *SP @ 37+12 -Y33- LT (SA

## NEUSE RIVER BASIN NON FEMA STUDY AREA CULVERT SURVEY & HYDRAULIC DESIGN REPORT N. C. DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS HYDRAULICS UNIT RALEIGH, N. C. 35 + 91 -Y33-I.D. No. . . I-5986B 47532.1.3 Stru. No. C01 JOHNSTON DRIVING BRANCH County . On Highway SR1219 (N. WALTON AVE.) Between DEAD END 2@6'X9' RCBC W/6" BEVELED HW (BURIED 1' BELOW STREAM BED) 30' SP-SP (NORMAL) Skew 68 DEGREES Recommended Width of Roadway. Recommended Location is (Up, At, (Down) Stream from Existing Crossing. 35.38440 -078.52650 Latitude 占 Statewide Tier 🛛 Sub-Regional Tier BM#7: BENCH TIE IN 17" POPULAR TREE, STA 1391 + 89 -L-, 187' RT Bench Mark is N 595120 E 2141144 Elev. 175.42 ft. Datum: NAVD 88 Temporary Crossing N/A (PROPOSED BOX CULVERT IS DOWNSTREAM OF EXISTING CROSSING) Jostus G Delitor WILL HINES, P.E. 1/23/2020 Assisted by: D. TALBERT, J. HARVEY, M. EDWARDS, F. REESE 2697 I

JOSHUA G. DALTON, P.E.

. Matthew J. York, P.E

SUNGATE DESIGN GROUP, P.A.

905 JONES FINALLINA 27000 NC COA No. C-0890

Project Engineer

Reviewed by:

#### Culvert Undercut Quantities Box Culvert over Driving Branch Creek



#### Portland Concrete Box Culvert (2 @ 6 ft x 9 ft)

Foundation Conditioning Material	
Beneath Culvert Footprint	
Single Culvert Inside Width	6
Number of Culverts	2
Number of Culvert Outside Walls	2
Number of Culvert Internal Divisions	1
Thickness of Walls & Divisions	2.00
Total Culvert(s) Outside Width	14.00
Culvert(s) Length (ft)	62.5
Per Structure Memo, UC to Outer + 4 feet	
UC Outside Width (ft)	18.00
UC depth (ft)	1
volume beneath culvert (ft^3)	1125
volume beneath culvert (cy)	41.67
Reqd. Foundation Conditioning Material for Culvert(s) (tons)	79.3
Reqd. Foundation Conditioning Material for Culvert(s) (tons)	<i>79</i>
Per Structure Memo, 1.904 tons/cy	1.904

Per Structure Memo, do not include FCM quantity for standard turned-back wings



Project: I-95 Widening (I-5986B)

Proj. #: 6235-17-048 Date: 3/6/2020

#### **Embankment & Culvert Calculations for Settle3D**

**Embankment Inputs** 

Slope: 3.0 (H):1(V) Skew: 68 deg
Angle: 17.15 deg. Skewed Slope: 3.24 (H):1(V)

Height: 5 ft (If culvert is not skewed, leave S8 blank & sheet will Top Length: 36.8 ft calculate as if it is perpendicular (90 deg skew))

Base Length: 69.2 ft

#### **Equivalent Culvert & Embankment Load Inputs**

<u>Culvert</u>

Unit Weight: 150 pcf Full Outside Width: 14.0 ft

Number: 2

Inside Width: 6 ft Equivalent Volume: 2291.7 ft^3
Inside Height: 9 ft Equivalent Weight: 343.8 kips

Length: 62.5 ft

Assumed Wall Thickness: 0.67 ft Equiv. Pressure: 0.393 ksf

Embankment

Unit Weight: 120 pcf Equivalent Volume: 2780.4 ft^3
Top Length: 36.8 ft Equivalent Weight: 333.6 kips

Bottom Length: 62.5 ft

Avg. Height: 4 ft Equiv. Pressure: 0.381 ksf

Total Equivalent Pressure: 0.774 ksf

<u>Traffic Loading:</u> 240 psf

0.240 ksf

Width: 30 ft

#### Parameters:

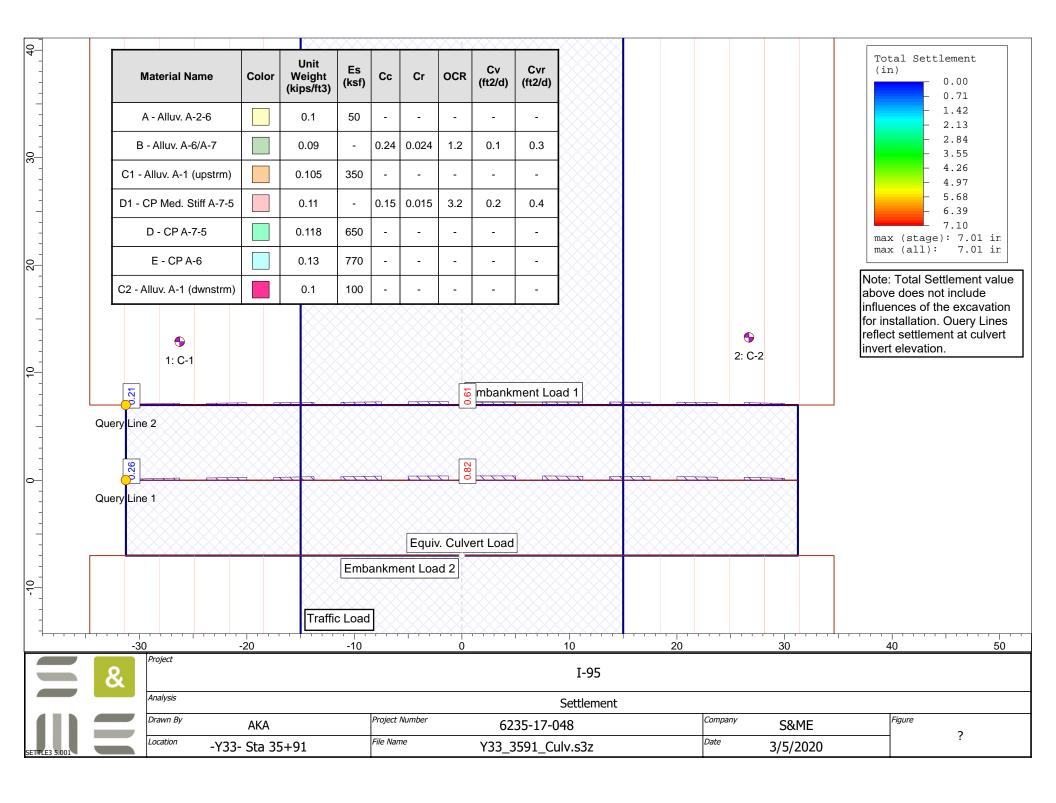
Parameters are based upon soil type, N-value, and lab testing correlations, where applicable. Where lab testings is not applicable, parameters are based on similar soils tested in the project and/or previous knowledge from the geology.

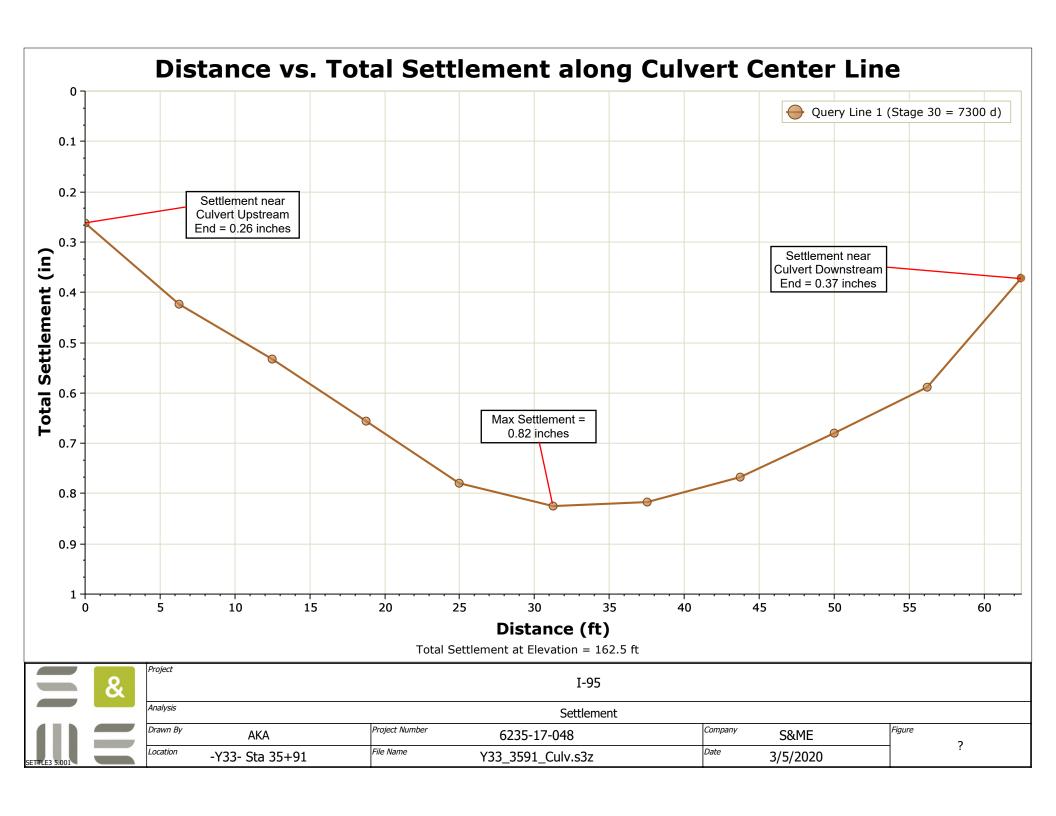
Cc/Cr - Terzaghi & Peck (1967); Cv/Cvr - NAVFAC DM 7.01 Ch. 3 Fig. 4

Es - AASHTO LRFD Table C10.4.6.3-1

Blue Values are inputs

Black Values are calculations/references





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# Settle3 Analysis Information I-95

### **Project Settings**

Document Name Y33\_3591\_Culv.s3z

Project Title I-95
Analysis Settlement
Author AKA
Company S $_{\&}$ ME

Date Created 3/6/2019, 3:30:56 PM

#### Comments

?

6235-17-048 -Y33- Sta 35+91 New Culvert

Stress Computation Method Boussinesq

Time-dependent Consolidation Analysis

Time Units days
Permeability Units feet/day
Minimum settlement ratio for subgrade modulus 0.9

Include buoyancy effect when material settles below water table

Use average properties to calculate layered stresses

Improve consolidation accuracy

Ignore negative effective stresses in settlement calculations

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### Stage Settings

Stage #	Name	Time [days]
1	Stage 1	0
2	Stage 2	1
3	Stage 3	2 3
4	Stage 4	
5	Stage 5	4
6	Stage 6	5
7	Stage 7	6
8	Stage 8	7
9	Stage 9	14
10	Stage 10	21
11	Stage 11	30
12	Stage 12	60
13	Stage 13	90
14	Stage 14	120
15	Stage 15	150
16	Stage 16	180
17	Stage 17	210
18	Stage 18	240
19	Stage 19	270
20	Stage 20	300
21	Stage 21	330
22	Stage 22	365
23	Stage 23	730
24	Stage 24	1095
25	Stage 25	1460
26	Stage 26	1825
27	Stage 27	2920
28	Stage 28	3650
29	Stage 29	5475
30	Stage 30	7300

#### Results

Time taken to compute: 0 seconds

Stage: Stage 1 = 0 d

Data Type	Minimum	Maximum
Total Settlement [in]	0	0
Total Consolidation Settlement [in]	0	0
Virgin Consolidation Settlement [in]	0	0
Recompression Consolidation Settlement [in]	0	0
Immediate Settlement [in]	0	0
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0	0
Loading Stress XX [ksf]	0	0
Loading Stress YY [ksf]	0	0
Effective Stress ZZ [ksf]	0	1.43788
Effective Stress XX [ksf]	0	1.43788
Effective Stress YY [ksf]	0	1.43788
Total Stress ZZ [ksf]	0	3.30364
Total Stress XX [ksf]	0	3.30364
Total Stress YY [ksf]	0	3.30364
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	0
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0
Total Strain	0	0
Pore Water Pressure [ksf]	0	1.86576
Excess Pore Water Pressure [ksf]	0	0
Degree of Consolidation [%]	0	0
Pre-consolidation Stress [ksf]	0.00054	2.21208
Over-consolidation Ratio	1	3.8
Void Ratio	0	0.841
Permeability [ft/d]	0	0.235789
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0

Stage: Stage 2 = 1 d

Data Type	Minimum	Maximum
Total Settlement [in]	0	0.375895
Total Consolidation Settlement [in]	-0.0133037	0
Virgin Consolidation Settlement [in]	0	0
Recompression Consolidation Settlement [in]	-0.0133037	0
Immediate Settlement [in]	0	0.376109
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0	0.774
Loading Stress XX [ksf]	-0.00381059	0.45236
Loading Stress YY [ksf]	0	0.708851
Effective Stress ZZ [ksf]	0	1.43788
Effective Stress XX [ksf]	0	1.45381
Effective Stress YY [ksf]	0	1.45452
Total Stress ZZ [ksf]	0	3.57494
Total Stress XX [ksf]	0	3.57268
Total Stress YY [ksf]	0	3.59666
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	59.8087
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	57.0329
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0
Total Strain	-0.000911934	0.00774
Pore Water Pressure [ksf]	0	2.15895
Excess Pore Water Pressure [ksf]	0	0.774
Degree of Consolidation [%]	0	0
Pre-consolidation Stress [ksf]	0.00054	2.21208
Over-consolidation Ratio	1	3.83599
Void Ratio	0	0.842714
Permeability [ft/d]	0	0.235789
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0

Stage: Stage 3 = 2 d

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Data Type	Minimum	Maximum
Total Settlement [in]	0	0.451371
Total Consolidation Settlement [in]	-0.00142163	0.199712
Virgin Consolidation Settlement [in]	0	0.190957
Recompression Consolidation Settlement [in]	-0.00142163	0.0439599
Immediate Settlement [in]	0	0.407825
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0	0.809594
Loading Stress XX [ksf]	-0.052422	0.532211
Loading Stress YY [ksf]	0.0225721	0.824725
Effective Stress ZZ [ksf]	0	1.37804
Effective Stress XX [ksf]	-0.052422	1.61969
Effective Stress YY [ksf]	0.0225721	1.89891
Total Stress ZZ [ksf]	0	3.64433
Total Stress XX [ksf]	-0.052422	3.75445
Total Stress YY [ksf]	0.0225721	3.81494
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	44.1258
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	52.1269
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	330.087
Total Strain	-0.00111148	0.276194
Pore Water Pressure [ksf]	0	2.55654
Excess Pore Water Pressure [ksf]	0	0.7263
Degree of Consolidation [%]	0	23.102
Pre-consolidation Stress [ksf]	0.00054	2.21208
Over-consolidation Ratio	1	3.84331
Void Ratio	0	0.843089
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	-1.30104e-018	0.0387355

Stage: Stage 4 = 3 d

Data Type	Minimum	Maximum
Total Settlement [in]	0	0.886579
Total Consolidation Settlement [in]	-0.00197142	0.708272
Virgin Consolidation Settlement [in]	0	0.666041
Recompression Consolidation Settlement [in]	-0.00197142	0.0698696
Immediate Settlement [in]	0	0.440664
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	5.58541e-005	0.852317
Loading Stress XX [ksf]	-0.0824862	0.609377
Loading Stress YY [ksf]	0.0469876	0.944868
Effective Stress ZZ [ksf]	5.58541e-005	1.38156
Effective Stress XX [ksf]	-0.0824862	1.72346
Effective Stress YY [ksf]	0.0469876	2.05043
Total Stress ZZ [ksf]	5.58541e-005	3.71055
Total Stress XX [ksf]	-0.0824862	3.93831
Total Stress YY [ksf]	0.0469876	4.02979
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	38.8815
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	48.3304
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	184.762
Total Strain	-0.0012438	0.281503
Pore Water Pressure [ksf]	0	2.62029
Excess Pore Water Pressure [ksf]	0	0.761446
Degree of Consolidation [%]	0	30.4928
Pre-consolidation Stress [ksf]	0.00054	2.21208
Over-consolidation Ratio	1	3.60659
Void Ratio	0	0.843338
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0400098

Stage: Stage 5 = 4 d

Data Type	Minimum	Maximum
Total Settlement [in]	0	1.24064
Total Consolidation Settlement [in]	-0.0014357	0.918477
Virgin Consolidation Settlement [in]	0	0.837593
Recompression Consolidation Settlement [in]	-0.0014357	0.0986428
Immediate Settlement [in]	0	0.473941
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.000273412	0.898506
Loading Stress XX [ksf]	-0.0906459	0.661412
Loading Stress YY [ksf]	0.058274	1.06792
Effective Stress ZZ [ksf]	0.000273412	1.38847
Effective Stress XX [ksf]	-0.0906459	1.80651
Effective Stress YY [ksf]	0.0608746	2.20964
Total Stress ZZ [ksf]	0.000273412	3.7727
Total Stress XX [ksf]	-0.0906459	4.12341
Total Stress YY [ksf]	0.0610746	4.23464
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	34.4088
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	45.4215
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	148.854
Total Strain	-0.00126446	0.282196
Pore Water Pressure [ksf]	0	2.67802
Excess Pore Water Pressure [ksf]	0	0.812898
Degree of Consolidation [%]	0	38.6631
Pre-consolidation Stress [ksf]	0.000725822	2.21208
Over-consolidation Ratio	1	3.4458
Void Ratio	0	0.843376
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0412628

Stage: Stage 6 = 5 d

Data Type	Minimum	Maximum
Total Settlement [in]	0	1.62874
Total Consolidation Settlement [in]	-0.00125618	1.28114
Virgin Consolidation Settlement [in]	0	1.17441
Recompression Consolidation Settlement [in]	-0.00125618	0.114063
Immediate Settlement [in]	0	0.50677
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.000626371	0.948061
Loading Stress XX [ksf]	-0.0796754	0.739658
Loading Stress YY [ksf]	0.0521195	1.19173
Effective Stress ZZ [ksf]	0.000626371	1.39861
Effective Stress XX [ksf]	-0.0790157	1.93747
Effective Stress YY [ksf]	0.0547692	2.37334
Total Stress ZZ [ksf]	0.000626371	3.83102
Total Stress XX [ksf]	-0.0790157	4.30925
Total Stress YY [ksf]	0.0549692	4.42558
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	18.0238
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	43.2099
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	114.448
Total Strain	-0.000948924	0.282339
Pore Water Pressure [ksf]	0	2.72777
Excess Pore Water Pressure [ksf]	0	0.863634
Degree of Consolidation [%]	0	47.7373
Pre-consolidation Stress [ksf]	0.00108046	2.21208
Over-consolidation Ratio	1	3.42145
Void Ratio	0	0.842782
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0565032

Stage: Stage 7 = 6 d

Data Type	Minimum	Maximum
Total Settlement [in]	0	1.928
Total Consolidation Settlement [in]	-0.000925932	1.56141
Virgin Consolidation Settlement [in]	0	1.43732
Recompression Consolidation Settlement [in]	-0.000925932	0.128303
Immediate Settlement [in]	0	0.540567
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00106005	0.997669
Loading Stress XX [ksf]	-0.0933493	0.81937
Loading Stress YY [ksf]	0.0469083	1.31291
Effective Stress ZZ [ksf]	0.00106005	1.41123
Effective Stress XX [ksf]	-0.0926679	2.0905
Effective Stress YY [ksf]	0.0495852	2.53719
Total Stress ZZ [ksf]	0.00106005	3.88379
Total Stress XX [ksf]	-0.0926679	4.49553
Total Stress YY [ksf]	0.0497852	4.59807
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	14.3725
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	40.0077
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0
Total Strain	-0.000691943	0.282335
Pore Water Pressure [ksf]	0	2.76858
Excess Pore Water Pressure [ksf]	0	0.905872
Degree of Consolidation [%]	0	53.9604
Pre-consolidation Stress [ksf]	0.00151567	2.21208
Over-consolidation Ratio	1	3.37271
Void Ratio	0	0.842274
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.058917

Stage: Stage 8 = 7 d

Data Type	Minimum	Maximum
Total Settlement [in]	0	2.19735
Total Consolidation Settlement [in]	-0.000400691	1.78684
Virgin Consolidation Settlement [in]	0	1.64746
Recompression Consolidation Settlement [in]	-0.000400691	0.149148
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	1.42543
Effective Stress XX [ksf]	-0.0762268	2.18101
Effective Stress YY [ksf]	0.0663101	2.61928
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.63068
Total Stress YY [ksf]	0.0665101	4.72433
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	3.23238
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	10.6433
Total Strain	-0.000494192	0.282765
Pore Water Pressure [ksf]	0	2.87109
Excess Pore Water Pressure [ksf]	0	1.05099
Degree of Consolidation [%]	0	55.5245
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.29467
Void Ratio	0	0.84191
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0622389

Stage: Stage 9 = 14 d

Data Type	Minimum	Maximum
Total Settlement [in]	0	4.07088
Total Consolidation Settlement [in]	0	3.39003
Virgin Consolidation Settlement [in]	0	3.18481
Recompression Consolidation Settlement [in]	0	0.242987
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	1.60463
Effective Stress XX [ksf]	-0.0762268	2.35523
Effective Stress YY [ksf]	0.0663101	2.82075
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.62194
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	1.11568
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	1.44988
Total Strain	4.67194e-005	0.282606
Pore Water Pressure [ksf]	-2.31299e-005	2.68822
Excess Pore Water Pressure [ksf]	-0.000200629	0.831203
Degree of Consolidation [%]	0	78.3383
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.0556
Void Ratio	0	0.840873
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	-5.44848e-005	0.0691228

**Stage: Stage 10 = 21 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	4.85416
Total Consolidation Settlement [in]	0	4.17332
Virgin Consolidation Settlement [in]	0	3.94092
Recompression Consolidation Settlement [in]	0	0.272425
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	1.60766
Effective Stress XX [ksf]	-0.0762268	2.35523
Effective Stress YY [ksf]	0.0663101	2.82075
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.6187
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.909318
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	1.16595
Total Strain	-3.62789e-005	0.282514
Pore Water Pressure [ksf]	-2.84238e-005	2.43337
Excess Pore Water Pressure [ksf]	-0.00118287	0.579586
Degree of Consolidation [%]	0	85.4925
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05644
Void Ratio	0	0.841067
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691228

Stage: Stage 11 = 30 d

Data Type	Minimum	Maximum
Total Settlement [in]	0	5.45219
Total Consolidation Settlement [in]	0	4.81728
Virgin Consolidation Settlement [in]	0	4.56568
Recompression Consolidation Settlement [in]	0	0.294969
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	1.82825
Effective Stress XX [ksf]	-0.0762268	2.44357
Effective Stress YY [ksf]	0.0663101	2.82075
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61616
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.822339
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.982844
Total Strain	-0.000134145	0.282428
Pore Water Pressure [ksf]	-3.13943e-005	2.20373
Excess Pore Water Pressure [ksf]	0	0.337968
Degree of Consolidation [%]	0	90.8851
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05713
Void Ratio	0	0.841247
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691228

**Stage: Stage 12 = 60 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	6.38239
Total Consolidation Settlement [in]	0	5.77051
Virgin Consolidation Settlement [in]	0	5.49374
Recompression Consolidation Settlement [in]	0	0.314742
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.12202
Effective Stress XX [ksf]	-0.0762268	2.73382
Effective Stress YY [ksf]	0.0663101	2.82747
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61263
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.699234
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.816282
Total Strain	-0.000263605	0.282272
Pore Water Pressure [ksf]	-3.43938e-005	1.94977
Excess Pore Water Pressure [ksf]	-0.00106	0.12163
Degree of Consolidation [%]	0	98.8333
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05805
Void Ratio	0	0.841485
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691228

Stage: Stage 13 = 90 d

Data Type	Minimum	Maximum
Total Settlement [in]	0	6.75415
Total Consolidation Settlement [in]	0	6.14226
Virgin Consolidation Settlement [in]	0	5.85955
Recompression Consolidation Settlement [in]	0	0.316787
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15017
Effective Stress XX [ksf]	-0.0762268	2.7609
Effective Stress YY [ksf]	0.0663101	2.85455
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61157
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.672658
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.786948
Total Strain	-0.000290392	0.282199
Pore Water Pressure [ksf]	-3.52083e-005	1.88664
Excess Pore Water Pressure [ksf]	-0.00251507	0.054006
Degree of Consolidation [%]	0	99.8677
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05827
Void Ratio	0	0.841535
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691228

**Stage: Stage 14 = 120 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	6.90273
Total Consolidation Settlement [in]	0	6.29084
Virgin Consolidation Settlement [in]	0	6.00675
Recompression Consolidation Settlement [in]	0	0.317028
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15398
Effective Stress XX [ksf]	-0.0762268	2.76436
Effective Stress YY [ksf]	0.0663101	2.858
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61121
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.665111
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.779868
Total Strain	-0.000299798	0.282165
Pore Water Pressure [ksf]	-3.55e-005	1.87095
Excess Pore Water Pressure [ksf]	-0.00562164	0.0231892
Degree of Consolidation [%]	0	99.9845
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05833
Void Ratio	0	0.841552
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691626

**Stage: Stage 15 = 150 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	6.96467
Total Consolidation Settlement [in]	0	6.35278
Virgin Consolidation Settlement [in]	0	6.06834
Recompression Consolidation Settlement [in]	0	0.317066
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15459
Effective Stress XX [ksf]	-0.0762268	2.76484
Effective Stress YY [ksf]	0.0663101	2.85849
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61109
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.662816
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.778034
Total Strain	-0.00030356	0.282149
Pore Water Pressure [ksf]	-3.56276e-005	1.86705
Excess Pore Water Pressure [ksf]	-0.00258401	0.00994969
Degree of Consolidation [%]	0	99.9982
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841559
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691735

**Stage: Stage 16 = 180 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	6.99108
Total Consolidation Settlement [in]	0	6.37919
Virgin Consolidation Settlement [in]	0	6.09466
Recompression Consolidation Settlement [in]	0	0.317076
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.1547
Effective Stress XX [ksf]	-0.0762268	2.76491
Effective Stress YY [ksf]	0.0663101	2.85856
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61104
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.662105
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.77755
Total Strain	-0.00030519	0.28214
Pore Water Pressure [ksf]	-3.80176e-005	1.86608
Excess Pore Water Pressure [ksf]	-0.00561128	0.00421342
Degree of Consolidation [%]	0	99.9999
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841562
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.069175

**Stage: Stage 17 = 210 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00199
Total Consolidation Settlement [in]	0	6.3901
Virgin Consolidation Settlement [in]	0	6.10554
Recompression Consolidation Settlement [in]	0	0.317079
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15473
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61103
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661883
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777422
Total Strain	-0.000306001	0.282136
Pore Water Pressure [ksf]	-3.91476e-005	1.86584
Excess Pore Water Pressure [ksf]	-0.00269724	0.00556153
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841563
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 18 = 240 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00653
Total Consolidation Settlement [in]	0	6.39464
Virgin Consolidation Settlement [in]	0	6.11009
Recompression Consolidation Settlement [in]	0	0.31708
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661814
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777388
Total Strain	-0.000306697	0.282134
Pore Water Pressure [ksf]	-3.96047e-005	1.86578
Excess Pore Water Pressure [ksf]	-0.00538007	0.00270954
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841565
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 19 = 270 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.0085
Total Consolidation Settlement [in]	0	6.39661
Virgin Consolidation Settlement [in]	0	6.11205
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661792
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777379
Total Strain	-0.000307032	0.282133
Pore Water Pressure [ksf]	-3.97897e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.00267732	0.00525408
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841565
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 20 = 300 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00934
Total Consolidation Settlement [in]	0	6.39745
Virgin Consolidation Settlement [in]	0	6.11289
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661786
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777376
Total Strain	-0.000307194	0.282133
Pore Water Pressure [ksf]	-3.98646e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.00510218	0.00236932
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841566
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 21 = 330 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.0097
Total Consolidation Settlement [in]	0	6.39782
Virgin Consolidation Settlement [in]	0	6.11325
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661783
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777376
Total Strain	-0.000307272	0.282133
Pore Water Pressure [ksf]	-3.98949e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.00264463	0.00496218
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841566
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 22 = 365 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00988
Total Consolidation Settlement [in]	0	6.39799
Virgin Consolidation Settlement [in]	0	6.11342
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661783
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777375
Total Strain	-0.000307314	0.282133
Pore Water Pressure [ksf]	-3.99081e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.0048043	0.00227044
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841566
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 23 = 730 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00998
Total Consolidation Settlement [in]	0	6.39809
Virgin Consolidation Settlement [in]	0	6.11352
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661782
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777375
Total Strain	-0.000307346	0.282133
Pore Water Pressure [ksf]	-3.99155e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.00436792	0.00212855
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841566
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 24 = 1095 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00998
Total Consolidation Settlement [in]	0	6.39809
Virgin Consolidation Settlement [in]	0	6.11352
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661782
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777375
Total Strain	-0.000307346	0.282133
Pore Water Pressure [ksf]	-3.99155e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.00257931	0.00428378
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841566
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 25 = 1460 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00998
Total Consolidation Settlement [in]	0	6.39809
Virgin Consolidation Settlement [in]	0	6.11352
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661782
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777375
Total Strain	-0.000307346	0.282133
Pore Water Pressure [ksf]	-3.99155e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.00424207	0.00211264
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841566
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 26 = 1825 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00997
Total Consolidation Settlement [in]	0	6.39808
Virgin Consolidation Settlement [in]	0	6.11352
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661782
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777375
Total Strain	-0.000307346	0.282133
Pore Water Pressure [ksf]	-3.99155e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.00210818	0.00422129
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841566
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 27 = 2920 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00997
Total Consolidation Settlement [in]	0	6.39808
Virgin Consolidation Settlement [in]	0	6.11352
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661782
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777375
Total Strain	-0.000307346	0.282133
Pore Water Pressure [ksf]	-3.99155e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.0041866	0.0025737
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841566
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 28 = 3650 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00998
Total Consolidation Settlement [in]	0	6.39809
Virgin Consolidation Settlement [in]	0	6.11352
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661782
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777375
Total Strain	-0.000307346	0.282133
Pore Water Pressure [ksf]	-3.99155e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.00418063	0.00209615
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841566
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

**Stage: Stage 29 = 5475 d** 

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00997
Total Consolidation Settlement [in]	0	6.39808
Virgin Consolidation Settlement [in]	0	6.11352
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661782
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777375
Total Strain	-0.000307346	0.282133
Pore Water Pressure [ksf]	-3.99155e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.00209073	0.00417003
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841566
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

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## Stage: Stage 30 = 7300 d

Data Type	Minimum	Maximum
Total Settlement [in]	0	7.00998
Total Consolidation Settlement [in]	0	6.39809
Virgin Consolidation Settlement [in]	0	6.11352
Recompression Consolidation Settlement [in]	0	0.317081
Immediate Settlement [in]	0	0.714042
Secondary Settlement [in]	0	0
Loading Stress ZZ [ksf]	0.00232683	1.18714
Loading Stress XX [ksf]	-0.0768665	0.877028
Loading Stress YY [ksf]	0.0636749	1.35085
Effective Stress ZZ [ksf]	0.00232683	2.15474
Effective Stress XX [ksf]	-0.0762268	2.76492
Effective Stress YY [ksf]	0.0663101	2.85857
Total Stress ZZ [ksf]	0.00232683	4.00084
Total Stress XX [ksf]	-0.0762268	4.61102
Total Stress YY [ksf]	0.0665101	4.71811
Modulus of Subgrade Reaction (Total) [ksf/ft]	0	0.661782
Modulus of Subgrade Reaction (Immediate) [ksf/ft]	0	38.7603
Modulus of Subgrade Reaction (Consolidation) [ksf/ft]	0	0.777375
Total Strain	-0.000307346	0.282133
Pore Water Pressure [ksf]	-3.99155e-005	1.86576
Excess Pore Water Pressure [ksf]	-0.00257012	0.00416455
Degree of Consolidation [%]	0	100
Pre-consolidation Stress [ksf]	0.00278597	2.21208
Over-consolidation Ratio	1	3.05834
Void Ratio	0	0.841566
Permeability [ft/d]	0	0.785962
Coefficient of Consolidation [ft^2/d]	0	0.4
Hydroconsolidation Settlement [in]	0	0
Average Degree of Consolidation [%]	0	0
Undrained Shear Strength	0	0.0691753

## Loads

## 1. Rectangular Load: "Traffic Load"

Length 30 ft
Width 300 ft
Rotation angle 0 degrees
Load Type Flexible
Area of Load 9000 ft²
Load 0.24 ksf
Depth -177.5 ft
Installation Stage 8 = 7 d

## Coordinates

Y [ft]
-150
-150
150
150

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## 2. Rectangular Load: "Equiv. Culvert Load"

Length 62.5 ft
Width 14 ft
Rotation angle 0 degrees
Load Type Flexible
Area of Load 875 ft²
Load 0.774 ksf
Depth -163.5 ft
Installation Stage 2 = 1 d

#### Coordinates

X [ft]	Y [ft]
-31.25	-7
31.25	-7
31.25	7
-31.25	7

## **Embankments**

#### 1. Embankment: "Embankment Load 1"

Label Embankment Load 1
Center Line (0, 7) to (0, 149)
Near End Angle 90 degrees
Far End Angle 90 degrees

Number of Layers 5 Base Width 69.2

Layer	Stage	Left Bench Width (ft)	Left Angle (deg)	Height (ft)	Unit Weight (kips/ft <sup>3</sup> )	Right Angle (deg)	Right Bench Width (ft)
1	Stage 3 = 2 d	0	17.17	1	0.12	17.17	0
2	Stage 4 = 3 d	0	17.17	1	0.12	17.17	0
3	Stage 5 = 4 d	0	17.17	1	0.12	17.17	0
4	Stage 6 = 5 d	0	17.17	1	0.12	17.17	0
5	Stage 7 = 6 d	0	17.17	1	0.12	17.17	0

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## 2. Embankment: "Embankment Load 2"

Label Embankment Load 2 Center Line (0, -7) to (0, -149)

Near End Angle 90 degrees Far End Angle 90 degrees

Number of Layers 5 Base Width 69.2

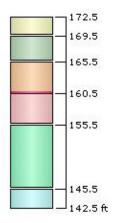
Layer	Stage	Left Bench Width (ft)	Left Angle (deg)	Height (ft)	Unit Weight (kips/ft <sup>3</sup> )	Right Angle (deg)	Right Bench Width (ft)
1	Stage 3 = 2 d	0	17.17	1	0.12	17.17	0
2	Stage 4 = 3 d	0	17.17	1	0.12	17.17	0
3	Stage 5 = 4 d	0	17.17	1	0.12	17.17	0
4	Stage 6 = 5 d	0	17.17	1	0.12	17.17	0
5	Stage 7 = 6 d	0	17.17	1	0.12	17.17	0

## Soil Layers

Ground Surface Drained: Yes

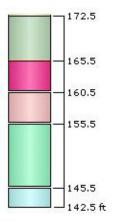
## C-1: (-26.235, 12.88)

Layer #	Туре	Thickness [ft]	Depth [ft]	Drained at Bottom
1	A - Alluv. A-2-6	3	-172.5	No
2	B - Alluv. A-6/A-7	4	-169.5	Yes
3	C1 - Alluv. A-1 (upstrm)	5	-165.5	No
4	C2 - Alluv. A-1 (dwnstrm)	0	-160.5	No
5	D1 - CP Med. Stiff A-7-5	5	-160.5	No
6	D - CP A-7-5	10	-155.5	No
7	E - CP A-6	3	-145.5	No



## C-2: (26.711, 13.27)

Layer #	Туре	Thickness [ft]	Depth [ft]	Drained at Bottom
1	A - Alluv. A-2-6	0	-172.5	No
2	B - Alluv. A-6/A-7	7	-172.5	Yes
3	C1 - Alluv. A-1 (upstrm)	0	-165.5	No
4	C2 - Alluv. A-1 (dwnstrm)	5	-165.5	No
5	D1 - CP Med. Stiff A-7-5	5	-160.5	No
6	D - CP A-7-5	10	-155.5	No
7	E - CP A-6	3	-145.5	No



## **Soil Properties**

Property	A - Alluv. A-2-6	B - Alluv. A-6/A-7	C1 - Alluv. A-1 (upstrm)	D1 - CP Med. Stiff A-7-5
Color				
Unit Weight [kips/ft <sup>3</sup> ]	0.1	0.09	0.105	0.11
Saturated Unit Weight [kips/ft <sup>3</sup> ]	0.1	0.09	0.105	0.11
К0	1	1	1	1
Immediate Settlement	Enabled	Disabled	Enabled	Disabled
Es [ksf]	50	-	350	-
Esur [ksf]	50	-	350	-
Primary Consolidation	Disabled	Enabled	Disabled	Enabled
Material Type		Non-Linear		Non-Linear
Cc	-	0.24	-	0.15
Cr	-	0.024	-	0.015
e0	-	0.841	-	0.7805
OCR	-	1.2	-	3.2
Cv [ft <sup>2</sup> /d]	-	0.1	-	0.2
Cvr [ft <sup>2</sup> /d]	-	0.3	-	0.4
B-bar	-	1	-	1
Undrained Su A [kips/ft2]	0	0	0	0
Undrained Su S	0.2	0.2	0.2	0.2
Undrained Su m	0.8	0.8	0.8	0.8
Piezo Line ID	1	1	1	1

Property	D - CP A-7-5	E - CP A-6	C2 - Alluv. A-1 (dwnstrm)
Color			
Unit Weight [kips/ft <sup>3</sup> ]	0.118	0.13	0.1
Saturated Unit Weight [kips/ft <sup>3</sup> ]	0.118	0.13	0.1
K0	1	1	1
Immediate Settlement	Enabled	Enabled	Enabled
Es [ksf]	650	770	100
Esur [ksf]	650	770	100
B-bar	-	-	-
Undrained Su A [kips/ft2]	0	0	0
Undrained Su S	0.2	0.2	0.2
Undrained Su m	0.8	0.8	0.8
Piezo Line ID	1	1	1

## **Groundwater**

Groundwater method Piezometric Lines Water Unit Weight 0.0624 kips/ft<sup>3</sup>

## **Piezometric Line Entities**

**ID Depth (ft)** 1 -172.4 ft

## **Query Lines**

Line #	<b>Query Line Name</b>	<b>Start Location</b>	<b>End Location</b>	<b>Horizontal Divisions</b>	<b>Vertical Divisions</b>
1	Query Line 1	-31.25, 0	31.25, 0	10	Auto: 77
2	Query Line 2	-31.25, 7	31.25, 7	10	Auto: 77

 5986B REFERENCE **CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

CROSS SECTION(S) BORE LOG(S) SITE PHOTOGRAPH(S)

TITLE SHEET

SITE PLAN

PROFILE(S)

SHEET NO.

5-6 7-II

4

## STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY JOHNSTON

PROJECT DESCRIPTION <u>I-95 FROM NORTH OF SR</u> 1002 (LONG BRANCH ROAD) (EXIT 71) TO I-40 (EXIT 81).

SITE DESCRIPTION BRIDGE 653 ON -Y29- (S. MARKET ST.) OVER -L- (I-95)

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
V.C.	I-5986B	1	12

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDING TO CLIMATIC CONDITIONS INCLORDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

E. BLONESHINE J. PREVATTE A. BLYTHE J. SWARTLEY INVESTIGATED BY \_\_\_\_\_S&ME, Inc. CHECKED BY S. MITCHELL

SUBMITTED BY S. MITCHELL DATE JANUARY 2020



9751 SOUTHERN PINE BLVD CHARLOTTE, NC 28273



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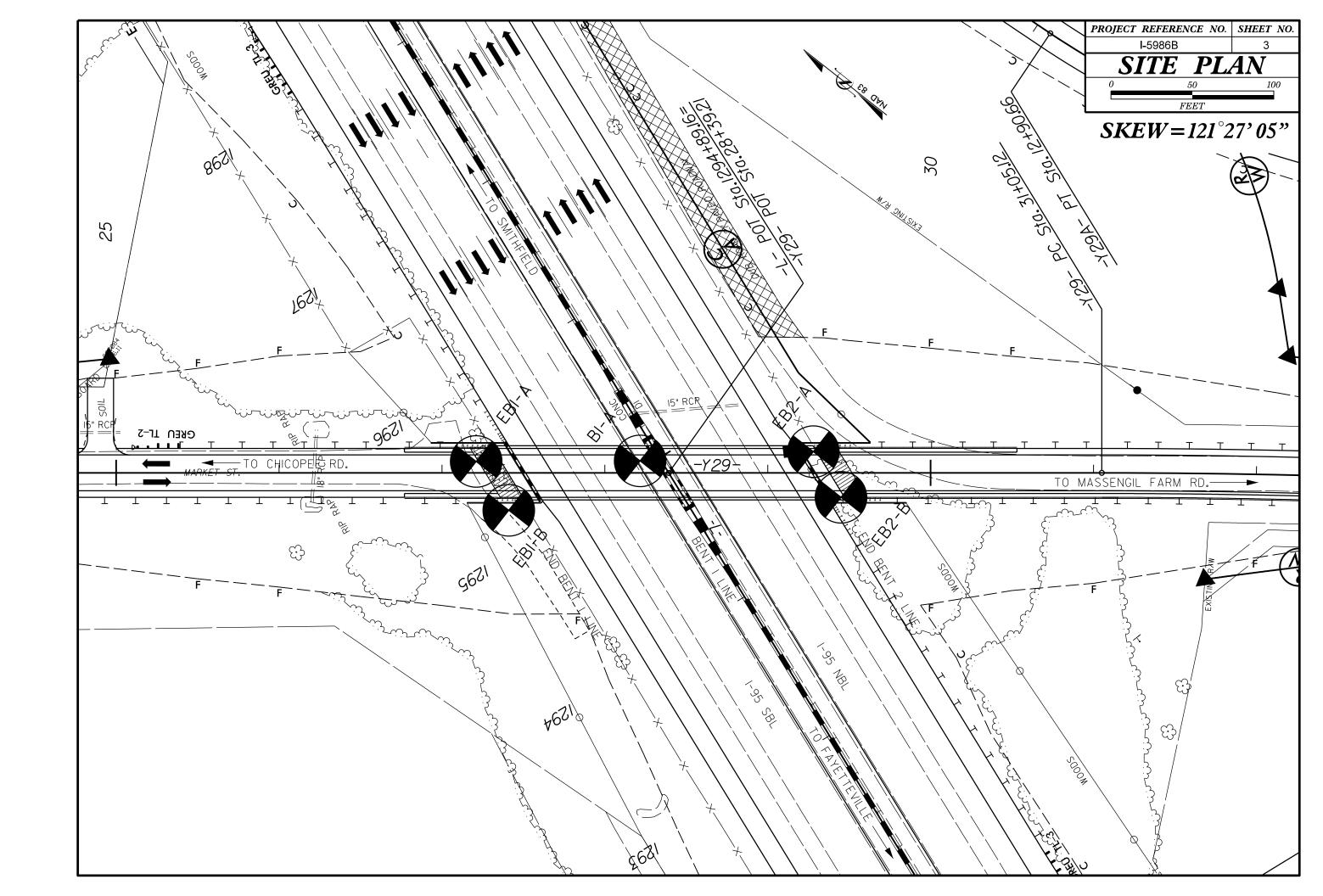
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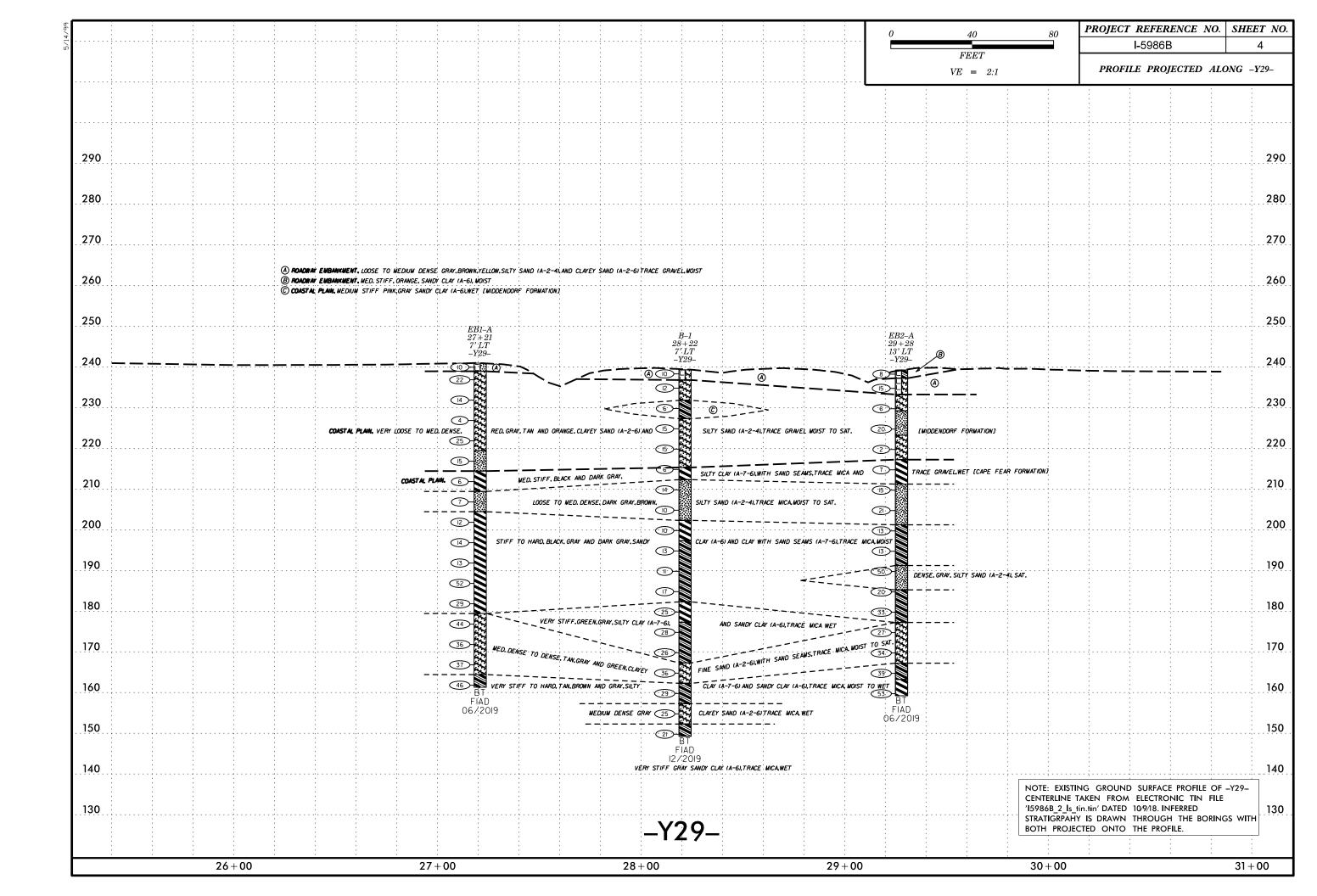
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

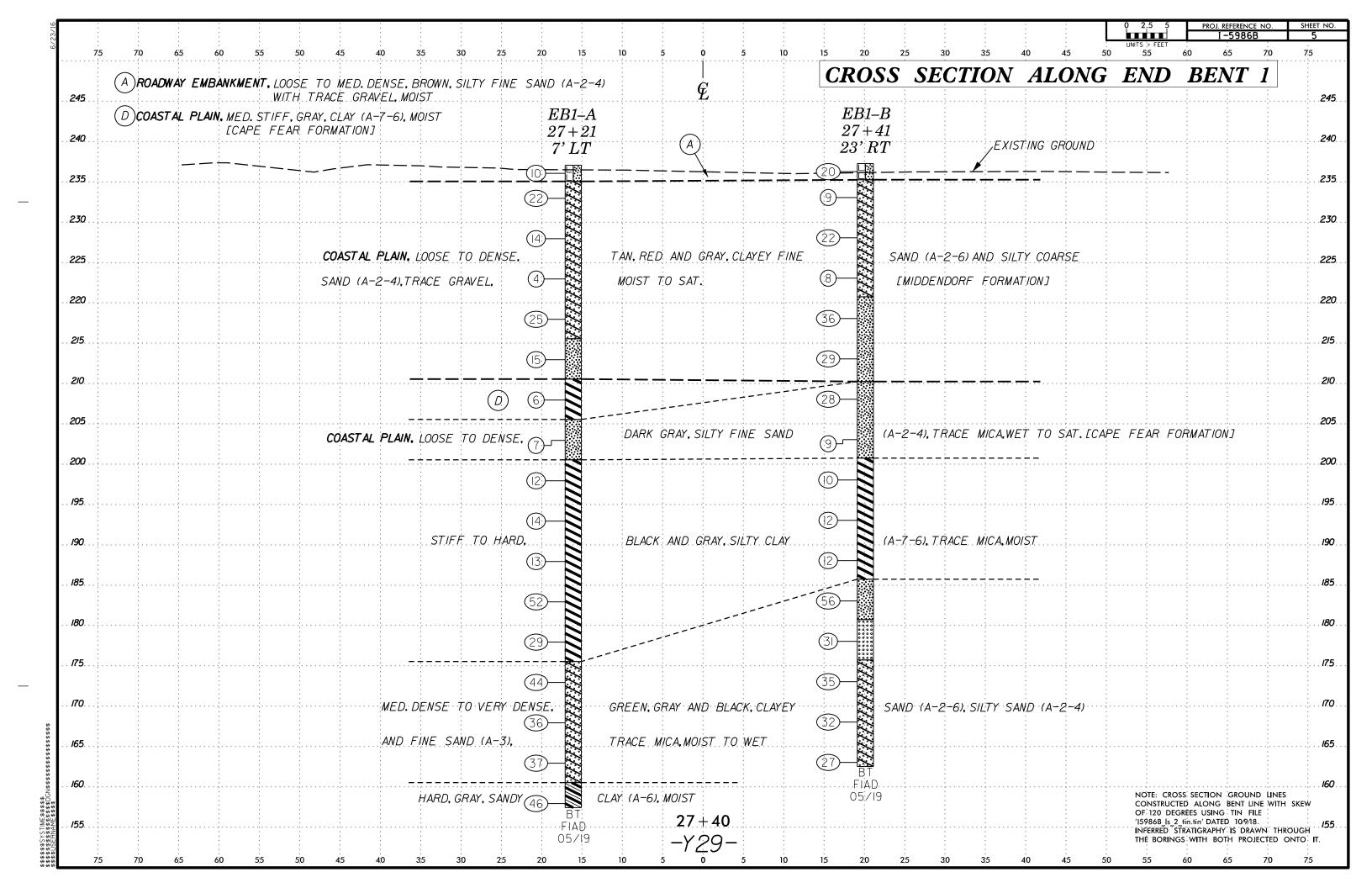
## SUBSURFACE INVESTIGATION

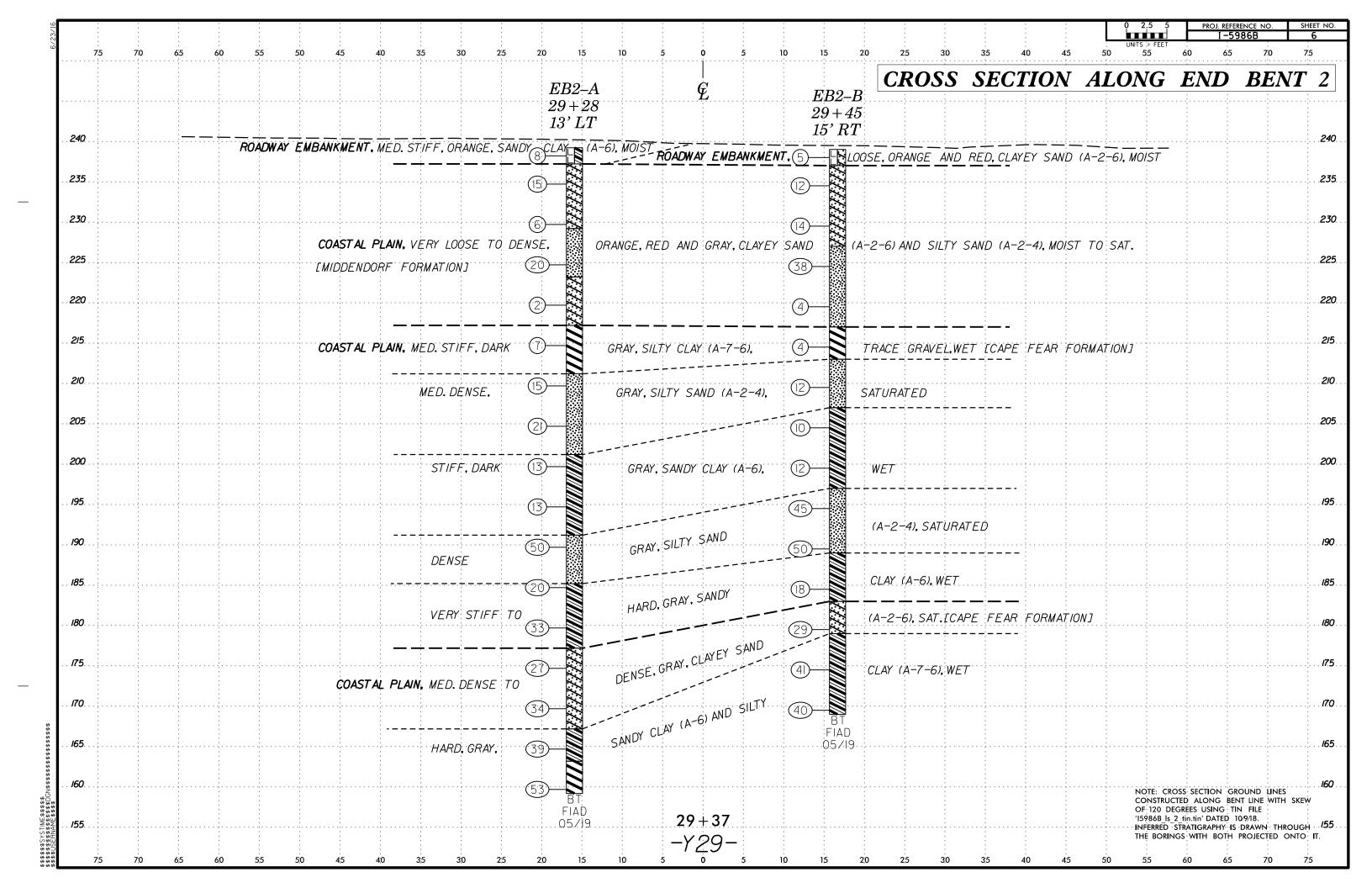
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

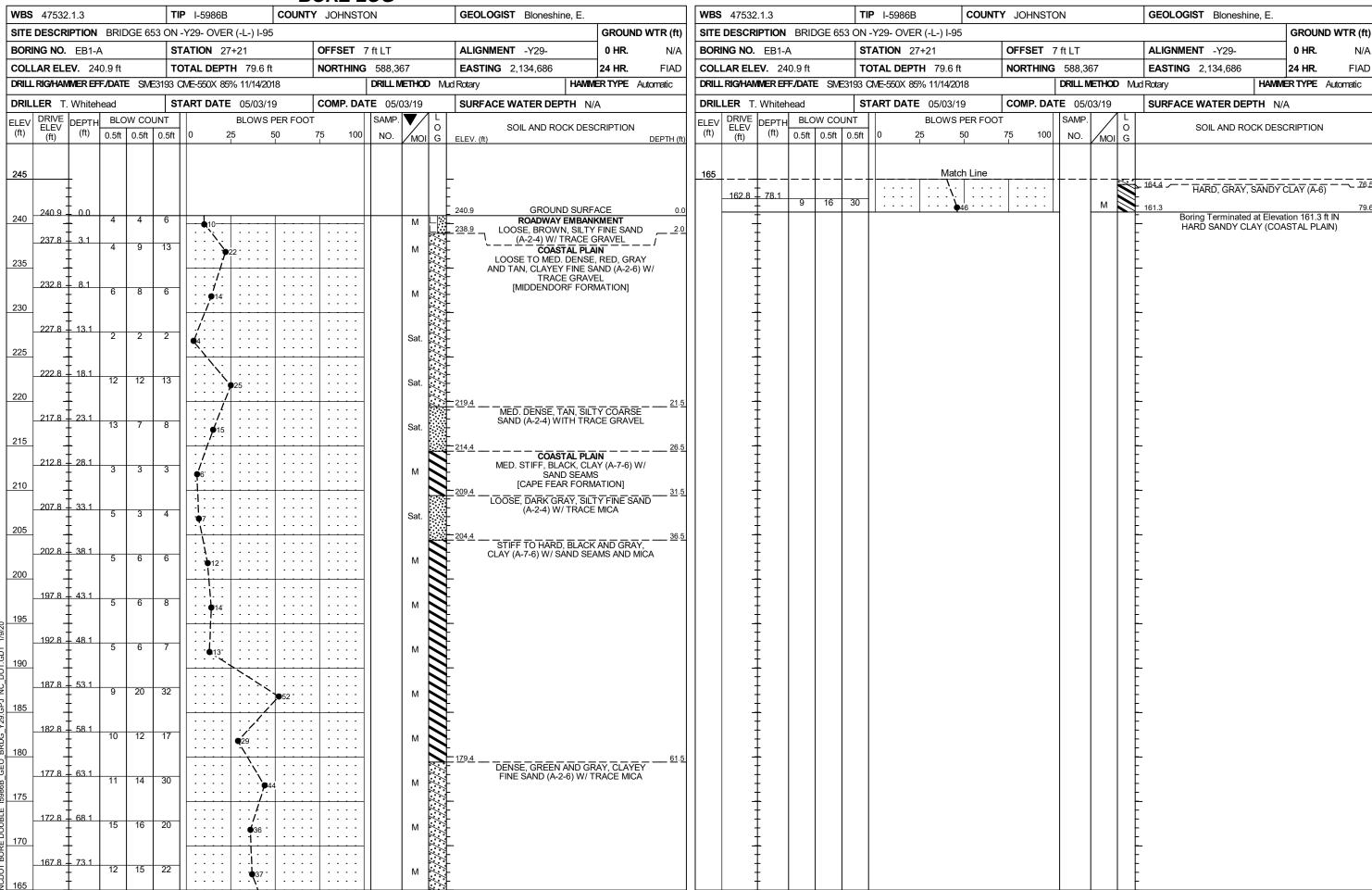
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF.GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
CENERAL CRANIII AR MATERIALS SILT-CLAY MATERIALS	MINERALOGICAL COMPOSITION	FINE TO COARSE CRAIN ICNEOUS AND METAMORPHIC ROCK THAT	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	WOULD YIELD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE,	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	UNEISS, OHOBRU, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-6 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-7-6	COMPRESSIBILITY	NON-CRYSTALLINE ROCK (NCR)  SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 0000 d000000	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
% PASSING SUIT-	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED (CP) SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR CLAY MUCK, *40 30 MX 50 MX 51 MN SOILS SOILS SOILS PEAT	PERCENTAGE OF MATERIAL	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*200   15 MX   25 MX   10 MX   35 MX   35 MX   35 MX   35 MX   36 MN   36 MN   36 MN   36 MN   36 MN   36 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%  LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40 48 MX 41 MN	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN 11 MN 11 MN HIGHLY	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOUS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
OF MAJOR GRAYEL, AND MATERIALS SAND GRAYEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER <u>24</u> HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
CEN PATING		(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	PARENT MATERIAL.
AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR POOR UNSUITABLE	SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI 0F A-7-5 SUBGROUP IS ≤ LL - 30 ;PI 0F A-7-6 SUBGROUP IS > LL - 30	-	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
CONSISTENCY CONSISTENCY (N-VALUE) (TONS/FT <sup>2</sup> )	WITH SOIL DESCRIPTION → OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE	SOIL SYMBOL  SOIL SYMBOL  SUPPLINT TEST BORING  SLOPE INDICATOR INSTALLATION	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIAL MEDIUM DENSE 10 TO 30 N/A	NT - STATE	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) DENSE 30 TO 50  VERY DENSE > 50	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT  AUGER BORING  CONE PENETROMETER TEST	VERY  ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE  SEVERE  BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25	— — INFERRED SOIL BOUNDARY — CORE BORING SOUNDING ROD	(V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5	TEST BORING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY   MEDIUM STIFF   4 TO 8   0.5 TO 1.0   MATERIAL   STIFF   8 TO 15   1 TO 2	INFERRED ROCK LINE "MONITORING WELL WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4	→ → → → → → → ALLUVIAL SOIL BOUNDARY △ PIEZOMETER INSTALLATION → SPT N-VALUE	ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
HARD > 30 > 4  TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNSUITABLE WASTE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
COARSE FINE	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SAND (SL.) (CL.)		MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(CSE, SD.) (F SD.) (GE.)	ABBRE VIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SOIL MOISTURE - CORRELATION OF TERMS	CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOU MOISTURE SCALE FIELD MOISTURE	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{ m d}$ - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS)  DESCRIPTION  GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u>	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
(SAT.) FROM BELOW THE GROUND WATER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES I INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID; REQUIRES DRYING TO	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RAINGE - WEI - (W) ATTAIN OPTIMUM MOISTURE	FRAGS FRAGMENTS $w$ - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: BY6-202 N: 588609 E: 2134438
" 1' PL L PLASTIC LIMIT	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS  VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT  DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: 241.76 FEET
SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:  CME-45C X CLAY BITS X AUTOMATIC MANUAL	MODERATELY CLOSE	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO	CI CONTINUOUS ELICUT AUSED	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	FIAD = FILLED IMMEDIATELY AFTER DRILLING
ATTAIN UPTIMUM MUISTURE	LURE SIZE:	THINLY LAMINATED < 0.008 FEET	
PLASTICITY	X 8* HOLLOW AUGERS	INDURATION	
PLASTICITY INDEX (PI)  DRY STRENGTH	X CME-550   HARD FACED FINGER BITS   -N	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.  RUBBING WITH FINGER FREES NUMEROUS GRAINS;	
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST TUNGCARBIDE INSERTS	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM	X CASING W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	X CME-750 TRICONE TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT VANE SHEAR TEST		
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED  SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;  SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1-
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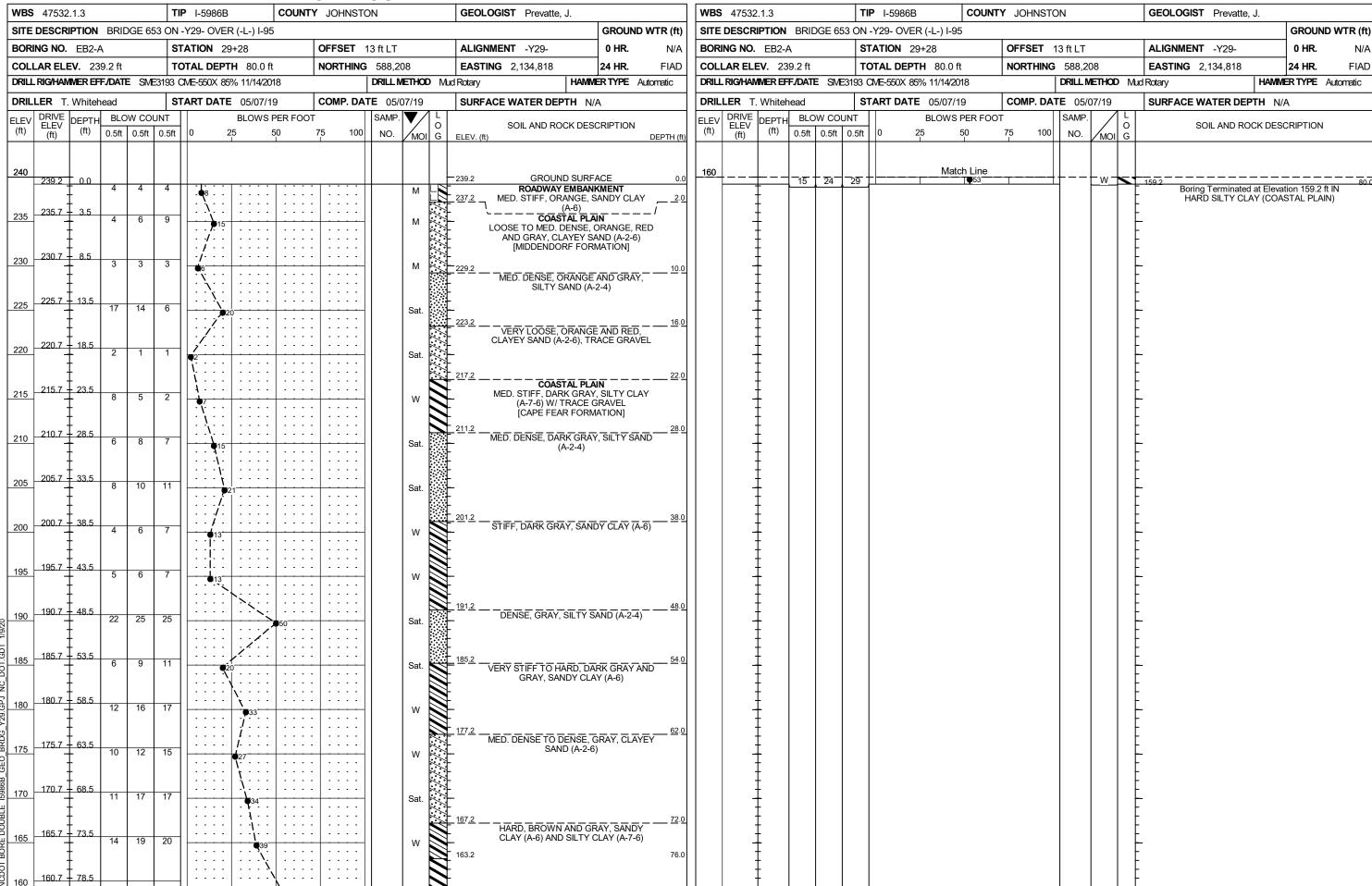






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223	200	222.0	‡ , ,				½ .				7		<b>**</b>	- CLAYE` · [MIDDEN	r FINE SAND ( IDORF FORM	(A-2-6) IATION]			‡										<del>-</del> -		
225 279 322 6 8 11 W 244 MD TOSCTOCUSE TAILSTY 1944 OCASE PARIT (A.S.) TRACE PARIT (A.S.) TRACE PARIT (A.S.) TRACE PARIT		232.9	+ 8.2	9	11	11	:::;	22			-	w	<b>/</b> //						‡												
277.6 - 13.2	230	_	‡				/-				_		<b>**</b>	• <del>-</del>					‡										- -		
255 2224 152 0 10 10 10 10 10 10 10 10 10 10 10 10 1		227.9	13.2	1	1	1	::/::				:		<b>*</b>	• •					‡										• •		
222	225		‡	4	4	4						W		•					‡										- -		
229 182	225	-	‡								-			MED. DENSE	TO DENSE.	TAN, SILTY	16.5		‡										<del>-</del> ·		
201 215 22 3 32 2 3 14 14 2		222.9	<u>  18.2</u> 		19	17		36			:	l w		COARSE SAN	D (A-2-4), TRA	ACE GRAVEL			‡										• •		
210	220	_	‡					· f · ·						<del>-</del>					‡										<u>-</u>		
215 20 20 20 20 20 20 20 20 20 20 20 20 20		217.9	23.2				::::	<i>;j</i> : : :			:			• •					‡										- -		
210 202 20 8 15 14 14 50			‡	8	18	11		<b>4</b> 29			1 1	w		• •					‡										- -		
210	215	-	<u>†</u>				<del> </del>	<del>   </del>						214.1			27.0		+										<del>-</del>		
200 201 302 2 5 5 5 7		212.9	28.2	8	14	14						l w		LOOSE TO M	ED. DENSE, D	DARK GRAY,			Ŧ										- -		
205 202 4 38 2 2 5 5 4 5 7 4 12 1 105 105 105 105 105 105 105 105 105 1	210		Ŧ					28			1 1	**							Ŧ										-		
205		207.0	1 33 2					1			-								Ŧ									F	•		
20.9 39.2 2 5 5 7		201.3	T 33.2	5	4	5	9				-	w		•					Ŧ										- -		
TRACE MICA  TRACE	205	-	Ŧ				-	<b>.</b>	<b></b>					- <u>204.6</u>	azar <del>a</del> z a :	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	<u>36.5</u>		Ŧ										- <del>-</del>		
200   197.9   43.2   5   5   7   192.9   48.2   4   5   7   193.0   187.9   53.2   19   29   27   27   28.2   27   28.2		202.9	38.2	1 2	5	5	::;:::				-	,,				AY (A-1-6),			Ŧ										•		
197.9 43.2 5 5 7 12.2 MM  199.9 48.2 4 5 7 12.2 MM  199.9 53.2 19 29 27	200		Ŧ				10 -			<b>I</b>	1 1	M		• •					‡										- -		
196   5 5 7   192   182   182   182   183   184		107.0	Ŧ 40.0				- 1	1			-								Ŧ										<del>-</del> ·		
192 9 48 2 4 5 7 192 9 19 29 27		197.9	+ 43.2 +	5	5	7					:	М		· ·					‡										- -		
192.9 48.2 4 5 7 12 2	195	_	‡					<b></b>						<del>-</del>					‡										- <del>-</del>		
190	6	192.9	48.2	1		1 7	:: ::				:			•					‡										• •		
187.9 53.2 19 29 27	100		‡	4		'	12	.[::::			:	M		• •					‡										<del>.</del> -		
187.9 53.2 19 29 27	3	40= -	‡					7.5.			-			<u>– 189.6</u> DENSE TO VE	RY DENSE, B	BLACK, SILTY	<u>51.5</u>		‡										<del>-</del> ·		
185	ار	187.9	+ 53.2 +	19	29	27	::::		<b>●</b> 56		:	w		COARSE SAN	D (A-2-4) AND	D FINE SAND			‡										- -		
177.9 68.2 10 14 18 932 MED. DENSE TO DENSE, GREEN AND GRAY, CLAYEY FINE SAND (A-2-6), TRACE MICA  177.9 68.2 10 14 18 932 MED. DENSE TO DENSE, GREEN AND GRAY, CLAYEY FINE SAND (A-2-6), TRACE MICA  M M 167.9 73.2 10 12 15 027 MM 166.4 74.7	185		‡					ļ · · · ·			_			- <del>-</del> 184.6	. ,		56.5		‡										- -		
180	7.	182.9	58.2	<u> </u>	144	17	::::	: //. :			:		0000	• •					‡										• •		
177.9 63.2 12 15 20	100		‡	9	14	1/	::::	<b>9</b> 31 · ·				W	0000	•					‡										<u> </u>		
175	180	-	‡					<del>-1</del>						<u> 179.6</u> <u> </u>	TO DENSE.	GREEN AND	61.5		‡										<del>-</del> •		
175	2	177.9	63.2	12	15	20	::::	35			:	М		GRAY, CLA	YEY FINE SAN	ND (A-2-6),			‡										- -		
172.9 68.2	175	_	‡					. T°°.			-		<b>//</b>	<del>-</del>	J IOL WIOA				‡										- <del>-</del>		
10 14 18		172.9	68.2				: : : :	-			:		//	• •					‡										• •		
167.9 73.2			±	10	14	18	: : : :	932		-	<u>-                                     </u>	М	<b>/</b> //	•					1									[	- -		
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	170	-	±					1					<b>//</b>	<del>-</del>					$\pm$										_		
1 V21 11 V21 100.4 14.1	<u> </u>	167.9	73.2		12	15		<u></u>			:	N4	<b>*</b> /-				74 7		Ī									F			
	2 2		<u> </u>					<u>V</u> 21 - · ·				171			ated at Elevation	ion 166.4 ft IN	14.1		<u> </u>									$oldsymbol{ol}}}}}}}}}}}}}}}}$			

<b>WBS</b> 4753	22 1 2			Π.	TID I	-5986B			TY JOHNS			GEO	LOGIST Goslin, G	<u> </u>		WRS	47532	0 1 2			TIE	P I-5986B		COLINTY	/ JOHNST	ON			GEOLOGIST Goslin		
SITE DESC			IDGE 6						TT JOHNS	JION .		GLO	LOGIST GOSIIII, O	GROUND	M/TD (ft)	l			BDIDG	2E 653		-Y29- OVER		COUNT	JOHNST	ON			GLOLOGIST GOSIII	<u> </u>	ND WTR (ft)
BORING NO			IDOL (			ON 28			OFFSET	7 #   T		ALIG	NMENT -Y29-	0 HR.	3.5	l	ING NO.		DIVIDO	JL 000		TATION 28+	` '		OFFSET	7 ft l T			ALIGNMENT -Y29-	0 HR.	` ′
			4					<b></b>			<u> </u>	_				l -			0 0 #		_						00				
DRILL RIG/H/							H 90.0		NORTHII	NG 588,288		Mud Rotary	<b>FING</b> 2,134,748	24 HR. HAMMER TYPE	FIAD		DIC/LIAN			CI /IEO		OTAL DEPTH OME-750 74%			NORTHING	DRILL N			EASTING 2,134,74	3 24 HR. HAMMER TYPE	
			IIE SI						1						AUIOTAIIC					SIVES											Automatic
DRILLER  ELEV DRIVE	- 1	T	0111 00		SIAR	IDAIE	12/09/			DATE 12/10		SURF	FACE WATER DEF	TH N/A			LER R			N 0011		ART DATE			COMP. DA			1	SURFACE WATER I	DEPTH N/A	
ELEV DRIVE (ft) ELEV (ft)	/	111	OW CO		t 0	2	25 	50			MOI G			CK DESCRIPTION	DEPTH (ft	(ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft (			0 25	BLOWS PI		75 100	SAMP.	MOI	O G	SOIL AND	ROCK DESCRIPTION	N
240	3 0.0					_						-239.3		ID SURFACE	0.0	160				-		-	Match	Line	T	 	  -w-		VERY STIFF	AN, GRAY, SANDY (	CLAY
	Ī	3	4	6		10		.			М	I°3I		EMBANKMENT BROWN, CLAYEY F	INE 2.5			Ī					j 						(A-6) TR	ACE MICA (continued)	)
235.8	3 + 3.5	5 4	6	6	┨┟	.l _ <b></b>				-	w		1	BROWN, CLAYEY F ID (A-2-6) W/ TRACE RAVEL	=	155	155.8	83.5	6	12	13	· · · · · j	25				l w		SAND (A	SE, GRAY, CLAYEY 1-2-6), TRACE MICA	FINE
	1					7:::		.		:	<i>***</i>		MEDIUM DENSE,	<b>TAL PLAIN</b> RED, GRAY, CLAYE	Υ			<u> </u>				: : : : <b>]</b>					''	<del>\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</del>	450.0		07.0
230.8	3 + 8.5	5			-	/: : :		.			<i>**</i> :	231.8	_ FINE TO COA	RSE SAND (A-2-6) RF FORMATION]	7.5		150.8	- 88.5				::::/							152.3VERY STIFF, (	RAY, SANDY CLAY	(A-6), 87.0
230	Ŧ	2	3	3	$\exists \vdash$	6	<u> </u>				w	<b>}</b>	MEDIUM STIFF, I	PINK GRAY, COĀRS	<u>-</u> '	150			10	9	12	21			<u> </u>	Ц	W		149.3	IVACE IVIICA	90.0
	Ī					<b>/</b> · · ·		.		-		227.3		CLAY (A-6)	12.0			<u> </u>										E		ited at Elevation 149.3 SANDY CLAY (COAS	
225.8	3 13.	.5	7	8	$\frac{1}{2}$	<u>\</u>	<u> </u>				W	<del>\</del>	MEDIUM DENSE, T FINE TO COA	AN, YELLOW, CLAY	r⊨Y			<u>†</u>										E		PLAIN)	
	Ŧ					<b>▼</b> 15				:]	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,						-	1										E			
220.9	I 18.	5				111	: : : :	.			<i>`</i> ,							<del>-</del>										l F			
220	<del> </del>	3	1	14	٦Ŀ	15					W 🔆						_	<del>-</del>										ΙF			
	Ŧ					7	: : : :	.			<i>/</i> /,							Ŧ										l F			
215.8	3 <del> </del> 23.	.5 5	4	2	415	/	: : : :	.		-	\\\\	215.3			24.0			Ŧ										F			
	Ŧ	ľ	'	-		<b>/</b> ····				-	W	5	MED. STIFF, DAF	T <b>AL PLAIN</b> RK GRAY, SILTY CLA	ΑY			Ŧ										F			
	, ‡ <u>.</u> .	_				/ : :		:   : : :		-		212.3	7 (A-7-6) W/ SAND CAPE FEA	SEAMS, TRACE MIC R FORMATION]	CA <u>27.0</u>			‡													
210	3 + 28.	5 1	5	9	┧┟╌	14-				_	w	<u>.</u>	MEDIUM DENS	E TO LOOSE, DARK			_	‡										-			
	‡					-		.		:			TRA	INE SAND (A-2-4) W CE MICA	/			‡													
205.8	3 + 33.	.5		<u> </u>	_  :	1		:   : : :				<u>::</u>						‡										-			
203	‡	4	3	'		10				-	W	<b>∷</b> -					-	‡													
	‡							:   : : :				202.3	STIFF DARK GRA	AY, SILTY CLAY (A-7	37.0			‡										-			
200 200.8	3 + 38.	.5 2	4	6	┨┟╌	10					w	\$	W/ SAND SEA	AMS, TRACE MICA	٥,		-	‡													
	‡					<i>]</i> : :		:   : : :		:		197.3			42.0			‡													
195.8	3 + 43.	.5			_  :	-1		-		-		197.3 -	STIFF TO VERY	STIFF GRAY, SAND ), TRACE MICA	Y			‡													
195	‡	3	6	7		13		<del>                                     </del>		<del>.</del>	W	*	SEAT (A-0	,, 110 OL WIOA			-	‡										-			
	‡				:	ij:::		:   : : :		:		<b>\$</b>						‡													
190 190.8	3 + 48.	.5 4	4	7	┧┟	.l		<u> </u>		<u>.</u>	w	\$					-	‡													
	‡				:	· /· ·		.		:		<b>*</b>						‡													
185.8	+ 3 + 53.	.5			_  :	] [		.		:		\$						‡										-			
185	‡	28	8	9		17				-	W	*					-	‡										-			
2	‡				:	<i>i, 'j</i>		.		:		182.3		EN GRAY, SILTY CL	<u>57.0</u>		:	‡													
180.8	3 + 58.	5 7	10	15	$\dashv$	\	25	<u> </u>		-	w	\$	(A-7-5) W/ SAND	SEAMS, TRACE MIC	Ä		_	‡										<u> </u>			
	‡				:	'	<u> </u>	.		:		177 9			62.0			‡													
175 8	3 + 63.	.5			] :		<u> </u>	.		:		<b>*</b> ""-	VERY STIFF GRA	Y, SANDY CLAY (A-	6),			‡													
175	+	13	9	19	$ \cdot $		28	<del></del>		<u>-</u>	w	<b>*</b>	IRA	NOE IVIIOA			-	‡										-			
5	‡				:			.		:		<b>\$</b>					:	‡										<u> </u>			
170.8	68.	5 8	12	14	٦Ŀ		j	.	-	<u>.</u>	w	<b>*</b>					-	<u>†</u>										L			
	Ī						\ \	.		:	<i>"</i>	<b>1</b>					-	<u>†</u>										l E			
165 0	3 73.	5			$ \cdot $		\	.		-	<b>,</b>	167.3	DENSE, GRAY,	CLAYEY FINE SAND	<u>72.0</u>			<u> </u>										F			
165	<del>Ŧ</del> ′³	11	14	22	<b></b> ∏-		36	+	1		W	<del>\</del>	(A-2-6),	TRACE MICA			-	<del>-</del>										F			
	Ŧ				11		: /: : :	.		:	<i>%</i> ,/,	162.3					:	Ŧ										F			
160.8	3 ‡ 78.	5	14	15	<b>  </b>		'/: : :	.		:		¥		, GRAY, SANDY CLA RACE MICA	AY ——-		:	‡													
		1 12	14				L						(,, , , ,																		



							UKE L	<del></del>				
WBS	47532.1	1.3			TI	TIP 1-5986B COUNT	/ JOHNSTO	N			GEOLOGIST Prevatte, J.	
SITE	DESCRIP	MOIT	BRID	OGE 65	3 ON	N-Y29- OVER (-L-) I-95						GROUND WTR (ft)
BORI	ING NO.	EB2-E	3		SI	<b>STATION</b> 29+45	OFFSET 15	5 ft RT			ALIGNMENT -Y29-	<b>0 HR.</b> N/A
COLI	LAR ELE\	<b>/</b> . 23	9.0 ft		т	OTAL DEPTH 70.0 ft	NORTHING	588,177	7		<b>EASTING</b> 2,134,807	24 HR. FIAD
DRILL	. RIG/HAMIV	NER EF	F/DATI	E SME	3193 (	CME-550X 85% 11/14/2018		DRILL ME	THOD	Mud	Rotary <b>HAMM</b>	ER TYPE Automatic
DRIL	LER T. \	Whiteh	nead		ST	START DATE 05/08/19	COMP. DATI	<b>E</b> 05/08	3/19		SURFACE WATER DEPTH N/A	4
ELEV (ft)	DRIVE ELEV (ft)	EPTH (ft)		W COL	JNT 0.5ft	BLOWS PER FOOT 0 25 50	75 100	SAMP.	MOI	L O G	SOIL AND ROCK DESC	CRIPTION DEPTH (ft)
	(1)								IVIOI		LLL V. (II)	DEF III (II)
240	239.0	0.0	5	3	2				м	-	239.0 GROUND SURFA	
	‡				-	<b>?</b> 5			IVI		237.0 LOOSE, ORANGE AND R	ED, CLAYEY2.0
235	235.5 +	3.5	5	6	6	12	• • • •		M		SAND (A-2-6) COASTAL PLA	in,
	‡								// //	$\searrow$	MED. DENSE, ORANGE CLAYEY SAND (A	
230	230.5	8.5							°/0 °/0	$\searrow$	[MIDDENDORF FOR	MATÍON]
230	<u> </u>	0.0	7	6	8	14	<del>   </del>		M	$\searrow$		
									, %	$\searrow$	227.0	12.0
225	225.5	13.5	14	17	21	1			د - د	::: <u>L</u>	LOOSE TO DENSE, ORAN SILTY SAND (A-:	
	1 1					38			Sat.			
	220.5	10 E				::::/:::: ::::						
220	220.5	10.5	5	1	3	-   <del>  •   •   •   •   •   •   •   •   • </del>	<del>   </del>	!	Sat.			
	1 1									***	217.0	22.0
215	215.5	23.5	1	2	2	<u>                                     </u>					<b>COASTAL PLA</b> SOFT, DARK GRAY, SILTY	IN CLAY (A-7-6)
	l Ŧ		'		2	• • • • • • • • • • • • • • • • • • • •			W	F	[CAPE FEAR FORM	ATION]
	I Ŧ					:/				::: <u>-</u>	MED. DENSE, GRAY, SILTY	'SAND (A-2-4)
210	210.5 +	28.5	4	5	7	<b>→</b> 12			Sat.	::- <u> </u>		
	‡										207.0	32.0
205	205.5	33.5				] :::::::::::::::::::::::::::::::::::::					STIFF, DARK GRAY, SAND	OY CLAY (A-6)
200	‡		4	5	5	10	<del>   </del>		W	3		
	‡											
200	200.5	38.5	4	6	6	_ · · · · · · · · · · · · · · · · · · ·			w	3		
	‡					12.						
405	195.5	43.5									197.0 DENSE, DARK GRAY, S	ILTY SAND — — 42.0
195	133.3	40.0	17	23	22	<b>D</b> 45	<del>   </del>		Sat.	:::: <u> </u>	(A-2-4)	
	1											
190	190.5	48.5	9	28	22					Ŀ		
	1					50		;	Sat.		VERY STIFF, GRAY, SANE	OY CLAY (A-6) 50.0
	l Ŧ											, ,
185	185.5	53.5	7	8	10	18			w	<b>3</b> -		
	l Ŧ								100		183.0MED. DENSE, GRAY, CL	AYFY SAND 56.0
180	180.5	58.5			- 10				°/• °/•	<b>\</b>	(A-2-6)	
.00	1 7		8	11	18	29		!	Sat.		179.0 HARD, GRAY, SANDY (	$\frac{1}{2}$ $\frac{1}$
	‡										TIAILD, CITAT, CAITET	SEAT (A-0)
175	175.5	63.5	10	12	29	1			w	1		
	‡											
170	170.5	68.5										
170	+	JJ.J	13	18	22	40			W	建	169.0	70.0
	‡									F	Boring Terminated at Eleva HARD SANDY CLAY (COA	tion 169.0 ft IN STAL PLAIN)
	‡									Ł		
										F		
										E		
	ı T		i	i l		1		· I		Г		



## SITE PHOTOGRAPH

Bridge 653 on -Y29- over -L- (I-95)



CE: I-5986B
CE:
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El
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**CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

BORE LOG(S) & CORE REPORT(S)

CROSS SECTION(S)

SOIL TEST RESULTS

SITE PHOTOGRAPH(S)

TITLE SHEET

SITE PLAN

PROFILE(S)

SHEET NO.

5-8

9-18

19

20

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY JOHNSTON

PROJECT DESCRIPTION <u>I-95 WIDENING FROM SR</u> 1811 (BUD HAWKINS RD.) (EXIT 70) TO I-40 (EXIT 81) -WIDEN TO EIGHT LANES

SITE DESCRIPTION BRIDGE NO. 654 ON I-95 OVER MINGO SWAMP

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	I-5986B	1	20

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDING TO CLIMATIC CONDITIONS INCLORDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

E. BLONESHINE

M. HARTMAN T. WHITEHEAD

A. BLYTHE

J. SWARTLEY

INVESTIGATED BY \_\_\_\_\_S&ME, Inc.

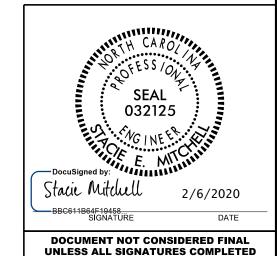
CHECKED BY S. MITCHELL

SUBMITTED BY S. MITCHELL

DATE \_\_FEBRUARY 2020



9751 SOUTHERN PINE BLVD CHARLOTTE, NC 28273



PROJECT REFERENCE NO. SHEET NO.

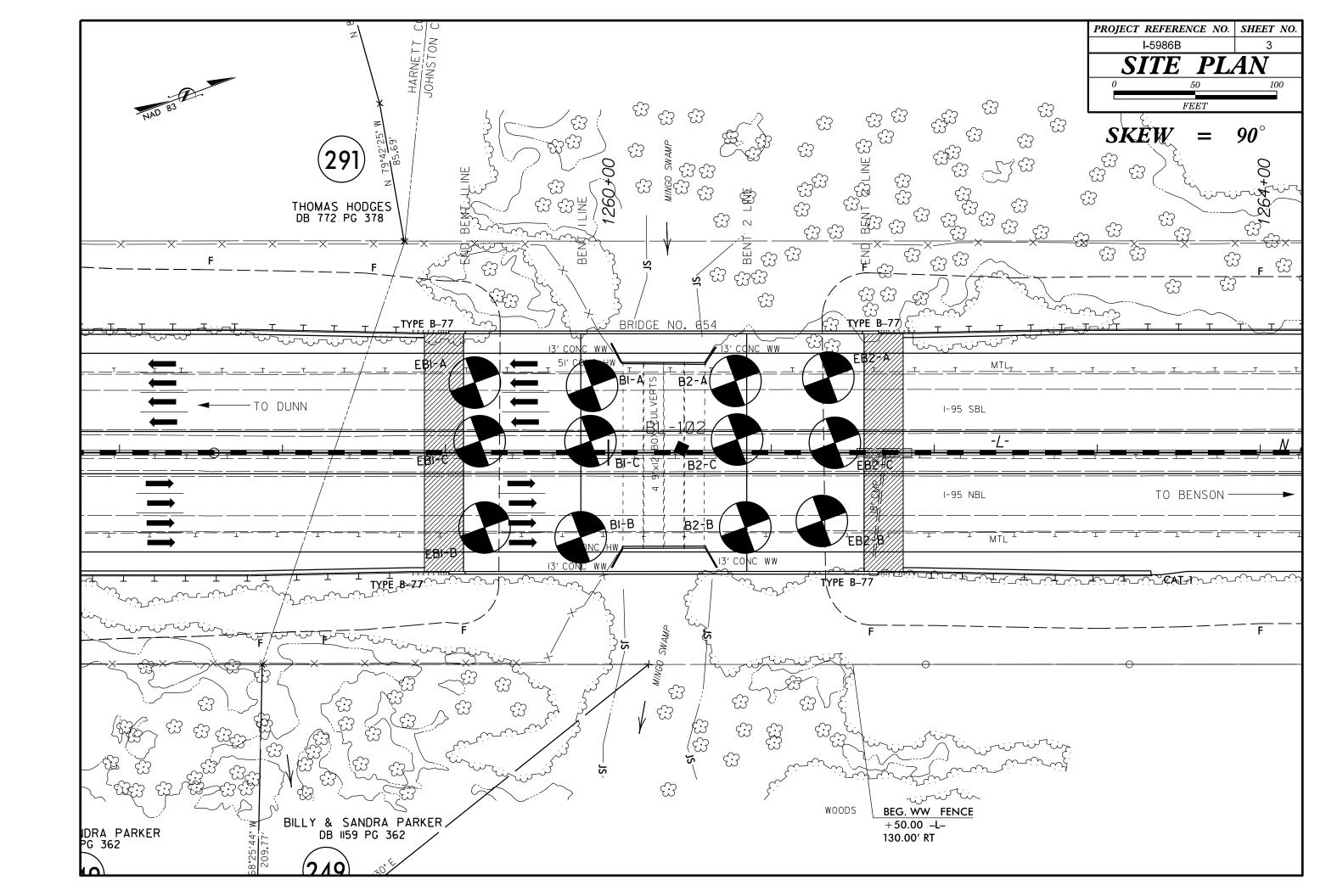
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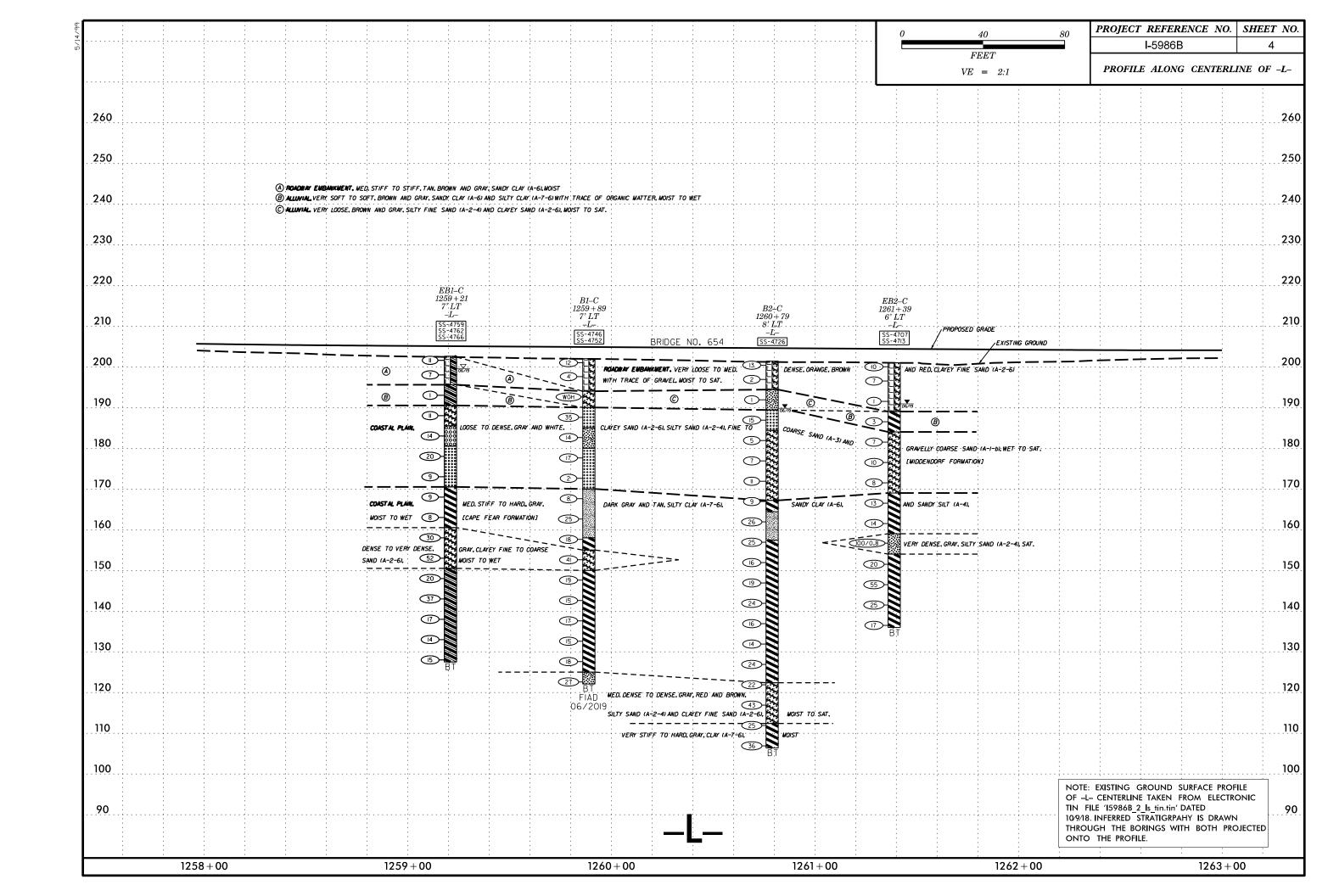
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

## SUBSURFACE INVESTIGATION

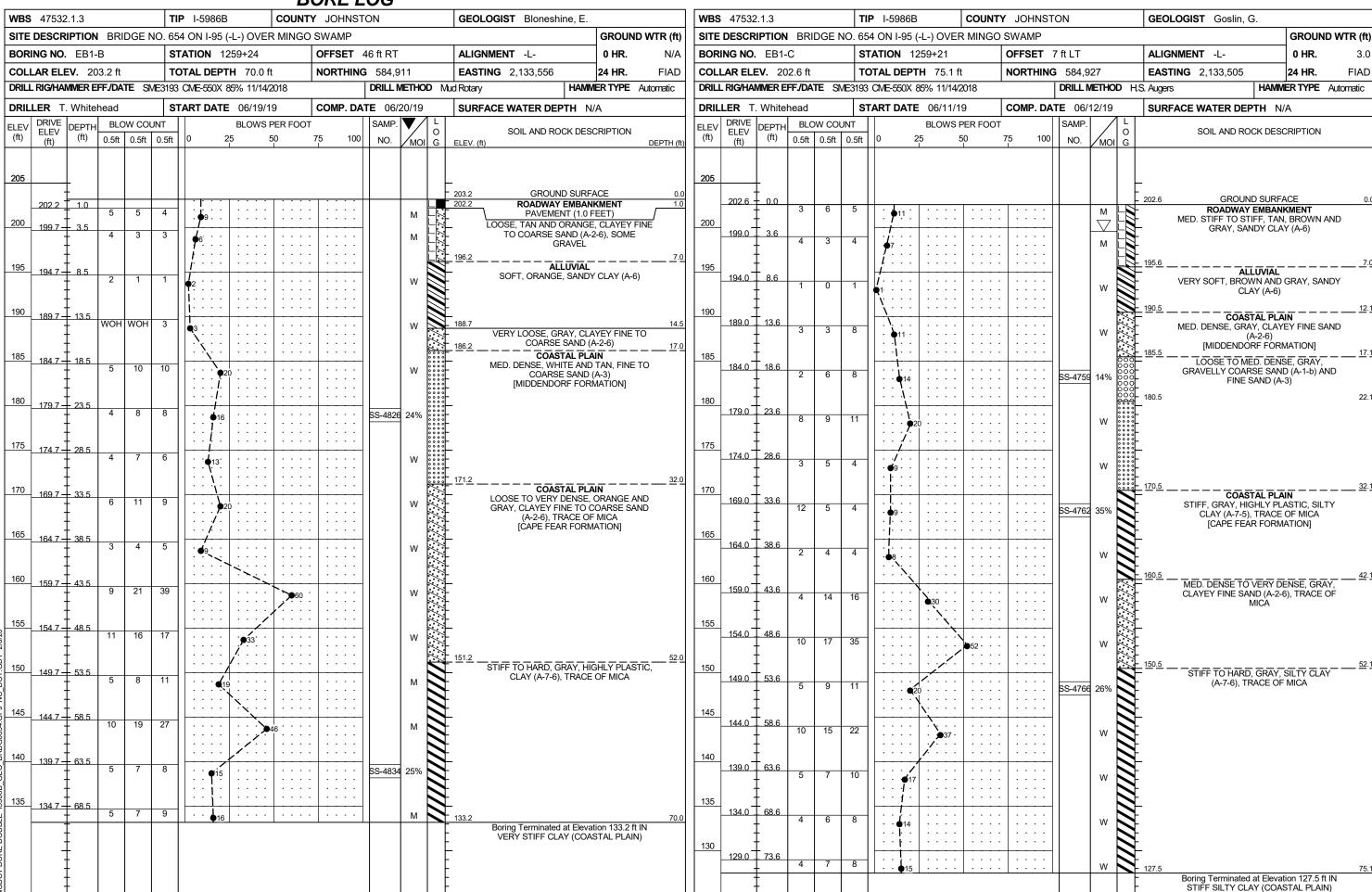
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

Part	R.  SAND OR THAT CONTAIN SAND OF CLAY MINERALS, OR HAVI HALE, SLATE, ETC.  O RISE ABOVE THE LEVEL AI TO OR ABOVE THE GROUND OF CALCIUM CARBONATE. VITY ON SLOPE OR AT BOTTO O IN THE CORE BARREL DIVI STRUCTURE OF ADJACENT OF INCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM OS DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO INCLINED FROM THE OTHER THAN STREAM OF ROCK.
Column   C	SAND OR THAT CONTAIN SAND OF CLAY MINERALS, OR HAVI HALE, SLATE, ETC.  O RISE ABOVE THE LEVEL AT TO OR ABOVE THE GROUND OF CALCIUM CARBONATE. VITY ON SLOPE OR AT BOTTO ON THE CORE BARREL DIVI STRUCTURE OF ADJACENT OF INCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM OS DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO COLORS, MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
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Address   Control   Cont	OF CLAY MINERALS, OR HAVII HALE, SLATE, ETC. OR RISE ABOVE THE LEVEL AT TO OR ABOVE THE GROUND OF CALCIUM CARBONATE. VITY ON SLOPE OR AT BOTTO OF THE CORE BARREL DIVII STRUCTURE OF ADJACENT SINCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM SIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS, MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
## ## ## ## ## ## ## ## ## ## ## ## ##	HALE, SLATE, ETC.  O RISE ABOVE THE LEVEL AT TO OR ABOVE THE GROUND  F CALCIUM CARBONATE.  VITY ON SLOPE OR AT BOTTO  D IN THE CORE BARREL DIVI  STRUCTURE OF ADJACENT  SINCLINED FROM THE  HORIZONTAL TRACE OF THE  BEEN DISPLACEMENT OF THE  LLEL PLANES.  ION AND DISLODGED FROM  SEDEPOSITED BY THE STREAM  MIZED AND TRACED IN THE  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  EDIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
MICHAEL OF STREET AND A SECTION OF A SECTI	O RISE ABOVE THE LEVEL AT TO OR ABOVE THE GROUND TO OR ABOVE THE GROUND OF CALCIUM CARBONATE.  VITY ON SLOPE OR AT BOTTO.  D IN THE CORE BARREL DIVID STRUCTURE OF ADJACENT OF THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LEEL PLANES.  ION AND DISLODGED FROM AN ADJACENT OF THE STREAM RIZED AND TRACED IN THE PRESENCE OF ROCK.
Mile	TO OR ABOVE THE GROUND OF CALCIUM CARBONATE. VITY ON SLOPE OR AT BOTTO D IN THE CORE BARREL DIVI STRUCTURE OF ADJACENT S INCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
Section   Control   Cont	OF CALCIUM CARBONATE.  VITY ON SLOPE OR AT BOTTO  D IN THE CORE BARREL DIVII  STRUCTURE OF ADJACENT  S INCLINED FROM THE  HORIZONTAL TRACE OF THE  BEEN DISPLACEMENT OF THE  LLEL PLANES.  ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM  NIZED AND TRACED.  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  EDIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
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Section   Control   Cont	STRUCTURE OF ADJACENT S INCLINED FROM THE HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
PRINCE TABLE STATE   1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	STRUCTURE OF ADJACENT  S INCLINED FROM THE  HORIZONTAL TRACE OF THE  BEEN DISPLACEMENT OF THE  LLEL PLANES.  ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM  NIZED AND TRACED IN THE  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  COLORS, MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
Part   10   10   10   10   10   10   10   1	HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
MODING ADDRESS   1	HORIZONTAL TRACE OF THE BEEN DISPLACEMENT OF THE LLEL PLANES. ION AND DISLODGED FROM S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
PARTING   THE	BEEN DISPLACEMENT OF THE  LEL PLANES.  ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM  NIZED AND TRACED IN THE  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  DIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
MORE PART OF THE START PARKED TO STATE START PART OF THE START PARKED TO START START PART OF THE START PARKED TO START S	BEEN DISPLACEMENT OF THE  LEL PLANES.  ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM  NIZED AND TRACED IN THE  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  DIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
CROUND WATER   CROWN WITH CAPPED   CROWN WATER   CROWN W	LLEL PLANES.  ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM  NIZED AND TRACED IN THE  IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  E DIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
SUBJECT   THE COMPANDED   THE STATE OF STATE   TH	ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE  IT HAS OCCURRED. KNESS IS SMALL COMPARED TO  DIRECTIONS. COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
STATIC WATER LEVEL AFTER 24 HOURS  SOUTHWAY SHOWN ISSUE SECTION PROTIONS OF SIGNAL SECTION IN SOUTH SECURITIES OF THE SOUTH SECTION IN SOUTH S	ION AND DISLODGED FROM  S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE  IT HAS OCCURRED. KNESS IS SMALL COMPARED TO  DIRECTIONS. COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
FRIENDE DELLIAT 10 0000 FAIL 10 POR MOUNTAINED TO STAND TO POR MOUNTAINED TO STAND THE PROPERTY OF THE STANDARD TO STANDARD TO STANDARD THE STANDARD TO STANDARD THE STANDARD	S DEPOSITED BY THE STREAM NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
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FIG. R-7-5 SECROLE IS. LL -39, FIG. 6-7-6 SECROLE IS. SECRILITY. SECROLE IS. SECROLE IS. SECROLE IS. SECROLE IS. SECROLE I	NIZED AND TRACED IN THE IT HAS OCCURRED. KNESS IS SMALL COMPARED TO EDIRECTIONS. COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
CONSISTENCY OF DENSEMBERS  MISCELLANEOUS SYMBOLS  MISCELANEOUS SYMBOLS  MISCELLANEOUS SYMBO	IT HAS OCCURRED.  KNESS IS SMALL COMPARED TO  DIRECTIONS.  COLORS. MOTTLING IN SOILS  ATER LEVEL BY THE PRESEN  OF ROCK.
PRIMARY SOIL TYPE COMPACTNESS OF CONTROL SECURITY PER COMPACTNESS OF CONTROL PER PREVAILED MANY PER PREVAILE	KNESS IS SMALL COMPARED TO E DIRECTIONS. COLORS, MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
PREVIOUS PREVIOUS PREVIOUS STREAM CORRESPOND TO STR	DIRECTIONS. COLORS, MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
GENERALLY LOSSE 4 TO 18 SULL SYMBOL SILE STEEL AND SECURED IN STRENGTH TO STRONG SOLL IN STRENGTH TO S	COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
CREMALLY CRANLAR MICHAELY CRANLAR MICHAELY CRANLAR MICHAELY CRANLAR MICHAELY CRANLAR MICHAELY CRANLAR MICHAELY CRANLAR MICHAEL MICHAELY CRANLAR MICHAEL MICHAELY CRANLAR MICHAEL MICHAELY CRANLAR MICHAEL MICH	COLORS. MOTTLING IN SOILS ATER LEVEL BY THE PRESEN OF ROCK.
MATERIAL DELINE UNION-COHESIVE)  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENTS ARE DISCERNIBLE  WENT DETSE  UNION-COHESIVE DISTRIBUTE ON CARRIE CREMENT DISTRIBUTION OF COMES STRONG BOX OF THE MEASURE OF PICK POILT.  WENT DETSE  UNION-COHES CREMENTS DISCERNIBLE  MITTOR CREMENTS DISCRIBED AND CARRIE CREMENT DISTRIBUTION OF CONTROL OF COMES OF THE MEASURE OF PICK POILT ON CARRIE CREMENT DISTRIBUTION OF CONTROL OF COMES OF COMES DEED CANADIS IN CREASE DISTRIBUTION OF CONTROL OF COMES DEED CAN	ATER LEVEL BY THE PRESEN
VERY SOFT CENSE > 50 UNDER CORE BORING SUNDING ROD  VERY SOFT 2 TO 4 0.25 TO 8.5  SILT-CLAY MEDIUM STIFF 8 TO 15 TO 1.0  MATERIAL STIFF 8 TO 50 I. BOUNDARY PART MAINTANDE WITH CORE  WITH	OF ROCK.
GENERALLY SOFT SILT-CLAY MEDIUM STIFF SILT-CLAY MEDIUM SILT-CLAY MEDIUM SILT-CLAY SCHEMED TO SORGHAM ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT. N VALUES C 108 BPF SCATTERED COCKDITAL NOT DISCERNIBLE, ORI DISCE	
SILT-CLAY MEDIUM STIFF 8 TO 15 10 30 10 10 11 TO 2 11 TO 2 10 2 TO 14 THE FERRED ROCK LINE 1 TO 2 2 TO 4 THE FERRED ROCK LINE 1 TO 2 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 2 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 30 STIFF 8 TO 30 2 TO 4 THE FERRED ROCK LINE 1 TO 30 STIFF 8 TO 30	
MATERIAL (CHESIVE) STIFF 15 TO 30 2 TO 4 30 AND EXPRESSED AS DIKES OR STRINGERS. SAPROLITE IS HARD SPECIMENS COULD TO RECEPTABLE WINDERS OF THAT RESULTS THAT AND ALLOW UNDERSOR SECRET THAN A NOR SECRET THAN A N	CCRIRED BY TOTAL LENGTH
TEXTURE OR GRAIN SIZE  U.S. STD. SIEVE SIZE 4 10 40 60 200 270 00 0.25 0.055 0.005  BOULDER (BURDA)  GRAIN SIZE  UNDERCUT UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK  BOULDER (BURDA)  GRAIN MM 305 75 2.0 0.25 0.05 0.05 0.05 0.05 0.05 0.05	
TEXTURE OR GRAIN SIZE  U.S. STD. SIEVE SIZE  U.S. STD. SIEVE SIZE  OPENING (MM)  OPENI	THE TOTAL ELEKOTT OF CORE
U.S. STD. SIEVE SIZE  4 10 40 60 200 270  OPENING (MM)  4.76 2.00 0.42 0.25 0.055  BOULDER (BLDR.)  CRAIN MM 305 75 2.0 0.25 0.05  SILT SAND  CRAIN MM 305 75 2.0 0.25  SILT AM INTRUSIVE BODY OF IGREDUS ROCK OF APPROXIMATELY  SHALLOW  UNCLASSIFIED EXCAVATION - MCCEPTABLE EXCAV	URE OR FABRIC OF THE PAR
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(BLDR.) (COB.) (GR.) (COB.) (GR.) (COB.) (GR.) (COB.) (GR.) (COB.) (GR.) (COB.)	
GRAIN MM 305 75 2.0 0.25 0.05 0.005 AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST  SIZE IN. 12 3 BY MODERATE BLOWS.  BT - BORING TERMINATED MICA, - MICACEOUS WEA WEATHERED MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A FOLLOW OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPT RESULTS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPT RESULTS OF THE WITH A 2	JM FRICTION ALONG A FAULT
SIZE IN. 12 3 BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PICK POINT.  CL CLAY MOD MODERATELY 7 - UNIT WEIGHT HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RECOVER.	1DED OF DLOVE (N. OD DDE) C
CL CLAY MOD MODERATELY 7 - UNIT WEIGHT HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT RE	
SOLE PIOLISTONE CONNECTATION OF TERMINATES   NP - NON PLASTIC /- DRY UNIT WEIGHT   PUINT OF A GEOLOGIST'S PICK.	SAL IS PENETRATION EQUAL
SOUL MOISTURE SCALE FIELD MOISTURE CORP. CORP. SCALE SCALE OF SCAL	AL RECOVERED DIVIDED BY
GUIDE FOR FIELD MOISTURE DESCRIPTION OF A PICK DIVINISHED TO STREAM AND EXPRESSION OF A PICK DIVINISHED TO STREAM OF THE CONTRACT OF THE CONTROL OF THE CONT	
DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK PIECES CAN BE BROKEN BY FINGER PRESSURE.  - SATURATED - USUALLY LIQUID; VERY WET, USUALLY E - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON SS - SPLIT S	JALITY DESCRIBED BY TOTAL
(SAT.) FROM BELOW THE GROUND WATER TABLE F - FINE SL SILT, SILTY ST - SHELBY TUBE CONT. OF PICK, PIECES I INCH.	ER THAN 4 INCHES DIVIDED
PLASTIC TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MA	R.
PANCE - VET - (VI) SEMISULIDE REQUIRES DEFINO TO	
(*) PL PLASTIC LIMITHI HIGHLY V - VERY RATIO <u>TERM SPACING</u> <u>TERM THICKNESS</u>	
EQUIPMENT USED ON SUBJECT PROJECT VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET EL	VATION: 199.73 FEE
DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: MODERATELY CLOSE 1 TO 3 FEET THINLY REDDED 0.16 - 1.5 FEET	
PEGUIDES ADDITIONAL WATER TO CHE-45C X CLAY BITS X AUTOMATIC MANUAL CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET 10-2-0.16	
- DRY - (D)  ATTAIN OPTIMUM MOISTURE  CME-55  CME-55  CME-55  CME-55  CME-55  CME-55  CME-55	
PLASTICITY X 8' HOLLOW AUGERS	
PLASTICITY INDEX (PI)  PLASTICITY INDEX (PI)  PLASTICITY INDEX (PI)  PLASTICITY INDEX (PI)  PROBLEM TIME FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.  PROBLEM TO THE FINANCE OF THE FORMAL PROBLEM TO THE FORMAL P	
NUN PLASTIC 0-5 VERY LOW   TUNG. CARBIDE INSERTS   FRIABLE ROODING WITH THOUGH THE TRANSPORT OF THE PROPERTY O	
SLIGHTLY PLASTIC 6-15 SLIGHT VANE SHEAR TEST V CASING V ADVANCER HAND TOOLS:	
POST HOLE DIGGER MODERATED A M	
COLOR  PORTABLE HOIST   TRICONESTEEL TEETH   HAND AUGER   BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR    PORTABLE HOIST   TRICONE STELL TEETH   HAND AUGER   SOUNDING ROD   INDURATED   SPECIAL TO BREAKS EASILY WHEN HIT WITH HAMMER.    COLOR   COLO	
COLOR  PORTABLE HOIST   TRICONE 'STEEL TEETH   HAND AUGER   BREAKS EASILY WHEN HIT WITH HAMMER.  COLOR   TRICONE 'TUNGCARB.   SQUINDING POR	DATE; 8-1





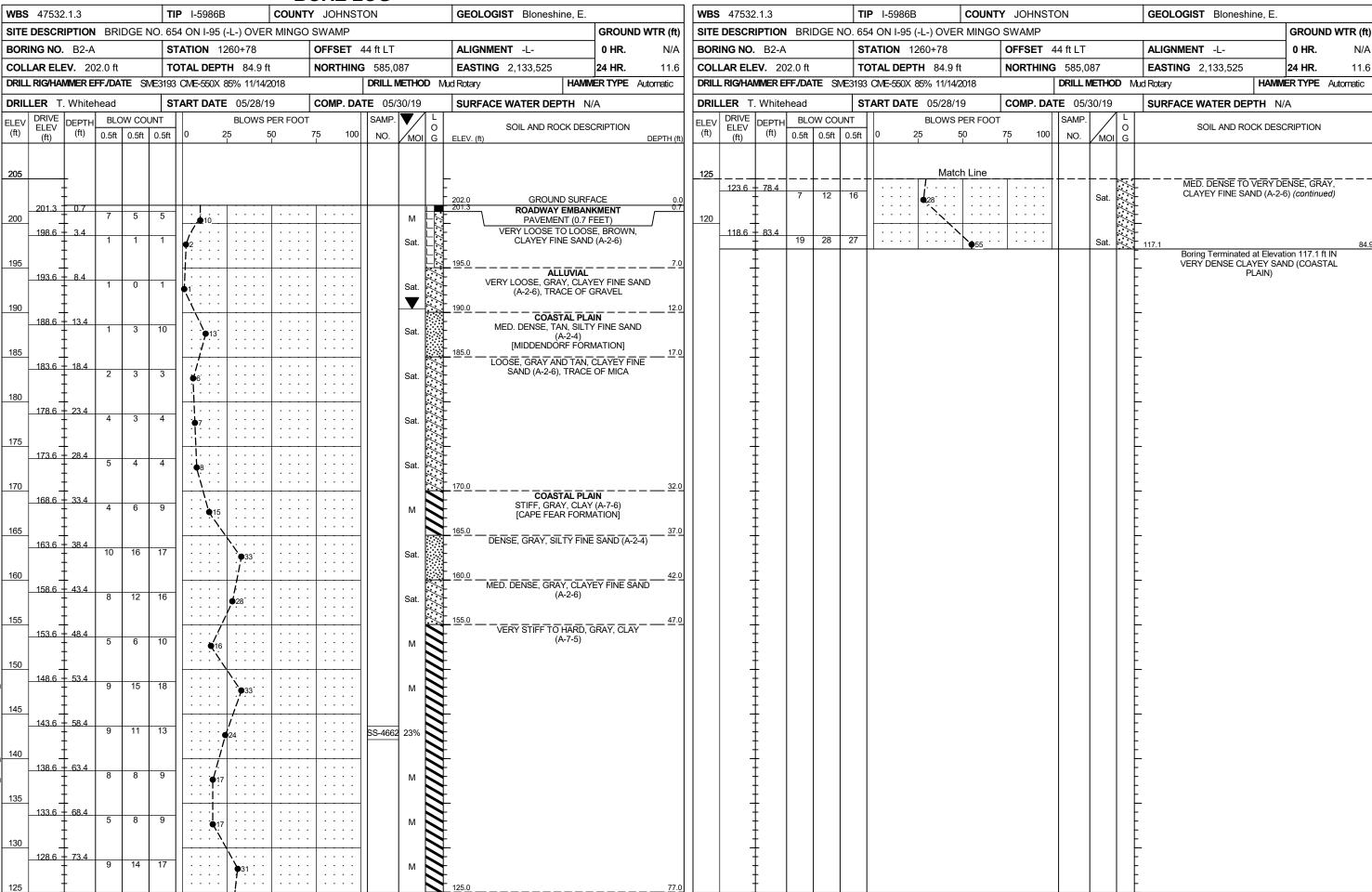
					-				ORE I					_				_		- 1			1						T				
	47532					P I-5986			Y JOHNS	STON		G	BEOLOGIST Bloneshine, E				47532.1				<b>FIP</b> 1-5			DUNTY JO		NC			GEOLOGIS	T Bloneshi			
-				DGE			(-L-) OVE	R MINGO						GROUND \	` '	-						,	-) OVER MI						1				WTR (ft)
	NG NO					TATION			OFFSET				ALIGNMENT -L-	0 HR.	N/A	-	RING NO.				STATIO				SET 4				ALIGNMEN			0 HR.	N/A
			203.5 ft				<b>PTH</b> 90.0		NORTHIN				<b>EASTING</b> 2,133,470	24 HR.	FIAD		LAR ELEV						90.0 ft			584,93			EASTING	2,133,470		4 HR.	FIAD
				TE S			85% 11/14			DRILL M				MMER TYPE AL	tomatic								% 11/14/2018			DRILL M		D Mu	· ·				Automatic
DRIL	LER T					TART DAT	<b>E</b> 06/03/		COMP. D			S	SURFACE WATER DEPTH	N/A		DRIL	LER T. W				START I		06/03/19		/IP. DAT	<b>E</b> 06/0	)4/19	1	SURFACE	WATER DEP	TH N/A		
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	··	0.5ft	0.5ft	0		PER FOOT	75 100	SAMP.	/ I	O G ELE	SOIL AND ROCK D		DEPTH (ft)	ELEV (ft)	DRIVE DE (ft)	EPTH (ft)	0.5ft 0.5		0	25 	BLOWS PER 50	FOOT 75	100	SAMP. NO.	MOI	O G		SOIL AND RO	CK DESCR	RIPTION	
205		+										203	3.5 GROUND SU	DEACE	0.0	125	125.0	′8.5  /	5 15	5 14	-		Match Lin				- <del>_</del> -		<u></u>	D. DENSE, GRA	AY. SILTY	FINE SAN	
200	202.8	0.7	9	7	5	· · · · · · · · · · · · · · · · · · ·					w	202	ROADWAY EMB. PAVEMENT (0.	ANKMENT .7 FEET)	0.7	120	120.0	20.5				: :   <b>'</b>							_121.5 MED	(A-2-4)  DENSE TO D	(continued	) AY, CLAY	
200	200.0	3.5	3	5	6	11 .					Ĺ	- <del>                                     </del>	SAND (A-2- MED. DENSE, BROWN, TO FINE SAND (A-2-	2-4) CLAYEY COARSI	_ 	120	120.0	55.5	12 16	3 18	11		• • 34	I			W		-	FINE TO COAF	RSE SAND	) (A-2-6)	
195	195.0	8.5	1	1	2	<u></u>					M % % %		GRÀVE  GRÀVE  ALLUVIA  VERY LOOSE, BROWN	LAL		115	115.0	38.5	13 12	2 16			28				W	/////	- 113.5				90.0
190	190.0	13.5			10						% % disse.	191	(A-2-6) 1.5 COASTAL F VERY LOOSE TO MED	PLAIN	12.0														Bori MED	ng Terminated ). DENSE CLA` Pl	at Elevation YEY SAND LAIN)	n 113.5 ft   ) (COAST <i>A</i>	IN AL
405		<u> </u>		9	10		19				M	_ _ _ _ 186	GRAY, GRAY AND WH SAND (A-2-4) AND FINE	IITE, SILTY FINE SAND (A-3) WITH L	H 17.0																		
185	185.0	18.5	3	1	2	<b>●</b> 3 · · · · · · · · · · · · · · · · · · ·					W	0000	[MIDDENDORF FO	DRMATION			+												-				
180	180.0	23.5	5	7	8	· · • 15	; · · · ·			-	W	000																	-				
175	175.0	28.5	4	6	6						W																		-				
170	170.0	33.5				. / :					000001/6 %	171	1.5 COASTAL F VERY LOOSE, GRAY, CL	PLAIN	32.0													-	_				
		<u> </u>	3	1	0	1					% % % <b>∜</b>	166	(A-2-6) [CAPE FEAR FO] STIFF, GRAY, SILTY	) RMATION]	37.0													-					
165	165.0	38.5	2	4	5	• • • • • • • • • • • • • • • • • • •				SS-4693	22%	161	31111, GIVAT, SILTT	CLAT (A-7-0)	42.0														-				
160	160.0	43.5	3	4	5	. I - I • •9 · ·					<i>K. %. %. %</i> S		LOOSE, LIGHT GRAY, CI (A-2-6)		D ****		+												-				
155 R	155.0	48.5	15	18	11	:::``;					<u>% •000000</u> ∀	156	6.5 MED. DENSE, DARK GI (A-3)	RAY, FINE SAND	47.0														-				
9/Z LGD1 Z/P	150.0	53.5					7				00000	1 <u>5</u> 1	1 <u>.5</u> STIFF TO HARD, GRA (A-7-6)	AY, SILTY CLAY	52.0														_				
NC_DOT		-	7	11	12		23			SS-4696	23%		(A-7-6)	)																			
G0654.GP.	145.0	58.5	10	19	21		- 40			-	М																		-				
140 030	140.0	63.5	6	10	11		21	: : : :		-	м																		-				
135 135	135.0	68.5	5	7	9	<i> </i> .   <i> </i> .   <del></del>	: : : :				м																		-				
TI DOUBLE	130.0	73.5				¶16     .     .					IVÍ																						
CDOT BOK		 	5	6	7						М	126	6 <u>.5</u>		77.0														-				



	<i></i>	BORE LOG		_			
<b>WBS</b> 47532.1.3	TIP I-5986B COUN	TY JOHNSTON	GEOLOGIST Bloneshine, E.	<b>WBS</b> 47532.1.3	TIP I-5986B COUN	TY JOHNSTON	GEOLOGIST Bloneshine, E.
SITE DESCRIPTION BRIDGE NO	D. 654 ON I-95 (-L-) OVER MING	O SWAMP	GROUND WTR (ft)	SITE DESCRIPTION BRIDGE N	NO. 654 ON I-95 (-L-) OVER MING	O SWAMP	GROUND WTR (ft)
BORING NO. B1-A	<b>STATION</b> 1259+90	OFFSET 41 ft LT	ALIGNMENT -L- 0 HR. N/A	BORING NO. B1-A	<b>STATION</b> 1259+90	OFFSET 41 ft LT	ALIGNMENT -L- 0 HR. N/A
COLLAR ELEV. 202.8 ft	TOTAL DEPTH 79.9 ft	NORTHING 585,003	<b>EASTING</b> 2,133,497 <b>24 HR</b> . 10.2	COLLAR ELEV. 202.8 ft	TOTAL DEPTH 79.9 ft	NORTHING 585,003	<b>EASTING</b> 2,133,497 <b>24 HR.</b> 10.2
DRILL RIG/HAMMER EFF/DATE SME	E3193 CME-550X 85% 11/14/2018	DRILL METHOD M	ud Rotary HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE SN	VIE3193 CME-550X 85% 11/14/2018	DRILL METHOD	Mud Rotary HAMMER TYPE Automatic
DRILLER T. Whitehead	<b>START DATE</b> 06/02/19	COMP. DATE 06/03/19	SURFACE WATER DEPTH N/A	DRILLER T. Whitehead	<b>START DATE</b> 06/02/19	COMP. DATE 06/03/19	SURFACE WATER DEPTH N/A
ELEV DRIVE DEPTH BLOW COUN		SAMP. L	SOIL AND ROCK DESCRIPTION	ELEV DRIVE DEPTH BLOW COU	JNT BLOWS PER FOO	OT SAMP.	SOIL AND ROCK DESCRIPTION
(ft) (ft) (ft) 0.5ft 0.5ft	0.5ft 0 25 50	75 100 NO. MOI G	ELEV. (ft) DEPTH (ft	(ft)   LLL     (ft)   0 = 0   0 = 0	0.5ft 0 25 50	75 100 NO. MOI G	
205				125	Match Line		
			202.8 GROUND SURFACE 0.0	124.4 78.4 5 10	12	· · · · · ·	MEDIUM DENSE, GRAY, CLAYEY FINE TO 122.9 COARSE SAND (A-2-6) (continued) 79.9
202.0 0.8 8 5	6 . 11		PAVEMENT (0.8 FEET)	4   1			Boring Terminated at Elevation 122.9 ft IN
200 199.4 3.4			= 199.8 STIFF, ORANGE AND BROWN, SANDY ,— 3.0	4			- MED. DENSE CLAYEY SAND (COASTAL PLAIN)
+ 4 4	3		CLAY (A-6)  LOOSE, ORANGE AND BROWN, CLAYEY				*NO RECOVERY FROM DEPTH 28.4' TO
195			FINE SAND (A-2-6), TRACE OF GRAVEL				- 29.9 <sup>t*</sup>
194.4 + 8.4   2   9	5		193.8 9.0				F
			COASTAL PLAIN LOOSE TO MED. DENSE, GRAY, LIGHT	]			-
190 189.4 13.4			190.8 GRAY AND WHITE, SILTY FINE SAND 12.0 - (A-2-4) AND FINE TO COARSE SAND (A-3),	<u>                                     </u>			<u></u>
5 12	15	M	TRACE OF GRAVEL [MIDDENDORF FORMATION]				
			185.8	,			<u>t</u>
185 184.4 18.4 4 6	5		-				-
	¶ . ¶11 .						Ł
180		0000	180.8 22.0				
179.4 23.4 4 6	8						-
							F
175 174.4 28.4			-				F
2 4	4	.					F
170		0000 0000 0000					F
169.4 + 33.4   2   2	3	·   · · · · ·     W	COASTAL PLAIN  LOOSE, GRAY AND TAN, CLAYEY FINE				F
			SAND (A-2-6) [CAPE FEAR FORMATION]				-
165 164.4 38.4			DENSE, LIGHT GRAY AND GRAY, SILTY	4			
13 20	26	:   : : : :     w     w	FINE SAND (A-2-4)				
							_
159.4 43.4 8 20	21		-				-
	41						t
155			_	$  \cdot   \cdot \pm \cdot   \cdot  $			Ł
S 154.4 + 48.4   11   11	11 •22	: ::::   M	153.8 49.0 STIFF TO VERY STIFF, GRAY, SILTY CLAY	4			Ŀ
			(A-7-5)				Ł
150 149.4 53.4 9	11	- <del></del>    <b>S</b>	-				F
	··     • • 20	SS-4679 24%					Ł
2 145 145 1 Fo 1			_				Ł
0 144.4 58.4 4 9	12 21	-     M					-
							<u>-</u>
140 139.4 63.4			-				F
0	ŏ						F
98   135							F
<u>ії 134.4 68.4 </u> щ + 5 6	7		-				F
	· · • • · · · · · · · · · · · · · · ·						‡
130 + 73.4	j	<u> </u>	-				<u></u>
0 1293 193 4 7	5						‡
		: ::::	125.8 77.0	]			‡
Z   Z	11 3 1 1				i i		I .

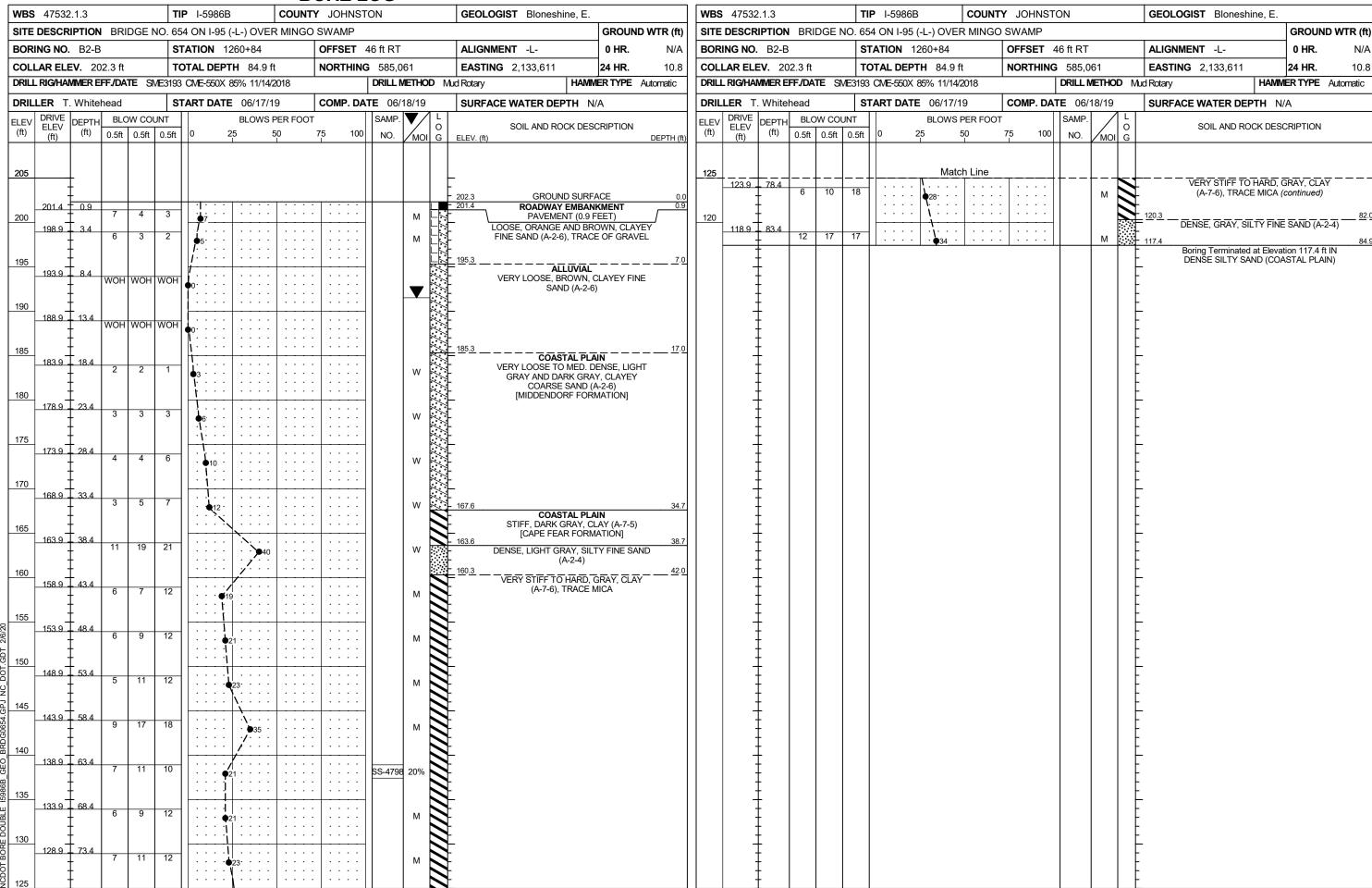
WRS	47532	213			TIE	I-5986B			Y JOHNS		<u> </u>		GEOL O	GIST Bloneshine			WE	<b>3S</b> 47532	213			TIP I-	5086B CO	UNTY JOHN	STON			GEOLOGIST Bloneshine, E.	
			I BRIDG	SF NO		I ON I-95 (				31014			OLOLO	Dionesimin		UND WTR (fi	→			I BRIF	OGF NO		N I-95 (-L-) OVER MIN					OLOCOTO Dionestino, L.	GROUND WTR (ft)
-		. B1-B				ATION 12	` ,		OFFSET	52 ft	RT		ALIGNM	MENT -L-	0 HR	•	′	RING NO.					ON 1259+83	OFFSE1		 RT		ALIGNMENT -L-	0 HR. N/A
		<b>EV.</b> 20			+	TAL DEPT		<u> </u>	NORTHII				_	I <b>G</b> 2,133,582	24 HR			LLAR ELI					. <b>DEPTH</b> 84.9 ft	NORTH				<b>EASTING</b> 2,133,582	<b>24 HR.</b> 11.0
				SME		CME-550X 8			1.1.0.1.1.1.1.			OD	Mud Rotary			PE Automatic							5-550X 85% 11/14/2018	1.0			IOD M		MER TYPE Automatic
DRIL	LER T	. White	head		ST	ART DATE	06/18/1	9	COMP. D	OATE	06/19/19	9	SURFAC	CE WATER DEPT	H N/A		DR	ILLER T	. White	head		START	<b>DATE</b> 06/18/19	COMP.	DATE (	6/19/19	9	SURFACE WATER DEPTH	√/A
	DRIVE	DEPTH		COUN			BLOWS F		1		MP.	<u> </u>	T'	SOIL AND ROCK		N.I.	ELE	DDI) /E			W COUNT		BLOWS PER F		SAM		/ [		
(ft)	ELEV (ft)	(ft)	0.5ft 0.	.5ft (	0.5ft	0 2	25 5	50	75 10	00 N	10. MC	O OI G	ELEV. (ft)	SOIL AND ROCK	N DESCRIPTIO	DEPTH (	(ft)	(ft)	(ft)	0.5ft	0.5ft 0.5	5ft 0	25 50	75 1	00 NC	). /мс	O OI G	SOIL AND ROCK DES	SCRIPTION
205		1															125	5 424		l			Match Lin	e	4				
		‡								Ш			202.8	GROUND			0.0	124.4	78.4	5	6 1	0 :	<u>1</u> 16		1 1	М		VERY STIFF, GRAY, CLAY OF MICA (conti	
200	201.9	0.9	6	5	5	. 1			<b>I</b>		М		201.9	ROADWAY EI PAVEMENT	(0.9 FEET)		120	,   :	‡						1 1				82.0
200	199.4	<del>+</del> 3.4 +	4	6	4	· 10 ·				-	l <sub>w</sub>			OOSE, TAN AND RE COARSE SAND (A-2			120	119.4	83.4	9	18 1	9 .	37		· SS-4	 320 18%	6	LIVIE CVVID /V	
		‡				: <b>/</b> :": :				-		J	<del>}</del>						•						•		1.,,	Boring Terminated at Elev DENSE CLAYEY SAND (C	ation 117.9 ft IN
195	194.4	8.4				<i>j</i> · · · ·							194.8	ALLU		8	. <u>o</u>	_	‡									- DENSE CEATET SAND (C	OASTAL FLAIN)
		Ī	2	1	2	<b>ф</b> 3				1 1		7	<u>.</u>	VERY LOOSE, DARI SAND (A-2-4), TR	K GRAY, SILTY				Ī									-	
190	100.4=	Ξ.,.								1 1			E	0/14D (/-2-4), 110	TIOL OF ORT	V LL		_	E										
	189.4	13.4	2	2	0	1 ¶2				1 1	w							-	ł									_	
405		‡				\\							185.8			17	.0		‡									- -	
185	184.4	18.4	2	6	8	\				-	l <sub>w</sub>	000	-	MED. DENSE, LIGH	IL PLAIN IT GRAY AND	TAN,			‡									<del>-</del> -	
		‡				7.14					"	000		FINE TO COARSE GRA	VEL				‡									<u>-</u>	
180	179.4	+ + 23.4				<u>                                   </u>						000		[MIDDENDORF	FORMATION	]		_	‡									- -	
		+	4	6	5	11 .			:   : : : :	:	w	000						-	‡									<del>-</del> -	
175		‡				:   : :						000	175.8	OOSE, TAN AND OR		27	. <u>o</u>		‡									-	
170	174.4	+ 28.4 +	2	4	5	. j				-	l w	:/://://	÷		SAND (A-2-6)	EY FINE		-	‡									-	
		Ŧ								.		·//-//	170.8			32			Ŧ										
170	169.4	33.4				-				$\dashv$ $lacksquare$				COASTA MED. STIFF TO STI	L PLAIN		<u></u>	_	‡									<u>-</u>	
		Ī	3	3	5	. ♠8			.	.   55-4	4810 21%	$^{\circ}$	<del>\</del>	PLASTIC, C	LAY (A-7-6)				Ī									-	
165	404.4=	Ī 4				. 1				1 1			<u> </u>	[0/4 2 1 2/40				_	£									<u></u>	
	164.4	38.4	3	6	9	15				·	М		}						<u> </u>									- -	
160		‡								1 1			160.8			42	. <u>o</u>		‡									-	
160	159.4	43.4	11 1	16	23					-	l <sub>w</sub>	/ <u>/</u> ///,	<del>}</del>	DENSE, GRAY, CL (A-2	LAYEY FINE S. 2-6)	AND		-	‡										
		‡					/		.	.	"	/·/·/·	<del>}</del> <del>}</del>						‡									- -	
155	154.4	† 48.4					: / : :					·//./	<del>*</del>						‡									- -	
2/6/2		Ī	9 1	15	18		33.			:	l w	·//////	<del>,</del>						Ŧ										
150		Ī,,,					/						150.8	ERY STIFF, GRAY,	CLAY (A-7-6)	TRACE 52	. <u>o</u>		E										
100	149.4	± 53.4	5	9	10				.	:	М		}	OF N	MICA (, , , ,				‡									-	
ON A45		‡				1							}						‡									- -	
145 145	144.4	58.4	8 1	12	14	\				-	M		}						‡									_ -	
BRDG0654		‡				· · · · /	P <sup>26</sup>			:	l IVI		}						‡									-	
	139 4	63.4				· · · · /							<u></u>					_	‡									<u>-</u>	
GEO		‡	5	7	10	17			:   : : : :	:	М		\$						‡									-	
135 135		‡				• • •   •				:			\$						‡									-	
LE 15	134.4	68.4	5	7	10	17				.	М		<b>\</b>					-	Ŧ									-	
DOOUB		Ī											<b>\</b>						Ŧ									-	
130	129.4	73.4	5	Ω						$\exists  $			<b>}</b>					-	Ŧ									<u> </u>	
OT B.		‡	5	8	9	17				:	M		}						‡								[	<u>-</u> -	
9 125		<u>†</u>				.				·			}						<u> </u>										

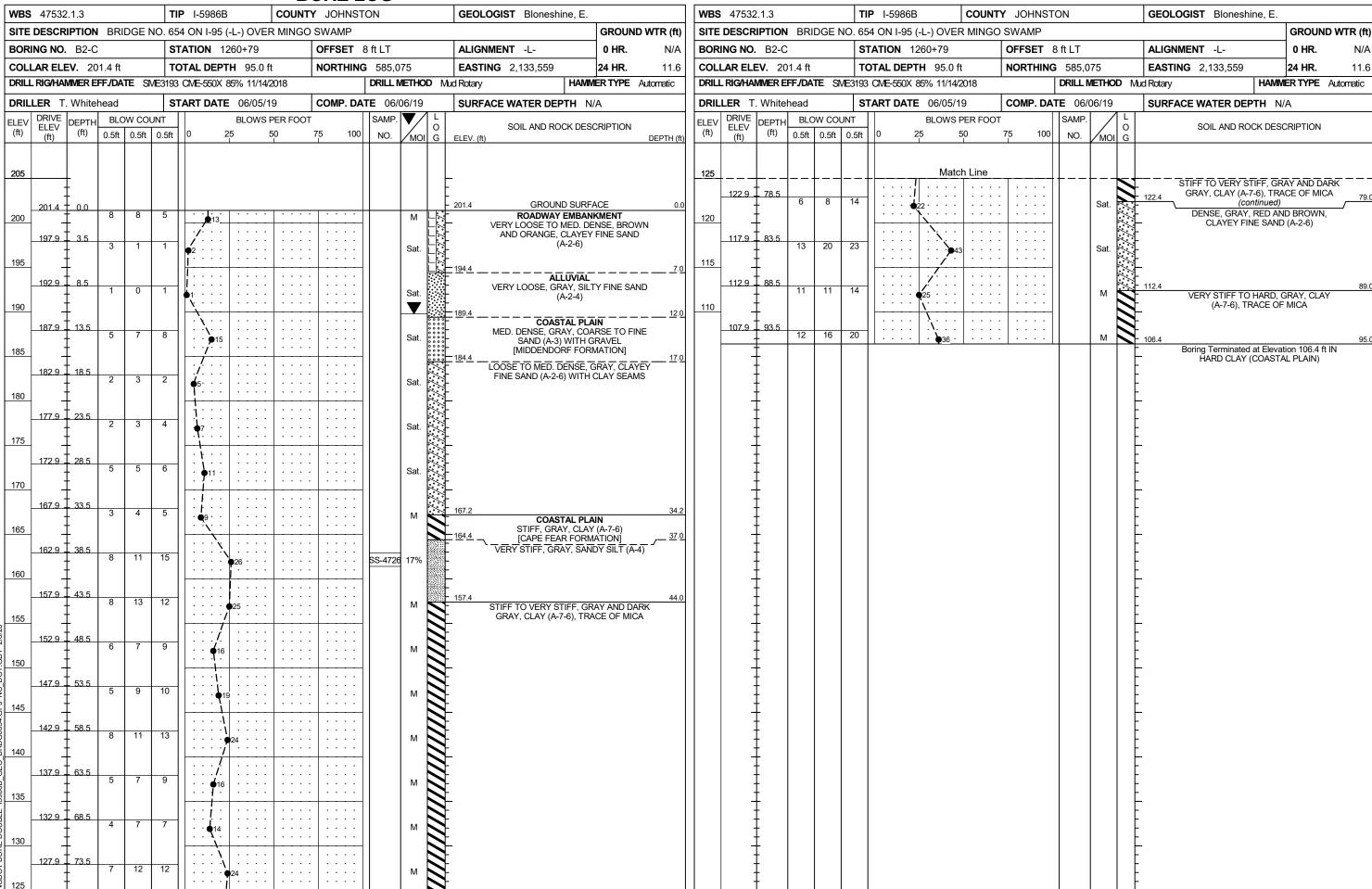
<b>WBS</b> 47532.1.3		ITY JOHNSTON	GEOLOGIST Bloneshine, E.	<b>WBS</b> 47532.1.3	TIP 1-5986B COUN	TY JOHNSTON	GEOLOGIST Bloneshine, E.
SITE DESCRIPTION BRIDGE N			GROUND WTR (ft)	SITE DESCRIPTION BRIDGE NO			GROUND WTR (ft)
BORING NO. B1-C	<b>STATION</b> 1259+89	OFFSET 7 ft LT	ALIGNMENT -L- 0 HR. N/A	BORING NO. B1-C	<b>STATION</b> 1259+89	OFFSET 7 ft LT	ALIGNMENT -L- 0 HR. N/A
COLLAR ELEV. 202.0 ft	TOTAL DEPTH 79.9 ft	<b>NORTHING</b> 584,991	<b>EASTING</b> 2,133,529 <b>24 HR.</b> FIAD	COLLAR ELEV. 202.0 ft	TOTAL DEPTH 79.9 ft	<b>NORTHING</b> 584,991	<b>EASTING</b> 2,133,529 <b>24 HR.</b> FIAD
DRILL RIG/HAMMER EFF./DATE SM	E3193 CME-550X 85% 11/14/2018	DRILL METHOD Mu		DRILL RIG/HAMMER EFF./DATE SME	=3193 CME-550X 85% 11/14/2018	DRILL METHOD	
DRILLER T. Whitehead	<b>START DATE</b> 06/06/19	COMP. DATE 06/07/19	SURFACE WATER DEPTH N/A	DRILLER T. Whitehead	<b>START DATE</b> 06/06/19	COMP. DATE 06/07/19	SURFACE WATER DEPTH N/A
ELEV (ft) DRIVE ELEV (ft) DEPTH BLOW COU		OT SAMP. V L O NO. MOI G	SOIL AND ROCK DESCRIPTION  ELEV. (ft) DEPTH (ft)	ELEV DRIVE ELEV (ft) DEPTH BLOW COUNTY (ft) 0.5ft 0.5ft 0		OT SAMP. L O NO. MOI G	SOIL AND ROCK DESCRIPTION
205			_ -	125	Match Line		MED. DENSE, GRAY, SILTY FINE SAND
202.0 0.0 4 7	5 . •12	M L .	202.0 GROUND SURFACE 0.0  ROADWAY EMBANKMENT	9 13	14	·   · · · · ·     M	122.1 (A-2-4) 79.9  Boring Terminated at Elevation 122.1 ft IN
198.6 + 3.4 3 2	2 4		LOOSE TO MED. DENSE, ORANGE AND BROWN, CLAYEY FINE SAND (A-2-6), TRACE OF GRAVEL				MEĎ. DENSE SILTY SAND (COASTAL PLAIN)
195 193.6 + 8.4 WOH WOH	WOH •0	Sat.					- - - -
190 188.6 - 13.4 4 16	19		(A-2-6) 12.0 12.0 12.0 VERY LOOSE TO DENSE, GRAY AND				
183.6 + 18.4	• • • • • • • • • • • • • • • • • • • •		- WHITE, COARSE SAND (A-3) WITH - GRAVEL, SILTY FINE SAND (A-2-4) AND - SAND (A-3) WITH CLAY SEAMS 17.0 - (22.0'-32.0')				-
180	8 14	Sat. Sat.	- [MIDDENDORF FORMATION]  _ 180.0 22.0				- - -
178.6 + 23.4   5   8	9   17	Sat.	- - - -				
173.6 - 28.4 1 1	1 2	Sat.	- - - -				-
168.6 + 33.4	4						
165 163.6 + 38.4 6 10	15	SS-4746 20%	- [CAPE FEAR FÓRMATION]  - -				<u>-</u> -
160			- - - - - - - - - - - - - - - - - - -				- - -
155 153.6 + 48.4	9	Sat.	VERY STIFF, GRAY, CLAY (A-7-6), TRACE  OF MICA  155.0  DENSE, GRAY, CLAYEY COARSE TO FINE SAND (A-2-6)				-
10   20   150   150   150   1   1   1   1   1   1   1   1   1	21 41	M M	- - 150.0 52.0				-
148.6 + 53.4   5   8   2   145	11 019	:   · · · · ·       M	VERY STIFF, GRAY AND DARK GRAY, CLAY (A-7-5), TRACE OF MICA				
145	8 15	· · · · · · · · · · · · · · · · · · ·	- - - -				- - -
138.6 + 63.4 4 7	10 1	· · · · · · · · · · · · · · · · · · ·	- - - -				
98 135 133.6 + 68.4 4 7	8	SS-4752 30%	- - - -				- - - -
133.6 + 68.4	1. 1. 1		- - - -				- - -
125	9	M	125.0 77.0				-

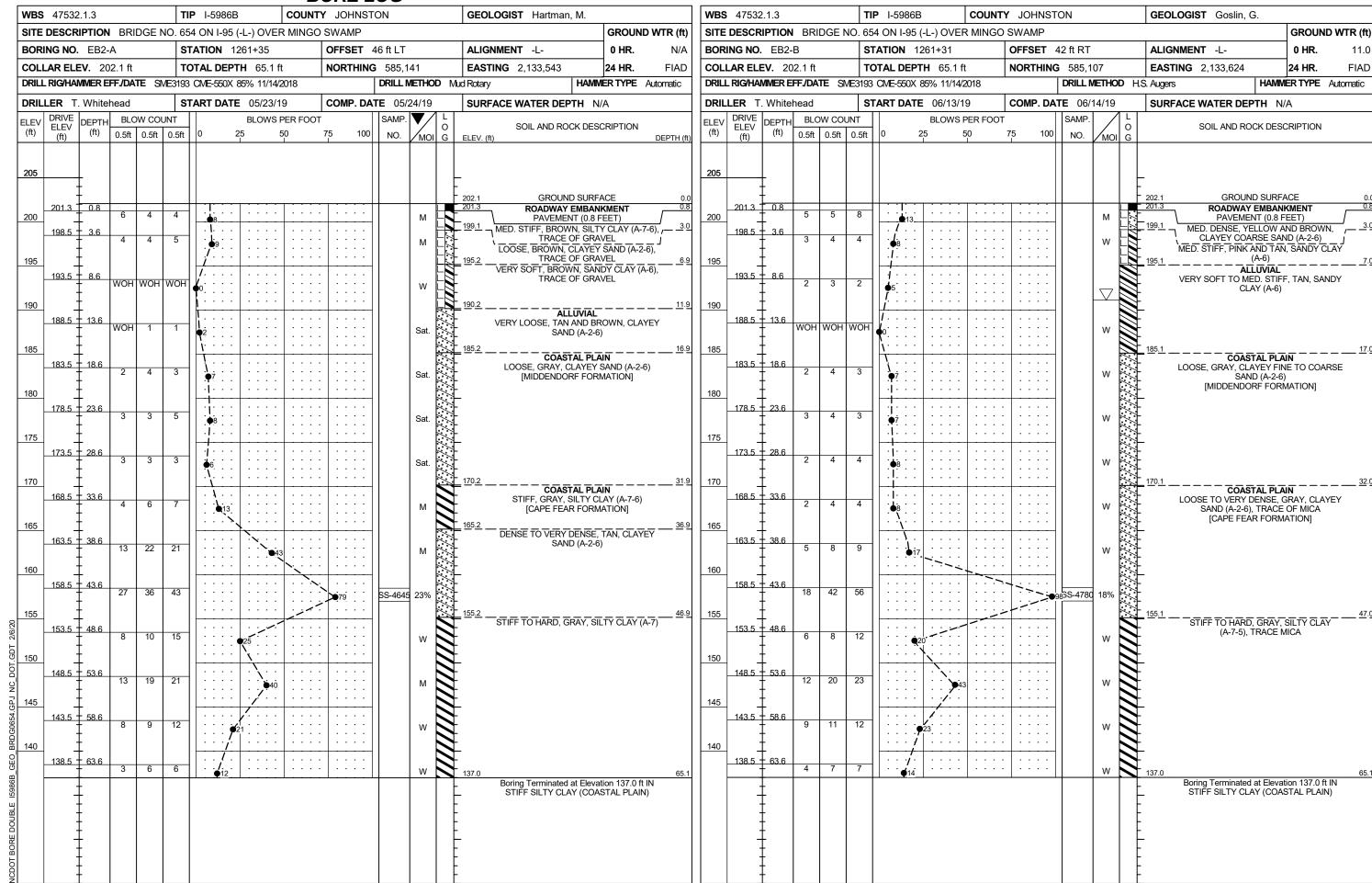


N/A

10.8







# GEOTECHNICAL BORING REPORT

SHEET 18

								ORE L				T			
	47532					<b>P</b> I-5986B		JOHNST	ON			GEOLOGIST Blonesh	ine, E.	1	
				DGE N		54 ON I-95 (-L-) OVEF						Т		1	D WTR (ft)
	ING NO.					<b>TATION</b> 1261+39		OFFSET (				ALIGNMENT -L-		0 HR.	N/A
	LAR ELE					OTAL DEPTH 65.0 f		NORTHING				<b>EASTING</b> 2,133,582		24 HR.	10.1
DRILL	_ RIG/HAI	VIMER E	FF./DA	TE SI	ИE3193	CME-550X 85% 11/14/2	2018		DRILL N	/IETHO	D M	ud Rotary	HAMM	ER TYPE	Automatic
DRIL	LER T.	White				TART DATE 06/04/1	9	COMP. DA	<b>FE</b> 06/	04/19	<del></del>	SURFACE WATER DEI	PTH N/	Ά	
LEV (ft)	DRIVE ELEV	DEPTH (ft)		W COL	_	<b> </b>	PER FOOT	75 400	SAMP.	▼/		SOIL AND RO	CK DESC	CRIPTION	
(11)	(ft)	(11)	0.5ft	0.5ft	0.5ft	0 25	50 T	75 100	NO.	/MOI	G	ELEV. (ft)			DEPTH (1
205		_										_			
	-											•			
200	201.0	0.0	7	5	5	10	L			М		201.0 GROUN ROADWAY	ID SURFA EMBANI		С
		[ <u></u>				7.10				'*'		VERY LOOSE TO RED, CLAYEY	LOOSE,	<b>BROWN A</b>	
	197.5	- 3.5 -	3	4	3					Sat.				(, , , _ 0)	
195	_	Ī										<del>-</del>			
	192.5	- - 8.5			0							•			
190	-		2	1	0	1						•			
		[ 									H		LUVIAL		12
	187.5	- 13.5 -	2	1	2				SS-4707	34%		SOFT, GRAY, SILT OF ORG	Y CLAY (	A-7-6), TR	ACE
185	_	_				1							4410 1411		17
	182.5	- - 18.5											TAL PLA		
180	-	-	3	3	4	7				Sat.			A-2-6)		5
	-	-				1						_ [MIDDENDC	KF FUKI	MATION	
	177.5 -	- 23.5 -	3	5	5	·   · ·   · · · · · · · · · · · · ·				Sat.		•			
175	_	-				1 - 1						· <del>-</del>			
	172.5	- - 28.5										•			
170	-	_	5	4	4	8				Sat.		•			
170	-	-				: 1 : : : : : : : : :									32
	167.5	- 33.5 -	3	6	7	\frac{7}{7}				М		STIFF, GRA	TAL PLA AY, CLAY	(A-7-6)	
165	_	-				13				'''		. [CAPÉ FEA -	R FORM	ATIONJ	
	- 162.5	- - 38.5													
400	-	-	4	6	8					М					
160	_	_										159.0			42
	157.5	43.5	40	60/0.3				\*\\.\_\_	SS-4713	21%		VERY DENSE, GR.	AY, SILTY	SAND (A-	·2-4)
155	_	_						100/0.8				• <del>-</del>			
	152.5	- - 48.5					-:								ARK 47
450	-102.0	- <del></del>	6	8	12	20				М		. GRAY,	CLAY (A-	7-6)	
150	_	<u>_</u> _										<del>_</del>			
	147.5	53.5	15	27	28	:::: ::::				١,,					
145	-	_					<b>9</b> 55			M		<del>-</del>			
	142.5	- - 58.5				:::: ::/:									
440	142.0	-	7	11	14	25				М					
140	_	_				<del>  / </del>						<del>-</del>			
	137.5	- 63.5 -	5	8	9	$\left \left \begin{array}{c} \dots, f_{i-1}^l \\ \dots, f_{i-1}^l \end{array}\right  \right  \dots \right $				м					
	_	_		Ť		<u> </u>	L	I I	1	IVI		136.0 Boring Terminated	l at Elevat	ion 136.0 f	t IN
	-	-										. VERY STIFF CL	AY (COAS	STAL PLAII	N)
	-	-													
	_	-										<del>-</del>			
	-	-													
	-	-									1				

#### **SUMMARY OF LABORATORY TEST DATA**

Soil Classification and Gradation



	S&ME, Inc. Raleigh, 3201	Spring Forest Road, Raleigh	n, North Carolina 2761	6	
S&ME Project #:	6235-17-048			Date Report:	8/1/2019
State Project No.:	47532.1.3	County:	Johnston	Date Tested:	7/1/2019
Federal ID No.:	N/A	TIP No.:	I-5986B		
Project Name:	Br No 654 on 1-95 (-1-) ov	or Mingo Swamp			

Project Name:

Br. No. 654 on I-95 (-L-) over Mingo Swamp

Client Name:

Michael Baker International

Client Address: Raleigh, NC

Client Nam	ie.			Michael Ba	iker inter	nation	dl		Client F	laaress.	Raieign,	, INC						
				Sample	AASH	-TO	Total % Passing			Total Mortar Fraction (%)								
Sample				Depth	Classific	cation		Siev	/e #		Coarse	Fine						Moist.
No.	Station	Offset	Alignment	(ft)			10	40	60	200	Sand	Sand	Silt	Clay	LL	PL	PI	%
SS-4645	1261+35	46' LT	-L-	43.6-45.1	A-2-6	(0)	100	89	ı	24.1	32	48	10	10	36	19	17	22.5
SS-4662	1260+78	44' LT	-L-	58.4-59.9	A-7-5	(10)	100	93	ı	65.5	16	28	42	14	46	31	15	22.7
SS-4679	1259+90	41' LT	-L-	53.4-54.9	A-7-6	(20)	100	99	1	81.1	2	33	44	21	51	29	22	23.8
SS-4693	1259+18	43' LT	-L-	38.5-40.0	A-7-6	(3)	99	68	-	36.7	46	19	4	31	41	17	24	22.0
SS-4696	1259+18	43' LT	-L-	53.5-55.0	A-7-6	(15)	100	95	-	68.3	10	33	40	17	50	28	22	22.6
SS-4707	1261+39	6' LT	-L-	13.5-15.0	A-7-6	(3)	87	82	-	35.8	15	47	3	35	44	19	25	33.5
SS-4713	1261+39	6' LT	-L-	43.5-45.0	A-2-4	(0)	100	87	-	33.5	28	43	16	13	23	14	9	20.5
SS-4726	1260+79	8' LT	-L-	38.5-40.0	A-4	(2)	100	99	-	17.3	2	56	24	18	31	22	9	17.3
SS-4746	1259+89	7' LT	-L-	38.4-39.9	A-4	(1)	100	100	-	47.2	3	60	17	20	25	16	9	20.0
SS-4752	1259+89	7' LT	-L-	68.4-69.9	A-7-5	(22)	100	96	-	84.5	7	17	58	18	54	32	22	30.1
SS-4759	1259+21	7' LT	-L-	18.6-20.1	A-1-b	(1)	50	43	-	4.6	64	28	3	5	N.P.	N.P.	N.P.	13.5
SS-4762	1259+21	7' LT	-L-	33.6-35.1	A-7-6	(30)	100	97	-	87.9	4	12	25	59	53	21	32	34.6
SS-4766	1259+21	7' LT	-L-	53.5-55.1	A-7-6	(16)	100	96	-	73.6	9	29	50	12	50	29	21	25.5
SS-4780	1261+31	42' RT	-L-	43.6-45.1	A-2-6	(0)	99	79	-	28.7	37	40	19	4	31	20	11	17.5
SS-4798	1260+84	46' RT	-L-	63.4-64.9	A-7-6	(16)	100	97	-	76.7	6	21	53	20	49	29	20	19.6
SS-4810	1259+83	52' RT	-L-	33.4-34.9	A-7-6	(32)	100	95	-	77.2	9	20	23	48	58	17	41	21.2
SS-4820	1259+83	52' RT	-L-	83.4-84.9	A-2-6	(0)	100	71	-	30.7	46	27	14	13	32	19	13	18.1
SS-4826	1259+24	46' RT	-L-	23.5-25.0	A-3	(1)	99	53	-	4.9	86	10	1	3	N.P.	N.P.	N.P.	23.9
SS-4834	1259+24	46' RT	-L-	63.5-65.0	A-7-6	(25)	100	99	-	85.9	2	28	49	21	52	25	27	24.7
Poforoncos / Commonts / Dovistions: Poforoncos / Commonts / De Poforoncos / Commonts / Dovistion Poforoncos / Commonts / Deforoncos / Commonts / Dovistion Poforoncos /												/						

References / Comments / Deviations: References / Comments / Deviation References / Deviation References / Deviation References / Comments / Deviation References / Deviation Ref

AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT

AASHTO T89: Determining the Liquid Limit of Soils

AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils

AASHTO T265: Laboratory Determination of Moisture Content of Soils

AASHTO M145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

Mal Krajan, ET

Technician Name:

Technician Name:

Technician Name:

Technician Name:

Technician Name:

Technician Name:

Technical Responsibility:

Position

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## SITE PHOTOGRAPH

Bridge No. 654 on –L– (I-95) over Mingo Swamp

