

#### **─** -Y17RPA-**≺** -SR17-**├**— -Lr 13′−9″± r 10′−0″± 12'-6"± --r-10′-0″± r—10′−0″± 10'-0"± ---.15'-0"±, 30'-0"± 40'-0"± 23′-3″± 29'-3"± 25'-0"± . 16′-3″± | 8'-3"± 10′−1″± . APPROXIMATE NATURAL -GROUND LINE

NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

MAXIMUM DESIGN FILL = 5.46'.

OF ALL VERTICAL WALLS.

MINIMUM DESIGN FILL = 2.35'.

FOR OTHER DESIGN DATA AND NOTES, SEE STANDARD NOTE SHEET.

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN STAGE I AND STAGE II OF CULVERT TO BE POURED IN THE FOLLOWING ORDER:

- 1. WING FOOTINGS AND FLOOR SLAB INCLUDING 4"
- 2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALLS.

CONCRETE IN STAGE III OF CULVERT TO BE POURED IN THE FOLLOWING ORDER:

- 1. FLOOR SLAB INCLUDING 4"OF ALL VERTICAL WALLS.
- 2. THE REMAINING PORTIONS OF THE WALLS FOLLOWED BY ROOF SLAB.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED IN THE BARREL. SPACED TO LIMIT THE POURS TO A MAXIMUM OF 70 FEET. LOCATION OF JOINTS SHALL BE SUBJECT TO APPROVAL OF THE ENGINEER.

STEEL IN THE BOTTOM SLAB MAY BE SPLICED AT THE PERMITTED CONSTRUCTION JOINT AT THE CONTRACTOR'S OPTION. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES WILL BE PAID FOR BY THE CONTRACTOR.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WALL AND BOTH FACES OF INTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT, THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS, EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC. SEE TRAFFIC CONTROL PLANS, FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS.

- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- THE COST FOR REMOVAL OF THE EXISTING TWO 48" Ø RCP SHALL BE INCLUDED IN THE PAY ITEM FOR "CULVERT EXCAVATION".

PROFILE ALONG & CULVERT

#### ROADWAY DATA

GRADE POINT ELEV. @ STATION 1220+34.00 -L- = 234.23 BED ELEV. @ STATION 1220+34.00 -L-= 223.13 ROADWAY SLOPES = 3:1

#### HYDRAULIC DATA

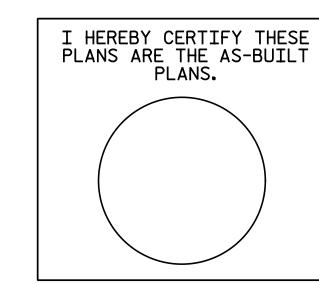
DESIGN DISCHARGE = 420 C.F.S. FREQUENCY OF DESIGN FLOOD = 50 YRS. DESIGN HIGH WATER ELEVATION = 230.00 = 0.93 SQ. MI. DRAINAGE AREA BASE DISCHARGE (Q100) = 460 C.F.S. BASE HIGH WATER ELEVATION = 230.40

#### OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE = 1070 C.F.S. FREQUENCY OF OVERTOPPING FLOOD = >500 YRS. OVERTOPPING FLOOD ELEVATION = 234.20

NOTE: OVERTOPS ROADWAY @ STA. -L- 1220+87.00

TOTAL STRUCT	TURE C	TITMAU	IES
CULVERT EXCAVATION		LUN	MP SUM
FOUNDATION CONDITIO	NING MATE	ERIAL	
STAGE I		94	_TONS
STAGE II		183	TONS
STAGE III		94	TONS
	TOTAL	371	_TONS
CLASS A CONCRETE			
STAGE I		112.8	C.Y.
STAGE II		207.1	C.Y.
STAGE III		99.8	C.Y.
	TOTAL	419.7	C.Y.
REINFORCING STEEL			
STAGE I		14,426	LBS.
STAGE II			
STAGE III		13,515	LBS.
	TOTAL	55,065	LBS.



PROJECT NO. I-5883 HARNETT COUNTY STATION: 1220+34.00 -L-

SHEET 1 OF 5

DOCUMENT NOT CONSIDERED FINAL

UNLESS ALL SIGNATURES COMPLETED

DOUBLE 7 FT. X 6 FT. CONCRETE BOX CULVERT

SHEET NO.

CI-I

DATE:

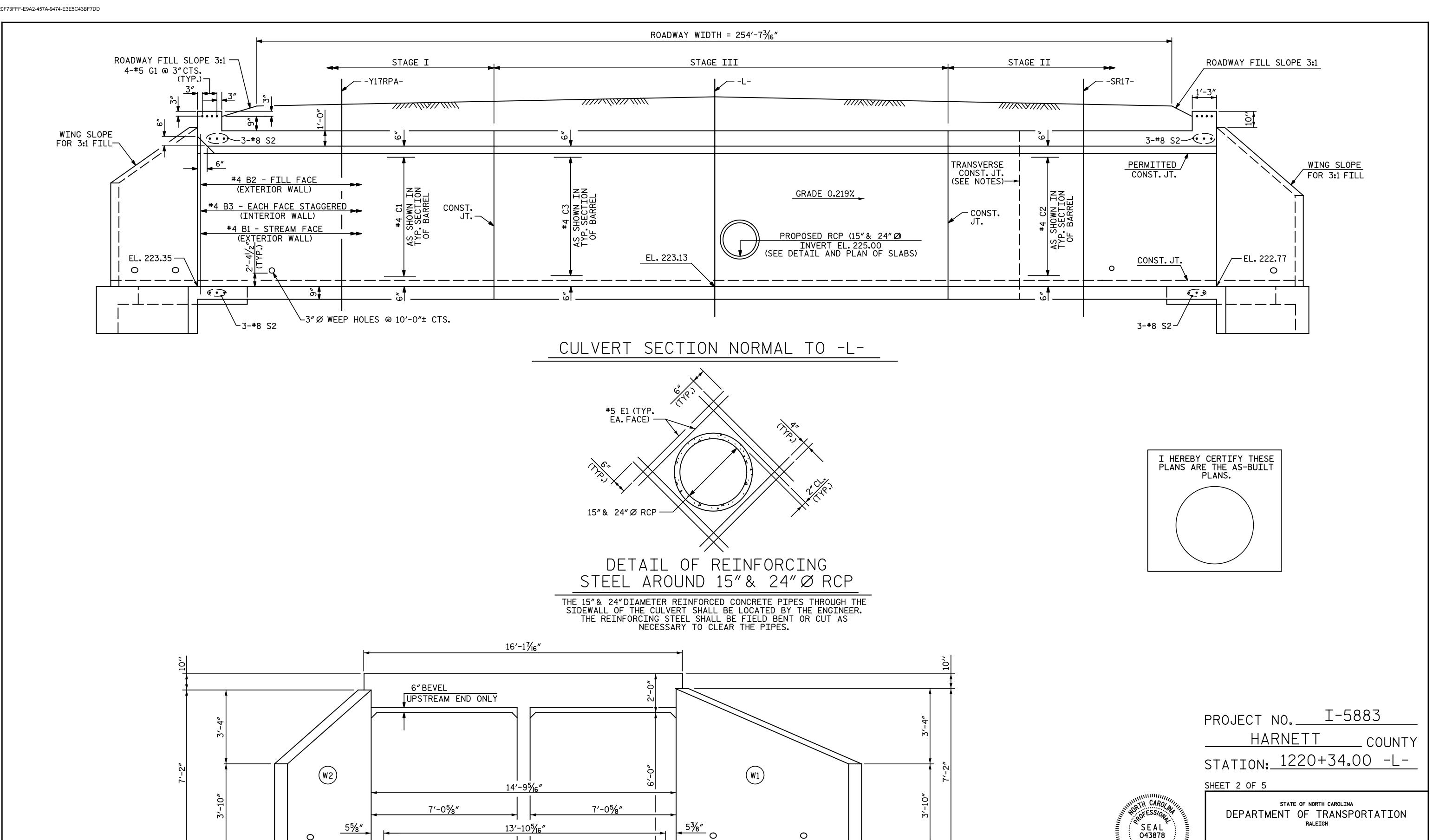
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

87° SKEW

**REVISIONS** NO. BY: DATE: BY: 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 INTERNATIONAL NC License No.: F-1084

DRAWN BY : N. B. SPEAKS DATE : 3-13-19 CHECKED BY: A. H. SHARPE DATE: 4-26-21



END ELEVATION NORMAL TO SKEW (INVERT END OF CULVERT SHOWN, OUTLET END OF CULVERT SIMILAR BUT WINGS OPPOSITE) DRAWN BY : N. B. SPEAKS DATE : 3-13-19 CHECKED BY : A. H. SHARPE DATE : 2-25-20

0

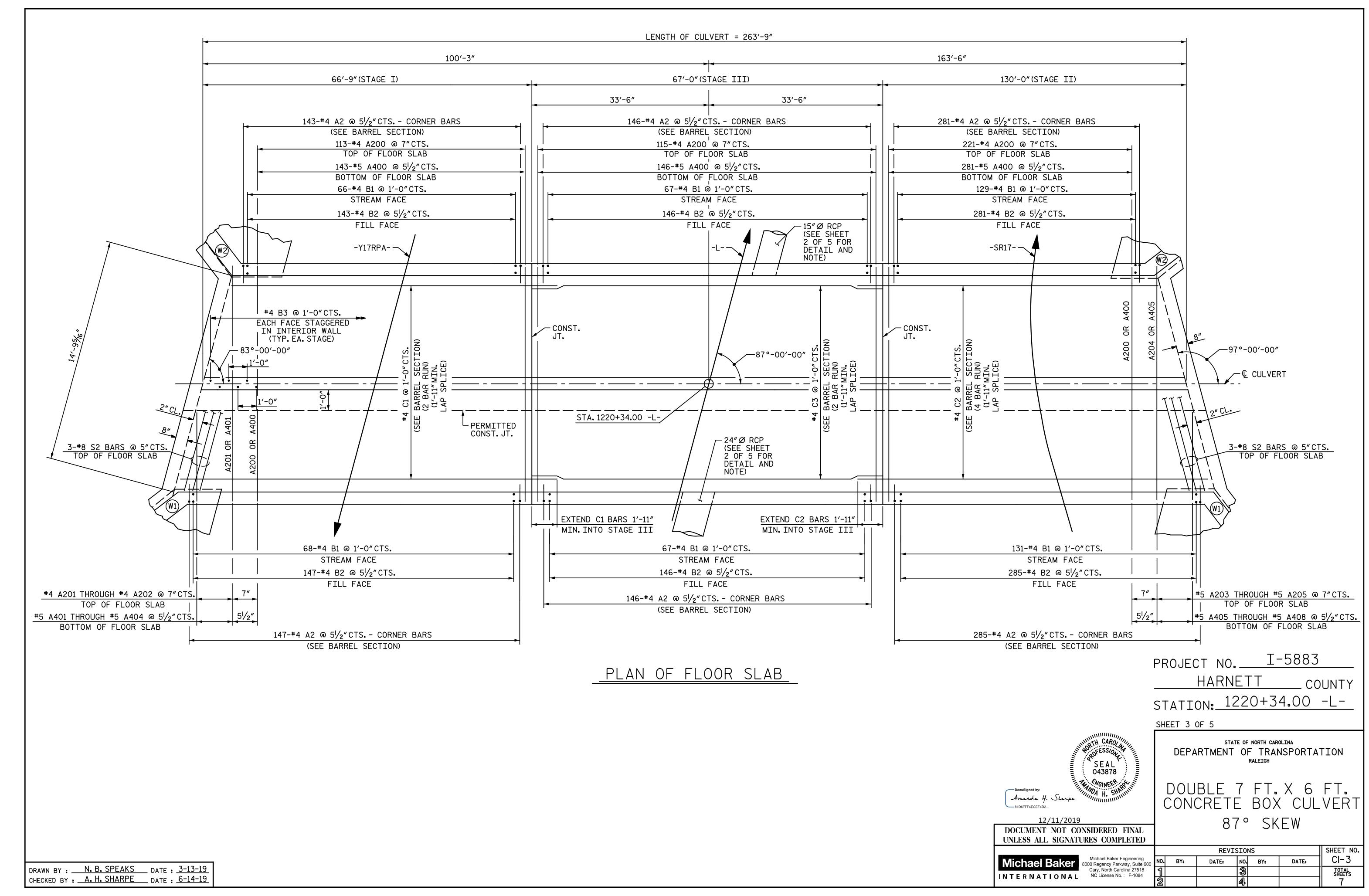
WGINEER SHARW Amanda 4. Sharpe 4/7/2020

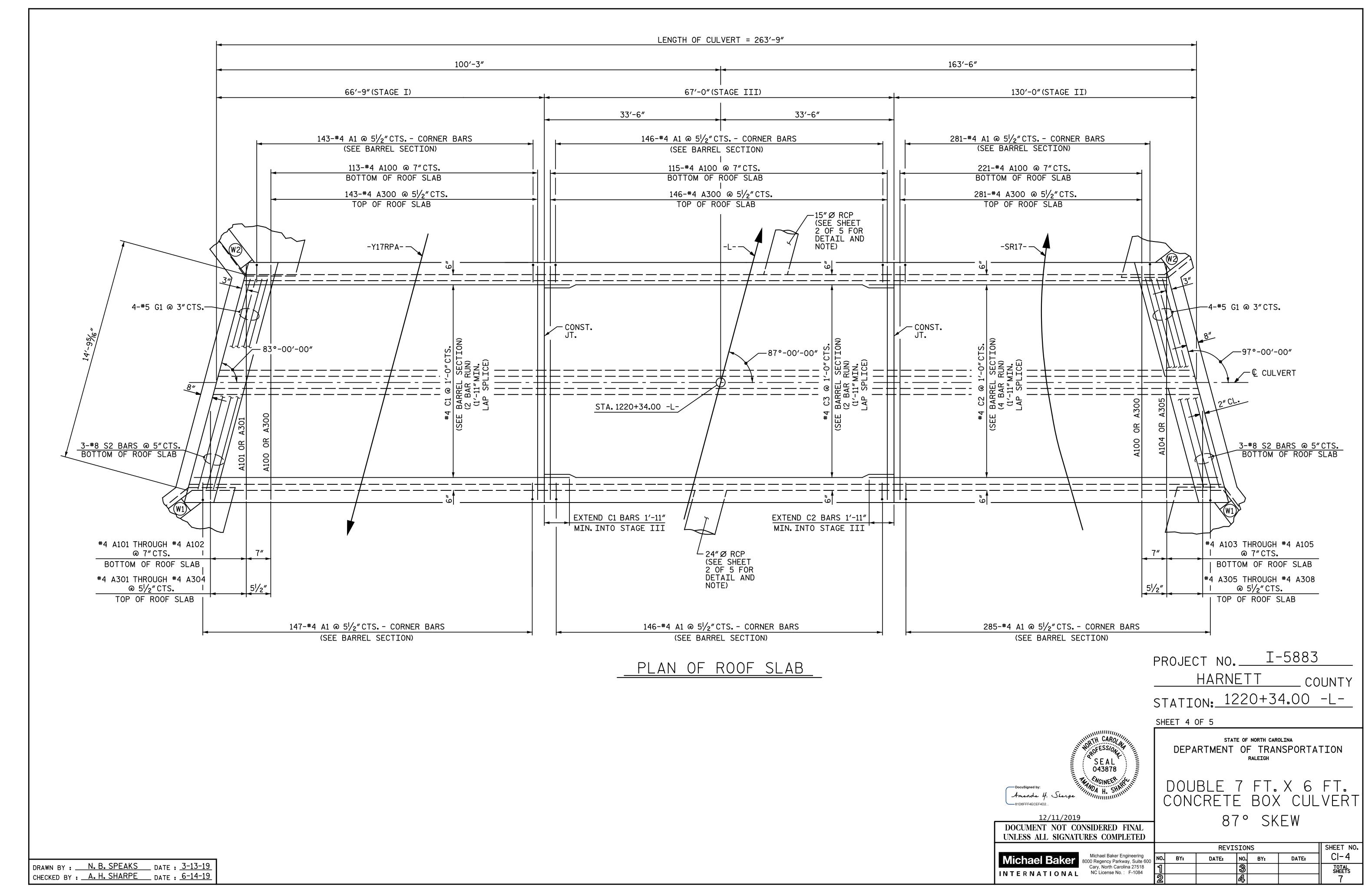
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DOUBLE 7 FT. X 6 FT. CONCRETE BOX CULVERT

87° SKEW

REVISIONS SHEET NO. Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084 DATE: NO. BY: CI-2 DATE: TOTAL SHEETS

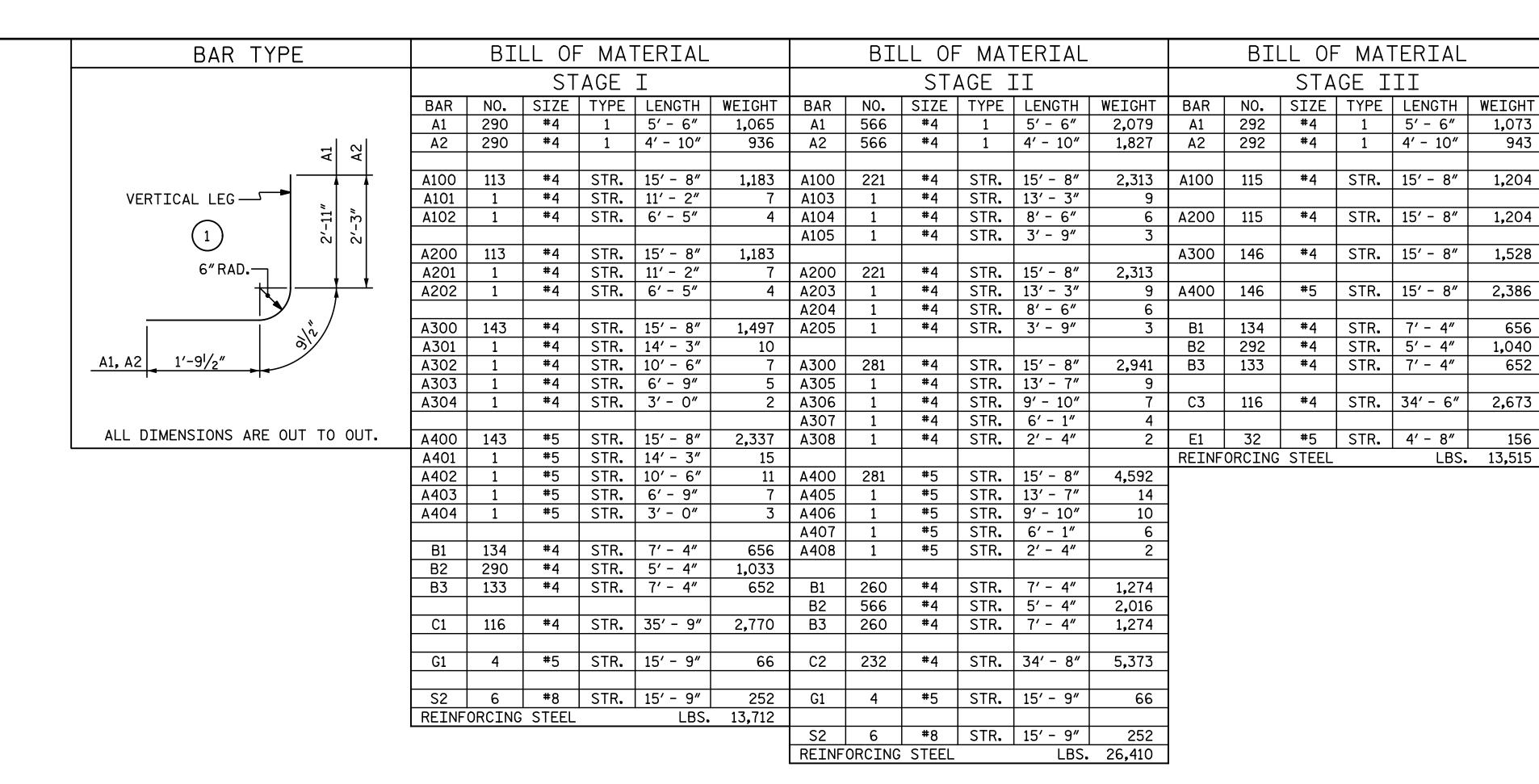


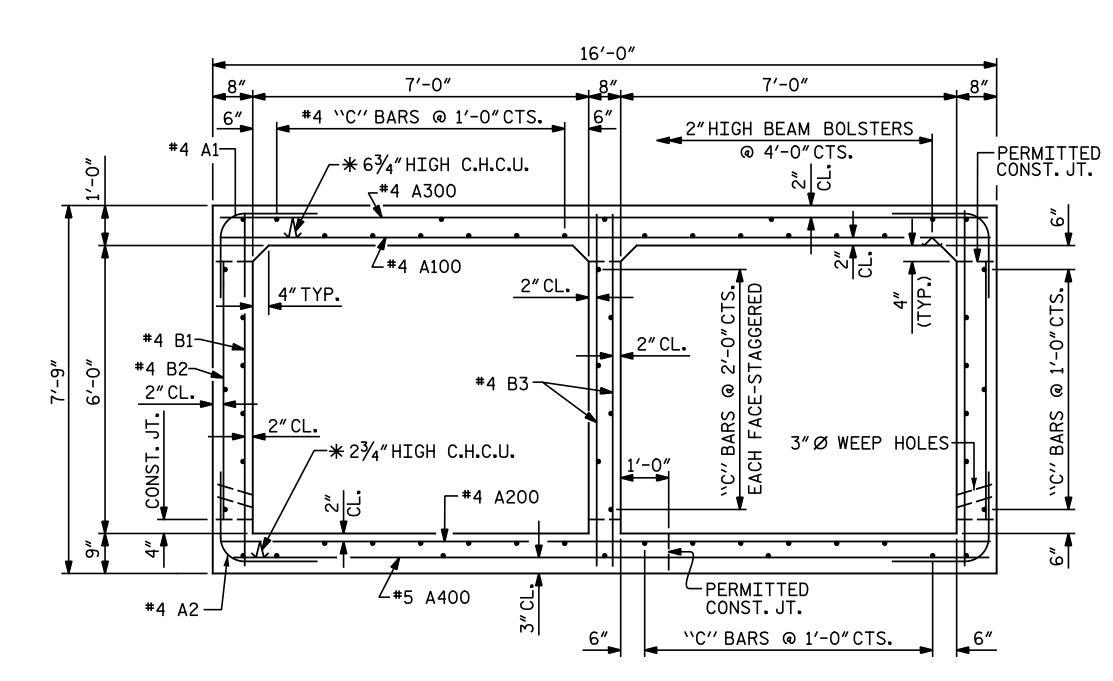


STAGE I QUANTITIES	
CULVERT EXCAVATION LUMP SUM	
FOUNDATION CONDITIONING MATERIAL 94 TONS	
CLASS A CONCRETE         BARREL @CY/FT99.4       C.Y.         WINGS, ETC13.4       C.Y.         TOTAL112.8       C.Y.	
REINFORCING STEEL  BARREL	

STAGE II QUANTITIES	
CULVERT EXCAVATION LUMP SUM	
FOUNDATION CONDITIONING MATERIAL 183 TONS	
CLASS A CONCRETE         BARREL @	
REINFORCING STEEL  BARREL	

STAGE III QUANT	ΓITIES	
CULVERT EXCAVATION	LUMP	SUM
FOUNDATION CONDITIONING MATERI	TAL <u>94</u>	_TONS
CLASS A CONCRETE  BARREL @ CY/FT  WINGS, ETC  TOTAL	99.8 0.0 99.8	_C.Y.
REINFORCING STEEL  BARREL WINGS, ETC  TOTAL	13 <b>,</b> 515 0 13 <b>,</b> 515	_LBS.





## RIGHT ANGLE SECTION OF BARREL

THERE ARE 58 "C" BARS IN SECTION OF BARREL. \* ALL CONTINUOUS HIGH CHAIR UPPER (C.H.C.U.)@ 3'-0"CTS.

SPLICE LENGTH CHART							
BAR	SIZE	SPLICE LENGTH					
A200	#4	2′-5″					
A400	#5	2′-2″					
B1, B3	#4	1′-5″					
C1, C2, C3	#4	1′-11″					

STAGE III

1 | 5' - 6" |

1,204

2,386

1,040

LBS. 13,515

652

#4

PROJECT NO. I-5883

HARNETT \_\_\_ COUNTY

STATION: 1220+34.00 -L-

SHEET 5 OF 5

Amanda H. Sharpe —81D6FFF4ECEF4D2...

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

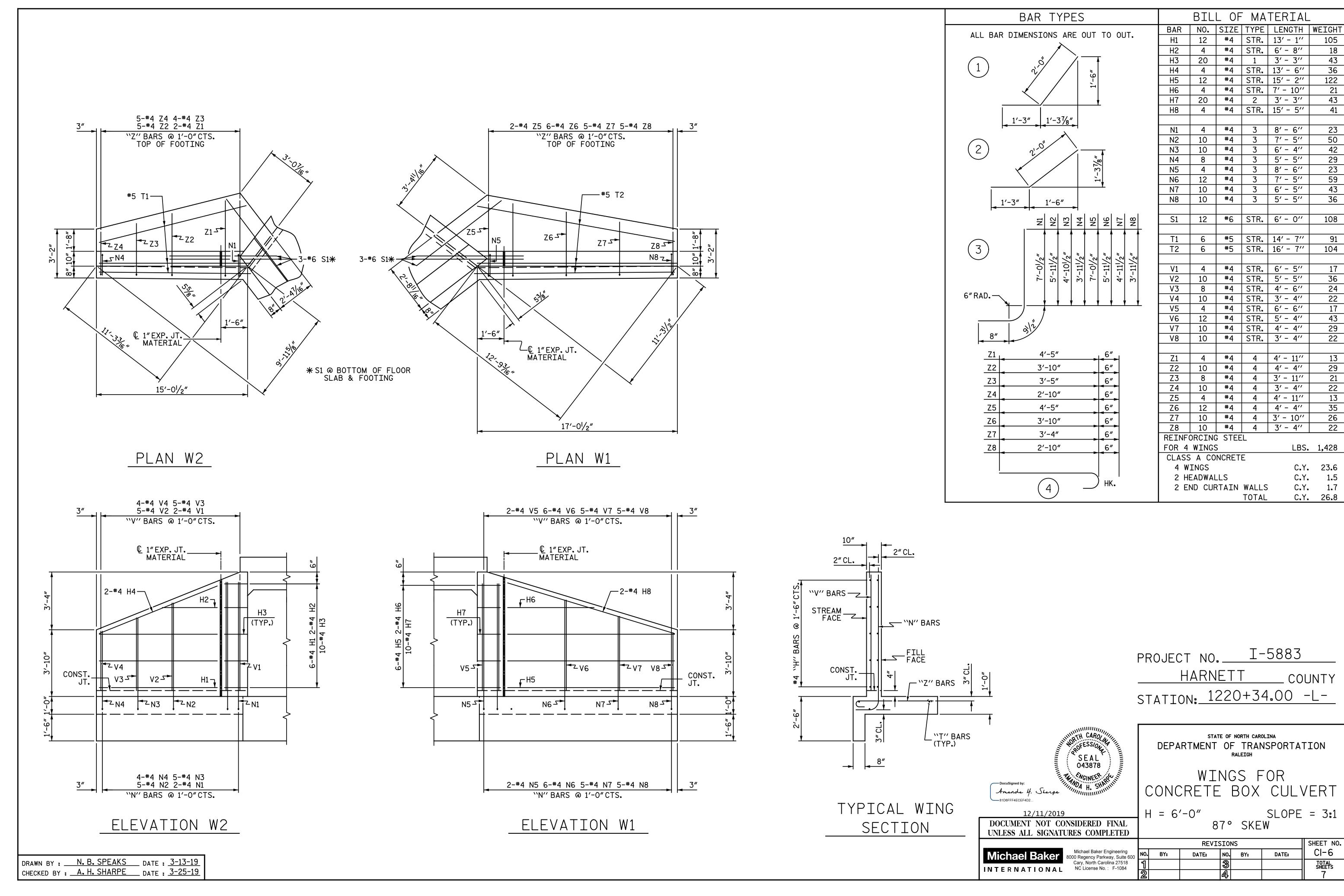
CONCRETE BOX CULVERT 87° SKEW

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Michael Baker	Michael Baker Engineering 8000 Regency Parkway, Suite Cary, North Carolina 27518				
INTERNATIONAL	NC License No.: F-1084				

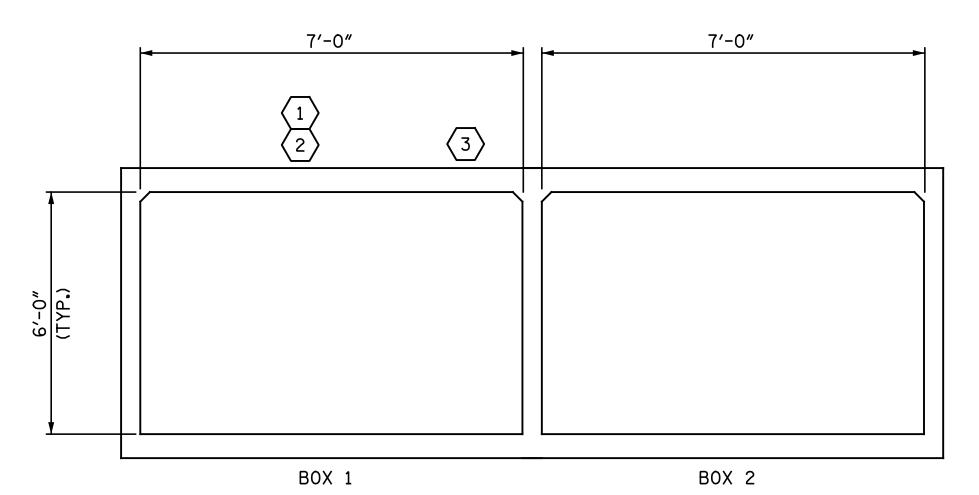
					_	_
		REVIS	SIO	NS		SHEET NO.
10.	BY:	DATE	NO.	BY:	DATE:	CI-5
1			<b>®</b>			TOTAL SHEETS
2			<u>A</u> J			l 7

DRAWN BY : N. B. SPEAKS DATE : 3-13-19 CHECKED BY : A. H. SHARPE DATE : 6-14-19



# LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

							STRENGTH I LIMIT STATE									
										MOMENT				SHEAR		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y <sub>LL</sub> )	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1 <b>.</b> 15		1 <b>.</b> 75	1 <b>.</b> 15	1	TOP SLAB	3.07	1.17	1	TOP SLAB	6.61	
DESIGN		HL-93 (OPERATING )	N/A		1.49		1 <b>.</b> 35	1.49	1	TOP SLAB	3.07	1 <b>.</b> 52	1	TOP SLAB	6.61	
DESIGN LOAD RATING		HS-20 (INVENTORY)	36.000	2	1.15	41.40	1 <b>.</b> 75	1 <b>.</b> 15	1	TOP SLAB	3 <b>.</b> 07	1.23	1	TOP SLAB	6 <b>.</b> 61	
		HS-20 (OPERATING)	36.000		1 <b>.</b> 50	54.00	1 <b>.</b> 35	1 <b>.</b> 50	1	TOP SLAB	3 <b>.</b> 07	1.60	1	TOP SLAB	6 <b>.</b> 61	
		SH	12.500		2.31	28.88	1.40	2.31	1	TOP SLAB	3.07	2.84	1	TOP SLAB	6 <b>.</b> 61	
		S3C	21.500		2.11	45.37	1.40	2.11	1	TOP SLAB	3.07	2.15	1	TOP SLAB	6.61	
	CLE	S3A	22.750		2.11	48.00	1.40	2.11	1	TOP SLAB	3.07	2.11	1	TOP SLAB	6.61	
	VEHICLE (V)	S4A	26.750		2.10	56.18	1.40	2.11	1	TOP SLAB	3.07	2.10	1	TOP SLAB	6.61	
	. Е. V (SV	S5A	30.500		2.04	62.22	1.40	2.19	1	TOP SLAB	3.07	2.04	1	TOP SLAB	6.61	
LEGAL	SINGLE \	S6A	34.500		1.98	68.31	1.40	2,20	1	TOP SLAB	3.07	1.98	1	TOP SLAB	6.61	
LOAD RATING	.S.	S7B	38.500	(3)	1.97	75 <b>.</b> 85	1.40	2.23	1	TOP SLAB	3.07	1.97	1	TOP SLAB	6.61	
		S7A	40.000		1.98	79.20	1.40	2.20	1	TOP SLAB	3.07	1.98	1	TOP SLAB	6.61	
	٣. ٣.	T4A	28.250		2.11	59.61	1.40	2.11	1	TOP SLAB	3.07	2.11	1	TOP SLAB	6.61	
	ACTO ILLEF	T5B	32.000		2.10	67.20	1.40	2.11	1	TOP SLAB	3.07	2.10	1	TOP SLAB	6.61	
	TRUCK TRACTOR SEMI-TRAILER (TTST)	T6A	36.000		2,08	74.88	1.40	2.11	1	TOP SLAB	<b>3.</b> 07	2.08	1	TOP SLAB	6.61	
	RUCK EMI	T7A	40.000		2.08	83.20	1.40	2.11	1	TOP SLAB	3.07	2.08	1	TOP SLAB	6.61	
	<del>-</del> 0	T7B	40.000		2.00	80.00	1.40	2.11	1	TOP SLAB	3.07	2.00	1	TOP SLAB	6.61	



LRFR SUMMARY
(LOOKING DOWNSTREAM)

DRAWN BY: N. B. SPEAKS DATE: 3-13-19
CHECKED BY: A. H. SHARPE DATE: 3-25-19

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR		
DC	1.25	0.90		
DW	1 <b>.</b> 50	0.65		
EV	1.30	0.90		
EH	1 <b>.</b> 35	0.90		
ES	1 <b>.</b> 35	0.90		
LS	1.75			
WA	1.00			

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

PROJECT NO. I-5883

HARNETT COUNTY

STATION: 1220+34.00 -L-



12/11/2019

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 NC License No.: F-1084

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

LRFR SUMMARY FOR
REINFORCED CONCRETE
BOX CULVERTS
(INTERSTATE TRAFFIC)

REVISIONS

BY: DATE: NO. BY: DATE: CI-7

3 TOTAL SHEETS
7

### STANDARD NOTES

#### DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) LIVE LOAD ----- SEE PLANS IMPACT ALLOWANCE - - - - - - - - - - - SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - - 27.000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION - GRADE 60 - - - 24.000 LBS. PER SQ. IN. STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS - - - 1,800 LBS, PER SQ, IN. COMPRESSION PERPENDICULAR TO GRAIN ---- 375 LBS, PER SQ, IN, OF TIMBER ---- 30 LBS. PER CU. FT. EQUIVALENT FLUID PRESSURE OF EARTH (MINIMUM)

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

#### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

#### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

## ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

#### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

#### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{1}{8}$ " Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 -  $\frac{1}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{1}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 -  $\frac{1}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST \( \frac{5}{16}'' \) IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

#### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

#### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH