

ELEVATION

REINFORCING STEEL DIMENSIONS AND DETAILS ARE TYPICAL FOR EACH COLUMN AND FOOTING. DIMENSIONS AND PILE LOCATIONS ARE TYPICAL FOR EACH FOOTING.

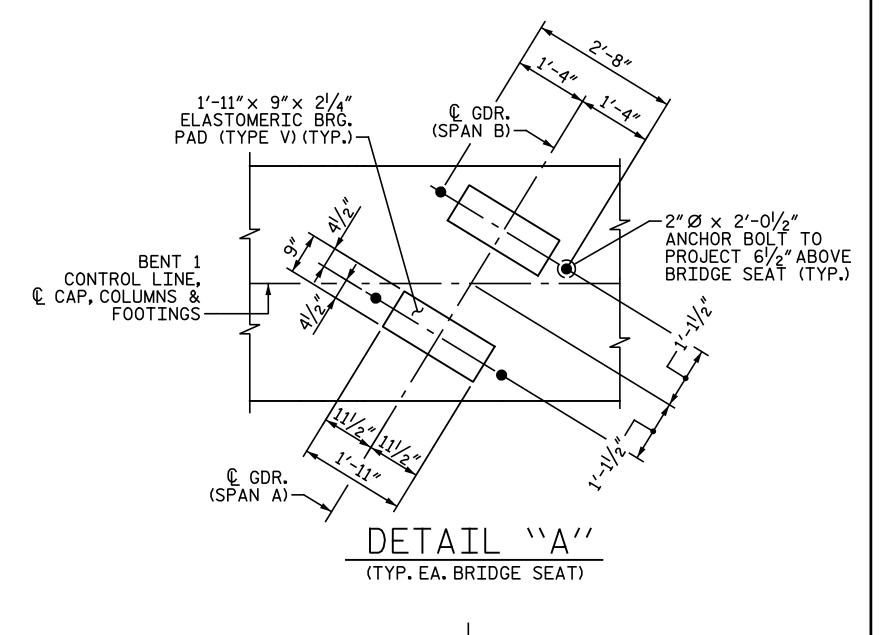
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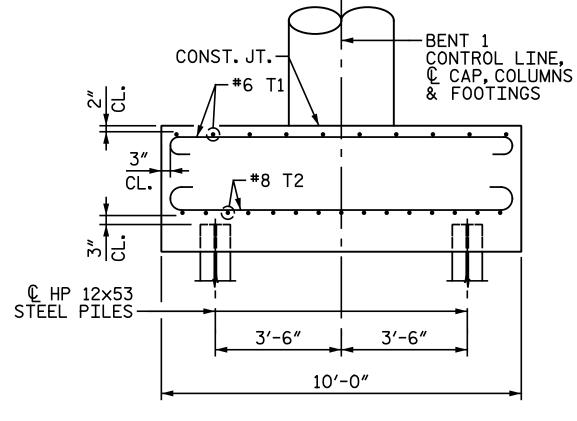
STIRRUPS AND "U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS AND COLUMN REINFORCING STEEL.

FOR "SECTION A-A" AND "SECTION B-B", SEE "BENT 1 DETAILS" SHEET.

HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

FOR PILE SPLICE DETAILS, SEE "END BENT 2 DETAILS" SHEET.





PARTIAL END ELEVATION

(FOR COLUMN REINFORCING STEEL, SEE ELEVATION)

PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 28+39.21 -Y29-



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE

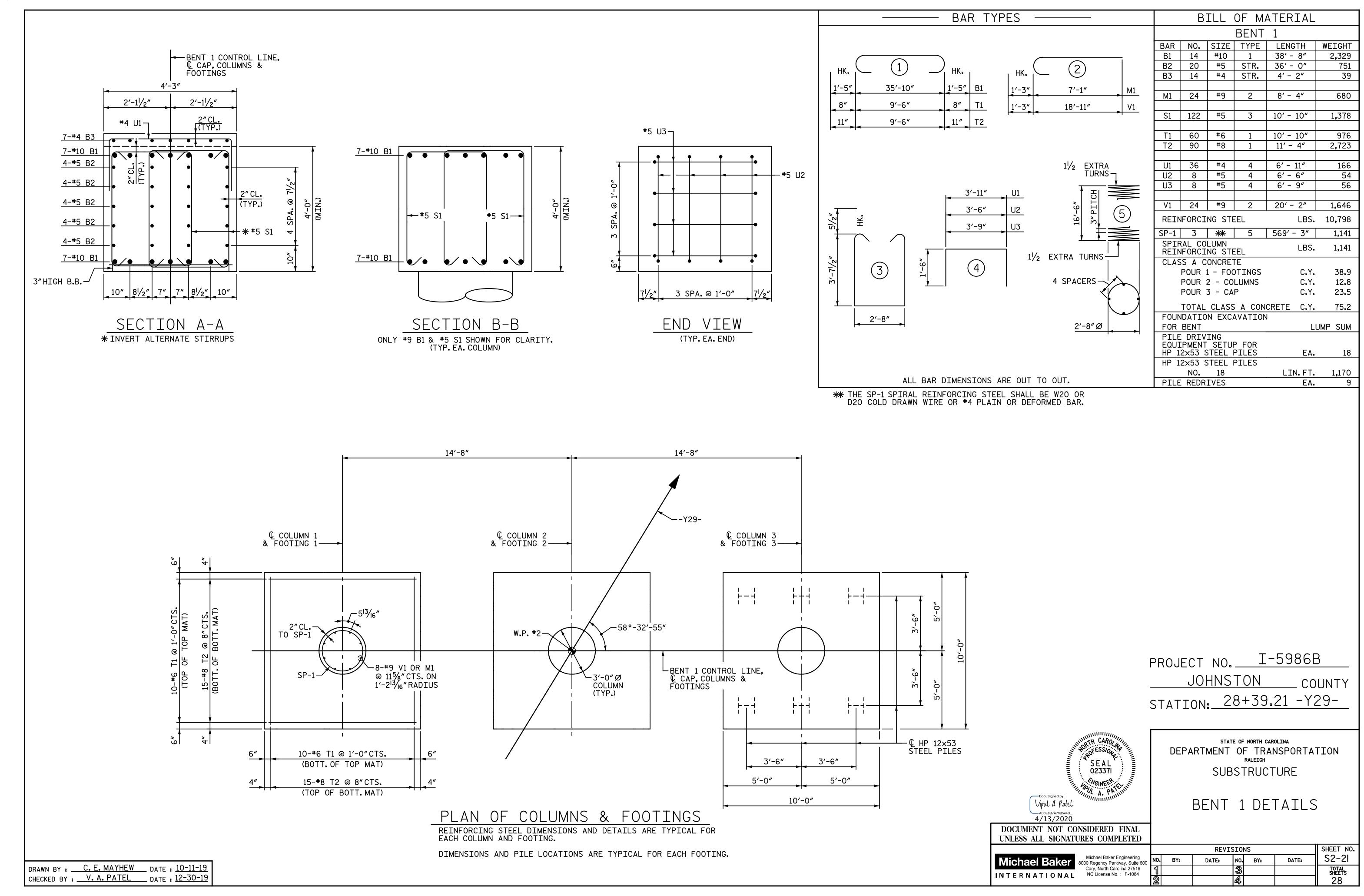
BENT 1

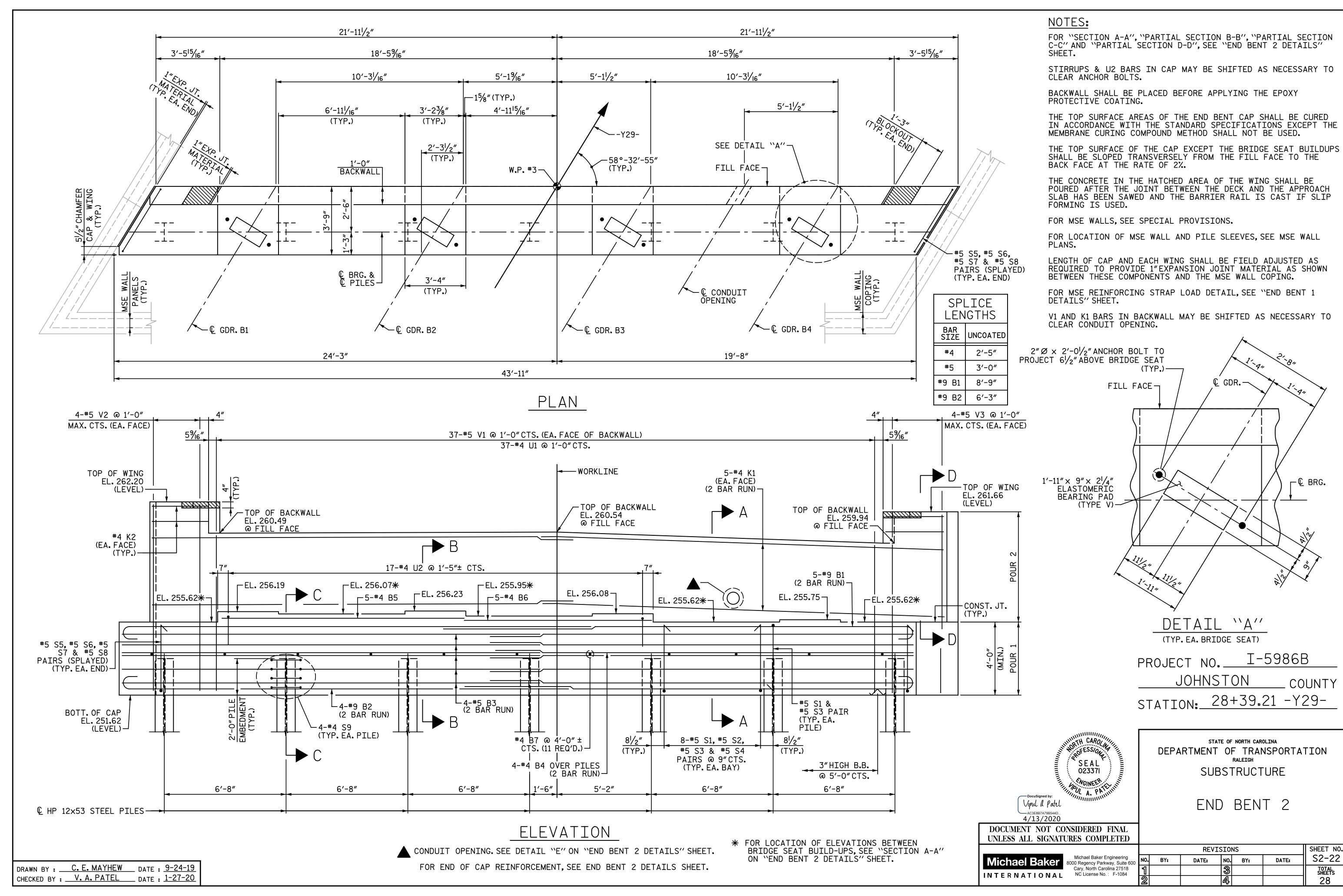
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

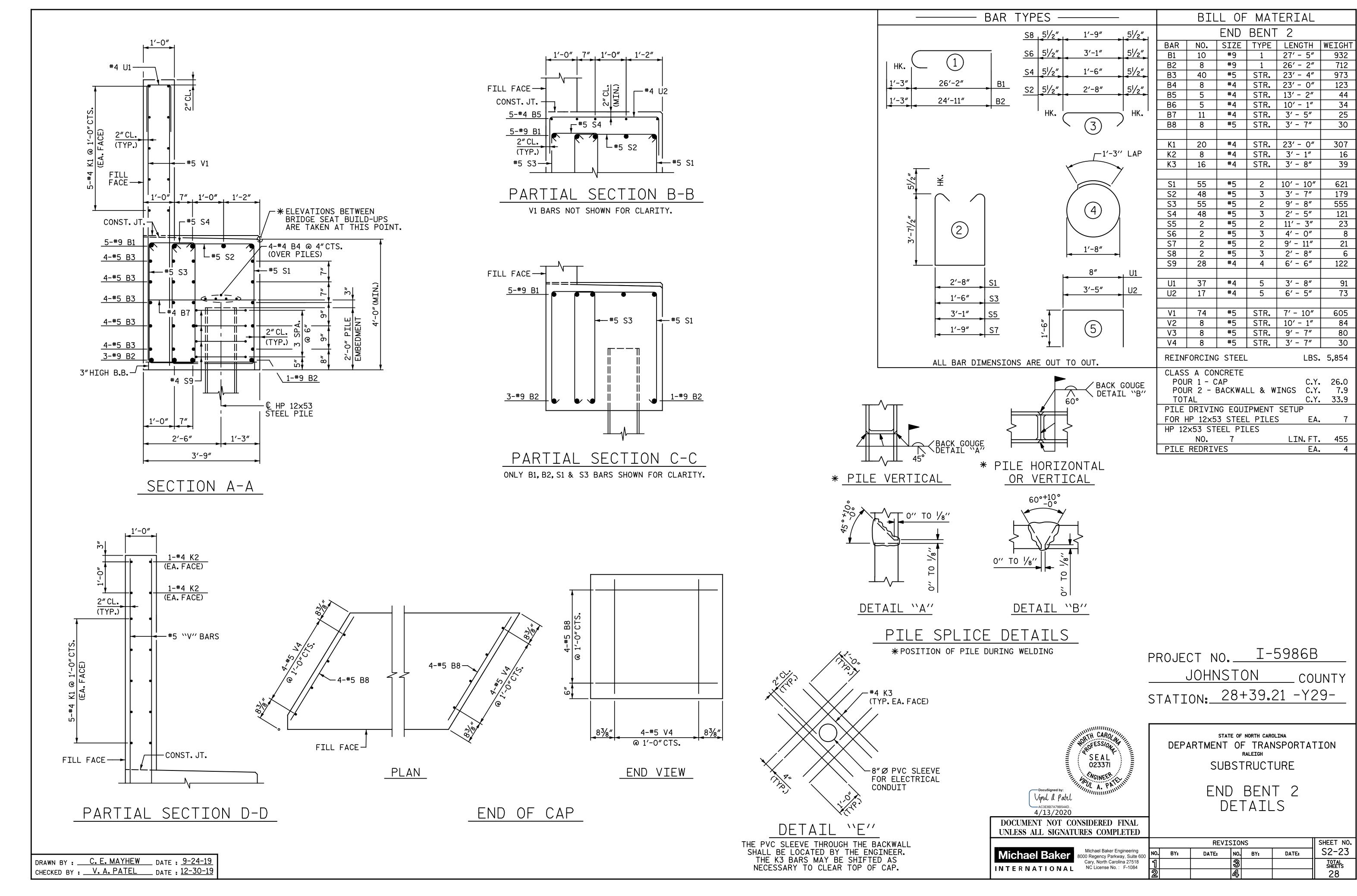
Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084

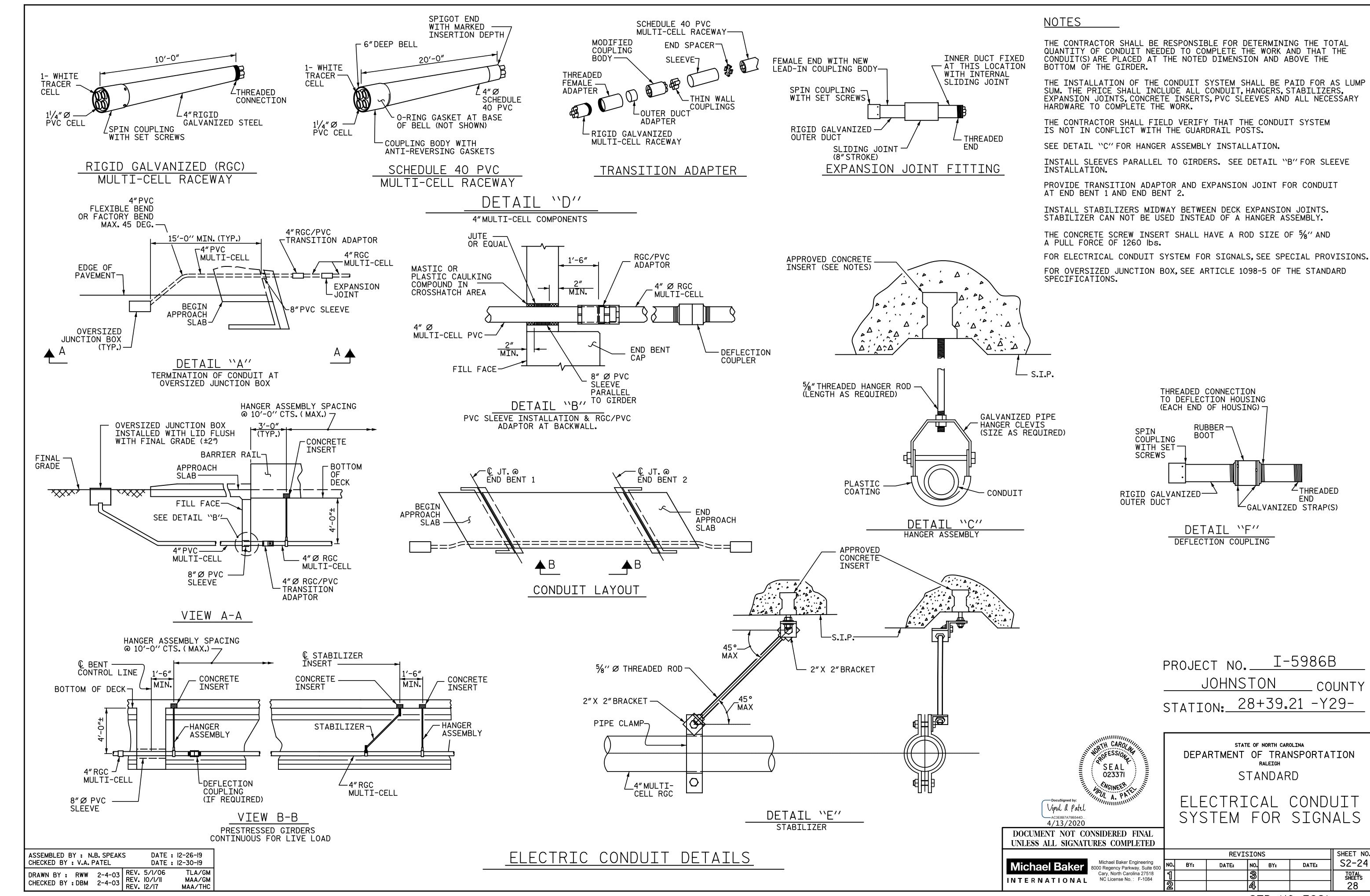
REVISIONS
SHEET NO. S2-20
SOLUTIONS
SHEET NO. S2-20
TOTAL SHEETS
28

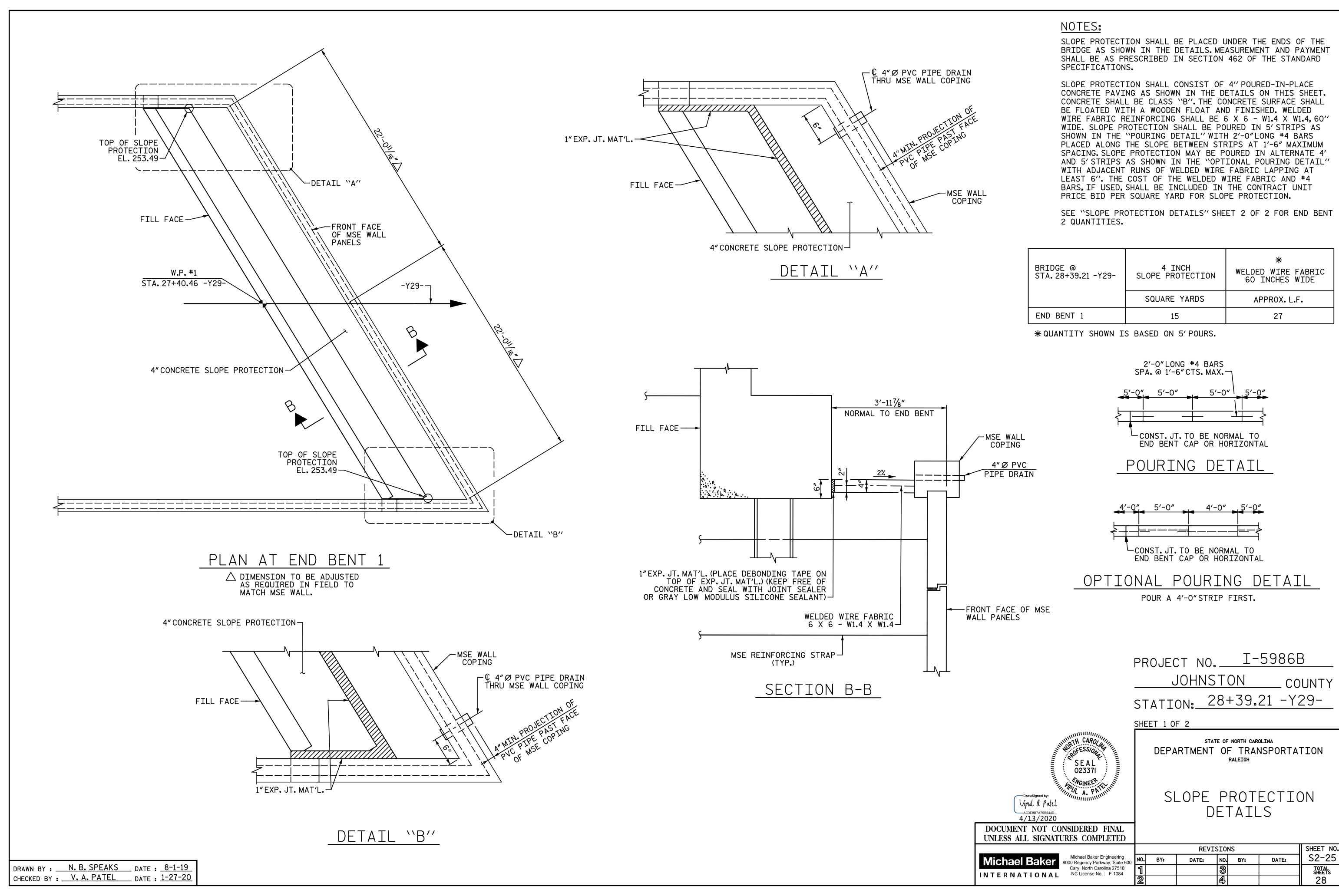
DRAWN BY: C.E.MAYHEW DATE: 11-12-19
CHECKED BY: V.A.PATEL DATE: 1-27-20

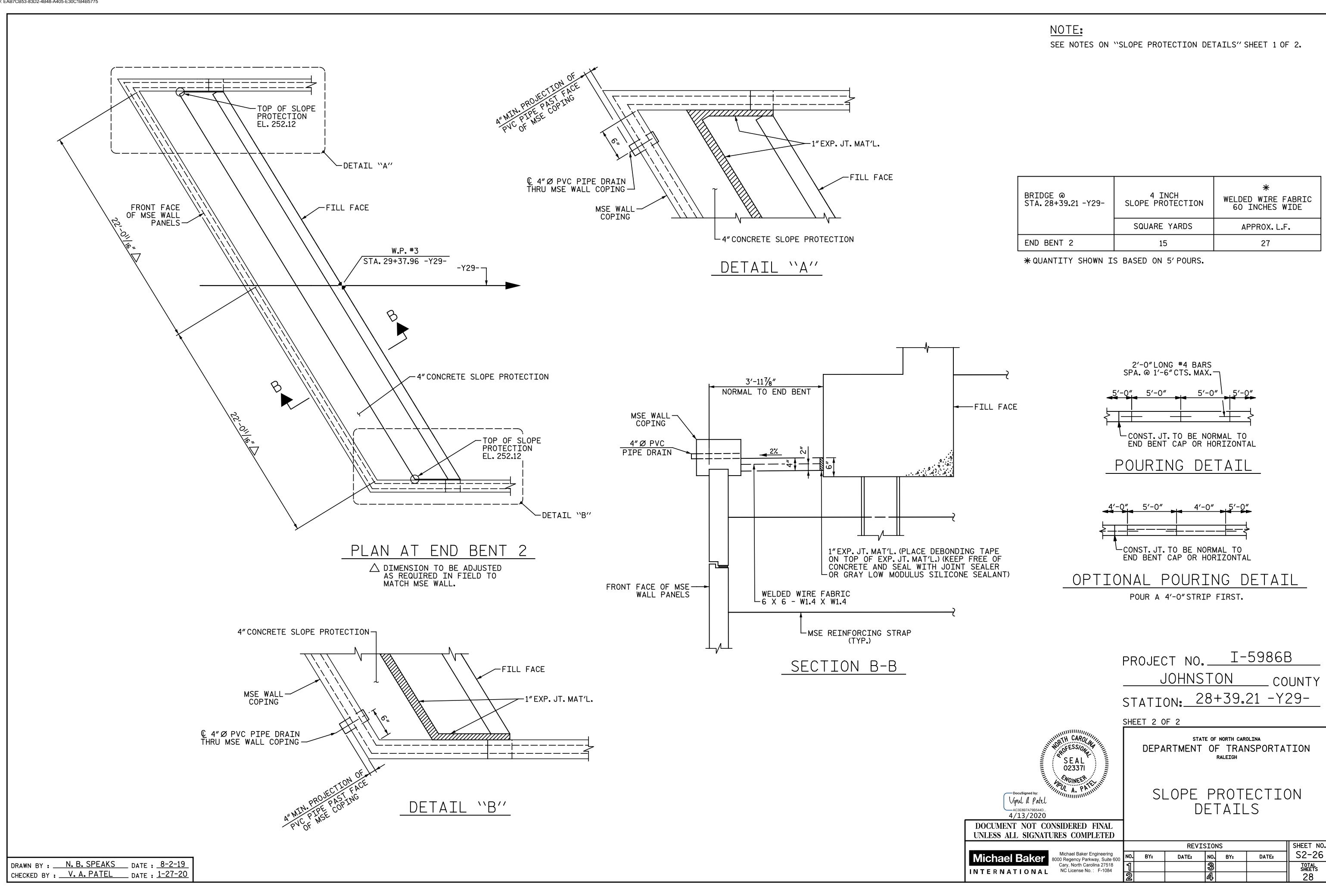


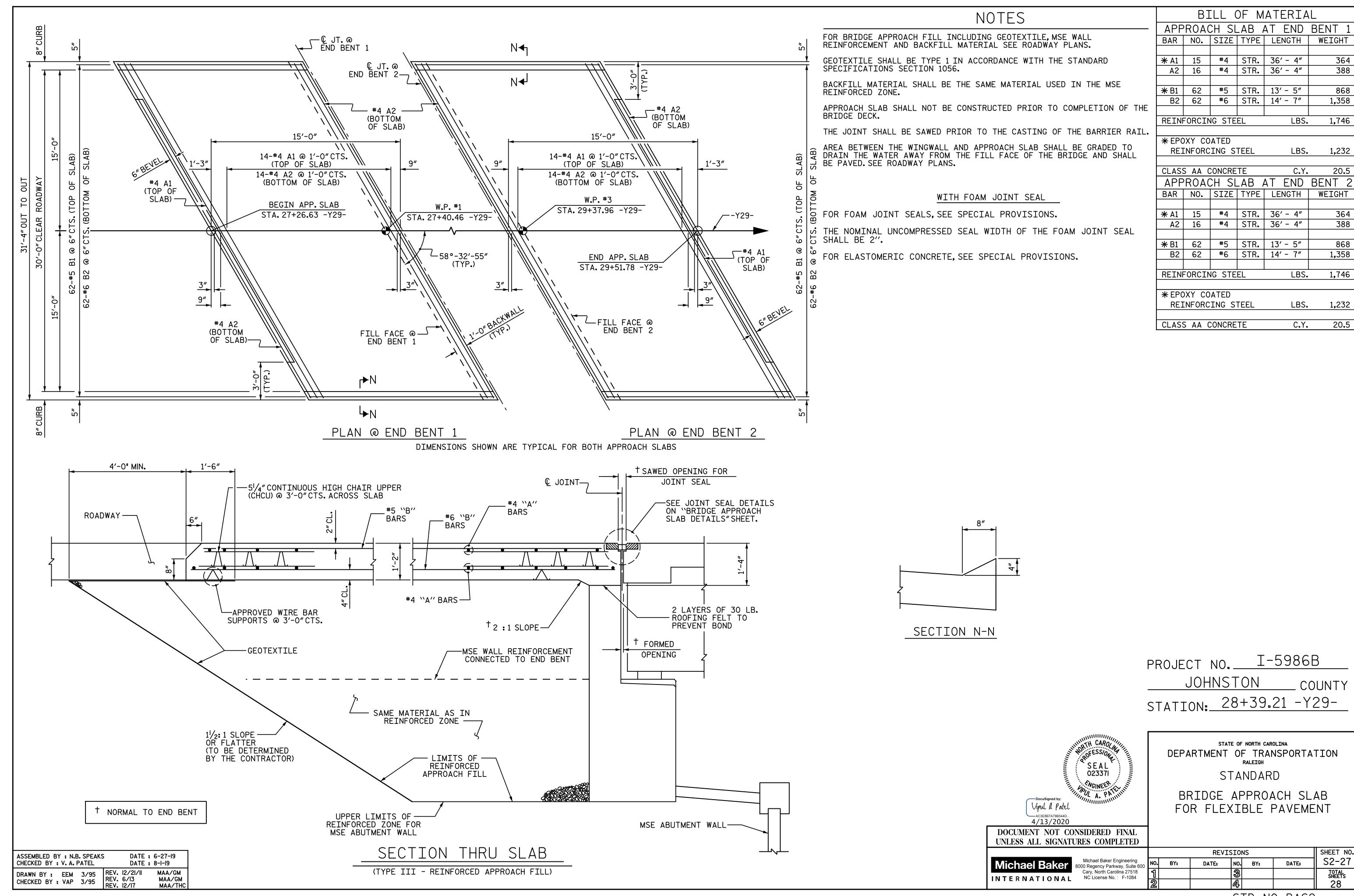


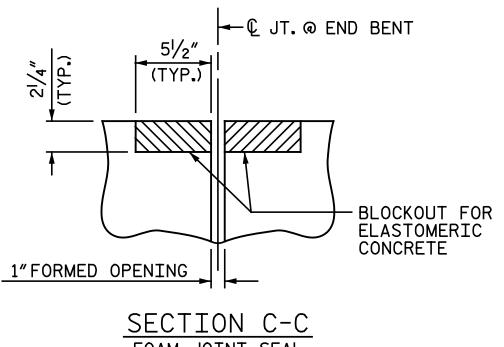




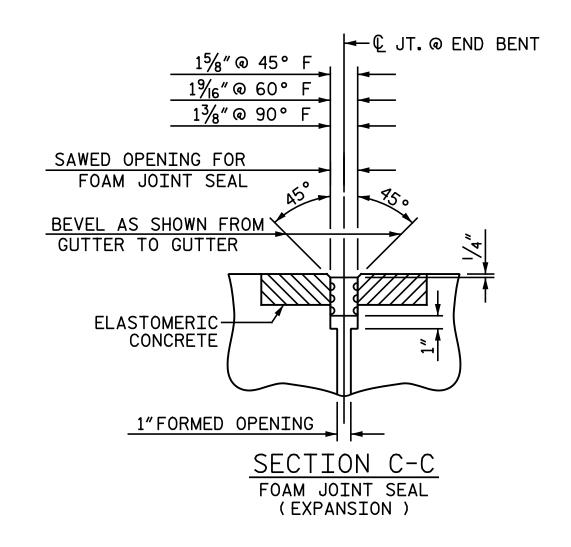






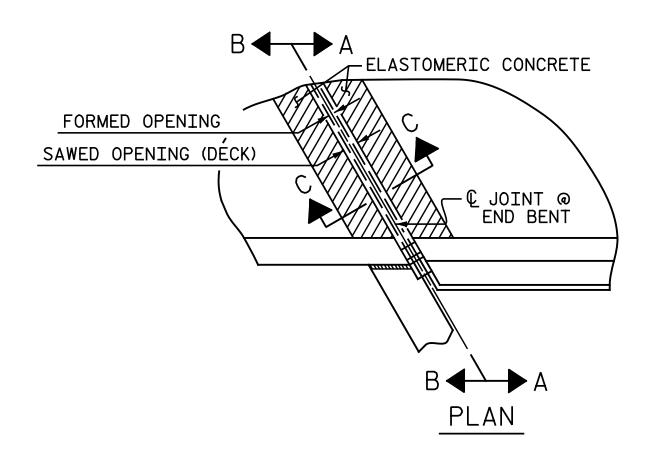


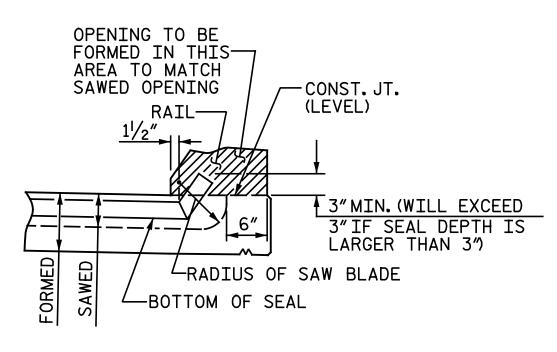
FOAM JOINT SEAL (PRE-SAWED ELASTOMERIC CONCRETE DIMENSIONS)



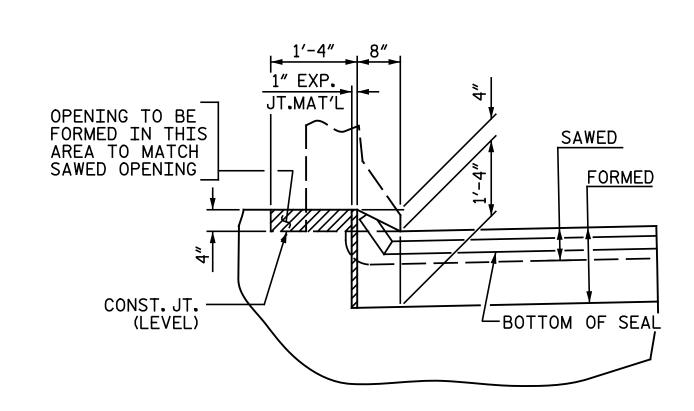
ELAST	OMERIC CO	NCRETE
END BENT NO.	ELASTOME CONCRETE (CU. FT	E *
1	6.0	
2	6.0	
TOTAL	12.0	

* BASED ON THE MINIMUM BLOCKOUT SHOWN.





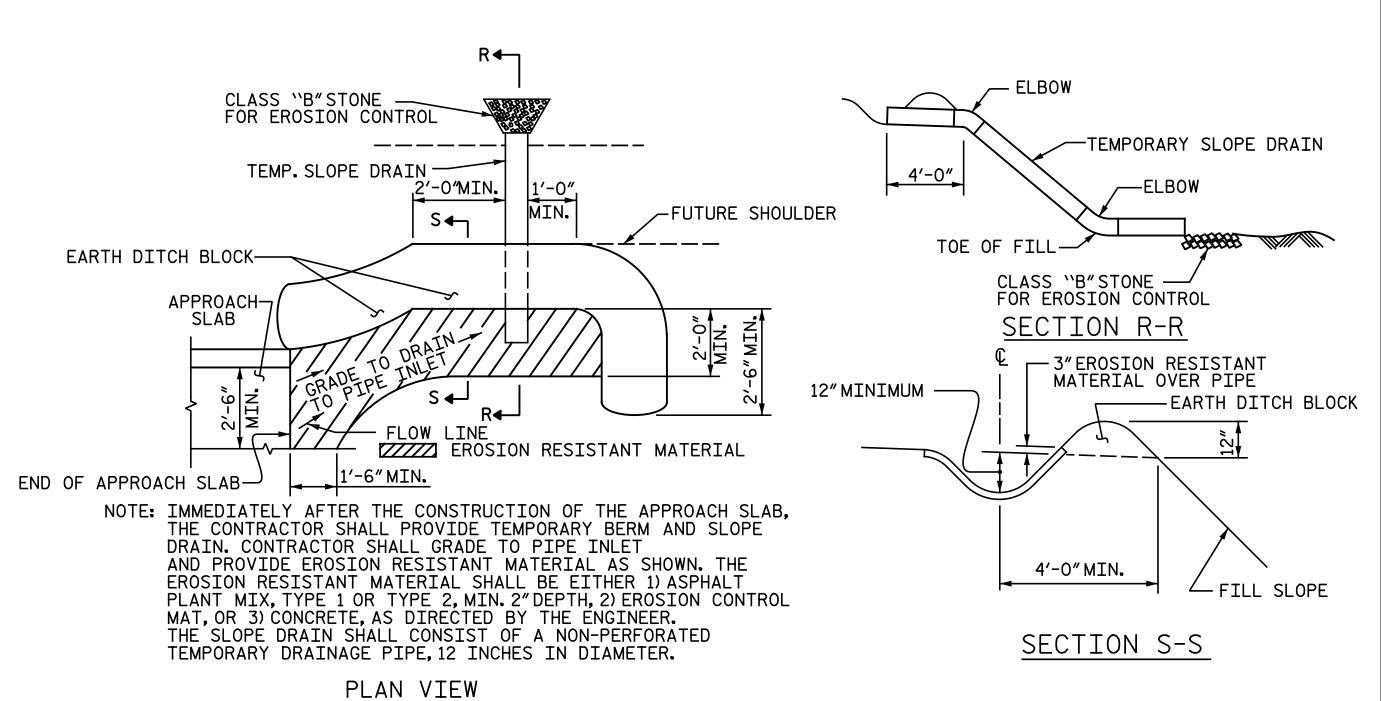
SECTION A-A



SECTION B-B

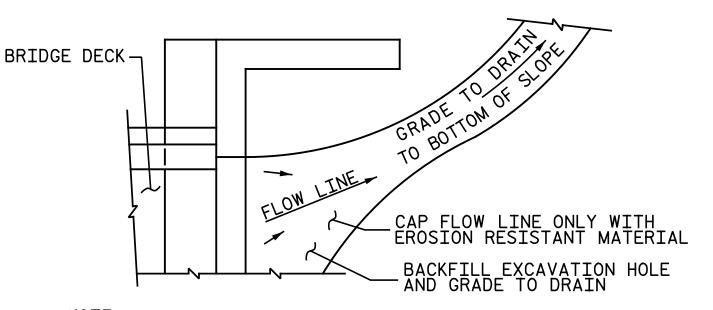
JOINT SEAL DETAILS @ END BENT

FOAM JOINT SEAL TO BE CUT, HEAT WELDED AND TURNED UP PARALLEL TO SLOPED FACE OF THE BARRIER RAIL. THE JOINT SHALL BE SAWED PRIOR TO THE CASTING OF THE BARRIER RAIL.



TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. I-5986B JOHNSTON COUNTY STATION: 28+39.21 -Y29-



Cary, North Carolina 27518

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

BRIDGE APPROACH SLAB DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

INTERNATIONAL NC License No.: F-1084

SHEET NO. **REVISIONS** 8000 Regency Parkway, Suite 600 NO. BY: S2-28 DATE: DATE: TOTAL SHEETS 28

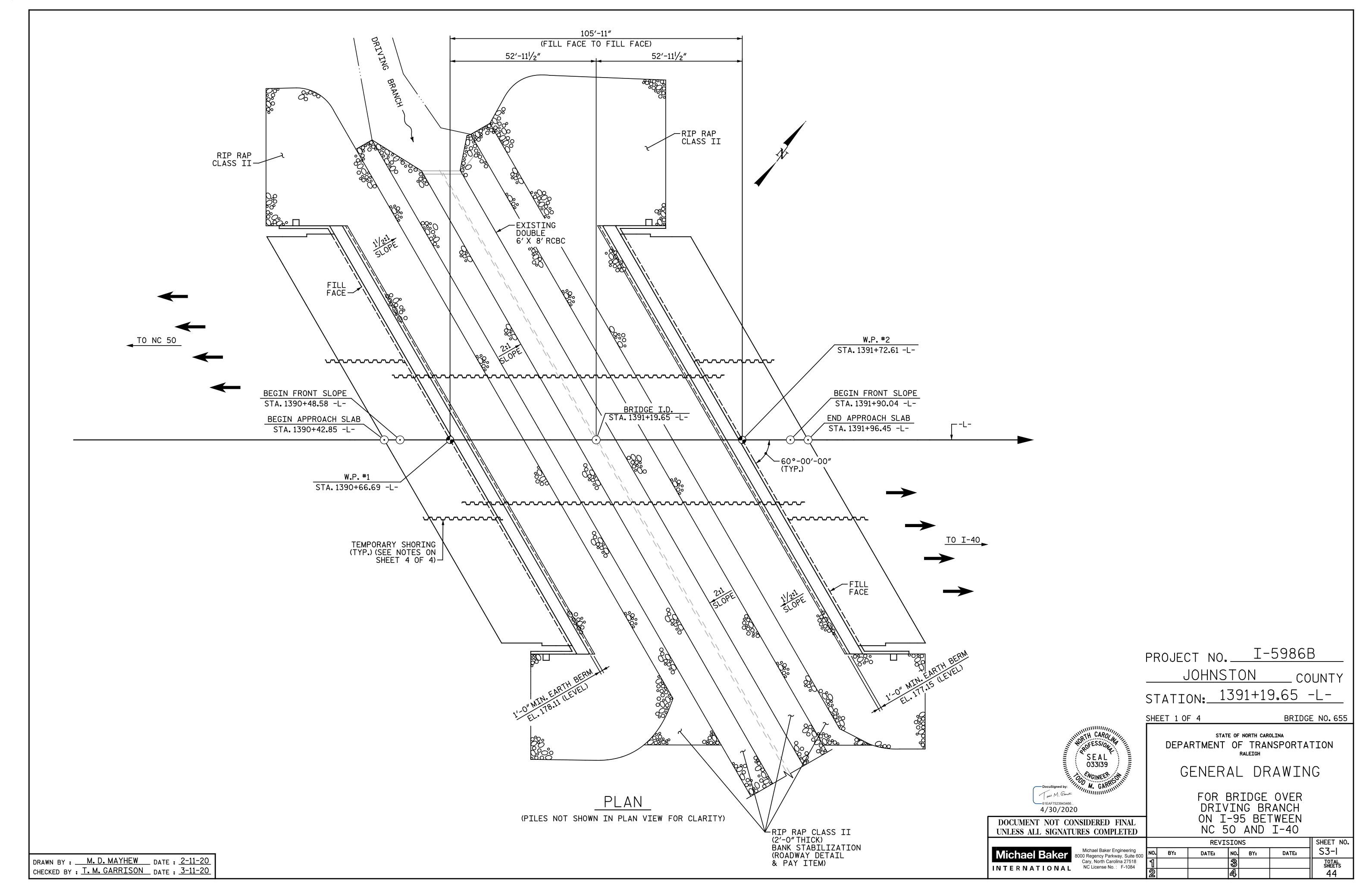
DRAWN BY: FCJ | 11/88 | REV. 6/13 | REV. 12/17 | REV. 5/18 MAA/GM MAA/THC MAA/THC

DATE : 6-27-19

DATE : 7-31-19

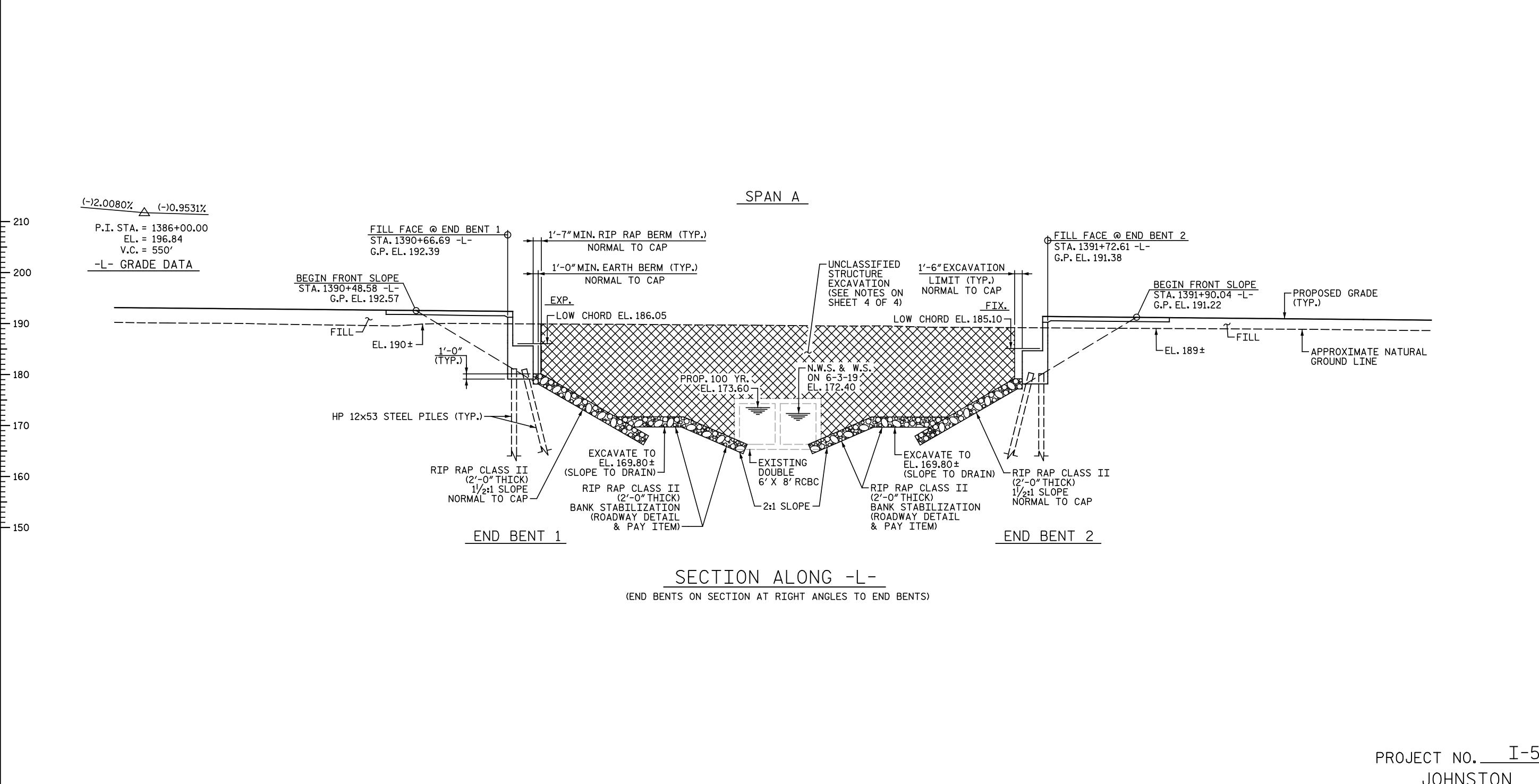
ASSEMBLED BY : N. B. SPEAKS

CHECKED BY : V. A. PATEL



1390+00

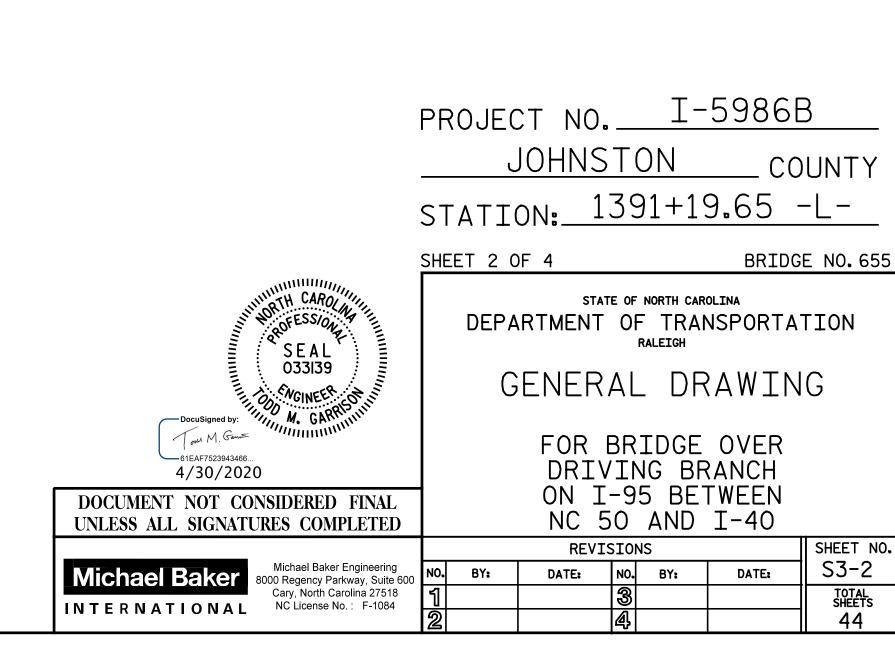
1390+50



1391+00

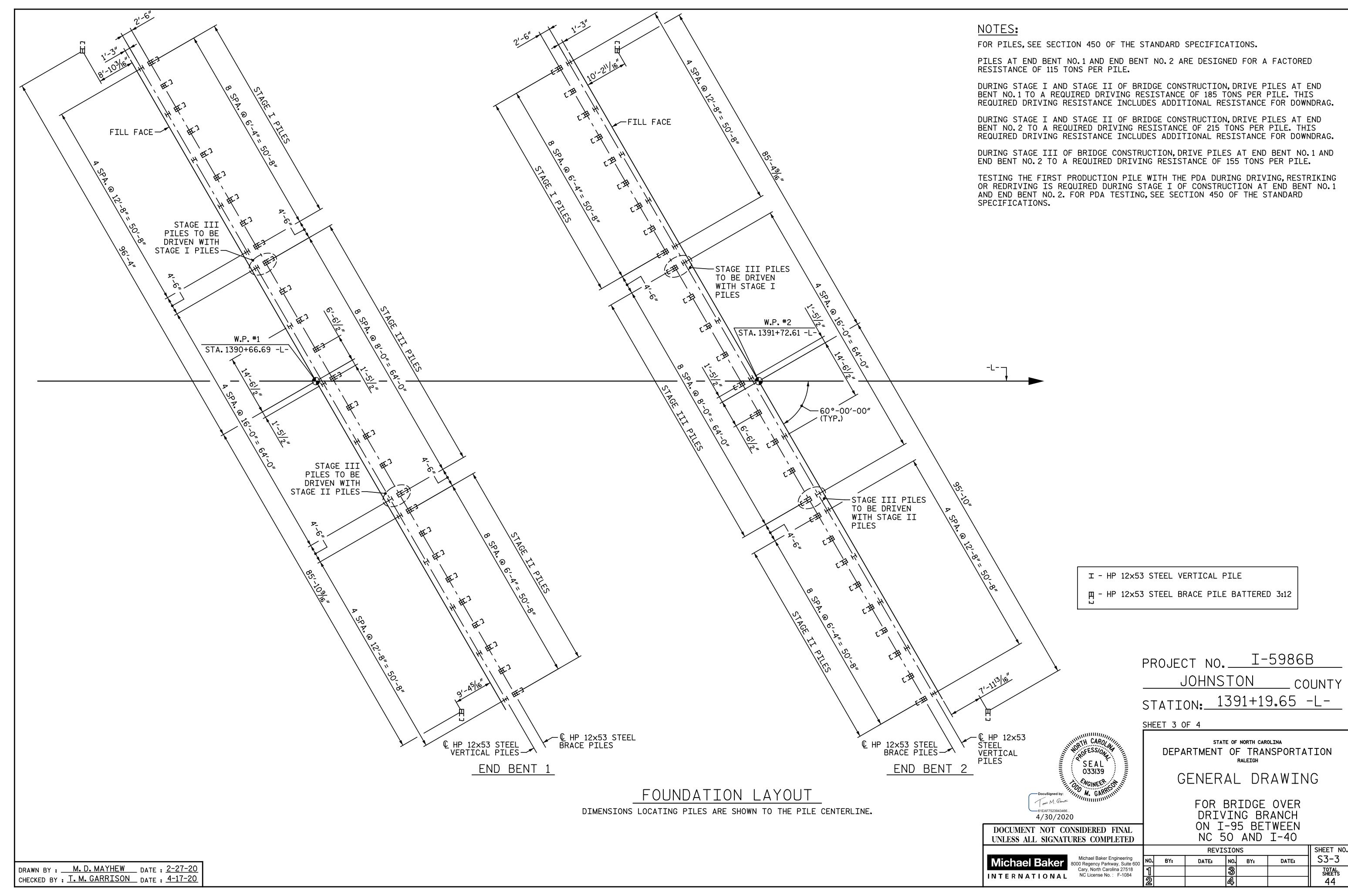
1391+50

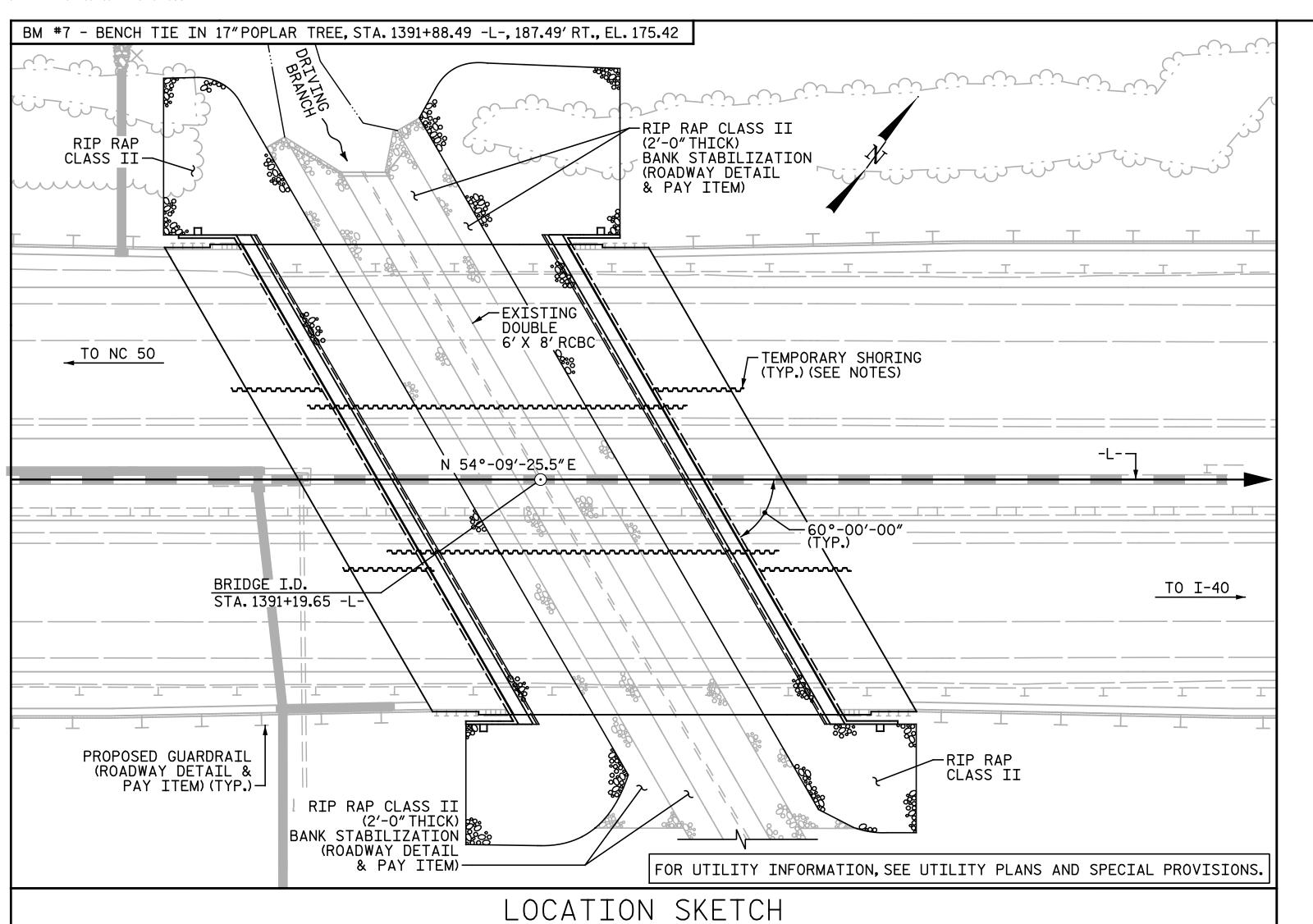
1392+00



1392+50

DRAWN BY: M.D. MAYHEW DATE: 2-11-20
CHECKED BY: T.M. GARRISON DATE: 4-17-20





NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 105 FEET± ON LEFT SIDE AND 155 FEET± ON RIGHT SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR ASBESTOS ASSESSMENT FOR CULVERT DEMOLITION, SEE ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES SPECIAL PROVISION.

									TOTAL BI	LL OF MA	ΓERI	AL									
LOCATION	REMOVAL OF EXISTING STRUCTURE	ASBESTOS ASSESSMENT	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES	HF STE	P 12X53 EL PILES	PILE REDRIVES	CONCRETE BARRIER RAIL	CONCRETE MEDIAN BARRIER	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	EXPANSION JOINT SEALS	PRE CONCR I	45″ STRESSED ETE FLORIDA -BEAMS
	LUMP SUM	LUMP SUM	EA.	LUMP SUM	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	EA.	NO.	LIN. FT.	EA.	LIN. FT.	LIN. FT.	TONS	SQ. YDS.	LUMP SUM	LUMP SUM	NO.	LIN. FT.
SUPERSTRUCTURE					15,430	20,594								248.94	153,41			LUMP SUM	LUMP SUM	16	1,614.33
END BENT 1							307.3		46,164	44	44	3,010	22			790	870				
END BENT 2							307.0		46,057	44	44	3,380	22			895	990				
TOTAL	LUMP SUM	LUMP SUM	2	LUMP SUM	15,430	20,594	614.3	LUMP SUM	92,221	88	88	6,390	44	248.94	153.41	1,685	1,860	LUMP SUM	LUMP SUM	16	1,614.33

HYDRAULIC DATA

DESIGN DISCHARGE = 530 C.F.S. FREQUENCY OF DESIGN FLOOD = 50 YR.

DESIGN HIGH WATER ELEVATION = 173.40
DRAINAGE AREA = 1.27 SQ. MI.

BASE DISCHARGE (Q100) = 570 C.F.S. BASE HIGH WATER ELEVATION = 173.60

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE = 1330 C.F.S. FREQUENCY OF OVERTOPPING = 500+ YR.

OVERTOPPING ELEVATION = 178.00 *

* SP AT STA. 23+00.00 -Y19RPB- RT (SAG)

PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 1391+19.65 -L-

SHEET 4 OF 4

4/27/2021

DOCUMENT NOT CONSIDERED FINAL

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER DRIVING BRANCH ON I-95 BETWEEN NC 50 AND I-40

UNLESS ALL SIGNATURES COMPLETED

REVISIONS

Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 NC License No.: F-1084

NC 50 AND 1-40

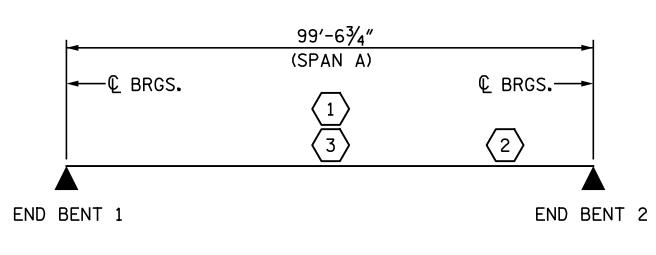
REVISIONS

SHEET NO. S3-4

TOTAL SHEETS 44

DRAWN BY: M.D.MAYHEW DATE: 4-26-21
CHECKED BY: T.M.GARRISON DATE: 4-26-21

		LOAD AND	RES	ISTAN	NCE	FACTO	DR F	RATI	NG (LRFF	R) S	SUMMA	RY I	FOR	PRE	STR	RESSE	ED C	ONCR	RETE	BEA	MS		
										STREN	IGTH	I LIM	IT ST	ATE				SEF	RVICE	III	LIMI	ГЅТ	ATE	
										MOMENT					SHEAR					M	IOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	BEAM LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	BEAM LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	BEAM LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.07		1.75	0.789	1 . 59	Α	1	49.78	1.021	1.09	Α	8	80.05	1.00	0.738	1.07	Α	8	49.78	1, 2
DESIGN LOAD		HL-93 (OPERATING)	N/A		1.45		1.35	0.789	2.06	Α	1	49.78	1.021	1.45	Α	8	19 . 51	N/A	-	-	-	-	_	2
LOAD RATING		HS-20 (INVENTORY)	36.000	2	1.45	52.20	1.75	0.789	2.21	Α	1	49.78	1.021	1.45	Α	8	80.05	1.00	0.789	1.50	Α	1	49.78	1, 2
		HS-20 (OPERATING)	36.000		1.93	69.48	1.35	0.789	2.87	Α	1	49.78	1.021	1.93	Α	8	80.05	N/A	-	-	-	-	-	2
		SH	12.500		3.86	48.25	1.40	0.789	7.12	Α	1	49.78	1.021	5 . 13	Α	8	80.05	1.00	0.738	3.86	Α	8	49.78	1, 2
		S3C	21.500		2,25	48.38	1.40	0.789	4.16	Α	1	49.78	1.021	2.95	Α	8	80.05	1.00	0.738	2,25	Α	8	49.78	1, 2
	CLE	S3A	22.750		2.14	48.69	1.40	0.789	3 . 94	Α	1	49.78	1.021	2.79	Α	8	19 . 51	1.00	0.789	2.14	Α	1	49.78	1, 2
	VEHI V)	S4A	26.750		1.87	50.02	1.40	0.789	3.45	Α	1	49.78	1.021	2.41	Α	8	19 . 51	1.00	0.738	1.87	Α	8	49.78	1, 2
	S	S5A	30.500		1.65	50.33	1.40	0.789	3.05	Α	1	49.78	1.021	2.19	Α	8	19 . 51	1.00	0.789	1 . 65	Α	1	49.78	1, 2
	SINGL	S6A	34.500		1.49	51.41	1.40	0.789	2.75	Α	1	49.78	1.021	1.96	Α	8	80.05	1.00	0.738	1.49	Α	8	49.78	1, 2
LEGAL LOAD RATING	S	S7B	38.500		1.35	51.98	1.40	0.789	2.50	Α	1	49.78	1.021	1.82	Α	8	19 . 51	1.00	0.738	1.35	Α	8	49.78	1, 2
RATING		S7A	40.000	3	1.33	53.20	1.40	0.789	2.46	Α	1	49.78	1.021	1.85	Α	8	19.51	1.00	0.738	1.33	Α	8	49.78	1, 2
	œ	T4A	28.250		1.83	51.70	1.40	0.789	3 . 38	Α	1	49.78	1.021	2.33	Α	8	19.51	1.00	0.738	1.83	Α	8	49.78	1, 2
	TRACTOR TRAILER TST)	T5B	32.000		1.61	51 . 52	1.40	0.789	2.97	Α	1	49.78	1.021	2.20	Α	8	19 . 51	1.00	0.738	1.61	Α	8	49.78	1, 2
	TRA TRA TST	T6A	36.000		1.47	52,92	1.40	0.789	2.71	Α	1	49.78	1.021	2.00	Α	8	80.05	1.00	0.738	1.47	Α	8	49.78	1, 2
	TRUCK SEMI-	T7A	40.000		1.36	54.40	1.40	0.789	2.51	Α	1	49.78	1.021	1.85	Α	8	19.51	1.00	0.789	1.36	Α	1	49.78	1, 2
	T S	Т7В	40.000		1.43	57.20	1.40	0.789	2.65	Α	1	49.78	1.021	1.74	Α	8	80.05	1.00	0.738	1.43	Α	8	49.78	1, 2



LRFR SUMMARY

ASSEMBLED BY: N. B. SPEAKS DATE: 2-14-20 CHECKED BY: T. M. GARRISON DATE: 2-14-20

DRAWN BY: MAA I/08 REV. II/I2/08RR REV. IO/I/II MAA/GM REV. I2/I7 MAA/THC

LOAD FACTORS:

DESIGN LOAD RATING FACTORS | LIMIT STATE | γ_{DC} | γ_{DW} | STRENGTH I | 1.25 | 1.50 | SERVICE III | 1.00 | 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- 1. A SERVICE III LIVE LOAD FACTOR OF 1.0 WAS USED TO BE CONSISTENT WITH THE VALUE USED DURING DESIGN.
- 2. DISTANCE FROM LEFT END OF SPAN IS GIVEN WITH RESPECT TO THE CENTERLINE OF BEARING AND IS MEASURED ALONG THE CONTROLLING BEAM.

(#) CONTROLLING LOAD RATING

- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

BEAM LOCATION

BEAM LOCATION PROVIDED UTILIZES BEAM NUMBER, WHERE BEAM 1 IS THE LEFT EXTERIOR BEAM LOOKING AHEAD STATION. SEE "BEAM LAYOUT" SHEET FOR ALL BEAM LOCATIONS.

PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 1391+19.65 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

STANDARD



LRFR SUMMARY FOR PRESTRESSED CONCRETE BEAMS (INTERSTATE TRAFFIC)

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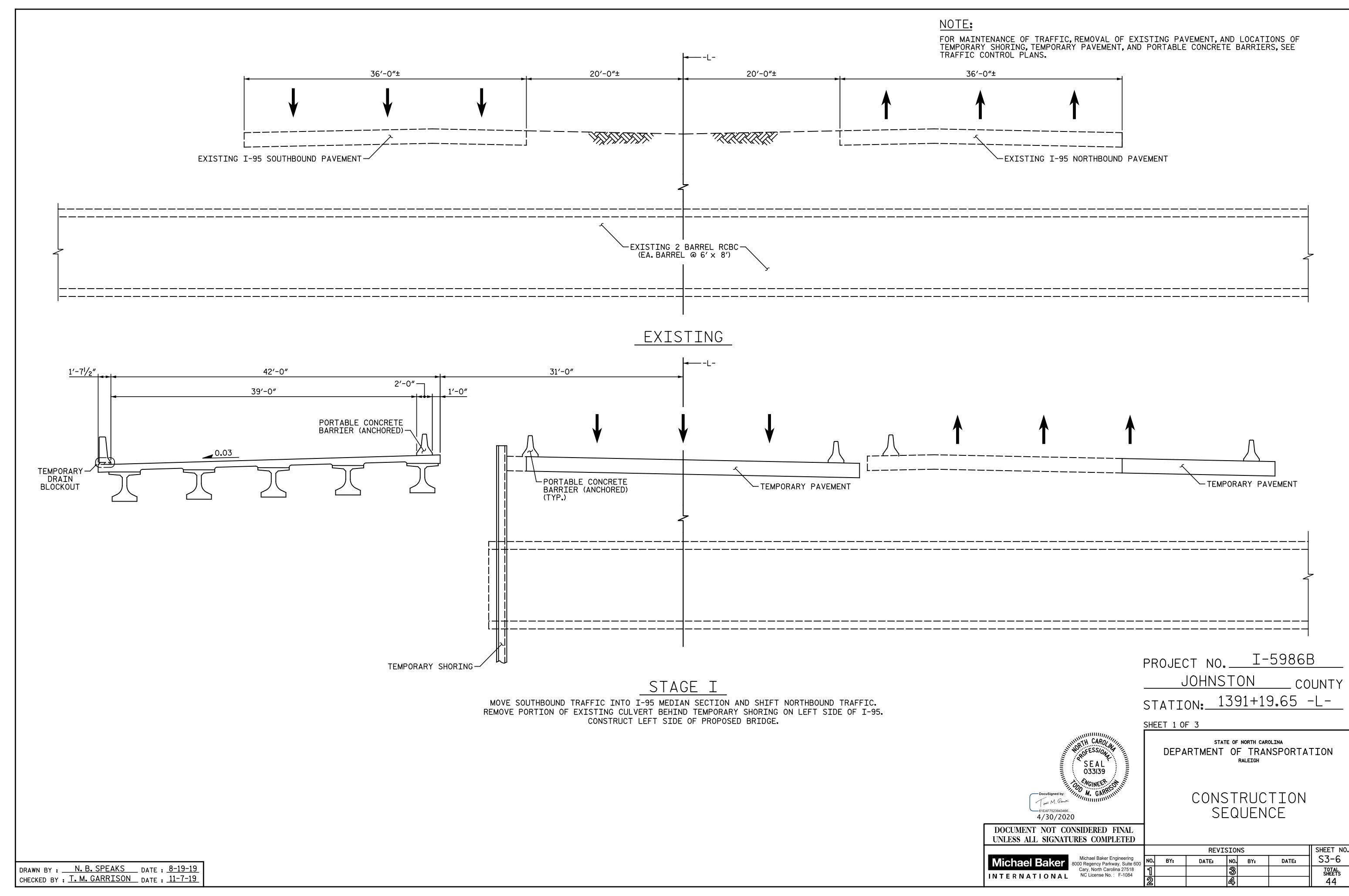
Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084

REVISIONS

BY: DATE: NO. BY: DATE: S3-5

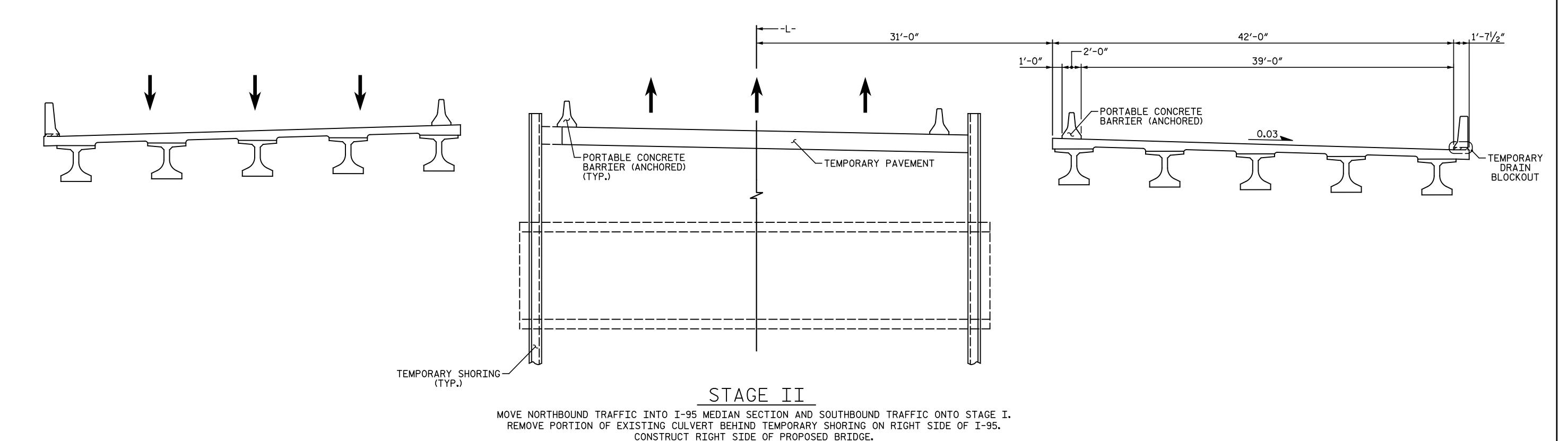
3 TOTAL SHEETS
44

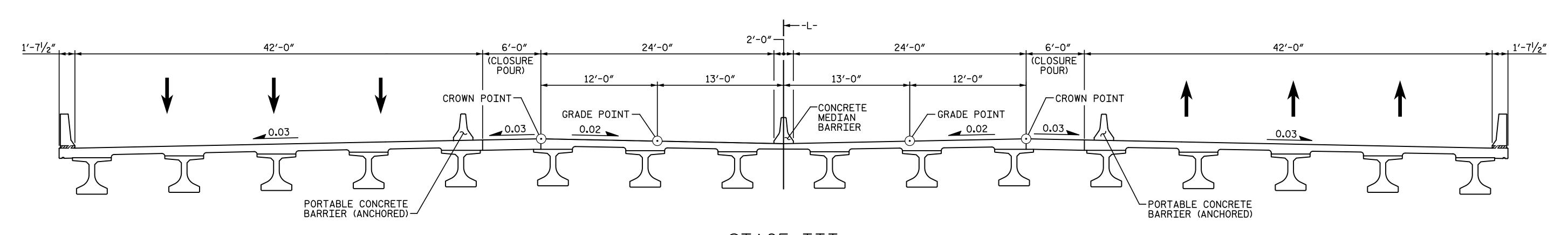
STD. NO. LRFR2



NOTE:

FOR MAINTENANCE OF TRAFFIC, REMOVAL OF EXISTING PAVEMENT, AND LOCATIONS OF TEMPORARY SHORING, TEMPORARY PAVEMENT, AND PORTABLE CONCRETE BARRIERS, SEE TRAFFIC CONTROL PLANS.





STAGE III

MOVE NORTHBOUND TRAFFIC ONTO STAGE II.
REMOVE REMAINING PORTION OF EXISTING CULVERT.
CONSTRUCT CENTER PORTION OF PROPOSED BRIDGE, INCLUDING
CLOSURE POURS AND CONCRETE MEDIAN BARRIER.

PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 1391+19.65 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

CONSTRUCTION SEQUENCE

4/30/2020

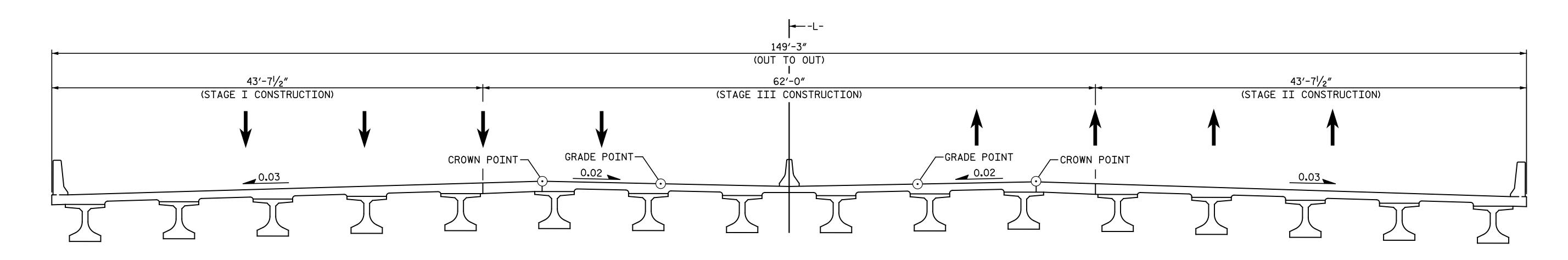
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DocuSigned by:

Michael Baker	Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518
INTERNATIONAL	_ ·

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-7
1			3			TOTAL SHEETS
2			4			44

DRAWN BY: N. B. SPEAKS DATE: 8-19-19
CHECKED BY: T. M. GARRISON DATE: 11-11-19



FINAL TYPICAL SECTION

FILL TEMPORARY DRAIN BLOCKOUTS IN EXTERIOR CONCRETE BARRIER RAILS WITH APPROVED GROUT.

REMOVE PORTABLE CONCRETE BARRIERS.

MOVE NORTHBOUND AND SOUTHBOUND TRAFFIC TO FINAL PATTERN.

PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 1391+19.65 -L-

SHEET 3 OF 3

DocuSigned by:

OM M. Games

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M. GARRIMINIA

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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

CONSTRUCTION SEQUENCE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

4/30/2020

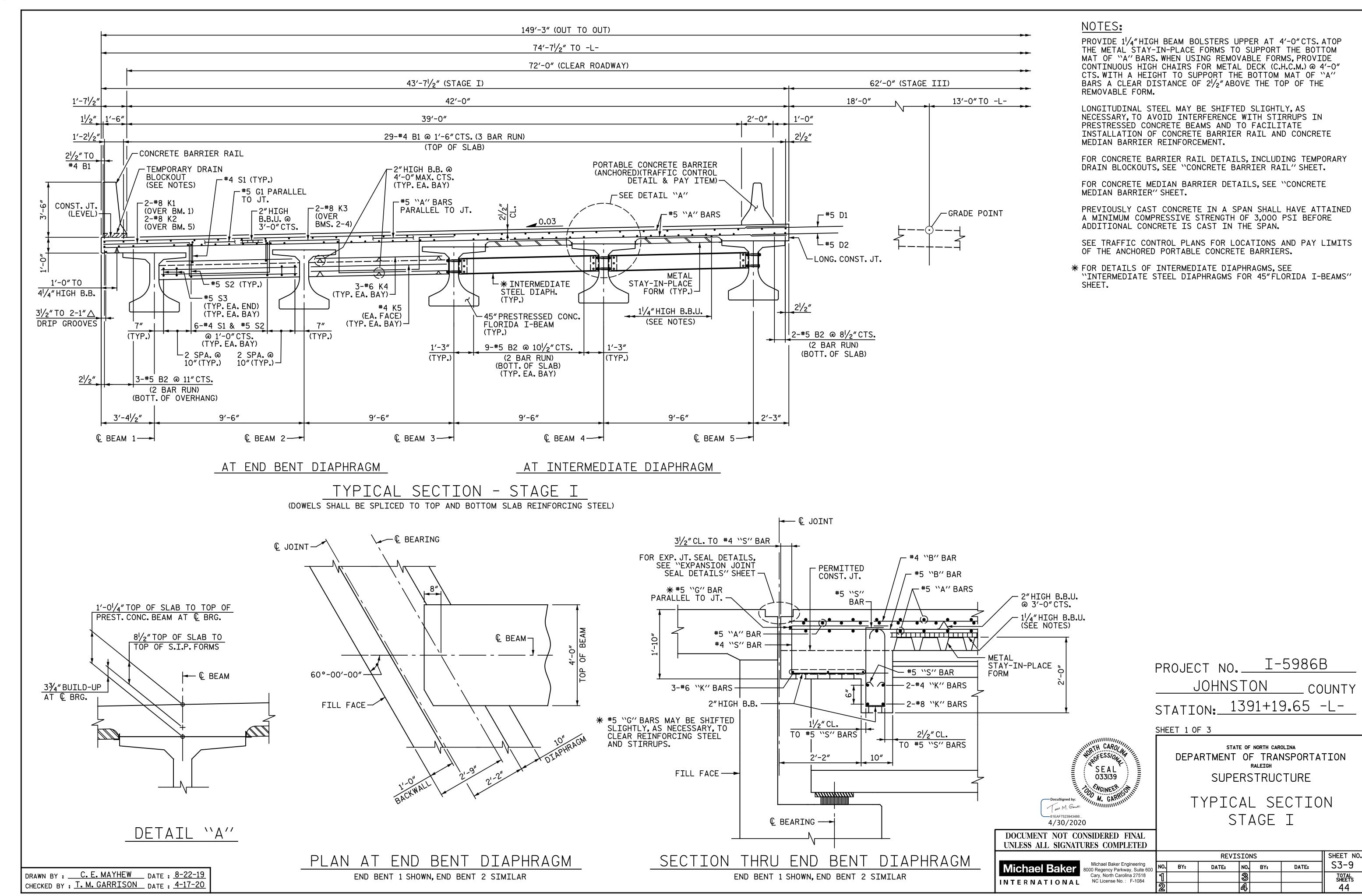
Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084

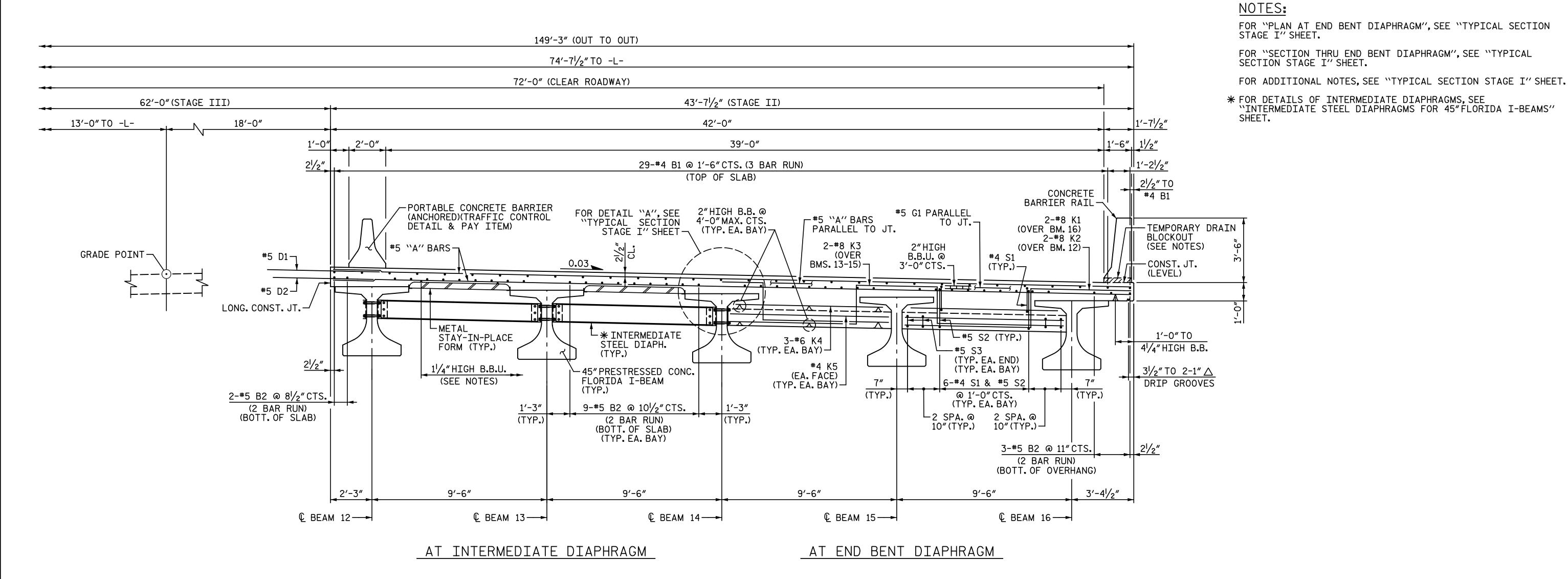
REVISIONS

NO. BY: DATE: NO. BY: DATE: S3-8

1 3 TOTAL SHEETS
2 44

DRAWN BY: N. B. SPEAKS DATE: 8-19-19
CHECKED BY: T. M. GARRISON DATE: 11-11-19





TYPICAL SECTION - STAGE II

(DOWELS SHALL BE SPLICED TO TOP AND BOTTOM SLAB REINFORCING STEEL)

PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 1391+19.65 -L-

SHEET 2 OF 3

DocuSigned by:

M. GARRISHIM

M. GARRISHIM

M. GARRISHIM

61EAF7523943466...

4/30/2020

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE

TYPICAL SECTION STAGE II

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084

REVISIONS

O. BY: DATE: NO. BY: DATE: S3-10

TOTAL SHEETS

A 44

DRAWN BY: C.E.MAYHEW DATE: 8-22-19
CHECKED BY: T.M.GARRISON DATE: 4-17-20

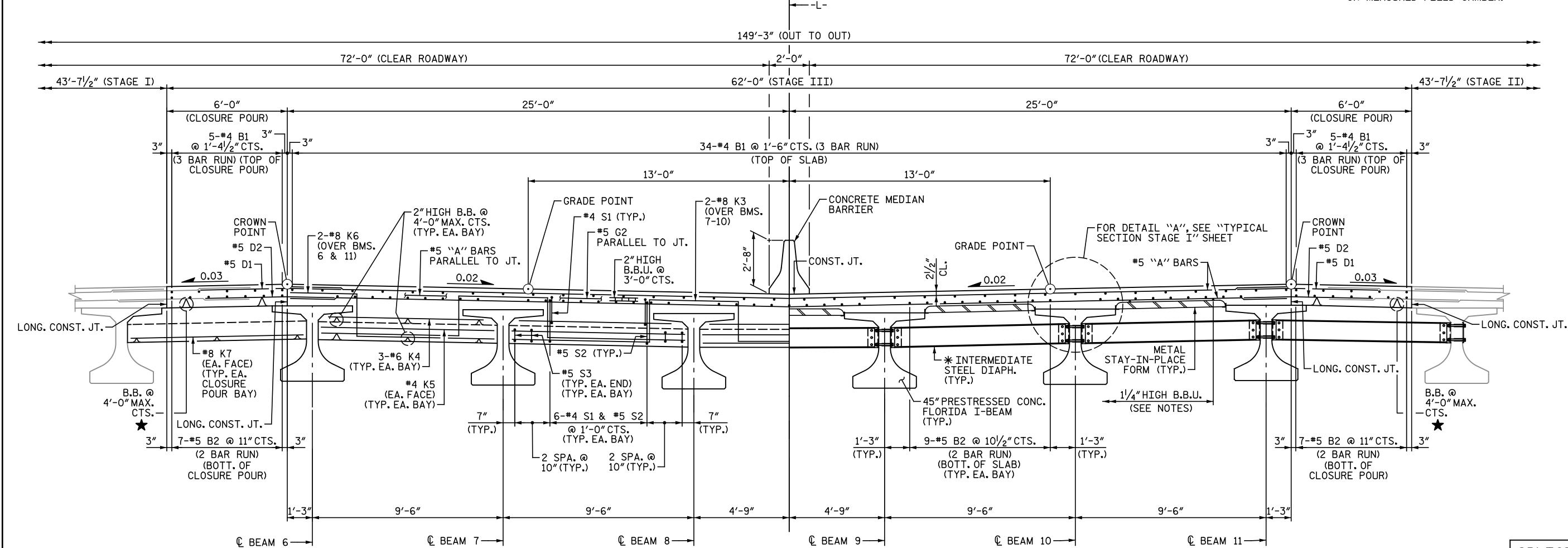
NOTES:

FOR "PLAN AT END BENT DIAPHRAGM", SEE "TYPICAL SECTION STAGE I" SHEET.

FOR "SECTION THRU END BENT DIAPHRAGM", SEE "TYPICAL SECTION STAGE I" SHEET.

FOR ADDITIONAL NOTES, SEE "TYPICAL SECTION STAGE I" SHEET.

- * FOR DETAILS OF INTERMEDIATE DIAPHRAGMS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 45"FLORIDA I-BEAMS" SHEET.
- ★ B.B. HEIGHTS IN CLOSURE POUR BAYS TO BE DETERMINED BASED ON MEASURED FIELD CAMBER.



AT END BENT DIAPHRAGM

AT INTERMEDIATE DIAPHRAGM

TYPICAL SECTION - STAGE III

(DIAPHRAGM REINFORCEMENT DENOTED AS TYP. EA. BAY ALSO TYP. EA. CLOSURE POUR BAY)

PROJECT NO. I-5986B JOHNSTON COUNTY STATION: 1391+19.65 -L-

SPLICE LENGTHS

UNCOATED

2'-2"

EPOXY COATED

2'-6"

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE TYPICAL SECTION

STAGE III

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

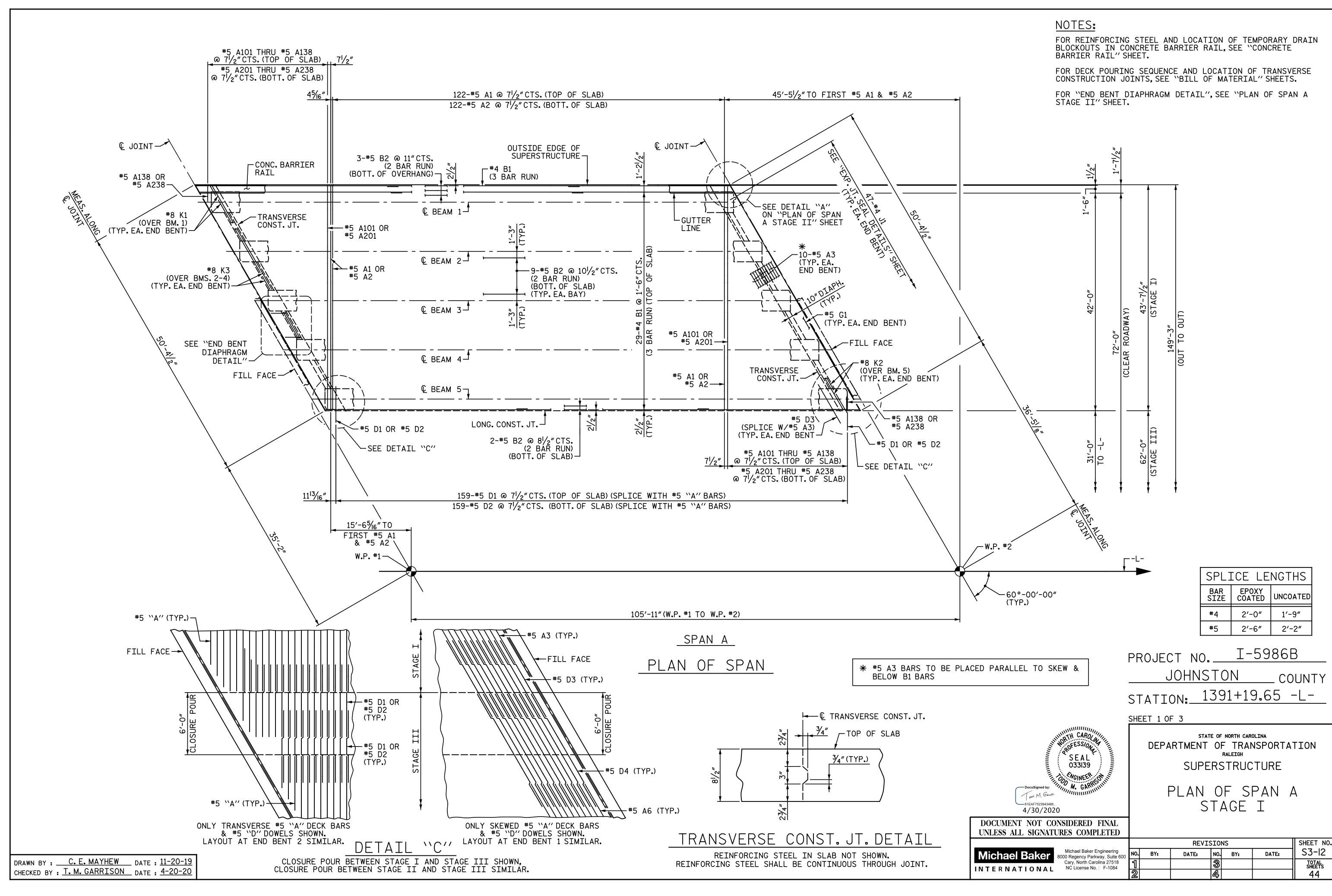
Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 INTERNATIONAL NC License No.: F-1084

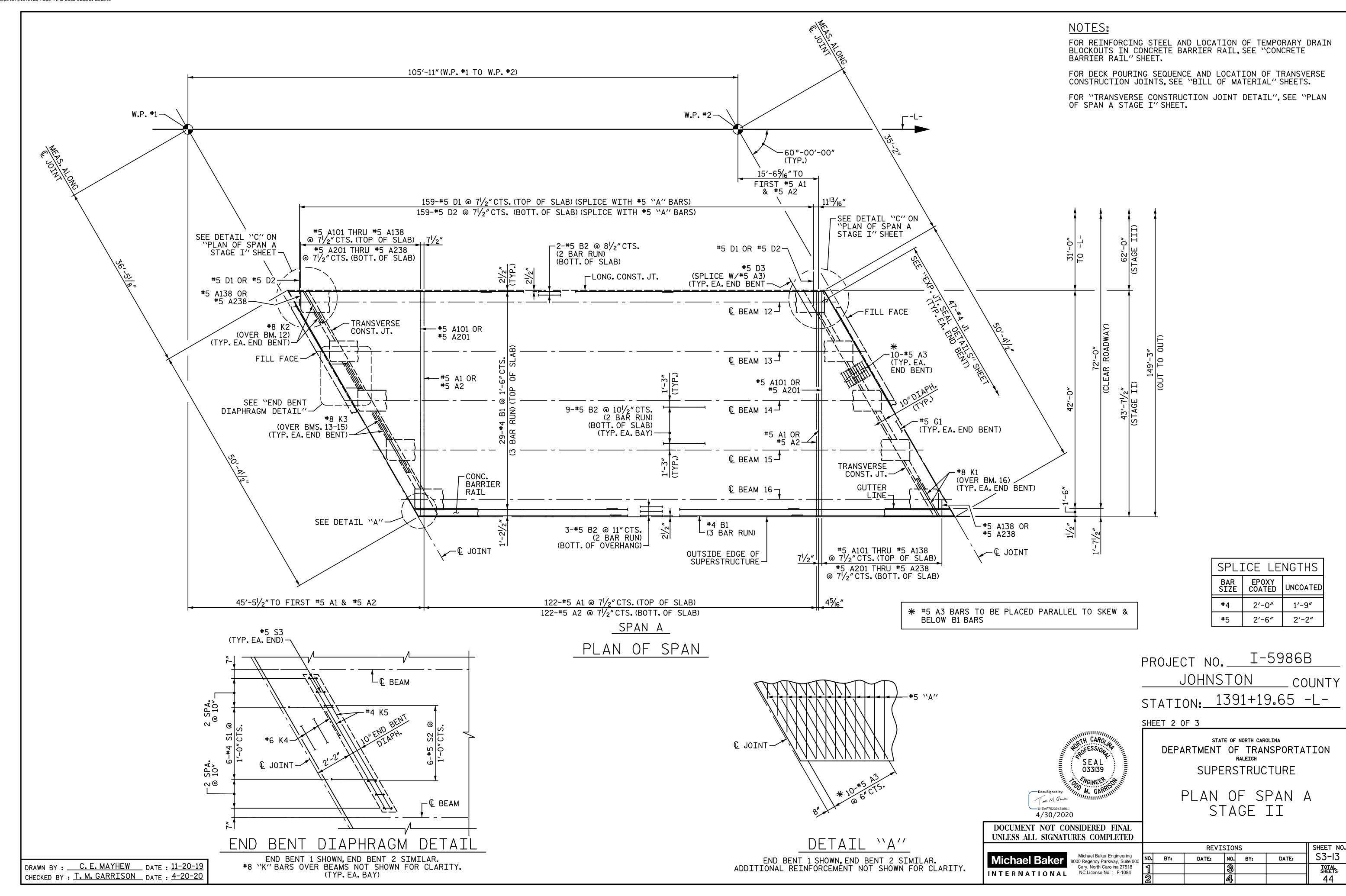
4/30/2020

REVISIONS IO. BY: DATE: NO. BY: DATE:										
:	NO. BY:	DATE:	S3-II							
	3		TOTAL SHEETS							
	4		44							

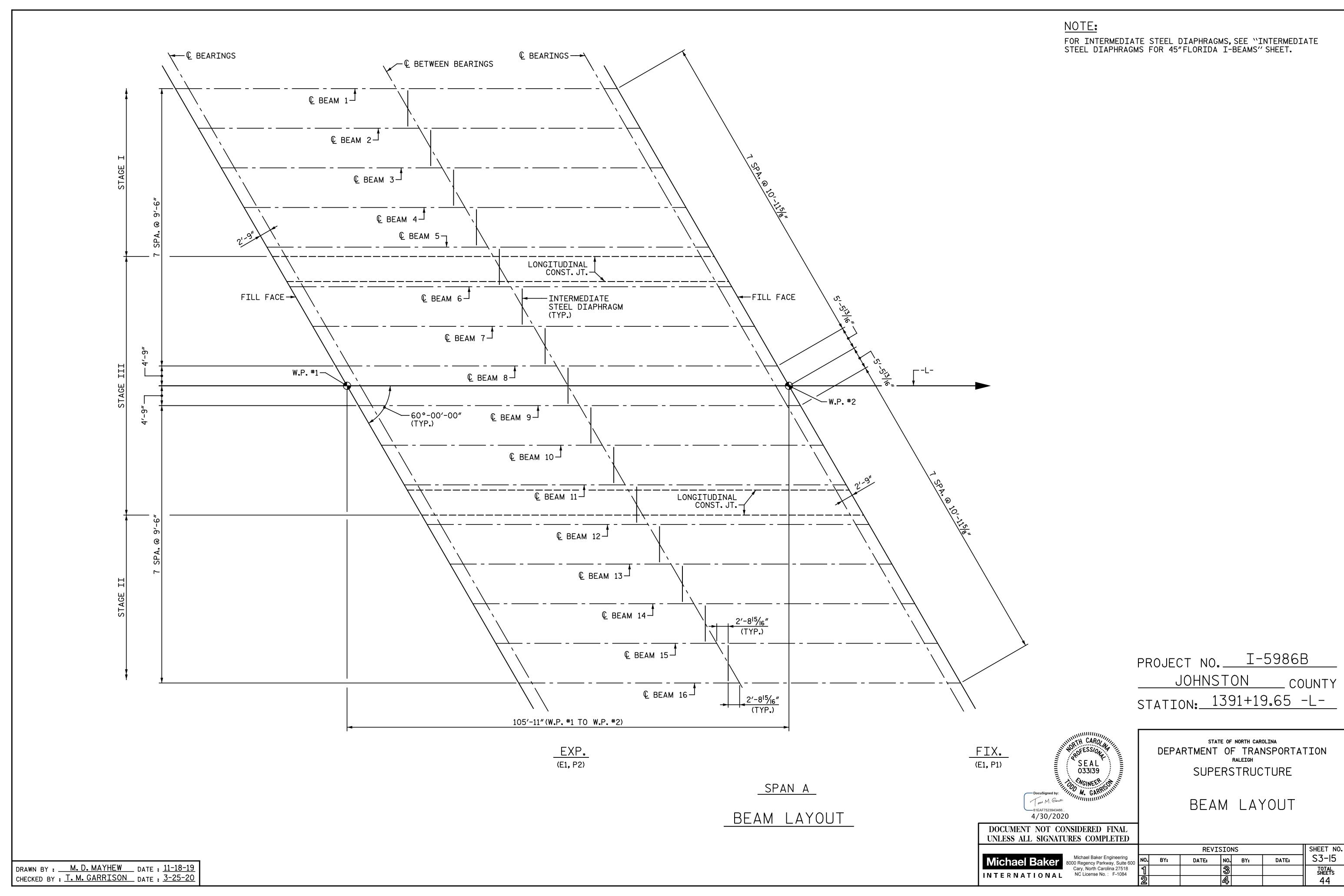


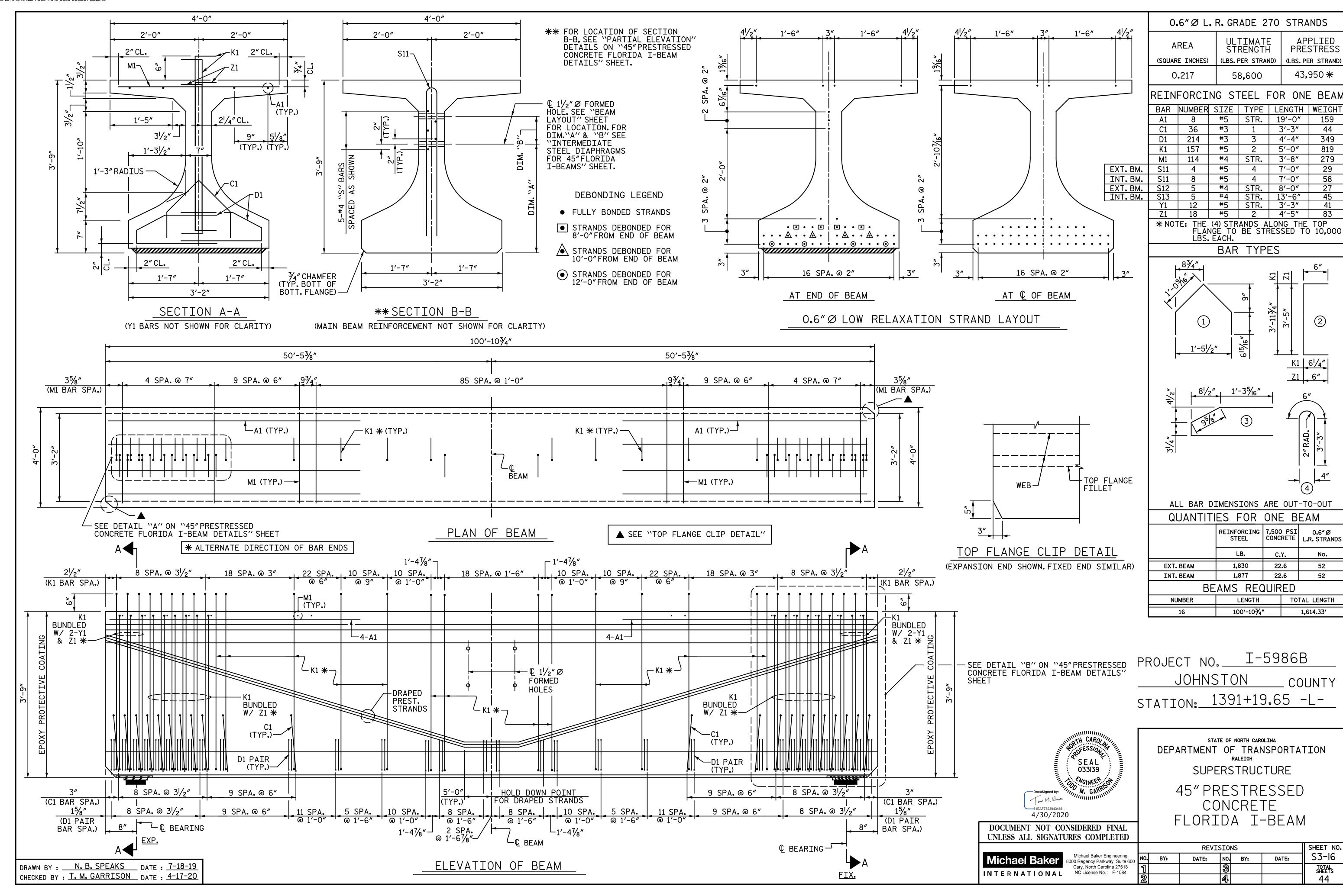
DRAWN BY: C.E.MAYHEW DATE: 8-22-19 CHECKED BY : T. M. GARRISON DATE : 4-18-20

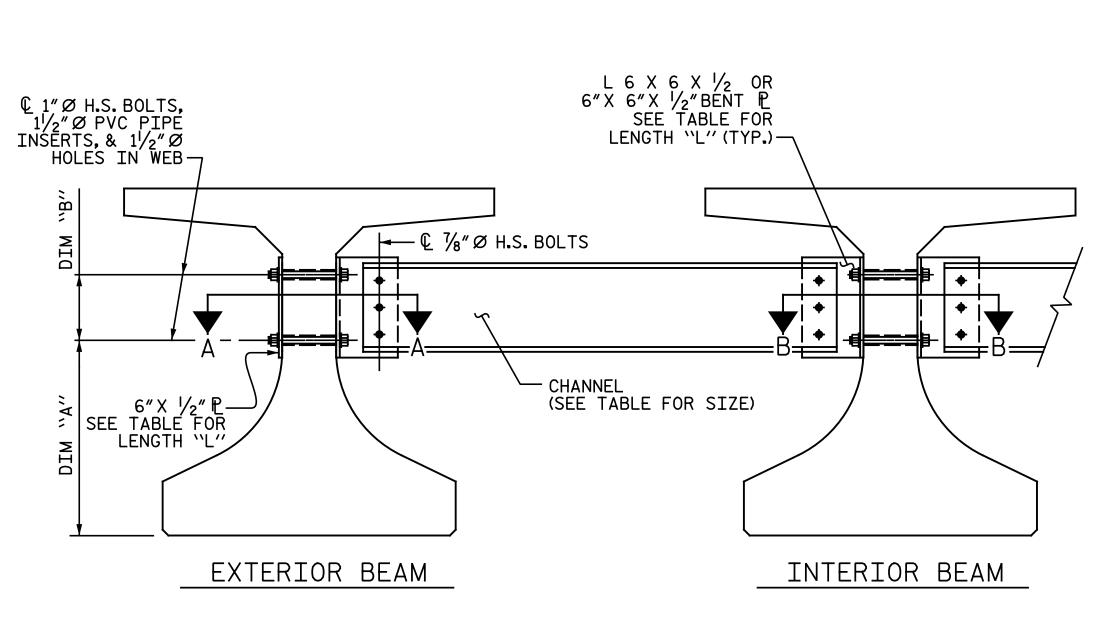




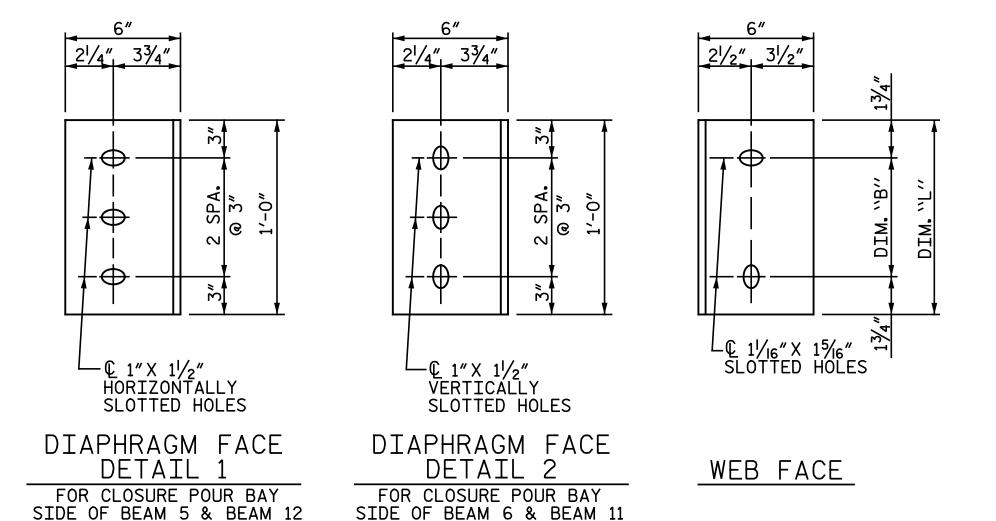
NOTES: FOR REINFORCING STEEL IN CONCRETE MEDIAN BARRIER, SEE "CONCRETE MEDIAN BARRIER" SHEET. FOR DECK POURING SEQUENCE AND LOCATION OF TRANSVERSE CONSTRUCTION JOINTS, SEE "BILL OF MATERIAL" SHEETS. FOR "TRANSVERSE CONSTRUCTION JOINT DETAIL", SEE "PLAN OF SPAN A STAGE I" SHEET. FOR "END BENT DIAPHRAGM DETAIL", SEE "PLAN OF SPAN A STAGE II" SHEET. #5 A401 THRU #5 A444 @ 71/2"CTS.(TOP OF SLAB) 16'-8<mark>|/</mark>2" TO 117-#5 A4 @ $7\frac{1}{2}$ " CTS. (TOP OF SLAB) #5 A501 THRU #5 A544 @ 7½"CTS.(BOTT.OF SLAB) 117-#5 A5 @ $7\frac{1}{2}$ CTS. (BOTT. OF SLAB) FIRST #5 A4 & #5 A5 -SEE DETAIL "C" ON "PLAN OF SPAN A SEE DETAIL "C" ON (SPLICE W/#5 A6 & #5 D3 FROM STAGE I "PLAN OF SPAN A STAGE I" SHEET STAGE I" SHEET OR STAGE II) (TYP. EA. SIDE OF STAGE III) — V—€ JOINT 16'-8¹/₂" TO € JOINT — FIRST #5 A4 & #5 A5 LONG. CONST. JT. #5 A444 OR #5 A544 6'-0" CLOSURE POUR ₩#5 D1 OR -#5 D1 OR #5 D2 ╒╫╤╒┾╼╼┾╼╼╼╼╼┿╆╼╼╼⋐═╼╼╼╼┶╼╼╼╼═╈╼╼╼╼⋐═╼╼╼╼┌╼═┷╧═╇╪╪ LONG. CONST. JT. © BEAM 6--#5 G2 (TYP.EA. END BENT) TRANSVERSE CONST. JT. (OVER BM. 6) (TYP. EA. END BENT)--#5 A401 OR #5 A501 FILL FACE-© BEAM 7--9-#5 B2 @ 101/2"CTS. (2 BAR RUN) (BOTT.OF_SLAB) (OVER BMS. 7-10) (TYP. EA. END BENT) -#5 A4 OR #5 A5 - GUTTER -FILL FACE LINE (TYP. EA. BAY) 62'-0" (STAGE III) SEE DETAIL "B" © BEAM 8-GUTTER LINE -60°-00'-00" (TYP.) FILL FACE-© BEAM 9-W.P. #1-→ ★ 10-#5 A6 -CONC. MEDIAN (TYP. EA. END BENT) BARRIER W.P. #1 SEE "END BENT DIAPHRAGM #8 K6 (OVER BM.11) (TYP.EA.END BENT) © BEAM 10-DETAIL"-#5 A4 OR #5 A5 → TRANSVERSE CONST. JT.-1'-31/8" #5 A444 OR #5 A544 SEE DETAIL ''C''ON— ''PLAN OF SPAN A ''STAGE I'' SHEET LONG. CONST. JT. 7 DETAIL "B" #5 D1 OR +5 D2 --(SIMILAR AT W.P. #2) LONG. CONST. JT.-#5 A401 THRU #5 A444 @ 71/2"CTS.(TOP OF SLAB) SEE DETAIL "C" ON "PLAN OF SPAN A STAGE I" SHEET 1′-3″_ #5 A501 THRU #5 A544 @ 7½"CTS.(BOTT.OF SLAB) PROJECT NO. I-5986B JOHNSTON COUNTY 1391+19.65 -L-159-#5 D1 @ 71/2"CTS. (TOP OF SLAB) (SPLICE WITH #5 "A" BARS & #5 D1 FROM STAGE I OR STAGE II) 159-#5 D2 @ 71/2"CTS. (BOTT. OF SLAB) (SPLICE WITH #5 "A" BARS & #5 D2 FROM STAGE I OR STAGE II) SHEET 3 OF 3 (TYP.EA.SIDE OF STAGE III) STATE OF NORTH CAROLINA 105'-11"(W.P. #1 TO W.P. #2) HON FESSION DEPARTMENT OF TRANSPORTATION * #5 A6 BARS TO BE PLACED PARALLEL TO SKEW & BELOW B1 BARS S E A L 033139 SPAN A **SUPERSTRUCTURE** ON WGINEER PLAN OF SPAN DocuSigned by: PLAN OF SPAN A SPLICE LENGTHS STAGE III 4/30/2020 EPOXY COATED BAR SIZE DOCUMENT NOT CONSIDERED FINAL UNCOATED UNLESS ALL SIGNATURES COMPLETED 1'-9" 2'-0" SHEET NO. #4 **REVISIONS** NO. BY: S3-I4 Michael Baker Engineering 8000 Regency Parkway, Suite 600 Michael Baker DATE: DATE: 2'-6" 2'-2" DRAWN BY : _____C. E. MAYHEW ____ DATE : 11-20-19 TOTAL SHEETS Cary, North Carolina 27518 INTERNATIONAL NC License No.: F-1084 CHECKED BY : T. M. GARRISON DATE : 4-20-20 44







PART SECTION AT INTERMEDIATE DIAPHRAGM



CONNECTOR PLATE DETAILS

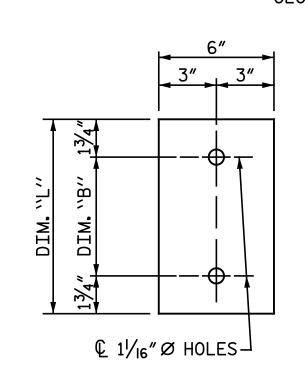
NON-CLOSURE POUR BAYS

-₩

-© 1½6″X 15⁄6″ ' SLOTTED HOLES

WEB FACE

CONNECTOR PLATE DETAILS CLOSURE POUR BAYS



21/4" 33/4"

┰Ϣ

 $\vdash \bigoplus$

 $-\mathbb{Q}^{15}/_{16}$ " X $1^{1}/_{8}$ " SLOTTED HOLES

DIAPHRAGM FACE

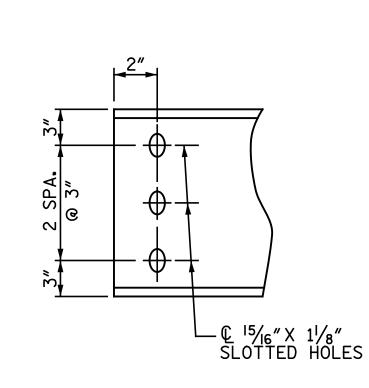
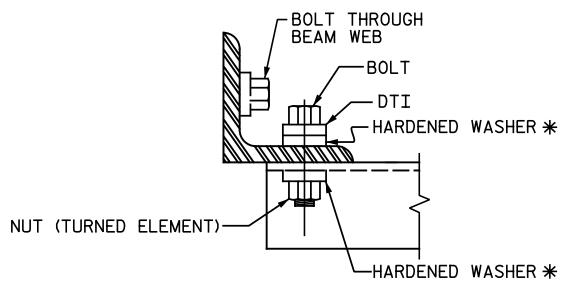


PLATE DETAILS

CHANNEL END



BOLT WITH DTI ASSEMBLY DETAIL

STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE CHANNEL MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE BEAM WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, CHANNELS, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY A THERMAL SPRAYED COATING WITH A SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE DEPARTMENTS THERMAL SPRAYED COATINGS (METALLIZATION) PROGRAM, THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE BEAM WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED BEAMS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BEAMS.

TABLE

BEAM TYPE	CHANNEL SIZE	DIM "A"	DIM "B"	DIM "L"
45″F.I.B.	MC 12 × 31	2'-2"	8 ¹ /2"	1'-0"

PROJECT NO. I-5986B JOHNSTON COUNTY 1391+19.65 -L-STATION:_



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUPERSTRUCTURE

INTERMEDIATE STEEL DIAPHRAGMS FOR 45"FLORIDA I-BEAMS

UNLESS ALL SIGNATURES COMPLETED SHEET NO. **REVISIONS** Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 NO. BY: S3-I7 BY: DATE: DATE: Michael Baker TOTAL SHEETS INTERNATIONAL NC License No.: F-1084 44

CONNECTION DETAILS

(SEE TABLE FOR SIZE)

(TYP.)

— * € 1/8"Ø H.S. BOLT, 2 HARDENED WASHERS AND

DTI (TYP.)

— ₡ 1″Ø H.S. BOLT AND — 2 HARDENED WASHERS (TYP.)

L 6 X 6 X 1/2 OR

6"X 6"X 1/2" BENT P

SEE TABLE FOR LENGTH "L" (TYP.)

NUTS ON BOLTS FOR CONNECTING CHANNELS TO CONNECTOR PLATES IN CLOSURE POUR BAYS SHALL BE LEFT LOOSE FOR PURPOSE OF ADJUSTMENT UNTIL BOTH SIDES OF SLAB HAVE BEEN POURED.

> * FOR CONNECTING CHANNELS TO CONNECTOR PLATES IN CLOSURE POUR BAYS, SQUARE HARDENED WASHERS SHALL BE USED IN LIEU OF CIRCULAR HARDENED WASHERS.

SECTION B-B

FOR BOLT CONNECTION,
SEE TYPICAL BOLT WITH
DTI ASSEMBLY DETAIL

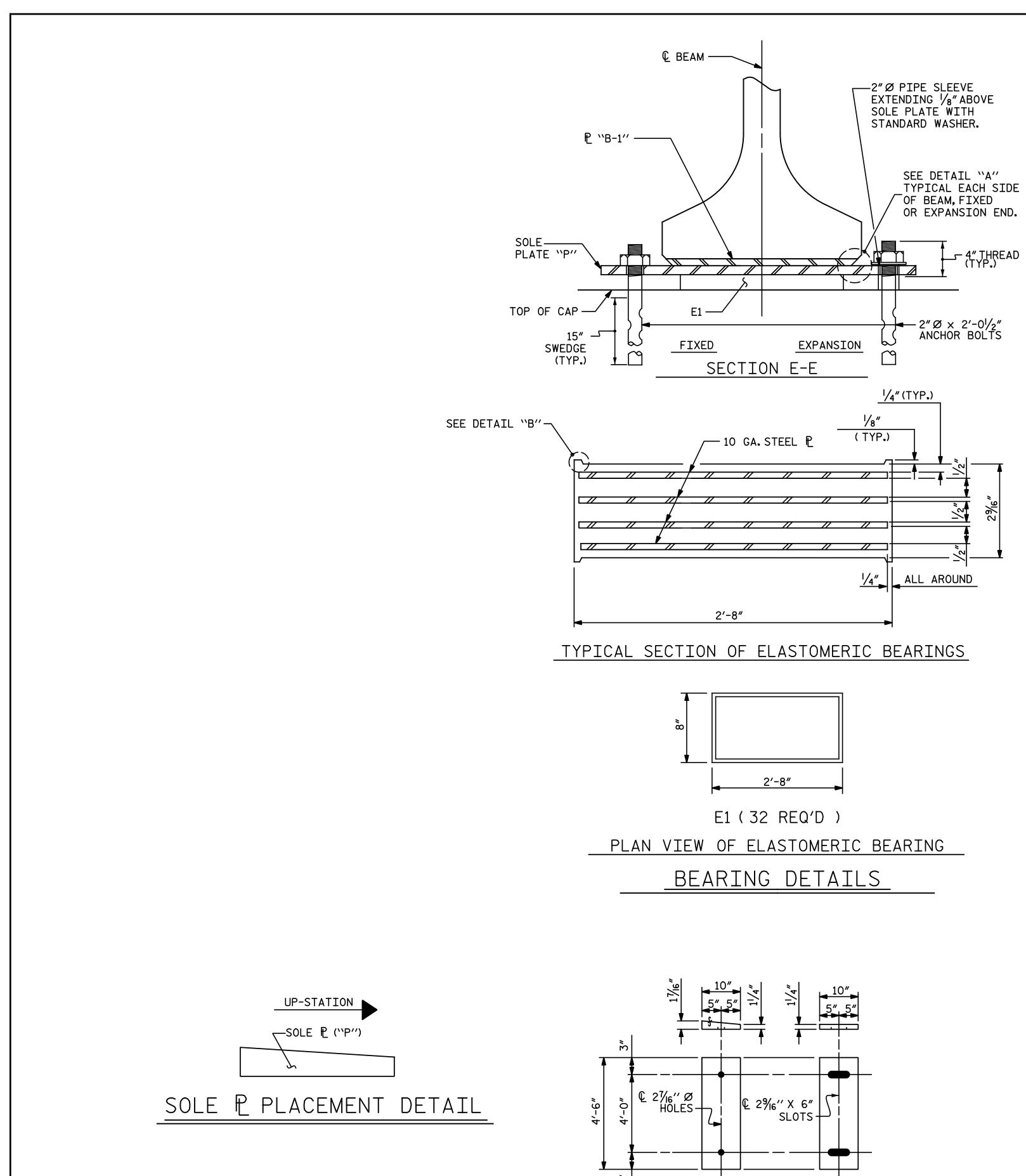
DRAWN BY : N. B. SPEAKS DATE : 7-18-19 CHECKED BY : T. M. GARRISON DATE : 4-17-20

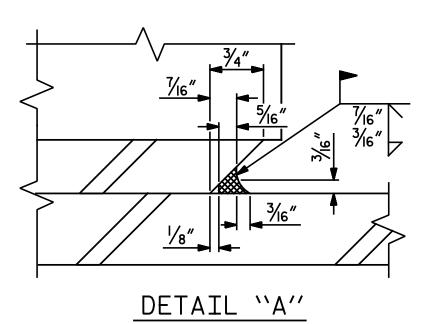
6"X ½" ₱—— SEE TABLE FOR

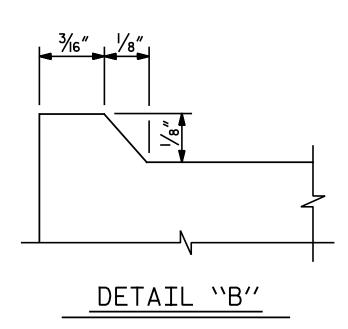
LENGTH "L" (TYP.)

SECTION A-A

DRAWN BY: N.B. SPEAKS DATE: 7-18-19
CHECKED BY: T.M. GARRISON DATE: 3-25-20







MAXIMUM ALLOWABLE SERVICE LOADS

D.L.+L.L. (NO IMPACT)

245 k

NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN, THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2"Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE BEAM, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE BEAMS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. NO SHOP DRAWINGS ARE REQUIRED FOR ANCHOR BOLTS. NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

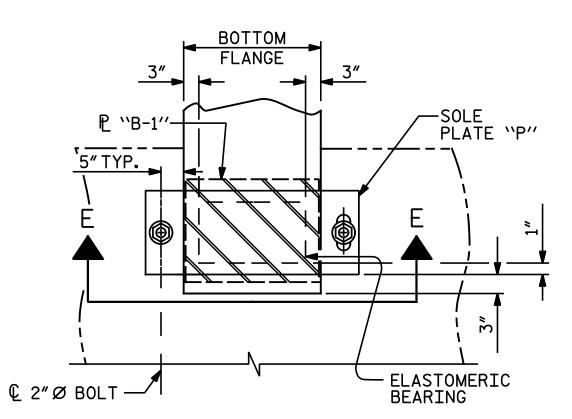
ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.110 KSI, IN ACCORDANCE WITH AASHTO M251.

STEEL PLATES IN BEARING PADS SHALL CONFORM TO ASTM A1011 GRADE 36, TYPE 1.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

ALL SOLE PLATES SHALL BE AASHTO M270 GRADE 36.



TYPICAL HALF-PLAN (SHOWING FIXED END)

TYPICAL HALF-PLAN (SHOWING EXPANSION END)

PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 1391+19.65 -L-



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE

ELASTOMERIC BEARING DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084

			REVIS	SIO	NS		SHEET NO
00	NO.	BY:	DATE:	NO.	BY:	DATE:	S3-18
	1			8			TOTAL SHEETS
	2			4 3			44

SOLE PLATE DETAILS ("P")

(16 REQ'D)

(16 REQ'D

	DEAD LOAD DEFLECTION TABLE FOR SPAN A										
0.6"Ø LOW RELAXATION		BEAMS 1 & 16									
TENTH POINTS	0.00	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	1.00
CAMBER (BEAM ALONE IN PLACE)	0.000	0.123	0,233	0.320	0.374	0.393	0.374	0.320	0,233	0.123	0.000
DEFLECTION DUE TO SUPERIMPOSED D.L. *	0.000	0.065	0.128	0.177	0.208	0.219	0.208	0.177	0.128	0.065	0.000
FINAL CAMBER	0"	11/16"	11/4"	1 / ₆ "	2"	2 / ₁₆ "	2"	1 ^{II} / _{I6} "	11/4"	11/16"	0"
0.6" Ø LOW RELAXATION					BEAM:	S 2 THROL	JGH 15				
TENTH POINTS	0.00	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	1.00
CAMBER (BEAM ALONE IN PLACE)	0.000	0.123	0,233	0.320	0.374	0.393	0.374	0.320	0,233	0.123	0.000
DEFLECTION DUE TO SUPERIMPOSED D.L. *	0.000	0.070	0.137	0.190	0,223	0.235	0.223	0.190	0.137	0.070	0.000
FINAL CAMBER	0"	5/8"	11/8"	1%6″	1 ¹³ / ₁₆ "	17/8"	1 ^{l3} / _{l6} "	1%6"	11/8"	5/8"	0"

^{*} INCLUDES FUTURE WEARING SURFACE.

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

APPLY EPOXY PROTECTIVE COATING TO END OF BEAM SURFACES INDICATED IN ELEVATION VIEW.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BEAM ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BEAM SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6,000 PSI.

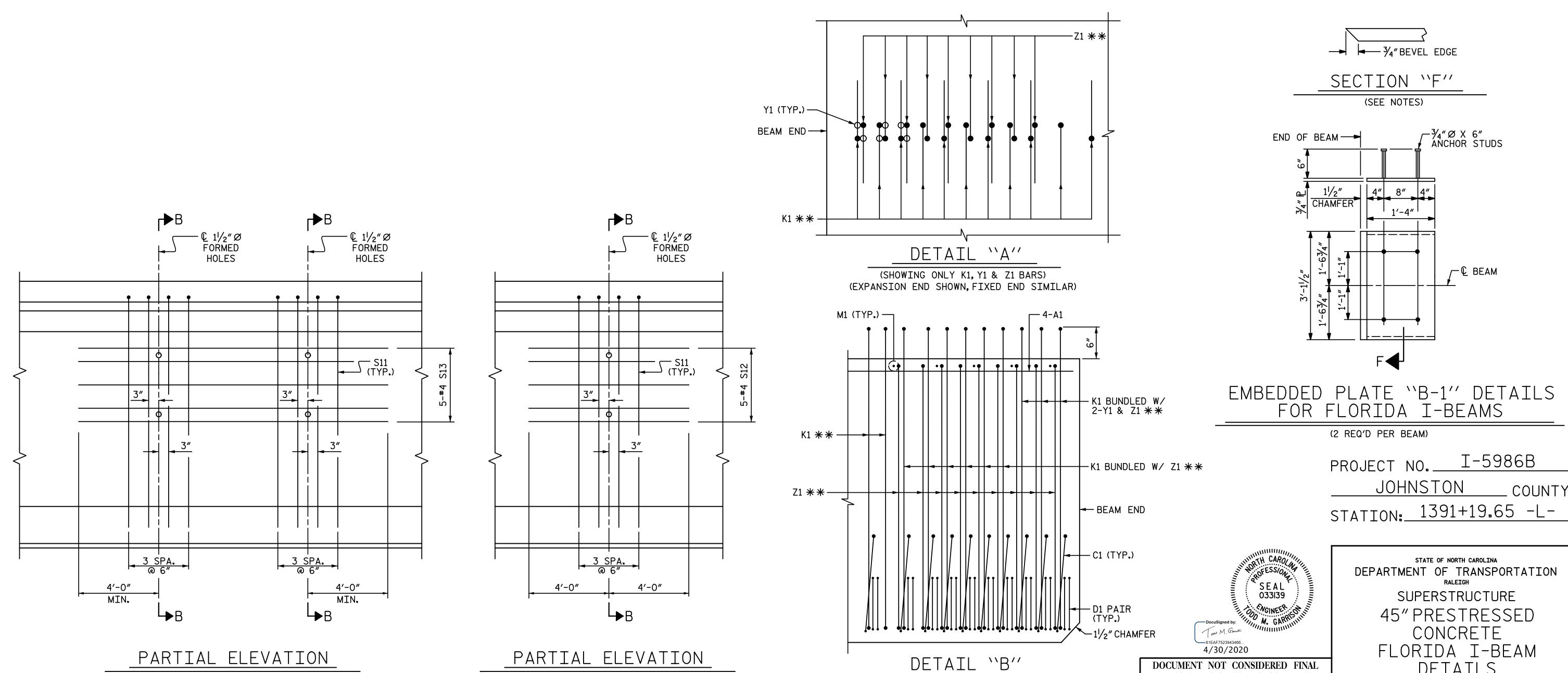
DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE BEAM.

THE TOP SURFACE OF THE BEAM, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF $\frac{1}{4}$ ".

WHEN DRAPED STRANDS ARE DETAILED, THE LONGITUDINAL LOCATION OF THE HOLD DOWN DEVICES SHALL BE WITHIN 6"OF THE LOCATION SHOWN AND THE CENTER OF GRAVITY OF THE GROUP OF DRAPED STRANDS SHALL BE LOCATED WITHIN $\frac{1}{2}$ " OF THE THEORETICAL LOCATION SHOWN.

TIE "K" AND "Z" BARS TO FULLY BONDED STRANDS IN THE BOTTOM OR CENTER ROW.

FOR 45" PRESTRESSED CONCRETE FLORIDA I-BEAM, SEE SPECIAL PROVISIONS.



(FLANGES NOT SHOWN FOR CLARITY)

(FIXED END SHOWN, EXPANSION END SIMILAR)

** ALTERNATE DIRECTION OF BAR ENDS

DRAWN BY : N. B. SPEAKS DATE : 7-18-19 CHECKED BY : T. M. GARRISON DATE : 4-17-20

SHOWING INTERMEDIATE STEEL DIAPHRAGM REINFORCING STEEL FOR INTERIOR BEAMS

FOR "SECTION B-B", SEE "45" PRESTRESSED CONCRETE FLORIDA I-BEAM" SHEET.

SHOWING INTERMEDIATE STEEL DIAPHRAGM REINFORCING STEEL FOR EXTERIOR BEAMS

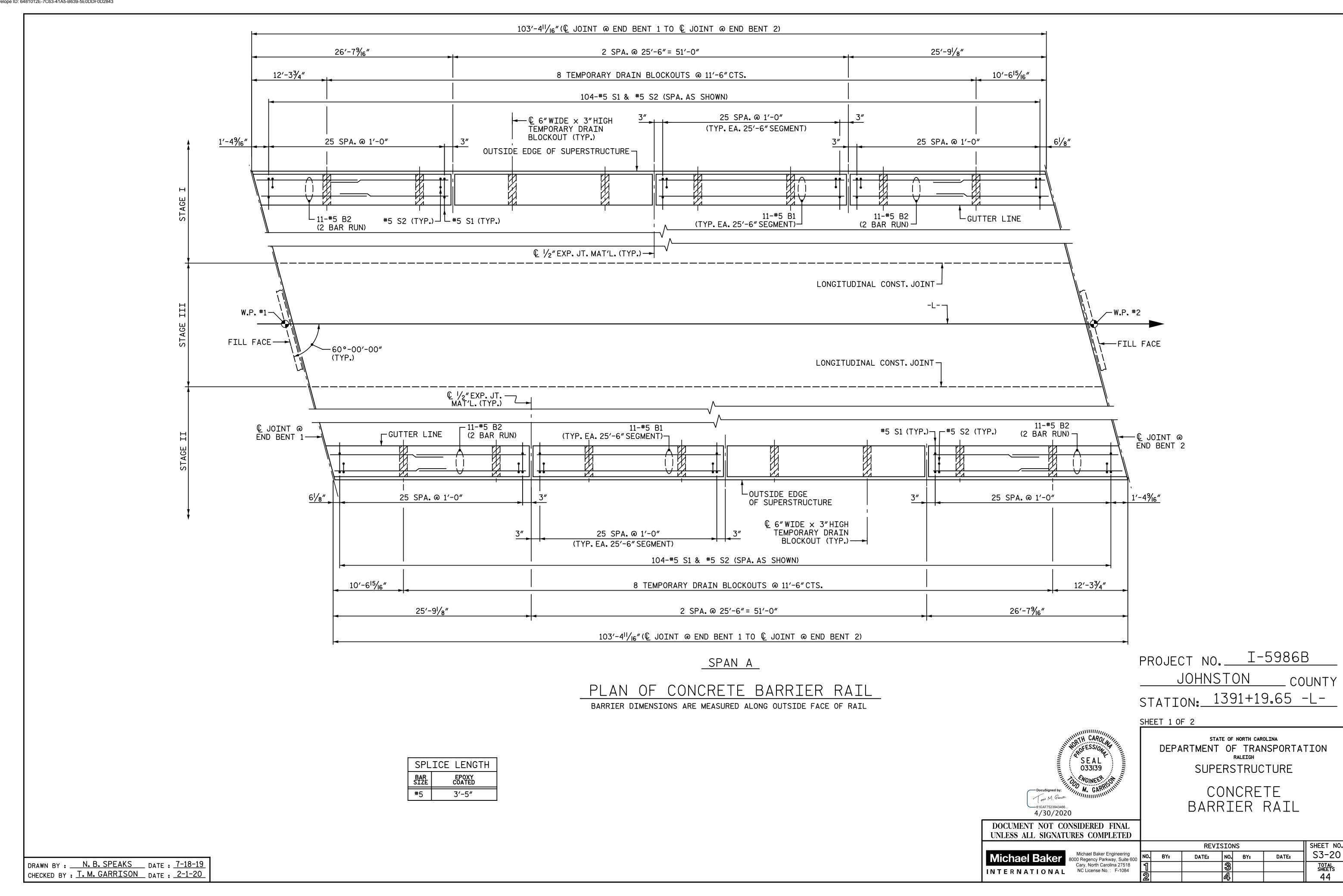
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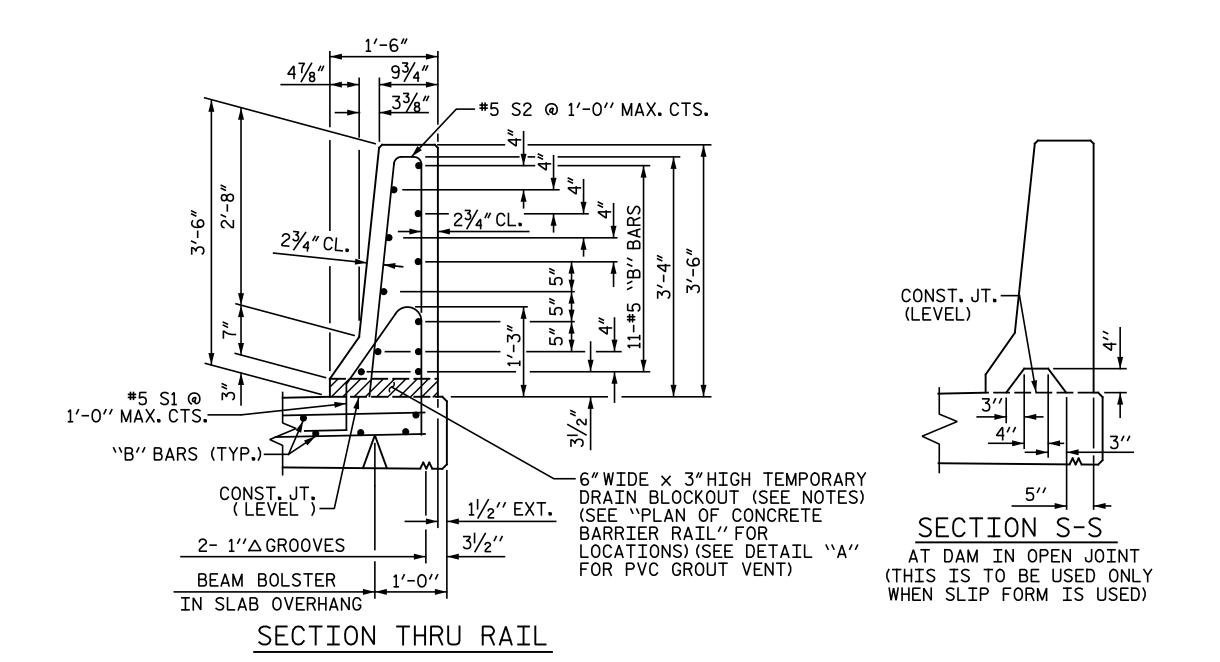
Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 INTERNATIONAL NC License No.: F-1084

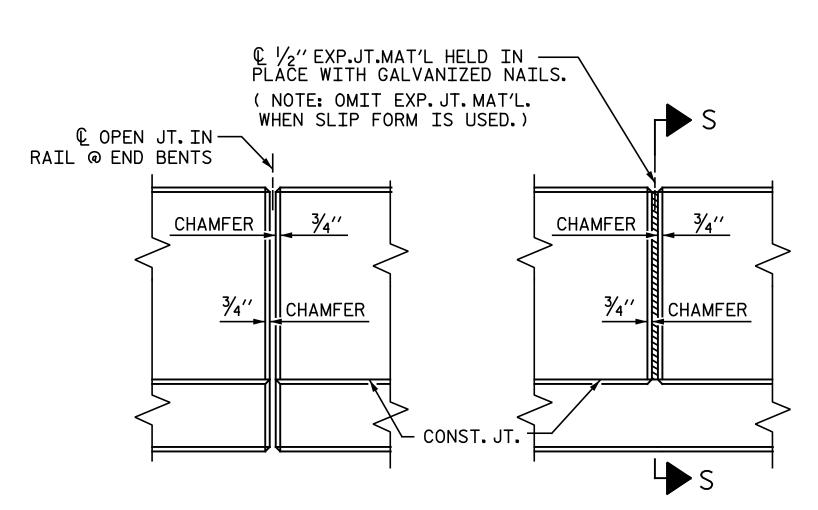
FLORIDA I-BEAM DETAILS

SHEET NO. **REVISIONS** S3-I9 DATE: NO. BY: DATE: TOTAL SHEETS 44

ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "FINAL CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM),







ELEVATION AT EXPANSION JOINTS

BARRIER RAIL DETAILS

NOTES

THE BARRIER RAIL IN THE SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

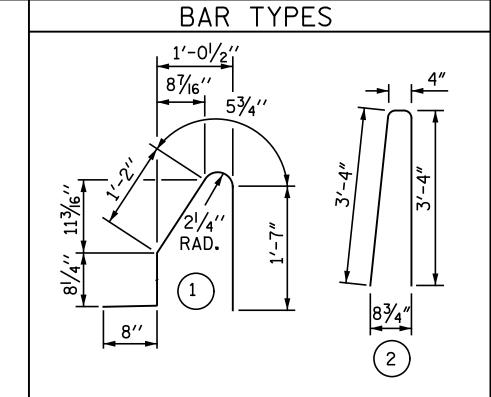
ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, 1/2"IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

TEMPORARY DRAIN BLOCKOUTS FOR STAGE I AND STAGE II CONDITIONS SHALL BE FILLED WITH APPROVED GROUT UPON COMPLETION OF STAGE III CONSTRUCTION.

TEMPORARY DRAIN BLOCKOUTS MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH CONCRETE BARRIER RAIL REINFORCEMENT AND 1/2" EXPANSION JOINTS.

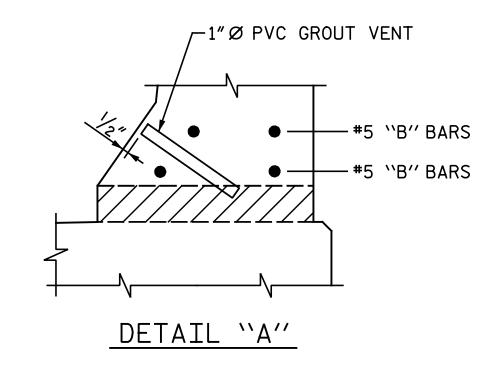
THE COST OF THE PVC GROUT VENTS SHALL BE INCLUDED IN THE UNIT PRICE BID FOR CONCRETE BARRIER RAIL.



ALL BAR DIMENSIONS ARE OUT TO OUT

ALL DAN DIMENSIONS AND OUT TO OUT										
	BI	ILL (OF M	ATERIA	۱L					
	FOR CO	NCRET	E BAR	RIER RAIL	ONLY					
		S	TAGE	ΞΙ						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT					
₩ B1	22	#5	STR.	25' - 1"	576					
 ₩ B2	44	#5	STR.	14' - 10"	681					
* S1	104	#5	1	4' - 7"	497					
* S2	104	#5	2	7' - 0"	759					
* EPOXY COATED										
REINFORCING STEEL LBS. 2,513										
CLASS AA CONCRETE CU. YDS. 14.0										
CONC	RETE	BARRIE	R RAI	L LIN.FT.	103.39					
		S	TAGE	II						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT					
₩ B1	22	#5	STR.	25' - 1"	576					
₩ B2	44	#5	STR.	14' - 10"	681					
* S1	104	#5	1	4' - 7"	497					
* S2	104	#5	2	7' - 0"	759					
* EPOXY COATED LBS. 2,513										
	CLASS AA CONCRETE CU. YDS. 14.0									
			R RAI							

QUANTITIES DO NOT INCLUDE BARRIER RAILS ON THE APPROACH SLABS, FOR BARRIER RAILS ON THE APPROACH SLABS, SEE "BRIDGE APPROACH SLAB DETAILS" SHEET.



PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 1391+19.65 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

CONCRETE BARRIER RAIL

4/30/2020

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

REVISIONS

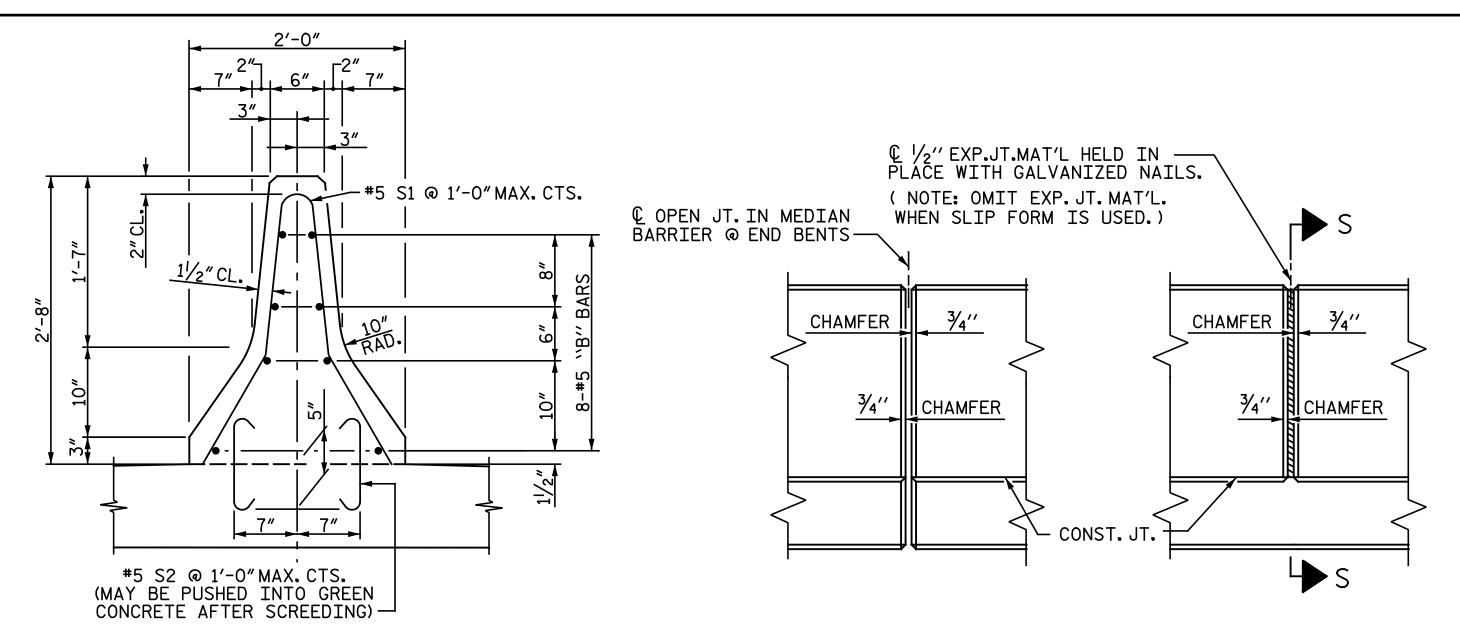
Michael Baker Eng 8000 Regency Parkwa Cary, North Carolin NC License No. :

Jou M. Game

LETED						_	_	
			REVI	SIO	NS		SHEET NO.	
Engineering way, Suite 600	NO.	BY:	DATE:	NO.	BY:	DATE:	S3-2I	
olina 27518 F-1084	1			3			TOTAL SHEETS	
	2			4			44	

STD. NO. CBR1

ASSEMBLED BY : N. B. SPEAK CHECKED BY : T. M. GARRISO		
DRAWN BY: ARB 5/87 CHECKED BY: SJD 9/87	REV. 7/I2 REV. 6/I3 REV. I2/I7	MAA/GM MAA/GM MAA/THC



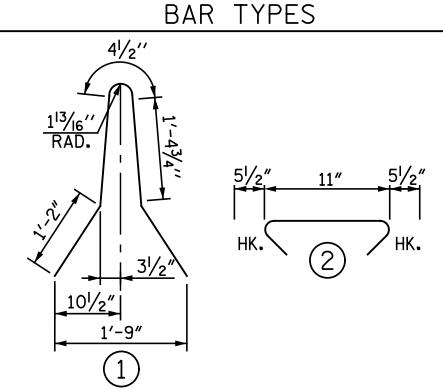
NOTES

THE CONCRETE MEDIAN BARRIER IN THE SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN CONCRETE MEDIAN BARRIER SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ "IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE CONCRETE MEDIAN BARRIER AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN CONCRETE MEDIAN BARRIER EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF CONCRETE MEDIAN BARRIER SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED DOWELS IN PLACE OF THE #5 S2 BARS DETAILED, LEVEL 2 FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE DOWELS IS 19 KIPS. THE DOWELS ARE TO BE HOOKED ON ONE END WITH A PROJECTION MATCHING THAT OF THE #5 S2 BARS. THE OPPOSITE END IS TO BE STRAIGHT AND EMBEDDED 6"INTO THE DECK SLAB.



ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL

FOR CONCRETE MEDIAN BARRIER ONLY

STAGE III BAR NO. SIZE TYPE LENGTH WEIGHT

#5 STR. 25′ - 1″ 16 419 *B2 32 #5 STR. 14' - 11" 498 * S1 | 104 | #5 | 1 | 5' - 6" 597 ***** S2 | 208 | *****5 | 2 | 1' - 10" 398

* EPOXY COATED

REINFORCING STEEL CLASS AA CONCRETE

CU. YDS. 10.6 CONCRETE MEDIAN BARRIER LIN. FT. 103.39 QUANTITIES DO NOT INCLUDE CONCRETE

LBS. 1,912

MEDIAN BARRIERS ON THE APPROACH SLABS. FOR MEDIAN BARRIERS ON THE APPROACH SLABS, SEE "BRIDGE APPROACH SLAB DETAILS" SHEET.

MEDIAN BARRIER DETAILS

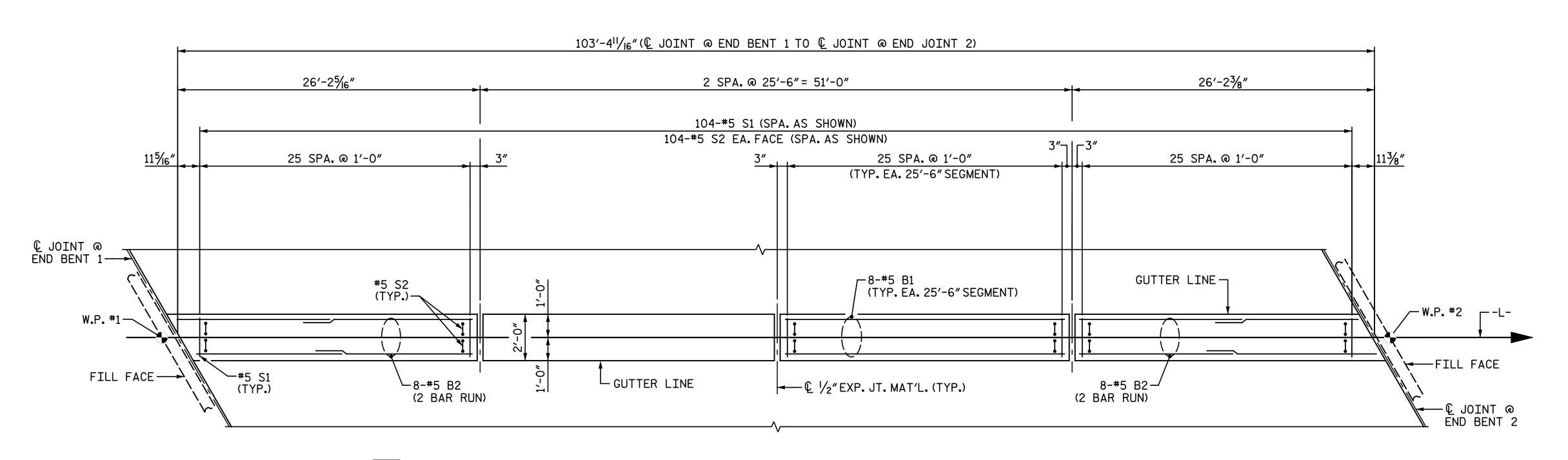
SECTION THRU MEDIAN BARRIER

CONST. JT.

SECTION S-S

AT DAM IN OPEN JOINT

(THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)



ELEVATION AT EXPANSION JOINTS

SPAN A

PLAN OF CONCRETE MEDIAN BARRIER

3′-5″

PROJECT NO. I-5986B JOHNSTON _ COUNTY STATION: 1391+19.65 -L-

SPLICE LENGTH EPOXY COATED

4/30/2020 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DocuSigned by:

ON CHRISTIAN M. GARR

Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 INTERNATIONAL NC License No.: F-1084

SUPERSTRUCTURE CONCRETE

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MEDIAN BARRIER

REVISIONS						SHEET NO.
10.	BY:	DATE:	NO.	BY:	DATE:	S3-22
1			3			TOTAL SHEETS
2			4			44

DRAWN BY : N. B. SPEAKS DATE : 8-5-19 CHECKED BY : T. M. GARRISON DATE : 3-11-20

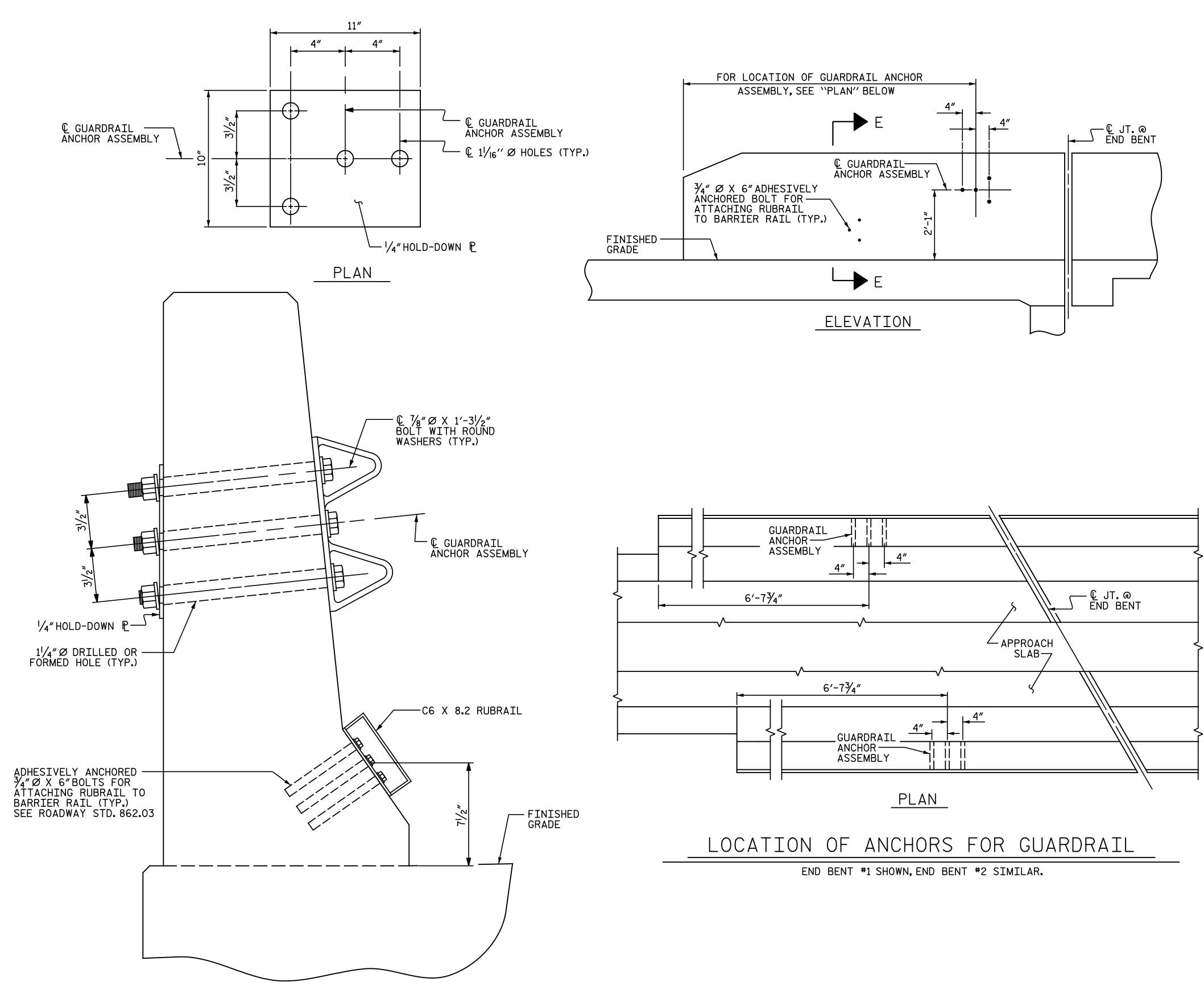
ASSEMBLED BY : N. B. SPEAKS CHECKED BY : T. M. GARRISON

DRAWN BY: TLA 5/06 CHECKED BY: GM 5/06

DATE : 7-18-19

DATE: 2-2-20

MAA/GM MAA/GM MAA/THC



SECTION E-E

GUARDRAIL ANCHOR ASSEMBLY DETAILS

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD-DOWN PLATE AND 4 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291, BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307, THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

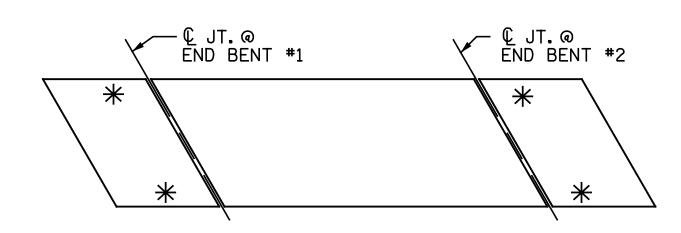
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT. SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED, ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



SKETCH SHOWING POINTS OF ATTACHMENTS

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. I-5986B JOHNSTON COUNTY STATION: 1391+19.65 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

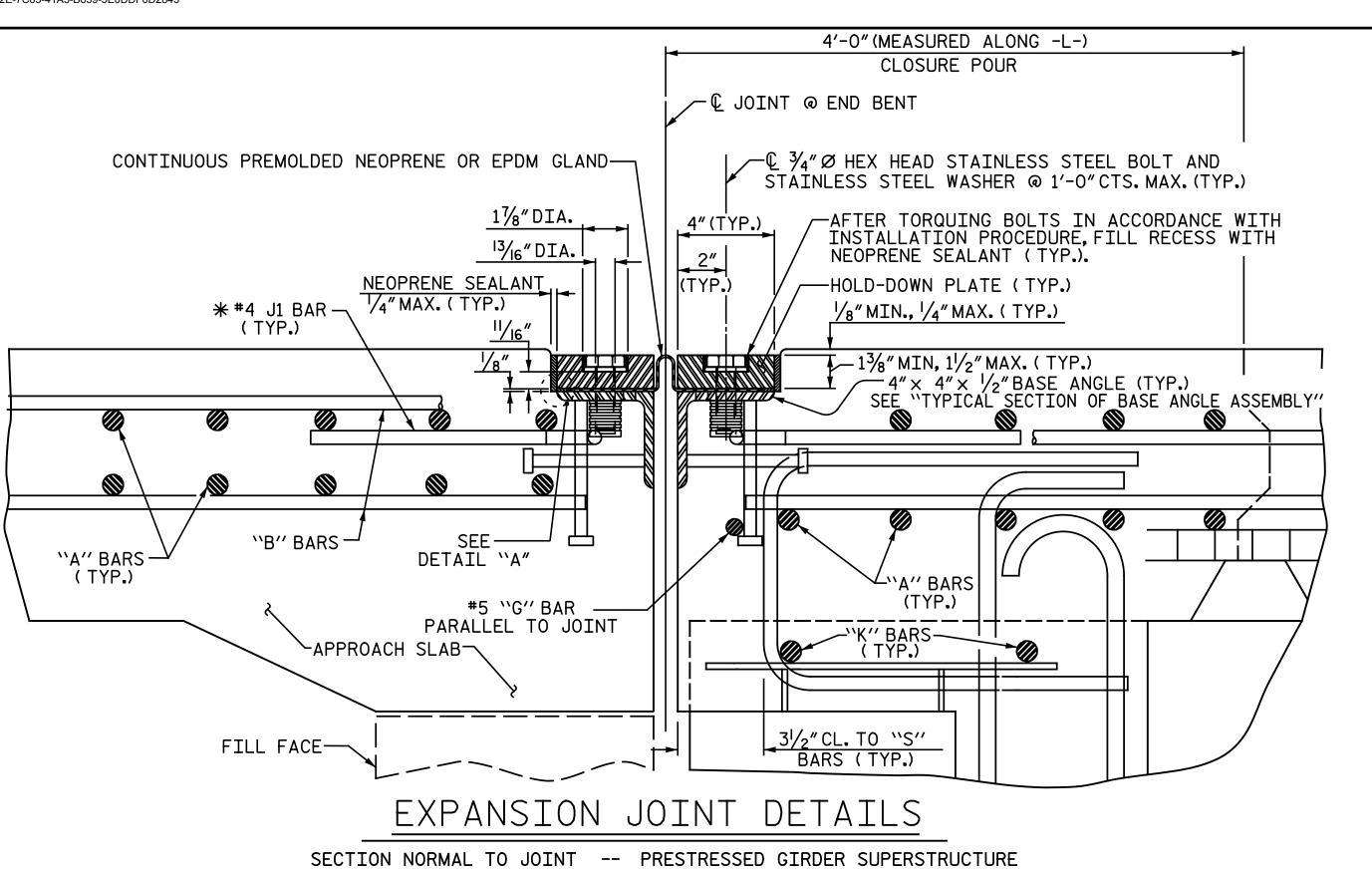
GUARDRAIL ANCHORAGE FOR BARRIER RAIL

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

INTERNATIONAL NC License No.: F-1084

Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518

SHEET NO. **REVISIONS** NO. BY: S3-23 DATE: DATE: 44



* THE QUANTITY OF #4 J1 BARS ON THE BILL OF MATERIAL IS BASED ON 1'-0" CENTERS.

ADDITIONAL J1 BARS WILL NOT BE REQUIRED.

-WHITE EDGE LINE

10

MAA/THC

MAA/THC

-OUTSIDE EDGE OF SUPERSTRUCTURE

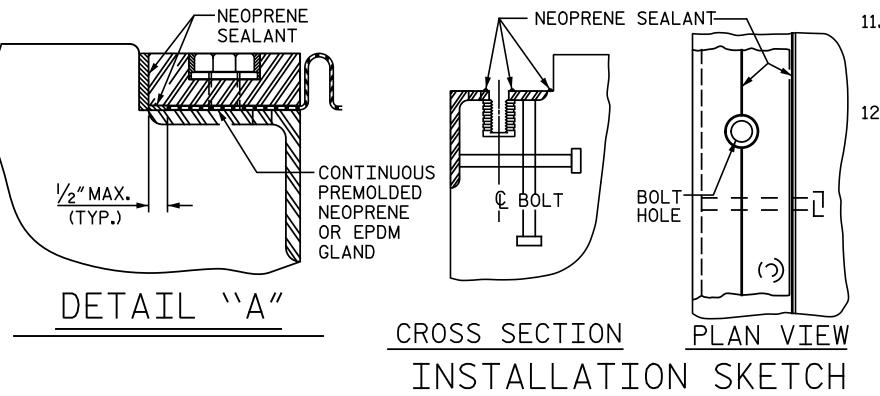
J1 BARS SHALL BE PLACED AT EACH VERTICAL STUD ANCHOR BOLT, IN THE EVENT THAT

THE NUMBER OF VERTICAL STUD ANCHORS EXCEEDS THE NUMBER OF J1 BARS SPECIFIED,

-GUTTER LINE

INSTALLATION PROCEDURE

- 1. A TEMPLATE OR OTHER SUITABLE DEVICE SHALL BE USED TO FORM THE TOP OF THE EXPANSION JOINT SEAL BLOCKOUT TO THE PROPER DEPTH AND WIDTH. THE TEMPLATE SHALL BE 41/8" TO 41/4" WIDE AND OF SUCH THICKNESS AS TO PROVIDE FOR CORRECT FINAL ELEVATION OF TOP OF HOLD-DOWN PLATES. THE TEMPLATE SHALL BE ATTACHED TO THE BASE ANGLE ASSEMBLY WITH THE 3/4" Ø HEX HEAD BOLTS PROVIDED FOR THE HOLD-DOWN PLATES. A 1" Ø HOLE SHALL BE PROVIDED IN THE TEMPLATE CENTERED OVER EACH WEEP HOLE IN THE 4"X 4"X 1/2" BASE ANGLE. OTHER METHODS OF INSURING DRAINAGE THROUGH WEEP HOLES MAY BE EMPLOYED SUBJECT TO ENGINEER'S APPROVAL.
- 2, AFTER THE CONCRETE HAS BEEN CAST ON BOTH SIDES OF THE JOINT. REMOVE THE TEMPLATE. THOROUGHLY CLEAN THE BOLT HOLES AND THE ANGLE PLATE. REMOVE ANY EXCESS CONCRETE THAT COMES OUT OF THE WEEP HOLES. ANY DAMAGED STEEL SHALL BE REPAIRED IN ACCORDANCE WITH THE SPECIAL PROVISION FOR THERMAL SPRAYED COATINGS (METALLIZATION).
- 3. LAY THE GLAND ON THE BASE ANGLE AND FIELD MARK THE GLAND FOR THE BOLT HOLES. HOLES IN THE GLAND SHALL BE PUNCHED 7/8" IN DIAMETER WITH A HAND PUNCH,
- 4. IN ORDER TO CHECK FOR PROPER ALIGNMENT, PLACE THE GLAND AND HOLD-DOWN PLATES ON THE BASE ANGLE. DO NOT APPLY NEOPRENE SEALANT, BOLT THE HOLD-DOWN PLATES TO THE BASE ANGLE BUT DO NOT TIGHTEN. THE ENGINEER SHALL INSPECT THE JOINT SEAL DEVICE FOR PROPER ALIGNMENT.
- 5. AFTER INSPECTION, REMOVE THE HOLD-DOWN PLATES AND GLAND, APPLY NEOPRENE SEALANT TO THE BASE ANGLE IN ACCORDANCE WITH THE "INSTALLATION SKETCH", PLACE GLAND AND HOLD-DOWN PLATES ON THE BASE ANGLE. BOLT THE HOLD-DOWN PLATES TO THE BASE ANGLE ASSEMBLY AND TORQUE THE BOLTS TO 88 FT-LBS WITH A TORQUE WRENCH, CHECK THE TORQUE AFTER THREE (3) HOURS AND, IF NECESSARY, RETIGHTEN TO 88 FT-LBS. A FINAL CHECK SHALL BE MADE AT SEVEN (7) DAYS, TORQUE SHALL NOT BE LESS THAN 80 FT-LBS AFTER SEVEN (7) DAYS.
- 6. AFTER PROPER TORQUING, CLEAN THE BOLT HOLE RECESSES, THE RECESS BETWEEN THE JOINT SEAL DEVICE AND CONCRETE, AND THE LIFTING HOLES IN THE HOLD-DOWN PLATE, AND COMPLETELY FILL THE RECESSES AND LIFTING HOLES WITH NEOPRENE SEALANT.



 $\mathbb{Q}^{1/2}$ " Ø WEEP HOLE-± 1'-0" CTS.

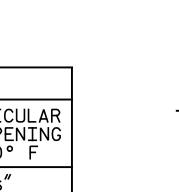
 $L 4 \times 4 \times \frac{1}{2}$

SURFACE TO BE-METALLIZED

GENERAL NOTES

- 1. FOR EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.
- 2. ALL PLATES AND ANGLES SHALL CONFORM TO AASHTO M270 GRADE 36 STEEL OR APPROVED EQUAL, ALL HOLD-DOWN BOLTS SHALL CONFORM TO ASTM F593 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL CONFORM TO ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL, ALL STUD ANCHORS SHALL CONFORM TO AASHTO M169, GRADES 1010 THRU 1020 OR APPROVED EQUAL. ALL CONCRETE INSERTS SHALL BE CLOSED END AND SHALL CONFORM TO AASHTO M169, GRADE 12L14. TENSILE CAPACITY SHALL BE 3000 LBS, MINIMUM.
- 3. A PREMOLDED CORRUGATED OR NON-CORRUGATED GLAND SHALL BE USED FOR JOINTS SKEWED BETWEEN 50° THRU 130° FOR JOINTS SKEWED LESS THAN 50° OR MORE THAN 130°, ONLY A CORRUGATED GLAND SHALL BE USED.
- 4. CLOSED END FERRULES AND STUD ANCHORS SHALL BE SHOP WELDED AND ALL HOLES SHALL BE SHOP DRILLED AS SHOWN ON PLANS, STUD ANCHORS SHALL BE ELECTRIC ARC END WELDED WITH COMPLETE FUSION.
- 5. SURFACES COMING IN CONTACT WITH NEOPRENE SHALL BE GROUND SMOOTH PRIOR TO METALLIZING.
- 6. UPON COMPLETION OF SHOP FABRICATION, THE HOLD-DOWN PLATE AND BASE ANGLE ASSEMBLY, AS SHOWN IN THE "TYPICAL SECTION OF BASE ANGLE ASSEMBLY" SHALL BE METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.
- 7. THE COVER PLATES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.
- B.BASE ANGLE ASSEMBLY SHALL BE CONTINUOUS FOR THE LENGTH OF THE JOINT. AT CROWN BREAKS. THE ENDS OF THE BASE ANGLE ASSEMBLY SHALL BE CUT PARALLEL TO THE BRIDGE CENTERLINE FOR SKEWS LESS THAN 80° AND GREATER THAN 100°. FINISHED WELD SHALL BE REPAIRED IN ACCORDANCE WITH THE SPECIAL PROVISION FOR THERMAL SPRAYED COATINGS (METALLIZATION).
- 9. FIELD SPLICES OF HOLD-DOWN PLATES SHALL BE KEPT TO A MINIMUM. CONTRACTOR SHALL FURNISH DETAILED PLANS SHOWING PROPOSED SPLICE LOCATIONS FOR APPROVAL, HOLD-DOWN PLATES SHALL NOT EXCEED 20' LENGTHS UNLESS APPROVED BY THE ENGINEER.
- 10. NO ALTERNATE JOINT DETAILS SHALL BE PERMITTED IN LIEU OF THOSE SHOWN ON THESE PLANS.
- 11. THE CONTRACTOR MAY, AT HIS OPTION, USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF CONCRETE INSERTS FOR COVER PLATES. THE YIELD LOAD OF THE $\frac{3}{4}$ " Ø BOLT IS 10 KIPS. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.
- 12. THE FABRICATOR SHALL PROVIDE $\frac{1}{2}$ Ø THREADED HOLES IN THE HOLD-DOWN PLATES TO ASSIST IN LIFTING AND PLACING. THE HOLES SHALL BE $\frac{3}{4}$ DEEP AT 6'-0" MAXIMUM SPACING AND A MINIMUM OF TWO HOLES PER PLATE.

(TYP.)



MOVEMENT AND SETTING AT JOINT PERPENDICULAR PERPENDICULAR PERPENDICULAR JOINT OPENING JOINT OPENING MOVEMENT BENT NO. SKEW ANGLE (ALONG & RDWY) AT 45° F AT 60° F AT 90° F 2¹/₁₆" 23/8" $2^{1}/_{4}''$ END BENT 1 60°-00'-00" 2¹/₄" $2^{1}/_{4}''$ 2¹/₄" END BENT 2 60°-00'-00"

 $-\mathbb{Q} \frac{1}{2}$ " Ø STUD ANCHOR, MIN. 5"LONG @ 1'-0"CTS. MAX.

 $\mathbb{Q}^{13}/_{16}$ " Ø HOLE FOR $\frac{3}{4}$ " Ø

HEX BOLT AND ¢ FERRULE.

3/16" MIN. (TYP.)

 $\frac{}{}$ \fra

PROJECT NO. I-5986B JOHNSTON COUNTY 1391+19.65 -L-SHEET 1 OF 3

DETAIL- FIELD WELD

SPLICE OF BASE ANGLE

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SEAL STANDARD 033|39 EXPANSION JOINT

SEAL DETAILS

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ONTONEER Tou M. Game 4/30/2020 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

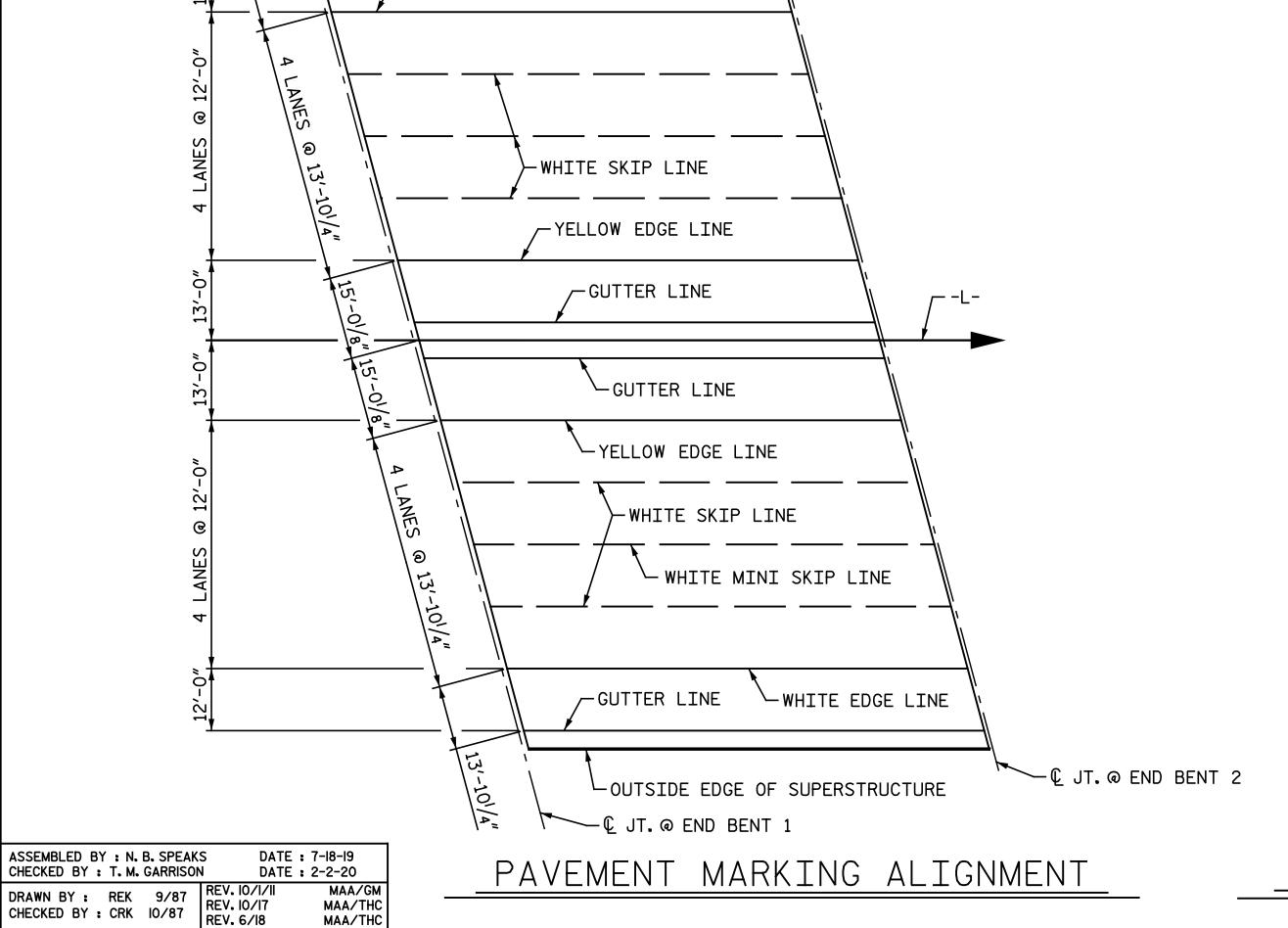
TYPICAL SECTION OF BASE ANGLE ASSEMBLY

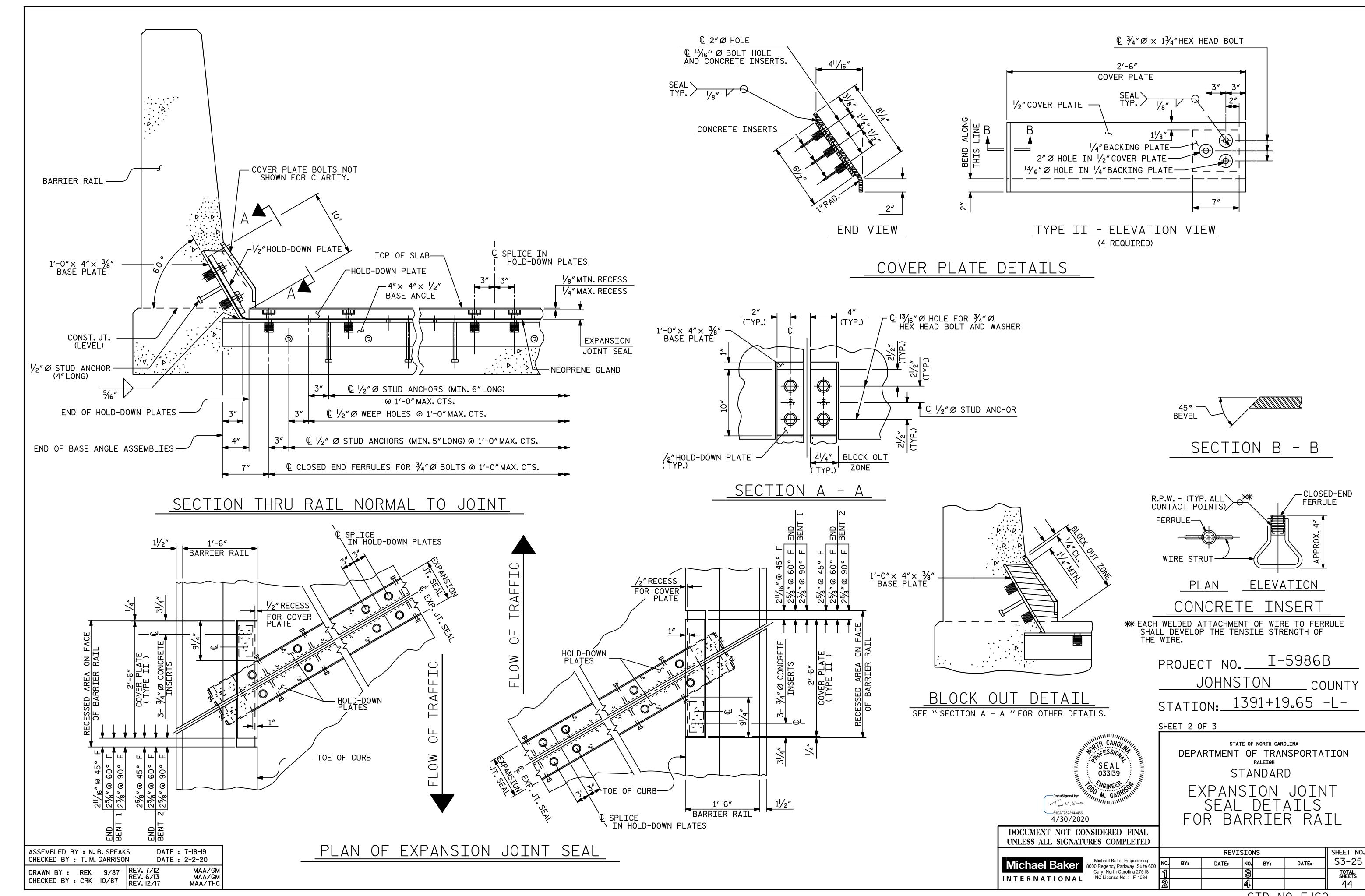
REVISIONS SHEET NO. S3-24 NO. BY: Michael Baker Engineering 8000 Regency Parkway, Suite 600 DATE: DATE: BY: Cary, North Carolina 27518 44

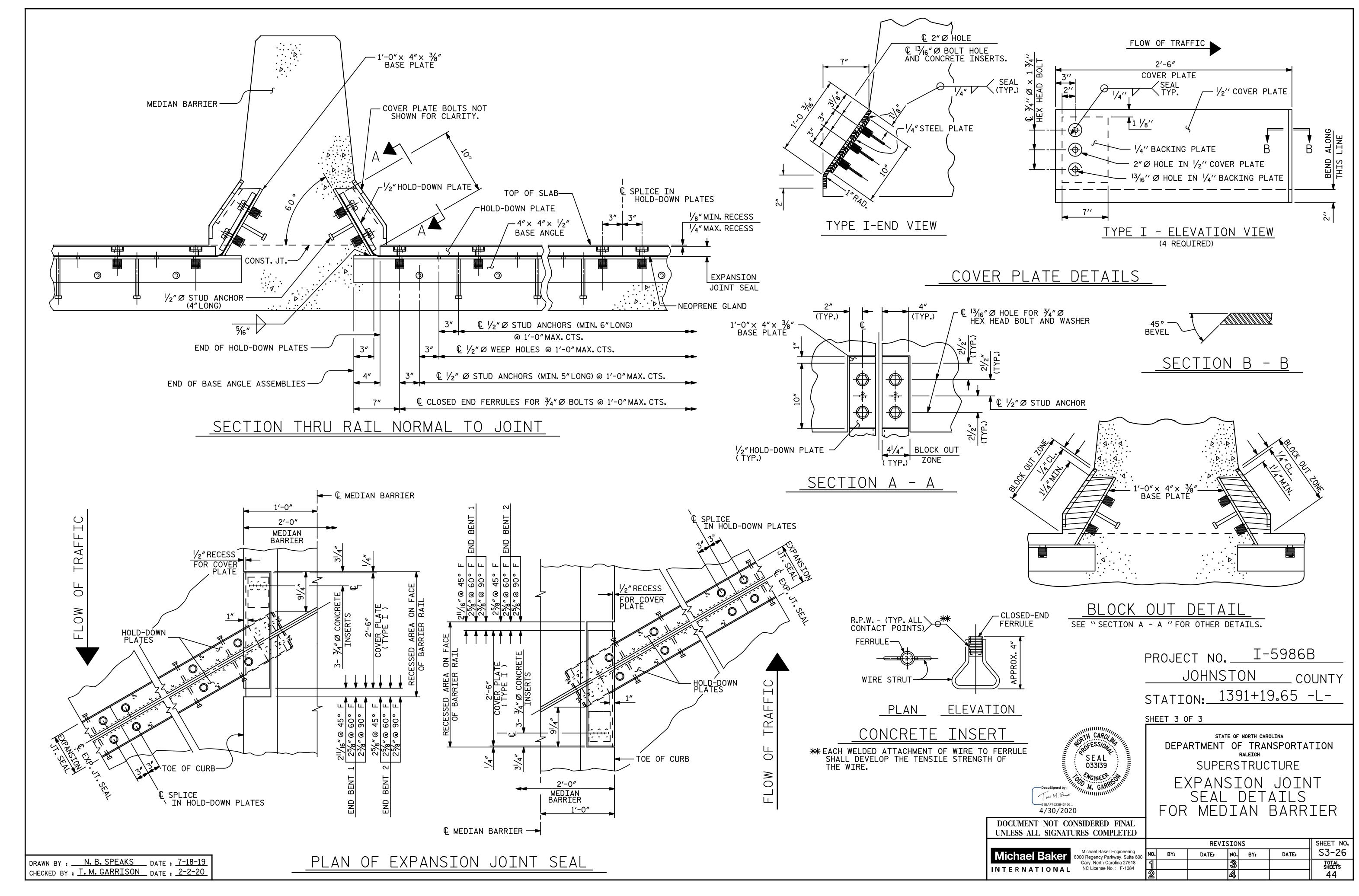
STD. NO. EJS1

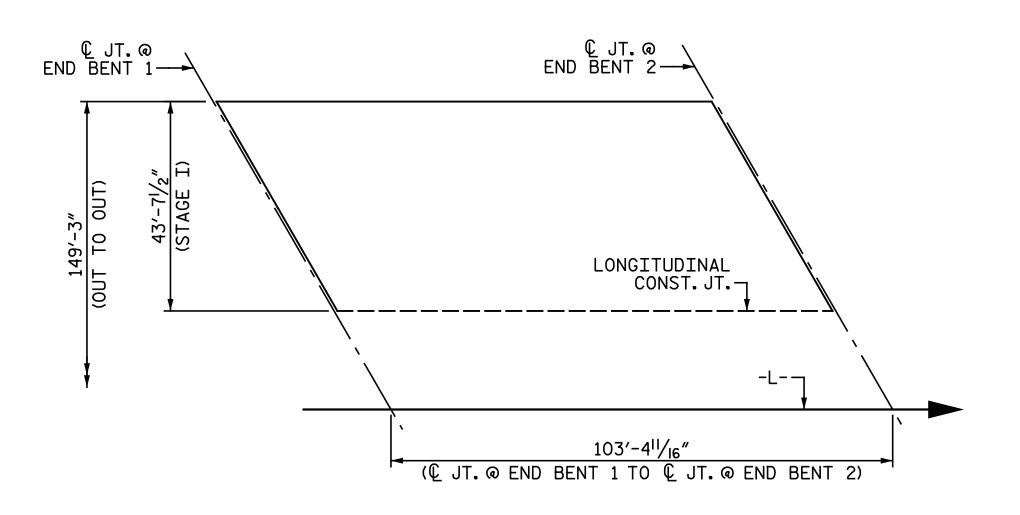
-HORIZONTAL

└─ VERTICAL LEG

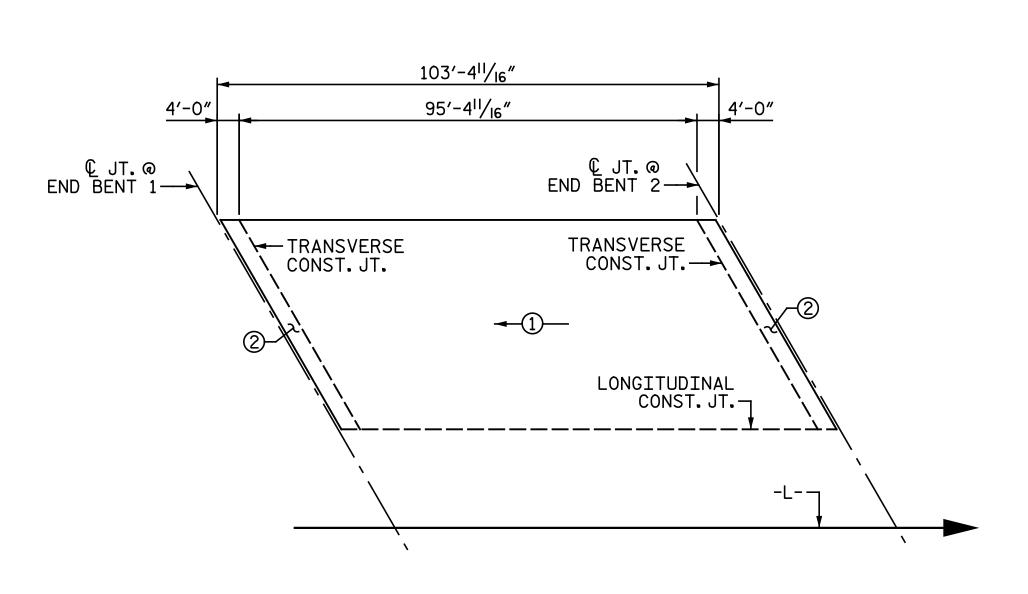








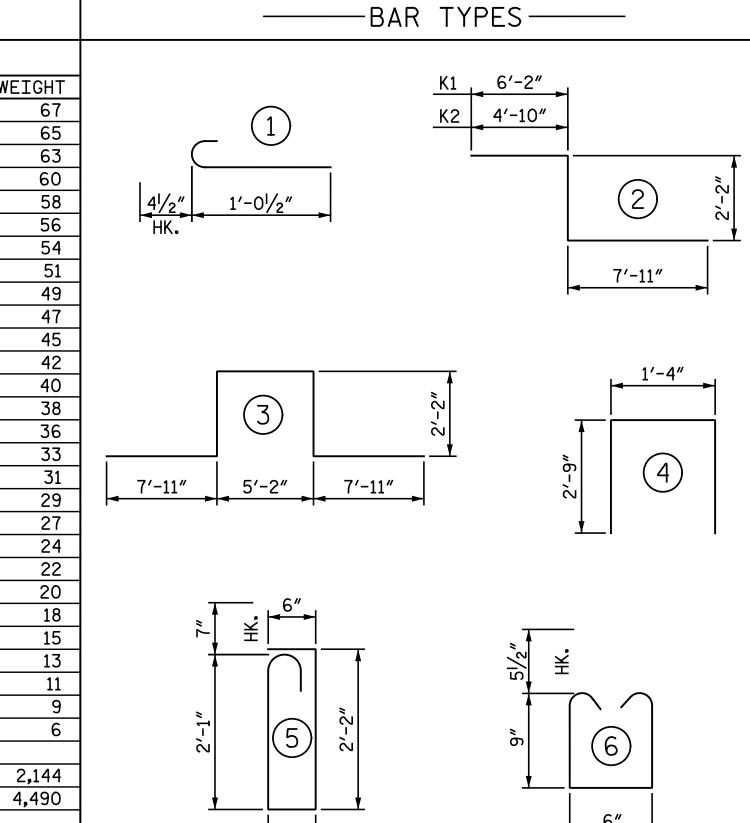
LAYOUT FOR COMPUTING AREA OF ______ REINFORCED CONCRETE DECK SLAB (STAGE I SQ.FT. = 4,510)



POURING SEQUENCE

DENOTES POUR NUMBER AND DIRECTION

REINFORCING BAR SCHEDULE											
SPAN A - STAGE I											
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	123	#5	STR.	43′ - 3″	5 , 548	A211	2	#5	STR.	32' - 2"	67
A2	123	#5	STR.	43' - 3"	5 , 548	A212	2	#5	STR.	31' - 1"	65
* A3	20	#5	STR.	46′ - 11″	979	A213	2	#5	STR.	30′ - 0″	63
						A214	2	#5	STR.	28′ - 11″	60
* A101	2	#5	STR.	43' - 0"	90	A215	2	#5	STR.	27' - 10"	58
* A102	2	#5	STR.	41' - 11"	87	A216	2	#5	STR.	26' - 9"	56
* A103	2	#5	STR.	40' - 10"	85	A217	2	#5	STR.	25' - 8"	54
* A104	2	#5	STR.	39' - 9"	83	A218	2	#5	STR.	24' - 8"	51
* A105	2	#5	STR.	38' - 8"	81	A219	2	#5	STR.	23' - 7"	49
* A106	2	#5	STR.	37' - 7"	78	A220	2	#5	STR.	22' - 6"	47
* A107	2	#5 #5	STR.	36' - 6"	76	A221	2	#5 #5	STR.	21' - 5"	45
* A108	2	#5 #5	STR.	35' - 5"	74	A222	2	#5 #5	STR.	20' - 4"	42
* A109	2	#5 #c	STR.	34' - 4"	72	A223	2	#5 #5	STR.	19' - 3"	40
* A110	2 2	#5 #5	STR.	33' - 3"	69	A224	2	#5 #5	STR.	18' - 2" 17' - 1"	38
* A111	2	#5	STR.	32' - 2" 31' - 1"	67 65	A225	2	#5	STR.	16' - 0"	36
* A112	2	#5	STR.	30' - 0"	65 63	A226	2	#5	STR.	14' - 11"	33 31
* A113	2	#5	STR. STR.	28' - 11"	60	A227 A228	2	#5	STR. STR.	13' - 10"	29
* A114 * A115	2	#5	STR.	27' - 10"	58	A226 A229	2	#5	STR.	12' - 9"	27
* A115	2	#5	STR.	26' - 9"	56	A223	2	#5	STR.	11' - 8"	24
* A117	2	#5	STR.	25' - 8"	54	A230	2	#5	STR.	10' - 7"	22
* A117	2	#5	STR.	24' - 8"	51	A232	2	#5	STR.	9' - 6"	20
* A119	2	#5	STR.	23' - 7"	49	A233	2	#5	STR.	8' - 5"	18
* A113	2	#5	STR.	22' - 6"	47	A234	2	#5	STR.	7' - 4"	15
* A121	2	#5	STR.	21' - 5"	45	A235	2	#5	STR.	6' - 3"	13
* A122	2	#5	STR.	20' - 4"	42	A236	2	#5	STR.	5' - 2"	11
* A123	2	#5	STR.	19' - 3"	40	A237	2	#5	STR.	4' - 1"	9
* A124	2	#5	STR.	18' - 2"	38	A238	2	#5	STR.	3' - 0"	6
* A125	2	#5	STR.	17' - 1"	36						
* A126	2	#5	STR.	16' - 0"	33	★ B1	90	#4	STR.	35' - 8"	2,144
* A127	2	#5	STR.	14' - 11"	31	B2	82	#5	STR.	52' - 6"	4,490
* A128	2	#5	STR.	13' - 10"	29						·
* A129	2	#5	STR.	12' - 9"	27	* D1	159	#5	STR.	7' - 0"	1,161
* A130	2	#5	STR.	11' - 8"	24	D2	159	#5	STR.	6' - 6"	1,078
* A131	2	#5	STR.	10' - 7"	22	₩ D3	20	#5	STR.	8' - 6"	177
* A132	2	#5	STR.	9' - 6"	20						
* A133	2	#5	STR.	8' - 5"	18	★ G1	2	#5	STR.	49′ - 11″	104
* A134	2	#5	STR.	7' - 4"	15						-
* A135	2	#5	STR.	6' - 3"	13	* J1	94	#4	1	1' - 5"	89
* A136	2	#5	STR.	5' - 2"	11						
* A137	2	#5	STR.	4' - 1"	9	∗ K1	4	#8	2	16' - 3"	174
* A138	2	#5	STR.	3' - 0"	6	 ★ K2	4	#8	2	14' - 11"	159
						 ★ K3	12	#8	3	25' - 4"	812
A201	2	#5	STR.	43' - 0"	90	* K4	24	#6	STR.	9' - 2"	330
A202	2	#5	STR.	41' - 11"	87	₩ K5	16	#4	STR.	9' - 10"	105
A203	2	#5	STR.	40' - 10"	85						
A204	2	#5	STR.	39' - 9"	83	* S1	48	#4	4	6' - 10"	219
A205	2	#5	STR.	38' - 8"	81	* S2	48	#5	5	5' - 10"	292
A206	2	#5	STR.	37' - 7"	78	* S3	32	#5	6	2' - 11"	97
A207	2	#5	STR.	36' - 6"	76			110 2==			40.515
A208	2	#5	STR.	35' - 5"	74	·					
A209	2	#5 #5	STR.	34' - 4"	72	·					
A210	2	#5	STR.	33' - 3"	69	<u> </u>					



ALL BAR DIMENSIONS ARE OUT TO OUT

---SUPERSTRUCTURE BILL OF MATERIAL-

	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL
	(CU.YDS.)	(LBS.)	(LBS.)
STAGE I	_	12,940	14,214
POUR 1	139.4	-	_
POUR 2	25.0	1	_
TOTALS **	164.4	12,940	14,214

** QUANTITIES FOR BARRIER RAILS & MEDIAN BARRIER ARE NOT INCLUDED

GROOVING BRIDGE FLOORS

STAGE I

APPROACH SLABS 1,914 SQ.FT.
BRIDGE DECK 4,130 SQ.FT.

TOTAL 6,044 SQ.FT.

PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 1391+19.65 -L-

SHEET 1 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE

OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE

OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE

BILL OF MATERIAL STAGE I

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Cary, North Carolina 27518
NC License No.: F-1084

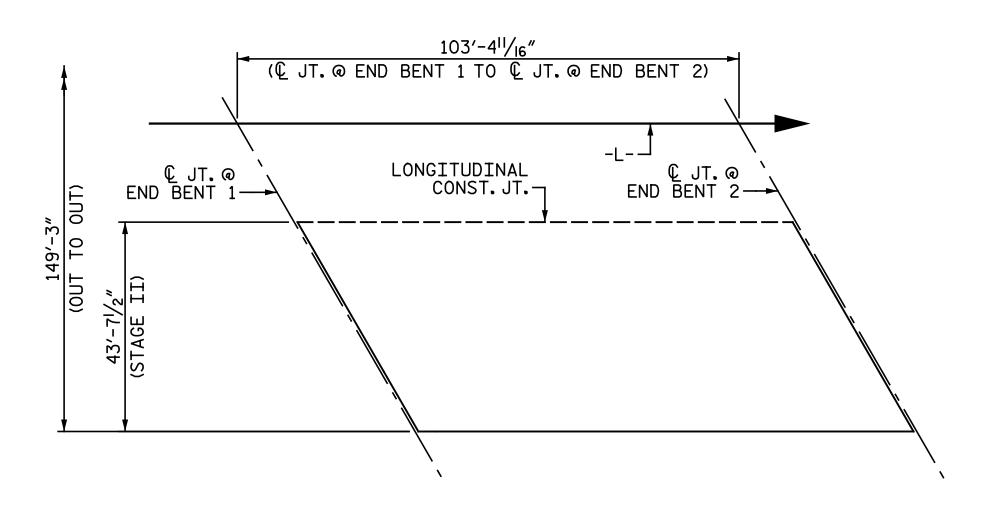
DOCUMENT NOT CONSIDERED FINAL

DocuSigned by:

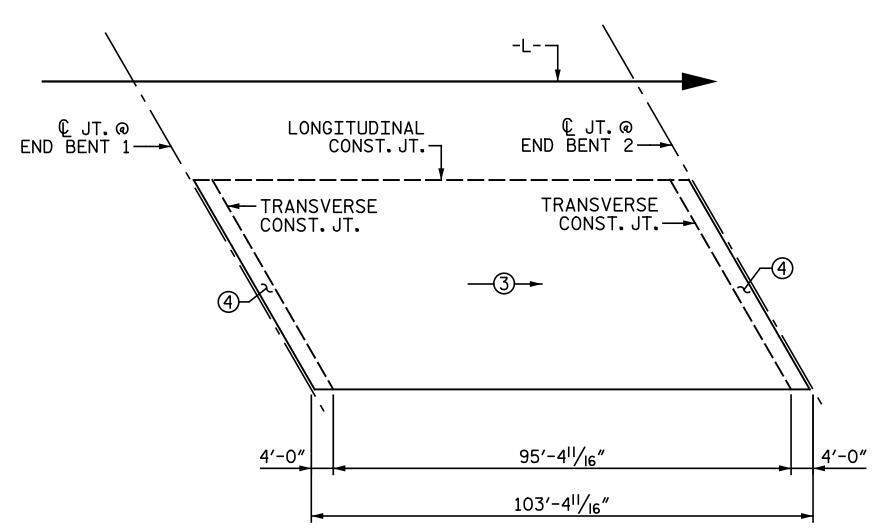
4/30/2020

D						_	_
			SHEET NO.				
ng e 600	NO.	BY:	DATE:	NO.	BY:	DATE:	S3-27
18 4	1			3			TOTAL SHEETS
	2			4			44

DRAWN BY: C.E. MAYHEW DATE: 12-10-19
CHECKED BY: T.M. GARRISON DATE: 12-15-19



LAYOUT FOR COMPUTING AREA OF ______ REINFORCED CONCRETE DECK SLAB (STAGE II SQ. FT. = 4,510)



REINFORCING BAR SCHEDULE SPAN A - STAGE II NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT 123 #5 | STR. | 43' - 3" 5,548 A211 #5 | STR. | 31' - 1" 65 #5 | STR. | 43' - 3" | A212 2 123 5,548 #5 STR. 46′ - 11″ A213 STR. 30' - 0" ***** A3 │ 20 979 #5 63 A214 2 #5 | STR. | 28' - 11" | 60 #5 STR. 27' - 10" #5 STR. 43' - 0" 90 | A215 | 2 58 ***** A101 #5 STR. 41' - 11" A216 2 #5 | STR. | 26' - 9" #5 STR. 40' - 10" 85 | A217 | 2 #5 | STR. | 25' - 8" #5 STR. 39' - 9" 83 | A218 | 2 #5 | STR. | 24′ - 8″ #5 STR. 38' - 8" #5 | STR. | 23' - 7" 49 81 | A219 | 2 #5 | STR. | 37' - 7" A220 STR. 47 78 #5 STR. 36′ - 6″ 45 ***** A107 A221 2 #5 | STR. | 21' - 5" #5 STR. 20' - 4" #5 STR. 35' - 5" 74 A222 2 42 #5 STR. 34' - 4" 72 A223 2 #5 STR. 19' - 3" 40 #5 STR. 33' - 3" 69 A224 2 #5 STR. 18' - 2" 38 #5 STR. 32' - 2" A225 2 #5 | STR. #5 STR. 31' - 1" * A112 65 | A226 | 2 #5 | STR. | 16' - 0" 33 #5 STR. 30' - 0" #5 STR. 14' - 11" ***** A113 A227 2 63 #5 STR. 28' - 11" #5 STR. 13' - 10" ***** A114 A228 2 29 #5 STR. 27' - 10" 58 A229 2 #5 STR. 12' - 9" ***** A115 27 #5 STR. 11' - 8" #5 STR. 26' - 9" 56 A230 2 24 #5 STR. 25′ - 8″ 54 A231 2 22 #5 | STR. | 10' - 7" #5 STR. 24' - 8" A232 2 #5 | STR. | 9' - 6" 20 #5 | STR. | 23' - 7" 49 | A233 | 2 #5 | STR. | 8' - 5" 18 #5 STR. 22' - 6" 47 A234 2 #5 | STR. #5 STR. 21' - 5" #5 | STR. | 6' - 3" ***** A121 45 | A235 | 2 13 #5 | STR. | 20' - 4" 42 A236 #5 | STR. | 5' - 2" #5 STR. 19' - 3" 40 | A237 | 2 #5 | STR. | 4' - 1" #5 STR. 18' - 2" 38 A238 2 #5 STR. 3' - 0" #5 STR. 17' - 1" #5 STR. 16' - 0" #4 STR. 35' - 8" 33 | * B1 | 90 2,144 B2 82 #5 STR. 14' - 11" #5 STR. 52' - 6" 31 4,490 #5 STR. 13' - 10" * A128 #5 STR. 12' - 9" 27 | * D1 | 159 #5 | STR. | 7' - 0" 1,161 #5 STR. 11' - 8" #5 STR. 6' - 6" 24 D2 | 159 | 1,078 #5 STR. 10' - 7" 22 ***** D3 20 #5 STR. 8' - 6" 177 #5 STR. 9' - 6" 20 #5 STR. 8' - 5" 18 ***** G1 2 ***** 5 STR. 49' - 11" #5 STR. 7' - 4" #5 | STR. | 6' - 3" 13 | * J1 | 94 | #4 | 1 | 1' - 5" #5 STR. 5' - 2" ***** A136 #5 STR. 4' - 1" 2 16' - 3" ***** A137 9 | * K1 | 4 #8 174 #5 | STR. | 3' - 0" 6 | * K2 | 4 #8 2 | 14' - 11" 159 **★** K3 | 12 #8 3 | 25' - 4" 812 #6 STR. 9' - 2" 90 ***** K4 24 #5 | STR. | 43' - 0" 330 #5 STR. 41' - 11" 87 ***** K5 16 #4 | STR. | 9' - 10" A203 2 #5 STR. 40' - 10" 85 #5 STR. 39' - 9" 83 ***** S1 48 4 6' - 10" 219 #5 STR. 38' - 8" 81 * S2 48 #5 292 5 | 5' - 10" 78 ***** S3 32 #5 #5 STR. 37′ - 7″ A206 2' - 11" 97 #5 STR. 36' - 6"

REINFORCING STEEL

72 * EPOXY COATED REINF. STEEL LBS. 14,214

74

-BAR TYPES-----K2 4'-10" 2 7′-11″ (4)5′-2″ 7′-11″ 7′-11″

EPOXY COATED REINFORCING CLASS AA REINFORCING CONCRETE STEEL STEEL

ALL BAR DIMENSIONS ARE OUT TO OUT

--- SUPERSTRUCTURE BILL OF MATERIAL---

(LBS.) (CU.YDS.) (LBS.) STAGE II 14,214 12,940 -POUR 3 139.4 _ _ 25.0 POUR 4 14,214 TOTALS ** 164.4 12,940

** QUANTITIES FOR BARRIER RAILS & MEDIAN BARRIER ARE NOT INCLUDED

POURING SEQUENCE —(#) → DENOTES POUR NUMBER AND DIRECTION

> GROOVING BRIDGE FLOORS STAGE II APPROACH SLABS 1,914 SQ.FT. 4,130 SQ.FT. BRIDGE DECK TOTAL 6,044 SQ.FT.

#5 STR. 35′ - 5″

#5 STR. 34' - 4"

#5 STR. 33' - 3"

A210 2

PROJECT NO. I-5986B JOHNSTON _ COUNTY STATION: 1391+19.65 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **SUPERSTRUCTURE** - ON WGINEER

BILL OF MATERIAL STAGE II

DOCUMENT NOT CONSIDERED FINAL

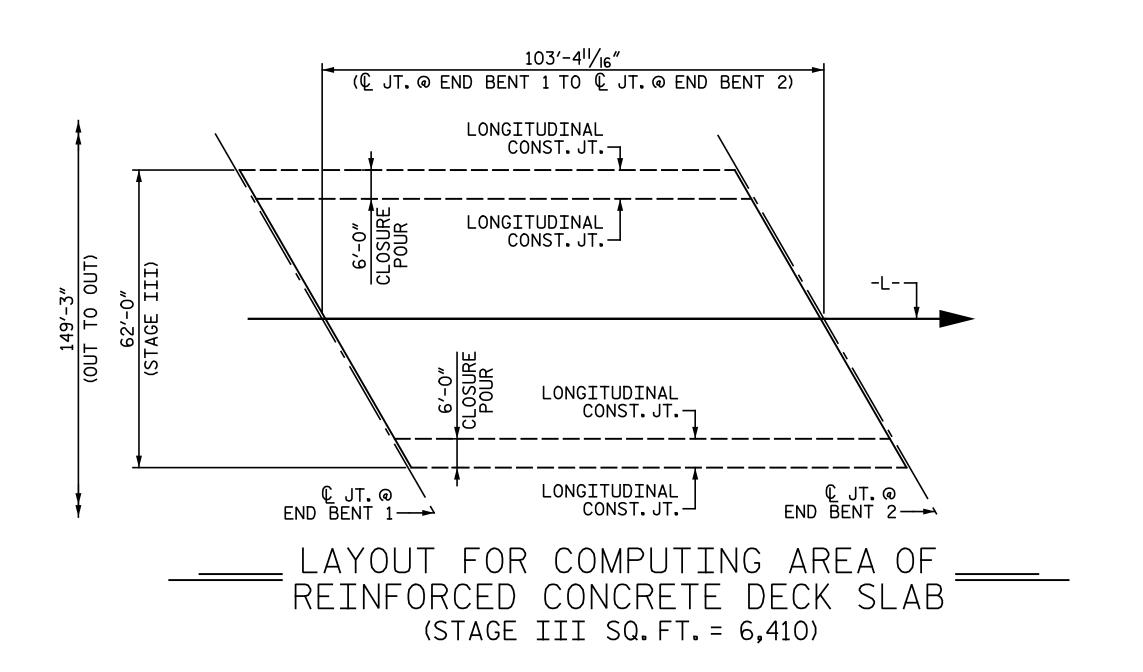
Tow M. Game

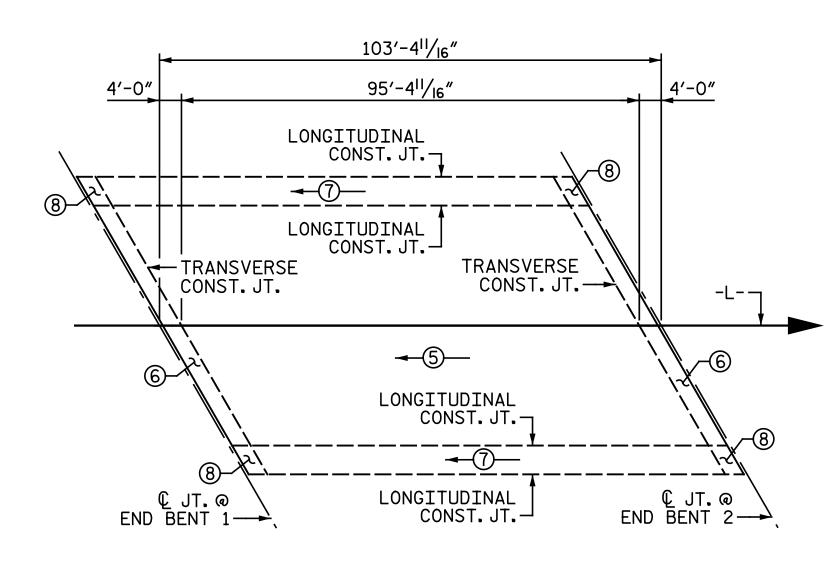
-61EAF7523943466... 4/30/2020

LBS. 12,940

UNLESS ALL SIGNAT	URES COMPLETED							
				REVI	SION	IS		SHEET NO.
Michael Baker	Michael Baker Engineering 8000 Regency Parkway, Suite 600	NO.	BY:	DATE:	NO.	BY:	DATE:	S3-28
INTERNATIONAL	Cary, North Carolina 27518 NC License No. : F-1084	1			3			TOTAL SHEETS
INTERNATIONAL	2.5555 1 1001	2			4			44

DRAWN BY : _____C. E. MAYHEW ____ DATE : 12-10-19 CHECKED BY : T. M. GARRISON DATE : 4-21-20





POURING SEQUENCE → DENOTES POUR NUMBER AND DIRECTION

			RE	EINFOR	CING E	BAR S	CHEC	ULE			
				SPAI	V Д -	STAG	FIT	T			
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A4	117	#5	STR.	49' - 8"	6,061	A511	2	#5	STR.	38' - 5"	80
A5	117	#5	STR.	49' - 8"	6,061	A512	2	#5	STR.	37' - 4"	78
₩ A6	20	#5	STR.	51' - 3"	1,069	A513	2	#5	STR.	36′ - 3″	76
						A514	2	#5	STR.	35′ - 2″	73
* A401	2	#5	STR.	49' - 3"	103	A515	2	#5 "-	STR.	34' - 1"	71
* A402	2	#5	STR.	48' - 2"	100	A516	2	#5 #5	STR.	33' - 0"	69
* A403 * A404	2	#5 #5	STR. STR.	47' - 1" 46' - 0"	98 96	A517 A518	2	#5 #5	STR.	31' - 11" 30' - 10"	67 64
* A404 * A405	2	#5	STR.	44' - 11"	94	A518	2	#5	STR.	29' - 9"	62
* A406	2	#5	STR.	43' - 10"	91	A520	2	#5	STR.	28' - 8"	60
* A407	2	#5	STR.	42' - 9"	89	A521	2	#5	STR.	27' - 7"	58
* A408	2	#5	STR.	41' - 8"	87	A522	2	#5	STR.	26' - 6"	55
* A409	2	#5	STR.	40' - 7"	85	A523	2	#5	STR.	25' - 5"	53
* A410	2	#5	STR.	39' - 6"	82	A524	2	#5	STR.	24' - 4"	51
* A411	2	#5	STR.	38' - 5"	80	A525	2	#5	STR.	23' - 3"	48
* A412	2	#5	STR.	37' - 4"	78	A526	2	#5	STR.	22' - 2"	46
* A413	2	#5	STR.	36' - 3"	76	A527	2	#5 #5	STR.	21' - 1"	44
* A414	2	#5 #5	STR.	35' - 2"	73	A528	2	#5 #5	STR.	20' - 0"	42
* A415 * A416	2	#5	STR. STR.	34' - 1" 33' - 0"	71 69	A529 A530	2	#5 #5	STR.	18' - 11" 17' - 10"	39 37
* A417	2	#5	STR.	31' - 11"	67	A530	2	#5	STR.	16' - 9"	35
* A418	2	#5	STR.	30' - 10"	64	A531	2	#5	STR.	15' - 8"	33
* A419	2	#5	STR.	29' - 9"	62	A533	2	#5	STR.	14' - 7"	30
* A420	2	#5	STR.	28' - 8"	60	A534	2	#5	STR.	13' - 6"	28
* A421	2	#5	STR.	27' - 7"	58	A535	2	#5	STR.	12' - 5"	26
₩ A422	2	#5	STR.	26' - 6"	55	A536	2	#5	STR.	11' - 4"	24
* A423	2	#5	STR.	25′ - 5″	53	A537	2	#5	STR.	10' - 3"	21
* A424	2	#5	STR.	24' - 4"	51	A538	2	#5	STR.	9' - 2"	19
* A425	2	#5 #5	STR.	23' - 3"	48	A539	2	#5 #5	STR.	8' - 1" 7' - 0"	17
* A426 * A427	2	#5 #5	STR. STR.	22' - 2" 21' - 1"	46 44	A540 A541	2	#5 #5	STR.	5' - 11"	15 12
* A428	2	#5	STR.	20' - 0"	42	A542	2	#5	STR.	4' - 10"	10
* A429	2	#5	STR.	18' - 11"	39	A543	2	#5	STR.	3' - 9"	8
* A430	2	#5	STR.	17' - 10"	37	A544	2	#5	STR.	2' - 8"	6
* A431	2	#5	STR.	16' - 9"	35						
₩ A432	2	#5	STR.	15' - 8"	33	₩ B1	132	#4	STR.	35′ - 8″	3 , 145
∗ A433	2	#5	STR.	14' - 7"	30	B2	118	#5	STR.	52′ - 6″	6,461
* A434	2	#5	STR.	13' - 6"	28						
* A435	2	#5	STR.	12' - 5"	26	* D1	318	#5	STR.	7' - 0"	2,322
* A436	2	#5 #5	STR.	11' - 4" 10' - 3"	24	D2	318	#5 #5	STR.	6' - 6"	2,156
* A437 * A438	2 2	#5	STR. STR.	9' - 2"	21 19	₩ D4	40	#5	STR.	12' - 7"	525
* A438 * A439	2	#5	STR.	8' - 1"	17	₩ G2	2	#5	STR.	57' - 4"	120
* A440	2	#5	STR.	7' - 0"	15	7 02			11\s	J 1 T	120
* A441	2	#5	STR.	5' - 11"	12	* J1	140	#4	1	1' - 5"	132
* A442	2	#5	STR.	4' - 10"	10		_	-		_	
₩ A443	2	#5	STR.	3′ - 9″	8	 ★ K3	16	#8	3	25' - 4"	1,082
* A444	2	#5	STR.	2' - 8"	6	 ★ K4	42	#6	STR.	9' - 2"	578
. = .				40.		* K5	28	#4	STR.	9' - 10"	184
A501	2	#5	STR.	49' - 3"	103	* K6	8	#8	2	13' - 10"	295
A502	2	#5 #5	STR.	48' - 2" 47' - 1"	100	 ★ K7	8	#8	STR.	9' - 10"	210
A503 A504	2 2	#5 #5	STR. STR.	46' - 0"	98 96	* S1	84	#4	4	6' - 10"	383
A504	2	#5	STR.	44' - 11"	96	* S2	84	#4 #5	5	5' - 10"	585_ 511
A505	2	#5	STR.	43' - 10"	91	* 32 * \$3	56	#5	6	2' - 11"	170
A500	2	#5	STR.	42' - 9"	89	7, 55	30			_ 11	110
A508	2	#5	STR.	41' - 8"	87	REI	NFORCI	NG STE	EL	LBS.	17,060
A509	2	#5	STR.	40' - 7"	85			TED RE			
A510	2	#5	STR.	39' - 6"	82		_	_	_		

	BAR TYPES
WEIGHT 80 78 76 73 71 69 67 64 62 60	1 3'-9" 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
58 55 53 51 48 46 44 42 39 37 35	3
33 30 28 26 24 21 19 17 15 12 10 8 6	2'-1"
3,145 6,461	ALL BAR DIMENSIONS ARE OUT TO OUT
2,322 2,156 525	CLASS AA CONCRETE REINFORCING STEEL CONCRETE CONCRETE STEEL CONCRETE CONC

** QUANTITIES FOR BARRIER RAILS & MEDIAN BARRIER ARE NOT INCLUDED

(CU. YDS.)

-

158.7

30.0

43.3

10.3

242.3

WGINEER S DocuSigned by: 4/30/2020

STAGE III

POUR 5

POUR 6

CLOSURE POUR 7 CLOSURE POUR 8

TOTALS**

PROJECT NO. I-5986B JOHNSTON ___ COUNTY STATION: 1391+19.65 -L-

(LBS.)

17,060

17,060

(LBS.)

19,169

19,169

_

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUPERSTRUCTURE

BILL OF MATERIAL STAGE III

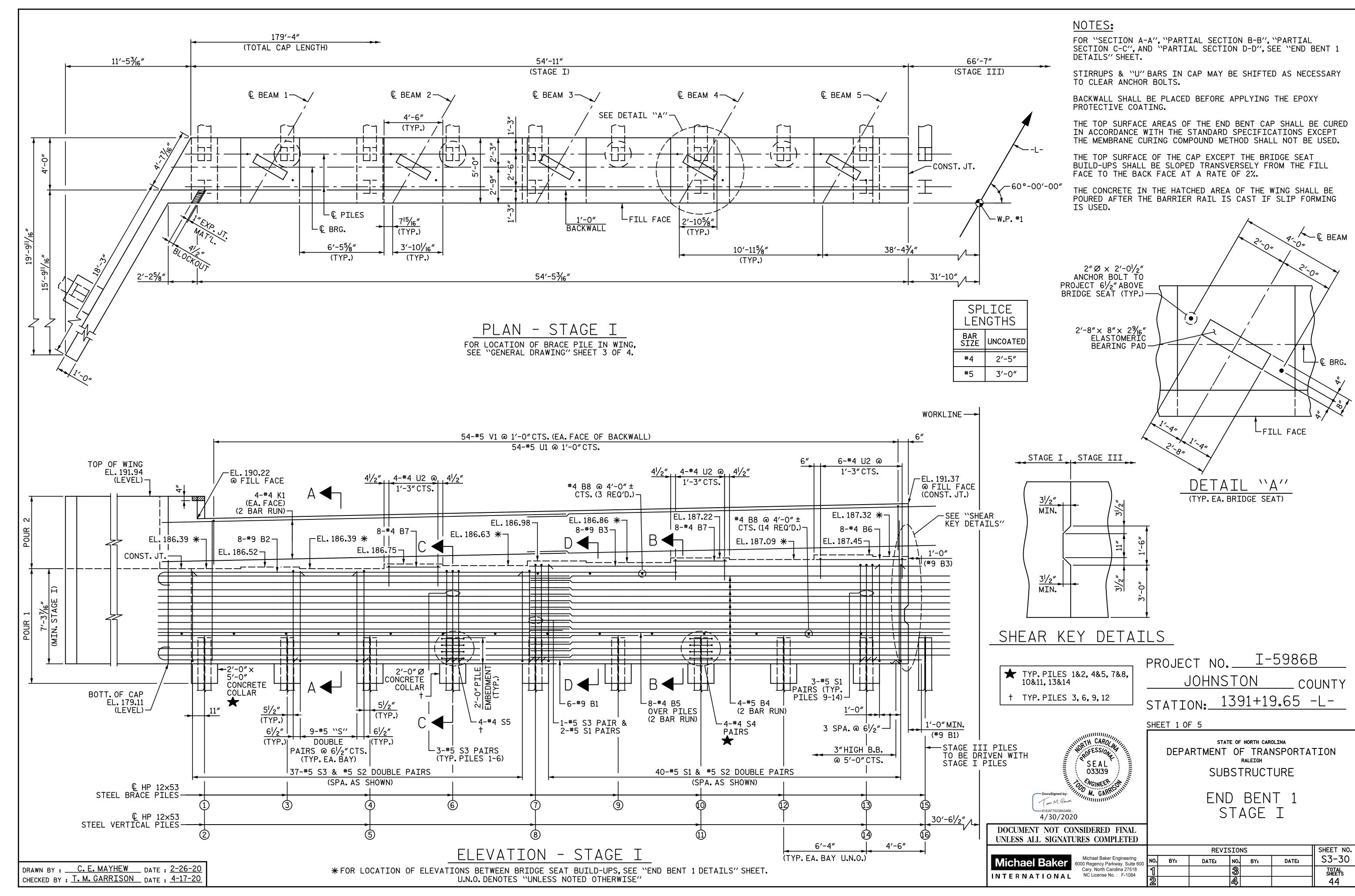
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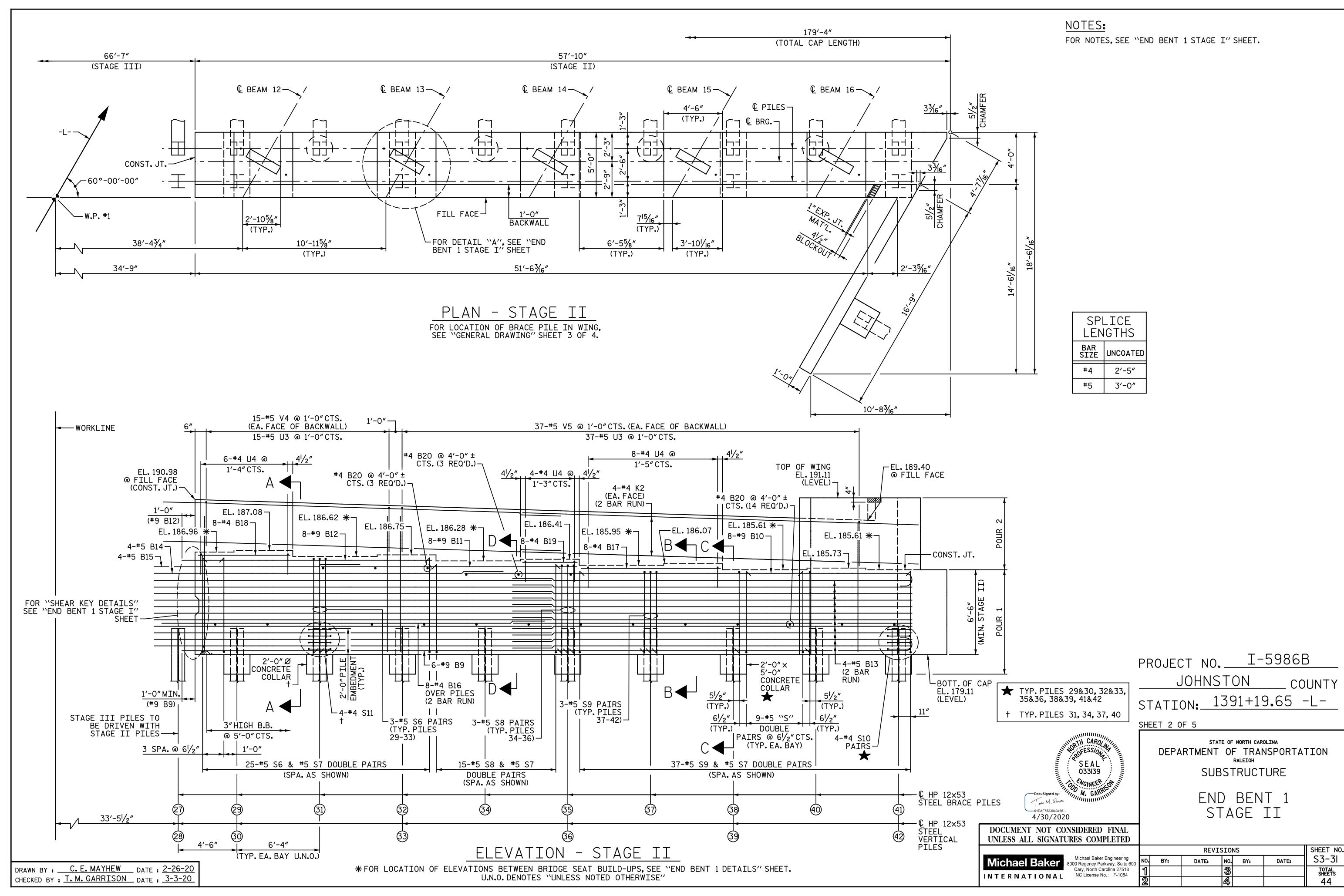
UNLESS ALL SIGNATI	URES COMPLETE
	Michael Baker Engineeri
Michael Baker	8000 Regency Parkway, Sui Cary, North Carolina 275
INTERNATIONAL	NC License No.: F-108

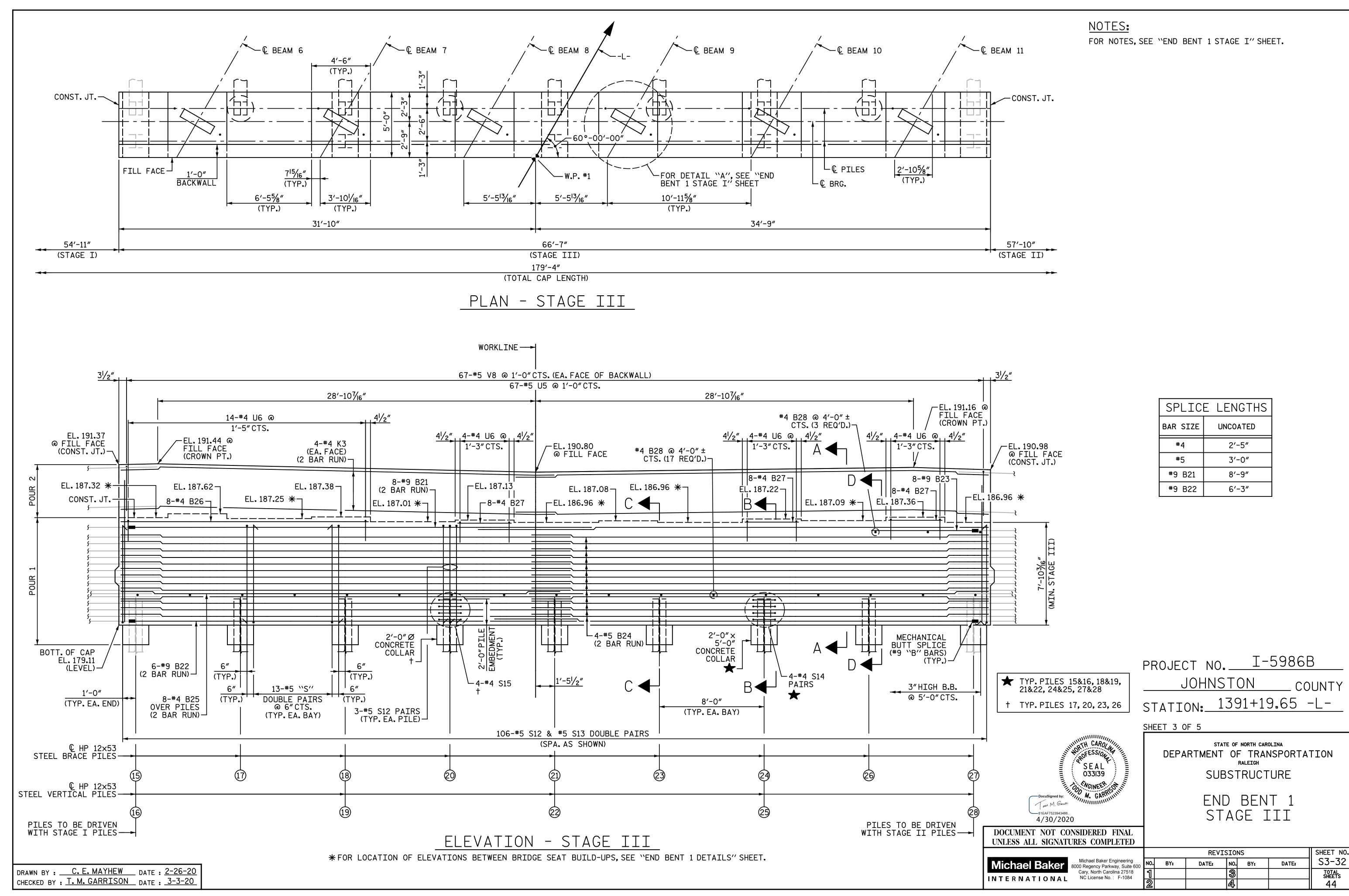
						_	_
			REVI	SIO	NS		SHEET NO.
0	NO.	BY:	DATE:	NO.	BY:	DATE:	S3-29
	1			3			TOTAL SHEETS
	2			4			44

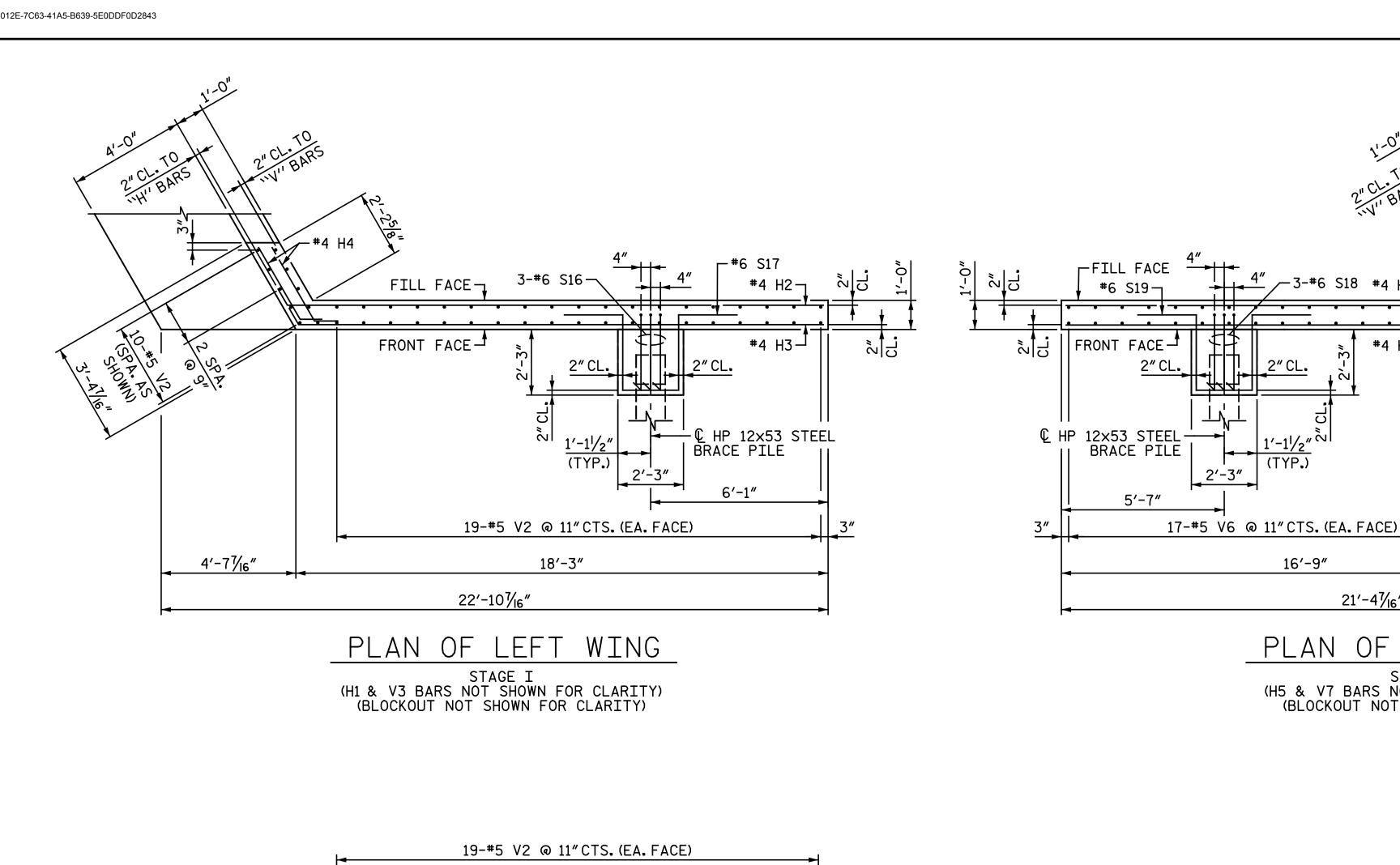
GROOVING BRIDGE FLOORS STAGE III APPROACH SLABS 2,694 SQ.FT. 5,812 SQ.FT. BRIDGE DECK 8,506 SQ.FT. TOTAL

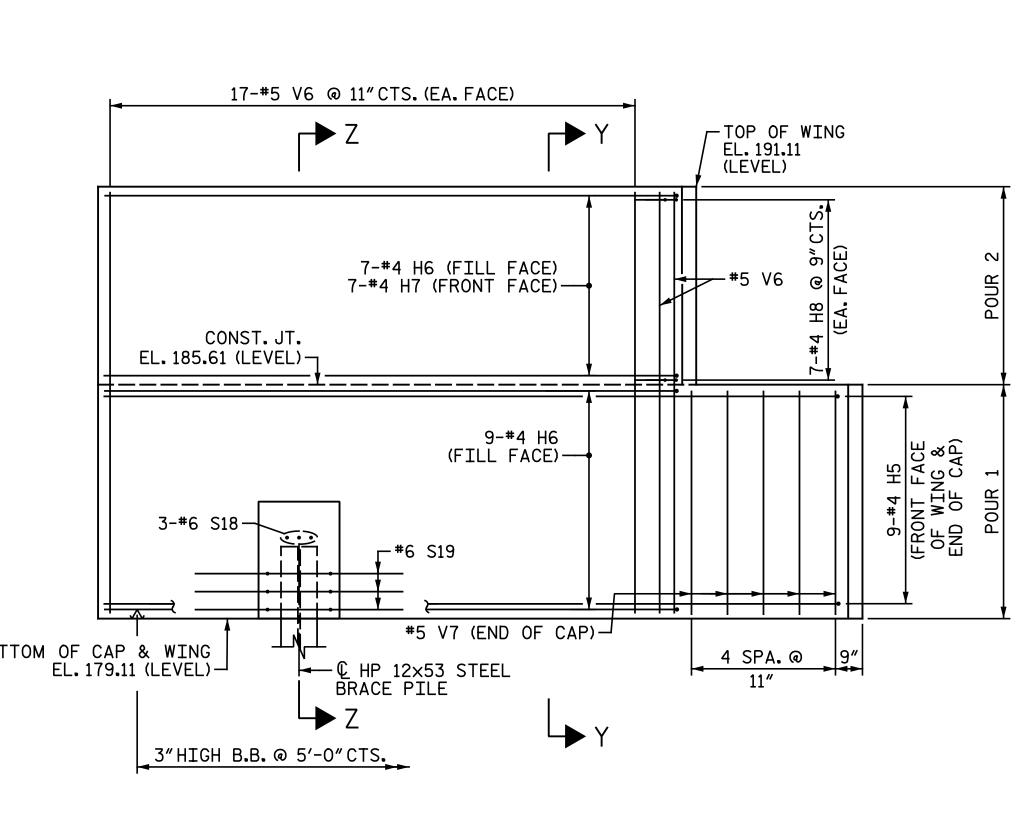
DRAWN BY : ____C. E. MAYHEW ___ DATE : 12-10-19 CHECKED BY : T. M. GARRISON DATE : 12-15-19











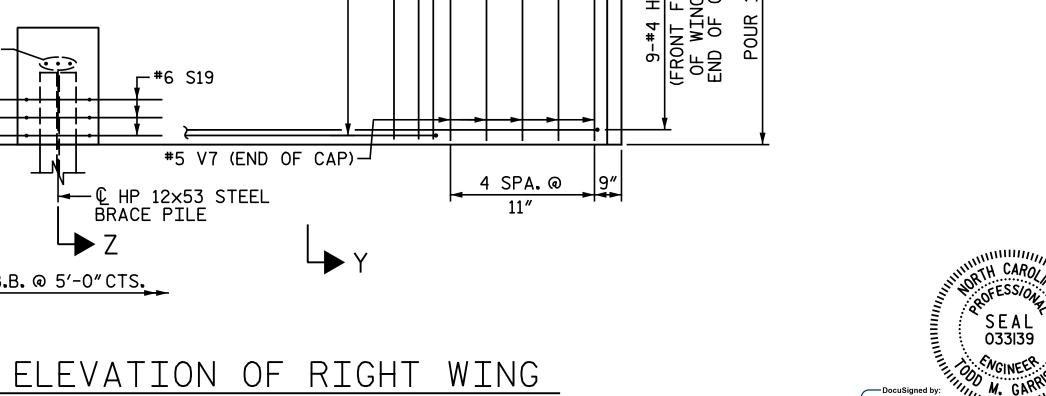
STAGE II

BOTTOM OF CAP & WING EL. 179.11 (LEVEL)

16'-9"

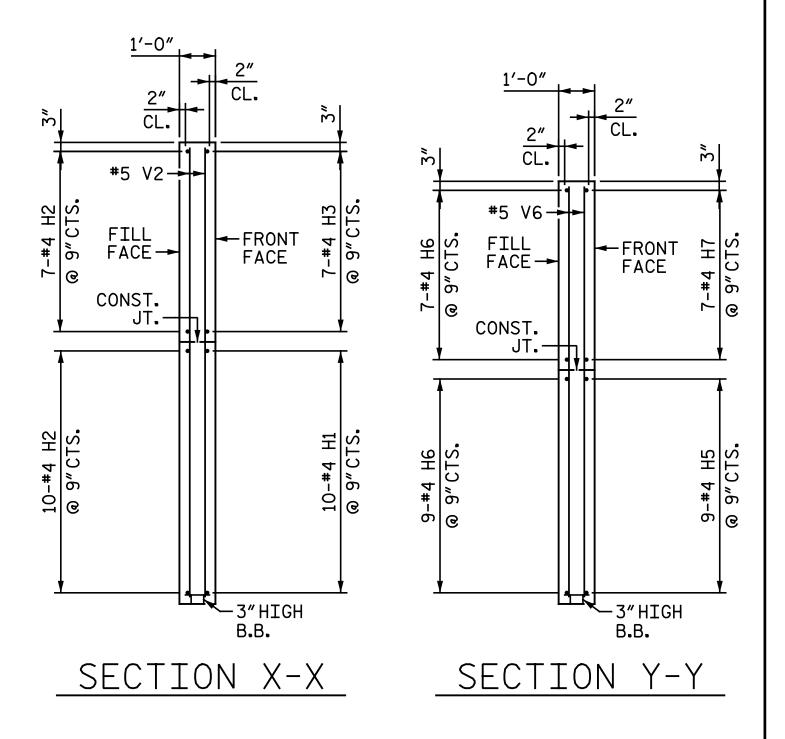
21'-47/16"

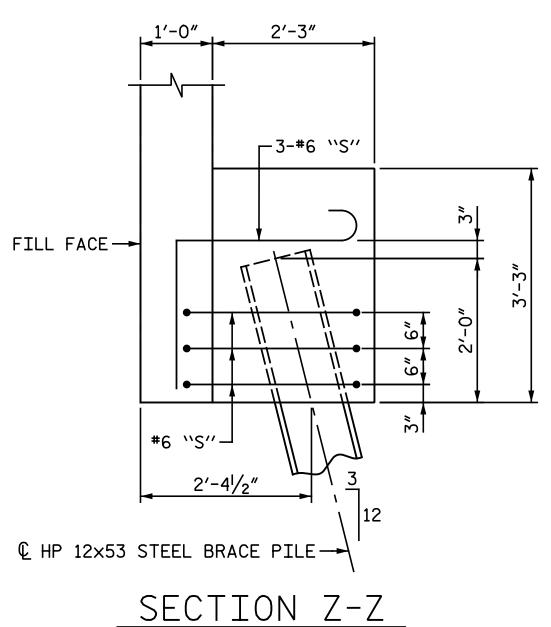
STAGE II (H5 & V7 BARS NOT SHOWN FOR CLARITY) (BLOCKOUT NOT SHOWN FOR CLARITY)



4'-7⁷/₁₆"

WING





PROJECT NO. I-5986B JOHNSTON _ COUNTY 1391+19.65 -L-

SHEET 4 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUBSTRUCTURE

> END BENT 1 WING DETAILS

> > SHEET NO.

S3-33

TOTAL SHEETS

44

DATE:

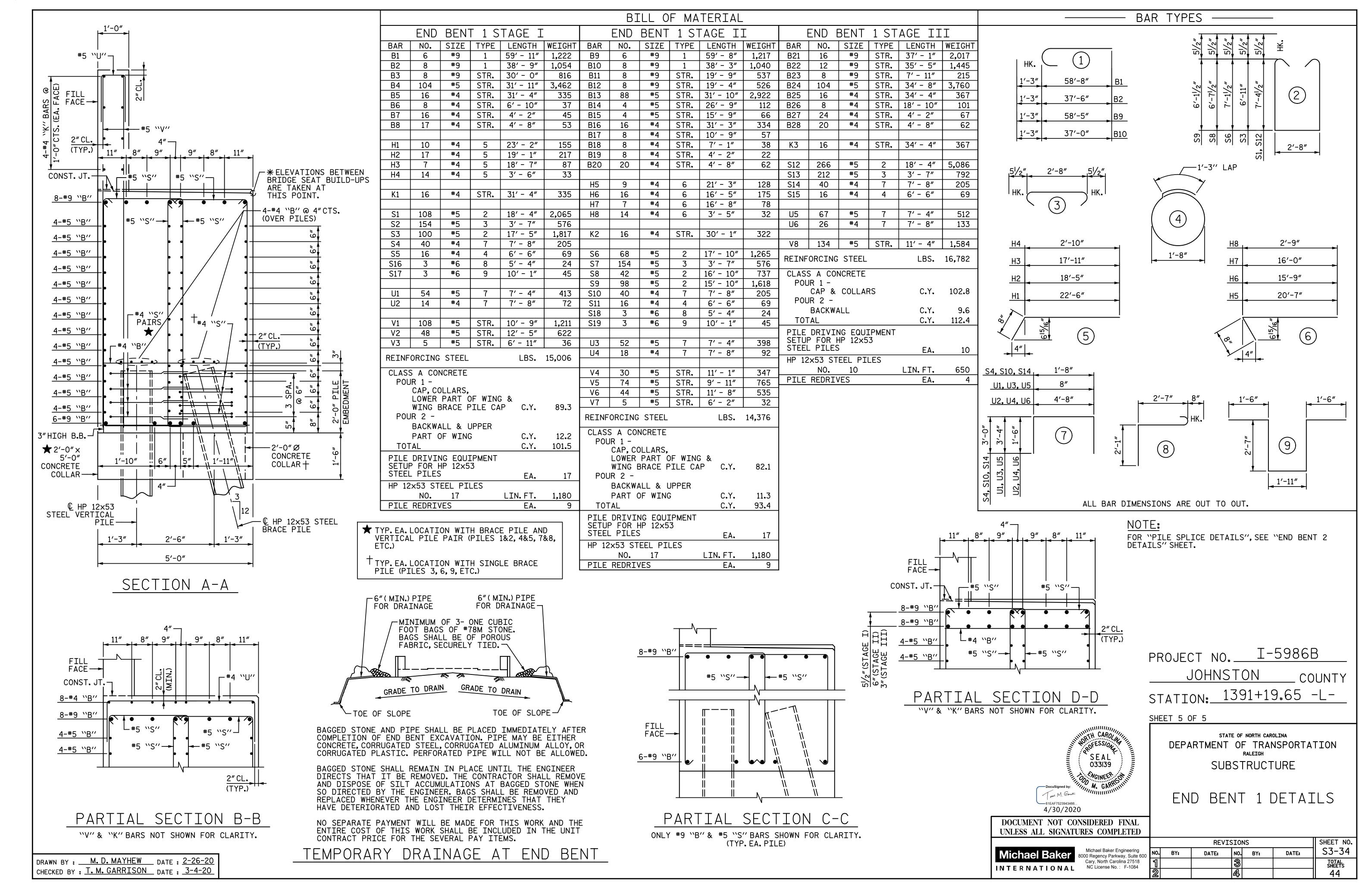
4/30/2020 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

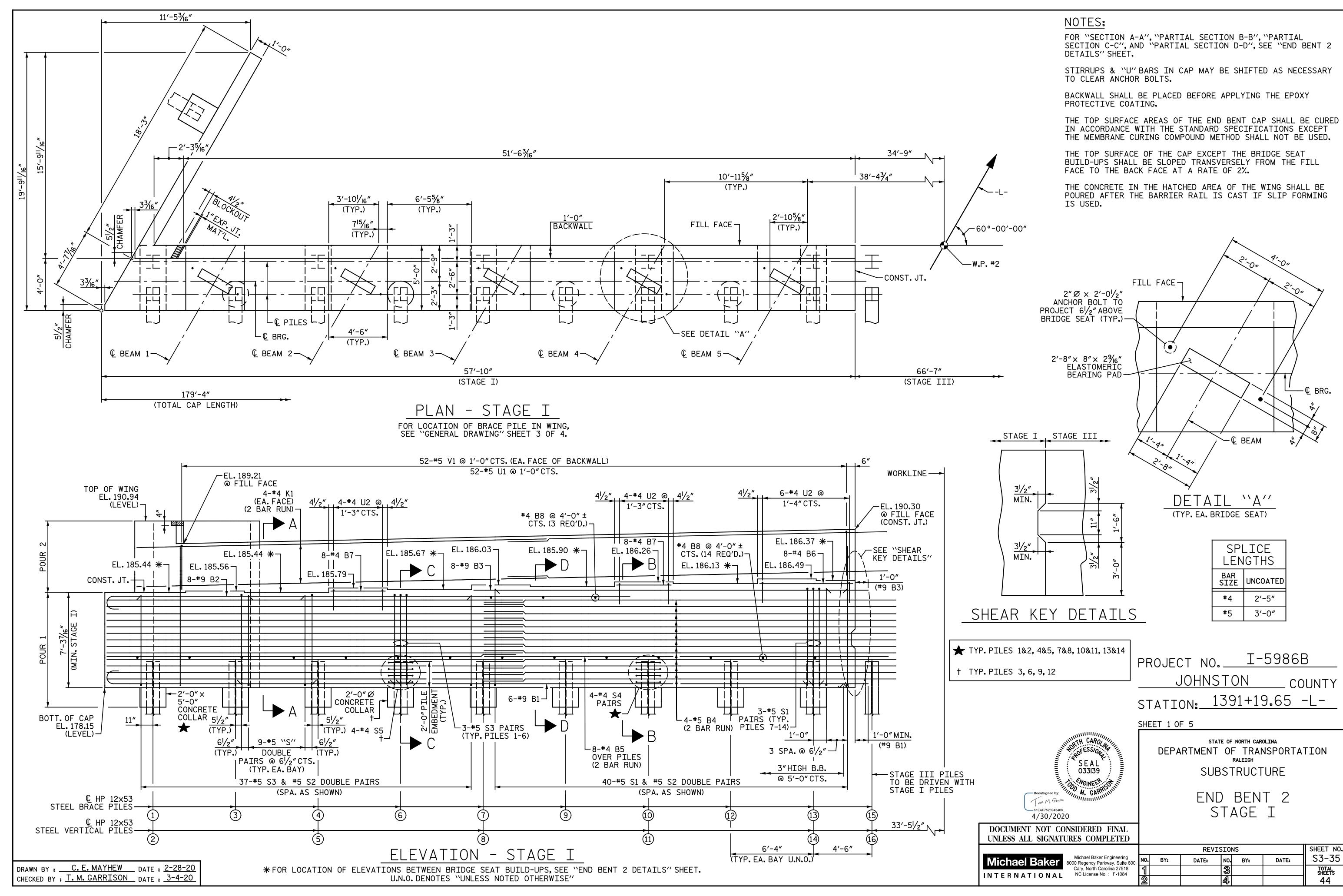
REVISIONS Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084 NO. BY: DATE: BY:

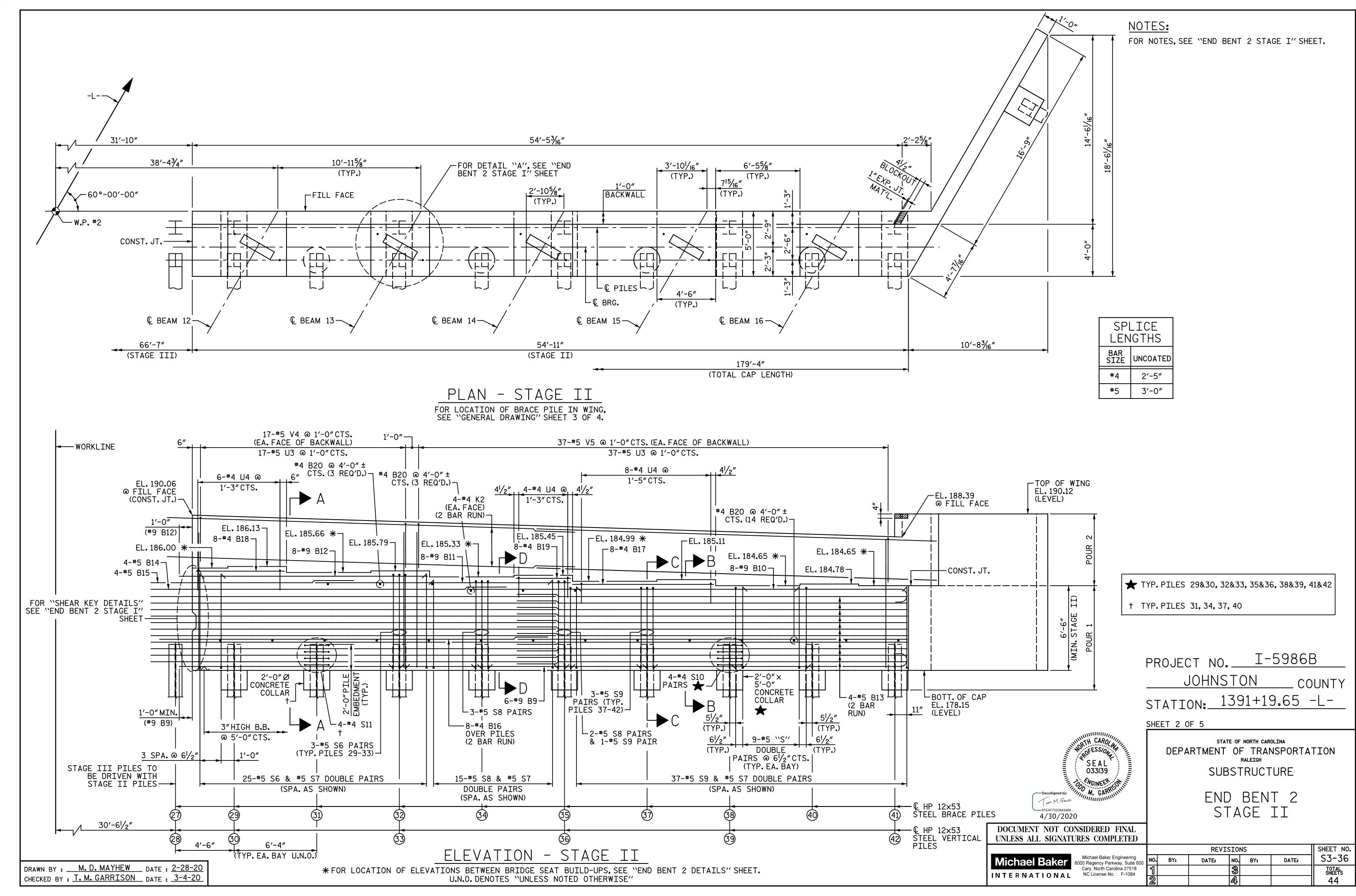
					IJ "J VZ W II CIJ, (LA, I ACL	<u>-/</u>
_			F WIN 191.9 (LEVE	94	├ X	Z
POUR 2		7-#4 H4 @ 9" CTS. (EA. FACE)	#5 V	2-	7-#4 H2 (FILI 7-#4 H3 (FRON CONST. JT. EL.186.39 (LEVEL)	- FACE) Γ FACE)
POUR 1	10-#4 H1 (FRONT FACE OF WING & END OF CAP)				#6 S17—	10-#4 H2 ILL FACE)————————————————————————————————————
	2"		SPA. @		*#5 V3 (END OF CAP) © HP 12×53 STEEL BRACE PILE	BOTTOM OF CAP & WING EL. 179.11 (LEVEL)
						Z . @ 5'-0"CTS.

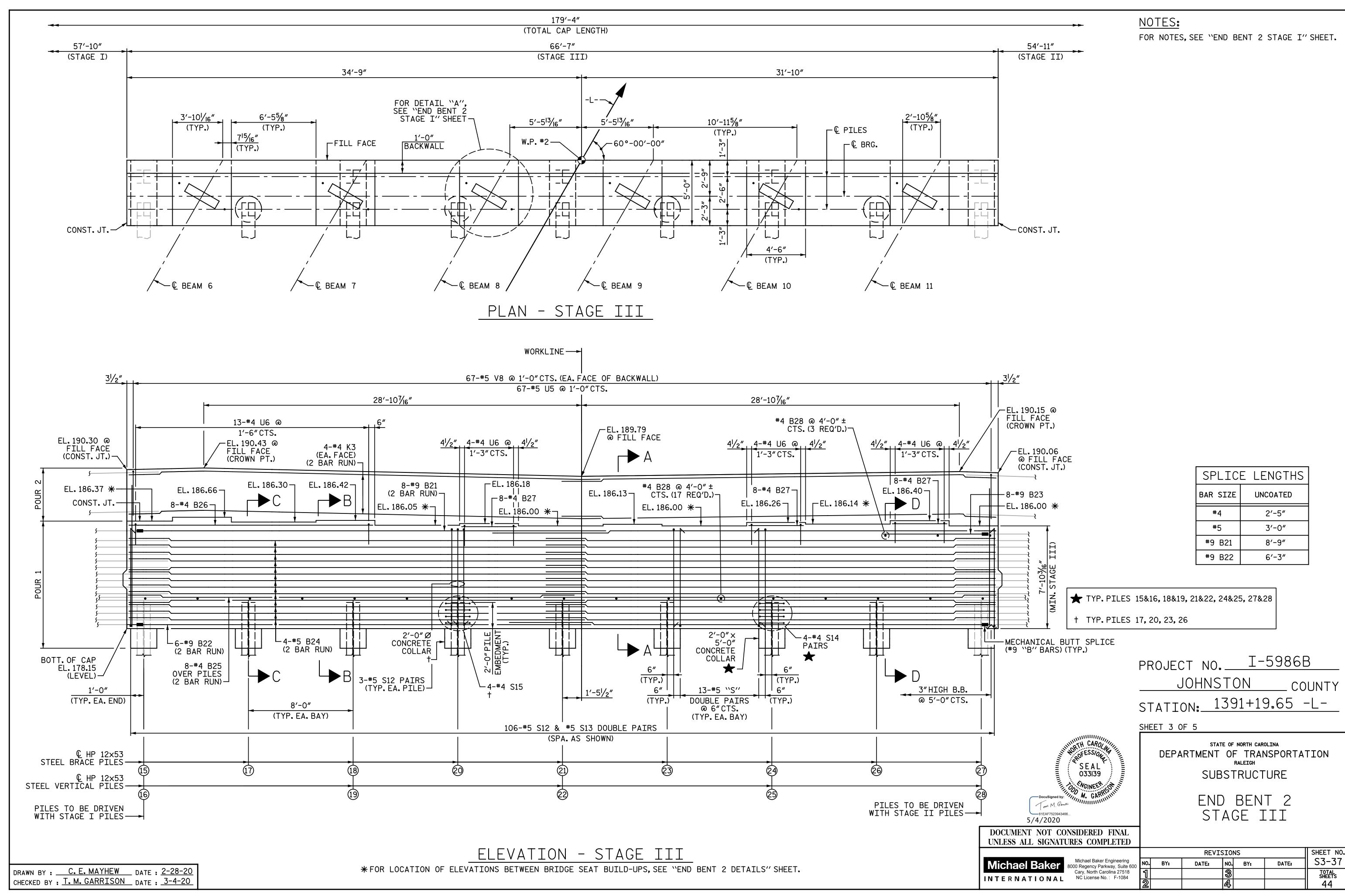
ELEVATION OF LEFT WING

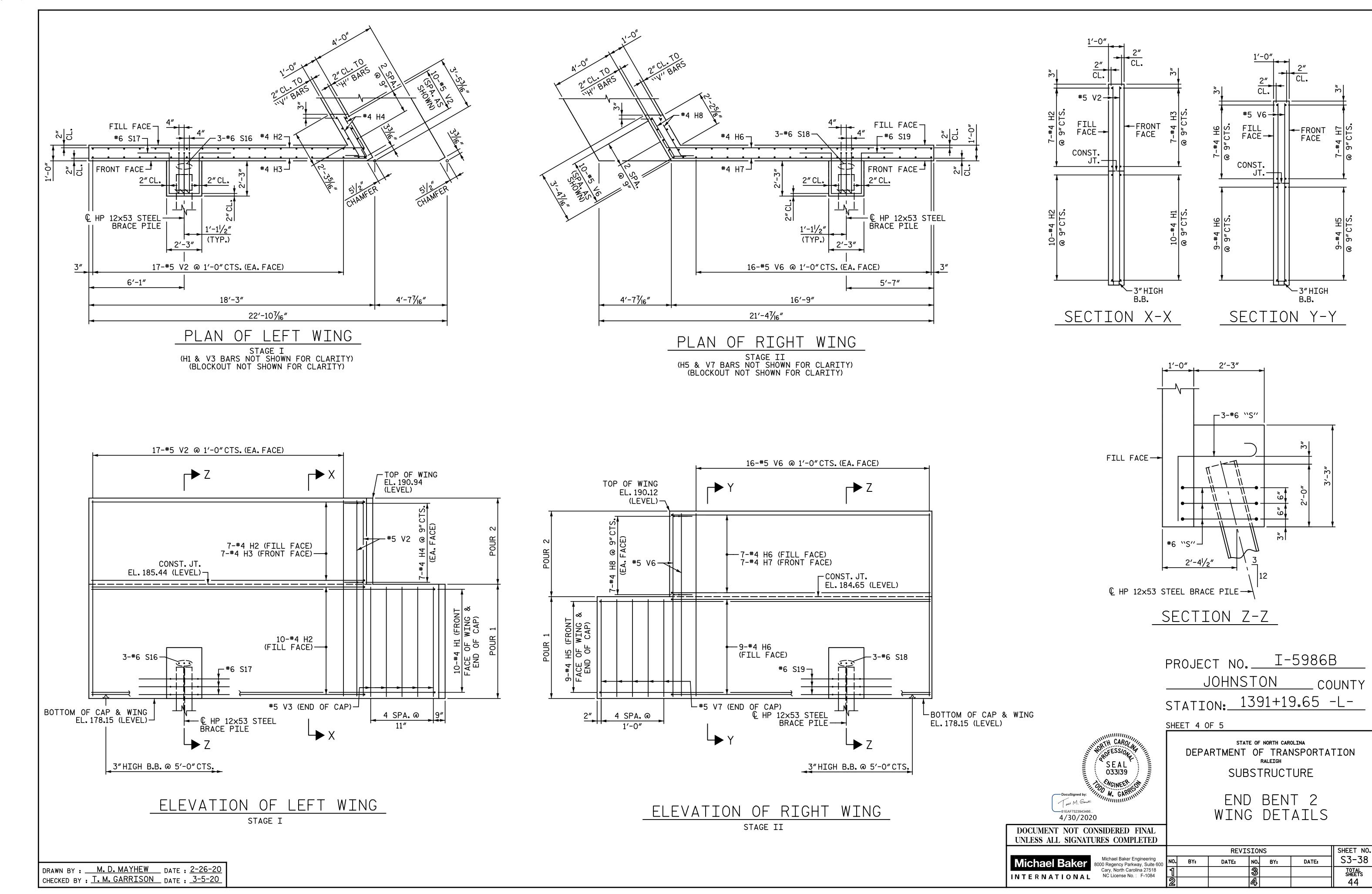
DRAWN BY : _____M. D. MAYHEW ____ DATE : 2-26-20 CHECKED BY : T. M. GARRISON DATE : 3-3-20

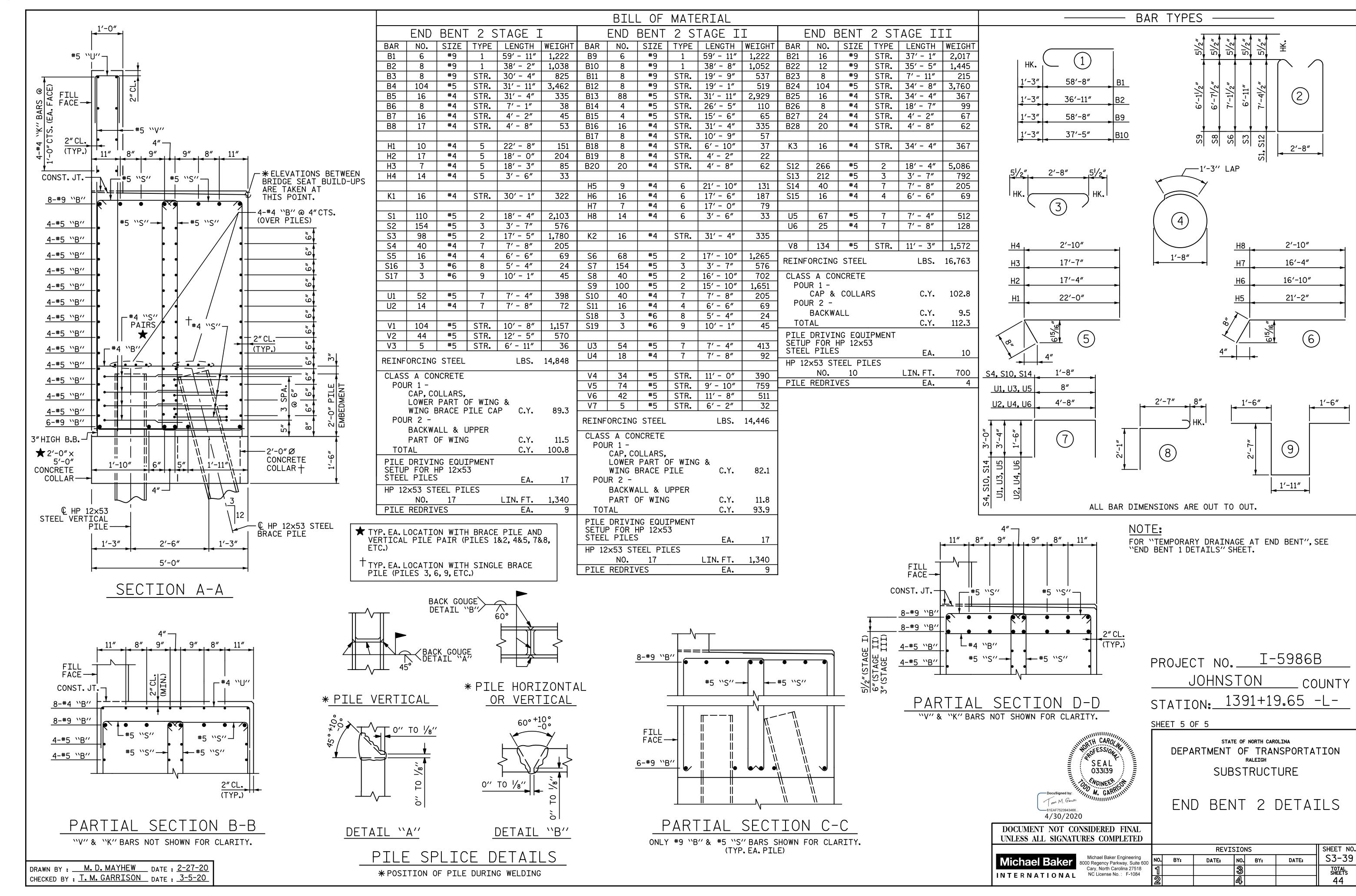


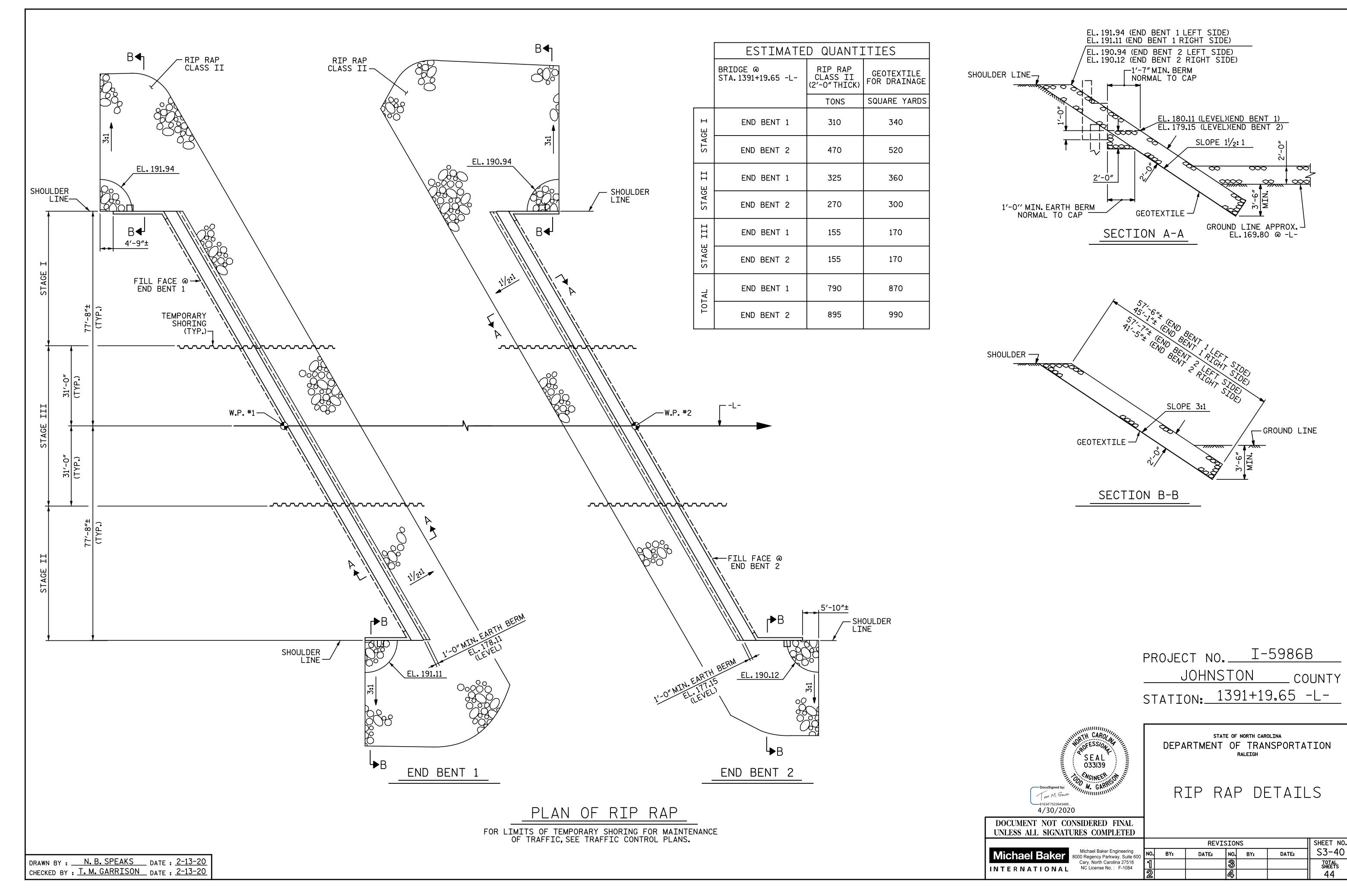


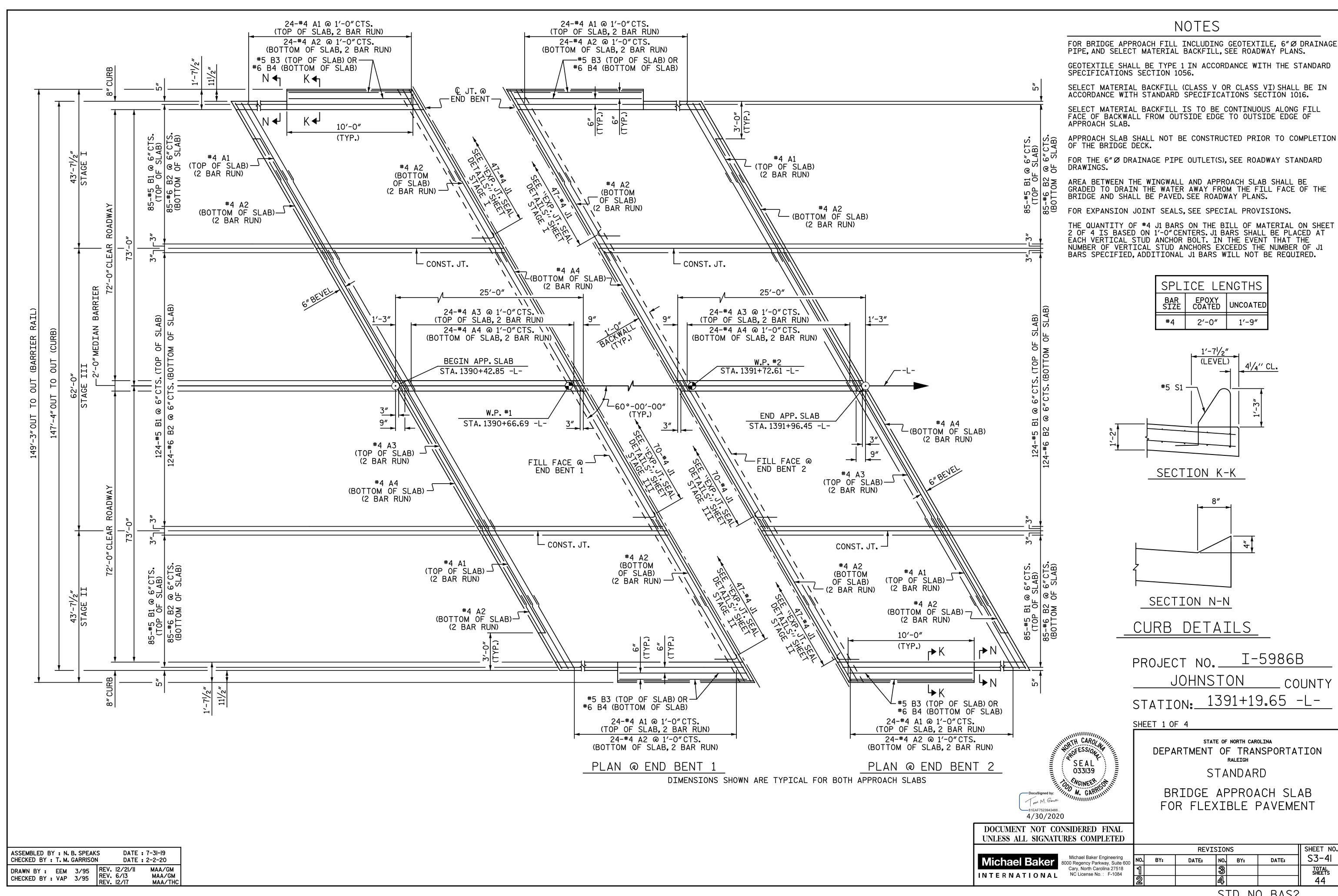


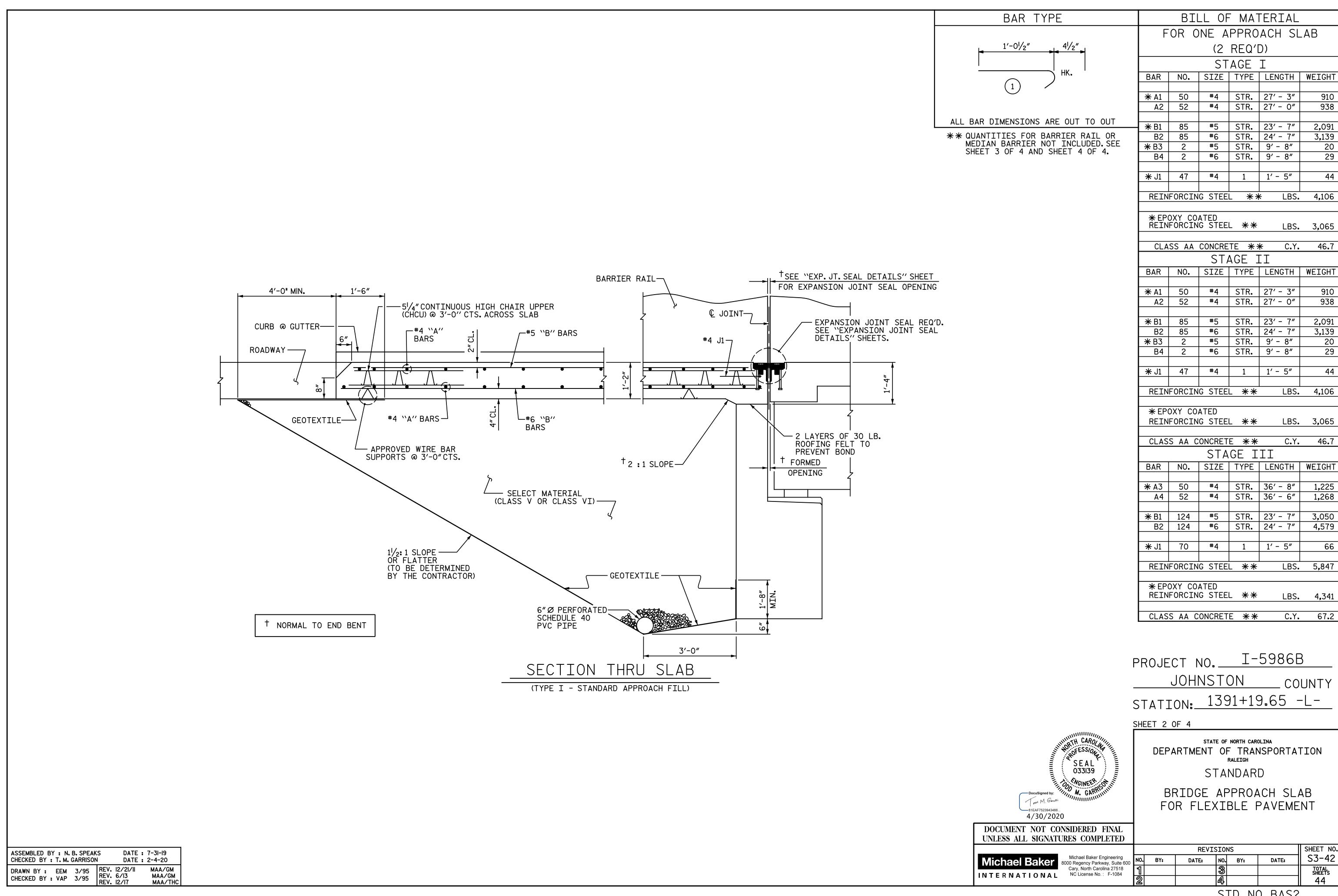


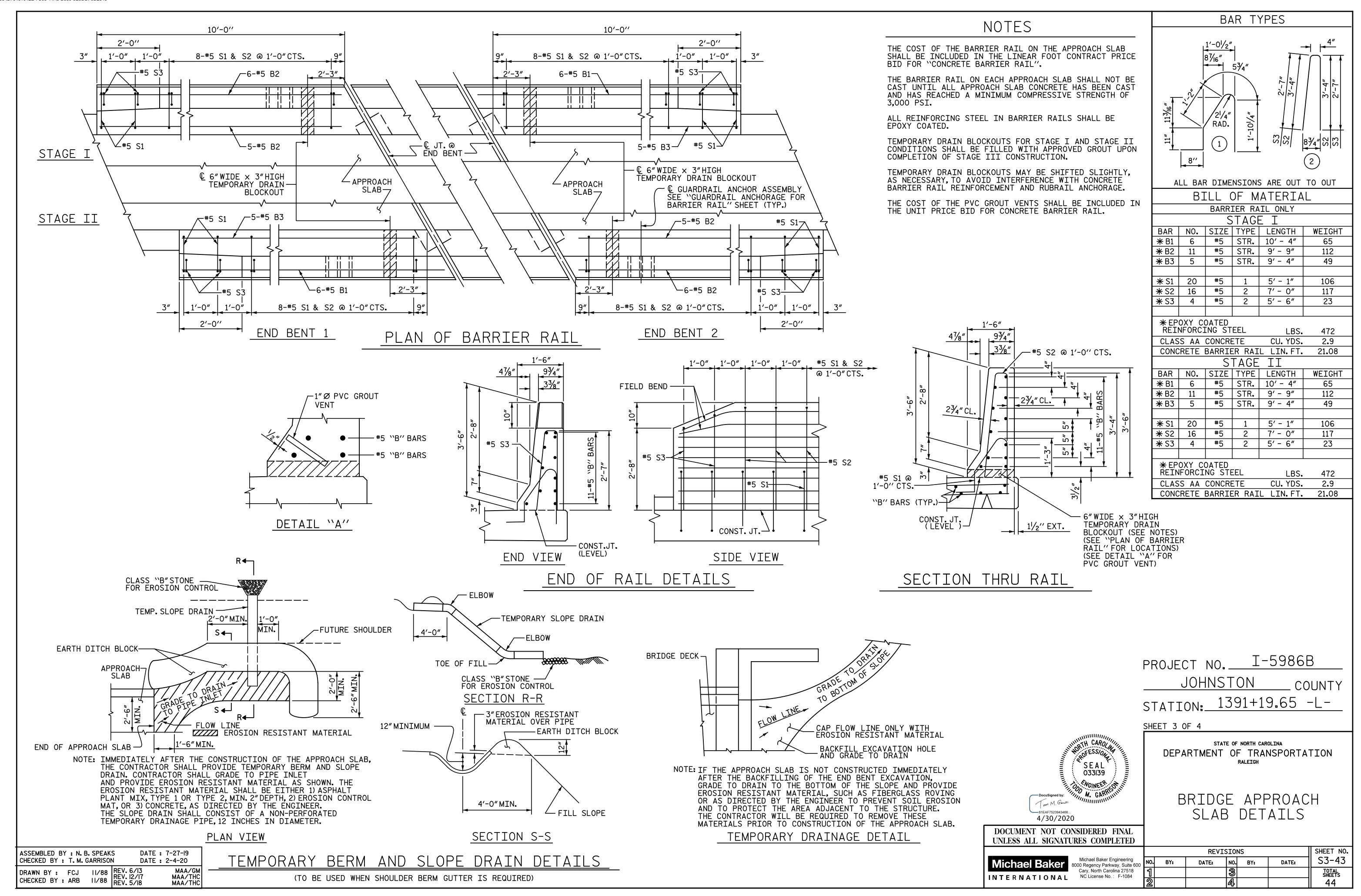


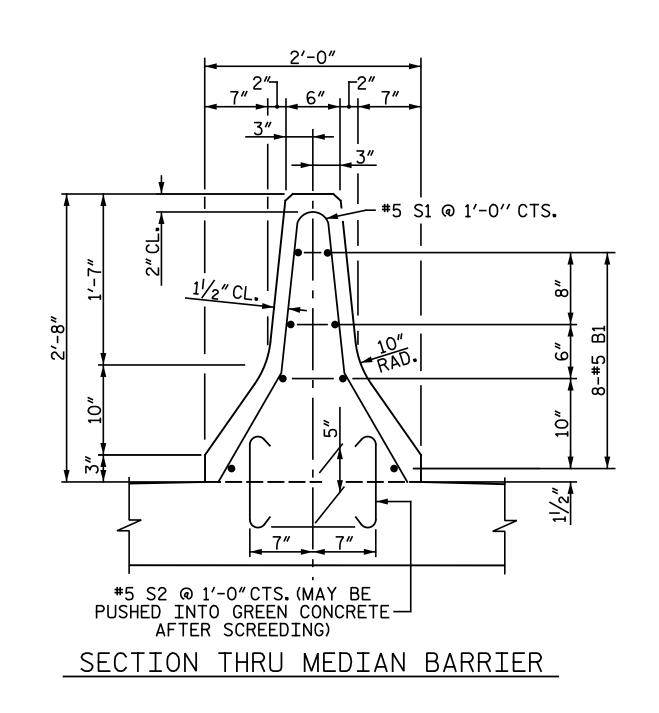


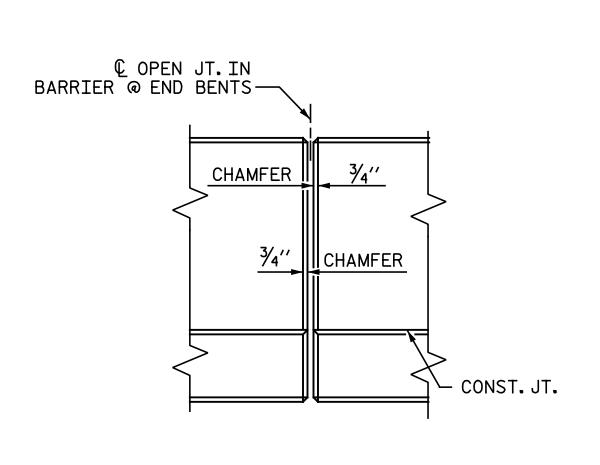












ELEVATION AT EXPANSION JOINTS

NOTES

THE COST OF THE MEDIAN BARRIER ON THE APPROACH SLAB SHALL BE INCLUDED IN THE LINEAR FOOT CONTRACT PRICE BID FOR "CONCRETE MEDIAN BARRIER".

THE MEDIAN BARRIER ON EACH APPROACH SLAB SHALL NOT BE CAST UNTIL ALL APPROACH SLAB CONCRETE HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN MEDIAN BARRIER SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ "IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE CONCRETE MEDIAN BARRIER AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN ENDS OF MEDIAN BARRIER SEGMENTS ON THE APPROACH SLABS.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED DOWELS IN PLACE OF THE #5 S2 BARS DETAILED. LEVEL 2 FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE DOWELS IS 19 KIPS, THE DOWELS ARE TO BE HOOKED ON ONE END WITH A PROJECTION MATCHING THAT OF THE #5 S2 BARS. THE OPPOSITE END IS TO BE STRAIGHT AND EMBEDDED 6"INTO THE APPROACH SLAB.

BAR SIZE

#5

SPLICE LENGTHS

EPOXY COATED

3′-5″

•	3 ¹ / ₂ " 1'-9"	2"	HK. 2	51/2" HK.
ALL B			is are out 1 ATERIAL	0 001
			RIER ONLY	
		AGE		
DAD NO				WETOUT
BAR NO.	SIZE	TYPE	LENGTH	WEIGHT
* B1 32	#5	STR.	13′ - 10″	462
	ш-	1	5' - 6"	275
* S1 48	# 5	+		
* S1 48 * S2 96	#5 #5	2	1' - 10"	184

CONCRETE MEDIAN BARRIER LIN. FT. 50.02

LBS.

CU. YDS.

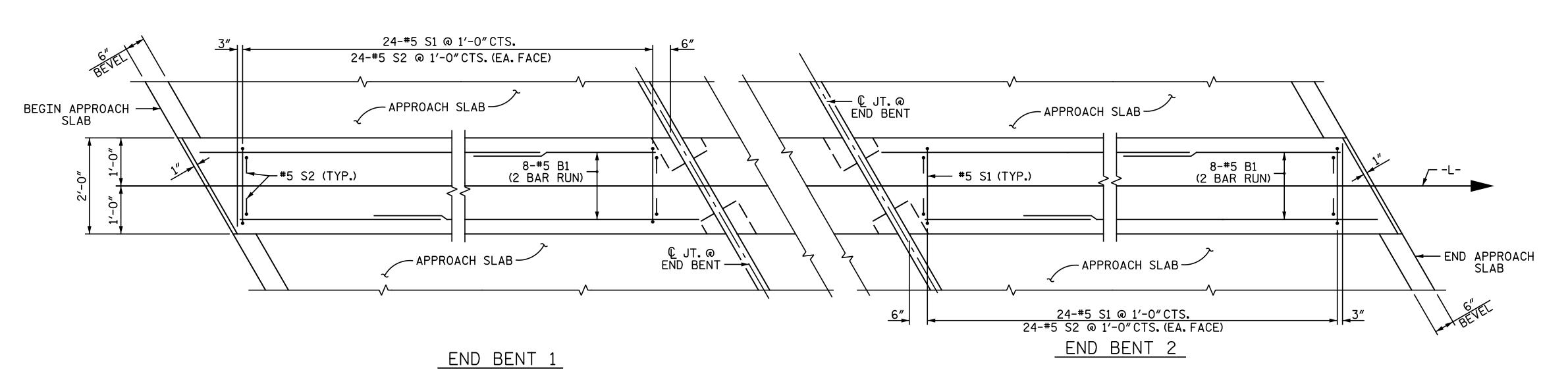
5.1

REINFORCING STEEL

CLASS AA CONCRETE

BAR TYPES

MEDIAN BARRIER DETAILS



PLAN OF CONCRETE MEDIAN BARRIER

DISCONTINUE BEVEL UNDER MEDIAN BARRIER



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084

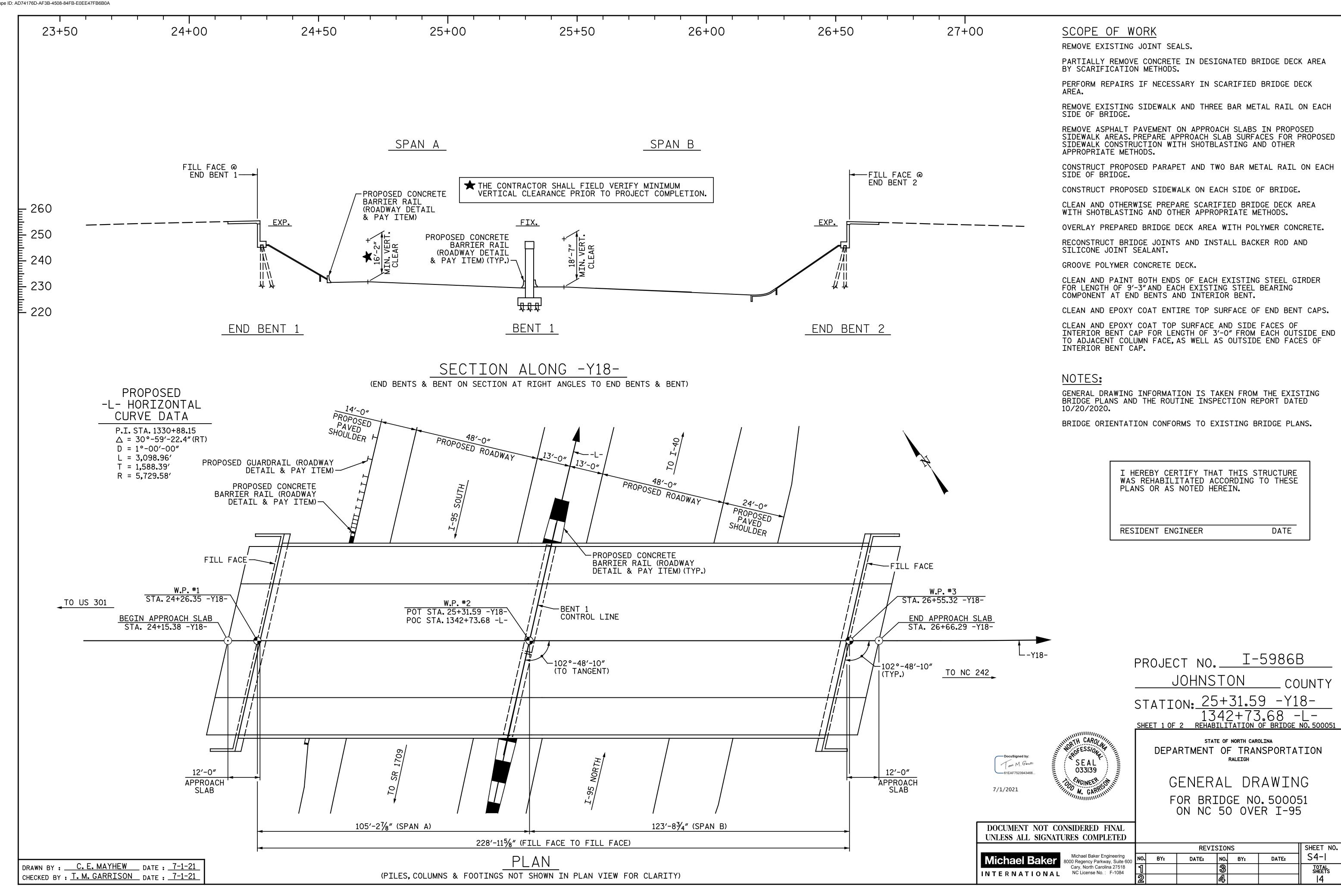
PROJECT NO. I-5986B JOHNSTON _ COUNTY 1391+19.65 -L-SHEET 4 OF 4

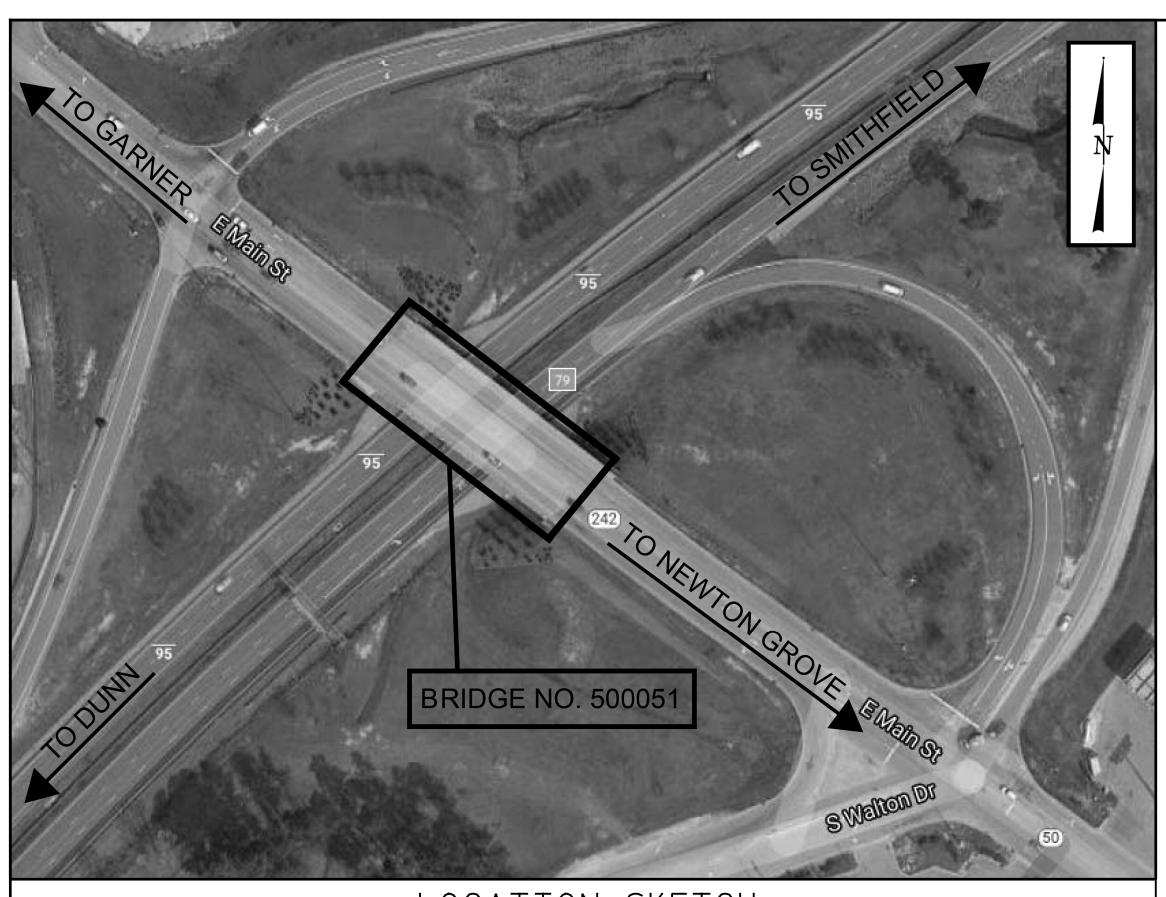
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

BRIDGE APPROACH SLAB DETAILS

REVISIONS SHEET NO. NO. BY: S3-44 DATE: DATE: TOTAL SHEETS 44

DRAWN BY : N. B. SPEAKS DATE : 8-5-19 CHECKED BY : T. M. GARRISON DATE : 3-11-20





LOCATION SKETCH

INFORMATION INDICATED ON THE LOCATION SKETCH SHALL BE CONSIDERED GENERAL INFORMATION ONLY. THE CONTRACTOR SHALL CONFIRM, THROUGH OTHER SOURCES, SPECIFIC INFORMATION REGARDING BRIDGES, ROADWAYS, UTILITIES, THE SURROUNDING AREA, AND ANY OTHER ASPECTS THAT MAY BE NECÉSSARY TO PERFORM AND COMPLETE THE PROJECT.

BRIDGE COORDINATES

LATITUDE: 35.3768° LONGITUDE: -78.5402° NOTES:

EXISTING DIMENSIONS AND BRIDGE CONDITION ARE FROM BEST INFORMATION AVAILABLE. THE CONTRACTOR SHALL FIELD VERIFY THE INFORMATION SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF ACTUAL DIMENSIONS AND CONDITIONS DIFFER.

THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THAT SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL STATE AND FEDERAL SAFETY REQUIREMENTS.

WORK ON THE BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL BELOW, THE CONTRACTOR SHALL SUBMIT PLANS FOR CONSTRUCTION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

PRIOR TO BEGINNING WORK, THE CONTRACTOR SHALL SUBMIT FOR REVIEW AND APPROVAL A COMPLETE SEQUENCE OF TASKS FOR EACH OPERATION AFFECTING THE BRIDGE SURFACE AND/OR TRAFFIC.

EXISTING JOINTS SHALL BE SEALED PRIOR TO BEGINNING SURFACE PREPARATION OF THE BRIDGE DECK.

THE CONTRACTOR SHALL PROVIDE A METHOD OF HANDLING UNEXPECTED BLOW THROUGH OF THE BRIDGE DECK.

LONGITUDINAL CONSTRUCTION JOINTS OF OVERLAYS SHALL BE LOCATED ALONG THE CENTERLINE OR EDGE OF TRAVEL LANES.

EXISTING BRIDGE CONCRETE DECK SHALL BE REPAIRED AFTER SCARIFICATION AND PRIOR TO THE SURFACE PREPARATION AND APPLICATION OF THE POLYMER CONCRETE OVERLAY AT LOCATIONS DETERMINED BY THE ENGINEER, IF NECESSARY, SUCH LOCATIONS SHALL BE REPAIRED AS OUTLINED IN THE SPECIAL PROVISIONS FOR OVERLAY SURFACE PREPARATION FOR POLYMER CONCRETE AND POLYMER CONCRETE BRIDGE DECK OVERLAY.

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE, IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.

EPOXY COATING ENTIRE TOP SURFACE OF END BENT CAPS, TOP SURFACES AND SIDE FACES OF INTERIOR BENT CAP FROM EACH OUTSIDE END TO ADJACENT COLUMN FACE, AND OUTSIDE END FACES OF INTERIOR BENT CAP WILL BE REQUIRED. AT THE TIME OF PREPARATION OF THESE PLANS, IT WAS NOT ANTICIPATED THAT EPOXY COATING WOULD BE REQUIRED ELSEWHERE. HOWEVER, IT MAY BE DETERMINED IN THE FIELD THAT EPOXY COATING ELSEWHERE WILL BE NECESSARY TO PROPERLY COMPLETE THE INTENDED BRIDGE PRESERVATION / REHABILITATION WORK. THE CONTRACTOR SHALL BE PREPARED TO PERFORM SUCH WORK IN A TIMELY MANNER. AS DETERMINED IN THE FIELD.

AT THE TIME OF PREPARATION OF THESE PLANS, IT WAS NOT ANTICIPATED THAT THE FOLLOWING WORK / PAY ITEMS WOULD BE REQUIRED:

- CLASS II SURFACE PREPARATION - CLASS III SURFACE PREPARATION

WORK IS ENCOUNTERED.

- CONCRETE DECK REPAIR FOR PC OVERLAY HOWEVER. IT MAY BE DETERMINED IN THE FIELD THAT THIS WORK OR OTHER WORK WILL BE NECESSARY TO PROPERLY COMPLETE THE INTENDED BRIDGE PRESERVATION / REHABILITATION WORK, THE CONTRACTOR SHALL BE PREPARED TO PERFORM SUCH WORK IN A TIMELY MANNER, AS DETERMINED IN THE FIELD, SUCH WORK SHALL BE CONSIDERED EXTRA WORK AND SHALL BE ADDRESSED AS PER ARTICLE 104-7 OF THE STANDARD SPECIFICATIONS. PROJECT SPECIAL PROVISIONS THAT OUTLINE REQUIREMENTS FOR THESE POTENTIAL ADDITIONAL WORK ITEMS HAVE BEEN PROVIDED IN PROJECT DOCUMENTS. BUT NO QUANTITIES HAVE BEEN LISTED, ACTUAL PAY ITEMS. QUANTITIES, AND COSTS WILL BE ESTABLISHED, AS REQUIRED, IF EXTRA

FOR SCOPE OF WORK, SEE SPECIAL PROVISIONS.

FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURES, SEE SPECIAL PROVISIONS,

FOR PAINTING EXISTING WEATHERING STEEL STRUCTURE, SEE SPECIAL PROVISIONS.

FOR POURABLE SILICONE JOINT SEALANT, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS. SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY. SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS,

FOR CONTROL OF TRAFFIC AND LIMITS ON PHASING OF CONSTRUCTION, SEE TRANSPORTATION MANAGEMENT PLAN.

FOR POLYMER CONCRETE BRIDGE DECK OVERLAY, SEE SPECIAL PROVISIONS.

FOR OVERLAY SURFACE PREPARATION FOR POLYMER CONCRETE, SEE SPECIAL PROVISIONS.

FOR EPOXY COATING AND DEBRIS REMOVAL, SEE SPECIAL PROVISIONS.

PAY ITEMS FOR CLASS AA CONCRETE AND EPOXY COATED REINFORCING STEEL INCLUDE PAYMENT FOR CONSTRUCTION OF SIDEWALKS ON BRIDGE AND APPROACH SLABS.

FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR REMOVAL OF EXISTING STRUCTURE, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

COORDINATE LOCATIONS AND LIMITS OF PROPOSED SIDEWALKS ON APPROACH SLABS WITH ASSOCIATED ROADWAY CONSTRUCTION, SEE ROADWAY PLANS.

ANY DAMAGE TO THE EXISTING DECK DURING THE REMOVAL OF EXISTING SIDEWALK CONCRETE AND 3 BAR METAL RAIL WILL BE REPAIRED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE DEPARTMENT,

ASPHALT WEARING SURFACE ON APPROACH SLABS IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS,

TOTAL BILL OF MATERIAL CLEANING AND PAINTING **EPOXY EPOXY** PAINTING POURABLE POLYESTER PLACING 8 1'-2" X 3'-7" REMOVAL OF GROOVING EXISTING **POLYMER ASBESTOS** COATED CLASS AA POLLUTION SILICONE POLYMER FINISHING SCARIFYING | SHOTBLASTING TWO BAR CONTAINMEN' EPOXY EXISTING BRIDGE CONCRETE CONCRETE WEATHERING BRIDGE ASSESSMENT BRIDGE DECK | BRIDGE DECK CONCRETE REINFORCING| CONTROL | METAL RAIL FOR BRIDGE JOINT CONCRETE COATING PC STRUCTURE **FLOORS** PARAPET MATERIALS STEEL SEALANT OVERLAY STEEL #500051 MATERIALS FOR BRIDGE (ALTERNATE #500051 LUMP SUM LUMP SUM SQ. FT. LBS. LUMP SUM LIN. FT. LUMP SUM LUMP SUM CU. YDS. CU. YDS. SQ. YDS. SQ. YDS. SQ. YDS. CU. YDS. LIN. FT. LIN. FT. SQ. FT. 500051 LUMP SUM LUMP SUM 9,662 205.7 9,487 LUMP SUM 437.96 453.67 LUMP SUM LUMP SUM 149.47 32.0 32.0 464 1,156 1,156 1,192

> RTH CAROL OFESSION Tou M. Game SEAL 033|39 : KNGINEER 5/17/2021

JOHNSTON STATION: 25+31.59 -Y18-SHEET 2 OF 2

PROJECT NO. I-5986B

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

COUNTY

GENERAL DRAWING FOR BRIDGE NO. 500051

ON NC 50 OVER I-95

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETE

INTERNATIONAL

8000 Regency Parkway, Sui Cary, North Carolina 275 NC License No.: F-108

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			REVI	SIO	NS		SHEET NO
ering uite 600	NO.	BY:	DATE:	NO.	BY:	DATE	S4-2
7518 084	1			3			TOTAL SHEETS
	2			4			14

DRAWN BY : M. D. MAYHEW DATE : 5-12-21 CHECKED BY : T. M. GARRISON DATE : 5-12-21

		LC	DAD A	ND F	RESIS	STANC	E FA	CTO	R RA	ATIN	G (L	RFR)	SUM	MAR	Y FO	R S	STEE	L GI	RDER	2S				
		STRENGTH I LIMIT STATE									SE	SERVICE II LIMIT STATE												
							MOMENT					SHEAR						MOMENT						
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	0.64		1.75	0.468	0.64	В	9	0.00	0.934	0.73	В	8	18.25	1.30	0.468	1.24	В	9	0.00	1
DESIGN LOAD		HL-93 (OPERATING)	N/A		0.83		1 . 35	0.468	0.83	В	9	0.00	0.934	0.94	В	8	18.25	1.00	0.468	1.61	В	9	0.00	1
LOAD RATING		HS-20 (INVENTORY)	36.000	2	1.06	38.16	1.75	0.468	1.07	В	9	0.00	0.934	1.06	В	8	18.25	1.30	0.468	1.99	В	9	72.98	1
	,	HS-20 (OPERATING)	36.000		1.37	49.32	1 . 35	0.468	1 . 39	В	9	0.00	0.934	1.37	В	8	18.25	1.00	0.468	2,59	В	9	72.98	1
		SNSH	13.500		1.91	25.79	1.40	0.468	1.91	В	9	0.00	0.467	2.74	В	9	18,25	1.30	0.468	2,95	В	9	0.00	1
	щ	SNGARBS2	20,000		1.70	34.00	1.40	0.468	1.70	В	9	0.00	0.467	2.29	В	9	18,25	1.30	0.468	2.63	В	9	0.00	1
	VEHICL SV)	SNAGRIS2	22,000		1.65	36.30	1.40	0.468	1.65	В	9	0.00	0.934	2.12	В	8	18.25	1.30	0.468	2,55	В	9	0.00	1
		SNCOTTS3	27.250		1.49	40.60	1.40	0.468	1.49	В	9	0.00	0.934	1.67	В	8	18.25	1.30	0.468	2.23	В	9	72.98	1
	NGLE (\$	SNAGGRS4	34.925		1.33	46.45	1.40	0.468	1 . 33	В	9	0.00	0.934	1.33	В	8	18.25	1.30	0.468	1.95	В	9	72.98	1
	SIN	SNS5A	35,550		1.32	46.93	1.40	0.468	1 . 32	В	9	0.00	0.934	1.33	В	8	18.25	1.30	0.468	1.93	В	9	72.98	1
		SNS6A	39.950		1.19	47 . 54	1.40	0.468	1 . 25	В	9	0.00	0.934	1 . 19	В	8	18.25	1.30	0.709	1.78	В	8	72.98	1
LEGAL LOAD RATING		SNS7B	42.000		1 . 15	48.30	1.40	0.468	1.21	В	9	0.00	0.934	1 . 15	В	8	18.25	1.30	0.709	1.70	В	8	72.98	1
RATING	ER	TNAGRIT3	33.000		1.37	45.21	1.40	0.468	1 . 37	В	9	0.00	0.934	1.43	В	8	18.25	1.30	0.468	2.06	В	9	72.98	1
	RAII	TNT4A	33.075		1.37	45.31	1.40	0.468	1.37	В	9	0.00	0.934	1.42	В	8	18.25	1.30	0.468	2.05	В	9	72.98	1
	1-I/	TNT6A	41.600		1.19	49.50	1.40	0.468	1.22	В	9	0.00	0.934	1.19	В	8	18.25	1.30	0.709	1.76	В	8	72.98	1
	SEM.	TNT7A	42.000		1.17	49.14	1.40	0.468	1.22	В	9	0.00	0.934	1.17	В	8	18,25	1.30	0.709	1.75	В	8	72.98	1
	CTOR (TT)	TNT7B	42,000		1.14	47.88	1.40	0.468	1.22	В	9	0.00	0.934	1.14	В	8	18.25	1.30	0.709	1.78	В	8	72.98	1
	TRAC	TNAGRIT4	43.000		1.11	47.73	1.40	0.468	1.20	В	9	0.00	0.934	1.11	В	8	18.25	1.30	0.709	1.72	В	8	72.98	1
	TRUCK	TNAGT5A	45.000		1.08	48.60	1.40	0.468	1.17	В	9	0.00	0.934	1.08	В	8	18.25	1.30	0.709	1.64	В	8	72.98	1
	TRI	TNAGT5B	45.000	3	1.06	47.70	1.40	0.468	1.17	В	9	0.00	0.934	1.06	В	8	18.25	1.30	0.709	1.62	В	8	72.98	1
FATIGUE		HL-93 (INVENTORY)	γ _{LL} = 0.80		1.45																			2

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1 . 25	1.50
FACTORS	SERVICE II	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE II LIMIT STATES.

ALLOWABLE STRESS FOR SERVICE II LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- 1. DISTANCE FROM LEFT END OF SPAN IS GIVEN WITH RESPECT TO THE CENTERLINE OF BEARING AND IS MEASURED ALONG THE CONTROLLING GIRDER.
- 2. A FATIGUE LIVE LOAD FACTOR OF 0.80 WAS USED TO BE CONSISTENT WITH THE LATEST EDITION OF THE MANUAL FOR BRIDGE EVALUATION.

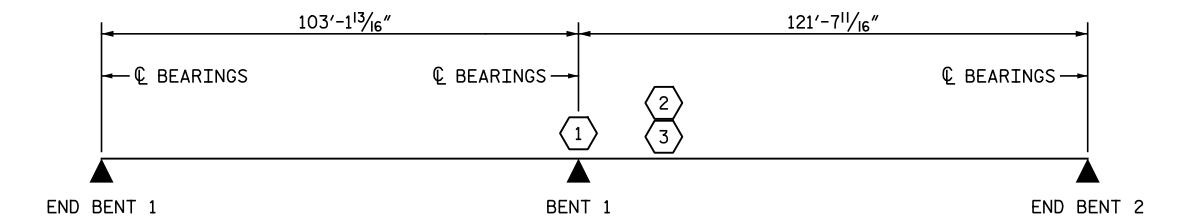


- 1 DESIGN LOAD RATING (HL-93) **
- $\langle 2 \rangle$ DESIGN LOAD RATING (HS-20) **
- 3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

GIRDER LOCATION PROVIDED UTILIZES GIRDER NUMBERS, WHERE GIRDER 1 IS LEFT EXTERIOR GIRDER LOOKING AHEAD STATION.

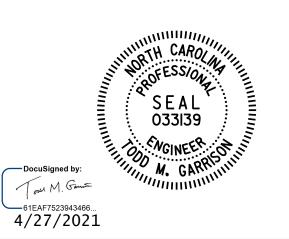


LRFR SUMMARY

PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 25+31.59 -Y18-



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

LRFR SUMMARY FOR STEEL GIRDERS (NON-INTERSTATE TRAFFIC)

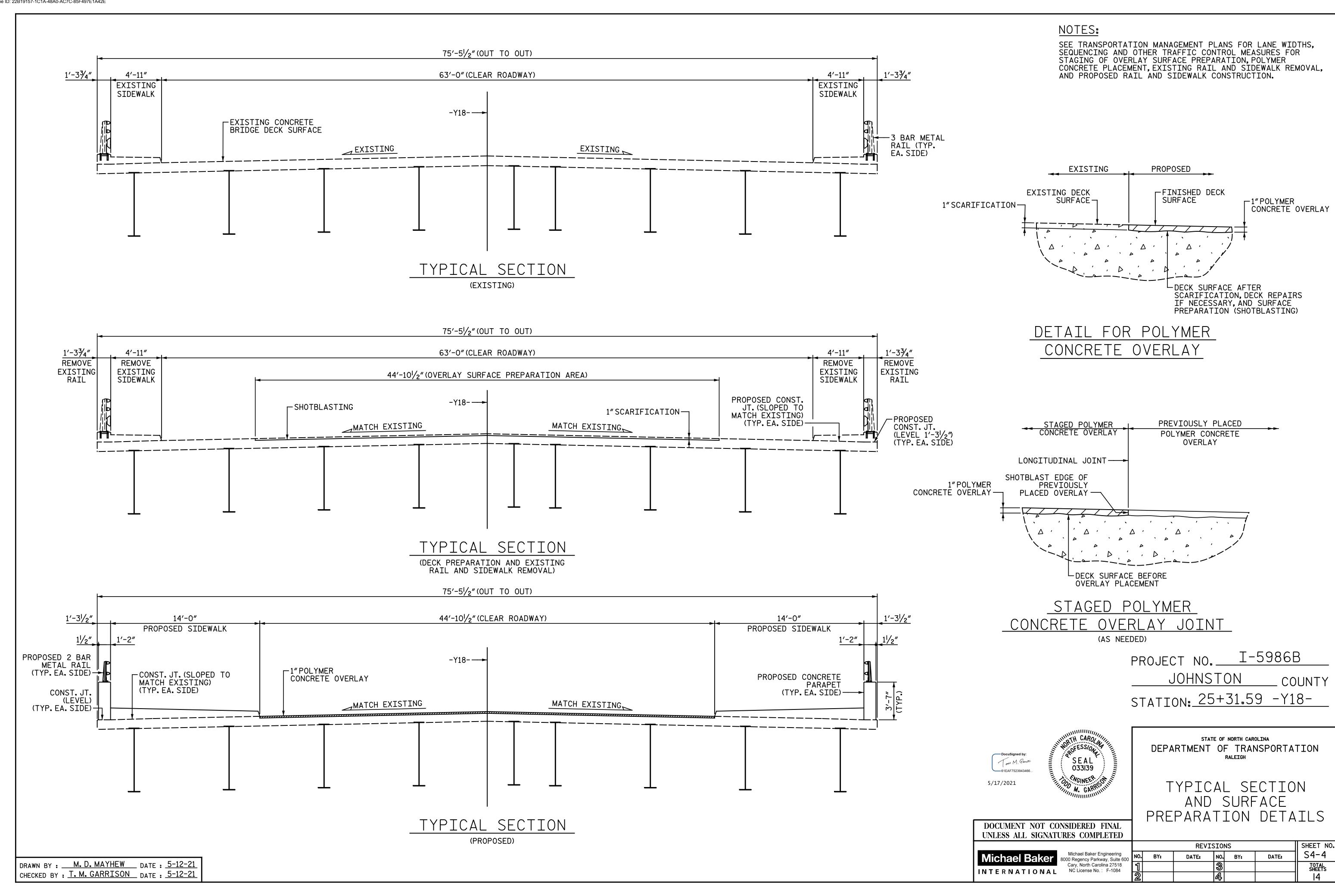
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

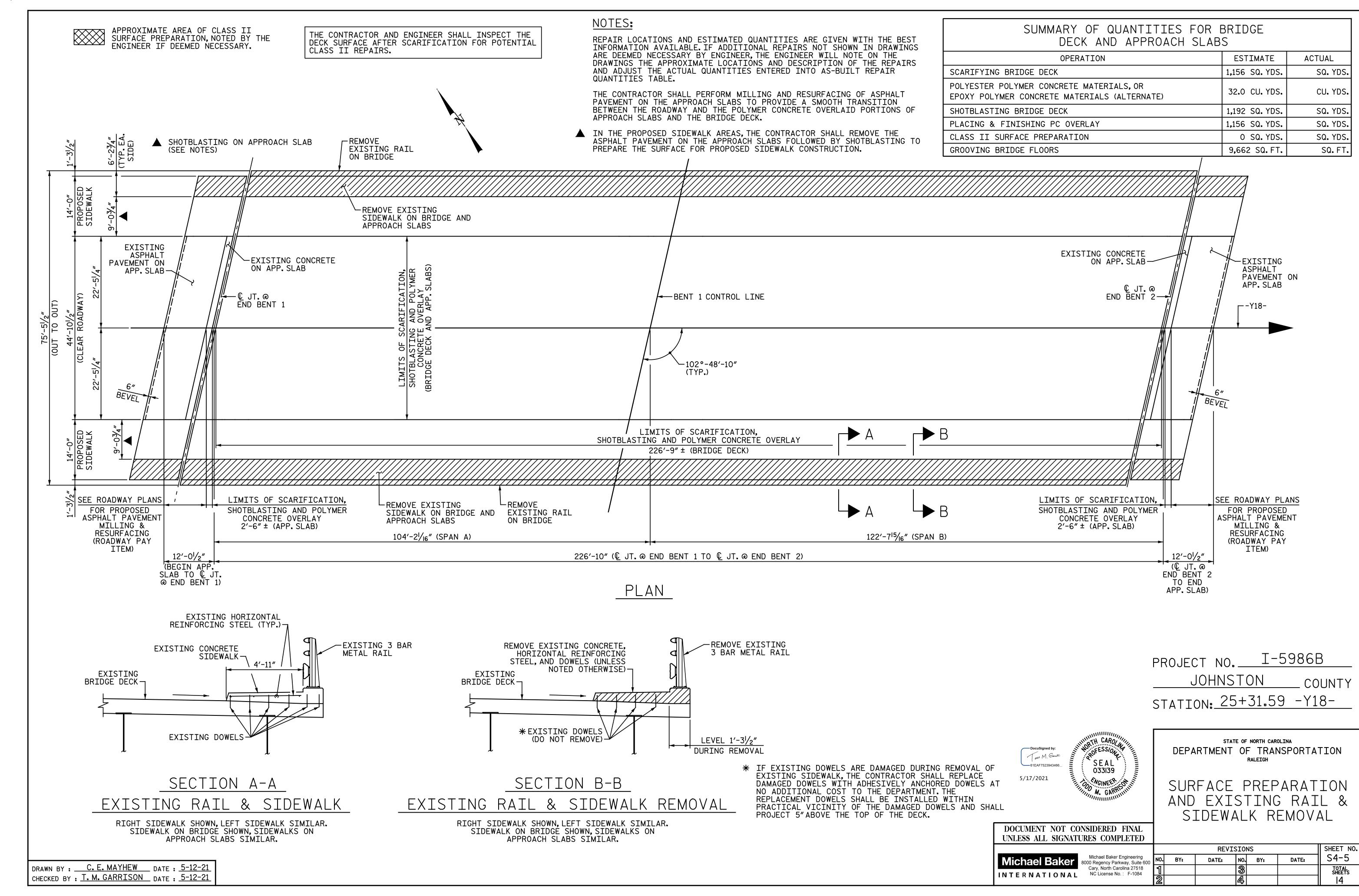
Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 NC License No.: F-1084

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0.	BY:	DATE:	NO.	BY:	DATE:	S4-3
]		-	3			TOTAL SHEETS
2		-	4			14

STD. NO. LRFR3

ASSEMBLED BY : N. B. SPEAK CHECKED BY : T. M. GARRISON		
INKAWN DIE MAA IZUO I	REV. II/I2/08RR REV. I0/I/II REV. I2/I7	MAA/GM MAA/GM MAA/THC



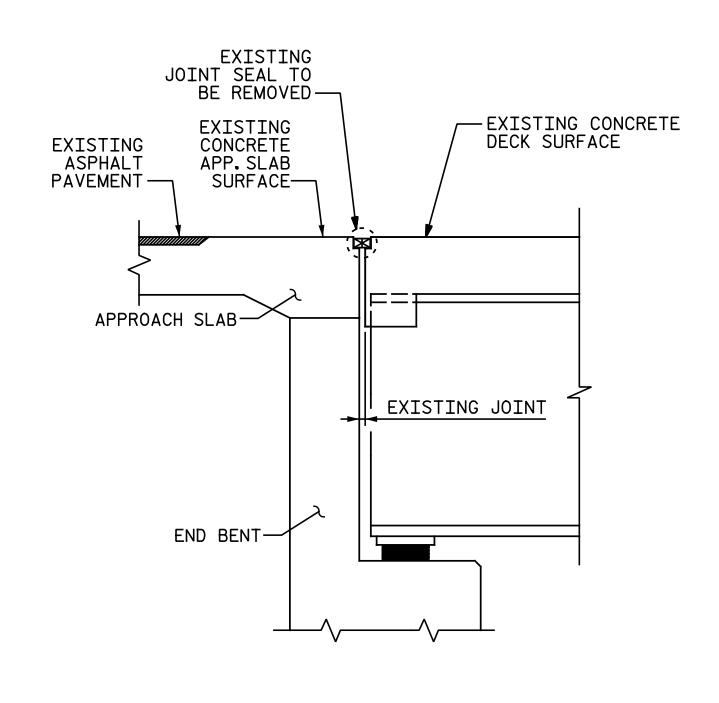


NOTES:

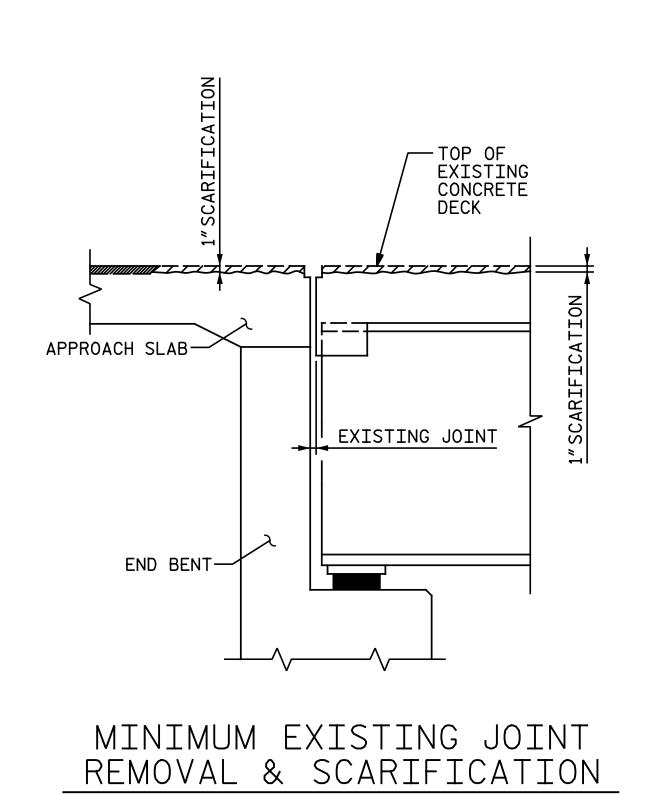
UNLESS NOTED OTHERWISE RETAIN ALL EXISTING REINFORCING STEEL. CLEAN AND REPAIR AS NEEDED.

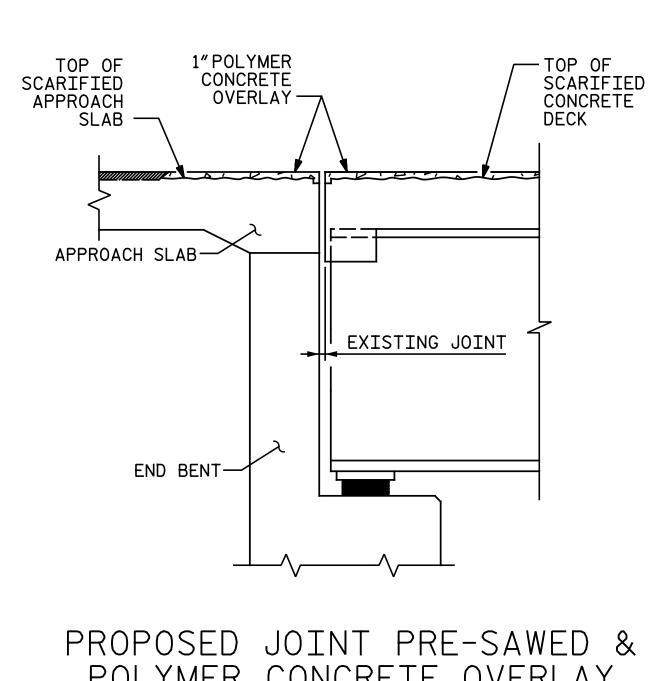
SILICONE JOINT SEALANT AND BACKER ROD SHALL BE INSTALLED AS PER MANUFACTURER'S RECOMMENDATION.

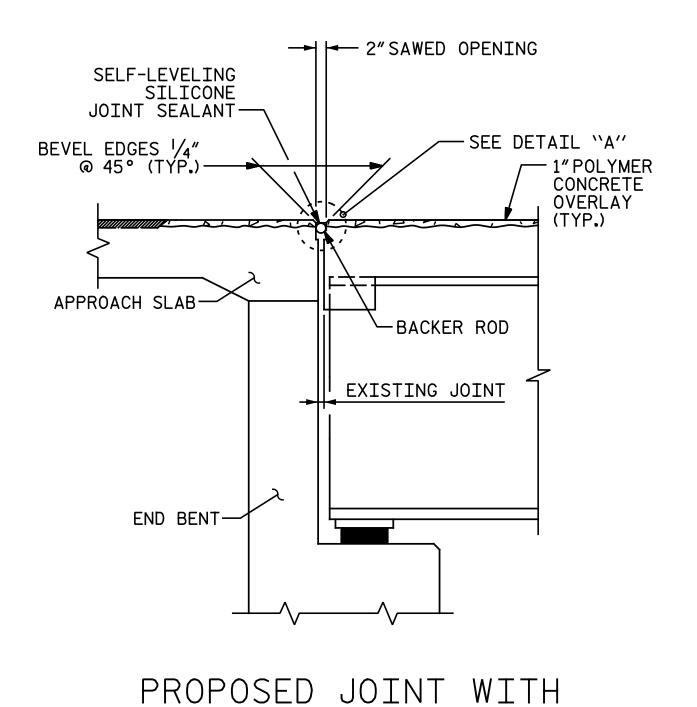
THE INSTALLED SILICONE JOINT SEALANT SHALL BE WATERTIGHT. FOR POURABLE SILICONE JOINT SEALANT, SEE SPECIAL PROVISIONS. THE CONTRACTOR SHALL NOT BE PERMITTED TO FORM THE JOINTS IN LIEU OF SAWING THE JOINTS.



EXISTING JOINT







SAWED DIMENSIONS

POLYMER CONCRETE OVERLAY

PROJECT NO. I-5986B

JOHNSTON _ COUNTY STATION: 25+31.59 -Y18-

WRTH CAROLANA OFESSION A Jose M. Game SEAL 033139 ON THEIR 5/17/2021

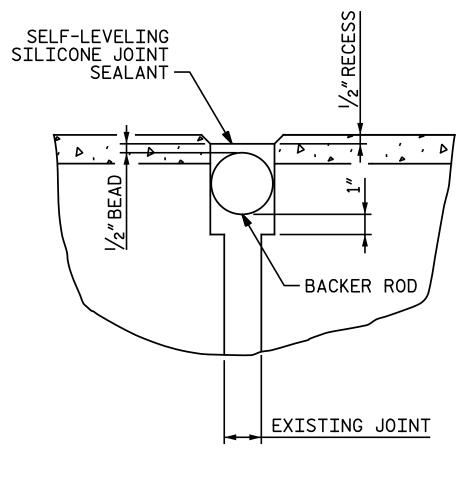
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

> JOINT REPAIR DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

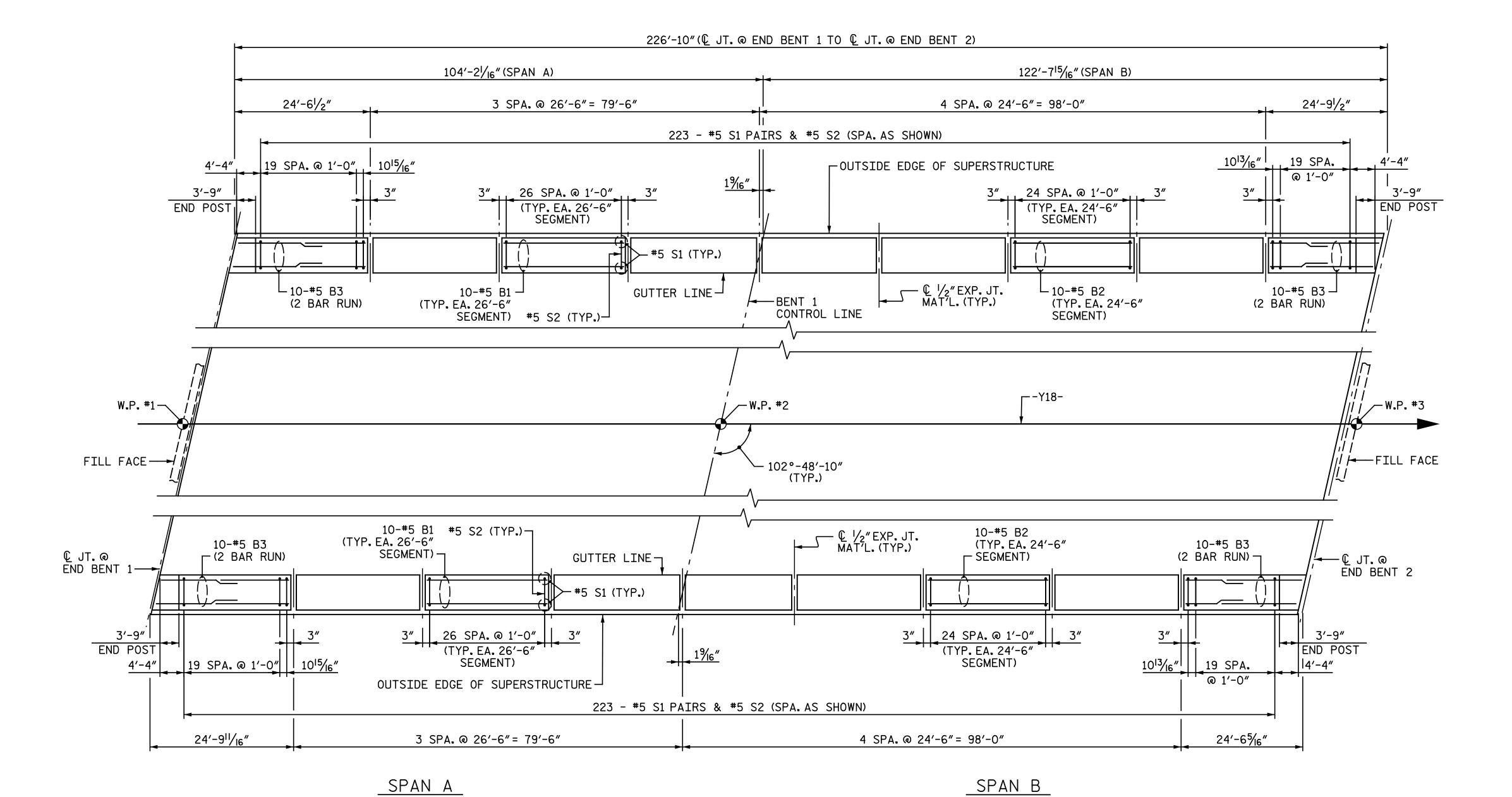
Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 INTERNATIONAL NC License No.: F-1084

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	1			3			TOTAL SHEETS
	ച						1 14



DETAIL "A"

DRAWN BY: N. B. SPEAKS DATE: 5-12-21 CHECKED BY: T. M. GARRISON DATE: 5-12-21



PLAN OF CONCRETE PARAPET

SEE "PARAPET AND END POST FOR TWO BAR RAIL" DETAILS ON "CONCRETE PARAPET DETAILS" SHEET 2 OF 2 FOR REINFORCING STEEL IN END POSTS.

SIDEWALKS NOT SHOWN FOR CLARITY.

SPLICE LENGTH				
BAR SIZE	EPOXY COATED			
#5	3′-5″			



PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 25+31.59 -Y18-

SHEET 1 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALETCH

CONCRETE PARAPET DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084

	SHEET NO.					
NO.	BY:	DATE	NO.	BY:	DATE:	S4-7
1			3			TOTAL SHEETS
2			4			14

DRAWN BY: N.B. SPEAKS DATE: 3-15-21 CHECKED BY: T.M. GARRISON DATE: 3-15-21

NOTES

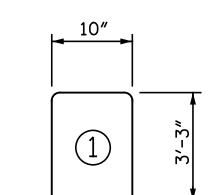
ALL REINFORCING STEEL IN PARAPETS AND END POSTS SHALL BE EPOXY COATED.

FOR DETAILS OF CONCRETE INSERTS IN END POSTS, SEE "RAIL POST SPACINGS

AND END OF RAIL DETAILS" SHEET.

GROOVED CONTRACTION JOINTS, 1/2"IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET AND END POSTS AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE #5 S1 AND S3 BARS SHALL BE INSTALLED USING AN ADHESIVE ANCHORING SYSTEM. THE YIELD LOAD FOR THE #5 BARS IS 18.6 KIPS. LEVEL TWO FIELD TESTING IS REQUIRED.



BAR TYPES

ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL

PROPOSED CONCRETE PARAPETS
AND END POSTS

C LCNOTH WETOUT					
E LENGTH WEIGHT					
R. 26' - 1" 1,632					
R. 24' - 1" 2,010					
R. 13' - 11" 1,161					
R. 3' - 8" 60					
R. 4' - 1" 67					
R. 4' - 7" 75					
R. 5' - 1" 83					
R. 5' - 5" 89					
R. 1' - 8" 20					
R. 3' - 0" 18					
23 24 25 25					
R. 3' - 2" 19					
R. 3' - 11" 24					
R. 3' - 6" 3,256					
7' - 4" 3,411					
R. 3' - 11" 131					
* EPOXY COATED					
REINFORCING STEEL LBS. 12,079					
CLASS AA CONCRETE CU. YDS. 71.0					

LIN. FT. 453.67

PROJECT NO. I-5986B

JOHNSTON COUNTY
STATION: 25+31.59 -Y18-

SHEET 2 OF 2

 $1'-2'' \times 3'-7''$

CONCRETE PARAPET

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

CONCRETE PARAPET DETAILS

REVISIONS

SHEET NO.

SAker Engineering
by Parkway, Suite 600
th Carolina 27518
se No.: F-1084

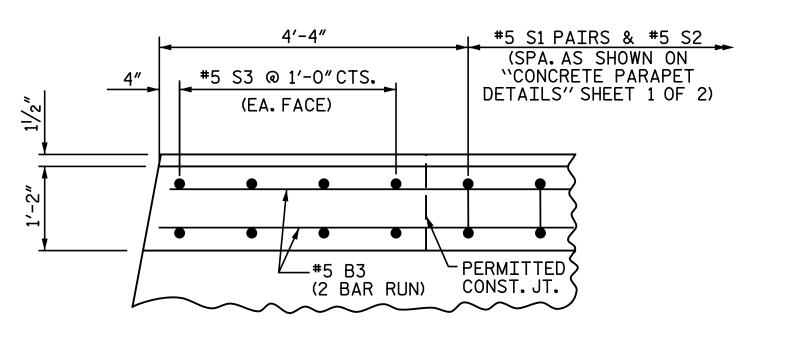
REVISIONS

SHEET NO.

S4-8

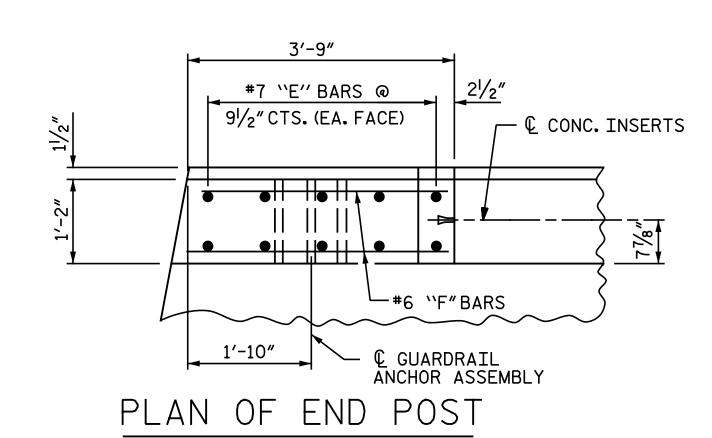
TOTAL
SHEETS

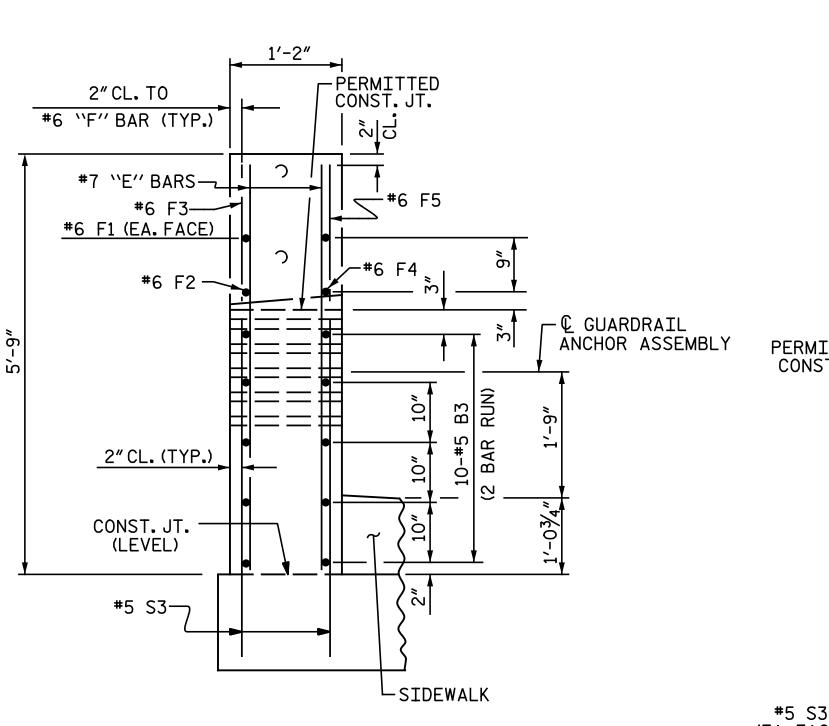
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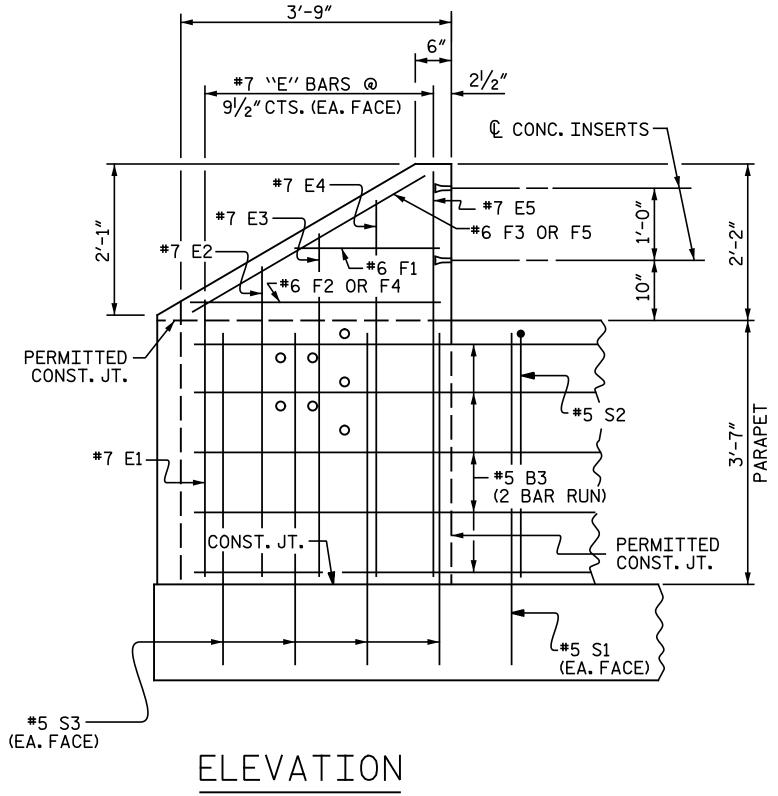


PLAN OF PARAPET

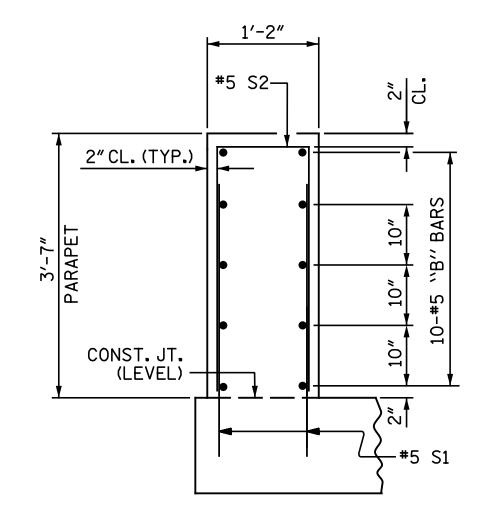
END VIEW







PARAPET AND END POST FOR TWO BAR RAIL



SECTION THRU PARAPET

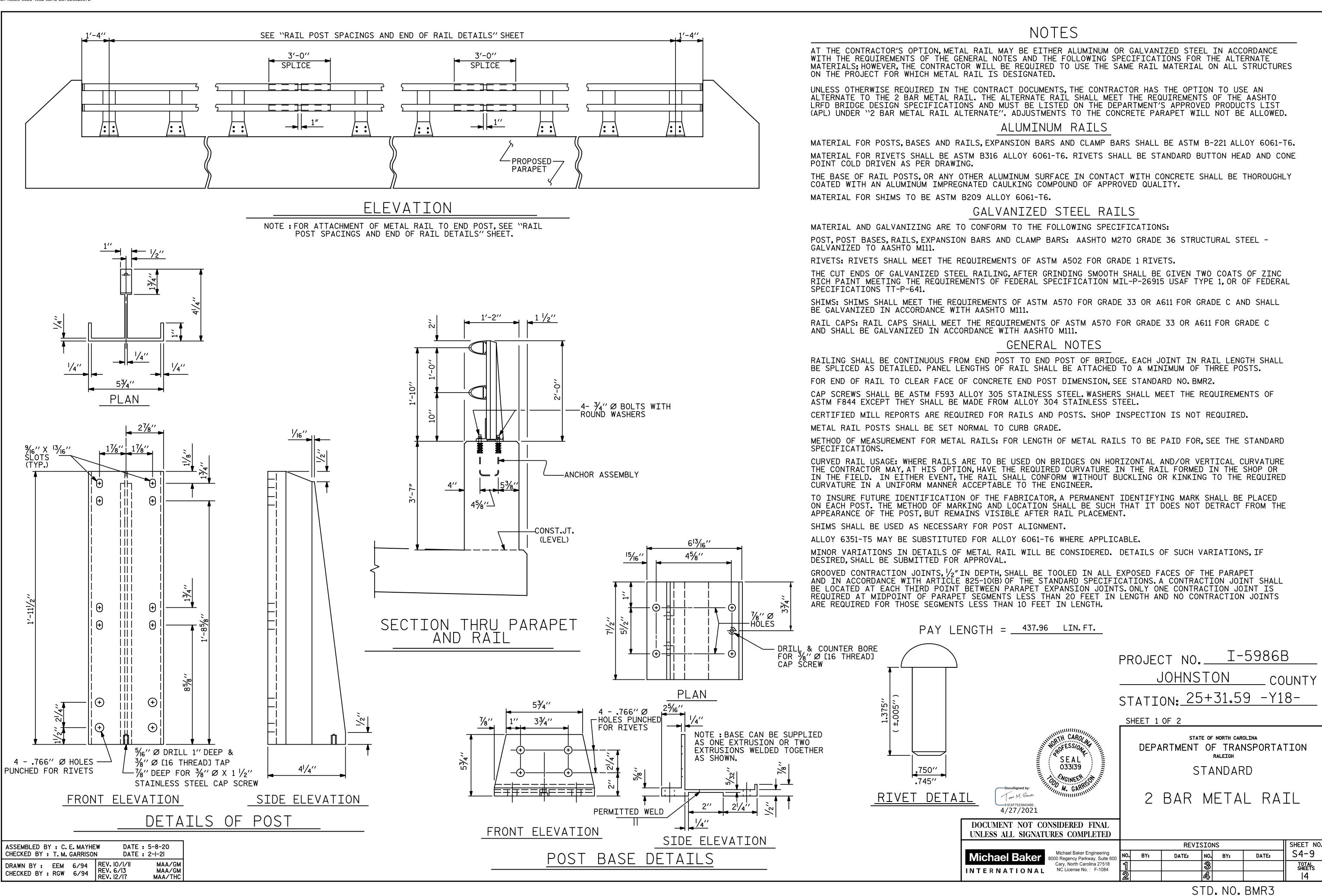
REINFORCING STEEL IN DECK NOT SHOWN FOR CLARITY. SIDEWALK NOT SHOWN FOR CLARITY.

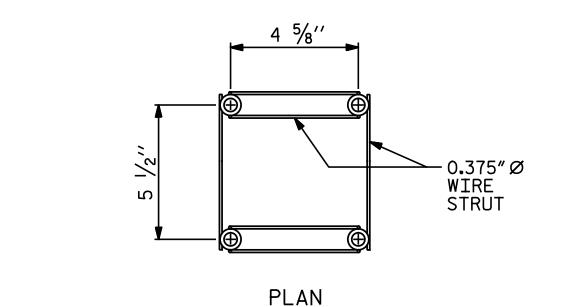


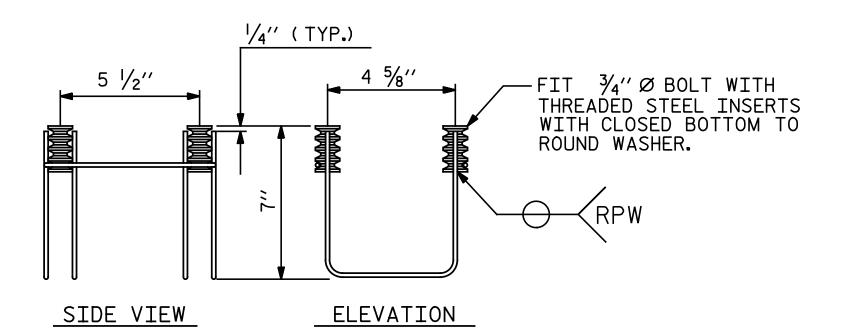
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Michael Baker	Michael Bake 8000 Regency P Cary, North C
INTERNATIONAL	NC License

DRAWN BY: N.B. SPEAKS DATE: 4-29-20 CHECKED BY: T.M. GARRISON DATE: 2-1-21







4-BOLT METAL RAIL ANCHOR ASSEMBLY

(76 ASSEMBLIES REQUIRED)

NOTES

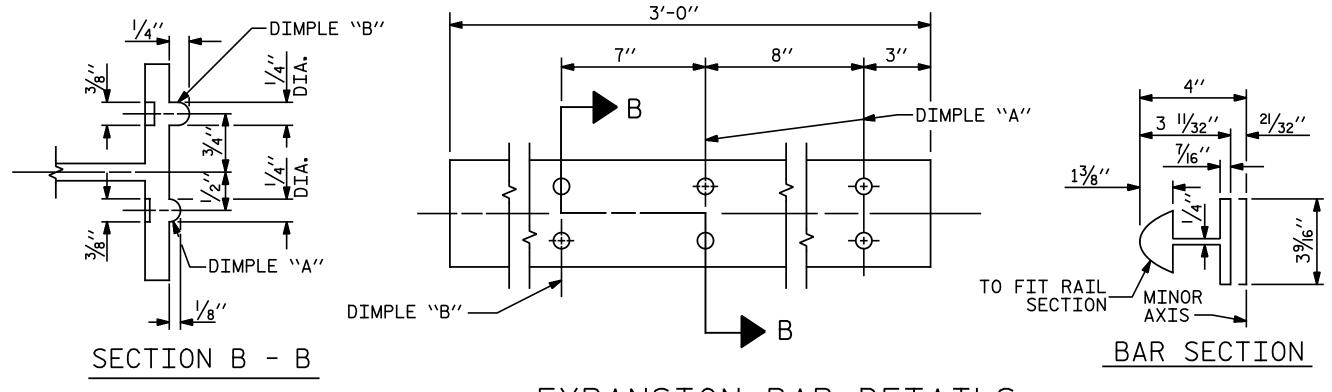
STRUCTURAL CONCRETE ANCHOR ASSEMBLY

THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

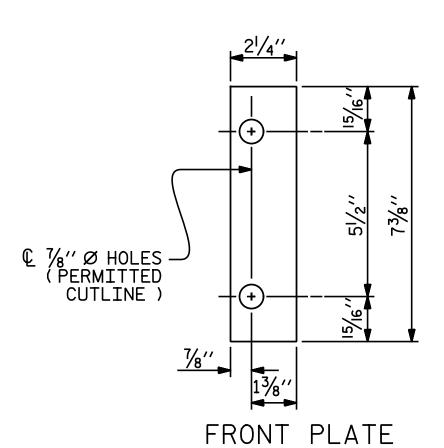
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 3/4" Ø X 21/2" BOLTS WITH WASHERS. BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 21/2" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A $7_{16}^{\prime\prime}$ Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT

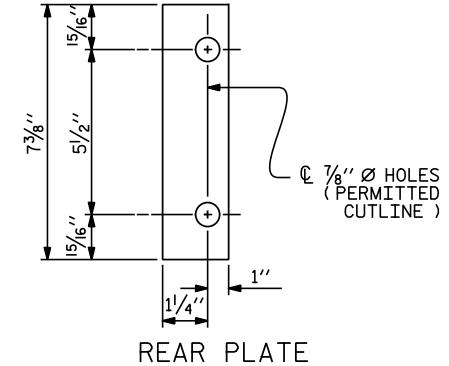
THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY, LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE THE STANDARD SPECIFICATIONS.

WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH, NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.



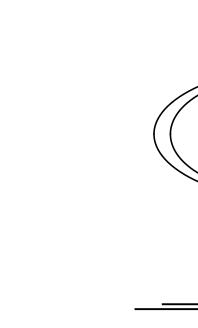
EXPANSION BAR DETAILS

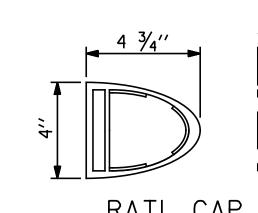


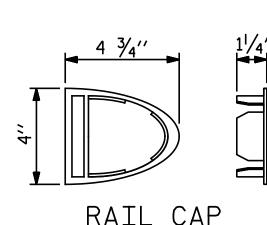


SHIM DETAILS

NOTE:
SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR
SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.







SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

/- SEMI-ELLIPSE

PROJECT NO. I-5986B

STATION: 25+31.59 -Y18-

JOHNSTON

MINOR (AXIS

RAIL SECTION

MAJOR AXIS

2 BAR METAL RAIL

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

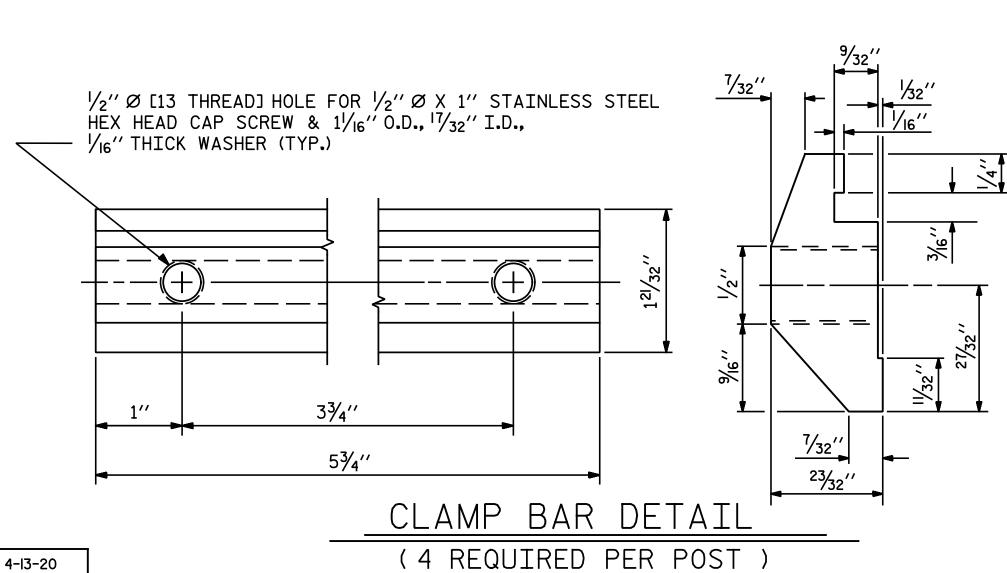
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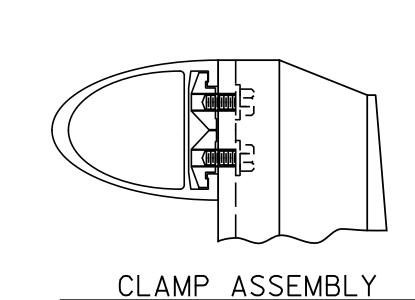
OD WGINEER

REVISIONS SHEET NO. Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 NO. BY: S4-I0 DATE: DATE: BY: INTERNATIONAL NC License No.: F-1084

STD. NO. BMR4

COUNTY





RAIL CAP

Tow M. Game

61EAF7523943466... 4/27/2021

ASSEMBLED BY : M. D. MAYHEW CHECKED BY : T. M. GARRISON DATE: 4-13-20 DATE : 2-1-21 DRAWN BY: EEM 6/94 REV. 5/I/06R CHECKED BY: RGW 6/94 REV. IO/I/II REV. I2/I7 MAA/GM

PLAN OF RAIL POST SPACINGS

SIDEWALKS NOT SHOWN FOR CLARITY.

NOTES

STRUCTURAL CONCRETE INSERT

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 11/2".
- B. 1 $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " BOLT WITH WASHER, BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307, BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE $\frac{3}{4}$ " \varnothing X $1\frac{5}{8}$ " GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A γ_6 " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

NOTES

METAL RAIL TO END POST CONNECTION

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

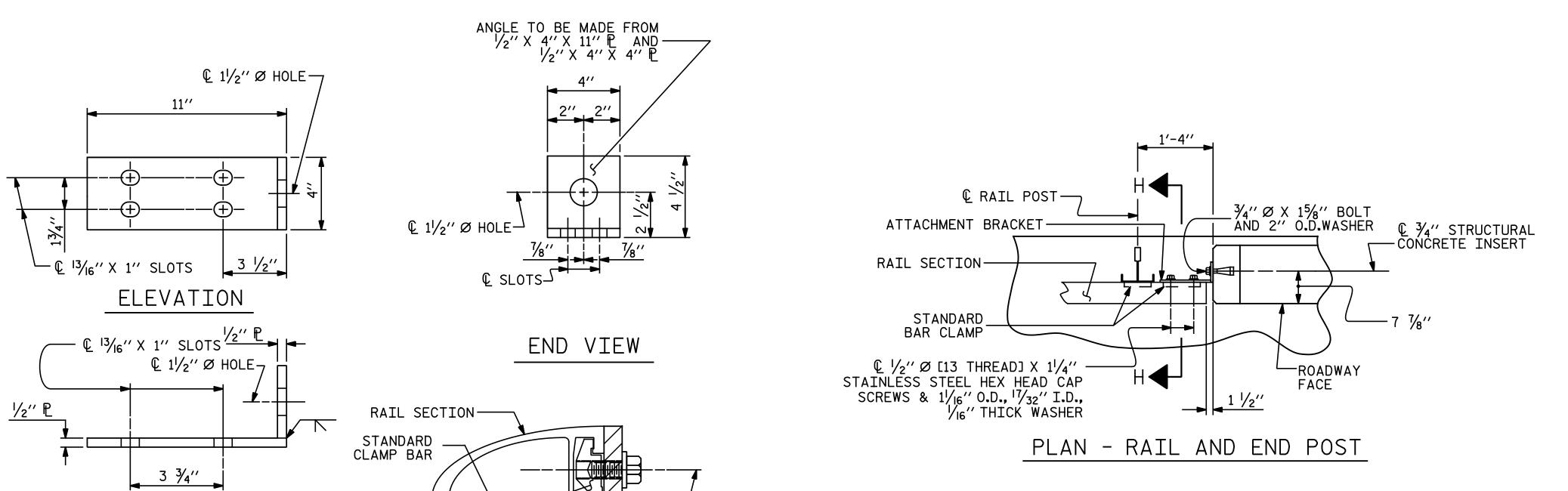
- A. $\frac{1}{2}$ " PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.
- B_{\bullet} $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A 3/4"Ø X 15/8" BOLT WITH 2" O.D. WASHER IN PLACE. THE 3/4"Ø X 15/8" BOLT SHALL HAVE N. C. THREADS.
- C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.
- D. STANDARD CLAMP BARS (SEE METAL RAIL SHEET).
- E. $\frac{1}{2}$ " Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 1 OR 2 BAR METAL RAILS.

THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE $\frac{1}{2}$ " PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE 3/200 X 15/800BOLT WITH WASHER SHALL BE REPLACED WITH A $\frac{3}{4}$ " Ø X $6\frac{1}{2}$ " BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE 34" Ø X 158" BOLT SHALL APPLY TO THE 34" Ø X 6 1/2" BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.



© 1/2" Ø [13 THREAD] X 11/4" STAINLESS STEEL HEX

HEAD CAP SCREWS & 11/16" O.D., 17/32" I.D., 1/16" THICK WASHER

DETAILS FOR ATTACHING METAL RAIL TO END POST

SECTION H-H

CLOSED-END R.P.W.(TYP.ALL\ CONTACT POINTS) FERRULE -**.**375′′Ø— WIRE STRUT

ELEVATION PLAN

STRUCTURAL CONCRETE

* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 25+31.59 -Y18-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

RAIL POST SPACINGS END OF RAIL DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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REVISIONS NO. BY: DATE: BY:

STD. NO. BMR2

DATE:

SHEET NO.

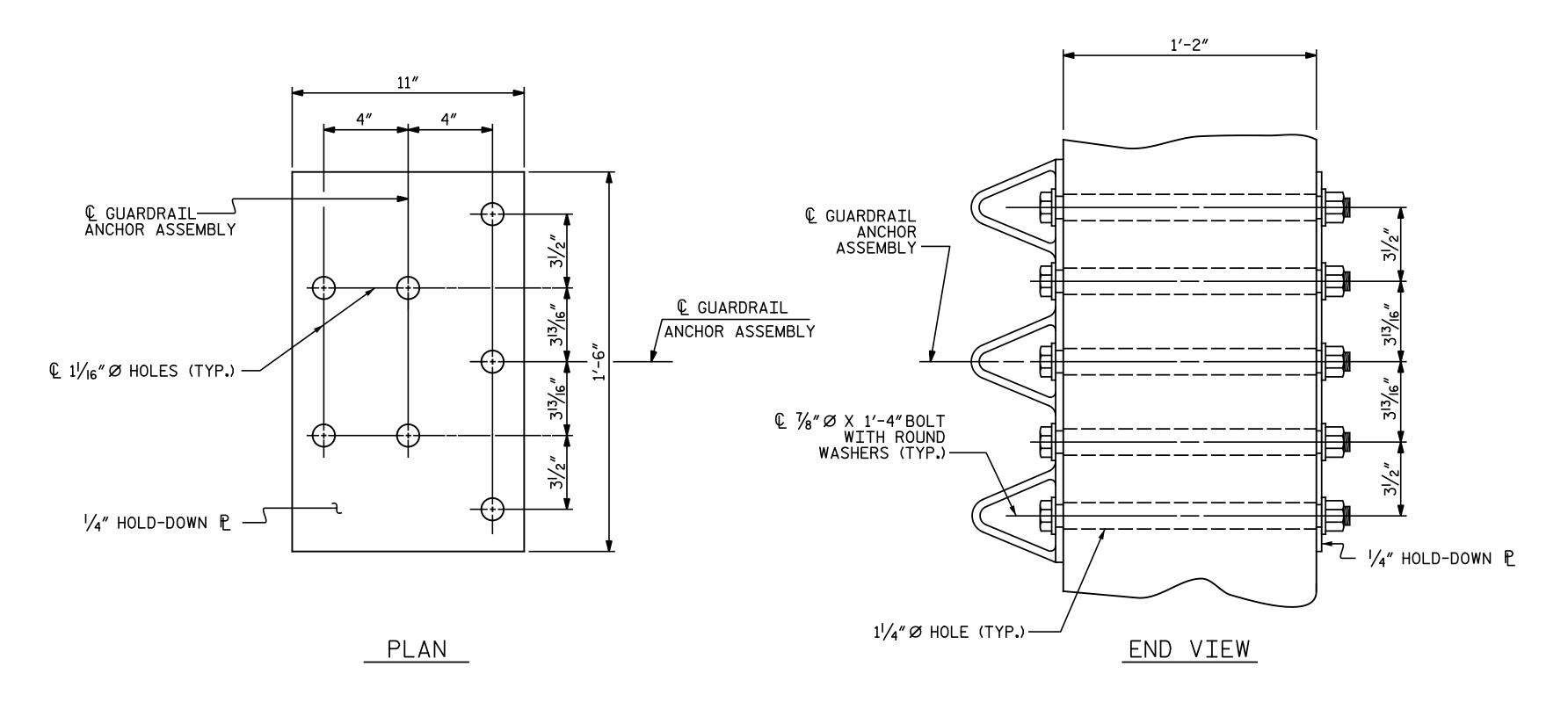
TOTAL SHEETS

S4-II

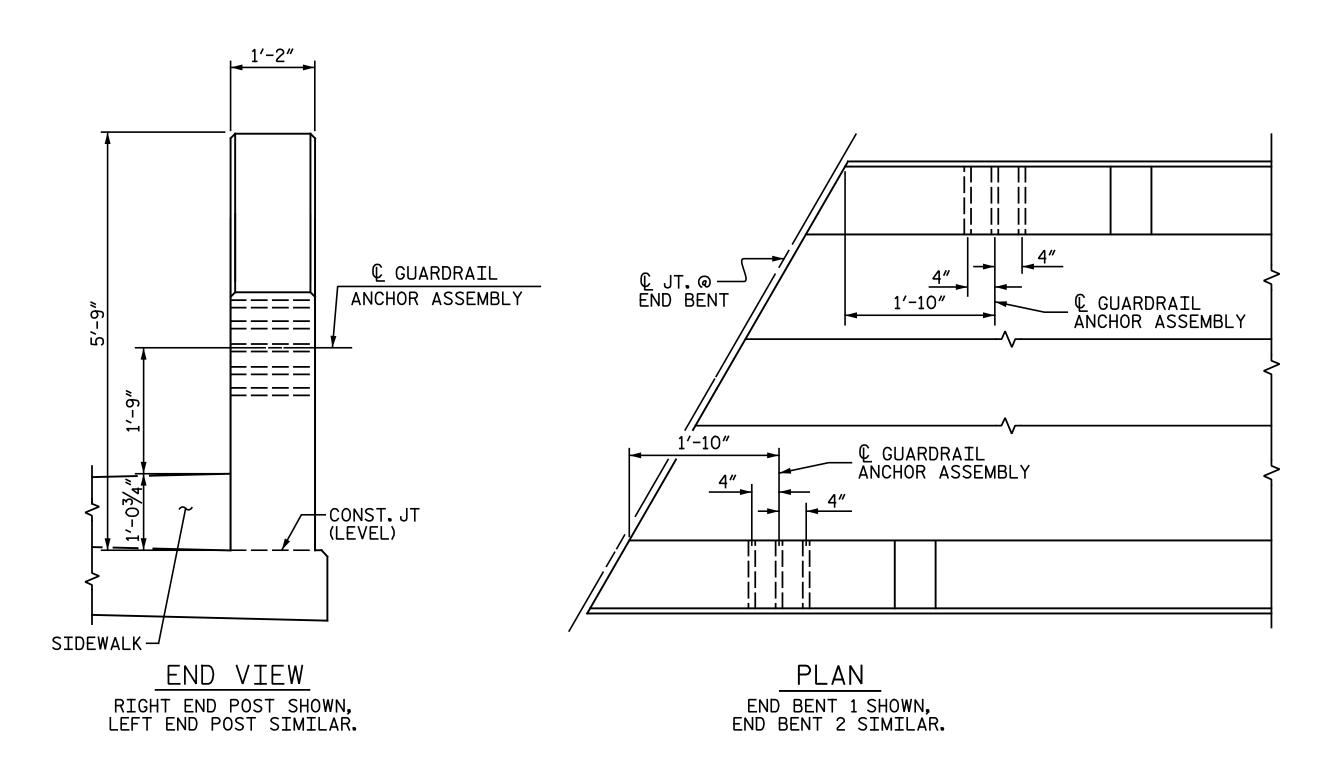
ASSEMBLED BY : C. E. MAYHEW CHECKED BY : T. M. GARRISON

DATE: 4-13-20 DATE : 2-1-21 TLA/GM DRAWN BY: FCJ 1/88 REV. 10/1/11 REV. 12/17 MAA/GM CHECKED BY : CRK 3/89 MAA/THC

TOP VIEW



GUARDRAIL ANCHOR ASSEMBLY DETAILS



ASSEMBLED BY : C. E. MAYHEW DATE: 4-13-20 DATE : 2-1-21 CHECKED BY : T. M. GARRISON REV. 1/15 REV. 12/17 REV. 5/18 MAA/TMG MAA/THC DRAWN BY: MAA 5/IO CHECKED BY: GM 5/IO

LOCATION OF GUARDRAIL ANCHOR AT END POST

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED, AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

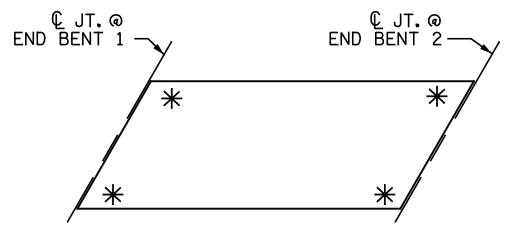
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

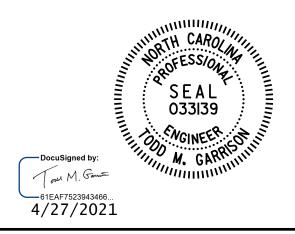
THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

*LOCATION OF GUARDRAIL ATTACHMENT

PROJECT NO. I-5986B JOHNSTON _ COUNTY STATION: 25+31.59 -Y18-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

GUARDRAIL ANCHORAGE DETAILS FOR METAL RAILS

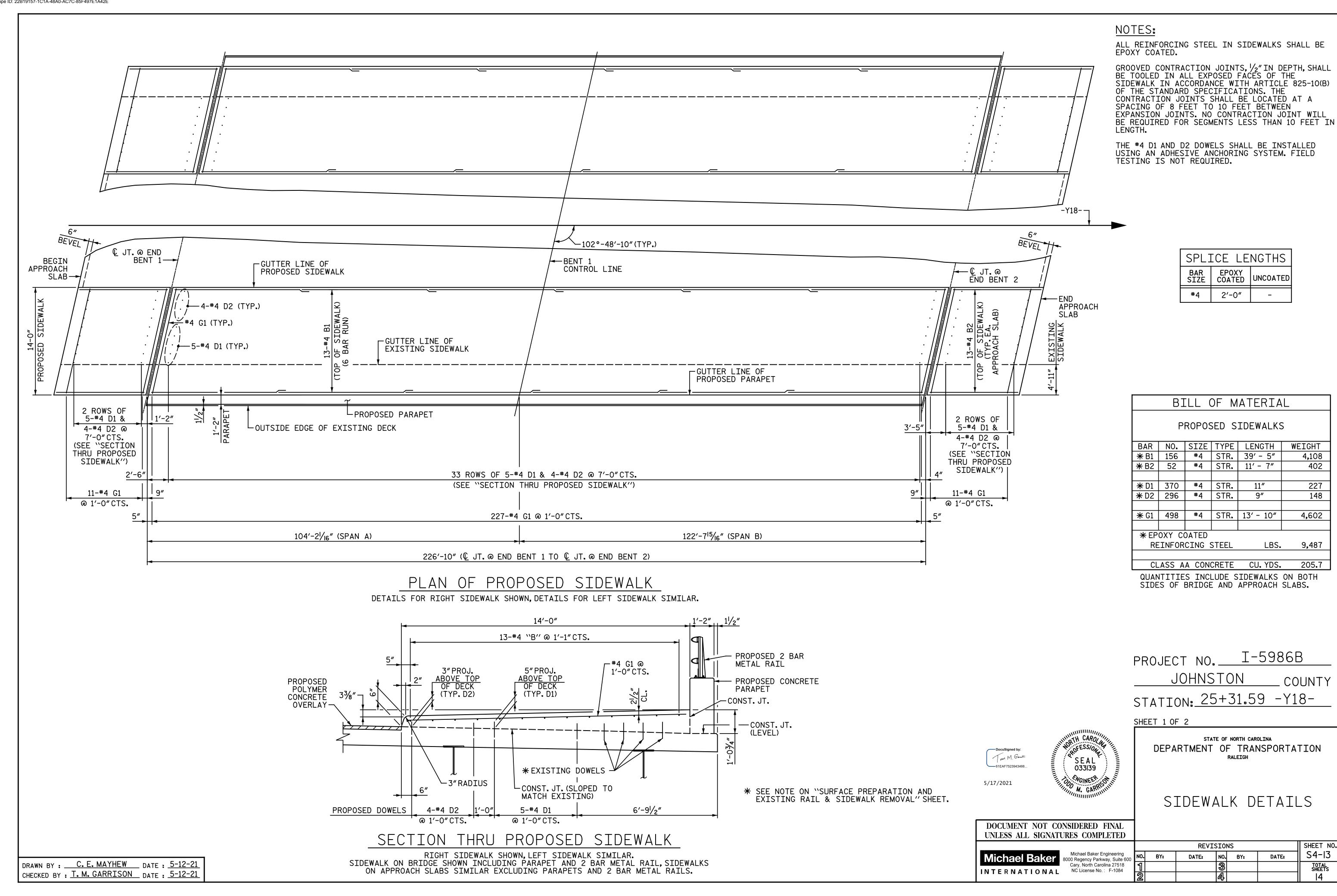
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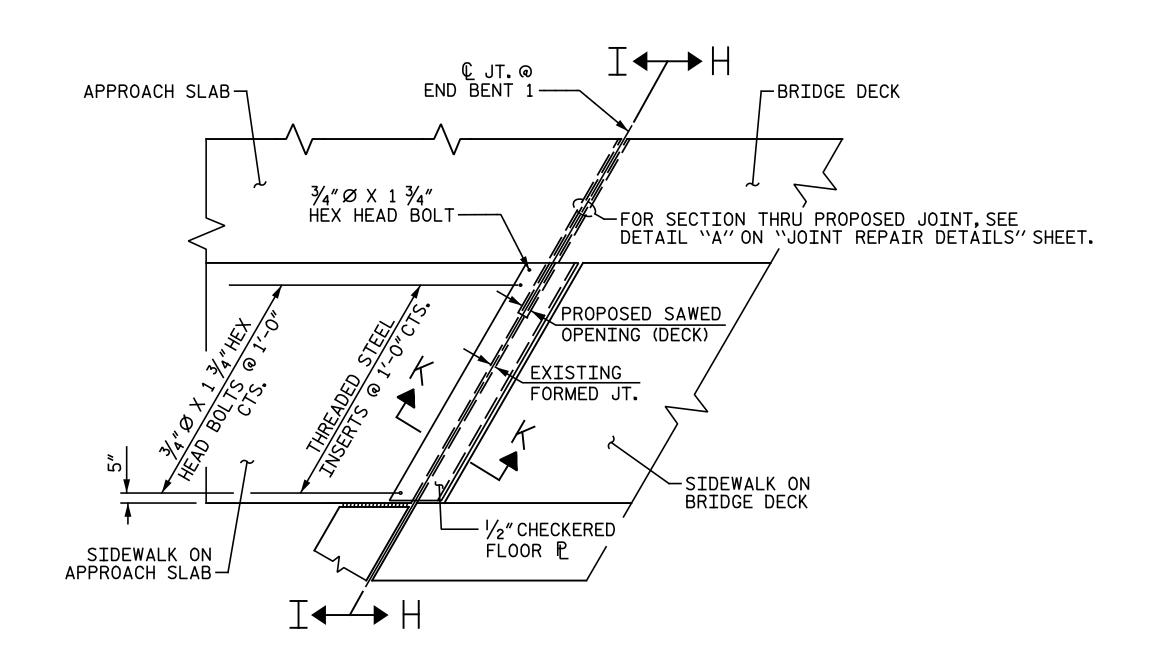
INTERNATIONAL NC License No.: F-1084

Cary, North Carolina 27518

SHEET NO. **REVISIONS** NO. BY: S4-I2 Michael Baker Engineering 8000 Regency Parkway, Suite 600 DATE: DATE: BY:

STD. NO. GRA3

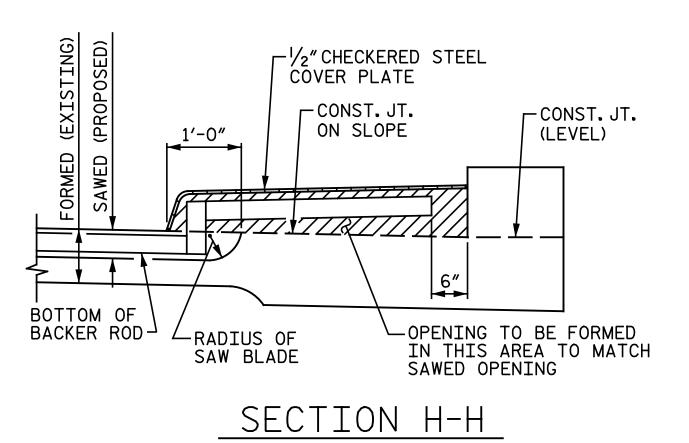


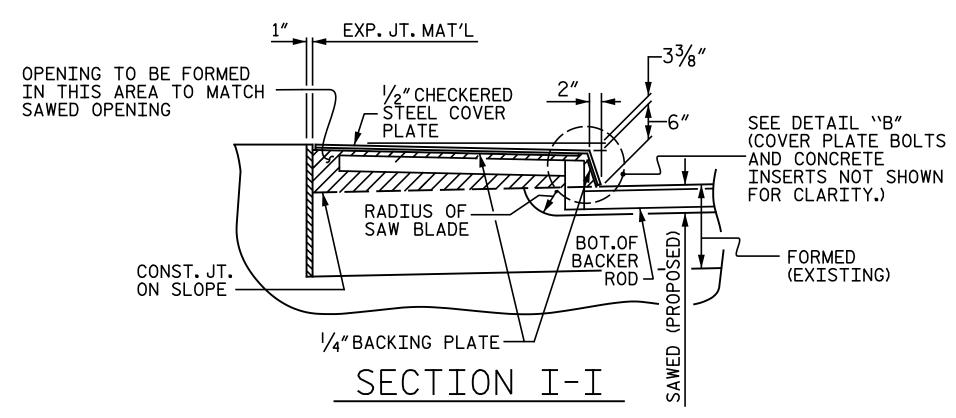


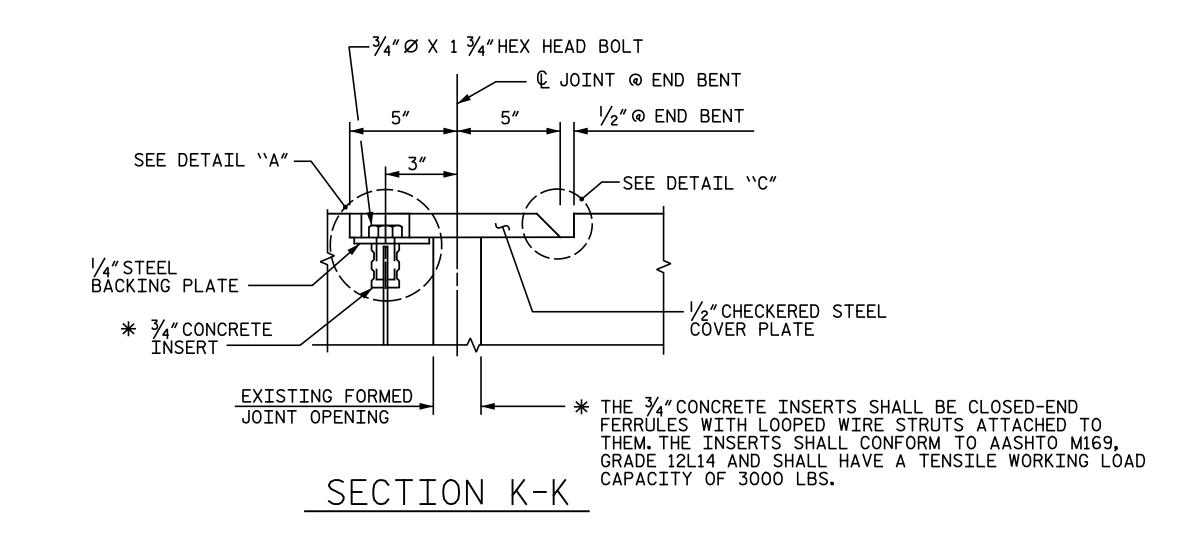
PLAN VIEW OF PROPOSED

JOINT @ END BENT FOR SIDEWALK

RIGHT SIDEWALK SHOWN, LEFT SIDEWALK SIMILAR. END BENT 1 SHOWN, END BENT 2 SIMILAR.







NOTES:

SEE SPECIAL PROVISIONS.

THE STEEL PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 OR APPROVED EQUAL. AFTER FABRICATION,

THE PLATES SHALL BE COMMERCIALLY BLAST CLEANED AND EITHER COATED WITH A MINIMUM THICKNESS OF 4 MILS (DRY) OF ZINC-RICH PAINT, GALVANIZED OR

METALLIZED TO A MINIMUM THICKNESS OF 6 MILS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION).

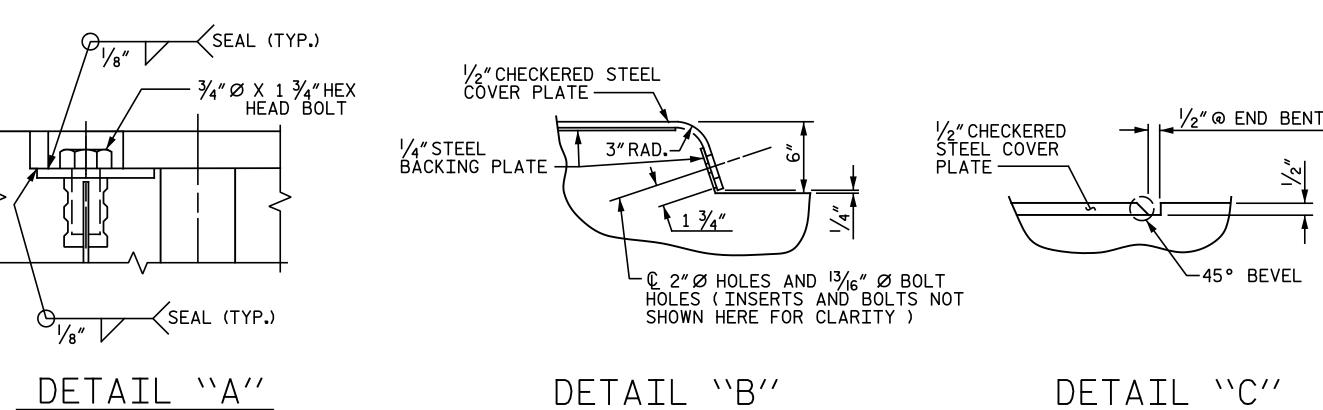
THE 3/4" DIAMETER HEX HEAD BOLTS SHALL CONFORM

OF THIS WORK SHALL BE INCLUDED IN THE LINEAR

NO SEPARATE PAYMENT WILL BE MADE FOR FURNISHING AND INSTALLING THE COVER PLATES. THE ENTIRE COST

FOOT PRICE FOR "POURABLE SILICONE JOINT SEALANT".

TO ASTM F593 ALLOY 304 STAINLESS STEEL.



- UNLESS AL

JOINT SEAL DETAILS @ END BENT

PROJECT NO. I-5986B

JOHNSTON COUNTY

STATION: 25+31.59 -Y18-

SHEET 2 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SIDEWALK DETAILS

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Jou M. Game

61EAF7523943466... 4/27/2021

Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084

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NO. BY: DATE:
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| REVISIONS | SHEET NO. | S4-14 | SHEETS | S4-14 | SHEETS | S4-14 | SHEETS | S4-14 | SHEETS |

DRAWN BY: C.E. MAYHEW DATE: 4-22-21
CHECKED BY: T. M. GARRISON DATE: 4-22-21