

RETAINING WALL ELEVATIONS									
₩-L- STA	WALL STATIONS	* OFFSET FROM (L -L- (LEFT)	ELEV @ TOP OF WALL	BOTTOM OF FOOTING ELEVATION					
11+25.00	10+00.00	30.00	2,124.70	2,116.70					
11+35.00	10+10.00	29.90	2,124.88	2,116.88					
11+45.00	10+19.82	29.97	2,125.00	2,117.00					
11+55.00	10+29.15	30.04	2,125.08	2,117.08					
11+65.00	10+38.48	30.07	2,125.12	2,117.12					
11+75.00	10+47.81	30.04	2,125.16	2,117.16					
11+85.00	10+57.17	29.63	2,125.19	2,117.19					
11+95.00	10+66.59	28.54	2,125.21	2,117.21					
12+05.00	10+76.03	27.44	2,125.18	2,117.18					
12+15.00	10+85.5	26.34	2,125.12	2,117.12					
12+25.00	10+94.99	25.23	2,125.01	2,117.01					
12+35.00	11+04.5	24.13	2,124.86	2,116.86					
12+45.00	11+14.04	23.03	2,124.67	2,116.67					
12+55.00	11+23.61	21.93	2,124.43	2,116.43					
12+65.00	11+33.19	20.83	2,124.16	2,116.16					
12+75.00	11+42.81	19.73	2,123.86	2,115.86					
12+85.00	11+52.42	19.00	2,123.55	2,115.55					
12+95.00	11+62.00	19.00	2,123.21	2,115.21					
13+05.00	11+71.58	19.00	2,122.87	2,114.87					
13+15.00	11+81.15	19.00	2,122.53	2,114.53					
13+25.00	11+90.73	19.00	2,122.19	2,114.19					
13+30.00	11+95.52	19.00	2,122.02	2,114.02					

* STATIONS AND OFFSETS ARE TO FILL FACE OF WALL.

U-5887 PROJECT NO.__ <u>HENDERSON</u> COUNTY

11+25.00 -L-STATION:

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> RETAINING WALL DETAILS

- Docusigned by 046056 Consulting Engineers

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CHECKED BY: RTS	DATE: 07/
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7/2020	/2020 REVISIONS									
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7/2020	1			3			TOTAL SHEETS			
172020	2			4			36			

- 1) FOR STANDARD CAST-IN-PLACE (CIP) CANTILEVER RETAINING WALLS, SEE CAST-IN-PLACE CANTILEVER RETAINING WALL SPECIAL PROVISION.
- 2) THIS WALL IS TO RECEIVE ARCHITECTURAL SURFACE TREATMENT. FOR ARCHITECTURAL SURFACE TREATMENT, SEE SPECIAL PROVISIONS.
- 3) FOR ANODIZED 2-BAR METAL RAIL, SEE SPECIAL PROVISIONS.
- 4) FOR STEEL BEAM GUARDRAIL, SEE ROADWAY PLANS AND SECTION 862 OF THE STANDARD SPECIFICATIONS.
- 5) DRAIN PIPES ARE REQUIRED FOR RETAINING WALL.
- 6) SUBSURFACE INFORMATION IS BASED ON RETAINING WALL BORINGS RW-1 THRU RW-5 PERFORMED BY ECS SOUTHEAST, LLP AS PART OF THE U-5887 (N. HIGHLAND LAKE ROAD) ROADWAY REALIGNMENT PROJECT.
- 7) STANDARD DESIGN OF THE CIP RETAINING WALL IS BASED ON THE FOLLOWING PARAMETERS:
 - A) MAXIMUM FACTORED VERTICAL PRESSURE ON FOUNDATION MATERIAL = 1,620 LBS/SF FOR RETAINING WALL.
 - B) MINIMUM REQUIRED TOE COVER AT FRONT FACE OF RETAINING WALL = 3'-3".
- 8) DESIGN RETAINING WALL FOR A LIVE LOAD (TRAFFIC) SURCHARGE OF 260 LBS./SF.
- 9) DO NOT PLACE CONCRETE FOR FOOTINGS FOR RETAINING WALL UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIALS ARE APPROVED.
- 10) AT THE CONTRACTOR'S OPTION, TEMPORARY SHORING FOR WALL CONSTRUCTION MAY BE USED TO CONSTRUCT RETAINING WALL. SEE CAST-IN-PLACE CANTILEVER RETAINING WALL SPECIAL PROVISION FOR TEMPORARY SHORING FOR WALL CONSTRUCTION.
- 11) CONTRACTOR SHALL ENSURE THAT THE FOUNDATION BEARING MATERIAL ACHIEVES A MINIMUM BEARING CAPACITY OF 2,500 LBS./SF.

TOTAL RETAINING WALL	L QUANTITIES
* CLASS 'A' CONCRETE	358.8CY
* REINFORCING STEEL	27,083 LBS.
* EPOXY COATED REINFORCING STEEL	4,120 LBS.
ANODIZED TWO BAR METAL RAIL	188.O LF.
* 4"Ø PERFORATED SCHEDULE 40 PVC DRAINAGE PIPE	196.0 LF.
₩ 4"Ø SCHEDULE 40 PVC OUTLET PIPE	30.0 LF.
* GEOTEXTILE	
* #78M STONE BACKFILL	120 TONS
ARCHITECTURAL SURFACE TREATMENT	1,990SF.

* THESE QUANTITIES ARE FOR INFORMATION ONLY, WALL SHALL BE PAID FOR PER SQUARE FOOT OF WALL.

** CIP CANTILEVER RETAINING WALLS 978 SQ.FT.

* * QUANTITY IS BASED ON TOTAL WALL AREA.

> PROJECT NO. U-5887 <u>HENDERSON</u> COUNTY 11+25.00 -L-STATION:_

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> RETAINING WALL DETAILS

Consulting Engineers

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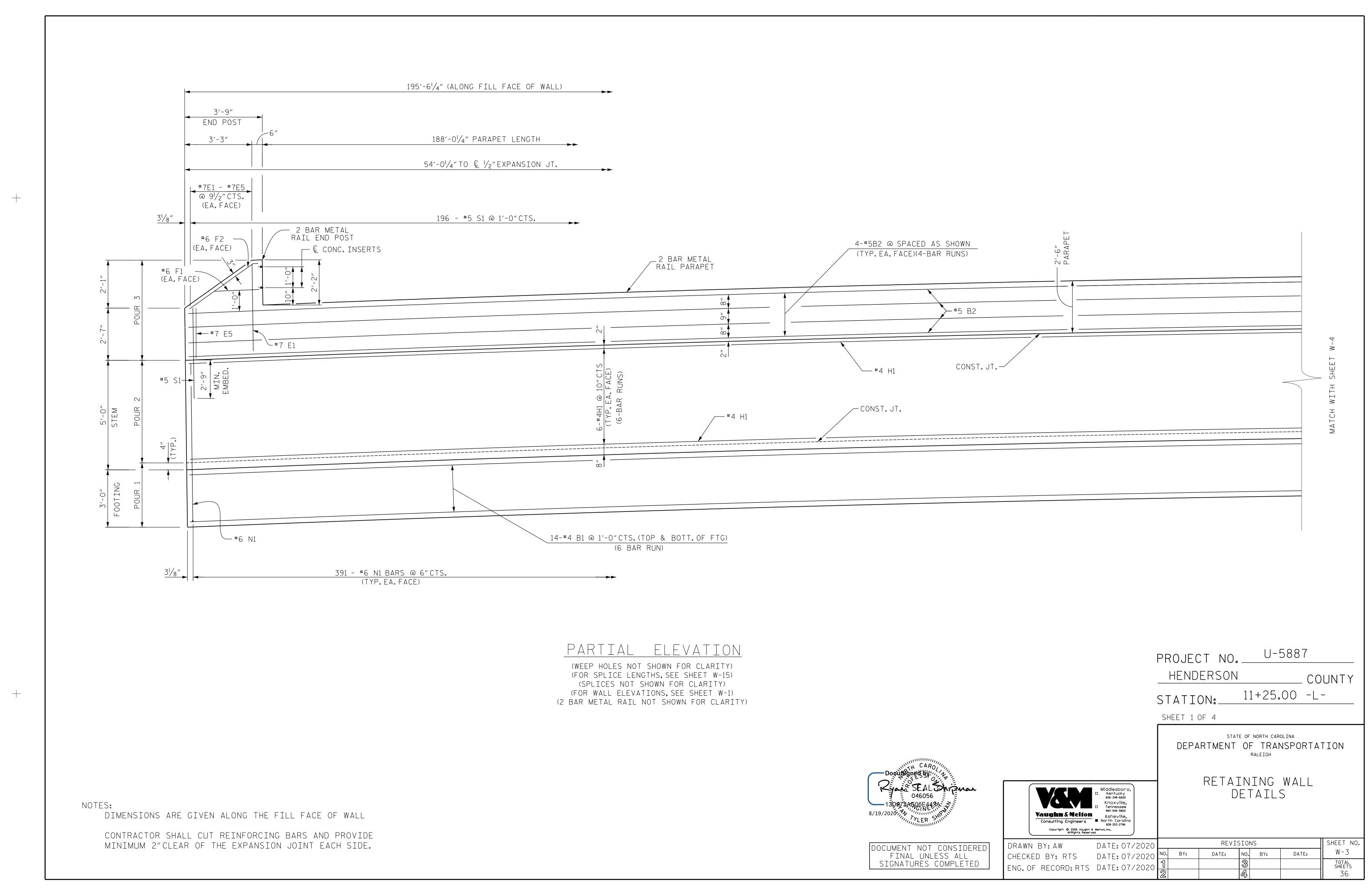
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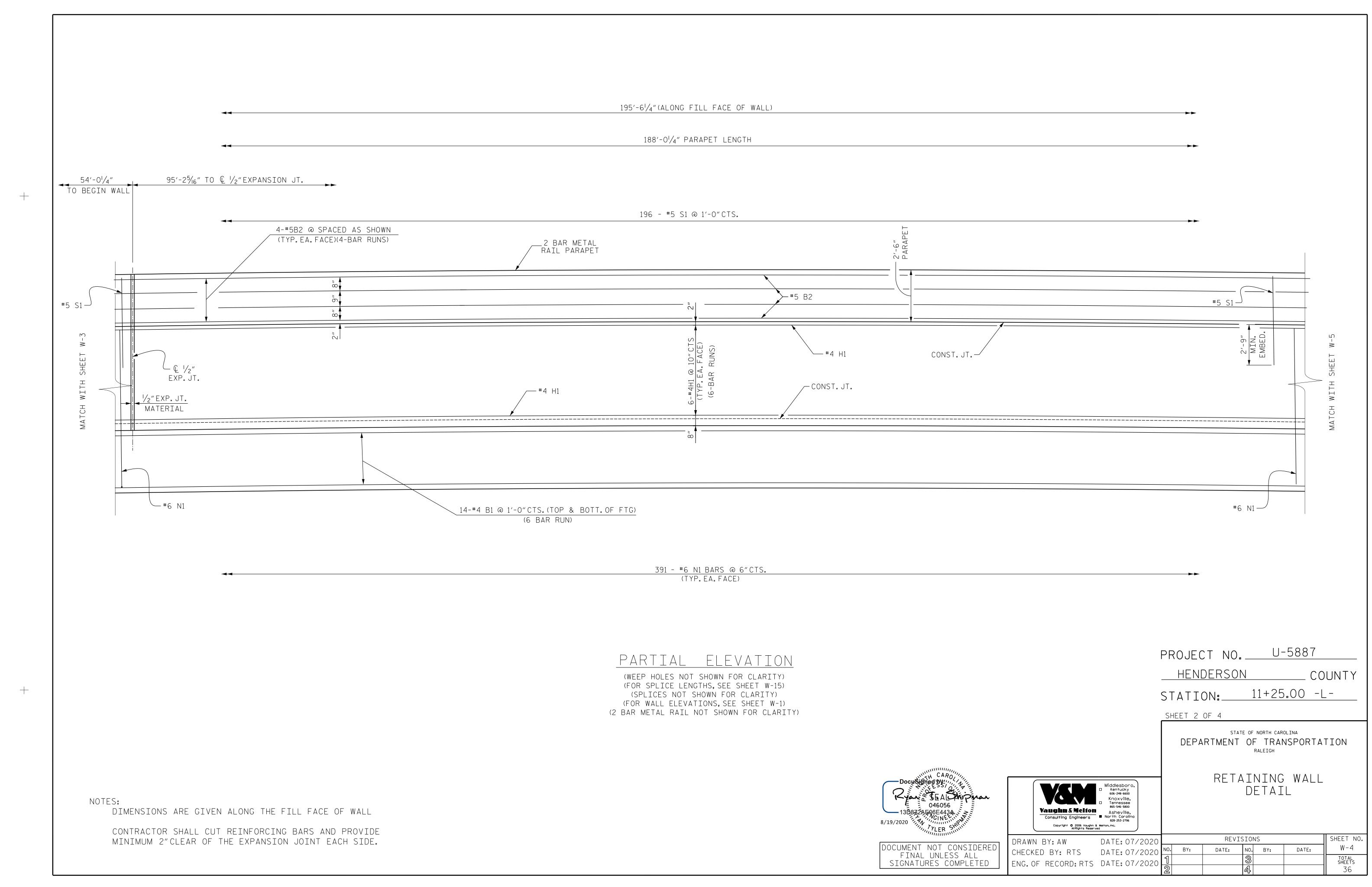
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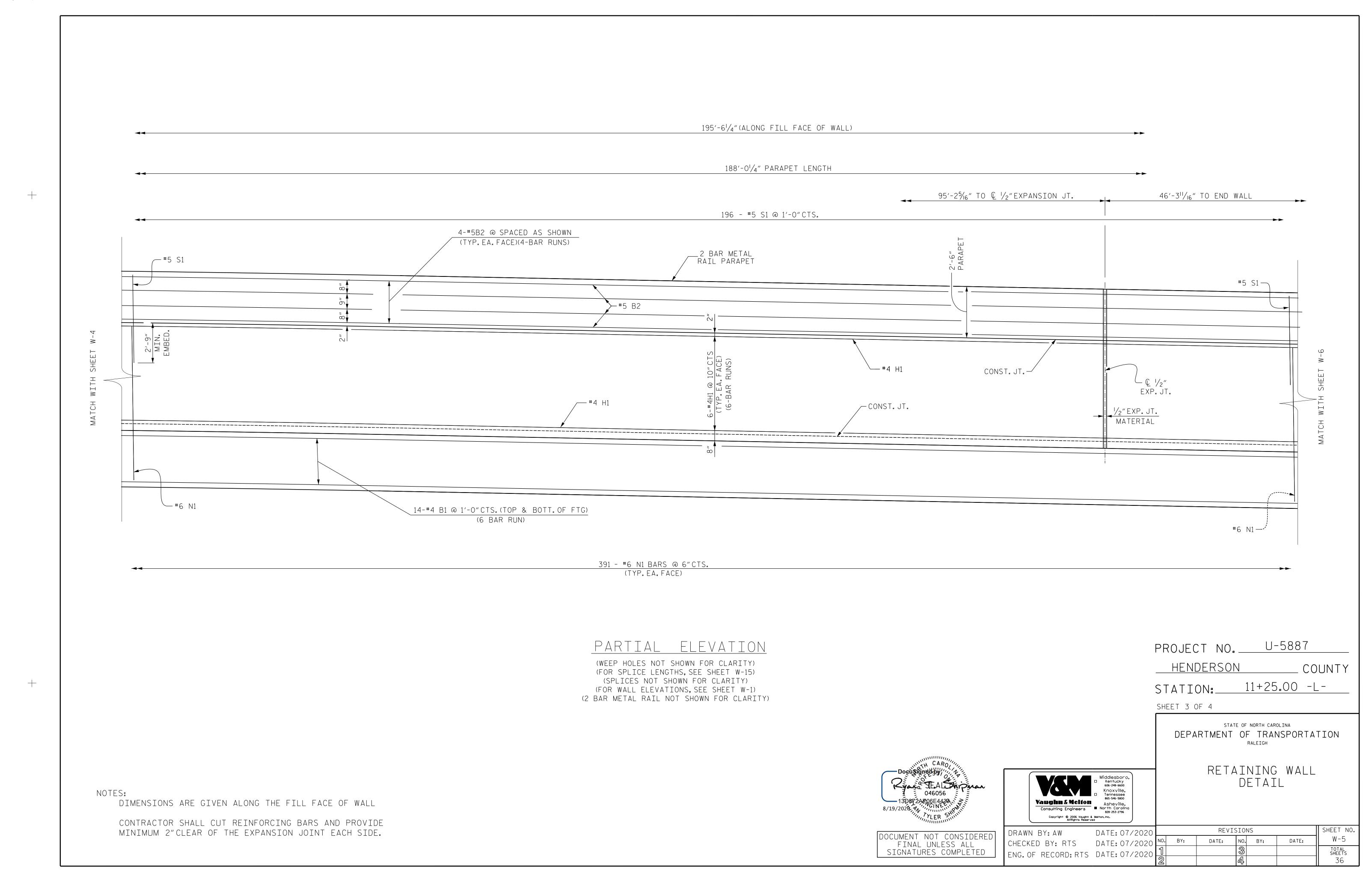
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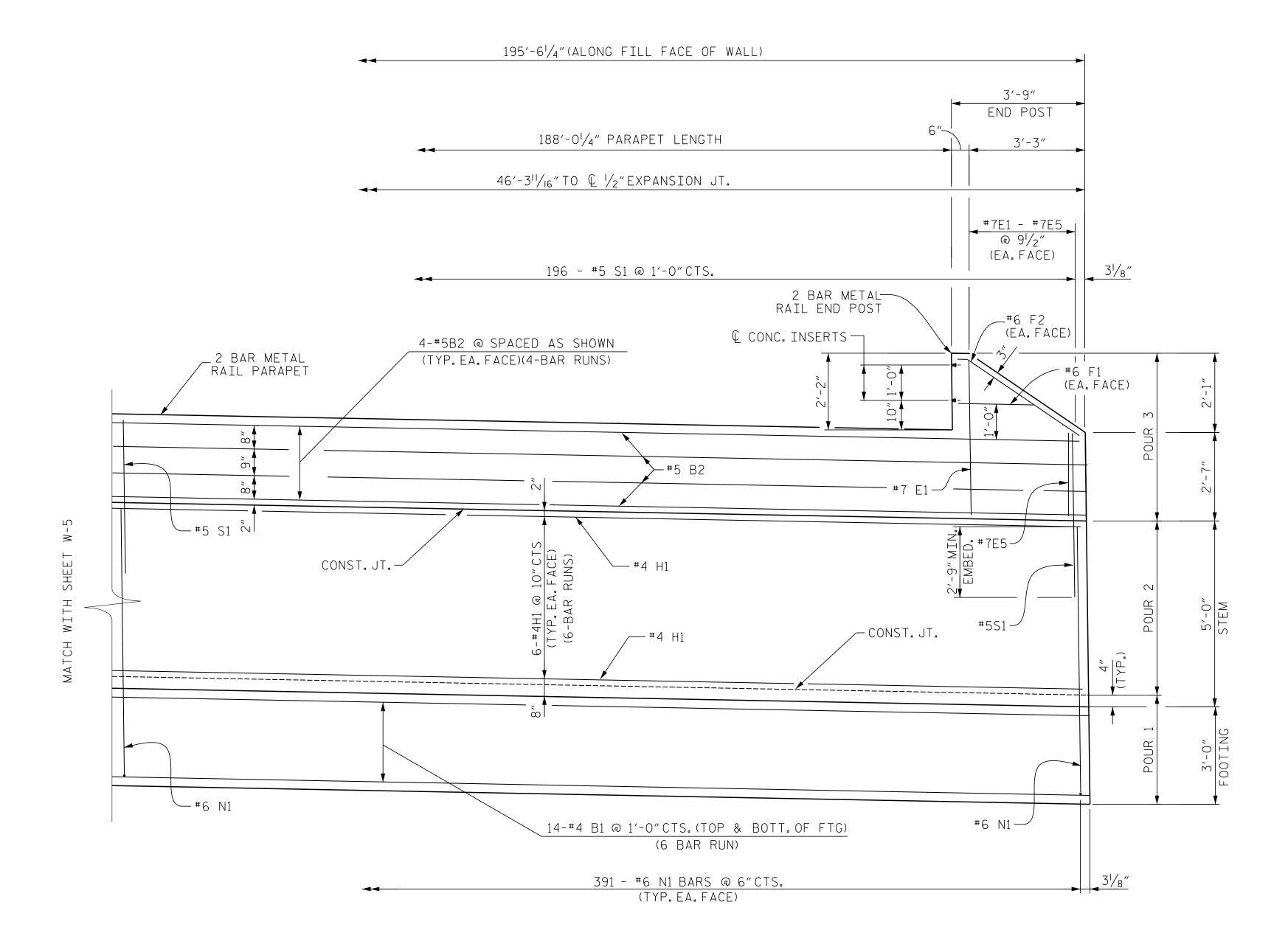
Ryan SEAMPMAN 046056

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PARTIAL ELEVATION

(WEEP HOLES NOT SHOWN FOR CLARITY)
(FOR SPLICE LENGTHS, SEE SHEET W-15)
(SPLICES NOT SHOWN FOR CLARITY)
(FOR WALL ELEVATIONS, SEE SHEET W-1)
(2 BAR METAL RAIL NOT SHOWN FOR CLARITY)

Docustoned by CAROLINA CAROLIN

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HENDERSON COUNTY

STATION: 11+25.00 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

RETAINING WALL DETAILS

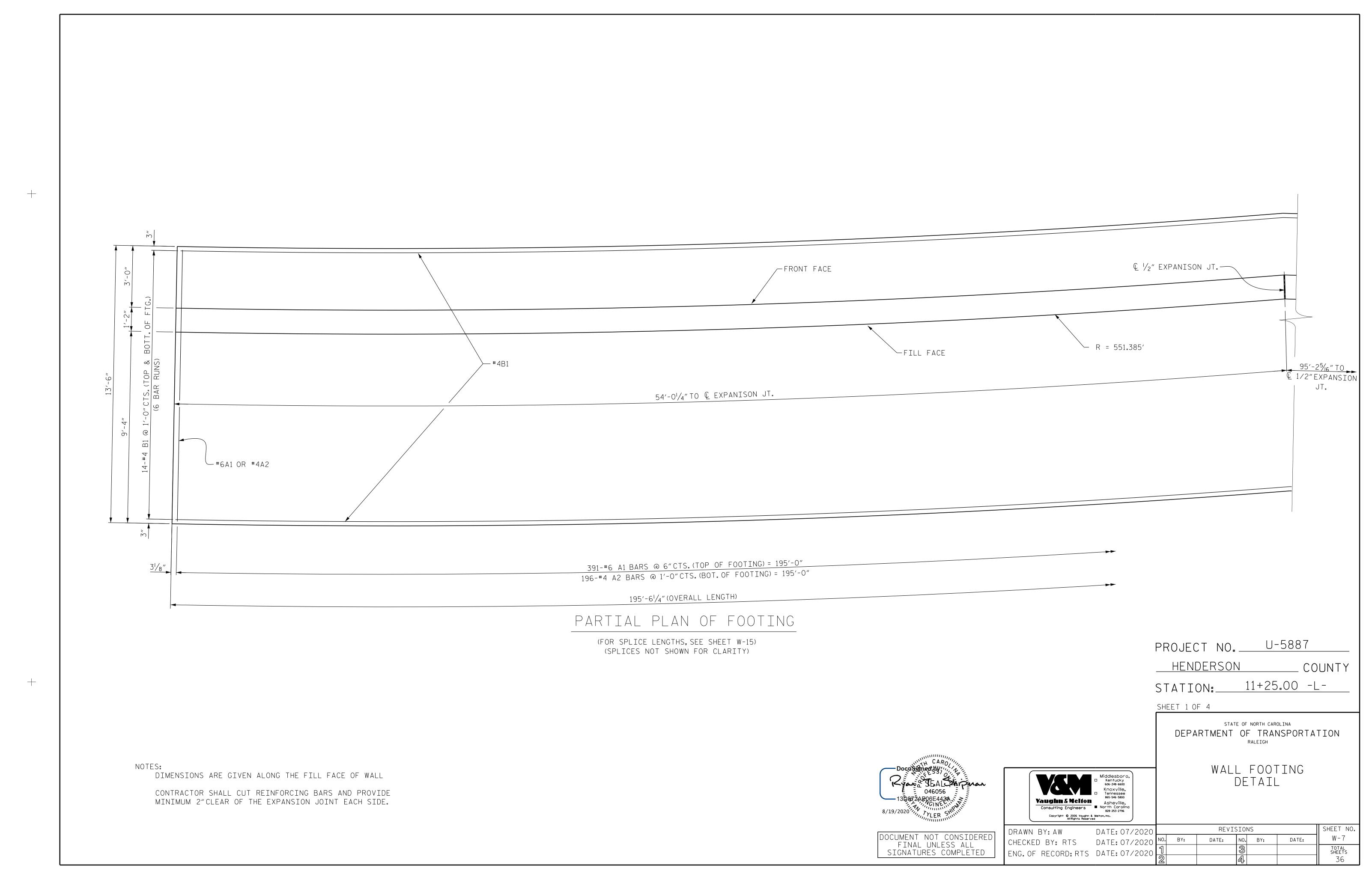
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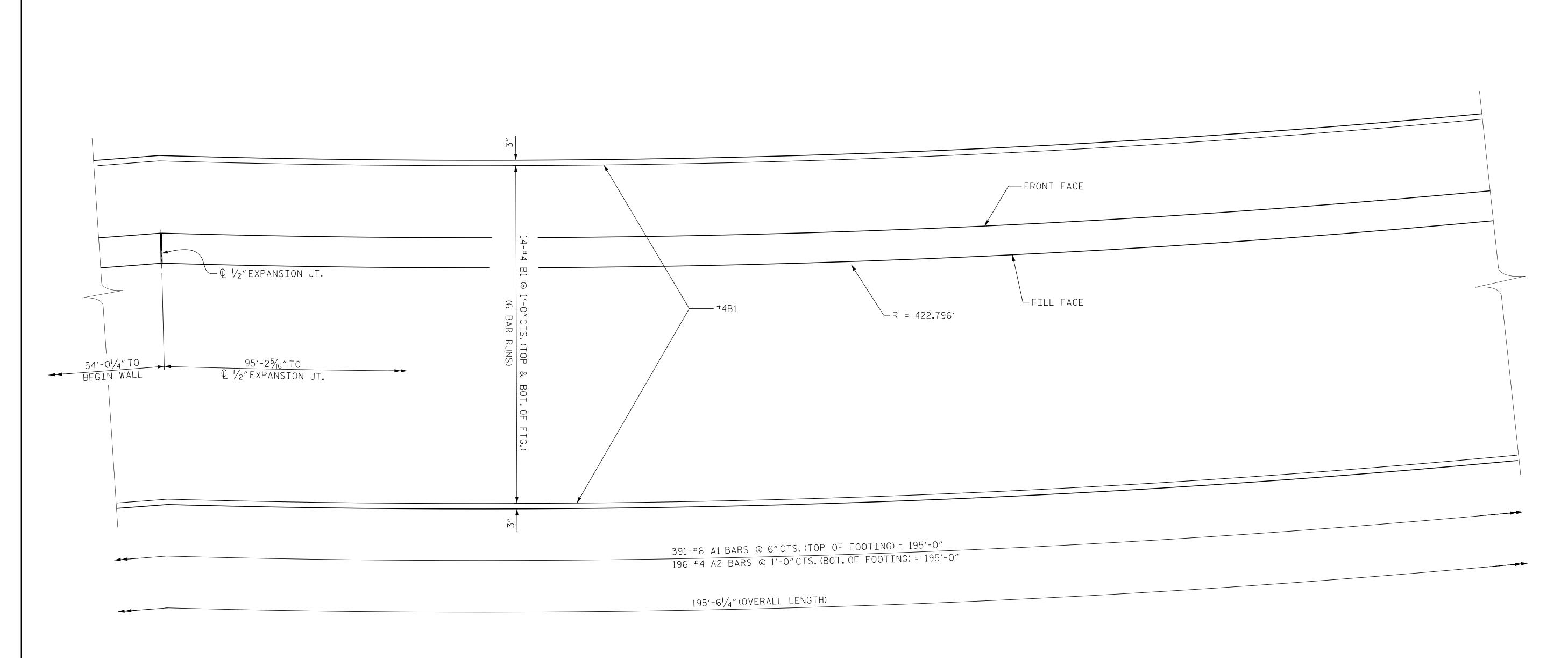
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NOTES:

DIMENSIONS ARE GIVEN ALONG THE FILL FACE OF WALL

CONTRACTOR SHALL CUT REINFORCING BARS AND PROVIDE MINIMUM 2"CLEAR OF THE EXPANSION JOINT EACH SIDE.





PARTIAL PLAN OF FOOTING

(FOR SPLICE LENGTHS, SEE SHEET W-15) (SPLICES NOT SHOWN FOR CLARITY)

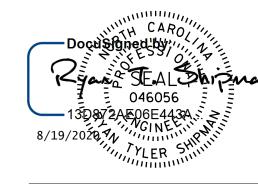
PROJECT NO. U-5887 <u>HENDERSON</u> _ COUNTY

11+25.00 -L-STATION:_

SHEET 2 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> WALL FOOTING DETAIL



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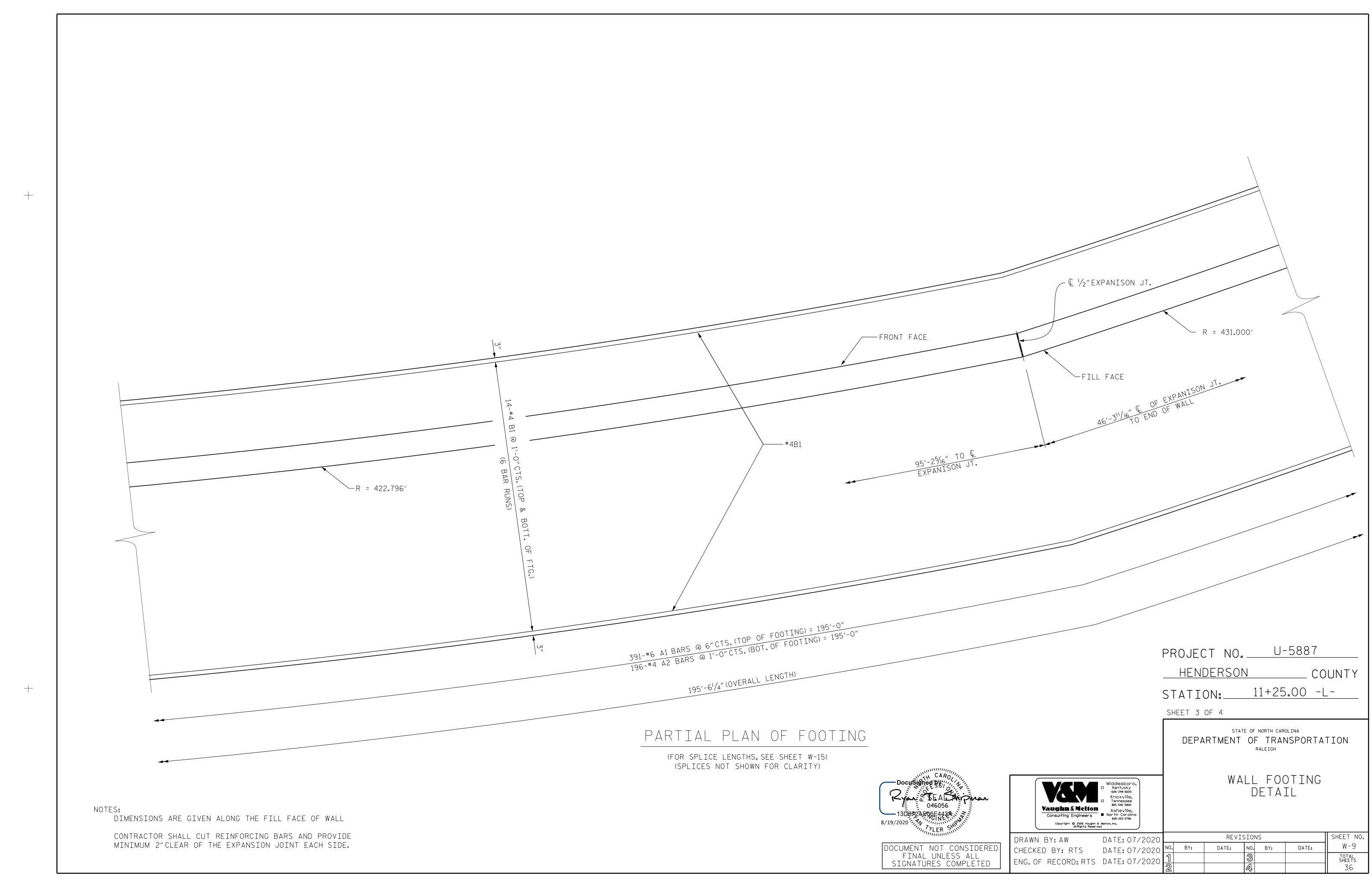
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NOTES:

DIMENSIONS ARE GIVEN ALONG THE FILL FACE OF WALL

CONTRACTOR SHALL CUT REINFORCING BARS AND PROVIDE MINIMUM 2"CLEAR OF THE EXPANSION JOINT EACH SIDE.



#6A1 OR #4A2-FRONT FACE — #4 B1 — R = 431.000′

PARTIAL PLAN OF FOOTING

(FOR SPLICE LENGTHS, SEE SHEET W-15) (SPLICES NOT SHOWN FOR CLARITY)

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WALL FOOTING DETAIL

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

11+25.00 -L-

COUNTY

PROJECT NO. U-5887

<u>HENDERSON</u>

STATION:_

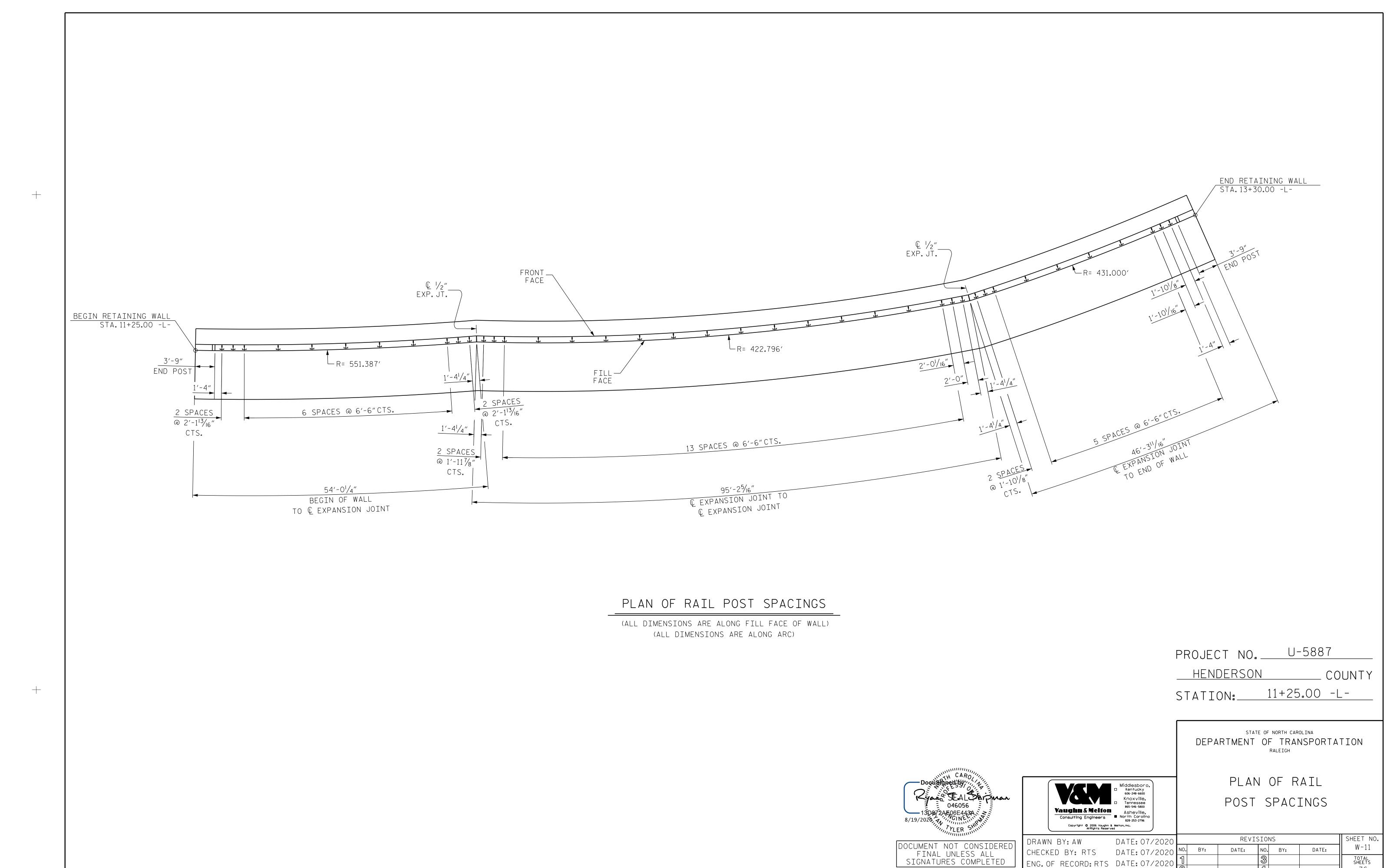
SHEET 4 OF 4

SHEET NO. REVISIONS DATE: 07/2020 DRAWN BY: AW W-10 DATE: DATE: CHECKED BY: RTS DATE: 07/2020 BY: TOTAL SHEETS 36 ENG. OF RECORD: RTS DATE: 07/2020

NOTES:

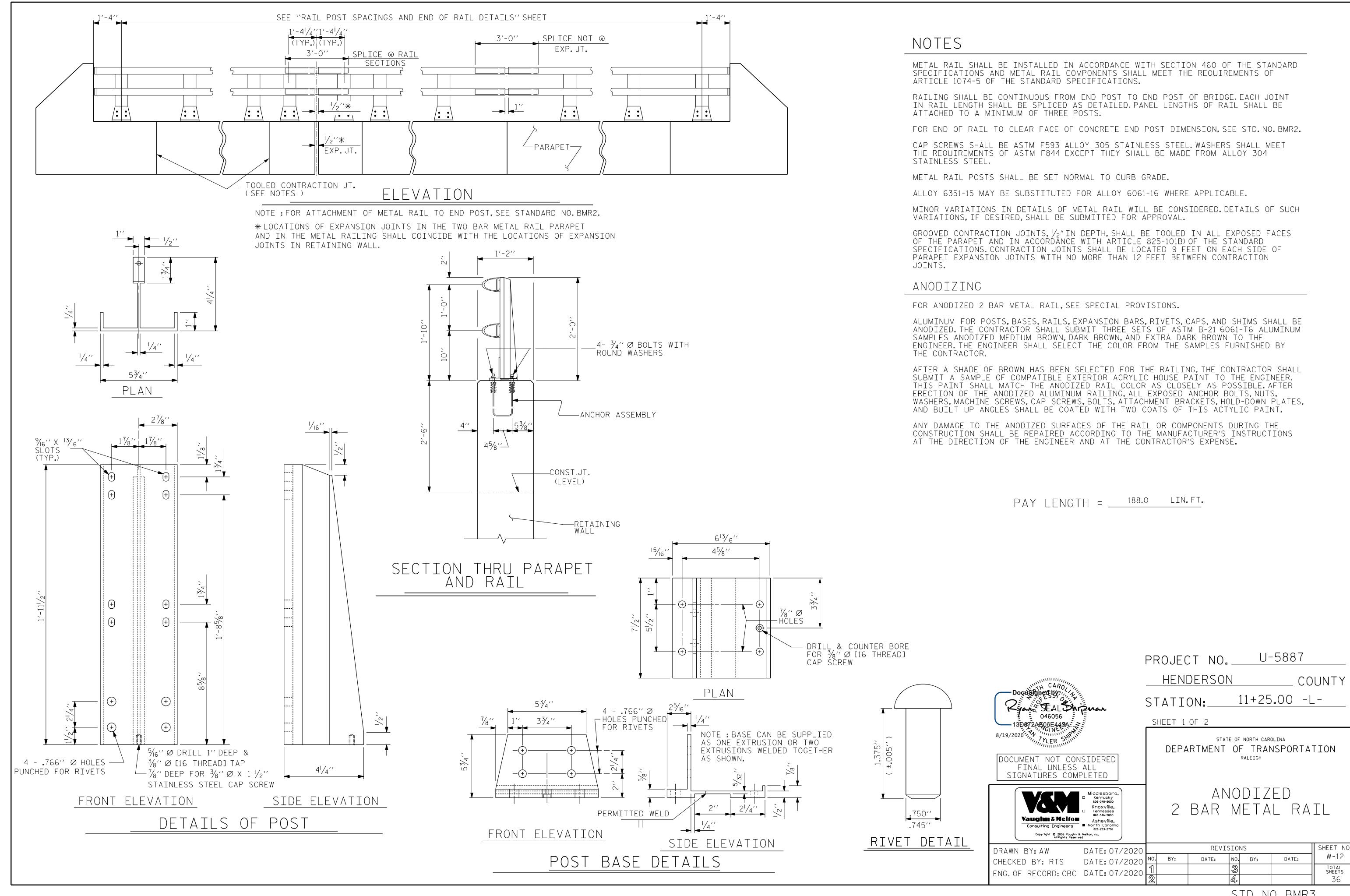
DIMENSIONS ARE GIVEN ALONG THE FILL FACE OF WALL

CONTRACTOR SHALL CUT REINFORCING BARS AND PROVIDE MINIMUM 2"CLEAR OF THE EXPANSION JOINT EACH SIDE.



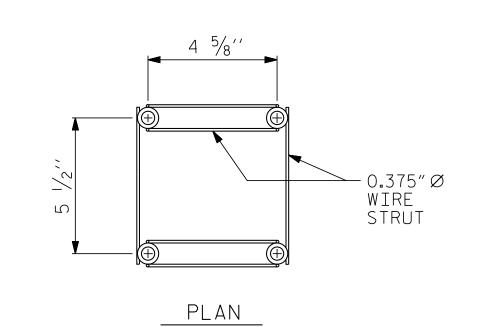
TOTAL SHEETS 36

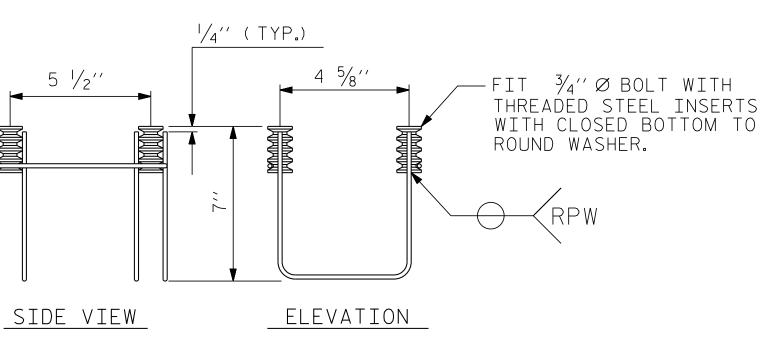
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MAA/GM





4-BOLT METAL RAIL ANCHOR ASSEMBLY

(39 ASSEMBLIES REQUIRED)

NOTES

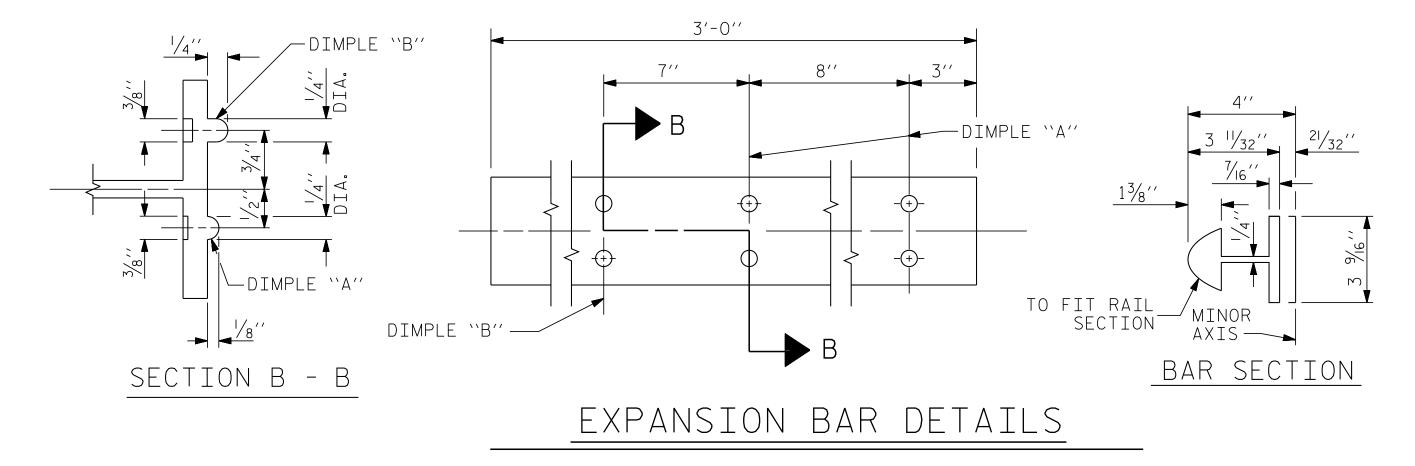
STRUCTURAL CONCRETE ANCHOR ASSEMBLY

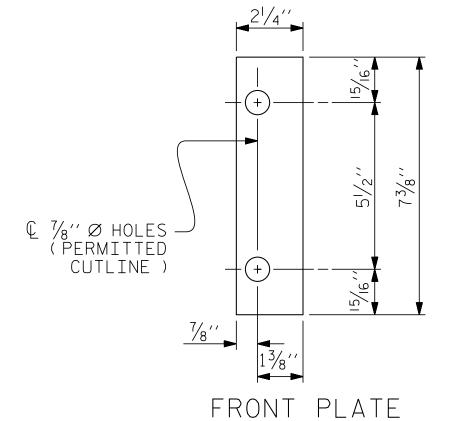
THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 $\frac{3}{4}$ " \varnothing X $2\frac{1}{2}$ " Bolts with Washers. Bolts shall conform to the REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{3}{4}$ " \varnothing X $2\frac{1}{2}$ " GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A $\frac{7}{16}$ " \varnothing wire strut with a minimum tensile STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

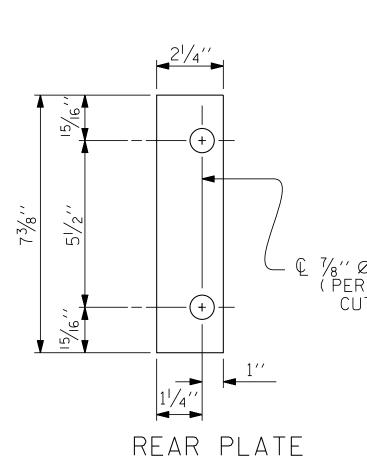
THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE $\frac{3}{4}$ " \varnothing BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE THE STANDARD SPECIFICATIONS.

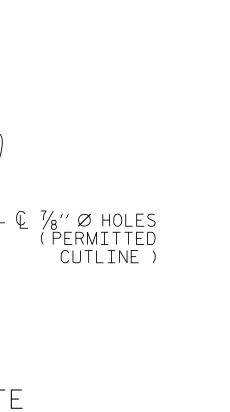
WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

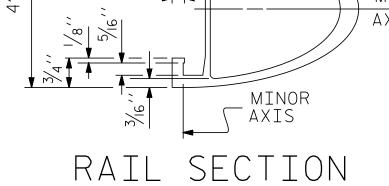




CLAMP ASSEMBLY







PROJECT NO._

HENDERSON

STATION:__

SHEET 2 OF 2

NOTE : Shims may be cut along permitted cutline or SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.

RAIL CAP

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

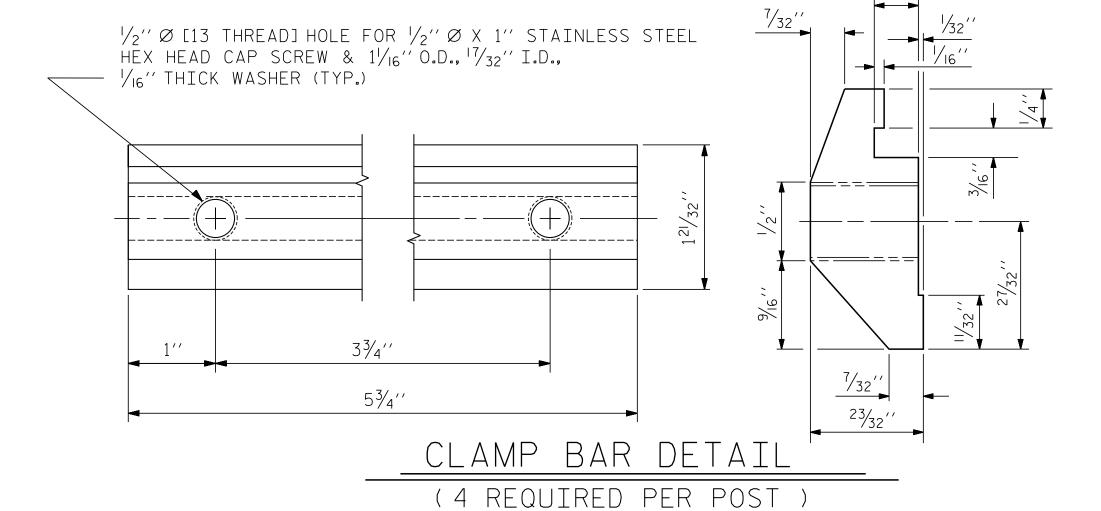
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COUNTY

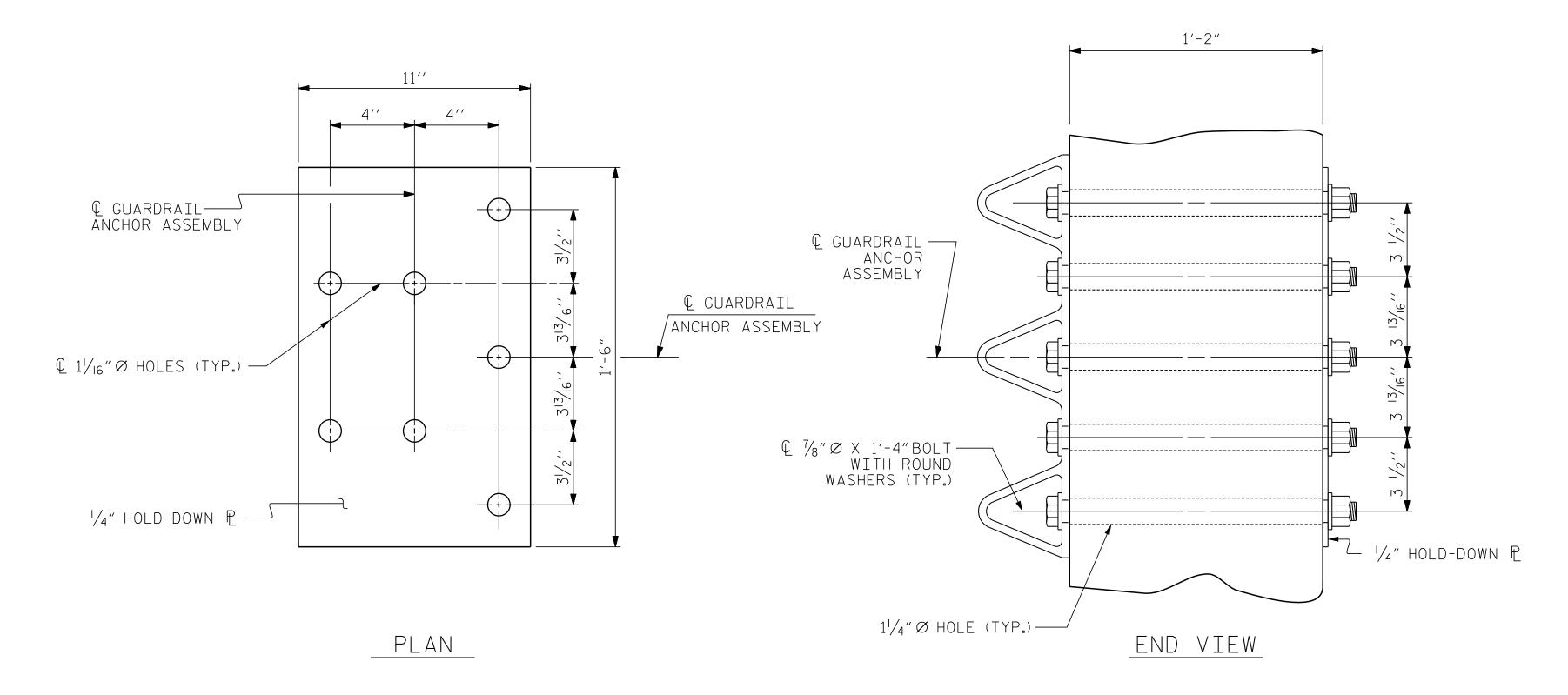
2 BAR METAL RAIL



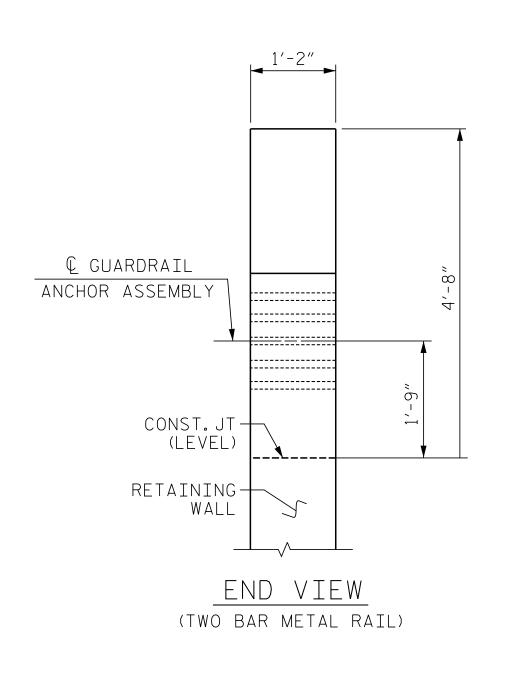
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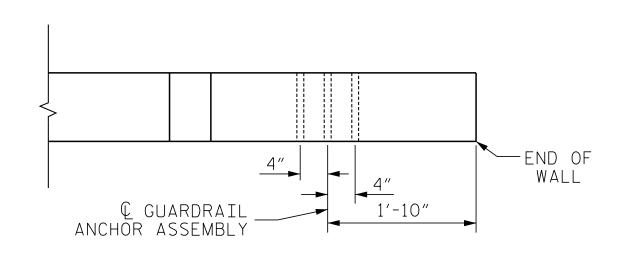
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GUARDRAIL ANCHOR ASSEMBLY DETAILS





PLAN (FOOTING NOT SHOWN FOR CLARITY)

LOCATION OF GUARDRAIL ANCHOR AT END POST

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 1/8" Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

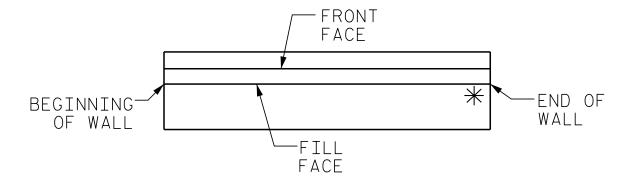
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

* LOCATION OF GUARDRAIL ATTACHMENT

U-5887 PROJECT NO._ HENDERSON

11+25.00 -L-STATION:

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COUNTY

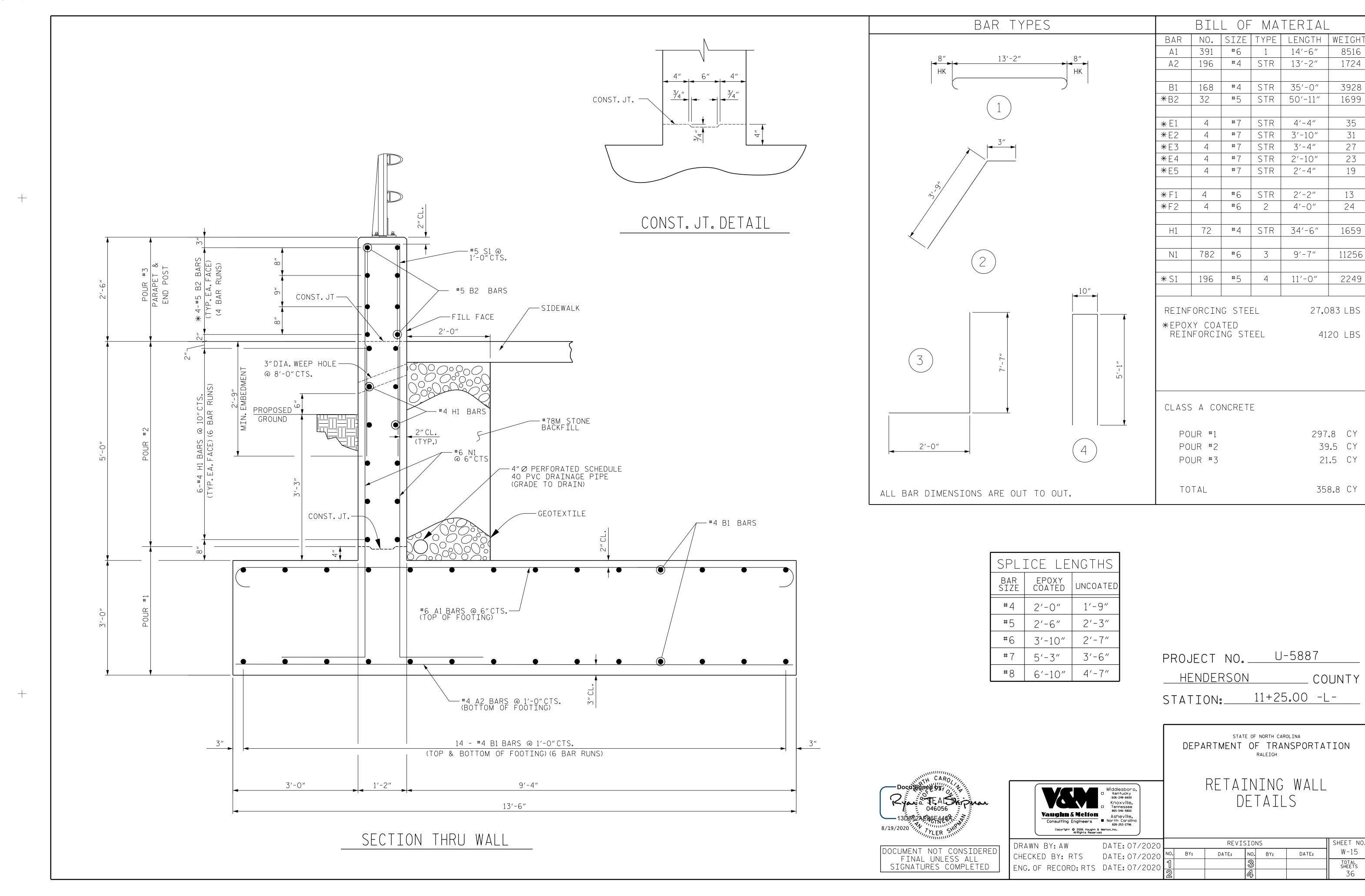
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GUARDRAIL ANCHORAGE DETAILS FOR METAL RAILS

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STANDARD NOTES

DESIGN DATA:

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

<u>ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:</u>

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY /16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

Christopher B. Cordell
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9/2/2020

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990