Stowe Rd. 3

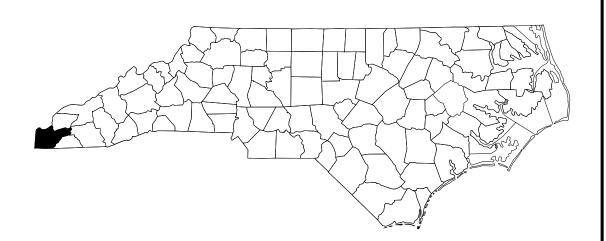
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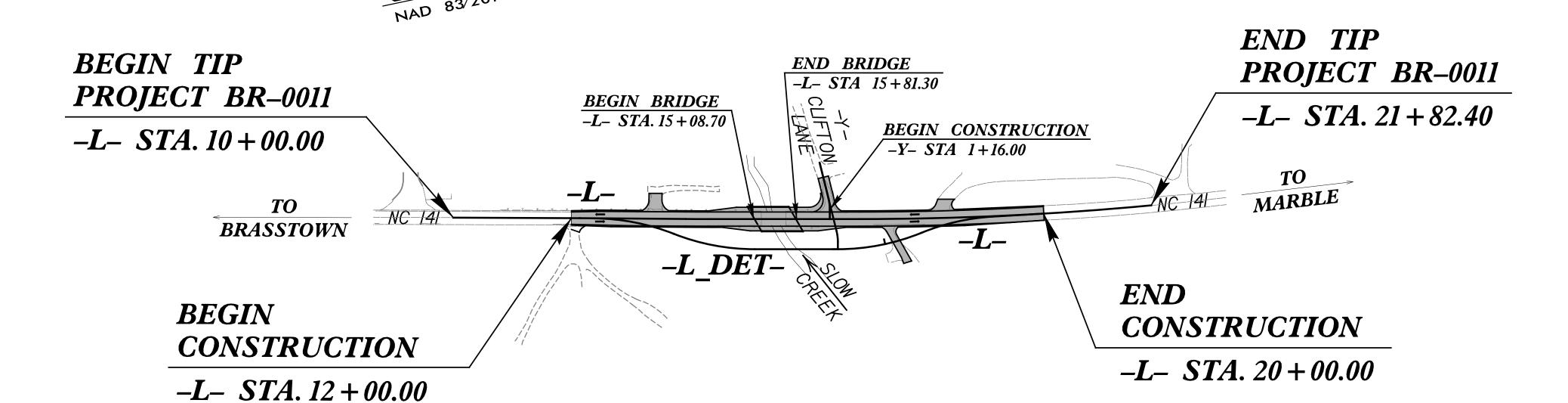
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# CHEROKEE COUNTY

LOCATION: BRIDGE 190002 ON NC141 OVER SLOW CREEK TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE

STATE	STATE	PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	E	3R-0011	1	
STAT	TE PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	TION
WBS	67011.1.1		PE	
WBS	67011.2.1		ROW,	UTL.
WBS	67011.3.1		CON	ST.





# STRUCTURE

See Sheet 1-A For Index of Sheets See Sheet 1-B For Conventional Symbols

Royal Oaks

Robert Barker

VICINITY MAP

PROJECT

SITE

Upper Peachtree Rd.

Hendrix Rd.

Greenlawn Cemetary Rd.

**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

# DESIGN DATA

ADT 2018 = 4200 VPDADT 2040 = 4800 VPD

> K = 9 %D = 55 %

V = 50 MPH\* TTST = 1% DUAL 5%

FUNC CLASS = MAJOR COLLECTOR REGIONAL TIER

# PROJECT LENGTH

= 0.210 MI.LENGTH OF ROADWAY TIP PROJECT BR-0011 LENGTH OF STRUCTURE TIP PROJECT BR-0011 = 0.014 MI.= 0.224 MI.TOTAL LENGTH OF TIP PROJECT BR-0011

2018 STANDARD SPECIFICATIONS MARK COLGAN, PE PROJECT ENGINEER LETTING DATE: APRIL 20, 2021 ELIZABETH LAWES, PE

Prepared for the North Carolina Department of Transportation

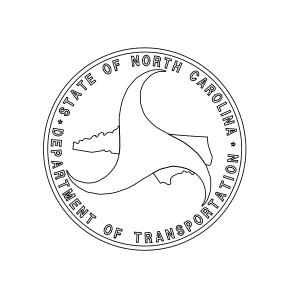
In the Office of:

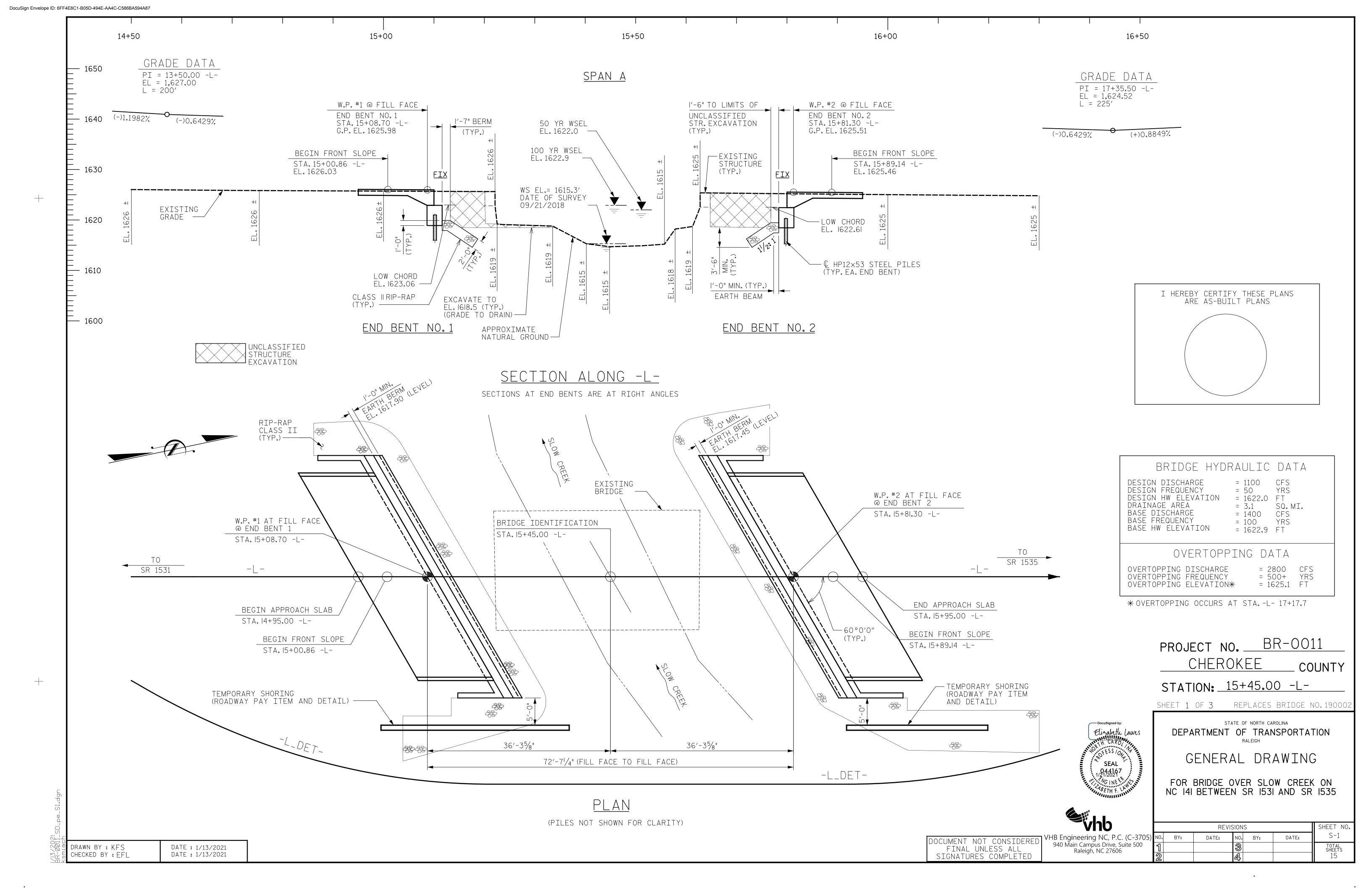
PROJECT DESIGN ENGINEER

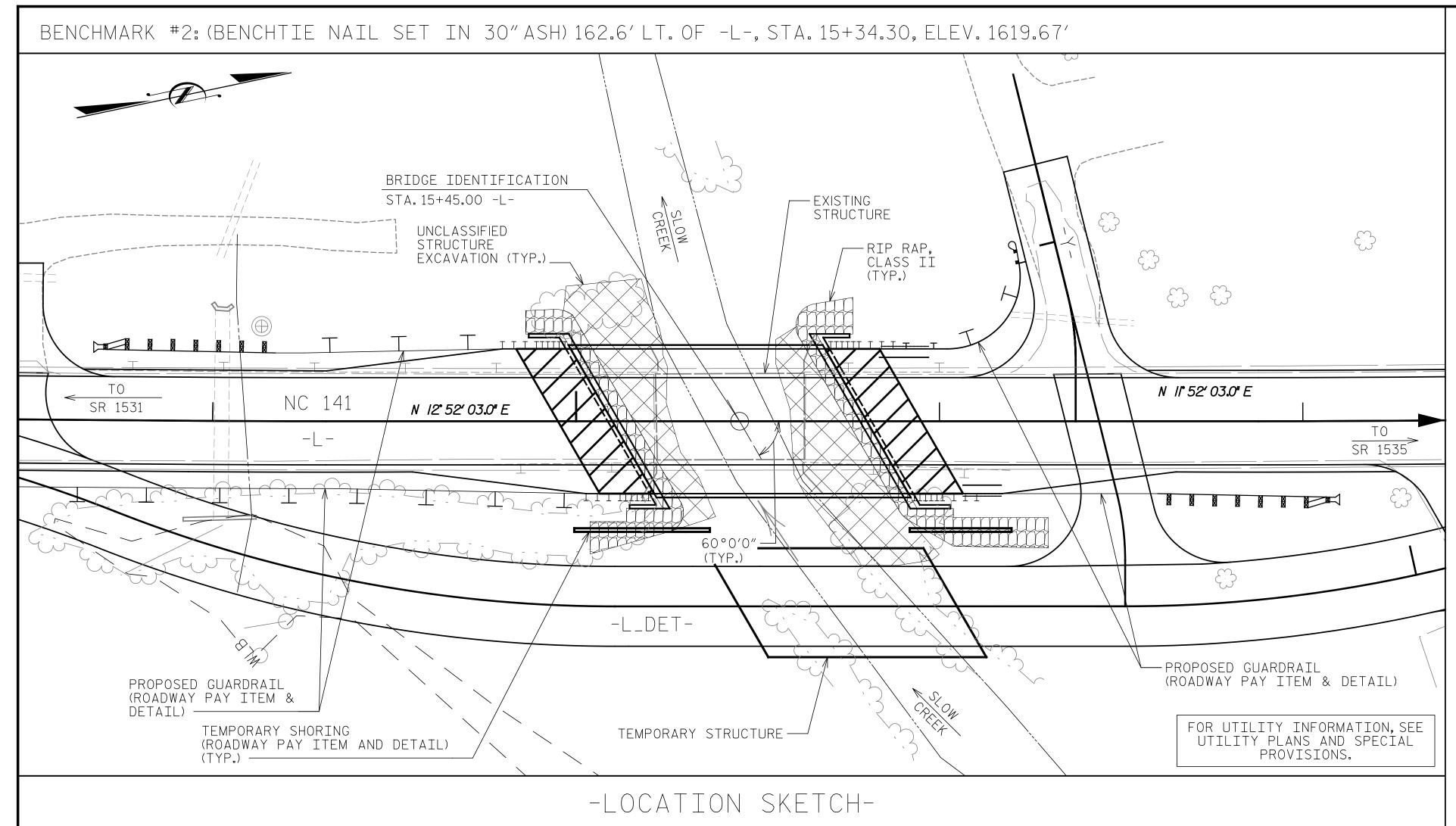
NCDOT CONTACT DAVID S. STUTTS, PE PROJECT ENGINEER-ROADWAY DESIGN



ELIZABETH LAWES, PE







	CONSTRUCTION MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURE	REMOVAL OF EXISTING STRUCTURE	ASBESTOS ASSESSMENT	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CONCRETE WEARING SURFACE	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL		
	LUMP SUM	LUMP SUM	LUMP SUM	NO.	LUMP SUM	SQ.FT.	SQ.FT.	CU. YDS.	LUMP SUM	LBS.		
SUPERSTRUCTURE						2789	3646					
END BENT 1								29.8		3680		
END BENT 2								29.8		3680		
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	1	LUMP SUM	2789	3646	59.6	LUMP SUM	7360		

		$T \bigcirc$	TAL	BILL	_ OF	MATER	IAL			
	PILE DRIVING EQUIPMENT SET UP FOR HP 12 X 53 STEEL PILES		12 X 53 EL PILES	STEEEL VERTICAL CONCRETE BARRIER RAIL		RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0" X 2'-0" PRESTRESSED CONCRETE CORED SLABS	
	NO.	NO.	LIN.FT.	NO.	LIN. FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN. FT.
SUPERSTRUCTURE					140.29				14	980.00
END BENT 1	7	7	210	7		71	53			
END BENT 2	7	7	175	7		90	67			
TOTAL	14	14	385	14	140.29	161	120	LUMP SUM	14	980.00

GENERAL NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES. SEE EROSION CONTROL PLANS.

THE EXISTING SINGLE SPAN BRIDGE, WITH SPAN OF 40'-6", WITH A CLEAR ROADWAY WIDTH OF 24'-6", WITH A STEEL PLANK FLOOR ON (11) LINES OF STEEL I-BEAMS, WITH SUBSTRUCTURES CONSISTING OF TIMBER CAP/TIMBER POSTS LOCATED AT THE PROPOSED BRIDGE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE".

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. THE INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 40 FT (LEFT) AND 30 FT (RIGHT) AT END BENT No. 1 AND 26 FT (LEFT) AND 40 FT (RIGHT) AT END BENT No. 2. OF THE CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

TEMPORARY SHORING WILL BE REQUIRED IN THE AREAS INDICATED IN THE PLAN VIEW, SHEET 1 OF 3.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

FOR CONCRETE WEARING SURFACE, SEE SPECIAL PROVISIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES".

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK. SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

OCUMENT NOT CONSIDERED

FINAL UNLESS ALL

SIGNATURES COMPLETED

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATIONS ACTIVITIES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE A TEMPORARY STRUCTURE AT STATION 53+43.00 -L\_DET- FOR USE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE.FOR CONSTRUCTION, MAINTENANCE, AND REMOVAL OF TEMPORARY STRUCTURE, SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, PRESTRESSED CONCRETE END BENT AND BENT CAPS MAY BE SUBSTITUTED IN PLACE OF THE CAST-IN-PLACE CAPS. THE CONTRACTOR SHALL COORDINATE WITH THE RESIDENT ENGINEER TO RECEIVE REVISED PLANS AND DETAILS FROM THE STRUCTURES MANAGEMENT UNIT. THE REDESIGN AND ANY ADDITIONAL MATERIALS NEEDED WILL BE AT NO ADDITIONAL COST TO THE CONTRACTOR.

PLACEMENT AND REMOVAL OF RIP RAP, CLASS II AND GEOTEXTILE FOR DRAINAGE FOR THE TEMPORARY STRUCTURE SHALL BE INCIDENTAL TO ITEM "CONSTRUCTION. MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURE AT STATION 53+43.00 -L\_DET-". SEE ROADWAY PLANS FOR LIMITS.

> PROJECT NO. BR-0011 CHEROKEE \_ COUNTY

STATION: 15+45.00 -L-

SHEET 3 OF 3 REPLACES BRIDGE NO.19000

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

FOR BRIDGE OVER SLOW CREEK ON NC 141 BETWEEN SR 1531 AND SR 1535



044167

<b>Vinb</b>
HB Engineering NC, P.C. (C-3705
940 Main Campus Drive, Suite 500
Raleigh, NC 27606

ı							_
		SHEET NO					
)	NO.	BY:	DATE:	NO.	BY:	DATE:	S-3
	1			3			TOTAL SHEETS
I	2			4			15

DATE: 1/21/2021 DATE: 1/21/2021

DRAWN BY : KFS

CHECKED BY : EFL

# LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

								STRENGTH I LIMIT STATE									SERVICE III LIMIT STATE							
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.06		1.75	0.248	1.14	70′	ER	34.423	0.655	1.06	70′	ER	6.885	0.80	0.248	1.11	70′	ER	34.423	
DESIGN	-	HL-93(0pr)	N/A		1.374		1.35	0.248	1.48	70′	ER	34.423	0.655	1.37	70′	ER	6.885	N/A						
LOAD RATING	-	HS-20(Inv)	36.000	2	1.32	47.508	1.75	0.248	1.48	70′	ER	34.423	0.655	1.32	70′	ER	6.885	0.80	0.248	1.44	70′	ER	34.423	
NATINO		HS-20(0pr)	36.000		1.711	61.585	1.35	0.248	1.91	70′	ER	34.423	0.655	1.71	70′	ER	6.885	N/A						
		SNSH	13.500		3.204	43.258	1.4	0.248	4.12	70′	ER	34.423	0.655	3.9	70′	ER	6.885	0.80	0.248	3.20	70′	ER	34.423	
		SNGARBS2	20.000		2.403	48.063	1.4	0.248	3.09	70′	ER	34.423	0.655	2.78	70′	ER	6.885	0.80	0.248	2.40	70′	ER	34.423	
		SNAGRIS2	22,000		2.282	50.21	1.4	0.248	2.94	70′	ER	34.423	0.655	2.58	70′	ER	6.885	0.80	0.248	2.28	70′	ER	34,423	
		SNCOTTS3	27.250		1.595	43.463	1.4	0.248	2.05	70′	ER	34.423	0.655	1.95	70′	ER	6.885	0.80	0.248	1.59	70′	ER	34.423	
	S	SNAGGRS4	34.925		1.339	46.755	1.4	0.248	1.72	70′	ER	34.423	0.655	1.62	70′	ER	6.885	0.80	0.248	1.34	70′	ER	34,423	
		SNS5A	35.550		1.309	46.526	1.4	0.248	1.68	70′	ER	34.423	0.655	1.65	70′	ER	6.885	0.80	0.248	1.31	70′	ER	34,423	
		SNS6A	39.950		1.203	48.069	1.4	0.248	1.55	70′	ER	34.423	0.655	1.5	70′	ER	6.885	0.80	0.248	1.20	70′	ER	34.423	
LEGAL		SNS7B	42.000		1.146	48.129	1.4	0.248	1.47	70′	ER	34.423	0.655	1.48	70′	ER	6.885	0.80	0.248	1.15	70′	ER	34.423	
LOAD RATING		TNAGRIT3	33.000		1.468	48.444	1.4	0.248	1.89	70′	ER	34.423	0.655	1.79	70′	ER	6.885	0.80	0.248	1.47	70′	ER	34.423	
I MATERIO	-	TNT4A	33.075		1.475	48.79	1.4	0.248	1.9	70′	ER	34.423	0.655	1.74	70′	ER	6.885	0.80	0.248	1.48	70′	ER	34.423	
		TNT6A	41.600		1.208	50.272	1.4	0.248	1.55	70′	ER	34.423	0.655	1.58	70′	ER	6.885	0.80	0.248	1.21	70′	ER	34,423	
		TNT7A	42,000		1.216	51.061	1.4	0.248	1.56	70′	ER	34,423	0.655	1.55	70′	ER	6.885	0.80	0.248	1.22	70′	ER	34.423	
		TNT7B	42.000		1.261	52.955	1.4	0.248	1.62	70′	ER	34.423	0.655	1.44	70′	ER	6.885	0.80	0.248	1.26	70′	ER	34.423	
		TNAGRIT4	43.000		1.197	51.476	1.4	0.248	1.54	70′	ER	34.423	0.655	1.4	70′	ER	6.885	0.80	0.248	1.20	70′	ER	34.423	
		TNAGT5A	45.000		1.128	50.745	1.4	0.248	1.45	70′	ER	34.423	0.655	1.39	70′	ER	6.885	0.80	0.248	1.13	70′	ER	34.423	
		TNAGT5B	45.000	3	1.113	50.088	1.4	0.248	1.43	70′	ER	34.423	0.655	1.33	70′	ER	6.885	0.80	0.248	1.11	70′	ER	34.423	

# LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{ extsf{DC}}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

#### NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

# <u>COM</u>MENTS:

2.

7

4.

(#) CONTROLLING LOAD RATING

(1) DESIGN LOAD RATING (HL-93)

 $\langle 2 \rangle$  design load rating (HS-20)

3 LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

## GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. BR-0011

CHEROKEE COUNTY

STATION: 15+45.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD LRFR SUMMARY

LRFR SUMMARY FOR 70' CORED SLAB UNIT 60° SKEW (NON-INTERSTATE TRAFFIC)

VHB Engineering NC, P.C. (C-3705)
940 Main Campus Drive, Suite 500
Raleigh, NC 27606

REVISIONS

SHEET NO.

S-4

TOTAL
SHEETS
15

<u>1</u> <u>2</u> <u>3</u>

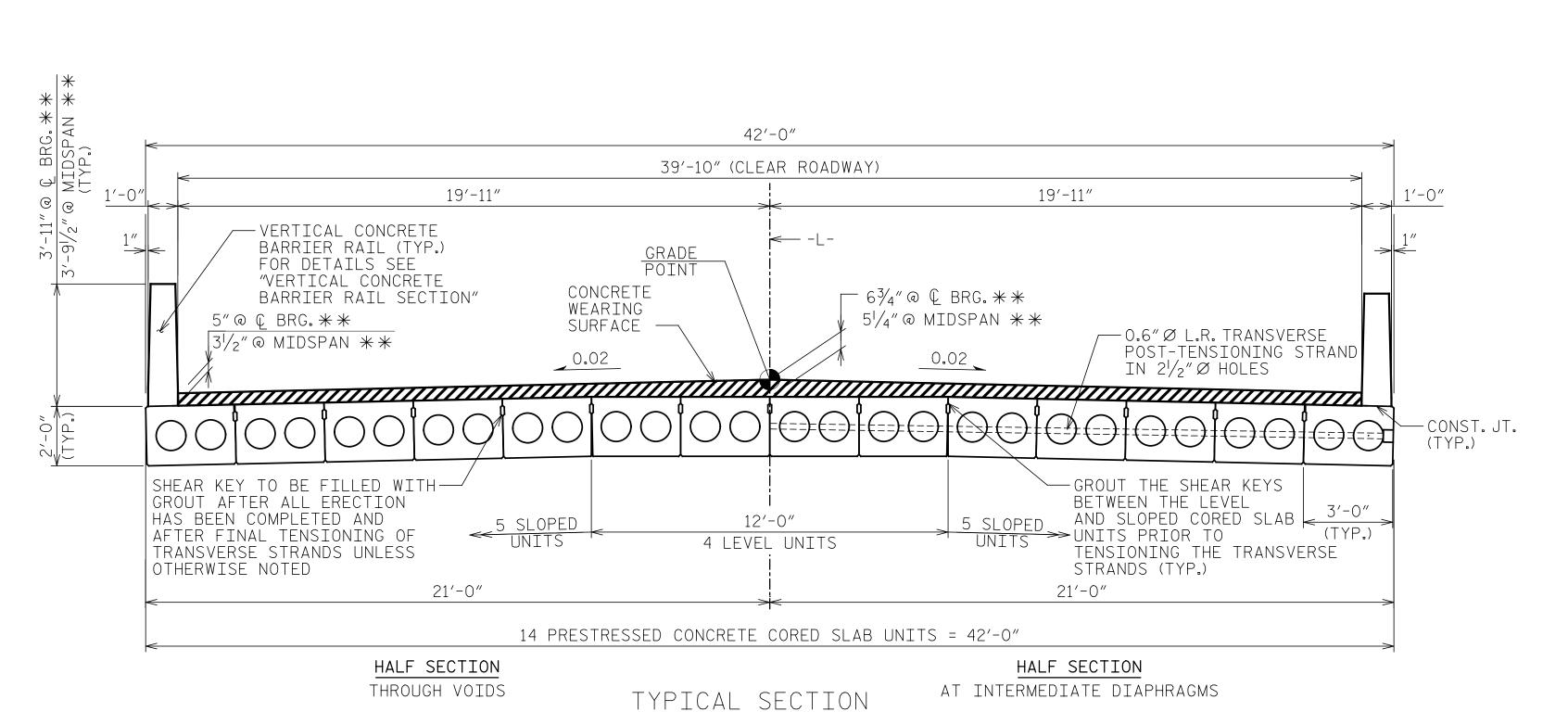
LRFR SUMMARY
FOR SPAN 'A'

ASSEMBLED BY: EFL
CHECKED BY: KFS

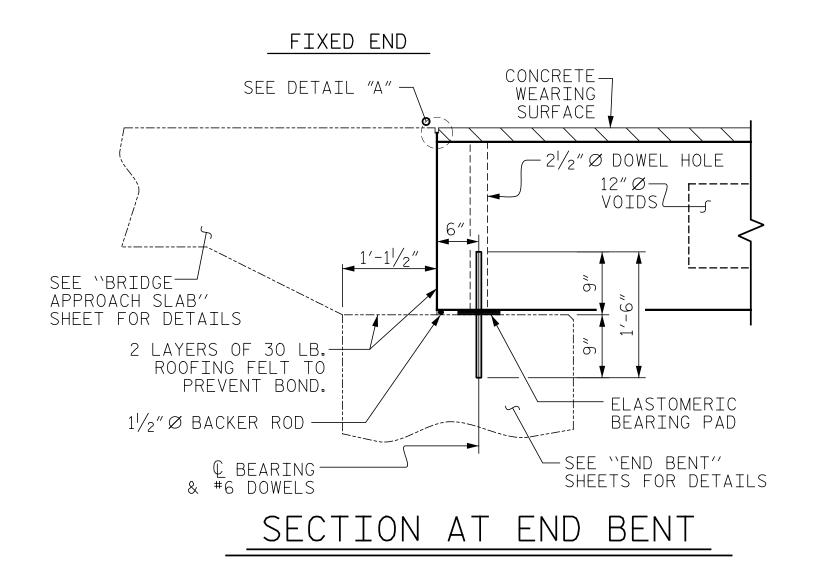
DATE: 1/13/2021
DRAWN BY: CVC 6/I0
CHECKED BY: DNS 6/I0

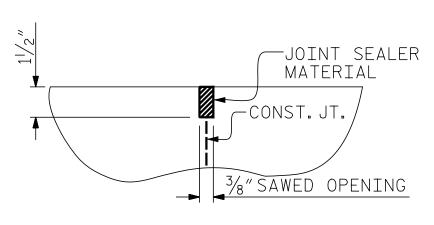
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1/13/2021 BR-0011\_SD\_LRFR\_S4.dgn ksmiach

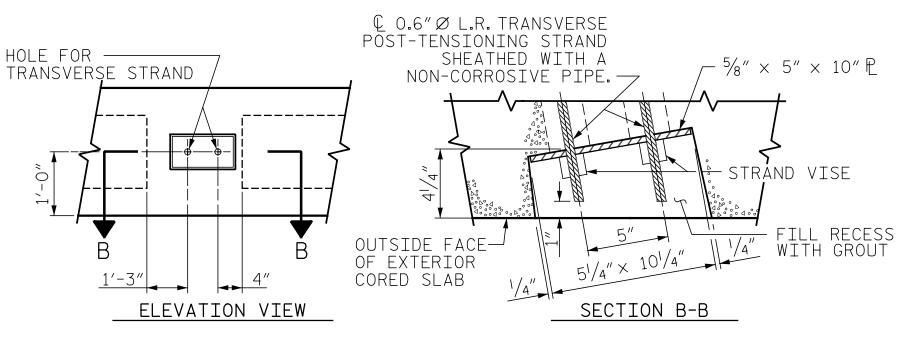


\*\* BASED ON PREDICTED FINAL CAMBER AND THEORETICAL GRADE LINE ELEVATIONS. THE CONCRETE WEARING SURFACE THICKNESS AND HEIGHT OF THE BARRIER RAIL VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE.

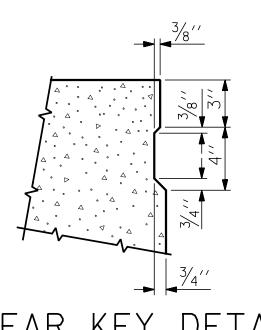




DETAIL "A" A 11/2" DEEP CONTRACTION JOINT AT EACH END BENT SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

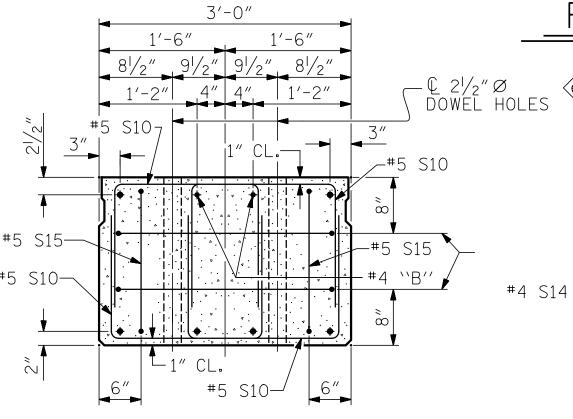


GROUTED RECESS AT END OF POST-TENSIONED STRAND CORED SLABS



DETAIL SHEAR KEY

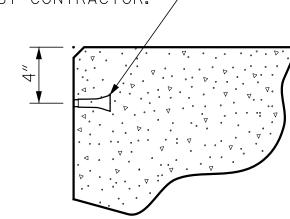
NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.



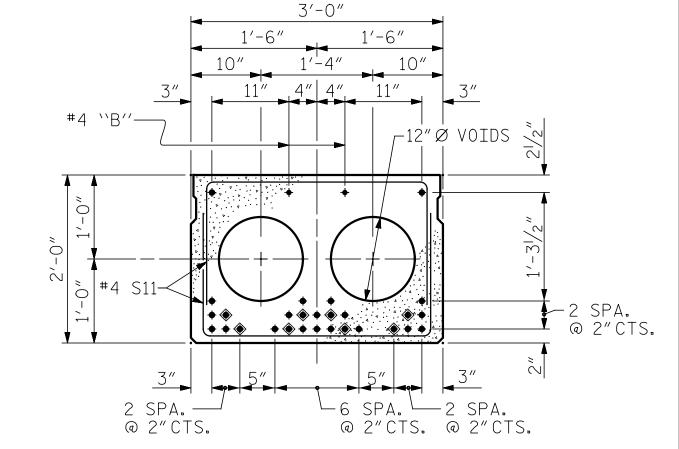
ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.) INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION,

PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF EXTERIOR UNIT AND RECESSED 3/8". SIZE TO BE DETERMINED BY CONTRACTOR. —



THREADED INSERT DETAIL

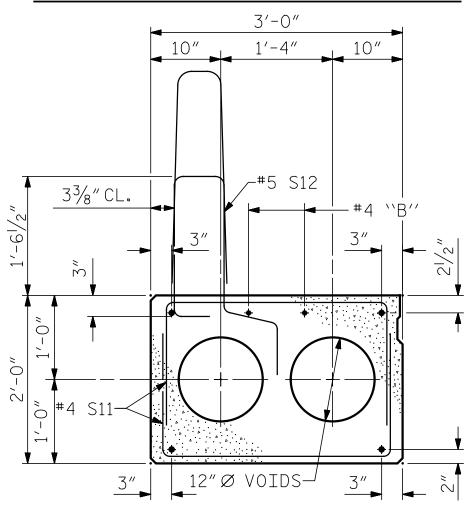


INTERIOR SLAB SECTION (70'UNIT) (28 STRANDS REQUIRED)

RELAXATION STRAND LAYOUT

- C 21/2" Ø BOND SHALL BE BROKEN ON THESE STRANDS FOR A DOWEL HOLES DISTANCE OF 12'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

# DEBONDING LEGEND



EXTERIOR SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

BR-0011 PROJECT NO.\_ CHEROKEE COUNTY

STATION: 15+45.00 -L-

SHEET 1 OF 3



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

3'-0'' X 2'-0'' PRESTRESSÉD CONCRETE CORED SLAB UNIT

SHEET NO REVISIONS VHB Engineering NC, P.C. (C-3705)

940 Main Campus Drive, Suite 500
Raleigh, NC 27606 S-5 DATE: DATE: BY: TOTAL SHEETS

OCUMENT NOT CONSIDERED FINAL UNLESS ALL

SIGNATURES COMPLETED

1/13/2021 BR-0011\_SD\_CS\_S5.dgn ksmiach

DATE: 1/13/2021

DATE: 1/13/2021

MAA/TMG

REV. 8/14

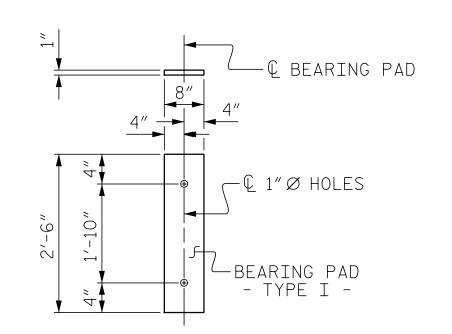
ASSEMBLED BY : EFL CHECKED BY : KFS

DRAWN BY : MAA

CHECKED BY : MKT 7/10

6/10

STD. NO. 24PCS4\_39\_60S



FIXED END (TYPE I - 28 REQ'D)

# ELASTOMERIC BEARING DETAILS

DEAD LOAD DEFLECTION AND CAMBER

70' CORED SLAB UNTT

CAMBER (SLAB ALONE IN PLACE

10/2

CONST. JT. —

REV. 5/18

6/10

SECTION THRU RAIL

MAA/THC

CONCRETE WEARING SURFACE

DEFLECTION DUE TO

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

 $3'-0'' \times 2'-0'$ 

0.6" Ø L.R.

1/2"

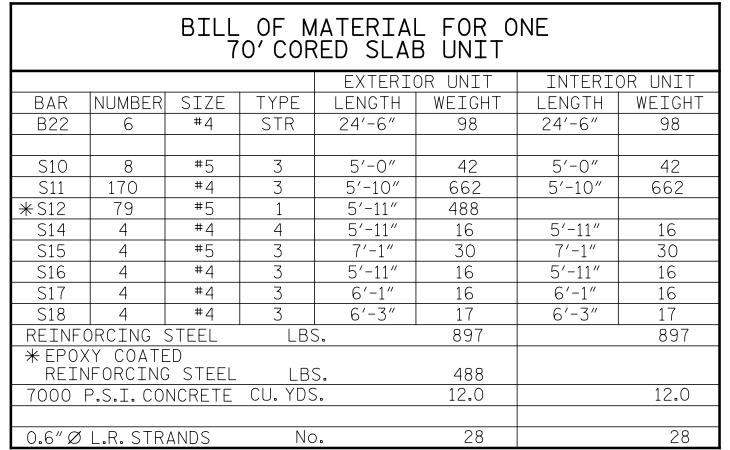
STRAND

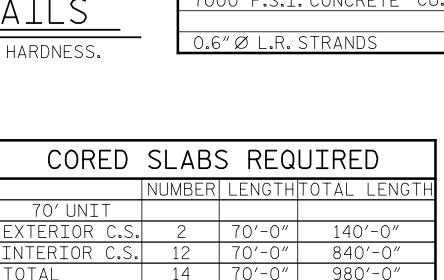
21/4"

(TYP.)

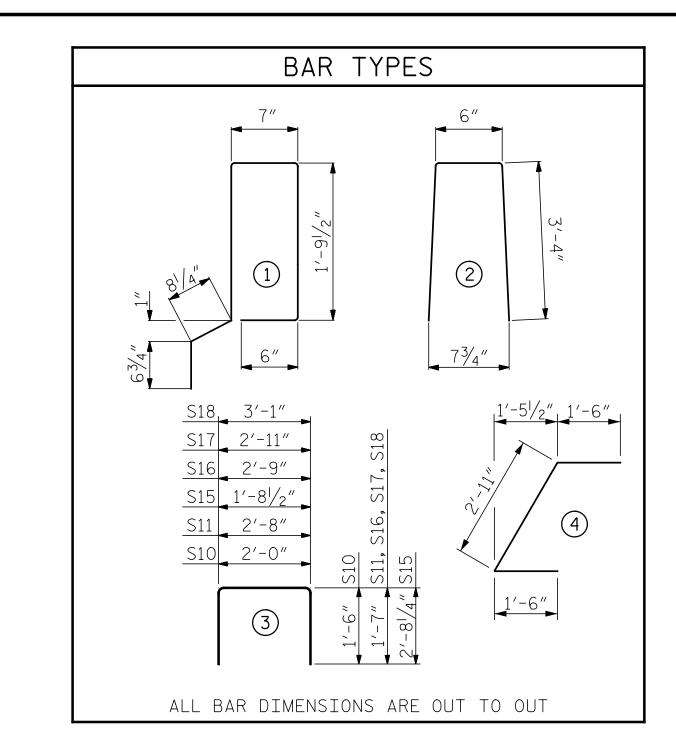
3"T0 2-#5 "B" BARS

--- #5 S12 SEE ``PLAN OF UNIT'' FOR SPACING





GROOVING	BRIDGE FLOORS
APPROACH SLABS	1068 SQ.FT.
BRIDGE DECK	2578 SQ.FT.
TOTAL	3646 SQ.FT.



# NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE  $2\frac{1}{2}$  % DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS, ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

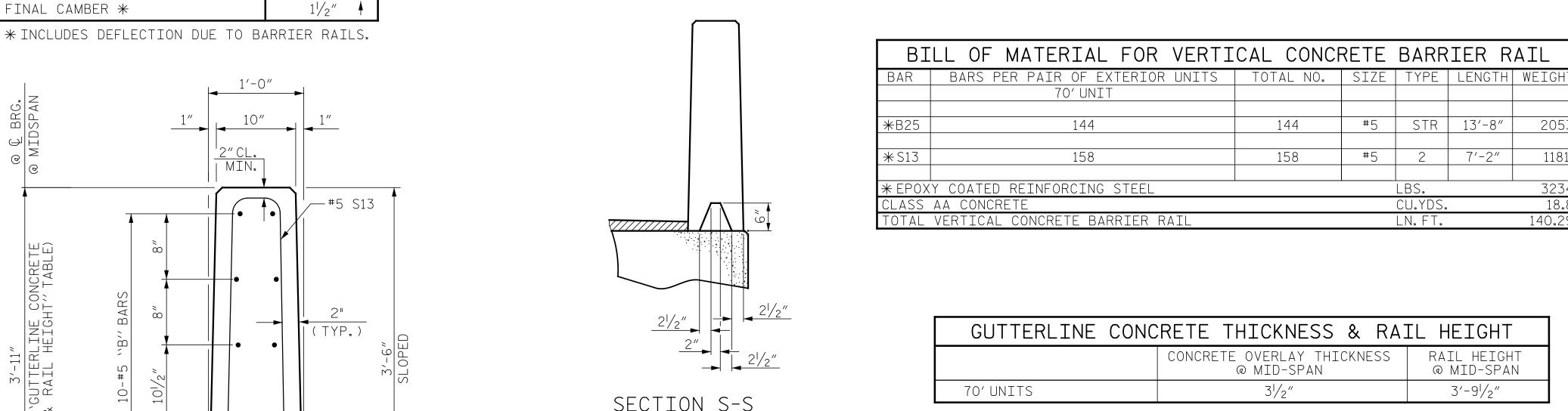
THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR. SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

POST-TENSIONING SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.



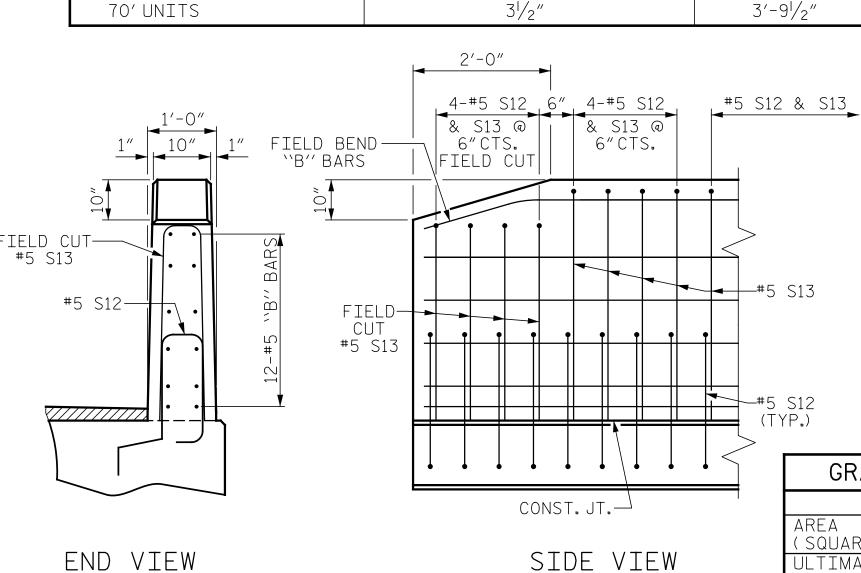
ELEVATION AT EXPANS

1/2" EXP. JT. MAT'L HELD IN

PLĀCĒ WITH GALVANIZED NAILS.

(NOTE: OMIT EXP. JT. MAT'L. WHEN SLIP FORM IS USED)

S USED)  S  FIEL #5  CHAMFER  3/4"  CHAMFER  S  S  S  S  S  S  S  S  S  S  S  S  S	21/2"	
S USED)  S  CHAMFER  3/4"  CHAMFER  3/4"  CHAMFER  S  S  FIEL  #5	AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY	
ION AT EXPANSION JOINTS	CHAMFER  3/4"  CHAMFER  3/4"  CHAMFER	FIEL
	ION AT EXPANSION JOINTS	_



GUTTERLINE CONCRETE THICKNESS & RAIL HEIGHT

CONCRETE OVERLAY THICKNESS

@ MID-SPAN

END VIEW

END OF RAIL DETAILS

STATION: 15+45.00 -L-UNIT PSI 70'UNITS 5500 SHEET 3 OF 3 Elizabeth Lawes DEPARTMENT OF TRANSPORTATION

GRADE 270 STRANDS 0.6" Ø L.R 0.217 SQUARE INCHES LTIMATE STRENGT 58,600 LBS.PER STRAND APPLIED PRESTRES 43,950 (LBS.PER STRAND

CONCRETE RELEASE STRENGTH

VHB Engineering NC, P.C. (C-3705) No.

SEAL 044167 1/21/2021 VG | NE K

940 Main Campus Drive, Suite 500 Raleigh, NC 27606

2'-0" PRESTRESSED CONCRETE CORED SLAB UNIT

> SHEET NO REVISIONS S-7 DATE: DATE: BY: TOTAL SHEETS

VERTICAL CONCRETE BARRIER RAIL DETAILS DATE: 1/13/2021 DATE: 1/13/2021

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

2053

1181

3234

18.8

140.29

RAIL HEIGHT

@ MID-SPAN

1/13/2021 BR-0011\_SD\_CS\_S7.dgn ksmiach

ASSEMBLED BY : EFL CHECKED BY : KFS

DRAWN BY: MAA

CHECKED BY: MKT 7/10

ARIES (SEE THICKNESS

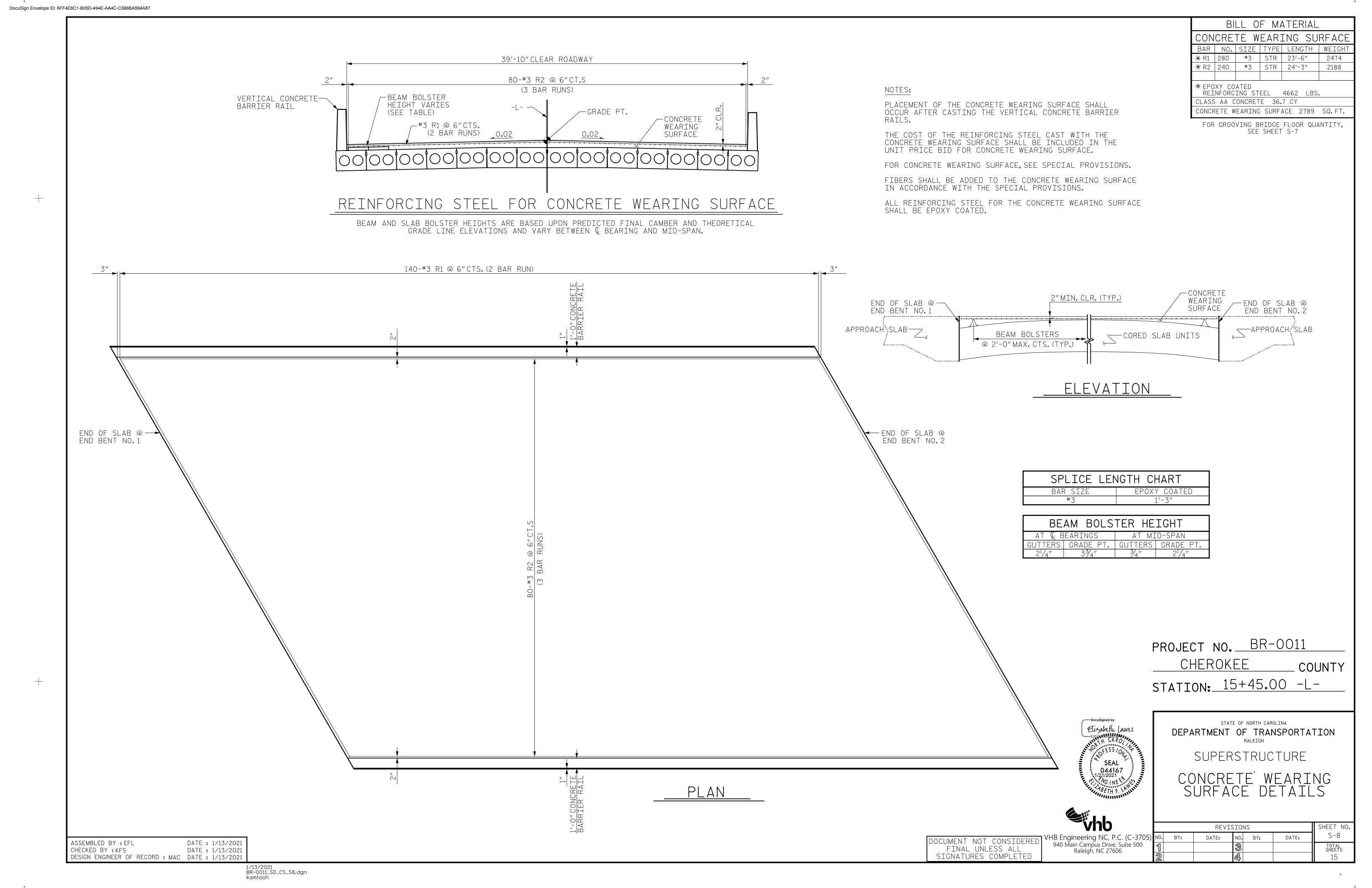
STATE OF NORTH CAROLINA

RALEIGH STANDARD

PROJECT NO. BR-0011

COUNTY

CHEROKEE



© GUARDRAIL——— ANCHOR ASSEMBLY

£ 11/16"∅ HOLES (TYP.) —

¼" HOLD-DOWN ₽ —

(†)

PLAN

1/4" HOLD-DOWN ₽

# NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $1/4^{\prime\prime}$  HOLD DOWN PLATE AND 7 -  $1/8^{\prime\prime}$  Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION. THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

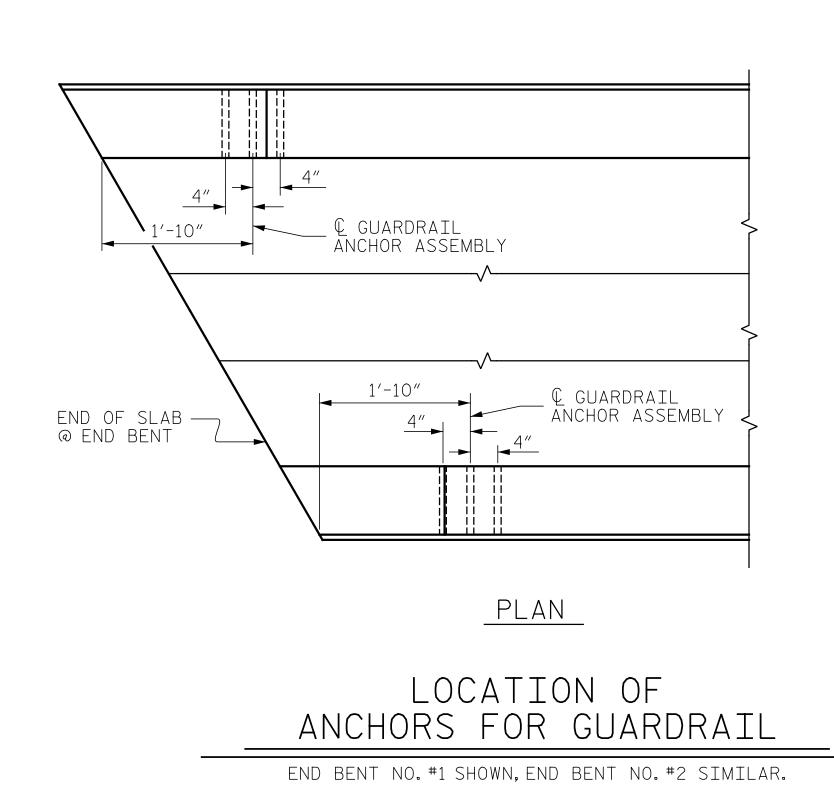
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1  $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



€ GUARDRAIL

ELEVATION

ANCHOR ASSEMBLY

FOR LOCATION OF GUARDRAIL ANCHOR

ASSEMBLY, SEE "PLAN" BELOW

FINISH GRADE —

END OF SLAB-

@ END BENT

- € ½″Ø X 1'-2″BOLT WITH ROUND

WASHERS (TYP.)

ANCHOR ASSEMBLY

-1<sup>1</sup>/<sub>4</sub>"Ø HOLE (TYP.)

C GUARDRAIL

·-----Hb 🔰

**"**□H------Hb

\_\_\_\_\_

SECTION E-E

GUARDRAIL ANCHOR ASSEMBLY DETAILS

Ų GUARDRAIL ANCHOR ASSEMBLY

> END OF SLAB @ END BENT NO.1 END OF SLAB
>
> @ END BENT NO.2

# SKETCH SHOWING POINTS OF ATTACHMENT

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

BR-0011 PROJECT NO.\_ CHEROKEE COUNTY

STATION: 15+45.00 -L-



DEPARTMENT OF TRANSPORTATION STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE BARRIER RAIL

STATE OF NORTH CAROLINA

REVISIONS SHEET NO S-9 DATE:

VHB Engineering NC, P.C. (C-3705)
940 Main Campus Drive, Suite 500
Raleigh, NC 27606 OCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

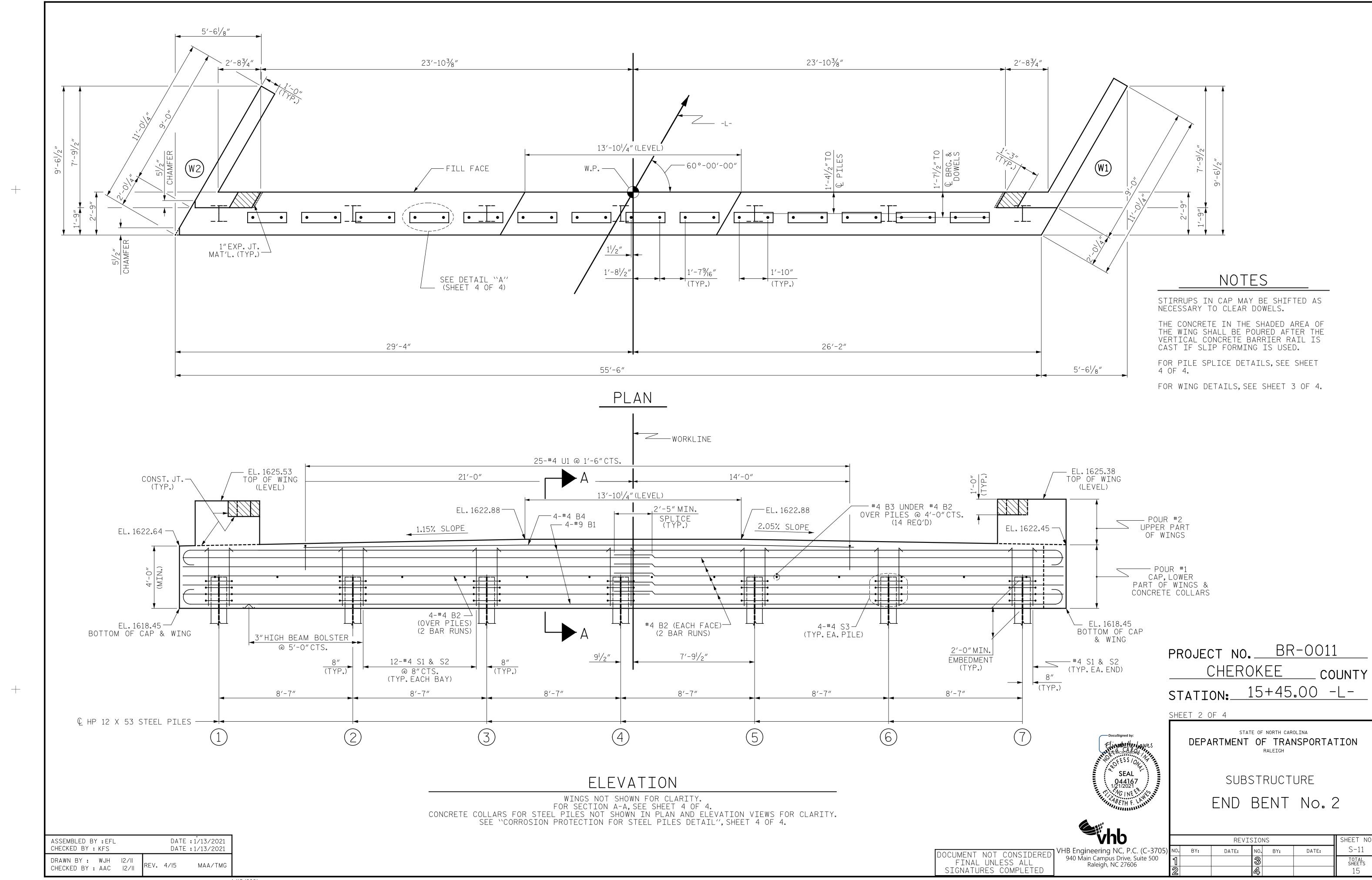
REV. 6/I3 REV. I/I5 REV. 5/I8 MAA/GM MAA/TMG MAA/TMG CHECKED BY: GM 5/10 1/13/2021 BR-0011\_SD\_GR\_S9.dgn ksmiach

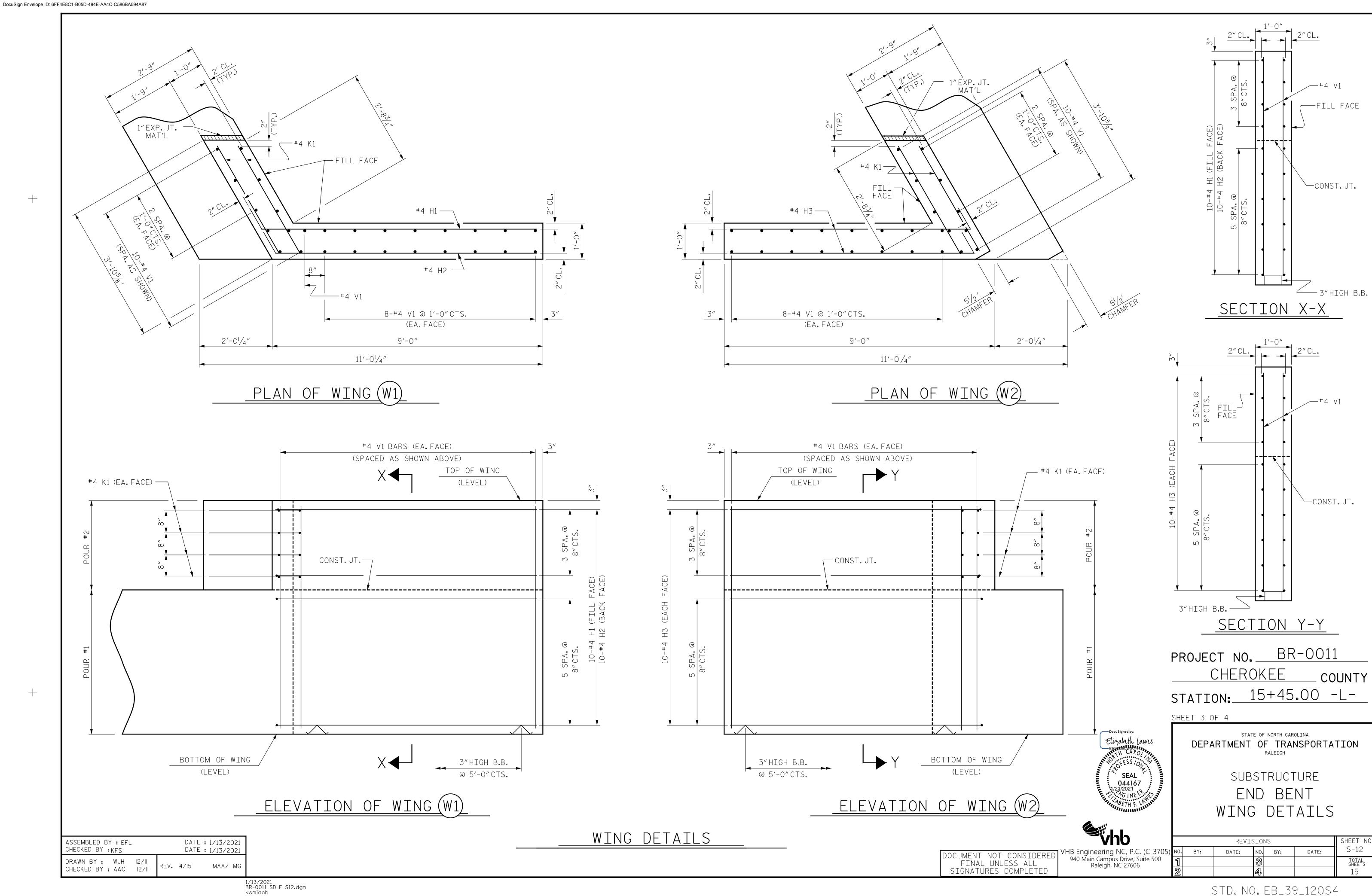
DATE: 1/13/2021 DATE: 1/13/2021

ASSEMBLED BY : EFL CHECKED BY : KFS

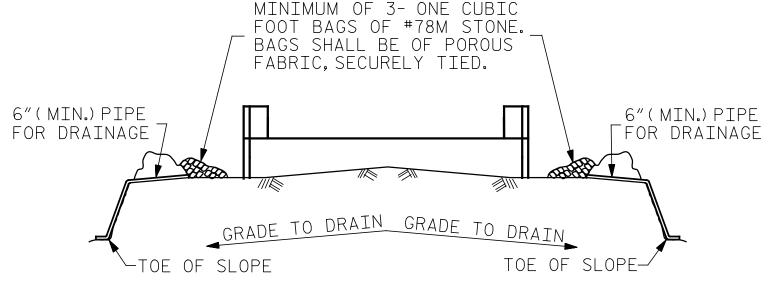
DRAWN BY: MAA 5/10

STD. NO. GRA3 (SHT 1b)





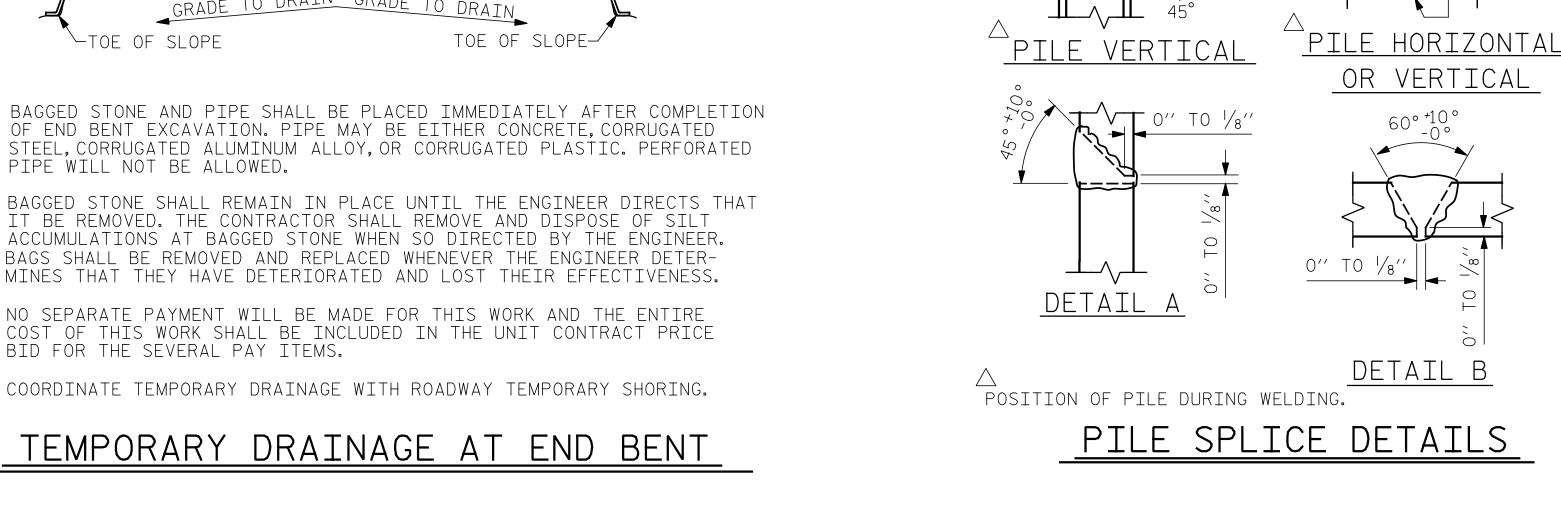
STD. NO. EB\_39\_120S4

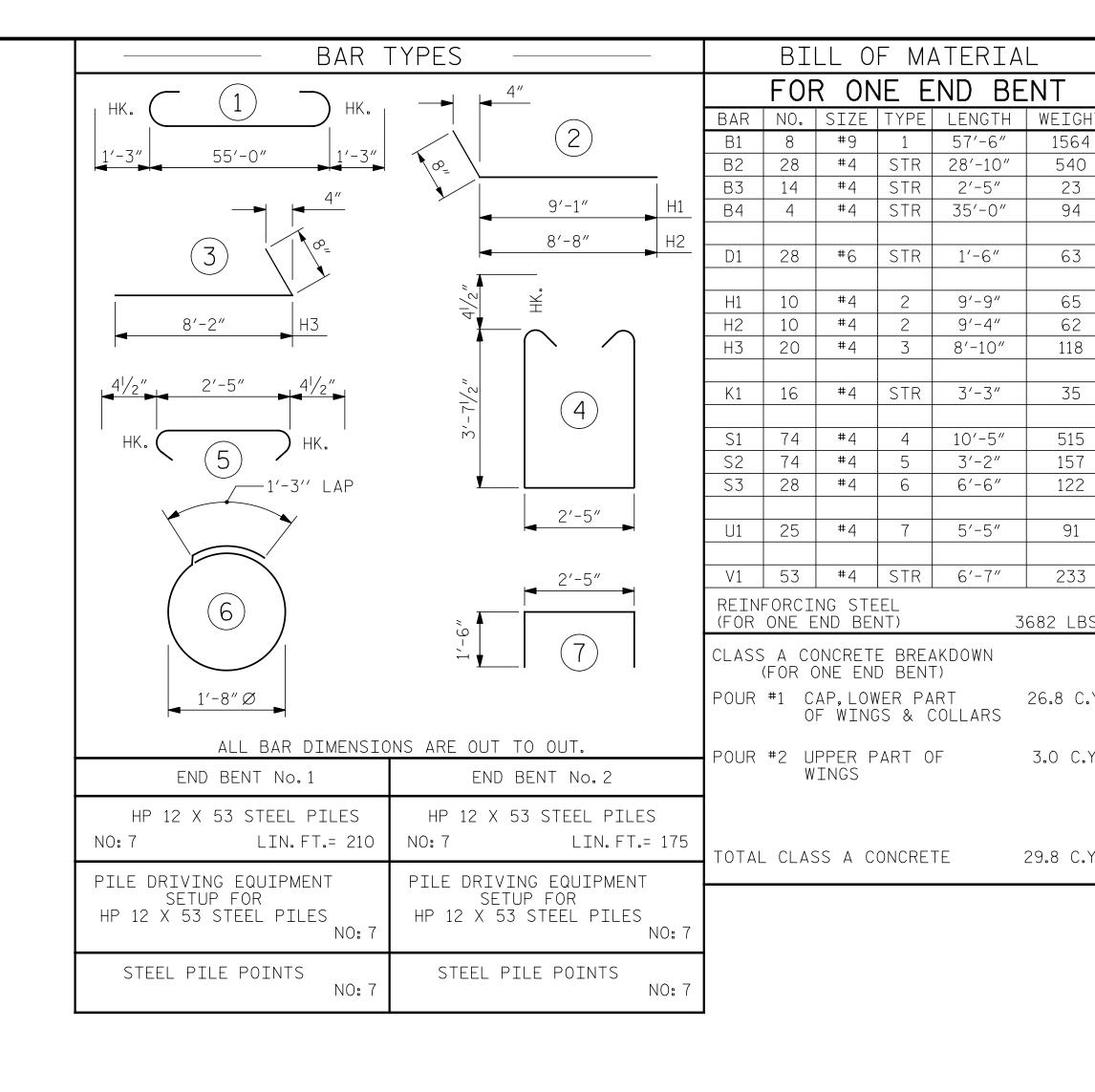


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-

COORDINATE TEMPORARY DRAINAGE WITH ROADWAY TEMPORARY SHORING.





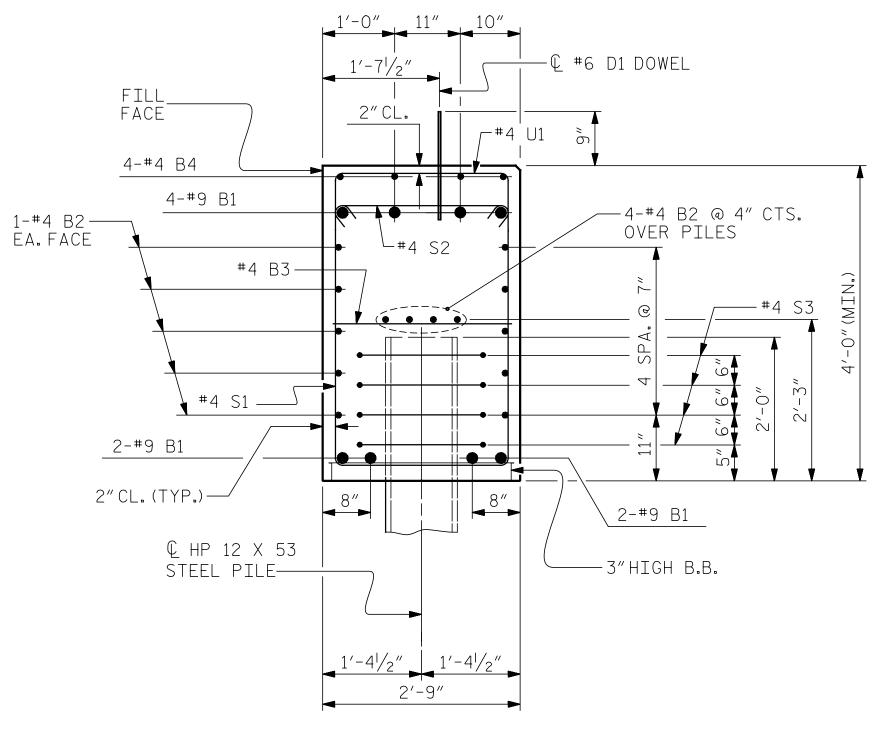
— SLAB UNIT 2'-6" #6 D1 DOWELS 1'-3" 1'-3" TO PROJECT 9" ABOVE CAP (TYP.) 11" 11" 1"X 8"X 2'-6" ——/ ELASTOMERIC BRG. PAD (TYPE I)(TYP.) 1'-10" - FILL FACE DETAIL "A" (END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION) CONCRETE COLLAR © PILES & — `CONCRETE COLLARS FILL FACE 2'-0"Ø CONCRETE COLLAR © HP 12 X 53 TEEL PILE (TYP.EACH PILE)

-BOTTOM OF CAP 2'-0" ELEVATION

CORROSION PROTECTION FOR STEEL PILES DETAIL

(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

ASSEMBLED BY : EFL CHECKED BY : KFS			: 1/13/2021 : 1/13/2021
DRAWN BY: WJH 12/II CHECKED BY: AAC 12/II	REV.	4/17	MAA/THC



/ BACK GOUGE

DETAIL B

SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

OCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

BR-0011 PROJECT NO. \_\_\_ CHEROKEE COUNTY

#9 | 1 | 57'-6"

#4 STR 28'-10"

#4 | STR | 35'-0"

9′-9″

9'-4"

8'-10"

10'-5"

3'-2"

6′-6″

5′-5″

#4 | STR | 2'-5"

#4

#4

#4

#4 | 3 |

#4 | STR | 3'-3"

6

1564

540

23

94

63

65

62

118

35

515

157

122

91

233

3682 LBS

26.8 C.Y.

3.0 C.Y.

29.8 C.Y.

15+45.00 -L-STATION:\_ SHEET 4 OF 4

Elizabeth Lawes

SEAL

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

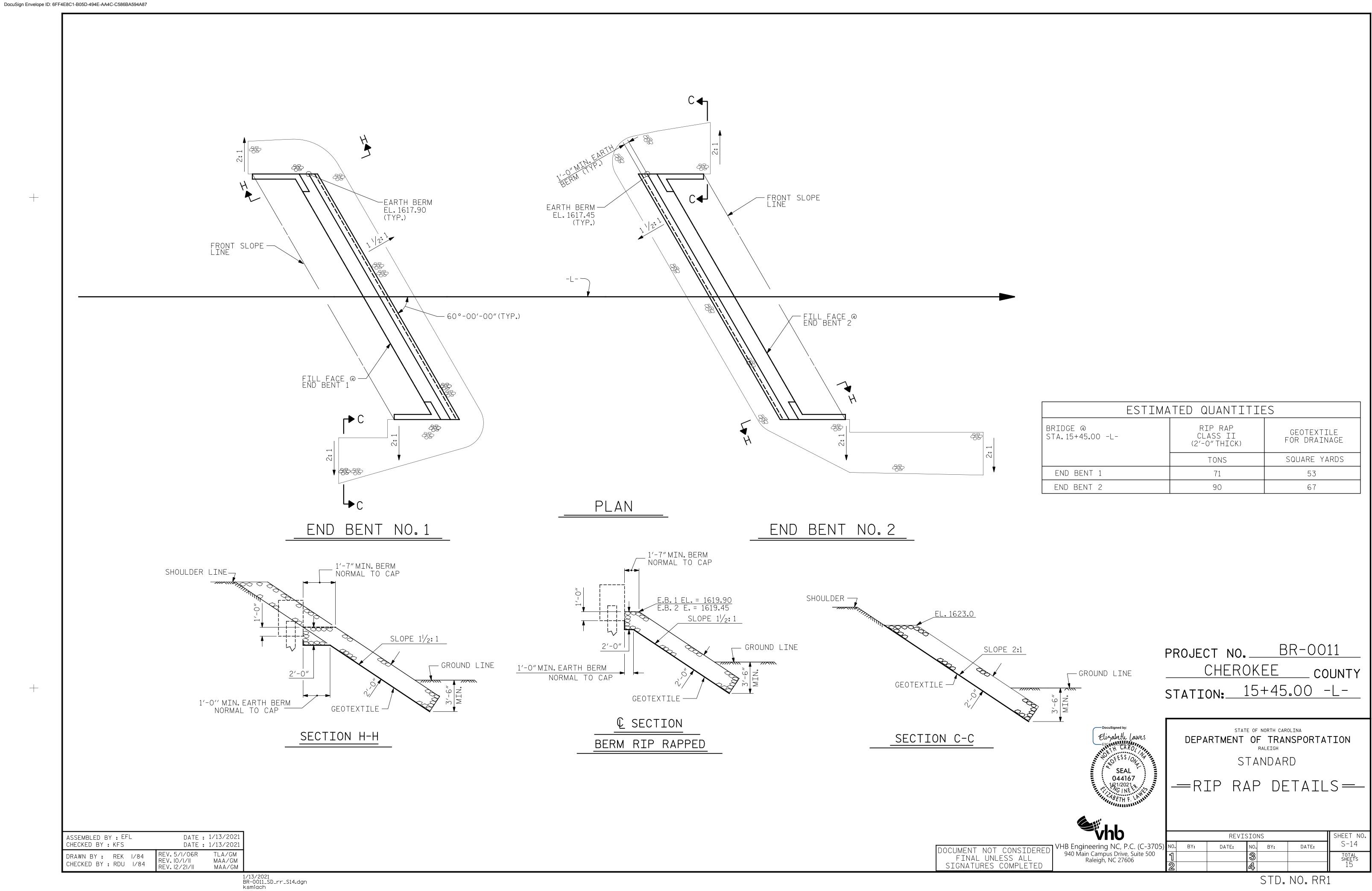
SUBSTRUCTURE

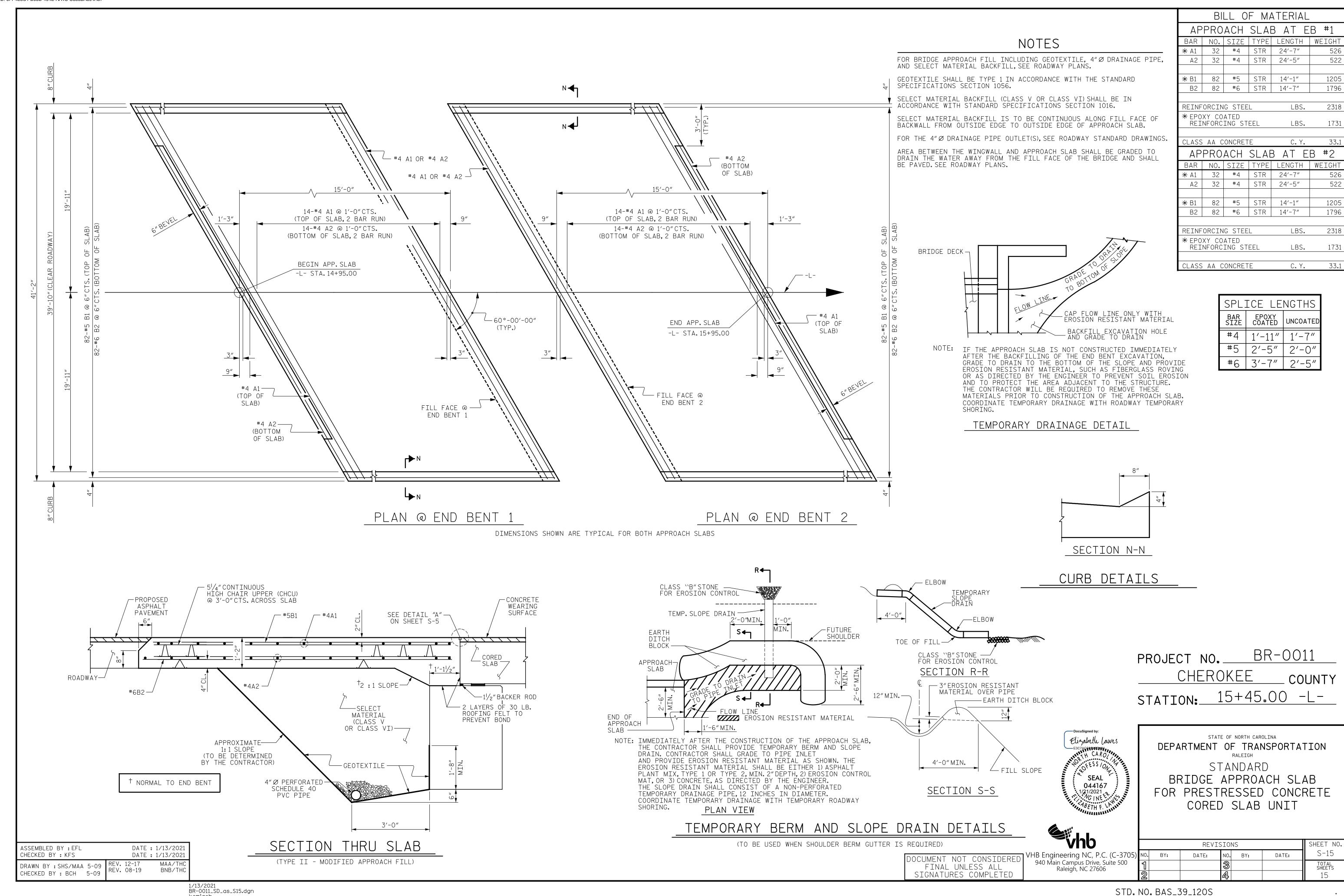
END BENT No.1 & 2 DETAILS

vno			SHEET NO.				
7 VHB Engineering NC, P.C. (C-3705)	NO.	BY:	DATE:	NO.	BY:	DATE:	S-13
940 Main Campus Drive, Suite 500 Raleigh, NC 27606	1			3			TOTAL SHEETS
Kaleigh, We 27 000	2			4			15

1/13/2021 BR-0011\_SD\_F\_S13.dgn ksmiach

PLAN





# STANDARD NOTES

#### DESIGN DATA:

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

EQUIVALENT FLUID PRESSURE OF EARTH - - - - 30 LBS. PER CU.FT.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

#### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

## CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

## DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT,

## ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

#### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

#### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE  $\frac{7}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST \( \frac{5}{16} \) "IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/6 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

#### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

#### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH