5600 Z REFERENCE **CONTENTS** 

**DESCRIPTION** 

TITLE SHEET LEGEND (SOIL & ROCK)

SITE PLAN

BORE LOGS SOIL TEST RESULTS

PROFILE

SHEET NO.

4-5

6-19

00056 S **PROIEC** 

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY **JOHNSTON** 

PROJECT DESCRIPTION US 70 FROM EAST OF US 70 BUSINESS TO WEST OF NEUSE RIVER

SITE DESCRIPTION RETAINING WALL 1 ON -WL1- FROM 10+00.00 TO 22+58.72 AND RETAINING WALL 2 ON -WL2-FROM 10+00.00 TO 24+04.64

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	W-5600	1	20

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR NSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-680. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (INP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NIDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE TOTAL WITH THE ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OF FOR ANY TEASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

M. ARNOLD S. WOODS M. DURWAY S. DAVIS D. AIELLO T. SHARPE A. STURCHIO INVESTIGATED BY  $\_F&R, Inc.$ DRAWN BY \_T.T. WALKER CHECKED BY \_C. WANG

SUBMITTED BY P. ALTON, P.E. 



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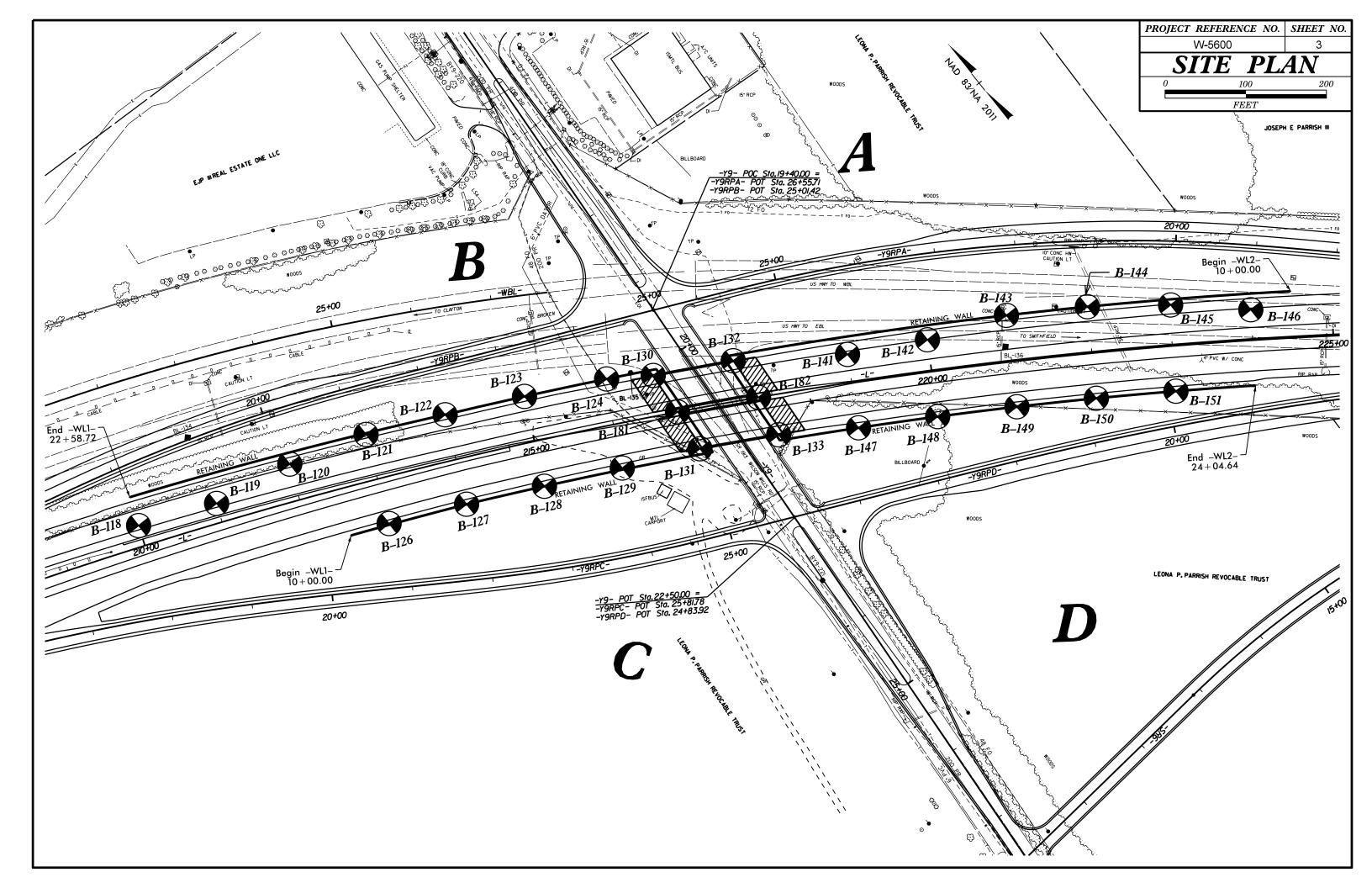
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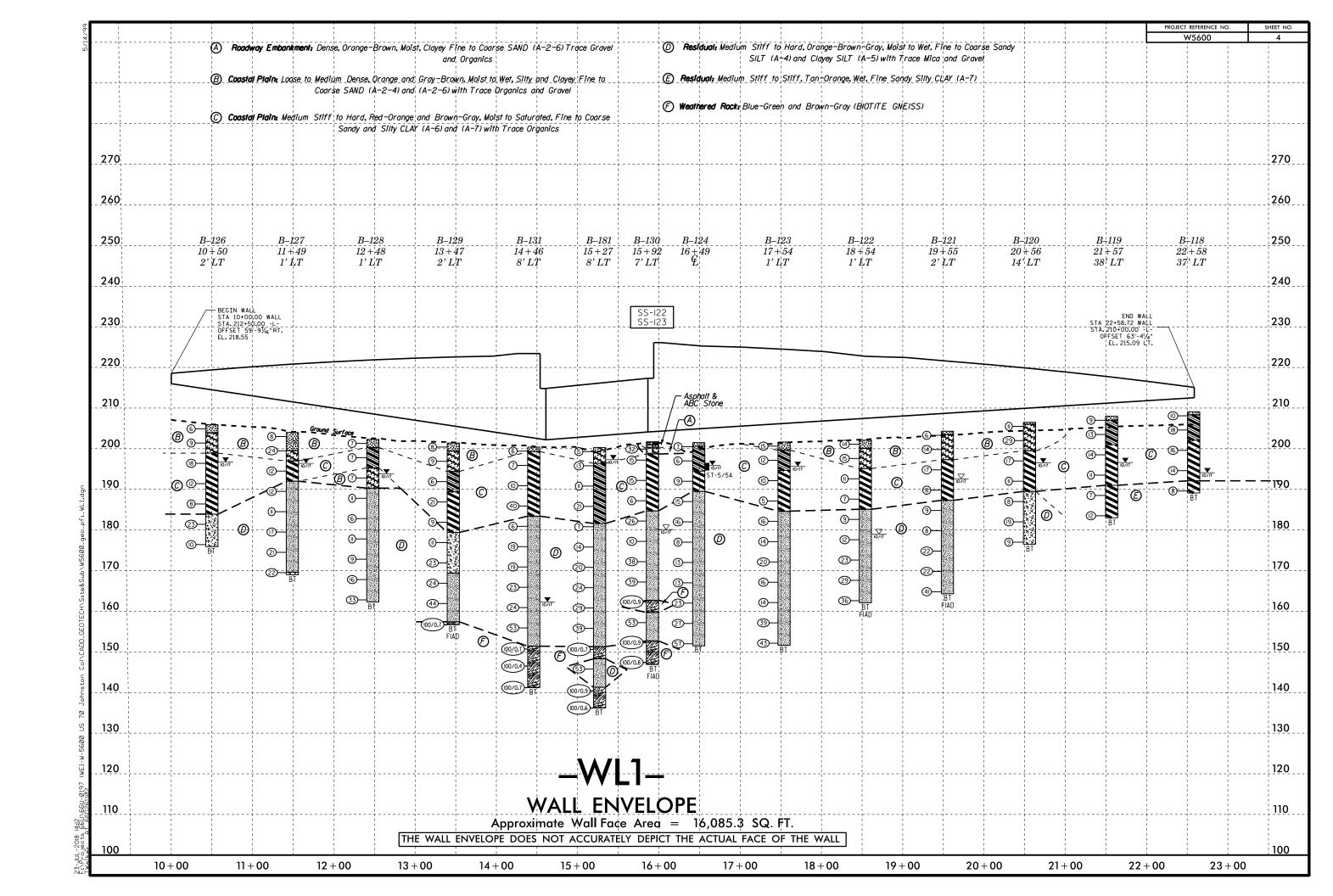
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

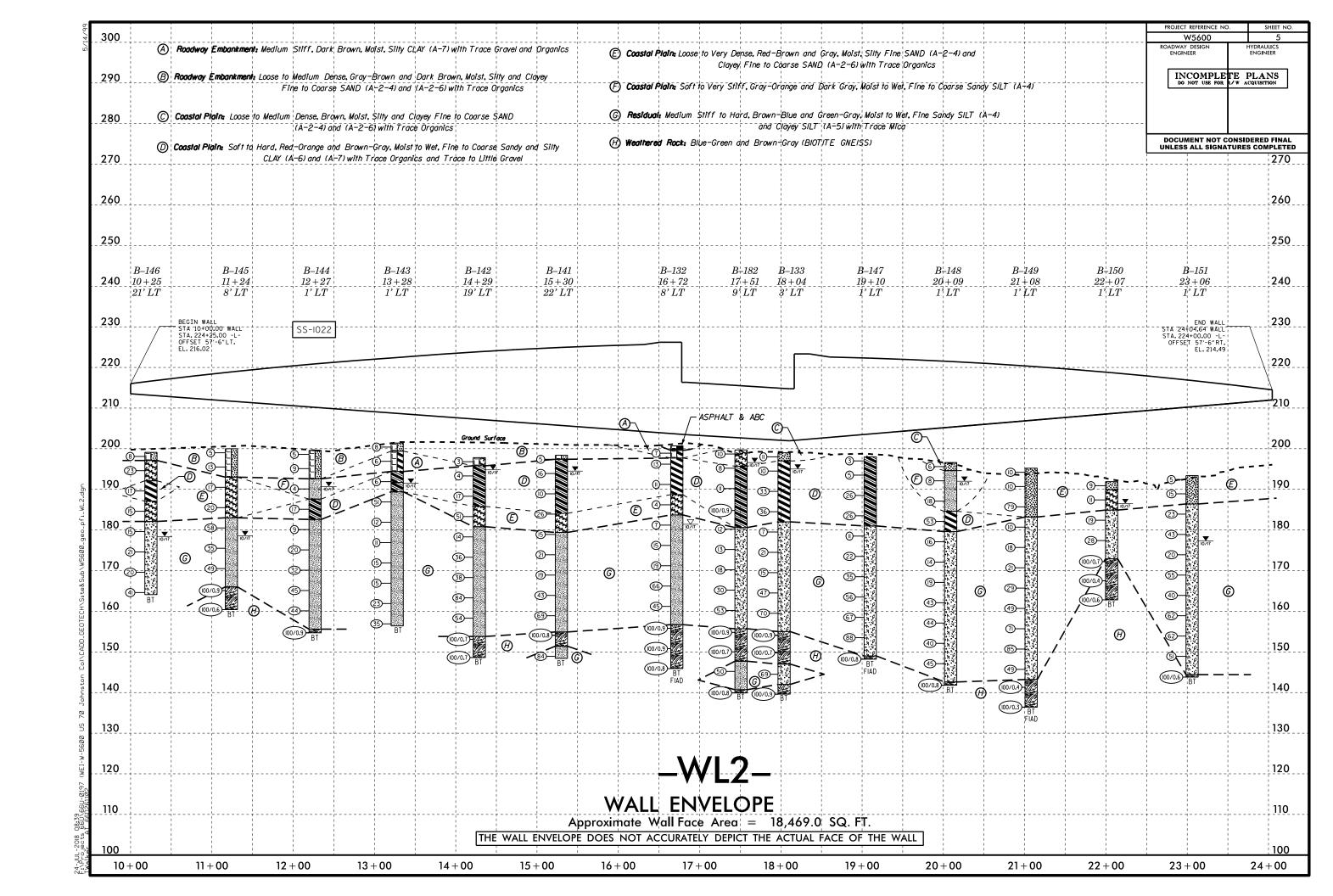
# SUBSURFACE INVESTIGATION

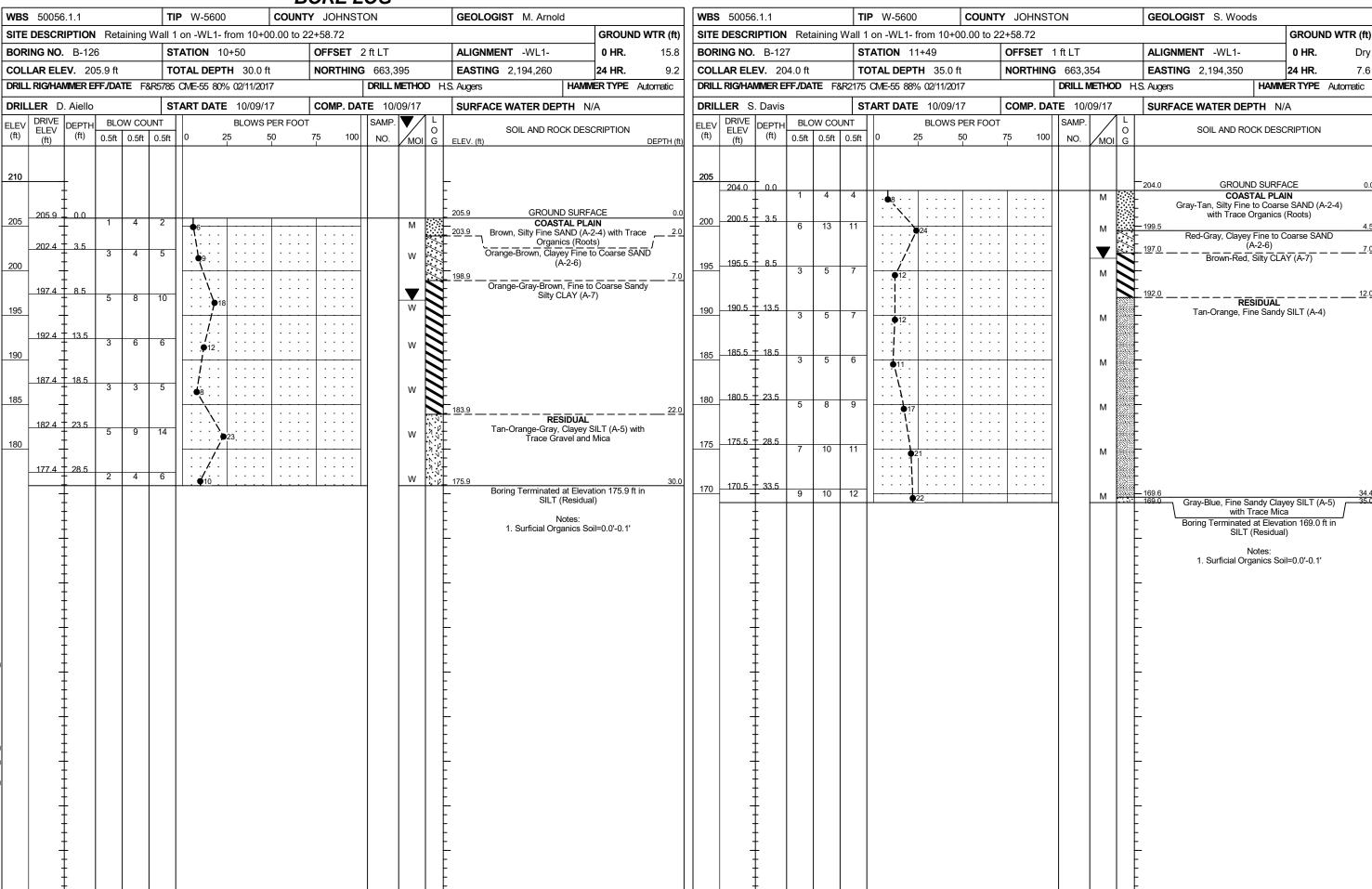
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

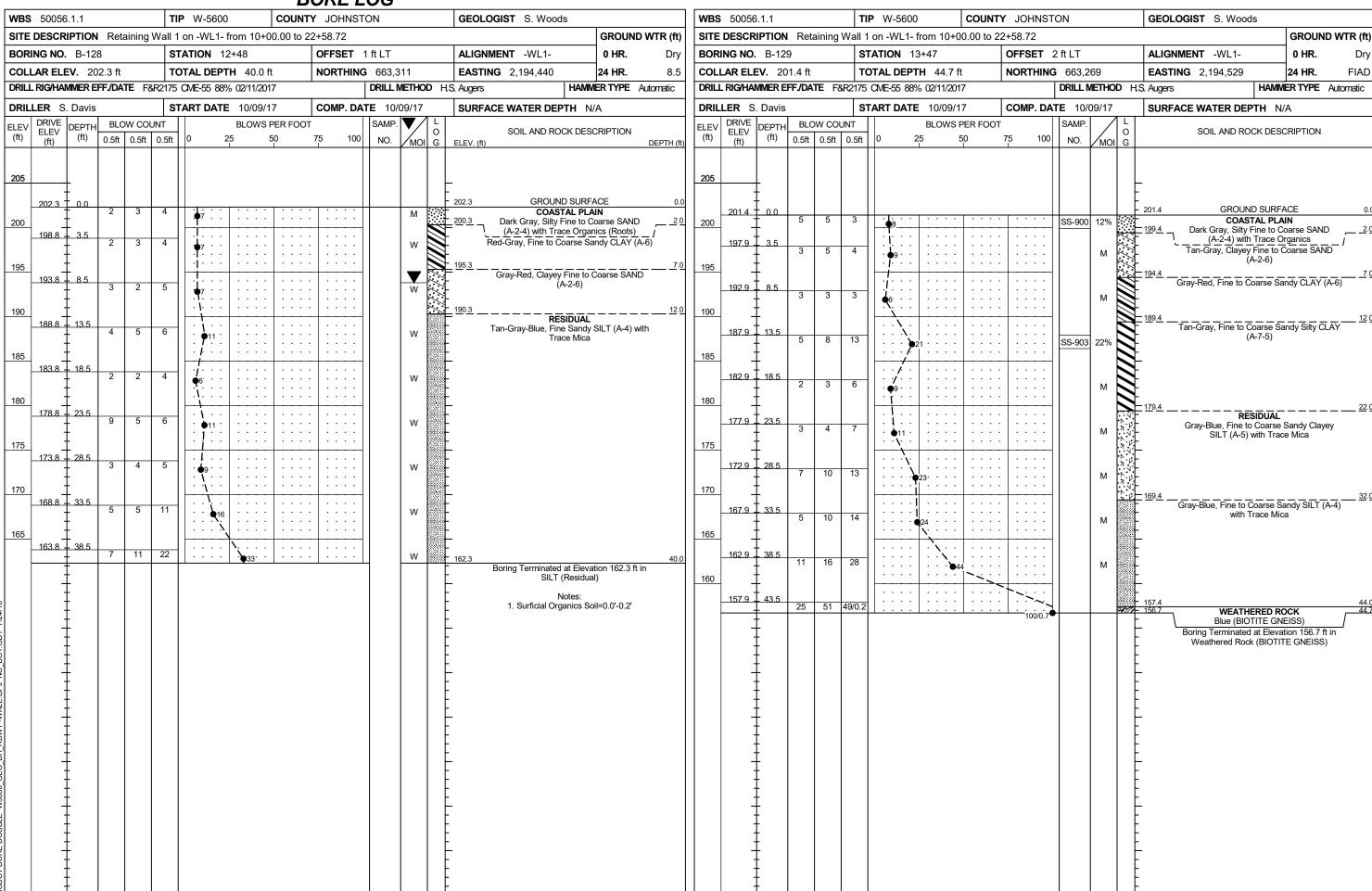
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL DESCRIPTION  SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED	
BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.  SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER,
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DI586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA,
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY. SILTY CLAY. MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC. A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	WEATHERED WILLIAM NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
CENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS OPERANIC MATERIALS	MINERALOGICAL COMPOSITION	CONSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	CRYSTALLINE ROCK (CR)  WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.  COMPRESSIBILITY	NON-CRYSTALLING FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-3 A-6, A-7	***	ROCK (NCR)  SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUYIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 000000000000000000000000000000000000	MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
7. PASSING SILT-	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR GLAY MUCK *40 30 MX 50 MX 51 MN SILM SOILS CLAY PEAT		WEATHERING	<u>DIKE</u> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
"200   15 MX   25 MX   10 MX   35 MX   35 MX   35 MX   35 MX   36 MN   36 MN   36 MN   36 MN   36 MN	GRANULAR SILT - CLAY  ORGANIC MATERIAL SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	HAMMER IF CRYSTALLINE.	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING #40 SOILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
LL	LITCH Y ODGANIC YOUR YORK YORK YORK AND ADOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
CROID INDEX A A A A MY A MY 12 MY 15 MY NO MY AMOUNTS OF ORGAN	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
ISIAL TYPES STONE FRACS ORGANIC	▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL, AND GAME GRAVEL AND GAME GAME GAME GAME GAME GAME GAME GAME		CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SANU		MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN (MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
GEN.RATING EXCELLENT TO GOOD FAIR TO POOR FAIR TO POOR UNSUITE	DE PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
P1 OF A-7-5 SUBGROUP IS ≤ LL - 3Ø ;P1 OF A-7-6 SUBGROUP IS > LL - 3Ø	── O-MM- SPRING OR SEEP	WITH FRESH ROCK.	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.
DANCE OF CTANDARD DANCE OF LINCONFINE		(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPRESSIVE STRENGT PENETRATION RESISTENCE COMPRESSIVE STRENGT	I	<u>IF TESTED, WOULD YIELD SPT REFUSAL</u>	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
(N-VALUE) (TUNS/FT-)	₩ITH SOIL DESCRIPTION → OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL, IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE	SOIL SYMBOL  OPT DMT TEST BORING  SLOPE INDICATOR INSTALLATION	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIAL MEDIUM DENSE 10 TO 30 N/A	VSI PAI	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AFRATION AND LACK OF GOOD DRAINAGE.
DENSE   30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY DENSE > 500  VERY SOFT < 2 < 0.25	— INFERRED SOIL BOUNDARY — CORE BORING ■ SOUNDING ROD	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5	TECT DODING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE  MONITORING WELL  TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	A ALLINIAL COLL POLINDARY A PIEZOMETER COLL SOT NEVALUE	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4	INSTALLATION	ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIF	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	1000 N. TUE TOD O ESST OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF ACCEPTABLE DEGRADABLE ROCK EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
(BLDR.) (COB.) (GR.) (SSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SOIL MOISTURE - CORRELATION OF TERMS	CL CLAY  MOD MODERATELY  7 - UNIT WEIGHT  CPT - CONE PENETRATION TEST  NP - NON PLASTIC  7 - DRY UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOLI MOISTURE SCALE FIELD MOISTURE	CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS)  DESCRIPTION  GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u>	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
(SAT.) FROM BELOW THE GROUND WATER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC   LIQUID LIMIT	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SUFT OR MURE IN THICKNESS CAN BE BRUKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE - WET - (W) SEMISULIU; REQUIRES DRYING TO	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	
(PI) PL PLASTIC LIMIT	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	BENCH MARK: BL-I35= N: 663354.9350, E: 2194610.616, -L- STA. 216+33.31, 58.59' LT. BL-I36= N: 663090.861, E: 2194994.322 -L STA. 220+94.43.
	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	18.45' LT ELEVATION: 200.72 FEET
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	
SL SHRINKAGE LIMIT	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	S' CONTINUOUS ELIGHT AUGER	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	FIAD= FILLED IMMEDIATELY AFTER DRILLING
111111111111111111111111111111111111111	X CME-55   \( \sum \)	INDURATION	NM= NOT MEASURED
PLASTICITY	<b>-</b>   -   -   -   -   -   -   -   -   -	INDURTH LON  FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	BL-135: B-118 to B-124, B-126 to B-133, B-147 to B-151, B-181, B-182
PLASTICITY INDEX (PI)  DRY STRENGTH	CME-550 HARD FACED FINGER BITS	DURRING WITH FINGER EREES NUMEROUS CRAINS.	BL-136: B-141 to B-146
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST TUNG,-CARBIDE INSERTS	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	DE-130: D-141 10 D-140
MODERATELY PLASTIC 16-25 MEDIUM	CASING W/ ADVANCER POST HOLE DIGGER	CRAING CAN DE CERADATER FROM CAMPLE MITH CIFEL PROPE	
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	MODERATELY INDURATED  ORANGE CAN BE SEPARATED FROM SAMPLE WITH SIEEL PROBE;  BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE TUNG,-CARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
DESCRIPTIONS MAY INCHINE COLOR OF COLOR COMBINATIONS (TAN DED VEH ON DECIMAL DELIC COASS	CORE BIT SOUNDING ROD VANE SHEAR TEST	DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).  MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	

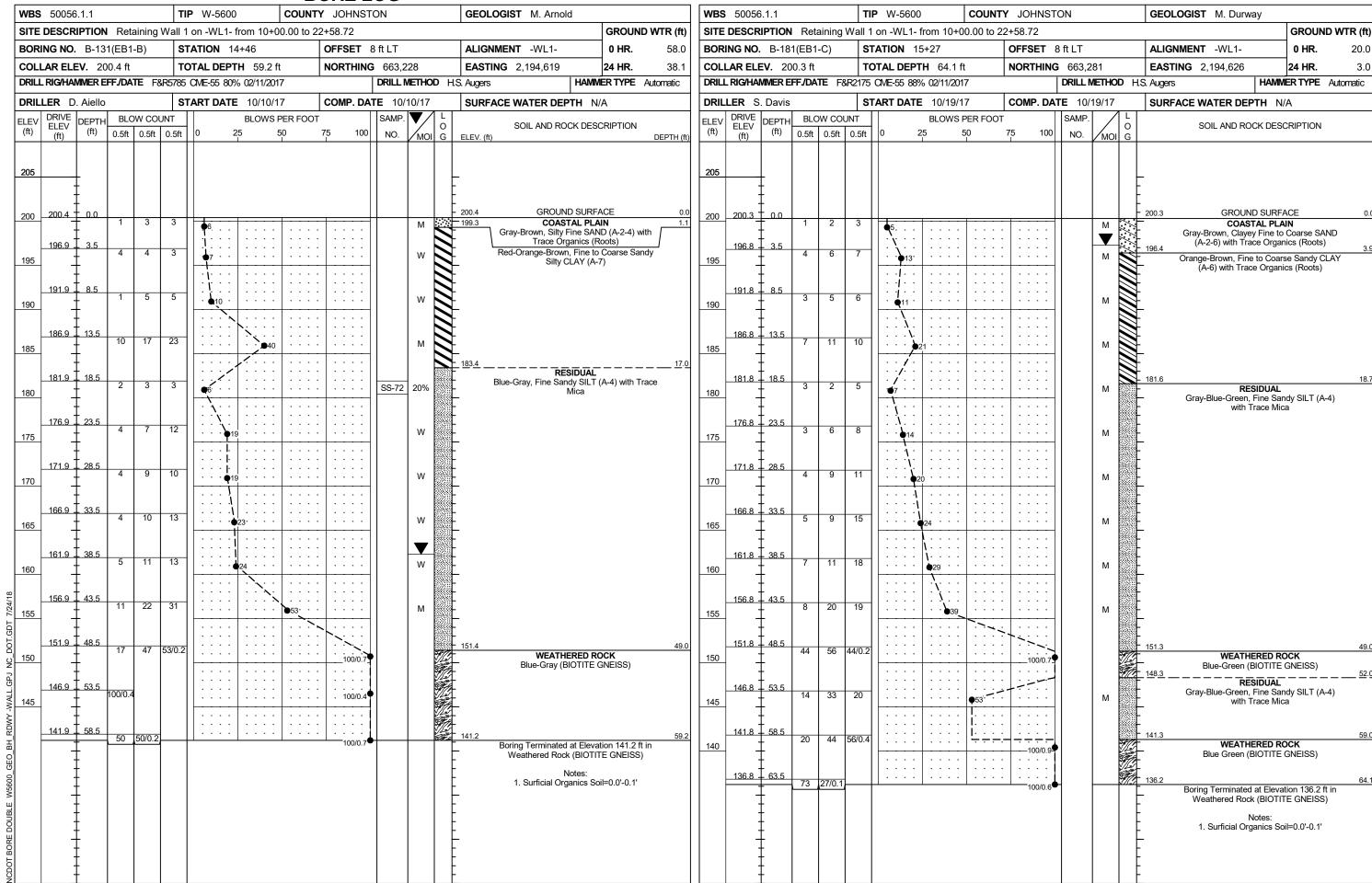


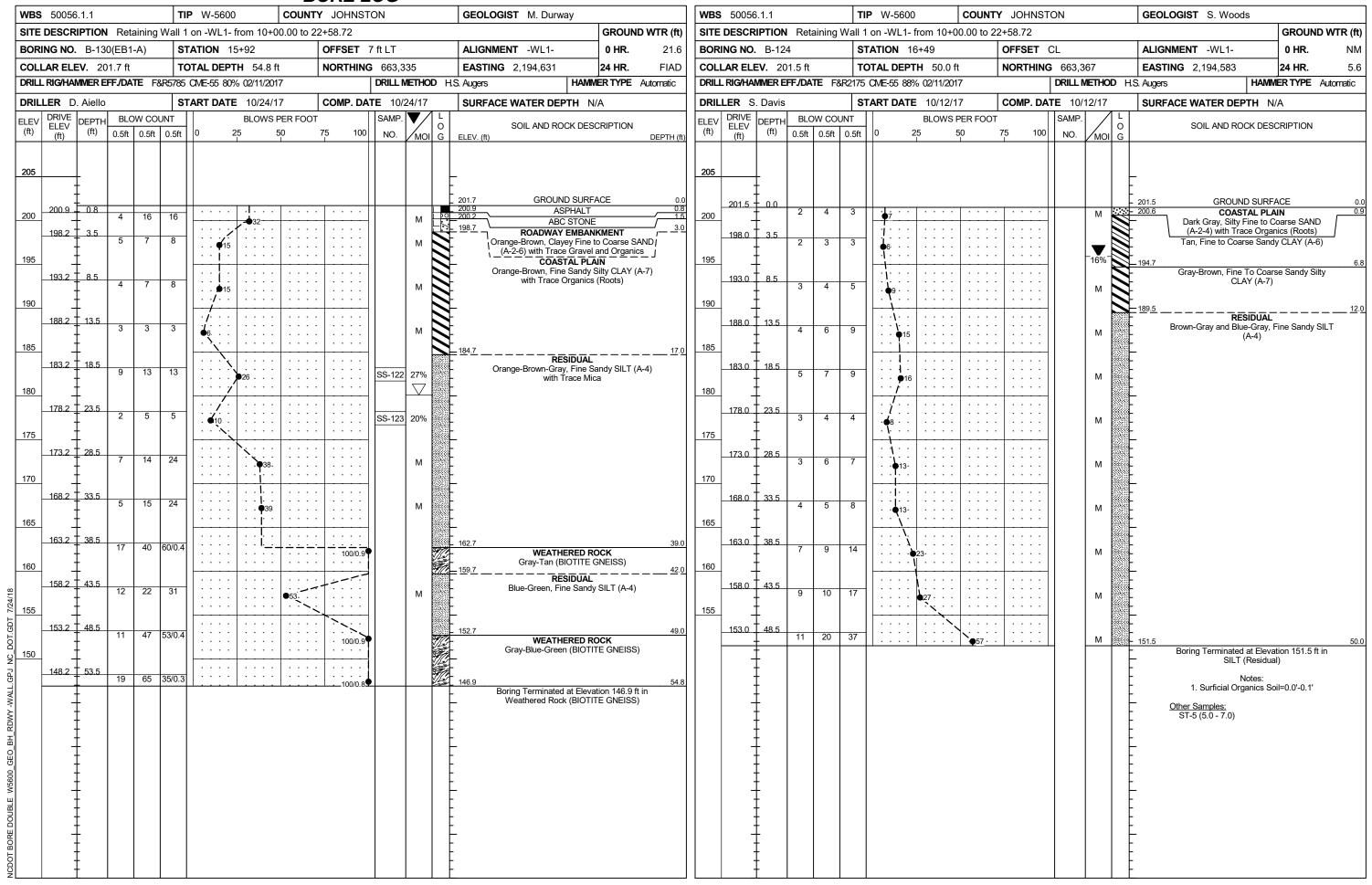


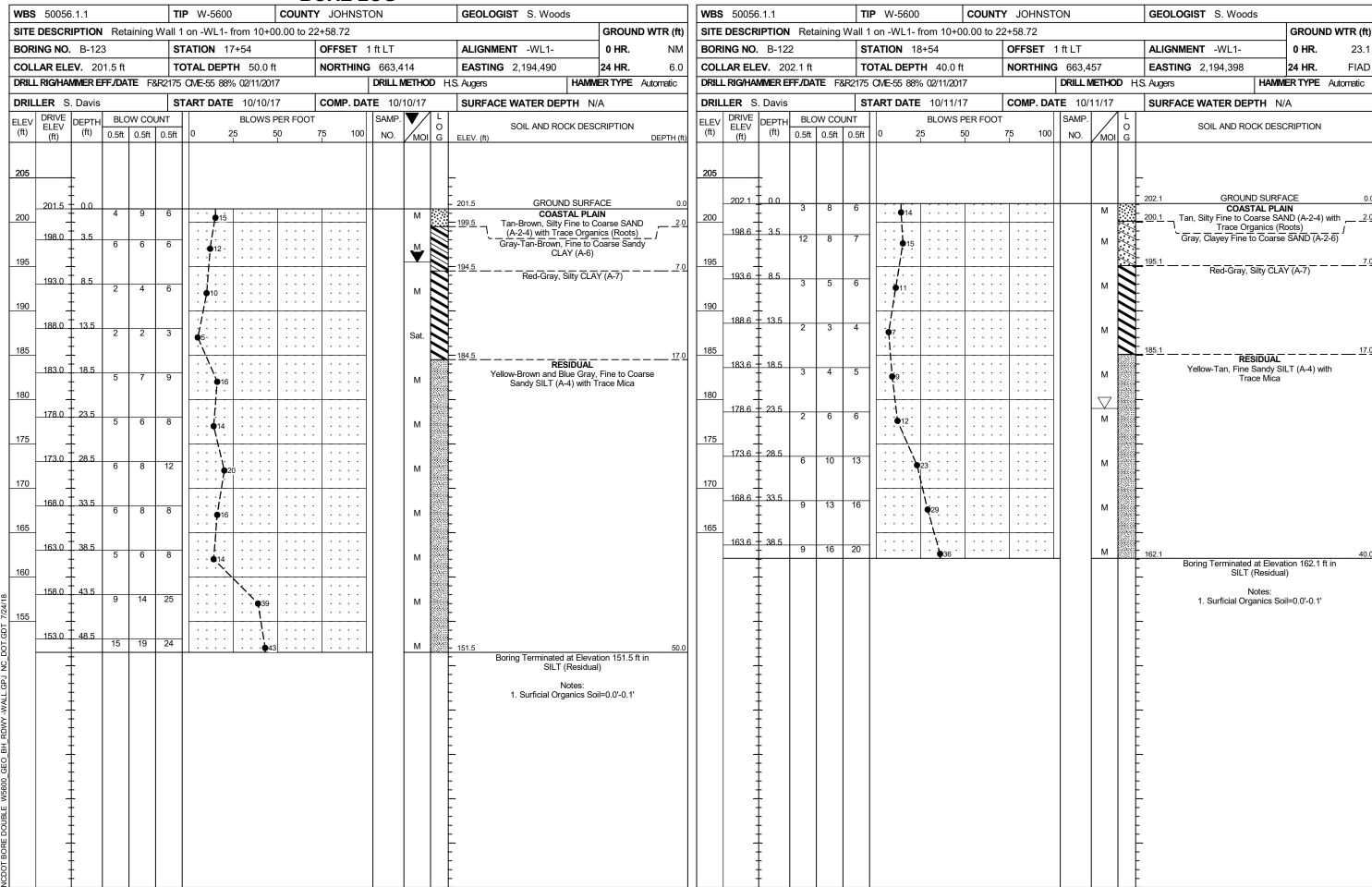


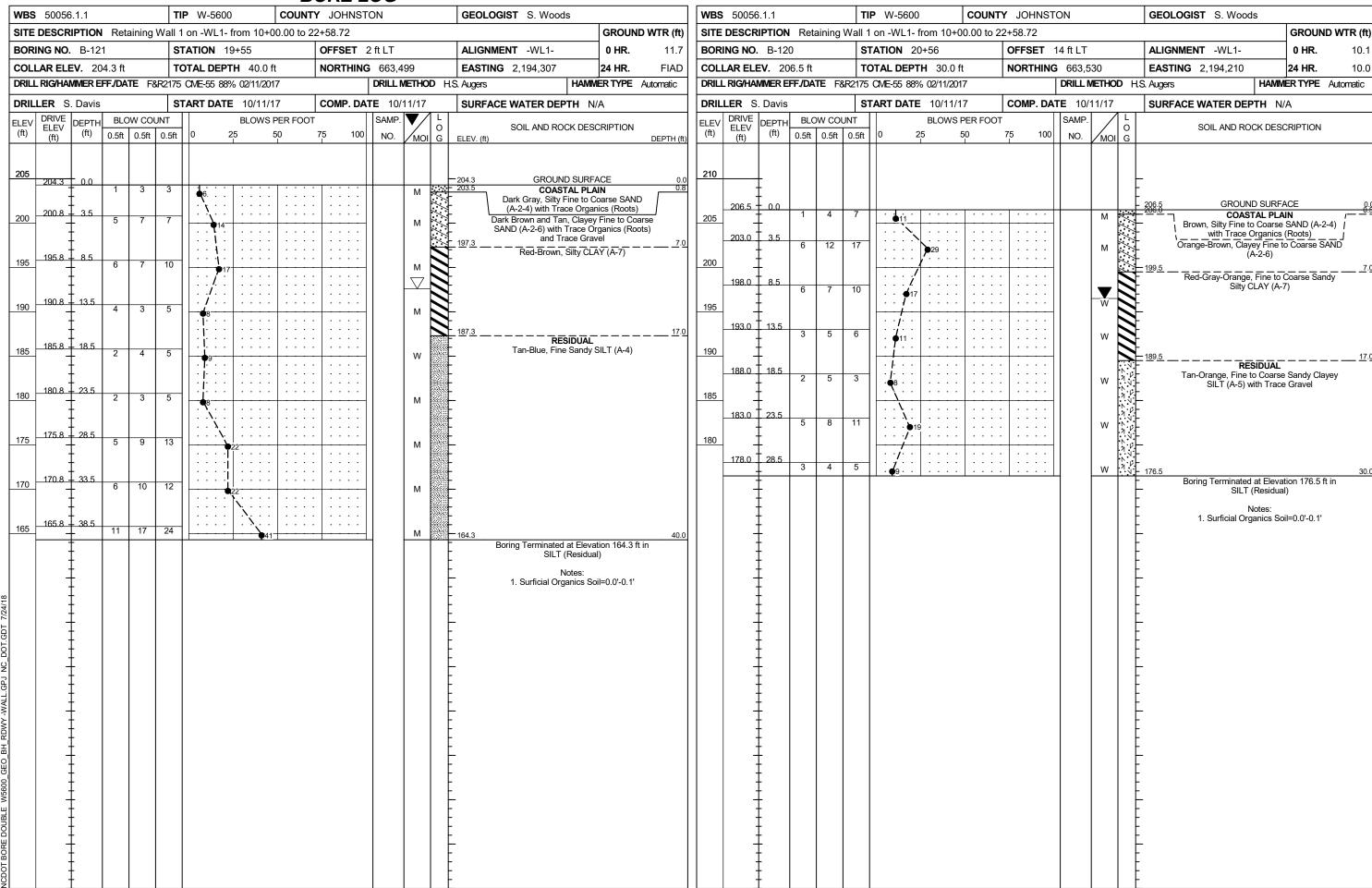




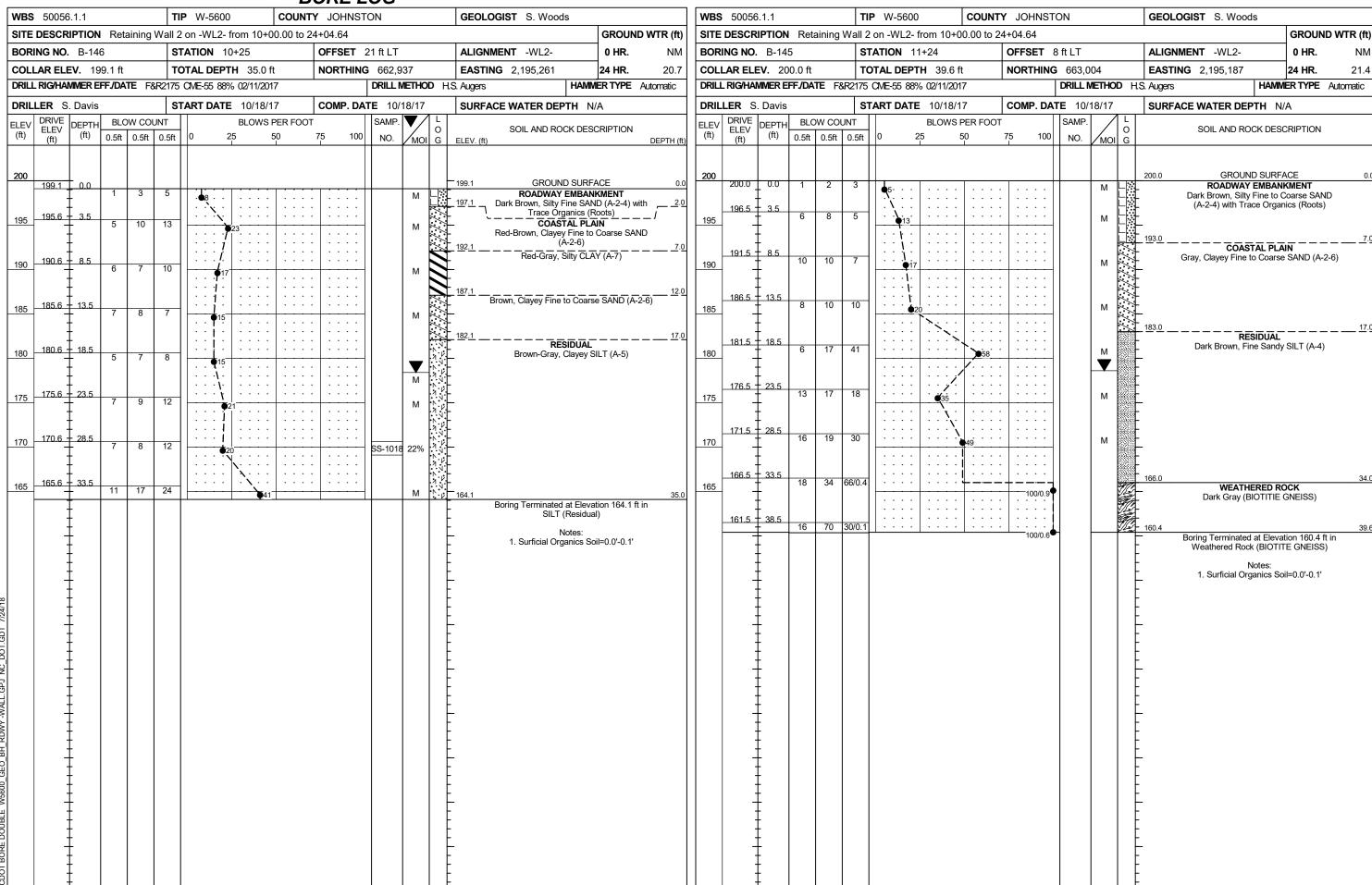


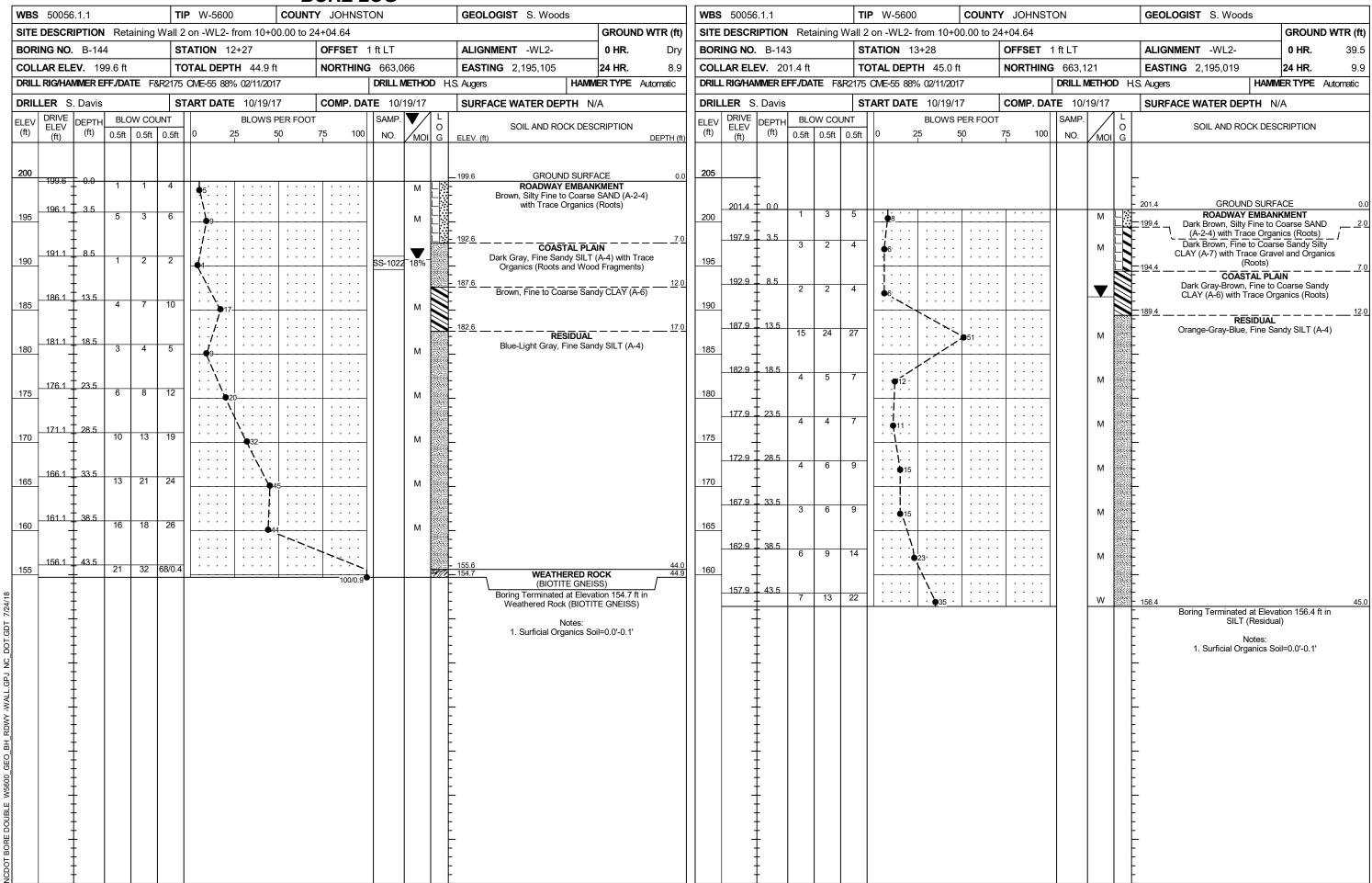


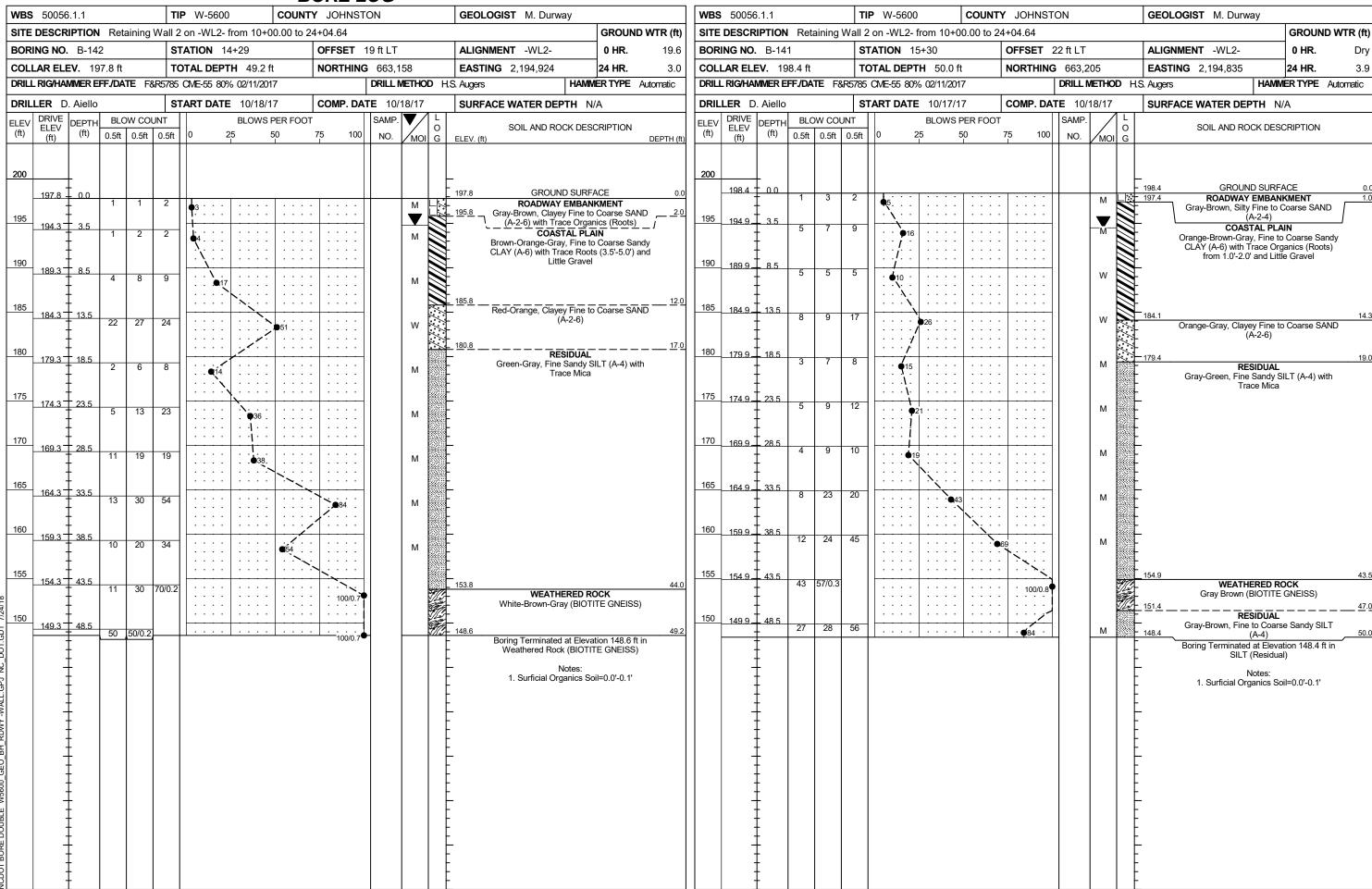


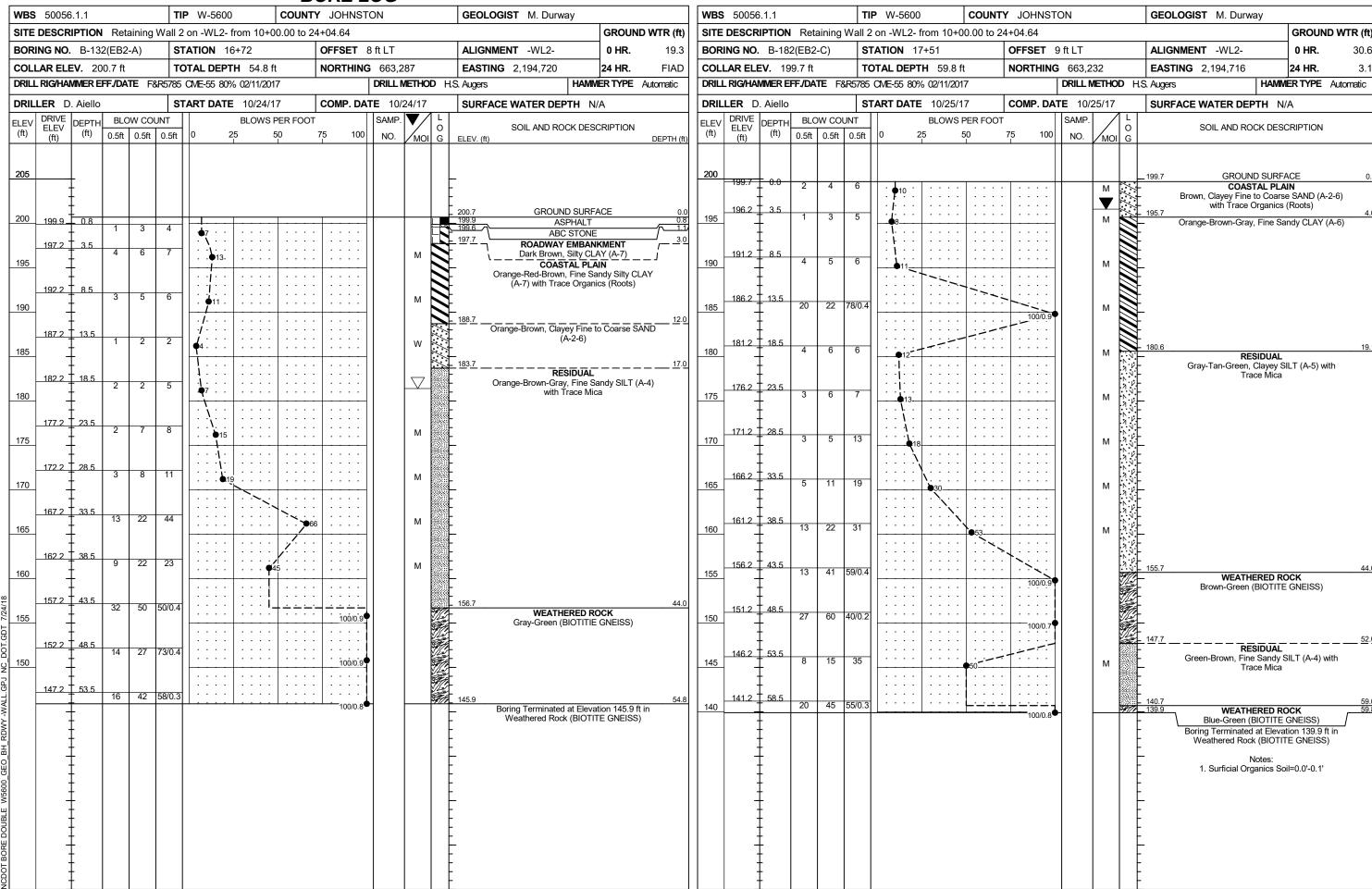


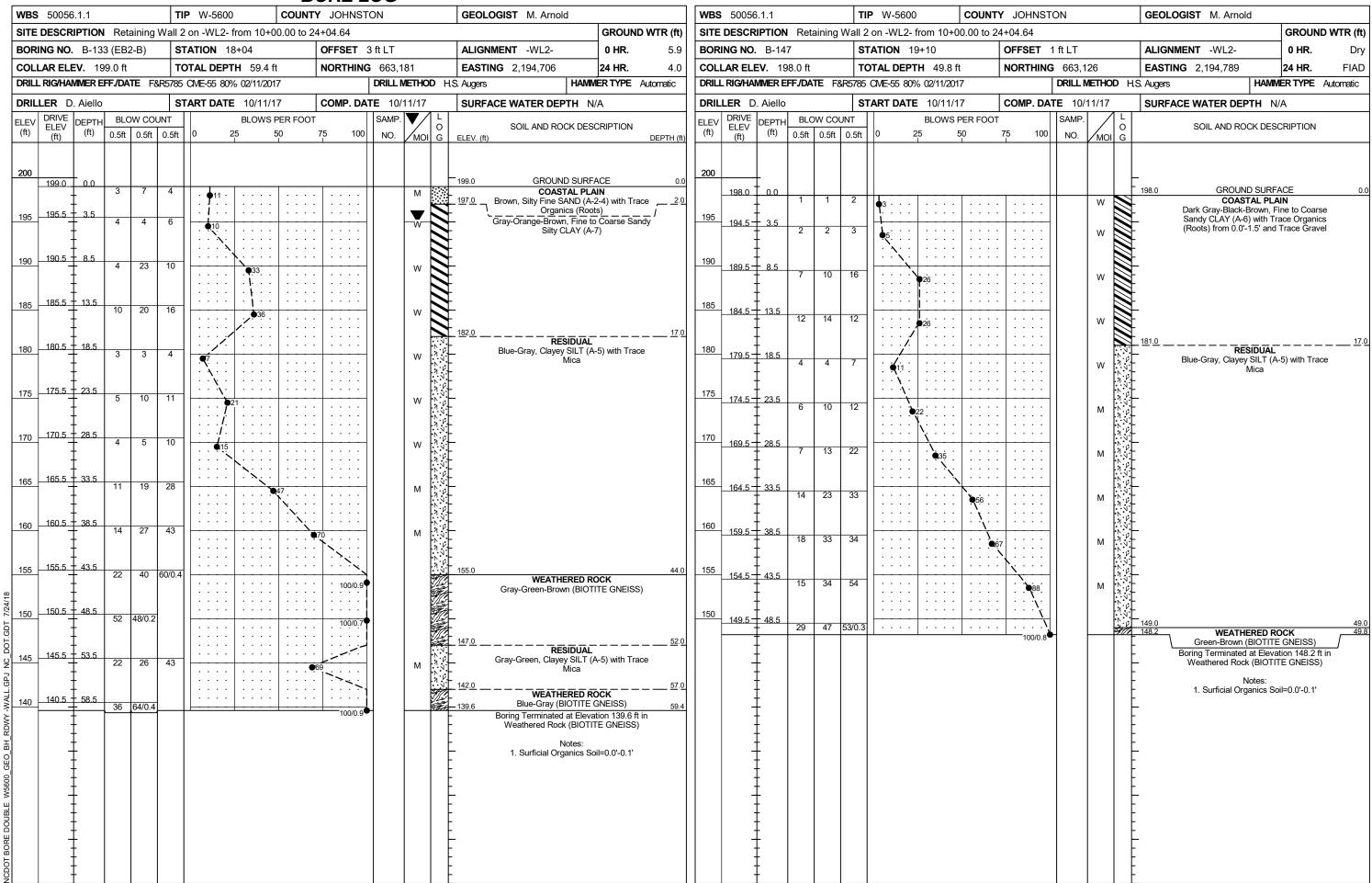
	BORE						
<b>WBS</b> 50056.1.1	TIP W-5600 COUNTY JOHNS	STON GEOLOGIST S. Woods		<b>WBS</b> 50056.1.1		JOHNSTON	GEOLOGIST S. Woods
SITE DESCRIPTION Retaining V	Vall 1 on -WL1- from 10+00.00 to 22+58.72		→ `´1 ⊦		Wall 1 on -WL1- from 10+00.00 to 22+5		GROUND WTR (ft)
BORING NO. B-119	STATION 21+57 OFFSET	38 ft LT ALIGNMENT -WL1-	<b>0 HR.</b> 15.3	BORING NO. B-118	STATION 22+58 OI	FFSET 37 ft LT	ALIGNMENT -WL1- 0 HR. NM
COLLAR ELEV. 208.0 ft			<b>24 HR.</b> 11.4	COLLAR ELEV. 209.1 ft	TOTAL DEPTH 20.0 ft NO	<b>ORTHING</b> 663,587	<b>EASTING</b> 2,194,016 <b>24 HR.</b> 15.0
DRILL RIG/HAMMER EFF/DATE F&F	R2175 CME-55 88% 02/11/2017	DRILL METHOD H.S. Augers HAMM	MER TYPE Automatic	DRILL RIG/HAMMER EFF/DATE F&	R2175 CME-55 88% 02/11/2017	DRILL METHOD ⊢	H.S. Augers HAMMER TYPE Automatic
DRILLER S. Davis	START DATE 10/11/17 COMP. D	DATE 10/11/17 SURFACE WATER DEPTH NA	i/A	DRILLER S. Davis	<b>START DATE</b> 10/11/17 <b>CO</b>	OMP. DATE 10/11/17	SURFACE WATER DEPTH N/A
ELEV CHU		SAMP. L O SOIL AND ROCK DESC NO. MOI G ELEV. (ft)	SCRIPTION DEPTH (ft)	ELEV CHI	<del> </del>	100 SAMP. L O MOI G	SOIL AND ROCK DESCRIPTION
210		208.0 GROUND SURFA		210 209.1 0.0 4 5	5		209.1 GROUND SURFACE 0.0 208.3 COASTAL PLAIN 0.8
208.0 0.0 4 5	9	M 207.0 COASTAL PLA Dark Brown, Silty Fine to C (A-2-4) with Trace O	AIN 1.0 Coarse SAND Drganics	205 205.6 + 3.5 4 7	11 11 118	SS-914 7%	Dark Brown, Silty Fine to Coarse SAND (A-2-4) with Trace Organics (Roots)  Red-Brown, Fine to Coarse Sandy CLAY (A-6)
200 199.5 8.5 4 6	8 . •14	Red-Gray, Fine Sandy Silt	ty CLAY (A-7)	200 200.6 + 8.5 5 7	9 16	м	
195 194.5 13.5 3 5	6			195	8 14	· · · · · · · · · · · · · · · · · · ·	192.1
190 189.5 18.5		RESIDUAL		190.6 + 18.5 3 4			RESIDUAL Tan-Orange, Fine Sandy Silty CLAY (A-7)
185 184.5 23.5	5				·   •8	W	Boring Terminated at Elevation 189.1 ft in CLAY (Residual)
	•   • • 12   · · · · · · · · · · · · · · · · · ·	Boring Terminated at Eleva CLAY (Residua	25.0 ation 183.0 ft in				- - -
		L Notes: 1. Surficial Organics So	il=0.0'-0.1'				
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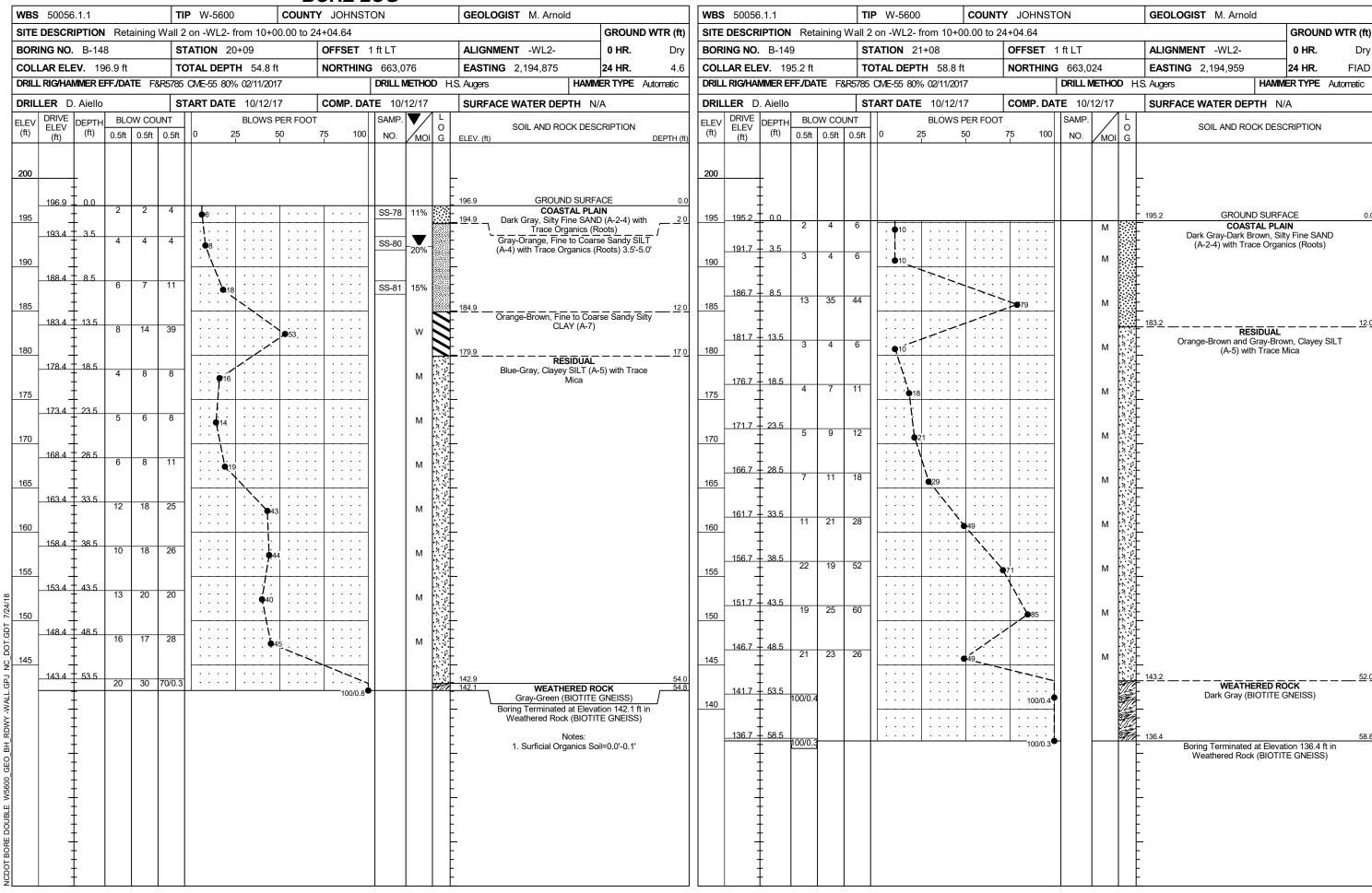


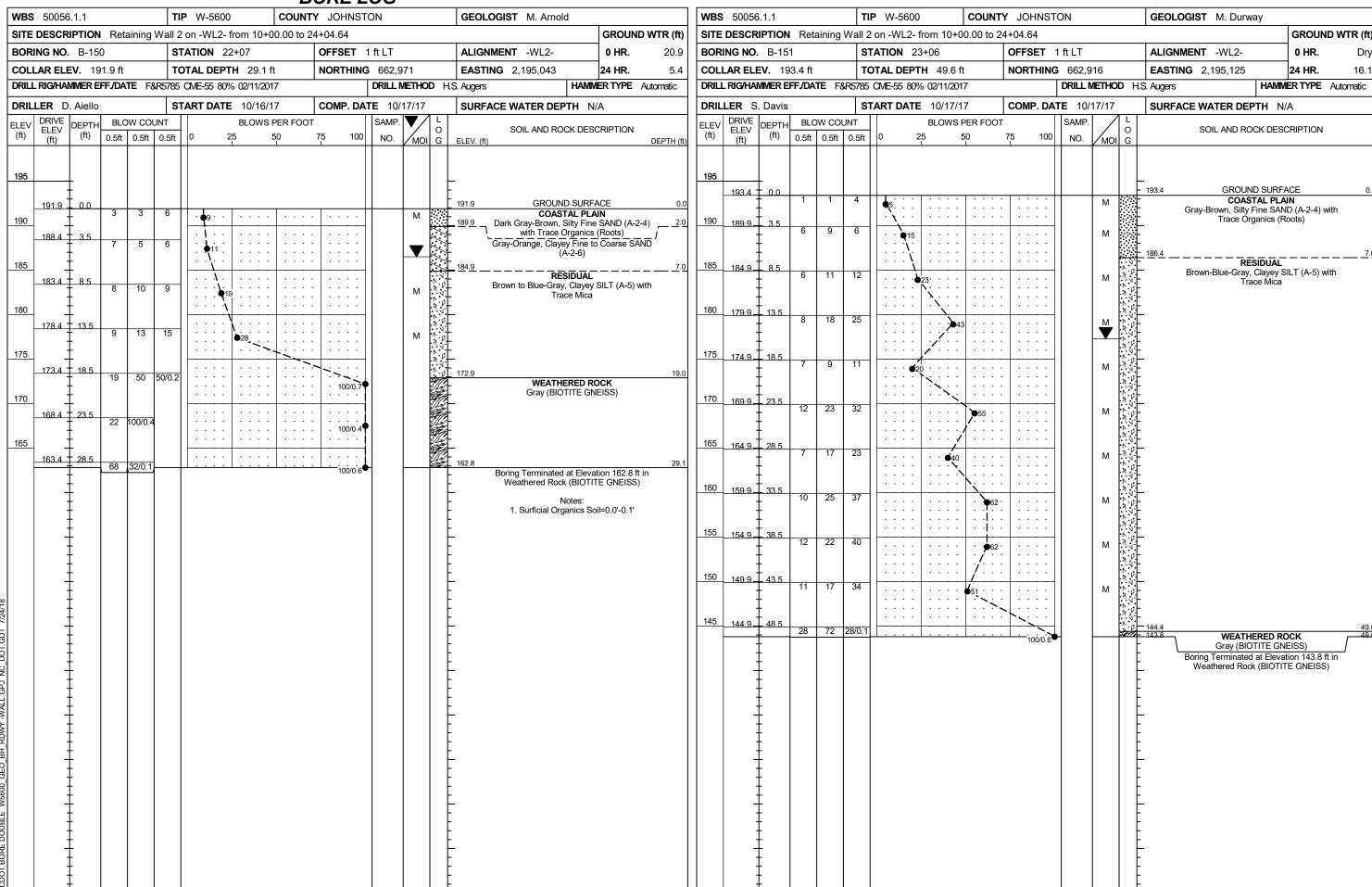












# North Carolina Department of Transportation Division of Highways Materials and Test Unit Soils Laboratory

T.I.P. ID NO.: W-5600

DESCRIPTION: US-70 Improvement from US 70 Business to the Neuse River Bridge

REPORT ON SAMPLES OF: SOIL FOR QUALITY

F&R PROJECT #: 66U-0197 COUNTY: Johnston

 DATE SAMPLED:
 9/17 to 10/17
 RECEIVED:
 10/17 to 12/17

 SAMPLED FROM:
 Various
 REPORTED:
 10/17 to 12/17

SUBMITTED BY: Cheng Wang BY: D. Jenks

Cert No. 101-02-0603

#### **TEST RESULTS**

PROJ. SAMPLE NO.	SS-1022	SS-79	SS-900	SS-941	SS-122	SS-123	ST-5	ST-5A
BORING NO.	B-144	B-148	B-129	B-118	B-130	B-130	B-124	B-124
Retained #4 Sieve %	0.0	NT	NT	NT	0.0	0.0	0.1	0.0
Passing #10 Sieve %	100.0	NT	NT	NT	99.9	100.0	8.8	100.0
Passing #40 Sieve %	86.9	NT	NT	NT	94.1	99.7	31.6	94.2
Passing #200 Sieve %	56.5	NT	NT	NT	57.3	68.6	59.5	67.2

SOIL MORTAR - 100%								
Coarse Sand Ret - #60 %	21.0	NT	NT	NT	11.8	3.1	16.9	12.0
Fine Sand Ret - #270 %	30.3	NT	NT	NT	44.4	40.1	29.2	28.1
Silt 0.053 - 0.010 mm %	33.9	NT	NT	NT	29.0	44.5	27.8	20.6
Clay < 0.010 mm %	14.8	NT	NT	NT	14.8	12.3	26.2	39.3
L.L.	19	NT	NT	NT	27	34	31	44
P.L.	NP	NT	NT	NT	NP	NP	12	18
P.I.	NP	NT	NT	NT	NP	NP	19	16
AASHTO Classification	A-4(0)	NT	NT	NT	A-4	A-4	A-6(8)	A-7-6(15)
Station	12+27	20+09	13+47	22+58	15+92	15+92	16+49	16+49
Offset	1'Lt	1'Lt	2'Lt	37'Lt	7'Lt	7'Lt	CL	CL
Depth (ft)	8.5	0.1	0.0	0.0	18.5	23.5	5.0	6.8
to	10.0	1.5	1.5	1.5	20.0	25.0	6.8	7.0
Alignment	-L-	-L-	-L-	-L-	-L-	-L-	-L-	-L-
Moisture Content (%)	18.4	11.3	12.3	7.2	27.1	19.7	15.6	15.6
Organic Content (%)	2.6	2.7	2.0	1.6	NT	NT	NT	NT

NP = Not plastic

NT = Not tested

ND = Not Determined

CL = Centerline

5600 Z REFERENCE **CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

TITLE SHEET

CROSS SECTIONS BORE LOG(S)

SOIL TEST RESULTS

SITE PLAN

PROFILE(S)

SHEET NO.

5-6

7-10

00056 S **PROIEC** 

# STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY **JOHNSTON** 

PROJECT DESCRIPTION US 70 IMPROVEMENTS FROM EAST OF US 70 BUSINESS TO WEST OF THE **NEUSE RIVER** 

SITE DESCRIPTION DUAL BRIDGES ON US 70 (-L-) OVER WILSON'S MILL ROAD (-Y9-, SR 1913) BETWEEN SR 1501 AND SR 1915

STATE PROJECT REPERENCE NO. W = 5600

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR NSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-680. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (INP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NIDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE TOTAL WITH THE ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

M. DURWAY S. DAVIS D. AIELLO

T. SHARPE

A. STURCHIO

M. ARNOLD

INVESTIGATED BY  $\_F \& R$ , Inc.

DRAWN BY \_T.T. WALKER CHECKED BY \_C. WANG

SUBMITTED BY \_P. ALTON

DATE MARCH 2018



#### Prepared in the Office of:

FROEHLING & ROBERTSON, INC.

Engineering Stability Since 1881

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SIGNATURE

**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

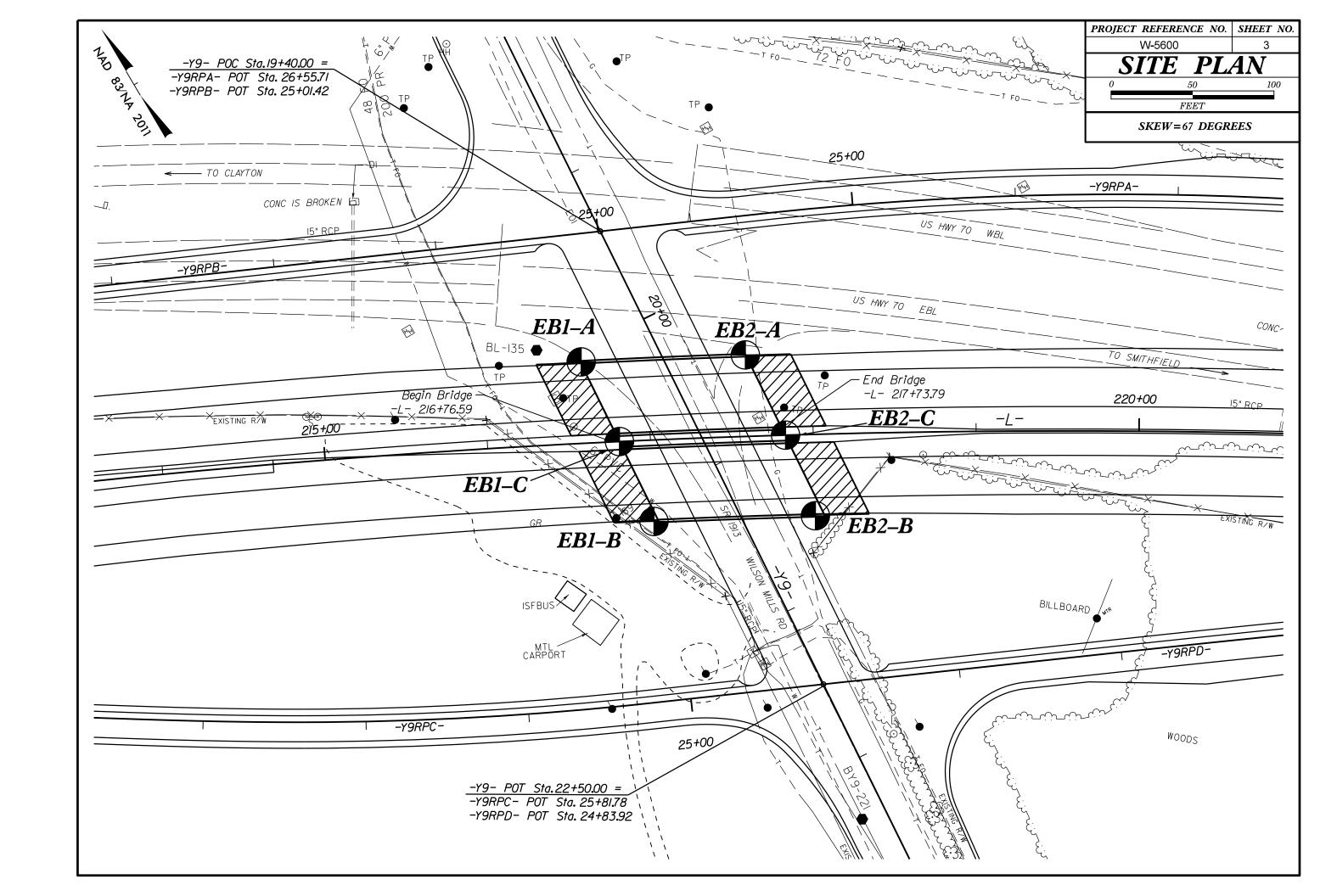
PROJECT REFERENCE NO.	SHEET NO.
W-5600	2

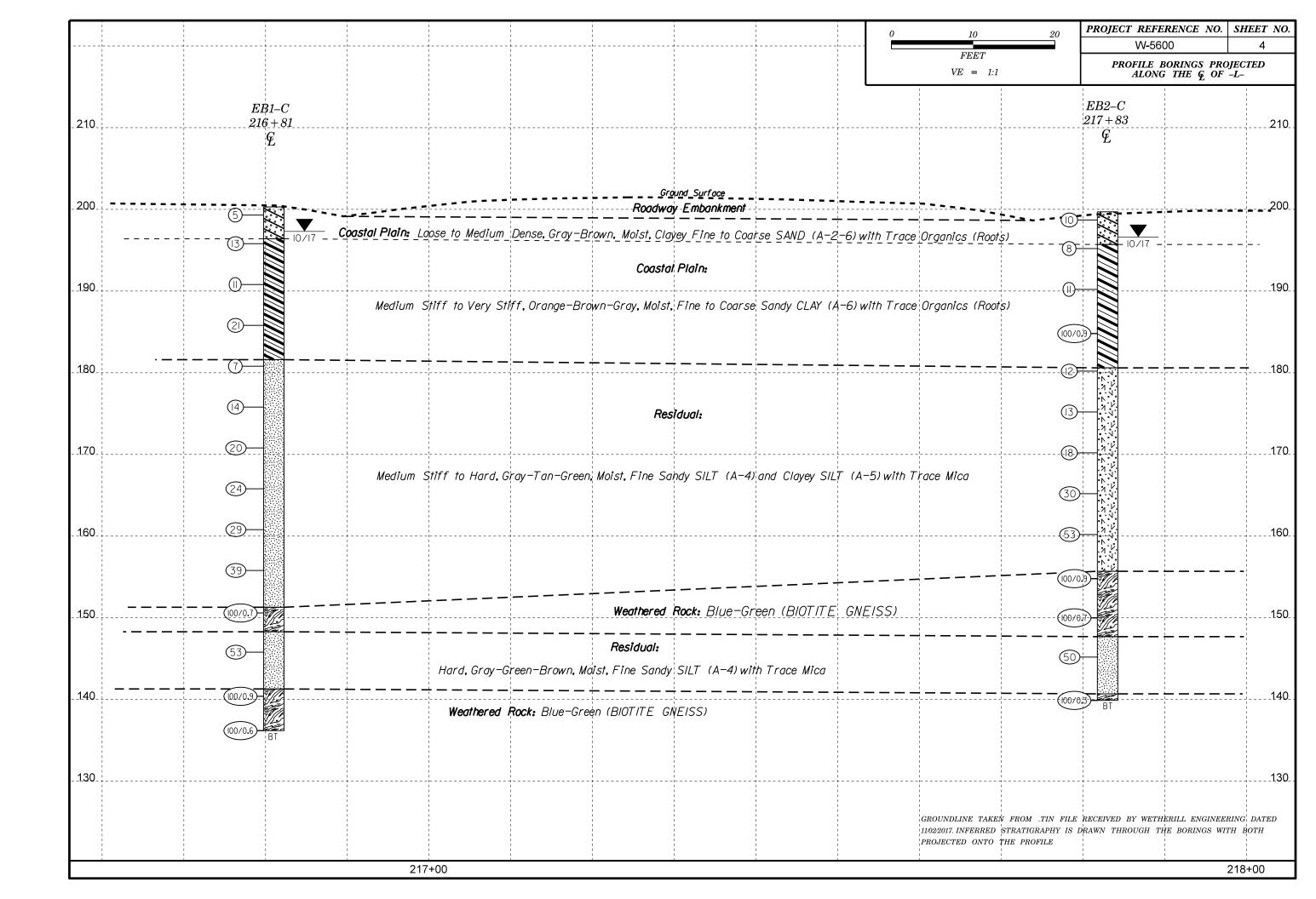
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

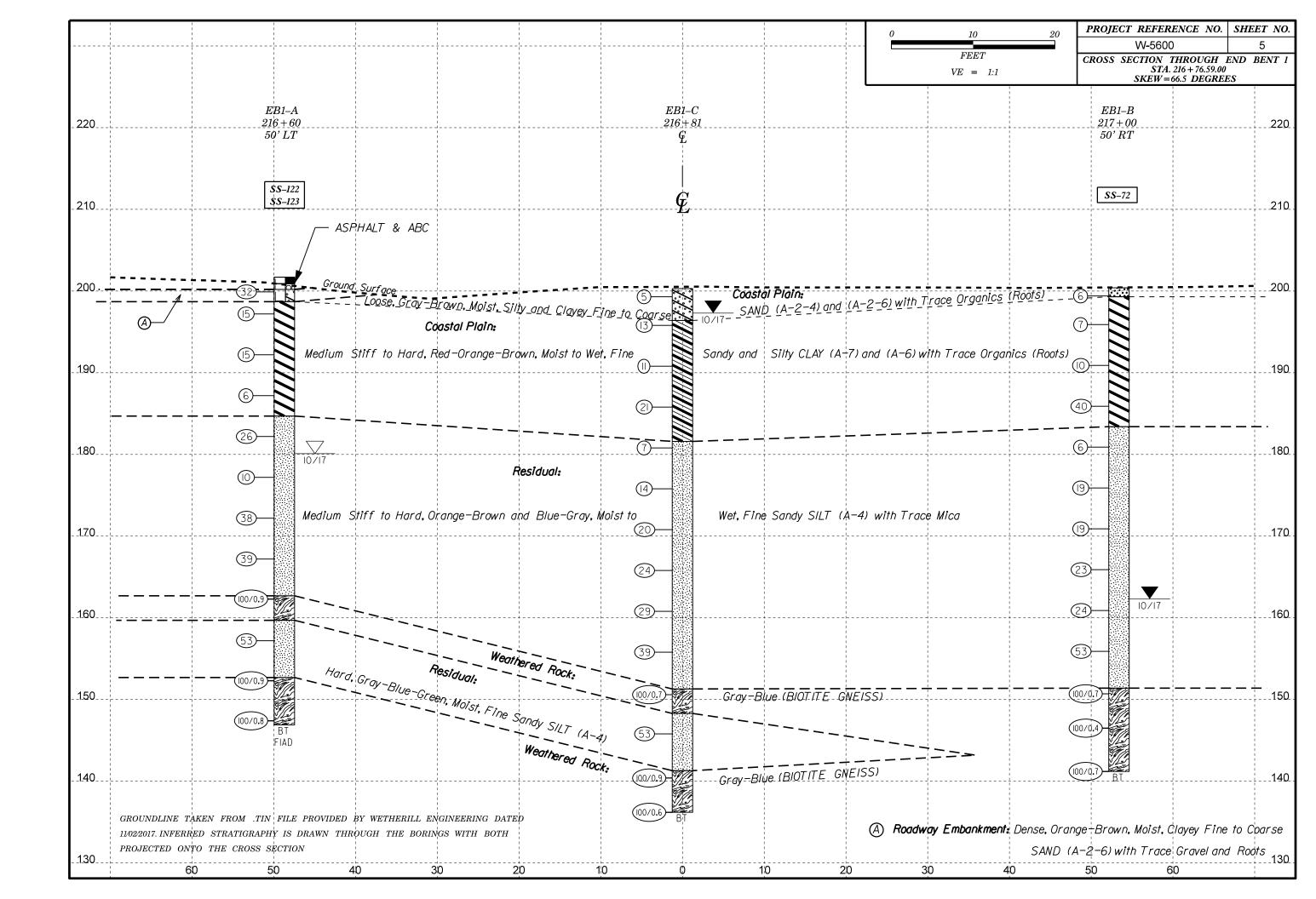
# SUBSURFACE INVESTIGATION

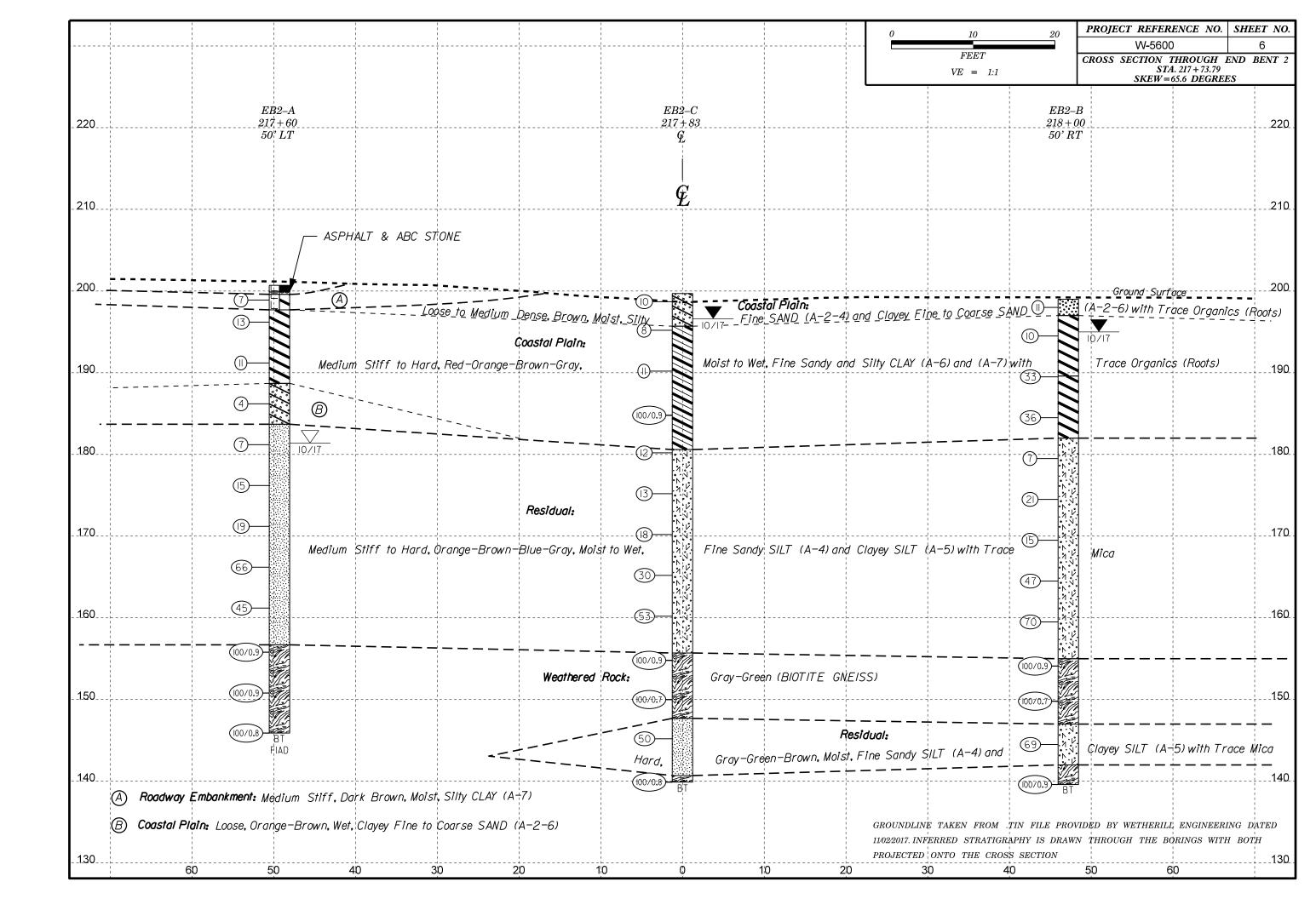
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

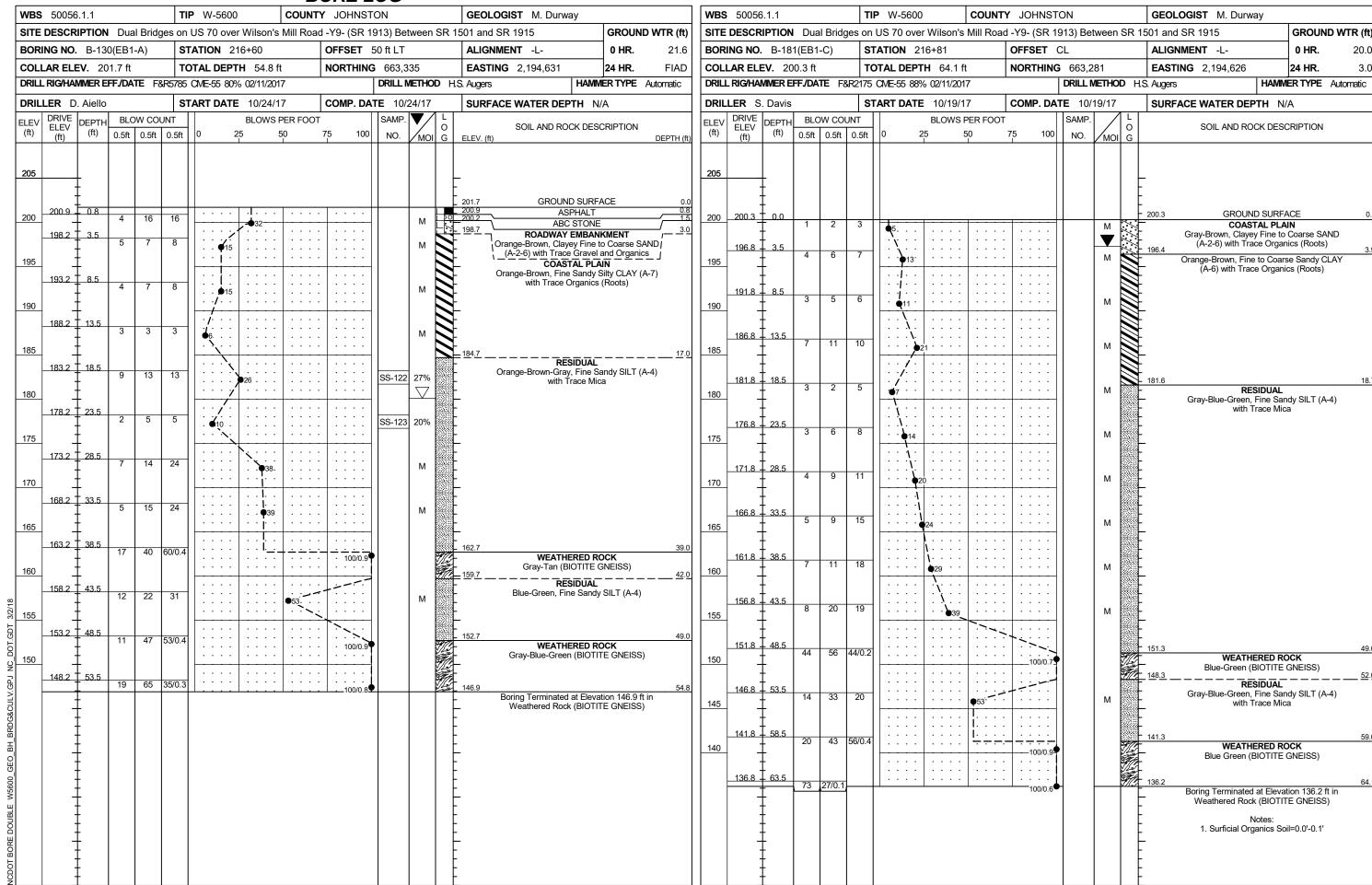
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DI586). SOIL CLASSIFICATION	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION  GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS	MINERALOGICAL COMPOSITION	CONCENTRALING FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (\$\leq 35% PASSING \(^2\)200) (> 35% PASSING \(^2\)200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE.	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	UNCISS, CHORNO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE ROCK (NCR)  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SANDSTONE, ETC.	COLLUYIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE,
SYMBOL 00000d0000d	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
% PASSING SILT-	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR GLAY MUCK, GRANULAR GLAY PEAT	PERCENTAGE OF MATERIAL	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
■200 15 MX 25 MX 18 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN 36 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.  DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL PACCING TO	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%  LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	HORIZONTAL.
PASSING *40   -   -   48 MX   41 MN   40 MX   41 MN   40 MX   41 MN   40 MX   41 MN   40 MX   41 MN   50ILS WITH   LITTLE OR	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP IW MX IW MX II MN II MN IW MX IW MX II MN II MN MODERATE OPPONIC	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOILS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAIOR CRAYEL AND FINE SILTY OR CLAYEY SILTY CLAYEY MATTER		(SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	▼ STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING EXCELLENT TO GOOD FAIR TO POOR POOR POOR UNSUITABLE	<u> </u>	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
AS SUBURADE PUUR	O-M SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30  CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
PANCE OF STANDARD PANCE OF UNICONSTITUTE	MISCELLANEOUS STABOLS	(MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTINESS OF PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE)  25/025  DIP & DIP DIRECTION  WITH SOIL DESCRIPTION  OF ROCK STRUCTURES	<u>IF TESTED, WOULD YIELD SPT REFUSAL</u>	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
VERY LOOSE ( 4	1 Y	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT  (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GRANIII AR LOOSE 4 TO 10	SOIL SYMBOL  OP DATE TEST BORING  SLOPE INDICATOR INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.  MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS
MATERIAL MEDIUM DENSE 10 10 30 N/A	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF  VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50	THAN ROADWAY EMBANKMENT THOUSEN BURING TEST	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25	— INFERRED SOIL BOUNDARY — CORE BORING SOUNDING ROD	(V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	WITH CORE	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4	ALLUVIAL SOIL BOUNDARY ALLUVIAL SOIL BOUNDARY SPT N-VALUE	ALSO AN EXAMPLE.	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNSUITABLE WASTE  SHALLOW  UNCLASSIFIED EXCAVATION -  USED IN THE TOP 3 FEET OF  SHALLOW  UNCLASSIFIED EXCAVATION -	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\dot{\gamma}_{ m d}$ - DRY UNIT WEIGHT	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION  (ATTERBERG LIMITS) DESCRIPTION	CSE COARSE ORG ORGANIC  DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	PIECES CAN BE BROKEN BY FINGER PRESSURE.	
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO   SD SAND, SANDY   SS - SPLIT SPOON   F - FINE   SL SILT, SILTY   ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK DUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
PLASTIC   SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	FRAC FRACTURED, FRACTURES TO TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	
PLASTIC LIMITATTAIN OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	BENCH MARK: BL-I35= N: 663354.9350, E: 2194610.616, -L- STA. 216+33.31 58.59' LT
- MOICT - (M) COLID. AT OR NEAR ORTIMUM MOICTURE	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: 200.72 FEET
OM _ OPTIMUM MOISTURE  SL _ SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	NOTES:
REQUIRES ADDITIONAL WATER TO	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	FIAD= FILLED IMMEDIATELY AFTER DRILLING
- UKY - (U) ATTAIN OPTIMUM MOISTURE	X CME-55 G* CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	NM= NOT MEASURED
PLASTICITY	X 8* HOLLOW AUGERS	INDURATION	THE TOT MEASURED
PLASTICITY INDEX (PI) DRY STRENGTH	CME-550 HARD FACED FINGER BITS	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NON PLASTIC Ø-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST TUNGCARBIDE INSERTS	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM	CASING W/ ADVANCER POST HOLE DIGGER	CRAING CAN BE SERVATED FROM CAMBLE WITH STEEL PROBE.	
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	MODERATELY INDURATED GRAINS CHING BE SEPARATED FROM SHIFTER WITH STEEL FROME; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT VANE SHEAR TEST	DIFFICULT TO BREAK WITH HAMMER.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14





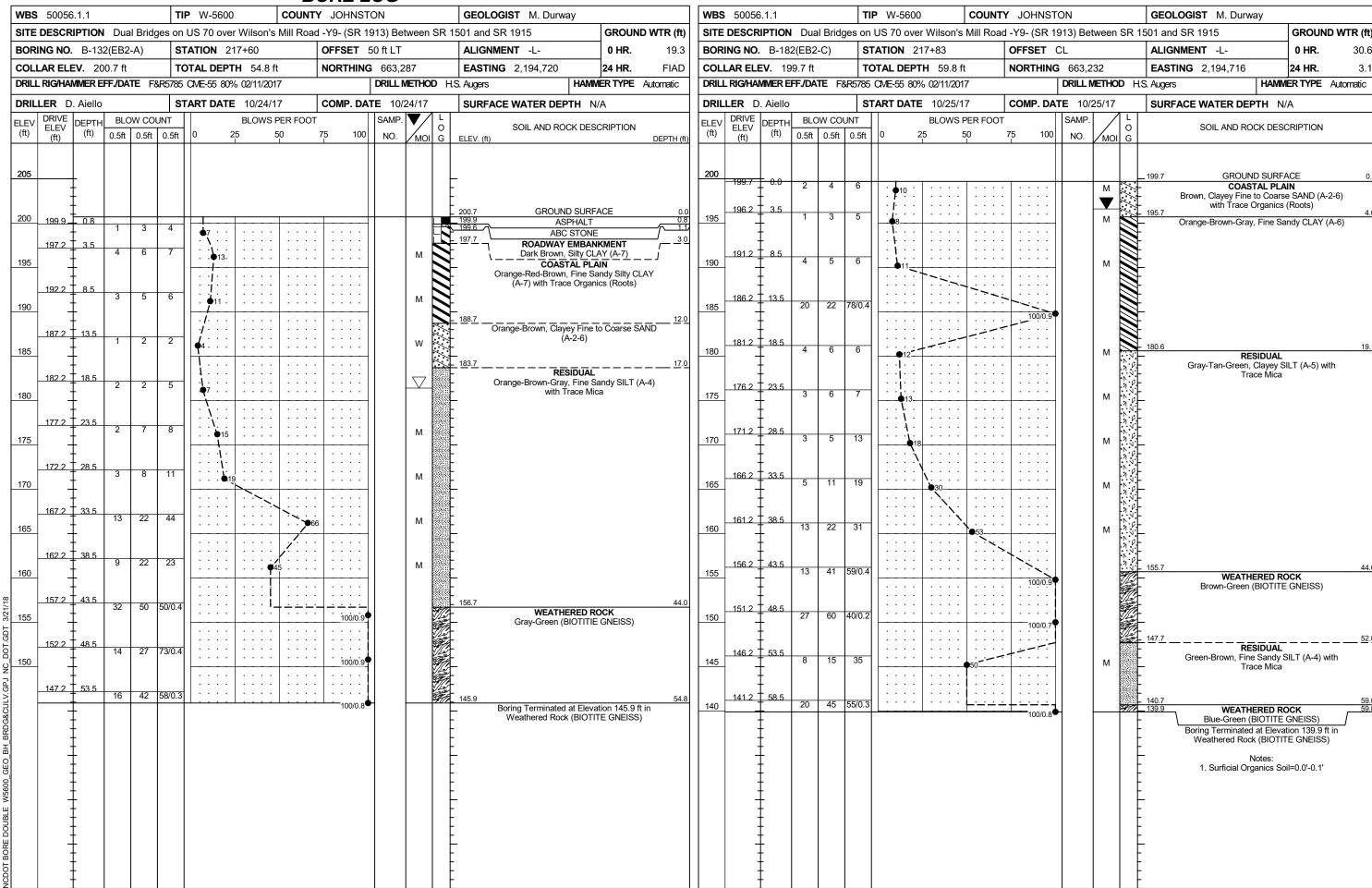






SHEET 8

COLLAR ELEV.   20.4 ft   TOTAL DEPTH   59.2 ft   NORTHING   663,228   EASTING   2.194.619   24 MR   36.1								B	<u>ORE L</u>	<u>OG</u>						
BORING NO. B-131(EB1-B)   STATION 217+00   OFFSET 50 R.RT   ALIGNMENT	WBS	50056	5.1.1			TI	<b>P</b> W-5600	COUNTY	/ JOHNST	ON			GEOLOGIST M. Arnol	d		
COLLAR ELEY. 20.4 ft   TOTAL DEPTH 59.2 ft   NORTHING 663.228   EASTING 2,194.619   24 Hz   38.1    FIGURI INCHAMMER EPFADATE RISKS CIAES 50% (2012D) T   COMP. DATE 101017    FIGURE RISKS CIAES 50%	SITE	DESCR	IPTION	<b>I</b> Dua	al Brid	ges on	US 70 over Wilson's	Mill Road	-Y9- (SR 1	913) Be	tween	SR 1	501 and SR 1915		GROUN	D WTR (ft)
DRILLER D. Alelo   START DATE   10/10/17   COMP. DATE   10/10/17   SURFACE WATER DEPTH   NA	BOR	ING NO.	B-13	1(EB1	-B)	S	<b>TATION</b> 217+00		OFFSET 5	50 ft RT			ALIGNMENT -L-		0 HR.	58.0
DRILLER   D. Aiolio   START DATE 10/10/17   COMP. DATE 10/10/17   SURFACE WATER DEPTH IN/A	COL	LAR ELE	<b>EV</b> . 20	0.4 ft		TO	OTAL DEPTH 59.2 ft		<b>NORTHING</b> 663,228				<b>EASTING</b> 2,194,619		24 HR.	38.1
BLOW   DEPTH   BLOW COUNT   RLOWS PER POOT   RLOWS PER	DRILL	L RIG/HAI	MMER E	FF./DA	TE F	&R5785	CME-55 80% 02/11/201	7		DRILL N	/IETHC	D H.	S. Augers	HAMME	R TYPE	Automatic
Column   C	DRIL	.LER D	. Aiello			S	TART DATE 10/10/1	7	COMP. DA	<b>ΓΕ</b> 10/	10/17		SURFACE WATER DEP	TH N/A	4	
100   100   0.5		DRIVE ELEV					1 1			SAMP.	lacktriangledown/		SOIL AND RO	CK DESC	RIPTION	
200 2004 0.0 1 3 3 3 1 1 1 3 3 3 1 1 1 1 1 1 1 1 1	(π)		(π)	0.5ft	0.5ft	0.5ft	0 25 5	i 	75 100	NO.	/MOI		ELEV. (ft)			DEPTH (ft)
200 2004 0.0 1 3 3 3 1 1 1 3 3 3 1 1 1 1 1 1 1 1 1																
195	205	-	_										<del>_</del>			
195		-											•			
195	200	200.4	0.0	1	3	3		<b>.</b>				-				0.0
195 190 191 192 193 194 195 196 197 198 198 198 198 198 198 198 198 198 198		-		'		"	6				М		Gray-Brown, Silty F	ine SANE	O (A-2-4) v	
190		196.9	3.5	4	4	3	7				w		Red-Orange-Brown	, Fine to 0	Coarse Sa	ndy
185	195	-	_				<del>                                   </del>						_ Silty C	CLAY (A-7	)	
180  181.9  181.		191.9	- - 8.5				] : [ : : : ] : : : : [									
185	190	_	L	1	5	5	. 10				W		<del>-</del>			
185		-	_													
183	405	186.9 -	_ 13.5	10	17	23					М					
181 9 18.5	185	-	_						1				- · 183./			17.0
176.9 23.5 4 7 12 19 9 10 171.9 28.5 4 9 10 13 166.9 33.5 4 10 13 161.9 38.5 5 11 13 13 161.9 38.5 17 47 53/0.2 100/0.7		181.9	_ _ 18.5										RES			
170  170  171  170  170  166.9 33.5 4 10 13	180	_	_	2	3	3	<b>●</b> 6· · · · · · ·			SS-72	20%				~-4) WILIT I	race
170  170  171  170  170  166.9 33.5 4 10 13		-	_													
171.9 28.5 4 9 10 9 10 166.9 33.5 4 10 13 23 W  160 161.9 38.5 5 111 13 22 31 W  156.9 43.5 11 22 31	475	176.9 -	23.5	4	7	12	19				w		•			
166.9 33.5 4 10 13 23 W  161.9 38.5 5 11 13 22 31 W  155.9 48.5 17 47 53/0.2 100/0.4 141.2 ft in Weathered Rock (BIOTITE GNEISS)  141.9 58.5 50 50/0.2 Boring Terminated at Elevation 141.2 ft in Weathered Rock (BIOTITE GNEISS)  160 166.9 33.5 4 10 13 4 10 13 4 10 13 4 14.2 ft in Weathered Rock (BIOTITE GNEISS)  150 151.9 48.5 50 50/0.2 100/0.4 50/0.2 100/0.7 141.2 ft in Weathered Rock (BIOTITE GNEISS)	1/5	-	_						<del>   </del>				<del>_</del>			
165		171.9	28.5													
165	170		Ļ	4	9	10	19				W		• <del>-</del>			
165		-	<u> </u>													
161.9 38.5 5 11 13 24 W  156.9 43.5 11 22 31 W  151.9 48.5 17 47 53/0.2 100/0.7 141.2 111.	165	166.9 -	_ 33.5	4	10	13					W		•			
156.9 43.5 11 22 31	100	-	-										<del>-</del> ·			
156.9 43.5 11 22 31		161.9	- - 38.5		14	10					V		•			
155	160		_	5	11	13	24				W		• <del>-</del>			
155			<u> </u>													
151.9 48.5 17 47 53/0.2 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 100/0.7 141.2 59 100/0.7 100/0.7 141.2 ft in Weathered Rock (BIOTITE GNEISS)  Notes:	155	156.9 -	<u>43.5</u>	11	22	31		53			М		•			
150	100	† -	<del> </del>						<u> </u>				<del>-</del> ·			
150		151.9	48.5	17	17	52/0.0										49.0
146.9 53.5 100/0.4 100/0.4 100/0.4 141.2 59  141.9 58.5 50 50/0.2 100/0.7 Boring Terminated at Elevation 141.2 ft in Weathered Rock (BIOTITE GNEISS)  Notes:	150	-	<b>†</b>	''	4/	03/0.2			100/0.7	1			WEATH			
141.9 58.5 50 50/0.2 100/0.7 141.2 Boring Terminated at Elevation 141.2 ft in Weathered Rock (BIOTITE GNEISS)  Notes:		1400	<u></u>												-,	
141.9 58.5 50 50/0.2 141.2 Boring Terminated at Elevation 141.2 ft in Weathered Rock (BIOTITE GNEISS)  Notes:	145	146.9	53.5	100/0.4	1								•			
50 50/0.2  100/0.7  Boring Terminated at Elevation 141.2 ft in Weathered Rock (BIOTITE GNEISS)  Notes:	170	1 -	<del> </del>										<del>-</del> ·			
Boring Terminated at Elevation 141.2 ft in Weathered Rock (BIOTITE GNEISS)  Notes:		141.9	58.5	50	50/0.2	,			1 1				· 141.2			59.2
T Notes:		_	-	50	30/0.2	1	'		100/0.7				Boring Terminated	at Elevati	on 141.2	t in
1. Surficial Organics Soil=0.0'-0.1'		-	F											•		,
		:	ļ.										1. Surficial Org	anics Soil	=0.0'-0.1'	
		-	F										<del>-</del> ·			
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SHEET 10

							В	ORE L	OG						
WBS	50056	5.1.1			TI	<b>P</b> W-5600	COUNT	Y JOHNST	ON			GEOLOGIST M. Arnol	d		
SITE	DESCR	IPTION	<b>I</b> Dua	al Brid	ges on	US 70 over Wilson's	Mill Road	d -Y9- (SR 1	913) Be	tween	SR 1	501 and SR 1915		GROUN	D WTR (ft)
BOR	ING NO.	B-13	3 (EB	2-B)	S	<b>FATION</b> 218+00		OFFSET	50 ft RT			ALIGNMENT -L-		0 HR.	5.9
	LAR ELE					OTAL DEPTH 59.4 f		NORTHING				<b>EASTING</b> 2,194,706		24 HR.	4.0
DRILL	_ RIG/HAI	MMER E	FF./DA	TE F	&R5785	CME-55 80% 02/11/201	7		DRILL	METHO	D H.	S. Augers	HAMM	ER TYPE	Automatic
DRIL	LER D	. Aiello				TART DATE 10/11/1		COMP. DA			<i>a</i>	SURFACE WATER DEF	PTH N/	4	
ELEV (ft)	DRIVE ELEV	DEPTH (ft)	-	OW CO			PER FOOT		SAMP.	/	0	SOIL AND RO	CK DESC	RIPTION	
(11)	(ft)	(11)	0.5ft	0.5ft	0.5ft	0 25	50	75 100	NO.	/MOI	G	ELEV. (ft)			DEPTH (ft)
200	199.0	0.0				<u> </u>							D SURFA		0.0
	-	‡	3	7	4	:   11 :   : : : :				М		_ 197.0 _ Brown, Silty Fine S		-4) with Tr	ace2.0
195	195.5	3.5	4	4	6	· I · · · · · ·				W_		Organ Organ Organ Organ Organ	ics (Roots		<i>J</i> andv
	-	‡		`						**		Silty C	LAY (A-7	)	
400	190.5	8.5													
190	-100.0	- 0.0	4	23	10			<del>   </del>		w		-			
	-	<u> </u>				:::: : : ::									
185	185.5	13.5	10	20	16		<u> </u>			l w		_			
	-	‡				36 · · · · · · · · · · · · · · · · · · ·				''		182.0			17.0
180	180.5	18.5				::;/:/ ::::					177	RE	SIDUAL		
100	-	†	3	3	4	.47		1		W	1,1	_ Blue-Gray, Clayey	Mica	o) with the	ice
	-	‡				: 1/2 :   : : : :					1,11				
175	175.5	23.5	5	10	11	21	ļ · · · · ·	<u> </u>		l w	1,11	_			
	-	†									1,1,1				
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170	-	†	4	5	10			1		W	1,1	-			
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165	165.5	33.5	11	19	28		47	ļ · · · · ·		М		-			
	-	†													
160	160.5	38.5	<b>.</b>		10										
	-	Ŧ	14	27	43		: : : <b>`</b> •	170		M		-			
	-	Ī					: : : :								
155	155.5	43.5	22	40	60/0.4		ļ · · · · ·				17		ERED RO	СК	44.0
	-	Ŧ					: : : :	100/0.9	Ί			Gray-Green-Brow			S)
150	150.5	48.5	F2	40/0.0			: : : :								
	-	E	52	48/0.2				100/0.7	'			_			
	1455	F2.5											SIDUAL		<u>52</u> .0
145	145.5	53.5	22	26	43			69		М		Gray-Green, Claye		-5) with Tr	ace
	-	<u> </u>									17.17	142.0			57.0
140	140.5	58.5	36	64/0.4								WEATHI - 139.6 Blue-Gray (B	ERED RO		59.4
	-		"	04/0.4			'	100/0.9	7			Boring Terminated Weathered Rock	at Elevat	ion 139.6	ft in
	-	<u> </u>											lotes:	L ONLIGO	'
	-	<u> </u>										1. Surficial Org		=0.0'-0.1	
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# North Carolina Department of Transportation Division of Highways Materials and Test Unit Soils Laboratory

T.I.P. ID NO.: W-5600

DESCRIPTION: Dual Bridges on US 70 over Wilson's Mill Road -Y9- (SR 1913) between SR 1501 and SR 1915

REPORT ON SAMPLES OF: SOIL FOR QUALITY

F&R PROJECT #: 66U-0197 COUNTY: Johnston

 DATE SAMPLED:
 9/17 to 10/17
 RECEIVED:
 10/17 to 12/17

 SAMPLED FROM:
 Various
 REPORTED:
 10/17 to 12/17

SUBMITTED BY: Cheng Wang BY: D. Jenks

Cert No. 101-02-0603

#### **TEST RESULTS**

PROJ. SAMPLE NO.	SS-122	SS-123	SS-72						
BORING NO.	B-130	B-130	B-131						
	EB1-A	EB1-A	EB1-B						
Retained #4 Sieve %	0.0	0.0	0.0						
Passing #10 Sieve %	99.9	100.0	100.0						
Passing #40 Sieve %	94.1	99.7	99.2						
Passing #200 Sieve %	57.3	68.6	57.8						

SOIL MORTAR - 100%									
Coarse Sand Ret - #60 %	11.8	3.1	6.6						
Fine Sand Ret - #270 %	44.4	40.1	46.0						
Silt 0.053 - 0.010 mm %	29.0	44.5	25.2						
Clay < 0.010 mm %	14.8	12.3	22.2						
L.L.	27	34	38						
P.L.	NP	NP	33						
P.I.	NP	NP	5						
AASHTO Classification	A-4	A-4	A-4(2)						
Station	216+60	216+60	217+00						
Offset	50'Lt	50'Lt	50'Rt						
Depth (ft)	18.5	23.5	18.5						
to	20.0	25.0	20.0						
Alignment	-L-	-L-	-L-						
Moisture Content (%)	27.1	19.7	20.4	 	 				 
Organic Content (%)	NT	NT	NT						

NP = Not plastic

NT = Not tested

ND = Not Determined

CL = Centerline

W.P. Alton, P.E.

Soils Engineer

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**CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

SUPPLEMENTAL LEGEND (GSI)

BORE LOGS, CORE REPORTS, & CORE PHOTOGRAPHS

TITLE SHEET

SITE PLAN PROFILE(S)

CROSS SECTION(S)

SOIL TEST RESULTS ROCK TEST RESULTS

SHEET NO.

2Α

5-7

# 0056 S **PROIEC**

### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY **JOHNSTON** 

PROJECT DESCRIPTION US 70 IMPROVEMENTS FROM EAST OF US 70 BUSINESS TO WEST OF THE *NEUSE RIVER* 

SITE DESCRIPTION BRIDGE ON SWIFT CREEK ROAD (-Y7-, SR 1501) OVER US 70 (-L-) BETWEEN SR 1913 AND SR 1907

STATE PROJECT REPERENCE NO. 19 W = 5600

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR NSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-680. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

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  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

M. DURWAY S. DAVIS D. AIELLO T. SHARPE A. STURCHIO S. WOODS INVESTIGATED BY  $F \otimes R$ , Inc. DRAWN BY \_T.T. WALKER

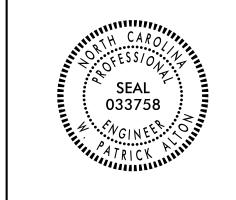
> CHECKED BY \_C. WANG SUBMITTED BY \_P. ALTON

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PROJECT REFERENCE NO.	SHEET NO.
W-5600	2

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

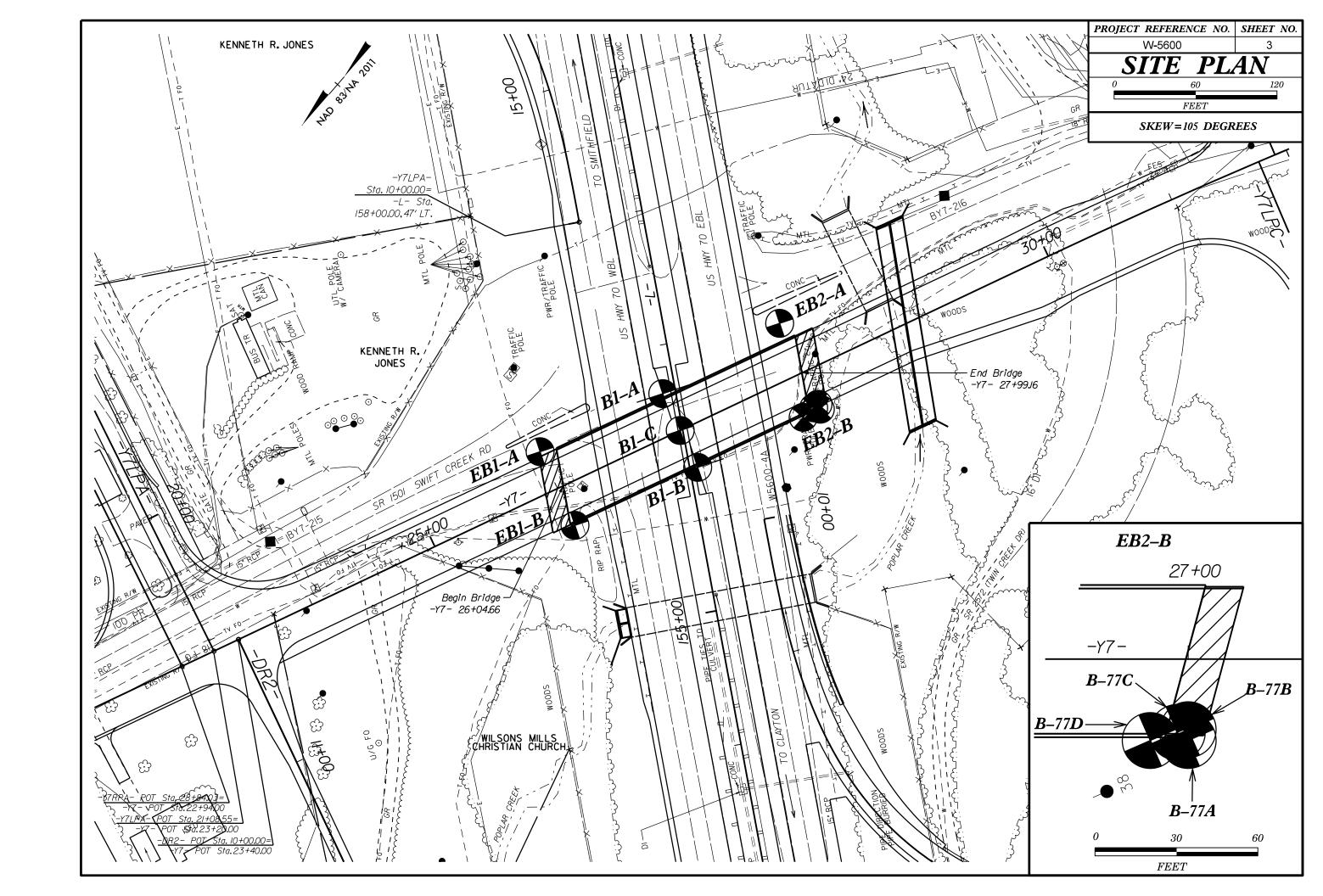
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS	
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DI586). SOIL CLASSIFICATION	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.  GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.  AQUIFER - A WATER BEARING FORMATION OR STRATA.	
IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	ANGULARITY OF GRAINS  THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	BELOWS IN NOW-COMSTRIC THE TIME THE THE THE THE THE THE THE THE TOWN BETWEEN SOLD HIND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK, ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.  ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING	
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAVERS, HIGHLY PLASTIC, A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.  MINERALOGICAL COMPOSITION	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.  FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.  ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT  WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND	
GRANULAR MATERIALS   SILT-CLAY MATERIALS   ORGANIC MATERIALS   CLASS. (≤ 35% PASSING "2000) (> 35% PASSING "2000)   ORGANIC MATERIALS   ORGANIC	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	CHYSTALLINE ROCK (CR)  WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	SURFACE.  CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.	
CLASS, A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-3 A-6, A-7  SYMBOL 888888888888888888888888888888888888	COMPRESSIBILITY  SLIGHTLY COMPRESSIBLE LL < 31	NON-CRYSTALLINE ROCK (NCR)  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SANDSTOME, ETC.	COLLUYIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.	
3 FROM 838888883	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50  PERCENTAGE OF MATERIAL	COASTAL PLAIN  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD  SEDIMENTARY ROCK  SPT REFUSAL ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED  CP)  SHELL BEDS, ETC.	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.	
*40 30 MX 50 MX 51 MN   SOILS SOILS PEAT	GRANULAR SILT - CLAY	WEATHERING	<u>DIKE</u> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.	
15 MA 25 MA 18 MA 35 MA 36 MA	ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL  TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE I - 10%  LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	${ m DIP}$ - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.	
PASSING *40	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	VERY SLIGHT ROCK CENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	<u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.	
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOILS	GROUND WATER  WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.	
OSMALT TIPES STUDE PRIESS. FINE SILTY OR CLAYEY SILTY CLAYEY OF MAJOR GRAVEL, AND SAND CRAYEL AND SAND SOILS SOILS  MATERIALS SAND SAND CRAYEL AND SAND SOILS	▼ STATIC WATER LEVEL AFTER <u>24</u> HOURS	CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.  MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN  MODD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.  FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.	
GEN. RATING AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA  SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.	
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 :PI OF A-7-6 SUBGROUP IS > LL - 30  CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	<u>FORMATION (FM.)</u> - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.	
COMPACTNESS OF RANGE OF STANDARD RANGE OF UNCONFINED	III 25/025	(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.  IF TESTED, WOULD YIELD SPT REFUSAL	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.	
PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (N-VALUE) (TONS/FTZ)	ROADWAY EMBANKMENT (RE)  POR DIP & DIP DIRECTION  OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT	<u>LEDGE</u> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.	
GENERALLY VERY LOOSE < 4  LOOSE 4 TO 10	SOIL SYMBOL  SPT OFT ONT TEST BORING SLOPE INDICATOR INSTALLATION	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.	
GRANULAR   MEDIUM DENSE	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANMENT AUGER BORING CONE PENETROMETER	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	MOTILED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.	
VERY SOFT < 2 < 0.25	INFERRED SOIL BOUNDARY	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRACMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTICES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.	
GENERALLY   SOFT   2 TO 4   0.25 TO 0.5	INFERRED ROCK LINE MN MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.  ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF	
(COHESIVE)         VERY STIFF         15 TO 30         2 TO 4           HARD         > 30         > 4	PIEZOMETER SPT N-VALUE	ALSO AN EXAMPLE.  ROCK HARDNESS	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.	
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.	
U.S. STD. SIEVE SIZE 4 10 40 60 200 270  OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053  COARSE FINE 0.17 0.44	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  SHALLOW UNCLASSIFIED EXCAVATION - UNCLASSI	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.	
BOULDER         COBBLE (CDB,)         GRAVEL (GR.)         SAND (CSE, SD.)         SAND (SL.)         SLT (CLY)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	<u>SLICKENSIDE</u> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.	
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN. 12 3	AR - AUGER REFUSAL MED MEDIUM YST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7' - UNIT WEIGHT	BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL	
SOIL MOISTURE - CORRELATION OF TERMS  SOIL MOISTURE SCALE FIELD MOISTURE COURS TO SEE AMOUNT OF TERMS	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\hat{\gamma}_{d}$ - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY	
(ATTERBERG LIMITS) DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u> DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL	
- SATURATED - USUALLY LIQUID, VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE  LL LIQUID LIMIT	e - VOID RATIO   SD SAND, SANDY   SS - SPLIT SPOON   F - FINE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES I INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.	
PLASTIC RANGE  (PI) PL PLASTIC LIMIT  - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS # - MOISTURE CONTENT CBR - CALIFORNIA BEARING HI HIGHLY V - VERY RATIO	FINGERNAIL. FRACTURE SPACING BEDDING	<u>TOPSOIL (TS.)</u> - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.  BENCH MARK: W-5600 4A= N: 665822.07, E: 2189197.88, -Y7- STA. 27+52.78,	
	EQUIPMENT USED ON SUBJECT PROJECT	TERM SPACING TERM THICKNESS  VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	71.57' RT ELEVATION: 193.2 FEET	
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL _ SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	NOTES:	
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	CME-45C CLAY BITS X AUTOMATIC MANUAL  6' CONTINUOUS FLIGHT AUGER  CORE SIZE:	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET  VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 F. 0.03 FEET  THINLY LAMINATED 0.008 FEET	FIAD= FILLED IMMEDIATELY AFTER DRILLING	
PLASTICITY	X CME-55   X 8" HOLLOW AUGERS   CORE SIZE:   -H	INDURATION	NM= NOT MEASURED	
PLASTICITY INDEX (PI) DRY STRENGTH	CME-550 HARD FACED FINGER BITS X-N Q	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.		
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	TUNGCARBIDE INSERTS	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.		
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	CASING W/ ADVANCER POST HAND TOULS!  PORTABLE HOIST TRICONE 'STEEL TEETH WAND AUSED.	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.		
COLOR	TRICONE TUNGCARB. SOUNDING ROD	INDUPATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;		
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	X CORE BIT VANE SHEAR TEST	DIFFICULT TO BREAK WITH HAMMER.  EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;  SAMPLE BREAKS APPROS GRAINS	DATE: 8-15-14	

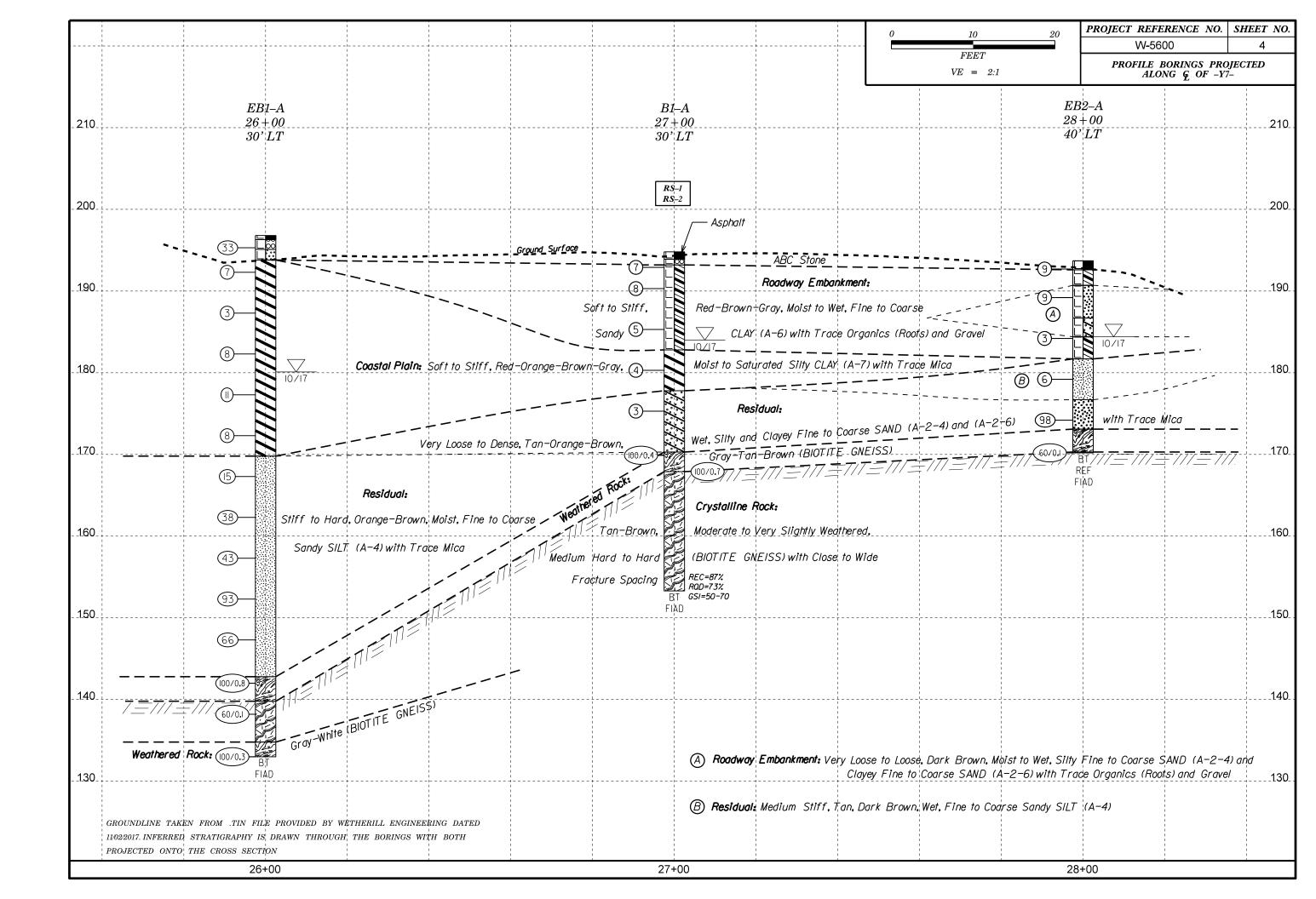
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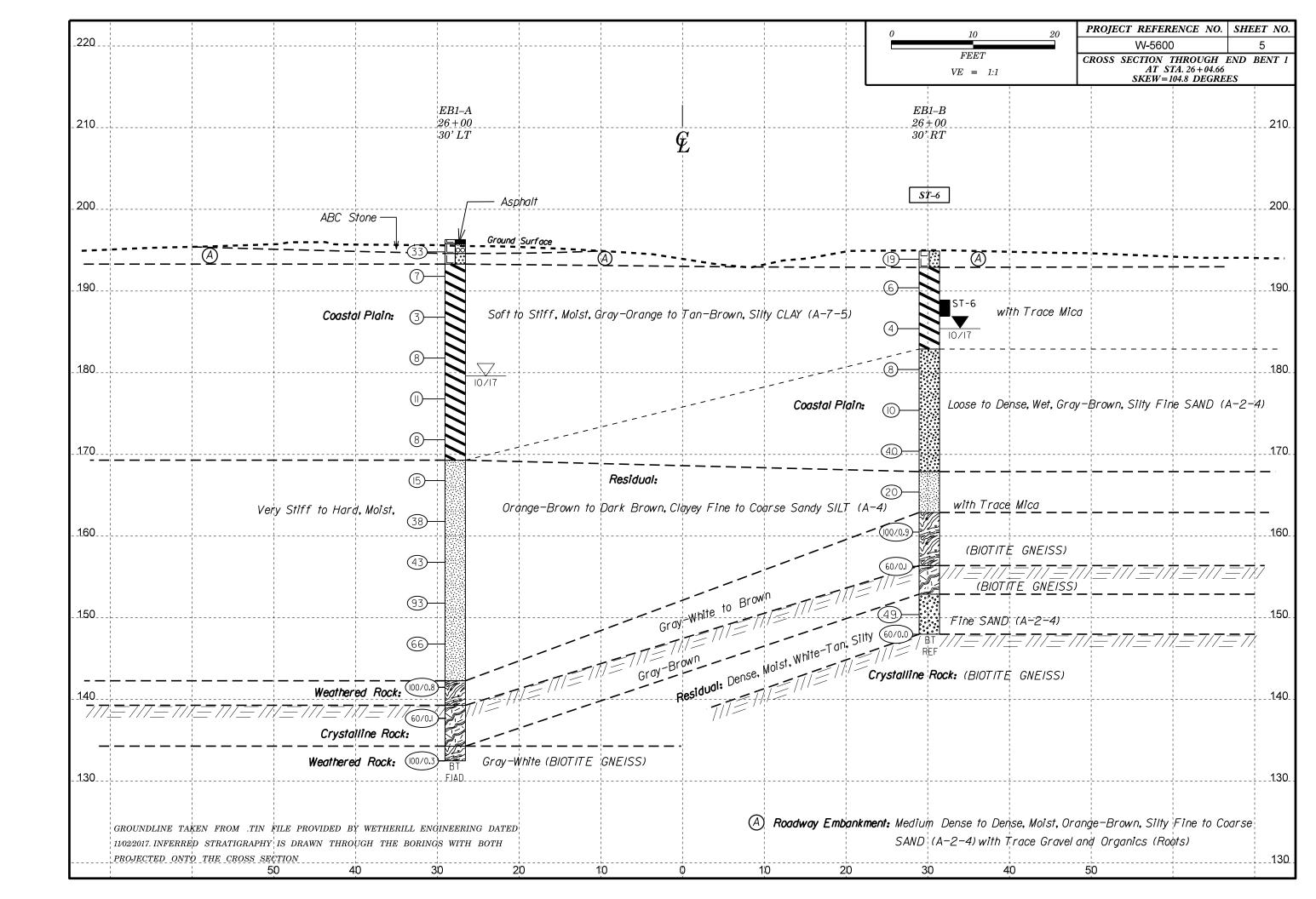
### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

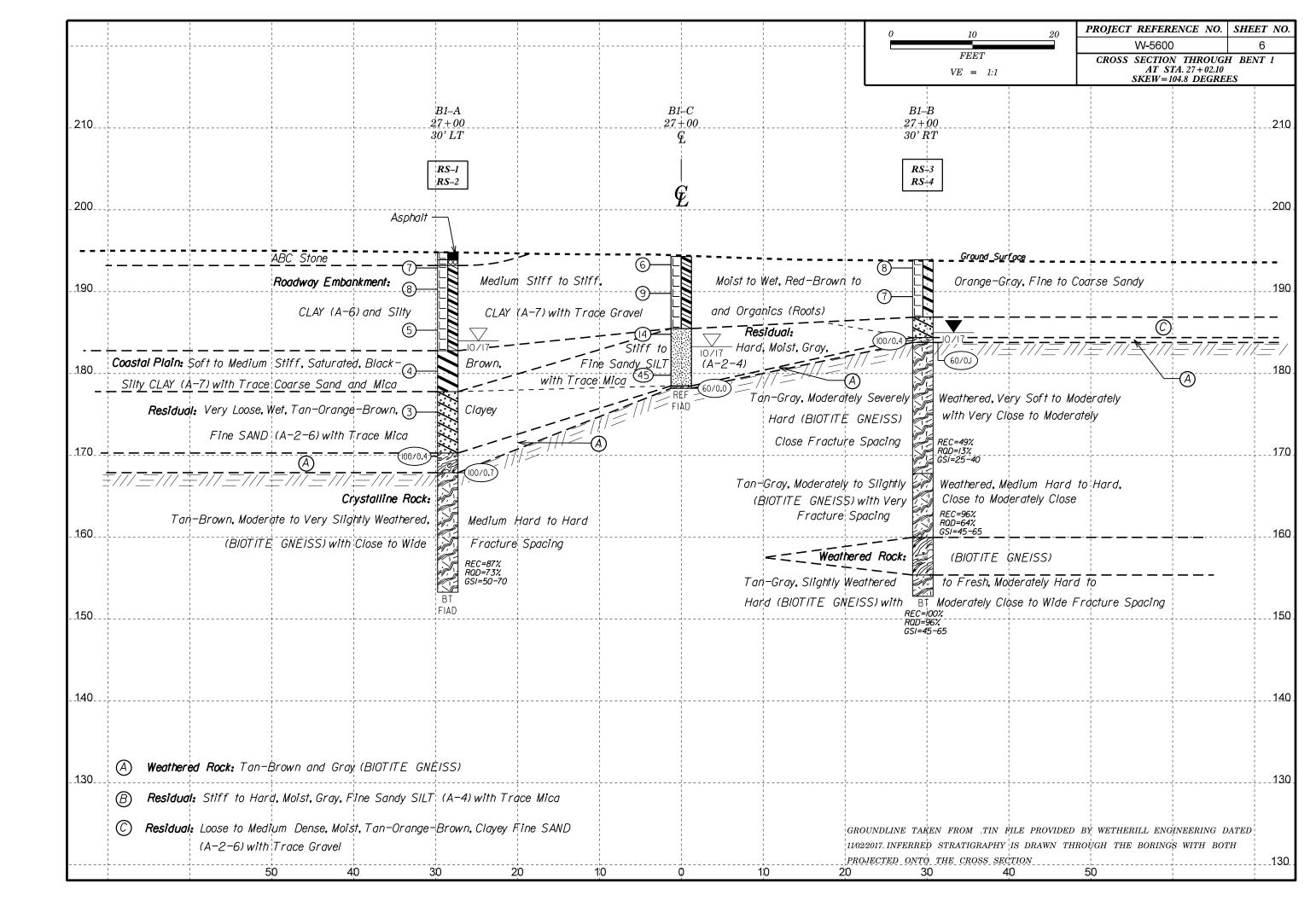
# SUBSURFACE INVESTIGATION

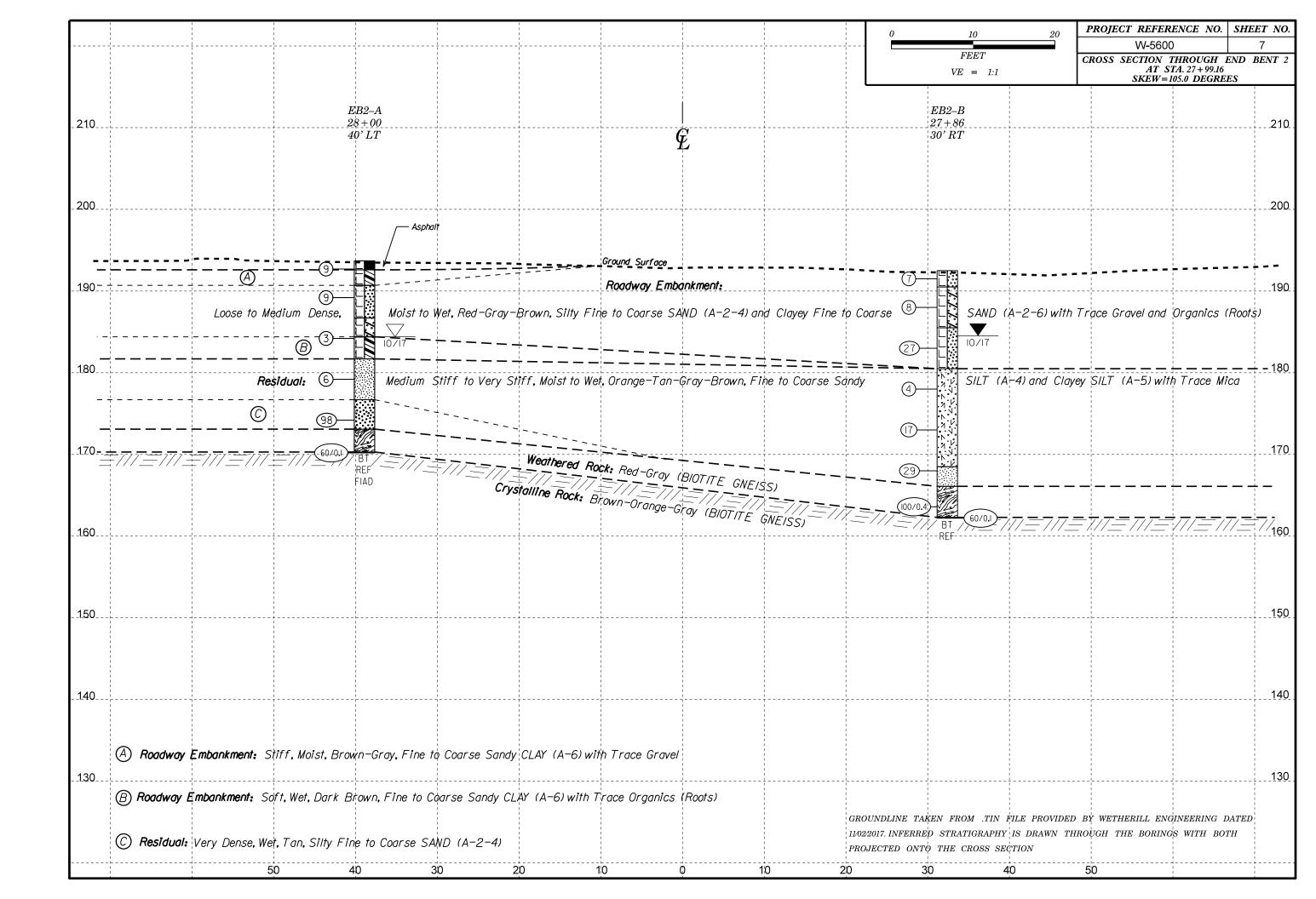
	SUPPLE	MENTAL LEGEND, ( FROM AASHTO LE	GEOLOGIC RFD BRID	SAL STRENGTH INDEX (GSI) TABLES GE DESIGN SPECIFICATIONS			
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Join	ted Rock Mass (Marinos and Ho	pek, 2000)		AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically De	formed Heterogeneous Ro	ck Masses (Marinos and Hoek, 20	(0002
GEOLOGICAL STRENGTH INDEX (GSI)FOR JOINTED ROCKS (Hoek and Marinos, 2000)	0 0 0	ν Φ Ο	s e o	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos.P and Hoek E., 2000)			
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS  VERY GOOD  Very rough, fresh unweathered surface  GOOD  Rough, slightly weathered, iron stained	FAIR Smooth altere	VERY POOR Slickensided, with soft cla	From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.	ss s w fl	- Smooth, moderateluered and altered surfaces with most occase and surfaces with ents	VERY POOR - Very smooth, slicken- sided or highly weathered surfaces
STRUCTURE	DECREASING	S SURFACE QUALITY =	<b>⇒</b>	COMPOSITION AND STRUCTURE			
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities  BLOCKY - well interlocked un-	PIECES 80	N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass, in shallow tunnels or slopes these bedding planes may cause structurally controlled instability.	70 A		
disturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets	Y 700 60			B. Sand- stone with stone and thin inter- layers of  C. Sand- stone and stone and siltstone with sand- siltstone with sand- stone layers shale with	50 B	C P E	
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets	LOCKING	50		siltstone amounts state lagers		0 / / /	
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	ASING INTERL	30		C. D. E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.		30 F 20	
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	DECK.	20		G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers  H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed		¢	10
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	V N/A N/A		10				DATE: 8-1

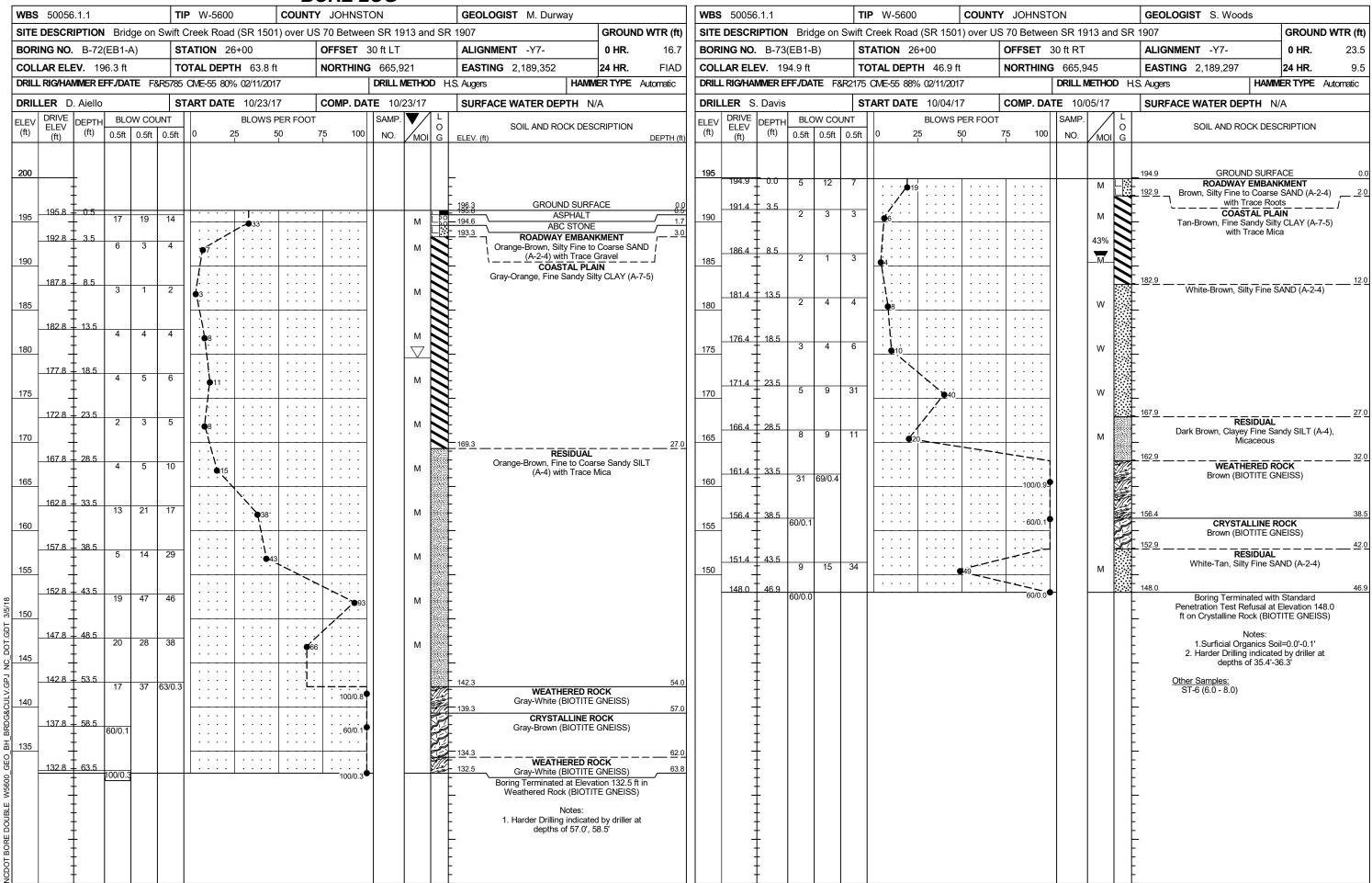












#### COUNTY JOHNSTON **WBS** 50056.1.1 **TIP** W-5600 **GEOLOGIST** M. Arnold SITE DESCRIPTION Bridge on Swift Creek Road (SR 1501) over US 70 Between SR 1913 and SR 1907 **GROUND WTR (ft)** OFFSET 30 ft LT STATION 27+00 ALIGNMENT -Y7-10.8 **BORING NO.** B-74(B1-A) 0 HR. COLLAR ELEV. 194.8 ft TOTAL DEPTH 41.5 ft **NORTHING** 665,830 **EASTING** 2,189,312 24 HR. FIAD **DRILL RIG/HAMMER EFF./DATE** F&R2175 CME-55 88% 02/11/2017 **DRILL METHOD** SPT Core Boring **HAMMER TYPE** Automatic **DRILLER** S. Davis **START DATE** 10/24/17 **COMP. DATE** 10/24/17 SURFACE WATER DEPTH N/A ELEV DRIVE DEPTH BLOW COUNT SAMP. **BLOWS PER FOOT** SOIL AND ROCK DESCRIPTION (ft) 0.5ft 0.5ft 0.5ft 75 100 NO. MOI G (ft) ELEV. (ft) DEPTH (ft **GROUND SURFACE** ASPHALT 193.9 0.9 ABC STONE ROADWAY EMBANKMENT М Red-Brown, Fine to Coarse Sandy CLAY 190 (A-6) with Trace Gravel W . . . . COASTAL PLAIN Black-Brown, Silty CLAY (A-7) with Trace Coarse Sand and Mica . . . . 180 Sat RESIDUAL Tan-Orange-Brown, Clayey Fine SAND (A-2-6) with Trace Mica W 175 . . . . 100/0.4 WEATHERED ROCK 168.6 + 26.2 Tan-Brown (BIOTITE GNEISS) 27/0.2 100/0.7 CRYSTALLINE ROCK Tan-Brown, Moderate to Very Slightly Weathered, Medium Hard to Hard (BIOTITE 165 RS-1 GNEISS) with Close to Wide Fracture Spacing RS-1: 29.4'-29.7', qu=13,917 psi, GSI=50-70 RS-2: 38.6-38.9', qu 21,117 psi, GSI=50-70 160 . . . . RS-2 155 Boring Terminated at Elevation 153.3 ft in Crystalline Rock (BIOTITE GNEISS) Notes: 1. Auger refusal at 26.2' 2. Begin coring at 26.9'

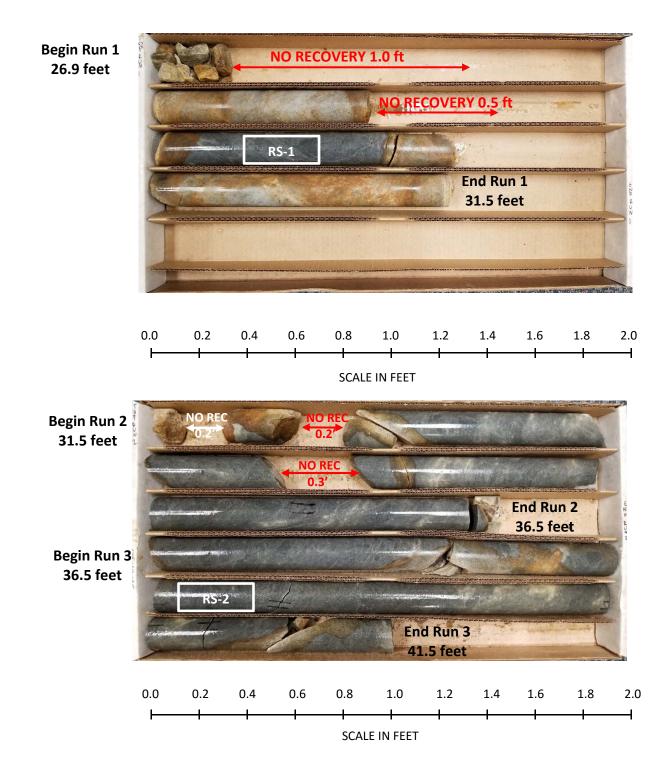
## GEOTECHNICAL BORING REPORT CORE LOG

									<u></u>	<u>Ur</u>	E LOG							
WBS	5005	3.1.1			TIP	W-56	00	С	OUNT	ΥJ	HNSTON GEO	GEOLOGIST M. Arnold						
SITE	DESCF	RIPTION	l Brid	lge on Sv	vift Cre	ek Ro	ad (SR 1	501) c	ver U	S 70	etween SR 1913 and SR 1907		GROUND	WTR (ft)				
BOR	ING NO	. B-74	(B1-A)	)	STA	TION	27+00			OF	SET 30 ft LT ALIG	NMENT -Y7-	0 HR.	10.8				
COLI	LAR EL	<b>EV.</b> 19	94.8 ft		TOT	AL DE	<b>PTH</b> 41	.5 ft		NO	THING 665,830 EAST	<b>TING</b> 2,189,312	24 HR.	FIAD				
DRILL	_ RIG/HA	MMER E	FF./DA	TE F&R2	175 CN	/IE-55 8	38% 02/11/	/2017			DRILL METHOD SPT Core B	Boring HAMMI	ER TYPE /	Automatic				
DRIL	LER S	. Davis			STAI	RT DA	<b>TE</b> 10/2	4/17		COMP. DATE 10/24/17 SURFACE WATER DEPTH N/A								
COR	E SIZE	NQ					<b>N</b> 14.6 f											
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN   SAMP.   REC.   RQD   (ft) (ft)   % %   %   %					L         O         DESCRIPTION AND REMARKS           G         ELEV. (ft)								
167.9	167.0	26.0			(2.5)	(= =)		(12 =)			Begin	n Coring @ 26.9 ft						
165 160	167.9	26.9	5.0	0:54/0.6 1:03/1.0 2:27/1.0 2:14/1.0 2:37/1.0 2:25/1.0 2:59/1.0 2:47/1.0	(3.5) 76% (4.3) 86%	(2.8) 61% (3.5) 70%	RS-1	(12.7) 87%	(10.6) 73%		Tan-Brown, Moderate to Vei (BIOTITE GNEISS) RS-1: 29.4'-29	RYSTALLINE ROCK ery Slightly Weathered, Medium with Close to Wide Fracture Sp 9.7', qu=13,917 psi, GSI=50-70 3.9', qu=21,117 psi, GSI=50-70		26.9				
100	158.3	36.5	5.0	2:56/1.0 2:36/1.0 3:03/1.0	(4.9)	(4.3)												
155	153.3	41.5		2:24/1.0 3:27/1.0 3:08/1.0 2:36/1.0	98%	86%	RS-2				153.3			41.5				
		†									Boring Terminated at Elev	vation 153.3 ft in Crystalline Roc GNEISS)	k (BIOTITE					
	-											Notes: Auger refusal at 26.2' Begin coring at 26.9'						





## CORE PHOTOGRAPHS: Bridge on Swift Creek Road (SR 1501) over US 70 between SR 1913 and SR 1907, B1-A: -Y7- Station 27+00, 30' LT





SHEET 11

									ORE L	<u>.UG</u>							
WBS	50056	5.1.1			TI	<b>P</b> W-560	0	COUNT	Y JOHNST	ON			GEOLOGI	ST M. Durv	vay		
SITE	DESCR	IPTION	<b>I</b> Brid	lge on	Swift (	Creek Roa	d (SR 150	01) over U	S 70 Betwee	en SR 19	913 an	nd SR	1907			GROUN	D WTR (ft)
BORI	NG NO.	B-75	A(B1-	C)	S	TATION 2	27+00		OFFSET	CL			ALIGNME	<b>NT</b> -Y7-		0 HR.	11.0
COLL	AR ELE	<b>EV</b> . 19	94.3 ft		TO	OTAL DEP	<b>TH</b> 16.1	ft	NORTHING	665,8	342		EASTING	2,189,285		24 HR.	FIAD
DRILL	RIG/HAI	VIMER E	FF./DA	TE F	R5785	CME-55 80	0% 02/11/20	)17		DRILL N	METHO	D H	.S. Augers		HAMIV	ER TYPE	Automatic
DRILL	<b>.ER</b> S	. Davis			Sī	TART DAT	<b>E</b> 10/27/	′17	COMP. DA	TE 10/	27/17		SURFACE	WATER DE	PTH N	/A	
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	O.5ft	0.5ft	JNT 0.5ft	0	BLOWS	PER FOO	Γ 75 100	SAMP. NO.	MOI	L O G	ELEV. (ft)	SOIL AND RO	OCK DES	CRIPTION	DEPTH (f
195	194.3 - -		2	3	3	<b>6</b> 6					М		—194.3 - - Oral	GROUI ROADWA\ nge-Brown, Fir (A-6) with Tra	e to Coar	<b>KMENT</b> se Sandy C	0. LAY
190	190.8 - - - - - 185.8 -	- 3.5 - - - - - 8.5	5	4	5	9					М		- - - - - 185.5	(740) war 116	oc Organi	03 (110013)	8.
185	180.8 -	- - -	5	6 24	8 21	14					M		_	<b>RI</b> v, Fine Sandy S	SIDUAL SILT (A-4)	with Trace	
100	178.2 - - - -	16.1 -	60/0.0		21			45	60/0.0		М	\mathred (1)	- 178.5 - 178.2		HERED RO	EISS) Standard	15
	- - - - - -	- - - - - -										-	- - - -	on Crystalline F 1. Surficial Or larder drilling in	Rock (BIOT Notes: ganics So	FITE GNES il=0.0'-0.1' y driller at 1	SS)
	- - - -	- - - -										-	- - - - -				
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		BORE L	JUG					
<b>WBS</b> 50056.1.1	<b>TIP</b> W-5600	COUNTY JOHNST	ON	GEOLOGIST M. Arnold				
SITE DESCRIPTION Brid	on Swift Creek Road (SR 150	01) over US 70 Betwee	n SR 1913 and SR	R 1907 GROUND WTR (				
<b>BORING NO.</b> B-75(B1-B)	<b>STATION</b> 27+00	OFFSET 3	30 ft RT	ALIGNMENT -Y7-	0 HR. NM			
COLLAR ELEV. 193.9 ft	TOTAL DEPTH 41.1	ft <b>NORTHING</b>	665,854	<b>EASTING</b> 2,189,257	<b>24 HR.</b> 8.9			
ORILL RIG/HAMMER EFF./DA	F&R2175 CME-55 88% 02/11/20	017	DRILL METHOD H.S	S. Augers <b>HAMM</b>	ER TYPE Automatic			
DRILLER S. Davis	START DATE 10/25/	/17 <b>COMP. DA</b>	<b>TE</b> 10/25/17	SURFACE WATER DEPTH N/	'A			
CLEV DRIVE ELEV (ft) DEPTH (ft) 0.5ft	COUNT BLOWS off 0.5ft 0 25	S PER FOOT 50 75 100	SAMP. L O NO. MOI G	SOIL AND ROCK DESC	CRIPTION DEPTH (1			
195   193.9   0.0   3   190.4   3.5   3   185.4   8.5   9   183.9   10.0   9   60/0.1   180   175   165   160   155   160   160   155   160   16	7 100/0.4	100/0.4 - 60/0.1	RS-4	ROADWAY EMBANI Orange-Brown-Gray, Silty  RESIDUAL Tan-Orange-Brown, Clayer (A-2-6) with Trace C WEATHERED RC Tan-Brown (BIOTITE C Tan-Gray, Moderately Sever Very Soft to Moderately Ha GNEISS) with Very Close t Close Fracture Spa RS-3: 20.3'-20.6', qu=26,764  Tan-Gray, Moderately to V Weathered, Medium Hard to GNEISS) with Very Close t Close Fracture Spa RS-4: 30.1'-30.4', qu=20,382  Tan-Gray, Slightly Weathe Moderately Hard to Hard (BIC with Moderately Close to W Spacing GSI=45-65 Boring Terminated at Elevat Crystalline Rock (BIOTIT  Notes: 1. Surficial Organics Soi 2. Auger refusal at 3. Begin coring at	WMENT CLAY (A-7)  y Fine SAND Gravel DCK GNEISS) OCK GNEISS) OCK SNEISS) OCK GNEISS) OCK SNEISS) OCK SNEISS OCK SNIISS OCK SN			

# GEOTECHNICAL BORING REPORT CORE LOG

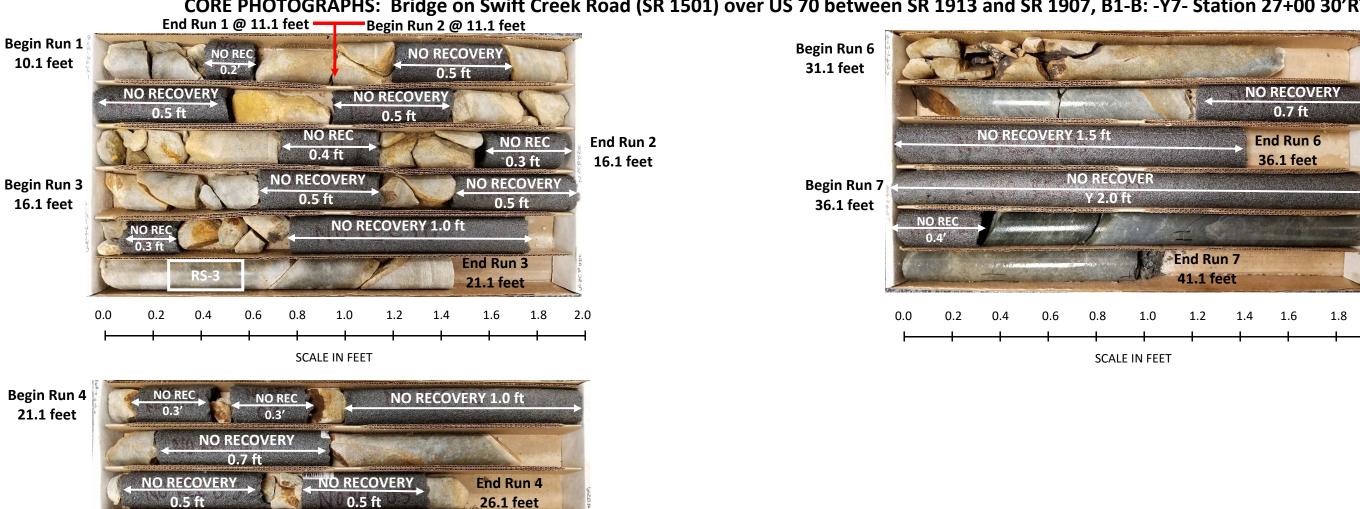
WBS         50056.1.1         TIP         W-5600         COUNTY         JOHNSTON         GEOLOGIST         M. Arr           SITE DESCRIPTION         Bridge on Swift Creek Road (SR 1501) over US 70 Between SR 1913 and SR 1907           BORING NO.         B-75(B1-B)         STATION 27+00         OFFSET 30 ft RT         ALIGNMENT -Y7-           COLLAR ELEV.         193.9 ft         TOTAL DEPTH 41.1 ft         NORTHING 665,854         EASTING 2,189,25           DRILL RIG/HAMMER EFF/DATE         F8R2175 CME-55 88% 02/11/2017         DRILL METHOD         H.S. Augers           DRILLER S. Davis         START DATE 10/25/17         COMP. DATE 10/25/17         SURFACE WATER D	GROUND WTR (fi
BORING NO.         B-75(B1-B)         STATION 27+00         OFFSET 30 ft RT         ALIGNMENT -Y7-           COLLAR ELEV.         193.9 ft         TOTAL DEPTH 41.1 ft         NORTHING 665,854         EASTING 2,189,25           DRILL RIG/HAMMER EFF,/DATE         F8R2175 CME-55 88% 02/11/2017         DRILL METHOD H.S. Augers	0 HR. NN
COLLAR ELEV.         193.9 ft         TOTAL DEPTH         41.1 ft         NORTHING         665,854         EASTING         2,189,25           DRILL RIG/HAMMER EFF./DATE         F&R2175         CME-55         88%         02/11/2017         DRILL METHOD         H.S. Augers	
DRILL RIG/HAMMER EFF./DATE F&R2175 CME-55 88% 02/11/2017 DRILL METHOD H.S. Augers	, <b> 44 fir.</b> 0.3
	HAMMER TYPE Automatic
CORE SIZE NQ TOTAL RUN 31.0 ft	EFIN N/A
RUN DRIII RUN   STRATA	
ELEV (ft) DEPTH RUN RATE REC. RQD SAMP. REC. RQD O DESCRIPTION AND REMA	RKS DEPTH (
183.8 Begin Coring @ 10.1	
183.8 + 10.1	
180 1:43/1.0 (2.8) (0.3) RS-3: 20.3'-20.6', qu=26,764 psi	
177.8 16.1 2:50/1.0 50 70 177.8 16.1 1:00/1.0	
5.0   1:40/1.0   (2.7)   (1.0)	
175 0:35/1.0 1:49/1.0	
172.8 <u>21.1</u> 4:34/1.0 RS-3	
170   0:30/1.0   32%   14%   0:58/1.0   0:58/1.0   14%   0:0000000000000000000000000000000000	
167.8 <u>26.1</u> 2:01/1.0 1:12/1.0 167.8	26
5.0 1:35/1.0 (4.7) (4.0) (7.5) (5.0) Tan-Gray, Moderately to Very Slightly Weather (BIOTITE GNEISS) with Very Close to Moderate	ely Close Fracture Spacing
165 2:28/1.0 3:00/1.0 RS-4: 30.1'-30.4', qu=20,382 psi	, GSI=45-65
162.8 <u>  31.1   2:42/1.0   RS-4  </u> - 5.0   1:30/1.0   (2.8)   (1.0)	
160 2:29/1.0 56% 20% 1:46/1.0 1:34/1.0 (0.0) (0.0) (0.0) WEATHERED ROCK	33
157.8 36.1 1:33/1.0 0% 0% (BIOTITE GNEISS)	•
5.0 1:04/1.0 (2.6) (2.5) 0:58/1.0 52% 50% 155.4	38
(2.5) (2.5) (2.5) CRYSTALINE ROUT	toly Hard to Hard (BIOTITE
152.8 41.1 3:48/1.0 100 100 100 100 100 100 100 100 100	
Boring Terminated at Elevation 152.8 ft in Cr	ystalline Rock (BIOTITE
Notes:	
1. Surficial Organics Soil=0 2. Auger refusal at 10.	
3. Begin coring at 10.	



2.0



### CORE PHOTOGRAPHS: Bridge on Swift Creek Road (SR 1501) over US 70 between SR 1913 and SR 1907, B1-B: -Y7- Station 27+00 30'RT



NO REC

0.2'

**End Run 5** 31.1 feet

1.4

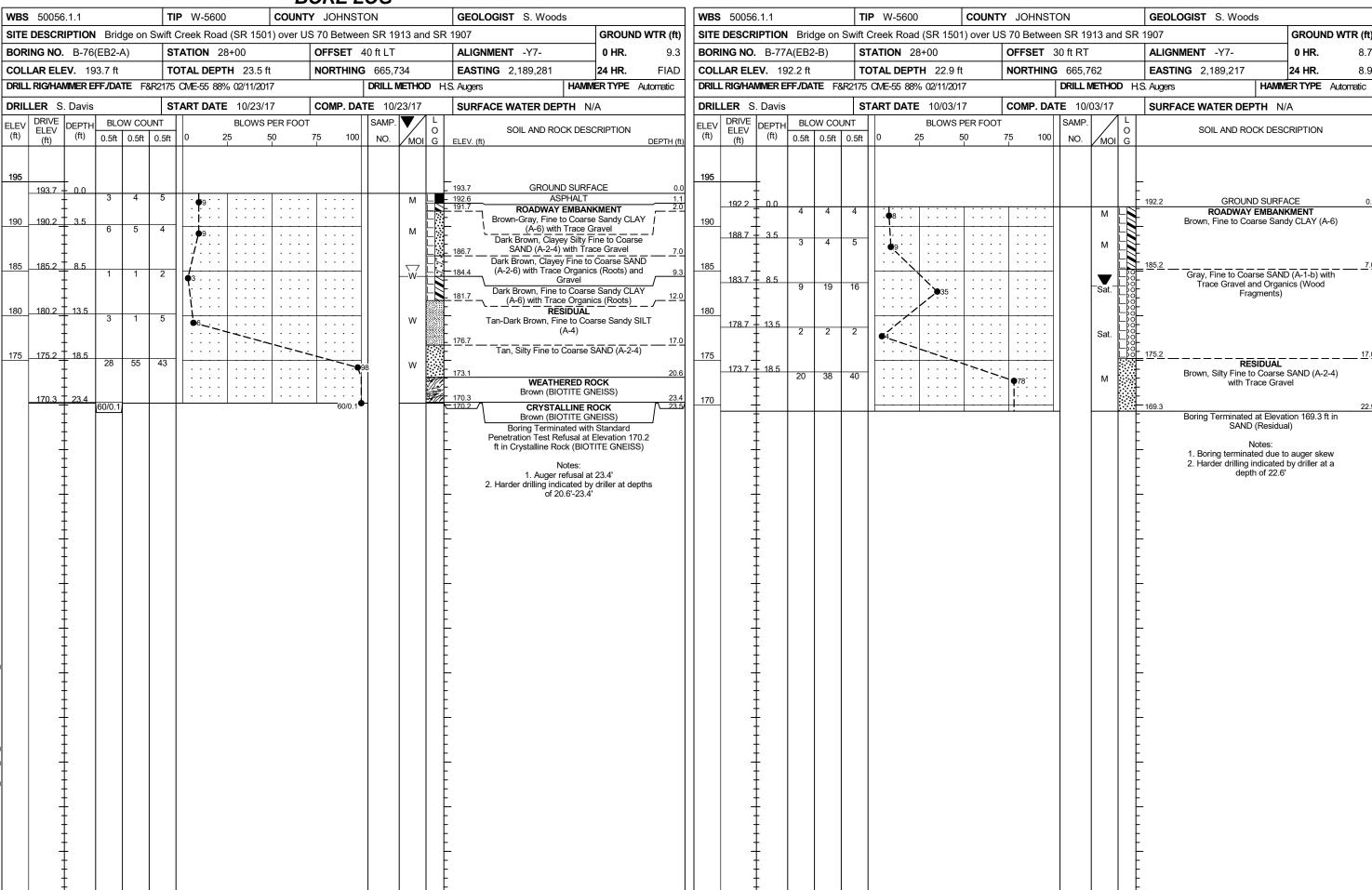
1.6

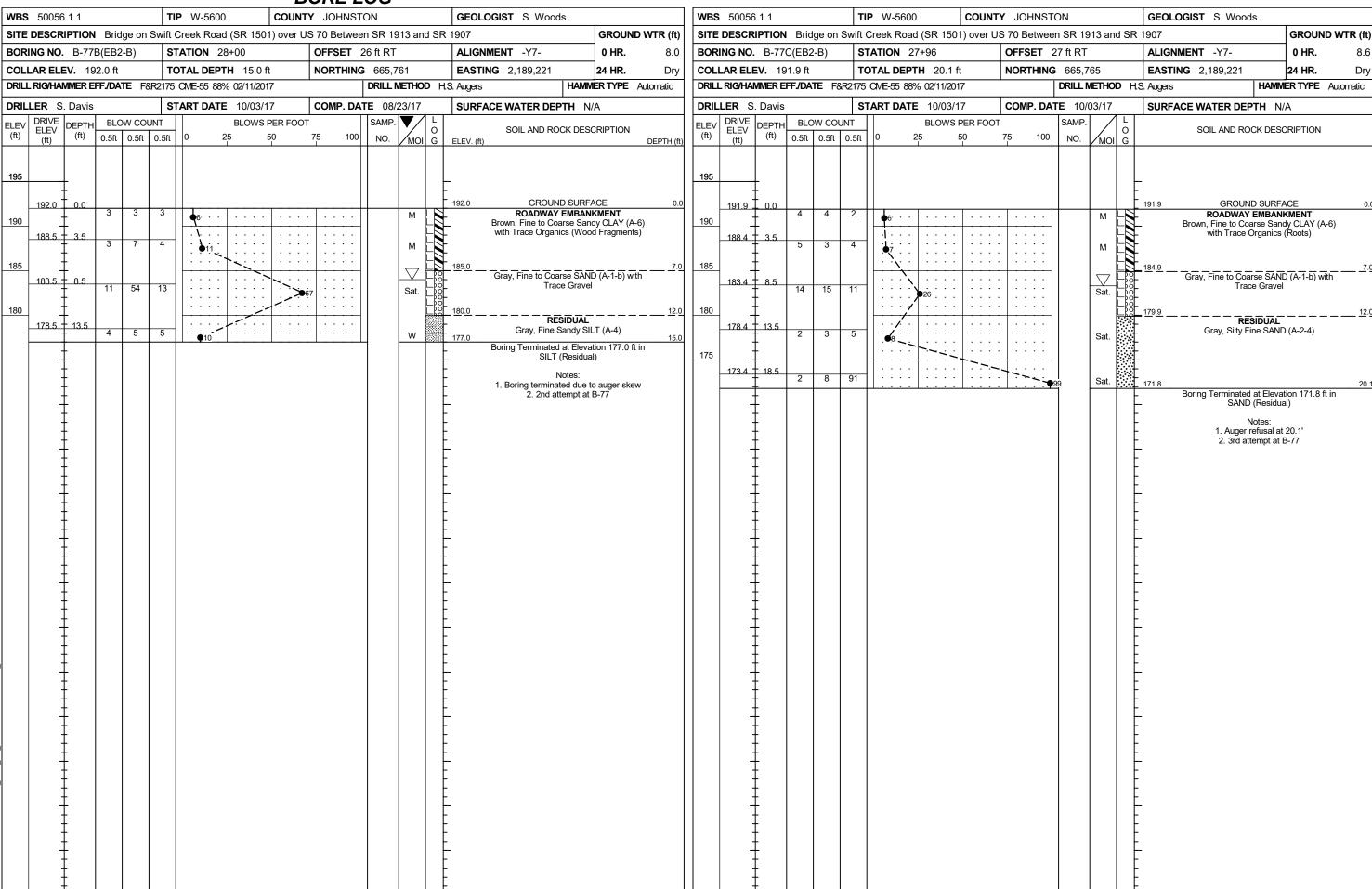
1.2

**SCALE IN FEET** 

Begin Run 5

26.1







SHEET 16

									В	ORE I	LOG	•						
WBS	50056	.1.1			TI	<b>P</b> W-56	00		COUNT	Y JOHNS	TON			GEOLOGIST M. Durway	у			
SITE	DESCR	IPTION	<b>I</b> Brid	lge on	Swift	Creek Ro	ad (SR	1501	) over U	S 70 Betwe	en SR 1	1913 ar	nd SF	R 1907	GROUN	D WTR (ft)		
BORI	ING NO.	B-77	D(EB2	2-B)	S <sup>-</sup>	TATION	27+86			OFFSET	30 ft R	Т		ALIGNMENT -Y7-	0 HR.	11.3		
COLL	LAR ELE	<b>EV</b> . 19	92.5 ft		TO	OTAL DE	PTH 3	30.3 ft		NORTHIN				<b>EASTING</b> 2,189,223	24 HR.	8.0		
DRILL	_RIG/HAI	MMER E	FF./DA	TE F	&R2175	CME-55 8	38% O2/°	11/2017	7		DRILL	METHO	DD H	I.S. Augers	HAMMER TYPE Automation			
DRIL	<b>LER</b> D	. Aiello			S <sup>-</sup>	TART DA	<b>TE</b> 10	)/27/1	7	COMP. D	ATE 10	)/27/17		SURFACE WATER DEPT	TH N/A			
ELEV	DRIVE ELEV	DEPTH	<b>'</b>	W CO					ER FOOT		SAMF	1.7		SOIL AND ROC	K DESCRIPTION			
(ft)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0	25		0	75 10	NO.	/MO	I G	ELEV. (ft)		DEPTH (ft)		
195	_	_												_				
	192.5	- 0.0	2	4	2						Ш		1		SURFACE	0.0		
190	_	Ī	2	4	3	7						M		<u>190.5</u> Red-Brown, Silty Fi	MBANKMENT ine to Coarse SAN	D		
	189.0	3.5	4	4	4						]	M		Red-Brown, Clavey F	Trace Gravel Fine to Coarse SA	<i>J</i> ND		
	-					:•%;:	:   : :					l IVI		(A-2-6) with	Trace Gravel			
185	184.0	8.5				· · · ``				+				- 185.5 Gray, Silty Fine to Coa	arse SAND (A-2-4)			
	-104.0	- 0.5	15	16	11		27 .					М		- Some	Gravel			
180	-	-				:::,	?] : :							- - 180.5		12.0		
100	179.0	13.5	2	2	2	. /				: : : :	11	١.,	7 7	─ <b>RESI</b> - Gray-Brown, Clayey	DUAL SILT (A-5) with Tra	ace		
	-	-	_	_	_	<b>■</b> 4 · · ·						M	7 7	- M	ica			
175	474.0	- 40 5				. / .		· · ·			41		, N	<u>-</u>				
	174.0	_ 18.5 -	2	6	11	: : 🍾	17					М	, , , , , , , , , , , , , , , , , , ,	-				
170	-	-				:::`	$i' \mid ::$						12.7	<u>-</u> -				
170	169.0	23.5		44	40		<del>`\</del>			+ : : : :			1,1	 - 168.5		24.0		
	-	_	6	11	18	:::	29					M		Orange-Tan-Brown, F	Fine Sandy SILT ( <i>i</i> ace Mica	A-4)		
165	-	_					.				<u>-</u>		TO	WEATHER	RED ROCK	26.4		
	164.0	28.5	100/0.4			: : :				. 100/0.4	.♦				OTITE GNEISS)	20.2		
	162.3	30.2	60/0.1							60/0.	•	-			LINE ROCK	30.2		
	_	_													IOTITE GNEISS) ed with Standard			
	-	_												Penetration Test Refu ft in Crystalline Rock	usal at Elevation 16 k (BIOTITE GNEIS	62.2 (S)		
	-	<u> </u>												_	tes:	-,		
	-	-												<ul> <li>1. Harder drilling indic</li> </ul>	cated by driller at 2 fusal at 30.2'	26.4'		
	-													- 2. Auger rei - 3. 4th atter	mpt at B-77			
	_	-												<del>-</del>				
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# North Carolina Department of Transportation Division of Highways Materials and Test Unit Soils Laboratory

T.I.P. ID NO.: W-5600

DESCRIPTION: Bridge on Swift Creek Road (SR1501) over Us 70 between SR 1913 and SR 1907

REPORT ON SAMPLES OF: SOIL FOR QUALITY

F&R PROJECT #: 66U-0197 COUNTY: Johnston

DATE SAMPLED: 9/17 to 10/17 RECEIVED: 10/17 to 12/17

SAMPLED FROM: Various REPORTED: 10/17 to 12/17

 SUBMITTED BY:
 Cheng Wang
 BY:
 D. Jenks

 Cert No. 101-02-0603
 Cert No. 101-02-0603

#### **TEST RESULTS**

PROJ. SAMPLE NO.	ST-6							
BORING NO.	B-73							
	EB1-B							
Retained #4 Sieve %	3.7							
Passing #10 Sieve %	3.7							
Passing #40 Sieve %	18.1							
Passing #200 Sieve %	74.4							

			T			I		Ī	
SOIL MORTAR - 100%									
Coarse Sand Ret - #60 %	7.6								
Fine Sand Ret - #270 %	18.4								
Silt 0.053 - 0.010 mm %	22.3								
Clay < 0.010 mm %	51.7								
L.L.	94								
P.L.	45								
P.I.	49								
AASHTO Classification	A-7-5(42)								
Station	26+00								
Offset	30'Rt								
Depth (ft)	6.0								
to	8.0								
Alignment	-Y7-								
Moisture Content (%)	42.9								
Organic Content (%)	NT								

NP = Not plastic

NT = Not tested

ND = Not Determined

CL = Centerline

#### LABORATORY SUMMARY SHEET FOR ROCK CORE SAMPLES

PROJECT NO.: 50056.1.1
TIP NO.: W-5600
COUNTY: Johnston

**DESCRIPTION:** Bridge on Swift Creek Road (SR 1501) over US 70 between SR 1913 and SR 1907

Sample #	Boring #	Alignment	Station	Offset	Depth (ft)	Rock Type	Geologic Map Unit	Run RQD	Length (in)	Diameter (in)	Unit Weight (pcf)	Unconfined Compressive Strength (psi)	Young's Modulus, E (ksi)	GSI
RS-1	B1-A	-Y7-	27+00	30' LT	29.4-29.7	Biotite Gneiss	CZbg	56%	4.35	1.78	164.2	13,917	1,905	50-70
RS-2	B1-A	-Y7-	27+00	30' LT	38.6-38.9	Biotite Gneiss	CZbg	86%	4.21	1.78	166.8	21,117	2,910	50-70
RS-3	B1-B	-Y7-	27+00	30' RT	20.3-20.6	Biotite Gneiss	CZbg	20%	4.36	1.78	162.5	26,764	2,726	30-50
RS-4	B1-B	-Y7-	27+00	30' RT	30.1-30.4	Biotite Gneiss	CZbg	80%	4.31	1.77	165.3	20,382	2,278	30-50