

NOTES:

DATA ON EXISTING END BENTS SHOWN BASED ON THE BEST AVAILABLE INFORMATION. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND MEASUREMENTS PRIOR TO BEGINNING CONSTRUCTION AND REPORT ALL VARIATIONS TO THE ENGINEER. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT DUE TO VARIATION BETWEEN THE PLANS AND EXISTING STRUCTURE.

ALL CUT SURFACES WITH EXPOSED REINFORCING SHALL BE GROUND SMOOTH.

APPLY A TYPE 4A EPOXY COAT TO ALL AREAS EXPOSED BY SAW CUT IN ACCORDANCE WITH SECTION 1081 OF THE STANDARD SPECIFICATIONS.

EXTENTS OF THE SAW CUTS SHALL NOT EXCEED THE STAGE IA LIMITS SHOWN.

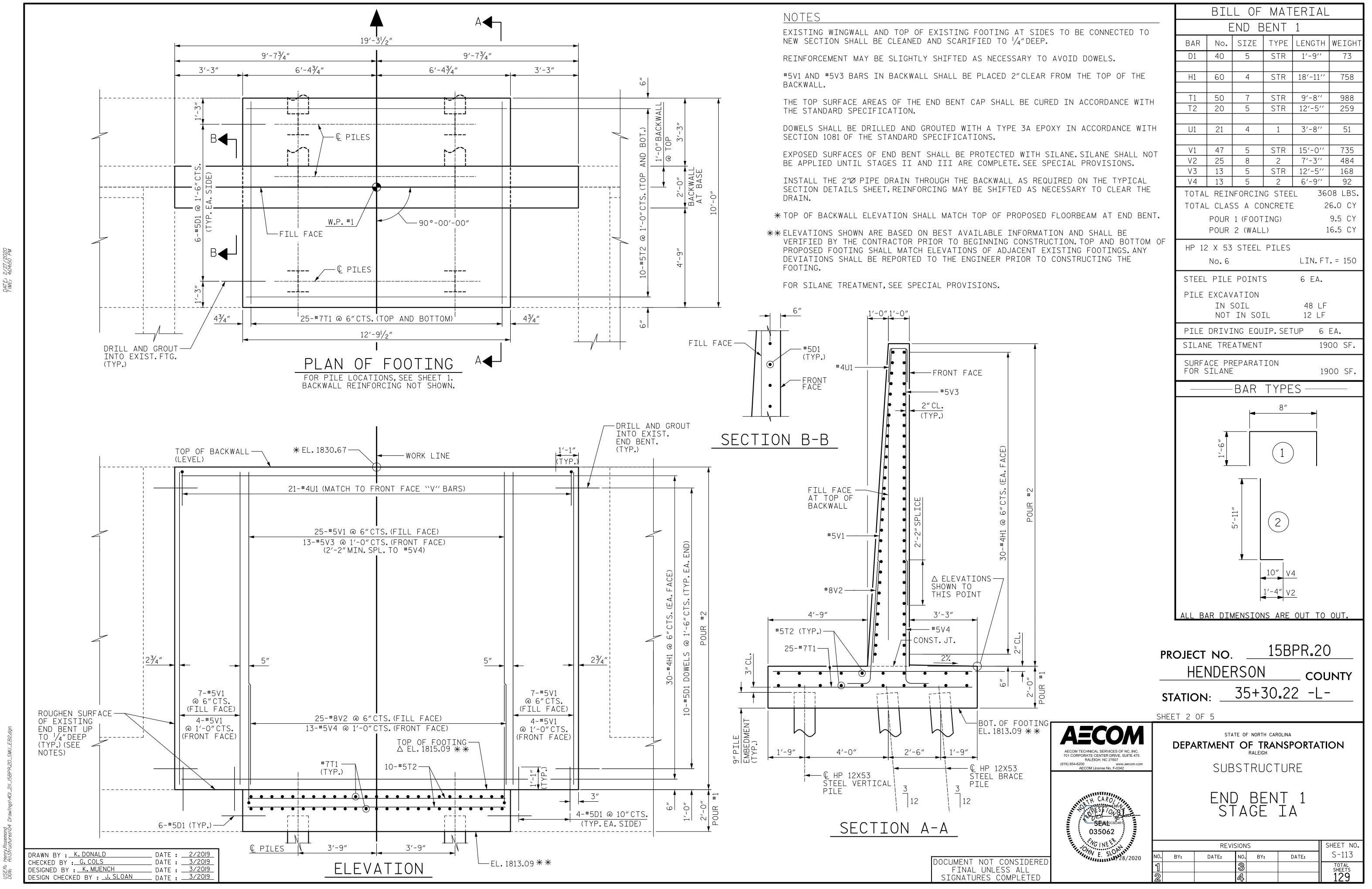
SEE ``DEMOLITION & CONSTRUCTION SEQUENCE'' PLANS FOR STAGING INFORMATION.

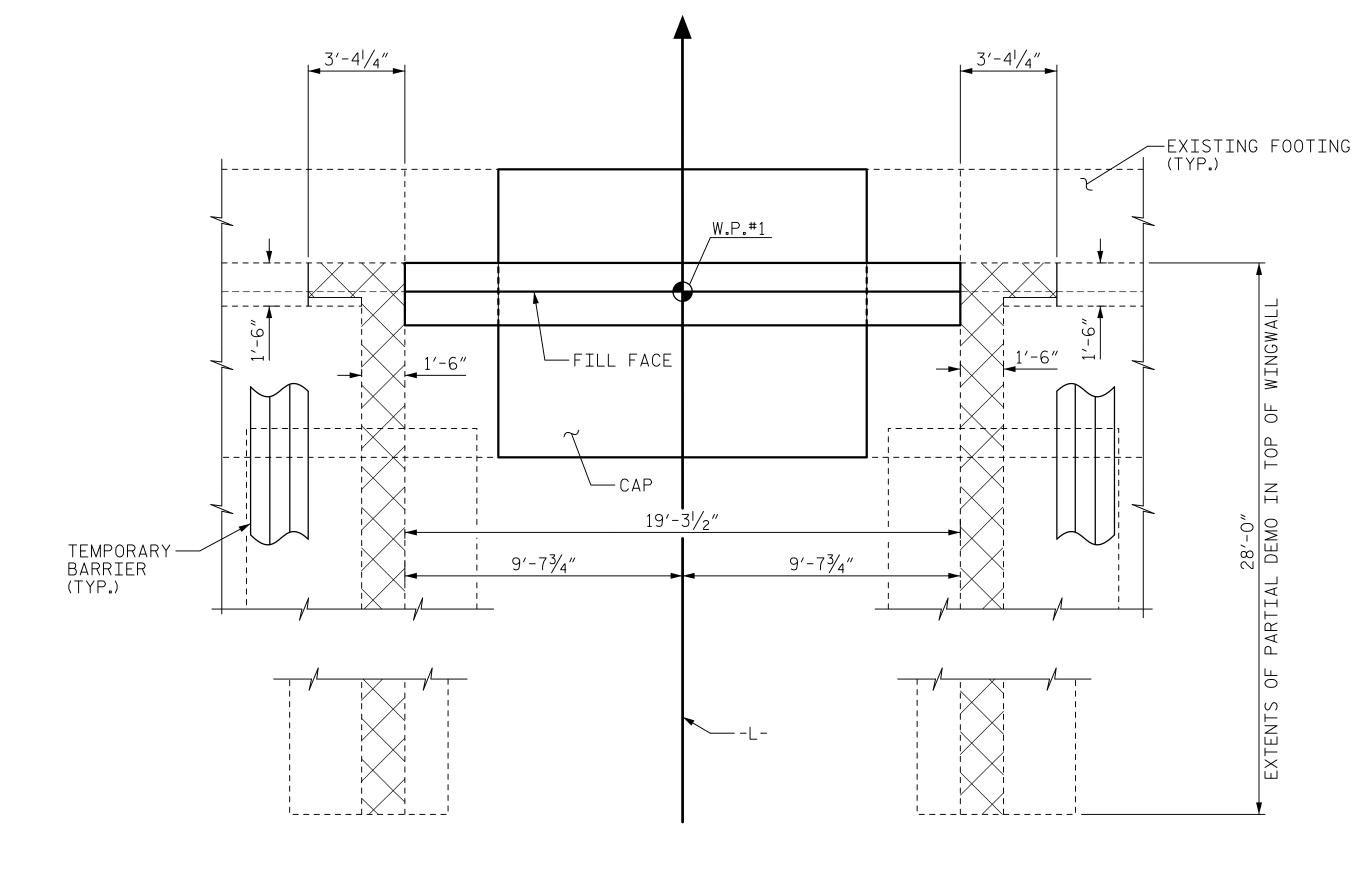
FOR REINFORCING, SEE SHEET 2.

DEMOLITION LIMITS ARE SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL SUBMIT DETAILED DEMOLITION SEQUENCES AND SHALL BE RESPONSIBLE FOR THE STABILITY OF THE PARTIALLY DEMOLISHED END BENT THROUGHOUT THE DURATION OF CONSTRUCTION.

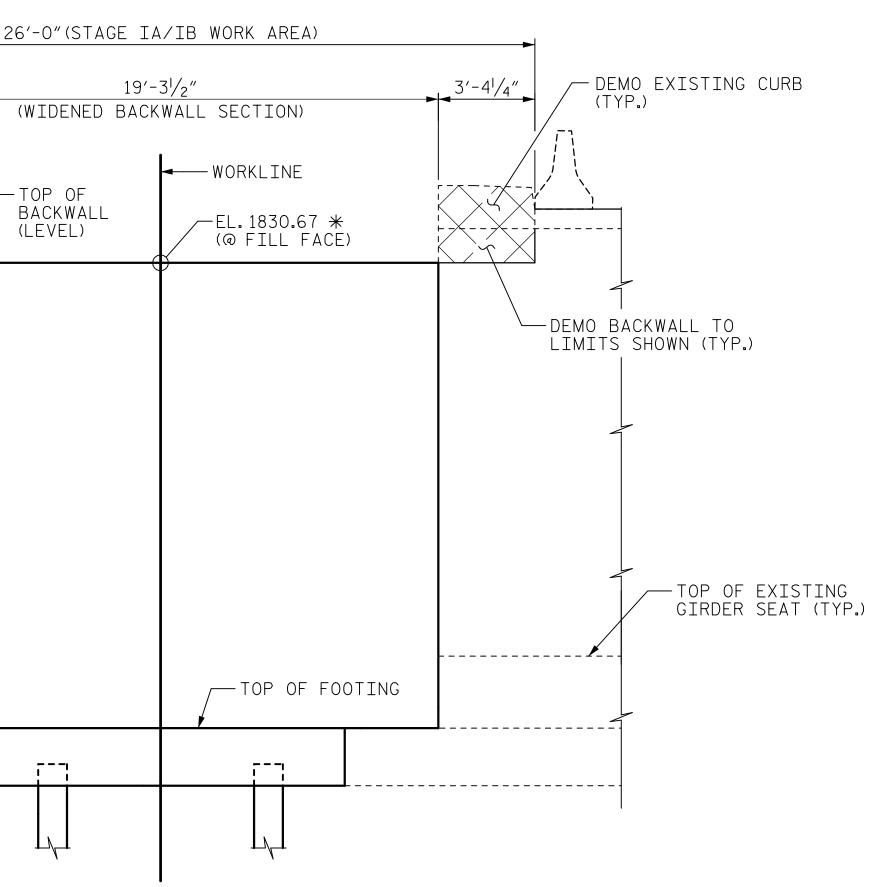
\*TOP OF BACKWALL ELEVATION SHALL MATCH TOP OF PROPOSED FLOORBEAM AT END BENT.

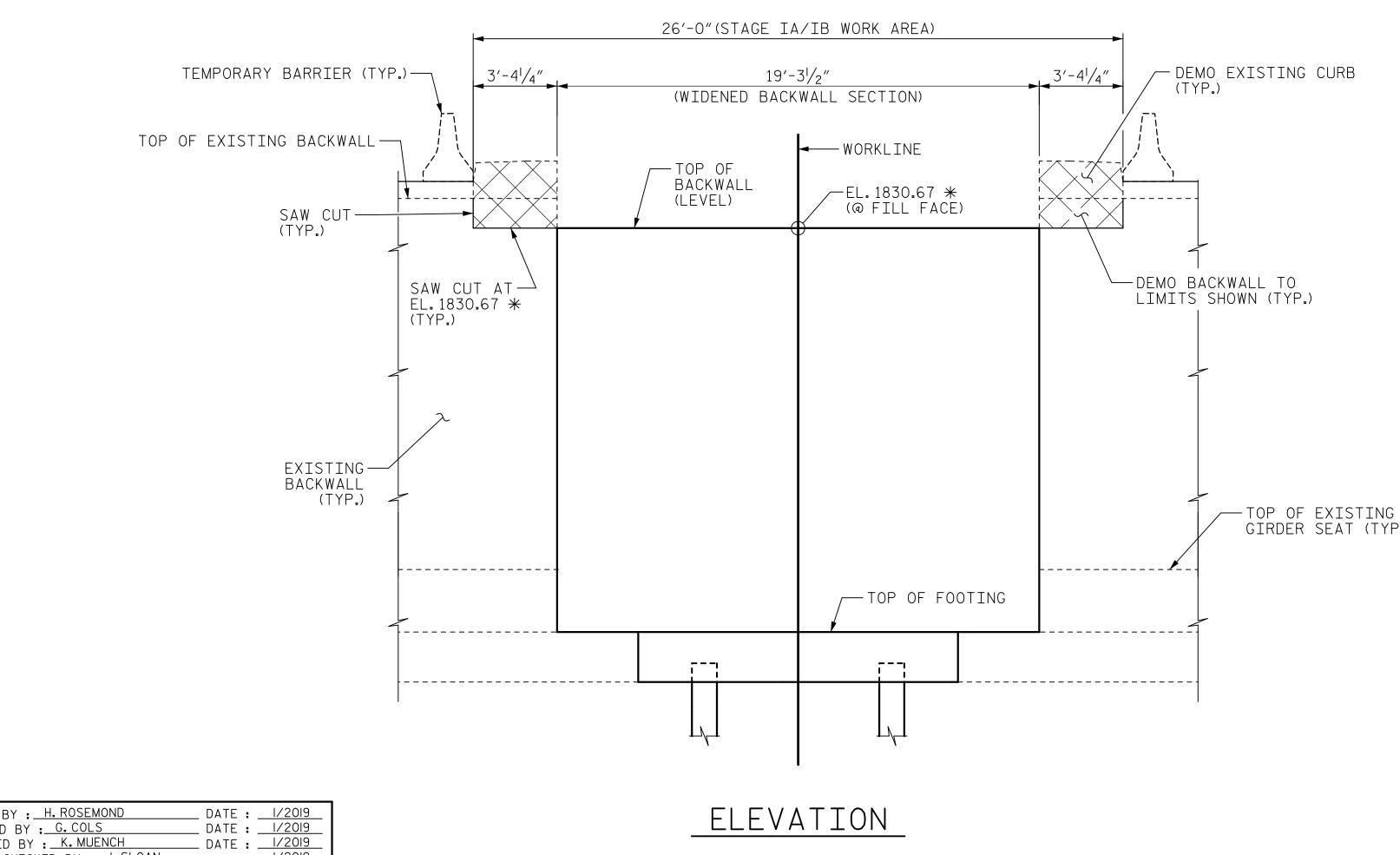
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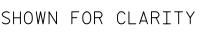


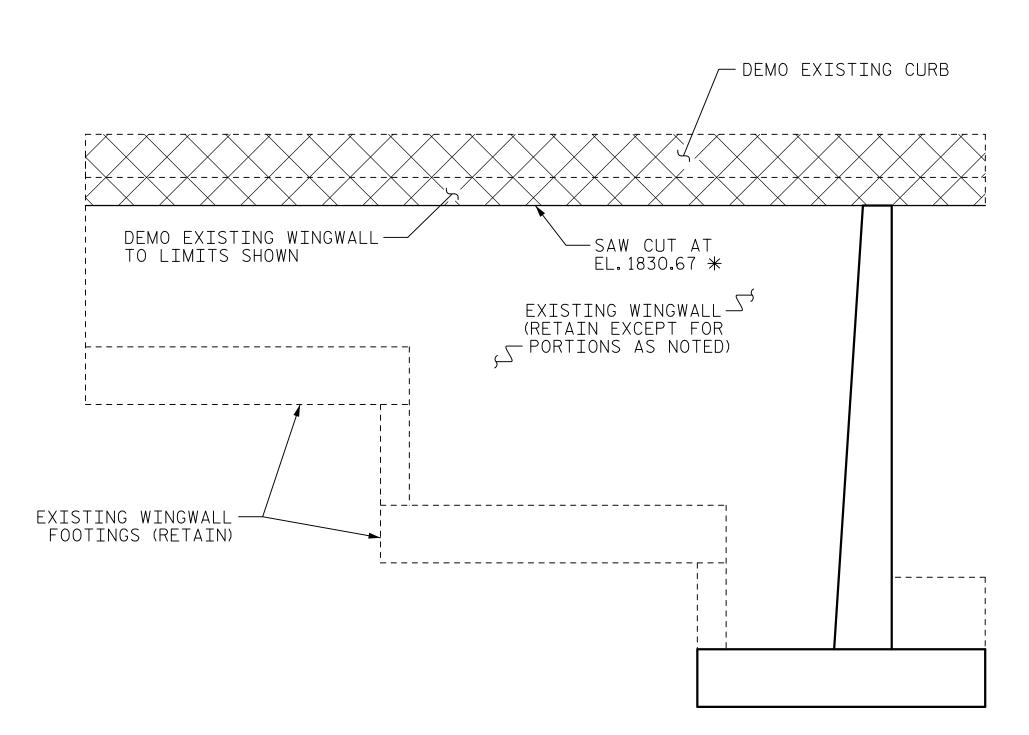


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SEE "DEMOLITION & CONSTRUCTION SEQUENCE" PL FOR STAGING INFORMATION.

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\* TOP OF BACKWALL ELEVATION SHALL MATCH TOP O PROPOSED FLOORBEAM AT END BENT.

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## DEMO SYMBOLOGY

## AREA TO BE REMOVED

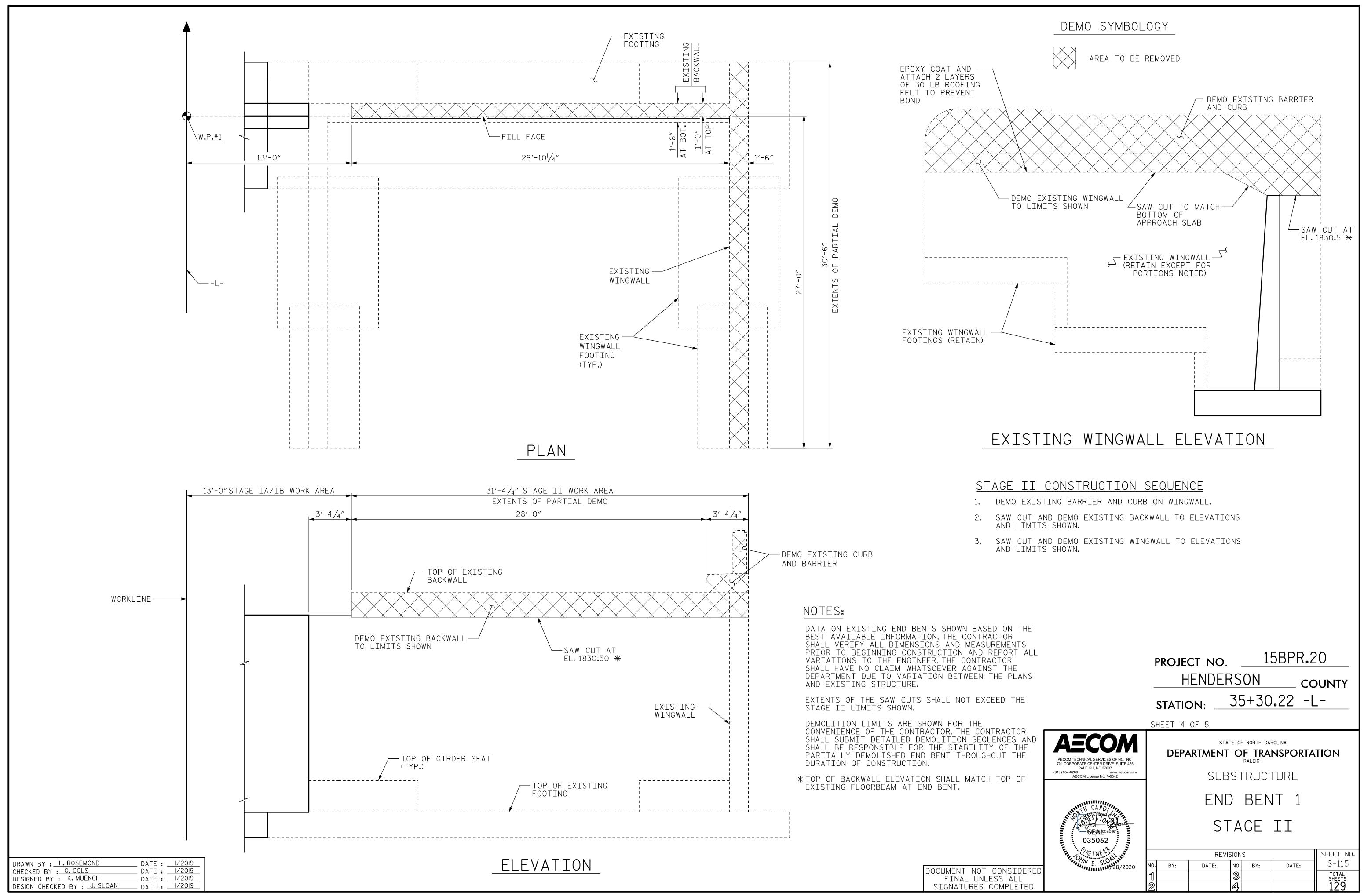
# EXISTING WINGWALL ELEVATION

SHOWN FOR LEFT-SIDE INTERIOR WINGWALL, SIMILAR FOR RIGHT-SIDE INTERIOR WINGWALL

## CONSTRUCTION SEQUENCE

TING CURB. DEMO EXISTING WINGWALL AND BACKWALL SHOWN.

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IE NS. - ANS	AECOM TECHNICAL SERVICES OF NC, INC. 701 CORPORATE CENTER DRIVE, SUITE 475 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE	
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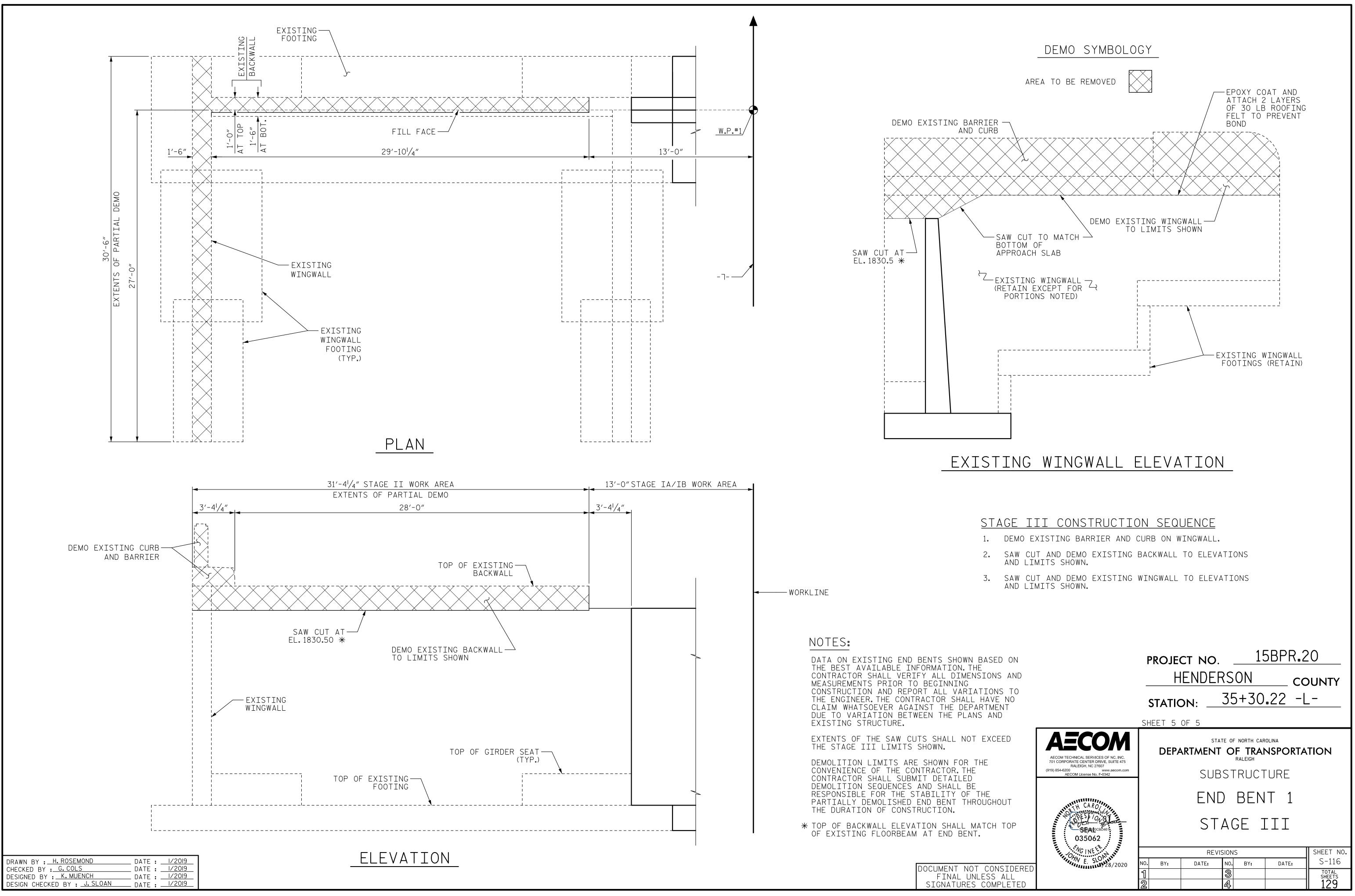


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## POST-TENSIONING SYSTEM CONSTRUCTION SEQUENCE

- 1. THIS SEQUENCE APPLIES TO BENTS 1 THRU 4.
- 2. USE A SAW TO CUT OUT THE STAY-IN-PLACE STEEL FORMWORK AT EACH END OF THE CAP AT THE LOCATION OF THE ANCHORAGE. MAINTAIN A BLADE DEPTH THAT DOES NOT CUT INTO THE EXISTING CONCRETE SUBSTRATE. PHOTOGRAPH THE CONCRETE SUBSTRATE AND DOCUMENT ITS CONDITION. SUBMIT THE PHOTOGRAPHS AND CONDITION REPORT TO THE ENGINEER. COMPLETE THE REMOVAL OF THE COLUMN STAY-IN-PLACE FORM AND EVALUATION OF THE BENT COLUMN CONCRETE IN PARALLEL WITH THIS STEP.
- 3. USING GROUND PENETRATING RADAR (GPR). DETERMINE THE LOCATION OF THE EXISTING REINFORCEMENT IN THE BENT IN THE VICINITY OF THE ANCHORAGE.
- 4. USING THE RESULTS OF THE GPR, DETERMINE THE PRECISE LOCATION OF ANCHORAGES THAT WILL BE DRILLED INTO THE EXISTING STRUCTURE. CONTRACTOR MAY PROPOSE TO ADJUST THE HORIZONTAL AND VERTICAL ANCHORAGE SPACING TO MISS EXISTING REINFORCEMENT.
- 5. SUBMIT SHOP DRAWINGS AND ERECTION METHODOLOGY FOR REVIEW BY THE ENGINEER. ALL CONTRACTOR PROPOSED CHANGES TO THE ANCHORAGE SHALL BE SUBMITTED WITH THE SHOP DRAWINGS FOR REVIEW.
- FOLLOWING REVIEW AND ACCEPTANCE OF THE SHOP DRAWINGS, DRILL HOLES INTO THE BENT USING A SEPARATE METAL TEMPLATE FOR EACH LOCATION. CONTRACTOR SHALL USE A DRILL BIT CAPABLE OF PENETRATING CONCRETE THAT IS NOT CAPABLE OF PENETRATING REINFORCING STEEL. CONTRACTOR SHALL CLEARLY MARK THE LOCATION AND ORIENTATION OF EACH TEMPLATE TO ENSURE THE HOLES WILL BE PROPERLY DRILLED FOR EACH ANCHORAGE. IF REINFORCEMENT IS ENCOUNTERED, STOP WORK AND REPORT TO THE ENGINEER.
- 7. INSTALL ALL ADHESIVE ANCHORS FOR THE ANCHORAGE.
- 8. USING A HYDRAULIC TEST JACK, PROOF LOAD EACH ADHESIVE ANCHOR TO 10 KIPS OF TENSION.
- 9. FOLLOWING THE COMPLETION OF DRILLING, INSTALLATION, AND PROOF LOADING OF ALL ADHESIVE ANCHORS, FABRICATE THE SOLE PLATES, CORBELS, JACK BEARINGS, AND POST-TENSIONING BARS. ANCHORAGE HOLES IN SOLE PLATES AND CORBELS SHALL BE SHOP DRILLED USING THE TEMPLATE THAT WAS USED TO DRILL THE HOLES IN THE BENT CAP.
- 10. INTENTIONALLY ROUGHEN THE CONCRETE SUBSTRATE OF THE BENT CAP AT THE LOCATION OF THE GROUT PAD TO AN AMPLITUDE OF 1/4".
- 11. PLACE A 1/4" THICK EXPANDED POLYSTYRENE SLEEVE AROUND THE ADHESIVE ANCHORS AT THE LOCATION OF THE GROUT PAD TO PREVENT BOND WITH GROUT DURING INSTALLATION.
- 12. PLACE THE ANCHORAGE AGAINST THE BENT CAP IN A LEVEL POSITION, SQUARE TO THE CAP. SET THE ANCHORAGE AT THE SPECIFIED DISTANCE AWAY FROM THE BENT TO ENSURE THE CORRECT GROUT PAD THICKNESS USING ADJUSTABLE SHIMS OR LUGS ON THE FACE OF THE ANCHORAGE. PLACEMENT AND SPACING METHODOLOGY SHALL BE SHOWN IN SHOP AND ERECTION DRAWINGS. LOCATE LUGS OR SHIMS TO PREVENT OVERSTRESS IN THE ANCHORAGE AND BENT CAP CONCRETE DURING THE STRESSING OF ADHESIVE ANCHORS PRIOR TO THE PLACEMENT OF THE GROUT PAD.
- 13. USING A HYDRAULIC JACK, PRETENSION AND LOCK OFF EACH ADHESIVE ANCHOR AT A TENSION OF 1 KIP. CONTRACTOR MAY PROPOSE STRESSING A SPECIFIC NUMBER OF ADHESIVE ANCHORS RATHER THAN ALL OF THE ANCHORS. STRESSING OF ADHESIVE ANCHORS SHALL BE SUFFICIENT TO FIX THE ANCHORAGE IN PLACE DURING INSTALLATION OF THE GROUT PAD BETWEEN THE ANCHORAGE AND THE BENT CAP, WITHOUT THE AID OF ANY EXTERNAL SUPPORTS.
- 14. INSTALL THE GROUT PAD BETWEEN THE ANCHORAGE AND THE BENT. PUMP THE GROUT FROM THE BOTTOM OF THE ANCHORAGE, AND ENSURE ADEQUATE PLACEMENT AND CONSOLIDATION OF THE GROUT USING WEEP HOLES AT THE TOP OF THE ANCHORAGE. CLOSE THE WEEP HOLES AFTER THE GROUT IS ALLOWED TO FLOW THROUGH THE HOLE.
- 15. ENSURE ADEQUATE CURE TIME FOR THE GROUT PAD TO REACH SPECIFIED STRENGTH BASED ON THE GROUT MANUFACTURER'S PUBLISHED CURE TIMES.
- 16. INSTALL THE 4-1 $\frac{3}{4}$ " Ø POST-TENSIONING BARS. INCLUDING NUTS AND ANCHOR PLATES. TIGHTEN ALL NUTS TO HAND TIGHT CONDITION.
- 17. STRESS THE POST-TENSIONING BARS IN THE ORDER SHOWN IN THE ``POST-TENSIONING STRESSING ORDER' DETAIL.
  - a. INITIALLY STRESS ALL BARS TO A TENSION OF 5 KIPS. STRESSING THEN SHALL PROCEED IN INCREMENTS OF 25 KIPS UP TO THE FINAL TENSION IN THE BARS, 235 KIPS PER b. BAR AFTER LOCK OFF.
  - NO BAR SHALL HAVE A LOAD THAT IS 25 KIPS GREATER OR LESS THAN THE LOADING IN ANY OTHER POST-TENSIONING BARS.
  - CONTRACTOR SHALL OBSERVE THE BENTS AND THE ANCHORAGES CONTINUOUSLY DURING STRESSING OPERATIONS. IF ANY DISTRESS IN THE BENT OR ANCHORAGE IS OBSERVED DURING STRESSING OPERATIONS. CONTRACTOR SHALL CEASE STRESSING OPERATIONS AND REPORT TO THE ENGINEER.
- 18. USING A HYDRAULIC JACK, PRETENSION AND LOCK OFF EACH ADHESIVE ANCHOR AT A SERVICE TENSION OF 1 KIP.
- 19. INSTALL SILICONE SEALANT AROUND THE EDGES OF THE ANCHORAGE THAT FACE THE BENT CAP.
- 20. INSTALL COVER ON TOP OF THE ANCHORAGE TO PREVENT WATER INTRUSION.

DRAWN BY :M.K. TOM CHECKED BY :J.E. SLOAN DESIGNED BY :J.E. SLOAN DESIGN CHECKED BY :D. TUTTLE	DATE :	2/201 2/201

## BENT COLUMN CONCRETE EVALUATION SEQUENCE

IN PARALLEL WITH THE INSTALLATION OF THE POST-TENSIONING SYSTEM AT THE BENT CAPS, THE CONTRACTOR AND THE ENGINEER SHALL EVALUATE THE CONCRETE IN THE COLUMNS. THE CONTRACTOR SHALL DRILL TWO 2"DIAMETER HOLES THRU THE STAY-IN-PLACE FORMWORK IN EACH COLUMN. ONE HOLE SHALL BE LOCATED AT A DISTANCE OF APPROXIMATELY 5'-O"BELOW THE SOFFIT OF THE CAP AND THE SECOND HOLE SHALL BE LOCATED APPROXIMATELY 5'-O"ABOVE THE GROUND SURFACE. USE THE FOLLOWING SEQUENCE TO DRILL THE HOLES AND PERFORM THE INSPECTION: THE ENGINEER SHALL SOUND THE FORMS AND CHOOSE THE LOCATION OF THE HOLE. THE HOLE LOCATIONS SHALL BE CHOSEN AT A POINT WHERE THE FORMWORK SOUNDS TO BE DELAMINATED FROM THE CONCRETE SUBSTRATE. THE CONTRACTOR SHALL DRILL THE HOLE THRU THE STAY-IN-PLACE FORM WITHOUT DAMAGING THE CONCRETE SUBSTRATE. THE ENGINEER SHALL INSPECT THE CONCRETE SUBSTRATE ONCE THE HOLE HAS BEEN DRILLED. THE CONTRACTOR SHALL PHOTOGRAPH THE HOLE AND CONCRETE SUBSTRATE AND DOCUMENT THE CONDITION OF THE CONCRETE. 4. THE CONTRACTOR SHALL SUBMIT THE PHOTOGRAPHS AND CONDITION REPORT TO THE ENGINEER FOLLOWING THE EVALUATION. 5. UPON ACCEPTANCE OF THE SUBSTRATE BY THE ENGINEER. THE CONTRACTOR SHALL SEAL WELD A 3"DIAMETER X  $\frac{1}{2}$ " THICK STEEL PLATE AROUND THE 2"DIAMETER HOLE.

- SHALL BE MADE FOR THIS SEQUENCE, BUT PAYMENT SHALL BE INCLUDED IN THE POST-TENSIONING ANCHORAGE PAY ITEM.

### POST-TENSIONING GENERAL NOTES

FOR FURTHER DETAILS, SEE POST-TENSIONING SYSTEM SPECIAL PROVISIONS. PT = POST-TENSIONING

EDGES OF STEEL PLATES IN THE PT ANCHORAGE THAT BEAR AGAINST THE SURFACES OF OTHER PLATES SHALL BE TIGHT FIT WITH AN ANSI ROUGHNESS HEIGHT VALUE NOT GREATER THAN 500.

USE ASTM A193 316 B8M CLASS II ADHESIVE ANCHORS.

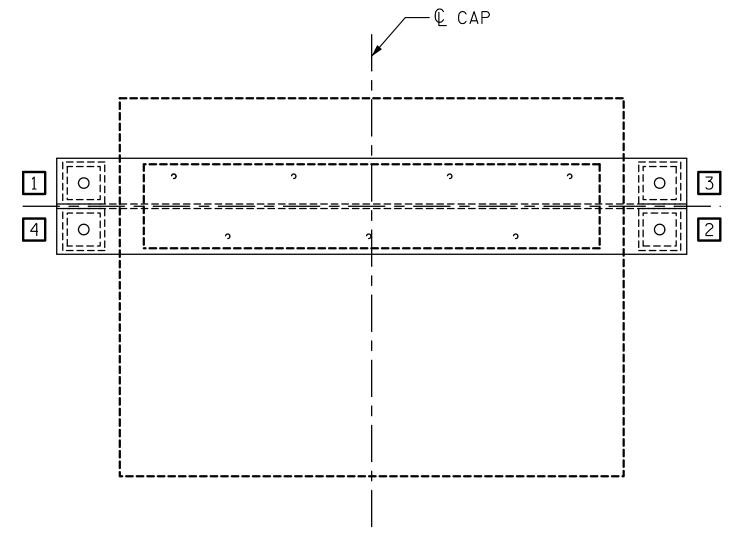
PLACE A CUSTOM COVERING OVER THE PT ANCHORAGE TO PREVENT WATER FROM COLLECTING ON THE ANCHORAGE. POST-TENSIONING BARS SHALL BE STAINLESS STEEL, ASTM A564, ALLOY S17400, TYPE 630 CONDITION H1025. ALL OTHER MECHANICAL PROPERTIES FOR THE BARS SHALL BE IN ACCORDANCE WITH ASTM A722, TYPE II. FOR FURTHER DETAILS, SEE

SPECIAL PROVISIONS.

ALL PHOTOGRAPHS OF THE CONCRETE SUBSTRATE SHALL BE SUBMITTED IN JPEG FORMAT. WHERE STAINLESS STEEL PLATES CONTACT WEATHERING STEEL PLATES, SHOP COAT BOTH SURFACES WITH NCDOT PAINT SYSTEM 4.

## BILL OF MATERIAL

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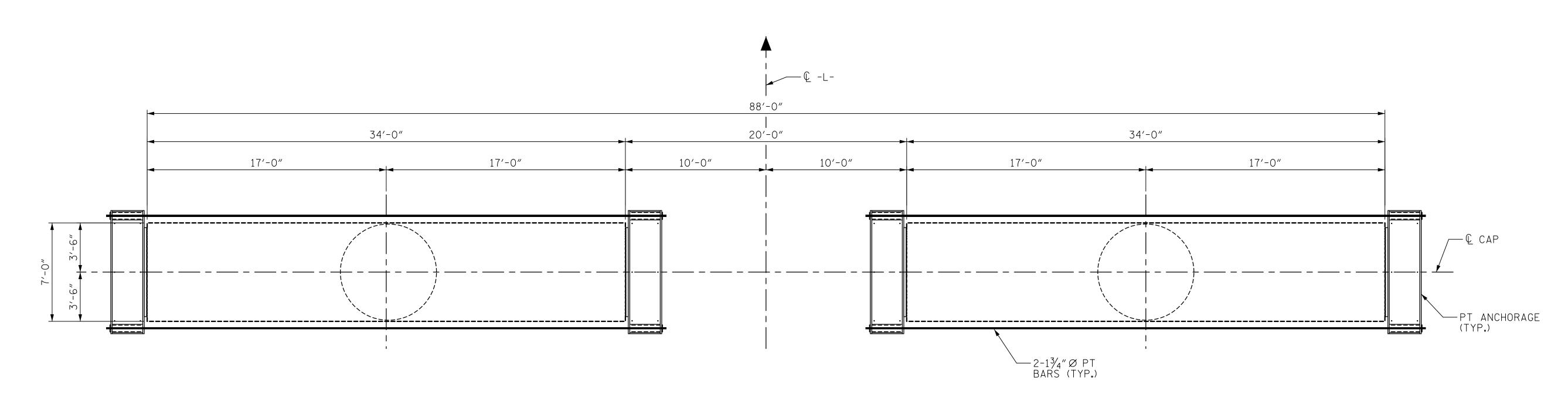


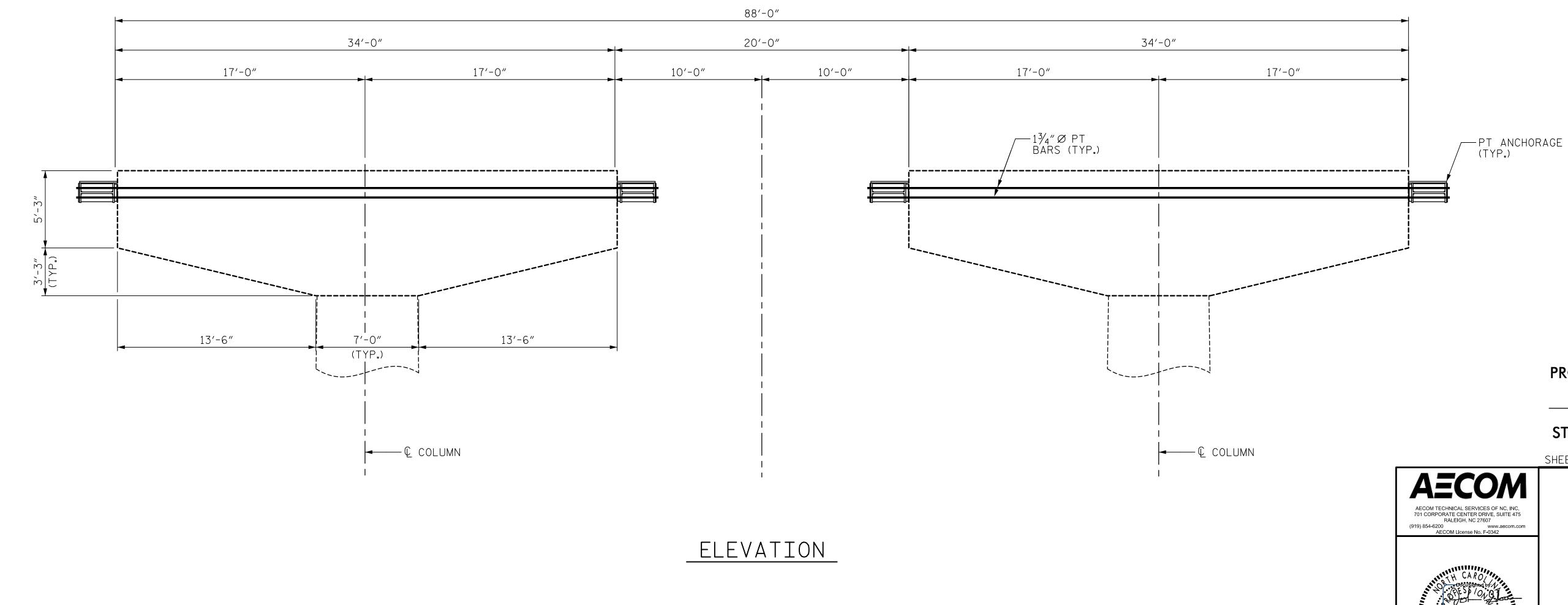
POST-TENSION STRESSING ORDER

COMPLETE THIS SEQUENCE IN PARALLEL WITH THE REMOVAL OF THE CAP STAY-IN-PLACE FORMWORK. NO SEPARATE PAYMENT

EEL WEIGHT ,654 LBS. ,479 LBS.

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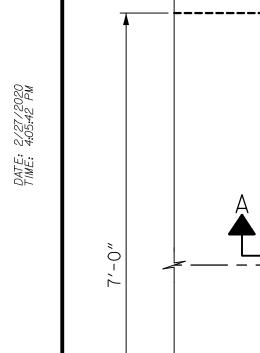
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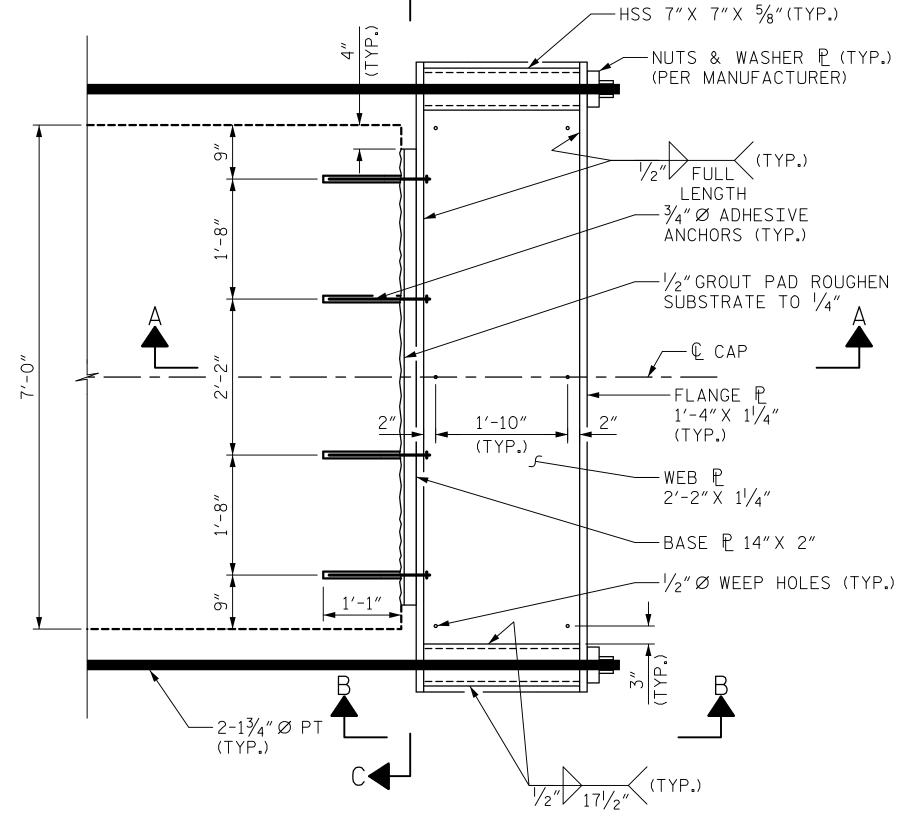
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# PLAN

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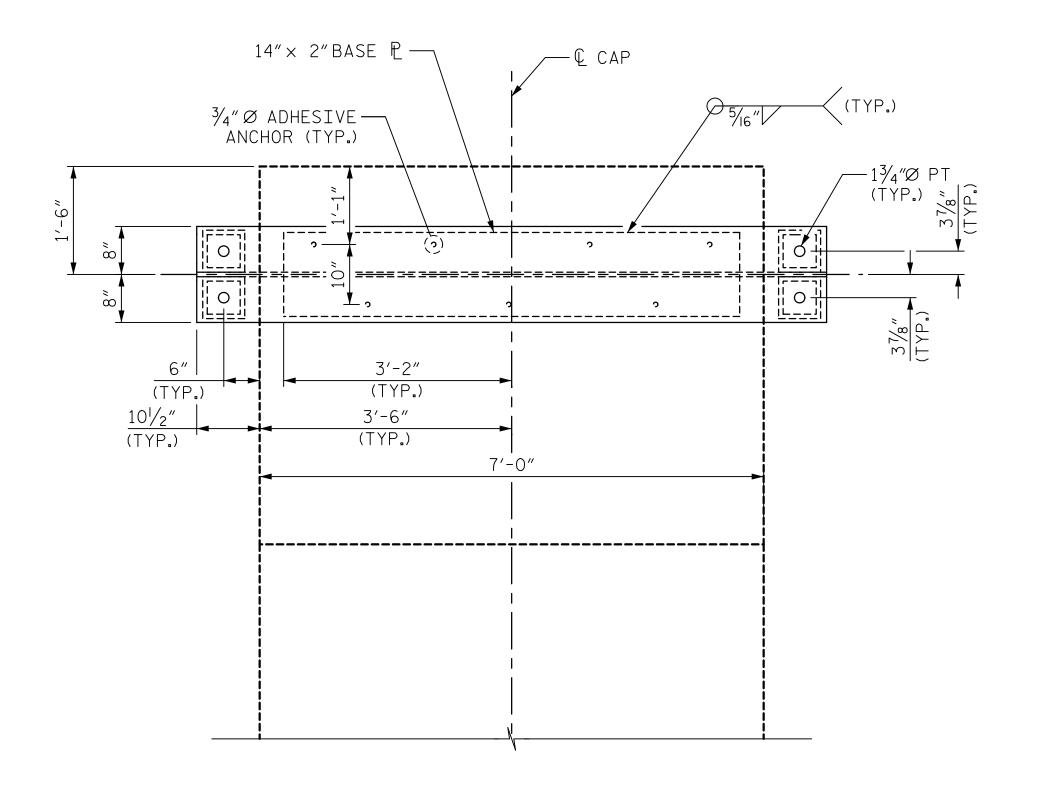


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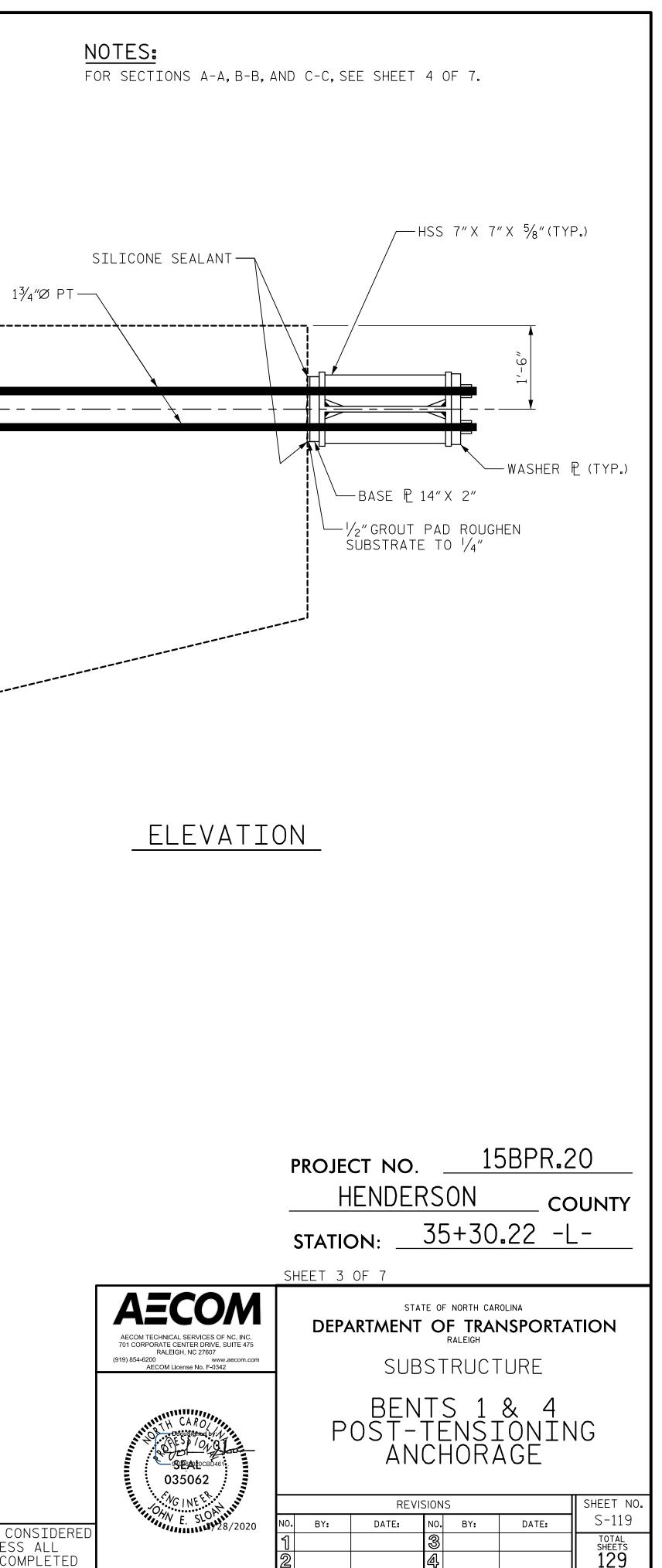
PLAN (COVER NOT SHOWN FOR CLARITY) (BOTTOM ROW OF ADHESIVE ANCHORS NOT SHOWN)

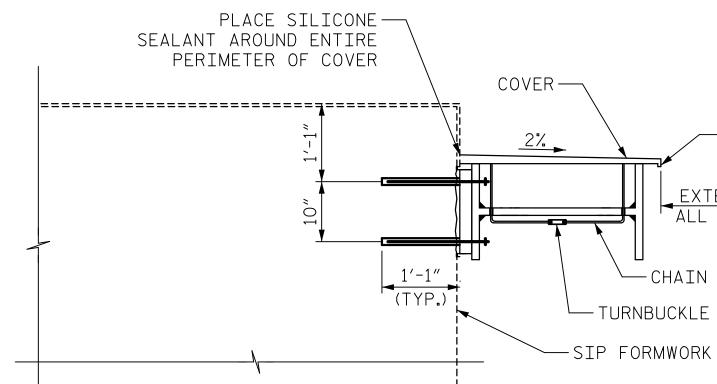
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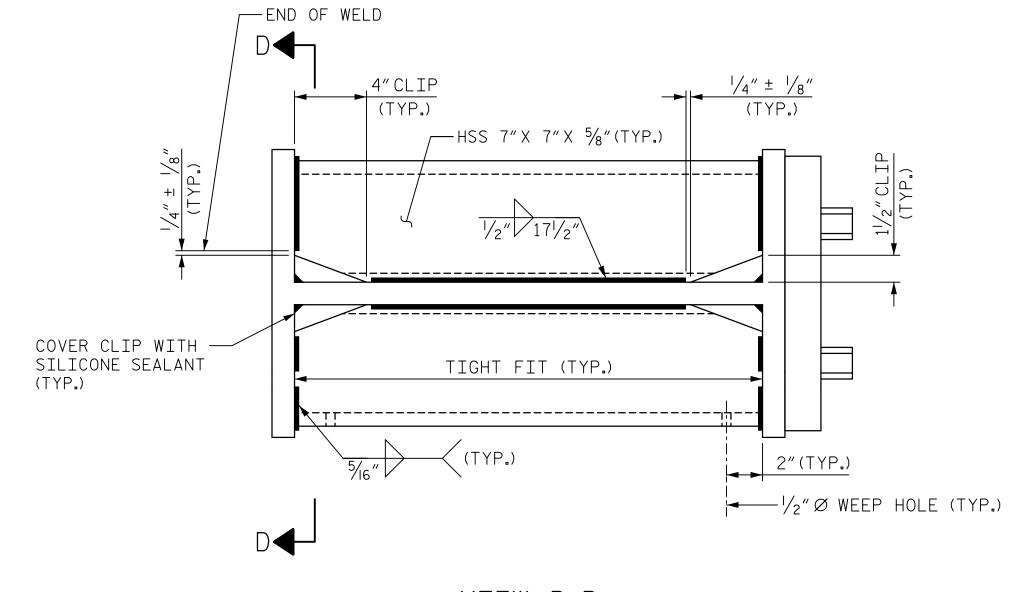


END ELEVATION (COVER NOT SHOWN FOR CLARITY)





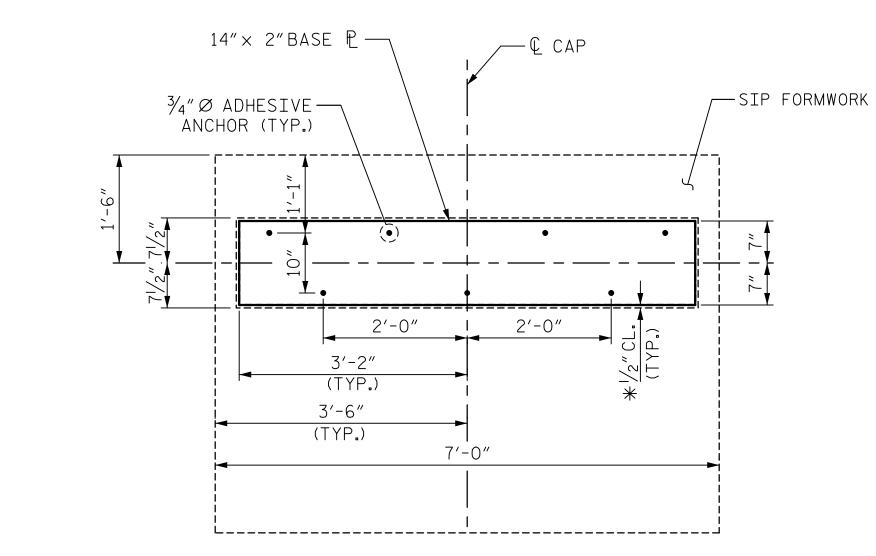
VIEW A-A NOTE: TIE DOWN COVER WITH CHAINS AND TURNBUCKLE AT EACH SET OF WEEP HOLES



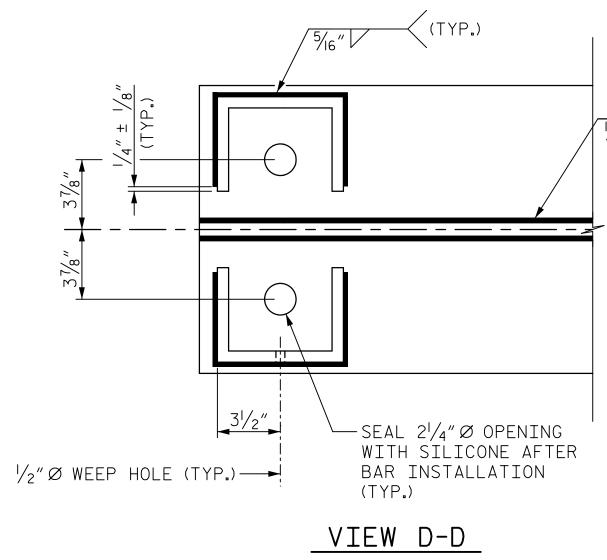
VIEW B-B (POST-TENSIONED BAR NOT SHOWN FOR CLARITY)

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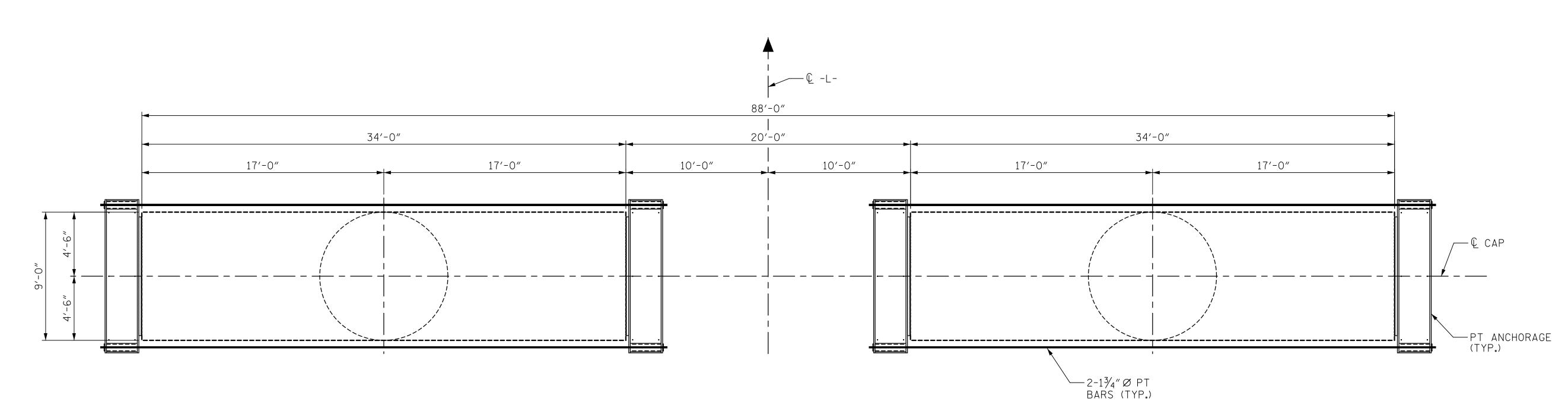


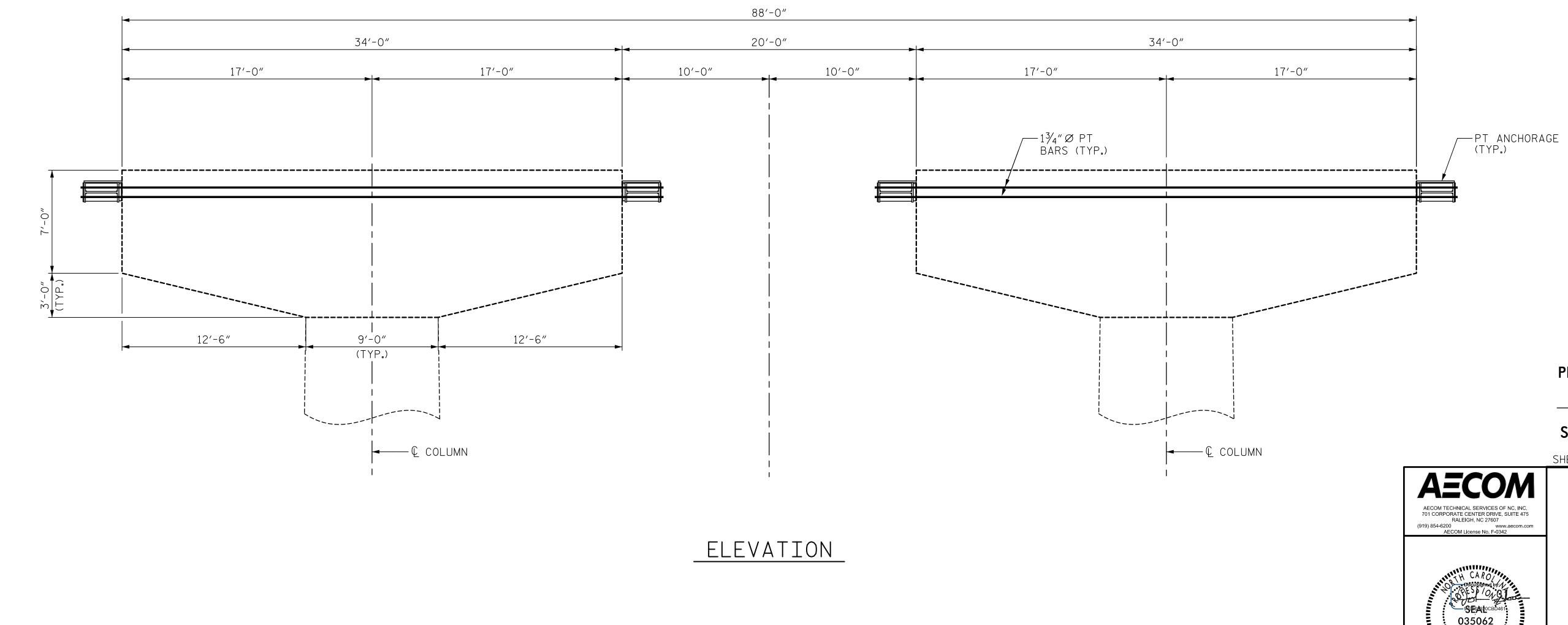
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AECOM TECHNICAL SERVICES OF NC, INC. 701 CORPORATE CENTER DRIVE, SUITE 475 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE
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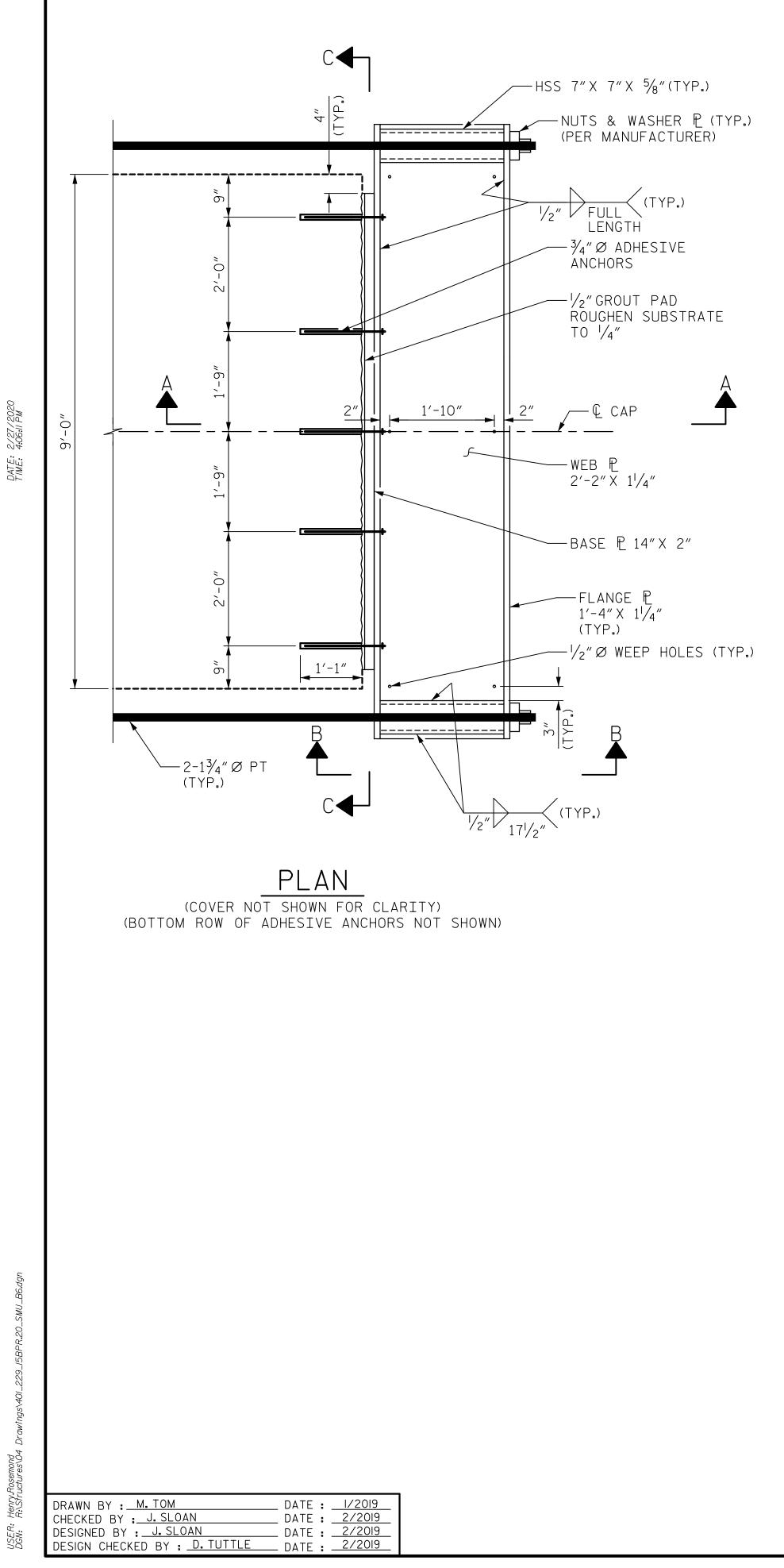
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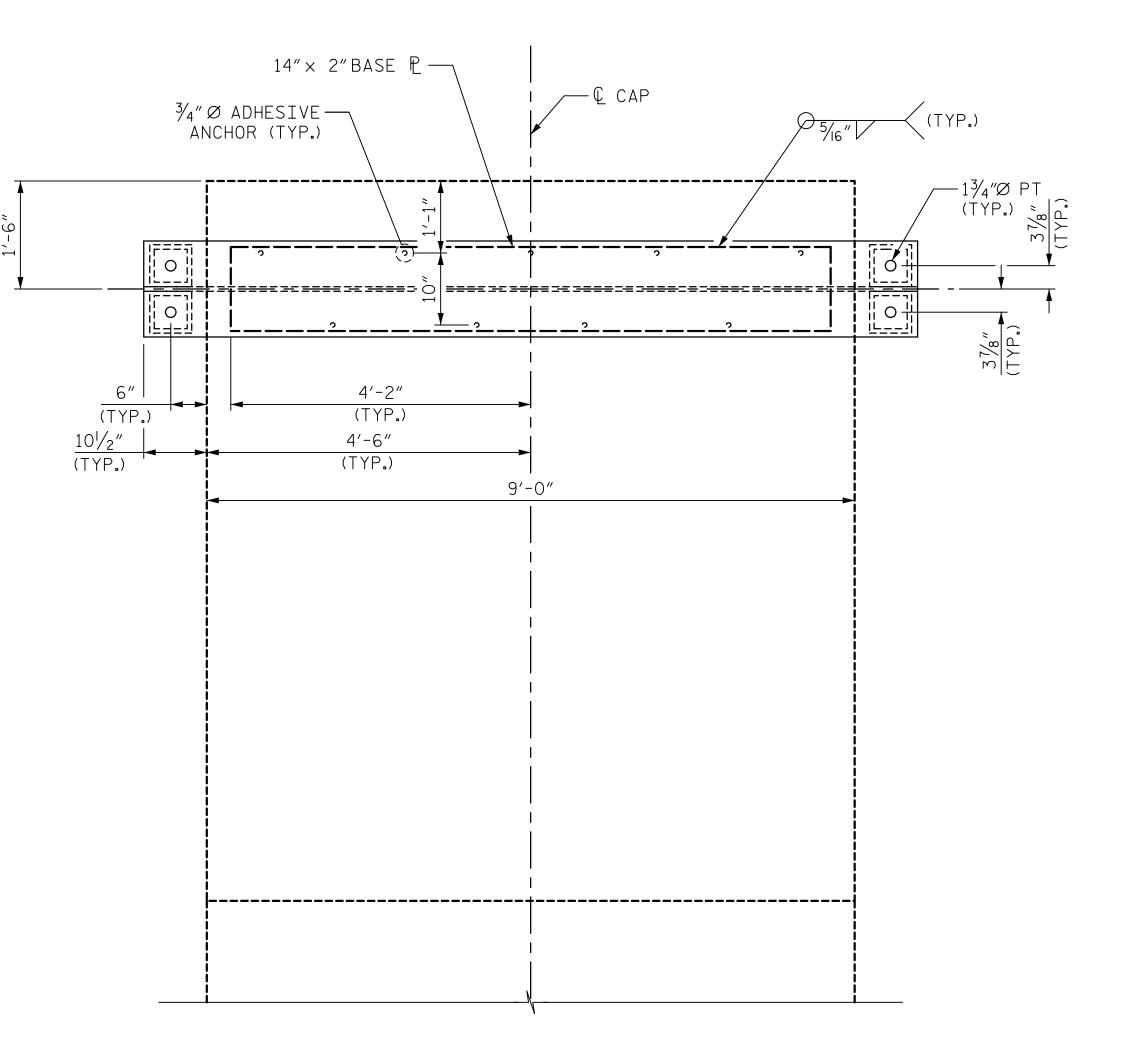
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# <u>PLAN</u>

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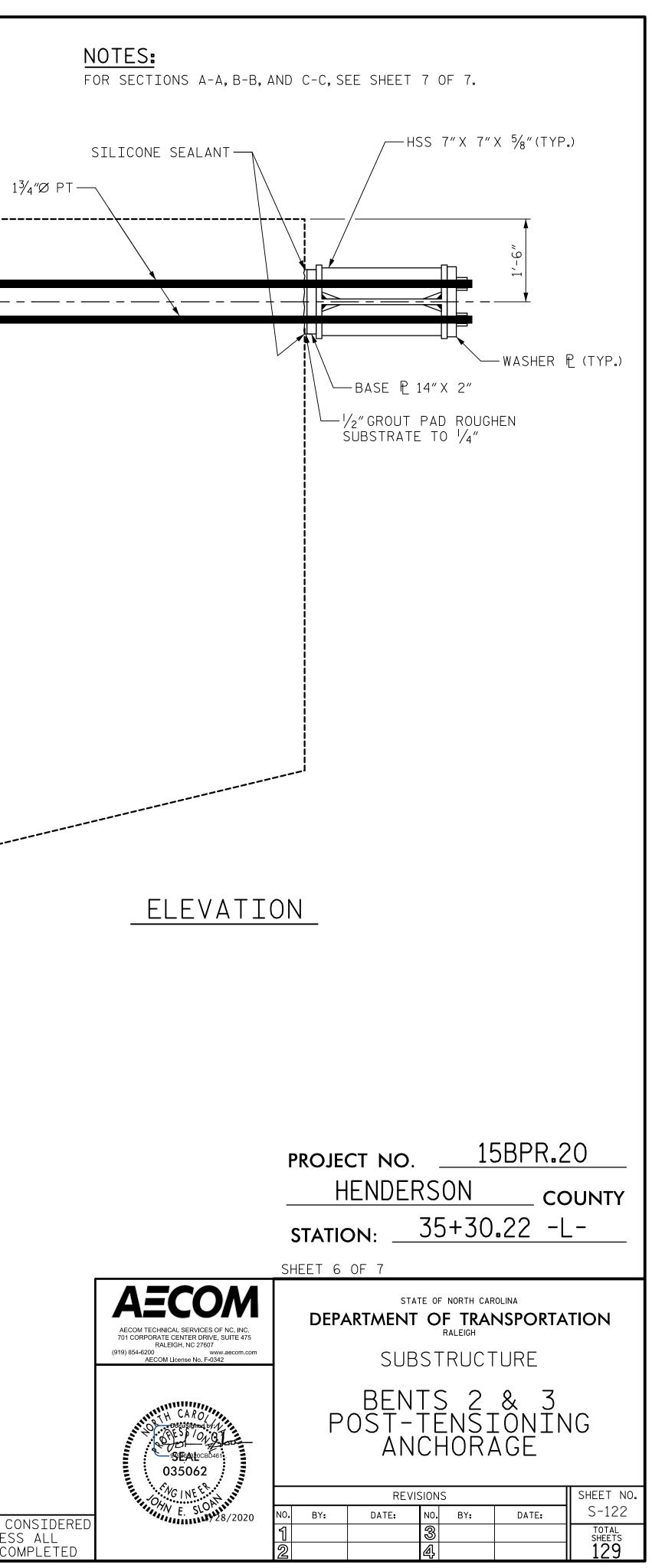
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	SHEET 5 OF 7
AECOM TECHNICAL SERVICES OF NC, INC. 701 CORPORATE CENTER DRIVE, SUITE 475 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE
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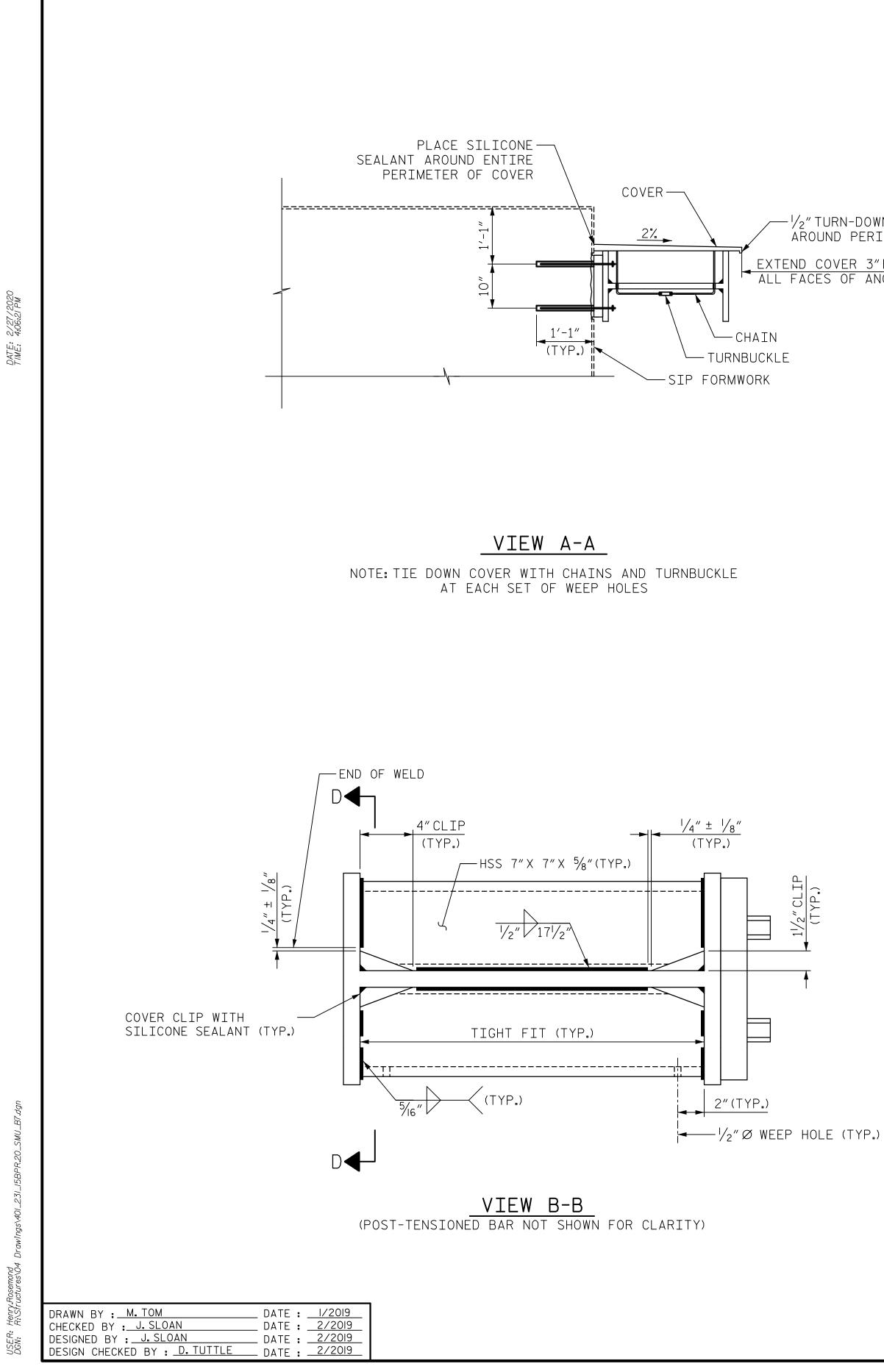


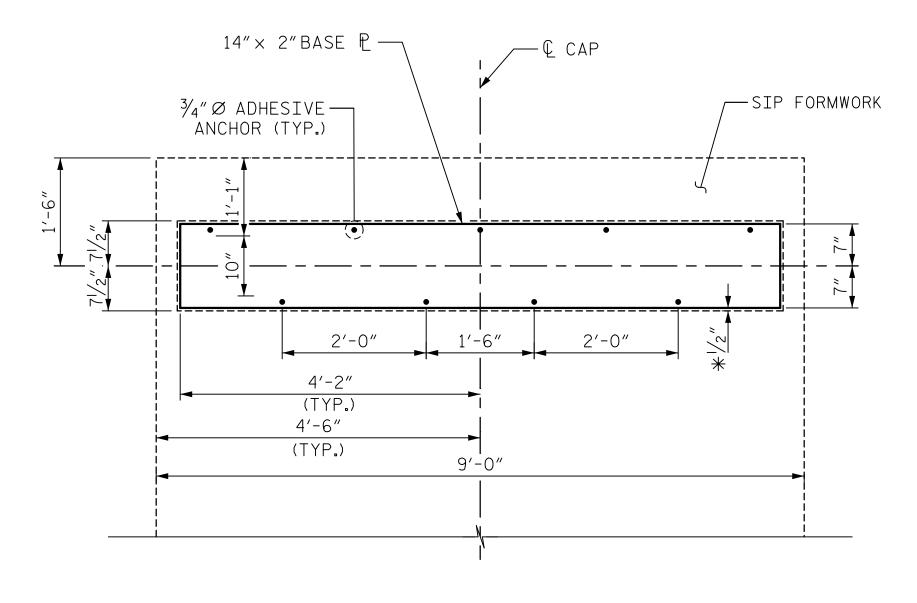


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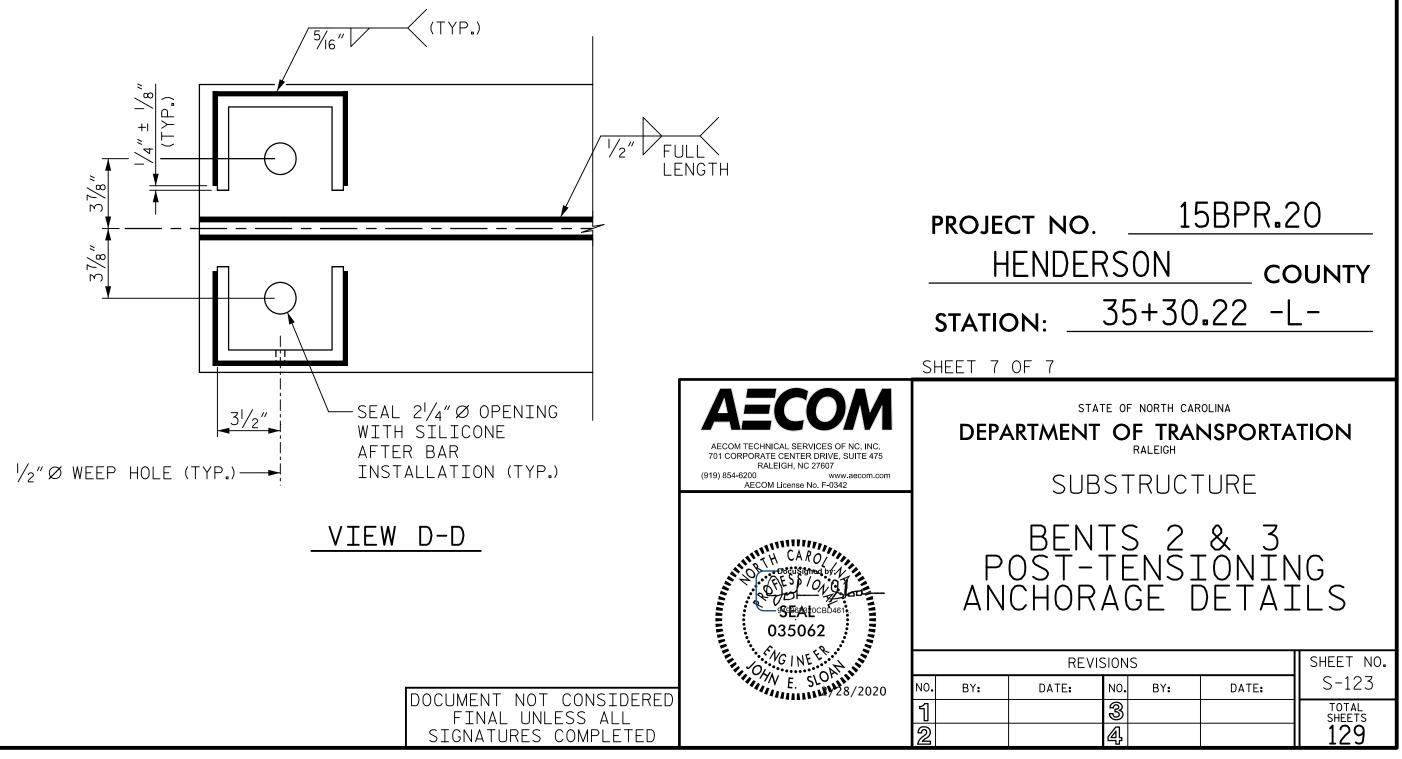
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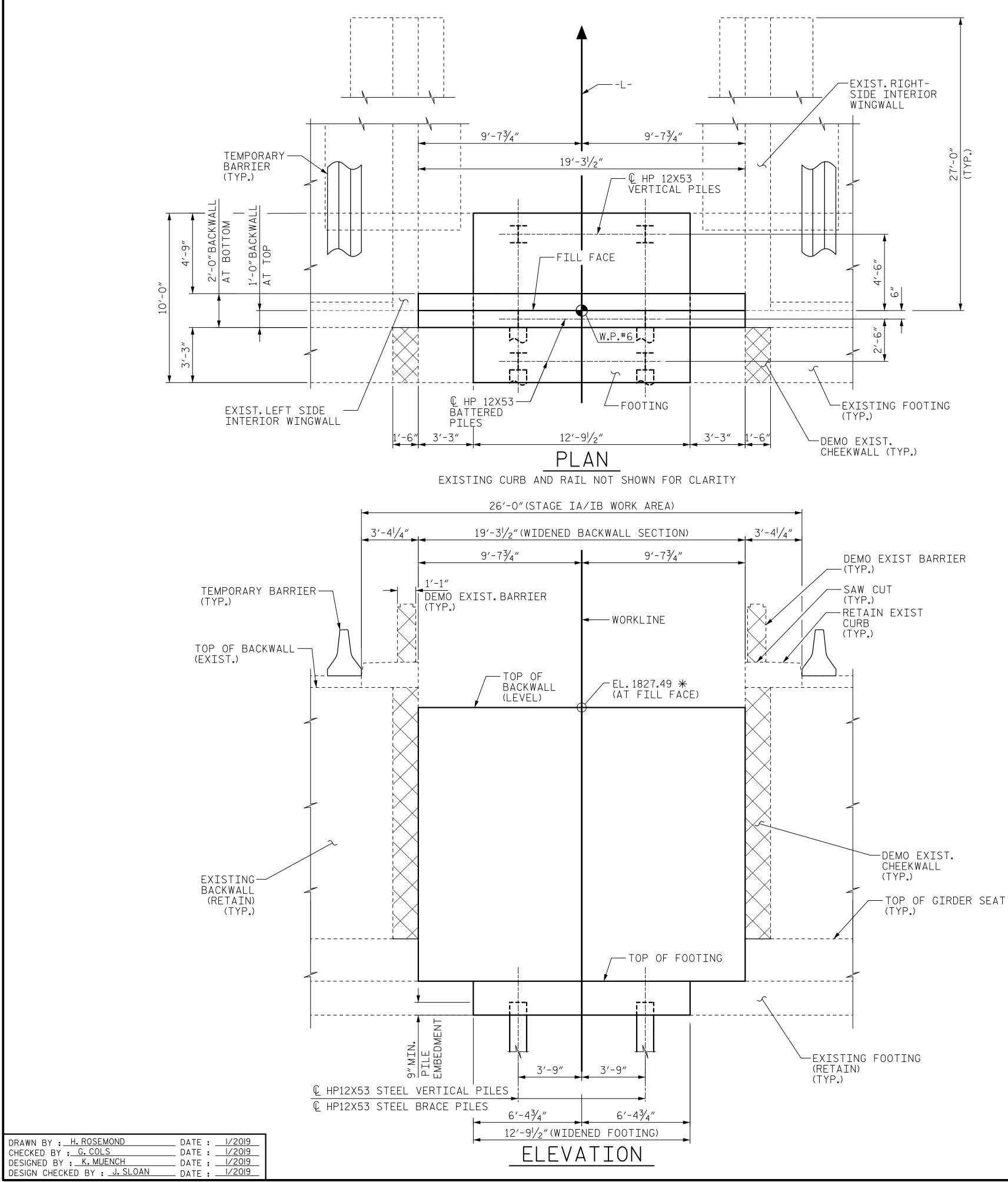
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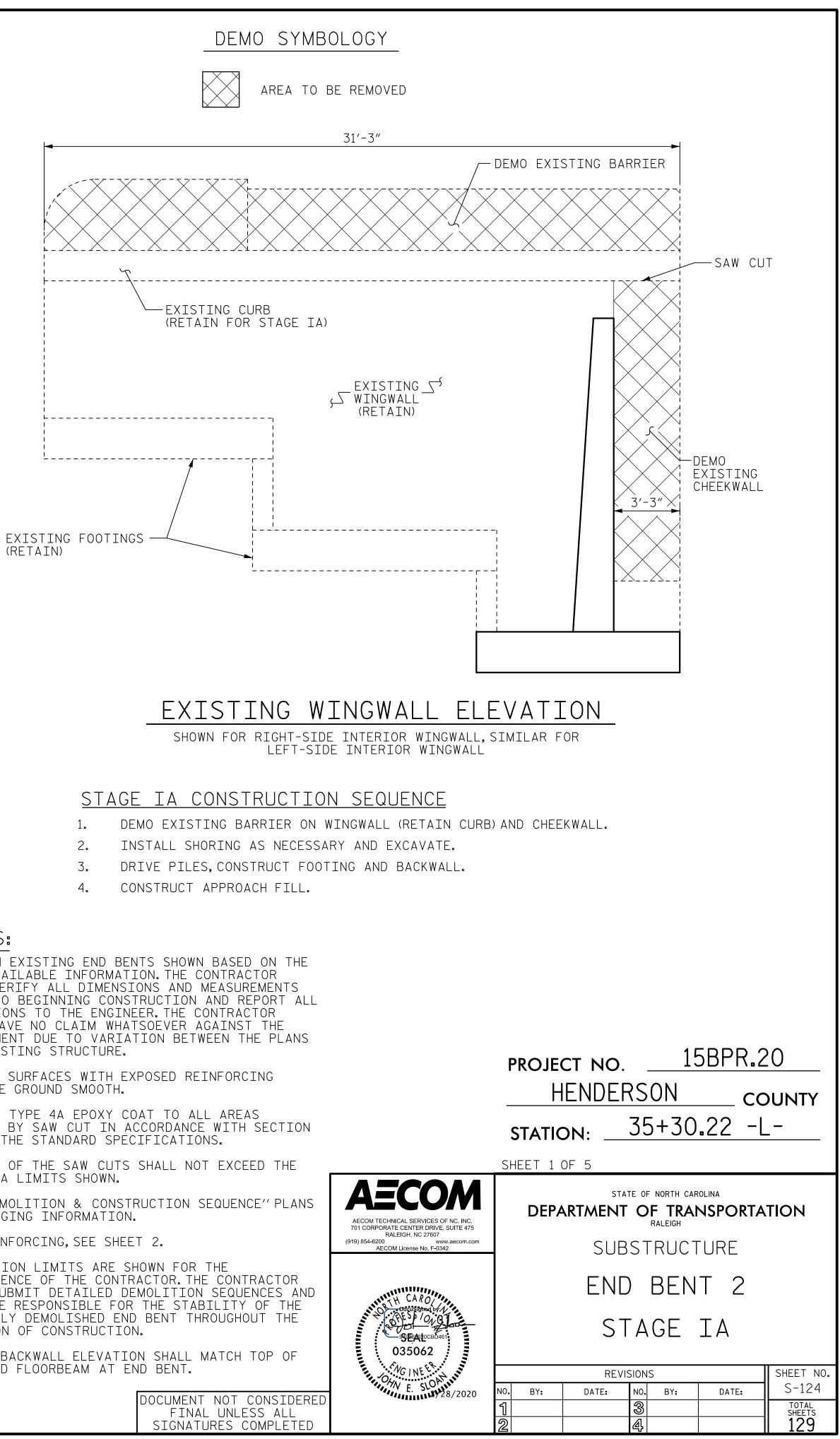


 $-\frac{1}{2}$ " TURN-DOWN AROUND PERIMETER EXTEND COVER 3" BEYOND ALL FACES OF ANCHORAGE

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EXTENTS OF THE SAW CUTS SHALL NOT EXCEED THE STAGE IA LIMITS SHOWN.

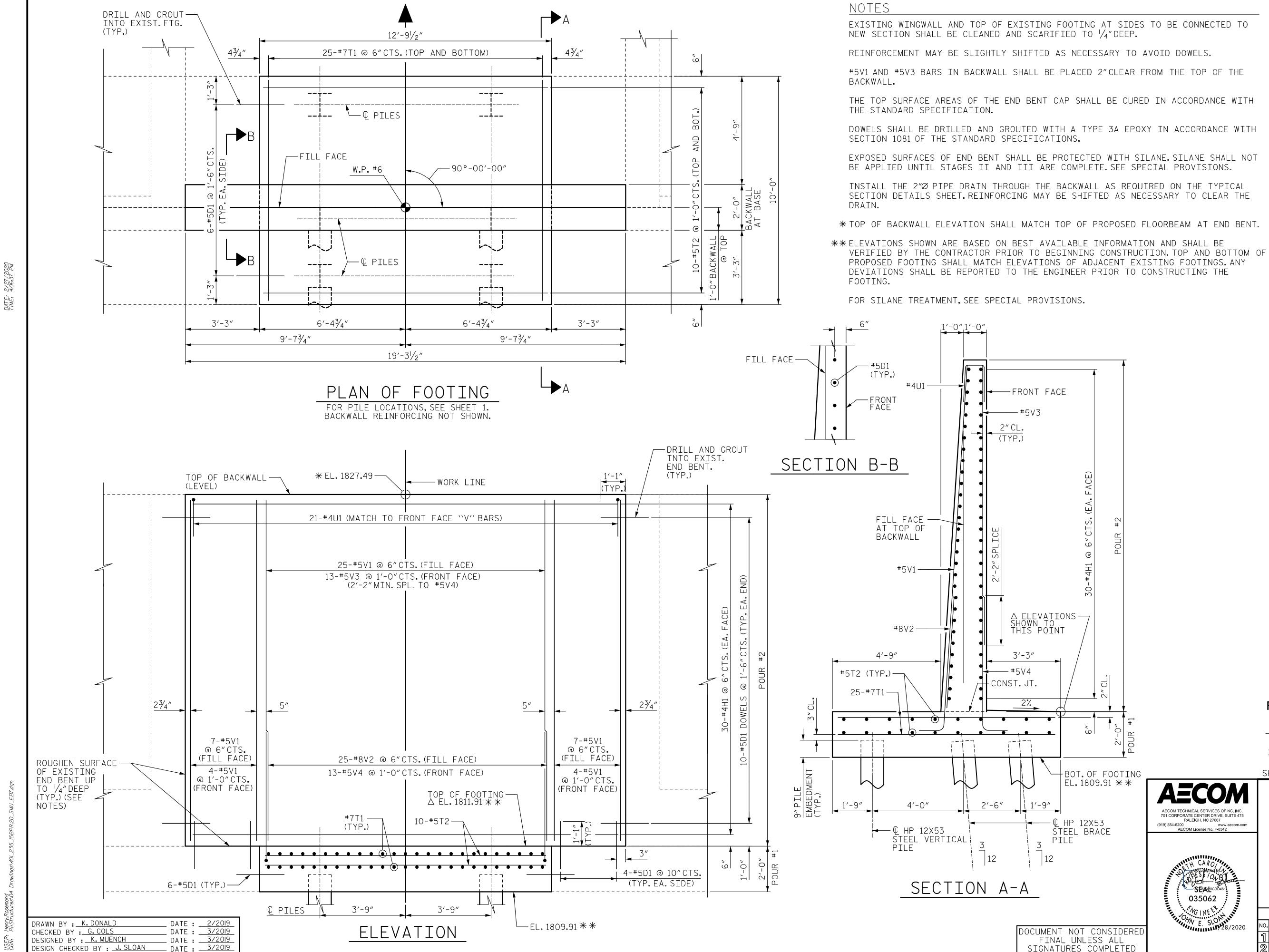
SEE ``DEMOLITION & CONSTRUCTION SEQUENCE'' PLANS FOR STAGING INFORMATION.

FOR REINFORCING, SEE SHEET 2.

DEMOLITION LIMITS ARE SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL SUBMIT DETAILED DEMOLITION SEQUENCES AND SHALL BE RESPONSIBLE FOR THE STABILITY OF THE PARTIALLY DEMOLISHED END BENT THROUGHOUT THE DURATION OF CONSTRUCTION.

\*TOP OF BACKWALL ELEVATION SHALL MATCH TOP OF PROPOSED FLOORBEAM AT END BENT.

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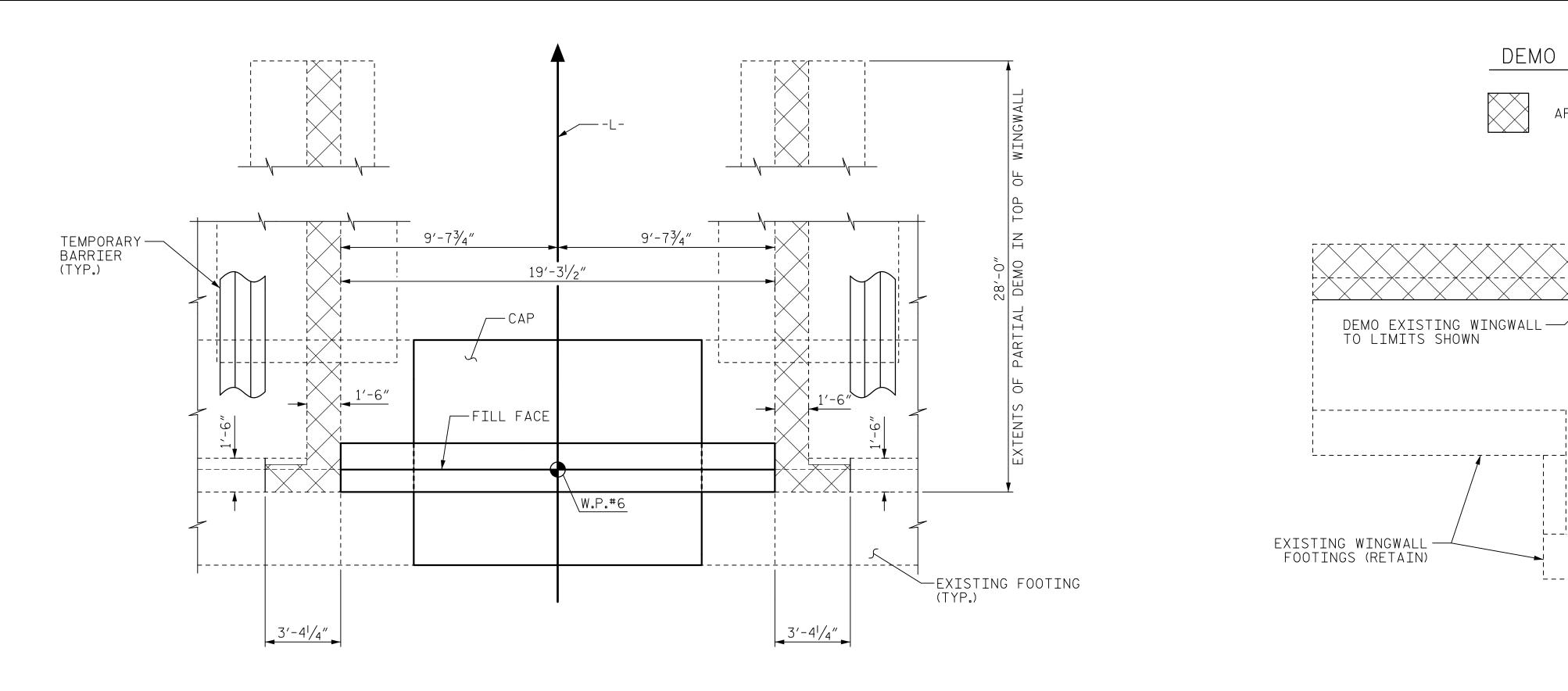
AECOM TECHNICAL SERVICES OF NC, INC. 701 CORPORATE CENTER DRIVE, SUITE 475 RALEIGH, NC 27607

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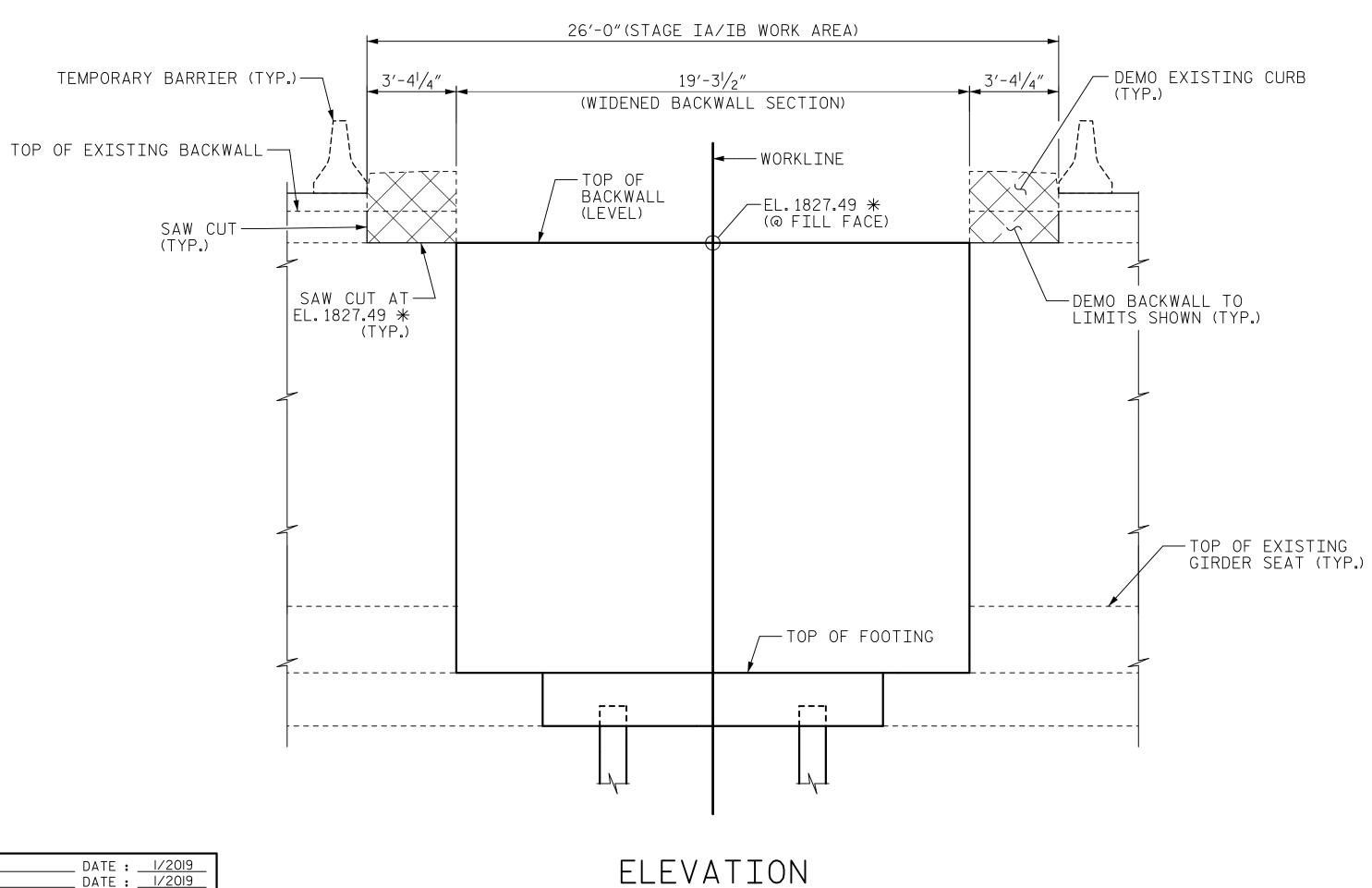
- **ЗЕАН**ОСВБ

035062

		_		MAT	ERIAL	
	BAR	No.	SIZE	TYPE	LENGTH	WEIGHT
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	H1	60	4	STR	18'-11''	757
	T1 T2	50 20	7 5	STR STR	9′-8′′ 12′-5′′	988 259
	U1	21	4	1	3'-8''	51
	V1	47	5	STR	15'-0''	735
	V2	25	8	2	7'-3''	484
	V3	13	5	STR	12'-5''	168
	V4	13	5	2	6'-9''	92
	ΤΟΤΑ ΤΟΤΑ	L CLAS	NFORCIN SS A CC	NCRETE		6.0 CY
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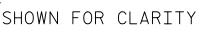


PLAN EXISTING CURB AND RAIL NOT SHOWN FOR CLARITY



2/27/2020 4.06:45 PM DATE: TIME:

DRAWN BY :_ H. ROSEMOND	_ DATE : _ 1/2019
CHECKED BY : G.COLS	DATE : 1/2019
DESIGNED BY : K. MUENCH	_ DATE :
	_ DATE :



1. DEMO EXISTING CURB. TO LIMITS SHOWN.



DATA ON EXISTING END BENTS SHOWN BASED ON BEST AVAILABLE INFORMATION. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND MEASUREMEN PRIOR TO BEGINNING CONSTRUCTION AND REPORT VARIATIONS TO THE ENGINEER. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT DUE TO VARIATION BETWEEN THE PLA AND EXISTING STRUCTURE.

ALL CUT SURFACES WITH EXPOSED REINFORCING SHALL BE GROUND SMOOTH.

APPLY A TYPE 4A EPOXY COAT TO ALL AREAS EXPOSED BY SAW CUT IN ACCORDANCE WITH SECT 1081 OF THE STANDARD SPECIFICATIONS.

EXTENTS OF THE SAW CUTS SHALL NOT EXCEED TH STAGE IB LIMITS SHOWN.

SEE "DEMOLITION & CONSTRUCTION SEQUENCE" PL FOR STAGING INFORMATION.

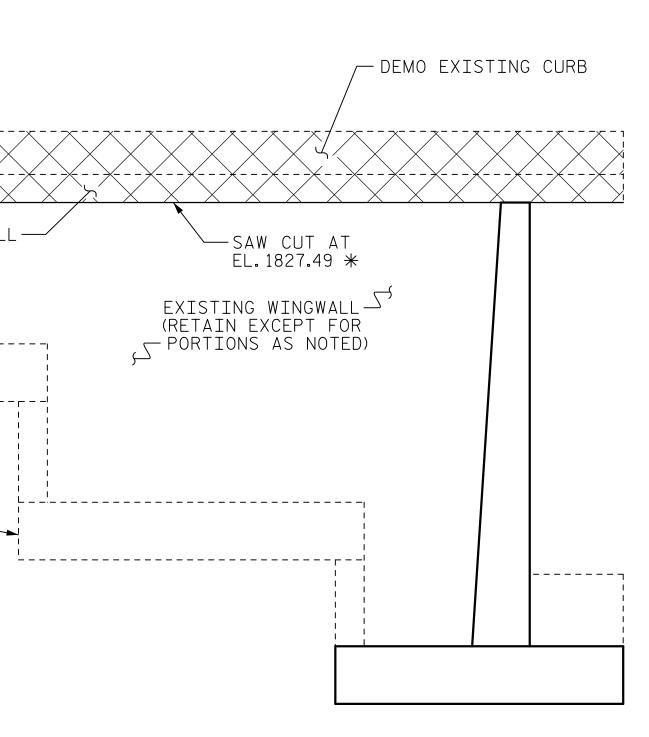
DEMOLITION LIMITS ARE SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTO SHALL SUBMIT DETAILED DEMOLITION SEQUENCES SHALL BE RESPONSIBLE FOR THE STABILITY OF T PARTIALLY DEMOLISHED END BENT THROUGHOUT TH DURATION OF CONSTRUCTION.

\* TOP OF BACKWALL ELEVATION SHALL MATCH TOP O PROPOSED FLOORBEAM AT END BENT.

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## DEMO SYMBOLOGY

AREA TO BE REMOVED



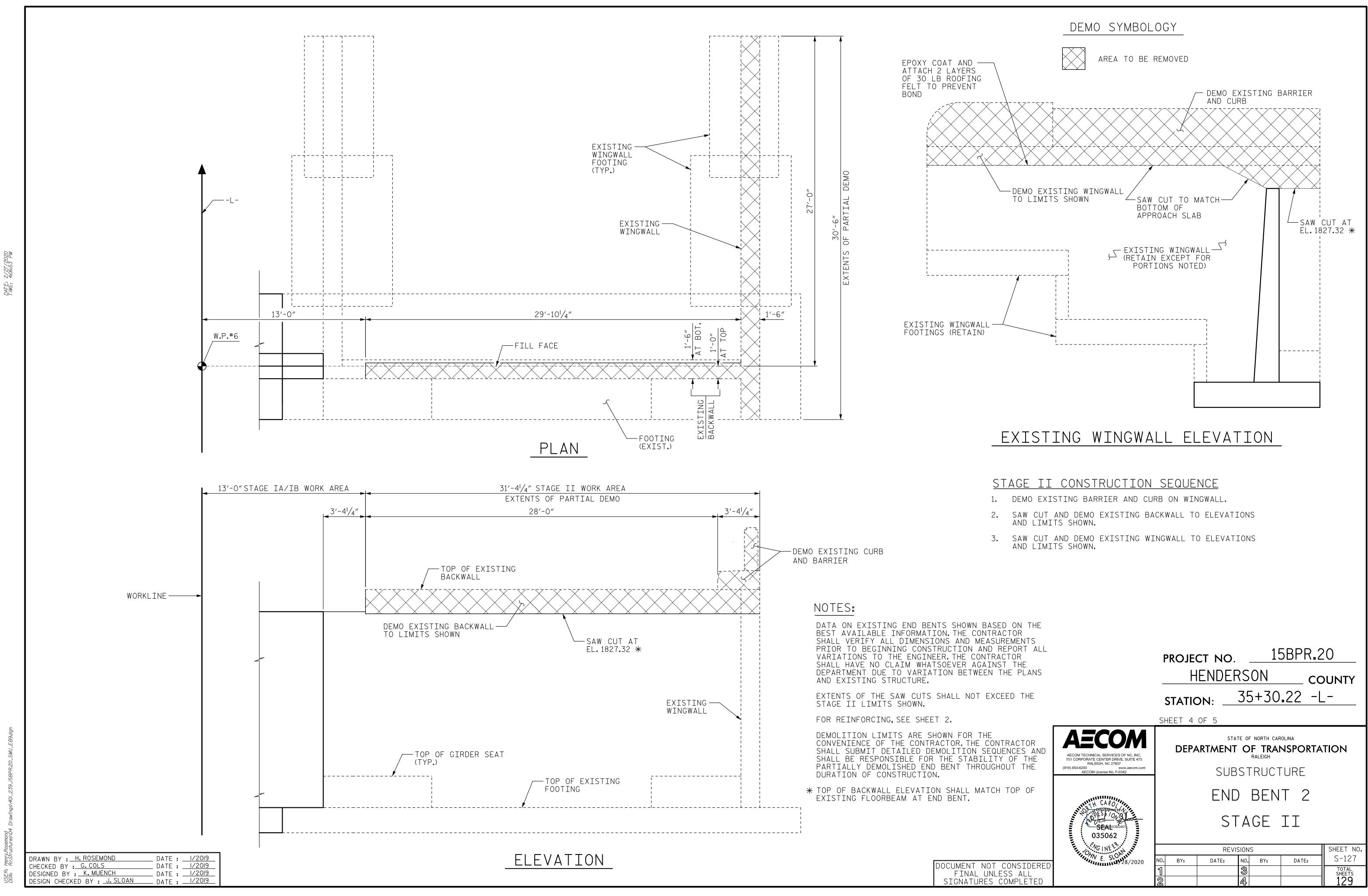
# EXISTING WINGWALL ELEVATION

SHOWN FOR RIGHT-SIDE INTERIOR WINGWALL, SIMILAR FOR LEFT-SIDE INTERIOR WINGWALL

## STAGE IB CONSTRUCTION SEQUENCE

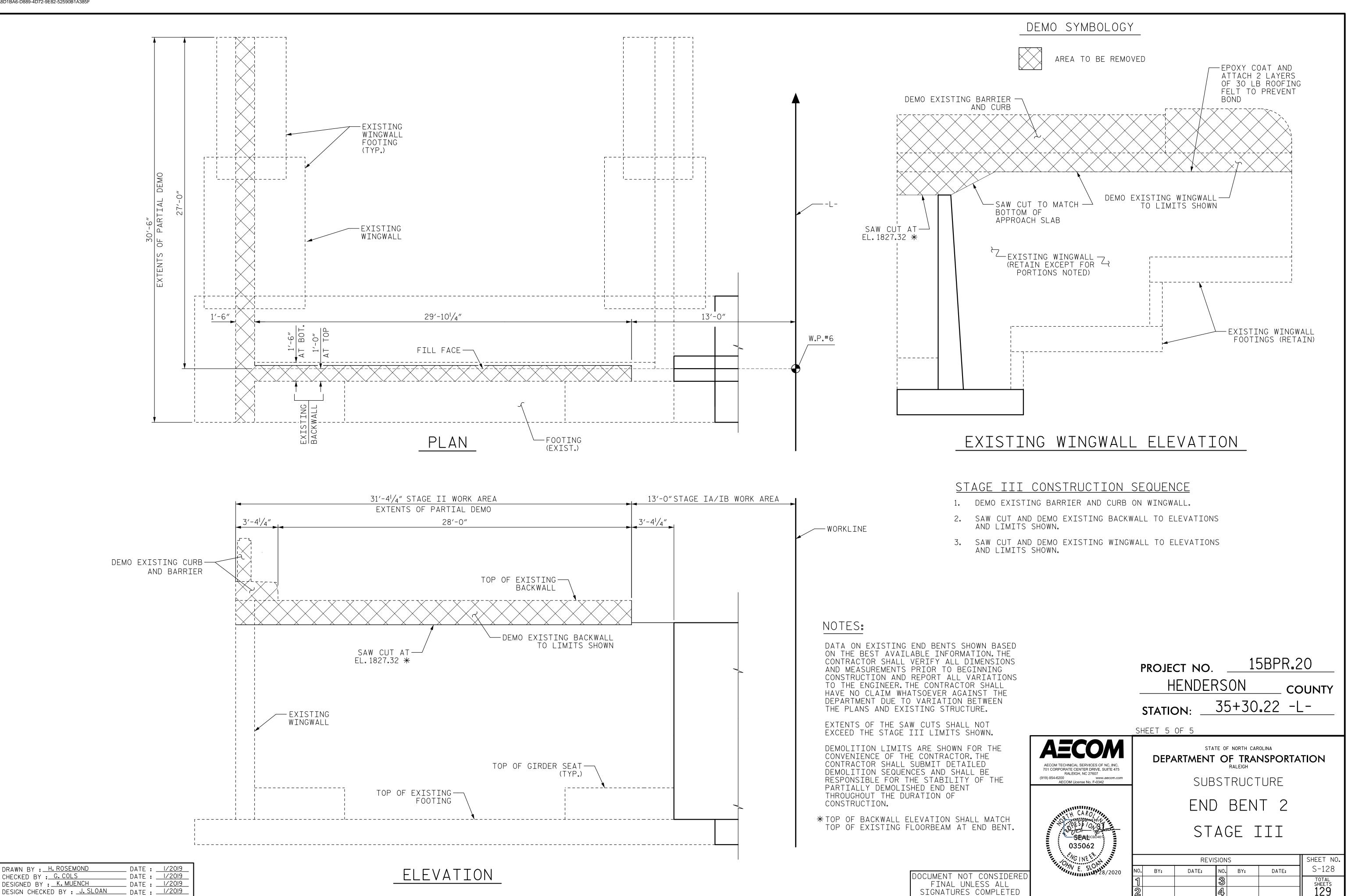
2. PARTIALLY DEMO EXISTING WINGWALL AND BACKWALL

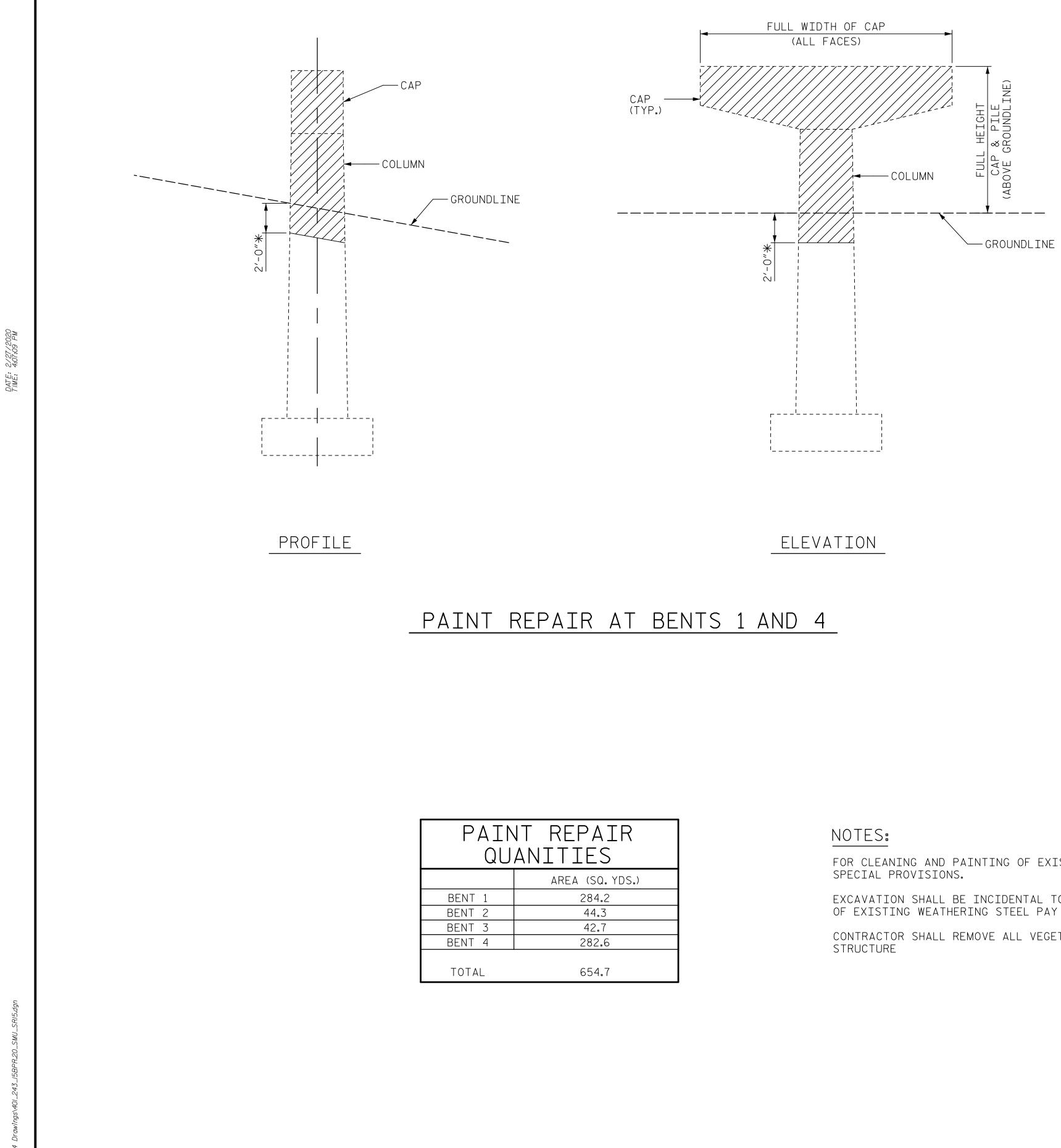
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IE .ANS	AECOM TECHNICAL SERVICES OF NC, INC. 701 CORPORATE CENTER DRIVE, SUITE 475 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342	state of north carolina <b>DEPARTMENT OF TRANSPORTA</b> Raleigh SUBSTRUCTURE	TION
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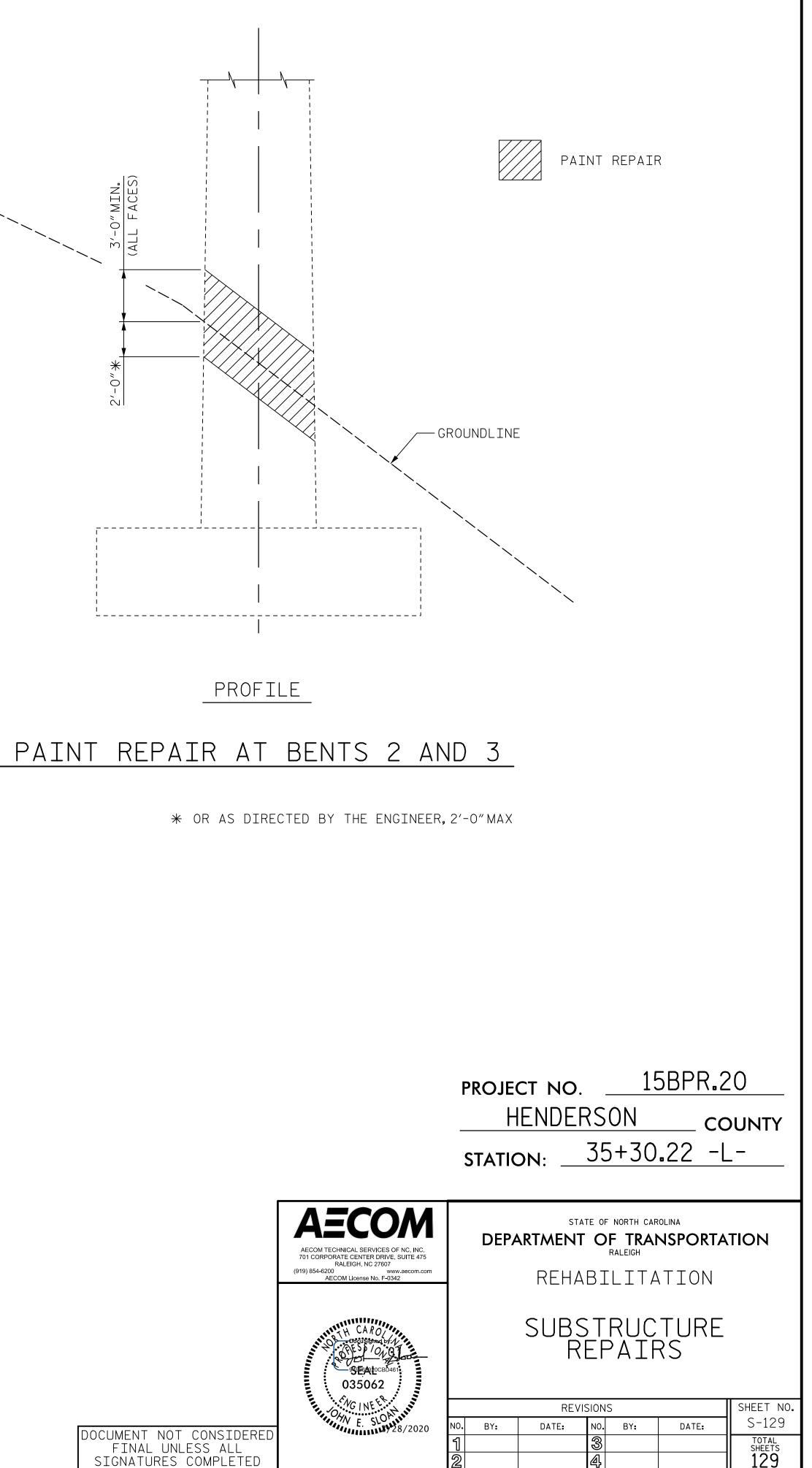
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	DRAWN BY : H. ROSEMOND	DATE : 01/2019
	CHECKED BY : G. COLS	DATE : 01/2019
		DATE : 01/2019
		DATE : 03/2019

3'-0" MIN. (ALL FACES)

FOR CLEANING AND PAINTING OF EXISTING WEATHERING STEEL, SEE

EXCAVATION SHALL BE INCIDENTAL TO THE CLEANING AND PAINTING OF EXISTING WEATHERING STEEL PAY ITEM.

CONTRACTOR SHALL REMOVE ALL VEGETATION AND FILTH FROM THE



2/27/2020 4:07:18 PM

DATE: TIME:

## DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED  $\frac{3}{4}$  with the following exceptions: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUIL INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

## STANDARD NOTES

### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK. THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES. SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{1}{6}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 1/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION. SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST  $\frac{5}{16}$ " IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.



STD. NO. SN