

FEBRUARY 16, 2021

2018 STANDARD SPECIFICATIONS

MARC CHEEK, PE STRUCTURES DESIGN ENGINEER

SIGNATURE:

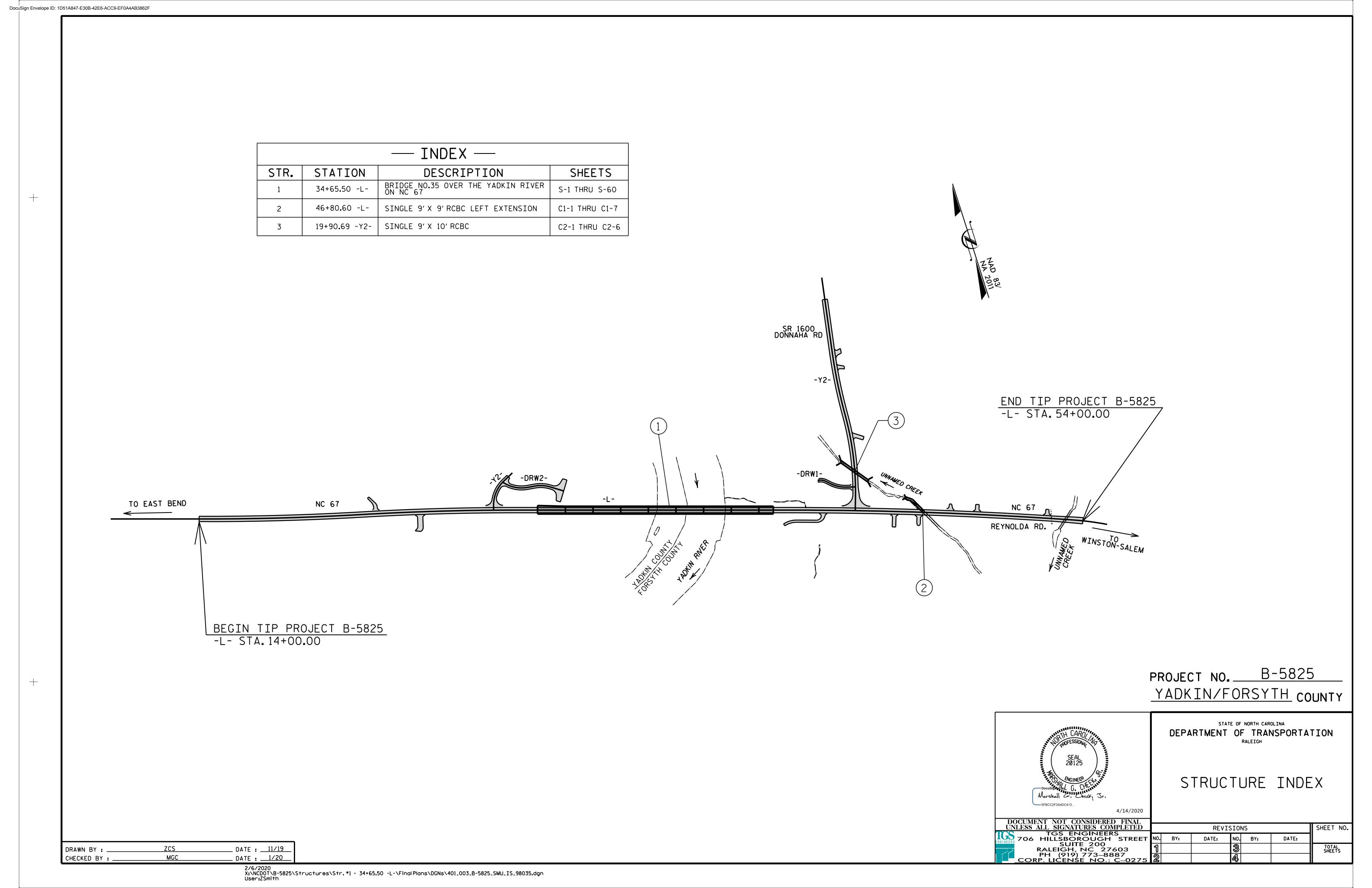
|2/7/2020 \$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$DGN\$\$\$\$\$\$ |Jser:sbwilligms

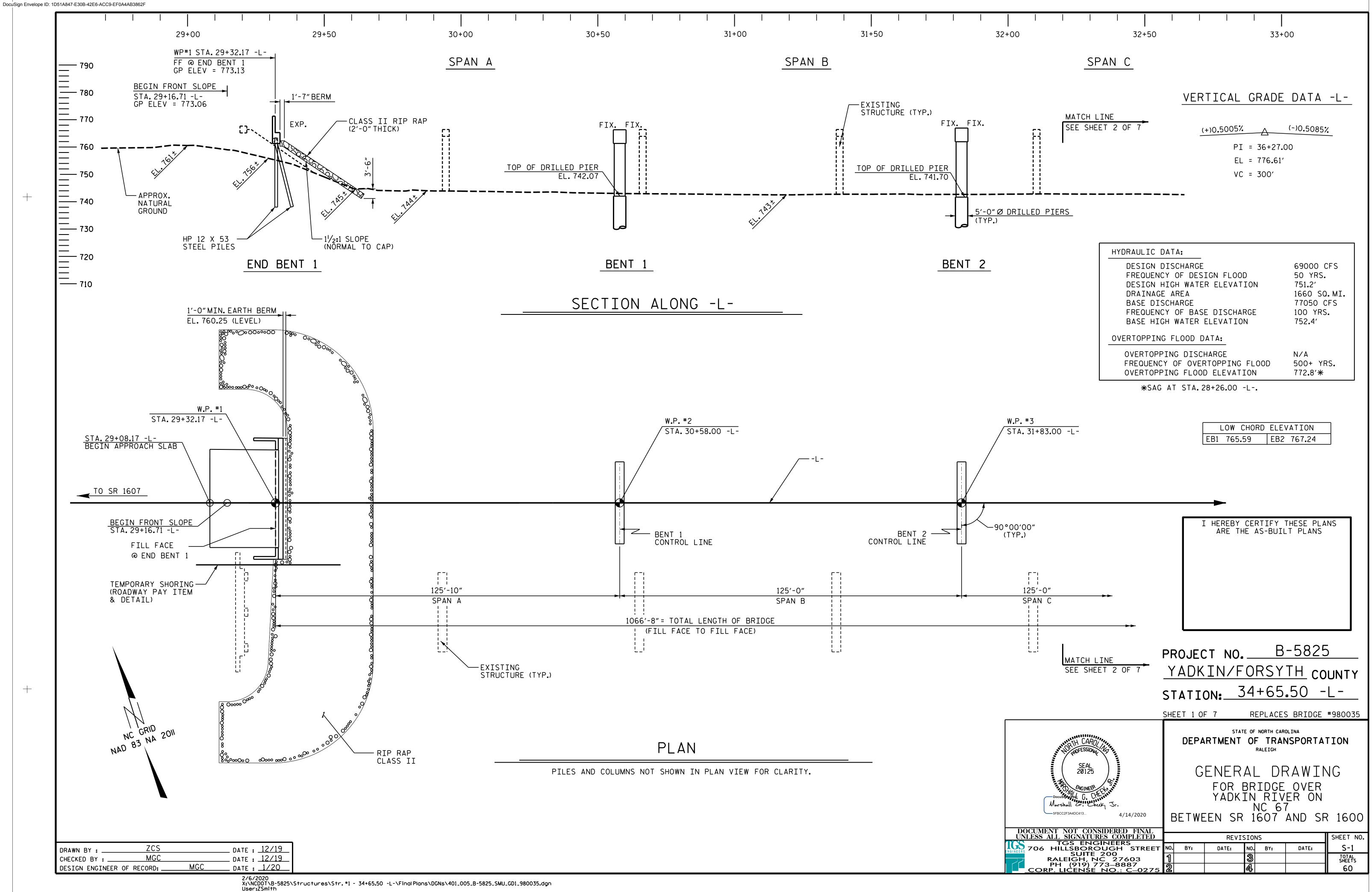
* TTST = 1% DUAL 3%

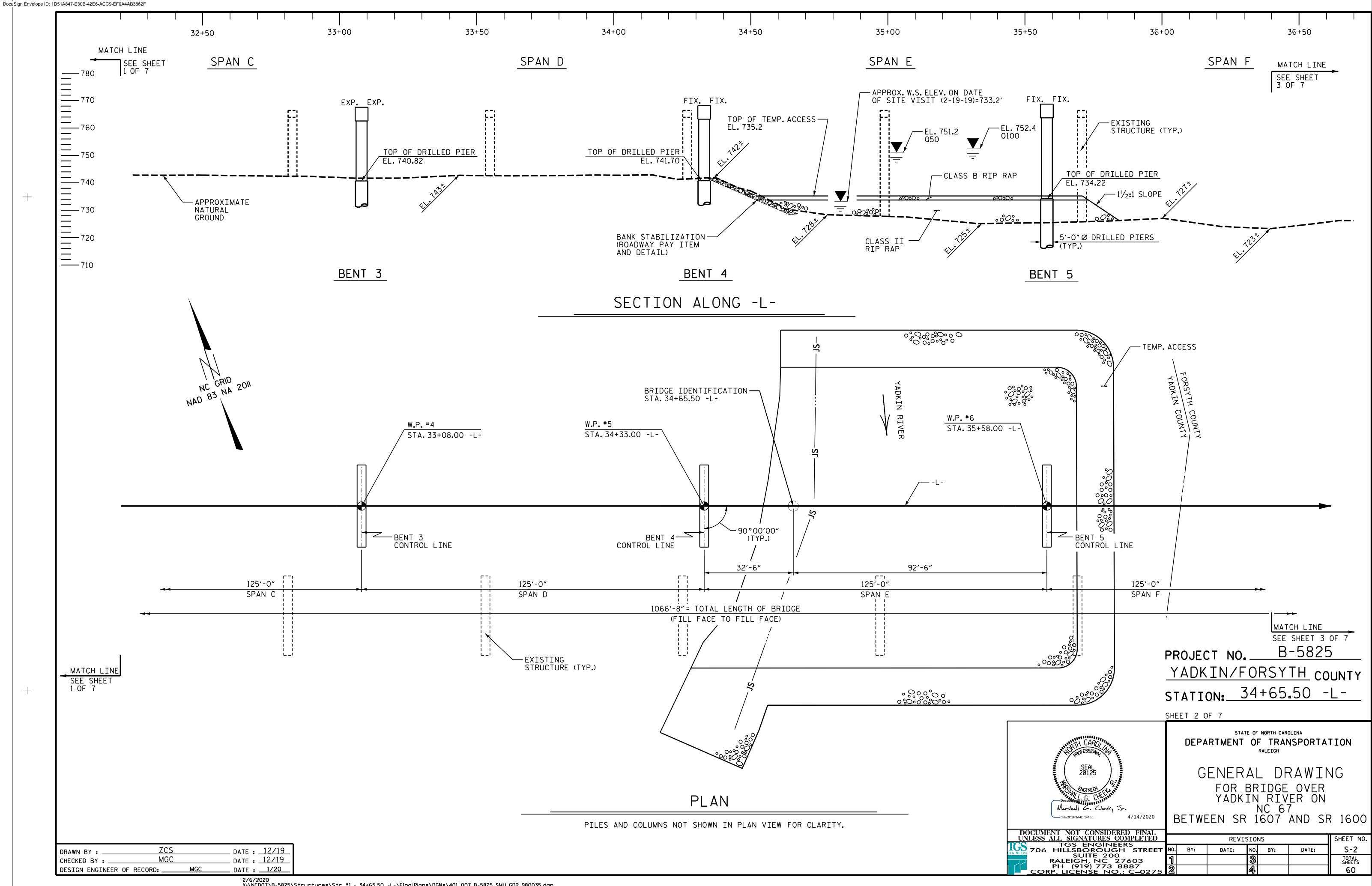
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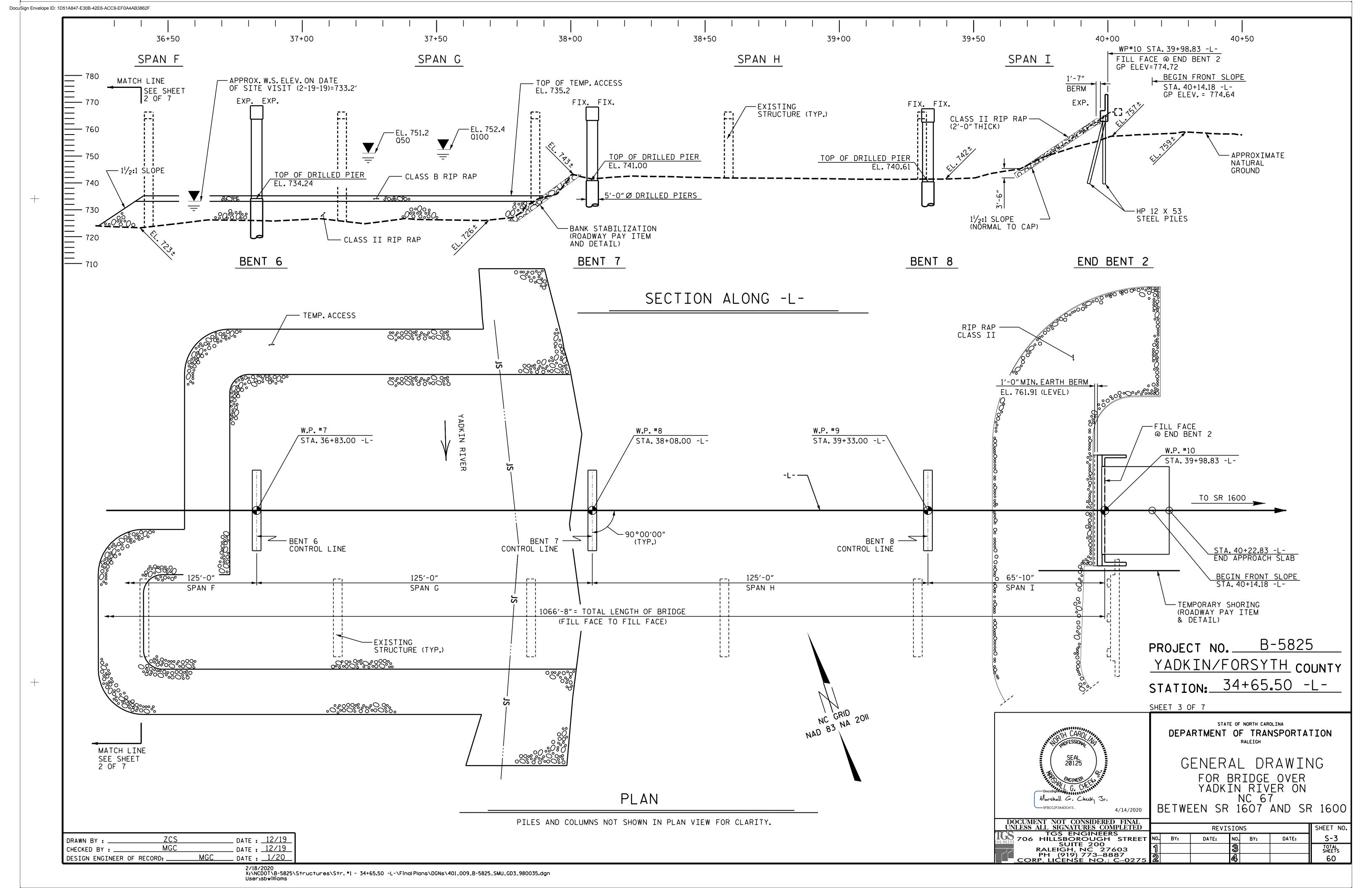
MINOR ARTERIAL

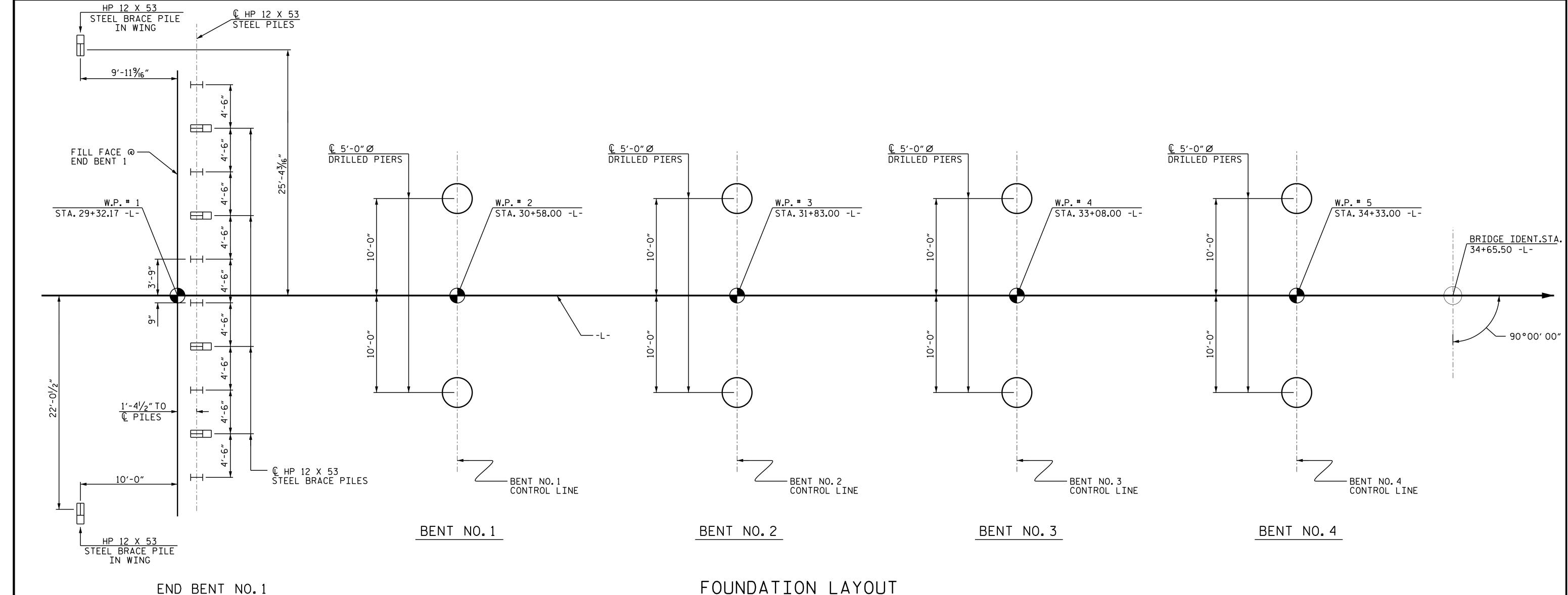
REGIONAL TIER











FOUNDATION LAYOUT

ALL END BENT PILES ARE HP 12 X 53. DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINE AT THE BOTTOM OF THE CAP. END BENT BRACE PILES ARE BATTERED 3:12.

NOTES

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 95 TONS PER PILE. DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 158 TONS PER PILE.

DRILLED PIERS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 784.0 TONS PER PIER.

INSTALL DRILLED PIERS AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 698.0 FEET (LT) AND 687 FEET (RT) WITH THE REQUIRED TIP RESISTANCE AND A PENETRATION OF AT LEAST 20 FEET OF

WEATHERED ROCK OR 12 FEET OF ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS. THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 IS ELEVATION 718.0 FEET. THE SCOUR CRITICAL

ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

DRILLED PIERS AT BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 784.0 TONS PER PIER.

INSTALL DRILLED PIERS AT BENT NO.2 TO A TIP ELEVATION NO HIGHER THAN 705.5 FEET WITH THE REQUIRED TIP RESISTANCE AND A PENETRATION OF AT LEAST 20 FEET INTO WEATHERED ROCK.

THE SCOUR CRITICAL ELEVATION FOR BENT NO. 2 IS ELEVATION 724.0 FEET. THE SCOUR CRITICAL ELEVATION ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

TO VERIFY BEARING STRATA, STANDARD PENETRATION TESTING (SPT) IS REQUIRED FOR DRILLED PIERS AT BENT NO. 2. PERFORM SPTs AT ELEVATION 725.5 FEET TO VERIFY TOP OF PARTIALLY WEATHERED ROCK AND AGAIN AT THE FINAL TIP ELEVATION INDICATED. FOR SPT TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

DRILLED PIERS AT BENT NO. 3 ARE DESIGNED FOR A FACTORED RESISTANCE OF 784.0 TONS PER PIER.

INSTALL DRILLED PIERS AT BENT NO. 3 TO A TIP ELEVATION NO HIGHER THAN 712.0 FEET WITH THE REQUIRED TIP RESISTANCE AND A PENETRATION OF AT LEAST 12 FEET OF ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS.

THE SCOUR CRITICAL ELEVATION OF BENT NO. 3 IS ELEVATION 724.0 FEET. SCOUR CRITICAL ELEVATION ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

PERMANENT STEEL CASING IS REQUIRED FOR DRILLED PIERS AT BENT NO. 4. DO NOT EXTEND PERMANENT CASING BELOW ELEVATION 726.0 FEET WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

DRILLED PIERS AT BENT NO. 4 ARE DESIGNED FOR A FACTORED RESISTANCE OF 784.0 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 95.0 TSF.

INSTALL DRILLED PIERS AT BENT NO. 4 TO A TIP ELEVATION NO HIGHER THAN 712.0 FEET WITH THE REQUIRED TIP RESISTANCE AND A PENETRATION OF AT LEAST 12 FEET OF ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS.

THE SCOUR CRITICAL ELEVATION FOR BENT NO. 4 IS ELEVATION 724.0 FEET. SCOUR CRITICAL ELEVATION ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

B-5825 PROJECT NO. ____ YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-

Marshall G. Check, Jr 5FBCC2F3A4DC413... 4/14/2020

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

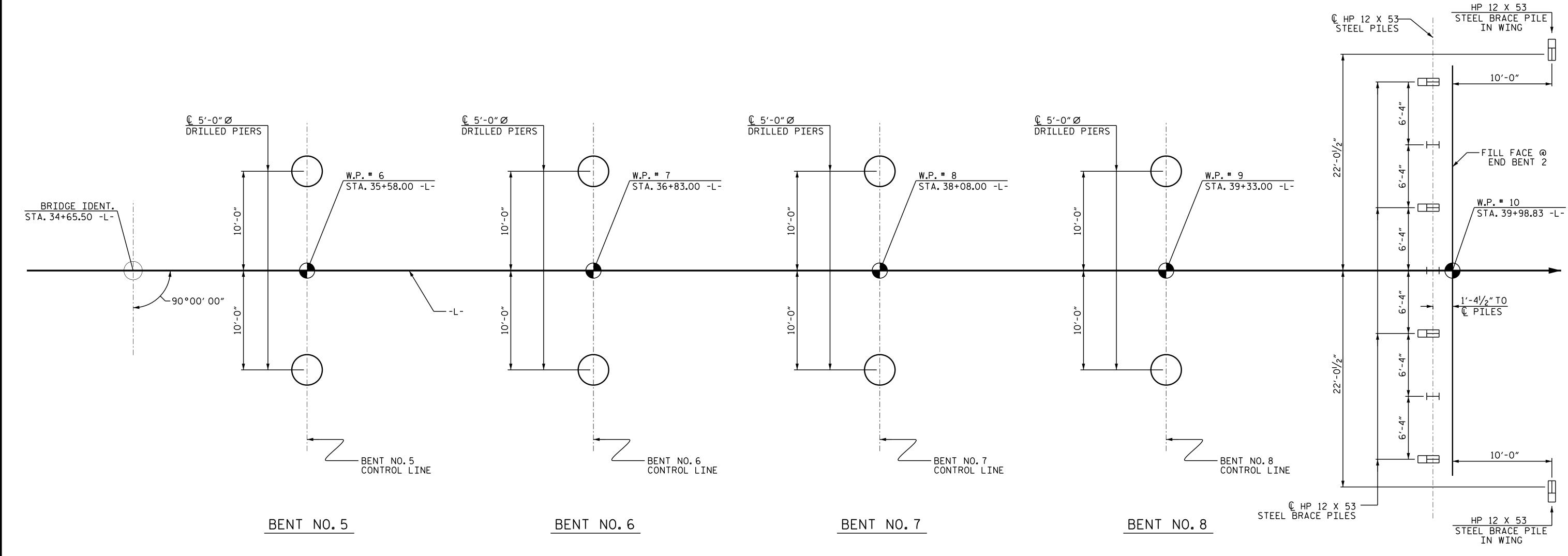
GENERAL DRAWING FOR BRIDGE OVER YADKIN RIVER ON NC 67

BETWEEN SR 1607 AND SR 1600 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SHEET 4 OF 7

ZCS _ DATE : <u>12/19</u> DRAWN BY : _ _ DATE : ___1/20 MGC CHECKED BY : . DESIGN ENGINEER OF RECORD: ______TBE DATE : 2/20

TGS ENGINEERS
706 HILLSBOROUGH STREET NO. SUITE 200 RALEIGH, NC 27603 PH (919) 773–8887 CORP. LICENSE NO.: C–0275



FOUNDATION LAYOUT

ALL END BENT PILES ARE HP 12 X 53. DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINE AT THE BOTTOM OF THE CAP. END BENT BRACE PILES ARE BATTERED 3:12.

NOTES

DRILLED PIERS AT BENT NO.5 ARE DESIGNED FOR A FACTORED RESISTANCE OF 776.0 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 95.0 TSF.

INSTALL DRILLED PIERS AT BENT NO.5 TO A TIP ELEVATION NO HIGHER THAN 712.0 FEET WITH THE REQUIRED TIP RESISTANCE AND A PENETRATION OF AT LEAST 12 FEET OF ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.5 IS ELEVATION 722.0 FEET. THE SCOUR CRITICAL ELEVATION ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

PERMANENT STEEL CASINGS ARE REQUIRED FOR DRILLED PIERS AT BENT NO.5.DO NOT EXTEND PERMANENT CASING BELOW ELEVATION 725.0 FEET WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

DRILLED PIERS AT BENT NO.6 ARE DESIGNED FOR A FACTORED RESISTANCE OF 776.0 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 95.0 TSF.

INSTALL DRILLED PIERS AT BENT NO.6 TO A TIP ELEVATION NO HIGHER THAN 712.0 FEET WITH THE REQUIRED TIP RESISTANCE AND A PENETRATION OF AT LEAST 12 FEET OF ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.6 IS ELEVATION 724.0 FEET. SCOUR CRITICAL ELEVATION ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

PERMANENT STEEL CASING IS REQUIRED FOR DRILLED PIERS AT BENT NO.6. DO NOT EXTEND CASING BELOW ELEVATION 726.0 FEET WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

DRILLED PIERS AT BENT NO. 7 ARE DESIGNED FOR A FACTORED RESISTANCE OF 784.0 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 95.0 TSF.

INSTALL DRILLED PIERS AT BENT NO. 7 TO A TIP ELEVATION NO HIGHER THAN 712.5 FEET WITH THE REQUIRED TIP RESISTANCE AND A PENETRATION OF AT LEAST 12 FEET OF ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS.

THE SCOUR CRITICAL ELEVATION FOR BENT NO. 7 IS ELEVATION 724.0 FEET. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

PERMANENT STEEL CASING IS REQUIRED FOR DRILLED PIERS AT BENT NO. 7. DO NOT EXTEND CASING BELOW ELEVATION 725.0 FEET WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

DRILLED PIERS AT BENT NO. 8 ARE DESIGNED FOR A FACTORED RESISTANCE OF 670.0 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 95.0 TSF.

INSTALL DRILLED PIERS AT BENT NO.8 TO A TIP ELEVATION NO HIGHER THAN 715.4 FEET WITH THE REQUIRED TIP RESISTANCE AND A PENETRATION OF AT LEAST 12 FEET OF ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.8 IS ELEVATION 726.0 FEET. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

SID INSPECTIONS MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SID INSPECTION. FOR SID INSPECTIONS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

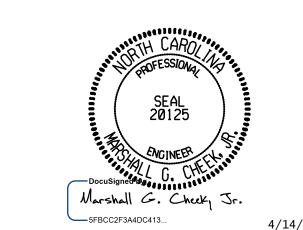
CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

OBSERVE A THREE MONTH WAITING PERIOD AFTER CONSTRUCTING THE EMBANKMENT TO WITHIN TWO FEET OF FINISHED GRADE BEFORE BEGINNING END BENT CONSTRUCTION AT END BENT NO. 2. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SECTION 235 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 90 TONS PER PILE. DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 150 TONS PER PILE.

B-5825 PROJECT NO. ___ YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-

END BENT NO. 2



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH GENERAL DRAWING

FOR BRIDGE OVER YADKIN RIVER ON NC 67

S-5

TOTAL SHEETS

60

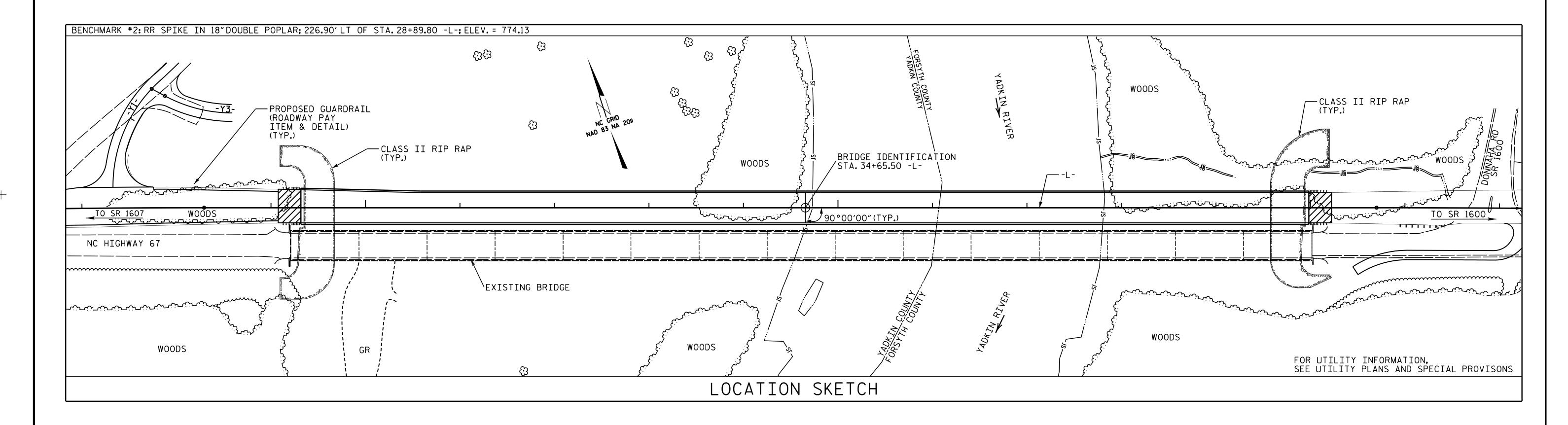
DATE:

BETWEEN SR 1607 AND SR 1600 4/14/2020 SHEET NO

SHEET 5 OF 7

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED **REVISIONS** TGS ENGINEERS
706 HILLSBOROUGH STREET NO. NO. BY: DATE: BY: SUITE 200 RALEIGH, NC 27603 PH (919) 773–8887 CORP. LICENSE NO.: C-0275

ZCS _ DATE : <u>12/19</u> DRAWN BY : _ MGC DATE : 1/20 CHECKED BY: DESIGN ENGINEER OF RECORD: ______TBE DATE : 2/20



NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1. FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE A TEMPORARY ACCESS FOR USE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE. FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS, SEE SPECIAL PROVISIONS.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP USED IN THE CAUSEWAY MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE."

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

THE EXISTING STRUCTURE CONSISTING OF 15 SPANS (1 @ 74'-0", 13 @ 72'-0", 1 @ 74'-0") WITH A REINFORCED CONCRETE DECK ON STEEL BEAMS WITH A CLEAR ROADWAY WIDTH OF 26'-0" AND A SUBSTRUCTURE CONSISTING OF REINFORCED CONCRETE CAPS/STEEL PILES ABUTMENTS AND REINFORCED CONCRETE POST-AND-BEAM BENTS AND LOCATED DOWNSTREAM FROM THE PROPOSED BRIDGE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT. FOR REMOVAL OF EXISTING STRUCTURE, SEE SPECIAL PROVISIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES." FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS. FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS, FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE

ROADWAY PLANS.

ALL GRADING REQUIRED FOR ACCESS TO THE TEMPORARY ACCESS SHALL BE INCLUDED IN THE LUMP SUM BID PRICE FOR "CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS."

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LOCATION AND PROFILE OF THE TEMPORARY ACCESS SHOWN ON THE PLANS IS FOR ESTIMATING PURPOSES ONLY. THE ACTUAL LOCATION AND PROFILE SHALL BE DETERMINED IN THE FIELD WITH THE APPROVAL OF THE ENGINEER.

FOR CLASS II RIP RAP AND CLASS B RIP RAP, SEE SPECIAL PROVISION, "CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS."

B-5825 PROJECT NO. ___ YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-

4/14/2020

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> GENERAL DRAWING FOR BRIDGE OVER YADKIN RIVER ON

NC 67 BETWEEN SR 1607 AND SR 1600

IGS 70

SOFESSION

SEAL 20125

Marshall G. Check, Jr.

8 ENGINEER

5FBCC2F3A4DC413...

UMENT NOT CONSIDERED FINAL							
ESS ALL SIGNATURES COMPLETED			REVI	SION	NS		SHEET NO.
TGS ENGINEERS				_			
06 HILLSBOROUGH STREET	NO.	BY:	DATE:	NO.	BY:	DATE:	S-6
SUITE 200	র			ହ			TOTAL SHEETS
RALEIGH, NC 27603	U			୬			SHEETS
PH (919) 773–8887	ଉ						60
PH (919) 773–8887 CORP. LICENSE NO.: C-0275							UØ

SHEET 6 OF 7

_ DATE : <u>12/19</u> DRAWN BY : _ DATE : <u>12/19</u> CHECKED BY: __ DATE : <u>2/20</u> MGC DESIGN ENGINEER OF RECORD: ___

						Т	OTAL BILL	OF MATE	RIAL								
ITEM	CONSTRUCTION, MAINTENANCE, AND REMOVAL OF TEMPORARY ACCESS	REMOVAL OF EXISTING STRUCTURE	ASBESTOS ASSESSMENT	5'-0"Ø DRILLED PIERS IN SOIL	5'-0"Ø DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 5'-0"Ø DRILLED PIERS	SID INSPECTIONS	SPT TESTING	CSL TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS "A" CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	72" P	ODIFIED RESTRESSED ONCRETE GIRDERS
	LUMP SUM	LUMP SUM	LUMP SUM	LIN.FT.	LIN.FT.	LIN.FT.	EACH	EACH	EACH	SQ.FT.	SQ.FT.	C.Y.	LUMP SUM	LBS.	LBS.	NO.	LIN.FT.
SUPERSTRUCTURE	LUMP SUM									37,716	32,437					36	4,246.04
END BENT 1												45.7		5,919			
BENT 1				67.16	32.00							57.1		18,867	4,536		
BENT 2				32.50	40.00			4				58.3		16,140	3,742		
BENT 3				30.66	27.00							60.1		15,134	3,361		
BENT 4				31.50	28.00	31.40						59.8		15,247	3,398		
BENT 5				18.50	26.00	18.44						69.2		15,332	3,466		
BENT 6				15.50	29.00	16.48						69.2		15,332	3,466		
BENT 7				33.00	24.00	32.00						60.6		15,162	3,369		
BENT 8				25.50	25.00							60.3		14,567	3,135		
END BENT 2												43.0		5,447			
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	254.32	231.00	98.32	2	4	1	37,716	32,437	583.3	LUMP SUM	137,147	28,473	36	4,246.04

			TOTAL E	BILL OF	MATERIAL	<u>-</u>		
ITEM	PILE DRIVING EQUIPMENT SETUP FOR HP 12×53 STEEL PILES		12×53 EL PILES	CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THK.)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	FOAM JOINT SEALS
	EACH	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE				2,129.18			LUMP SUM	LUMP SUM
END BENT 1	12	12	690		810	900		
BENT 1								
BENT 2								
BENT 3								
BENT 4								
BENT 5								
BENT 6								
BENT 7								
BENT 8								
END BENT 2	9	9	340		835	930		
TOTAL	21	21	1,030	2,129.18	1,645	1,830	LUMP SUM	LUMP SUM

PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-

SHEET 7 OF 7

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING
FOR BRIDGE OVER
YADKIN RIVER ON
NC 67
BETWEEN SR 1607 AND SR 1600

DOCUMENT NOT CONSIDERED FINAL			
UNLESS ALL SIGNATURES COMPLETED			
TGS ENGINEERS 706 HILLSBOROUGH STREET	NO.	BY:	
SUITE 200 RALEIGH, NC 27603	1		
PH (919) 773–8887 CORP. LICENSE NO.: C-0275	2		

SHEET NO. S-7 DATE: TOTAL SHEETS 60

DRAWN BY: ZCS DATE: 12/19
CHECKED BY: MGC DATE: 1/20
DESIGN ENGINEER OF RECORD: MGC DATE: 2/20

LEGAL LOAD RATING SNS7B

TNT4A

TNT6A

TNT7A

TNT7B

TNAGRIT4

TNAGT5A

TNAGRIT3

42.000

33.000

33.075

41.600

42.000

42.000

43.000

45.000

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS SERVICE III LIMIT STATE STRENGTH I LIMIT STATE SHEAR MOMENT MOMENT DISTRIBUTION FACTORS (DF) DIST/ LEFT SPAN DIST, LEFT SPAN IST $\langle 1 \rangle$ 61.50 0.978 1.34 1.07 61.50 HL-93 (INVENTORY) N/A 1.07 1.75 0.917 1.44 EL 11.73 0.80 0.917 EL DESIGN 1.35 0.917 1.87 61.50 0.978 1.78 HL-93 (OPERATING) 1.78 N/A EL 11.73 N/A LOAD RATING $\langle 2 \rangle$ 56.88 1.58 1.75 0.894 1.96 31.50 0.968 1.90 1.58 36.000 0.917 61.50 HS-20 (INVENTORY) 24.60 0.80 2.52 90.72 2.55 31.50 0.968 2.52 36.000 HS-20 (OPERATING) 24.60 13.500 5.37 31.50 0.968 6.29 61.50 0.894 SNSH 51.84 1.40 24.60 0.80 0.917 3.84 54.80 31.50 0.968 4.33 61.50 SNGARBS2 20.000 2.74 1.40 4.07 0.917 2.74 EL 24.60 0.80 3.89 0.968 2.55 56.10 0.894 31.50 3.98 0.917 2.55 61.50 22.000 1.40 SNAGRIS2 24.60 0.80 27.250 52.05 2.67 31.50 0.968 3.03 61.50 SNCOTTS3 0.917 24.60 1.91 34.925 54.13 31.50 0.968 2.43 1.55 61.50 1.55 0.894 2.26 SNAGGRS4 1.40 24.60 0.80 0.917 35.550 54.04 31.50 61.50 1.52 2.21 0.968 2.43 0.917 1.52 SNS5A 1.40 EL 24.60 0.80 54.73 2.17 39.950 1.37 1.40 0.894 2.04 31.50 0.968 0.917 61.50 SNS6A 24.60 0.80 1.37

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

61.50

61.50

61.50

61.50

61.50

61.50

61.50

61.50

0.917

0.917

0.917

0.917

0.917

0.917

0.917

0.917

0.80

0.80

0.80

0.80

0.80

0.80

1.31

1.67

1.67

1.35

1.35

1.37

1.32

1.25

24.60

24.60

24.60

24.60

24.60

24.60

24.60

24.60

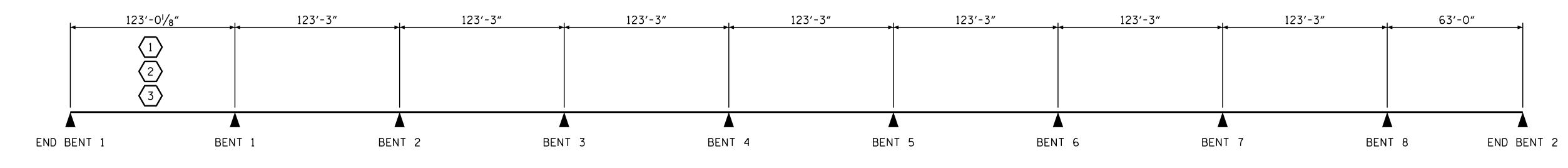
MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

- (#) CONTROLLING LOAD RATING
- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING **
- ** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER



31.50

31.50

31.50

31.50

31.50

31.50

31.50

31.50

EL

EL

1.94

2.49

2.50

2.06

2.08

2.16

2.05

1.92

0.894

0.894

0.894

1.40

1.40

1.40

1.40

1.40

1.40

1.40

0.968

0.968

0.968

0.968

0.968

0.968

0.968

0.968

31.50 0.968 1.91

2.11

2.65

2.60

2.21

2.16

2.07

2.02

1.97

PROJECT NO. B-5825
YADKIN/FORSYTH COUNTY
STATION: 34+65.50 -L-

<u>LRFR SUMMARY</u>

SPAN LENGTHS SHOWN ARE BEARING TO BEARING.



DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
LRFR SUMMARY FOR
PRESTRESSED
CONCRETE GIRDERS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
706 HILLSBOROUGH STREET SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

(NON-INTERSTATE TRAFFIC)

FINAL
PLETED

REVISIONS
STREET
NO. BY: DATE: NO. BY: DATE: S-8

TOTAL
SHEETS

ASSEMBLED BY: TBE DATE: 01/20 CHECKED BY: MGC DATE: 01/20

DRAWN BY: MAA I/08 REV. II/I2/08RR REV. IO/I/II MAA/GM REV. I2/I7 MAA/THC

55.02

55.11

55.24

56.16

56.70

57.54

56.76

1.67

1.67

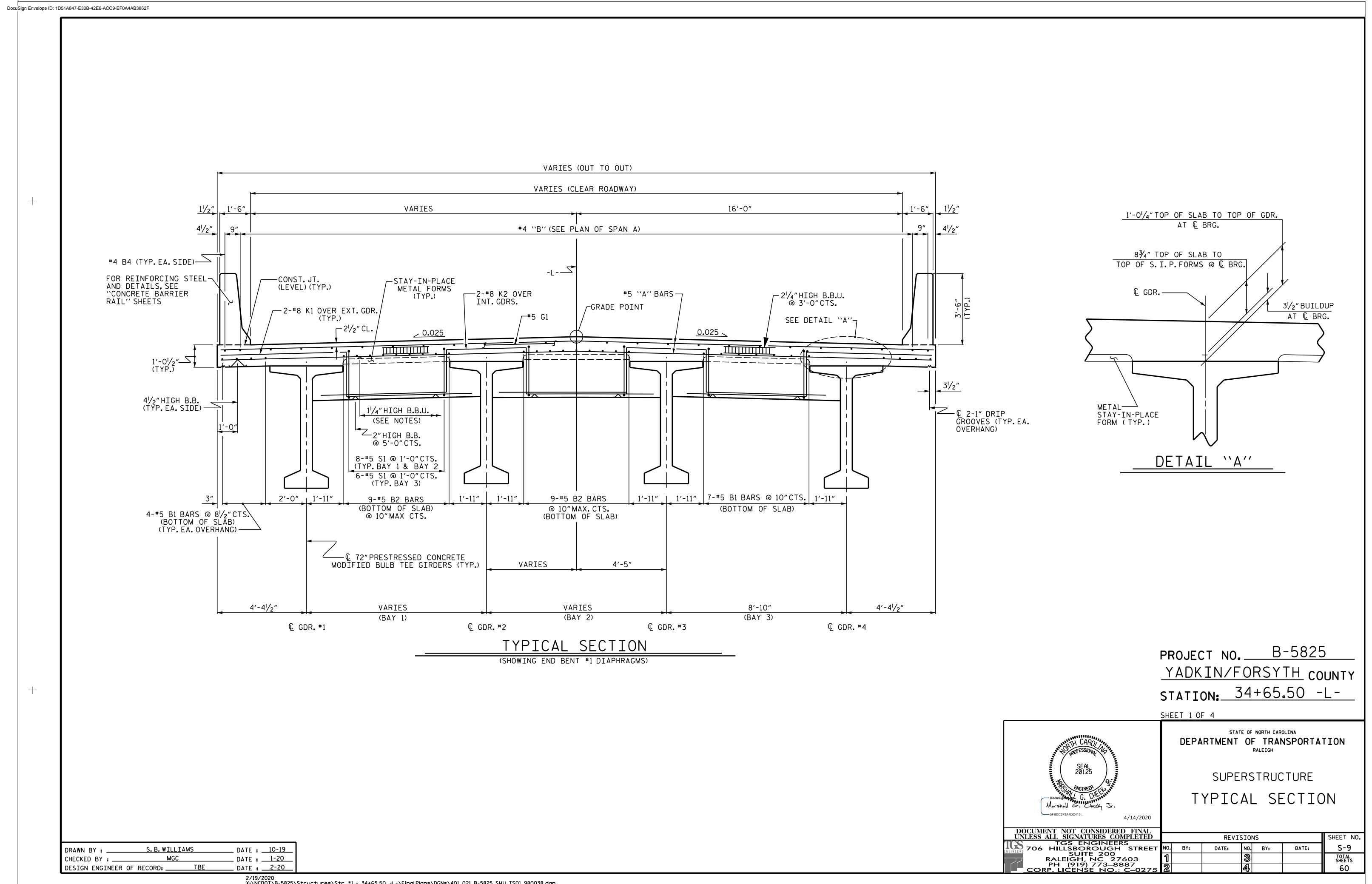
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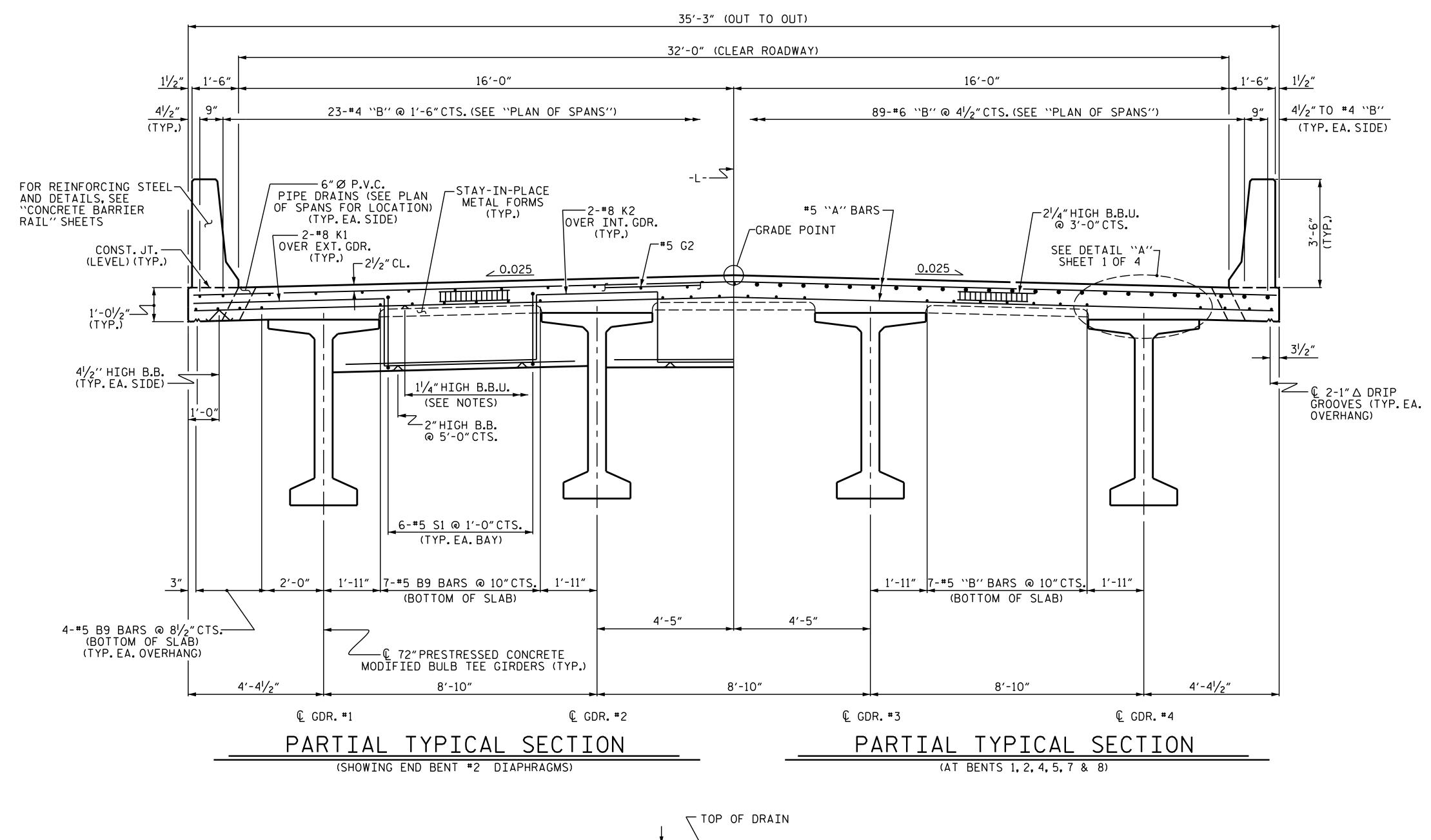
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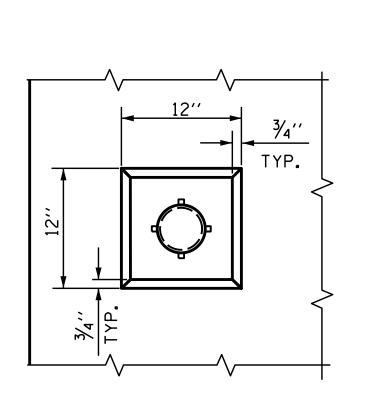
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1.32

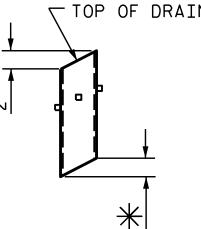
1.25







PLAN OF RECESS



* TO BE SET TO MATCH SLOPE OF BOTTOM OF OVERHANG (130 DRAINS REQUIRED)

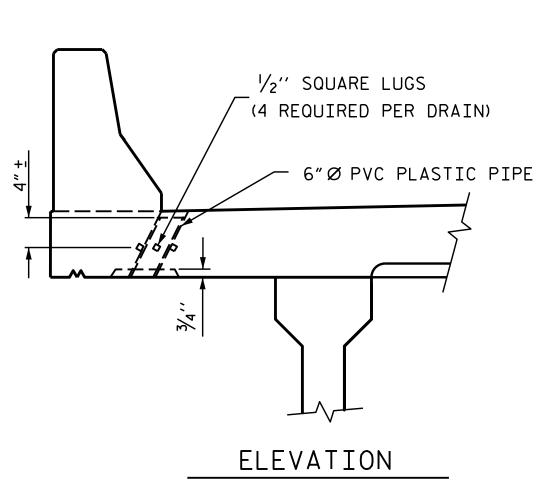
PIPE DETAIL

TOP OF FLOOR DRAINS TO BE SET 3/8" BELOW SURFACE OF SLAB. 4 - $\frac{1}{2}$ SQUARE LUGS TO BE GLUED TO THE P.V.C. PLASTIC PIPE AT EQUAL SPACES AROUND THE PIPE DRAIN APPROXIMATELY 4" FROM THE TOP OF THE PIPE.

THE 6" Ø PVC PLASTIC PIPE AND FITTINGS SHALL BE SCHEDULE 40 AND CONFORM TO ASTM D1785.

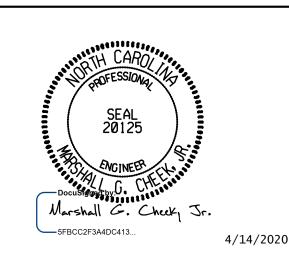
S.B.WILLIAMS _ DATE : <u>10-19</u> DRAWN BY : _ _ DATE : <u>1-20</u> MGC _ DATE : <u>2-20</u> DESIGN ENGINEER OF RECORD: TBE

DRAIN DETAILS



PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-

SHEET 2 OF 4



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE

TYPICAL SECTION

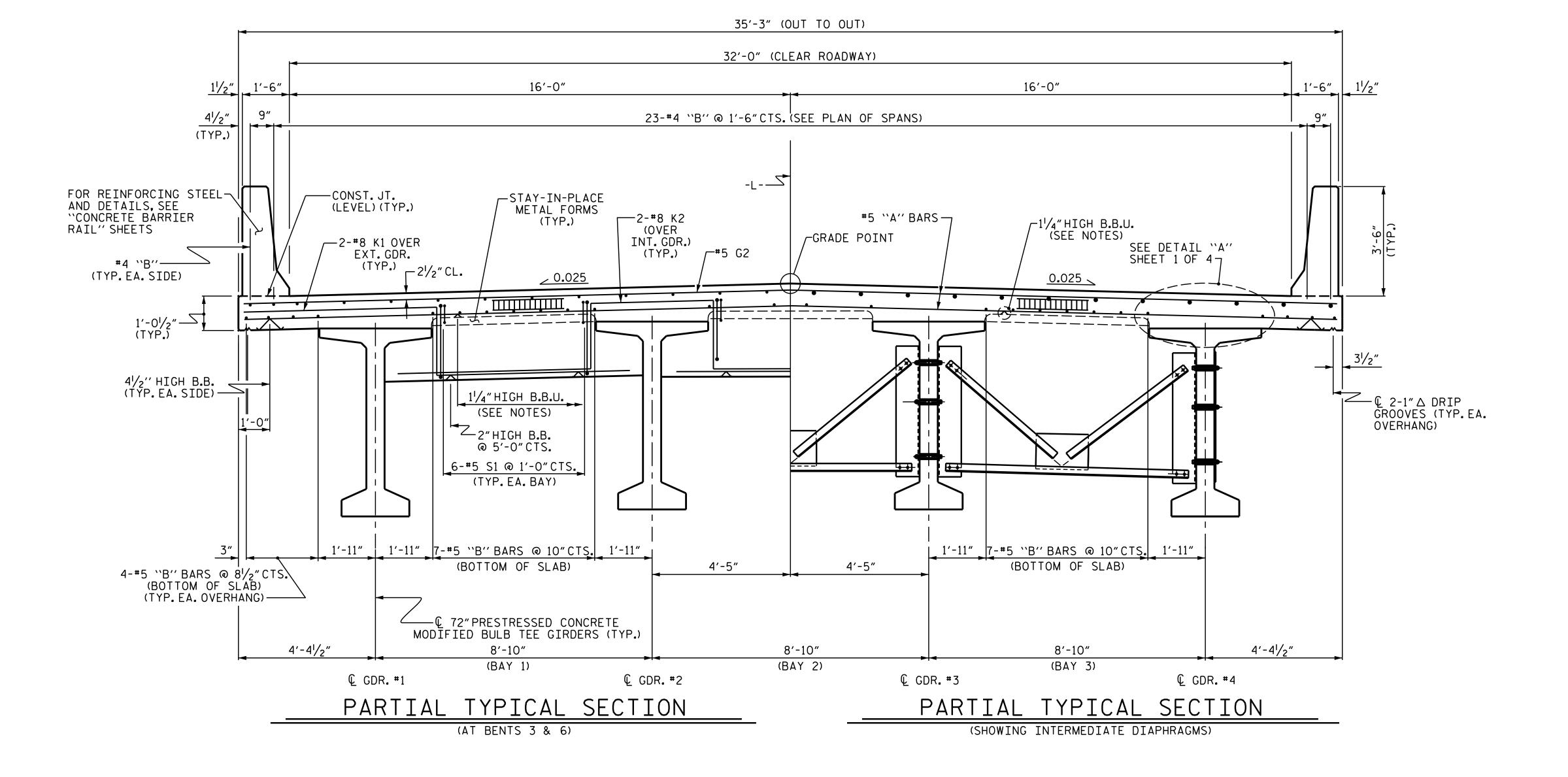
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS

706 HILLSBOROUGH STREET
SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275 SHEET NO **REVISIONS** NO. BY: S-10 DATE: DATE: TOTAL SHEETS

2/19/2020 X:\NCDOT\B-5825\Structures\Str. #1 - 34+65.50 -L-\FinalPlans\DGNs\401_023_B-5825_SMU_TS02_980038.dgn

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NOTES

PROVIDE 1 1/4"HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF 'A' BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0"CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF 'A' BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.

LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.

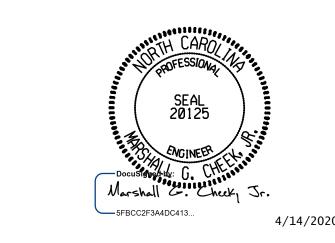
PREVIOUSLY CAST CONCRETE IN A CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.

BARRIER RAIL IN A CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE UNIT HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

PROJECT NO. B-5825

YADKIN/FORSYTH COUNTY

STATION: 34+65.50 -L-



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DEPARTMENT OF TRANSPORTATION

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TGS ENGINEERS
706 HILLSBOROUGH STREET SUITE 200 RALEIGH, NC 27603 PH (919) 773–8887 CORP. LICENSE NO.: C-0275

CORP. LICENSE NO.: C-0275

REVISIONS

REVISIONS

SHEET NO.

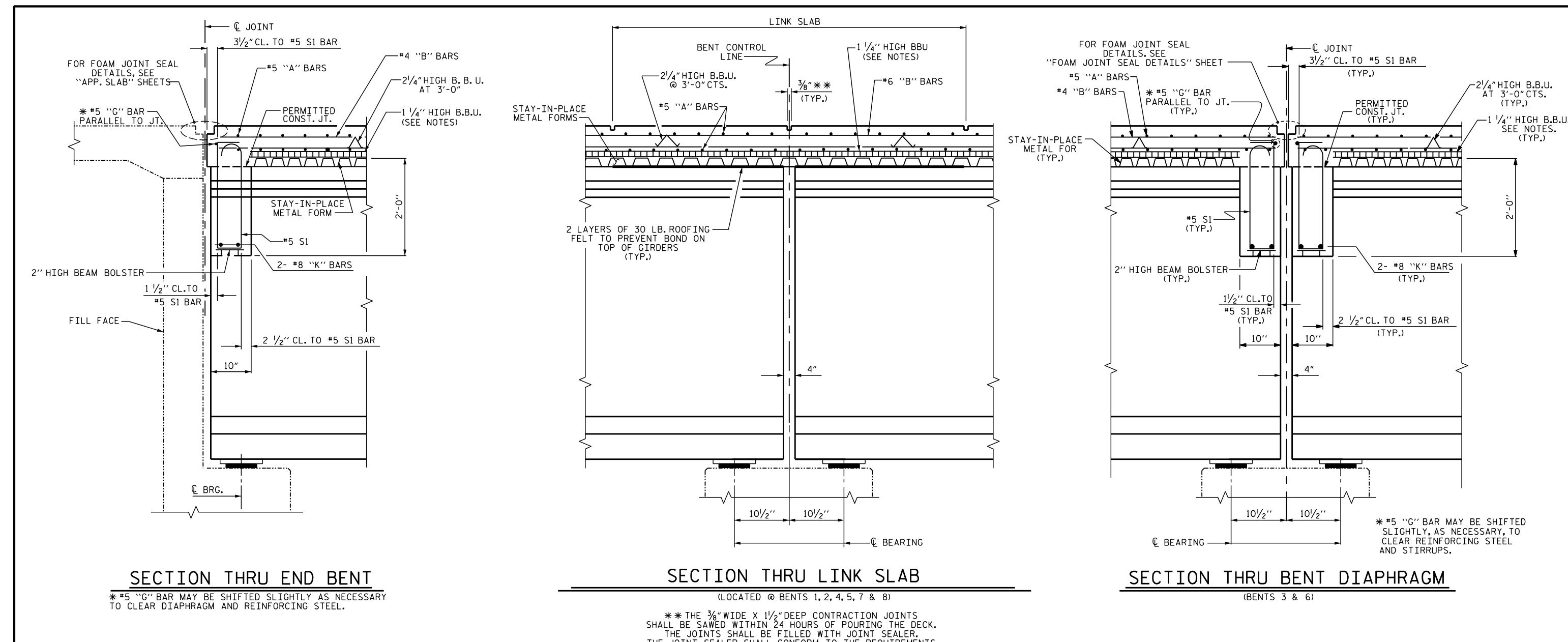
BY: DATE: NO. BY: DATE: S-11

TOTAL SHEETS
60

SHEET 3 OF 4

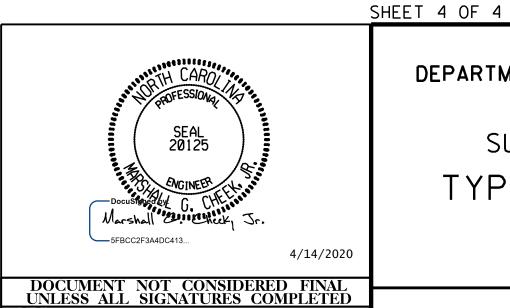
DRAWN BY: S.B. WILLIAMS DATE: 10-19
CHECKED BY: MGC DATE: 1-20
DESIGN ENGINEER OF RECORD: TBE DATE: 2-20

2/19/2020 X:\NCDOT\B-5825\Structures\Str. #1 - 34+65.50 -L-\FinalPlans\DGNs\401_025_B-5825_SMU_TS03_980038.dgn User:sbwilliams



THE JOINT SEALER SHALL CONFORM TO THE REQUIREMENTS OF TYPE B LOW MODULUS SILICONE SEALANT. SEE SECTION 1028
OF THE STANDARD SPECIFICATIONS.

> PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-



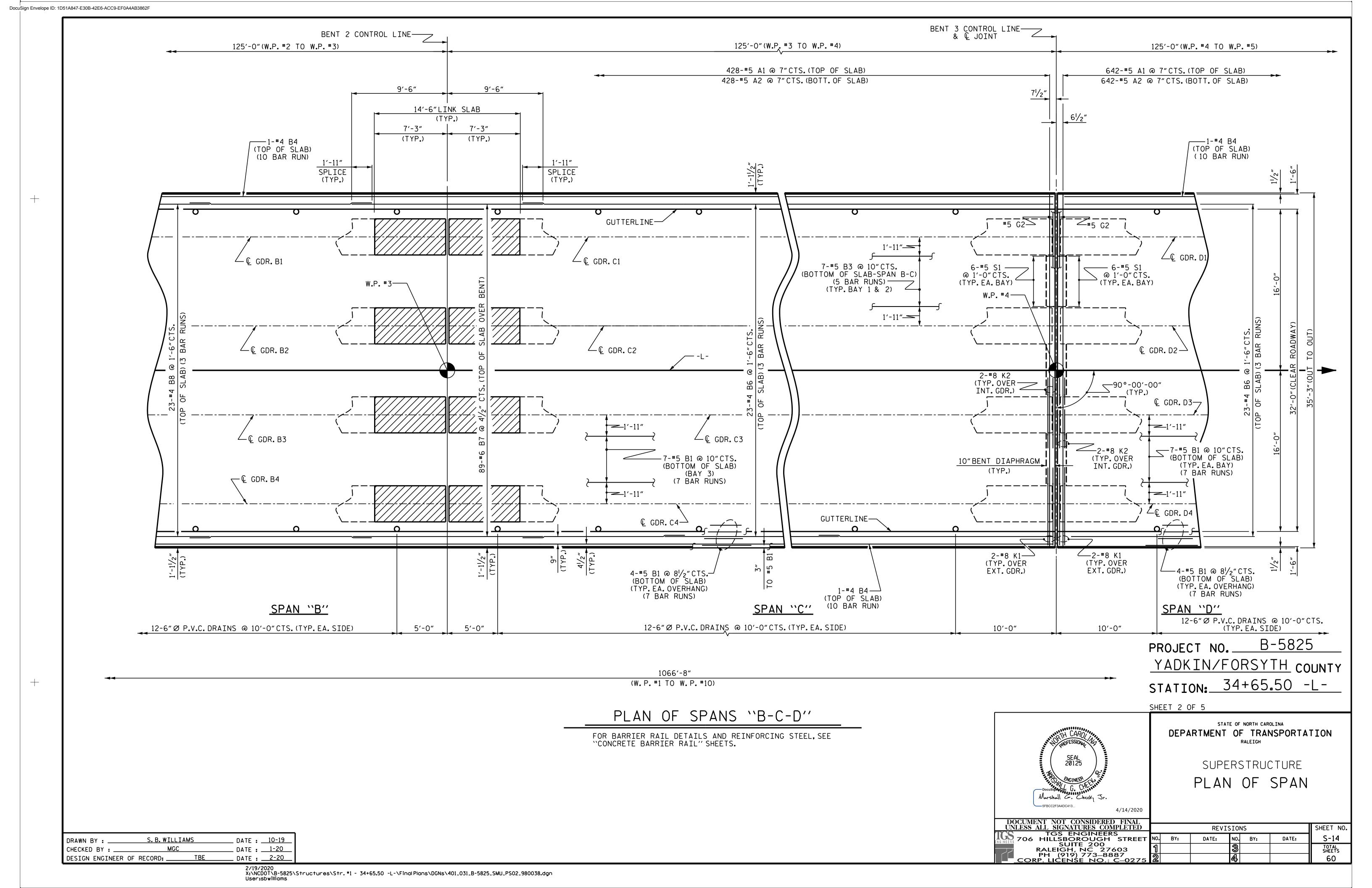
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

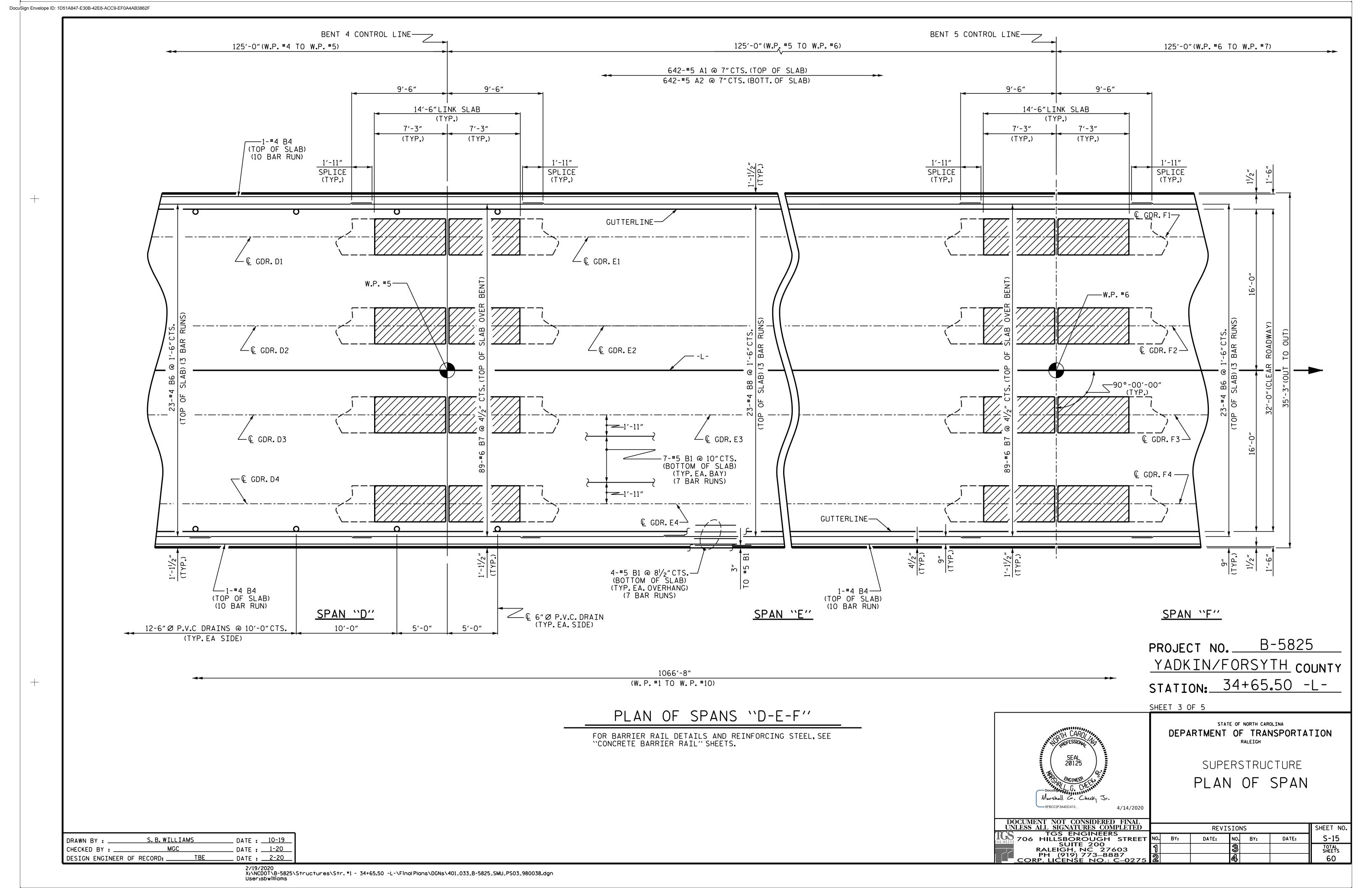
SUPERSTRUCTURE

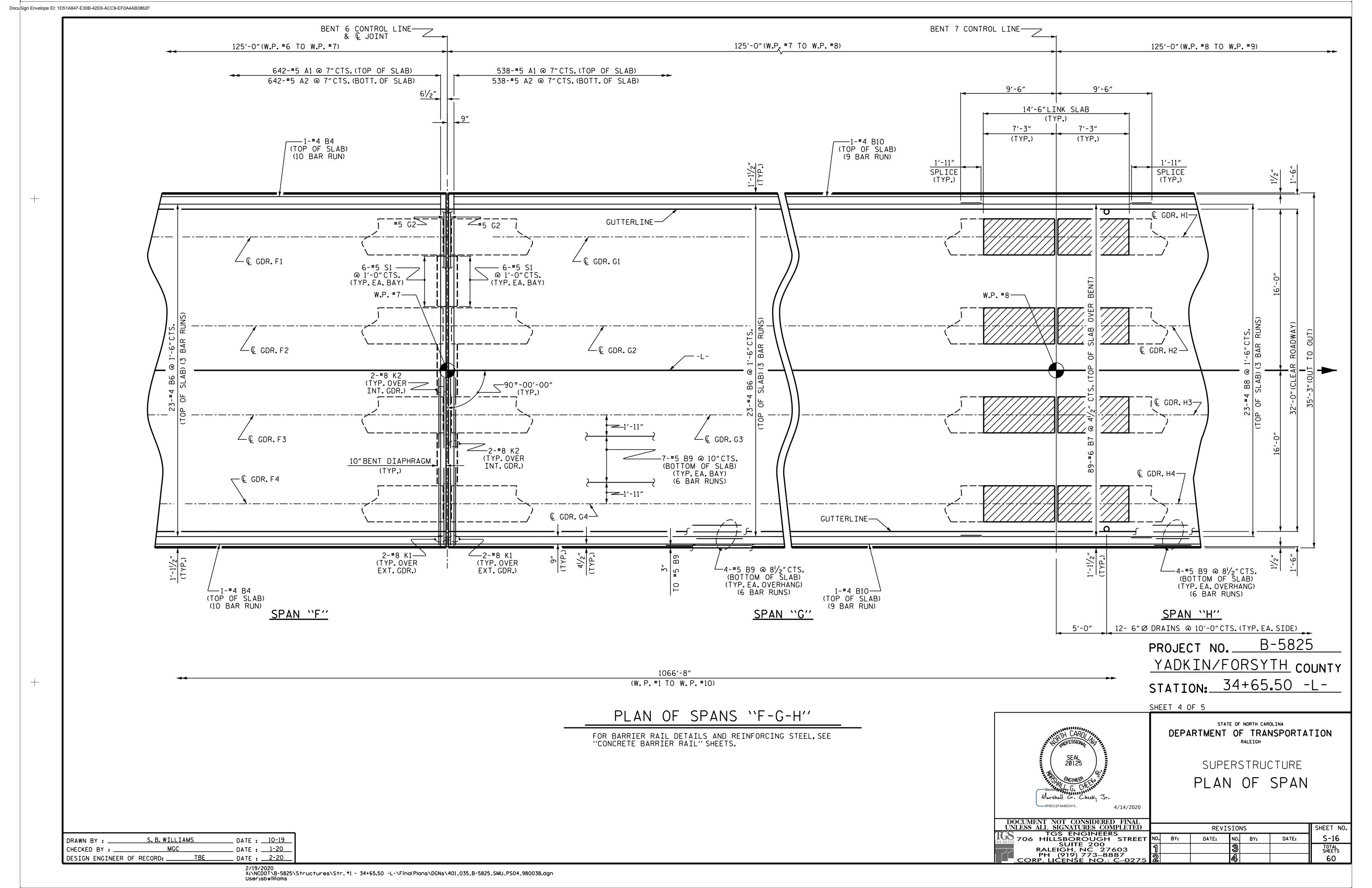
TYPICAL SECTION DETAILS

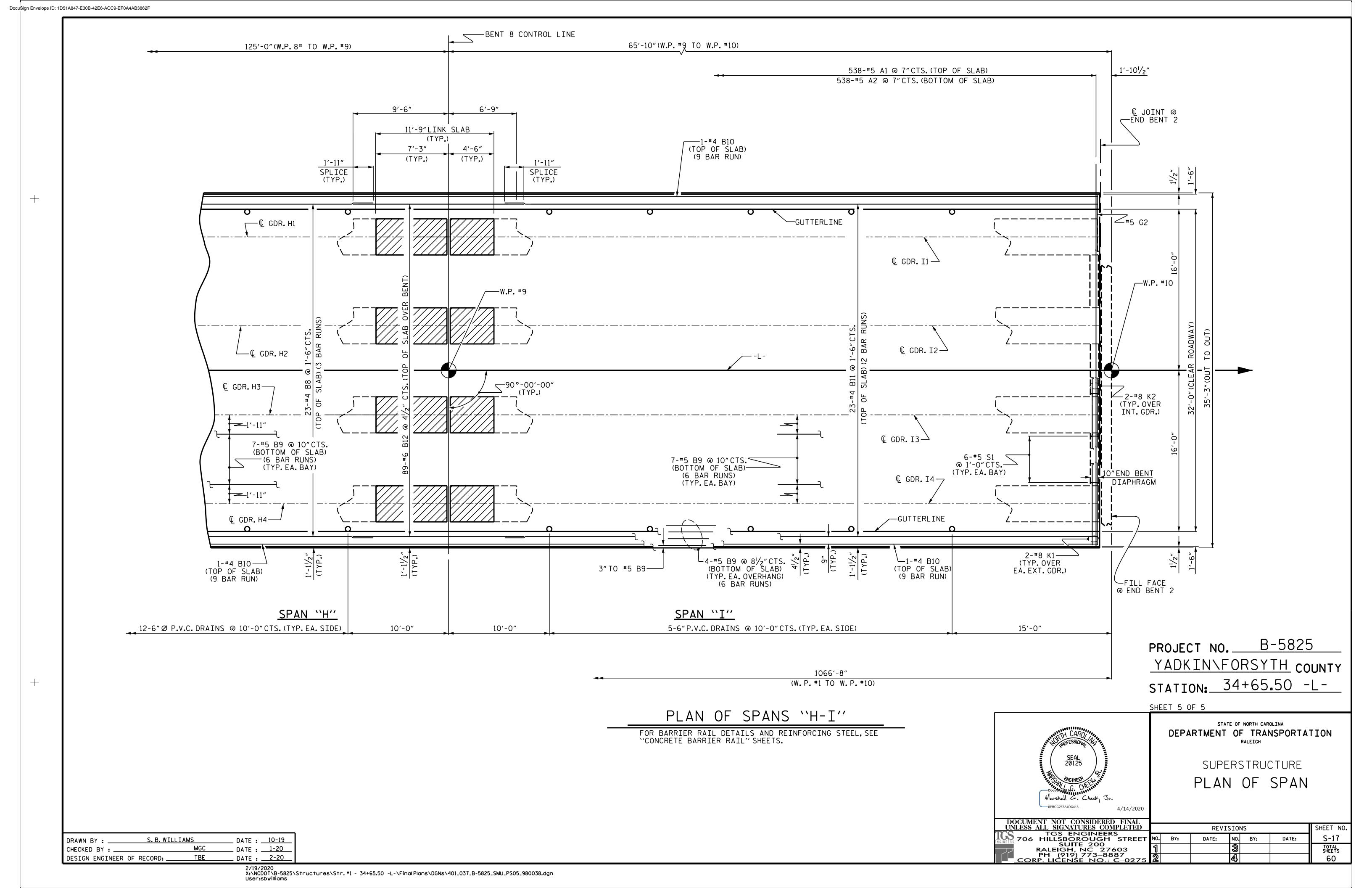
SHEET NO. REVISIONS TGS ENGINEERS
706 HILLSBOROUGH STREET
SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275 S-12 NO. BY: DATE: BY: DATE: TOTAL SHEETS 60

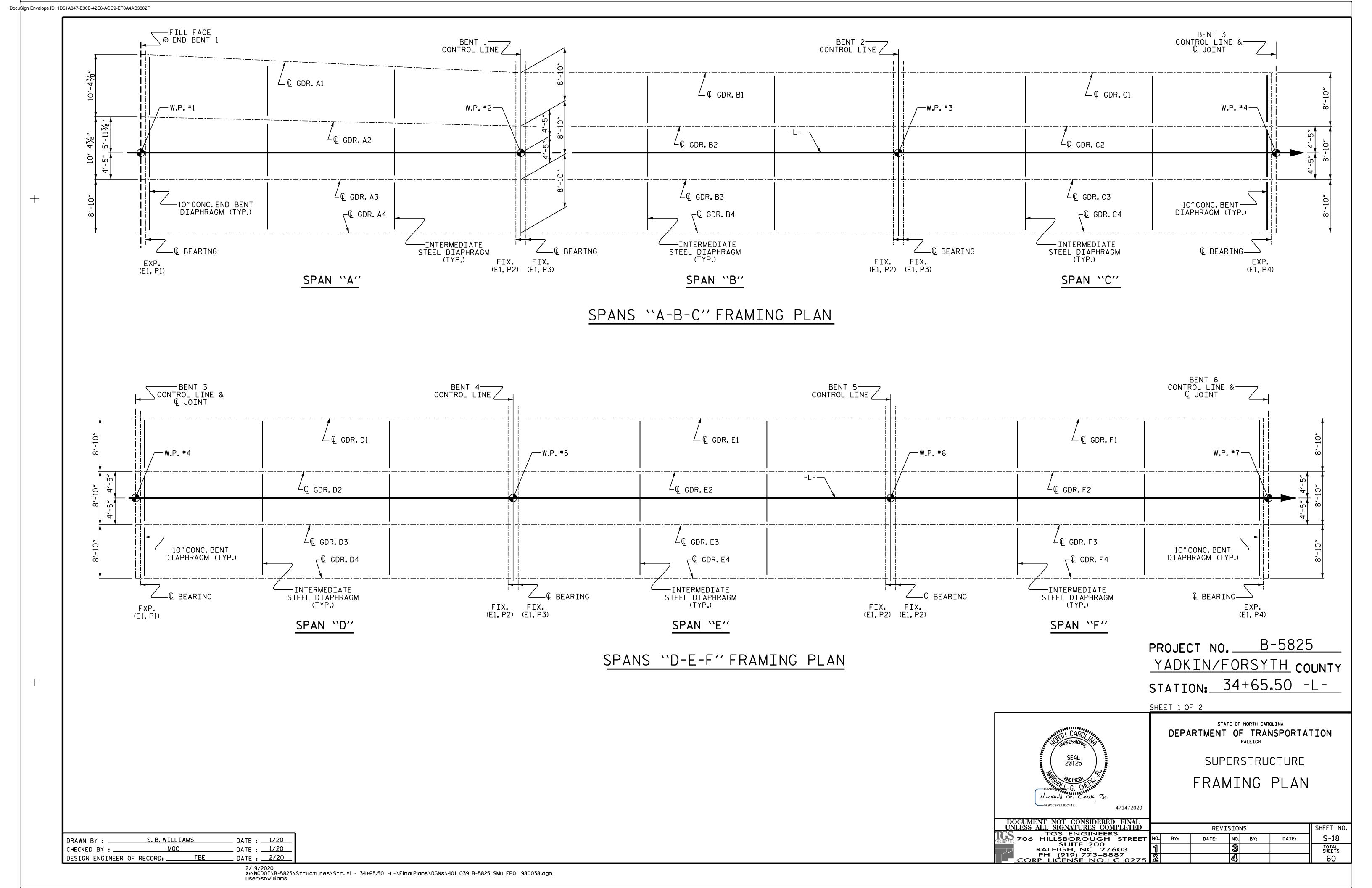
S.B.WILLIAMS DRAWN BY : __ _ DATE : <u>1-20</u> MGC DESIGN ENGINEER OF RECORD: TBE __ DATE : <u>2-20</u>

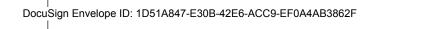


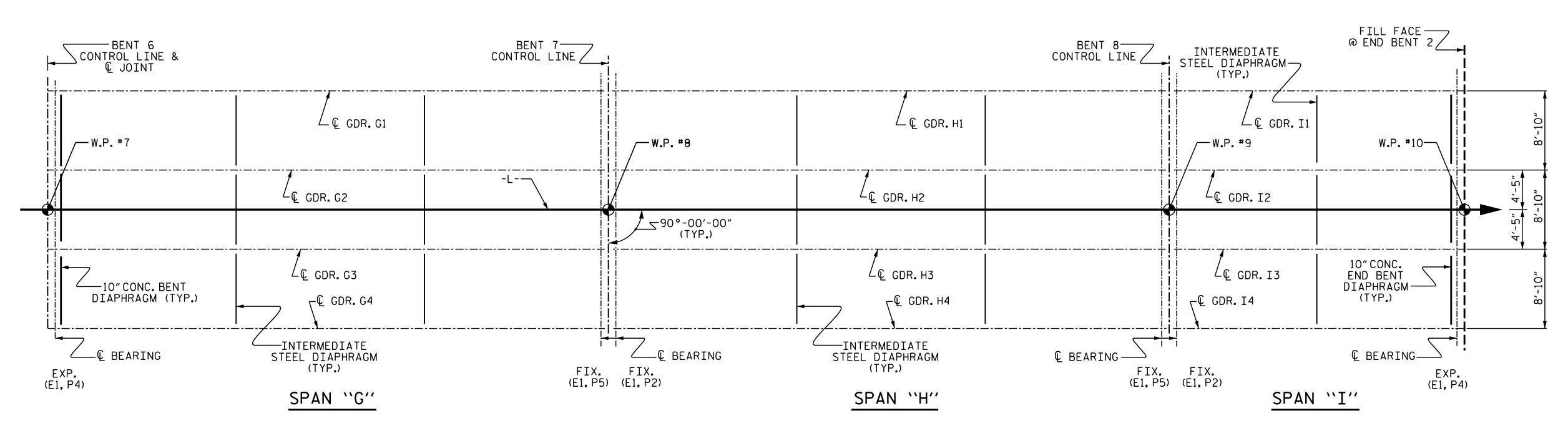








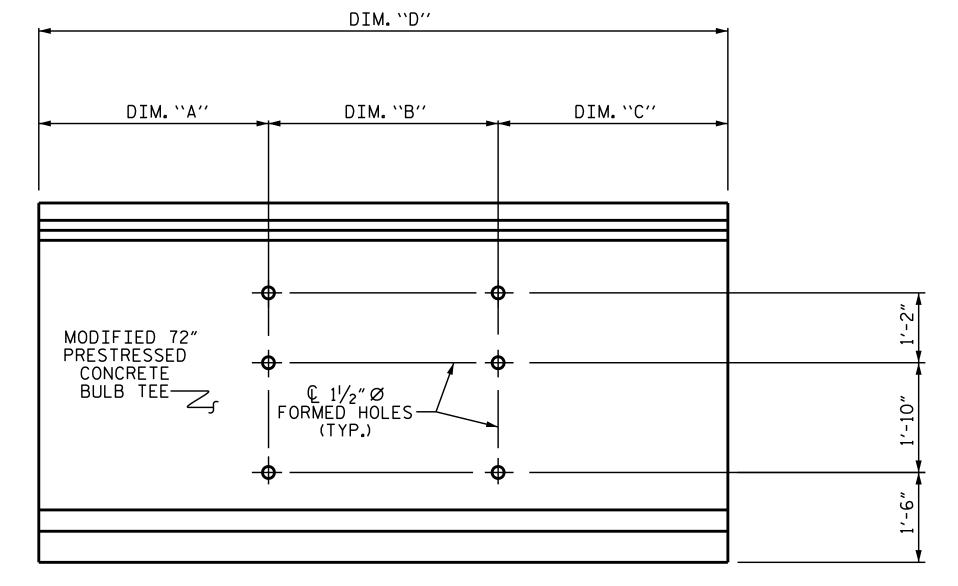




SPANS "G-H-I" FRAMING PLAN

GIRDER	``A''	``B''	``C''	``D′′
A1	41′-85⁄8″	41′-01/8″	41′-85⁄8″	124′-5 <mark>%</mark> ″
A2	41′-8%6″	41'-01/16"	41'-81/2"	124′-5 <mark>1</mark> /8″
А3	41'-81/2"	41'-0"	41'-81/2"	124′-5″
Δ4	41'-8 ¹ /2"	41'-0"	41′-8 ¹ /2″	124′-5″
B1 THRU B4	41'-9 ¹ / ₂ "	41'-1"	41'-9 ¹ / ₂ "	124′-8″
C1 THRU C4	41'-91/2"	41'-1"	41'-91/2"	124'-8"
D1 THRU D4	41'-91/2"	41'-1"	41'-91/2"	124'-8"
E1 THRU E4	41'-91/2"	41'-1"	41'-91/2"	124'-8"
F1 THRU F4	41'-91/2"	41'-1"	41'-91/2"	124'-8"
G1 THRU G4	41'-9 ¹ / ₂ "	41'-1"	41'-91/2"	124′-8″
H1 THRU H4	41′-91/2″	41′-1″	41′-91/2″	124′-8″
I1 THRU I4	32'-2 ¹ / ₂ "	-	32'-21/2"	64′-5″

11/2" Ø HOLE LOCATION



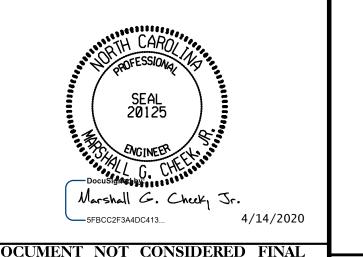
ELEVATION- 11/2" Ø HOLE LOCATION

PROJECT NO. B-5825

YADKIN/FORSYTH COUNTY

STATION: 34+65.50 -L-

SHEET 2 OF 2



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUPERSTRUCTURE

FRAMING PLAN

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RALEIGH, NC 27603
PH (919) 773–8887
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SHEET NO.
BY: DATE: NO. BY: DATE: S-19

TOTAL SHEETS
60

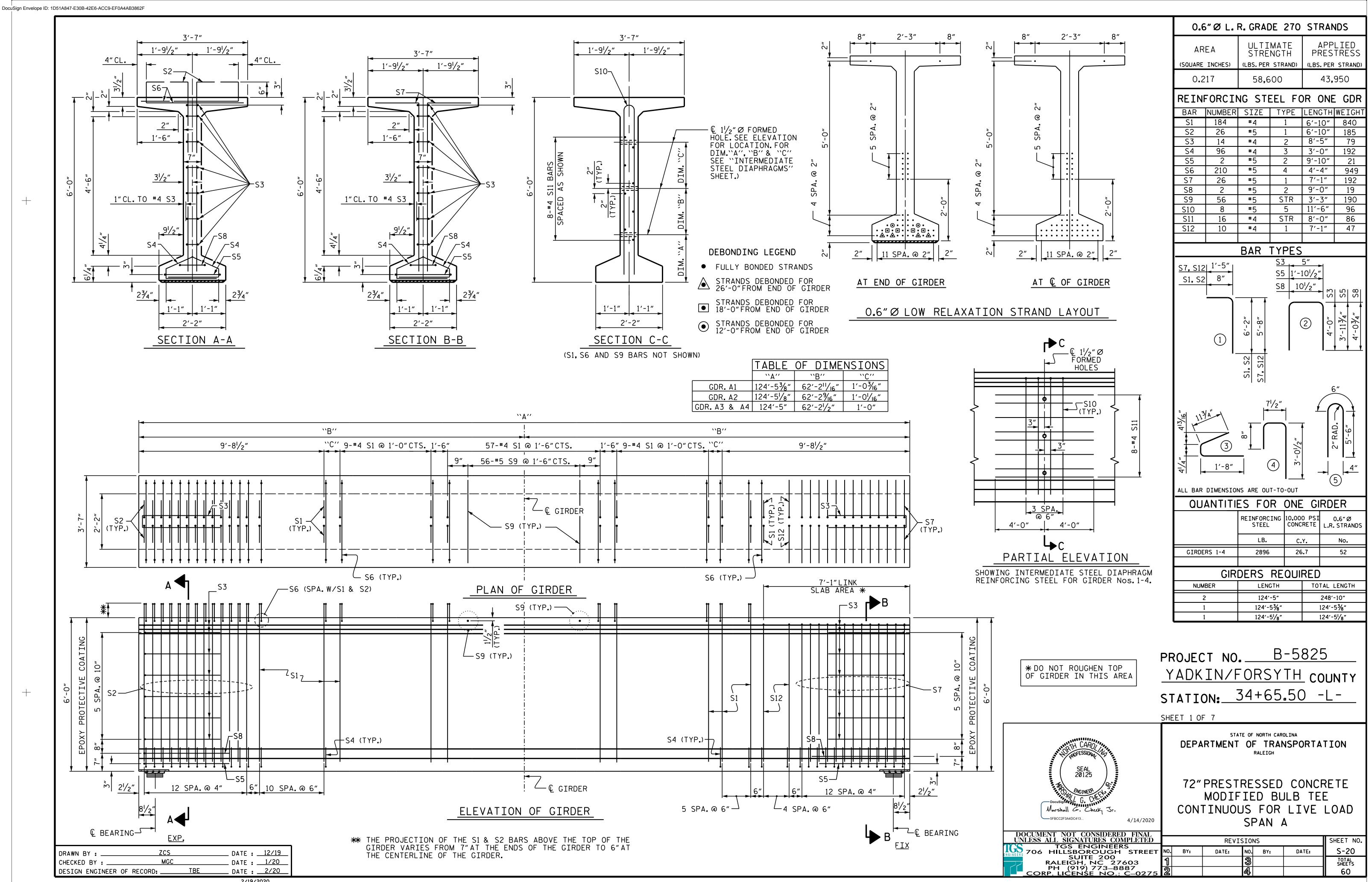
DRAWN BY: S.B. WILLIAMS

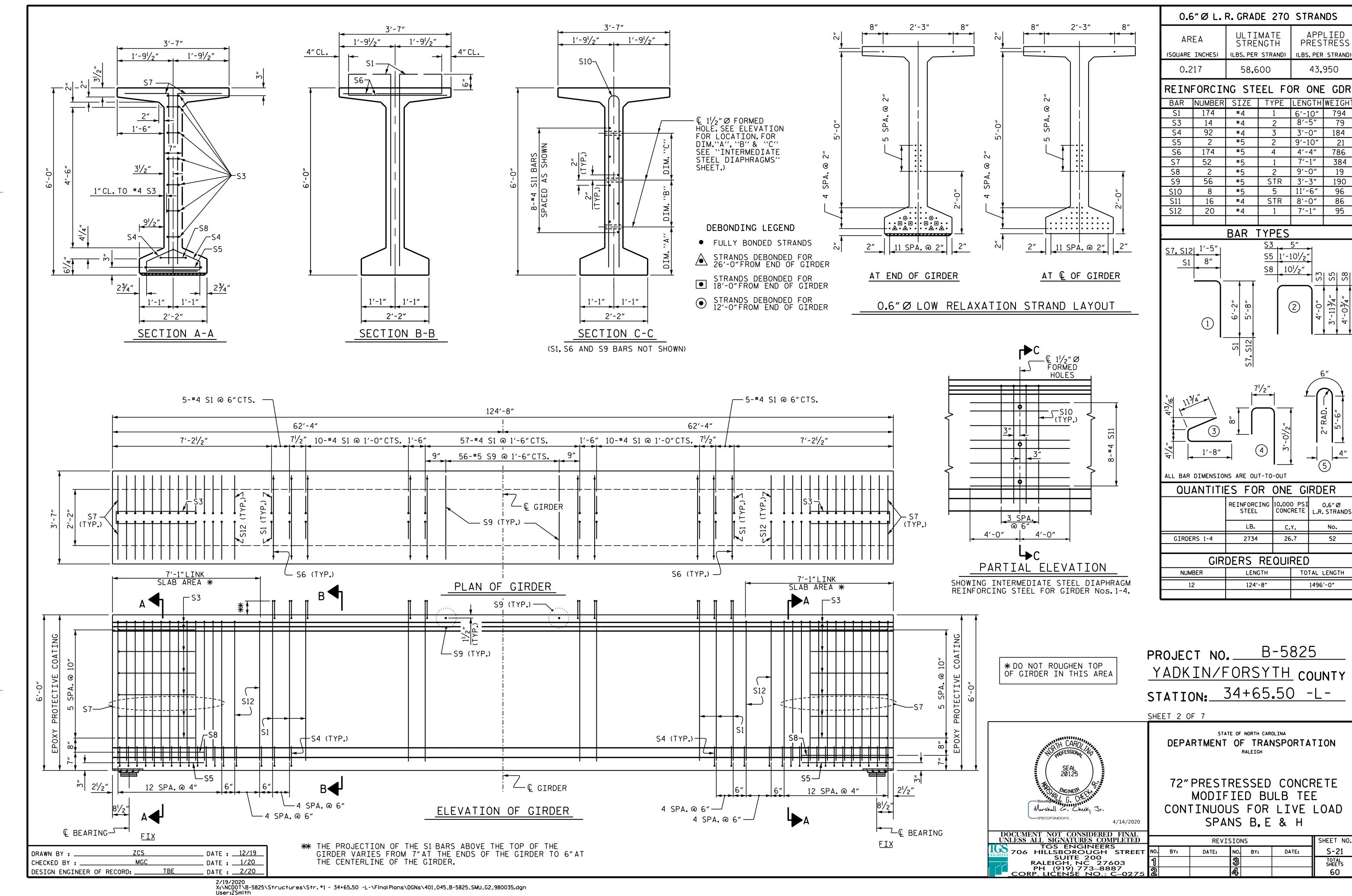
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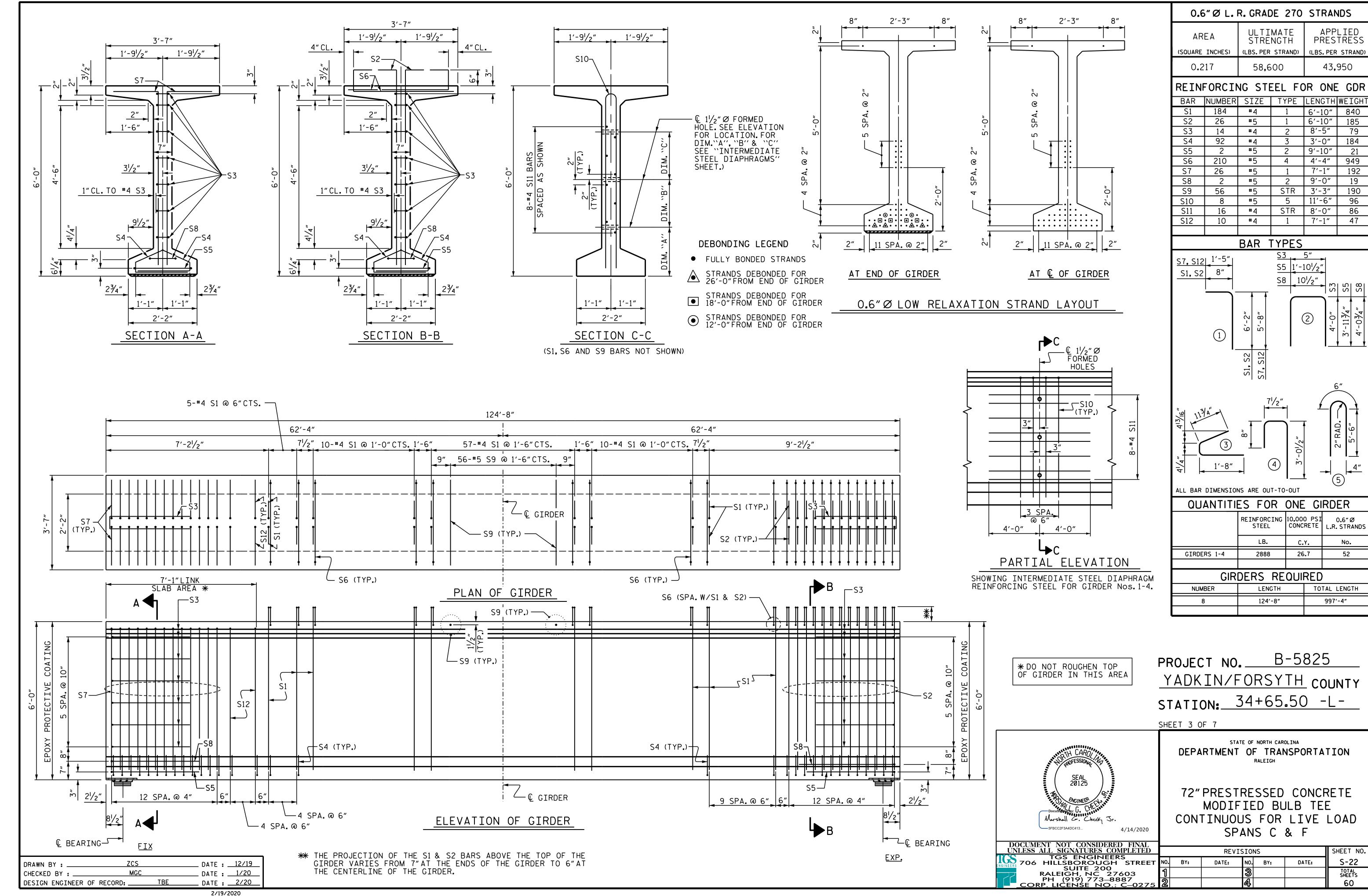
DATE: 1-20

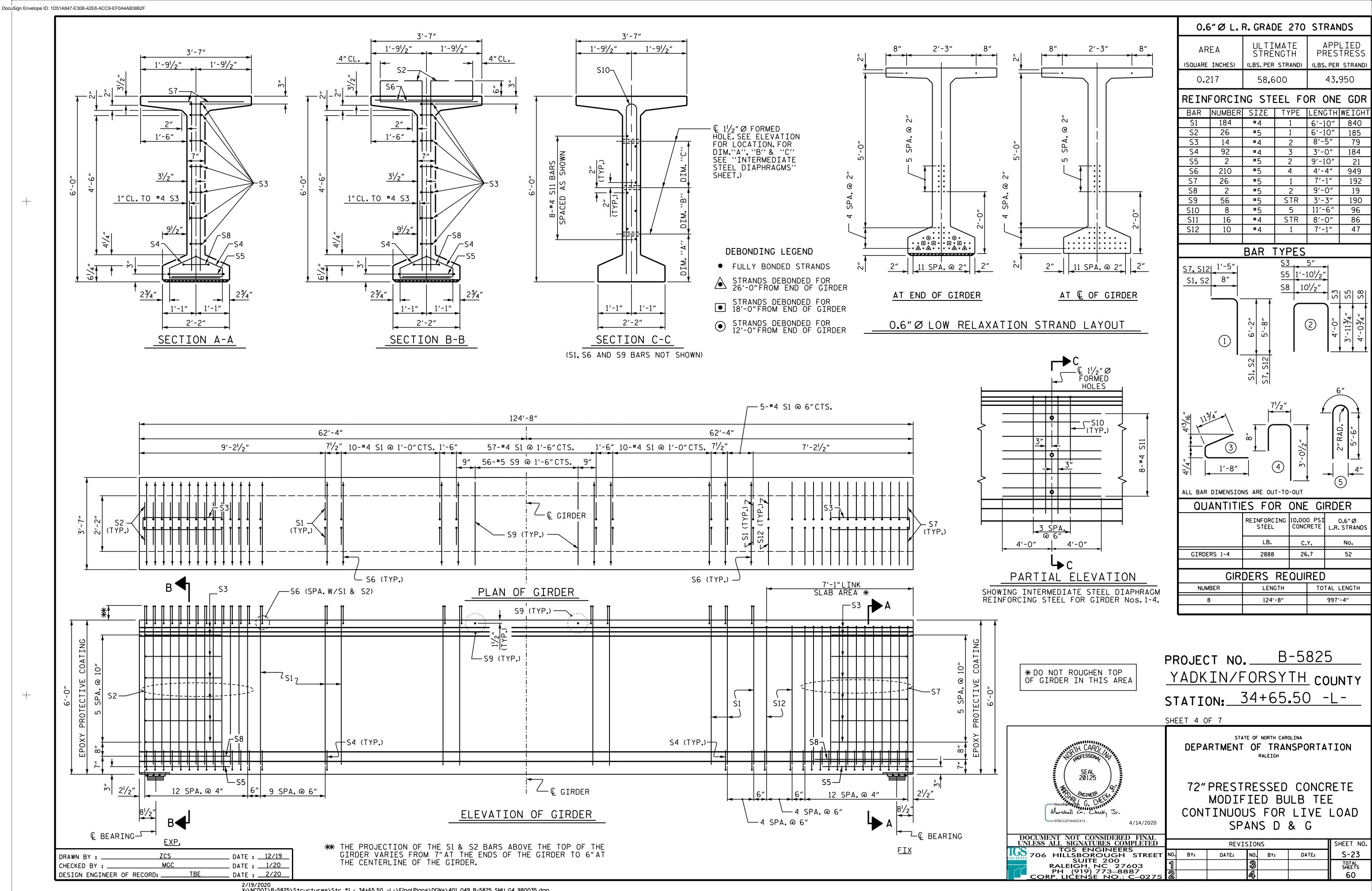
DESIGN ENGINEER OF RECORD: TBE

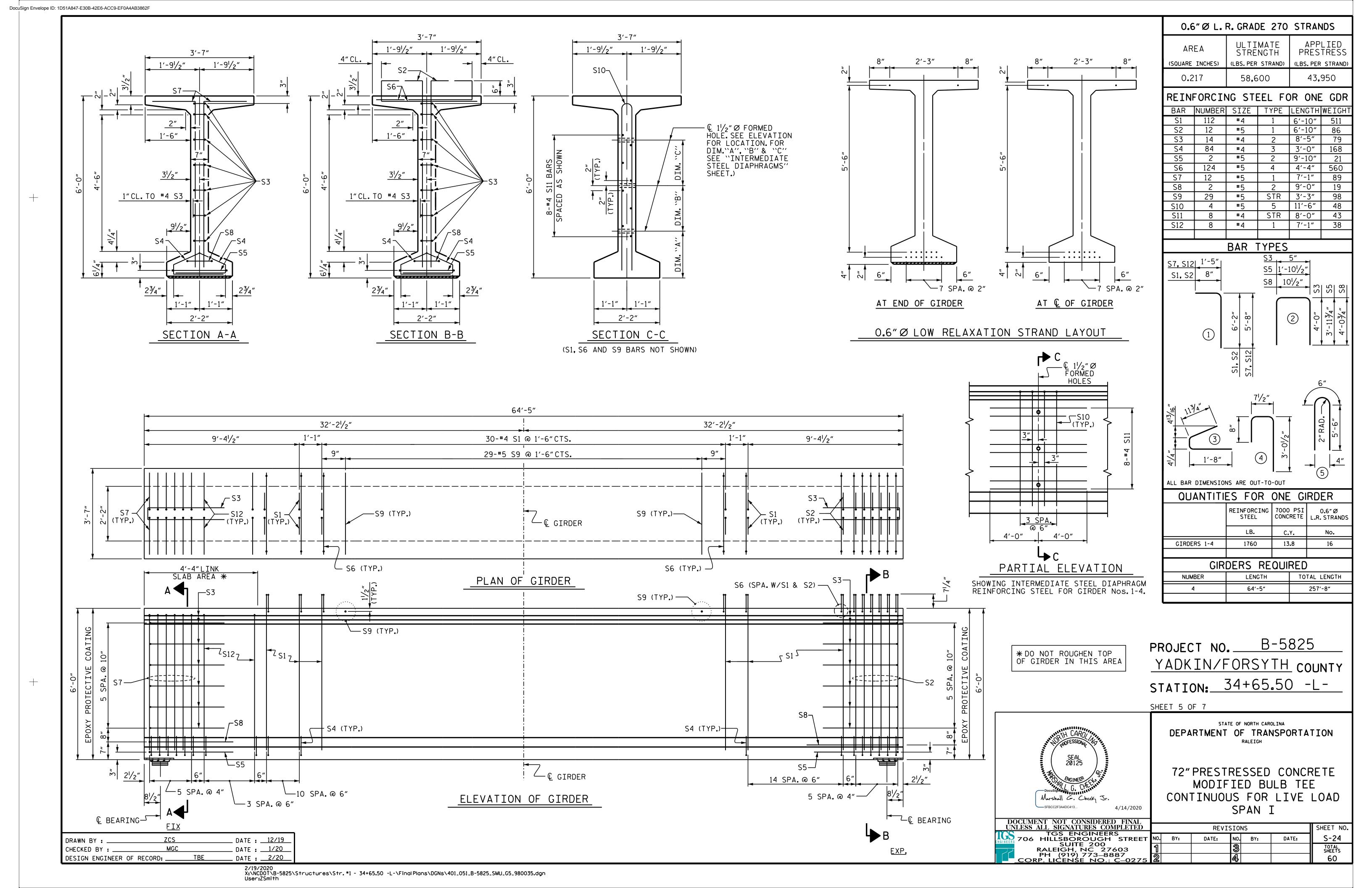
DATE: 2-20











			<u> </u>	DEAD	LOA	D DE	FLEC	CTIO	Ν ΤΔ	BLE	FOR	GIR	DERS					_				
0.6"Ø LOW RELAXATION STRANDS									S	SPAN A	. G]	[RDER	1									
		© BRG05 .10 .15 .20 .25 .30 .35 .40 .45 .50 .55 .60 .65 .70 .75 .80 .85 .90 .95															€ BRG					
CAMBER (GIRDER ALONE IN PLACE)	<u></u>	We BRG. .05 .10 .15 .20 .25 .30 .35 .40 .45 .50 .55 .60 .65 .70 .75 .80 .85 .90 .95 We 0.000 0.050 0.099 0.144 0.187 0.224 0.255 0.281 0.299 0.310 0.314 0.310 0.299 0.281 0.255 0.224 0.187 0.144 0.099 0.050 0															0.000					
* DEFLECTION DUE TO SUPERIMPOSED D.L.	\	0.000	0.033	0.067	0.098	0.130	0.155	0.180	0.196	0.211	0.217	0.222	0.217	0.211	0.196	0.180	0.155	0.130	0.098	0.067	0.033	0.000
FINAL CAMBER	†	0	3/16"	3/8"	9/16"	11/16"	13/16"	15/16"	1"	11/16"	11/8"	11/8"	11/8"	11/16"	1"	¹⁵ /16"	13/16"	11/16"	9/16"	3/8"	3/16"	0

			— [DEAD	LOA	D DE	FLEC	CTIO	N TA	BLE	FOR	GIR	DERS	<u> </u>								
0.6" Ø LOW RELAXATION STRANDS									S	SPAN A	\ G]	RDER	2									
	,	© BRG05 .10 .15 .20 .25 .30 .35 .40 .45 .50 .55 .60 .65 .70 .75 .80 .85 .90 .95															₡ BRG.					
CAMBER (GIRDER ALONE IN PLACE)	1	L BRG. .05 .10 .15 .20 .25 .30 .35 .40 .45 .50 .55 .60 .65 .70 .75 .80 .85 .90 .95 .90 0.000 0.050 0.099 0.144 0.187 0.224 0.255 0.281 0.299 0.310 0.314 0.310 0.299 0.281 0.255 0.224 0.144 0.099 0.050															0.000					
* DEFLECTION DUE TO SUPERIMPOSED D.L.	\	0.000	0.033	0.067	0.099	0.131	0.156	0.181	0.196	0.212	0.218	0.223	0.218	0.212	0.196	0.180	0.155	0.131	0.099	0.067	0.033	0.000
FINAL CAMBER	†	0	3/16"	3/8"	9/16"	11/16"	13/16"	7⁄8"	1"	11/16"	11/8"	11/8"	11/8"	11/16"	1"	7/8"	13/16"	11/16"	9/16"	3/8"	3/16"	0

			— [DEAD	LOA	D DE	FLEC	CTIO	N TA	BLE	FOR	GIR	DERS					_				
0.6"Ø LOW RELAXATION STRANDS									S	SPAN A	GI	RDER	3									
																	ℚ BRG.					
CAMBER (GIRDER ALONE IN PLACE)	1	0.000 0.050 0.099 0.144 0.187 0.224 0.255 0.281 0.299 0.310 0.314 0.310 0.299 0.281 0.255 0.224 0.187 0.144 0.099 0.050 0.															0.000					
* DEFLECTION DUE TO SUPERIMPOSED D.L.	\	0.000	0.032	0.065	0.096	0.127	0.151	0.175	0.191	0.206	0.211	0.215	0.211	0.206	0.191	0.175	0.151	0.127	0.096	0.065	0.032	0.000
FINAL CAMBER	†	0	³/ ₁₆ "	3/8"	9/16"	11/16"	7∕ ₈ ″	15/16"	11/16"	11/8"	13/16"	1¾6"	1¾6"	11/8"	1½ ₆ "	15/16"	7⁄8"	11/16"	%6″	3/8"	³ / ₁₆ "	0

			— [DEAD	LOA	D DE	FLE(CTIO	N TA	BLE	FOR	GIR	DERS									
0.6"Ø LOW RELAXATION STRANDS									S	SPAN A	GI	RDER	4									
		© BRG05 .10 .15 .20 .25 .30 .35 .40 .45 .50 .55 .60 .65 .70 .75 .80 .85 .90 .95 ©															ℚ BRG.					
CAMBER (GIRDER ALONE IN PLACE)	1	0.000	0.050	0.099	0.144	0.187	0.224	0.255	0.281	0.299	0.310	0.314	0.310	0.299	0.281	0.255	0.224	0.187	0.144	0.099	0.050	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L.	¥	0.000	0.032	0.065	0.096	0.126	0.151	0.175	0.190	0.205	0.211	0.216	0.211	0.205	0.190	0.175	0.151	0.126	0.096	0.065	0.032	0.000
FINAL CAMBER	1	0	3/16"	3/8"	9/16"	3/4"	7/8"	15/16"	11/16"	11/8"	13/16"	1 ³ / ₁₆ "	1 ³ / ₁₆ "	11/8"	11/16"	15/16"	7/8"	3/4"	9/16"	3/8"	3/16"	0

			<u> </u>	DEAD	LOA	D DE	FLE(CTIO	N TA	BLE	FOR	GIR	DERS	,				_				
0.6" Ø LOW RELAXATION STRANDS									SPA	NS B	- H (GIRDEF	RS 1 &	4								
																	€ BRG					
CAMBER (GIRDER ALONE IN PLACE)	†	0.000	0.050	0.099	0.145	0.187	0.224	0.256	0.281	0.300	0.311	0.315	0.311	0.300	0.281	0.256	0.224	0.187	0.145	0.099	0.050	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.035	0.068	0.100	0.129	0.154	0.176	0.194	0.206	0.214	0.217	0.214	. 206	0.194	0.176	0.154	0.129	0.100	0.068	0.035	0.000
FINAL CAMBER	†	0	3/16"	3/8"	9/16"	11/16"	13/16"	15/16"	11/16"	11/8"	13/16"	13/16"	13/16"	11/8"	11/16"	15/16"	13/16"	11/16"	9/16"	3/8"	3/16"	0

			<u> </u>	DEAD	LOA	D DE	FLEC	CTIO	ΝΤΔ	BLE	FOR	GIR	DERS									
0.6" Ø LOW RELAXATION STRANDS									SPA	NS B	- H	GIRDE	RS 2	& 3								
		© BRG. 05 .05 .10 .15 .20 .25 .30 .35 .40 .45 .50 .55 .60 .65 .70 .75 .80 .85 .90 .95 © E															€ BRG.					
CAMBER (GIRDER ALONE IN PLACE)	1	0.000	0.050	0.099	0.145	0.187	0.224	0.256	0.281	0.300	0.311	0.315	0.311	0.300	0.281	0.256	0.224	0.187	0.145	0.099	0.050	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L.	\	0.000	0.034	0.066	0.097	0.125	0.150	0.172	0.189	0.201	0.209	0.211	0.209	0.201	0.189	0.172	0.150	0.125	0.097	0.066	0.034	0.000
FINAL CAMBER	†	0	3/16"	3/8"	9/16"	3/4"	7⁄8″	1"	11/8"	13/16"	11/4"	11/4"	11/4"	13/16"	11/8"	1"	7/8"	3/4"	9/16"	3/8"	3/16"	0

0.6"Ø LOW RELAXATION STRANDS	SPAN I GIRDERS 1 & 4											
		© BRG.	.10	. 20	.30	.40	. 50	.60	.70	.80	.90	€ BRG.
CAMBER (GIRDER ALONE IN PLACE)	†	0.000	0.016	0.030	0.041	0.048	0.050	0.048	0.041	0.030	0.016	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.005	0.010	0.013	0.016	0.017	0.016	0.013	0.010	0.005	0.000
FINAL CAMBER	†	0	1/8"	1/4"	⁵ / ₁₆ "	3/8"	3/8"	3/8"	5/16"	1/4"	1/8"	0

0.6" Ø LOW RELAXATION STRANDS	SPAN I GIRDERS 2 & 3											
		ℚ BRG.	. IO	. 20	.30	. 40	. 50	.60	.70	.80	. 90	ℚ BRG.
CAMBER (GIRDER ALONE IN PLACE)	†	0.000	0.016	0.030	0.041	0.048	0.050	0.048	0.041	0.030	0.016	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L.	\	0.000	0.005	0.009	0.013	0.015	0.016	0.015	0.013	0.009	0.005	0.000
FINAL CAMBER	†	0	1/8"	1/4"	5/16"	3/8"	3/8"	3/8"	5/16"	1/4"	1/8"	0

→ ¾" BEVEL EDO

SECTION "F"

* INCLUDES FUTURE WEARING SURFACE.
VALUES SHOWN ARE IN FEET (DECIMAL FORM),
EXCEPT "FINAL CAMBER," WHICH IS SHOWN
IN INCHES (FRACTION FORM).

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

APPLY EPOXY PROTECTIVE COATING TO END OF GIRDER SURFACES INDICATED IN ELEVATION VIEW.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2"BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

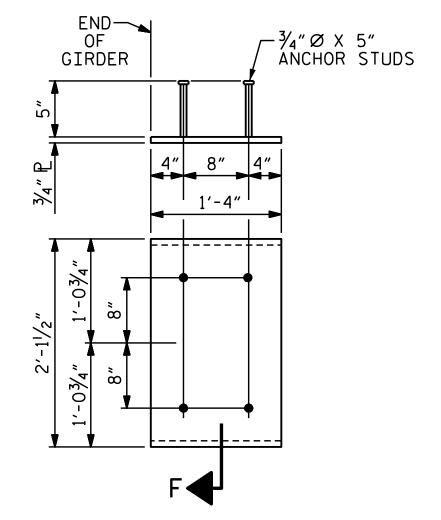
THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 8,000 PSI FOR SPANS A-H AND NOT LESS THAN 5,000 PSI FOR SPAN I.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4", EXCEPT AS NOTED IN THE LINK SLAB AREAS.

A 2" \times 2"CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 63"AND 72"MODIFIED BULB TEES ONLY.

THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD OF 4500 lbs.



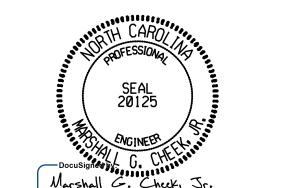
EMBEDDED PLATE "B-1" DETAIL

(2 REQ'D PER GIRDER)

PROJECT NO. B-5825

YADKIN/FORSYTH COUNTY

STATION: 34+65.50-L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

DEAD LOAD
DEFLECTIONS

Marshall G. Check Jr.

5FBCC2F3A4DC413... 4/14/2020

MENT NOT CONSIDERED FINAL

SIGNATURES COMPLETED

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
706 HILLSBOROUGH STREET
SUITE 200
RALEIGH, NC 27603
PH (919) 773-8887
CORP. LICENSE NO.: C-0275

REVISIONS

REVISIONS

SHEET NO.

BY: DATE: NO. BY: DATE: S-25

TOTAL SHEETS
60

SHEET 6 OF 7

DRAWN BY: TBE DATE: 1/20
CHECKED BY: MGC DATE: 1/20
DESIGN ENGINEER OF RECORD: TBE DATE: 2/20

6"X ½"₽—— SEE TABLE FOR LENGTH "L" (TYP.)

ZCS

ASSEMBLED BY :

DRAWN BY: RWW II/09 CHECKED BY : GM II/09

CHECKED BY:

SECTION A-A

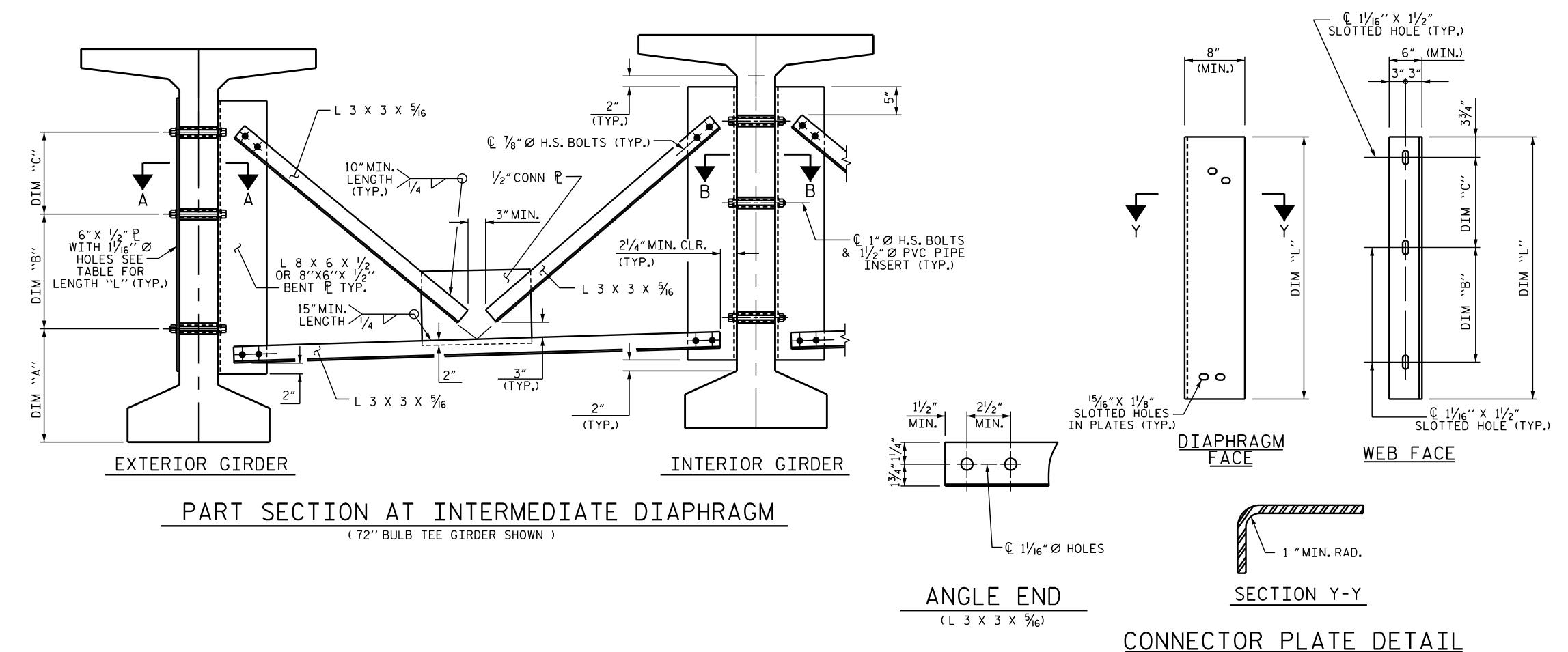
DATE : 1/20

DATE:

REV. 10/1/11 REV. 12/17

1/20

MAA/GM MAA/THC



STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY A THERMAL SPRAYED COATING WITH A SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE DEPARTMENTS THERMAL SPRAYED COATINGS (METALLIZATION) PROGRAM, THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST $\frac{1}{4}$ " PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

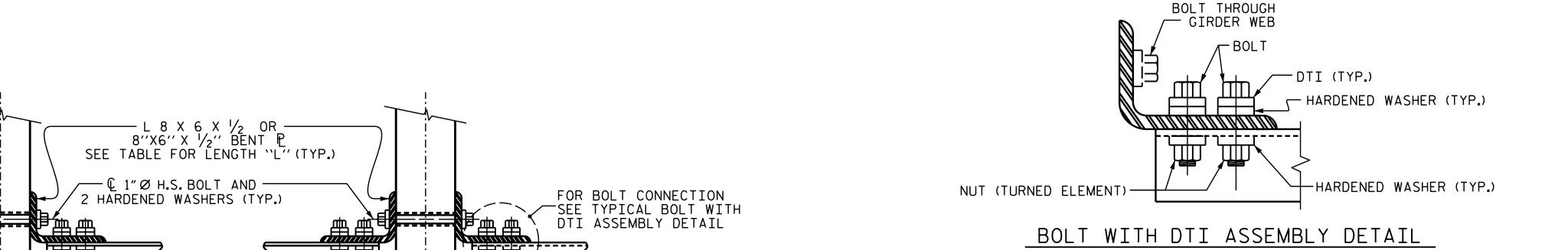
SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

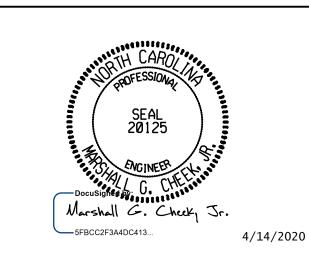
TABLE

GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"	DIM "L"	
72" BULB TEE	1′-6′′	1′-10′′	1'-2''	4′-2′′	



PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-

SHEET 7 OF 7



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR 63" & 72" MODIFIED BULB TEE PRESTRESSED CONCRETE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TGS ENGINEERS

706 HILLSBOROUGH STREET
SUITE 200

RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

GIRDERS SHEET NO **REVISIONS** S-26 DATE: BY: DATE: BY: TOTAL SHEETS 60

2/18/2020 X:\NCDOT\B-5825\Structures\Str. *1 - 34+65.50 -L-\FinalPlans\DGNs\401_055_B-5825_SMU_G7_980035.dgn

SECTION B-B

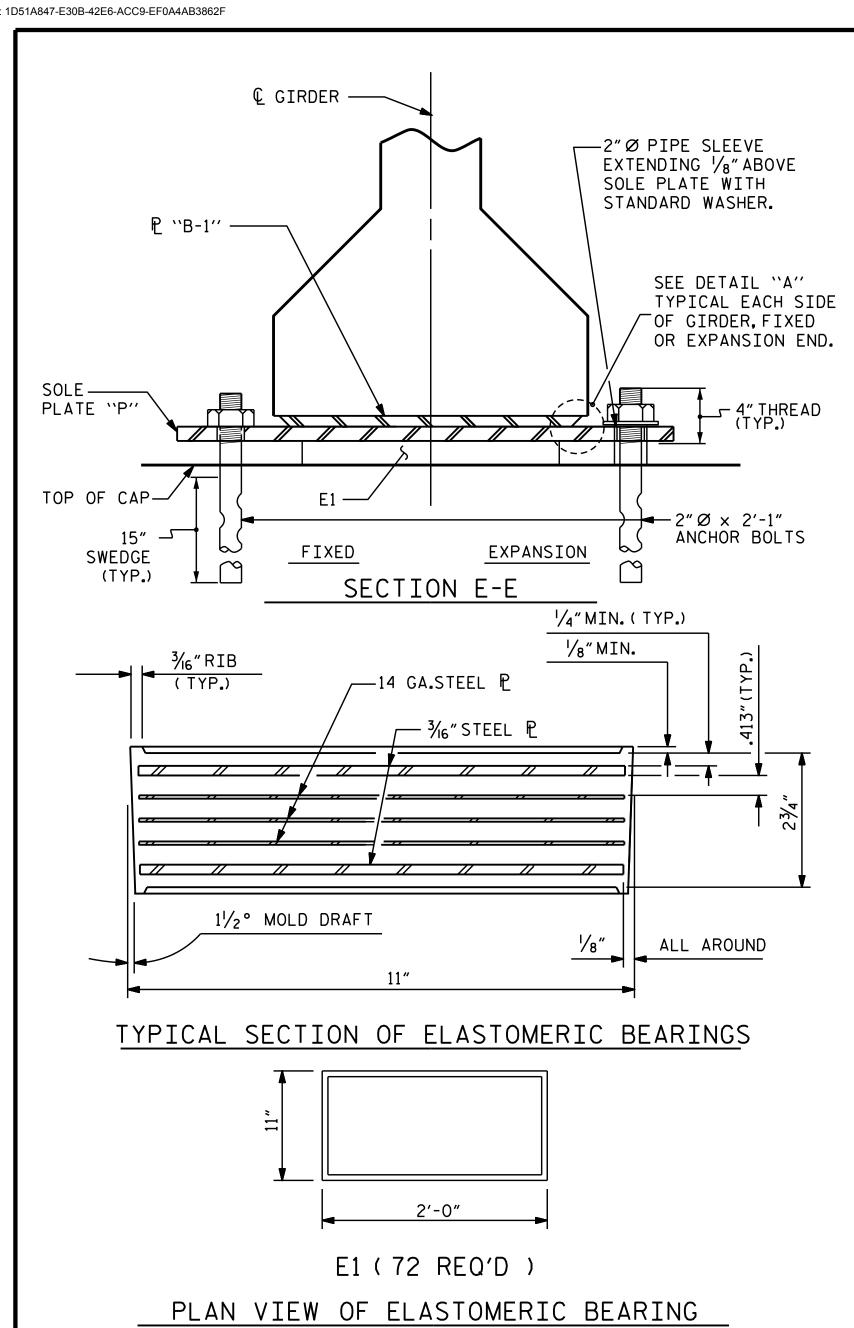
½ 1/8" Ø H.S. BOLT, —

2 HARDENED WASHERS AND DTI (TYP.)

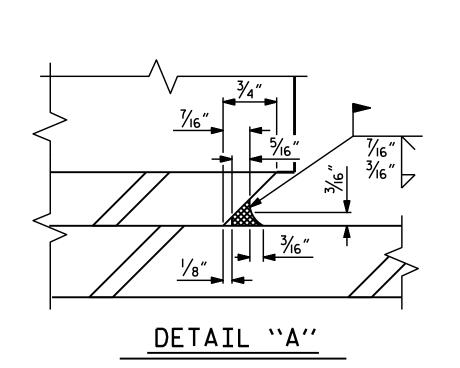
CONNECTION DETAILS

 $-L 3 X 3 X \frac{5}{16}$

+

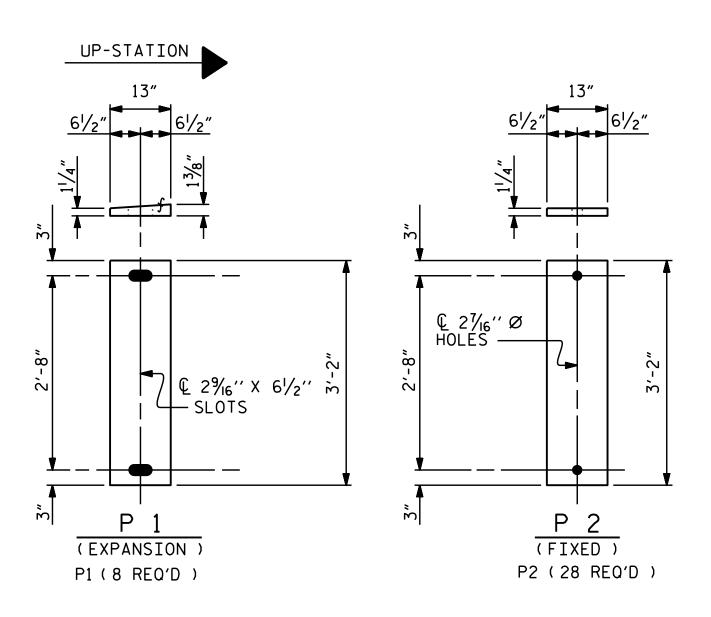


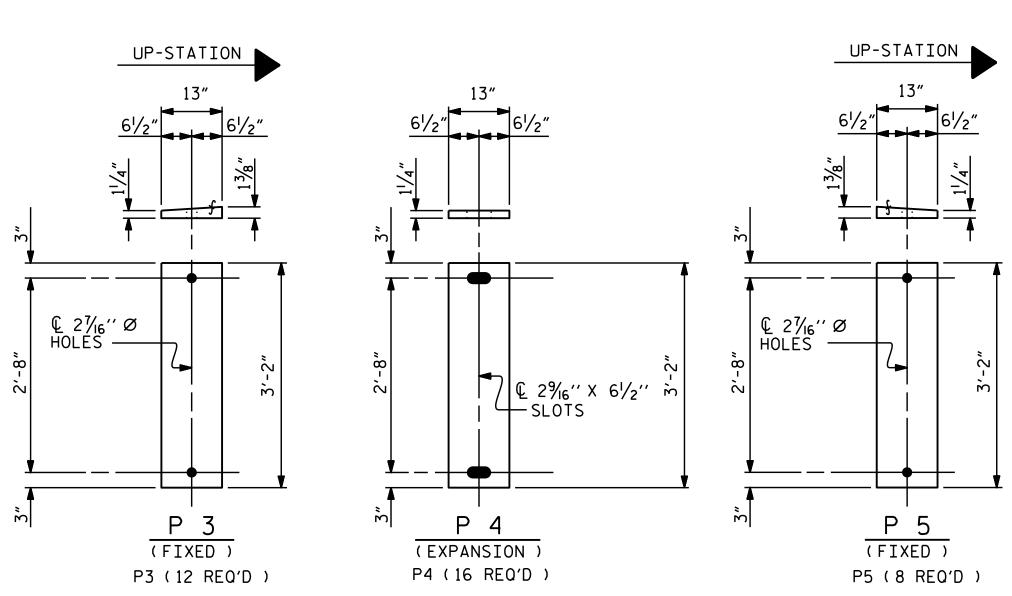
TYPE VII



ZCS MGC DATE: 11/19 DATE: 12/19 ASSEMBLED BY : CHECKED BY : AAC/MAA MAA/TMG MAA/THC DRAWN BY: EEM 2/97 REV. 6/13 REV. 1/15 REV. 12/17

MAXIMUM ALLOWABLE SERVICE LOADS D.L.+L.L.(NO IMPACT) 470 k





SOLE PLATE DETAILS ("P")

FOR SOLE PLATE LOCATIONS, SEE "FRAMING PLAN" SHEET.

NOTES

AT ALL FIXED POINTS OF SUPPORT. NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BÈ BURRED WITH A SHARP POINTED TOOL.

THE 2"Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

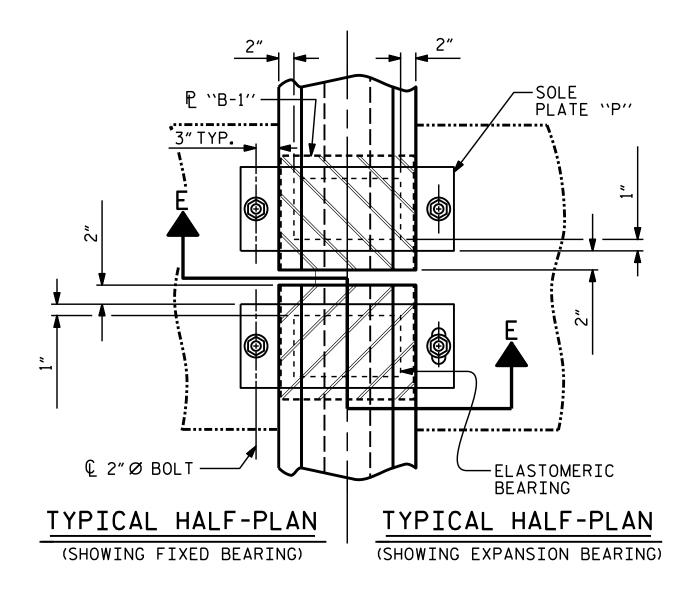
ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. NO SHOP DRAWINGS ARE REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

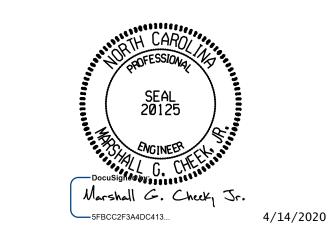
THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

ALL SOLE PLATES SHALL BE AASHTO M270 GRADE 36.



PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

ELASTOMERIC BEARING DETAILS ——

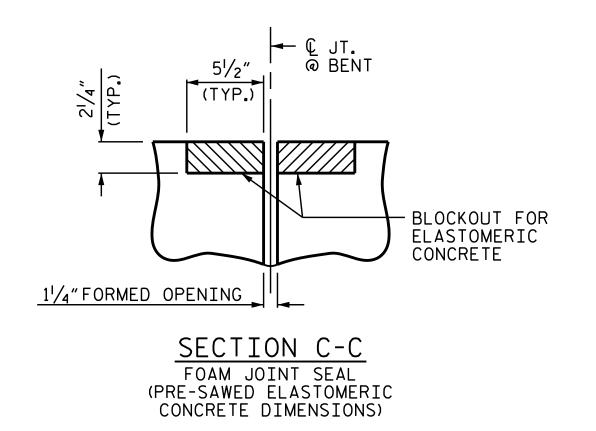
PRESTRESSED CONCRETE GIRDER

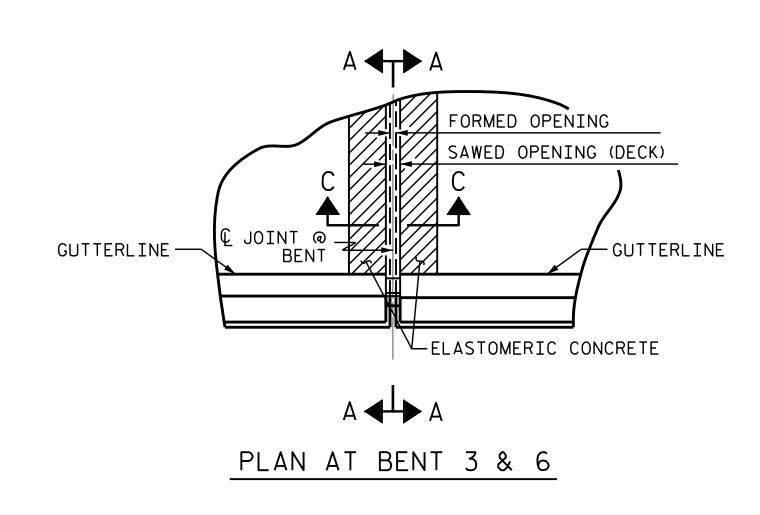
SUPERSTRUCTURE DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

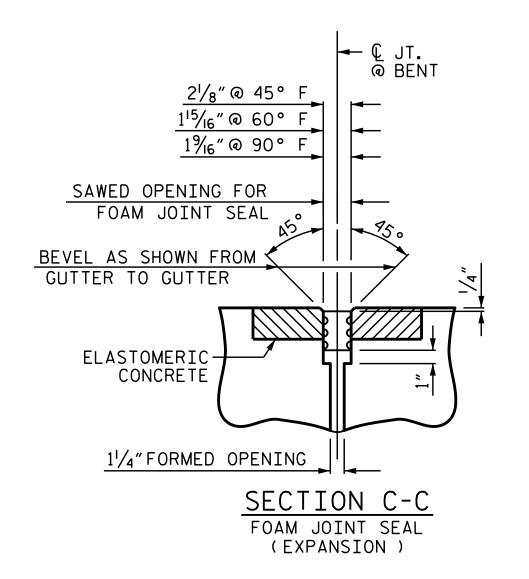
TGS ENGINEERS
706 HILLSBOROUGH STREET SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275 SHEET NO. **REVISIONS** S-27 DATE: DATE: BY: BY: TOTAL SHEETS 60

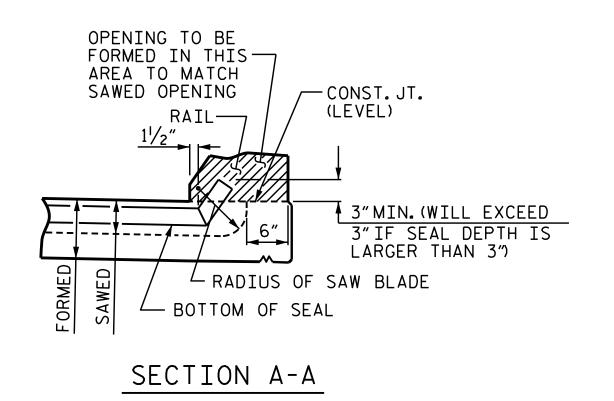
STD. NO. EB4 (SHT 3)











NOTES

FOR FOAM JOINT SEALS, SEE SPECIAL PROVISIONS.

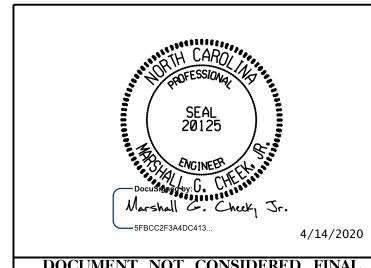
THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE FOAM JOINT SEAL SHALL BE $2^{l}/_{2}$ ".

FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.

ELAST	OMERIC	CONCRETE				
BENT NO.	CONC	TOMERIC RETE * J.FT.)				
3		5.50				
6		5.50				
TOTAL	1	1.00				

* BASED ON THE MINIMUM BLOCKOUT SHOWN.

PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

> FOAM JOINT SEAL DETAILS

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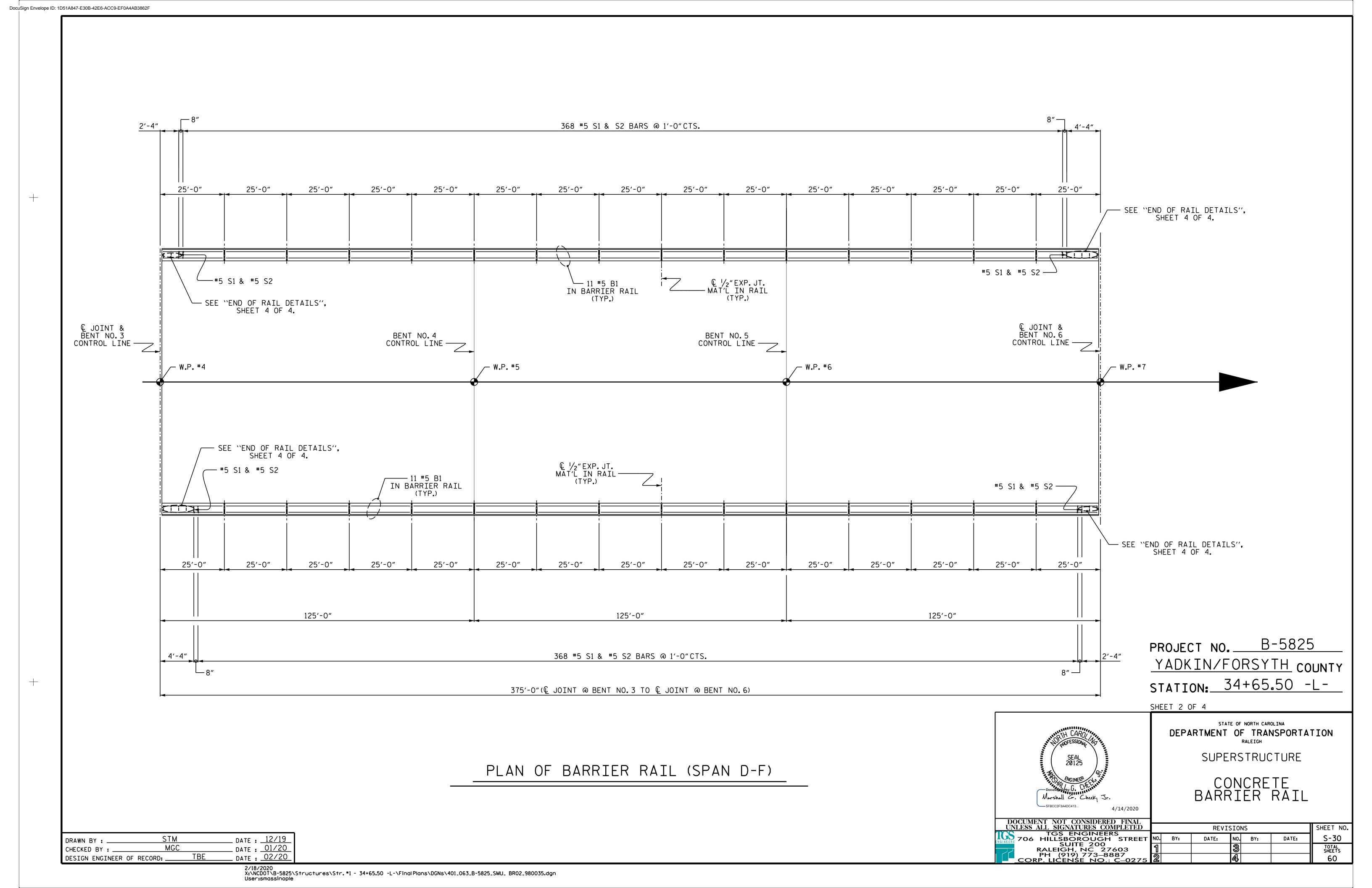
TGS ENGINEERS
706 HILLSBOROUGH STREET SUITE 200
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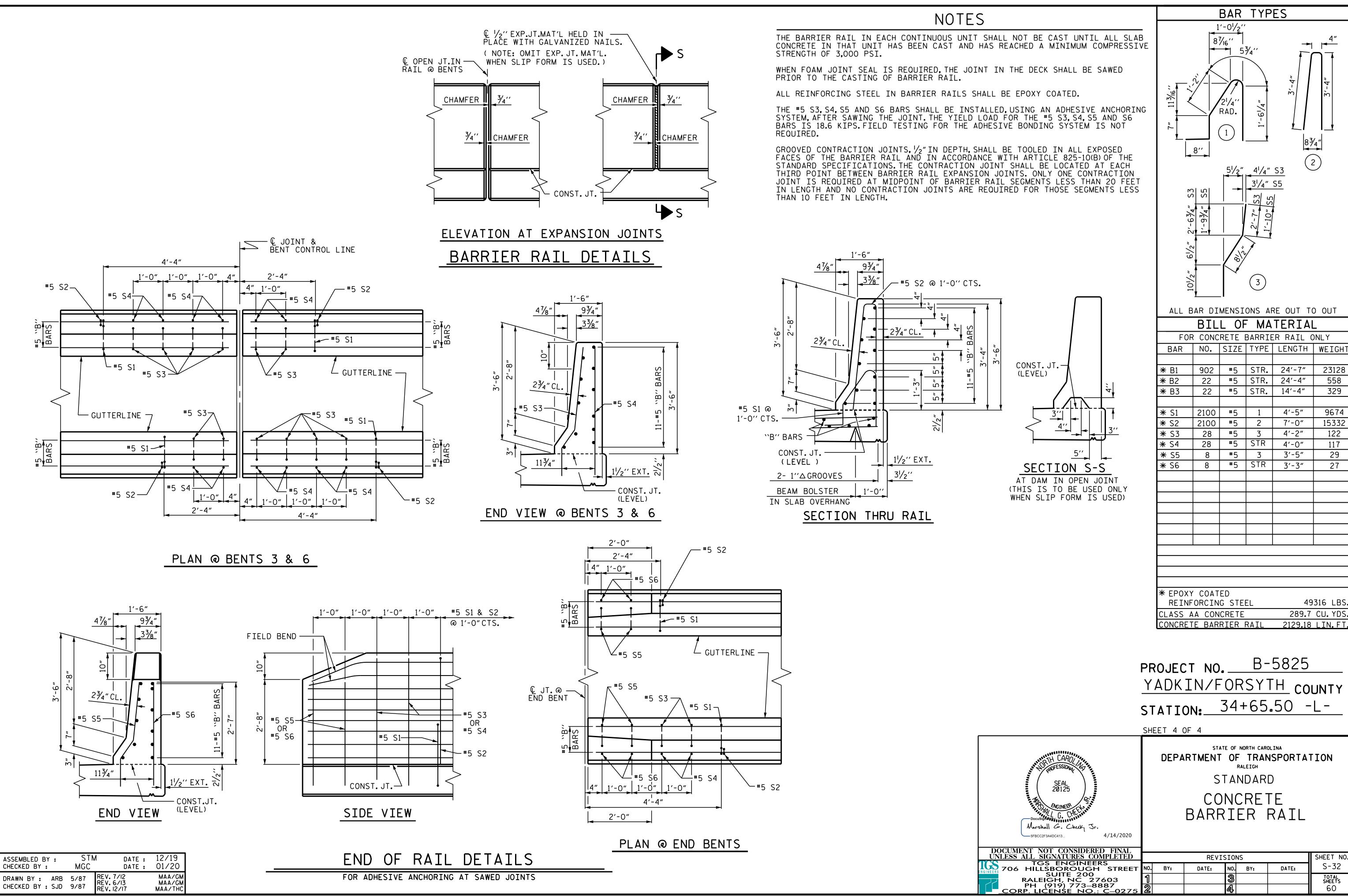
REVISIONS SHEET NO. NO. BY: S-28 DATE: DATE: BY: TOTAL SHEETS 60

STD. NO. BAS4

ZCS __ DATE : <u>1/20</u> DRAWN BY : _ __ DATE : ____1/20 MGC CHECKED BY : _ DESIGN ENGINEER OF RECORD: TBE DATE: 02/20

+





DATE:

SHEET NO

S-32

TOTAL SHEETS

→ | -

83/4"

2

23128

329

9674

15332

122

29

49316 LBS

289.7 CU. YDS

2129.18 LIN. FT

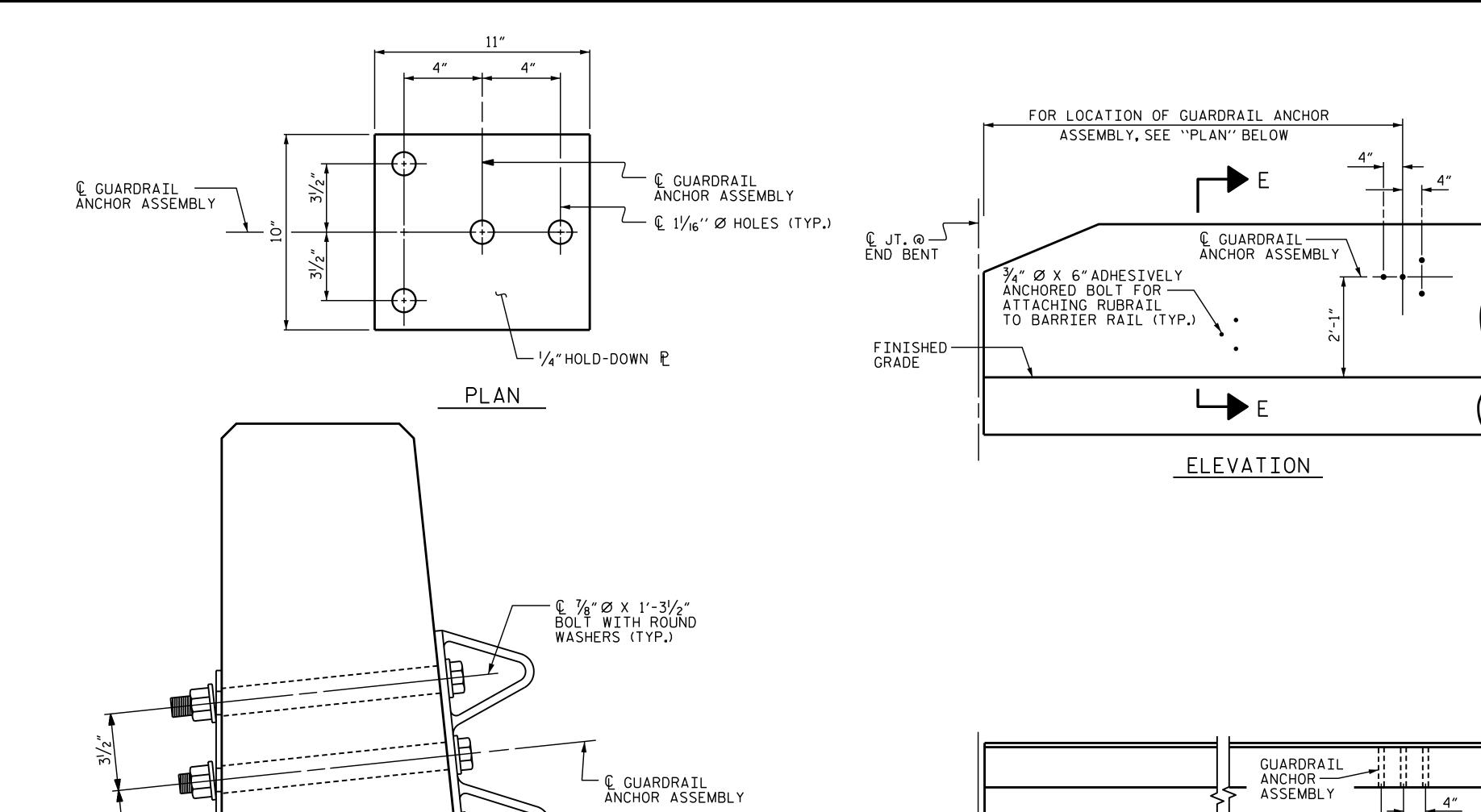
14'-4"

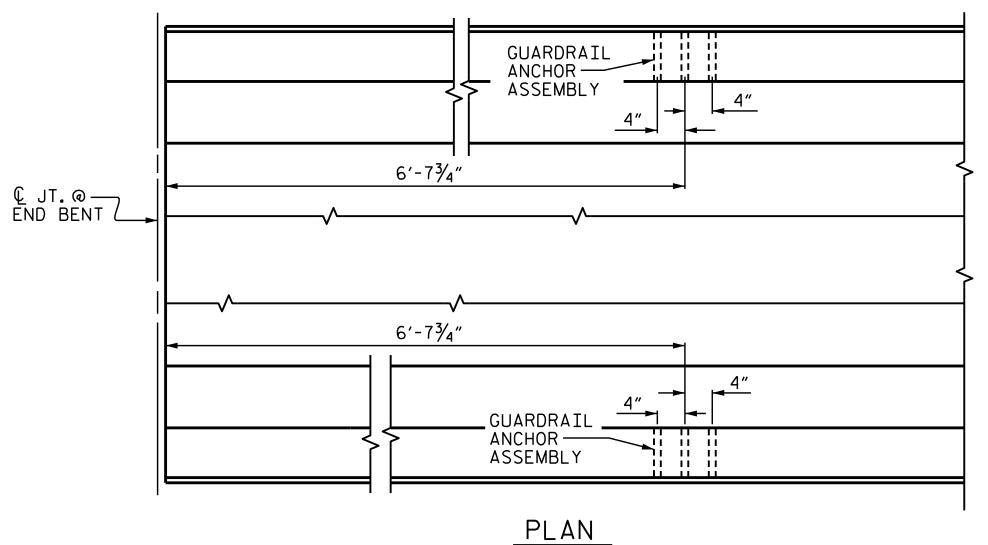
4'-5"

7′-0″

4'-2"

3′-5″





LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD-DOWN PLATE AND 4 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{1}{8}$ " \varnothing GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

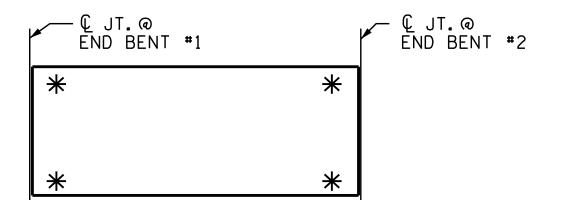
AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE $\frac{3}{4}$ " \varnothing X 6"BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND

YIELD LOAD OF THE $\frac{3}{4}$ " Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



SKETCH SHOWING POINTS OF ATTACHMENTS

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

B-5825 PROJECT NO. ____ YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

GUARDRAIL ANCHORAGE FOR BARRIER RAIL

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TGS ENGINEERS

706 HILLSBOROUGH STREET
SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

SHEET NO **REVISIONS** S-33 DATE: DATE: BY: BY:

DATE: 01/20 DATE: 01/20 ASSEMBLED BY : CHECKED BY : MAA/GM MAA/GM MAA/THC DRAWN BY: TLA 5/06 REV. 7/12 CHECKED BY: GM 5/06 REV. 6/13 REV. 12/17

¼"HOLD-DOWN ₽—

11/4"Ø DRILLED OR FORMED HOLE (TYP.)

ADHESIVELY ANCHORED — 3/4" Ø X 6" BOLTS FOR ATTACHING RUBRAIL TO

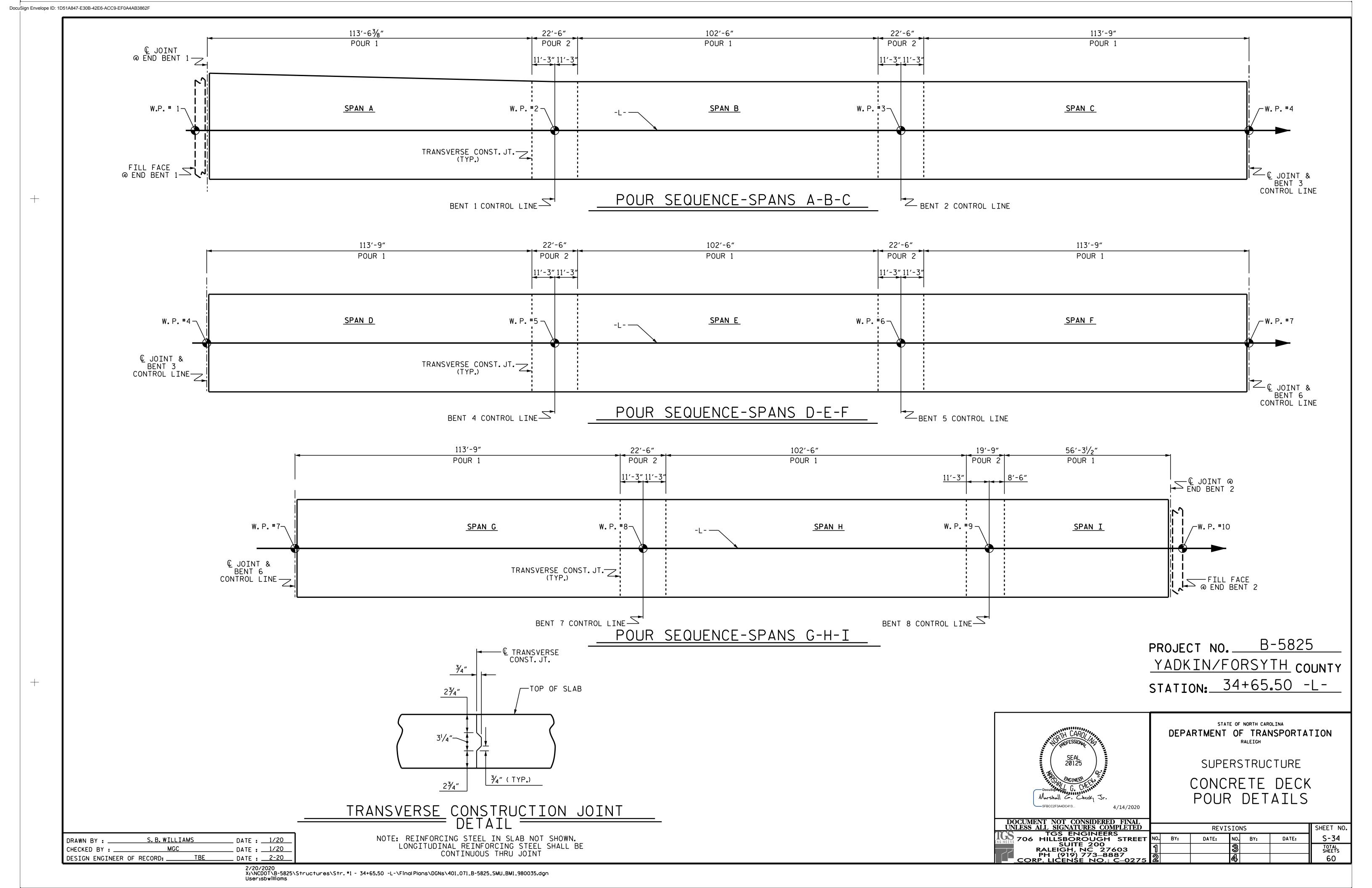
BARRIER RAIL (TYP.) SEE ROADWAY STD. 862.03

SECTION E-E

GUARDRAIL ANCHOR ASSEMBLY DETAILS

-C6 X 8.2 RUBRAIL

— FINISHED GRADE



ASSEMBLED BY : S. B. WILLIAMS

DRAWN BY: JMB 5/87 CHECKED BY: SJD 9/87

CHECKED BY : MGC

DATE : 10/19 DATE : 1/20

MAA/GM MAA/THC BNB/THC

1D51A847-E30E	D51A847-E30B-42E6-ACC9-EF0A4AB3862F																
	SPANS A-B-C					SPANS D-E-F					SPANS G-H-I						
BAR	No.	SIZE	TYPE	LENGTH	WEIGHT	BAR	No.	SIZE	TYPE	LENGTH	WEIGHT	BAR	No.	SIZE	TYPE	LENGTH	WEIGHT
* A1 A2	463 463	#5 #5	STR. STR.	34'-11'' 34'-11''	16,862 16,862	* A1 A2	642 642	#5 #5	STR. STR.	34'-11'' 34'-11''	23,380 23,380	* A1 A2	538 538	#5 #5	STR. STR.	34'-11'' 34'-11''	19 , 593 19 , 593
* A10 * A10 * A10 * A10 * A10	2 35 3 35 4 35	#5 #5 #5 #5	STR. STR. STR. STR. STR.	35'- 5'' 35'-11'' 36'- 5'' 36'-11'' 37'- 5''	1,293 1,311 1,329 1,348 1,483	B1 * B4 * B6 * B7 * B8	203 20 138 178 69	#5 #4 #4 #6 #4	STR. STR. STR. STR. STR.	55'- 4'' 39'-3'' 40'-5'' 19'-0'' 37'-11''	11,716 524 3,726 5,080 1,748	* B6 * B7 * B8 B9 * B10 * B11	69 89 69 174 18 46	#4 #6 #4 #5 #4	STR. STR. STR. STR. STR. STR.		1,863 2,540 1,748 9,845 440 950
A20 A20		#5 #5	STR. STR.	35'- 5'' 35'-11''	1 , 293 1 , 311	∗ G2	2	#5	STR.	34'-11''	73	* B12	89	#6	STR.	16'- 3"	2172
A20 A20	3 35	#5 #5	STR. STR.	36'- 5'' 36'-11''	1,329 1,348	* K1 * K2	8 8	#8 #8	1 2	14'-0'' 20'-5''	299 436	* G2	2	# 5	STR.	34′-11′′	73
A20		# 5	STR.	37'- 5''	1,483	* S1	36	#5	3	6′-0′′	225	* K1 * K2	8 8	#8 #8	1 2	14'-0'' 20'-5''	299 436
B1 B2 B3 * B4	105 54 70 20	#5 #5 #5 #4	STR. STR. STR. STR.	55'- 4'' 42'- 8'' 51'- 7'' 39'-3''	6,060 2,403 3,766 524			NG STE			96 LBS.	∗ S1 REIN	36 NFORCI	#5 ING STE	3 EEL	6′-0′′ 29 . 4	225 38 LBS.
* B5 * B6 * B7 * B8	138 178 69	#4 #4 #6 #4	STR. STR. STR. STR.	47'-8'' 40'-5'' 19'-0'' 37'-11''	127 3,726 5,080 1,748	* EPO) REII	XY COA NFORCI	ATED ING STE	EL	<u>35,</u>	491 LBS.	* EP0)	KY COA				39 LBS.
* G1 * G2	1 1	#5 #5		37′-11′′ 34′-11′′	40 36												
* K1 * K2	8 8	#8 #8	1 2	14'-0'' 20'-5''	299 436												
* S1	40	# 5	3	6'-0''	250												
REI	REINFORCING STEEL 35,855 LBS.																
	* EPOXY COATED REINFORCING STEEL 35,892 LBS.																

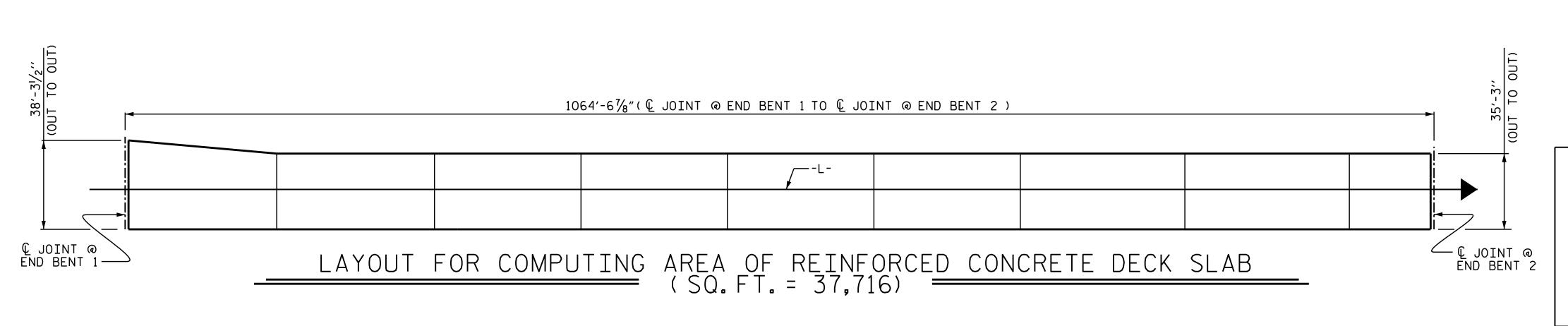
CLASS AA CONCRETE BREAKDOWN POUR #1 SPAN A 146.0 CU. YDS. SPAN B 124.5 CU. YDS. SPAN C _ 139.8 CU. YDS. SPAN D ____ 139.8 CU. YDS. SPAN E 124.5 CU. YDS. SPAN F ___ 139.8 CU. YDS. SPAN G _ 139.8 CU. YDS. SPAN H _ 124.5 CU. YDS. SPAN I 70.0 CU. YDS. TOTAL POUR #1 ___ ___1,148.7 CU. YDS. <u>POUR #2</u> @ W.P.#2 27.4 CU. YDS. 27.3 CU. YDS. @ W.P.#3 @ W.P.#5 27.3 CU. YDS. @ W.P.#6 27.3 CU. YDS. @ W.P.#8 27.3 CU. YDS. @ W.P.#9 24.0 CU. YDS. _ 160.6 CU. YDS. TOTAL POUR #2_ ___ 1,309.3 CU. YDS.

> FOR LOCATION OF POURS, SEE "CONCRETE DECK POUR DETAILS" SHEET

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS

BAR SIZE	SUPERSTF EXCEPT A SLABS, PA AND BARRI	APPROACH ARAPETS.	APPROAC	PARAPETS AND BARRIER	
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAILS
#4	1'-11"	1'-7"	1'-11"	1'-7"	2'-6"
* 5	2′-5″	2'-0"	2′-5″	2′-0″	3'-1"
#6	2′-10″	2′-5″	3'-7"	2′-5″	3′-8″
# 7	4′-2″	2'-9"			
# 8	4′-9″	3′-2″			

GROOVING	BRIDGE	FL	<u> </u>
APPROACH SLABS	1,45	54	SO.FT.
BRIDGE DECK	30,98	33	SO.FT.
TOTAL	32,43	37	SO.FT.



5'-11'' 5'-11'' 5'-11'' ALL BAR DIMENSIONS ARE OUT TO OUT

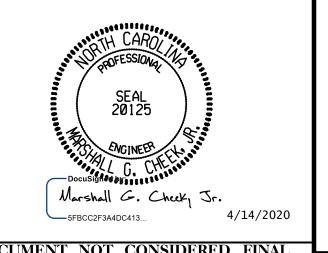
-BAR TYPES-

---SUPERSTRUCTURE BILL OF MATERIAL---EPOXY COATED REINFORCING STEEL CLASS AA CONCRETE REINFORCING STEEL

(LBS.) (CU. YDS.) (LBS.) SPANS A-B-C 465.0 35,892 35,855 SPANS D-E-F 35,491 458.7 35,096 SPANS G-H-I 29,438 30,339 1309.3 TOTALS** 100,389 101,722

**QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE BILL OF MATERIAL

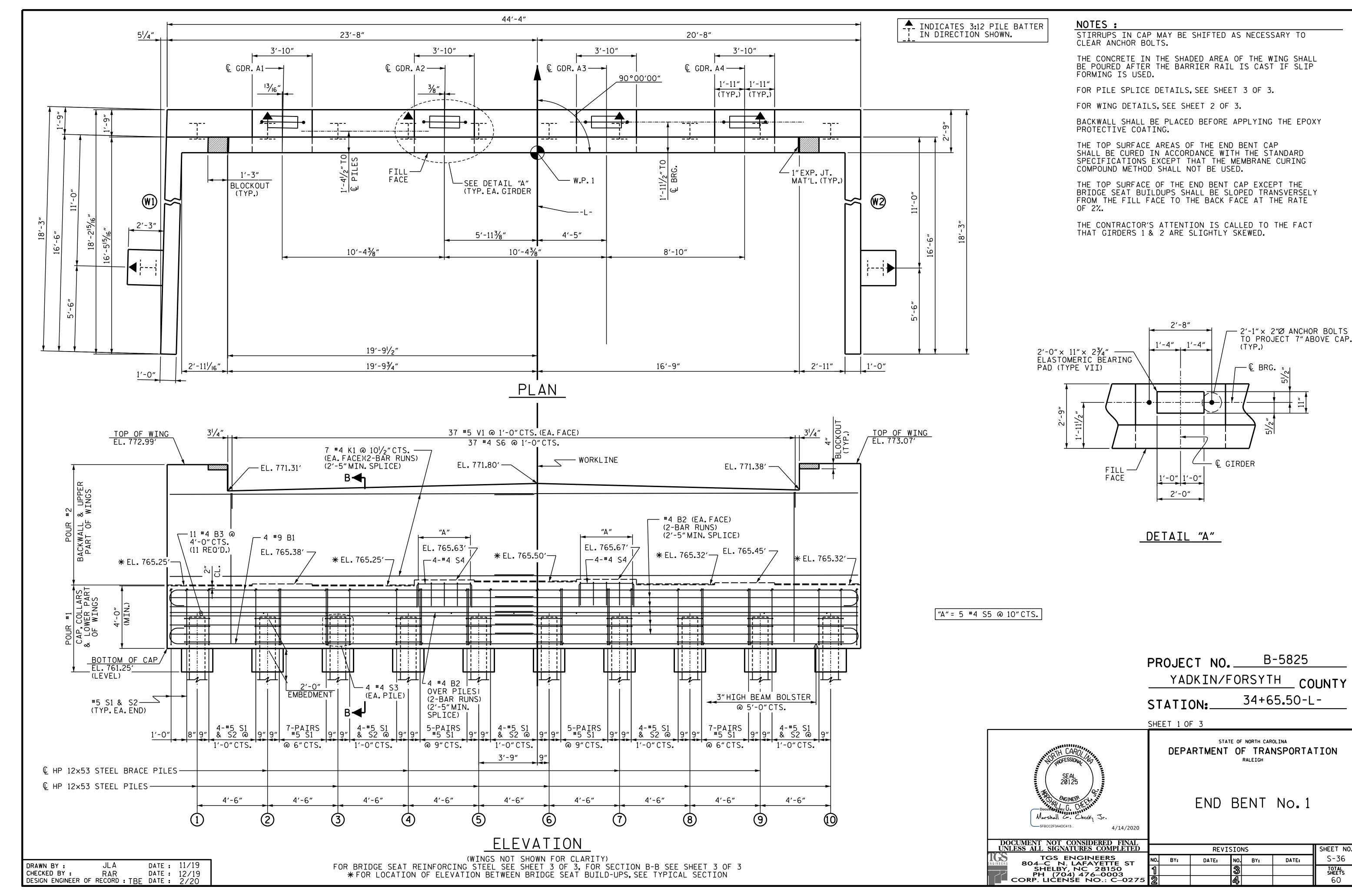
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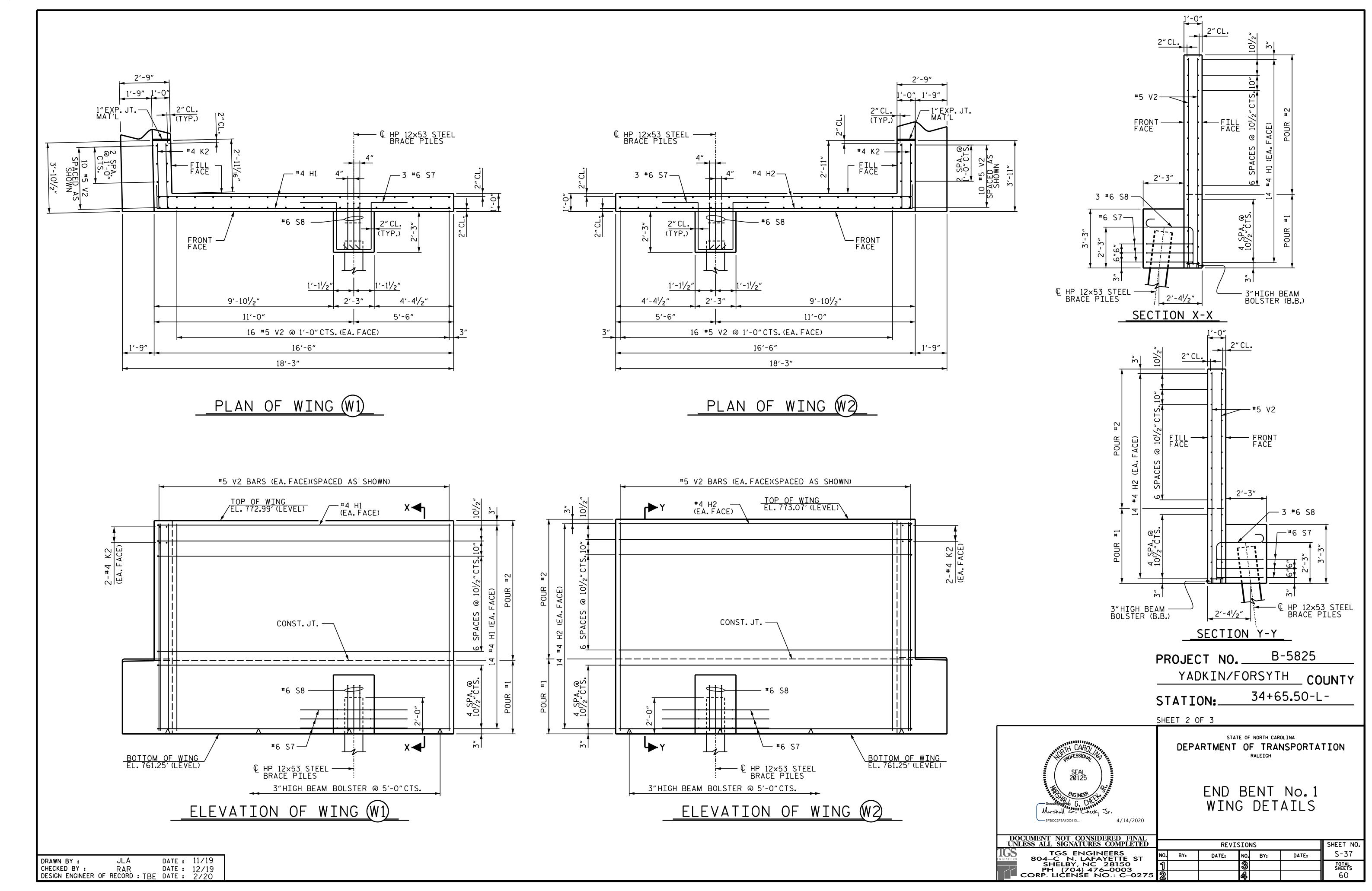
TGS ENGINEERS

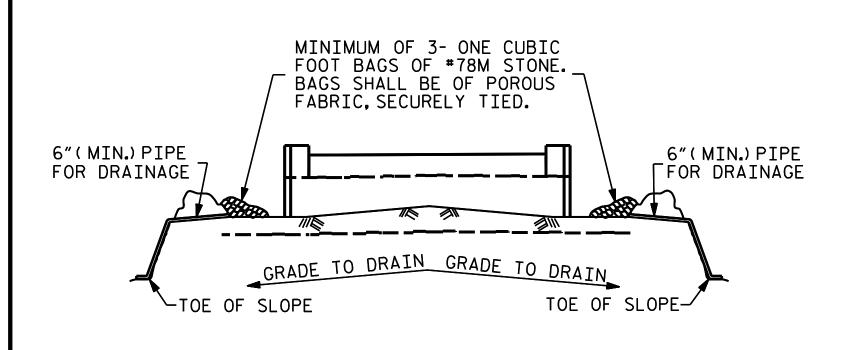
706 HILLSBOROUGH STREET SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275 SHEET NO. **REVISIONS** NO. BY: DATE: DATE: BY:

S-35

TOTAL SHEETS





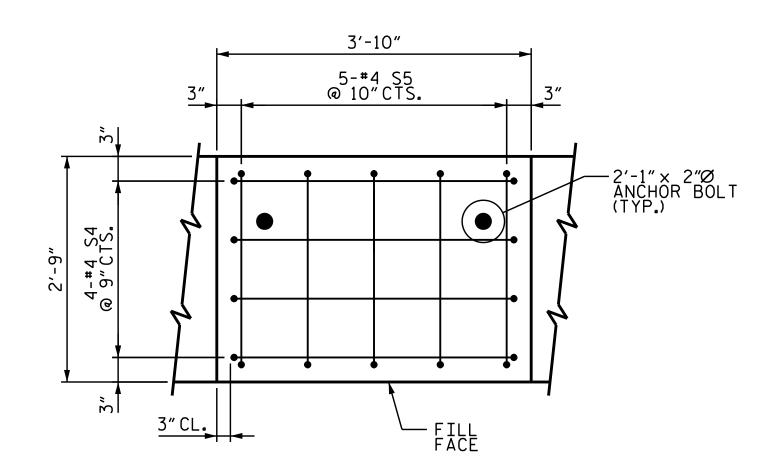


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

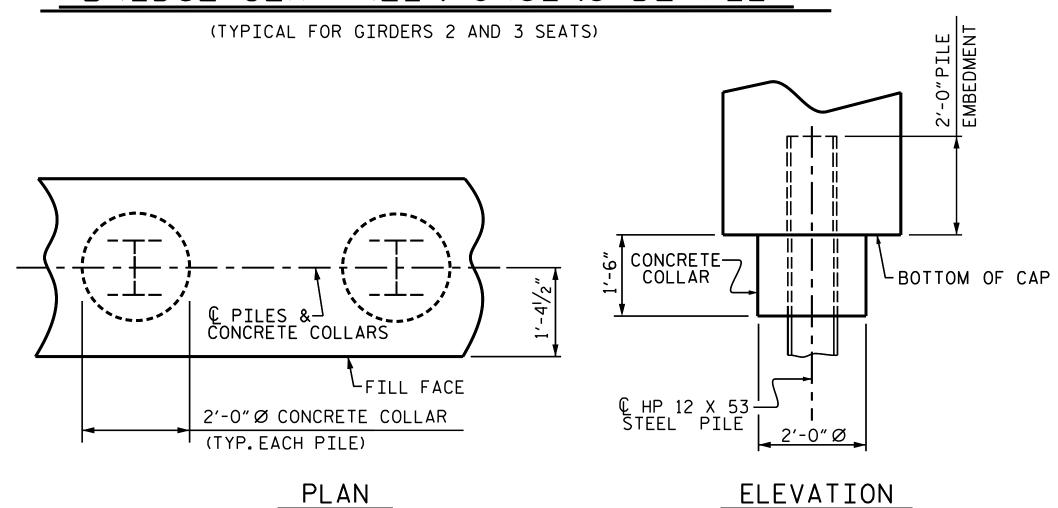
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



BRIDGE SEAT REINFORCING DETAIL

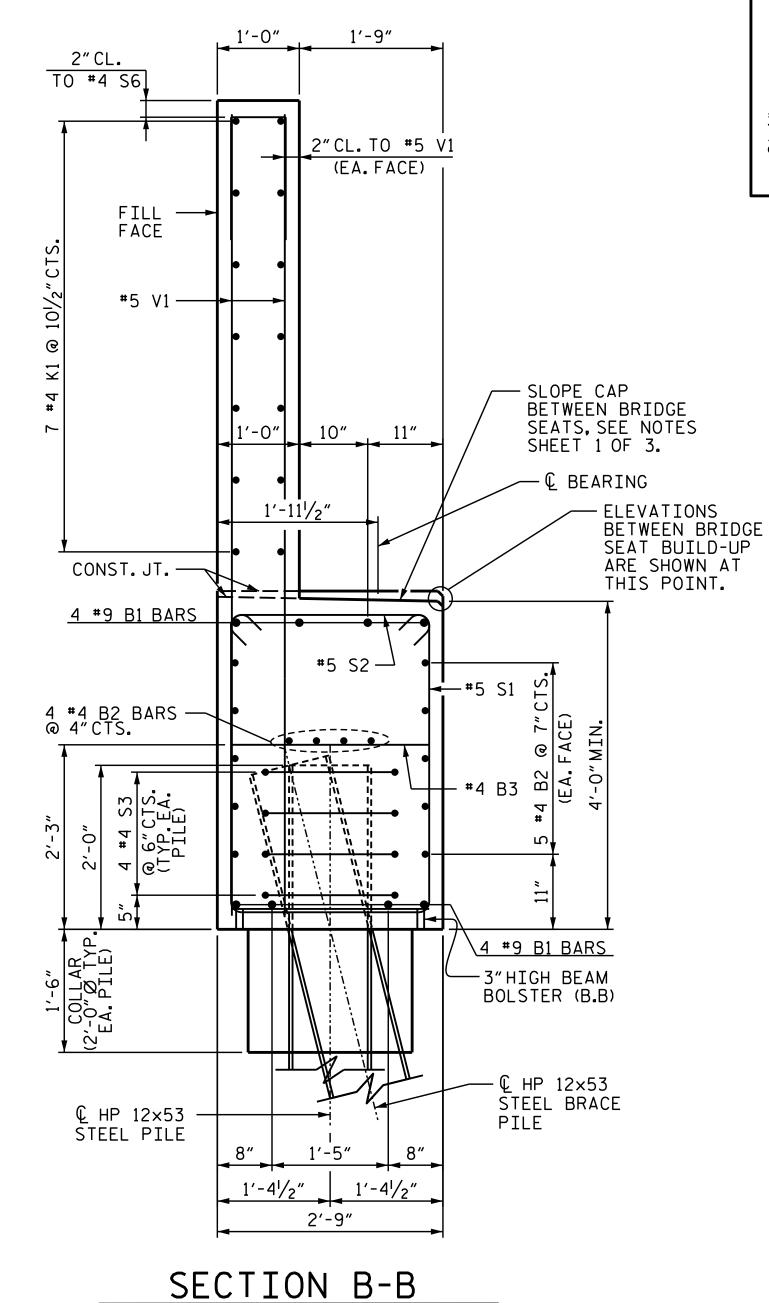


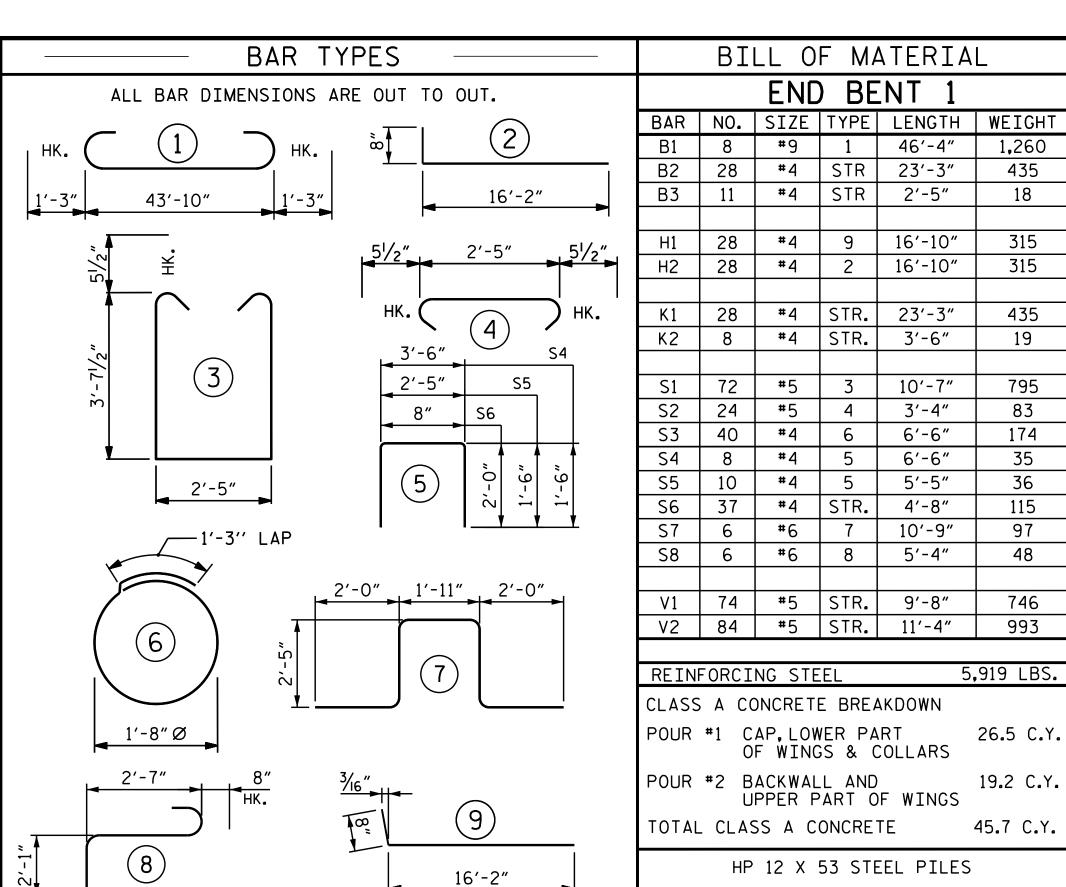
CORROSION PROTECTION FOR STEEL PILES DETAIL

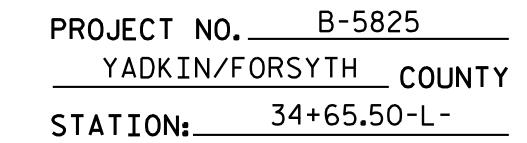
DRAWN BY: JLA DATE: 11/19
CHECKED BY: RAR DATE: 12/19
DESIGN ENGINEER OF RECORD: TBE DATE: 2/20

/ BACK GOUGE BACK GOUG DETAIL B BACK GOUGE DETAIL A PILE VERTICAL <u> PILE HORIZONTAL</u> OR VERTICAL O'' TO 1/8" DETAIL A DETAIL B POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS





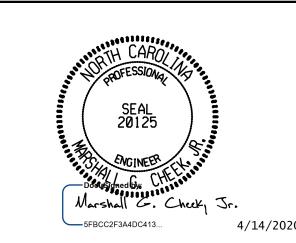


LIN. FT.= 690

PILE DRIVING EQUIPMENT SETUP

FOR HP 12 x 53 STEEL PILES EACH = 12

SHEET 3 OF 3



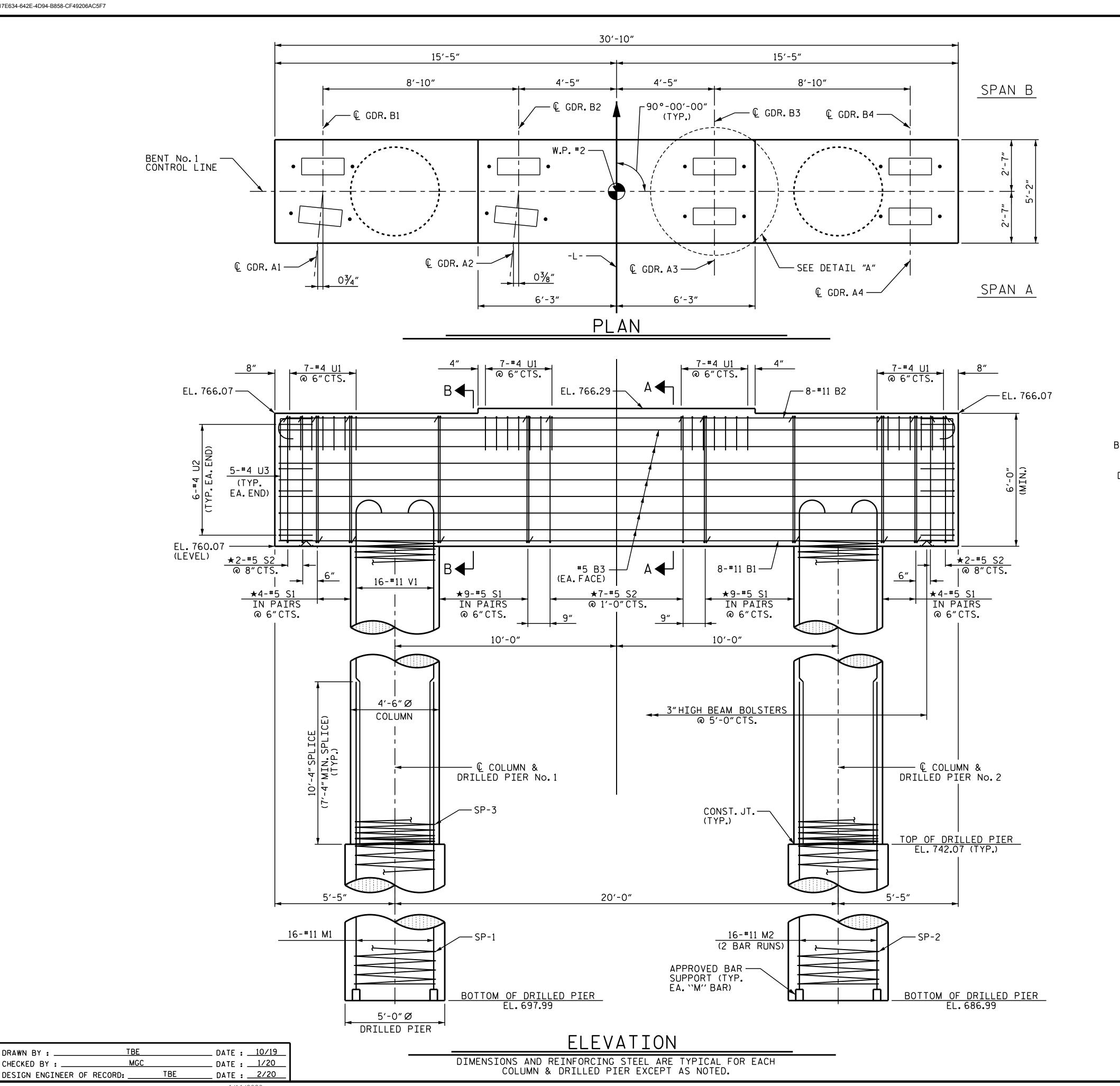
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

END BENT No.1 DETAILS

TOTAL SHEETS 60

DOCUMENT NOT CONSIDERE UNLESS ALL SIGNATURES CO TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275

4/14/2020							
ED FINAL DMPLETED	<u> </u>		REVIS	SION	NS		SHEET NO
RS TTE ST	NO.	BY:	DATE:	NO.	BY:	DATE:	S-38



NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

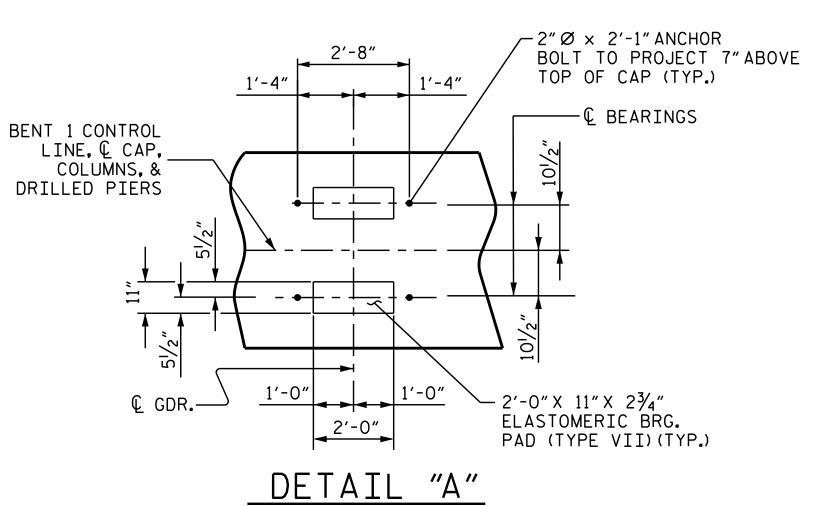
ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL."

★ INVERT ALTERNATE STIRRUPS.

THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND LINE ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT 1 FOOT BELOW THE GROUND LINE.

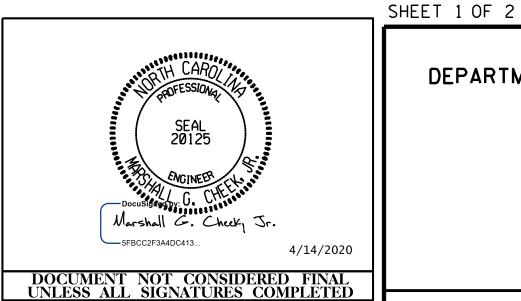
THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT GIRDERS 1 & 2 ARE SLIGHTLY SKEWED.



DIMENSIONS ARE TYPICAL FOR EACH BEARING.

PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50-L-



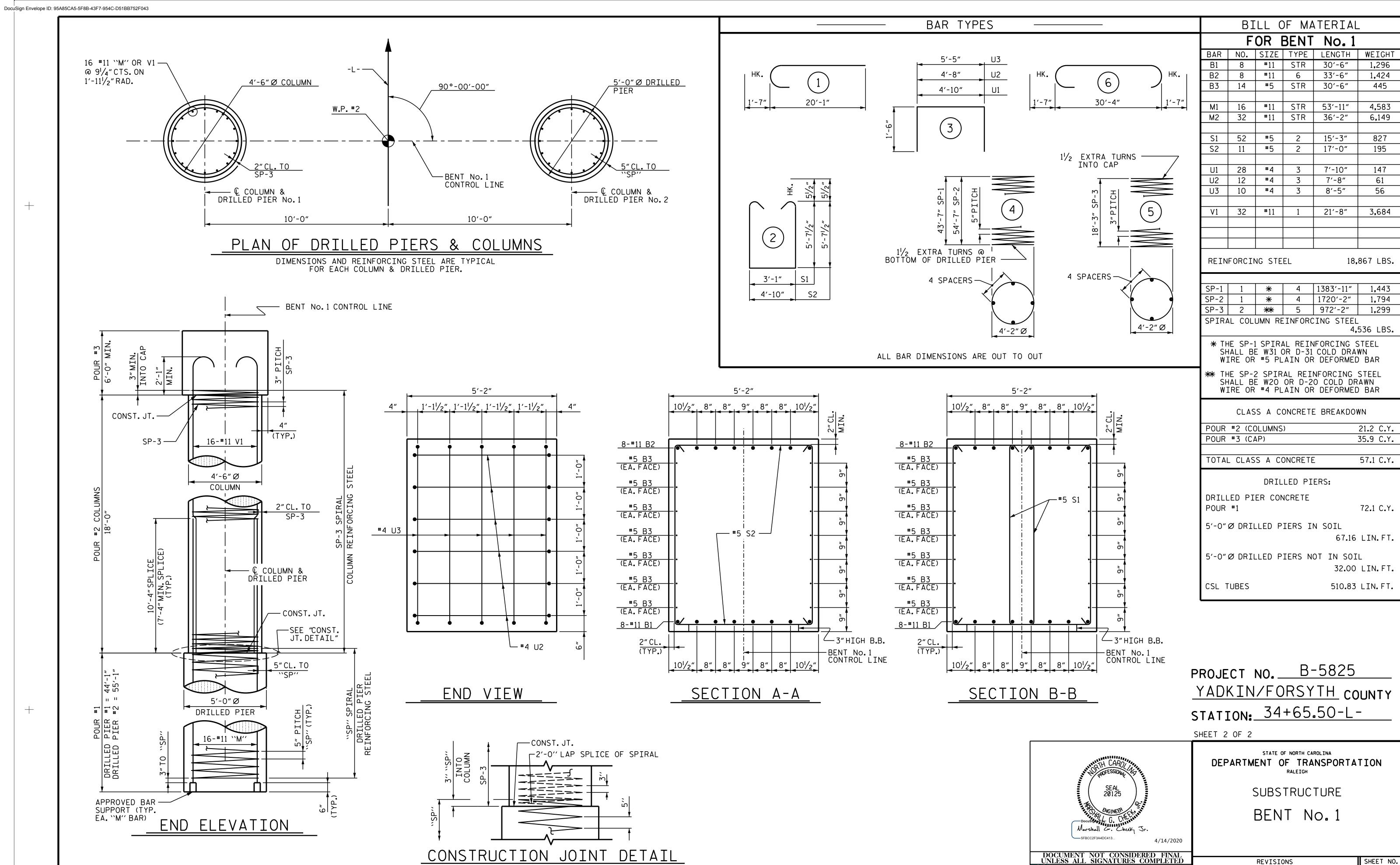
TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

BENT No. 1

SHEET NO. **REVISIONS** S-39 NO. BY: DATE: DATE: TOTAL SHEETS



2/13/2020 X:\NCDOT\B-5825\Structures\Str. #1 - 34+65.50 -L-\FinalPlans\DGNs\401_083_B-5825_SMU_ B102_980035.dgn User:ZSmith

__ DATE : __10/19__

_ DATE : ___1/20

DATE : 2/20

MGC

TBE

DRAWN BY :

DESIGN ENGINEER OF RECORD: _

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
706 HILLSBOROUGH STREET SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

REVISIONS

REVISIONS

1
3
4

S-40

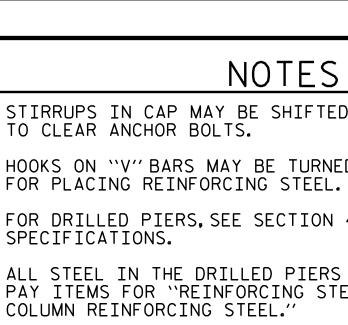
TOTAL SHEETS

60

DATE:

DESIGN ENGINEER OF RECORD: _

MGC



STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

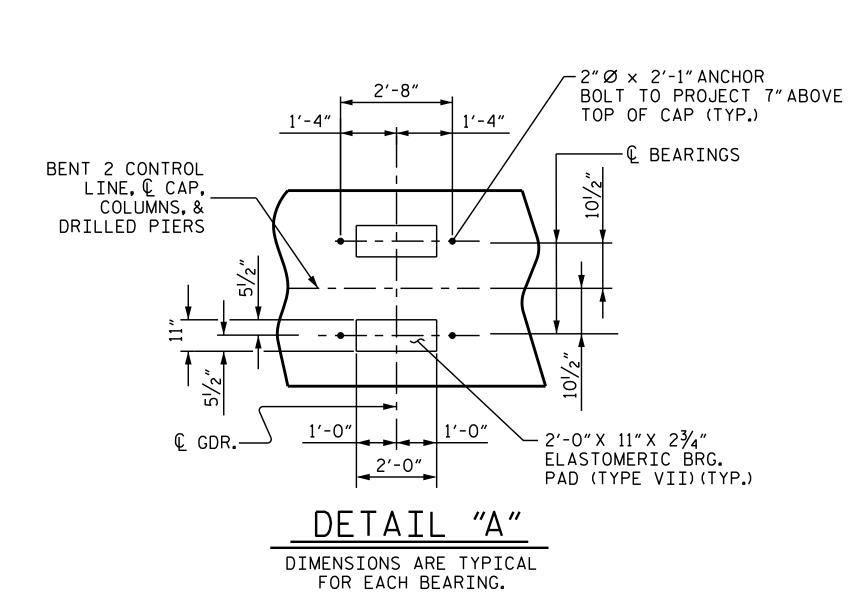
FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

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THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.



PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50-L-



SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

BENT No. 2

NO. BY:

REVISIONS

DATE:

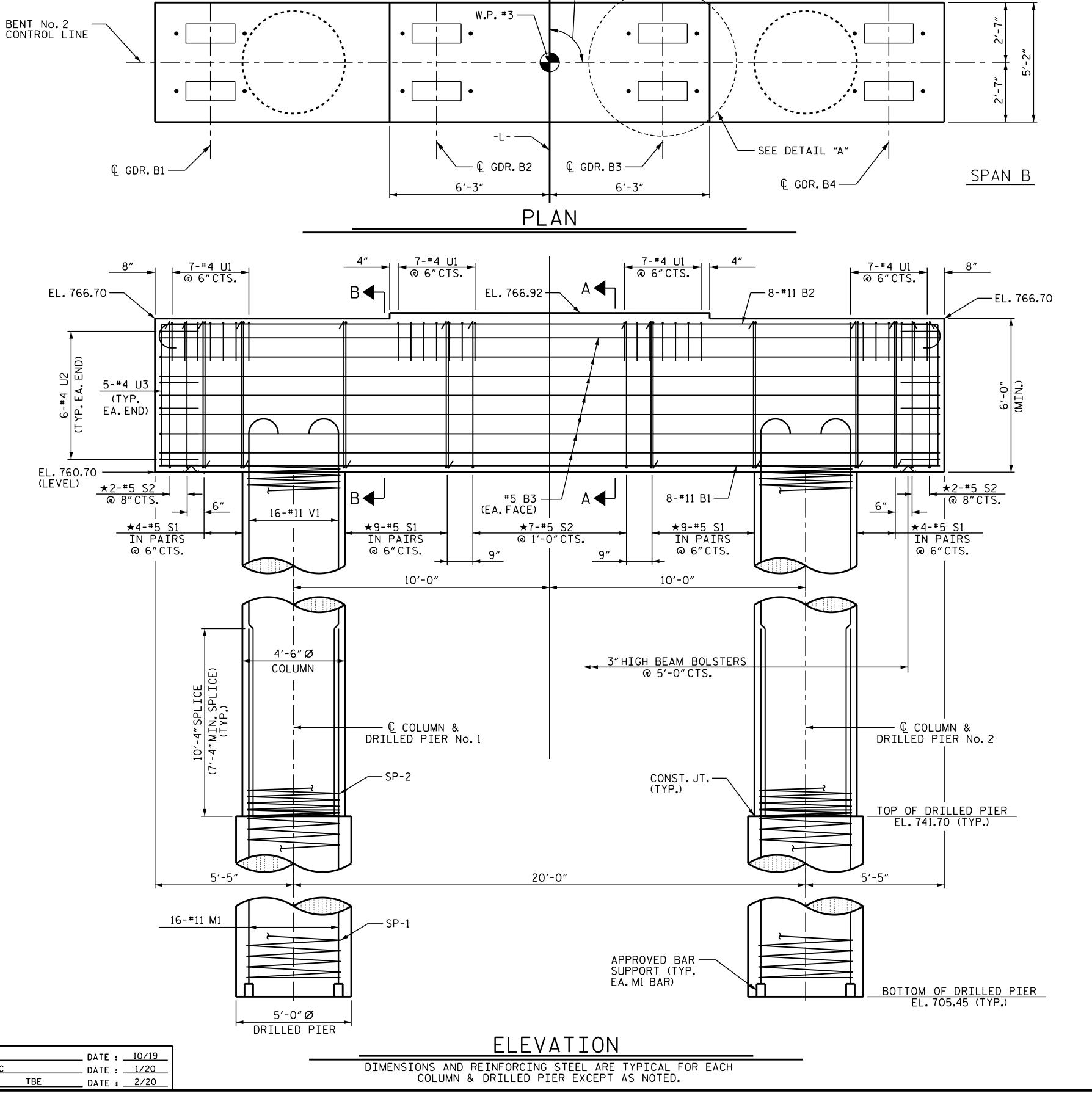
SHEET NO.

S-41

TOTAL SHEETS

DATE:

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275



30'-10"

4'-5"

_90°-00'-00"

(TYP.)

4'-5"

— € GDR.C2

15'-5"

-ℚ GDR.C3

8'-10"

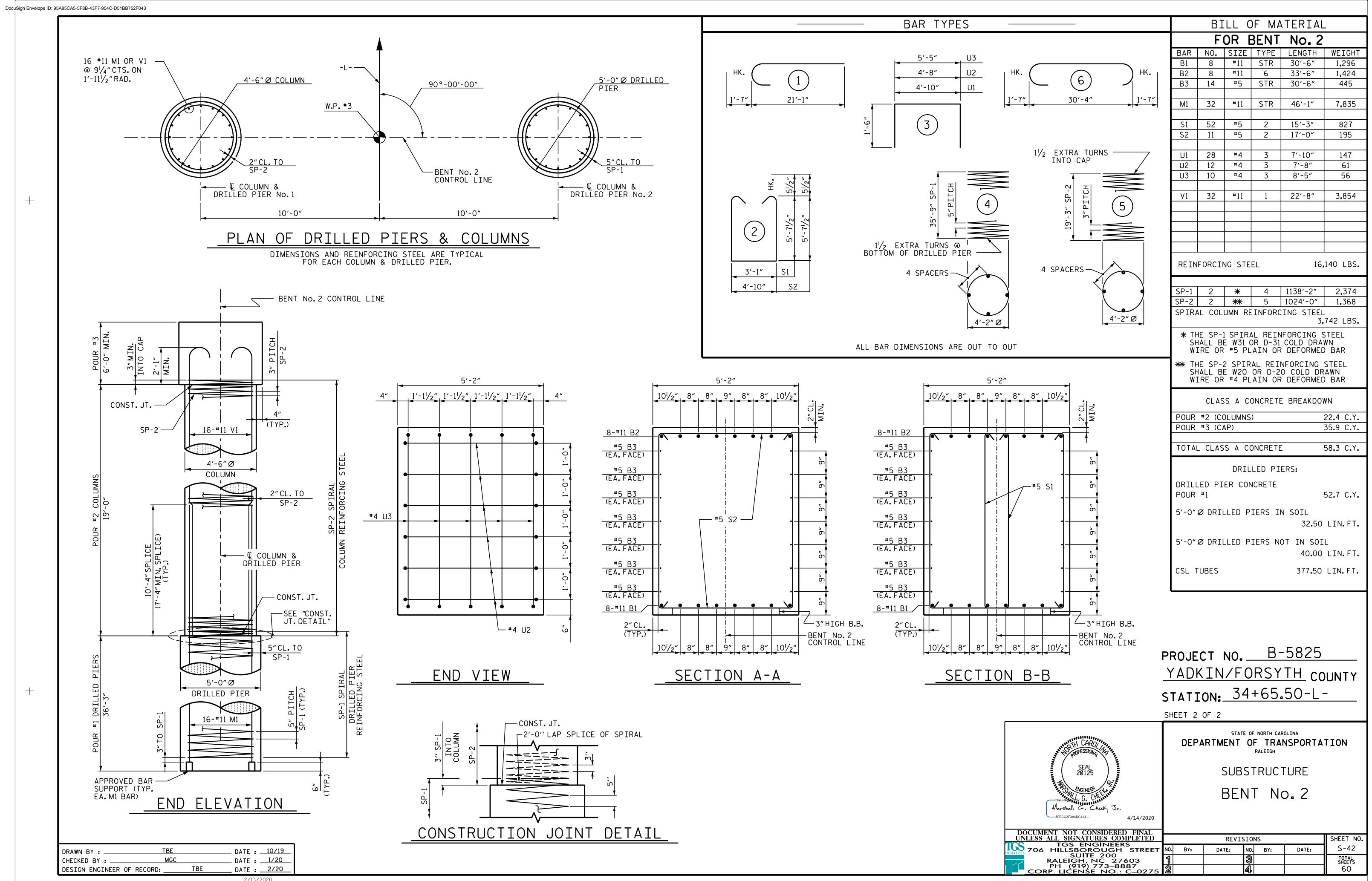
Q GDR. C4 —

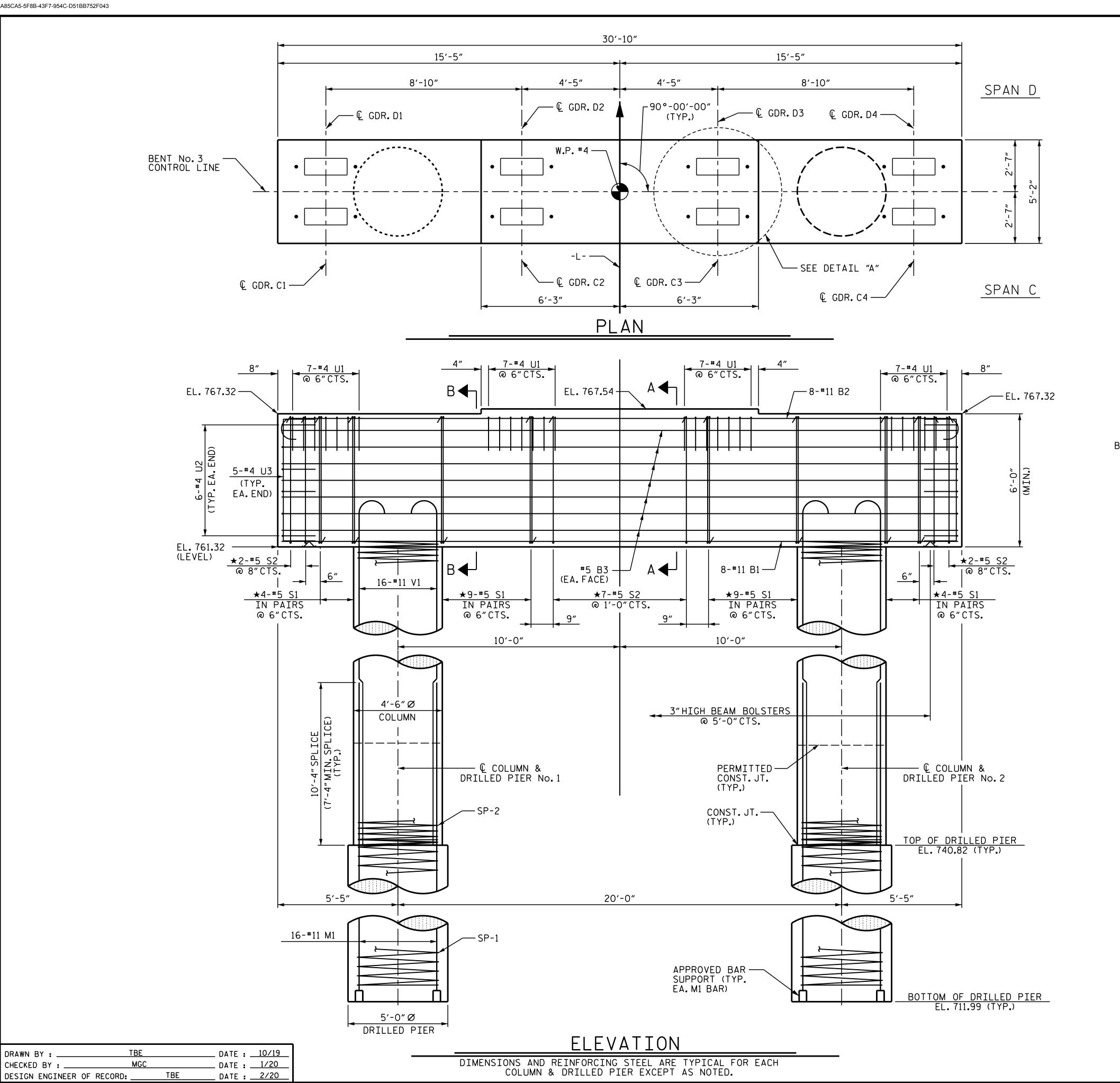
SPAN C

15'-5"

8'-10"

____ € GDR. C1





NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

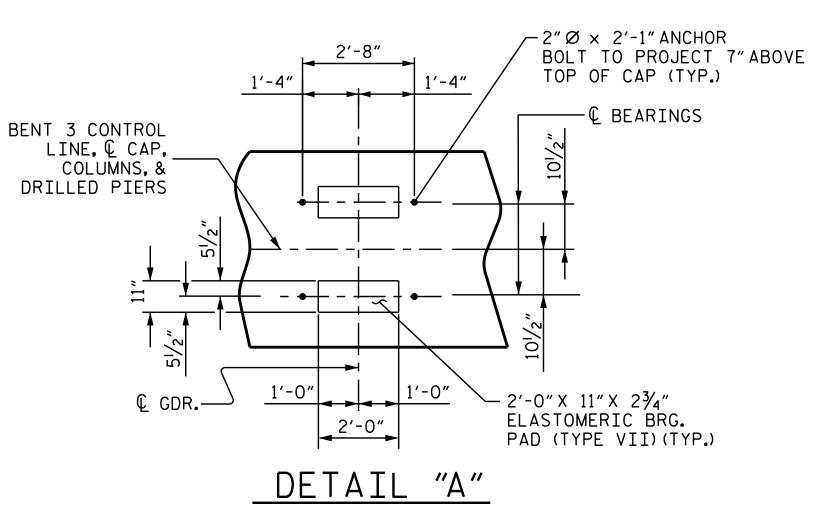
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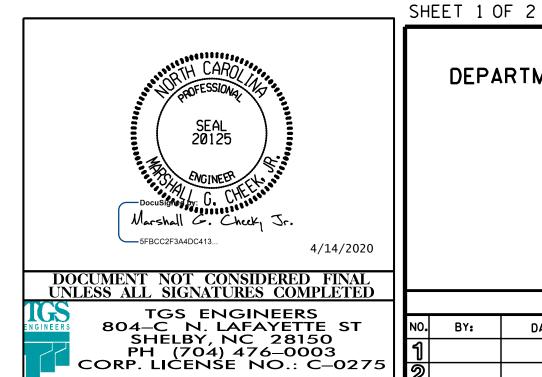
THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.

THE TOP SURFACE AREAS OF THE BENT CAP SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THAT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.



DIMENSIONS ARE TYPICAL FOR EACH BEARING.

PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50-L-

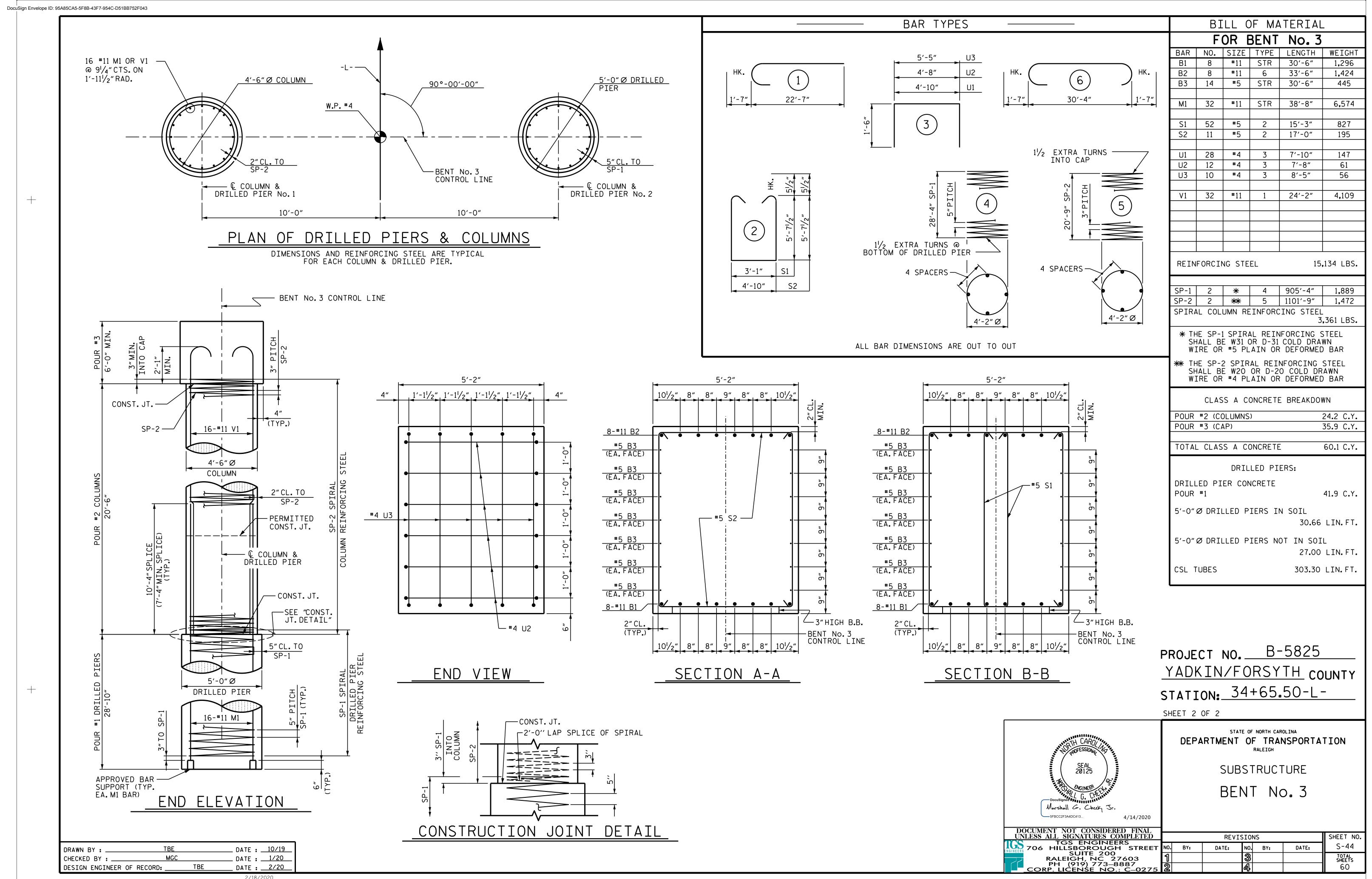


STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

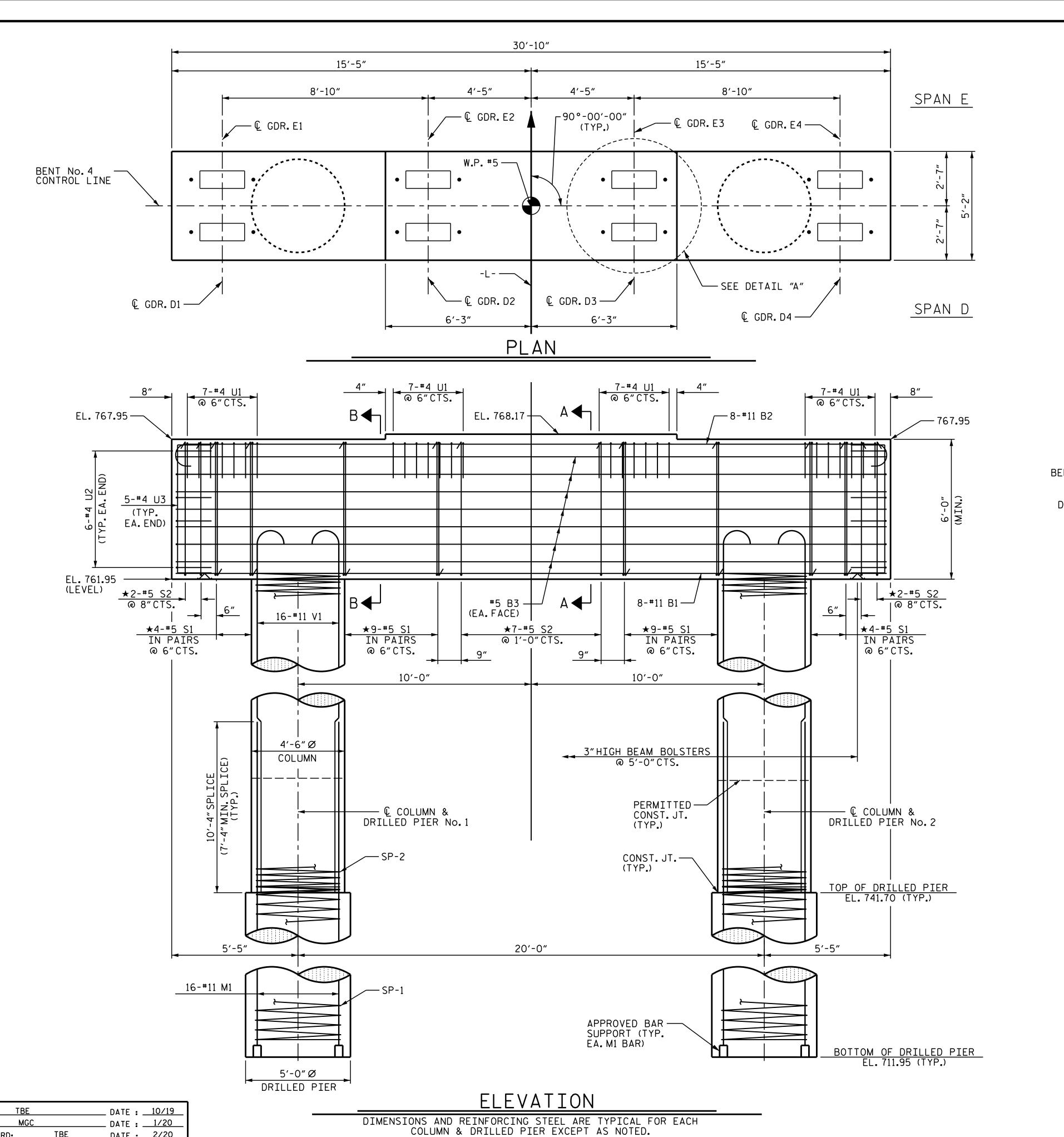
SUBSTRUCTURE

BENT No. 3

SHEET NO **REVISIONS** S-43 NO. BY: DATE: DATE: TOTAL SHEETS



DESIGN ENGINEER OF RECORD: _



NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

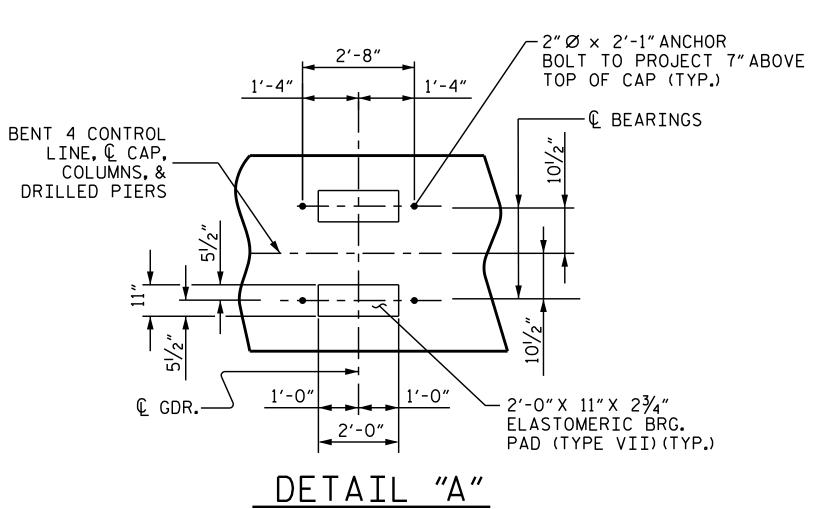
FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

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★ INVERT ALTERNATE STIRRUPS.

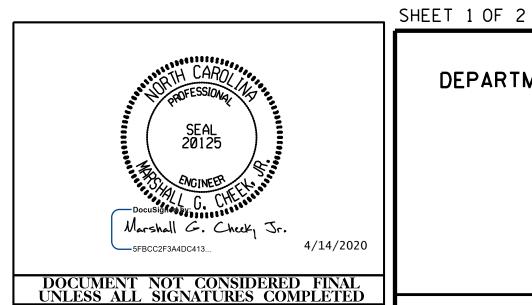
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DIMENSIONS ARE TYPICAL FOR EACH BEARING.

PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50-L-



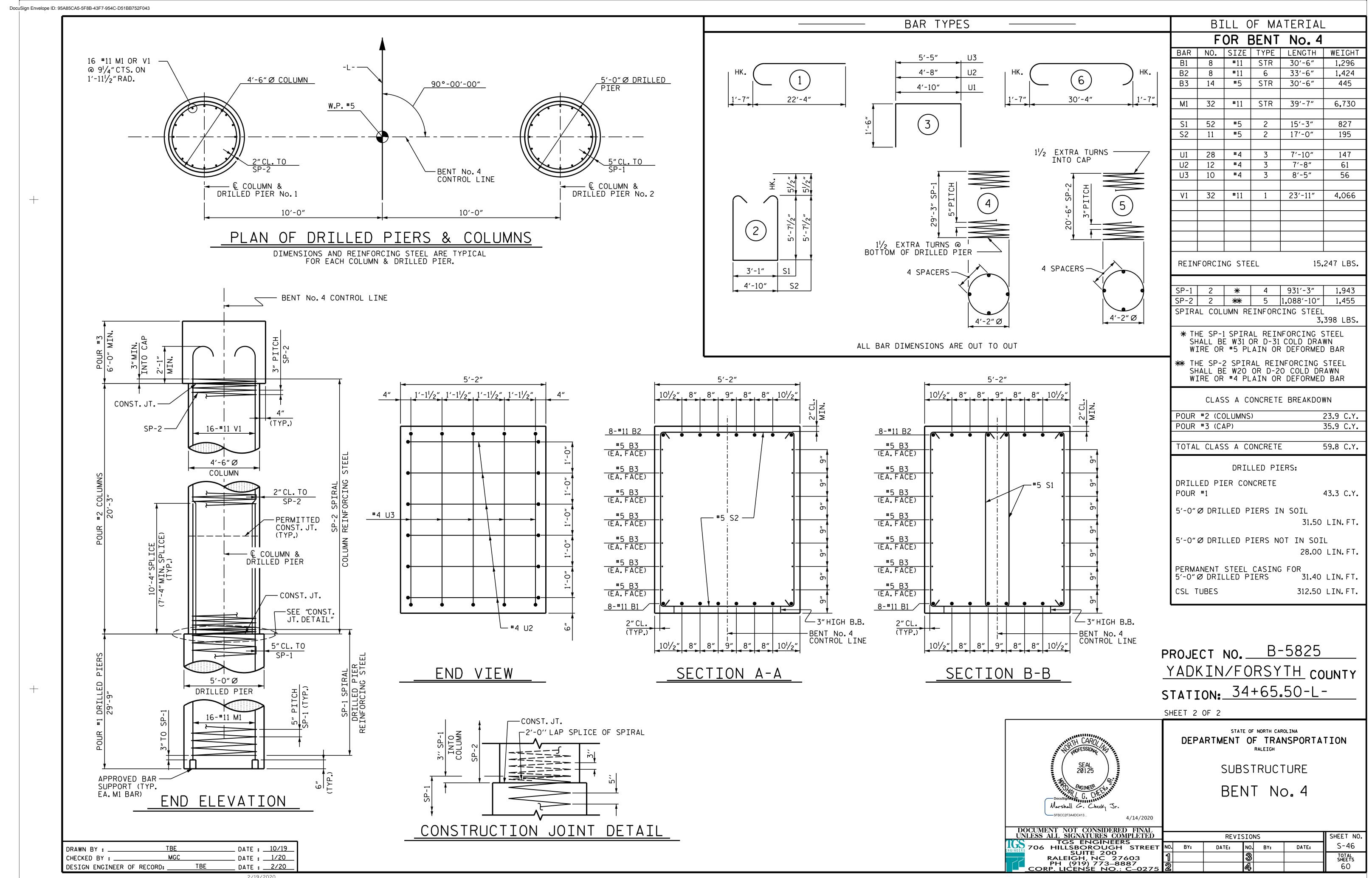
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

BENT No. 4

SHEET NO **REVISIONS** TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275 S-45 NO. BY: DATE: DATE: TOTAL SHEETS

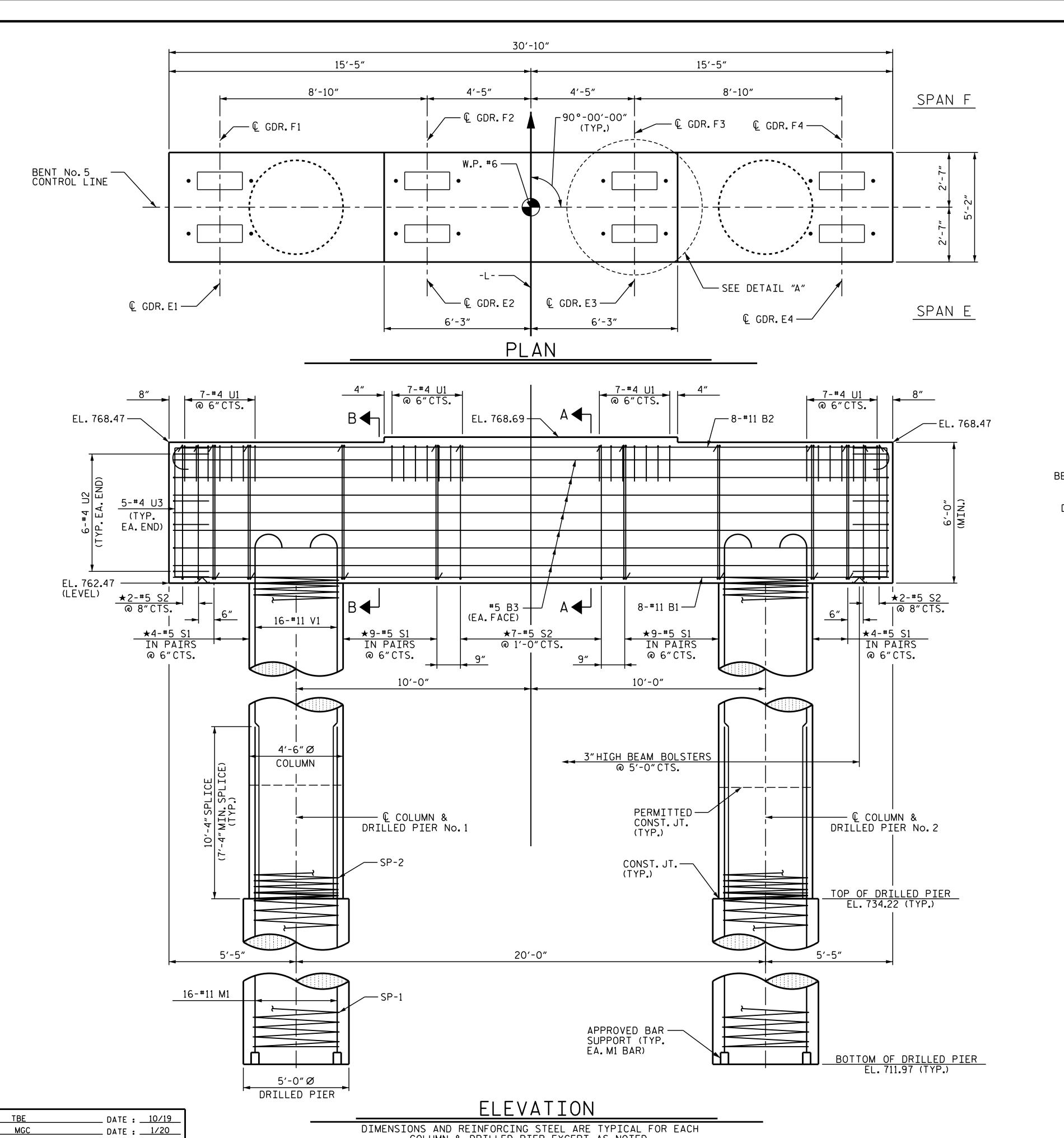
_ DATE : ____2/20



DESIGN ENGINEER OF RECORD: _

MGC

_ DATE : ____2/20



DIMENSIONS AND REINFORCING STEEL ARE TYPICAL FOR EACH COLUMN & DRILLED PIER EXCEPT AS NOTED.

NOTES

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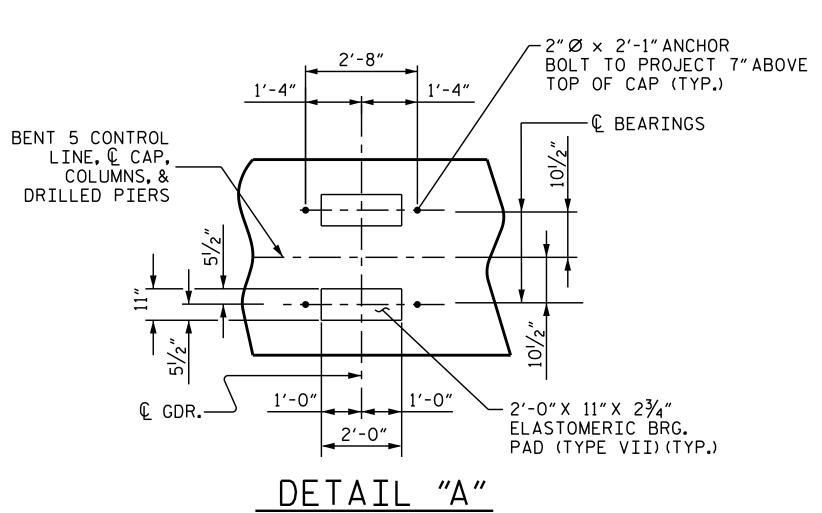
FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

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★ INVERT ALTERNATE STIRRUPS.

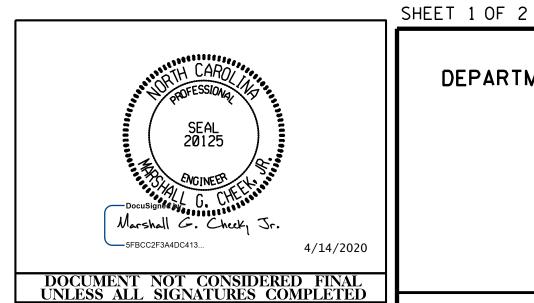
DRILLED PIERS SHALL BE TERMINATED ONE FOOT ± ABOVE WATER SURFACE FOR SHAFTS LOCATED IN WATER.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.



DIMENSIONS ARE TYPICAL FOR EACH BEARING.

PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50-L-

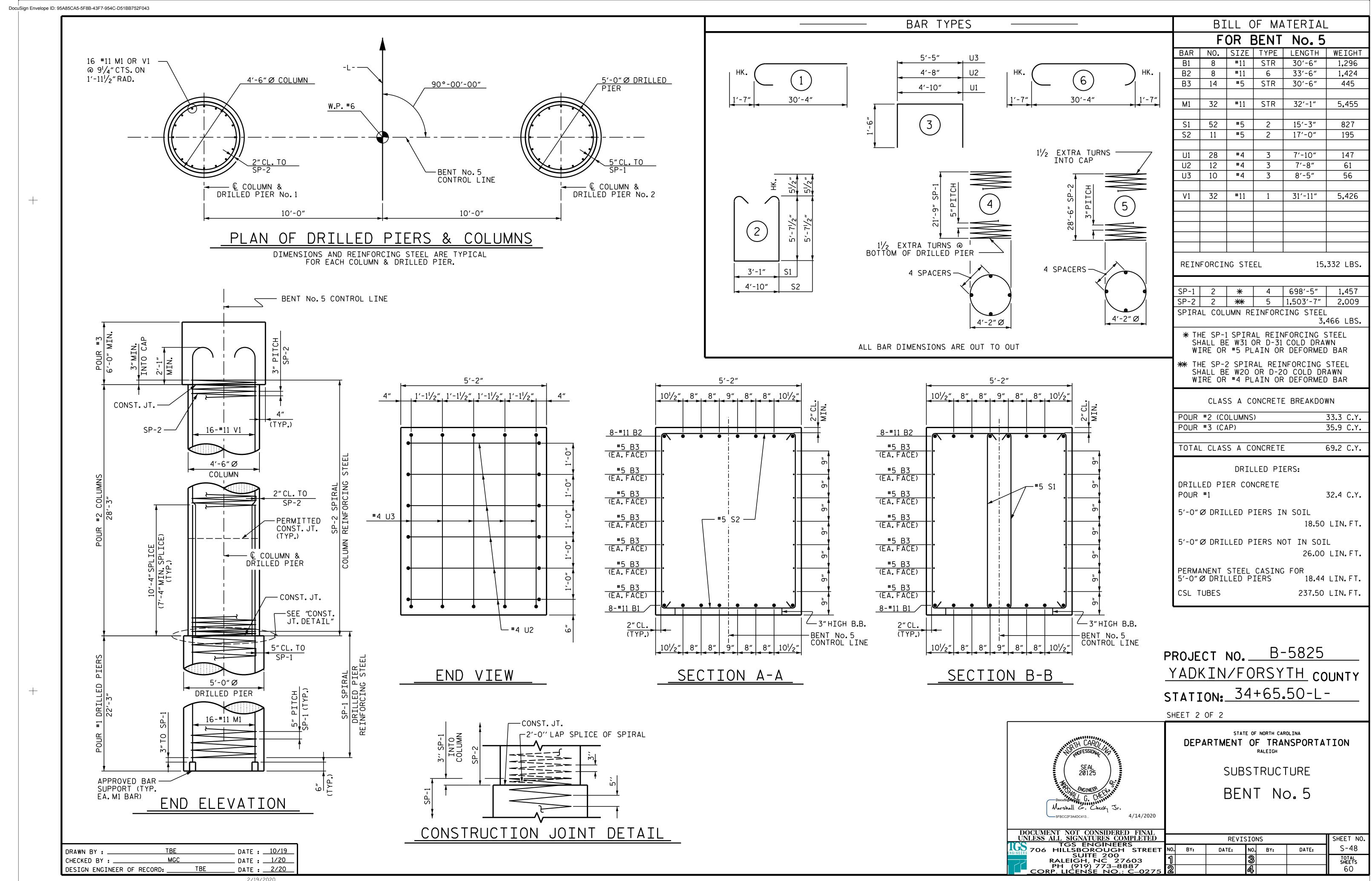


STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

BENT No. 5

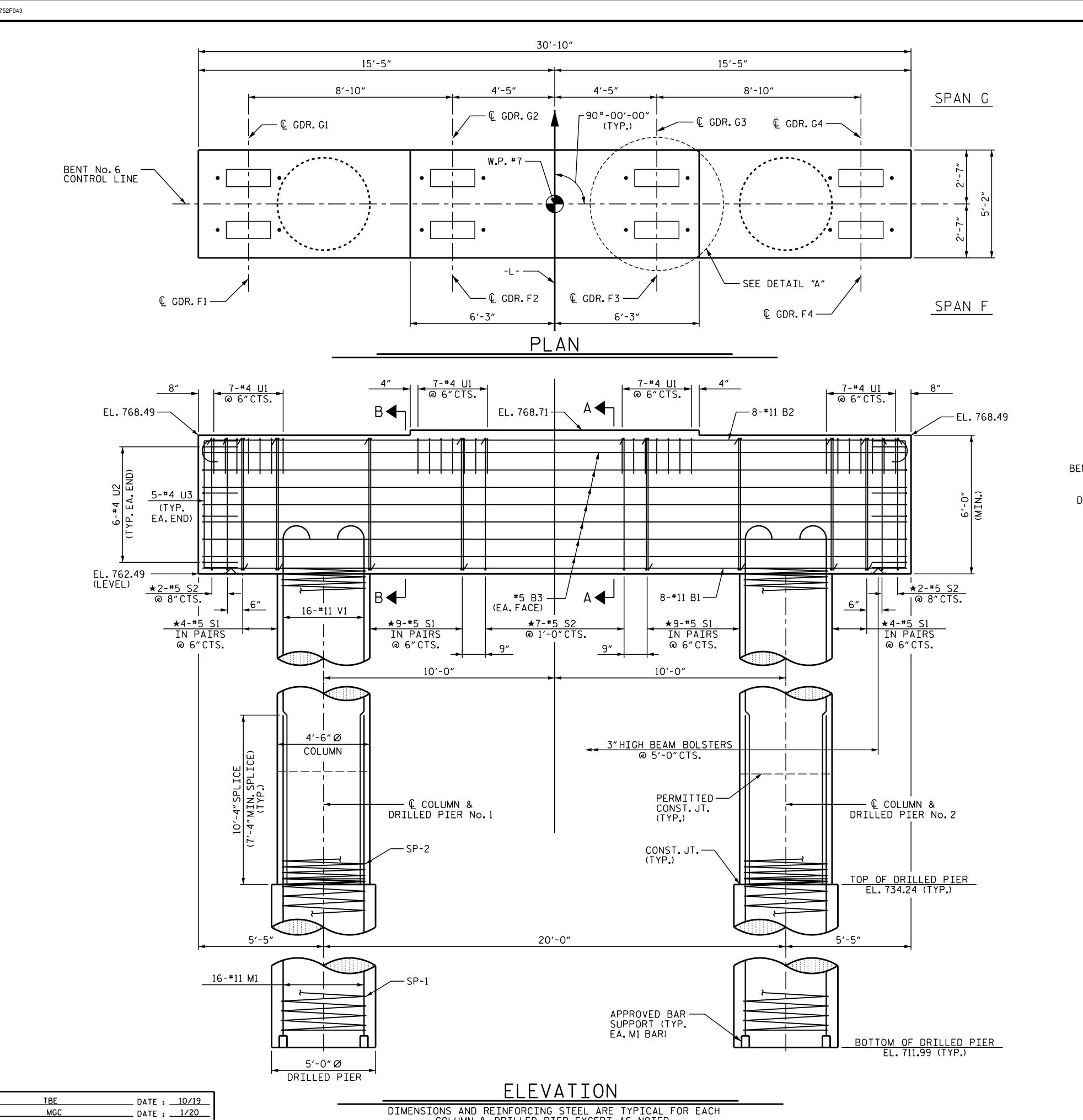
SHEET NO REVISIONS TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275 NO. BY: S-47 DATE: DATE: TOTAL SHEETS 60



DESIGN ENGINEER OF RECORD: _

MGC

_ DATE : ____2/20



DIMENSIONS AND REINFORCING STEEL ARE TYPICAL FOR EACH COLUMN & DRILLED PIER EXCEPT AS NOTED.

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

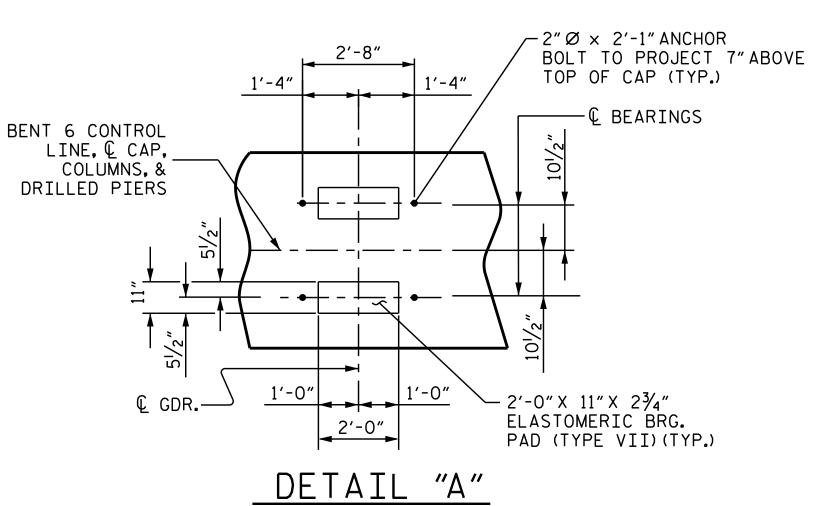
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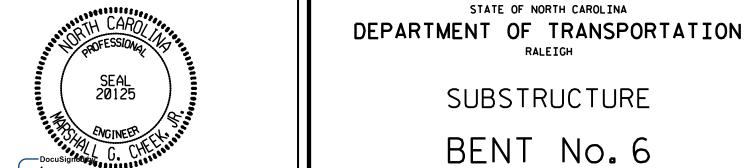
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DIMENSIONS ARE TYPICAL FOR EACH BEARING.

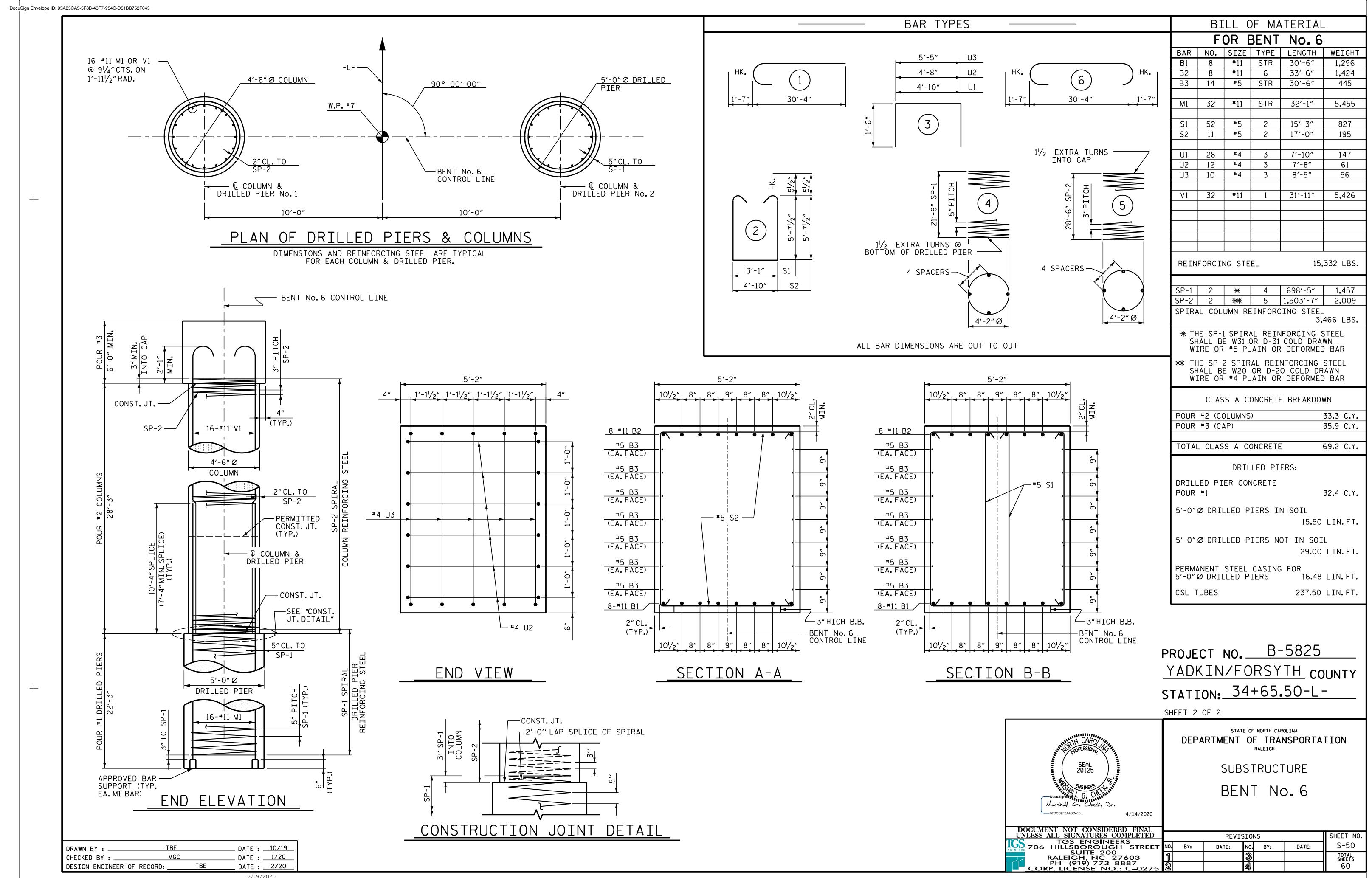
PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50-L-

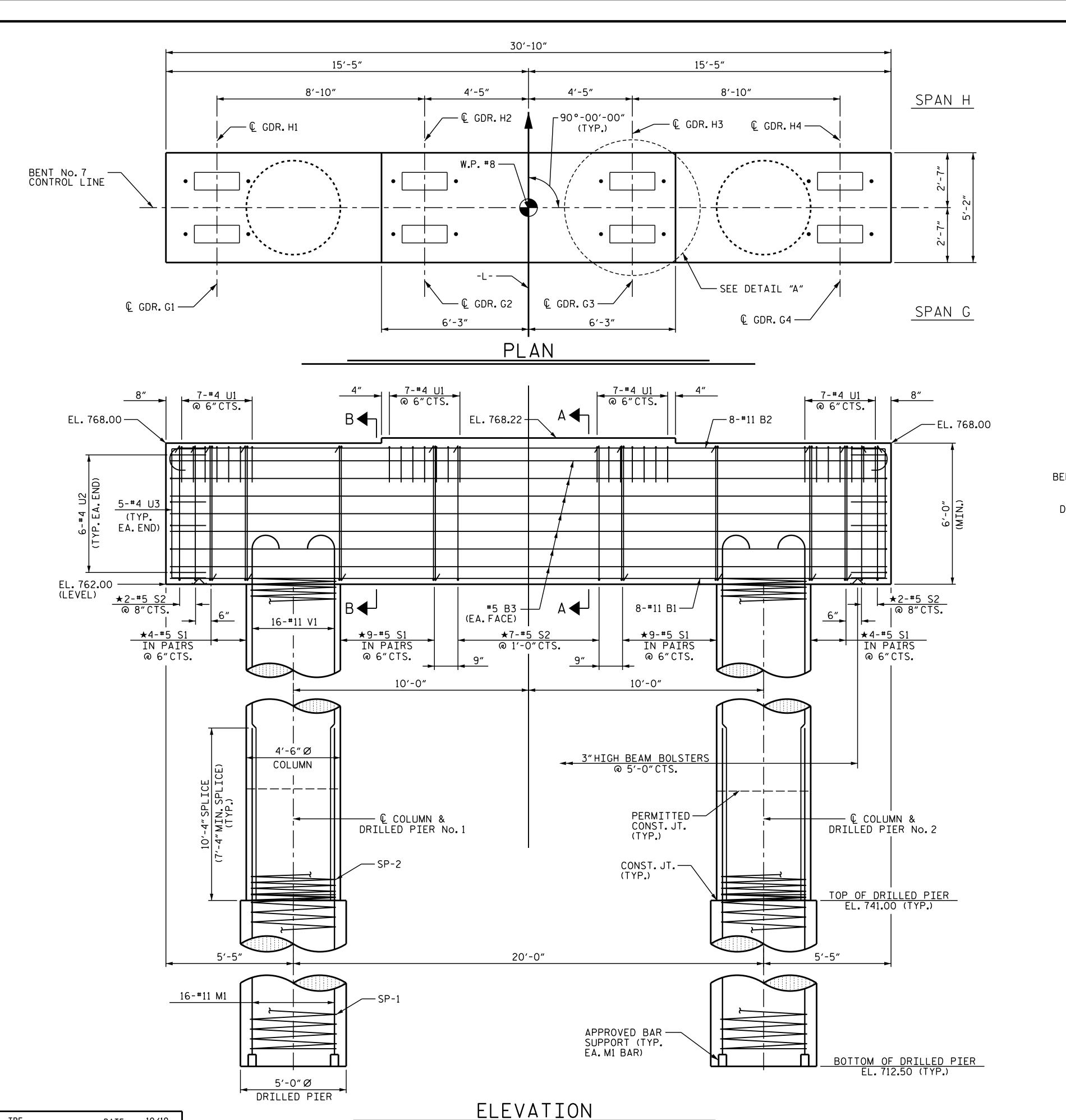




DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED SHEET NO. **REVISIONS** TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275 S-49 NO. BY: DATE: DATE: TOTAL SHEETS

2/18/2020 X:\NCDOT\B-5825\Structures\Str. #1 - 34+65.50 -L-\FinalPlans\DGNs\401_101_B-5825_SMU_ B601_980035.dgn User:ZSmith





DIMENSIONS AND REINFORCING STEEL ARE TYPICAL FOR EACH COLUMN & DRILLED PIER EXCEPT AS NOTED.

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

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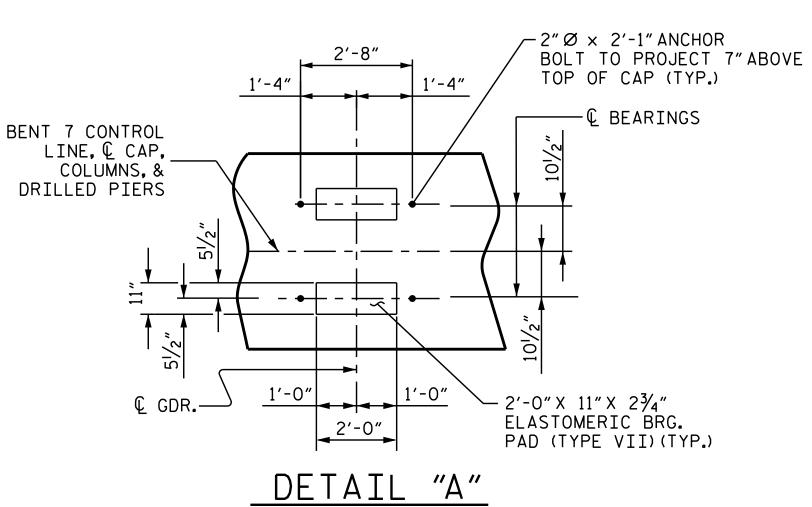
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DIMENSIONS ARE TYPICAL FOR EACH BEARING.

PROJECT NO. B-5825

YADKIN/FORSYTH COUNTY

STATION: 34+65.50-L-



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

BENT No. 7

COMENT NOT CONSIDERED FINAL NLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
804—C N. LAFAYETTE ST
SHELBY, NC 28150
PH (704) 476—0003
CORP. LICENSE NO.: C—0275

TOTAL SHEETS
60

SHEET 1 OF 2

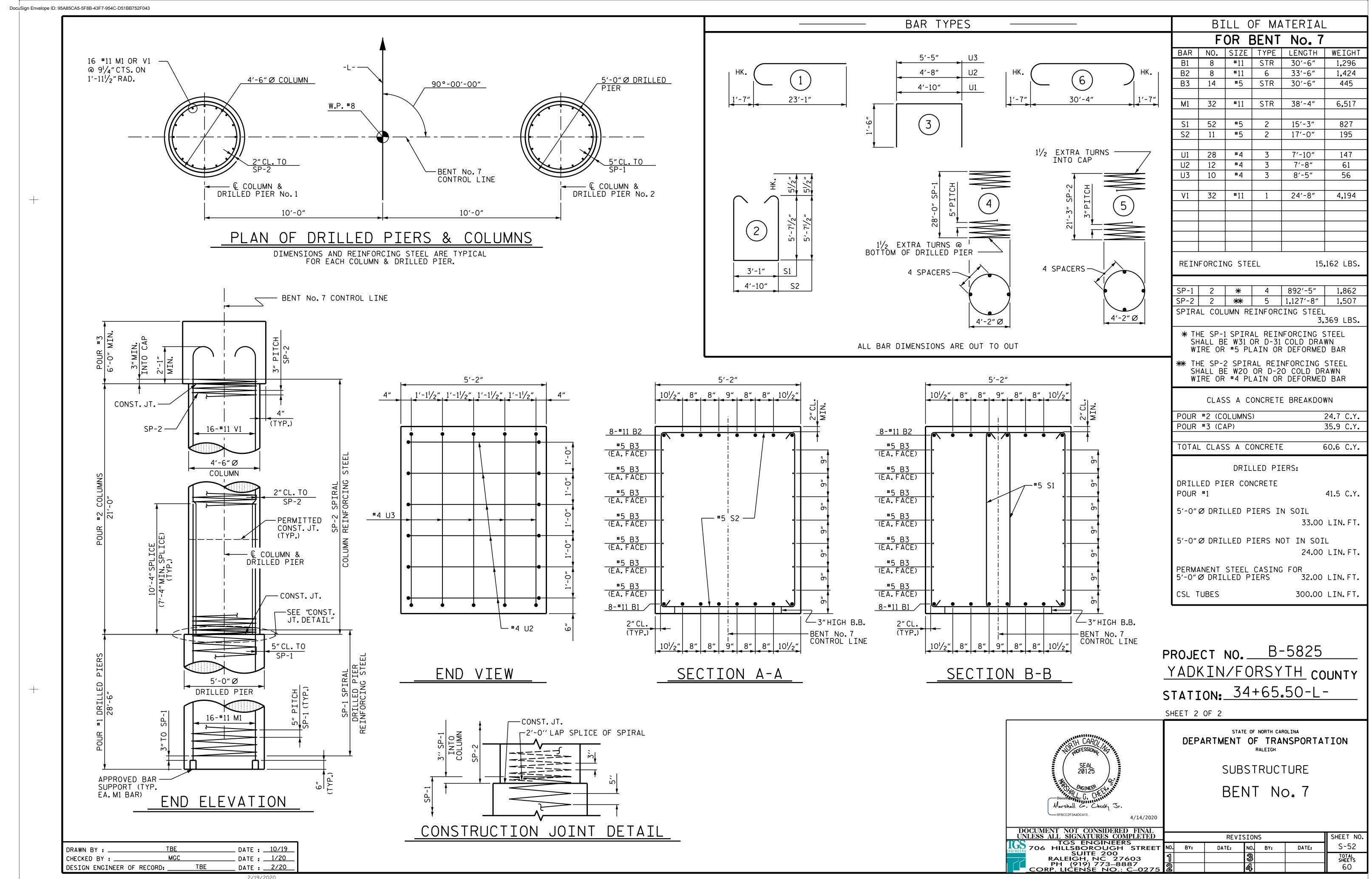
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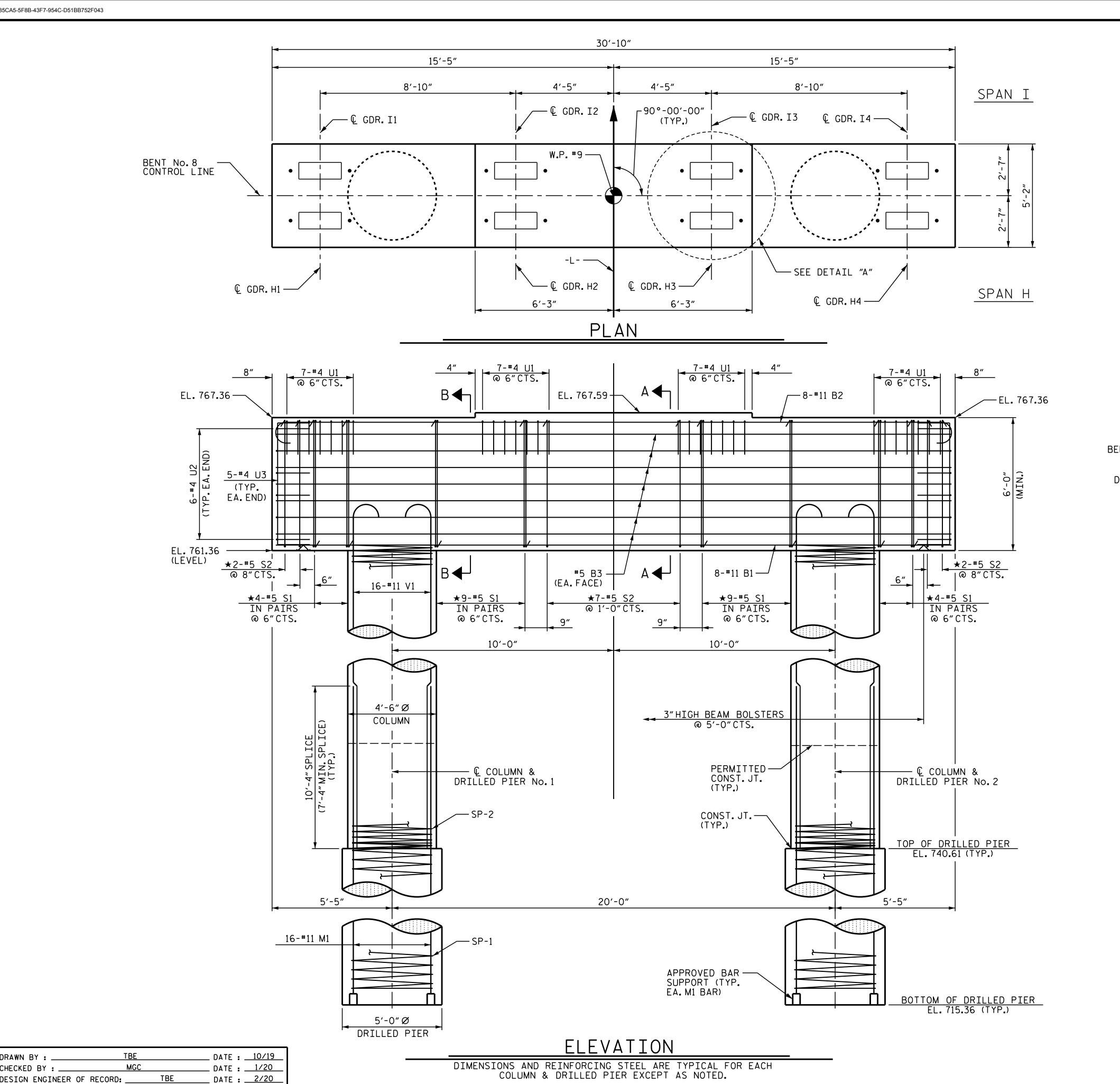
_ DATE : ____2/20

MGC

DRAWN BY :

DESIGN ENGINEER OF RECORD: _





NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

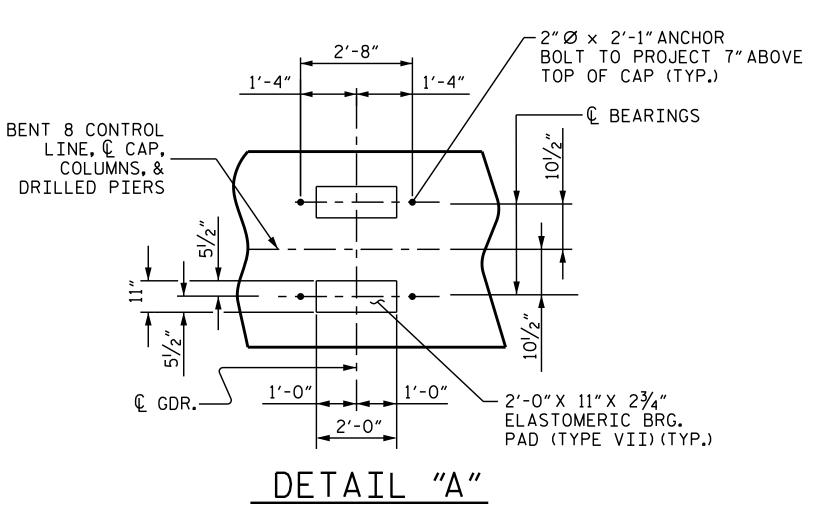
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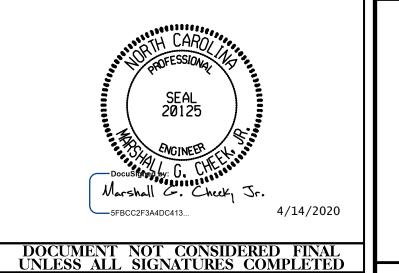
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DIMENSIONS ARE TYPICAL FOR EACH BEARING.

PROJECT NO. B-5825 YADKIN/FORSYTH COUNTY STATION: 34+65.50-L-





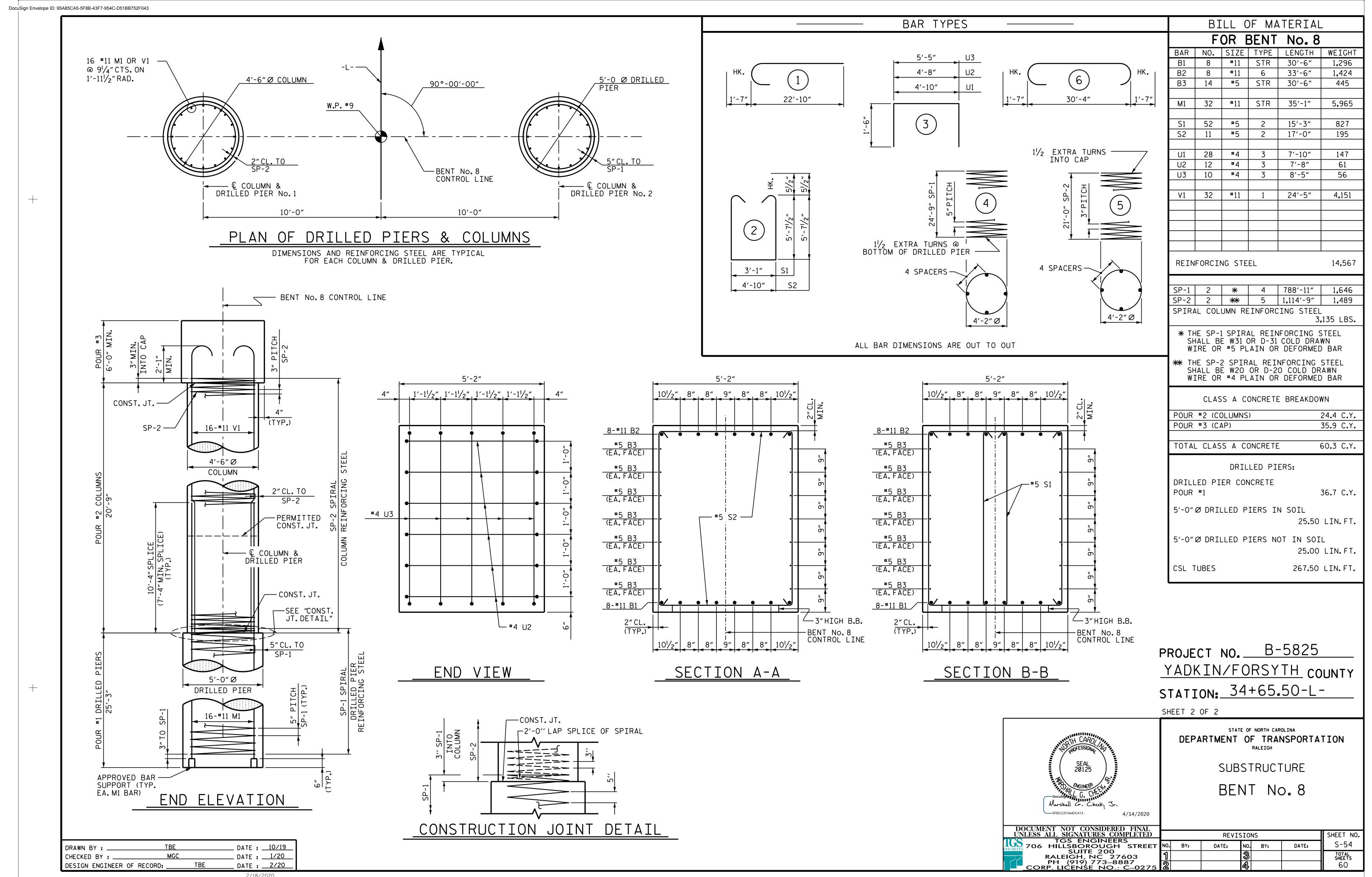
TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275

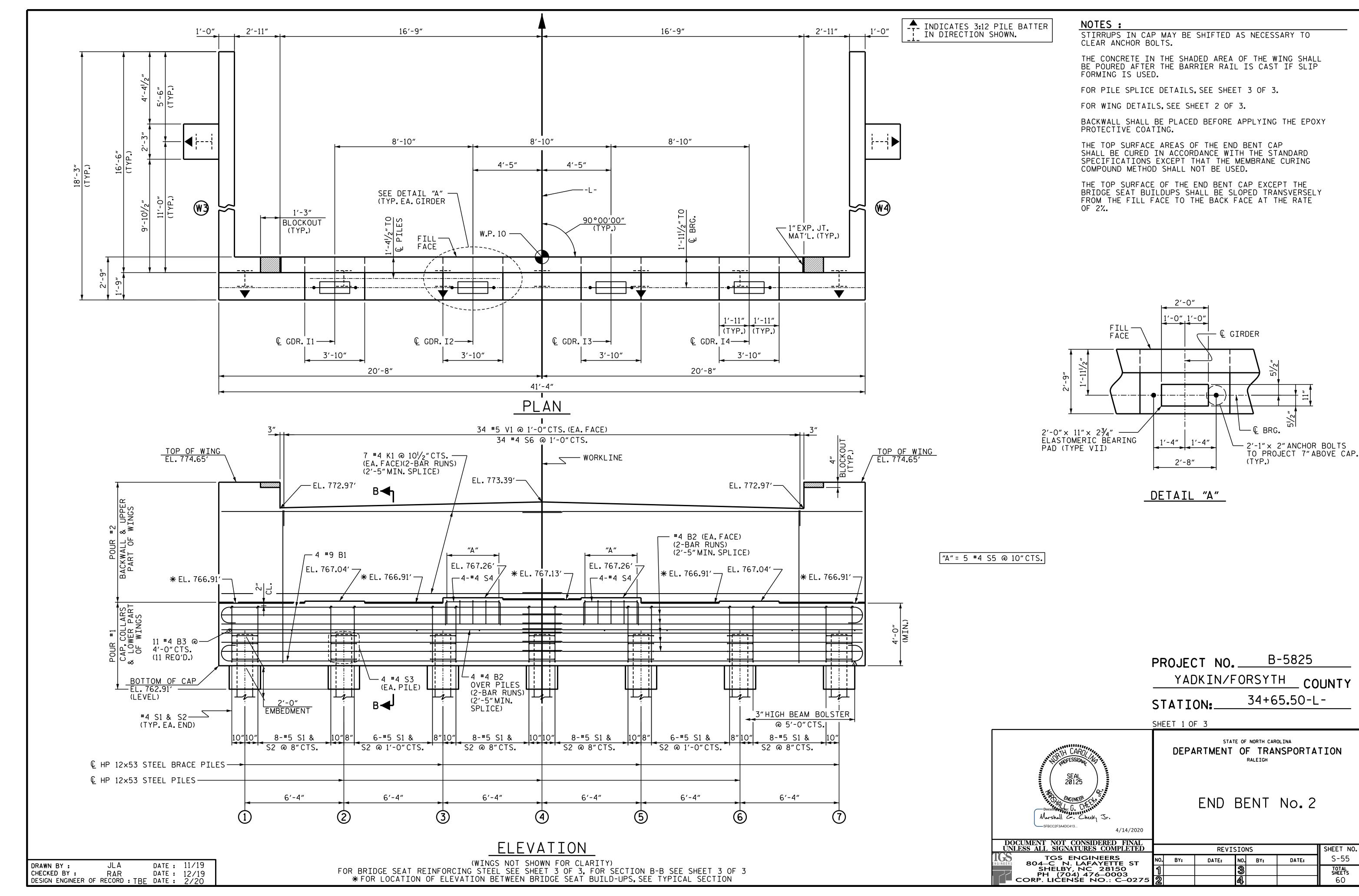
SUBSTRUCTURE

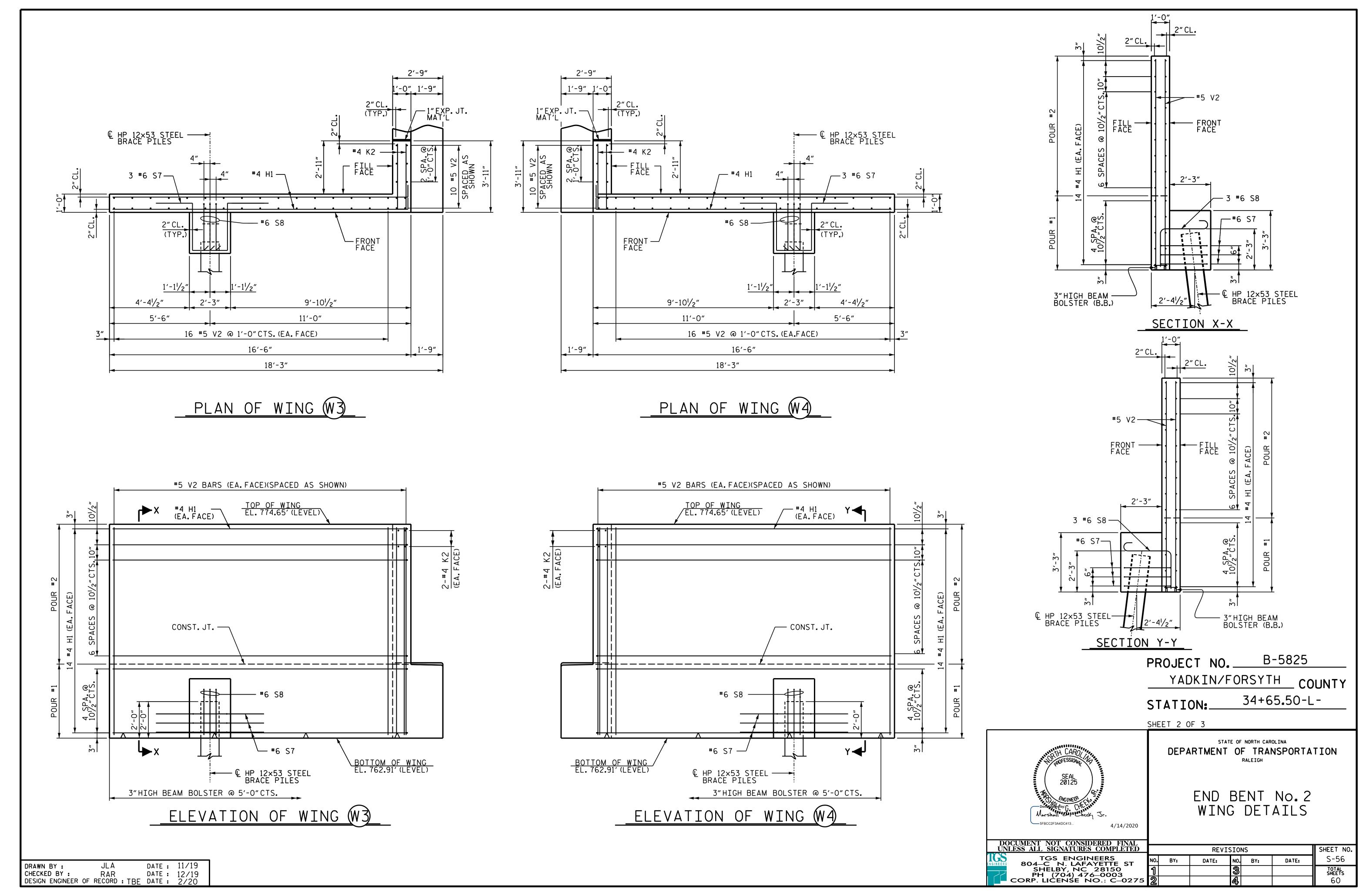
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

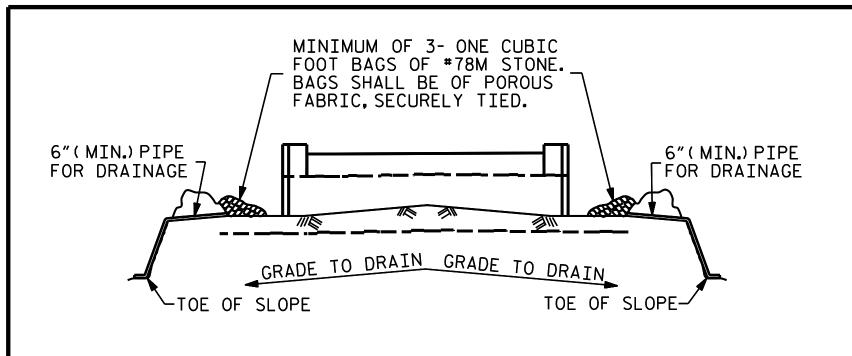
BENT No. 8

SHEET NO **REVISIONS** S-53 NO. BY: DATE: DATE: TOTAL SHEETS 60







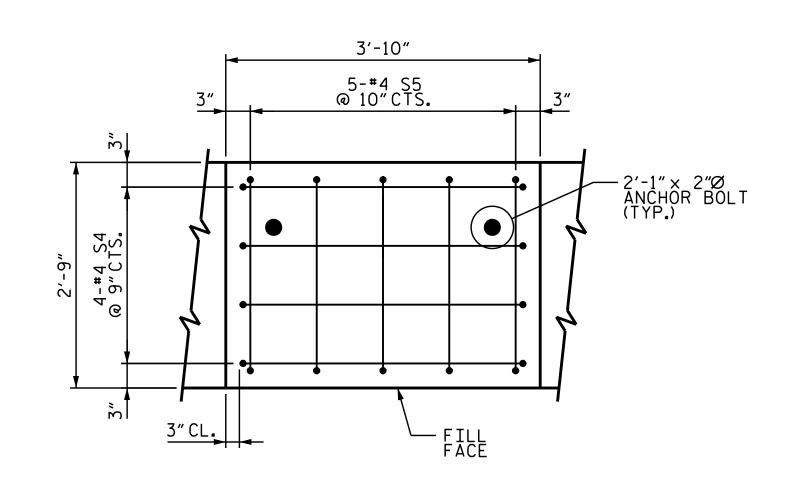


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

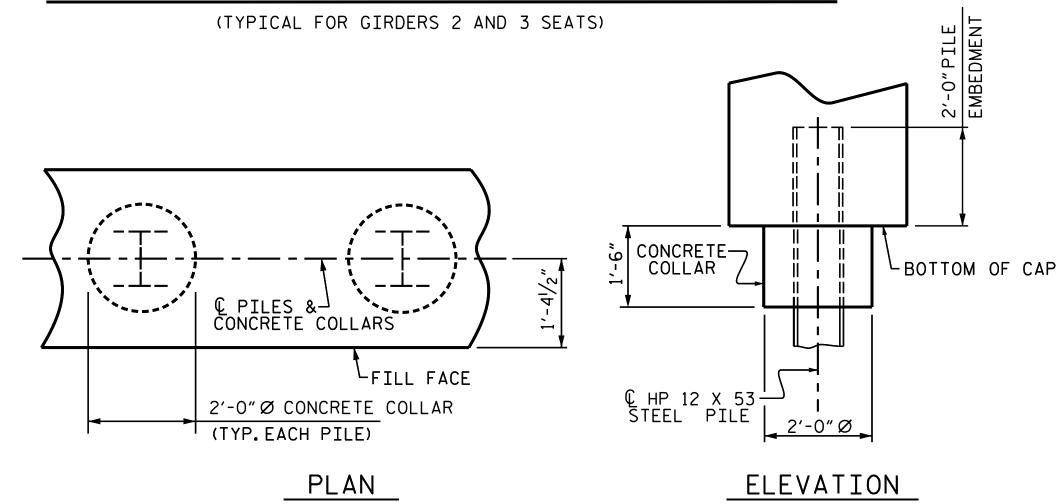
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TEMPORARY DRAINAGE AT END BENT

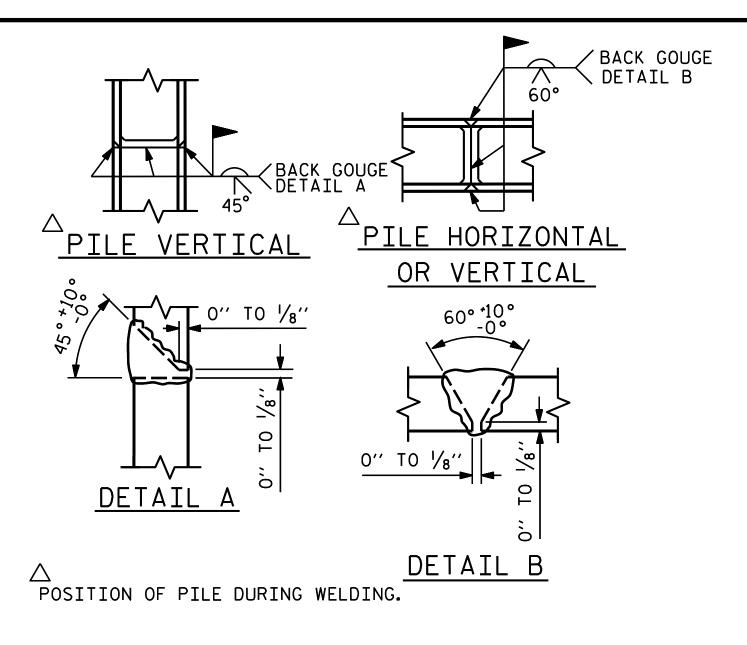


BRIDGE SEAT REINFORCING DETAIL

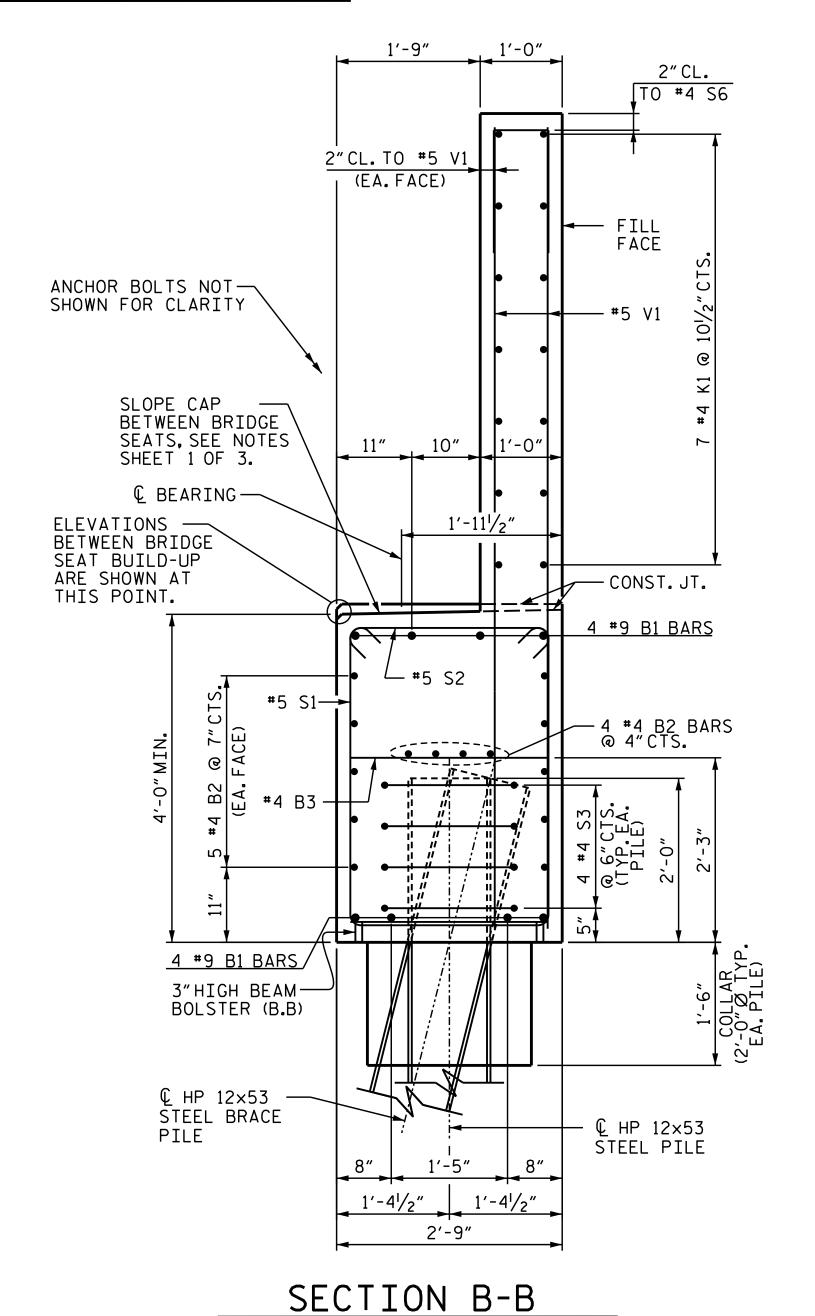


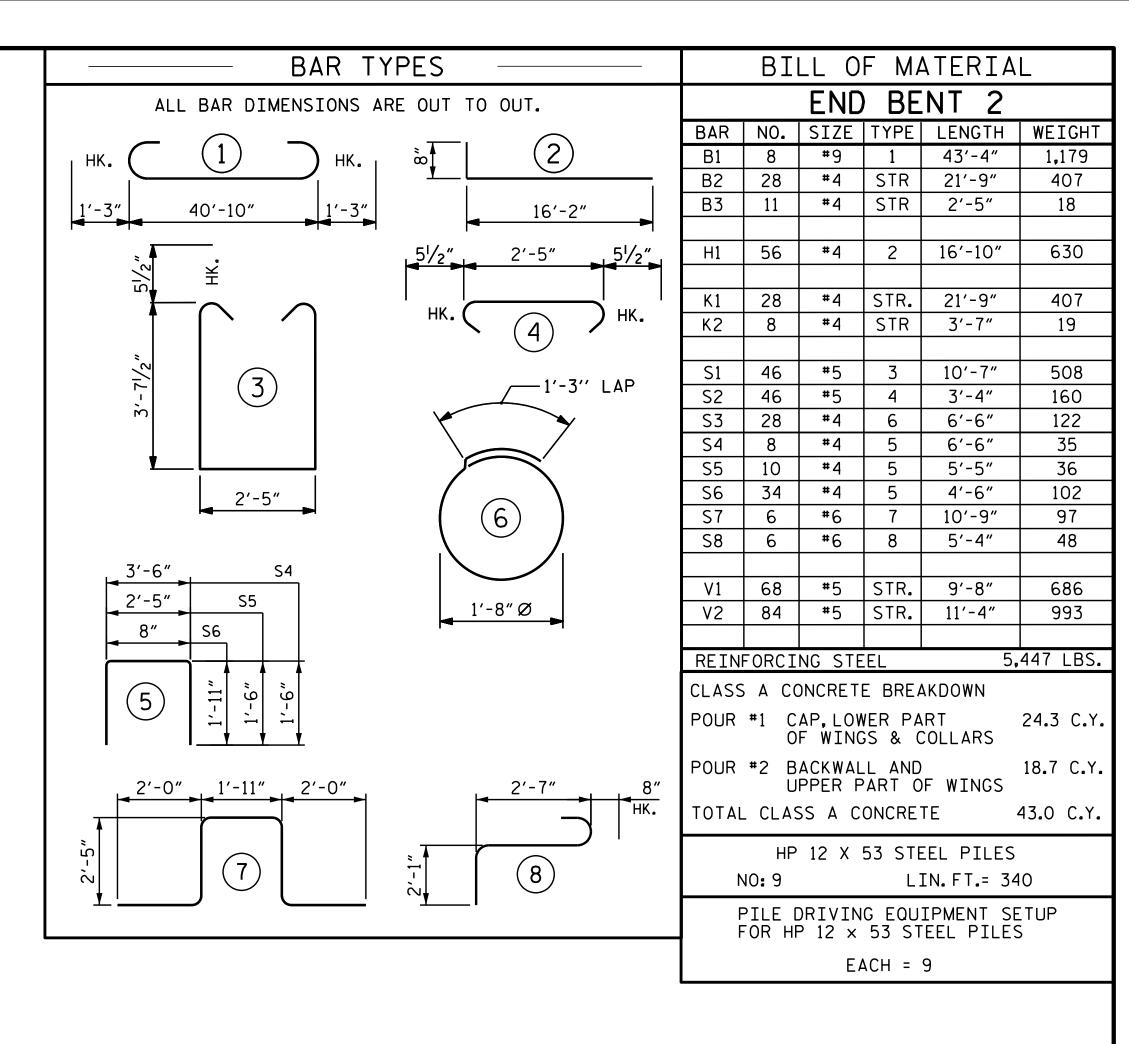
CORROSION PROTECTION FOR STEEL PILES DETAIL

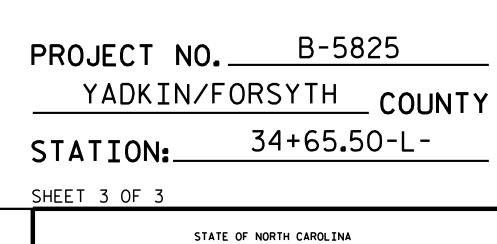
DRAWN BY: JLA DATE: 11/19
CHECKED BY: RAR DATE: 12/19
DESIGN ENGINEER OF RECORD: TBE DATE: 2/20



PILE SPLICE DETAILS









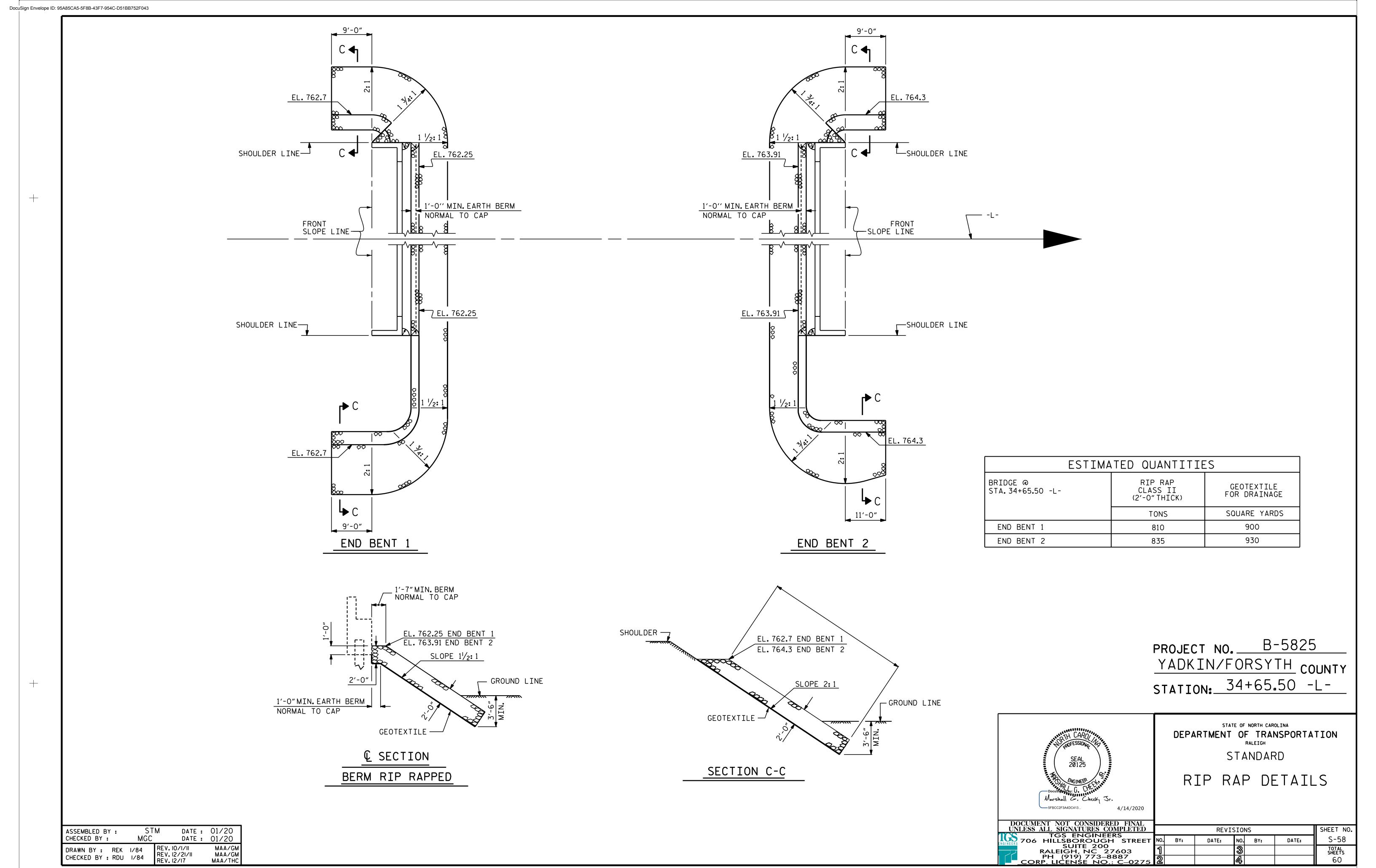
DEPARTMENT OF TRANSPORTATION
RALEIGH

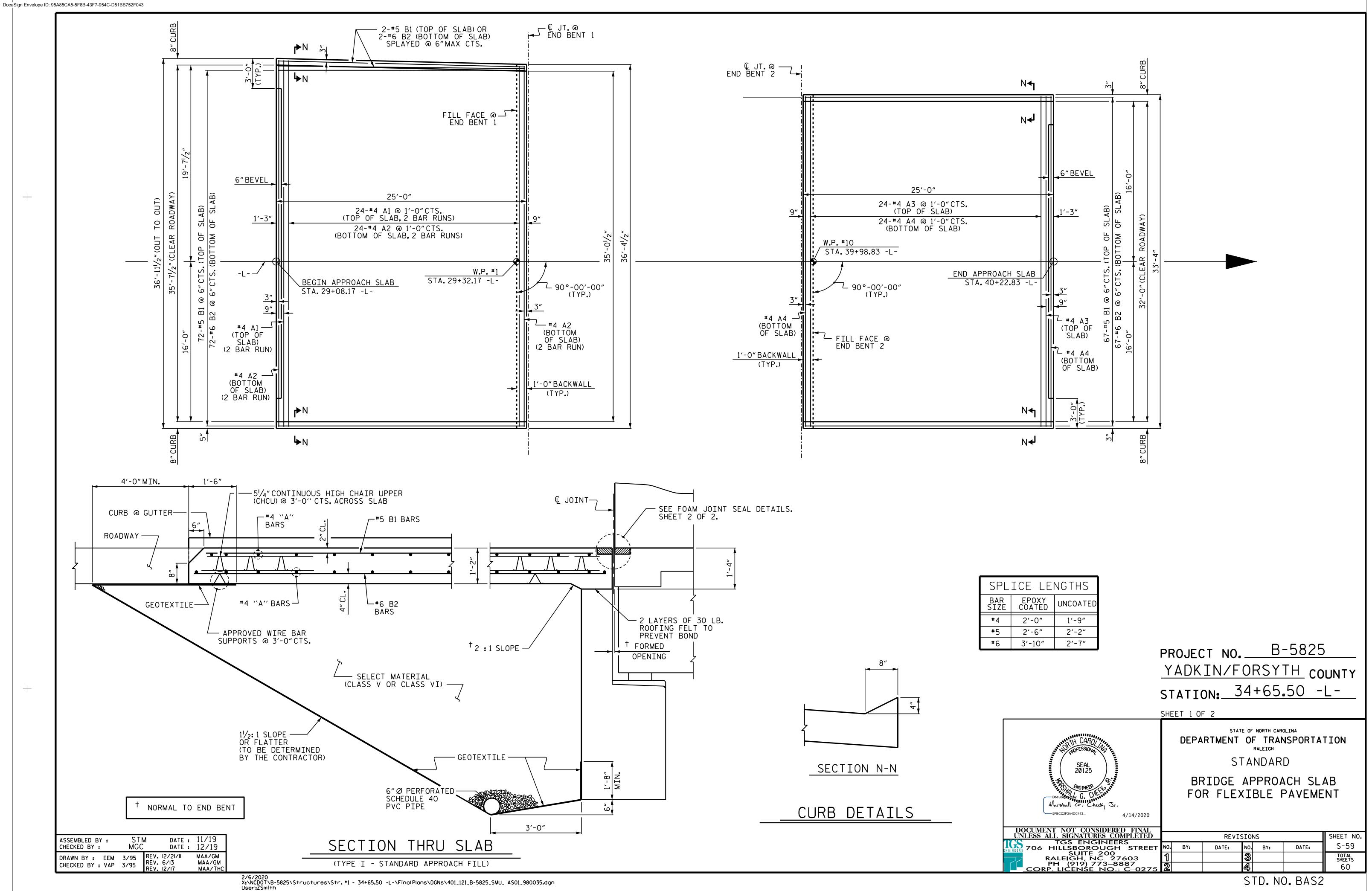
END BENT No. 2 DETAILS

DOCUMENT NOT CONSIDERED FINAL			
UNLESS ALL SIGNATURES COMPLETED			
TGS ENGINEERS 804-C N. LAFAYETTE ST	NO.	BY:	C
SHELBY, NC 28150	1		
PH (704) 476-0003 CORP. LICENSE NO.: C-0275	2		

DETAILS

	SHEET NO			
١٥.	BY:	S-57		
1		3		TOTAL SHEETS
୭୮		A		60

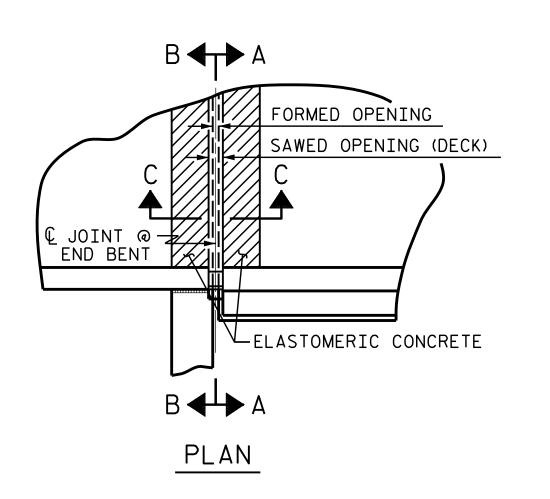


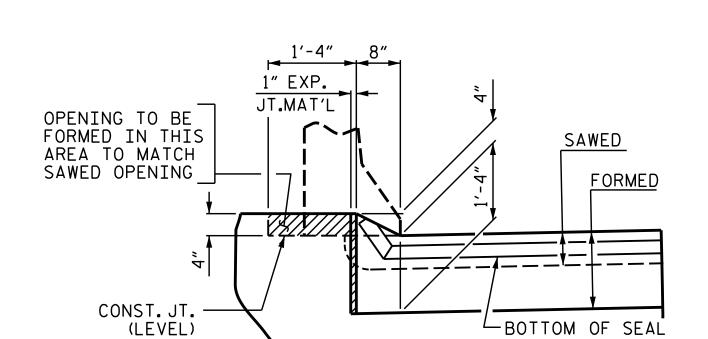


+

ELASTOMERIC CONCRETE ELASTOMERIC * **BENT** (CU.FT.) NO. 6.02 5.50 11.52 TOTAL

* BASED ON THE MINIMUM BLOCKOUT SHOWN.





SECTION B-B

BOTTOM OF SEAL

JOINT SEAL DETAILS @ END BENT

FOAM JOINT SEAL TO BE CUT, HEAT WELDED AND TURNED UP PARALLEL TO SLOPED FACE OF THE BARRIER RAIL.

THE JOINT SHALL BE SAWED PRIOR TO THE CASTING OF THE BARRIER RAIL.

DATE: 11/19

DATE: 12/19

MAA/GM

MAA/GM

MAA/THO

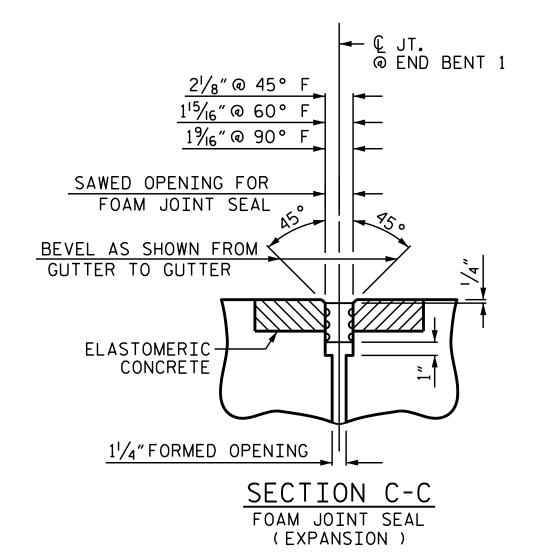
STM

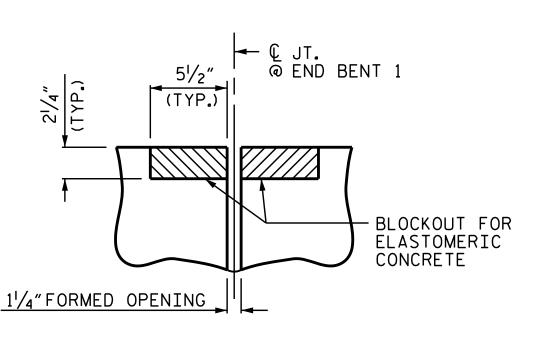
MGC

DRAWN BY: FCJ 11/88 REV. 7/12 REV. 6/13 REV. 12/17

ASSEMBLED BY:

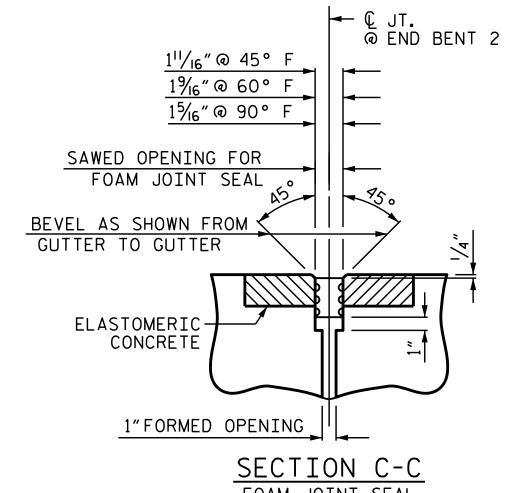
CHECKED BY :



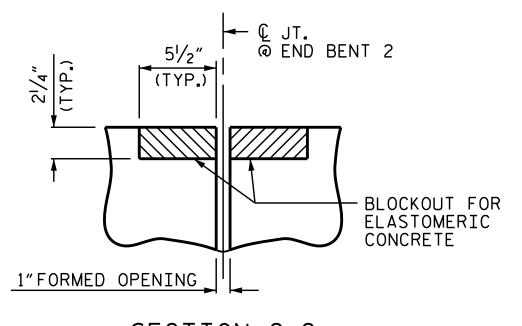


SECTION C-C FOAM JOINT SEAL (PRE-SAWED ELASTOMERIC CONCRETE DIMENSIONS)

END BENT :

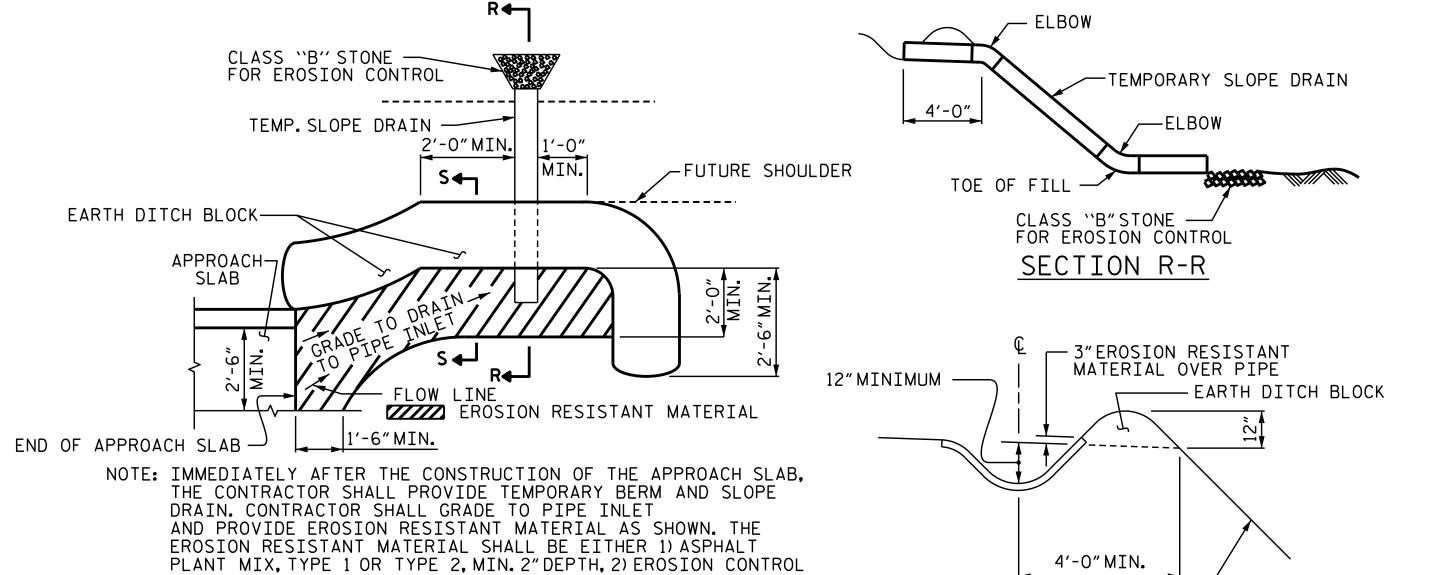


FOAM JOINT SEAL (EXPANSION)



SECTION C-C FOAM JOINT SEAL (PRE-SAWED ELASTOMERIC CONCRETE DIMENSIONS)

END BENT 2



PLAN VIEW

MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED

TEMPORARY DRAINAGE PIPE. 12 INCHES IN DIAMETER.

<u>TEMPORARY BERM AND SLOPE DRAIN DETAILS</u>

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 6" Ø DRAINAGE PIPE, AND SELECT MATERIAL BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR THE 6" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

FOR FOAM JOINT SEALS, SEE SPECIAL PROVISIONS.

FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.

THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE FOAM JOINT SEAL AT END BENT 1 SHALL BE 21/2".

THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE FOAM JOINT SEAL AT END BENT 2 SHALL BE 2".

APPROACH SLAB AT BENT 1								
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
* A1	50	#4	STR	19'-4"	646			
A2	52	#4	STR	19'-3"	669			
∗ B1	74	#5	STR	23′-9″	1833			
B2	74	#6	STR	24'-8"	2742			

BILL OF MATERIAL

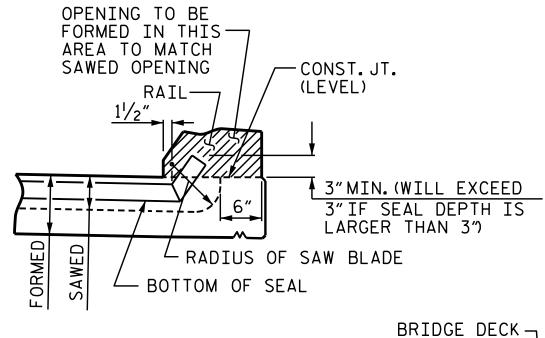
REINFORCING STEEL 3411 LBS. * EPOXY COATED REINFORCING STEEL 2479 LBS

CLASS AA CONCRETE 39.3 C.Y APPROACH SLAB AT BENT 2 BAR | NO. | SIZE | TYPE | LENGTH WEIGHT **★** A3 | 25 #4 | STR | 33'-0" 551 A4 26 STR 573 #4 33'-0" 1635 B2 | 66 #6 STR 24'-8" 2445

REINFORCING STEEL 3018 LBS. * EPOXY COATED REINFORCING STEEL 2186 LBS

36.0 C.Y

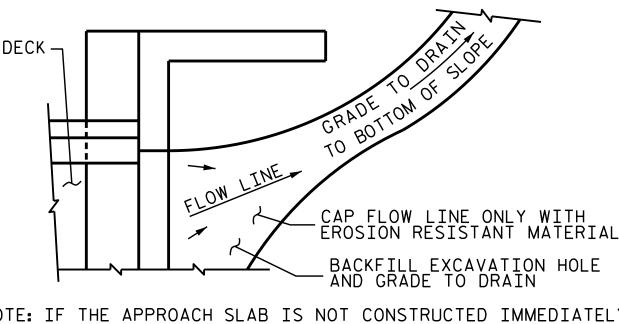
CLASS AA CONCRETE



SECTION A-A

/_ FILL SLOPE

SECTION S-S



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION. GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

B-5825 PROJECT NO._ YADKIN/FORSYTH COUNTY STATION: 34+65.50 -L-

SHEET 2 OF 2



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

BRIDGE APPROACH SLAB DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TGS ENGINEERS
706 HILLSBOROUGH STREET NO. BY: DATE: SUITE 200

RALEIGH, NC 27603

PH (919) 773–8887

CORP. LICENSE NO.: C-0275

SHEET NO **REVISIONS** S-60 NO. DATE: BY: TOTAL SHEETS 60