

REFERENCE: I-5700

PROJECT: 50118

SEE SHEET 3 FOR PLAN SHEET LAYOUT  
AT TIME OF INVESTIGATION

**STATE OF NORTH CAROLINA**  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	I-5700	1	130

# ROADWAY SUBSURFACE INVESTIGATION

COUNTY WAKE  
PROJECT DESCRIPTION I-40 AND SR 3015 (AIRPORT  
BOULEVARD) REVISE INTERCHANGE AND  
CONSTRUCT AUXILIARY LANE ON I-40 WESTBOUND  
FROM SR 3015 (AIRPORT BOULEVARD) TO I-540

## INVENTORY

**CONTENTS**

LINE	STATION	PLAN	PROFILE
-L-	19+60.00 - 62+25.00	4-7	N/A
-RPA-	10+00.00 - 22+25.73	6	N/A
-RPASPUR-	10+00.00 - 12+06.55	6	N/A
-RPB-	10+00.00 - 23+93.68	6,10	N/A
-RPBSPUR-	10+00.00 - 14+09.31	6	N/A
-RPC-	10+00.00 - 22+86.11	6,10	N/A
-RPCSPUR-	20+40.66 - 22+80.65	6	N/A
-RPD-	10+00.00 - 26+69.97	6	N/A
-RPDSPUR-	10+00.00 - 14+78.07	6	N/A
-Y-	25+32.85 - 113+06.23	6,8-13	N/A
-Y1-	12+97.46 - 14+11.34	5	N/A
-Y2-	10+05.00 - 11+76.05	5	N/A
-Y3-	15+18.75 - 30+73.03	7,14	N/A
-Y5-	10+70.00 - 12+61.43	4	N/A
-Y8-	17+00.00 - 21+78.46	14	N/A
-NBL-	56+18.52 - 61+00.00	7	N/A
-NBLRPA-	12+29.11 - 17+47.62	7	N/A

**CROSS SECTIONS**

LINE	STATION	SHEETS
-L-	23+00.00 - 60+00.00	15-31
-RPA-	12+49.86 - 21+50.00	32-38
-RPASPUR-	10+65.00 - 12+29.71	39-41
-RPB-	11+00.00 - 22+00.00	42-49
-RPBSPUR-	10+42.93 - 13+16.41	50-51
-RPC-	11+99.81 - 22+00.00	52-57
-RPCSPUR-	21+55.00	58
-RPD-	14+60.45 - 25+60.00	59-65
-RPDSPUR-	13+00.00	66
-Y-	37+00 - 113+06.23	67-97
-Y1-	14+00.00	98
-Y2-	11+00.00	99
-Y3-	15+50.00 - 29+50.00	100-107
-Y5-	12+00	108
-Y8-	18+00 - 20+00	109-110

**APPENDICES**

APPENDIX	TITLE	SHEETS
A	LABORATORY RESULTS	III-130

**CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTES:

1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

C. DRISCOLL

TRIGON EXPLORATION

INVESTIGATED BY C. DRISCOLL

DRAWN BY C. DRISCOLL

CHECKED BY T. WELLS

SUBMITTED BY KLEINFELDER, INC.

DATE AUGUST 2018

Prepared in the Office of:



DocuSigned by:

Thomas Wells

8/27/2018

7DA5D2D0518F4B0

SIGNATURE

DATE

**DOCUMENT NOT CONSIDERED FINAL  
UNLESS ALL SIGNATURES COMPLETED**



09/28/99

23-AUG-2018 15:50 W:\share\GEO\TECHNICAL\Projects\Active Projects\2015\548-053A I-5700 Roadway\5700\_GEO\_RDWY\CADD\_GEO\TECH\PlanProj\I-5700\_geo\_psh01\_tsh.dgn

**CONTRACT:** TIP PROJECT: I-5700

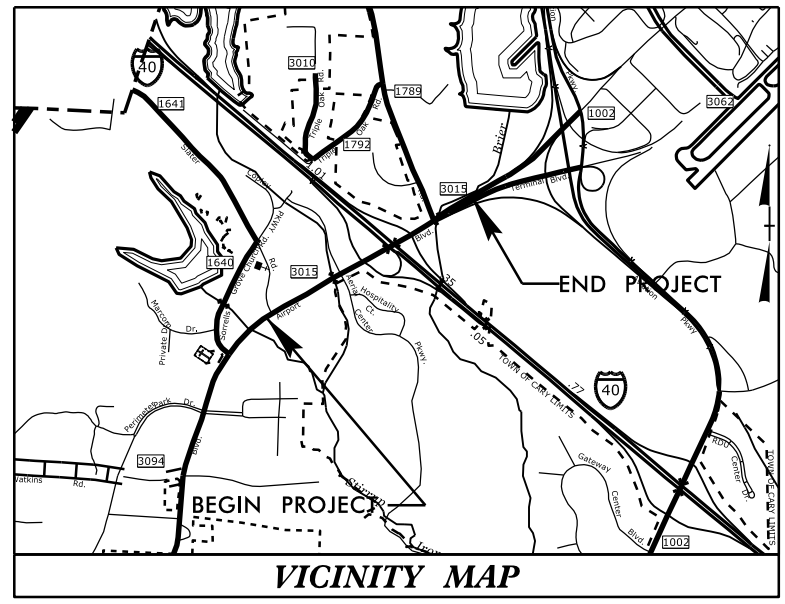
STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

**WAKE COUNTY**

**LOCATION: I-40 AND SR 3015 (AIRPORT BLVD.), REVISE INTERCHANGE AND CONSTRUCT AUXILIARY LANE ON I-40 WESTBOUND FROM SR 3015 (AIRPORT BLVD. TO I-540.**

**TYPE OF WORK: ITS, DRAINAGE, GRADING, PAVING, SIGNALS  
CULVERTS AND STRUCTURES**

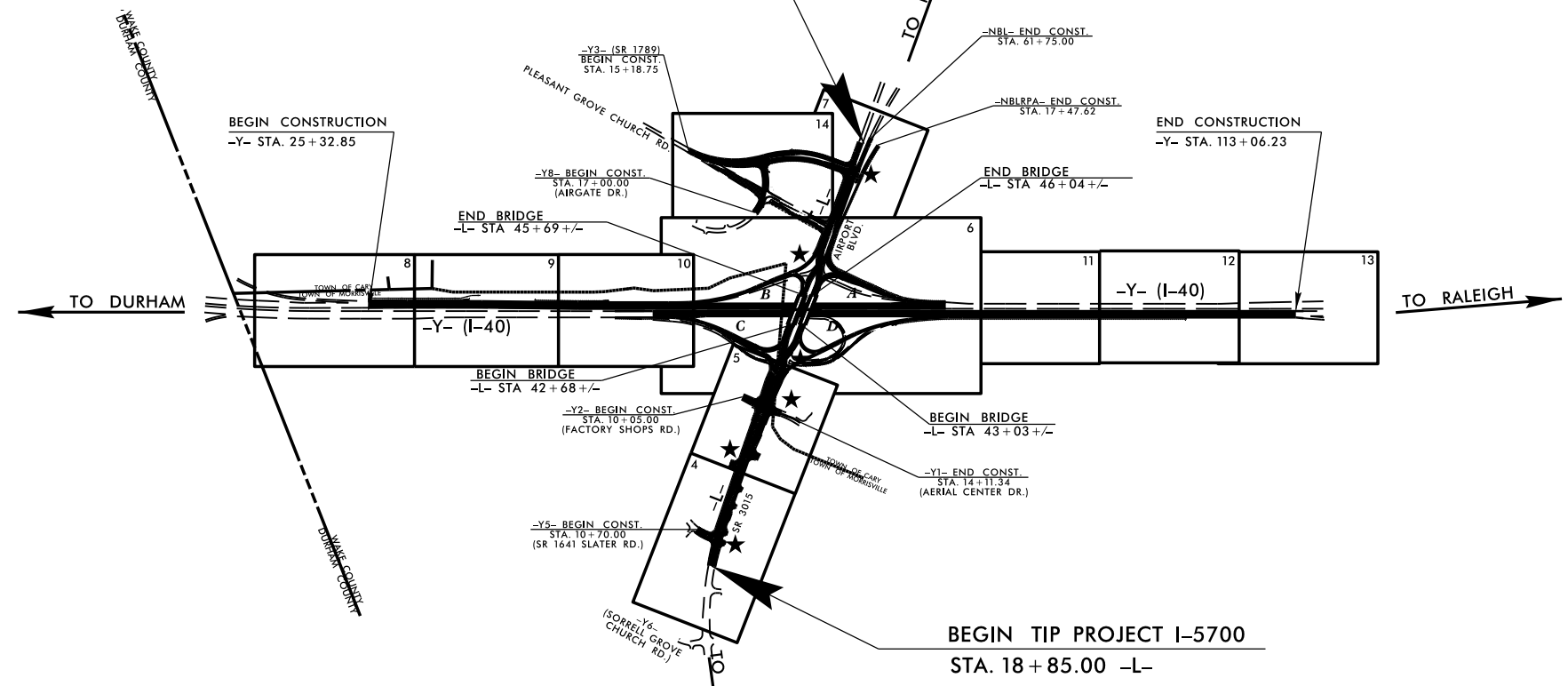
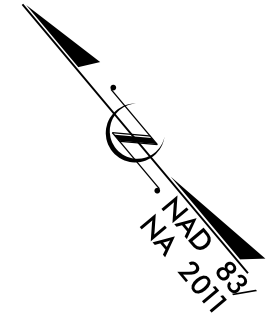
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	I-5700	3	130
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
50118.1.FS1	NHPP-040-1(259)286	PE	
<b>DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED</b>			



VICINITY MAP

END TIP PROJECT I-5700  
STA. 61+00.00 -L-

25%  
SUBMITTAL



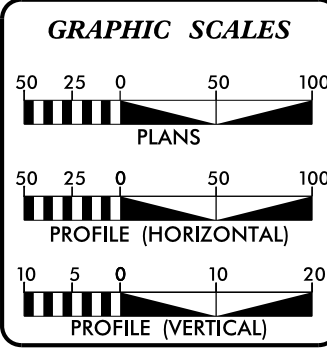
BEGIN CONSTRUCTION  
-Y- STA. 25+32.85

END CONSTRUCTION  
-Y- STA. 113+06.23

BEGIN TIP PROJECT I-5700  
STA. 18+85.00 -L-

A PORTION OF THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARIES OF MORRISVILLE.  
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD         .  
THIS IS A PARTIAL CONTROLLED- ACCESS PROJECT WITH ACCESS BEING LIMITED TO POINTS SHOWN ON THE PLANS.

★ PROPOSED SIGNALS



**DESIGN DATA**

ADT 2019 =	33,660
ADT 2040 =	46,500
K =	9 %
D =	65 %
T =	6 % *
V =	50 MPH
* TTST = 2%	DUAL = 4%
FUNC CLASS =	ARTERIAL
STATEWIDE TIER	

**PROJECT LENGTH**

LENGTH ROADWAY TIP PROJECT I-5700	=	0.741 MILES
LENGTH STRUCTURE TIP PROJECT I-5700	=	0.057 MILES
TOTAL LENGTH OF TIP PROJECT I-5700	=	0.798 MILES

**WETHERILL ENGINEERING**  
1223 JONES FRANKLIN ROAD  
Raleigh, N.C. 27606  
License No. F-0377  
Fax: 919 851 8077  
E-mail: 919 851 8077

Prepared for the North Carolina Department of Transportation in the Office of:

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: **EDWARD G. WETHERILL, PE**  
PROJECT ENGINEER  
SEPTEMBER 21, 2018

LETTING DATE: **BOB A. MAY, PE**  
PROJECT DESIGN ENGINEER  
SEPTEMBER 17, 2019

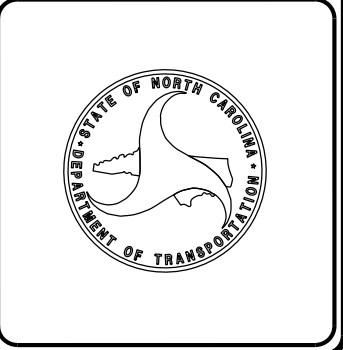
NCDOT CONTACT: **TATIA WHITE, PE, PLS**  
ROADWAY DESIGN-PROJECT ENGINEER

**HYDRAULICS ENGINEER**

SIGNATURE: \_\_\_\_\_ P.E.

**ROADWAY DESIGN ENGINEER**

SIGNATURE: \_\_\_\_\_ P.E.



PRELIMINARY PLANS  
DO NOT USE FOR CONSTRUCTION  
INCOMPLETE PLANS  
DO NOT USE FOR R/W ACQUISITION



August 15, 2018

STATE PROJECT: 50118.1.FS1 (I-5700)  
 COUNTY: Wake  
 DESCRIPTION: I-40 and SR 3015 (Airport Boulevard) Revise Interchange and Construct Auxiliary Lane on I-40 Westbound from SR 3015 (Airport Boulevard) to I-540

**SUBJECT: GEOTECHNICAL REPORT - INVENTORY**

**PROJECT DESCRIPTION**

This project consists of a revised interchange at I-40 and SR 3015 (Airport Boulevard) including a dual bridge and realigned ramps (-RPA-, -RPASPUR-, -RPB-, -RPBSPUR-, -RPC-, -RPCSPUR-, -RPD-, -RPDSPUR-). This project will also include the construction of an auxiliary lane on I-40 westbound from SR 3015 (Airport Boulevard) to I-540. Lane widening is proposed for Aerial Center Parkway (-Y1-), Factory Shops Road (-Y2-), Slater Road (-Y5-) as well as the realignment and widening of Pleasant Grove Church Road (-Y3-) and extension to Airgate Drive (-Y8-).

The geotechnical investigation was conducted between April and June 2018. Standard Penetration Test borings were advanced with a CME-55 drill rig with an automatic hammer. Hand Augers were also performed in areas where the use of a drill rig was restricted. Representative soil samples were collected for visual classification in the field and selected samples were submitted for laboratory analysis by Geotechnics and F&R.

The following alignments, totaling 4.44 miles, were investigated. Plan sheets and cross sections of these alignments are included in this report.

<u>LINE</u>	<u>STATIONS</u>
-L-	19+60 to 62+25
-RPA-	10+00 to 22+26
-RPASPUR-	10+00 to 12+07
-RPB-	10+00 to 23+94
-RPBSPUR-	10+00 to 14+09
-RPC-	10+00 to 22+86
-RPCSPUR-	20+41 to 22+81
-RPD-	10+00 to 27+00
-RPDSPUR-	10+00 to 14+78
-Y-	25+33 to 113+06
-Y1-	12+97 to 14+11
-Y2-	10+05 to 11+76
-Y3-	15+19 to 30+73
-Y5-	10+70 to 12+61
-Y8-	17+00 to 21+78
-NBL-	56+19 to 61+00
-NBLRPA-	12+29 to 17+48

**PHYSIOGRAPHY AND GEOLOGY**

The project is located in the Piedmont Physiographic Province. The project corridor is comprised primarily of residential to urban properties and undeveloped wooded areas. The general topography along the project is generally flat to gently sloping.

Geologically, the project is located within the Durham sub-basin of the Deep River Triassic Basin. Soils are derived from the underlying bedrock which consists of Triassic age sedimentary rocks (interbedded sandstone, mudstone, siltstone, and conglomerate) of the Chatham group (Sanford formation).

**SOIL PROPERTIES**

Soils encountered during this investigation are separated into four categories based on origin. They consist of roadway embankment, artificial fill, alluvial soil, and Triassic residual soils.

Roadway Embankment soils are present along the existing roadways on the project. The roadway embankment encountered generally consist of dry to moist, soft to very stiff, sandy clays and sandy silts (A-4, A-6) with varying amounts of gravel and moist, medium stiff, silty clay (A-7). The plasticity index of the roadway embankment soils tested ranged from 5 to 13.

Soil identified as artificial fill are present adjacent to the existing roadway. The artificial fill encountered generally consists of dry to moist, medium stiff to hard, coarse to fine sandy clays and fine sandy silts (A-4, A-6) with varying amounts of rock fragments and dry, loose, coarse sands (A-1) with variable amounts of asphalt fragments and gravel. The plasticity index of the artificial fill silt tested was 2.

Alluvial soils are present along the existing streams and creeks on the project. The alluvial soils generally consist of moist to wet, very soft to hard, sandy silts, clayey silts, and sandy clays (A-4, A-5, A-6) moist to wet, soft to very stiff, highly plastic, silty clay (A-7), and moist to saturated, medium dense, silty coarse to fine sands and coarse sands (A-2-4, A-2-5, A-3) with variable amounts of organic matter and gravel. The plasticity of the alluvial silt tested was 3.

Triassic residual soils are derived from the weathering of underlying Triassic sandstone, siltstone, mudstone, and conglomerate. The majority of the Triassic residual soils encountered consist of moist, soft to hard, fine sandy silts and sandy clays (A-4, A-6) with variable amounts of rock fragments, moist, medium stiff, highly plastic, silty clays (A-7), and dry, non-plastic to slightly plastic, silty coarse to fine sands (A-2-4). The plasticity index of the Triassic residual silts and clays tested ranged from 5 to 16.



**ROCK PROPERTIES**

Weathered rock was encountered along the existing roadways (-L-, -RPA-, -RPASPUR-, -RPB-, -RPBSPUR-, -RPC-, -RPD-, -Y-, -Y3-, -Y5-, and -Y8-) at elevations ranging from 270.3 to 358.4 feet (MSL). The majority of the weathered rock consists of Triassic sandstone and siltstone with lessor amounts of Triassic mudstone and conglomerate. Triassic weathered rock exhibits high slaking characteristics when exposed at the surface to air and water.

Non-Crystalline rock was encountered along the existing roadways (-L-, -RPB-, -RPC-, -RPD-, -Y-, -Y2-, and -Y8-) at elevations ranging from 264.7 to 350.7 feet (MSL). The majority of the non-crystalline rock consists of Triassic sandstone and siltstone with lessor amounts of Triassic mudstone and conglomerate. Triassic non crystalline rock exhibits high slaking characteristics when exposed at the surface to air and water.

**GROUNDWATER**

Groundwater was encountered at elevations ranging from 277.5 to 306.7 feet and is typically 8 feet or more below the existing ground surface.

**AREAS OF SPECIAL GEOTECHNICAL INTEREST**

1) Highly Plastic Clays: Highly plastic clays (PI > 25) were encountered on the project at the following locations:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	28+00 to 31+50	LT
-L-	34+50 to 38+00	LT
-L-	59+00 to 61+00	LT
-RPD-	13+50 to 15+50	LT
-Y-	84+00 to 86+00	RT
-Y-	88+00 to 90+00	RT
-Y-	92+50 to 97+00	RT
-Y2-	10+05 to 12+00	RT

A discussion of these highly plastic clay soils is located below in the section titled "Soil Properties."

2) Groundwater: The following areas exhibit a high water table, seasonal high groundwater or the potential for groundwater related construction problems:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-RPC-	21+00 to 23+00	LT to RT
-RPD-	13+50 to 15+50	LT to RT
-Y-	70+00 to 72+00	RT
-Y-	84+00 to 101+00	RT

2) Alluvial Soil: Alluvial Soil was encountered on the project at the following locations:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	29+00 to 34+00	LT
-L-	31+50 to 34+00	RT
-L-	51+00 to 52+80	RT
-L-	53+50 to 64+30	LT
-RPA-	10+00 to 20+50	RT
-RPD-	10+00 to 17+00	LT
-Y-	42+00 to 45+00	LT
-Y-	79+00 to 87+80	LT
-Y-	79+00 to 100+50	RT
-Y3-	26+00 to 28+00	LT
-Y3-	28+00 to 30+10	LT to RT

3) Artificial Fill: Artificial Fill was encountered on the project at the following locations:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	35+20 to 38+20	LT
-RPD-	19+50 to 21+50	LT to RT
-Y-	22+50 to 47+60	LT

4) Weathered Rock and Non-Crystalline Rock: The following areas exhibit shallow non-crystalline rock within 6 feet of the proposed grade (including at or above the proposed grades):

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	23+75 to 27+25	LT
-RPA-	17+70 to 21+50	LT to RT
-RPASPUR-	10+00 to 12+07	LT to RT
-RPB-	11+00 to 17+25	LT
-RPB-	17+75 to 21+75	LT to RT
-RPBSPUR-	10+00 to 13+00	LT to RT
-Y-	47+75 to 49+25	LT
-Y-	49+75 to 51+25	LT
-Y-	57+75 to 63+25	LT
-Y-	70+00 to 72+75	LT
-Y-	102+75 to 113+06	RT
-Y3-	15+25 to 16+25	RT
-Y3-	17+25 to 20+50	LT to RT

Prepared by,  
**KLEINFELDER, INC.**  
NC License No. F-1312



Daniel Kubinski, PE  
Staff Professional



Thomas R. Wells, PE  
Senior Professional

DHK/TRW:cas

Undisturbed Samples and Bulk Samples

<u>Sample No.</u>	<u>Alignment</u>	<u>STA.</u>	<u>Offset</u>	<u>Depth (ft)</u>	<u>Tests Performed</u>
ST-6	-RPA-	14+49	39' RT	5.0-7.0	CU Triaxial, Consolidation
S-12	-RPB-	17+61	116' LT	0.0-10.0	Standard Proctor, CBR

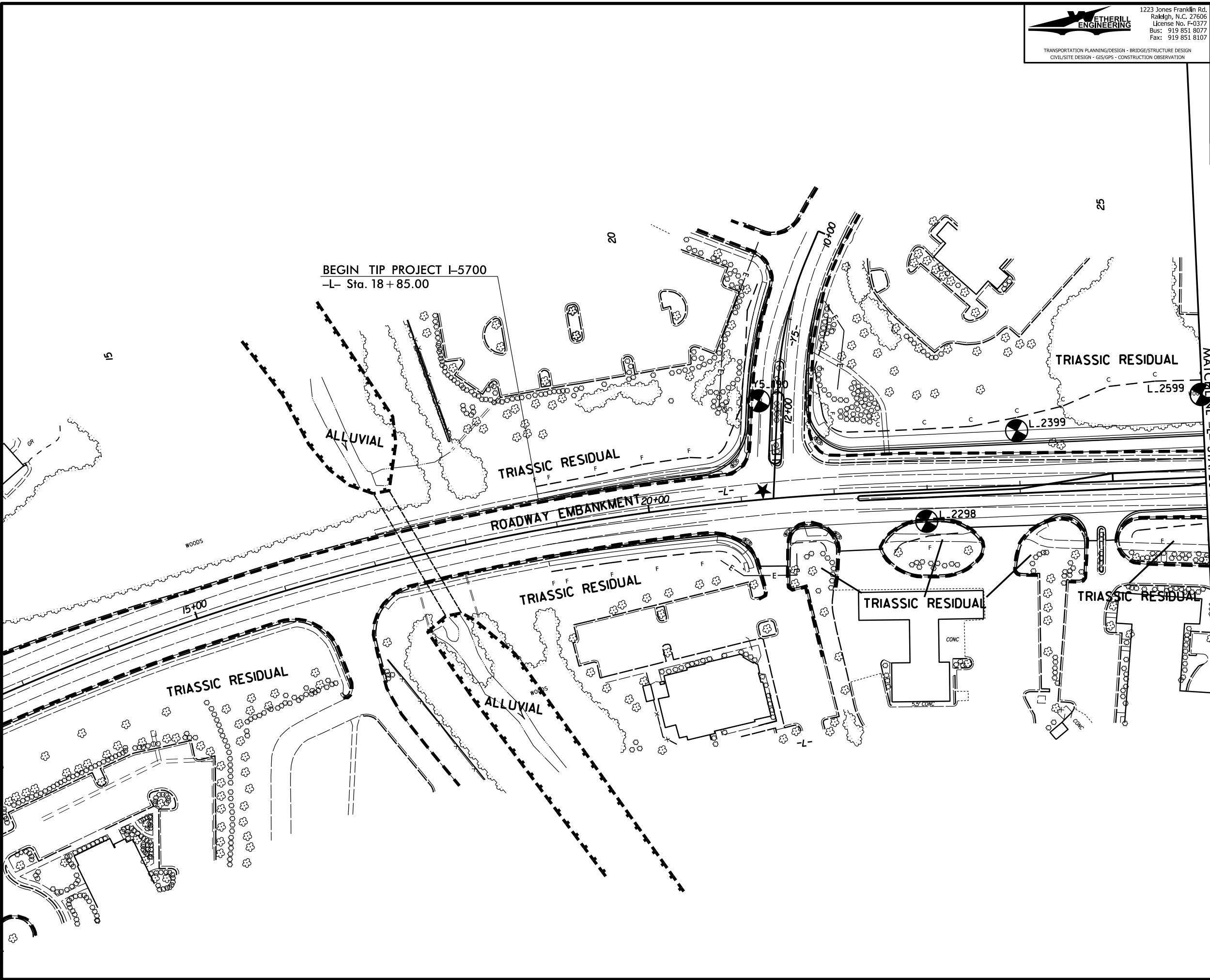
5/14/20

**ETHERILL ENGINEERING**  
 1223 Jones Franklin Rd.  
 Raleigh, N.C. 27606  
 License No. F-0377  
 Bus: 919 851 8077  
 Fax: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN  
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. <b>I-5700</b>	SHEET NO. <b>4</b>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> <b>UNLESS ALL SIGNATURES COMPLETED</b>	

14-AUG-2018 15:00  
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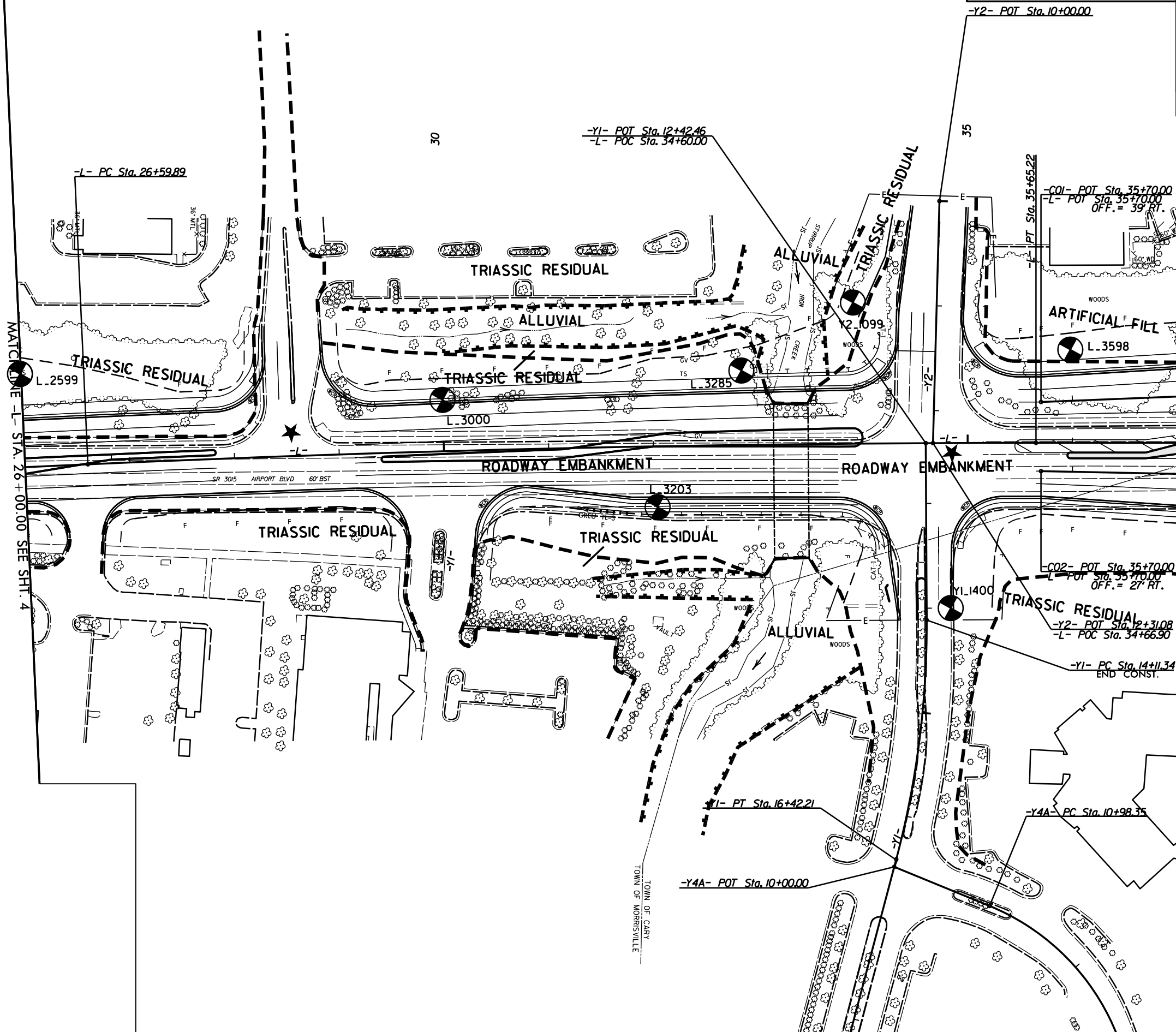
★ PROPOSED TRAFFIC SIGNAL

**ETHERILL ENGINEERING**  
 1223 Jones Franklin Rd.  
 Raleigh, N.C. 27606  
 License No. F-0377  
 Bus: 919 851 8077  
 Fax: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN  
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. <b>1-5700</b>	SHEET NO. <b>5</b>
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	

**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED**



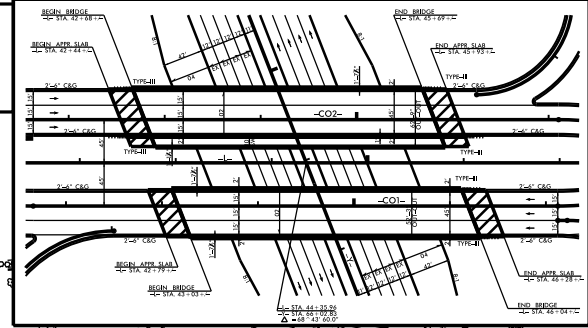
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MATCHLINE - STA. 37+00.00 SEE SHIT. 6

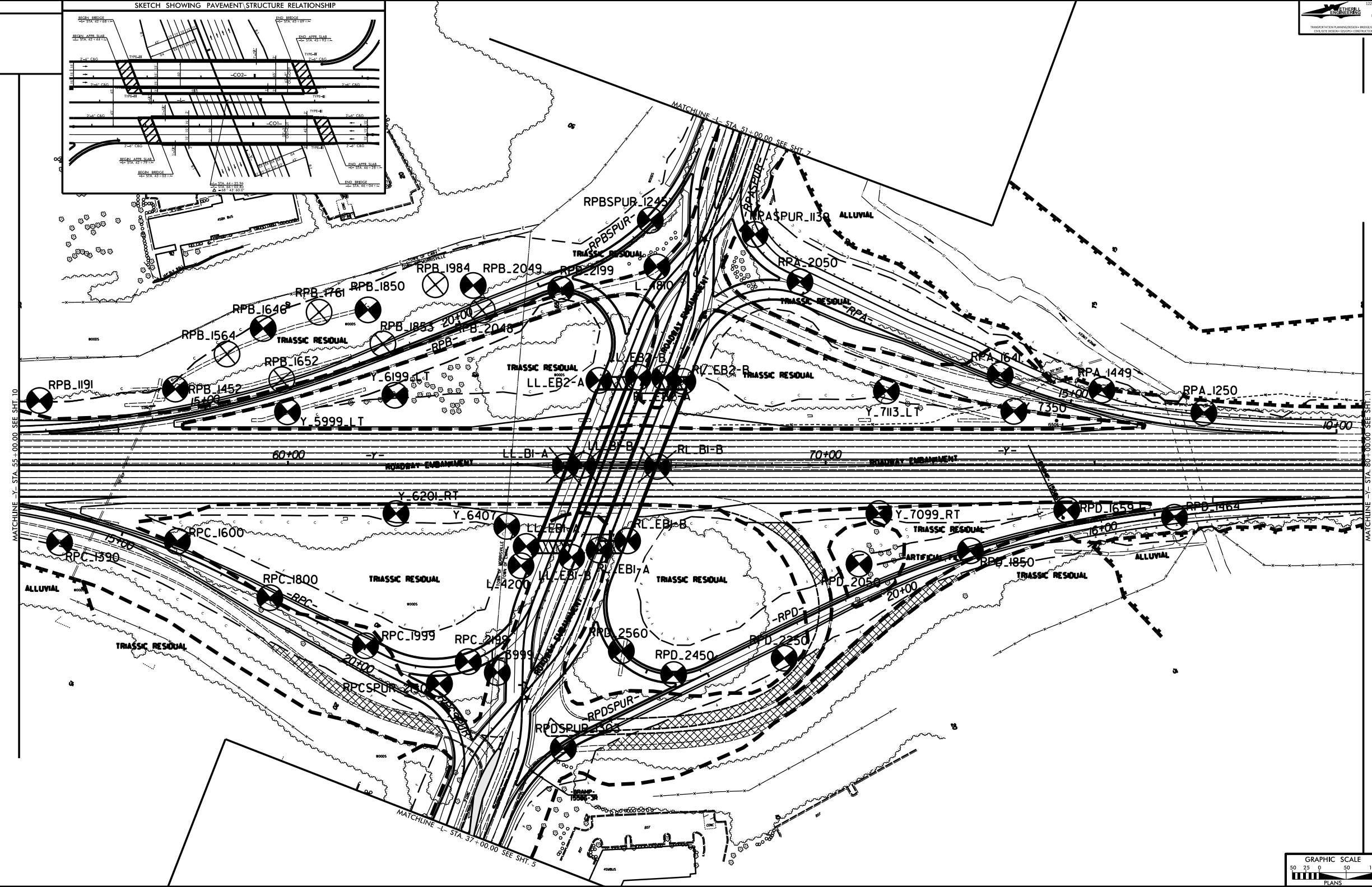


REVISIONS

SKETCH SHOWING PAVEMENT/STRUCTURE RELATIONSHIP



PROJECT REFERENCE NO. <b>C-500</b>		SHEET NO. <b>6</b>
ROADWAY DESIGN ENGINEER		REGISTERED ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR THE PROJECT		
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED		

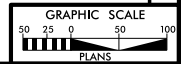


MATCHLINE -Y- STA. 55+00.00 SEE SHT. 10

MATCHLINE -Y- STA. 80+00.00 SEE SHT. 11

MATCHLINE -L- STA. 37+00.00 SEE SHT. 5

MATCHLINE -L- STA. 51+00.00 SEE SHT. 7



LOCATION: SR 3015 (JANPORT BLVD) WITH L&O	NO. 14700	COUNTY: WAKE
DESIGNED BY:		
CHECKED BY:		DATE:

★ PROPOSED TRAFFIC SIGNAL

C:\Users\j15788\OneDrive\Documents\Projects\SR 3015\SR 3015.dwg

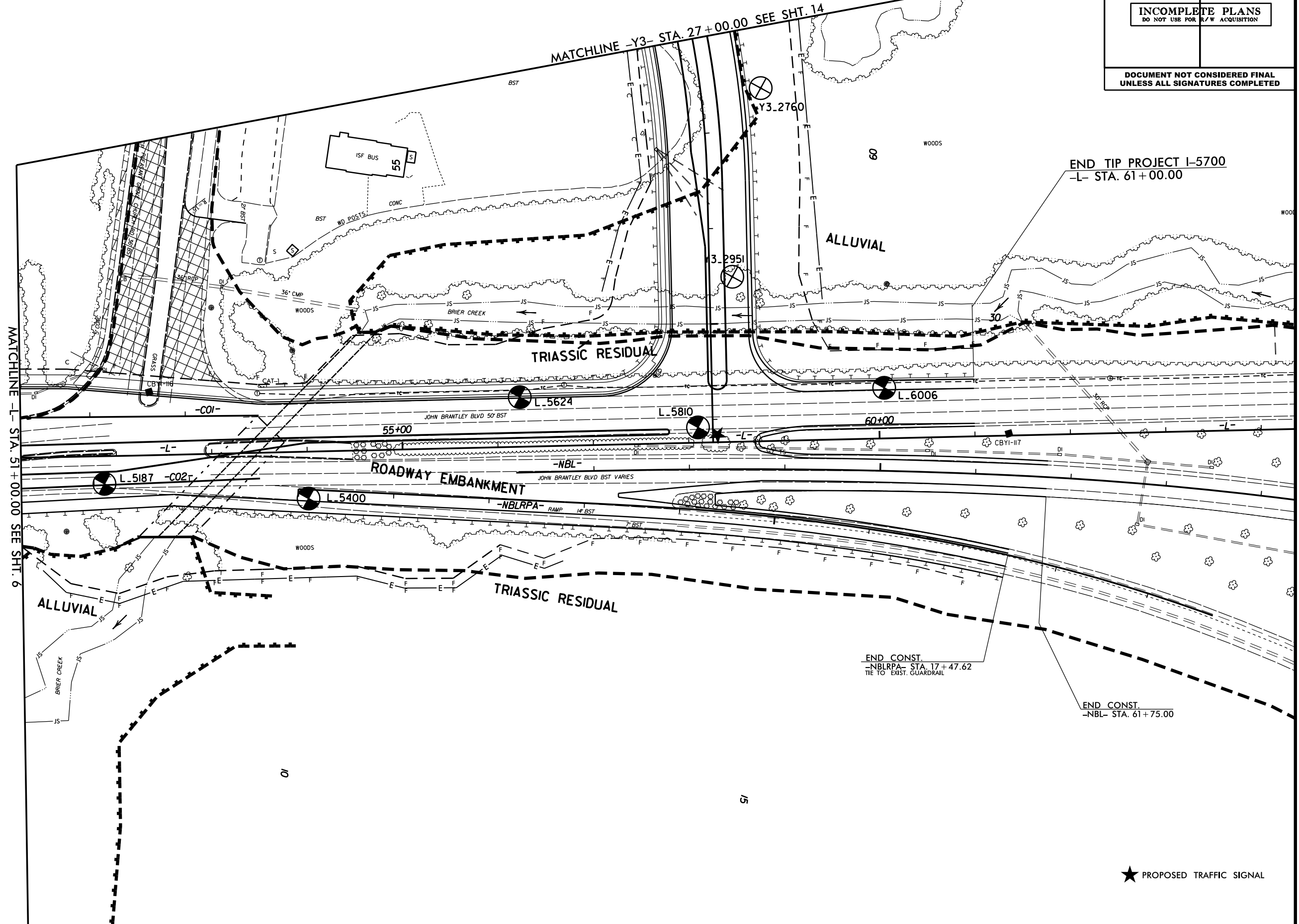
5/14/20

14-AUG-2018 15:00  
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 Bus: 919 851 8077  
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TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN  
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. <b>I-5700</b>	SHEET NO. <b>7</b>
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	



★ PROPOSED TRAFFIC SIGNAL

5/14/09

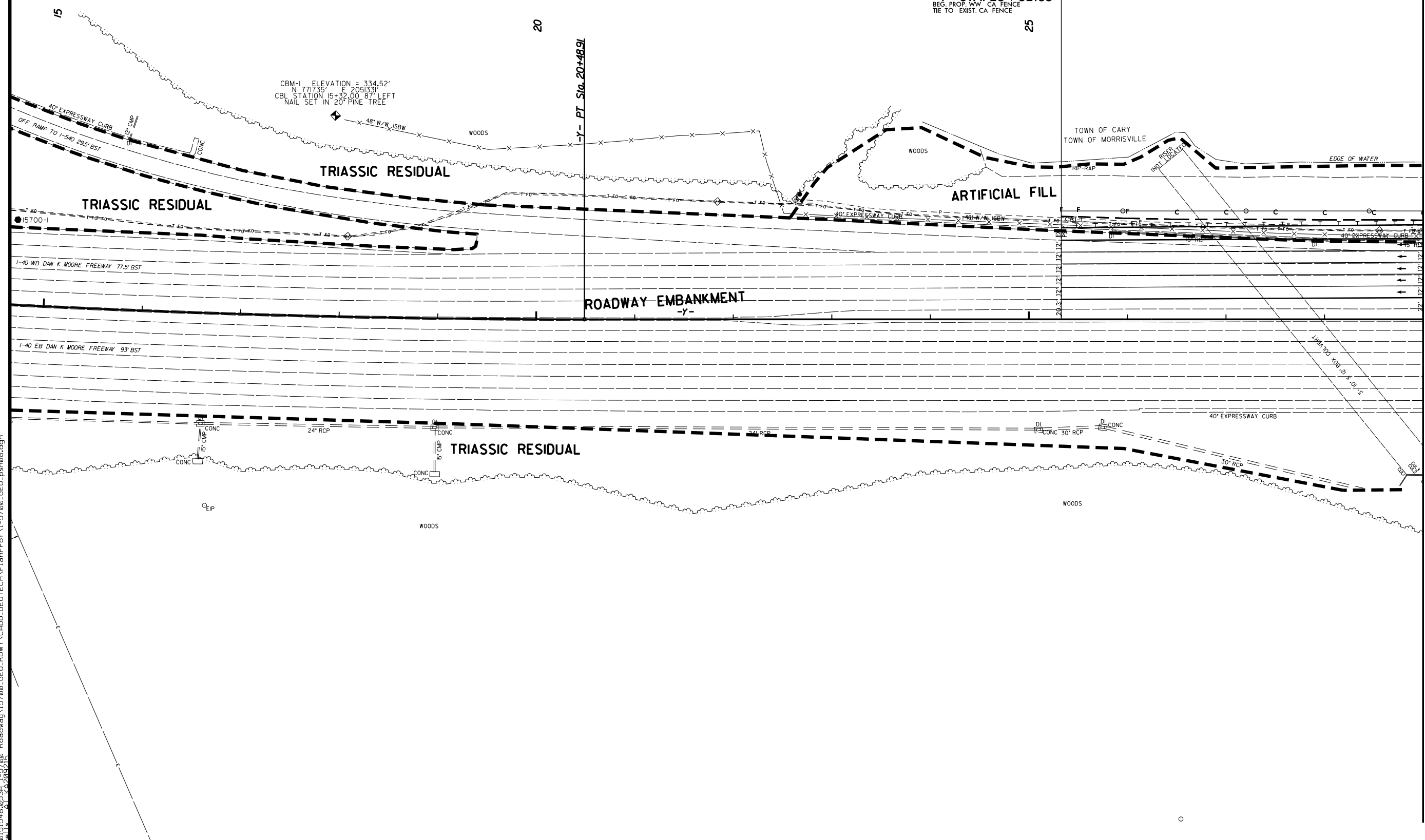
**WETHERILL ENGINEERING**  
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TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN  
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. <b>1-5700</b>	SHEET NO. <b>8</b>
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	



**BEGIN CONST.**  
 -Y- STA. 25+32.85  
 BEG. PROP. WW CA FENCE  
 TIE TO EXIST. CA FENCE



MATCHLINE -Y- STA. 29+00.00 SEE SHT. 9

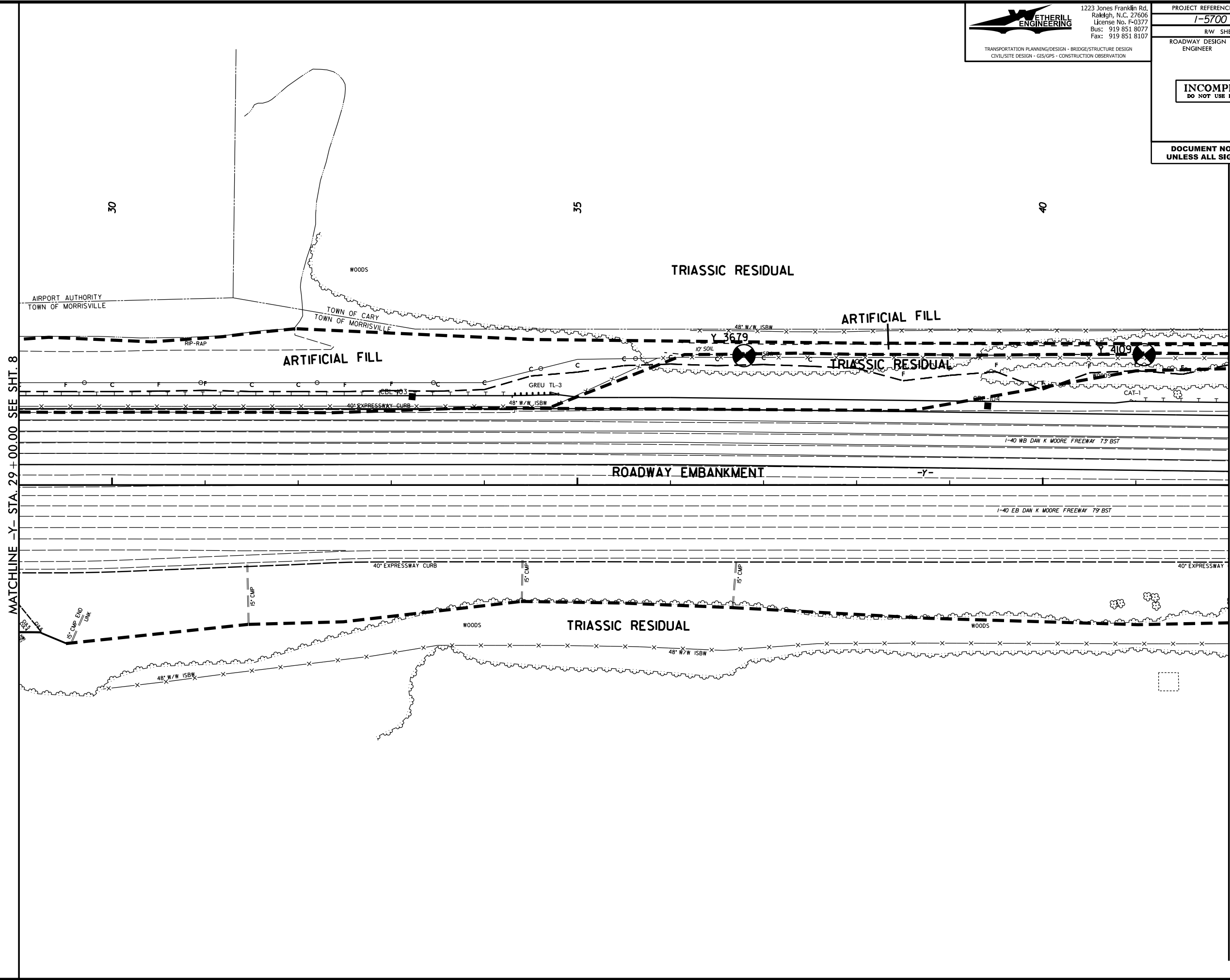
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5/14/99

**WETHERILL ENGINEERING**  
 1223 Jones Franklin Rd.  
 Raleigh, N.C. 27606  
 License No. F-0377  
 Bus: 919 851 8077  
 Fax: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN  
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. <b>1-5700</b>	SHEET NO. <b>9</b>
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	



14-AUG-2018 15:00  
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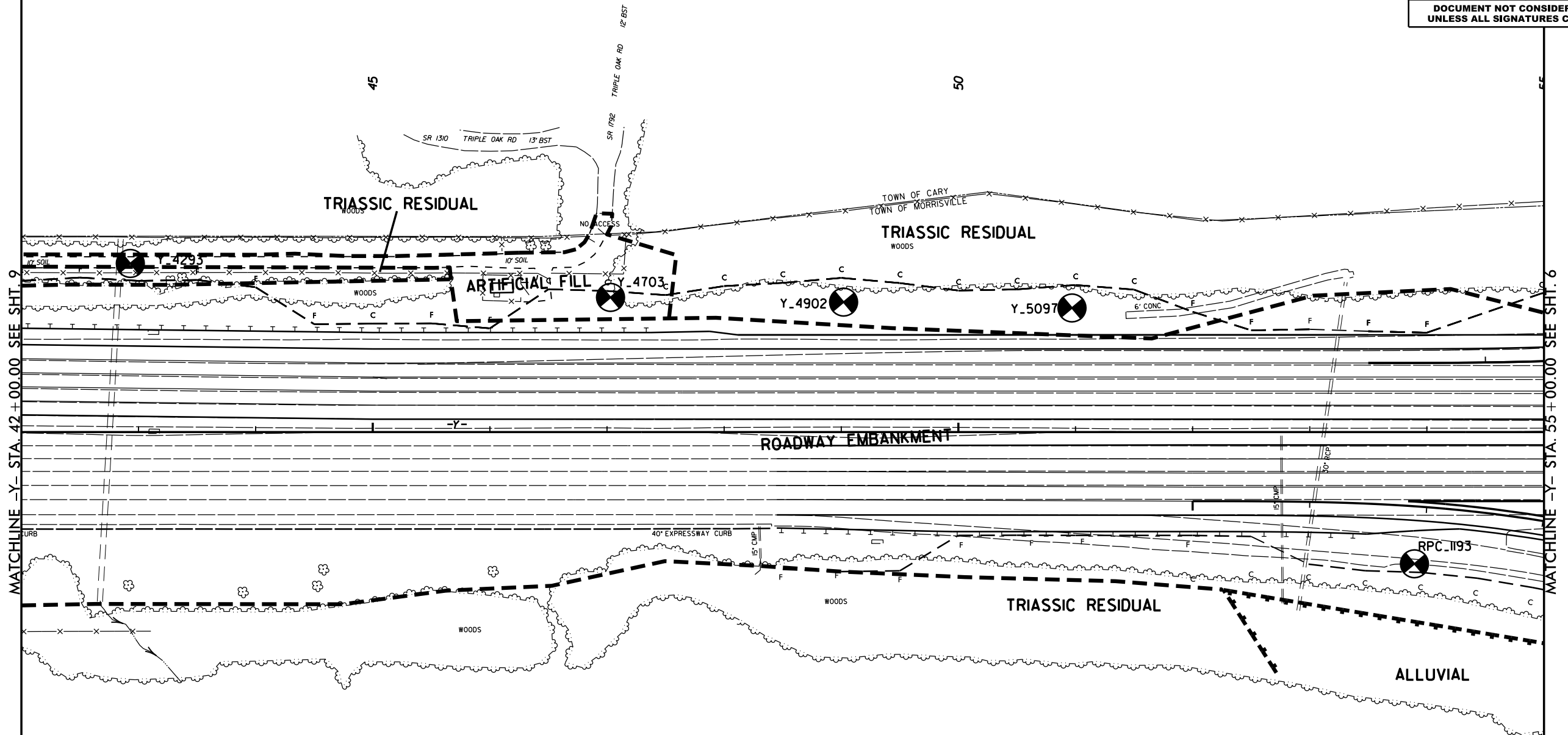


5/14/09

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Raleigh, N.C. 27606  
License No. F-0377  
Bus: 919 851 8077  
Fax: 919 851 8107  
TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN  
CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. <b>1-5700</b>	SHEET NO. <b>10</b>
R/W SHEET NO.	
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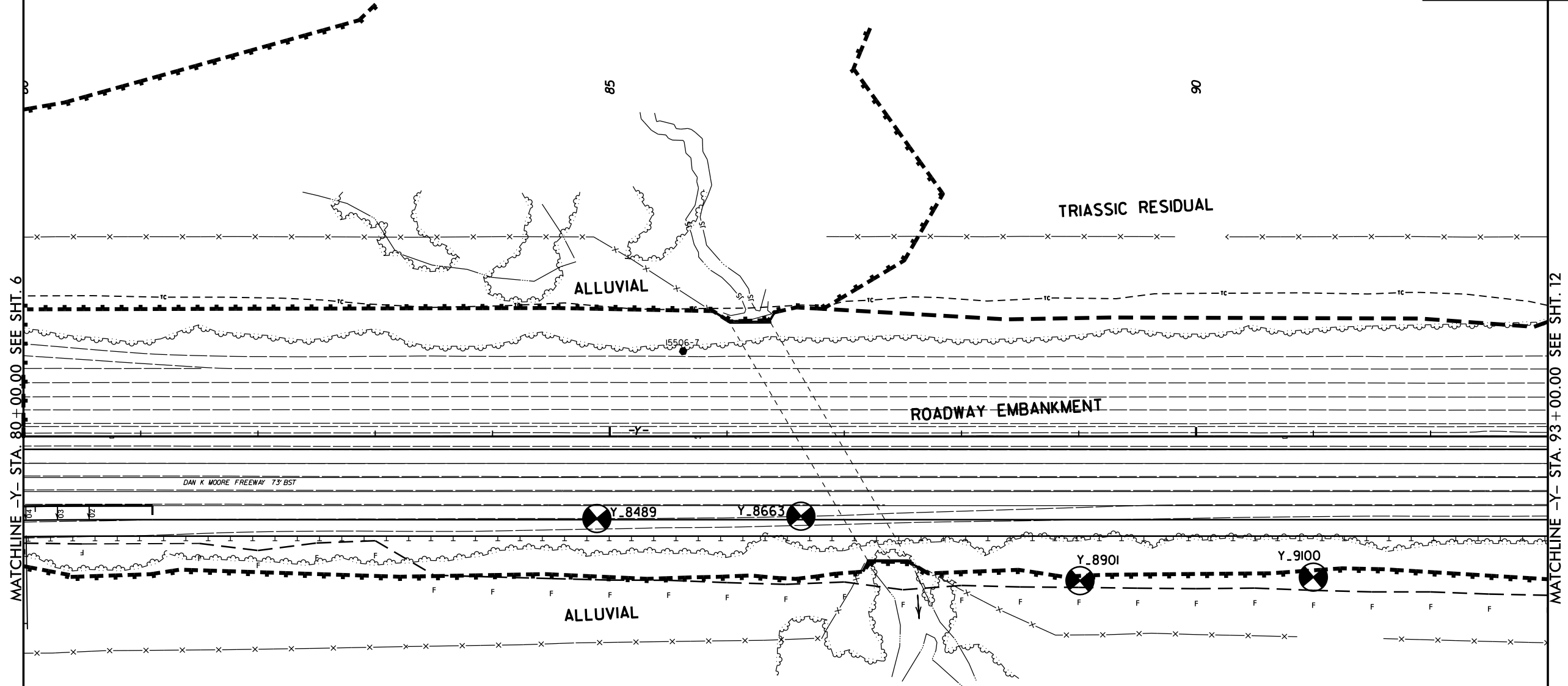


5/14/99

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 Raleigh, N.C. 27606  
 License No. F-0377  
 Bus: 919 851 8077  
 Fax: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN  
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. <b>1-5700</b>	SHEET NO. <b>11</b>
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	



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MATCHLINE -Y- STA. 93+00.00 SEE SHT. 12

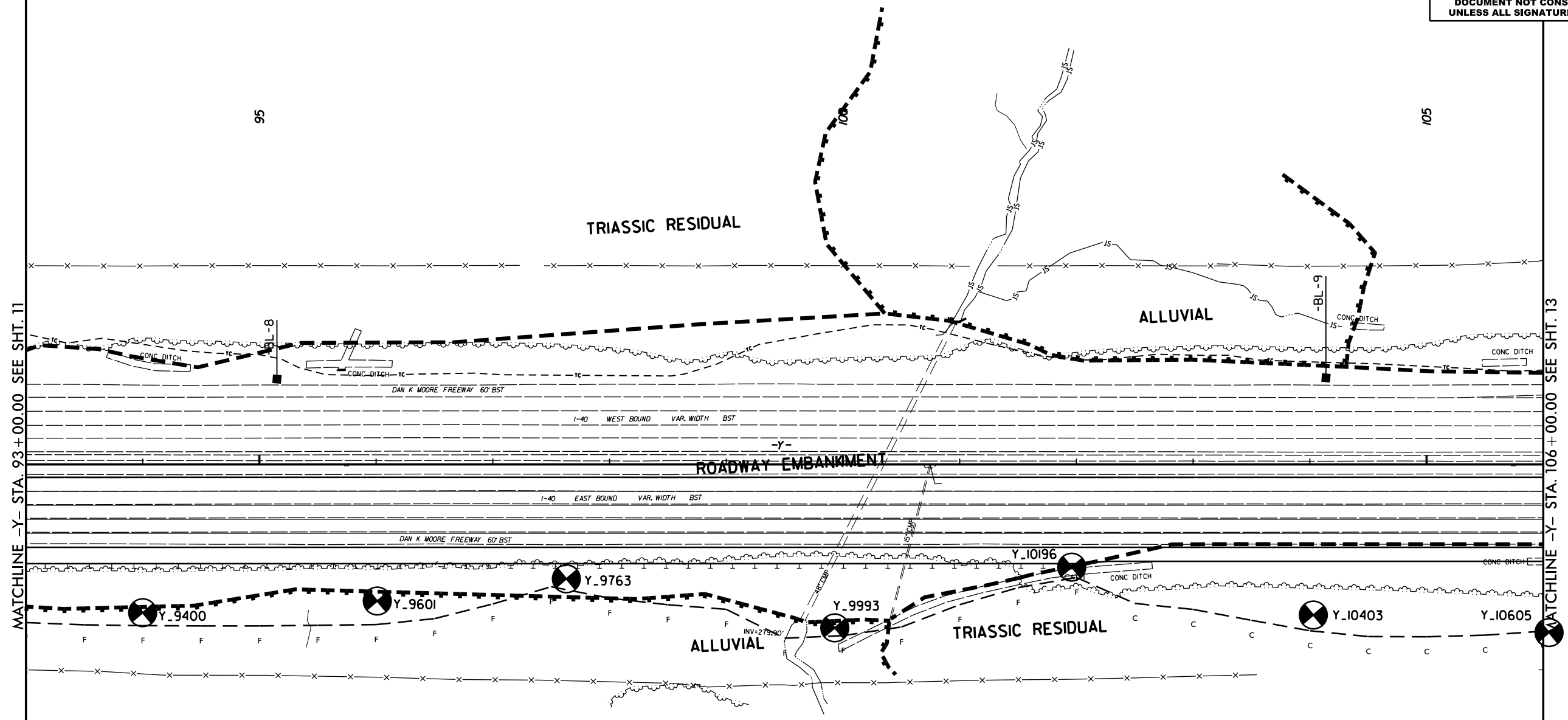
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 6/15/18 8:28:21 AM

5/14/99

**WETHERILL ENGINEERING**  
 1223 Jones Franklin Rd.  
 Raleigh, N.C. 27606  
 License No. F-0377  
 Bus: 919 851 8077  
 Fax: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN  
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. <b>1-5700</b>	SHEET NO. <b>12</b>
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
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<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	



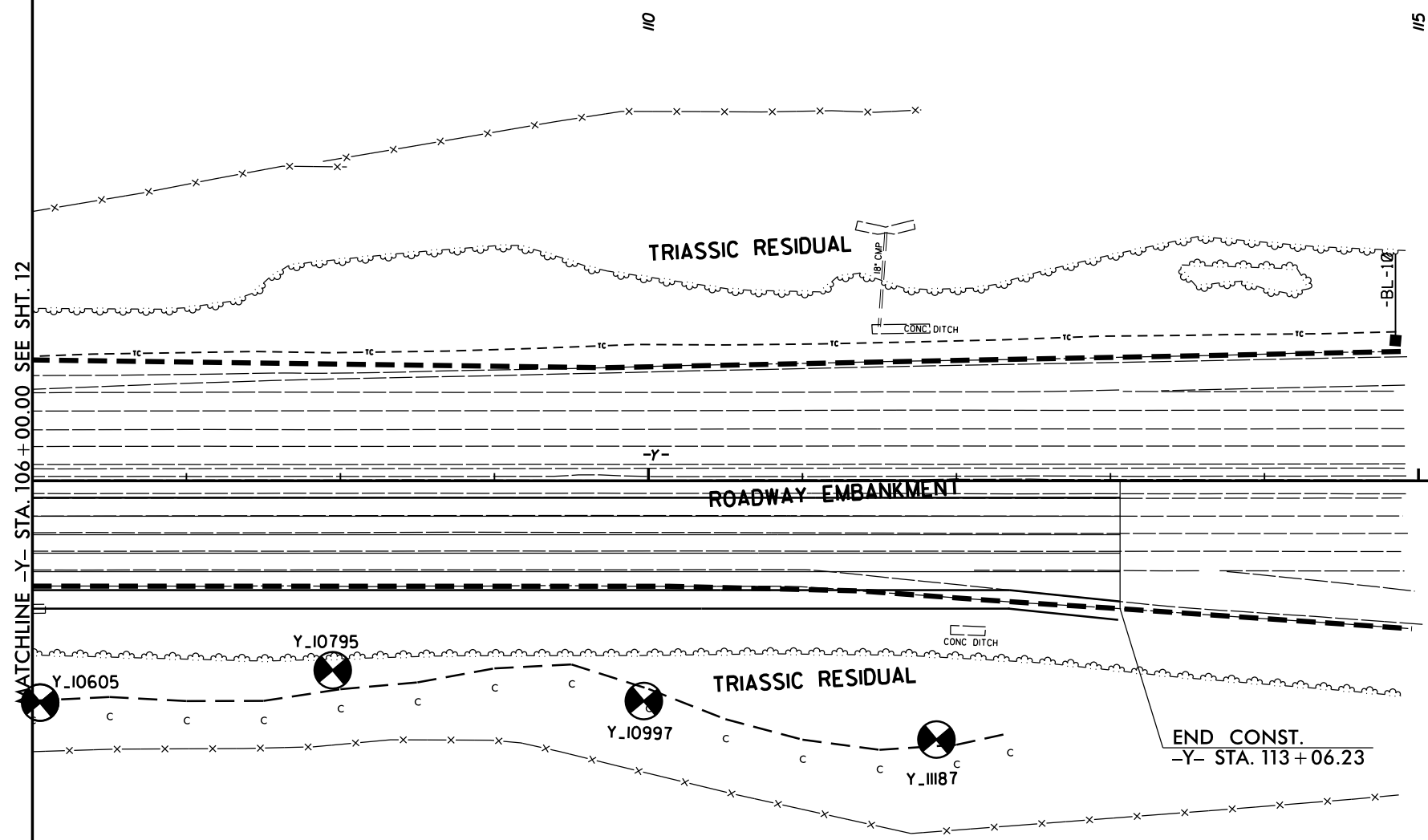
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5/14/99

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 1223 Jones Franklin Rd.  
 Raleigh, N.C. 27606  
 License No. F-0377  
 Bus: 919 851 8077  
 Fax: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN  
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. <b>1-5700</b>	SHEET NO. <b>13</b>
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR A/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	



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SEE SHTS. 20 THRU 23 FOR -Y- PROFILE



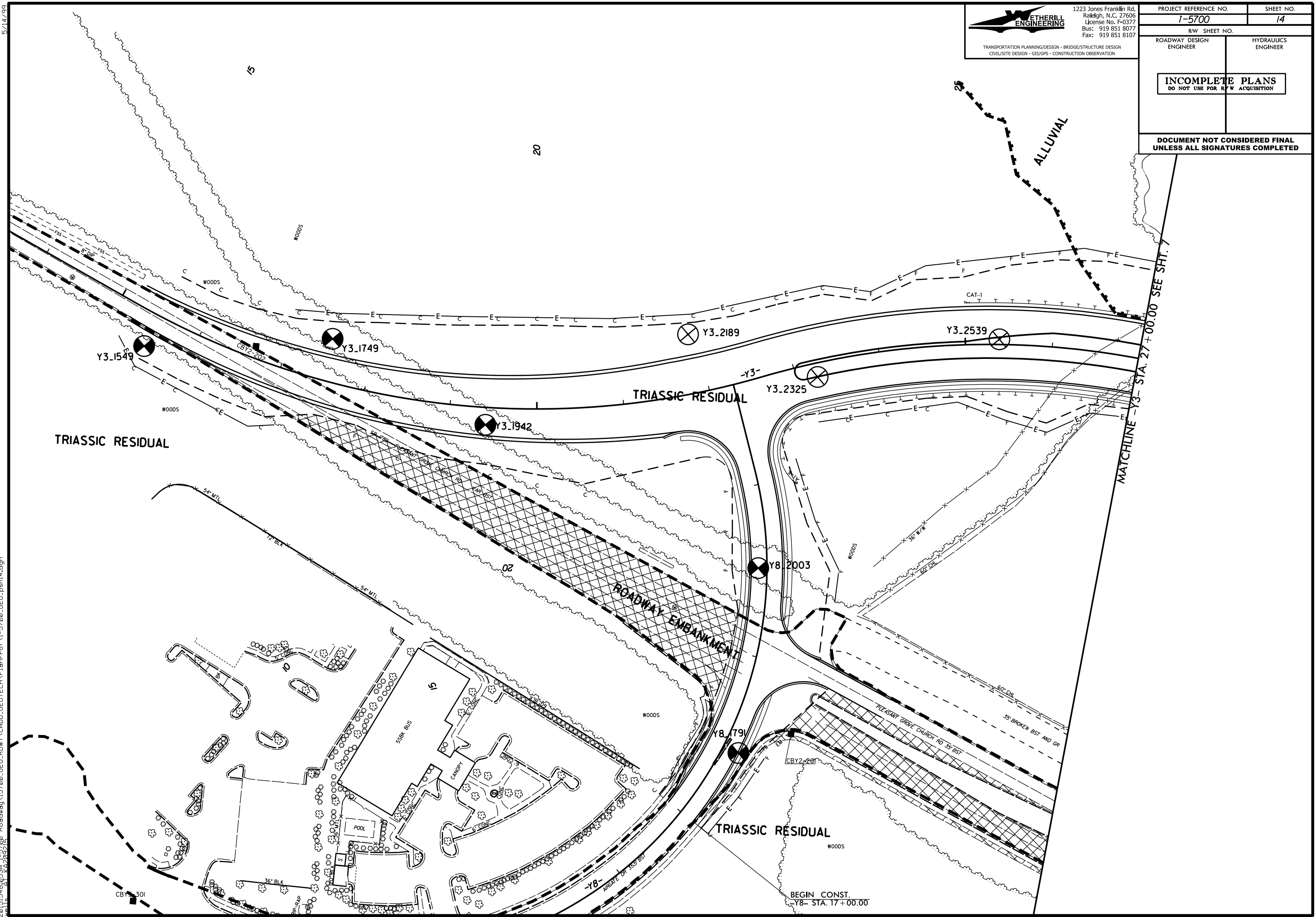
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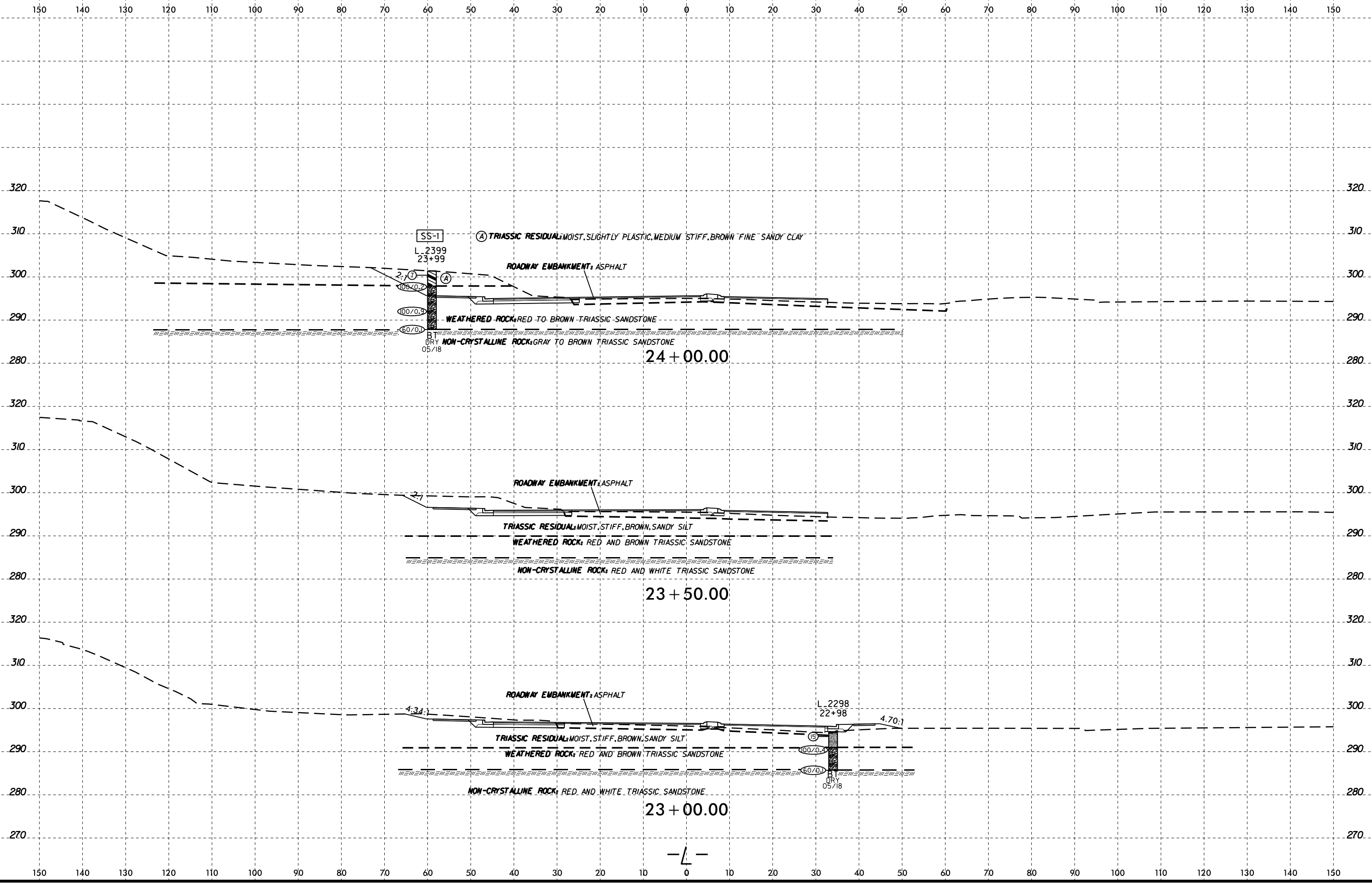
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**ETHERILL ENGINEERING**  
 1223 Jones Franklin Rd.  
 Raleigh, N.C. 27606  
 License No. F-0377  
 Bus: 919 851 8077  
 Fax: 919 851 8107

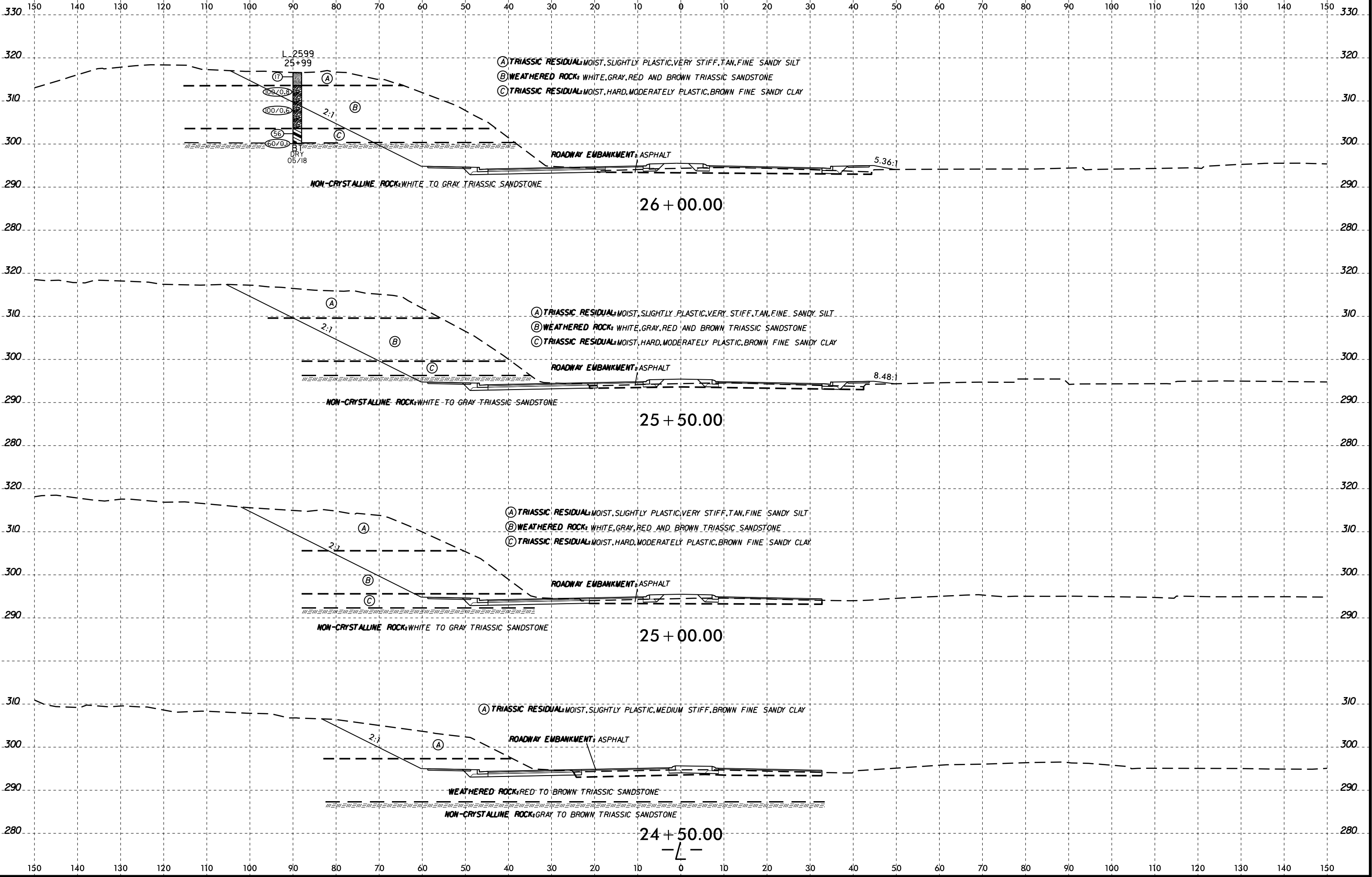
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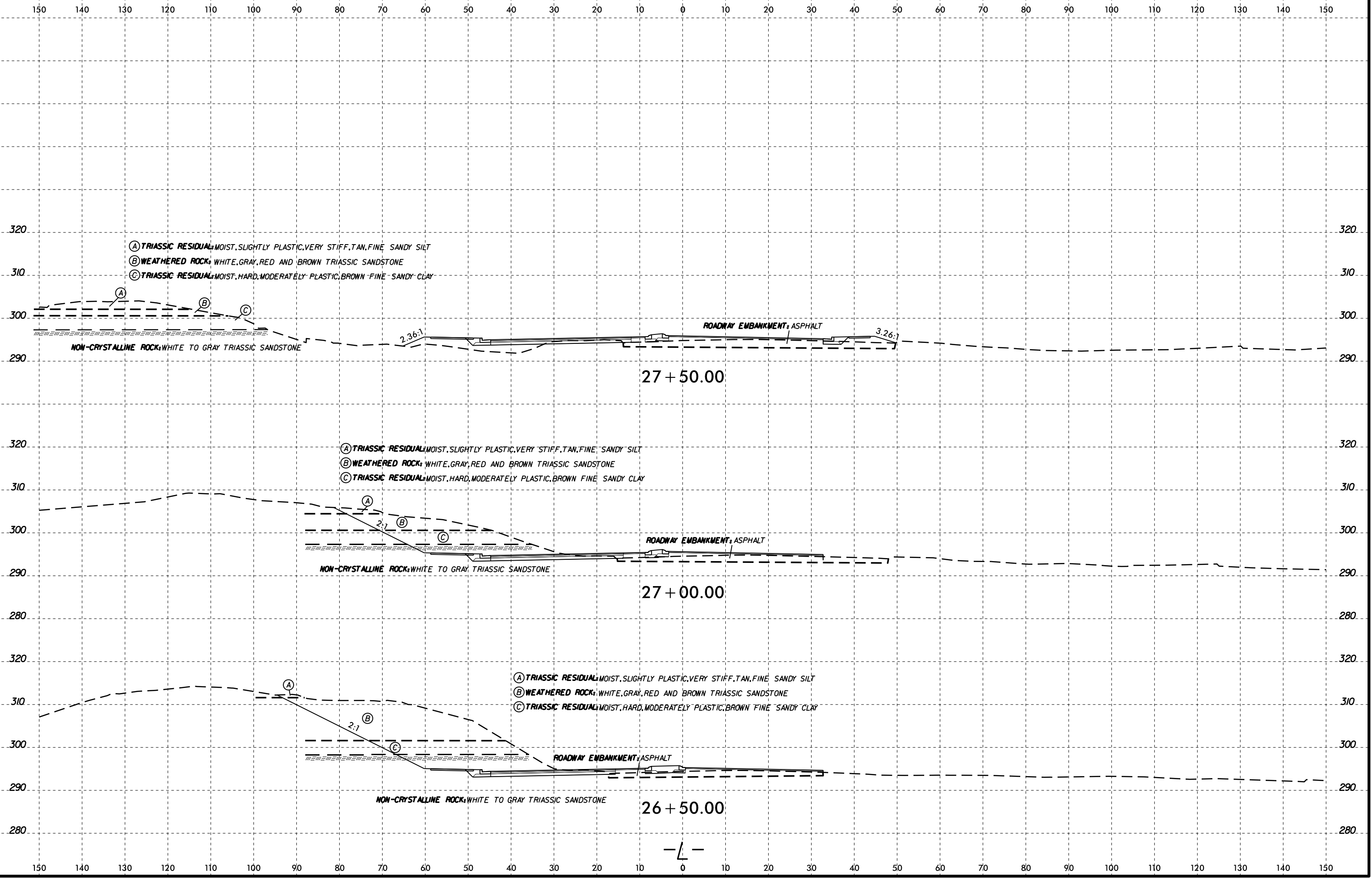
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R/W SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	



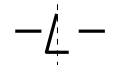


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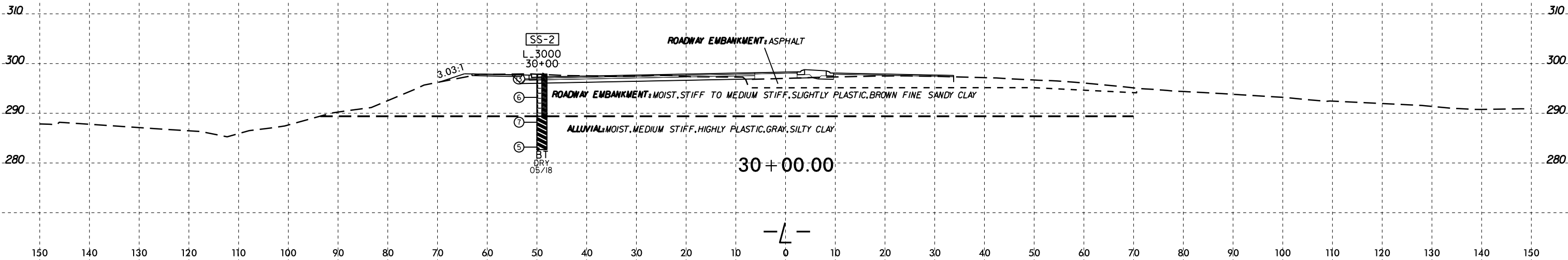
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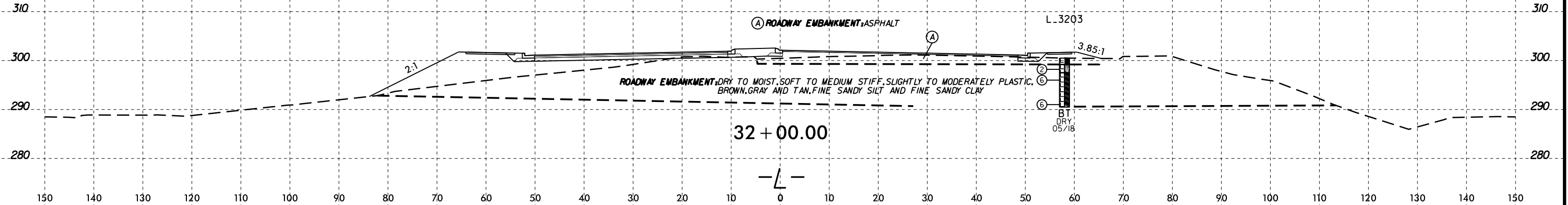
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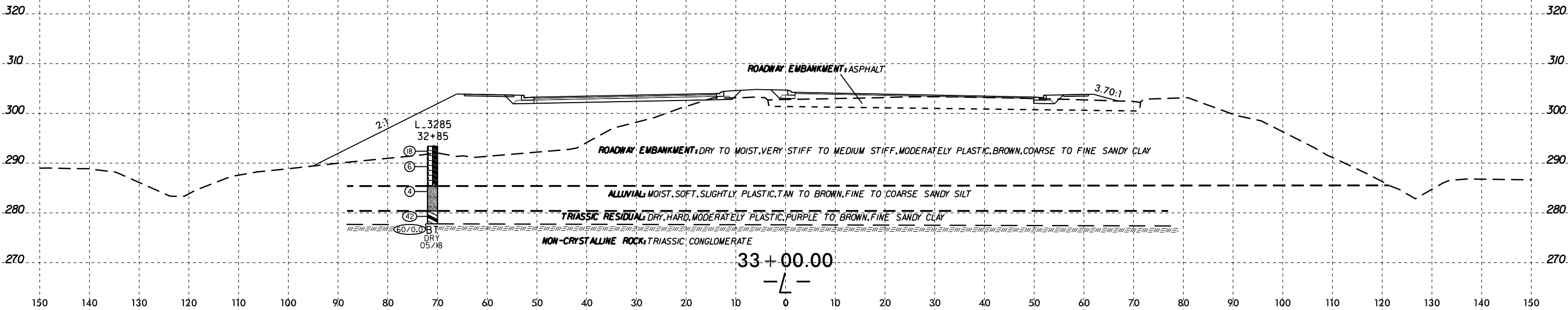
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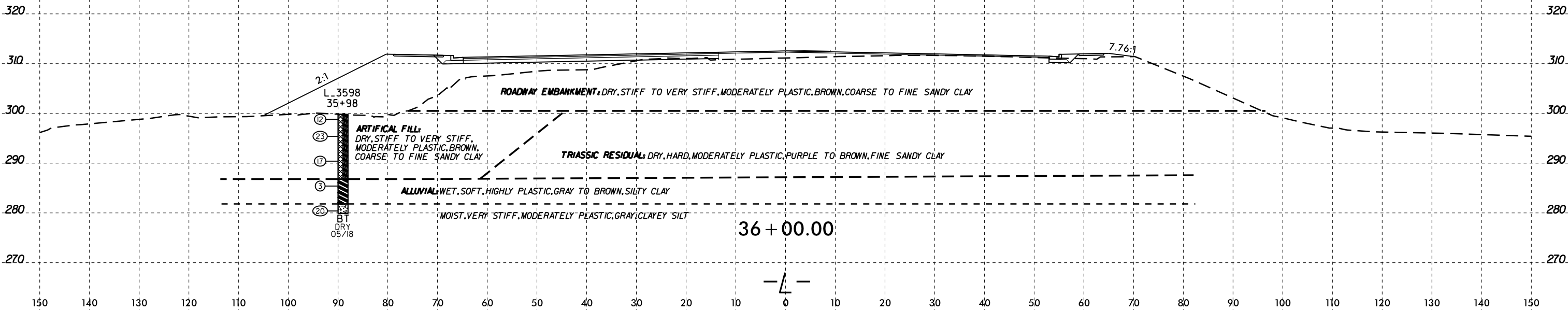
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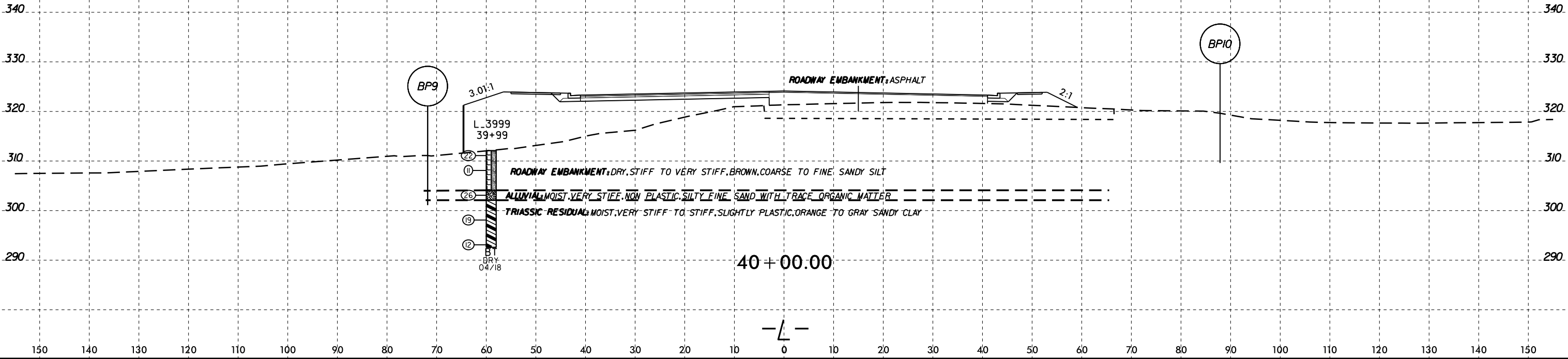


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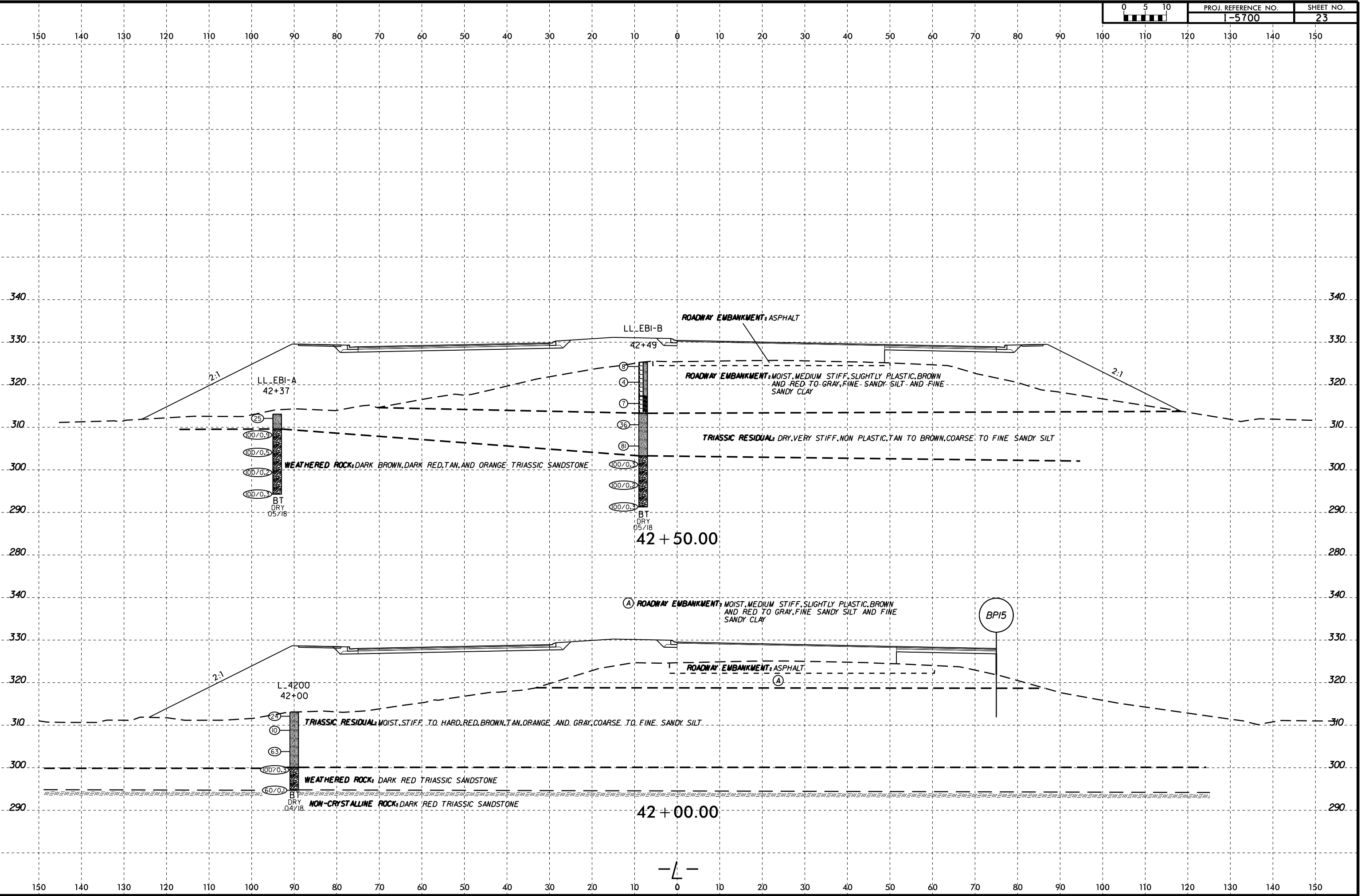




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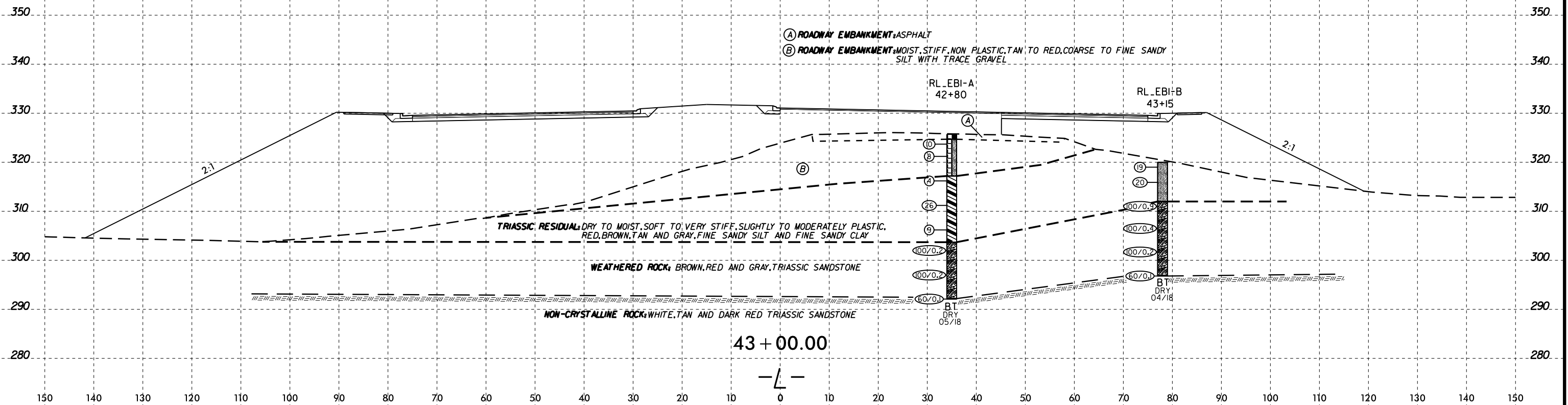


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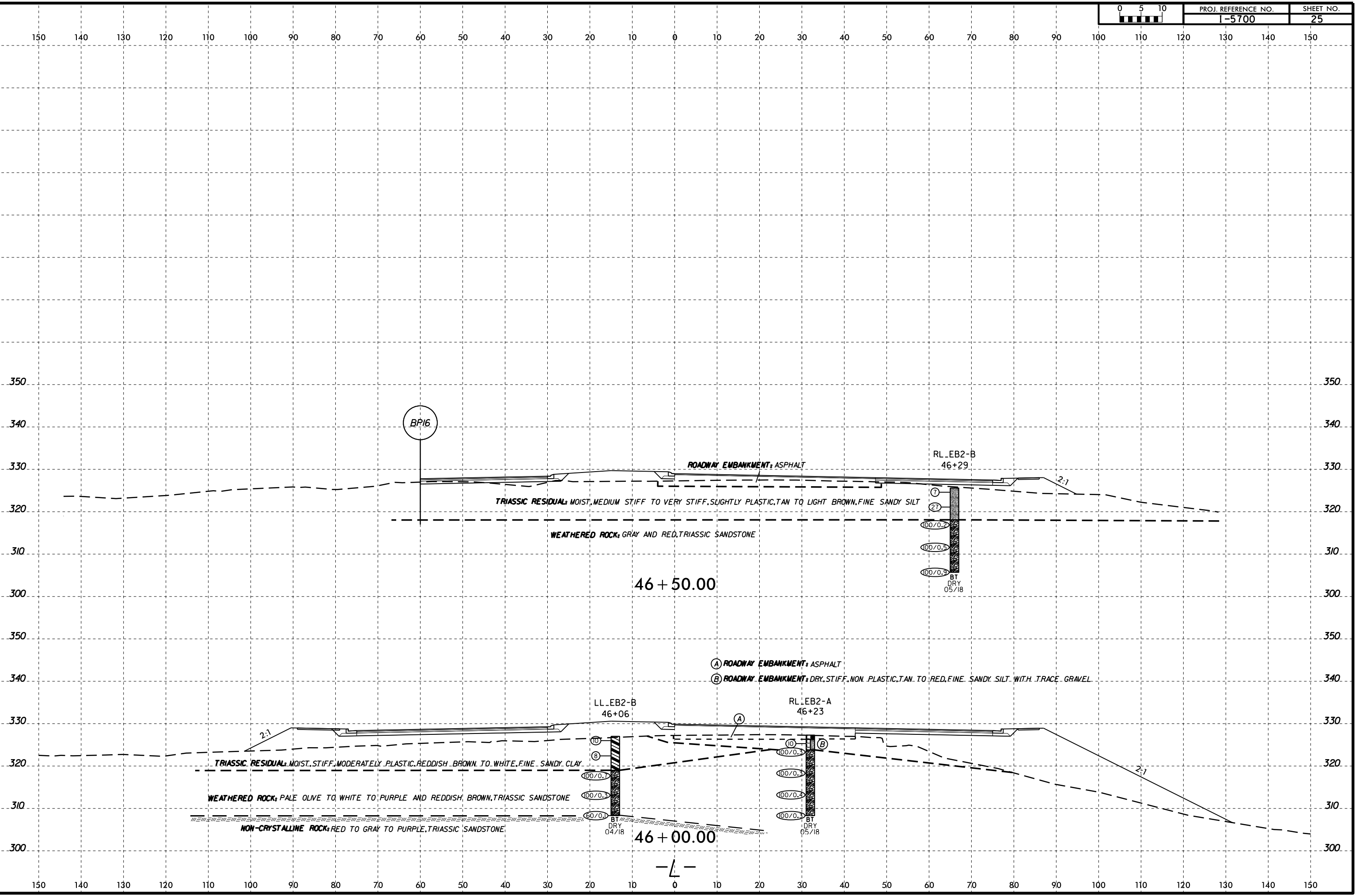
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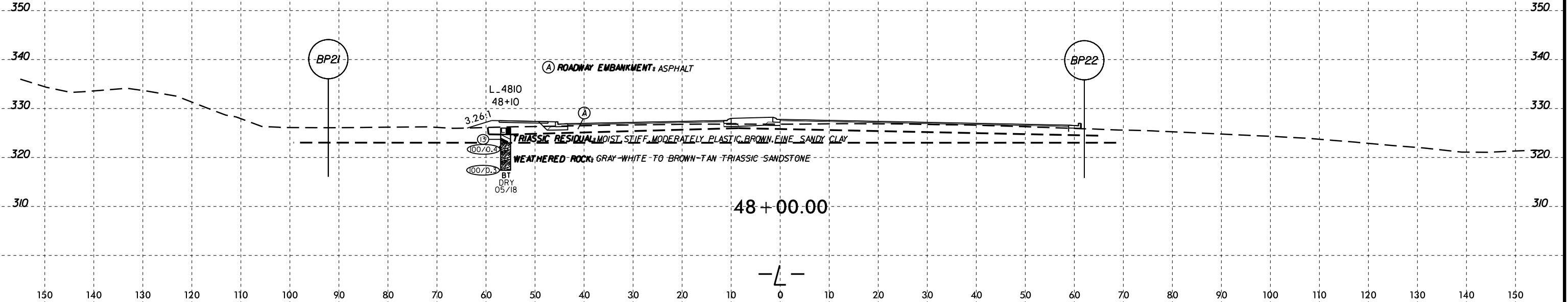


6/23/16  
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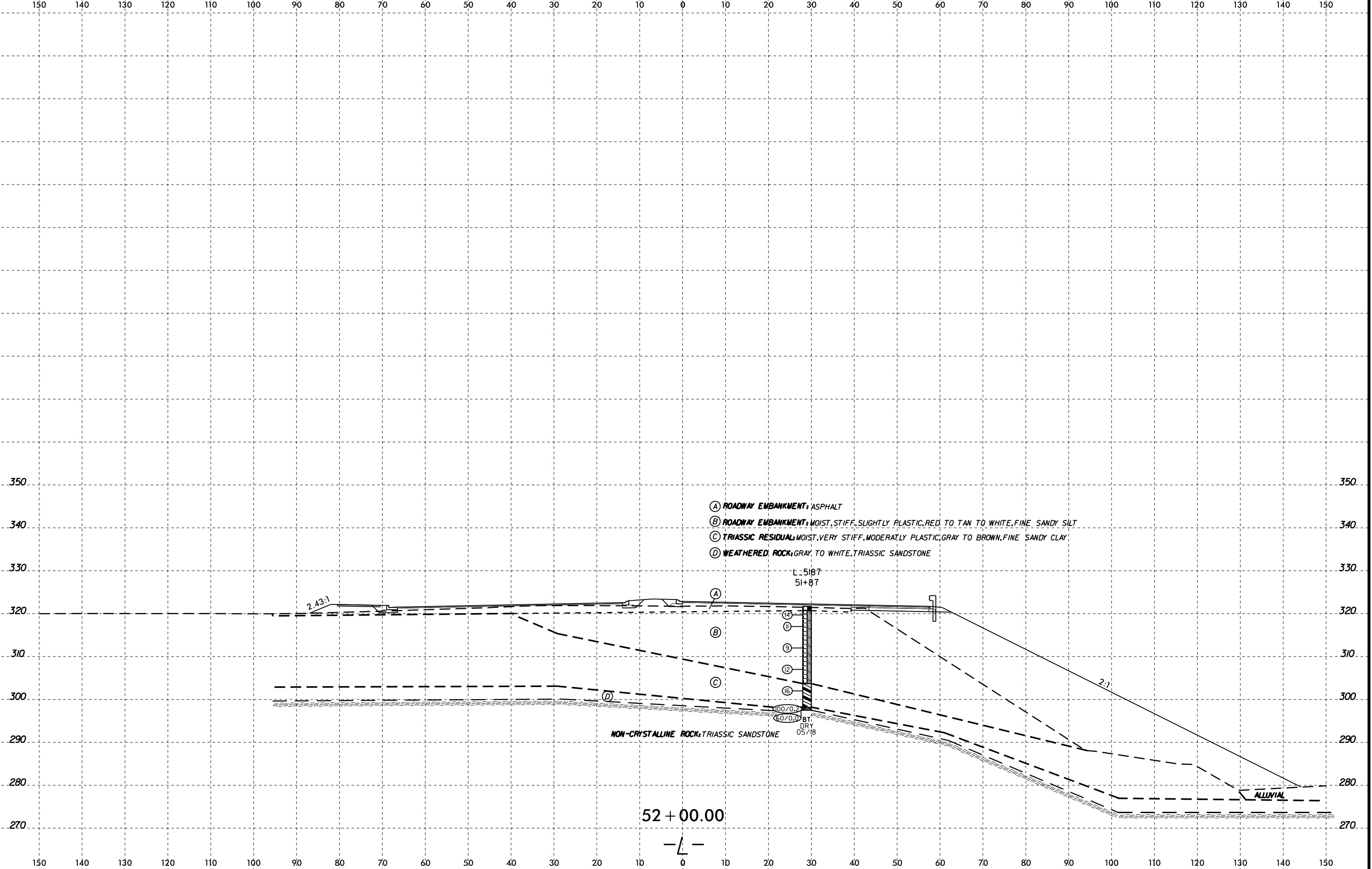


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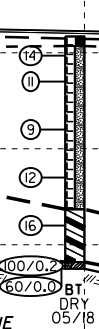


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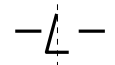
- (A) ROADWAY EMBANKMENT, ASPHALT
- (B) ROADWAY EMBANKMENT, MOIST, STIFF, SLIGHTLY PLASTIC, RED TO TAN TO WHITE, FINE SANDY SILT
- (C) TRIASSIC RESIDUAL, MOIST, VERY STIFF, MODERATLY PLASTIC, GRAY TO BROWN, FINE SANDY CLAY
- (D) WEATHERED ROCK, GRAY TO WHITE, TRIASSIC SANDSTONE

L-5187  
51+87



NON-CRYSTALLINE ROCK, TRIASSIC SANDSTONE

52 + 00.00

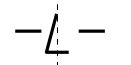
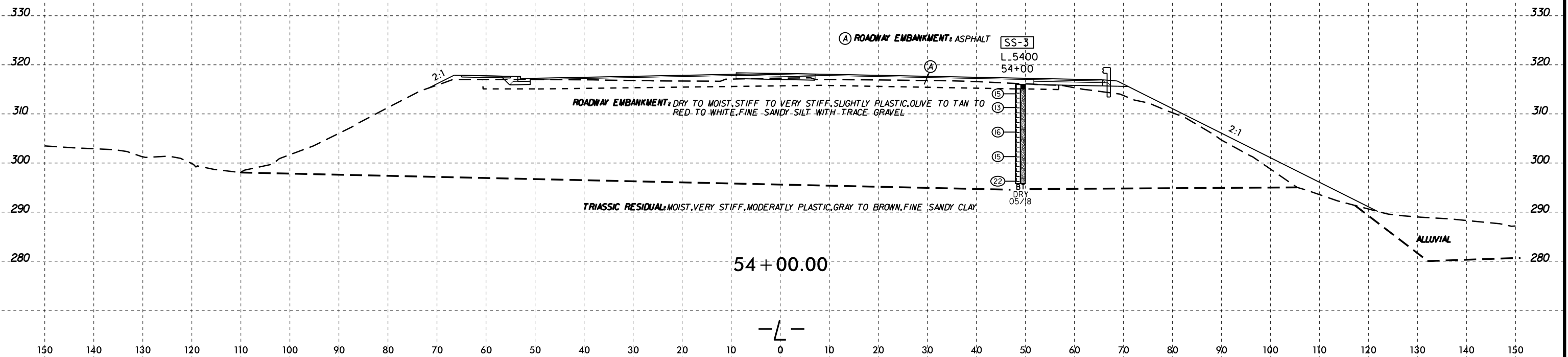


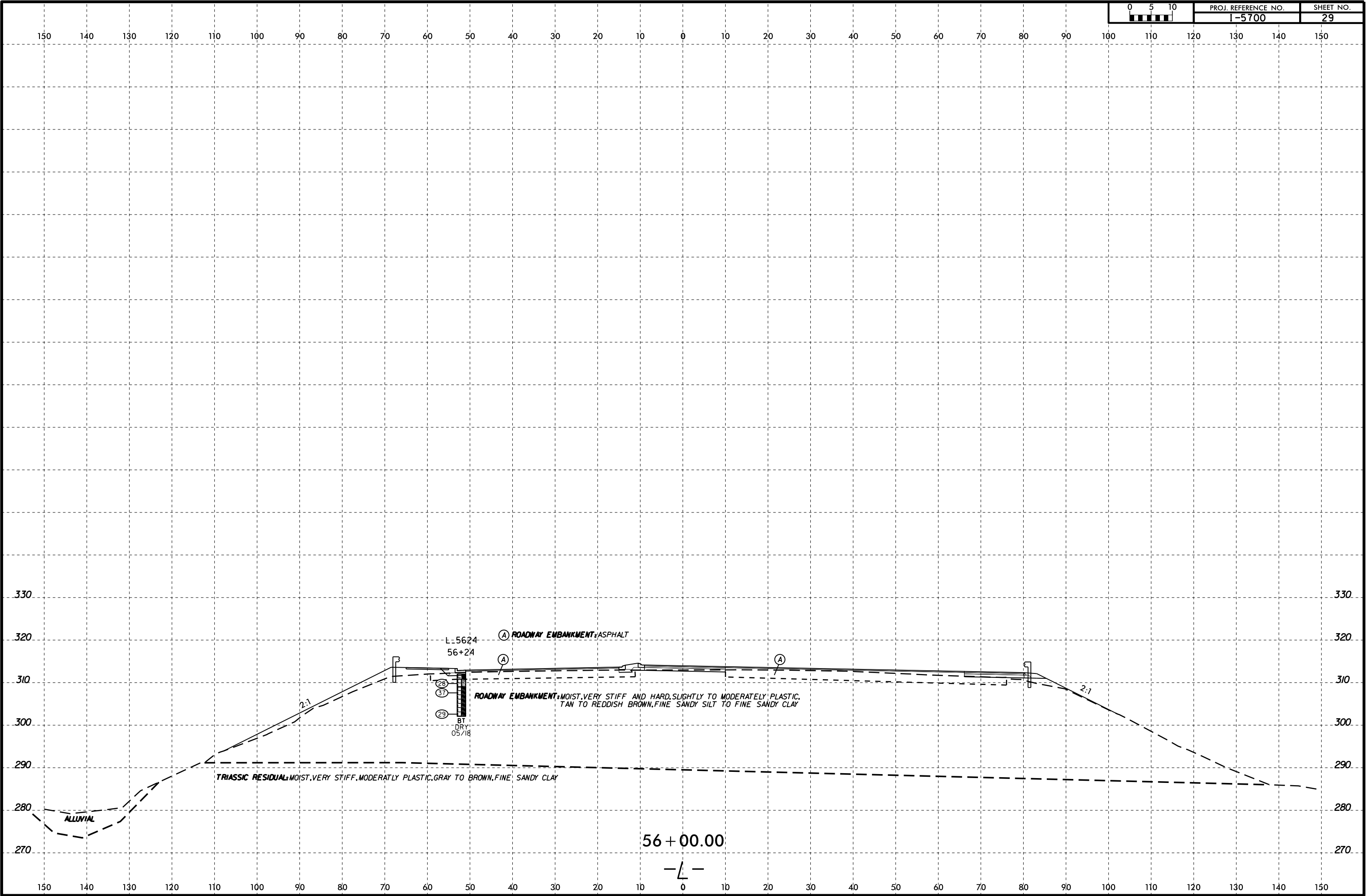
ALLUVIAL



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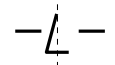
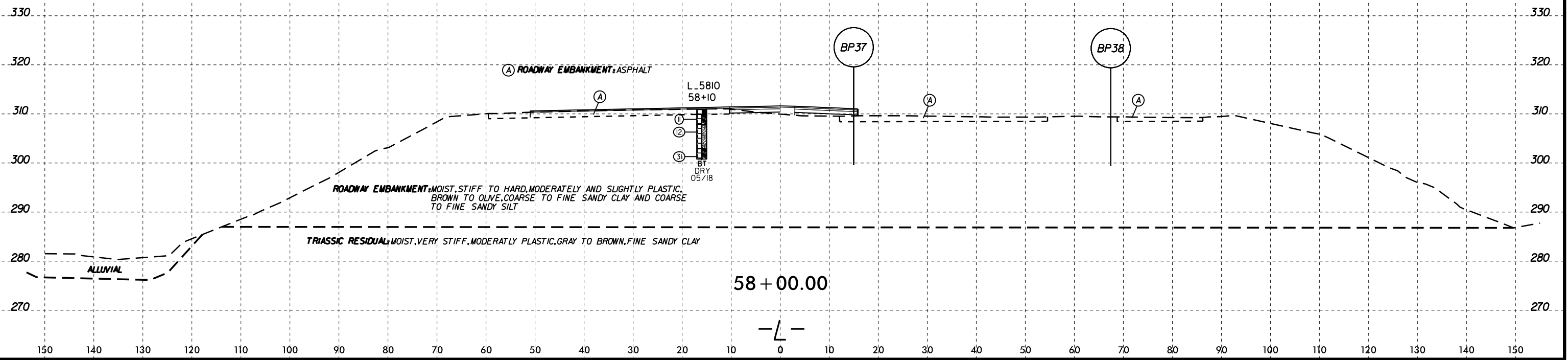
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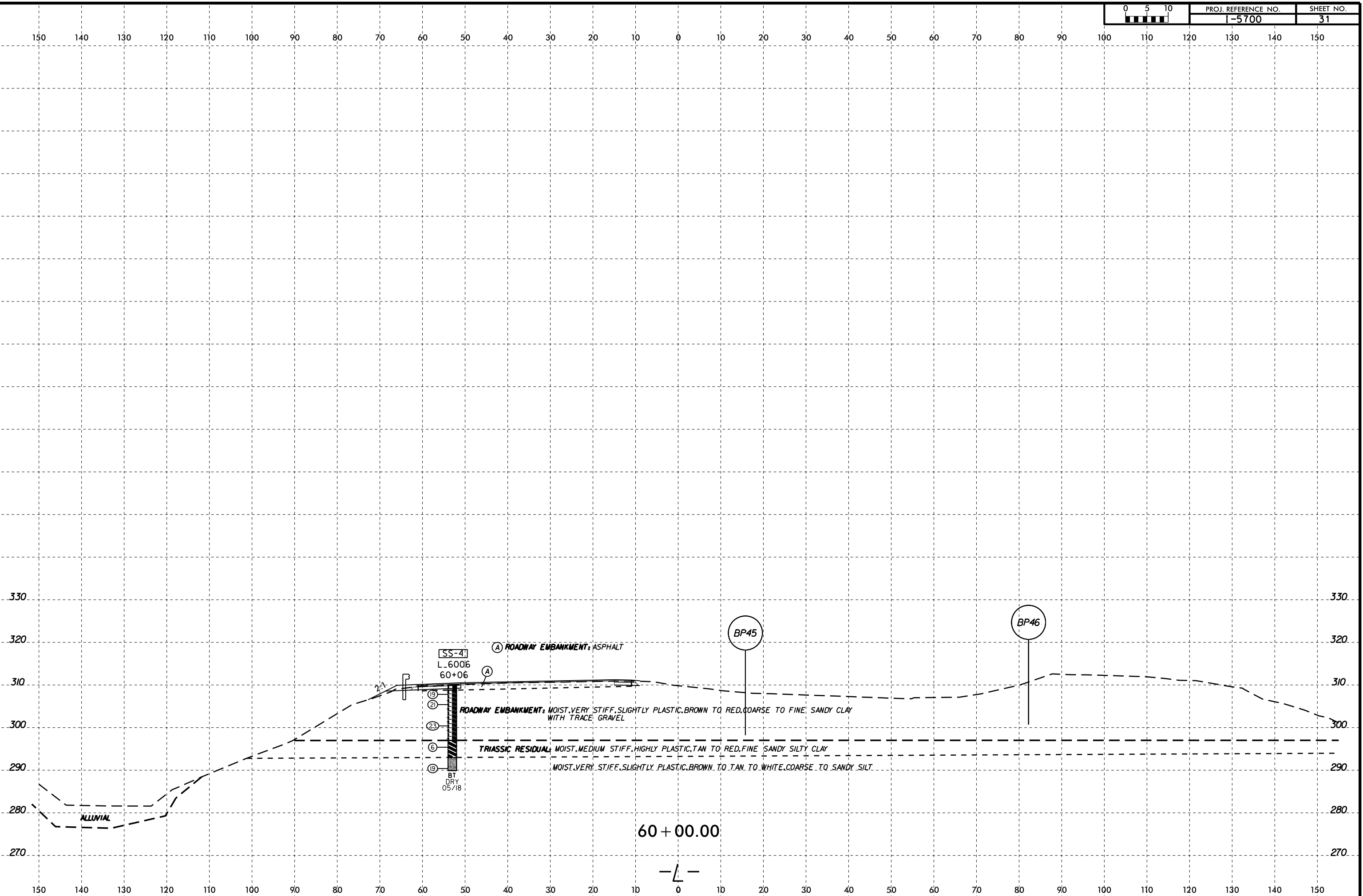


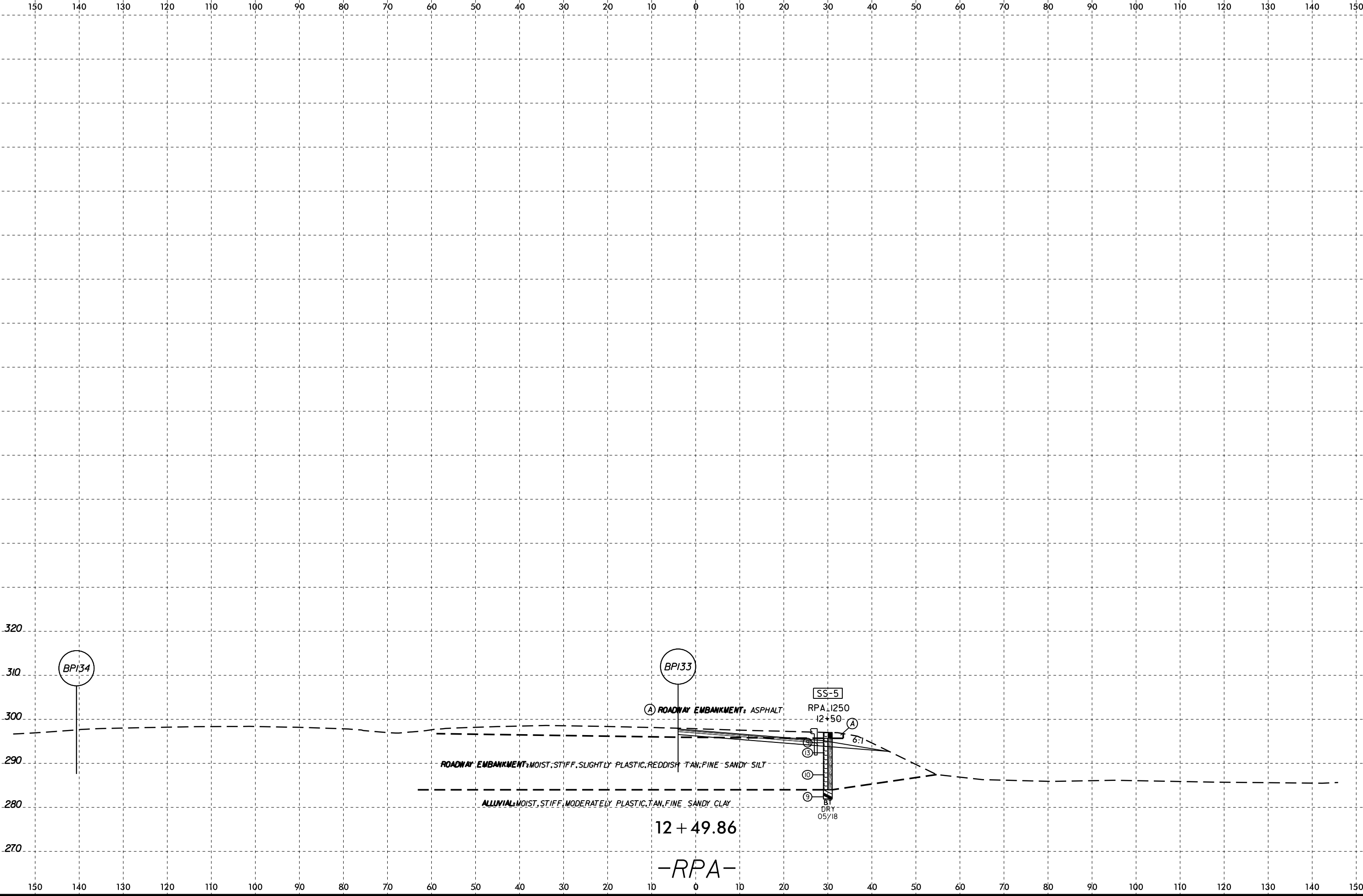


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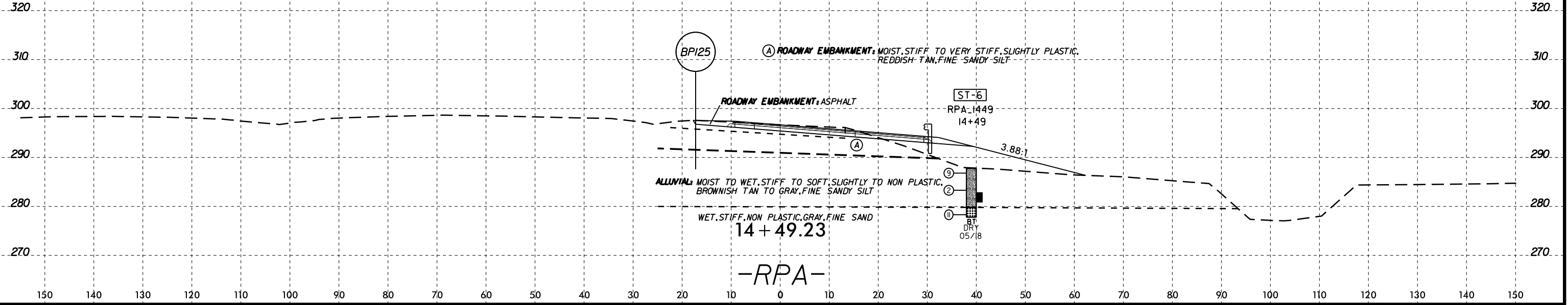








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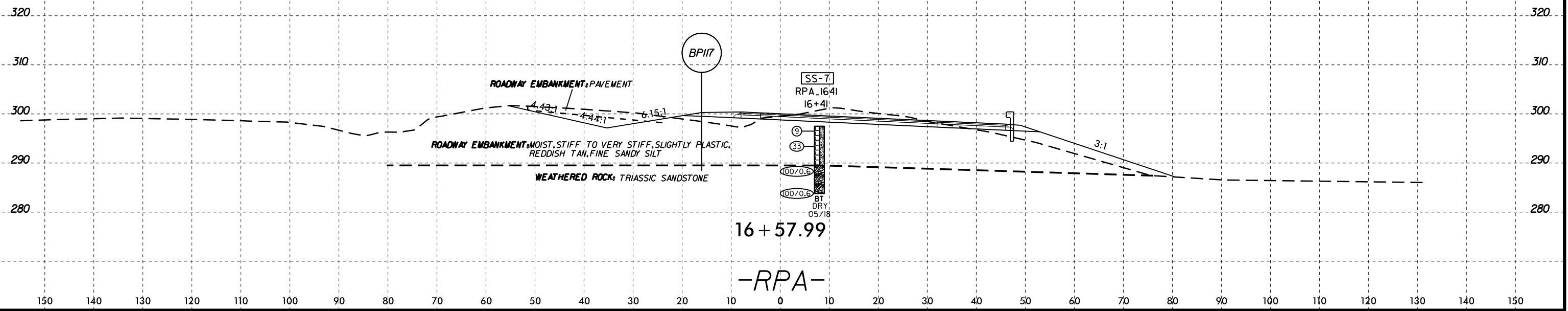


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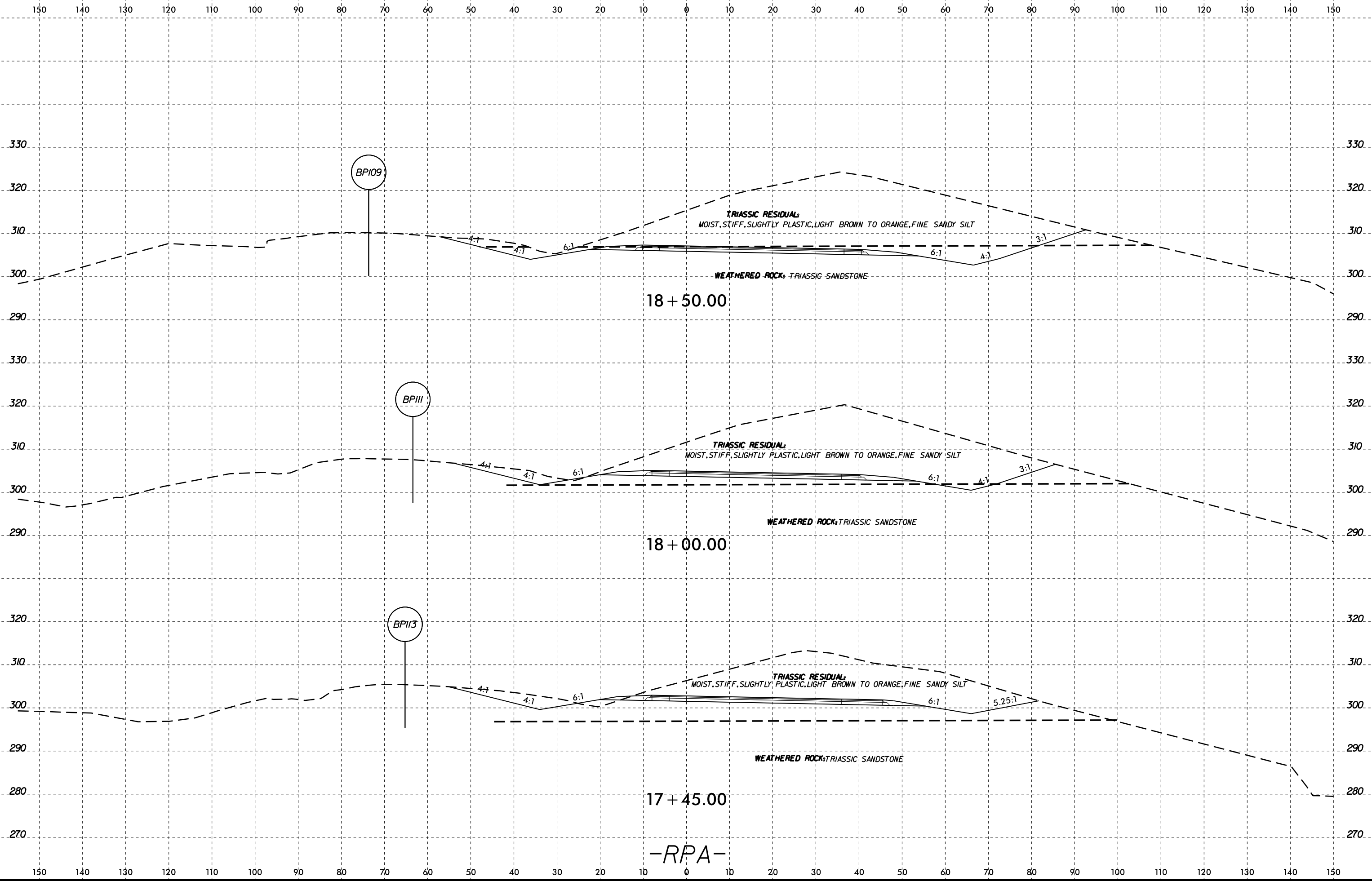
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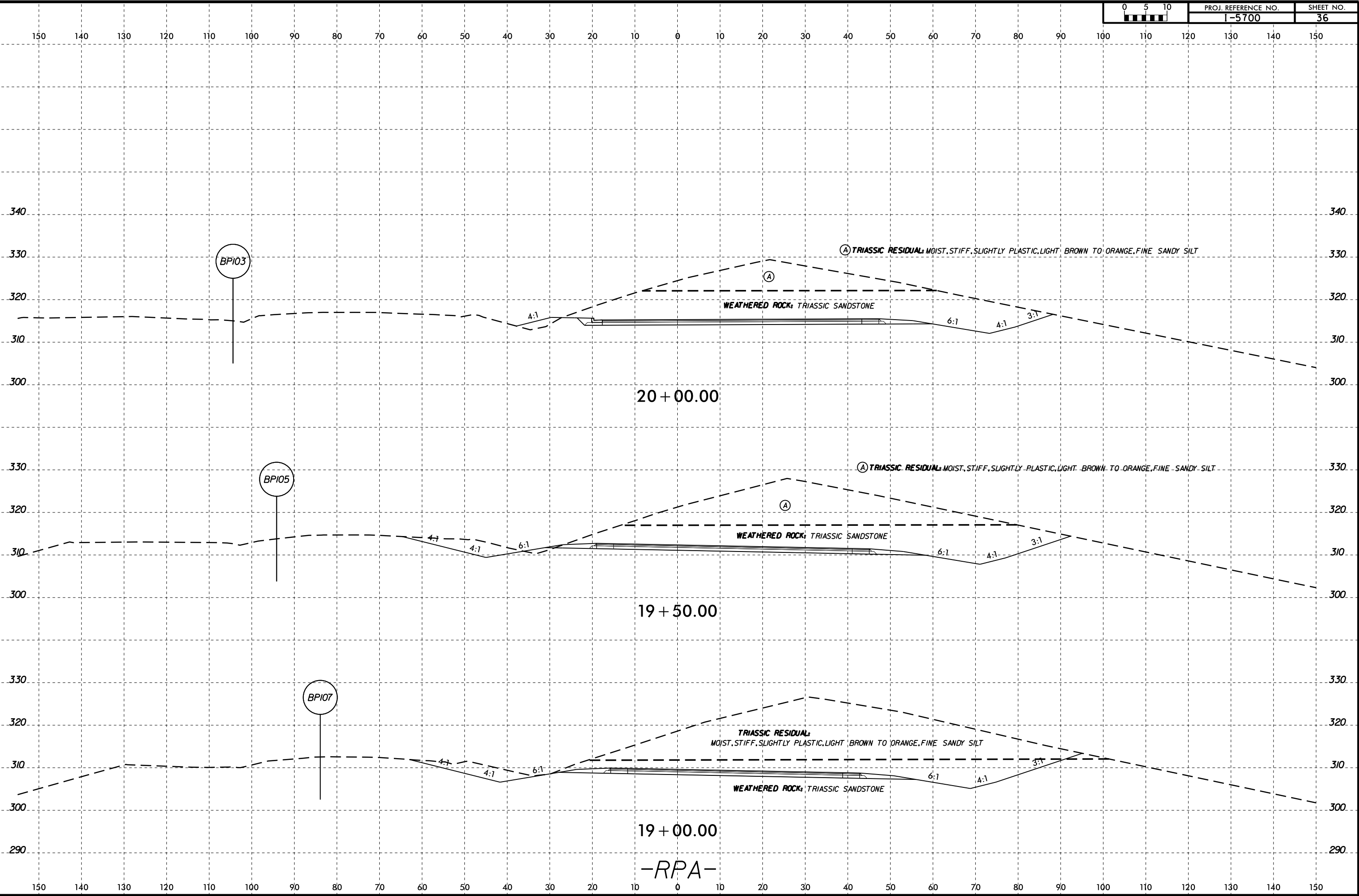


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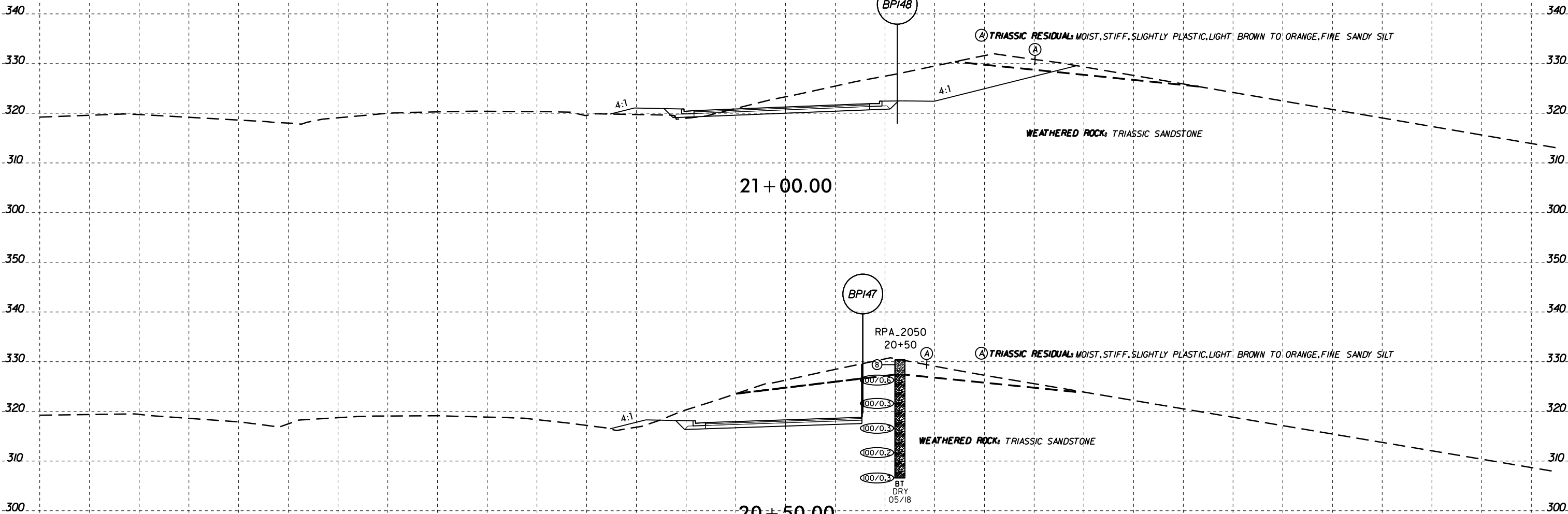


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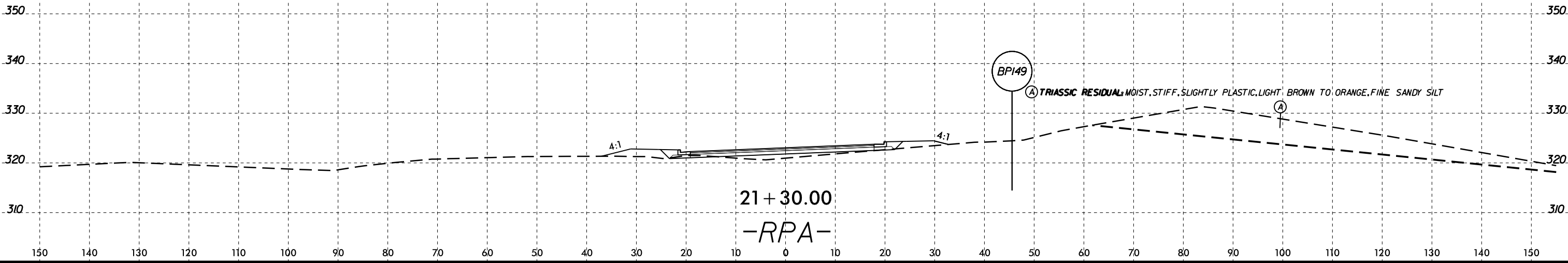
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PROJ. REFERENCE NO.	SHEET NO.
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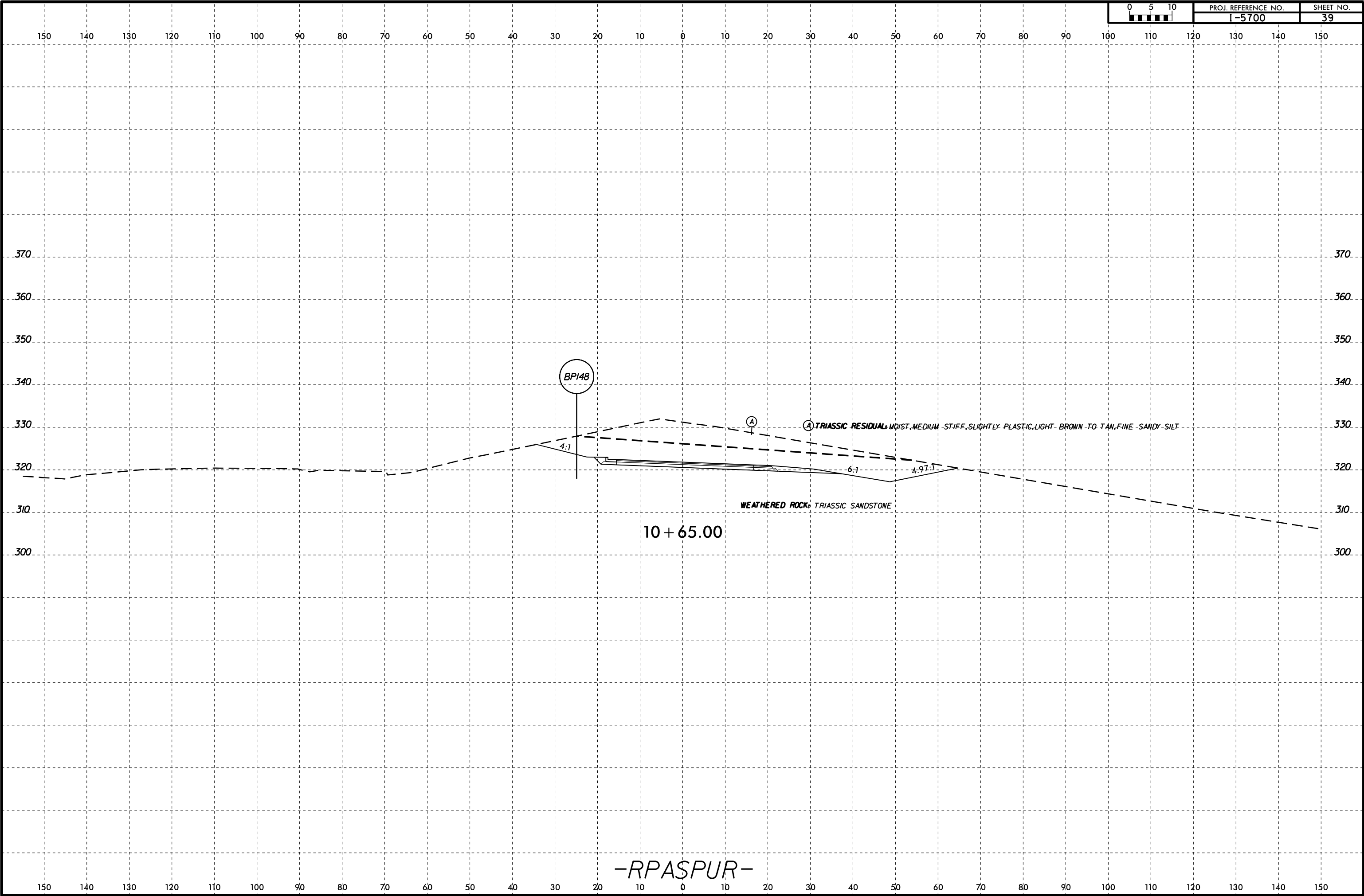
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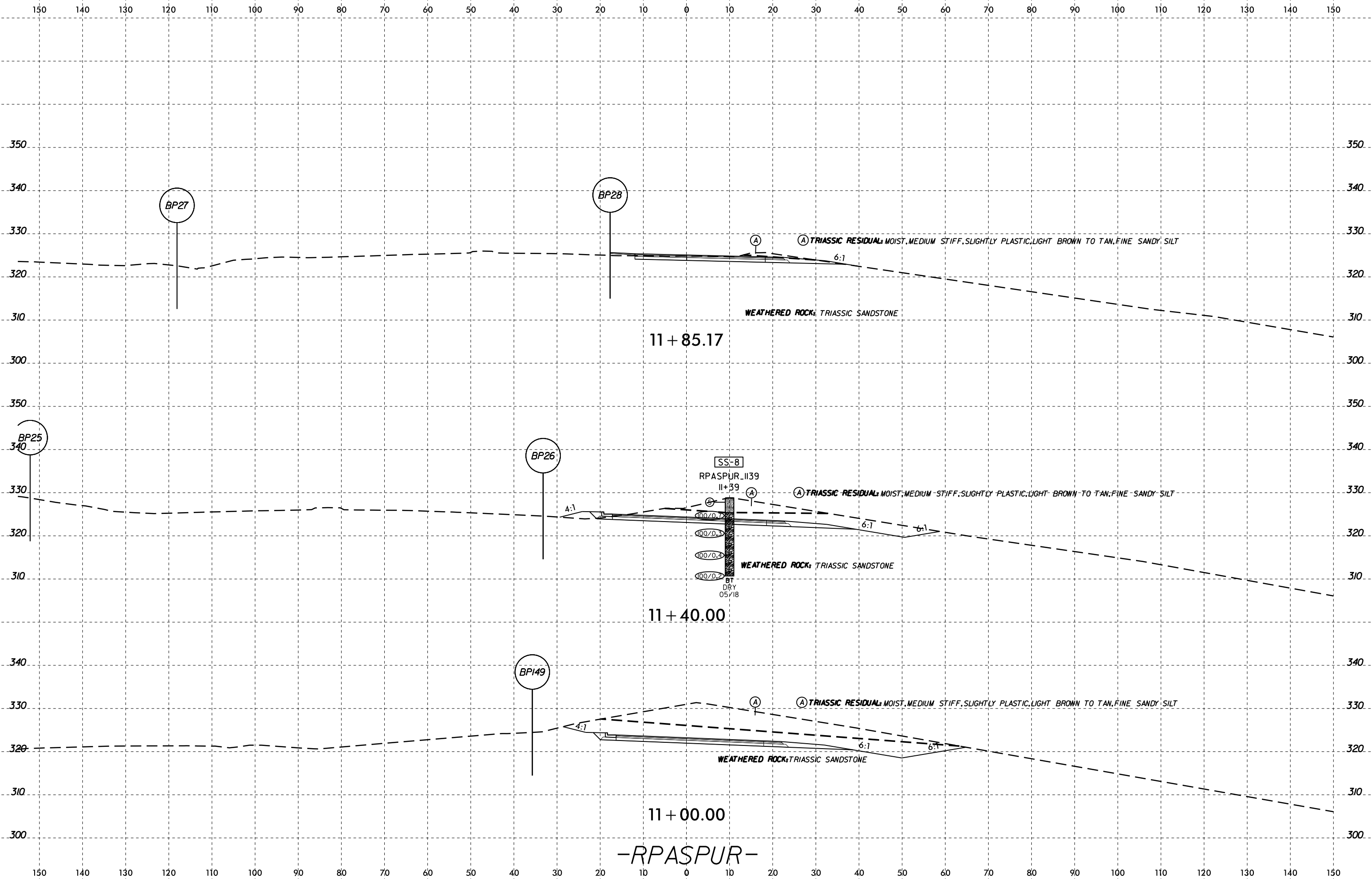
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6/23/16  
twells



-RPASPUR-

6/23/16  
14-AUG-2018 15:01  
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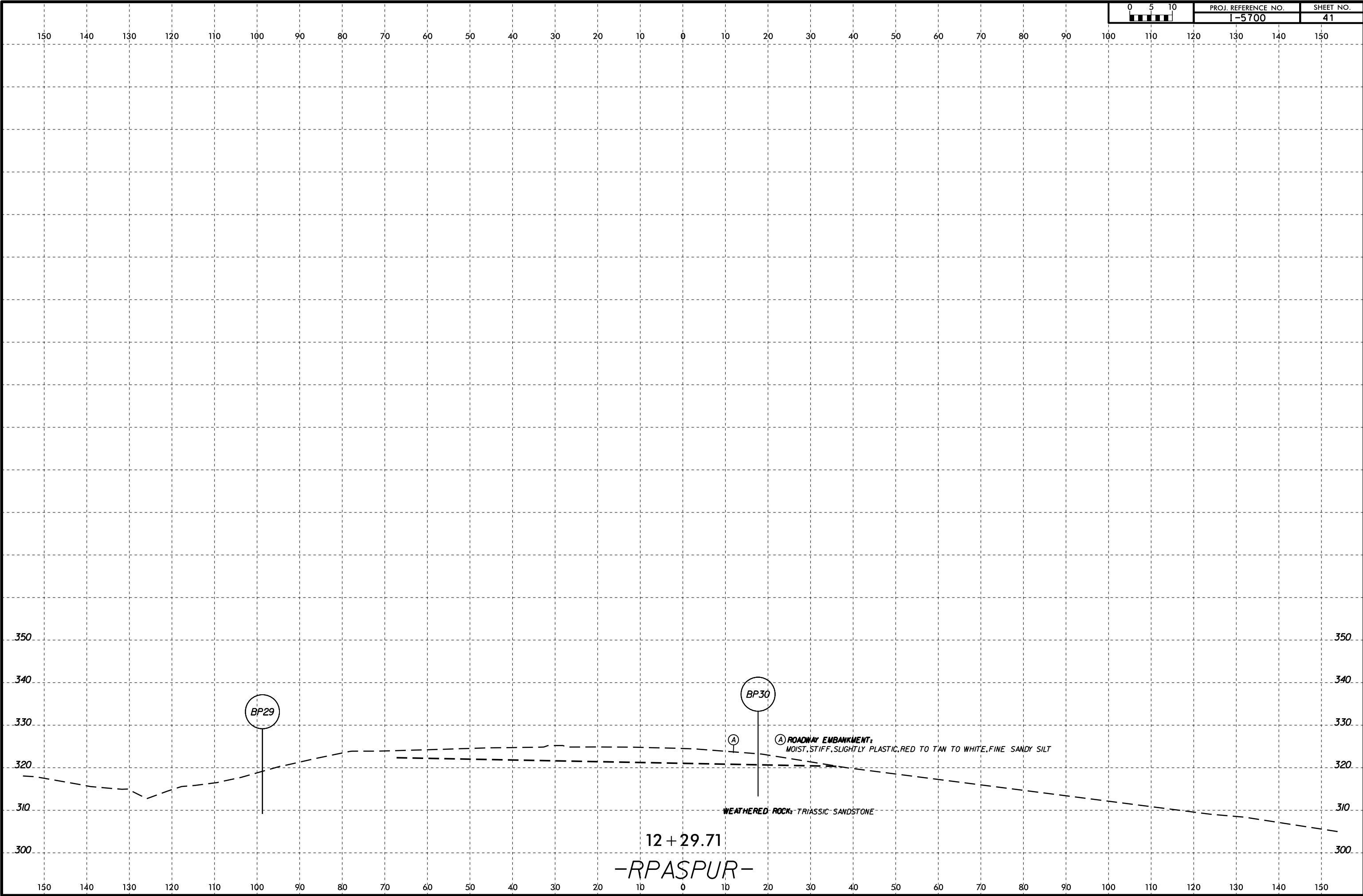
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11 + 40.00

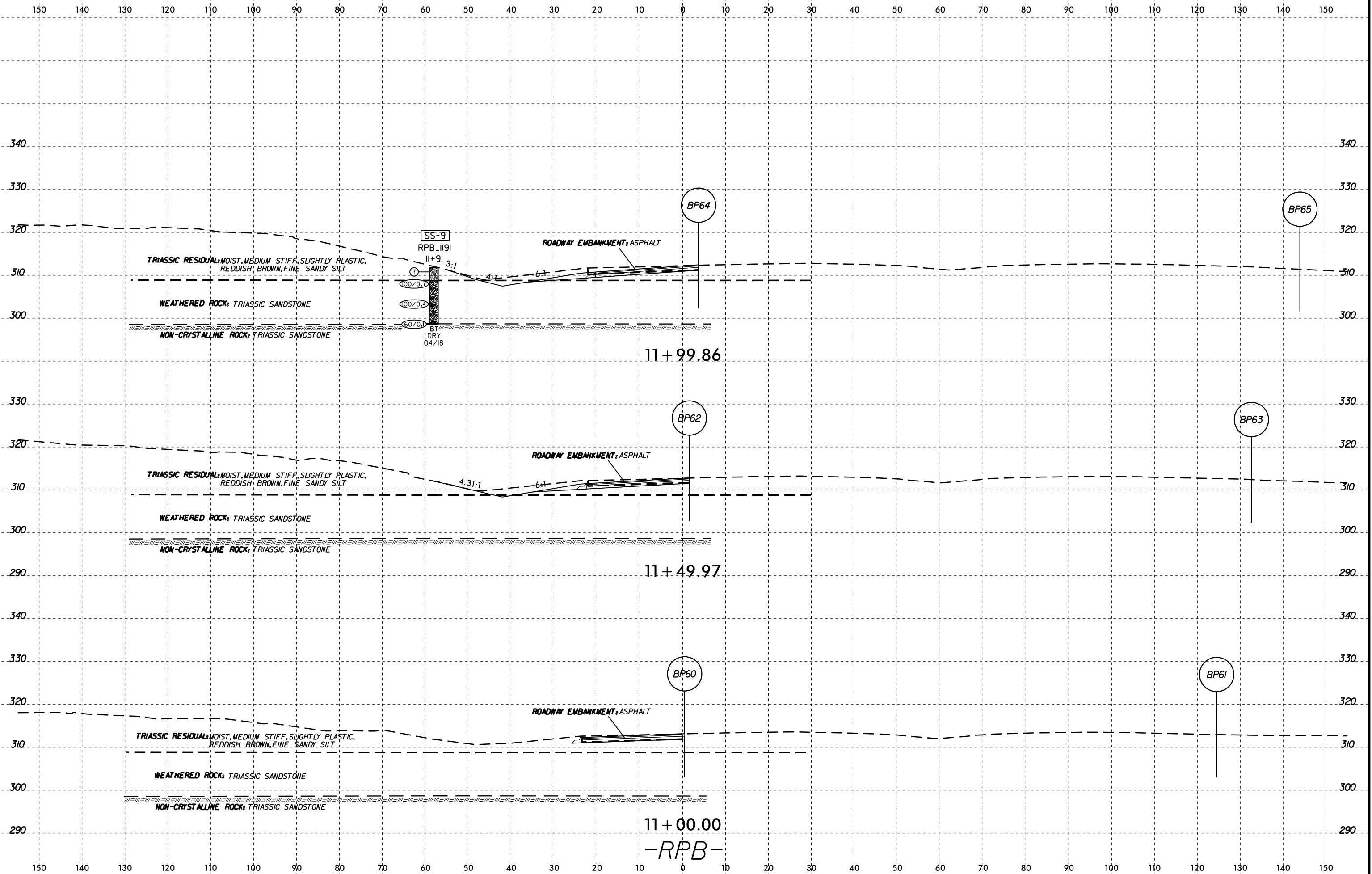
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12 + 29.71  
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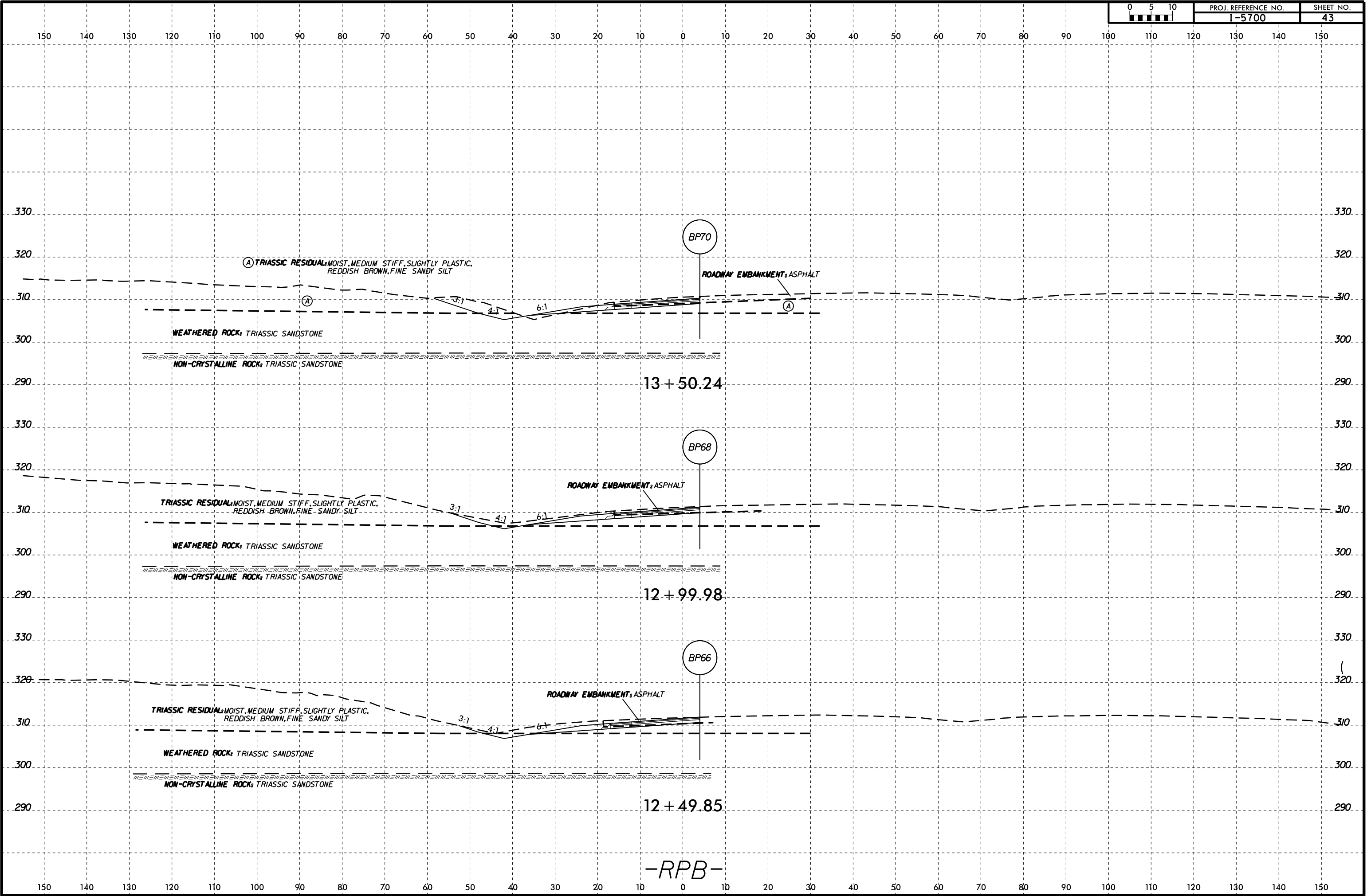


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6/23/16



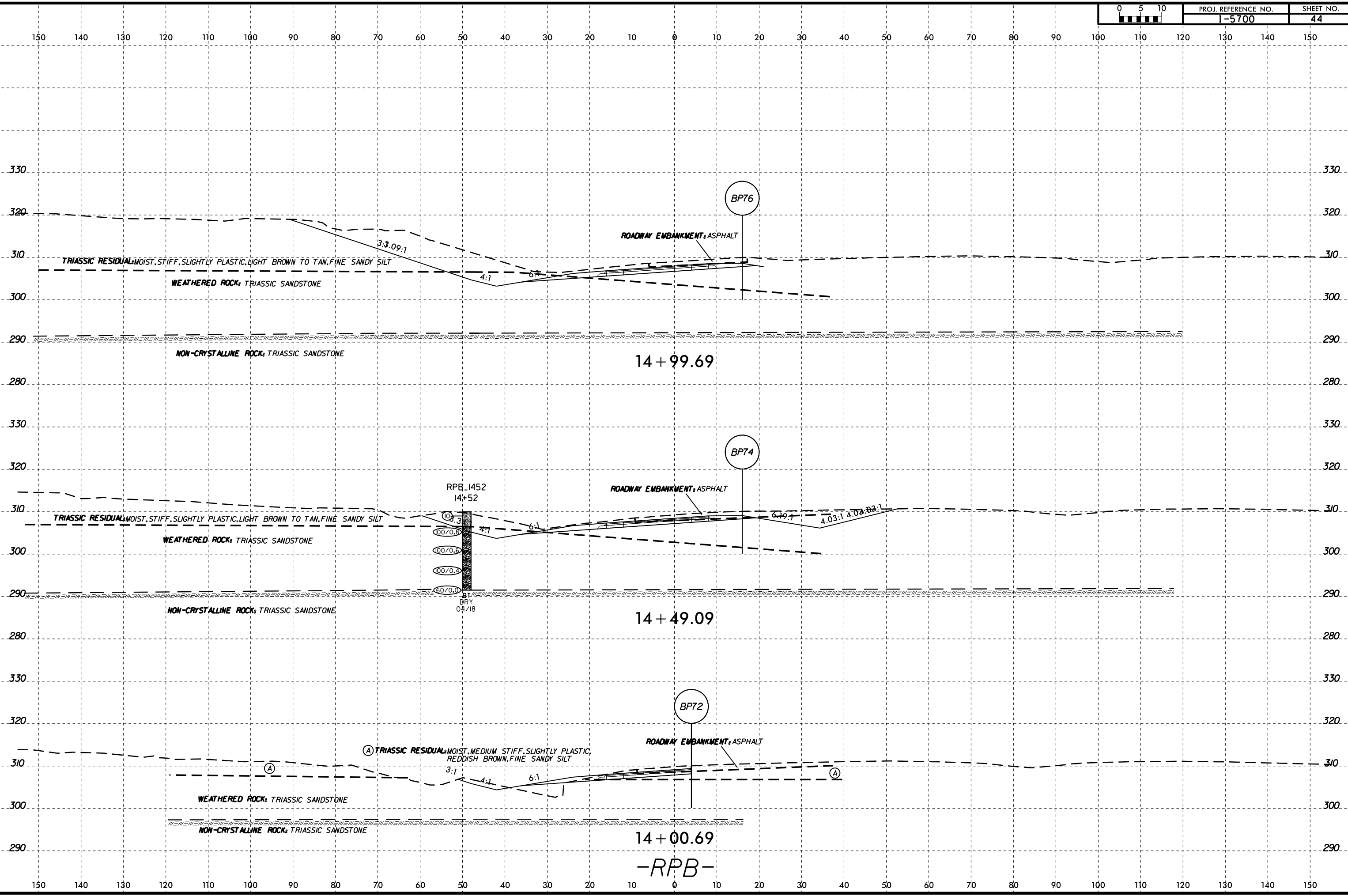
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I-5700	43

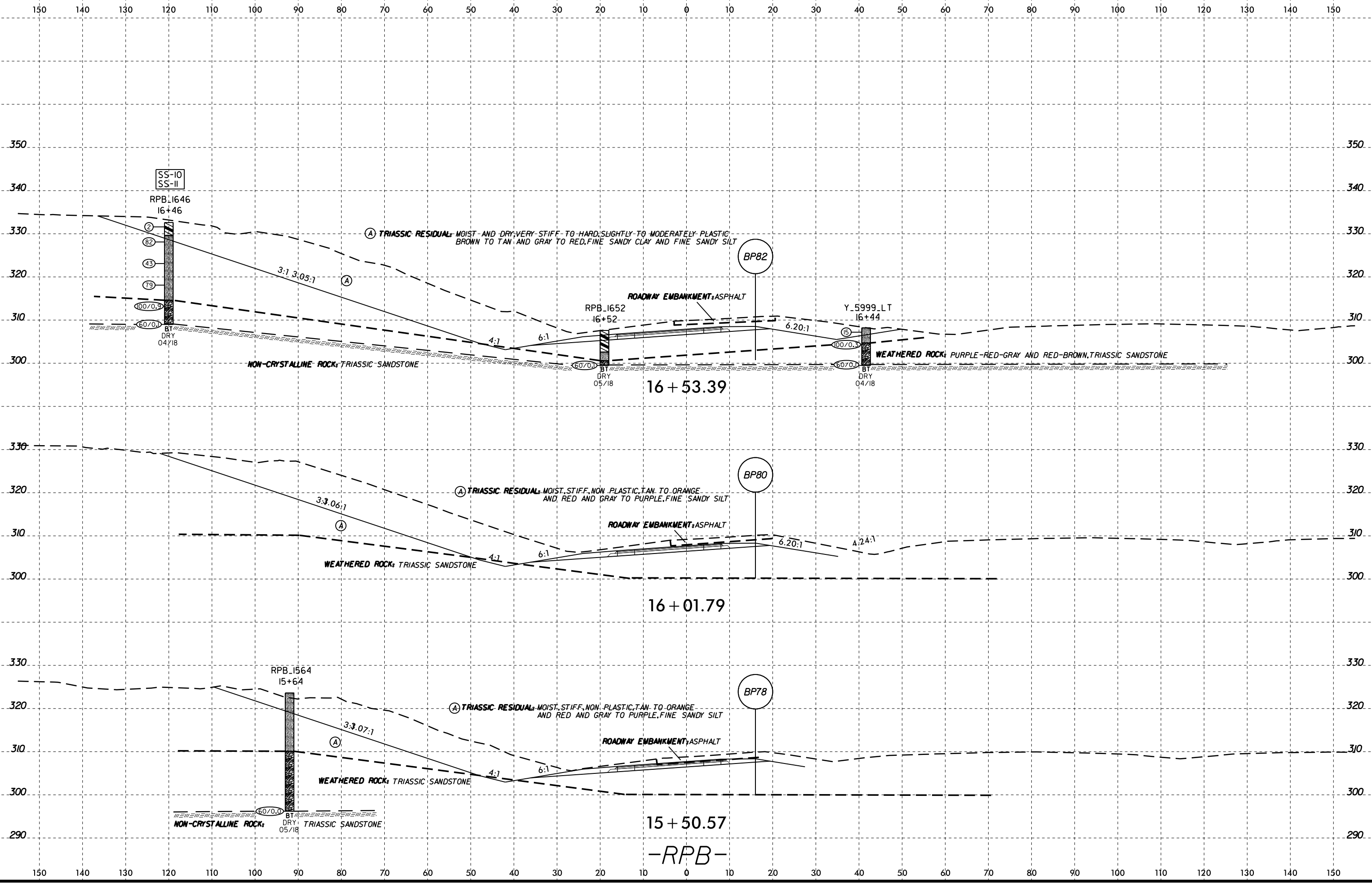


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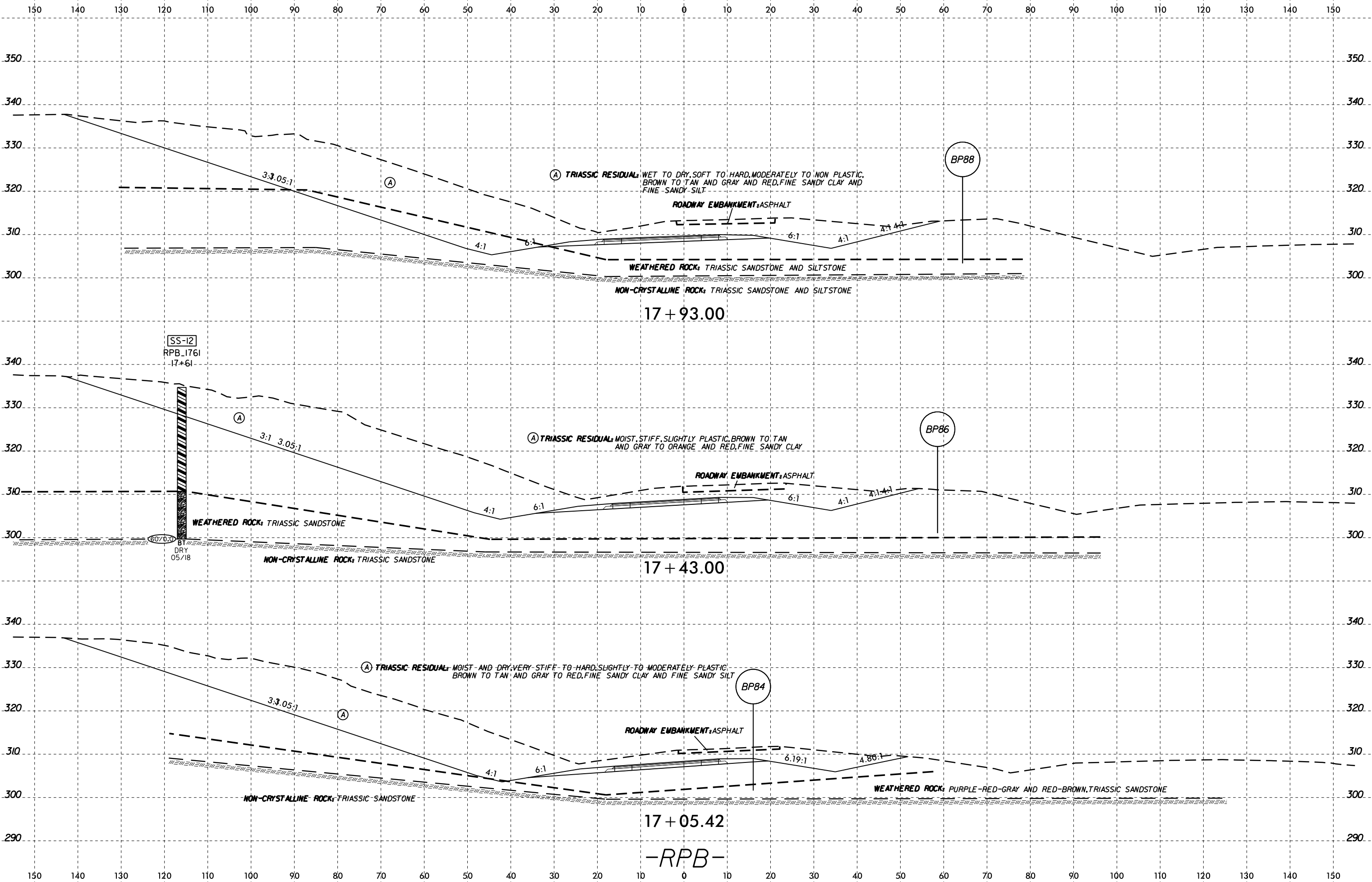
-RPB-

14-AUG-2018 15:01  
W:\shore\GEO\TECHNICAL\Projects\Active Projects\151548.053A 1-5700 Roadway\15700 Roadway\15700\_GEO\RDWY\CADD\_GEO\TECH\15700\_Geo\_xst1\_RPB.dgn  
6/23/16

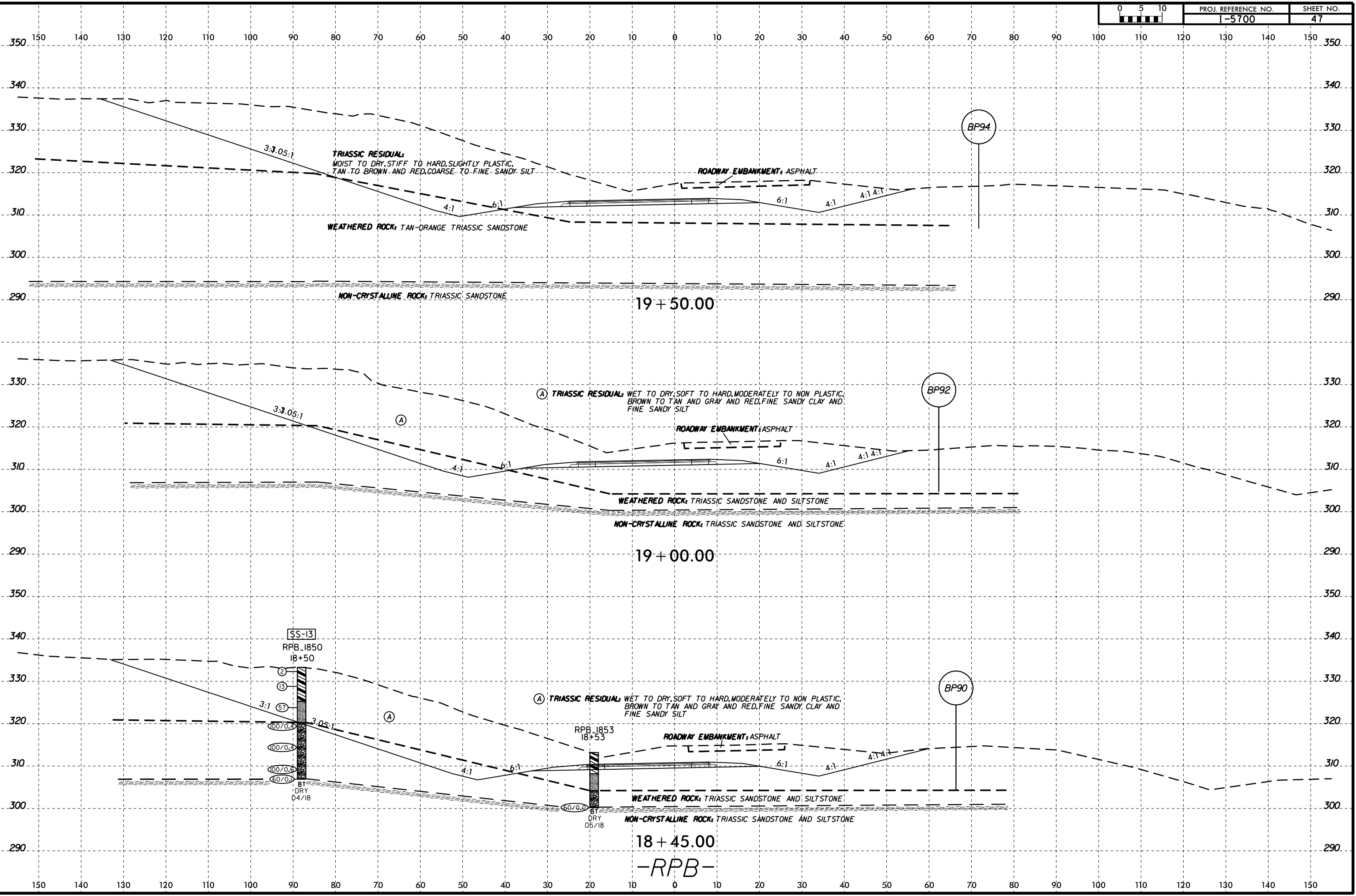


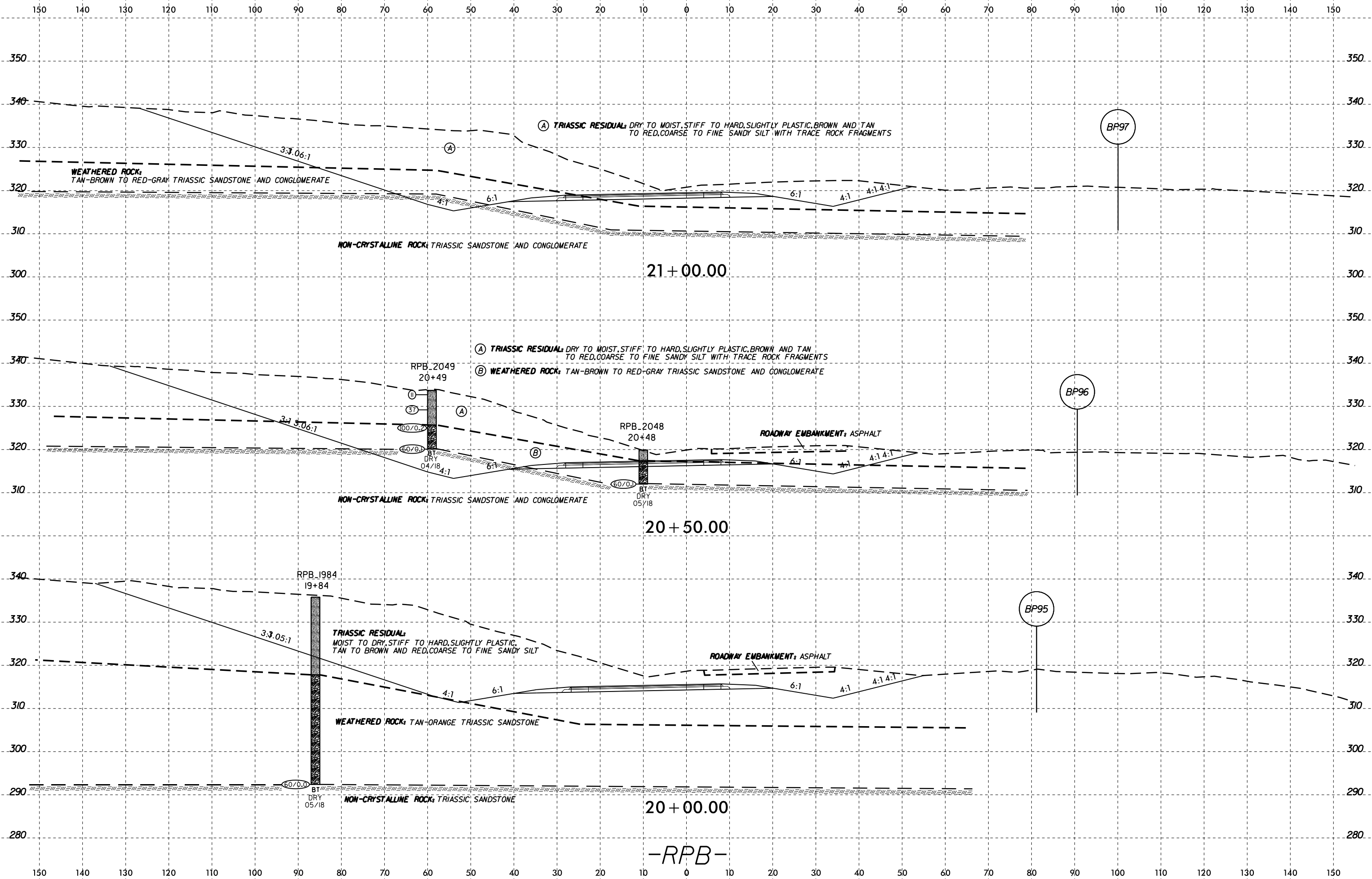


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 14-AUG-2018 15:01  
 W:\shore\GEO\TECH\15700\_GEO\ROADWAY\CADD\GEO\TECH\15700\_GEO\_xst1\_RPB.dgn  
 14-AUG-2018 15:01  
 W:\shore\GEO\TECH\15700\_GEO\ROADWAY\CADD\GEO\TECH\15700\_GEO\_xst1\_RPB.dgn  
 14-AUG-2018 15:01



6/23/16  
 I4-AUG-2018 15:02  
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 Wells

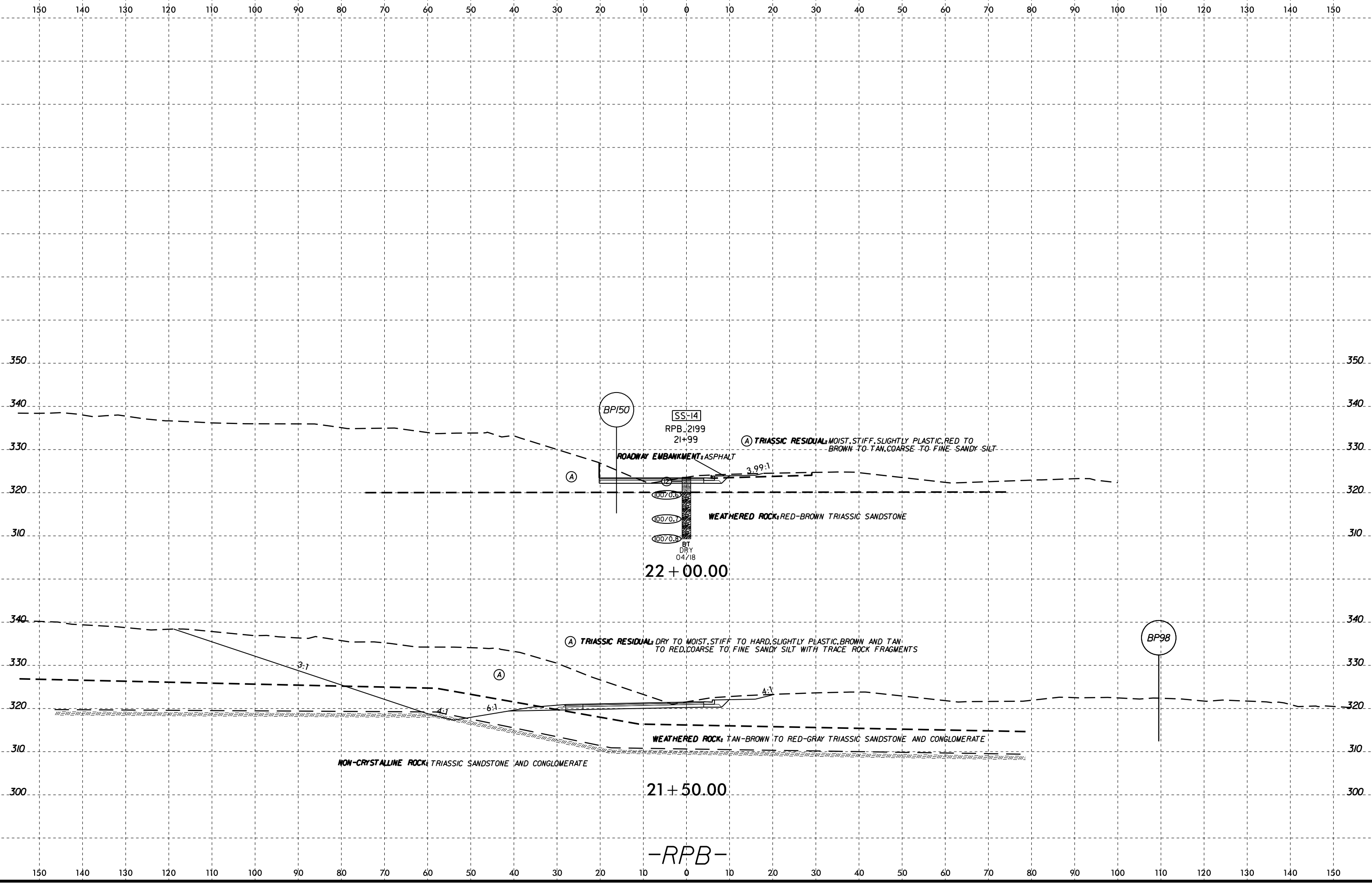




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 At 11/20/2015

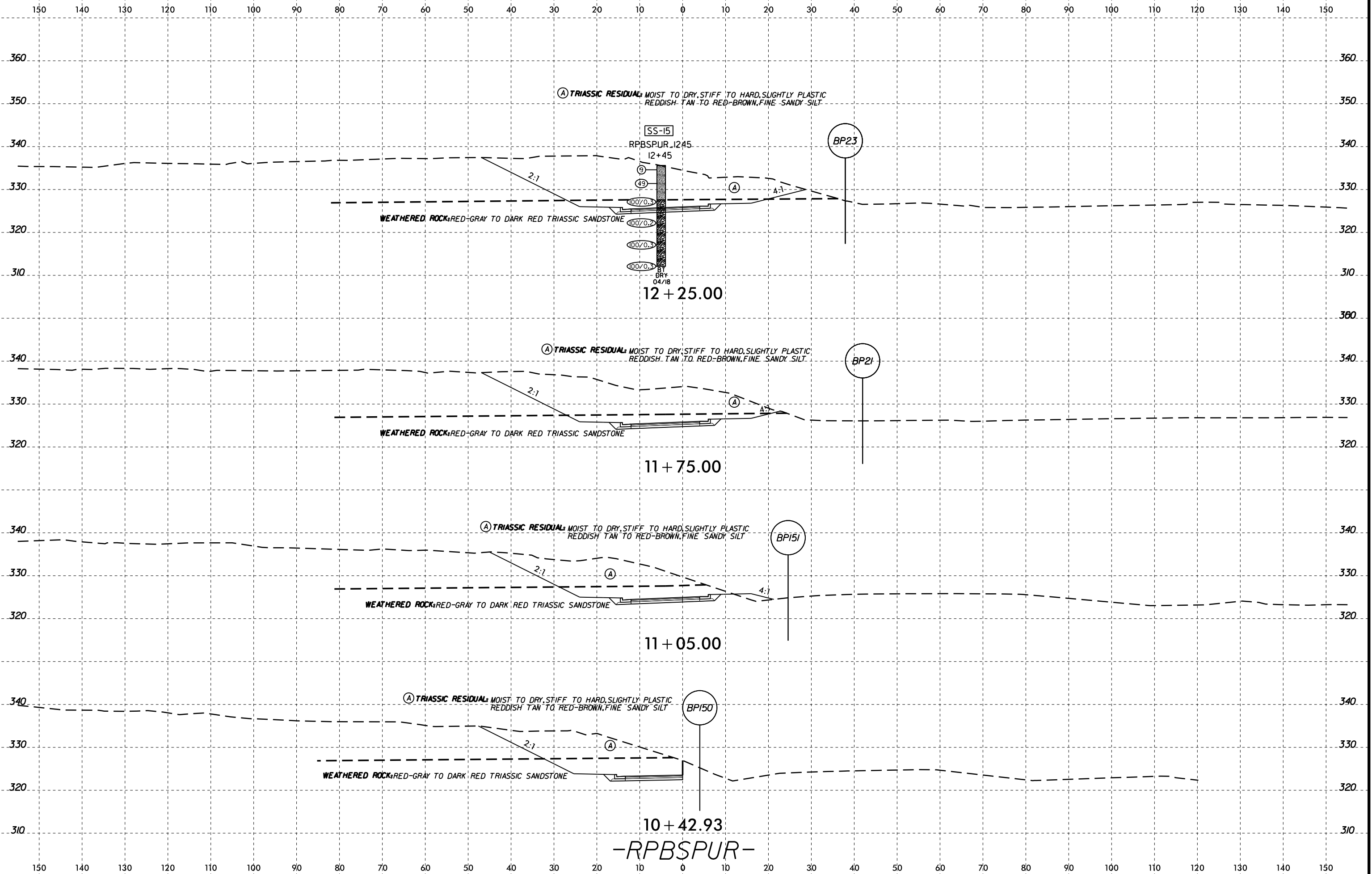
-RPB-





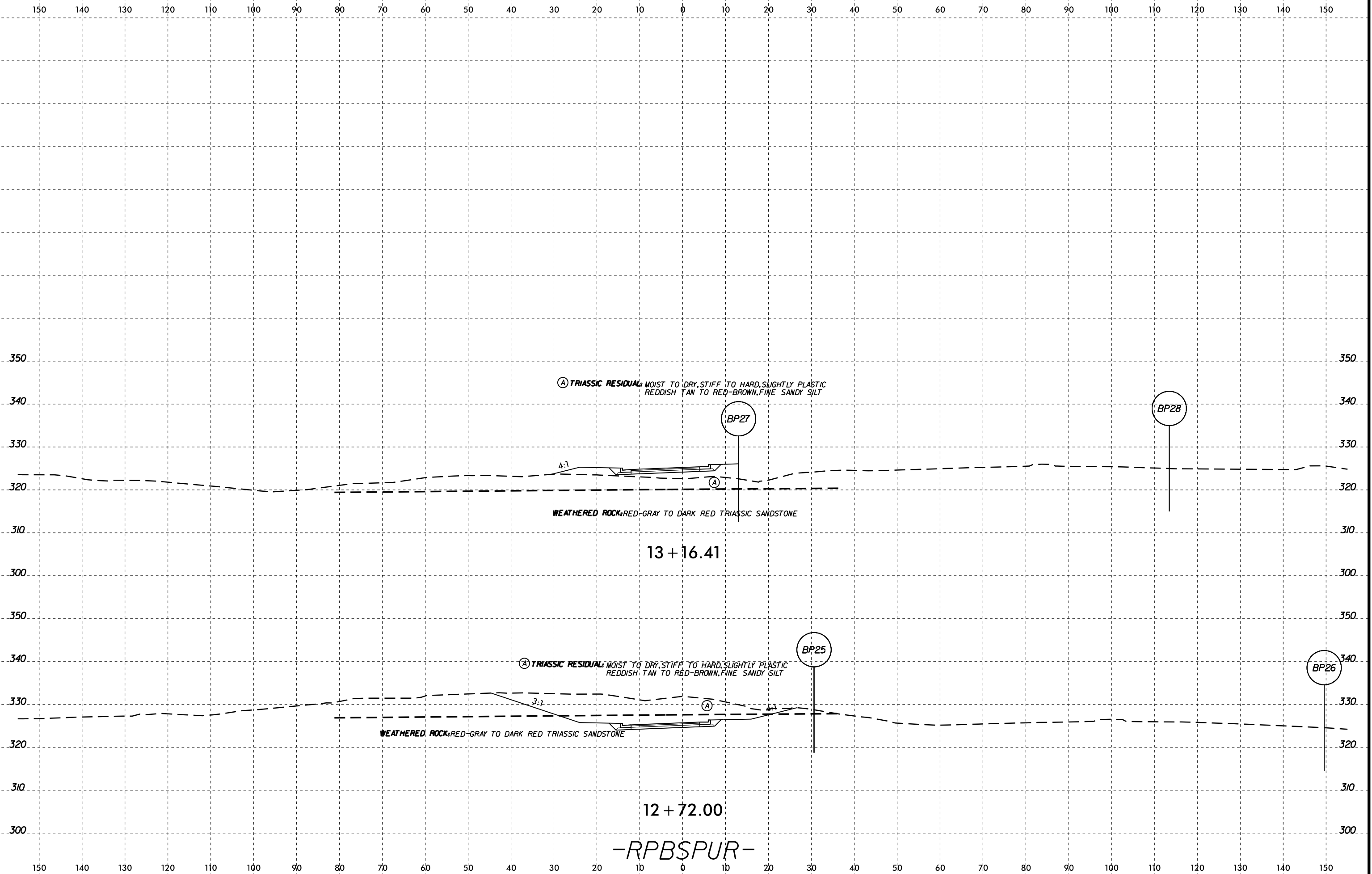
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 Wells At KA20215

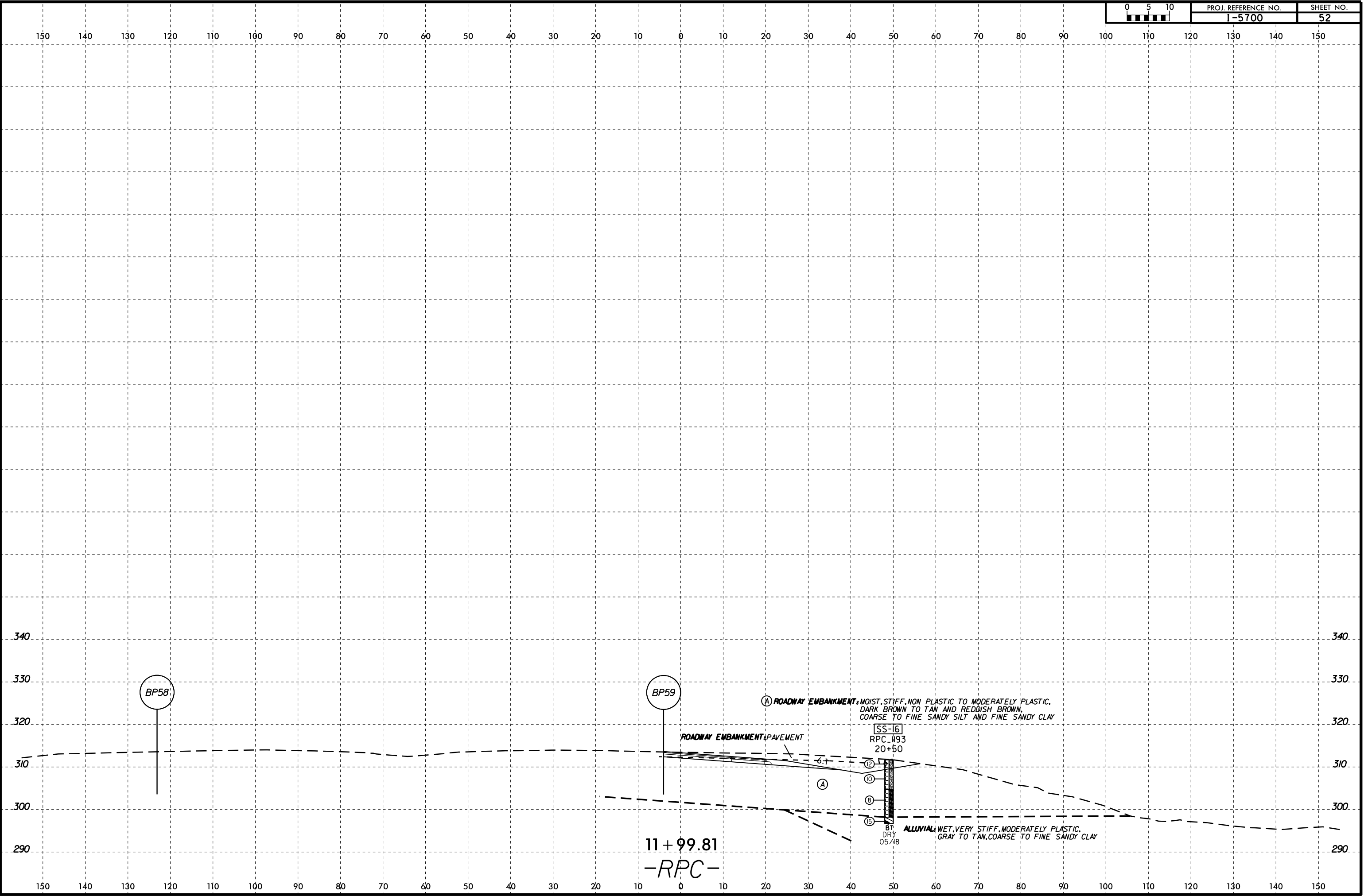
-RPB-



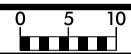
-RPBSPUR-

14-AUG-2018 15:02  
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6/23/16  
15  
15

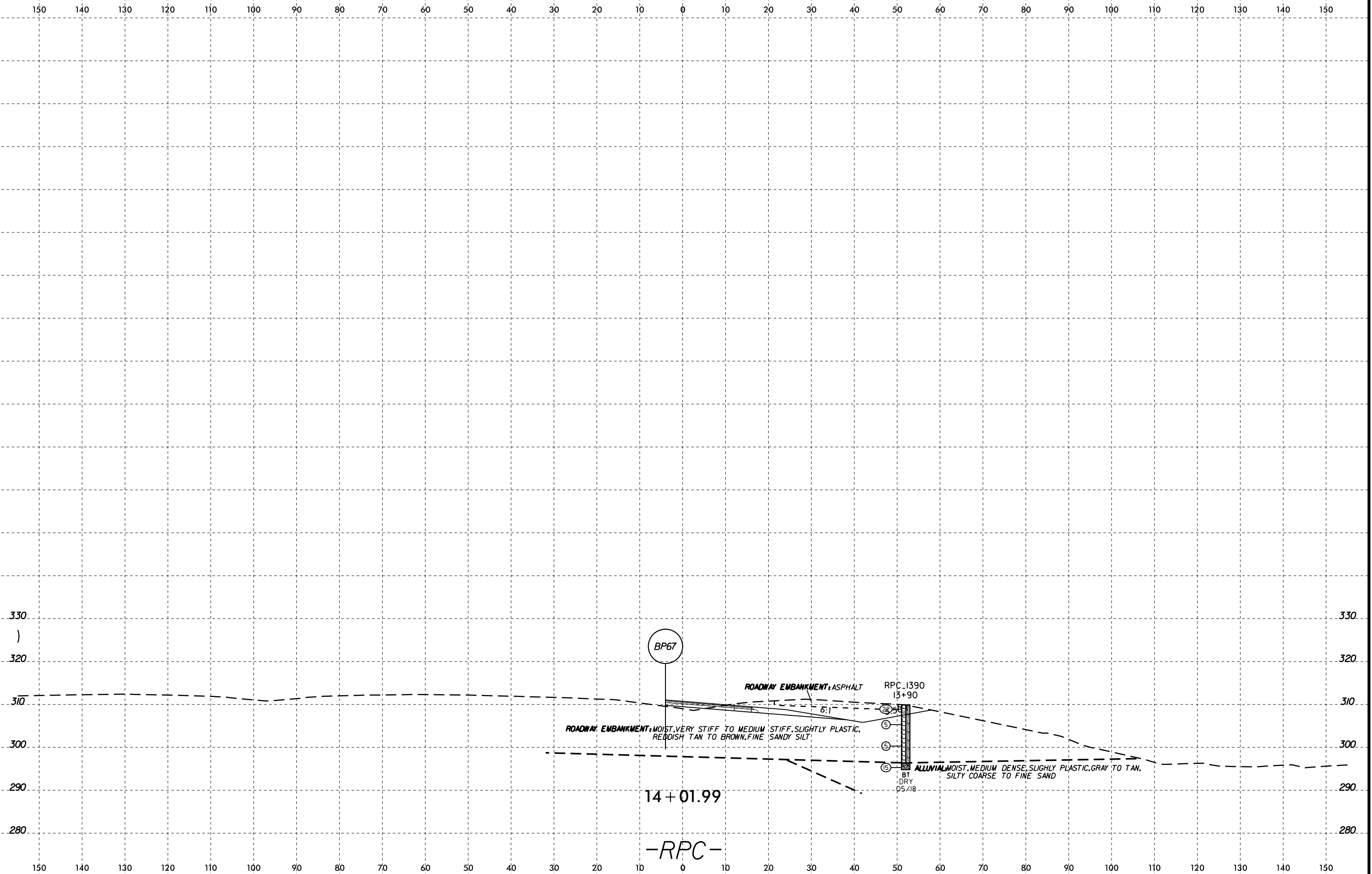




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6/23/16



PROJ. REFERENCE NO.	SHEET NO.
1-5700	53



BP67

ROADWAY EMBANKMENT: ASPHALT

ROADWAY EMBANKMENT: MOIST, VERY STIFF TO MEDIUM STIFF, SLIGHTLY PLASTIC, REDDISH TAN TO BROWN, FINE SANDY SILT

RPC 1390  
13+90

6:1

22.5

5

5

15

BT  
DRY  
05/18

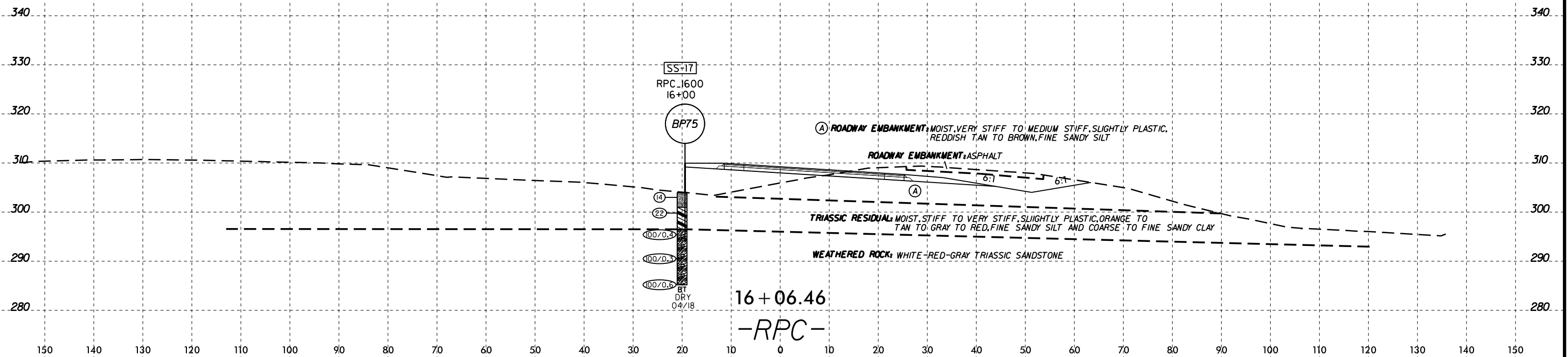
ALLUVIAL MOIST, MEDIUM DENSE, SLIGHTLY PLASTIC, GRAY TO TAN, SILTY COARSE TO FINE SAND

14 + 01.99

-RPC-



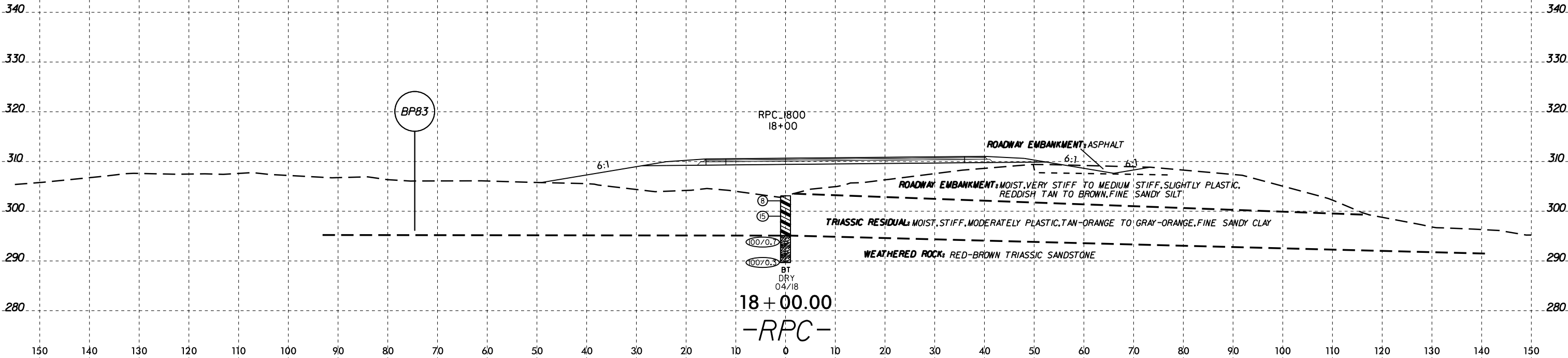
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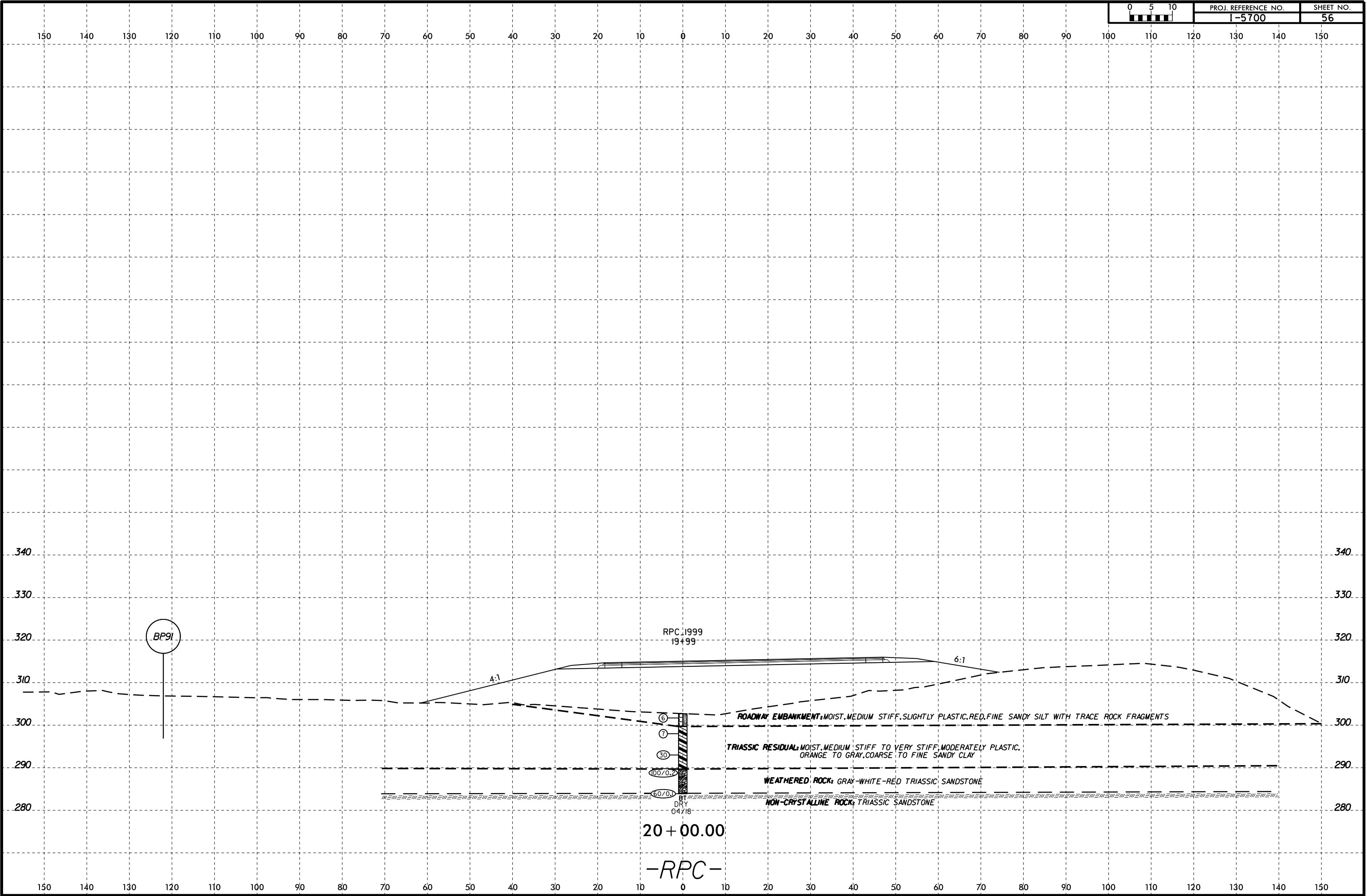


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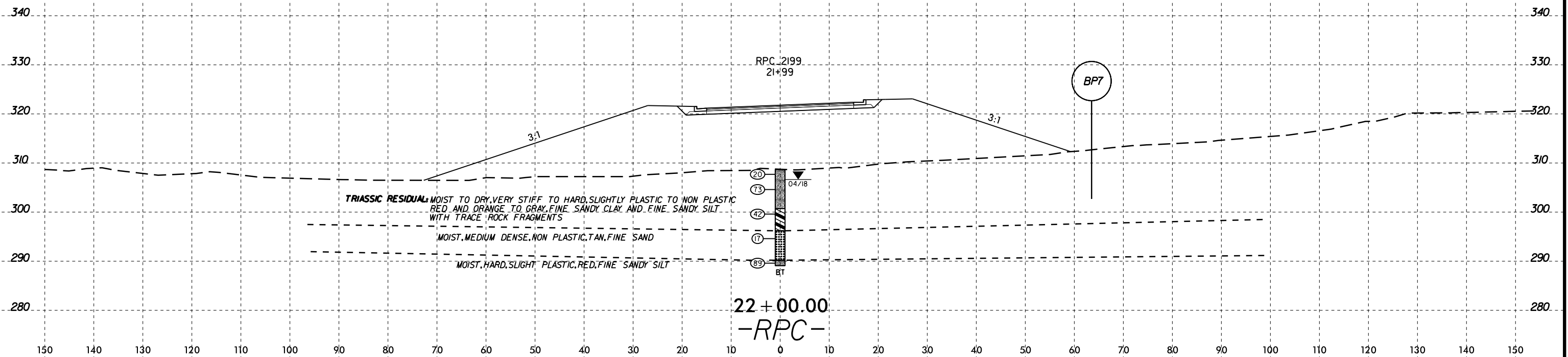








150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



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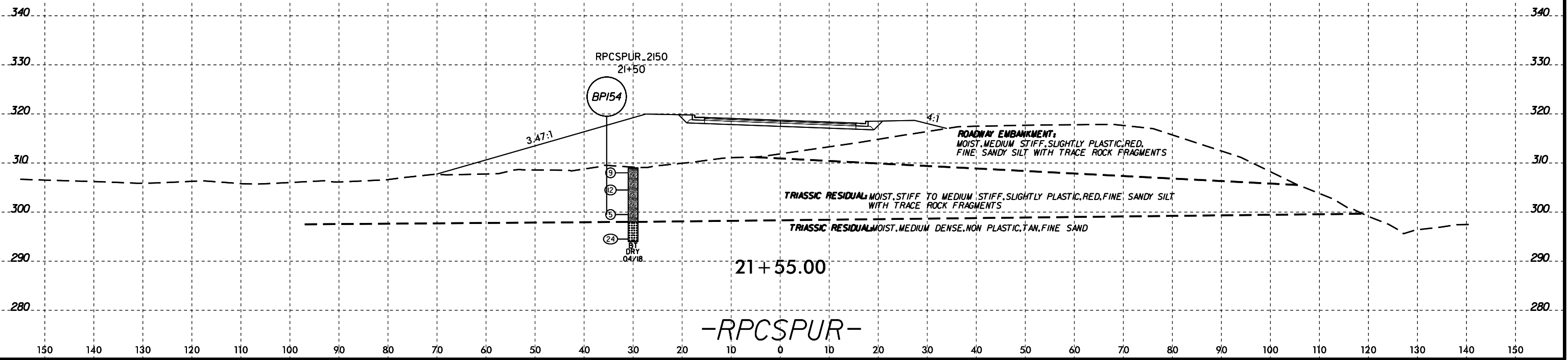
6/23/16



PROJ. REFERENCE NO.	SHEET NO.
-5700	58

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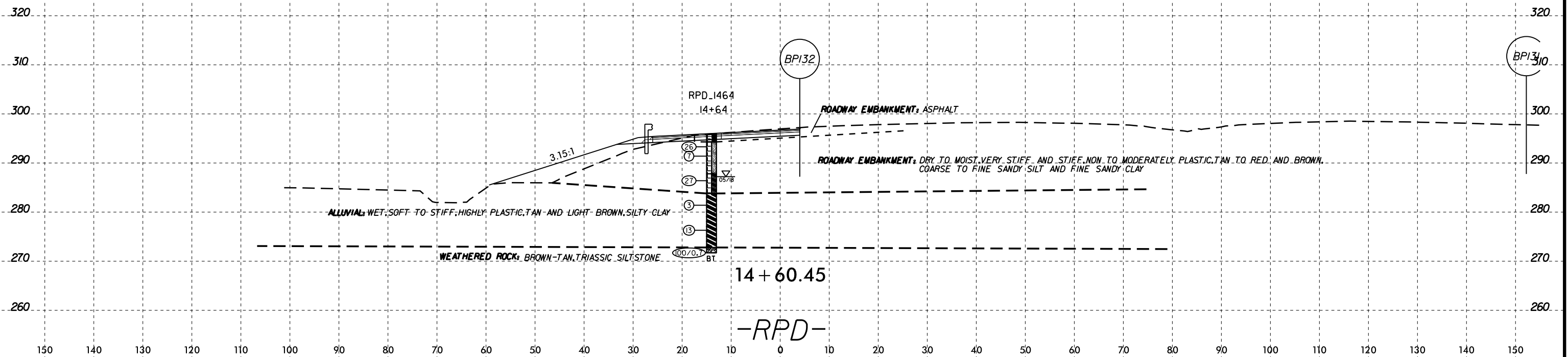
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150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

I:\4-AUG-2018 15:02\W:\shore\GEO\TECHNICAL\Projects\Active Projects\151548\053A I-5700 Roadway\15700\_GEO\RDW\CADD\_GEO\TECH\15700\_Geo\_xst\_RPD.dgn



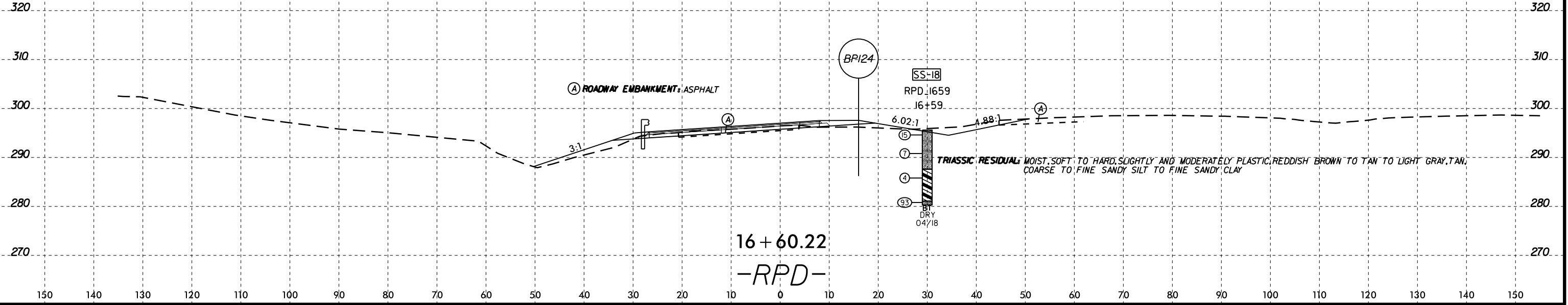
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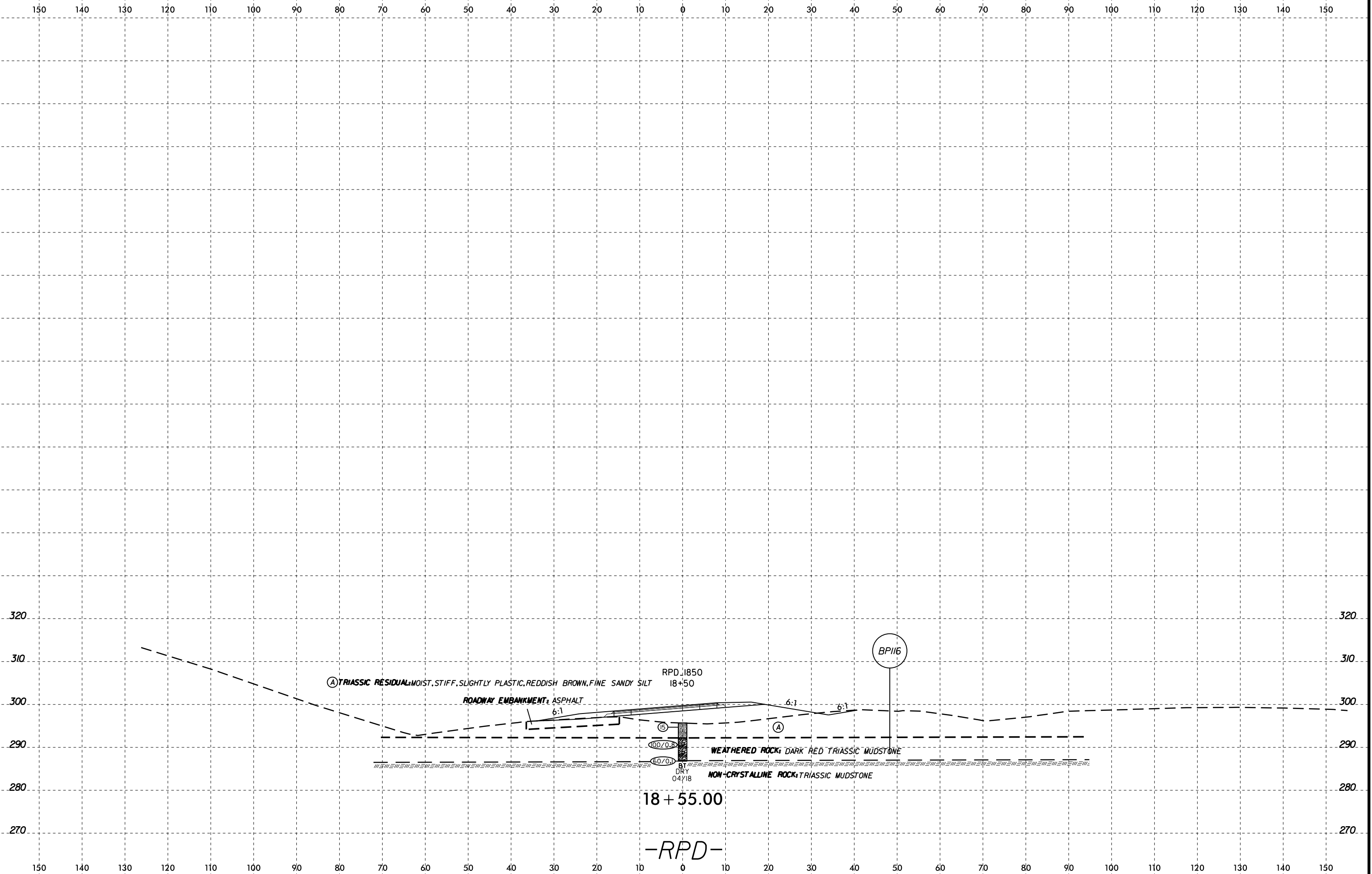


PROJ. REFERENCE NO.	SHEET NO.
I-5700	60

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I:\4-AUG-2018 15:02  
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 Wells





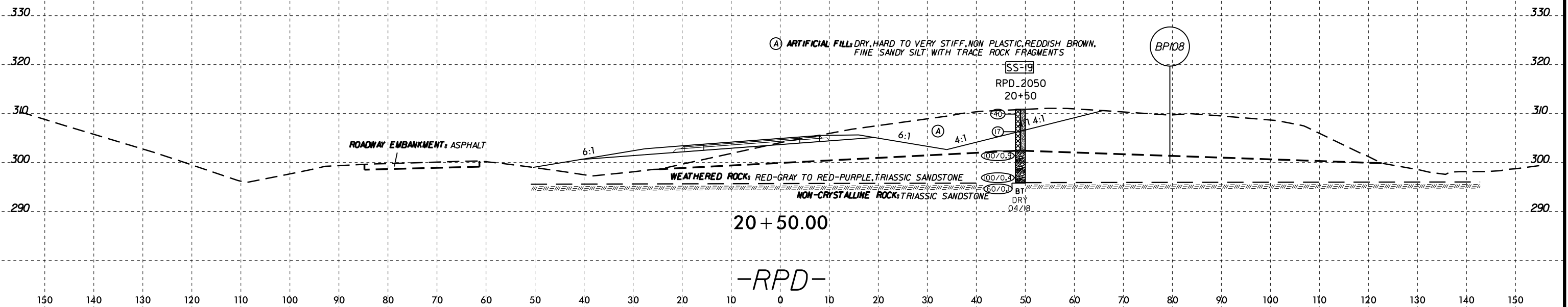
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18 + 55.00  
-RPD-

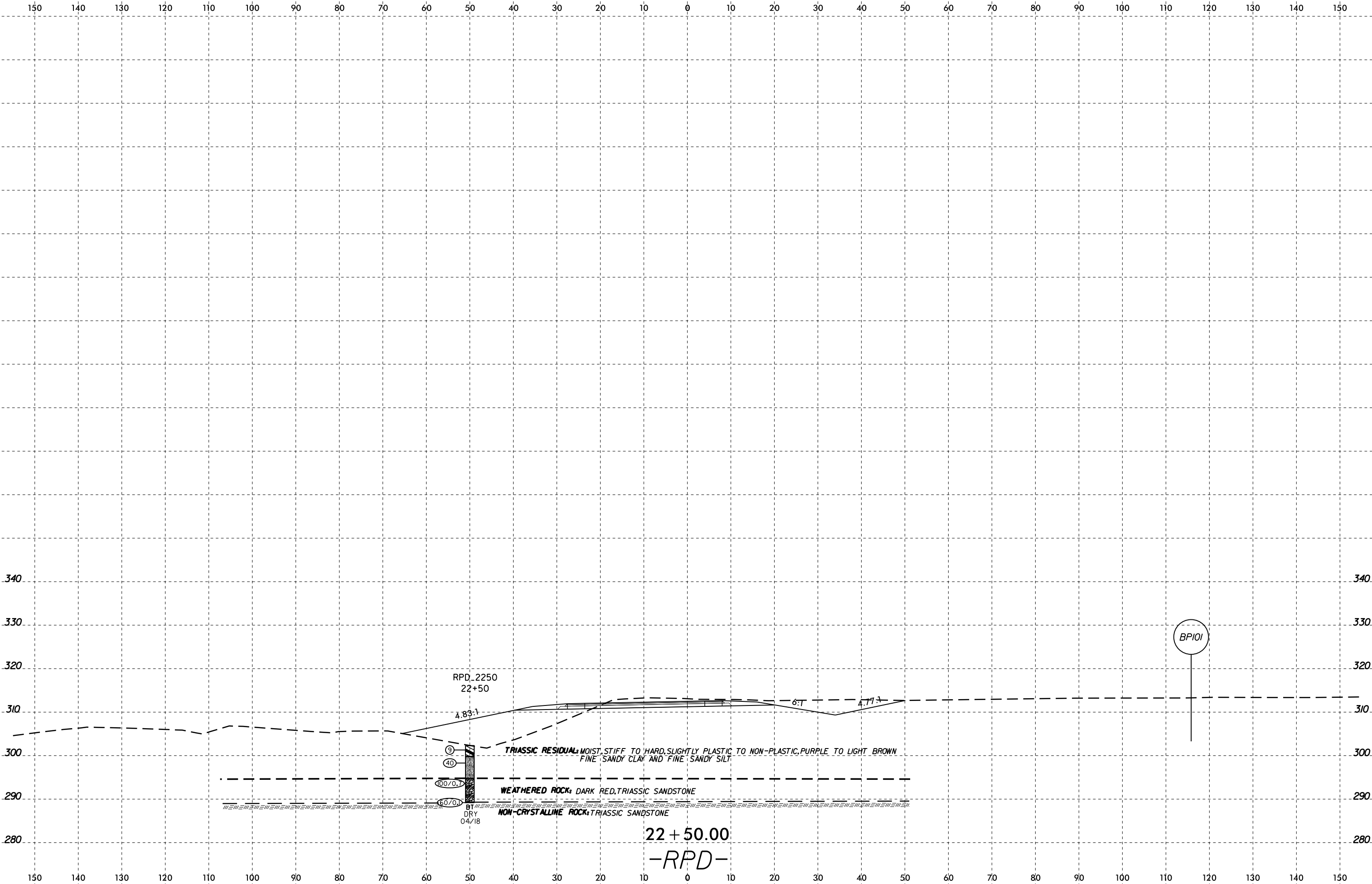


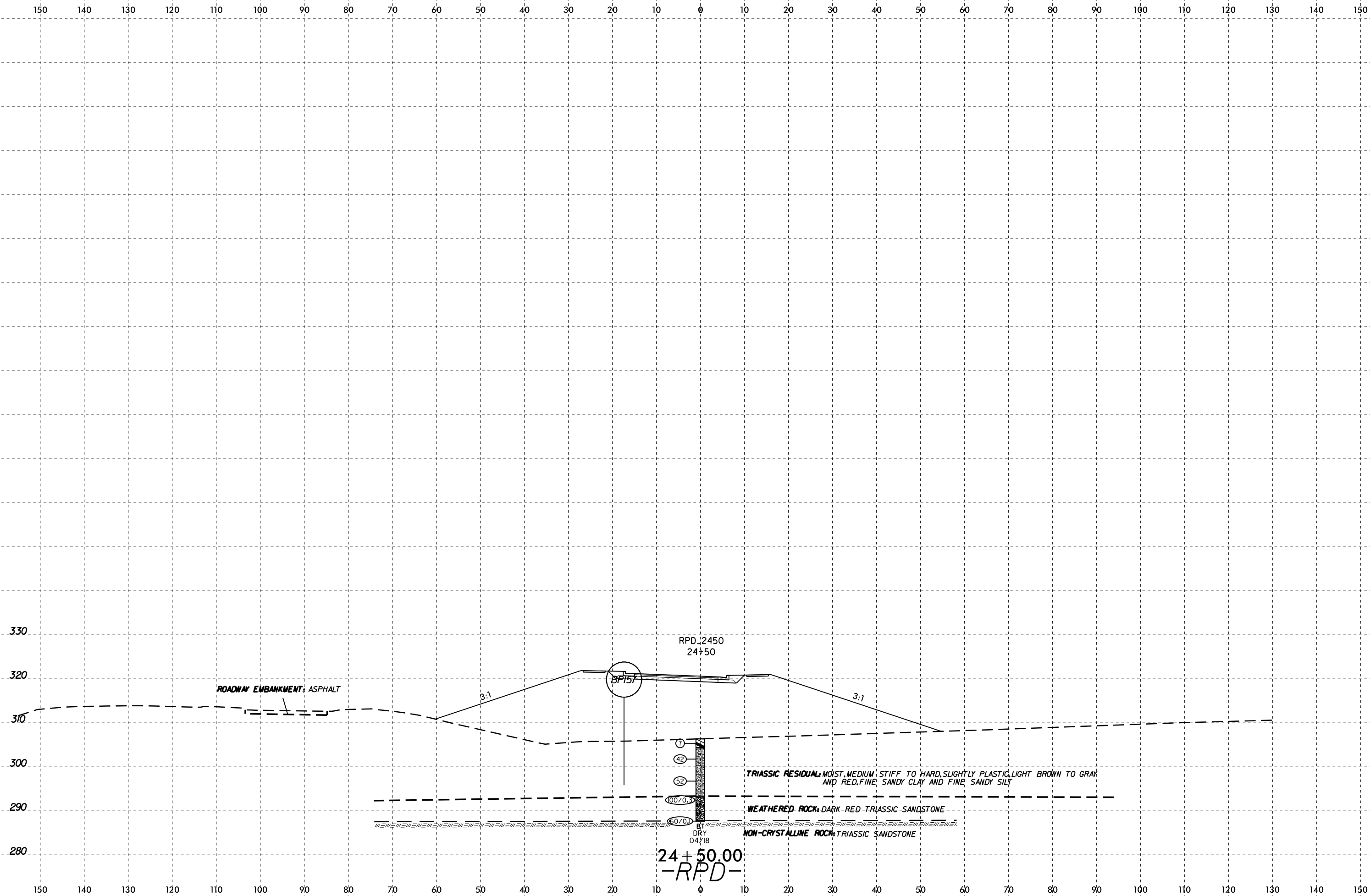
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 W:\shore\GEO\TECH\15700\_GEO\RDW\CADD\GEO\TECH\15700\_Geo\_xst\_RPD.dgn



150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



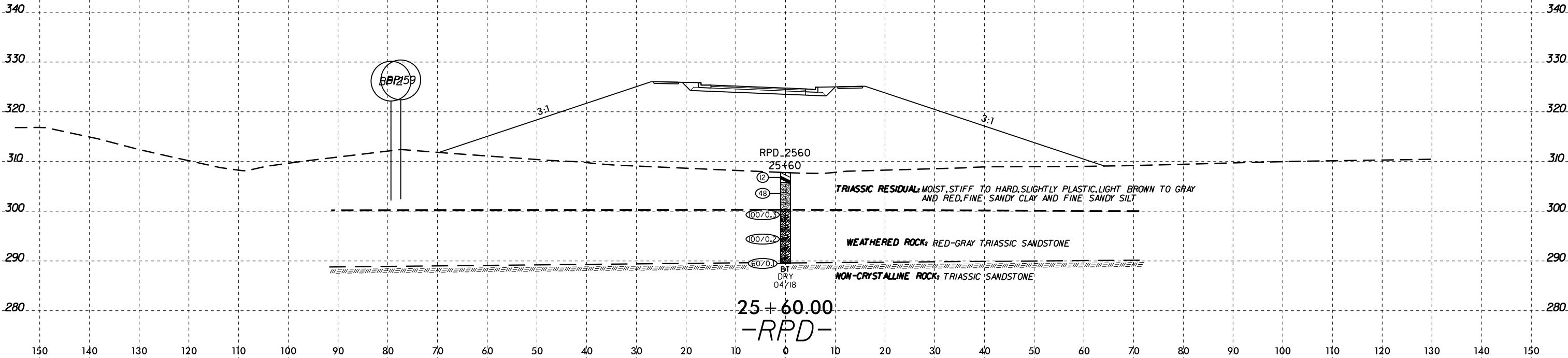




6/23/16

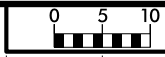
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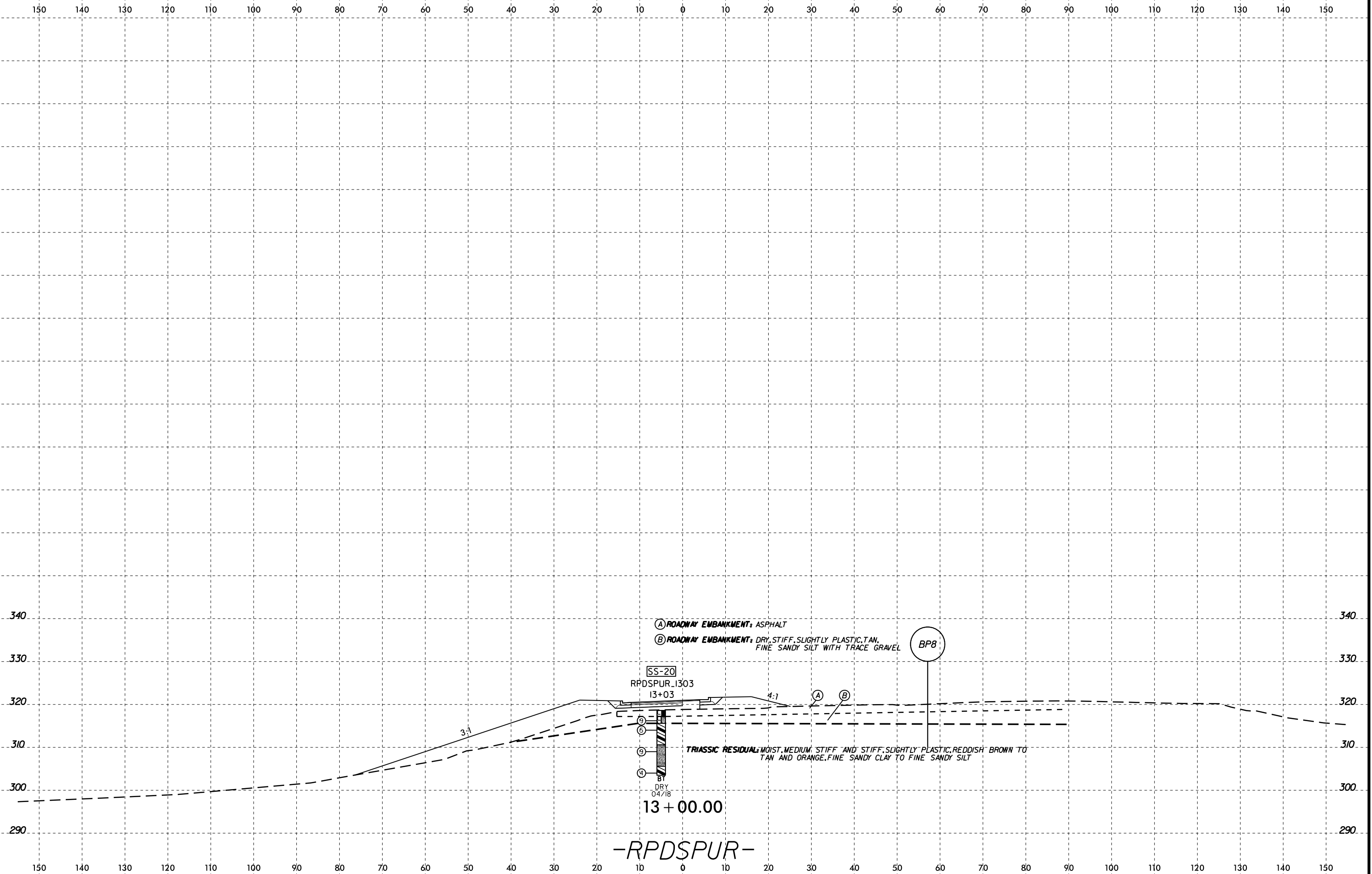


25+60.00  
 -RPD-

14-AUG-2018 15:02 W:\shore\GEO\TECHNICAL\Projects\Active Projects\151548\053A 1-5700 Roadway\15700\_GEO\RDW\CADD\_GEO\RDW\15700\_Geo\_xst\RPDSPUR.dgn 6/23/16

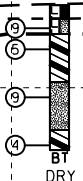


PROJ. REFERENCE NO.	SHEET NO.
1-5700	66



- (A) ROADWAY EMBANKMENT: ASPHALT
- (B) ROADWAY EMBANKMENT: DRY, STIFF, SLIGHTLY PLASTIC, TAN, FINE SANDY SILT WITH TRACE GRAVEL

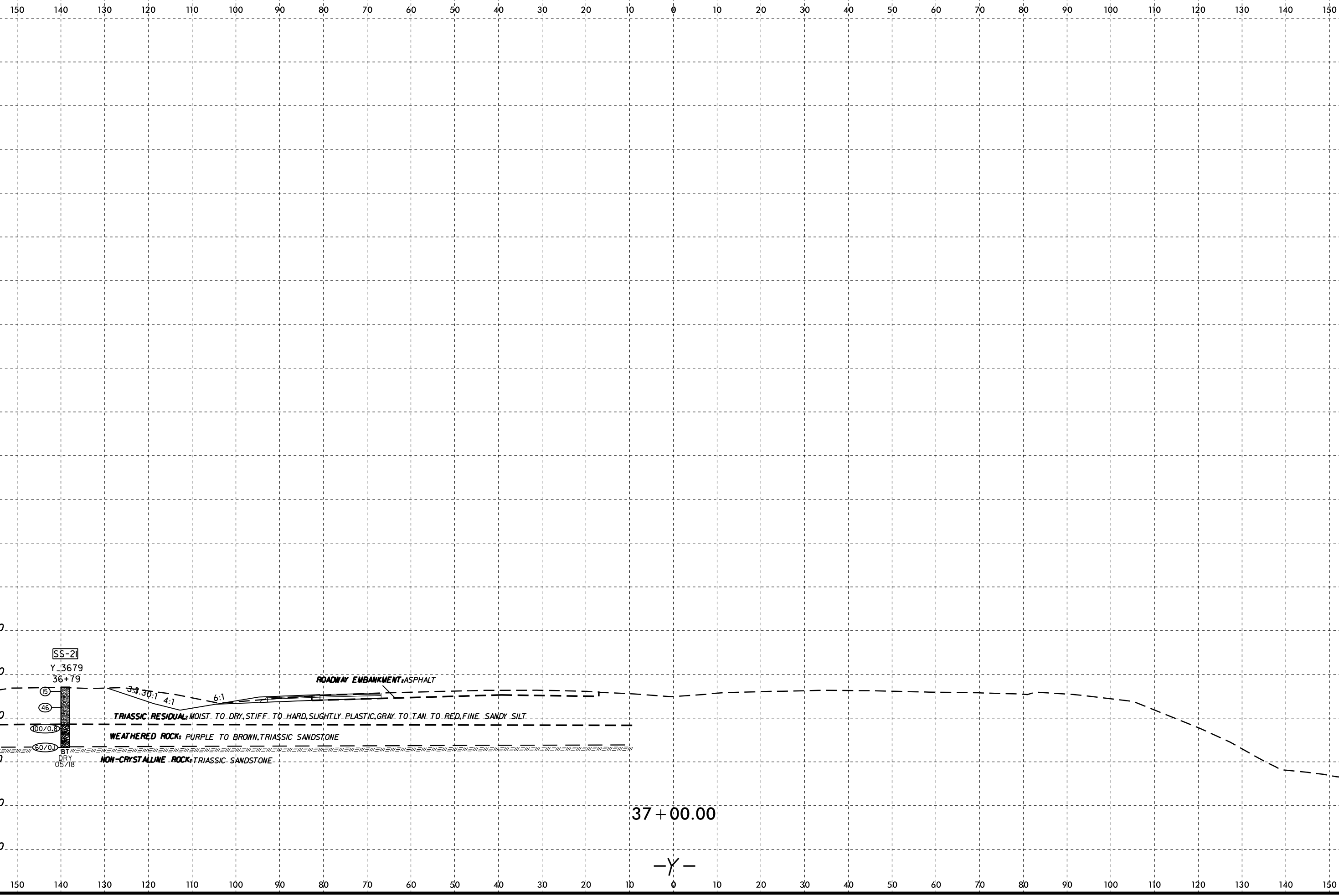
SS-20  
RPDSPUR\_1303  
13+03



TRIASSIC RESIDUAL: MOIST, MEDIUM STIFF AND STIFF, SLIGHTLY PLASTIC, REDDISH BROWN TO TAN AND ORANGE, FINE SANDY CLAY TO FINE SANDY SILT

13 + 00.00

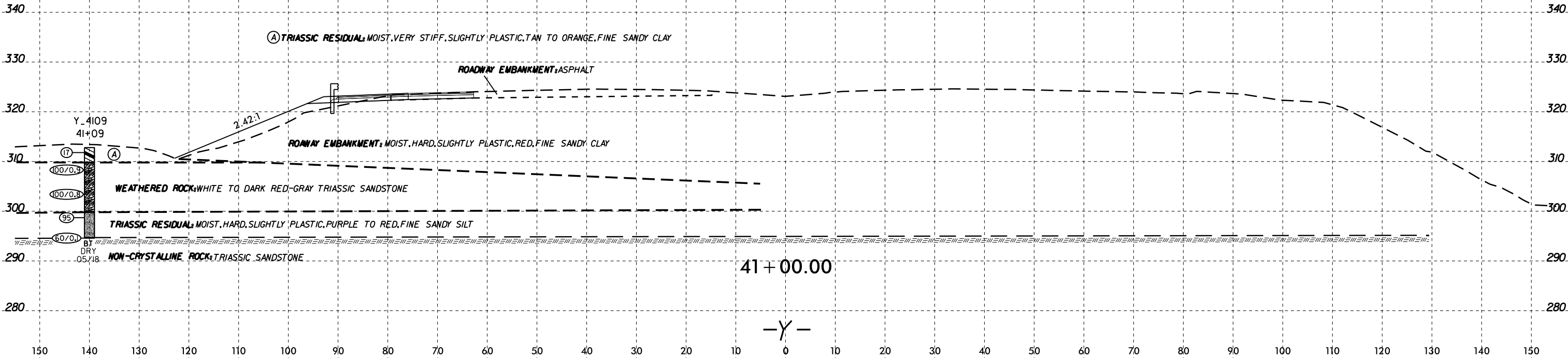
-RPDSPUR-

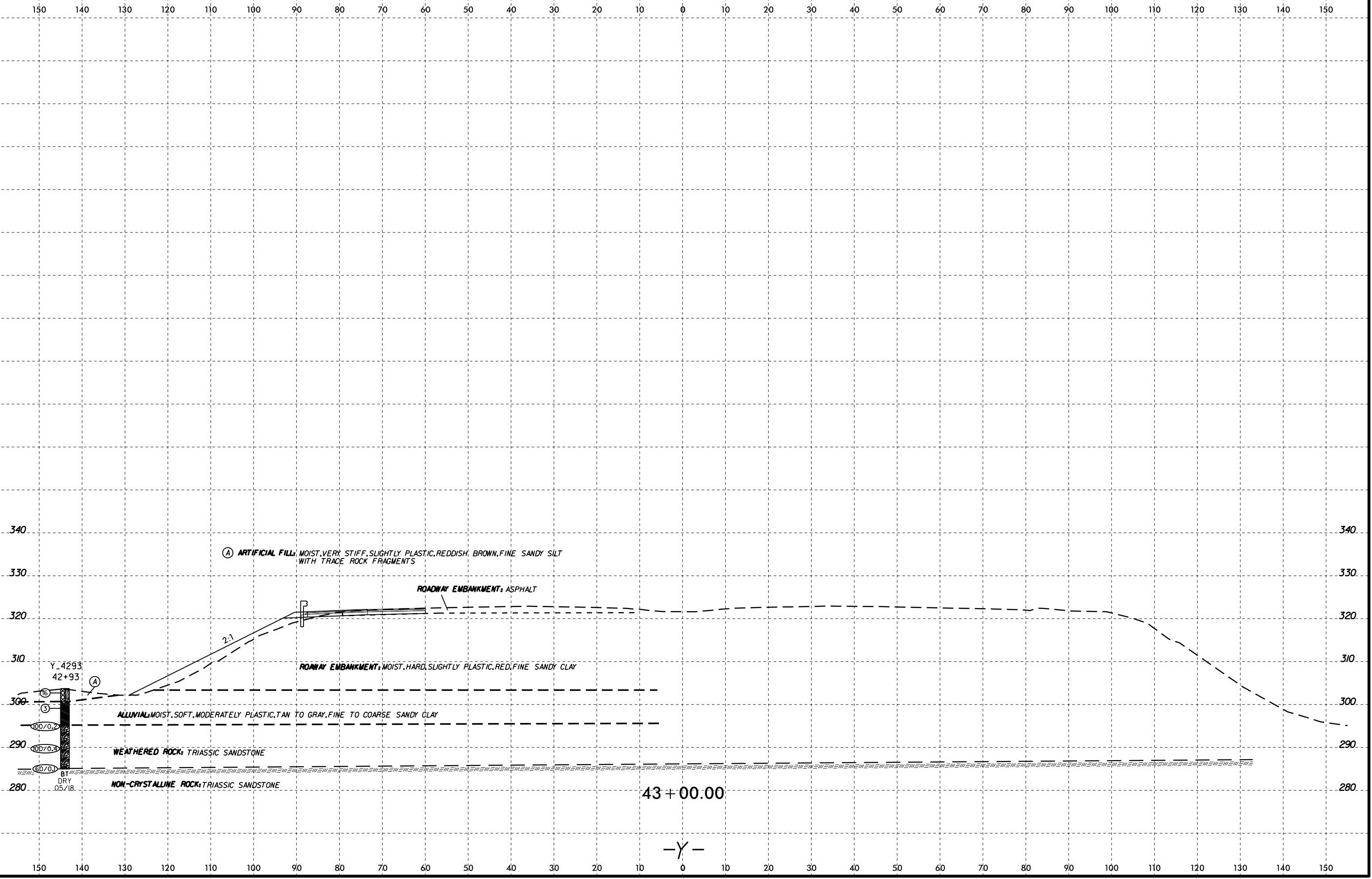


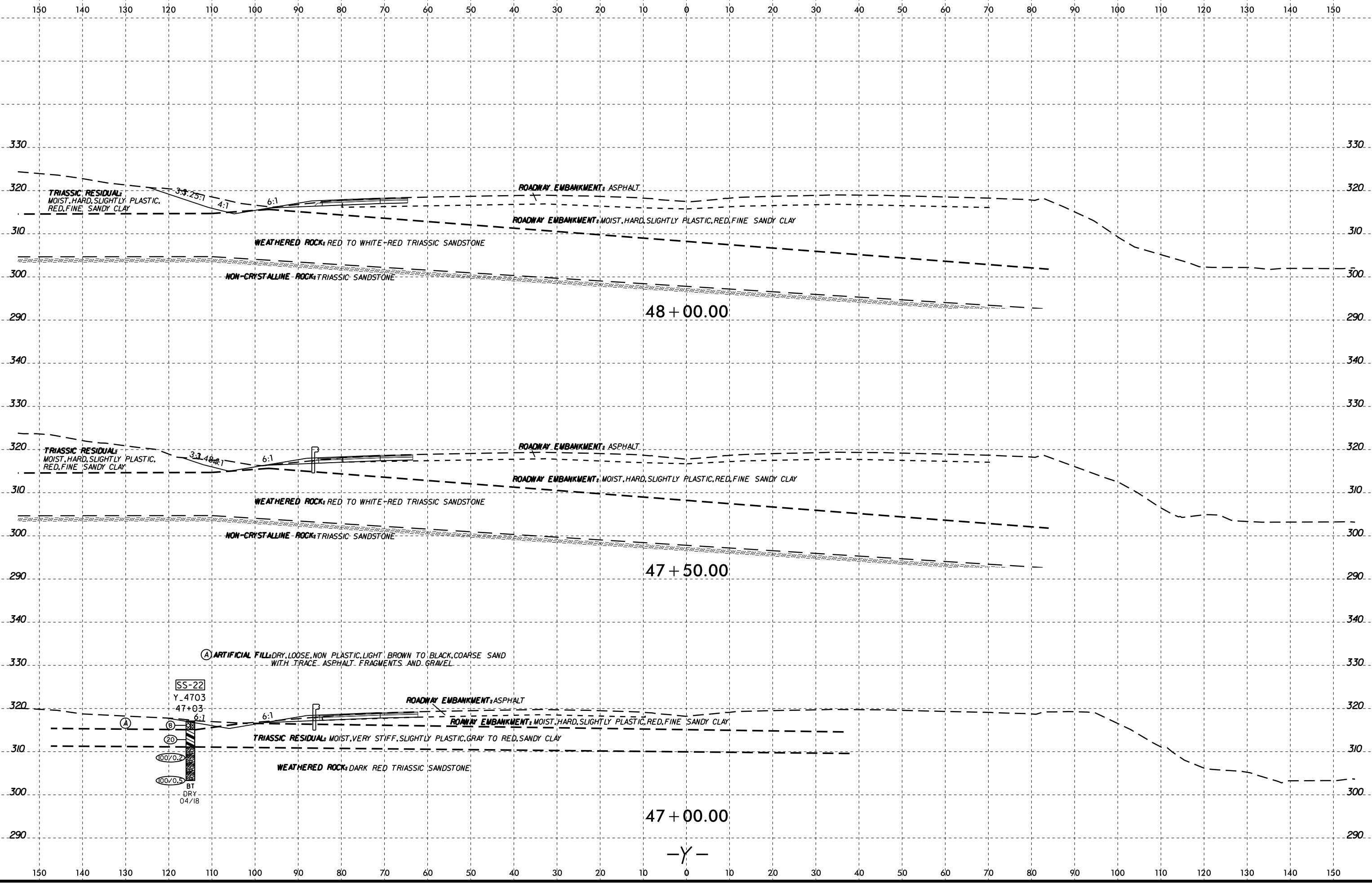
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 14-AUG-2018 15:02  
 W:\shore\GEO\TECHNICAL\Projects\Active\Projects\20151548.053A I-5700 Roadway\GEO\ROADWAY\CADD\GEO\TECH\15700\_Geo\_xst\_Y.dgn  
 14-AUG-2018 15:02  
 W:\shore\GEO\TECHNICAL\Projects\Active\Projects\20151548.053A I-5700 Roadway\GEO\ROADWAY\CADD\GEO\TECH\15700\_Geo\_xst\_Y.dgn  
 14-AUG-2018 15:02

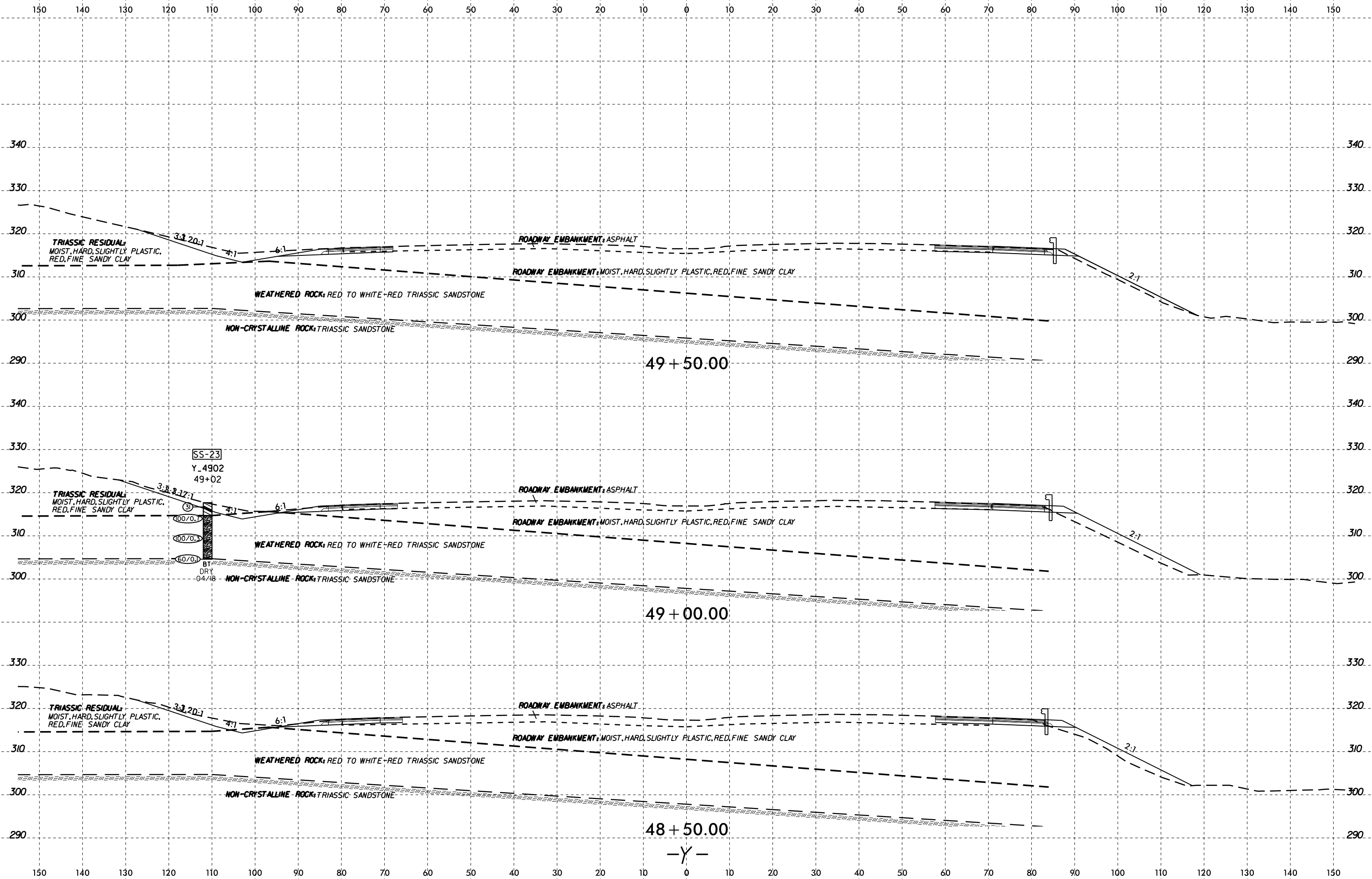


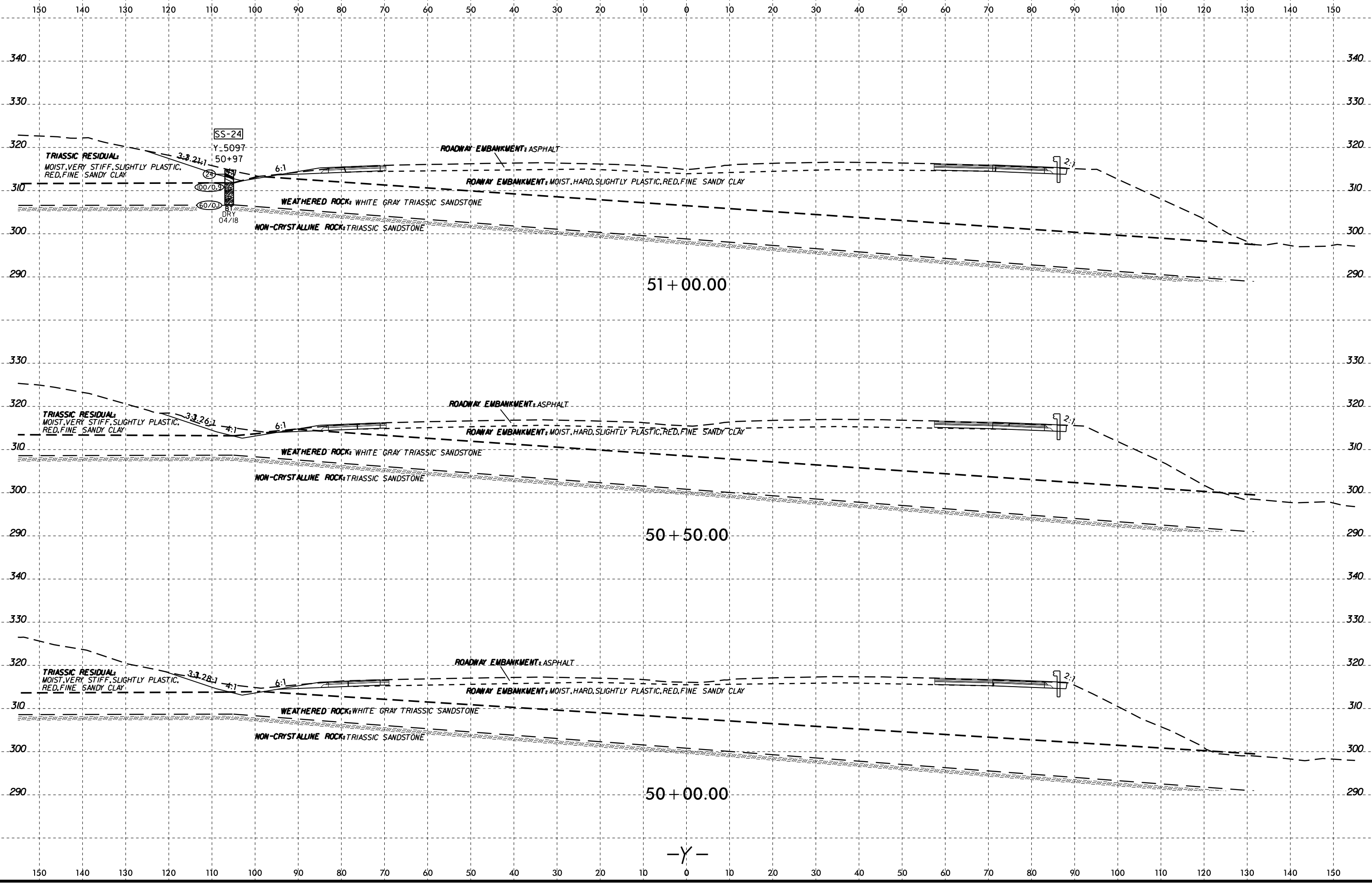
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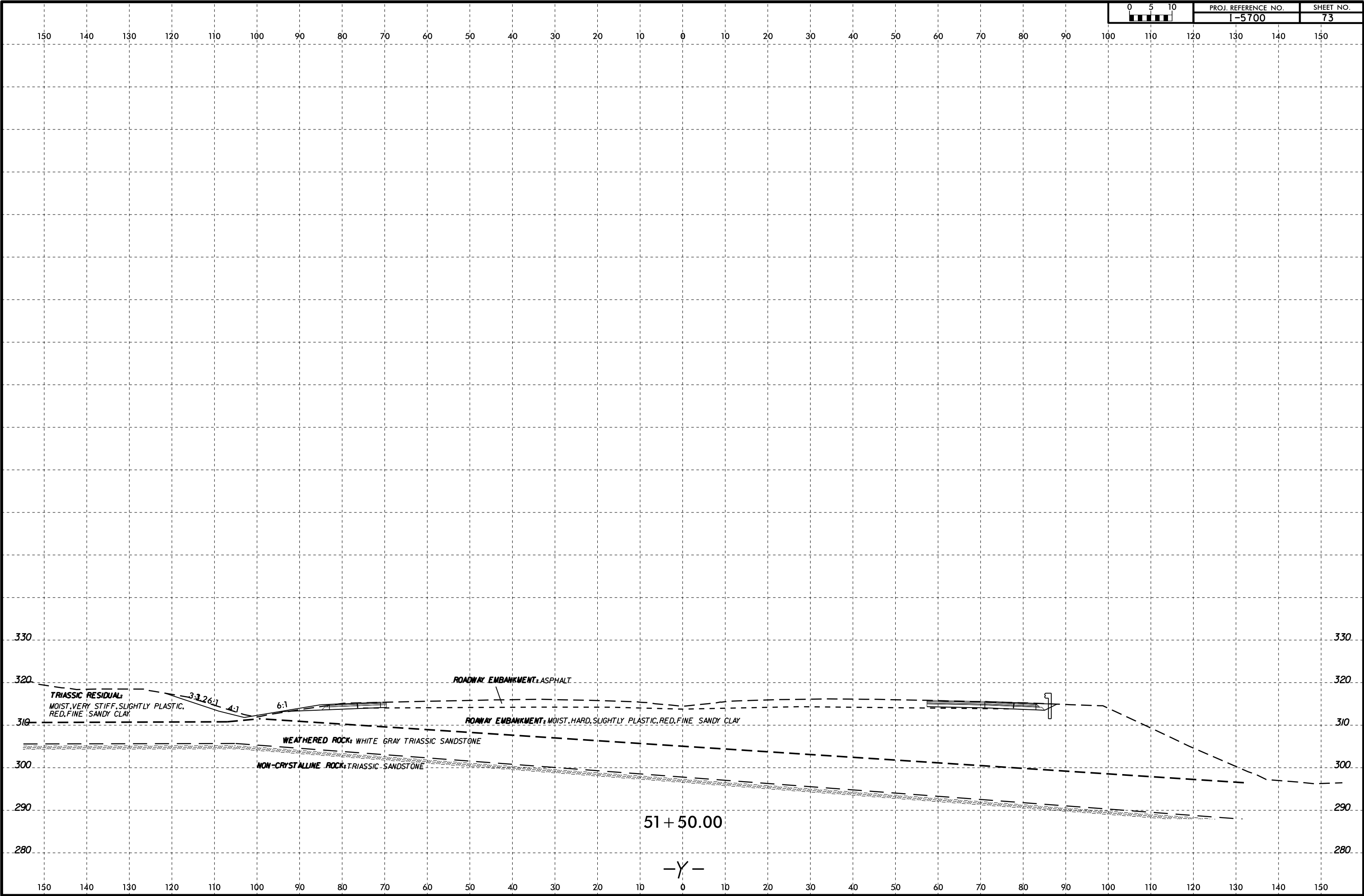






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 At 15:03 6/23/16



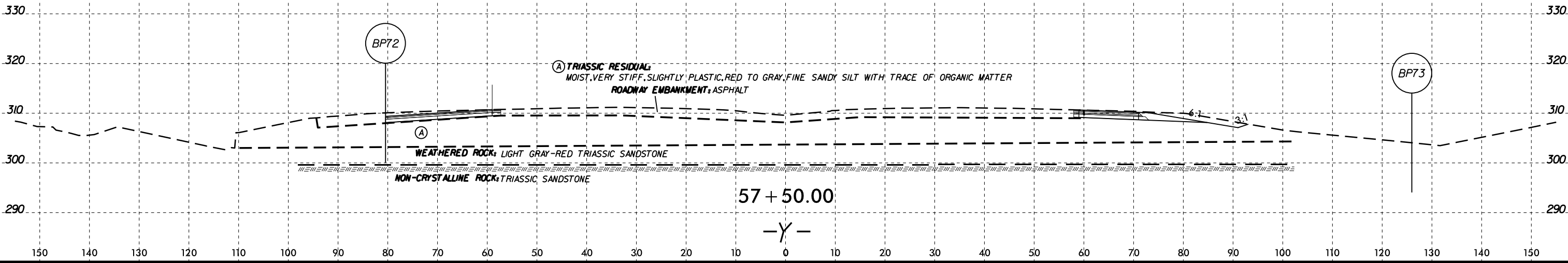


51 + 50.00

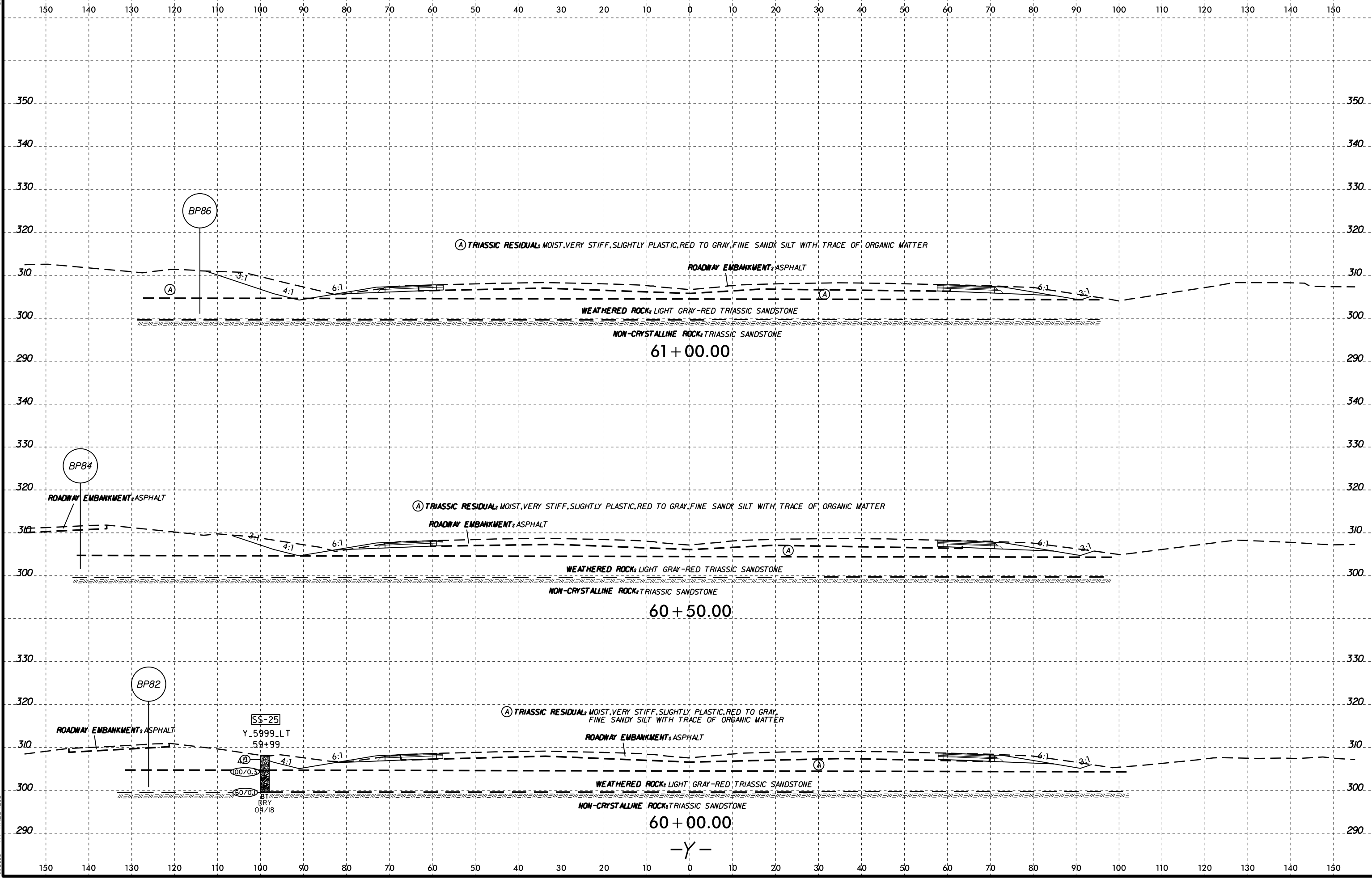
-Y-



150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



14-AUG-2018 15:03 W:\shore\GEO\TECHNICAL\Projects\Active Projects\20151548.053A 1-5700 Roadway\15700\_GEO\RDWY\CADD\_GEO\TECH\15700\_Geo\_xst\_1.dgn



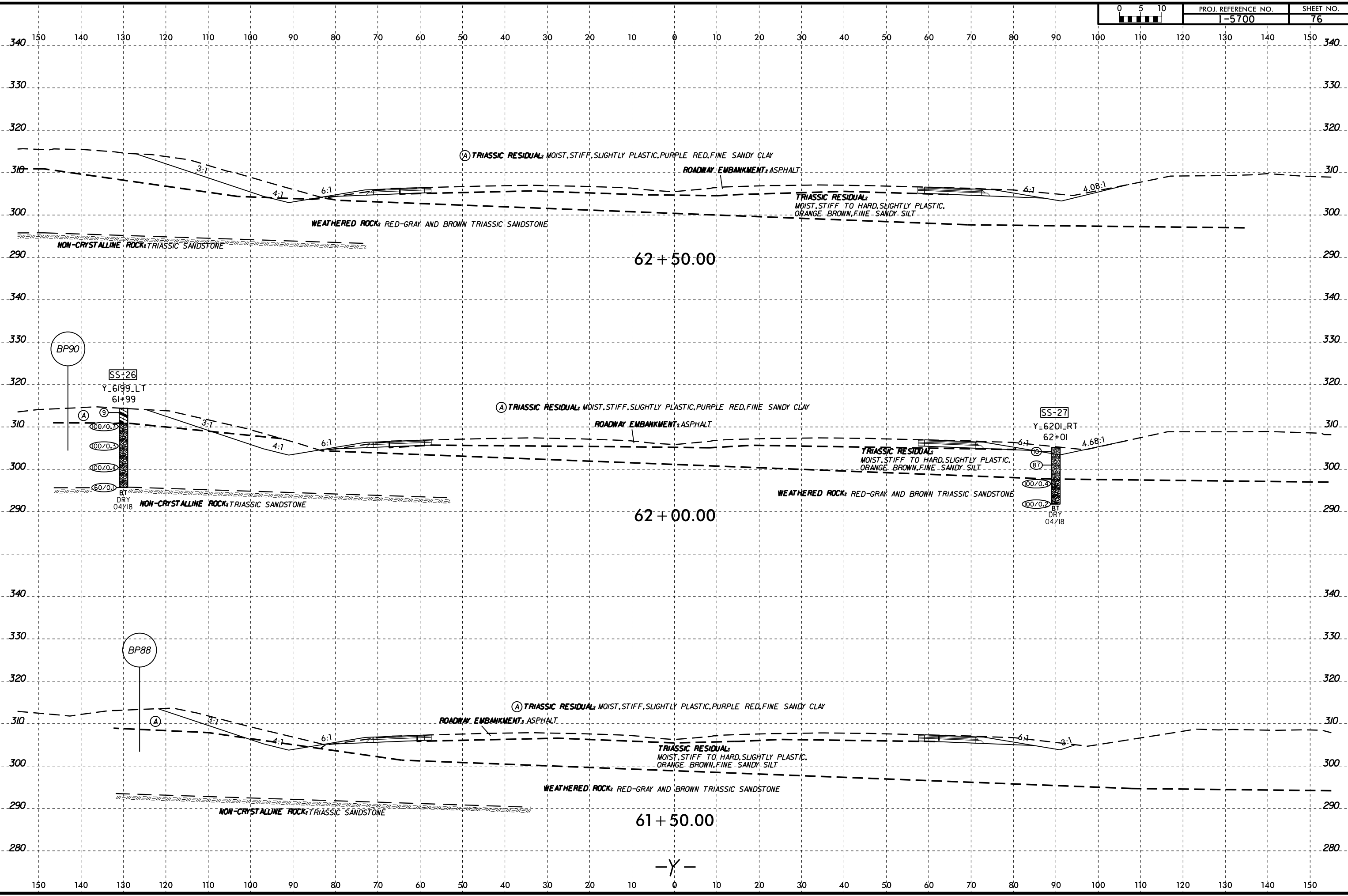
61 + 00.00

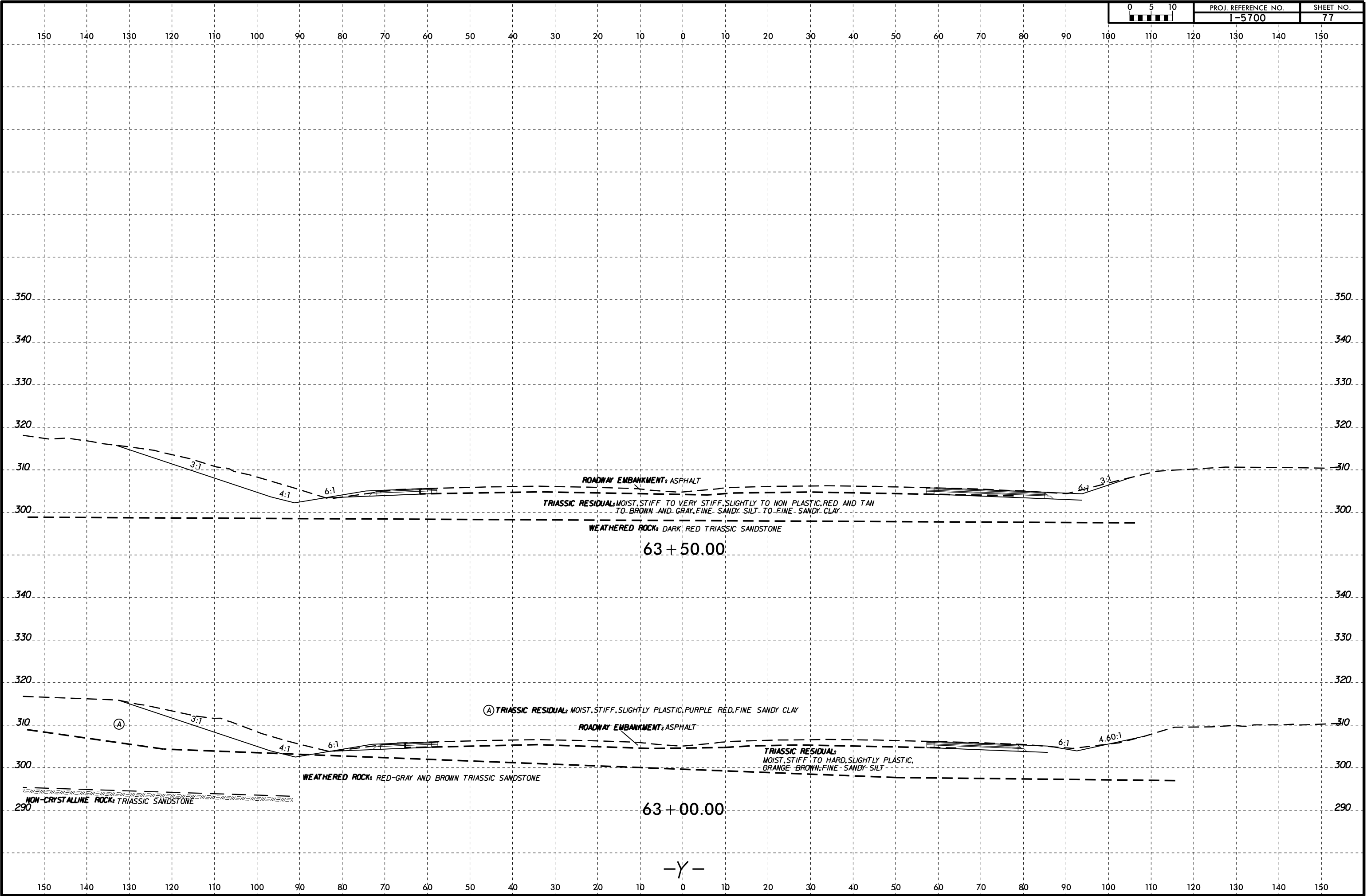
60 + 50.00

60 + 00.00

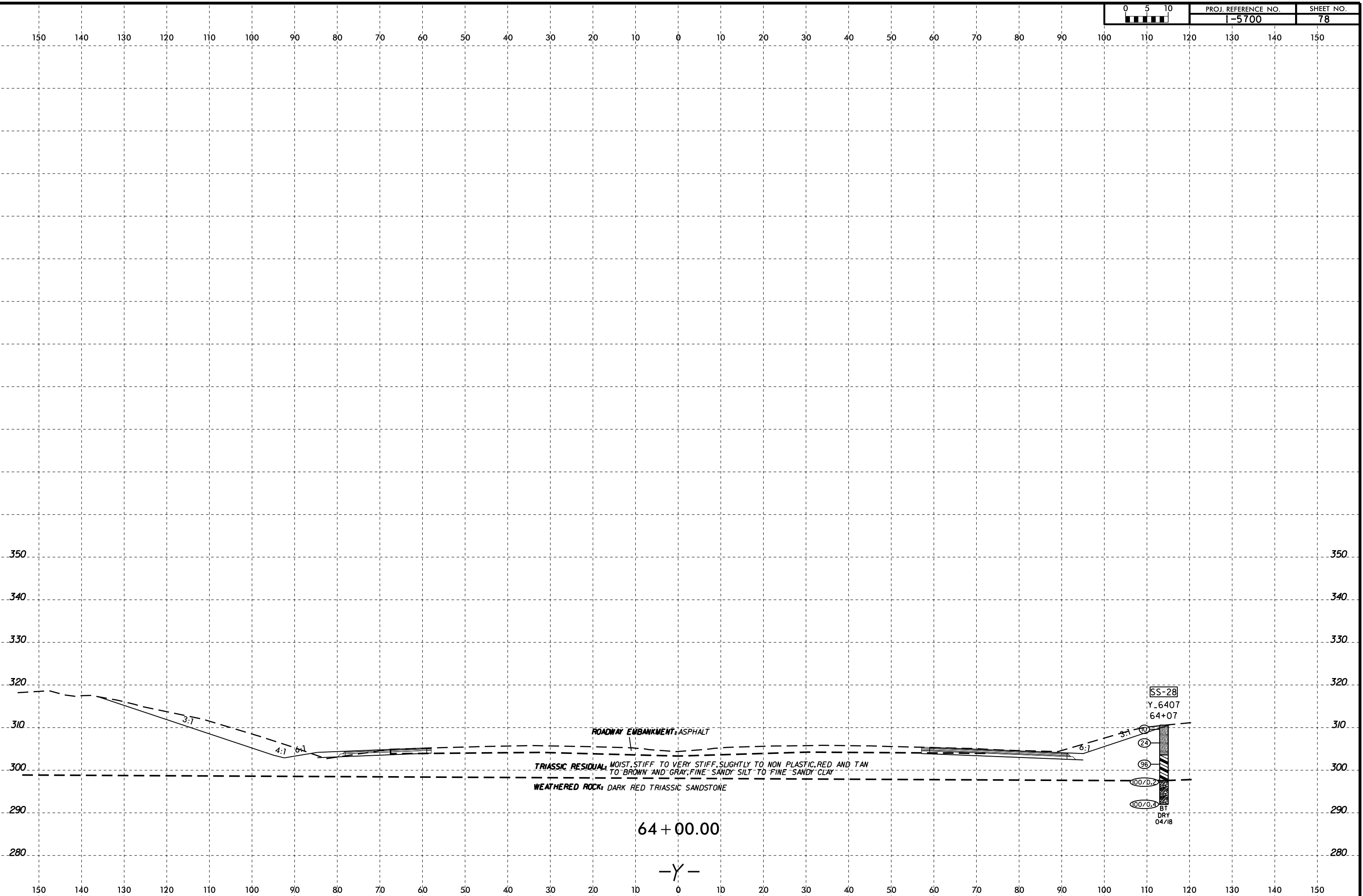
-Y-

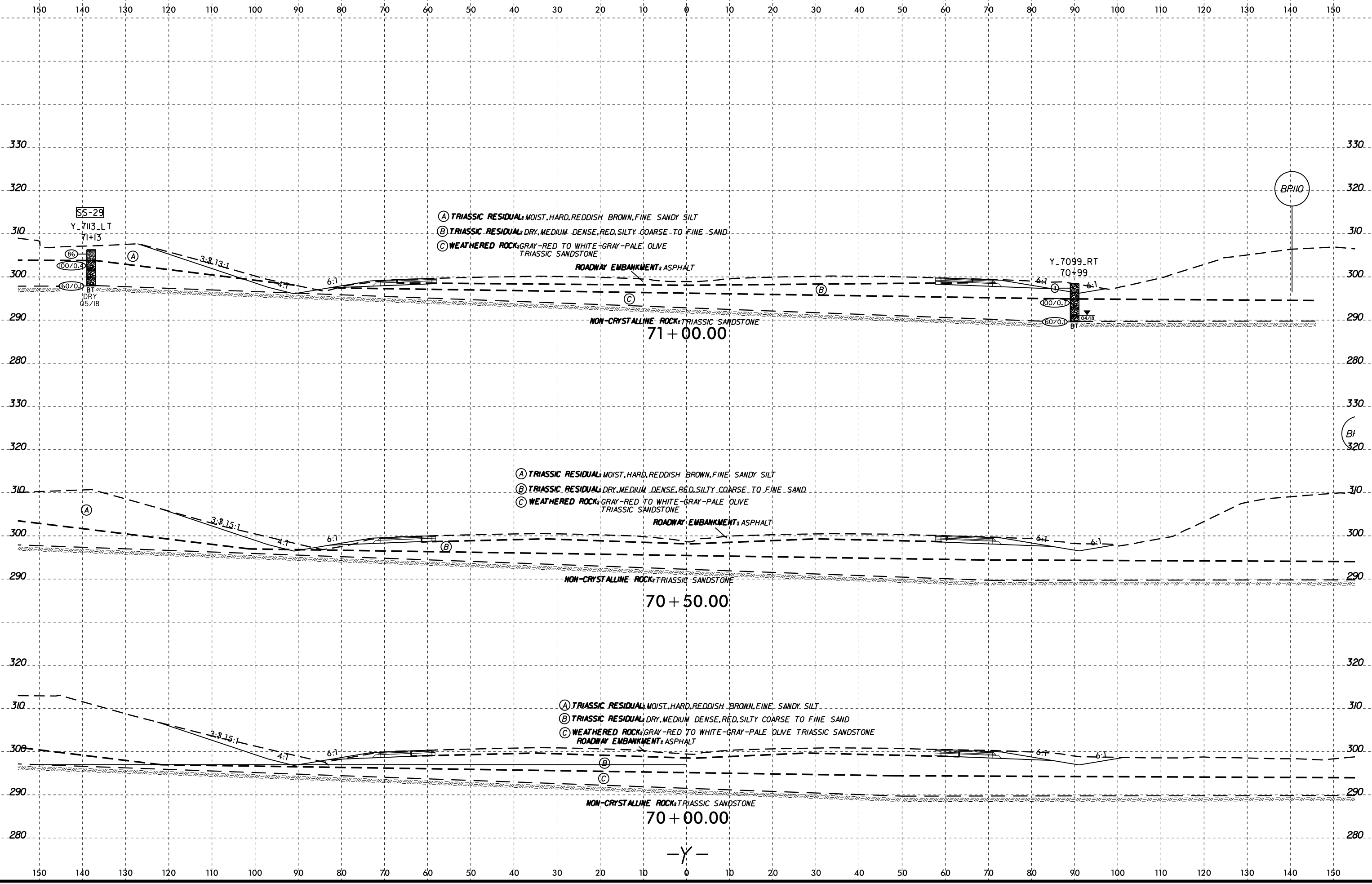
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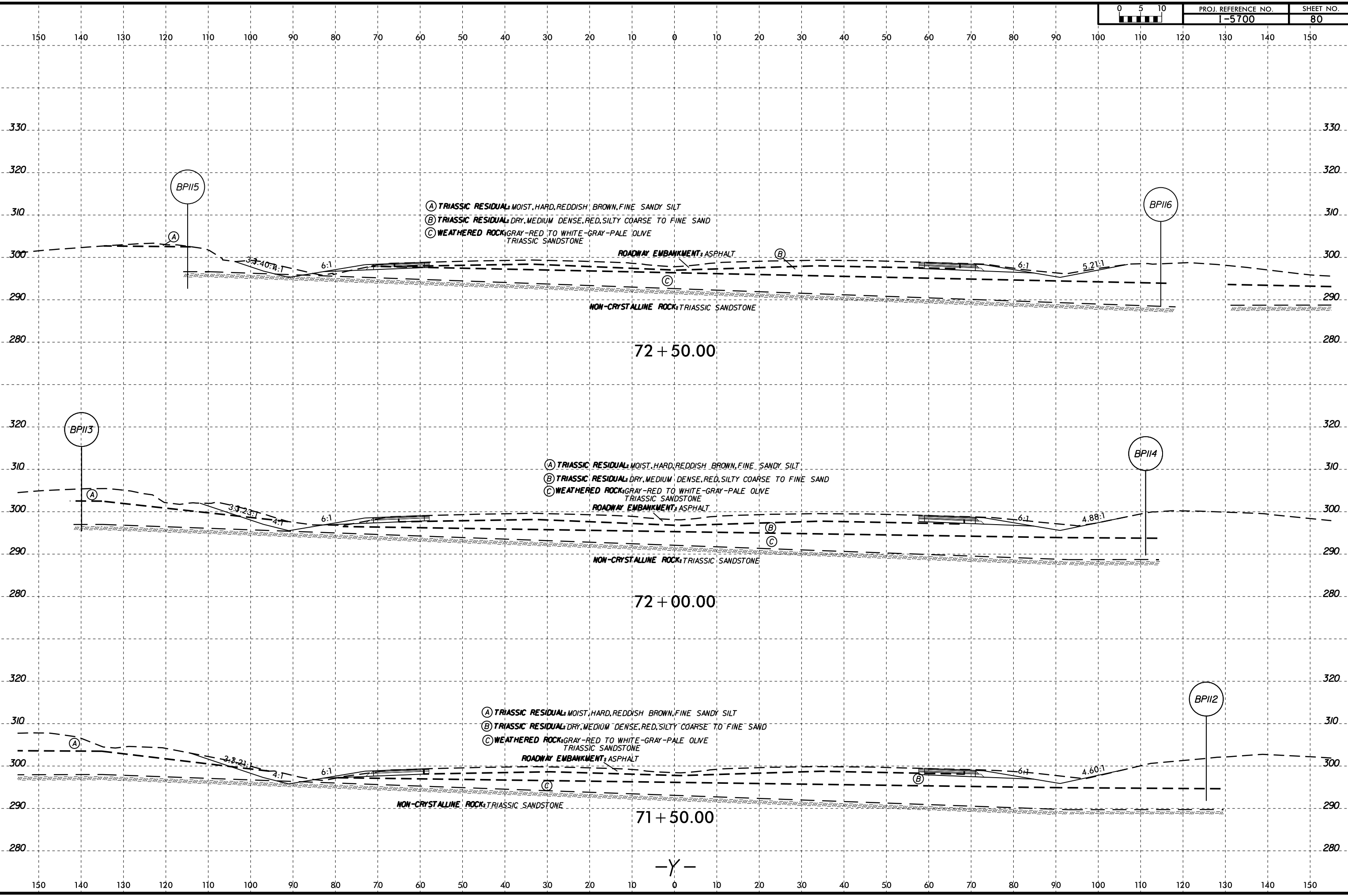
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6/23/16





I:\4-AUG-2018 15:03 W:\shore\GEO\TECHNICAL\Projects\Active Projects\20151548.053A I-5700 Roadway\15700\_GEO\ROADWAY\CADD\_GEO\TECH\15700\_Geo\_xst\_1.dgn  
 At: KA20215

6/23/16  
14-AUG-2018 15:03  
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twells



- (A) TRIASSIC RESIDUAL, MOIST, HARD, REDDISH BROWN, FINE SANDY SILT
- (B) TRIASSIC RESIDUAL, DRY, MEDIUM DENSE, RED, SILTY COARSE TO FINE SAND
- (C) WEATHERED ROCK, GRAY-RED TO WHITE-GRAY-PALE OLIVE TRIASSIC SANDSTONE

ROADWAY EMBANKMENT, ASPHALT

NON-CRYSTALLINE ROCK, TRIASSIC SANDSTONE

72 + 50.00

- (A) TRIASSIC RESIDUAL, MOIST, HARD, REDDISH BROWN, FINE SANDY SILT
- (B) TRIASSIC RESIDUAL, DRY, MEDIUM DENSE, RED, SILTY COARSE TO FINE SAND
- (C) WEATHERED ROCK, GRAY-RED TO WHITE-GRAY-PALE OLIVE TRIASSIC SANDSTONE

ROADWAY EMBANKMENT, ASPHALT

NON-CRYSTALLINE ROCK, TRIASSIC SANDSTONE

72 + 00.00

- (A) TRIASSIC RESIDUAL, MOIST, HARD, REDDISH BROWN, FINE SANDY SILT
- (B) TRIASSIC RESIDUAL, DRY, MEDIUM DENSE, RED, SILTY COARSE TO FINE SAND
- (C) WEATHERED ROCK, GRAY-RED TO WHITE-GRAY-PALE OLIVE TRIASSIC SANDSTONE

ROADWAY EMBANKMENT, ASPHALT

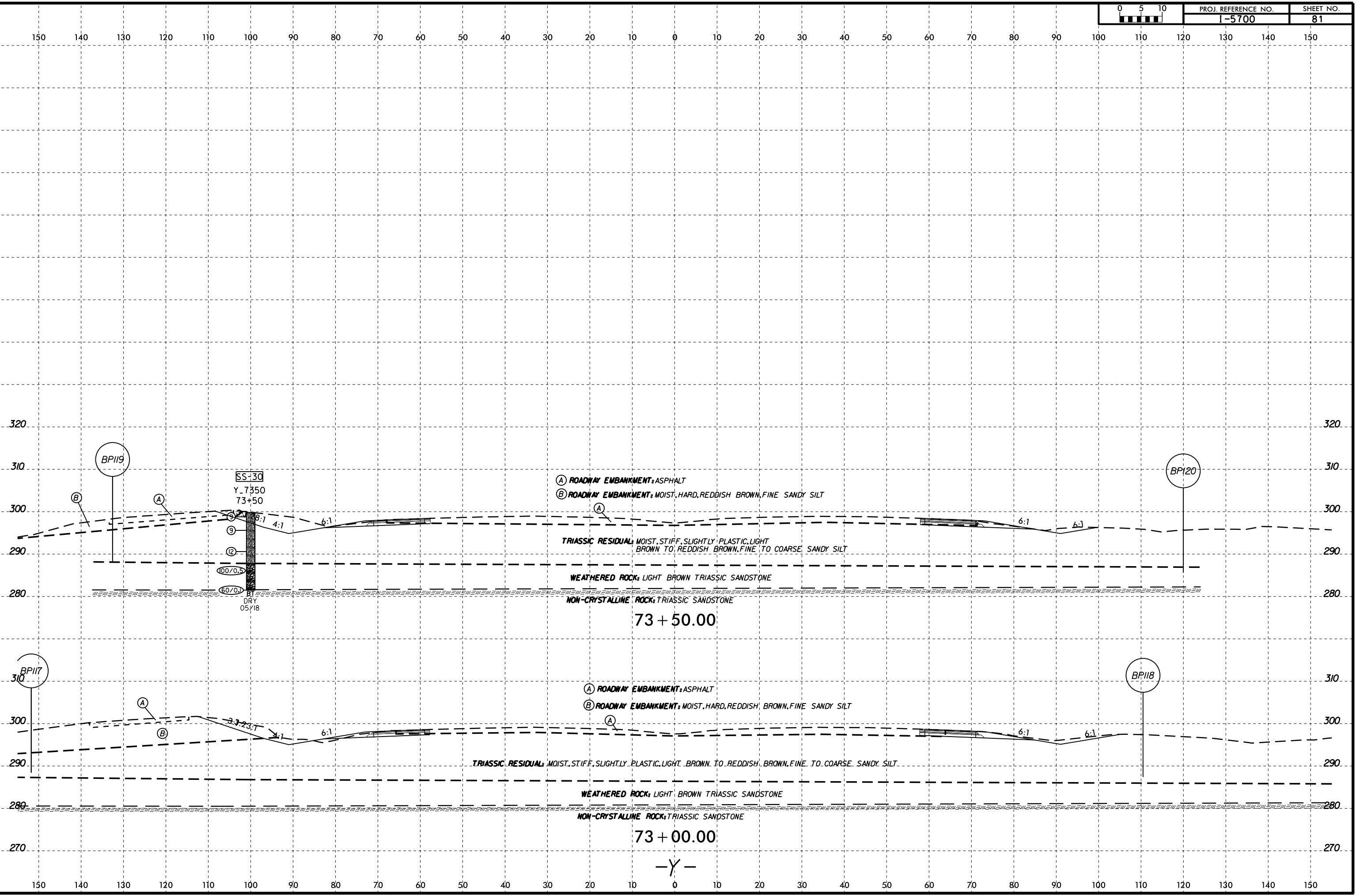
NON-CRYSTALLINE ROCK, TRIASSIC SANDSTONE

71 + 50.00

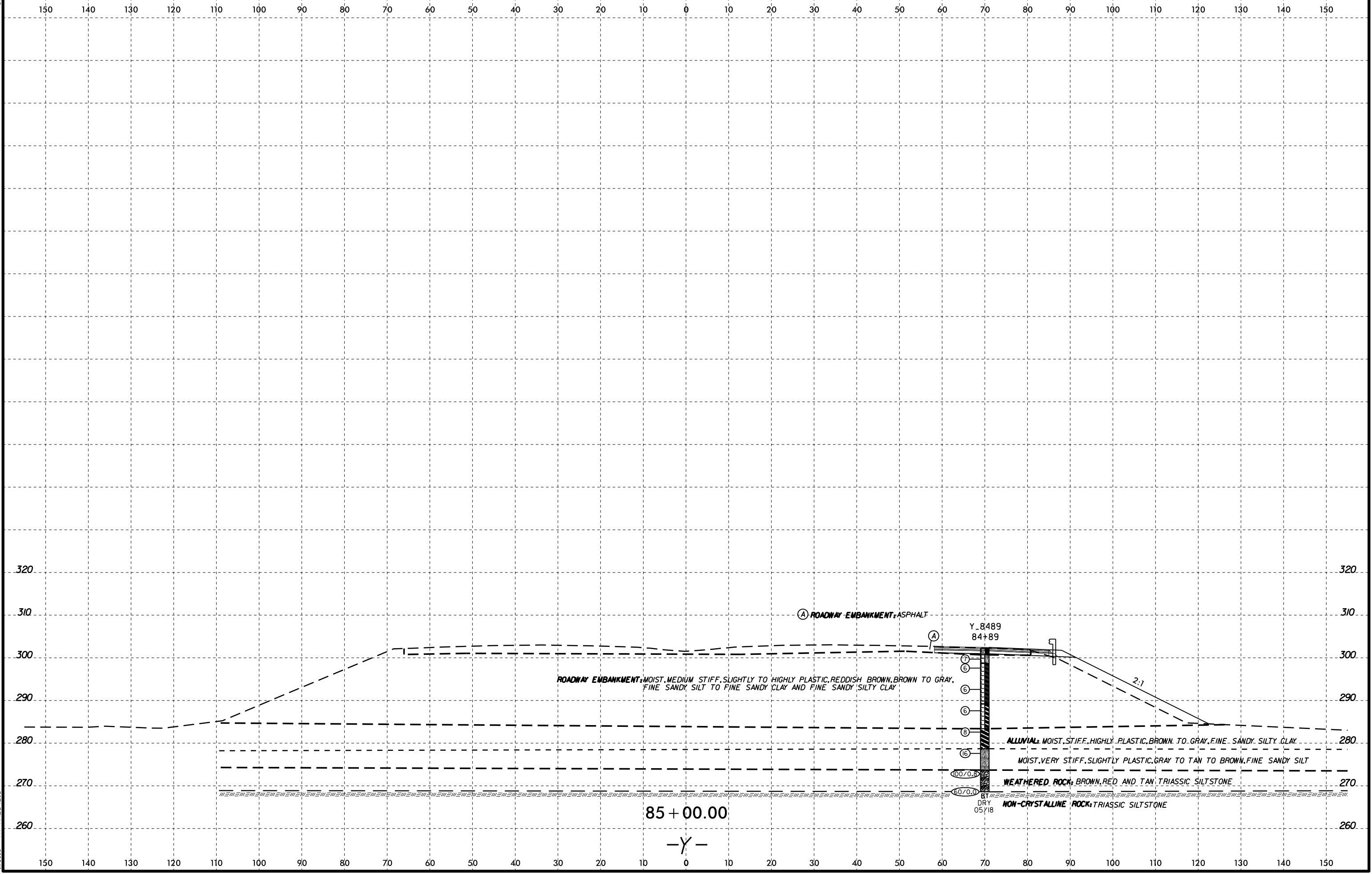
-Y-

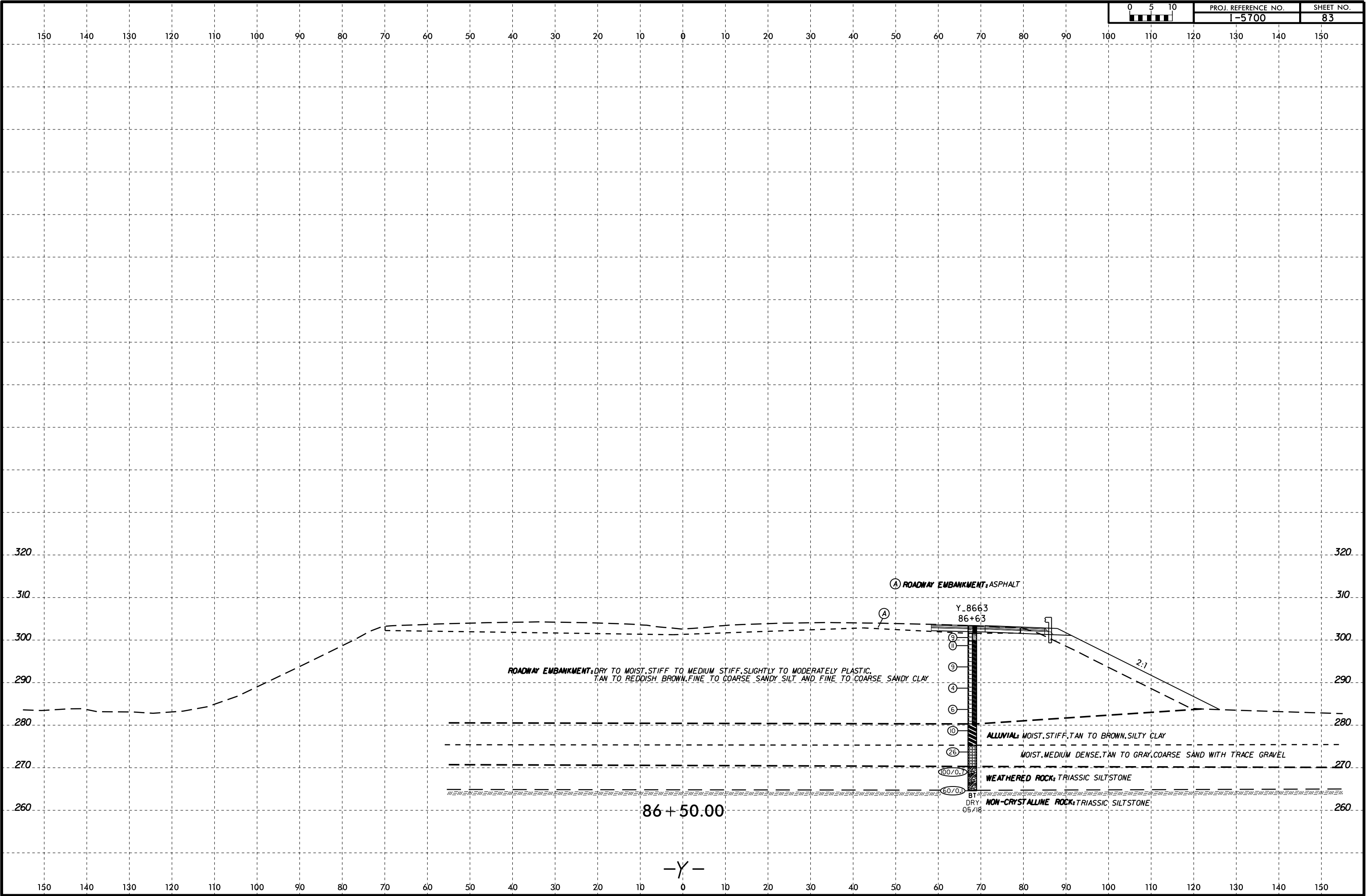


14-AUG-2018 15:03  
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6/23/16

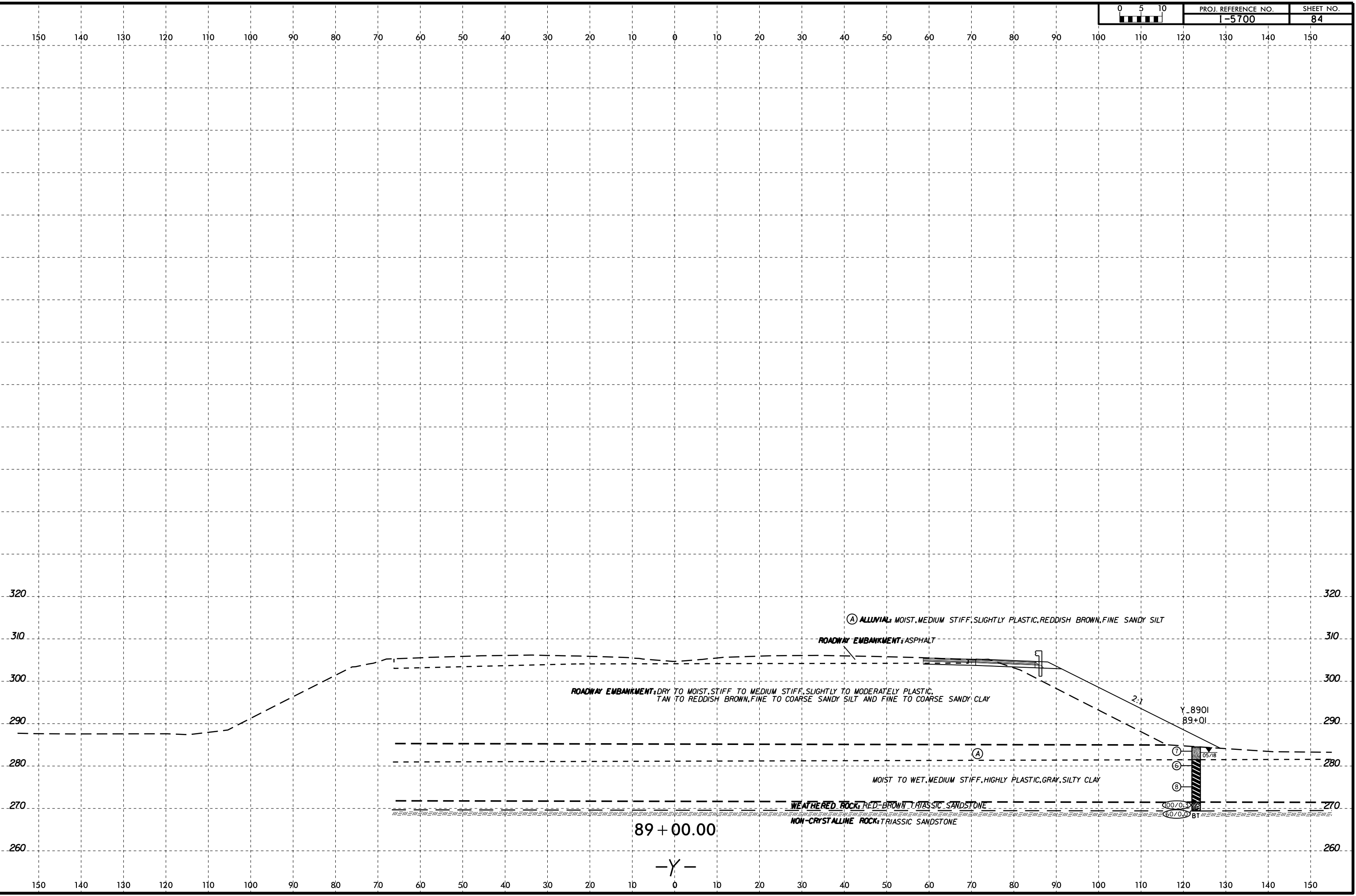


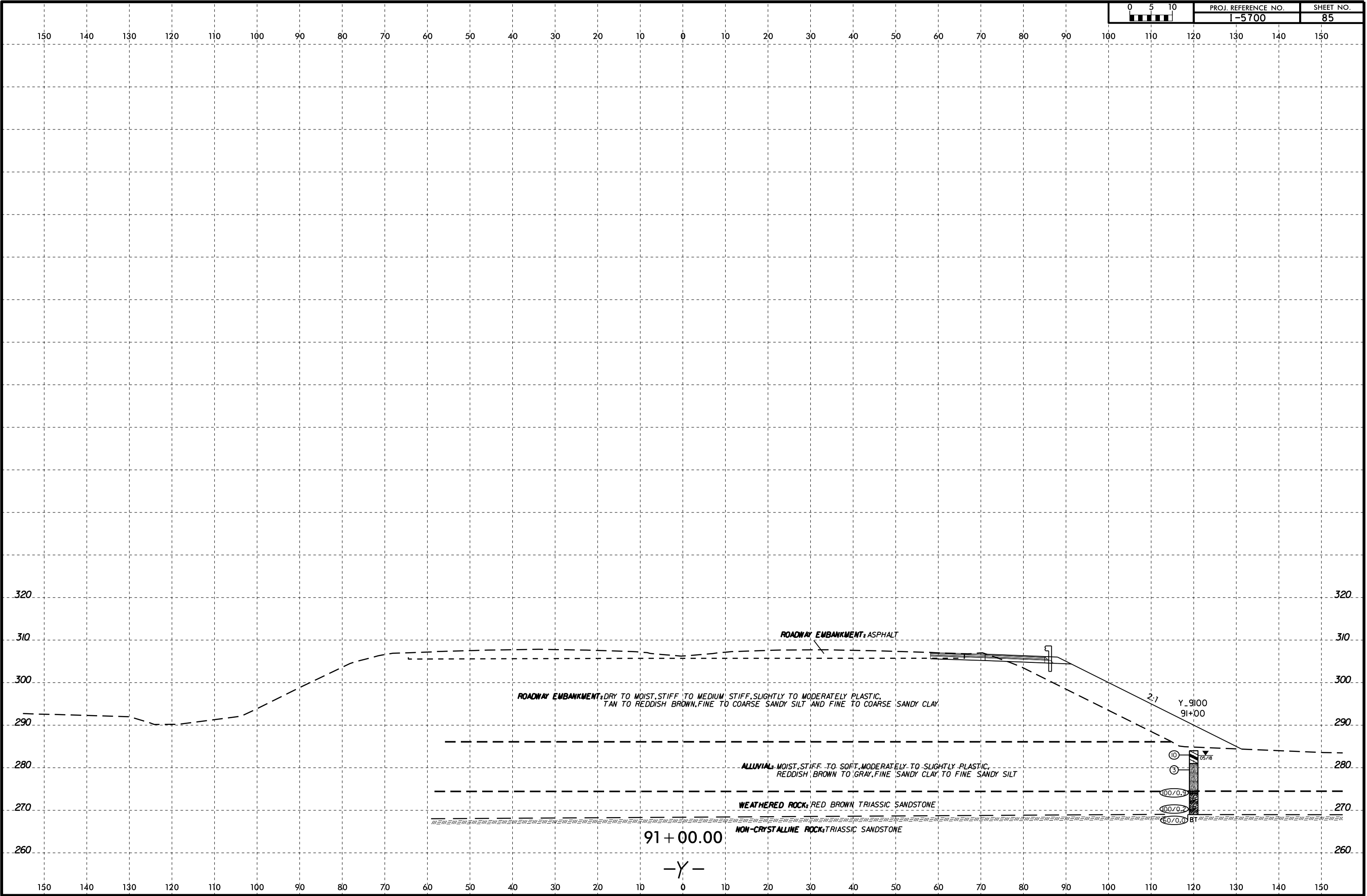
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 6/23/16

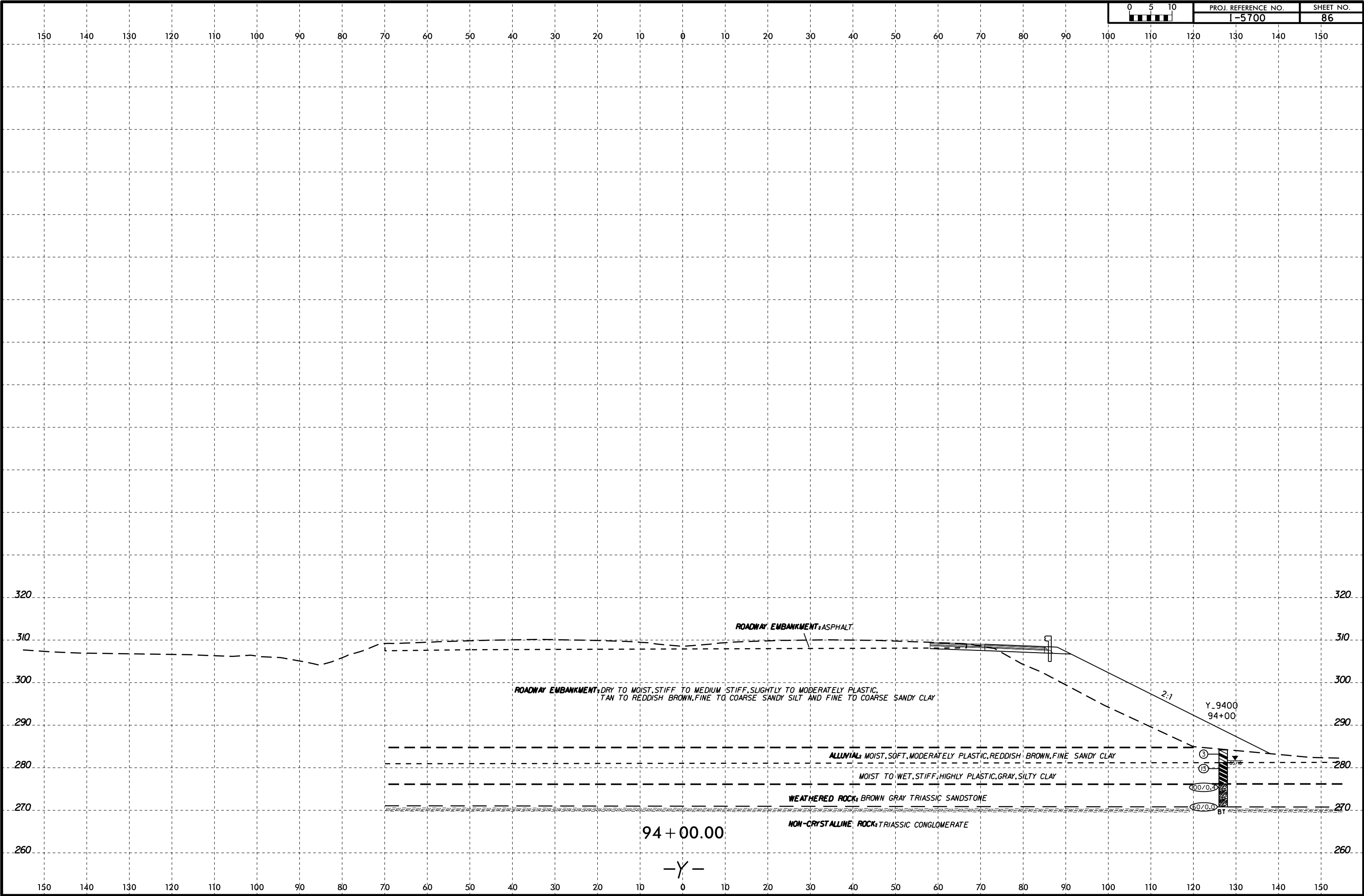


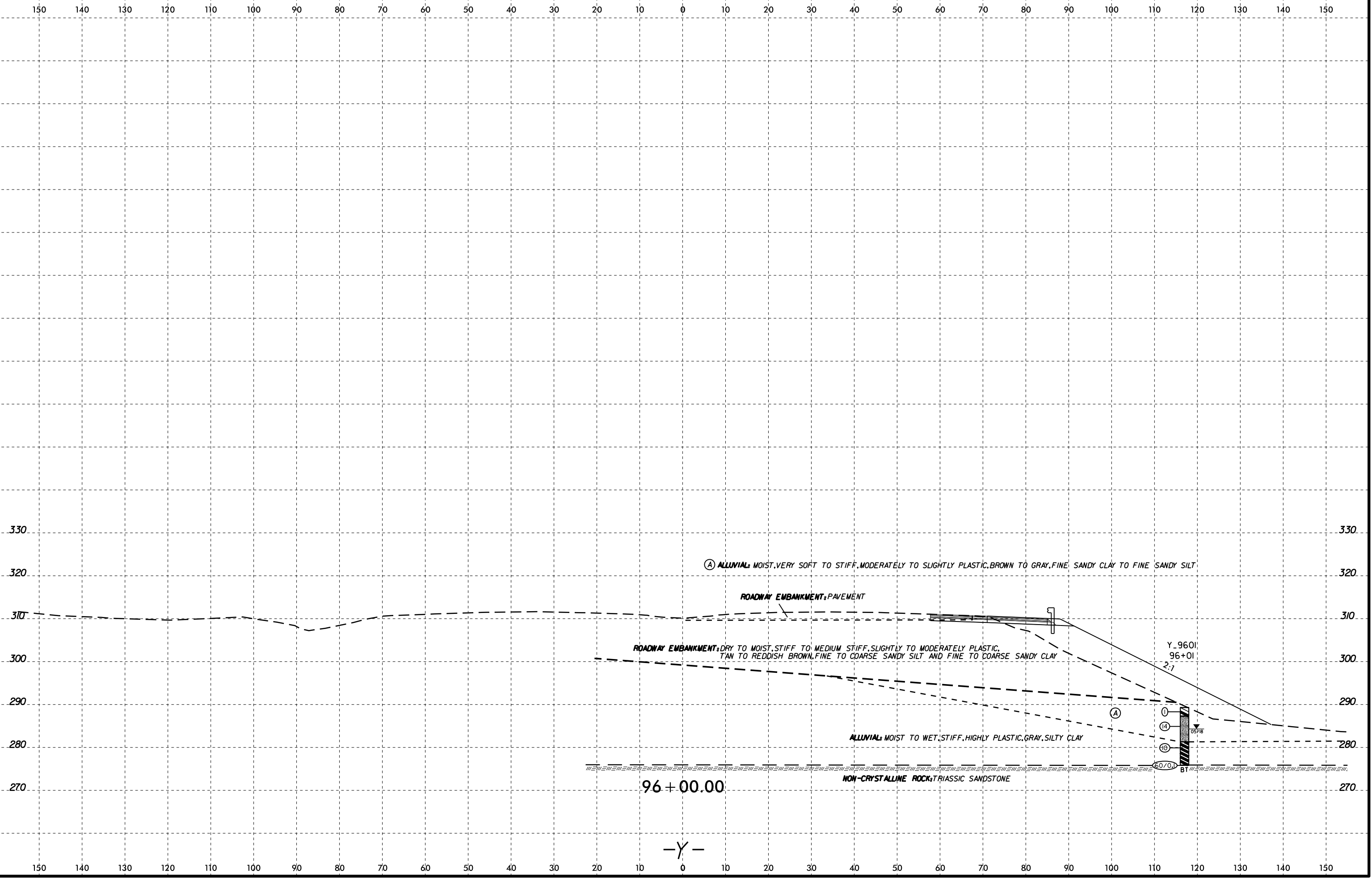


6/23/16  
 14-AUG-2018 15:03  
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 twells



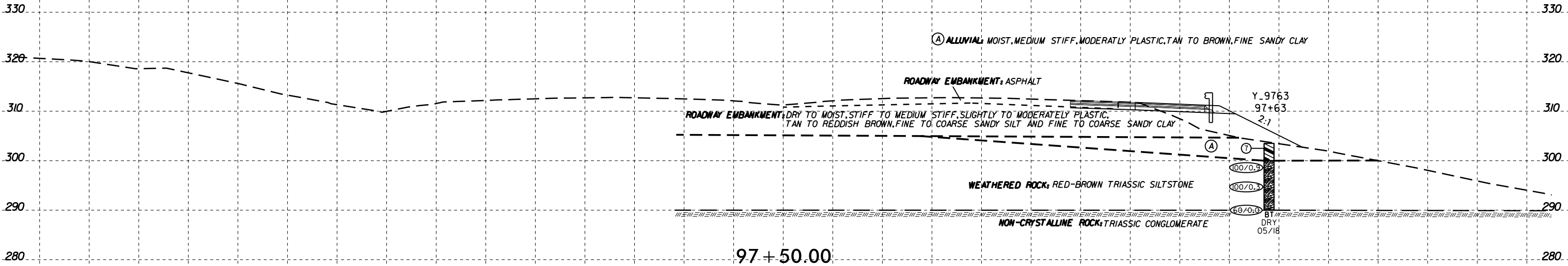








150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



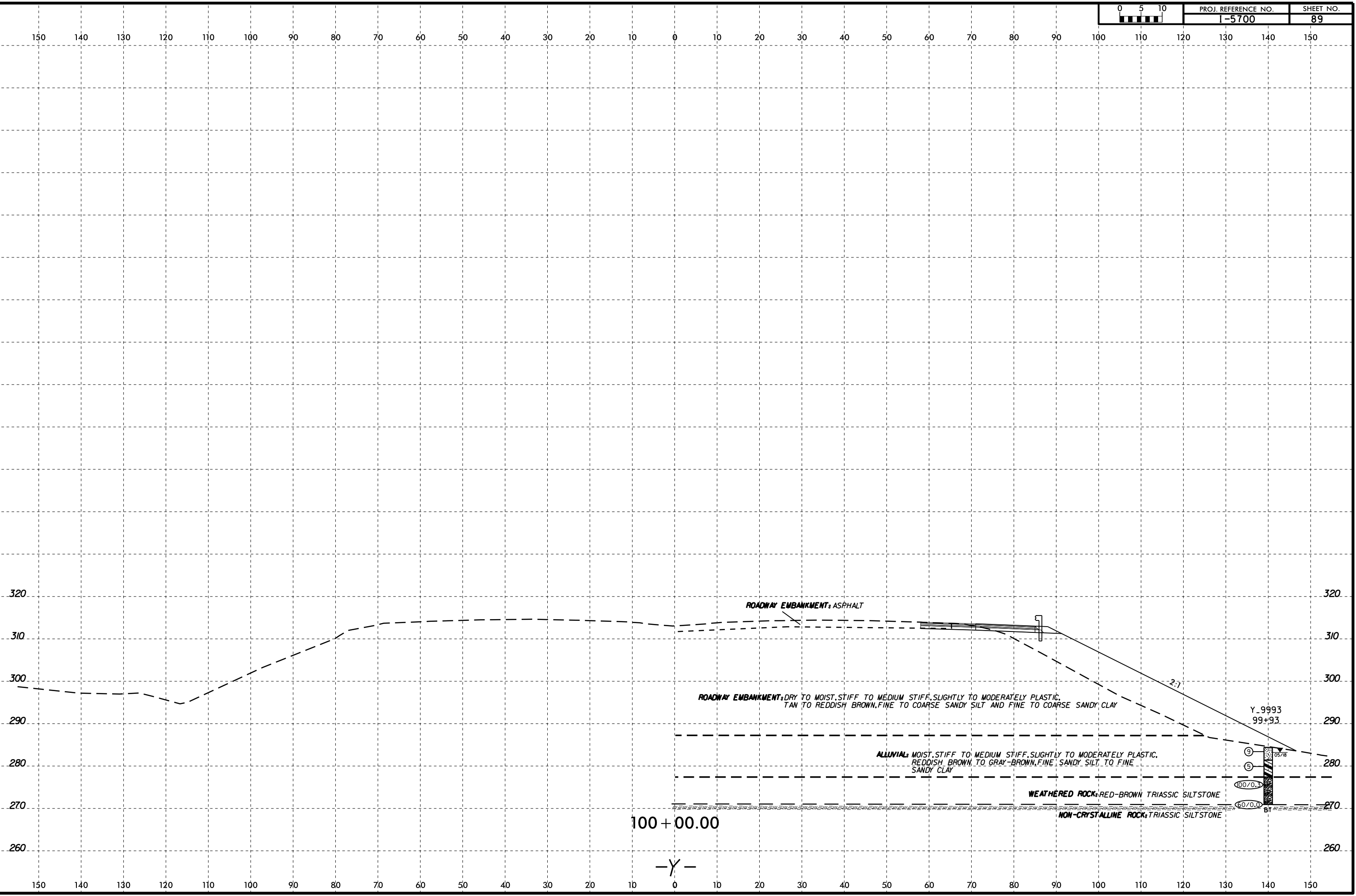
97+50.00

-Y-

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6/23/16  
14-AUG-2018 15:03  
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15700\_GEO\ROAD\CADD\_GEO\TECH\15700\_Geo.xst\_Y.dgn



ROADWAY EMBANKMENT: ASPHALT

ROADWAY EMBANKMENT: DRY TO MOIST, STIFF TO MEDIUM STIFF, SLIGHTLY TO MODERATELY PLASTIC, TAN TO REDDISH BROWN, FINE TO COARSE SANDY SILT AND FINE TO COARSE SANDY CLAY

ALLUVIAL: MOIST, STIFF TO MEDIUM STIFF, SLIGHTLY TO MODERATELY PLASTIC, REDDISH BROWN TO GRAY-BROWN, FINE SANDY SILT TO FINE SANDY CLAY

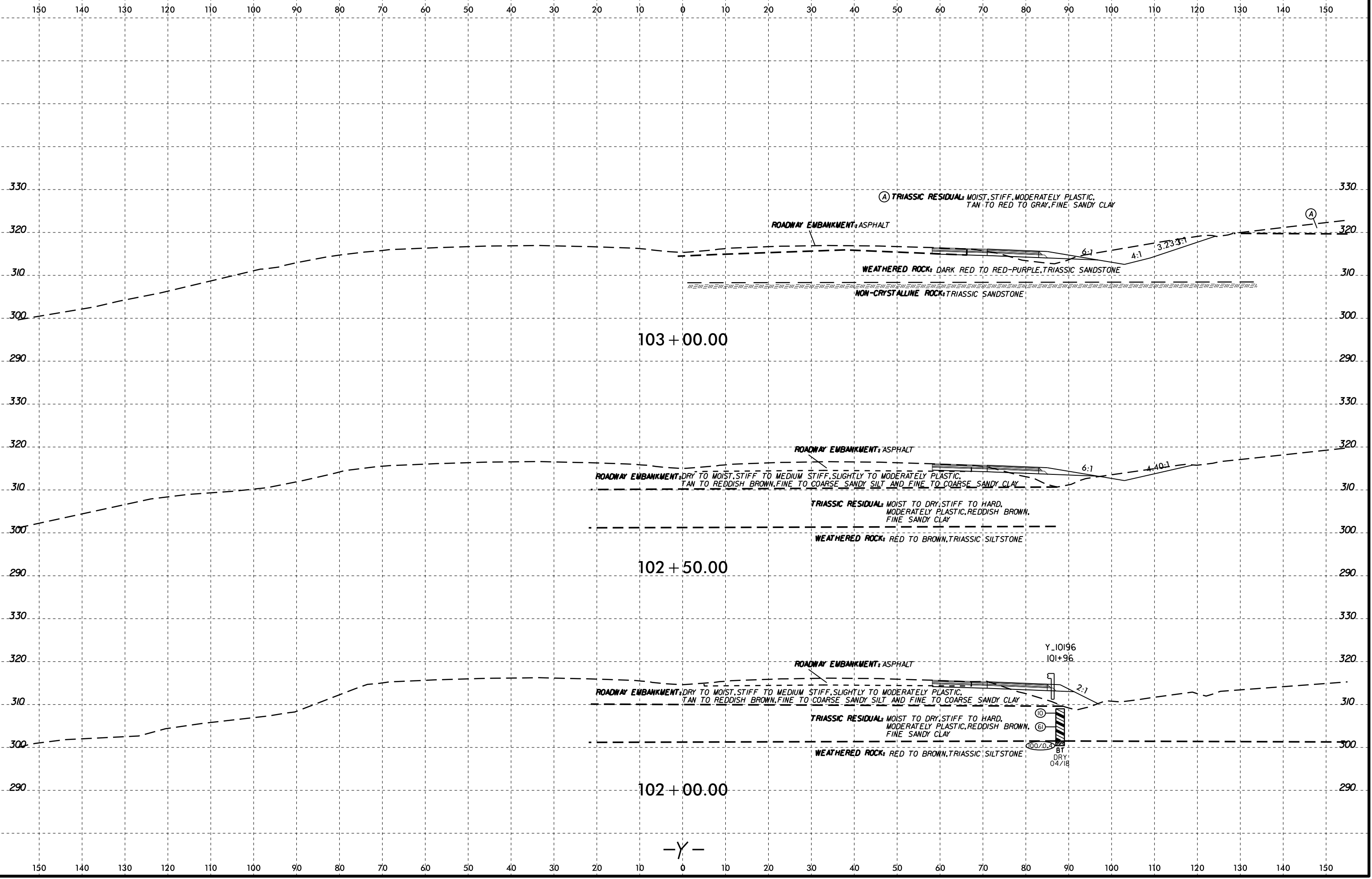
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NON-CRYSTALLINE ROCK: TRIASSIC SILTSTONE

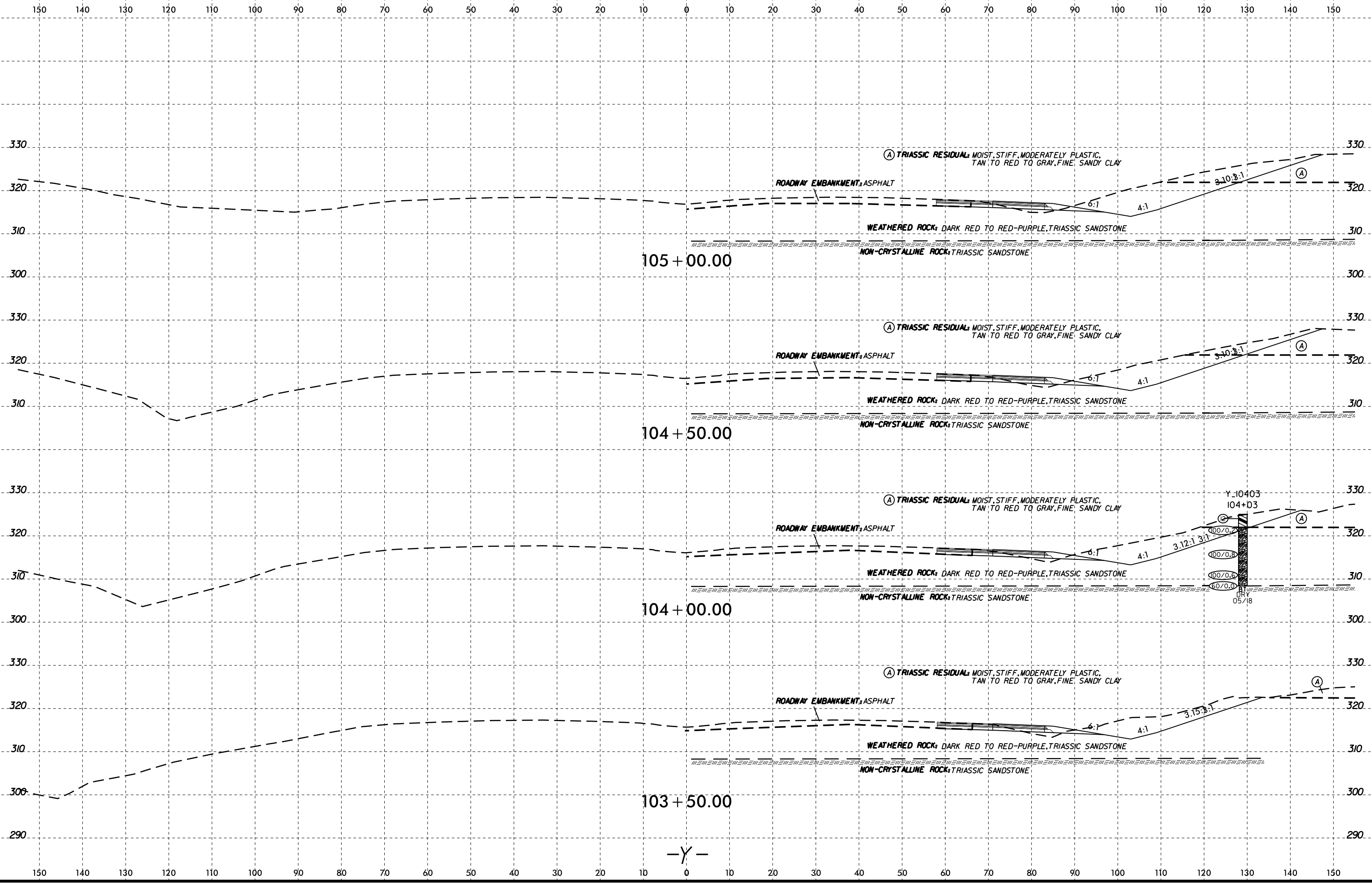
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99+93  
05/78  
060/0.0  
BT

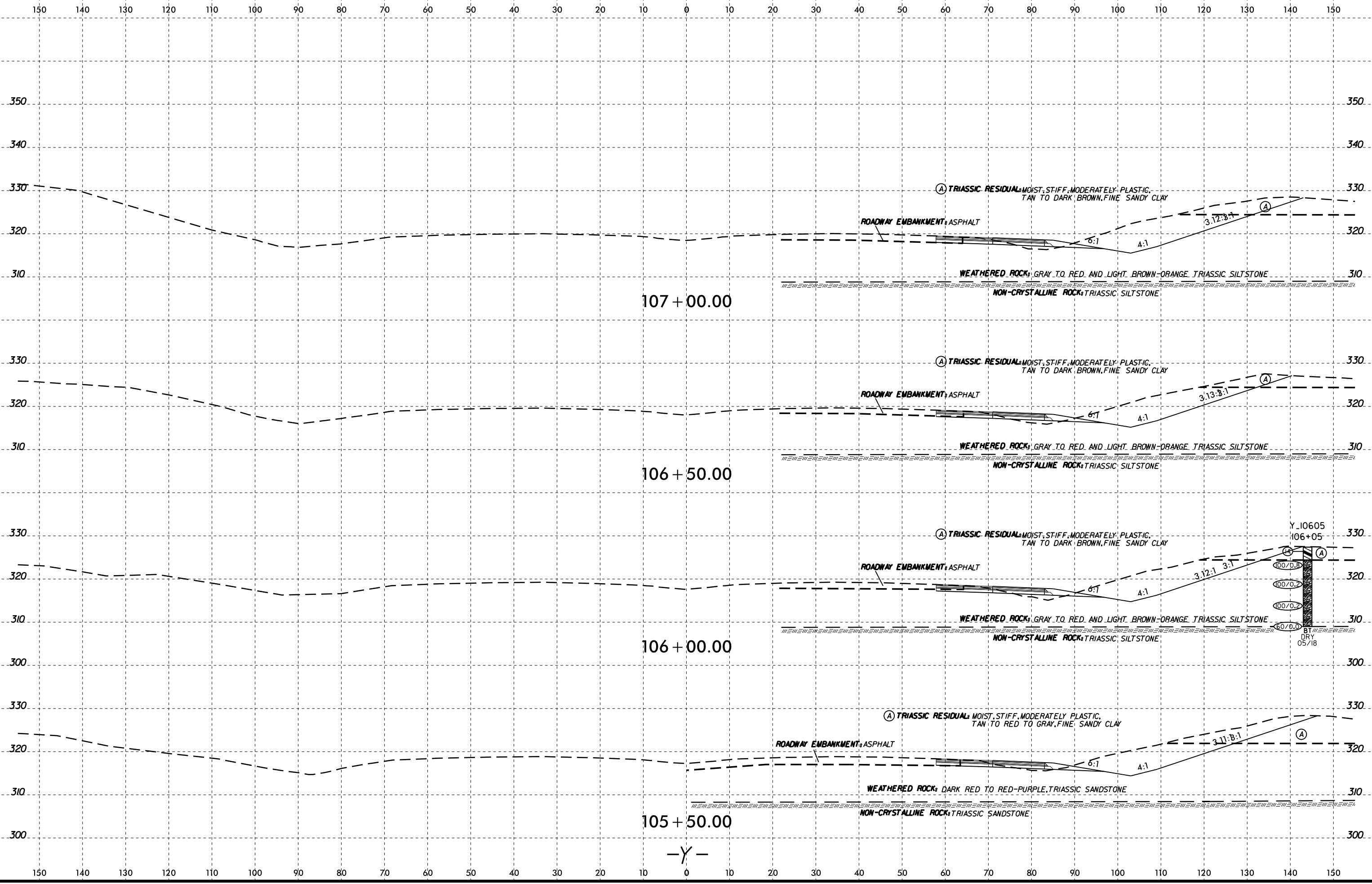
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-Y-

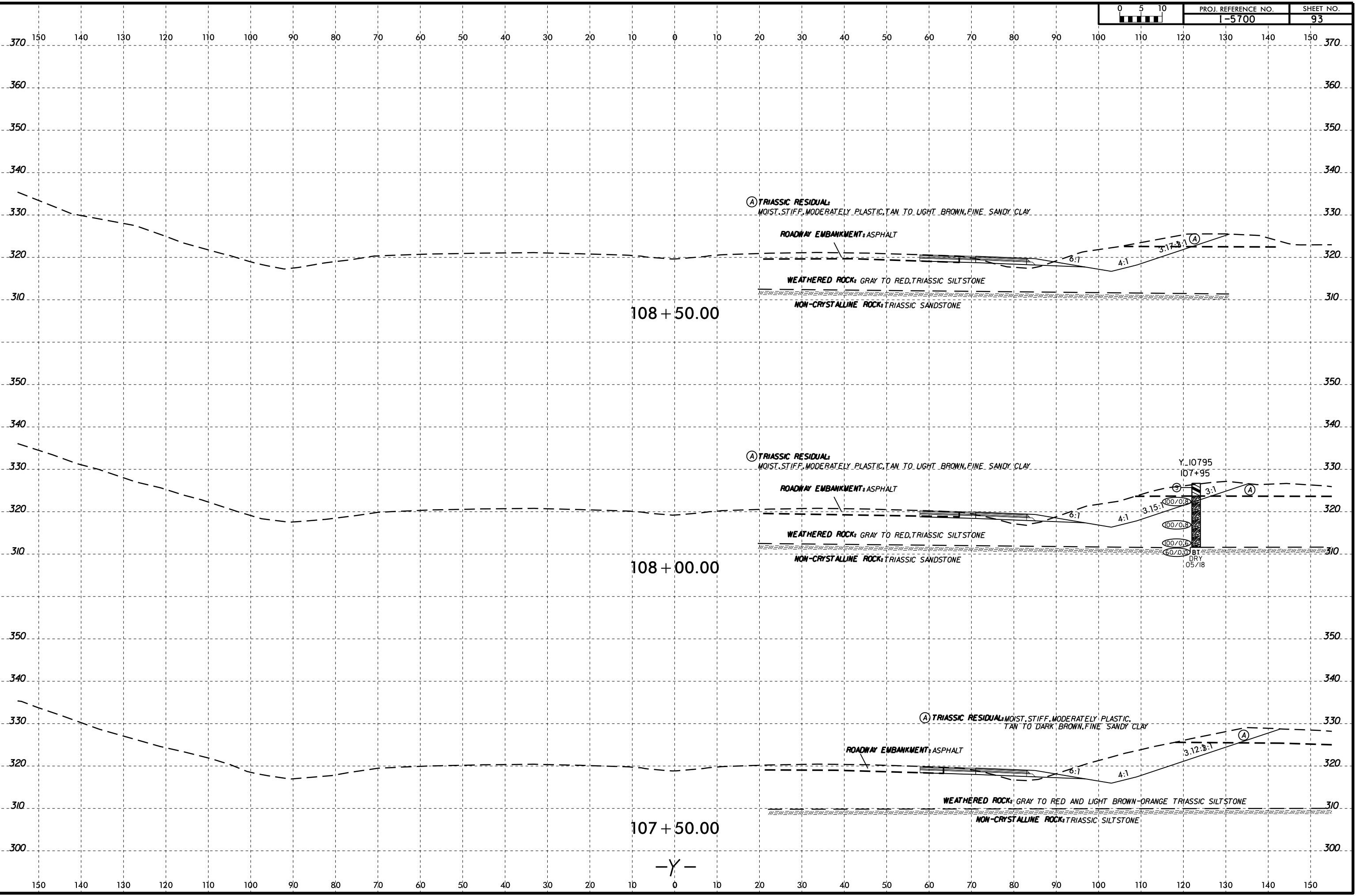


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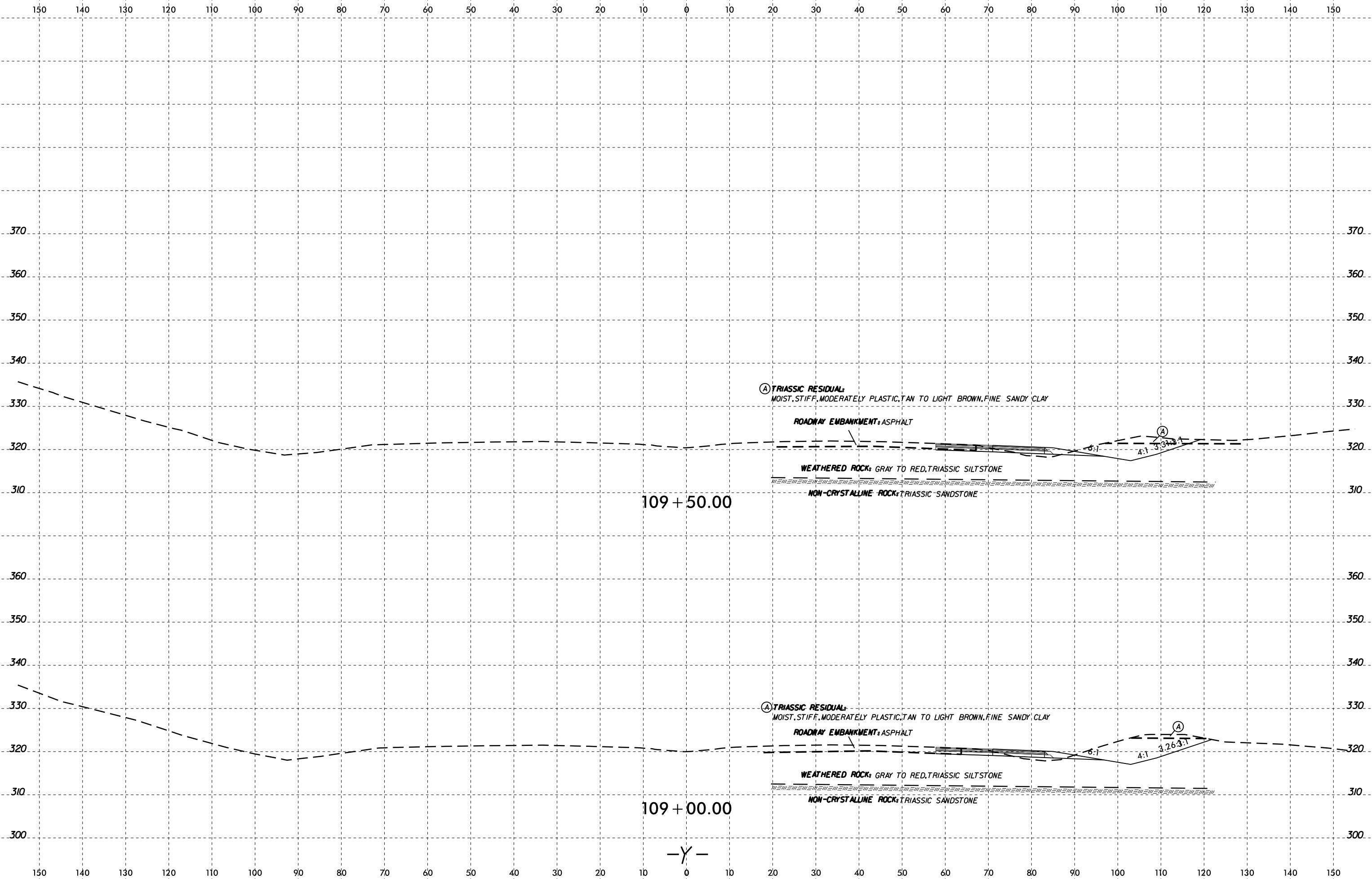




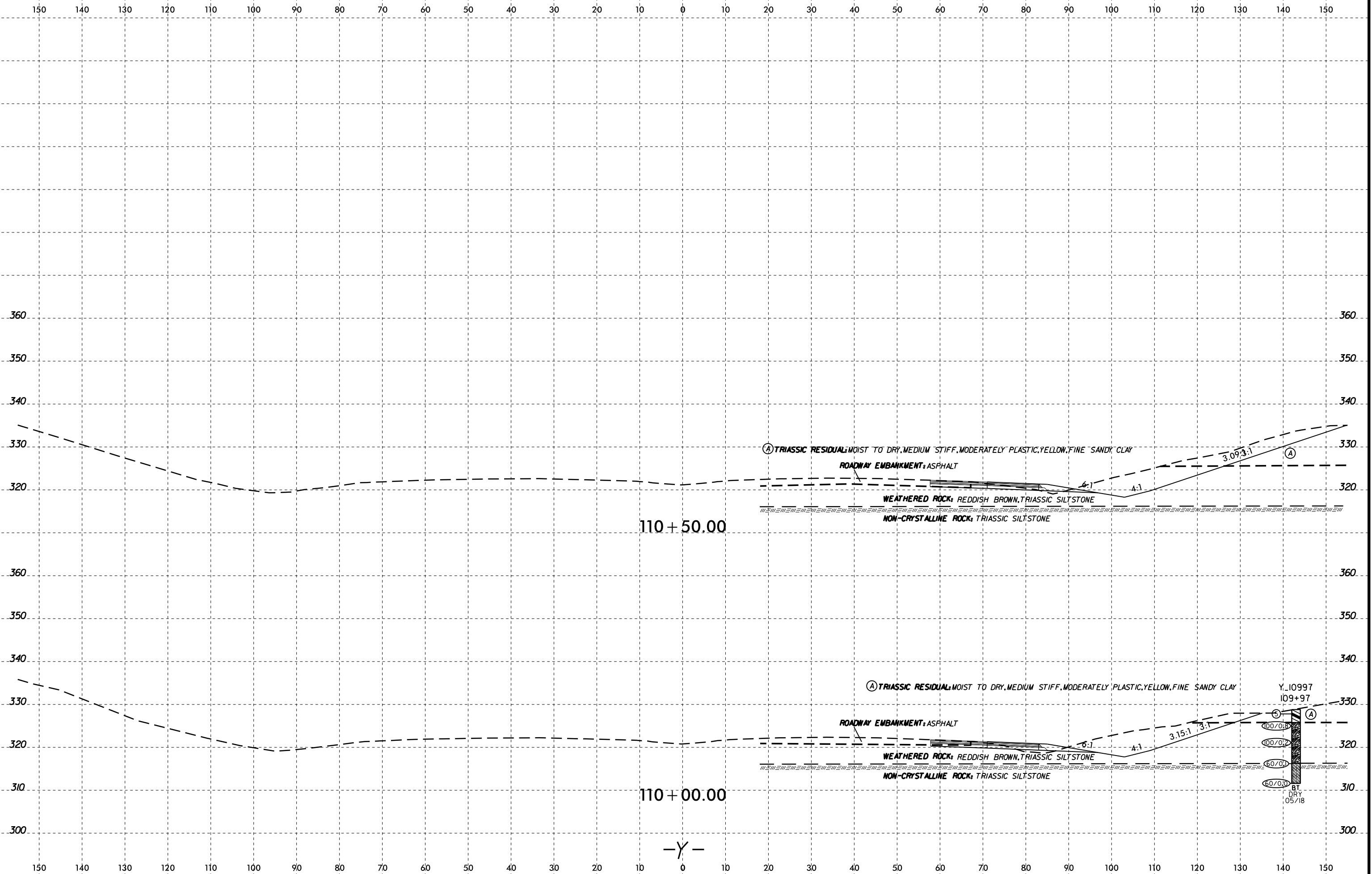
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twell



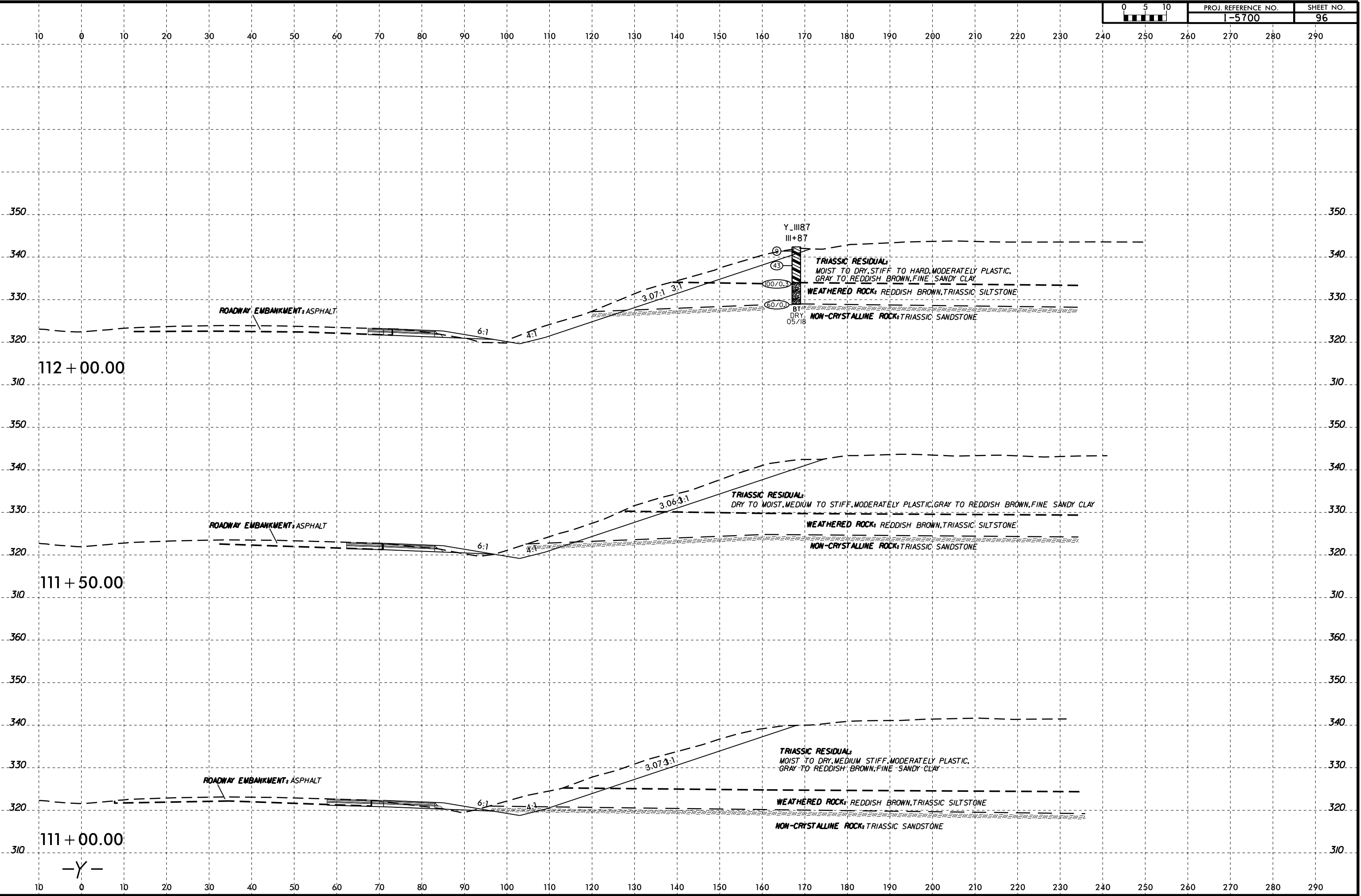
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6/23/16



6/23/16  
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twells



112 + 00.00

111 + 50.00

111 + 00.00

-Y-

ROADWAY EMBANKMENT, ASPHALT

ROADWAY EMBANKMENT, ASPHALT

ROADWAY EMBANKMENT, ASPHALT

Y-III87  
III+87  
9  
43  
100/0.3  
60/0.3  
81  
DRY  
05/18

TRIASSIC RESIDUAL:  
MOIST TO DRY, STIFF TO HARD, MODERATELY PLASTIC,  
GRAY TO REDDISH BROWN, FINE SANDY CLAY  
WEATHERED ROCK, REDDISH BROWN, TRIASSIC SILTSTONE  
NON-CRYSTALLINE ROCK, TRIASSIC SANDSTONE

TRIASSIC RESIDUAL:  
DRY TO MOIST, MEDIUM TO STIFF, MODERATELY PLASTIC, GRAY TO REDDISH BROWN, FINE SANDY CLAY  
WEATHERED ROCK, REDDISH BROWN, TRIASSIC SILTSTONE  
NON-CRYSTALLINE ROCK, TRIASSIC SANDSTONE

TRIASSIC RESIDUAL:  
MOIST TO DRY, MEDIUM STIFF, MODERATELY PLASTIC,  
GRAY TO REDDISH BROWN, FINE SANDY CLAY  
WEATHERED ROCK, REDDISH BROWN, TRIASSIC SILTSTONE  
NON-CRYSTALLINE ROCK, TRIASSIC SANDSTONE

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3.06:1 3:1

3.07:1 3:1

6:1

6:1

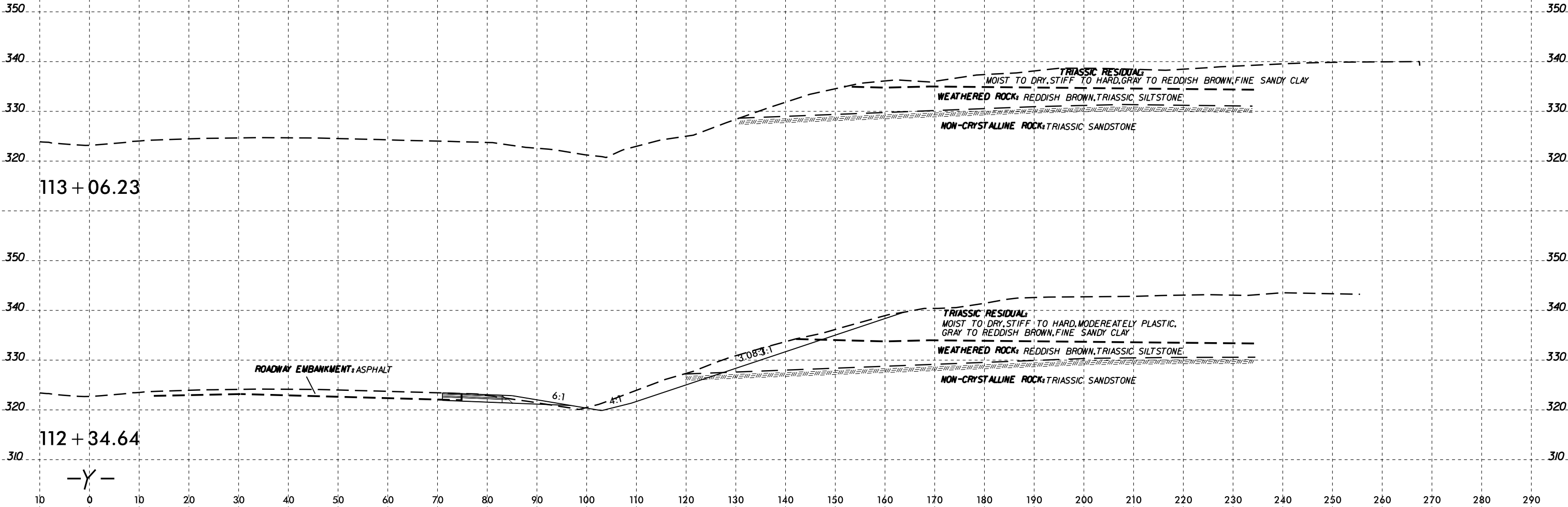
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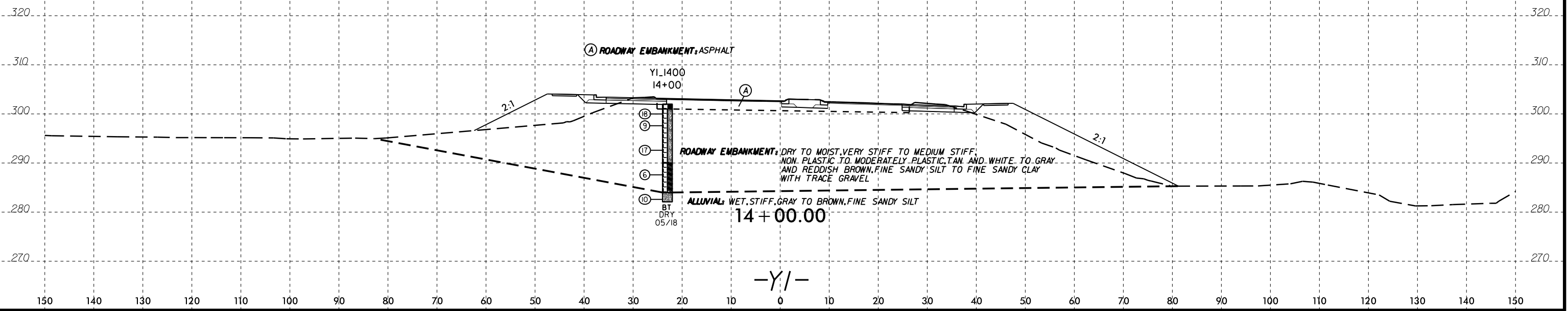
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PROJ. REFERENCE NO.	SHEET NO.
I-5700	98

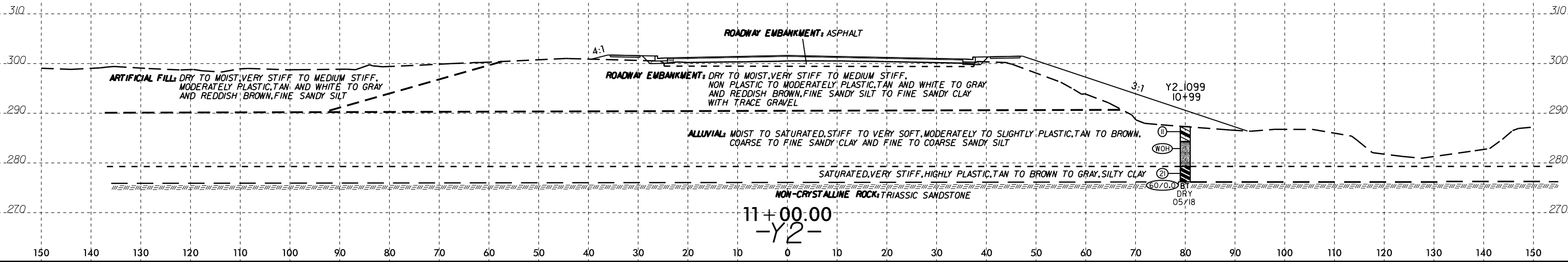
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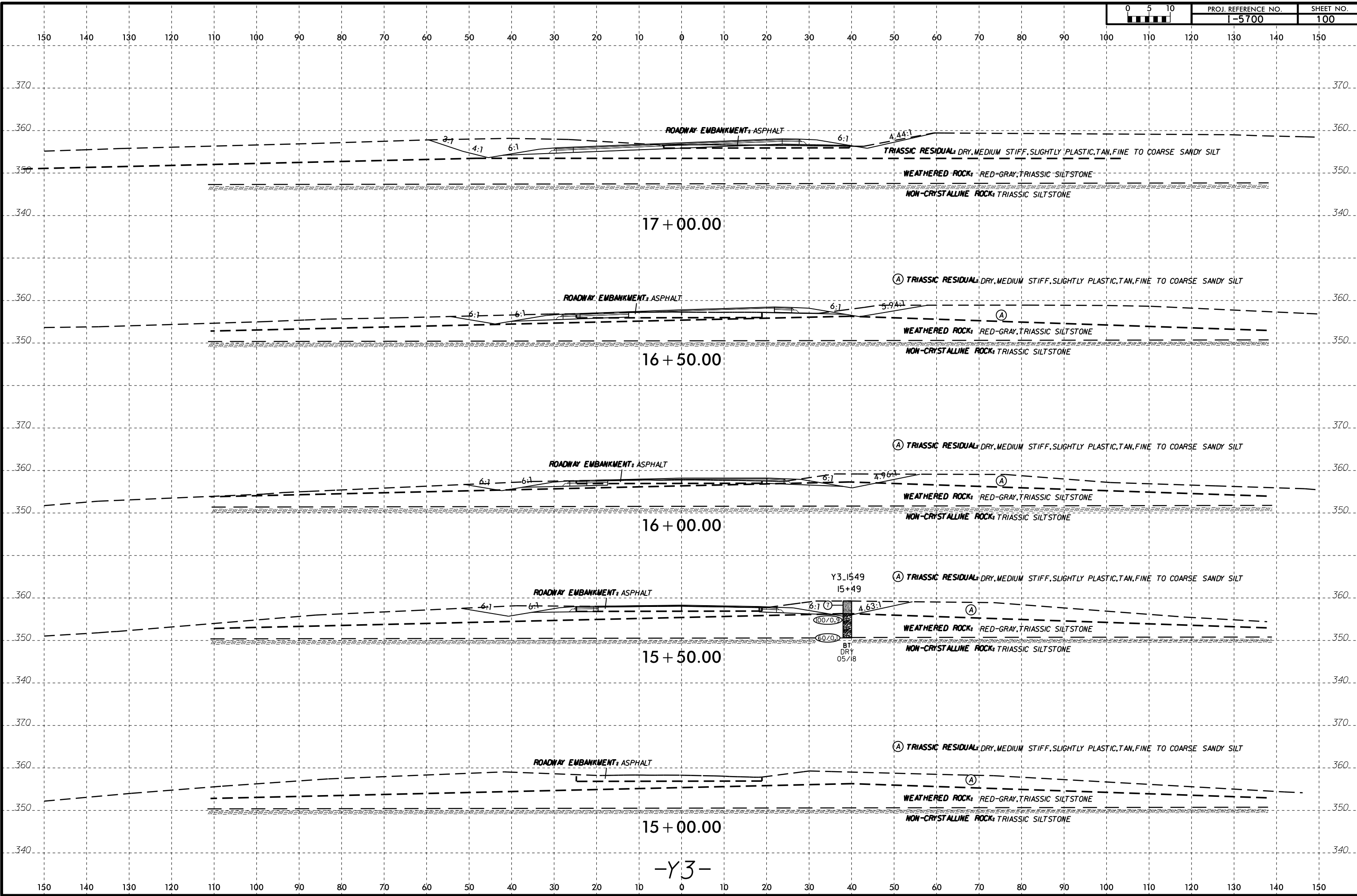




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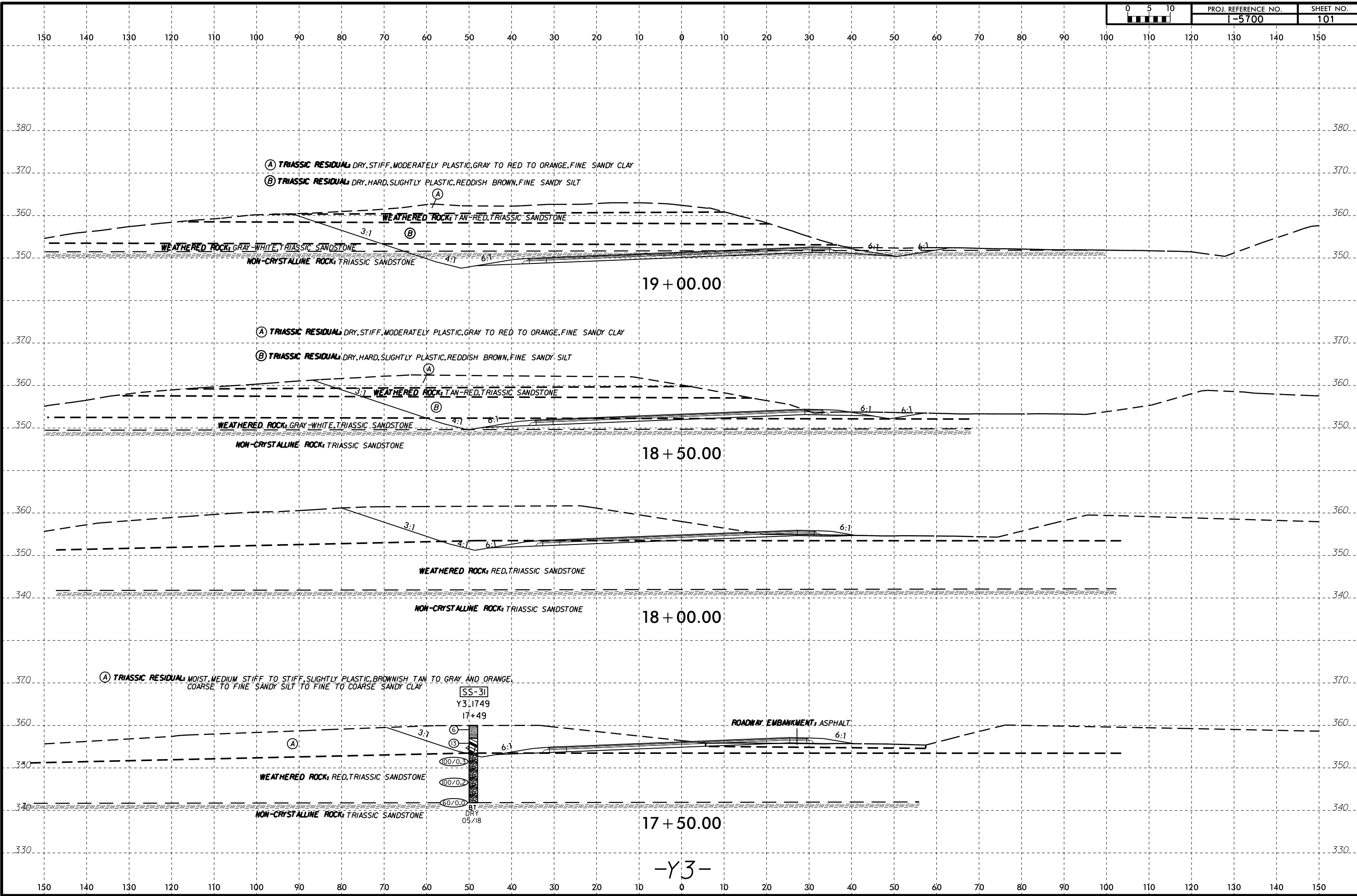


6/23/16



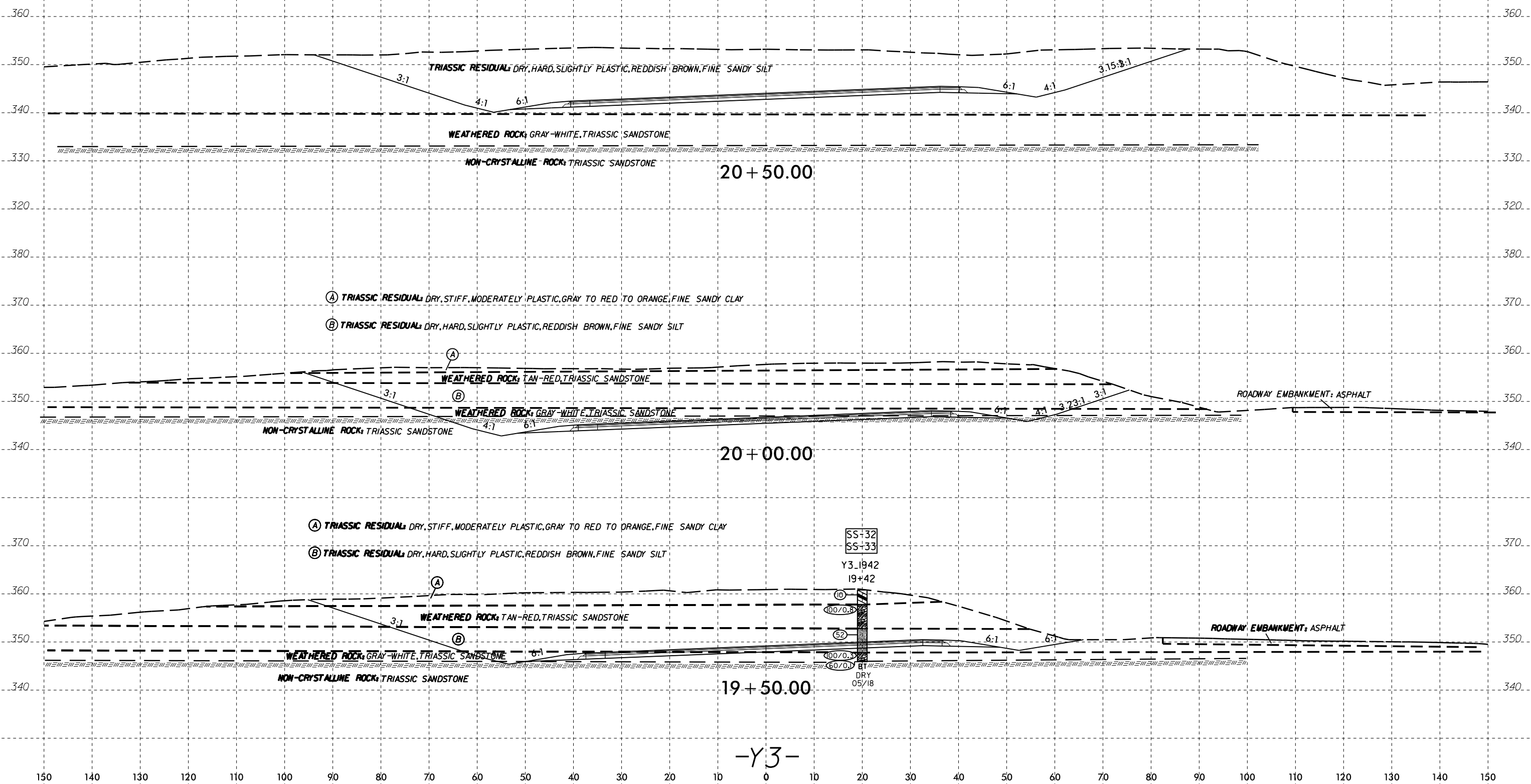
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-Y3-



-Y3-

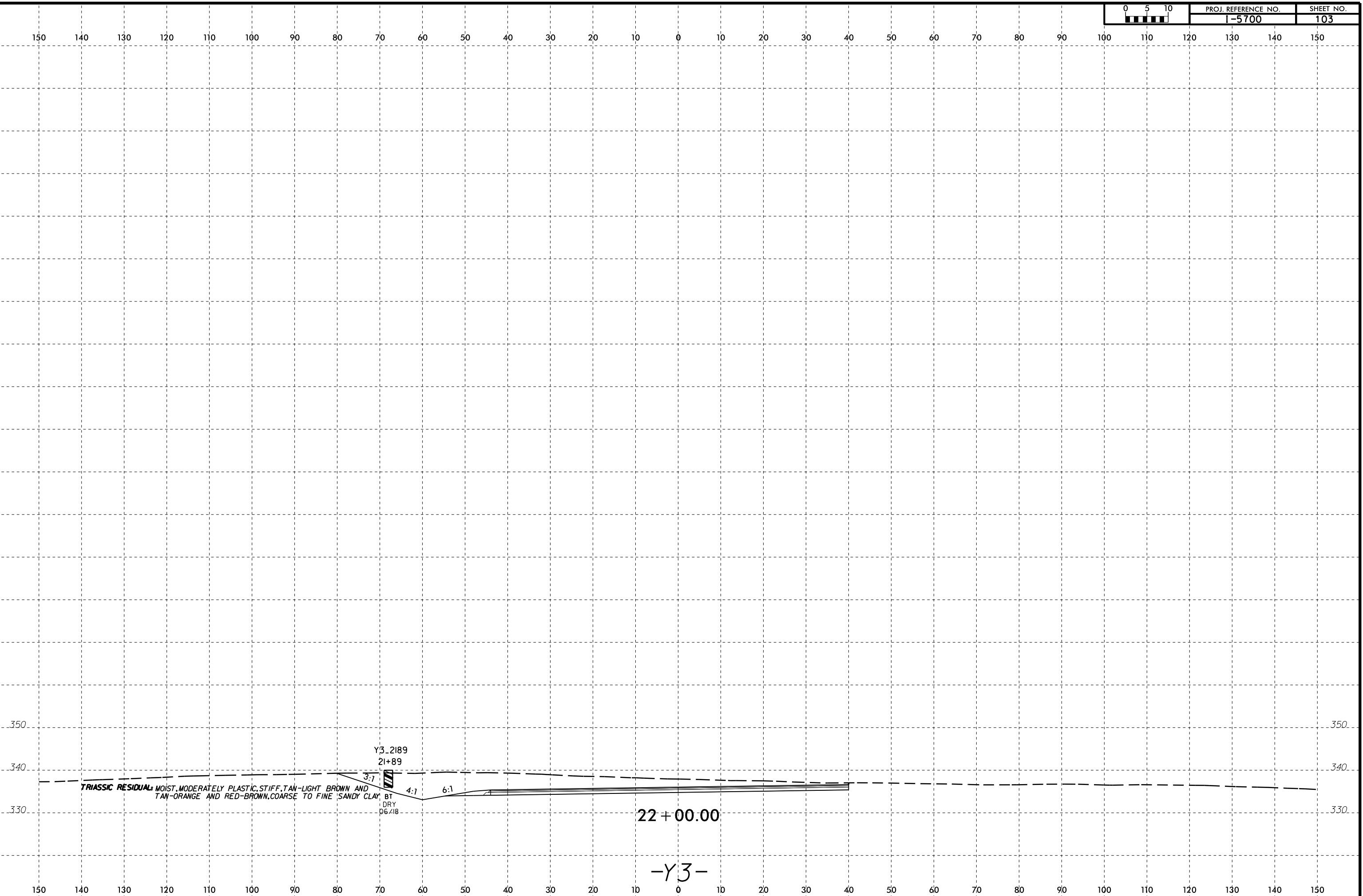
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twellis



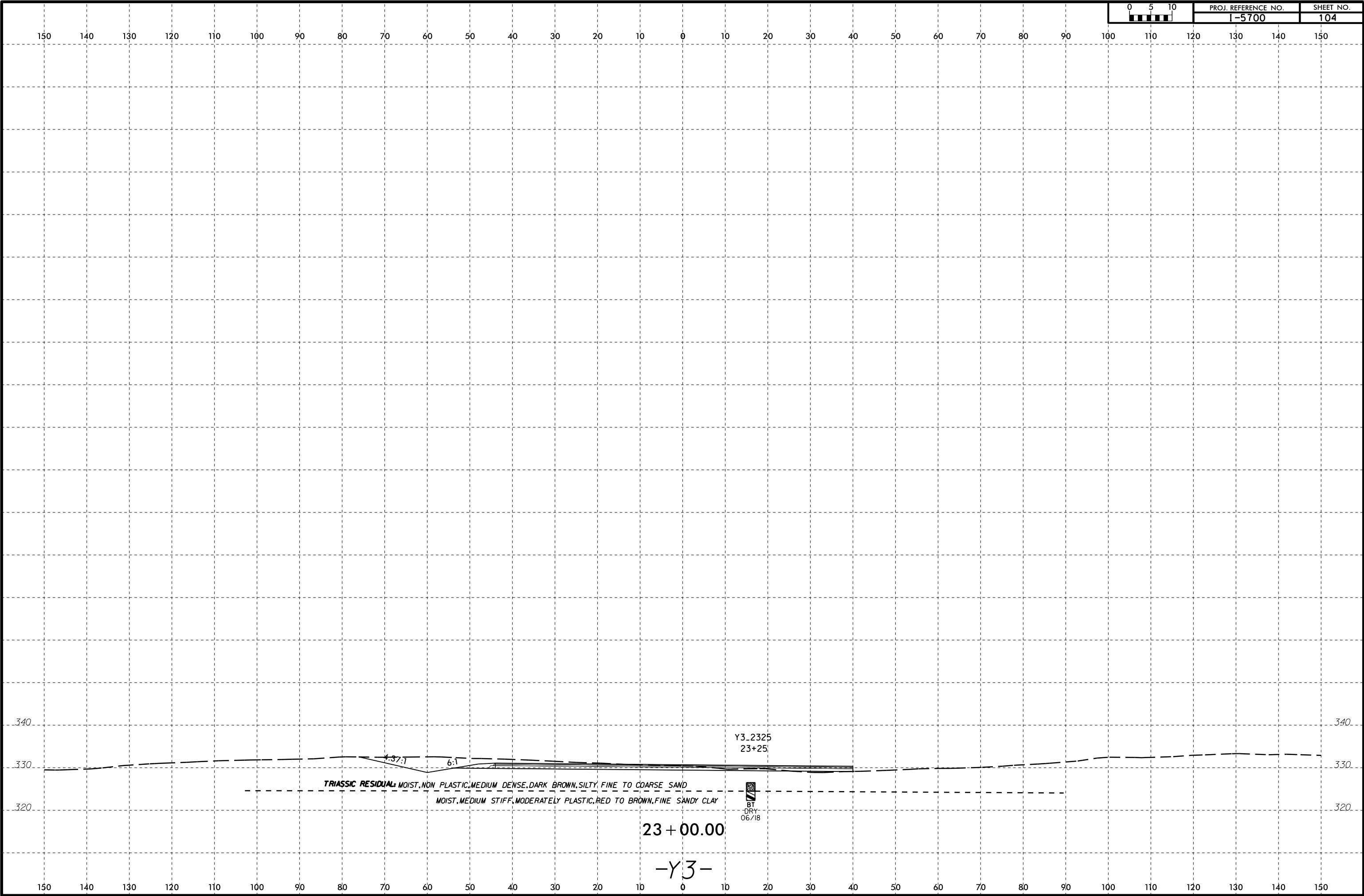
TRIASSIC RESIDUAL: MOIST, MODERATELY PLASTIC, STIFF, TAN-LIGHT BROWN AND TAN-ORANGE AND RED-BROWN, COARSE TO FINE SANDY CLAY

Y3\_2189  
21+89  
BT  
DRY  
06/18

22+00.00

-Y3-

14-AUG-2018 15:04  
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6/23/16



TRIASSIC RESIDUAL MOIST, NON PLASTIC, MEDIUM DENSE, DARK BROWN, SILTY, FINE TO COARSE SAND  
MOIST, MEDIUM STIFF, MODERATELY PLASTIC, RED TO BROWN, FINE SANDY CLAY

Y3.2325  
23+25

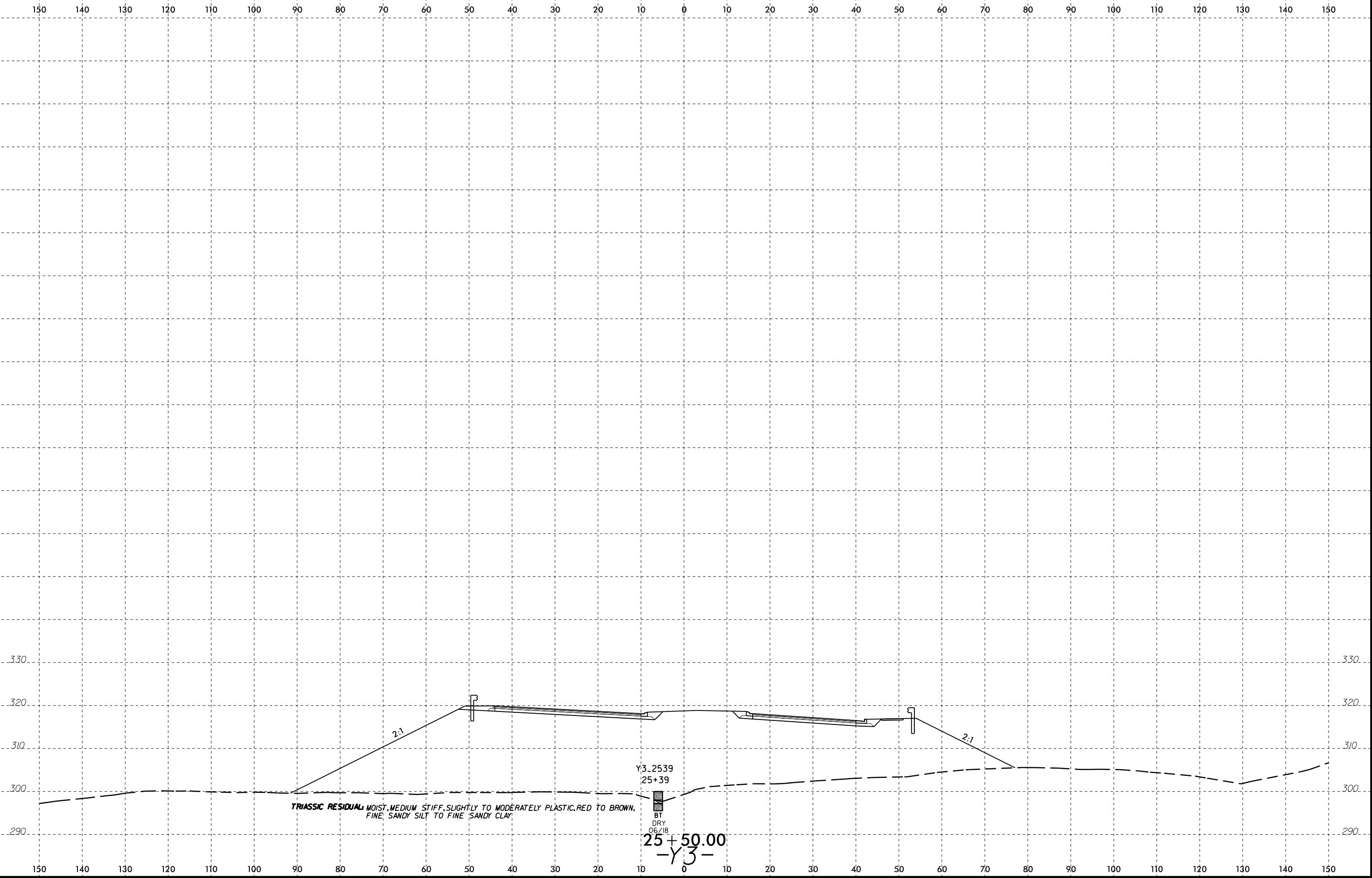
BT  
DRY  
06/18

23 + 00.00

-Y3-



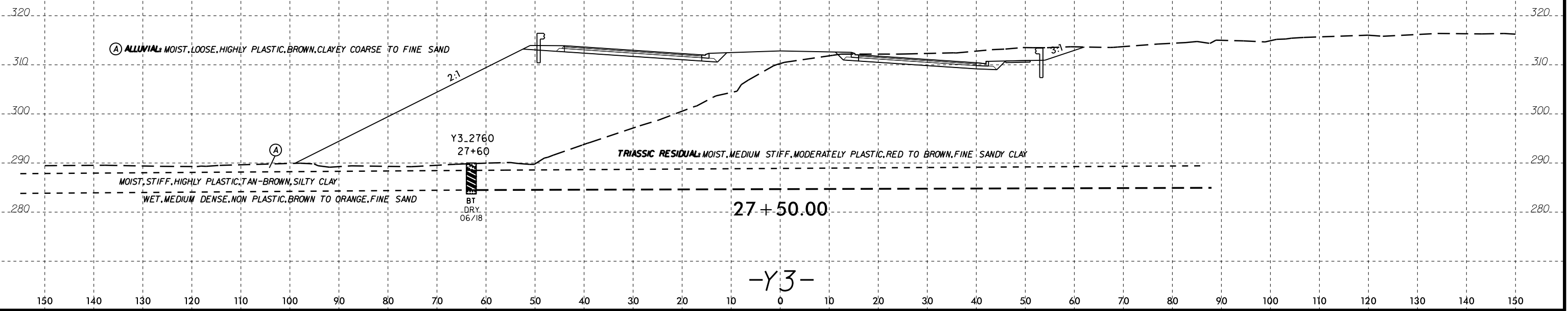
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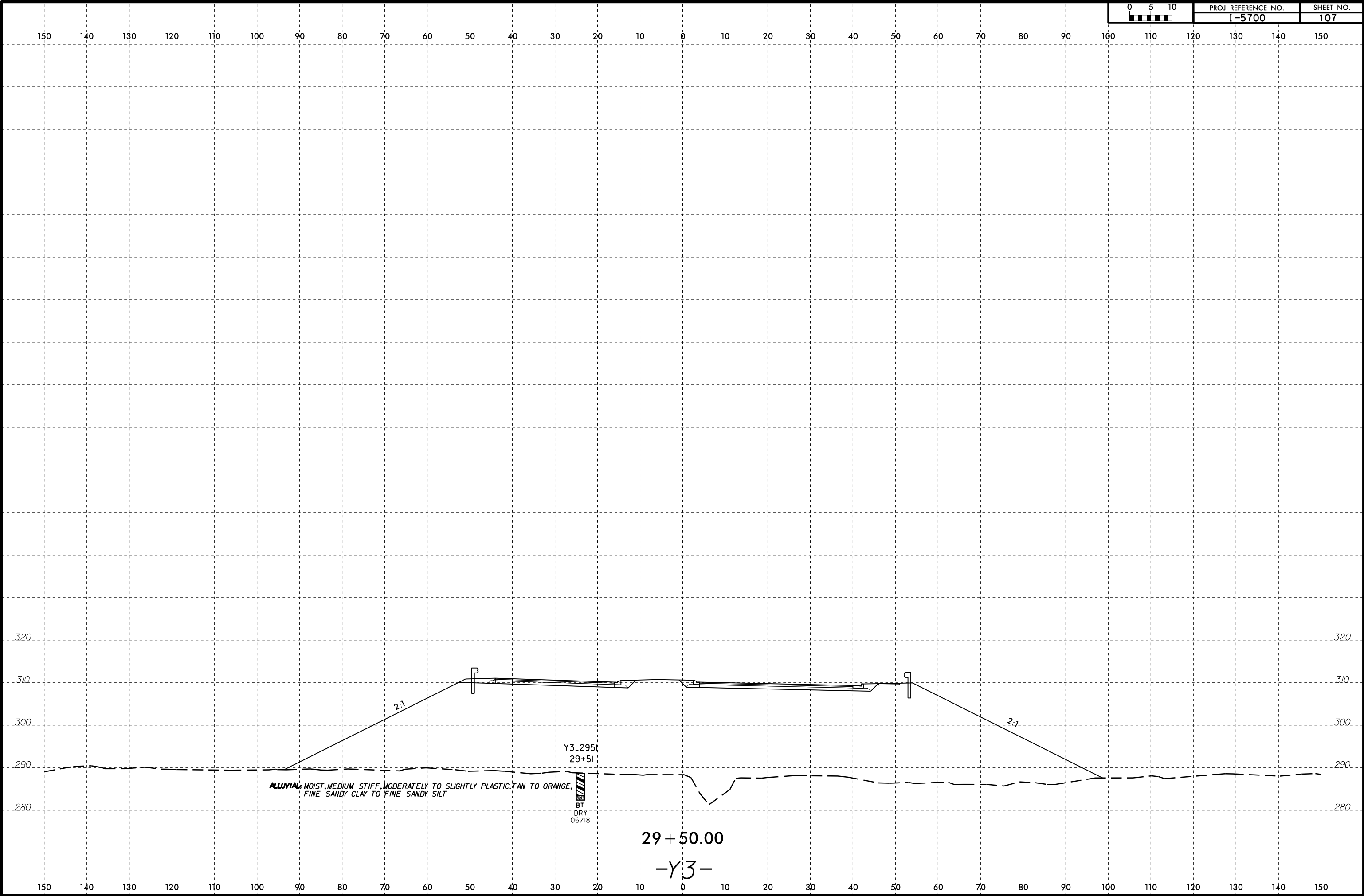
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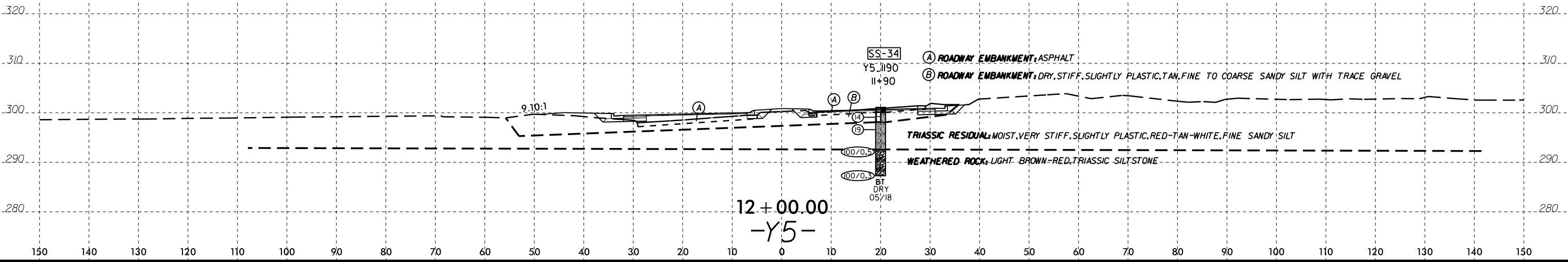
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PROJ. REFERENCE NO.	SHEET NO.
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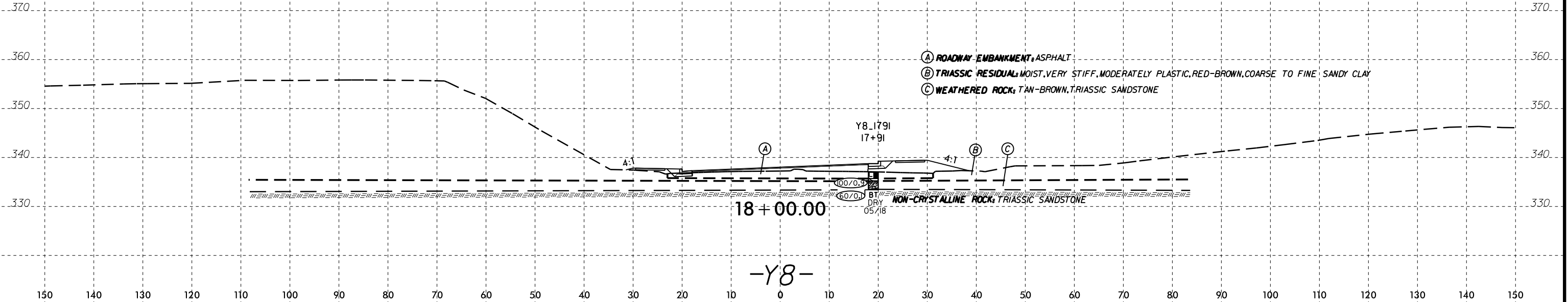
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 twells





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18+00.00  
-Y8-

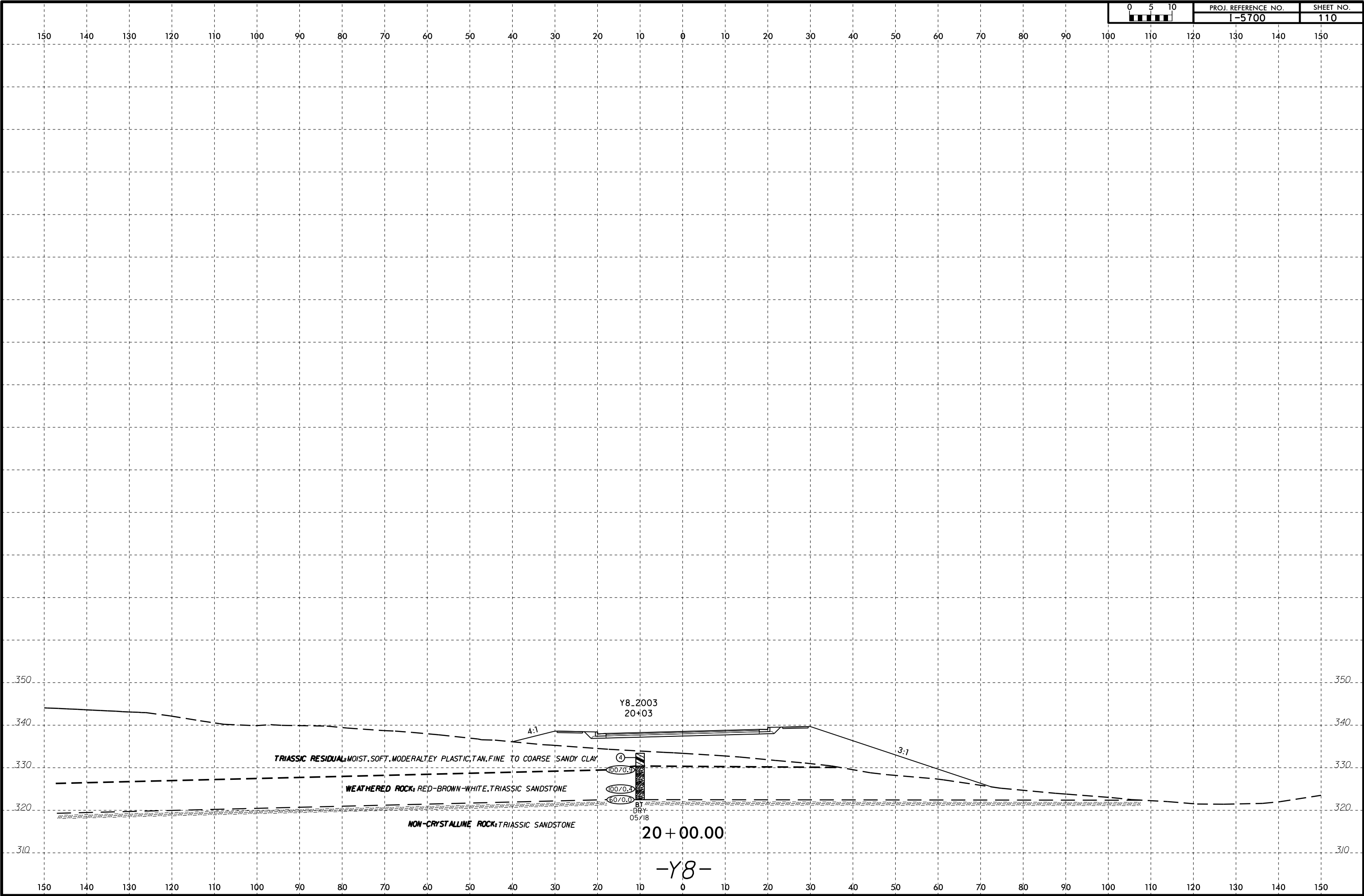
- (A) ROADWAY EMBANKMENT, ASPHALT
- (B) TRIASSIC RESIDUAL, MOIST, VERY STIFF, MODERATELY PLASTIC, RED-BROWN, COARSE TO FINE SANDY CLAY
- (C) WEATHERED ROCK, TAN-BROWN, TRIASSIC SANDSTONE

Y8\_1791  
17+91

0.9  
0.9  
BT  
DRY  
05/18

NON-CRYSTALLINE ROCK, TRIASSIC SANDSTONE

14-AUG-2018 15:04  
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6/23/16



*NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT  
SUBSURFACE INVESTIGATION  
APPENDIX A  
LABORATORY RESULTS*

*REFERENCE: I-5700*

*PROJECT: 50118*

INITIALS

DATE

**LABORATORY SUMMARY SHEET FOR SOIL SAMPLES**

SHEET 112

**PROJECT NO.: 50118.1.FS1 (I-5700)**

**COUNTY: WAKE**

**I-40 AND SR 3015 (AIRPORT BOULEVARD) REVISE INTERCHANGE AND CONSTRUCT AUXILIARY LANE**

Sample No.	Boring Number	Alignment	Station	Offset	Sample Depth (ft.)	Natural Moisture Content (%)	AASHTO Class.	Atterberg Limits			Gradation Results							
								L.L.	P.L.	P.I.	Retained #4 Sieve	Pass #10 Sieve	Pass #40 Sieve	Pass #200 Sieve	Coarse Sand (%)	Fine Sand (%)	Silt (%)	Clay (%)
SS-1	L_2399	-L-	23+99	59' LT	0.0 - 1.5	10.6	A-6	34	22	12	2.0	95.6	82.2	54.5	23.5	23.2	29.4	23.8
SS-2	L_3000	-L-	30+00	49' LT	0.0 - 1.5	9.9	A-6	31	18	13	11.9	82.4	69.9	52.1	22.4	17.6	34.7	25.4
SS-3	L_5400	-L-	54+00	49' RT	3.7 - 5.2	--	A-4	26	19	7	23.9	67.8	54.6	35.8	27.6	25.1	30.3	17.0
SS-4	L_6006	-L-	60+06	53' LT	1.2 - 2.7	7.8	A-6	27	16	11	17.8	76.5	61.5	39.9	30.4	21.6	29.3	18.7
SS-5	RPA_1250	-RPA-	12+50	30' RT	1.3 - 2.8	13.2	A-4	24	16	8	16.8	76.0	65.7	49.7	19.7	20.6	38.0	21.7
ST-6	RPA_1449	-RPA-	14+49	39' RT	5.0 - 7.0	--	A-4	21	18	3	0.0	100.0	99.6	65.4	2.7	40.4	38.0	18.9
SS-7	RPA_1641	-RPA-	16+41	8' RT	3.1 - 4.6	4.7	A-4	25	18	7	0.2	99.3	90.8	60.7	17.0	28.3	34.5	20.2
SS-8	RPASPUR_1139	-RPASPUR-	11+39	10' RT	0.4 - 1.5	20.3	A-4	31	21	10	0.1	99.4	92.9	70.3	13.1	21.5	33.6	31.8
SS-9	RPB_1191	-RPB-	11+91	58' LT	0.0 - 1.5	13.8	A-4	26	18	8	2.6	96.0	85.1	59.7	18.5	25.0	32.1	24.4
SS-10	RPB_1646	-RPB-	16+46	120' LT	0.0 - 1.5	15.2	A-6	36	22	14	4.0	93.8	79.4	57.1	22.5	21.5	25.9	30.1
SS-11	RPB_1646	-RPB-	16+46	120' LT	3.5 - 5.0	10.6	A-4	28	22	6	0.1	99.6	93.1	59.1	15.5	31.1	30.7	22.7
S-12	RPB_1761	-RPB-	17+61	116' LT	0.0 - 10.0	9.1	A-6	36	20	16	5.4	91.9	81.2	62.6	17.7	18.6	27.1	36.6
SS-13	RPB_1850	-RPB-	18+50	88' LT	8.5 - 10.0	12.8	A-4	27	19	8	0.1	99.6	96.0	81.6	6.8	16.2	44.3	32.7
SS-14	RPB_2199	-RPB-	21+99	CL	0.2 - 1.5	12.6	A-4	27	18	9	2.7	94.5	78.2	54.2	26.0	21.9	29.4	22.7
SS-15	RPBSPUR_1245	-RPBSPUR-	12+45	5' LT	3.2 - 4.7	7.9	A-4	28	19	9	0.1	99.4	92.6	73.3	13.4	17.4	40.9	28.3
SS-16	RPC_1193	-RPC-	11+93	49' RT	3.5 - 5.0	8.0	A-4	21	16	5	5.7	91.3	66.7	35.6	40.4	24.7	22.3	12.6
SS-17	RPC_1600	-RPC-	16+00	20' LT	3.2 - 4.7	15.8	A-6	33	22	11	0.9	98.4	85.2	53.8	22.2	29.6	25.9	22.3
SS-18	RPD_1659	-RPD-	16+59	30' RT	0.0 - 1.5	12.7	A-4	29	19	10	25.4	71.4	60.8	45.0	21.9	19.5	31.9	26.7
SS-19	RPD_2050	-RPD-	20+50	49' RT	0.1 - 1.5	4.8	A-4	22	20	2	0.4	98.6	77.0	37.1	37.8	28.9	20.3	13.0
SS-20	RPDSPUR_1303	-RPDSPUR-	13+03	5' LT	3.6 - 5.1	--	A-6	29	15	14	9.3	86.8	75.1	55.5	20.8	20.3	33.6	25.3
SS-21	Y_3679	-Y-	36+79	139' LT	3.7 - 5.2	9.5	A-4	33	24	9	0.0	100.0	99.0	95.7	1.5	5.9	54.9	37.7
SS-22	Y_4703	-Y-	47+03	115' LT	3.3 - 4.8	14.2	A-6	36	21	15	0.0	100.0	95.3	80.7	7.7	17.6	38.8	35.9
SS-23	Y_4902	-Y-	49+02	111' LT	0.0 - 1.5	10.0	A-6	33	21	12	0.0	99.8	97.1	80.8	6.1	17.5	46.1	30.3
SS-24	Y_5097	-Y-	50+97	106' LT	0.3 - 1.5	10.8	A-6	31	19	12	1.2	98.5	94.3	82.2	8.1	12.1	45.6	34.2
SS-25	Y_5999_LT	-Y-	59+99	99' LT	0.0 - 1.5	12.7	A-4	27	19	8	9.2	87.7	73.4	41.9	27.9	29.1	20.7	22.3
SS-26	Y_6199_LT	-Y-	61+99	130' LT	0.1 - 1.5	13.0	A-6	34	20	14	4.7	92.7	83.1	68.7	15.7	13.9	38.1	32.3
SS-27	Y_6201_RT	-Y-	62+01	90' RT	3.2 - 4.7	7.9	A-4	29	19	10	0.9	98.2	93.6	82.1	7.5	14.9	47.6	30.0
SS-28	Y_6407	-Y-	64+07	114' RT	8.2 - 9.7	6.3	A-6	29	17	12	0.0	99.5	93.9	74.6	12.2	15.9	32.9	39.0
SS-29	Y_7113_LT	-Y-	71+13	138' LT	0.2 - 1.5	4.4	A-4	26	21	5	0.0	100.0	99.4	70.8	2.3	39.0	40.0	18.7
SS-30	Y_7350	-Y-	73+50	100' LT	3.2 - 4.7	13.7	A-4	26	17	9	1.3	96.8	86.3	66.9	16.8	18.9	36.0	28.3
SS-31	Y3_1749	-Y3-	17+49	49' LT	3.2 - 4.7	--	A-6	34	22	12	1.3	97.0	78.9	45.7	32.4	24.1	26.3	17.2
SS-32	Y3_1942	-Y3-	19+42	20' RT	0.0 - 1.5	--	A-6	35	19	16	0.0	99.4	88.9	57.7	22.3	22.6	23.0	32.2
SS-33	Y3_1942	-Y3-	19+42	20' RT	8.3 - 9.8	8.0	A-4	28	18	10	0.1	99.7	94.6	77.3	10.1	15.7	43.8	30.4
SS-34	Y5_1190	-Y5-	11+90	20' RT	3.5 - 5.0	17.5	A-4	36	26	10	0.0	99.6	97.3	78.3	4.8	24.4	51.1	19.7





**CONSOLIDATED UNDRAINED TRIAXIAL TEST  
WITH PORE PRESSURE READINGS  
AASHTO T-297**

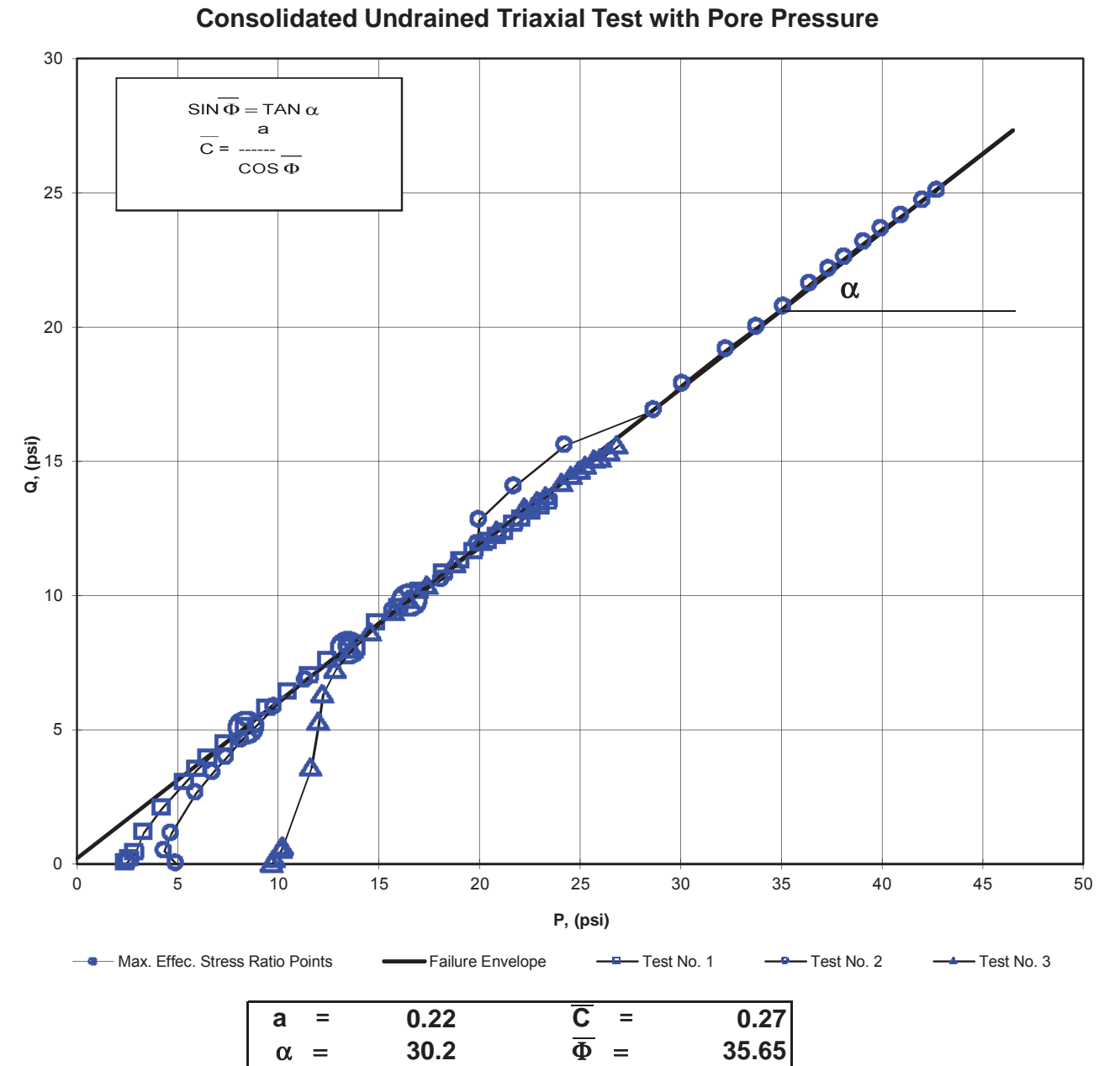
**SPECIFIC GRAVITY  
AASHTO T-100-15**

Client:	Kleinfelder	Boring No.:	RPA_1449
Client Reference:	I-5700 / 20151548.053A	Depth (ft):	5.0-7.0
Project No.:	R-2018-133-001	Sample No.:	ST-6
Lab ID:	R-2018-133-001-007		

Client	Kleinfelder	Boring No.	RPA_1450
Client Reference	I-5700 / 20151548.053A	Depth (ft)	5.0-7.0
Project No.	R-2018-133-001	Sample No.	ST-1
Lab ID	R-2018-133-001-007	Visual Description	<b>Brown</b> (Minus No.4 sieve material, airdried)

Replicate Number	1	2
Pycnometer ID	R 347	R 543
Weight of Pycnometer + Soil + Water (gm)	708.17	700.58
Temperature, T ( °Celsius )	23.7	23.6
Weight of Pycnometer + Water (gm)	669.25	661.23
Tare Number	347	543
Weight of Tare + Dry Soil (gm)	233.69	226.6
Weight of Tare (gm)	170.71	163.17
Weight of Dry Soil (gm)	62.98	63.43
Specific Gravity of Soil @ T	2.617	2.635
Specific Gravity of Water @ T	0.9974	0.9974
Conversion Factor for Temperature T	0.9992	0.9992
Specific Gravity @ 20° Celsius	2.619	2.637

Average Specific Gravity @ 20° Celsius	2.63
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Tested By 129-05-0411 Date 5/23/18 Checked By GEM Date 5/23/18

DCN: CT-S5 Date: 03/24/05 Revision: 10RZ:2018 PROJECTS\KLEINFELDER\2018-133 KLEINFELDER - I-5700 20151548.053A\2018-133-001-007 DOTSp. Gravity.xlsm\Sheet1

Tested By: 129-04-0411 Date: 5/16/18 Approved By: MPS Date: 5/23/18

page 1 of 11 DCN: CT-S28 DATE: 4/12/13 REVISION: 3

Sigmatrax.xls

**MOHR TOTAL STRENGTH ENVELOPE**  
AASHTO T-297



Client: Kleinfelder  
 Client Reference: I-5700 / 20151548.053A  
 Project No.: R-2018-133-001  
 Lab ID: R-2018-133-001-007  
 Visual Description: BROWN SANDY CLAY (UNDISTURBED)

Boring No.: RPA\_1449  
 Depth (ft): 5.0-7.0  
 Sample No.: ST-6

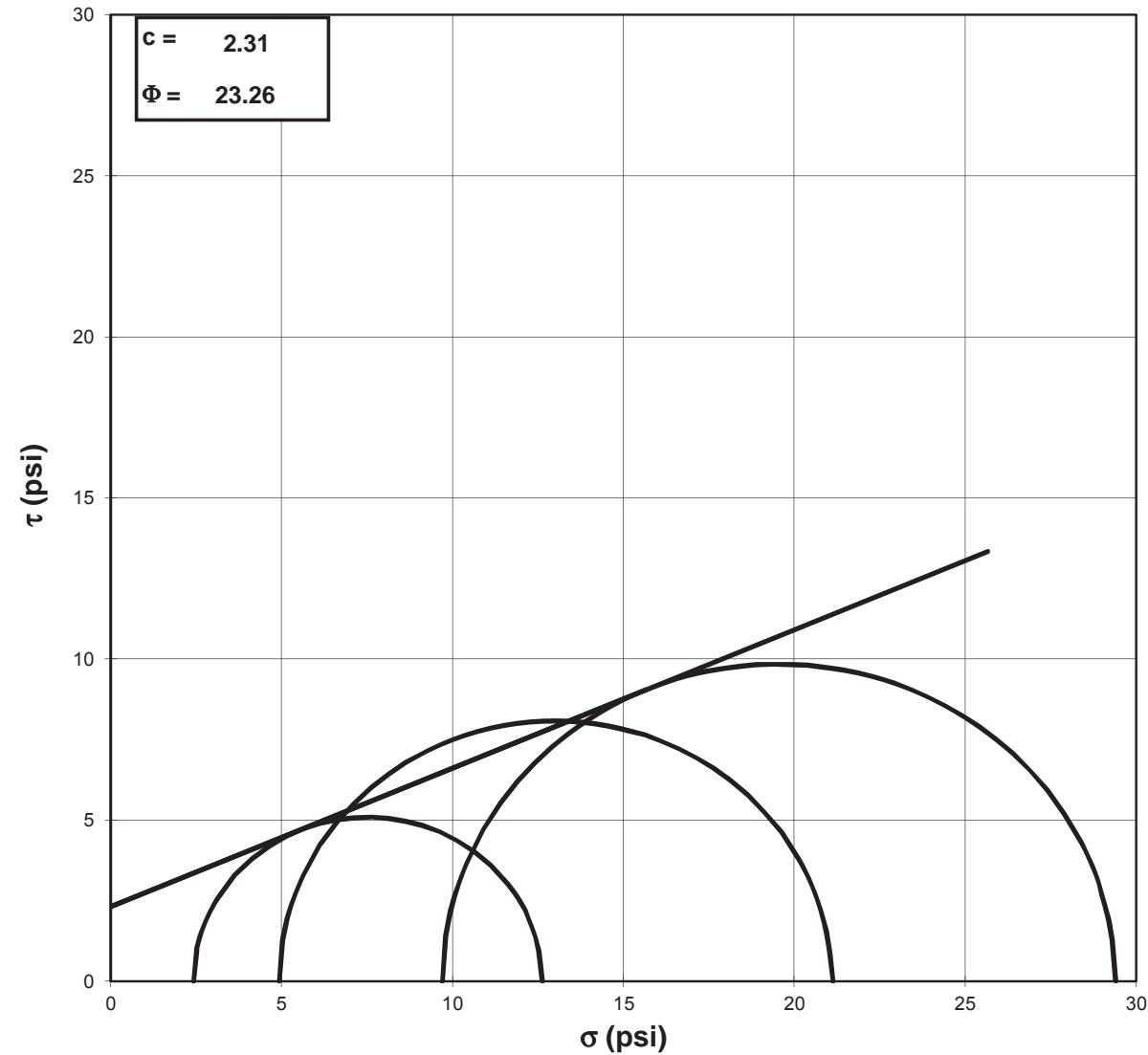
**CONSOLIDATED UNDRAINED TRIAXIAL TEST**  
**WITH PORE PRESSURE READINGS**  
AASHTO T-297



Client: Kleinfelder  
 Client Reference: I-5700 / 20151548.053A  
 Project No.: R-2018-133-001  
 Lab ID: R-2018-133-001-007

Boring No.: RPA\_1449  
 Depth (ft): 5.0-7.0  
 Sample No.: ST-6

Visual Description: BROWN SANDY CLAY (UNDISTURBED)



Stage No.	1
Test No.	1

**INITIAL SAMPLE DIMENSIONS (in)**

Length 1:	5.920	Diameter 1:	2.837
Length 2:	5.947	Diameter 2:	2.857
Length 3:	5.919	Diameter 3:	2.849
Length 4:	5.968	Diameter 4:	2.839
Avg. Length:	5.939	Avg. Diam.:	2.846

**PRESSURES (psi)**

Cell Pressure (psi)	52.4
Back Pressure (psi)	50.0
Eff. Conf. Pressure (psi)	2.4
Pore Pressure Response (%)	99

**VOLUME CHANGE**

Initial Burette Reading (ml)	24.0
Final Burette Reading (ml)	21.0
Final Change (ml)	3.0

**MAXIMUM OBLIQUITY POINTS**

$\bar{P}$	=	8.36
Q	=	5.09

Initial Dial Reading (mil)	217
Dial Reading After Saturation (mil)	230
Dial Reading After Consolidation (mil)	245

LOAD (LB)	DEFORMATION (IN)	PORE PRESSURE (PSI)
8.5	0.000	50.0
10.5	0.001	50.0
10.6	0.003	50.0
13.2	0.009	49.9
23.2	0.014	50.3
34.6	0.020	50.3
46.8	0.029	50.1
52.9	0.038	50.0
58.1	0.050	49.8
65.4	0.071	49.5
73.9	0.100	49.2
83.0	0.135	48.7
91.2	0.170	48.3
100.0	0.212	47.8
107.6	0.241	47.5
116.6	0.283	47.1
128.1	0.339	46.5
137.4	0.398	45.9
146.6	0.442	45.5
157.5	0.501	45.0
165.3	0.545	44.6
171.4	0.590	44.3
177.7	0.634	43.9
181.8	0.664	43.7
184.9	0.694	43.5
190.0	0.723	43.3
193.9	0.753	43.1
199.2	0.797	42.9
203.9	0.841	42.6
208.2	0.889	42.4

Failure Based on Maximum Effective Principal Stress Ratio

NOTE: GRAPH NOT TO SCALE

Tested By: 129-04-0411 Date: 5/16/18 Approved By: MPS Date: 5/23/18

page 2 of 11 DCN: CT-S28 DATE: 4/12/13 REVISION: 3

Tested By: 129-04-0411 Date: 5/16/18 Input Checked By: GEM Date: 5/23/18

page 3 of 11 DCN: CT-S28 DATE: 4/12/13 REVISION: 3

Sigmatrax.xls

**CONSOLIDATED UNDRAINED TRIAXIAL TEST  
WITH PORE PRESSURE READINGS  
AASHTO T-297**



**CONSOLIDATED UNDRAINED TRIAXIAL TEST  
WITH PORE PRESSURE READINGS  
AASHTO T-297**



Client: Kleinfelder Boring No.: RPA\_1449  
 Client Reference: I-5700 / 20151548.053A Depth (ft): 5.0-7.0  
 Project No.: R-2018-133-001 Sample No.: ST-6  
 Lab ID: R-2018-133-001-007

Client: Kleinfelder Boring No.: RPA\_1449  
 Client Reference: I-5700 / 20151548.053A Depth (ft): 5.0-7.0  
 Project No.: R-2018-133-001 Sample No.: ST-6  
 Lab ID: R-2018-133-001-007

Visual Description: BROWN SANDY CLAY (UNDISTURBED)

Visual Description: BROWN SANDY CLAY (UNDISTURBED)

Effective Confining Pressure (psi)	2.4	Stage No.	1
		Test No.	1

Stage No.	1
Test No.	2

**INITIAL SAMPLE DIMENSIONS (in)**

Length 1:	6.016	Diameter 1:	2.831
Length 2:	6.059	Diameter 2:	2.834
Length 3:	6.053	Diameter 3:	2.842
Length 4:	6.001	Diameter 4:	2.854
Avg. Length:	6.032	Avg. Diam.:	2.840

INITIAL DIMENSIONS		VOLUME CHANGE	
Initial Sample Length (in)	5.94	Volume After Consolidation (in <sup>3</sup> )	37.33
Initial Sample Diameter (in)	2.85	Length After Consolidation (in)	5.91
Initial Sample Area (in <sup>2</sup> )	6.36	Area After Consolidation (in <sup>2</sup> )	6.316
Initial Sample Volume (in <sup>3</sup> )	37.76		

**PRESSURES (psi)**

Cell Pressure (psi)	54.9
Back Pressure (psi)	50.0
Eff. Conf. Pressure (psi)	4.9
Pore Pressure	
Response (%)	100

**VOLUME CHANGE**

Initial Burette Reading (ml)	24.0
Final Burette Reading (ml)	18.5
Final Change (ml)	5.5

Strain (%)	Deviation Stress	$\Delta U$	$\bar{\sigma}_1$	$\bar{\sigma}_3$	Effective Principle Stress Ratio	$\bar{A}$	$\bar{P}$	$Q$
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**MAXIMUM OBLIQUITY POINTS**

$\bar{P}$	=	13.44
$Q$	=	8.07

Initial Dial Reading (mil)	250
Dial Reading After Saturation (mil)	276
Dial Reading After Consolidation (mil)	279

0.02	0.33	-0.03	2.79	2.5	1.134	-0.10	2.63	0.16
0.05	0.33	-0.05	2.81	2.5	1.134	-0.14	2.64	0.17
0.15	0.75	-0.08	3.26	2.5	1.299	-0.10	2.88	0.37
0.24	2.32	0.28	4.48	2.2	2.081	0.12	3.31	1.16
0.34	4.13	0.27	6.29	2.2	2.908	0.07	4.23	2.06
0.49	6.04	0.09	8.38	2.3	3.581	0.01	5.36	3.02
0.64	6.99	-0.04	9.46	2.5	3.827	-0.01	5.97	3.49
0.84	7.80	-0.20	10.42	2.6	3.969	-0.03	6.52	3.90
1.20	8.91	-0.47	11.81	2.9	4.076	-0.05	7.35	4.46
1.70	10.19	-0.84	13.45	3.3	4.117	-0.08	8.36	5.09
2.28	11.53	-1.27	15.23	3.7	4.114	-0.11	9.47	5.76
2.88	12.73	-1.69	16.85	4.1	4.090	-0.13	10.49	6.37
3.58	13.98	-2.16	18.57	4.6	4.045	-0.16	11.58	6.99
4.08	15.06	-2.50	19.98	4.9	4.057	-0.17	12.46	7.53
4.79	16.29	-2.95	21.67	5.4	4.029	-0.18	13.53	8.15
5.74	17.86	-3.53	23.82	6.0	3.996	-0.20	14.89	8.93
6.73	19.04	-4.09	25.55	6.5	3.922	-0.22	16.03	9.52
7.48	20.24	-4.51	27.17	6.9	3.918	-0.22	17.05	10.12
8.48	21.59	-5.02	29.04	7.5	3.898	-0.23	18.25	10.80
9.23	22.53	-5.39	30.36	7.8	3.881	-0.24	19.09	11.27
9.98	23.23	-5.73	31.39	8.2	3.845	-0.25	19.78	11.61
10.73	23.92	-6.07	32.42	8.5	3.813	-0.26	20.46	11.96
11.24	24.36	-6.29	33.08	8.7	3.794	-0.26	20.90	12.18
11.74	24.66	-6.49	33.58	8.9	3.765	-0.27	21.25	12.33
12.23	25.23	-6.69	34.35	9.1	3.767	-0.27	21.73	12.61
12.74	25.62	-6.87	34.92	9.3	3.754	-0.27	22.11	12.81
13.49	26.13	-7.14	35.70	9.6	3.731	-0.28	22.63	13.06
14.24	26.54	-7.38	36.35	9.8	3.706	-0.28	23.08	13.27
15.04	26.86	-7.60	36.89	10.0	3.679	-0.29	23.46	13.43

LOAD (LB)	DEFORMATION (IN)	PORE PRESSURE (PSI)
15.4	0.000	50.0
21.2	0.001	51.1
29.4	0.002	51.4
48.2	0.007	51.6
57.6	0.013	51.6
64.8	0.019	51.4
72.9	0.027	51.4
79.9	0.036	51.2
88.5	0.048	51.0
101.4	0.068	50.4
117.6	0.096	49.6
135.3	0.133	48.6
151.0	0.169	47.4
169.0	0.209	46.9
181.5	0.238	47.7
198.9	0.279	47.2
221.0	0.337	46.2
240.5	0.395	43.1
255.7	0.440	42.7
275.7	0.499	41.8
289.1	0.544	41.1
301.9	0.589	40.5
316.1	0.633	40.1
325.6	0.662	39.7
333.3	0.691	39.4
343.3	0.722	39.0
352.2	0.752	38.6
361.9	0.796	38.1
373.1	0.840	37.6
380.5	0.869	37.3
386.7	0.899	36.9

**CONSOLIDATED UNDRAINED TRIAXIAL TEST  
WITH PORE PRESSURE READINGS  
AASHTO T-297**



Client: Kleinfelder Boring No.: RPA\_1449  
 Client Reference: I-5700 / 20151548.053A Depth (ft): 5.0-7.0  
 Project No.: R-2018-133-001 Sample No.: ST-6  
 Lab ID: R-2018-133-001-007

Visual Description: BROWN SANDY CLAY (UNDISTURBED)

Effective Confining Pressure (psi)	4.9	Stage No.	1
		Test No	2

**INITIAL DIMENSIONS**

Initial Sample Length (in)	6.03
Initial Sample Diameter (in)	2.84
Initial Sample Area (in <sup>2</sup> )	6.34
Initial Sample Volume (in <sup>3</sup> )	38.22

**VOLUME CHANGE**

Volume After Consolidation (in <sup>3</sup> )	37.39
Length After Consolidation (in)	6.00
Area After Consolidation (in <sup>2</sup> )	6.228

Strain (%)	Deviation Stress	$\Delta U$	$\bar{\sigma}_1$	$\bar{\sigma}_3$	Effective Principle Stress Ratio	$\bar{A}$	$\bar{P}$	Q
0.02	0.93	1.06	4.81	3.9	1.240	1.13	4.35	0.47
0.03	2.25	1.37	5.82	3.6	1.631	0.61	4.70	1.13
0.12	5.27	1.64	8.57	3.3	2.596	0.31	5.93	2.63
0.22	6.77	1.56	10.15	3.4	3.006	0.23	6.76	3.39
0.31	7.91	1.45	11.40	3.5	3.269	0.18	7.45	3.96
0.46	9.20	1.36	12.78	3.6	3.575	0.15	8.18	4.60
0.60	10.30	1.17	14.07	3.8	3.732	0.11	8.92	5.15
0.79	11.65	0.96	15.63	4.0	3.927	0.08	9.80	5.82
1.12	13.65	0.39	18.20	4.5	4.001	0.03	11.38	6.83
1.61	16.15	-0.43	21.51	5.4	4.008	-0.03	13.44	8.07
2.22	18.83	-1.36	25.12	6.3	3.991	-0.07	15.71	9.41
2.81	21.17	-2.60	28.70	7.5	3.810	-0.12	18.12	10.58
3.49	23.81	-3.09	31.84	8.0	3.967	-0.13	19.93	11.91
3.97	25.62	-2.25	32.80	7.2	4.565	-0.09	20.00	12.81
4.65	28.10	-2.75	35.79	7.7	4.656	-0.10	21.74	14.05
5.61	31.16	-3.76	39.86	8.7	4.582	-0.12	24.28	15.58
6.59	33.76	-6.86	45.56	11.8	3.862	-0.20	28.68	16.88
7.33	35.77	-7.29	47.99	12.2	3.927	-0.20	30.11	17.88
8.31	38.32	-8.18	51.44	13.1	3.921	-0.21	32.28	19.16
9.06	39.98	-8.86	53.77	13.8	3.898	-0.22	33.78	19.99
9.81	41.49	-9.46	55.89	14.4	3.882	-0.23	35.14	20.74
10.54	43.20	-9.89	58.02	14.8	3.914	-0.23	36.42	21.60
11.02	44.32	-10.29	59.54	15.2	3.912	-0.23	37.38	22.16
11.51	45.18	-10.65	60.76	15.6	3.899	-0.24	38.17	22.59
12.02	46.32	-11.03	62.28	16.0	3.902	-0.24	39.12	23.16
12.53	47.31	-11.38	63.63	16.3	3.899	-0.24	39.97	23.66
13.26	48.26	-11.91	65.11	16.8	3.865	-0.25	40.98	24.13
13.98	49.41	-12.41	66.75	17.3	3.849	-0.25	42.04	24.70
14.48	50.14	-12.75	67.83	17.7	3.836	-0.25	42.75	25.07
14.98	50.69	-13.07	68.69	18.0	3.816	-0.26	43.34	25.34

**CONSOLIDATED UNDRAINED TRIAXIAL TEST  
WITH PORE PRESSURE READINGS  
AASHTO T-297**



Client: Kleinfelder Boring No.: RPA\_1449  
 Client Reference: I-5700 / 20151548.053A Depth (ft): 5.0-7.0  
 Project No.: R-2018-133-001 Sample No.: ST-6  
 Lab ID: R-2018-133-001-007

Visual Description: BROWN SANDY CLAY (UNDISTURBED)

Stage No.	1
Test No.	3

**INITIAL SAMPLE DIMENSIONS (in)**

Length 1:	5.833	Diameter 1:	2.779
Length 2:	5.870	Diameter 2:	2.809
Length 3:	5.875	Diameter 3:	2.804
Length 4:	5.899	Diameter 4:	2.841
Avg. Length:	5.869	Avg. Diam.:	2.808

**PRESSURES (psi)**

Cell Pressure (psi)	59.7
Back Pressure (psi)	50.0
Eff. Conf. Pressure (psi)	9.7
Pore Pressure Response (%)	97

**VOLUME CHANGE**

Initial Burette Reading (ml)	24.0
Final Burette Reading (ml)	14.0
Final Change (ml)	10.0

**MAXIMUM OBLIQUITY POINTS**

$\bar{P}$	=	16.51
Q	=	9.84

Initial Dial Reading (mil)	101
Dial Reading After Saturation (mil)	107
Dial Reading After Consolidation (mil)	110

LOAD (LB)	DEFORMATION (IN)	PORE PRESSURE (PSI)
7.9	0.000	50.0
10.2	0.001	50.1
14.1	0.003	50.1
14.4	0.009	50.1
14.9	0.014	50.1
15.1	0.021	50.1
15.8	0.029	50.1
51.8	0.038	51.7
72.8	0.049	53.0
85.8	0.069	53.8
97.5	0.099	54.1
107.5	0.133	54.0
115.9	0.168	53.8
126.1	0.208	53.3
132.7	0.238	53.1
140.2	0.278	52.6
152.0	0.333	52.1
164.0	0.392	51.6
170.9	0.435	51.2
184.2	0.493	50.7
188.9	0.537	50.3
192.6	0.581	50.0
201.0	0.625	49.7
205.6	0.654	49.5
209.7	0.682	49.4
213.1	0.711	49.2
217.6	0.740	49.0
220.1	0.784	48.8
225.1	0.827	48.6
229.9	0.857	48.4
233.5	0.887	48.3

Tested By: 129-04-0411 Date: 5/16/18 Input Checked By: GEM Date: 5/23/18

**CONSOLIDATED UNDRAINED TRIAXIAL TEST  
WITH PORE PRESSURE READINGS  
AASHTO T-297**



Client: Kleinfelder Boring No.: RPA\_1449  
 Client Reference: I-5700 / 20151548.053A Depth (ft): 5.0-7.0  
 Project No.: R-2018-133-001 Sample No.: ST-6  
 Lab ID: R-2018-133-001-007

Visual Description: BROWN SANDY CLAY (UNDISTURBED)

Effective Confining Pressure (psi)	9.7	Stage No.	1
		Test No	3

**INITIAL DIMENSIONS**

Initial Sample Length (in)	5.87
Initial Sample Diameter (in)	2.81
Initial Sample Area (in <sup>2</sup> )	6.19
Initial Sample Volume (in <sup>3</sup> )	36.35

**VOLUME CHANGE**

Volume After Consolidation (in <sup>3</sup> )	35.63
Length After Consolidation (in)	5.86
Area After Consolidation (in <sup>2</sup> )	6.080

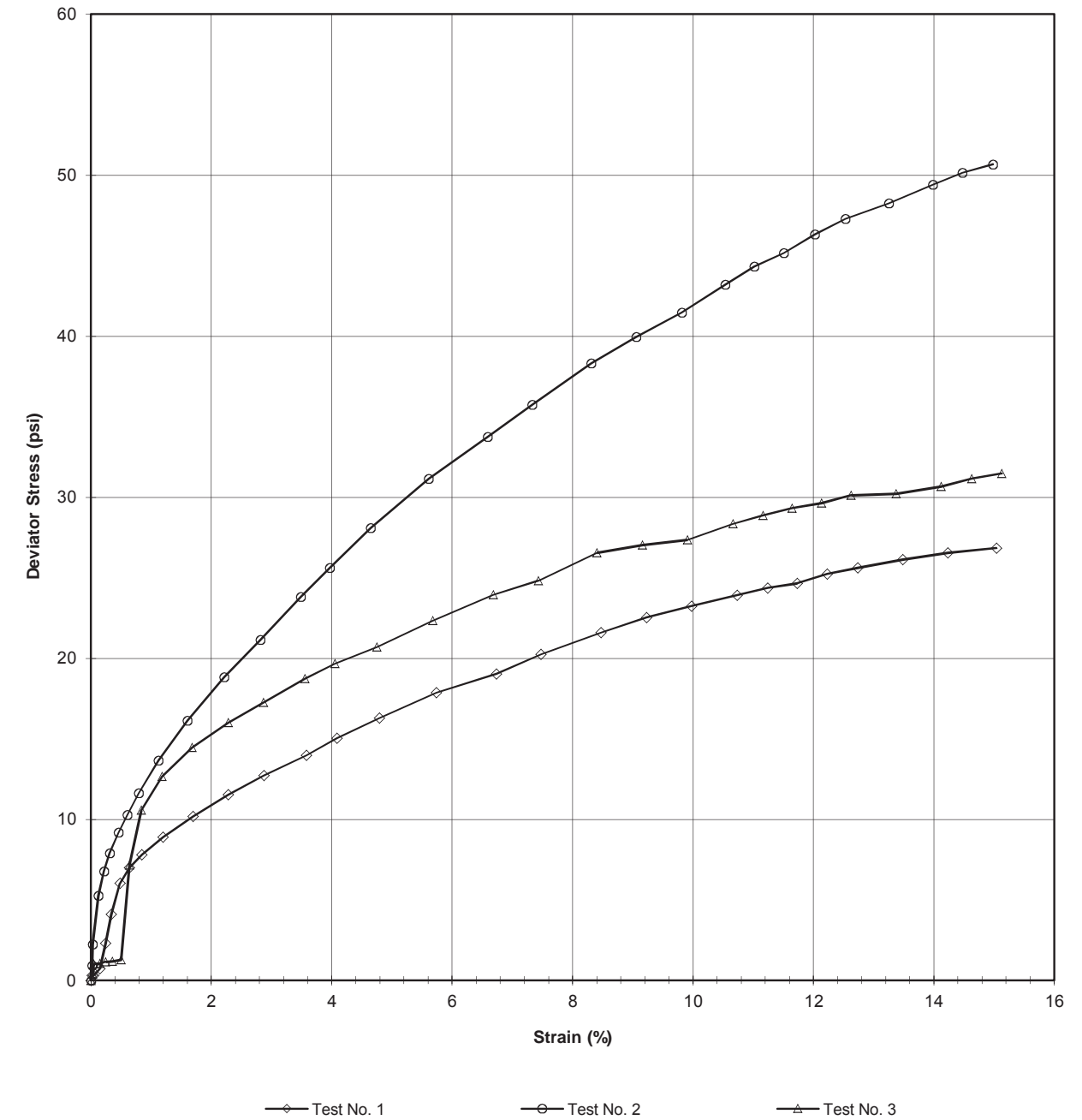
Strain (%)	Deviation Stress	Δ U	$\bar{\sigma}_1$	$\bar{\sigma}_3$	Effective Principle Stress Ratio	A	P	Q
0.02	0.38	0.04	10.04	9.7	1.039	0.12	9.85	0.19
0.05	1.01	0.04	10.68	9.7	1.105	0.04	10.17	0.51
0.15	1.06	0.08	10.69	9.6	1.110	0.07	10.16	0.53
0.25	1.15	0.08	10.77	9.6	1.119	0.07	10.20	0.57
0.35	1.18	0.09	10.79	9.6	1.122	0.08	10.20	0.59
0.50	1.29	0.11	10.88	9.6	1.134	0.09	10.24	0.64
0.64	7.17	1.63	15.25	8.1	1.887	0.23	11.66	3.58
0.84	10.58	2.96	17.32	6.7	2.569	0.29	12.03	5.29
1.18	12.66	3.81	18.56	5.9	3.148	0.31	12.23	6.33
1.68	14.48	4.08	20.10	5.6	3.575	0.29	12.86	7.24
2.28	16.00	3.99	21.71	5.7	3.799	0.26	13.71	8.00
2.87	17.25	3.73	23.23	6.0	3.888	0.22	14.60	8.63
3.56	18.74	3.31	25.13	6.4	3.931	0.18	15.76	9.37
4.05	19.68	3.04	26.35	6.7	3.952	0.16	16.51	9.84
4.75	20.71	2.62	27.80	7.1	3.922	0.13	17.45	10.36
5.68	22.36	2.06	30.00	7.6	3.925	0.10	18.82	11.18
6.69	23.96	1.53	32.13	8.2	3.932	0.07	20.15	11.98
7.43	24.81	1.18	33.34	8.5	3.908	0.05	20.93	12.40
8.41	26.56	0.66	35.60	9.0	3.937	0.03	22.32	13.28
9.16	27.04	0.26	36.48	9.4	3.863	0.01	22.96	13.52
9.91	27.37	0.00	37.08	9.7	3.819	0.00	23.39	13.68
10.66	28.37	-0.28	38.36	10.0	3.840	-0.01	24.18	14.19
11.16	28.89	-0.50	39.09	10.2	3.831	-0.02	24.65	14.44
11.65	29.32	-0.67	39.70	10.4	3.825	-0.02	25.04	14.66
12.14	29.64	-0.84	40.19	10.5	3.812	-0.03	25.36	14.82
12.62	30.12	-1.02	40.85	10.7	3.809	-0.03	25.79	15.06
13.37	30.23	-1.26	41.19	11.0	3.758	-0.04	26.08	15.12
14.12	30.68	-1.47	41.85	11.2	3.746	-0.05	26.51	15.34
14.63	31.16	-1.64	42.51	11.3	3.746	-0.05	26.93	15.58
15.13	31.49	-1.76	42.95	11.5	3.746	-0.06	27.21	15.74

**CONSOLIDATED UNDRAINED TRIAXIAL TEST  
WITH PORE PRESSURE READINGS  
AASHTO T-297**



Client: Kleinfelder Boring No.: RPA\_1449  
 Client Reference: I-5700 / 20151548.053A Depth (ft): 5.0-7.0  
 Project No.: R-2018-133-001 Sample No.: ST-6  
 Lab ID: R-2018-133-001-007

Visual Description: BROWN SANDY CLAY (UNDISTURBED)



Tested By: 129-04-0411 Date: 5/16/18 Approved By: MPS Date: 5/23/18





**CONSOLIDATED UNDRAINED TRIAXIAL TEST  
WITH PORE PRESSURE READINGS  
AASHTO T-297**



**CONSOLIDATED UNDRAINED TRIAXIAL TEST  
WITH PORE PRESSURE READINGS  
AASHTO T-297**

Client: Kleinfelder  
 Client Reference: I-5700 / 20151548.053A  
 Project No.: R-2018-133-001  
 Lab ID: R-2018-133-001-007

Boring No.: RPA\_1449  
 Depth (ft): 5.0-7.0  
 Sample No.: ST-6

Client: Kleinfelder  
 Client Reference: I-5700 / 20151548.053A  
 Project No.: R-2018-133-001  
 Lab ID: R-2018-133-001-007      Specific Gravity (Measured)      2.63

Visual Description: BROWN SANDY CLAY (UNDISTURBED)

**SAMPLE CONDITION SUMMARY**

	RPA_1449	RPA_144	RPA_1449
Boring No.:	RPA_1449	RPA_144	RPA_1449
Depth (ft):	5.0-7.0	5.0-7.0	5.0-7.0
Sample No.:	ST-6	ST-6	ST-6
Test No.	T1	T2	T3
Deformation Rate (in/min)	0.0018	0.0018	0.0018
Back Pressure (psi)	50.0	50.0	50.0
Consolidation Time (days)	1	1	1
Moisture Content (%) (INITIAL)	18.8	18.9	20.4
Total Unit Weight (pcf)	131.4	133.1	135.4
Dry Unit Weight (pcf)	110.6	112.0	112.4
Moisture Content (%) (FINAL)	20.0	19.6	18.8
Initial State Void Ratio, e	0.484	0.466	0.460
Void Ratio at Shear, e	0.468	0.435	0.432

**TEST 1 INITIAL**



**TEST 1 FINAL**



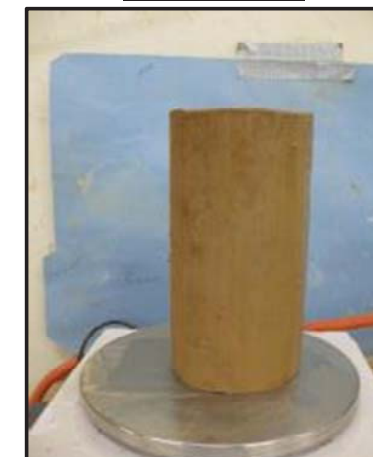
**TEST 2 INITIAL**



**TEST 2 FINAL**



**TEST 3 INITIAL**



**TEST 3 FINAL**



Tested By: 129-04-0411      Date: 5/16/18      Input Checked By: GEM      Date: 5/23/18

page 10 of 11      DCN: CT-S28    DATE: 4/12/13    REVISION: 3

Tested By 129-04-0411      Date 5/16/18      Approved By MPS      Date 5/23/18

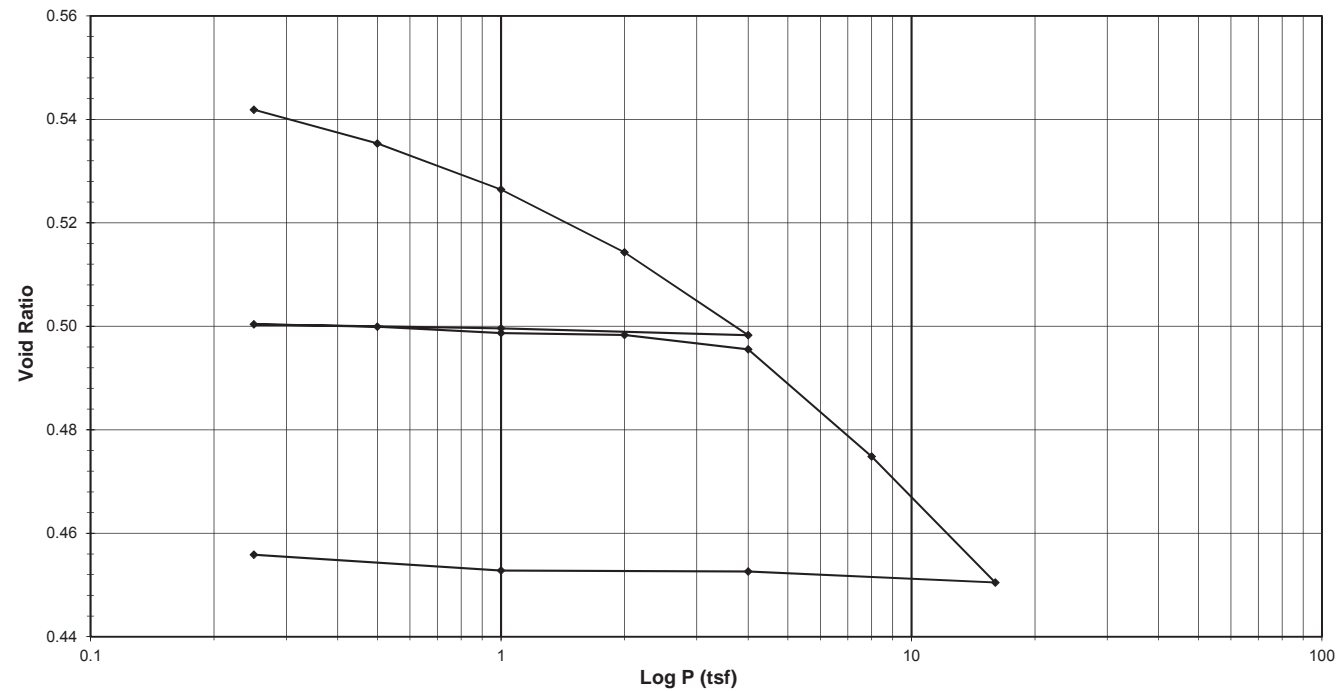
page 11 of 11      DCN: CT-S28    DATE: 4/12/13    REVISION: 3      Z:\2018 PROJECTS\KLEINFELDER\2018-133 KLEINFELDER - I-5700 20151548.053A\2018-133-001-007 DOT SIGMA TRIAXIAL\THIRD



**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216

Client Kleinfelder Boring No. RPA\_1449  
 Client Reference I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Tested By 129-04-0411 Date 5/17/18 Approved By MPS Date 5/23/18



**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216

Client Kleinfelder Boring No. RPA\_1449  
 Client Reference I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED

Consolidometer No. R470  
 1 Division = 0.0001 (in.)

Sample Properties	Initial	Final
Water Content		
Tare Number	911	807
Wt. Tare & WS (g)	408.90	258.74
Wt. Tare & DS (g)	360.27	235.58
Wt. Water (g)	48.63	23.16
Wt. Tare (g)	101.89	101.13
Wt. DS (g)	258.38	134.45
Water Content (%)	18.82	17.23
Sample Parameters		
Sample Diameter (in)	2.5	2.5
Sample Height (in)	1.0000	0.9340
Sample Volume (cc)	80.44	75.13
Wt. Wet Sample + Ring (g)	375.47	373.30
Wt. of Ring (g)	214.20	214.20
Wt. of Wet Sample (g)	161.27	159.10
Wet Density (pcf)	125.10	132.14
Wet Density (g/cc)	2.00	2.12
Water Content (%)	18.82	17.23
Wt. of Dry Sample (g)	135.73	135.73
Dry Density (pcf)	105.29	112.73
Dry Density (g/cc)	1.69	1.81
Void Ratio	0.5587	0.4558
Saturation (%)	88.60	99.38
Specific Gravity	2.63	Measured

Test Data Summary							
Applied Pressure (tsf)	Final Dial Reading (div)	Machine Deflection (div)	Corrected Reading (div)	Height of Sample (mm)	Volume (cc)	Dry Density (g/cc)	Void Ratio
Seating	0	0	0	25.400	80.440	1.68729	0.55872
0.25	121.2	13.1	108.1	25.125	79.570	1.70573	0.54186
0.5	177.5	27.4	150.1	25.019	79.232	1.71301	0.53531
1	258.1	51.0	207.1	24.874	78.774	1.72296	0.52644
2	372.2	87.1	285.0	24.676	78.147	1.73679	0.51429
4	512.4	124.7	387.7	24.415	77.321	1.75534	0.49828
1	450.9	71.5	379.3	24.436	77.388	1.75382	0.49959
0.25	410.6	36.3	374.3	24.449	77.429	1.75289	0.50038
0.5	418.1	41.0	377.1	24.442	77.407	1.75340	0.49994
1	435.1	50.0	385.1	24.422	77.342	1.75487	0.49868
2	472.4	85.0	387.4	24.416	77.323	1.75530	0.49832
4	531.1	125.9	405.2	24.371	77.180	1.75854	0.49556
8	701.3	163.1	538.2	24.033	76.110	1.78327	0.47482
16	910.2	215.8	694.4	23.636	74.854	1.81319	0.45048
4	829.9	149.1	680.8	23.671	74.964	1.81054	0.45260
1	770.6	91.1	679.5	23.674	74.974	1.81030	0.45280
0.25	708.4	48.4	660.0	23.724	75.131	1.80651	0.45585

Tested By 129-04-0411 Date 5/17/18 Input Checked By GEM Date 5/23/18



**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216

Client Kleinfelder Boring No. RPA\_1449  
 Client Reference I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

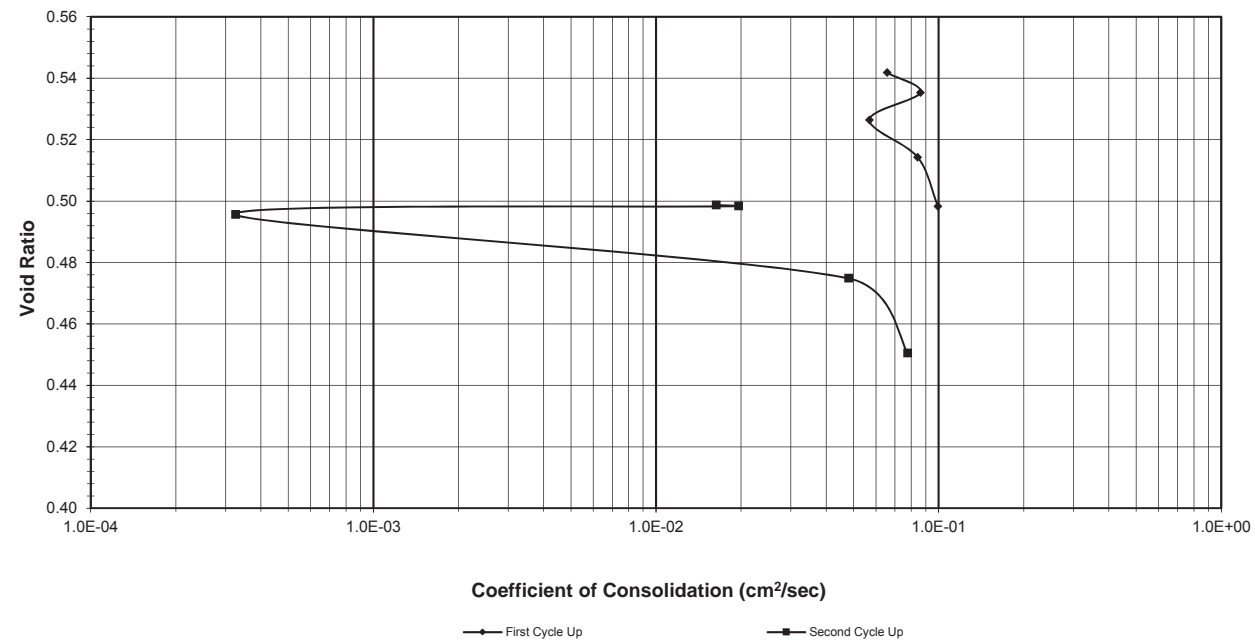
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED

**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216

Client Kleinfelder Boring No. RPA\_1449  
 Client Reference I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED

Consolidometer No. R470  
 1 Division = 0.0001 (in.)



Sample Properties	Initial	Final	C <sub>v</sub> Test Data Summary					Time t <sub>50</sub> (min.)	C <sub>v</sub> (cm <sup>2</sup> /sec)
			Load Increment (tsf)	Dial Reading @ t <sub>50</sub> (div)	Machine Deflection (div)	Corrected Dial Reading @ t <sub>50</sub> (div)	Sample Height @ t <sub>50</sub> (cm)		
Water Content									
Tare Number	911	807							
Wt. Tare & WS (g)	408.90	258.74							
Wt. Tare & DS (g)	360.27	235.58							
Wt. Water (g)	48.63	23.16	0 - 0.25	54.0	13.1	40.9	2.530	0.08	0.06566
Wt. Tare (g)	101.89	101.13	0.25 - 0.5	147.4	27.4	120.0	2.510	0.06	0.08616
Wt. DS (g)	258.38	134.45	0.5 - 1.0	219.6	51.0	168.6	2.497	0.09	0.05687
Water Content (%)	18.82	17.23	1.0 - 2.0	318.2	87.1	231.1	2.481	0.06	0.08423
			2.0 - 4.0	435.2	124.7	310.5	2.461	0.05	0.09944
			4.0 - 1.0	NA	71.5	NA	NA	NA	NA
Sample Parameters			1.0 - 0.25	NA	36.3	NA	NA	NA	NA
Sample Diameter (in)	2.5	2.5	0.25 - 0.5	NA	41.0	NA	NA	NA	NA
Sample Height (in)	1.000	0.934	0.5 - 1.0	431.1	50.0	381.1	2.443	0.30	0.01633
Sample Volume (cc)	80.44	75.13	1.0 - 2.0	466.6	85.0	381.6	2.443	0.25	0.01960
Wt. Wet Sample + Ring (g)	375.47	373.30	2.0 - 4.0	520.6	125.9	394.7	2.440	15.00	0.00033
Wt. of Ring (g)	214.20	214.20	4.0 - 8.0	642.4	163.1	479.3	2.418	0.10	0.04800
Wt. of Wet Sample (g)	161.27	159.10	8.0 - 16.0	835.4	215.8	619.6	2.383	0.06	0.07766
Wet Density (pcf)	125.10	132.14	16.0 - 4.0	NA	149.1	NA	NA	NA	NA
Wet Density (g/cc)	2.00	2.12	4.0 - 1.0	NA	91.1	NA	NA	NA	NA
Water Content (%)	18.82	17.23	1.0 - 0.25	NA	48.4	NA	NA	NA	NA
Wt. of Dry Sample (g)	135.73	135.73							
Dry Density (pcf)	105.29	112.73							
Dry Density (g/cc)	1.69	1.81							
Void Ratio	0.5587	0.4558							
Saturation (%)	88.60	99.38							
Specific Gravity	2.63	Measured							

Tested By 129-04-0411 Date 5/17/18 Input Checked By GEM Date 5/23/18

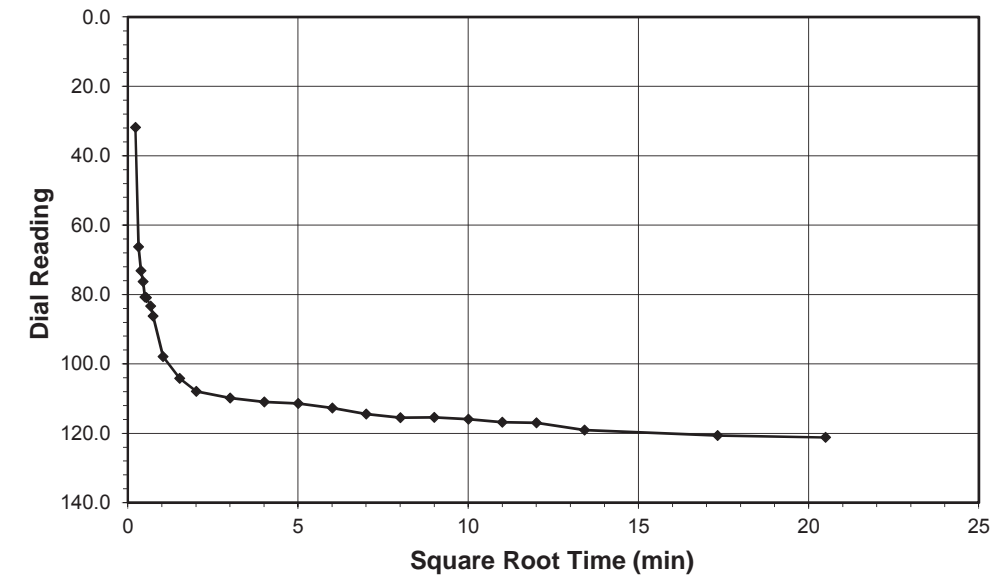


**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



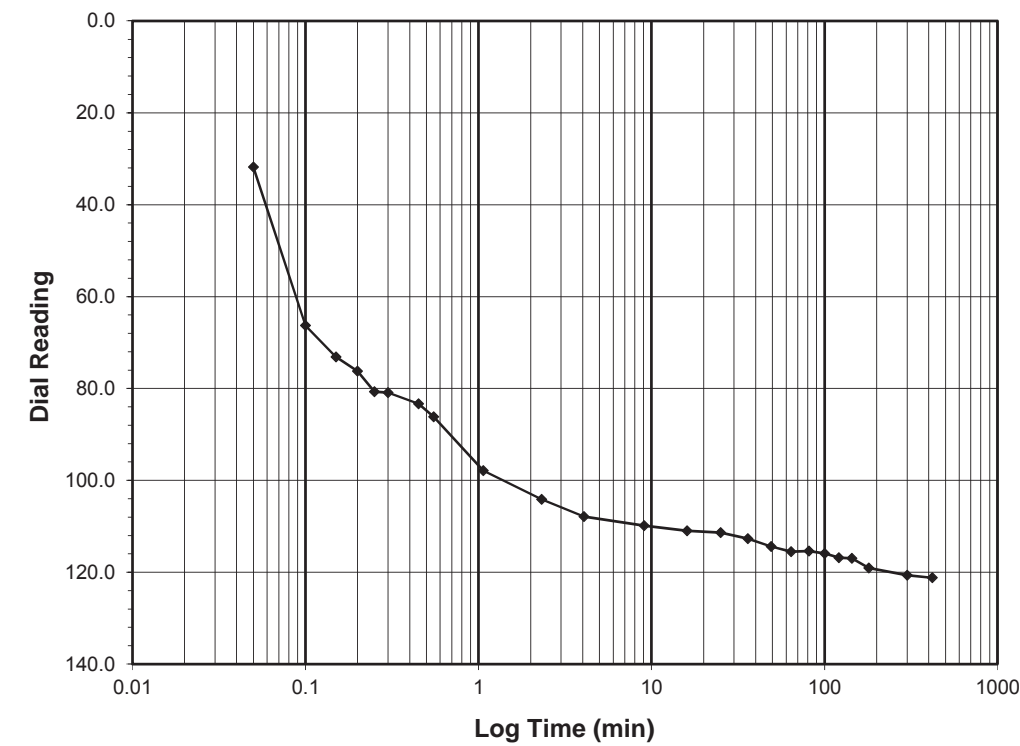
Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.0-0.25  
 Final Reading (div) 121.2  
 Consolidometer No. R470  
 1 Division (in) 0.0001  
 Start Date 5/17/18  
 Start Time 9:28:58

Elapsed Time (min)	Dial Reading (div)
Initial	0.0
0.05	31.8
0.10	66.3
0.15	73.1
0.20	76.2
0.25	80.7
0.30	80.9
0.45	83.3
0.55	86.2
1.07	97.9
2.32	104.1
4.07	107.9
9.07	109.8
16.07	111.0
25.07	111.4
36.07	112.7
49.07	114.4
64.07	115.5
81.07	115.4
100.07	115.9
121.07	116.8
144.07	116.9
180.07	119.1
300.07	120.6
420.28	121.2

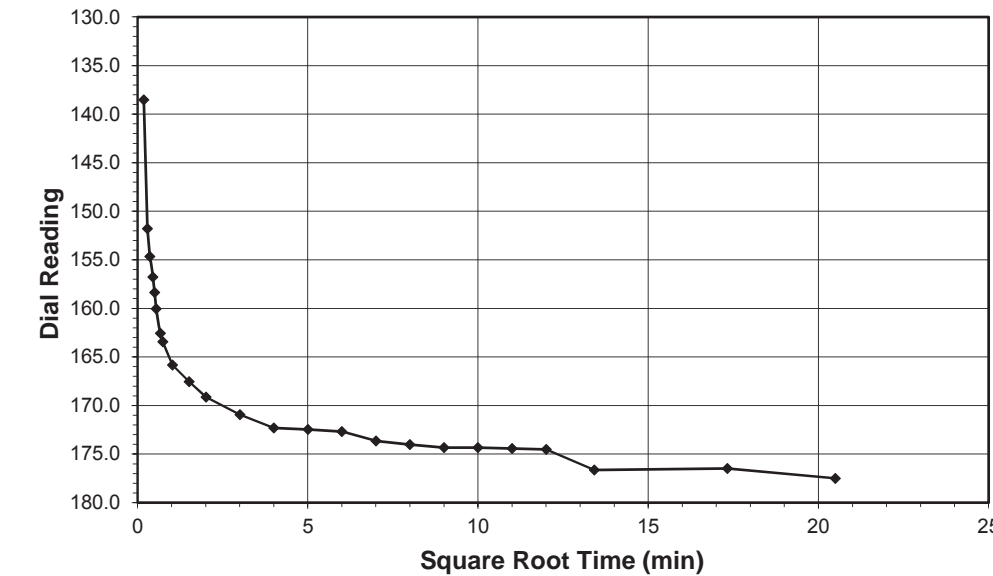


**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



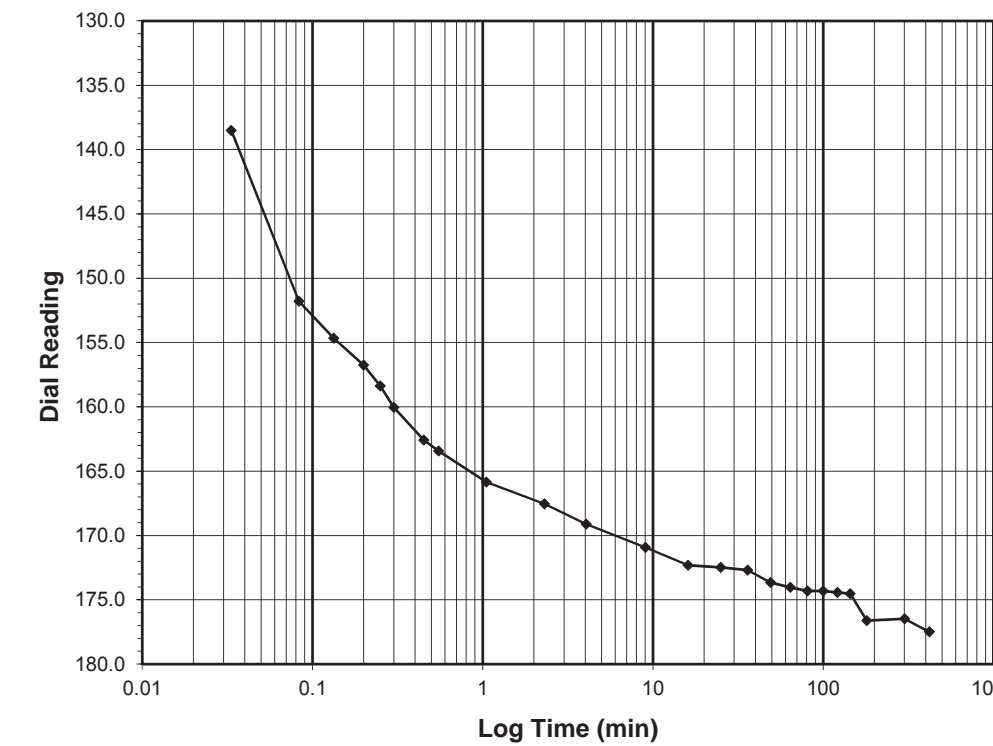
Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.25-0.5  
 Final Reading (div) 177.5  
 Consolidometer No. R470  
 1 Division (in) 0.0001  
 Start Date 5/17/18  
 Start Time 16:29:15

Elapsed Time (min)	Dial Reading (div)
Initial	121.2
0.03	138.5
0.08	151.8
0.13	154.7
0.20	156.8
0.25	158.4
0.30	160.0
0.45	162.6
0.55	163.4
1.05	165.8
2.30	167.6
4.05	169.1
9.05	170.9
16.05	172.3
25.05	172.5
36.05	172.7
49.05	173.7
64.05	174.0
81.05	174.3
100.05	174.3
121.07	174.4
144.07	174.5
180.07	176.6
300.07	176.5
420.17	177.5



Tested By 129-04-0411 Date 5/17/18 Checked By GEM Date 5/23/18

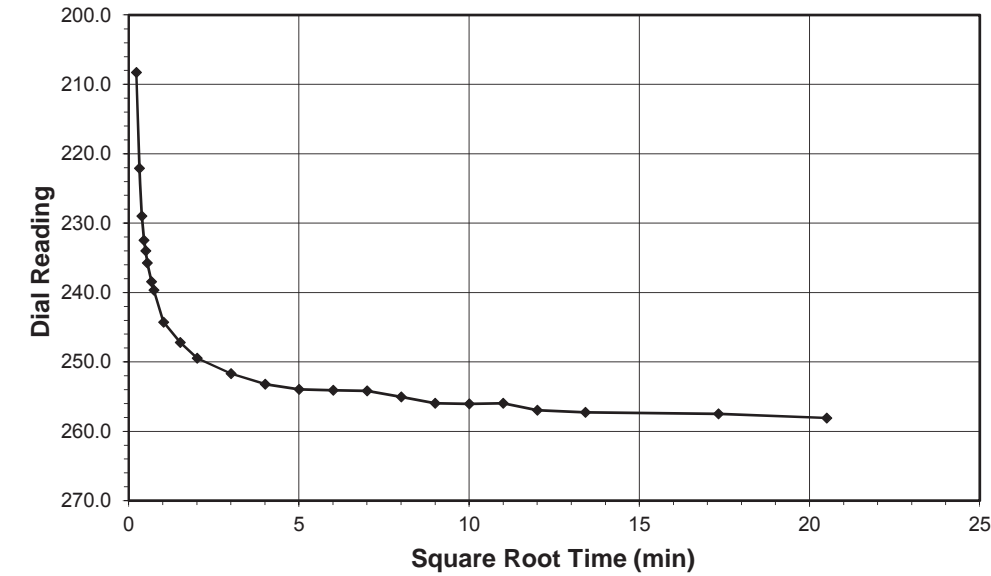
Tested By 129-04-0411 Date 5/17/18 Checked By GEM Date 5/23/18

**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

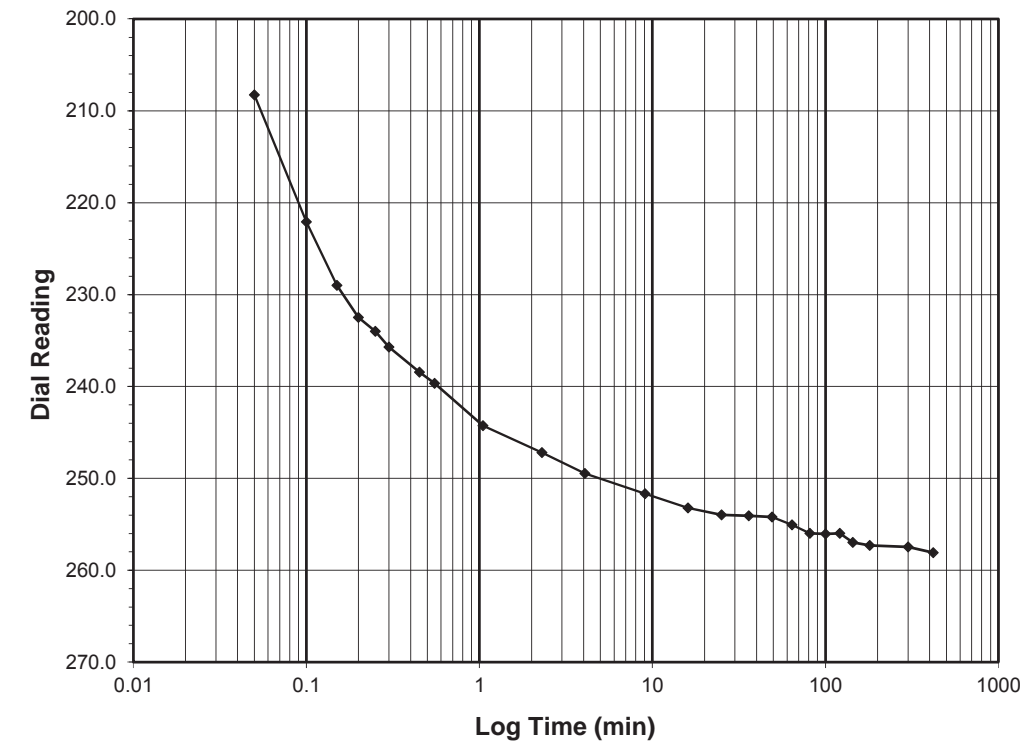
**Sample Conditions:** UNDISTURBED, INUNDATED AND DOUBLE DRAINED



**Test Load (tsf) 0.5-1.0**  
**Final Reading (div) 258.1**  
 Consolidometer No. **R470**  
 1 Division (in) 0.0001

Start Date 5/17/18  
 Start Time 23:29:26

Elapsed Time (min)	Dial Reading (div)
<b>Initial</b>	<b>177.5</b>
0.05	208.3
0.10	222.1
0.15	229.0
0.20	232.5
0.25	234.0
0.30	235.7
0.45	238.5
0.55	239.7
1.05	244.3
2.30	247.2
4.07	249.5
9.07	251.7
16.07	253.2
25.07	254.0
36.07	254.1
49.07	254.2
64.07	255.1
81.07	256.0
100.07	256.0
121.07	256.0
144.07	257.0
180.07	257.3
300.07	257.5
420.37	258.1

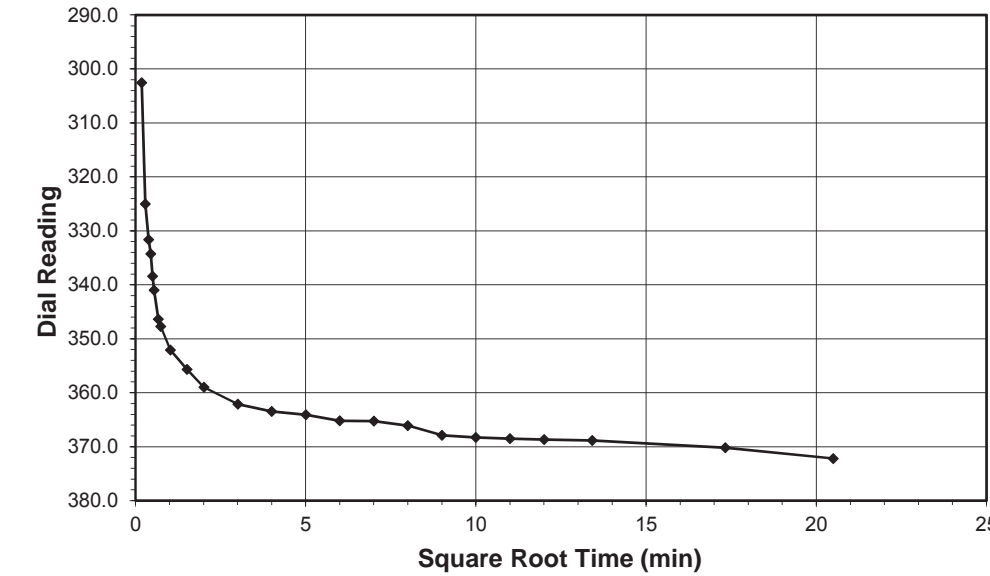


**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

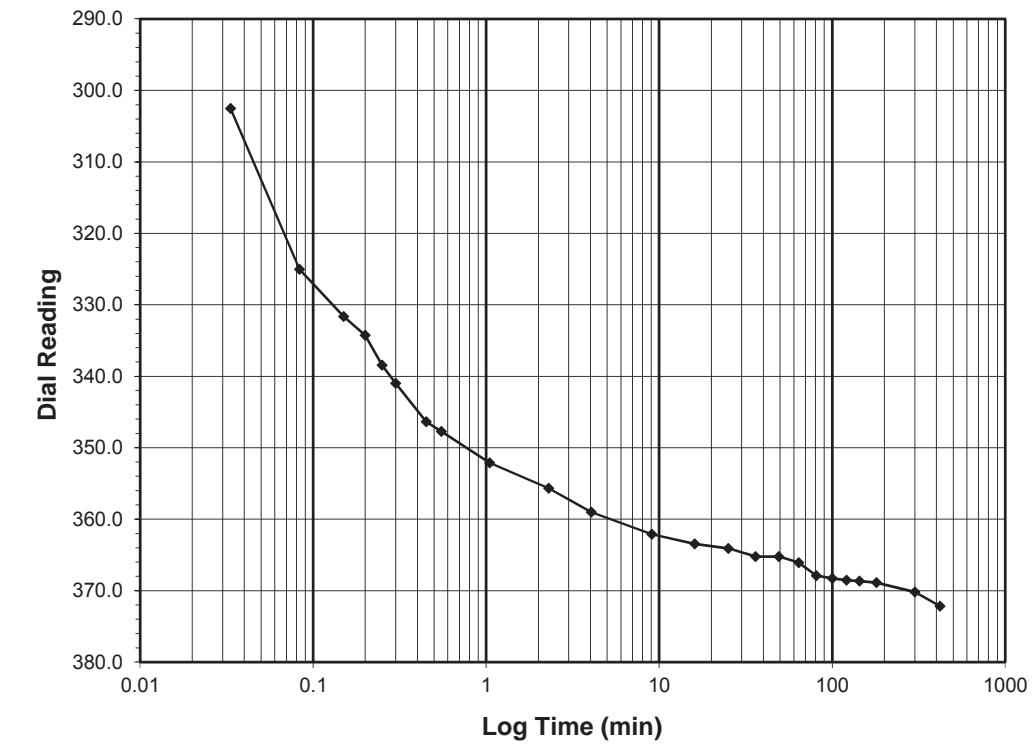
**Sample Conditions:** UNDISTURBED, INUNDATED AND DOUBLE DRAINED



**Test Load (tsf) 1.0-2.0**  
**Final Reading (div) 372.2**  
 Consolidometer No. **R470**  
 1 Division (in) 0.0001

Start Date 5/18/18  
 Start Time 6:29:48

Elapsed Time (min)	Dial Reading (div)
<b>Initial</b>	<b>258.1</b>
0.03	302.5
0.08	325.0
0.15	331.6
0.20	334.3
0.25	338.4
0.30	341.0
0.45	346.4
0.55	347.7
1.05	352.1
2.30	355.7
4.05	359.0
9.05	362.1
16.05	363.4
25.05	364.1
36.05	365.2
49.05	365.2
64.05	366.1
81.05	367.9
100.05	368.3
121.05	368.5
144.05	368.7
180.05	368.9
300.05	370.2
420.27	372.2



Tested By 129-04-0411 Date 5/17/18 Checked By GEM Date 5/23/18

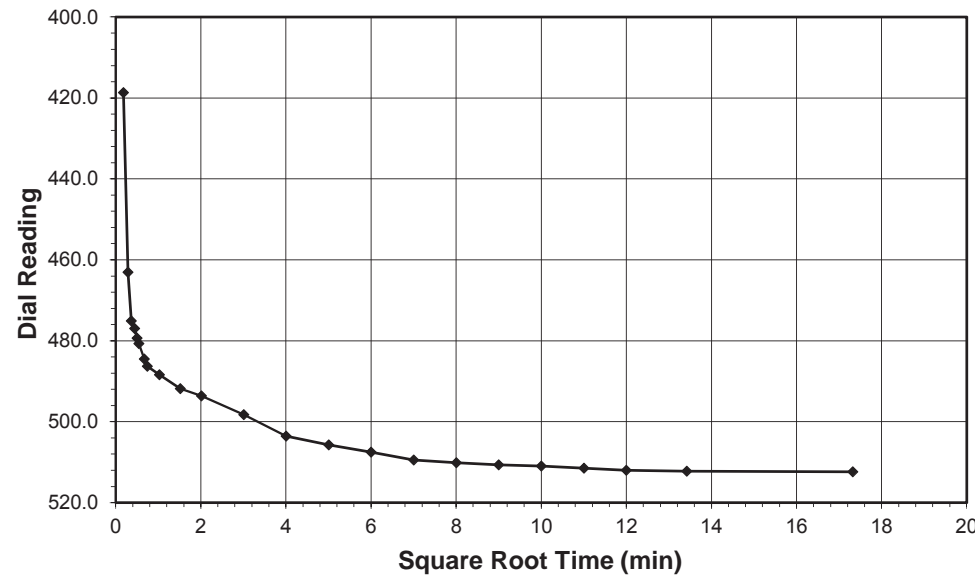
Tested By 129-04-0411 Date 5/18/18 Checked By GEM Date 5/23/18

**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



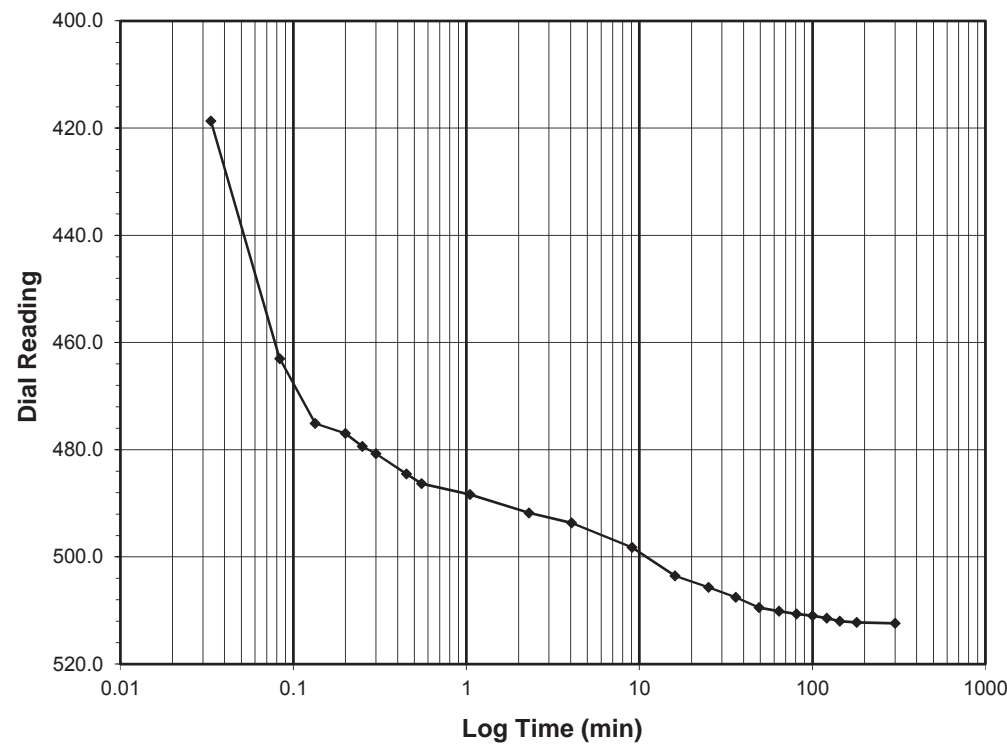
Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 2.0-4.0  
 Final Reading (div) 512.4  
 Consolidometer No. R470  
 1 Division (in) 0.0001  
 Start Date 5/18/18  
 Start Time 13:30:05

Elapsed Time (min)	Dial Reading (div)
Initial	372.2
0.03	418.7
0.08	463.0
0.13	475.1
0.20	477.0
0.25	479.4
0.30	480.7
0.45	484.5
0.55	486.3
1.05	488.4
2.30	491.8
4.05	493.6
9.05	498.3
16.05	503.5
25.05	505.7
36.05	507.5
49.05	509.5
64.05	510.1
81.05	510.6
100.05	511.0
121.05	511.4
144.07	512.0
180.07	512.2
300.07	512.4

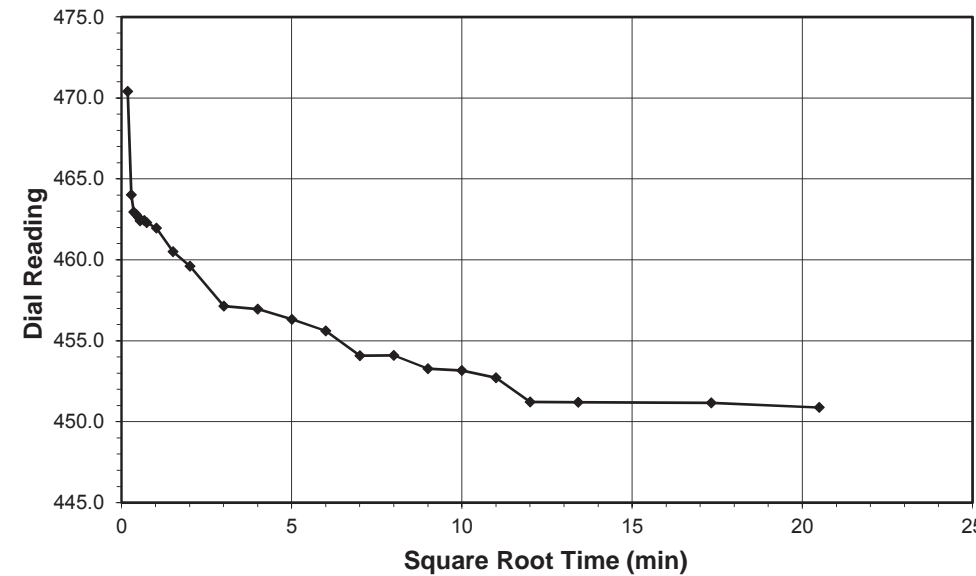


**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



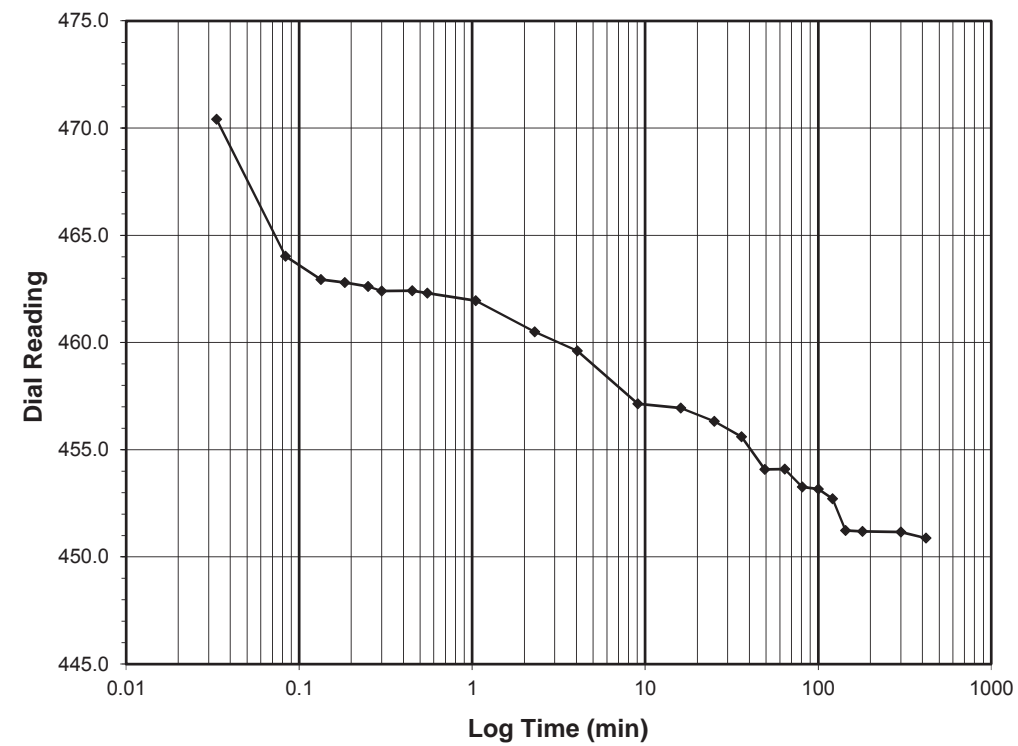
Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 4.0-1.0  
 Final Reading (div) 450.9  
 Consolidometer No. R470  
 1 Division (in) 0.0001  
 Start Date 5/18/18  
 Start Time 20:30:29

Elapsed Time (min)	Dial Reading (div)
Initial	512.4
0.03	470.4
0.08	464.0
0.13	462.9
0.18	462.8
0.25	462.6
0.30	462.4
0.45	462.4
0.55	462.3
1.05	462.0
2.30	460.5
4.05	459.6
9.05	457.1
16.05	456.9
25.05	456.3
36.05	455.6
49.05	454.1
64.05	454.1
81.05	453.3
100.05	453.2
121.05	452.7
144.05	451.2
180.05	451.2
300.07	451.2
420.25	450.9



Tested By 129-04-0411 Date 5/18/18 Checked By GEM Date 5/23/18

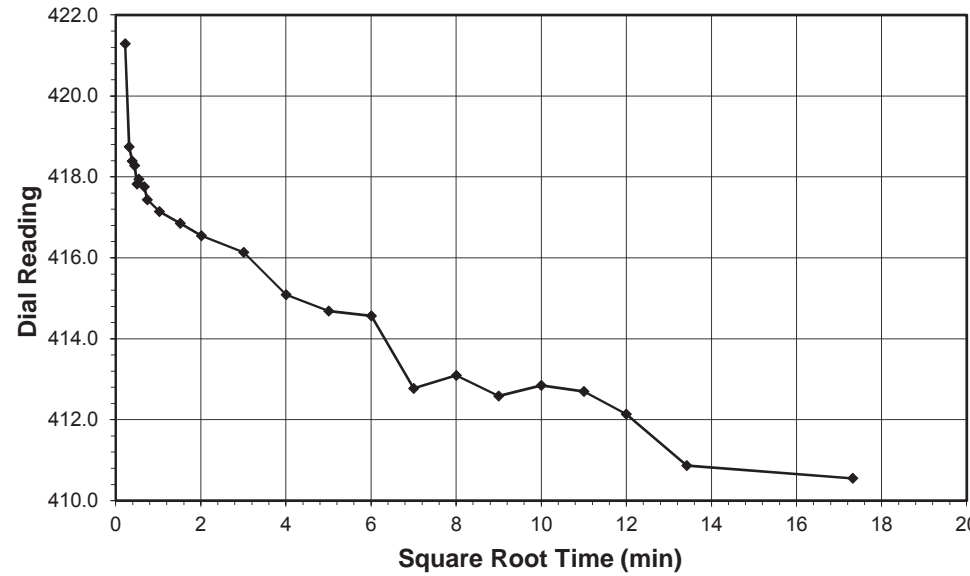
Tested By 129-04-0411 Date 5/18/18 Checked By GEM Date 5/23/18

**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

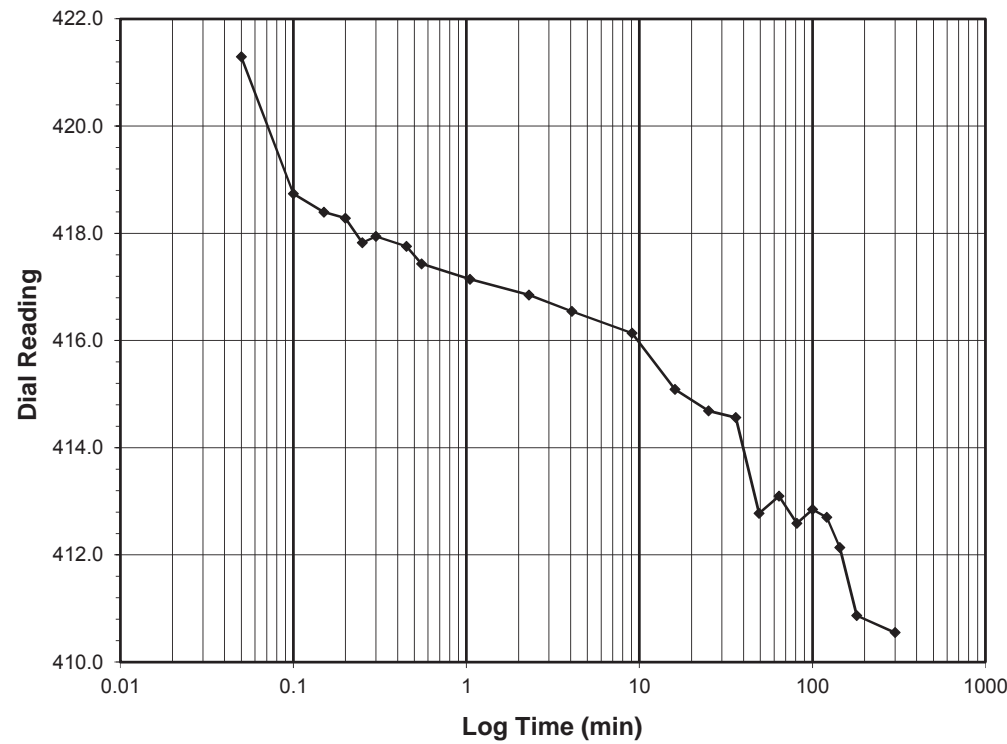
**Sample Conditions:** UNDISTURBED, INUNDATED AND DOUBLE DRAINED



**Test Load (tsf) 1.0-0.25**  
**Final Reading (div) 410.6**  
 Consolidometer No. **R470**  
 1 Division (in) 0.0001

Start Date 5/19/18  
 Start Time 3:30:45

Elapsed Time (min)	Dial Reading (div)
<b>Initial</b>	<b>450.9</b>
0.05	421.3
0.10	418.7
0.15	418.4
0.20	418.3
0.25	417.8
0.30	417.9
0.45	417.8
0.55	417.4
1.05	417.1
2.30	416.8
4.07	416.5
9.07	416.1
16.07	415.1
25.07	414.7
36.07	414.6
49.07	412.8
64.07	413.1
81.07	412.6
100.07	412.9
121.07	412.7
144.07	412.1
180.07	410.9
300.07	410.6

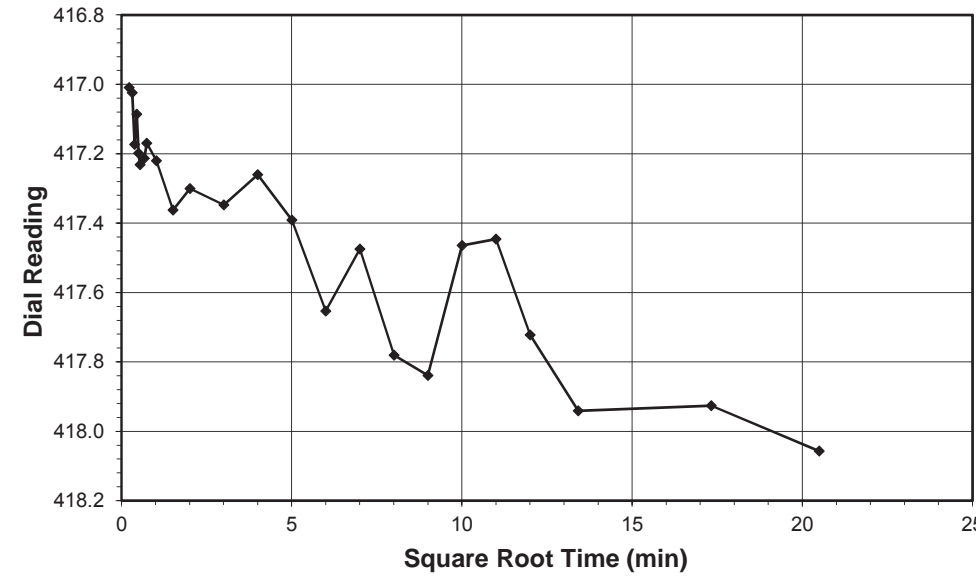


**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

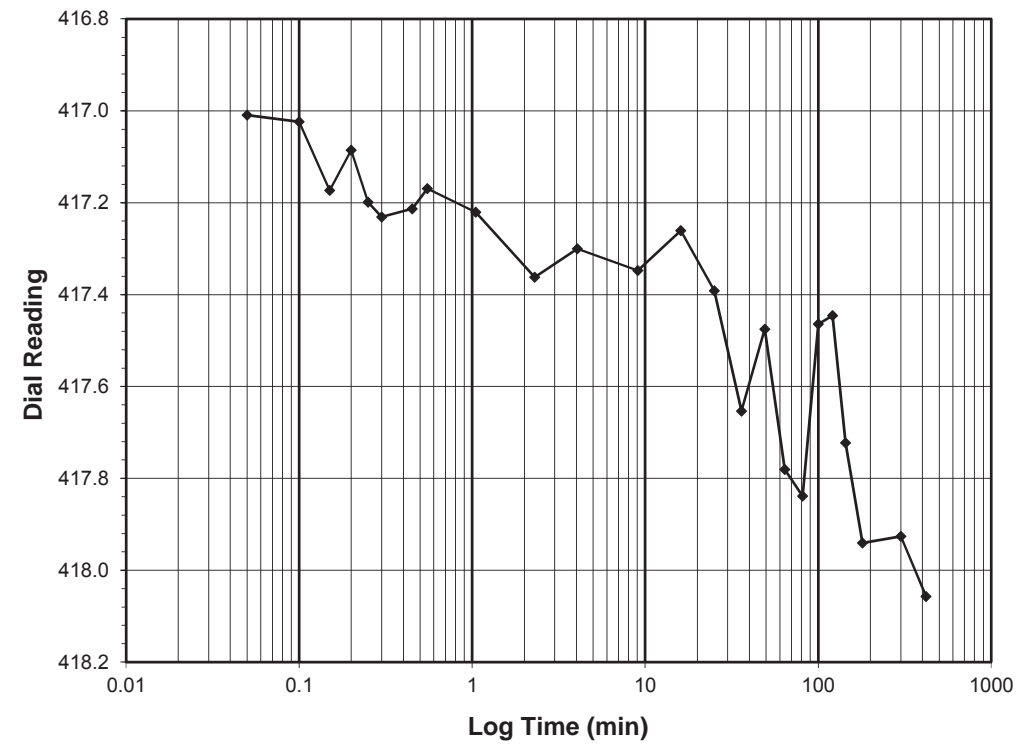
**Sample Conditions:** UNDISTURBED, INUNDATED AND DOUBLE DRAINED



**Test Load (tsf) 0.25-0.5**  
**Final Reading (div) 418.1**  
 Consolidometer No. **R470**  
 1 Division (in) 0.0001

Start Date 5/19/18  
 Start Time 10:31:01

Elapsed Time (min)	Dial Reading (div)
<b>Initial</b>	<b>410.6</b>
0.05	417.0
0.10	417.0
0.15	417.2
0.20	417.1
0.25	417.2
0.30	417.2
0.45	417.2
0.55	417.2
1.05	417.2
2.30	417.4
4.05	417.3
9.05	417.3
16.05	417.3
25.05	417.4
36.05	417.7
49.07	417.5
64.07	417.8
81.07	417.8
100.07	417.5
121.07	417.4
144.07	417.7
180.07	417.9
300.07	417.9
420.32	418.1



Tested By 129-04-0411 Date 5/19/18 Checked By GEM Date 5/23/18

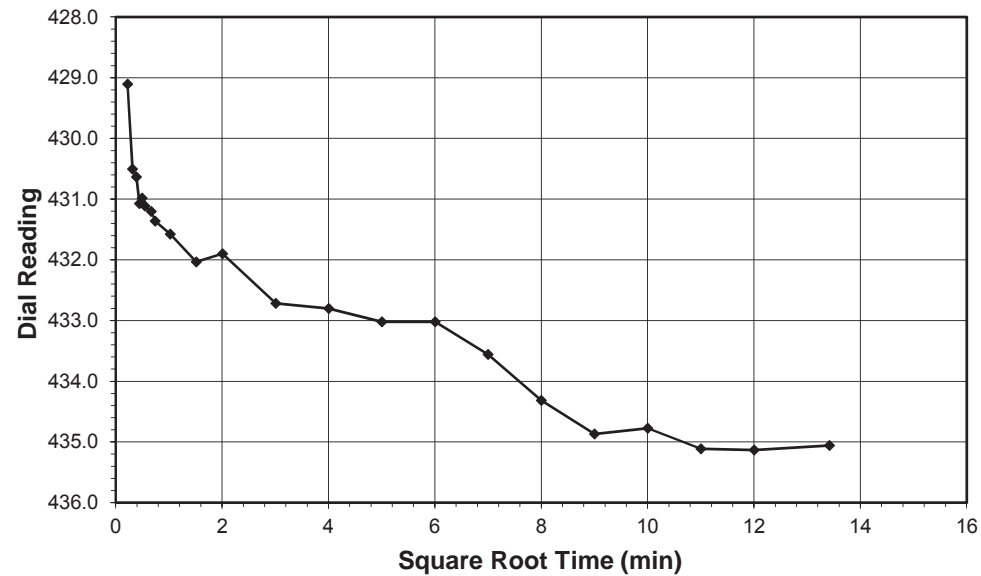
Tested By 129-04-0411 Date 5/19/18 Checked By GEM Date 5/23/18

**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

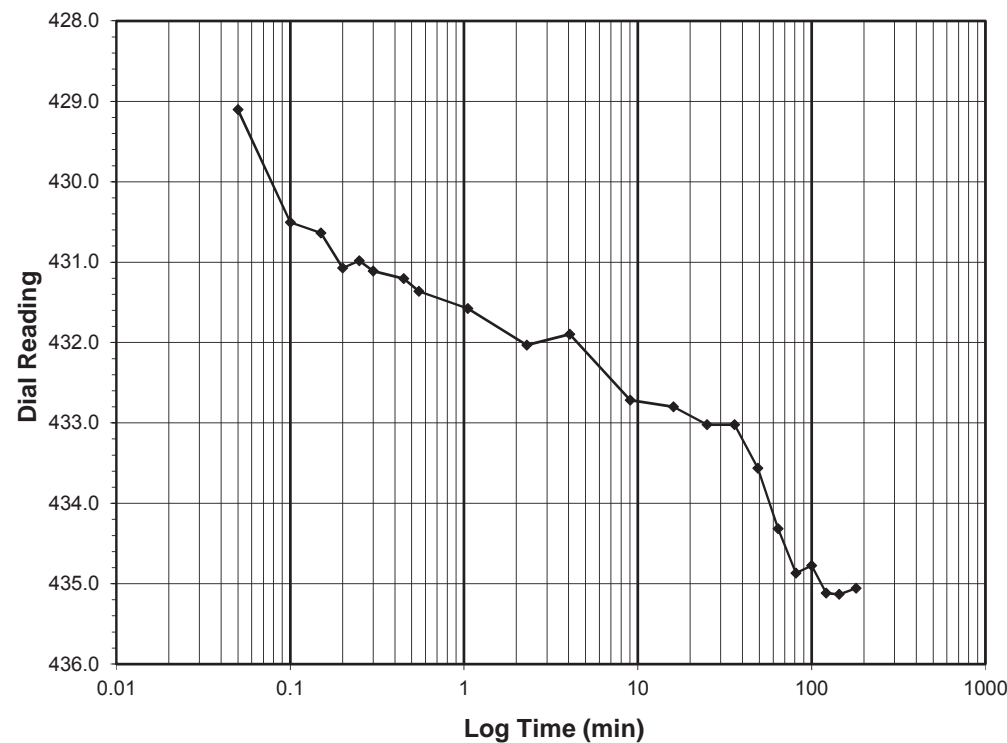
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



**Test Load (tsf) 0.5-1.0**  
**Final Reading (div) 435.1**  
 Consolidometer No. **R470**  
 1 Division (in) 0.0001

Start Date 5/19/18  
 Start Time 17:31:20

Elapsed Time (min)	Dial Reading (div)
<b>Initial</b>	<b>418.1</b>
0.05	429.1
0.10	430.5
0.15	430.6
0.20	431.1
0.25	431.0
0.30	431.1
0.45	431.2
0.55	431.4
1.05	431.6
2.30	432.0
4.05	431.9
9.05	432.7
16.05	432.8
25.05	433.0
36.05	433.0
49.05	433.6
64.05	434.3
81.05	434.9
100.07	434.8
121.07	435.1
144.07	435.1
180.07	435.1

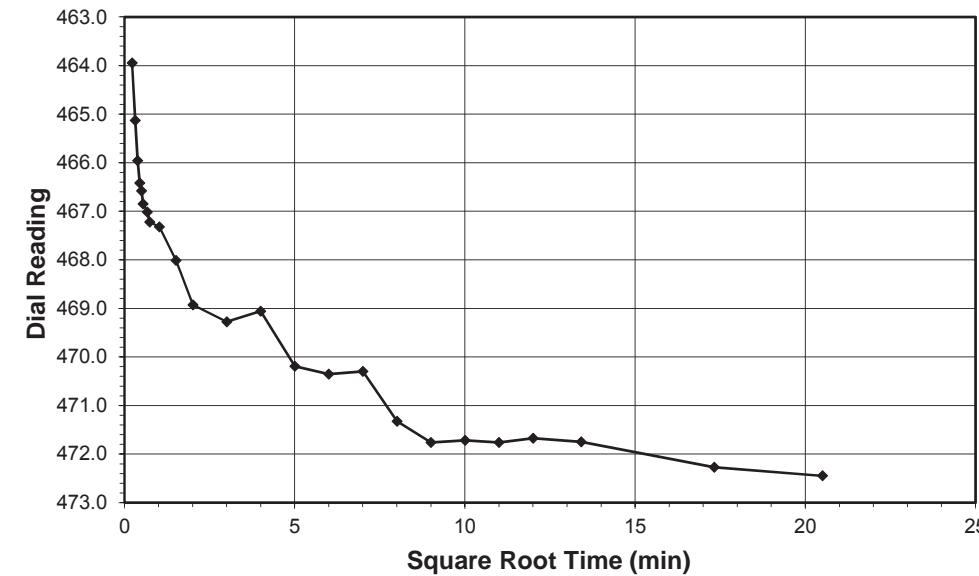


**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

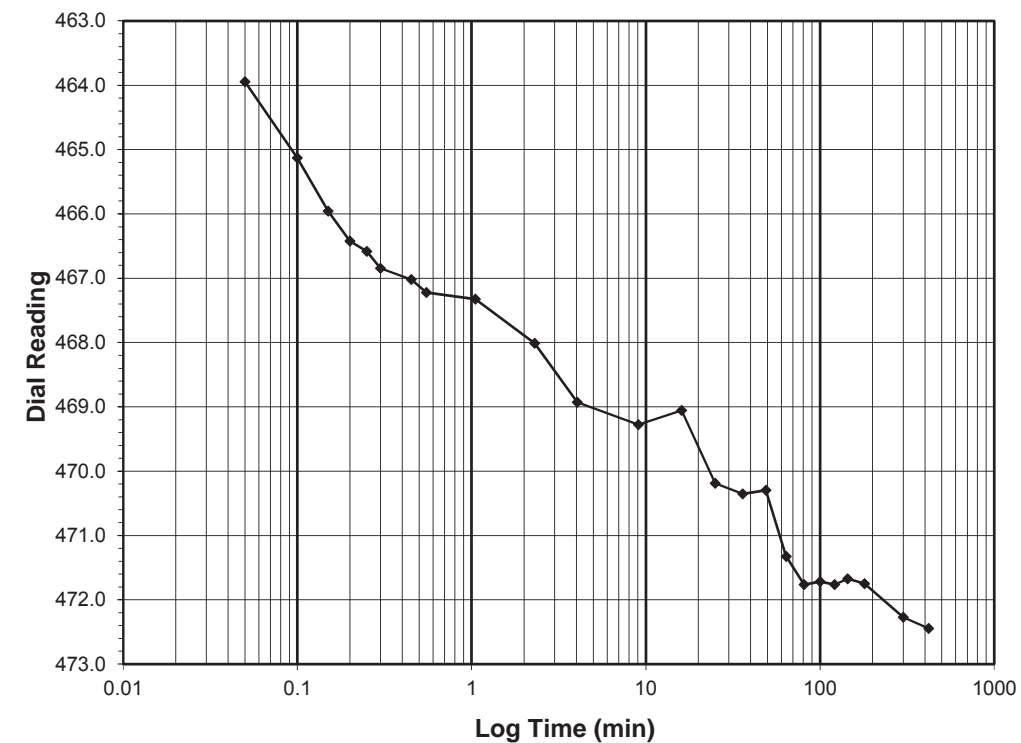
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



**Test Load (tsf) 1.0-2.0**  
**Final Reading (div) 472.4**  
 Consolidometer No. **R470**  
 1 Division (in) 0.0001

Start Date 5/20/18  
 Start Time 0:31:41

Elapsed Time (min)	Dial Reading (div)
<b>Initial</b>	<b>435.1</b>
0.05	463.9
0.10	465.1
0.15	466.0
0.20	466.4
0.25	466.6
0.30	466.8
0.45	467.0
0.55	467.2
1.05	467.3
2.30	468.0
4.05	468.9
9.05	469.3
16.05	469.1
25.05	470.2
36.05	470.4
49.05	470.3
64.07	471.3
81.07	471.8
100.07	471.7
121.07	471.8
144.07	471.7
180.07	471.7
300.07	472.3
420.37	472.4



Tested By 129-04-0411 Date 5/19/18 Checked By GEM Date 5/23/18

Tested By 129-04-0411 Date 5/20/18 Checked By GEM Date 5/23/18

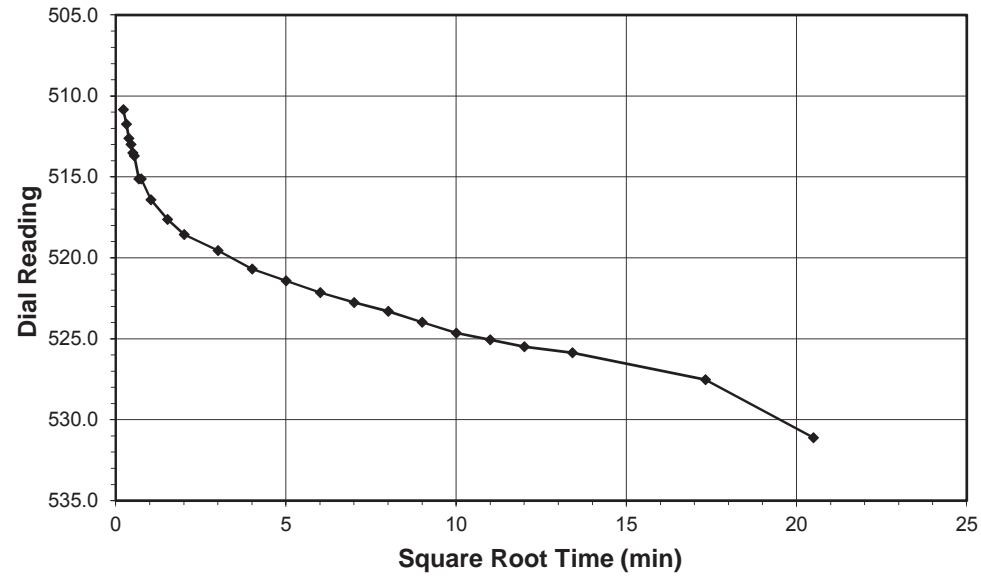


**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

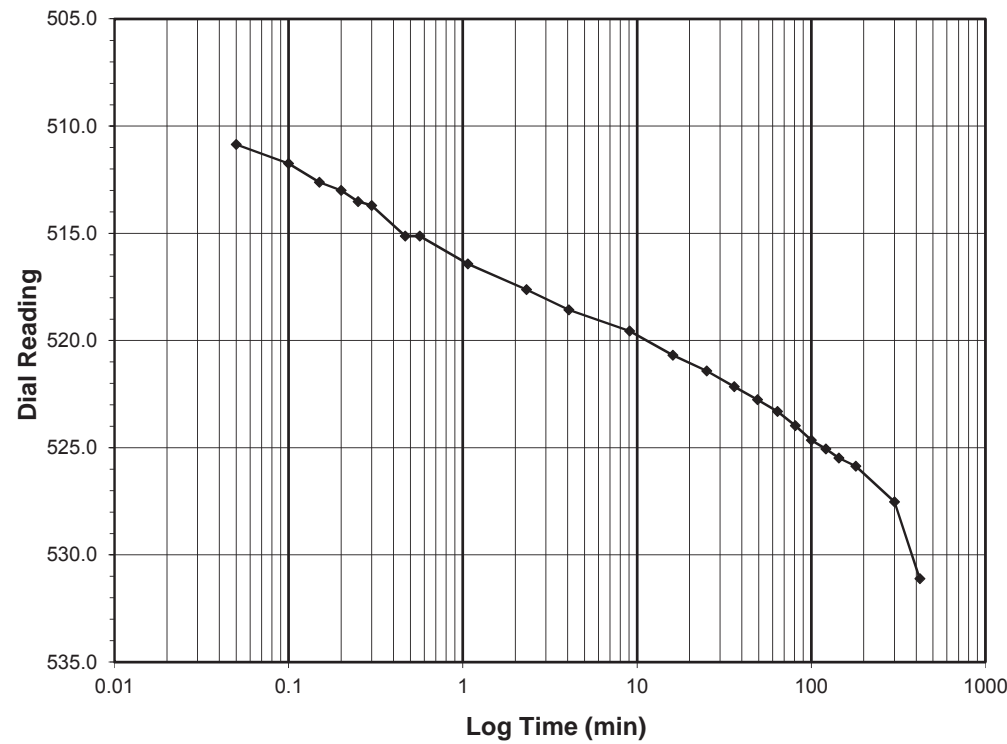
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



**Test Load (tsf) 2.0-4.0**  
**Final Reading (div) 531.1**  
 Consolidometer No. **R470**  
 1 Division (in) 0.0001

Start Date 5/20/18  
 Start Time 7:32:03

Elapsed Time (min)	Dial Reading (div)
Initial	472.4
0.05	510.9
0.10	511.7
0.15	512.6
0.20	513.0
0.25	513.5
0.30	513.7
0.47	515.1
0.57	515.1
1.07	516.4
2.32	517.6
4.07	518.6
9.07	519.6
16.07	520.7
25.07	521.4
36.07	522.2
49.07	522.8
64.07	523.3
81.07	524.0
100.07	524.6
121.07	525.1
144.07	525.5
180.07	525.9
300.08	527.5
420.23	531.1

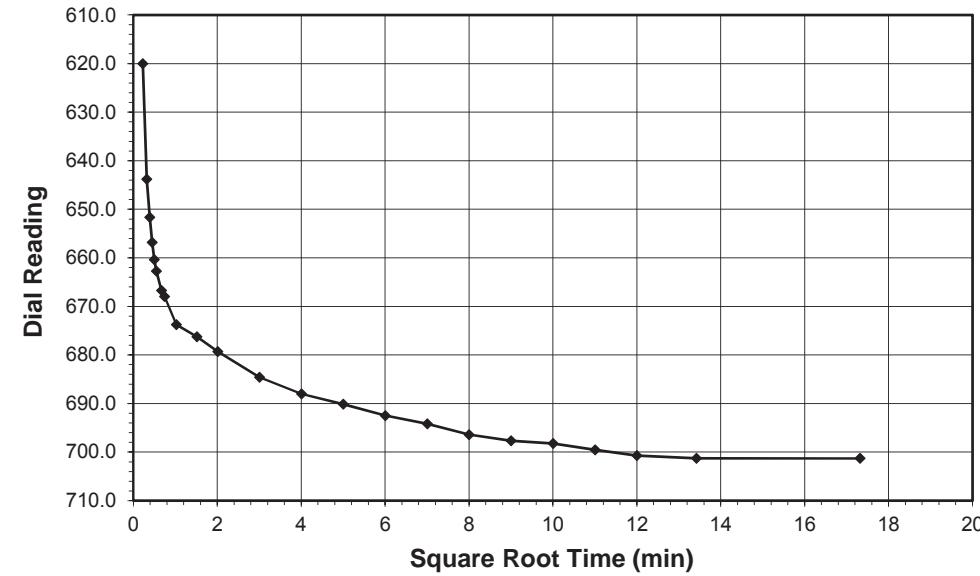


**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

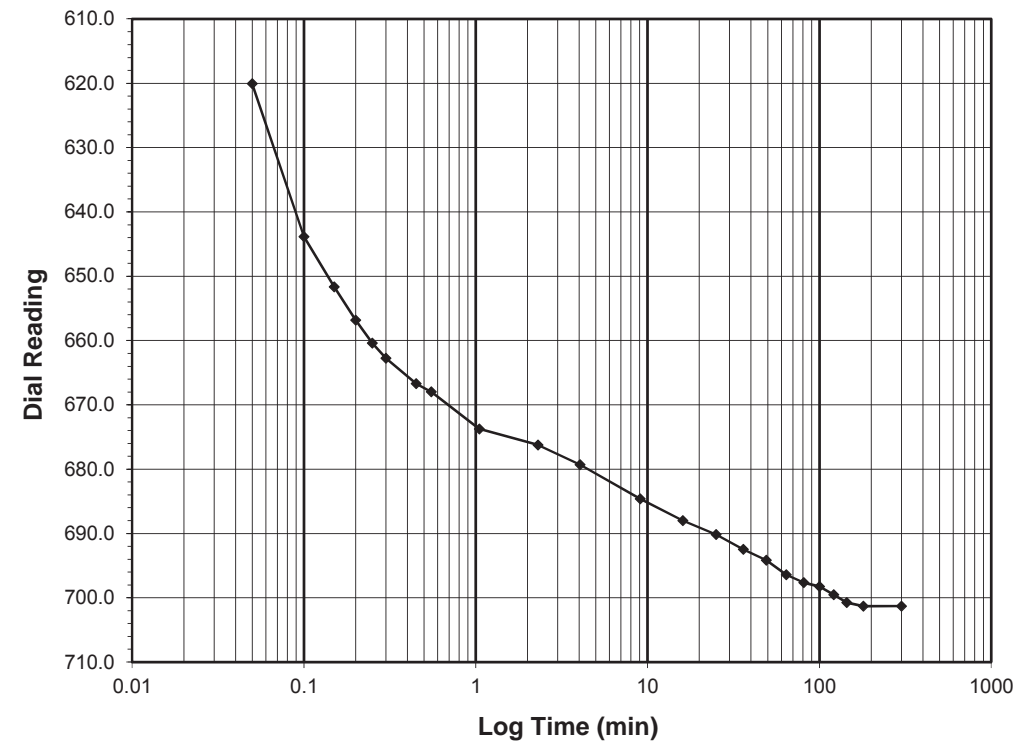
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



**Test Load (tsf) 4.0-8.0**  
**Final Reading (div) 701.3**  
 Consolidometer No. **R470**  
 1 Division (in) 0.0001

Start Date 5/20/18  
 Start Time 14:32:17

Elapsed Time (min)	Dial Reading (div)
Initial	531.1
0.05	620.1
0.10	643.8
0.15	651.7
0.20	656.8
0.25	660.4
0.30	662.8
0.45	666.7
0.55	668.0
1.05	673.8
2.30	676.2
4.05	679.3
9.05	684.6
16.05	688.0
25.05	690.2
36.05	692.5
49.05	694.2
64.07	696.4
81.07	697.7
100.07	698.2
121.07	699.5
144.07	700.7
180.07	701.3
300.07	701.3



Tested By 129-04-0411 Date 5/20/18 Checked By GEM Date 5/23/18

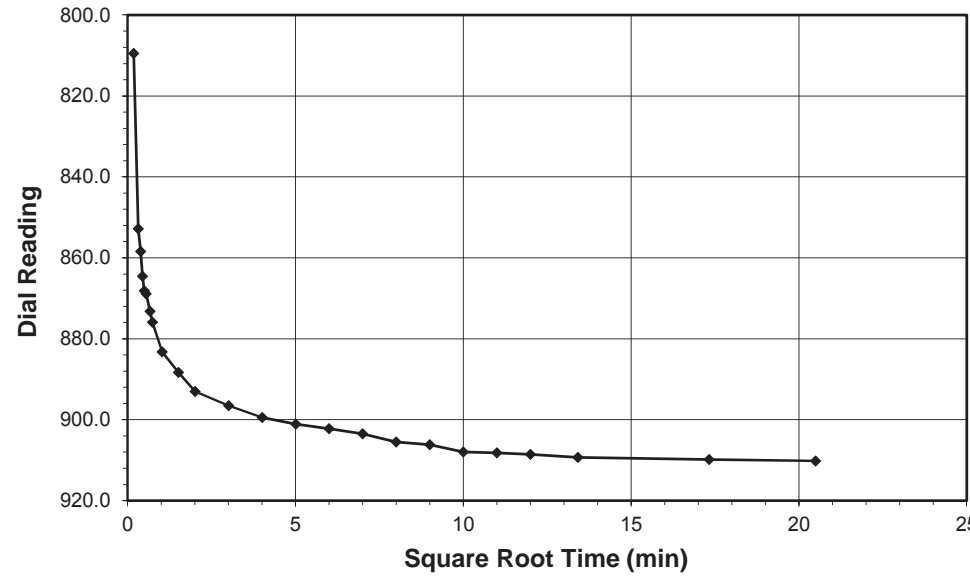
Tested By 129-04-0411 Date 5/20/18 Checked By GEM Date 5/23/18

**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

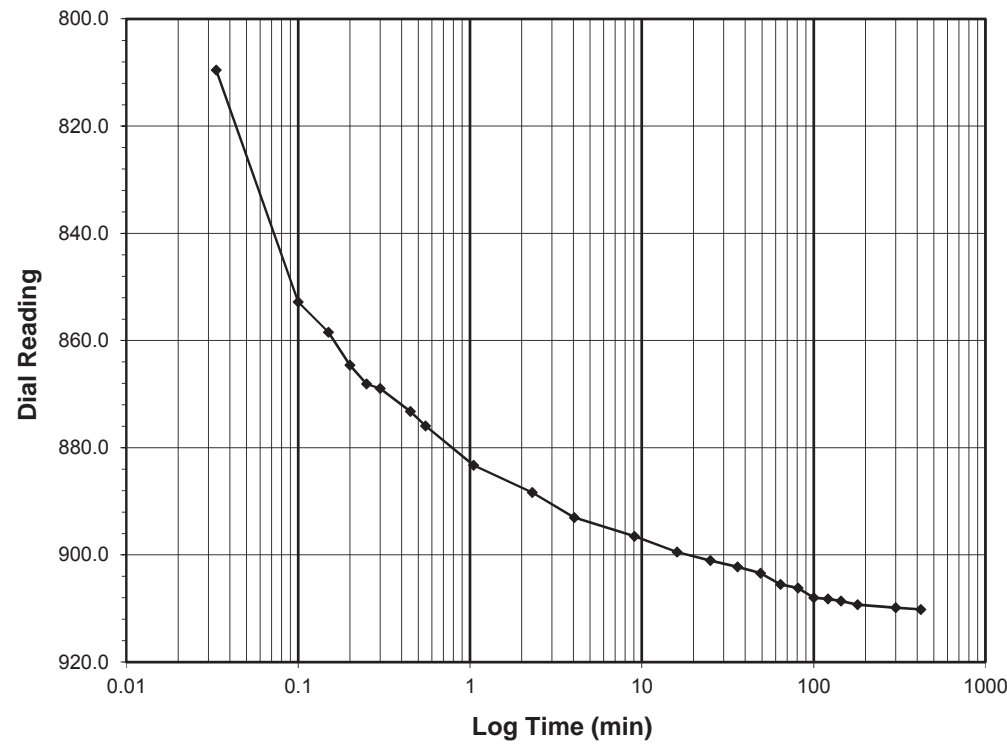
**Sample Conditions:** UNDISTURBED, INUNDATED AND DOUBLE DRAINED



**Test Load (tsf) 8.0-16.0**  
**Final Reading (div) 910.2**  
 Consolidometer No. **R470**  
 1 Division (in) 0.0001

Start Date 5/20/18  
 Start Time 21:32:30

Elapsed Time (min)	Dial Reading (div)
<b>Initial</b>	<b>701.3</b>
0.03	809.5
0.10	852.8
0.15	858.4
0.20	864.6
0.25	868.1
0.30	868.9
0.45	873.2
0.55	875.9
1.05	883.3
2.30	888.3
4.05	893.1
9.05	896.5
16.07	899.4
25.07	901.1
36.07	902.2
49.07	903.4
64.07	905.5
81.07	906.2
100.07	908.0
121.07	908.2
144.07	908.6
180.07	909.3
300.07	909.8
420.15	910.2

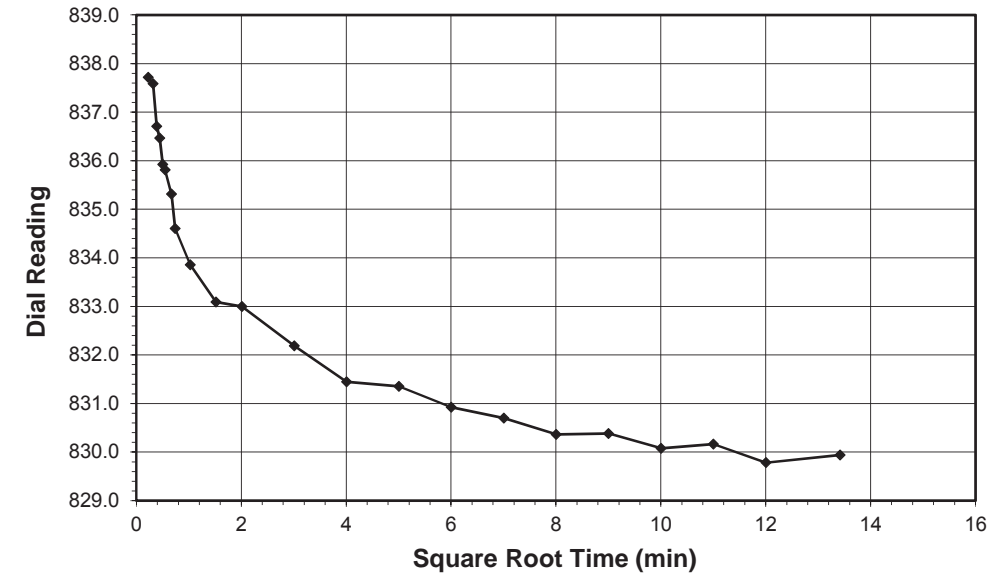


**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

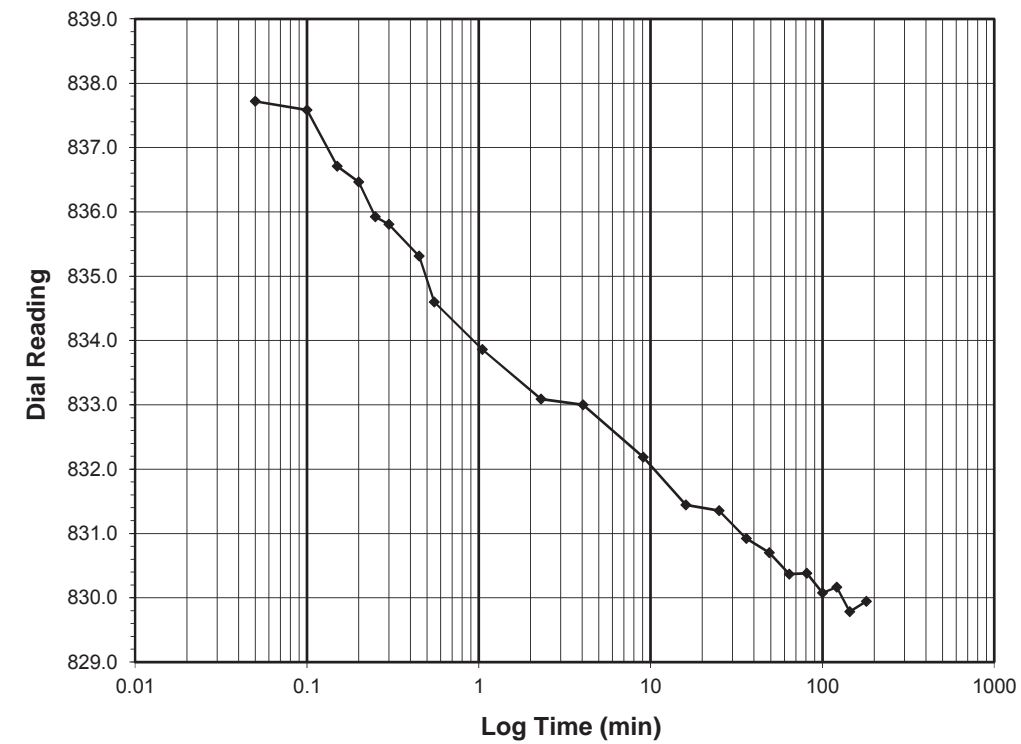
**Sample Conditions:** UNDISTURBED, INUNDATED AND DOUBLE DRAINED



**Test Load (tsf) 16.0-4.0**  
**Final Reading (div) 829.9**  
 Consolidometer No. **R470**  
 1 Division (in) 0.0001

Start Date 5/21/18  
 Start Time 4:32:40

Elapsed Time (min)	Dial Reading (div)
<b>Initial</b>	<b>910.2</b>
0.05	837.7
0.10	837.6
0.15	836.7
0.20	836.5
0.25	835.9
0.30	835.8
0.45	835.3
0.55	834.6
1.05	833.9
2.30	833.1
4.05	833.0
9.05	832.2
16.05	831.4
25.05	831.4
36.05	830.9
49.05	830.7
64.05	830.4
81.05	830.4
100.05	830.1
121.05	830.2
144.05	829.8
180.05	829.9



Tested By 129-04-0411 Date 5/20/18 Checked By GEM Date 5/23/18

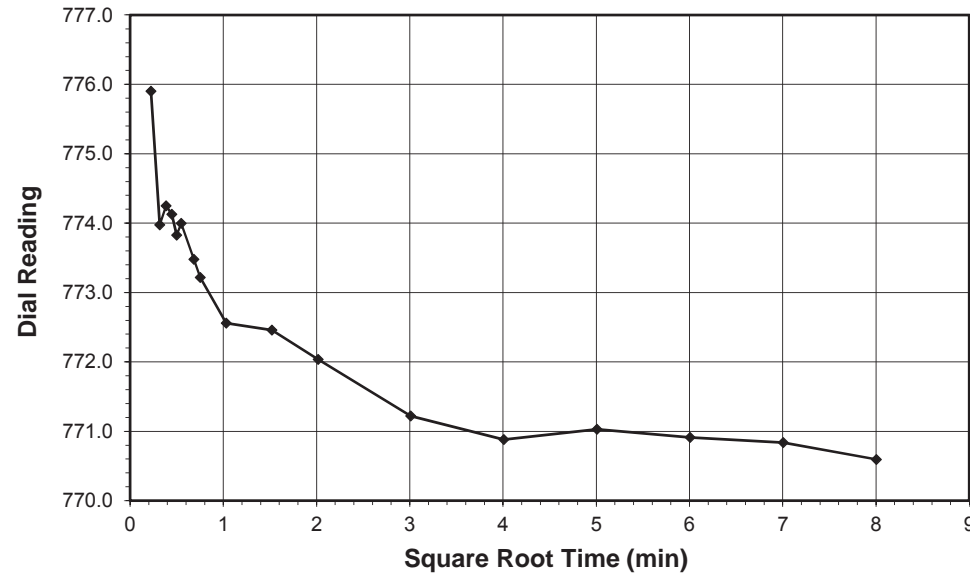
Tested By 129-04-0411 Date 5/21/18 Checked By GEM Date 5/23/18

**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



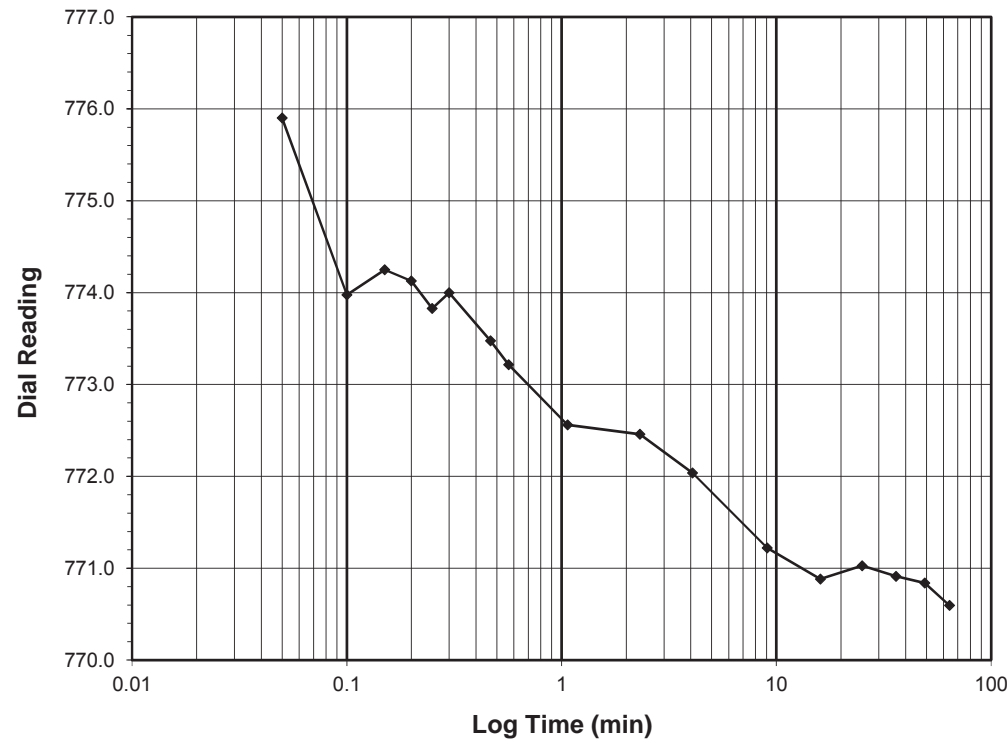
Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 4.0-1.0  
 Final Reading (div) 770.6  
 Consolidometer No. R470  
 1 Division (in) 0.0001  
 Start Date 5/21/18  
 Start Time 11:33:01

Elapsed Time (min)	Dial Reading (div)
Initial	829.9
0.05	775.9
0.10	774.0
0.15	774.2
0.20	774.1
0.25	773.8
0.30	774.0
0.47	773.5
0.57	773.2
1.07	772.6
2.32	772.5
4.07	772.0
9.07	771.2
16.07	770.9
25.07	771.0
36.07	770.9
49.07	770.8
64.07	770.6

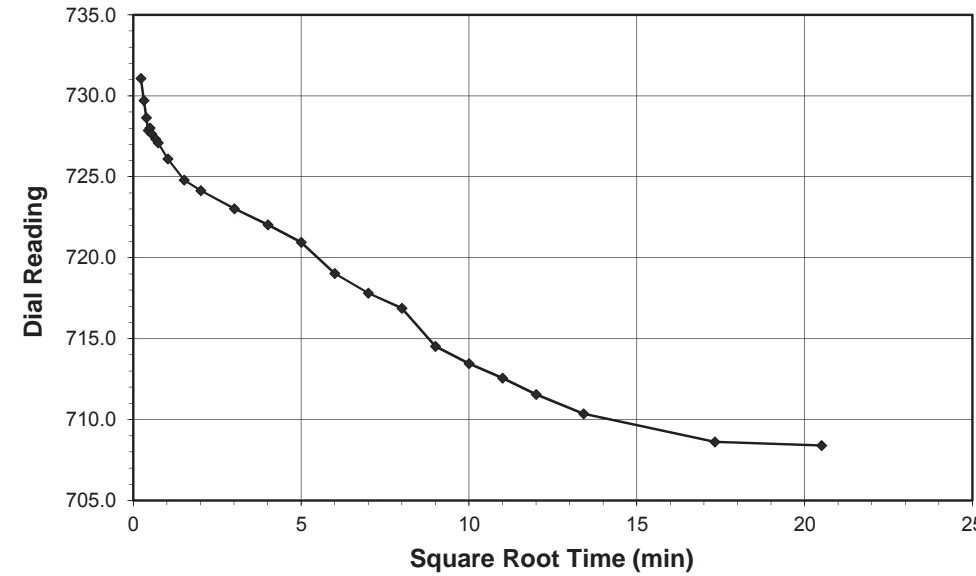


**ONE DIMENSIONAL CONSOLIDATION**  
AASHTO T-216



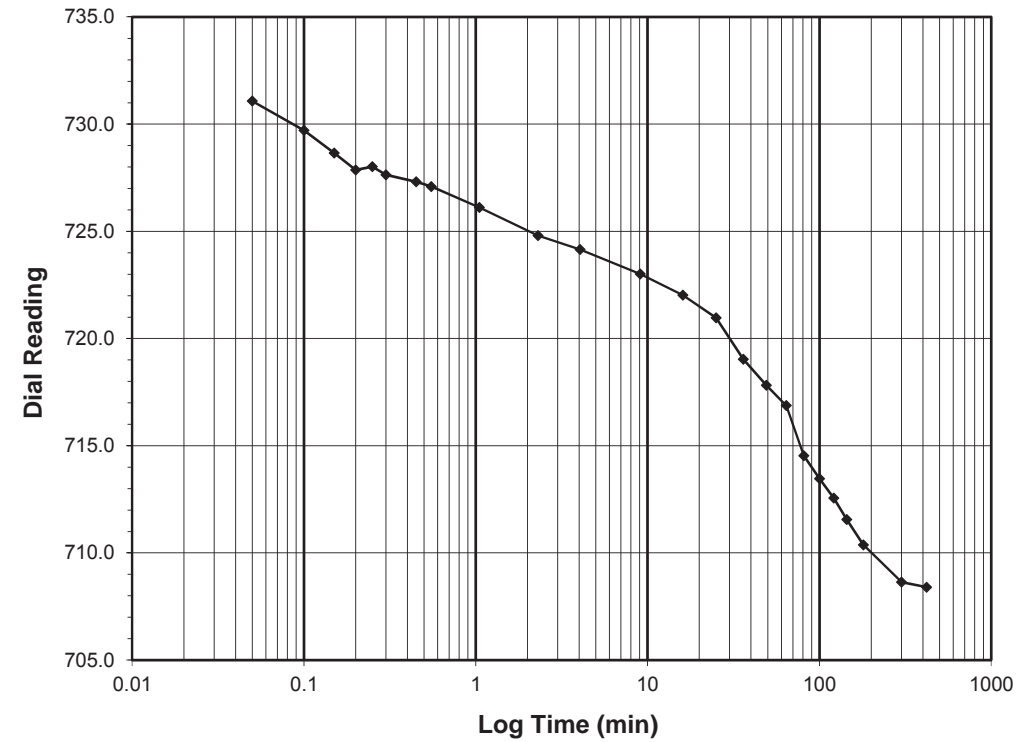
Client Kleinfelder Boring No. RPA\_1449  
 Client Project I-5700 / 20151548.053A Depth (ft) 5.0-7.0  
 Project No. R-2018-133-001 Sample No. ST-6  
 Lab ID R-2018-133-001-007 Visual Description BROWN SANDY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 1.0-0.25  
 Final Reading (div) 708.4  
 Consolidometer No. R470  
 1 Division (in) 0.0001  
 Start Date 5/21/18  
 Start Time 18:33:12

Elapsed Time (min)	Dial Reading (div)
Initial	770.6
0.05	731.1
0.10	729.7
0.15	728.6
0.20	727.9
0.25	728.0
0.30	727.6
0.45	727.3
0.55	727.1
1.05	726.1
2.30	724.8
4.05	724.2
9.05	723.0
16.05	722.0
25.05	721.0
36.05	719.0
49.07	717.8
64.07	716.9
81.07	714.5
100.07	713.5
121.07	712.6
144.07	711.6
180.07	710.4
300.07	708.6
420.38	708.4



Tested By 129-04-0411 Date 5/21/18 Checked By GEM Date 5/23/18

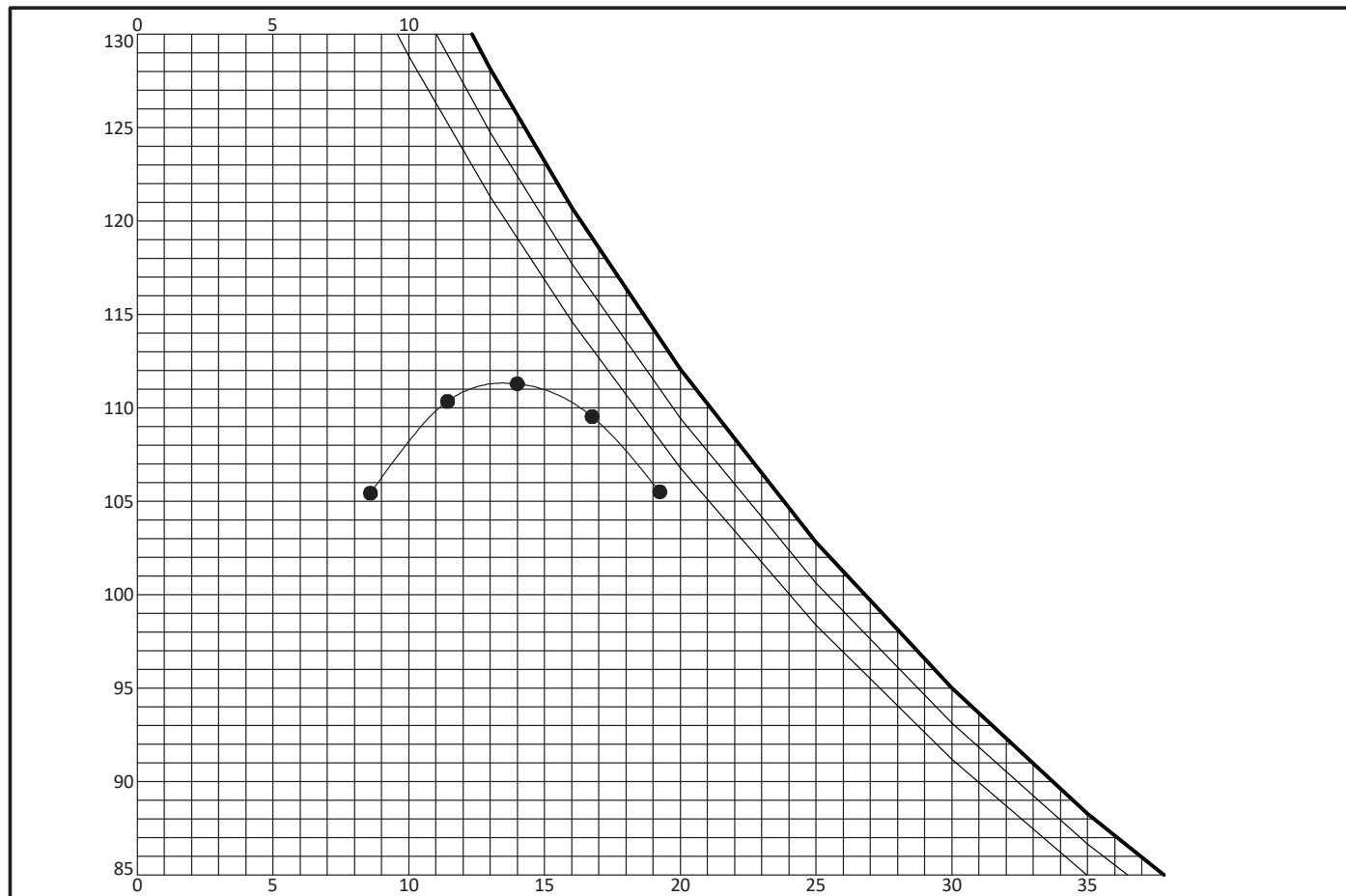
Tested By 129-04-0411 Date 5/21/18 Checked By GEM Date 5/23/18





**MOISTURE-DENSITY  
RELATIONSHIP**

Project No: 66W-0056  
 Client: Kleinfelder  
 Project: I-5700  
 City/State: N.A.



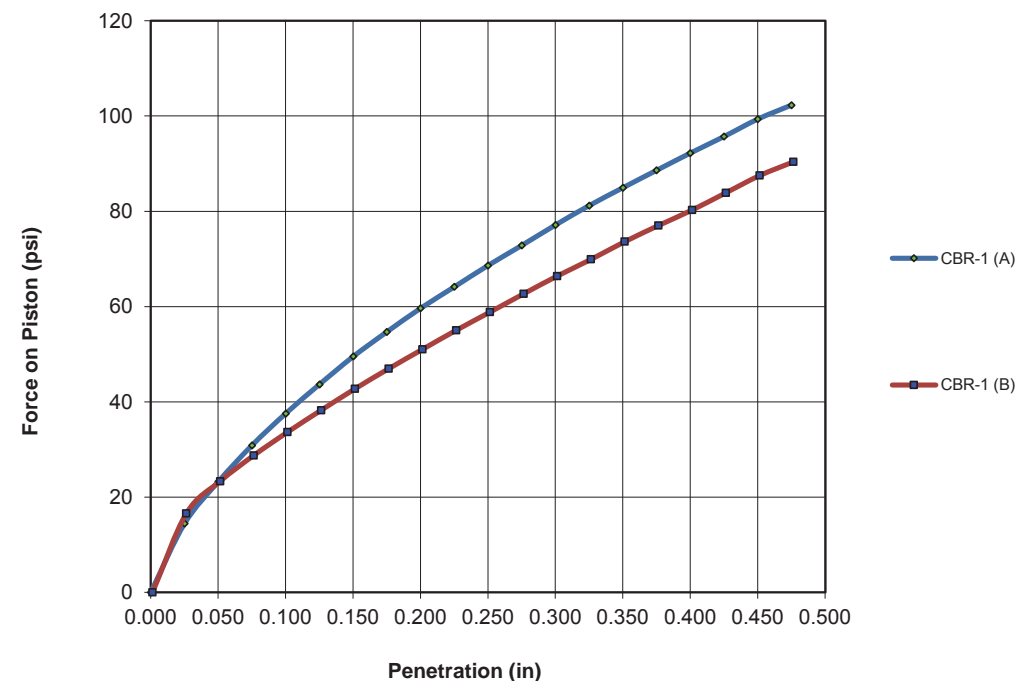
Sample	Depth (ft)	Sample Description	LL	PL	PI	% GRAVEL	% SAND	% FINES
● S-12	0.0' - 10.0'	Brown, SANDY LEAN CLAY (CL)	36	20	16	5.4	32.0	62.6

Sample	Optimum Moisture (%)	Max Dry Density (pcf)	AASHTO Classification	AASHTO Method	Sample Location	Natural Moisture (%)
● S-12	13.5	111.3	A-6 (8)	T-99 B	RPB-176	13.1



**FROEHLING & ROBERTSON, INC.**  
 California Bearing Ratio Test Report

**Load Penetration Curve**



**SOAKED CBR TEST RESULTS**

Results	CBR-1 (A)	CBR-1 (B)
0.1 in Pen.	3.8	3.4
0.2 in Pen.	4.0	3.4
Moisture (%)	12.8	13.0
Density (pcf)	111.5	110.3
Final Moisture (%)	18.1	18.3
Final Density (pcf)	111.4	110.6

**Project Information**

**Natural Moisture (%)**

Project Name:	I-5700	CBR-1	13.1
Client Name:	Kleinfelder		
Project Number:	66W-0056	<b>Percent Swell</b>	<b>A</b> <b>B</b>
Date Received:	5/11/2018	CBR-1	2.600   2.690
Project Location:	N.A.		

**Sample Information**

**Proctor Value (ASTM D-698)**

<b>Sample Number</b>	<b>Classification</b>	<b>Max. Dry Density (pcf)</b>	<b>Optimum Moisture %</b>
S-12	A-6 (8)	111.3	13.5
<b>Sample Number</b>	<b>Sample Location</b>	<b>AASHTO</b>	<b>% - #200</b>
S-12	RPB-1761	A-6 (8)	62.6

PROCTOR CURVE 66W-0056 CLASSIFICATIONS & PROCTORS.GPJ F&R.GDT 6/5/18

<b>Specimen S-12 (A) Information</b>	<b>Froehling &amp; Robertson, Inc.</b>
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**Project Information**

Project No. 66W-0056 Project Name: I-5700 Client: Kleinfelder Sample Location: RPB-1761 Sample Description: A-6 (8)	Date Tested 5/29/2018 Natural Moisture: 13.1 AASHTO: A-6 (8) Surcharge Weight (lb.): 10 % Retained on 4.75 mm: 5.4%
---	---

**Specimen Data**

<b>Hours Soaked</b>	<b>96</b>
Soaked Height (in):	4.6960001
Swell (%):	2.5999999

Liquid Limit:	36	Max Dry Dens. (pcf):	111.3
Plastic Limit:	16	Opt. Moisture (%):	13.5

Compacted Specimen	
Height (in)	4.58
Weight (g)	8039.30
Soil Weight + Mold (g)	27.15
Soil Weight (g)	4275.05
Mold Volume (in3)	129.4
Dry Density (pcf)	111.5
Percent Compaction	100.2

	Moisture Percentage	
	Initial	Final
Moist Soil + tare (g)	357.10	1527.80
Dry Soil + tare (g)	329.10	1327.70
tare (g)	110.20	223.90
Moisture (%)	12.8	18.1

**Specimen Test Data**

Read Number	Load (lbs)	Disp. (in)	Force on Piston (psi)	Penetration (in)	CBR
0	-6.96	0.00006	0.0	0.000	0.00
2	62.72	0.05013	23.2	0.051	0.00
3	85.62	0.07510	30.9	0.076	0.00
4	105.66	0.10010	37.5	0.101	3.75
5	124.01	0.12513	43.7	0.127	0.00
6	141.53	0.15013	49.5	0.152	0.00
7	157.06	0.17508	54.7	0.177	0.00
8	171.92	0.20008	59.6	0.203	3.97
9	185.41	0.22509	64.1	0.228	0.00
10	198.89	0.25010	68.6	0.253	0.00
11	211.51	0.27507	72.8	0.279	0.00
12	224.41	0.30010	77.1	0.304	4.06
13	236.61	0.32508	81.2	0.329	0.00
14	247.87	0.35013	84.9	0.355	0.00
15	258.83	0.37507	88.6	0.380	0.00
16	269.69	0.40009	92.2	0.405	4.01
18	291.08	0.45011	99.3	0.456	0.00
19	299.89	0.47507	102.3	0.482	0.00

Test Performed By: Drew Council

<b>Specimen S-12 Information</b>	<b>Froehling &amp; Robertson, Inc.</b>
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**Project Information**

Project No. 66W-0056 Project Name: I-5700 Client: Kleinfelder Sample Location: RPB-1761 Sample Description: A-6 (8)	Date Tested 5/29/2018 Natural Moisture: 13.1 AASHTO: A-6 (8) Surcharge Weight (lb.): 10 % Retained on 4.75 mm: 5.4%
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**Specimen Data**

<b>Hours Soaked</b>	<b>96</b>
Soaked Height (in):	4.6989999
Swell (%):	2.6900001

Liquid Limit:	36	Max Dry Dens. (pcf):	111.3
Plastic Limit:	16	Opt. Moisture (%):	13.5

Compacted Specimen	
Height (in)	4.58
Weight (g)	8061.90
Soil Weight + Mold (g)	27.11
Soil Weight (g)	4234.08
Mold Volume (in3)	4275.1
Dry Density (pcf)	110.3
Percent Compaction	99.1

	Moisture Percentage	
	Initial	Final
Moist Soil + tare (g)	397.60	1387.70
Dry Soil + tare (g)	364.60	1211.90
tare (g)	109.90	251.30
Moisture (%)	13.0	18.3

**Specimen Test Data**

Read Number	Load (lbs)	Disp. (in)	Force on Piston (psi)	Penetration (in)	CBR
0	-22.2	0.00139	0.0	0.000	0.00
2	47.7	0.05147	23.3	0.051	0.00
3	64.0	0.07646	28.8	0.076	0.00
4	78.7	0.10145	33.6	0.101	3.36
5	92.5	0.12640	38.2	0.127	0.00
6	105.9	0.15148	42.7	0.152	0.00
7	118.6	0.17643	46.9	0.177	0.00
8	130.8	0.20142	51.0	0.203	3.40
9	142.8	0.22641	55.0	0.228	0.00
10	154.2	0.25142	58.8	0.253	0.00
11	165.7	0.27644	62.7	0.279	0.00
12	177.0	0.30143	66.4	0.304	3.49
13	187.4	0.32641	69.9	0.329	0.00
14	198.6	0.35146	73.6	0.355	0.00
15	208.8	0.37648	77.0	0.380	0.00
16	218.5	0.40143	80.3	0.405	3.49
18	240.2	0.45147	87.5	0.456	0.00
19	248.9	0.47647	90.4	0.481	0.00

Test Performed By: Drew Council