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PREPARED BY: J. R. HAMM	DATE: 5/15/19
REVIEWED BY: S. C. CROCKETT	DATE: 5/17/19



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PRECAST CONCRETE PANEL (TYP)	
PRECAST CONCRETE PANEL (TYP)	
G"MIN G"MIN ADJA ALIG NG PAD STEP REIN ADJA ALIG	P TOP OF LEVELING PAD SO IFORCEMENT LAYERS BETWEEN CENT PRECAST PANELS ARE GNED AS SHOWN
PRECAST PANELS Eveling pad step [S Detail
PR STA	OJECT NO.: <u>A-0011C</u> <u>CLAY</u> COUNTY ATION: <u>168+75, 60' LT TO 172+25, 56.75' LT</u>
VORTH CAROLINA EENT OF TRANSPORTATION ISION OF HIGHWAYS	RETAINING WALL NO. 2 ISE WALL WITH PRECAST PANELS TYPICAL DETAIL
EOTECHNICAL INEERING UNIT	REVISIONS BY DATE NO. BY DATE NO. 3 4 4 4 4

GEOTECHNICAL ENGINEER

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7) IN-SITU ASSUMED MATERIAL PARAMETERS: FRICTION MATERIAL TYPE UNIT WEIGHT PCF DEGRE BACKFILL 120 30 FOUNDATION 110 28 DESIGN RETAINING WALL NO.2 FOR THE FOLLOWING:

DESIGN RETAINING WALL NO.1 FOR THE FOLLOWING:

NOTES:

SPECIFICATIONS ARE REQUIRED.

1) H = DESIGN HEIGHT + EMBEDMENT

5) MINIMUM EMBEDMENT DEPTH = 2 FT

6) REINFORCED ZONE AGGREGATE PARAMETERS:

2) DESIGN LIFE = 100 YEARS

aggregate type*

COARSE

FINE

MATERIAL REQUIREMENTS.

1) H = DESIGN HEIGHT + EMBEDMENT

2) DESIGN LIFE = 100 YEARS 3) MAXIMUM FACTORED VERTICAL PRESSURE ON FOUNDATION MATERIAL = 4,500 PSF 4) MINIMUM REINFORCEMENT LENGTH (L) = 0.8 H OR 6 FT, WHICHEVER IS LONGER 5) MINIMUM EMBEDMENT DEPTH = 4 FT

6) REINFORCED ZONE AGGREGATE PARAMETERS:							
AGGREGATE TYPE *	UNIT WEIGHT (y) PCF	FRICTION (ф DEGRE					
COARSE	110	38					
FINE	115	34					

*SEE MSE RETAINING WALLS PROVISION FOR COARSE AND FINE AGGREGATE MATERIAL REQUIREMENTS.

7) TNI_STTIL ASSUMED

7) IN-SITU ASSUMED MATH	ERIAL PARAMETERS:	
MATERIAL TYPE	UNIT WEIGHT (y) PCF	FRICTION (¢ DEGR
BACKFILL	120	3(
FOUNDATION	110	28

DESIGN RETAINING WALL NO.1 FOR A LIVE LOAD (TRAFFIC) SURCHARGE. EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, GUARDRAIL, FENCE OR HANDRAIL POSTS, PAVEMENTS, PIPES, INLETS OR UTILITIES WILL INTERFERE WITH REINFORCEMENT FOR RETAINING WALL NO.1.

DO NOT PLACE LEVELING PAD CONCRETE, AGGREGATE OR REINFORCEMENT FOR RETAINING WALL NO.1 UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIAL ARE APPROVED. AT THE CONTRACTOR'S OPTION, "TEMPORARY SHORING FOR WALL CONSTRUCTION" MAY BE USED TO CONSTRUCT RETAINING WALL NO.1. SEE MSE RETAINING WALLS PROVISION FOR TEMPORARY SHORING FOR WALL CONSTRUCTION.



N ANGLE COHESION (C)PSF REES 0 0

)N ANGLE COHESION (C)RES PSF 0 0

N ANGLE) EES	COHESION (c) PSF
)	0
3	0

3) MAXIMUM FACTORED VERTICAL PRESSURE ON FOUNDATION MATERIAL = 2,050 PSF 4) MINIMUM REINFORCEMENT LENGTH (L) = 0.85 H OR 6 FT, WHICHEVER IS LONGER

COHESION

(C)

PSF

0

0

AT THE CONTRACTOR'S OPTION, USE FINE AGGREGATE IN THE REINFORCED ZONE OF RETAINING WALLS NO.1 OF 2.

FRICTION ANGLE

38

34

DEGREES

CIP REINFORCED CONCRETE COPING IS REQUIRED FOR RETAINING WALL NO.1.

UNIT WEIGHT

 (γ)

PCF

110

115

*SEE MSE RETAINING WALLS PROVISION FOR COARSE AND FINE AGGREGATE

A SEPARATION GEOTEXTILE IS REQUIRED AT THE BACK OF THE REINFORCED ZONE FOR RETAINING WALLS NO.1 AND 2. A DRAIN IS REQUIRED FOR RETAINING WALLS NO.1 AND 2. BEFORE BEGINNING MSE WALL DESIGN FOR RETAINING WALLS NO.1 AND 2, SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE)FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED.

FOR STEEL BEAM GUARDRAIL, SEE ROADWAY PLANS AND SECTION 862 OF THE STANDARD SPECIFICATIONS.

USE AN MSE WALL SYSTEM WITH SEGMENTAL RETAINING WALL (SRW) UNITS THAT MEET ARTICLE 1040-4 OF THE STANDARD SPECIFICATIONS FOR RETAINING WALL NO.1.

WHEN USING AN MSE WALL SYSTEM WITH SRW UNITS FOR RETAINING WALLS NO.1 AND 2, FREEZE-THAW DURABLE SRW UNITS THAT MEET ARTICLE 1040-4 OF THE STANDARD

FOR MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALLS, SEE MECHANICALLY STABILIZED EARTH RETAINING WALLS PROVISION.

	PROJECT NO.: A-0011C
	CLAY COUNTY
	STATION: <u>59+85, 54' RT TO 61+75, 58' RT</u> 168+75, 60' LT TO 172+25, 56,75' LT
	SHEET 6 OF 8
ORTH CAROLINA NT OF TRANSPORTATION ION OF HIGHWAYS	RETAINING WALLS NO. 1 AND 2 MSE WALL NOTES
OTECHNICAL	
NEERING UNIT	REVISIONS SHEET NO.NO.BYDATENO.BYDATENO.13W-7





PREPARED BY: J. R. HAMM	DATE: 5/15/19
REVIEWED BY: S. C. CROCKETT	DATE: 5/17/19

NOTES:

FOR ANCHORED RETAINING WALLS, SEE ANCHORED RETAINING WALLS PROVISION.

FOR STEEL BEAM GUARDRAIL,SEE ROADWAY PLANS AND SECTION 862 OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTOR'S OPTION, USE DRIVEN H-PILES FOR RETAINING WALL NO.3.

BEFORE BEGINNING ANCHORED WALL DESIGN FOR RETAINING WALL NO.3, SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE)FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED.

DESIGN RETAINING WALL NO.3 FOR THE FOLLOWING:

1) H = DESIGN HEIGHT + WALL EMBEDMENT 2) DESIGN LIFE = 100 YEARS

3) MINIMUM WALL EMBEDMENT ELEVATION = 1,818.3 FT

4) MINIMUM PILE PENETRATION INTO ROCK = N/A 5) IN-SITU ASSUMED MATERIAL PARAMETERS ABOVE ELEVATION 1,805 FT: UNIT WEIGHT, γ = 115 PCF FRICTION ANGLE, ϕ = 28 DEGREES

COHESION, c = O PSF 6) IN-SITU ASSUMED MATERIAL PARAMETERS BELOW ELEVATION 1,805 FT: UNIT WEIGHT, g = 125 PCF FRICTION ANGLE, f = 32 DEGREES COHESION, c = O PSF

THE MINIMUM WALL EMBEDMENT ELEVATION FOR RETAINING WALL NO.3 INCLUDES EMBEDMENT FOR SCOUR.

DESIGN WALL FOR FULL EMBEDMENT EXPOSURE (BOTTOM OF WALL EQUAL TO EMBEDMENT ELEVATION) DUE TO SCOUR POTENTIAL.

DESIGN RETAINING WALL NO.3 FOR A LIVE LOAD (TRAFFIC) SURCHARGE.

DESIGN RETAINING WALL NO.3 FOR A PIPE AND CULVERT EXTENDING UNDER OR THROUGH THE WALL AS SHOWN. VERIFY PIPE AND CULVERT LOCATION AND ELEVATION BEFORE BEGINNING ANCHORED WALL DESIGN OR CONSTRUCTION.

EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, GUARDRAIL, FENCE OR HANDRAIL POSTS, PAVEMENTS, PIPES, INLETS OR UTILITIES MAY INTERFERE WITH GROUND ANCHORS FOR RETAINING WALL NO. 3.

PREPARED BY: J. R. HAMM	DATE: 5/15/19
REVIEWED BY: S. C. CROCKETT	DATE: 5/17/19





REINFORCED WEB DETAILS

#DETAILS SHOWN ARE FOR 12"H-PILES WITH 6"DIA. GROUND ANCHORS. FOR DIFFERENT DIAMETER ANCHORS, SUBMIT ALTERNATE REINFORCED WEB DETAILS FOR ACCEPTANC



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CAROLINA F TRANSPORTATION OF HIGHWAYS		ANCH	ETA ORE	ININ D S(FS A	G DL	WAI DIE	LL NO. R PILE FTAILS	3 WA	LL
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GEOTECHNICAL ENGINEER