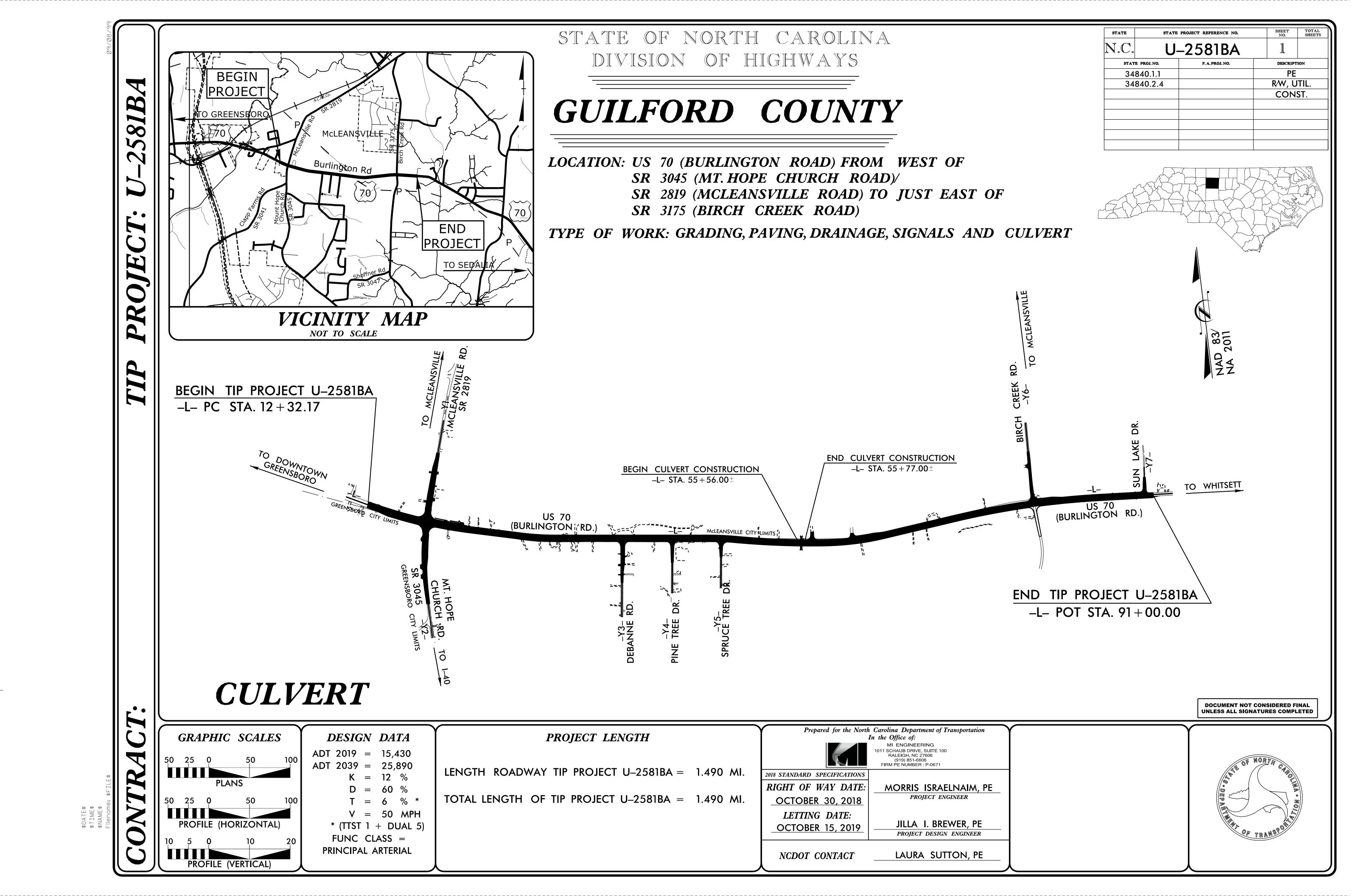
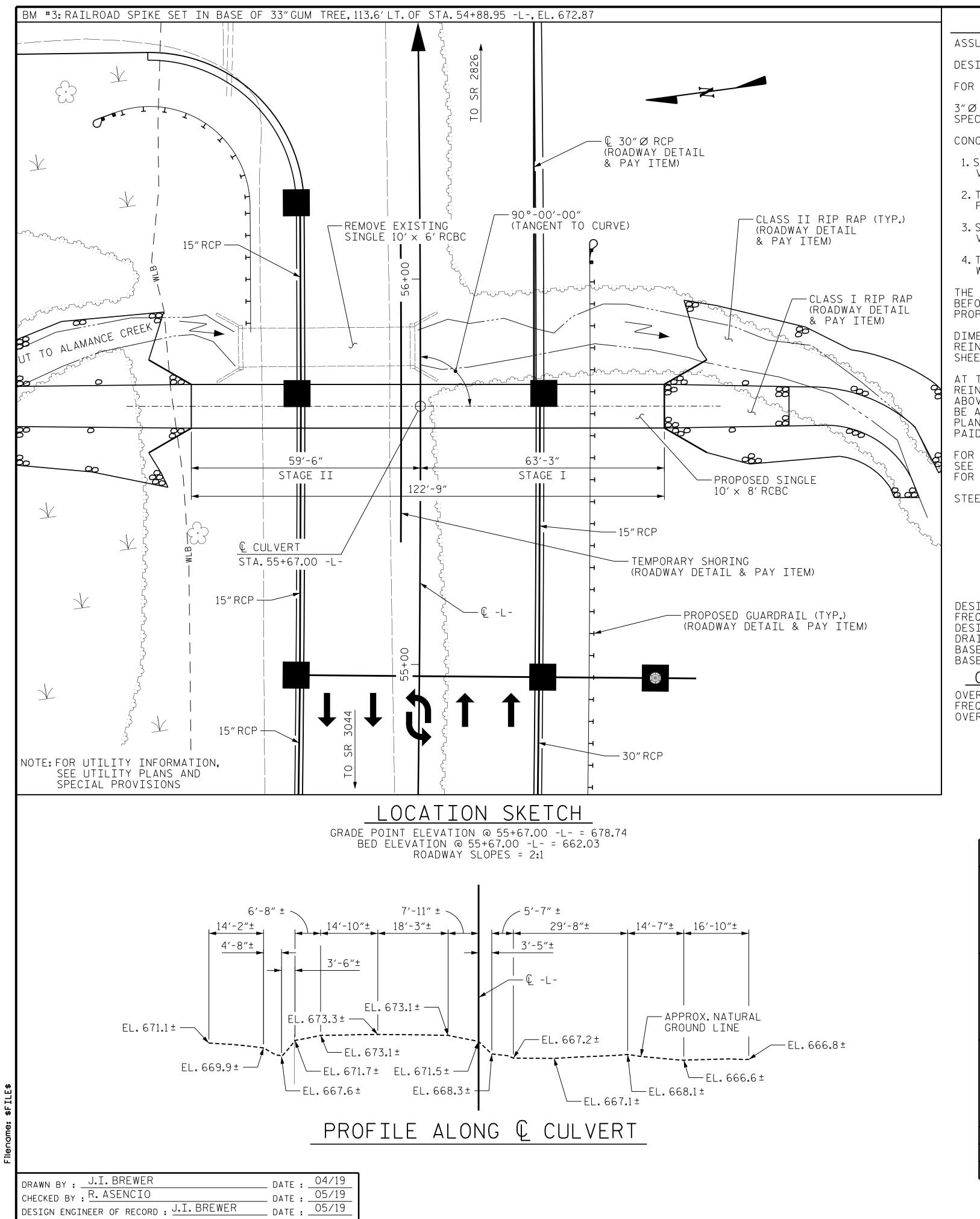
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ASSUMED LIVE LOAD ------HL-93 OR ALTERNATE LOADING.

DESIGN FILL------6.51 FT.

FOR OTHER DESIGN DATA AND NOTES, SEE STANDARD NOTES SHEET.

3" Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN CULVERTS TO BE POURED IN THE FOLLOWING ORDER:

- 1. STAGE I WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.
- 2. THE REMAINING PORTIONS OF STAGE I WALLS AND STAGE I WINGS FULL HEIGHT, FOLLOWED BY ROOF SLAB AND HEADWALL.
- 3. STAGE II WING FOOTINGS AND FLOOR SLAB INCLUDING 4"OF ALL VERTICAL WALLS.
- 4. THE REMAINING PORTIONS OF STAGE II WALLS AND STAGE II WINGS FULL HEIGHT, FOLLOWED BY ROOF SLAB AND HEADWALL.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WALL ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC. SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

STEEL SHEET PILING REQUIRED FOR SHORING SHALL BE HOT ROLLED.

NOTES

THE EXISTING STRUCTURE CONSISTING OF A SINGLE 10 FT X 6 FT RCBC LOCATED AT THE SITE OF THE PROPOSED STRUCTURE SHALL BE REMOVED.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION

EXCAVATE AT LEAST 1 FOOT BELOW BOTTOM OF CULVERT AND REPLACE WITH FOUNDATION CONDITIONING MATERIAL IN ACCORDANCE WITH ARTICLE 414-4 OF THE STANDARD SPECIFICATIONS.

FOR CULVERT DIVERSION DETAILS AND PAY ITEM. SEE EROSION CONTROL PLANS.

FOR CONSTRUCTION SEQUENCE, SEE EROSION CONTROL PLANS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITES, SEE SPECIAL PROVISIONS.

HYDRAULIC DATA

DESIGN DISCHARGE = 550 CFS FREQUENCY OF DESIGN FLOOD = 50 YRS. DESIGN HIGH WATER ELEVATION = 671.0 FT. DRAINAGE AREA = 0.64 SQ. MI. BASE DISCHARGE (Q100)..... 600 CFS BASE HIGH WATER ELEVATION 671.5 FT.

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE = 1325 CFS FREQUENCY OF OVERTOPPING FLOOD = 500+ YRS. OVERTOPPING FLOOD ELEVATION = 678.7 FT. *

-L- PROFILE DATA PVI STA.56+30.00 -L-

PVI EL. = 672.33 VC = 712.00 g1 = -3.0843% $g^2 = +4.3268\%$

TOTAL STRUCTU	RE QUANTITI	ES
CLASS A CONCRETE		
STAGE I	91.8	_C.Y.
STAGE II	86.7	_C.Y.
TOTAL	178.5	_C.Y.
REINFORCING STEEL		
STAGE I	10,839	_LBS.
STAGE II	10,122	_LBS.
TOTAL	20,961	_LBS.
FOUNDATION CONDITIONIN	NG MATERIAL	
STAGE I	69	TONS
STAGE II	65	TONS
TOTAL	17/	_TONS
CULVERT EXCAVATION	LUMF	SUM
REMOVAL OF EXISTING S	TRUCTURE LUM	P SUM
ASBESTOS ASSESSMENT	LUN	MP SUM

PROJECT NO. U-2581BA GUILFORD COUNTY 55+67.00 -L-STATION:

SHEET 1 OF 8

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

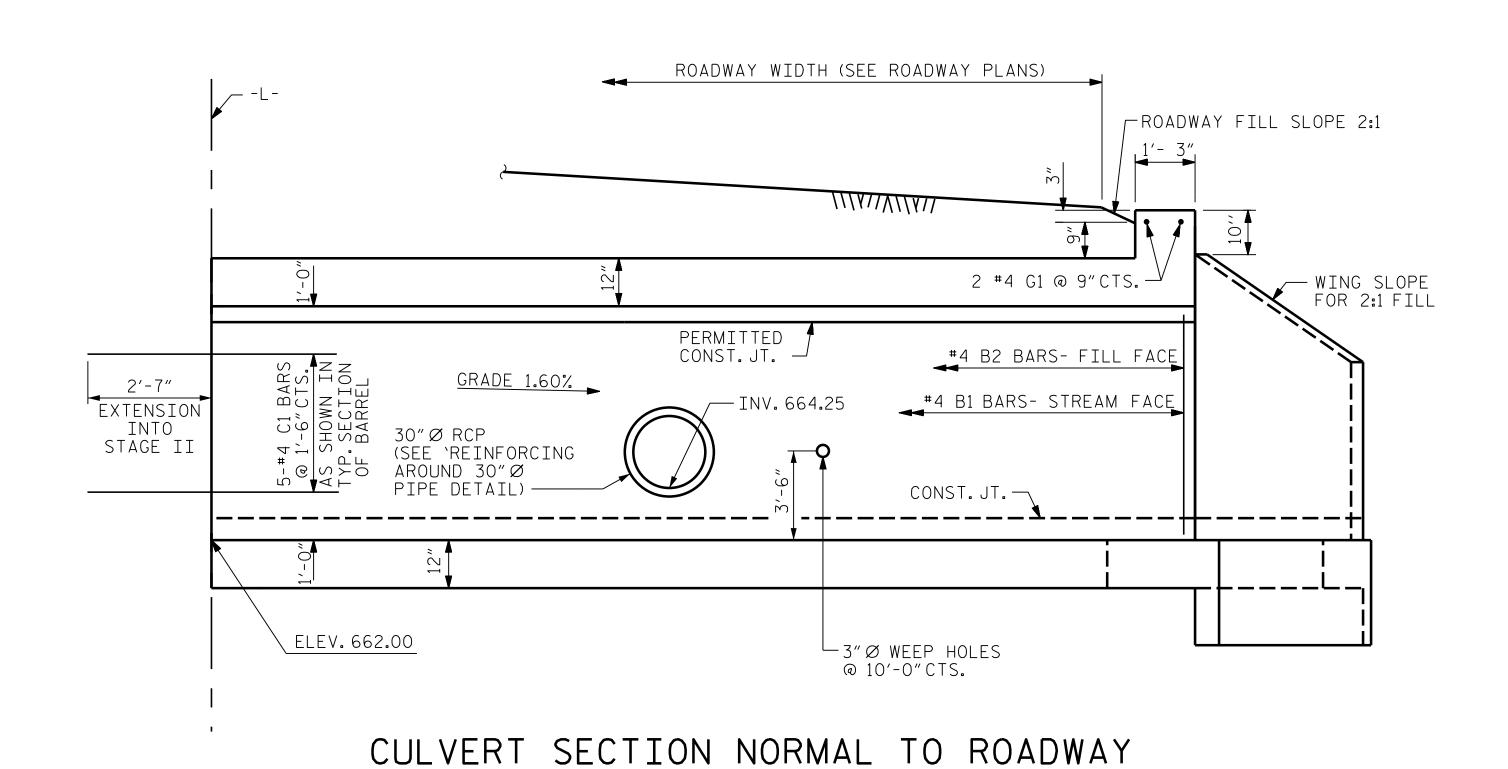
SINGLE IO FT. X 8 FT. CONCRETE BOX CULVERT STAGE I AND II 90° SKEW

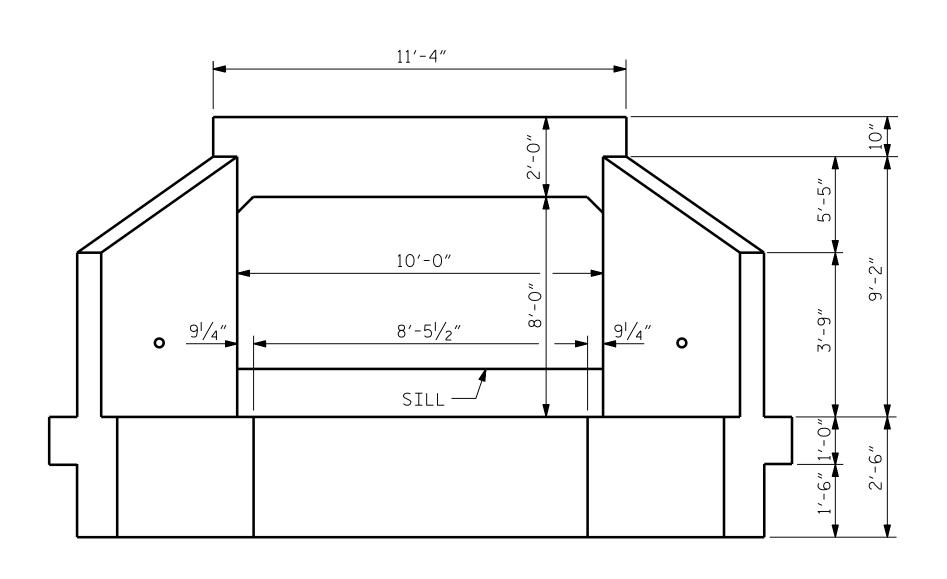


*OVERTOPS SAG AT STA.55+70.30 -L-

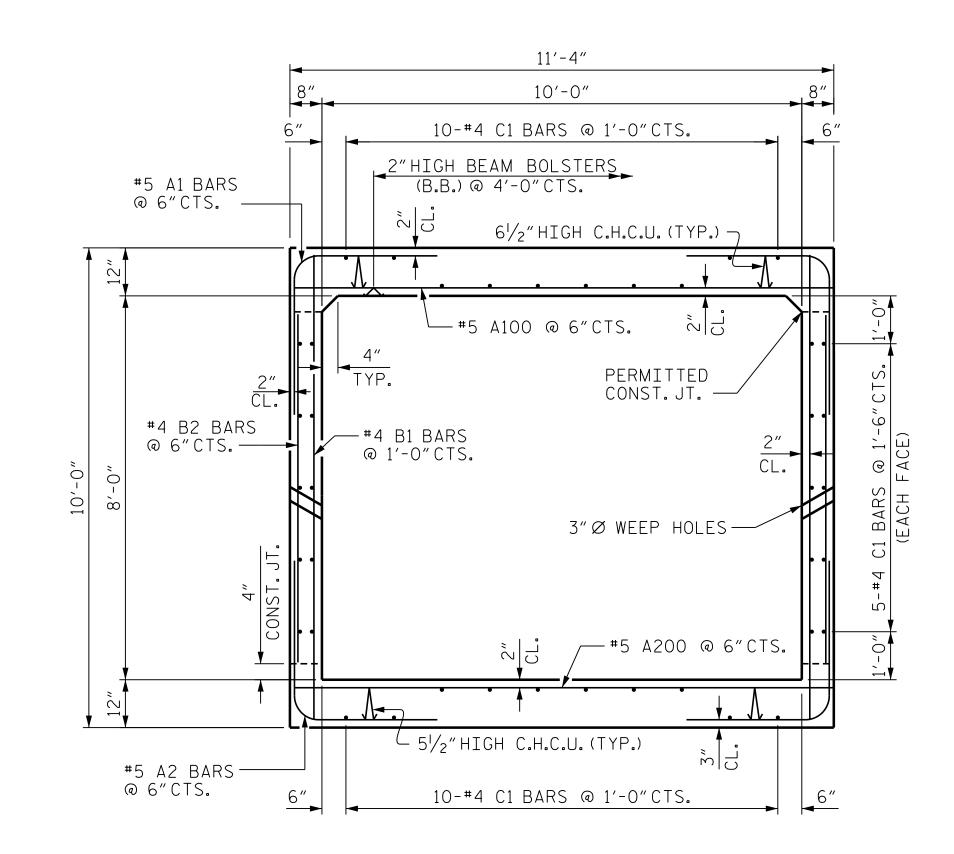
1011 SCHAUB DRIVE, SUITE 100 RALEIGH, NC 27606 (919) 851-6606

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	C-I
		જ			TOTAL SHEETS
		4			 8





OUTLET ELEVATION NORMAL TO SKEW



#5 E1 (TYP. EACH FACE)-30″Ø R.C. PIPE−

DETAIL OF REINFORCING AROUND 30" Ø PIPE

NOTE: 30" Ø PIPE THROUGH THE SIDEWALL OF THE CULVERT SHALL BE LOCATED BY THE ENGINEER. THE REINFORCING STEEL SHALL BE FIELD BENT AS NECESSARY TO CLEAR PIPE.

TOTAL STAGE I	QUANTITIES
CLASS A CONCRETE	
BARREL @ 1.24 CY/FT	78.4 C.Y.
WINGS, ETC.	12.2 C.Y.
SILLS	1.2 C.Y.
TOTAL	91.8 C.Y.
REINFORCING STEEL	
BARREL	10,118 LBS.
WINGS, ETC.	721 LBS.
TOTAL	10,839 LBS.
FOUNDATION CONDITIONING	MATERIAL 69 TONS
CULVERT EXCAVATION	LUMP SUM

PROJECT NO. U-2581BA GUILFORD _ COUNTY 55+67.00 -L-STATION:_

SHEET 2 OF 8

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

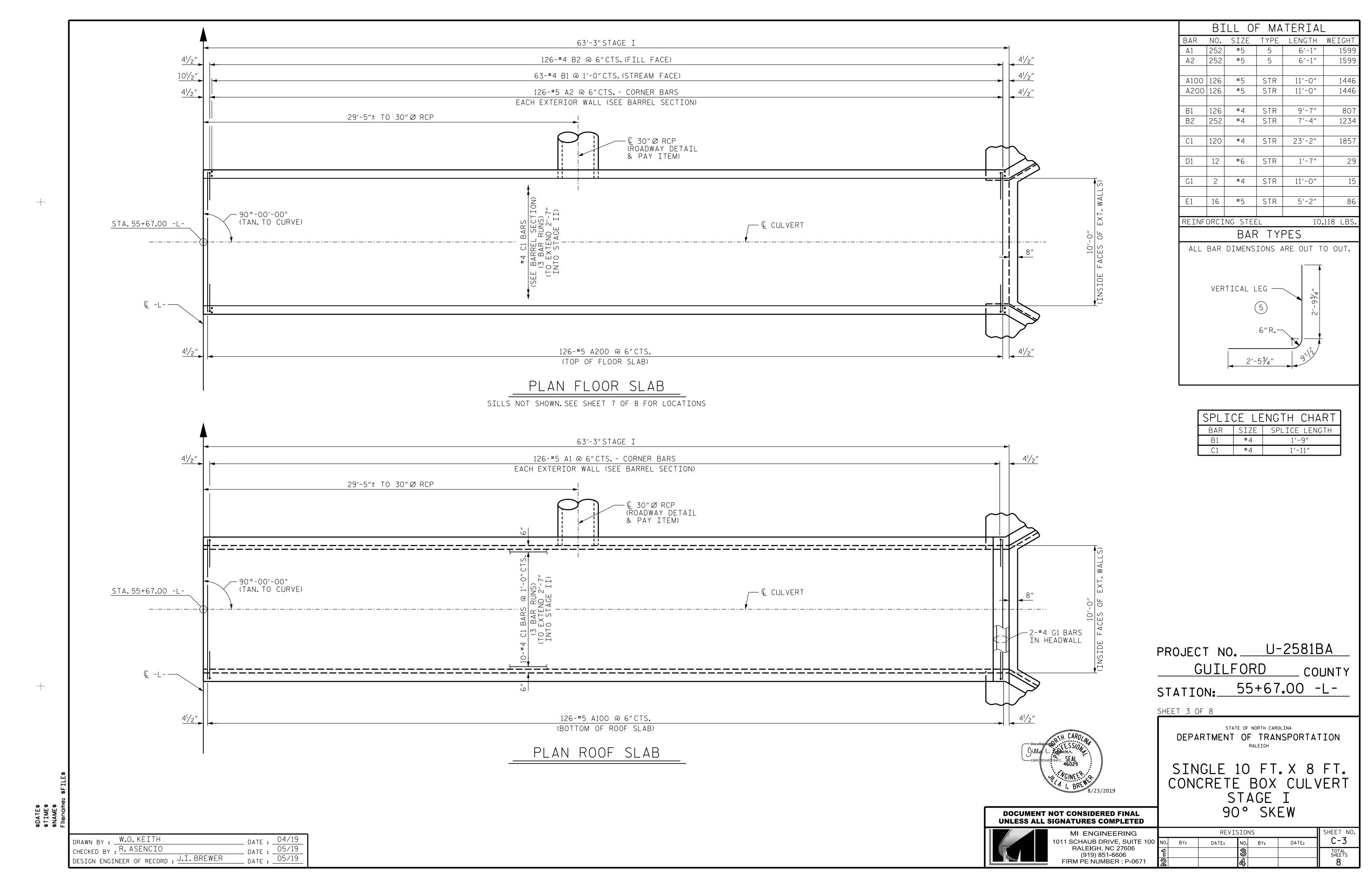
SINGLE 10 FT.X 8 FT. CONCRETE BOX CULVERT STAGE I 90° SKEW

C-2

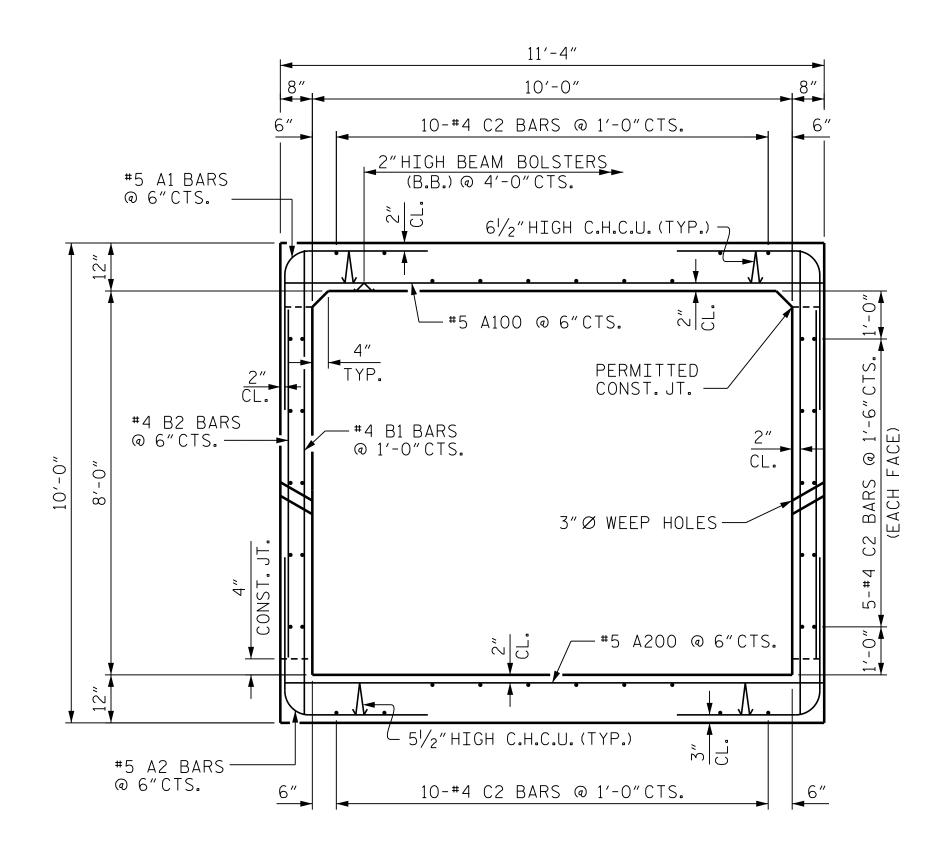
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED REVISIONS MI ENGINEERING 011 SCHAUB DRIVE, SUITE 100
RALEIGH, NC 27606
(919) 851-6606
FIRM PE NUMBER : P-0671 DATE: NO. BY: DATE:

RIGHT ANGLE SECTION OF BARREL THERE ARE 40 "C" BARS IN SECTION OF BARREL

DRAWN BY: _ W.O. KEITH	DATE :	04/19
CHECKED BY: R. ASENCIO	DATE :	05/19
DESIGN ENGINEER OF RECORD : J.I. BREWER	DATE :	05/19



INLET ELEVATION NORMAL TO SKEW



RIGHT ANGLE SECTION OF BARREL

THERE ARE 40 "C" BARS IN SECTION OF BARREL

ROADWAY WIDTH (SEE ROADWAY PLANS) ROADWAY FILL SLOPE 2:1-V 2 #4 G1 @ 9"CTS. WING SLOPE -FOR 2:1 FILL PERMITTED CONST. JT. #4 B2 BARS- FILL FACE GRADE 1.60% #4 B1 BARS- STREAM FACE CONST. JT. 3"Ø WEEP HOLES @ 10'-0"CTS. ELEV.662.00

CULVERT SECTION NORMAL TO ROADWAY

TOTAL STAGE II	QUANTITIES
CLASS A CONCRETE	
BARREL @ 1.24 CY/FT	73.7 C.Y.
WINGS, ETC.	12.2 C.Y.
SILLS	O.8 C.Y.
TOTAL	86.7 C.Y.
REINFORCING STEEL	
BARREL	9,401 LBS.
WINGS, ETC.	721 LBS.
TOTAL	10,122 LBS.
FOUNDATION CONDITIONING	MATERIAL 65 TONS
CULVERT EXCAVATION	LUMP SUM

PROJECT NO. U-2581BA GUILFORD _ COUNTY 55+67.00 -L-STATION:__

SHEET 4 OF 8

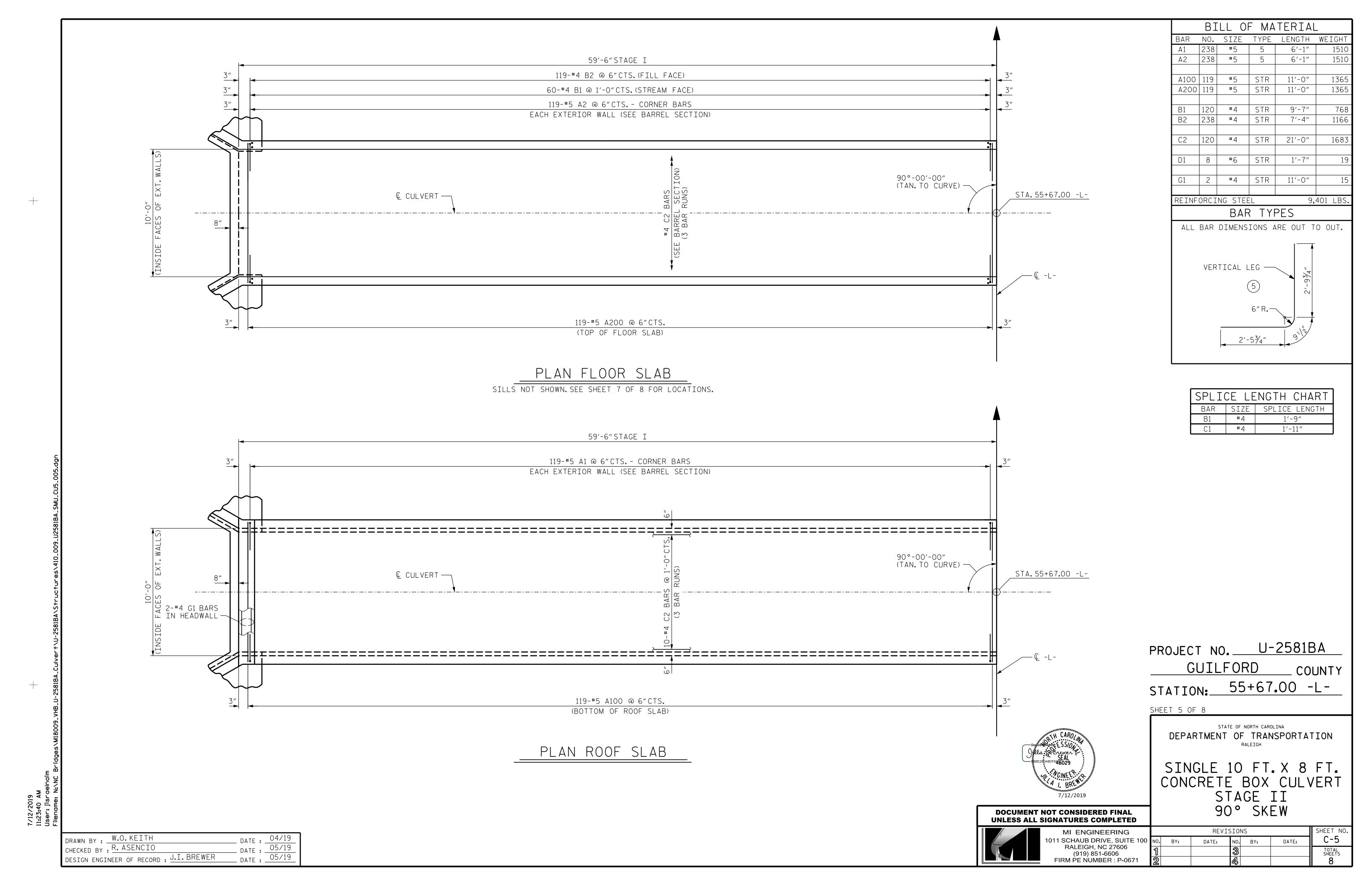
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

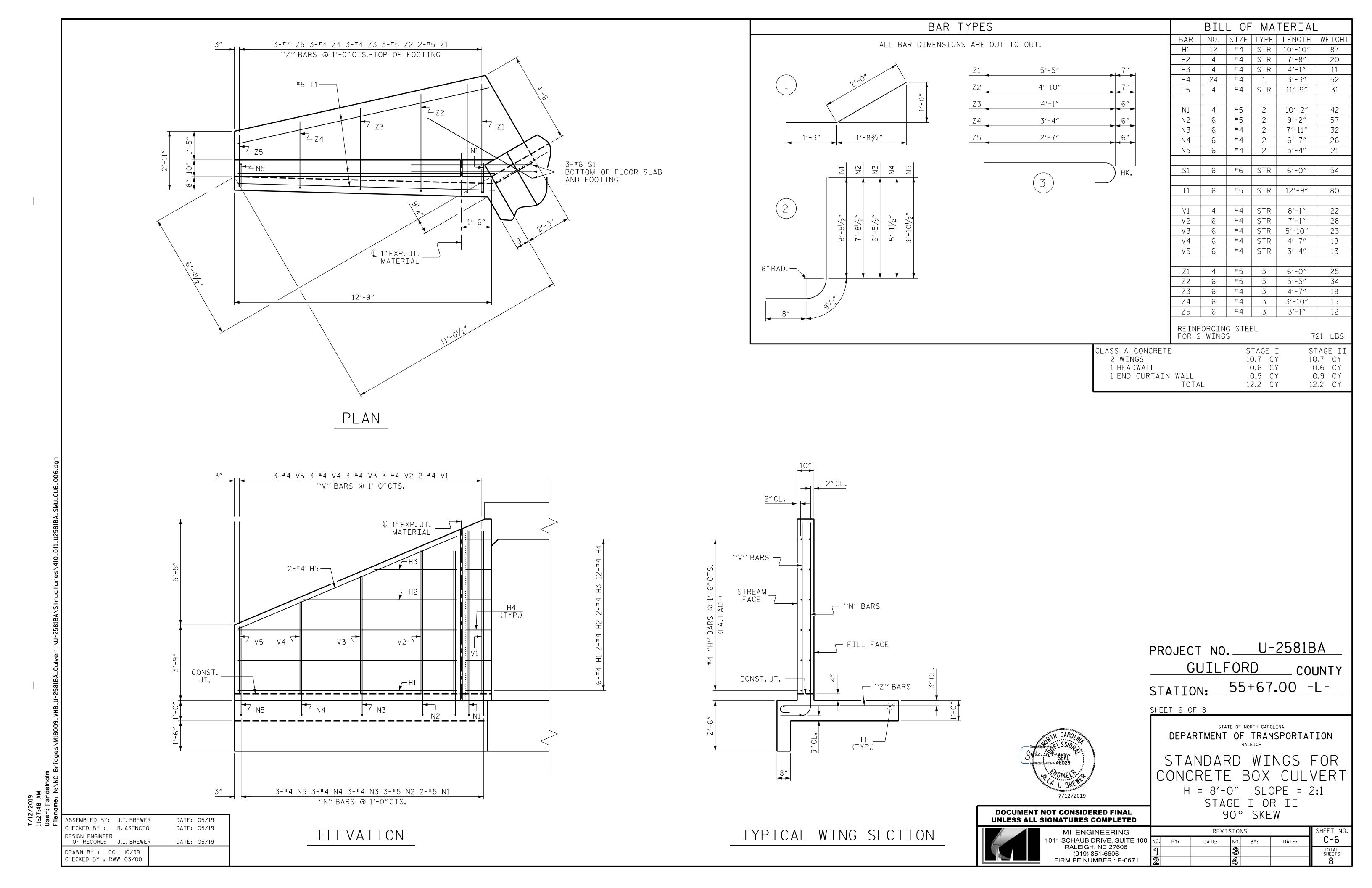
SINGLE 10 FT.X 8 FT. CONCRETE BOX CULVERT STAGE II 90° SKEW

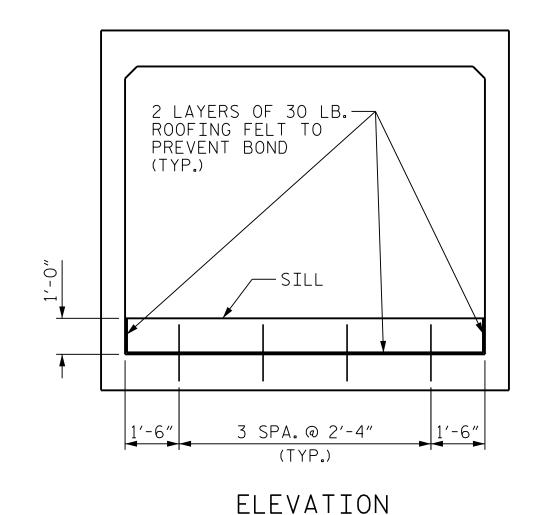
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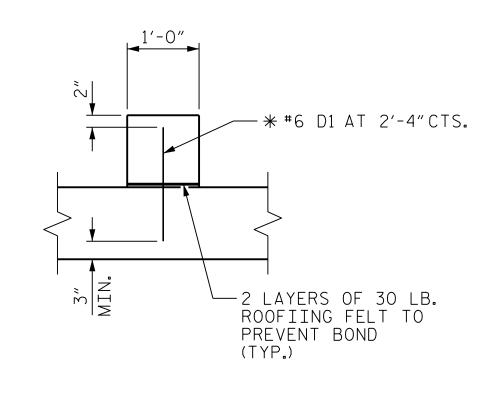
REVISIONS 1011 SCHAUB DRIVE, SUITE 100 NO. BY:
RALEIGH, NC 27606
(919) 851-6606
FIRM PE NUMBER : P-0671 C-4 DATE: NO. BY: DATE:

DRAWN BY: W.O. KEITH CHECKED BY: R. ASENCIO DATE: 04/19
DATE: 05/19
DATE: 05/19 DESIGN ENGINEER OF RECORD : J.I. BREWER









SECTION THROUGH SILL * DOWELS MAY BE PUSHED INTO GREEN CONCRETE AFTER SLAB HAS BEEN FLOAT FINISHED.

CULVERT SILL DETAILS

(LOOKING DOWNSTREAM)

59'-6"(STAGE II) 63'-3"(STAGE I) -1'-0" WIDE X 1'-0" HIGH SILLS (TYP.) 29′-81/4″ 29'-8¹/₄" 27′-9¾″ 29'-8[|]/₄"

PLAN VIEW SHOWING SILL LOCATIONS

PROJECT NO. U-2581BA GUILFORD STATION: 55+67.00 -L-

SHEET 7 OF 8

NOTES

THE LOW FLOW CHANNEL. BED MATERIAL SHALL BE SUBJECT TO APPROVAL BY THE

ENGINEER, AND MAY BE SUBJECT TO PERMIT CONDITIONS.

PRICE BID FOR CULVERT EXCAVATION.

THE VARIOUS PAY ITEMS.

FLOW BARREL.

ELEVATION.

MATERIAL EXCAVATED FROM THE EXISTING BED SHALL BE STOCKPILED FOR USE IN THE

PROPOSED CULVERT AND SHALL PROVIDE A CONTINUOUS LOW FLOW CHANNEL AS SHOWN.
THE MATERIAL SHALL BE NATURAL STONE WITH A GRADATION SIZE SIMILAR TO THAT
OF CLASS B RIP RAP. STONES LARGER THAN 6 INCHES SHALL NOT BE PLACED WITHIN

THE STOCKPILED MATERIAL SHALL BE PLACED TO PROVIDE A DEPTH OF 1 FOOT IN LOW

THE TOP OF BED MATERIAL IN THE LOW FLOW BARREL SHOULD MATCH THE STREAM BED

THE ENTIRE COST OF THE WORK REQUIRED TO PLACE EXCAVATED OR SUPPLEMENTAL MATERIAL AS SHOWN ON THE PLANS SHALL BE INCLUDED IN THE CONTRACT LUMP SUM

THE ENTIRE COST OF WORK REQUIRED TO CONSTRUCT THE SILLS SHALL BE INCLUDED IN

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SINGLE 10 FT. X 8 FT. CONCRETE BOX CULVERT STAGE I OR II 90° SKEW

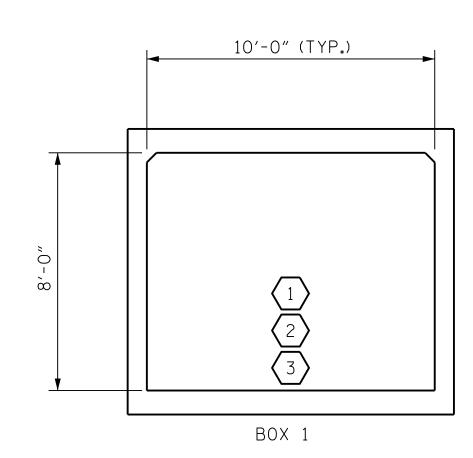
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

111 SCHAUB DRIVE, SUITE 100 RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER : P-0671

REVISIONS BY: DATE: NO. BY: DATE:

DRAWN BY: J.I. BREWER CHECKED BY: R. ASENCIO DESIGN ENGINEER OF RECORD : J.I. BREWER

							STRENGTH I LIMIT STATE									
										MOMENT				SHEAR		1
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (++)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (++)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	2.10		1.75	2.10	1	BOTTOM SLAB	5.67	5.03	1	BOTTOM SLAB	1.00	
DESIGN LOAD		HL-93 (OPERATING)	N/A		2.26		1.35	2.26	1	BOTTOM SLAB	5.67	6.52	1	BOTTOM SLAB	1.00	<u> </u>
RATING		HS-20 (INVENTORY)	36.000	2	2.10	75.60	1.75	2.10	1	BOTTOM SLAB	5.67	5.03	1	BOTTOM SLAB	1.00	<u> </u>
		HS-20 (OPERATING)	36.000		2.26	81.36	1.35	2.26	1	BOTTOM SLAB	5.67	6.52	1	BOTTOM SLAB	1.00	<u></u>
	ICLE	SNSH	13.500		3.83	51.71	1.40	3.83	1	EXTERIOR WALL	5.00	9.97	1	EXTERIOR WALL	1.00	1
		SNGARBS2	20.000		2.71	54.20	1.40	2.71	1	EXTERIOR WALL	5.00	9.96	1	EXTERIOR WALL	1.00	1
		SNAGRIS2	22.000		2.71	59.62	1.40	2.71	1	EXTERIOR WALL	5.00	9.96	1	EXTERIOR WALL	1.00	1
	VEH (SNCOTTS3	27.250	3	2.62	71.40	1.40	2.62	1	BOTTOM SLAB	5.67	6.20	1	BOTTOM SLAB	1.00	<u> </u>
	SLE (S	SNAGGRS4	34.925		2.71	94.65	1.40	2.71	1	EXTERIOR WALL	5.00	6.20	1	BOTTOM SLAB	1.00	1
	SINGL	SNS5A	35.550		2.71	96.34	1.40	2.71	1	EXTERIOR WALL	5.00	6.20	1	BOTTOM SLAB	1.00	1
		SNS6A	39.950		2.71	108.26	1.40	2.71	1	EXTERIOR WALL	5.00	6.20	1	BOTTOM SLAB	1.00	1
LEGAL LOAD		SNS7B	42.000		2.71	113.82	1.40	2.71	1	EXTERIOR WALL	5.00	6.20	1	BOTTOM SLAB	1.00	1
RATING	LER	TNAGRIT3	33.000		2.71	89.43	1.40	2.71	1	EXTERIOR WALL	5.00	9.82	1	EXTERIOR WALL	1.00	1
	TRAI	TNT4A	33.075		2.71	89.63	1.40	2.71	1	EXTERIOR WALL	5.00	7.38	1	BOTTOM SLAB	1.00	1
	SEMI-T	TNT6A	41.600		2.71	112.74	1.40	2.71	1	EXTERIOR WALL	5.00	6.69	1	BOTTOM SLAB	1.00	1
	(/)	TNT7A	42.000		2.71	113.82	1.40	2.71	1	EXTERIOR WALL	5.00	6.69	1	BOTTOM SLAB	1.00	1
	CTOR (TT	TNT7B	42.000		2.71	113.82	1.40	2.71	1	EXTERIOR WALL	5.00	6.67	1	BOTTOM SLAB	1.00	1
	TRAC	TNAGRIT4	43.000		2.71	116.53	1.40	2.71	1	EXTERIOR WALL	5.00	6.67	1	BOTTOM SLAB	1.00	1
	TRUCK	TNAGT5A	45.000		2.71	121.95	1.40	2.71	1	EXTERIOR WALL	5.00	6.67	1	BOTTOM SLAB	1.00	1
	TRI	TNAGT5B	45.000		2.71	121.95	1.40	2.71	1	EXTERIOR WALL	5.00	6.67	1	BOTTOM SLAB	1.00	1



LRFR SUMMARY (LOOKING DOWNSTREAM)

SSEMBLED BY: J.I. BREWER DATE: 05/19 DATE: 05/19 CHECKED BY: R. ASENCIO DATE: 05/19 DRAWN BY : WMC 7/II CHECKED BY : GM 7/II

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	
WA	1.00	

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE. THE LIVE LOAD RATING FOR VEHICLES ON -L- WERE COMPUTED WITH A DESIGN FILL DEPTH OF 6.51 FT.

COMMENTS:

1. VERTICAL ELEMENTS ARE REFERENCED STARTING AT THE BOTTOM.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

PROJECT NO. U-2581BA GUILFORD ___ COUNTY STATION: 55+67.00 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> > STANDARD

LRFR SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

UNLESS ALL SIGNATURES COMPLETED



1011 SCHAUB DRIVE, SUITE 100 NO. BY:
RALEIGH, NC 27606
(919) 851-6606
FIRM PE NUMBER : P-0671

(NON-INTERSTATE TRAFFIC) REVISIONS DATE: NO. BY: DATE:

SHEET 8 OF 8

DOCUMENT NOT CONSIDERED FINAL

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $\frac{1}{2}$ RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A $\frac{1}{4}$ FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT,

ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST \(\frac{5}{6}'' \) IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY //16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

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