

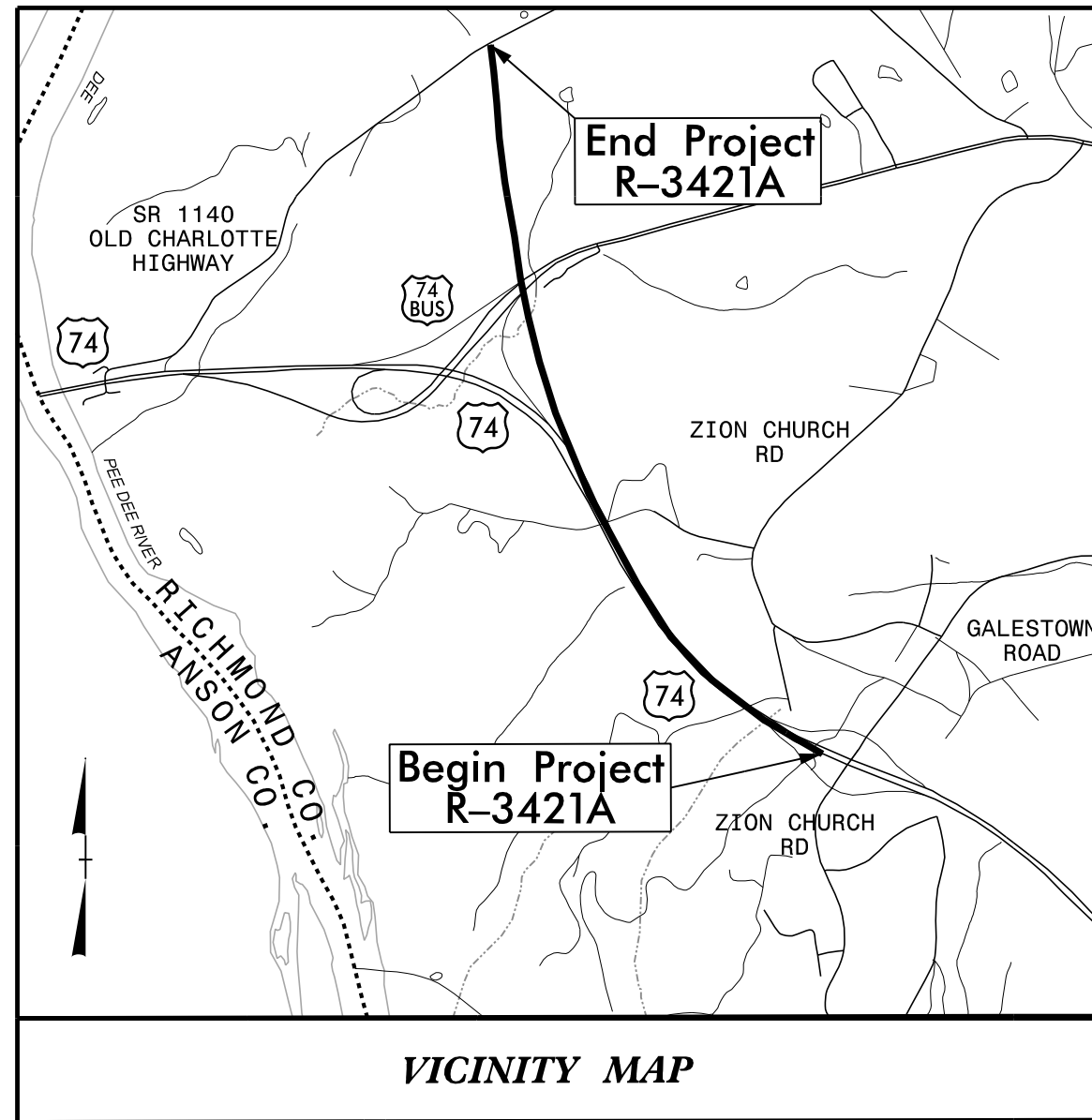
09/08/09

See Sheet 1-A For Index of Sheets
See Sheet 1-B For Conventional Symbols

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PLAN FOR PROPOSED HIGHWAY EROSION CONTROL

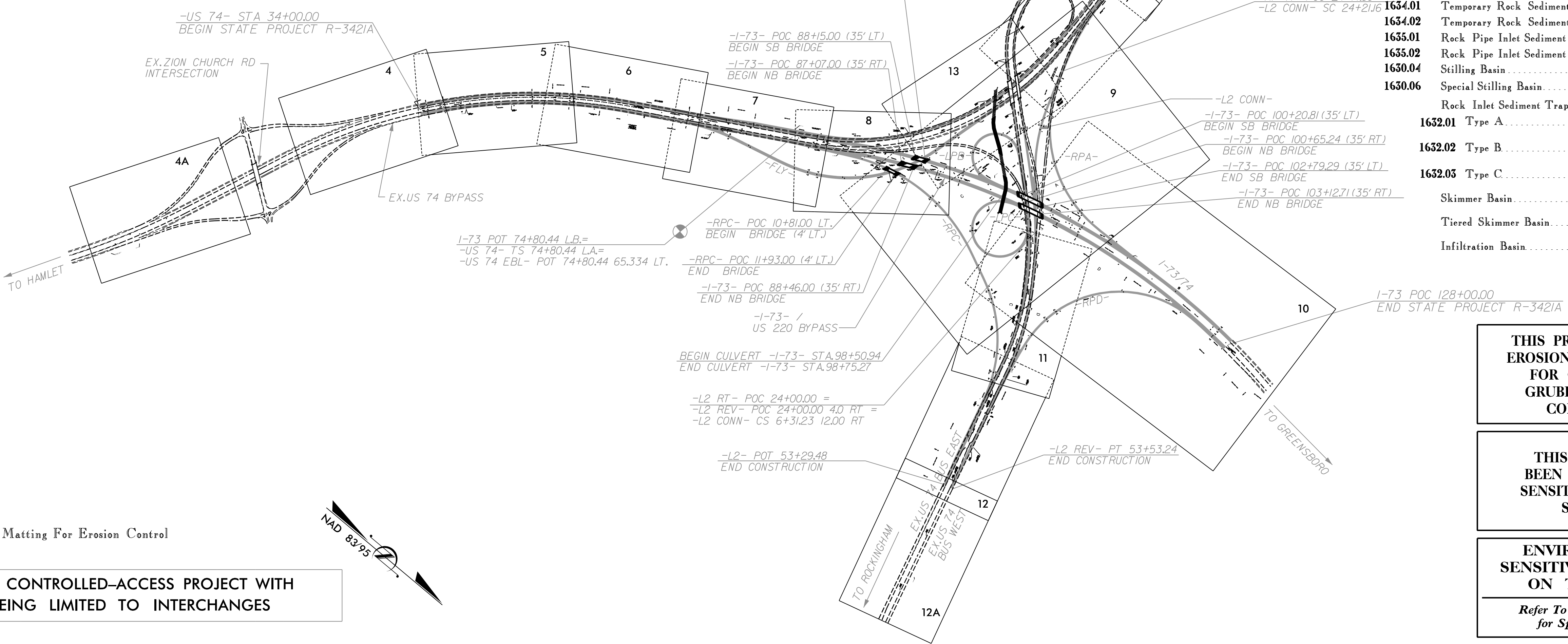
TIP PROJECT: R-3421A



RICHMOND COUNTY

LOCATION: US I-73/74 BYPASS FROM US 74 BYPASS WEST OF ROCKINGHAM AT SR 1109 (ZION CHURCH RD.) INTERCHANGE TO 0.3 MILES SOUTH OF SR 1140 (OLD CHARLOTTE HWY.)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, SIGNING, STRUCTURES, CULVERT, & RETAINING WALLS



Matting For Erosion Control

THIS IS A CONTROLLED-ACCESS PROJECT WITH ACCESS BEING LIMITED TO INTERCHANGES

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-3421A	EC-1	
STATE PROJ.NO.	F.A.PROJ.NO.	DESCRIPTION	
34542.1.FR4	HPPF-0220(75)	P.E.	
34542.2.4	HPPF-0220(30)	R/W, UTIL.	
34542.3.FS4	HPPF-0220(30)	P.E.	
34542.3.6	HPPF-0220(30)	CONST.	

EROSION AND SEDIMENT CONTROL MEASURES

Std. #	Description	Symbol
1630.03	Temporary Silt Ditch	
1630.05	Temporary Diversion	
1605.01	Temporary Silt Fence	
1606.01	Special Sediment Control Fence	
1622.01	Temporary Berms and Slope Drains	
1630.02	Silt Basin Type B	
1633.01	Temporary Rock Silt Check Type-A	
	Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM)	
1633.02	Temporary Rock Silt Check Type-B	
	Wattle/Coir Fiber Wattle	
	Wattle/Coir Fiber Wattle with Polyacrylamide (PAM)	
1634.01	Temporary Rock Sediment Dam Type-A	
1634.02	Temporary Rock Sediment Dam Type-B	
1635.01	Rock Pipe Inlet Sediment Trap Type-A	
1635.02	Rock Pipe Inlet Sediment Trap Type-B	
1630.04	Stilling Basin	
1630.06	Special Stilling Basin	
	Rock Inlet Sediment Trap:	
1632.01	Type A	
1632.02	Type B	
1632.03	Type C	
	Skimmer Basin	
	Tiered Skimmer Basin	
	Infiltration Basin	

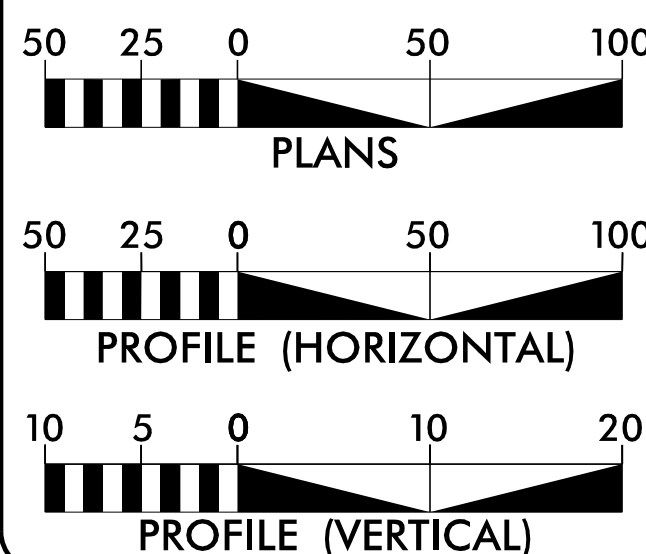
THIS PROJECT CONTAINS EROSION CONTROL PLANS FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION.

THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATERSHED STANDARDS.

ENVIRONMENTALLY SENSITIVE AREA(S) EXIST ON THIS PROJECT

Refer To E. C. Special Provisions for Special Considerations.

GRAPHIC SCALES



THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE APRIL 1, 2019 ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER QUALITY.

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT R-3421A = 1.707 miles
LENGTH STRUCTURES TIP PROJECT R-3421A = 0.073 miles
TOTAL LENGTH OF TIP PROJECT R-3421A = 1.780 miles
(SB LANES WERE USED FOR LENGTH OF PROJECT)

Reviewed In the Office of:

ROADSIDE ENVIRONMENTAL UNIT

1 South Wilmington St.
Raleigh, NC 27611

Prepared in the Office of:
RK&K
FOR NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

2012 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
JUNE 20, 2008

LETTING DATE:
OCTOBER 15, 2019

NCDOT CONTACT:

MICHAEL T. MERRITT, PE
PROJECT ENGINEER
RK&K, LLP
SCOTT BLEVINS, P.E.
PROJECT DESIGN ENGINEER
RK&K, LLP
ROBERT B. HUSKEY, PE
EROSION CONTROL DESIGN ENGINEER NO.4943
RK&K, LLP

GREGORY S. DAVIS, P.E.
DIVISION 8 PROJECT ENGINEER

Roadway Standard Drawings

The following roadway english standards as appear in "Roadway Standard Drawings"- Roadway Design Unit - N. C. Department of Transportation - Raleigh, N. C., dated January 2012 and the latest revision thereto are applicable to this project and by reference hereby are considered a part of these plans.

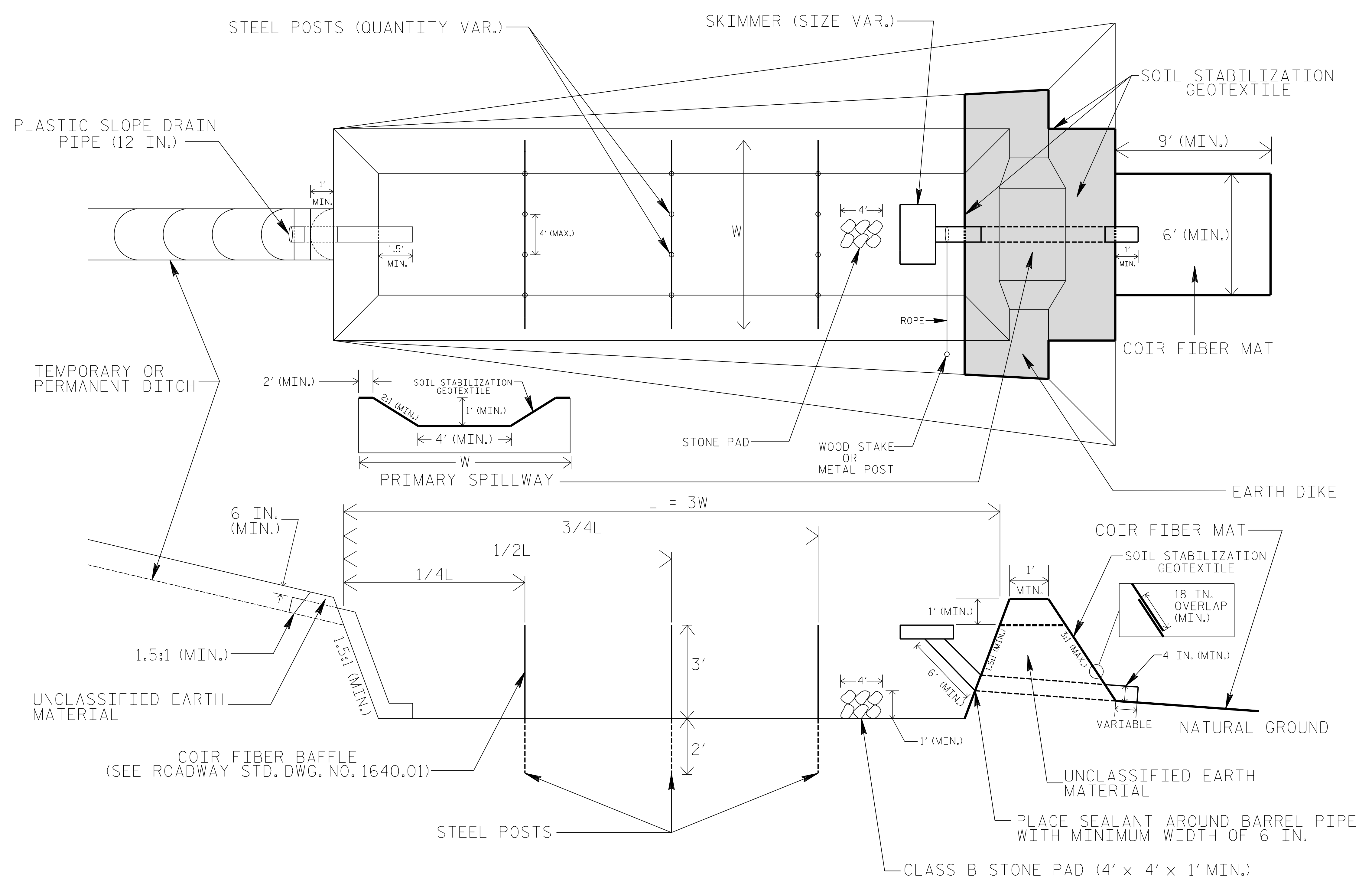
1604.01	Railroad Erosion Control Detail	1632.01	Rock Inlet Sediment Trap Type A
1605.01	Temporary Silt Fence	1632.02	Rock Inlet Sediment Trap Type B
1606.01	Special Sediment Control Fence	1632.03	Rock Inlet Sediment Trap Type C
1607.01	Gravel Construction Entrance	1633.01	Temporary Rock Silt Check Type A
1622.01	Temporary Berms and Slope Drains	1633.02	Temporary Rock Silt Check Type B
1630.01	Riser Basin	1634.01	Temporary Rock Sediment Dam Type A
1630.02	Silt Basin Type B	1634.02	Temporary Rock Sediment Dam Type B
1630.03	Temporary Silt Ditch	1635.01	Rock Pipe Inlet Sediment Trap Type A
1630.04	Stilling Basin	1635.02	Rock Pipe Inlet Sediment Trap Type B
1630.05	Temporary Diversion	1640.01	Coir Fiber Baffle
1630.06	Special Stilling Basin	1645.01	Temporary Stream Crossing
1631.01	Matting Installation		

7/1/2019 R:\Hydraulics\CADD\PSH\Erosion Control\R3421a_EC_tsh.dgn cplber

CONTRACT: C204368

PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-2
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

SKIMMER BASIN WITH BAFFLES DETAIL



NOTES

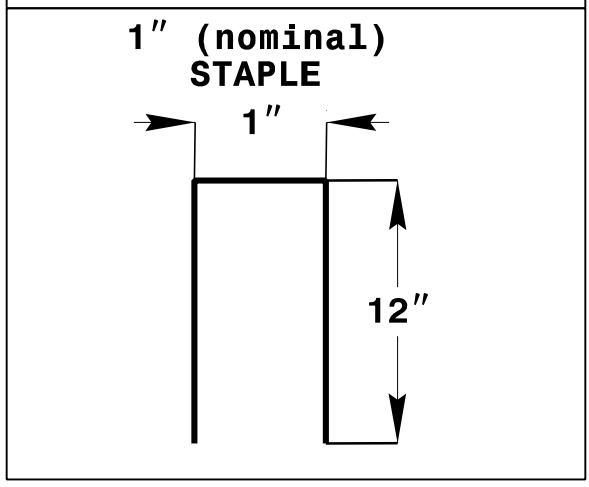
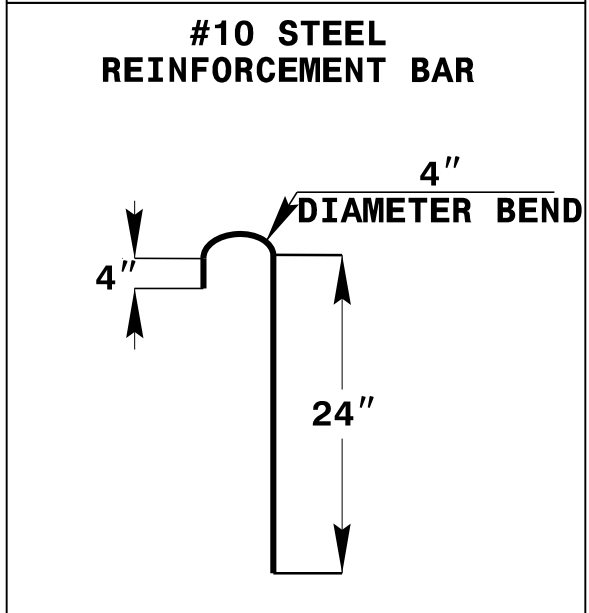
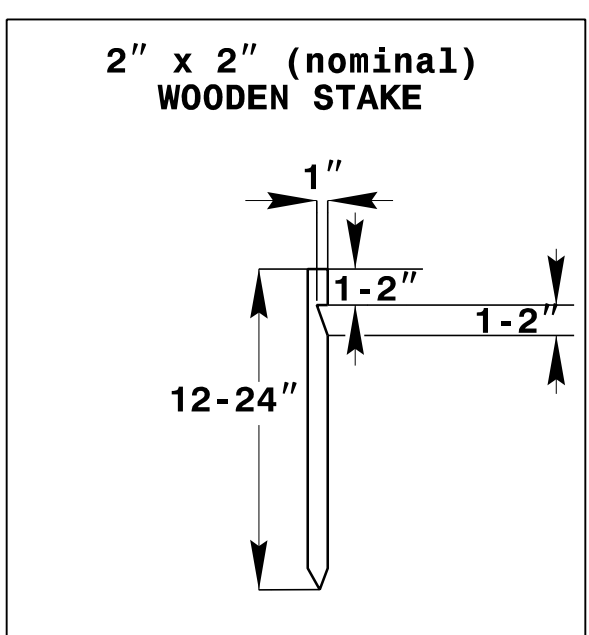
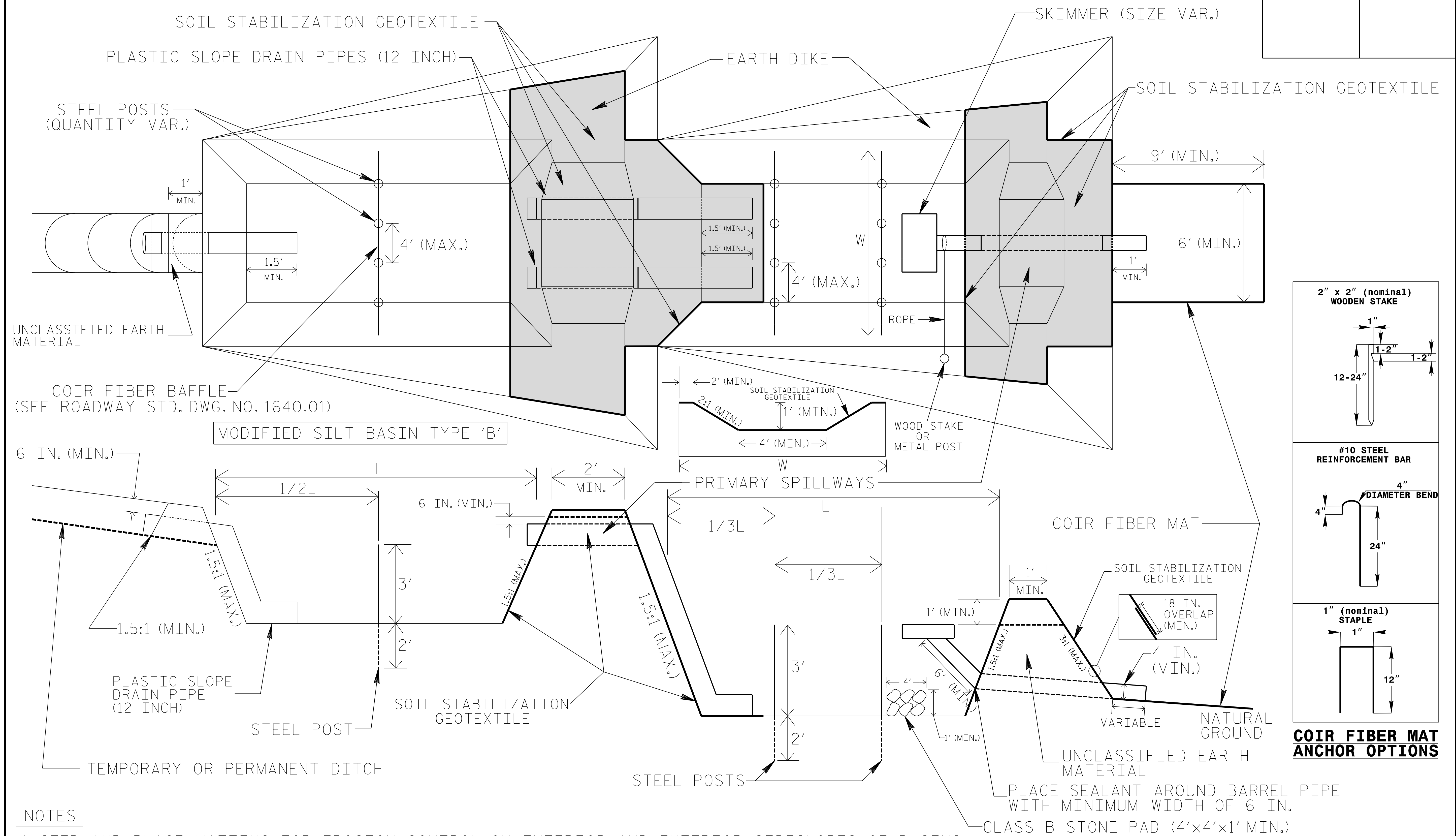
1. SEED AND PLACE MATTING FOR EROSION CONTROL ON INTERIOR AND EXTERIOR SIDESLOPES.
2. LIMIT EARTH DIKE HEIGHT TO 5 FT.
3. FOR BASIN DEPTH OF 3 FT., THE MINIMUM BASIN WIDTH SHALL BE 9 FT.
4. DETERMINE PRIMARY SPILLWAY WEIR LENGTH (FT.) USING $Q/0.4$, WHERE Q IS FLOW RATE (CFS) INTO BASIN.
5. PLASTIC SLOPE DRAIN PIPE AT INLET OF BASIN MAY BE REPLACED BY FILTRATION GEOTEXTILE OR TARP AS DIRECTED.
6. SOIL STABILIZATION GEOTEXTILE FOR PRIMARY SPILLWAY SHALL BE ONE CONTINUOUS PIECE OF MATERIAL OR OVERLAPPED 18 IN. (MIN.).

NOT TO SCALE

COIR FIBER MAT ANCHOR OPTIONS

TIERED SKIMMER BASIN DETAIL

PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-2A
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



COIR FIBER MAT ANCHOR OPTIONS

NOTES

1. SEED AND PLACE MATTING FOR EROSION CONTROL ON INTERIOR AND EXTERIOR SIDESLOPES OF BASINS.
2. LIMIT HEIGHT OF EARTH DIKES TO 5 FT.
3. ADDITIONAL MODIFIED SILT BASINS TYPE 'B' MAY BE NEEDED DEPENDING ON SLOPE.
4. FOR BASIN DEPTHS OF 3FT., THE MINIMUM BASIN WIDTHS SHALL BE 9 FT.
5. DETERMINE PRIMARY SPILLWAY LENGTHS (FT.) USING $Q/0.4$, WHERE Q IS FLOW RATE (CFS) INTO UPPER BASIN.
6. SOIL STABILIZATION GEOTEXTILE FOR PRIMARY SPILLWAYS SHALL BE ONE CONTINUOUS PIECE OF MATERIAL OR OVERLAPPED 18 IN. (MIN.).

NOT TO SCALE

PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-2B
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

COIR FIBER WATTLE WITH POLYACRYLAMIDE (PAM) DETAIL

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

ONLY INSTALL WATTLE(S) TO A HEIGHT IN DITCH SO FLOW WILL NOT WASH AROUND WATTLE AND SCOUR DITCH SLOPES AND AS DIRECTED.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.

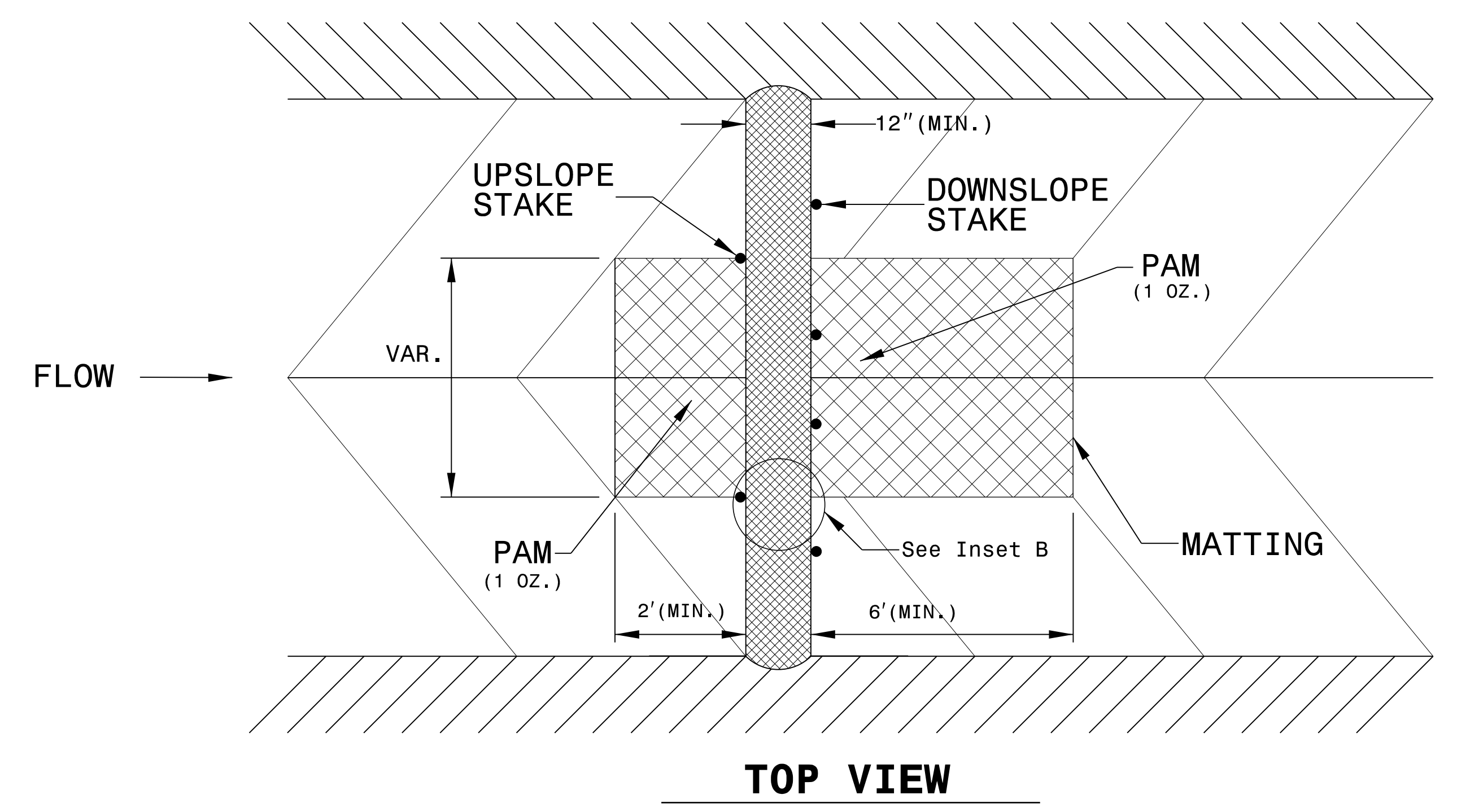
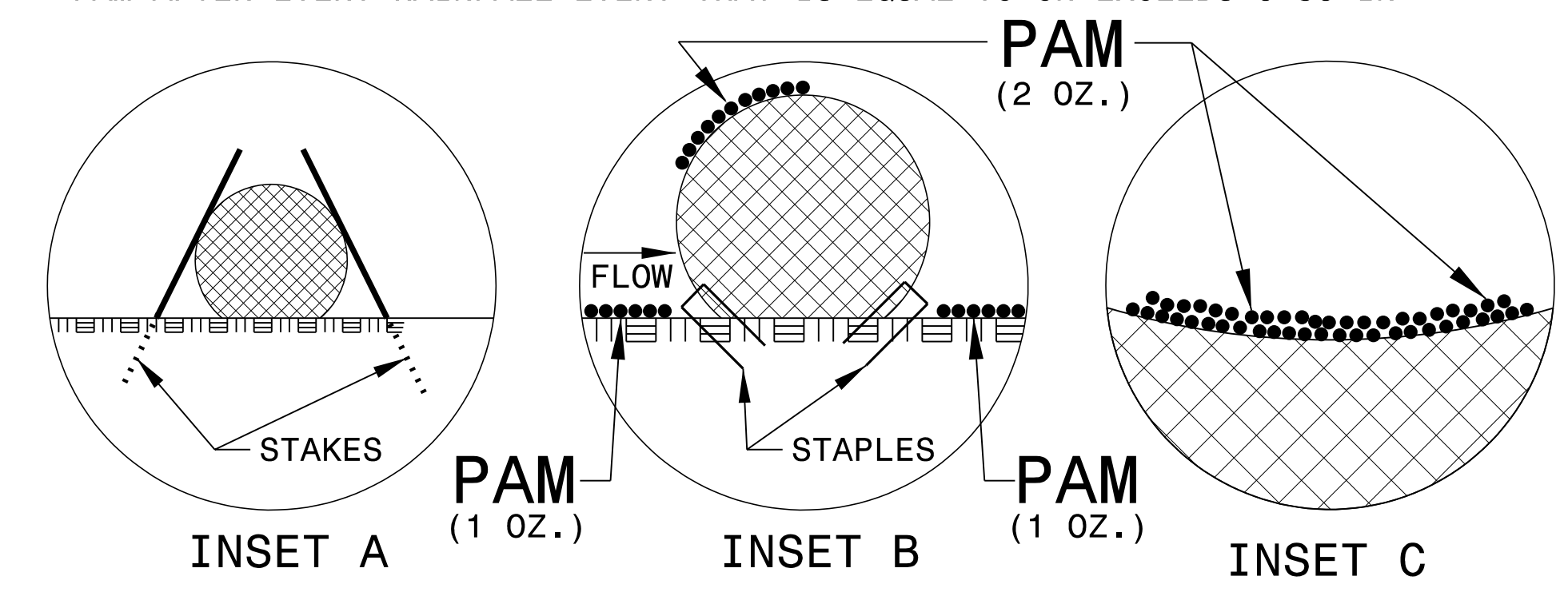
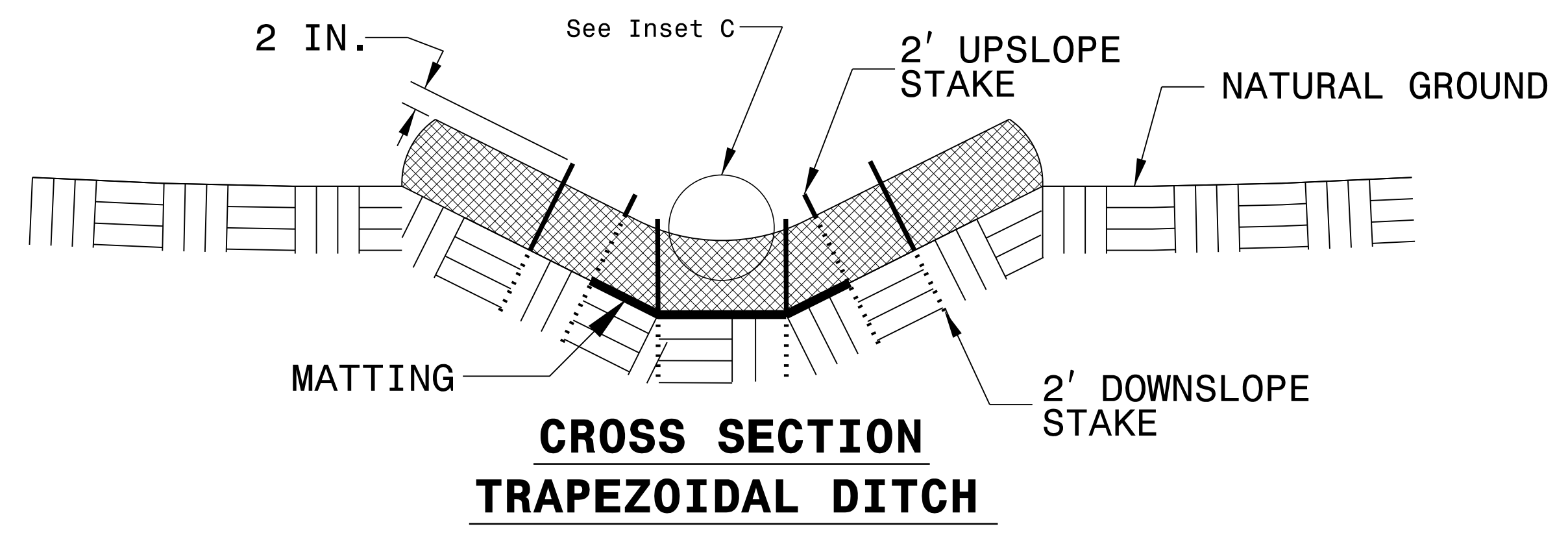
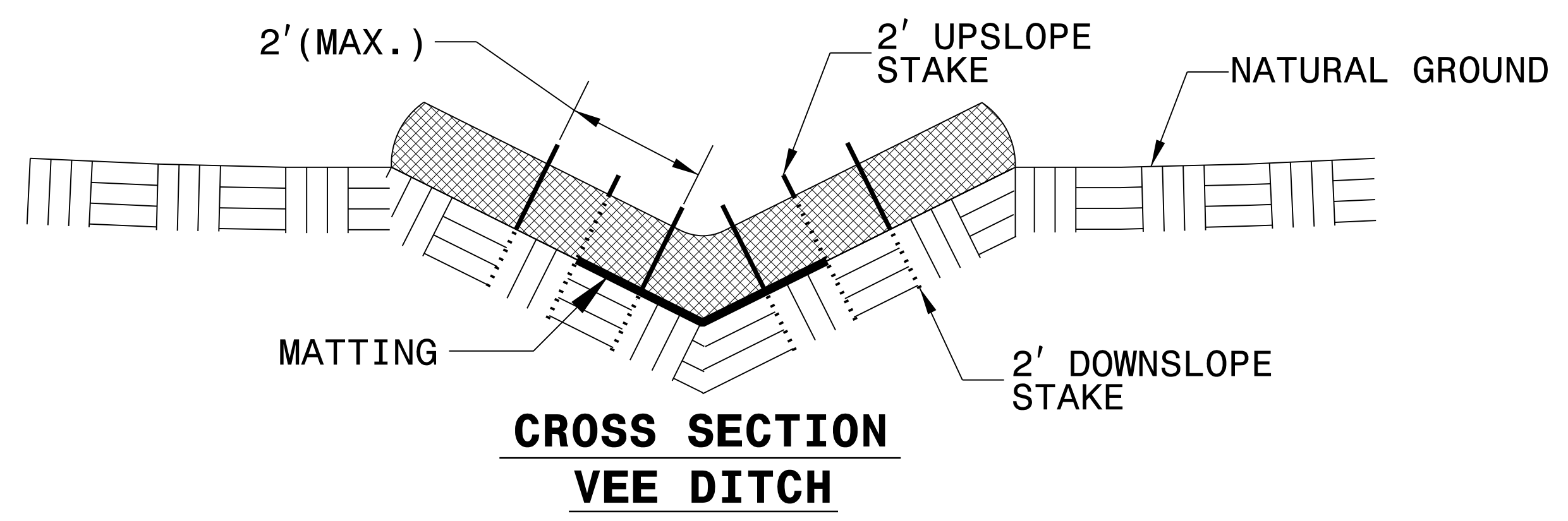
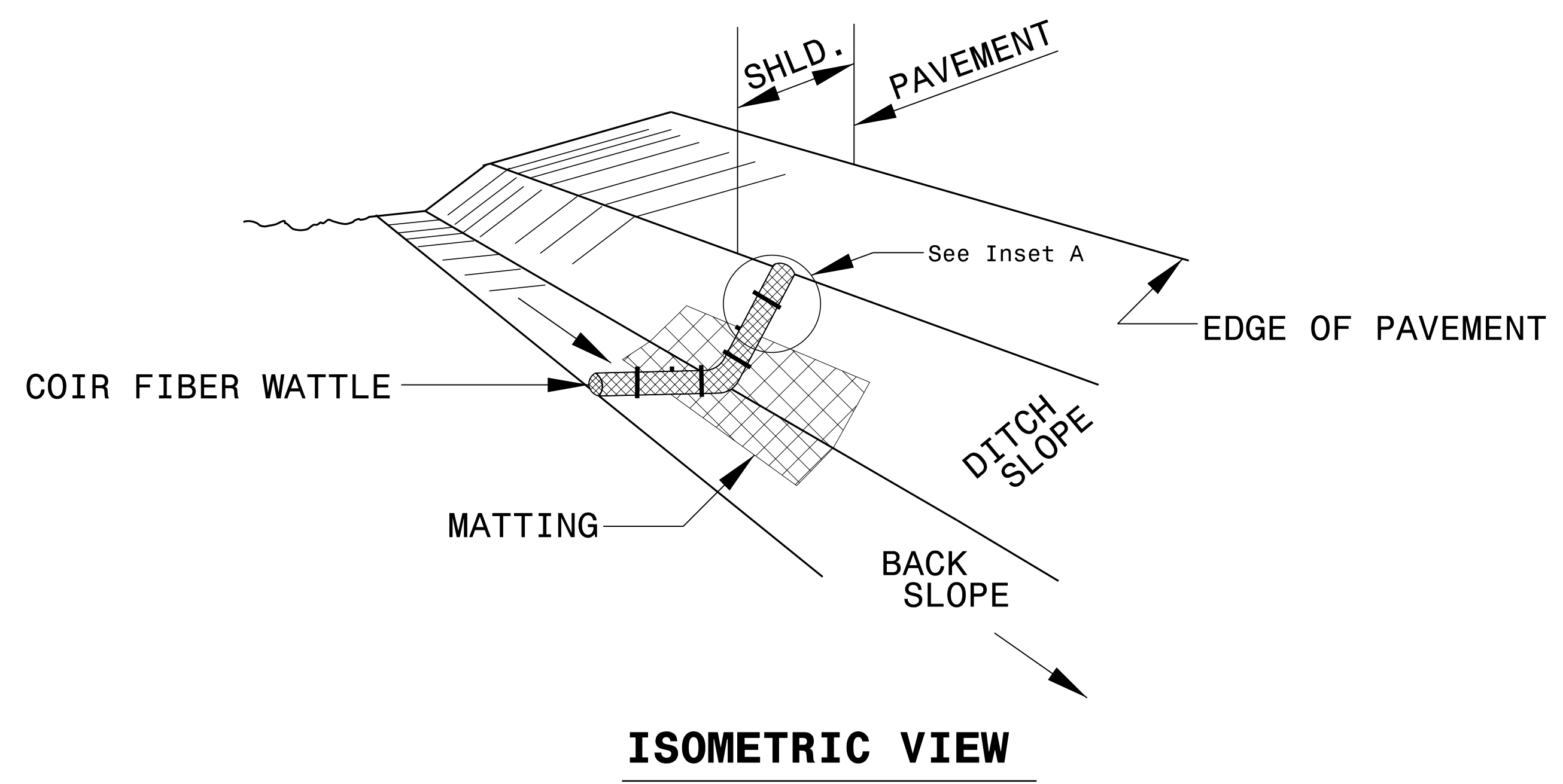
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

INSTALL MATTING IN ACCORDANCE WITH SECTION 1631 OF THE STANDARD SPECIFICATIONS.

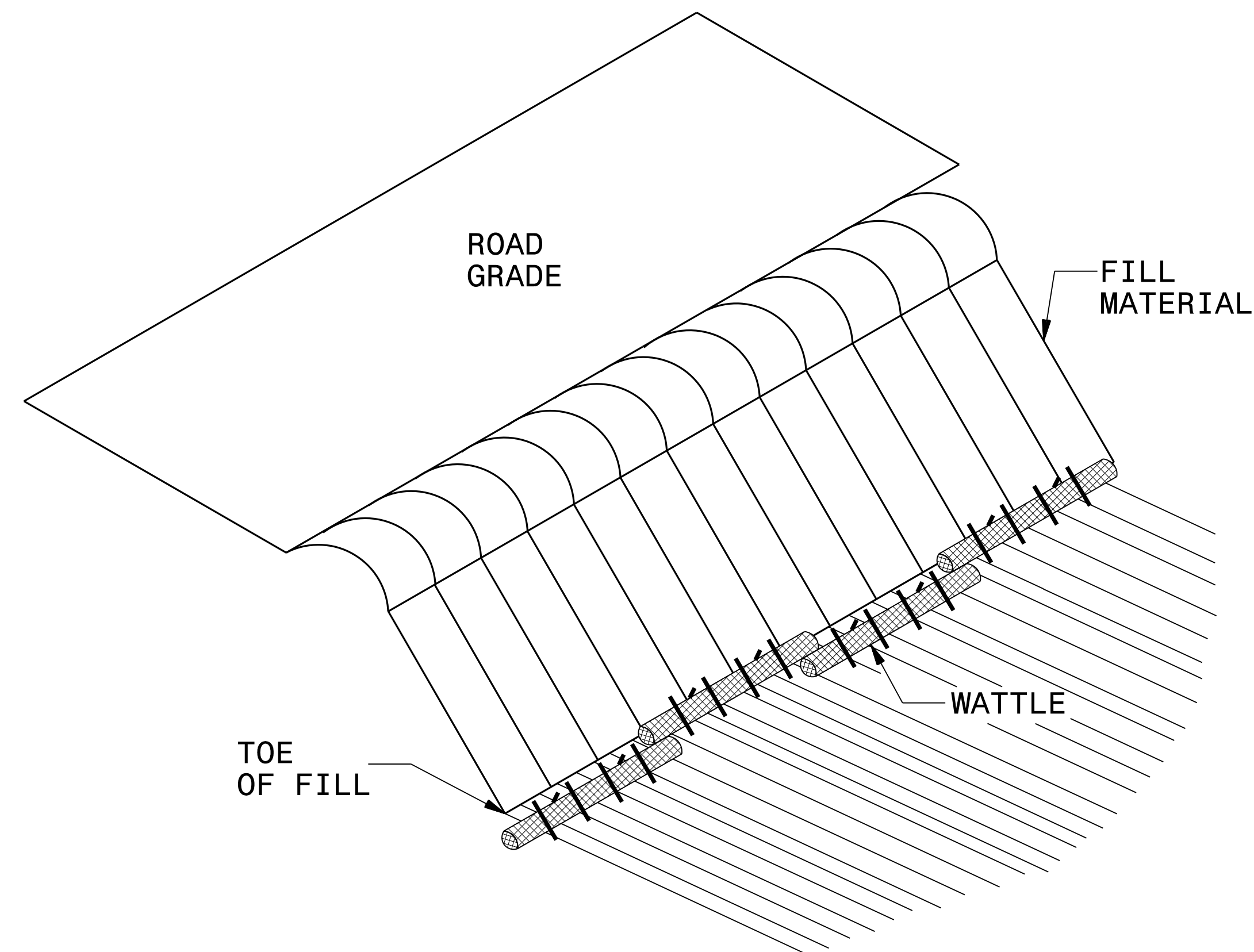
PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH WATTLE.

INITIALLY APPLY 2 OUNCES OF ANIONIC OR NEUTRALLY CHARGED PAM OVER WATTLE WHERE WATER WILL FLOW AND 1 OUNCE OF PAM ON EACH SIDE OF WATTLE. REAPPLY PAM AFTER EVERY RAINFALL EVENT THAT IS EQUAL TO OR EXCEEDS 0.50 IN.

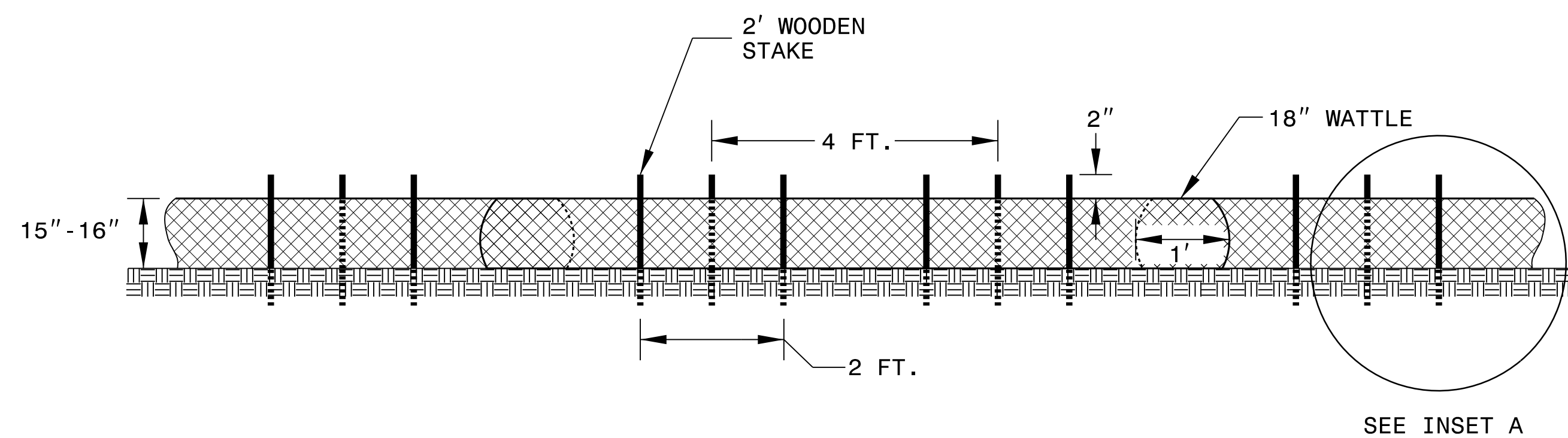


PROJECT REFERENCE NO. <i>R-3421A</i>	SHEET NO. <i>EC-2C</i>
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

WATTLE BARRIER DETAIL



ISOMETRIC VIEW



FRONT VIEW

NOTES:

USE MINIMUM 18 IN. NOMINAL DIAMETER EXCELSIOR WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 2 TO 3 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

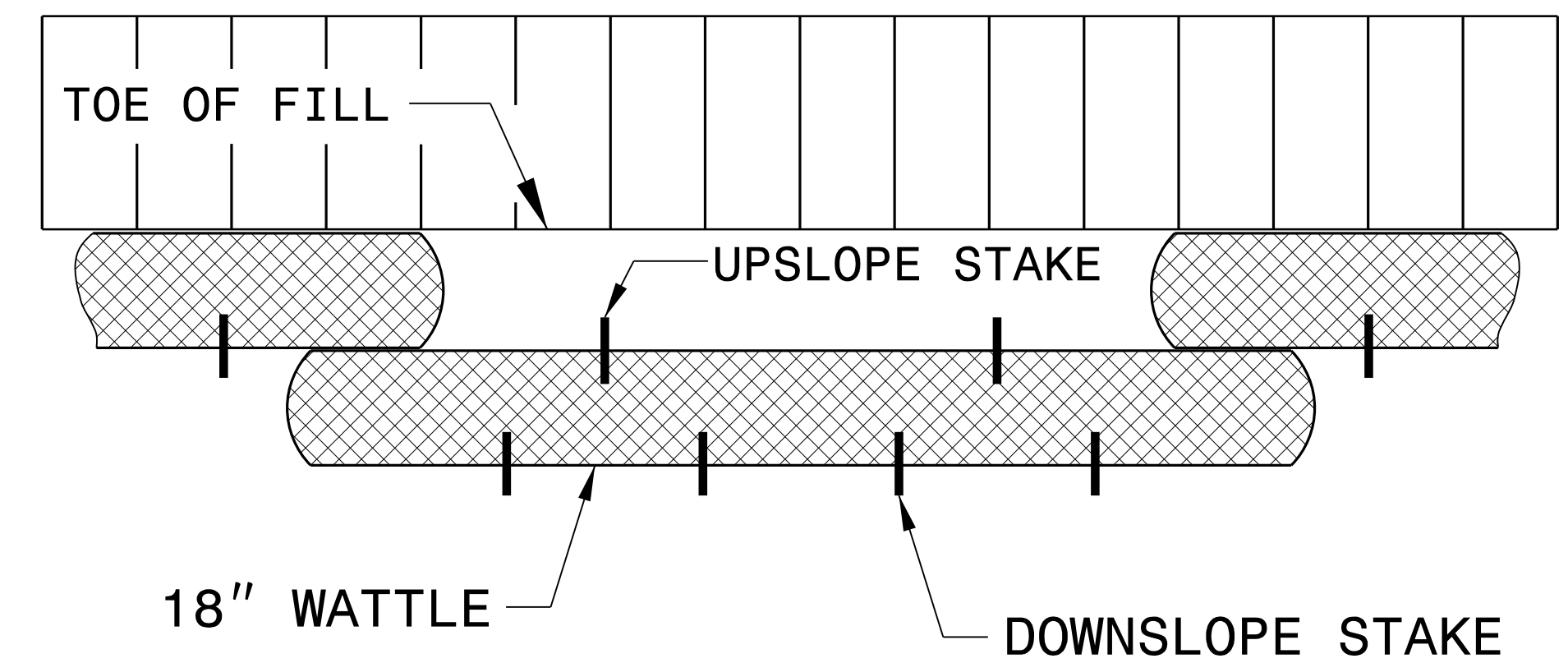
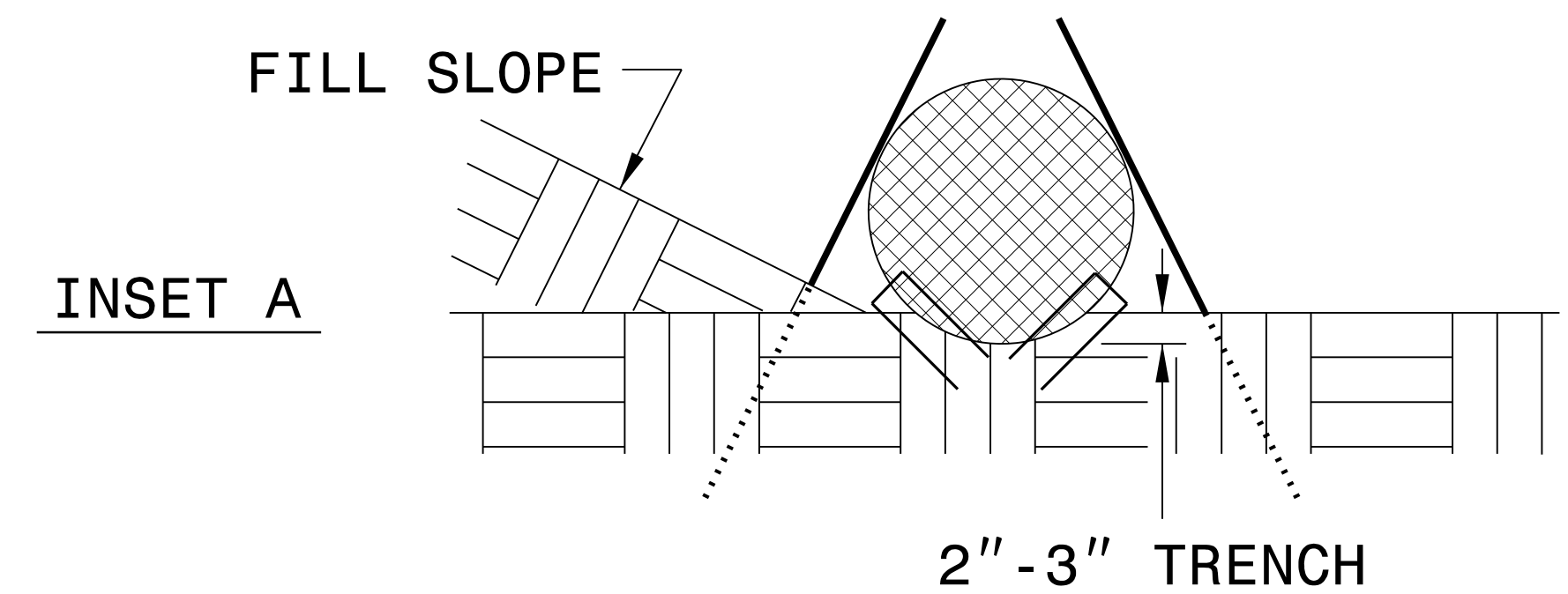
USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

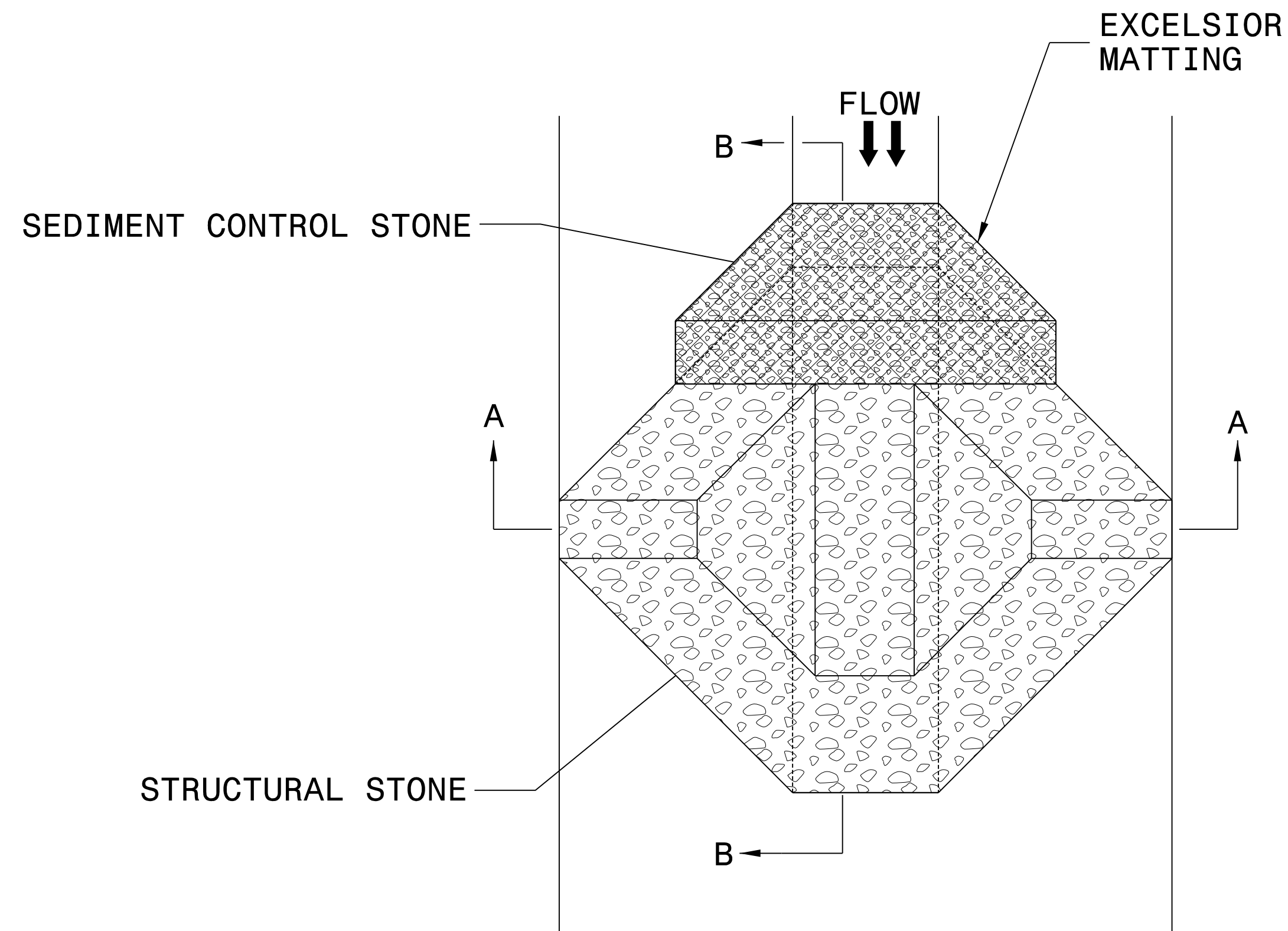
FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 25 FT.



TOP VIEW

PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-2D
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)



PLAN

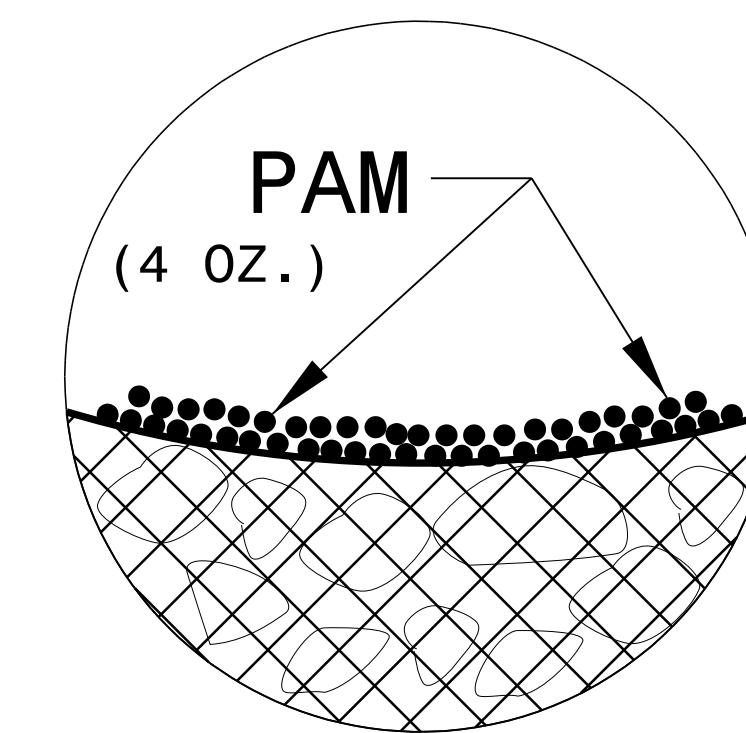
NOTES:

INSTALL TEMPORARY ROCK SILT CHECK TYPE A IN ACCORDANCE WITH ROADWAY STANDARD DRAWING NO. 1633.01.

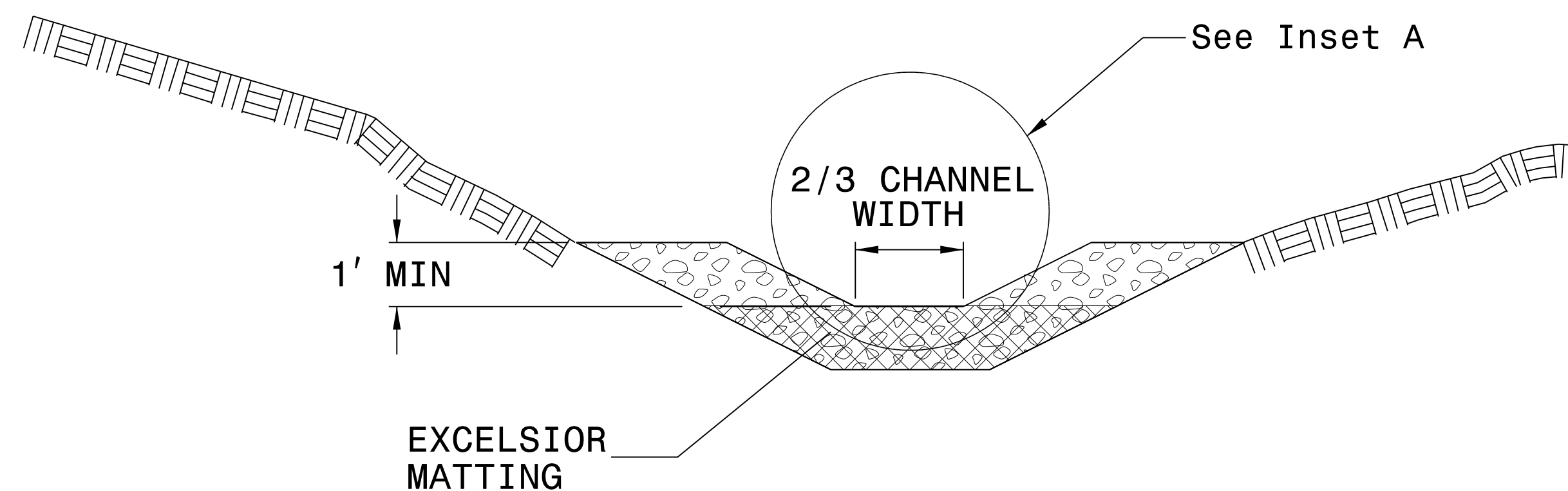
USE EXCELSIOR FOR MATTING MATERIAL AND ANCHOR MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

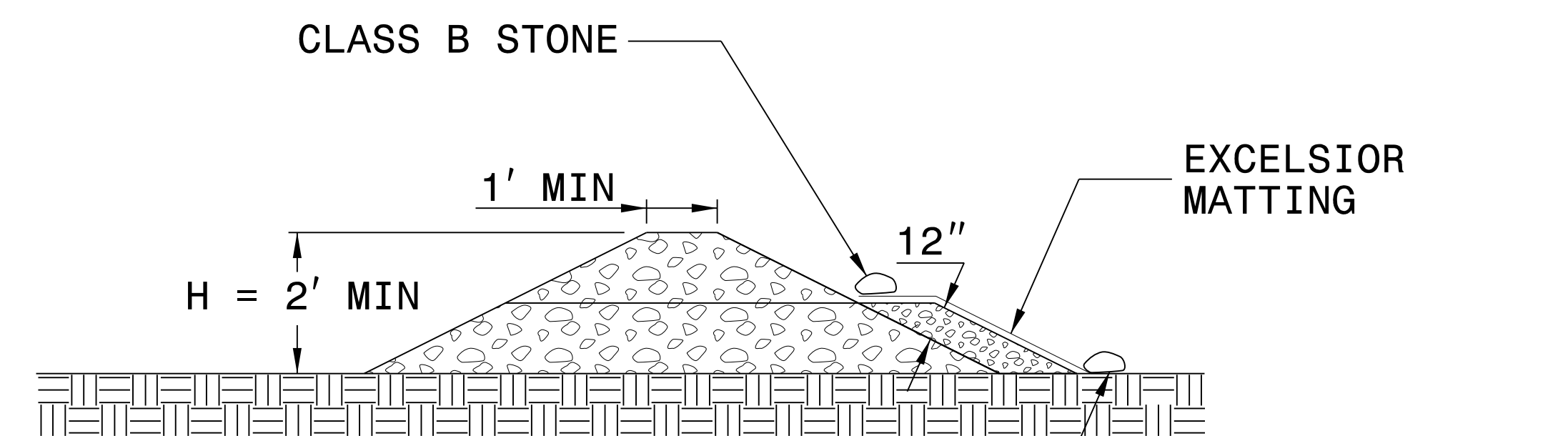
INITIALLY APPLY 4 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.



INSET A



SECTION A-A



SECTION B-B

NOT TO SCALE

DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA

PROJECT REFERENCE NO. <i>R-3421A</i>	SHEET NO. <i>EC-3</i>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

SOIL STABILIZATION SUMMARY SHEET

EROSION CONTROL MATTING IN DITCHES

EROSION CONTROL MATTING IN DITCHES

CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)
5	US 74	45+00	50+00	RT	670
6	US 74	50+00	54+50	RT	605
6	US 74	52+50	55+00	LT	335
7	FLY	07+25	10+00	RT	370
7	FLY	10+00	12+50	RT	335
7	FLY	12+50	15+00	RT	335
7-8	FLY	15+00	17+50	RT	335
7-8	FLY	15+00	17+50	LT	335
7	I-73	73+87	77+28	RT	460
7-8	I-73	75+00	80+80	CL	1030
7-8	I-73	77+28	79+77	RT	335
7-8	I-73	77+50	80+60	LT	415
7	US 74	67+00	69+27	RT	305
7	US 74	69+00	70+25	RT	170
7	US 74	70+25	73+87	RT	485
8	FLY	17+50	20+00	RT	335
8	FLY	20+00	24+00	RT	540
8	I-73	79+11	81+50	RT	320
8	I-73	80+60	82+00	LT	190
8	I-73	80+80	85+50	CL	835
8	I-73	85+50	86+50	CL	180
8-9	I-73	89+50	95+00	CL	980
8-9	RPC	13+50	18+00	RT	605
9	I-73	95+00	100+43	CL	965
9-10	I-73	103+75	107+50	LT	425
9	L2 CONN	14+81	17+31	RT	335
9	L2 CONN	17+31	18+31	RT	135
9	L2 CONN	18+31	20+00	RT	230
9	L2 REV	15+68	18+27	LT	350
9	L2 REV	18+27	20+50	LT	300

SUBTOTAL 13,245

CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)
9	L2 REV	19+50	25+50	RT	805
9	L2 REV	23+00	25+00	LT	270
9-13	L2 RT	10+50	16+00	LT	740
9	L2 RT	12+00	14+00	RT	270
9	L2 RT	14+00	18+00	RT	540
9	L2 RT	16+00	18+00	LT	270
9	L2 RT	18+00	19+50	RT	205
9	RPA	07+00	13+65	RT	890
9-10	RPA	07+55	11+14	LT	405
9	RPA	11+14	13+25	LT	285
9	RPA	16+50	18+00	RT	205
9	RPA	18+00	19+34	RT	180
9	RPC	18+00	21+00	RT	405
9	RPC	21+00	22+50	RT	205
9	RPC	22+50	26+00	RT	470
9	RPC	26+00	28+00	RT	270
9-11	RPC	28+00	32+50	RT	605
10	I-73	107+50	110+00	CL	445
10	I-73	110+00	115+00	CL	890
10	I-73	110+50	115+00	LT	605
10	I-73	115+00	123+50	CL	1510
10	I-73	115+00	123+50	LT	1140
10	I-73	123+00	125+50	RT	285
10	I-73	123+50	127+00	CL	625
10	I-73	123+50	127+50	LT	540
10	I-73	125+00	126+50	LT	75
10	I-73	127+00	128+00	CL	180
10-11	RPD	05+00	09+50	LT	510
10-11	RPD	07+14	09+11	RT	265
10	RPD	09+11	10+00	RT	120

SUBTOTAL 14,210

DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA

PROJECT REFERENCE NO. <i>R-3421A</i>	SHEET NO. <i>EC-3B</i>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

SOIL STABILIZATION SUMMARY SHEET
PERMANENT SOIL REINFORCEMENT MAT IN DITCHES

CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)
7	FLY	07+25	12+50	LT	705
7	FLY	12+50	15+00	LT	335
8	FLY	17+50	20+00	LT	335
8	FLY	20+00	22+00	LT	270
8-13	FLY	29+00	31+45	RT	330
8-9	I-73	89+00	95+00	RT	805
9-13	FLY	32+50	34+50	RT	270
9	RPA	13+65	16+50	RT	385
9	RPC	18+00	24+00	RT	300
9	RPC	19+50	22+00	LT	335
9	RPC	19+50	22+00	LT	335
9	RPC	22+00	26+00	LT	540
10	I-73	121+00	122+50	LT	75
10	I-73	124+00	125+00	LT	50
10	RPD	14+00	15+00	RT	135
10	RPD	15+00	18+00	RT	405
10	RPD	18+00	21+65	RT	490
11-12	L2	38+85	43+88	RT	675
11	L2 REV	35+15	36+30	LT	155
12	L2	44+50	49+45	RT	665
12	L2	49+45	53+00	RT	480
12	L2 REV	45+50	46+12	LT	85
15	US 74	122+50	124+00	RT	75
			PROJECT TOTAL		8,235
				SAY	8,500

7/1/2019 R:\Hydro\ulics\CADD\PSH\Erosion Control\1r-3421a-EC-ps03.dgn

DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA

PROJECT REFERENCE NO. <i>R-3421A</i>	SHEET NO. <i>EC-3C</i>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

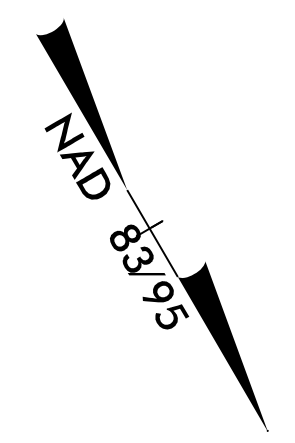
SOIL STABILIZATION TIMEFRAMES

<i>SITE DESCRIPTION</i>	<i>STABILIZATION TIME</i>	<i>TIMEFRAME EXCEPTIONS</i>
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10' OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	14 DAYS	7 DAYS FOR SLOPES GREATER THAN 50' IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	14 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.

PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-4/CONST.4
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

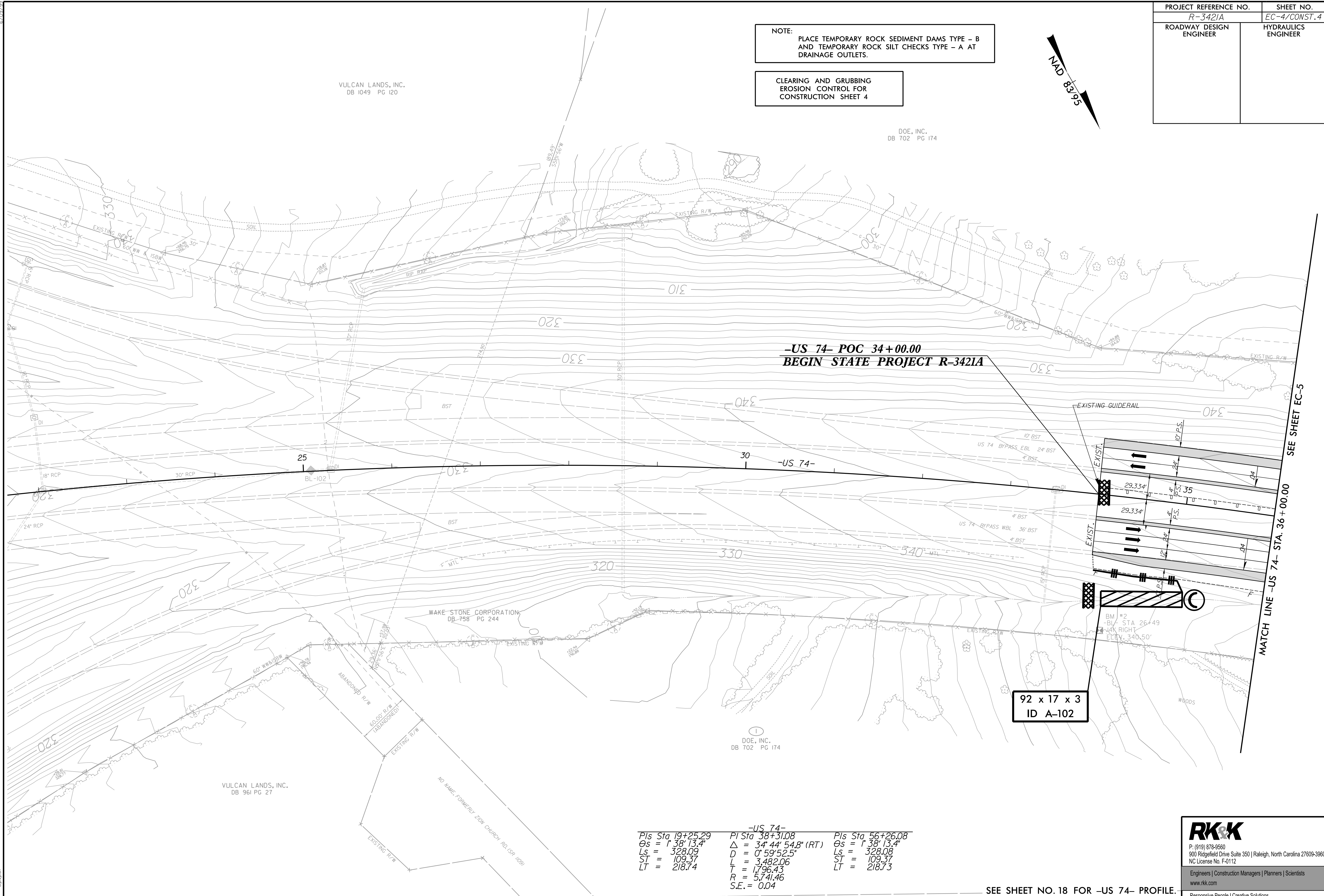
NOTE:
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B
AND TEMPORARY ROCK SILT CHECKS TYPE - A AT
DRAINAGE OUTLETS.

CLEARING AND GRUBBING
EROSION CONTROL FOR
CONSTRUCTION SHEET 4



VULCAN LANDS, INC.
DB 1049 PG 120

DOE, INC.
DB 702 PG 174



-US 74- PIs Sta 19+25.29 Os = 1° 38' 13.4" Ls = 328.09 ST = 109.37 LT = 218.74	-US 74- PI Sta 38+31.08 Δ = 34° 44' 54.8" (RT) D = 0° 59' 52.5" L = 3,482.06 T = 1,796.43 R = 5,741.46 S.E. = 0.04	PIs Sta 56+26.08 Os = 1° 38' 13.4" Ls = 328.08 ST = 109.37 LT = 218.73
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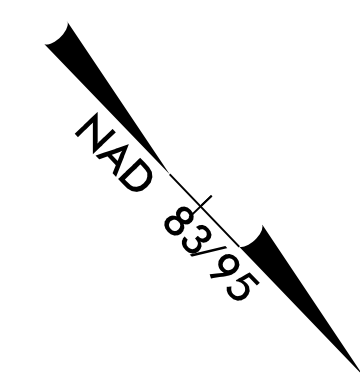
SEE SHEET NO. 18 FOR -US 74- PROFILE.

RK&K
 P: (919) 878-9560
 900 Ridgely Drive Suite 350 | Raleigh, North Carolina 27609-3960
 NC License No. F-0112
 Engineers | Construction Managers | Planners | Scientists
 www.rkk.com
 Responsive People | Creative Solutions

6/10/09
 7/1/09
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 RPK

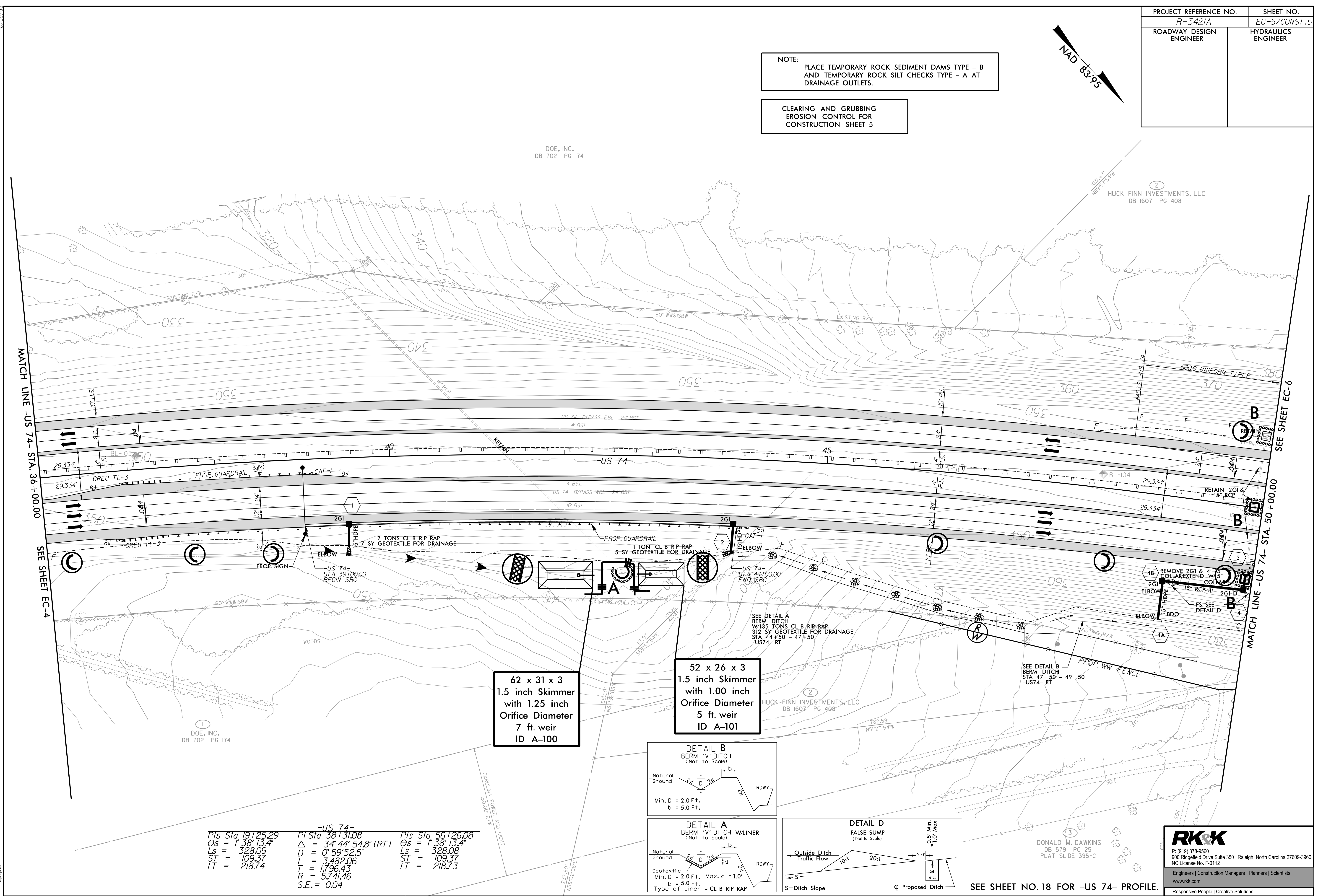
NOTE:
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILT CHECKS TYPE - A AT DRAINAGE OUTLETS.

CLEARING AND GRUBBING
EROSION CONTROL FOR
CONSTRUCTION SHEET 5



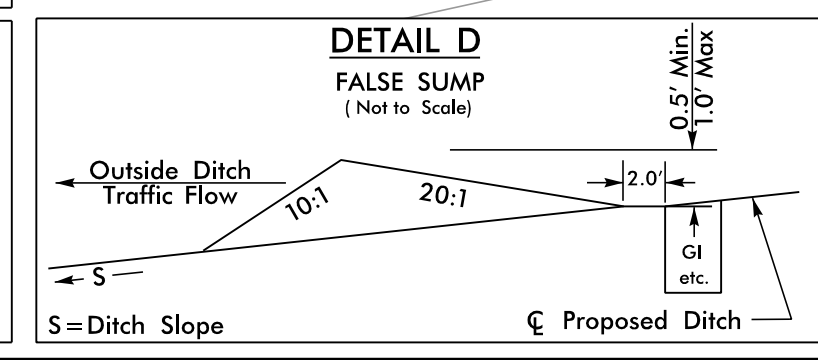
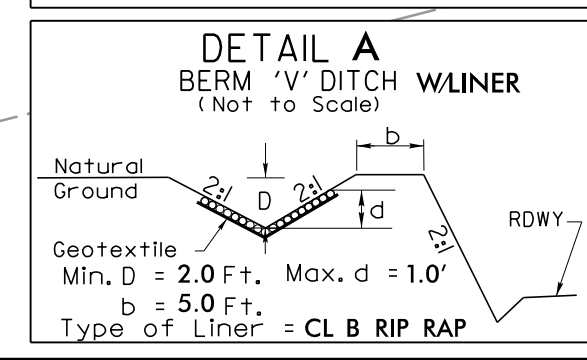
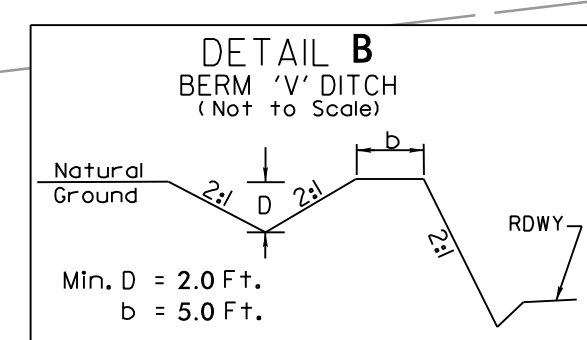
DOE, INC.
DB 702 PG 174

HUCK FINN INVESTMENTS, LLC
DB 1607 PG 408



62 x 31 x 3
1.5 inch Skimmer
with 1.25 inch
Orifice Diameter
7 ft. weir
ID A-100

52 x 26 x 3
1.5 inch Skimmer
with 1.00 inch
Orifice Diameter
5 ft. weir
ID A-101



-US 74-		
PIs Sta 19+25.29	PI Sta 38+31.08	PIs Sta 56+26.08
Os = 1' 38" 13.4"	Δ = 34' 44" 54.8" (RT)	Os = 1' 38" 13.4"
Ls = 328.09	D = 0' 59" 52.5"	Ls = 328.08
ST = 109.37	L = 3,482.06	ST = 109.37
LT = 218.74	L = 1,796.43	LT = 218.73
	R = 5,741.46	
	S.E. = 0.04	

SEE SHEET NO. 18 FOR -US 74- PROFILE.

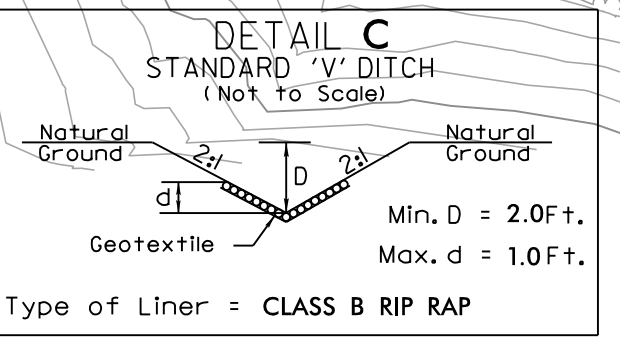
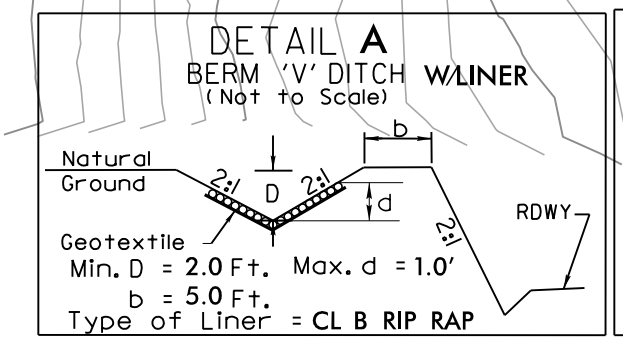
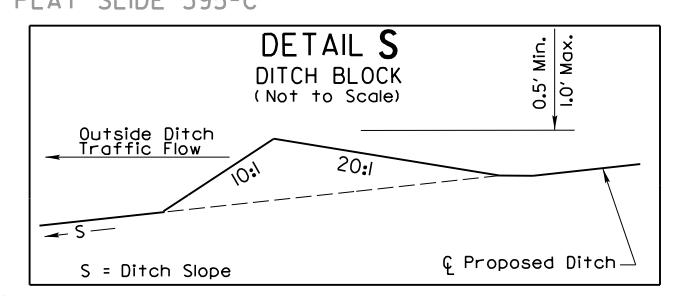
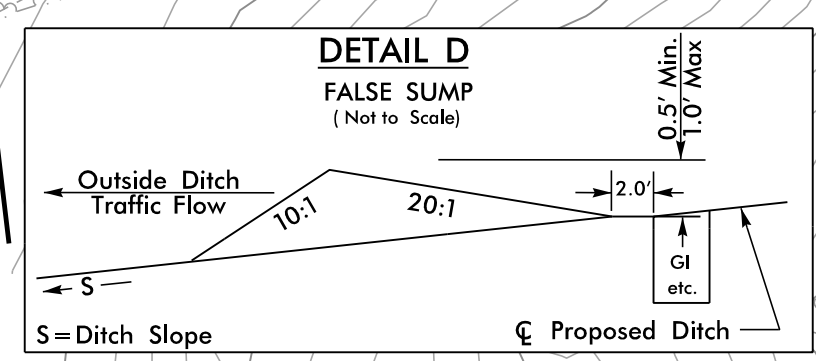
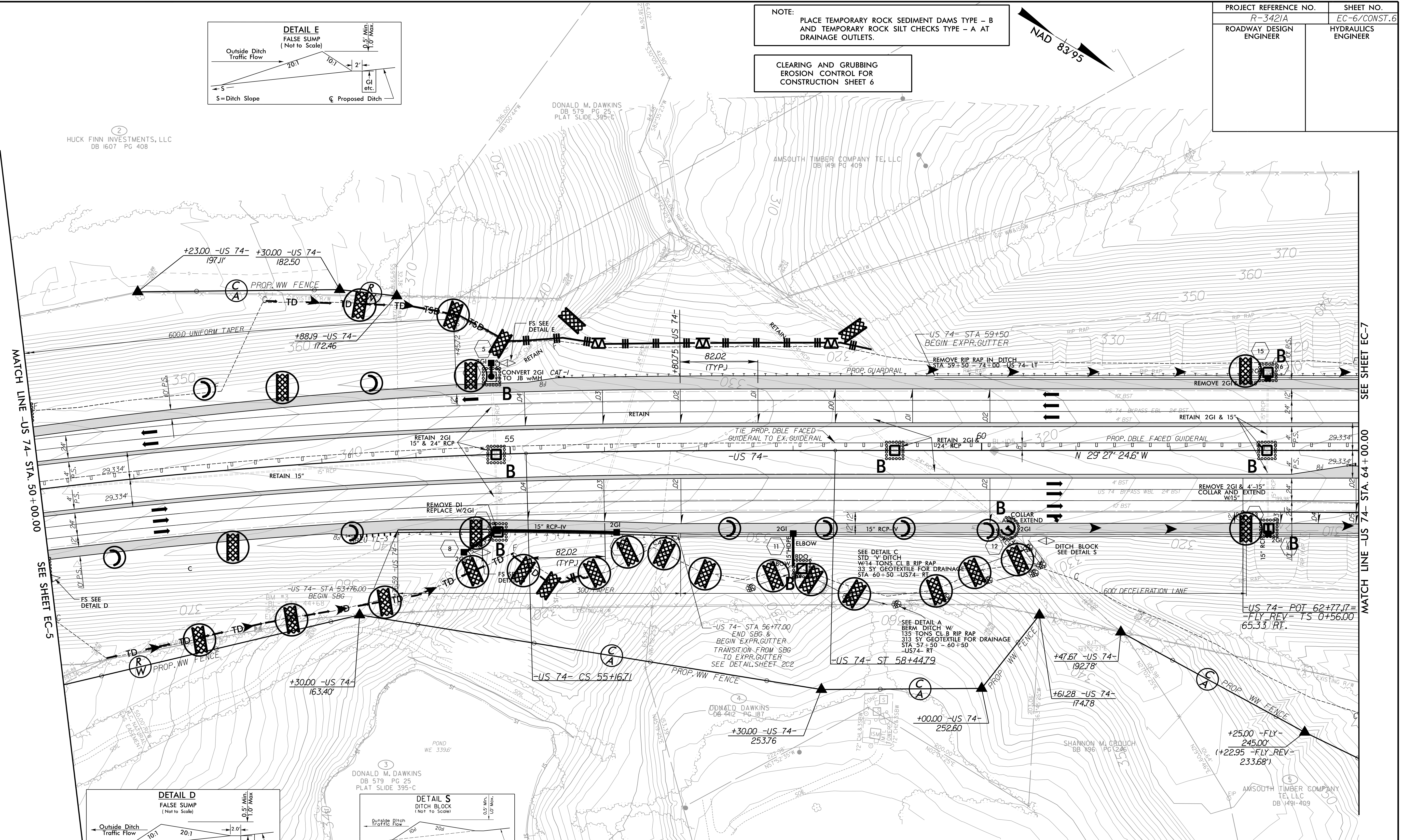
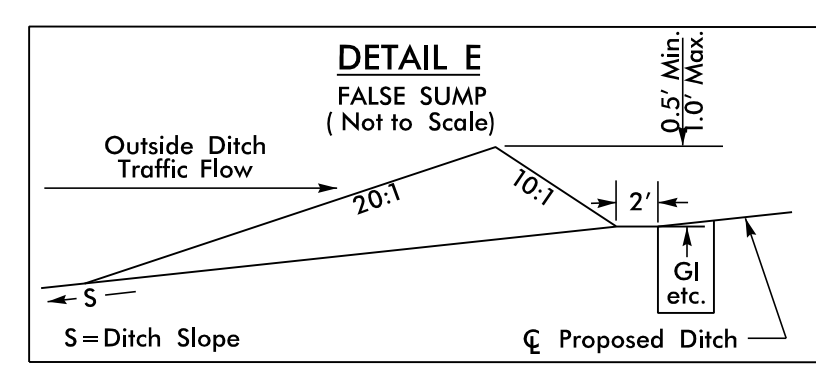
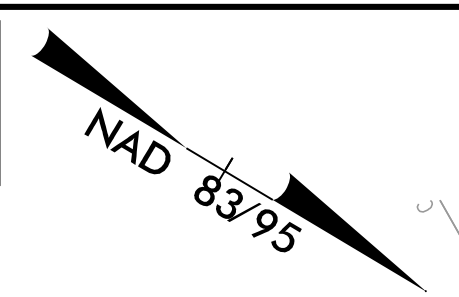
RK&K
P: (919) 878-8560
900 Ridgeway Drive Suite 350 | Raleigh, North Carolina 27609-3960
NC License No. F-0112
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6/09/05
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NOTE:
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILT CHECKS TYPE - A AT DRAINAGE OUTLETS.

CLEARING AND GRUBBING EROSION CONTROL FOR CONSTRUCTION SHEET 6



-US 74-		-FLY REV-	
PIs Sta 19+25.29	PI Sta 38+31.08	PIs Sta 1+89.36	PI Sta 4+10.47
$\Theta_s = 1^\circ 38' 13.4''$	$\Delta = 34^\circ 44' 54.8''$ (RT)	$\Theta_s = 3^\circ 08' 53.2''$	$\Delta = 9^\circ 42' 08.9''$ (RT)
LS = 328.09	D = 0' 59' 52.5"	LS = 200.00'	D = 3' 08' 53.2"
ST = 109.37	L = 3,482.06	LT = 133.35'	L = 308.20'
LT = 218.74	T = 1,796.43	T = 154.47'	T = 154.47'
	R = 5,741.46	R = 1,820.00'	R = 1,820.00'
	S.E. = 0.04	S.E. = 0.07	S.E. = 0.07

SEE SHEET NO. 19 FOR -US 74- PROFILE.
SEE SHEET NO. 27 FOR -FLY- PROFILE.

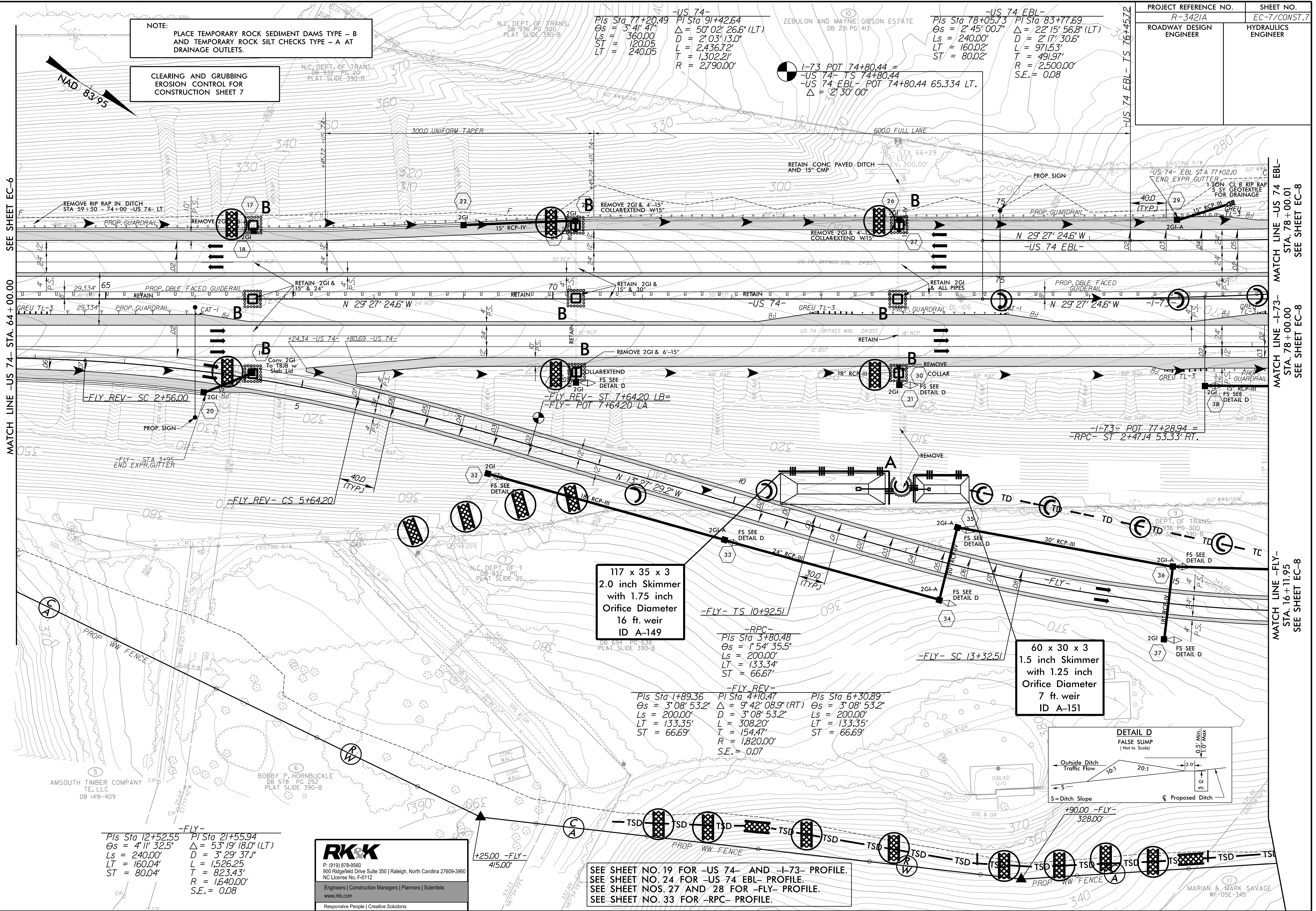
RK&K
P: (919) 878-9560
900 Ridgefield Drive Suite 350 | Raleigh, North Carolina 27609-3960
NC License No. F-0112
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6/10/2018
\\Hydra\utils\CADD\PSH\Erosion_Control\3421a_EC_psh06.dgn
csp:pr

PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-7/CONST.7
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

NOTE:
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILT CHECKS TYPE - A AT DRAINAGE OUTLETS.

CLEARING AND GRUBBING EROSION CONTROL FOR CONSTRUCTION SHEET 7



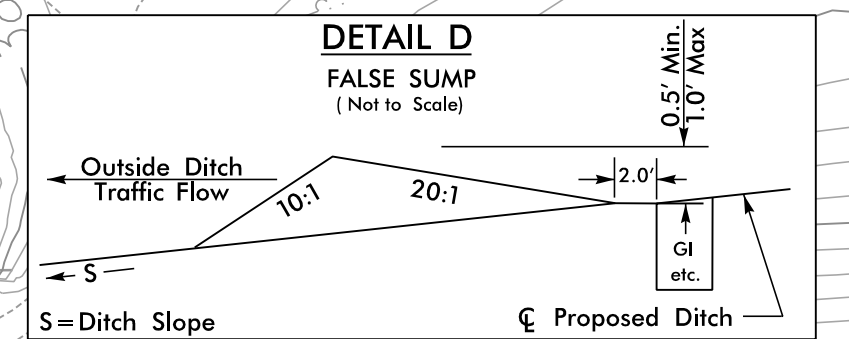
-FLY-
 PIs Sta 12+52.55 PIs Sta 21+55.94
 $\theta_s = 4'11'' 32.5''$ $\Delta = 5'3'19'' 18.0''$ (LT)
 Ls = 240.00' D = 3'29' 37.1"
 LT = 160.04' L = 1526.25'
 ST = 80.04' T = 823.43'
 R = 1640.00'
 S.E. = 0.08

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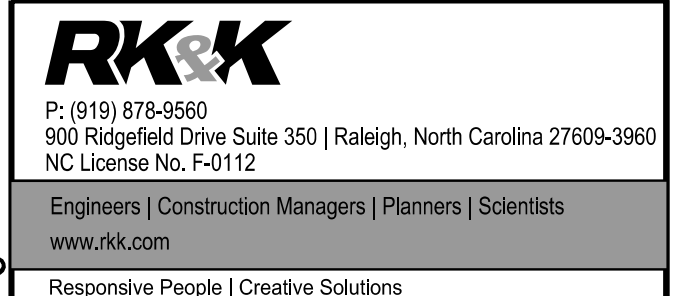
SEE SHEET NO. 19 FOR -US 74- AND -I-73- PROFILE.
 SEE SHEET NO. 24 FOR -US 74 EBL- PROFILE.
 SEE SHEET NOS. 27 AND 28 FOR -FLY- PROFILE.
 SEE SHEET NO. 33 FOR -RPC- PROFILE.

117 x 35 x 3
 2.0 inch Skimmer
 with 1.75 inch
 Orifice Diameter
 16 ft. weir
 ID A-149

60 x 30 x 3
 1.5 inch Skimmer
 with 1.25 inch
 Orifice Diameter
 7 ft. weir
 ID A-151



5/10/2018
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 7/1/2018
 C:\Hydra\autocad\p18\ER-0501\ER-0501.dwg



-US 74-
 Pls Sta 77+20.49
 Os = 3° 41' 47"
 Ls = 360.00
 ST = 120.05
 LT = 240.05

-US 74 EBL-
 PI Sta 91+42.64
 Δ = 50° 02' 26.6" (LT)
 D = 2° 03' 13.0"
 L = 2436.72'
 T = 1302.21'
 R = 2790.00'

-US 74 EBL-
 PI Sta 78+05.73
 Os = 2° 45' 00.7"
 Ls = 240.00'
 LT = 160.02'
 ST = 80.02'

-US 74 EBL-
 PI Sta 83+77.69
 Δ = 22° 15' 56.8" (LT)
 D = 2° 17' 30.6"
 L = 971.53'
 T = 491.97'
 R = 2500.00'
 S.E. = 0.08

-US 74 EBL-
 PI Sta 95+61.05
 Δ = 28° 43' 16.5" (LT)
 D = 2° 05' 03.3"
 L = 1378.02'
 T = 703.81'
 R = 2749.00'

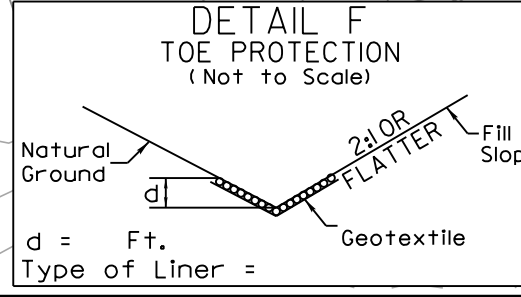
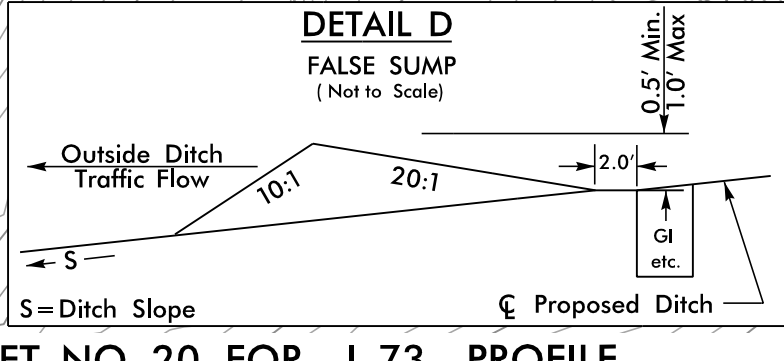
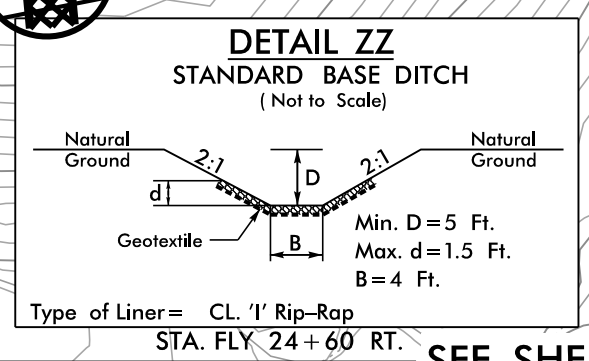
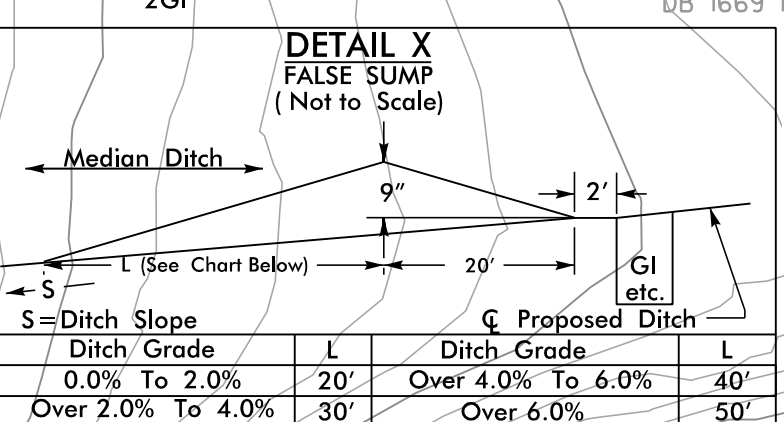
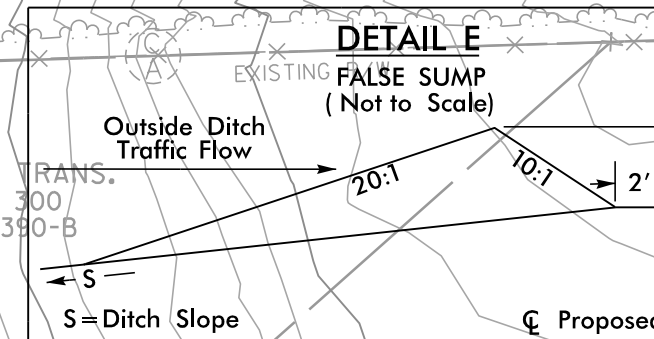
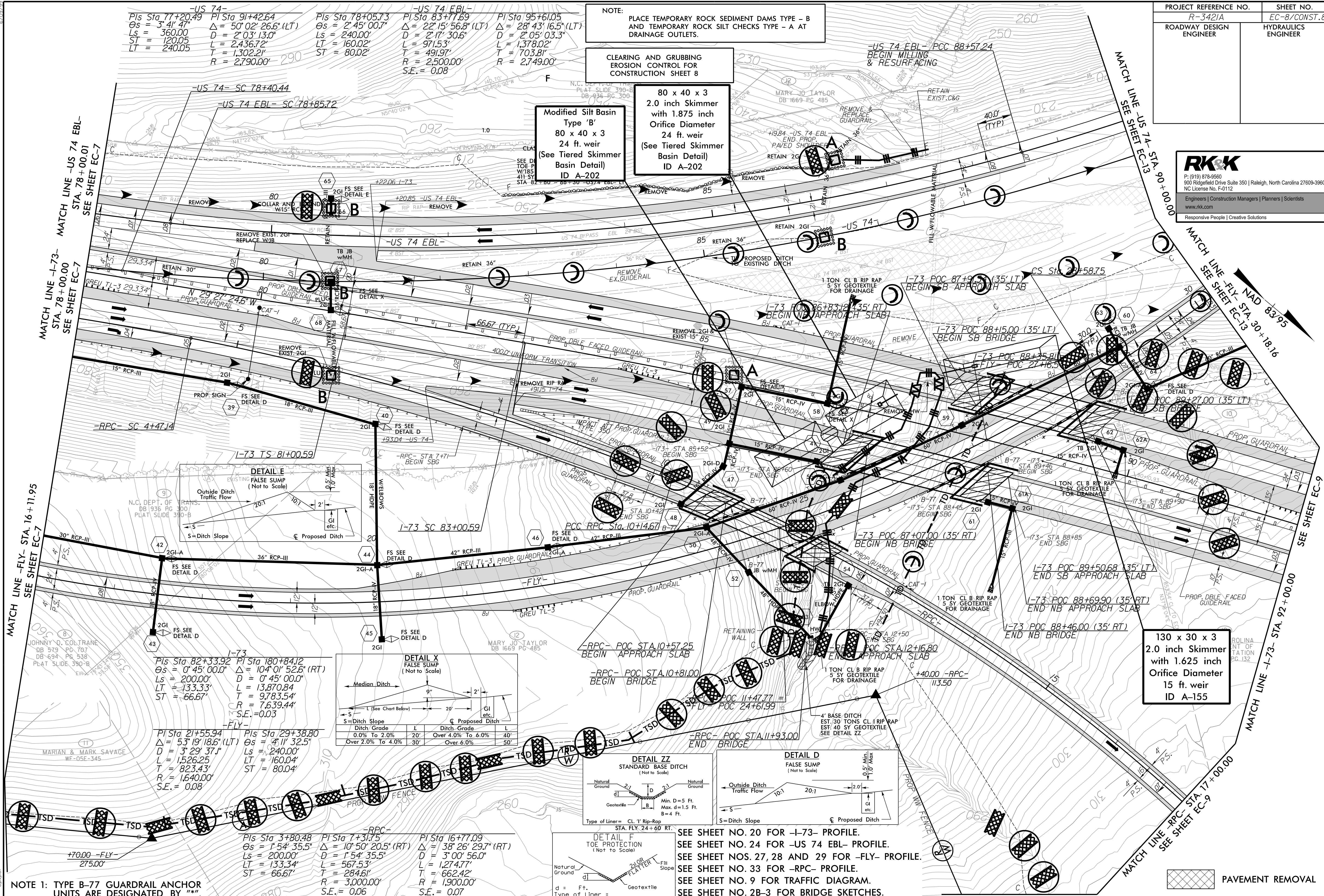
NOTE:
 PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILT CHECKS TYPE - A AT DRAINAGE OUTLETS.

CLEARING AND GRUBBING
 EROSION CONTROL FOR
 CONSTRUCTION SHEET 8

Modified Silt Basin
 Type 'B'
 80 x 40 x 3
 24 ft. weir
 (See Tiered Skimmer
 Basin Detail)
 ID A-202

80 x 40 x 3
 2.0 inch Skimmer
 with 1.875 inch
 Orifice Diameter
 24 ft. weir
 (See Tiered Skimmer
 Basin Detail)
 ID A-202

130 x 30 x 3
 2.0 inch Skimmer
 with 1.625 inch
 Orifice Diameter
 15 ft. weir
 ID A-155



SEE SHEET NO. 20 FOR -I-73- PROFILE.
 SEE SHEET NO. 24 FOR -US 74 EBL- PROFILE.
 SEE SHEET NOS. 27, 28 AND 29 FOR -FLY- PROFILE.
 SEE SHEET NO. 33 FOR -RPC- PROFILE.
 SEE SHEET NO. 9 FOR TRAFFIC DIAGRAM.
 SEE SHEET NO. 2B-3 FOR BRIDGE SKETCHES.

NOTE 1: TYPE B-77 GUARDRAIL ANCHOR UNITS ARE DESIGNATED BY "A".



PROJECT REFERENCE NO. **R-3421A** SHEET NO. **EC-9/CONSTR.3**

AWY SHEET NO. **EC-9/CONSTR.3**

ROADWAY DESIGN ENGINEER

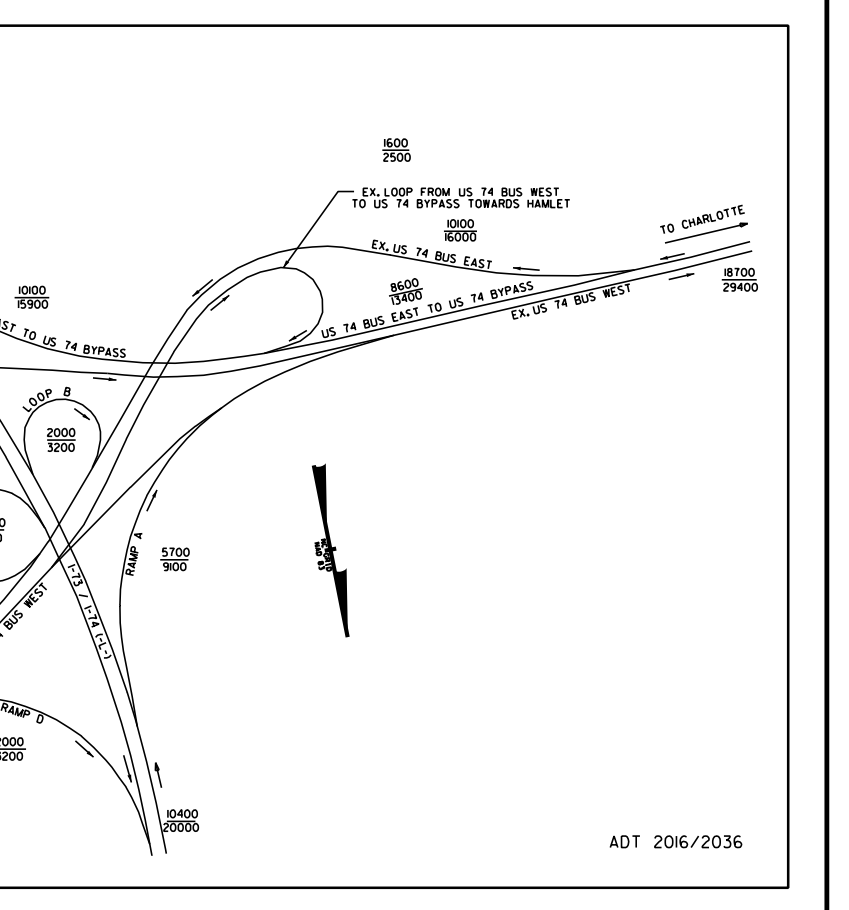
HYDRAULIC ENGINEER

GRAPHIC SCALE
100 50 0 100 200
PLANS

NOTE:
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILE CHECKS TYPE - A AT DRAINAGE OUTLETS.

CLEANING AND GRUBBING EROSION CONTROL FOR CONSTRUCTION SHEET 9

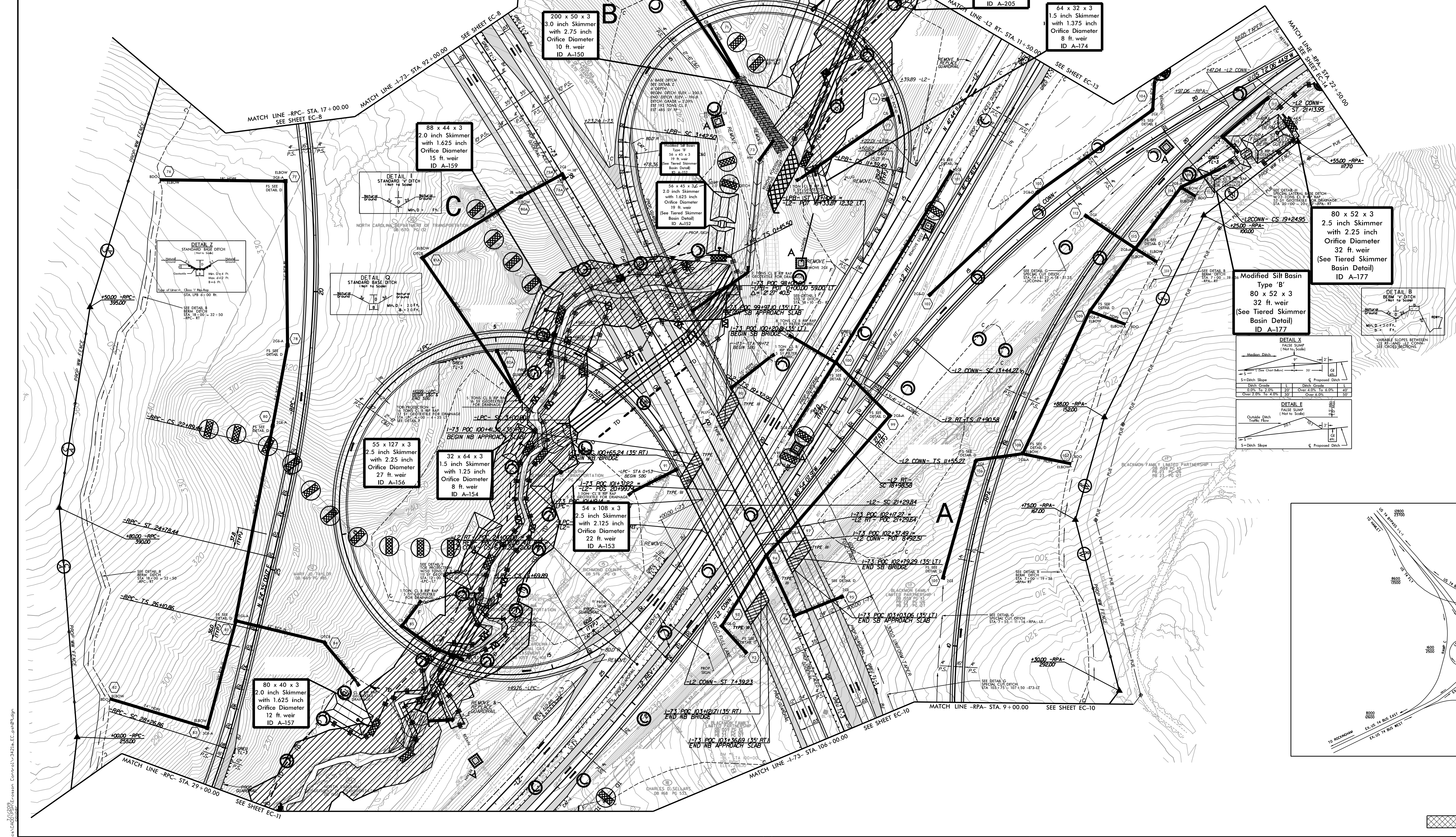
-12 RT-		-12 CONN-		-12 CONN-	
PI SIG	PI SIG	PI SIG	PI SIG	PI SIG	PI SIG
18162.58	21482.52	18162.58	21482.52	18162.58	21482.52
68 = 0'45.52'	Δ = 7'01.15'(RT)	68 = 0'45.52'	Δ = 7'01.15'(RT)	68 = 0'45.52'	Δ = 7'01.15'(RT)
LS = 12200	D = 7'26.45'	LS = 12200	D = 7'26.45'	LS = 12200	D = 7'26.45'
LT = 7200	L = 50.45'	LT = 7200	L = 50.45'	LT = 7200	L = 50.45'
ST = 3600	T = 246.99'	ST = 3600	T = 246.99'	ST = 3600	T = 246.99'
R = 39600'	R = 39600'	R = 39600'	R = 39600'	R = 39600'	R = 39600'
S.E. = 0.04	S.E. = 0.04	S.E. = 0.04	S.E. = 0.04	S.E. = 0.04	S.E. = 0.04



SEE SHEET NO. 21 FOR -I-73- PROFILE.
SEE SHEET NO. 31 FOR -RPA- PROFILE.
SEE SHEET NO. 34 FOR -RPC- PROFILE.
SEE SHEET NO. 36 FOR -LPB- PROFILE.
SEE SHEET NOS. 37 AND 38 FOR -LPC- PROFILE.
SEE SHEET NO. 39 FOR -L2 RT--L2 REV- PROFILE.
SEE SHEET NO. 41 FOR -L2 CONN- PROFILE.
SEE SHEET NO. 28-3 FOR BRIDGE SKETCHES.

REVISIONS

NO.	DESCRIPTION

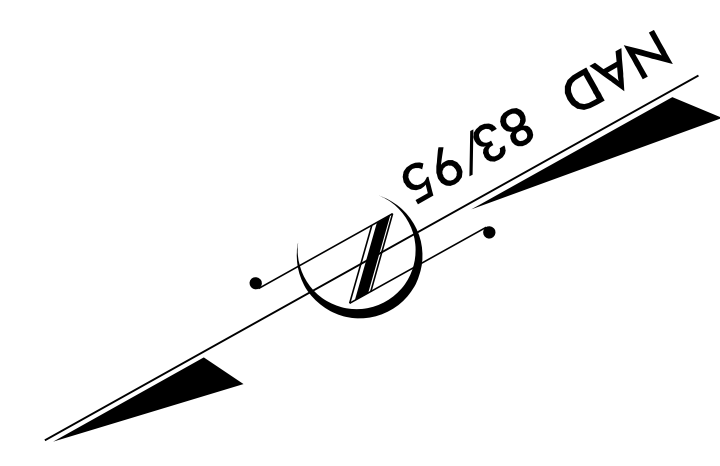
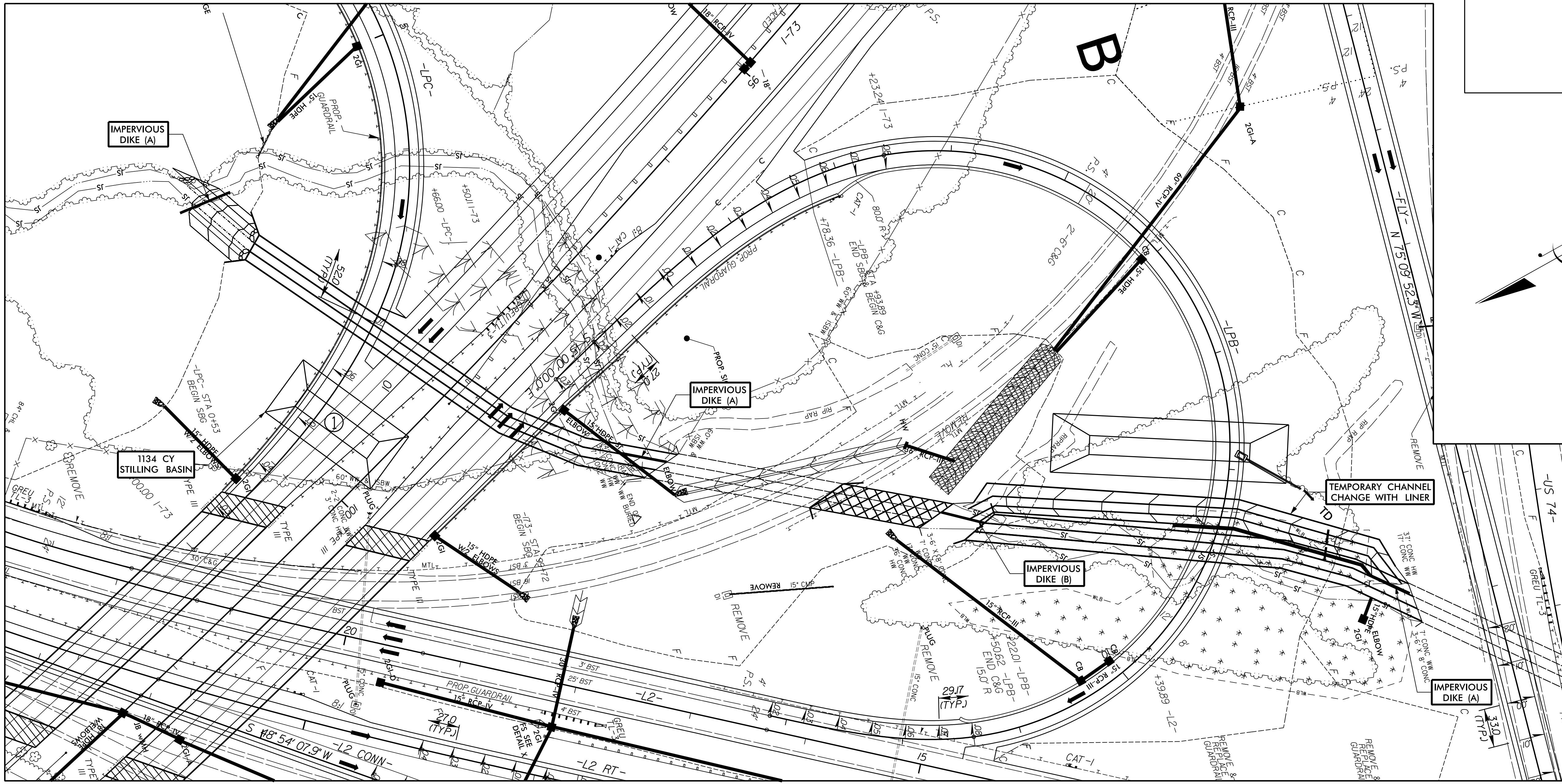


6/26/06

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ADT 206/2036

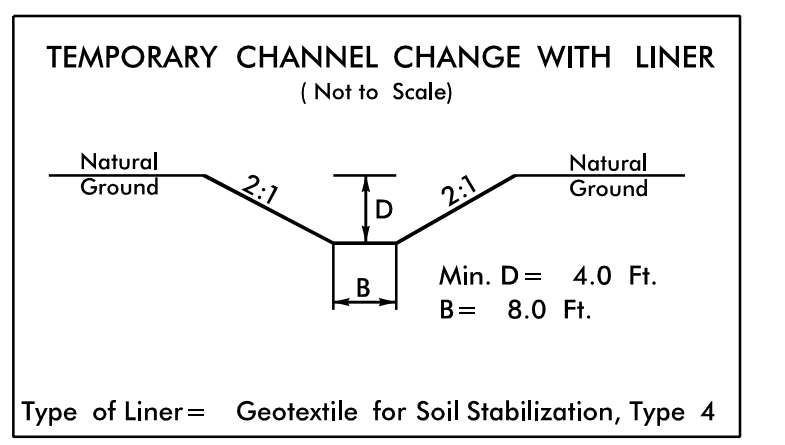
PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-9A
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



**CULVERT CONSTRUCTION SEQUENCE -I-73- STA. 98+63
&
CULVERT CONSTRUCTION SEQUENCE -LPB- STA. 8+69**

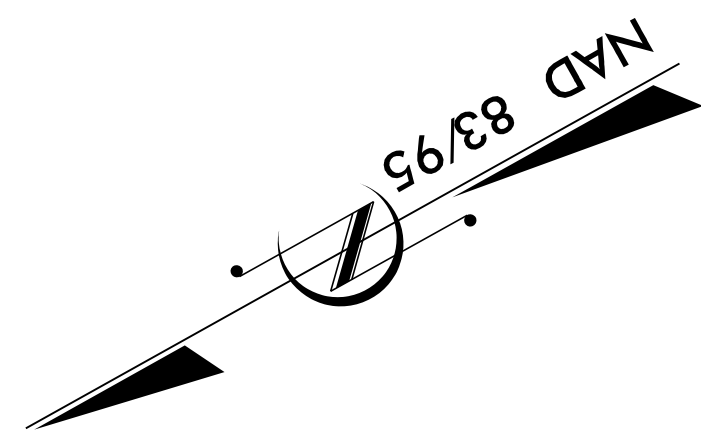
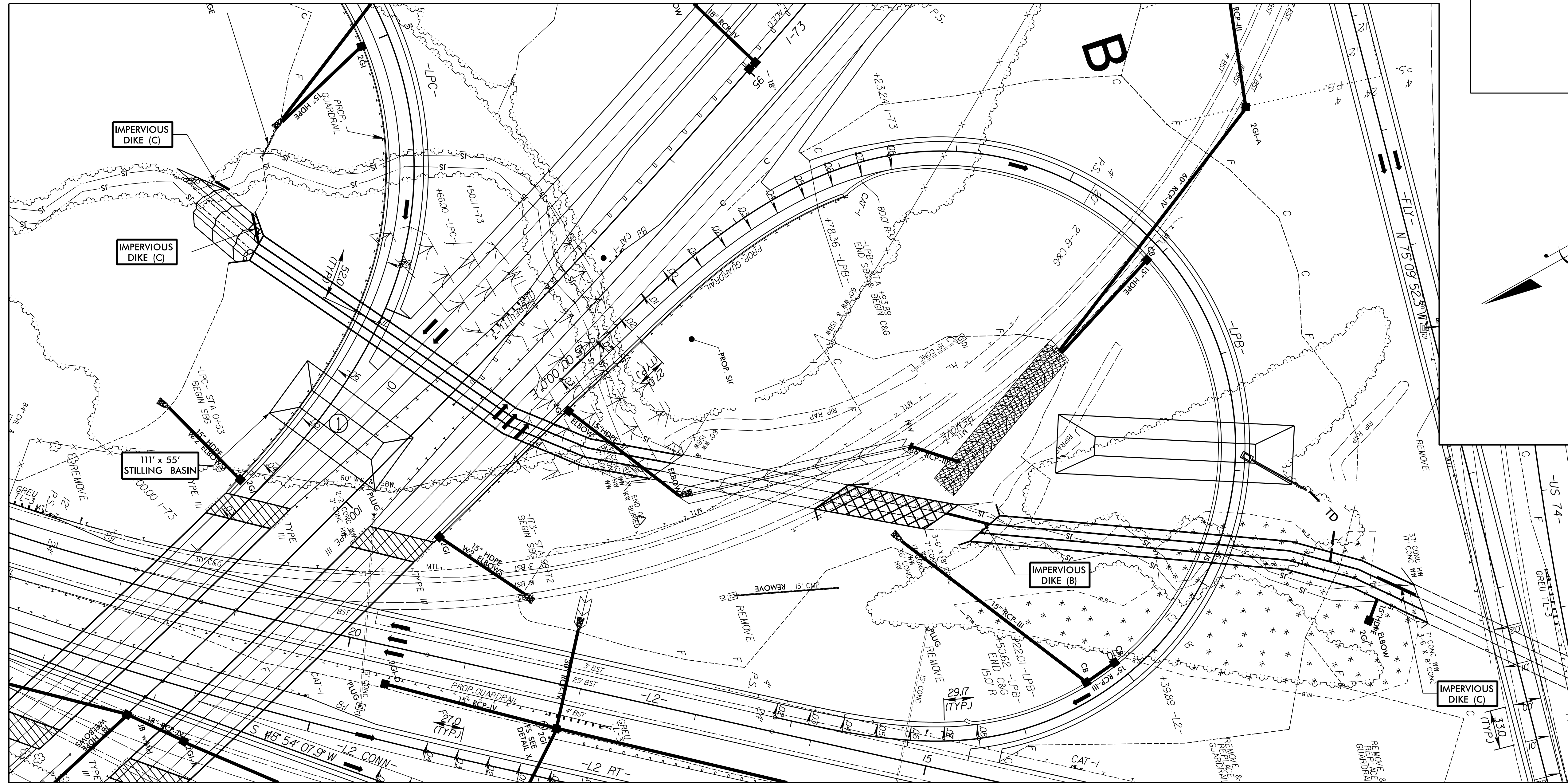
NOTE: UTILIZE TEMPORARY SKIMMER BASINS AS STILLING BASIN WHERE APPLICABLE.

1. CONSTRUCT STILLING BASIN #1 WITH A MINIMUM CAPACITY OF 1134 C.Y.
2. UTILIZE SKIMMER BASIN A-150 AS A STILLING BASIN DURING CONSTRUCTION OF THE PROPOSED DOWNSTREAM CULVERT.
3. CONSTRUCT TEMPORARY CHANNEL CHANGE WITH LINER (8 FT BASE; 4.0 FT DEEP; 2:1 SIDE SLOPES).
4. CONSTRUCT IMPERVIOUS DIKES A & B.
5. DIVERT STREAM THROUGH TEMPORARY CHANNEL AND EASTERN BARREL OF BOTH EXISTING CULVERTS AS SHOWN ON PLAN.
6. CONSTRUCT THE FULL LENGTHS OF THE TWO WESTERN BARRELS, BOTH UPSTREAM AND DOWNSTREAM OF THE EXISTING CULVERT, INCLUDING BOTH WESTERN WINGWALLS. CONSTRUCT APPROXIMATELY 353' OF THE UPSTREAM EASTERN BARREL, 225' OF THE DOWNSTREAM EASTERN BARREL OF THE PROPOSED CULVERT. REMOVE TWO WESTERN BARRELS OF EXISTING CULVERT. CONSTRUCT WESTERN WINGWALL AT OUTLET OF EXISTING CULVERT AND CONSTRUCT PROPOSED CHANNELS TO TIE TO WESTERN BARRELS OF PROPOSED UP AND DOWN STREAM CULVERTS AND CONSTRUCT ROCK VANES IN PROPOSED CHANNEL AT UPSTREAM END AS SHOWN ON PLAN.



6/20/08
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PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-9B
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



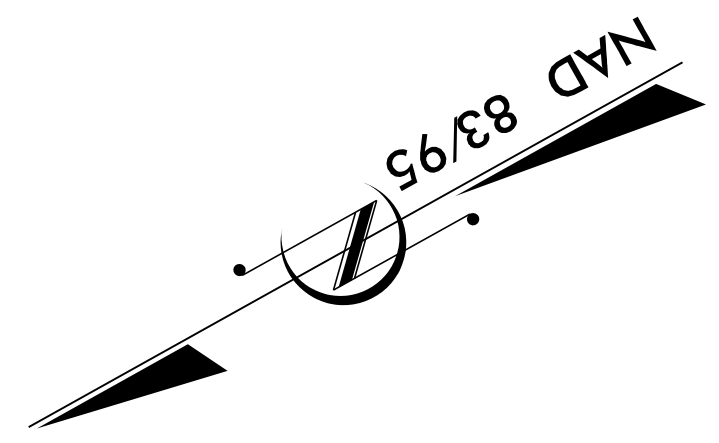
CULVERT CONSTRUCTION SEQUENCE -I-73- STA. 98+63
&
CULVERT CONSTRUCTION SEQUENCE -LPB- STA. 8+69

NOTE:
 UTILIZE TEMPORARY SKIMMER BASINS
 AS STILLING BASIN WHERE APPLICABLE.

1. REMOVE IMPERVIOUS DIKES (A), MAINTAIN IMPERVIOUS DIKES (B) AND INSTALL IMPERVIOUS DIKES (C) AS SHOWN ON PLAN.
2. ABANDON TEMPORARY CHANNEL AND DIVERT STREAM THROUGH THE TWO WESTERN BARRELS OF BOTH THE PROPOSED AND THE EXISTING CULVERTS.
3. CONSTRUCT APPROXIMATELY 54' OF THE UPSTREAM EASTERN BARREL AND APPROXIMATELY 133' OF THE DOWNSTREAM EASTERN BARREL. REMOVE EASTERN BARREL OF EXISTING CULVERT, CONSTRUCT EASTERN HEADWALL AT EXISTING CULVERT AND GRADE CHANNEL TO INLET OF DOWNSTREAM PROPOSED CULVERT.
4. REMOVE IMPERVIOUS ALL DIKES AND DIVERT STREAM THROUGH NEW CULVERT.
5. REMOVE STILLING BASINS.
6. CONSTRUCT ROADWAY FILL.

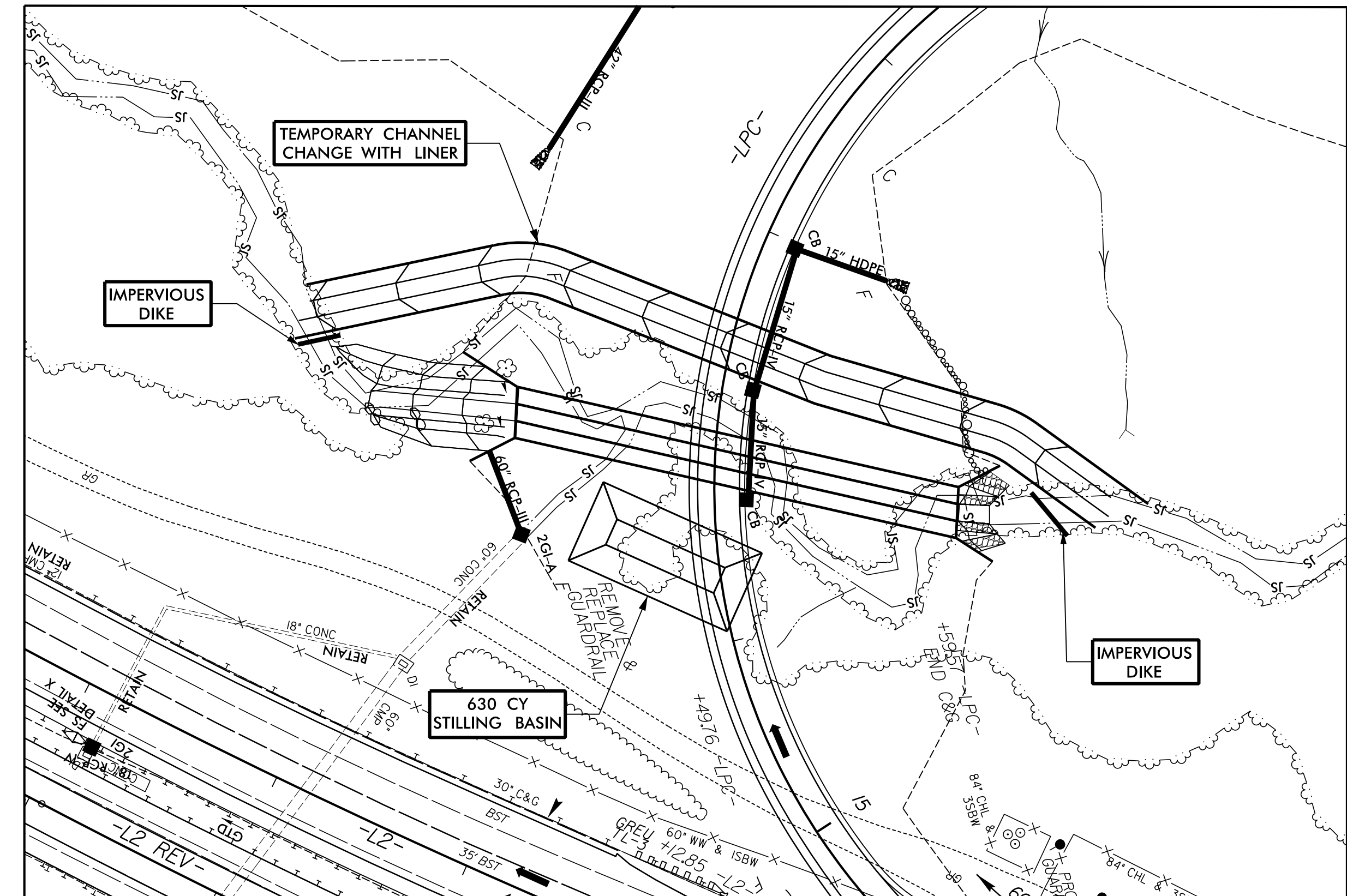
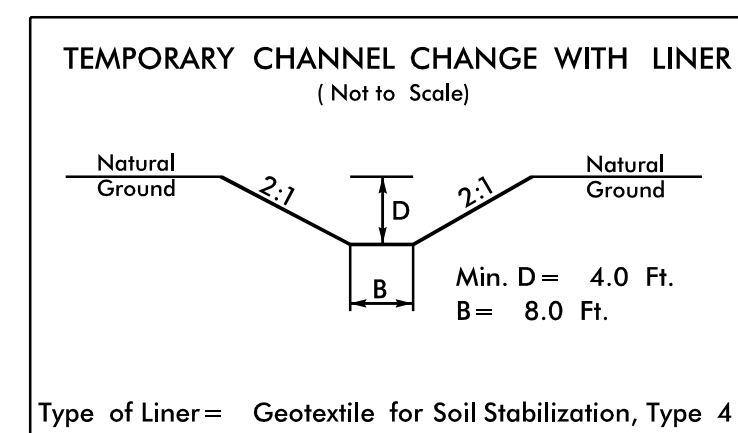
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PROJECT REFERENCE NO. R-3421A		SHEET NO. EC-9C
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER



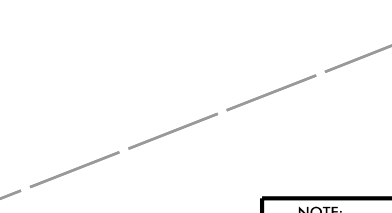
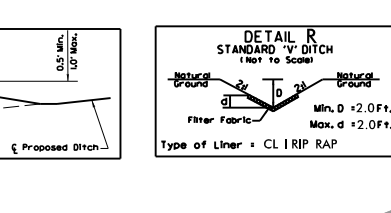
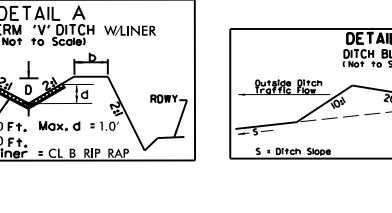
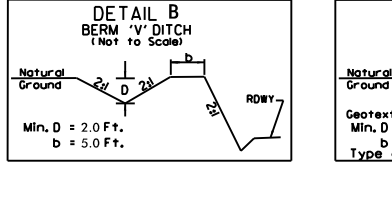
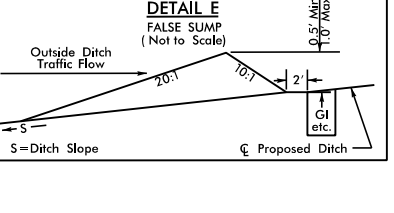
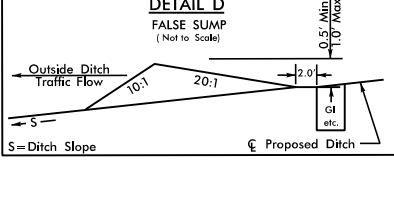
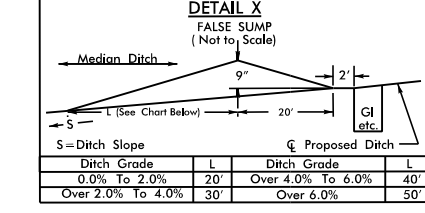
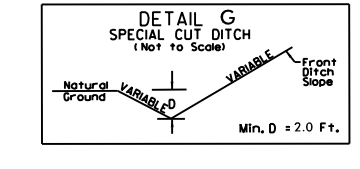
CULVERT CONSTRUCTION SEQUENCE -LPC- STA. 13+11

1. CONSTRUCT STILLING BASIN WITH A MINIMUM CAPACITY OF 630 C.Y.
2. CONSTRUCT TEMPORARY CHANNEL CHANGE WITH LINER (8 FT BASE; 4.0 FT DEEP; 2:1 SIDE SLOPES).
3. CONSTRUCT IMPERVIOUS DIKES.
4. DIVERT STREAM THROUGH TEMPORARY CHANNEL.
5. CONSTRUCT STREAM RELOCATION, PROPOSED CULVERT, ROCK SILLS, ROCK VANES AND CHANNEL IMPROVEMENTS.
6. REMOVE IMPERVIOUS DIKES AND DIVERT STREAM THROUGH NEW CULVERT.
7. REMOVE STILLING BASIN.
8. CONSTRUCT ROADWAY FILL.



6/10/09
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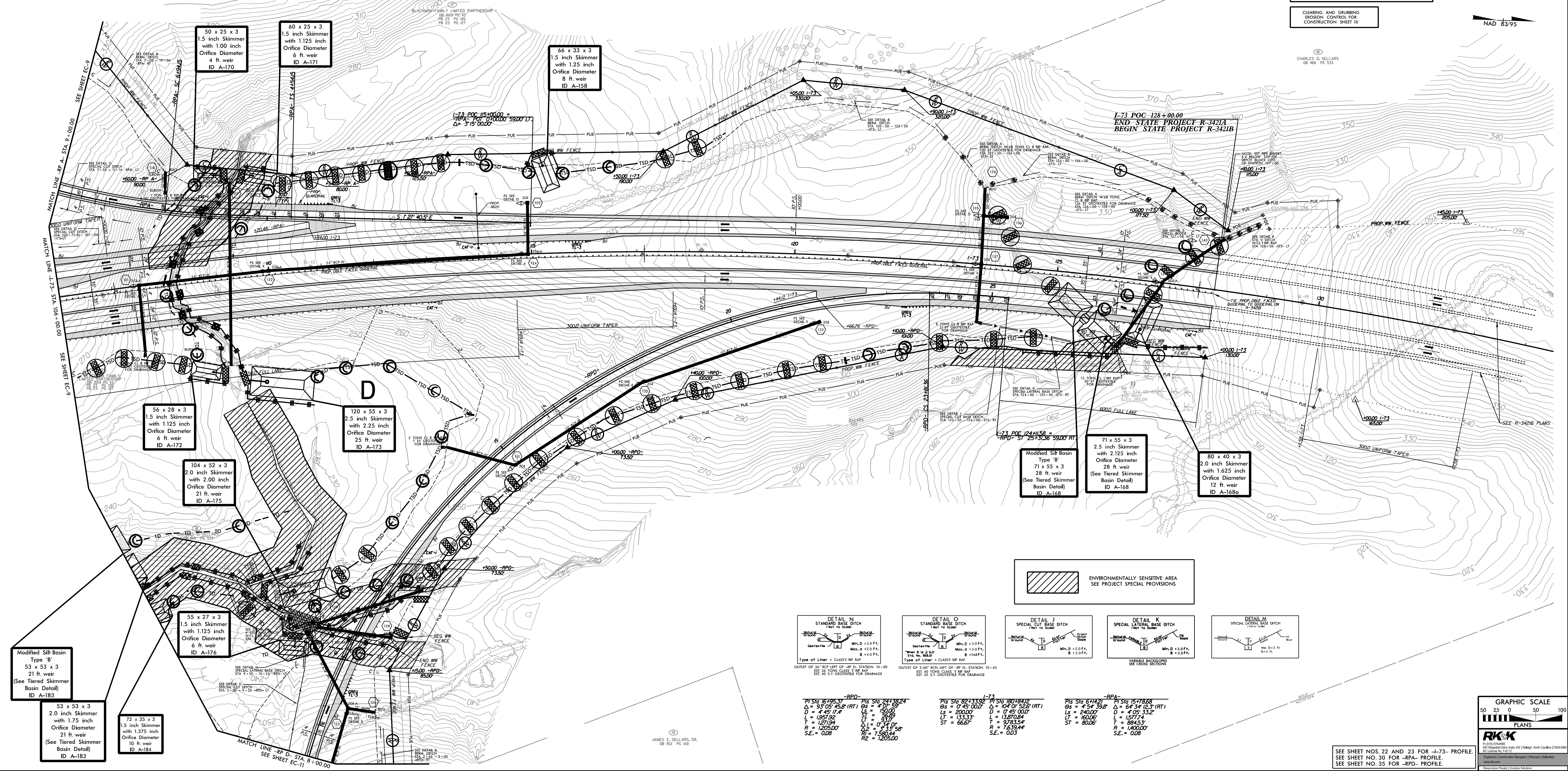
REVISIONS



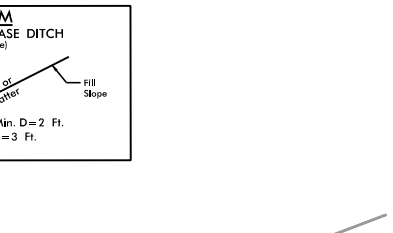
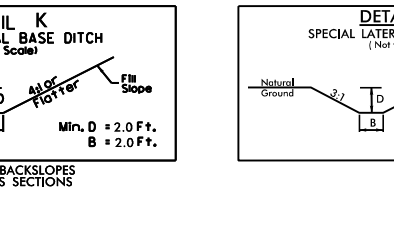
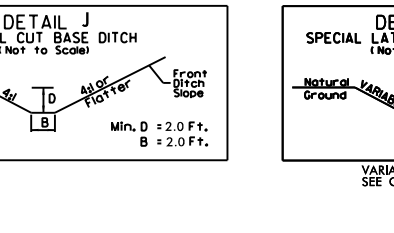
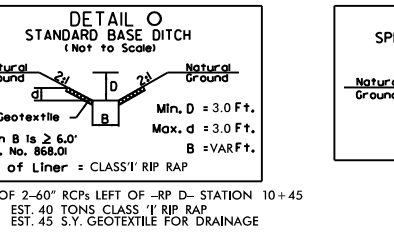
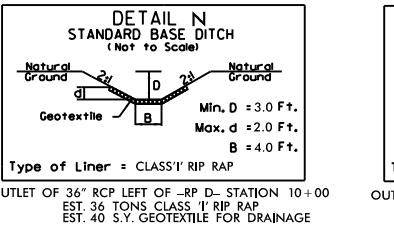
NOTE:
 PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SBT CHECKS TYPE - A AT DRAINAGE OUTLETS.
 CLEARING AND GRUBBING EROSION CONTROL FOR CONSTRUCTION SHEET 10.

NAD 83/95

CHARLES D. SELLARS
 DB REG. PG. 533



ENVIRONMENTALLY SENSITIVE AREA
 SEE PROJECT SPECIAL PROVISIONS



RPD-1

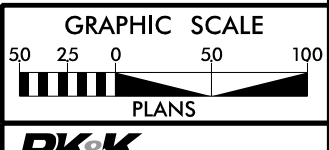
PI STA 16+95.37	PI STA 24+18.24
Δ = 93°05'45.8 (RT)	Δ = 4°11'35"
D = 445.00 FT	LS = 150.00
L = 1897.92	LT = 357.69
T = 1271.94	Δ = 0°24'18"
R = 1205.00	RS = 75.50 FT
SE = 0.08	R2 = 1205.00

1-73

PI STA 82+33.82	PI STA 100+94.02
Δ = 4°45'00.0 (RT)	Δ = 104°07'52.8 (RT)
D = 0°45'00.0	D = 0°45'00.0
LS = 200.00	L = 13.87024
LT = 133.33	T = 57.83544
ST = 66.67	R = 7.63244
SE = 0.03	SE = 0.03

RPD-2

PI STA 61+42.1	PI STA 15+78.68
Δ = 4°54'39.8	Δ = 6°34'12.3 (RT)
D = 0°45'00.0	D = 0°45'00.0
LS = 240.00	L = 1577.74
LT = 160.00	T = 884.53
ST = 80.00	R = 1400.00
SE = 0.08	SE = 0.08

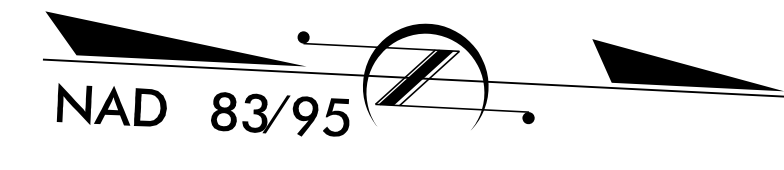


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 919.876.0000
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SEE SHEET NOS. 22 AND 23 FOR 1-73- PROFILE.
 SEE SHEET NO. 30 FOR RPD- PROFILE.
 SEE SHEET NO. 35 FOR RPD- PROFILE.

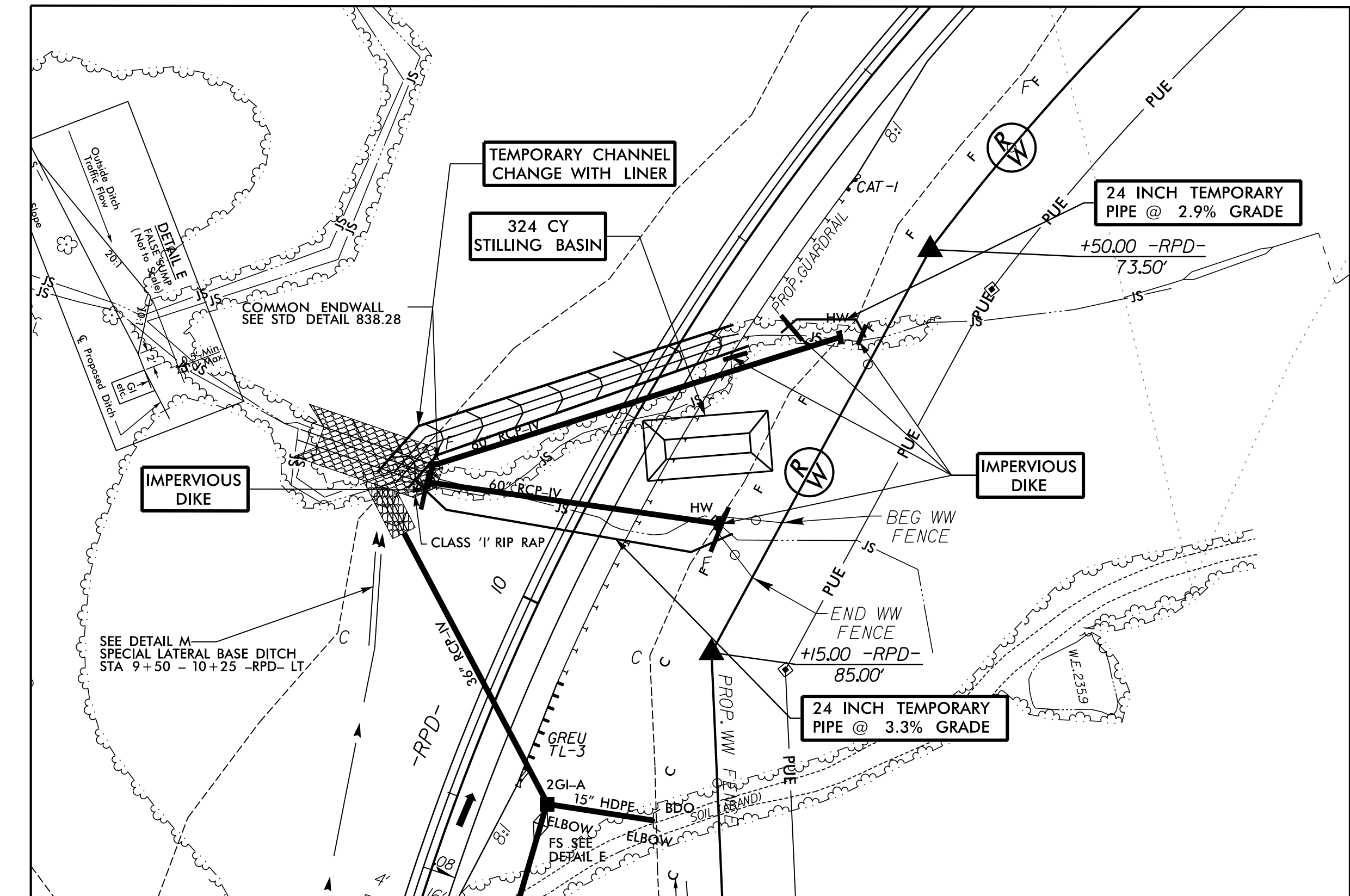
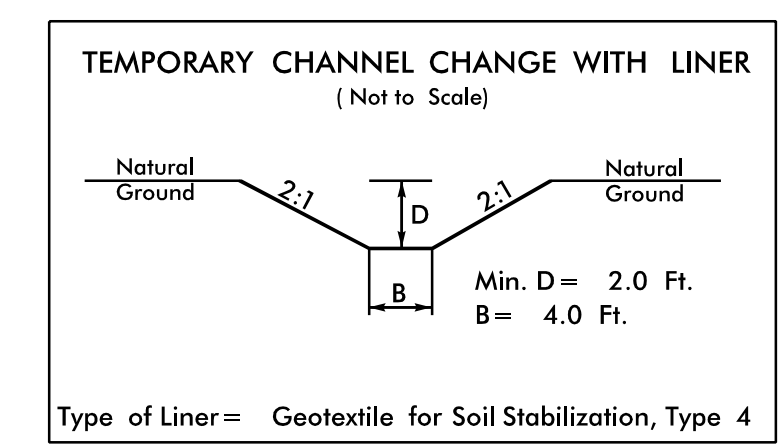
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PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-10A
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



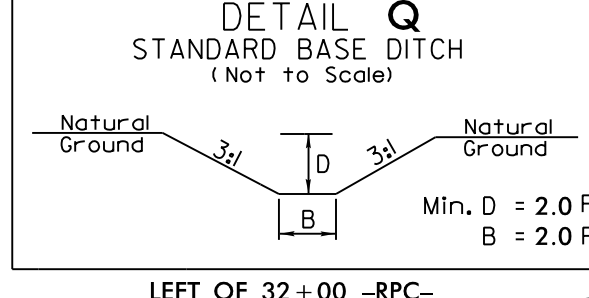
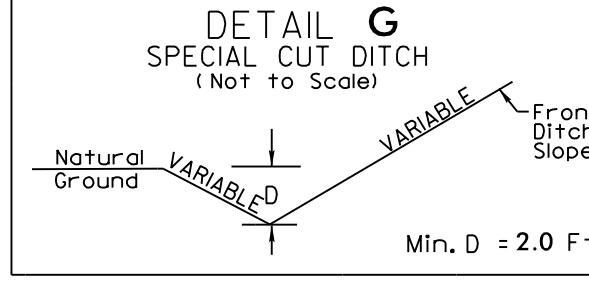
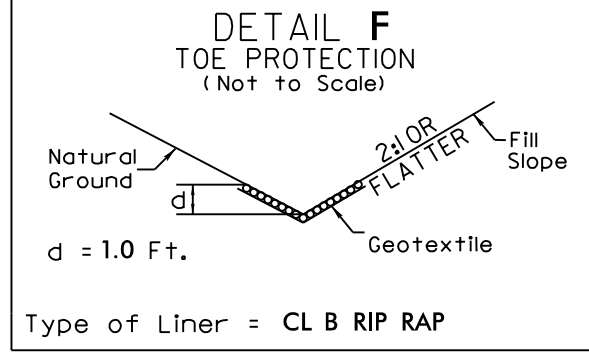
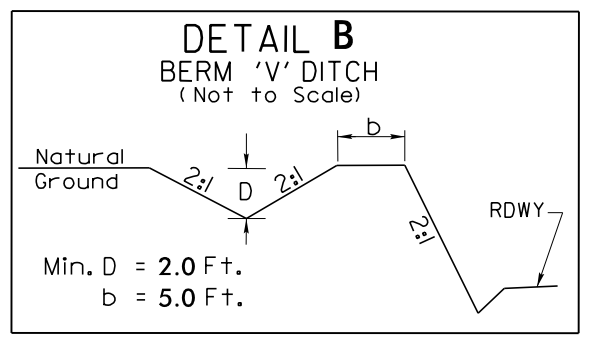
CULVERT CONSTRUCTION SEQUENCE -RPD- STA. 9+25

1. CONSTRUCT STILLING BASIN WITH A MINIMUM CAPACITY OF 324 C.Y.
2. CONSTRUCT TEMPORARY CHANNEL CHANGE WITH LINER (4 FT BASE; 2.0 FT DEEP; 2:1 SIDE SLOPES) AND INSTALL BOTH 24" TEMPORARY PIPES.
3. CONSTRUCT IMPERVIOUS DIKES AND DIVERT STREAMS THROUGH TEMPORARY CHANNEL AND TEMPORARY PIPES.
4. CONSTRUCT BOTH NEW 60" REINFORCED CONCRETE PIPES.
5. REMOVE IMPERVIOUS DIKES, TEMPORARY PIPES, TEMPORARY CHANNEL CHANGE AND DIVERT STREAMS THROUGH NEW REINFORCED CONCRETE PIPES.
6. REMOVE STILLING BASIN.
7. CONSTRUCT ROADWAY FILL.



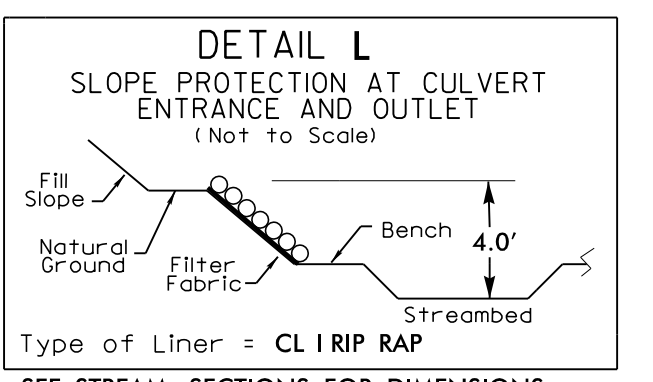
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PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-II/CONST.II
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



NOTE:
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILT CHECKS TYPE - A AT DRAINAGE OUTLETS.

CLEARING AND GRUBBING EROSION CONTROL FOR CONSTRUCTION SHEET 11



SEE STREAM SECTIONS FOR DIMENSIONS USE APPROX 35 LF. OF RIP RAP PER EACH APPROX 100 SY GEOTEXTILE FOR DRAINAGE TOTAL APPROX 80 TONS CL I RIP RAP TOTAL

-RPD- PI Sta 3+51.49 Δs = 5' 08" 06.8" Ls = 216.00' LT = 144.06' ST = 72.06'	-RPD- PI Sta 16+95.37 Δ = 93' 05" 45.8" (RT) D = 4' 45" 17.4" L = 1,957.92 T = 1,271.94 R = 1,205.00' S.E. = 0.08	-L2 REV- PI Sta 26+46.99 Δ = 7' 07" 51.2" (RT) D = 1' 26" 43.4" L = 493.35' T = 246.99' R = 3,964.00' S.E. = 0.04	-L2 REV- PI Sta 34+78.58 Δ = 18' 51" 57.5" (RT) D = 1' 56" 32.0" L = 971.36' T = 490.11' R = 2,950.00' S.E. = 0.06
---	--	--	---

-RPC- PI Sta 34+08.41 Δ = 42' 22" 57.6" (RT) D = 3' 49" 11.0" L = 1,109.58' T = 581.55' R = 1,500.00' S.E. = 0.08	PAUL V. SCHOLL, JR. DB 459 PG 332
--	--------------------------------------

-DRI-POT STA 12+09.51 = -L2_REV- STA 40+04.41

Modified Silt Basin Type 'B'
53 x 53 x 3
21 ft. weir
(See Tiered Skimmer Basin Detail)
ID A-183

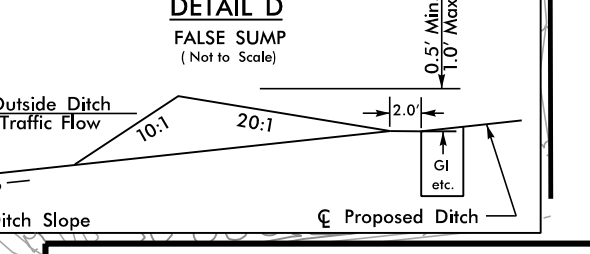
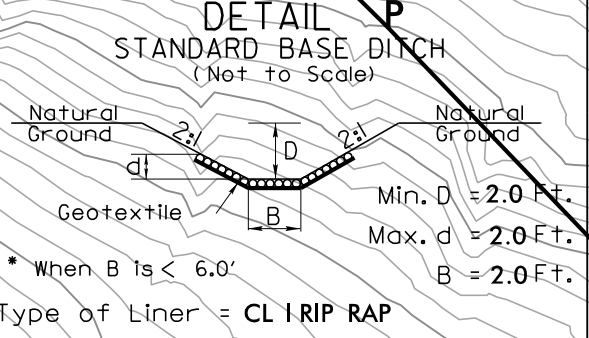
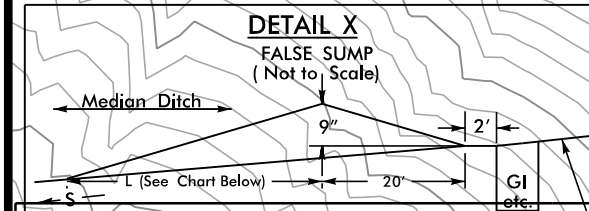
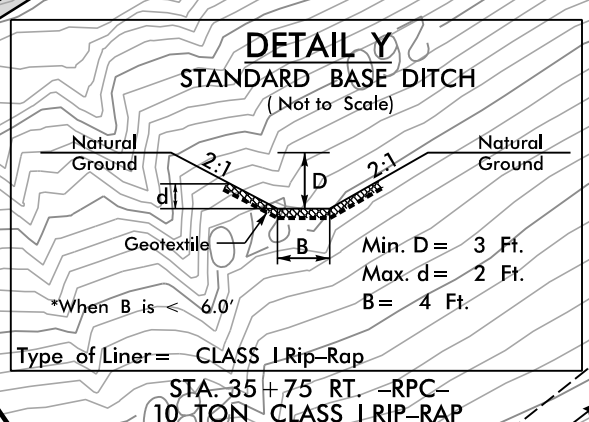
53 x 53 x 3
2.0 inch Skimmer with 1.75 inch Orifice Diameter
21 ft. weir
(See Tiered Skimmer Basin Detail)
ID A-183

35 x 72 x 3
1.5 inch Skimmer with 1.375 inch Orifice Diameter
10 ft. weir
ID A-184

80 x 40 x 3
2.0 inch Skimmer with 1.625 inch Orifice Diameter
12 ft. weir
ID A-148

73 x 36 x 3
1.5 inch Skimmer with 1.375 inch Orifice Diameter
10 ft. weir
ID A-186

ENVIRONMENTALLY SENSITIVE AREA
SEE PROJECT SPECIAL PROVISIONS



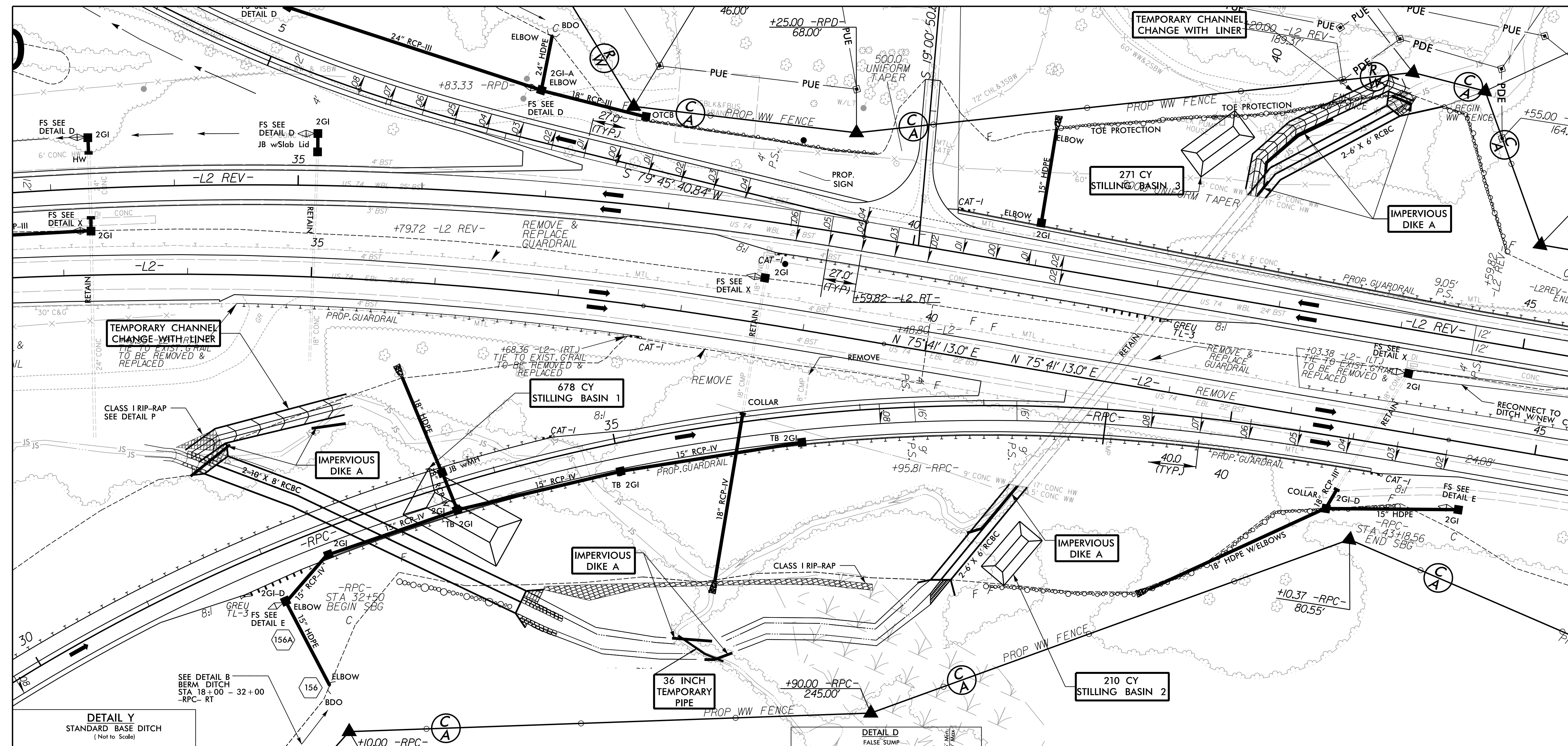
SEE SHEET NO. 34 FOR RPD PROFILE.
SEE SHEET NO. 35 FOR RPD PROFILE.
SEE SHEET NO. 39 AND NO. 40 FOR -L2 REV- /-L2 RT- PROFILE.

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MATCH LINE - RP C - STA. 38 + 00.00 SEE SHEET EC-12

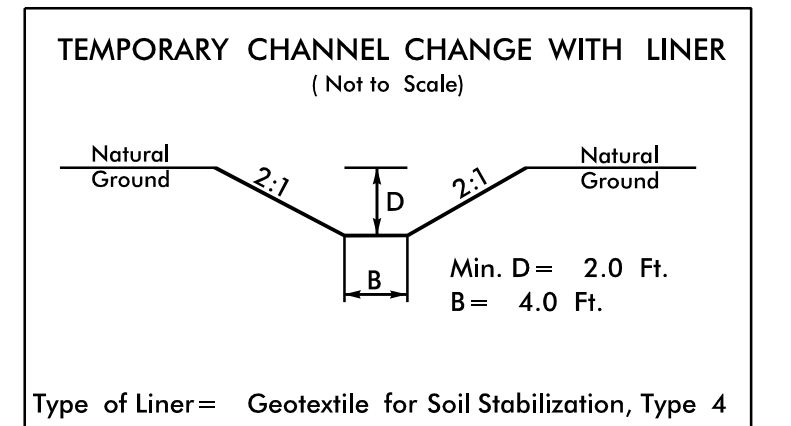
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PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-11A
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



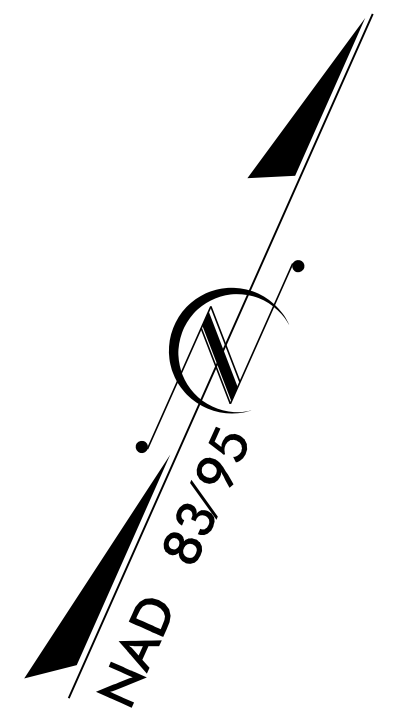
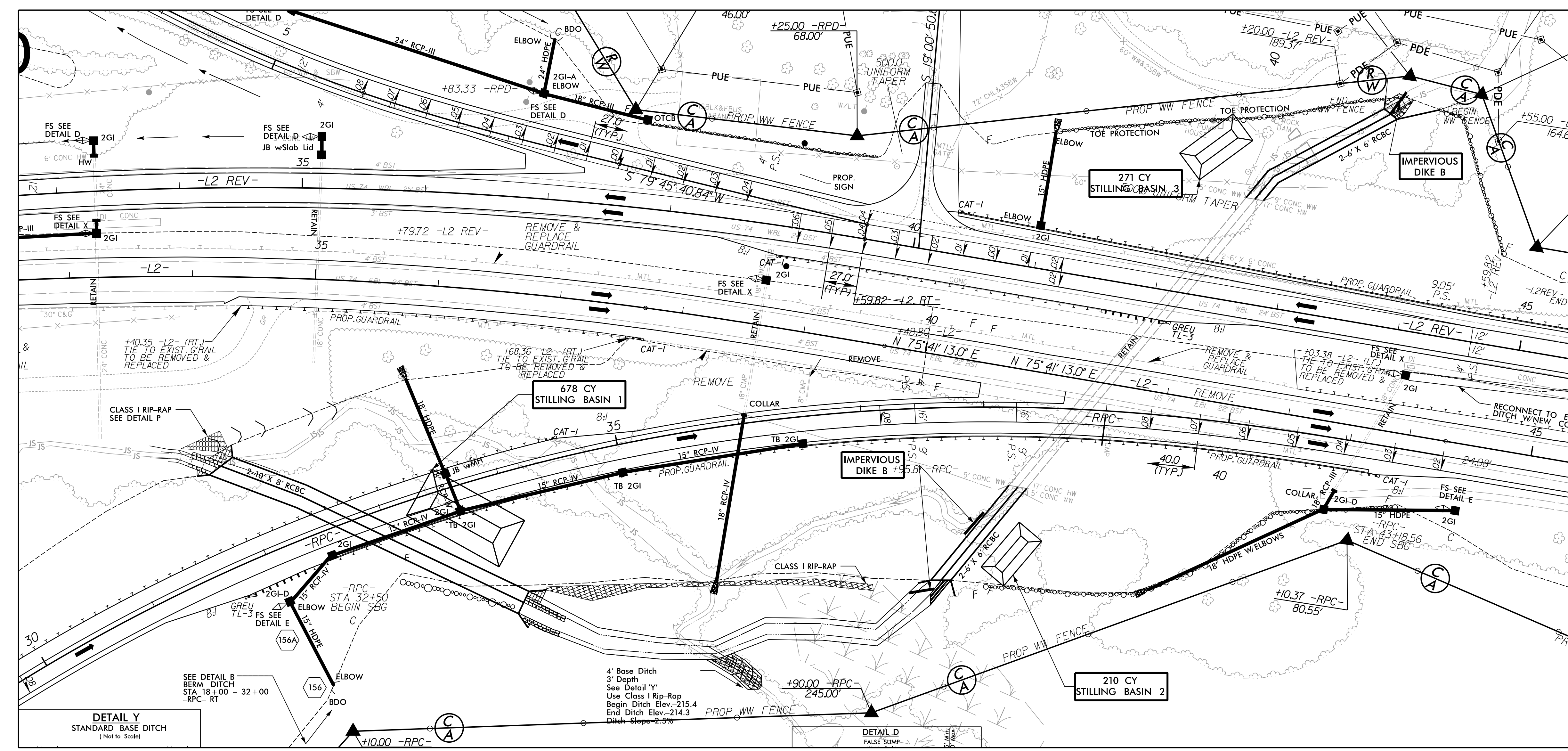
CULVERT CONSTRUCTION SEQUENCE -RPC- STA. 32+75 AND -L2- STA. 41+47

1. CONSTRUCT STILLING BASIN 1 WITH A MINIMUM CAPACITY OF 678 C.Y., STILLING BASIN 2 WITH A MINIMUM CAPACITY OF 210 C.Y. AND STILLING BASIN 3 WITH A MINIMUM CAPACITY OF 271 C.Y.
2. CONSTRUCT TEMPORARY CHANNEL CHANGE WITH LINER (4 FT BASE; 2.0 FT DEEP; 2:1 SIDE SLOPES) FOR -RPC- PROPOSED CULVERT & -L2- CULVERT EXTENSION.
3. CONSTRUCT IMPERVIOUS DIKES (A) FOR -RPC- PROPOSED CULVERT, AT BOTH ENDS OF -L2- CULVERT EXTENSION AND AT CHANNEL RELOCATION. INSTALL 36" TEMPORARY PIPE AT CHANNEL RELOCATION.
4. DIVERT STREAM THROUGH THE CHANNEL CHANGE AT -L2- CULVERT EXTENSION, THE WESTERN BARREL OF EXISTING CULVERT AND THROUGH TEMPORARY CHANNEL CHANGE AT -RPC- PROPOSED CULVERT.
5. CONSTRUCT THE ENTIRE LENGTH OF -RPC- PROPOSED CULVERT, INCLUDING THE WINGWALLS AND CHANNEL IMPROVEMENTS, ENTIRE LENGTH OF THE EASTERN BARREL AT INLET AND OUTLET ENDS, APPROXIMATELY 108 LF OF THE WESTERN BARREL AT THE INLET END AND APPROXIMATELY 60 LF AT THE OUTLET END OF -L2REV- CULVERT.
6. CONSTRUCT ENTIRE LENGTH OF CHANNEL RELOCATION BETWEEN INLET END OF -RPC- CULVERT AND OUTLET END OF -L2- CULVERT EXTENSION.



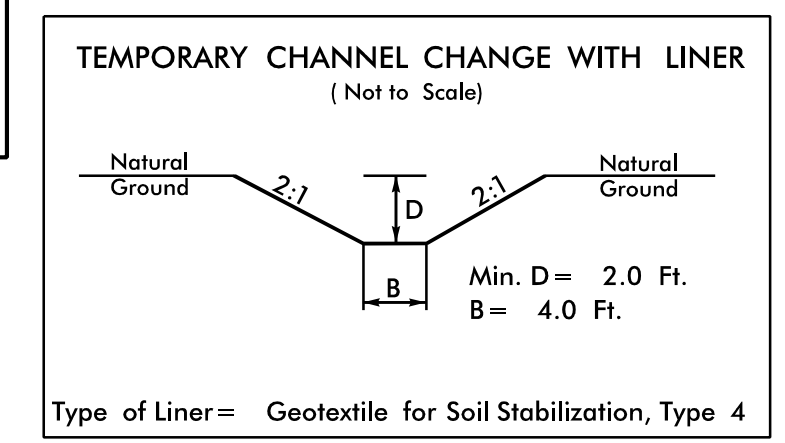
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PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-11B
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



CULVERT CONSTRUCTION SEQUENCE -RPC- STA. 32+75 AND -L2- STA. 41+47

1. CONSTRUCT IMPERVIOUS DIKES (B) AS SHOWN ON PLAN. REMOVE IMPERVIOUS DIKES (A) AND TEMPORARY PIPES FROM PHASE I.
2. DIVERT STREAM THROUGH NEW EASTERN BARREL AT INLET AND OUTLET END OF -L2- CULVERT EXTENSIONS, NEW RELOCATED STREAM, AND BOTH BARRELS OF NEW -RPC- CULVERTS. FILL IN BOTH TEMPORARY CHANNEL CHANGES.
3. CONSTRUCT REMAINING SECTIONS OF WESTERN BARRELS AT BOTH THE INLET AND OUTLET ENDS OF -L2- CULVERT EXTENSION. USING PUMP AROUND OPERATIONS, INSTALL RIP RAP ON EXISTING STREAM ENTERING STREAM RELOCATION CHANNEL SHOWN ON PLAN.
4. REMOVE IMPERVIOUS DIKES (B) AND ALLOW STREAM TO FLOW THROUGH BOTH BARRELS OF -L2 CULVERT EXTENSION.
5. REMOVE STILLING BASINS.
6. CONSTRUCT ROADWAY FILL.

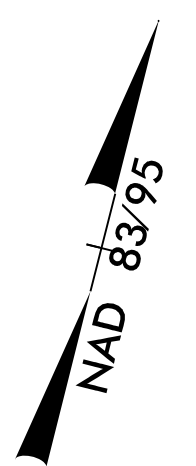


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NOTE:
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILT CHECKS TYPE - A AT DRAINAGE OUTLETS.

CLEARING AND GRUBBING EROSION CONTROL FOR CONSTRUCTION SHEET 12

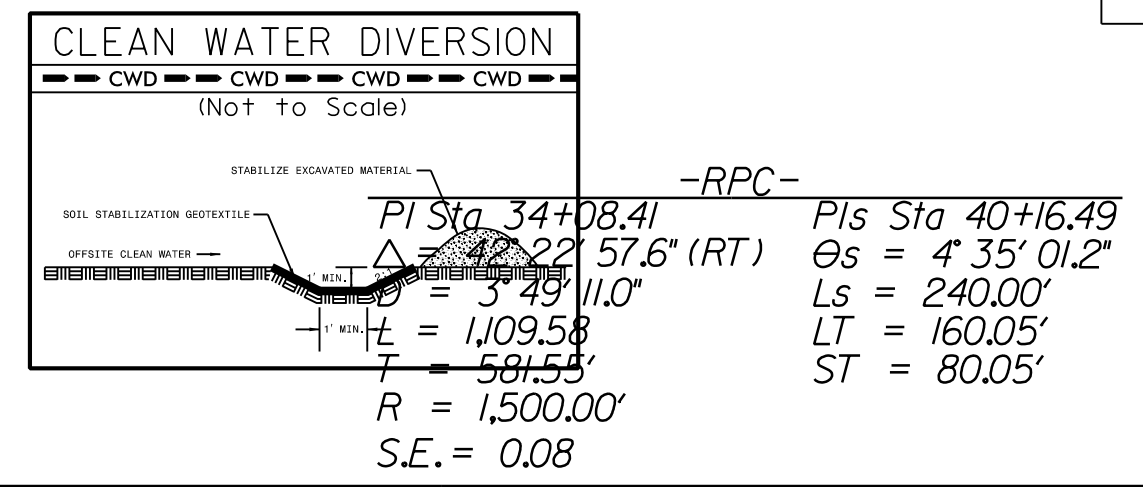
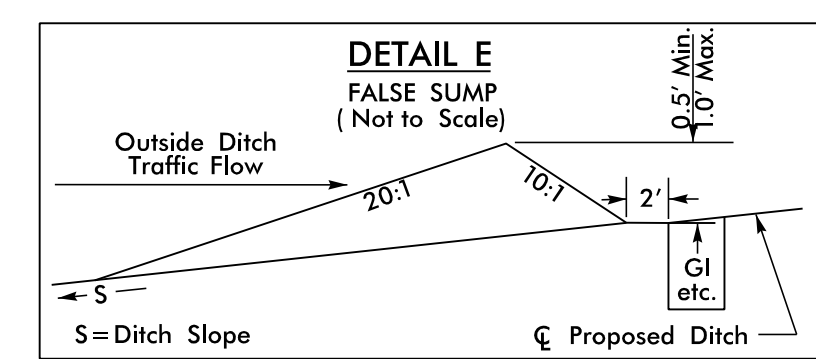
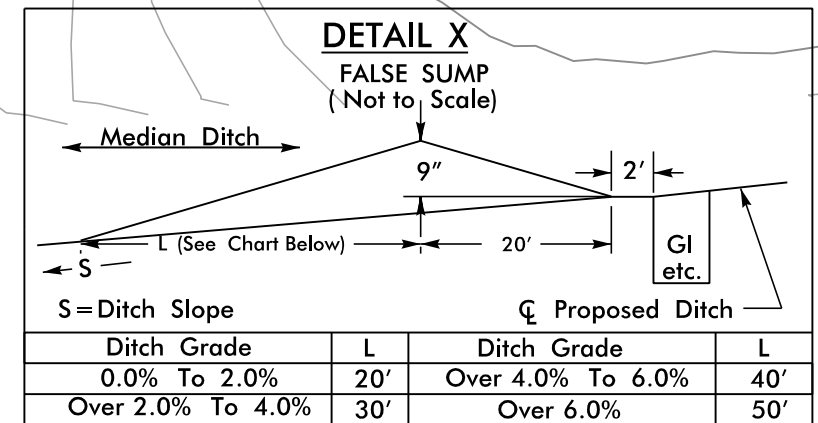
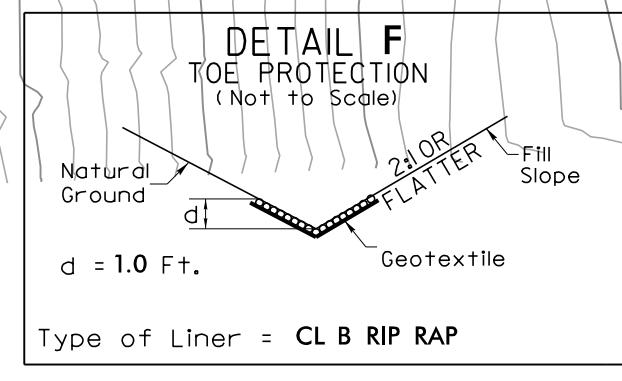
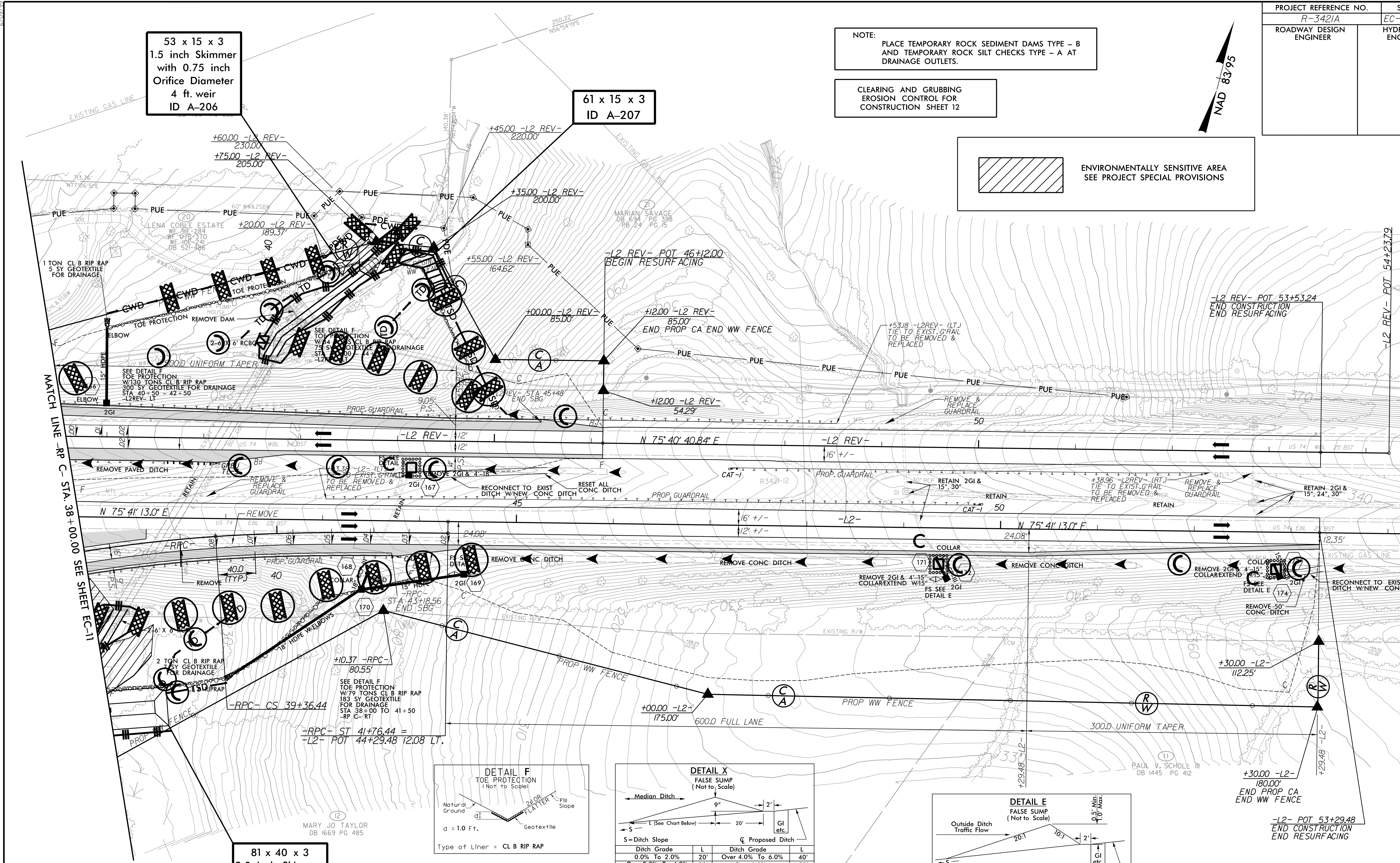
ENVIRONMENTALLY SENSITIVE AREA SEE PROJECT SPECIAL PROVISIONS



53 x 15 x 3
1.5 inch Skimmer
with 0.75 inch
Orifice Diameter
4 ft. weir
ID A-206

61 x 15 x 3
ID A-207

81 x 40 x 3
2.0 inch Skimmer
with 1.625 inch
Orifice Diameter
12 ft. weir
ID A-187



SEE SHEET NO. 34 FOR -RPC- PROFILE.
SEE SHEET NO. 40 FOR -L2 REV- PROFILE.

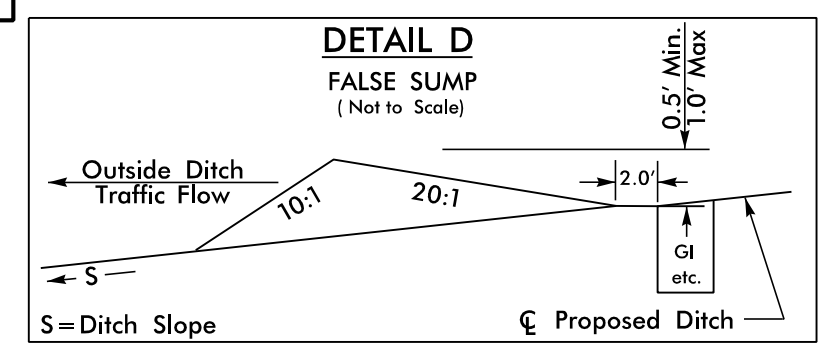
PAVEMENT REMOVAL

6/10/2018
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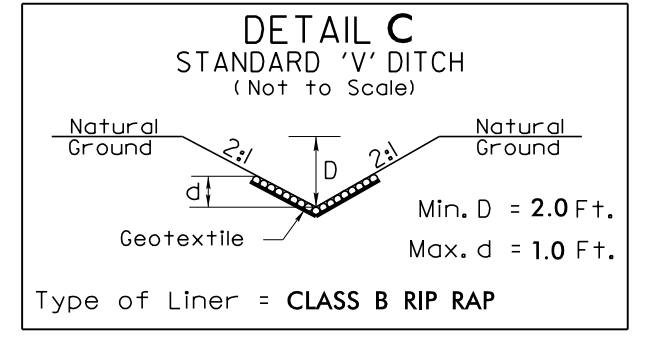
PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-13/CONST.13
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

NOTE:
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILT CHECKS TYPE - A AT DRAINAGE OUTLETS.

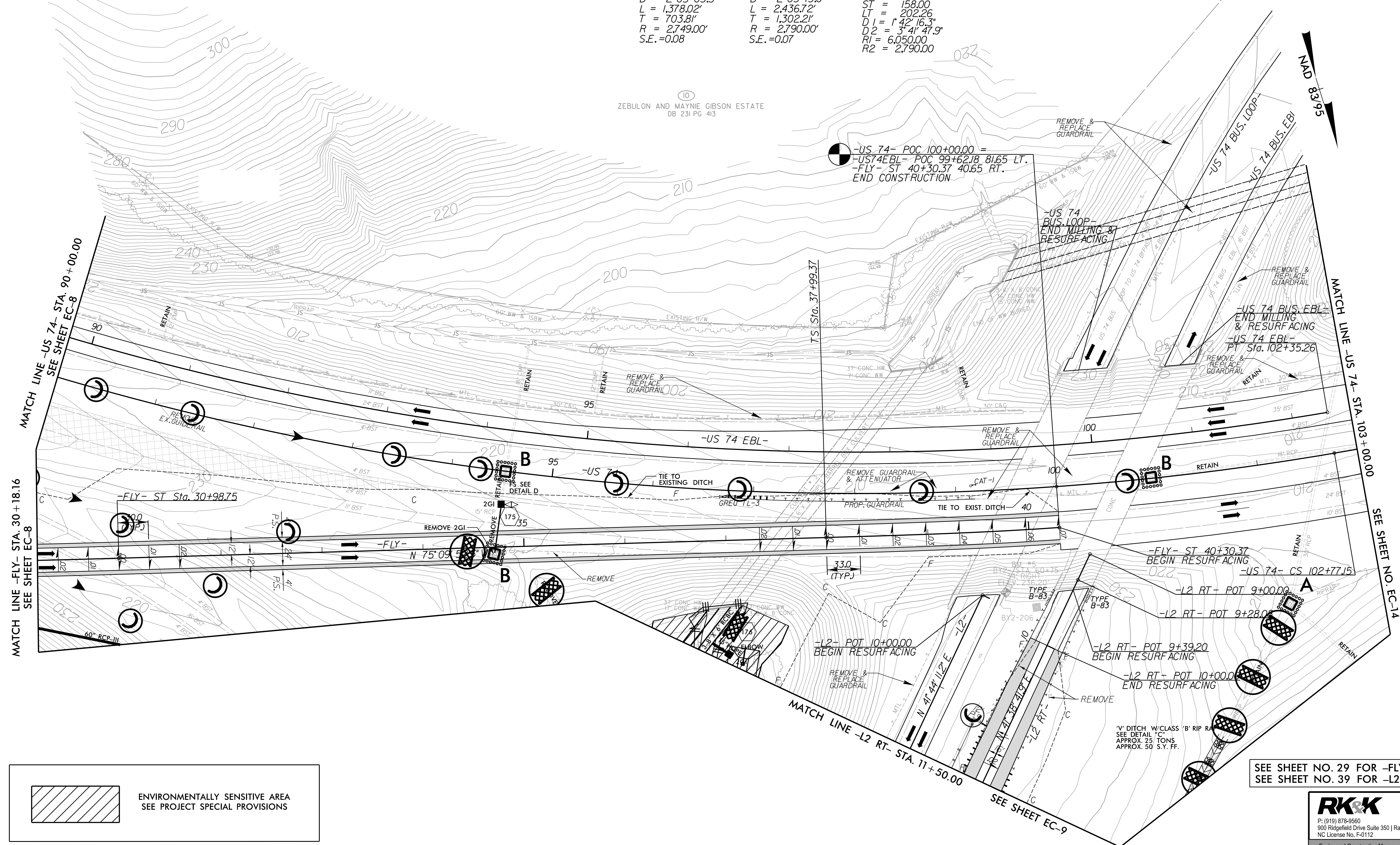
CLEARING AND GRUBBING EROSION CONTROL FOR CONSTRUCTION SHEET 13



-FLY-		-FLY-	
PIs Sta 12+52.55	PI Sta 21+55.94	PIs Sta 29+38.80	PIs Sta 39+53.39
$\theta_s = 4' 11'' 32.5''$	$\Delta = 53' 19'' 18.0''$ (LT)	$\theta_s = 4' 11'' 32.5''$	$\theta_s = 2' 20'' 16.3''$
Ls = 240.00'	D = 3' 29' 37.1"	Ls = 240.00'	Ls = 231.00'
LT = 160.04'	L = 1,526.25'	LT = 160.04'	LT = 154.01'
ST = 80.04'	T = 823.43'	ST = 80.04'	ST = 77.01'
	R = 1,640.00'		
	S.E. = 0.08		



-US 74 EBL-		-US 74-	
PI Sta 95+61.05	PI Sta 91+42.64	PIs Sta 104+35.15	
$\Delta = 28' 43'' 16.5''$ (LT)	$\Delta = 50' 02'' 26.6''$ (LT)	$\theta_s = 5' 24'' 04.2''$	
D = 2' 05' 03.3"	D = 2' 03' 13.0"	Ls = 360.00	
L = 1,378.02'	L = 2,436.72'	ST = 158.00	
T = 703.81'	T = 1,302.21'	LT = 202.26	
R = 2,749.00'	R = 2,790.00'	D1 = 1' 42' 16.3"	
S.E. = 0.08	S.E. = 0.07	D2 = 3' 41' 47.9"	
		RI = 6,050.00	
		R2 = 2,790.00	



ENVIRONMENTALLY SENSITIVE AREA
SEE PROJECT SPECIAL PROVISIONS

SEE SHEET NO. 29 FOR -FLY- PROFILE.
SEE SHEET NO. 39 FOR -L2 RT- PROFILE.

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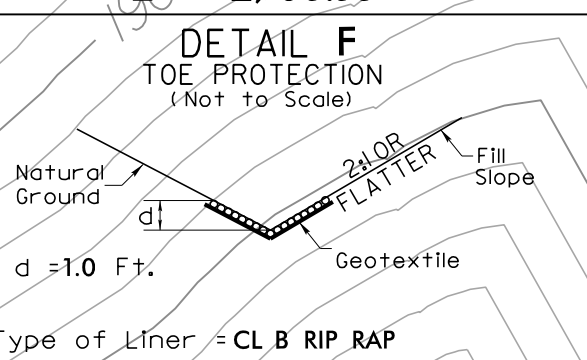
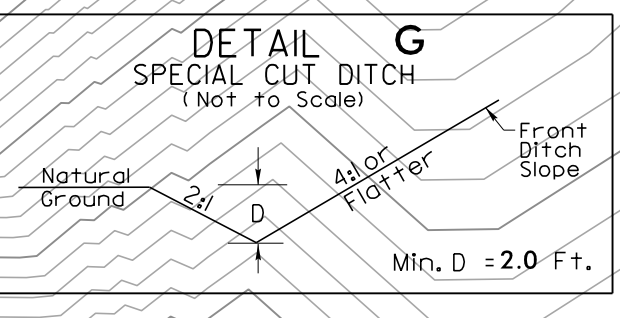
PAVEMENT REMOVAL

6/10/08
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PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-14/CONST.14
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

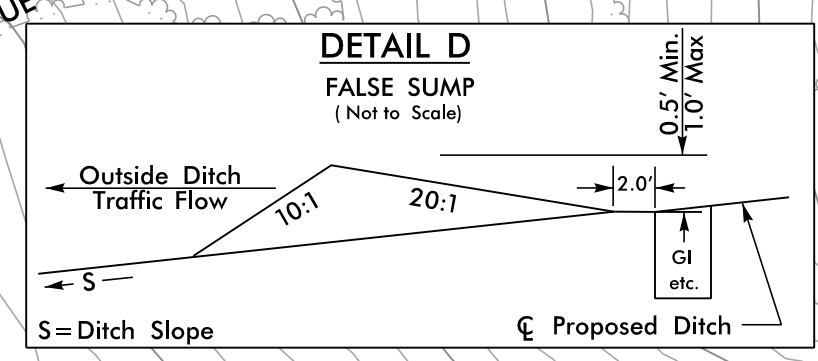
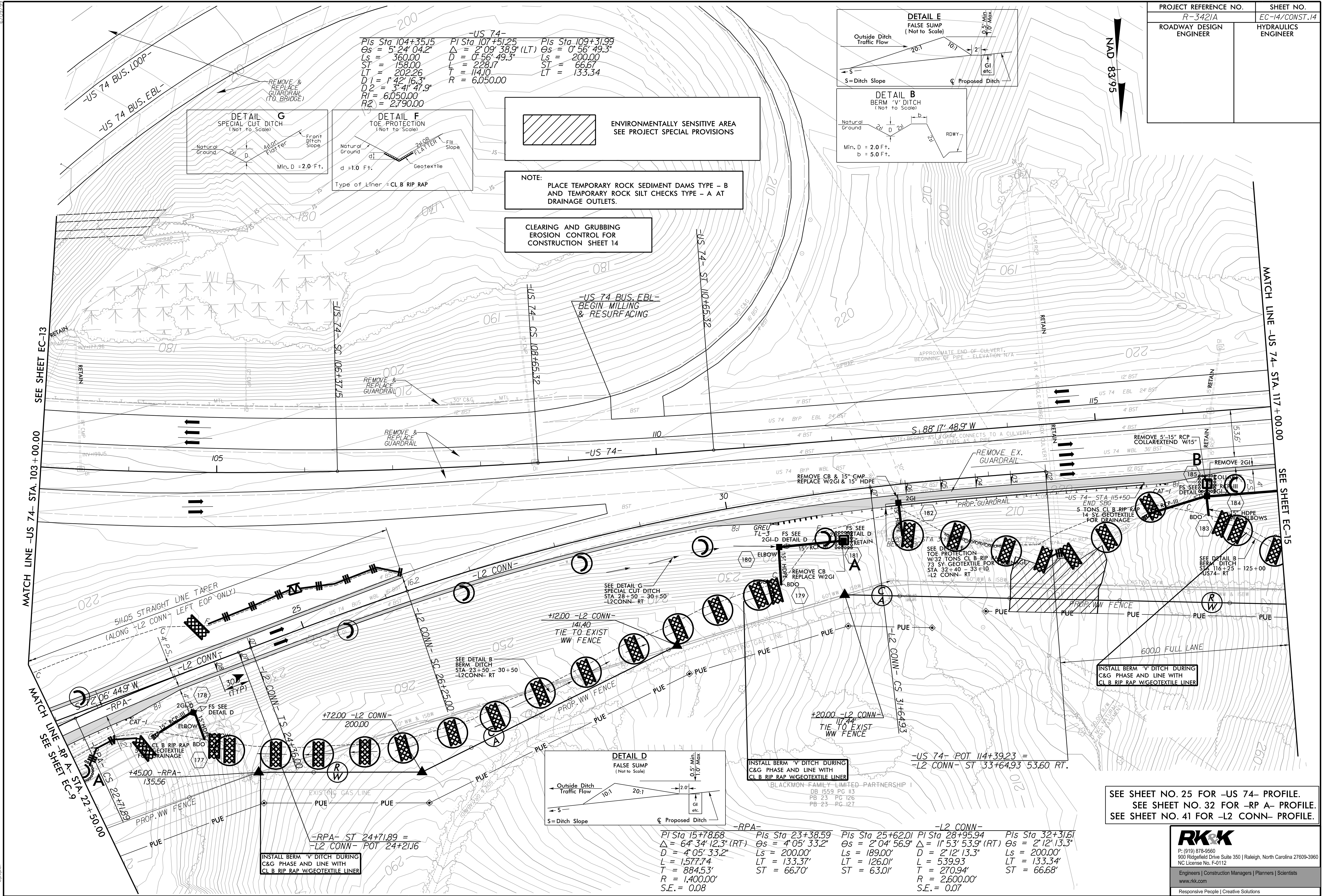
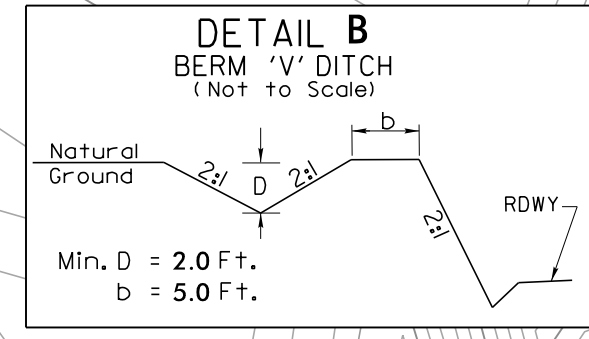
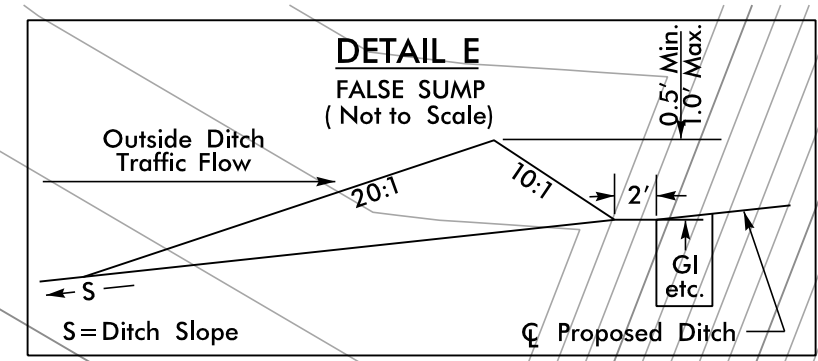
NAD 83 95

-US 74-
 PIs Sta 104+35.15 PI Sta 107+51.25 PIs Sta 109+31.99
 $\Delta = 5^{\circ} 24' 04.2''$ $\Delta = 2^{\circ} 09' 38.9''$ (LT) $\Delta = 0^{\circ} 56' 49.3''$
 $L_s = 360.00$ $D = 0^{\circ} 56' 49.3''$ $L_s = 200.00$
 $ST = 158.00$ $L = 228.17$ $ST = 66.67$
 $LT = 202.26$ $T = 114.10$ $LT = 133.34$
 $D1 = 1^{\circ} 42' 16.3''$ $R = 6,050.00$
 $D2 = 3^{\circ} 41' 47.9''$
 $R1 = 6,050.00$
 $R2 = 2,790.00$



NOTE:
 PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILT CHECKS TYPE - A AT DRAINAGE OUTLETS.

CLEARING AND GRUBBING EROSION CONTROL FOR CONSTRUCTION SHEET 14



INSTALL BERM 'V' DITCH DURING C&G PHASE AND LINE WITH CL B RIP RAP W/GEOTEXTILE LINER

SEE SHEET NO. 25 FOR -US 74- PROFILE.
 SEE SHEET NO. 32 FOR -RP A- PROFILE.
 SEE SHEET NO. 41 FOR -L2 CONN- PROFILE.

-RPA-
 PIs Sta 15+78.68 PIs Sta 23+38.59 PIs Sta 25+62.01 PIs Sta 28+95.94 PIs Sta 32+31.61
 $\Delta = 64^{\circ} 34' 12.3''$ (RT) $\Delta = 4^{\circ} 05' 33.2''$ $\Delta = 2^{\circ} 04' 56.9''$ $\Delta = 11^{\circ} 53' 53.9''$ (RT) $\Delta = 2^{\circ} 12' 13.3''$
 $D = 4^{\circ} 05' 33.2''$ $L_s = 200.00'$ $L_s = 189.00'$ $D = 2^{\circ} 12' 13.3''$ $L_s = 200.00'$
 $L = 1,577.74$ $LT = 133.37'$ $L = 539.93$ $L = 539.93$ $LT = 133.34'$
 $T = 884.53'$ $ST = 66.70'$ $T = 270.94'$ $T = 270.94'$ $ST = 66.68'$
 $R = 1,400.00'$ $ST = 63.01'$ $R = 2,600.00'$
 $S.E. = 0.08$ $S.E. = 0.07$

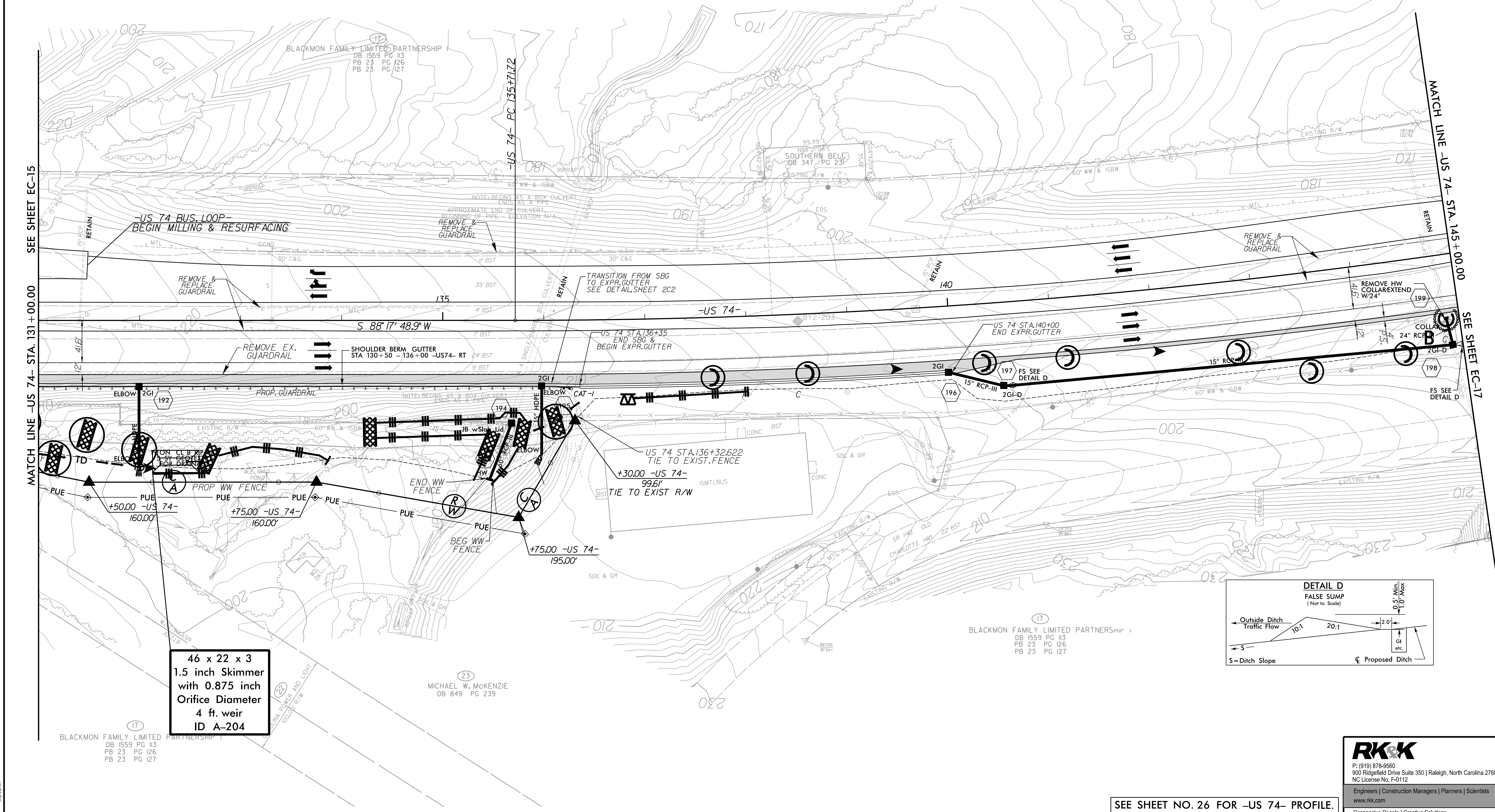
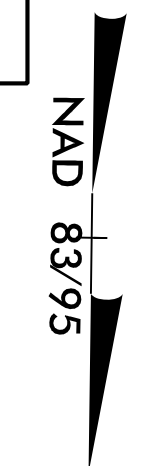
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PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-16/CONST.16
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

-US 74-
 PI Sta 140+67.57
 $\Delta = 8' 43" 28.9" (LT)$
 $D = 0' 52" 53.3"$
 $L = 989.786$
 $T = 495.85$
 $R = 6,500.00$

NOTE:
 PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B
 AND TEMPORARY ROCK SILT CHECKS TYPE - A AT
 DRAINAGE OUTLETS.

CLEARING AND GRUBBING
 EROSION CONTROL FOR
 CONSTRUCTION SHEET 16



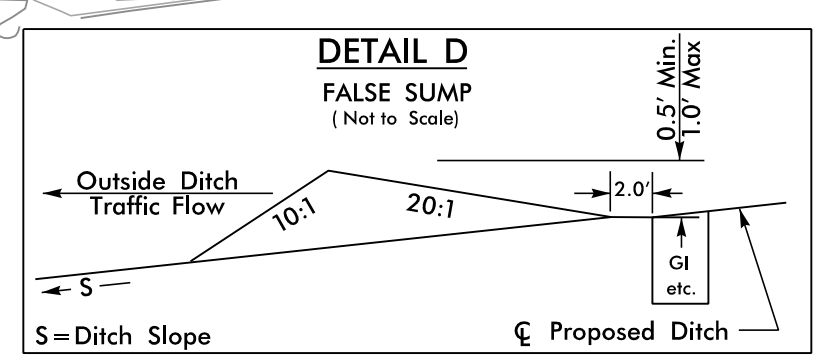
SEE SHEET EC-15

MATCH LINE -US 74- STA. 131+00.00

MATCH LINE -US 74- STA. 145+00.00

SEE SHEET EC-17

46 x 22 x 3
 1.5 inch Skimmer
 with 0.875 inch
 Orifice Diameter
 4 ft weir
 ID A-204



BLACKMON FAMILY LIMITED PARTNERSHIP
 DB 1559 PG 113
 PB 23 PG 126
 PB 23 PG 127

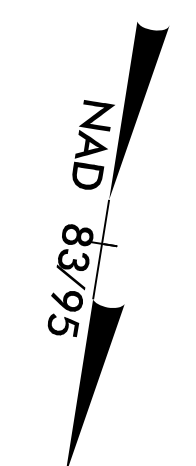
MICHAEL W. MCKENZIE
 DB 849 PG 239

BLACKMON FAMILY LIMITED PARTNERSHIP
 DB 1559 PG 113
 PB 23 PG 126
 PB 23 PG 127

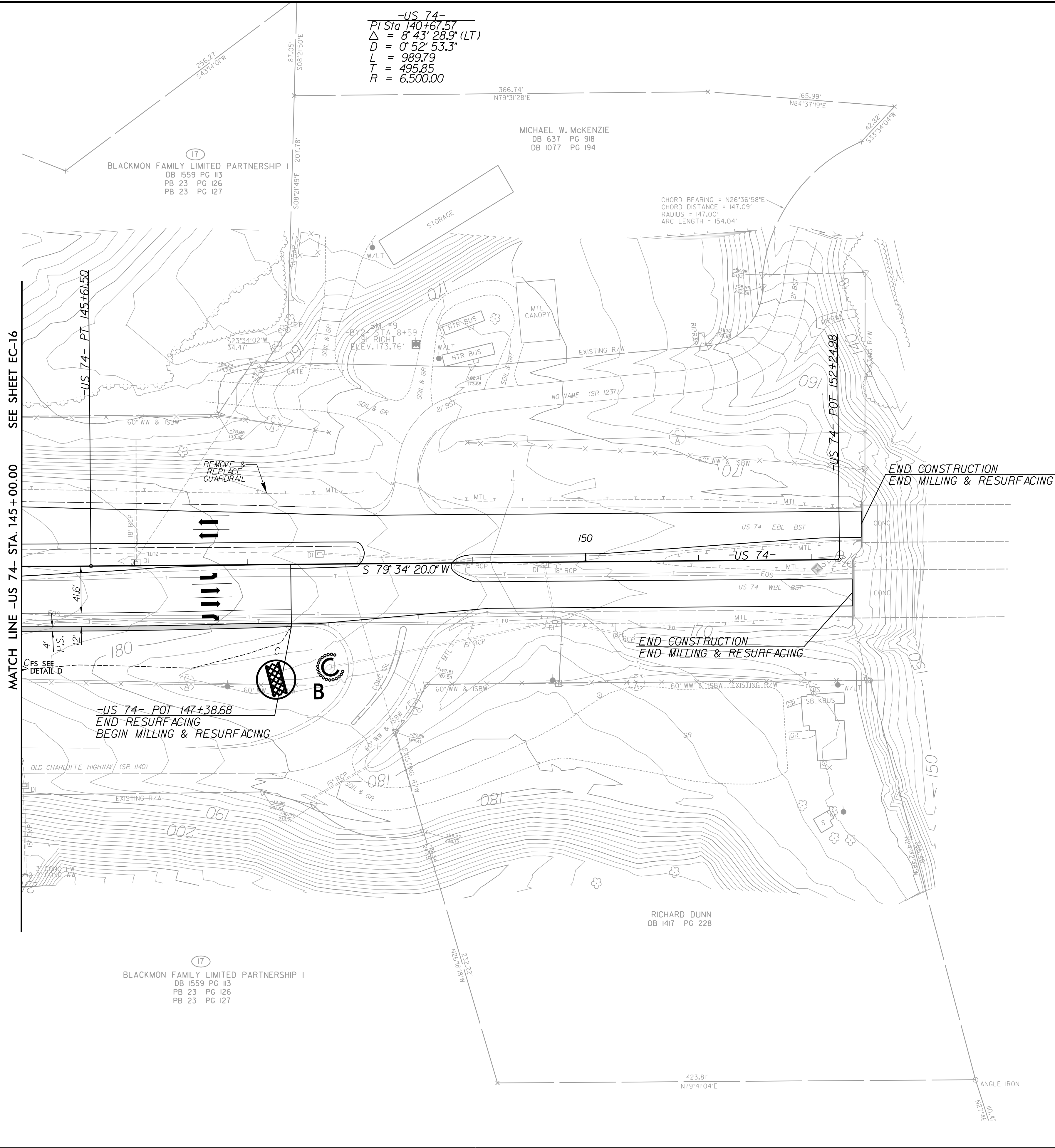
SEE SHEET NO. 26 FOR -US 74- PROFILE.

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PROJECT REFERENCE NO.		SHEET NO.	
R-3421A		EC-17/CONST.17	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	



-US 74-
 PI Sta 140+67.57
 $\Delta = 8^{\circ} 43' 28.9" (LT)$
 $D = 0^{\circ} 52' 53.3"$
 $L = 989.79$
 $T = 495.85$
 $R = 6,500.00$



NOTE:
 PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B
 AND TEMPORARY ROCK SILT CHECKS TYPE - A AT
 DRAINAGE OUTLETS.

CLEARING AND GRUBBING
 EROSION CONTROL FOR
 CONSTRUCTION SHEET 17

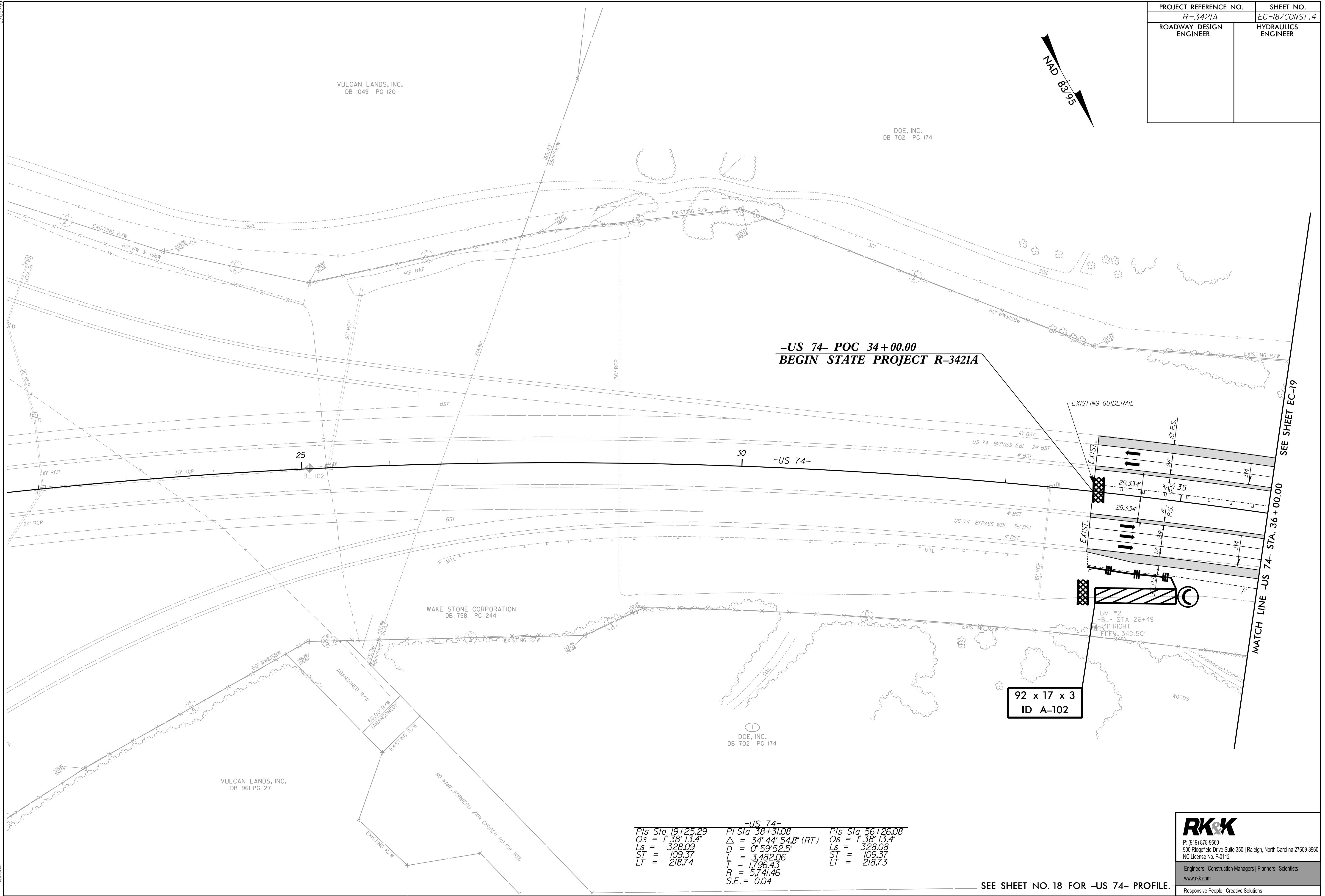
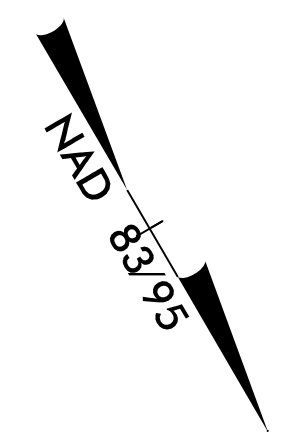
MATCH LINE -US 74- STA. 145+00.00 SEE SHEET EC-16

SEE SHEET NO. 26 FOR -US 74- PROFILE.

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PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-18/CONST. 4
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



**-US 74- POC 34+00.00
BEGIN STATE PROJECT R-3421A**

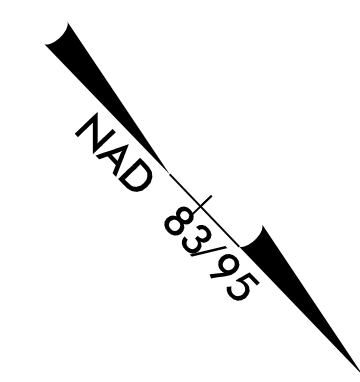
SEE SHEET EC-19
MATCH LINE -US 74- STA. 36+00.00

-US 74-		
PIs Sta 19+25.29	PI Sta 38+31.08	PIs Sta 56+26.08
Os = 1° 38' 13.4"	Δ = 34° 44' 54.8" (RT)	Os = 1° 38' 13.4"
Ls = 328.09	D = 0° 59' 52.5"	Ls = 328.08
ST = 109.37	L = 3,482.06	ST = 109.37
LT = 218.74	T = 1,796.43	LT = 218.73
	R = 5,741.46	
	S.E. = 0.04	

SEE SHEET NO. 18 FOR -US 74- PROFILE.

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6/19/09
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 7/1/2018
 C:\Hydraulics\CADD\ERession Control\3421a_EC_pah18.dgn

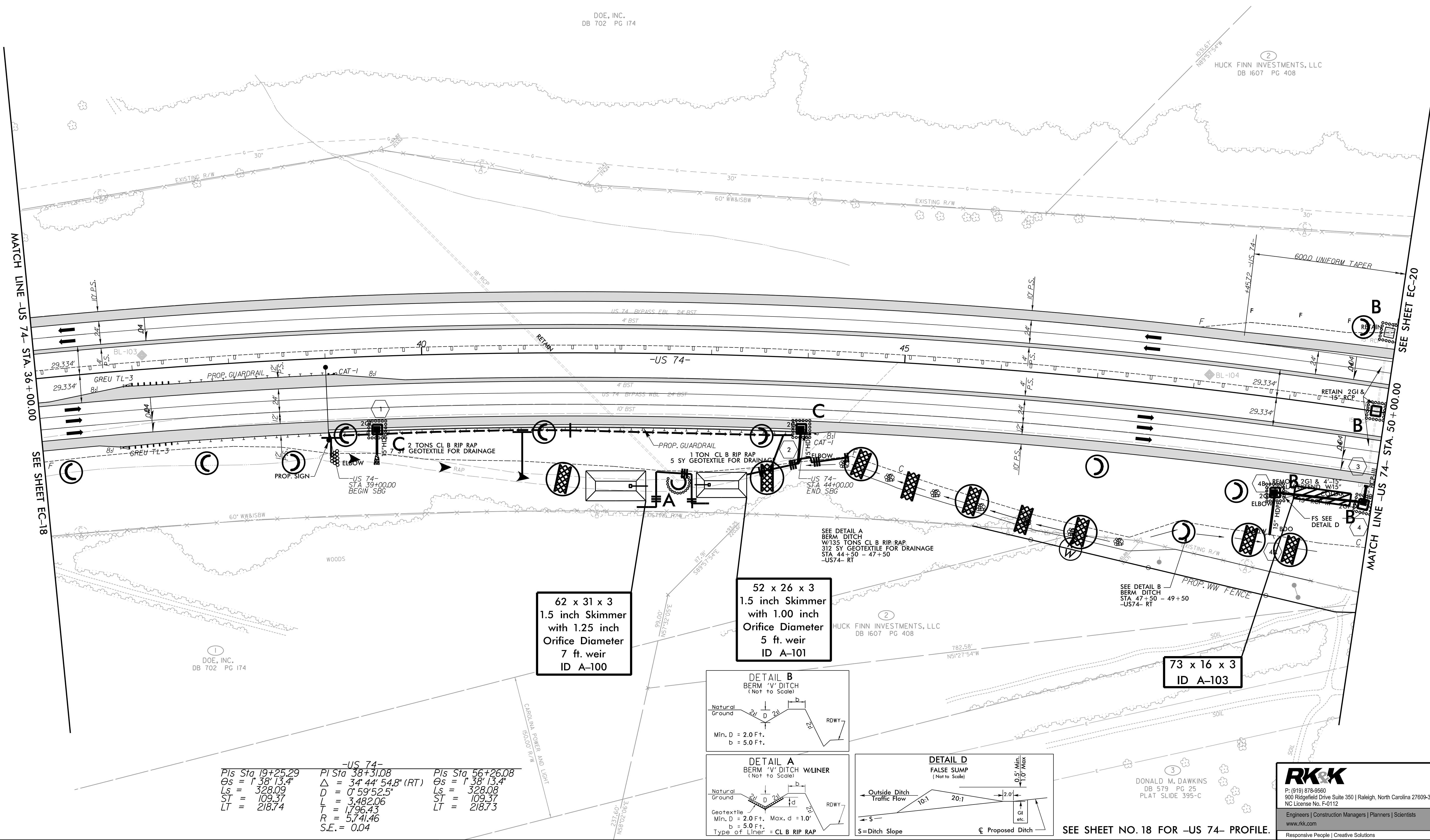


DOE, INC.
DB 702 PG 174

HUCK FINN INVESTMENTS, LLC
DB 1607 PG 408

MATCH LINE -US 74- STA. 36+00.00

MATCH LINE -US 74- STA. 50+00.00

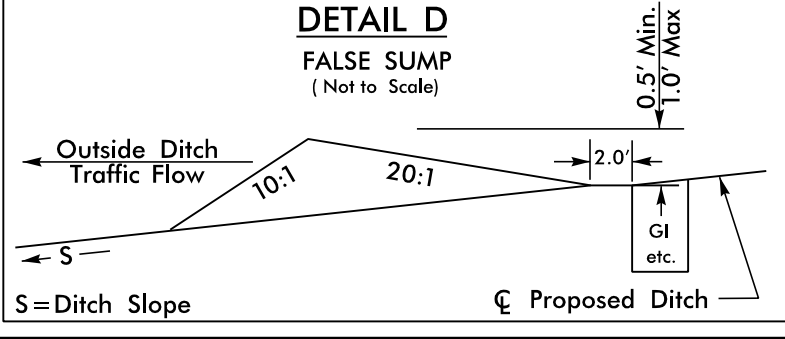
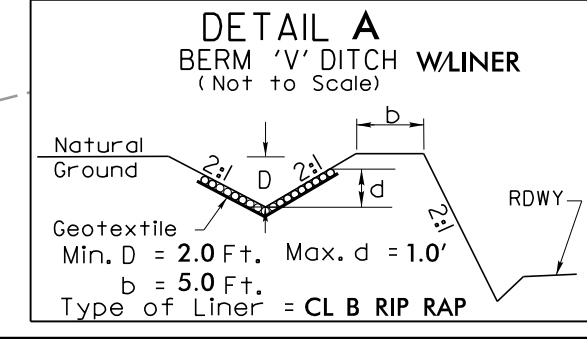
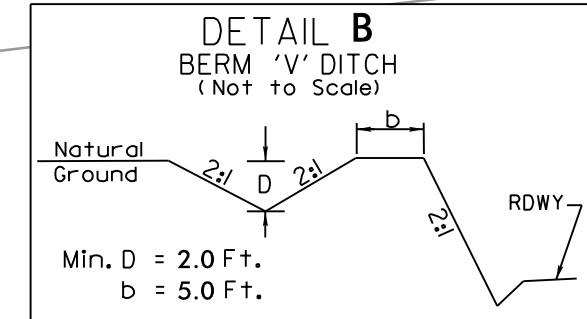


62 x 31 x 3
1.5 inch Skimmer
with 1.25 inch
Orifice Diameter
7 ft. weir
ID A-100

52 x 26 x 3
1.5 inch Skimmer
with 1.00 inch
Orifice Diameter
5 ft. weir
ID A-101

73 x 16 x 3
ID A-103

-US 74-		
PIs Sta 19+25.29	PI Sta 38+31.08	PIs Sta 56+26.08
Os = 1' 38" 13.4"	Δ = 34' 44" 54.8" (RT)	Os = 1' 38" 13.4"
Ls = 328.09	D = 0' 59" 52.5"	Ls = 328.08
ST = 109.37	L = 3,482.06	ST = 109.37
LT = 218.74	L = 1,796.43	LT = 218.73
	R = 5,741.46	
	S.E. = 0.04	



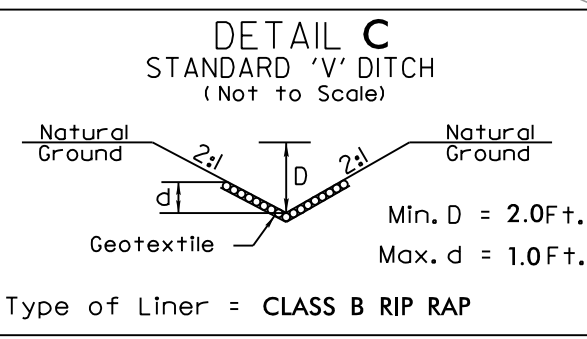
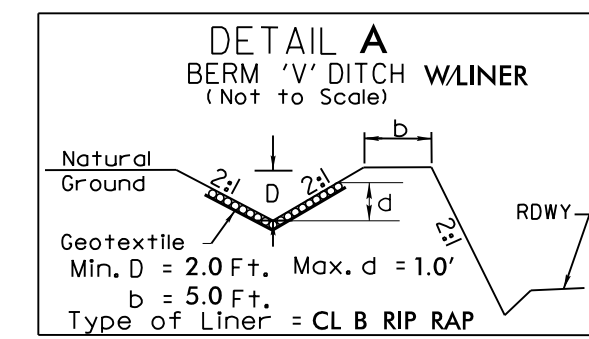
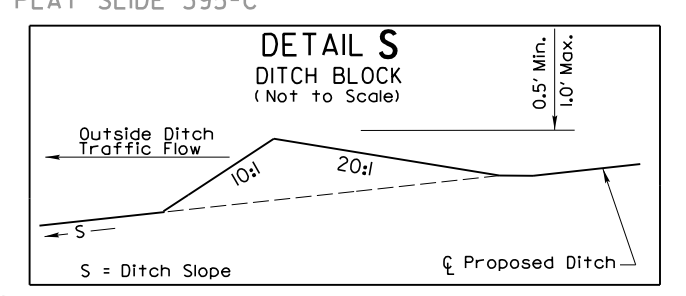
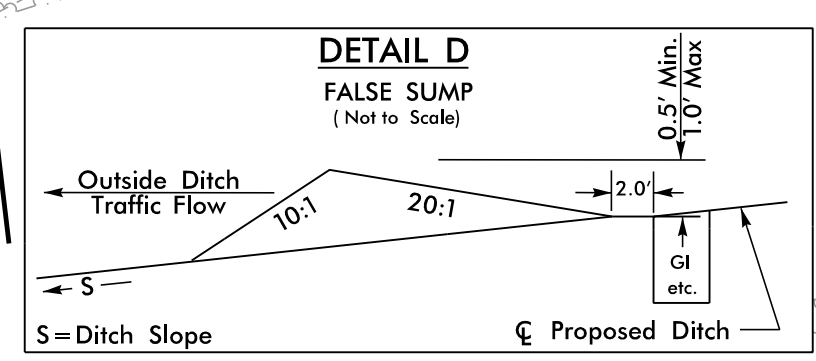
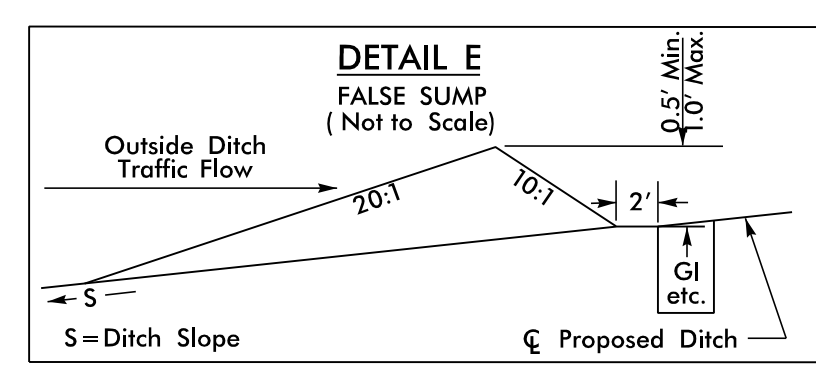
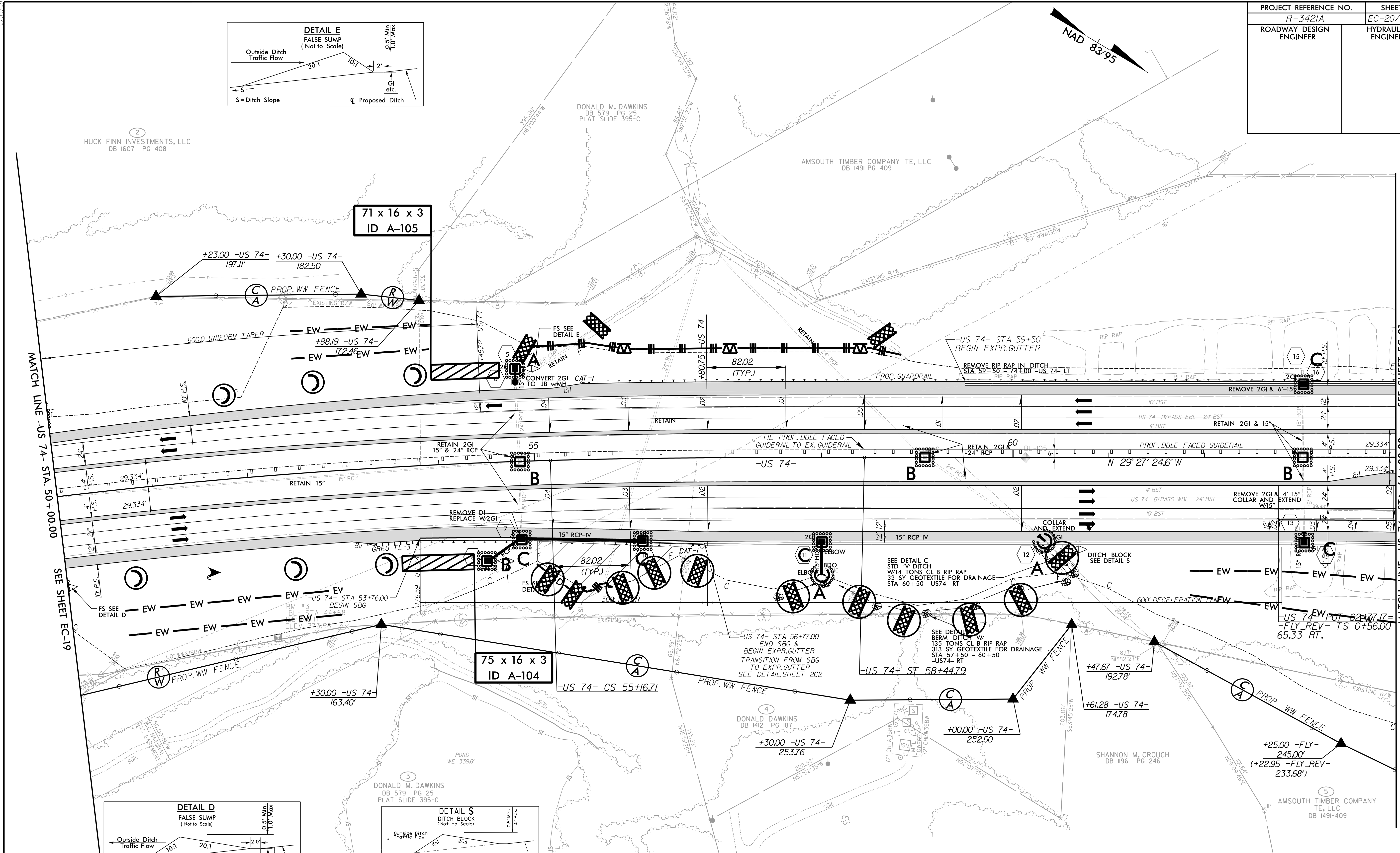
SEE SHEET NO. 18 FOR -US 74- PROFILE.

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DONALD M. DAWKINS
DB 579 PG 25
PLAT SLIDE 395-C

6/19/2018 11:28:18 AM \\Hydraulics\CADD\p19\EC-19\EC-pah19.dgn



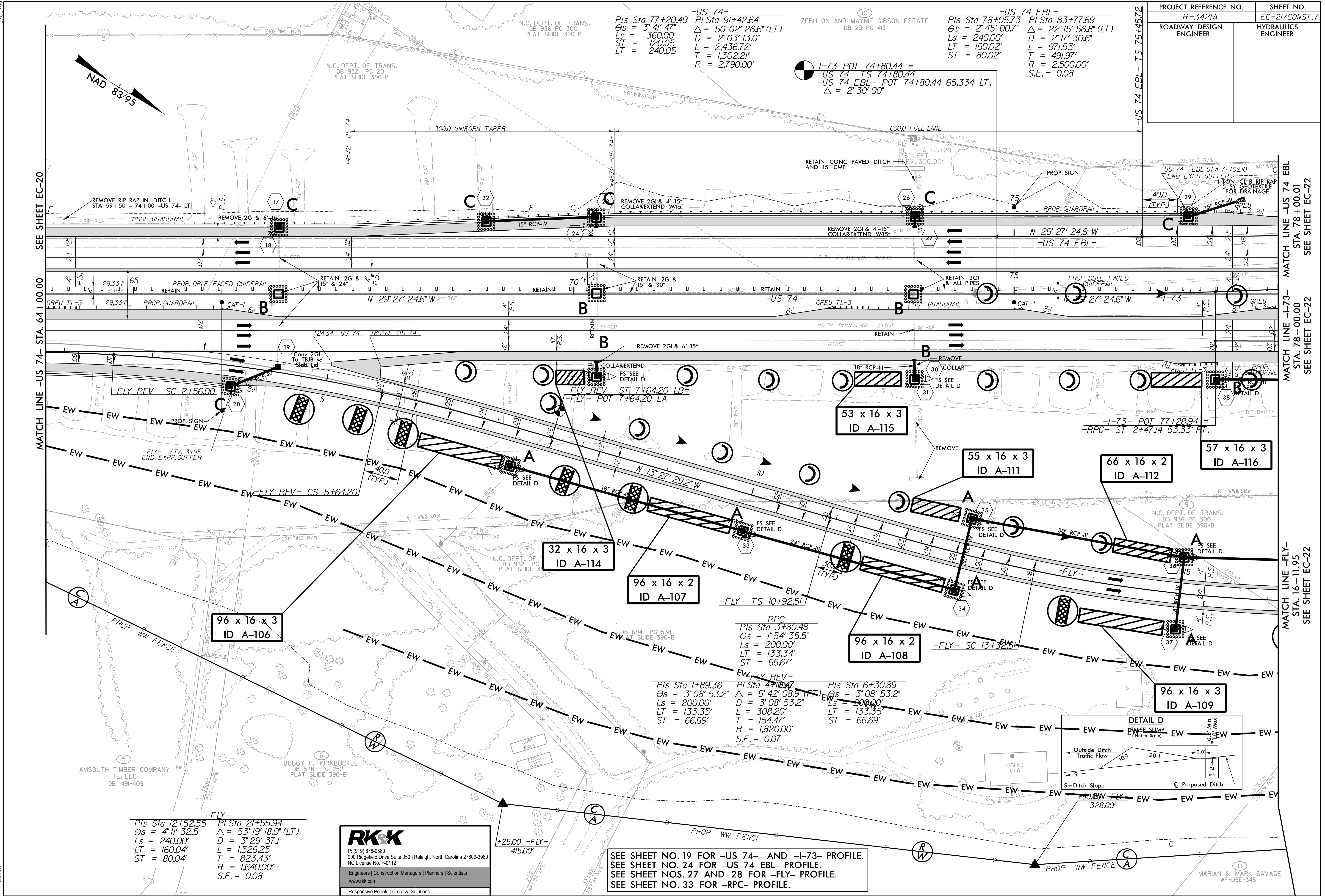
-US 74-		-FLY REV-	
PIs Sta 19+25.29	PI Sta 38+31.08	PIs Sta 56+26.08	PI Sta 4+10.47
$\Theta_s = 1' 38'' 13.4''$	$\Delta = 34' 44'' 54.8'' (RT)$	$\Theta_s = 1' 38'' 13.4''$	$\Delta = 9' 42'' 08.9'' (RT)$
$L_s = 328.09$	$D = 0' 59'' 52.5''$	$L_s = 328.08$	$D = 3' 08'' 53.2''$
$ST = 109.37$	$L = 3,482.06$	$ST = 109.37$	$L_s = 200.00'$
$LT = 218.74$	$T = 1,796.43$	$LT = 218.73$	$LT = 133.35'$
	$R = 5,741.46$		$T = 154.47'$
	$S.E. = 0.04$		$R = 1,820.00'$
			$S.E. = 0.07$

SEE SHEET NO. 19 FOR -US 74- PROFILE.
SEE SHEET NO. 27 FOR -FLY- PROFILE.

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6/10/2018
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c:\psr

PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-21/CONST.7
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



MATCH LINE -US 74- STA. 64+00.00 SEE SHEET EC-20

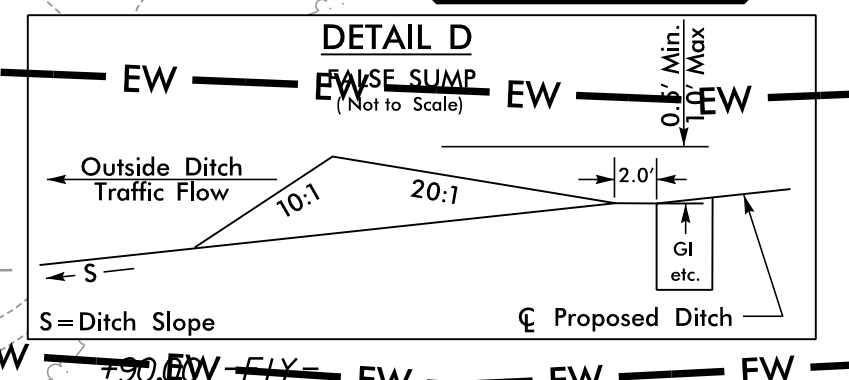
MATCH LINE -US 74 EBL- STA. 78+00.01 SEE SHEET EC-22

MATCH LINE -FLY- STA. 16+11.95 SEE SHEET EC-22

-FLY-
 Pts Sta 12+52.55 PI Sta 21+55.94
 $\Theta s = 4^{\circ} 11' 32.5''$ $\Delta = 53^{\circ} 19' 18.0''$ (LT)
 Ls = 240.00' D = 3' 29' 37.1"
 LT = 160.04' L = 1526.25'
 ST = 80.04' T = 823.43'
 R = 1640.00'
 S.E. = 0.08

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SEE SHEET NO. 19 FOR -US 74- AND -I-73- PROFILE.
 SEE SHEET NO. 24 FOR -US 74 EBL- PROFILE.
 SEE SHEET NOS. 27 AND 28 FOR -FLY- PROFILE.
 SEE SHEET NO. 33 FOR -RPC- PROFILE.



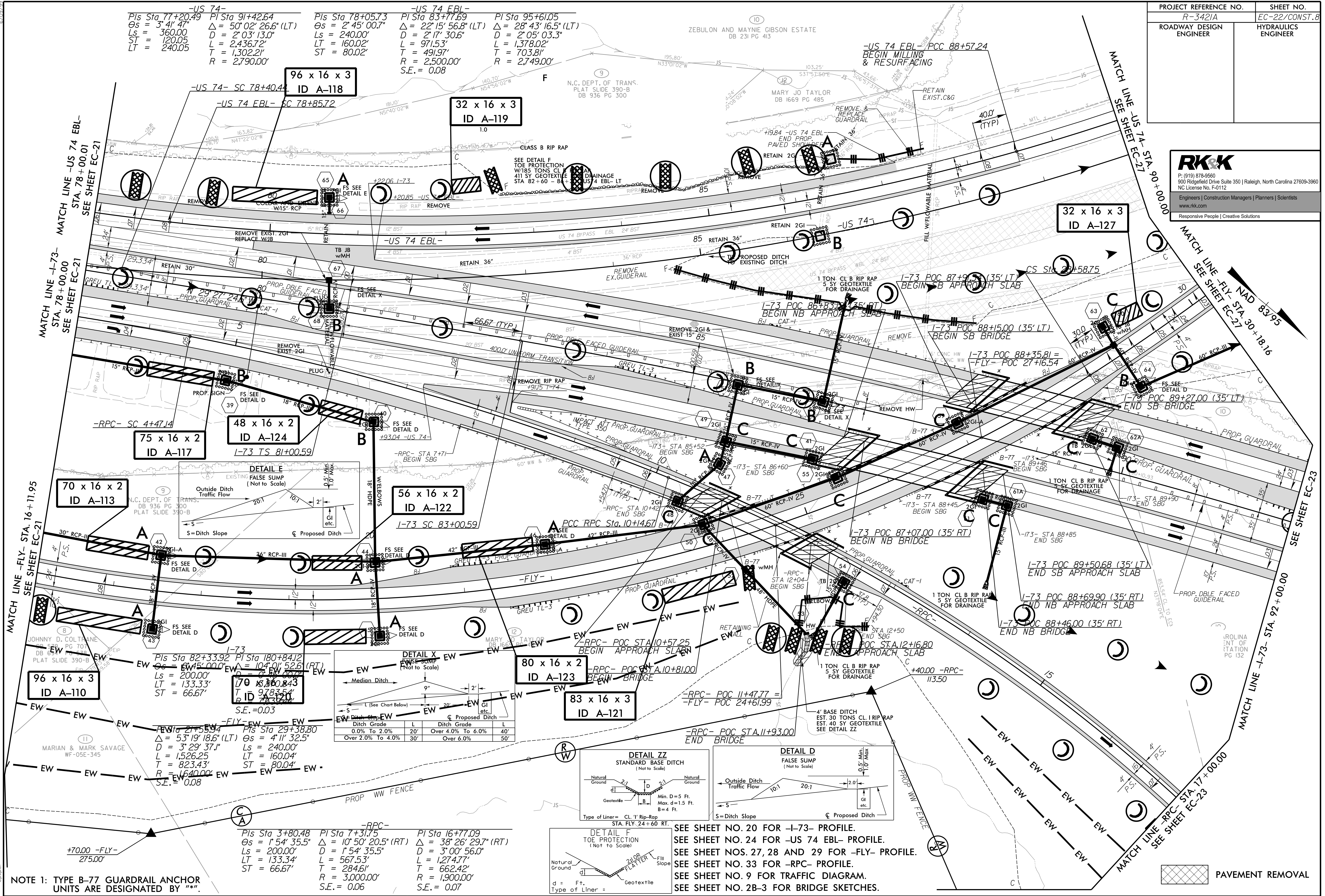
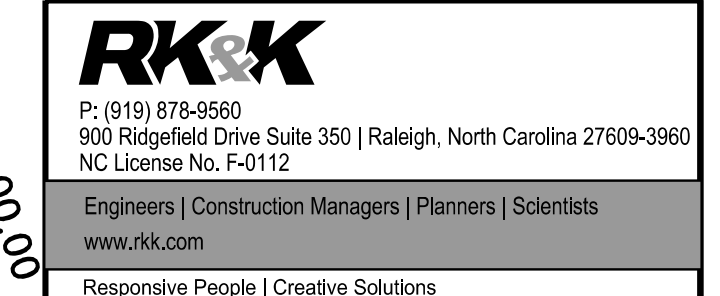
-RPC-
 Pts Sta 3+80.48
 $\Theta s = 1^{\circ} 54' 35.5''$
 Ls = 200.00'
 LT = 133.34'
 ST = 66.67'

-FLY REV-
 Pts Sta 1+89.36 PI Sta 4+10.67
 $\Theta s = 3^{\circ} 08' 53.2''$ $\Delta = 9^{\circ} 42' 08.9''$ (RT)
 Ls = 200.00' D = 3' 08' 53.2"
 LT = 133.35' L = 308.20'
 ST = 66.69' T = 154.47'
 R = 1,820.00'
 S.E. = 0.07

-FLY-
 Pts Sta 6+30.89
 $\Theta s = 3^{\circ} 08' 53.2''$
 Ls = 200.00'
 LT = 133.35'
 ST = 66.69'

+25.00 -FLY- 415.00'

7/1/2018
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 5/10/2018



-US 74-
 PIs Sta 77+20.49 PI Sta 91+42.64
 $\Delta s = 3^\circ 41' 47''$ $\Delta = 50^\circ 02' 26.6''$ (LT)
 $L_s = 360.00'$ $D = 2^\circ 03' 13.0''$
 $ST = 120.05'$ $L = 2,436.72'$
 $LT = 240.05'$ $T = 1,302.21'$
 $R = 2,790.00'$

-US 74 EBL-
 PIs Sta 78+05.73 PI Sta 83+77.69 PI Sta 95+61.05
 $\Delta s = 2^\circ 45' 00.7''$ $\Delta = 22^\circ 15' 56.8''$ (LT) $\Delta = 28^\circ 43' 16.5''$ (LT)
 $L_s = 240.00'$ $D = 2^\circ 17' 30.6''$ $D = 2^\circ 05' 03.3''$
 $L = 971.53'$ $L = 1,378.02'$
 $T = 491.97'$ $T = 703.81'$
 $R = 2,500.00'$ $R = 2,749.00'$
 $S.E. = 0.08$

MATCH LINE -FLY- STA. 16+11.95
 SEE SHEET EC-21

MATCH LINE -US 74 EBL- STA. 78+00.01
 SEE SHEET EC-21

MATCH LINE -US 74 EBL- STA. 90+00.00
 SEE SHEET EC-27

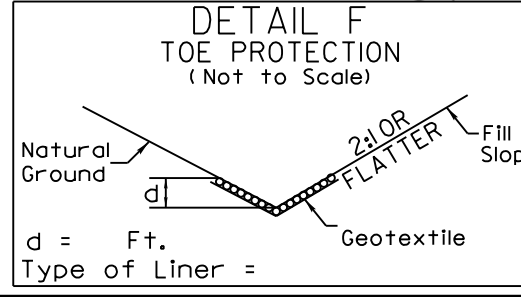
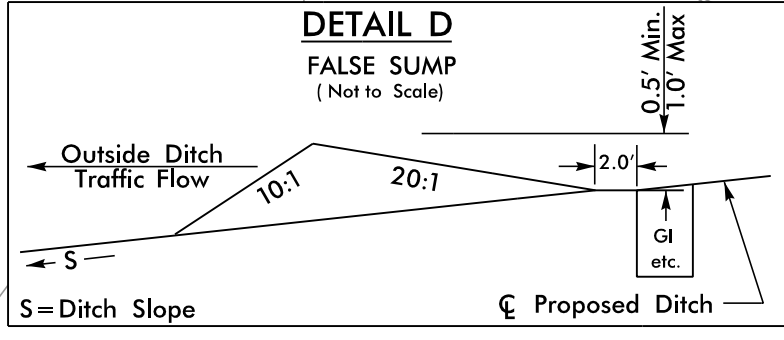
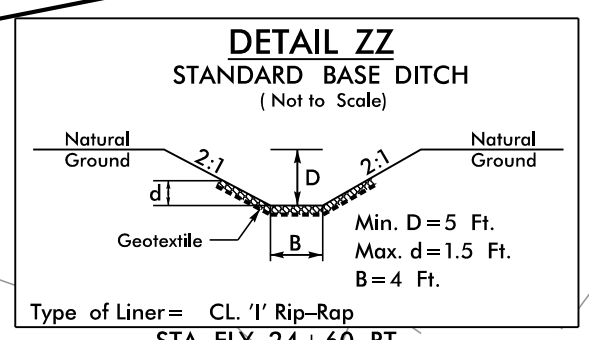
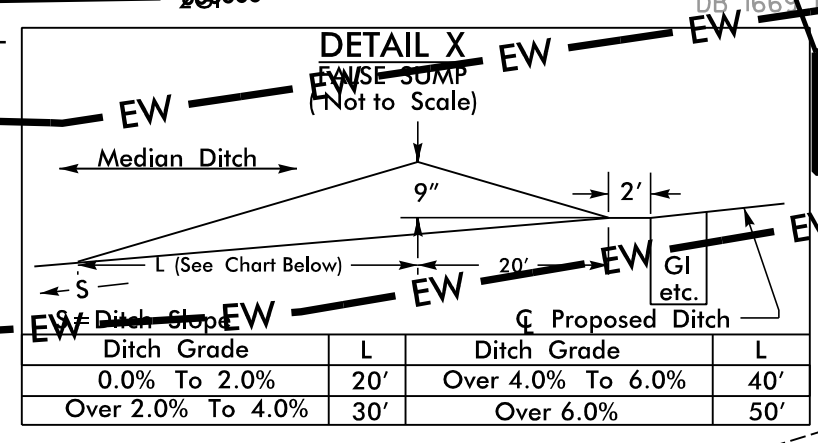
MATCH LINE -RPC- STA. 17+00.00
 SEE SHEET EC-23

-I-73-
 PIs Sta 82+33.92 PI Sta 180+84.12
 $\Delta s = 0^\circ 15' 00.0''$ $\Delta = 104^\circ 01' 52.6''$ (RT)
 $L_s = 200.00'$ $D = 0^\circ 00' 00.0''$
 $ST = 66.67'$ $L = 2,835.41'$
 $T = 783.54'$
 $R = 2,835.41'$
 $S.E. = 0.03$

-FLY-
 PIs Sta 21+55.94 PI Sta 29+38.80
 $\Delta s = 53^\circ 19' 18.6''$ (LT) $\Delta s = 4^\circ 11' 32.5''$
 $D = 3^\circ 29' 37.1''$ $L_s = 240.00'$
 $L = 1,526.25'$ $L = 160.04'$
 $T = 823.43'$ $T = 80.04'$
 $R = 1,640.00'$ $R = 80.04'$
 $S.E. = 0.08$

-RPC-
 PIs Sta 3+80.48 PI Sta 7+31.75 PI Sta 16+77.09
 $\Delta s = 1^\circ 54' 35.5''$ $\Delta = 10^\circ 50' 20.5''$ (RT) $\Delta = 38^\circ 26' 29.7''$ (RT)
 $L_s = 200.00'$ $D = 1^\circ 54' 35.5''$ $D = 3^\circ 00' 56.0''$
 $LT = 133.34'$ $L = 567.53'$ $L = 1,274.77'$
 $ST = 66.67'$ $T = 284.61'$ $T = 662.42'$
 $R = 3,000.00'$ $R = 1,900.00'$
 $S.E. = 0.06$ $S.E. = 0.07$

NOTE 1: TYPE B-77 GUARDRAIL ANCHOR UNITS ARE DESIGNATED BY "A".



SEE SHEET NO. 20 FOR -I-73- PROFILE.
 SEE SHEET NO. 24 FOR -US 74 EBL- PROFILE.
 SEE SHEET NOS. 27, 28 AND 29 FOR -FLY- PROFILE.
 SEE SHEET NO. 33 FOR -RPC- PROFILE.
 SEE SHEET NO. 9 FOR TRAFFIC DIAGRAM.
 SEE SHEET NO. 2B-3 FOR BRIDGE SKETCHES.

PAVEMENT REMOVAL

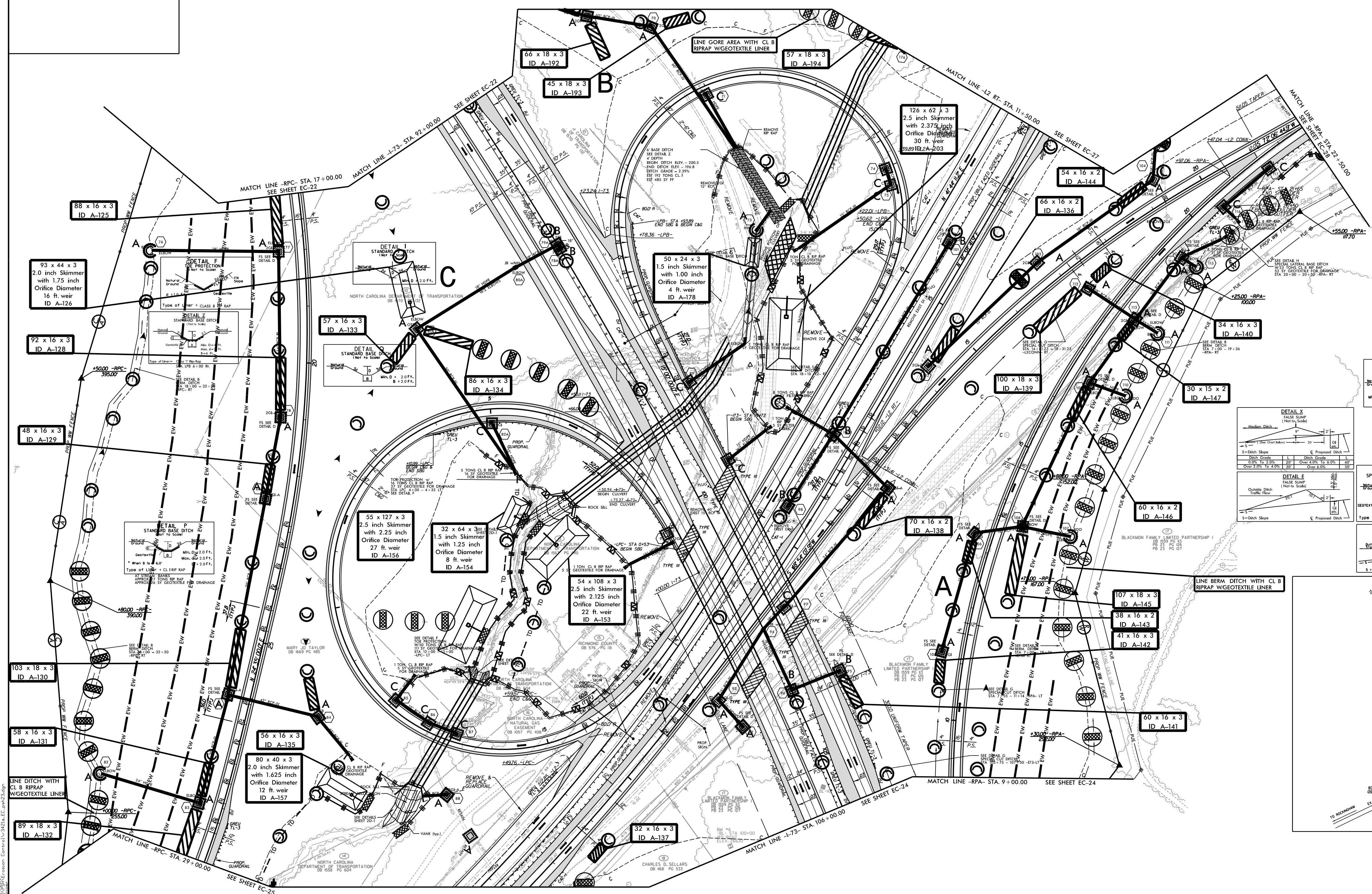
RICK
 CONSULTING ENGINEERS
 10000 W. HARRIS LANE, SUITE 100
 CHARLOTTE, NORTH CAROLINA 28226
 (704) 582-1000
 WWW.RICKENGINEERS.COM

PROJECT REFERENCE NO. **R-3421A**
 SHEET NO. **EC-23/CONV.3**

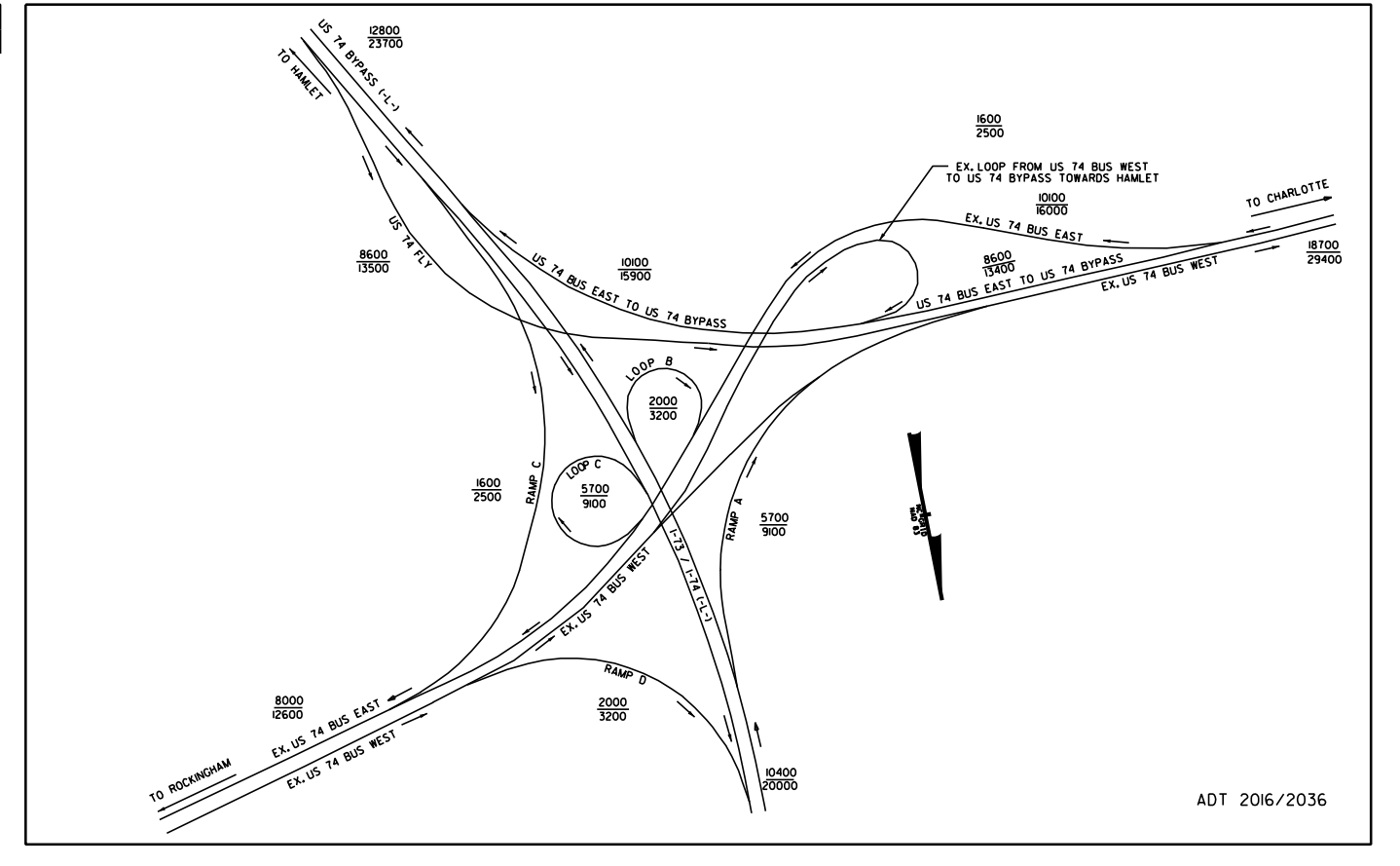
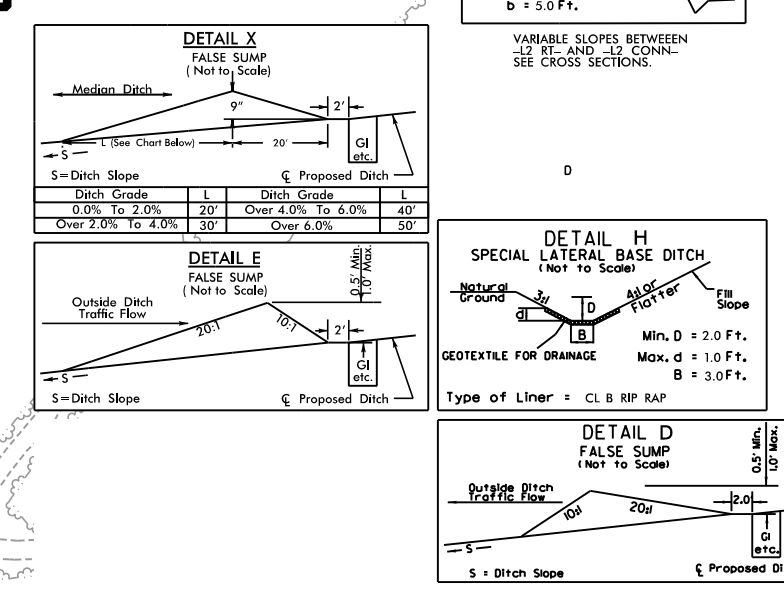
GRAPHIC SCALE
 100 50 0 100 200
 PLANS

ROADWAY DESIGN ENGINEER
 HYDRAULIC ENGINEER

REVISIONS

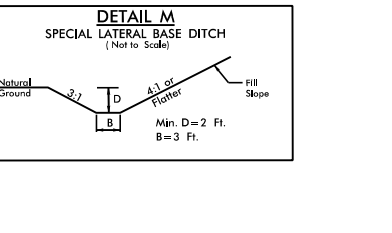
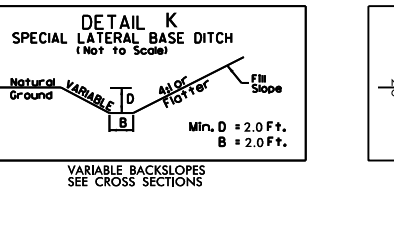
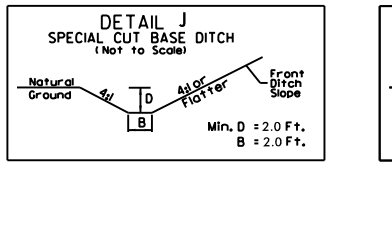
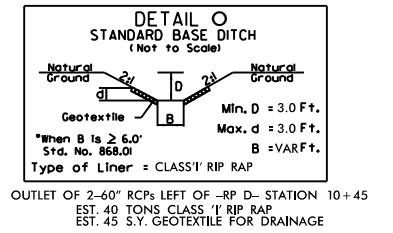
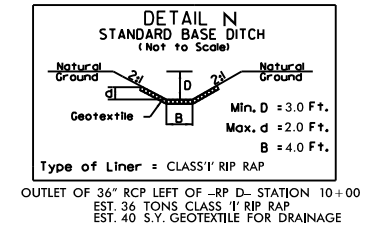
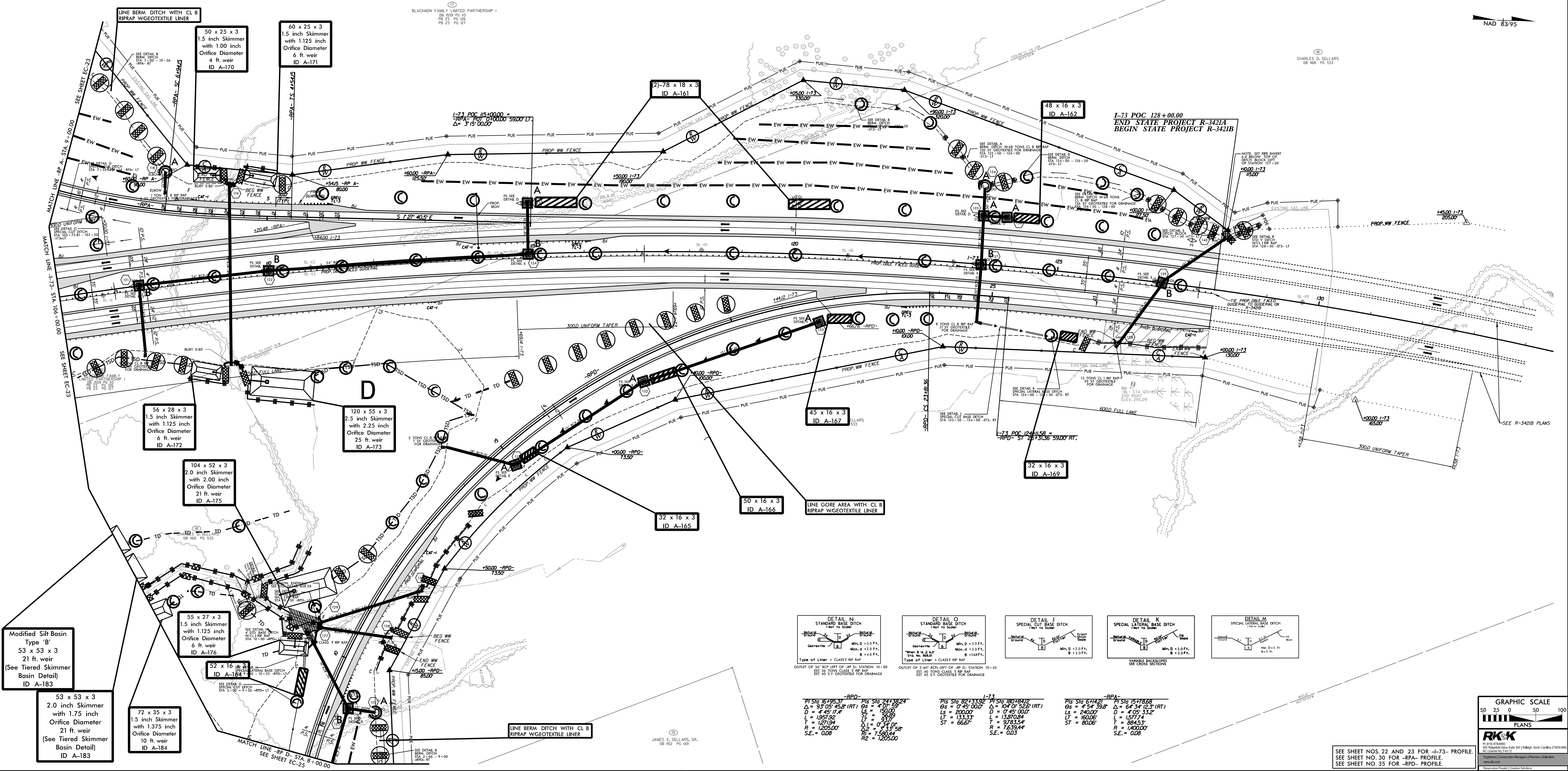
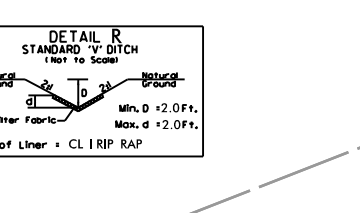
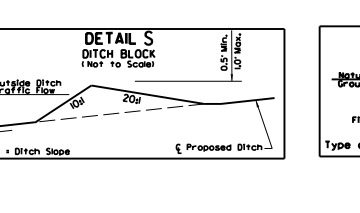
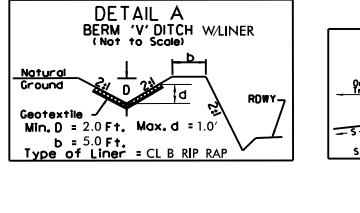
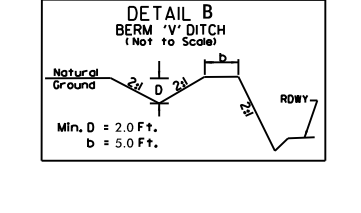
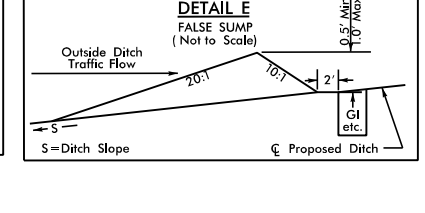
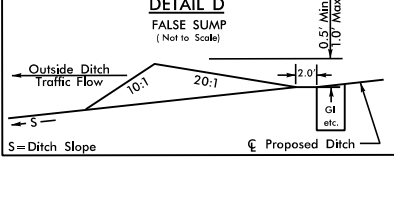
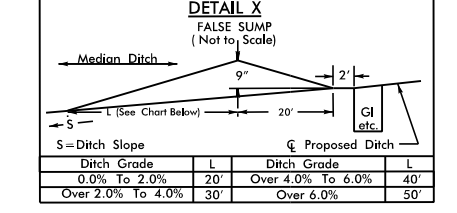
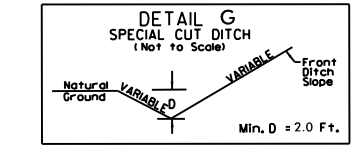


PISig 18462.28 Gs = 0'45"452" Ls = 108.00' LT = 72.00' ST = 36.00' R = 396.00' S.E. = 0.04	PISig 21449.22 Gs = 2'50"585" Ls = 189.00' LT = 126.00' ST = 63.00' R = 1400.00' S.E. = 0.08	PISig 25146.29 Gs = 7'01"312" (RT) Ls = 249.50' LT = 166.33' ST = 83.17' R = 1500.00' S.E. = 0.04	PISig 18187.26 Gs = 2'30"590" Ls = 189.00' LT = 126.00' ST = 63.00' R = 1400.00' S.E. = 0.07	PISig 23146.29 Gs = 4'04"398" Ls = 249.50' LT = 166.33' ST = 83.17' R = 1500.00' S.E. = 0.08	PISig 21449.22 Gs = 2'50"585" Ls = 189.00' LT = 126.00' ST = 63.00' R = 1400.00' S.E. = 0.08	PISig 27145.20 Gs = 4'07"317" Ls = 249.50' LT = 166.33' ST = 83.17' R = 1500.00' S.E. = 0.08	PISig 34108.41 Gs = 4'22"576" (RT) Ls = 349.10' LT = 109.58' ST = 54.79' R = 500.00' S.E. = 0.08
--	--	---	--	--	--	--	--

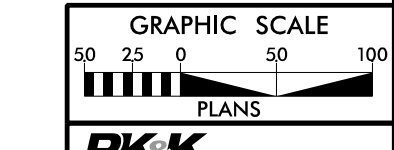


SEE SHEET NO. 21 FOR -I-73- PROFILE.
 SEE SHEET NO. 31 FOR -RPA- PROFILE.
 SEE SHEET NO. 34 FOR -RPC- PROFILE.
 SEE SHEET NO. 36 FOR -LPB- PROFILE.
 SEE SHEET NOS. 37 AND 38 FOR -LPC- PROFILE.
 SEE SHEET NO. 39 FOR -L2 RT-L2 REV- PROFILE.
 SEE SHEET NO. 41 FOR -L2 CONVA- PROFILE.
 SEE SHEET NO. 28-3 FOR BRIDGE SKETCHES.

REVISIONS

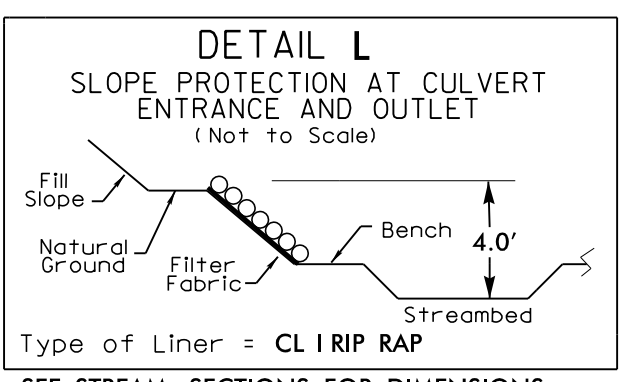
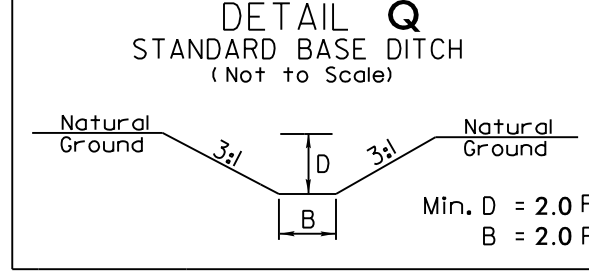
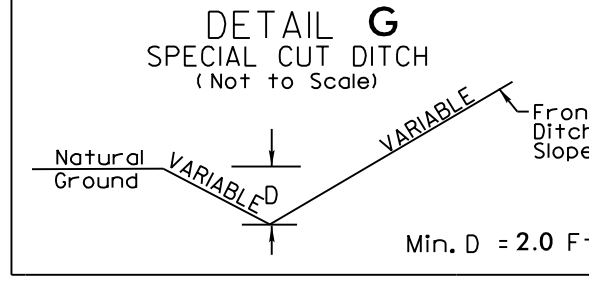
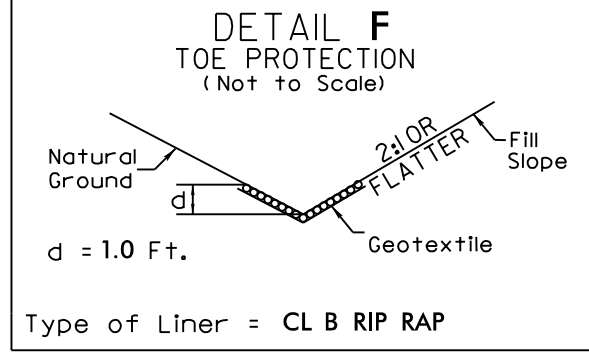
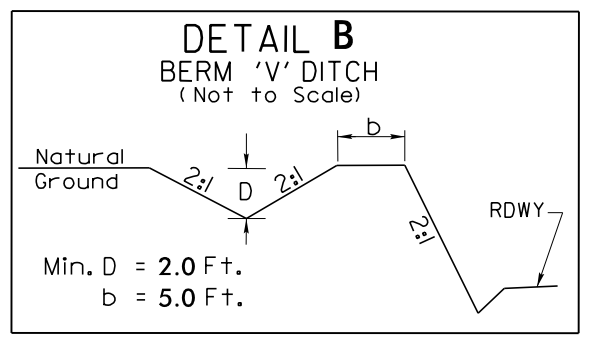


Station	PI	SI	PI	SI	PI	SI	PI	SI	PI	SI
16+95.37	94.1824	94.1824	82+332	100.9402	61421	61421	61421	61421	61421	61421
Δ = 93.05' 45.8" (RT)	68 = 4.11' 18"	68 = 4.11' 18"	Δ = 104' 05.8" (RT)	68 = 4.54' 39.8"	Δ = 6.34' 12.3" (RT)	Δ = 6.34' 12.3" (RT)	Δ = 6.34' 12.3" (RT)	Δ = 6.34' 12.3" (RT)	Δ = 6.34' 12.3" (RT)	Δ = 6.34' 12.3" (RT)
D = 4.45' 07.4"	LE = 150.0'	LE = 150.0'	D = 17.45' 00.0"	LE = 240.0'	LE = 240.0'	LE = 240.0'	LE = 240.0'	LE = 240.0'	LE = 240.0'	LE = 240.0'
L = 1897.92'	LT = 857.8'	LT = 857.8'	L = 13870.84'	LT = 1577.4'	LT = 1577.4'	LT = 1577.4'	LT = 1577.4'	LT = 1577.4'	LT = 1577.4'	LT = 1577.4'
T = 1271.94'	Δ = 0.7414'	Δ = 0.7414'	T = 57835.54'	Δ = 8845.3'	Δ = 8845.3'	Δ = 8845.3'	Δ = 8845.3'	Δ = 8845.3'	Δ = 8845.3'	Δ = 8845.3'
R = 1205.00'	RI = 7.58254'	RI = 7.58254'	R = 76324.4'	R = 1400.00'	R = 1400.00'	R = 1400.00'	R = 1400.00'	R = 1400.00'	R = 1400.00'	R = 1400.00'
S.E. = 0.08	RI = 1.20500	RI = 1.20500	S.E. = 0.03	S.E. = 0.08	S.E. = 0.08	S.E. = 0.08	S.E. = 0.08	S.E. = 0.08	S.E. = 0.08	S.E. = 0.08

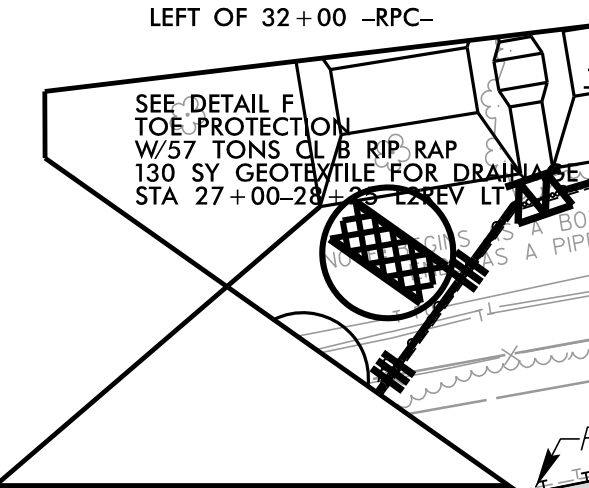


SEE SHEET NOS. 22 AND 23 FOR -I-73- PROFILE.
 SEE SHEET NO. 30 FOR -RPA- PROFILE.
 SEE SHEET NO. 35 FOR -RPD- PROFILE.

PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-25/CONST.11
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



SEE STREAM SECTIONS FOR DIMENSIONS
USE APPROX 35 LF. OF RIP RAP PER EACH
APPROX 100 SY GEOTEXTILE FOR DRAINAGE TOTAL
APPROX 80 TONS CL I RIP RAP TOTAL



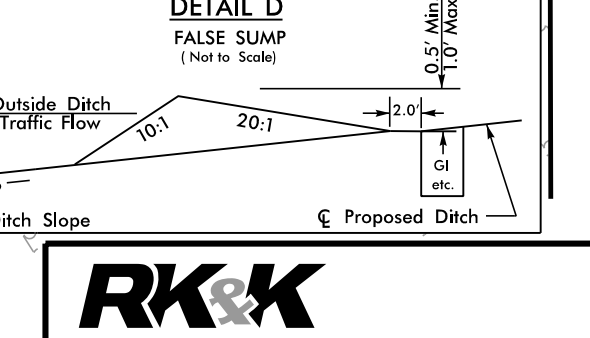
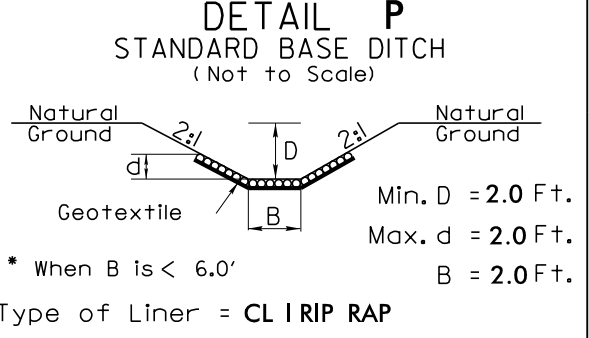
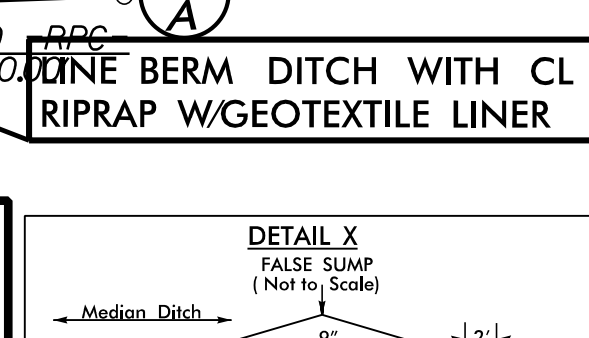
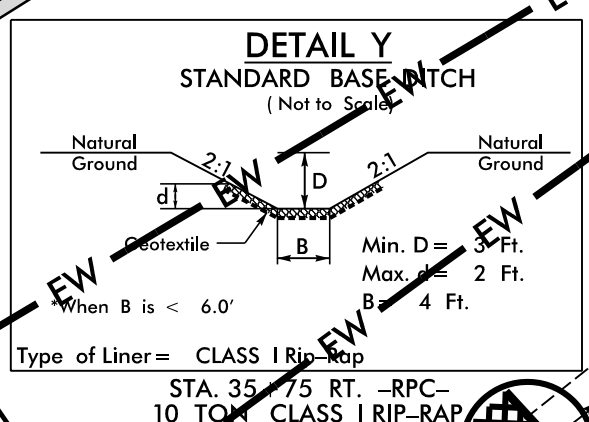
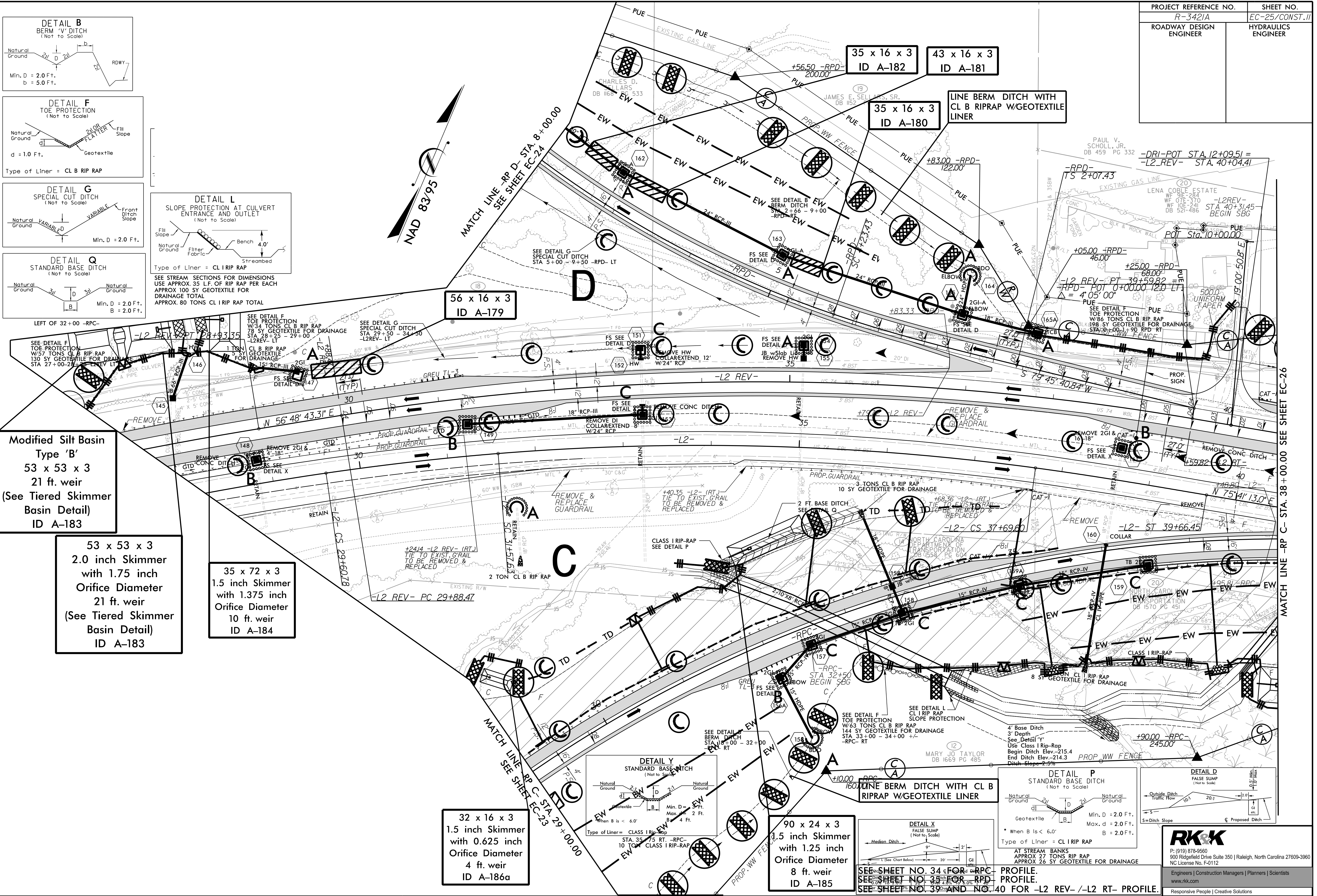
Modified Silt Basin
Type 'B'
53 x 53 x 3
21 ft. weir
(See Tiered Skimmer Basin Detail)
ID A-183

53 x 53 x 3
2.0 inch Skimmer
with 1.75 inch Orifice Diameter
21 ft. weir
(See Tiered Skimmer Basin Detail)
ID A-183

35 x 72 x 3
1.5 inch Skimmer
with 1.375 inch Orifice Diameter
10 ft. weir
ID A-184

32 x 16 x 3
1.5 inch Skimmer
with 0.625 inch Orifice Diameter
4 ft. weir
ID A-186a

90 x 24 x 3
1.5 inch Skimmer
with 1.25 inch Orifice Diameter
8 ft. weir
ID A-185

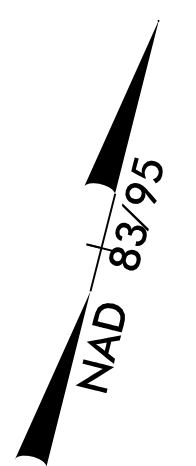


SEE SHEET NO. 34 FOR -RPC- PROFILE.
SEE SHEET NO. 35 FOR -RPC- PROFILE.
SEE SHEET NO. 39 AND NO. 40 FOR -L2 REV- /-L2 RT- PROFILE.

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MATCH LINE - RP C - STA. 38+00.00 SEE SHEET EC-26

Hydraulics CADD (P&R) Erosion Control V-3421a.EC_psh25.dgn



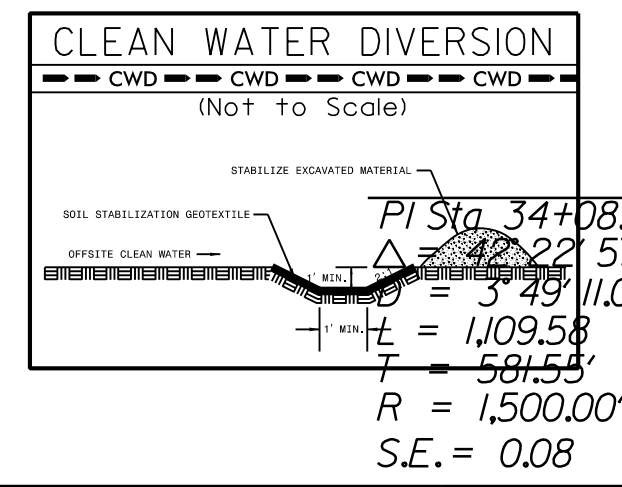
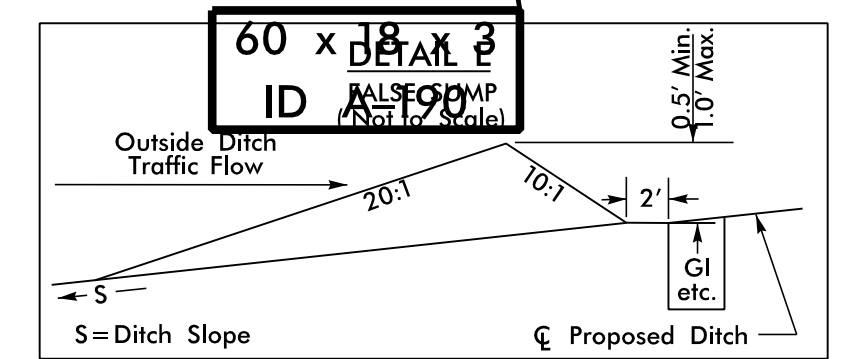
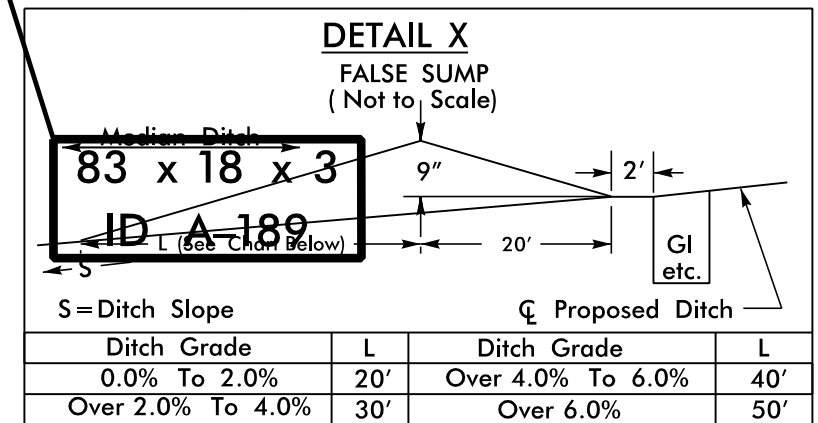
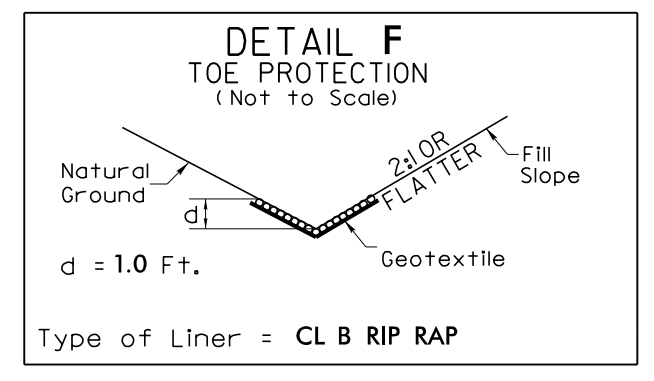
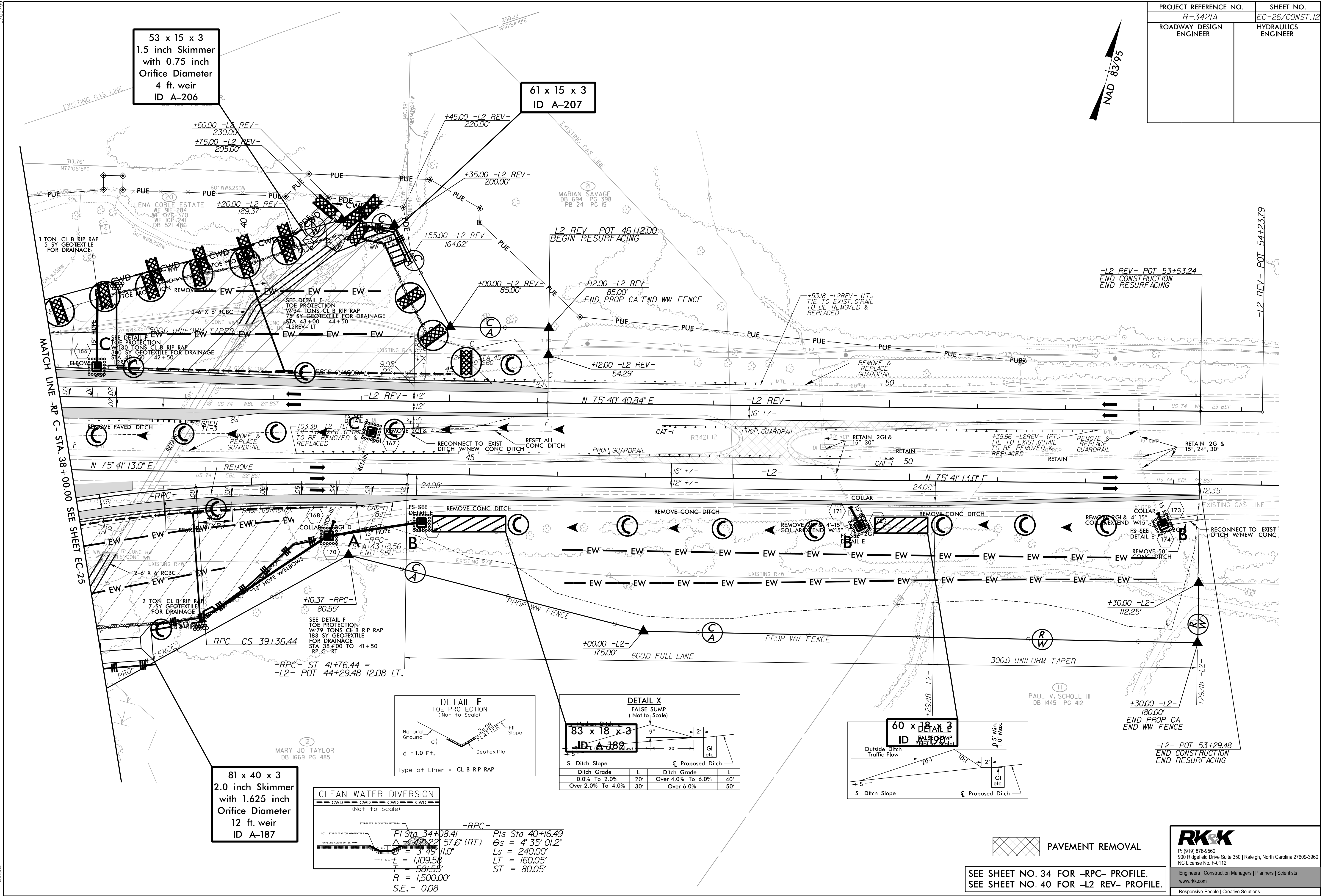
53 x 15 x 3
1.5 inch Skimmer
with 0.75 inch
Orifice Diameter
4 ft. weir
ID A-206

61 x 15 x 3
ID A-207

81 x 40 x 3
2.0 inch Skimmer
with 1.625 inch
Orifice Diameter
12 ft. weir
ID A-187

83 x 18 x 3
ID A-189

60 x 18 x 3
ID A-190



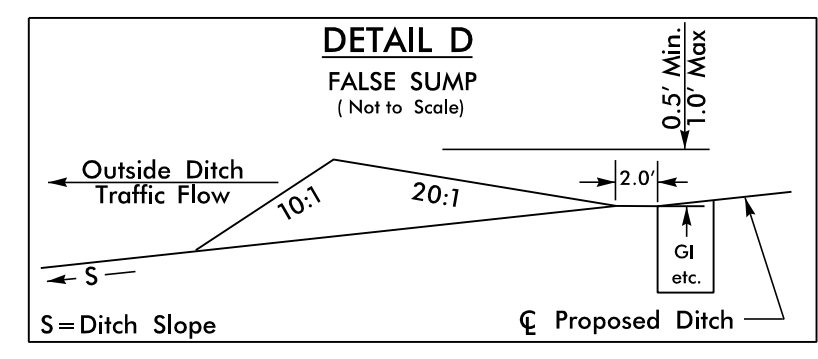
-RPC-
PI Sta 34+08.41
 $\Delta = 42.22$ 57.6° (RT)
 $\theta_s = 4' 35'' 01.2''$
 $L_s = 240.00'$
 $LT = 160.05'$
 $ST = 80.05'$
 $R = 1,500.00'$
 $S.E. = 0.08$

SEE SHEET NO. 34 FOR -RPC- PROFILE.
SEE SHEET NO. 40 FOR -L2 REV- PROFILE.

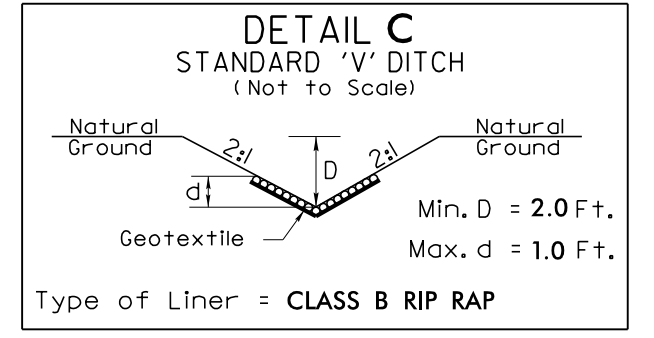


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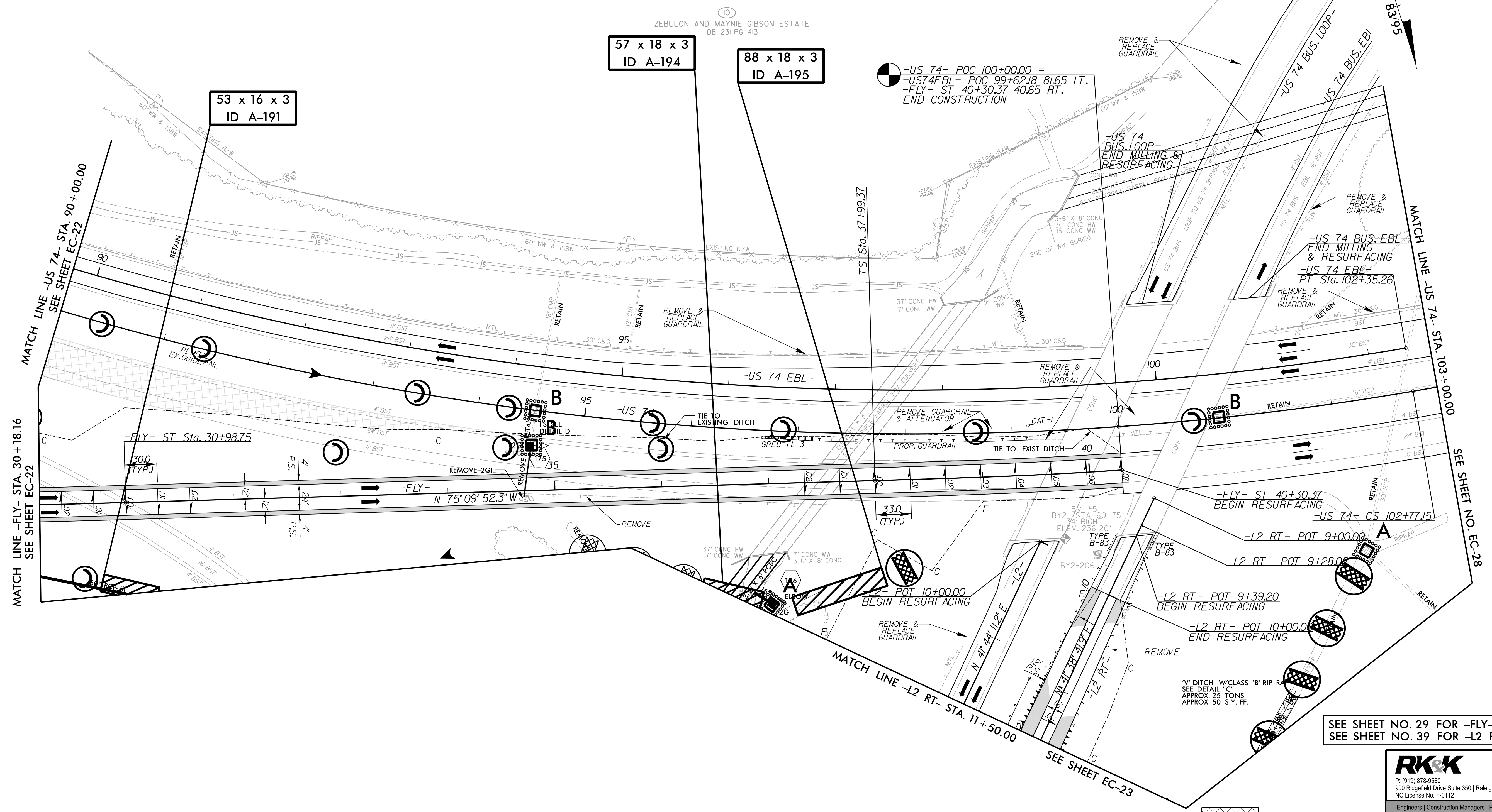
PROJECT REFERENCE NO.	R-3421A	SHEET NO.	EC-27/CONST.13
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	



-FLY-
 PIs Sta 12+52.55 $\Delta = 4' 11'' 32.5''$ $L_s = 240.00'$ $LT = 160.04'$ $ST = 80.04'$
 PIs Sta 21+55.94 $\Delta = 5' 3'' 18.0''$ (LT) $L = 1,526.25'$ $T = 823.43'$ $R = 1,640.00'$ $S.E. = 0.08$
 PIs Sta 29+38.80 $\Delta = 4' 11'' 32.5''$ $L_s = 240.00'$ $LT = 160.04'$ $ST = 80.04'$
-FLY-
 PIs Sta 39+53.39 $\Delta = 2' 20'' 16.3''$ $L_s = 231.00'$ $LT = 154.01'$ $ST = 77.01'$



-US 74 EBL-
 PIs Sta 95+61.05 $\Delta = 28' 43'' 16.5''$ (LT) $D = 2' 05'' 03.3''$ $L = 1,378.02'$ $T = 703.81'$ $R = 2,749.00'$ $S.E. = 0.08$
 PIs Sta 21+55.94 $\Delta = 50' 02'' 26.6''$ (LT) $D = 2' 03'' 13.0''$ $L = 2,436.72'$ $T = 1,302.21'$ $R = 2,790.00'$ $S.E. = 0.07$
-US 74-
 PIs Sta 104+35.15 $\Delta = 5' 24'' 04.2''$ $L_s = 360.00'$ $ST = 158.00'$ $LT = 202.26'$ $D2 = 1' 42'' 16.3''$ $R1 = 6,050.00'$ $R2 = 2,790.00'$



53 x 16 x 3
ID A-191

57 x 18 x 3
ID A-194

88 x 18 x 3
ID A-195

-US 74- POC 100+00.00 =
 -US 74 EBL- POC 99+62.18 81.65 LT.
 -FLY- ST 40+30.37 40.65 RT.
 END CONSTRUCTION

SEE SHEET NO. 29 FOR -FLY- PROFILE.
 SEE SHEET NO. 39 FOR -L2 RT- PROFILE.

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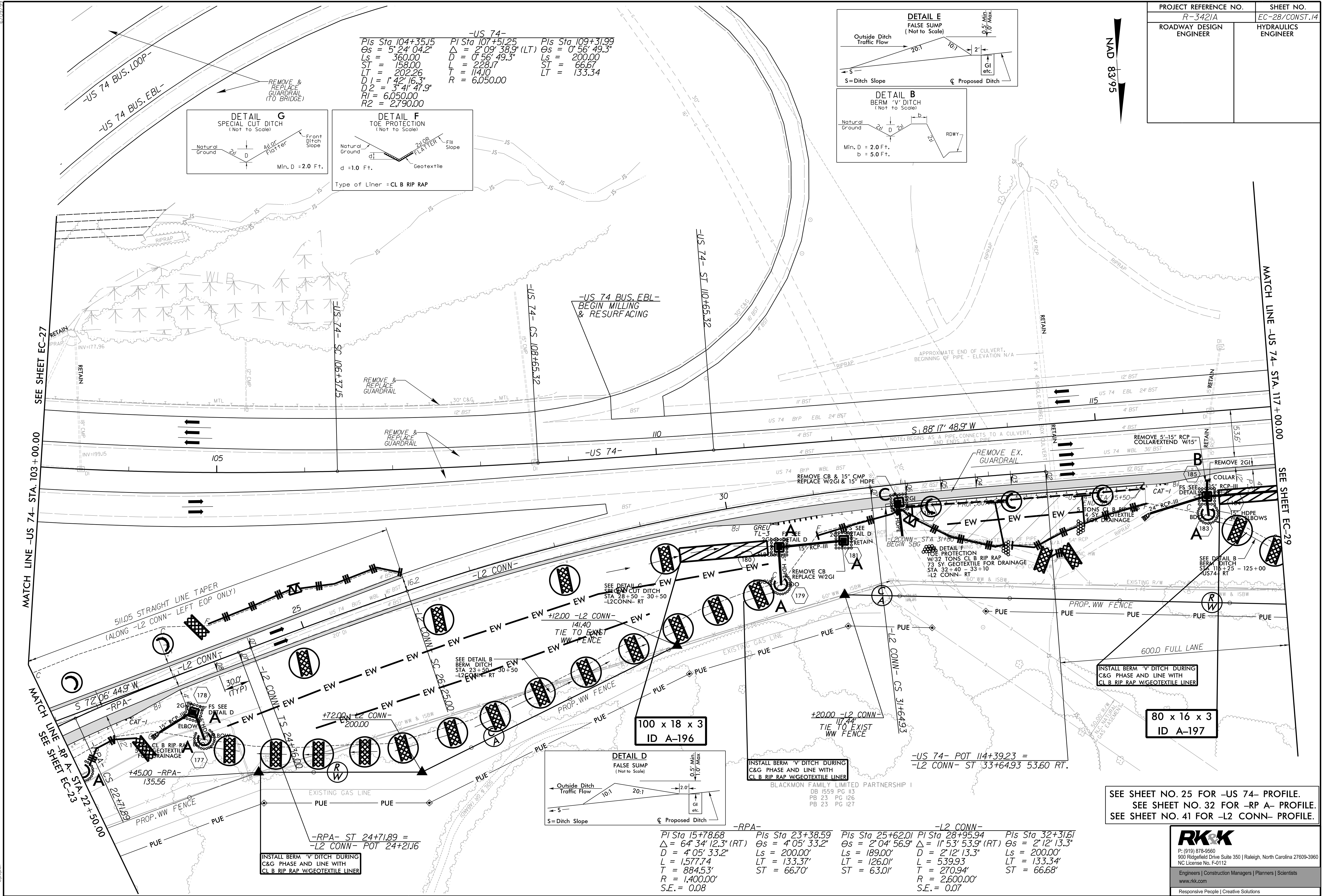
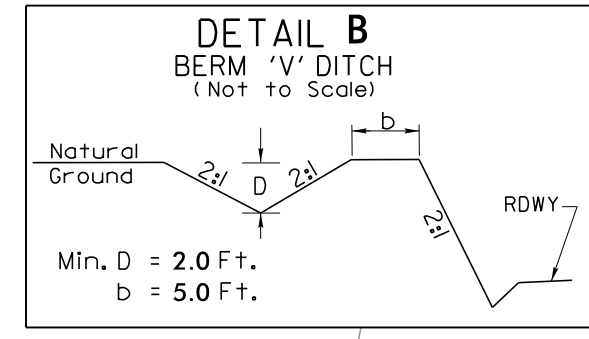
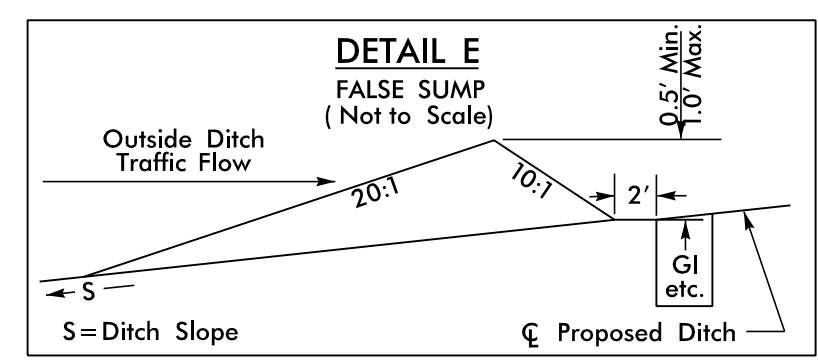
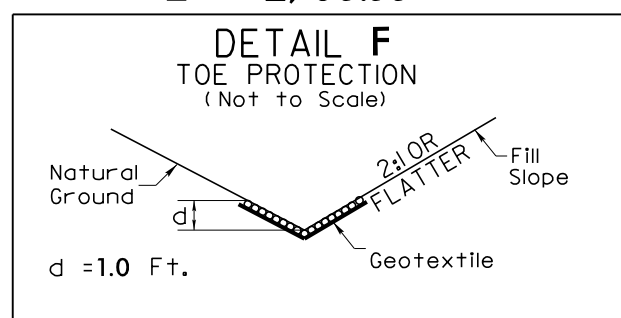
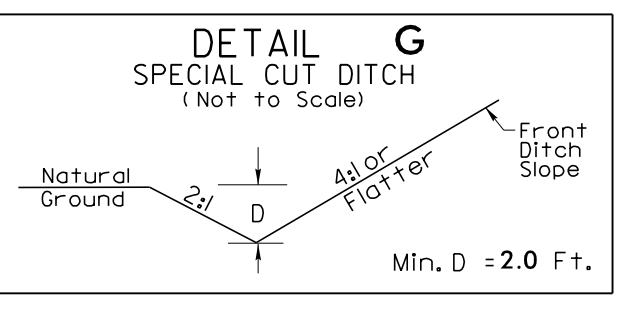
PAVEMENT REMOVAL

6/09/08
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 7/1/2018
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PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-28/CONST.14
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

NAD 83/95

-US 74-
 Pls Sta 104+35.15 PI Sta 107+51.25 Pls Sta 109+31.99
 $\Delta = 5' 24' 04.2''$ $\Delta = 2' 09' 38.9''$ (LT) $\Delta = 0' 56' 49.3''$
 $L_s = 360.00$ $D = 0' 56' 49.3''$ $L_s = 200.00$
 $ST = 158.00$ $L = 228.17$ $ST = 66.67$
 $LT = 202.26$ $T = 114.10$ $LT = 133.34$
 $D1 = 1' 42' 16.3''$ $R = 6,050.00$
 $D2 = 3' 41' 47.9''$ $R2 = 2,790.00$



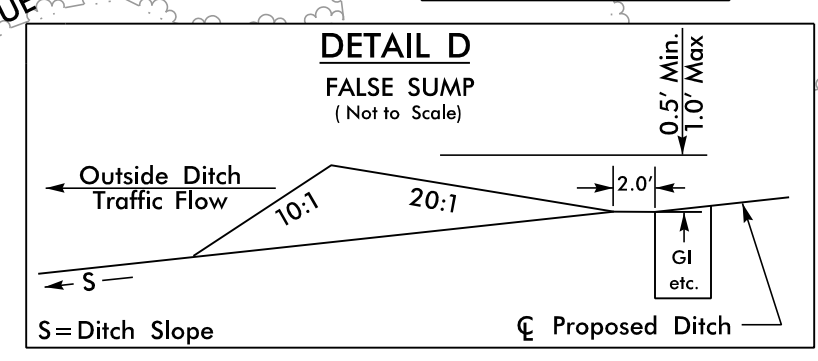
SEE SHEET EC-27

MATCH LINE -US 74- STA. 103+00.00

MATCH LINE -RPA- STA 22+50.00

MATCH LINE -US 74- STA. 117+00.00

SEE SHEET EC-29



-RPA-
 Pls Sta 15+78.68 PI Sta 23+38.59 Pls Sta 25+62.01 PI Sta 28+95.94 Pls Sta 32+31.61
 $\Delta = 64' 34' 12.3''$ (RT) $\Delta = 4' 05' 33.2''$ $\Delta = 2' 04' 56.9''$ $\Delta = 11' 53' 53.9''$ (RT) $\Delta = 2' 12' 13.3''$
 $D = 4' 05' 33.2''$ $L_s = 200.00'$ $L_s = 189.00'$ $D = 2' 12' 13.3''$ $L_s = 200.00'$
 $L = 1,577.74$ $LT = 133.37'$ $LT = 126.01'$ $L = 539.93'$ $LT = 133.34'$
 $T = 884.53'$ $ST = 66.70'$ $ST = 63.01'$ $T = 270.94'$ $ST = 66.68'$
 $R = 1,400.00'$ $R = 2,600.00'$
 $S.E. = 0.08$ $S.E. = 0.07$

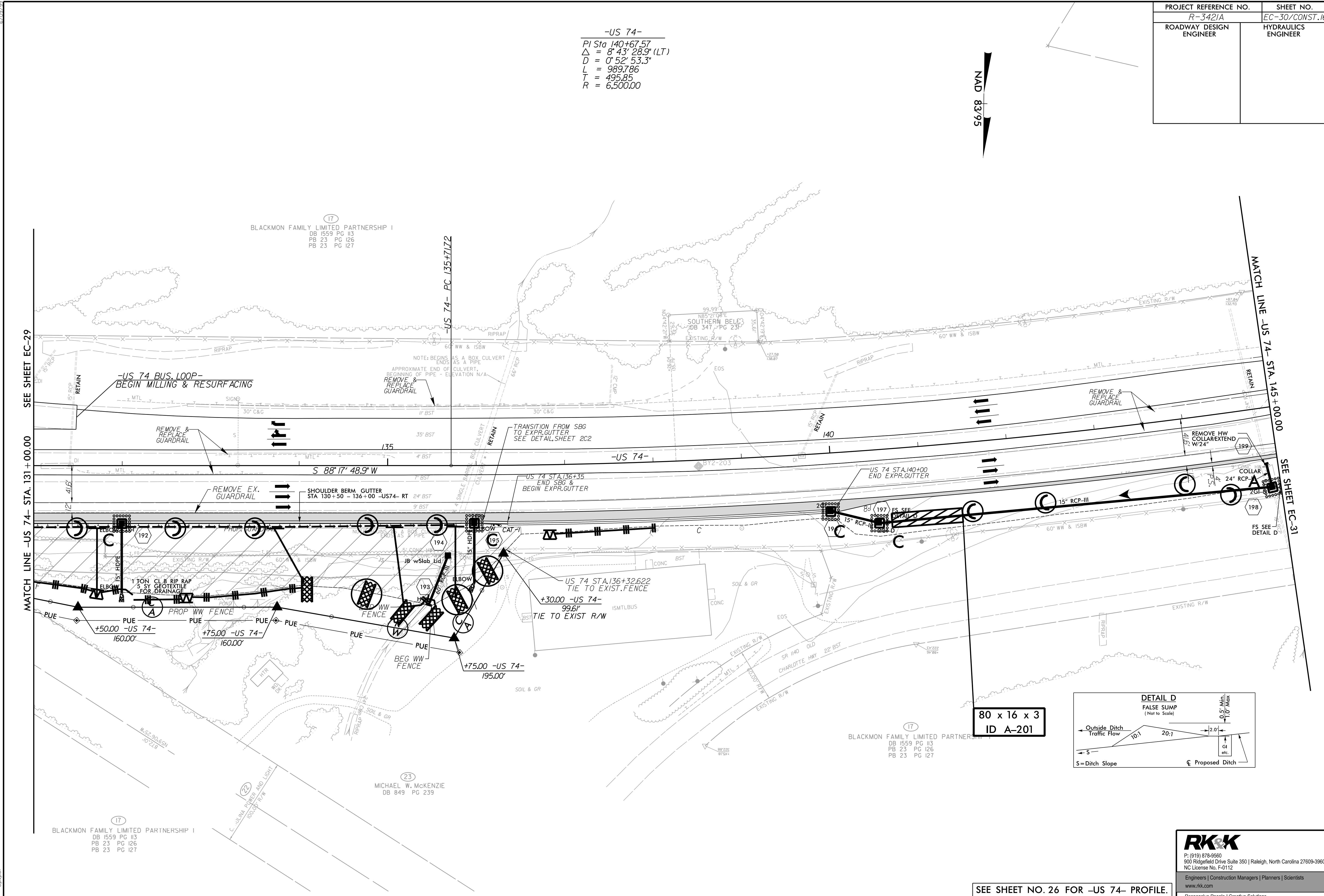
SEE SHEET NO. 25 FOR -US 74- PROFILE.
 SEE SHEET NO. 32 FOR -RPA- PROFILE.
 SEE SHEET NO. 41 FOR -L2 CONN- PROFILE.

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PROJECT REFERENCE NO. R-3421A	SHEET NO. EC-30/CONST.16
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

-US 74-
 PI Sta 140+67.57
 $\Delta = 8' 43" 28.9" (LT)$
 $D = 0' 52" 53.3"$
 $L = 989.786$
 $T = 495.85$
 $R = 6,500.00$

NAD 83/95



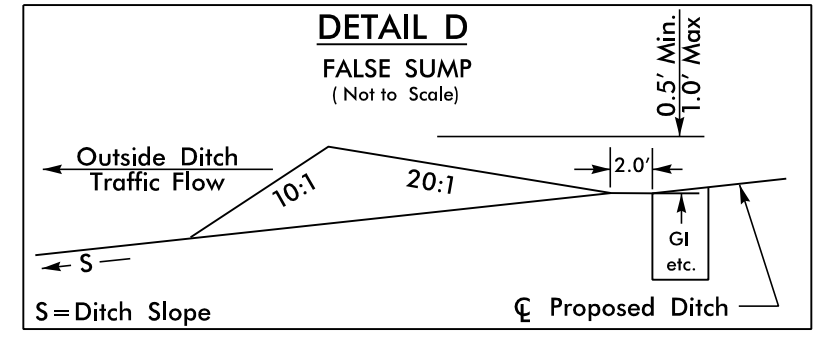
SEE SHEET EC-29

MATCH LINE -US 74- STA. 131+00.00

MATCH LINE -US 74- STA. 145+00.00

SEE SHEET EC-31

80 x 16 x 3
ID A-201



BLACKMON FAMILY LIMITED PARTNERSHIP I
 DB 1559 PG 113
 PB 23 PG 126
 PB 23 PG 127

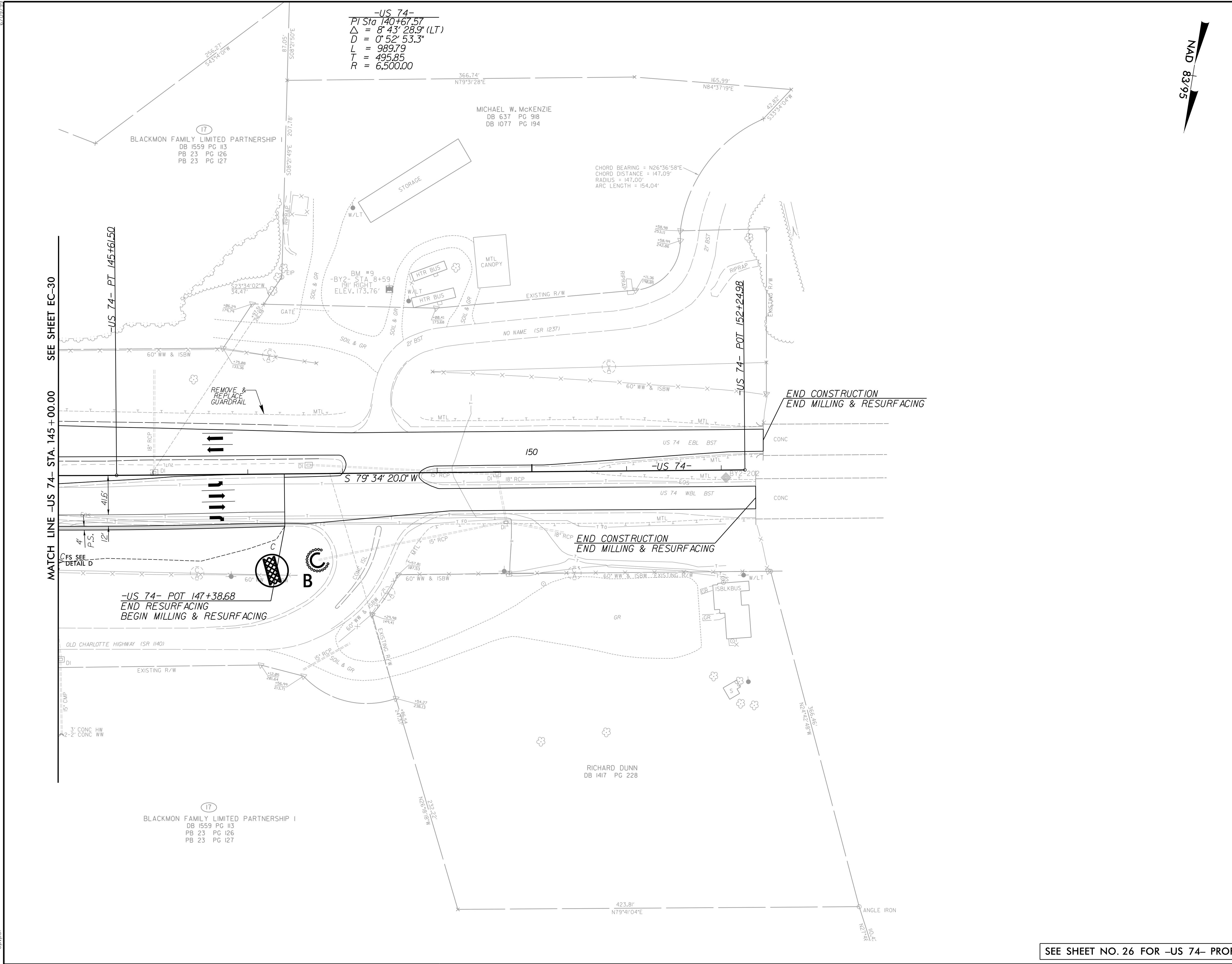
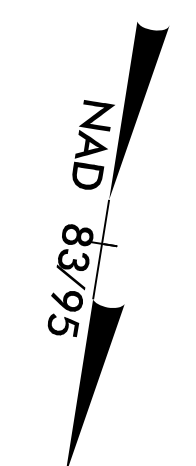
MICHAEL W. MCKENZIE
 DB 849 PG 239

BLACKMON FAMILY LIMITED PARTNERSHIP I
 DB 1559 PG 113
 PB 23 PG 126
 PB 23 PG 127

SEE SHEET NO. 26 FOR -US 74- PROFILE.

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PROJECT REFERENCE NO.		SHEET NO.	
R-3421A		EC-31/CONST.17	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	



MATCH LINE -US 74- STA. 145+00.00 SEE SHEET EC-30

-US 74- POT 147+38.68
END RESURFACING
BEGIN MILLING & RESURFACING

END CONSTRUCTION
END MILLING & RESURFACING

END CONSTRUCTION
END MILLING & RESURFACING

BLACKMON FAMILY LIMITED PARTNERSHIP I
DB 1559 PG 113
PB 23 PG 126
PB 23 PG 127

-US 74-
PI Sta 140+67.57
Δ = 8° 43' 28.9" (LT)
D = 0° 52' 53.3"
L = 989.79
T = 495.85
R = 6,500.00

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SEE SHEET NO. 26 FOR -US 74- PROFILE.

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