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REFERENCE: I-4400BB

PROJECT: 34232

SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

ROADWAY
SUBSURFACE INVESTIGATION

COUNTY HENDERSON
PROJECT DESCRIPTION I-26 FROM US 64 (EXIT 49)
TO US 25 BUSINESS (EXIT 44)

INVENTORY

CONTENTS

<u>LINE</u>	<u>STATION</u>	<u>PLAN</u>	<u>CROSS SECTION</u>
-L-	439+89 TO 648+08	4-23	24-120
-YIRPA-	17+00 TO 17+50	4	121-122
-YIRPD-	16+00 TO 16+50	4	123
-Y2-	21+55 TO 23+28	8	124-126
-Y5-	18+03 TO 20+29	16	127-129
-Y7-	15+24 TO 16+71	22-23	130-132
-Y9-	13+00	23	133

CONTENTS

<u>SHEET NO.</u>	<u>DESCRIPTION</u>
134-149	BORE LOG(S), CORE REPORT(S), & CORE PHOTOGRAPH(S)
150	ROCK TEST RESULT(S)
151	SOIL TEST RESULT(S)

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	I-4400BB	1	151

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 - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

<u>A. STURCHIO</u>	<u>M. ARNOLD</u>
<u>D. RACEY</u>	<u>S. WOODS</u>
	<u>M. DURWAY</u>
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	<u>T. BEARD</u>

INVESTIGATED BY F&R, Inc.
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 CHECKED BY D. RACEY DR
 SUBMITTED BY P. ALTON, P.E.
 DATE JUNE 2019

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DocuSigned by:

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION, GRADATION, ROCK DESCRIPTION, TERMS AND DEFINITIONS, SOIL LEGEND AND AASHTO CLASSIFICATION, CONSISTENCY OR DENSENESS, TEXTURE OR GRAIN SIZE, SOIL MOISTURE - CORRELATION OF TERMS, PLASTICITY, COLOR, RECOMMENDATION SYMBOLS, ABBREVIATIONS, EQUIPMENT USED ON SUBJECT PROJECT, ROCK HARDNESS, FRACTURE SPACING, BEDDING, INDURATION, NOTES

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION

**SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES
 FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS**

AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

<p>GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)</p> <p>From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.</p> <p>STRUCTURE</p>	<p>SURFACE CONDITIONS</p> <p>VERY GOOD Very rough, fresh unweathered surfaces</p> <p>GOOD Rough, slightly weathered, iron stained surfaces</p> <p>FAIR Smooth, moderately weathered and altered surfaces</p> <p>POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments</p> <p>VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings</p> <p>DECREASING SURFACE QUALITY →</p>	<p>GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos, P and Hoek E., 2000)</p> <p>From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.</p> <p>SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)</p> <p>VERY GOOD - Very Rough, fresh unweathered surfaces</p> <p>GOOD - Rough, slightly weathered surfaces</p> <p>FAIR - Smooth, moderately weathered and altered surfaces</p> <p>POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments</p> <p>VERY POOR - Very smooth, slickensided or highly weathered surfaces with soft clay coatings or fillings</p>
<p>INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities</p> <p>BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets</p> <p>VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets</p> <p>BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity</p> <p>DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces</p> <p>LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes</p> <p>DECREASING INTERLOCKING OF ROCK PIECES ↓</p>	<p>90</p> <p>80</p> <p>70</p> <p>60</p> <p>50</p> <p>40</p> <p>30</p> <p>20</p> <p>10</p> <p>N/A</p> <p>N/A</p>	<p>A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.</p> <p>B. Sandstone with thin inter-layers of siltstone</p> <p>C. Sandstone and siltstone in similar amounts</p> <p>D. Siltstone or silty shale with sandstone layers</p> <p>E. Weak siltstone or clayey shale with sandstone layers</p> <p>F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure</p> <p>G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers</p> <p>H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.</p> <p>→ Means deformation after tectonic disturbance</p>
		<p>70</p> <p>60</p> <p>50</p> <p>40</p> <p>30</p> <p>20</p> <p>10</p>

7/24/2017

CONTRACT: I-4400BB

CONTRACT: I-4400BB

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

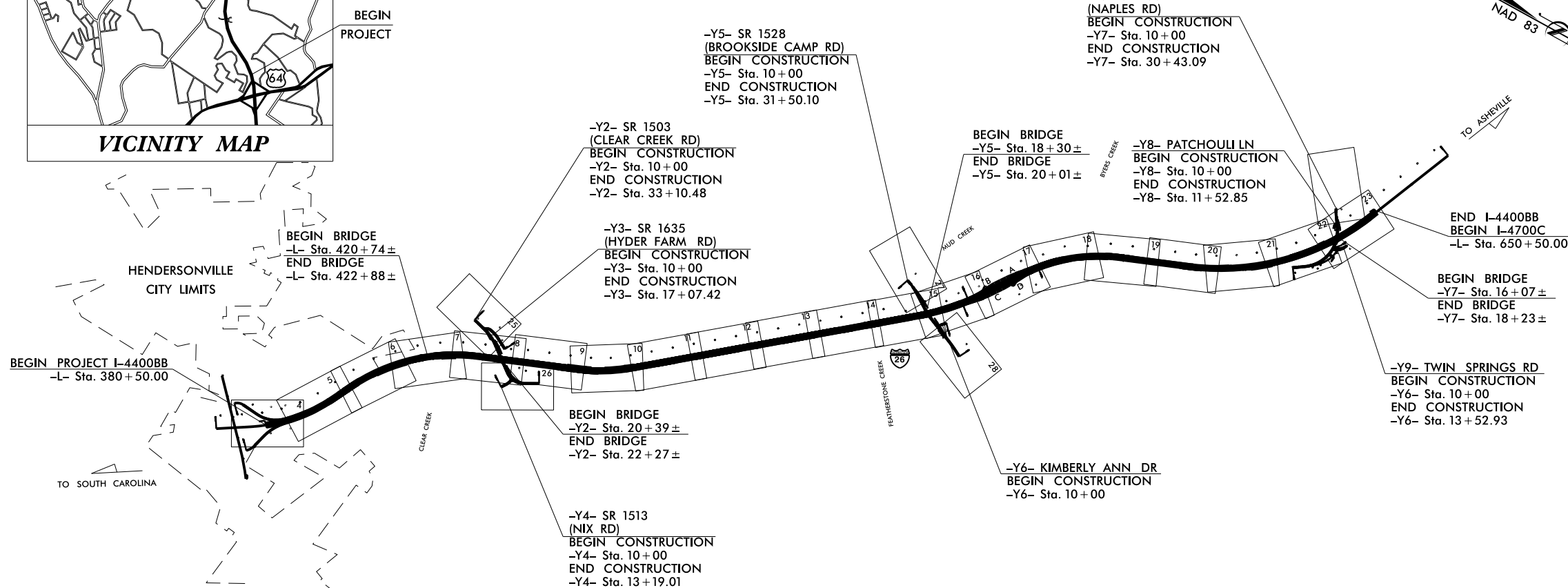
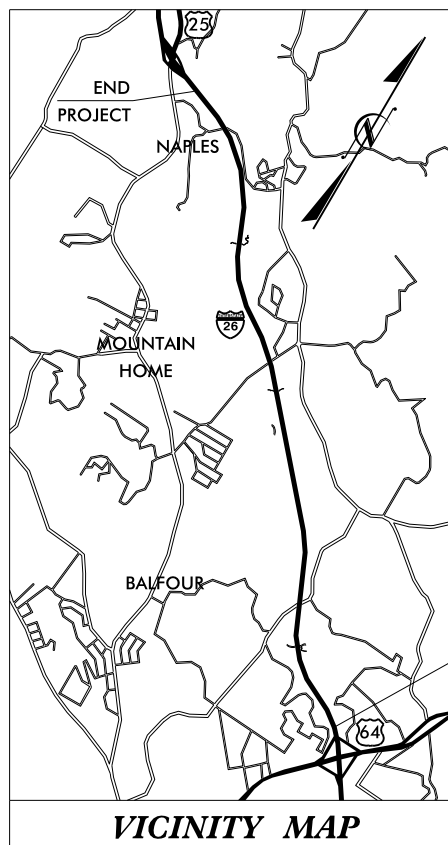
HENDERSON COUNTY

LOCATION: I-26 FROM 0.05MI NORTH OF US 64
TO 0.5MI SOUTH OF US 25 (ASHEVILLE HIGHWAY)

TYPE OF WORK: GRADING, PAVING, DRAINAGE, RESURFACING, AND STRUCTURES

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	I-4400BB	3	151
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
34232.1.6	NHP-0026(015)	I-4400BB (P.E.)	
34232.2.10	NHP-0026(015)	I-4400BB (RWUTIL)	
34232.3.8	NHP-0026(015)	I-4400BB (CONST.)	

25% PRELIMINARY PLANS

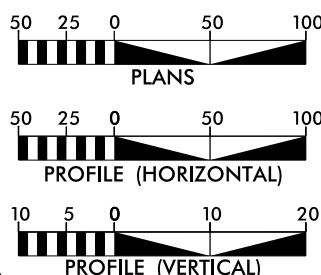


- NOTE:
- CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.
 - THIS PROJECT IS PARTIALLY WITHIN THE MUNICIPAL BOUNDARIES OF THE CITY OF HENDERSONVILLE.
 - THIS IS A CONTROLLED- ACCESS PROJECT WITH ACCESS BEING LIMITED TO INTERCHANGES.

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

GRAPHIC SCALES



DESIGN DATA

ADT 2019 =84,200
ADT 2039 =116,400
K =10%
D =55%
T =10% *
V =70 MPH
* TTST =4% DUAL 6%
FUNC CLASS =INTERSTATE
STATEWIDE TIER

PROJECT LENGTH

TOTAL LENGTH OF ROADWAY TIP PROJECT
I-4400BB = 5.073 MI
TOTAL LENGTH OF STRUCTURES OF TIP PROJECT
I-4400BB = 0.041 MI
TOTAL LENGTH OF TIP PROJECT
I-4400BB = 5.114 MI
NOTE: LENGTHS WERE CALCULATED USING THE WBL ALIGNMENT

Prepared in the Office of:
HNTB
HNTB NORTH CAROLINA, P.C.
343 E. Six Forks Road, Suite 200
Raleigh, North Carolina 27609
NC License No. C-1554

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
November, 2018

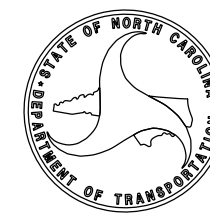
LETTING DATE:
June 2019

JOSEPH OLSON, P.E.
PROJECT ENGINEER
JEFF HESS, P.E.
PROJECT DESIGN ENGINEER
WANDA AUSTIN, P.E.
NCDOT CONTACT

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.
ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.



7/24/2017 5:40:00 PM RDY_TSH.dgn 3:11 PM



June 27, 2019

State Project No.: 34232.1.3
TIP No.: I-4400BB
Project ID: 30850
County: Henderson
Description: I-26 from US 64 (Exit 49) to US 25 Business (Exit 44)

SUBJECT: Geotechnical Report – Inventory

Project Description

This project primarily involves the widening of approximately 5 miles of existing I-26 (-L-) in Asheville and Hendersonville, Henderson County, North Carolina. The majority of the widening generally remains within existing right-of-way along I-26. The existing roadway consists of two eastbound and two westbound lanes separated by a grass median, and there are no major interchanges in this section.

As shown on plan sheets initially received on 9/18/17, the project consisted of: widening I-26 (-L-) from approximately 1,100 feet south of the intersection of I-26 with US 64 (Exit 49) and ending approximately 700 feet north of the intersection of I-26 with US 25 Business; proposed retaining walls along -L- and Ramp B at US 64; and proposed bridge realignment and replacements at Clear Creek, Clear Creek Road, Brookside Camp Road, and Naples Road.

Based on revised plans received on 9/24/18, the project begins approximately 750 feet north of the intersection of I-26 with US 64 (Exit 49) and ends approximately 1,000 feet south of the intersection of I-26 with Naples Road (-Y7-). The revised plans exhibit slight alignment shifts along -L-; the addition, removal, and changes in lengths of proposed retaining walls; and -Y- line designation changes and realignments.

The typical proposed section for the project will include four eastbound and four westbound lanes separated by a concrete barrier. More-specifically, the typical section will incorporate 12-foot lanes, 4-foot outside shoulders, and 12-foot inside shoulders. The mainline of the proposed widening of I-26 will cross over Clear Creek, and under Clear Creek Road (-Y2-), Brookside Camp Road (-Y5-), and Naples Road (-Y7-). In addition, several bridges, culverts, and retaining walls are proposed, although the specifics of such structures were unknown at the time of this investigation, and all of which were not fully investigated at that time. Additional structures are also located along the project limits but were not investigated as part of our scope. The locations shown below represent areas where additional subsurface investigation was performed in anticipation of the listed structures:

Bridge on -Y2- over -L-;
Bridge on -Y5- over -L-;
Bridge on -Y7- over -L-;

Retaining wall along -Y1RPB- (part of original alignment but is now outside of project limits);
Retaining wall from approximate -L- station 405+40 to 414+40, left;
Retaining wall from approximate -L- station 440+30 to 449+85, left;
Retaining wall from approximate -L- station 456+30 to 472+80, left;
Retaining wall from approximate -L- station 615+25 to 625+30, right;
Retaining wall from approximate -L- station 634+25 to 638+30, left;

Culvert at approximate -L- station 495+00;
Culvert at approximate -L- station 526+00;
Culvert at approximate -L- station 588+18;
60" CMP at approximate -L- station 611+00.

The boring locations were selected by NCDOT based on the plans dated 9/8/17, and the geotechnical field investigation was performed between December 2017 and March 2018. During this time period, a total of 166 Standard Penetration Test (SPT) borings were advanced with ATV- and track-mounted CME-55 drill rigs with automatic hammers. In addition, 4 hand auger borings with sounding rods were completed due to restrictive drill rig access. However, it was determined upon receipt of plans dated 9/24/18 that borings B-1 through B-9 and B-162 through B-165 were drilled outside of the revised project limits and the results of these borings are not discussed in this report. Representative soil samples were collected from the split spoon or hand auger cuttings for visual classification in the field and for analysis by F&R's testing laboratory.

Generally the following alignments were investigated, although a majority of the borings were for the purpose of investigating anticipated structures listed above. The limits of the listed alignments were determined by the attached plan sheets:

<u>Alignment</u>	<u>Station (±)</u>
-L-	439+89 to 648+08
-Y1RPA-	17+00 to 17+50
-Y1RPD-	16+00 to 16+50
-Y2-	21+55 to 23+28
-Y5-	18+03 to 20+29
-Y7-	15+24 to 16+71

Areas of Special Geotechnical Interest

1) **Crystalline Rock**: The following areas were found to contain crystalline rock above or within six feet of the proposed grade and will likely require ripping or blasting for removal:

<u>Alignment</u>	<u>Station (±)</u>
-L-	408+75 to 409+25, left
-L-	412+00 to 412+75, left
-L-	413+00 to 414+75, left
-L-	467+00 to 469+00, left
-L-	472+00 to 472+75, left
-L-	488+75 to 490+75, left
-L-	504+25 to 510+25, right
-L-	517+75 to 522+25, right
-L-	531+25 to 538+25, left
-L-	581+00 to 583+25, right
-L-	620+00 to 621+00, right
-Y1RPA-	16+75 to 17+25, right

2) **Weathered Rock**: The following areas were found to contain weathered rock above or within six feet of the proposed grade and have a potential to require ripping or blasting for removal:

<u>Alignment</u>	<u>Station (±)</u>
-L-	410+00 to 410+50, left
-L-	412+50 to 413+00, left
-L-	413+50 to 414+75, left
-L-	439+25 to 439+75, left
-L-	440+00 to 440+50, left
-L-	466+50 to 468+00, left
-L-	468+50 to 470+00, left
-L-	472+00 to 472+75, left
-L-	517+25 to 522+75, right
-L-	531+25 to 538+25, left
-L-	580+75 to 583+25, right
-L-	600+50 to 601+60, right
-L-	620+50 to 621+00, right
-L-	621+50 to 622+00, right
-L-	625+00 to 625+50, right
-Y1RPA-	16+75 to 17+75, right

3) **Soft, Loose and/or Wet Soils**: The following areas contain relatively soft or loose (clay N≤5, sand N≤7) and/or wet, near-surface soils that have the potential to cause subgrade problems during construction:

<u>Alignment</u>	<u>Station (±)</u>
-L-	405+25 to 406+75, left
-L-	440+50 to 441+00
-L-	460+50 to 462+50, left
-L-	465+50 to 466+50, left
-L-	471+75 to 472+25, left
-L-	490+00 to 490+25, left
-L-	495+75 to 496+25, left
-L-	520+25 to 520+75, right
-L-	539+50 to 540+00
-L-	581+00 to 581+50
-L-	588+25 to 588+75, right
-L-	596+00 to 596+75, right
-L-	600+75 to 601+25, right
-L-	608+00 to 608+50, right
-L-	615+50 to 616+50, right
-L-	618+00 to 618+50, right
-L-	622+50 to 623+00, right
-L-	639+50 to 640+25, right
-L-	642+00 to 642+50
-L-	647+00 to 648+25
-Y1RPA-	17+00 to 17+25, right
-Y5-	20+00 to 20+50

4) **Cohesive Soils**: The following areas contain cohesive soils (AASHTO A-5, A-6 & A-7 soils) at existing subgrade in fill areas or at/near proposed subgrade that have the potential to cause subgrade problems during construction:

<u>Alignment</u>	<u>Station (±)</u>
-L-	460+00 to 461+50, left
-L-	610+50 to 611+00, left
-L-	642+00 to 642+50, left
-L-	647+00 to 647+50, left
-Y2-	22+25 to 22+75
-Y5-	17+75 to 18+50
-Y5-	20+00 to 20+50

- 5) Cohesive Soils: The following areas contain deeper deposits of relatively soft cohesive soils (AASHTO A-5, A-6 & A-7 soils) that have the potential to cause embankment instability or long-term settlement problems:

<u>Alignment</u>	<u>Station (±)</u>
-L-	460+75 to 462+50, left
-L-	495+50 to 496+25, left
-L-	525+75 to 526+75
-L-	610+50 to 611+00, left
-L-	634+00 to 635+00, left
-L-	640+75 to 641+25, left
-L-	642+00 to 642+50
-Y5-	20+00 to 20+50

- 6) Groundwater: The following areas exhibited groundwater within six feet of the proposed grade:

<u>Alignment</u>	<u>Station (±)</u>
-L-	520+25 to 520+75, right
-L-	533+00 to 533+50, left
-L-	539+25 to 540+50
-L-	636+50 to 638+50, left
-L-	647+00 to 647+50, left
-Y1RPA-	16+75 to 17+25, right

- 7) The following areas exhibited groundwater within three feet of existing grade, which have the potential to cause subgrade problems during construction:

<u>Alignment</u>	<u>Station (±)</u>
-L-	539+25 to 540+50
-L-	637+00 to 638+00, left
-L-	642+00 to 642+50, left
-L-	647+00 to 647+50, left
-Y1RPA-	16+75 to 17+25, right
-Y5-	18+00 to 18+50

- 8) Highly Plastic Soils: The following areas of unclassified excavation contain soils with plasticity indices of 26 through 38:

<u>Alignment</u>	<u>Station (±)</u>
-L-	405+25 to 408+00, left
-L-	577+00 to 577+50, right
-L-	584+25 to 584+75, right

-L-	596+00 to 596+50, right
-L-	600+75 to 601+25, right

- 9) Artificial Fill: The following locations contain artificial fill. These soils have the potential to be highly variable, which could cause subgrade problems during construction if undetected pockets of organics, debris, or soft/loose/wet soils are present. In addition, a majority of these soils are cohesive, which may cause embankment instability or long-term settlement problems.

<u>Alignment</u>	<u>Station (±)</u>
-L-	518+00 to 521+34, right
-L-	525+85 to 527+00, right
-L-	612+29 to 616+10, right
-L-	619+25 to 622+25, right

Physiography and Geology

The proposed road widening will accommodate additional lanes in the eastbound and westbound directions. The widening will generally remain within existing right-of-way. At the beginning of the project at -L- station 380+50, the roadway elevation (EL) starts at EL ±2,166 feet and gradually descends to EL ±2,120 feet at about -L- station 426+00. From this point the roadway elevation ascends to the maximum elevation of the project, EL ±2,186 feet at about -L- station 462+00, and then descends again to the lowest elevation of the project, EL ±2072 feet at about -L- station 526+00. The remainder of the existing roadway grade ascends to EL ±2,106 at about -L- station 568+00, descends to EL ±2,083 at about -L- station 590+50, ascends to EL ±2,151, then descends to EL ±2,122 feet at -L- station 650+50, the end of the project.

The surface water across the project is generally drained by Mud Creek and several other smaller tributaries, which are all tributaries of the French Broad River. Mud Creek is located approximately 300 to 3,500 feet west of the proposed widening and runs northwest, generally parallel to the existing interstate. The majority of the smaller tributaries run generally in a northeast-to-southwest direction.

The project is located in the Blue Ridge Province of North Carolina. More-specifically, it is located in an area mapped as muscovite-biotite gneiss (Zatm). Typical weathered rock samples recovered from our borings primarily exhibit the characteristics of biotite gneiss. Soils weathered from the parent rock generally consist of sandy silts and silty sands, along with some silty and sandy clays. The in-situ soils are the residual product of in-place chemical weathering of rock that was similar to the rock presently underlying the site.

Soils Properties

The subsurface conditions discussed below and those shown on the attached drawings represent an estimate of the subsurface conditions based on interpretation of the boring data using normally-accepted geotechnical engineering judgments. The transitions between different soil strata are usually less distinct than those shown on the boring logs and cross sections. Sometimes the relatively small sample obtained in

the field is insufficient to definitively describe the origin of the subsurface material. Although individual soil test borings are representative of the subsurface conditions at the boring locations on the dates shown, they are not necessarily indicative of subsurface conditions at other locations or at other times. Borings B-1 through B-9 and B-162 through B-165 were drilled outside of the revised project limits and the results of these borings are not discussed in this section of the report.

Soils within the area of this project have been divided into four categories: artificial fill, roadway embankment, alluvial soils, and residual soils.

Artificial Fill: Artificial fill (AF) was encountered at the surface of borings B-105, B-106, B-137, and B-137(2). The AF extended to depths up to about 6.6 feet. The artificial fill was described as moist to wet, loose to medium dense, silty SAND (A-2-4), soft to stiff, clayey SILT (A-5), and silty CLAY (A-7). A majority of the samples contained trace organic matter and mica. There is also an area of artificial fill near approximate -L- station 621+29 to 616+10, right, as indicated in the field by NCDOT GEU personnel, although no borings were performed in this area and the actual dimensions of the area are unknown. This area is as shown on the attached plans.

Roadway Embankment: Roadway embankment (RE) soils were encountered at the surface of 80 borings; however, boring B-71(2) was an offset of B-71, B-72(2) was an offset of B-72, and B-75(2) was an offset of B-75. The RE extended to depths up to 32 feet. The fill was variable and typically described as moist to wet, loose to medium dense, silty and clayey SAND (A-2-4 & A-2-6), soft to stiff, sandy and clayey SILT (A-4 & A-5), and sandy and silty CLAY (A-6 & A-7). A majority of the samples contained trace organic matter and gravel. Probable boulders were encountered in borings B-71, B-72, B-73, B-75, and B-77 from approximate -L- stations 457+00 to 495+50, left, and may also be encountered in other unexplored areas of the project.

Alluvial Soil: Alluvial soils were encountered in 25 borings. The alluvial soils were typically encountered below artificial fill and roadway embankment. The alluvial soils extended to depths up to 37 feet with an average depth of about 7.5 feet. The alluvial soil types were variable and typically described as moist to wet, loose to medium dense, silty SAND (A-2-4), soft to stiff, sandy and clayey SILT (A-4 & A-5), and sandy and silty CLAY (A-6 & A-7). A majority of the soil samples contained trace organic matter, mica, and gravel.

Residual Soil: A majority of the soils encountered on this project were residual soils. Residual soils were encountered at the surface of 67 borings, below artificial fill at 1 boring, below weathered rock at 1 boring, below alluvial soils at 17 borings, and below roadway embankment at 58 borings. No residual soils were encountered in 13 borings, and these borings were terminated in alluvial soils, artificial fill, roadway embankment, weathered rock, or crystalline rock. The residual soils were variable and typically described as moist to wet, loose to medium dense, silty and clayey SAND (A-2-4 & A-2-6), medium stiff to very stiff, sandy and clayey SILT (A-4 & A-5), and soft to stiff, silty and sandy CLAY (A-6 & A-7). A majority of the samples contained varying amounts of mica, manganese, and rock fragments.

Rock Properties

Weathered Rock (WR) was encountered in 47 borings. Of these 47 borings, 5 of the borings terminated in residual soils, 17 terminated in WR, and 25 terminated in or on Crystalline Rock (CR). Thirteen of the borings encountered intermediate layers of WR before re-encountering soil or rock below this layer. Of these 13 borings, five borings terminated in residual soils and the remaining eight borings re-encountered WR and/or CR at boring termination. These intermediate zones of WR ranged in thickness from about 1.5 to 15 feet. Excluding the intermediate zones of WR, the WR was encountered at depths ranging from about 2 to 57 feet and elevations ranging from about 2,028 to 2,187 feet.

CR was encountered in 36 borings as indicated by auger and SPT refusal. All of these borings were terminated in or on CR. Of these 36 borings, 12 transitioned directly from residual soils to CR, and 24 encountered WR before being terminated. Boring B-91 encountered an intermediate layer of CR before re-encountering soil and rock below this layer. The intermediate zone of CR was about 2.1 feet thick. The WR layers above the CR varied in thickness from 0.9 to 18.5 feet. The CR was encountered at depths ranging from about 4 to 67 feet, or elevations ranging from about 2,018 to 2,183 feet. The rock typically consisted of gneiss. Refusal is a designation applied to any material that cannot be penetrated by the soil auger, and is typically caused by encountering boulders, hard rock lenses/ledges or bedrock. The nature of the materials causing refusal was not explored in the majority of these borings, but is anticipated to represent the CR level.

Rock coring was performed in borings B-14, B-21, and B-26 after encountering auger refusal. The coring extended to depths of 49.9, 65.9, and 56.0 feet, respectively, after SPT refusal was achieved. The rock generally consisted of very slightly to severely weathered, soft to very hard, biotite gneiss. The strata recovery and RQD are indicated on the cross sections. Strata recovery ranged from 51% to 82%, and strata RQD ranged from 6% to 64%.

Groundwater Properties

Generally, groundwater measurements were attempted in a majority of the 157 borings along the project immediately upon their completion and after a stabilization period of approximately 24 hours. Nine borings were backfilled immediately upon their completion. Immediately upon completion, groundwater was encountered in 29 borings at depths ranging from 0 to 56.2 feet, and elevations ranging from about 2,055 to 2,172 feet. Stabilized groundwater was encountered in 45 borings at depths ranging from 0 to 47.1 feet, and elevations ranging from 2,058 to 2,154 feet. Groundwater was not encountered in the remaining 107 borings. The recovered soil samples were generally described as moist above the groundwater level and wet or saturated below the groundwater level. It should be noted that the groundwater levels fluctuate depending upon seasonal factors such as precipitation and temperature. As such, soil moisture and groundwater conditions at other times may vary or be different from those described in this report.

We appreciate the opportunity to work with you on this project. Please contact us if you have any questions regarding this report or if we may be of further service.

Sincerely,
FROEHLING & ROBERTSON, INC.

DocuSigned by:

 Patrick Alton, P.E.
 Transportation Services Manager

DocuSigned by:

 Derrick Racey
 Geotechnical Project Manager

Appendix A

Bulk Samples

The following bulk samples were obtained and transported to our laboratory for testing to determine the engineering properties of the soil:

Sample No.	Boring No.	Line	Station	Offset	Depth (ft)	Test(s) Performed
CBR-1	B-10	-Y1RPD-	16+23	105' Lt.	3.0-18.0'	Standard Proctor, CBR
CBR-2	B-109	-L-	549+16	138' Rt.	3.3-25.0'	Standard Proctor, CBR
CBR-3	B-110	-L-	577+18	101' Rt.	3.5-10.0'	Standard Proctor, CBR
CBR-4	B-118	-L-	601+01	161' Rt.	5.0-23.5'	Standard Proctor, CBR
CBR-5	B-119	-L-	608+24	160' Rt.	3.5-25.0'	Standard Proctor, CBR

Undisturbed Samples

Undisturbed Shelby tube samples were not requested to be collected as part of this investigation.

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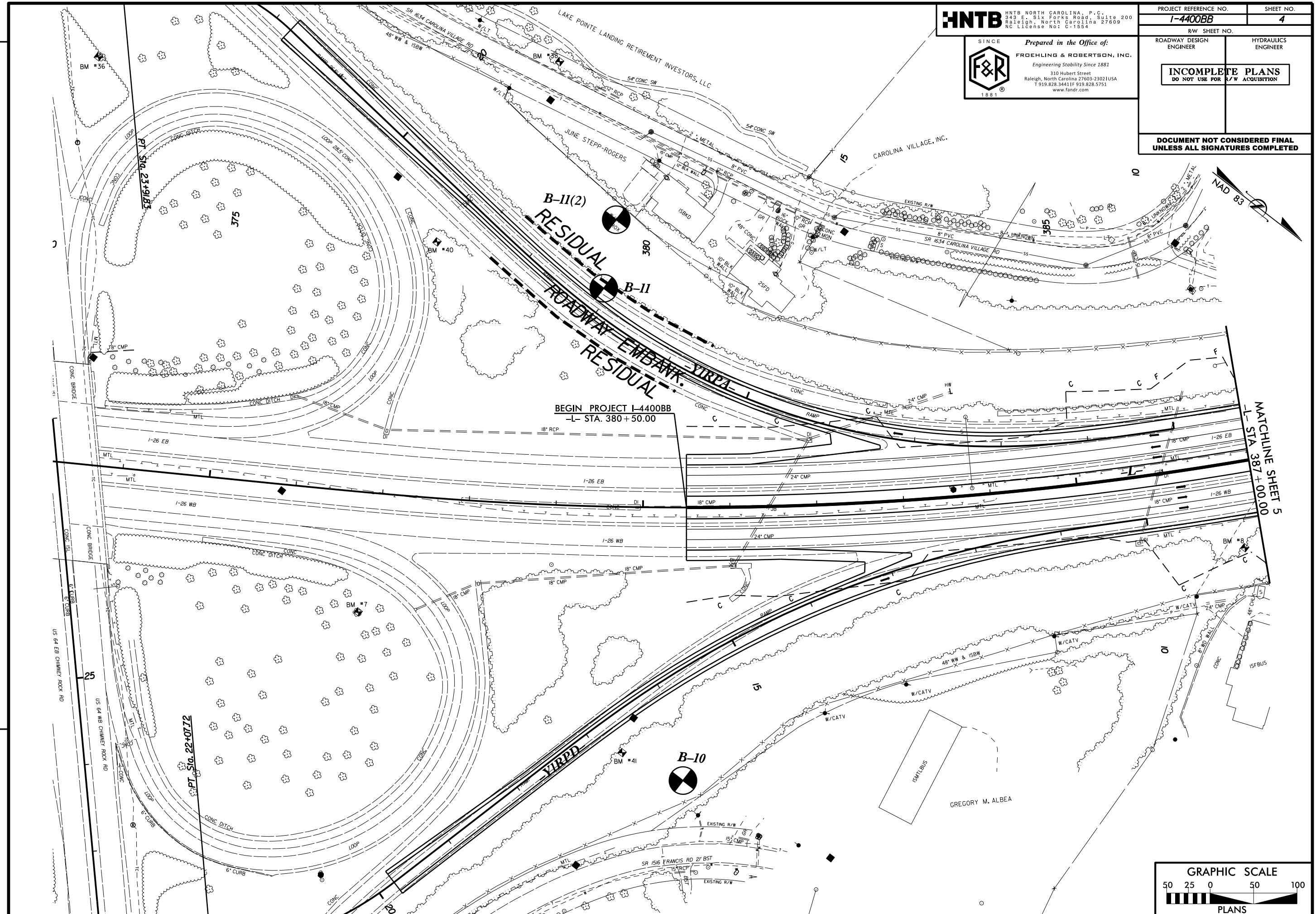
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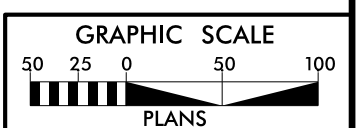
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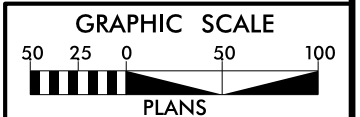
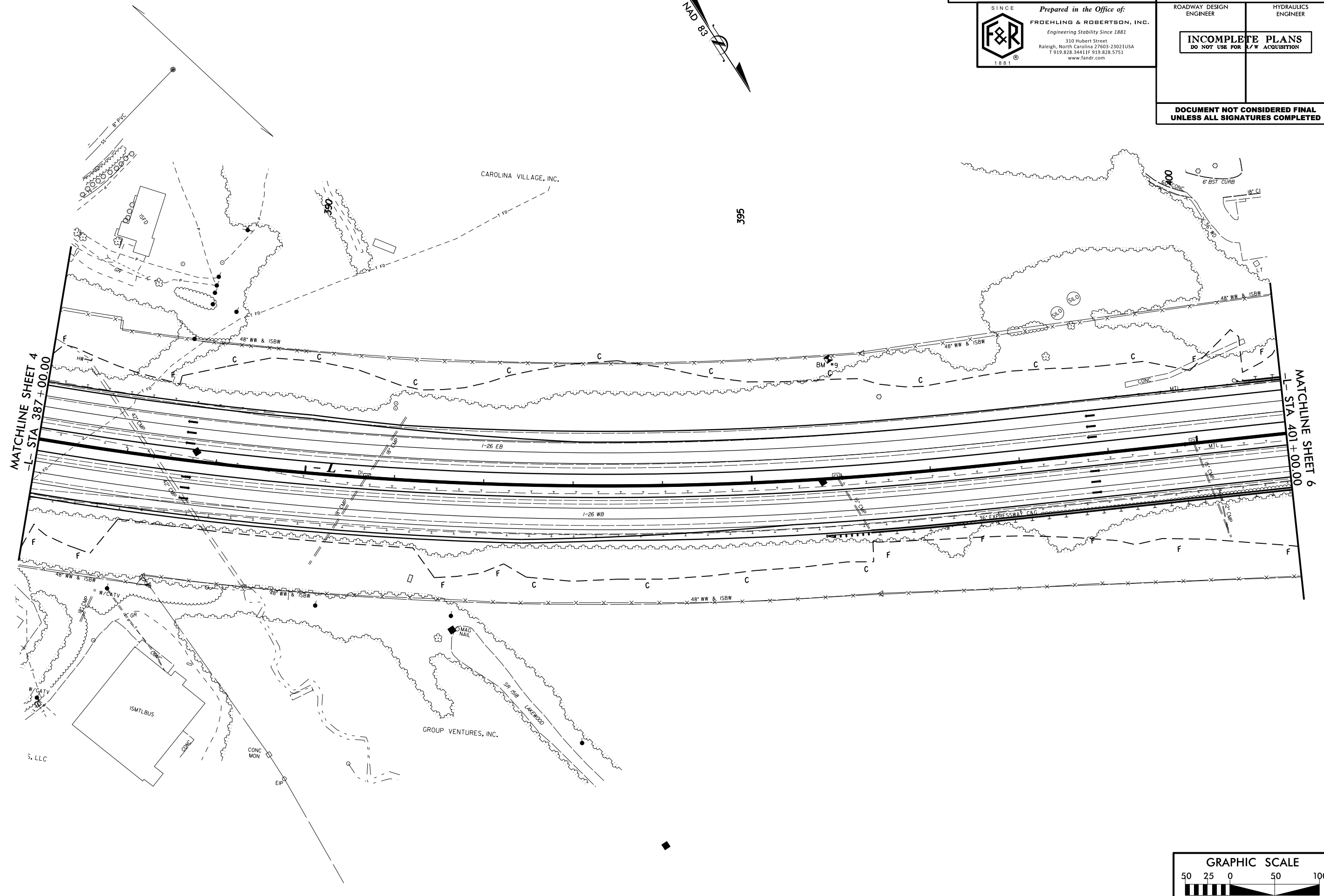
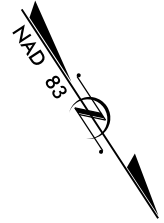
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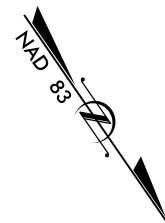
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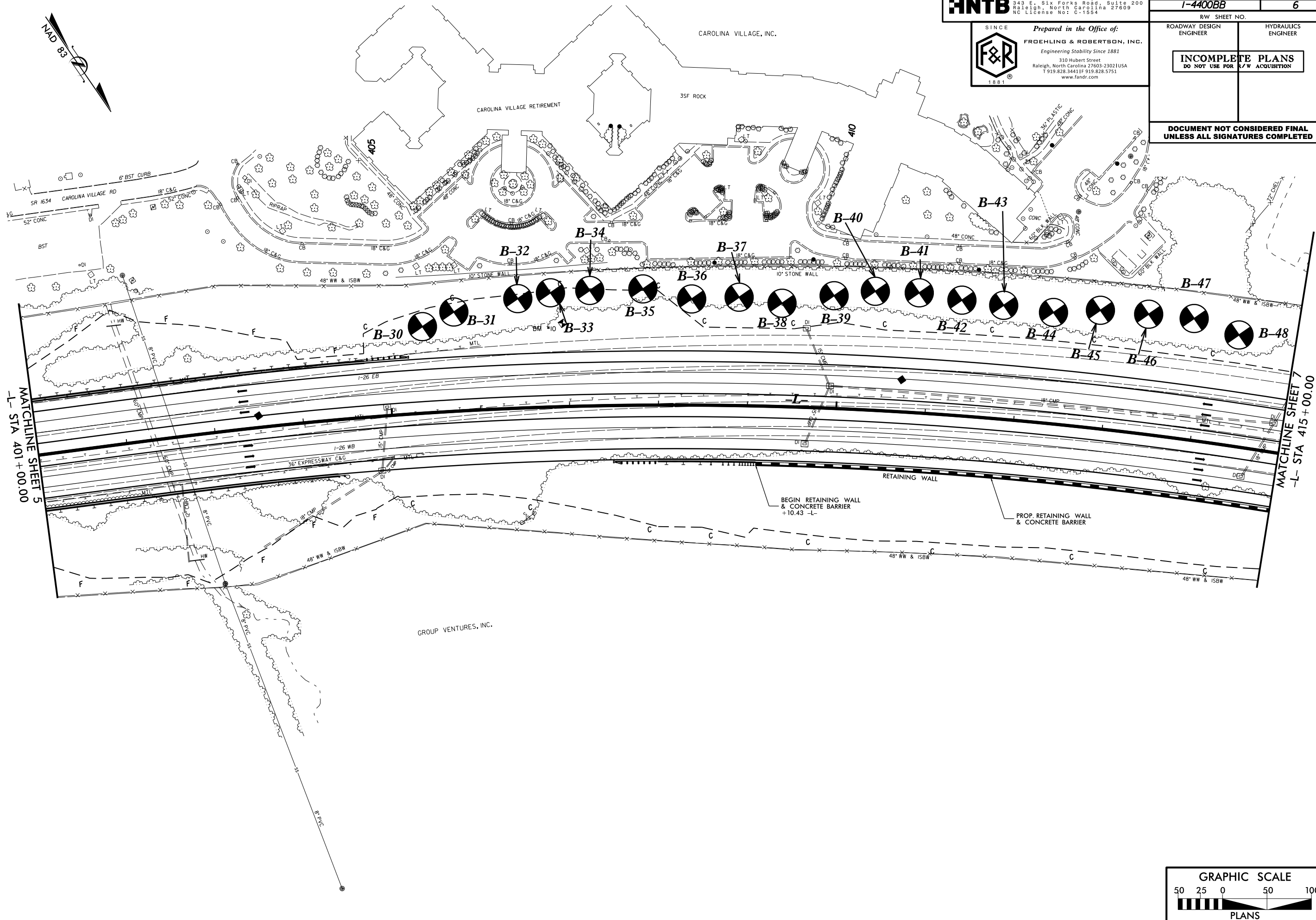


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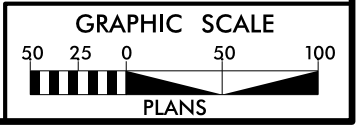
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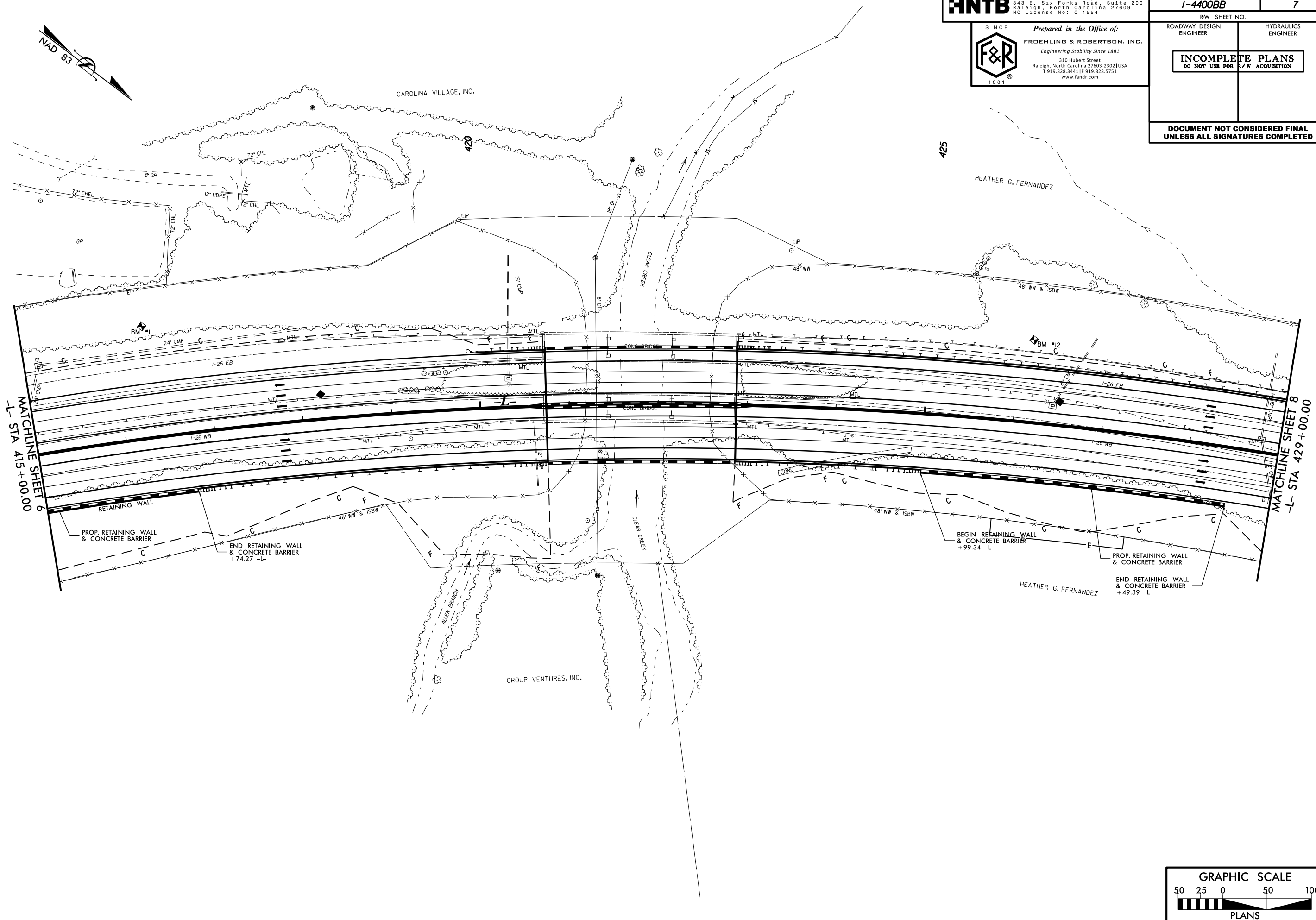
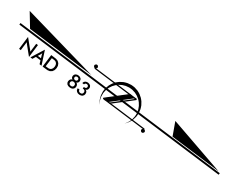


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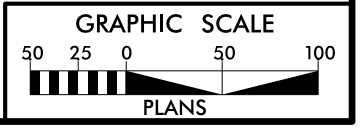
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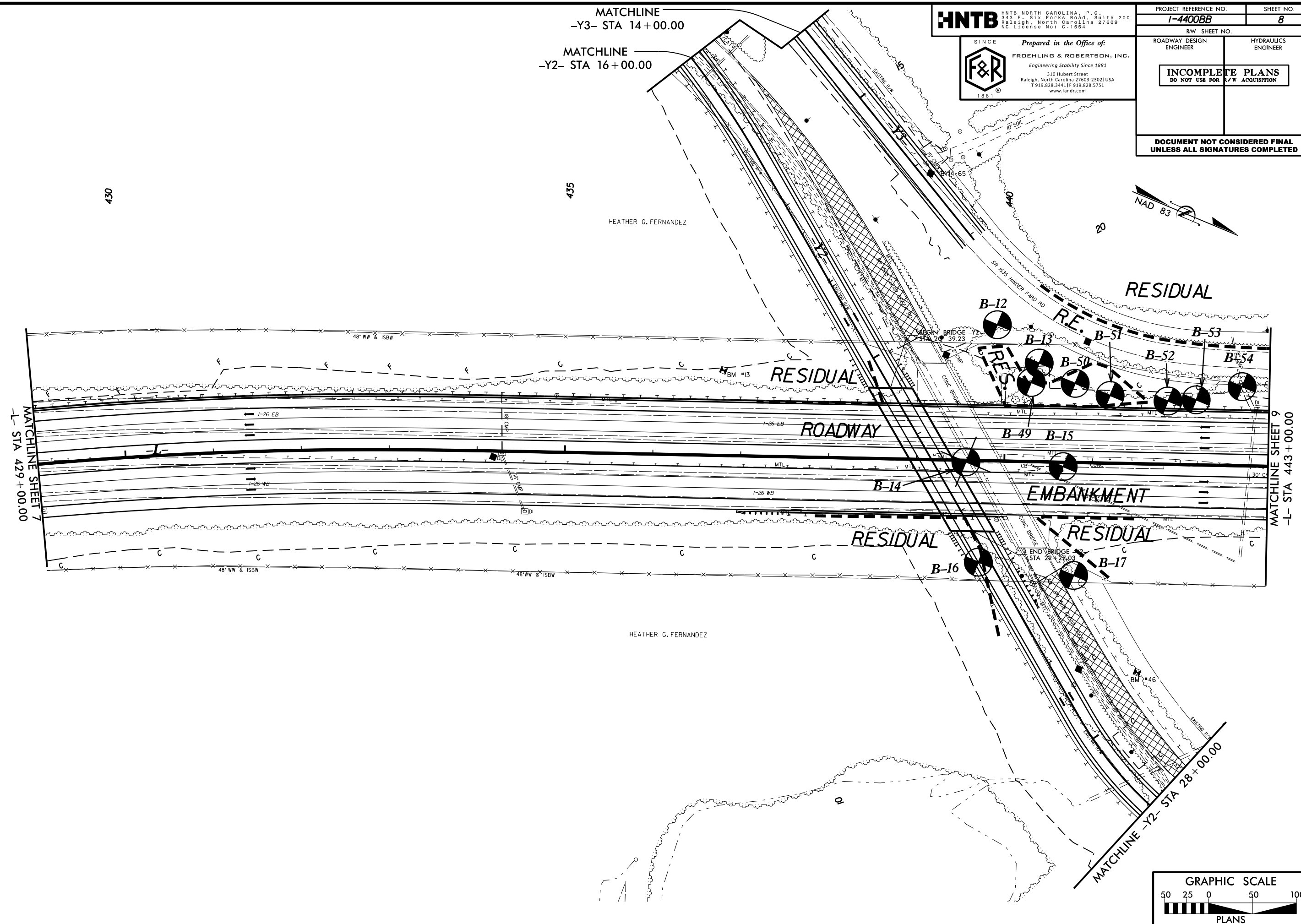
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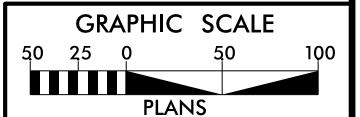
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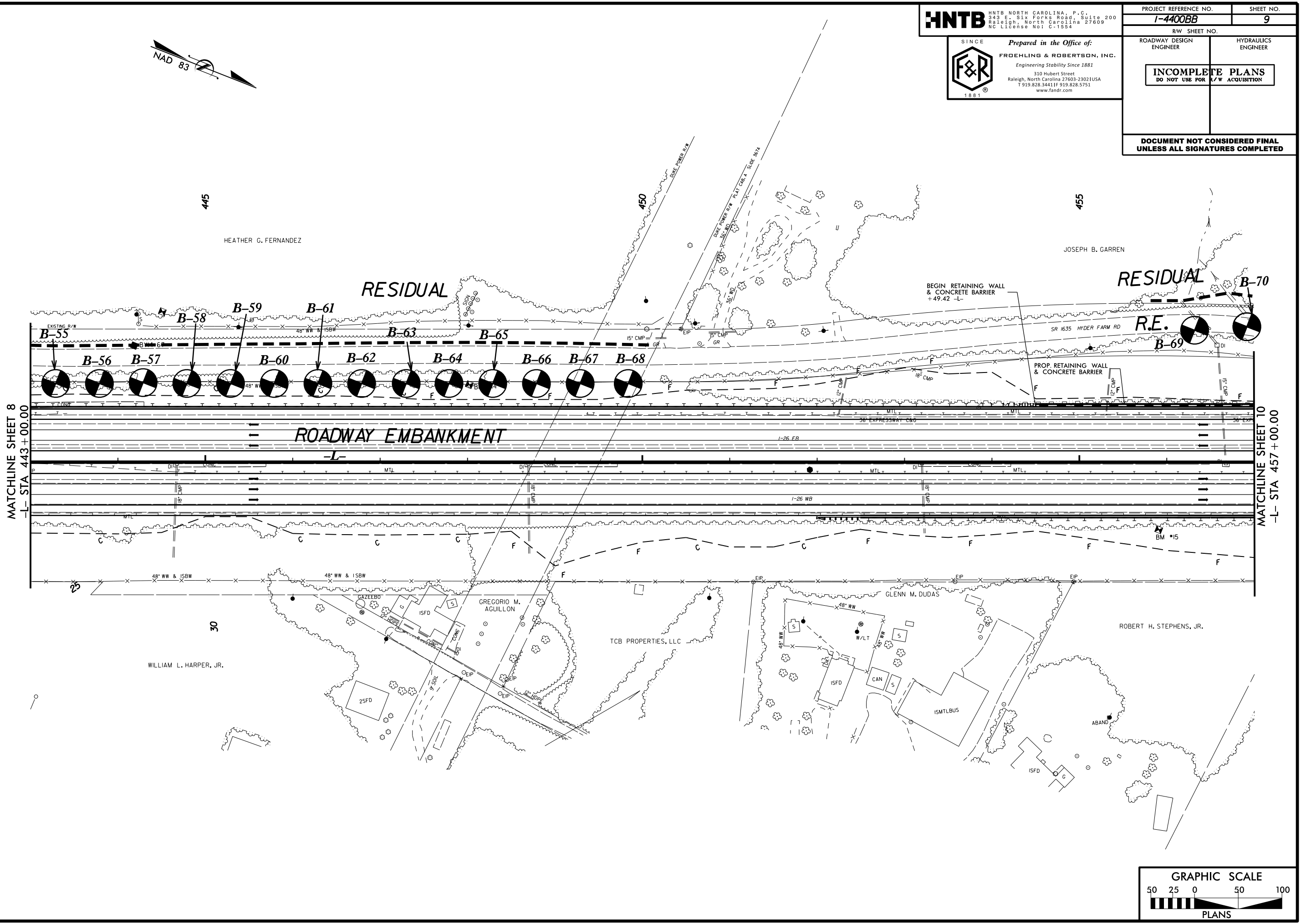
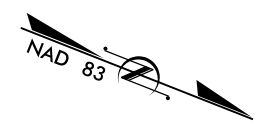


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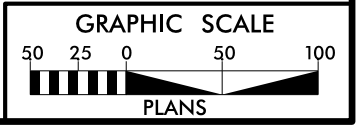
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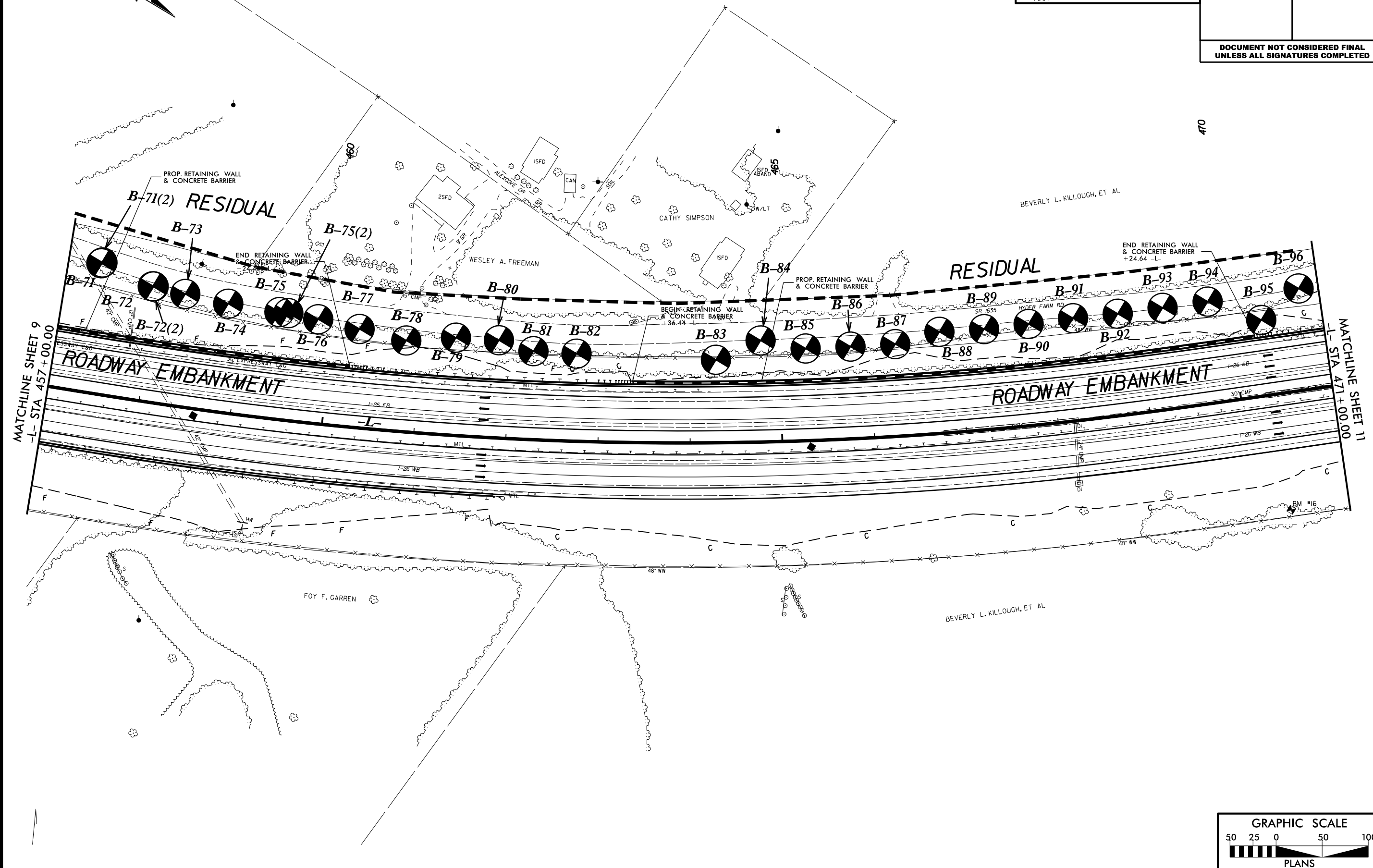
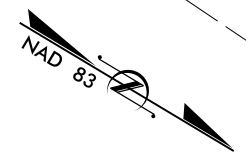
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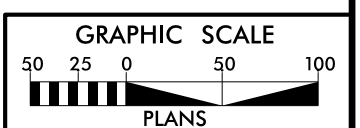
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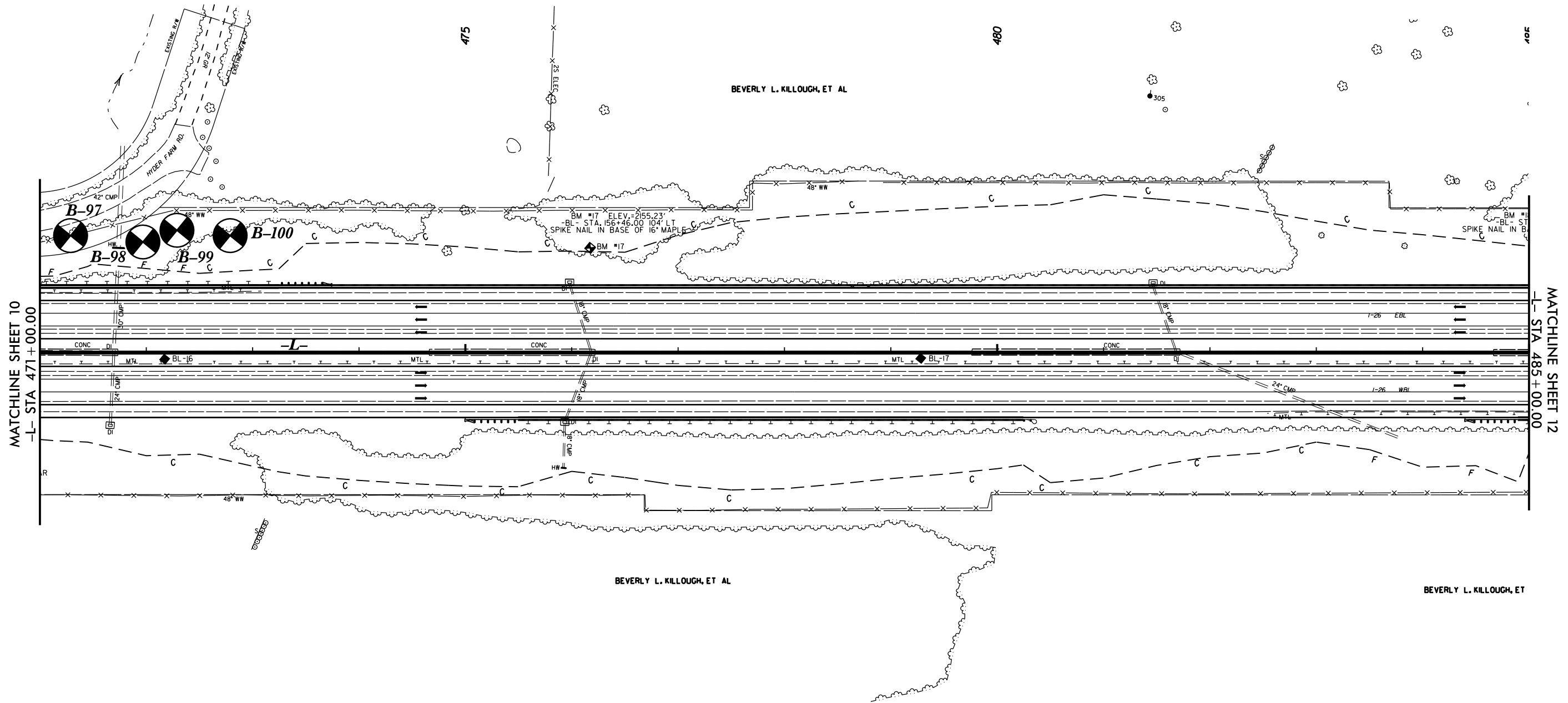
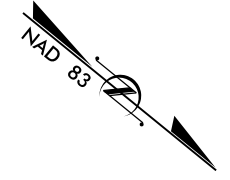


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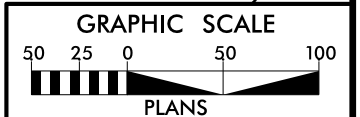
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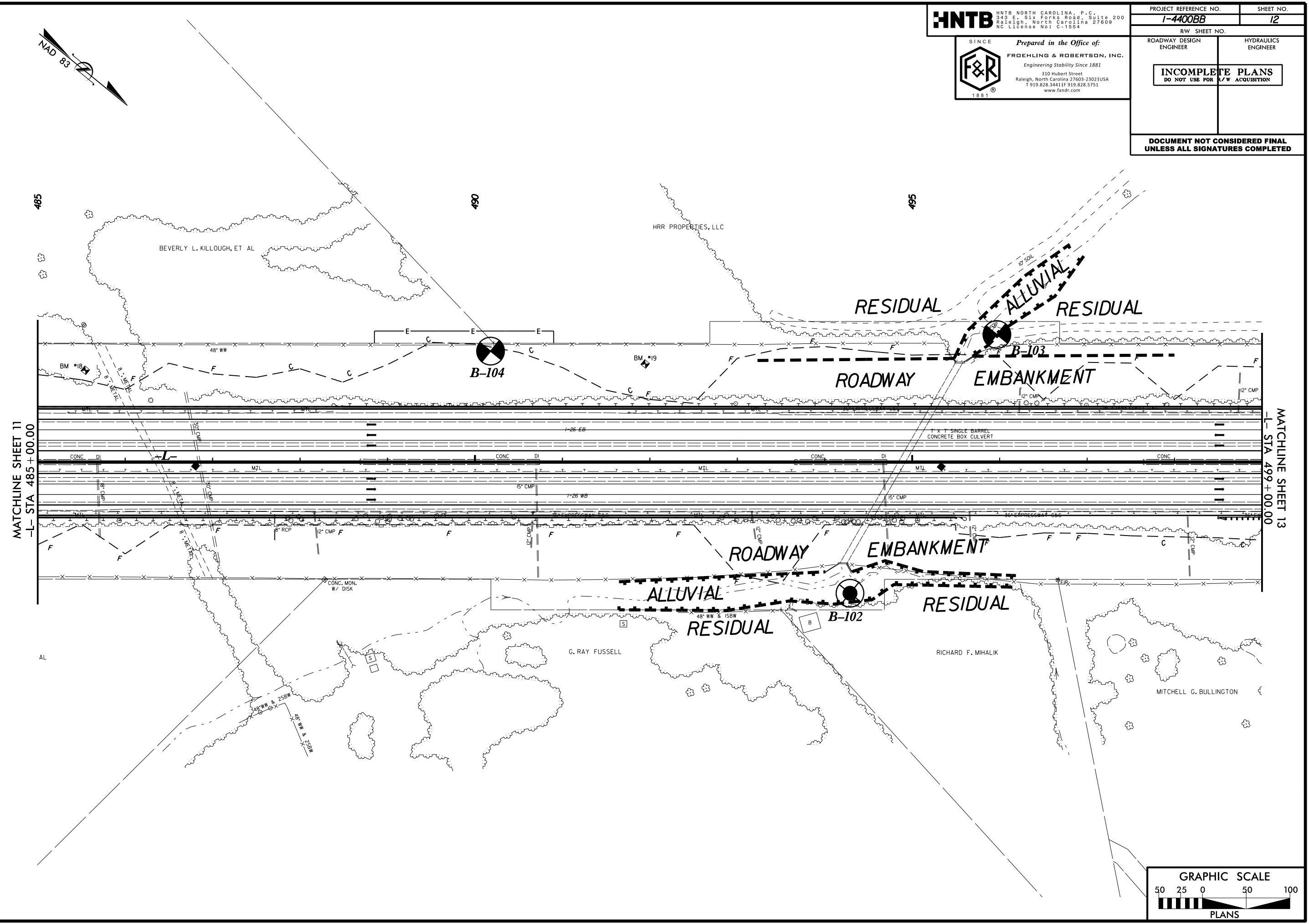
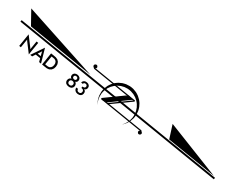


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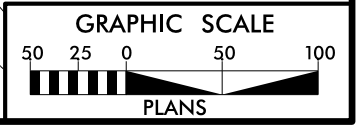
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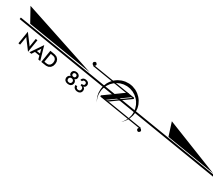
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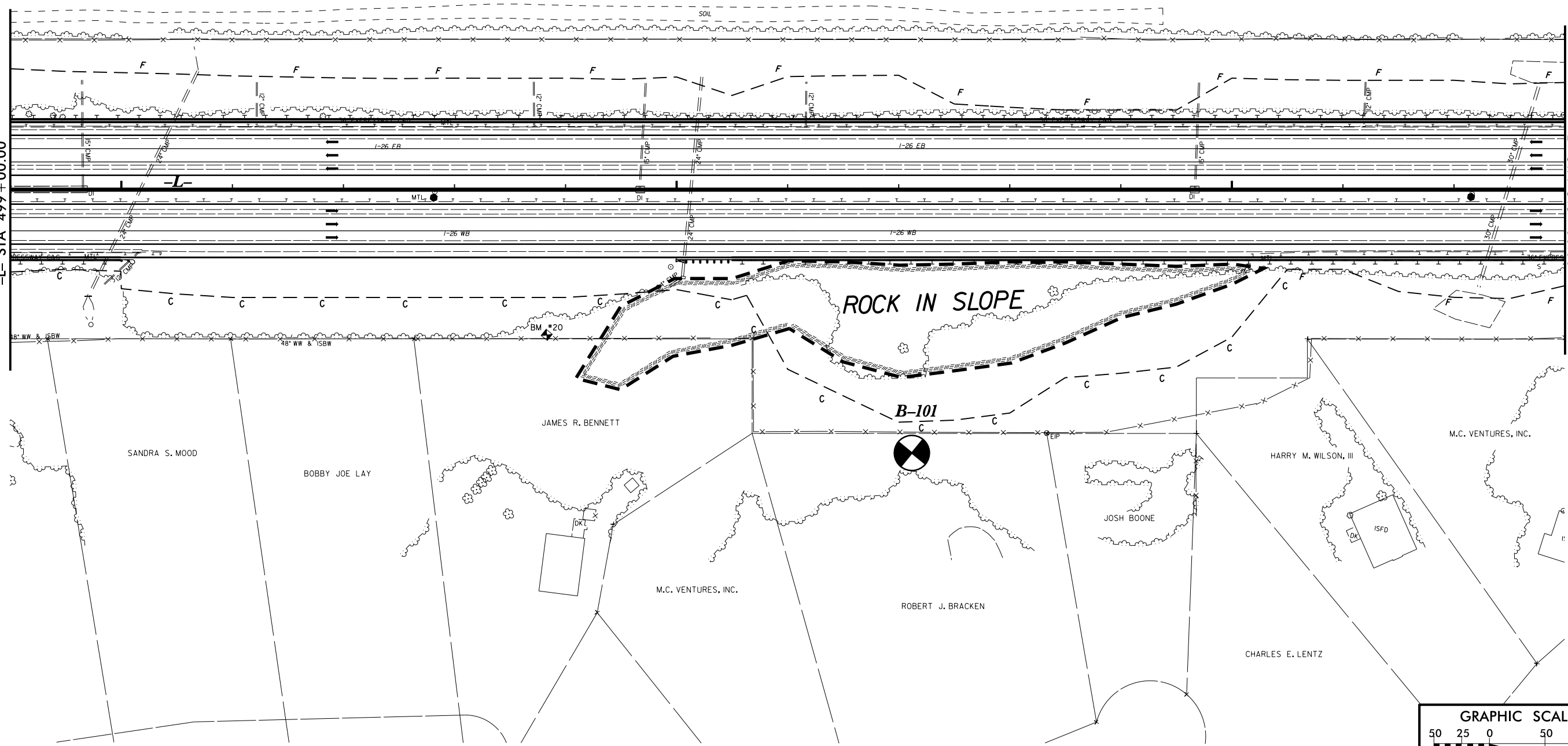
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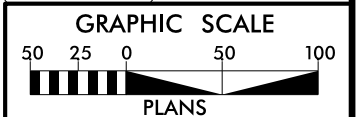
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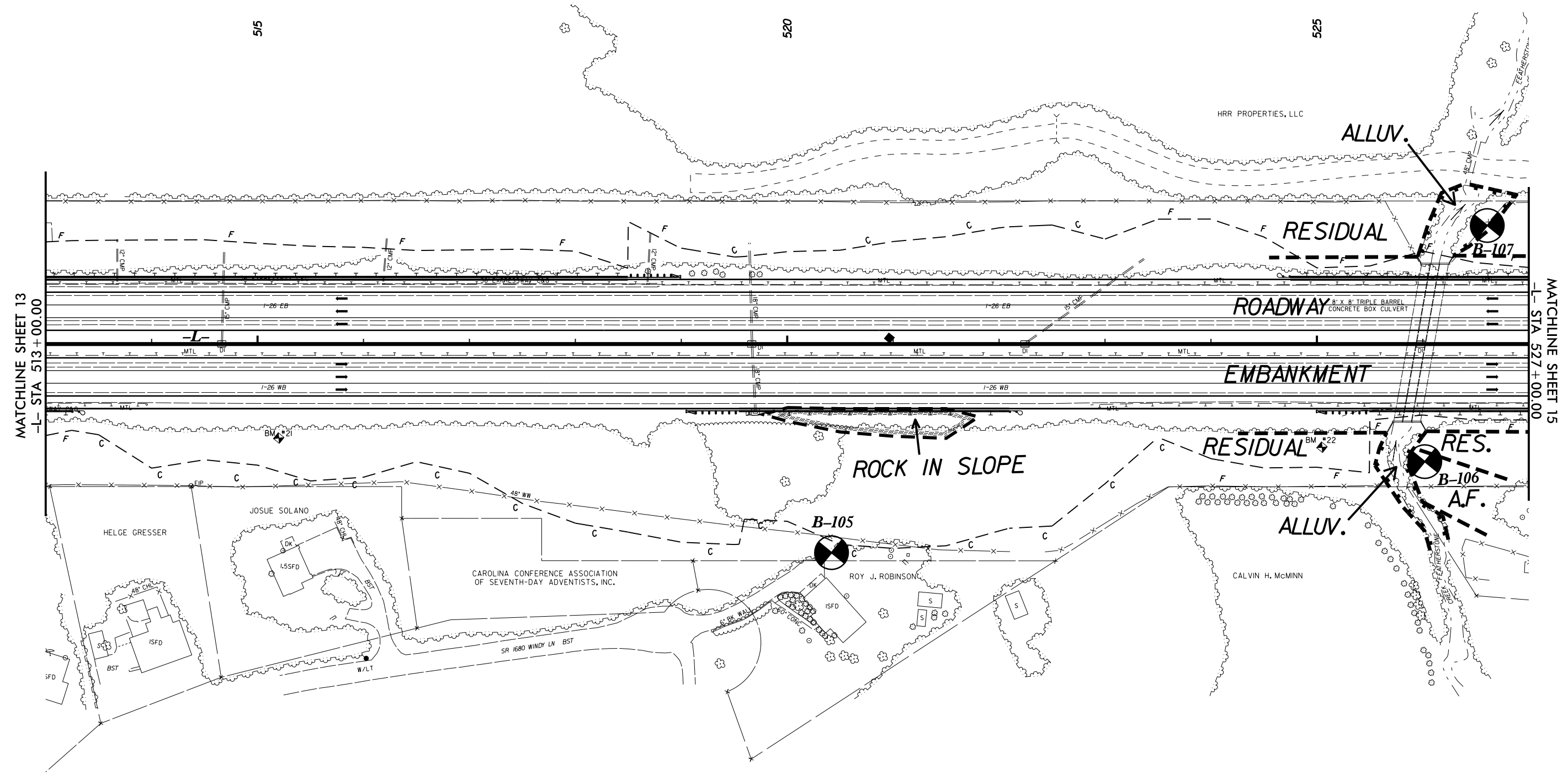
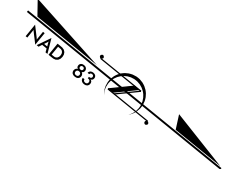


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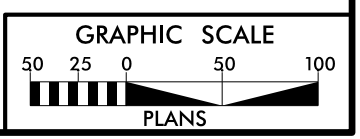
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PROJECT REFERENCE NO. 1-4400BB	SHEET NO. 14
RW SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	ROADWAY DESIGN ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



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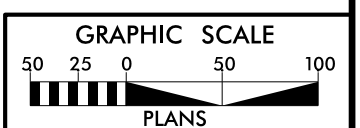
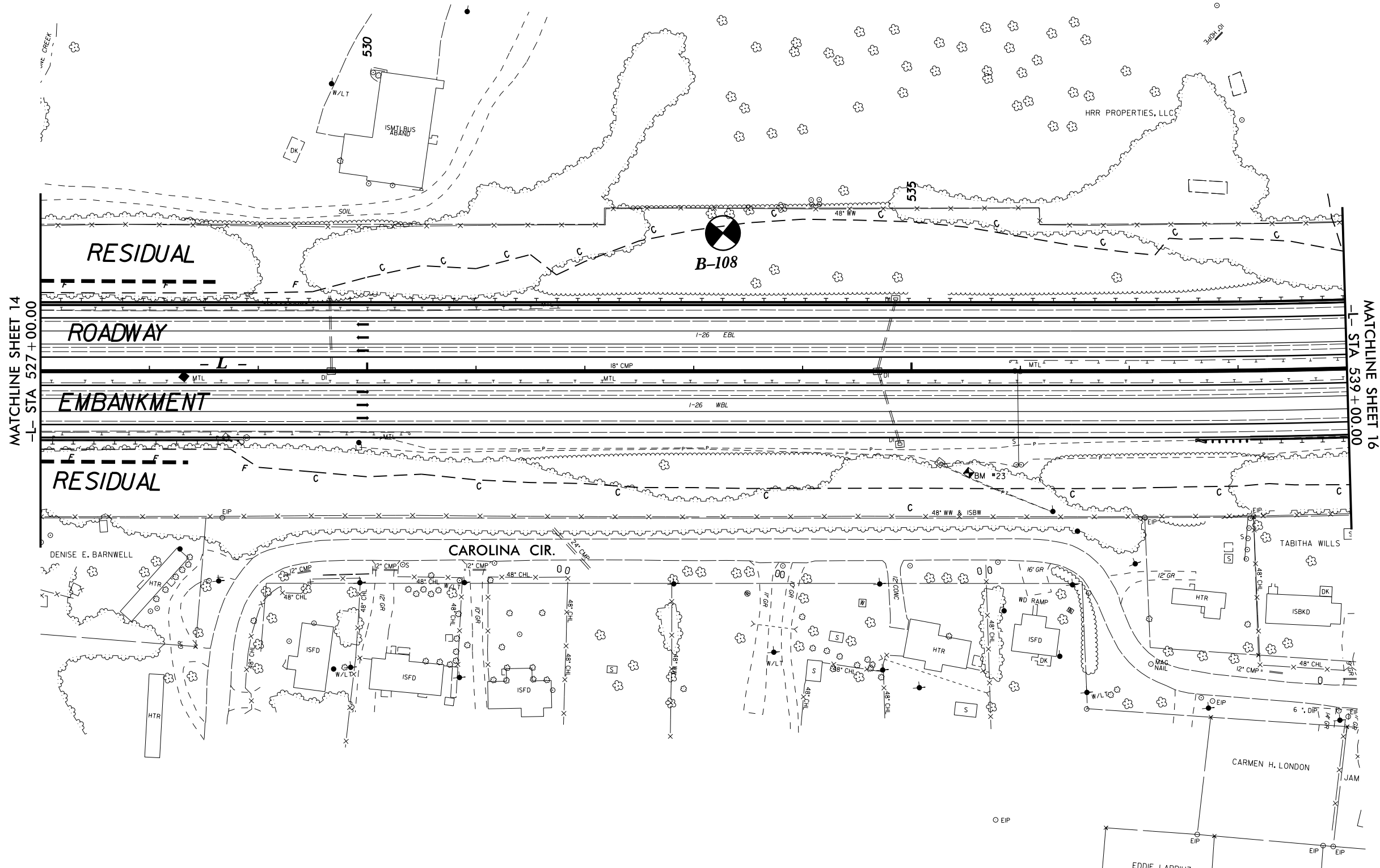
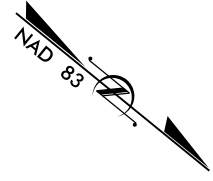
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PROJECT REFERENCE NO. I-4400BB	SHEET NO. 15
RW SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

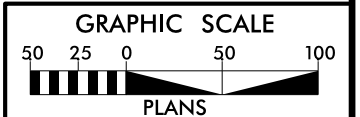
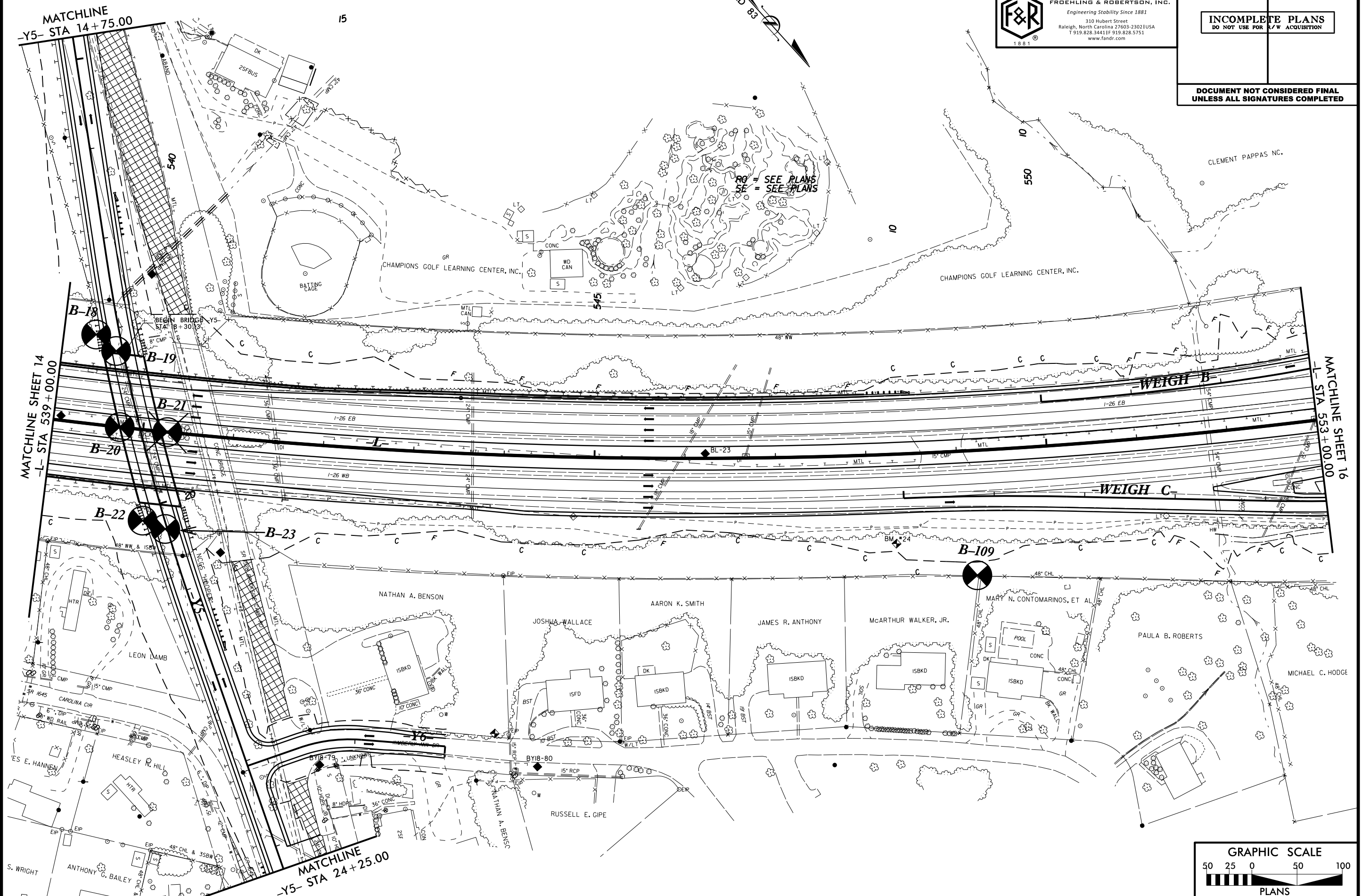
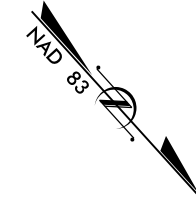


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PROJECT REFERENCE NO. 1-4400BB	SHEET NO. 16
RW SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
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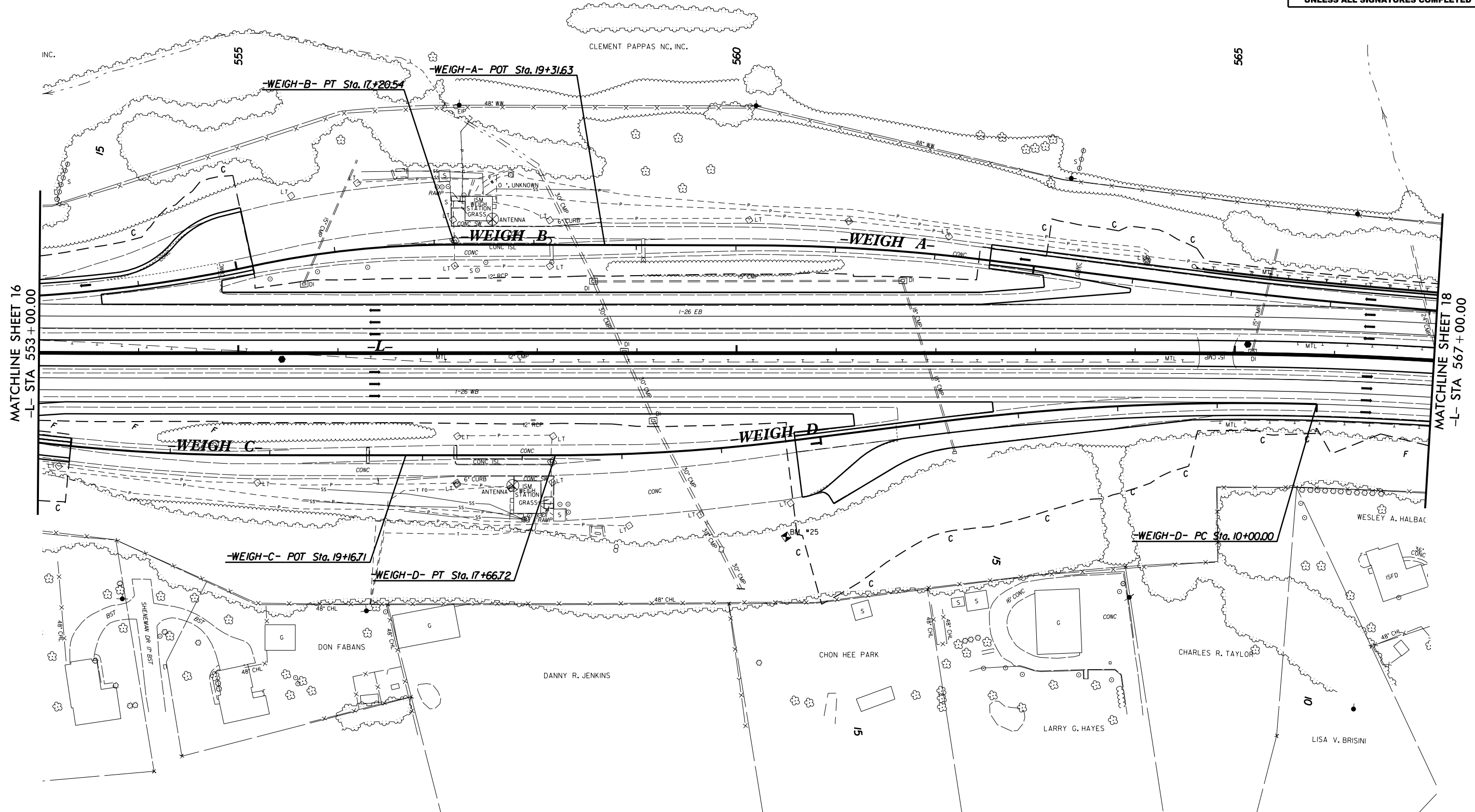
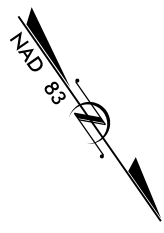
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 NC LICENSE NO: C-1554

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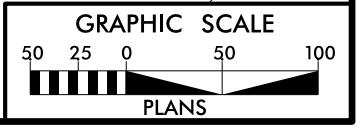
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PROJECT REFERENCE NO. I-4400BB	SHEET NO. 17
RW SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
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MATCHLINE SHEET 16
 -L- STA 553+00.00

MATCHLINE SHEET 18
 -L- STA 567+00.00



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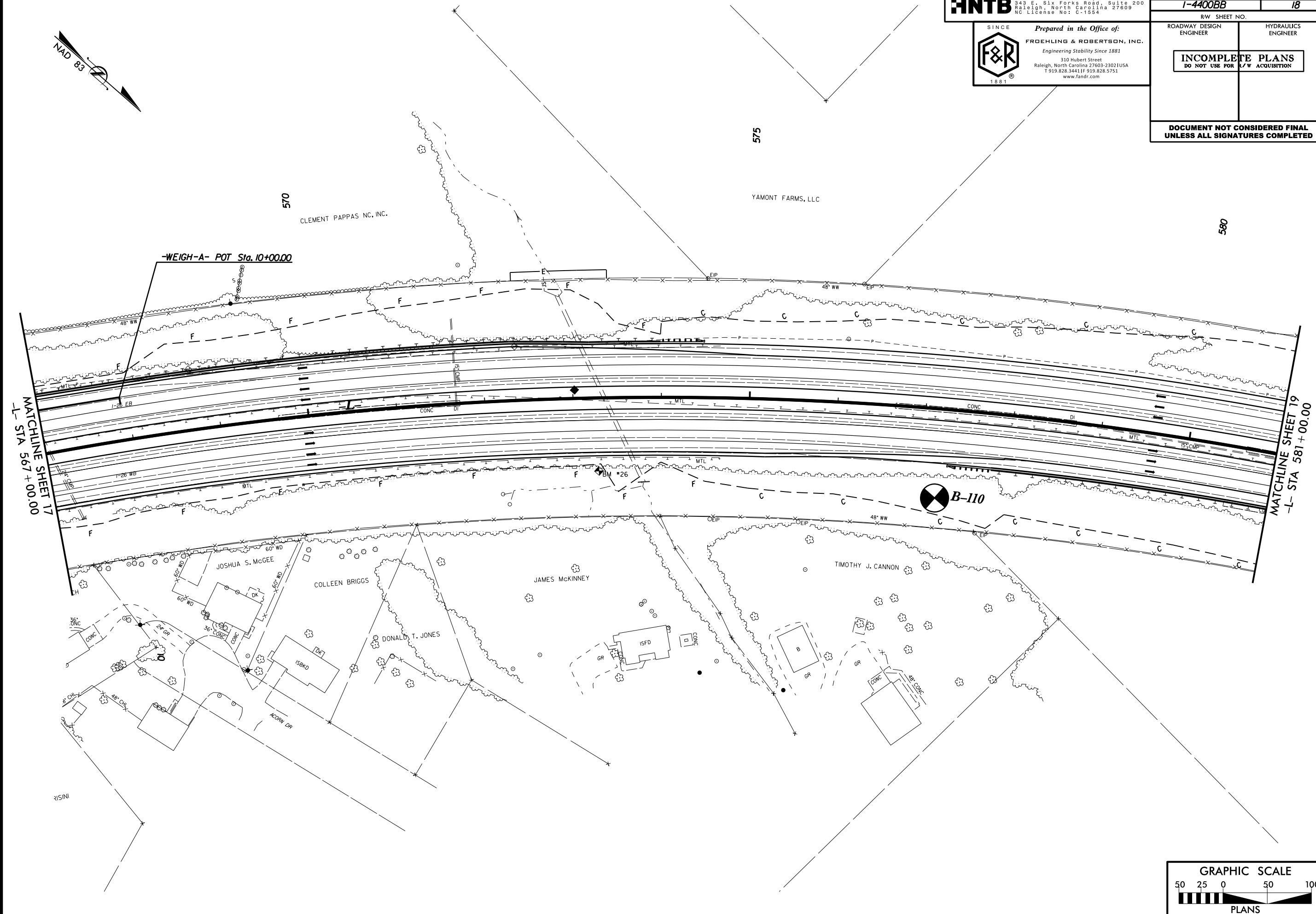
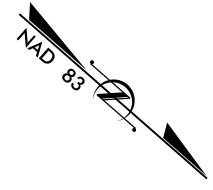
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 NC LICENSE NO: C-1554

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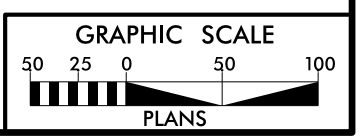
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PROJECT REFERENCE NO. I-4400BB	SHEET NO. 18
RW SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	ROADWAY DESIGN ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
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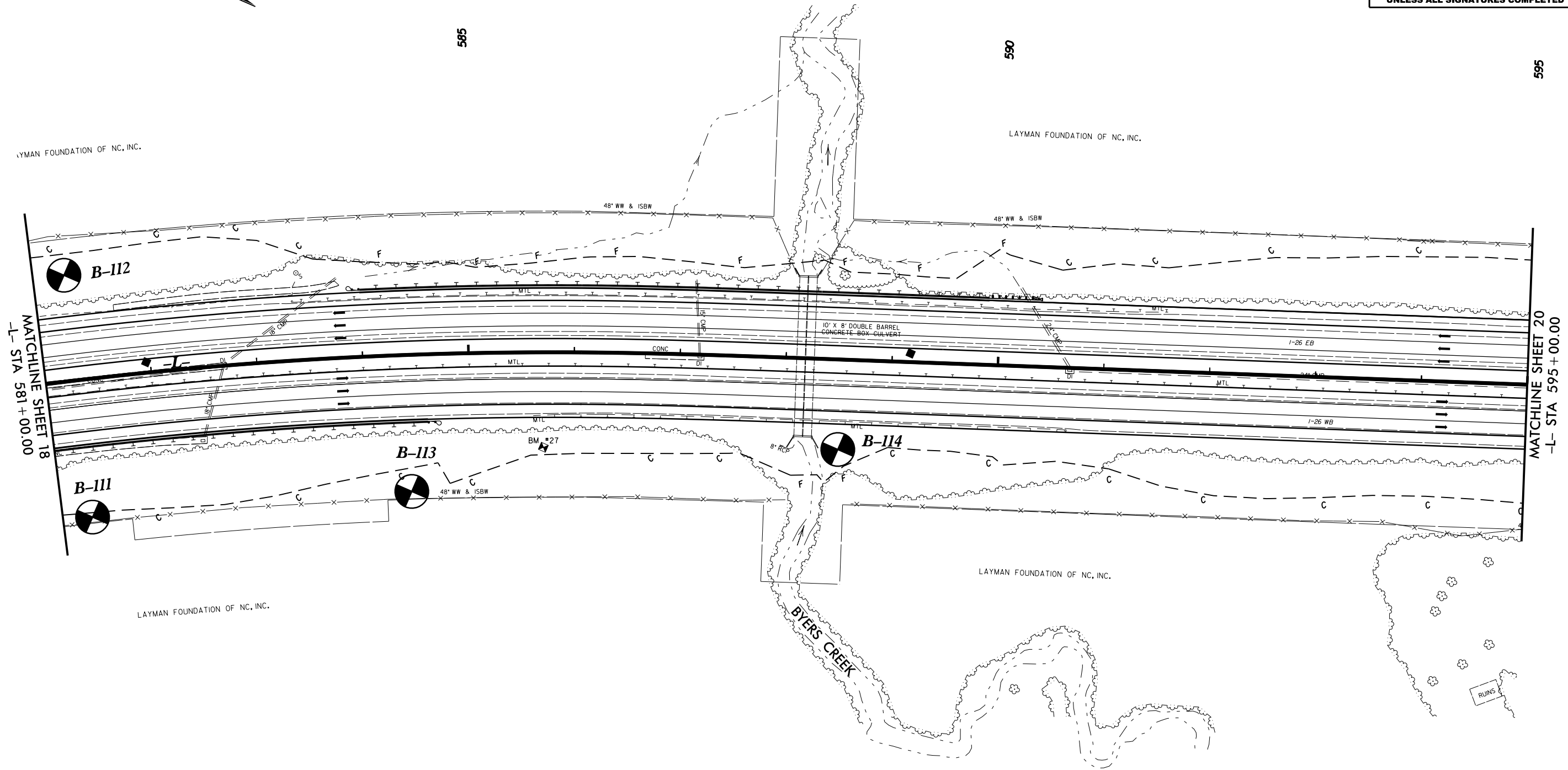
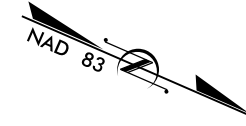


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PROJECT REFERENCE NO. 1-4400BB	SHEET NO. 19
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
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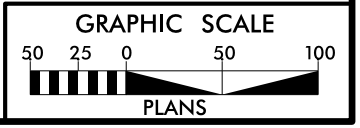


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MATCHLINE SHEET 20
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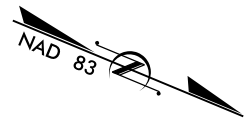


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PROJECT REFERENCE NO. 1-4400BB	SHEET NO. 20
RW SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	ROADWAY DESIGN ENGINEER
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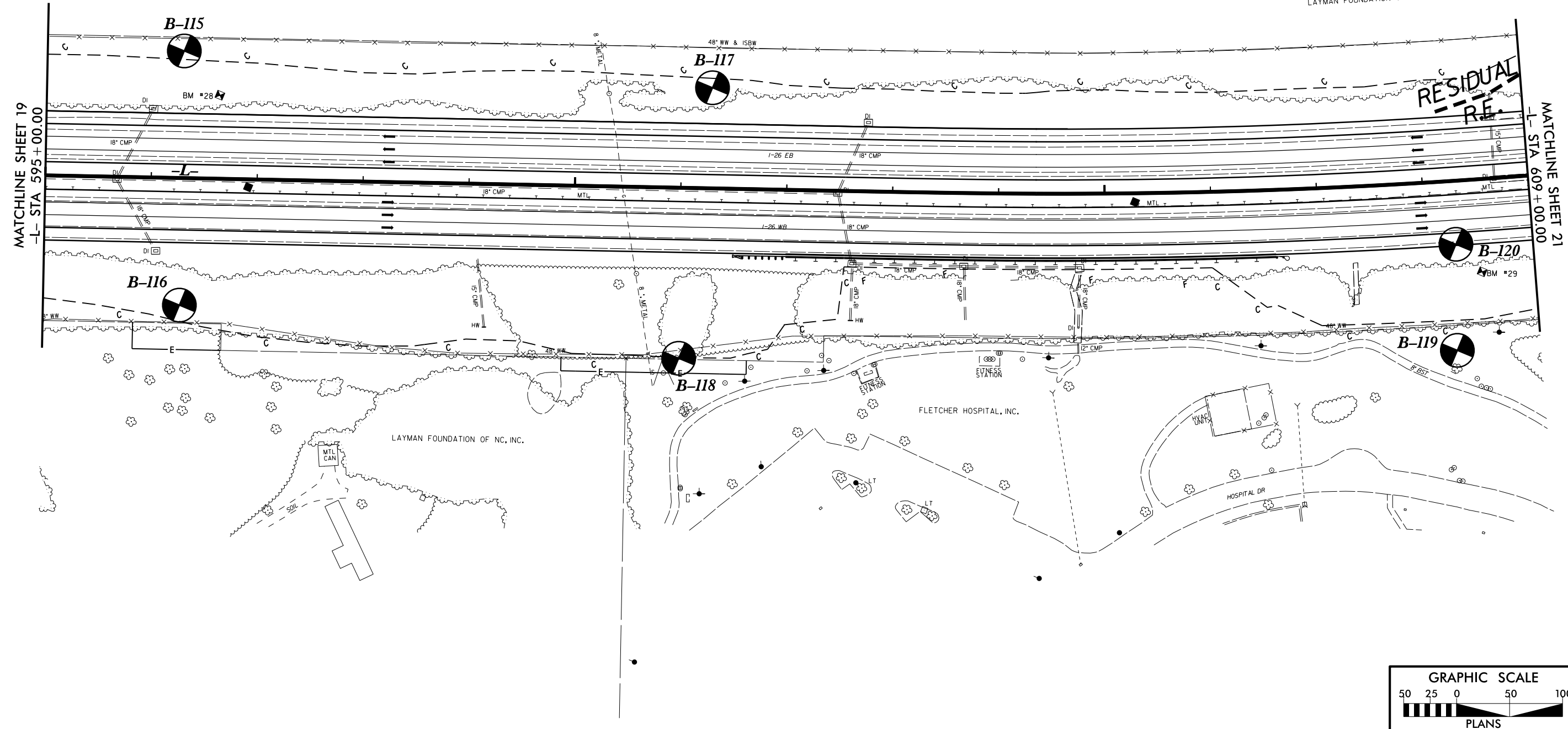
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LAYMAN FOUNDATION OF NC, INC.

LAYMAN FOUNDATION OF NC, INC.



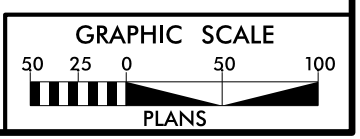
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RESIDUAL RE.

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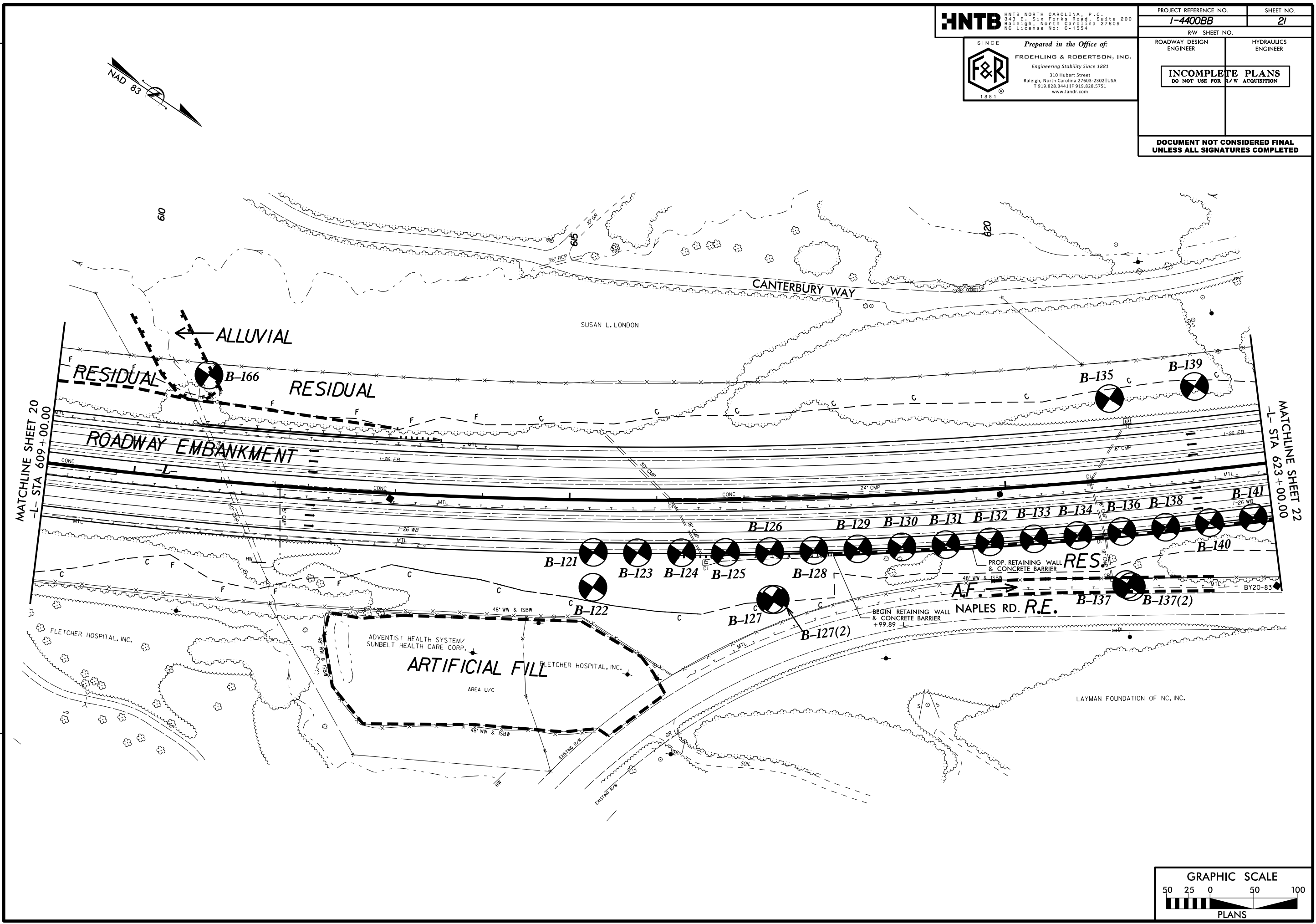


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PROJECT REFERENCE NO. I-4400BB	SHEET NO. 21
R/W SHEET NO. ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
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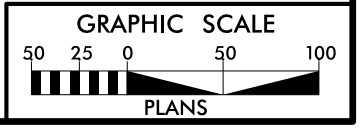


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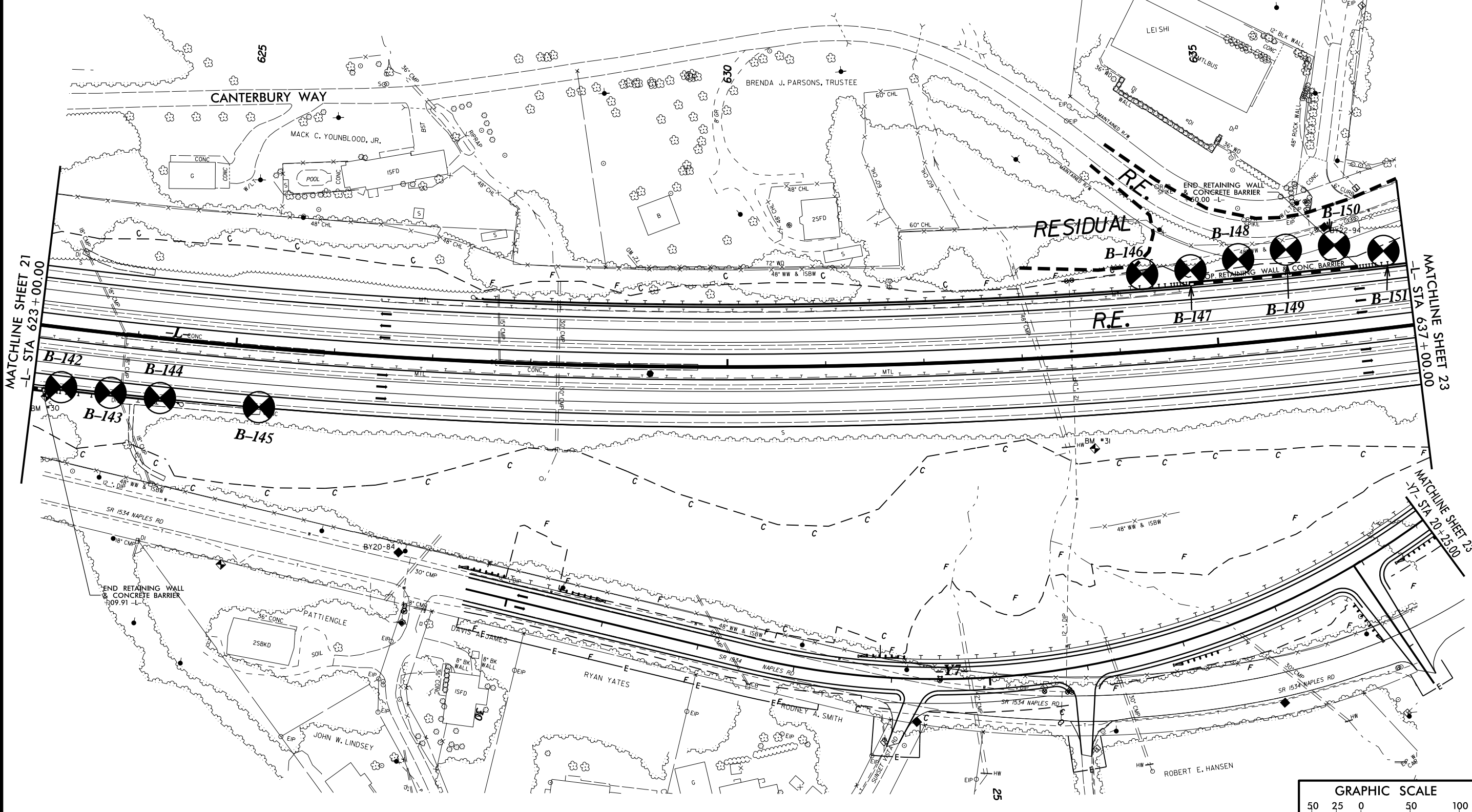
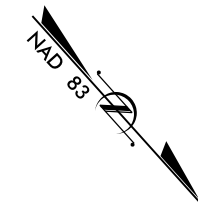


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 NC LICENSE NO: C-1554

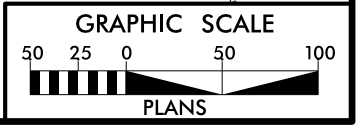
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PROJECT REFERENCE NO. 1-4400BB	SHEET NO. 22
RW SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
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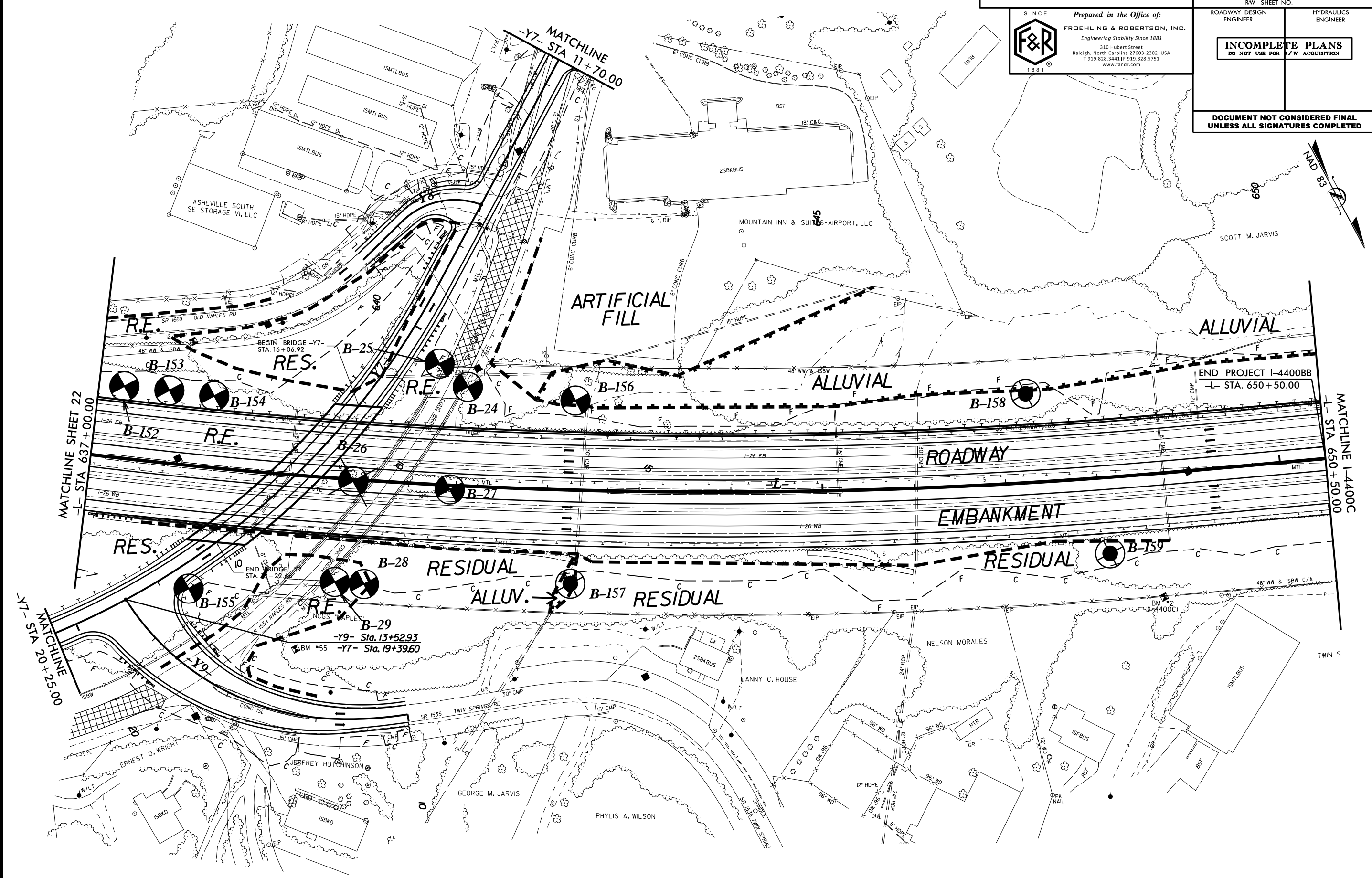


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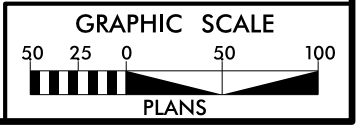
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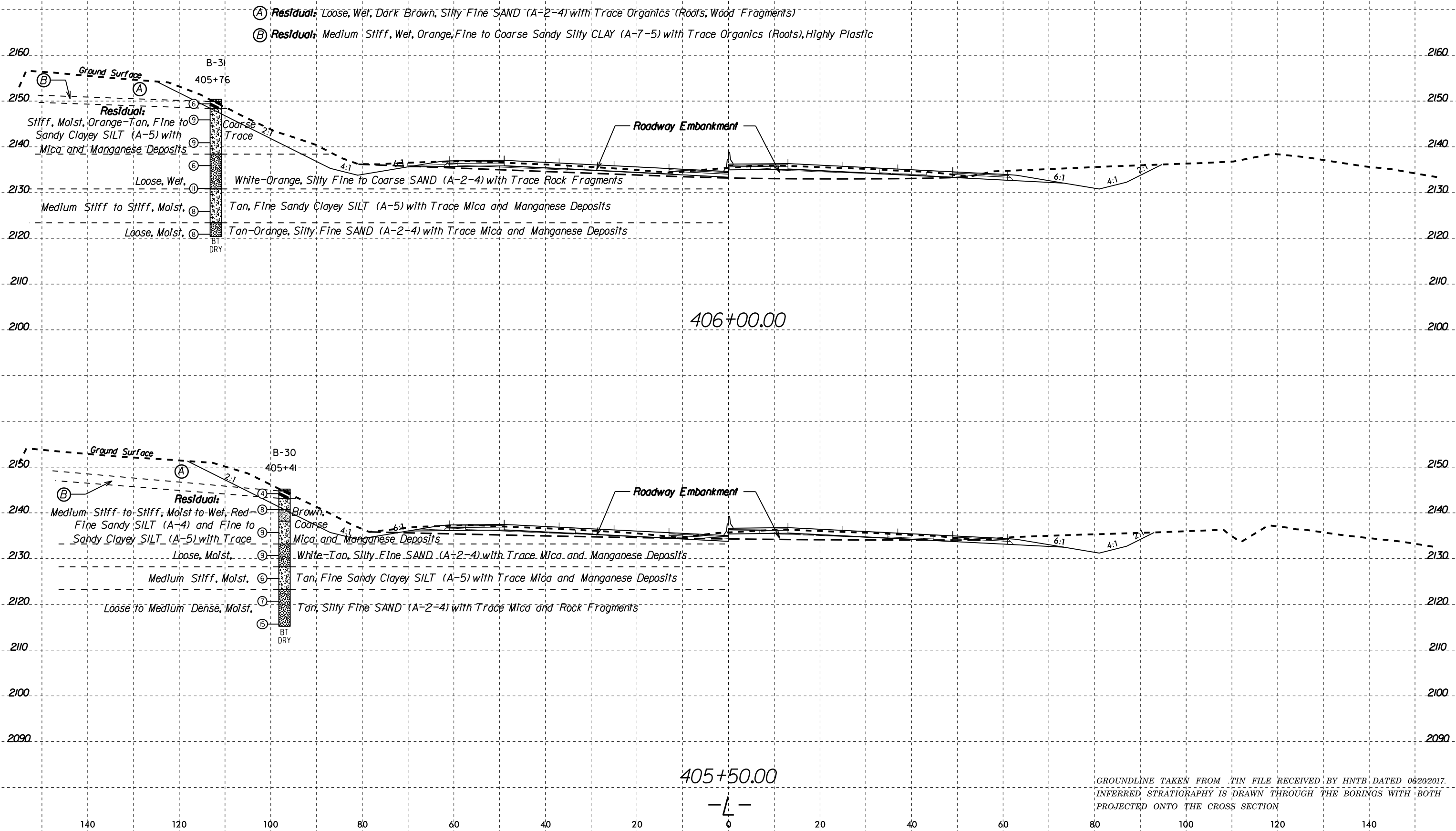
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PROJECT REFERENCE NO. I-4400BB	SHEET NO. 23
RW SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	ROADWAY DESIGN ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



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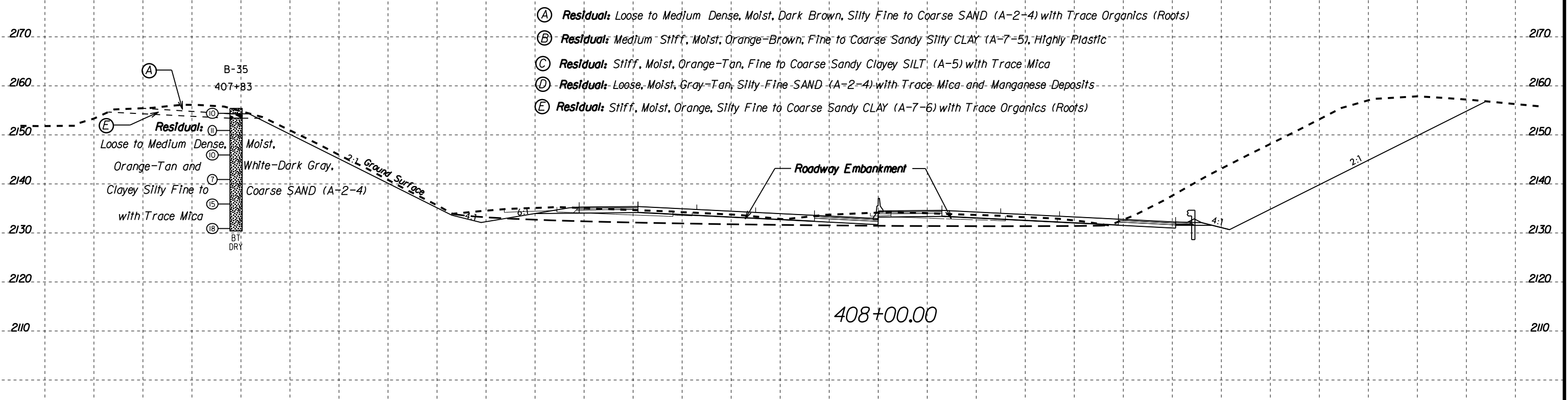




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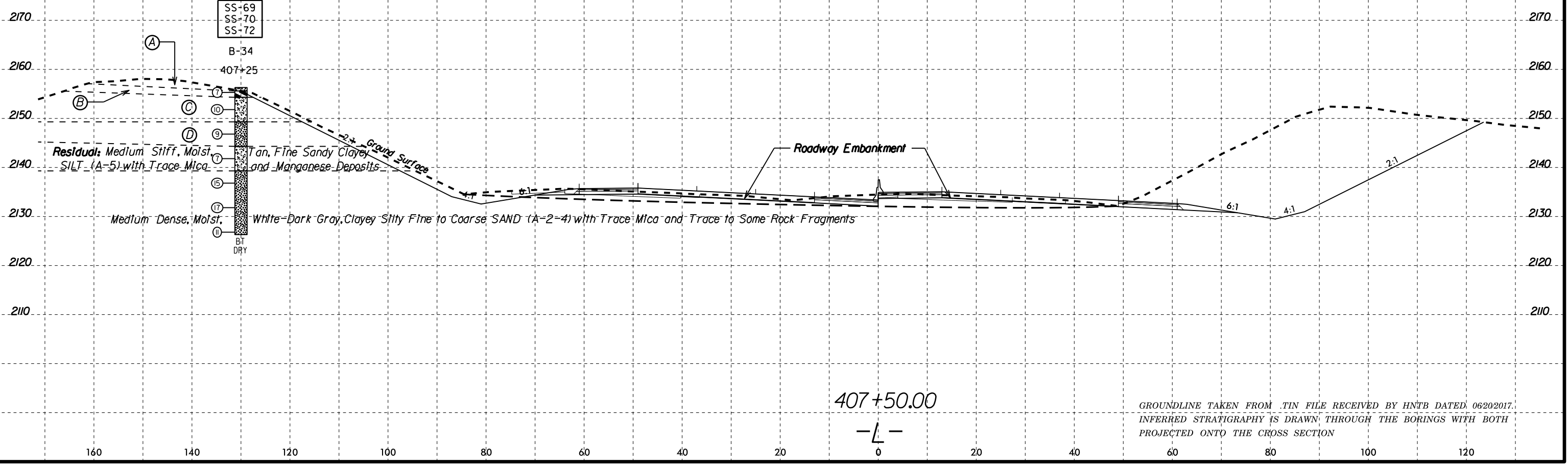
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6/23/16



SOIL TEST RESULTS

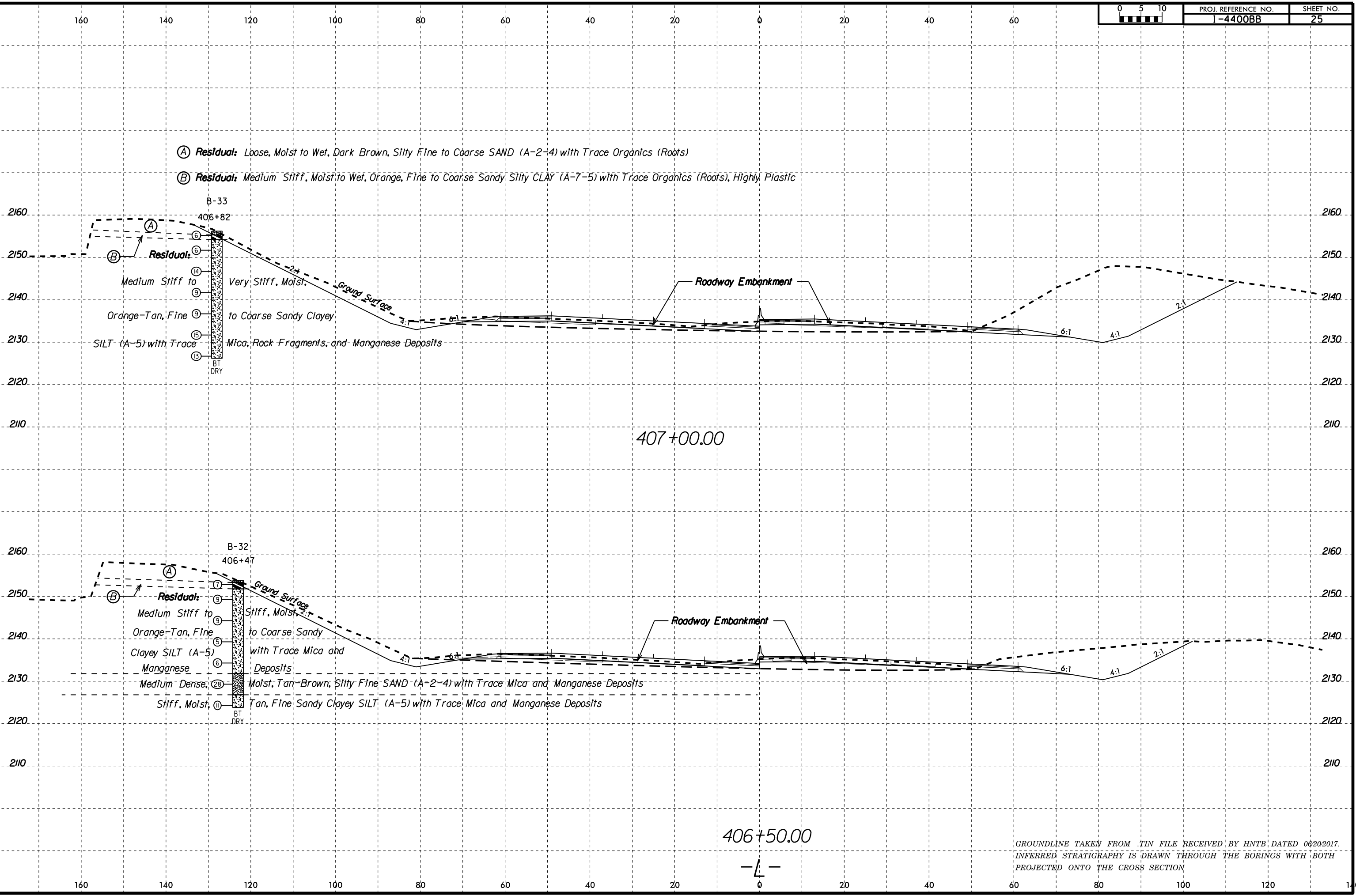
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-69	130' LT	407+25	0.7 - 1.5	A-7-5(24)	60	29	11.3	16.9	14.9	56.9	99.6	93.6	75.1	30.2	-
SS-70	130' LT	407+25	3.5 - 5.0	A-5(0)	39	NP	14.1	35.7	26.5	23.7	100.0	93.5	59.4	21.8	-
SS-72	130' LT	407+25	13.5 - 15.0	A-5(0)	47	NP	3.4	62.6	24.1	9.9	100.0	99.5	49.1	24.2	-



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 Walker-A 66026102

6/23/16



(A) Residual: Loose, Moist to Wet, Dark Brown, Silty Fine to Coarse SAND (A-2-4) with Trace Organics (Roots)

(B) Residual: Medium Stiff, Moist to Wet, Orange, Fine to Coarse Sandy, Silty CLAY (A-7-5) with Trace Organics (Roots), Highly Plastic

B-33
 406+82

Residual: Medium Stiff to Very Stiff, Moist, Orange-Tan, Fine to Coarse Sandy Clayey SILT (A-5) with Trace Mica, Rock Fragments, and Manganese Deposits

BT DRY

407+00.00

B-32
 406+47

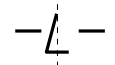
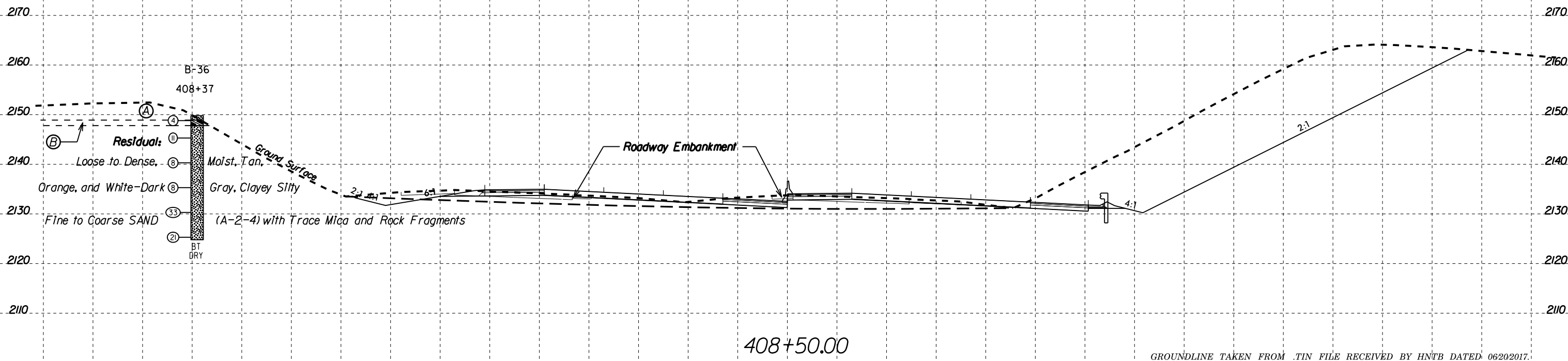
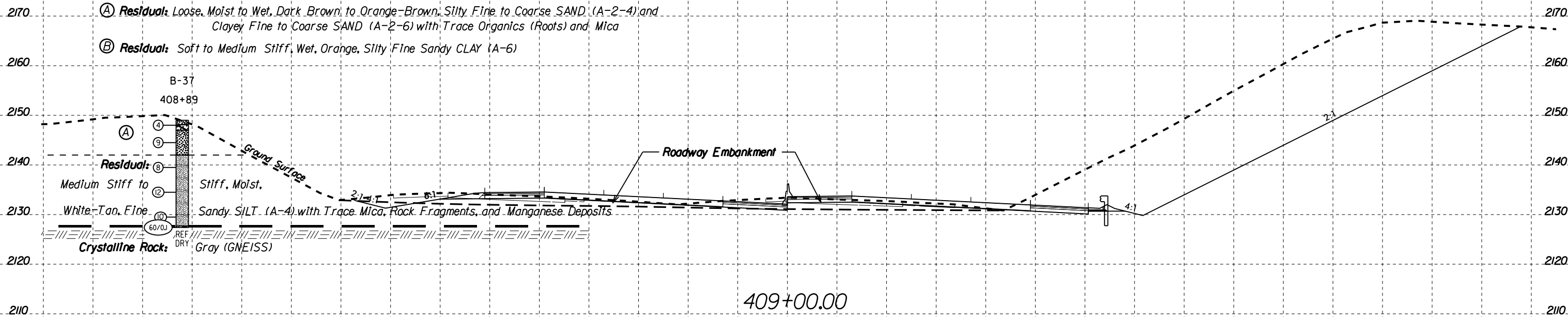
Residual: Medium Stiff to Stiff, Moist, Orange-Tan, Fine to Coarse Sandy Clayey SILT (A-5) with Trace Mica and Manganese Deposits
 Medium Dense, Moist, Tan-Brown, Silty Fine SAND (A-2-4) with Trace Mica and Manganese Deposits
 Stiff, Moist, Tan, Fine Sandy Clayey SILT (A-5) with Trace Mica and Manganese Deposits

BT DRY

406+50.00

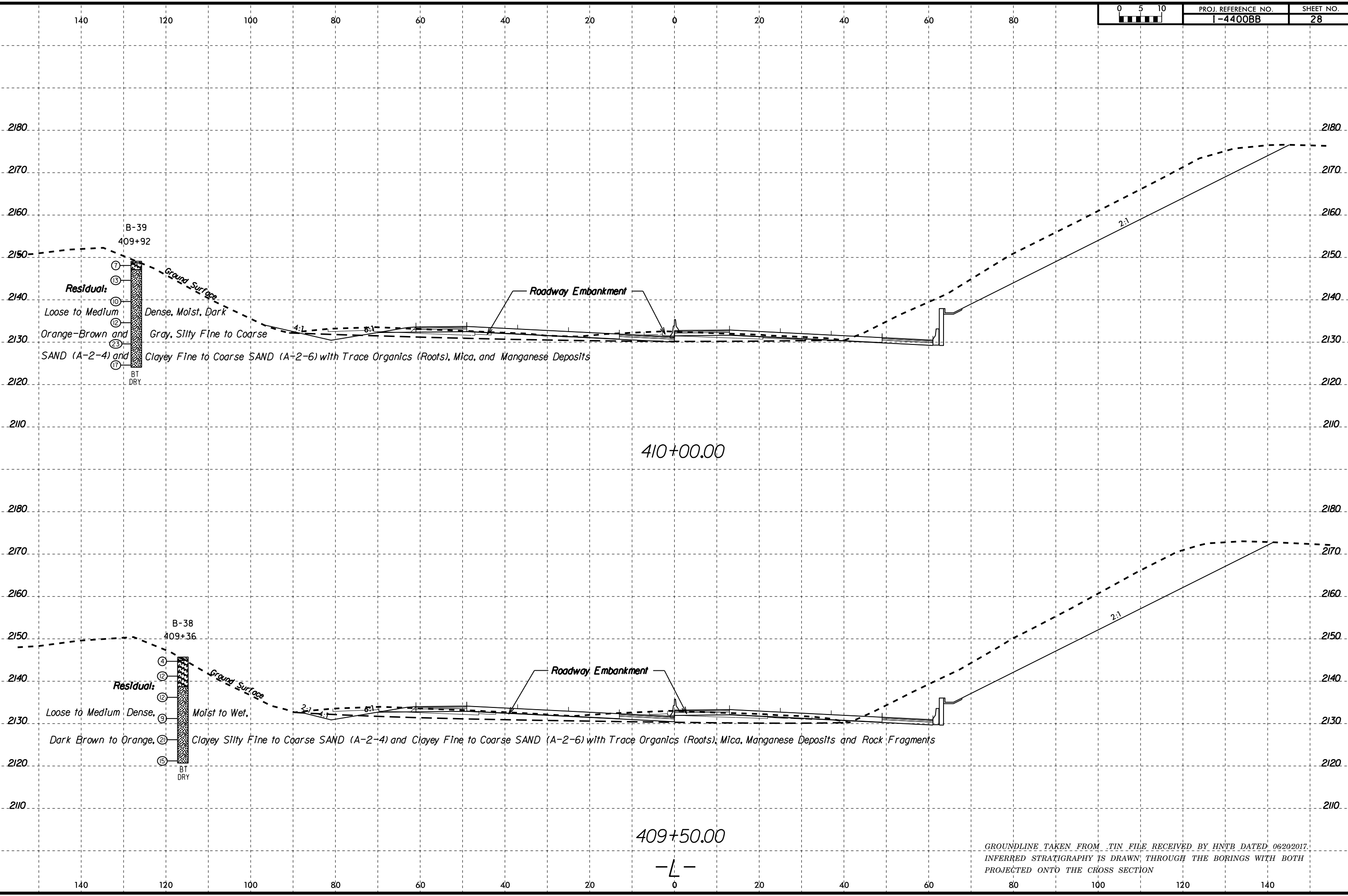
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B-39
409+92

Residual:
Loose to Medium Dense, Moist, Dark Orange-Brown and Gray, Silty Fine to Coarse SAND (A-2-4) and Clayey Fine to Coarse SAND (A-2-6) with Trace Organics (Roots), Mica, and Manganese Deposits

BT
DRY

Roadway Embankment

410+00.00

B-38
409+36

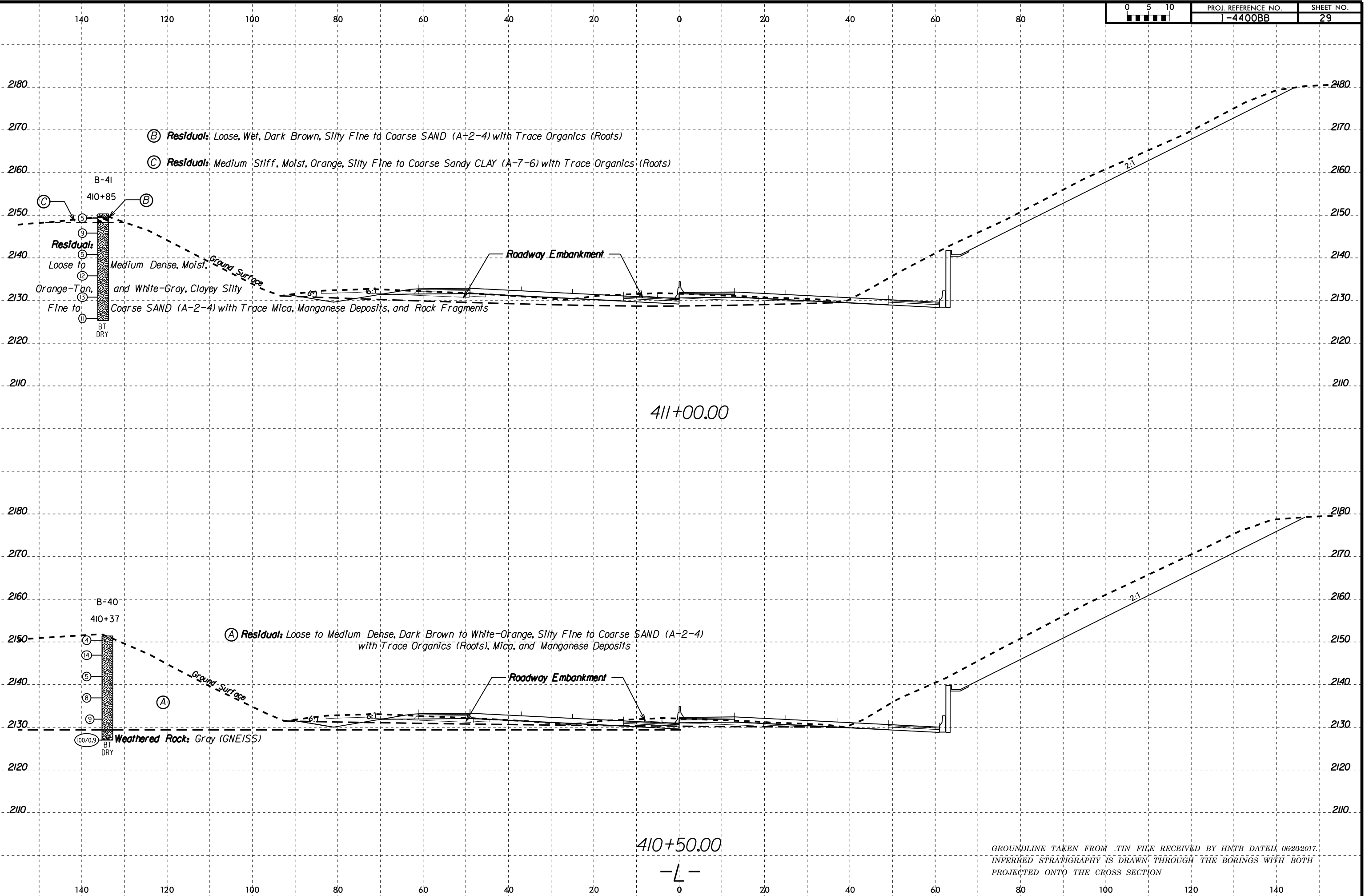
Residual:
Loose to Medium Dense, Moist to Wet, Dark Brown to Orange, Clayey Silty Fine to Coarse SAND (A-2-4) and Clayey Fine to Coarse SAND (A-2-6) with Trace Organics (Roots), Mica, Manganese Deposits and Rock Fragments

BT
DRY

Roadway Embankment

409+50.00

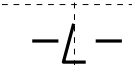
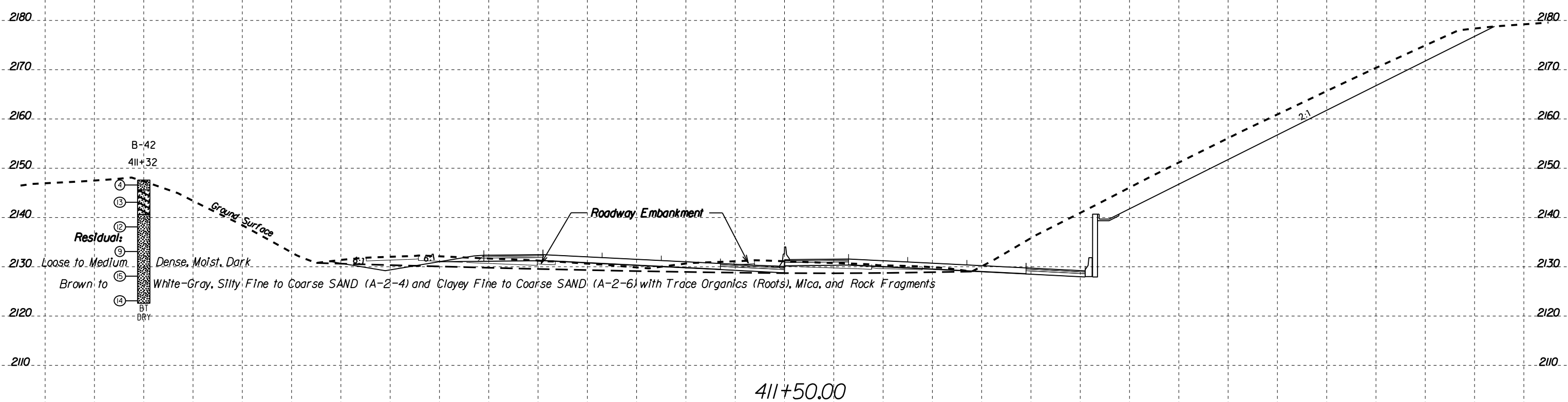
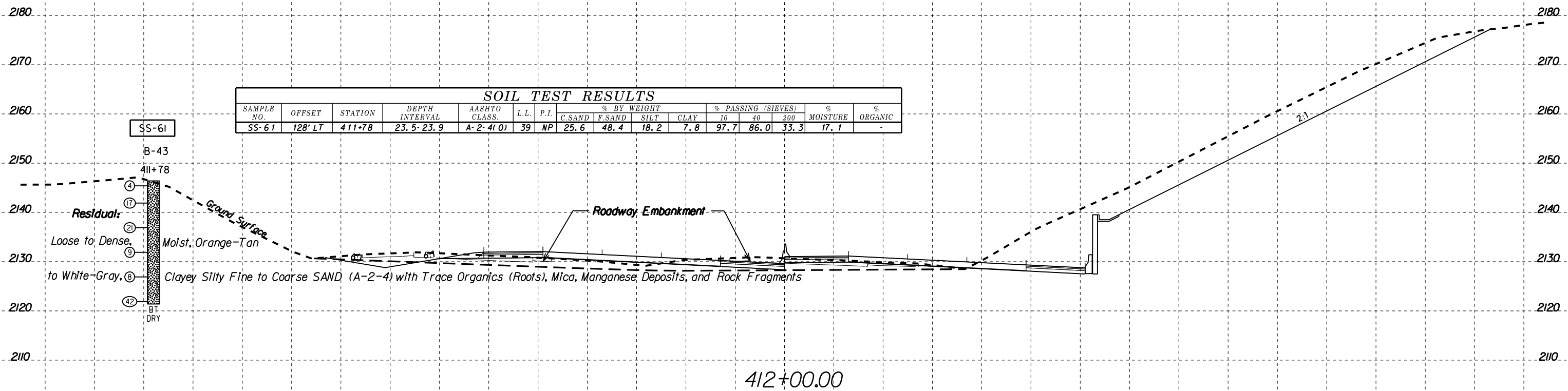
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INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION



25-JUN-2018 15:02 F:\Projects\65166\65166V-0174\NCDDOT-1-4400B Henderson Co.\1-4400B.GEOTECH\SSC\14400B.geo.xsi.L-1.dgn

GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED, 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

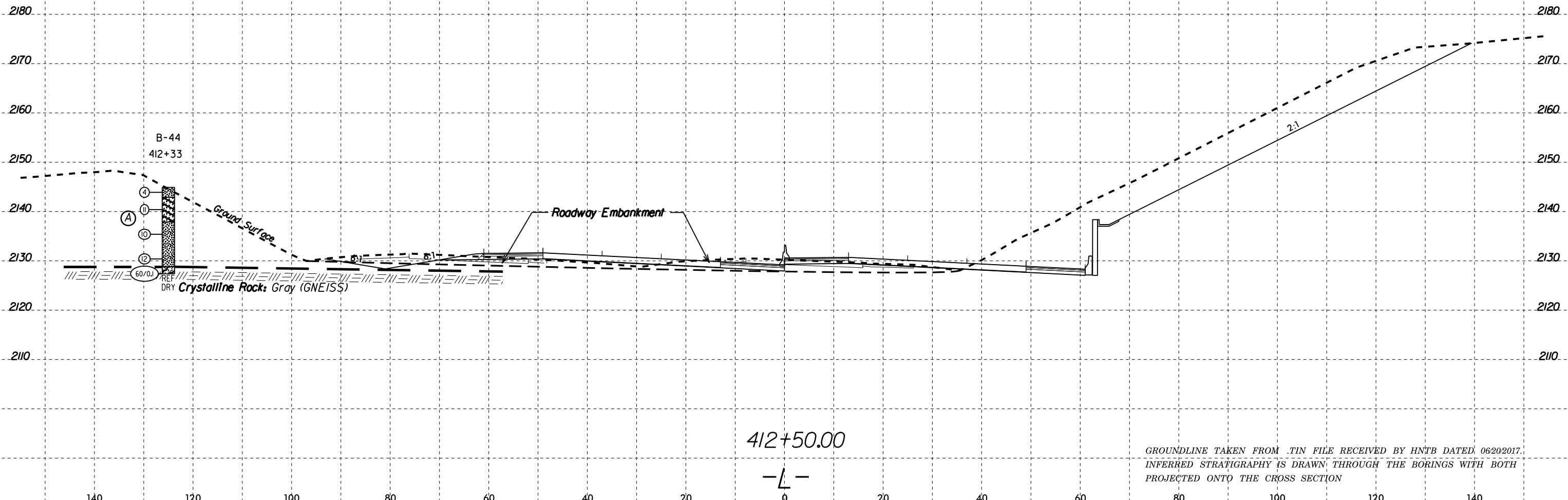
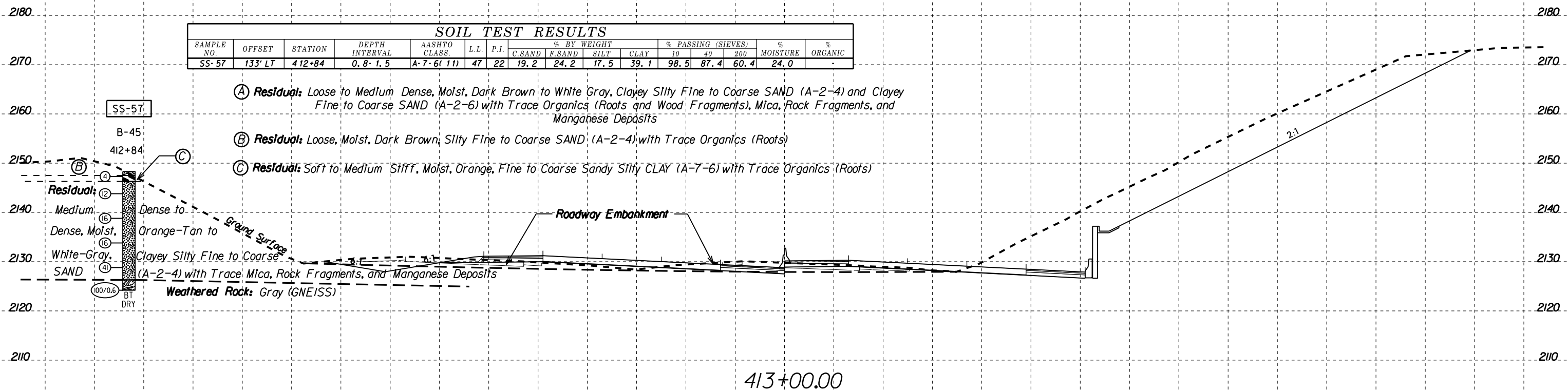
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-61	128' LT	411+78	23.5-23.9	A-2-4(0)	39	NP	25.6	48.4	18.2	7.8	97.7	86.0	33.3	17.1	-



GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

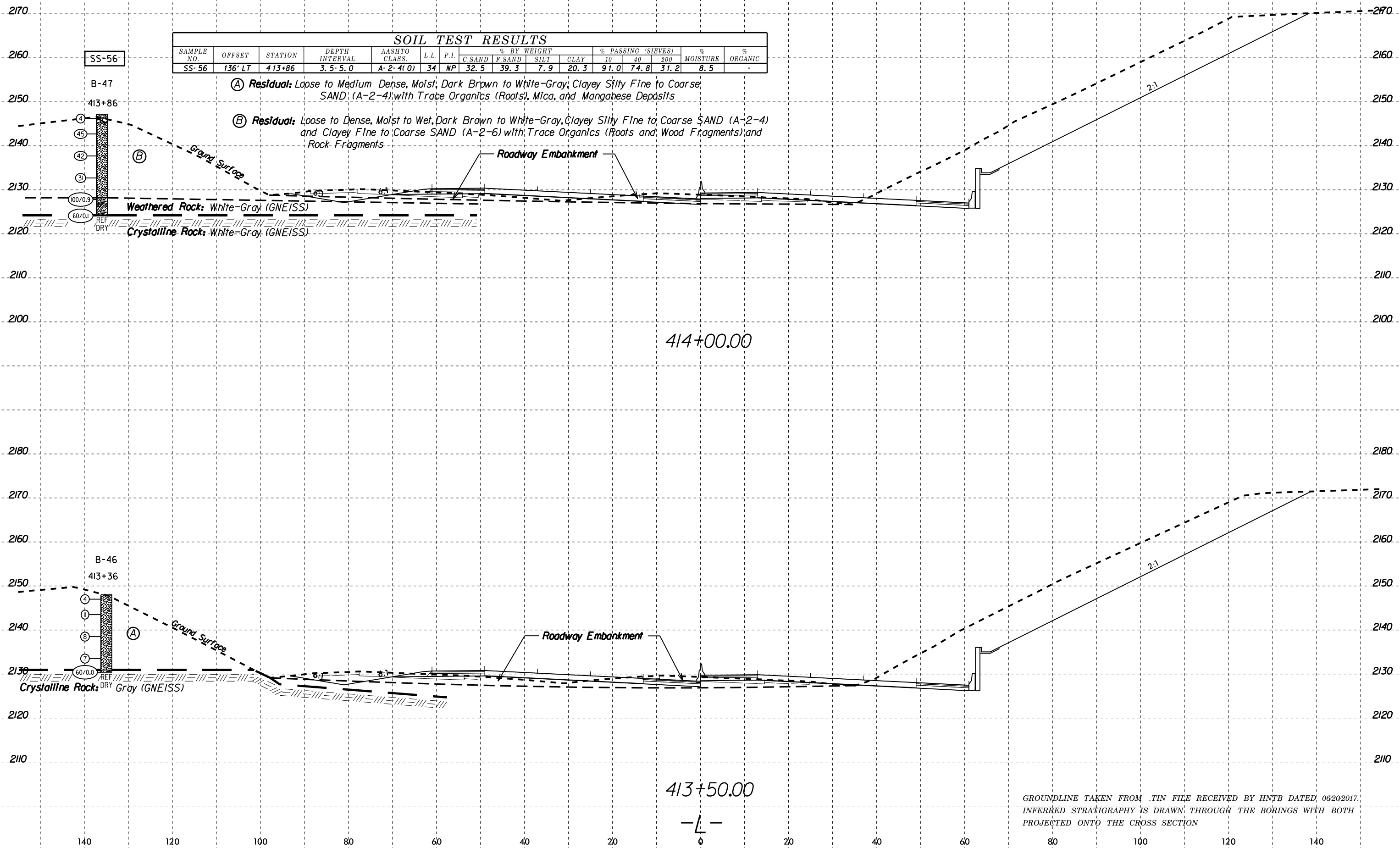
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-57	133' LT	412+84	0.8-1.5	A-7-6(11)	47	22	19.2	24.2	17.5	39.1	98.5	87.4	60.4	24.0	-

- (A) Residual: Loose to Medium Dense, Moist, Dark Brown to White Gray, Clayey Silty Fine to Coarse SAND (A-2-4) and Clayey Fine to Coarse SAND (A-2-6) with Trace Organics (Roots and Wood Fragments), Mica, Rock Fragments, and Manganese Deposits
- (B) Residual: Loose, Moist, Dark Brown, Silty Fine to Coarse SAND (A-2-4) with Trace Organics (Roots)
- (C) Residual: Soft to Medium Stiff, Moist, Orange, Fine to Coarse Sandy Silty CLAY (A-7-6) with Trace Organics (Roots)



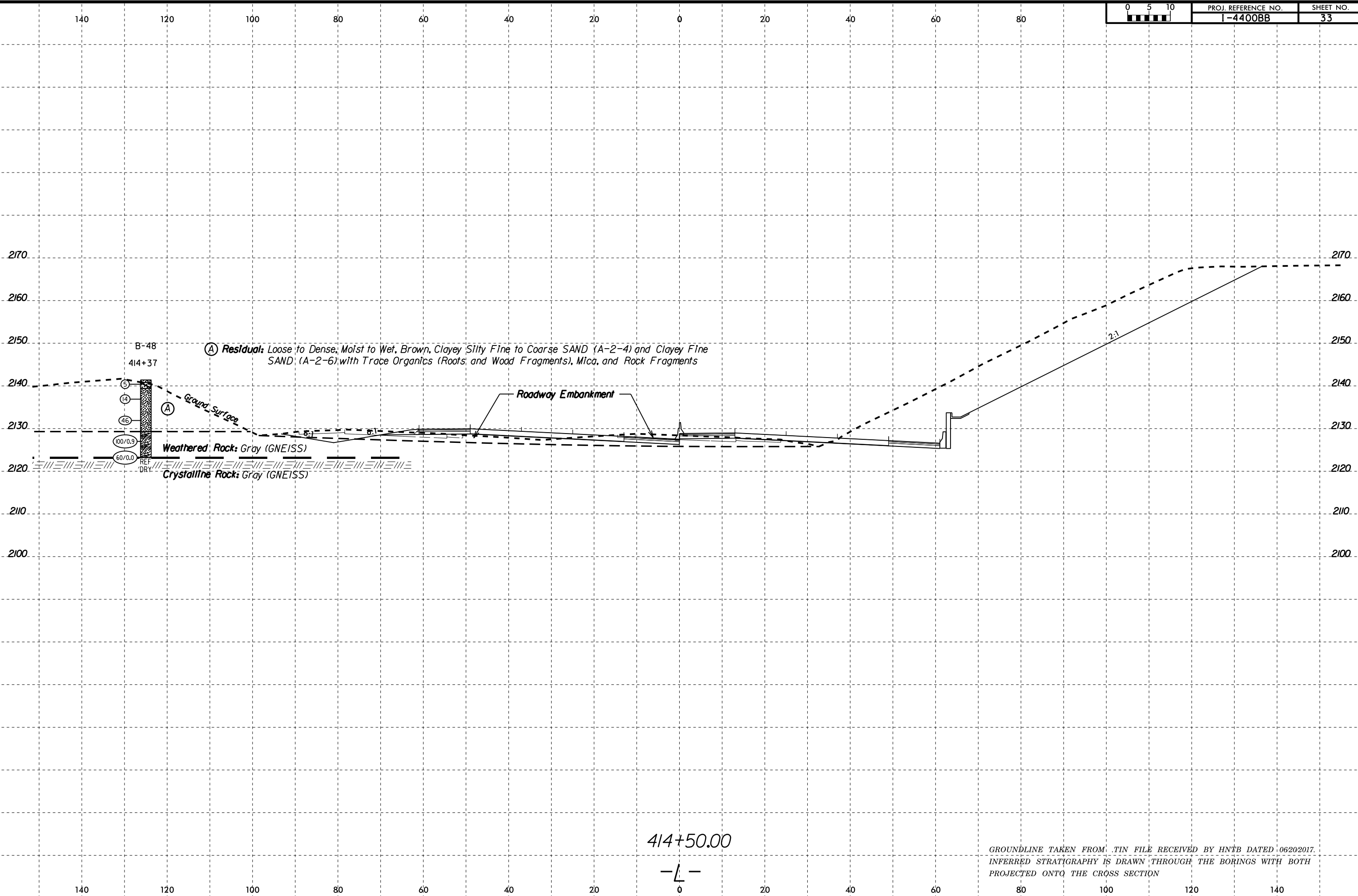
SOIL TEST RESULTS															
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SS-56	136' LT	413+86	3.5-5.0	A-2-4(0)	34	NP	C.SAND	F.SAND	SILT	CLAY	10	40	200	8.5	-
							32.5	39.3	7.9	20.3	91.0	74.8	31.2		

- (A) Residual: Loose to Medium Dense, Moist, Dark Brown to White-Gray, Clayey Silty Fine to Coarse SAND (A-2-4) with Trace Organics (Roots), Mica, and Mangahese Deposits
- (B) Residual: Loose to Dense, Moist to Wet, Dark Brown to White-Gray, Clayey Silty Fine to Coarse SAND (A-2-4) and Clayey Fine to Coarse SAND (A-2-6) with Trace Organics (Roots and Wood Fragments) and Rock Fragments



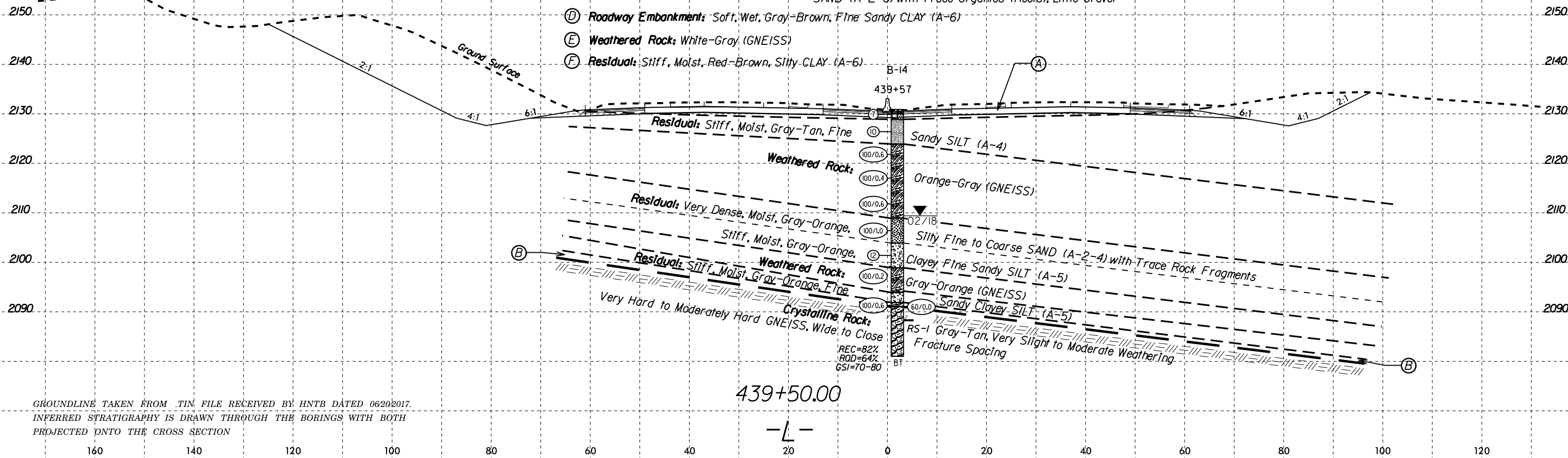
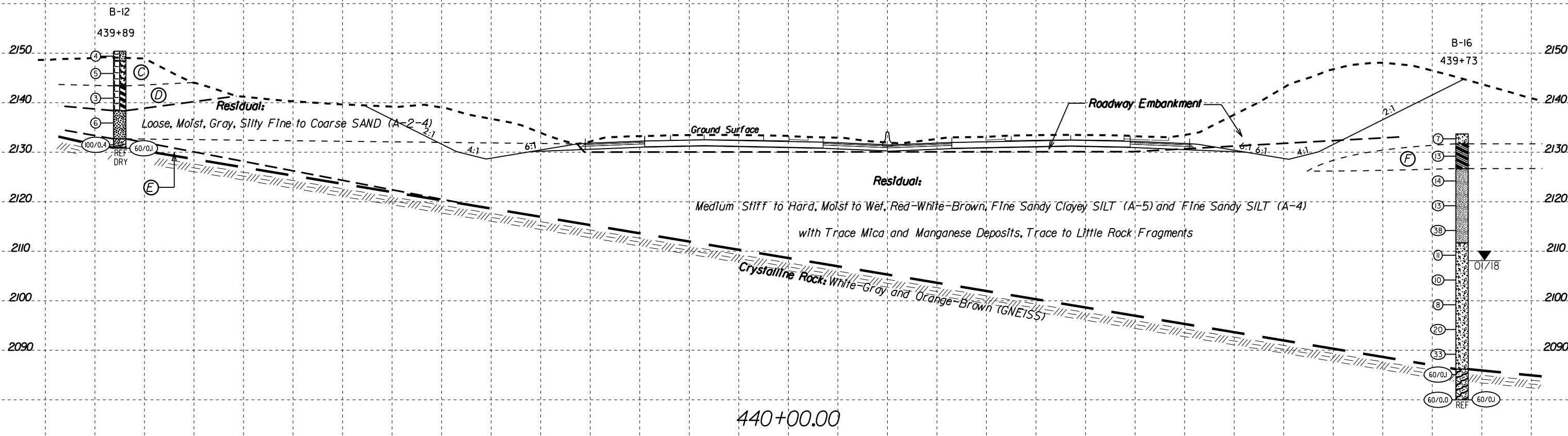
GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

6/23/16



25-JUN-2018 15:04
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Walker A 66026102

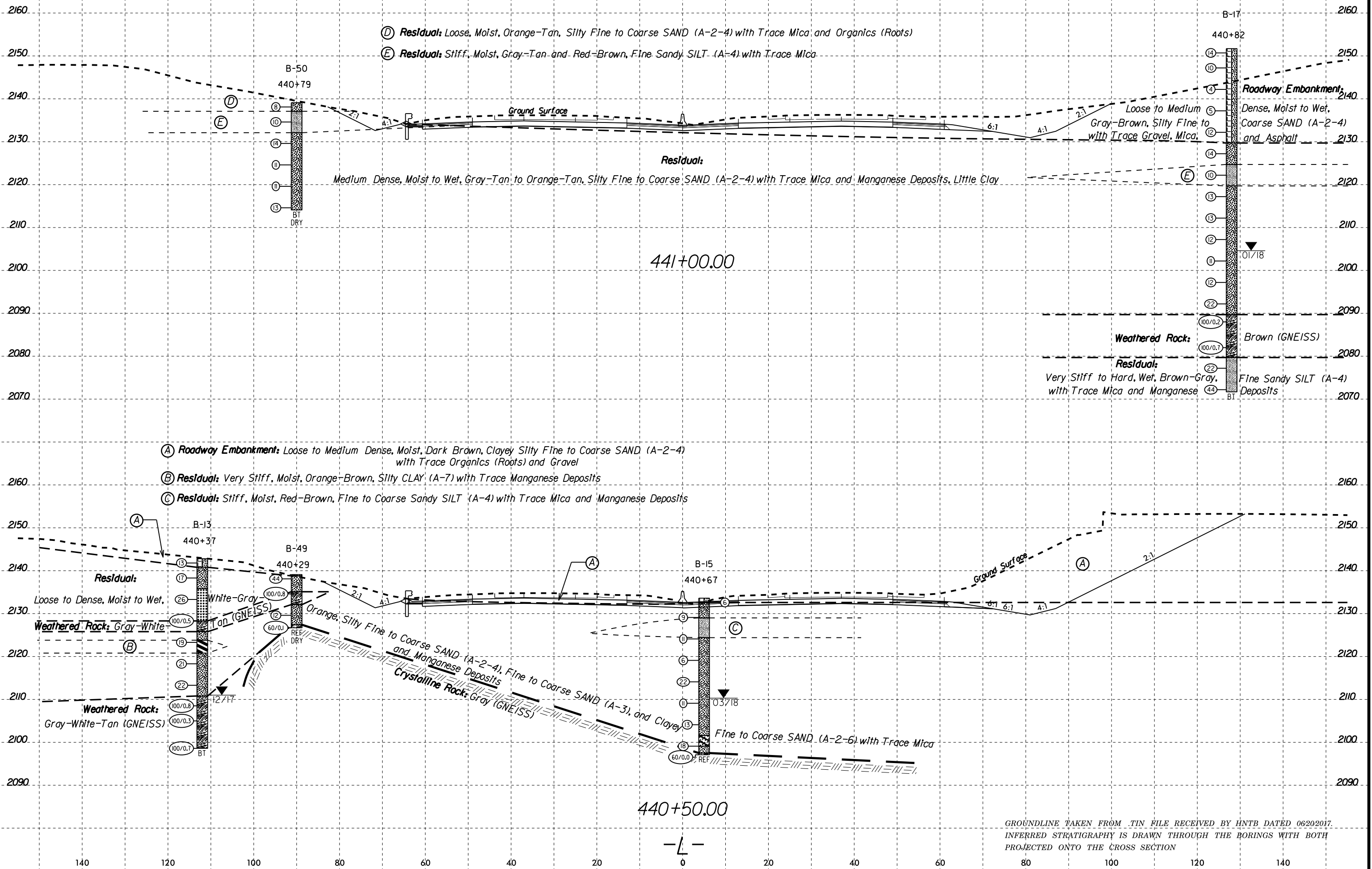
GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06202017.
INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION



- (A) Roadway Embankment: Loose, Moist, Orange-Gray, Clayey Fine SAND (A-2-6) and Silty Fine to Coarse SAND (A-2-4)
- (B) Weathered Rock: Gray-Orange (GNEISS)
- (C) Roadway Embankment: Loose, Wet, Orange-Gray, Clayey Silty Fine to Coarse SAND (A-2-4) and Clayey Fine to Coarse SAND (A-2-6) with Trace Organics (Roots), Little Gravel
- (D) Roadway Embankment: Soft, Wet, Gray-Brown, Fine Sandy CLAY (A-6)
- (E) Weathered Rock: White-Gray (GNEISS)
- (F) Residual: Stiff, Moist, Red-Brown, Silty CLAY (A-6)

GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06202017.
 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
 PROJECTED ONTO THE CROSS SECTION

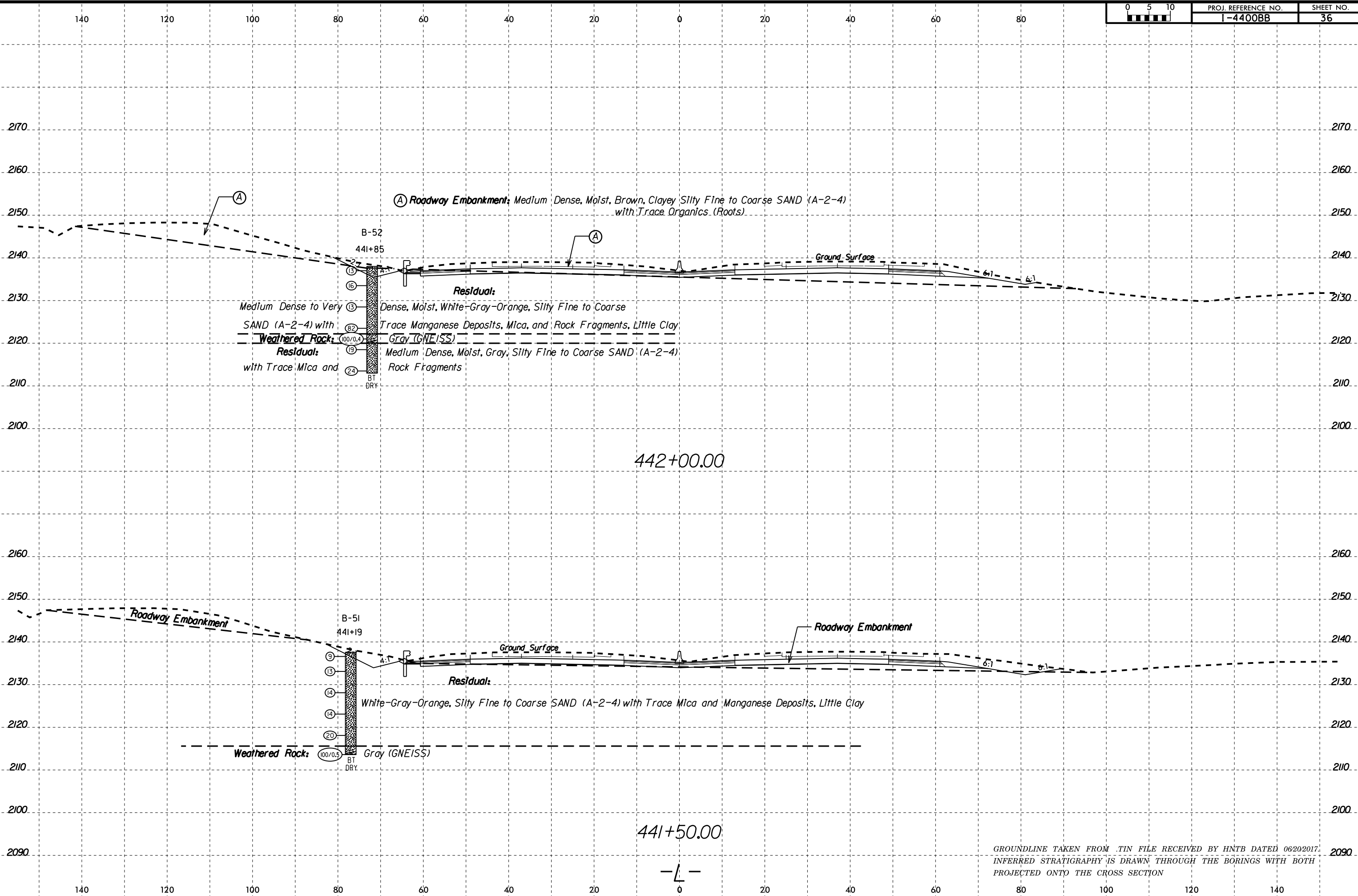
6/23/16



GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
 PROJECTED ONTO THE CROSS SECTION

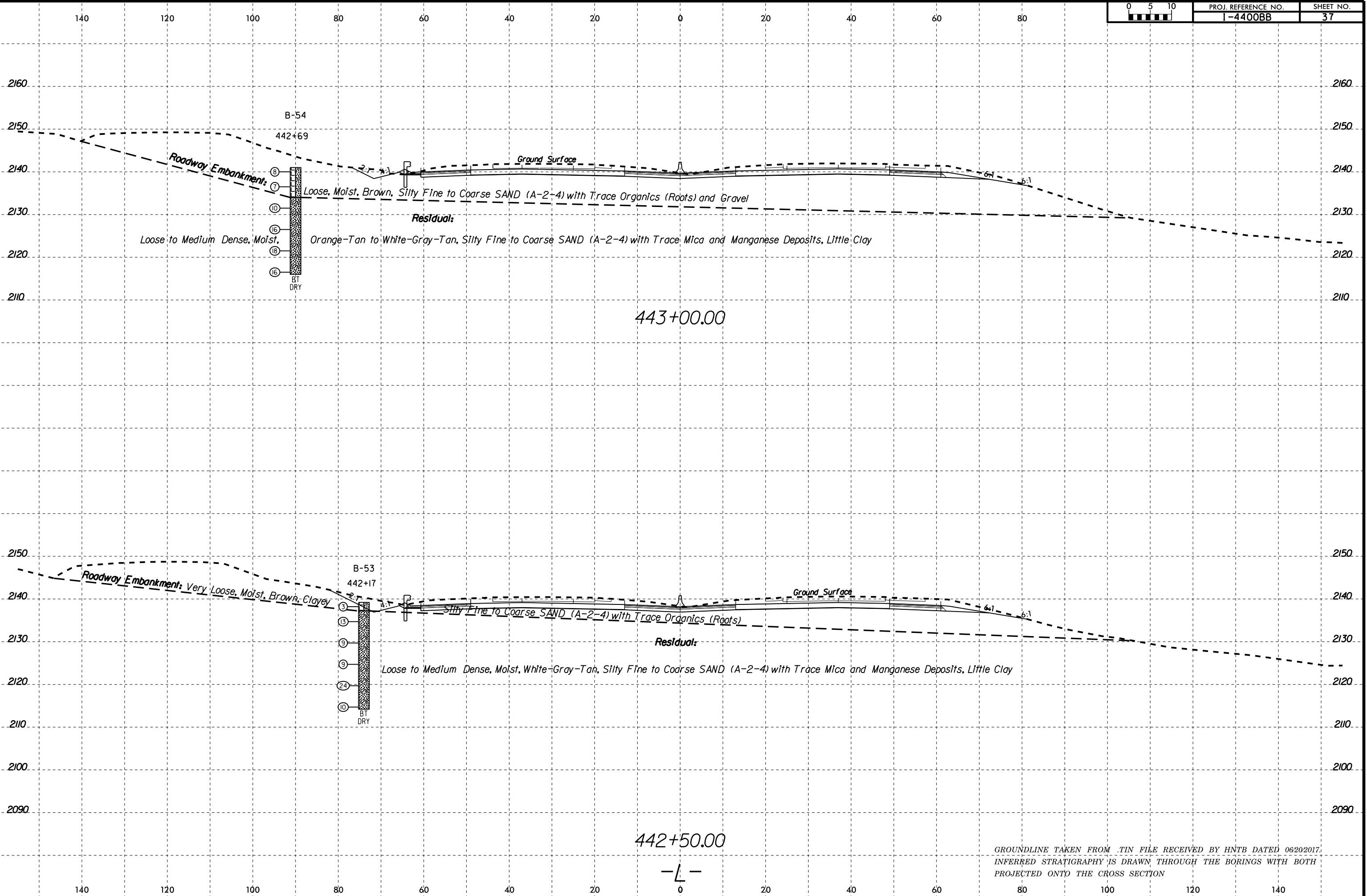
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 Walker

6/23/16



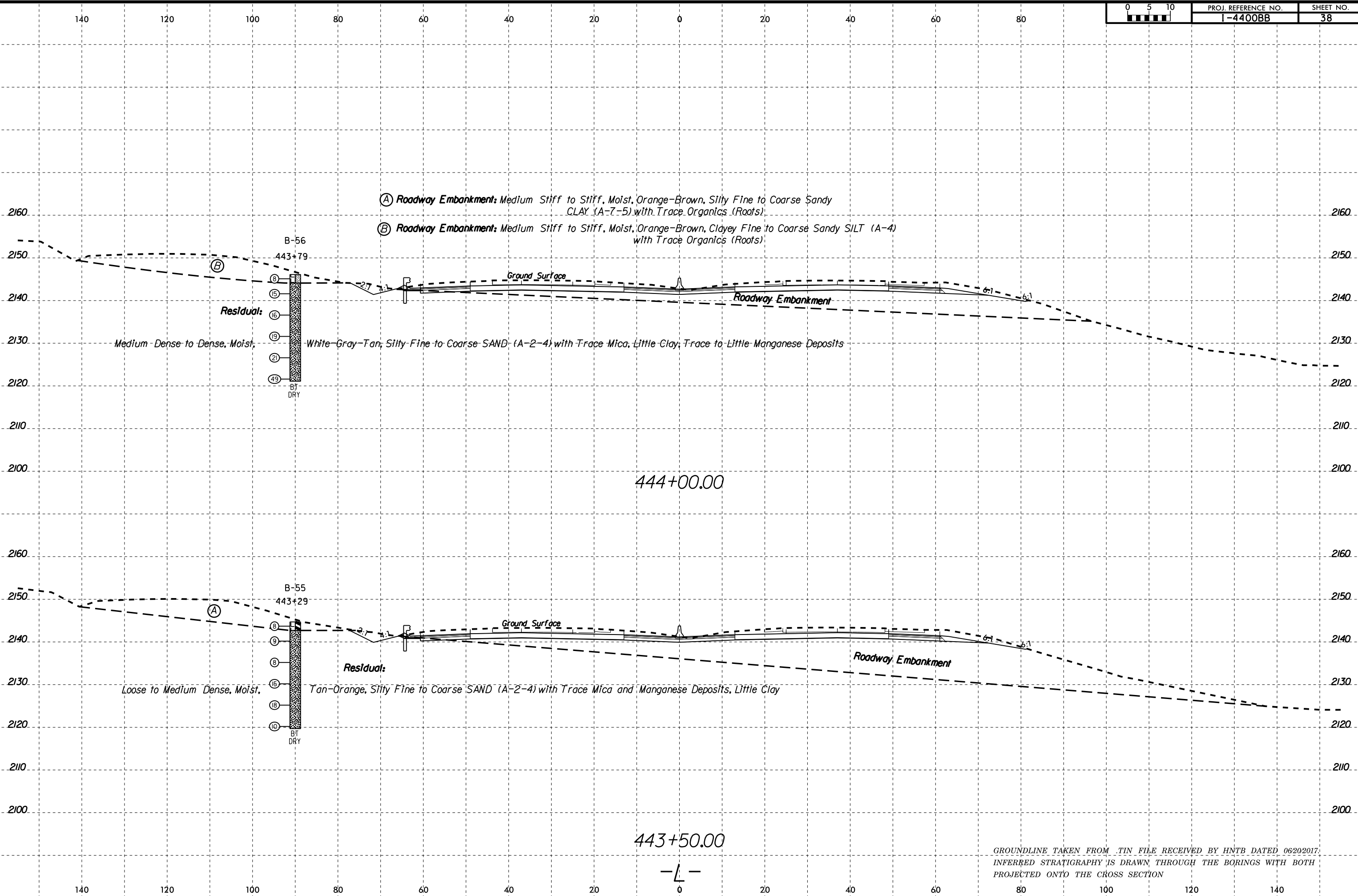
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Walker

GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

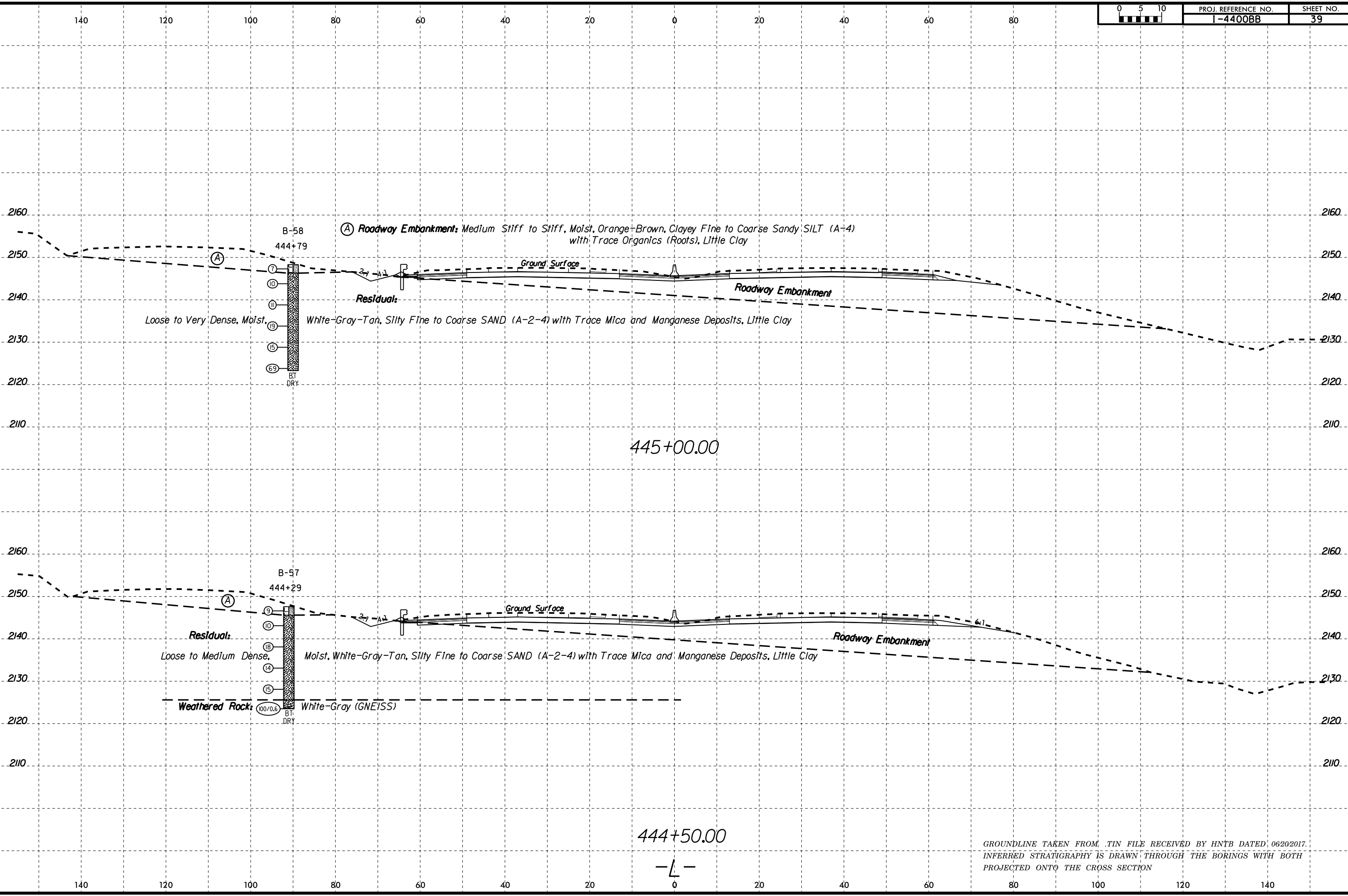


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GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

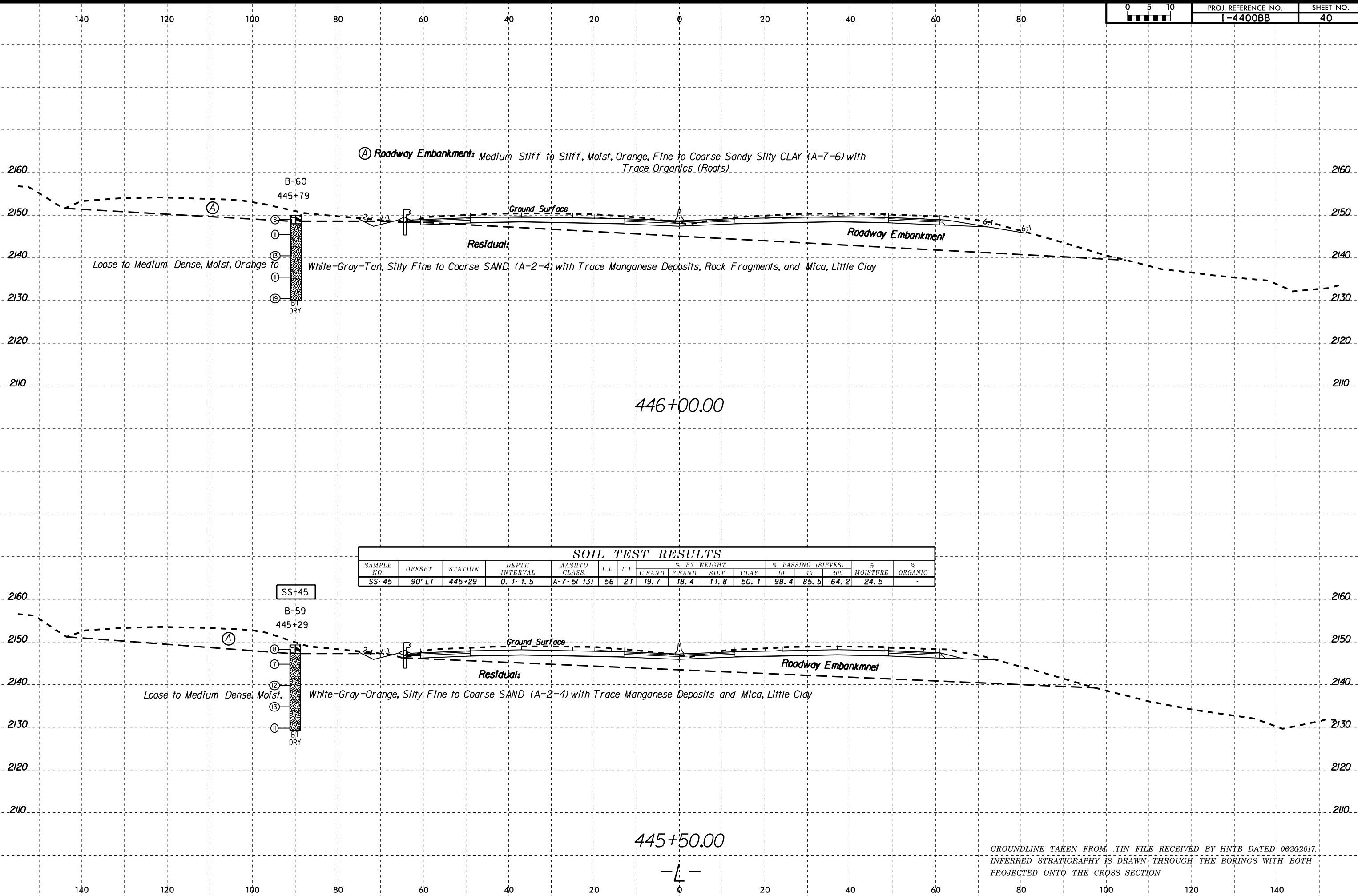


6/23/16



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GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION



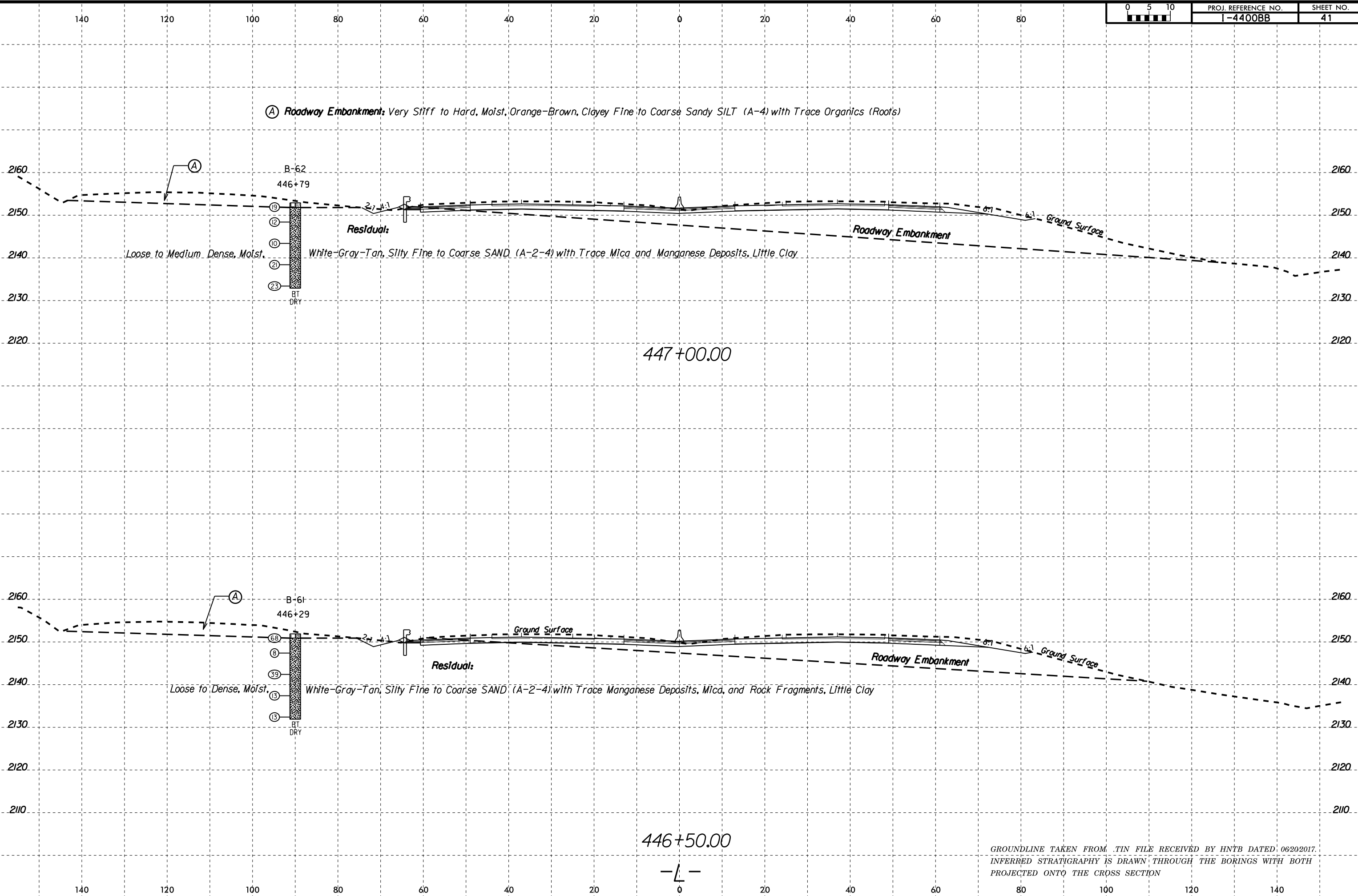
Ⓐ **Roadway Embankment:** Medium Stiff to Stiff, Moist, Orange, Fine to Coarse Sandy Silty CLAY (A-7-6) with Trace Organics (Roots)

Loose to Medium Dense, Moist, Orange to White-Gray-Tan, Silty Fine to Coarse SAND (A-2-4) with Trace Manganese Deposits, Rock Fragments, and Mica, Little Clay

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-45	90' LT	445+29	0.1-1.5	A-7-5(13)	56	21	19.7	18.4	11.8	50.1	98.4	85.5	64.2	24.5	-

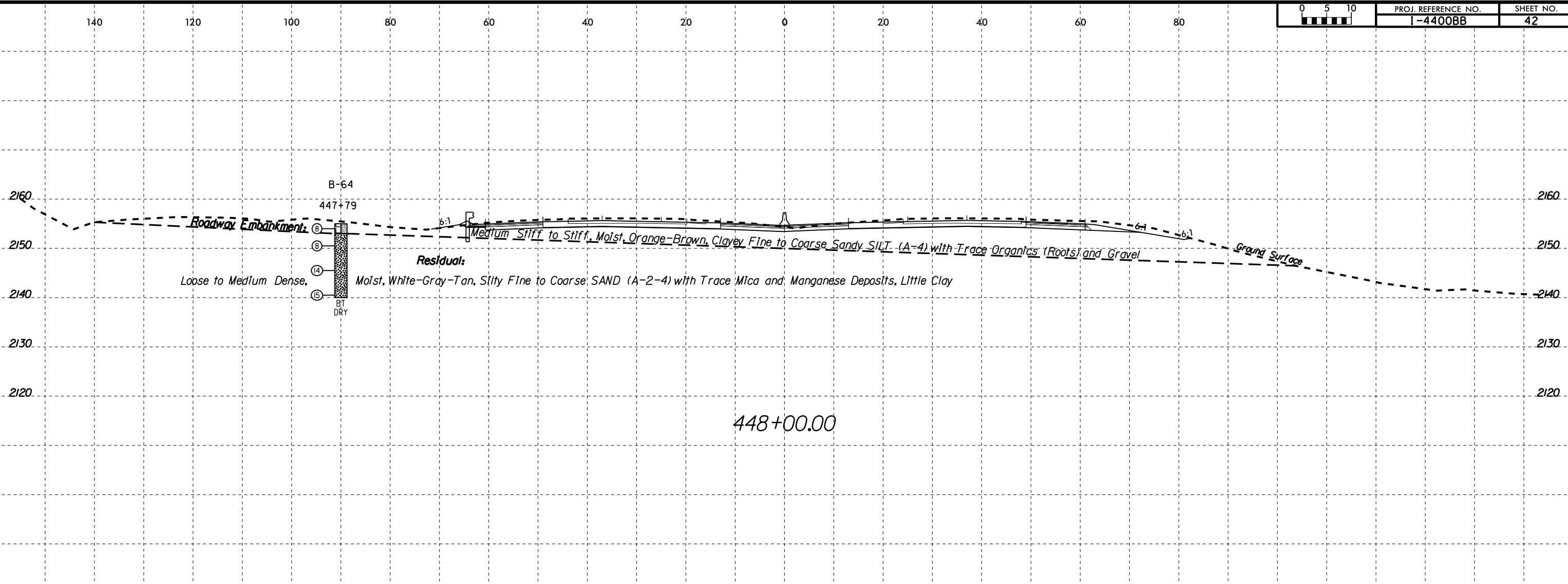
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 Walker-A 66026102

GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
 PROJECTED ONTO THE CROSS SECTION



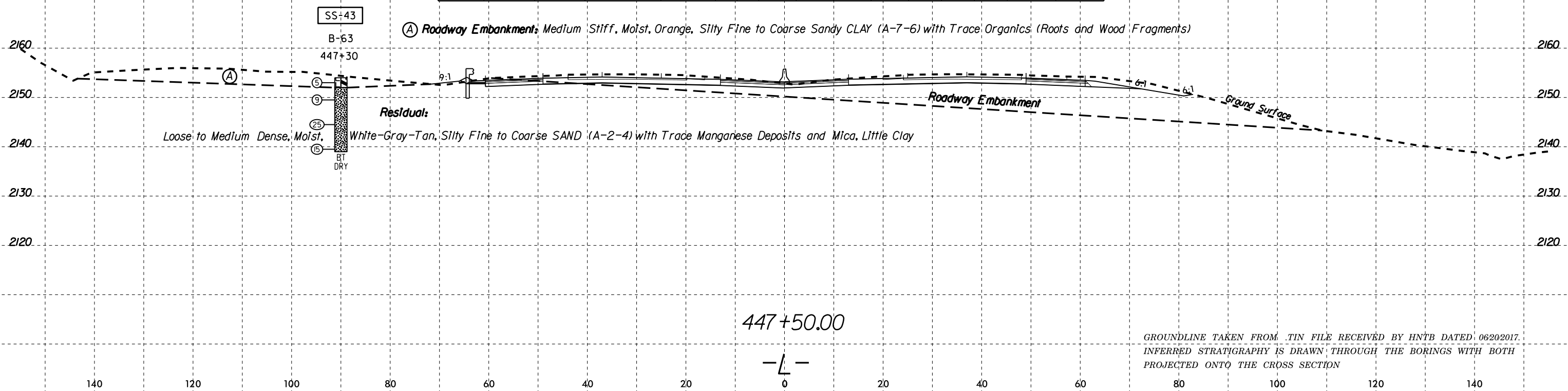
GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

6/23/16



448+00.00

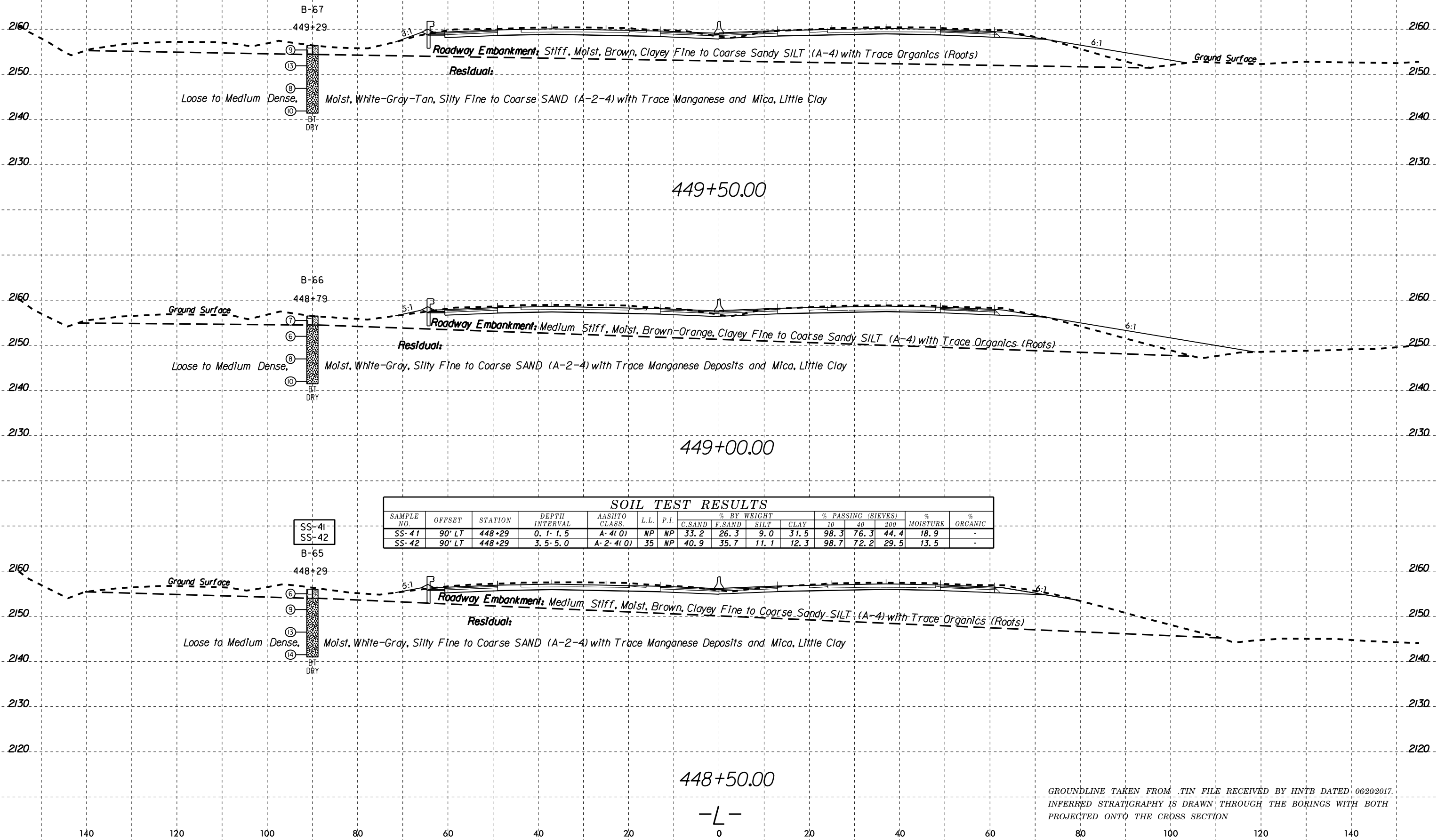
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-43	90' LT	447+30	0.1-1.5	A-7-6(10)	46	20	22.8	23.1	12.1	42.0	99.5	84.8	58.2	23.2	-



447+50.00

GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

25-JUN-2018 15:08 F:\Projects\65166\65166-0174\NCDDOT-1-4400B Henderson Co.\1-4400B.GEOTECH\sec\14400B_geo_xsi.L -1.dgn Walker-A 66026102



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-41	90' LT	448+29	0.1-1.5	A-4(0)	NP	NP	33.2	26.3	9.0	31.5	98.3	76.3	44.4	18.9	-
SS-42	90' LT	448+29	3.5-5.0	A-2-4(0)	35	NP	40.9	35.7	11.1	12.3	98.7	72.2	29.5	13.5	-

25-JUN-2018 15:08 F:\Projects\65166\666V-0174\NCDDOT-1-4400B Henderson Co.\1-4400B.GEO\RDWY\CADD\GEO\TECH\sec\14400B_geo_xsl_1.dgn Walker-A 66026102

GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

6/23/16

140

120

100

80

60

40

20

0

20

40

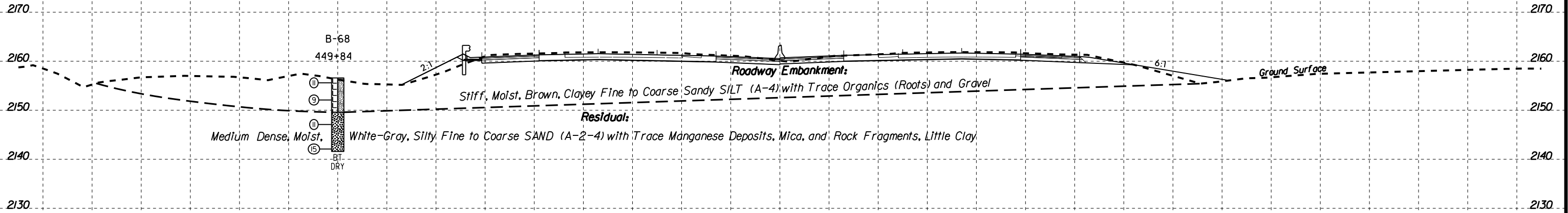
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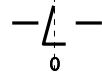


PROJ. REFERENCE NO.	SHEET NO.
I-4400BB	44

SHEET NO.
44



450+00.00



GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
 PROJECTED ONTO THE CROSS SECTION

25-JUN-2018 15:09
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 I:\Walker\A

140

120

100

80

60

40

20

0

20

40

60

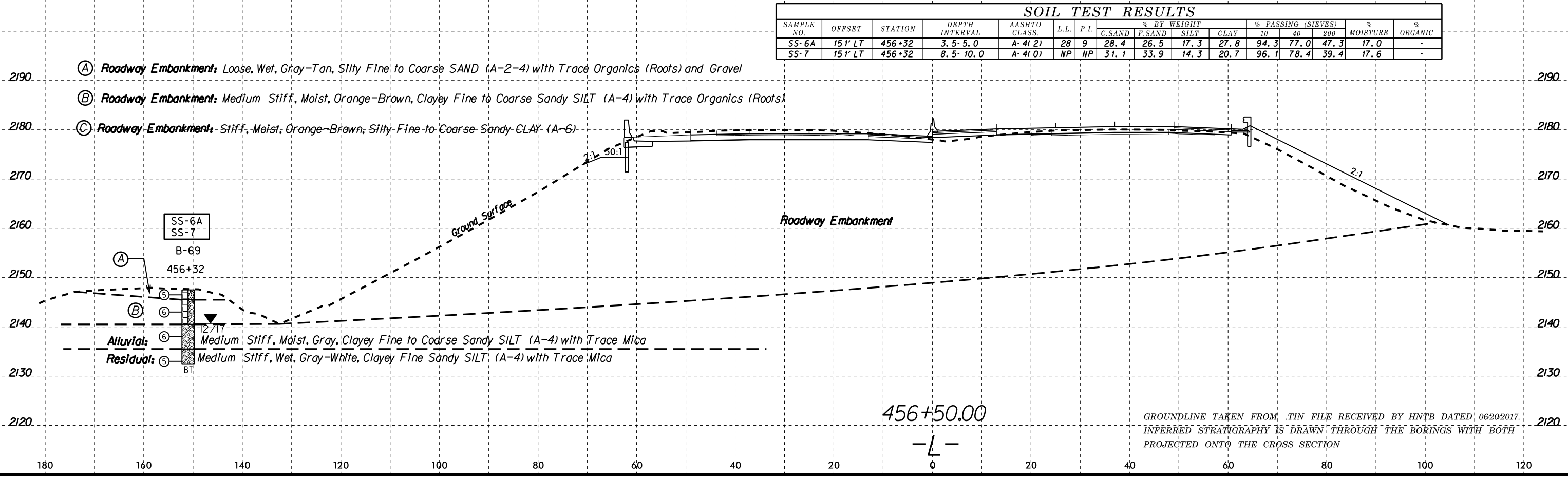
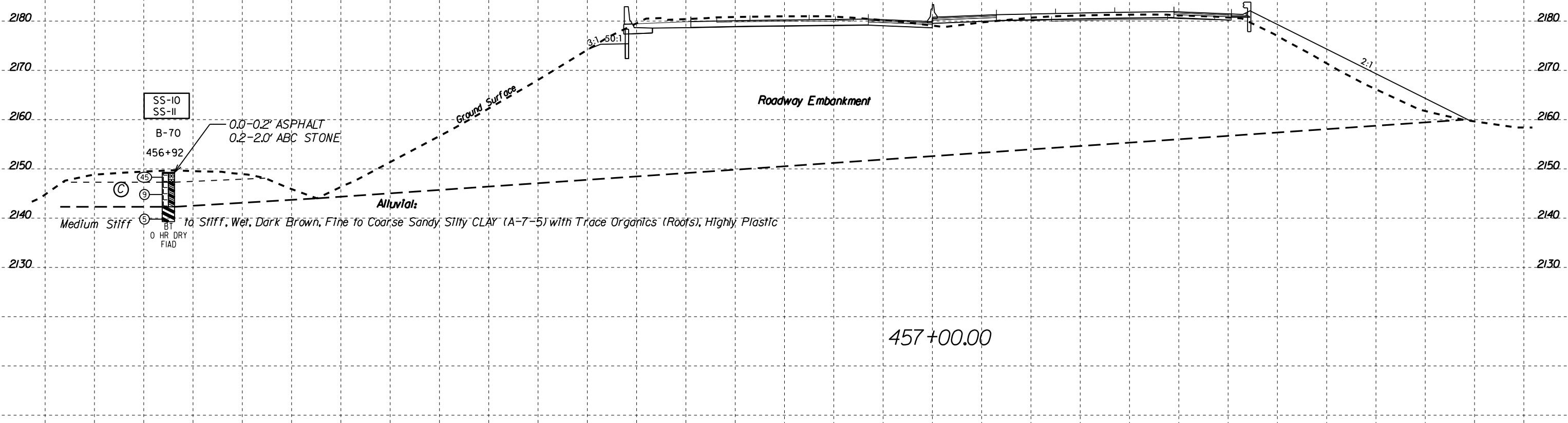
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100

120

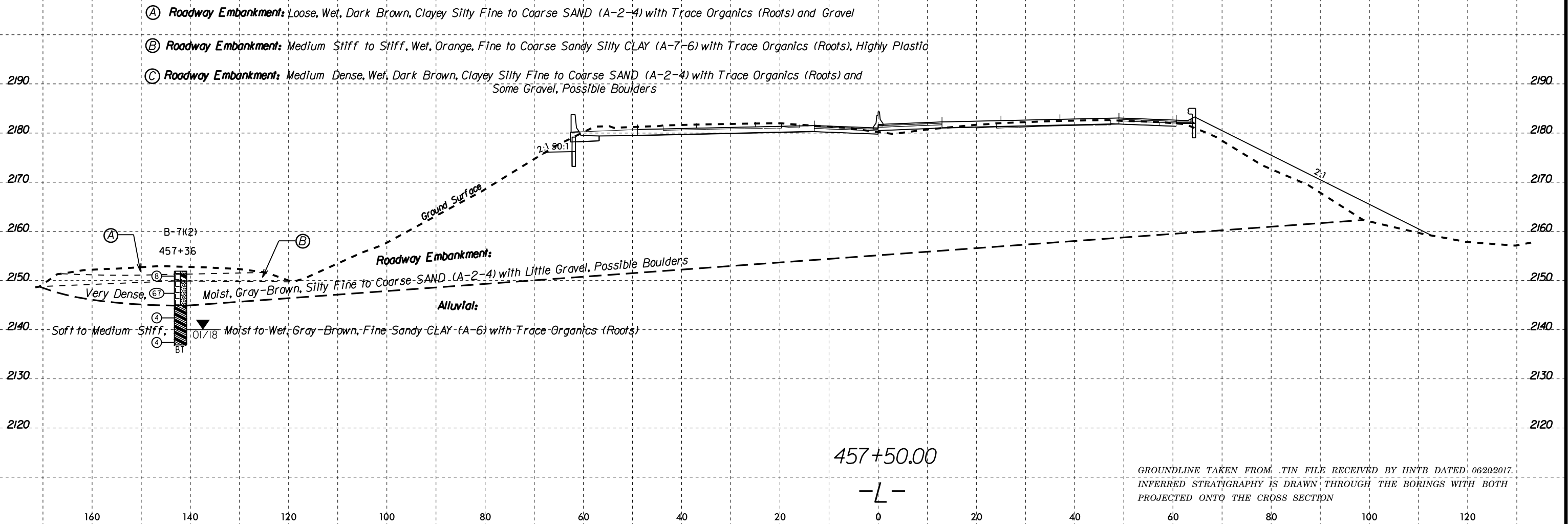
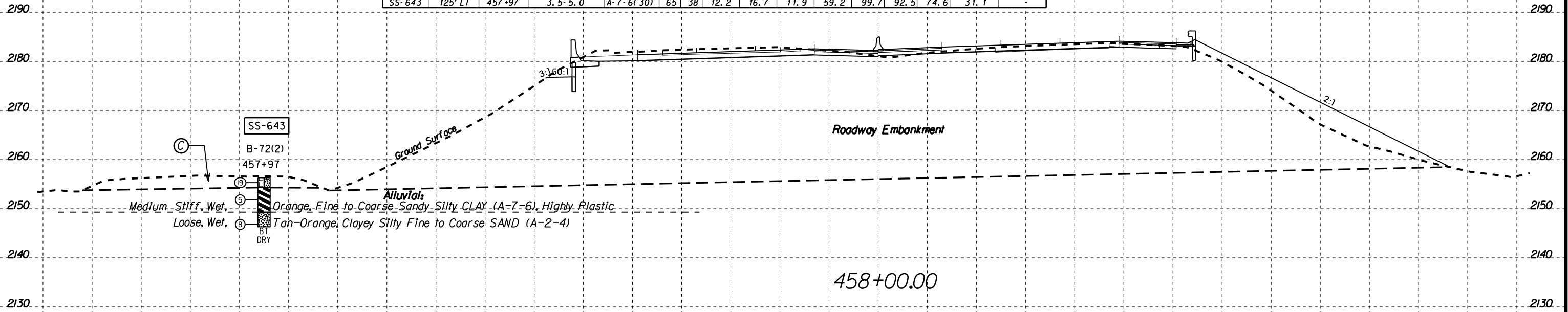
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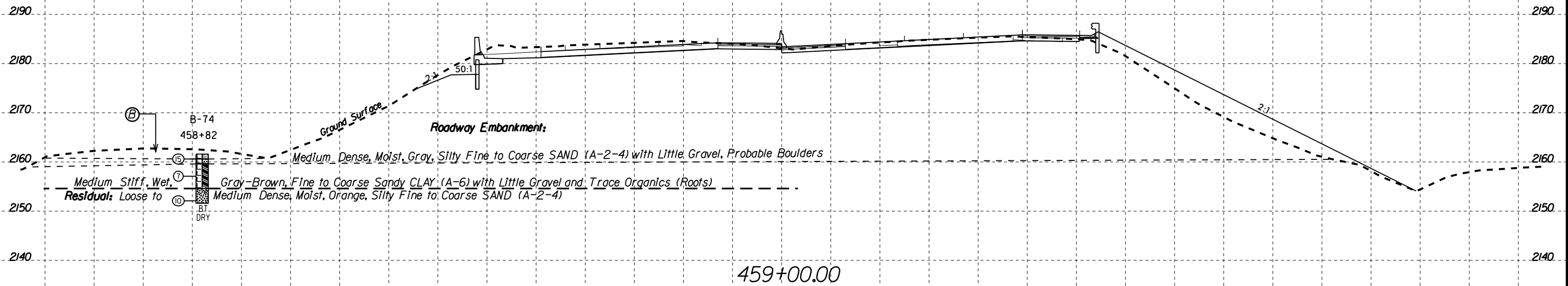
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-10	155' LT	456+92	3.5-5.0	A-6(6)	38	16	24.6	25.5	15.4	34.5	97.8	82.7	53.6	17.7	-
SS-11	155' LT	456+92	8.5-10.0	A-7-5(26)	60	30	9.2	12.7	27.2	50.9	98.9	93.1	80.6	32.0	-



GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

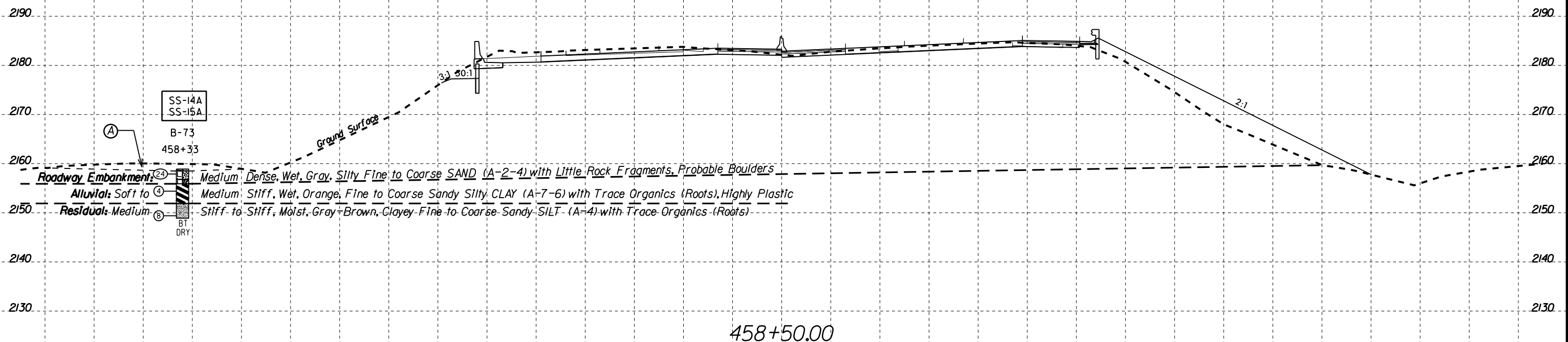
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-643	125' LT	457+97	3.5-5.0	A-7-6(30)	65	38	12.2	16.7	11.9	59.2	99.7	92.5	74.6	31.1	-



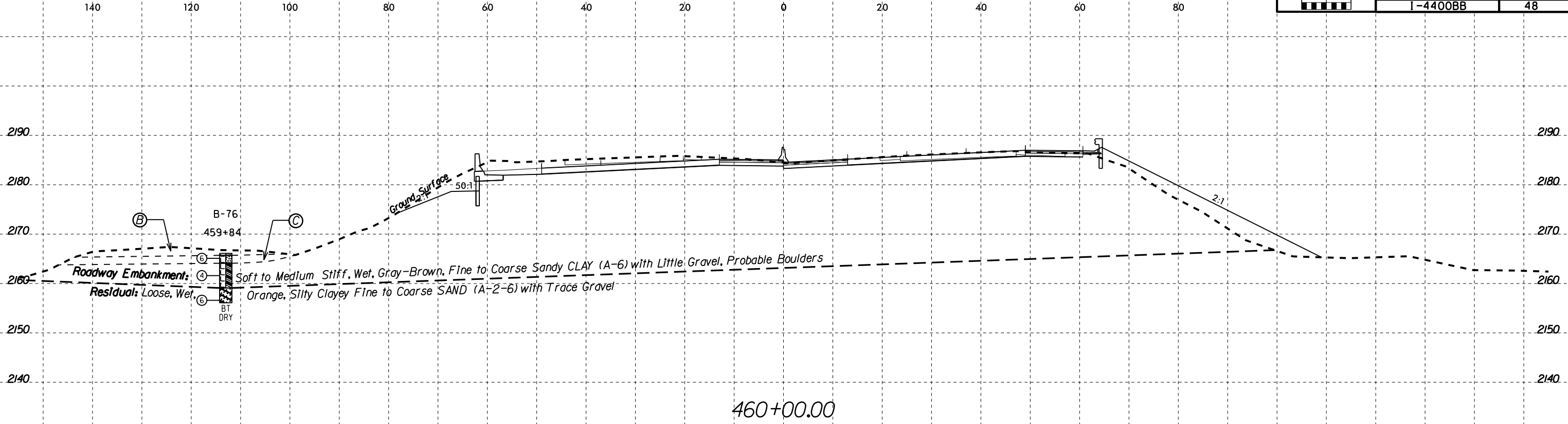


- (A) Roadway Embankment: Very Stiff, Wet, Orange-Brown, Clayey Fine to Coarse Sandy SILT (A-4) with Trace Organics (Roots) and Gravel, Possible Boulders
- (B) Roadway Embankment: Stiff to Very Stiff, Moist, Orange-Brown, Clayey Fine to Coarse Sandy SILT (A-4) with Trace Organics (Roots), Possible Boulders

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-14A	122' LT	458+33	3.5- 5.0	A-7-6(22)	63	35	19.9	17.5	14.6	48.0	98.6	86.0	65.1	27.6	-
SS-15A	122' LT	458+33	8.5- 10.0	A-4(0)	NP	NP	17.2	29.8	23.4	29.6	97.8	87.5	59.6	17.1	-

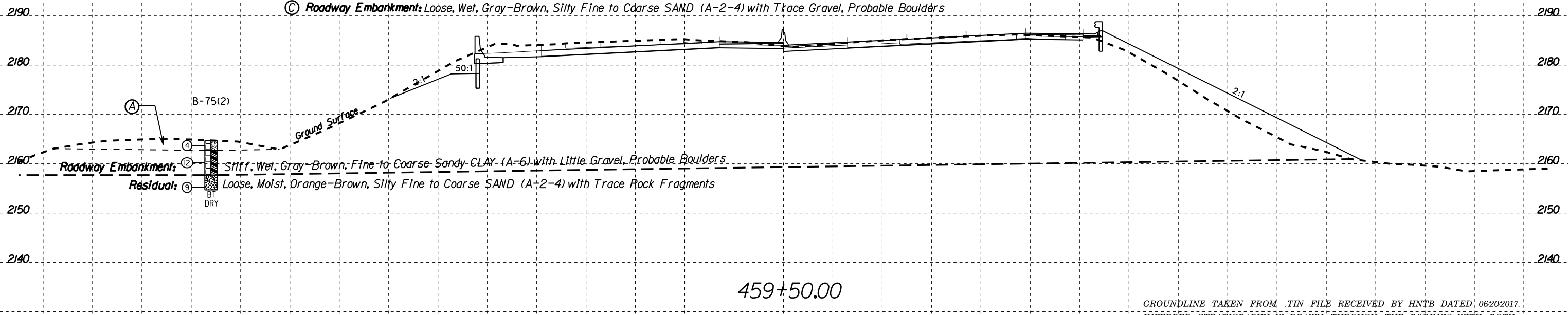


GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
 PROJECTED ONTO THE CROSS SECTION

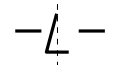


460+00.00

- (A) *Roadway Embankment: Soft to Medium Stiff, Wet, Orange-Dark Brown, Clayey Fine to Coarse Sandy SILT (A-4) with Trace Organics (Roots) and Little Gravel, Possible Boulders*
- (B) *Roadway Embankment: Medium Stiff, Wet, Orange-Brown, Clayey Fine to Coarse Sandy SILT (A-4) with Trace Organics (Roots) and Gravel, Possible Boulders*
- (C) *Roadway Embankment: Loose, Wet, Gray-Brown, Silty Fine to Coarse SAND (A-2-4) with Trace Gravel, Probable Boulders*

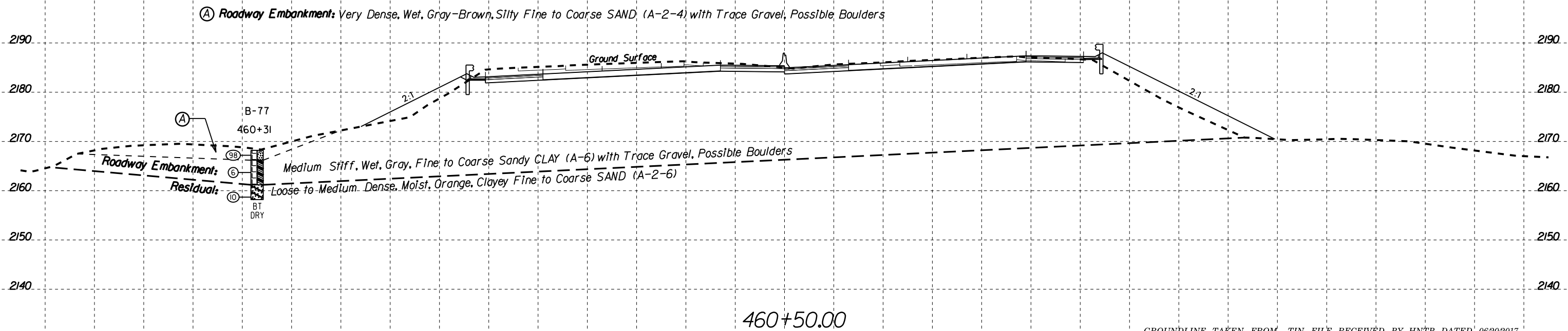
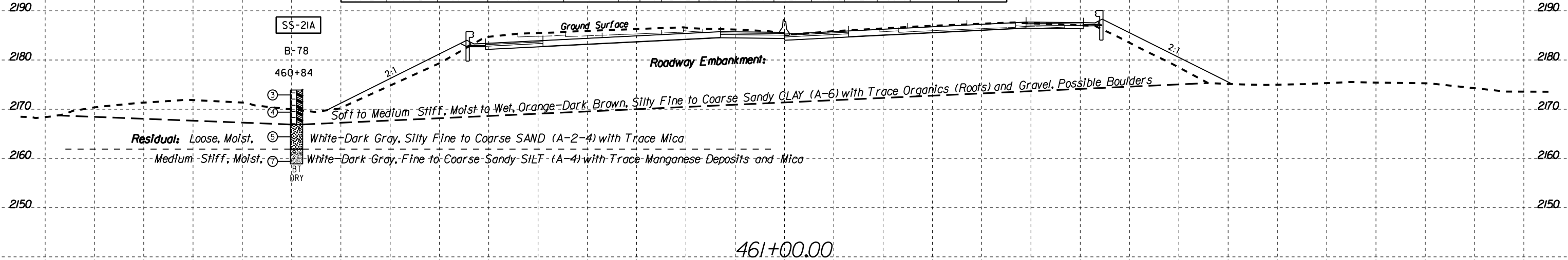


459+50.00



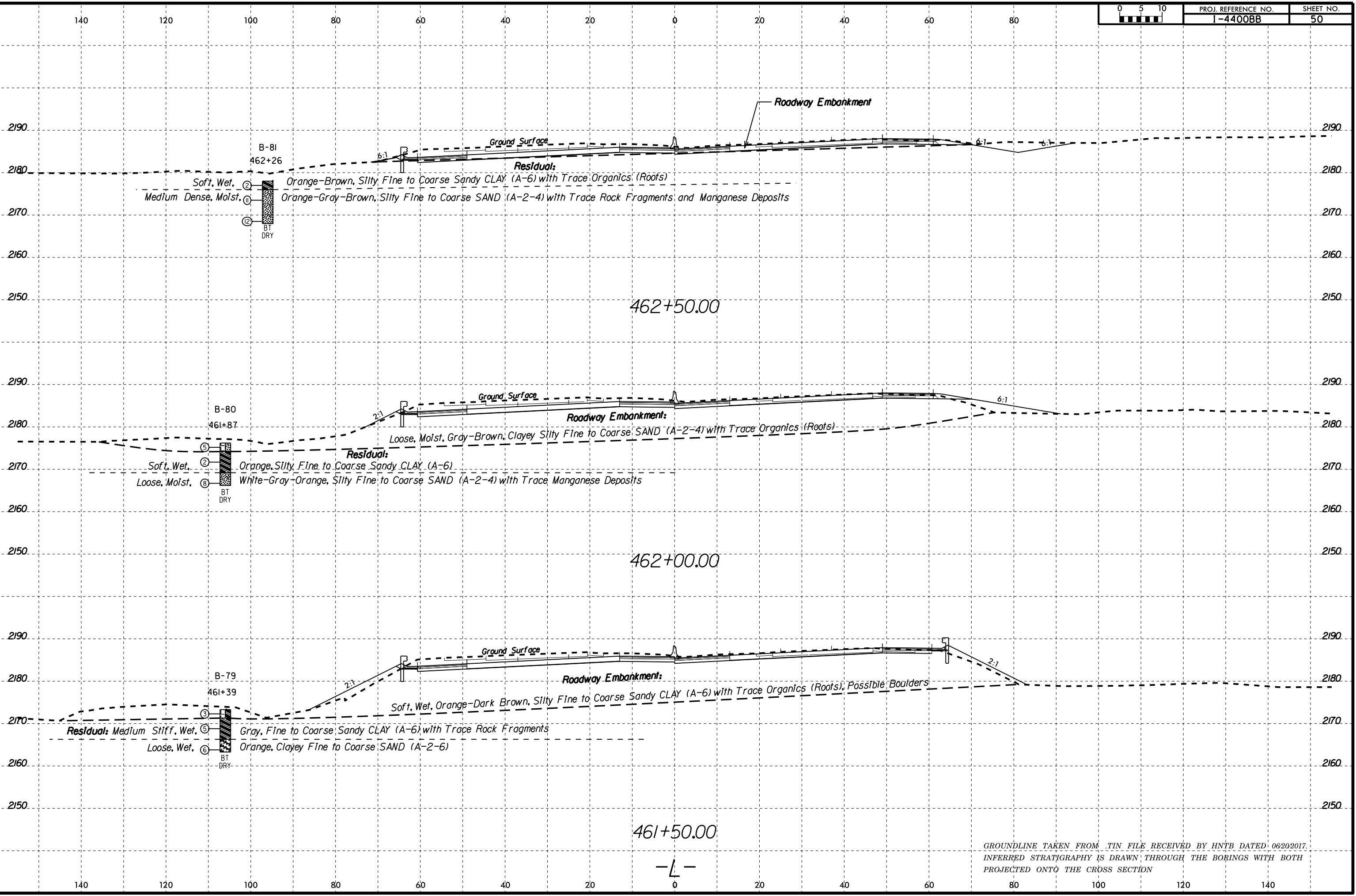
GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06202017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-21A	99' LT	460+84	3.5-5.0	A-6(4)	31	13	24.2	22.8	20.9	32.1	96.9	82.4	55.9	18.1	-



GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
 PROJECTED ONTO THE CROSS SECTION

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 Walker



GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
 PROJECTED ONTO THE CROSS SECTION

140

120

100

80

60

40

20

0

20

40

60

80

2210

2200

2190

2180

2170

2160

2210

2200

2190

2180

2170

2160

(A) Roadway Embankment: Loose to Medium Dense, Moist, Orange-Tan, Clayey, Silty Fine to Coarse SAND (A-2-4) with Trace Organics (Roots) and Gravel

(B) Roadway Embankment: Stiff, Moist, Orange-Brown, Fine to Coarse Sandy CLAY (A-6)

B-82

462+74

(10)

Medium Dense, Moist,

(19)

(27)

BT

DRY

White-Dark Gray, Silty Fine to Coarse SAND (A-2-4) with Trace Manganese Deposits and Rock Fragments

Ground Surface

6:1

4:1

463+00.00

-L-

140

120

100

80

60

40

20

0

20

40

60

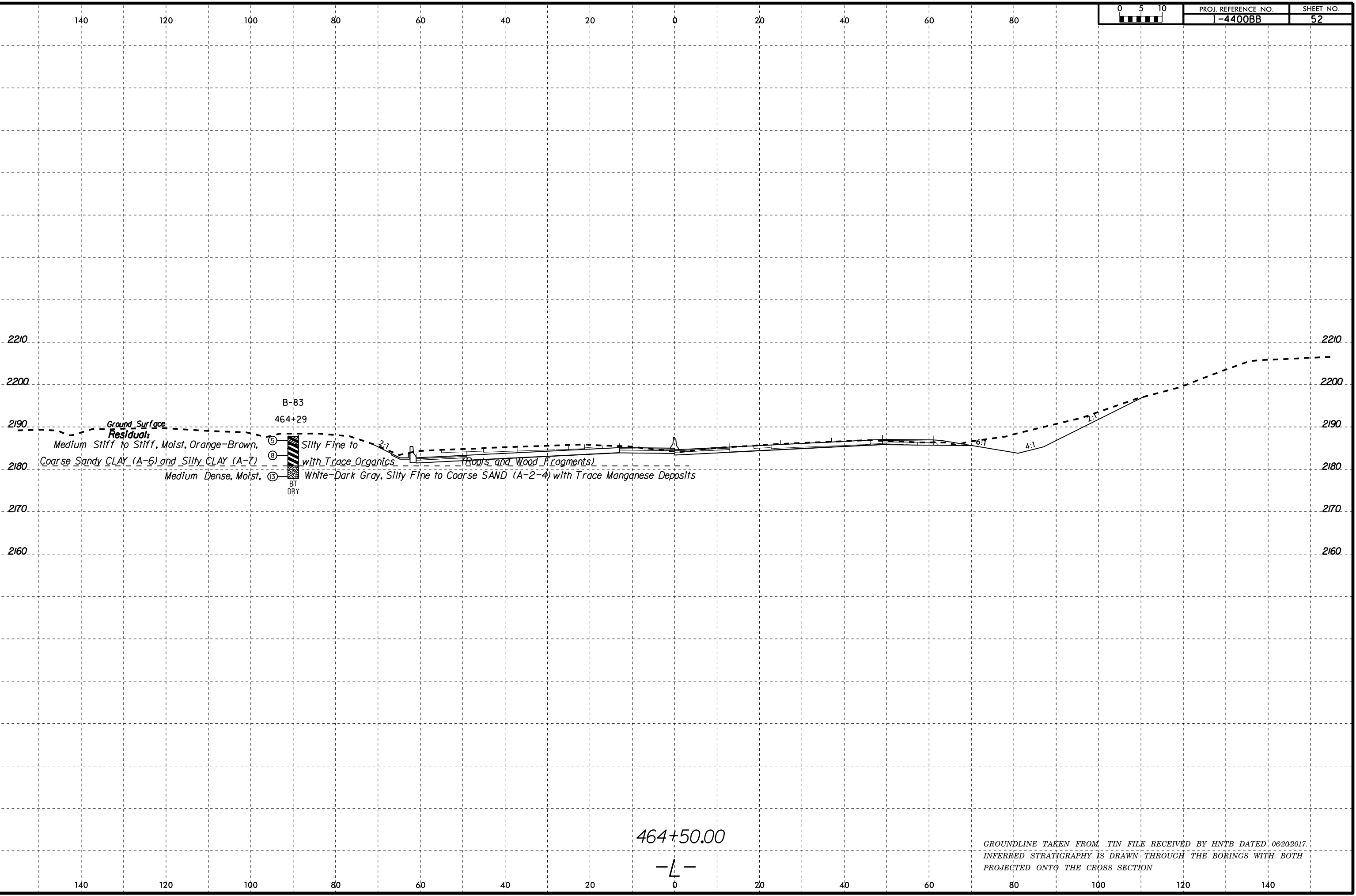
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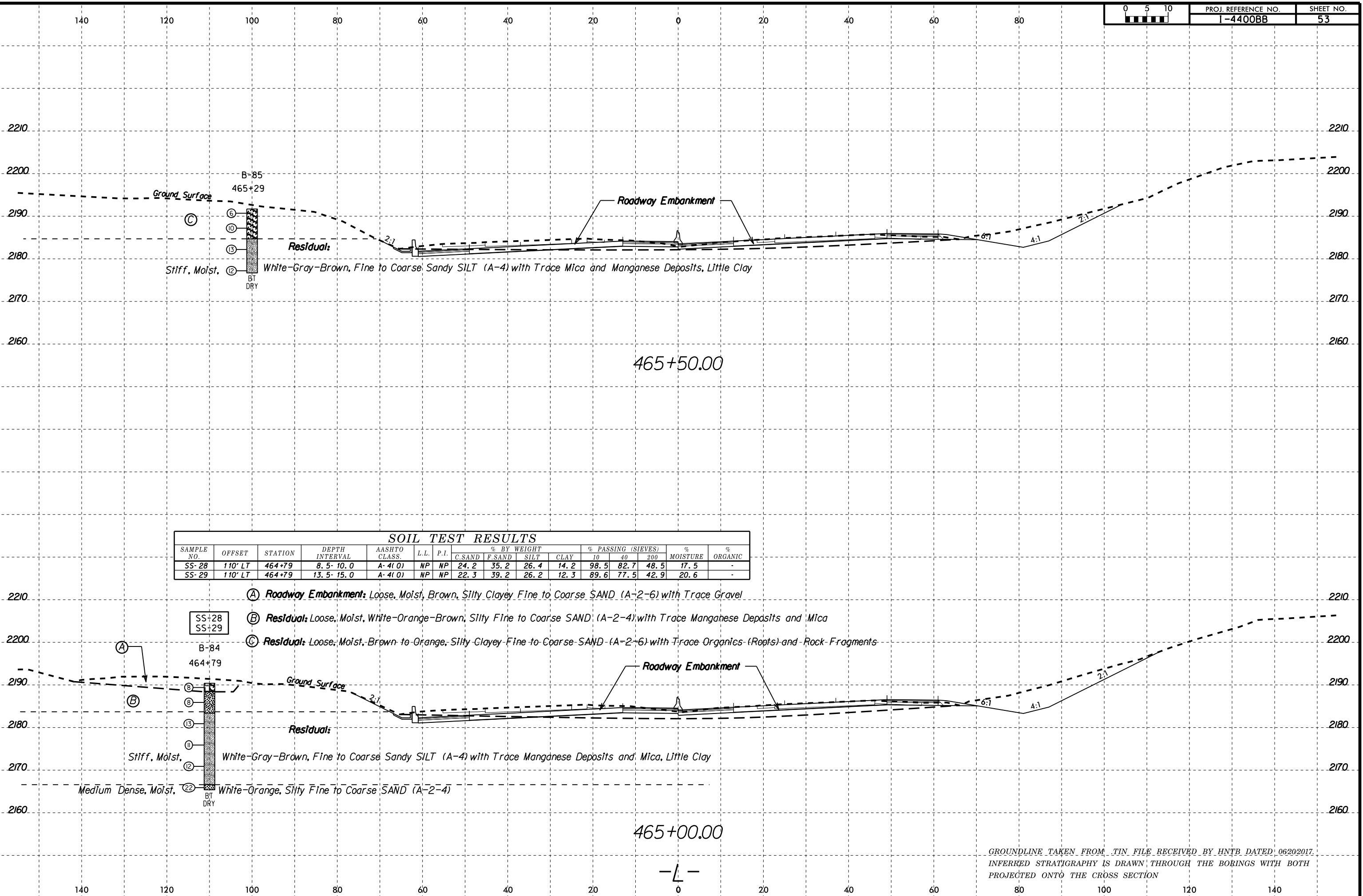
140

6/23/16
25-JUN-2018 15:12
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Walker-A 66026102



GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

6/23/16



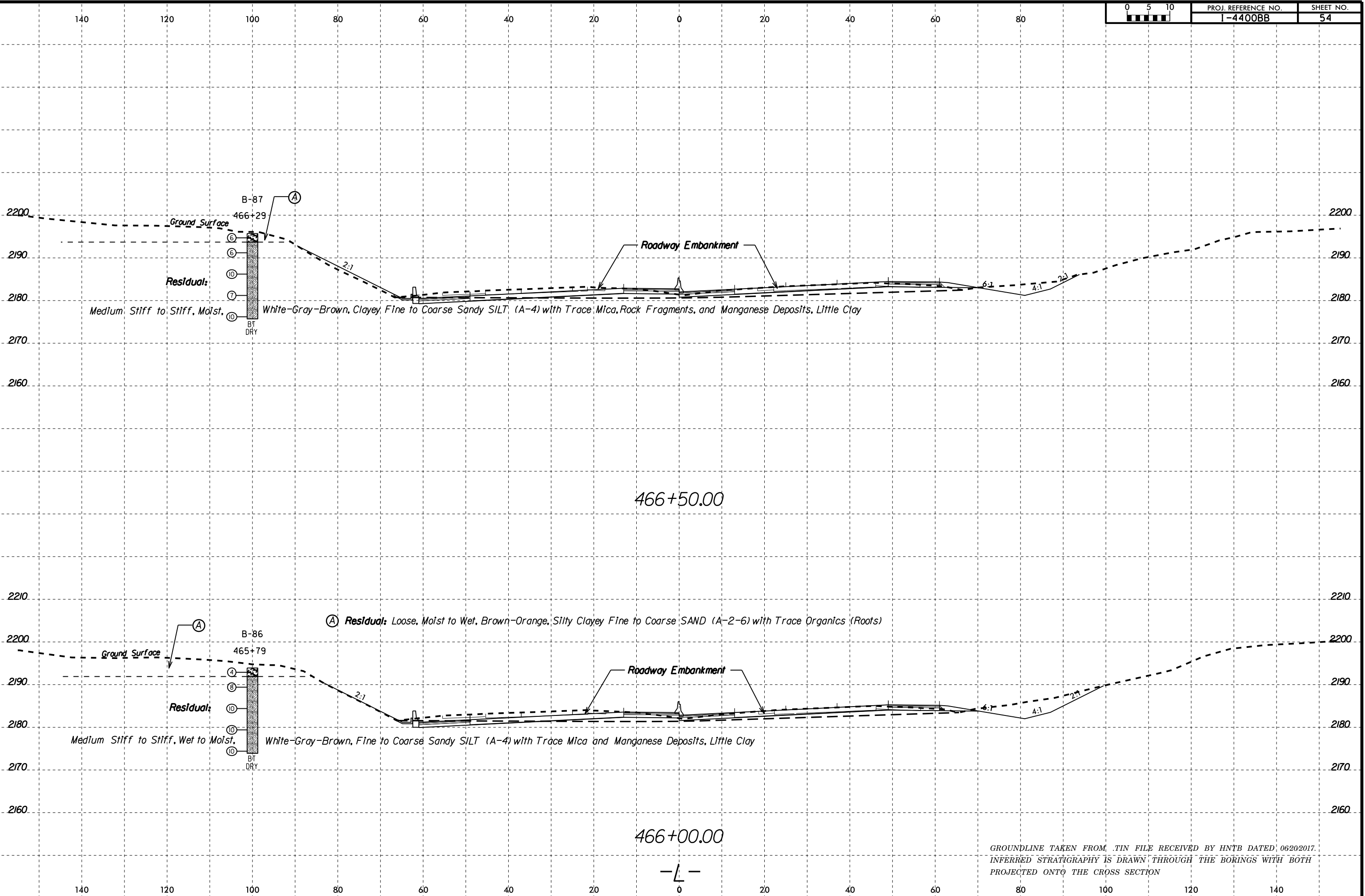
SOIL TEST RESULTS

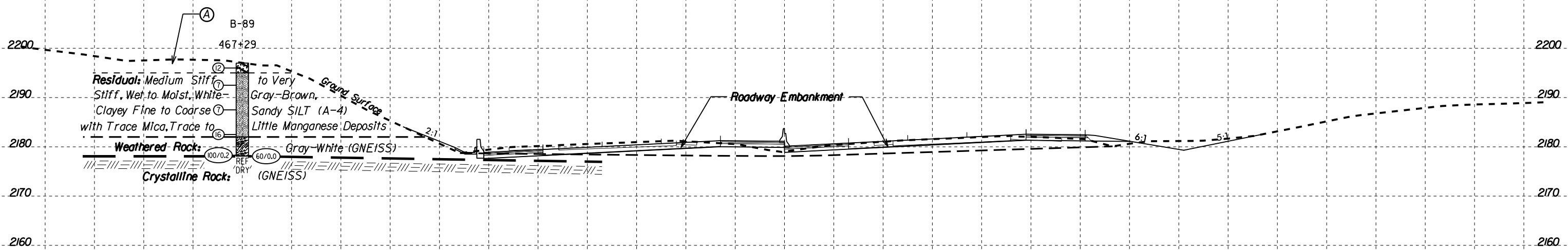
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							C.SAND	F.SAND	SILT	CLAY	10	40			200
SS-28	110' LT	464+79	8.5'-10.0'	A-4(0)	NP	NP	24.2	35.2	26.4	14.2	98.5	82.7	48.5	17.5	-
SS-29	110' LT	464+79	13.5'-15.0'	A-4(0)	NP	NP	22.3	39.2	26.2	12.3	89.6	77.5	42.9	20.6	-

- (A) Roadway Embankment: Loose, Moist, Brown, Silty Clayey Fine to Coarse SAND (A-2-6) with Trace Gravel
- (B) Residual: Loose, Moist, White-Orange-Brown, Silty Fine to Coarse SAND (A-2-4) with Trace Manganese Deposits and Mica
- (C) Residual: Loose, Moist, Brown to Orange, Silty Clayey Fine to Coarse SAND (A-2-6) with Trace Organics (Roots) and Rock Fragments

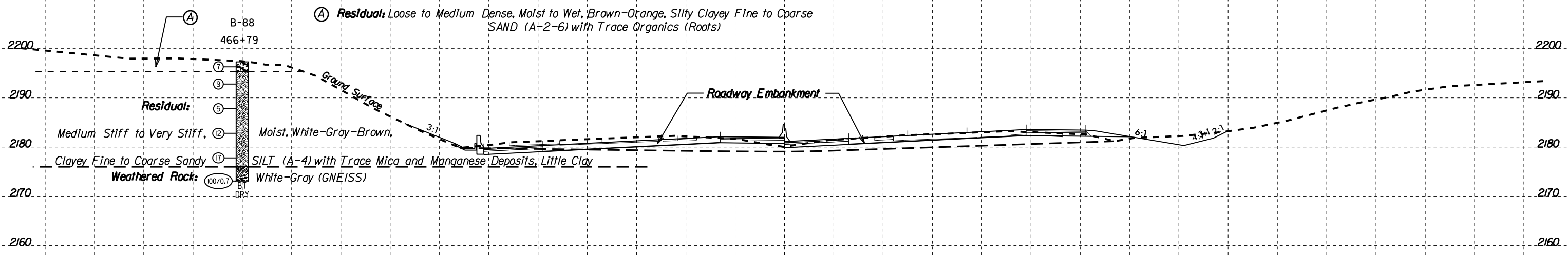
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Walker-A 66026102

GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION





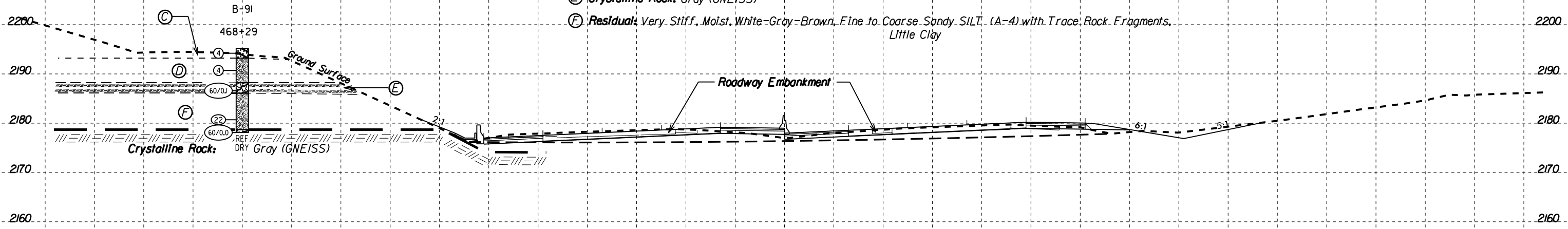
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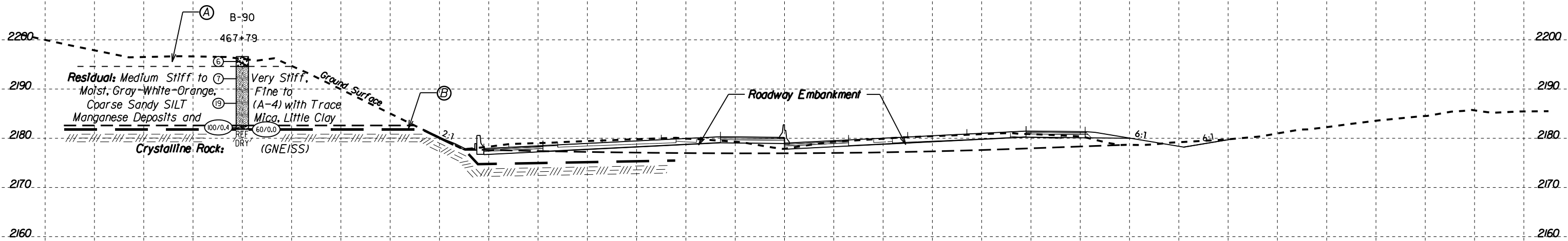
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GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

- (A) *Residual: Loose, Moist, Orange, Silty Clayey Fine to Coarse SAND (A-2-6) with Trace Organics (Roots) and Mica*
- (B) *Weathered Rock: Gray (GNEISS)*
- (C) *Residual: Loose, Wet, Brown, Silty Clayey Fine to Coarse SAND (A-2-6) with Trace Organics (Roots)*
- (D) *Residual: Soft to Medium Stiff, Moist, White-Gray-Brown, Fine to Coarse Sandy SILT (A-4) with Little Clay*
- (E) *Crystalline Rock: Gray (GNEISS)*
- (F) *Residual: Very Stiff, Moist, White-Gray-Brown, Fine to Coarse Sandy SILT (A-4) with Trace, Rock Fragments, Little Clay*



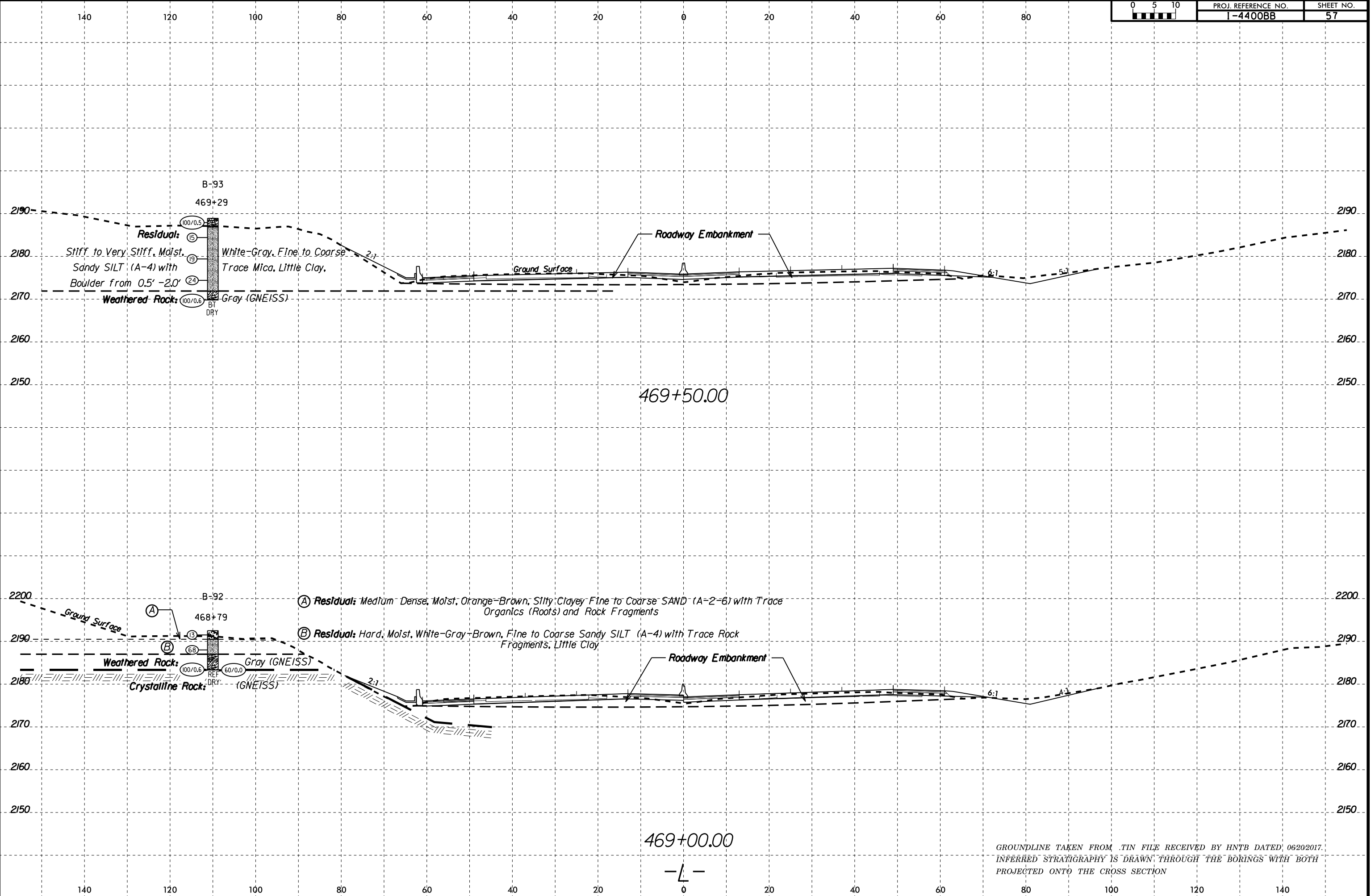
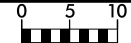
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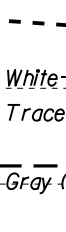
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GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

6/23/16



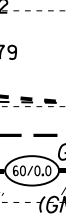
B-93
469+29
Residual: Stiff to Very Stiff, Moist, Sandy SILT (A-4) with Boulder from 0.5' - 2.0'
Weathered Rock: Gray (GNEISS)



White-Gray, Fine to Coarse Trace Mica, Little Clay.

469+50.00

B-92
468+79
Residual: Medium Dense, Moist, Orange-Brown, Silty, Clayey Fine to Coarse SAND (A-2-6) with Trace Organics (Roots) and Rock Fragments
Residual: Hard, Moist, White-Gray-Brown, Fine to Coarse Sandy SILT (A-4) with Trace Rock Fragments, Little Clay
Weathered Rock: Gray (GNEISS)
Crystalline Rock: (GNEISS)

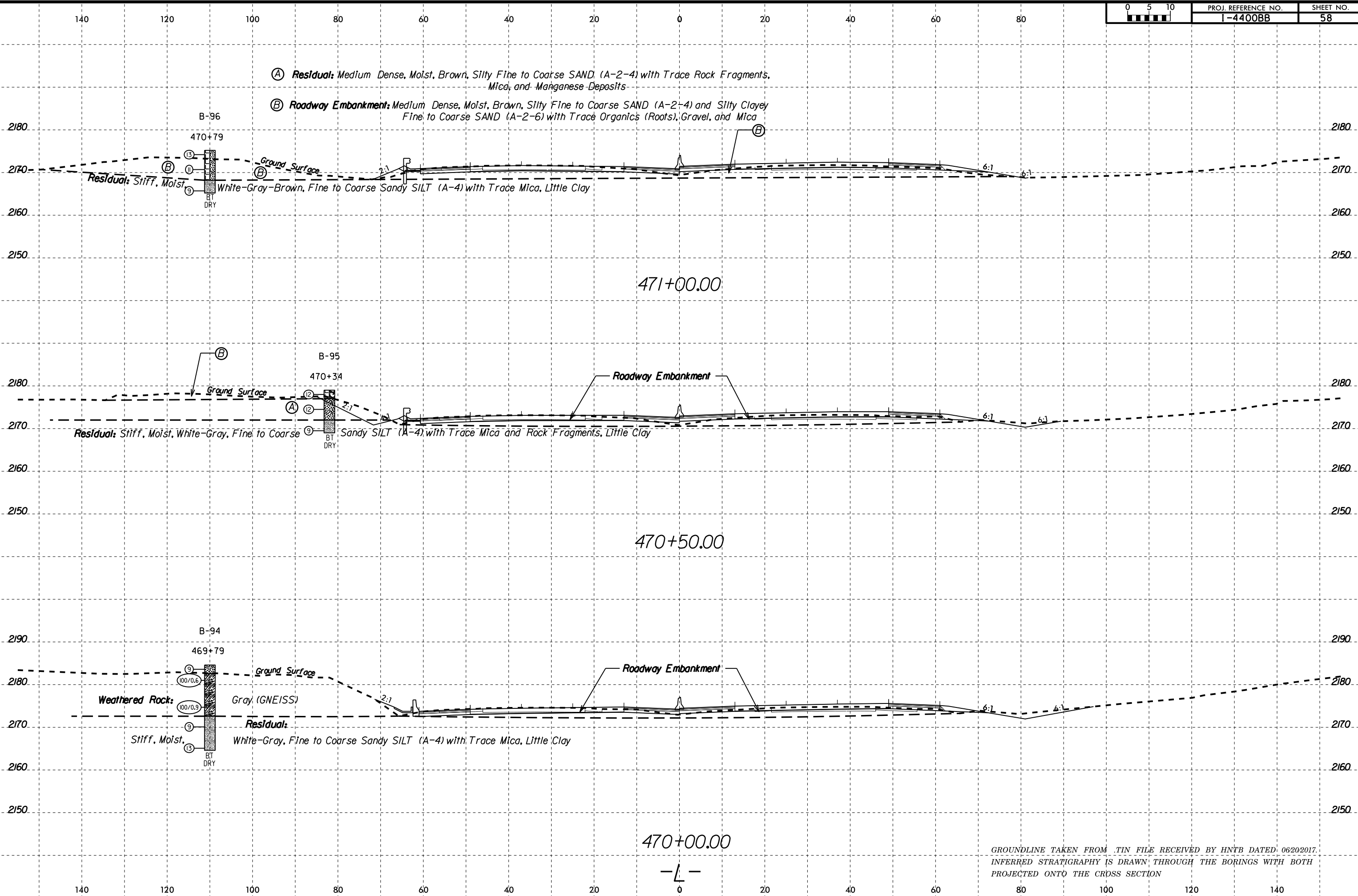


Gray (GNEISS)

469+00.00

GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

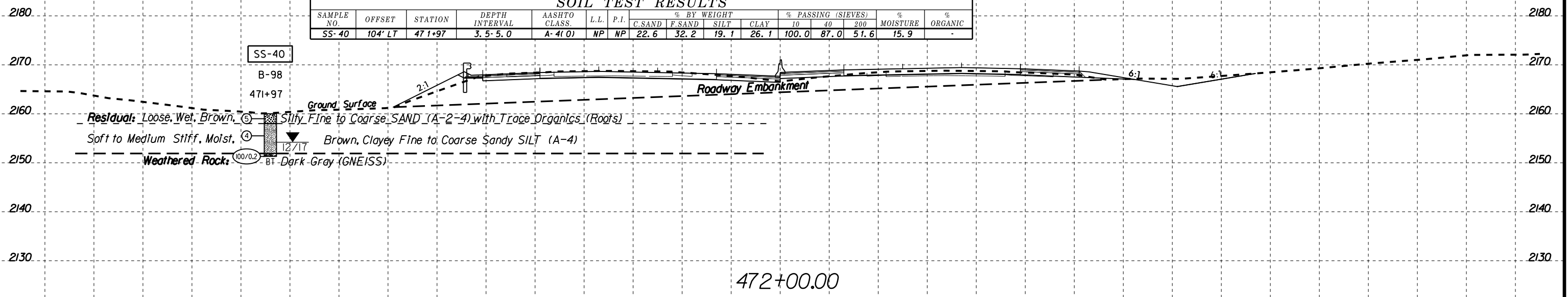
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GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

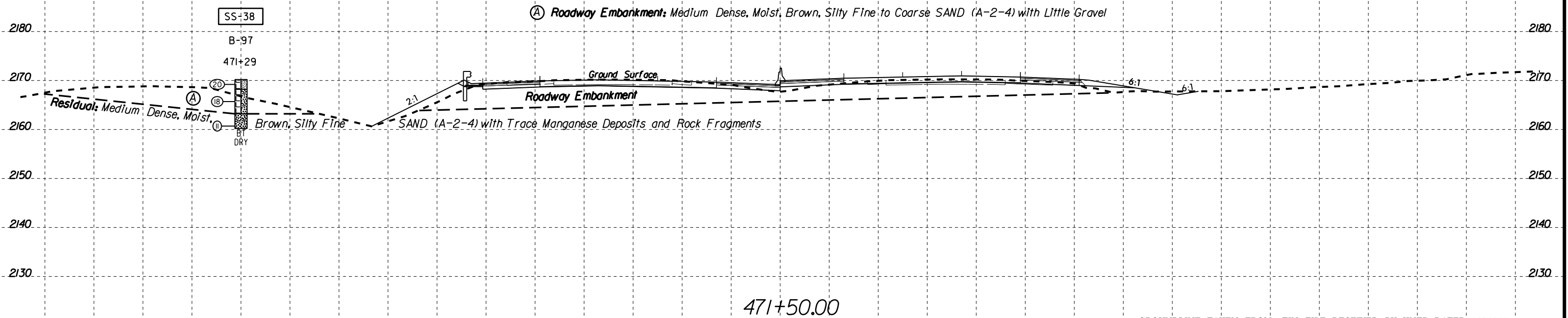
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-40	104' LT	471+97	3.5-5.0	A-4(0)	NP	NP	22.6	32.2	19.1	26.1	100.0	87.0	51.6	15.9	-



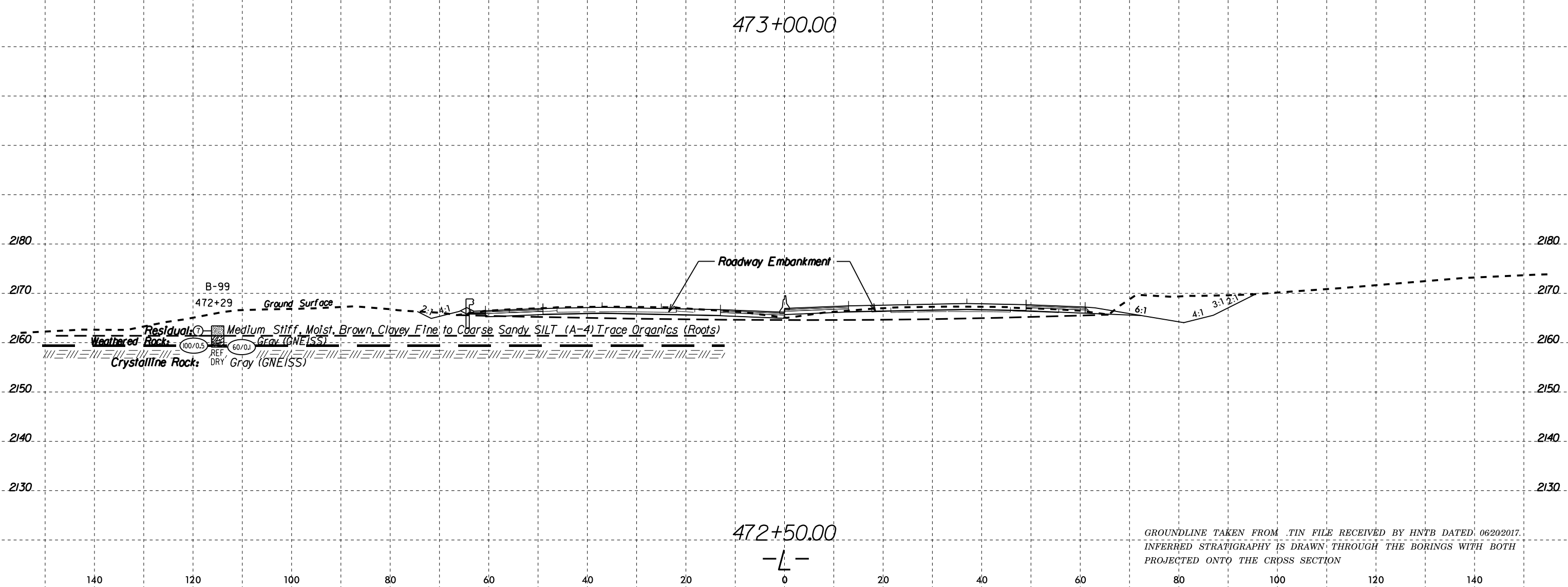
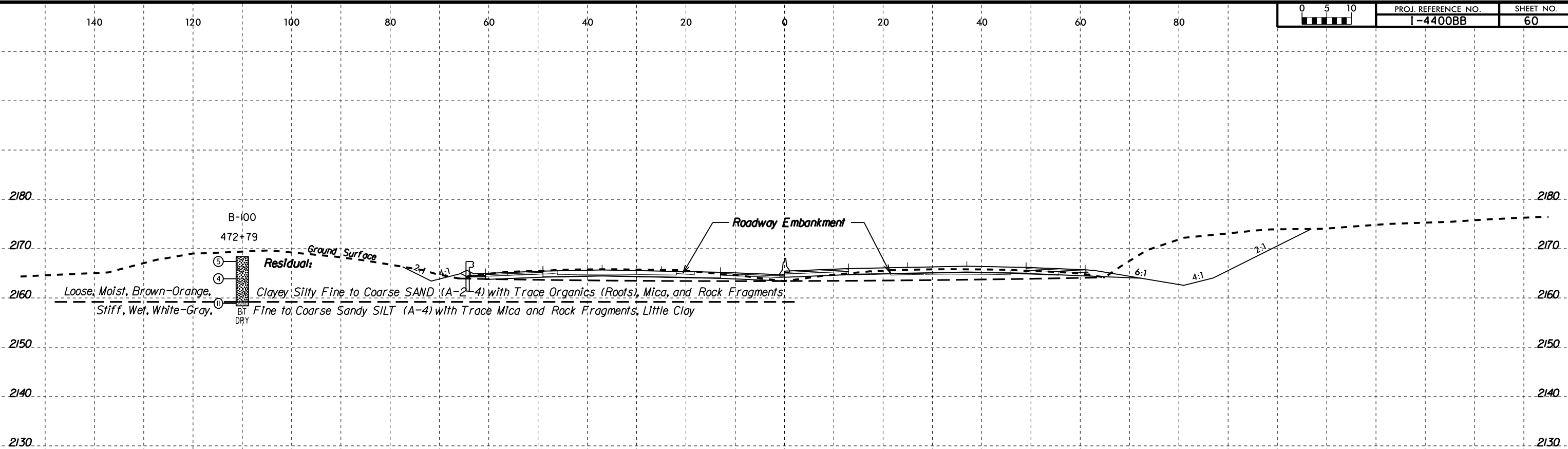
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-38	110' LT	471+29	0.1-1.5	A-4(0)	NP	NP	21.9	24.2	13.8	40.1	90.9	76.9	52.3	13.7	-



GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
 PROJECTED ONTO THE CROSS SECTION

6/23/16



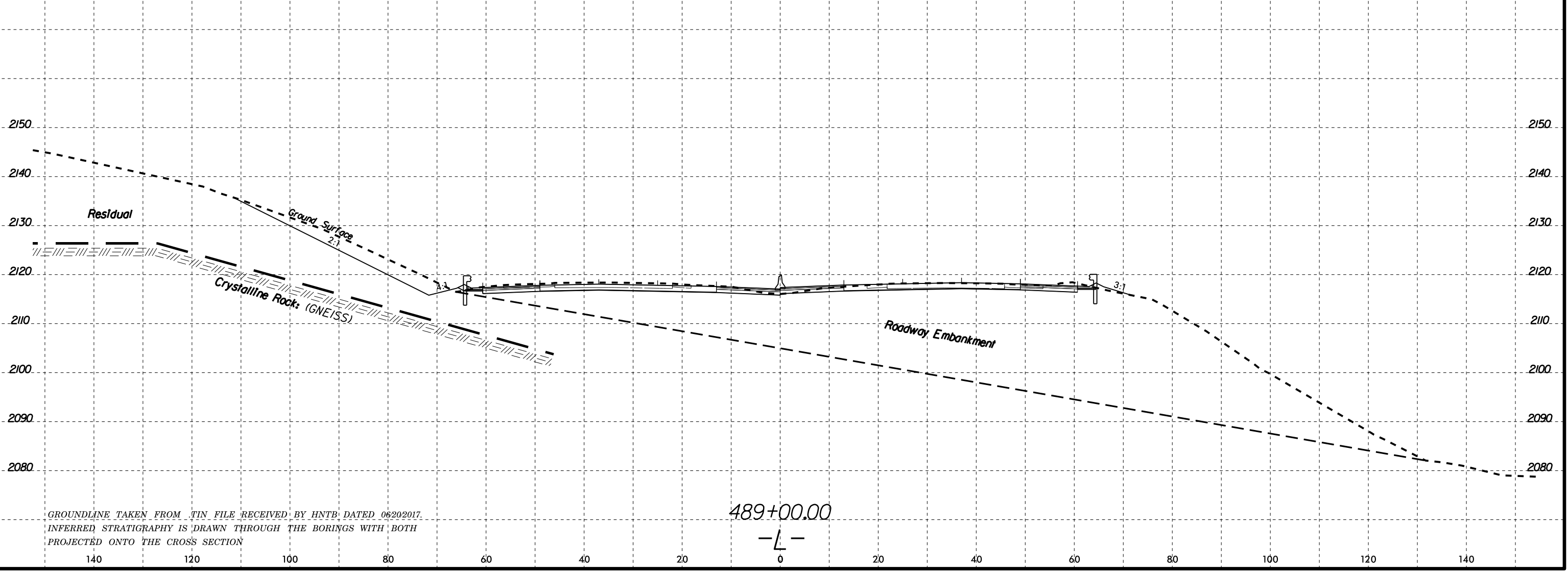
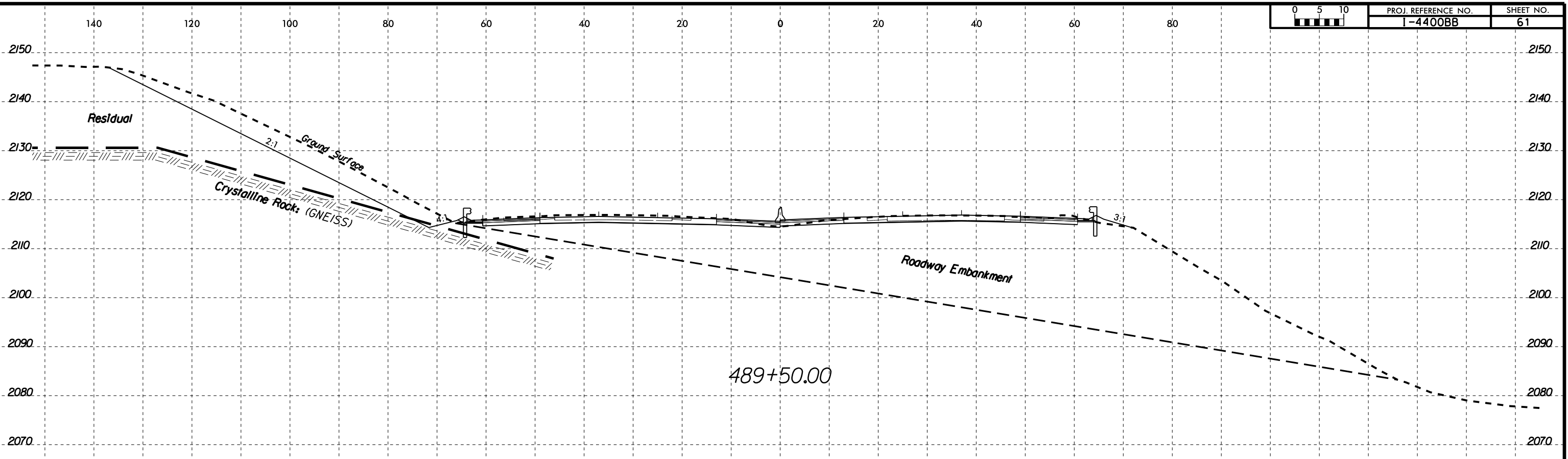
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6/23/16



PROJ. REFERENCE NO.	SHEET NO.
I-4400BB	61



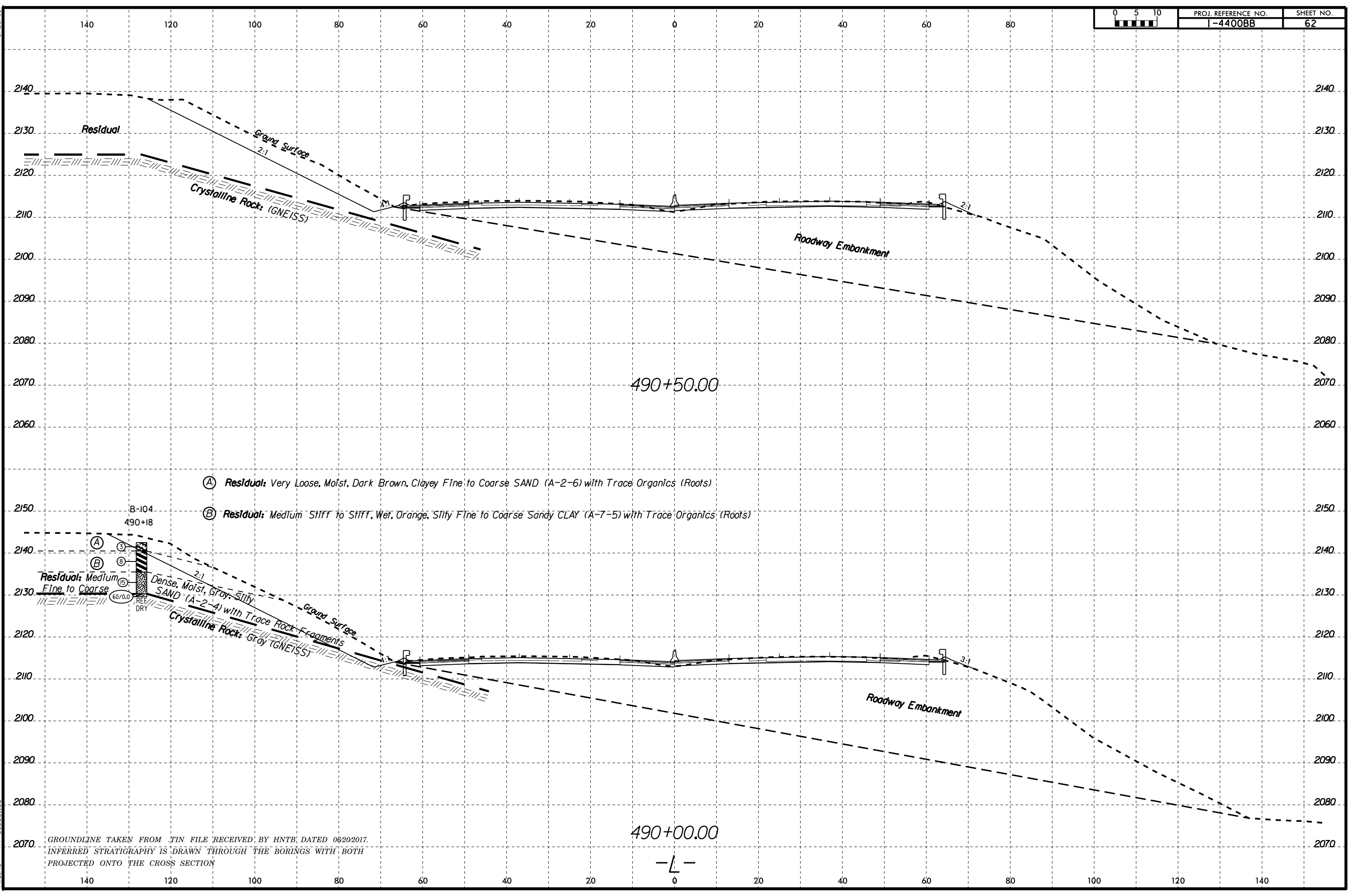
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6/23/16
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Walker-A 66026102



PROJ. REFERENCE NO.	SHEET NO.
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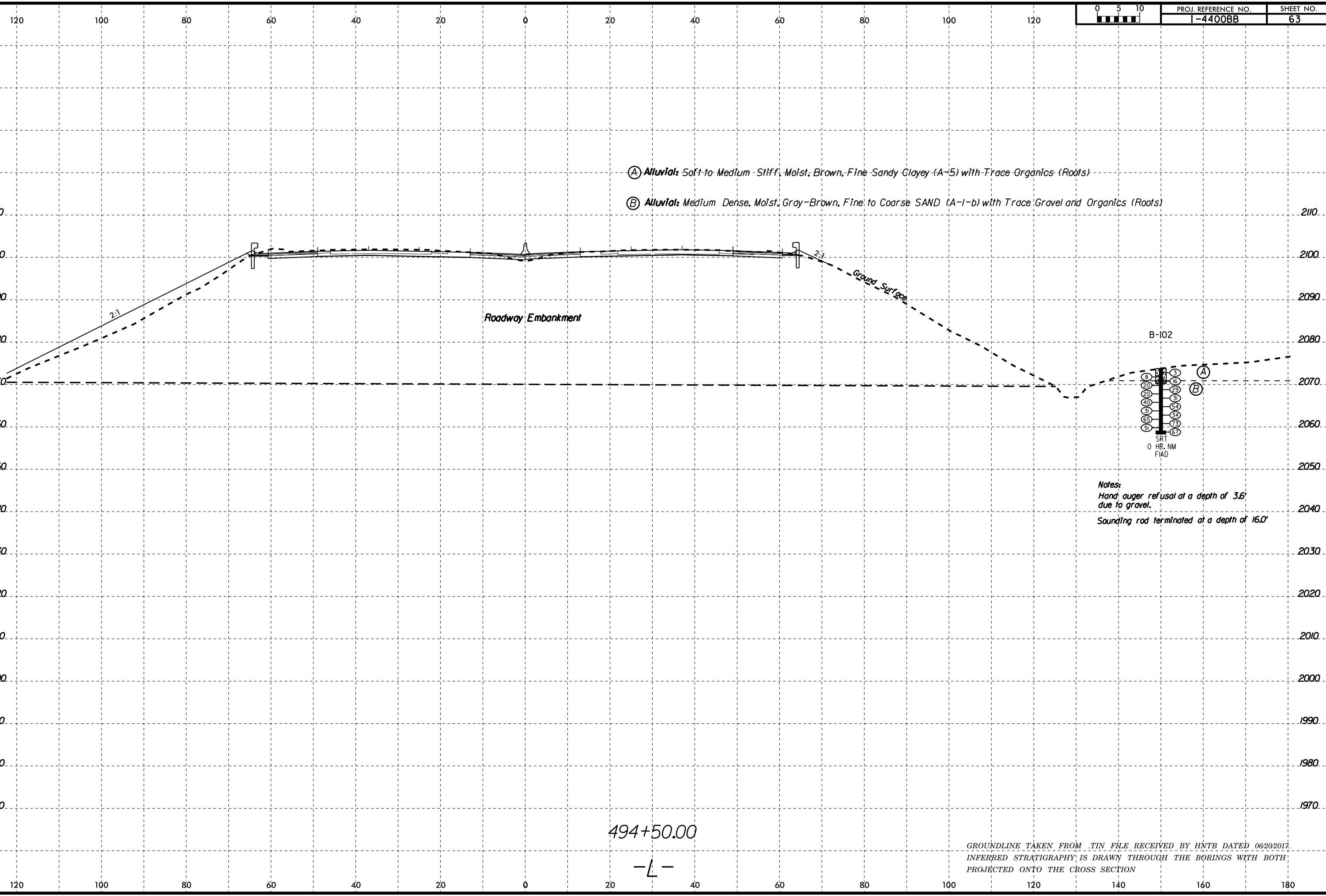


GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

490+00.00

490+50.00

6/23/16
25-JUN-2018 15:55
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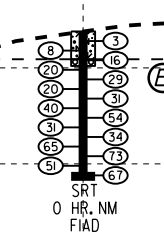
(A) Alluvial: Soft to Medium Stiff, Moist, Brown, Fine Sandy Clayey (A-5) with Trace Organics (Roots)

(B) Alluvial: Medium Dense, Moist, Gray-Brown, Fine to Coarse SAND (A-1-b) with Trace Gravel and Organics (Roots)

Roadway Embankment

Ground Surface

B-102



Notes:
Hand auger refusal at a depth of 3.6'
due to gravel.
Sounding rod terminated at a depth of 16.0'

494+50.00

-L-

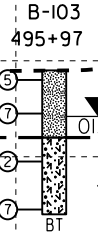
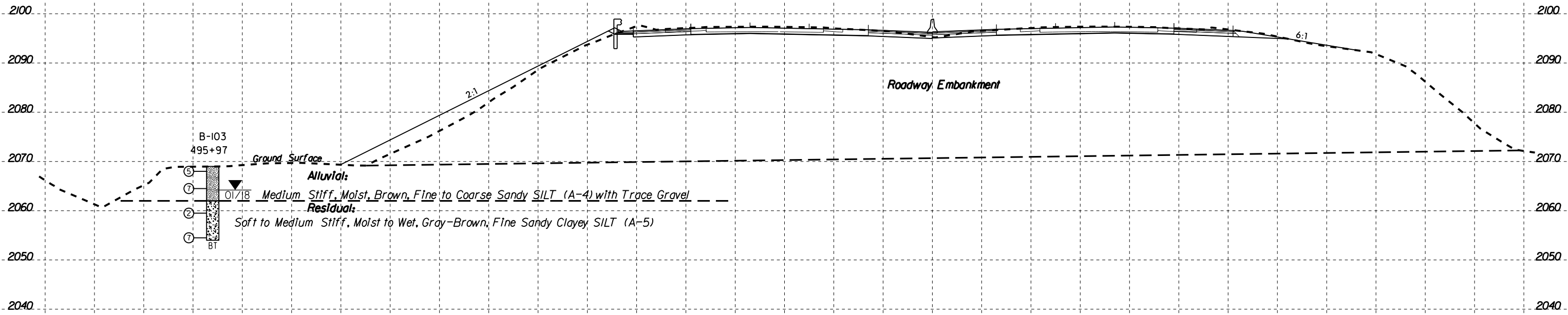
GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

6/23/16



PROJ. REFERENCE NO.	SHEET NO.
I-4400BB	64

140 120 100 80 60 40 20 0 20 40 60 80



Ground Surface
Alluvial:
 Medium Stiff, Moist, Brown, Fine to Coarse Sandy SILT (A-4) with Trace Gravel
Residual:
 Soft to Medium Stiff, Moist to Wet, Gray-Brown, Fine Sandy Clayey SILT (A-5)

496+00.00



GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
 PROJECTED ONTO THE CROSS SECTION

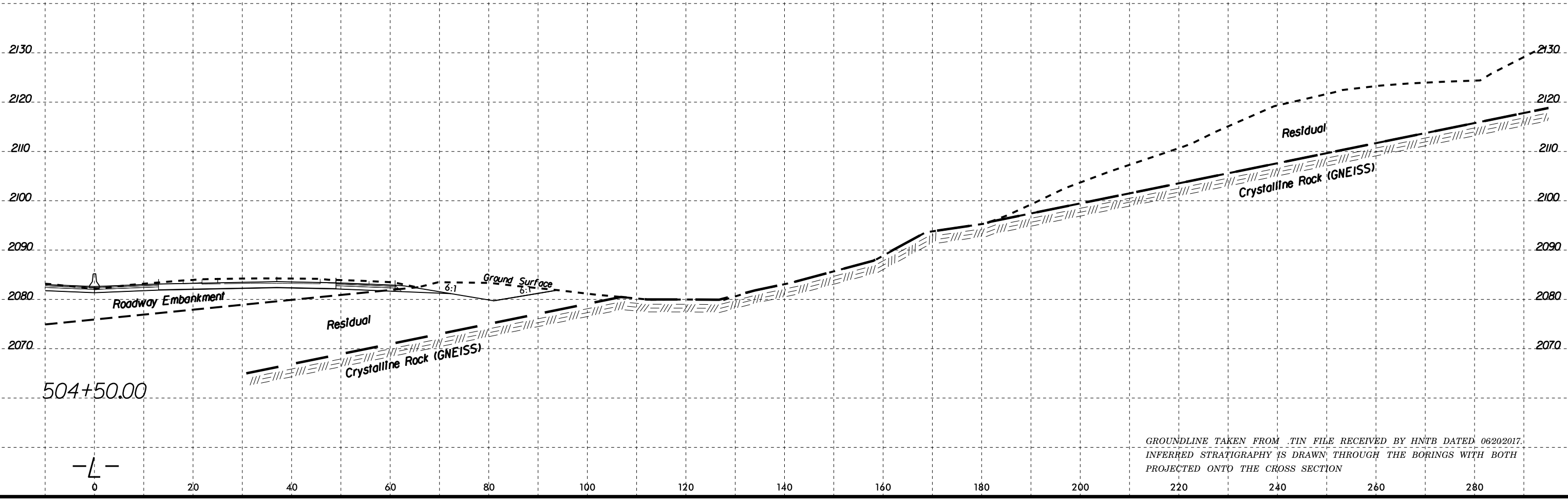
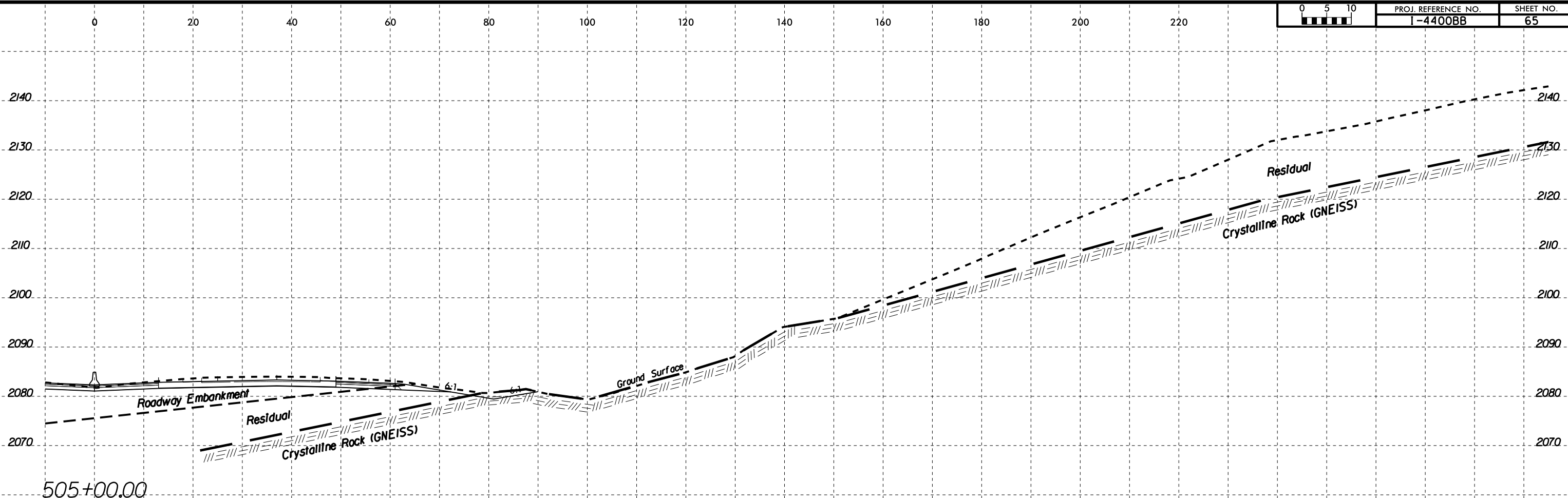
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6/23/16



PROJ. REFERENCE NO.	SHEET NO.
I-4400BB	65

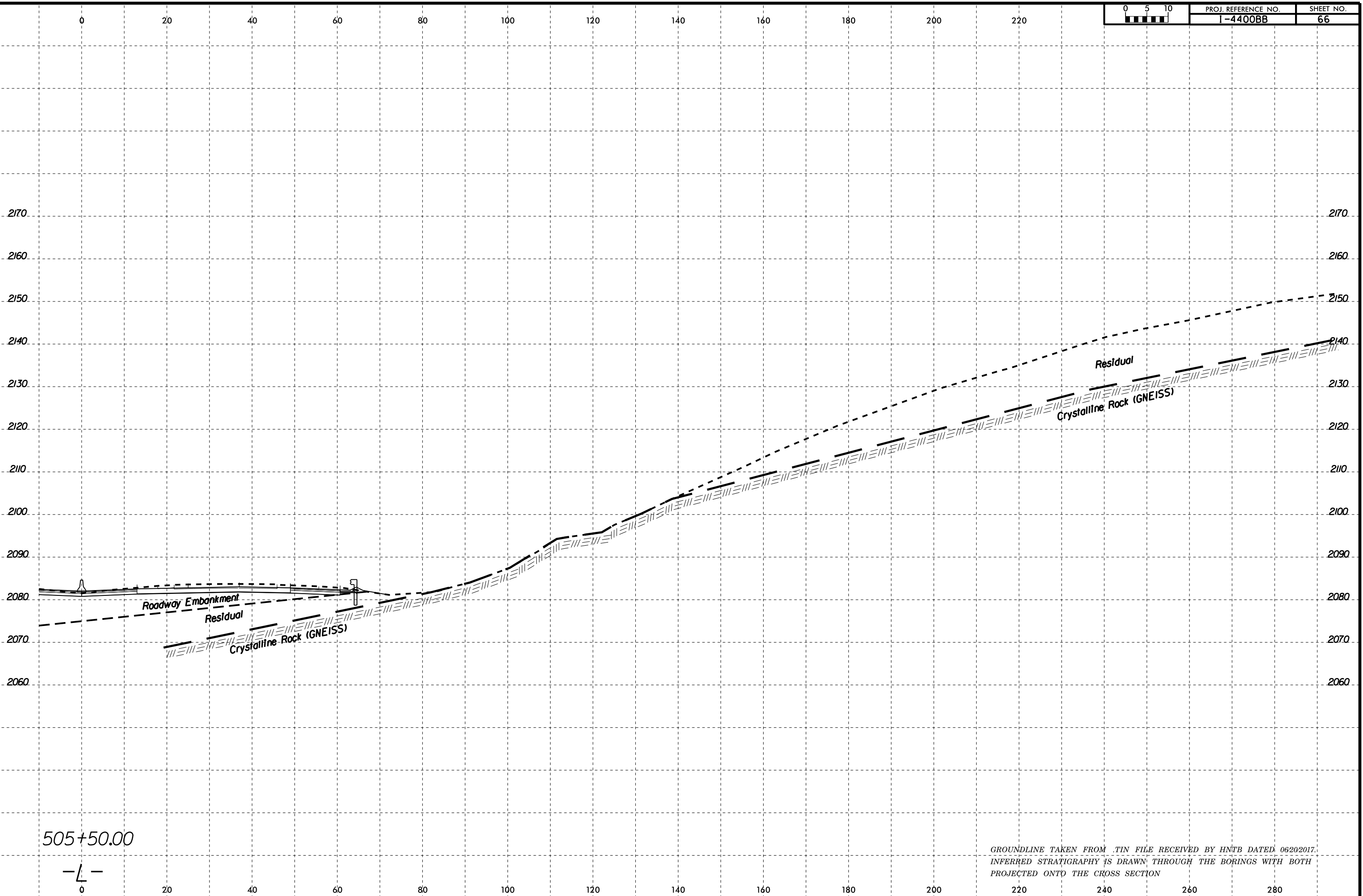
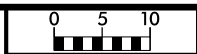


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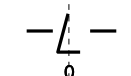


GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

6/23/16
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Walker



505+50.00



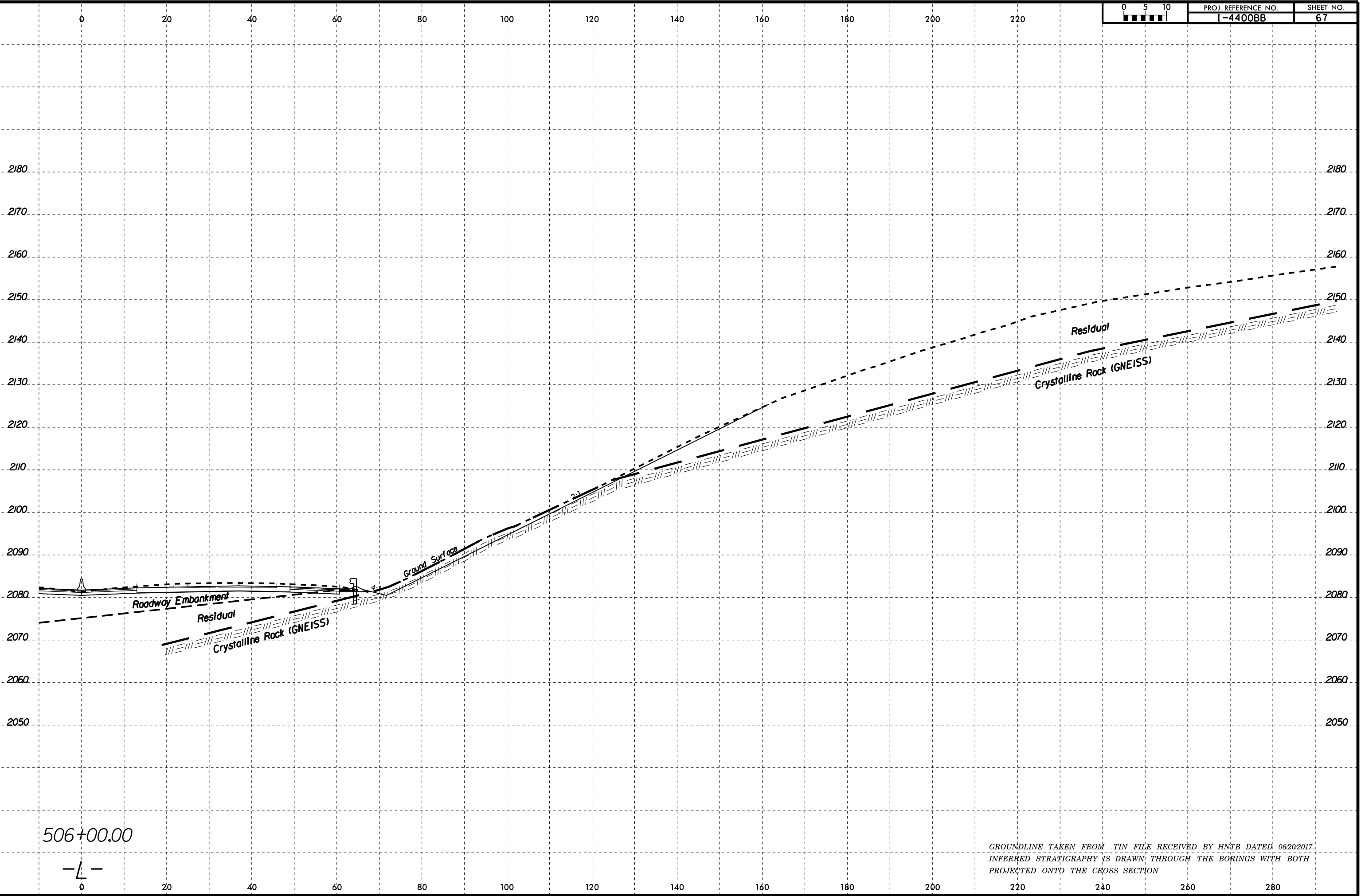
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INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

6/23/16

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PROJ. REFERENCE NO.	SHEET NO.
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506+00.00

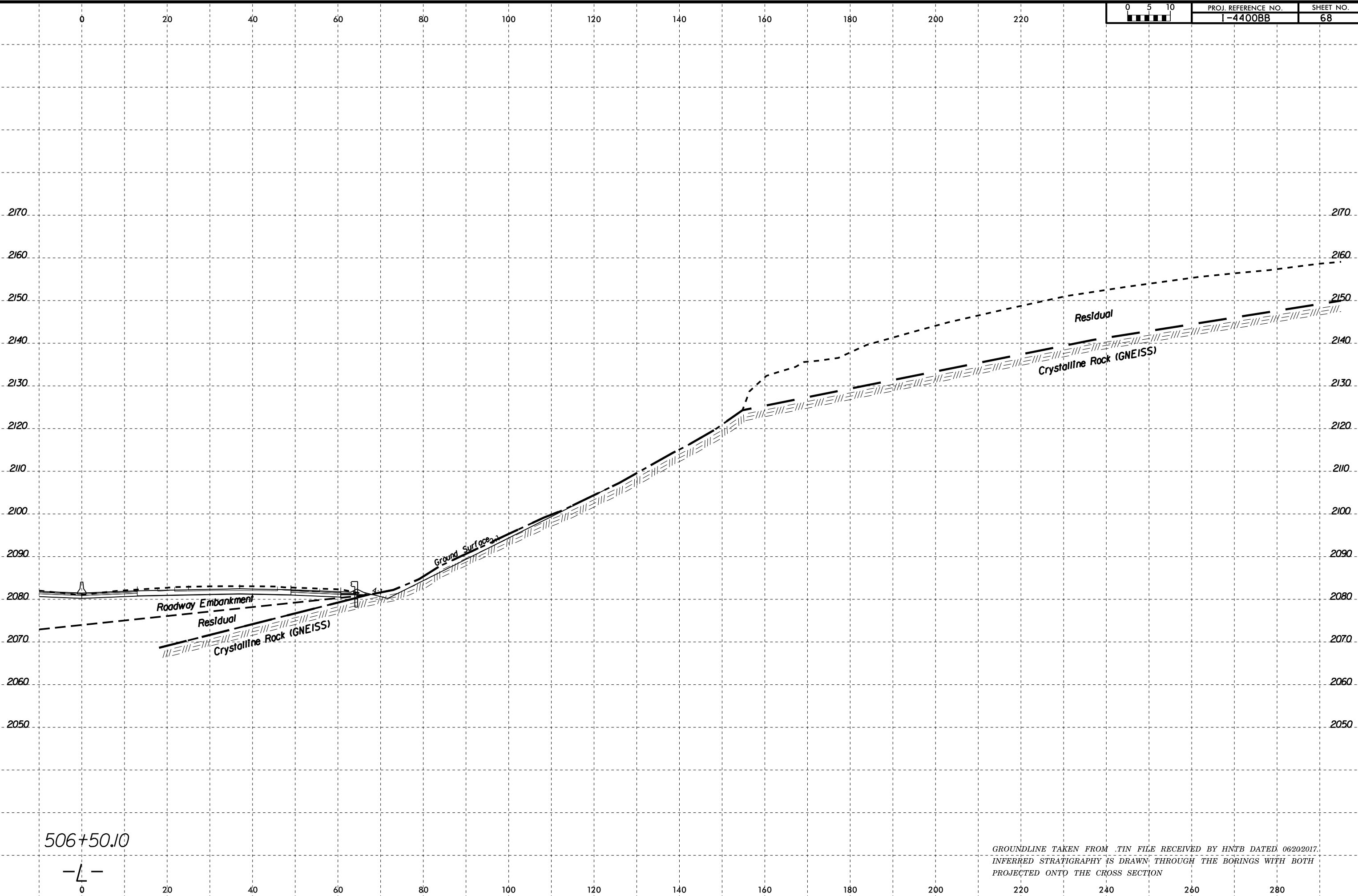


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INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

6/23/16



PROJ. REFERENCE NO.	SHEET NO.
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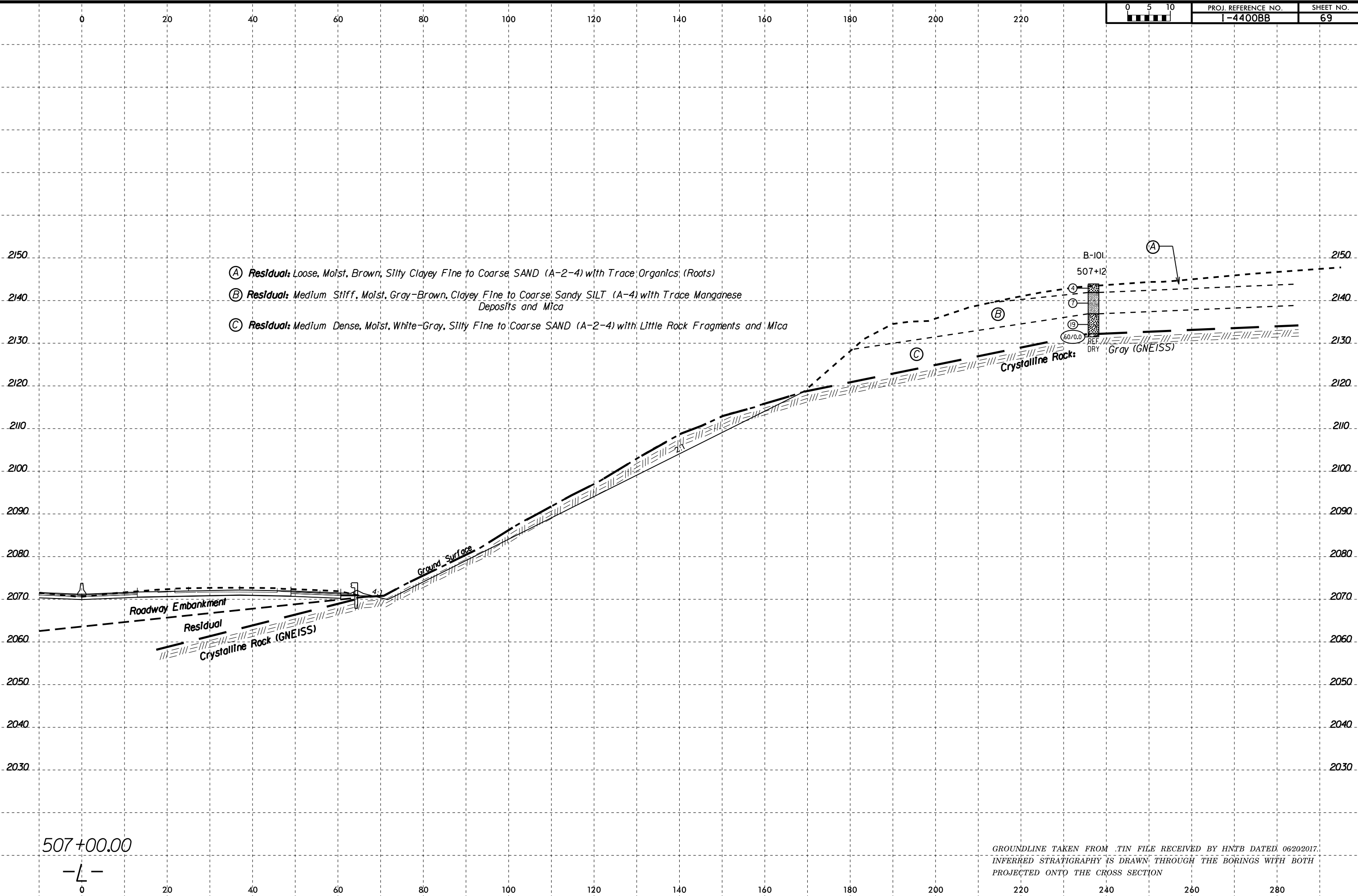
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506+50.10



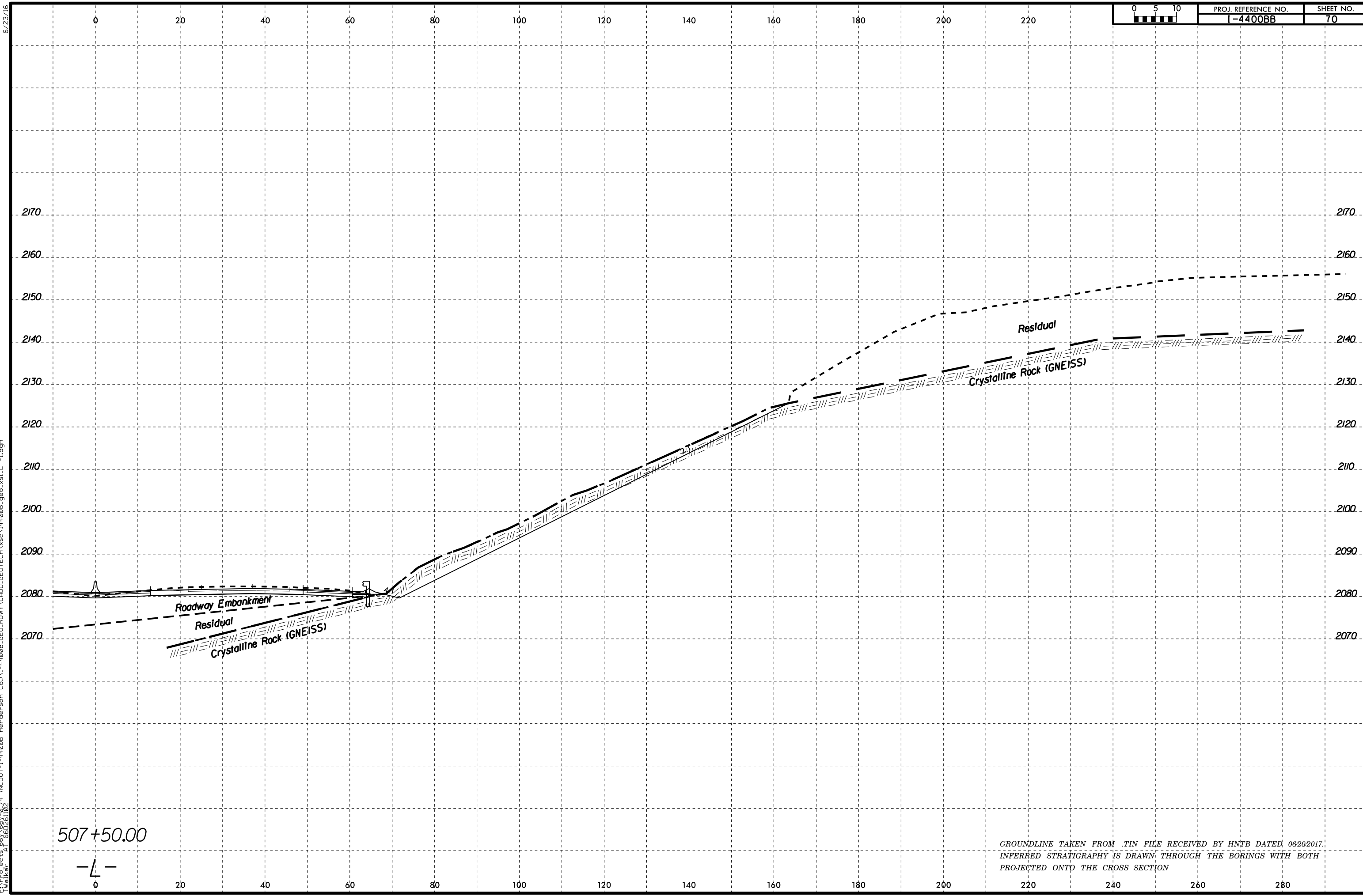
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6/23/16

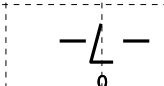


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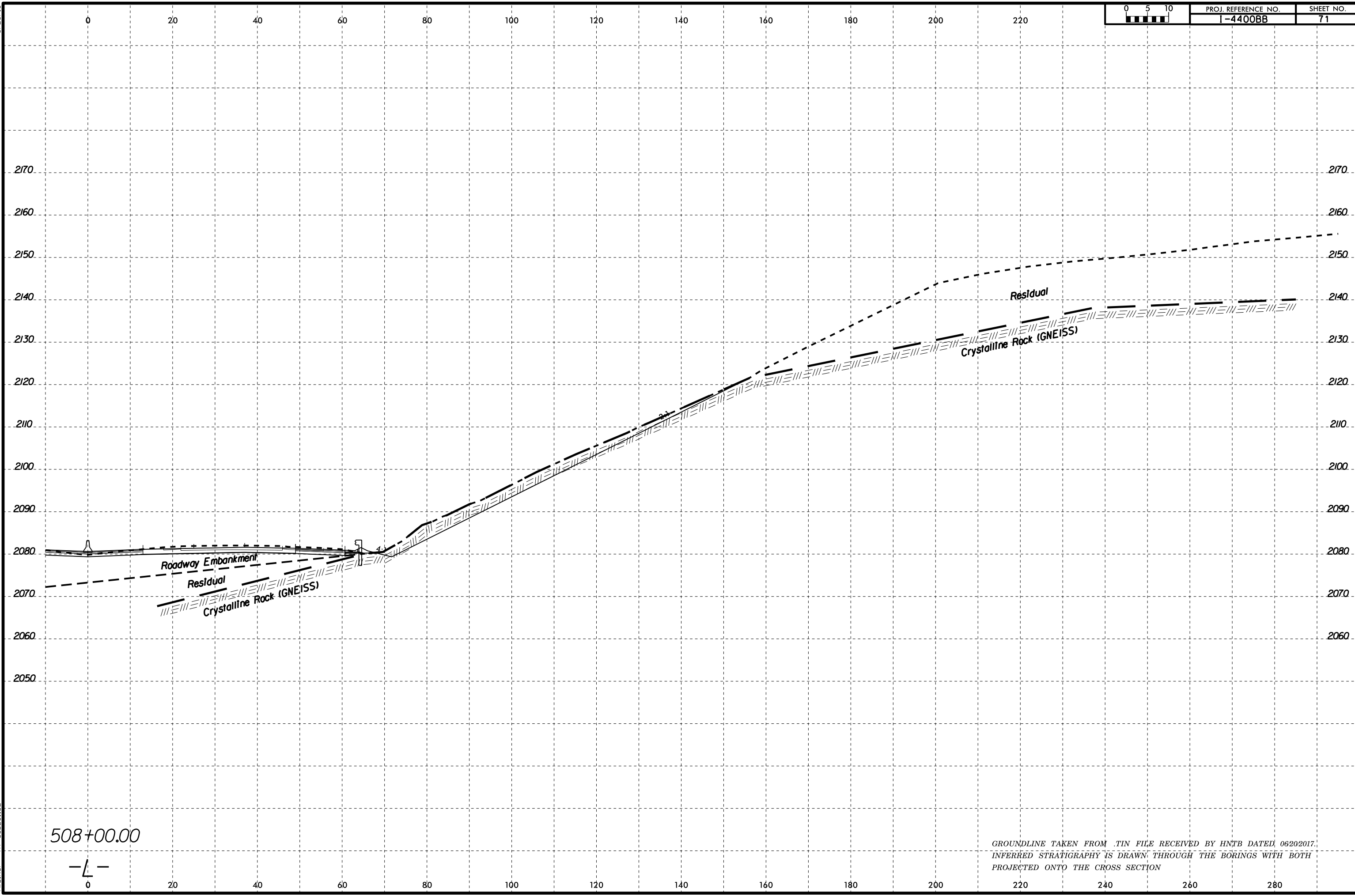


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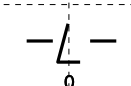


GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017. INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION

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Walker

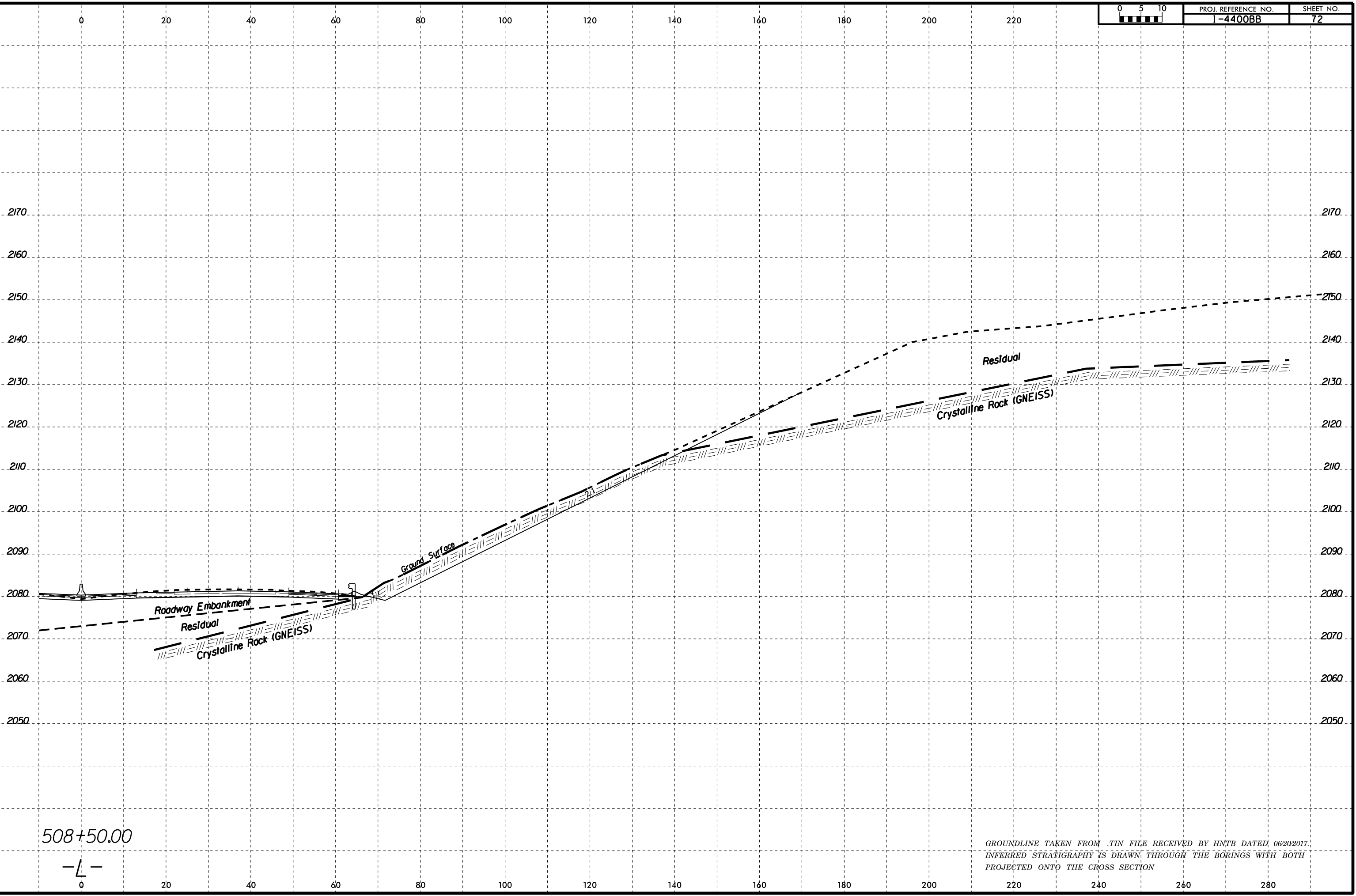


508+00.00



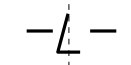
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INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

6/23/16



25-JUN-2016 15:27
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Walker

508+50.00

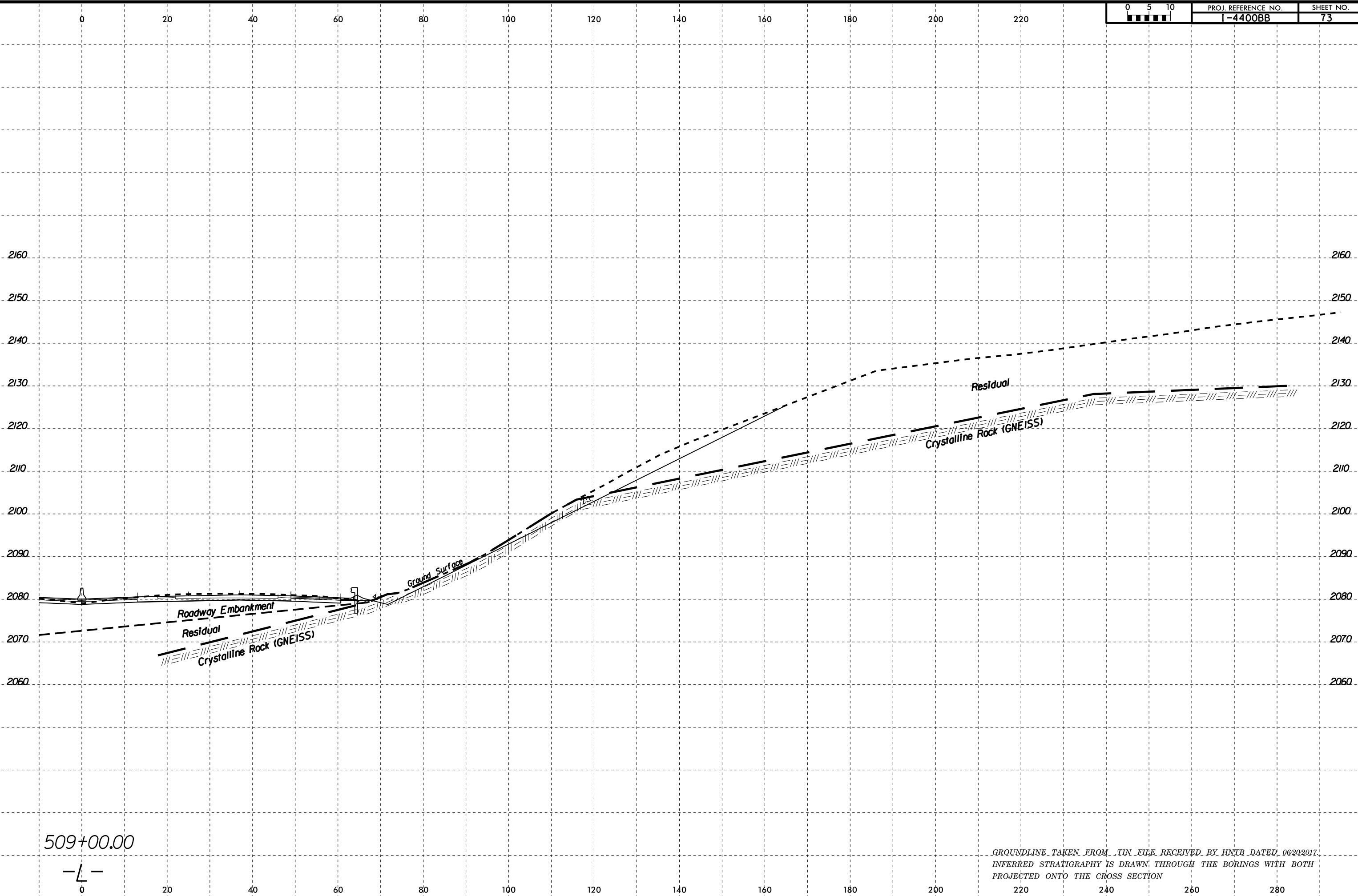


GROUNDLINE TAKEN FROM .TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

6/23/16



PROJ. REFERENCE NO.	SHEET NO.
I-4400BB	73

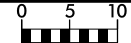


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 Walker A 66026102

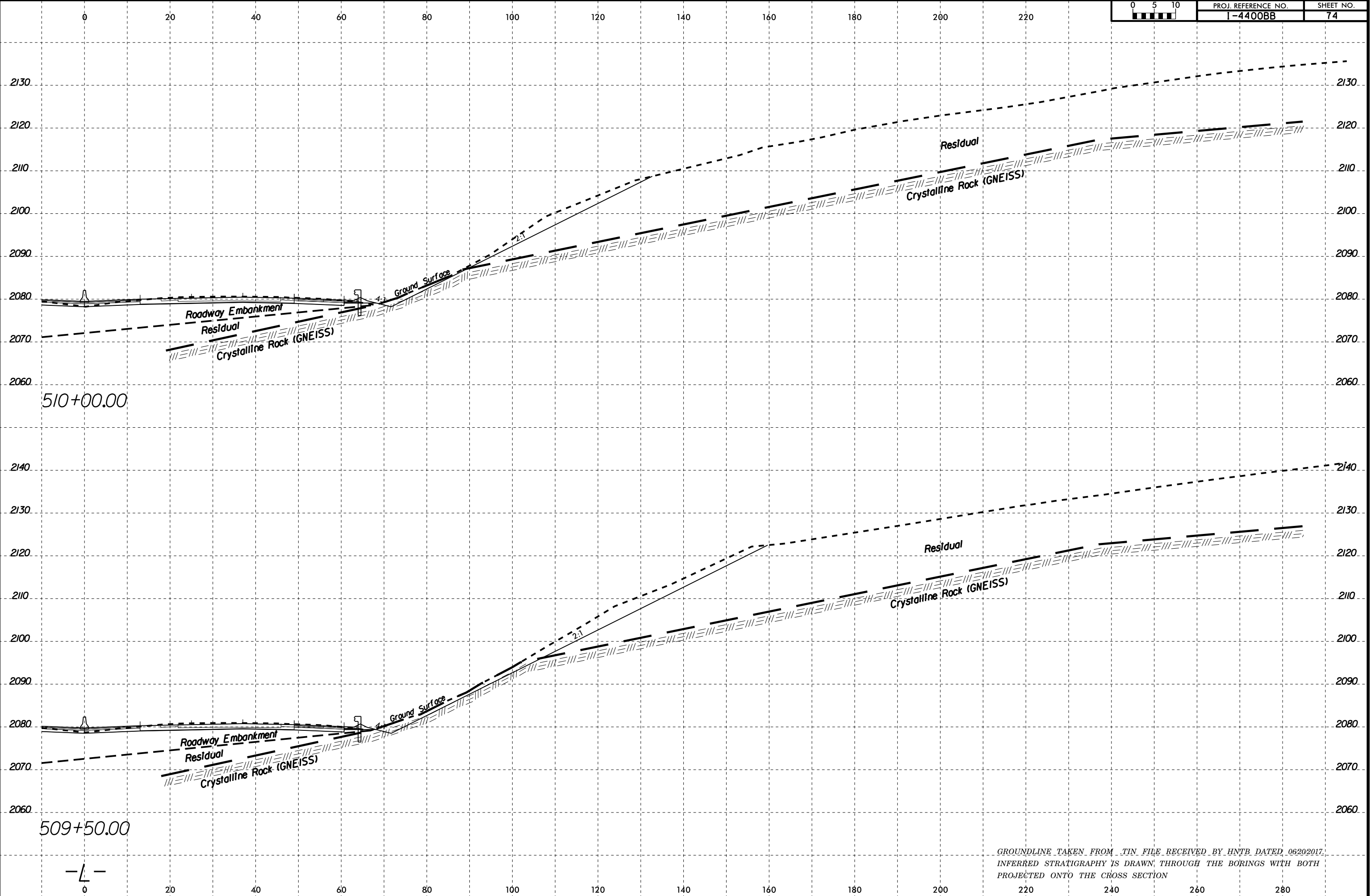
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GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06202017.
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 PROJECTED ONTO THE CROSS SECTION

6/23/16



PROJ. REFERENCE NO. I-4400BB SHEET NO. 74



510+00.00

509+50.00

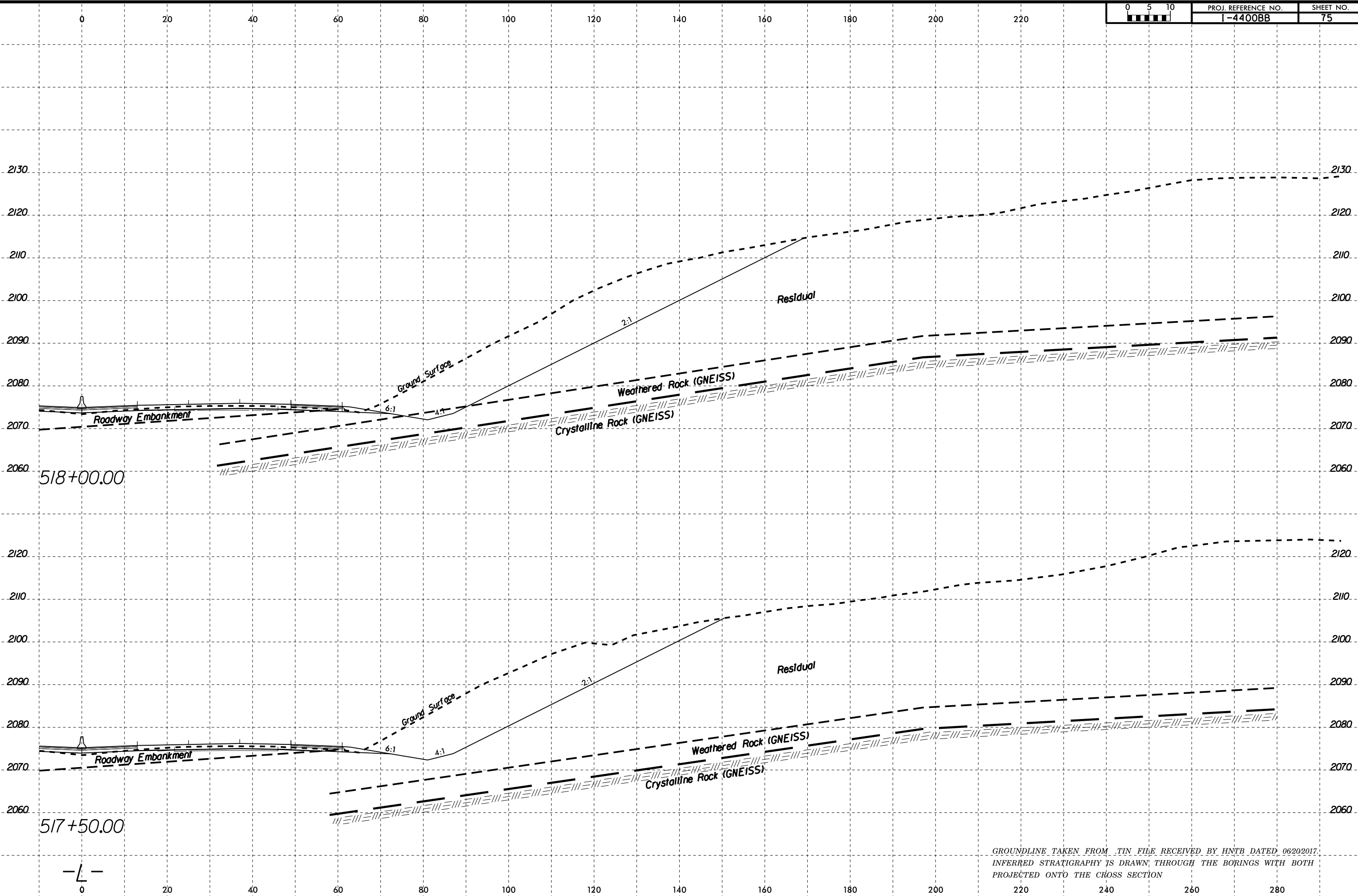
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INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION

27 JUN 2019 13:21:56 \\S61-0174 (NCDDOT-I-4400B Henderson Co.)\I-4400B.GEO\RDWY\CADD_GEO\TECH\ssc\14400B.geo_xsl.L -2.dgn

6/23/16



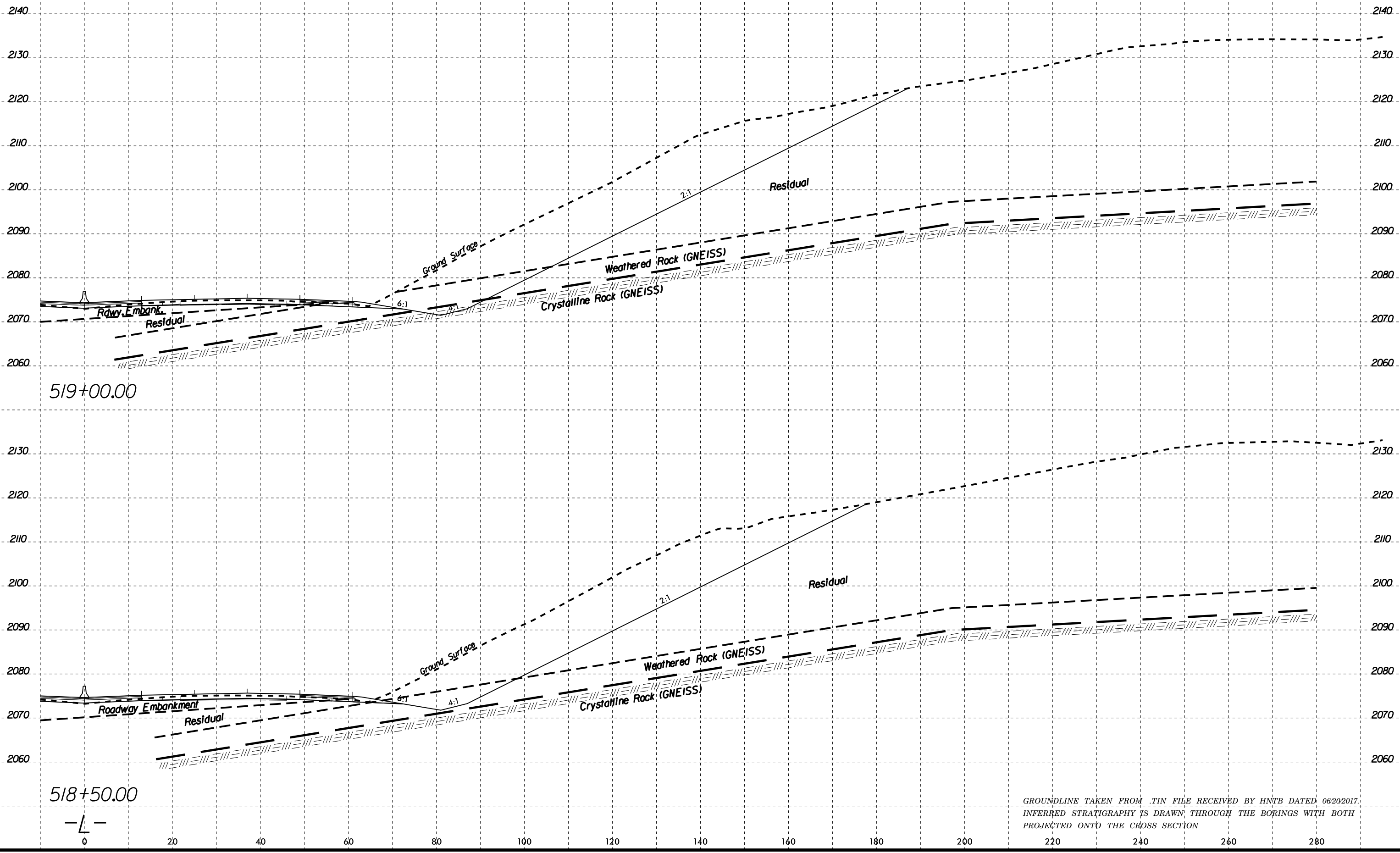
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GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
 PROJECTED ONTO THE CROSS SECTION

6/23/16



25-JUN-2016 15:28
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Walker

GROUNDLINE TAKEN FROM TIN FILE RECEIVED BY HNTB DATED 06/20/2017.
INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE CROSS SECTION