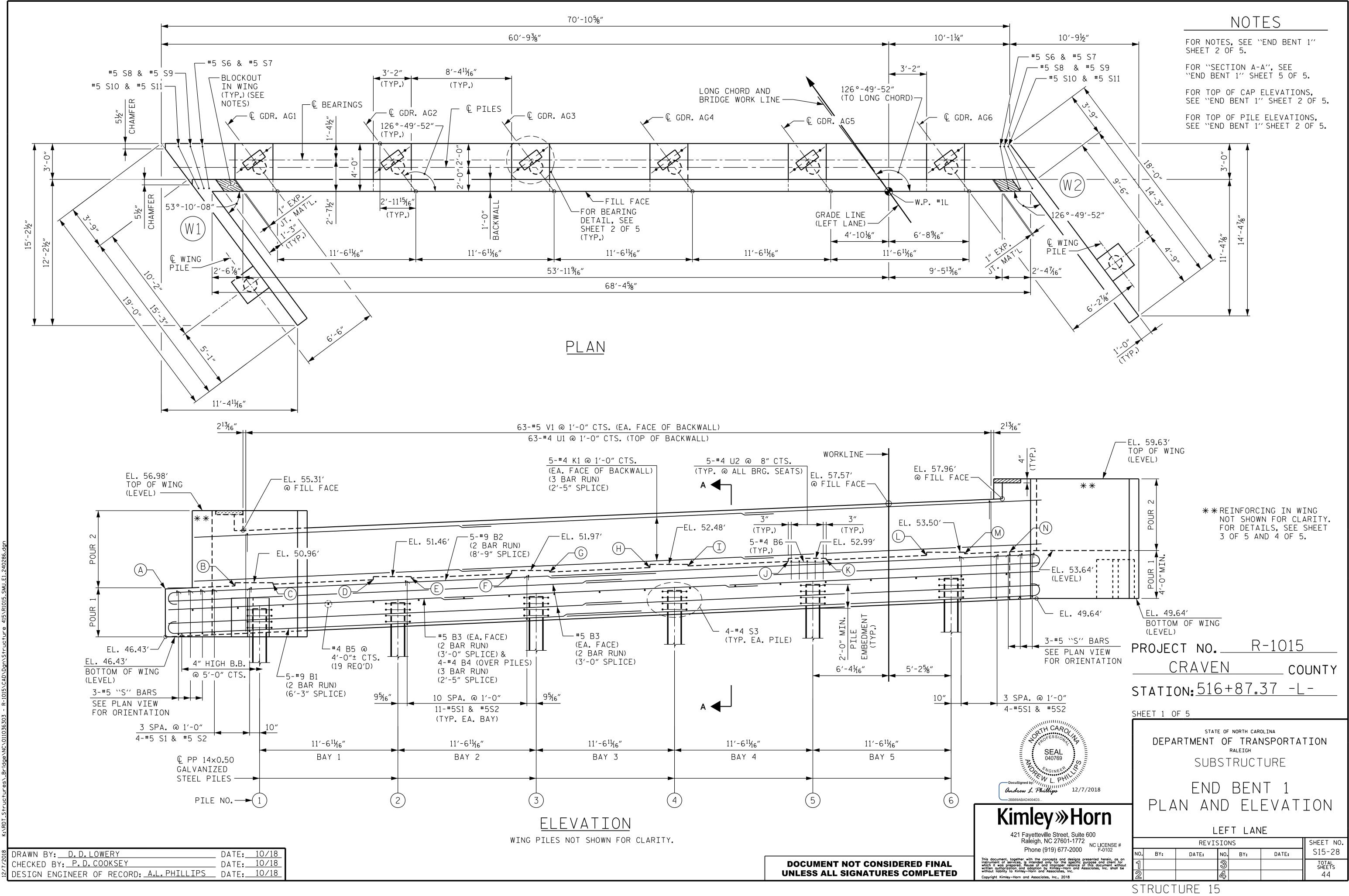
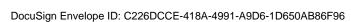
This electronic collection of documents is provided for the convenience of the user and is Not a Certified Document -

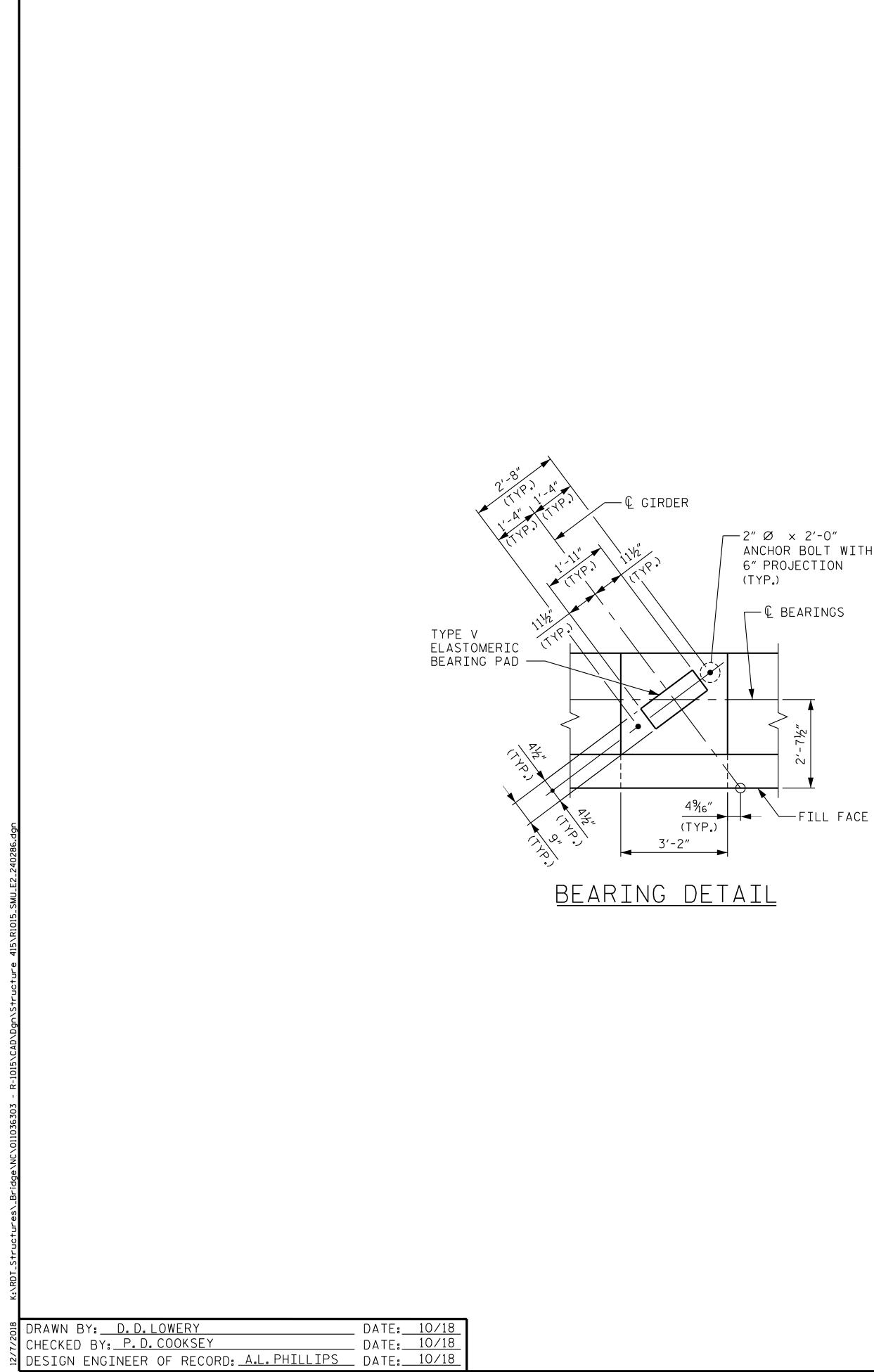
The documents contained herein were originally issued and sealed by the individuals whose names and license numbers appear on each page, on the dates appearing with their signature on that page. This file or an individual page shall not be considered a certified document.

BILL OF MATERIAL							
BAR NO. SIZE TYPE LENGTH WEIGHT	BAR NO. SIZE TYPE LENGTH WEIGHT	BAR NO. SIZE TYPE LENGTH	WEIGHT BAR NO. SIZE	TYPE LENGTH WEIGHT BAR NO.	SIZE TYPE LENGTH WEIGHT	BAR NO. SIZE TYPE LENGTH WEIGHT	
A1E2695STR50'-11"14,286A22695STR50'-11"14,286	A169E 1 5 STR 12'-3" 13 A170E 1 5 STR 11'-8" 12	A243E 1 5 STR 20'-5" A244E 1 5 STR 19'-11"	21A4391521A44015	STR29'-2"30A5131STR28'-7"30A5141	5 STR 36'-10" 38 5 STR 36'-4" 38	A567 1 5 STR 7'-4" 8 A568 1 5 STR 6'-10" 7	
A3E 6 5 STR 3'-3" 20	A171E 1 5 STR 11'-2" 12	A245E 1 5 STR 19'-4"	20 A441 1 5	STR 28'-1" 29 A515 1 STR 27'-6" 29 A516 1	5 STR 36'-9" 38 5 STR 35'-3" 37	A569 1 5 STR 6'-3" 7 A570 1 5 STR 5'-9" 6	
A4 6 5 STR 3'-3" 20 A5E 6 6 STR 6'-0" 54	A173E 1 5 STR 10'-0" 10	A247E 1 5 STR 18'-3"	19 A443 1 5	STR 26'-11" 28 A517 1	5 STR 34'-8" 36	A571 1 5 STR 5'-2" 5	
A101E 1 5 STR 50'-7" 53	A174E 1 5 STR 9'-5" 10 A175E 1 5 STR 8'-10" 9	A248E15STR17'-9"A249E15STR17'-2"	19A4441518A44515	STR26'-4"27A5181STR25'-10"27A5191	5 STR 34'-1" 36 5 STR 33'-7" 35	A572 1 5 STR 4'-8" 5 A573 1 5 STR 4'-1" 4	
A102E 1 5 STR 50'-0" 52 A103E 1 5 STR 49'-5" 52	A176E 1 5 STR 8'-4" 9 A177E 1 5 STR 7'-9" 8	A250E 1 5 STR 16'-7" A251E 1 5 STR 16'-1"	17 A446 1 5 17 A447 1 5	STR25'-3"26A5201STR24'-8"26A5211	5 STR 33'-0" 34 5 STR 32'-6" 34	A574 1 5 STR 3'-6" 4 A575 1 5 STR 3'-0" 3	
A104E 1 5 STR 48'-10" 51	A178E 1 5 STR 7'-2" 7	A252E 1 5 STR 15'-6"	16 A448 1 5	STR 24'-1" 25 A522 1	5 STR 31'-11" 33	A576 1 5 STR 2'-5" 3	
A105E 1 5 STR 48'-4" 50 A106E 1 5 STR 47'-9" 50	A179E 1 5 STR 6'-7" 7 A180E 1 5 STR 6'-1" 6	A253E15STR15'-0"A254E15STR14'-5"	16A4491515A45015	STR23'-7"25A5231STR23'-0"24A5241	5 STR 31'-5" 33 5 STR 30'-10" 32	A577 1 5 STR 2'-0" 2	
A107E 1 5 STR 47'-2" 49 A108E 1 5 STR 46'-7" 49	A181E 1 5 STR 5'-6" 6 A182E 1 5 STR 4'-11" 5	A255E 1 5 STR 13'-11" A256E 1 5 STR 13'-4"	15 A451 1 5 14 A452 1 5	STR22'-5"23A5251STR21'-10"23A5261	5 STR 30'-3" 32 5 STR 29'-9" 31	B1E724STR30'-0"1,443B2E724STR25'-0"1,202	
A109E 1 5 STR 46'-1" 48 A110E 1 5 STR 45'-6" 47	A183E 1 5 STR 4'-4" 5 A184E 1 5 STR 3'-9" 4	A257E 1 5 STR 12'-10" A258E 1 5 STR 12'-3"	13 A453 1 5 13 A454 1 5	STR21'-3"22A5271STR20'-9"22A5281	5 STR 29'-2" 30 5 STR 28'-8" 30	B3E 36 6 STR 54'-6" 2,947 B4E 33 6 STR 35'-0" 1,735	
A111E 1 5 STR 44'-11" 47	A185E 1 5 STR 3'-3" 3	A259E 1 5 STR 11'-9"	12 A455 1 5	STR 20'-2" 21 A529 1	5 STR 28'-1" 29	B4L 55 6 511 55 6 1,155 B5 231 5 STR 51'-9" 12,468	
A112E 1 5 STR 44'-4" 46 A113E 1 5 STR 43'-10" 46	A186E 1 5 STR 2'-8" 3 A187E 1 5 STR 2'-1" 2	A260E 1 5 STR 11'-2" A261E 1 5 STR 10'-7"	12 A456 1 5 11 A457 1 5	STR19'-7"20A5301STR19'-0"20A5311	5 STR 27'-7" 29 5 STR 27'-0" 28	G1E 2 5 STR 32'-9" 68	
A114E 1 5 STR 43'-3" 45 A115E 1 5 STR 42'-8" 45	A188E 1 5 STR 50'-7" 53 A189E 1 5 STR 50'-0" 52	A262E 1 5 STR 10'-1" A263E 1 5 STR 9'-6"	11A4581510A45915	STR18'-6"19A5321STR17'-11"19A5331	5 STR 26'-6" 28 5 STR 25'-11" 27	G2E 2 5 STR 33'-8" 70	
A116E 1 5 STR 42'-1" 44 A117E 1 5 STR 41'-7" 43	A190E 1 5 STR 49'-6" 52 A191E 1 5 STR 48'-11" 51	A264E 1 5 STR 9'-0" A265E 1 5 STR 8'-5"	9 A460 1 5 9 A461 1 5	STR 17'-4" 18 A534 1 STR 16'-9" 17 A535 1	5 STR 25'-4" 26 5 STR 24'-10" 26	J1E 122 4 9 1'-5" 115	
A118E 1 5 STR 41'-0" 43	A192E 1 5 STR 48'-4" 50	A266E 1 5 STR 7'-11"	8 A462 1 5	STR 16'-3" 17 A536 1	5 STR 24'-3" 25	K1E 8 8 1 15'-0" 320 K2E 16 2 25'/2" 1.075	
A119E 1 5 STR 40'-5" 42 A120E 1 5 STR 39'-10" 42	A193E 1 5 STR 47'-10" 50 A194E 1 5 STR 47'-3" 49	A267E 1 5 STR 7'-4" A268E 1 5 STR 6'-10"	8 A463 1 5 7 A464 1 5	STR15'-8"16A5371STR15'-1"16A5381	5 STR 23'-9" 25 5 STR 23'-2" 24	K2E 16 8 2 25'-2" 1,075 K3E 30 6 STR 9'-0" 406	
A121E 1 5 STR 39'-4" 41 A122E 1 5 STR 38'-9" 40	A195E 1 5 STR 46'-9" 49 A196E 1 5 STR 46'-2" 48	A269E 1 5 STR 6'-3" A270E 1 5 STR 5'-9"	7 A465 1 5 6 A466 1 5	STR14'-6"15A5391STR13'-11"15A5401	5 STR 22'-8" 24 5 STR 22'-1" 23	K4104STR7'-4"49K5204STR9'-5"126	
A123E 1 5 STR 38'-2" 40 A124E 1 5 STR 37'-7" 39	A197E 1 5 STR 45'-7" 48 A198E 1 5 STR 45'-1" 47	A271E 1 5 STR 5'-2" A272E 1 5 STR 4'-8"	5 A467 1 5 5 A468 1 5	STR 13'-5" 14 A541 1 STR 12'-10" 13 A542 1	5 STR 21'-6" 22 5 STR 21'-0" 22	K6 30 4 STR 10'-3" 205 K7 12 4 7 7'-3" 58	
A125E 1 5 STR 37'-1" 39	A199E 1 5 STR 44'-6" 46	A273E 1 5 STR 4'-1"	4 A469 1 5	STR 12'-3" 13 A543 1	5 STR 20'-5" 21	K1 12 1 13 30 K8 24 4 8 14'-2" 227	
A126E 1 5 STR 36'-6" 38 A127E 1 5 STR 35'-11" 37	A200E 1 5 STR 44'-0" 46 A201E 1 5 STR 43'-5" 45	A274E 1 5 STR 3'-6" A275E 1 5 STR 3'-0"	4 A470 1 5 3 A471 1 5	STR11'-8"12A5441STR11'-2"12A5451	5 STR 19'-11" 21 5 STR 19'-4" 20	S1E 40 4 5 6'-0" 160	
A128E 1 5 STR 35'-4" 37 A129E 1 5 STR 34'-10" 36	A202E 1 5 STR 42'-11" 45 A203E 1 5 STR 42'-4" 44	A276E 1 5 STR 2'-5" A277E 1 5 STR 2'-0"	3 A472 1 5 2 A473 1 5	STR10'-7"11A5461STR10'-0"10A5471	5 STR 18'-10" 20 5 STR 18'-3" 19	S2E 40 5 3 5'-9" 240 S3 200 4 4 3'-2" 423	
A130E 1 5 STR 34'-3" 36 A131E 1 5 STR 33'-8" 35	A204E 1 5 STR 41'-9" 44 A205E 1 5 STR 41'-3" 43	A401 1 5 STR 50'-7"	A474 1 5 53 A475 1 5	STR9'-5"10A5481STR8'-10"9A5491	5 STR 17'-9" 19 5 STR 17'-2" 18	S4E 80 4 6 5'-4" 285	
A132E 1 5 STR 33'-1" 35	A206E 1 5 STR 40'-8" 42	A402 1 5 STR 50'-0"	52 A476 1 5	STR 8'-4" 9 A550 1	5 STR 16'-7" 17	U1 40 4 5 9'-9" 261	
A133E 1 5 STR 32'-7" 34 A134E 1 5 STR 32'-0" 33	A207E 1 5 STR 40'-2" 42 A208E 1 5 STR 39'-7" 41	A403 1 5 STR 49'-5" A404 1 5 STR 48'-10"	51 A478 1 5	STR7'-9"8A5511STR7'-2"7A5521	5 STR 16'-1" 17 5 STR 15'-6" 16		
A135E 1 5 STR 31'-5" 33 A136E 1 5 STR 30'-10" 32	A209E 1 5 STR 39'-1" 41 A210E 1 5 STR 38'-6" 40	A405 1 5 STR 48'-4" A406 1 5 STR 47'-9"	50 A479 1 5 50 A480 1 5	STR 6'-7" 7 A553 1 STR 6'-1" 6 A554 1		EPOXY COATED REINFORCING STEEL 29,278 LBS.	
A137E15STR30'-4"32A138E15STR29'-9"31	A211E 1 5 STR 37'-11" 40 A212E 1 5 STR 37'-5" 39	A407 1 5 STR 47'-2" A408 1 5 STR 46'-7"	49A4811549A48215	STR 5'-6" 6 A555 1 STR 4'-11" 5 A556 1	5 STR 13'-11" 15 I 5 STR 13'-4" 14	REINFORCING STEEL 32,975 LBS.	
A139E 1 5 STR 29'-2" 30	A213E 1 5 STR 36'-10" 38	A409 1 5 STR 46'-1"	48 A483 1 5	STR 4'-4" 5 A557 1	5 STR 12'-10" 13		
A140E 1 5 STR 28'-7" 30 A141E 1 5 STR 28'-1" 29	A215E 1 5 STR 36'-9" 38	A410 1 5 STR 45'-6" A411 1 5 STR 44'-11"		STR 3'-3" 3 A559 1	5 STR 12'-3" 13 5 STR 11'-9" 12		
A142E 1 5 STR 27'-6" 29 § A143E 1 5 STR 26'-11" 28	A216E 1 5 STR 35'-3" 37 A217E 1 5 STR 34'-8" 36	A412 1 5 STR 44'-4" A413 1 5 STR 43'-10"	46A4861546A48715	STR 2'-8" 3 A560 1 STR 2'-1" 2 A561 1	5 STR 11'-2" 12 5 STR 10'-7" 11		
A144E 1 5 STR 26'-4" 27 A145E 1 5 STR 25'-10" 27	A218E 1 5 STR 34'-1" 36 A219E 1 5 STR 33'-7" 35	A414 1 5 STR 43'-3" A415 1 5 STR 42'-8"	45A4881545A48915	STR50'-7"53A5621STR50'-0"52A5631	5 STR 10'-1" 11 5 STR 9'-6" 10		
A146E 1 5 STR 25'-3" 26	A220E 1 5 STR 33'-0" 34	A416 1 5 STR 42'-1"	44 A490 1 5	STR 49'-6" 52 A564 1	5 STR 9'-0" 9		
A148E 1 5 STR 24'-1" 25	A222E 1 5 STR 31'-11" 33	A418 1 5 STR 41'-0"	43 A491 1 5 43 A492 1 5	STR 48'-4" 50 A566 1	5 STR 8'-5" 9 5 STR 7'-11" 8		
A149E 1 5 STR 23'-7" 25 A150E 1 5 STR 23'-0" 24	A223E 1 5 STR 31'-5" 33 A224E 1 5 STR 30'-10" 32	A419 1 5 STR 40'-5" A420 1 5 STR 39'-10"	42A4931542A49415	STR47'-10"50STR47'-3"49			
A151E 1 5 STR 22'-5" 23 A152E 1 5 STR 21'-10" 23	A225E 1 5 STR 30'-3" 32 A226E 1 5 STR 29'-9" 31	A421 1 5 STR 39'-4" A422 1 5 STR 38'-9"	41A4951540A49615	STR46'-9"49STR46'-2"48			
A153E 1 5 STR 21'-3" 22 A154E 1 5 STR 20'-9" 22	A227E 1 5 STR 29'-2" 30 A228E 1 5 STR 28'-8" 30	A423 1 5 STR 38'-2" A424 1 5 STR 37'-7"	40 A497 1 5 39 A498 1 5	STR 45'-7" 48 STR 45'-1" 47		PROJECT NO. <u>R-1015</u>	
6 A155E 1 5 STR 20'-2" 21	A229E 1 5 STR 28'-1" 29	A425 1 5 STR 37'-1"	39 A499 1 5	STR 44'-6" 46		<u>CRAVEN</u> COUNTY	
A156E15STR19'-7"20A157E15STR19'-0"20	A230E 1 5 STR 27'-7" 29 A231E 1 5 STR 27'-0" 28	A426 1 5 STR 36'-6" A427 1 5 STR 35'-11"		STR44'-0"46STR43'-5"45		STATION: 516+87.37 -L-	
A158E 1 5 STR 18'-6" 19 A159E 1 5 STR 17'-11" 19	A232E 1 5 STR 26'-6" 28 A233E 1 5 STR 25'-11" 27	A42815STR35'-4"A42915STR34'-10"	37A5021536A50315	STR42'-11"45STR42'-4"44		STATION: STOPOLOST L	
A160E 1 5 STR 17'-4" 18 Mail A161E 1 5 STR 16'-9" 17	A234E 1 5 STR 25'-4" 26 A235E 1 5 STR 24'-10" 26	A430 1 5 STR 34'-3" A431 1 5 STR 33'-8"	36 A504 1 5 35 A505 1 5	STR 41'-9" 44 STR 41'-3" 43		SHEET 3 OF 3	
A162E 1 5 STR 16'-3" 17	A236E 1 5 STR 24'-3" 25	A432 1 5 STR 33'-1"	35 A506 1 5	STR 40'-8" 42	ROFESSION 2	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION	
A163E 1 5 STR 15'-8" 16 A164E 1 5 STR 15'-1" 16	A237E 1 5 STR 23'-9" 25 A238E 1 5 STR 23'-2" 24	A433 1 5 STR 32'-7" A434 1 5 STR 32'-0"	34 A507 1 5 33 A508 1 5	STR 40'-2" 42 STR 39'-7" 41	SEAL 040769	RALEIGH	
A165E 1 5 STR 14'-6" 15 A166E 1 5 STR 13'-11" 15	A239E 1 5 STR 22'-8" 24 A240E 1 5 STR 22'-1" 23	A435 1 5 STR 31'-5" A436 1 5 STR 30'-10"	33A5091532A51015	STR 39'-1" 41 STR 38'-6" 40	S S S S S S S S S S S S S S S S S S S	SUPERSTRUCTURE	
AIGOL I S STR IS IS AIG7E 1 5 STR 13'-5" 14 AIG8E 1 5 STR 12'-10" 13	A241E 1 5 STR 21'-6" 22 A242E 1 5 STR 21'-0" 22	A437 1 5 STR 30'-4" A438 1 5 STR 29'-9"	32 A510 1 5 32 A511 1 5 31 A512 1 5	STR 37'-11" 40 STR 37'-5" 39	Andrew L Phillips 12/7/2018	BILL OF MATERIAL	
""""""""""""""""""""""""""""""""""""""					Vimlou Uorn	DILL UI WAICALAL	
					Kimley » Horn	LEFT LANE	
					421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102	REVISIONS SHEET NO.	
CHECKED BY: <u>P.D.COOKSEY</u> D	ATE: <u>10/18</u> ATE: <u>10/18</u>			DOCUMENT NOT CONSIDERED FINAL	This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without written authorization and adaption by Kimley-Horn and Associates, Inc. shall be		
DESIGN ENGINEER OF RECORD: <u>A.L. PHILLIPS</u> D	ATE: 10/18			UNLESS ALL SIGNATURES COMPLETED	without liability to Kimley-Horn and Associates, Inc. Copyright Kimley-Horn and Associates, Inc., 2018	2	
						STRUCTURE 15	









Ø	× 2'-	-0″	
ICHO	r bol	T	WITH
PRO	JECT	ION	1
YP.)			

ТОР	OF CAP	ELEVA	TIONS
A	50.43′	H	52.22′
B	50.69′	I	52.36′
C	50.83′	U	52.73′
D	51.20′	K	52.87′
E	51.34′	Ĺ	53.24′
F	51.71′	M	53.38′
G	51.85′	N	53.54′

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

This document, tr instrument of ser which it was pre-written authorizat without liability to Copyright Kimley-

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

FOR PILE SPLICE DETAILS, SEE ``14"STEEL PIPE PILE'' SHEET.

BACKWALL SHALL BE PLACED BEFORE APPLYING THE PROTECTIVE COATING.

THE TOP SURFACE AREAS OF THE END BENT CAP SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THAT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.

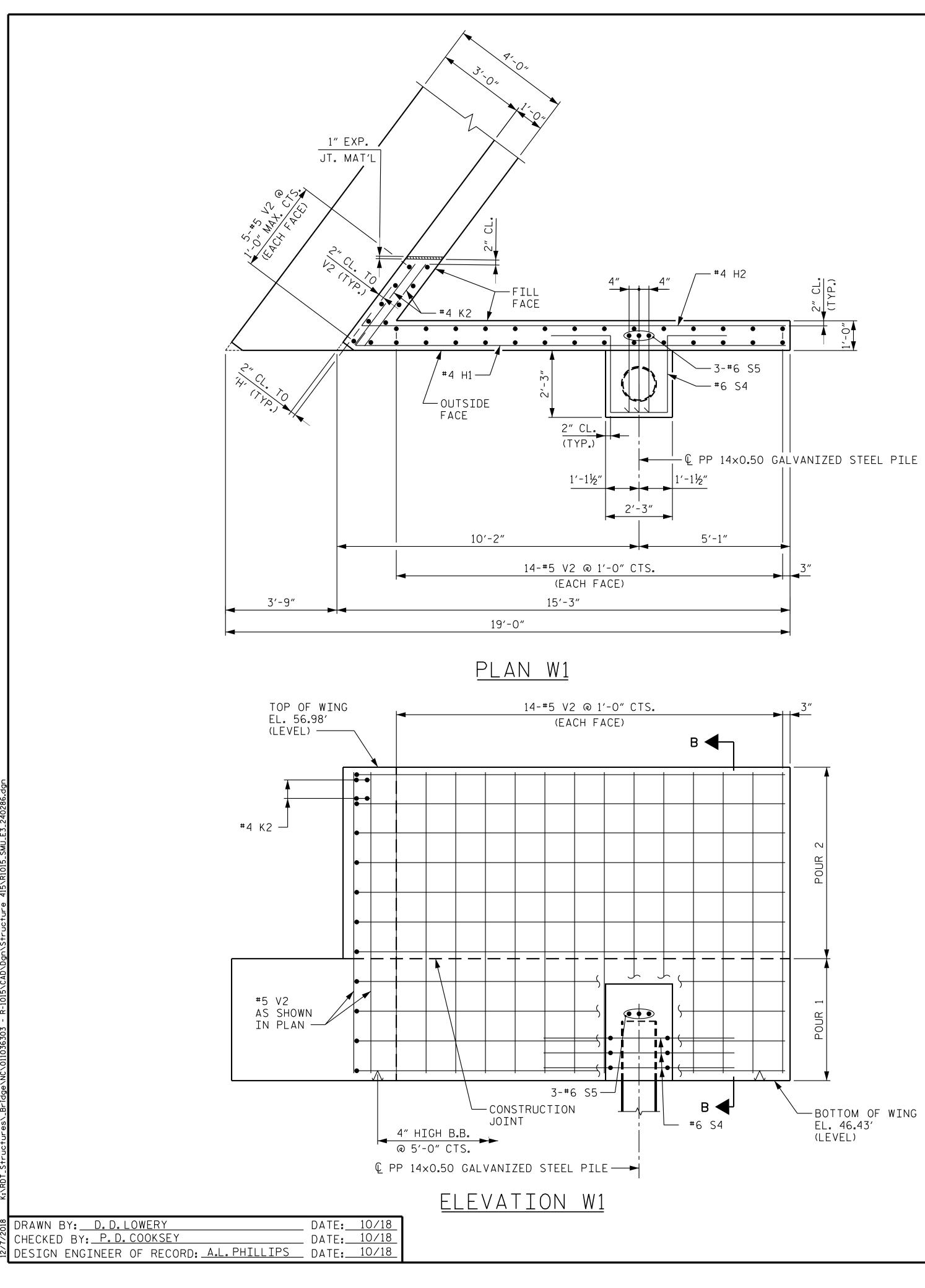
THE TOP SURFACE OF THE CAP EXCEPT THE BRIDGE SEAT BUILDUPS SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE BACK FACE AT THE RATE OF 2%.

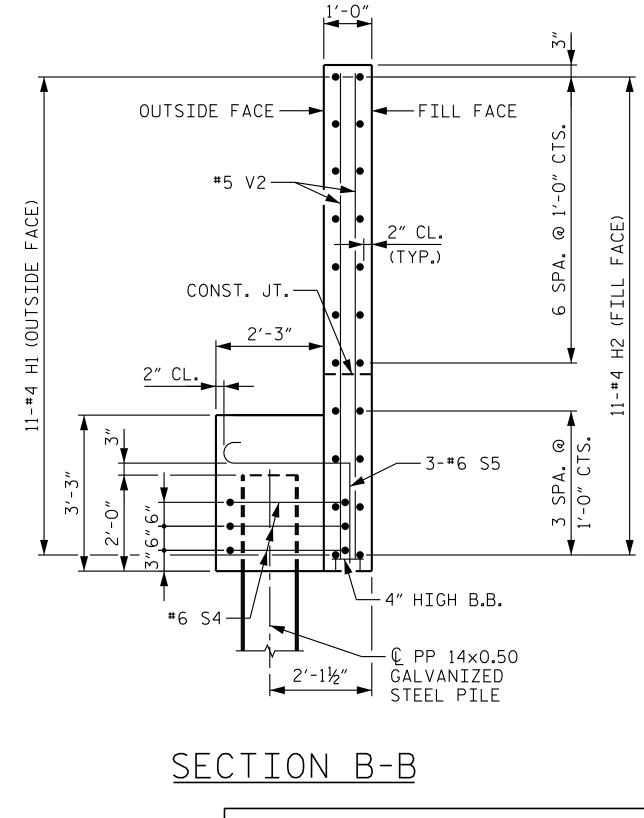
THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL ARE CAST IF SLIP FORMING IS USED.

FOR ``27"Ø CSP CASING DETAIL' SEE ``GENERAL DRAWING' SHEET 2 OF 4.

TOP OF PILE	ELEVATIONS
PILE NO.	ELEVATION
1	48.78′
2	49.29′
3	49.80′
4	50.30′
5	50.81′
6	51.32′

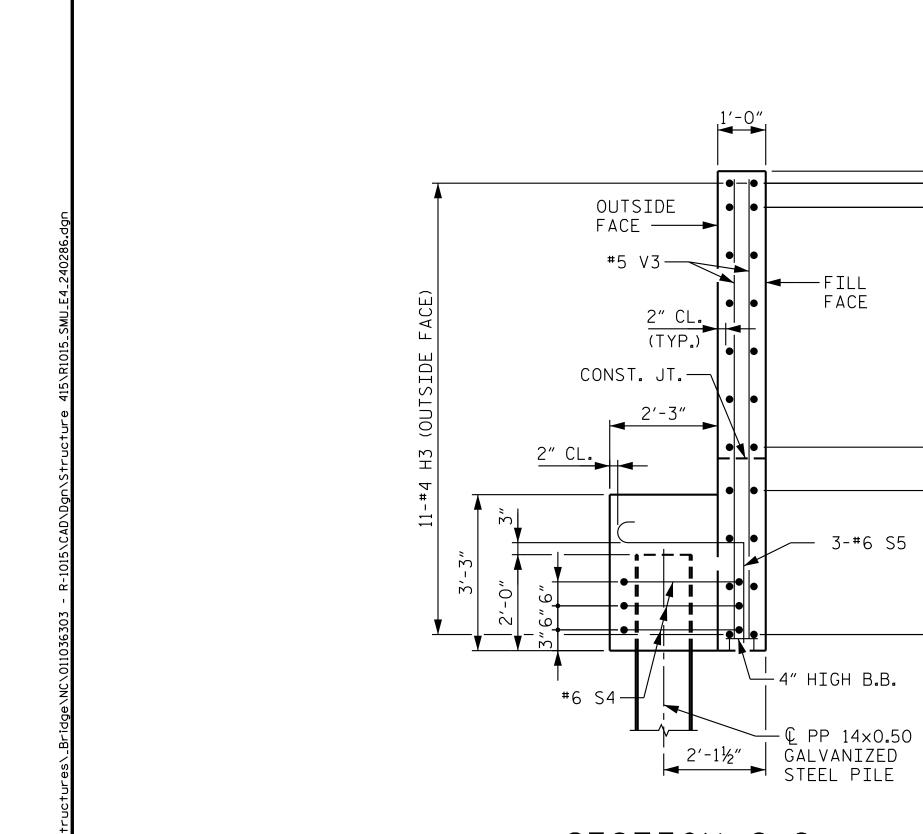
	PROJECT NO. <u>R-1015</u> <u>CRAVEN</u> COUNTY
	STATION: <u>516+87.37</u> -L-
	SHEET 2 OF 5
Docusigned by:	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE
DocuSigned by: 11/1/11/11/11/11/11/11/11/11/11/11/11/1	END BENT 1
Kimley »Horn	DETAILS
-	LEFT LANE
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: S15-29
cument, together with the concepts and designs presented herein, as an ent of services, is intended only for the specific purpose and client for t was prepared. Reuse of and improper reliance of this document without authorization and adoption by Kimley-Horn and Associates, inc. shall be liability to Kimley-Horn and Associates, Inc.	No.DATE:NO.DATE:DATE:DIODIO13TOTAL SHEETS2444
ht Kimley-Horn and Associates, Inc., 2018	





DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT NO. R-1015 CRAVEN COUNTY STATION: 516+87.37 -L-SHEET 3 OF 5 TH CARC STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SEAL 040769 RALEIGH SUBSTRUCTURE END BENT 1 andrew L Phillips 12/7/2018 SECTIONS AND DETAILS **Kimley Worn** LEFT LANE 421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102 REVISIONS SHEET NO. S15-30 NO. BY: DATE: DATE: O. BY: This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc. TOTAL SHEETS 44 Copyright Kimley-Horn and Associates, Inc., 2018



<u>Section C-C</u>

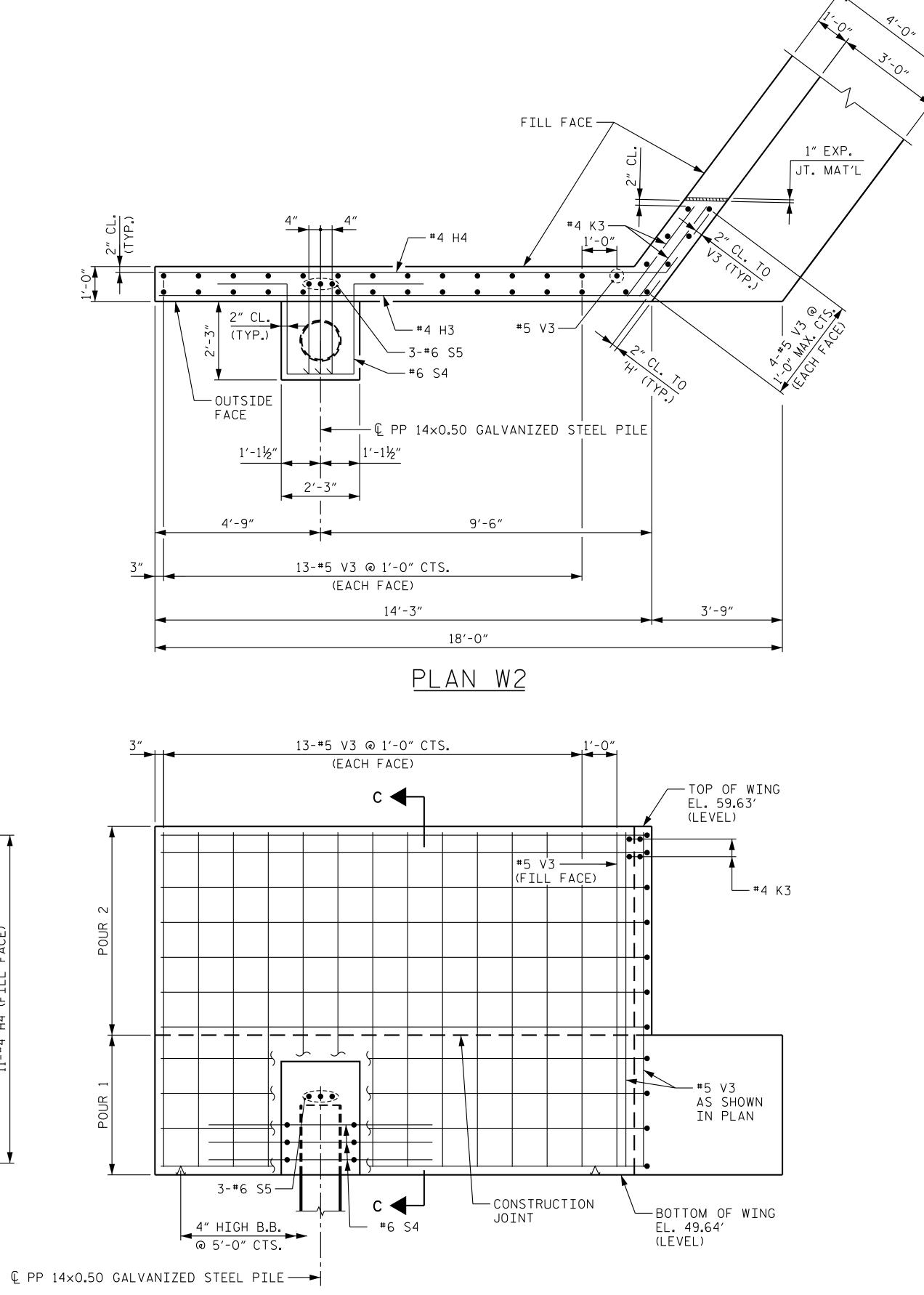
5 SPA. @ 1'-0" CTS. L FACE)

3 SPA. @ 1'-0" CTS.

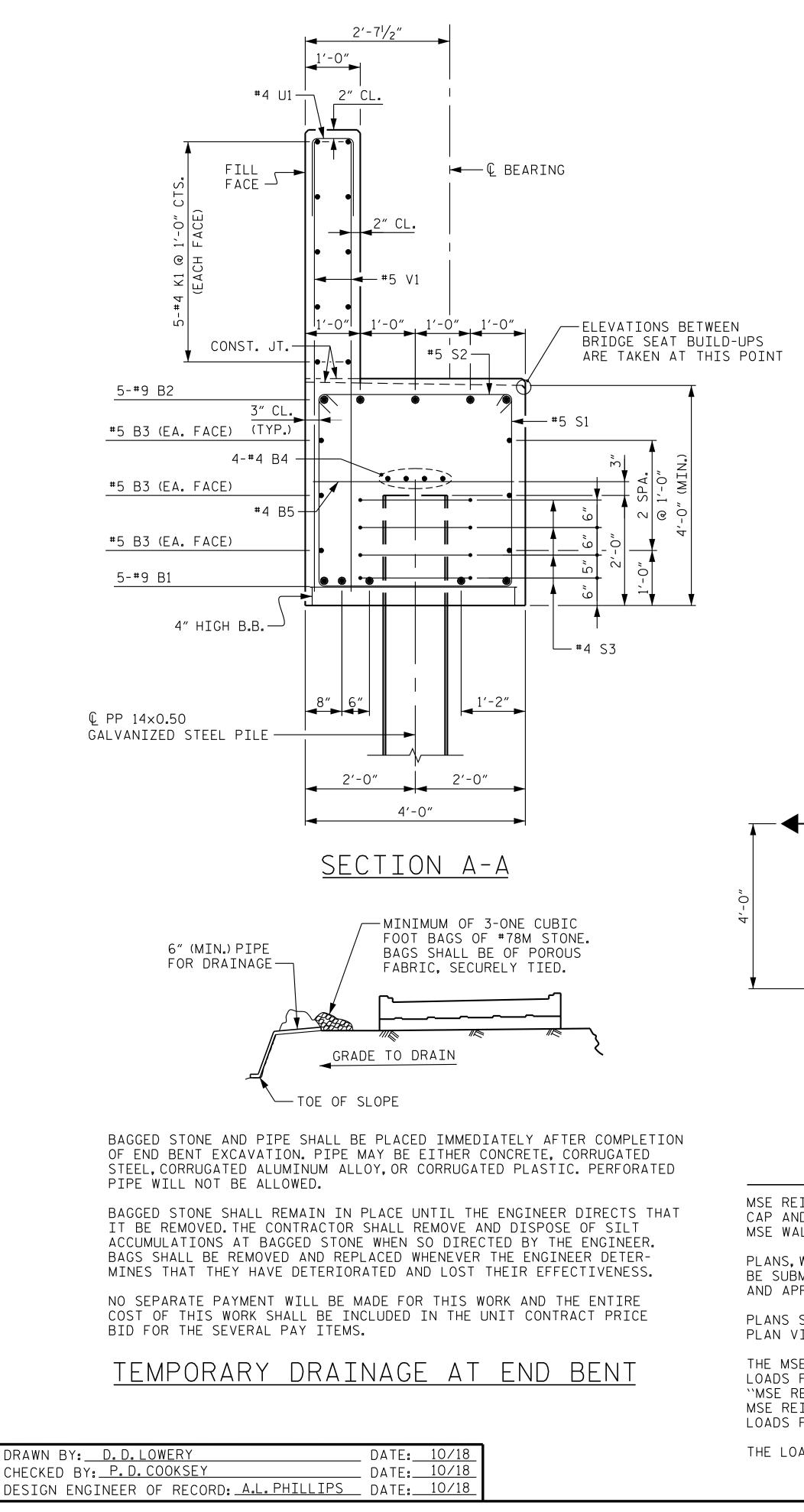
018	DRAWN BY: <u>D.D.LOWERY</u>	DATE:	10/18
7/2	CHECKED BY: P.D.COOKSEY	DATE:	10/18
127	DESIGN ENGINEER OF RECORD: <u>A.L. PHILLIPS</u>	DATE:	10/18

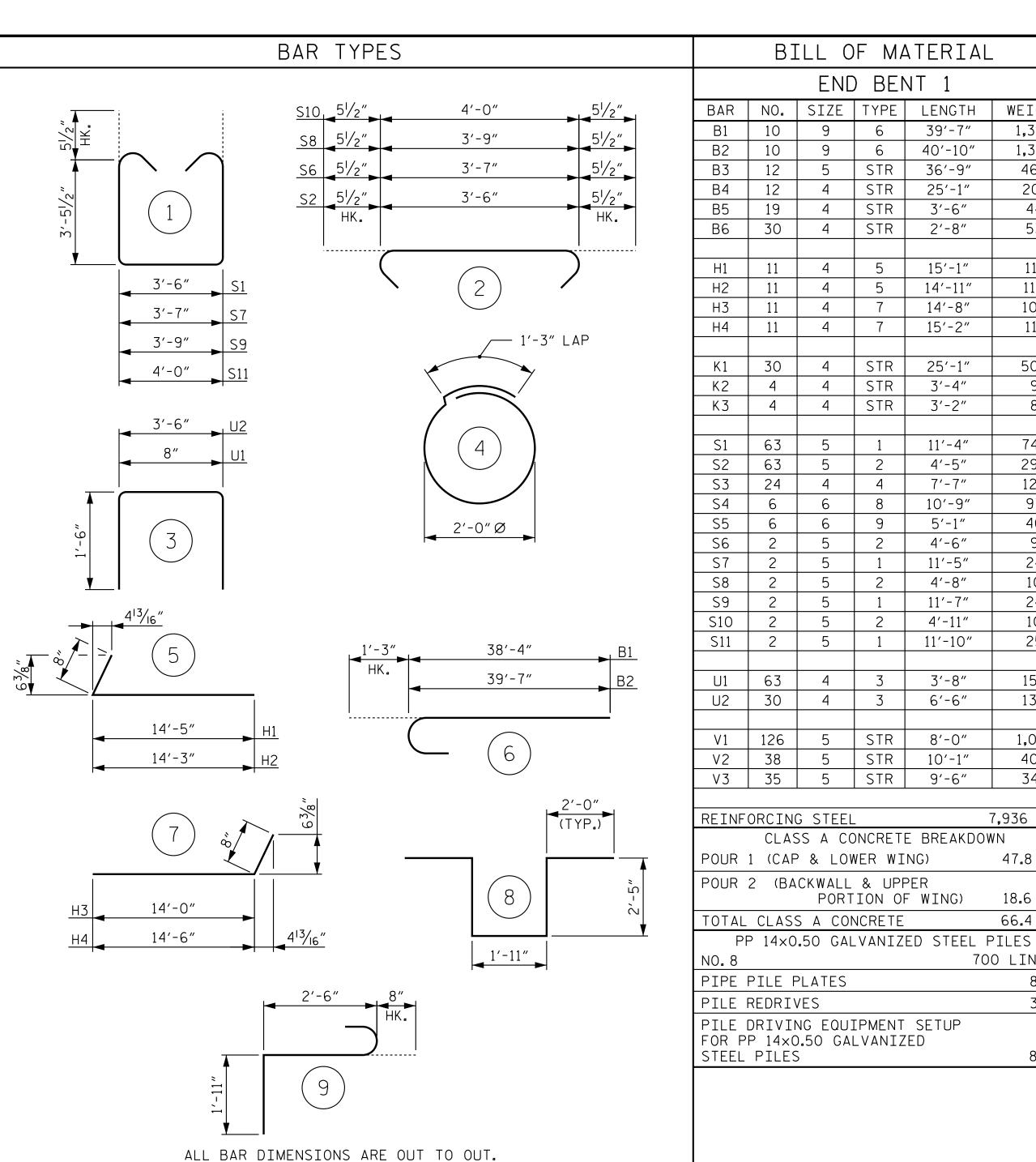


<u>Elevation W2</u>



PROJECT NO. R-1015 CRAVEN COUNTY STATION: 516+87.37 -L-SHEET 4 OF 5 TH CAR STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SEAL 040769 SUBSTRUCTURE END BENT 1 andrew L Phillips 12/7/2018 SECTIONS AND DETAILS **Kimley Worn** LEFT LANE 421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102 REVISIONS SHEET NO. S15-31 NO. BY: DATE: DATE: BY: This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc. total sheets 44 Copyright Kimley—Horn and Associates, Inc., 2018





2.7 K/FT

MSE REINFORCING STRAP LOAD DETAIL

MSE REINFORCING STRAP NOTES

MSE REINFORCING STRAPS SHALL BE ATTACHED TO THE END BENT CAP AND/OR BACKWALL. FOR DESIGN CRITERIA AND DETAILS, SEE MSE WALL SHEETS AND SPECIAL PROVISIONS.

PLANS, WORKING DRAWINGS, AND DESIGN CALCULATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER FOR REVIEW AND APPROVAL, SEE SPECIAL PROVISIONS.

PLANS SUBMITTED FOR REVIEW SHALL INCLUDE THE FOLLOWING: PLAN VIEW, ELEVATION VIEW, TYPICAL SECTIONS, AND STRAP DETAILS.

THE MSE REINFORCING STRAPS SHALL BE DESIGNED TO CARRY THE LOADS FROM THE BRIDGE SUPERSTRUCTURE AS INDICATED IN THE "MSE REINFORCING STRAP LOAD DETAIL". IN ADDITION, THE MSE REINFORCING STRAPS SHALL ALSO BE DESIGNED TO CARRY LOADS FROM SOIL PRESSURE AS OUTLINED IN THE SPECIAL PROVISION.

THE LOADS IN THE DETAIL ABOVE ARE FACTORED LOADS.

	PROJECT NO. <u>R-1015</u> <u>CRAVEN</u> county Station: <u>516+87.37</u> -L-
	SHEET 5 OF 5
DocuSigned by:	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE
DocuSigned by: Andrew L. Phillips 12/7/2018 2BB69ABAD4004D3	END BENT 1 Sections and details
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772	LEFT LANE
Raleigh, NC 27601-1772 NC LICENSE # Phone (919) 677-2000 F-0102	REVISIONS SHEET NO.
FIIOTIE (919) 077-2000 F-0102 incument, together, with the concepts and designs presented herein, as an tent of services, is intended only for the specific purpose and client for t was prepared. Reuse of and improper reliance of this document without authorization and adaption by Kimley-Horn and Associates, Inc. shall be liability to Kimley-Horn and Associates, Inc. ht Kimley-Horn and Associates, Inc., 2018	NO.BY:DATE:S15-3213TOTAL SHEETS24
	STRUCTURE 15

WEIGHT

1,346

1,388

460

201

44

53

111

110

108

111

503

9

8

745

290

122

97

46

9

24

10

24

10

25

154

130

1,051

400

347

7,936 LBS

47.8 C.Y

18.6 C.Y

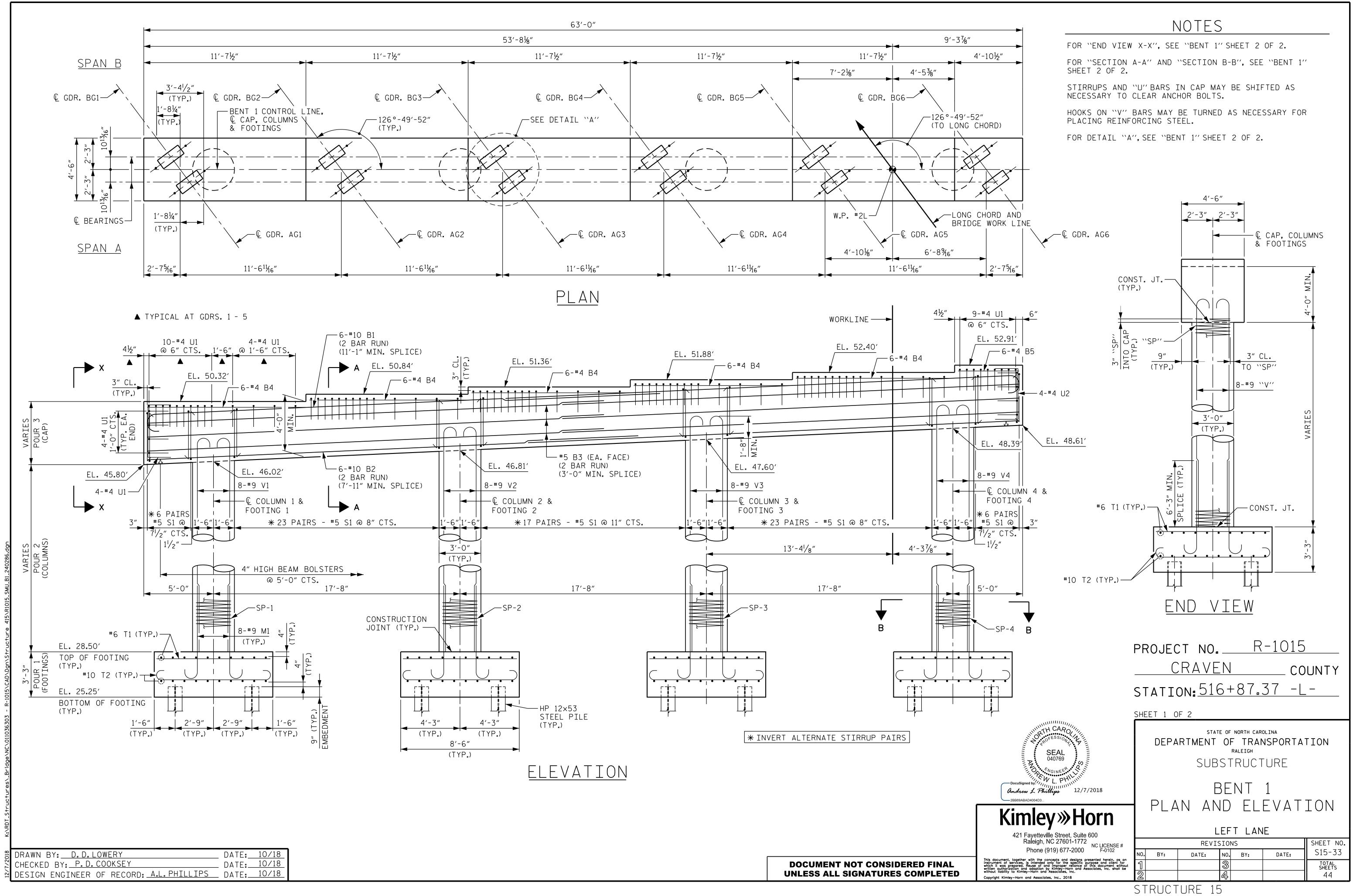
66.4 C.Y

8 EA.

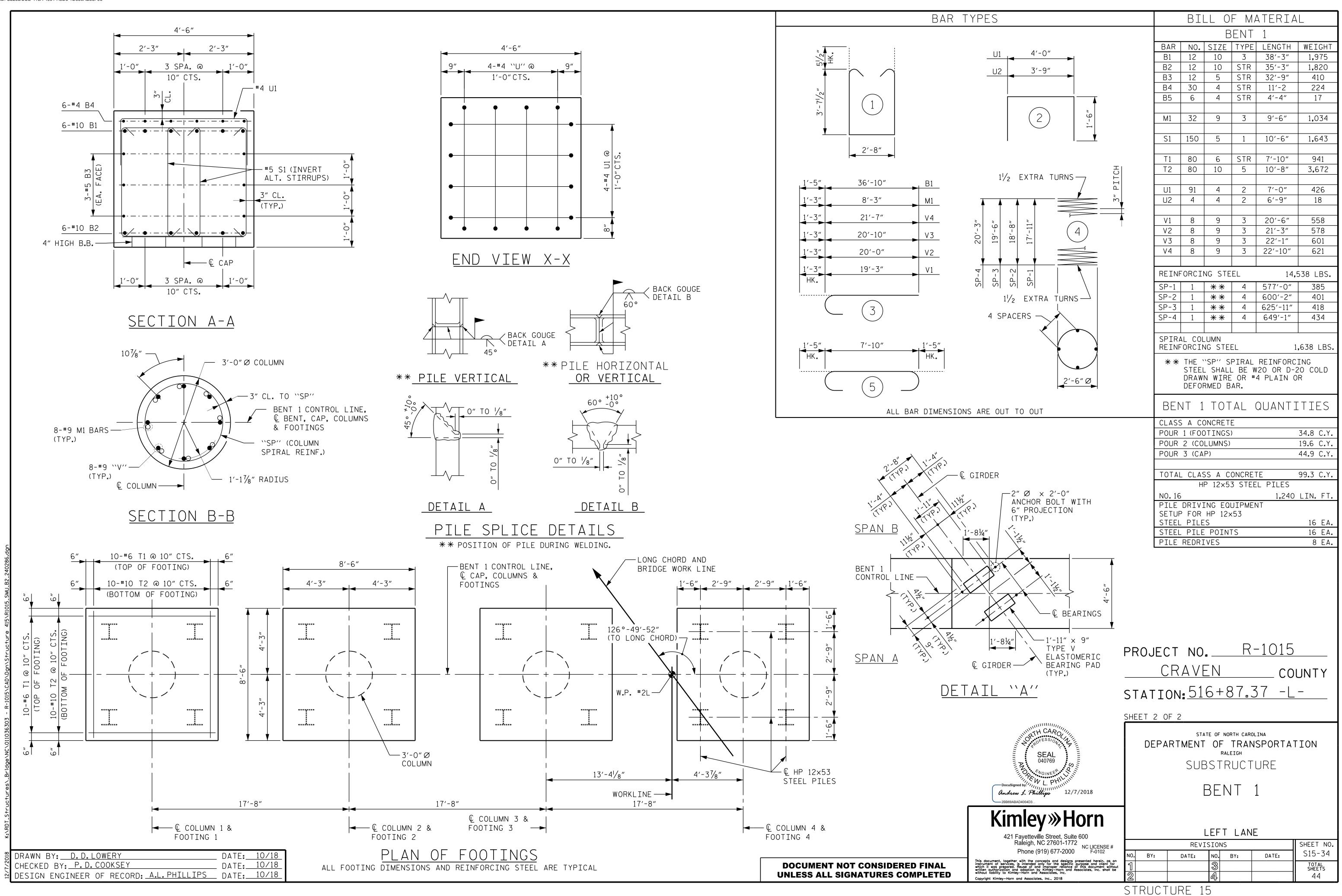
3 EA.

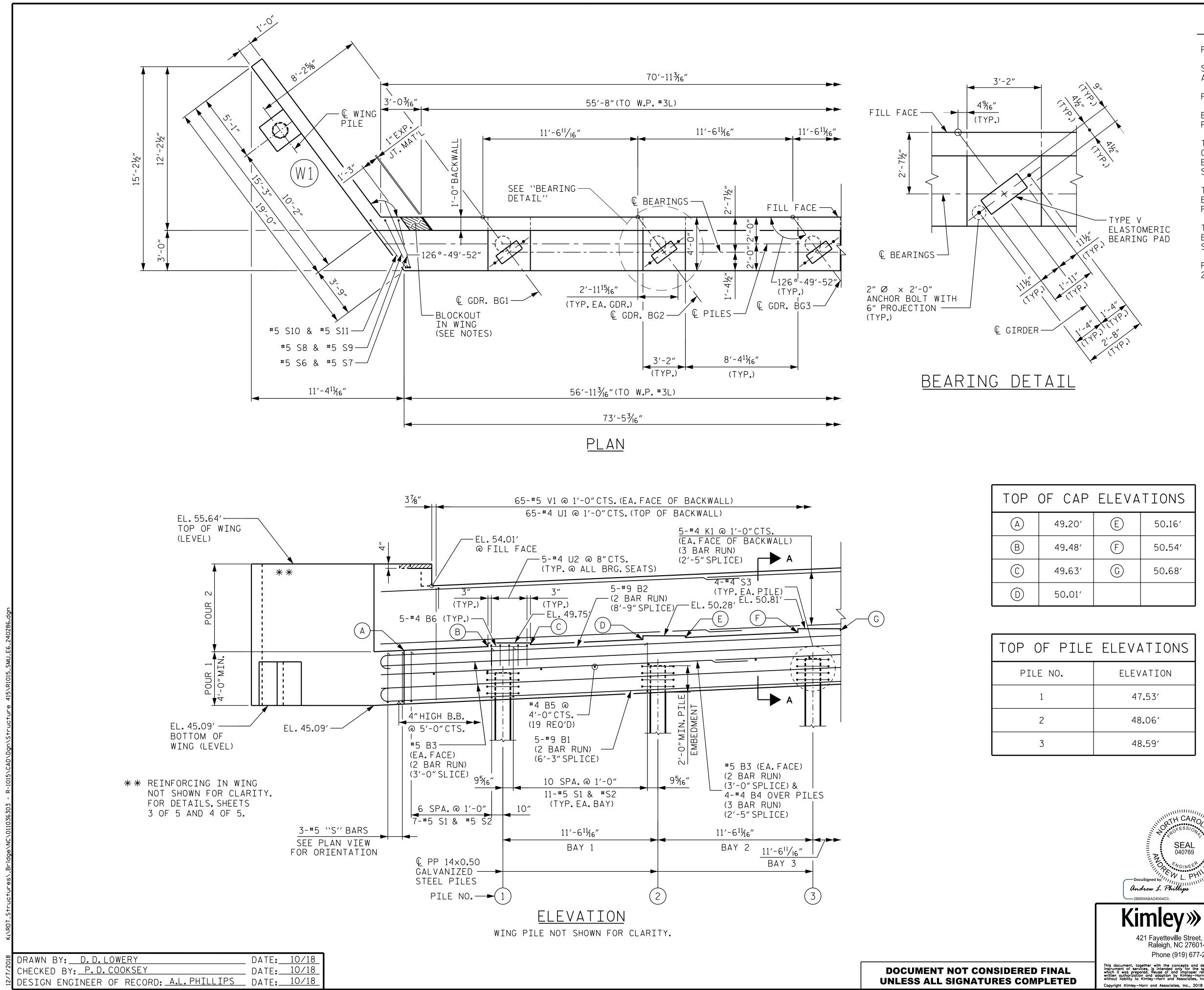
8 E A

700 LIN.FT.









NOTES

FOR ``SECTION A-A'', SEE ``END BENT 2" SHEET 5 OF 5.

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

FOR PILE SPLICE DETAILS, SEE ``14" STEEL PIPE PILE'' SHEET.

BACKWALL SHALL BE PLACED BEFORE APPLYING THE PROTECTIVE COATING.

THE TOP SURFACE AREAS OF THE END BENT CAP SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THAT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.

THE TOP SURFACE OF THE CAP EXCEPT THE BRIDGE SEAT BUILDUPS SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE BACK FACE AT THE RATE OF 2%.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE PARAPET AND END POST ARE CAST IF SLIP FORMING IS USED.

FOR ``27"Ø CSP CASING DETAIL'' SEE ``GENERAL DRAWING'' SHEET 2 OF 4.



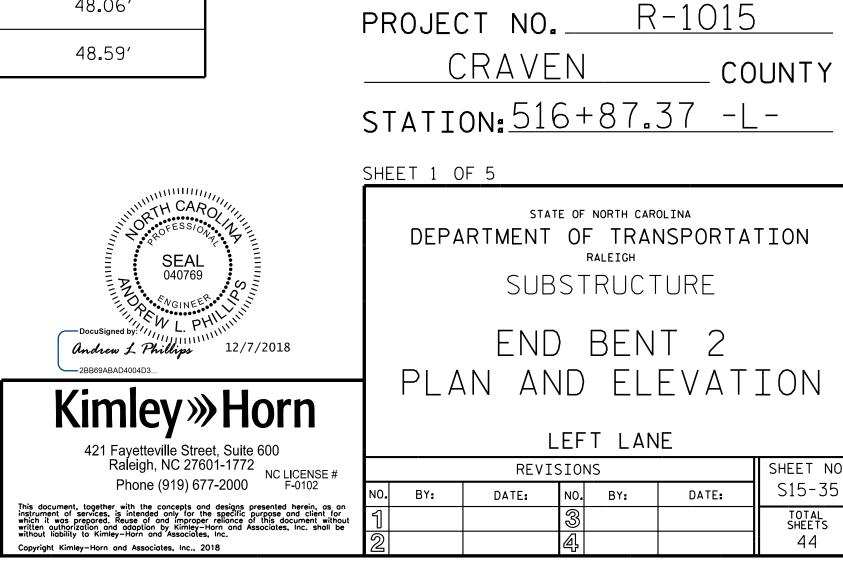
EVATIONS			
Ē	50.16′		
F	50.54′		
G	50.68′		

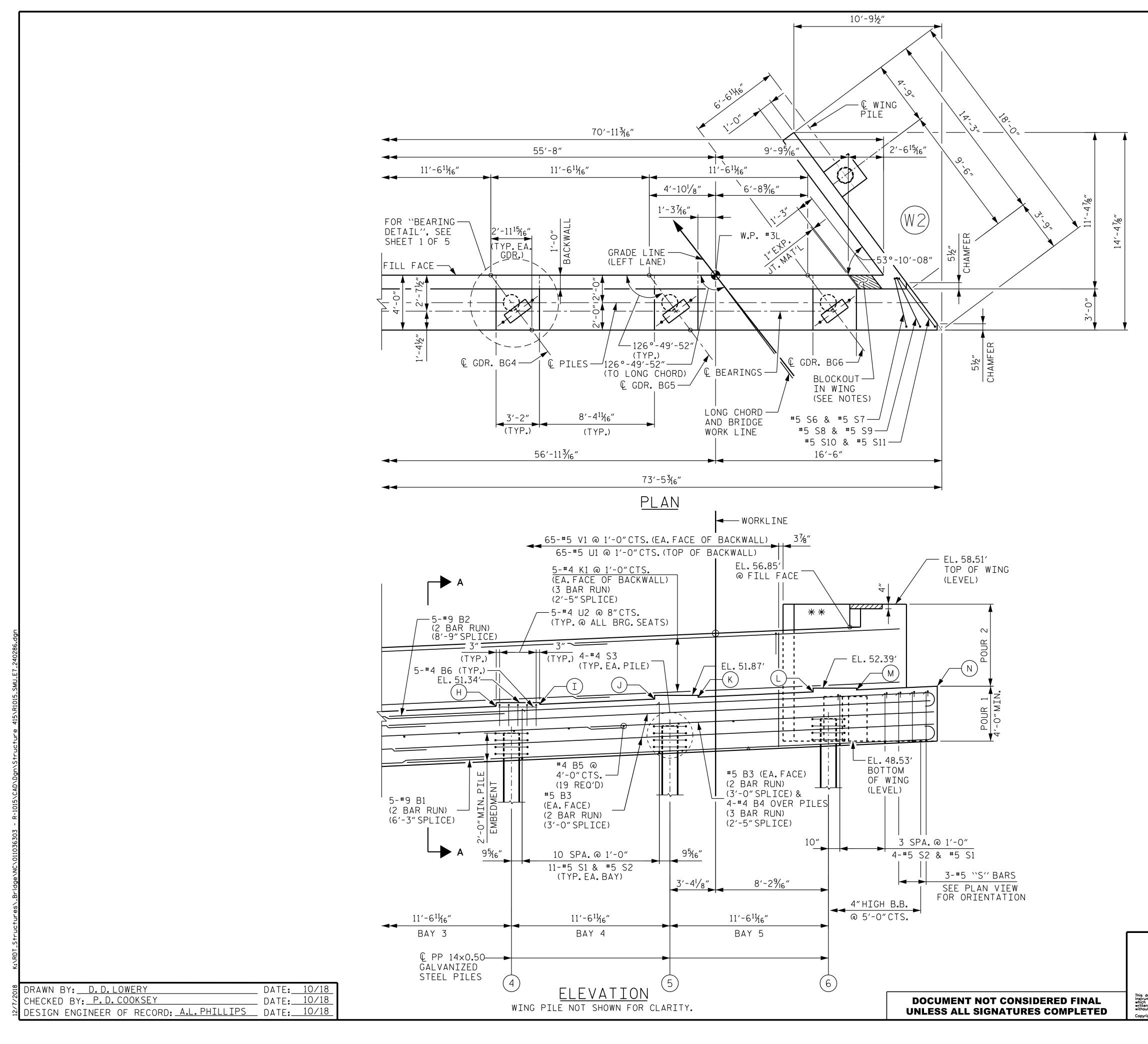
EVATIONS
ELEVATION
47.53′
48.06′
48.59′

TH CAR

SEAL 040769

Andrew L Phillips



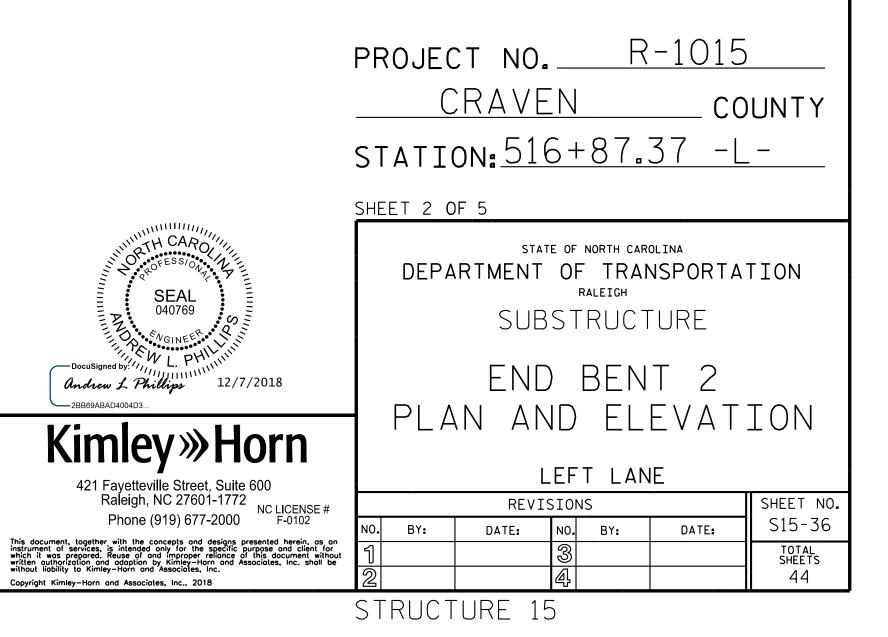


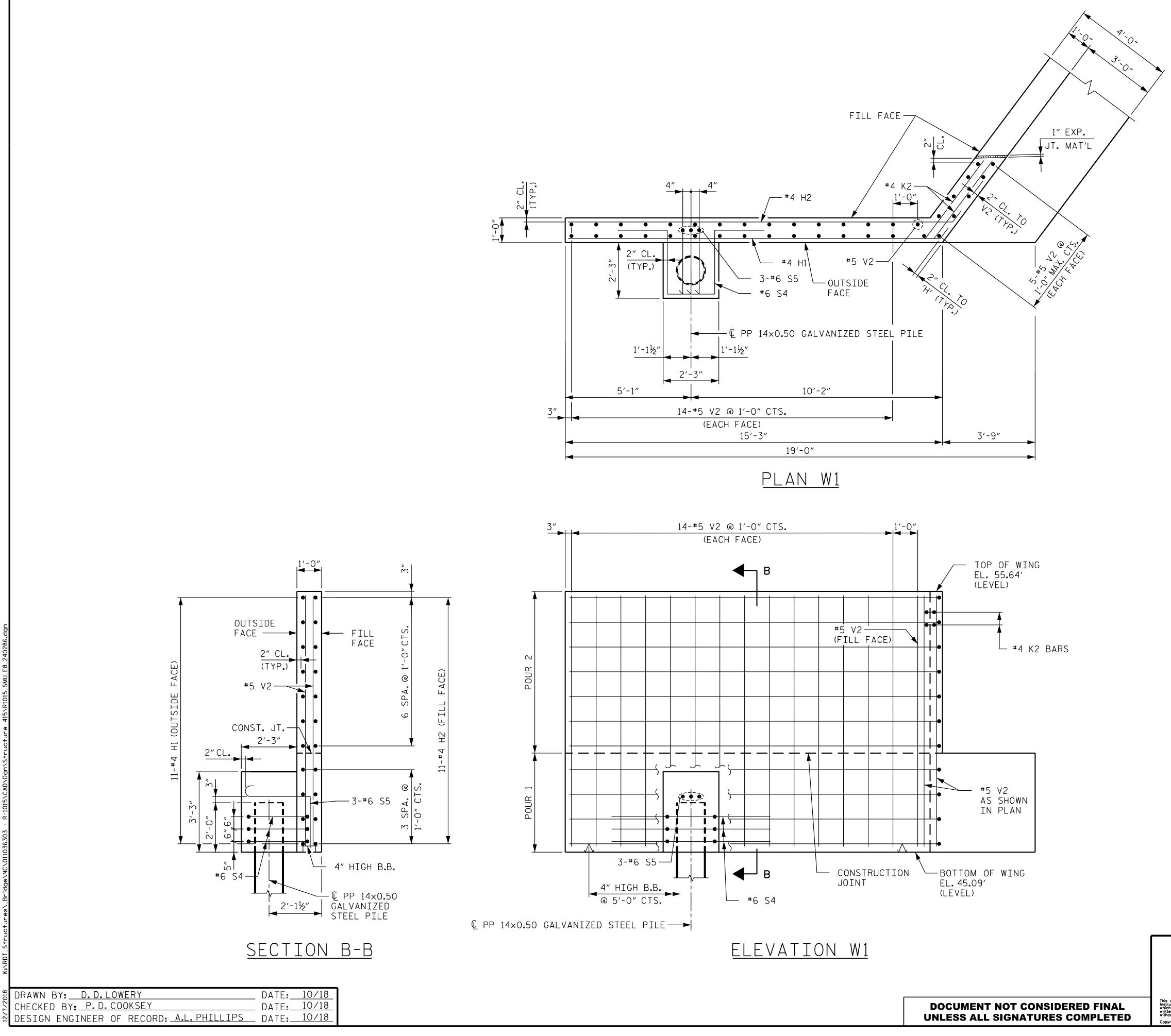
NOTES

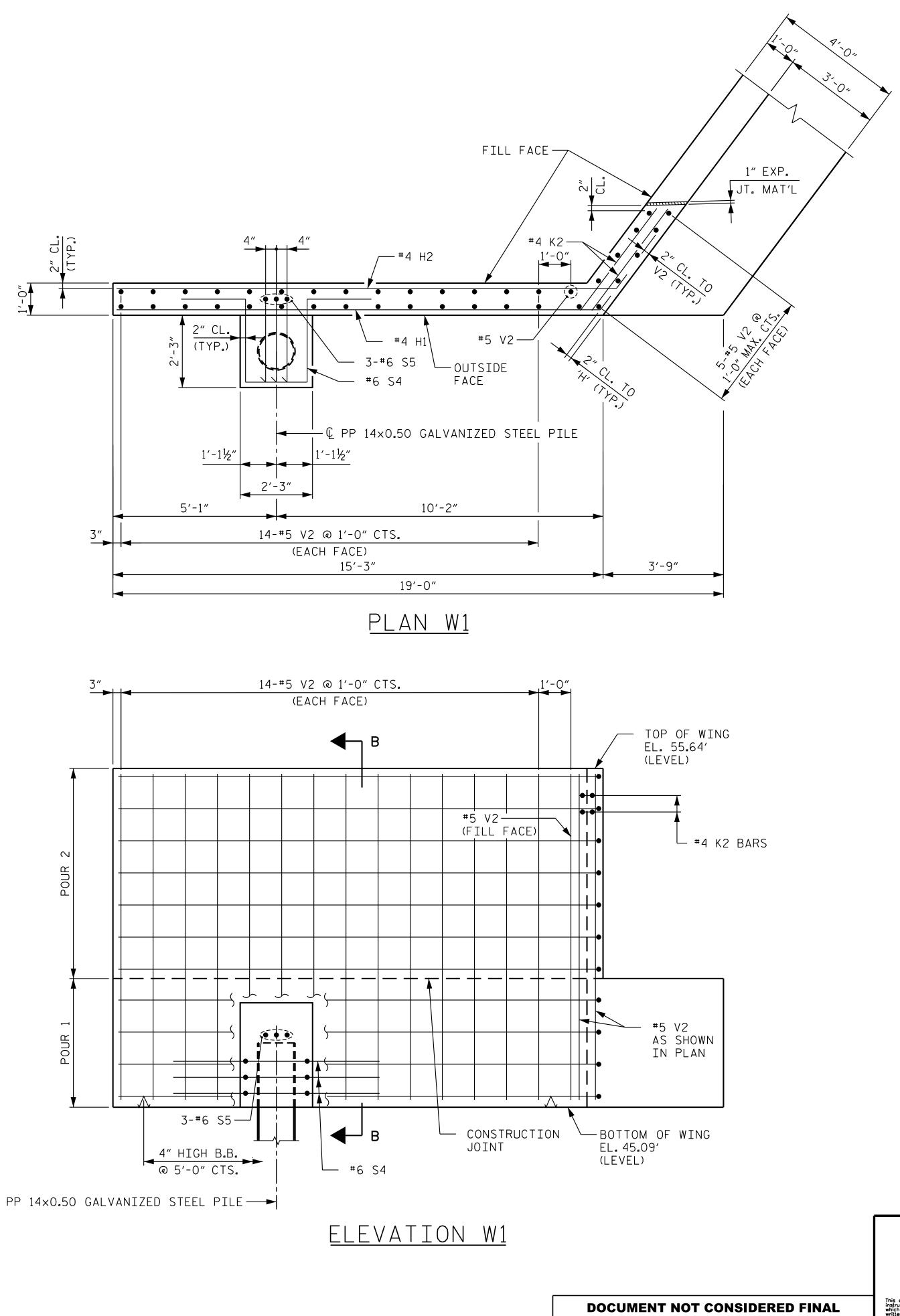
FOR ``SECTION A-A", SEE ``END BENT 2" SHEET 5 OF 5. FOR NOTES SEE ``END BENT 2" SHEET 1 OF 5.

ТОР	of cap	ELEVA	TIONS
H	51.07′	Ĺ	52.12′
I	51.21′	M	52 . 27′
U	51.59′	N	52.53′
K 51.74'			

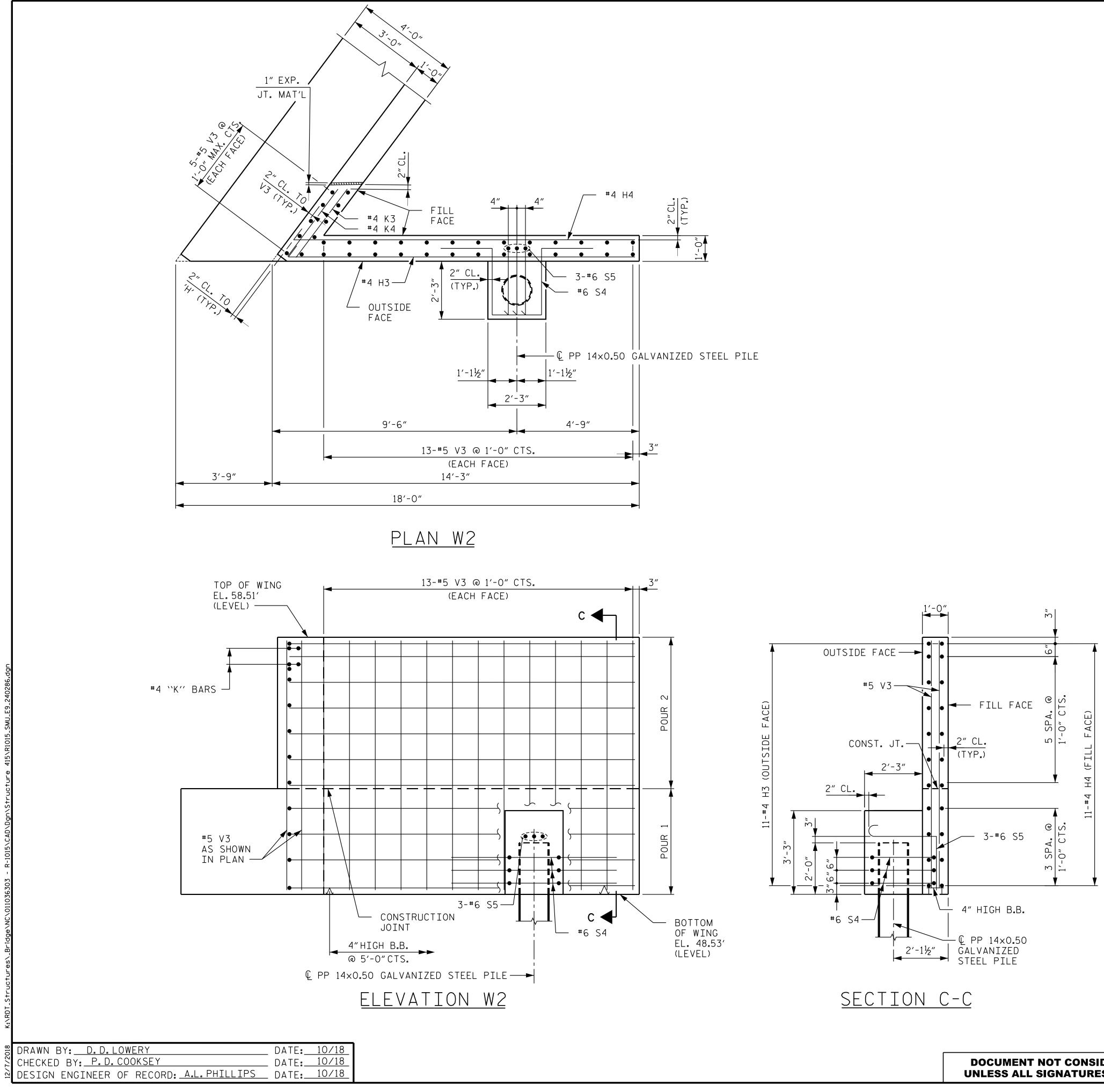
TOP OF PILE	ELEVATIONS
PILE NO.	ELEVATION
4	49.11′
5	49.64′
6	50.17′





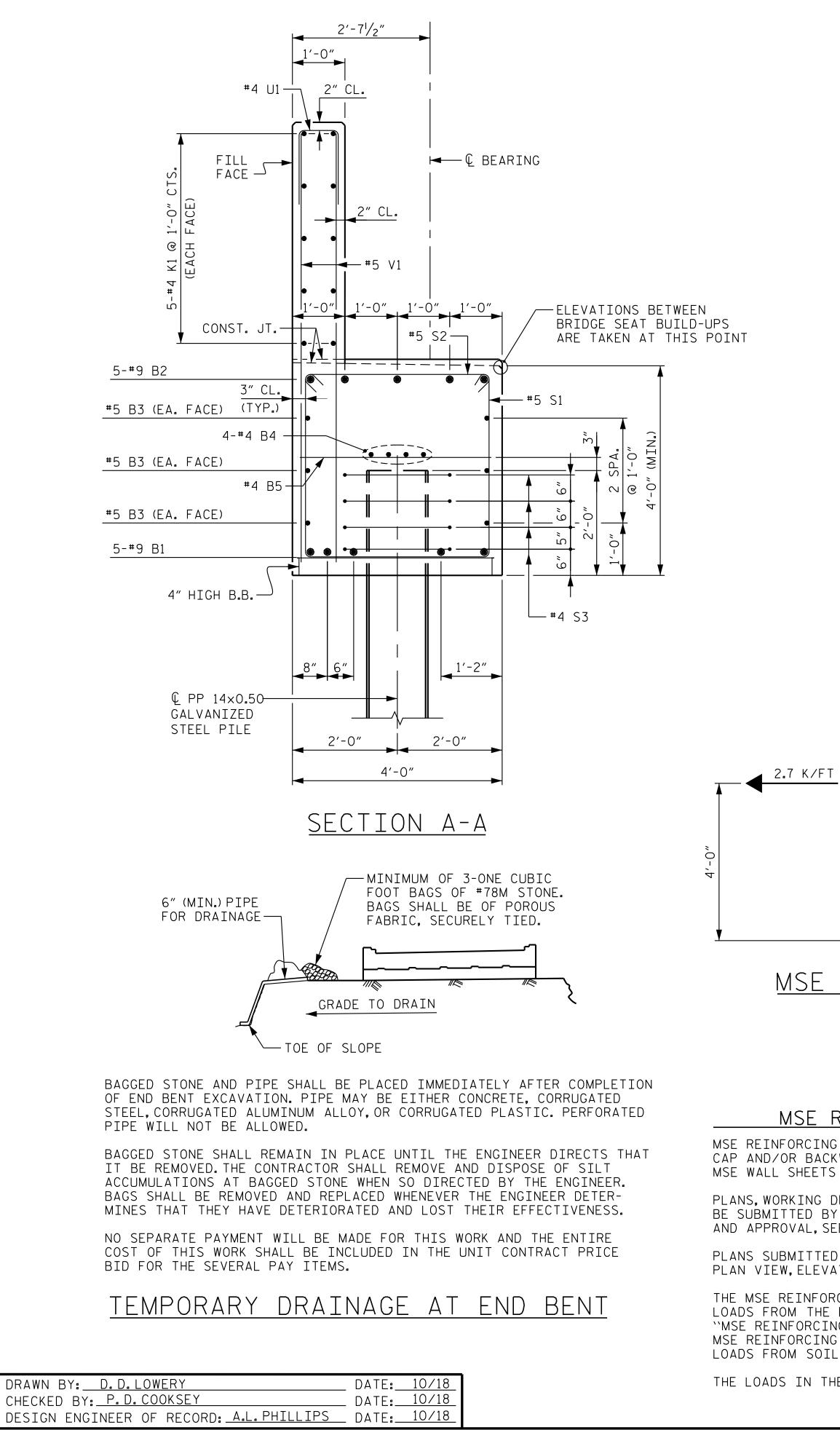


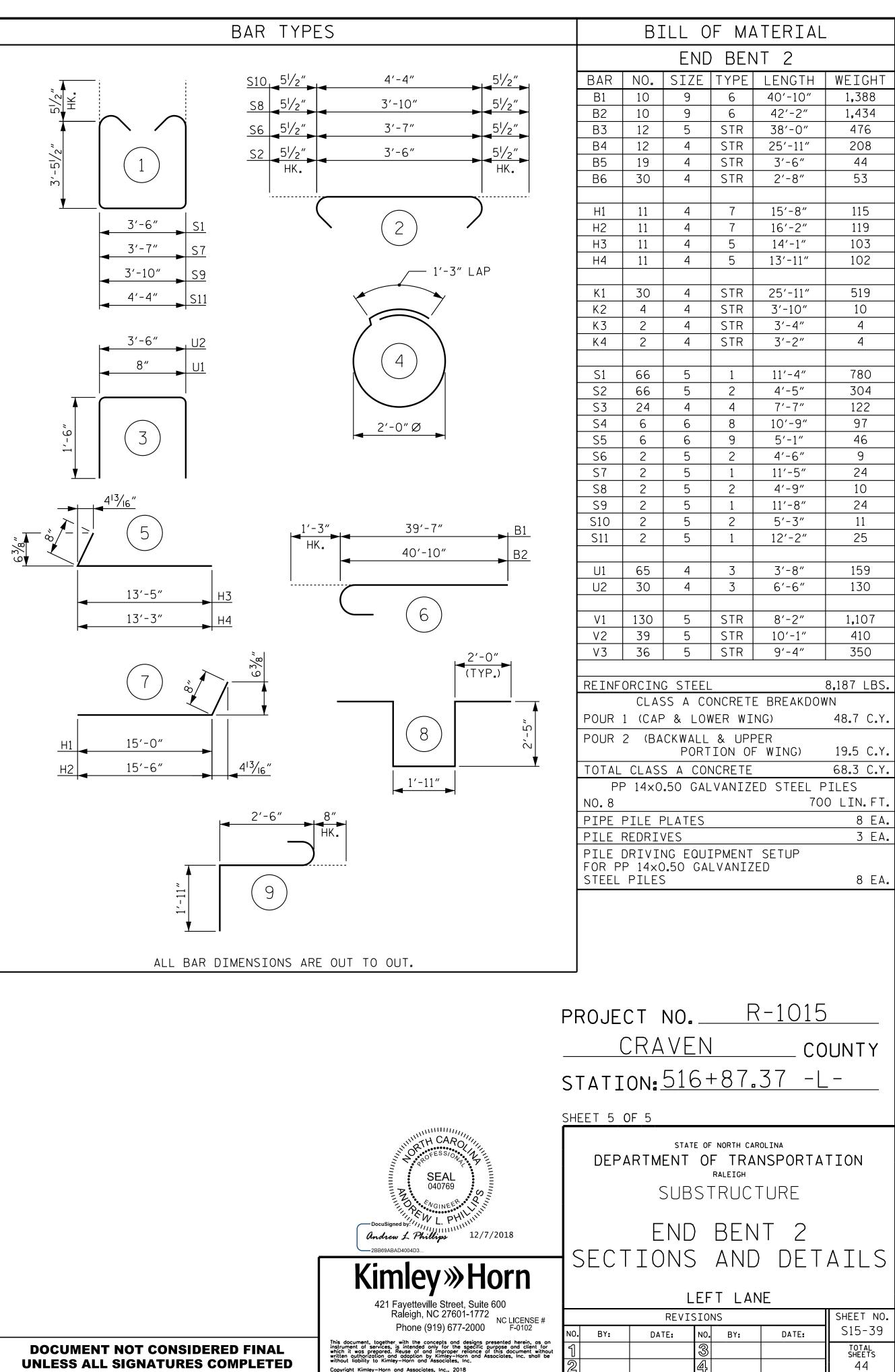
PROJECT NO. R-1015 CRAVEN COUNTY STATION: 516+87.37 -L-SHEET 3 OF 5 TH CAR STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SEAL 040769 SUBSTRUCTURE END BENT 2 andrew L Phillips 12/7/2018 SECTIONS AND DETAILS **Kimley Worn** LEFT LANE 421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102 REVISIONS SHEET NO. S15-37 NO. BY: DATE: DATE: BY: This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc. total sheets 44 Copyright Kimley—Horn and Associates, Inc., 2018



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT NO. R-1015 CRAVEN COUNTY STATION: 516+87.37 -L-SHEET 4 OF 5 TH CAR STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SEAL 040769 RALEIGH SUBSTRUCTURE END BENT 2 andrew L Phillips 12/7/2018 SECTIONS AND DETAILS **Kimley Worn** LEFT LANE 421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102 REVISIONS SHEET NO. S15-38 NO. BY: DATE: DATE: BY: This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc. TOTAL SHEETS 44 Copyright Kimley—Horn and Associates, Inc., 2018





MSE REINFORCING STRAP LOAD DETAIL

MSE REINFORCING STRAP NOTES

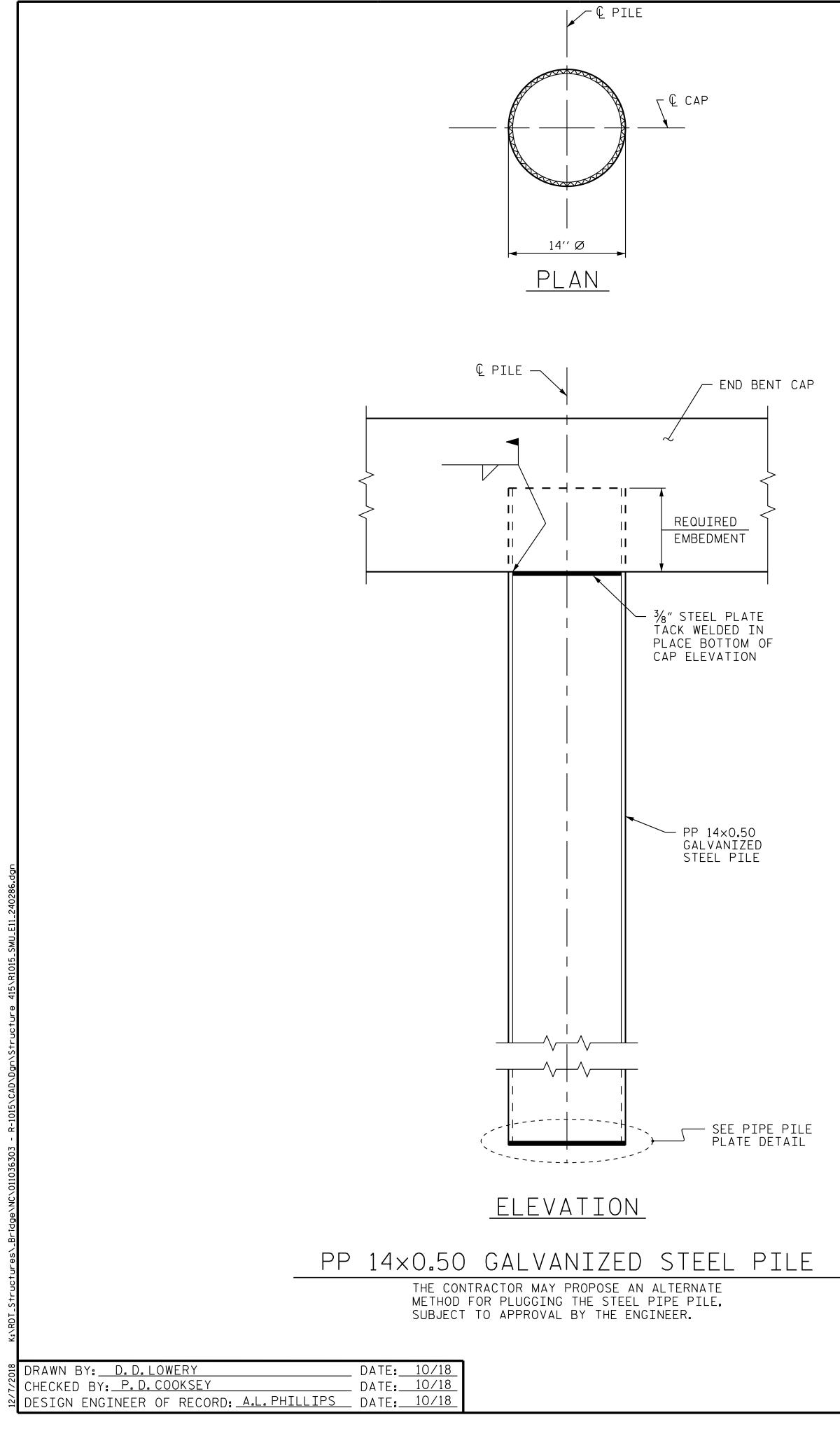
MSE REINFORCING STRAPS SHALL BE ATTACHED TO THE END BENT CAP AND/OR BACKWALL. FOR DESIGN CRITERIA AND DETAILS, SEE MSE WALL SHEETS AND SPECIAL PROVISIONS.

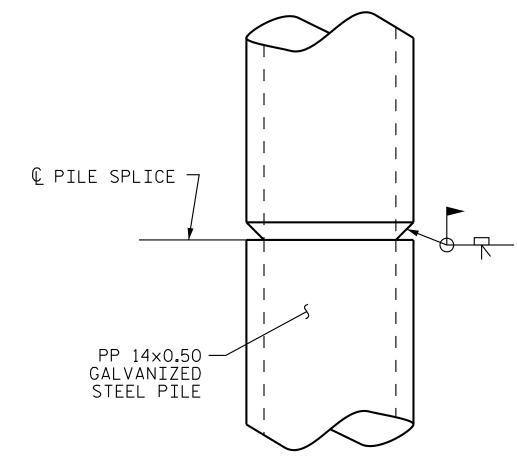
PLANS, WORKING DRAWINGS, AND DESIGN CALCULATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER FOR REVIEW AND APPROVAL, SEE SPECIAL PROVISIONS.

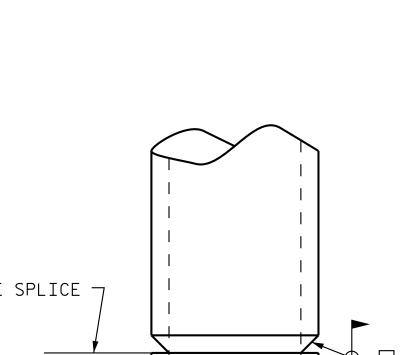
PLANS SUBMITTED FOR REVIEW SHALL INCLUDE THE FOLLOWING: PLAN VIEW, ELEVATION VIEW, TYPICAL SECTIONS, AND STRAP DETAILS.

THE MSE REINFORCING STRAPS SHALL BE DESIGNED TO CARRY THE LOADS FROM THE BRIDGE SUPERSTRUCTURE AS INDICATED IN THE "MSE REINFORCING STRAP LOAD DETAIL". IN ADDITION, THE MSE REINFORCING STRAPS SHALL ALSO BE DESIGNED TO CARRY LOADS FROM SOIL PRESSURE AS OUTLINED IN THE SPECIAL PROVISION.

THE LOADS IN THE DETAIL ABOVE ARE FACTORED LOADS.

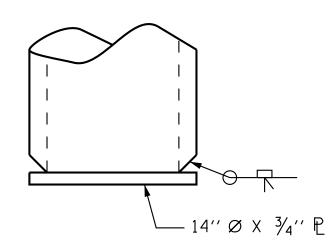






PIPE PILE SPLICE DETAIL





NOTES

PIPE PILES SHALL BE IN ACCORDANCE WITH SECTION 1084 OF THE STANDARD SPECIFICATIONS.

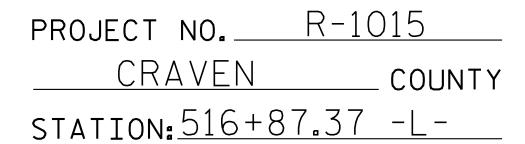
GALVANIZE STEEL PIPE PILES IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS UNLESS METALLIZING IS REQUIRED. GALVANIZING OR METALLIZING PIPE PILE PLATES IS NOT REQUIRED.

PIPE PILE PLATES, IF REQUIRED, SHALL BE IN ACCORDANCE WITH SECTION 450 OF THE STANDARD SPECIFICATIONS.

REMOVE AND REPLACE OR REPAIR TO THE SATISFACTION OF THE ENGINEER PILES THAT ARE DAMAGED, DEFORMED OR COLLAPSED DURING INSTALLATION OR DRIVING.

PILE SPLICES SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND AWS D1.1.

THE GALVANIZING IS CONSIDERED INCIDENTAL TO THE CONTRACT UNIT PRICE BID PER LINEAR FOOT FOR PP 14×0.50 GALVANIZED STEEL PILES.



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

14" STEEL PIPE PILE

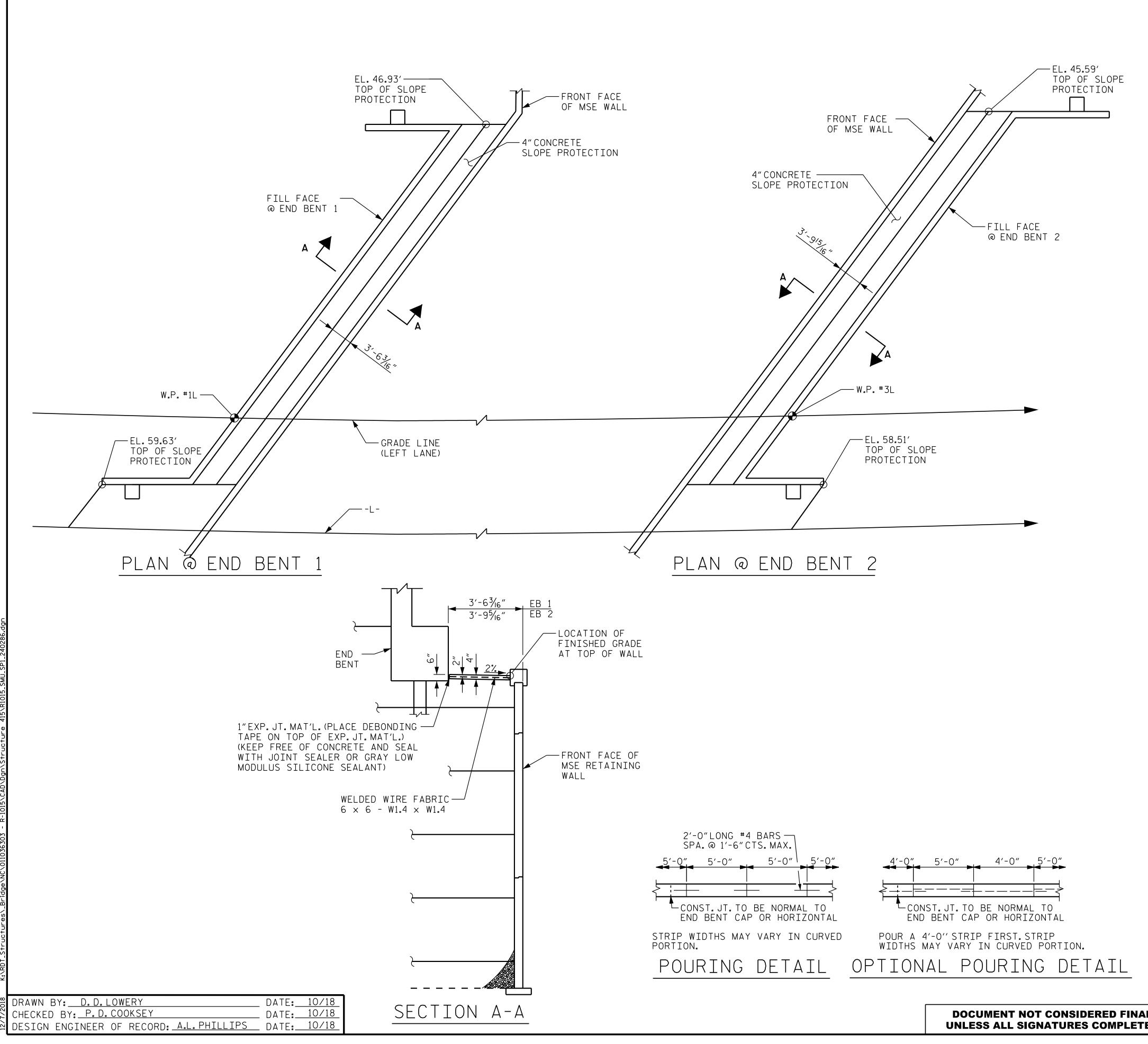
	LEFT LANE						
	REVISIONS						
NO.	NO. BY: DATE: NO. BY: DATE: S15-40						
Г			හු			TOTAL SHEETS	
2			4			44	
S1	STRUCTURE 15						



421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE # F-0102

Insurant, together with the concepts and designs presented herein, as a instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document withon written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc.

Copyright Kimley-Horn and Associates, Inc., 2018



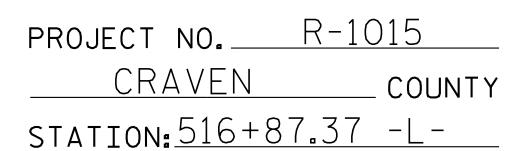
NOTES

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS.STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.FOR BERM WIDTH, SEE GENERAL DRAWING.

SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5'STRIPS AS SHOWN IN THE "POURING DETAIL' WITH 2'-O"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING.SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.516+87.37 (LEFT LANE)	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX.L.F.
END BENT 1	54	105
END BENT 2	58	110

* QUANTITY SHOWN IS BASED ON 5' POURS.



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SLOPE PROTECTION DETAILS

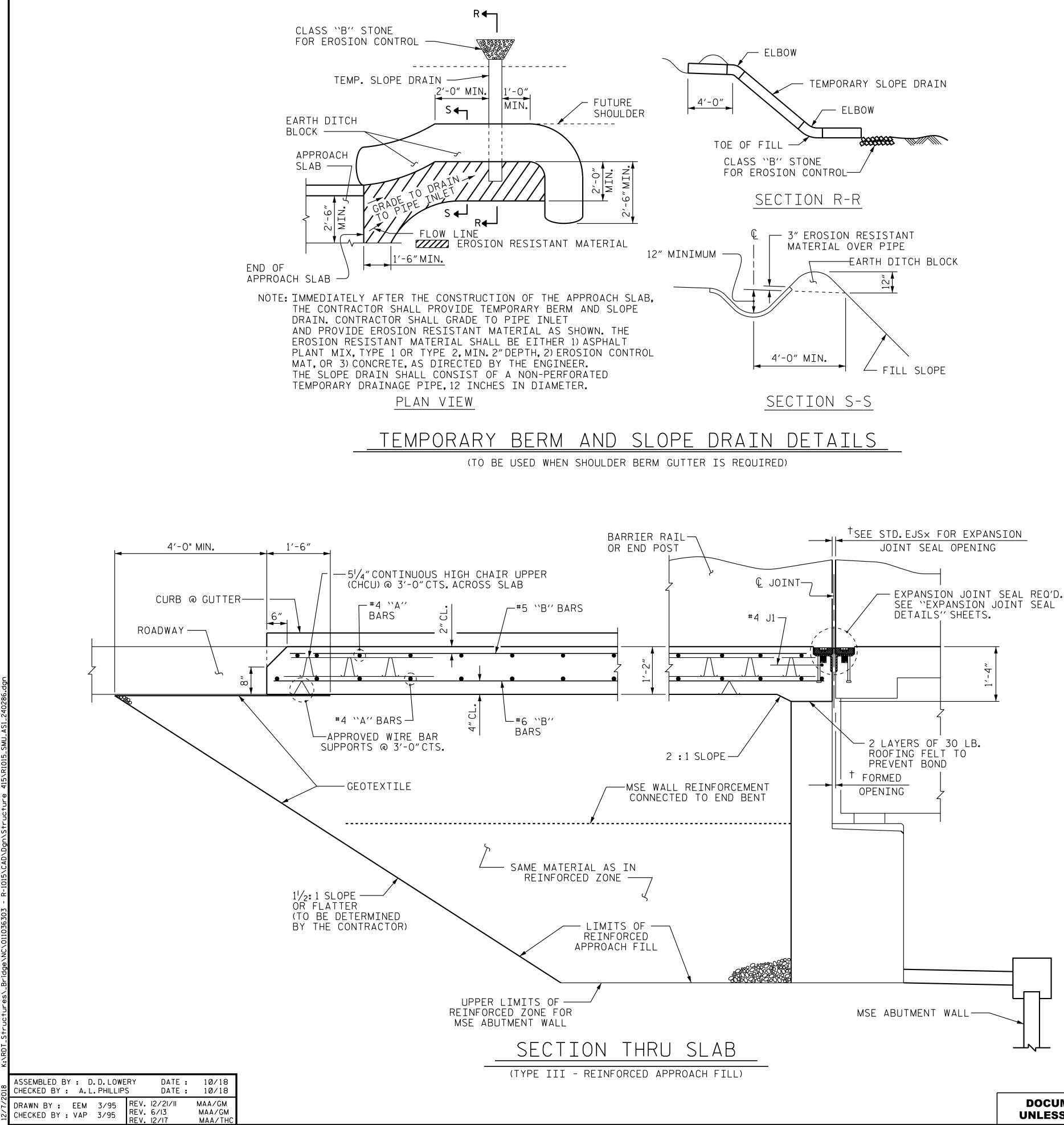
LANE

		SHEET NO.						
NO.	BY:	DATE:	N0.	BY:	DATE:	S15-41		
1			3			TOTAL SHEETS		
2			4			44		
S	STRUCTURE 15							



421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE # F-0102

This document, together with the concepts and designs presented herein, as instrument of services, is intended only for the specific purpose and client fc. which it was prepared. Reuse of and improper reliance of this document with written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates here.



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

ZONE.

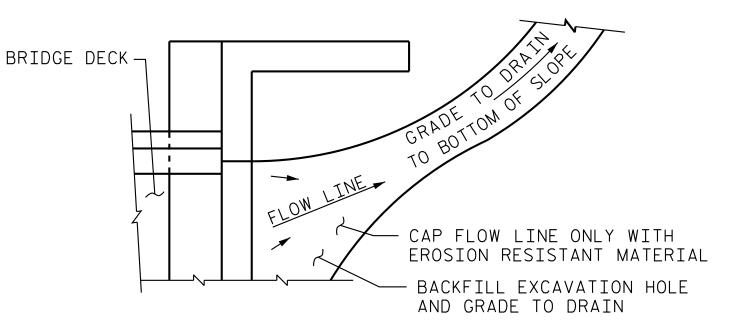
FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, MSE WALL REINFORCEMENT AND BACKFILL MATERIAL SEE ROADWAY PLANS. GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056. BACKFILL MATERIAL SHALL BE THE SAME MATERIAL USED IN THE MSE REINFORCED APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

ROADWAY PLANS.

NOTE	S
------	---

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED.SEE

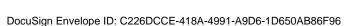
FOR EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.

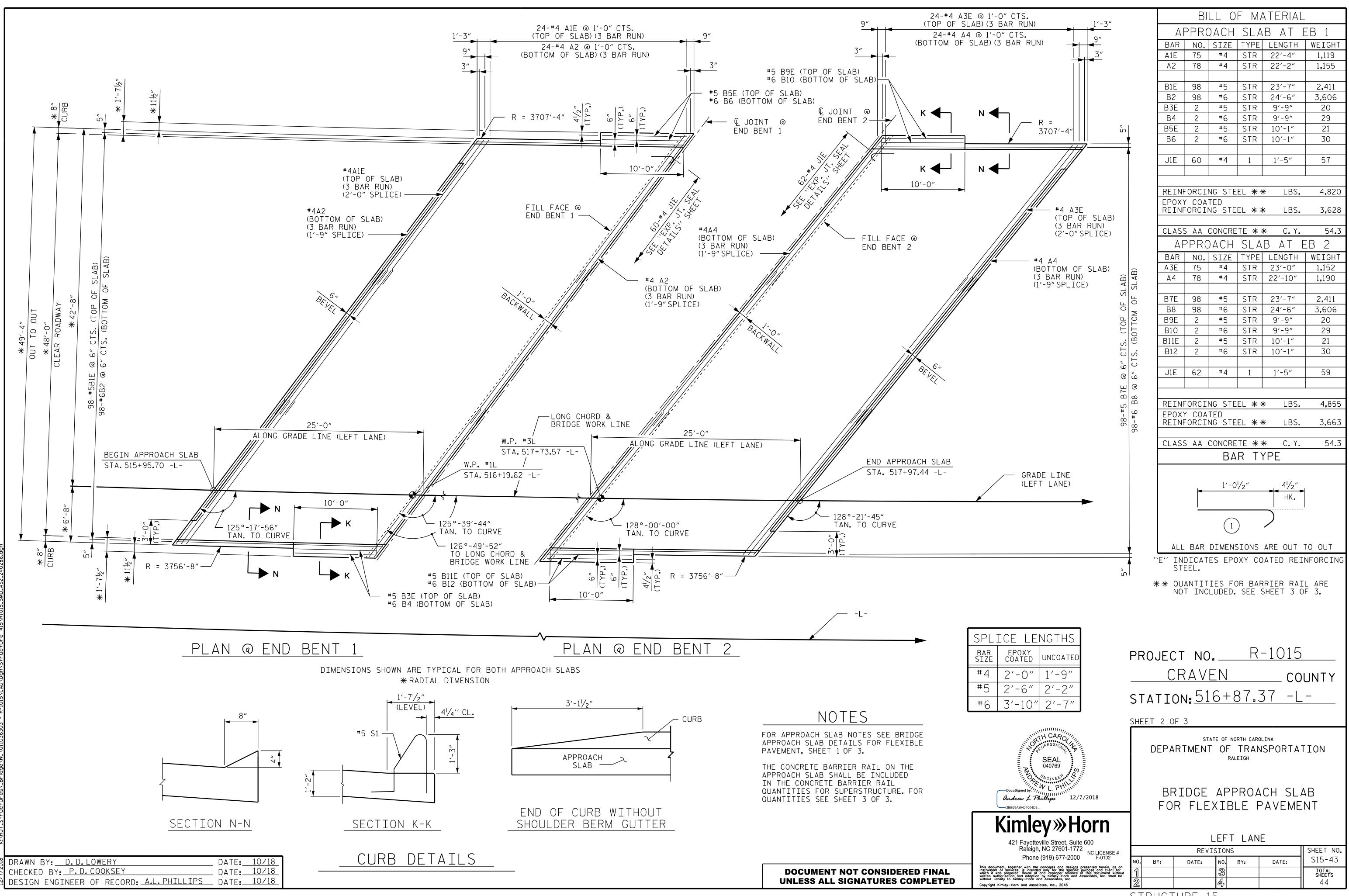


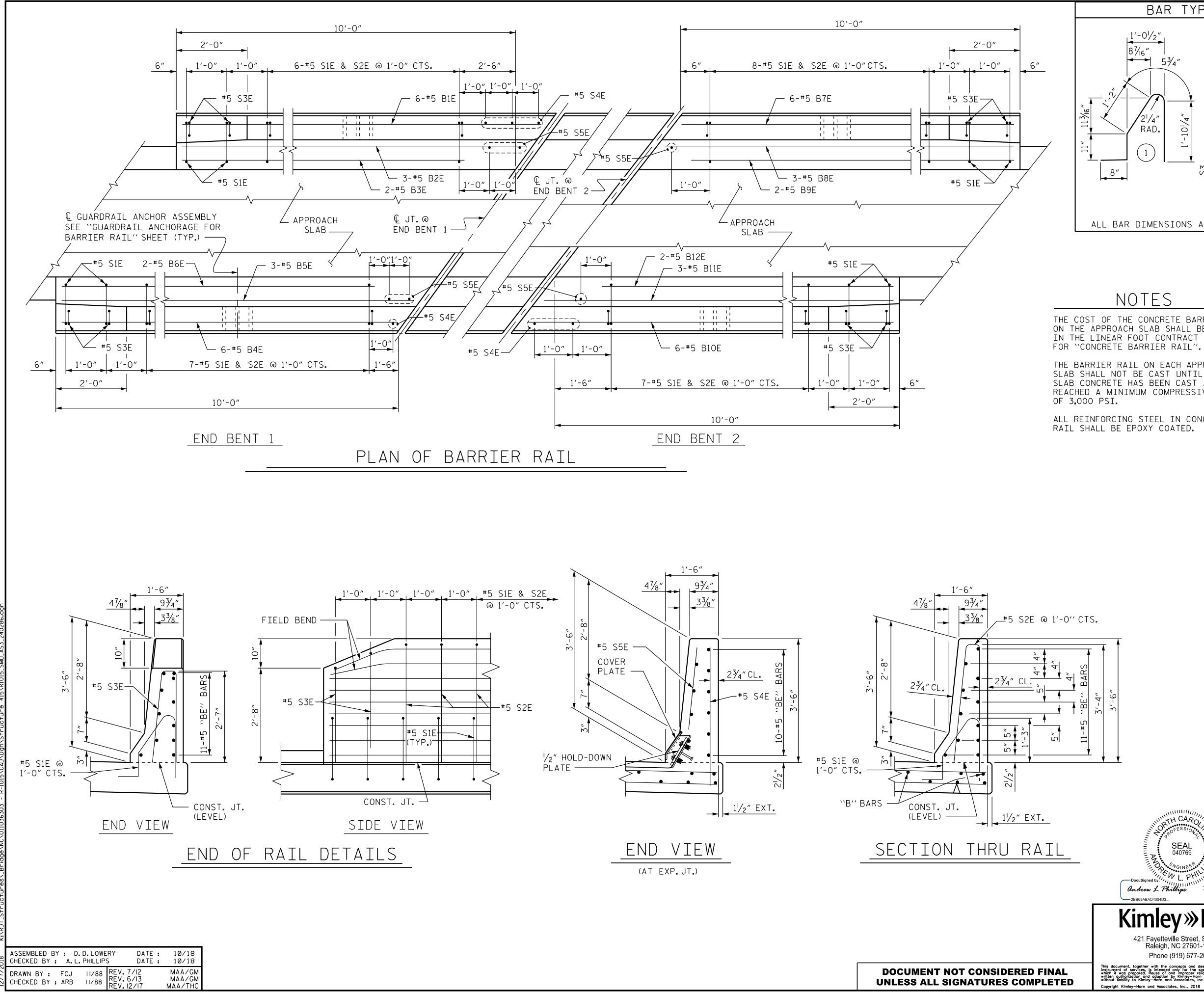
NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

	PROJEC <u>C</u> STATIC	RAVE	N		UNTY
	SHEET 1 O	F 3			
Becusigned by: Docusigned by: Andrew 1. Phillips 12/7/2018 28B69ABAD4004D3	BF	RTMENT	raleigh TANDAF APPROA	NSPORTA RD ACH SL4	٩B
Kimley Worn					
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102		REVIS	IONS		SHEET NO.
This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc.	NO. BY: 1 2		NO. BY: 3 4	DATE:	S15-42 TOTAL SHEETS 44
		URE 15	1	STD.N	O.BAS2







BAR TYPES		BILI	_ OF	MA	TERIAL	
	FOR	CONCR	ETE BA	ARRIER	RAIL AT	EB 1
1' - 0'/2''	BAR	NO.	SIZE	TYPE	LENGTH	WEIGH
87/16"	B1E	6	#5	STR	10′-6″	66
5^{3}_{4} "	B2E	3	#5	STR	10'-3"	32
	B3E	2	#5	STR	8'-10"	18
	B4E	6	#5	STR	9'-9"	61
	B5E	3	#5	STR	10'-0"	31
	B6E	2	#5	STR	9′-7″	20
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	S1E	17	#5	1	5′-1″	90
	S2E	13	#5	2	7'-0″	95
	S3E	4	# 5	2	5′-6″	23
8"	S4E	4	#5	STR	3'-11"	16
	S5E	4	# 5	STR	2'-4"	10
(2)	EPOXY C	COATED				
	REINFOR	RCING	STEEL		Δ	462 LBS
BAR DIMENSIONS ARE OUT TO OUT	CLASS A	A CON	CRETE		2.9	CU. YDS
	CONCRET	<u>e</u> bari	RIER F	RAIL	20.0	LIN.F
		BILI	_ OF	MA	TERIAL	
	FOR	CONCRE	ETE BA	RRIER	RAIL AT	EB 2
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGH
	B7E	6	#5	STR	9′-9″	61
NOTES	B8E	3	#5	STR	10'-0"	31
	B9E	2	#5	STR	9'-2"	19
OF THE CONCRETE BARRIER RAIL PROACH SLAB SHALL BE INCLUDED	B10E	6	# 5	STR	10'-7"	66
EAR FOOT CONTRACT PRICE BID	B11E	3	# 5	STR	10'-4"	32
RETE BARRTER RATI "		2	#5	стр	0/ ///	17

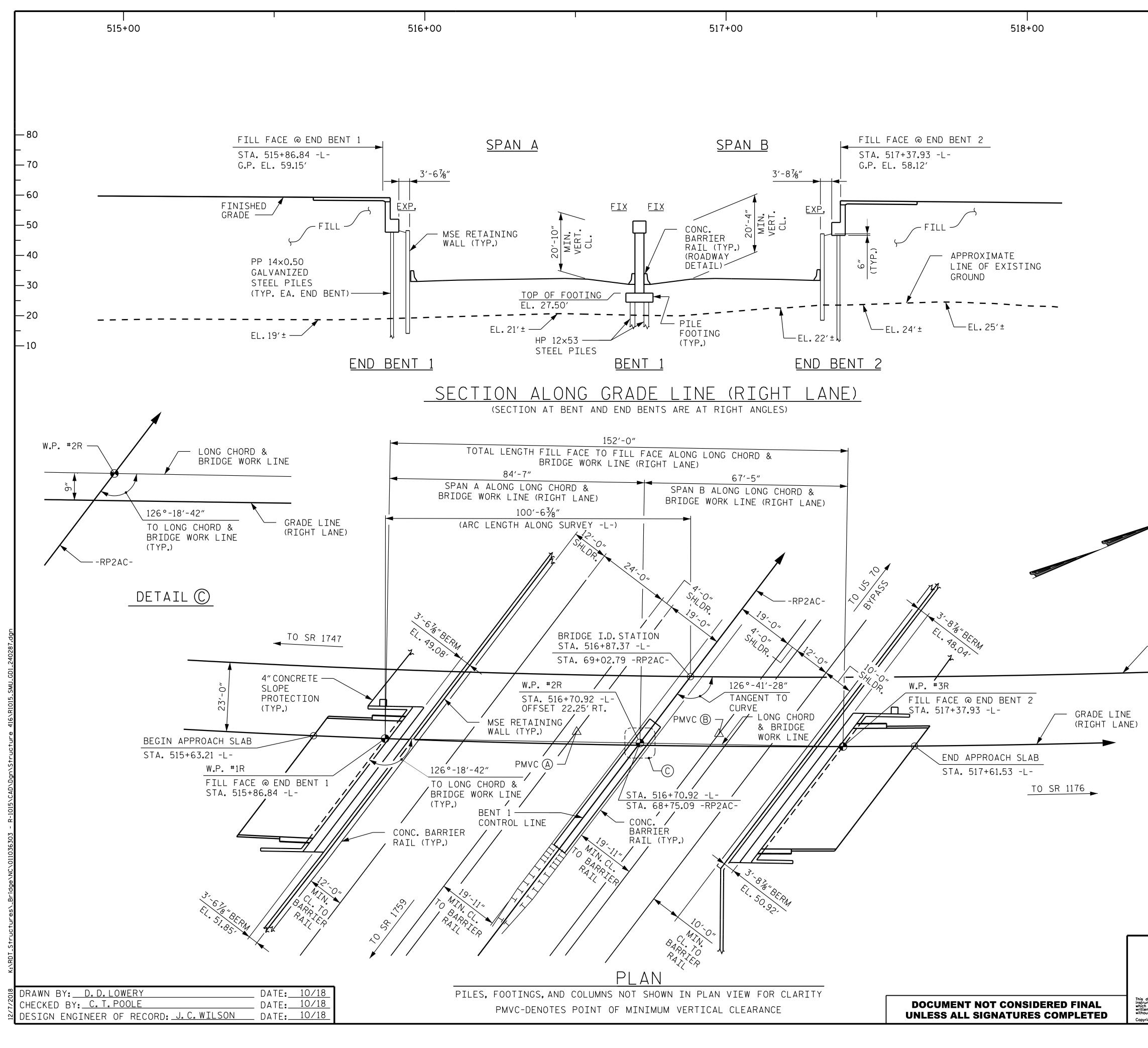
THE BARRIER RAIL ON EACH APPROACH SLAB SHALL NOT BE CAST UNTIL ALL APPROACH SLAB CONCRETE HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH

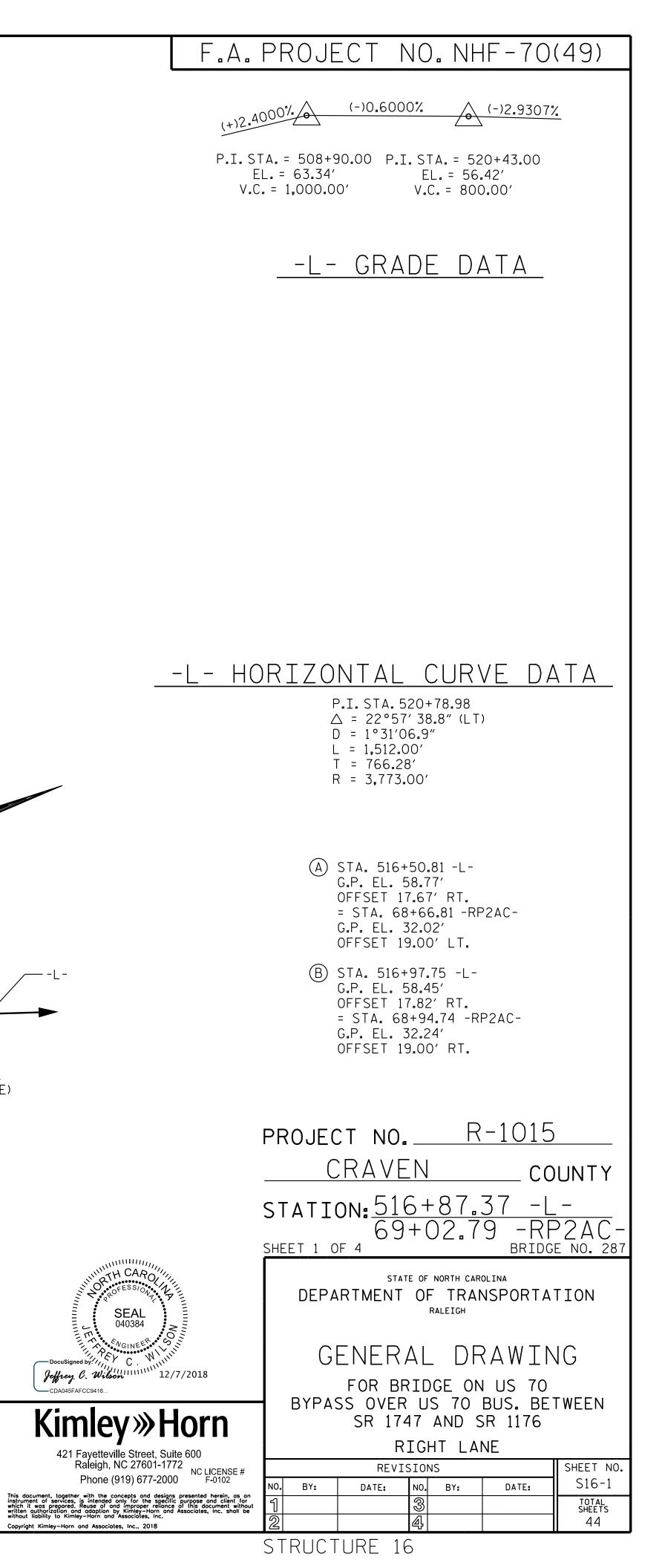
ALL REINFORCING STEEL IN CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

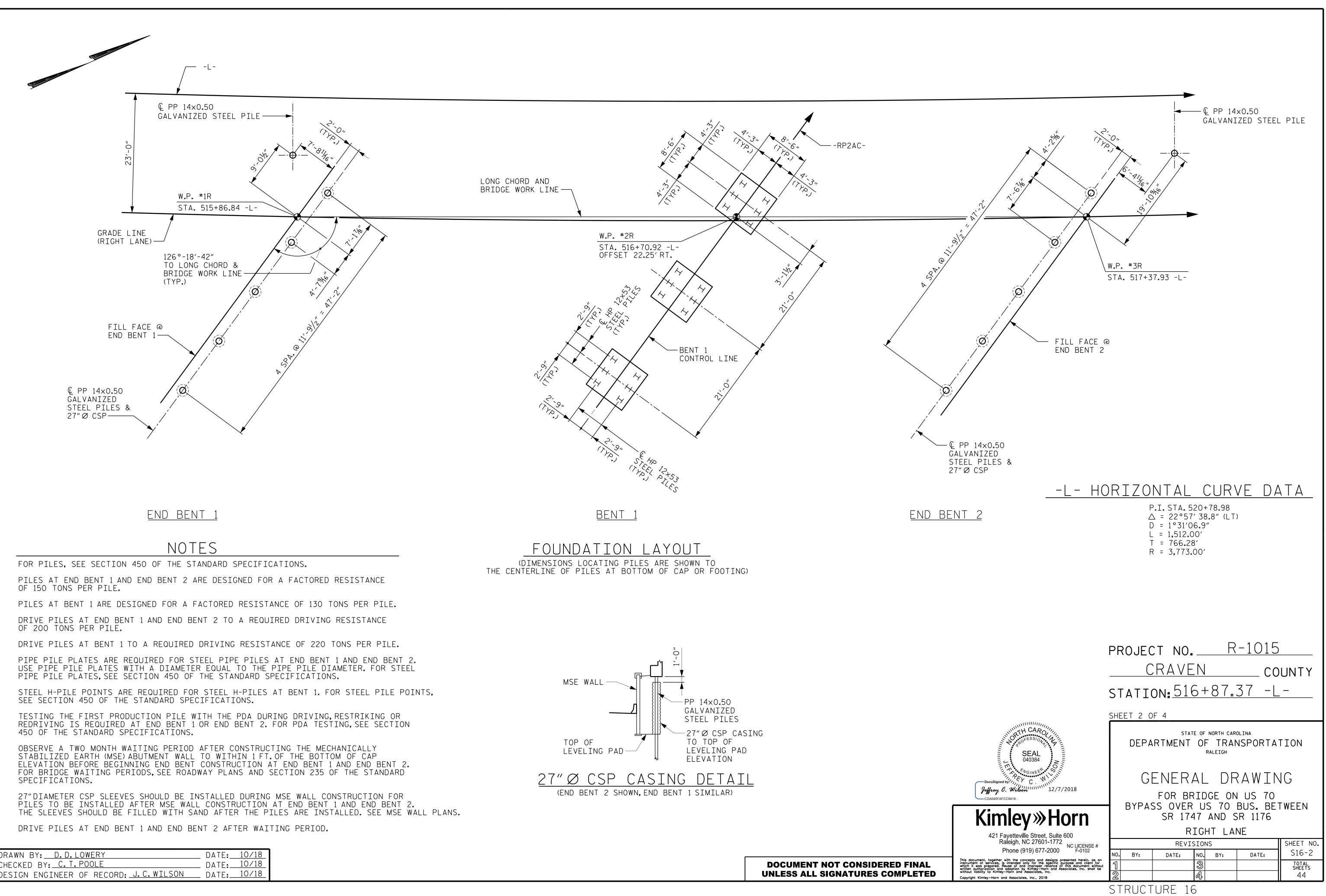
FOR	FOR CONCRETE BARRIER RAIL AT EB I						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
B1E	6	#5	STR	10'-6"	66		
B2E	3	#5	STR	10'-3"	32		
B3E	2	#5	STR	8'-10"	18		
B4E	6	# 5	STR	9'-9"	61		
B5E	3	# 5	STR	10'-0"	31		
B6E	2	# 5	STR	9′-7″	20		
S1E	17	# 5	1	5′-1″	90		
S2E	13	# 5	2	7'-0″	95		
S3E	4	# 5	2	5′-6″	23		
S4E	4	# 5	STR	3'-11"	16		
S5E	4	# 5	STR	2'-4"	10		
EPOXY C	CATED						
REINFOR	RCING	STEEL		4	462 LBS.		
CLASS A				2.9	CU.YDS.		
CONCRET	<u>e bari</u>	RIER F			LIN.FT.		
BILL OF MATERIAL							
FOR	FOR CONCRETE BARRIER RAIL AT EB 2						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
B7E	6	#5	STR	9′-9″	61		
B8E	3	#5	STR	10'-0"	31		
B9E	2	#5	STR	9′-2″	19		
B10E	6	#5	STR	10'-7"	66		
B11E	3	#5	STR	10'-4"	32		
B12E	2	# 5	STR	8'-4"	17		
S1E					101		
	19	#5	1	5'-1"			
S2E	15	#5	2	7'-0"	110		
S3E	15 4	#5 #5	2 2	7'-0" 5'-6"	110 23		
S3E S4E	15 4 2	#5 #5 #5	2 2 STR	7'-0" 5'-6" 3'-11"	110 23 8		
S3E	15 4	#5 #5	2 2	7'-0" 5'-6"	110 23		
S3E S4E	15 4 2	#5 #5 #5	2 2 STR	7'-0" 5'-6" 3'-11"	110 23 8		
S3E S4E S5E EPOXY C	15 4 2 2 0ATED	#5 #5 #5 #5	2 2 STR	7'-0" 5'-6" 3'-11" 2'-4"	110 23 8 5		
S3E S4E S5E EPOXY C REINFOR	15 4 2 2 COATED	#5 #5 #5 \$TEEL	2 2 STR	7'-0" 5'-6" 3'-11" 2'-4"	110 23 8 5 473 LBS.		
S3E S4E S5E EPOXY C REINFOR CLASS A	15 4 2 2 COATED CING A CON	#5 #5 #5 STEEL	2 STR STR	7'-0" 5'-6" 3'-11" 2'-4" 2.9	110 23 8 5 473 LBS. CU. YDS.		
S3E S4E S5E EPOXY C REINFOR	15 4 2 2 COATED CING A CON	#5 #5 #5 STEEL	2 STR STR	7'-0" 5'-6" 3'-11" 2'-4" 2.9	110 23 8 5 473 LBS.		

"E" INDICATES EPOXY COATED REINFORCING STEEL.

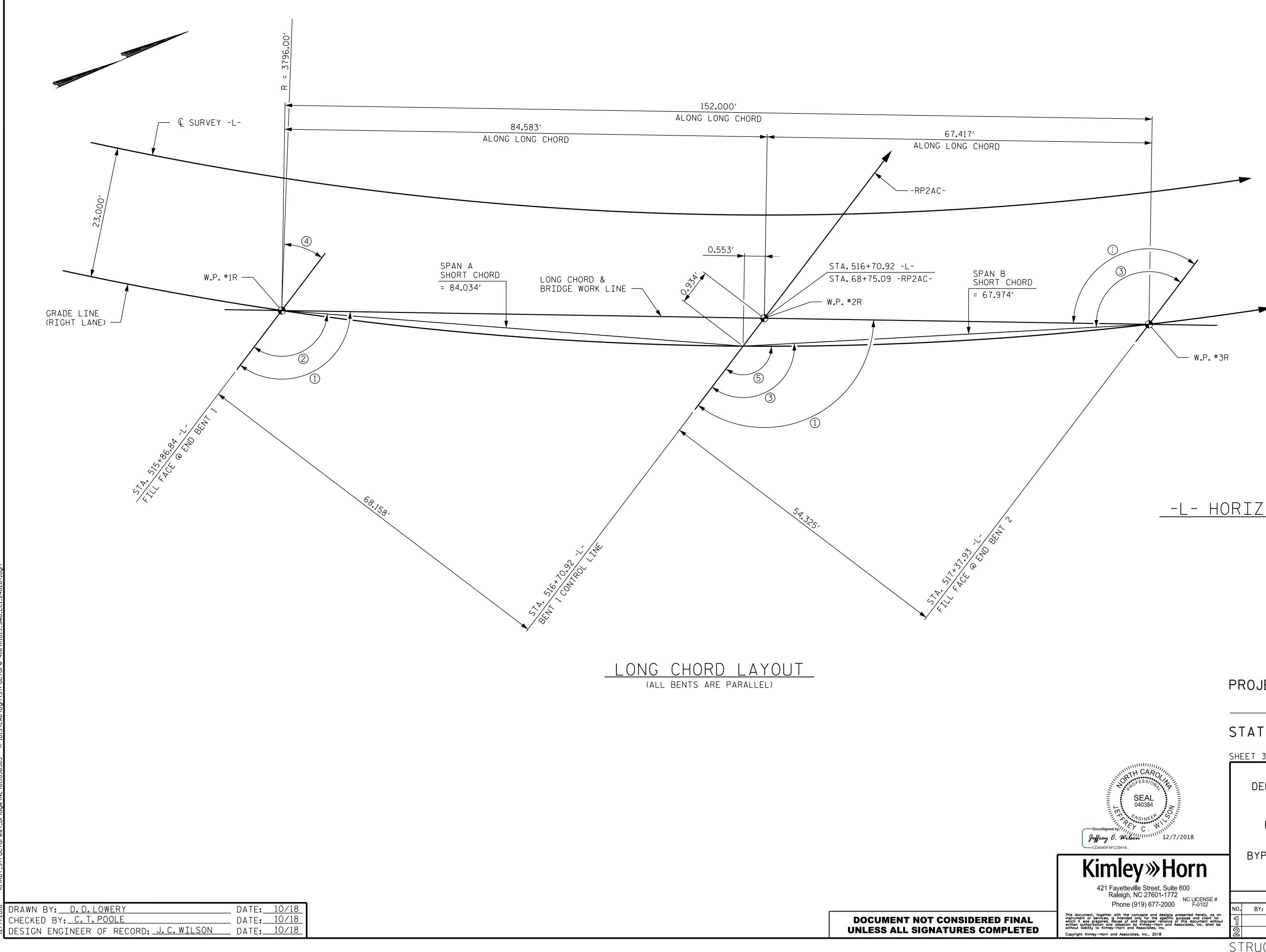
3′-6″	project no. <u>R-1015</u>
	<u>CRAVEN</u> COUNTY
	STATION: 516+87.37 -L-
	SHEET 3 OF 3
SEAL 040769	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
	STANDARD
DocuSigned by!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!	BRIDGE APPROACH SLAB DETAILS
Kimley Worn	
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 NC LICENSE #	REVISIONS SHEET NO.
Phone (919) 677-2000 F-0102 document, together with the concepts and designs presented herein, as an ument of services, is intended only for the specific purpose and client for h it was prepared. Reuse of and improper reliance of this document without en authorization and adaption by kimley-Horn and Associates, Inc. shall be but liability to Kimley-Horn and Associates, Inc., 2018	NO. BY: DATE: NO. BY: DATE: S15-44 1 3 3 TOTAL SHEETS 344 2 44 44 44
	STRUCTURE 15 STD. NO. BAS4







018	DRAWN BY: <u>D.D.LOWERY</u>	DATE:_	10/18
772(CHECKED BY: C.T.POOLE	DATE:	10/18
	DESIGN ENGINEER OF RECORD: J.C.WILSON	DATE:	10/18



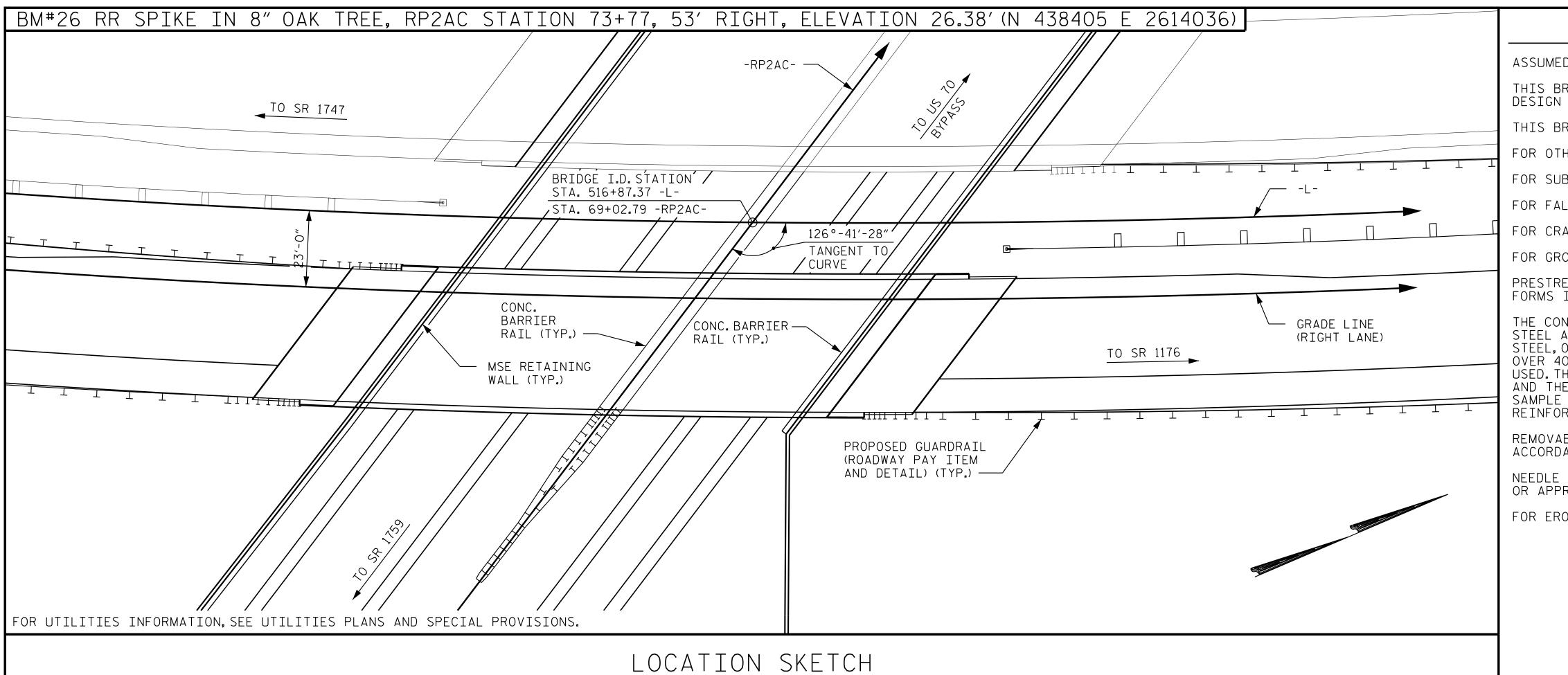
TAE	BLE OF ANGLES				
No.	ANGLES				
1	126°-18′-42″				
2	125°-47′-55″				
3	126°-56′-45″				
4	35°-09′-52″				
5	126°-25′-58″ (TANGENT TO CURVE)				

<u>-L- HORIZONTAL CURVE DATA</u>

P.I. STA. 520+78.98
△ = 22°57′38.8″(LT)
D = 1°31′06.9″
L = 1,512.00′
T = 766.28′
P = 377300'

R = 3,//3.00'

	PROJEC <u>C</u> STATIC	RAVE	IN		0 UNTY
	SHEET 3 O	F 4			
SEAL 040384 0405 0405 0405 0405 0405 0405 0405 04		rtment ENER/	e of north car OF TRAI raleigh AL DF	NSPORTA RAWIN	
imley »Horn	BYPAS	SS OVER		BUS. BE	TWEEN
			IGHT LA		
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 NC LICENSE #	RIGHI LANE REVISIONS SHEET NO.				
Phone (919) 677-2000 F-0102	NO. BY:	DATE:	NO. BY:	DATE:	S16-3
together with the concepts and designs presented herein, as an services, is initended only for the specific purpose and client for reported. Reuse of and improper reliance of this document without cation and adaption by Kimley-Horn and Associates, Inc. shall be to Kimley-Horn and Associates, Inc. ey-Horn and Associates, Inc., 2018	1		3 4		total sheets 44
	C T D L L C T		~		



	TOTAL BILL OF MATERIAL																				
	PDA TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE (BRIDGE)	BRIDGE APPROACH SLABS STA.516+87.37 -L-	(BRIDGE)	SPIRAL COLUMN REINFORCING STEEL (BRIDGE)	54" PRESTRESSED CONCRETE GIRDERS	PILE DRIVING EQUIPMENT SETUP FOR HP 12×53 STEEL PILES	PILE DRIVING EQUIPMENT SETUP FOR PP 14×0.50 GALVANIZED STEEL PILES	HP STEI	9 12×53 El PILI	PP 14× GALVAN STEEL F	(0.50 NIZED PILES	STEEL PILE POINTS	PIPE PILE PLATES	PILE REDRIVES	CONCRETE BARRIER RAIL	4″ SLOPE PROTECTION	ELASTOMERIC BEARINGS	EXPANSION JOINT SEALS
	EA.	SQ.FT.	SQ.FT.	CU.YDS.	LUMP SUM	LBS.	LBS.	NO. LIN.FT.	EA.	EA.	NO.	LIN.F	T. NO. LI	N.FT.	EA.	EA.	EA.	LIN.FT.	SQ. YDS.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE		6,444	7,218		LUMP SUM			10 730.31										337.2		LUMP SUM	LUMP SUM
END BENT 1				57.2		7,562				6			6	540		6	3		52		
BENT 1				82.6		18,720	1,414		18		18	1,305	5		18		8				
END BENT 2				59.0		7,689				6			6	570		6	3		54		
TOTAL	1	6,444	7,218	198.8	LUMP SUM	33,971	1,414	10 730.31	18	12	18	1,305	5 12 1	.,110	18	12	14	337.2	106	LUMP SUM	LUMP SUM

	PLE BAR
REPL	ACEMENT
SIZE	LENGTH
#3	6'-2"
#4	7'-4"
#5	8'-6"
#6	9'-8"
#7	10'-10"
#8	12'-0"
#9	13'-2″
#10	14'-6"
#11	15'-10″

NOTE: Sample bar	REP
BASED ON 30 TWO SPLICE	

B DRAWN BY: <u>D.D.LOWERY</u>	DATE:	10/18
B DRAWN BY: D.D.LOWERY CHECKED BY: C.T.POOLE	DATE:	10/18
DESIGN ENGINEER OF RECORD: J.C. WILSO	N DATE:	10/18

NOTES
ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE SAMPLE BARS SHOULD COME FROM STEEL ACTUALLY USED IN THE PROJECT AND THE SAMPLE BARS SHOULD BE REPLACED BY SPLICED BARS AS SPECIFIED IN THE SAMPLE BAR REPLACEMENT CHART. PAYMENT FOR THE SAMPLE BARS AND REPLACEMENT REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

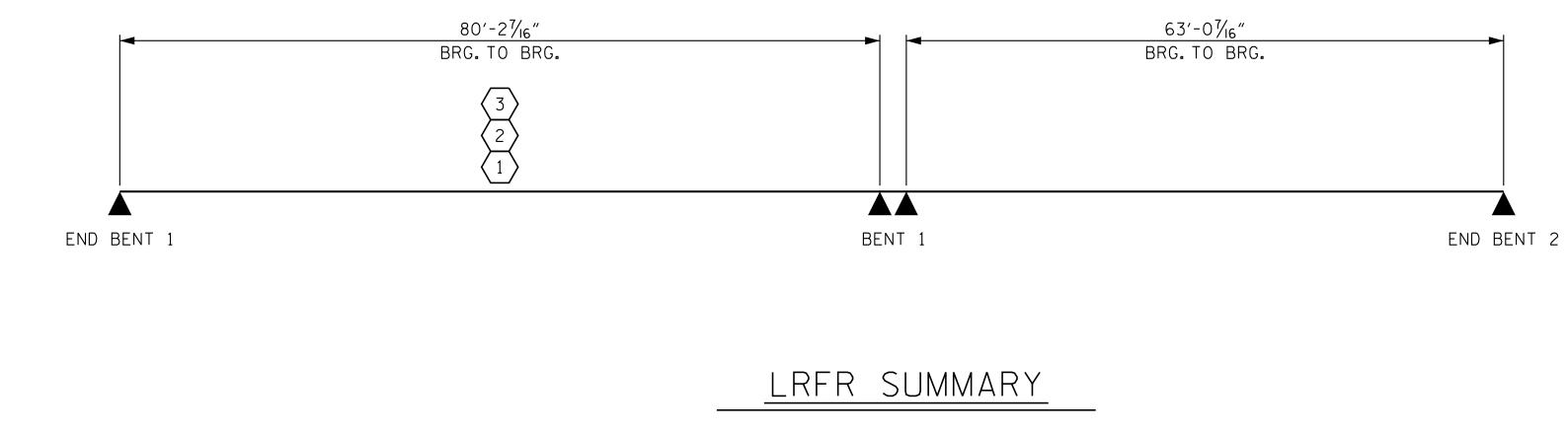
NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

-		_											
14	337.2	106	LUMP SUM	LUMP SUM]								
			PROJ	ECT NO.	R	-1015							
				CRAVENCOUNTY									
	STATION: <u>516+87.37 -L-</u>												
			SHEET	4 OF 4									
	DocuSigned by:		DE	STATI EPARTMENT	OF NORTH CAR OF TRAI RALEIGH		TION						
ſ		EAL 40384 C. WINNIN 12/7/2018		GENERA	AL DF	RAWIN	IG						
	CDA045FAFCC9416		BY	FOR BR PASS OVER SR 174		N US 70 BUS. BE ⁻ SR 1176	TWEEN						
	-	/ »Hor i			GHT LA								
	-	e Street, Suite 600 C 27601-1772		REVISIONS									
This docu	,	,	NO. BY	: DATE:	NO. BY:	DATE:	S16-4						
	was prepared. Reuse of and uthorization and adaption by ability to Kimley–Horn and A Kimley–Horn and Associates	cepts and designs presented he hy for the specific purpose and improper reliance of this docu Kimley-Horn and Associates, li ssociates, inc. , inc., 2018	iclient for ment without nc. shall be		<u>3</u> 4		total sheets 44						
-													

								STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					νΤΕ	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING #	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	NZA	$\langle 1 \rangle$	1.04		1.75	0.743	1.56	А	I	40.100	1.034	1.20	А	I	7.500	0.80	0.736	1.04	А	I	40.100	
DESIGN LOAD		HL-93 (OPERATING)	N⁄A		1.58		1.35	0.743	2.02	А	I	40.100	1.034	1.58	А	I	7.500	N⁄A						
RATING		HS-20 (INVENTORY)	36.000	2	1.40	50.40	1.75	0.743	2.09	А	I	40.100	1.034	1.57	А	I	7.500	0.80	0.736	1.40	А	I	40.100	
	-	HS-20 (OPERATING)	36.000		2.07	74.52	1.35	0.743	2.70	А	I	40.100	1.034	2.07	А	I	7.500	N/A						
		SNSH	13.500		3.23	43.61	1.40	0.743	6.03	А	I	40.100	1.034	4.99	А	I	7.500	0.80	0.736	3.23	А	I	40.100	
	ш	SNGARBS2	20.000		2.38	47.60	1.40	0.743	4.43	А	I	40.100	1.034	3.50	А	I	7.500	0.80	0.736	2.38	А	I	40.100	
	ICL	SNAGRIS2	22.000		2.23	49.06	1.40	0.743	4.16	А	I	40.100	1.034	3.24	А	I	7.500	0.80	0.736	2.23	А	I	40.100	
	VEH	SNCOTTS3	27.250		1.60	43.60	1.40	0.743	2.99	А	I	40.100	1.034	2.41	А	I	7.500	0.80	0.736	1.60	А	I	40.100	
	CLE (S	SNAGGRS4	34.925		1.33	46.45	1.40	0.743	2.48	А	I	40.100	1.034	1.93	А	I	7.500	0.80	0.736	1.33	А	I	40.100	
	SIN	SNS5A	35.550		1.30	46.22	1.40	0.743	2.42	А	I	40.100	1.034	1.95	А	I	7.500	0.80	0.736	1.30	А	I	40.100	
		SNS6A	39.950		1.19	47.54	1.40	0.743	2.21	А	I	40.100	1.034	1.76	А	I	7.500	0.80	0.736	1.19	А	I	40.100	
LEGAL LOAD		SNS7B	42.000		1.13	47.46	1.40	0.743	2.11	А	I	40.100	1.034	1.72	А	I	7.500	0.80	0.736	1.13	А	I	40.100	
RATING	LER	TNAGRIT3	33.000		1.45	47.85	1.40	0.743	2.70	А	I	40.100	1.034	2.36	А	I	7.500	0.80	0.736	1.45	А	I	40.100	
	RAI	TNT4A	33.075		1.45	47.96	1.40	0.743	2.71	А	I	40.100	1.034	2.15	А	I	7.500	0.80	0.736	1.45	А	I	40.100	
	T-TM	TNT6A	41.600		1.18	49.09	1.40	0.743	2.20	А	I	40.100	1.034	1.92	А	I	7.500	0.80	0.736	1.18	А	I	40.100	
	SEI ST)	ΤΝΤ7Α	42.000		1.19	49.98	1.40	0.743	2.21	А	I	40.100	1.034	1.79	А	I	7.500	0.80	0.736	1.19	А	I	40.100	
	СТО (ТТ	TNT7B	42.000		1.22	51.24	1.40	0.743	2.27	А	I	40.100	1.034	1.68	А	I	7.500	0.80	0.736	1.22	А	I	40.100	
	TRA(TNAGRIT4	43.000		1.17	50.31	1.40	0.743	2.17	А	I	40.100	1.034	1.70	А	I	7.500	0.80	0.736	1.17	А	I	40.100	
	UCK	TNAGT5A	45.000		1.10	49.50	1.40	0.743	2.05	А	I	40.100	1.034	1.68	А	I	7.500	0.80	0.736	1.10	А	I	40.100	
	TRI	TNAGT5B	45.000	$\overline{3}$	1.09	49.05	1.40	0.743	2.03	А	I	40.100	1.034	1.63	А	I	7.500	0.80	0.736	1.09	А	I	40.100	1





×			
018	ASSEMBLED BY : D.D.LOWE CHECKED BY : J.C.WILSON	RY DATE : DATE :	10/18 10/18
12/7/2		REV. II/I2/08RR REV. I0/I/II REV. I2/I7	MAA/GM MAA/GM MAA/THC

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

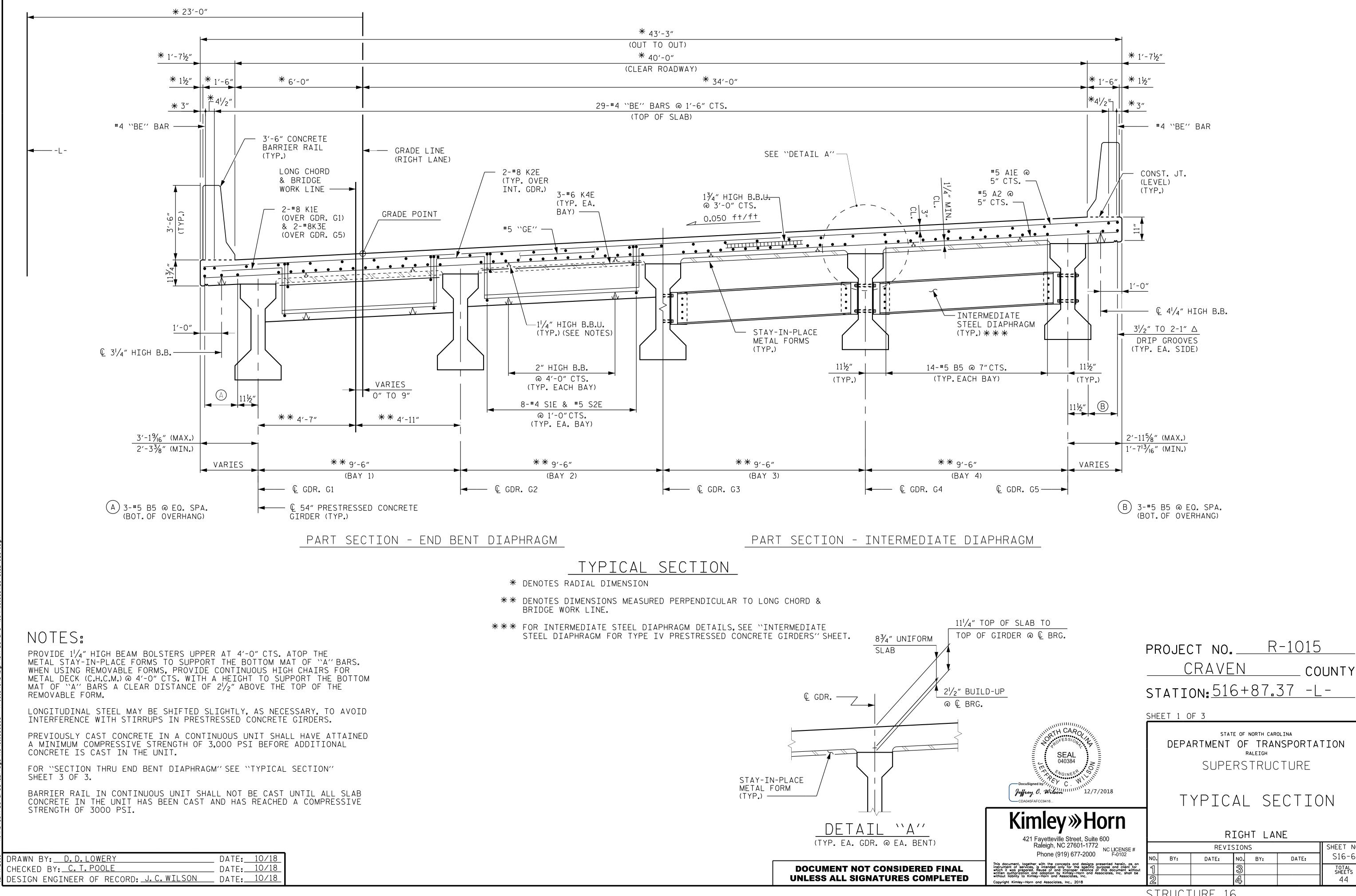
- 1. 2.
- 3.
- 4.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
$\left< \frac{3}{3} \right>$ LEGAL LOAD RATING **
* * SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER – EXTERIOR RIGHTGIRDER

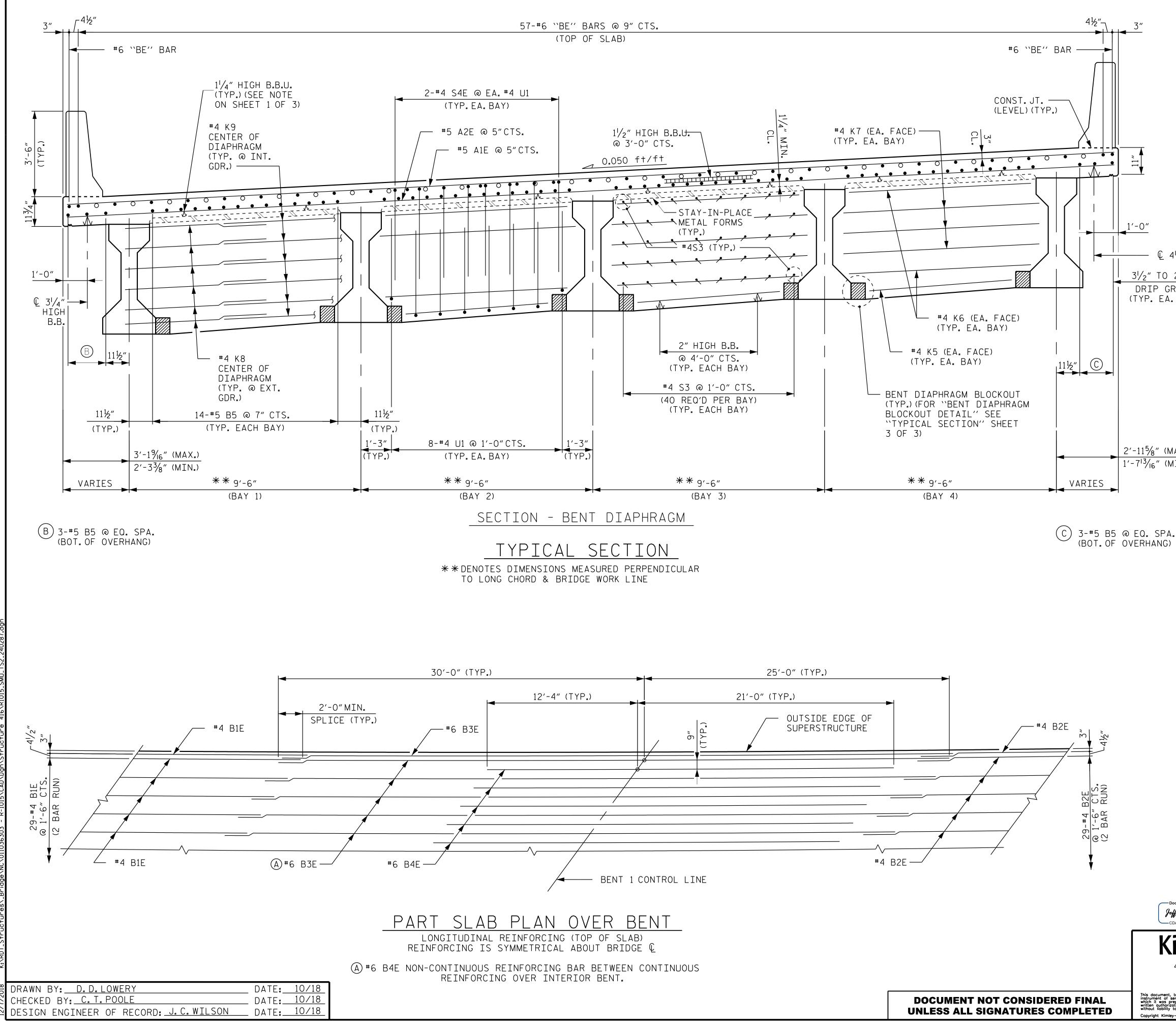
	PROJEC <u>C</u> STATIO	RAVE	<u>N</u>		0 UNTY
DocuSigned by: Jeffrey C. Wilson CDA045FAFCC9416		RTMENT S RFR_S	raleigh TANDAF	NSPORTA RD RY_F	
421 Fayetteville Street, Suite 600	C((NOI	DNCRE	TE G	IRDE E traff	RS FIC)
Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102	NO. BY:	REVIS	SIONS	DATE:	SHEET NO. S16-5
This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc. Copyright Kimley-Horn and Associates, Inc., 2018	1		8 종 좌		total sheets 44

STRUCTURE 16

STD. NO. LRFR1



SHEET NO S16-6 TOTAL SHEETS 44



	PROJEC C STATIC	RAVE	IN .	• • •	UNTY					
	SHEET 2 OF 3									
DocuSigned by: Jeffrey. C. Wilson: CDA045FAFCC9416	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE									
Jeffrey. C. Wilson 12/7/2018 CDA045FAFCC9416	T ,	YPTC	al se	ECTTC)N					
Kimley »Horn			IGHT LA							
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102		REVIS	SIONS		SHEET NO.					
ument, together with the concepts and designs presented herein, as an th of services, is intended only for the specific purpose and client for was prepared. Reuse of and improper reliance of this document without ulthorization and adaption by Kimley-Horn and Associates, Inc. shall be iobility to Kimley-Horn and Associates, Inc. t Kimley-Horn and Associates, Inc., 2018	№. вү: 1 2	DATE:	NO. ВҮ: 3 4	DATE:	S16-7 total sheets 44					
	STRUCT	URE 16	$\hat{\mathbf{D}}$							

— € 4¼″ HIGH B.B.

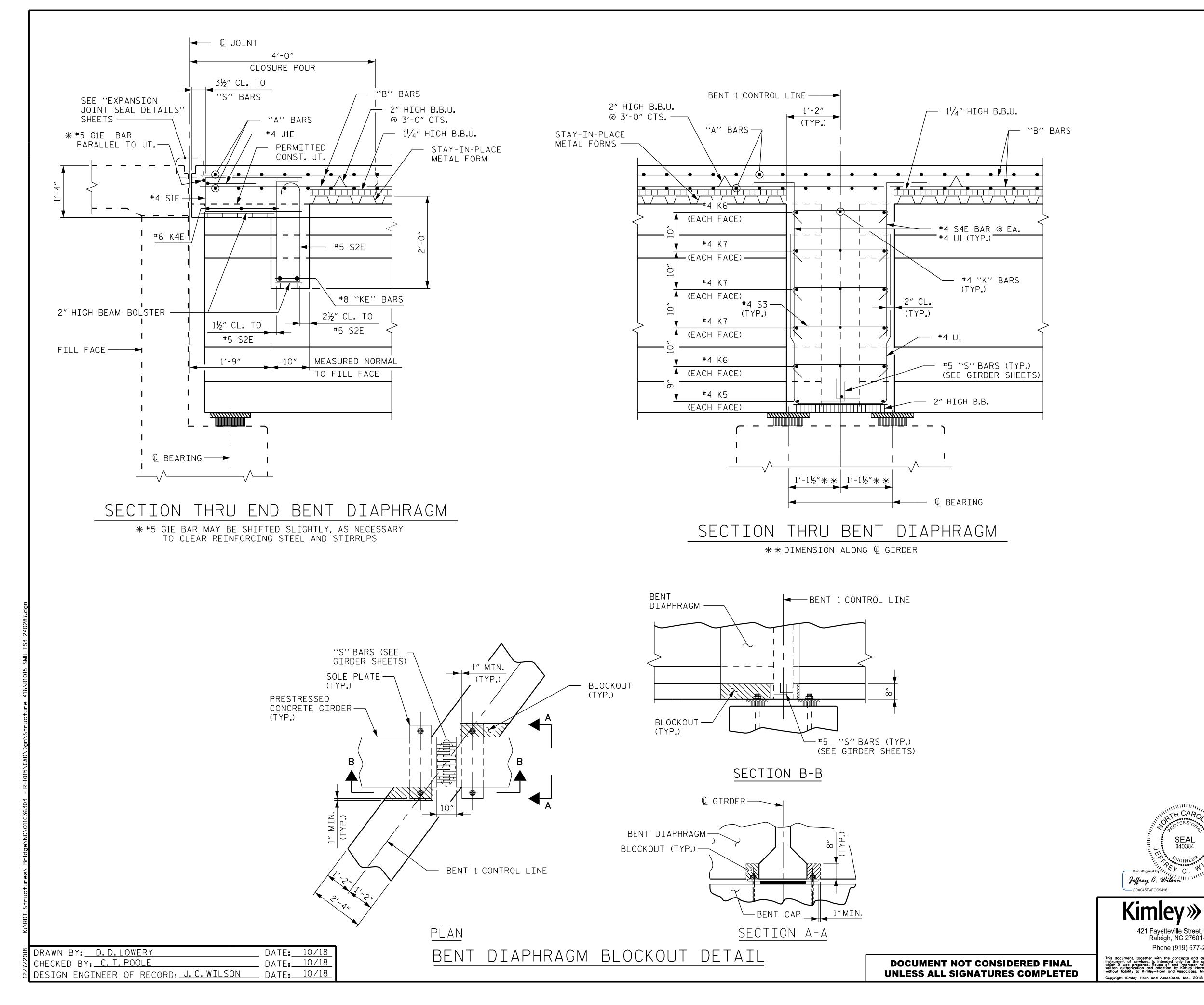
3^I/₂″ TO 2-1″ ∆ DRIP GROOVES (TYP. EA. SIDE)

2'-115/8" (MAX.) $1' - 7^{13}/_{16}$ " (MIN.)

STEEL OVER BENT. • INDICATES CONTINUOUS REINFORCING STEEL FROM END BENT 1 TO END BENT 2.

• INDICATES NON-CONTINUOUS REINFORCING

NOTE: FOR SUPERSTRUCTURE NOTES, SEE ``TYPICAL SECTION'', SHEET 1 OF 3.



PROJECT NO. R-1015 CRAVEN

COUNTY

STATION: 516+87.37 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE

TYPICAL SECTION

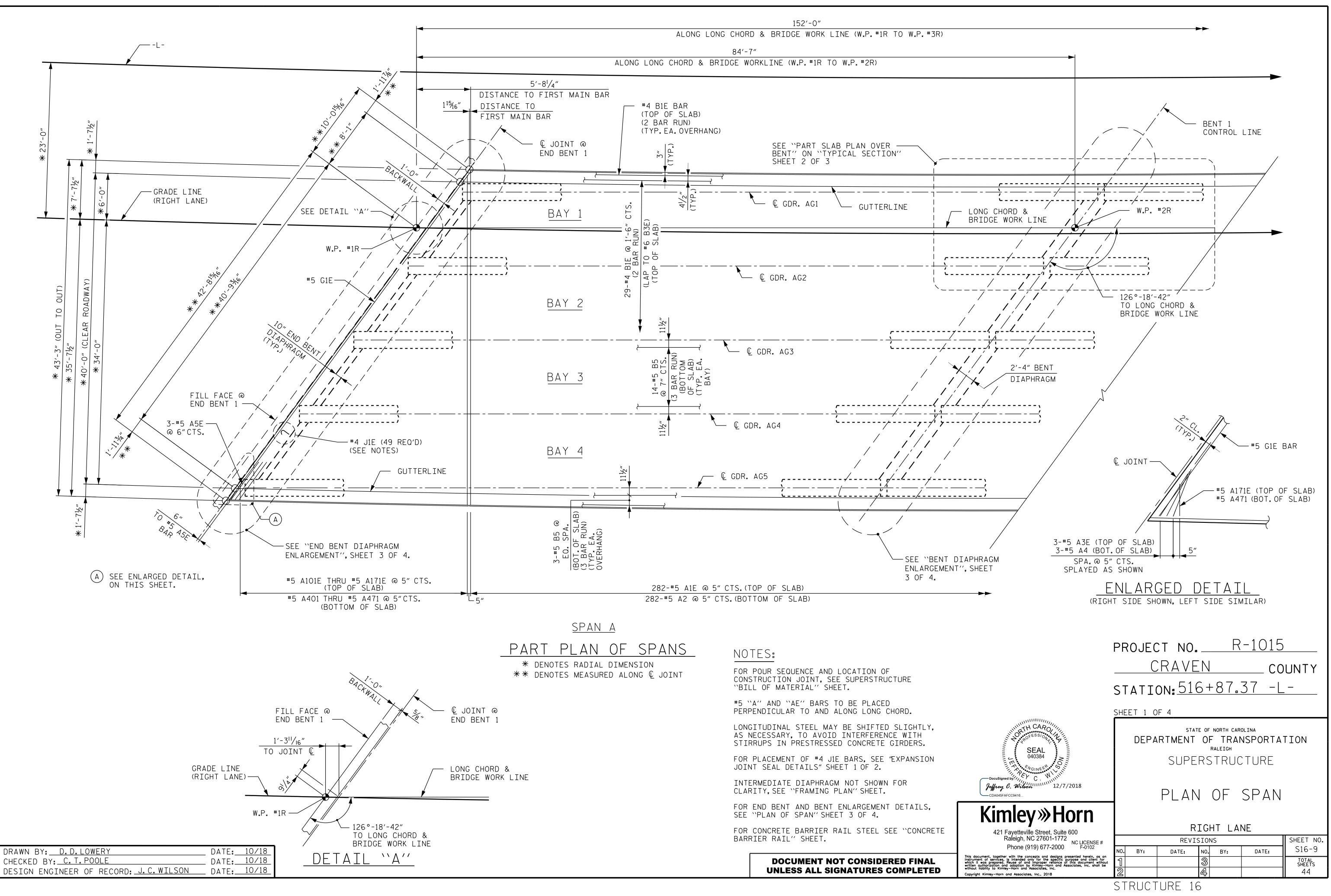
RIGHT	LANE
-------	------

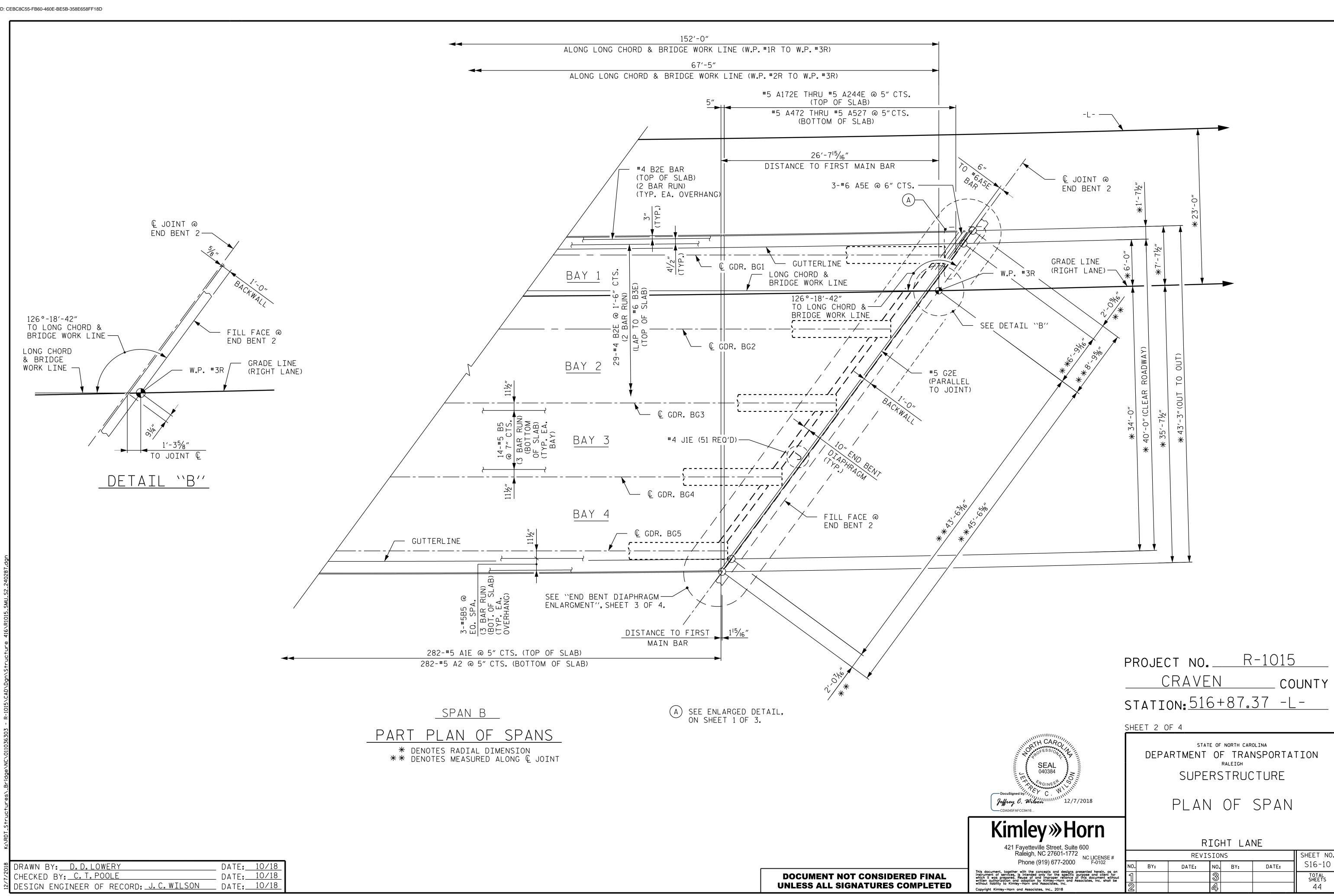
		SHEET NO.								
10.	BY:	DATE:	N0.	BY:	DATE:	S16-8				
1			ß			TOTAL SHEETS				
2			4			44				
· T	TDUCTUDE 16									

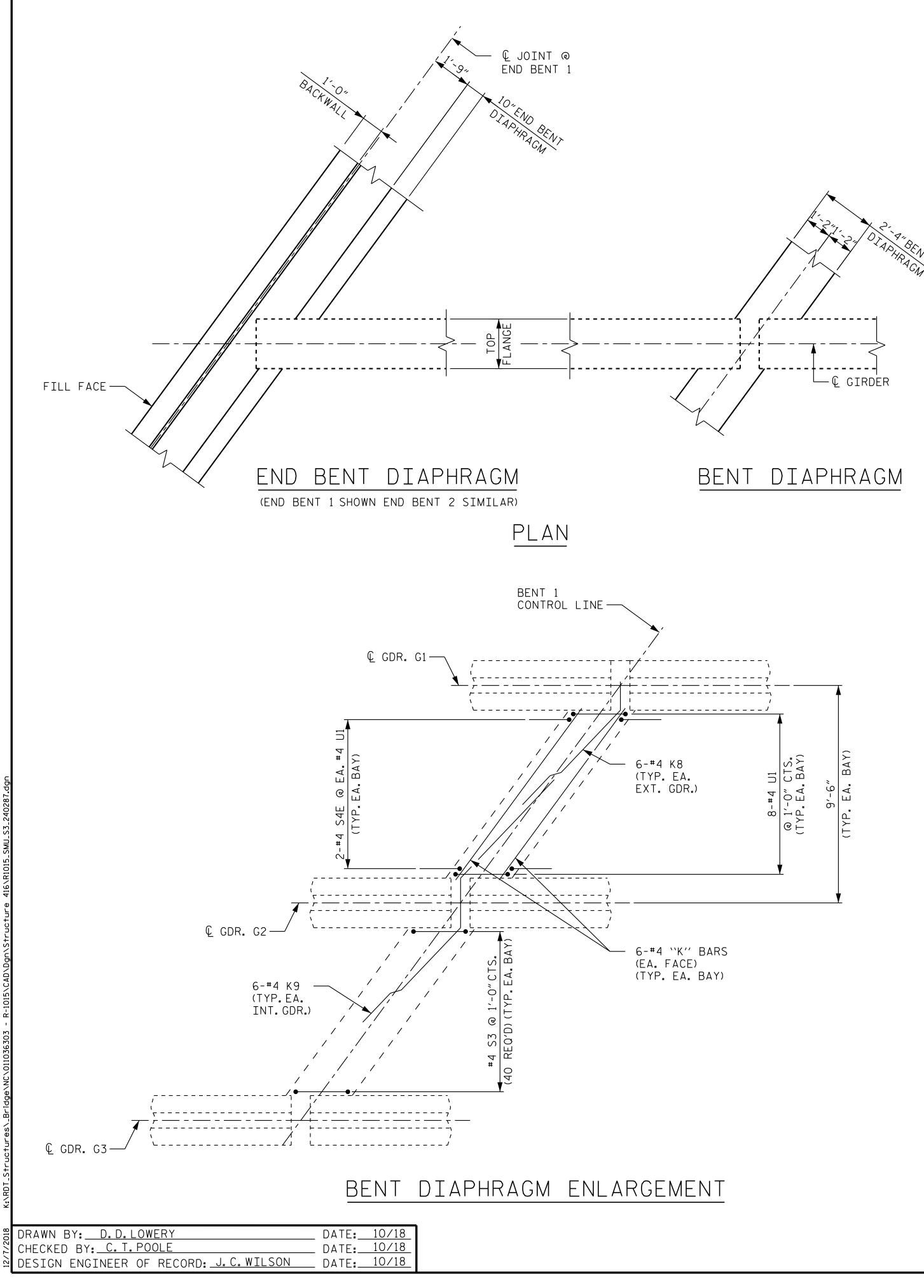


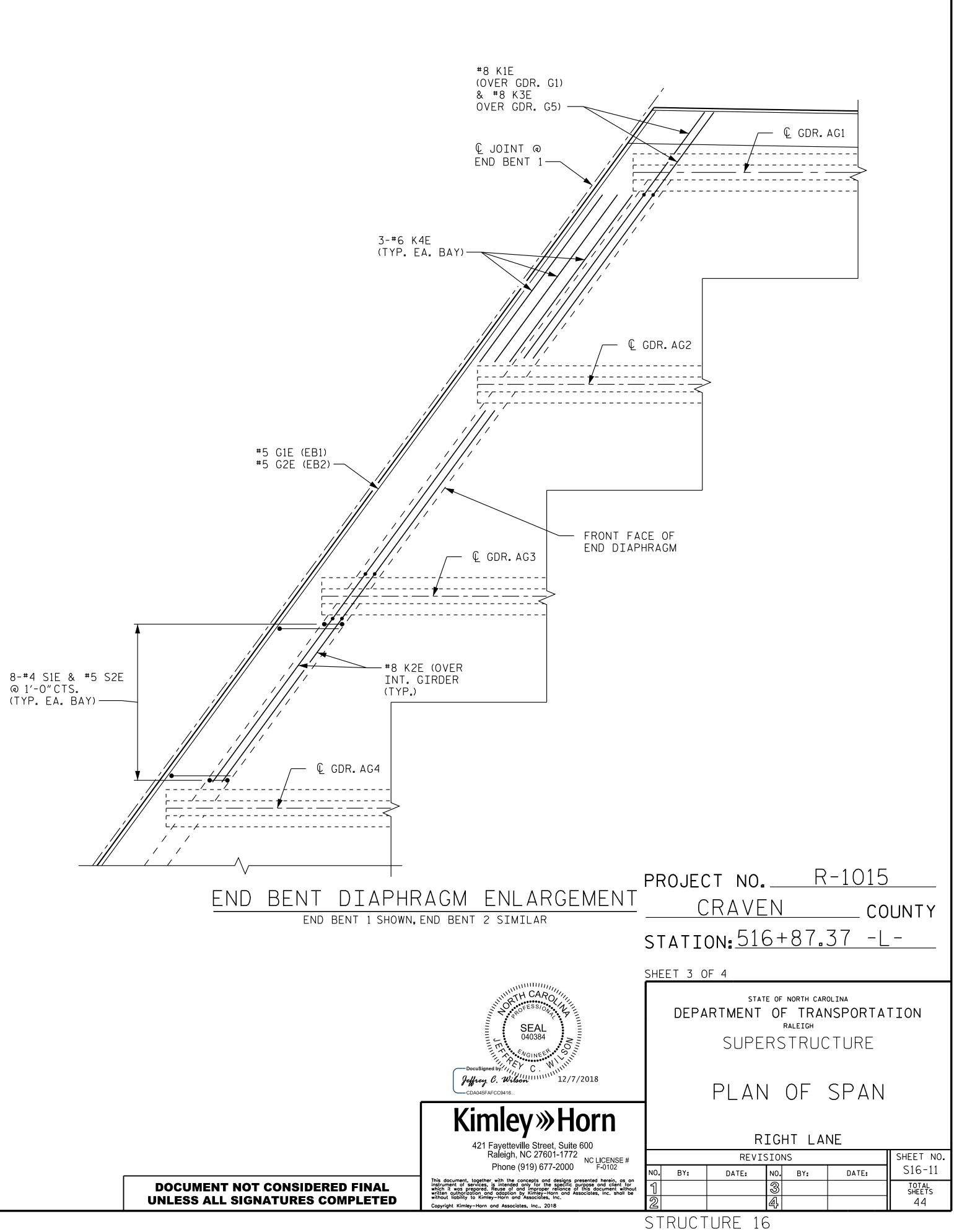
Kimley Worn

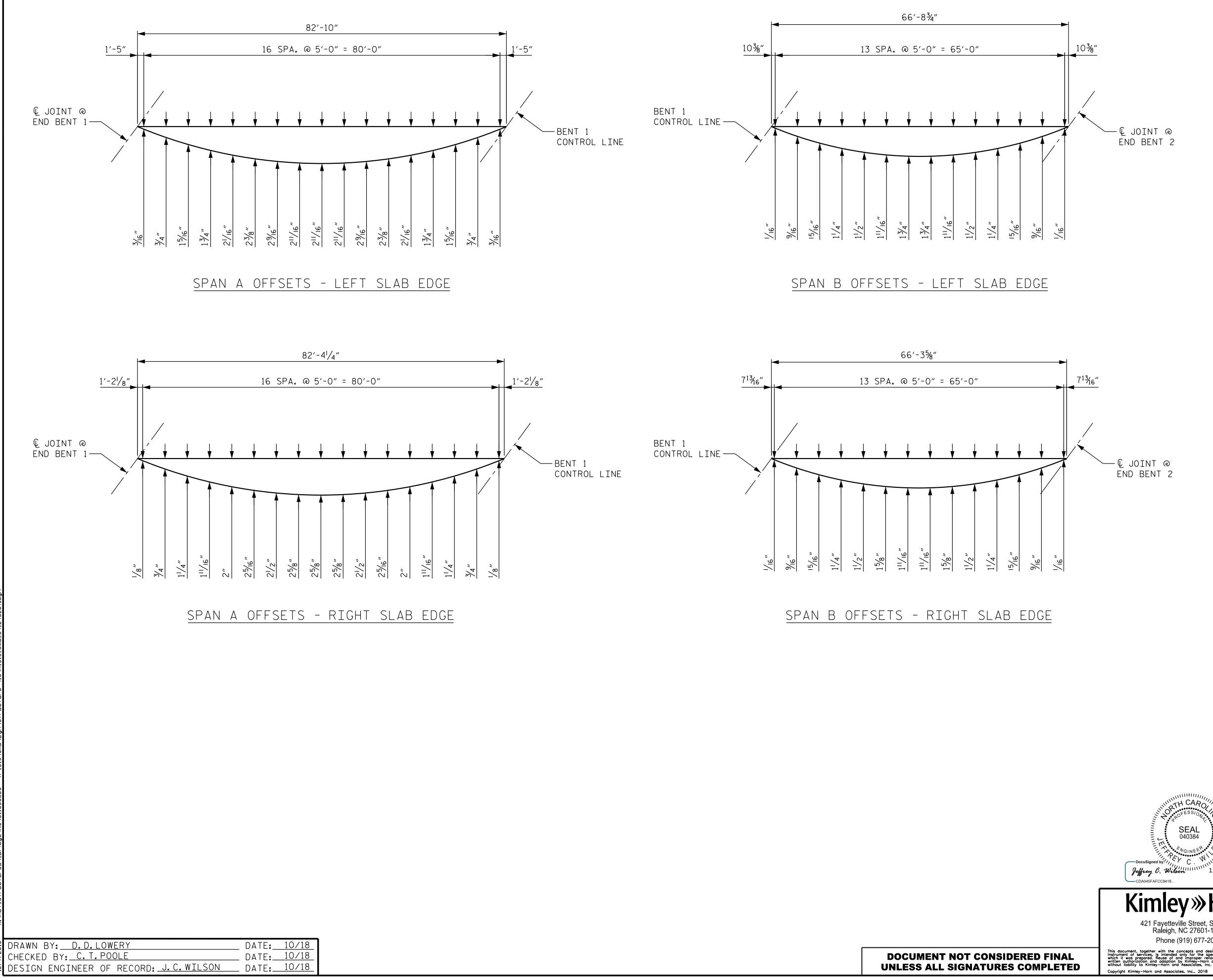
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE # F-0102







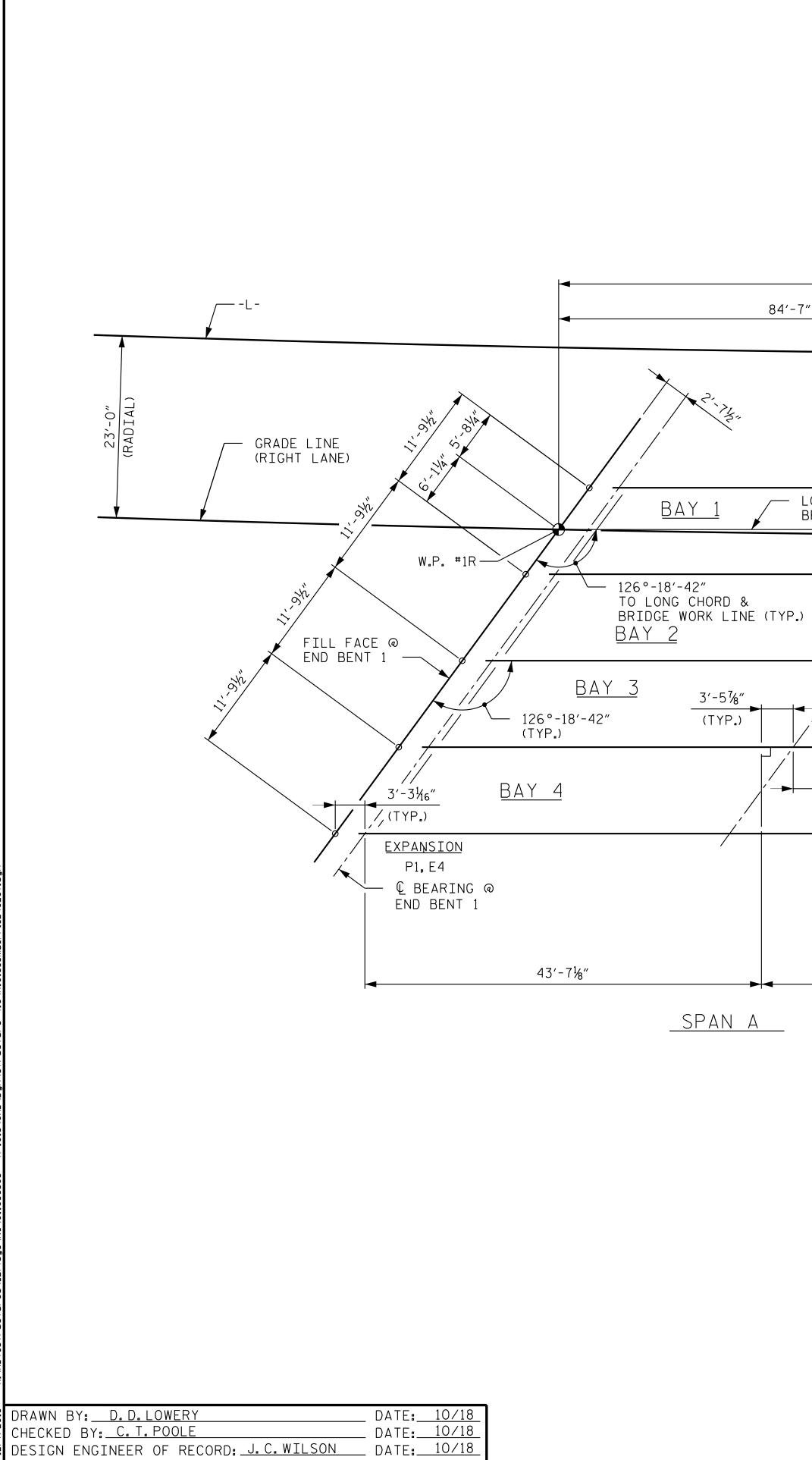




-€JOINT @ END BENT 2

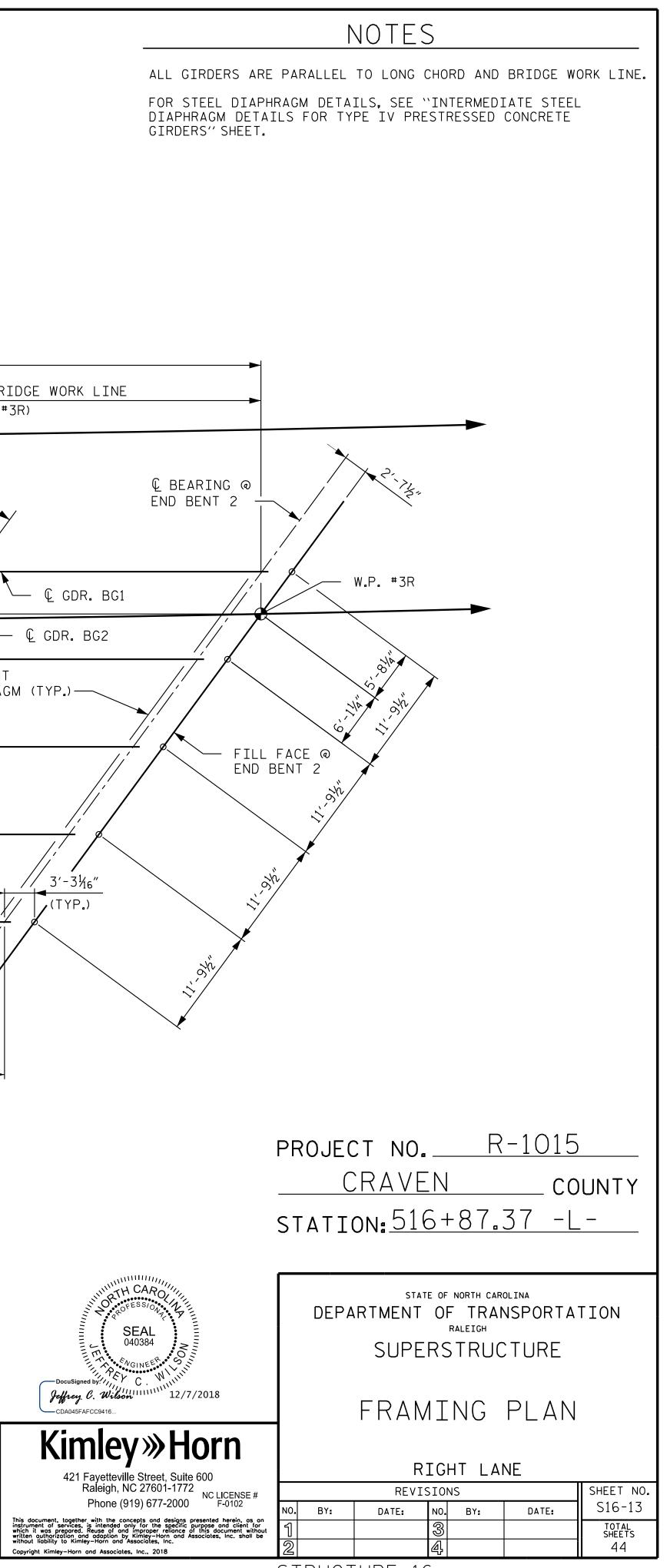
— € JOINT @ END BENT 2

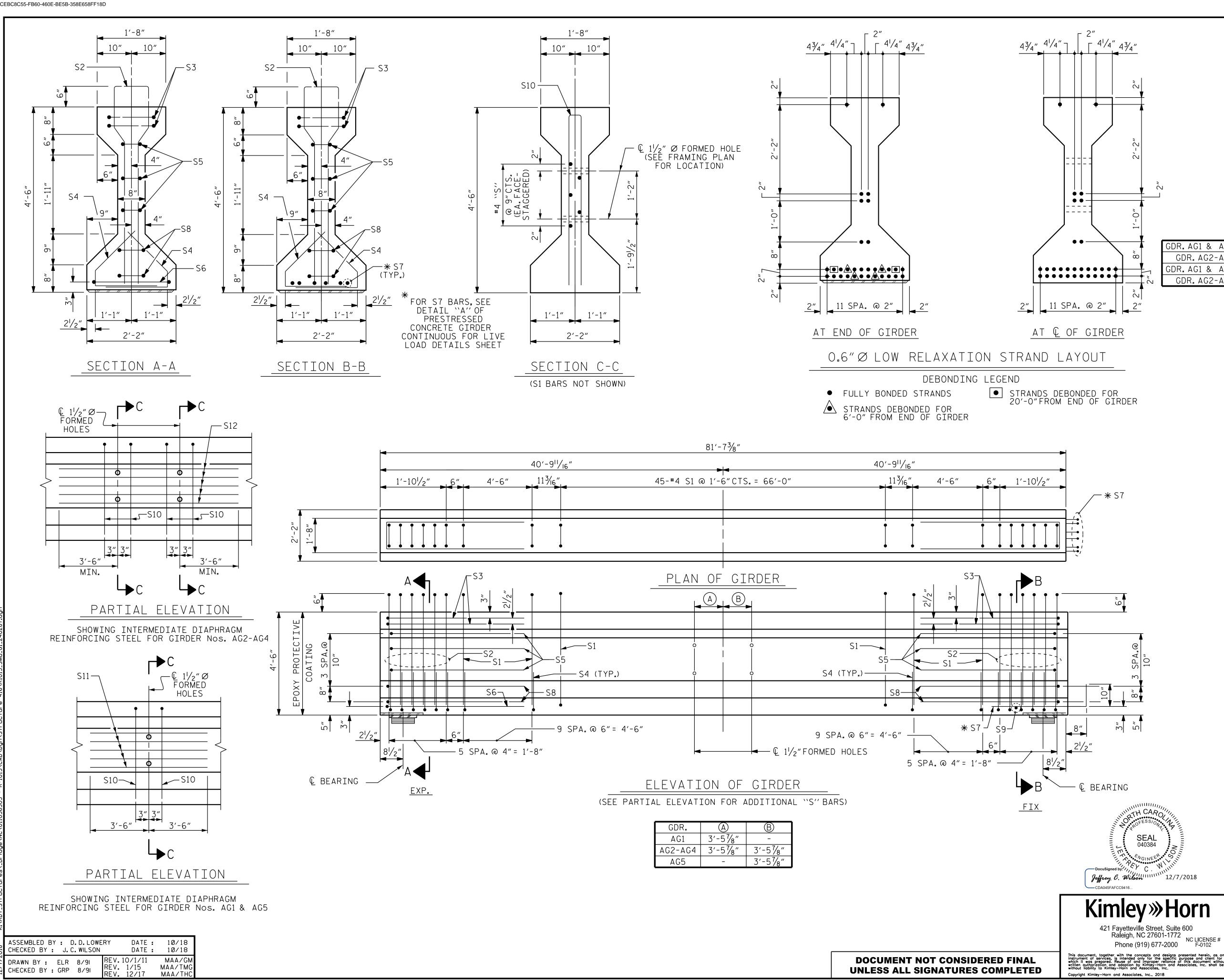
		С	ct no. CRAVE On: 51	<u>EN</u>			UNTY
SHEET 4 OF 4							
DocuSigned by: Jeffrey C. Wilson Jeffrey C. Wilson Jeffrey C. Wilson Jeffrey C. Wilson Jeffrey C. Wilson	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE						TION
CDA045FAFCC9416			PLAN	\mathbf{V}	DF	SPAN	
Kimley » Horn			R	IGH	T LAI	NE	
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 NC LICENSE #	REVISIONS SHEET NO.						
Phone (919) 677-2000 F-0102 document, together with the concepts and designs presented herein, as an	NO.	BY:	DATE:	NO.	BY:	DATE:	S16-12
document, together with the concepts and designs presented herein, as an ument of services, is intended only for the specific purpose and client for h it was prepared. Reuse of and improper reliance of this document without en authorization and adaption by Kimley-Horn and Associates, Inc. shall be but liability to Kimley-Horn and Associates, Inc.	1 2			3 4			TOTAL SHEETS 44
right Kimley—Horn and Associates, Inc., 2018	凶			5			44



	(W.P. #1R TO W.P. #3R)		
7"ALONG LONG CHORD & BRIDGE WORK LINE (W.P. #1R TO W.P. #2R)			ONG CHORD & BRIDGE V.P.#2R TO W.P.#3R)
© INTERMEDIATE DIAPHRAGM	BENT 1 CONTROL LI & BEARING —	NE —	ERMEDIATE
LONG CHORD &	C GDR. AG1		
	W.P. #2R		
,) 	DR. AG2	1, 1, 0, 1, 1, 0, 1/2 0, 1, 1, 0, 1/2 0, 1, 1, 0, 1/2	END BENT DIAPHRAGM (
INTERMEDIATE © GDR. AG3 STEEL DIAPHRAGMS (TYP.)	// 126°-18'-42 (TYP.)		└── (L GDR. BG3
$\frac{3'-5\sqrt[7]{8''}}{(TYP.)}$ © GDR. AG4 $\frac{1'-1\frac{1}{2}''}{(TYP.)}$	1'-1 ¹ /2" (TYP.)	Ç GI	DR. BG4
© GDR. AG5 <u>FIXED</u> P2, E4	P2, E4	Ç GDR. BG5	EXPANSION P1, E4
36′-75⁄ ₁₆ ″ ►	35′-0⅛″	28′-0	¹⁵ ∕16″
		SPAN B	
<u>Framing</u> f	<u>PLAN</u>		

152'-O"ALONG LONG CHORD & BRIDGE WORK LINE (W.P. #1R TO W.P. #3R)

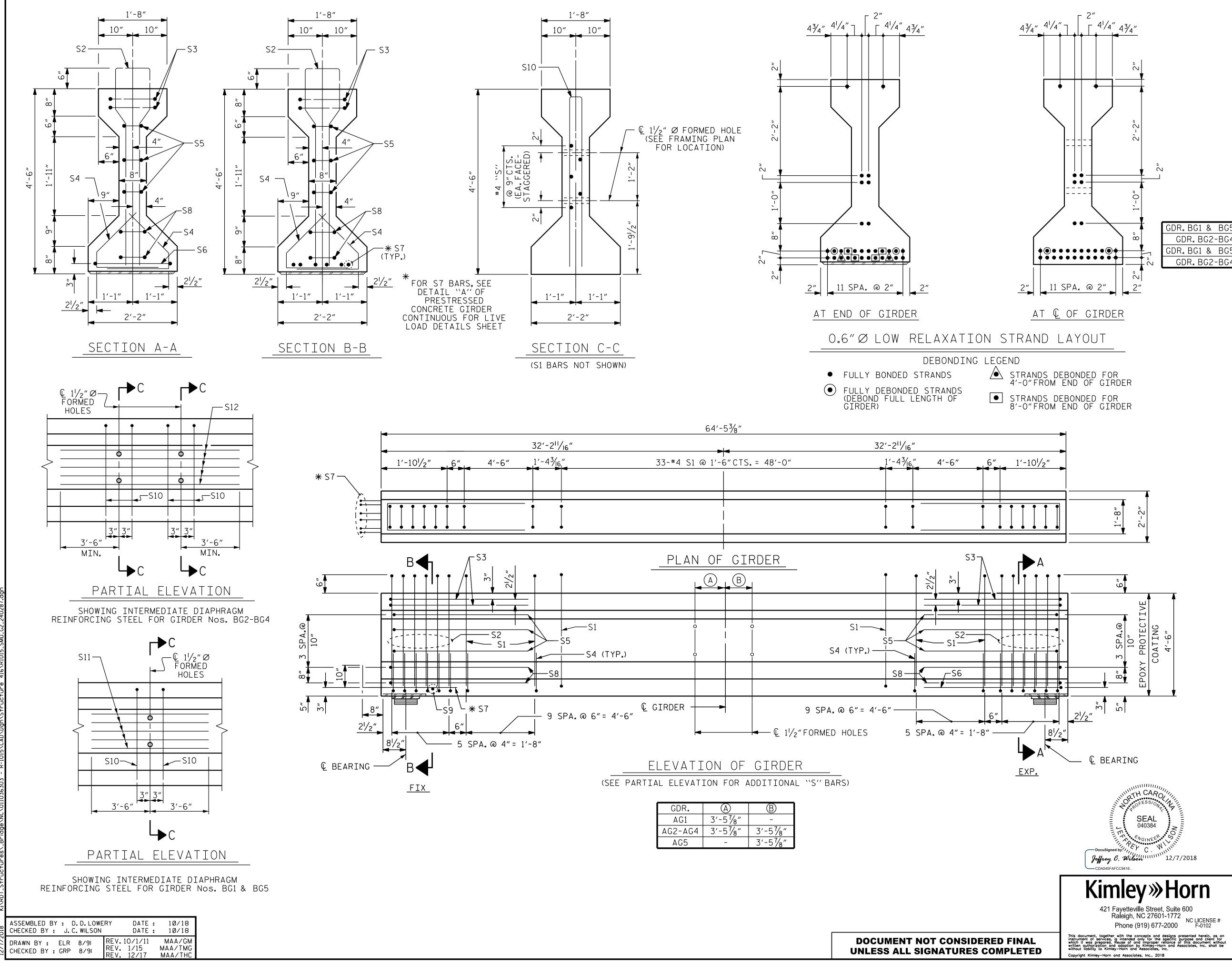




	0.6″ 4	ØL.R	.GRAD	E 270) STI	RANDS	
		ULTIMATE			APPLIED		
	ARI		STRE	NGTH	PRE	STRESS	
		INCHES)	(LBS. PER			ER STRAND)	
	0.2	17	58,6	00	43	3,950	
	REINFO	ORCING	STEEI	FOR	ONE	GIRDER	
	BAR	NUMBER	SIZE	TYPE	LENGTH		
	S1	65	#4	1	10'-8"		
	S2 S3	12 4	#6 #4	1	10'-8' 9'-1"		
	S3S4	64	#4	3	3'-5"		
	S5	6	#4	2	8'-5"	_	
	S6 * S7	1	#4 #5	2 STR	9'-11" 3'-8"		
	S8	4	#4	2	8'-7"		
CE	S9	1	#3 #5	STR	1'-10" 8'-8"		
AG5 AG4	S10 S10	4	#5 #5	2	8'-8"		
\G5	S11	5	#4	STR	7'-0"	23	
AG4	S12	5	#4	STR	14'-0"	47	
	* NC		L BARS_SH			EFORE	
		SHI NOT		HEAT BEI OWED.	NDING	SHALL	
			BAR ⁻	ΓΥΡΕ S			
		all Bar	DIMENSIO	NS ARE OL			
	S1		10″	1	1'-1" S3	5	
					5″ S5		
	1'-3″ 1'-0″				l'-11″ s	6	
			(1)		7″ S8		
	41/4 41/4				4″ S10)	
				1			
	3'-2 ¹ /4" 3'-51/."	× 4″		Γ			
	ά Μ	ר ר 	┥ ┝━─		(2)	4'-2"	
			/	·		4	
		115/6"	1.4"				
		¥	_/ //	3)	(510 S5, & S8	
			1'-6"			S3, S	
						10,	
	QU/		ES FO				
			REINFORCI STEEL		PSI C	0.6″ØL.R. STRANDS	
			LB.		Υ.	No.	
	GDR. AG1 GDR. AG		954 996	16 16	.6	32 32	
			RDERS			JL	
	NUM				1	_ LENGTH	
	5		81'-7	' 3/8″	408	8'-07/8"	
					\circ \cdot $-$		
Ρ	ROJE	CT NO)	K-1	015		
	(CRAV	FN			UNTY	
				7 7 7			
S	TATI	0N: <u>)</u>	10+9	1.31	<u> </u>		
Sł	HEET 1 (DF 4					
			TATE OF NOR				
	DEP	ARTMEN	IT OF T RALEI		ORTAT	ION	
			STAN				
			HTO T	YPF	$T\overline{V}$		
	PRFST			NCRF	TF C	IRDER	
_			US F				
			(SPAN		. v ட. L		
			RIGHT				
F			VISIONS			SHEET NO. S16-14	
n Z		DATE:	NO. B	r: D	ATE:	TOTAL SHEETS	
° .	2		4 4			SHEETS 44	
C	TRUC	TURF	16	5-	ED NI	D.PCG6	

STRUCTURE 16

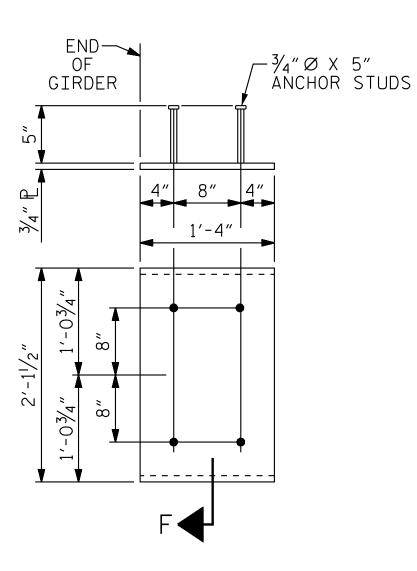
STD. NO. PCG6



	0.6″ 4	ØL.R	. GRAD)E 270) STR	ANDS
						LIED
	ARI	ΞA	ULII STRE	MATE NGTH		LIED TRESS
	(SQUARE	INCHES)	(LBS. PER		(LBS. PER	STRAND)
	0.2	17	58,6	00	43.	950
	REINFO	DRCING	STEEI	_ FOR	ONE (GIRDER
	BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
	S1 S2	53 12	#4 #6	1	10'-8" 10'-8"	378 192
		4	#6 #4	2	10 -8 9'-1"	24
	S4	64	#4	3	3'-5"	146
	S5	6	#4	2	8′-5″	34
	S6	1	#4	2	9'-11"	7
	* S7 S8	6	#5 #4	STR 2	3'-8" 8'-7"	23 23
	S9	1	#3	STR	1'-10"	1
5	S10	2	#5	2	8′-8″	18
4	S10	4	#5	2	8'-8"	36
5 4	S11 S12	5	#4 #4	STR STR	7'-0" 14'-0"	23 47
-	512	5		311	17 0	
	* NC		BARS SH			ORE
		SH1 NOT		HEAT BEN OWED.	NDING SH	HALL
			BAR	TYPES		
		ALL BAR	DIMENSIO	NS ARE OL	JT-TO-OUT	
			10″		1′-1″ S3	
	S1				5″ S5	
	1'-0"					
	1'	-	(1)		'-11″ S6	
			\sim)		7″ <u>58</u>	
	41/2				4″ S10	
	× ×					
	3'-2 /4" 3'-5 /,"	25 ³ /4"		Γ		
	д, - З, -	4" b	┥┝			
	V	¥				
	_	•			(2) ×	4,-0
		, , , 9			4، -2"	4′-0″
		115/6"				↓ ↓ _m
	<u> </u>	", 11 ⁵ / ₆ "		3)	S10 4'-2	55 , k S8
		7" 115/6"		3		3, 55, & S8
		7" 115/6"	- <u>1'-6"</u>	3		S5, S8
	QUA	7" 11 ⁵ / ₆ "		3		3, 55, & S8
	QUA		ES FO	3) R ONE NG 6500	OIS OIS FSI O.6	S6 & S8 S6 & S8
	QUA		ES FO REINFORCI	3) R ONE	GIRD PSI 0.6 RETE S	ER S [°] ØL.R. TRANDS
			ES FO REINFORCI STEEL LB.	3 RONE NG 6500 CONC	GIRD PSI 0.6 RETE S	ER 5″ØL.R. TRANDS No.
	QU/ GDR. BG1 GDR. BG1	ANTITI & BG5	ES FO REINFORCI	3) R ONE	GIRD PSI 0.6 RETE S Y.	ER S [°] ØL.R. TRANDS
		► ANTITI & BG5 2-BG4	ES FO REINFORCI STEEL LB. 869	3) R ONE NG 6500 CONC C. 13 13	OIS OIS OIS OIS OIS OIS OIS OIS	ER 5" Ø L. R. TRANDS No. 32
		ANTITI & BG5 2-BG4 GIF	ESFO REINFORCI STEEL LB. 869 911	3) R ONE NG 6500 CONC C. 13 13 REQUIF	OIS OIS OIS OIS OIS OIS OIS OIS	ER 5" Ø L. R. TRANDS No. 32 32
	GDR. BG	ANTITI & BG5 2-BG4 GIF BER	ES FO REINFORCI STEEL LB. 869 911 RDERS	3) R ONE NG 6500 CONC C. 13 13 REQUIR GTH	GIRD PSI 0.6 RETE S Y. .1 .1 .1 .1	ER "ØL.R. TRANDS No. 32 32 LENGTH
	GDR. BG	ANTITI & BG5 2-BG4 GIF BER	ES FO REINFORCI STEEL LB. 869 911 RDERS LENG	3) R ONE NG 6500 CONC C. 13 13 REQUIR GTH	GIRD PSI 0.6 RETE S Y .1 .1 .1 .1 .1 .1 .1 .1 .1 .1	ER "ØL.R. TRANDS No. 32 32 LENGTH
	GDR. BG	ANTITI & BG5 2-BG4 GIF BER	ES FO REINFORCI STEEL LB. 869 911 RDERS LENG	3) R ONE NG 6500 CONC C. 13 13 REQUIR GTH	GIRD PSI 0.6 RETE S Y .1 .1 .1 .1 .1 .1 .1 .1 .1 .1	ER "ØL.R. TRANDS No. 32 32 LENGTH
	GDR. BG	ANTITI & BG5 2-BG4 GIF 3ER	ES FO REINFORCI STEEL LB. 869 911 RDERS LENC 64'-5	3) R ONE NG 6500 CONC C. 13 13 REQUIR 5%"	GIRD PSI 0.6 RETE S Y. .1 .1 RED TOTAL 322'-	ER "ØL.R. TRANDS No. 32 32 LENGTH
P	GDR. BG	ANTITI & BG5 2-BG4 GIF 3ER	ES FO REINFORCI STEEL LB. 869 911 RDERS LENC 64'-5	3) R ONE NG 6500 CONC C. 13 13 REQUIR GTH	GIRD PSI 0.6 RETE S Y. .1 .1 RED TOTAL 322'-	ER "ØL.R. TRANDS No. 32 32 LENGTH
P	GDR. BG	ANTITI & BG5 2-BG4 GIF BER CTN(LENC LENC CALLENCE CALE	3) R ONE NG 6500 CONC C. 13 13 REQUIR 5%"	GIRD PSI 0.6 RETE S Y. .1 .1 RED TOTAL 322'-	ER 5" Ø L. R. TRANDS No. 32 32 LENGTH
	GDR. BG NUMI 5 ROJE(ANTITI & BG5 2-BG4 GIF BER CTNC CTNC CTNC	ESFO REINFORCI STEEL LB. 869 911 RDERS LENC 64'-5	3) R ONE NG 6500 СОNC С. 13 13 REQUIF GTH 53% "	GIRD PSI 0.6 PSI 0.6 Y. 0.6 PSI 0.6 SY. 0.6	ER S"ØL.R. TRANDS NO. 32 32 LENGTH 27%"
	GDR. BG NUMI 5 ROJE(ANTITI & BG5 2-BG4 GIF BER CTNC CTNC CTNC	LENC LENC CALLENCE CALE	3) R ONE NG 6500 СОNC С. 13 13 REQUIF GTH 53% "	GIRD PSI 0.6 PSI 0.6 Y. 0.6 PSI 0.6 SY. 0.6	ER S"ØL.R. TRANDS NO. 32 32 LENGTH 27%"
S	GDR. BG	ANTITI & BG5 2-BG4 GIF 3ER CTNC CTNC CTNC CRAV ON: 5	ESFO REINFORCI STEEL LB. 869 911 RDERS LENC 64'-5	3) R ONE NG 6500 СОNC С. 13 13 REQUIF GTH 53% "	GIRD PSI 0.6 PSI 0.6 Y. 0.6 PSI 0.6 SY. 0.6	ER S"ØL.R. TRANDS NO. 32 32 LENGTH 27%"
S	GDR. BG NUMI 5 ROJE(ANTITI & BG5 2-BG4 GIF 3ER CTNC CTNC CTNC CRAV ON: 5	ESFO REINFORCI STEEL LB. 869 911 RDERS LENC 64'-5	3) R ONE NG 6500 СОNC С. 13 13 REQUIF GTH 53% "	GIRD PSI 0.6 PSI 0.6 Y. 0.6 PSI 0.6 SY. 0.6	ER S"ØL.R. TRANDS NO. 32 32 LENGTH 27%"
S	GDR. BG	ANTITI & BG5 2-BG4 GIF BER CTNC CTNC CRAV ON: 5 OF 4	ES FO ES FO REINFORCI STEEL LB. 869 911 RDERS LENC 64'-5 0. 16+8 TATE OF NOR	3) R ONE NG 6500 CONC C. 13 13 REQUIF 37 7.37 TH CAROLINA	GIRD PSI 0.6 Y. 0.6 Y. 0.6 Y. 0.6 Y. 0.6 SY. 0.6 S	ER S"ØL.R. TRANDS No. 32 32 LENGTH -27/8"
S	GDR. BG	ANTITI & BG5 2-BG4 GIF BER CTNC CTNC CRAV ON: 5 OF 4	LENC LENC CATE OF NOR LENC CATE OF NOR LENC CATE OF NOR LENC CATE OF NOR LENC CATE OF NOR LENC CATE OF NOR	3 R ONE NG 6500 CONC C. 13 13 REQUIF GTH 5% R - 1 7.37 TH CAROLINA FRANSP	GIRD PSI 0.6 Y. 0.6 Y. 0.6 Y. 0.6 Y. 0.6 SY. 0.6 S	ER S"ØL.R. TRANDS No. 32 32 LENGTH -27/8"
S	GDR. BG	ANTITI & BG5 2-BG4 GIF BER CTNC CTNC CRAV ON: 5 OF 4	LENC C I	3 R ONE NG 6500 CONC C. 13 13 REQUIF 37 7.37 TH CAROLINA TH CAROLINA TH CAROLINA CAROLINA	GIRD PSI 0.6 Y. 0.6 Y. 0.6 Y. 0.6 Y. 0.6 SY. 0.6 S	ER S"ØL.R. TRANDS No. 32 32 LENGTH -27/8"
S	GDR. BG	ANTITI ANTITI ANTITI BER CT NO CT NO CRAV ON: 5 OF 4	LENC LENC CALLENCE CALE	3) R ONE NG 6500 CONC C. 13 13 REQUIF 3%" REQUIF 3%" REQUIF 37.37 TH CAROLINA TH CAROLINA TH CAROLINA CAROLINA CAROLINA CAROLINA	GIRD PSI 0.6 Y. 0.6 Y. 0.6 Y. 0.6 Y. 0.6 SY. 0.6 S	ER S"ØL.R. TRANDS No. 32 32 LENGTH -27/8"
S SF	GDR. BG	ANTITI ANTITI ANTITI BER CT NO CT NO CRAV ON: 5 OF 4 ARTMEN AASI	LENC C I	3 R ONE R ONE NG 6500 CONC C. 13 13 REQUIF 37. 7. 7. 7. 7. 7. 7. 7. 7. 7.	GIRD PSI 0.6 Y. 0.6 PSI 0.6 Y. 0.6 Y. 0.6 Y. 0.6 S Y. 0.7 Y. 0.7	ER 5" Ø L. R. TRANDS No. 32 32 LENGTH 27/8" NTY
S SF	GDR. BG	ANTITI ANTITI	LENC LENC CALLENCE CALE	3 R ONE NG 6500 CONC C. 13 13 REQUIF 37.37 TH CAROLINA TH CAROLINA TANSP IGH DARD YPE NCRE	GRTATI	ER S"ØL.R. TRANDS No. 32 32 LENGTH 27%" NTY ON
S SF	GDR. BG	ANTITI ANTITI	LENC C I	3 R ONE NG 6500 CONC C. 13 13 REQUIF 37.37 TH CAROLINA TH CAROLINA TANSP IGH DARD YPE NCRE	GRTATI	ER STRANDS NO. 32 32 LENGTH 27% NTY ON
S SF	GDR. BG	ANTITI ANTITI	LENC LENC C C C C C C C C C C C C C	3 R ONE R ONE NG 6500 CONC C. 13 13 13 REQUIF GTH 5%" R - 1 7.37 TH CAROLINA RANSP IGH D A R D TH CAROLINA IGH D A R D Y PE NCRE D A R D Y PE NCRE D A R D Y PE NCRE D A R D	GRTATI	ER STRANDS NO. 32 32 LENGTH 27% NTY ON
S SF	GDR. BG	ANTITI ANTITI ANTITI BER GIF BER CT NO CT NO	LENC C C C C C C C C C C C C C	3 R ONE R ONE NG 6500 CONC C. 13 13 13 REQUIF GTH 5%" R - 1 7.37 TH CAROLINA RANSP IGH D A R D TH CAROLINA IGH D A R D Y PE NCRE D A R D Y PE NCRE D A R D Y PE NCRE D A R D	GRTATI ORTATI ORTATI ORTATI	ER ST Ø L. R. TRANDS NO. 32 32 LENGTH 27% NTY ON
SF	GDR. BG	ANTITI ANTITI	LENC C C C C C C C C C C C C C	3 R ONE R ONE NG 6500 CONC C. 13 13 REQUIF 37. 7. 7. 7. 7. 7. 7. 7. 7. 7.	GRTATI ORTATI ORTATI ORTATI	ER ST Ø L. R. TRANDS NO. 32 32 LENGTH 27/8" NTY ON CON
St	<u>GDR. BG</u> NUM 5 ROJE((ТАТІ неет 2 (DEP/ PREST CON	ANTITI ANTITI ANTITI BER GIF BER CT NO CT NO	LENC C I I'-6" ESFO REINFORCI STEEL LB. 869 911 RDERS LENC 64'-5 CIN 16+8 TATE OF NOR 16+8 TATE OF NOR 16+8 TATE OF NOR 10 10 10 10 10 10 10 10 10 10	3 R ONE R ONE NG 6500 CONC C. 13 13 REQUIF 37.37 TH CAROLINA RANSP 7.37 TH CAROLINA RANSP DARD YPE NCRE OR LI N B) LANE	GRTATI ORTATI ORTATI ORTATI	ER 5" Ø L. R. TRANDS No. 32 32 LENGTH 27%" ON SHEET NO. S16-15
SF	<u>GDR. BC</u> NUM 5 ROJE((TATI HEET 2 (DEPA PREST CON 0. вү:	ANTITI ANTITI	LENC C C C C C C C C C C C C C	3 R ONE R ONE NG 6500 CONC C. 13 13 REQUIF 37. 7. 7. 7. 7. 7. 7. 7. 7. 7.	GRTATI ORTATI ORTATI ORTATI	ER ST Ø L. R. TRANDS NO. 32 32 LENGTH 27/8" NTY ON CON
St I	<u>GDR. BC</u> NUM 5 ROJE((TATI HEET 2 (DEPA PREST CON 0. вү:	ANTITI ANTITI	LENC C S F O REINFORCI STEEL LB. 869 911 RDERS LENC 64'-5 C. C. C. C. C. C. C. C. C. C.	3 R ONE R ONE NG 6500 CONC C. 13 13 REQUIF 37 7.37 FRANSP 7.37 TH CAROLINA RANSP OARD YPE NCRE OARD YPE NCRE OR LI N B) LANE	GIRD PSI 0.6 PSI 0.6 Y. 0.6 PSI 0.6 Y. 0.6 Y. 0.6 Y. 0.6 S Y. 0.6 S O TOTAL S O TOTAL S O TOTAL S S ATE: 0 S S S S S S S S S S S S S	ER ST Ø L. R. TRANDS NO. 32 32 LENGTH 27%" ON NTY ON SHEET NO. S16-15 TOTAL SHEETS

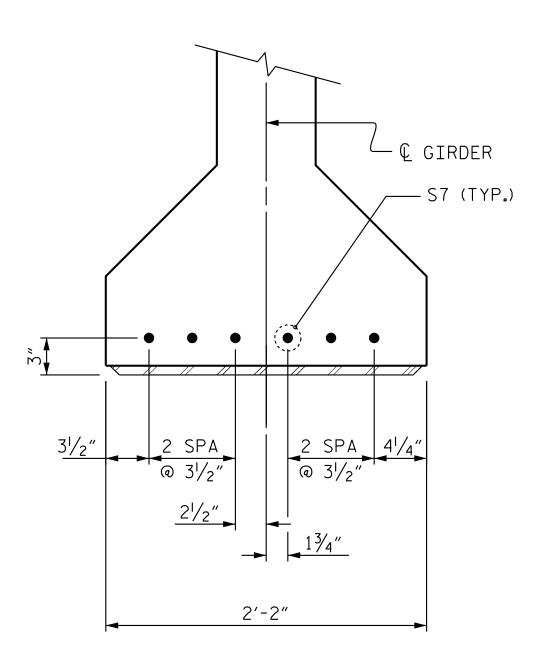
416\R1015_SMU_G3_240287	
11036303 - R-1015\CAD\Dgn\Structure	
ructures_Bridge\NC\C	
K:\RDT_S†	

K:\RDT_Structures_				
018	ASSEMBLED BY : D.D.LOWE CHECKED BY : J.C.WILSON	ΪRΥ	DATE : DATE :	10/18 10/18
12/7/2018	DRAWN BY : ELR 11/91 CHECKED BY : GRP 11/91	REV. REV. REV.	1/15 2/15 12/17	MAA/TMG MAA/TMG MAA/THC
-				



EMBEDDED PLATE ``B-1'' DETAILS FOR AASHTO TYPE IV GIRDER

(2 REQ'D PER GIRDER)



DETAIL ``A"

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

APPLY EPOXY PROTECTIVE COATING TO END OF GIRDER SURFACES INDICATED IN ELEVATION VIEW.

SPECIFICATIONS.

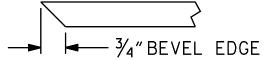
ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE ``B'' REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2"BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 5,000 PSI.

DEPTH OF $\frac{1}{4}$ ".

FOR SECTION C-C, SEE "AASHTO TYPE IV PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD' SHEETS 1 OF 4 & 2 OF 4.



SECTION ``F''

(SEE NOTES)

NOTES

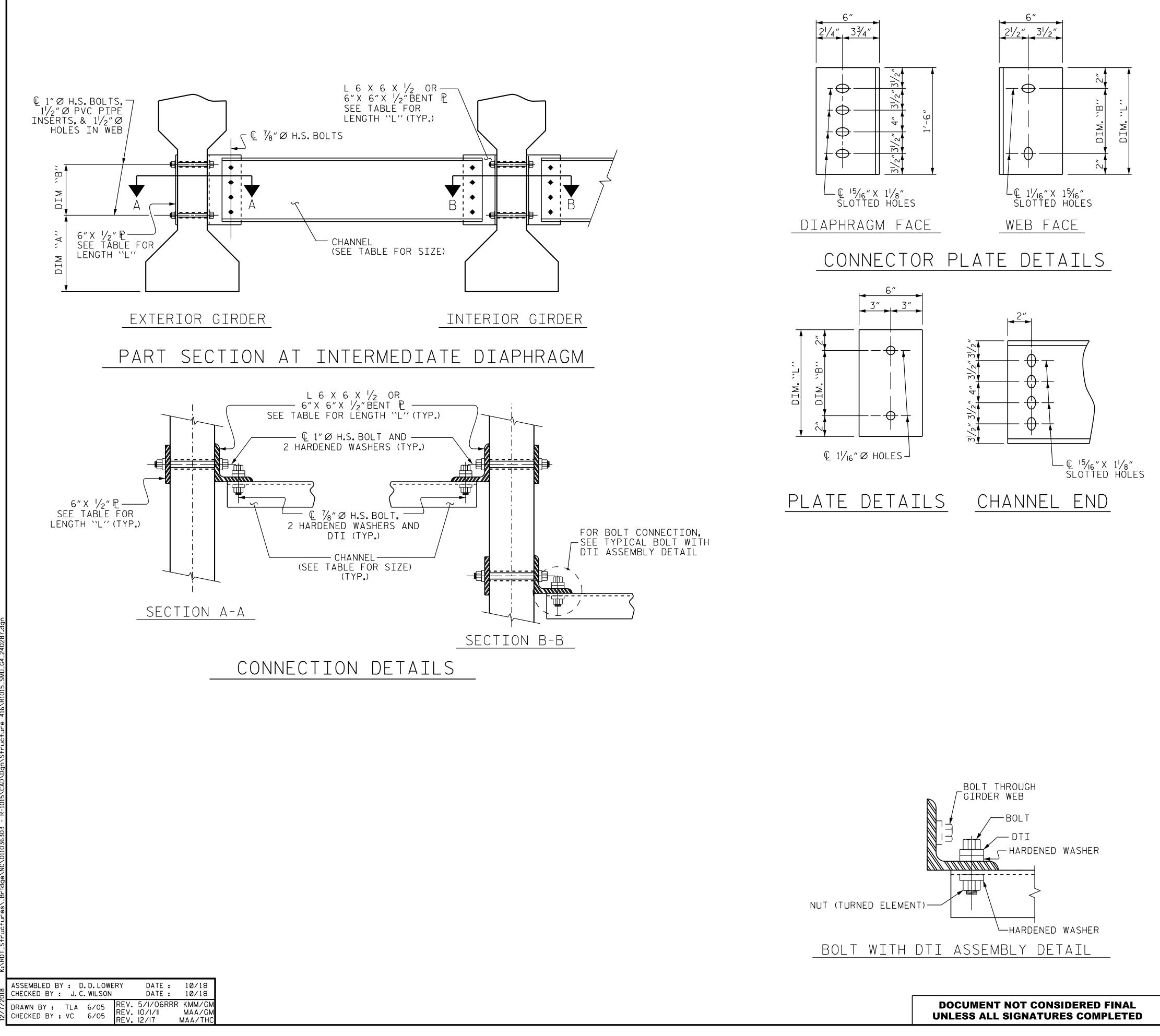
EMBEDDED PLATE ``B-1'' SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A

		CT NO. <u>CRAVE</u> [On: 516 _{of 4}	N	• •	UNTY
Docusigned by: Jeffrey C. Wilson CDA045FAFCC9416	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD				
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772		[DETAIL	S	
Raleign, NC 27601-1772 NC LICENSE # Phone (919) 677-2000 F-0102			SIONS	DATE	SHEET NO. S16-16
This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc. Copyright Kimley-Horn and Associates, Inc., 2018	NO. вт: 1 2	DATE:	NO. BY: ③ 4]	DATE:	TOTAL SHEETS 44
	STRUC	TURE 16	6	STD.N	O.PCG9





STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE CHANNEL MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, CHANNELS, AND ANGLES SHALL BE METALIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY A THERMAL SPRAYED COATING WITH A SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE DEPARTMENT'S THERMAL SPRAYED COATINGS (METALIZATION) SEE SPECIAL PROVISIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

TABLE

GIRDER TYPE	CHANNEL SIZE	DIM ``A''	DIM ``B''	DIM ``L''
IV	MC 18 × 42.7	1′-9 /2″	1'-2″	1'-6"

	PROJEC <u>C</u> STATIC	RAVE		• • •	UNTY 	
	SHEET 4 OF 4					
DocuSigned by: SEAL 040384 SEAL 040384 C. Within 12/7/2018 CDA045FAFCC9416	SHEET 4 OF 4 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR TYPE IV PRESTRESSED CONCRETE GIRDERS					
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772		REVIS	SIONS		SHEET NO.	
Phone (919) 677-2000 F-0102	NO. BY:	DATE:	NO. BY:	DATE:	S16-17	
locument, logether with the concepts and designs presented herein, as an ment of services, is intended only for the specific purpose and client for it was prepared. Reuse of and improper reliance of this document without a authorization and a graphic they and the specific purpose t liability to Kimley-Horn and Associates, Inc. shall be t liability to Kimley-Horn and Associates, Inc. ght Kimley-Horn and Associates, Inc., 2018	12		3 좌		total sheets 44	
	STRUCT	URE 10	Ô	STD. NC).PCG10	

DEAD LOAD DEFLECTION TABLE FOR GIRDERS											
		SPAN A									
0.6″ Ø LOW RELAXATION STRANDS		GIRDERS AG1 AND AG5									
TENTH POINTS	BRG.	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	BRG.
CAMBER (GIRDER ALONE IN PLACE)	0.000	0.047	0.088	0.121	0.141	0.148	0.141	0.121	0.088	0.047	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L.		0.020	0.039	0.054	0.064	0.067	0.064	0.054	0.039	0.020	0.000
FINAL CAMBER	0	5⁄16″	9/16″	3⁄4″	7⁄8″	15/16″	7⁄8″	3⁄4″	9/16″	5/16″	0

* INCLUDES FUTURE WEARING SURFACE. ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT ``FINAL CAMBER'', WHICH IS GIVEN IN INCHES (FRACTION FORM).

DEAD LOAD DEFLECTION TABLE FOR GIRDERS											
0.6″ Ø LOW RELAXATION STRANDS		SPAN A									
0.0 Ø LOW RELAXATION STRANDS		GIRDERS AG2, AG3, AND AG4									
TENTH POINTS	BRG.	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	BRG.
CAMBER (GIRDER ALONE IN PLACE)	0.000	0.047	0.088	0.121	0.141	0.148	0.141	0.121	0.088	0.047	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L.		0.024	0.047	0.065	0.076	0.080	0.076	0.065	0.047	0.024	0.000
FINAL CAMBER O 1/4" 7/16" 5/8" 3/4"					³ ⁄4″	3⁄4″	⁵ ⁄8″	7/16″	¹ /4″	0	

* INCLUDES FUTURE WEARING SURFACE.

ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT ``FINAL CAMBER'', WHICH IS GIVEN IN INCHES (FRACTION FORM).

DEAD LOAD DEFLECTION TABLE FOR GIRDERS											
0.6″ Ø LOW RELAXATION STRANDS		SPAN B									
0.8 Ø LOW RELAXATION STRANDS		GIRDERS BG1 AND BG5									
TENTH POINTS		0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	BRG.
CAMBER (GIRDER ALONE IN PLACE)		0.032	0.060	0.083	0.097	0.102	0.097	0.083	0.060	0.032	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L.		0.008	0.015	0.021	0.024	0.026	0.024	0.021	0.015	0.007	0.000
FINAL CAMBER	o	¹ /4″	1/2″	"/16″	13/16″	7⁄8″	13/16″	"/16″	1/2″	¹ /4″	0

* INCLUDES FUTURE WEARING SURFACE. ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT ``FINAL CAMBER', WHICH IS GIVEN IN INCHES (FRACTION FORM).

DEAD LOAD DEFLECTION TABLE FOR GIRDERS											
0.6″ Ø LOW RELAXATION STRANDS		SPAN B									
0.8 Ø LOW RELAXATION STRANDS		GIRDERS BG2,BG3,AND BG4									
TENTH POINTS		0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	BRG.
CAMBER (GIRDER ALONE IN PLACE)		0.032	0.060	0.083	0.097	0.102	0.097	0.083	0.060	0.032	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L.		0.009	0.018	0.025	0.029	0.031	0.029	0.025	0.018	0.009	0.000
FINAL CAMBER	0	1/4″	1/2″	"/16″	13/16″	13/16″	13/16″	"/16″	¹ /2″	۱/ ₄ ″	0

* INCLUDES FUTURE WEARING SURFACE. ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT ``FINAL CAMBER'', WHICH IS GIVEN IN INCHES (FRACTION FORM).

ΩIΩ	DRAWN BY: D.D.LOWERY	DATE:	10/18
171	CHECKED BY: C. T. POOLE	DATE:	10/18
17/	DESIGN ENGINEER OF RECORD: J.C.WILSON	DATE:	10/18

PROJECT NO. R-1015 CRAVEN COUNTY

STATION:<u>516+87.37</u> -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE

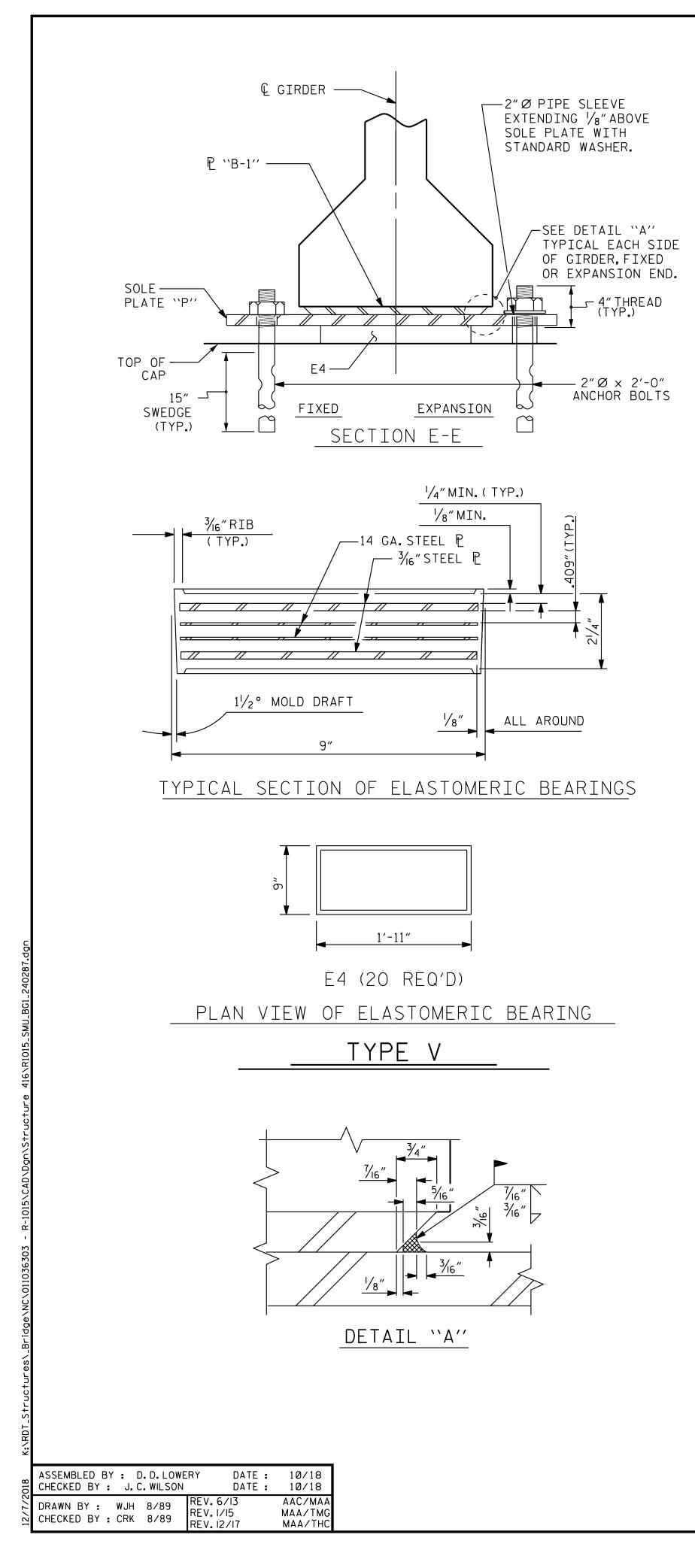


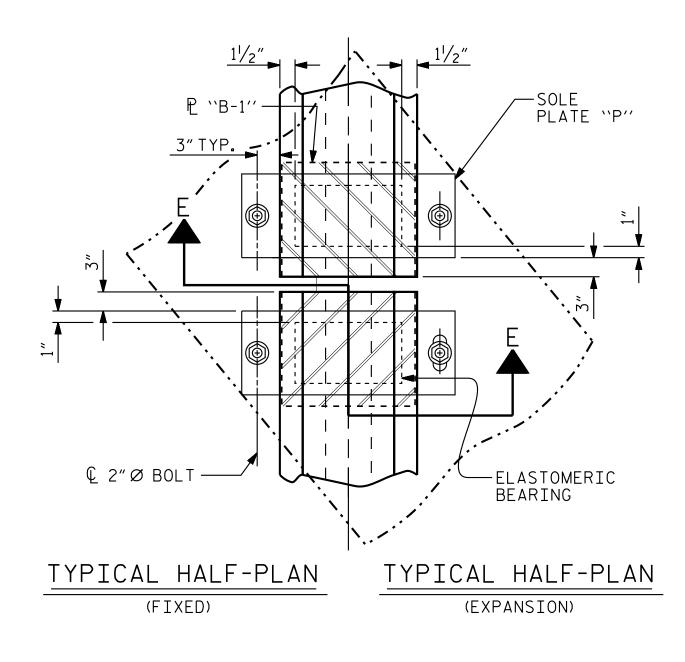
		RIGHI LANE									
ŧ		SHEET NO.									
	NO.	BY:	DATE:	N0.	BY:	DATE:	S16-18				
an or hout be	1			S			TOTAL SHEETS				
	2			4			44				
	СТ			~							



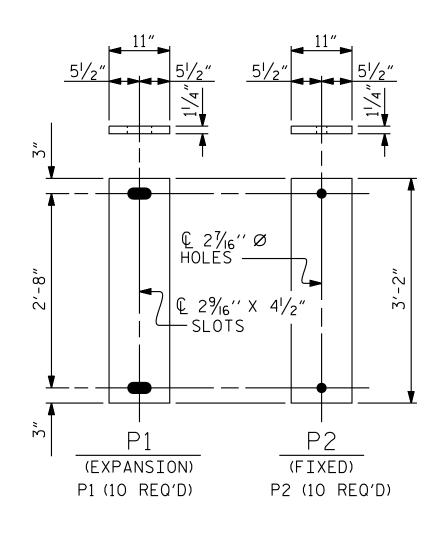
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102

This document, together with the concepts and designs presented herein, as a instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document withou written authorization and adoption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc. Copyright Kimley-Horn and Associates, Inc., 2018





MAXIMUM ALLOWABLE SERVICE LOADS						
D.L.+L.L.(NO IMPACT)						
TYPE V	365 k					



SOLE PLATE DETAILS (``P'')

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2"Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE ``P'', BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLT, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

ALL SOLE PLATES SHALL BE AASHTO M270 GRADE 36.

PROJECT NO. <u>R-1015</u> <u>CRAVEN</u> COUNTY STATION: <u>516+87.37</u> -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

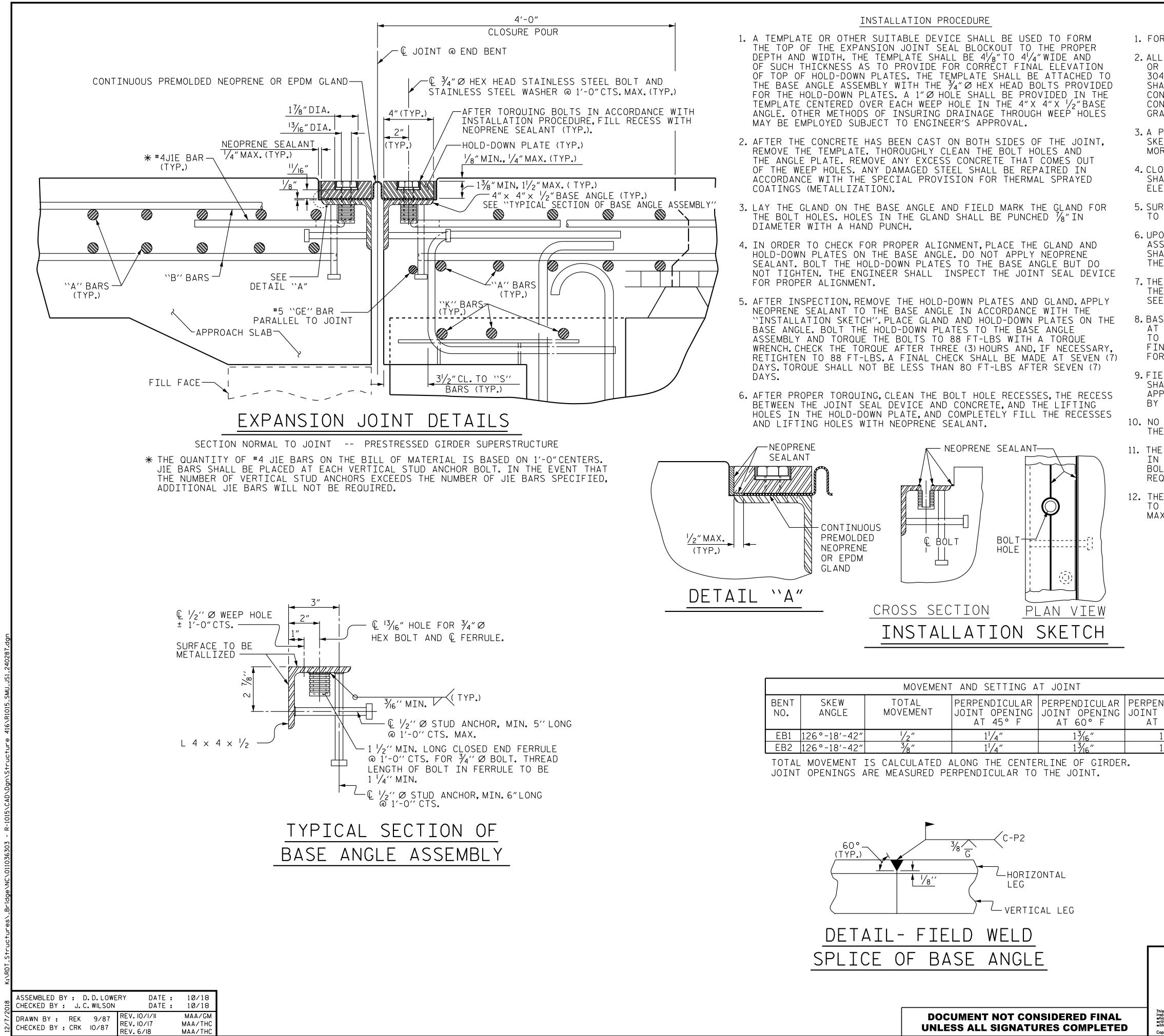
		SHEET NO.				
NO.	BY:	DATE:	N0.	BY:	DATE:	S16-19
1			3			TOTAL SHEETS
2			4			44
S	TRUCT	URE 1	6		STD.	NO.EB4



Phone (919) 677-2000 F-0102 This document, together with the concepts and designs presented herein, as a instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document with written authorization and adaption by Kimley-Horn and Associates, Inc. shall t without liability to Kimley-Horn and Associates, Inc.

Copyright Kimley-Horn and Associates, Inc., 2018

MAA/THC



MOVEMENT AND SETTING AT JOINT								
BENT NO.	SKEW ANGLE	TOTAL MOVEMENT	PERPENDICULAR JOINT OPENING AT 45° F	PERPENDICULAR JOINT OPENING AT 60° F	PERPEND JOINT O AT 9			
EB1	126°-18'-42″	1/2"	11/4″	1 ³ / ₁₆ ″	11/			
FB2	126°-18'-42"	3/, "	11/4"	13/c''	11/			

which writter withou

UNLESS ALL SIGNATURES COMPLETED

GENERAL NOTES

1. FOR EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.

2. ALL PLATES AND ANGLES SHALL CONFORM TO AASHTO M270 GRADE 36 STEEL OR APPROVED EQUAL. ALL HOLD-DOWN BOLTS SHALL CONFORM TO ASTM F593 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL CONFORM TO ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL. ALL STUD ANCHORS SHALL CONFORM TO AASHTO M169, GRADES 1010 THRU 1020 OR APPROVED EQUAL. ALL CONCRETE INSERTS SHALL BE CLOSED END AND SHALL CONFORM TO AASHTO M169, GRADE 12L14. TENSILE CAPACITY SHALL BE 3000 LBS. MINIMUM.

3. A PREMOLDED CORRUGATED OR NON-CORRUGATED GLAND SHALL BE USED FOR JOINTS SKEWED BETWEEN 50° THRU 130°. FOR JOINTS SKEWED LESS THAN 50° OR MORE THAN 130°, ONLY A CORRUGATED GLAND SHALL BE USED.

4. CLOSED END FERRULES AND STUD ANCHORS SHALL BE SHOP WELDED AND ALL HOLES SHALL BE SHOP DRILLED AS SHOWN ON PLANS. STUD ANCHORS SHALL BE ELECTRIC ARC END WELDED WITH COMPLETE FUSION.

5. SURFACES COMING IN CONTACT WITH NEOPRENE SHALL BE GROUND SMOOTH PRIOR TO METALLIZING.

6. UPON COMPLETION OF SHOP FABRICATION, THE HOLD-DOWN PLATE AND BASE ANGLE ASSEMBLY, AS SHOWN IN THE `` TYPICAL SECTION OF BASE ANGLE ASSEMBLY", SHALL BE METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

7. THE COVER PLATES SHALL BE METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.FOR THERMAL SPRAYED COATINGS (METALLIZATION). SEE SPECIAL PROVISIONS.

8. BASE ANGLE ASSEMBLY SHALL BE CONTINUOUS FOR THE LENGTH OF THE JOINT. AT CROWN BREAKS. THE ENDS OF THE BASE ANGLE ASSEMBLY SHALL BE CUT PARALLEL TO THE BRIDGE CENTERLINE FOR SKEWS LESS THAN 80° AND GREATER THAN 100°. FINISHED WELD SHALL BE REPAIRED IN ACCORDANCE WITH THE SPECIAL PROVISION FOR THERMAL SPRAYED COATINGS (METALLIZATION).

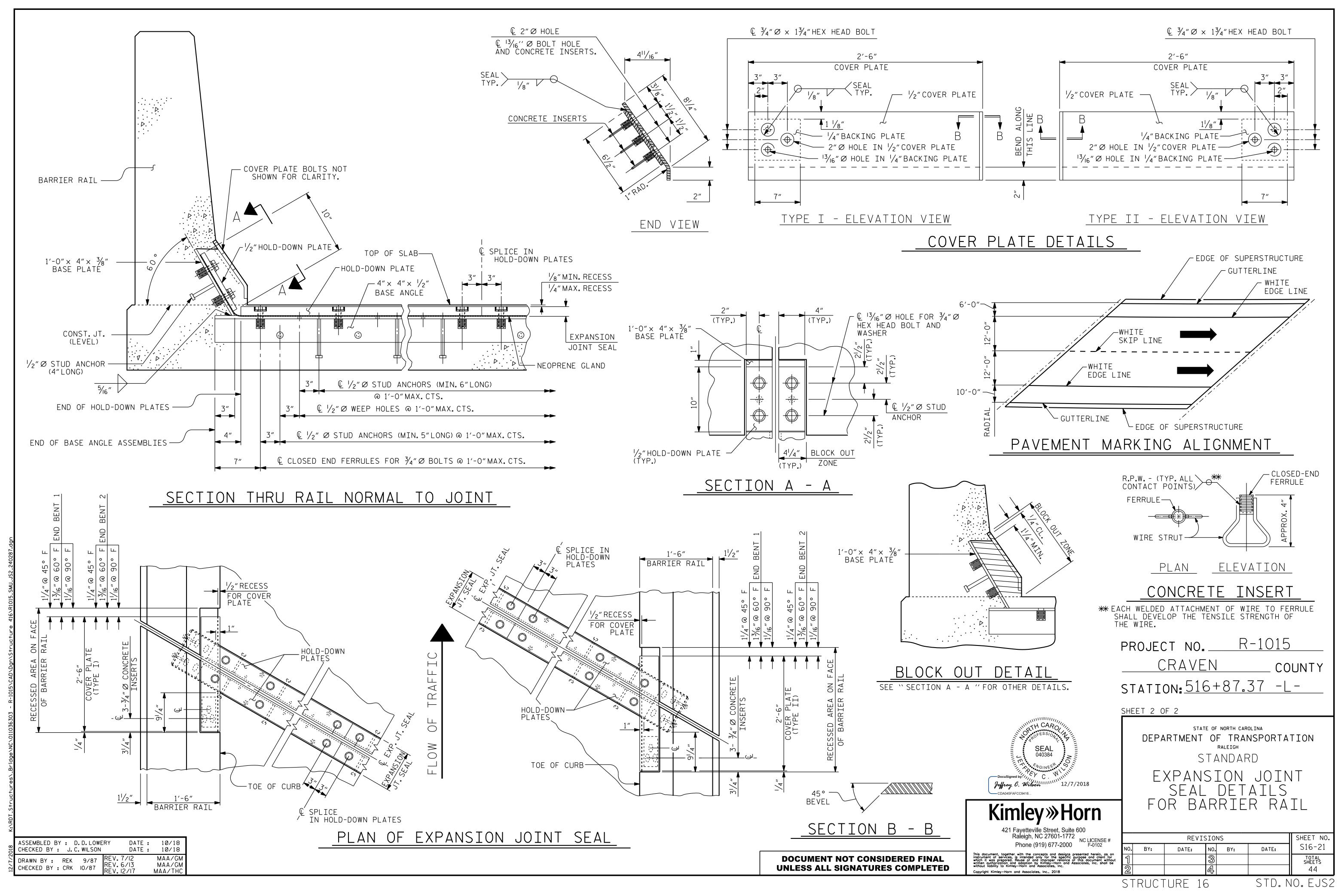
9. FIELD SPLICES OF HOLD-DOWN PLATES SHALL BE KEPT TO A MINIMUM. CONTRACTOR SHALL FURNISH DETAILED PLANS SHOWING PROPOSED SPLICE LOCATIONS FOR APPROVAL. HOLD-DOWN PLATES SHALL NOT EXCEED 20' LENGTHS UNLESS APPROVED BY THE ENGINEER.

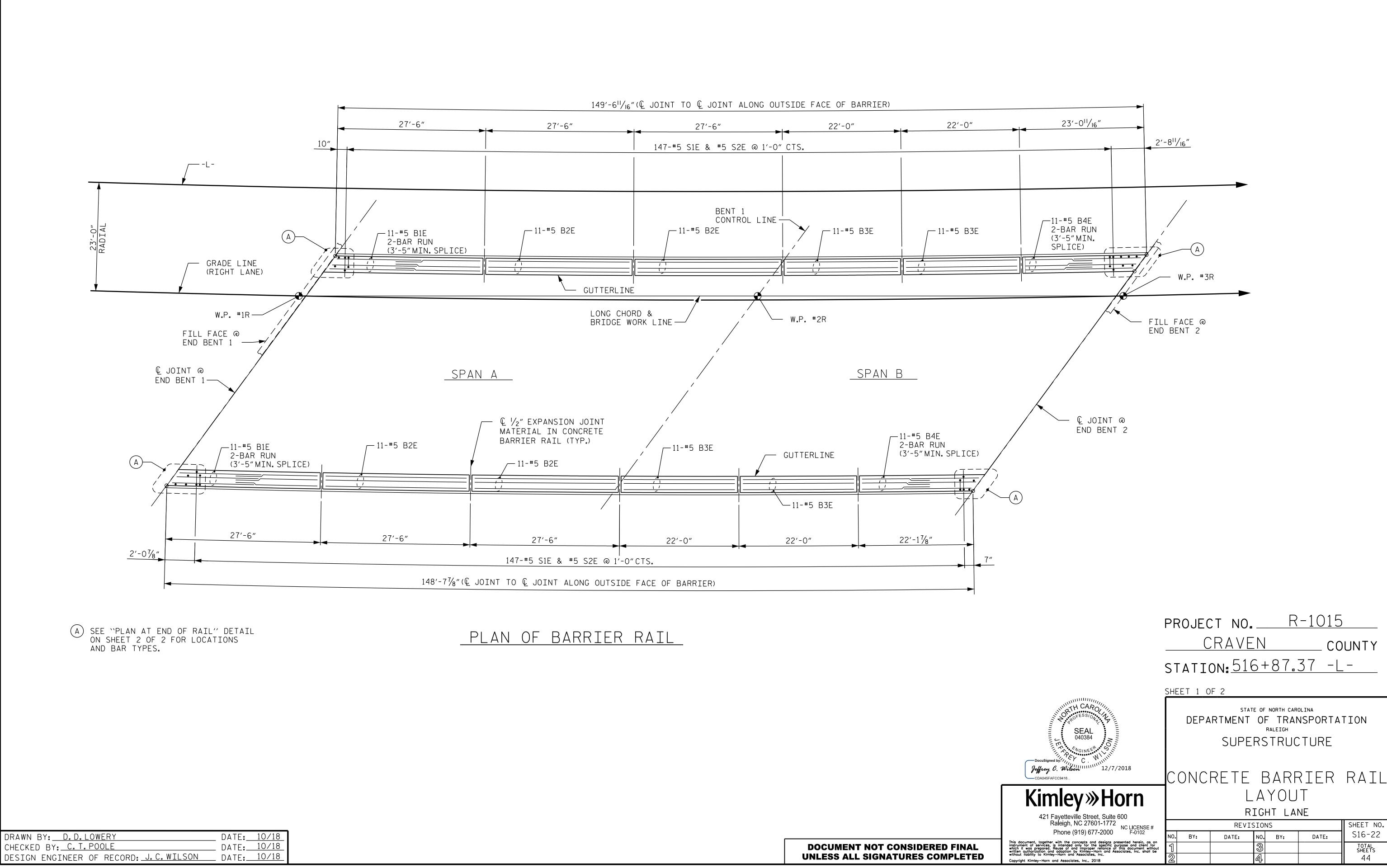
10. NO ALTERNATE JOINT DETAILS SHALL BE PERMITTED IN LIEU OF THOSE SHOWN ON THESE PLANS.

11. THE CONTRACTOR MAY, AT HIS OPTION, USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF CONCRETE INSERTS FOR COVER PLATES. THE YIELD LOAD OF THE $\frac{3}{4}$ " Ø BOLT IS 10 KIPS. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

12. THE FABRICATOR SHALL PROVIDE $\frac{1}{2}$ " Ø THREADED HOLES IN THE HOLD-DOWN PLATES TO ASSIST IN LIFTING AND PLACING. THE HOLES SHALL BE $\frac{3}{4}$ " deep at 6'-0" MAXIMUM SPACING AND A MINIMUM OF TWO HOLES PER PLATE.

DICULAR DPENING 90°F /16″							
	PROJECT NO. <u>R-1015</u>						
	CRAVEN COUNTY						
	STATION: <u>516+87.37</u> -L-						
	SHEET 1 OF 2						
SEAL 040384	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						
	STANDARD						
Jeffrey C. Wilson 12/7/2018 CDA045FAFCC9416	EXPANSION JOINT						
Kimley Worn	SEAL DETAILS						
421 Fayetteville Street, Suite 600 Raleigh NC 27601-1772	REVISIONS SHEET NO.						
Phone (919) 677-2000 F-0102	NO. BY: DATE: NO. BY: DATE: S16-20						
document, together with the concepts and designs presented herein, as an iment of services, is intended only for the specific purpose and client for it was prepared. Reuse of and improper reliance of this document without n authorization and adaption by Kimley-Horn and Associates, Inc. shall be ut liability to Kimley-Horn and Associates, Inc. right Kimley-Horn and Associates, Inc., 2018	1 3 TOTAL 2 4 4 44						
	STRUCTURE 16 STD. NO. EJS1						





NOTES

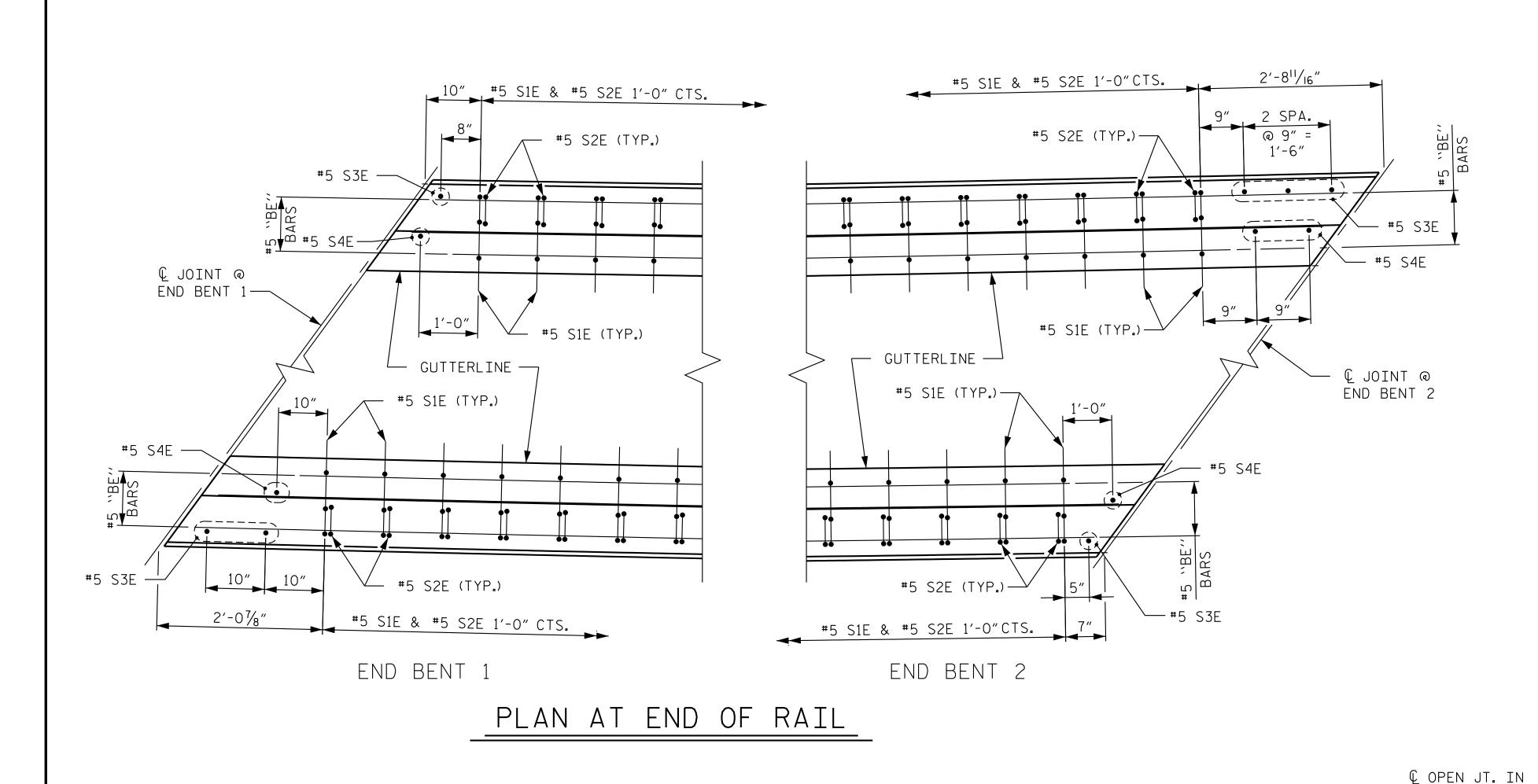
ALL DIMENSIONS ARE MEASURED ALONG OUTSIDE FACE OF CONCRETE BARRIER RAIL.

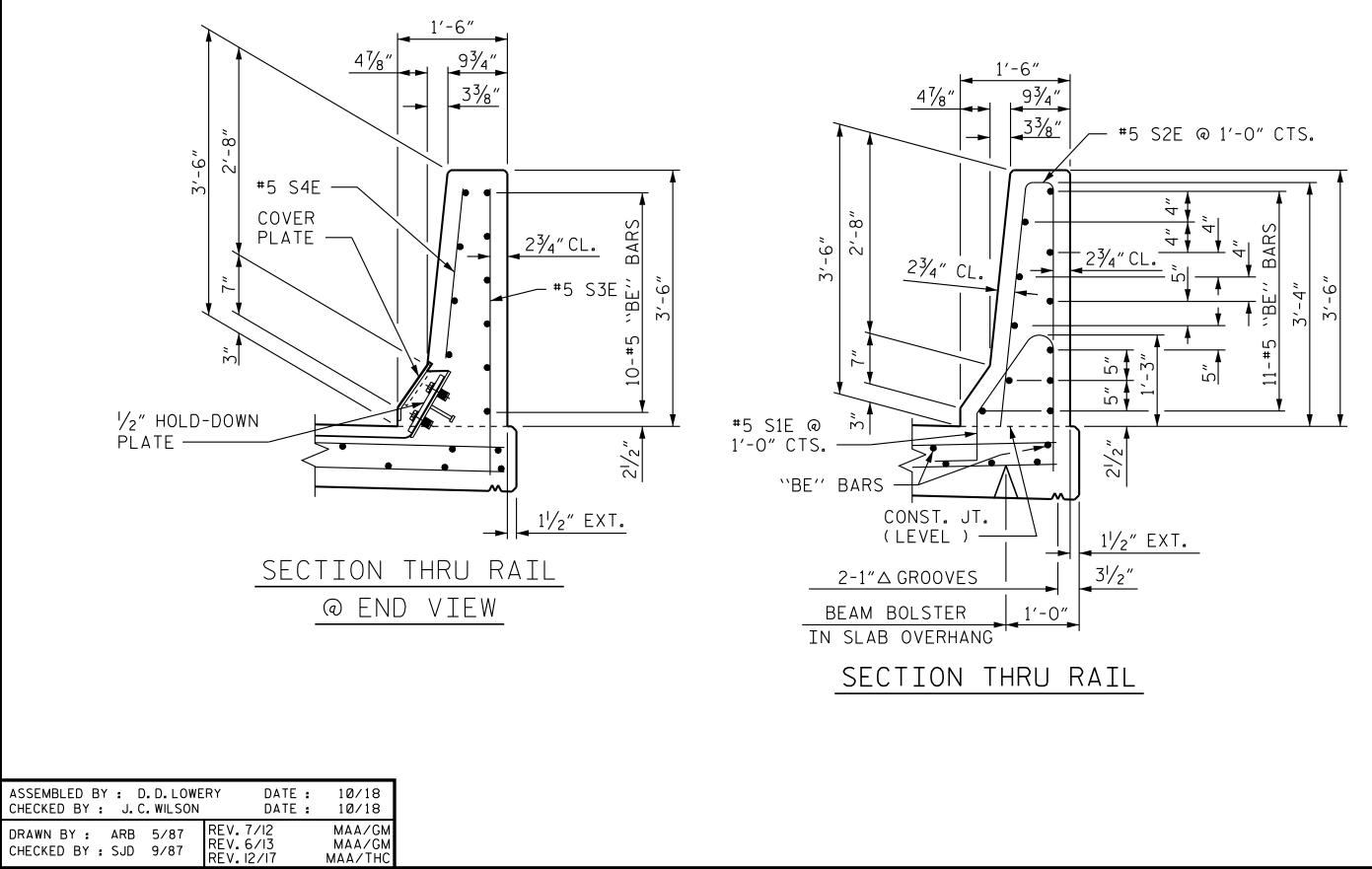
STRUCTURE 16

SHEET NO. S16-22

TOTAL SHEETS

44





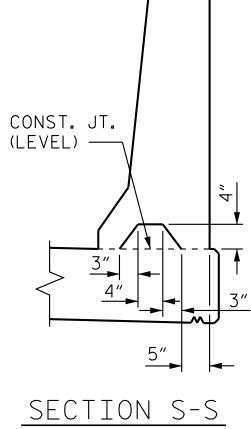
NOTE

THE BARRIER RAIL IN EACH SPAN S UNTIL ALL SLAB CONCRETE IN THAT CAST AND HAS REACHED A MINIMUM STRENGTH OF 3,000 PSI.

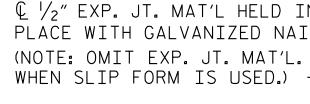
ALL REINFORCING STEEL IN BARRIE EPOXY COATED.

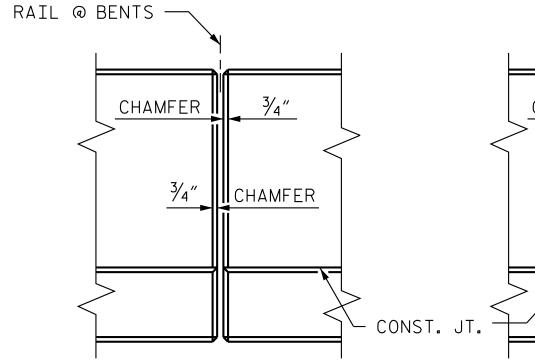
GROOVED CONTRACTION JOINTS, 1/2" TOOLED IN ALL EXPOSED FACES OF IN ACCORDANCE WITH ARTICLE 825-SPECIFICATIONS. THE CONTRACTION AT EACH THIRD POINT BETWEEN BAN JOINTS. ONLY ONE CONTRACTION JO MIDPOINT OF BARRIER RAIL SEGMEN IN LENGTH AND NO CONTRACTION JO THOSE SEGMENTS LESS THAN 10 FEE

QUANTITIES FOR BARRIER RAIL ON INCLUDED ON BRIDGE APPROACH SLA



AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

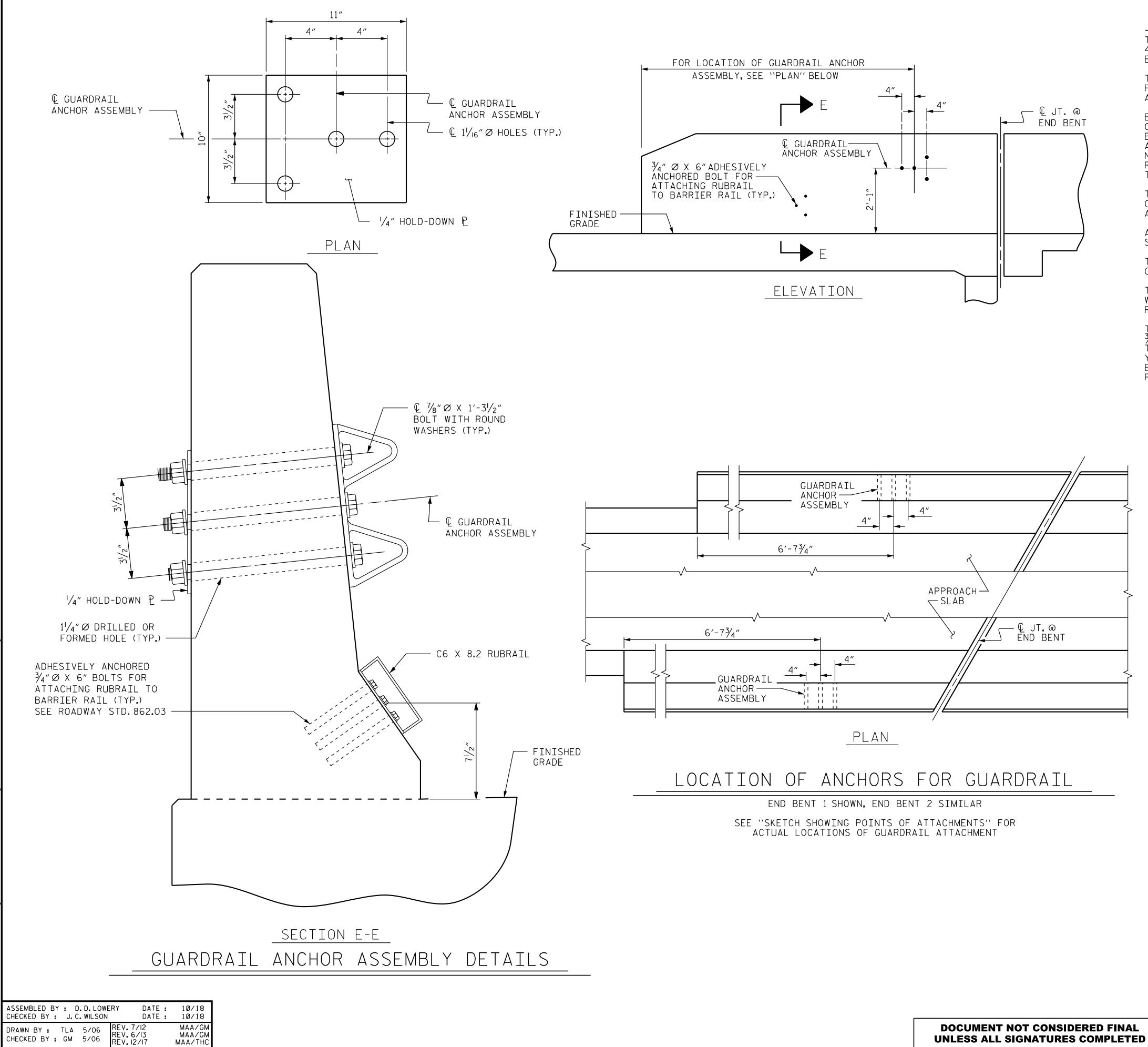




<u>ELEVATION AT EXPANSION</u>

This c instru which writte withou

	BAR TYPES						
TES N SHALL NOT BE CAST HAT SPAN HAS BEEN HAT SPAN HAS BEEN HAT SOMPRESSIVE RIER RAILS SHALL BE 2" IN DEPTH, SHALL BE 0" IN DEPTH,	$\frac{1'-0!/2''}{87/6''}$						
	BILL OF MATERIAL FOR CONCRETE BARRIER RAIL ONLY BAR NO. SIZE TYPE LENGTH WEIGHT B1E 44 #5 STR 15'-8" 719 B2E 44 #5 STR 27'-1" 1243 B3E 44 #5 STR 21'-7" 991 B4E 44 #5 STR 13'-0" 597 B4E 44 #5 STR 13'-0" 597 B4E 44 #5 STR 13'-0" 597 S1E 294 #5 1 4'-8" 1431 S2E 294 #5 2 7'-0" 2146 S3E 7 #5 STR 3'-11" 29 S4E 5 #5 STR 2'-4" 12						
IN AILS. CHAMFER J4″ GHAMFER J4″ CHAMFER J4″ CHAMFER J4″ CHAMFER J4″ CHAMFER J4″ CHAMFER J4″ CHAMFER J4″ CHAMFER J4″ S	EPOXY COATED REINFORCING STEEL 7,168 LBS. CLASS AA CONCRETE 40.5 CU. YDS. CONCRETE BARRIER RAIL ** 297.2 LIN.FT. "E" INDICATES EPOXY COATED REINFORCING STEEL. * * DOES NOT INCLUDE BARRIER RAIL ON APPROACH SLAB.						
<u>en joints</u> <u>etails</u>	PROJECT NO. <u>R-1015</u> <u>CRAVEN</u> COUNTY STATION: 516+87.37 -L- SHEET 2 OF 2						
DocuSigned by: Jeffrey C. Wilson CDA045FAFCC9416	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD CONCRETE BARRIER RAIL						
Kindey Horn and Associates, Inc., 2018	RIGHT LANEREVISIONSSHEET NO.NO.BY:DATE:NO.SIG-2313						



NOTES

4 - 7/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS. THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

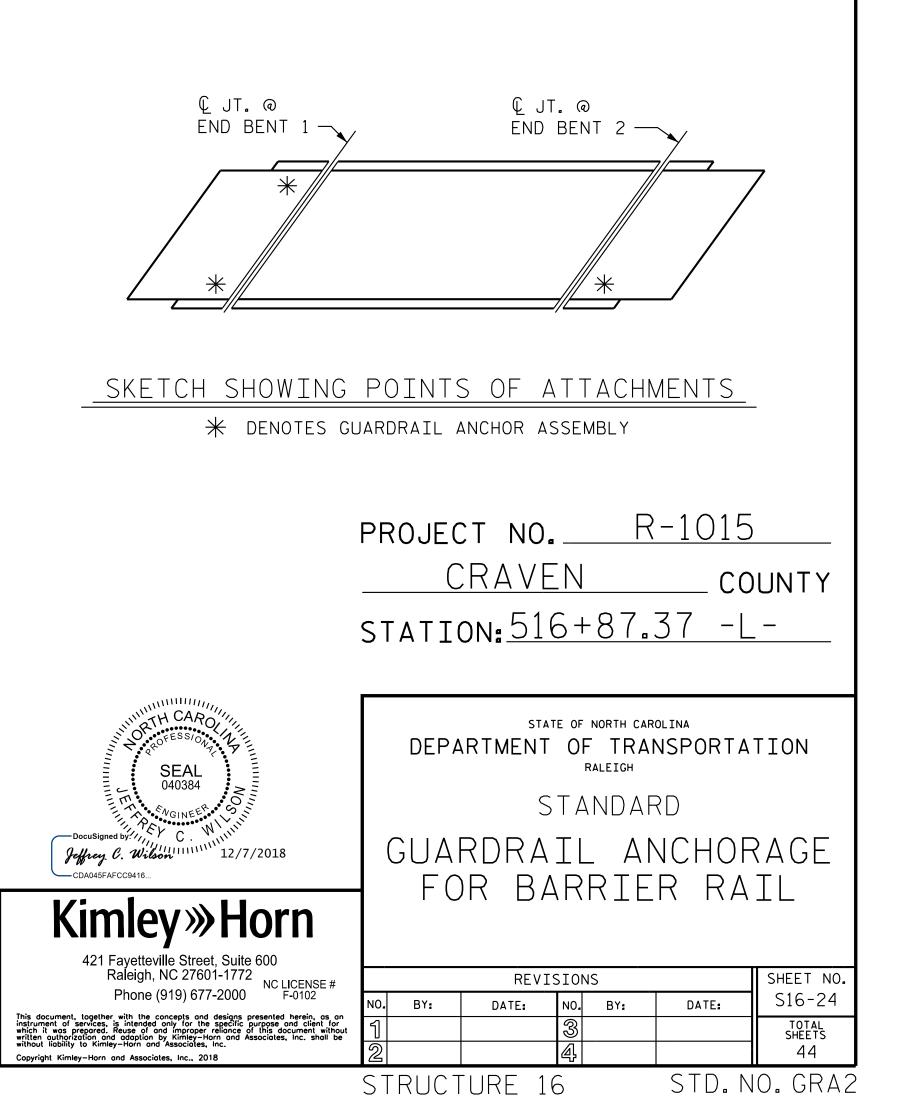
AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

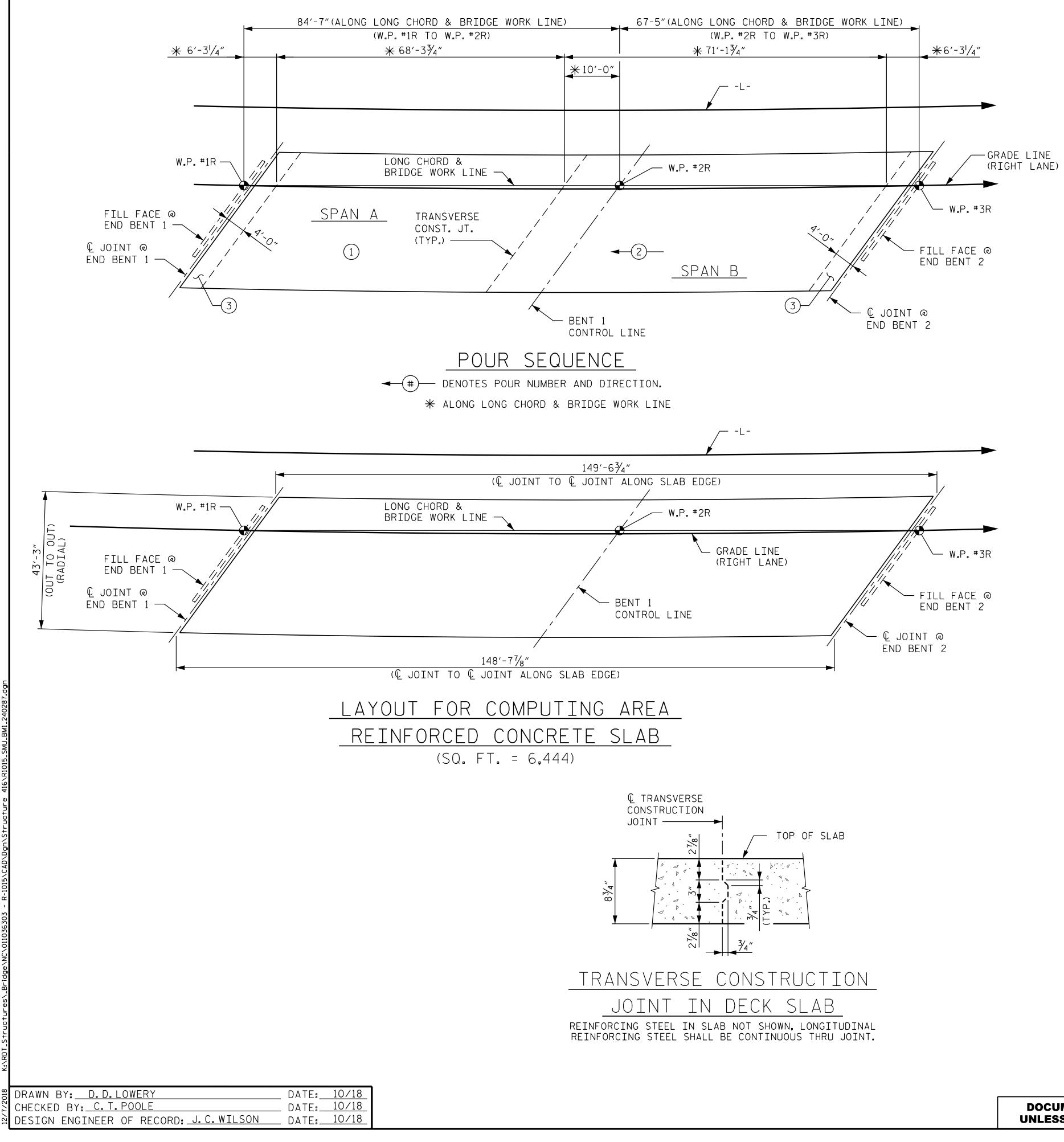
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE $\frac{3}{4}$ " Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND

YIELD LOAD OF THE $\frac{3}{4}$ " Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.





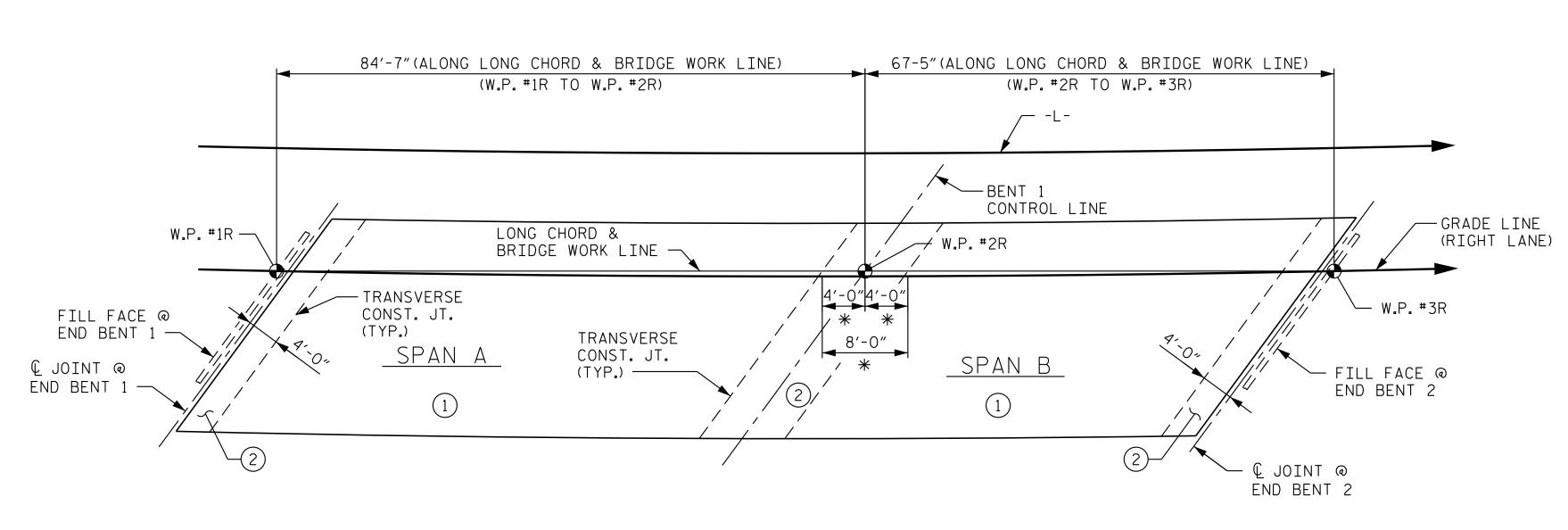


GROOVING	BRIDGE FL	OORS
APPROACH SLABS	1,753	SQ.FT.
BRIDGE DECK	5,465	SQ.FT.
TOTAL	7,218	SQ.FT.

SUPERS	TRUCTURE E	BILL OF MA	TERIAL
	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL
	(CU.YDS.)	(LBS_)	(LBS.)
POUR 1	93.7		
POUR 2	115.8		
POUR 3	22.8		
TOTALS **	232.3	26,981	24,647

** QUANTITIES FOR BARRIER RAILS ARE NOT INCLUDED.

	PROJECT NO. <u>R-1015</u> <u>CRAVEN</u> COUNTY STATION: <u>516+87.37</u> -L-
	SHEET 1 OF 3
DocuSigned by: Jeffrey C. Wilson LandsFAFCC9416	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE
CDA045FAFCC9416	BILL OF MATERIAL
Kimley»Horn	
	RIGHT LANE
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772	
Phone (919) 677-2000 F-0102	S16-25
This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc.	NO. BY: DATE: NO. BY: DATE: STO 25 1 3 TOTAL SHEETS
without liability to Kimley-Horn and Associates, Inc. Copyright Kimley-Horn and Associates, Inc., 2018	2 44 44
	STRUCTURE 16



OPTIONAL POUR SEQUENCE

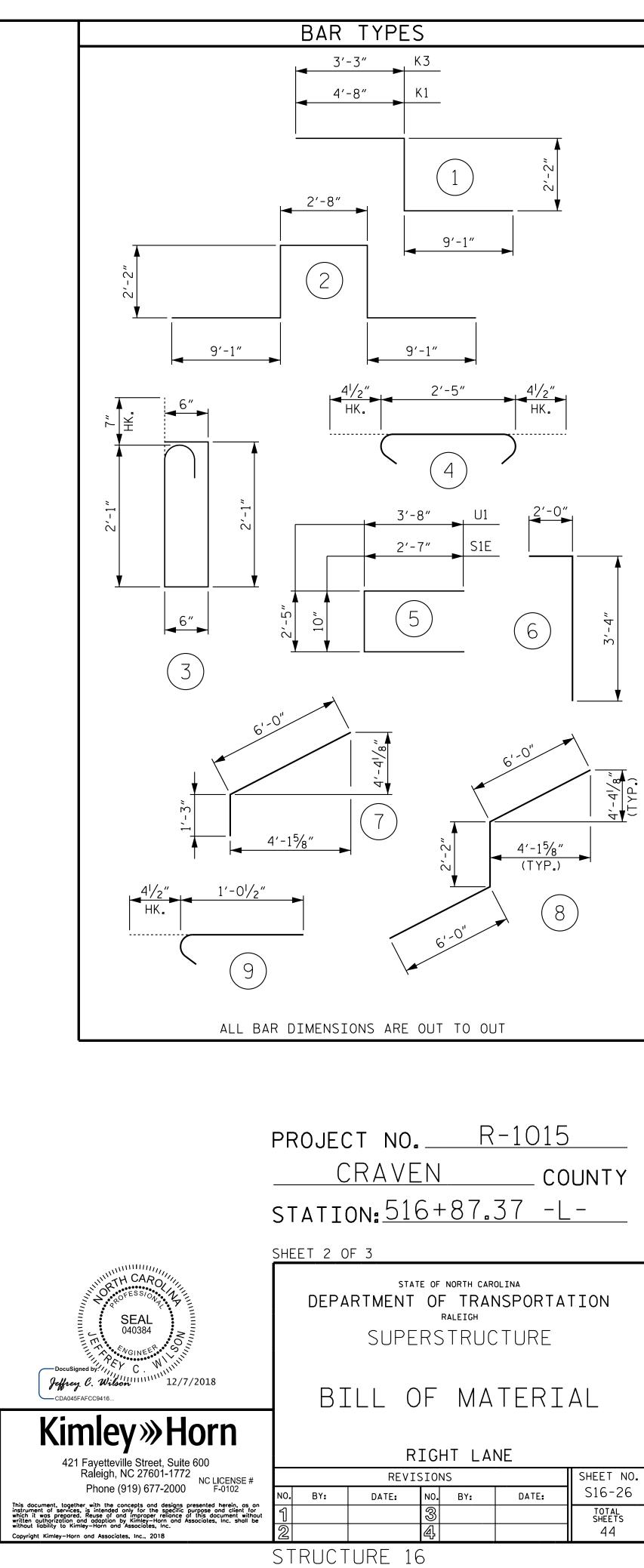
POUR #2 CAN NOT BE STARTED UNTIL BOTH ADJACENT POUR #1 REACH A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

* ALONG LONG CHORD & BRIDGE WORK LINE.

Υ.			
2018	DRAWN BY: <u>D.D.LOWERY</u> Checked by: <u>C.T.Poole</u>	DATE:	10/18
12/7/		DATE: . date:	10/18

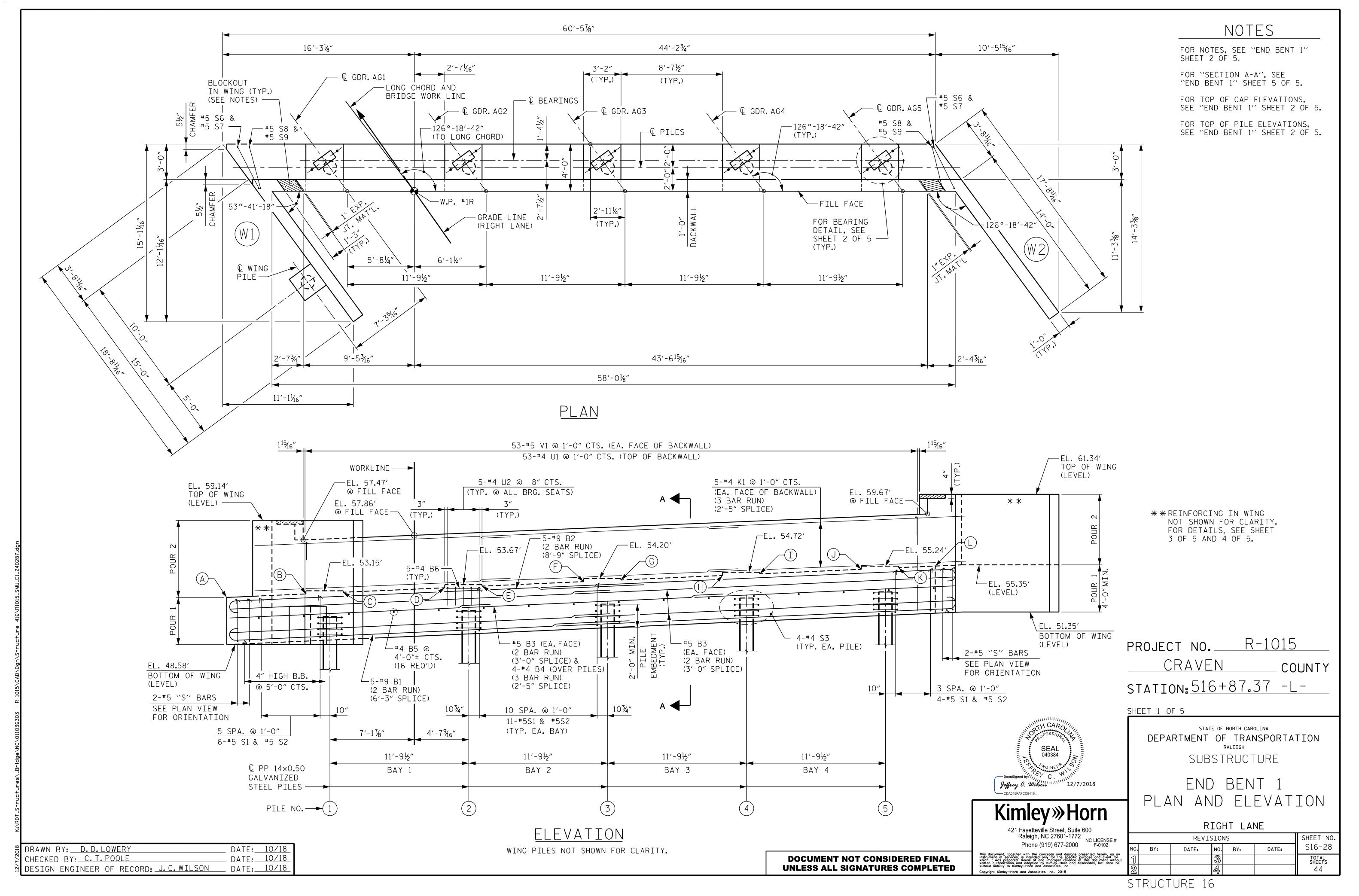


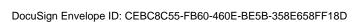
	SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS										
BAR SIZE	SUPERSTE EXCEPT A SLABS, P AND BARR	H SLABS	PARAPET AND BARRIER RAIL								
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL						
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"						
#5	2'-6"	2'-2"	2'-6"	2'-2"	3′-5″						
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"						
#7	5'-3"	3'-6"									
#8	6'-10"	4'-7"									

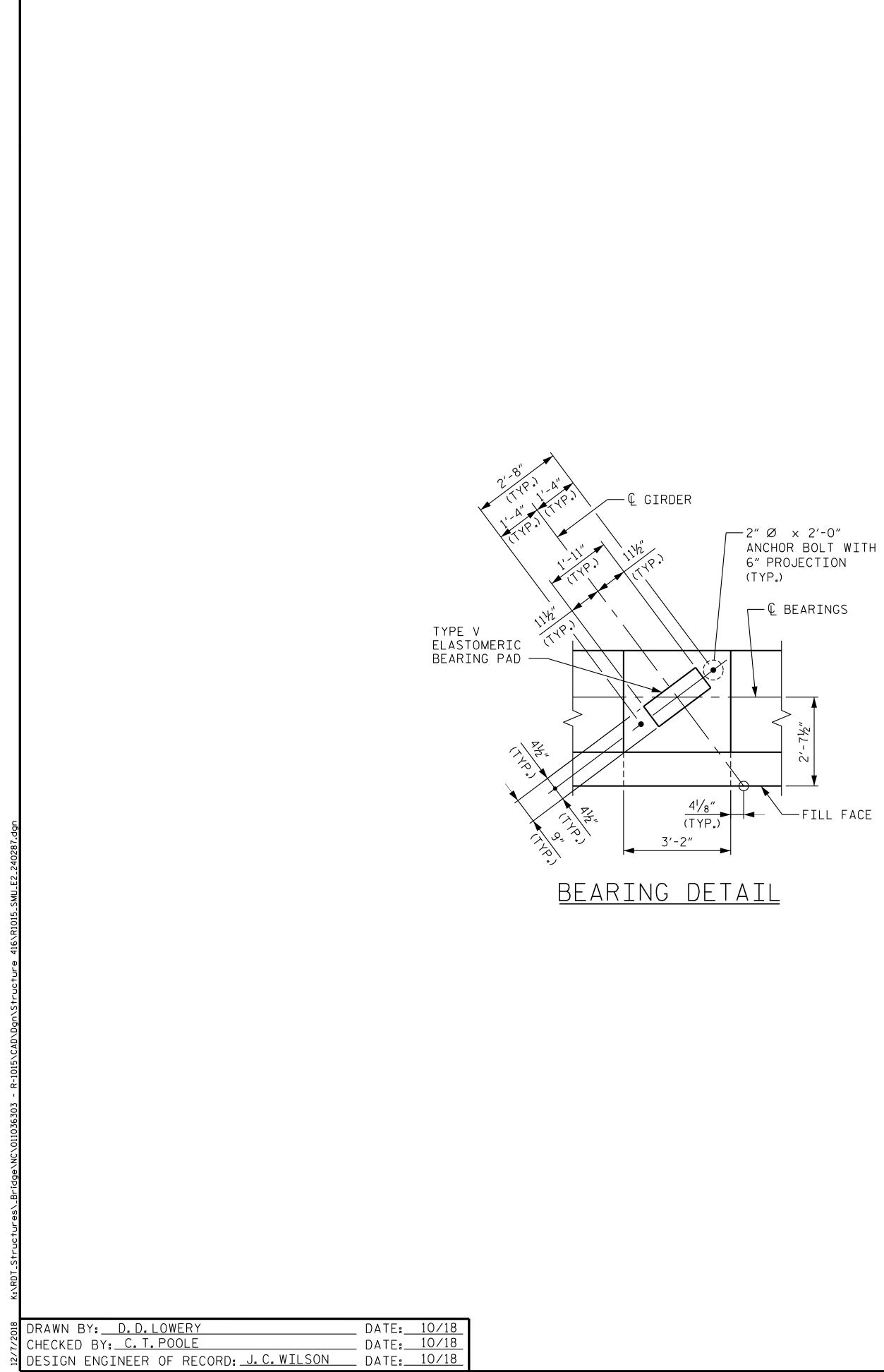


													BILL	OF M	ATERIAL	-														
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO. SIZ	E TYPE	LENGTH	WEIGHT	BAR	NO. S	SIZE TYP	LENGT	H WEIGHT	BA	AR N	10. S	IZE	TYPE	LENGTH	WEIGHT		
A1E A2 A3E	282 282 6	5 5 5	STR STR STR	42'-11" 42'-11" 3'-3"	12,623 12,623 20	A165E A166E A167E	1 1 1	5 5 5	STR STR STR	5'-6" 4'-11" 4'-4"	6 5 5	A235E A236E A237E	1 5 1 5 1 5	STR STR STR	7'-0" 6'-6" 5'-11"	7 7 6	A460 A461 A462	1	5 STR 5 STR 5 STR 5 STR	7'-9'	′ 8	A5	530 531 532	1 1 1	5 5 5	STR STR STR	9'-10" 9'-3" 8'-9"	10 10 9	_	
A4 A5E	6 6	56	STR STR	3'-3" 6'-0"	20 54	A168E A169E	1 1	5 5	STR STR	3'-9" 3'-2"	4 3	A238E A239E	1 5 1 5	STR STR	5'-4" 4'-10"	6 5	A463 A464		5 STR 5 STR				533 534	1 1	5 5	STR STR	8'-2" 7'-7"	9 8	_	
A101E	1	5	STR	42'-4"	44	A170E A171E	1 1	5 5	STR STR	2'-7" 2'-0"	3 2	A240E A241E	1 5 1 5	STR STR	4'-3" 3'-8"	4	A465 A466		5 STR 5 STR				535 536	1 1	5 5	STR STR	7'-0" 6'-6"	777	-	
A102E A103E	1 1	5	STR STR	41'-9" 41'-2"	44 43	A172E A173E	1	5 5	STR STR	42'-4" 41'-9"	44	A242E A243E	1 5 1 5	STR STR	3'-1" 2'-7"	3	A467 A468		5 STR 5 STR				537 538	1 1	5	STR STR	5′-11″ 5′-4″	6 6	-	
A104E A105E	1	5	STR STR	40'-7" 40'-0"	42	A174E A175E	1	5	STR STR	41'-3" 40'-8"	43	A244E	1 5	STR	2'-0"	2	A469 A470		5 STR 5 STR			A5 A5	539 540	1	5	STR STR	4'-10" 4'-3"	5 4		
A106E A107E	1	5	STR STR	39'-5" 38'-11"	41	A176E A177E	1	5	STR STR	40'-1" 39'-6"	42	A401 A402	1 5	STR STR	42'-4" 41'-9"	44	A471 A472	1	5 STR 5 STR	2'-0'	· 2	A5	541 542	1	5	STR STR	<u> </u>	4	-	
A108E A109E	1	5	STR STR	<u>38'-4"</u> <u>37'-9"</u>	40 39	A178E A179E	1	5	STR STR STR	<u> </u>	41 40	A403 A404	1 5 1 5 1 5	STR STR	<u>41'-2"</u> 40'-7"	43	A473 A474	1	5 STR 5 STR	41'-9	" 44	A5	543	1	5	STR STR	2'-7" 2'-0"	3	-	
A110E A111E	1	5	STR STR	<u> </u>	39 38	A180E A181E	1	5	STR STR STR	37'-10" 37'-3"	39 39	A405 A406	1 5 1 5 1 5	STR STR	40'-0"	42	A475 A476	1	5 STR 5 STR	40'-8	" 42			52	<u>л</u>	STR	28'-4"	1,173		
A112E	1	5	STR	36′-0″	38	A182E	1	5	STR	36′-9″	38	A407	1 5 1 5	STR	38'-11"	41	A477	1	5 STR	39'-6	" 41	B2	2E 6	52 52	4	STR	22'-9"	942	_	
A113E A114E	1	5	STR STR	35'-5" 34'-10"	36	A183E A184E	1	5	STR STR	36'-2" 35'-7"	38 37	A408 A409	1 5 1 5	STR STR	<u> </u>	40	A478 A479	1	5 STR 5 STR	38'-5	" 40	B4	4E 2	28	6	STR STR	55'-0" 33'-4"	2,561 1,402		
A115E A116E	1	5 5	STR STR	34'-3" 33'-8"	36 35	A185E A186E	1 1	5 5	STR STR	35'-0" 34'-6"	37 36	A410 A411	1 5 1 5	STR STR	<u> </u>	39 38	A480 A481	1	5 STR 5 STR	37'-3	<i>"</i> 39			86	5	STR	51'-2"	9,926	_	
A117E A118E	1 1	5 5	STR STR	33'-2" 32'-7"	35 34	A187E A188E	1	5 5	STR STR	33'-11" 33'-4"	35 35	A412 A413	1 5 1 5	STR STR	36'-0" 35'-5"	38 37	A482 A483		5 STR 5 STR				1E 2E	1	5 5	STR STR	52'-5" 53'-11"	55 56		
A119E A120E	1	5 5	STR STR	32'-0" 31'-5"	33 33	A189E A190E	1	5 5	STR STR	32'-9" 32'-3"	34 34	A414 A415	1 5 1 5	STR STR	34'-10" 34'-3"	36 36	A484 A485	1	5 STR 5 STR			J1	1E 1	00	4	10	1'-5″	95		
A121E A122E	1	5	STR STR	30'-10" 30'-3"	32 32	A191E A192E	1 1	5	STR STR	31'-8" 31'-1"	33 32	A416 A417	1 5 1 5	STR STR	33'-8" 33'-2"	35 35	A486 A487	1	5 STR 5 STR			K	1E	4	8	1	15'-11"	170	-	
A123E A124E	1	5	STR STR	29'-8" 29'-1"	31 30	A193E A194E	1	5	STR STR	30'-7" 30'-0"	32 31	A418 A419	1 5 1 5	STR STR	<u> </u>	34	A488 A489	1	5 STR 5 STR	33'-4	″ 35	K2	2E 1 3E	12	8 8	2	25'-2" 14'-6"	806 155	-	
A125E A126E	- 1 1	5	STR STR	28'-6" 27'-11"	30 30 29	A195E A196E	- 1 1	5	STR STR STR	29'-5" 28'-10"	31 30	A420 A421	1 5 1 5	STR STR	<u> </u>	33 32	A490 A491	1	5 STR 5 STR	32'-3	" 34	K4		24	6	STR STR	9'-3" 7'-2"	333	-	
A127E	1	5	STR STR STR	27'-4″	29	A197E	1	5	STR	28'-4"	30	A422	1 5 1 5 1 5	STR	30'-3"	32	A492	1	5 STR 5 STR	31'-1	" 32	К	6 1	16 24	4	STR	9′-3″	99		
A128E A129E	1	5	STR	26'-10" 26'-3"	28	A198E A199E	1	5	STR STR	27'-9" 27'-2"	29 28	A423 A424	1 5 1 5 1 5	STR STR	29'-8" 29'-1"	31 30	A493 A494	1	5 STR	30'-0	<i>"</i> 31	К	.8 1	12	4	STR 7	10'-6" 7'-3"	168 58	_	
A130E A131E	1	5	STR STR	25'-8" 25'-1"	26	A200E A201E	1	5	STR STR	26'-7" 26'-1"	28 27	A425 A426	1 5 1 5	STR STR	28'-6" 27'-11"	30 29	A495 A496	1	5 STR 5 STR	28'-10)″ 30			18	4	8	14'-2"	170	_	
A132E A133E	1	5	STR STR	24'-6" 23'-11"	26 25	A202E A203E	1	5	STR STR	25'-6" 24'-11"	27 26	A427 A428	1 5 1 5	STR STR	27'-4" 26'-10"	29 28	A497 A498	1	5 STR 5 STR	27'-9	″ 29	SZ	2E 6	54 54	4 5	5 3	6'-0" 5'-9"	257 384		
A134E A135E	1	5 5	STR STR	23'-4" 22'-9"	24 24	A204E A205E	1	5 5	STR STR	24'-5" 23'-10"	25 25	A429 A430	1 5 1 5	STR STR	26'-3" 25'-8"	27 27	A499 A500		5 STR 5 STR					60 54	4	4	3'-2" 5'-4"	338 228		
A136E A137E	1 1	5 5	STR STR	22'-2" 21'-7"	23 23	A206E A207E	1 1	5 5	STR STR	23'-3" 22'-8"	24 24	A431 A432	1 5 1 5	STR STR	<u> 25'-1" </u> 24'-6"	26 26	A501 A502		5 STR 5 STR			U	J1 3	32	4	5	9'-9"	208		
A138E A139E	1 1	5 5	STR STR	21'-0" 20'-6"	22 21	A208E A209E	1	5 5	STR STR	22'-2" 21'-7"	23 23	A433 A434	1 5 1 5	STR STR	23'-11" 23'-4"	25 24	A503 A504		5 STR 5 STR	24'-11 24'-5		EP0>	XY COA	TED						
A140E A141E	1 1	5	STR STR	19'-11" 19'-4"	21 20	A210E A211E	1	5	STR STR	21'-0" 20'-6"	22 21	A435 A436	1 5 1 5	STR STR	22'-9" 22'-2"	24 23	A505 A506	1	5 STR 5 STR)" 25 " 24		NFORCI NFORCI					4,647 LBS. 26,981 LBS.		
A142E	1	5	STR STR	18'-9" 18'-2"	20 19	A212E A213E	1	5	STR STR	19'-11" 19'-4"	21 20	A437 A438	1 5 1 5	STR STR	21'-7" 21'-0"	23 22	A507 A508	1	5 STR 5 STR	22'-8	″ 24									
A144E A145E	1	5	STR STR	<u>17'-7"</u> 17'-0"	18 18	A214E A215E	1	5	STR STR	18'-9" 18'-3"	20 19	A439 A440	1 5 1 5	STR STR	20'-6" 19'-11"	21	A509 A510	1	5 STR 5 STR	21'-7	" 23									
A146E A147E	1	5	STR STR	16′-5″	17	A216E	1	5	STR	<u>17'-8"</u> 17'-1"	18 18	A 4 4 1 A 4 4 2	1 5 1 5 1 5	STR STR	19'-4"	20	A511	1	5 STR 5 STR	20'-6	.″ 21									
A148E	<u> </u>	5	STR	15'-10" 15'-3"	16	A217E A218E	1 1 1	5	STR STR	16′-7″	17	A443	1 5 1 5	STR	<u>18'-9"</u> <u>18'-2"</u> <u>17'-7"</u>	20 19	A512 A513	1	5 STR	19'-4	" 20									
A149E A150E	1 1	5 5	STR STR	14'-8" 14'-1"	15 15	A219E A220E	1	5 5	STR STR	16'-0" 15'-5"	17 16	A444 A445	1 5 1 5	STR STR	<u>17'-7"</u> <u>17'-0"</u>	18 18	A514 A515	1	5 STR 5 STR	18'-3	" 19									
A151E	1	5 5	STR STR	13'-6" 13'-0"	14	A221E A222E		5	STR STR	14'-10" 14'-4"	15 15	A446 A447	1 5 1 5	STR STR	16'-5" 15'-10"	17	A516 A517		5 STR 5 STR	17'-1	″ 18									
A153E A154E	1	5 5	STR STR	12'-5" 11'-10"	13 12	A223E A224E	1	5 5	STR STR	13'-9" 13'-2"	14	A448 A449	1 5 1 5	STR STR	<u>15'-3"</u> <u>14'-8"</u>	16 15	A518 A519		5 STR 5 STR	16'-0	" 17								PROJECT NO.	
Δ155E A156E	1 1	5 5	STR STR	11'-3" 10'-8"	12 11	A225E A226E	1	5 5	STR STR	12'-8" 12'-1"	13 13	A450 A451	1 5 1 5	STR STR	<u>14'-1"</u> <u>13'-6"</u>	15 14	A520 A521	1	5 STR 5 STR	14'-10)″ 15								<u>CRAVEN</u>	COUNTY
A157E A158E	1	5 5	STR STR	10'-1" 9'-6"	11 10	A227E A228E	1	5 5	STR STR	11'-6" 10'-11"	12 11	A452 A453	1 5 1 5	STR STR	13'-0" 12'-5"	14 13	A522 A523		5 STR 5 STR										STATION: <u>516+87</u>	<u>3(-L-</u>
ິ <u>A159E</u> ລິດ160E	1 1	5	STR STR	8'-11" 8'-4"	9	A229E A230E	1	5	STR STR	10'-5" 9'-10"	11 10	A454 A455	1 5 1 5	STR STR	11'-10" 11'-3"	12 12	A524 A525	1	5 STR 5 STR	13'-2	" 14						MIIII.		SHEET 3 OF 3	
A161E A162E	1	5	STR STR	7'-9" 7'-2"	8	A231E A232E	1	5	STR STR	9'-3" 8'-9"	10 9	A456 A457	1 5 1 5	STR STR	10'-8" 10'-1"	11	A526 A527	1	5 STR 5 STR	12'-1	" 13]				L'INING	TH CARO		STATE OF NORTH CA	
A163E	- 1 1	5	STR STR	6'-7" 6'-0"	7	A233E A234E	1	5	STR STR STR	8'-2" 7'-7"	9	A458 A459	1 5 1 5	STR STR	<u>9'-6"</u> 8'-11"	10 9	A528 A529	1	5 STR 5 STR	10'-11	.‴ 11	1					SEAL 040384		RALEIGH	
<u>9</u>	FIX DE	ENOTES			INFORCING				511			L AJJ	C I I		1 0 -11		AJZJ		אונ ן כ	1 10 - 3							ENGINEER S		SUPERSTRU	UIUKE
stures																									(DocuSigned by ?//// effrey C. Wi CDA045FAFCC9416	12/1 12/1	7/2018	BILL OF MA	V T F R T VI
-Struc																								Г	K	iml	ey»H	orn		\neg \downarrow \Box \uparrow \downarrow \downarrow \vdash \vdash
K:\RDT																													RIGHT L	ANE
B DRAWN E	}Y; D	.D.LOW	WERY		٦Л	ATE: <u>10∕</u>	18																				. ,		REVISIONS NO. BY: DATE: NO. BY:	DATE: SHEET NO
🗧 CHECKED	BY:(<u>C. T. PO</u>	DOLE	J.C.WTI	D/ D/ D/	ΔΤΕ: <u>10/</u>	18														NOT CONSIDI GIGNATURES							presented herein, as an purpose and client for of this document without Associates, Inc. shall be		TOTAL SHEETS 44
	LINO TIN			. <u> </u>	UF	·· La <u> </u>																			Copyright Kiml	ley-Horn and Asso	ociates, Inc., 2018		STRUCTURE 16	









ТОР	OF CAP	ELEVA	TIONS		
A	52.59′	G	54.07′		
B	52.88′	H	54 . 45′		
0	53.03′	I	54.59′		
D	53.41′	U	54.98′		
E	53.55′	K	55.12′		
F	53.93′	Ĺ	55.25′		

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

FOR PILE SPLICE DETAILS, SEE ``14"STEEL PIPE PILE'' SHEET.

BACKWALL SHALL BE PLACED BEFORE APPLYING THE PROTECTIVE COATING.

THE TOP SURFACE AREAS OF THE END BENT CAP SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THAT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.

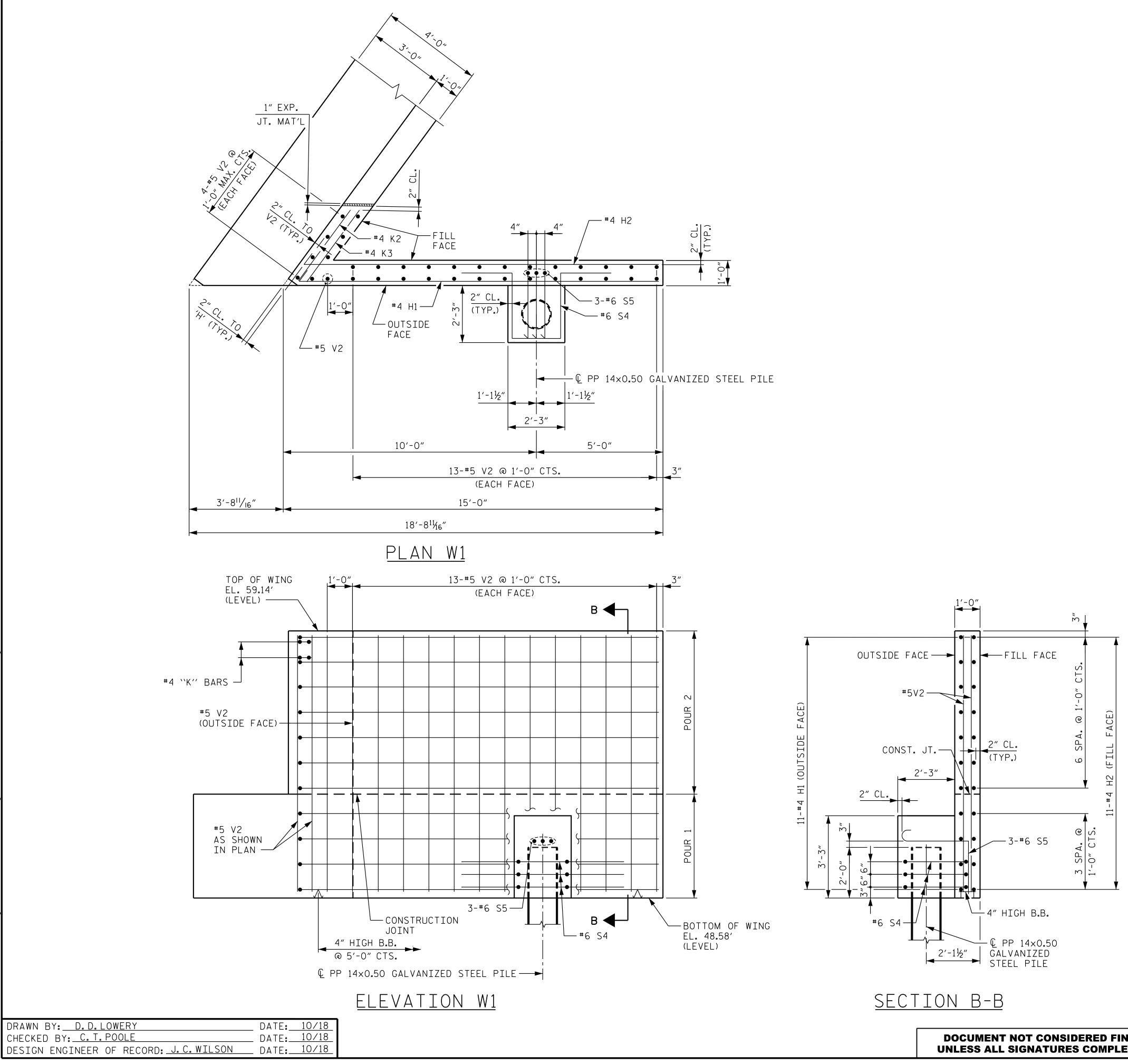
THE TOP SURFACE OF THE CAP EXCEPT THE BRIDGE SEAT BUILDUPS SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE BACK FACE AT THE RATE OF 2%.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL ARE CAST IF SLIP FORMING IS USED.

FOR ``27"Ø CSP CASING DETAIL' SEE ``GENERAL DRAWING' SHEET 2 OF 4.

TOP OF PILE	ELEVATIONS
PILE NO.	ELEVATION
1	50.97′
2	51.49′
3	52.02′
4	52.54′
5	53.06′

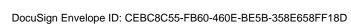
		CRAVE 2 n: 516			UNTY
DocuSigned by: Jeffrey. C. Wilson CDA045FAFCC9416	DEPA	rtment SUB ENE	OF NORTH CAR OF TRAN Raleigh STRUCT) BEN ETAIL	nsporta Ture T 1	TION
Kinley»Horn 421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE # F-0102		R	LIALL IGHT LAI		SHEET NO.
cournent, together with the concepts and designs presented herein, as an nent of services, is intended only for the specific purpose and client for it was prepared. Reuse of and improper reliance of this document without authorization and adaption by Kimley-Horn and Associates, Inc. shall be t liability to Kimley-Horn and Associates, Inc. ght Kimley-Horn and Associates, Inc., 2018	NO. ВҮ: 1 2	DATE:	NO. ВҮ: 3 4	DATE:	S16-29 total sheets 44

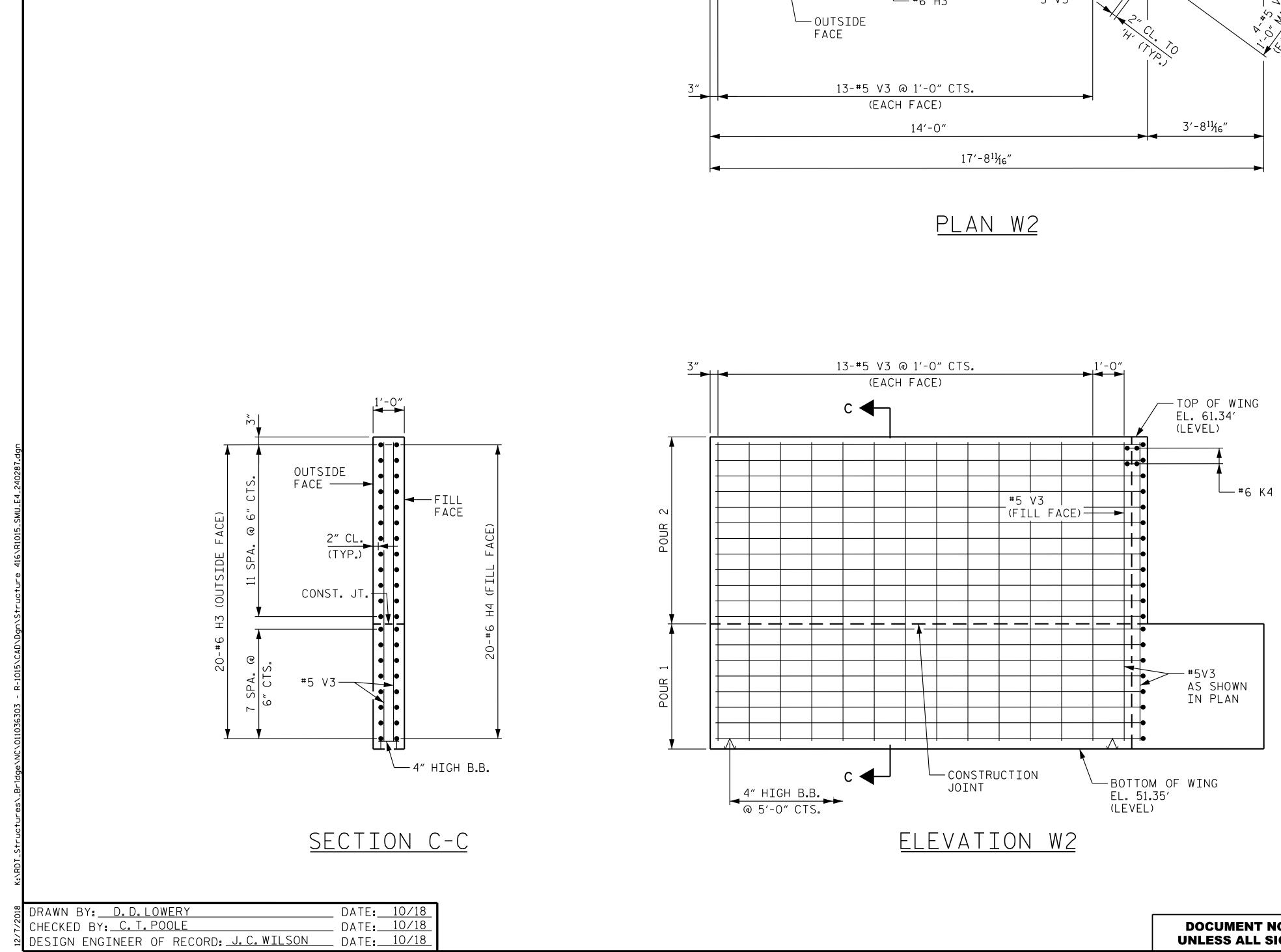


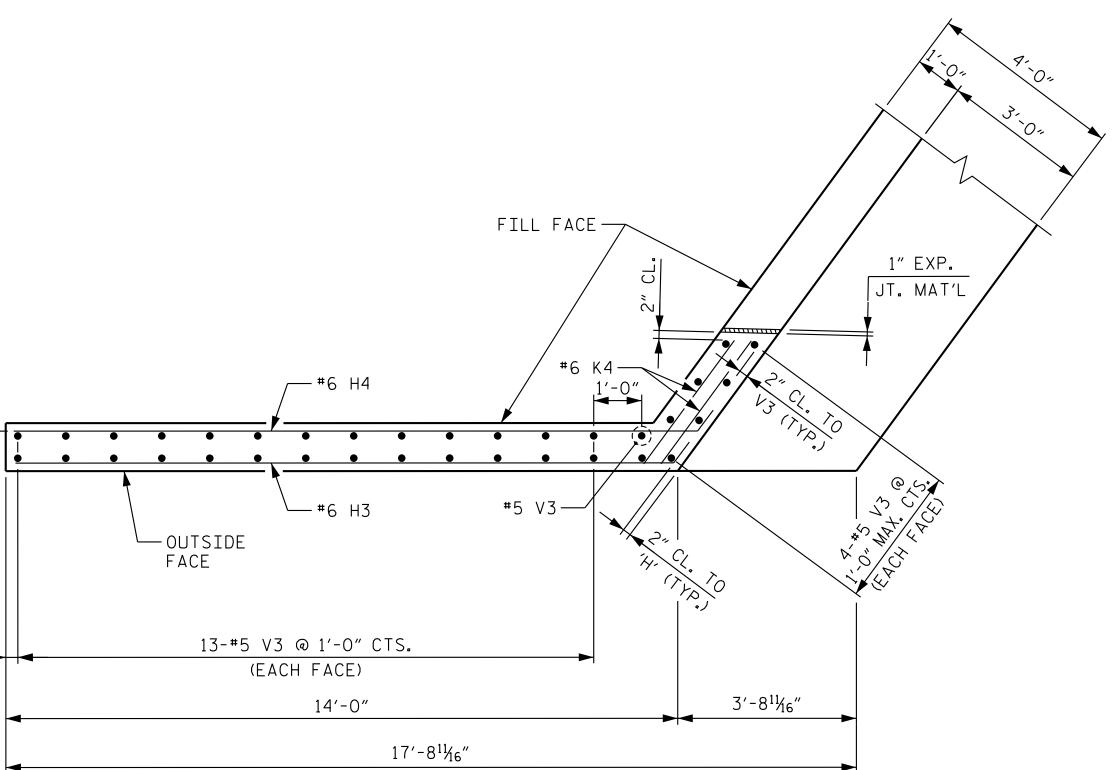


DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

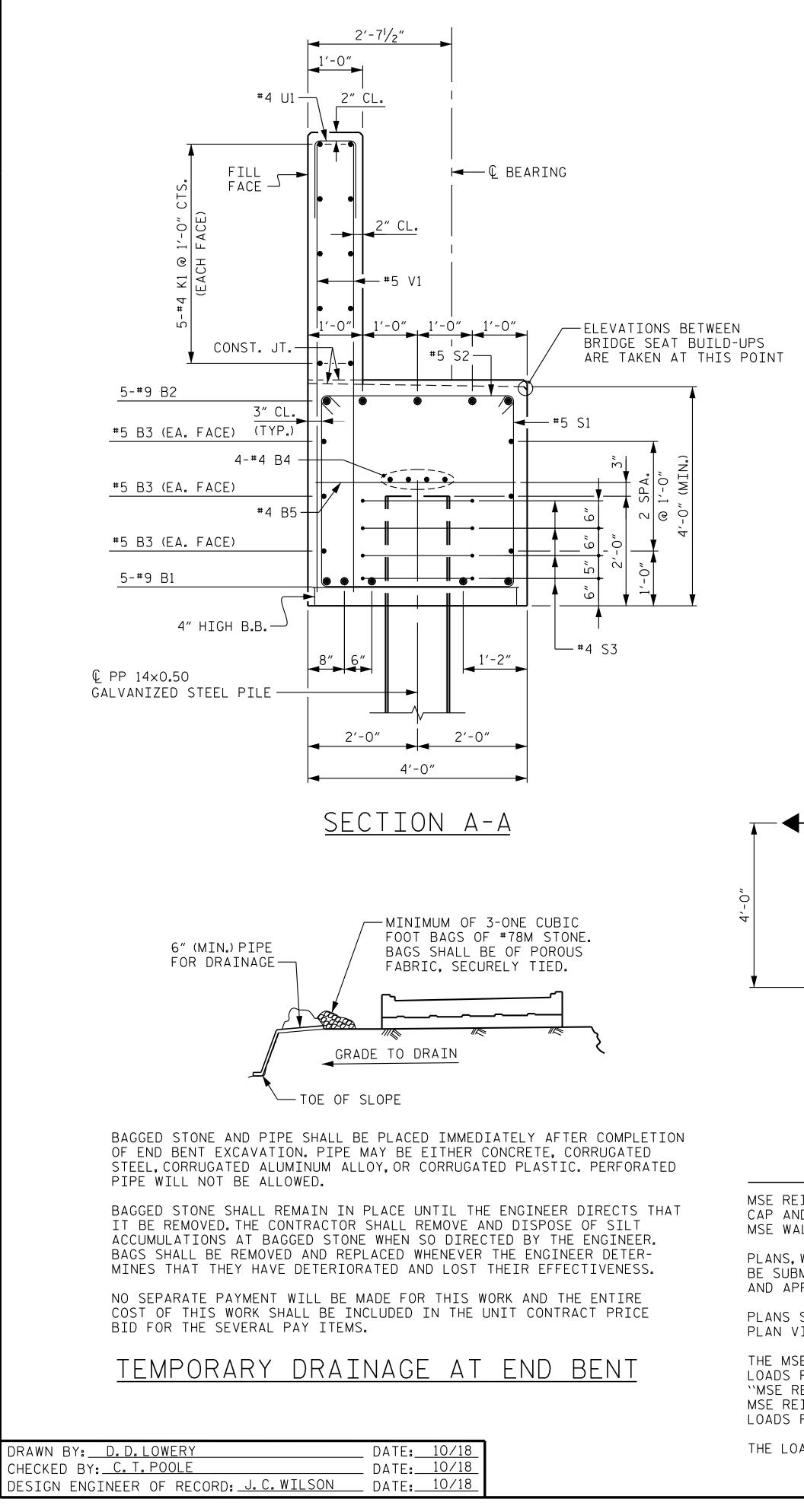
PROJECT NO. R-1015 CRAVEN COUNTY STATION: 516+87.37 -L-SHEET 3 OF 5 TH CAR STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SEAL 040384 SUBSTRUCTURE END BENT 1 Jeffrey C. Wilson 12/7/2018 -CDA045FAFCC9416. SECTIONS AND DETAILS **Kimley Worn** RIGHT LANE 421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102 REVISIONS SHEET NO. S16-30 NO. BY: DATE: DATE: BY: This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc. TOTAL SHEETS 44 Copyright Kimley-Horn and Associates, Inc., 2018





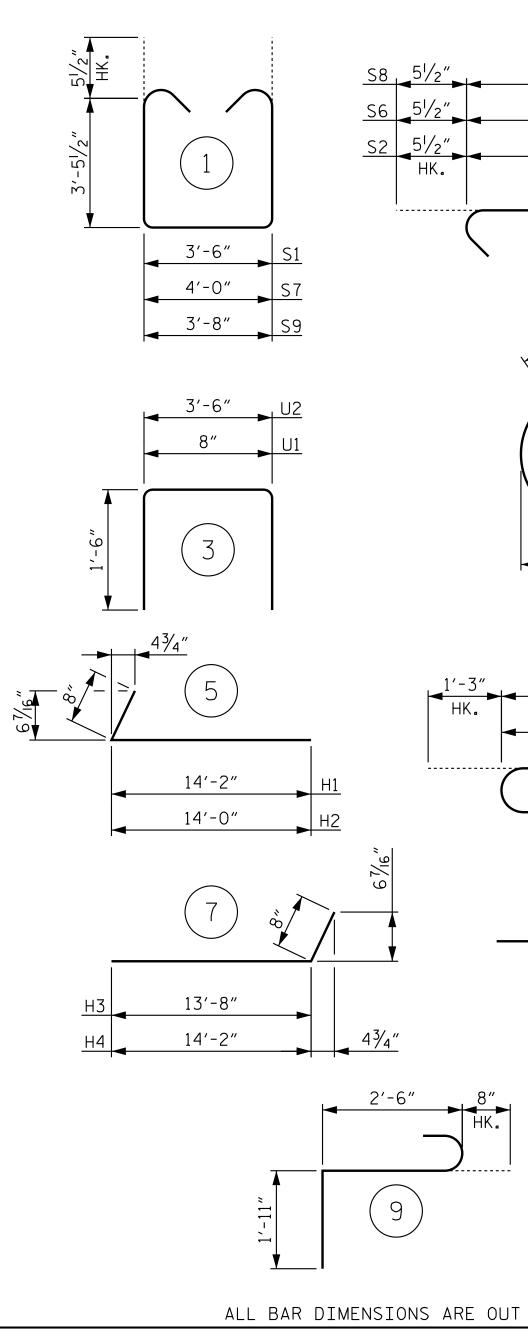


PROJECT NO. R-1015 CRAVEN _ COUNTY STATION: <u>516+87.37</u> -L-SHEET 4 OF 5 TH CAR STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SEAL 040384 SUBSTRUCTURE END BENT 1 Jeffrey C. Wilson 12/7/2018 CDA045FAFCC9416... SECTIONS AND DETAILS **Kimley**»Horn RIGHT LANE 421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102 SHEET NO. REVISIONS S16-31 NO. BY: O. BY: DATE: DATE: This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without written authorization and adaption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc. total sheets 44 Copyright Kimley—Horn and Associates, Inc., 2018



K:\RDT Structures\ Bridge\NC\O11036303 - R-1015\CAD\Don\Structure 416\R1015 SMU F5 240287.d

BAR TYPES



- **4** 2.7 K/FT

MSE REINFORCING STRAP LOAD DETAIL

MSE REINFORCING STRAP NOTES

MSE REINFORCING STRAPS SHALL BE ATTACHED TO THE END BENT CAP AND/OR BACKWALL. FOR DESIGN CRITERIA AND DETAILS, SEE MSE WALL SHEETS AND SPECIAL PROVISIONS.

PLANS, WORKING DRAWINGS, AND DESIGN CALCULATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER FOR REVIEW AND APPROVAL, SEE SPECIAL PROVISIONS.

PLANS SUBMITTED FOR REVIEW SHALL INCLUDE THE FOLLOWING: PLAN VIEW,ELEVATION VIEW,TYPICAL SECTIONS,AND STRAP DETAILS.

THE MSE REINFORCING STRAPS SHALL BE DESIGNED TO CARRY THE LOADS FROM THE BRIDGE SUPERSTRUCTURE AS INDICATED IN THE ``MSE REINFORCING STRAP LOAD DETAIL'. IN ADDITION, THE MSE REINFORCING STRAPS SHALL ALSO BE DESIGNED TO CARRY LOADS FROM SOIL PRESSURE AS OUTLINED IN THE SPECIAL PROVISION.

THE LOADS IN THE DETAIL ABOVE ARE FACTORED LOADS.

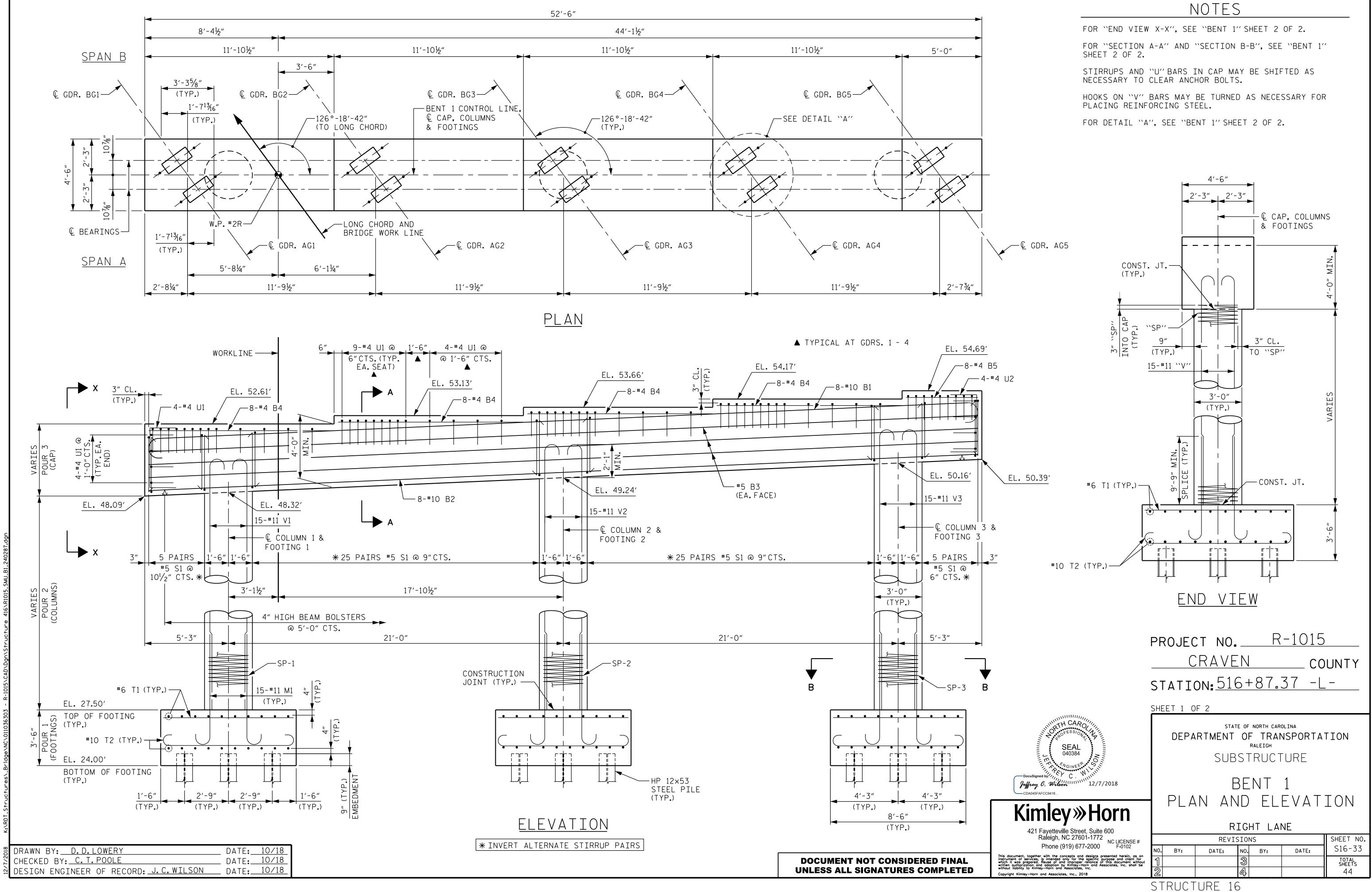
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

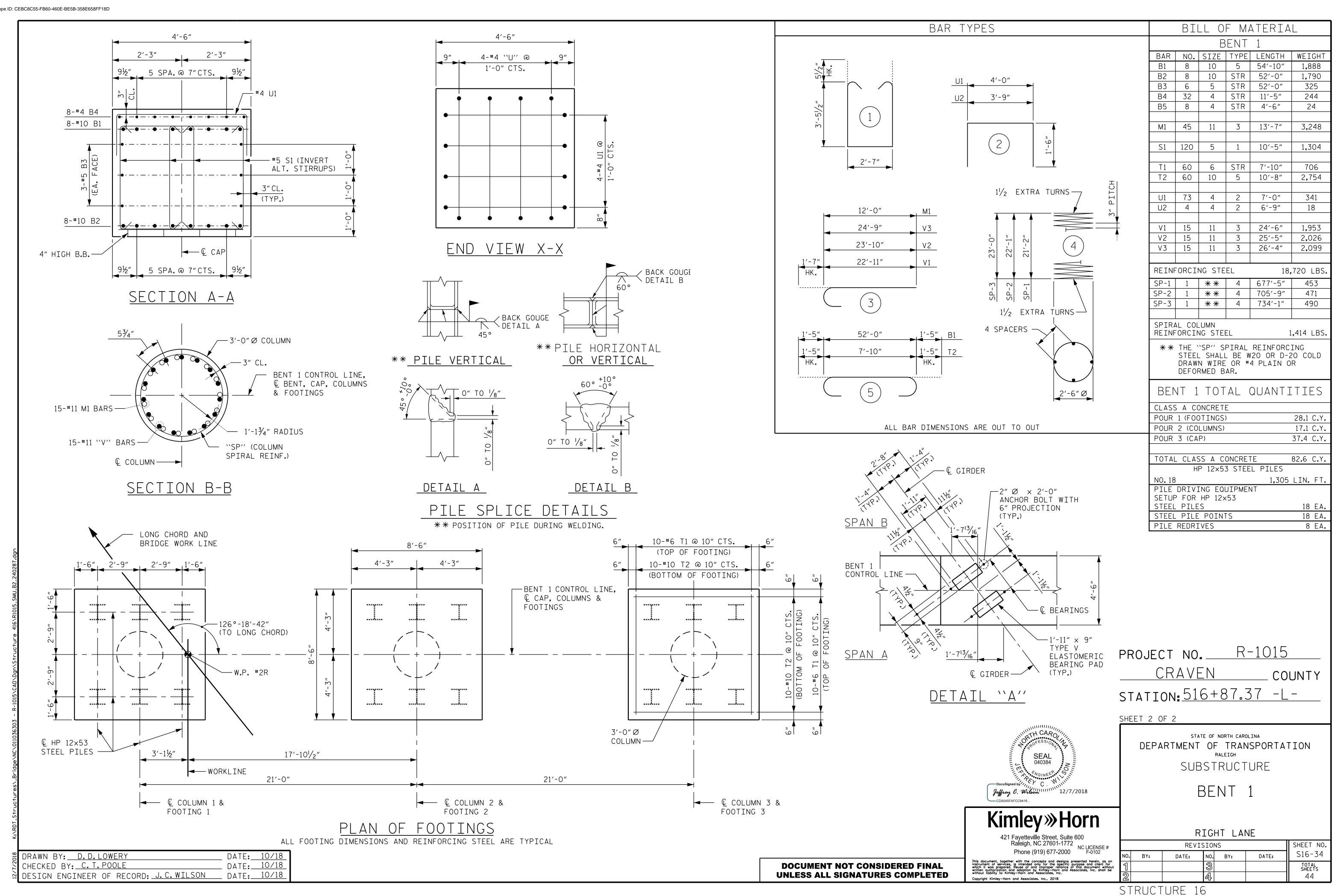
Copyright Kimley-Horn and Associates, Inc., 2018

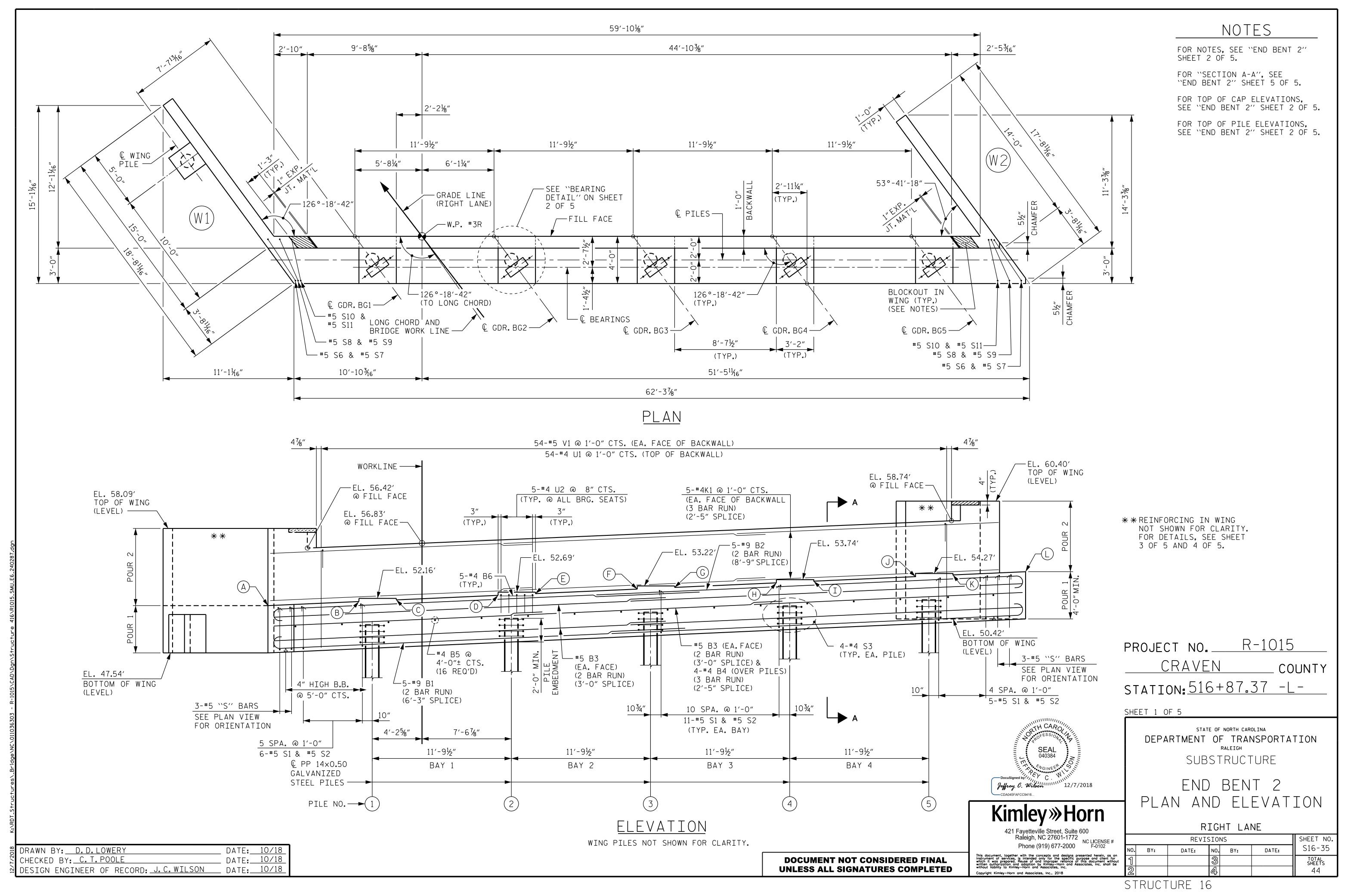
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE #

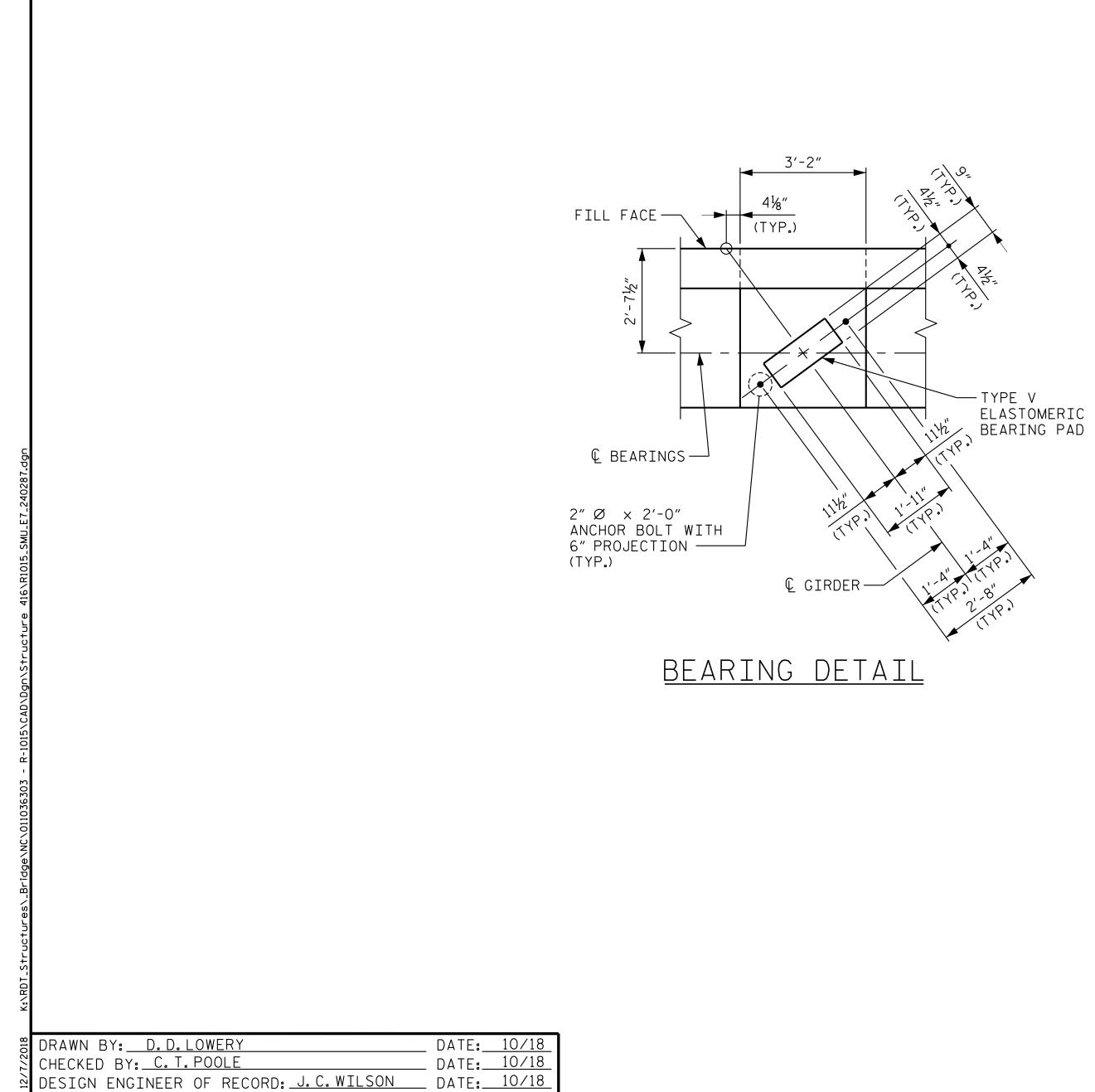
		BILL OF MATERIAL								
			ENC) BEN	NT 1					
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT				
	B1	10	9	6	34'-7"	1,176				
<u> </u>	B2	10	9	6	35'-8"	1,213				
4'-0" 51/2"	B3	12	5	STR	31'-7"	395				
	B4	12	4	STR	21'-8"	174				
3′-6″ <u>5¹/2</u> ″	B5	16	4	STR	3'-6"	37				
HK.	B5 B6	25	4	STR	2'-8"	45				
		25		311	2 0	75				
	111	11	1		14'-10"	109				
	H1	11	4	5						
(2)	H2	11	4	5	14'-8"	108				
)	H3	20	6	7	14'-4"	431				
/── 1′-3″ LAP	H4	20	6	7	14'-10"	446				
	K1	30	4	STR	21'-8"	434				
	К2	2	4	STR	3'-4"	4				
	КЗ	2	4	STR	3′-6″	5				
$\left(\frown \right)$	K4	4	6	STR	3'-2"	19				
$\mathbf{V} \sim \mathbf{V}$	S1	54	5	1	11'-4"	638				
	S2	54	5	2	4'-5"	249				
	S3	20	4	4	7'-7"	101				
2'-0"Ø	S4	3	6	8	10'-9"	48				
	S5	3	6	9	5'-1"	23				
	S6	2	5	2	4'-11"	10				
	S7	2	5	1	11'-10"	25				
		2	5	2	4'-7"	10				
	<u> </u>	2	5	1	11'-6"	24				
33'-4" B1		<u>۲</u>		1	11 0	<u></u>				
	U1	53	4	3	3'-8"	130				
34'-5" B2		25	4	3	6'-6"	109				
	U2	25	4	5	0-0	109				
	\/1	100	E	стр	<u> </u>	0.0.4				
	V1	106	5	STR	8'-0"	884				
- (6)	V2	35	5	STR	10'-1"	368				
\bigcirc	V3	35	5	STR	9′-6″	347				
2'-0"										
	REINF		G STEEL			7,562 LBS.				
		CLAS	SS A CC	NCRET	E BREAKDOV	۷N				
	POUR	1 (CAF	°& LOV	VER WI	NG)	40.4 C.Y.				
	POUR	2 (BA	CKWALL	& UPF	PER					
5,-5					F WING)	16.8 C.Y.				
Ň	TOTAL	CLASS	S A CON	NCRETE		57.2 C.Y.				
└──── └ ──── ⋎	Р	P 14×0	.50 GA	LVANIZ	ED STEEL I	PILES				
1'-11"	NO. 6					540				
		PTIF F	PLATES			6 EA.				
4	PIPE					3 EA.				
						J LA.				
	FOR P	DRIVII P 14×(NG EQU]).50 GAI	VANT7	SEIUP FD					
-	STEEL		ALC UAL			6 EA.				
TTO OUT.										
		о т		С	-1015					
	PROJE	CI	N0	Г	<u>R-1015</u>)				
	(
	(JRA	<u>VEN</u>		CC	DUNTY				
					י ככ					
	STATI	ON:	<u>- 01C</u>	FQ (,	<u>.37 -L</u>					
	SHEET 5	OF 5								
NI OF EESSION				NORTH CA		T T A b b				
	DEPARTMENT OF TRANSPORTATION Raleigh SUBSTRUCTURE									
LEF & KALL R & CO			SUBS_	IRUC	IURE					
VGINEE W										
DocuSigned by 11, 7 C. 11, 11, 11, 11, 11, 11, 11, 11, 11, 11		F		RFN	VT 1					
CDA045FAFCC9416		L 								
	HSFC	[]] C	INS	ΑΝΓ) DET	AILS				
Kimley»Horn		· - V	🗸							
			DTO	цт і и						
421 Favetteville Street, Suite 600			КТС	HT L4						

		SHEET NO.				
N0.	BY:	DATE:	NO.	BY:	DATE:	S16-32
ſ			N			TOTAL SHEETS
2			4			44
C T		<u> </u>				









1/2	CHECKED) BY: <u>C.T</u> .	PO	OLE	DATE:	10/
12/7	DESIGN	ENGINEER	OF	RECORD: J.C.WILSON	DATE:	10/

TOP	OF CAP	ELEVA	TIONS
A	51.54′	G	53.09′
B	51.89′	H	53.47'
C	52.03′	I	53.62′
D	52.42′	J	54.00′
E	52.56′	K	54.15′
F	52.96′		54.42′



STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

FOR PILE SPLICE DETAILS, SEE ``14"STEEL PIPE PILE'' SHEET.

BACKWALL SHALL BE PLACED BEFORE APPLYING THE PROTECTIVE COATING.

THE TOP SURFACE AREAS OF THE END BENT CAP SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THAT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.

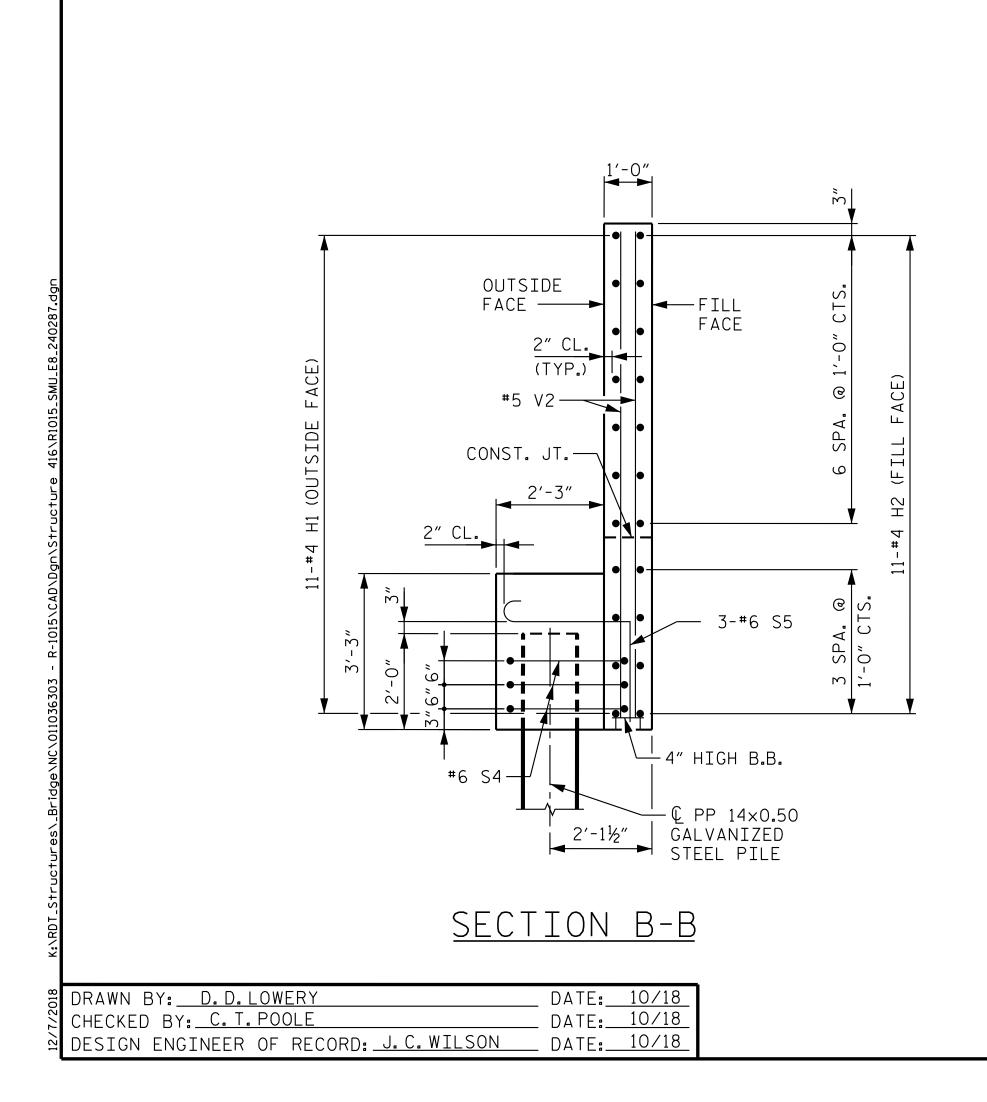
THE TOP SURFACE OF THE CAP EXCEPT THE BRIDGE SEAT BUILDUPS SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE BACK FACE AT THE RATE OF 2%.

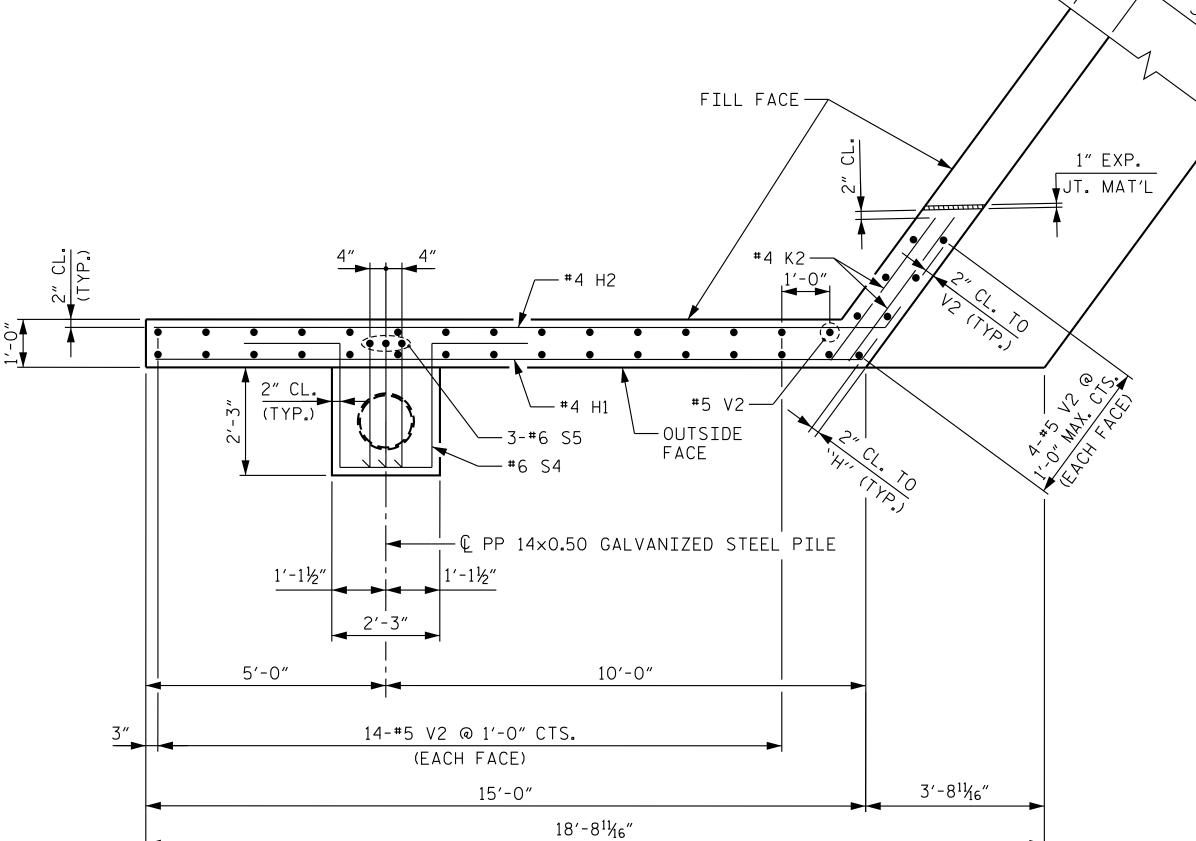
THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL ARE CAST IF SLIP FORMING IS USED.

FOR ``27"Ø CSP CASING DETAIL' SEE ``GENERAL DRAWING' SHEET 2 OF 4.

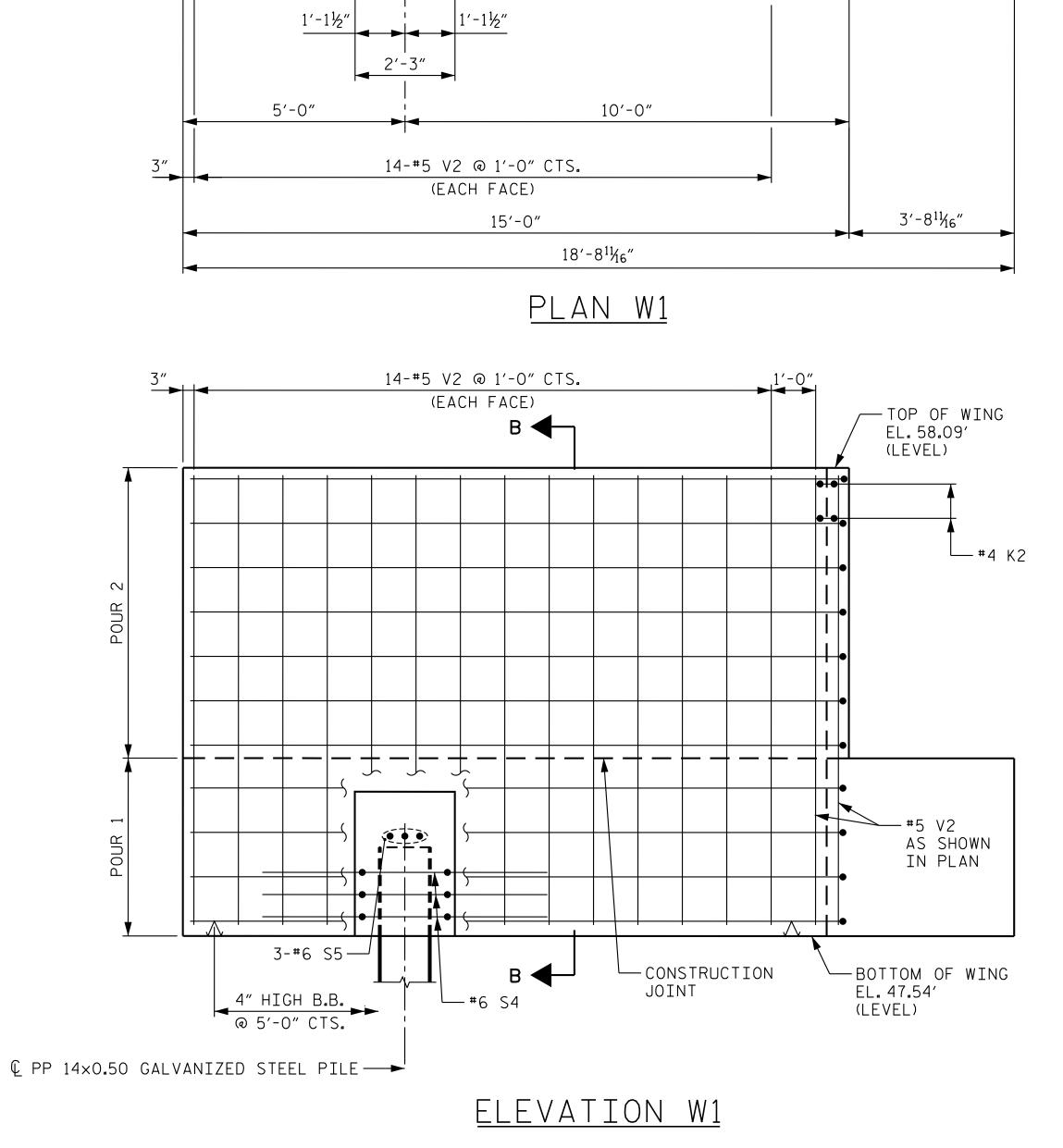
TOP OF PILE	ELEVATIONS
PILE NO.	ELEVATION
1	49.94′
2	50.46′
3	50.99′
4	51.52′
5	52.05′

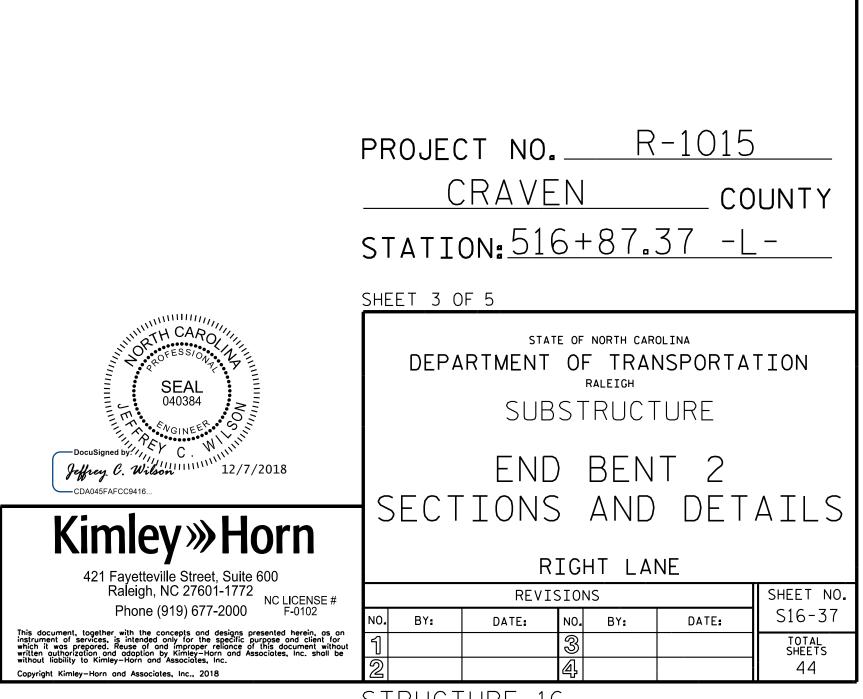
	PROJECT NO. <u>R-1015</u> <u>CRAVEN</u> COUNTY STATION: 516+87.37 -L- SHEET 2 OF 5
Docusigned by: SEAL 040384 040384 040384 040384 040384 12/7/2018 CDA045FAFCC9416 Kindley » Hoon Raleigh, NC 27601-1772 Phone (919) 677-2000 NC LICENSE # F-0102	SHEET 2 OF 3 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE END BENT 2 DETAILS RIGHT LANE REVISIONS NO. BY: DATE: NO. BY: DATE: SHEET NO. S16-36
This document, together, with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without without inability to Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc. Copyright Kimley-Horn and Associates, Inc., 2018	1 3 TOTAL 2 4 4 44
	STRUCTURE 16

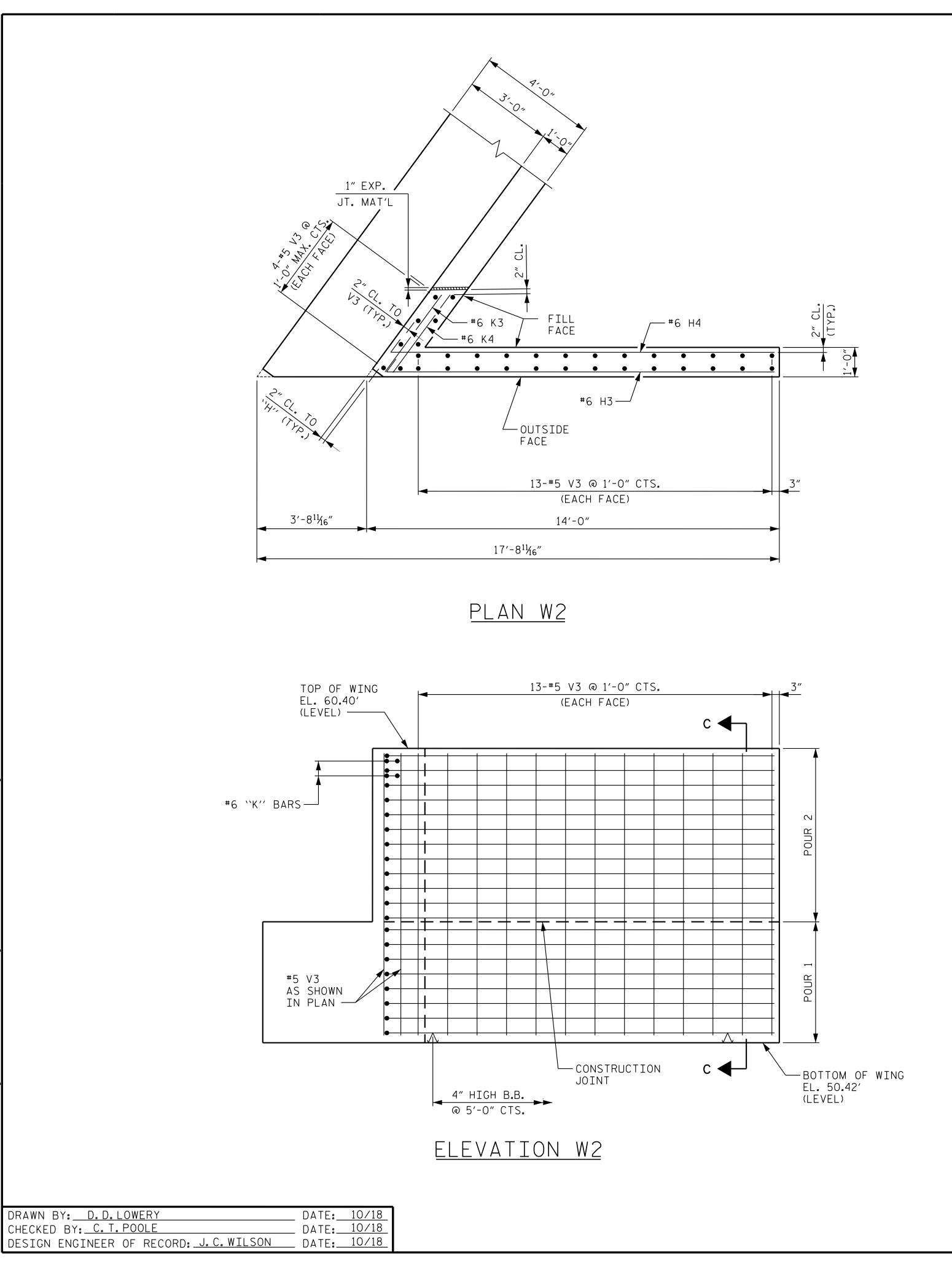


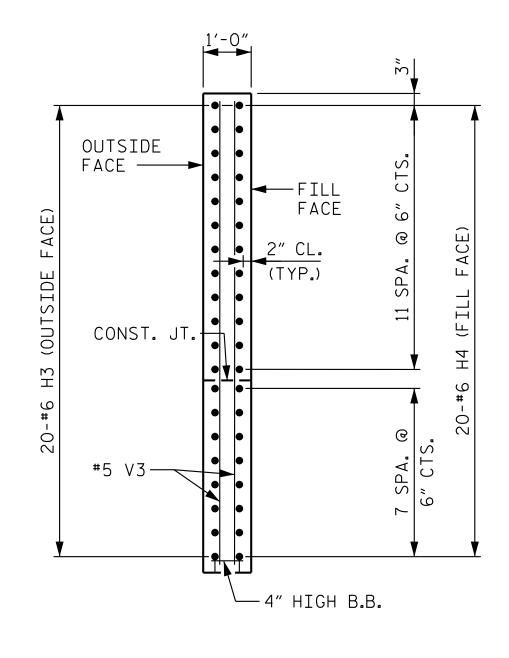








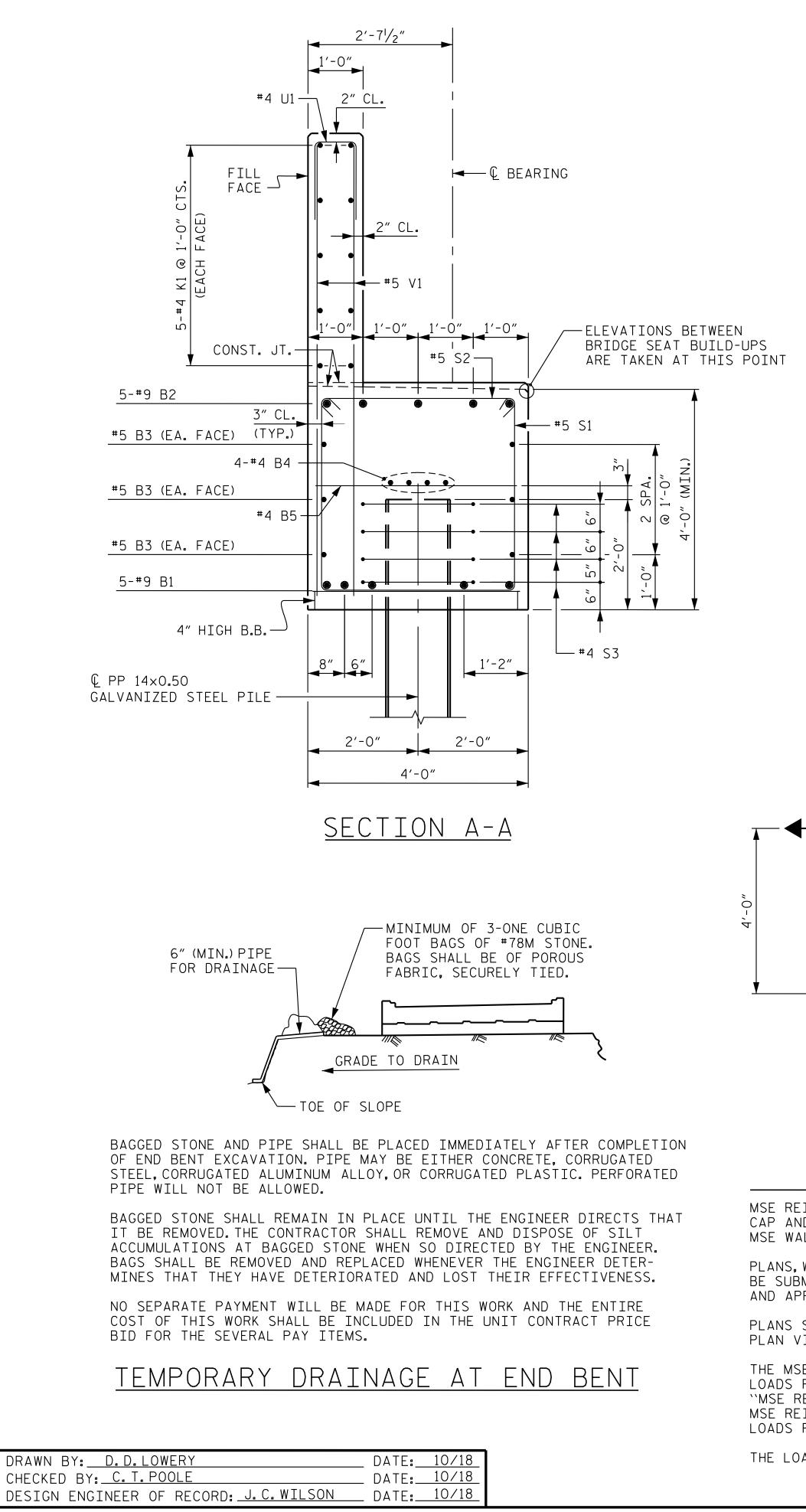


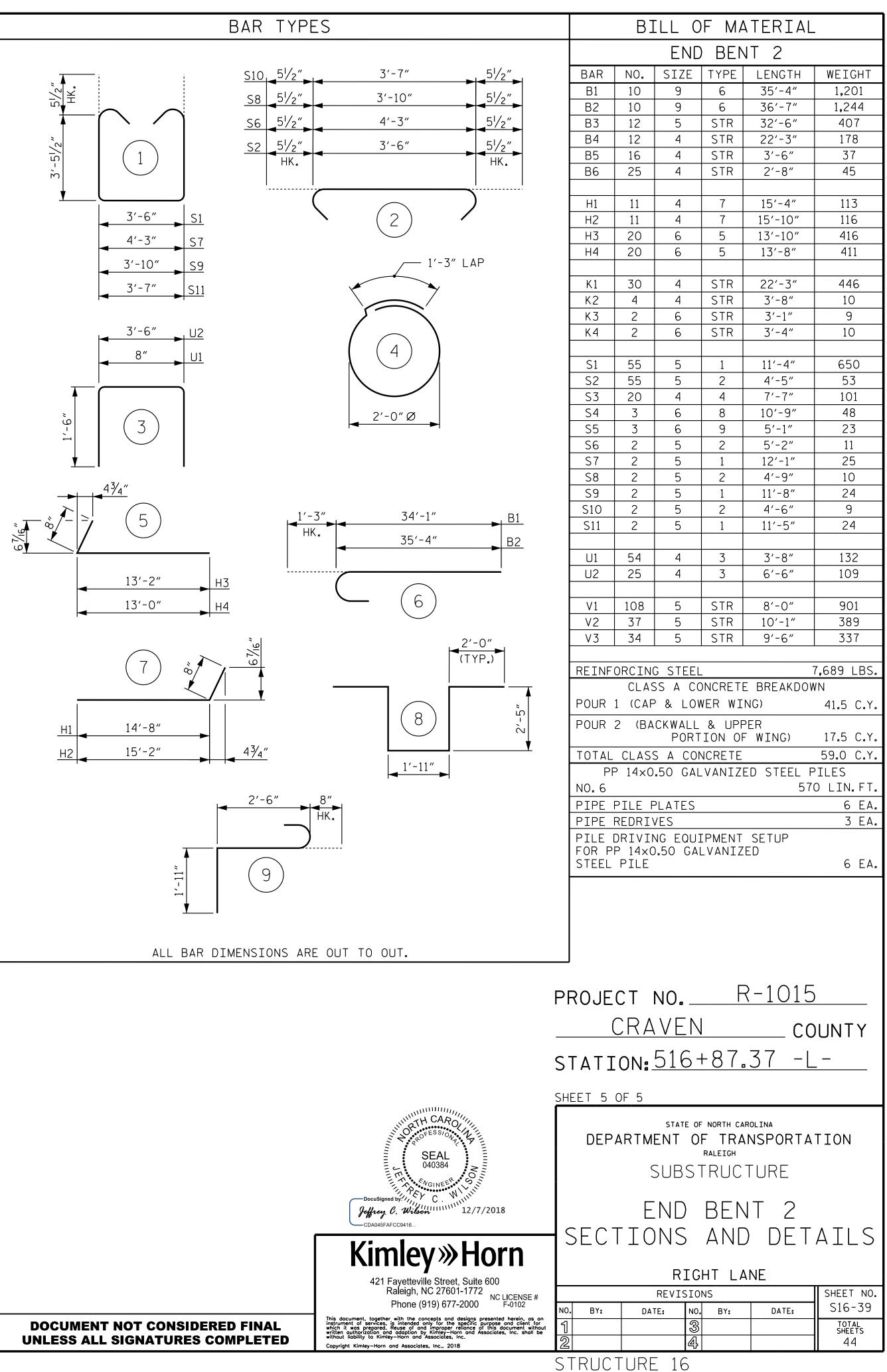


<u>SECTION C-C</u>

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT NO. R-1015 CRAVEN _ COUNTY STATION: 516+87.37 -L-SHEET 4 OF 5 TH CAR STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SEAL 040384 SUBSTRUCTURE END BENT 2 Jeffrey C. Wilson 12/7/2018 -CDA045FAFCC9416. SECTIONS AND DETAILS **Kimley Worn** RIGHT LANE 421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102 REVISIONS SHEET NO. S16-38 NO. BY: DATE: DATE: O. BY: This document, together with the concepts and designs presented herein, as an instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document without without inbility to kimley-Horn and Associates, Inc. shall be without liability to kimley-Horn and Associates, Inc. TOTAL SHEETS 44 Copyright Kimley—Horn and Associates, Inc., 2018





2.7 K/FT

MSE REINFORCING STRAP LOAD DETAIL

MSE REINFORCING STRAP NOTES

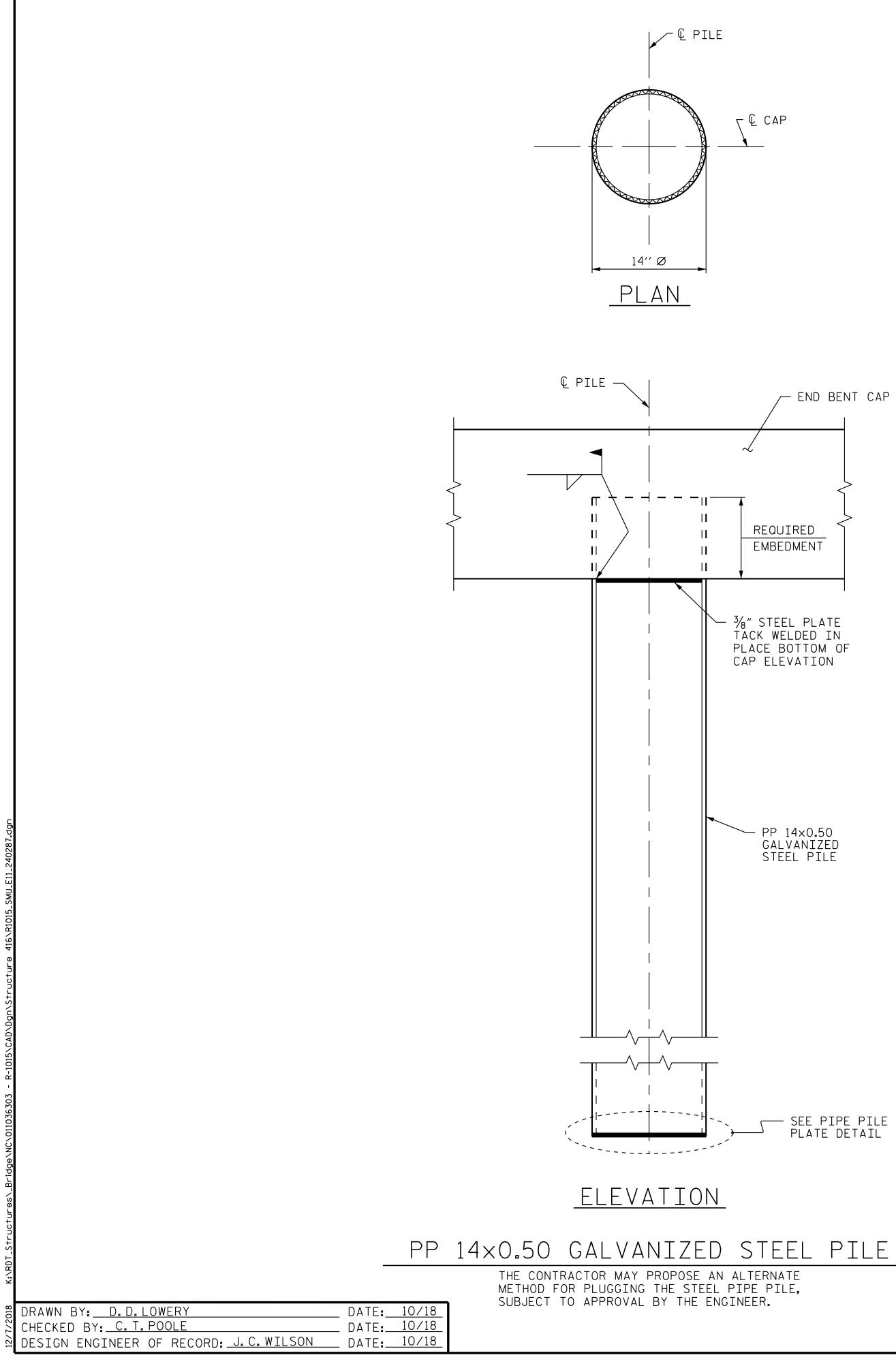
MSE REINFORCING STRAPS SHALL BE ATTACHED TO THE END BENT CAP AND/OR BACKWALL. FOR DESIGN CRITERIA AND DETAILS, SEE MSE WALL SHEETS AND SPECIAL PROVISIONS.

PLANS, WORKING DRAWINGS, AND DESIGN CALCULATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER FOR REVIEW AND APPROVAL, SEE SPECIAL PROVISIONS.

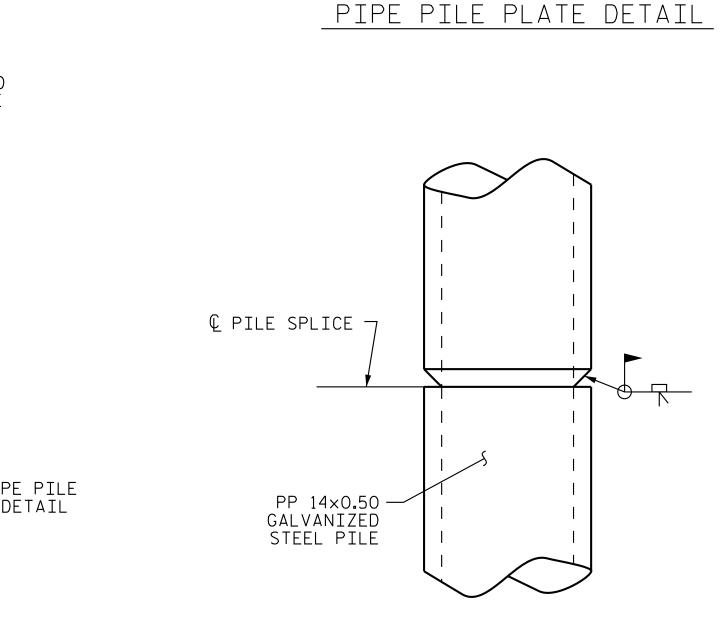
PLANS SUBMITTED FOR REVIEW SHALL INCLUDE THE FOLLOWING: PLAN VIEW, ELEVATION VIEW, TYPICAL SECTIONS, AND STRAP DETAILS.

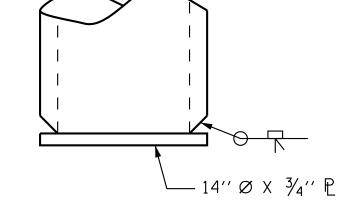
THE MSE REINFORCING STRAPS SHALL BE DESIGNED TO CARRY THE LOADS FROM THE BRIDGE SUPERSTRUCTURE AS INDICATED IN THE "MSE REINFORCING STRAP LOAD DETAIL". IN ADDITION, THE MSE REINFORCING STRAPS SHALL ALSO BE DESIGNED TO CARRY LOADS FROM SOIL PRESSURE AS OUTLINED IN THE SPECIAL PROVISION.

THE LOADS IN THE DETAIL ABOVE ARE FACTORED LOADS.



PIPE PILE SPLICE DETAIL





NOTES

PIPE PILES SHALL BE IN ACCORDANCE WITH SECTION 1084 OF THE STANDARD SPECIFICATIONS.

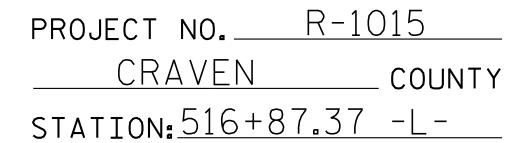
GALVANIZE STEEL PIPE PILES IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS UNLESS METALLIZING IS REQUIRED. GALVANIZING OR METALLIZING PIPE PILE PLATES IS NOT REQUIRED.

PIPE PILE PLATES, IF REQUIRED, SHALL BE IN ACCORDANCE WITH SECTION 450 OF THE STANDARD SPECIFICATIONS.

REMOVE AND REPLACE OR REPAIR TO THE SATISFACTION OF THE ENGINEER PILES THAT ARE DAMAGED, DEFORMED OR COLLAPSED DURING INSTALLATION OR DRIVING.

PILE SPLICES SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND AWS D1.1.

THE GALVANIZING IS CONSIDERED INCIDENTAL TO THE CONTRACT UNIT PRICE BID PER LINEAR FOOT FOR PP 14×0.50 GALVANIZED STEEL PILES.



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

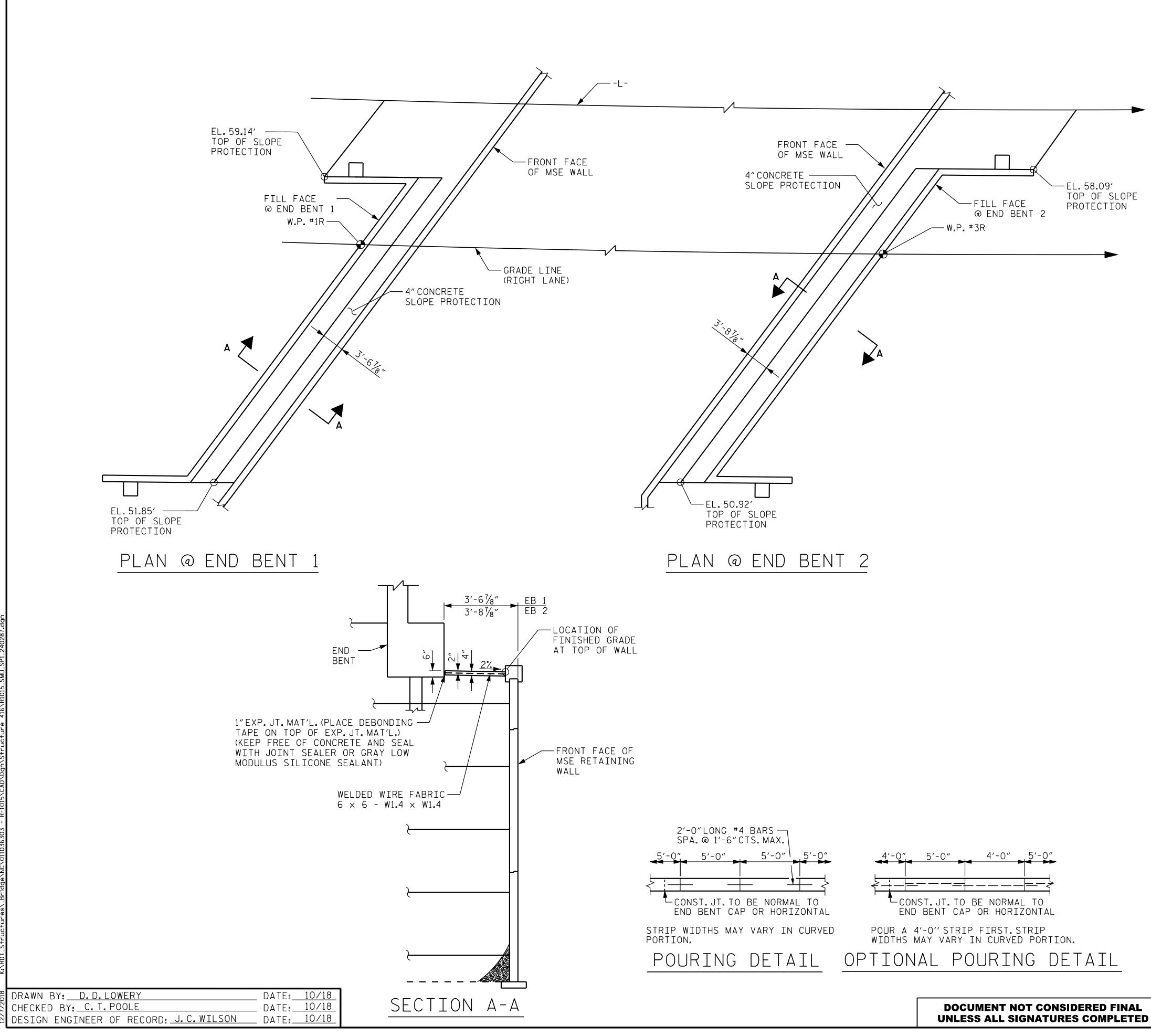
14" STEEL PIPE PILE

RIGHT LANE						
					SHEET NO.	
NO.	BY:	DATE:	N0.	BY:	DATE:	S16-40
1			හ			TOTAL SHEETS
2			4			44
STRUCTURE 16						



421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102

Inis document, together with the concepts and designs presented herein, as ar instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document withou written authorization and adoption by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates. Inc. Copyright Kimley-Horn and Associates, Inc., 2018



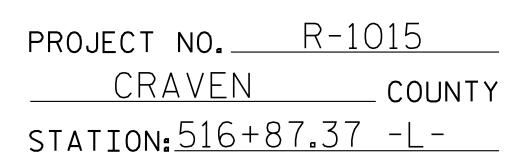
NOTES

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS.STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.FOR BERM WIDTH, SEE GENERAL DRAWING.

SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL''WITH 2'-O"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.516+87.37 (RIGHT LANE)	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX.L.F.
END BENT 1	52	100
END BENT 2	54	105

* QUANTITY SHOWN IS BASED ON 5' POURS.



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SLOPE PROTECTION DETAILS

RIGHT LANE

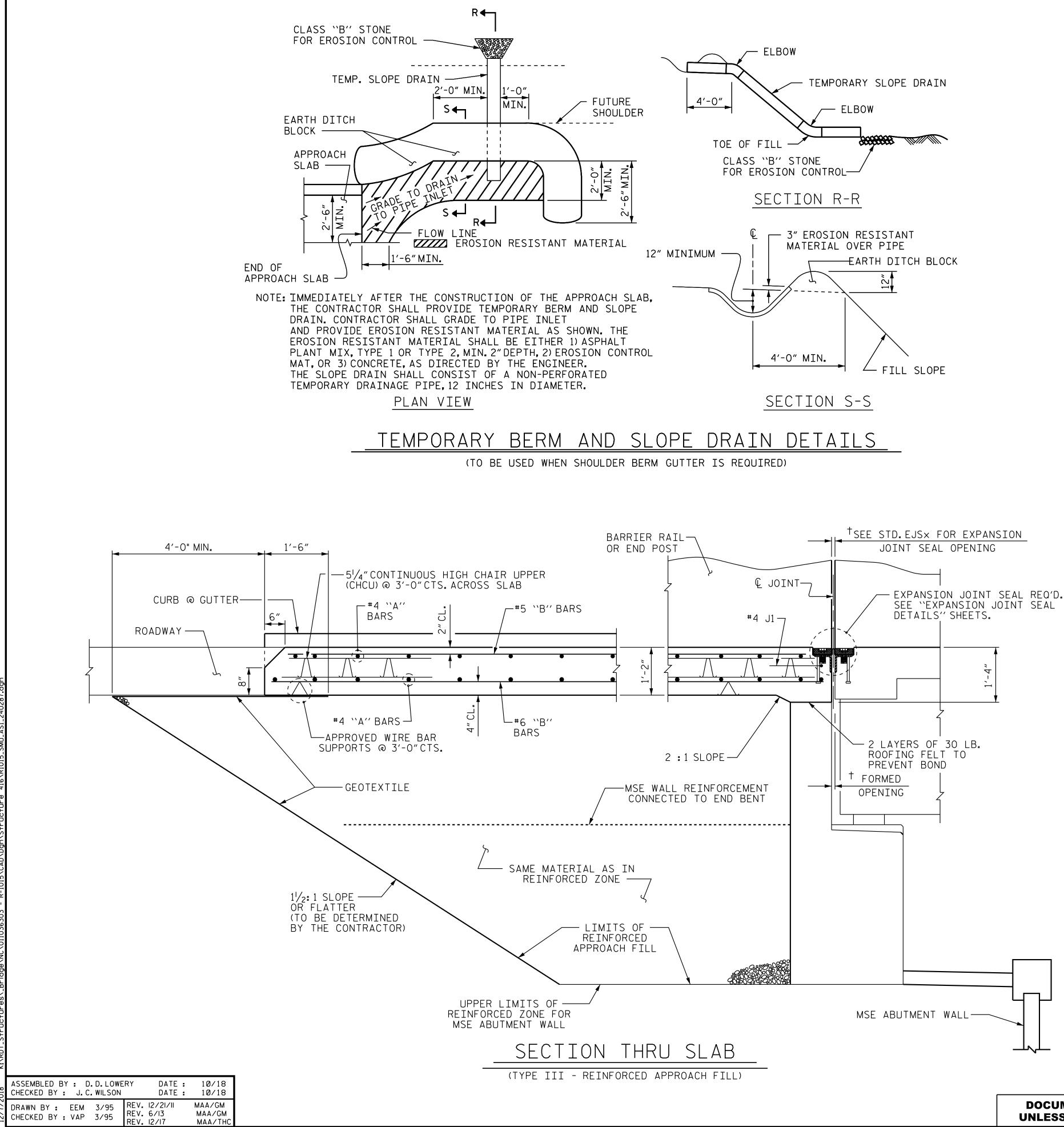
	REVISIONS				SHEET NO.	
N0.	BY:	DATE:	N0.	BY:	DATE:	S16-41
ſ			I			TOTAL SHEETS
2			4			44
C T		URE 16				



421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772 Phone (919) 677-2000 F-0102

This document, together with the concepts and designs presented herein, as instrument of services, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance of this document with written authorization and adaption by Kimley-Horn and Associates inc. shall be without liability to Kimley-Horn and Associates inc.

SIKULIUKE 16



ROADWAY PLANS.

ZONE.

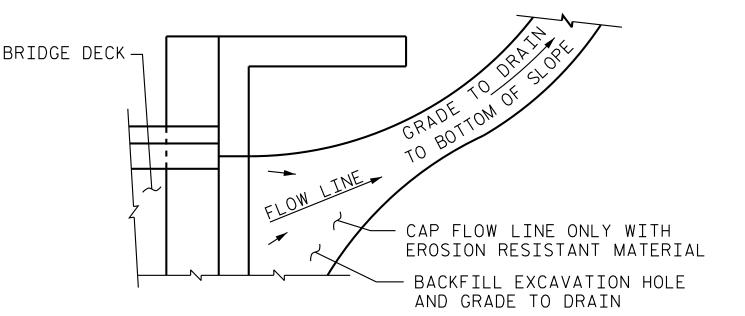
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

	Ν	0	Т	ES	
--	---	---	---	----	--

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, MSE WALL REINFORCEMENT AND BACKFILL MATERIAL SEE ROADWAY PLANS. GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056. BACKFILL MATERIAL SHALL BE THE SAME MATERIAL USED IN THE MSE REINFORCED APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

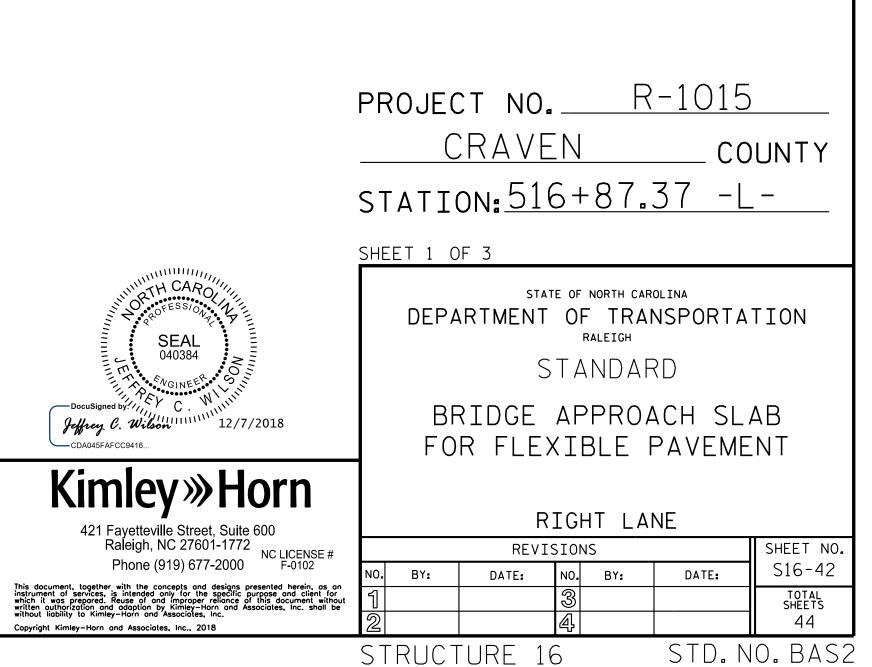
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED.SEE

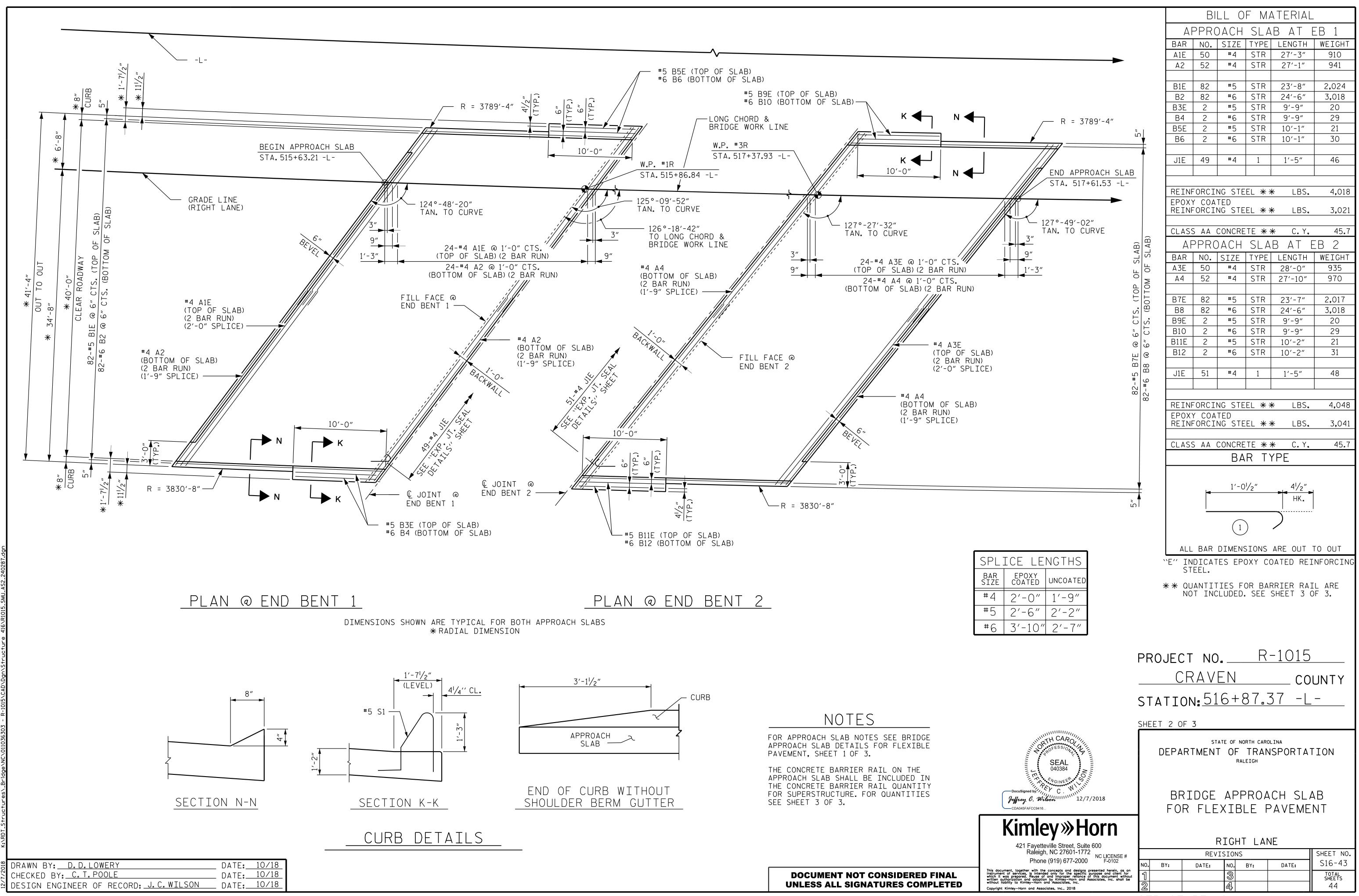
FOR EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.

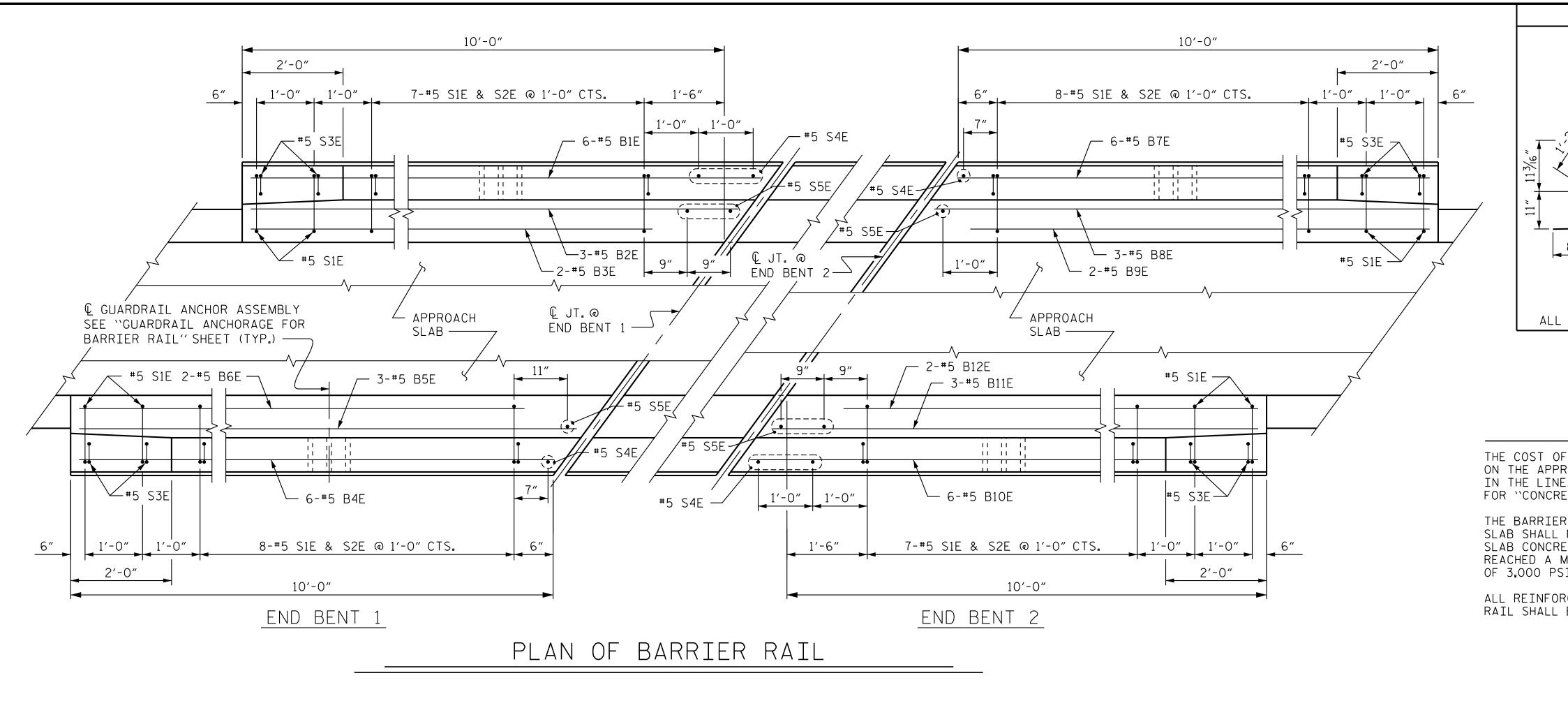


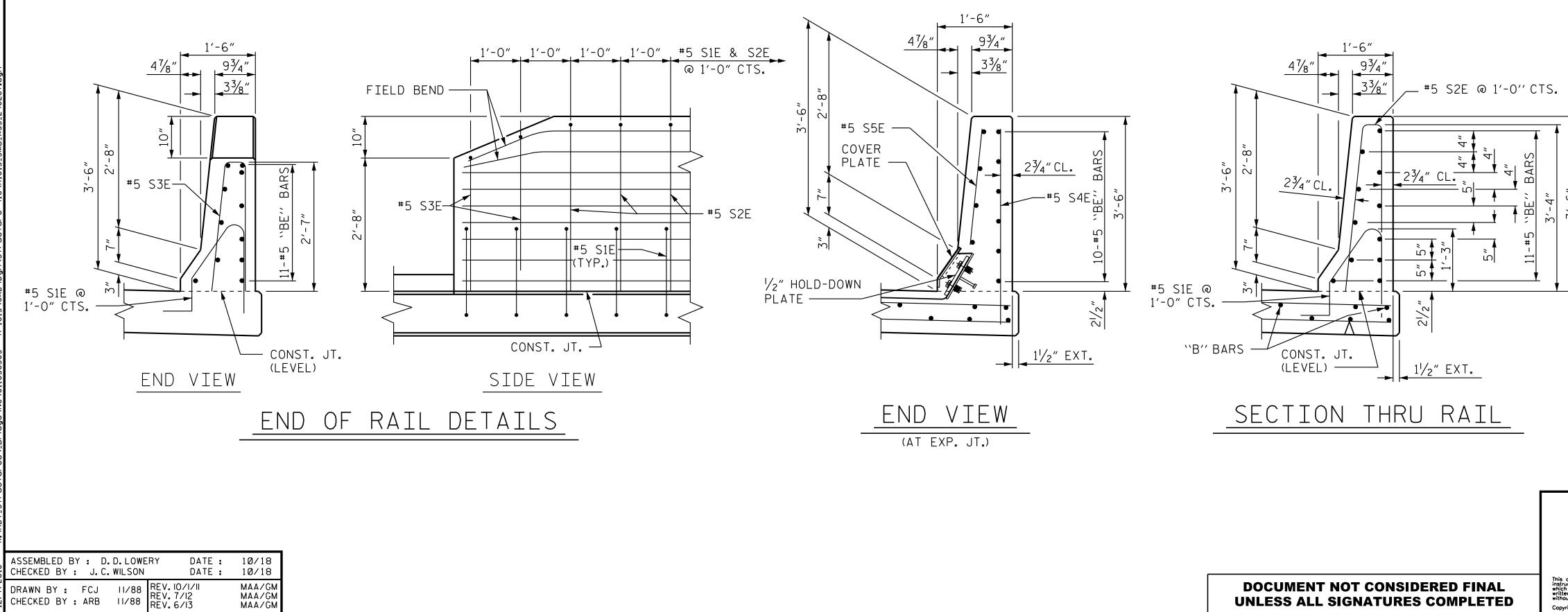
NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION. GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL









_								
BAR TYPES							TERIAL	
1' - 0'/2''		BAF		NO.	ETE BA SIZE	ARRIER TYPE	RAIL AT	EB 1 WEIGHT
87/16"		B1E		6	#5	STR	10'-5"	65
$ 5^{3}/_{4}'' $	1″	B2E		3	# 5	STR	10'-0"	31
		B3E		2	#5	STR	8′-5″	18
	† †	B4E		6	#5	STR	9'-10"	62
		B5E		3	# 5	STR	10'-2"	32
		B6E	<u> </u>	2	#5	STR	9'-6"	20
$\begin{array}{c c} & & & & & \\ & & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & & \\$	⊳ `\	S1E	<u>-</u>	19	#5	1	5′-1″	101
		S1E S2E		<u>19</u> 15	#5 #5	2	5'-1" 7'-0"	101
		S3E		4	#5	2	5'-6"	23
8″		S4E		3	#5	STR	3'-11"	12
	S S J S	S5E	Ξ	3	#5	STR	2'-4"	7
(2)				OATED				
BAR DIMENSIONS ARE OUT TO O	IIT				STEEL		2.9	481 LBS. CU. YDS.
BAR DIMENSIONS ARE OUT TO O		CONC			<u>CRETE</u> RIER F	2 A T I	2.9	LIN.FT.
				<u>bari</u> BTI I			TERIAL	
					_ •	RRIER		- EB 2
		BAF		NO.		TYPE		WEIGHT
		BAR B7E		6	*5	STR	9'-10"	62
NOTES		B8E		3	#5	STR	10'-3"	32
		B9E		2	#5	STR	9'-8"	20
F THE CONCRETE BARRIER RAIL ROACH SLAB SHALL BE INCLUDED		B10	E	6	#5	STR	10'-6"	66
EAR FOOT CONTRACT PRICE BID		B11		3	#5	STR	10'-1"	32
ETE BARRIER RAIL''.		B12	E	2	#5	STR	8'-10"	18
R RAIL ON EACH APPROACH		C 1 F		10	+-	1	E/ 1//	101
NOT BE CAST UNTIL ALL APPROACH	Н	S1E S2E		<u>19</u> 15	#5 #5	1 2	5'-1" 7'-0"	101 110
ETE HAS BEEN CAST AND HAS MINIMUM COMPRESSIVE STRENGTH		52E S3E		4	#5	2	5'-6"	23
SI.		S4E		3	#5	STR	3'-11"	12
RCING STEEL IN CONCRETE BARRIEF	7	S5E		3	#5	STR	2'-4"	7
BE EPOXY COATED.								
				DATED				
					STEEL			483 LBS.
					CRETE			CU. YDS.
					<u>rier f</u> S epo:			LIN.FT.
3,-6, [*]	PRC	_	_	ΝΟ. <u>Α V Ε</u>	• 	R-	<u>1015</u>	JNTY
Y	STA	ATI(ON:	51	6+8	7.3	<u> </u>	_
		Т 3 О						
DocuSigned by: Jeffrey C. Wilson CDA045FAFCC9416		DEPA		MENT S RID	rale TAN	TRANS IGH DARE NPPR	Sportat) OACH	ION
421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772				REVT	SIONS			SHEET NO.
Phone (919) 677-2000 F-0102	NO.	BY:		DATE:		Y:	DATE:	S16-44
is document, logether with the concepts and designs presented herein, as an trument of services, is intended only for the specific purpose and client for ich it was prepared. Reuse of and improper reliance of this document without titen authorization and adaption by Kimley-Horn and Associates, Inc. shall be hout liability to Kimley-Horn and Associates, Inc.	1				3		 	TOTAL SHEETS
pyright Kimley-Horn and Associates, Inc. 2018	2				4			44
	СТГ				\sim	C		RASZ

STRUCTURE 16

STD.NO.BAS4

DocuSign Envelope ID: CEBC8C55-FB60-460E-BE5B-358E658FF18D

—

-

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/2" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 1/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{1}{16}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{16}$ " in thickness and DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

