This electronic collection of documents is provided for the convenience of the user and is Not a Certified Document –

The documents contained herein were originally issued and sealed by the individuals whose names and license numbers appear on each page, on the dates appearing with their signature on that page.

This file or an individual page shall not be considered a certified document.

58 REFERENCE

> 0466 S **PROIE**

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

CONTENTS

SHEET NO.

28-29

ı	IIILE SHEEI
2	LEGEND (SOIL & ROCK)
2A	SUPPLEMENTAL LEGEND (GSI)
3	SITE PLAN
4-5	PROFILE(S)
6-13	CROSS SECTION(S)
14-26	BORE LOG(S), CORE REPORT(S) & CORE PHOTOGRAPH(S)
27	ROCK TEST RESULT(S)

DESCRIPTION

SITE PHOTOGRAPH(S)

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY	DAVIE				
SITE DE	SCRIPTION	BRIDGE	NO. 29 ANL	BRIDGE	<i>NO. 32</i>
ON I-	40 OVER	HUNTING	G CREEK		

STATE PROJECT REPERENCE NO. I - 582330

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR NSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-680. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

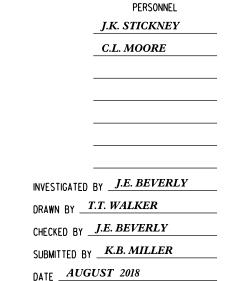
CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU INN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INN-RENET IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS THE ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR THE OPENION OF THE DEPARTMENT AS TO THE TOP OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.





Prepared in the Office of:

FROEHLING & ROBERTSON, INC.

Engineering Stability Since 1881

310 Hubert Street Raleigh, North Carolina 27603-2302 | USA T 919.828.3441 | F 919.828.5751

HINTH CARO



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO.	SHEET NO.
I-5823	2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

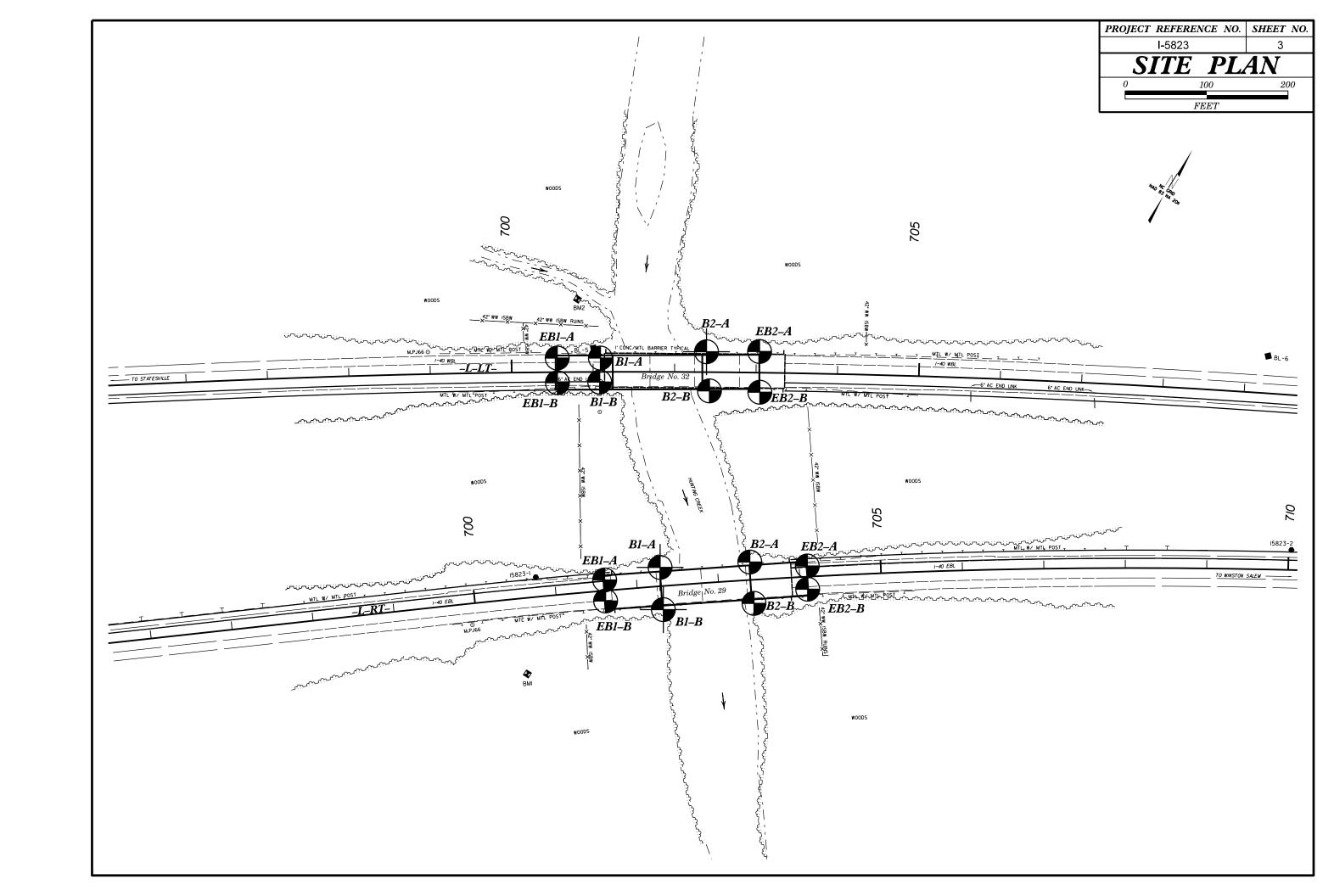
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO I 206, ASTM DI586), SOIL CLASSIFICATION	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF,GRAY.SILTY CLAY.MOIST WITH INTERBEDDED FINE SAND LAYERS,HIGHLY PLASTIC,A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
CENERAL CRANIII AR MATERIAI S SILT-CLAY MATERIAI S	MINERALOGICAL COMPOSITION	FINE TO COARSE CRAIN ICNEOUS AND METAMORPHIC POCK THAT	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) UNGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	CRYSTALLINE ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, CNEISS, GABBRO, SCHIST, ETC.	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-0 A-1-b A-2-4 A-2-5 A-2-6 A-2-7 B-2-7 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
7 PASSING	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR CILY MUCK, CLAY SOILS SOILS PEAT	PERCENTAGE OF MATERIAL	(CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*260 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN 36 MN M SOILS SOILS	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	ROCKS OR CUTS MASSIVE ROCK. <u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40 AB MY 41 MN 48 MY 41 MN 48 MY 41 MN 48 MY 41 MN 48 MY 41 MN 50ILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN 11 MN MODERATE OPERATION	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
USUAL TYPES STONE FRACS. FINE SILTY OF CLAVEY SILTY OF GAYEY MATTER	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
OF MAJOR GRAVEL AND SAND GRAVEL AND SAND SOILS SOILS	▼ STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FIDSTILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN.RATING AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	√PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP	WITH FRESH ROCK.	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.
PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
CONSISTENCY (N-VALUE) (TONS/FT ²)	WITH SOIL DESCRIPTION → OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4 TO 10 GRANULAR LOOSE 4 TO 10	SOIL SYMBOL SOIL SYMBOL SLOPE INDICATOR INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIAL DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT TEST	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50	INFERRED SOIL BOUNDARY	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
VERY SOFT < 2 < 0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.5 CH J CLAY	MW - TEST DODING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u>	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0 MATERIAL STIFF 8 TO 15 1 TO 2	MUNITURING WELL WITH CORE	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD > 30 > 4	TTT ALLUVIAL SOIL BOUNDARY A INSTALLATION SPT N-VALUE	ALSO AN EXAMPLE.	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
BOULDER CORRE GRAVEL COARSE FINE SILT CLAY	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF ACCEPTABLE DEGRADABLE ROCK EMBANKMENT OR BACKFILL	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOOLDER CUBBLE GRAVEL SAND SAND (SL.) (CL.) (BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.)	ABBRE VIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SIZE IN. 12 3 SOIL MOISTURE - CORRELATION OF TERMS	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE SCALE FIELD MOISTURE CHIDE FOR FIELD MOISTURE DESCRIPTION	CPT - CONE PENETRATION TEST NP - NON PLASTIC γ_{d} - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTEMBERG LIMITS) DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH	LENGTH OF ROCK SEGMENTS WITHIN STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
LL LIQUID LIMIT	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
BANGE Z SEMISULID; REGUIRES DRYING TO	FRAGS FRAGMENTS	FRACTURE SPACING BEDDING	BENCH MARK: I-5823-I; N: 785I78.8425, E: I5II723.9405
(PI) PL PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	HI, - HIGHLY V - VERY RATIO EQUIPMENT USED ON SUBJECT PROJECT	<u>TERM</u> <u>SPACING</u> <u>TERM</u> <u>THICKNESS</u> VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	FLEWATION 700 to FEET
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	ELEVATION: 700.42 FEET BENCH MARK: BL-5; N: 785454.745, E: I5II634.2490
SL _ SHRINKAGE LIMIT	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE	
- DRY - (D) ATTAIN OPTIMUM MOISTURE	G* CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	ELEVATION: 700.78 FEET
PLASTICITY	X 8" HOLLOW AUGERS	INDURATION	NOTES:
PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 0-5 VERY LOW	CME-550 HARD FACED FINGER BITS X -N X	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. RUBBING WITH FINGER FREES NUMEROUS GRAINS;	Bridge No. 29 BM= I-5823-I
SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	Bridge No.32 BM= BL-5
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER,	
COLOR	TRICONE TUNC -CAPP	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	X CME-550X X CORE BIT SOUNDING ROD VANE SHEAR TEST	DIFFICULI TO BREAK WITH HAMMER,	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14

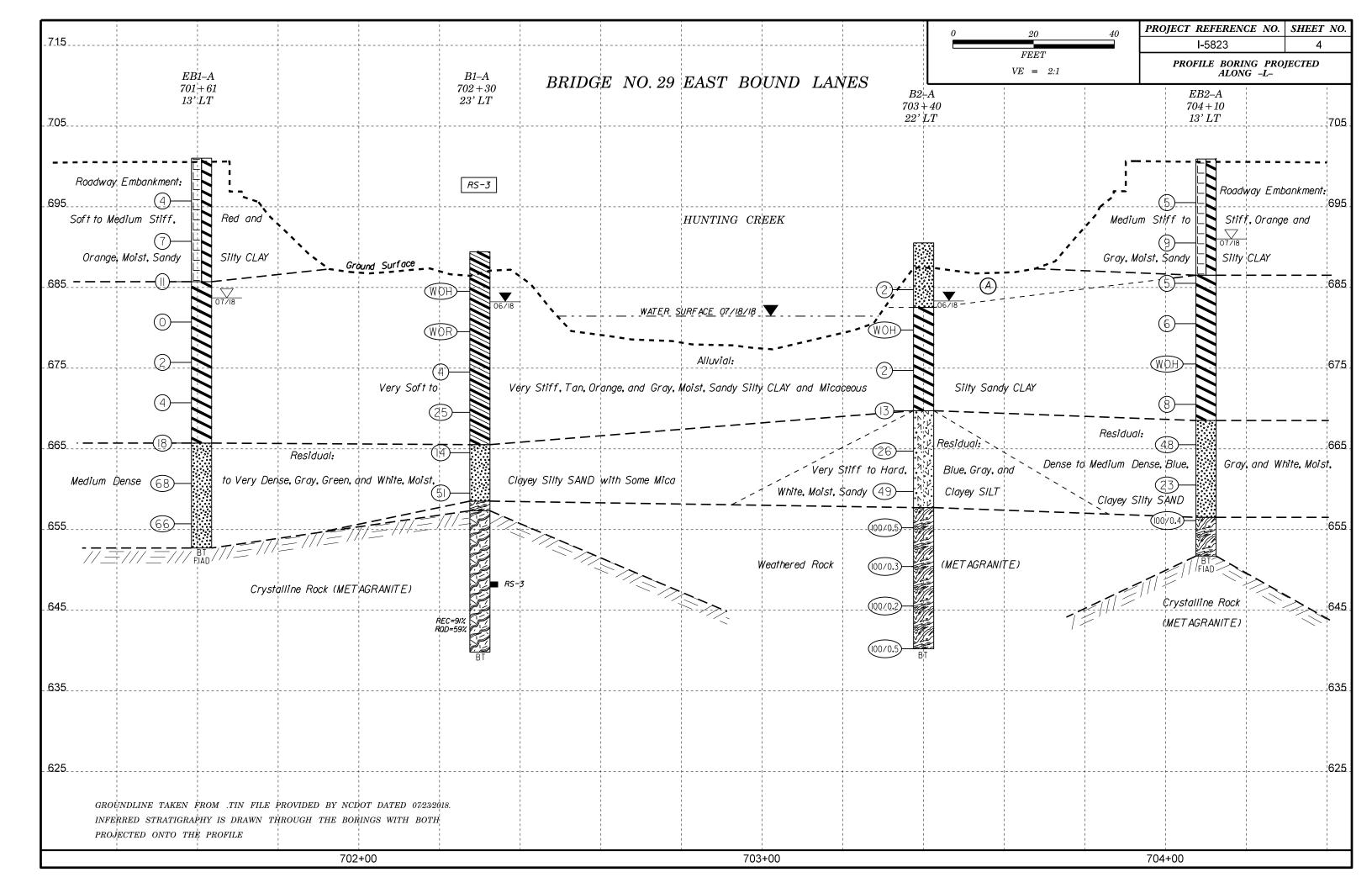
OJECT REFERENCE NO.	SHEET NO.
L-5823	2A

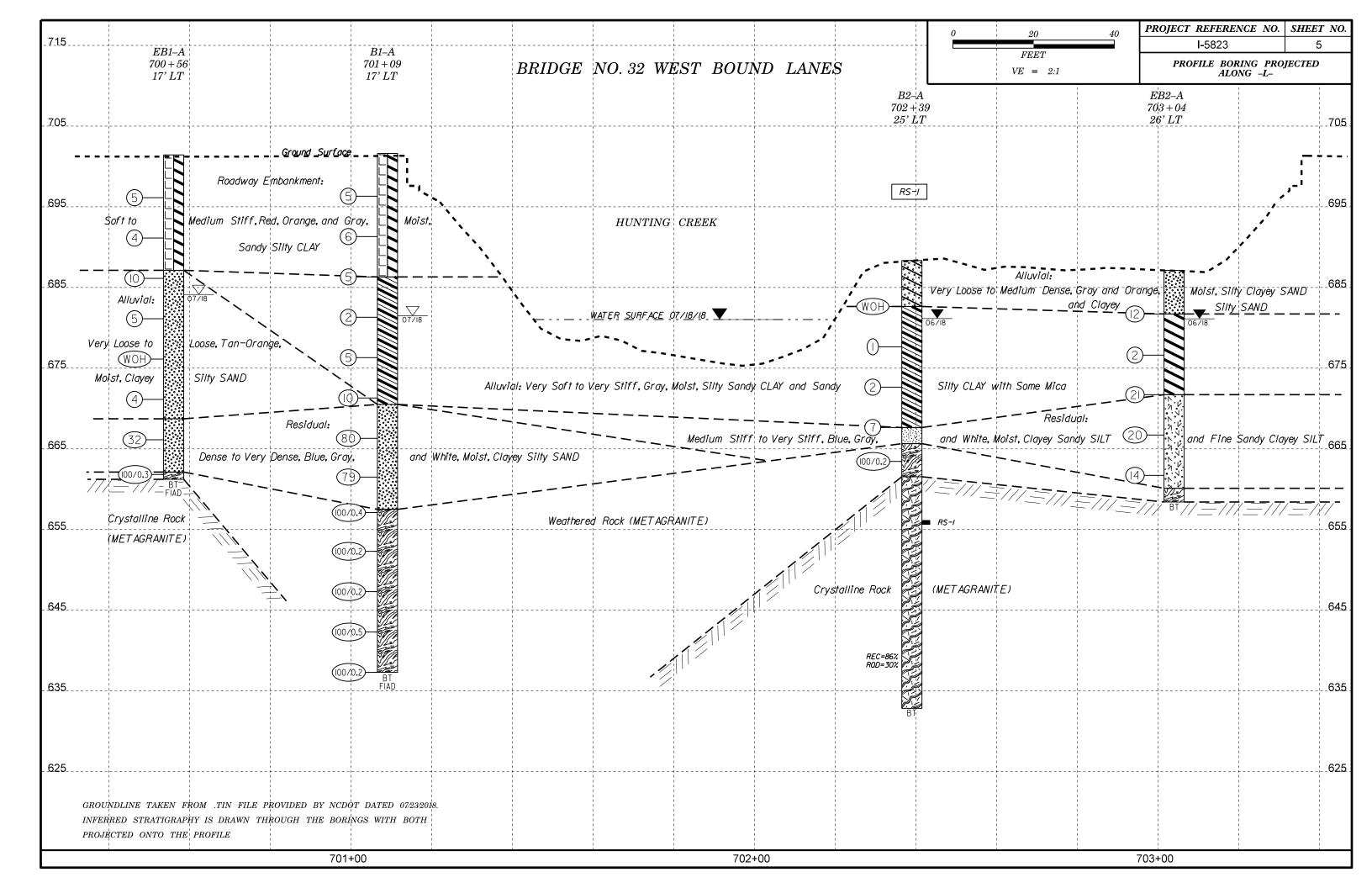
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

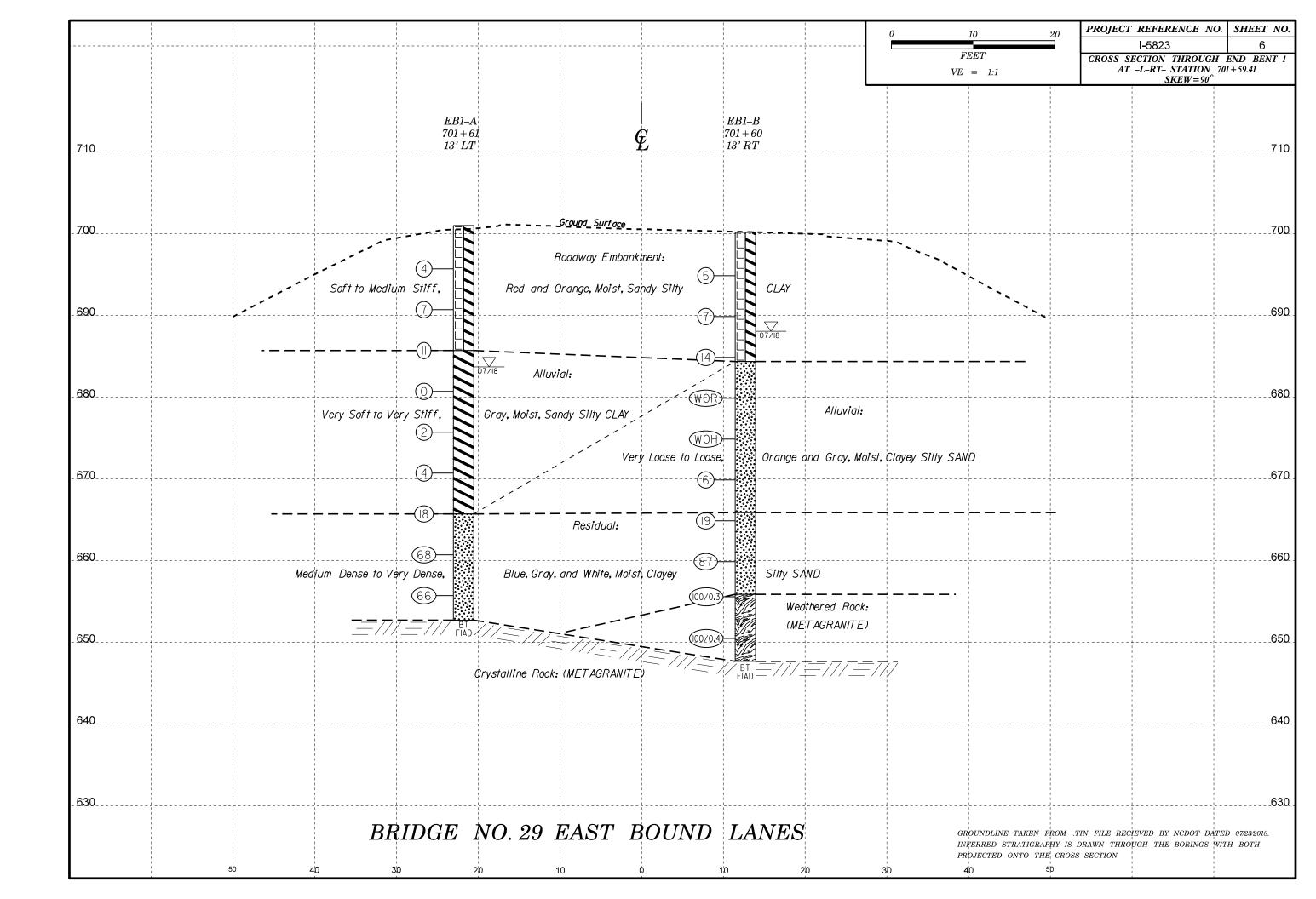
SUBSURFACE INVESTIGATION

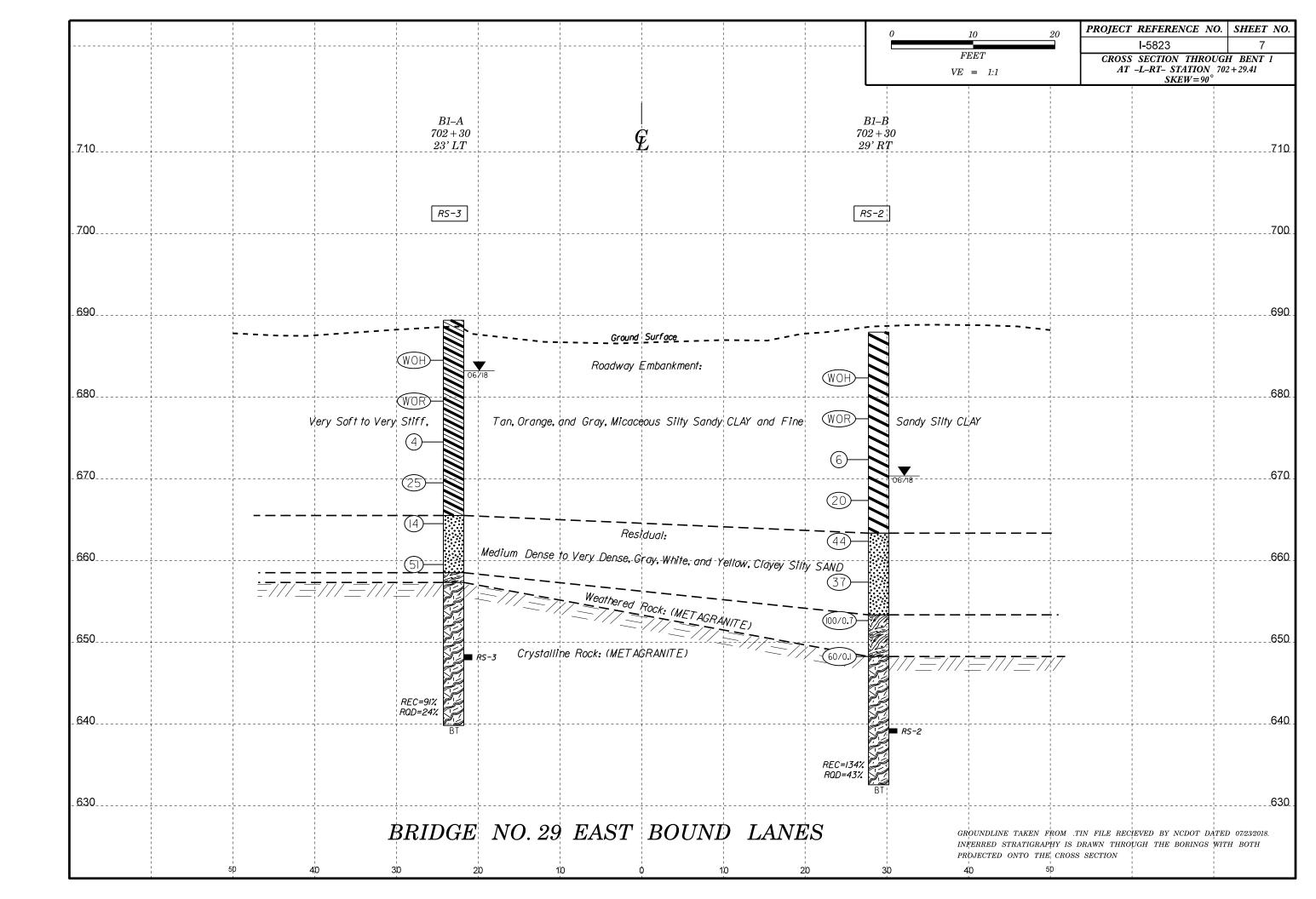
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000) AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000) GEOLOGICAL STRENGTH INDEX (GSI) FOR GSI FOR HETEROGENEOUS ROCK MASSES SUCH JOINTED ROCKS (Hoek and Marinos, 2000) AS FLYSCH (Marinos, P and Hoek E., 2000) From a description of the lithology, structure and ,occasionally es with compact s with angular POOR - Very smooth, slicken-l or highly weathered surfaces soft clay coatings or fillings From the lithology, structure and surface and conditions of the discontinuities, estimate the average value of GSI. Do not try to highly weathered sur coatings or fillings agments surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the planes) be too precise. Quoting a range from 33 to 37 is more realistic than stating that unweathered position in the box that corresponds to the condition ФФ weather Y POOR kensided, highly weathere soft clay coatings or f of the discontinuities and estimate the average value GSI = 35. Note that the table does not of GSI from the contours. Do not attempt to be too apply to structurally controlled failures. Where weak planar structural planes are precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the ITIONS OF ES Rough, slightly s present in an unfavorable orientation smooth, o | surfaces fillings v Hoek-Brown criterion does not apply to structurally with respect to the excavation face, CONDITIONS these will dominate the rock mass controlled failures. Where unfavourably oriented behaviour. The shear strength of surfaces continuous weak planar discontinuities are present, in rocks that are prone to deterioration slightly es these will dominate the behaviour of the rock mass. POOR Slickensided, h with compact o as a result of changes in moisture content will be reduced if water is - Very sersided or from the contents of the contents or from the contents or from the contents or from the contents or from the contents of the contents of the contents or from the contents of the co 1 0 The strength of some rock masses is reduced by the **G00D** G00D thered presence of groundwater and this can be allowed for present. When working with rocks in the by a slight shift to the right in the columns for fair, fair to very poor categories, a shift to the right may be made for wet conditions. th, r ed AIR -GOOD Rough, s surface poor and very poor conditions. Water pressure does VERY I VERY | sided with s FAIR Smoot alter VERY Slick With Water pressure is dealt with by effective VERY not change the value of GSI and it is dealt with by stress analysis. using effective stress analysis. 2 <u>G</u> DECREASING SURFACE QUALITY COMPOSITION AND STRUCTURE STRUCTURE INTACT OR MASSIVE - intact A. Thick bedded, very blocky sandstone .90 rock specimens or massive in N/A N/A The effect of pelitic coatings on the bedding planes is minimized by the confinement of situ rock with few widely spaced PIECES the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally discontinuities 80 controlled instability. 60 BLOCKY - well interlocked undisturbed rock mass consisting ROCK of cubical blocks formed by three intersecting discontinuity sets 50 N. Syltstone F. Weak R. Sand-C. Sandor silty shale siltstone stone with stone and Ы or clayey thin inter siltstone with sand-С shale with layers of ın sımılar stone layers VERY BLOCKY - interlocked. INTERLOCKING mounts sands tone 40 partially disturbed mass with 50 multi-faceted angular blocks formed by 4 or more joint sets C. D. E. and G - may be more or . Tectonically deformed, BLOCKY/DISTURBED/SEAMY -30 less folded than illustrated but ntensively folded/faulted, folded with angular blocks this does not change the strength. sheared clayey shale or siltstone formed by many intersecting Tectonic deformation, faulting and with broken and deformed DECREASING loss of continuity moves these discontinuity sets. Persistence andstone layers forming an 30 categories to F and H. of bedding planes or schistosity almost chaotic structure 20 DISINTEGRATED - poorly inter-locked, heavily broken rock mass 20 H. Tectonically deformed silty with mixture of angular and or clayey shale with or clayey shale forming a 10 rounded rock pieces or without a few very chaotic structure with pockets thin sandstone layers of clay. Thin layers of sandstone are transformed nto small rock pieces. LAMINATED/SHEARED - Lack of blockiness due to close spacing N/A N/A → Means deformation after tectonic disturbance of weak schistosity or shear planes DATE: 8-19-16

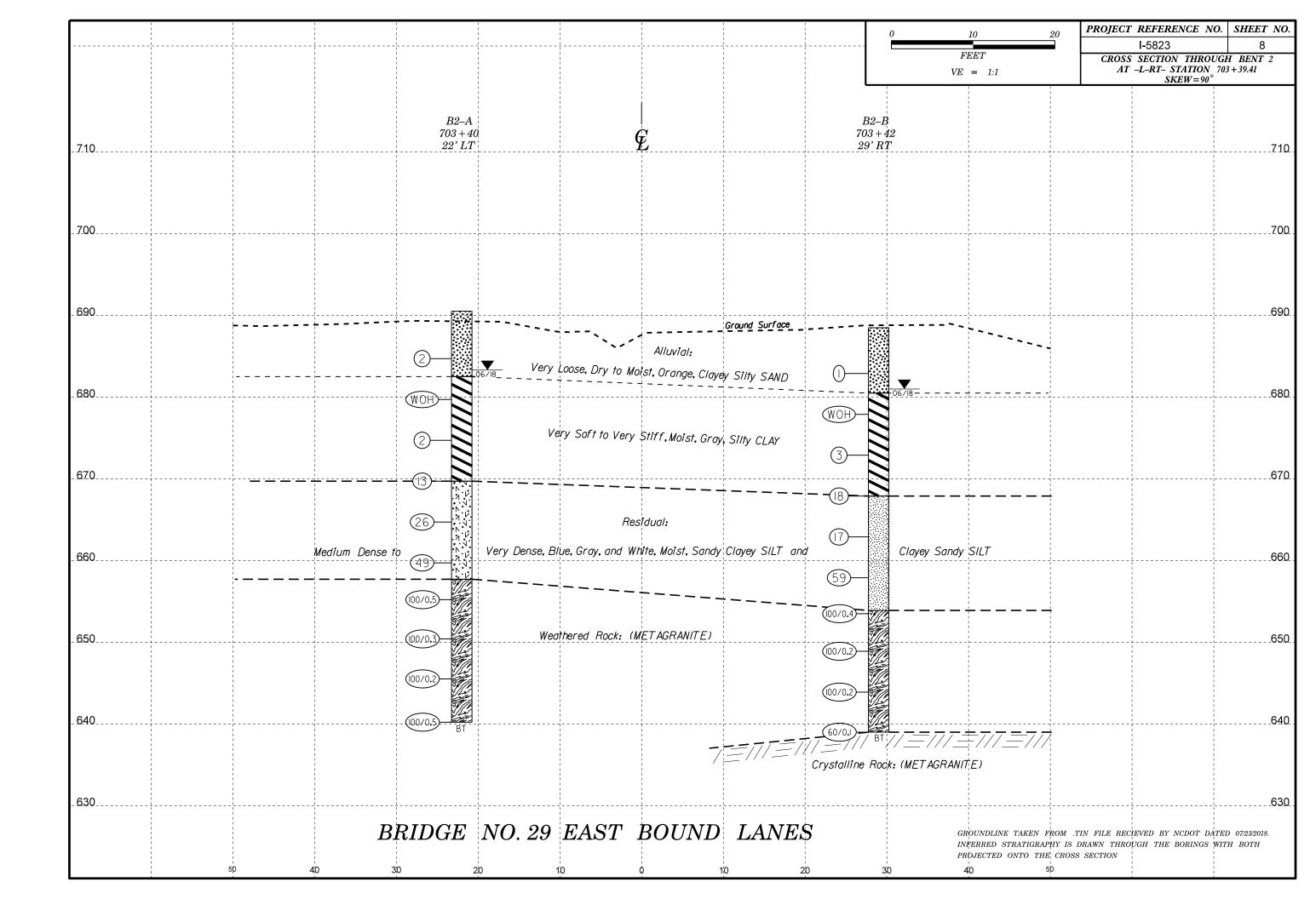


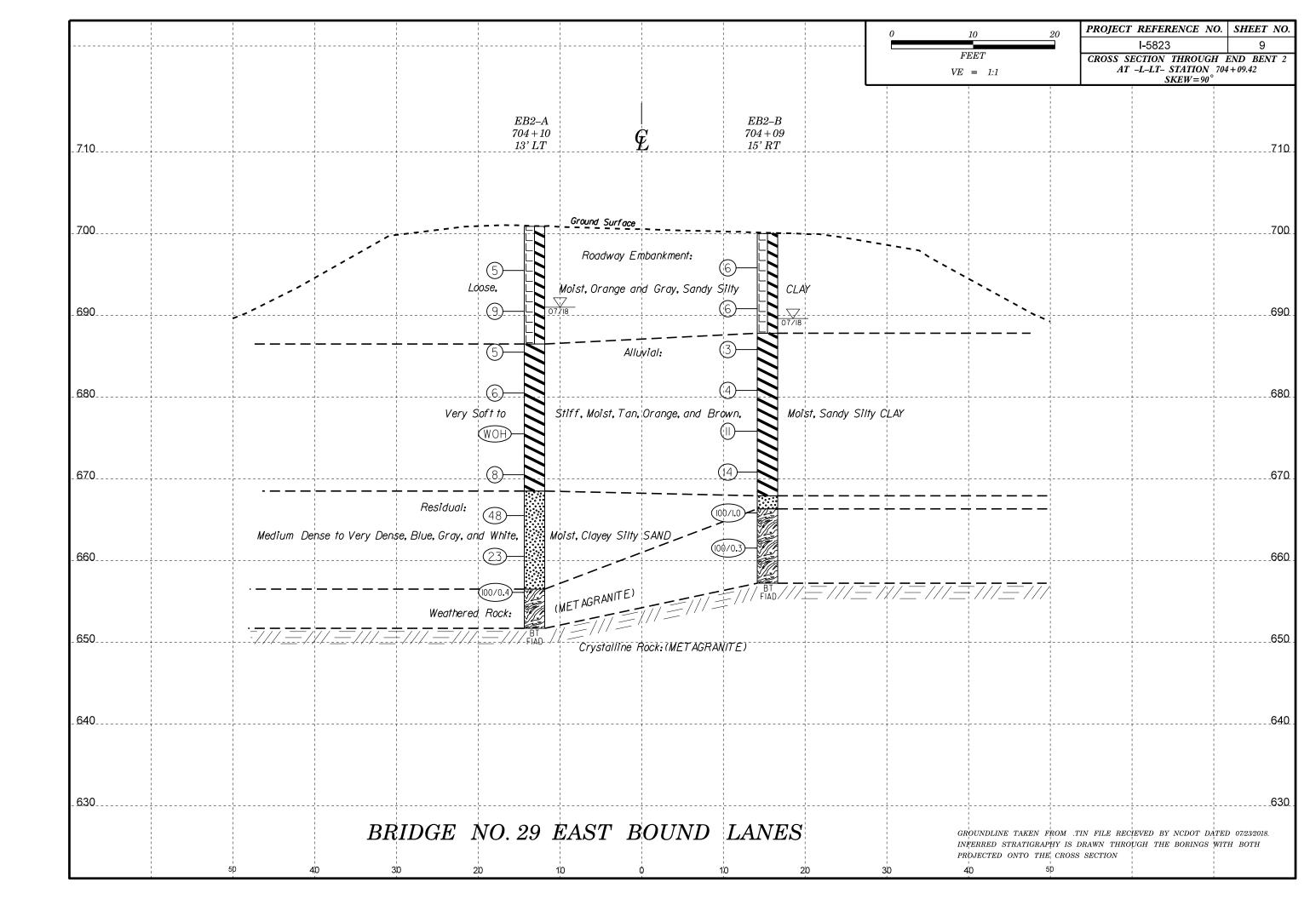


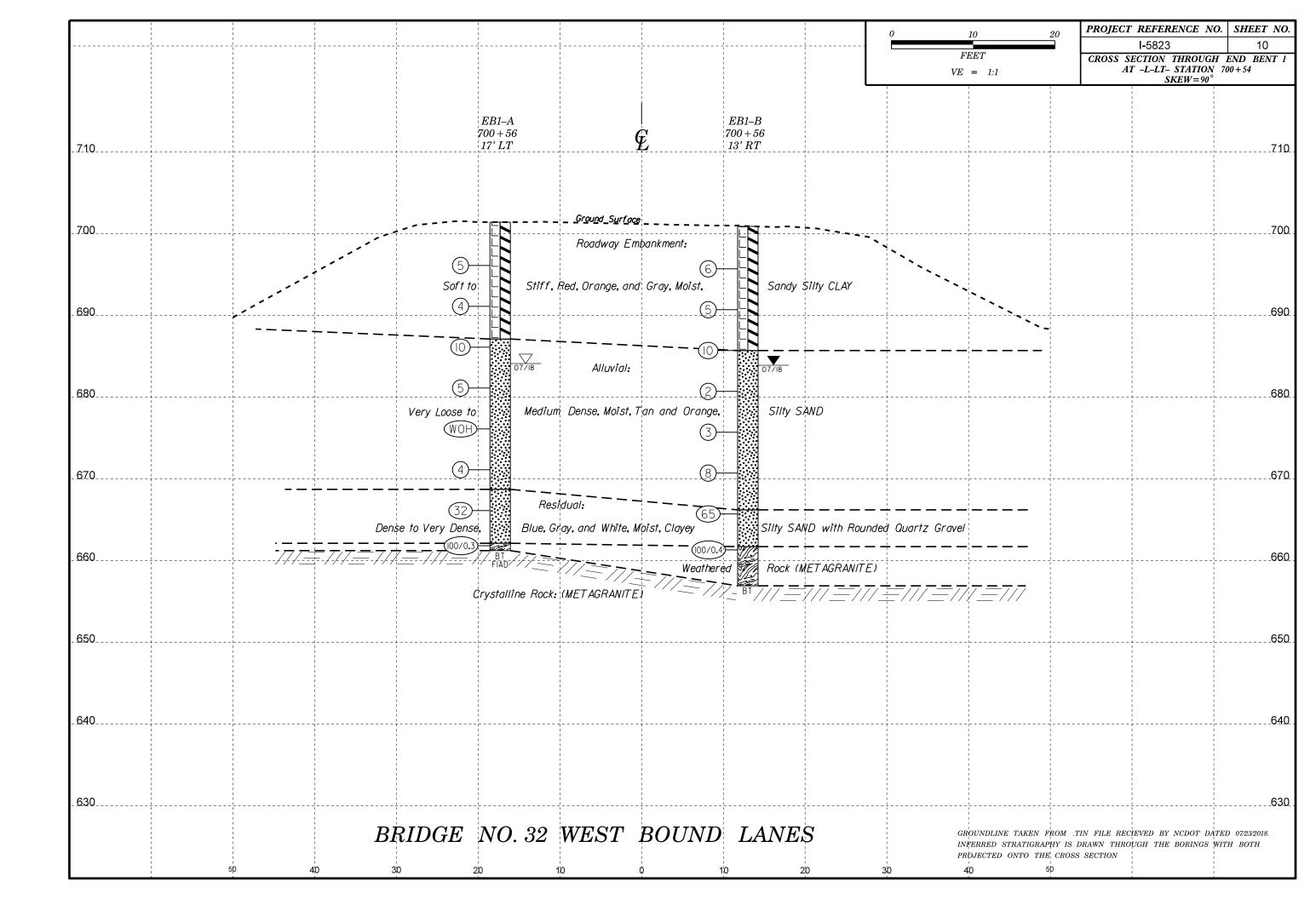


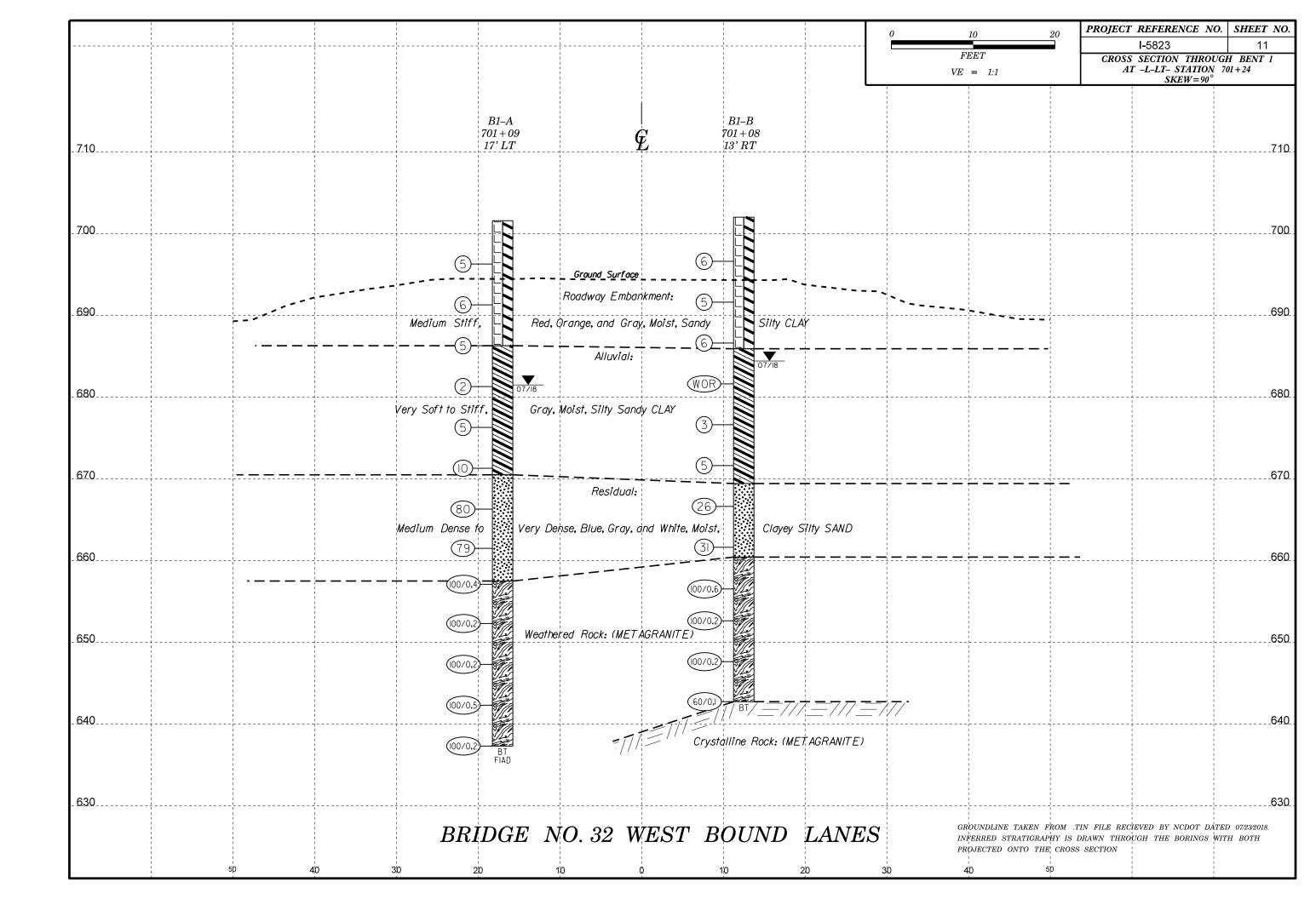


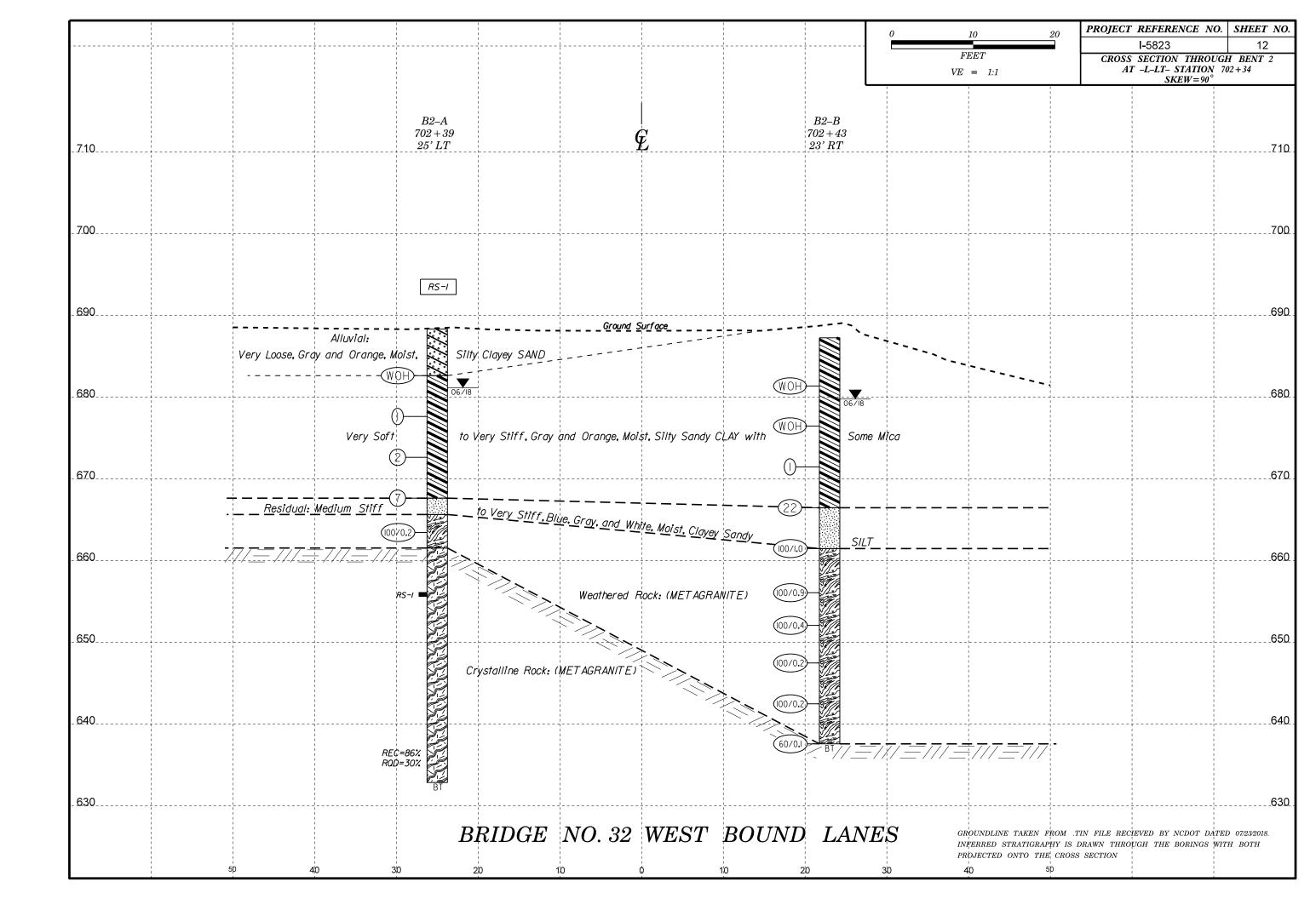


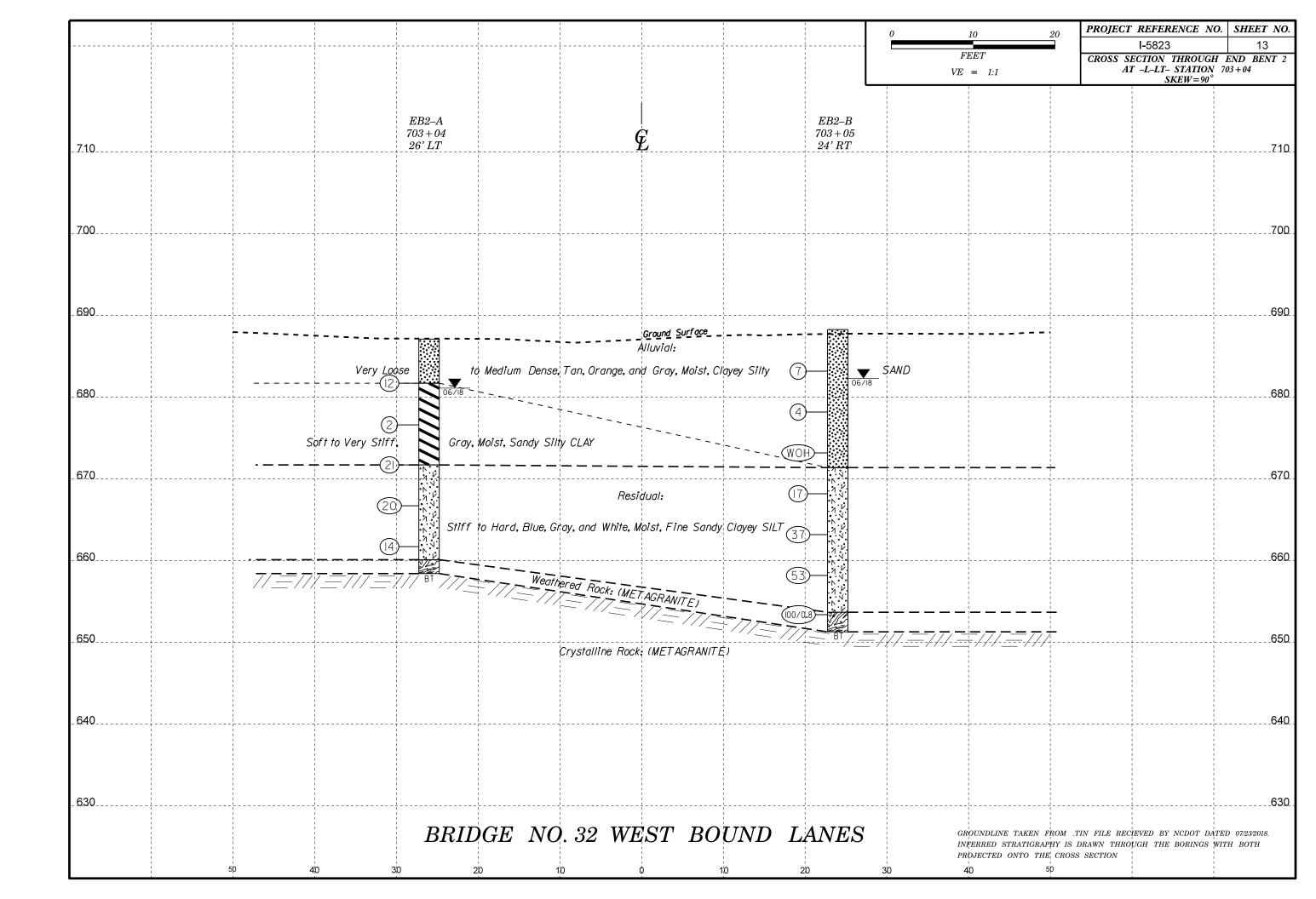


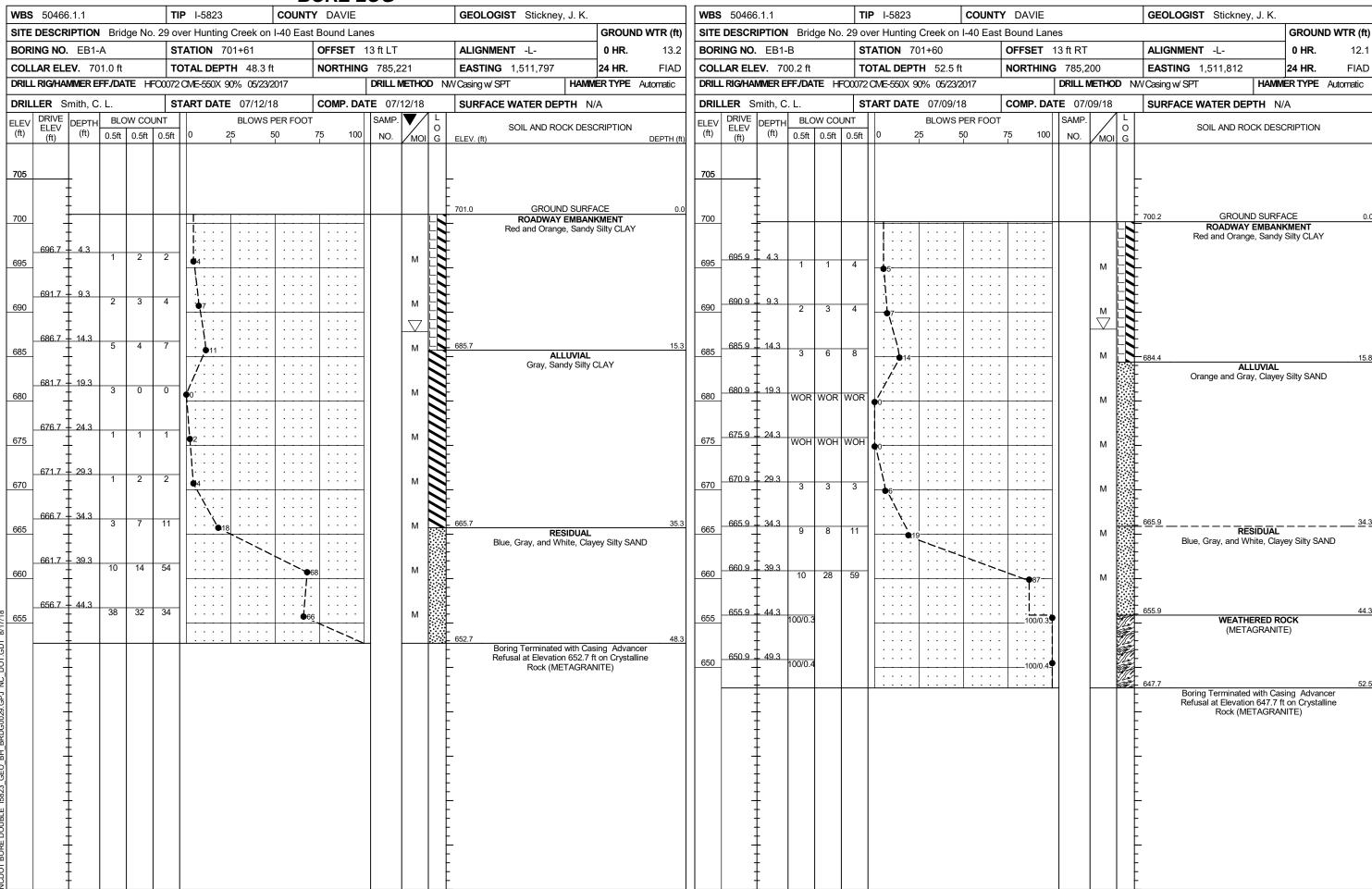












	50400						1.5000			1				<u> </u>			0501 0010T 001 1 1 1/4		
	50466		L De	dara N			I-5823			COUN							GEOLOGIST Stickney, J. K.	CDC! IN:	NATE (C)
				age No						n I-40 E	_						ALIONINATAIT I	GROUND	
	NG NO.				-		TION			<u> </u>	-			23 ft LT			ALIGNMENT -L-	0 HR.	7.3
	AR ELI						AL DEP					NORT	HING	785,2		, O	EASTING 1,511,847	24 HR.	6.2
	.RIG/HA			NE H							_							IER TYPE	Automatic
DRILI	LER S	1	1	0147.5.5		TAF	RT DAT					COMP	DAT	FE 06/	22/18	1 1 L T	SURFACE WATER DEPTH N	/A	
ELEV (ft)	DRIVE ELEV	DEPTH (ft)	0.5ft	OW CO	_	$\left\ \cdot \right\ _{0}$)	25	BLOWS	PER FO 50	OT 7:	5	100	SAMP.		0	SOIL AND ROCK DES	CRIPTION	
()	(ft)	(1-)	0.511	0.511	0.511	╫	,			<u> </u>	i		100	NO.	/MO	l G	ELEV. (ft)		DEPTH (fi
690		<u> </u>				#		.		1	1						- 689.4 GROUND SURF	ACE	0.0
		‡						.			: :		: :				Tan, Orange, and Gray, N Sandy CLA	icaceous Silt	у
685	685.5	3.9	 WOH	WOH	WOH					<u> </u>	· ·	• •	• •		_M		_ Sandy CLA		
		‡					o 				: :				\ <u>₩</u>		- -		
	- 680.5	8.9				П					: :						- -		
680	- 000.5	0.3	WOR	WOR	WOR	T 🛵	0	+		+ : :					М		- -		
		‡				\											- -		
675	675.5	13.9	1	1	3	إل	<u> </u>	:		<u> </u>		• •			М		- -		
	-	‡				'	•4 	:							l IVI		- -		
070	670.5 ·	18.9															- -		
670		10.5	7	12	13	11		2	.5	+ : :					М		_ -		
	-	‡						/			: :						- -		
665	665.5	23.9	3	6	8	┨┞	/.			<u> </u>	· ·	• •	• •		М		- 665.5		23.9
	-	‡					•14								l IVI		Gray, Green, and White, Cl with Some Mi	ayey Silty SAI	ND
000	660.5	28.9						: `									- -		
660		- 20.0	22	27	24	11		+	· · · ·	9 51					М		 - 658.5		30.9
		‡								<u> </u>			1				- 657.3 WEATHERED R - (METAGRANI		32.1
655	_	‡						:		<u> </u>		• •					CRYSTALLINE I	OCK	
	-	‡						:					: :				(METAGRANI	E)	
050		‡						:					: :				-		
650	-	‡						+		+ : :							_ -		
	-	‡						:			: :		: :	RS-3	7		- -		
645	-	‡								<u> </u>		• •					<u>-</u>		
		‡									: :		: :				-		
640		‡						:			· ·		::				-		
640	-	† 		1		╀							—	+			639.8 - Boring Terminated at Elev	tion 639.8 ft	49.6 in
	-	‡															- Crystalline Rock (MET) -	(GRANITE)	
	_	‡															<u>-</u>		
		‡															-		
		‡															- -		
	-	ţ															_		
		‡															- -		
	_	ţ															- -		
		‡															- -		
		‡															<u>-</u> -		
	-	t															_		
	-	ł																	
		Ŧ															-		
	-	Ŧ															-		
		Ŧ															-		
		<u>t</u>															-		

									<u>C</u>	<u>OF</u>	LOG	
WBS	50466	5.1.1			TIP	I-5823	3	C	OUNT	Υ [IE GEOLOGIST Stickney, J. K.	
SITE	DESCR	IPTION	I Brid	lge No. 2				on I-4	0 Eas	t Boı	Lanes GROUND WTF	₹ (ft)
BOR	ING NO	. B1-A	١		STA	TION	702+30			OF	T 23 ft LT ALIGNMENT -L- 0 HR.	7.3
	LAR ELI						PTH 49			NO	HING 785,270 EASTING 1,511,847 24 HR.	6.2
DRILI	L RIG/HA	MMER E	FF./DA	TE HFO	0072 CIV	/E-550X	90% 05/	/23/2017	7		DRILL METHOD NW Casing W/SPT & Core HAMMER TYPE Automa	atic
DRIL	LER S	mith, C	. L.		STAI	RT DA	TE 06/2	2/18		СО	DATE 06/22/18 SURFACE WATER DEPTH N/A	
COR	E SIZE	NX		T			N 17.5 f					
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	JN RQD (ft) %	SAMP. NO.	REC. (ft)	RQD (ft) %	L O G	DESCRIPTION AND REMARKS LEV. (ft) DEP	PTH (ft)
657.33	657.3	32.1	0.5	4 00/0 5	(4.0)	(0.0)		(40.0)	(40.0)	·	Begin Coring @ 32.1 ft	
655	654.8	34.6	5.0	1:36/0.5 1:39/1.0 1:36/1.0 1:43/1.0	(1.9) 76% (4.2)	(0.6) 24% (2.6)		(16.0) 91%	(10.3) 59%		77.3 CRYSTALLINE ROCK Light Gray, Slightly Weathered, Hard, METAGRANITE with Moderately Close Fracture Spacing GSI=40-55	32.1
650	649.8	39.6		1:48/1.0 1:36/1.0 1:44/1.0 1:40/1.0	84%	52%					GGI-40-00	
			5.0	1:50/1.0 1:41/1.0 1:43/1.0	(4.9) 98%	(3.0) 60%	RS-3					
645	644.8	44.6	5.0	1:44/1.0 1:47/1.0 2:10/1.0 2:19/1.0	(5.0) 100%	(4.1) 82%						
640	639.8	49.6		2:15/1.0 2:30/1.0 2:11/1.0	10070	0270					9.8	49.6
	:	<u> </u>									Boring Terminated at Elevation 639.8 ft in Crystalline Rock (METAGRANITE)	
	-	+ - - - - - - - - - - - -										

CORE PHOTOGRAPHS: Bridge No. 29 over Hunting Creek on I-40 East Bound Lanes, B1-A 702+30, 23' LT

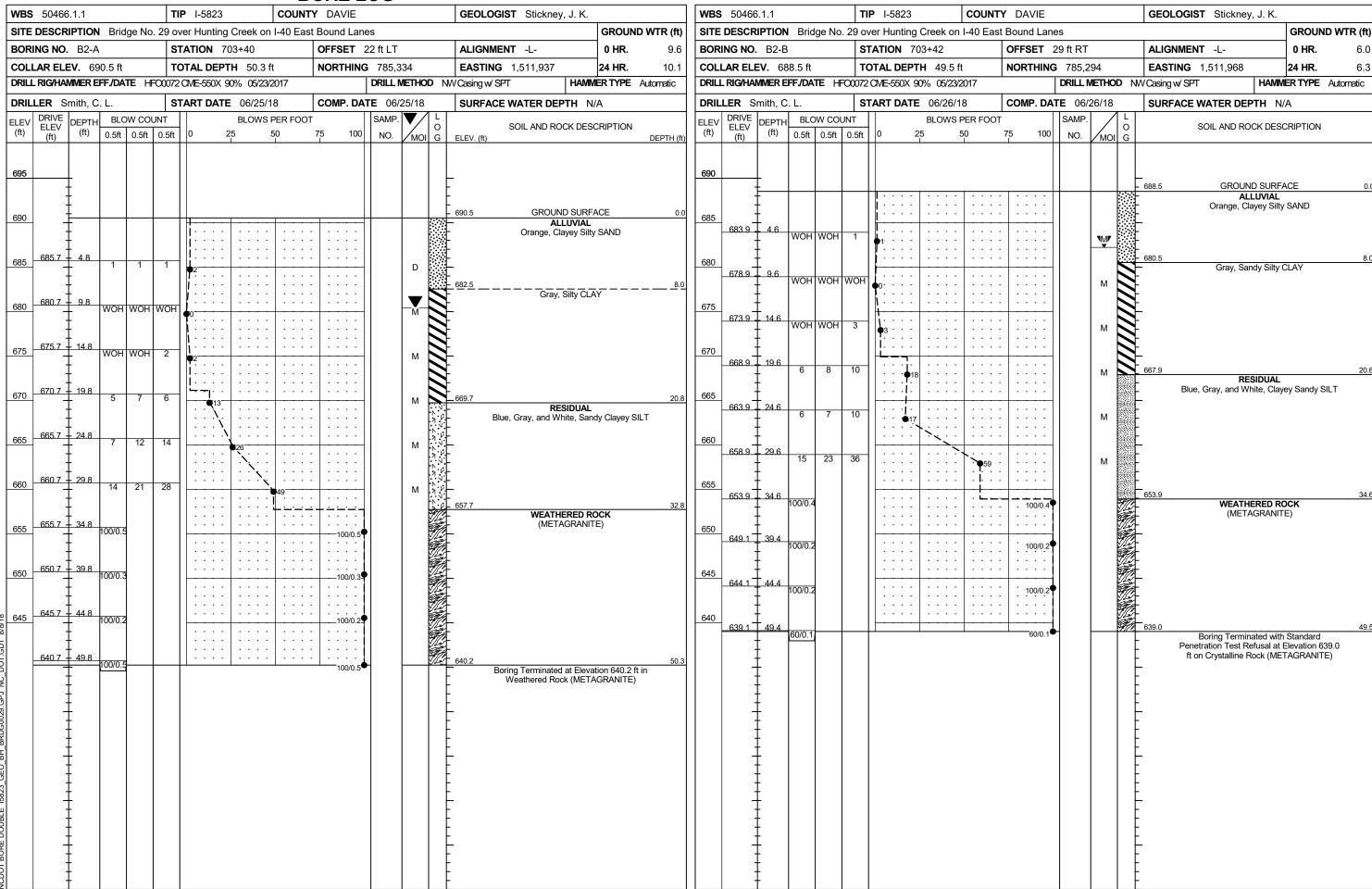


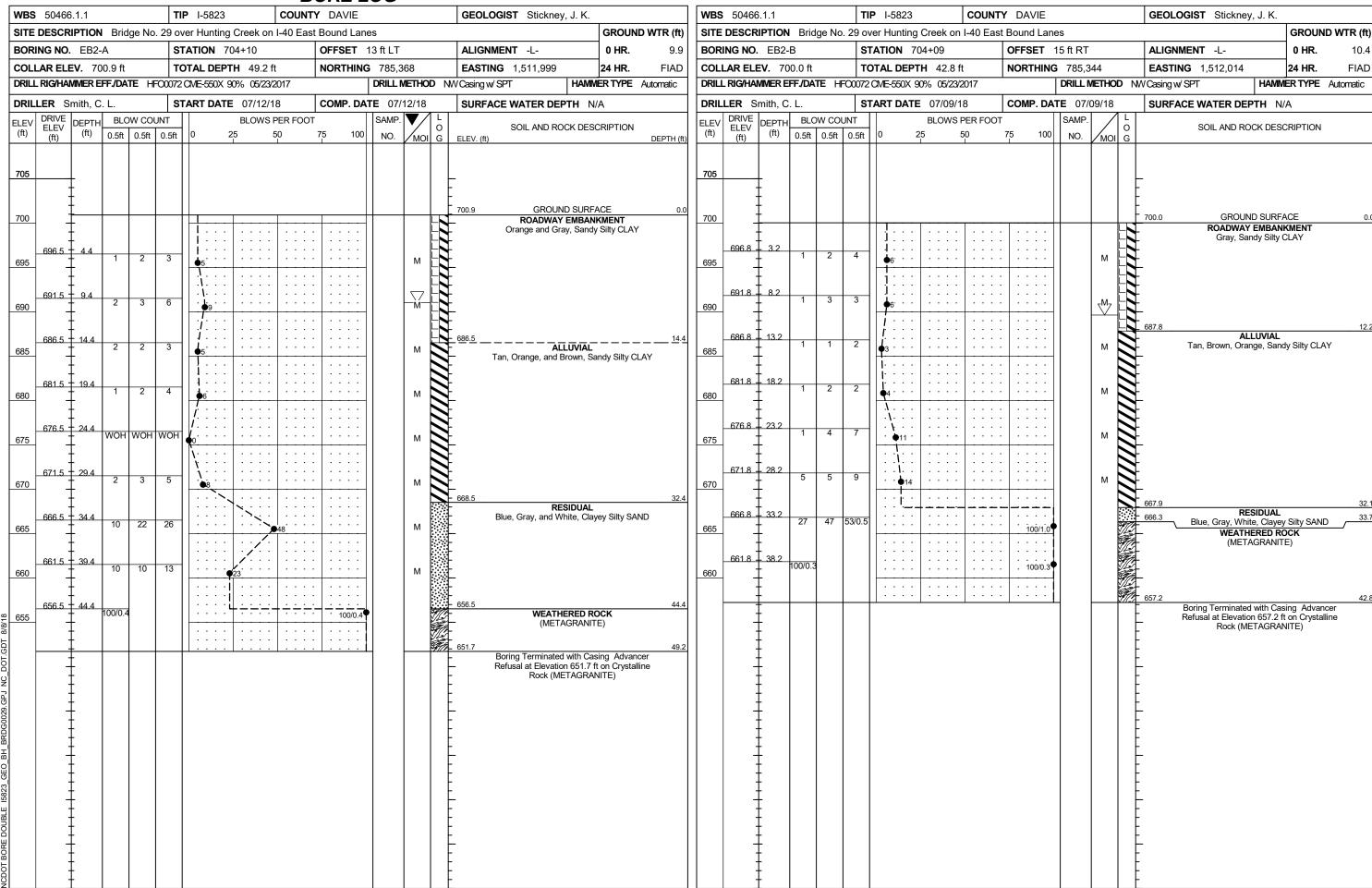
NBS						D 1 5000								050100107 0011 114	
						IP I-5823			DAVIE					GEOLOGIST Stickney, J. K.	
				dge No			Creek on I-40	East						1	GROUND WTR (ft)
		B1-B				TATION 70			OFFSET					ALIGNMENT -L-	0 HR. 7.7
		EV . 68				OTAL DEPT			NORTHIN					' '	24 HR. 7.2
				VIE H			0% 05/23/2017						א ט		ER TYPE Automatic
		mith, C				TART DATE			COMP. D	- 1		21/18	1	SURFACE WATER DEPTH N/	A
LEV L		DEPTH (ft)		OW CO		0 2	BLOWS PER F 5 50		75 10	- 1 1	SAMP.	▼/	O	SOIL AND ROCK DESC	
(11)	(ft)	(1.5)	0.5ft	0.5ft	0.5ft		J J		75 10	+	NO.	/MOI	G	ELEV. (ft)	DEPTH (1
690	_	ł												_	
	-	<u> </u>				H			Τ	+				688.0 GROUND SURFA ALLUVIAL	
685	_	Ł												Tan, Orange, and Gray, Mic Sandy Silty CLA	caceous Fine Y
	683.4	4.6	WOH	WOH	WOH	::::									
	-	ł	VVOIT	VVOIT	VVOIT	∮ 0						M			
680	-					 			+	$\exists 1$		·		_	
	678.4	9.6	WOR	WOR	WOR	0	: : : : :					М			
675	_	Ł				<u> </u>								_	
	673.4	14.6	1	1	_	\			: : : :						
	-	†	1	1	5	 						М	Y		
670	-	100							 	+				-	
	668.4	19.6	2	9	11	2						М			
665	_	<u> </u>												_	
	663.4	24.6	10	00	00									663.4	24.
	-	-	12	22	22		44					М	I I	RESIDUAL Gray, White, and Yellow, Clay	yey Silty SAND
660		<u> </u>							+ : : : :	+				_	
	658.4	29.6	14	17	20		37		: : : :			М		•	
655	-	-												•	
	653.4	34.6	10	54/0.0			[·	1]				653.4	34.
	-	F	46	54/0.2					100/0.	1				WEATHERED RO (METAGRANITI	
650	_	F							+	$\ \ $				· =	
	648.4	39.6	60/0.1						60/0.	1				CRYSTALLINE RO (METAGRANITI	39. DCK
645	-	Ŧ							: : : :					(METAGRANITE	≣)
	-	Ŧ								71				- ·	
	-	Ŧ													
640	_	F					· · · · ·		+	$\{ ackslash all_{ar{ar{ar}}}$	DC C			· -	
	-	Ŧ								1	RS-2			· 637.6	50.
635	-	E							<u> </u>					(METAGRANITI	-)
	-	<u> </u>								71				 · 632.6	55.
		т		1	i .				1					UDZ.U	ממ

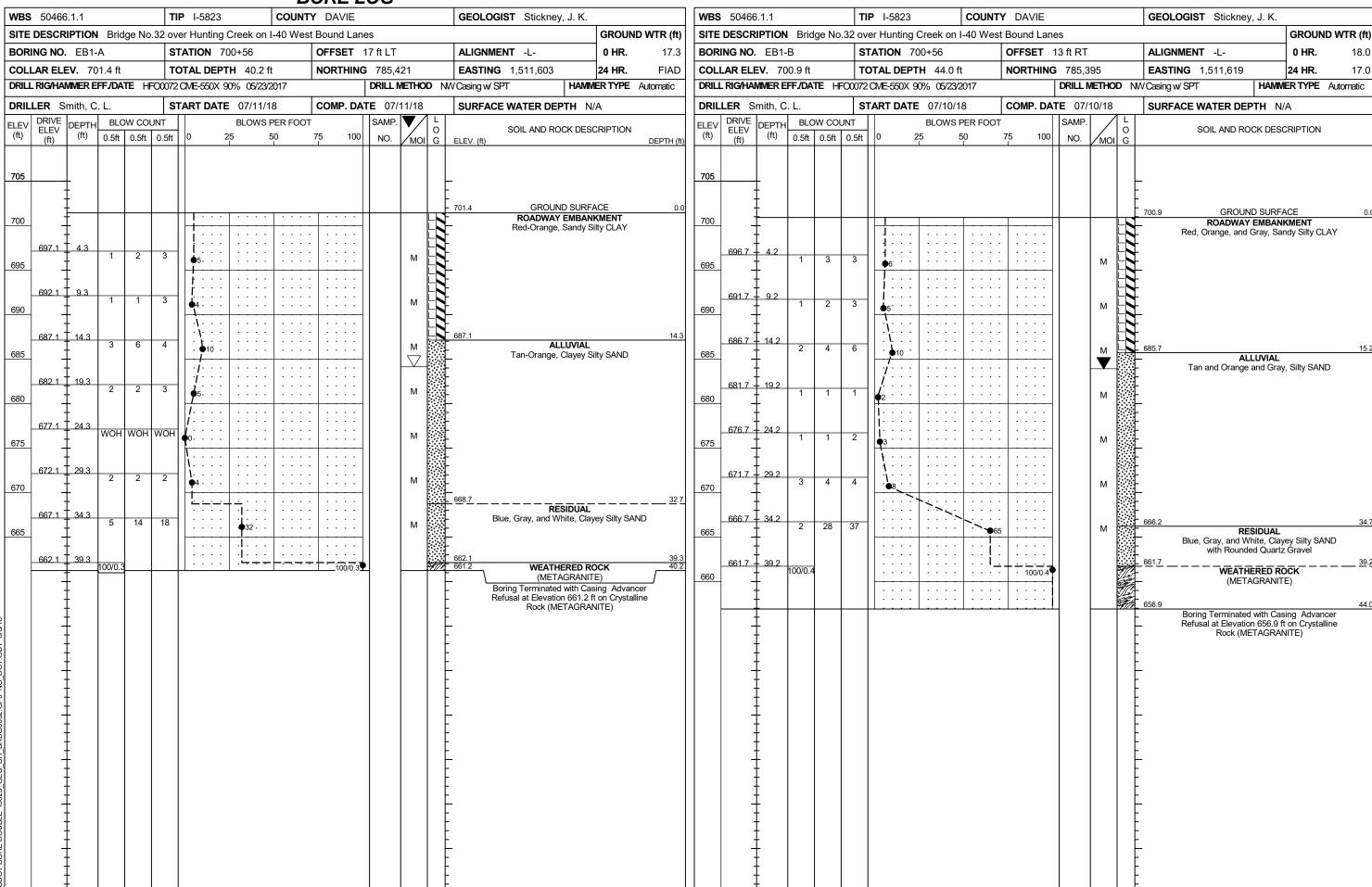
									<u></u>	UI	RE L	JG				
WBS	50466	3.1.1			TIP	I-582	3	C	OUNT	Υ	DAVIE			GEOLOGIST Stickney, J.		
				lge No. 2				on I-4	0 Eas	_	und Lane					IND WTR (ft)
	ING NO				1		702+30			1	FSET 29			ALIGNMENT -L-	0 HR.	
l	LAR ELI						PTH 55			NC		785,228		EASTING 1,511,878	24 HR.	
-				TE HFO			(90% 05/		7			DRILL METHOD				E Automatic
	LER S		. L.				TE 06/2			CC	MP. DAT	E 06/21/18		SURFACE WATER DEPTH	N/A	
COR	E SIZE	NX		DDILL		AL RU	N 15.7 f		RATA	 	1					
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	RQD (ft) %	SAMP. NO.	REC. (ft)	RQD (ft) %	L O G	ELEV. (ft)		DE	ESCRIPTION AND REMARKS		DEPTH (ft)
648.27 645	648.3	39.7	5.7	0:56/1.0 0:59/1.0	(5.2) 91%	(0.6) 11%		(14.3) 134%	(4.6) 43%		648.3	Light Gray to Lig		Begin Coring @ 39.7 ft CRYSTALLINE ROCK own, Moderately Weathered, Har with Close Fracture Spacing	d, METAGRA	39.7 NITE
043	642.6	45.4	5.0	0:47/1.0 0:57/1.7 1:00/1.0	(4.1)	(0.9)					- - -			GSI=20-25		
640	<u>.</u>	<u> </u>	5.0	1:04/1.0 1:02/1.0 1:00/1.0	(4.1) 82%	18%	RS-2				-					
635	637.6	50.4	5.0	1:05/1.0 1:10/1.0 0:58/1.0 0:50/1.0	(5.0) 100%	(3.1) 62%					- 637.6 - -	Light Gray, Mod with	deratye	e to Very Slightly Weathered, Har e to Moderately Close Fracture S GSI=40-55	d, METAGRA pacing	50.4 NITE
	632.6	55.4		1:30/1.0 1:22/1.0							632.6			ated at Elevation 632.6 ft in Crys		55.4

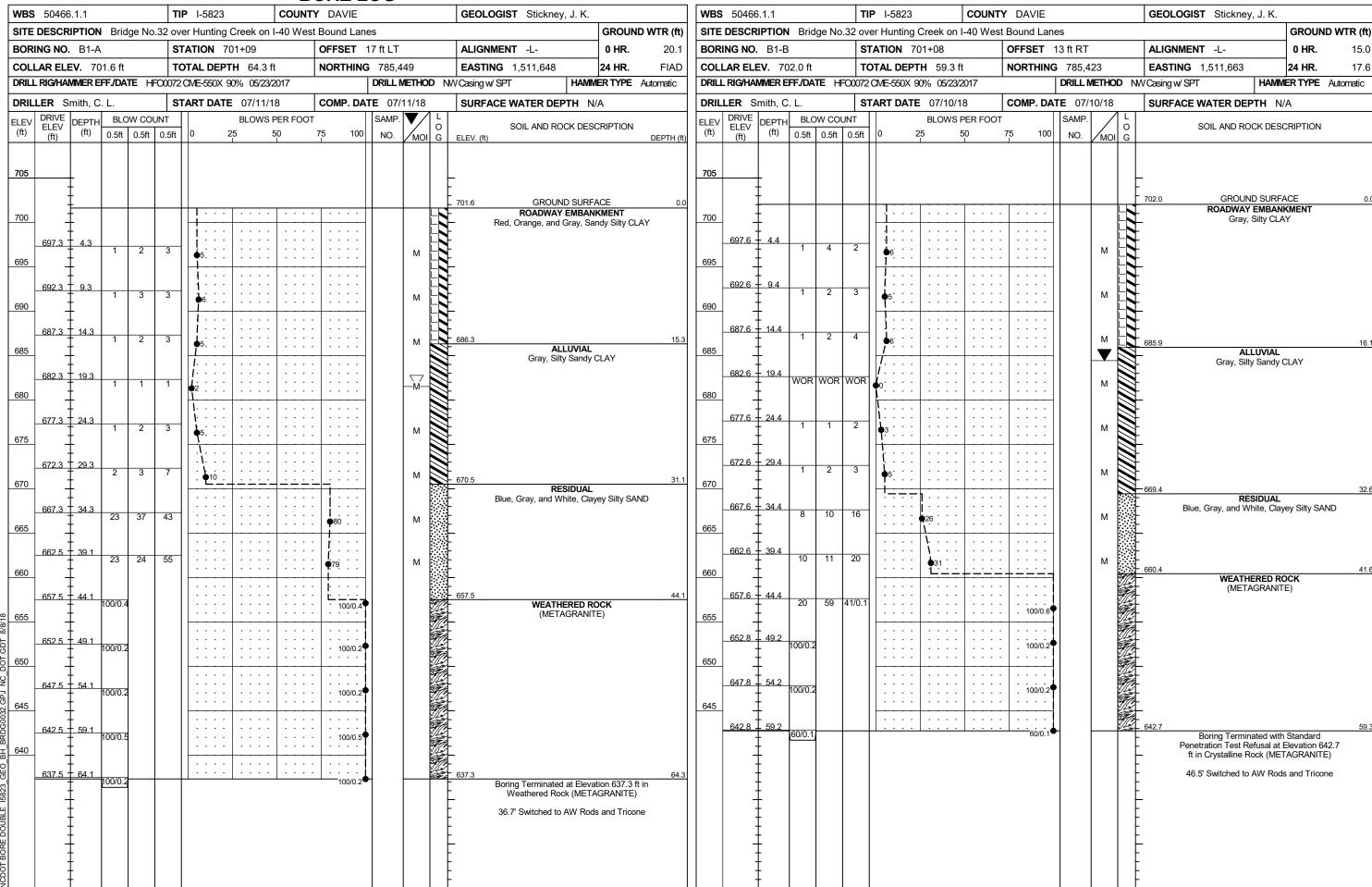
CORE PHOTOGRAPHS: Bridge No. 29 over Hunting Creek on I-40 East Bound Lanes, B1-B 702+30, 29' RT











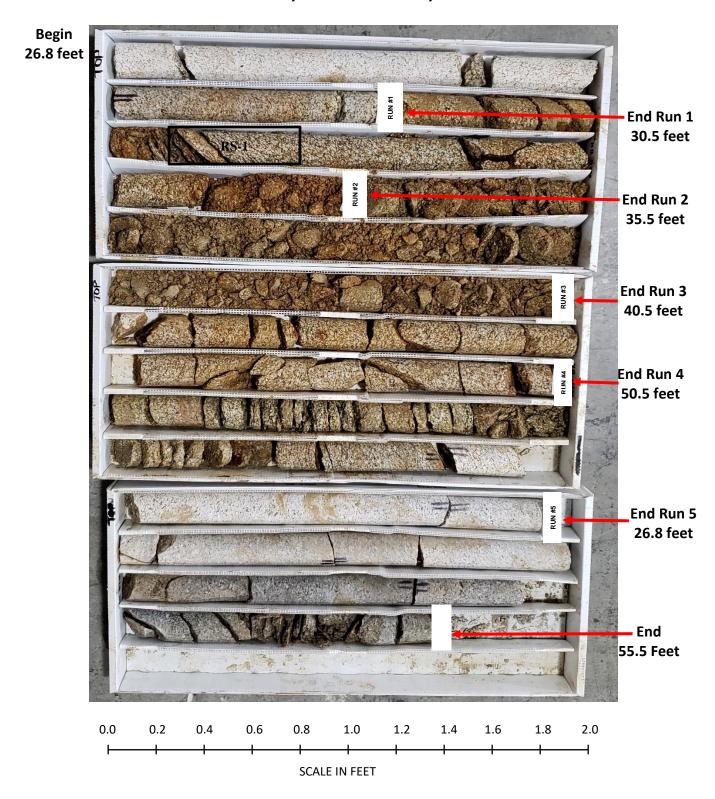
SHEET 23

GEOTECHNICAL BORING REPORT BORE LOG

							В	DRE L	<u>UG</u>					
	50466					IP I-5823	COUNTY					GEOLOGIST Stickney, J. K.	1	
SITE	DESCR	IPTION	I Brid	dge No		ver Hunting Creek on I	-40 West I	Bound Lane	s				GROUN	ID WTR (ft)
	NG NO.					TATION 702+39		OFFSET 2				ALIGNMENT -L-	0 HR.	6.4
	AR ELE				- 1	OTAL DEPTH 55.5 ft	I	NORTHING				EASTING 1,511,754	24 HR.	7.2
PRILL	. RIG/HAI	VIMER E	FF./DA	TE H	FO0072	2 CME-550X 90% 05/23/2	2017		DRILL N	METHO	D N	W Casing W/SPT & Core HAMM	ER TYPE	Automatic
DRIL	LER S	mith, C	. L.		S	TART DATE 06/26/1	8	COMP. DAT	E 06/2	26/18		SURFACE WATER DEPTH N/	A	
LEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft	0.5ft		┫╽	PER FOOT 50 7	75 100	SAMP. NO.	MOI	L O G	SOIL AND ROCK DESC ELEV. (ft)	CRIPTION	DEPTH (f
90	-	-					1					 - 688.4 GROUND SURFA ALLUVIAL	ACE	0
85	- - - 683.7 -	- - - 4.7									///////	Gray and Orange, Silty Cl	ayey SANI	0
80	-	- - -	WOH	WOH	WOH	0				M		- 682.7 - Gray, Silty Sandy CLAY wit -	h Some Mi	ica
75	678.7 - - - -	- 9.7 - -	WOH	WOH	1	1 · · · · · · · · · · · · · · · · · · ·				M		- - - -		
	673.7 - -	- - 14.7 -	WOH	1	1	42				М		- - - -		
70	- 668.7 - -	- 19.7	1	3	4	• • • • • • • • • • • • • • • • • • •				M		- - - - 667.7 - RESIDUAL		20
65	663.7	- - 24.7	100/0.2	2							<i>77.</i>	Blue, Gray, and White, Clay WEATHERED RO (METAGRANIT	CK	SILT
60	- - -	- - -										- 661.6 - CRYSTALLINE R - (METAGRANIT		26
55	-	-							RS-1			- - - -		
50	-											- - - -		
	- - - -	-										- - - -		
45	- - -	- - -										_ - - -		
40	-											- - - -		
35	- - -	- - -												55
		- - - - - - - - - - - - - - - - - - -										Boring Terminated at Elevat Crystalline Rock (META)	ion 632.9 i	it in

										RE LUG		
	50466.1.1			1	I-582					AVIE	GEOLOGIST Stickney, J. K	1
	DESCRIPTIO		dge No.32	_) Wes	_			GROUND WTR (ft)
-	ING NO. B2-			_		702+39			+	FSET 25 ft LT	ALIGNMENT -L-	0 HR. 6.4
	LAR ELEV. 6					PTH 55			NO	RTHING 785,525	EASTING 1,511,754	24 HR. 7.2
DRILL	RIG/HAMMER	EFF./DA	TE HFO	0072 CN	/IE-550>	(90% 0	5/23/2017	7		DRILL METHOD	NW Casing W/SPT & Core HAN	MIMER TYPE Automatic
DRIL	LER Smith, (C. L.		STA	RT DA	TE 06/	26/18		CO	MP. DATE 06/26/18	SURFACE WATER DEPTH	N/A
COR	E SIZE NX					N 28.7						
ELEV	RUN DEPTI	H RUN	DRILL RATE	REC.	UN RQD	SAMP.	STF REC.	RATA	L		DESCRIPTION AND REMARKS	
(ft)	(ft) (ft)	(ft)	(Min/ft)	(ft) %	(ft) %	NO.	(ft) %	(ft) %	Ğ	ELEV. (ft)	DESCRIFTION AND REMARKS	DEPTH (ft)
661.55											Begin Coring @ 26.8 ft	
660	661.6 - 26.8	3.7	1:20/1.0 1:10/1.0	(3.0) 81%	(2.3) 62%		(24.7) 86%	(8.5) 30%		- 661.6 — Light Grav, Slightly	CRYSTALLINE ROCK Weathered, Hard, METAGRANITE, CI	26.8 ose to Moderately
	657.9 1 30.5		1:09/1.0								Close Fracture Spacing	,
	İ	5.0	0:45/1.0 0:53/1.0	(3.6) 72%	(0.9) 18%					- -		
655	+		0:51/1.0 1:01/1.0			RS-1	1			_		
	652.9 35.5	5.0	0:55/1.0 0:40/1.0	(4.4)	(0.0)					- -		
650	‡	3.0	0:36/1.0 0:34/1.0	88%	0%					- -		
- 000	647 9 7 40 5		0:42/1.0							- -		
	647.9 40.5	5.0	0:38/1.0 0:38/1.0	(3.7)	(0.0)					- -		
645			0:44/1.0	74%	0%					- -		
	642.9 1 45.5		0:57/1.0 0:59/1.0							- -		
	+	5.0	0:51/1.0	(5.0) 100%	(2.5) 50%					-		
640	-		0:47/1.0 0:53/1.0 0:55/1.0	10070	30 70					-		
	637.9 50.5		0:50/1.0	(5.5)	(0.0)					- -		
625	‡	5.0	0:53/1.0 0:51/1.0 0:46/1.0	(5.5) 110%	(2.8) 56%					- -		
635	 ‡		0:55/1.0	1						- -		
	632.9 55.5		0:57/1.0							_ 632.9 _ Boring Te	rminated at Elevation 632.9 ft in Crystal	line Rock
	1									- -	(METAGRANITE)	
	Ŧ									-		
	Ŧ									- -		
	‡									- -		
	‡									- -		
	‡									- -		
	†									- -		
	<u> </u>									.		
	1									<u>-</u>		
	+									-		
	Ŧ									-		
	‡									- -		
	‡									- -		
	‡									- -		
	+									_ -		
	+									-		
	Ŧ									•		
	7									- -		
	‡									- -		
	#									- 		
	‡									<u>-</u> -		
	1									- -		
										- -		
	‡									- -		
	‡									<u>-</u>		
	†									<u>-</u> -		
	1									<u>-</u>		
			1	1	1	l	1		1	-		

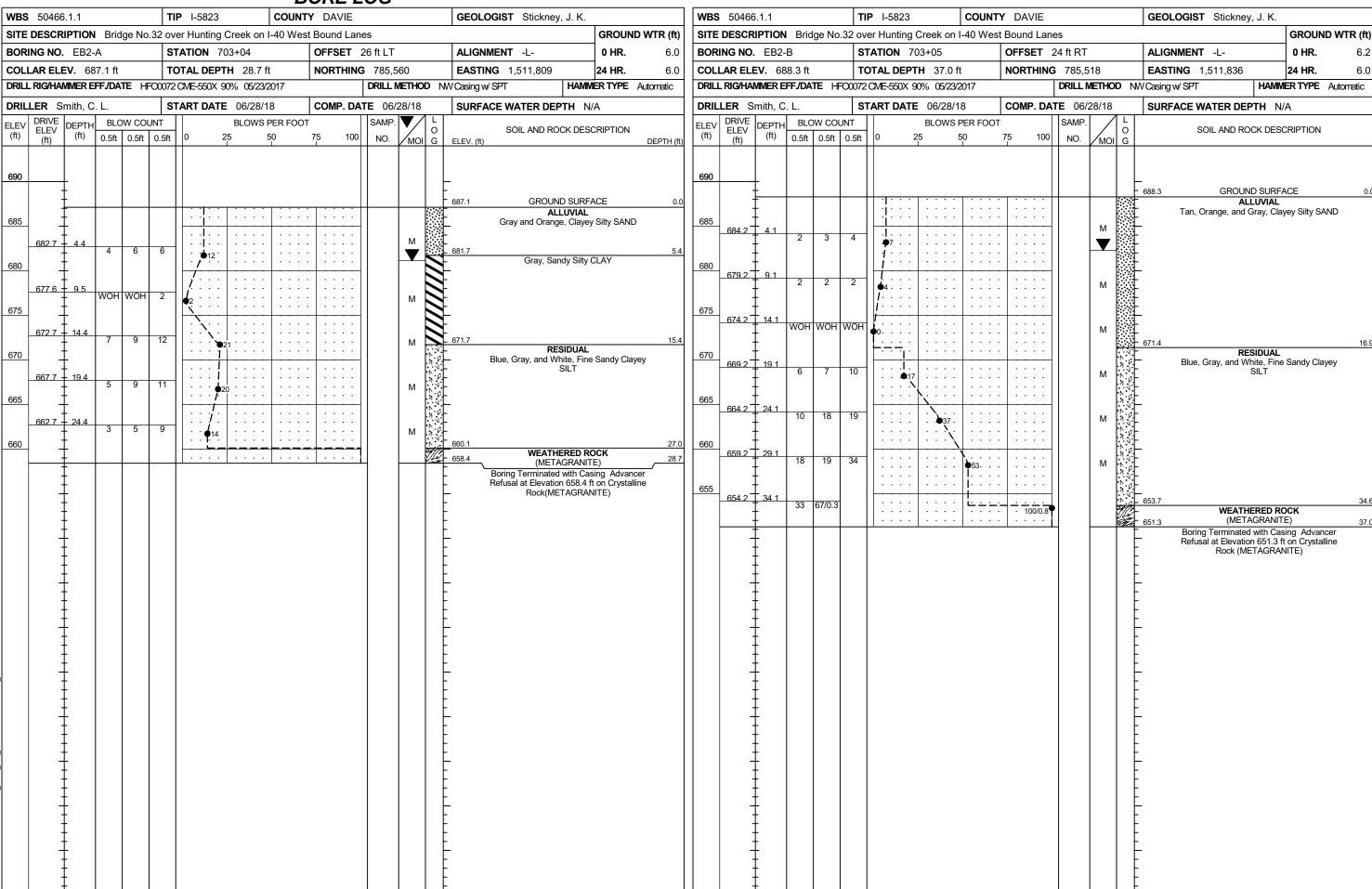
CORE PHOTOGRAPHS: Bridge No. 32 over Hunting Creek on I-40 West Bound Lanes, B2-A 702+39, 25' LT





SHEET 25

											В	<u>UKI</u>	<u> </u>	OG							
WBS	50466	3.1.1			Т	IP -	5823			СО	TNU	Y DA	√IE				GEOLOGIST S	tickney	, J. K.		
SITE	DESCR	IPTION	I Brid	dge No	o.32 ov	/er Ηι	ınting	Cree	k on	I-40 \	West	Bound	d Lan	es						GROUN	ID WTR (f
BORI	NG NO.	. B2-E	3		s	TATIO	ON 7	02+4	3			OFFS	ET 2	23 ft RT			ALIGNMENT -L			0 HR.	0.0
	AR ELE					OTAL						NORT	HING	785,4			EASTING 1,51	1,783		24 HR.	7.
DRILL	RIG/HAI	MMER E	FF./DA	TE H	FO0072	2 CME-	550X 9	90%	05/23/	2017				<u> </u>			W Casing w/ SPT		HAMIV	IER TYPE	Automatic
DRILI	LER S	mith, C				TART	DATI						P. DA	TE 06/	27/18	4 .	SURFACE WATE	ER DEF	PTH N	/A	
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft	0.5ft	1	0	:	BL 25		PER F		75	100	SAMP.	МО	0 1 6	SOIL / ELEV. (ft)	AND RO	CK DES	CRIPTION	DEPTH (
690	=======================================	-																	D SURF	ACE	(
685	-	-				-		-		-	 						Gray, Oran	ge, Silty	LUVIAL Sandy C Mica	LAY with S	ome
	682.4	4.9	1,4,611) MOLI	1,4,011			:				: :					-				
680	-	-	WOH	WOH	WOH	0.	· · ·	-	· · ·	ļ: -	· · ·	: :			M		_				
	677.5	9.8	I WOLL	WOLL	I WOLL			:				: :					-				
675	-	-	WOH	WOH	WOH	0.	· · ·	-		ļ: -	· · ·	: :			M		-				
-	672.5	14.8	WOH	WOH	1	1.			· · ·	:	 				M		- -				
670	-	-					· · · ·			-							- -				
-	667.5	19.8	5	6	16	1 -	· · · · · · · · · · · · · · · · · ·	22			 	: :			М		- 666.5				20
665	_	Ŧ				<u> -</u>	· · · · · · · · · · · · · · · · · · ·	Ţ. 		:							Blue, Gray		SIDUAL nite, Clay	ey Sandy S	SILT
_	662.5	24.8			155/0					:		: :					-				
660	-	Ŧ	20	45	55/0.5		!	+=		+÷	:	+ ÷ 10	00/1.0)		TT S	- 661.5 -		ERED R		25
	-	Ŧ						1		1:							-	(META	AGRANIT	E)	
-	657.5	29.8	8	29	71/0.4	-		:									-				
655	_	‡				<u> -</u>		<u> </u>		ļ:		- 10	00/0.9	1			-				
_	652.5	34.8	100/0					:									-				
650	-	‡	100/0.4	4				:			 	: 10	00/0.4	'			-				
		Ť						1		1:							-				
-	647.7 -	39.6	100/0.2	2							 	. 10	00/0.2	•			-				
645	-	‡				<u> </u>		<u> </u>		ļ:		: :					-				
	642.7 -	44.6	400/0 /					:			· · ·						-				
640	-	‡	100/0.2	1				:			 	. 10	00/0.2				-				
	-	Ī ,,,																			40
	637.7 -	49.6	60/0.1			<u> </u>	<u></u>	1.				 · · ·	50/0.1	+			- 637.6 - Boring	Termin	ated with	Standard	49 07.0
	-	Ŧ															Penetrationft on Crys	stalline R	lock (ME	TAGRANIT	E)
	-	Ē															35.2' Swit	tched to	AW Rod	s and Trico	ne
	-	£															_				
	-	Ŧ																			
	-	ŧ															_				
	-	ł																			
	-	ł															_				
	_	ţ															- -				
	-	‡															_				
	-	<u> </u>															_				
	-	t																			
	-	†															_				
	-	ł			1												-				



LABORATORY SUMMARY SHEET FOR ROCK CORE SAMPLES

PROJECT NO.: 50466.1.1

TIP: I-5823

COUNTY: DAVIE

Bridge No. 29 and No. 32 on I-40 over Hunting Creek

								Unit	Unconfined	
				Geologic	Run			Weight	Compressive	
Sample #	Boring #	Depth (ft)	Rock Type	Map Unit	RQD (%)	Length (in)	Diameter (in)	(PCF)	Strength (PSI)	Remarks
RS-1	B2-A	32.2-32.75	MetaGranite	CZg	30	0.55	1.86	156.1	468	Bridge No.32
RS-2	B1-B	48.5-49.1	MetaGranite	CZg	43	0.6	1.86	164.2	1,469	Bridge No. 29
RS-3	B1-A	40.9-41.6	MetaGranite	CZg	59	0.7	1.86	167.7	6,300	Bridge No. 29

BRIDGE NO. 29 SITE PHOTOGRAPH



Photograph No. 1: Looking at End Bent 1 toward End Bent 2

BRIDGE NO. 32 SITE PHOTOGRAPH





Photograph No. 1: Looking at End Bent 1 toward End Bent 2

Photograph No. 2: Looking Downstream