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REFERENCE: R-3825B

PROJECT: 34552

SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-3825B	1	217

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CROSS SECTIONS

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-Y6-	10+50 TO 17+00	183-187
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-Y13-	14+84 TO 34+50	208-217

ROADWAY SUBSURFACE INVESTIGATION

COUNTY JOHNSTON
PROJECT DESCRIPTION NC 42 FROM EAST OF SR 1902
(GLEN LAUREL ROAD) TO
SR 1003 (BUFFALO ROAD)

INVENTORY

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

GEOSYNTEC _____

GEOLOGIC EXPLORATION _____

CAROLINA DRILLING _____

INVESTIGATED BY NJOROGE WAINAINA

DRAWN BY CHLUCK TURLINGTON

CHECKED BY NJOROGE WAINAINA

SUBMITTED BY NJOROGE WAINAINA

DATE FEBRUARY 2017



DocuSigned by:
Njoroge Wainaina 3/1/2017

SIGNATURE DATE

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**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, *VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6*

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)							ORGANIC MATERIALS				
	A-1	A-3	A-2	A-2-4	A-2-5	A-2-6	A-2-7	A-4	A-5	A-6	A-7	A-1, A-2	A-3	A-4, A-5	A-6, A-7				
GROUP CLASS.	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7		A-4	A-5	A-6	A-7	A-1, A-2	A-3	A-4, A-5	A-6, A-7				
SYMBOL	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	[Symbol]	
% PASSING #10 #40 #200	50 MX 30 MX 15 MX	50 MX 25 MX	51 MN 35 MX	35 MX	35 MX	35 MX	35 MX	36 MN	36 MN	36 MN	36 MN	36 MN	36 MN						
MATERIAL PASSING #40 LL PI	-	-	40 MX 10 MN	41 MN 10 MN	40 MX 11 MN	41 MN 11 MN	40 MX 11 MN	41 MN 11 MN	40 MX 11 MN	41 MN 11 MN									
GROUP INDEX	0	0	0	4 MX	8 MX	12 MX	16 MX	NO MX											
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND	FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND	SILTY SOILS	CLAYEY SOILS														
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD							FAIR TO POOR				FAIR TO POOR	POOR	UNSATURABLE					

PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE OPENING (MM)	4	10	40	60	200	270
	4.76	2.00	0.42	0.25	0.075	0.053
BOULDER (BLDR.)						
COBBLE (COB.)						
GRAVEL (GR.)						
COARSE SAND (CSE. SD.)						
FINE SAND (F SD.)						
SILT (SL.)						
CLAY (CL.)						

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
LL - LIQUID LIMIT	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
PL - PLASTIC LIMIT	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
OM - OPTIMUM MOISTURE SHRINKAGE LIMIT	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
SL - SHRINKAGE LIMIT	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

NON PLASTIC	PLASTICITY INDEX (PI)	DRY STRENGTH
SLIGHTLY PLASTIC	0-5	VERY LOW
MODERATELY PLASTIC	6-15	SLIGHT
HIGHLY PLASTIC	16-25	MEDIUM
	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE LL < 31
MODERATELY COMPRESSIBLE LL = 31 - 50
HIGHLY COMPRESSIBLE LL > 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE 1 - 10%
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE 10 - 20%
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME 20 - 35%
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY 35% AND ABOVE

GROUND WATER

- Water level in bore hole immediately after drilling
- Static water level after 24 hours
- Perched water, saturated zone, or water bearing strata
- Spring or seep

MISCELLANEOUS SYMBOLS

ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION

SOIL SYMBOL

ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT

INFERRED SOIL BOUNDARY

INFERRED ROCK LINE

ALLUVIAL SOIL BOUNDARY

DIP & DIP DIRECTION OF ROCK STRUCTURES

TEST BORING

AUGER BORING

CORE BORING

MONITORING WELL

PIEZOMETER INSTALLATION

SLOPE INDICATOR INSTALLATION

CONE PENETROMETER TEST

SOUNDING ROD

TEST BORING WITH CORE

SPT N-VALUE

RECOMMENDATION SYMBOLS

UNDERCUT

SHALLOW UNDERCUT

UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE

UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK

UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL

ABBREVIATIONS

- AR - AUGER REFUSAL
- BT - BORING TERMINATED
- CL - CLAY
- CPT - CONE PENETRATION TEST
- CSE - COARSE
- DMT - DILATOMETER TEST
- DPT - DYNAMIC PENETRATION TEST
- e - VOID RATIO
- F - FINE
- FOSS. - FOSSILIFEROUS
- FRAC. - FRACTURED, FRACTURES
- FRAGS. - FRAGMENTS
- HI. - HIGHLY
- MED. - MEDIUM
- MICA - MICACEOUS
- MOD. - MODERATELY
- NP - NON PLASTIC
- ORG. - ORGANIC
- PMT - PRESSUREMETER TEST
- SAP. - SAPROLITIC
- SD. - SAND, SANDY
- SL. - SILT, SILTY
- SLI. - SLIGHTLY
- TCR - TRICONE REFUSAL
- w - MOISTURE CONTENT
- V - VERY
- VST - VANE SHEAR TEST
- WEA. - WEATHERED
- UNIT WEIGHT
- DRY UNIT WEIGHT
- SAMPLE ABBREVIATIONS
- S - BULK
- SS - SPLIT SPOON
- ST - SHELBY TUBE
- RS - ROCK
- RT - RECOMPACTED TRIAXIAL
- CBR - CALIFORNIA BEARING RATIO

EQUIPMENT USED ON SUBJECT PROJECT

- DRILL UNITS:
 - CME-45C
 - CME-55
 - CME-550
 - VANE SHEAR TEST
 - PORTABLE HOIST
 - DIEDRICH D-50
- ADVANCING TOOLS:
 - CLAY BITS
 - 6" CONTINUOUS FLIGHT AUGER
 - 8" HOLLOW AUGERS
 - HARD FACED FINGER BITS
 - TUNG-CARBIDE INSERTS
 - CASING W/ ADVANCER
 - TRICONE *STEEL TEETH
 - TRICONE *TUNG-CARB.
 - CORE BIT
- HAMMER TYPE:
 - AUTOMATIC MANUAL
- CORE SIZE:
 - B
 - H
 - N
- HAND TOOLS:
 - POST HOLE DIGGER
 - HAND AUGER
 - SOUNDING ROD
 - VANE SHEAR TEST

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)	NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
CRYSTALLINE ROCK (CR)	FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
NON-CRYSTALLINE ROCK (NCR)	FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTARY ROCK (CP)	COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

- FRESH** - ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
- VERY SLIGHT (IV SLI.)** - ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
- SLIGHT (SLI.)** - ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
- MODERATE (MOD.)** - SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
- MODERATELY SEVERE (MOD. SEV.)** - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*
- SEVERE (SEV.)** - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*
- VERY SEVERE (IV SEV.)** - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*
- COMPLETE** - ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

- VERY HARD** - CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
- HARD** - CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
- MODERATELY HARD** - CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
- MEDIUM HARD** - CAN BE GROOVED OR GOUGED 0.25 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
- SOFT** - CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
- VERY SOFT** - CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.

FRACTURE SPACING

TERM	SPACING	TERM	THICKNESS
VERY WIDE	MORE THAN 10 FEET	VERY THICKLY BEDDED	4 FEET
WIDE	3 TO 10 FEET	THICKLY BEDDED	1.5 - 4 FEET
MODERATELY CLOSE	1 TO 3 FEET	THINLY BEDDED	0.16 - 1.5 FEET
CLOSE	0.16 TO 1 FOOT	VERY THINLY BEDDED	0.03 - 0.16 FEET
VERY CLOSE	LESS THAN 0.16 FEET	THICKLY LAMINATED	0.008 - 0.03 FEET
		THINLY LAMINATED	< 0.008 FEET

INDURATION

- FRIABLE** - RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
- MODERATELY INDURATED** - GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
- INDURATED** - GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
- EXTREMELY INDURATED** - SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

- ALLUVIUM (ALLUV.)** - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
- AQUIFER** - A WATER BEARING FORMATION OR STRATA.
- ARENACEOUS** - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
- ARGILLACEOUS** - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
- ARTESIAN** - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
- CALCAREOUS (CALC.)** - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
- COLLUVIUM** - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
- CORE RECOVERY (REC.)** - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
- DIKE** - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
- DIP** - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
- DIP DIRECTION (DIP AZIMUTH)** - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
- FAULT** - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
- FISSILE** - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
- FLOAT** - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOADED FROM PARENT MATERIAL.
- FLOOD PLAIN (FP)** - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
- FORMATION (FM)** - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
- JOINT** - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
- LEDGE** - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
- LENS** - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
- MOTTLED (MOT.)** - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
- PERCHED WATER** - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
- RESIDUAL (RES.) SOIL** - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
- ROCK QUALITY DESIGNATION (ROD)** - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
- SAPROLITE (SAP.)** - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
- SILL** - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
- SLICKENISE** - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
- STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT)** - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
- STRATA CORE RECOVERY (SREC.)** - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- STRATA ROCK QUALITY DESIGNATION (SROD)** - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
- TOPSOIL (TS.)** - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK:

ELEVATION: FEET

NOTES:

FIAD = FILLED IN AFTER DRILLING
SPT REF = STANDARD PENETRATION TEST REFUSAL

TIN FILE NAME "R3825b_1s.tn1.tin" WITH FILE DATE 4-5-2016, WAS USED TO GENERATE BORING ELEVATIONS.

9/20/19

See Sheet 1A For Index of Sheets
See Sheet 1B For Conventional Symbols

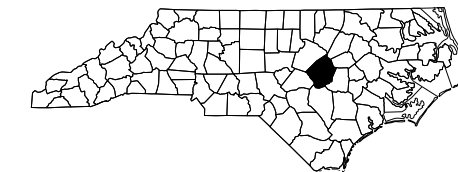
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

JOHNSTON COUNTY

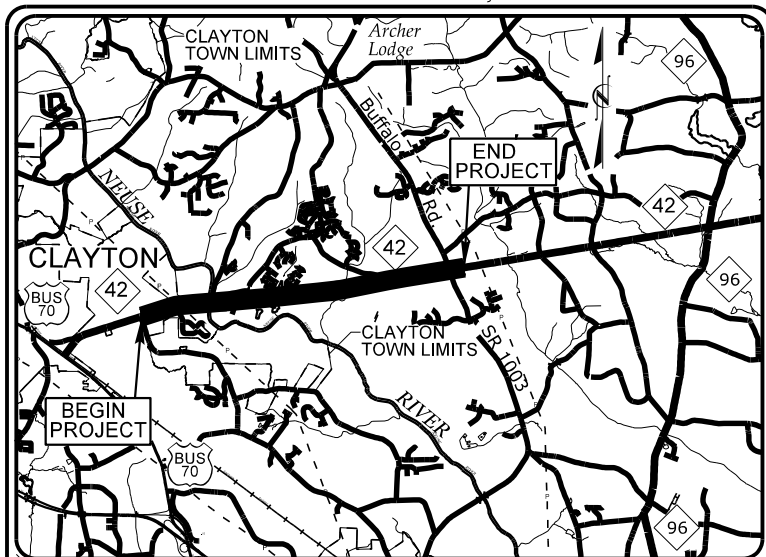
LOCATION: NC 42 FROM SR 1902 (GLEN LAUREL RD)
TO SR 1003 (BUFFALO RD)

TYPE OF WORK: GRADING, PAVING, STRUCTURES,
DRAINAGE AND SIGNALS

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-3825B	3	217
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
34552.1.FR3	STP-0042(58)	P.E.	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			

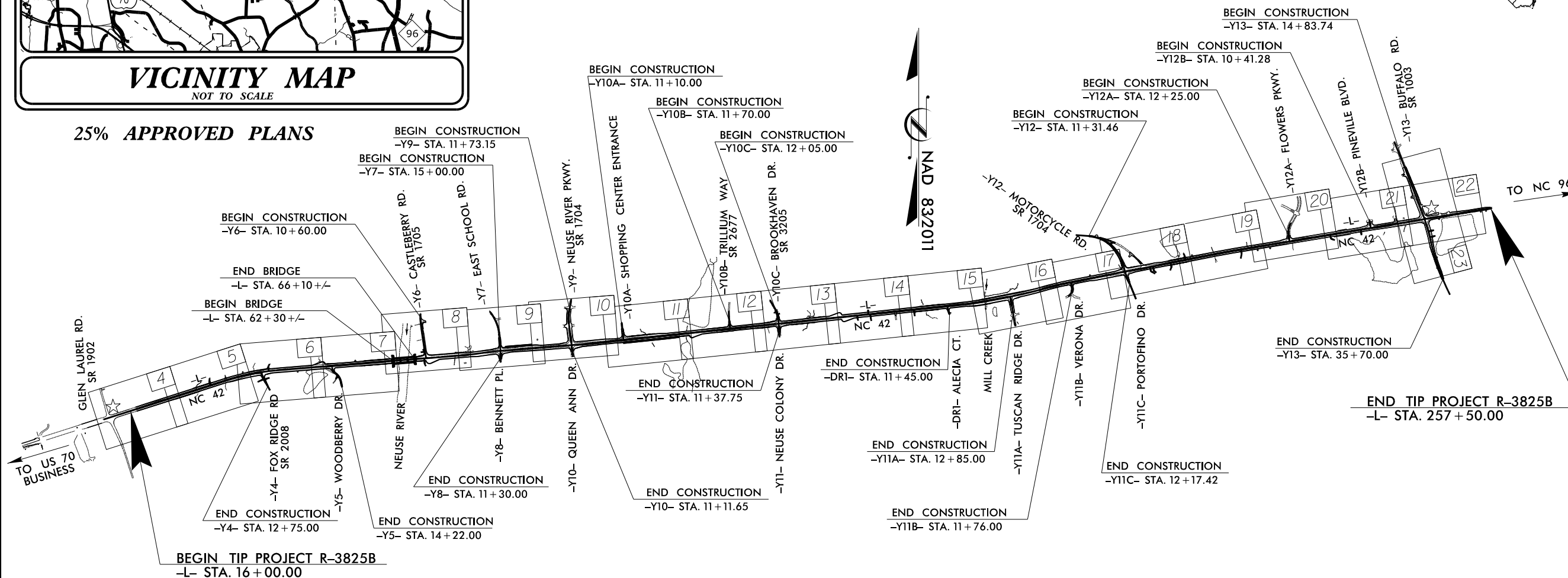


TIP PROJECT: R-3825B



VICINITY MAP
NOT TO SCALE

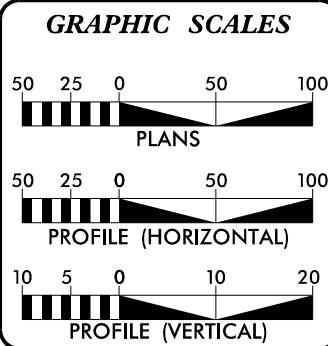
25% APPROVED PLANS



CLEARING ON THIS PROJECT WILL BE PERFORMED
TO THE LIMITS ESTABLISHED BY NCDOT METHOD

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

CONTRACT:



DESIGN DATA

ADT 2015 =	19,000
ADT 2040 =	37,400
K =	9%
D =	65%
T =	6% *
V =	60 MPH
* (TTST % + DUAL %)	
FUNC. CLASS = RURAL MINOR ARTERIAL REGIONAL TIER	
DESIGN EXCEPTION (GRADE)	

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT R-3825B =	4.521 MILES
LENGTH BRIDGE TIP PROJECT R-3825B =	0.072 MILES
TOTAL LENGTH OF TIP PROJECT R-3825B =	4.593 MILES

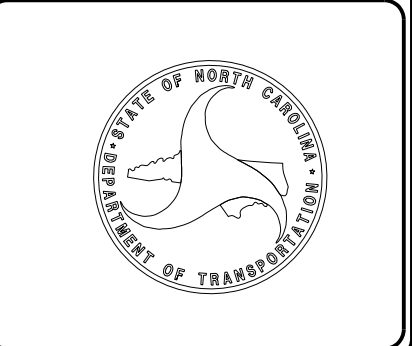
URS
URS Corporation - North Carolina
1600 Perimeter Park Drive
Morrisville, North Carolina 27560
TELEPHONE: (919) 461-1100 FAX: (919) 461-1415
NC LICENSE # C-2043

2012 STANDARD SPECIFICATIONS	ED EDENS, P.E. PROJECT ENGINEER
RIGHT OF WAY DATE: JUNE 17, 2016	KEVIN VAN METRE, P.E. PROJECT DESIGN ENGINEER
LETTING DATE: JUNE 19, 2018	GARY R. LOVERING, P.E. PROJECT ENGINEER NCDOT ROADWAY DESIGN

HYDRAULICS ENGINEER

P.E.
SIGNATURE:
ROADWAY DESIGN ENGINEER

P.E.
SIGNATURE:



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February 22, 2017

WBS NUMBER 34552.1.FR3
TIP NUMBER R-3825B
F. A. Proj.#: STP-0042(58)
COUNTY: Johnston

DESCRIPTION: NC 42 from East of SR 1902 (Glen Laurel Rd.) to SR 1003
 (Buffalo Road)

SUBJECT: Geotechnical Report – Inventory

Geosyntec Consultants has completed a subsurface investigation for this project and present the following inventory.

Project Description

The project area lies east of the town of Clayton along the existing NC 42 beginning just east of Glen Laurel Road (Station 15+00) and extends eastward 4.6 miles to just east of Buffalo Road (Station 257+50). The proposed project consist of widening of the existing NC 42 from two lanes to four with median and turn lanes with multiple embankments of up to 30 feet and cut sections as deep as 30 feet. The widening will be accomplished by addition of two new travel lanes on the southern side of the existing NC 42 on majority of the roadway segments and addition of two new travel lanes on each side of the existing NC 42 in some roadway segments. Additionally, some widening and realignment is proposed for some of the intersecting -Y- lines.

The geotechnical investigation was conducted in March and April of 2016. Borings were advanced with ATV mounted Dietrich D50 and CME 45C drill machines. Both drill machines were equipped with automatic hammers. Standard Penetration tests were performed in all borings. Representative soil samples were collected for visual classification in the field and for laboratory analysis by Falcon Engineering.

The following alignments, totaling 5.5 miles were investigated. Subsurface cross sections of these alignments are included in this report.

<u>Line</u>	<u>Station</u>
-L-	15+00 to 257+50
-Y4-	10+00 to 12+75
-Y5-	10+00 to 14+22
-Y6-	10+60 to 17+91
-Y7-	15+00 to 17+35
-Y9-	11+73 to 18+12
-Y10A	11+10 to 13+13
-Y10C-	12+05 to 14+53
-Y11A-	10+00 to 12+85
-Y11B-	10+00 to 11+76
-Y12-	11+31 to 20+99
-Y13-	14+84 to 35+70

Physiography and Geology

The portion of the project corridor west of the Neuse River is within the Piedmont Physiographic Province and the portion east of the Neuse River is within the boundary of the Piedmont Province and the Coastal Plain Physiographic Province. Surficial soils within the Piedmont Province consist of residual sandy silts derived from weathering of the underlying metamorphic rocks of the Raleigh Belt. Within the Coastal Plain Province, the surficial soils consist of Terrace deposits and Upland sediments consisting of gravel, sands, clayey sands, sandy silts and sandy clays. The Coastal Plain surficial soils are underlain by the residual soils of the Piedmont Province.

The topography west of the Neuse River is generally sloping down towards the river. The elevation at the beginning of the project is approximately 310± feet and decreases to approximately 140± feet on the west bank of the Neuse River. The topography east of the Neuse River is generally rolling and ranges in elevation from a high of approximately 310± feet to approximately 150± feet. Surface waters generally flows towards the Neuse River and surface drainage is generally good.

Soil Properties

Soils encountered in this project have been divided into five categories, 1) roadway embankment 2) alluvial soils, 3) Coastal Plain soils, 4) residual soils, 5) artificial fill.

Roadway embankment soils are present along existing NC 42. They typically consist of red, brown, soft to medium stiff, moist, sandy and clayey silts (A-4 and A-5) and loose to medium dense, moist, silty sands (A-2-4 and A-2-5).

Alluvial soils are encountered throughout the project along the flood plains of the Neuse river, creeks and adjacent to wetlands. Alluvial soils are either present on the surface or under roadway embankment soils. These soils consist of tan, gray, red and brown loose to medium dense, moist, gravelly sands (A-1-a and A-1-b), and red, brown, soft to stiff, moist, sandy silts, sandy clays and silty clays (A-4, A-6 and A-7-6). These soils range in thickness from 2± to 6± feet.

Coastal Plain soils were encountered east of the Neuse River either on the surface, or under the roadway embankment or alluvial soils. Coastal plain deposits range in thickness from 2± to 10± feet. They consist of brown, red and gray, very loose to medium dense, moist, silty and clayey sands (A-2-4 and A-2-6) and soft to stiff, moist, sandy silts, sandy clays and silty clays (A-4, A-6 and A-7-6).

Residual soils were encountered throughout the project. Residual soils primarily consist of red, yellow and brown, medium stiff to very stiff, moist, sandy and clayey silts (A-4 and A-5) with trace weathered rock fragments.

Artificial fills are encountered in several areas along the -L- alignment and along -Y11B- and -Y13-. The artificial fill located on the right side, starting from Station 46+00 to 49+00 consist of red, brown, very loose to medium dense, moist, clayey sand (A-2-7) mixed with chunks of asphalt, concrete pavement materials, and concrete. The other areas of the artificial fill consist of soil materials including red and brown, loose to medium dense, moist, silty sand (A2-4) to clayey sand (A-2-6) and stiff to very stiff, moist, clayey and sandy silt (A-4) and (A-5).

Rock Properties

Weathered rock was encountered in deeper borings and some shallow borings in lower elevations. Weathered rock is derived from the weathering of the underlying schist. Crystalline rock was encountered at the following stations:

<u>Line</u>	<u>Stations</u>
-L-	58+75 to 59+25
-L-	61+75 to 62+75
-L-	106+25 to 107+25
-L-	135+25 to 135+75

-L-	155+75 to 156+75
-L-	165+25 to 165+75
-L-	166+25 to 157+25

Ground Water

Ground water data was collected during March and April of 2016, during a time of normal precipitation. Ground water occurs in low lying areas and adjacent to bodies of water at depths ranging from 0 to 15 feet from the existing ground surface elevation. Seasonal fluctuations of the ground water table should be anticipated.

Seeps: One area where water was seeping at the interface between the embankment and natural ground was noted at the following stations:

<u>Line</u>	<u>Station(±)</u>	<u>Offsets</u>
-L-	60+80 to 62+00	RT

Water Wells: Water wells were noted within or in close proximity to the construction limits at the following locations:

<u>Line</u>	<u>Stations</u>	<u>Offsets</u>
-L-	44+60	39' RT
-L-	162+60	70' RT
-L-	163+35	65' RT
-L-	184+75	39' LT
-L-	236+10	60' RT

Ponds: Three ponds were noted within the construction limit at the following location:

<u>Line</u>	<u>Stations</u>	<u>Offsets</u>
-L-	116+50 to 118+50	LT
-L-	200+50 to 202+50	LT
-L-	248+50 to 249+50	RT

Areas of Special Geotechnical Interest

1) High Ground Water: Ground water was encountered within 6 feet of final grade at the following locations:

<u>Line</u>	<u>Station</u>	<u>Offsets</u>
-L-	41+75 to 43+25	RT
-L-	75+25 to 76+75	LT
-L-	79+75 to 82+25	LT and RT
-L-	110+25 to 113+25	LT and RT
-L-	234+25 to 236+25	LT and RT
-L-	243+75 to 248+75	LT and RT
-L-	251+00 to 253+25	LT and RT
-L-	255+50 to 257+75	LT and RT
-Y7-	14+75 to 16+50	LT and RT

2) Artificial Fill: Areas of artificial fill were encountered at the following locations:

<u>Line</u>	<u>Station</u>	<u>Offsets</u>
-L-	46+00 to 51+50	RT
-L-	95+00 to 100+00	LT
-L-	107+00 to 109+00	RT
-L-	112+17 to 114+00	RT
-L-	117+00 to 119+00	LT
-L-	154+45 to 155+45	LT
-L-	167+70 to 170+80	RT
-L-	172+50 to 182+00	RT
-L-	183+00 to 186+00	RT
-L-	199+60 to 200+50	LT
-L-	201+00 to 202+00	LT
-L-	207+25 to 208+25	LT
-L-	213+25 to 213+75	LT
-L-	215+50 to 217+00	LT
-L-	248+75 to 250+00	LT
-Y6-	13+25 to 16+25	LT
-Y11B-	10+70 to 12+20	RT
-Y13-	21+00 to 22+50	RT

3) Soft Soils: The following areas were found to contain soft clay soils

<u>Line</u>	<u>Station</u>	<u>Offsets</u>
-L-	25+50 to 29+00	LT and RT
-L-	66+50 to 67+50	LT and RT
-L-	75+25 to 77+00	LT
-L-	85+50 to 88+00	LT and RT
-L-	136+50 to 137+00	RT
-L-	165+50 to 166+50	LT and RT
-L-	230+00 to 233+50	LT and RT

4) High Plastic Clays: Clay soils with high plasticity (PI > 15) were encountered either within three feet of final grade or above final grade in cut areas at the following locations:

<u>Line</u>	<u>Station</u>	<u>Offsets</u>
-L-	77+00 to 79+50	RT and LT
-L-	82+00 to 83+50	LT and RT
-L-	92+50 to 94+50	LT and RT
-L-	177+50 to 181+00	LT and RT
-L-	207+00 to 208+50	RT
-L-	214+75 to 215+75	RT
-L-	217+80 to 218+50	LT and RT
-L-	220+50 to 222+00	LT
-L-	224+00 to 226+00	LT and RT
-L-	243+00 to 246+50	LT and RT
-L-	251+50 to 253+00	LT and RT
-Y6-	14+00 to 15+25	LT
-Y7-	14+50 to 16+50	LT and RT

Culverts

Culvert at -L- 107+30

Natural ground elevations range from 160± feet at the bottom of the stream to 170± feet along the adjacent floodplain. Borings completed in the vicinity show approximately 5± feet of very loose alluvial clayey sand (A-2-6) underlain by weathered rock. Ground water was measured at an elevation of 157± feet during this investigation.

Culvert at -L- 114+10

Natural ground elevations range from 147± feet at the bottom of the stream to 170± feet along the adjacent floodplain. Borings completed in the vicinity show approximately 10± feet of very loose alluvial clayey sand (A-2-6) underlain by 10± feet of very loose Coastal Plain clayey sand (A-2-6) and very soft to soft sandy clay (A-6) and weathered rock. Ground water was measured at an elevation of 153± feet during this investigation.

Culvert at -L- 135+50

Natural ground elevations range from 165± feet at the bottom of the stream to 177± feet along the adjacent floodplain. The boring on the downstream side show approximately 5± feet of very loose to loose alluvial clayey sand (A-2-6) underlain by weathered rock. The boring on the upstream side show approximately 18± feet of very stiff to hard residual sandy silt (A-4) and dense to very dense gravelly sand (A-1-a) and silty sand (A-2-4) underlain by weathered rock. Ground water was measured at an elevation of 165± feet during this investigation.

Culvert at -L- 156+30

Natural ground elevations range from 160± feet at the bottom of the stream to 180± feet along the adjacent floodplain. The borings completed in the vicinity show approximately 6± feet of soft to stiff alluvial clayey sand (A-2-6) underlain by 15± feet of stiff to hard residual sandy silt (A-4) and weathered rock. Ground water was measured at an elevation of 157± feet during this investigation.

Culvert at -L- 167+10

Natural ground elevations range from 150± feet at the bottom of the stream to 155± feet along the adjacent floodplain. The borings completed in the vicinity show approximately 5± feet of soft to medium stiff alluvial clayey sand (A-2-6) underlain by 10± feet of hard residual sandy silt (A-4) and weathered rock. Ground water was measured at an elevation of 154± feet during this investigation.

Stream Relocation

Stream Relocation at -L- 73+20 to 76+50

A jurisdictional stream that drains into a 24" RCP at Station 73+20 will require relocation to an alignment outside of the proposed embankment slope stakes. The borings in the vicinity of the proposed alignment show approximately 10± feet of medium stiff to very stiff, alluvial, sandy clays (A-6) underlain by medium stiff to hard, residual, sandy silt (A-4). Shallow ground water less than 4± feet was encountered at the boring closest to the proposed alignment.

Stream Relocation at -L- 135+50 to 137+00

A jurisdictional stream that drains into the culvert at Station 135+50 will require relocation to an alignment outside of the proposed embankment slope stakes. The borings in the vicinity of the proposed alignment show approximately 3± feet of very loose to loose, alluvial, clayey sands (A-2-6) underlain by very stiff, residual, sandy silt (A-4). Shallow ground water less than 4± feet was encountered at the boring closest to the proposed alignment.

Stream Relocation at -L- 167+00 to 171+00

A jurisdictional stream that drains into the culvert at Station 167+10 will require relocation to an alignment outside of the proposed embankment slope stakes. The borings in the vicinity of the proposed alignment show approximately 7± feet of soft to medium stiff, alluvial, sandy silts (A-4) underlain by medium stiff, residual, sandy silt (A-4).

Prepared by

Njoroge Wainaina
Senior Consultant

APPENDIX A

Undisturbed and Bulk Samples

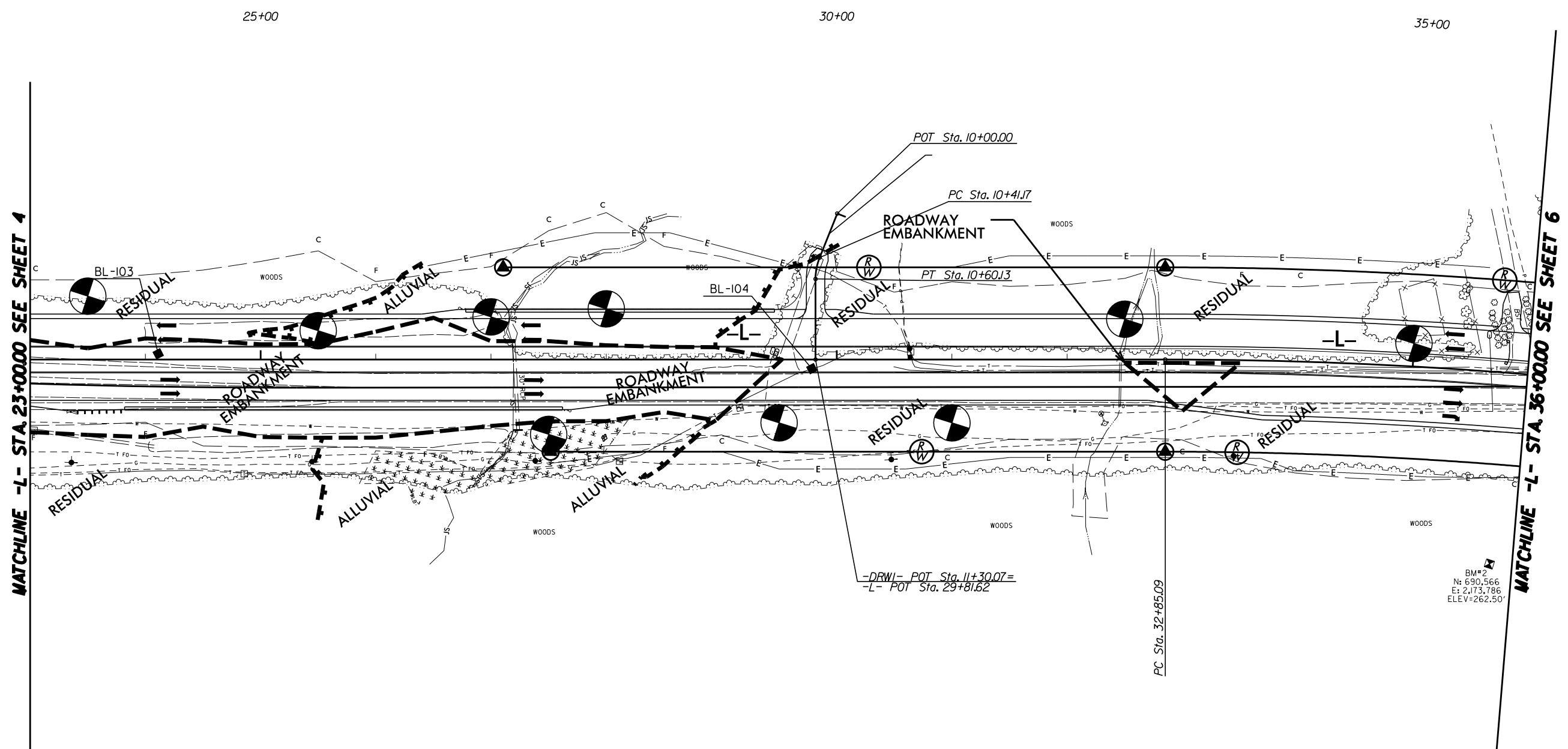
Undisturbed Samples: Undisturbed thin wall Shelby tube samples were collected at the following locations and submitted for testing.

<u>Sample No.</u>	<u>Station</u>	<u>Depth (ft)</u>	<u>Test</u>
ST-1	49+50, 35' RT	9.0-11.0	Consolidation, Triaxial CU
ST-2	28+00, 44' LT	7.0-9.0	Consolidation, Triaxial CU

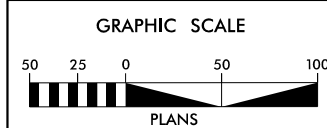
Bulk samples were collected at the following locations and submitted for testing.

<u>Sample No.</u>	<u>Station</u>	<u>Depth (ft)</u>	<u>Test</u>
BS-1	53+50, 55' RT and 55+00, 55RT	5.0 to 32.0	CBR
BS-2	51+50, 65' LT	8.0 to 10.0	CBR

PROJECT REFERENCE NO. R-3825B	SHEET NO. 5
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



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E: 2173.786
ELEV=262.50'



REVISIONS

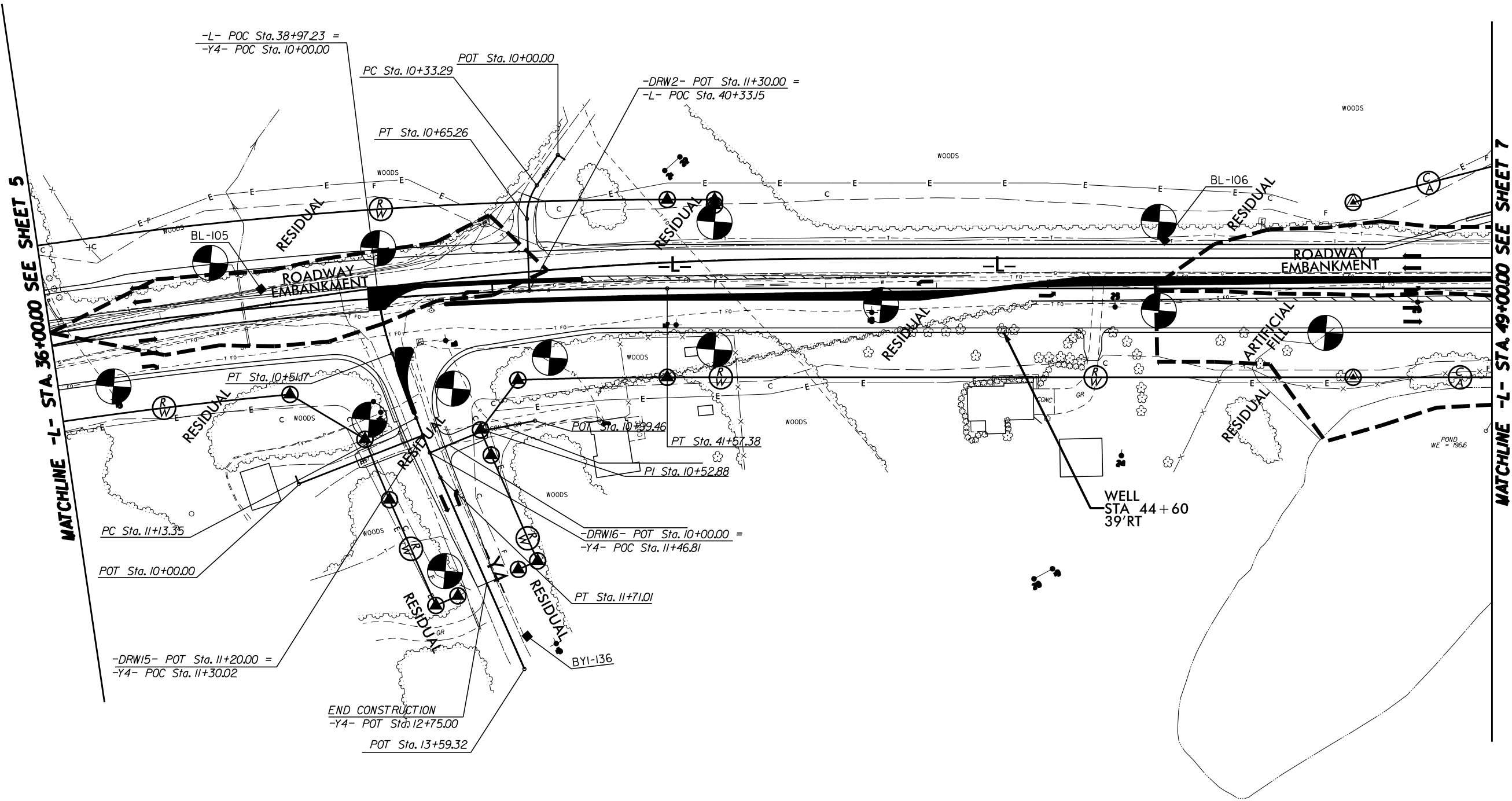
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40+00

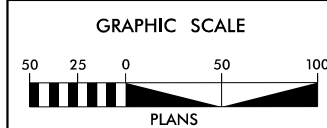
45+00



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MATCHLINE -L- STA. 49+00.00 SEE SHEET 7

END CONSTRUCTION
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POT Sta. 13+59.32



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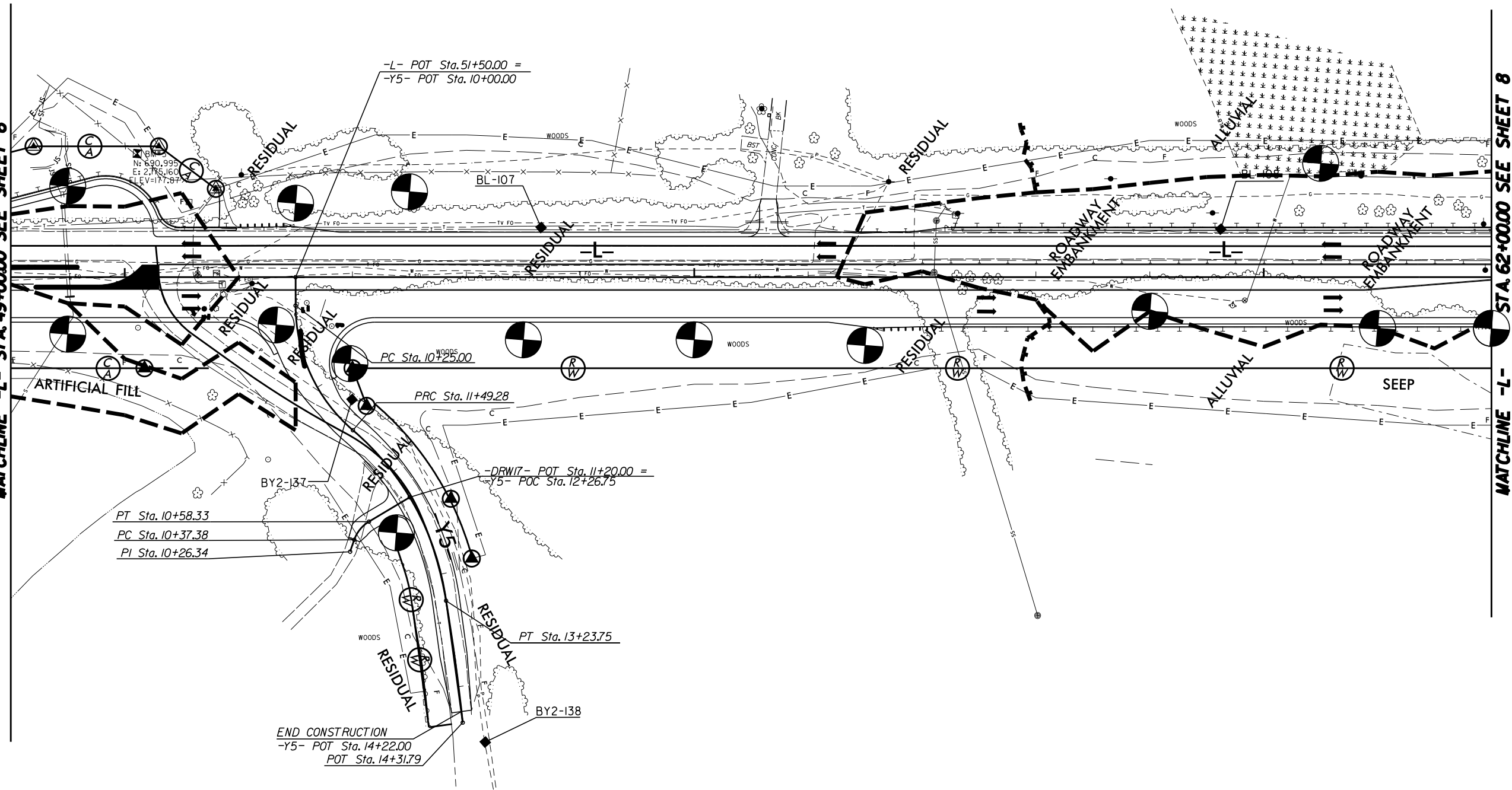
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55+00

60+00

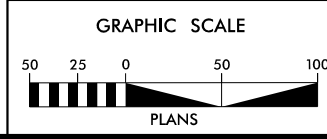
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MATCHLINE -L- STA. 62+00.00 SEE SHEET 8



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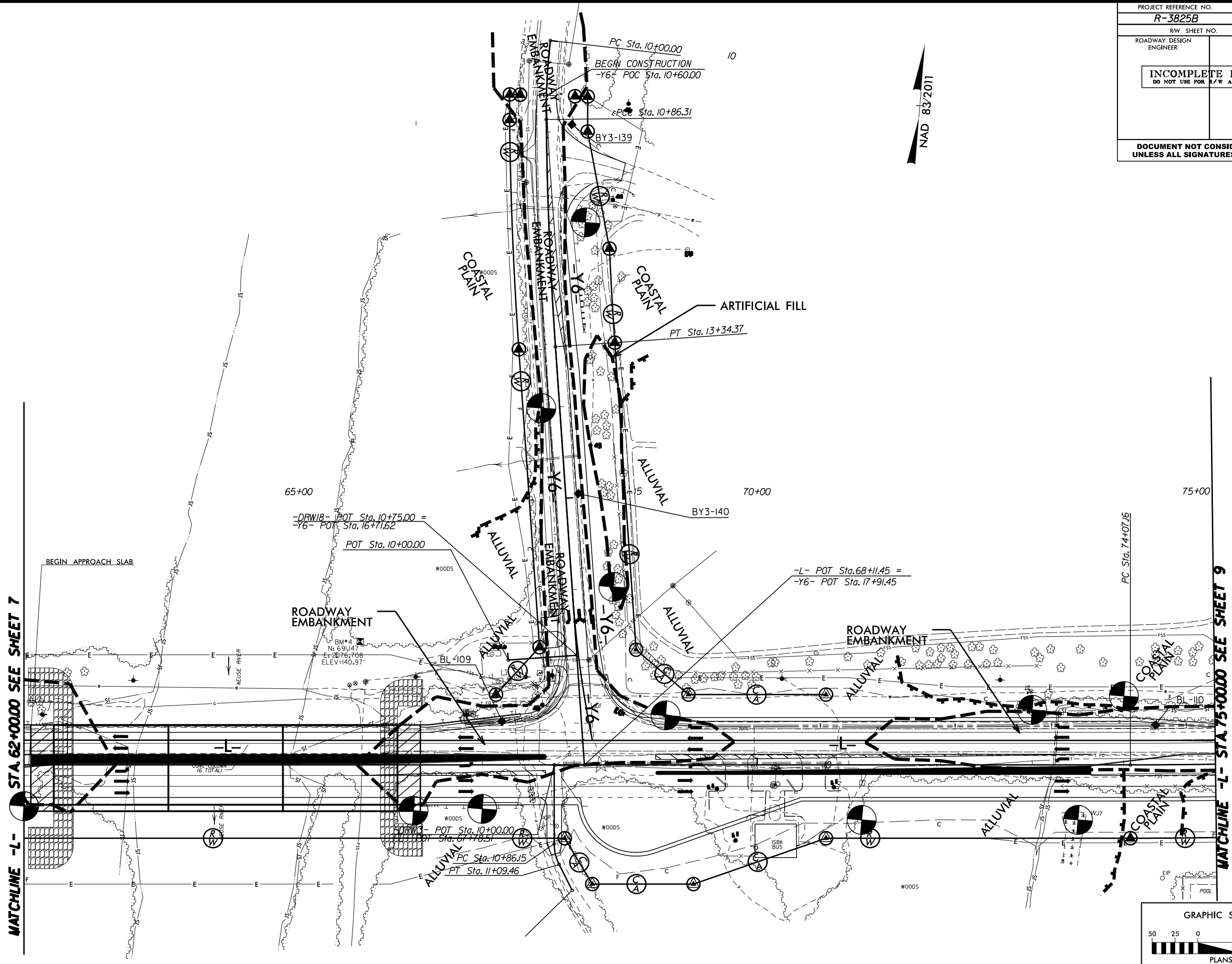
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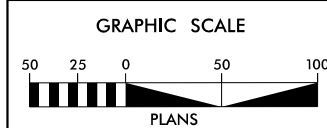
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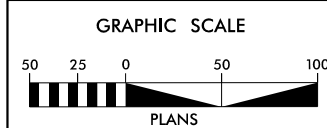
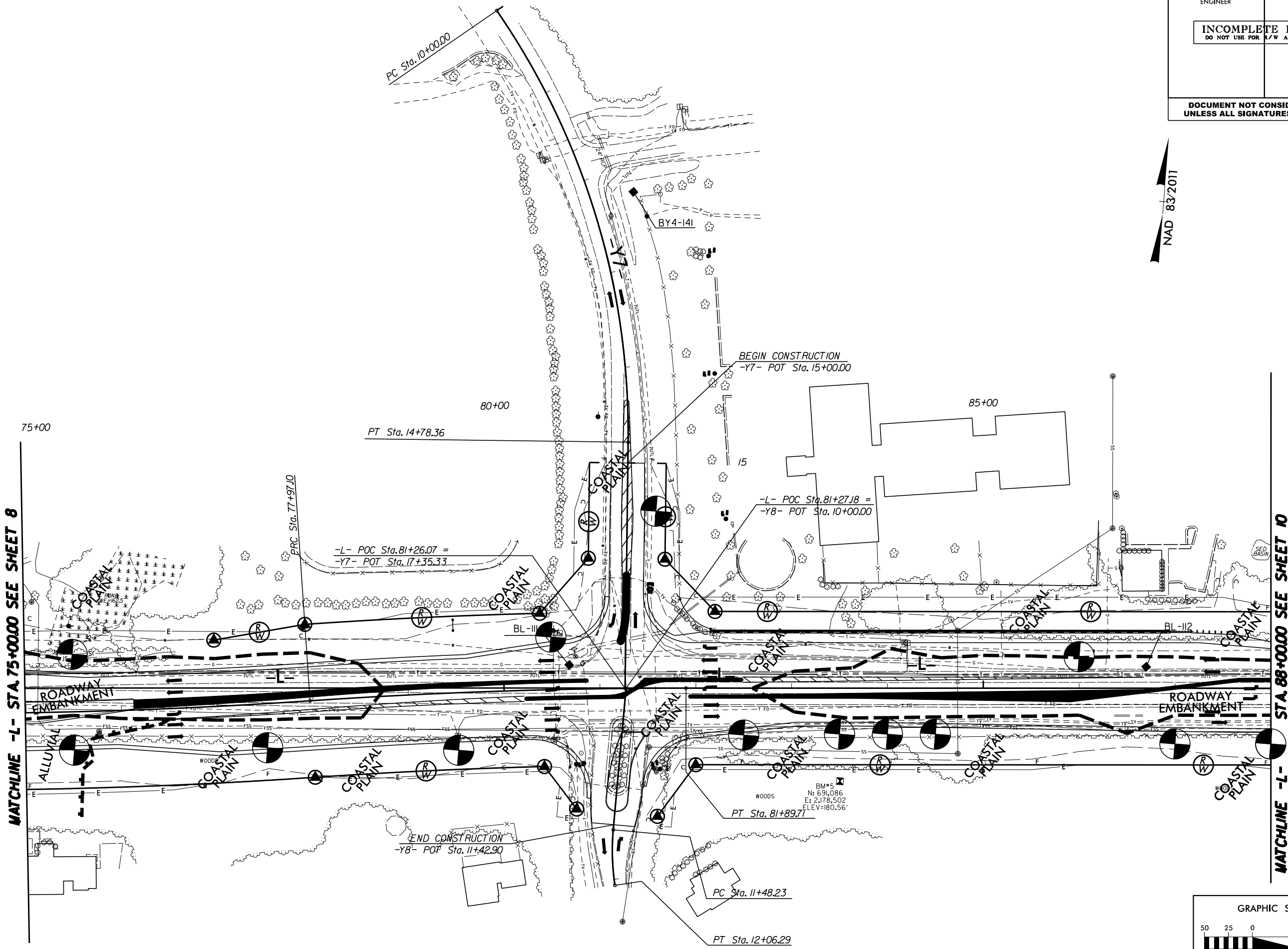
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MATCHLINE -L- STA 75+00.00 SEE SHEET 9



PROJECT REFERENCE NO. R-3825B	SHEET NO. 9
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ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
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NAD 83/2011

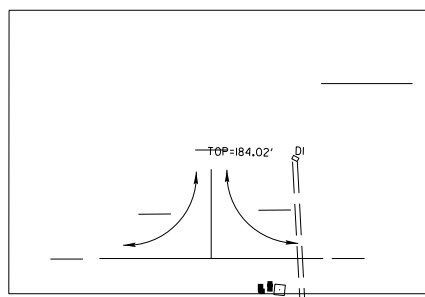
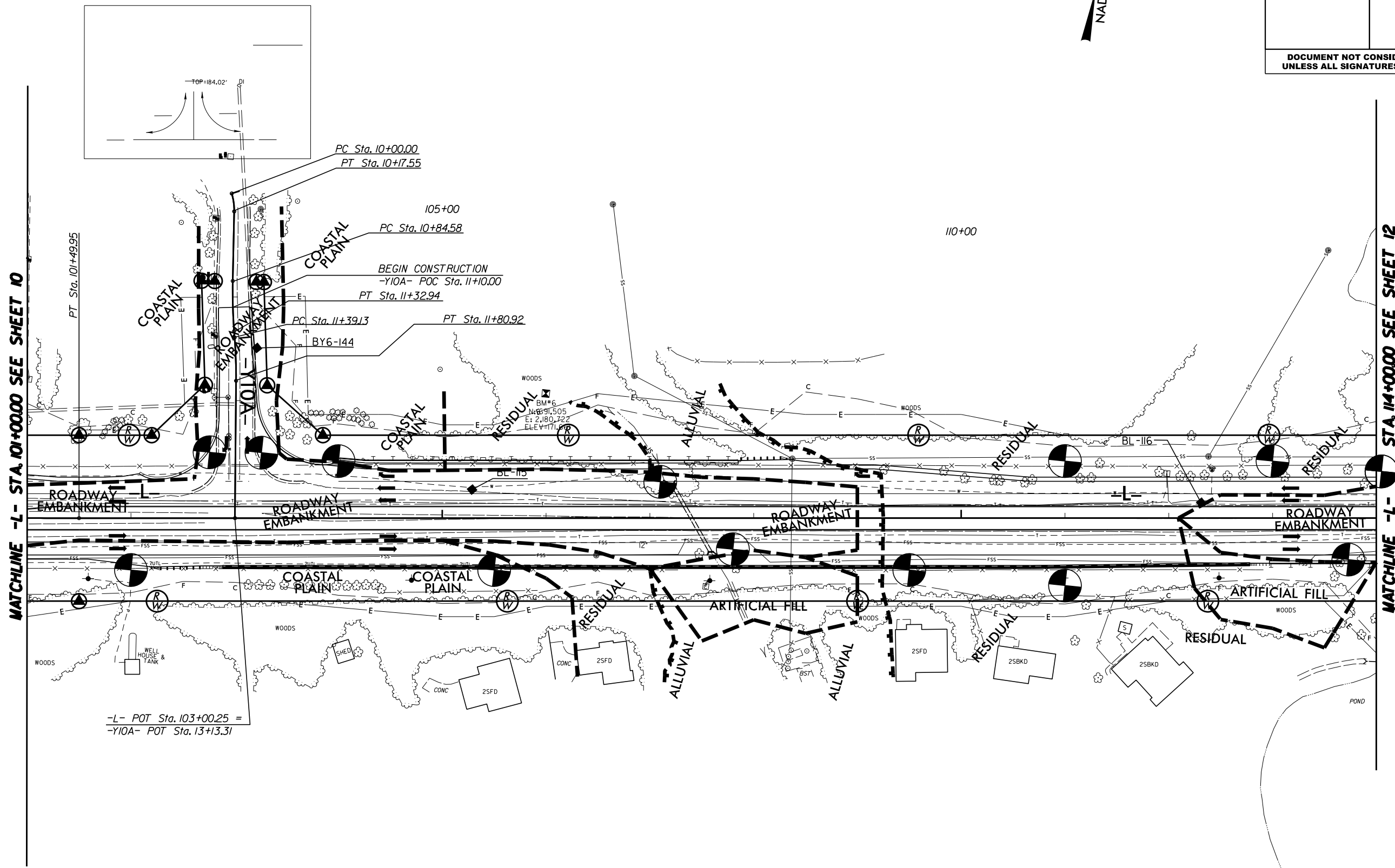


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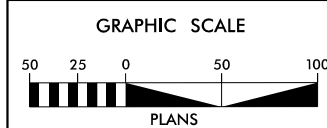
NAD 83/2011



MATCHLINE -L- STA 101+00.00 SEE SHEET 10

MATCHLINE -L- STA 114+00.00 SEE SHEET 12

-L- POT Sta. 103+00.25 =
-Y10A- POT Sta. 13+13.31



REVISIONS

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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

-Y10C- BROOKHAVEN DR

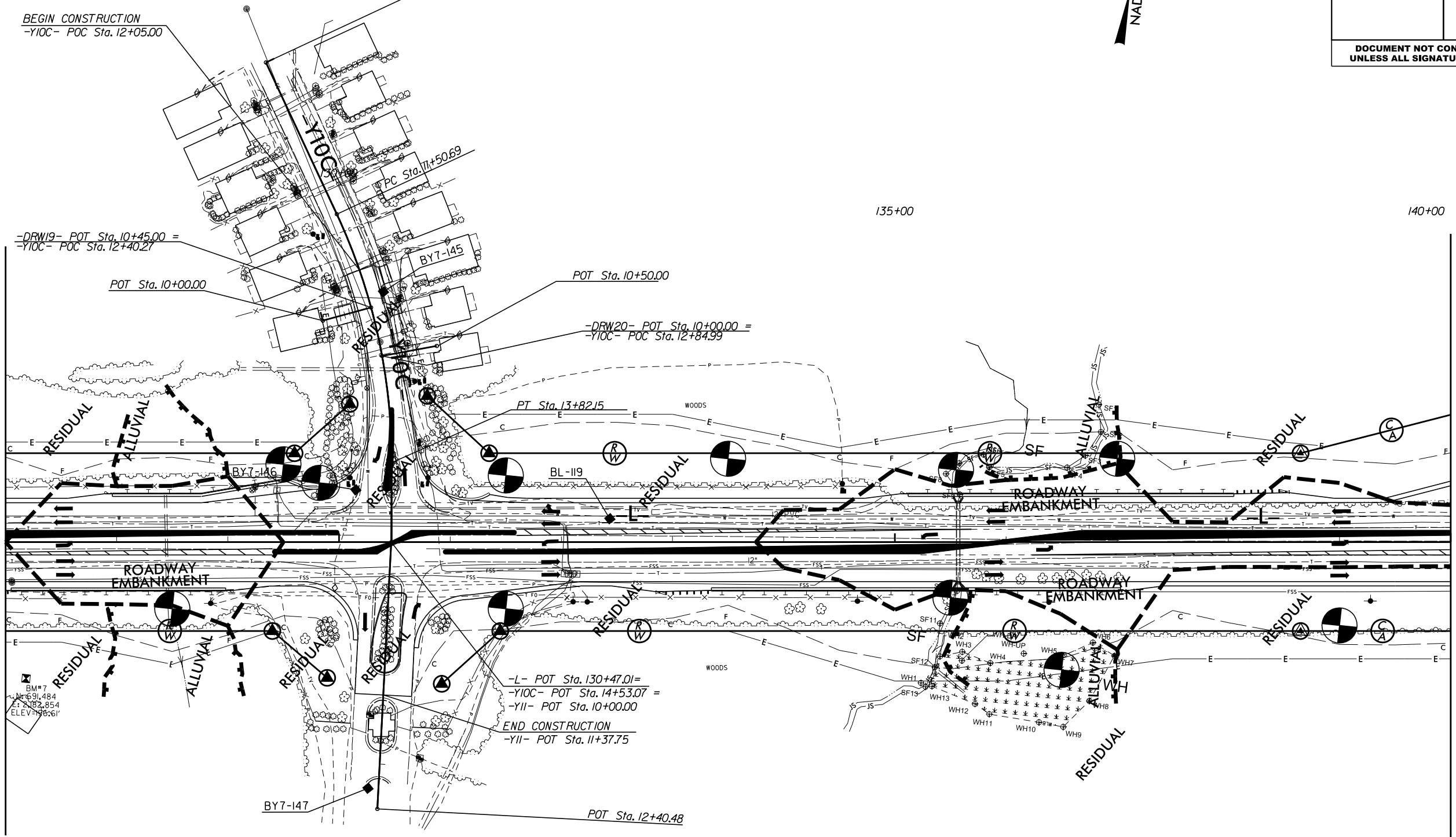
PI Sta 12+68.33
 $\Delta = 25' 15" 36.6" (RT)$
 $D = 10' 54" 48.5"$
 $L = 231.46'$
 $T = 117.64'$
 $R = 525.00'$



BEGIN CONSTRUCTION
 -Y10C- POC Sta. 12+05.00

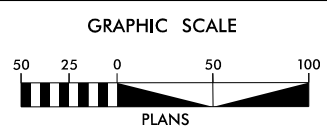
MATCHLINE -L- STA. 127+00.00 SEE SHEET 12

MATCHLINE -L- STA. 140+00.00 SEE SHEET 14



BM# 7
 N=691.484
 E=2182.854
 ELEV=98.61'

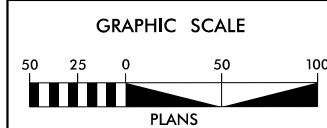
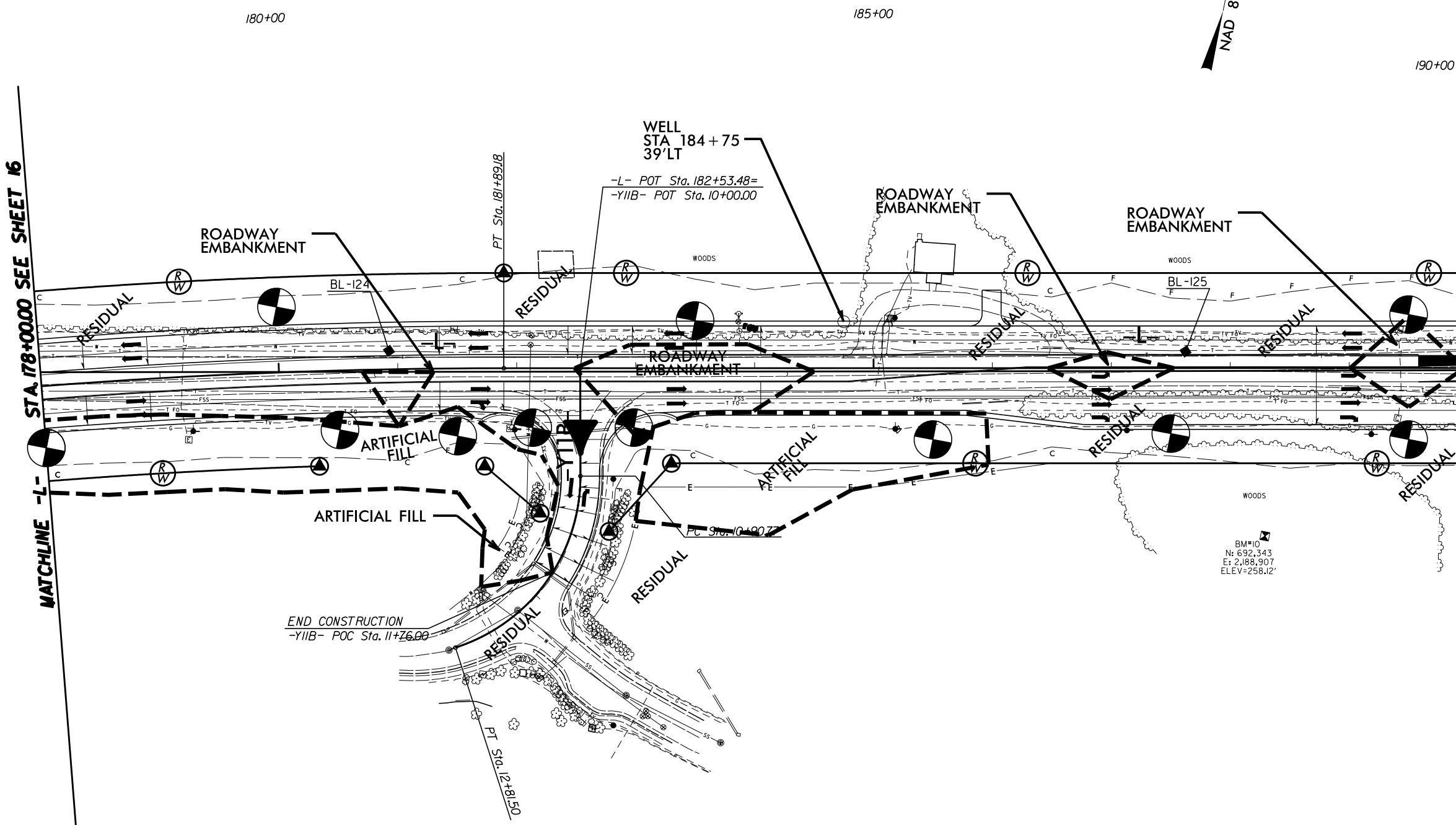
-L- POT Sta. 130+47.01=
 -Y10C- POT Sta. 14+53.07 =
 -Y11- POT Sta. 10+00.00
 END CONSTRUCTION
 -Y11- POT Sta. 11+37.75



REVISIONS

2/23/2011 8:17:99 2:\kx\rcd01\vr-3825b\vr-3825b-r-3825b-ged_rdw-mof1.e\cadd_ged\tech\plan\vr-3825b-inv_013.dgn

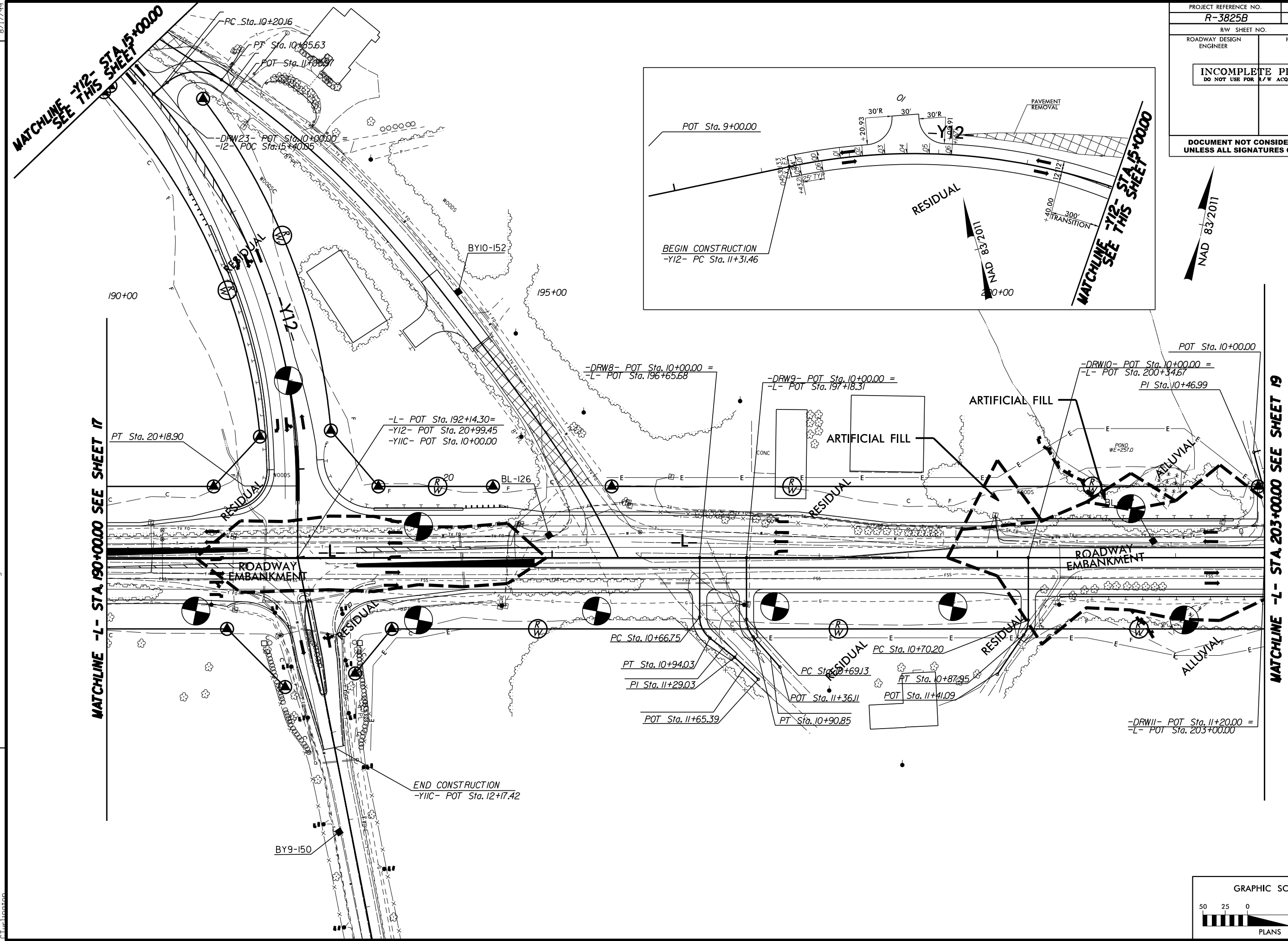
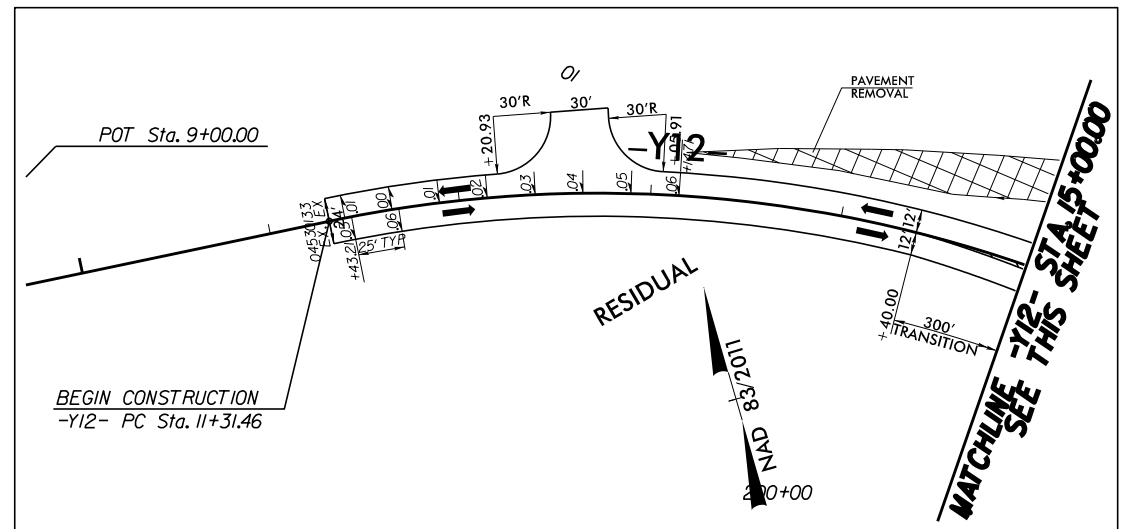
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RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



REVISIONS

8/17/09
 8/31/2011
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 C:\CADD\GEO\TECH\Plan\Prj\R3825B\Prj\RDWY_mod1.dgn

PROJECT REFERENCE NO. R-3825B	SHEET NO. 18
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



MATCHLINE -Y12- STA.15+00.00
SEE THIS SHEET

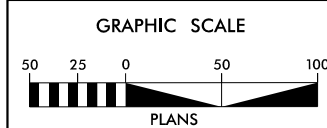
MATCHLINE -Y12- STA.15+00.00
SEE THIS SHEET

MATCHLINE -L- STA.190+00.00 SEE SHEET 17

MATCHLINE -L- STA.203+00.00 SEE SHEET 19

END CONSTRUCTION
-Y11C- POT Sta. 12+17.42

BEGIN CONSTRUCTION
-Y12- PC Sta. 11+31.46



REVISIONS

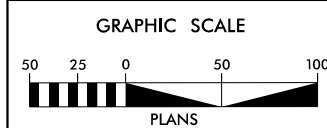
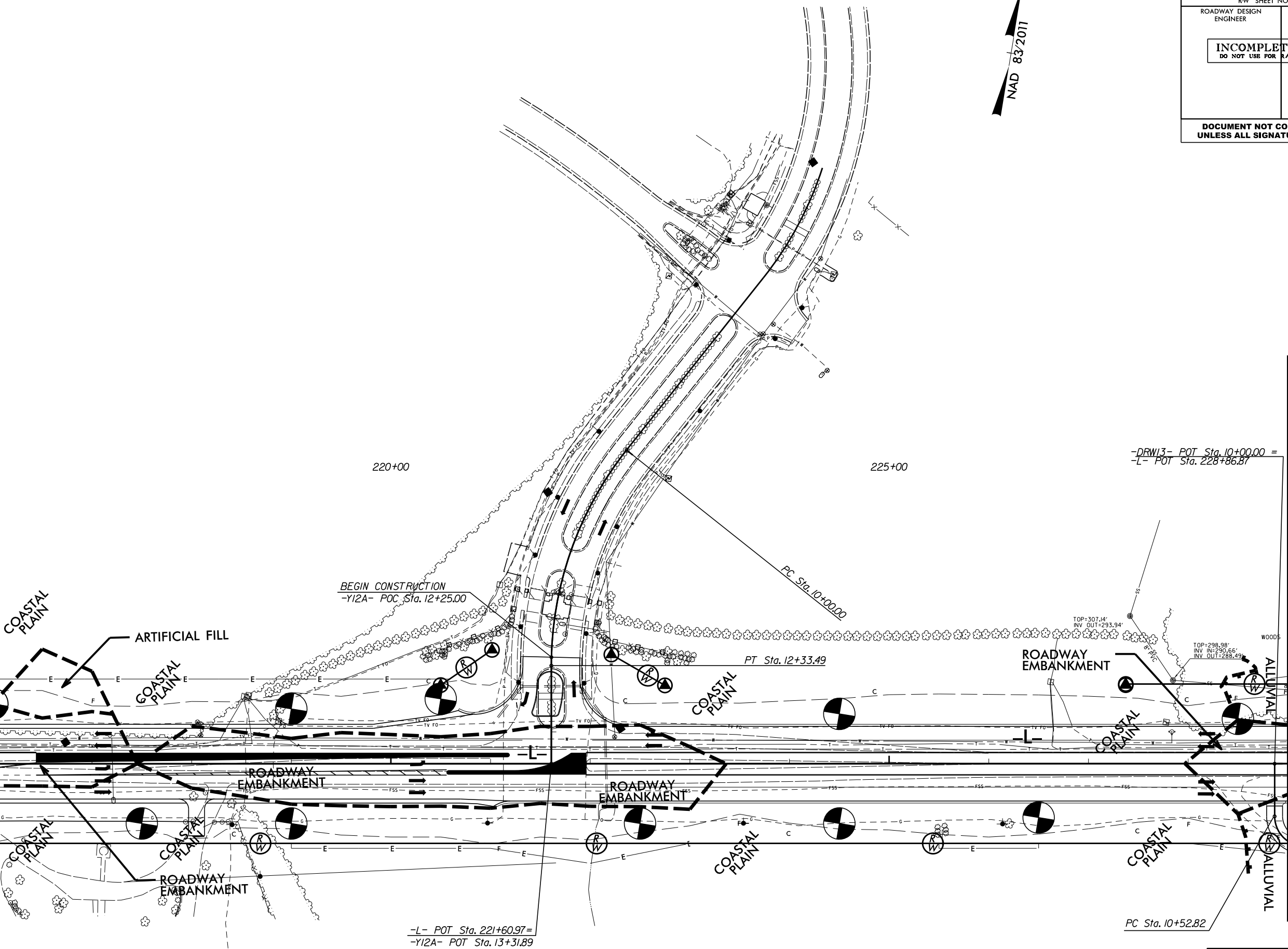
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R-3825B\PR-3825B.GEO_RDWY_modif.dgn

PROJECT REFERENCE NO. R-3825B	SHEET NO. 20
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

NAD 83/2011

MATCHLINE -L- STA. 26+00.00 SEE SHEET 19

MATCHLINE -L- STA. 229+00.00 SEE SHEET 21

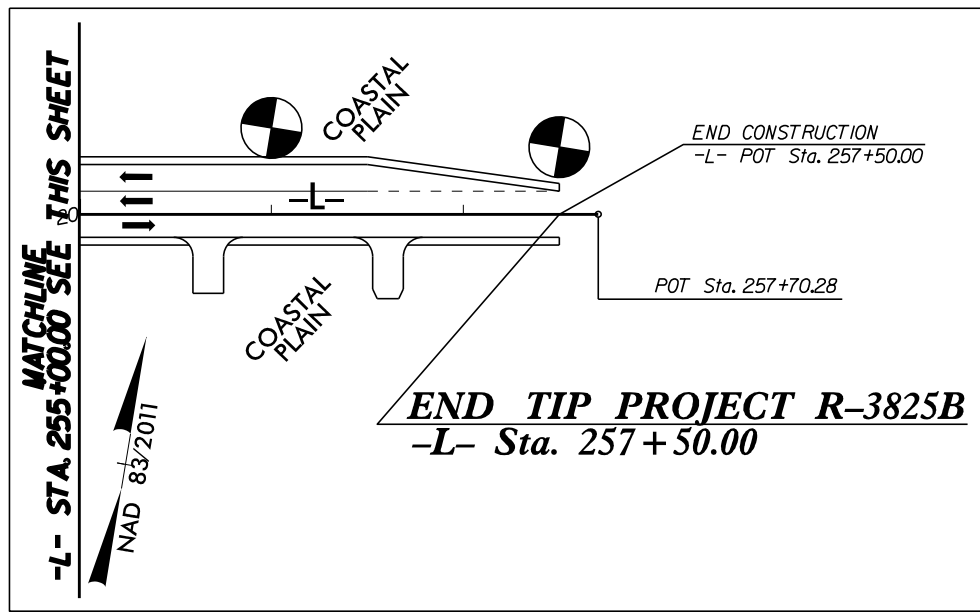
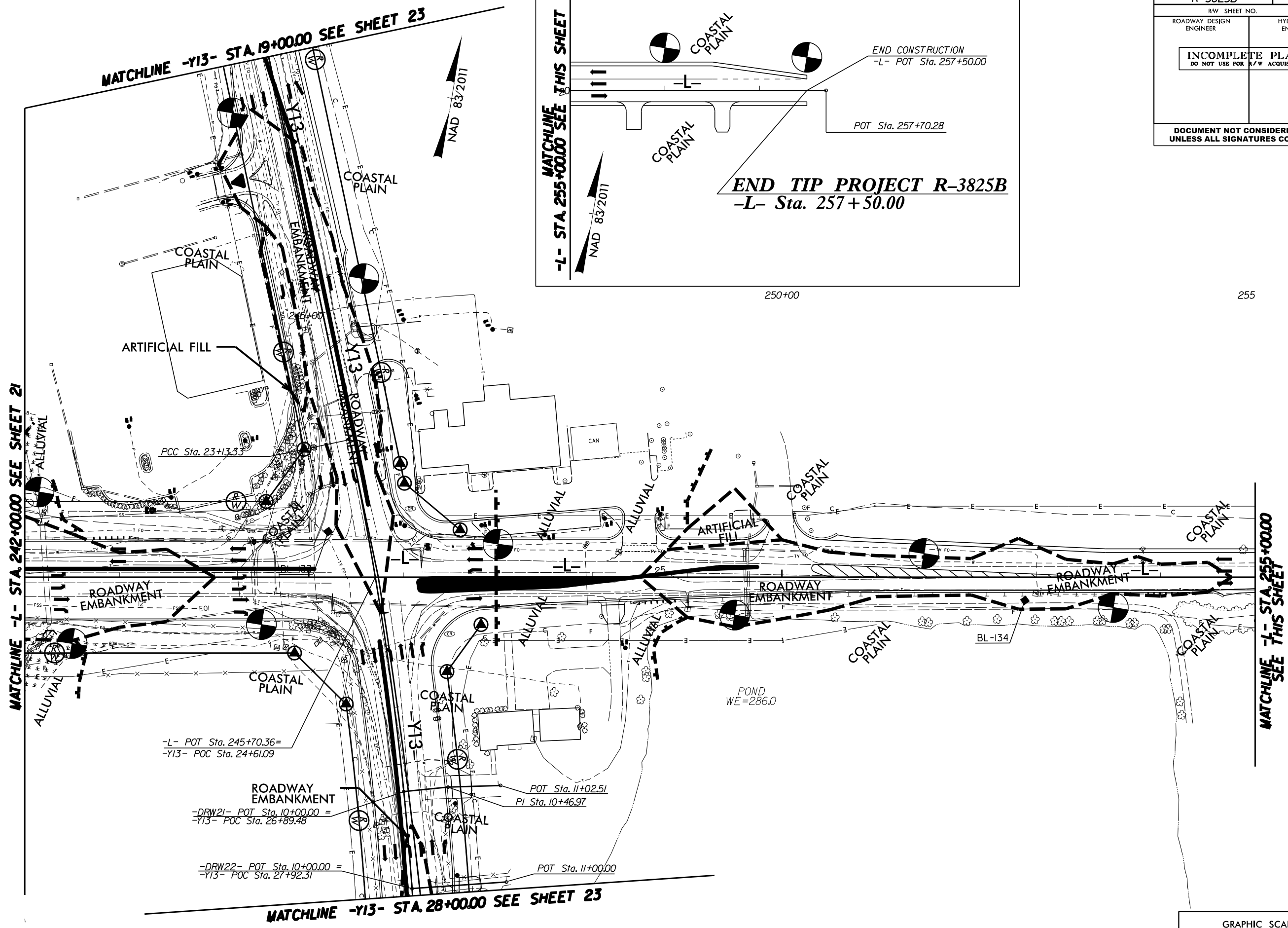


REVISIONS

8/17/09
8/31/2011
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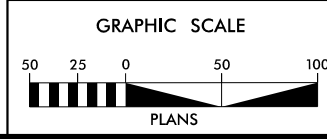
PROJECT REFERENCE NO. R-3825B	SHEET NO. 22
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

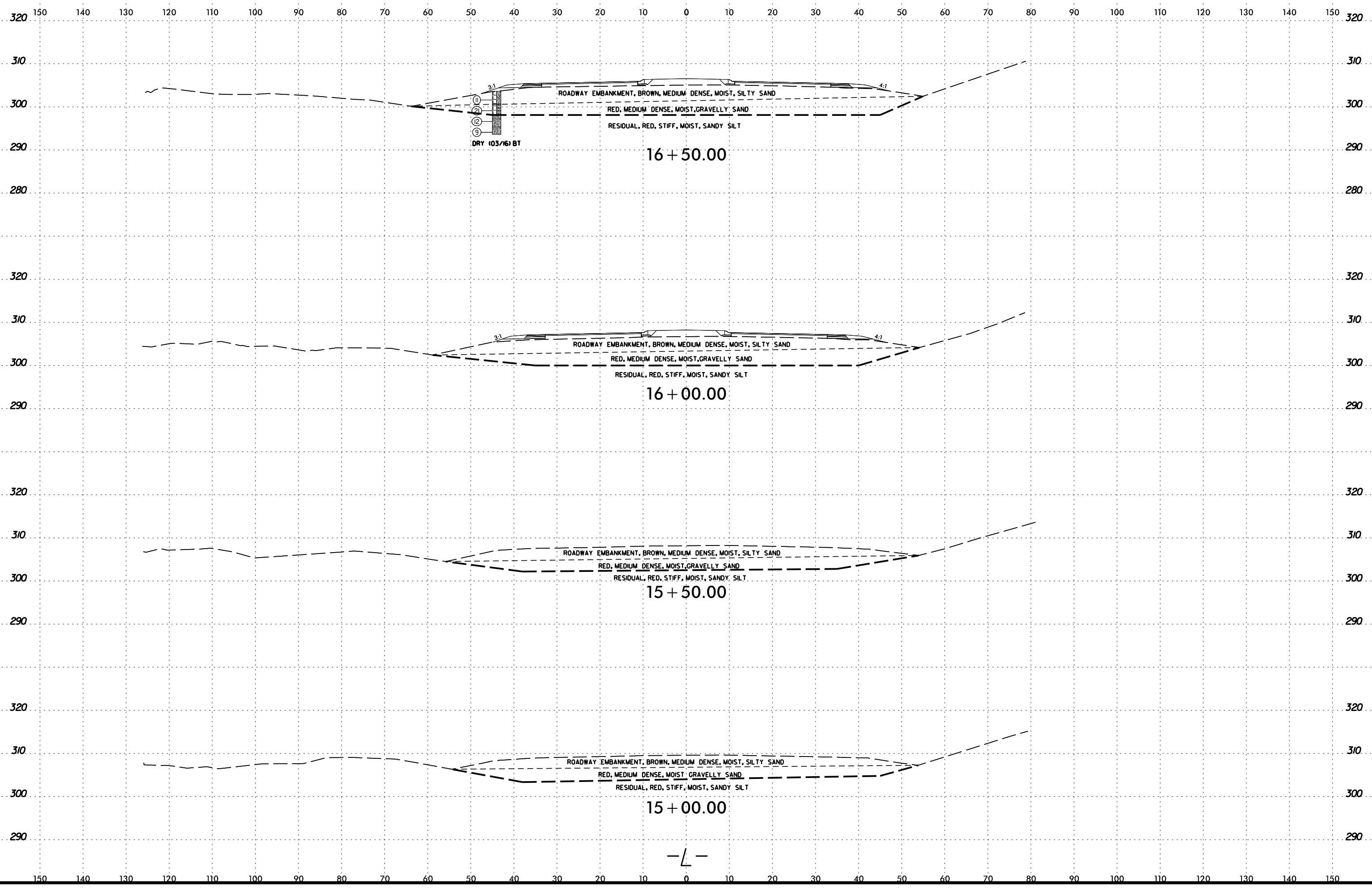
8/17/09
 REVISIONS
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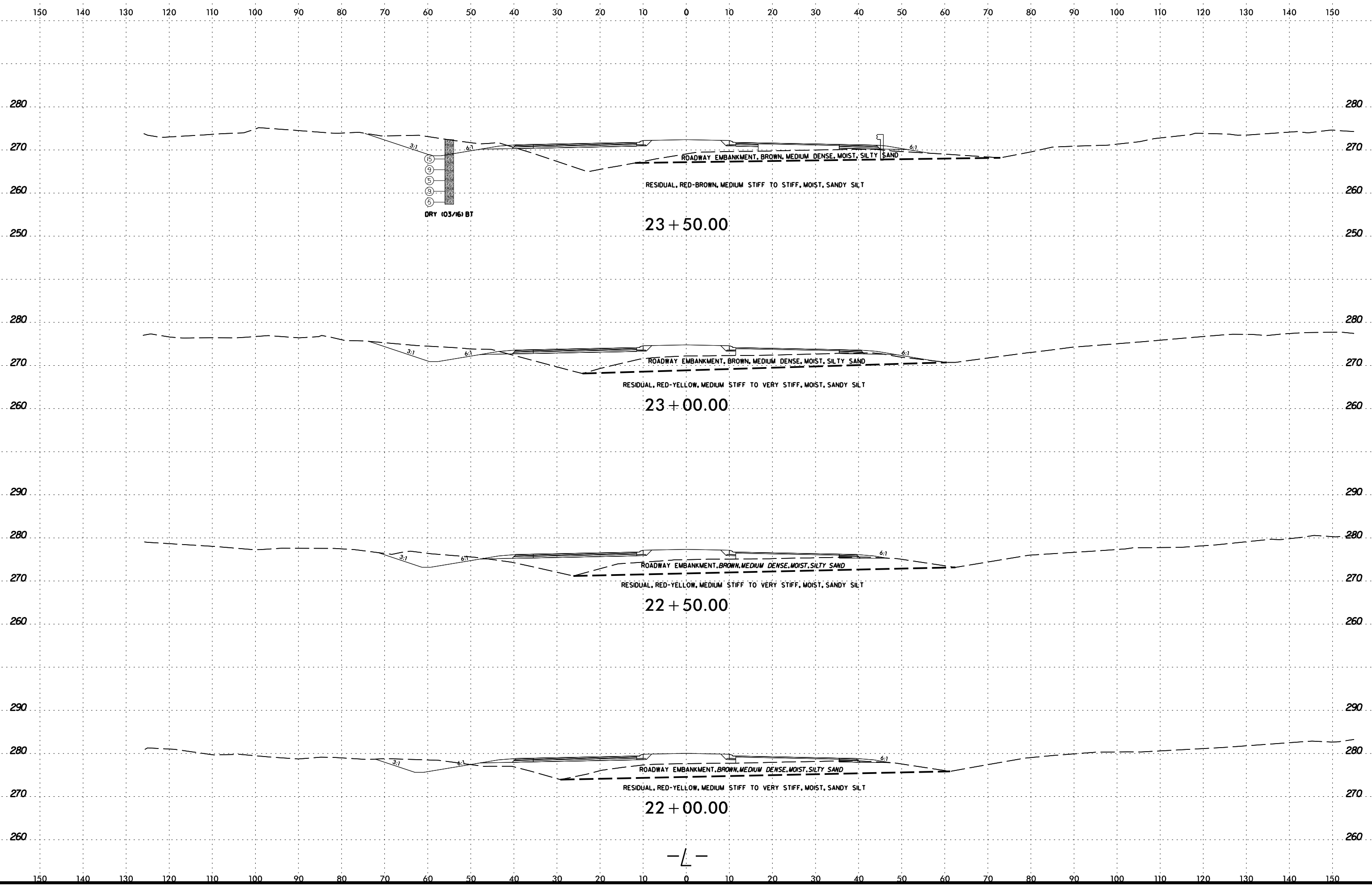


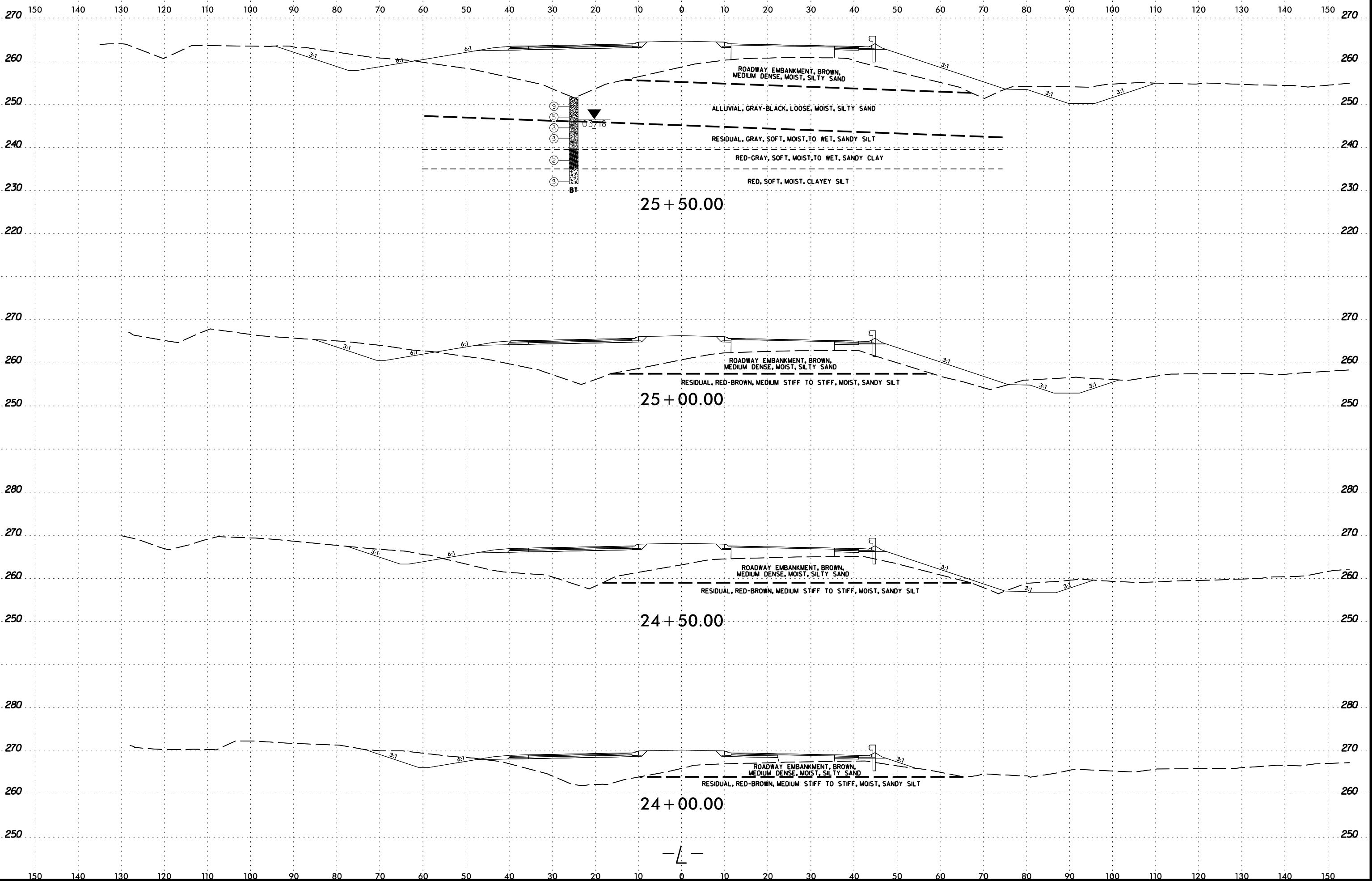
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MATCHLINE -L- STA. 255+00.00 SEE THIS SHEET

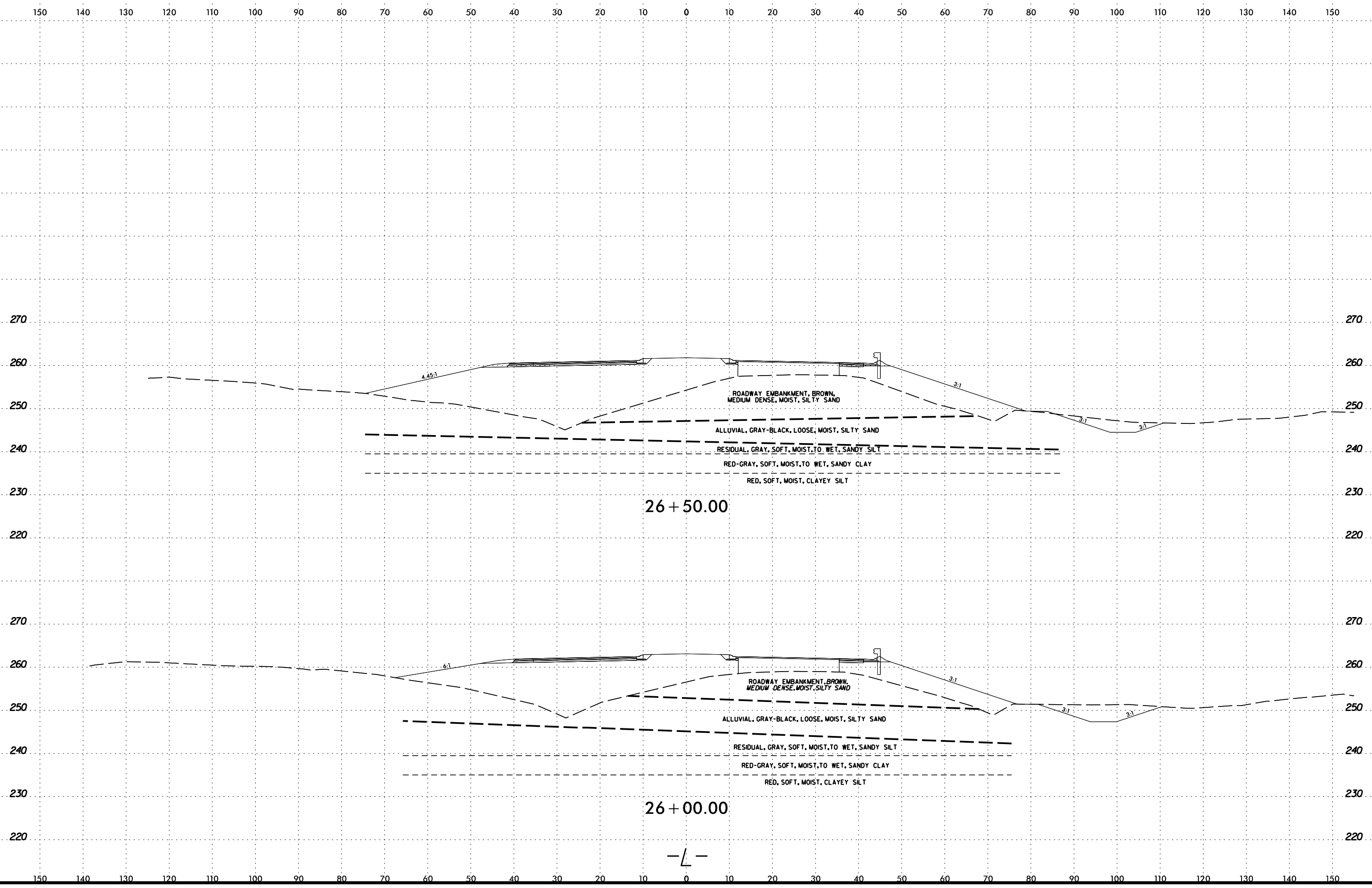






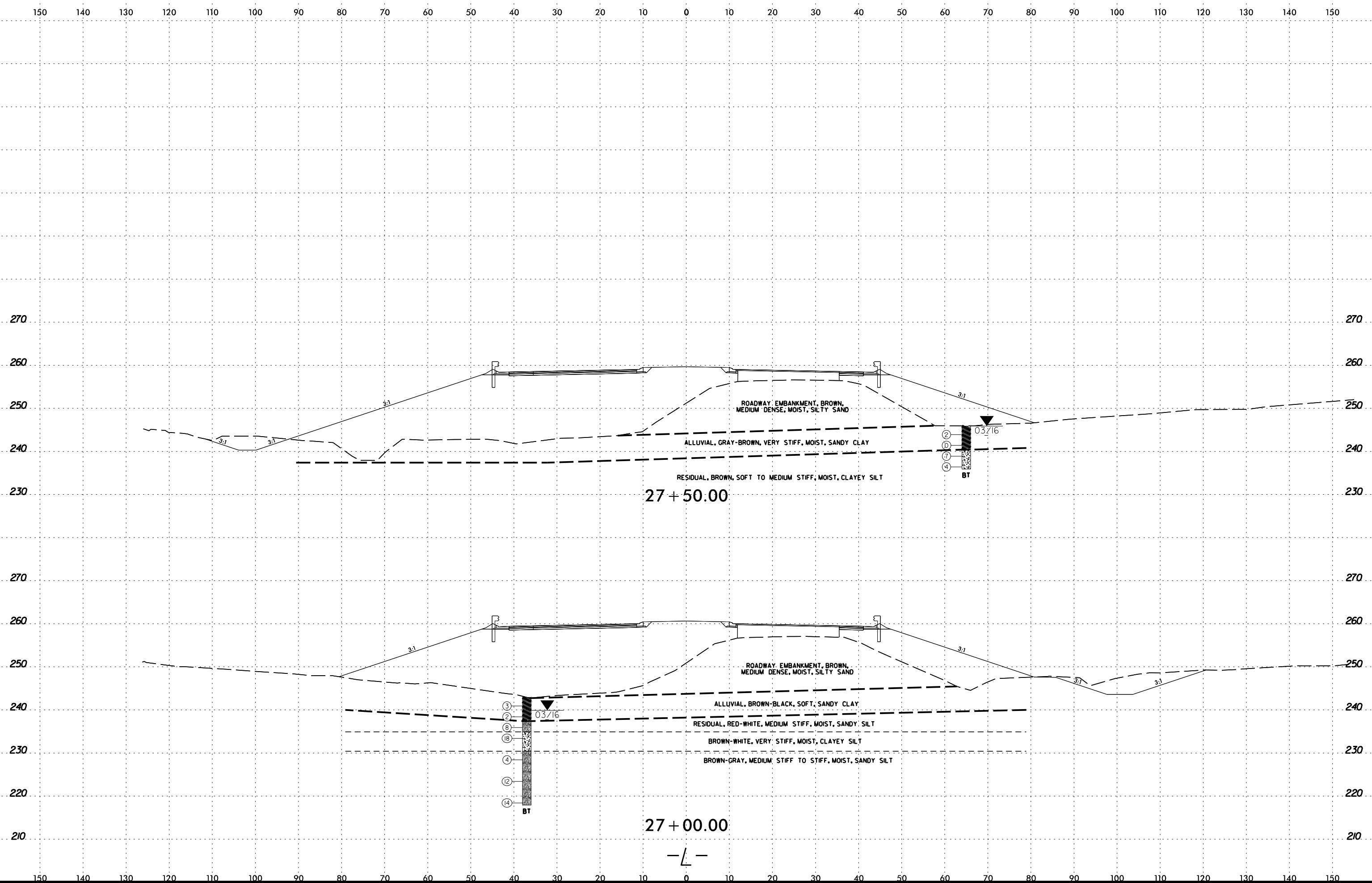


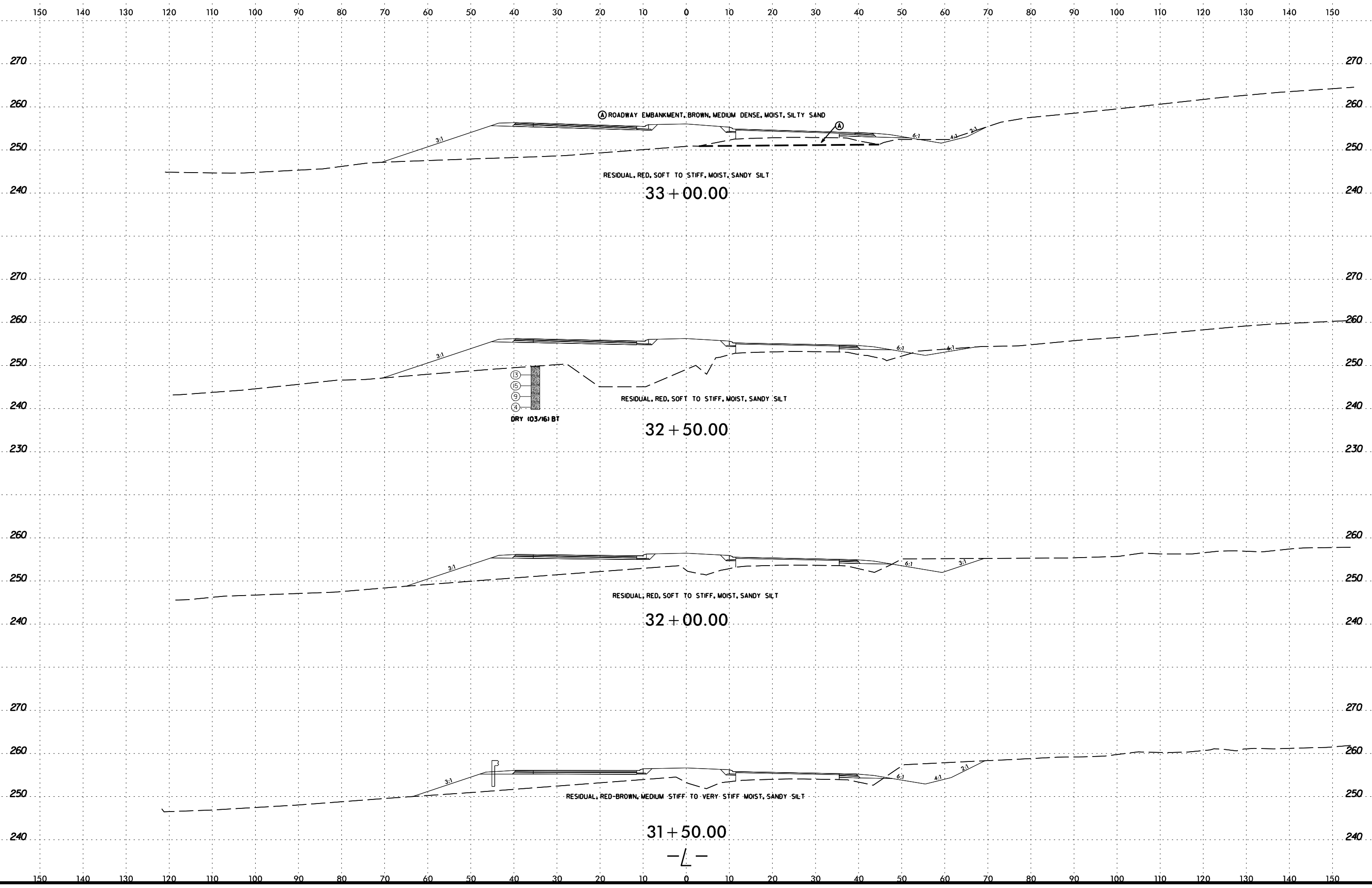
6/23/16



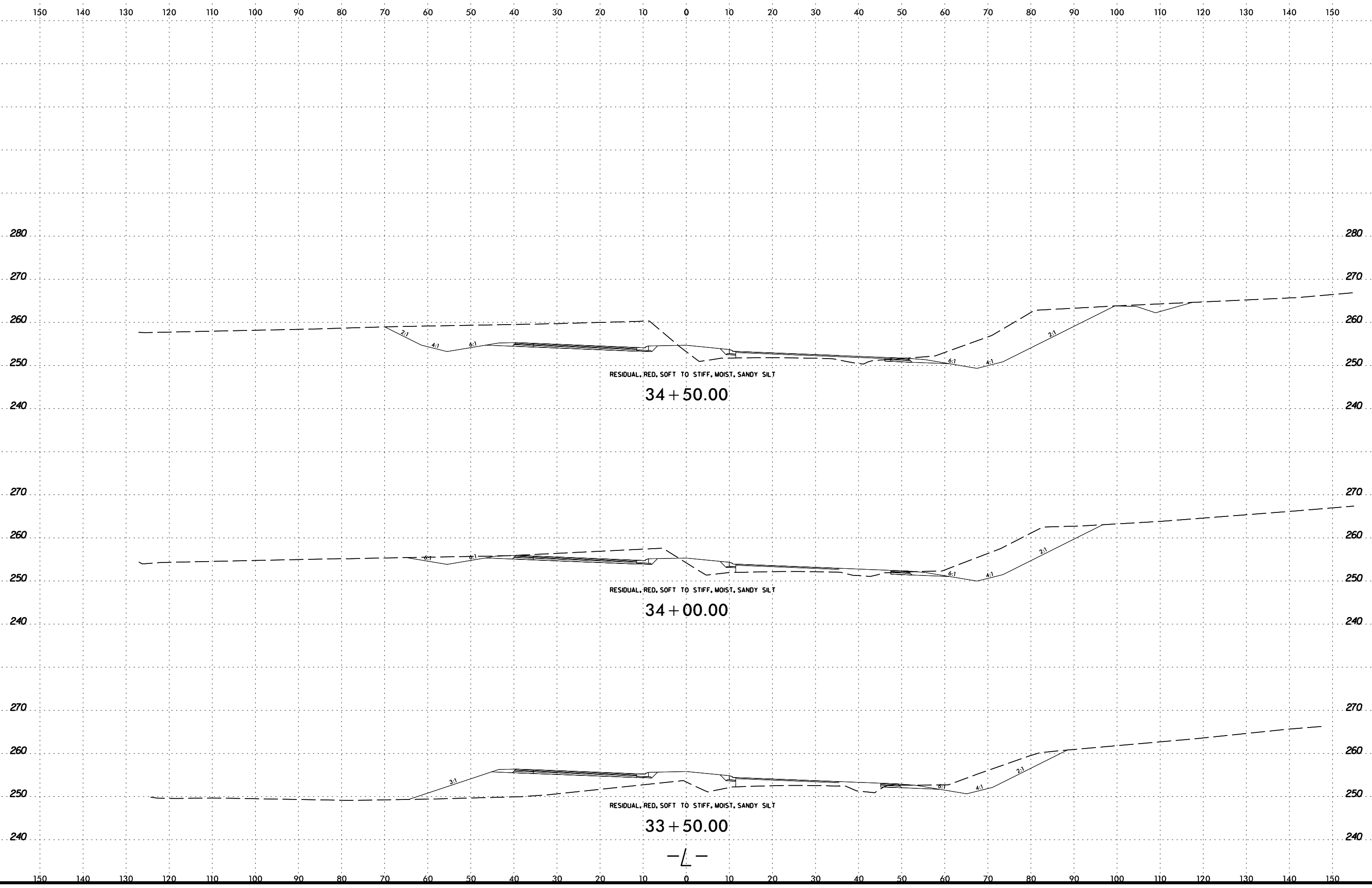
2/23/2017
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6/23/16
2/23/2017
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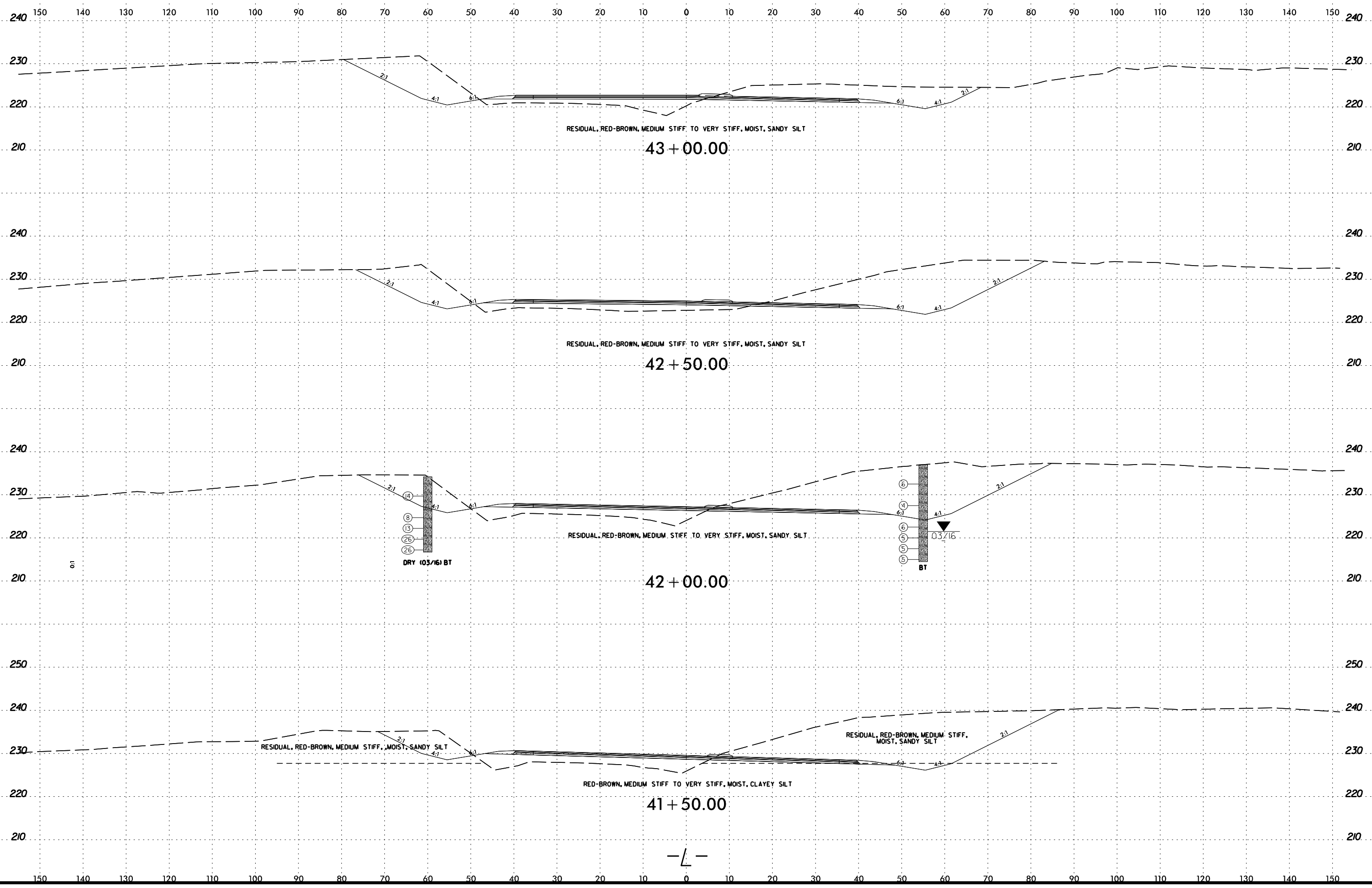


6/23/16



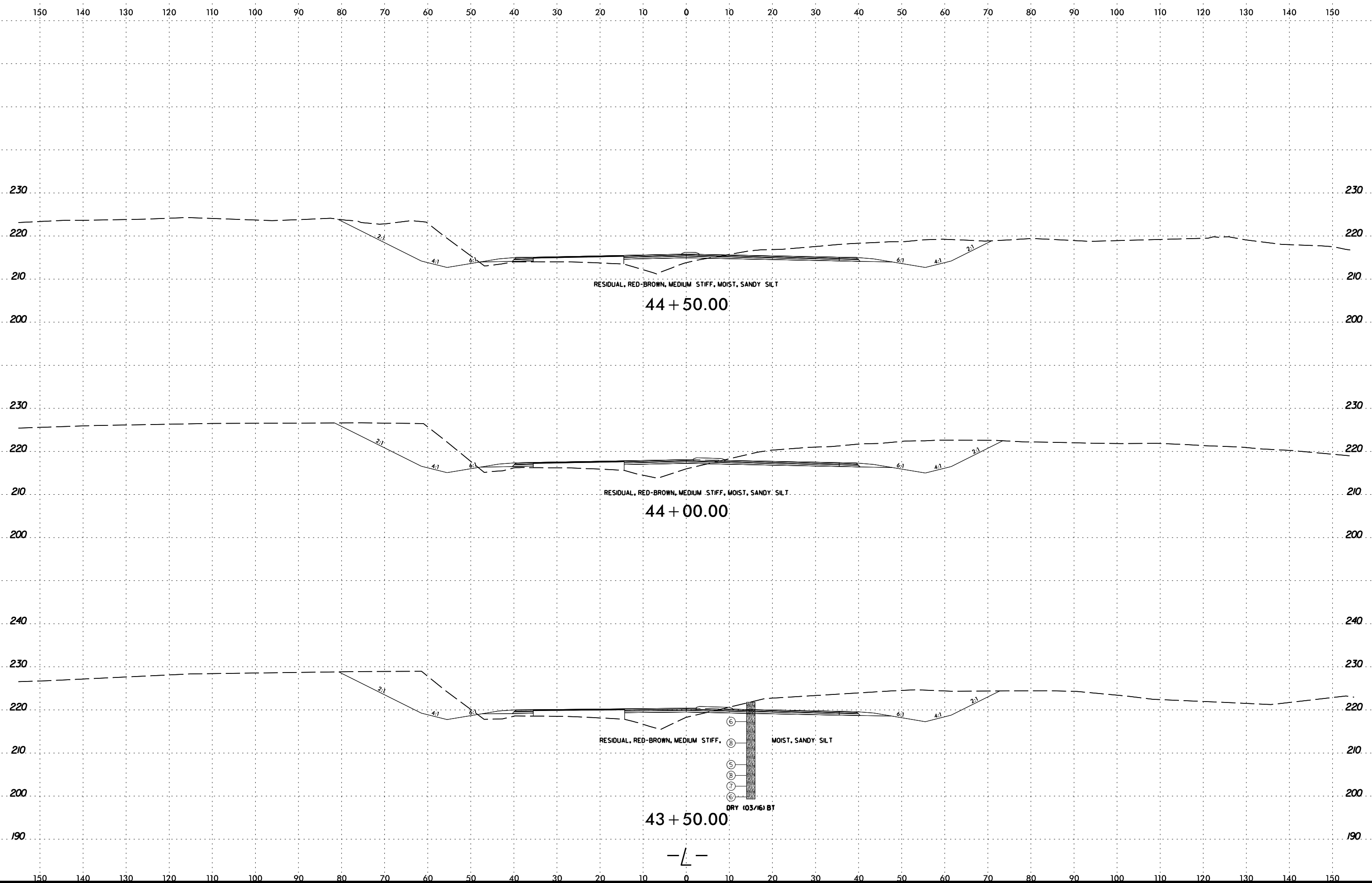
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2/23/2017
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44 + 50.00

44 + 00.00

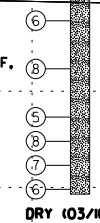
43 + 50.00

RESIDUAL, RED-BROWN, MEDIUM STIFF, MOIST, SANDY SILT

RESIDUAL, RED-BROWN, MEDIUM STIFF, MOIST, SANDY SILT

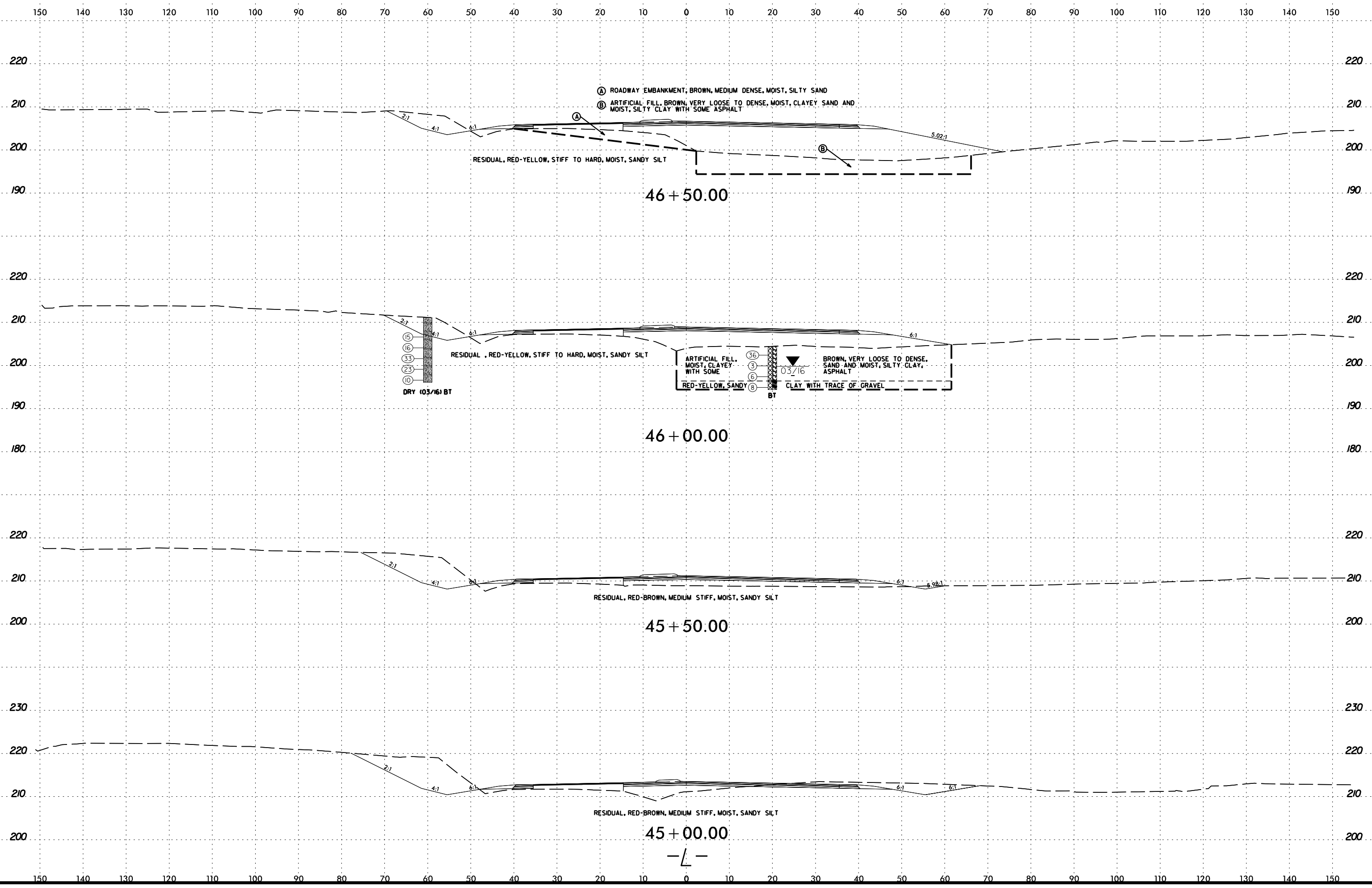
RESIDUAL, RED-BROWN, MEDIUM STIFF.

MOIST, SANDY SILT



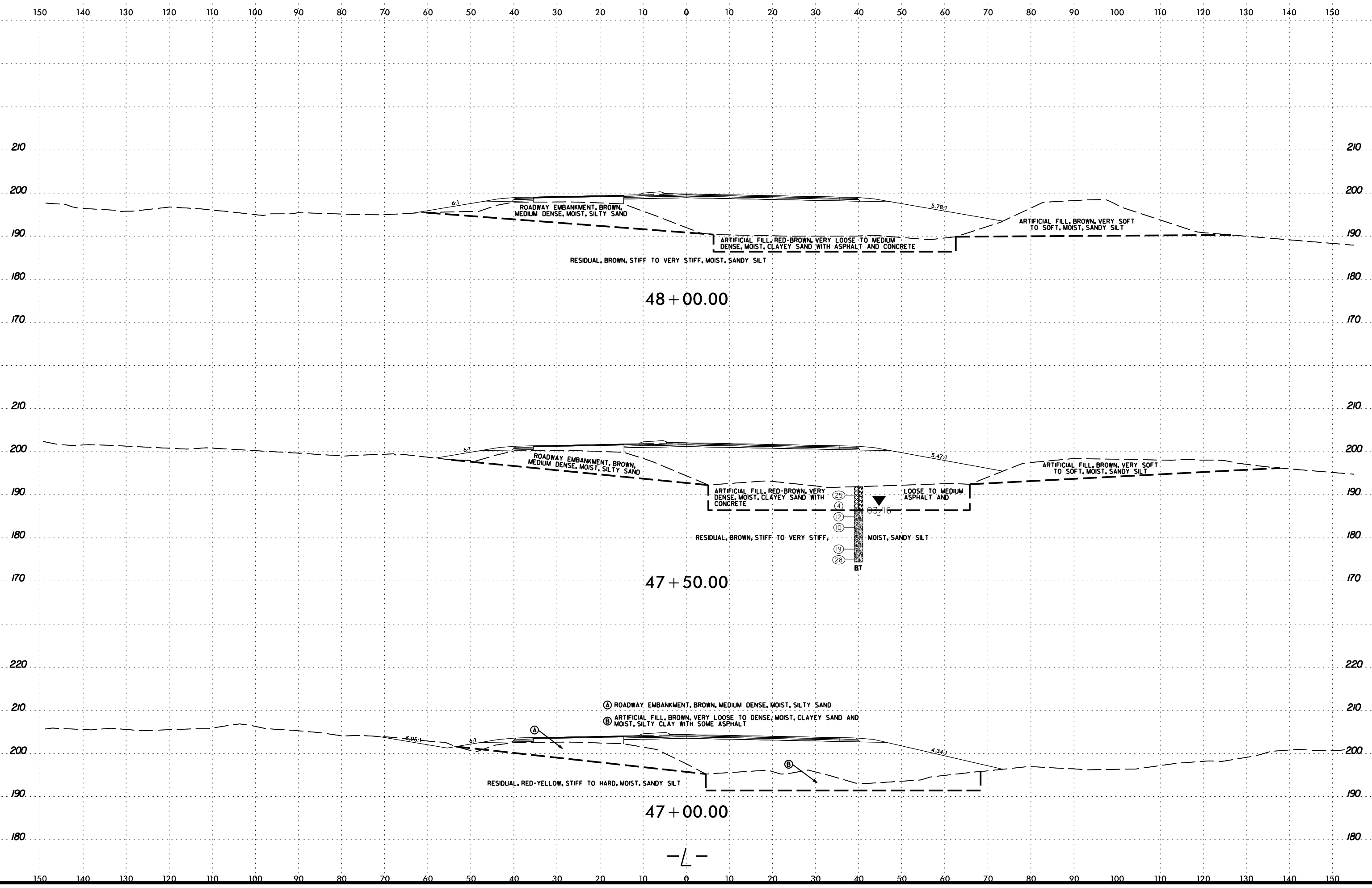
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6/23/16



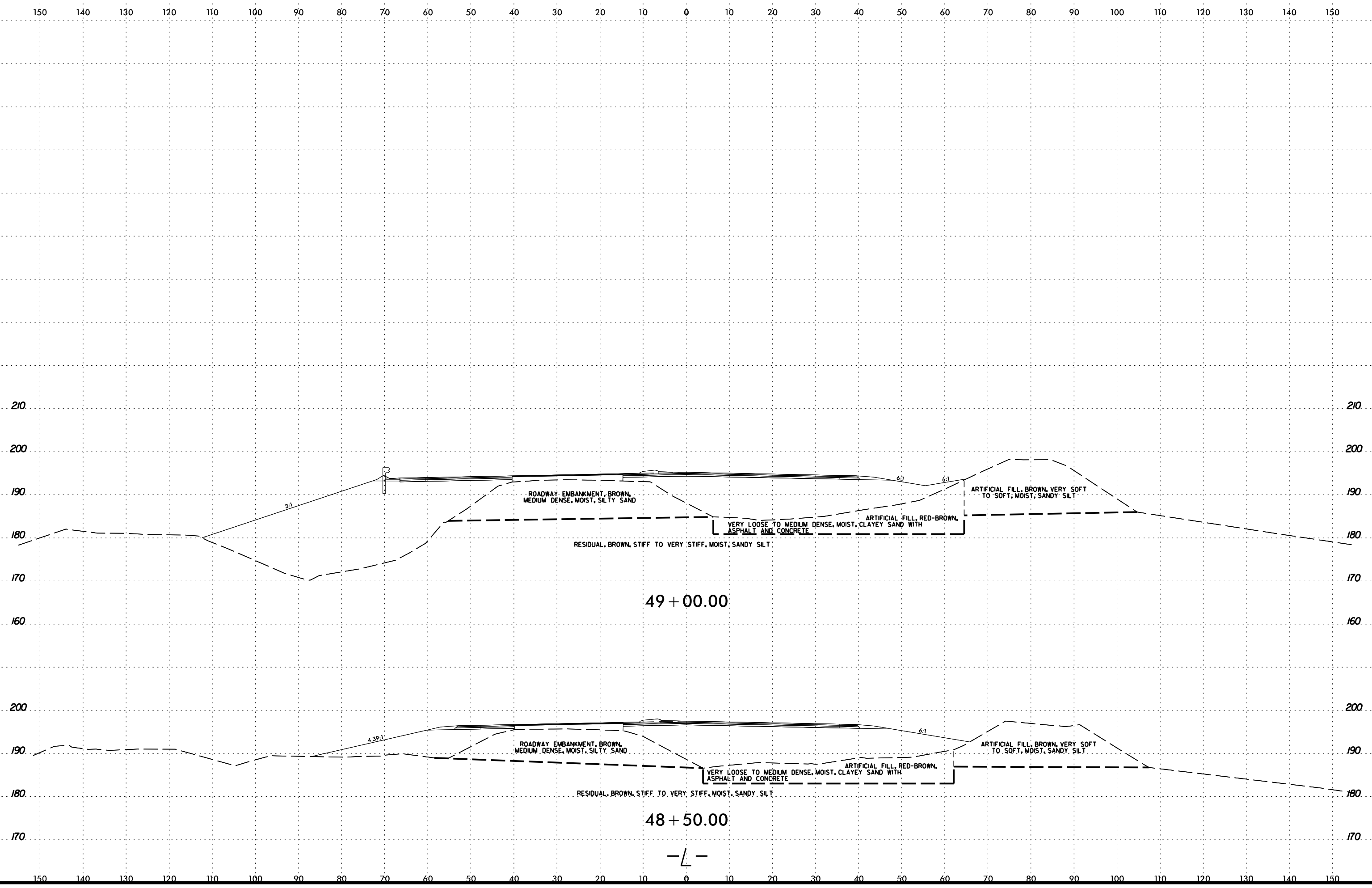
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6/23/16

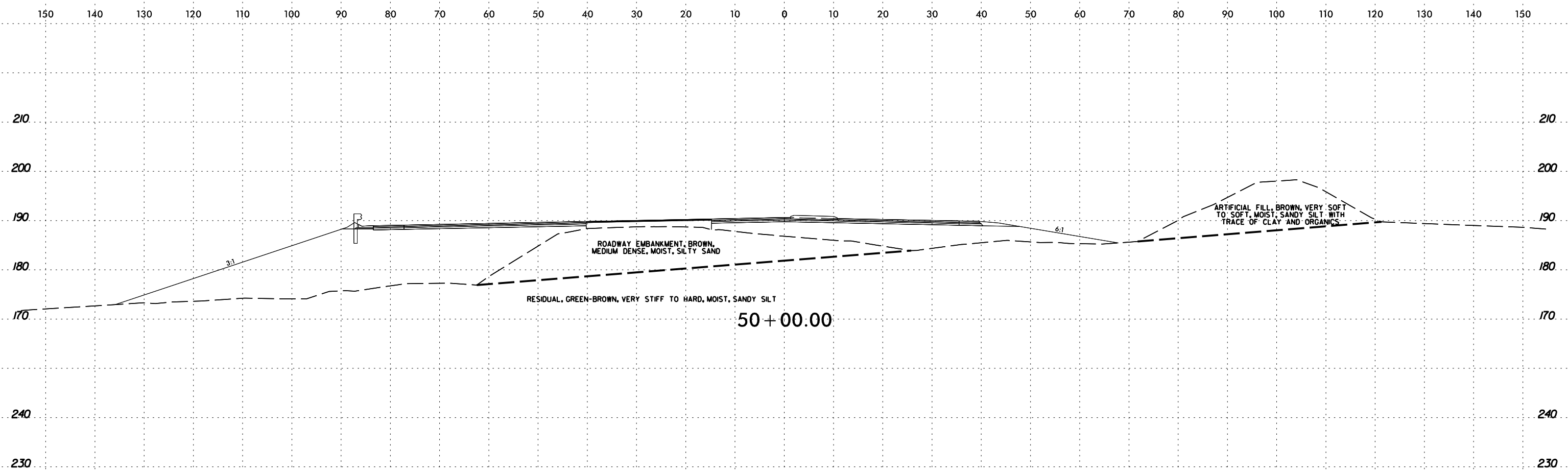


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6/23/16

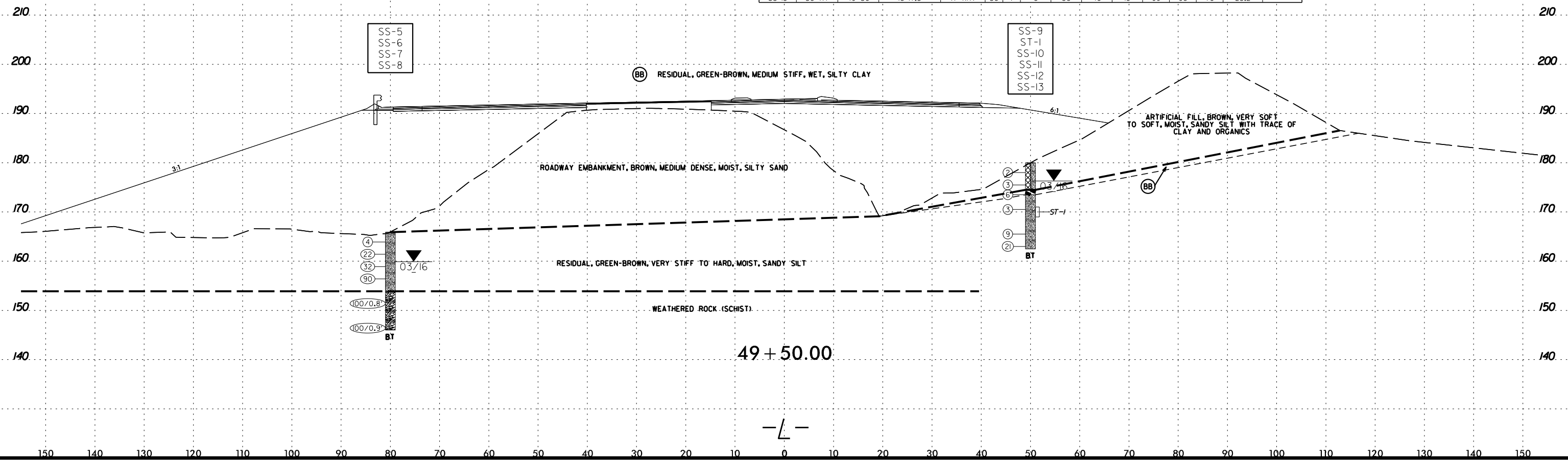


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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-5	80' LT	49+50	1.0-2.5	A-4(2)	32	NP	10	45	28	10	99	94	56	18.9	-
SS-6	80' LT	49+50	3.5-4.5	A-4(3)	37	5	11	43	39	7	96	89	57	12.7	-
SS-7	80' LT	49+50	6.0-7.5	A-4(0)	31	NP	2	48	44	6	100	99	68	10	-
SS-8	80' LT	49+50	8.5-10.0	A-4(0)	32	NP	4	51	41	4	100	98	62	6.1	-

SOIL TEST RESULTS																
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC	
							C.SAND	F.SAND	SILT	CLAY	10	40	200			
SS-9	50' RT	49+50	6.0-6.5	A-7-6(18)	45	21	-	4	24	35	37	94	92	76	26.6	-
ST-1	50' RT	49+50	9.0-11.0	-	-	-	-	-	-	-	-	-	-	-	-	
SS-10	50' RT	49+50	8.5-10.0	A-4(2)	33	9	6	36	32	26	94	91	66	26.8	-	
SS-11	50' RT	49+50	13.5-14.4	A-4(0)	31	1	6	37	41	16	80	77	56	33.5	-	
SS-12	50' RT	49+50	14.4-15.0	A-5(13)	42	6	15	44	31	10	99	89	52	24.0	-	
SS-13	50' RT	49+50	16-17.5	A-4(7)	36	7	6	36	40	18	99	96	70	25.2	-	

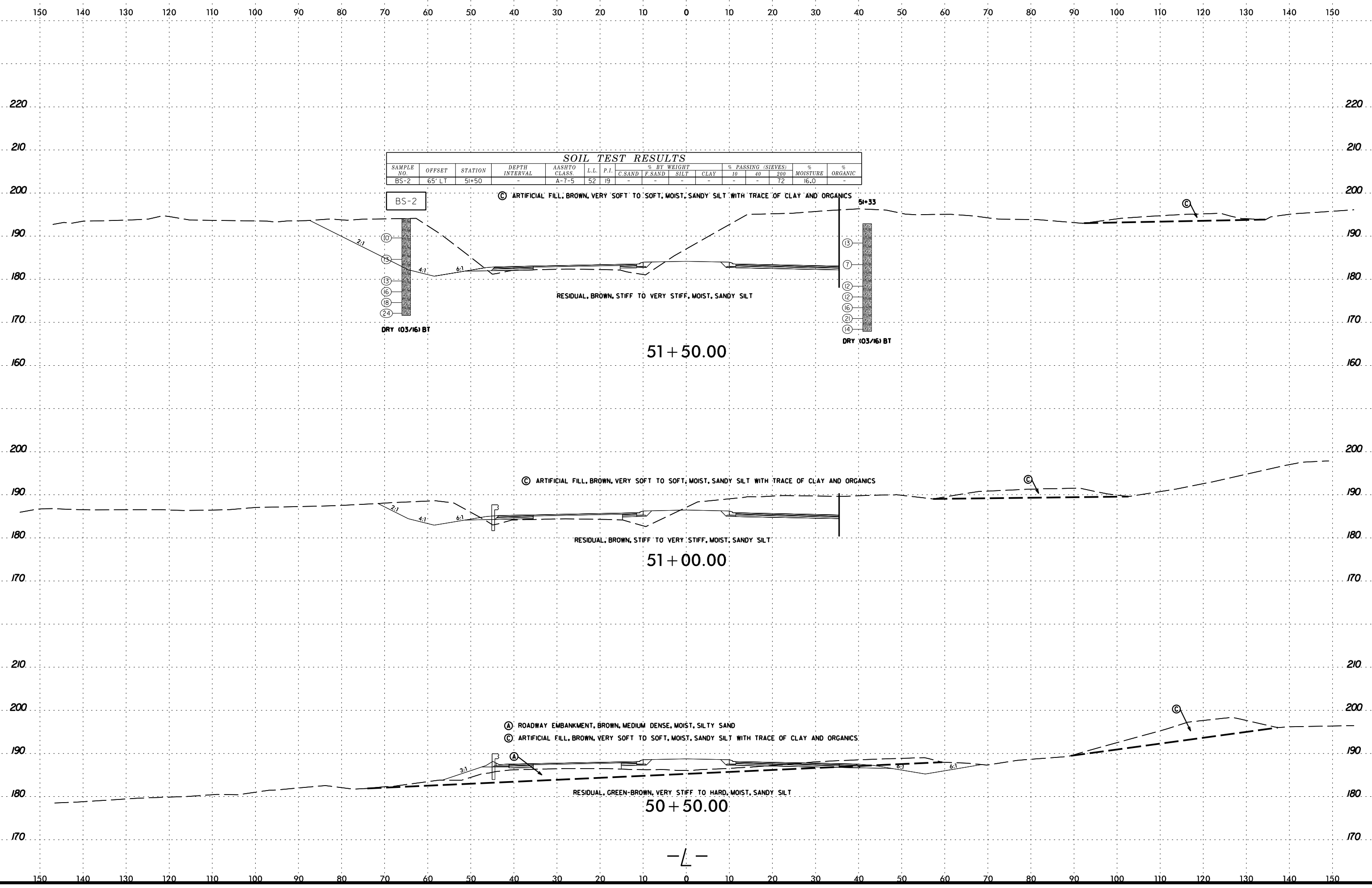


SS-5
SS-6
SS-7
SS-8

SS-9
ST-1
SS-10
SS-11
SS-12
SS-13

4
22
32
90
100/0.8
100/0.9
BT

2
3
6
9
21
BT



SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT			% PASSING (SIEVES)		% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10		
BS-2	65' LT	51+50	-	A-7-5	52	19	-	-	-	-	-	72	16.0

Ⓒ ARTIFICIAL FILL, BROWN, VERY SOFT TO SOFT, MOIST, SANDY SILT WITH TRACE OF CLAY AND ORGANICS

RESIDUAL, BROWN, STIFF TO VERY STIFF, MOIST, SANDY SILT

51 + 50.00

Ⓒ ARTIFICIAL FILL, BROWN, VERY SOFT TO SOFT, MOIST, SANDY SILT WITH TRACE OF CLAY AND ORGANICS

RESIDUAL, BROWN, STIFF TO VERY STIFF, MOIST, SANDY SILT

51 + 00.00

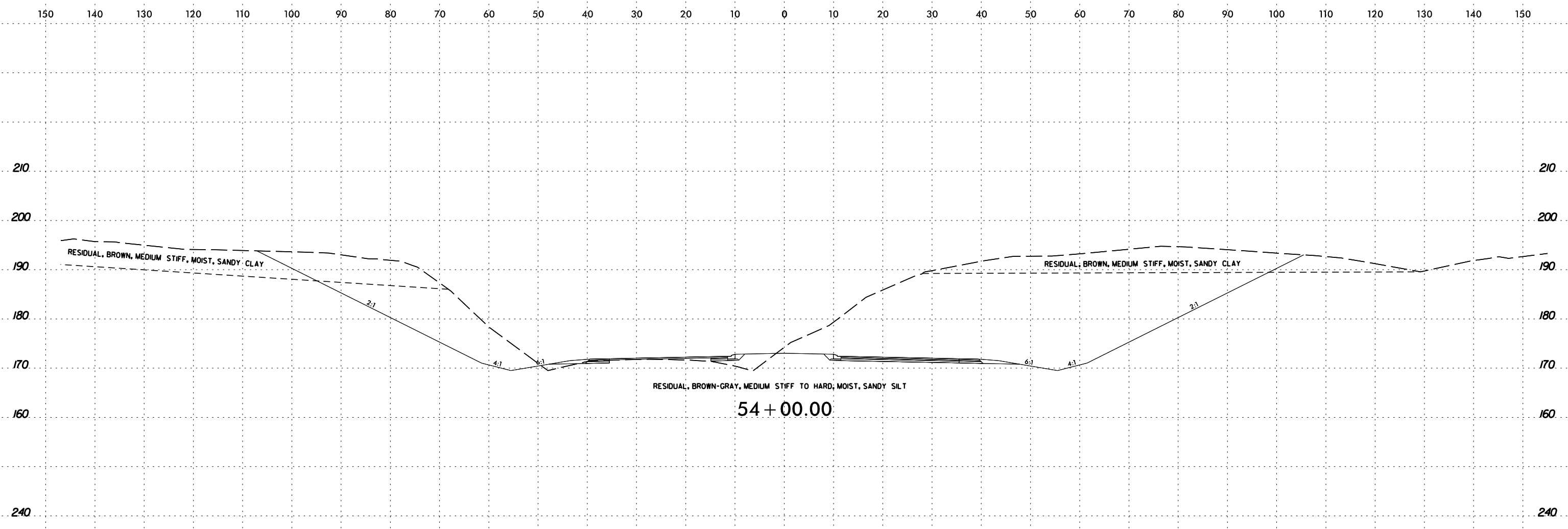
Ⓐ ROADWAY EMBANKMENT, BROWN, MEDIUM DENSE, MOIST, SILTY SAND

Ⓒ ARTIFICIAL FILL, BROWN, VERY SOFT TO SOFT, MOIST, SANDY SILT WITH TRACE OF CLAY AND ORGANICS

RESIDUAL, GREEN-BROWN, VERY STIFF TO HARD, MOIST, SANDY SILT

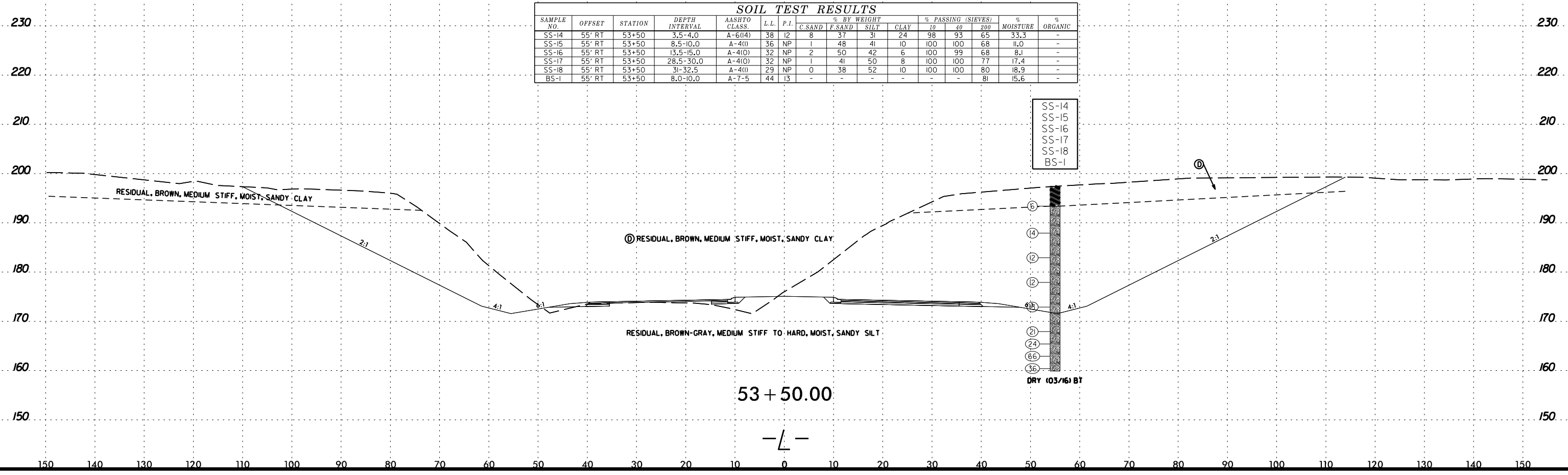
50 + 50.00

6/23/16



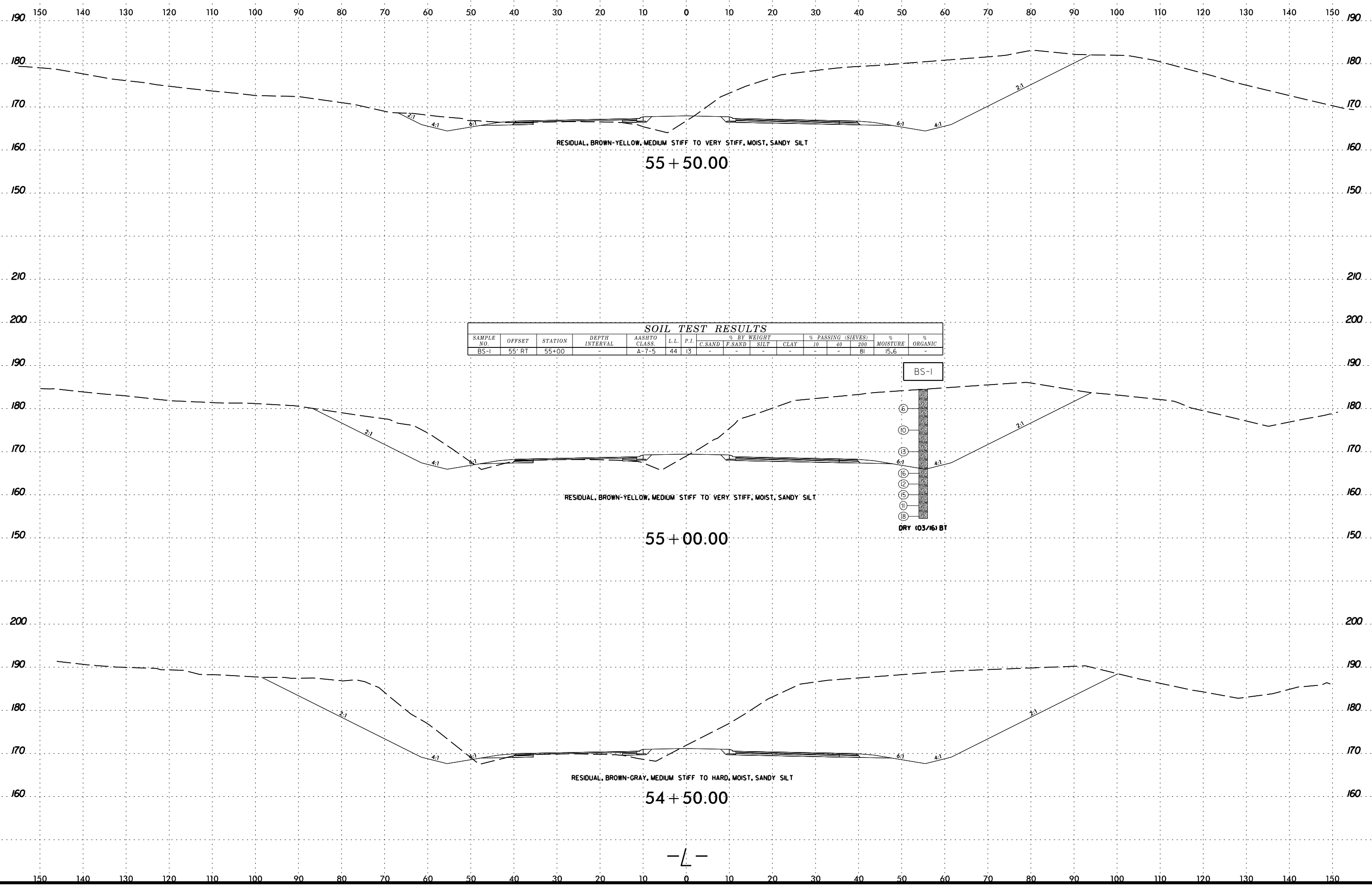
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-14	55' RT	53+50	3.5-4.0	A-6(14)	38	12	8	37	31	24	98	93	65	33.3	-
SS-15	55' RT	53+50	8.5-10.0	A-4(1)	36	NP	1	48	41	10	100	100	68	11.0	-
SS-16	55' RT	53+50	13.5-15.0	A-4(0)	32	NP	2	50	42	6	100	99	68	8.1	-
SS-17	55' RT	53+50	28.5-30.0	A-4(0)	32	NP	1	41	50	8	100	100	77	17.4	-
SS-18	55' RT	53+50	31-32.5	A-4(1)	29	NP	0	38	52	10	100	100	80	18.9	-
BS-1	55' RT	53+50	8.0-10.0	A-7-5	44	13	-	-	-	-	-	-	81	15.6	-



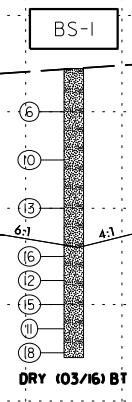
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6/23/16

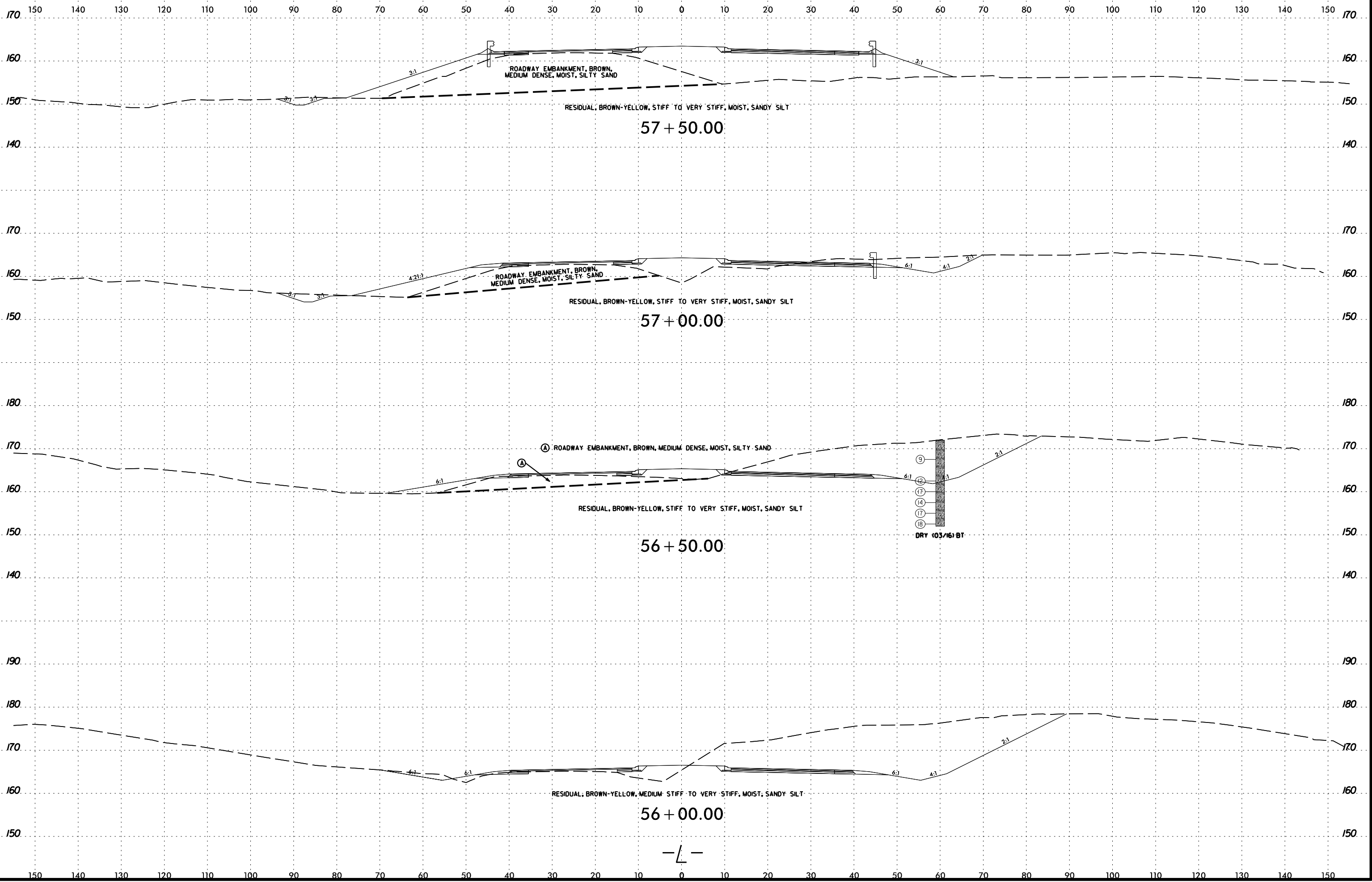


SOIL TEST RESULTS

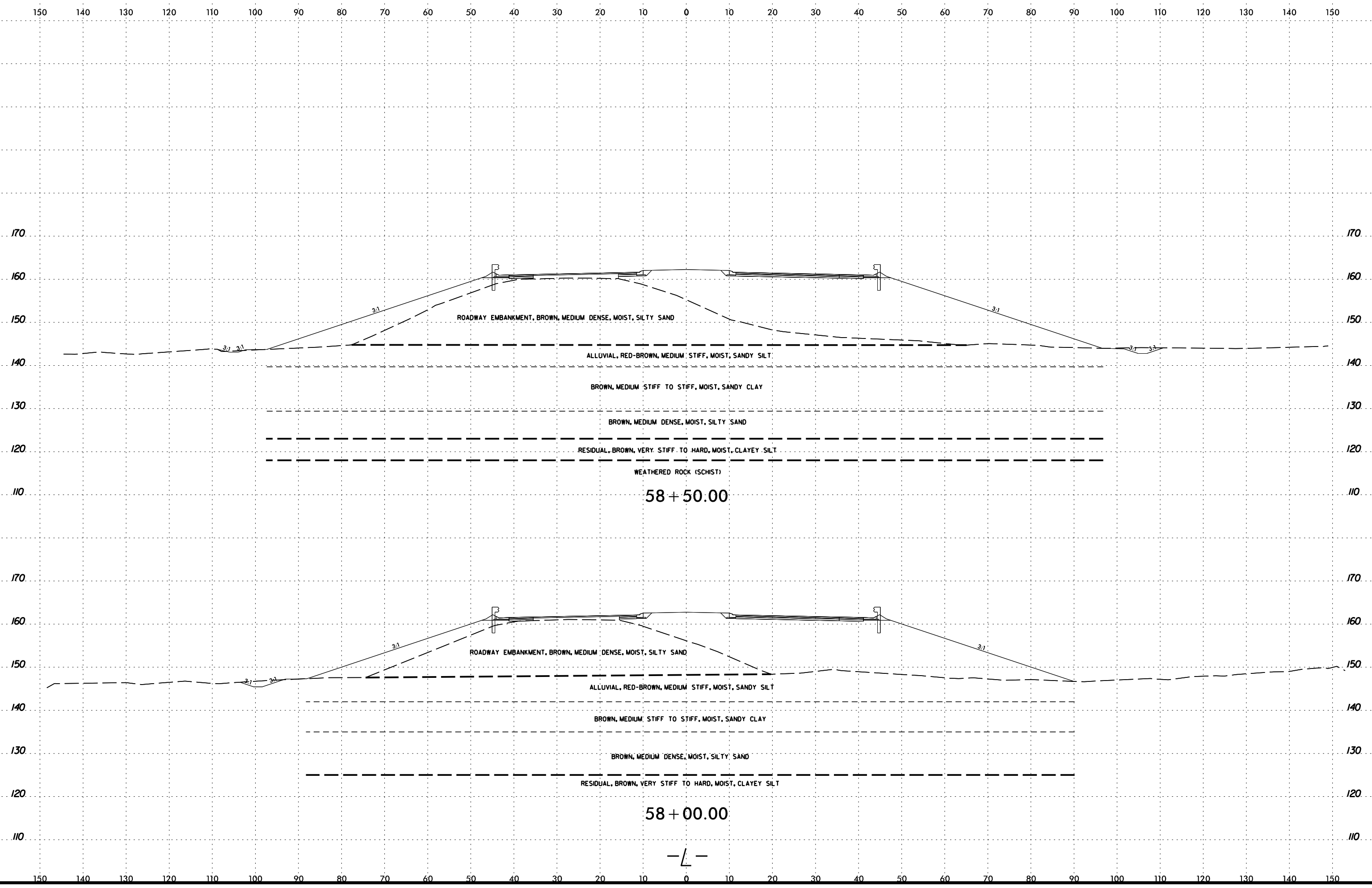
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							C.SAND	F.SAND	SILT	CLAY	10	40	200		
BS-1	55' RT	55+00	-	A-7-5	44	13	-	-	-	-	-	-	81	15.6	-



2/23/2017
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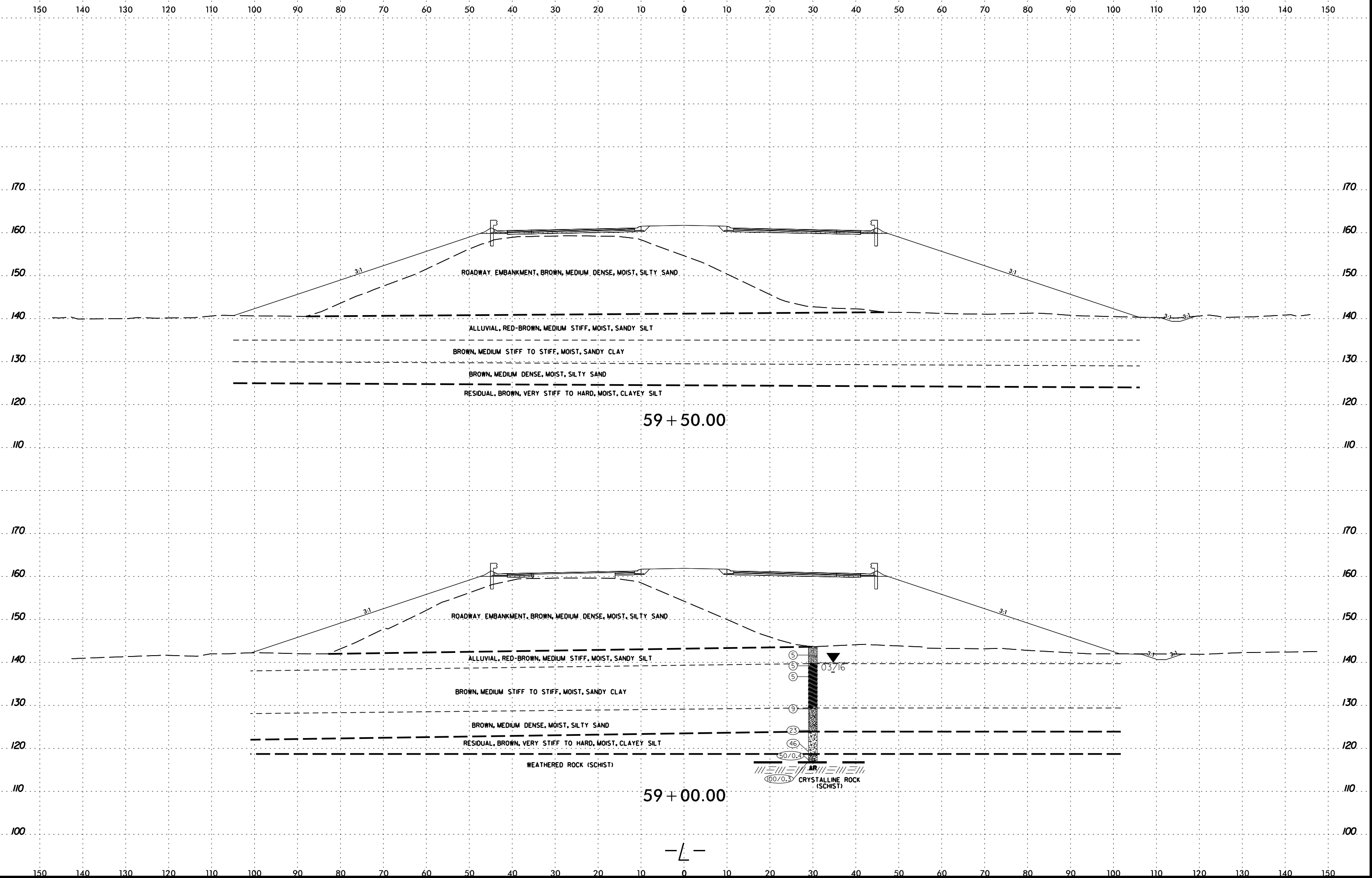


6/23/16



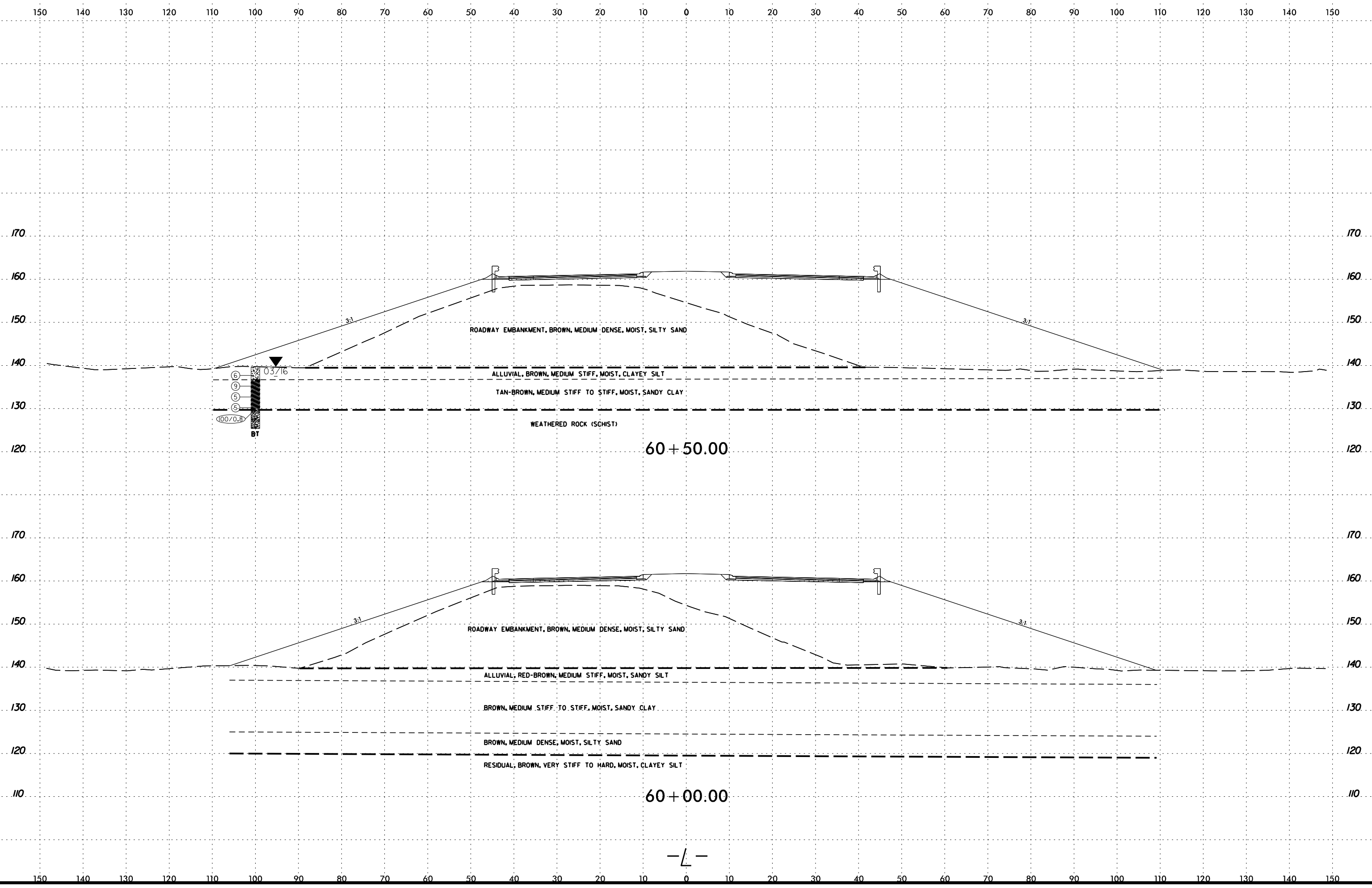
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6/23/16



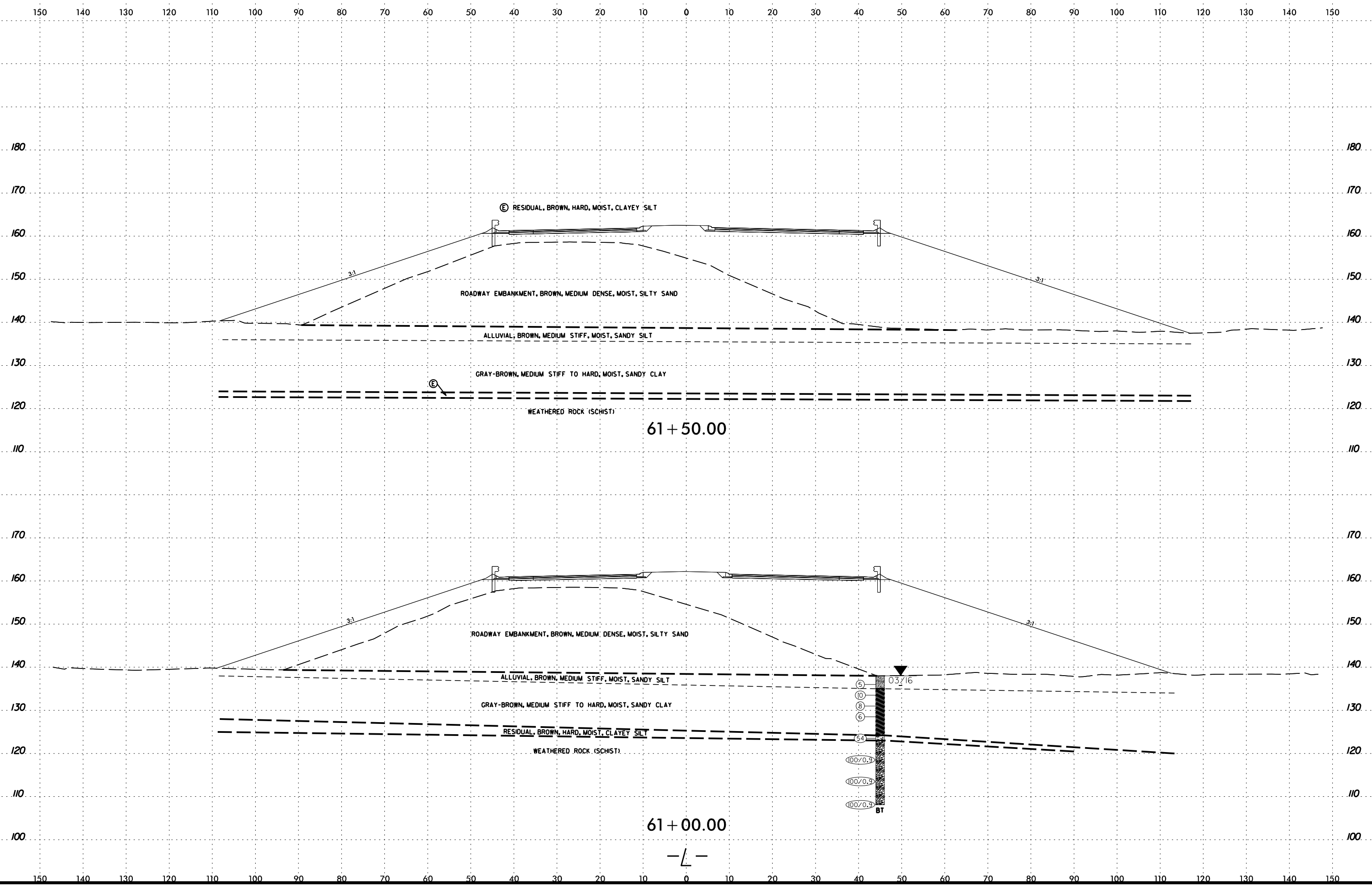
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6/23/16



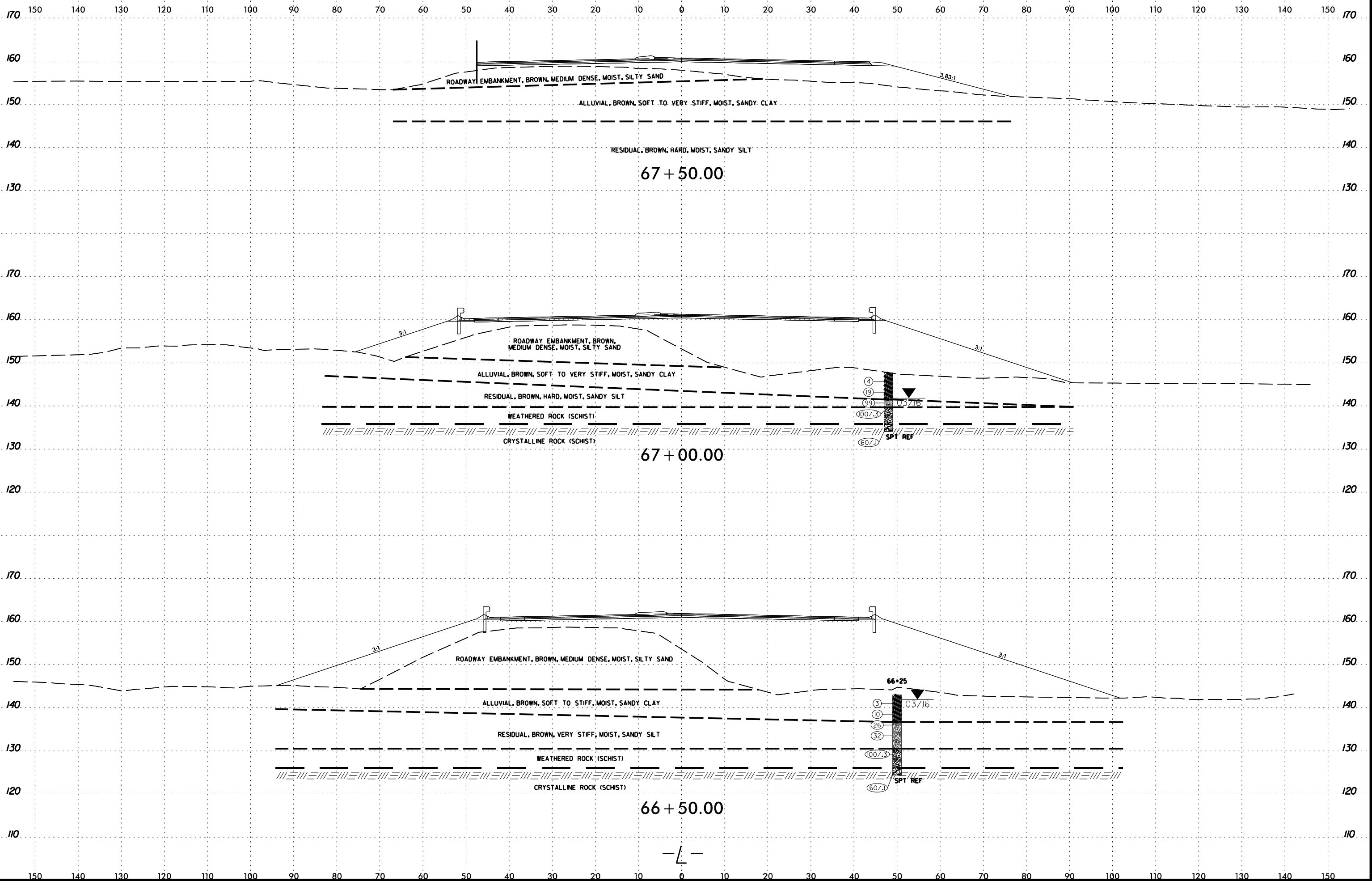
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6/23/16

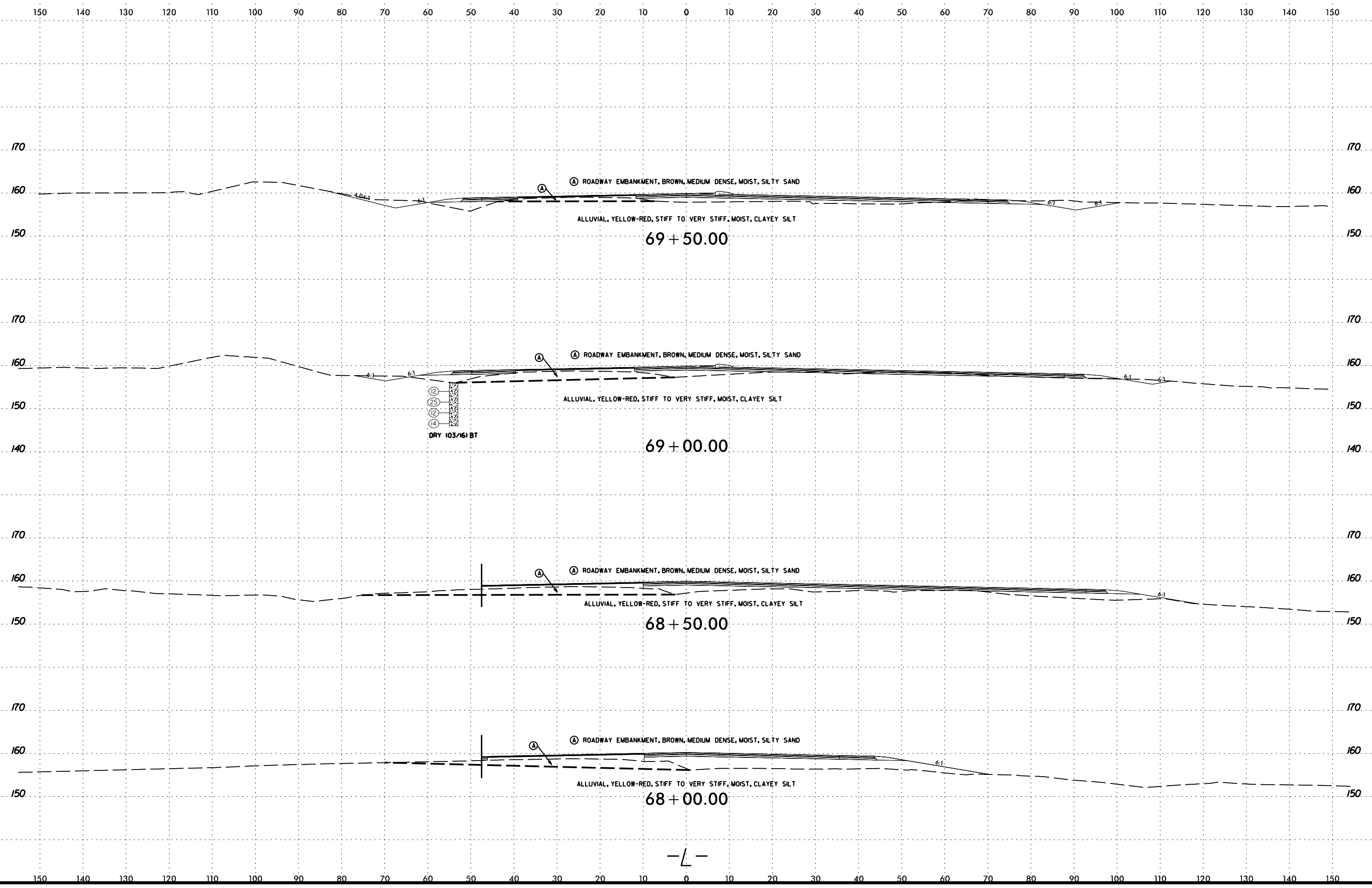


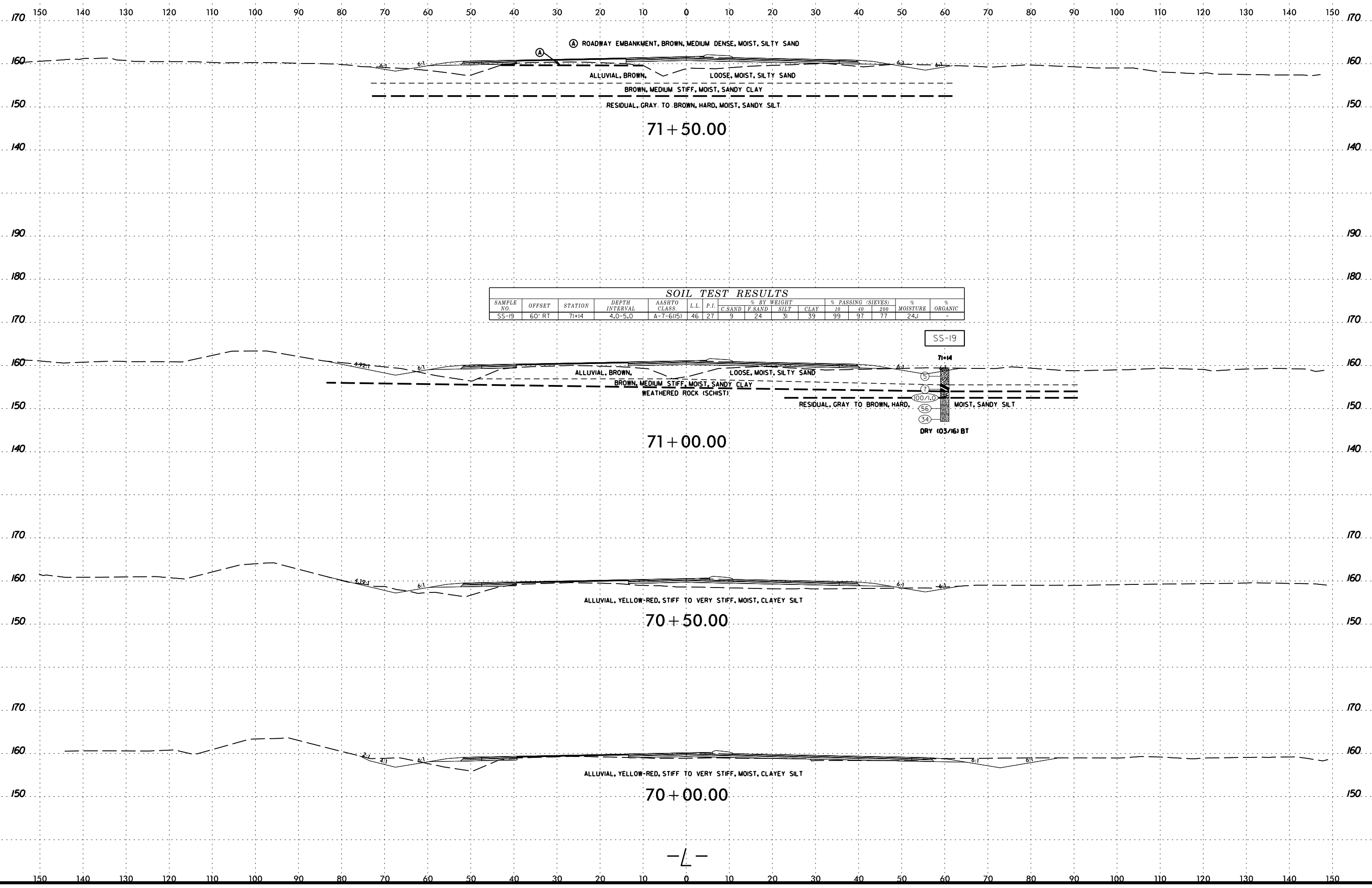
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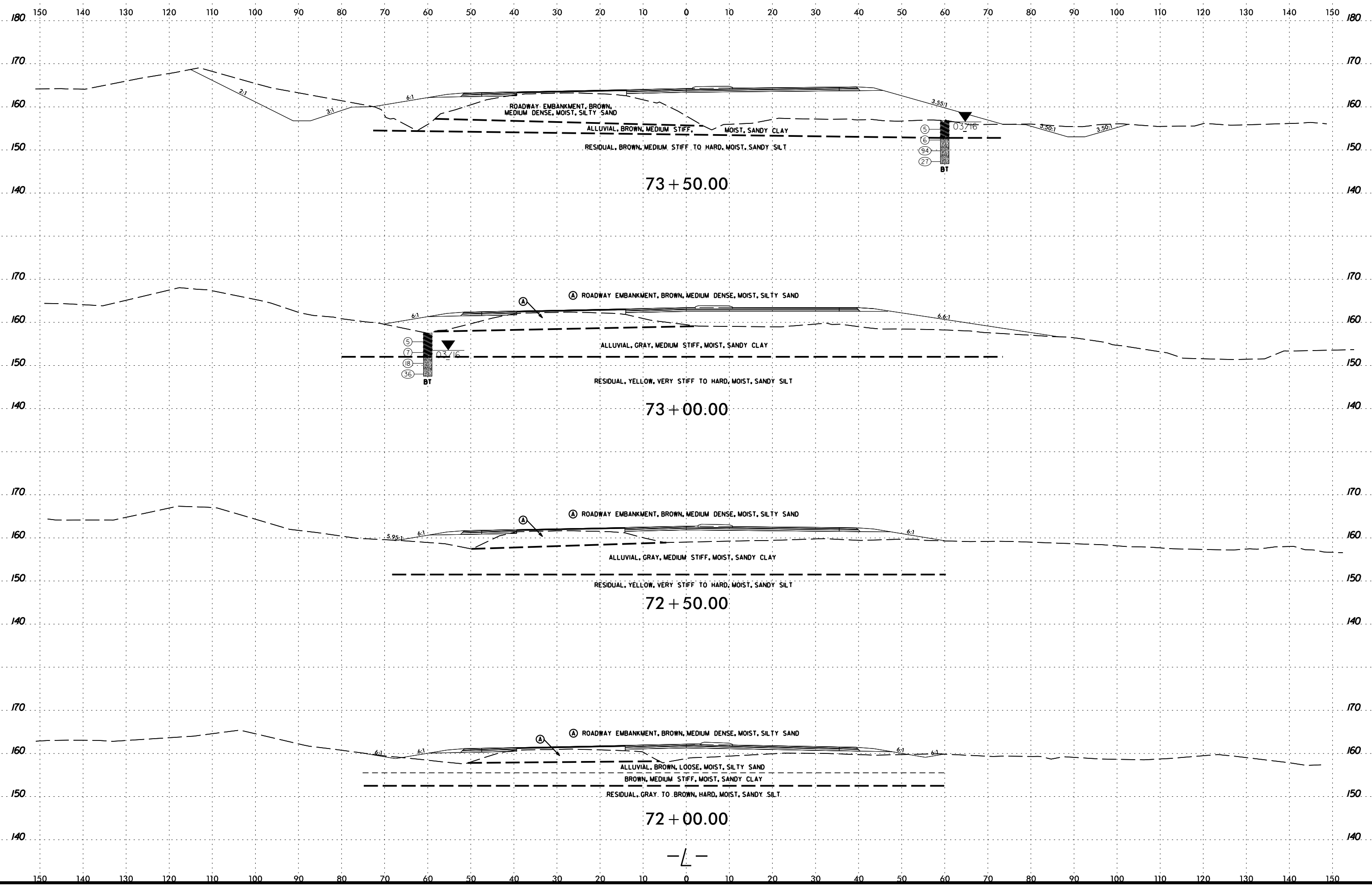
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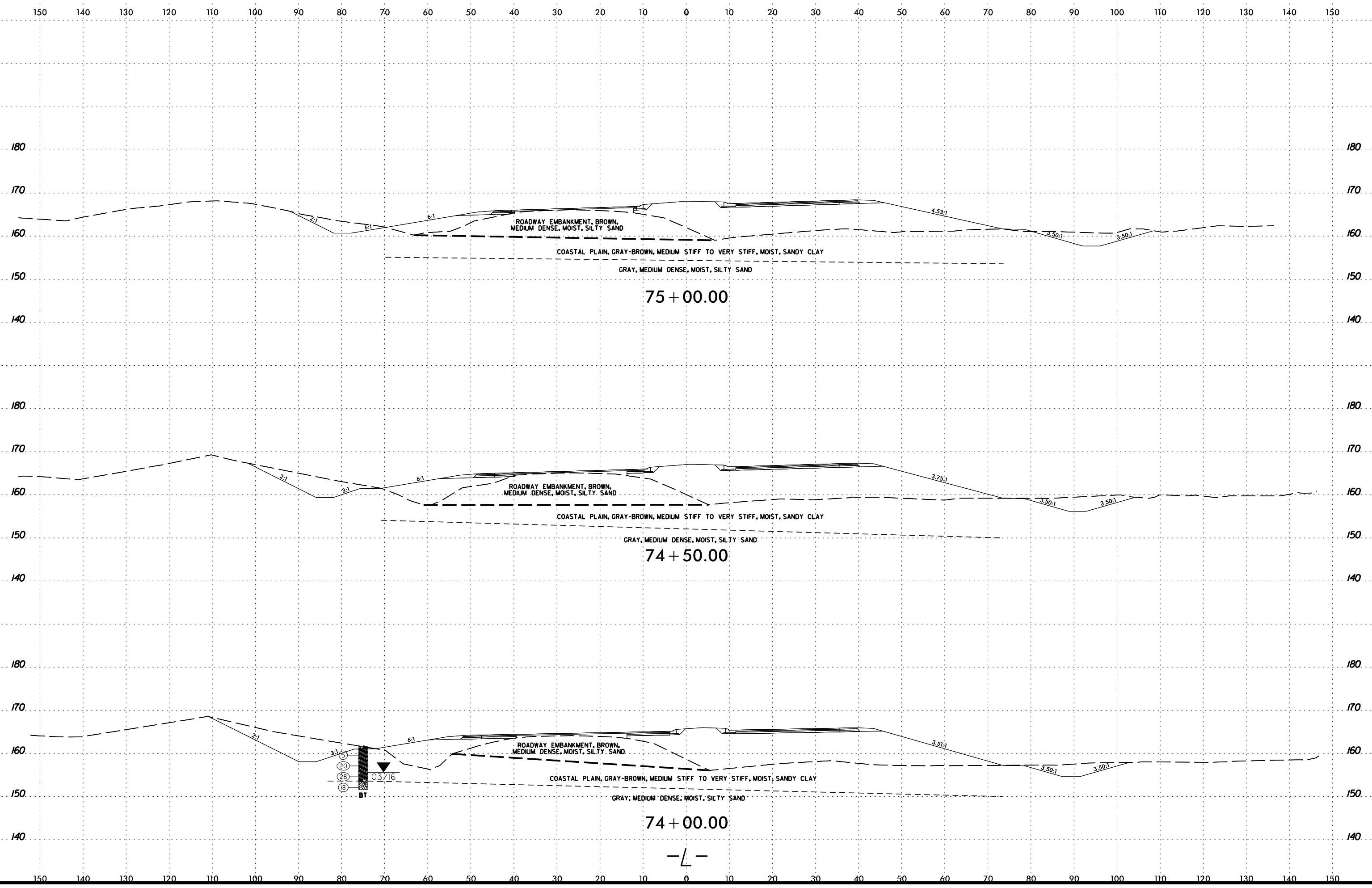
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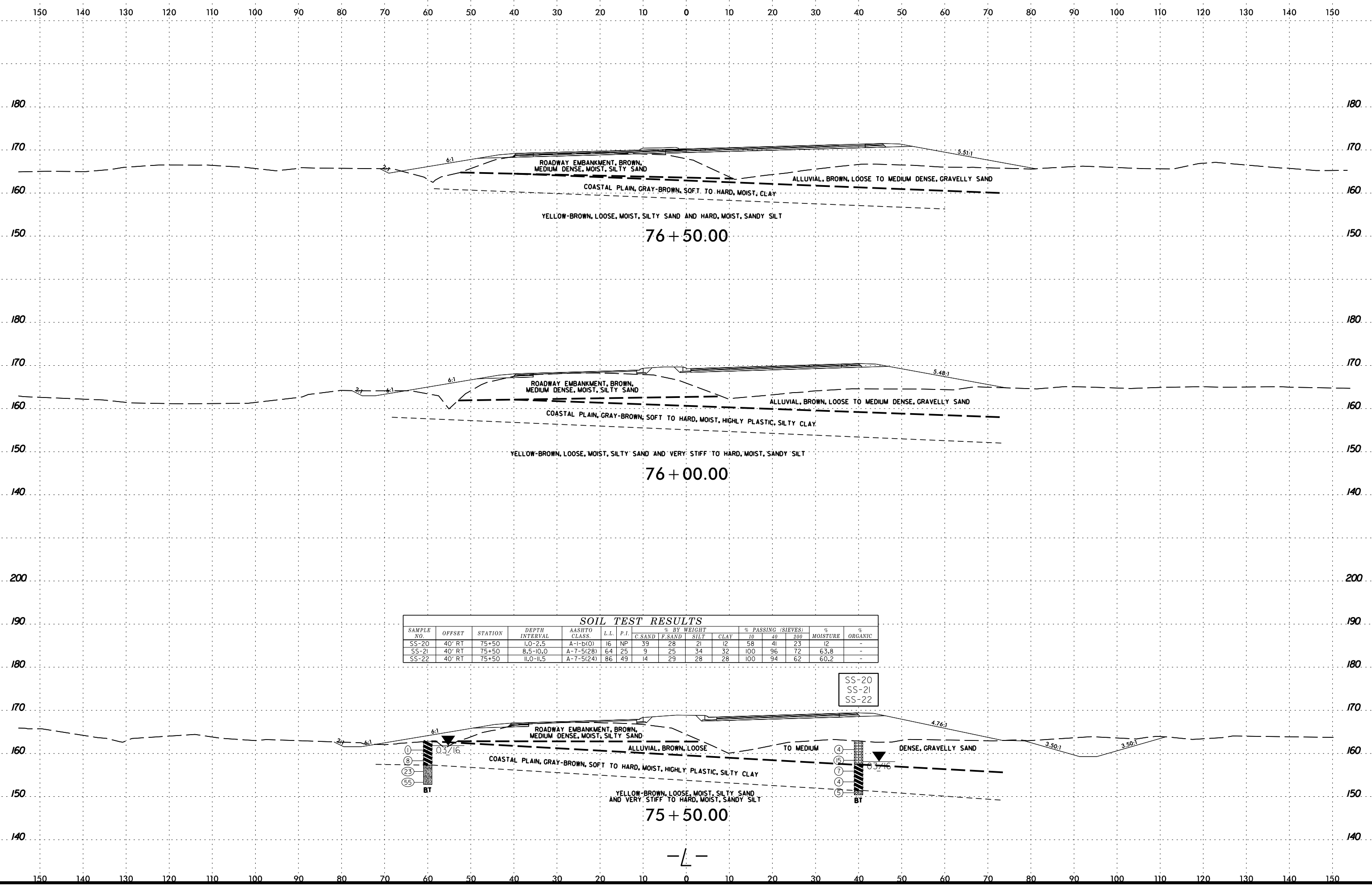




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SOIL TEST RESULTS

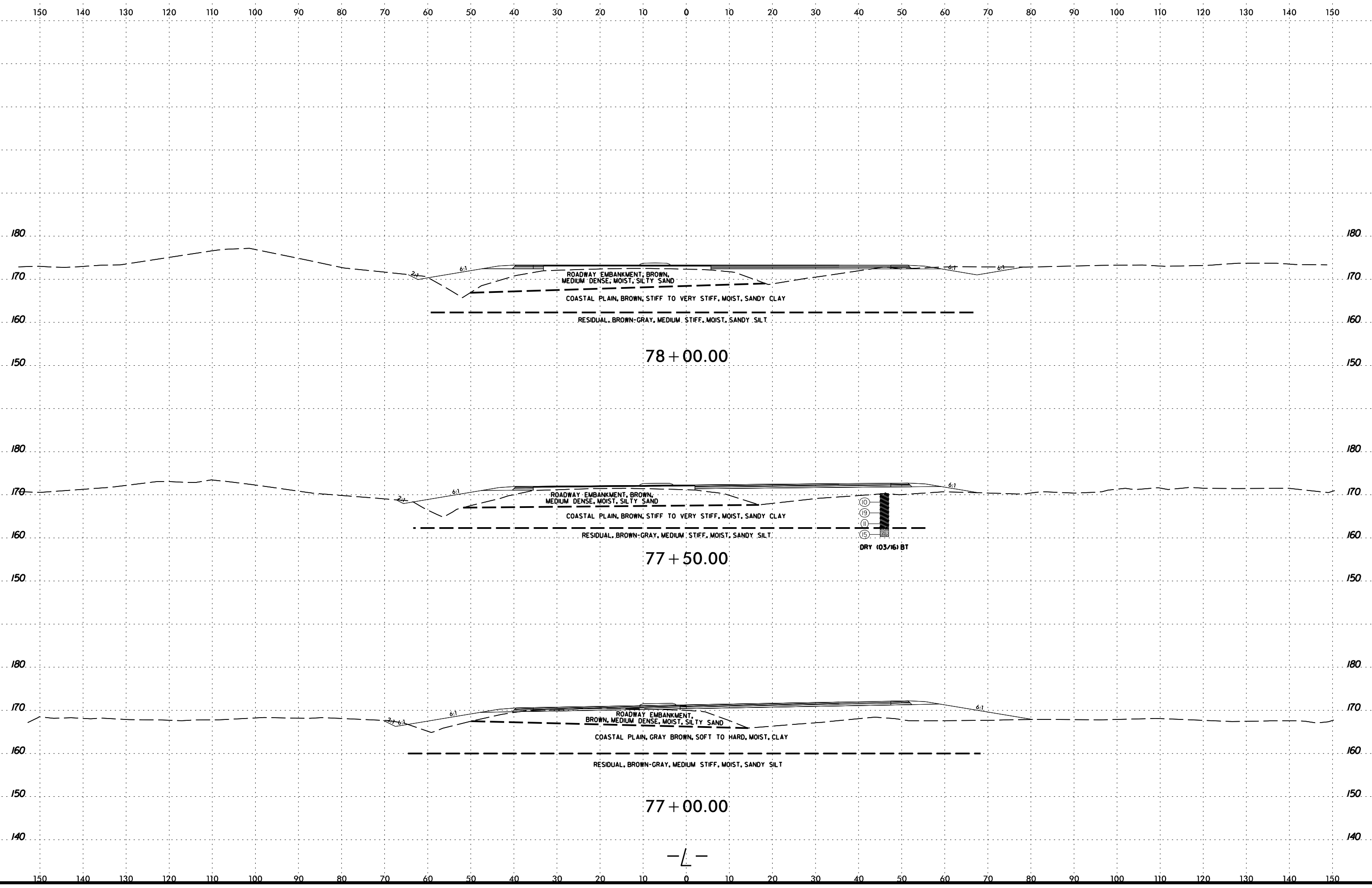
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							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-20	40' RT	75+50	1.0-2.5	A-1-b(0)	16	NP	39	28	21	12	58	41	23	12	-
SS-21	40' RT	75+50	8.5-10.0	A-7-5(28)	64	25	9	25	34	32	100	96	72	63.8	-
SS-22	40' RT	75+50	11.0-11.5	A-7-5(24)	86	49	14	29	28	28	100	94	62	60.2	-

SS-20
SS-21
SS-22

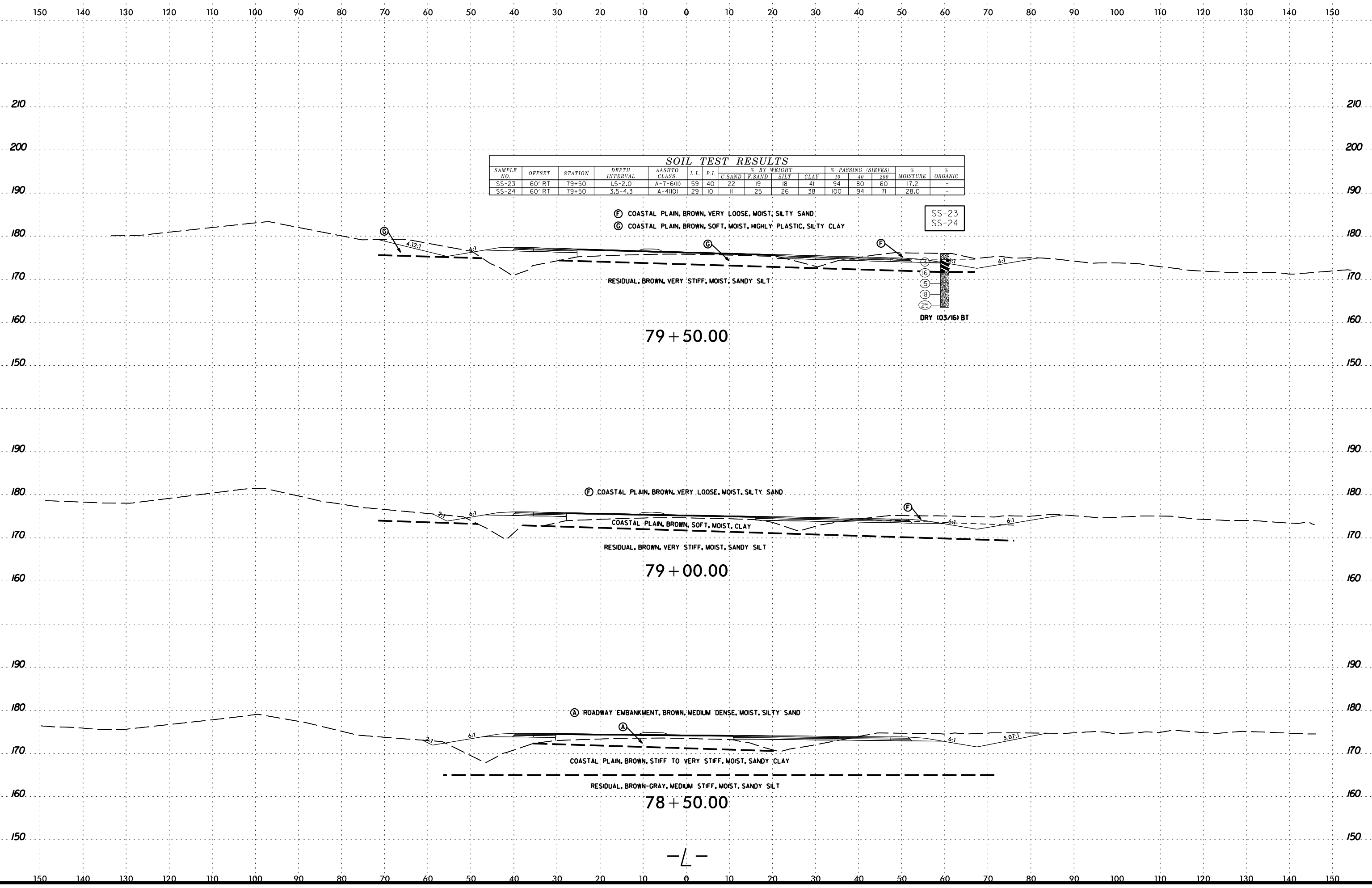
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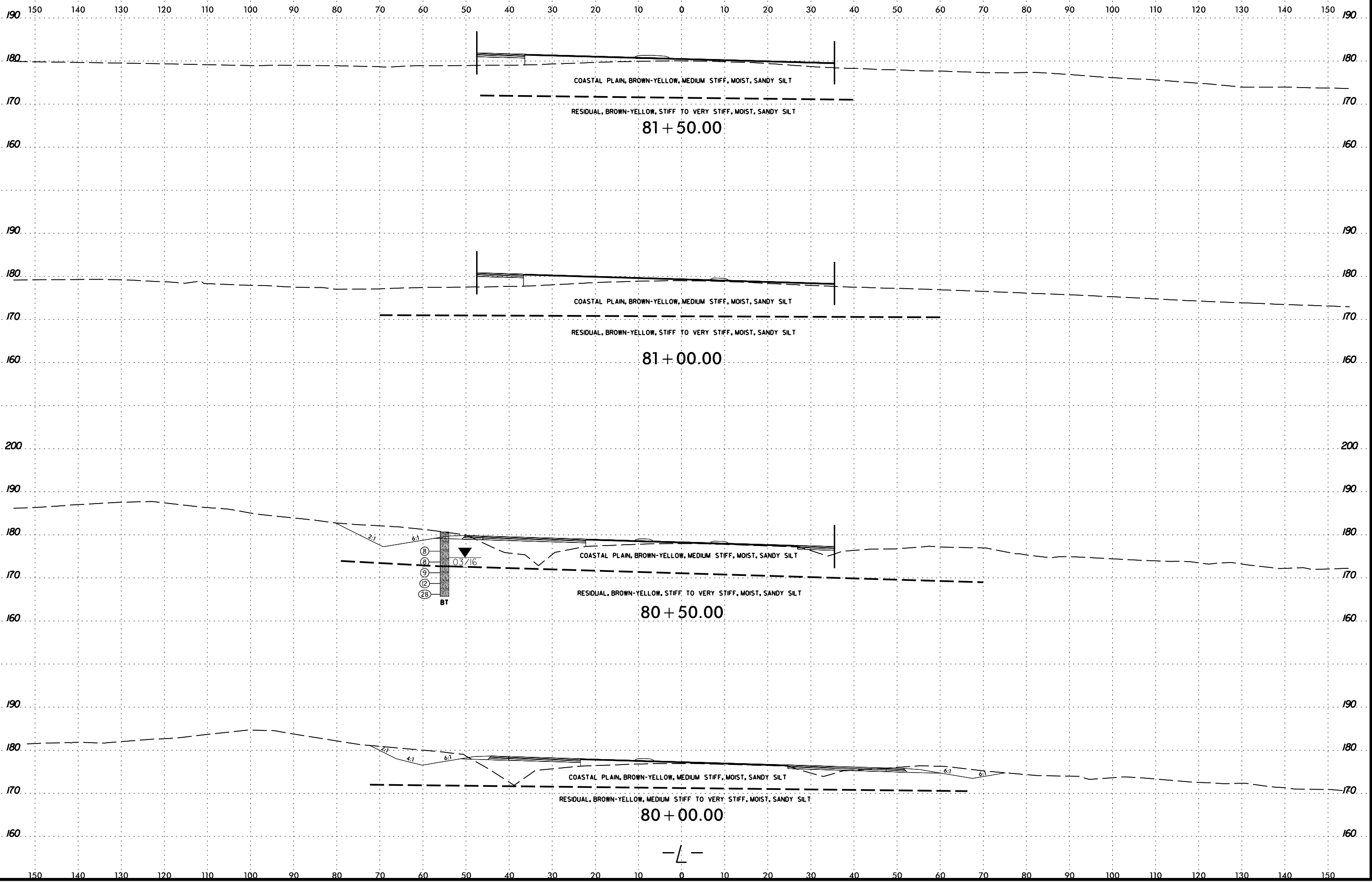
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2/23/2017
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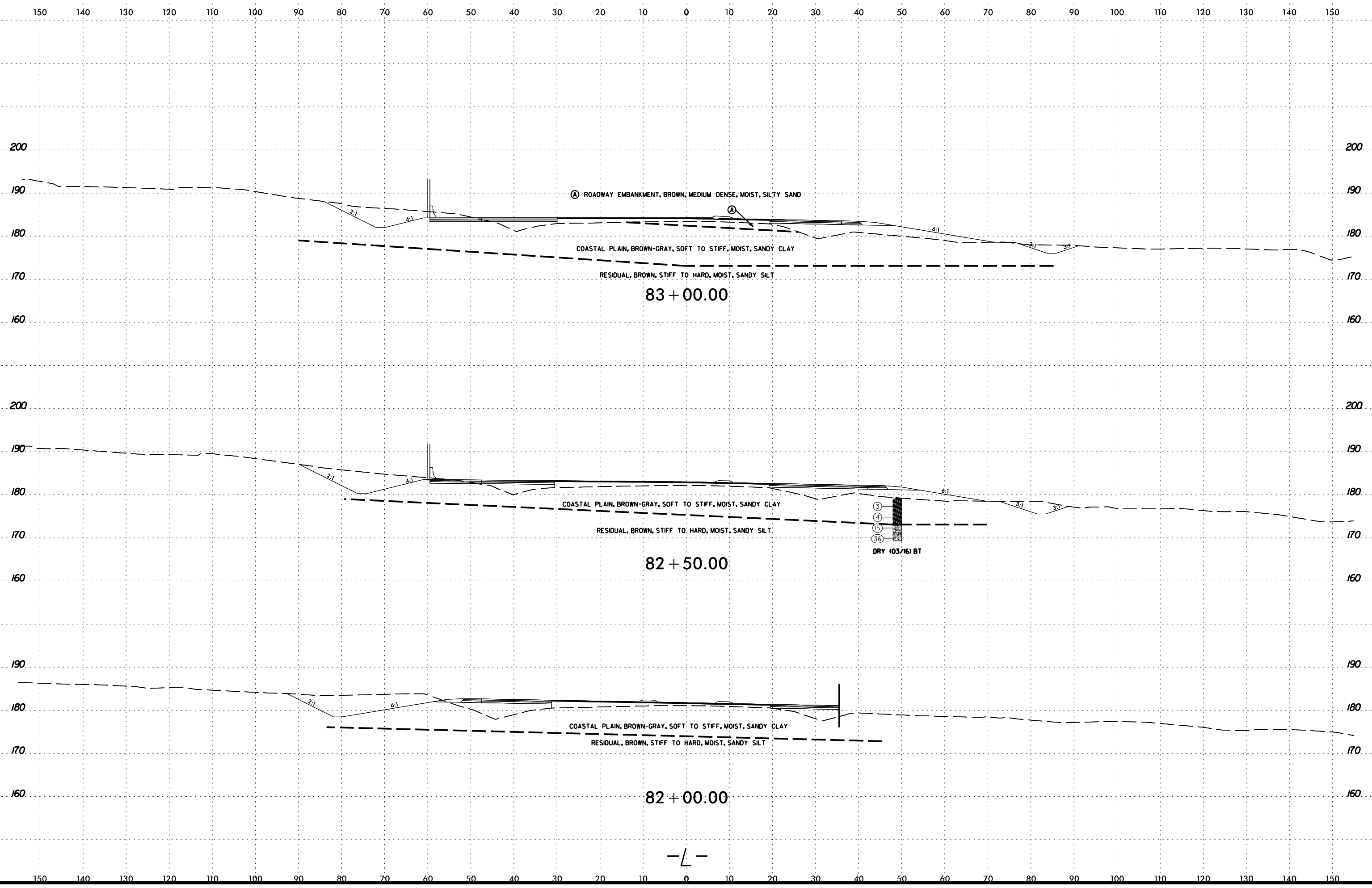
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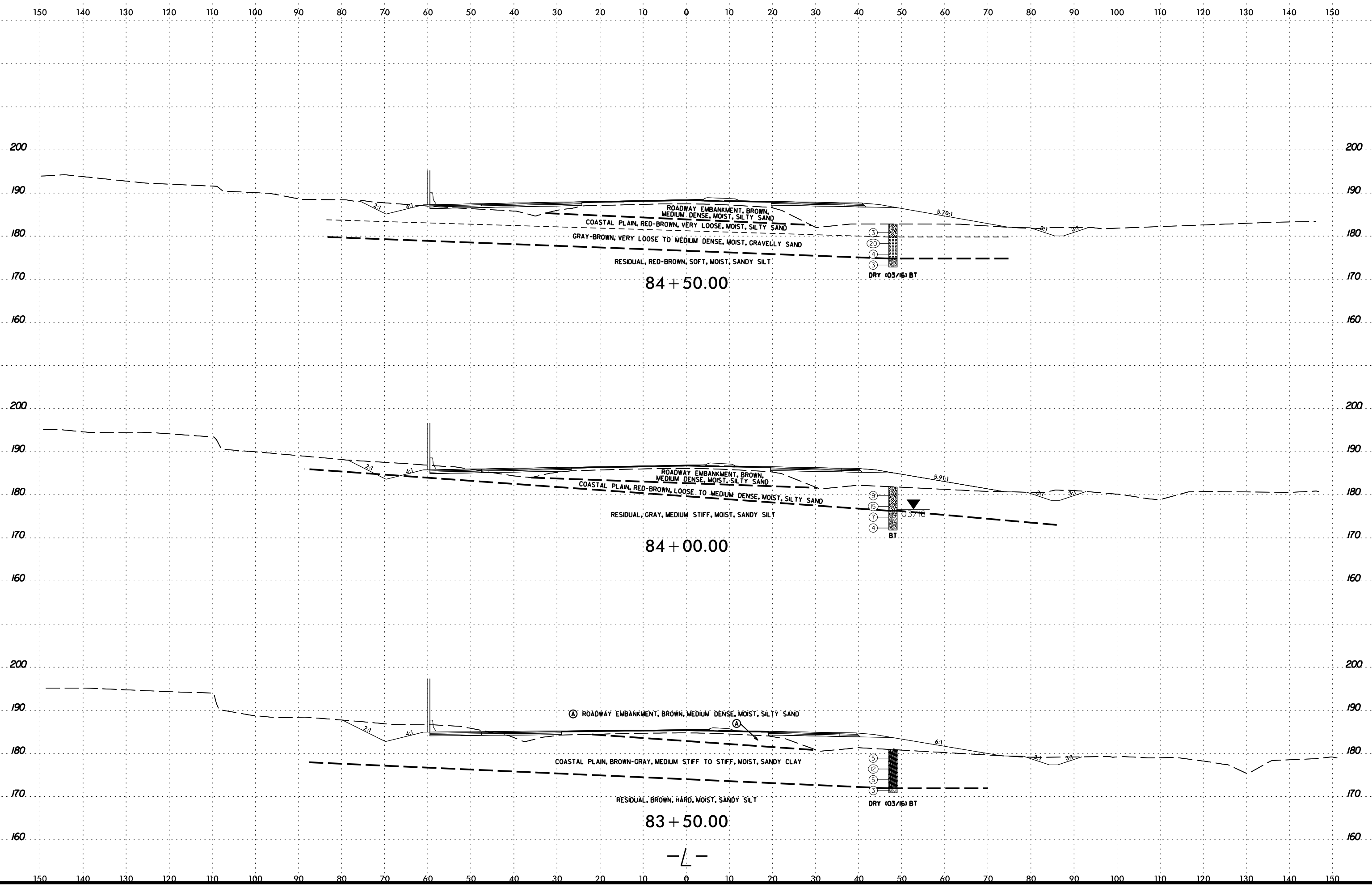


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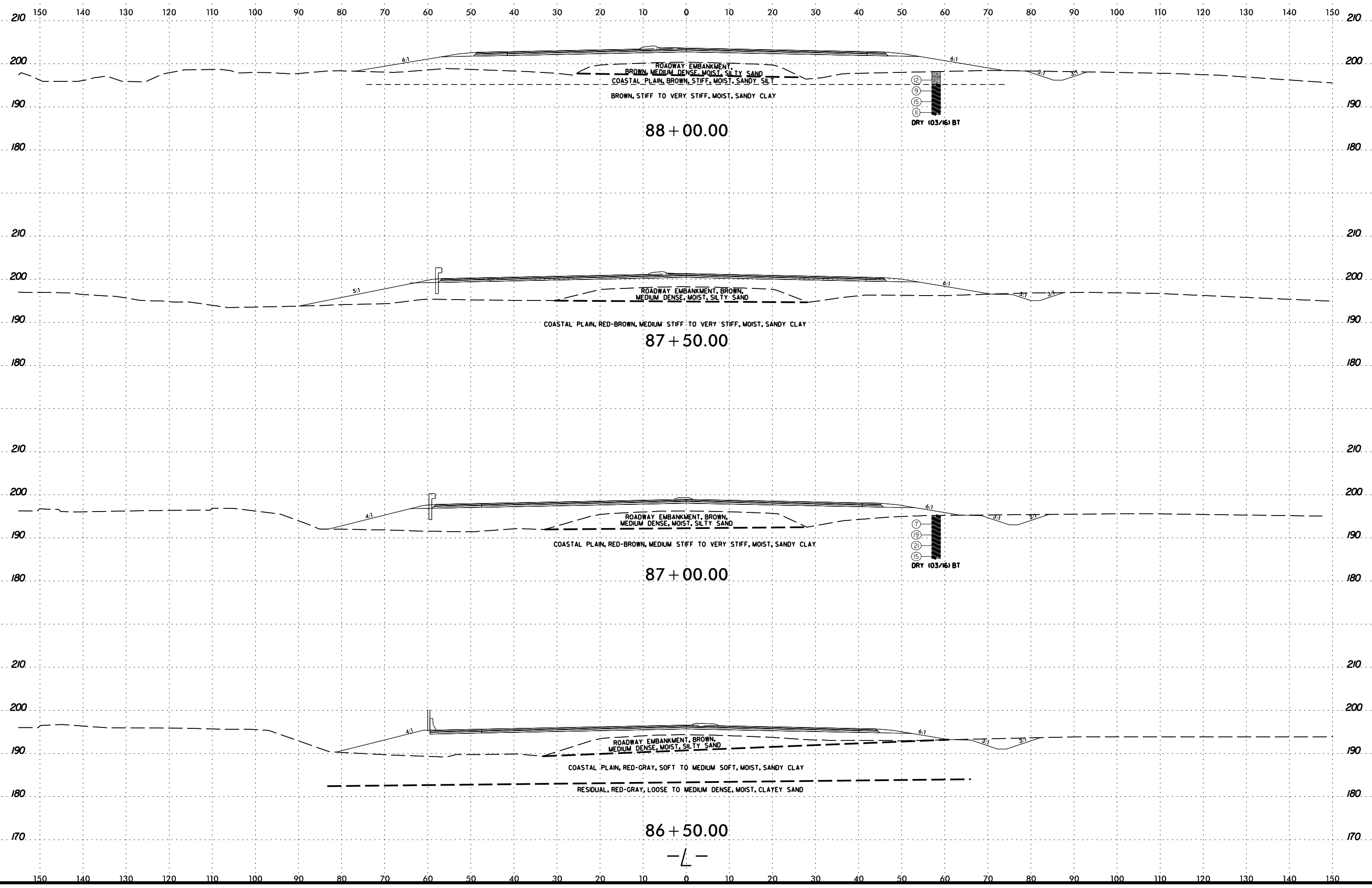
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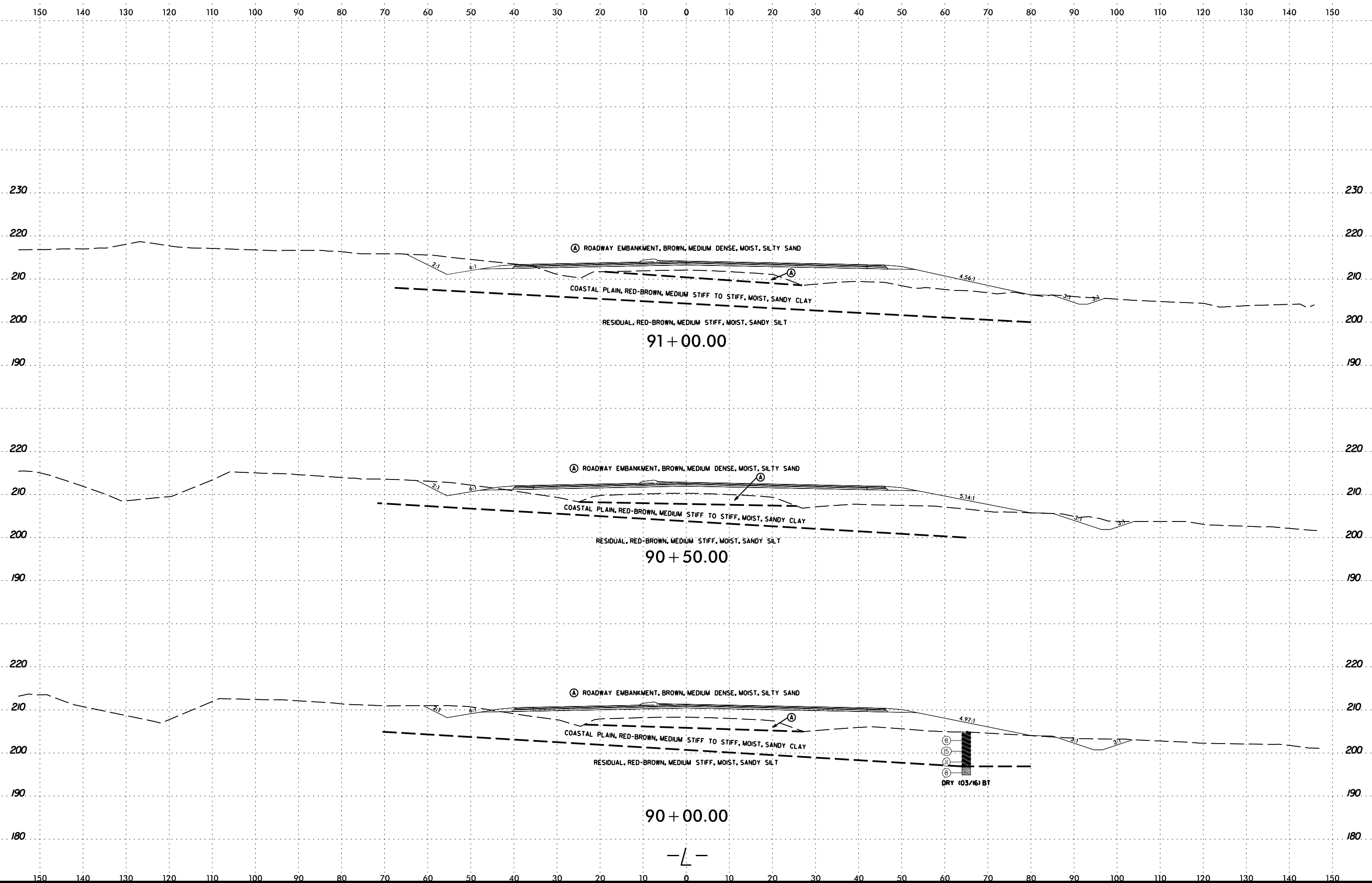
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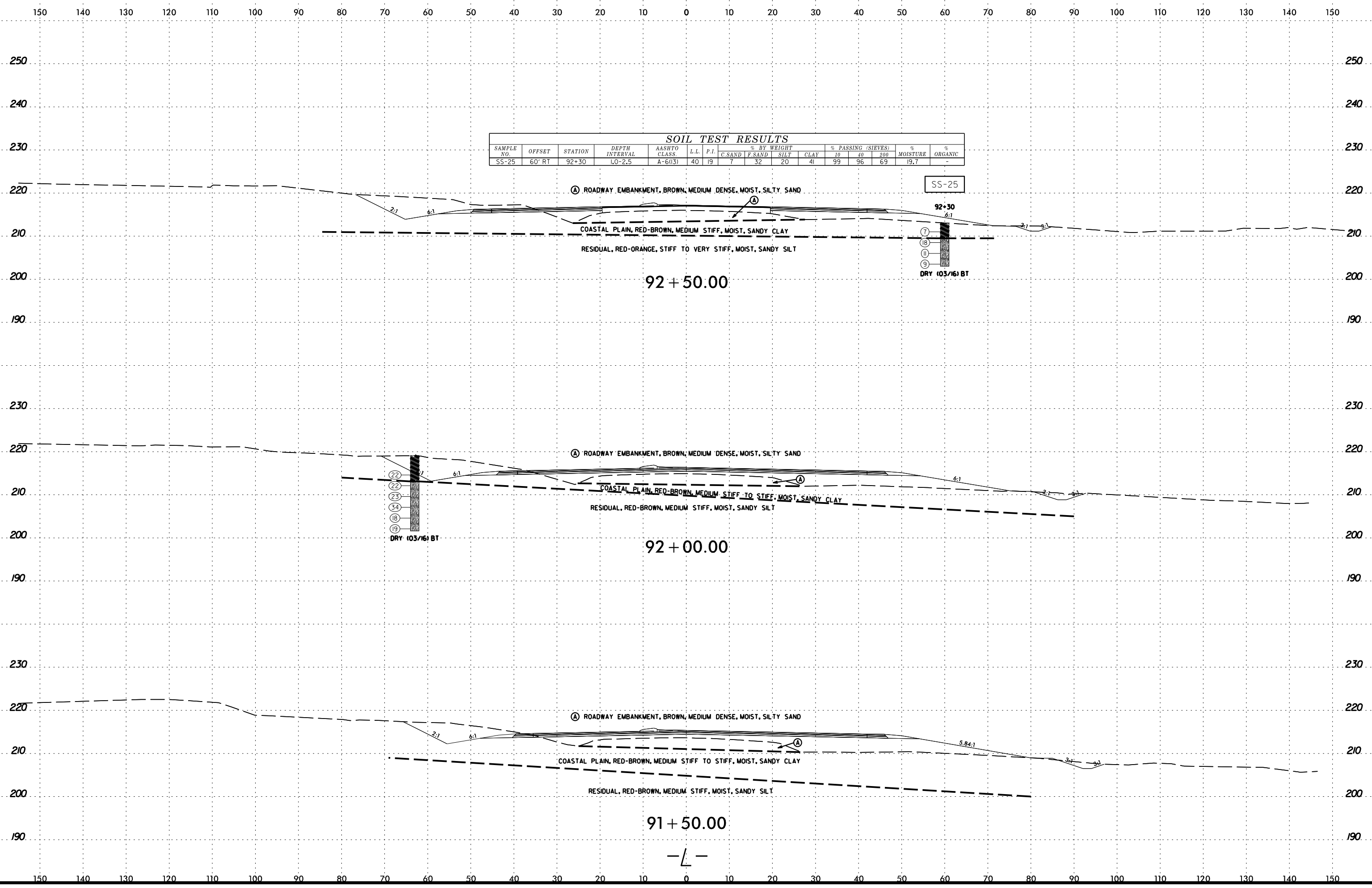


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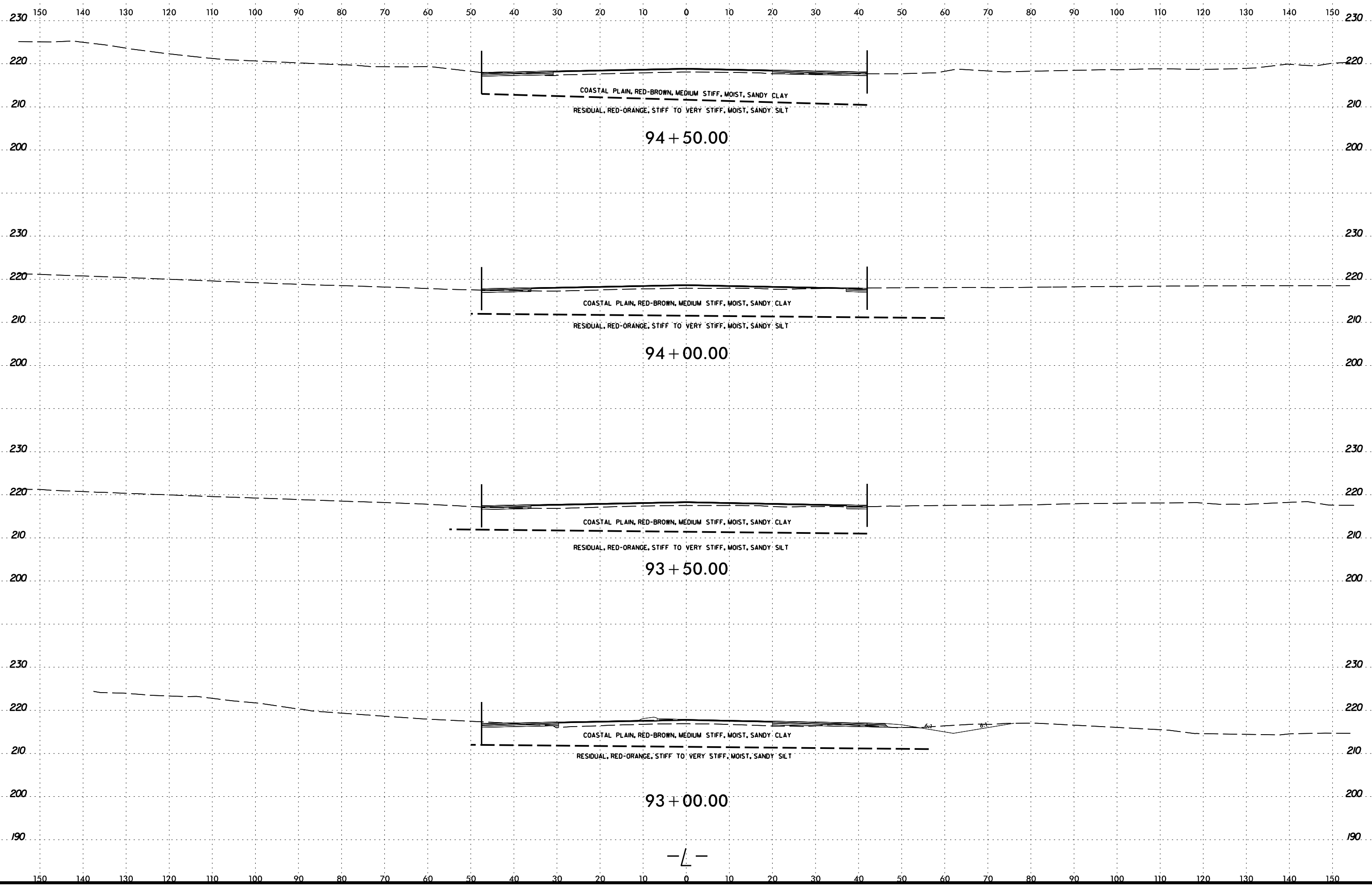


6/23/16



SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C SAND	F SAND	SILT	CLAY	10	40	200		
SS-25	60' RT	92+30	1.0-2.5	A-6(03)	40	19	7	32	20	41	99	96	69	19.7	-

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94 + 50.00

94 + 00.00

93 + 50.00

93 + 00.00

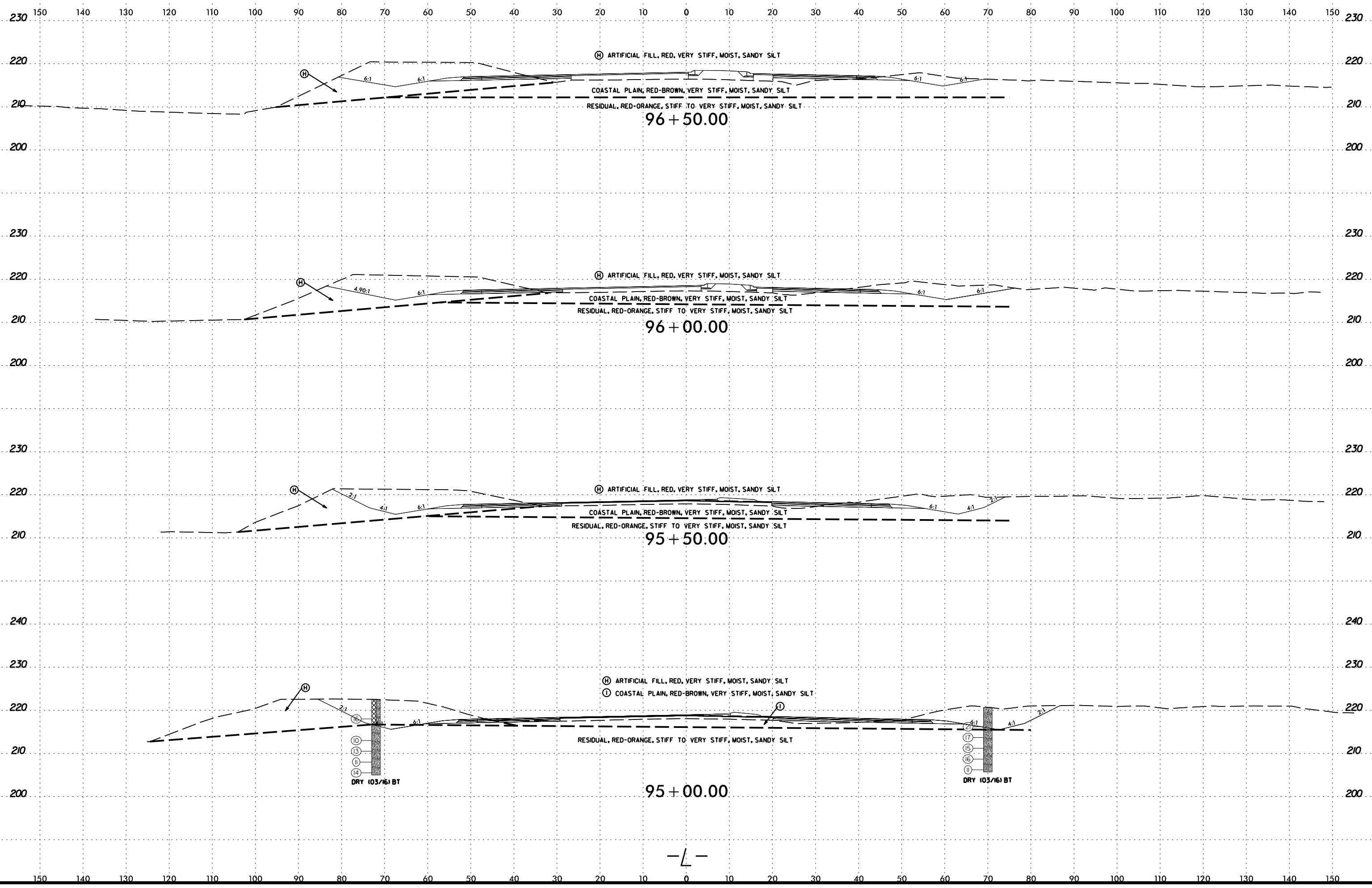
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 RESIDUAL, RED-ORANGE, STIFF TO VERY STIFF, MOIST, SANDY SILT

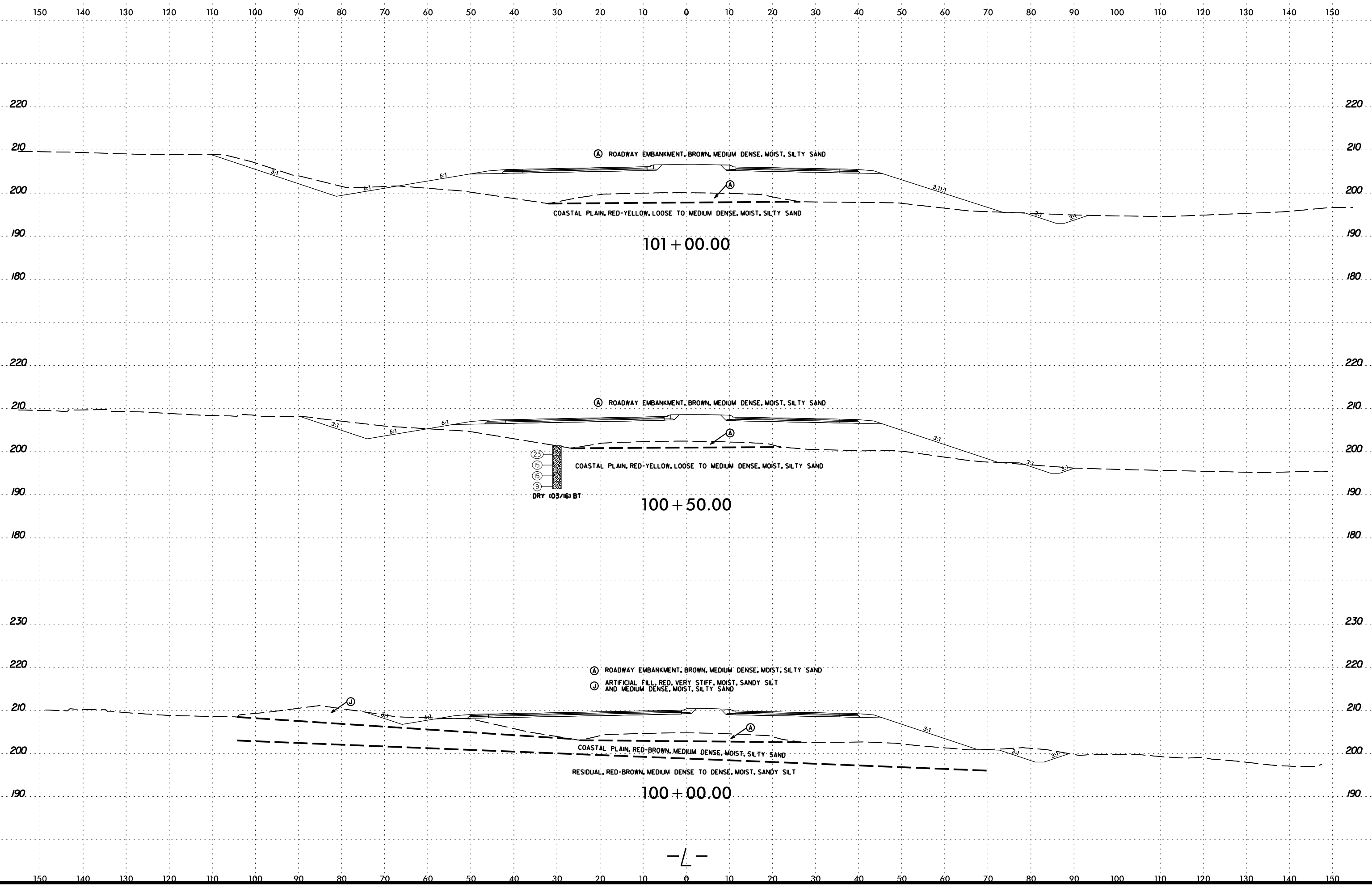
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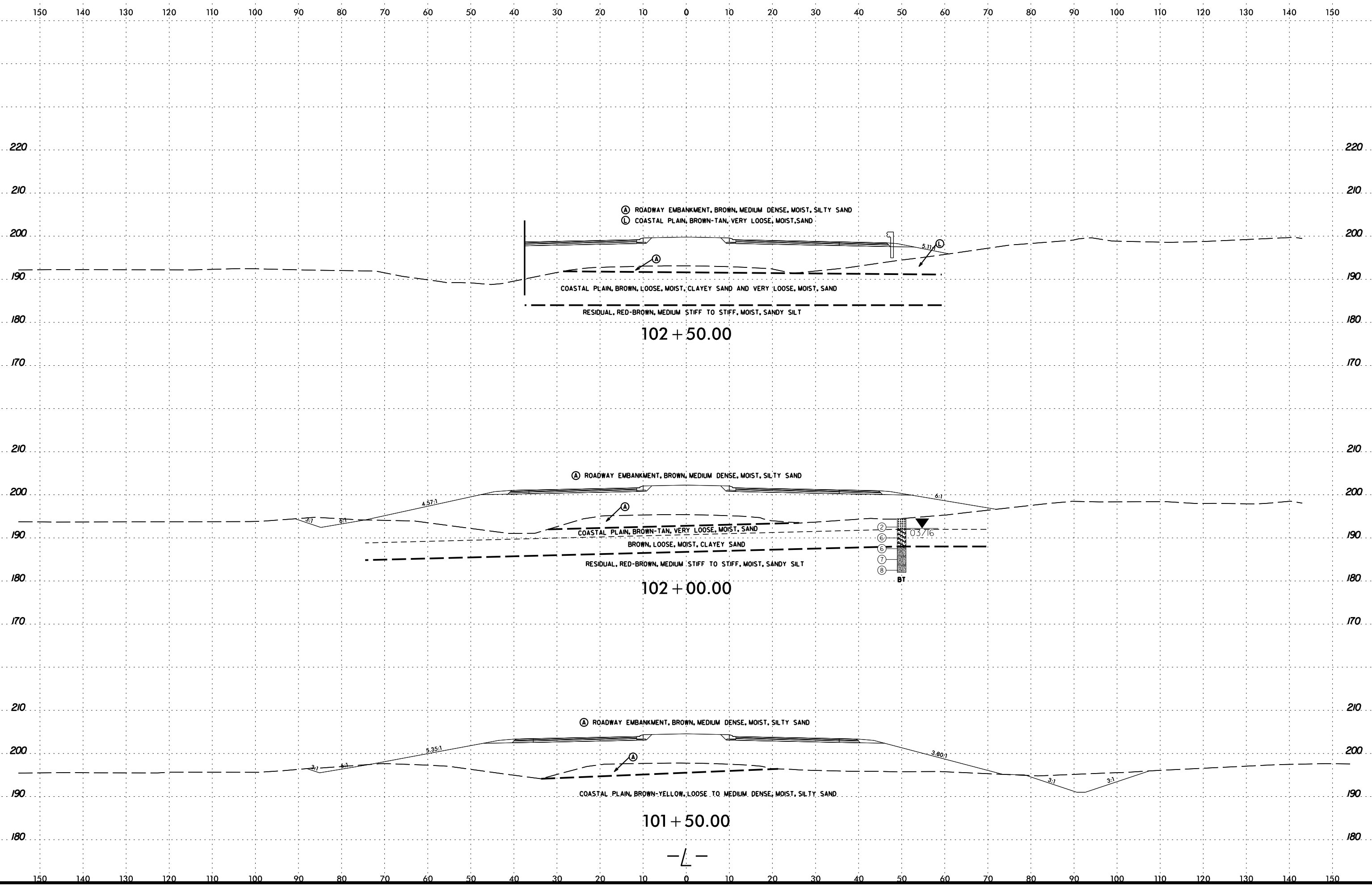
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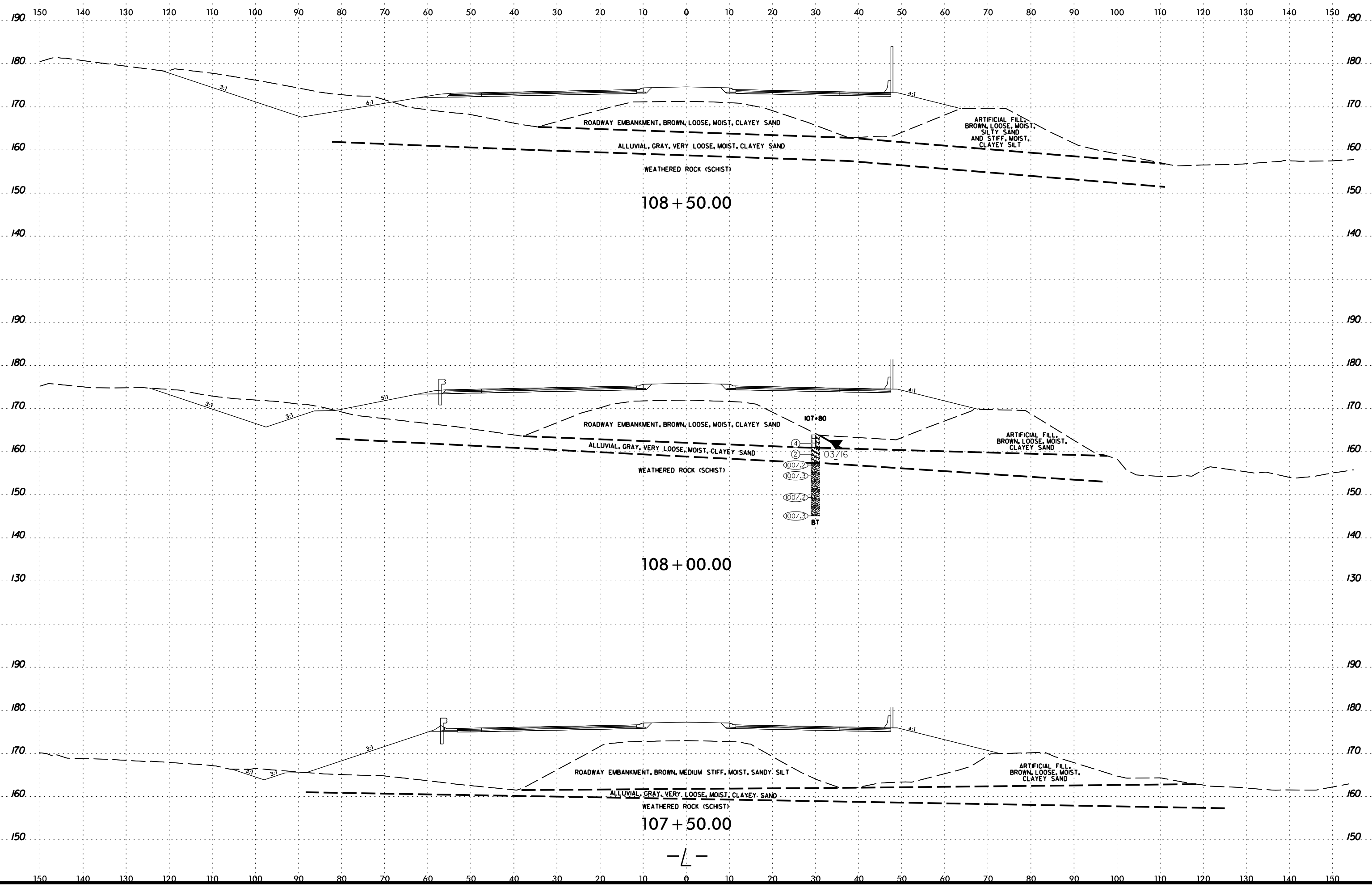




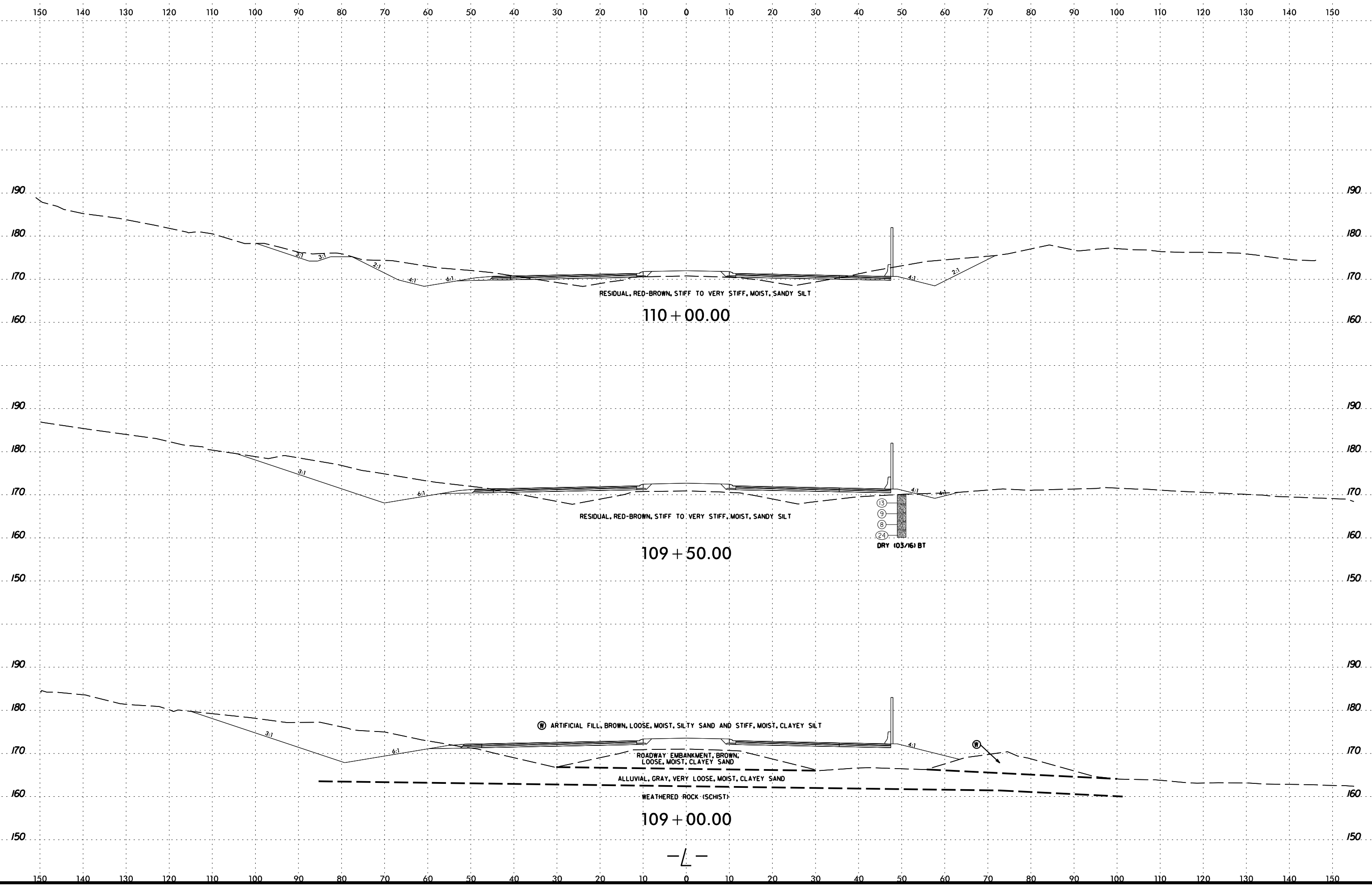
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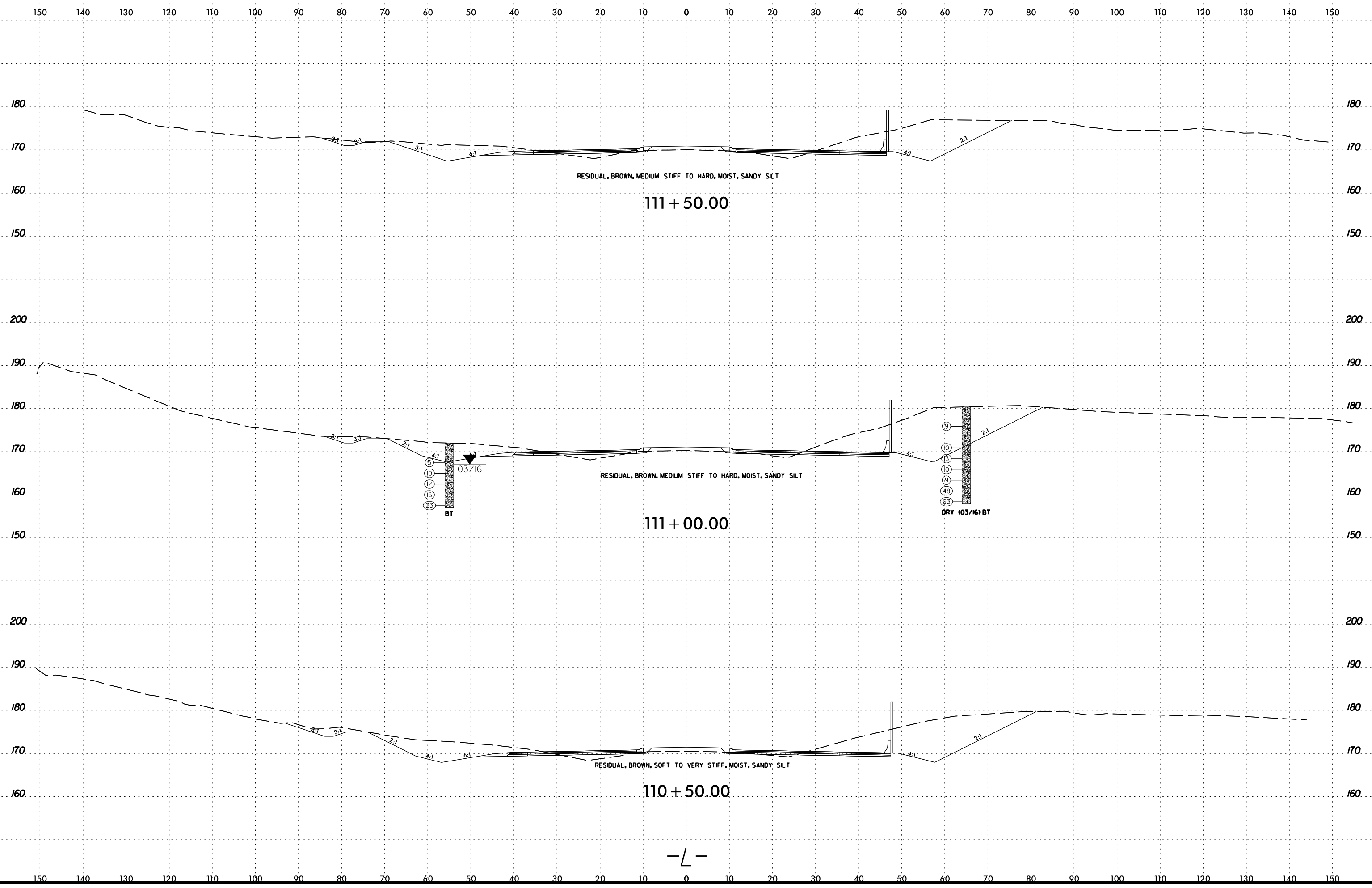


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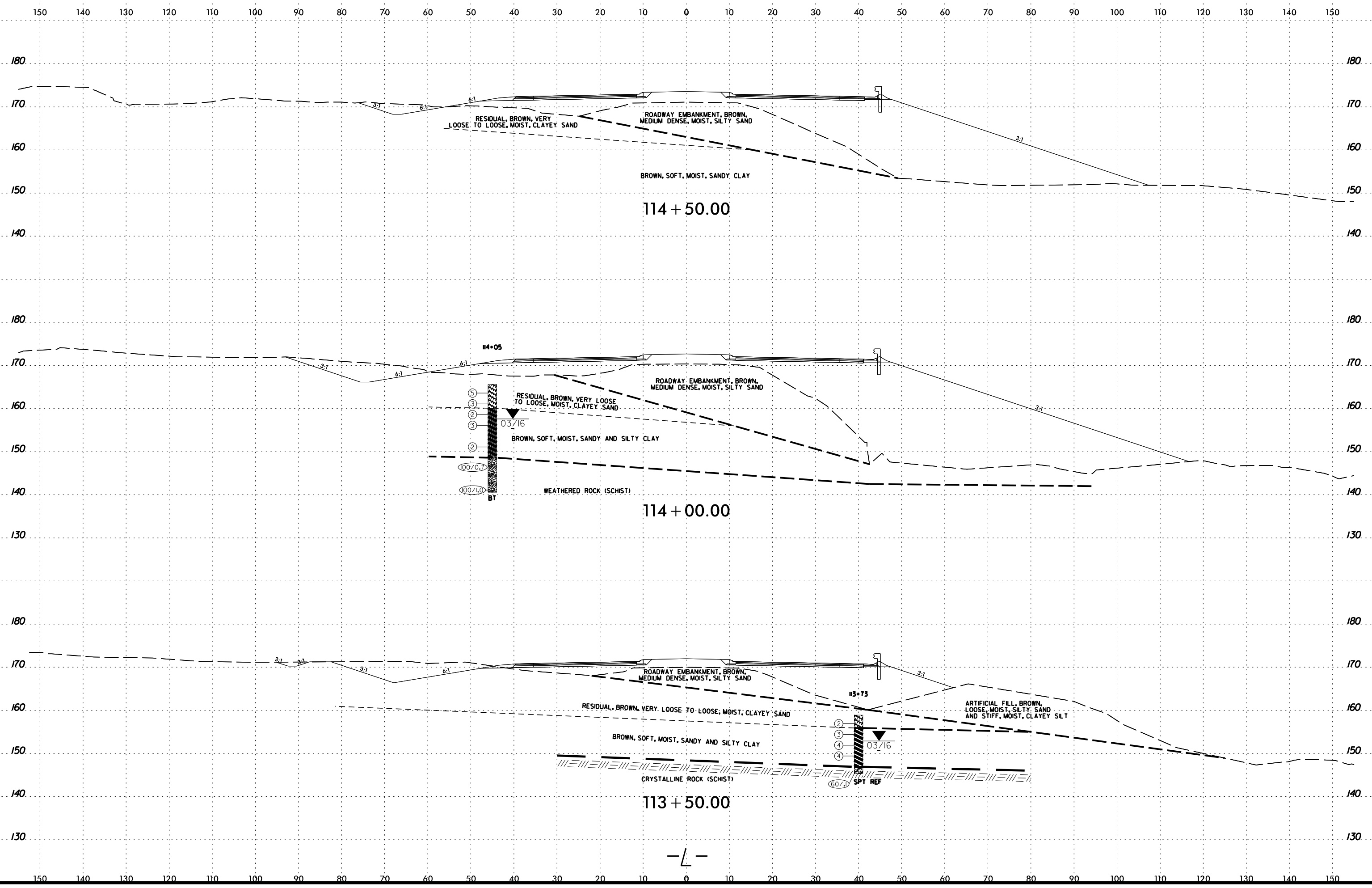
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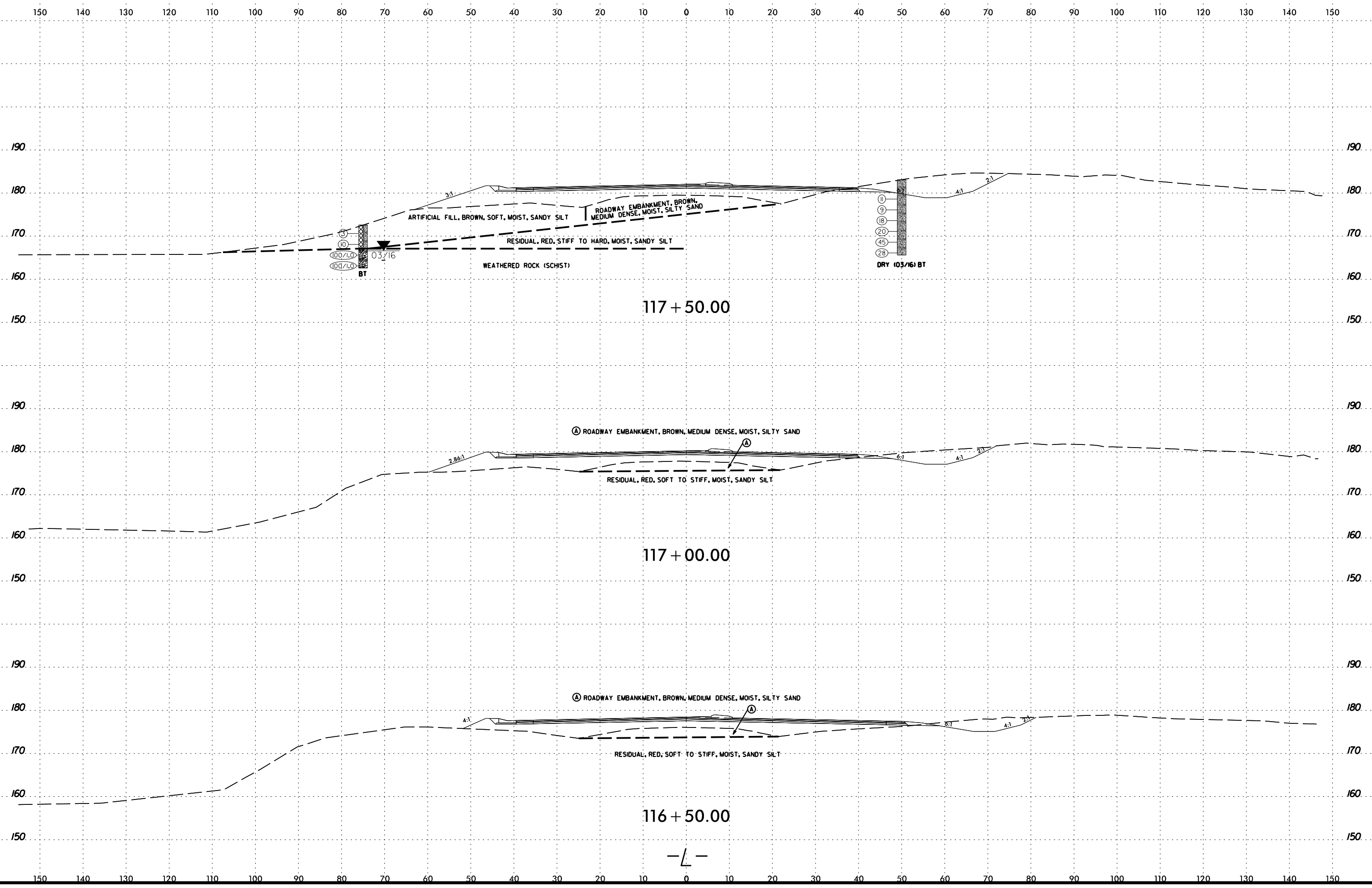
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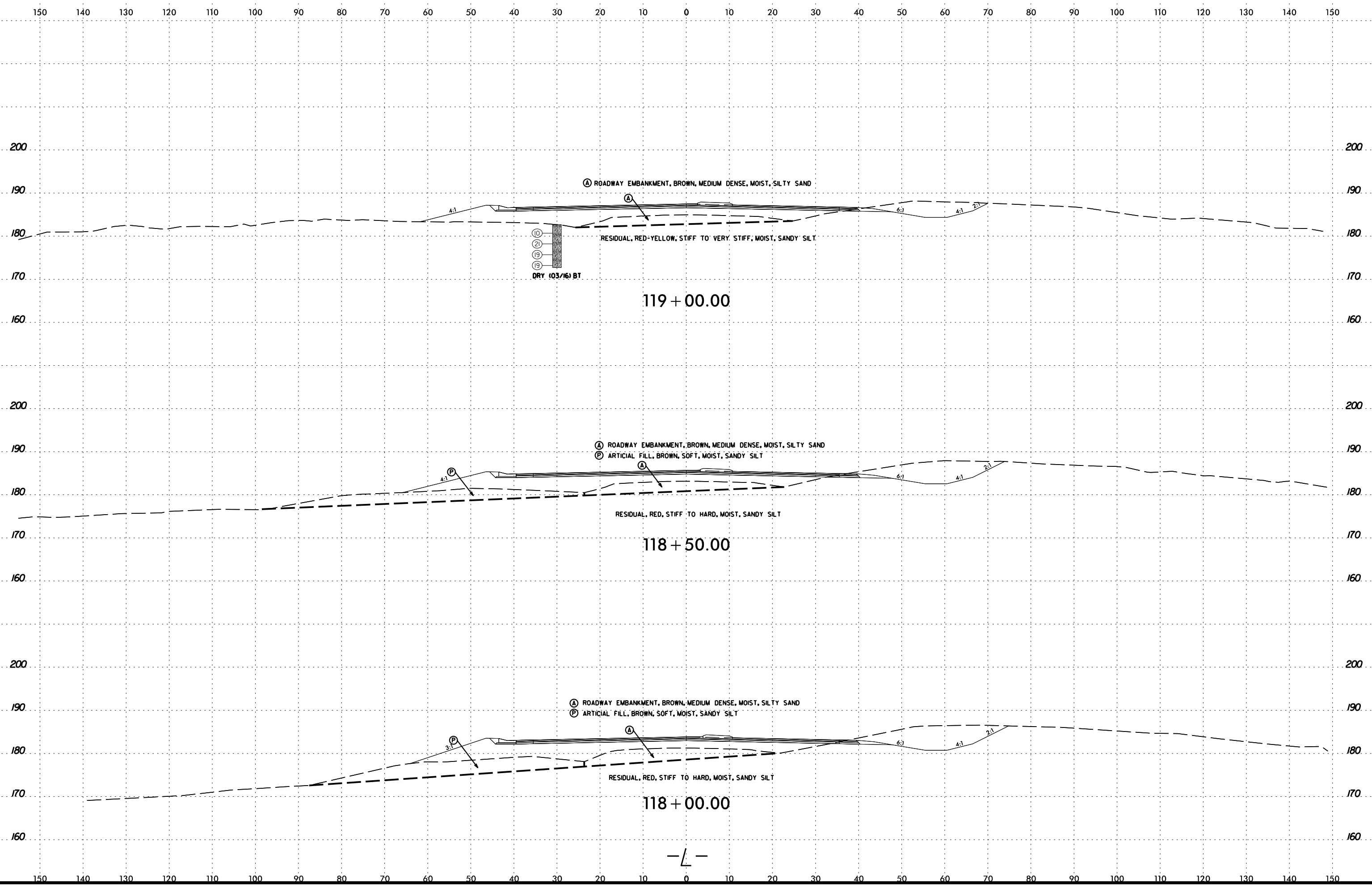
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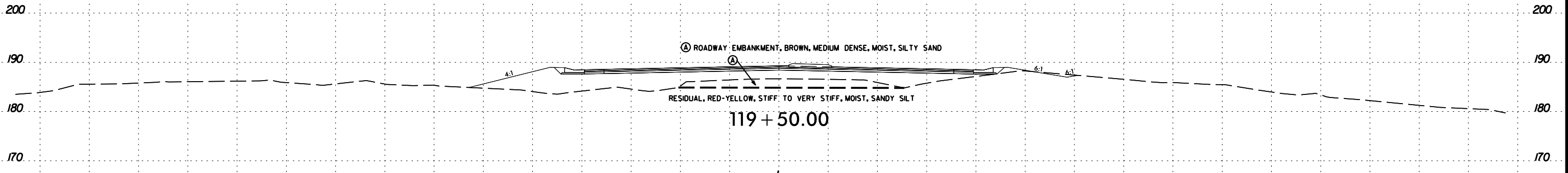
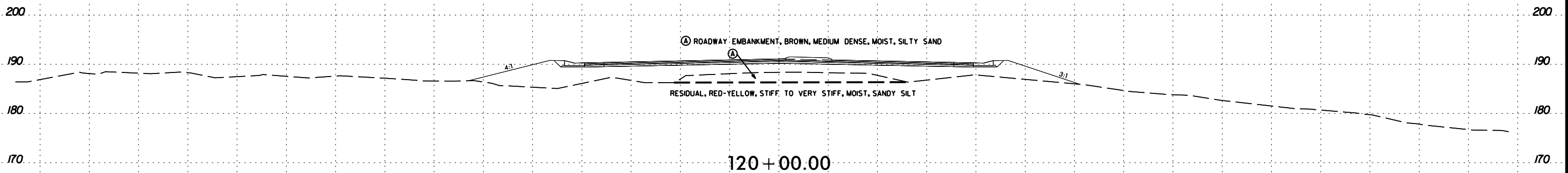
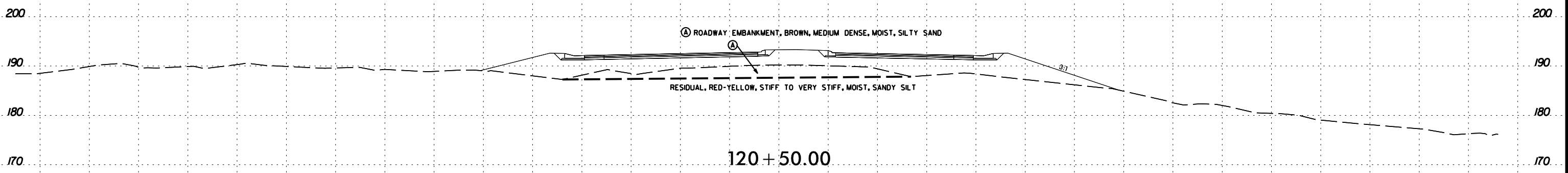
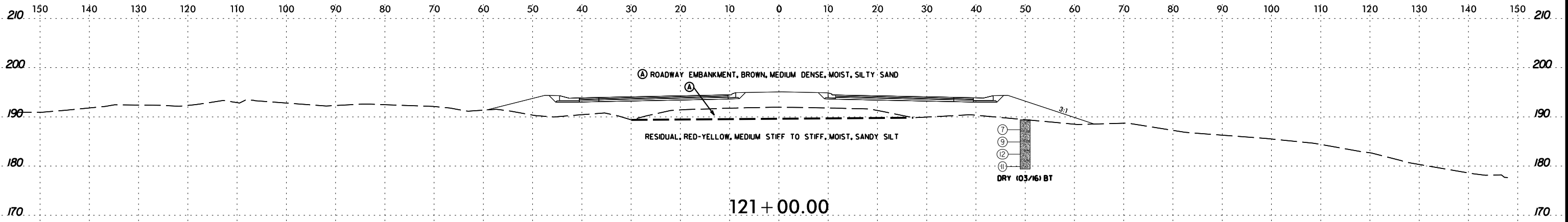


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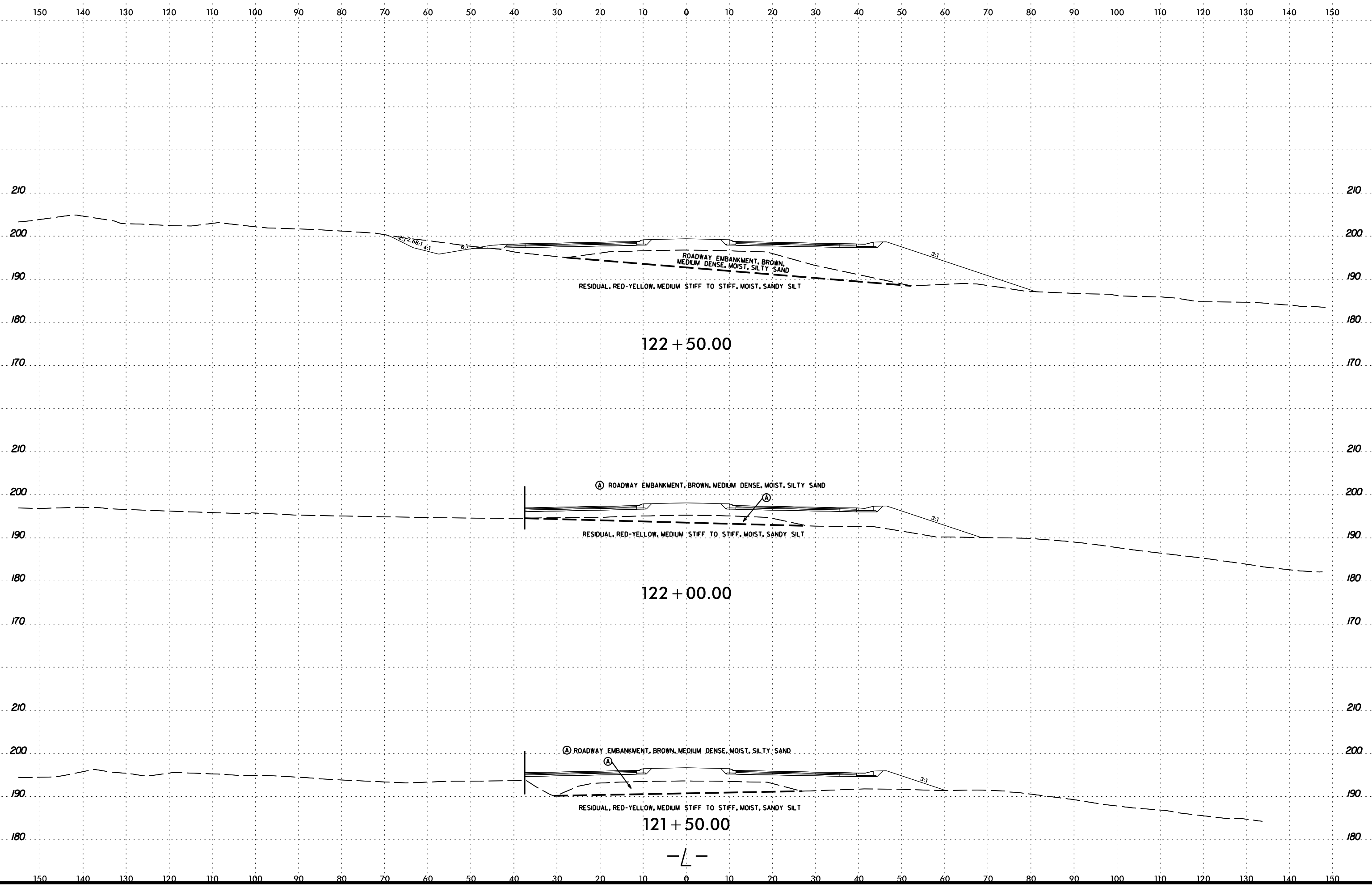
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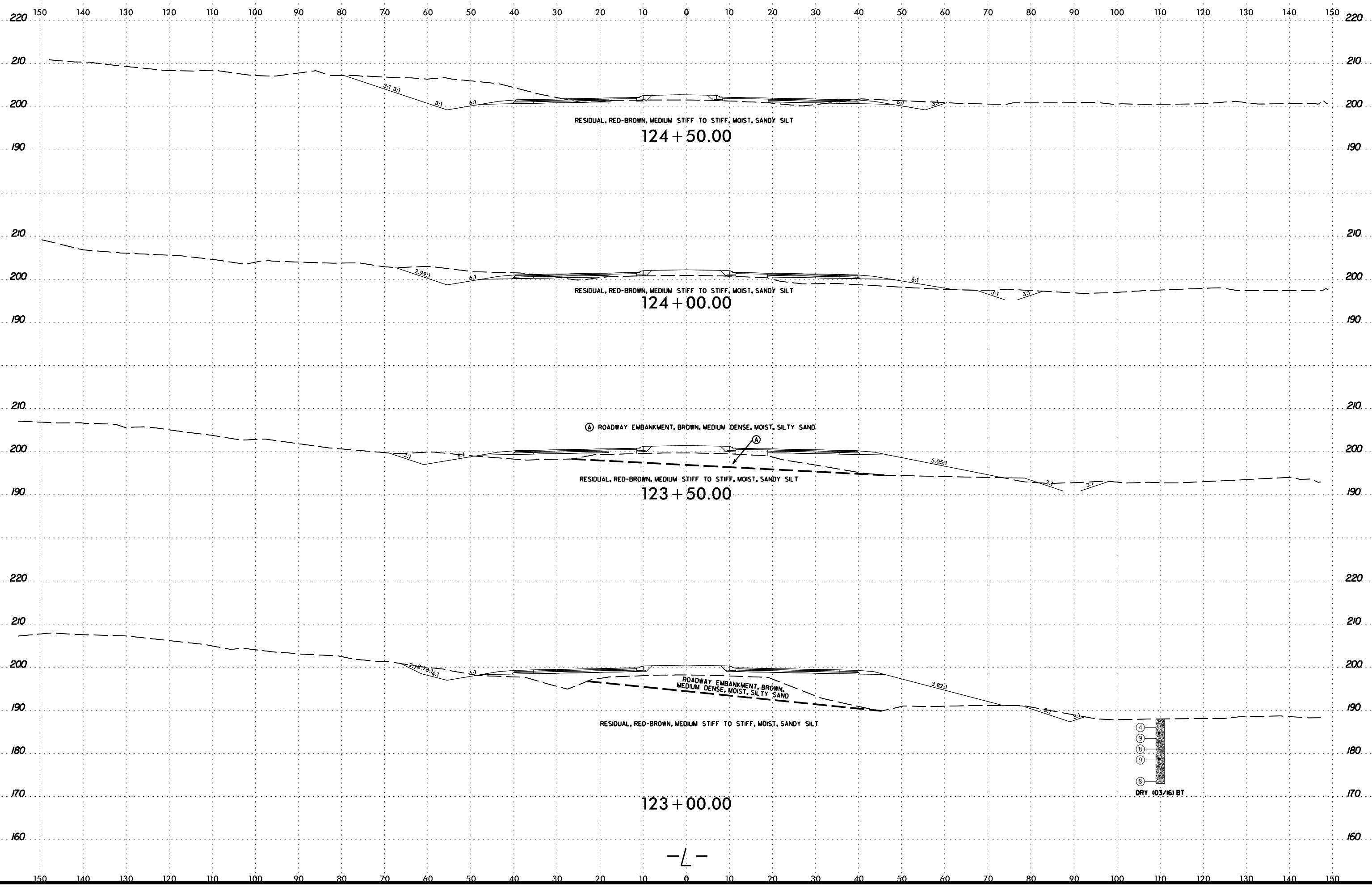
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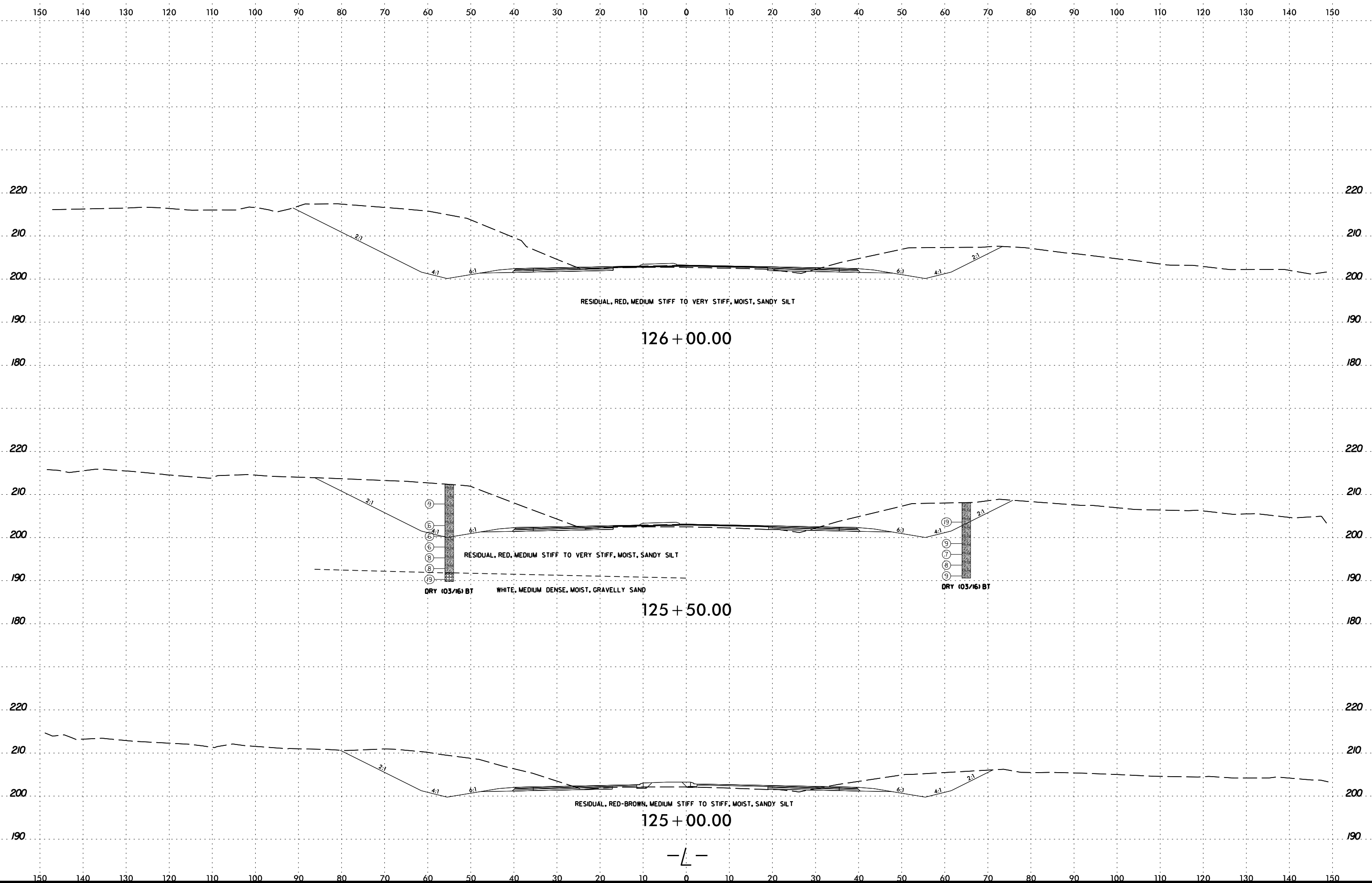


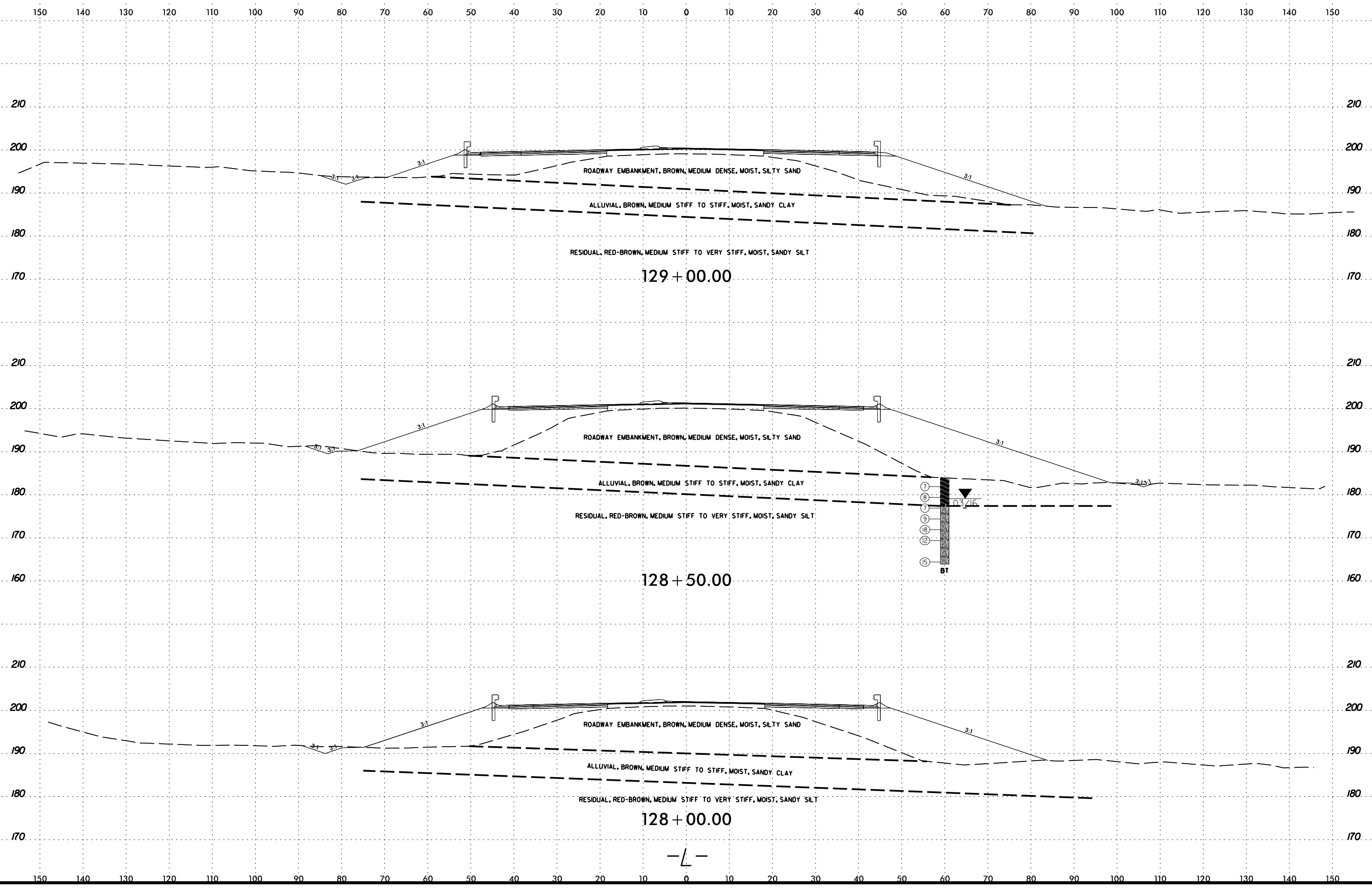
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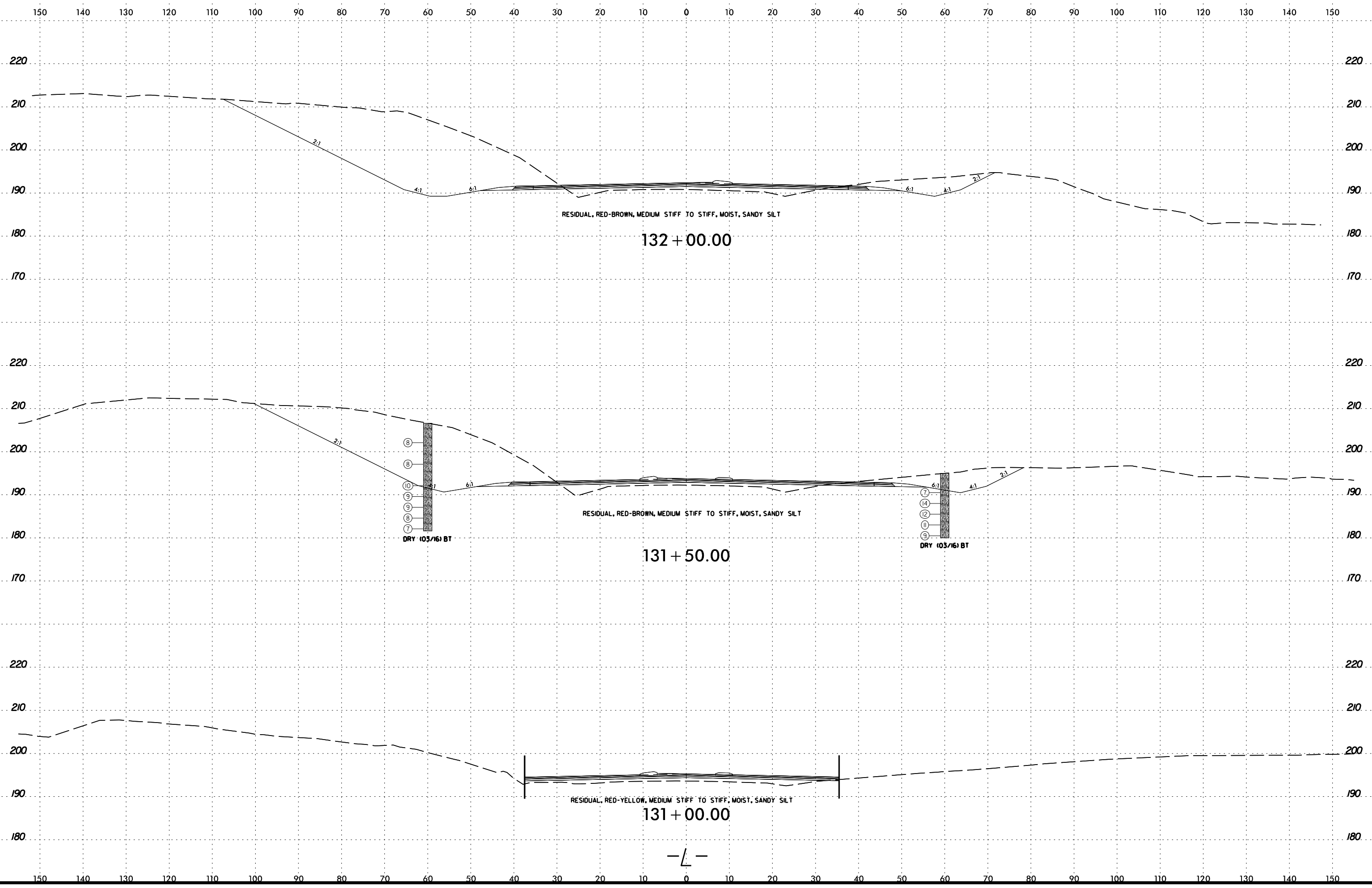
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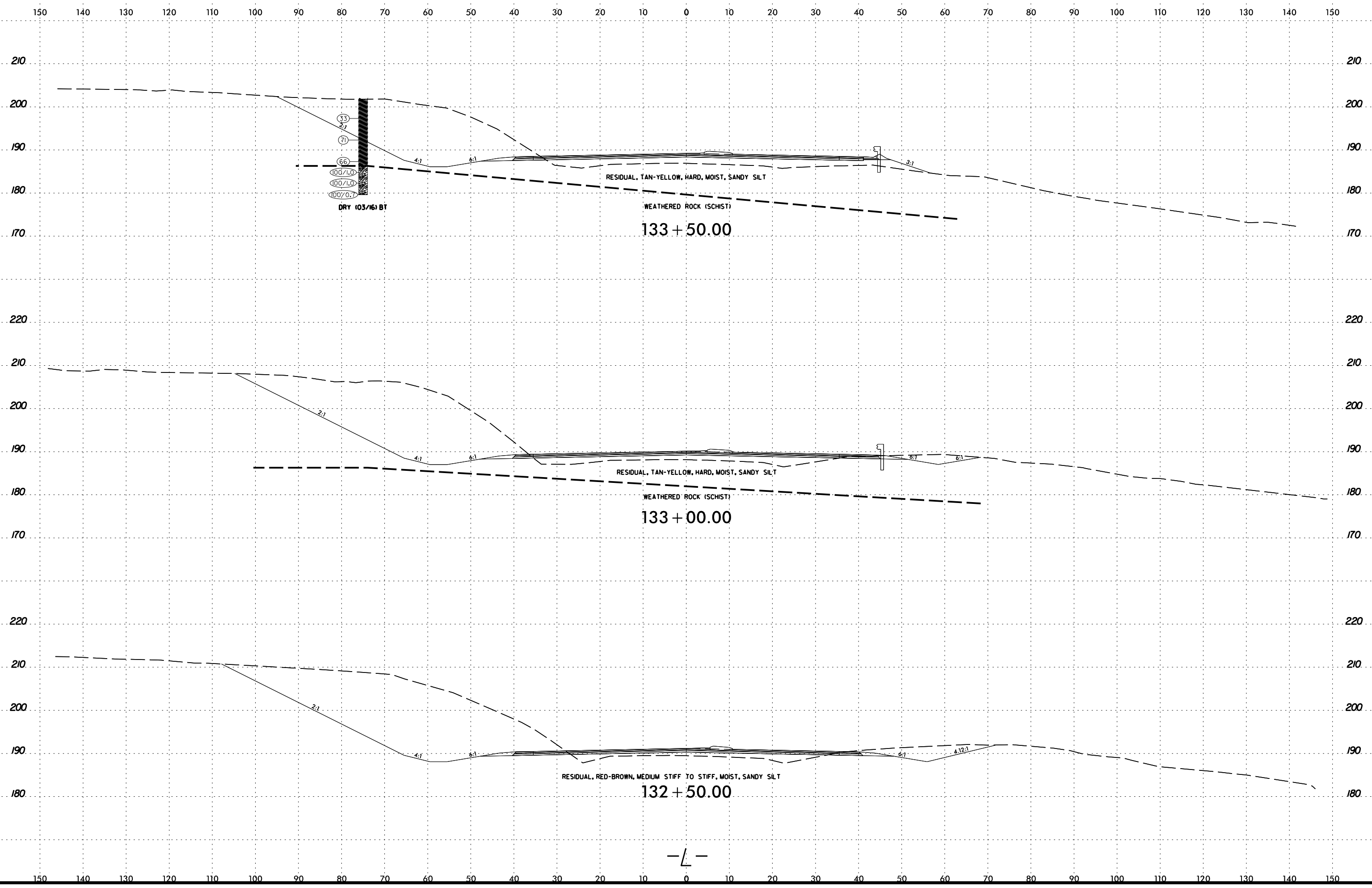


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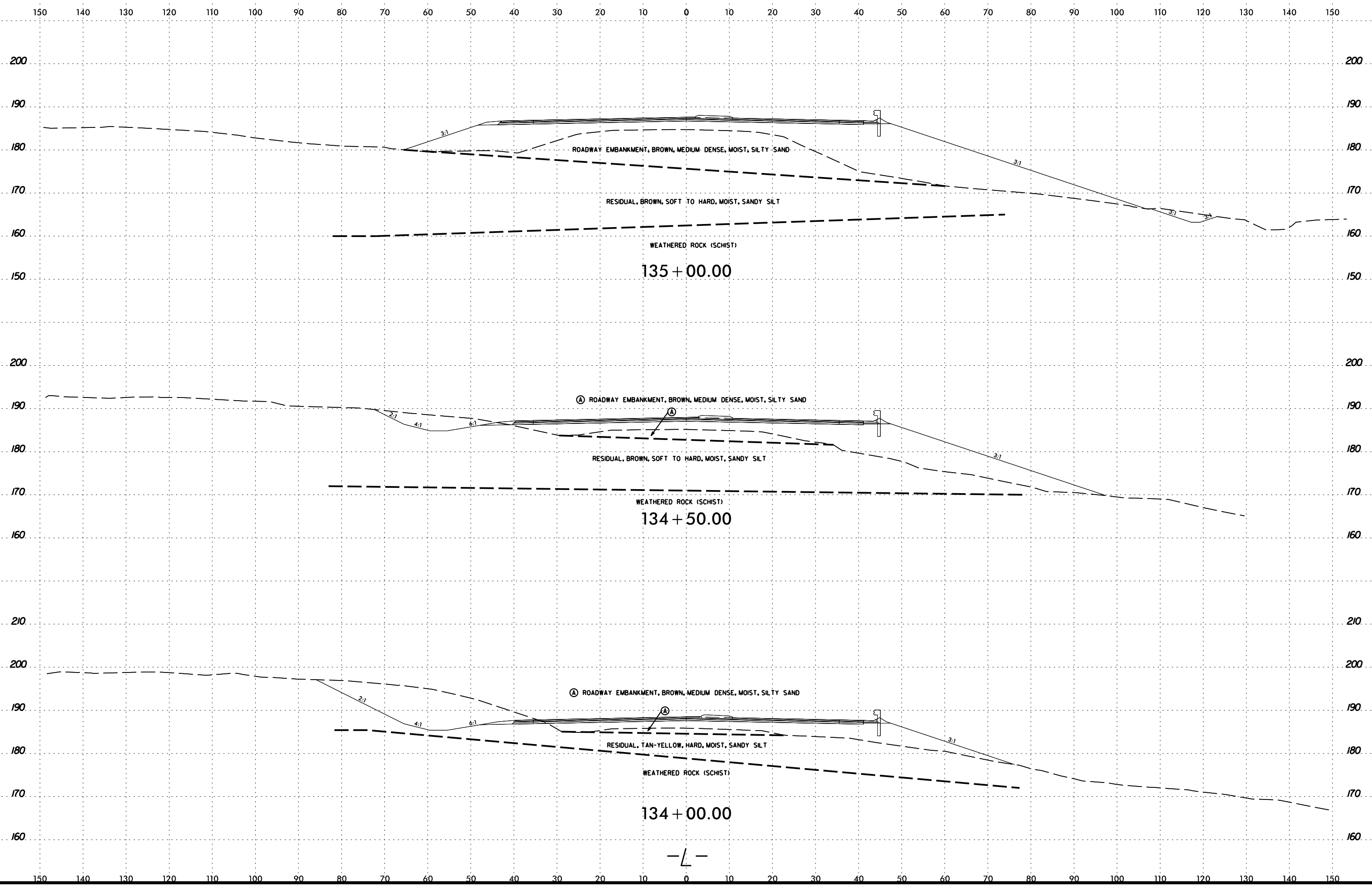
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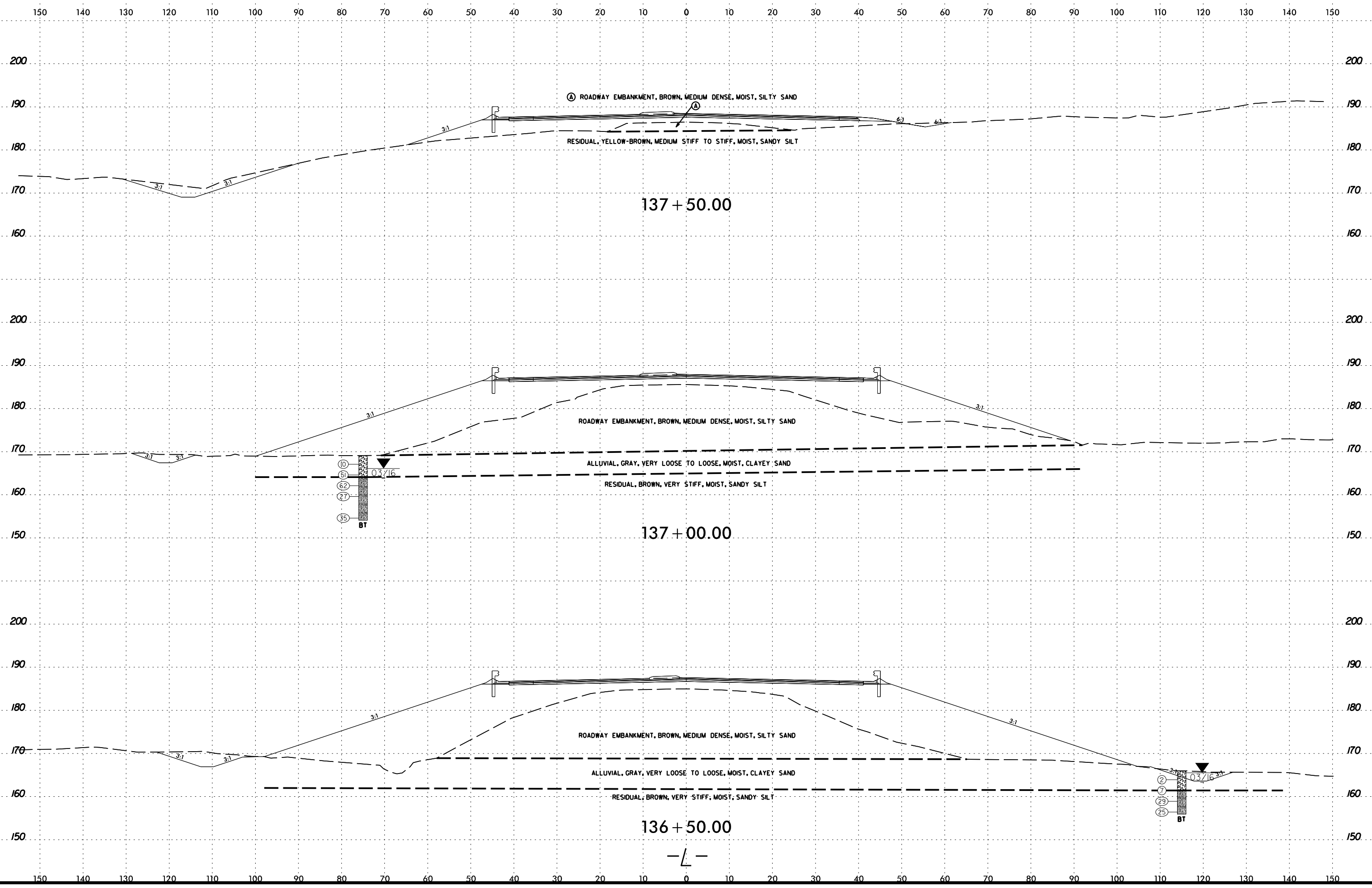
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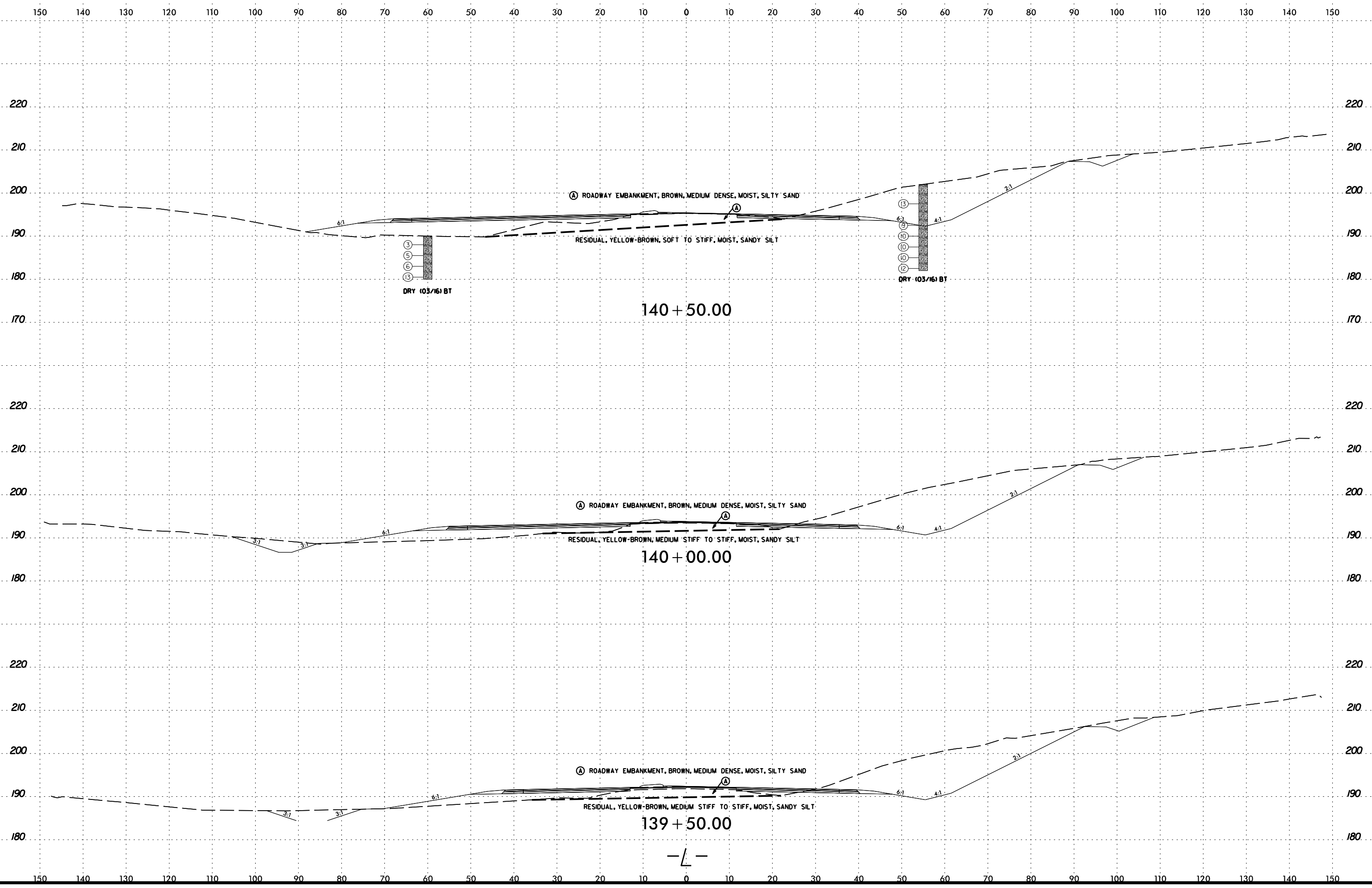


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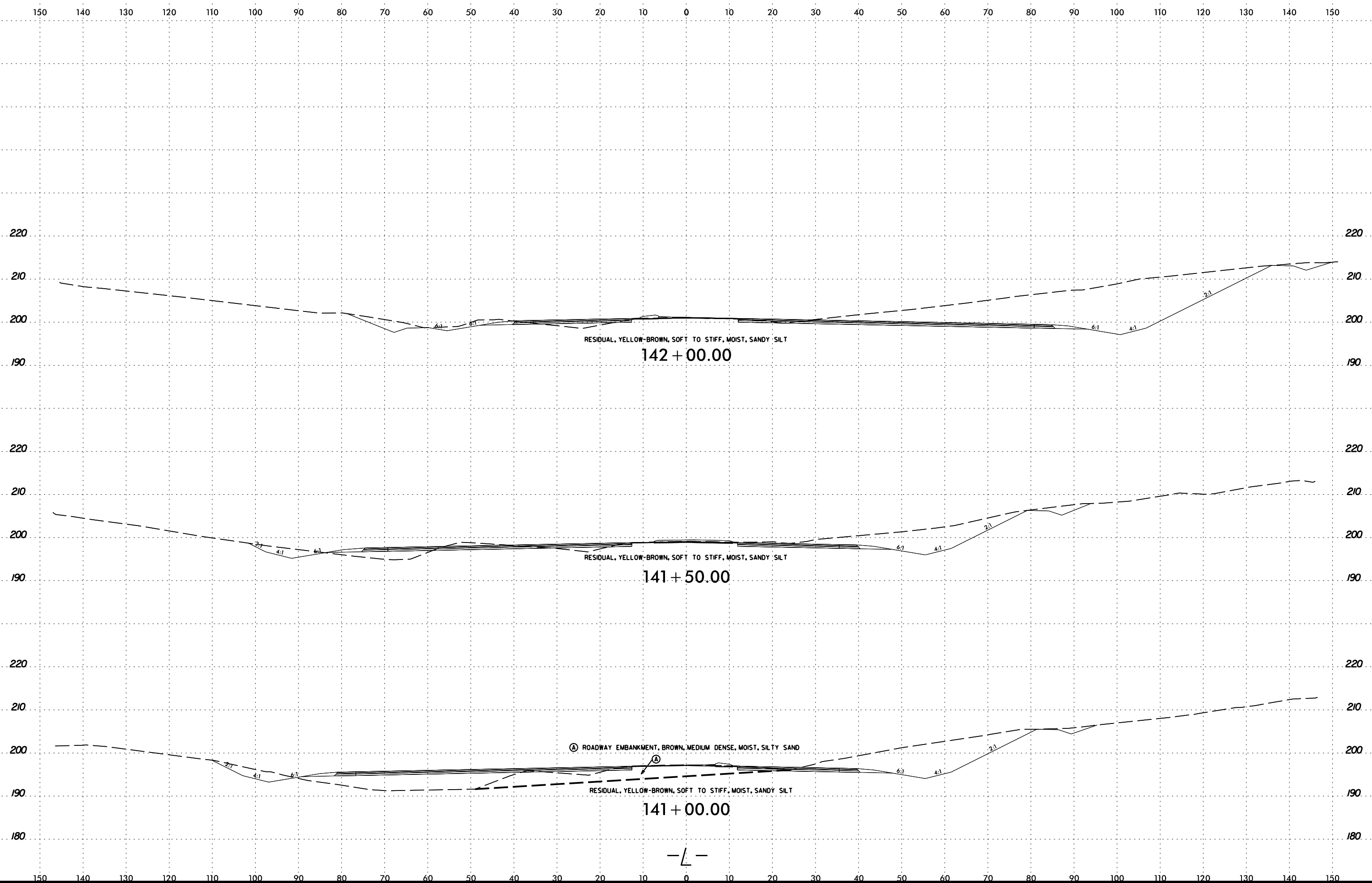
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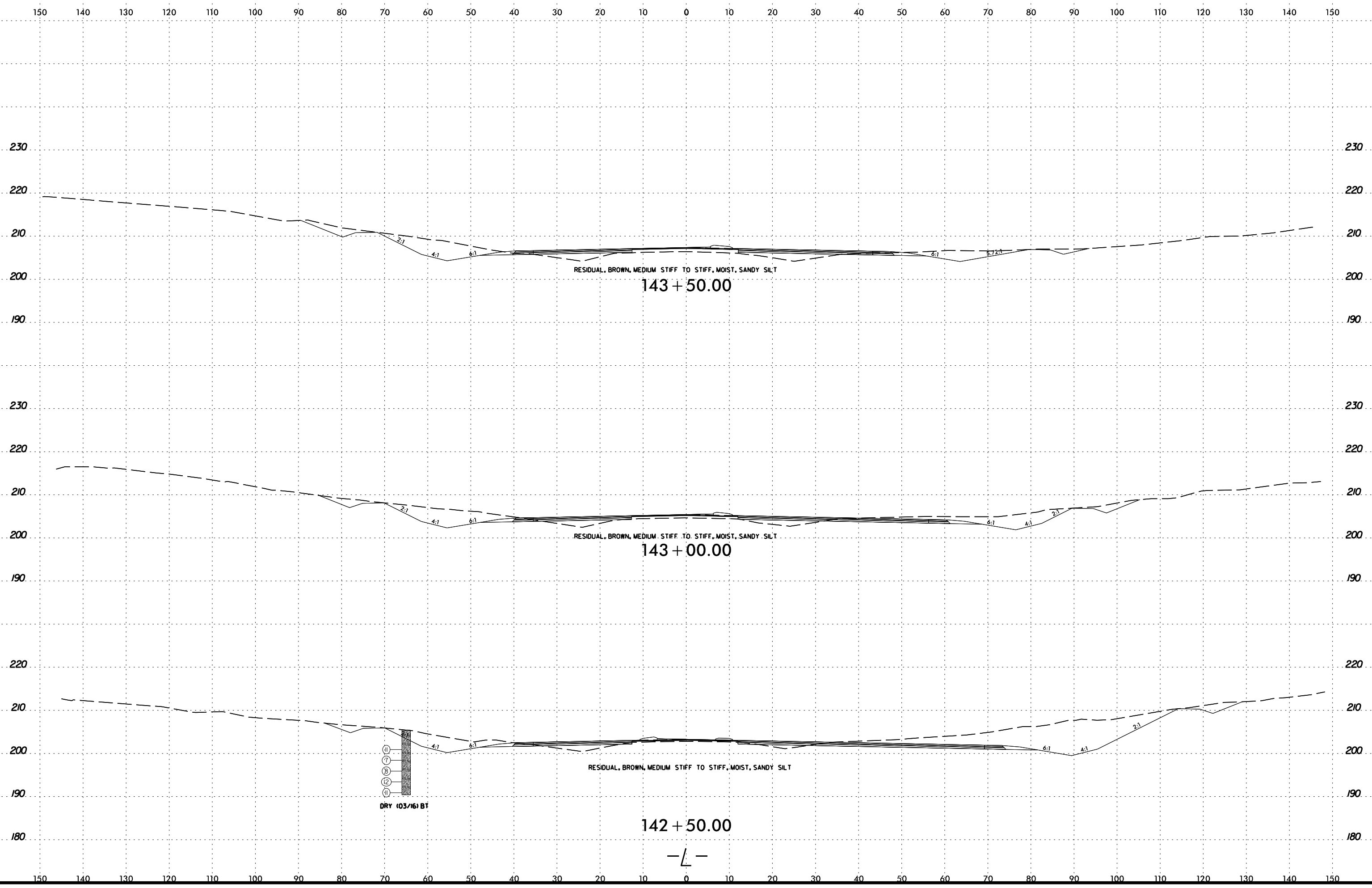
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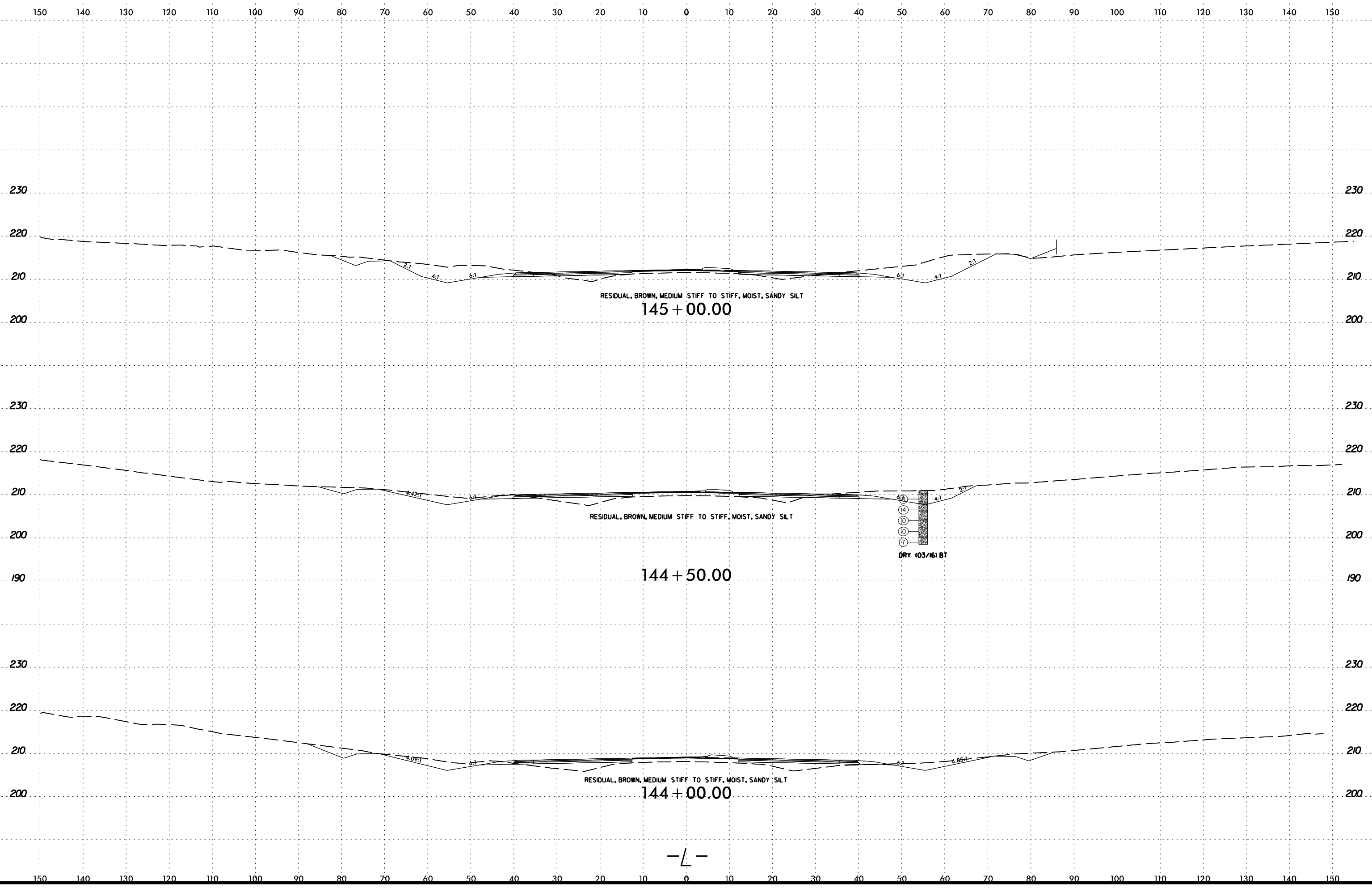


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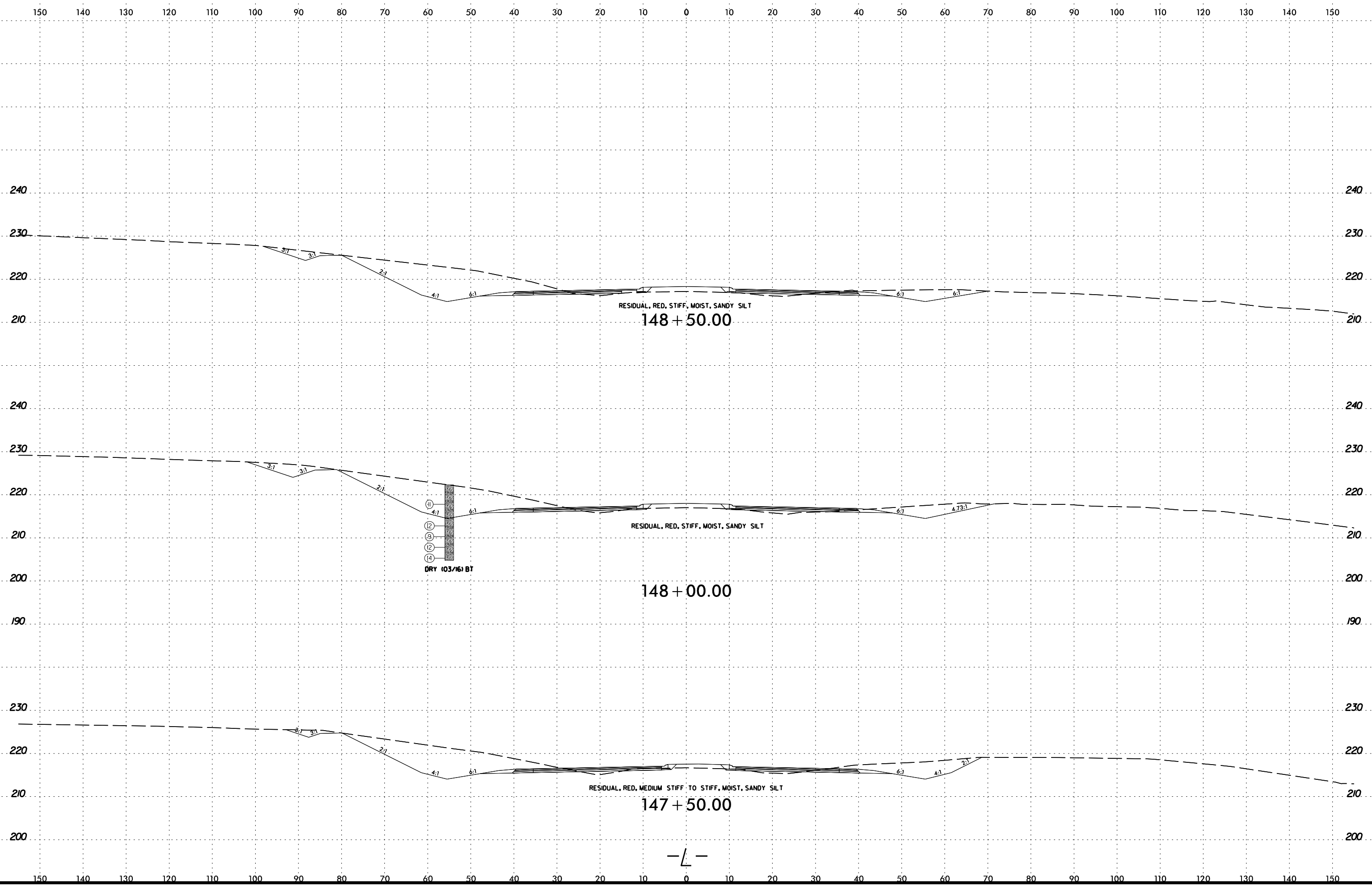
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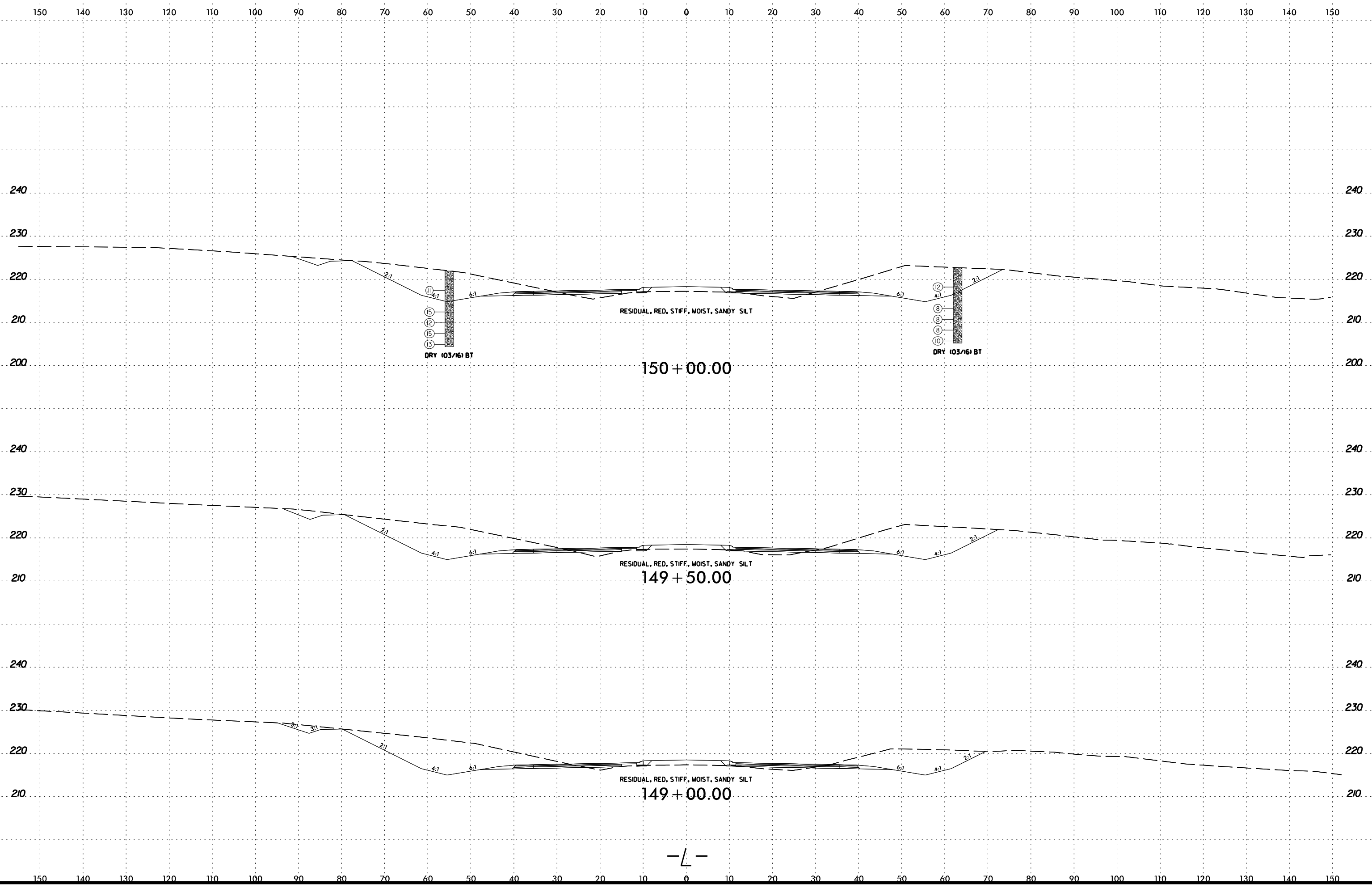
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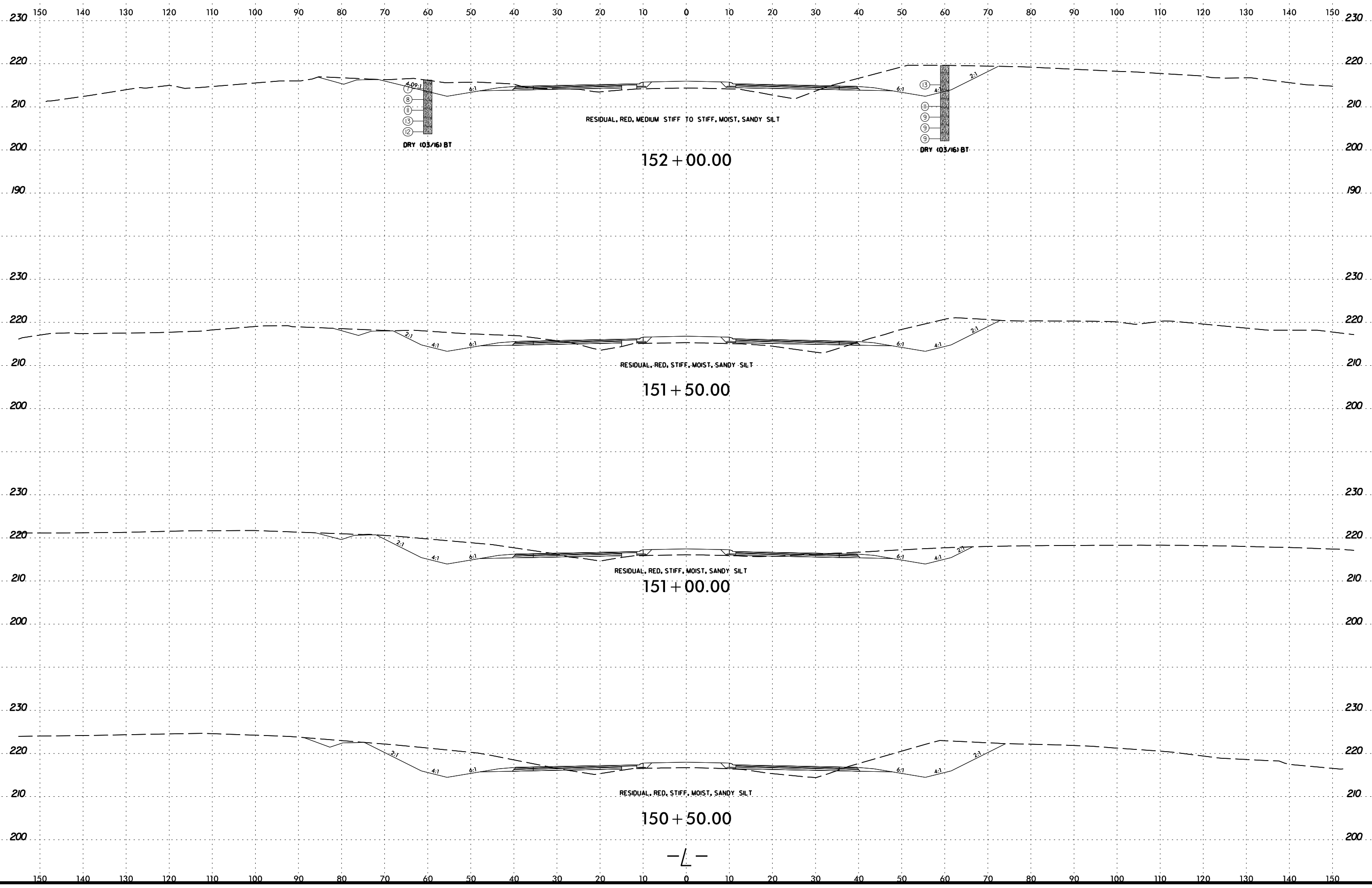
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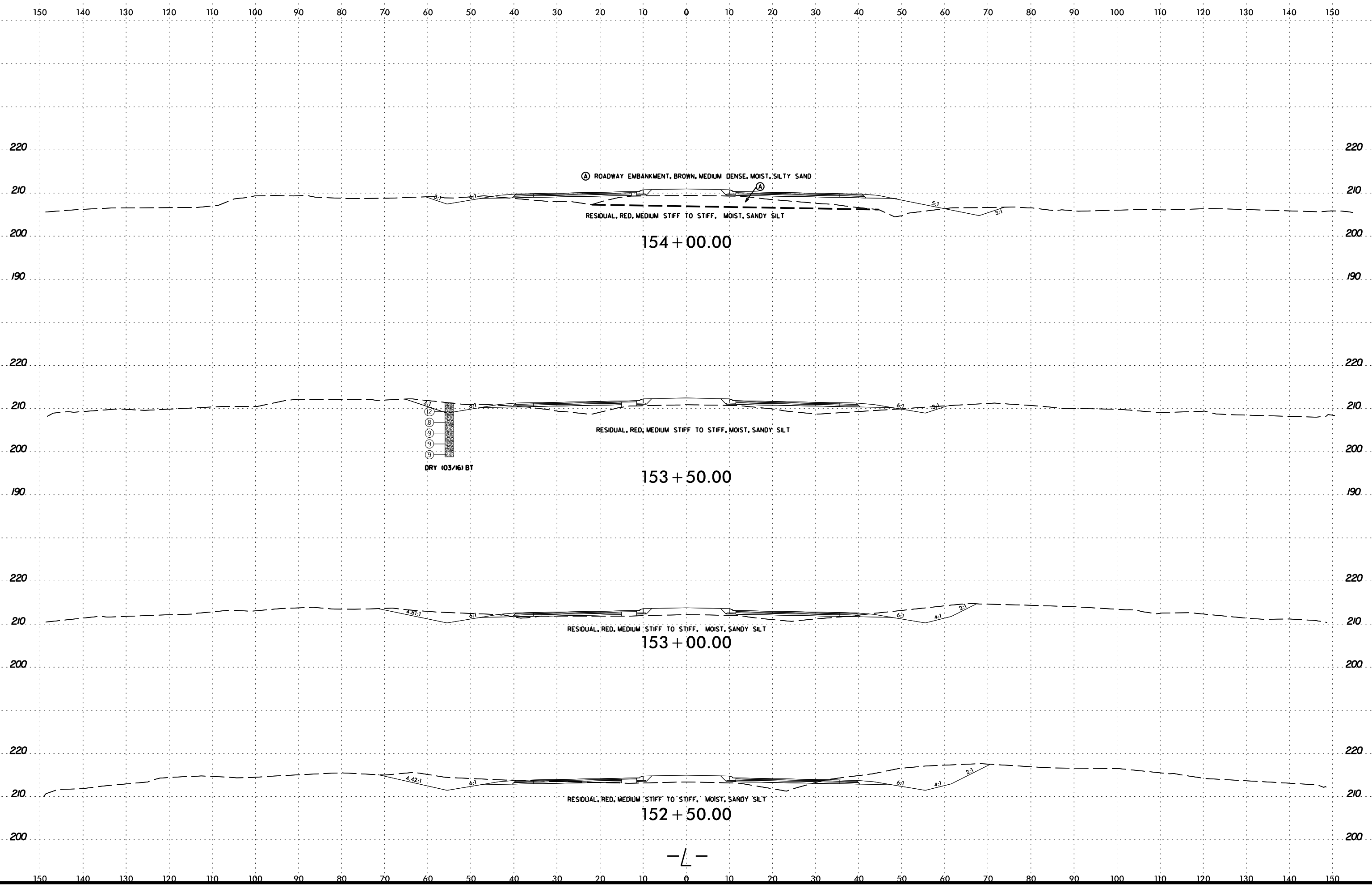
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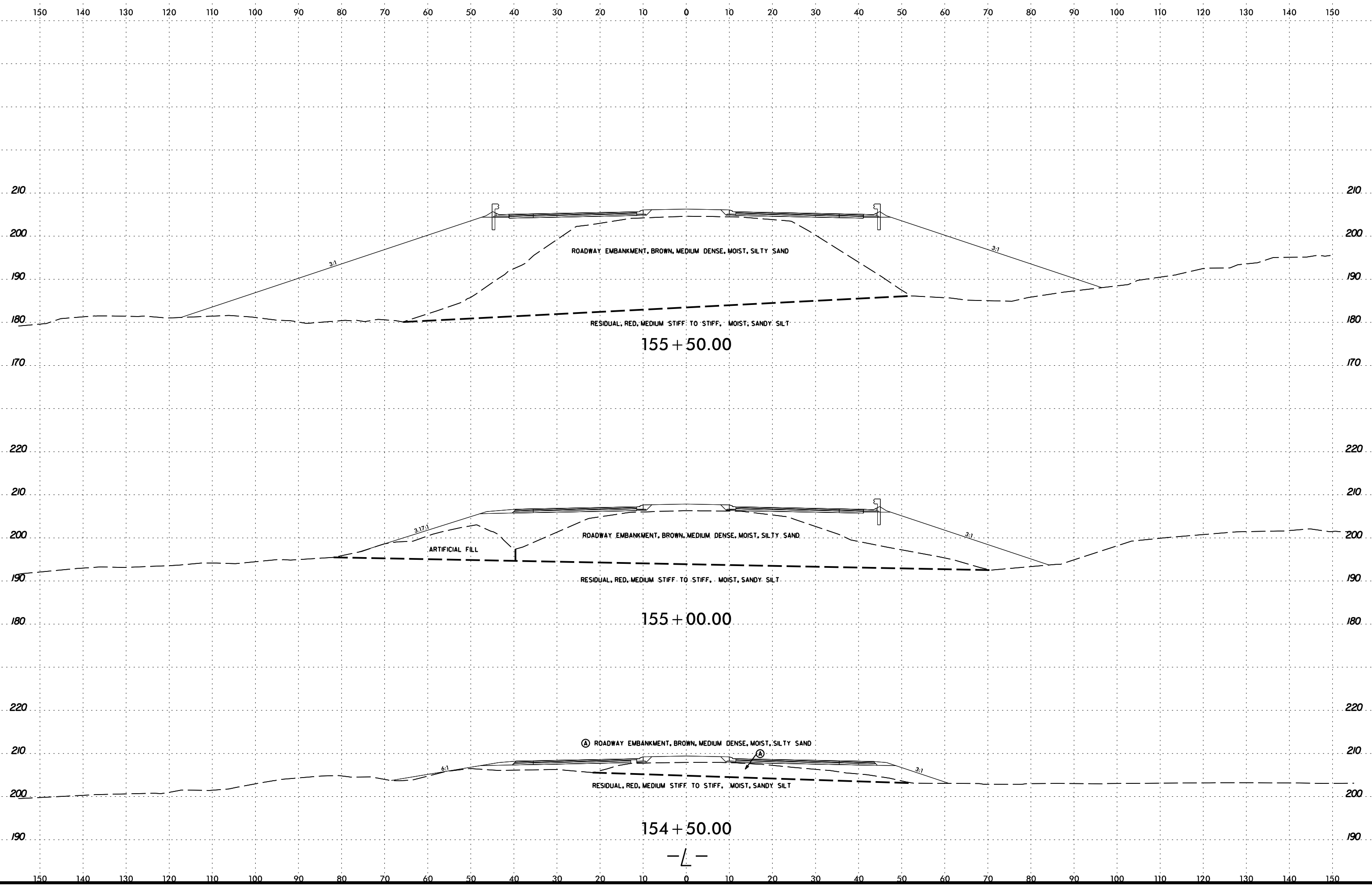
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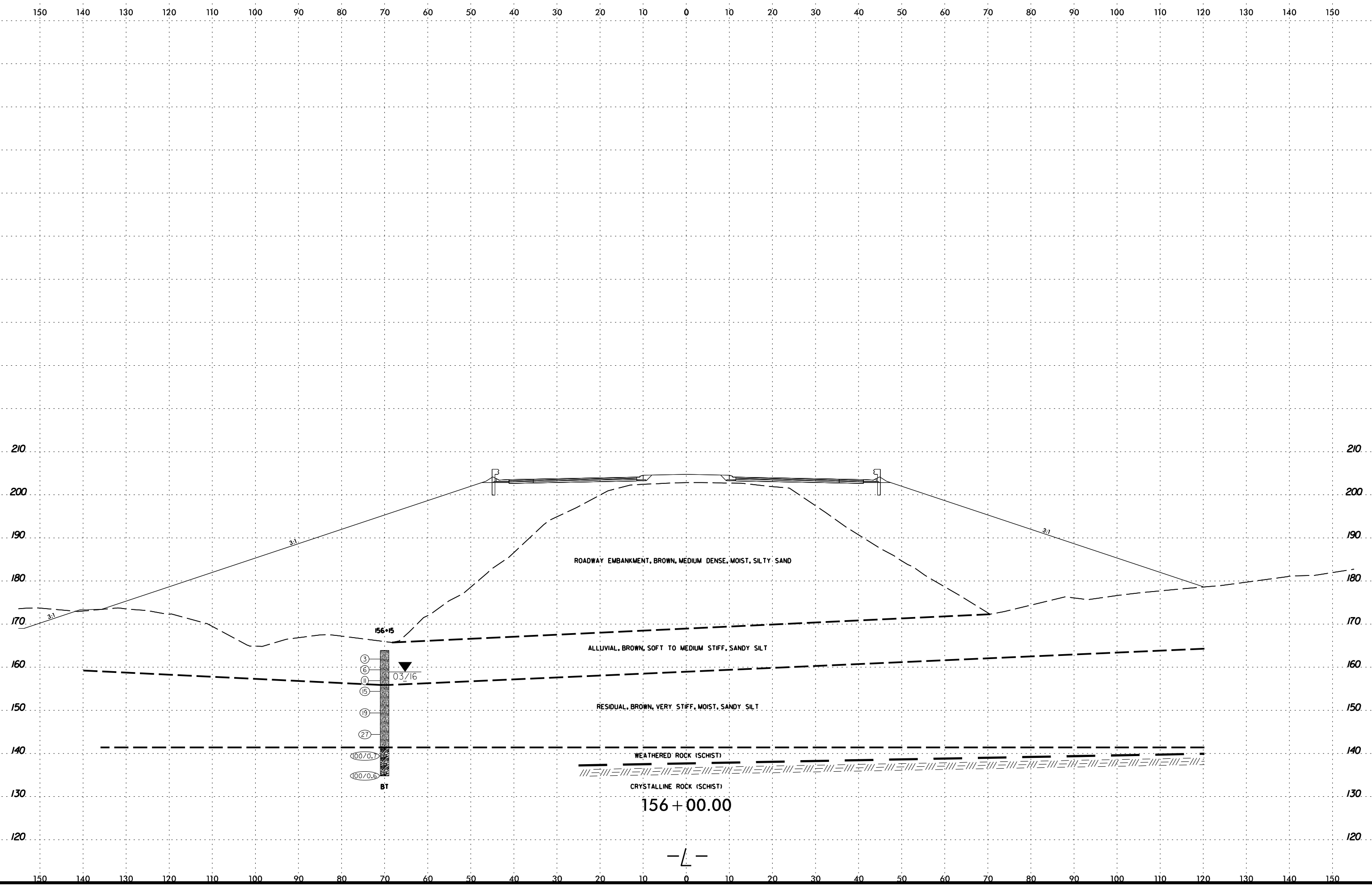
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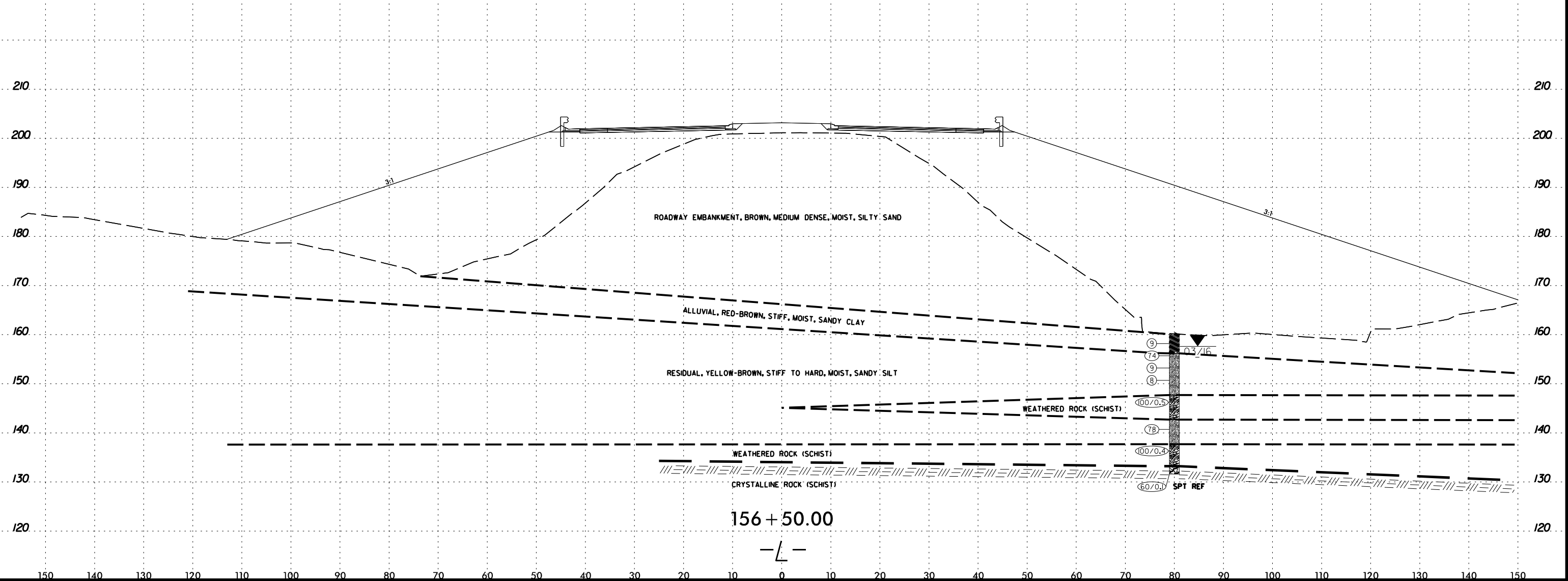
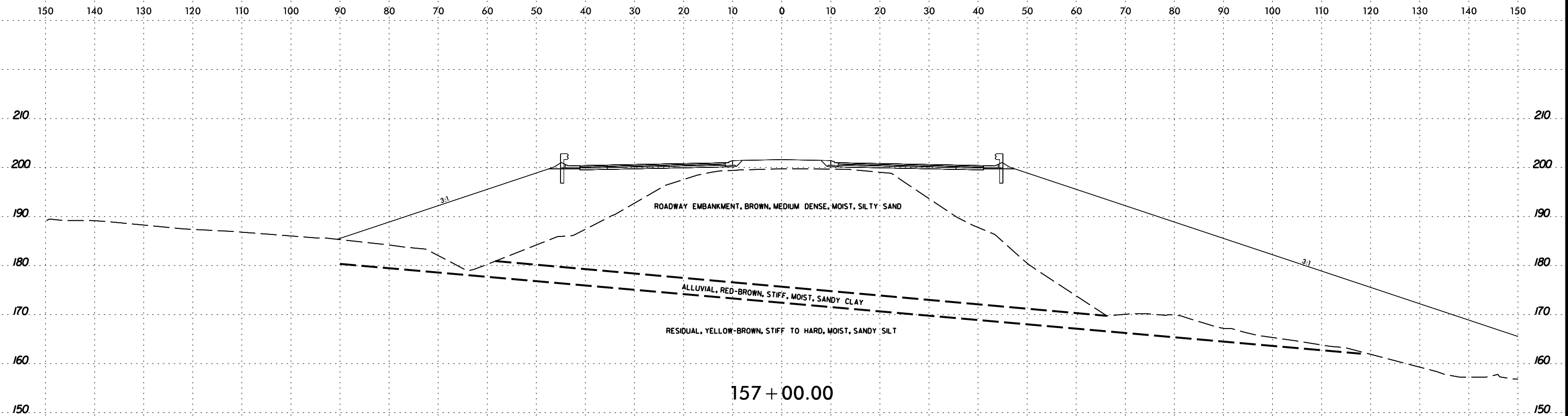


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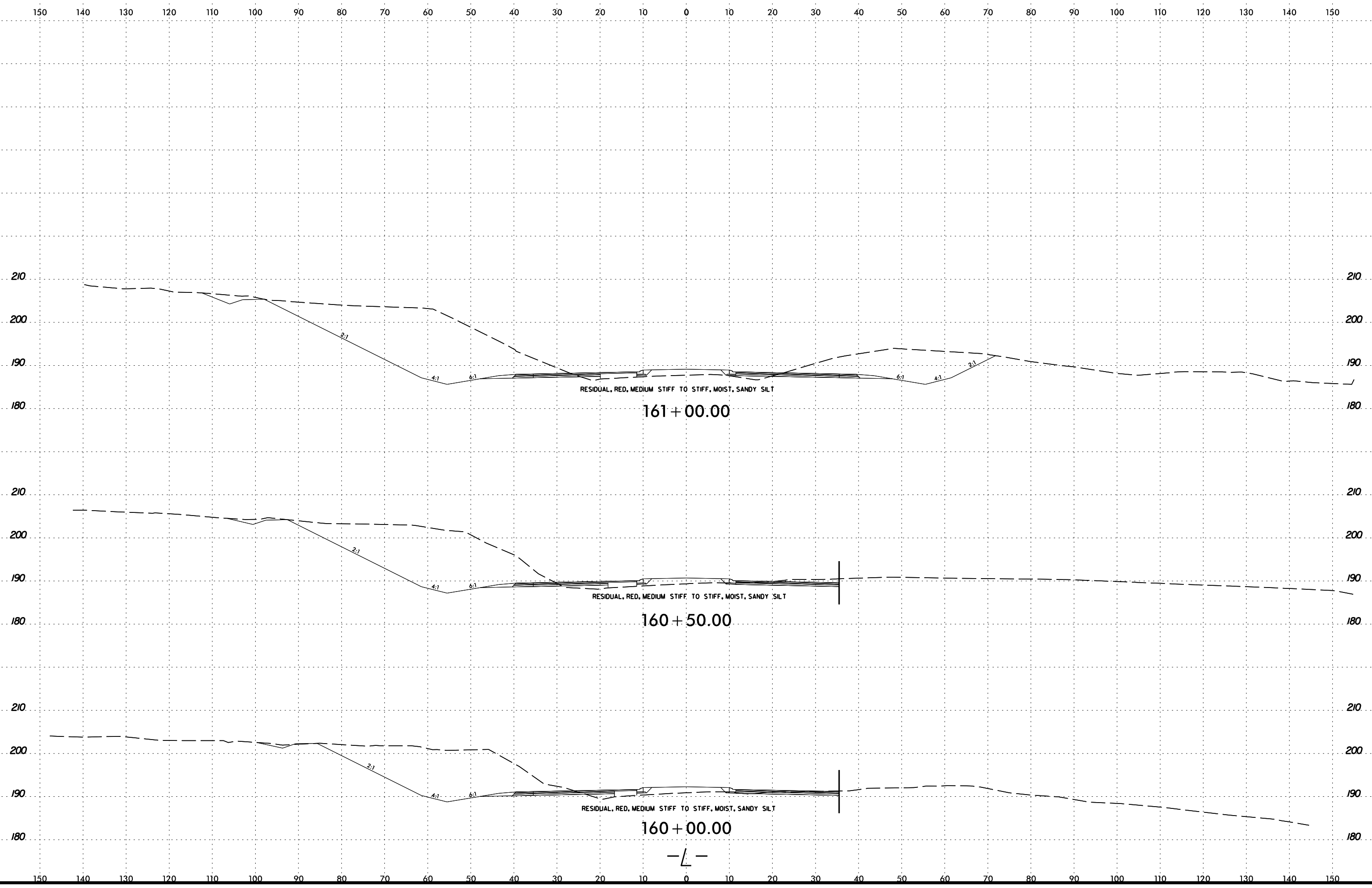
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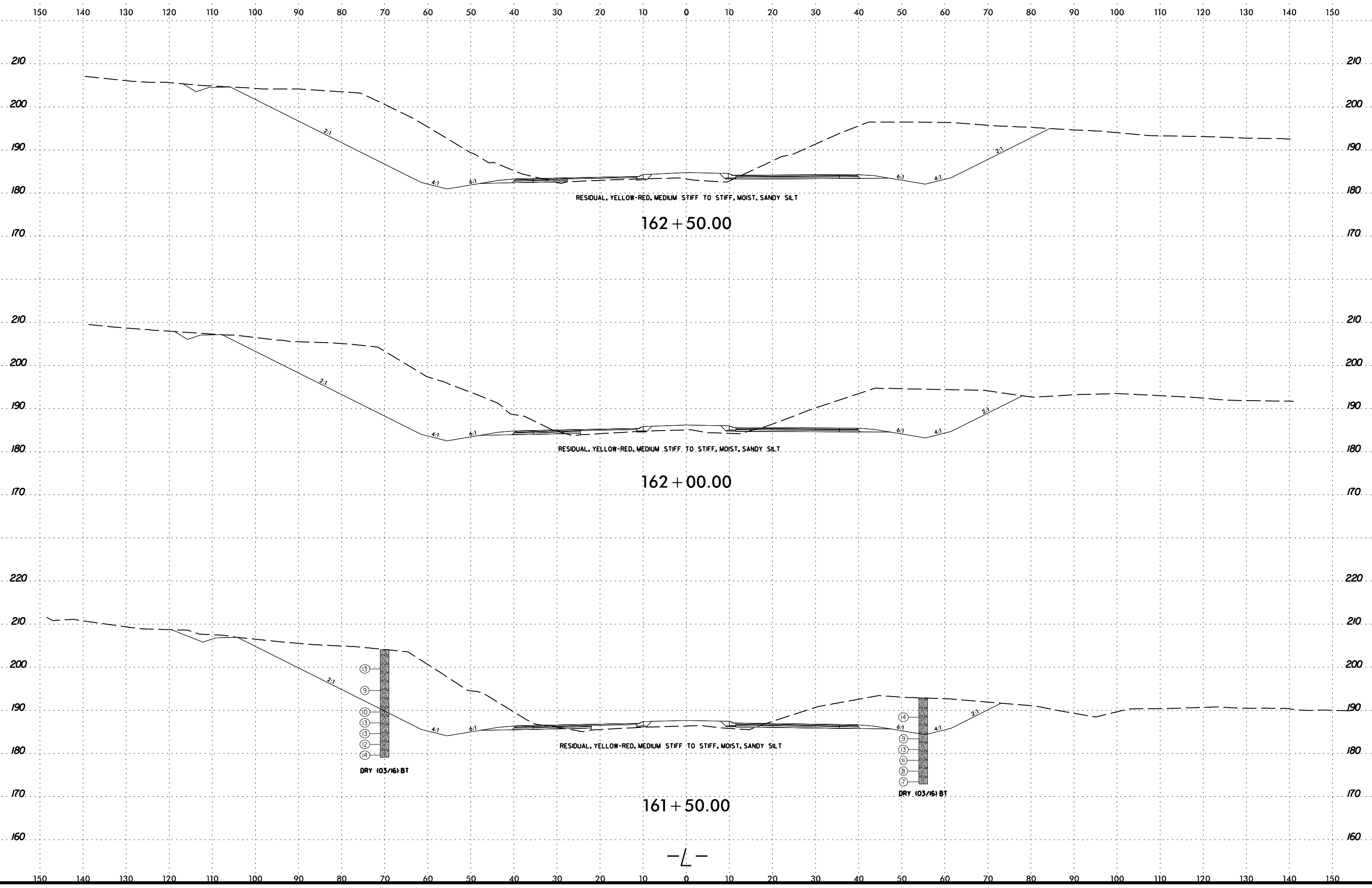
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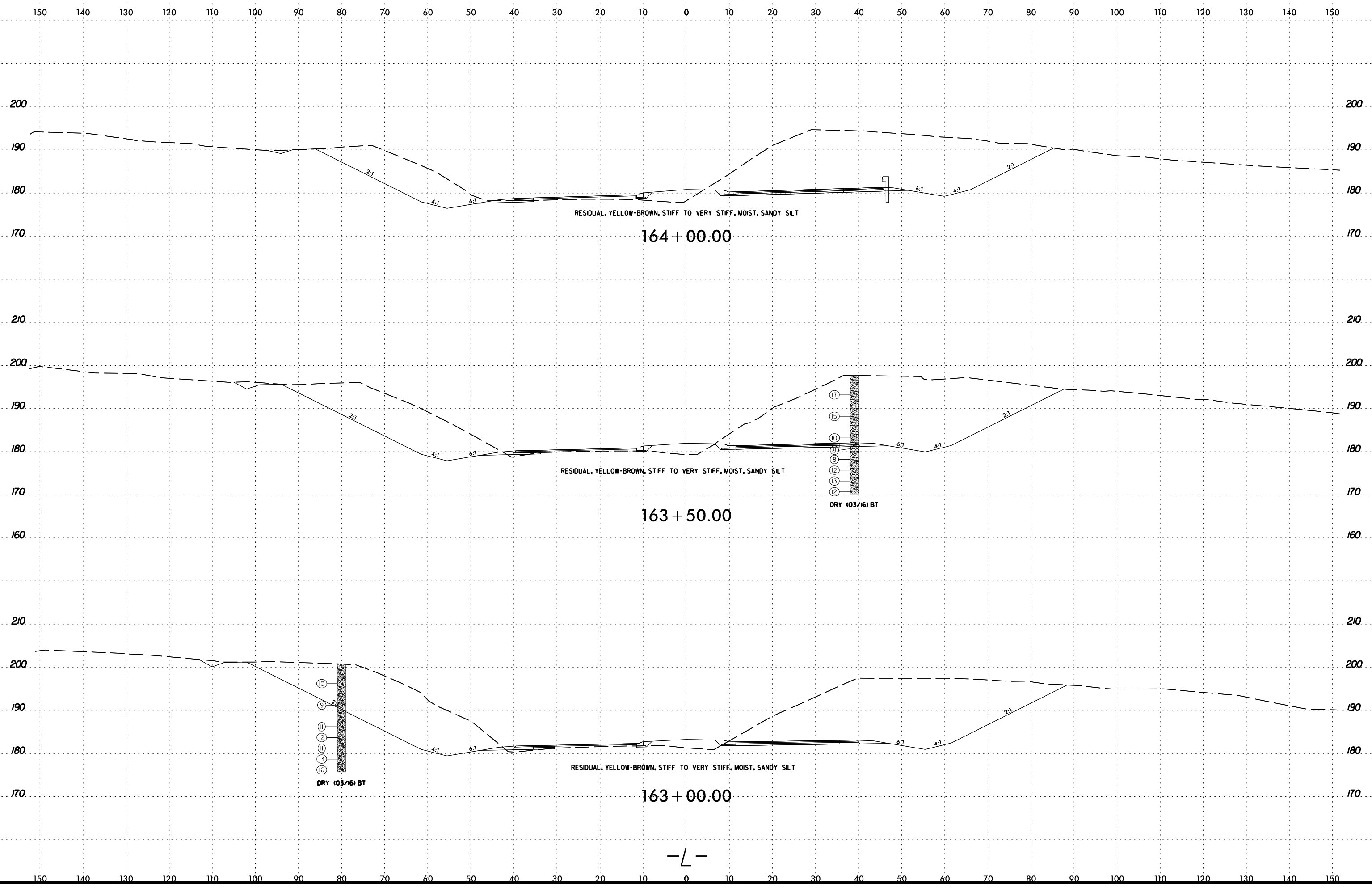


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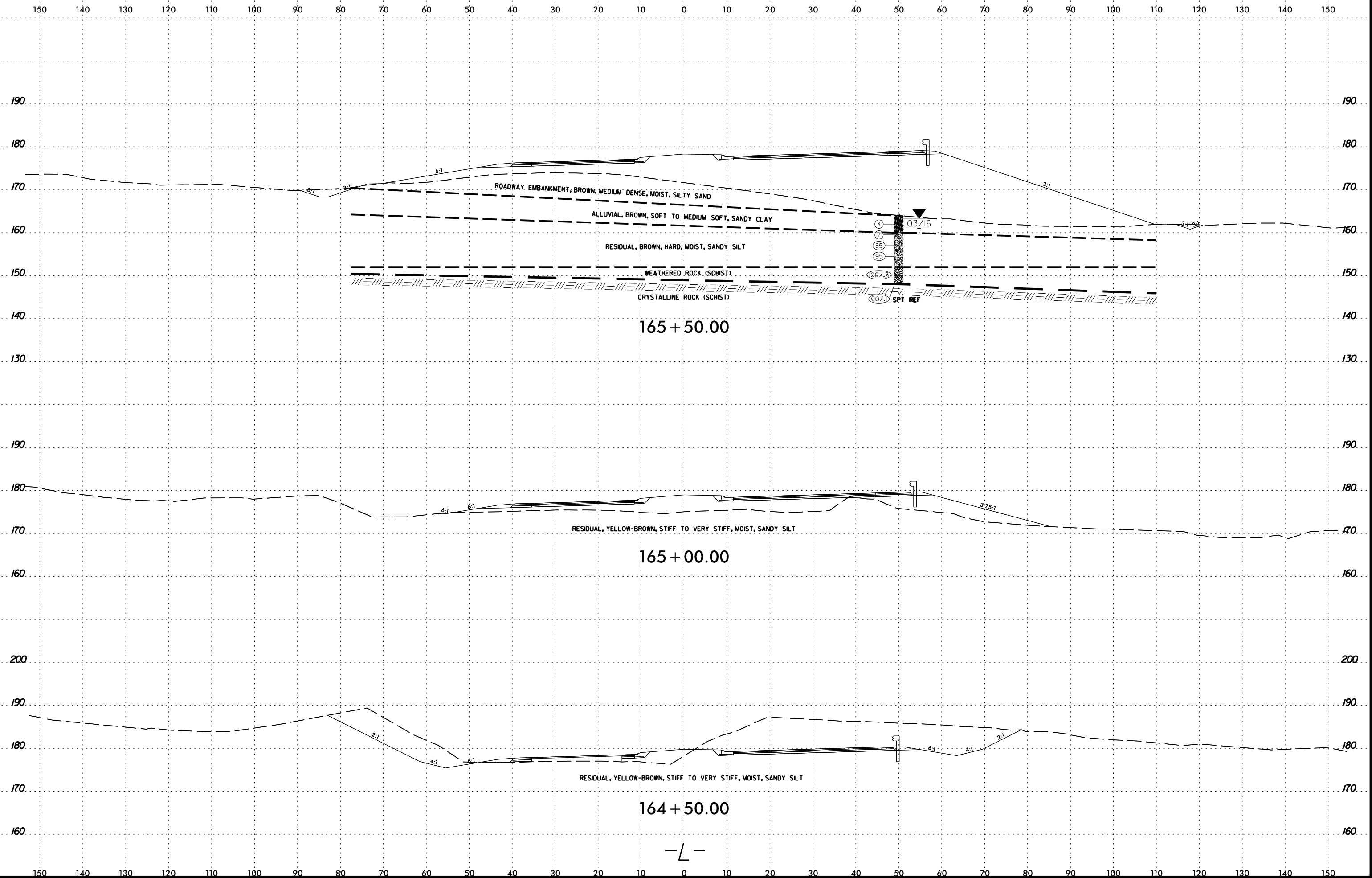


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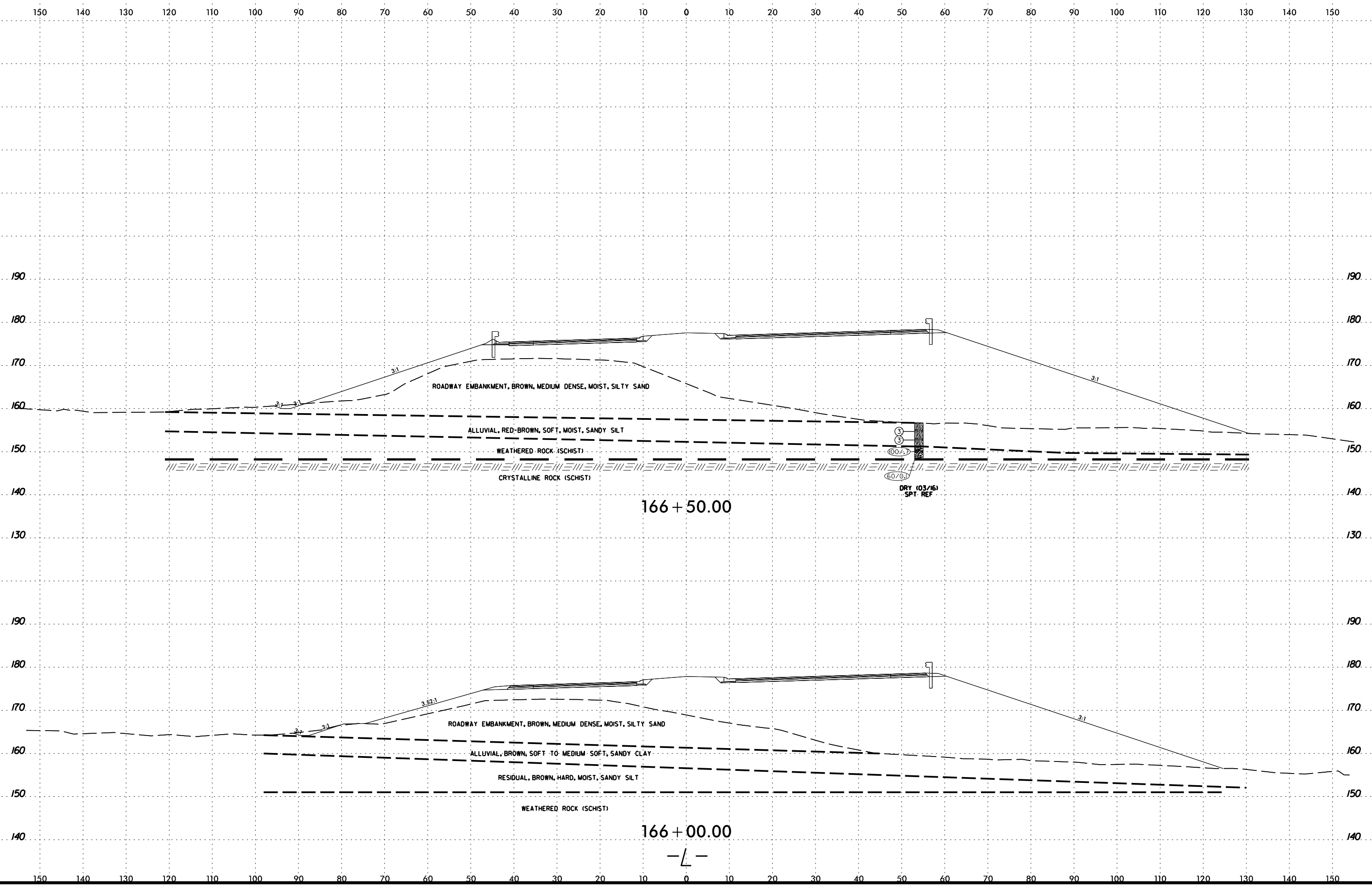
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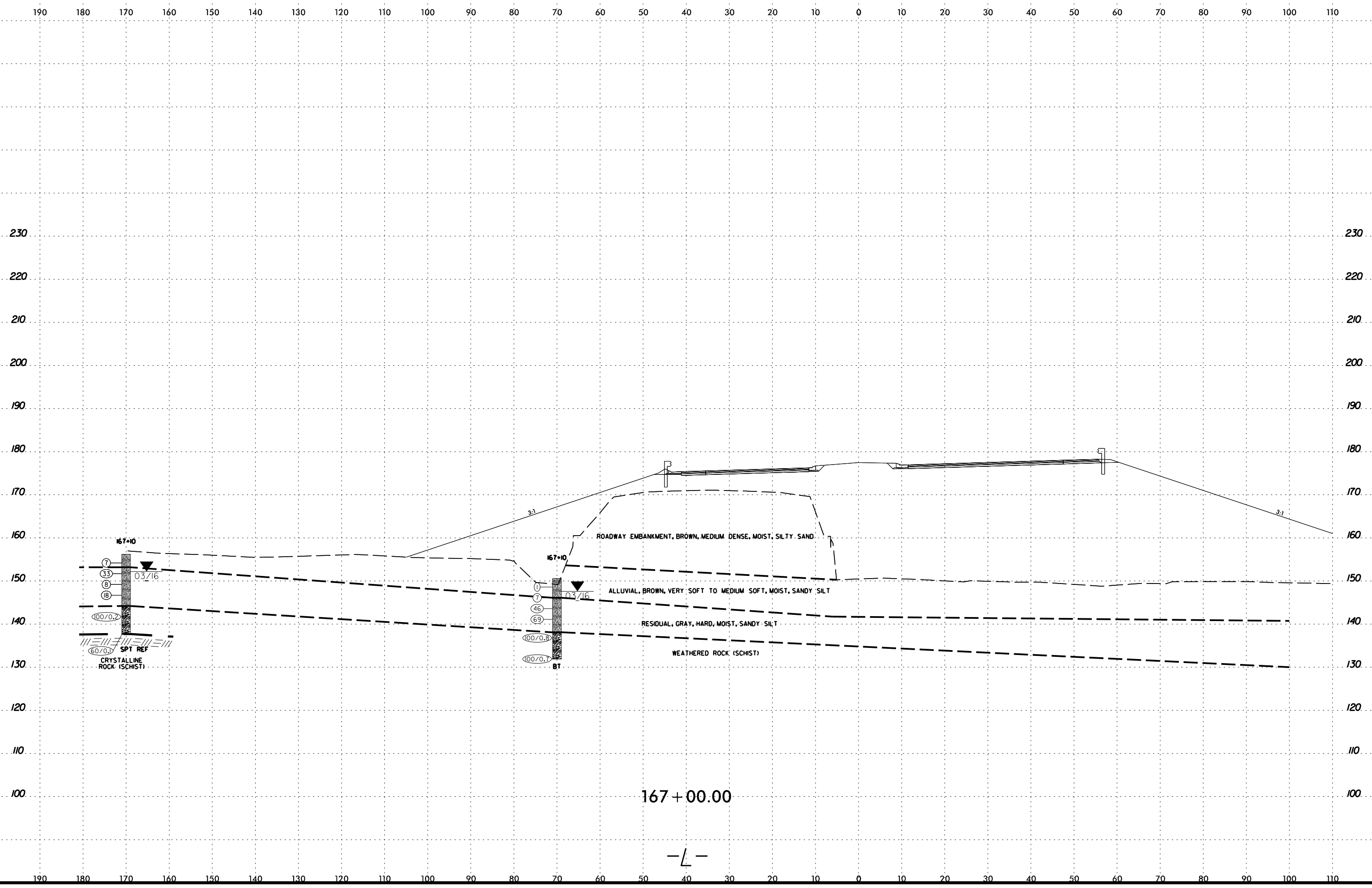


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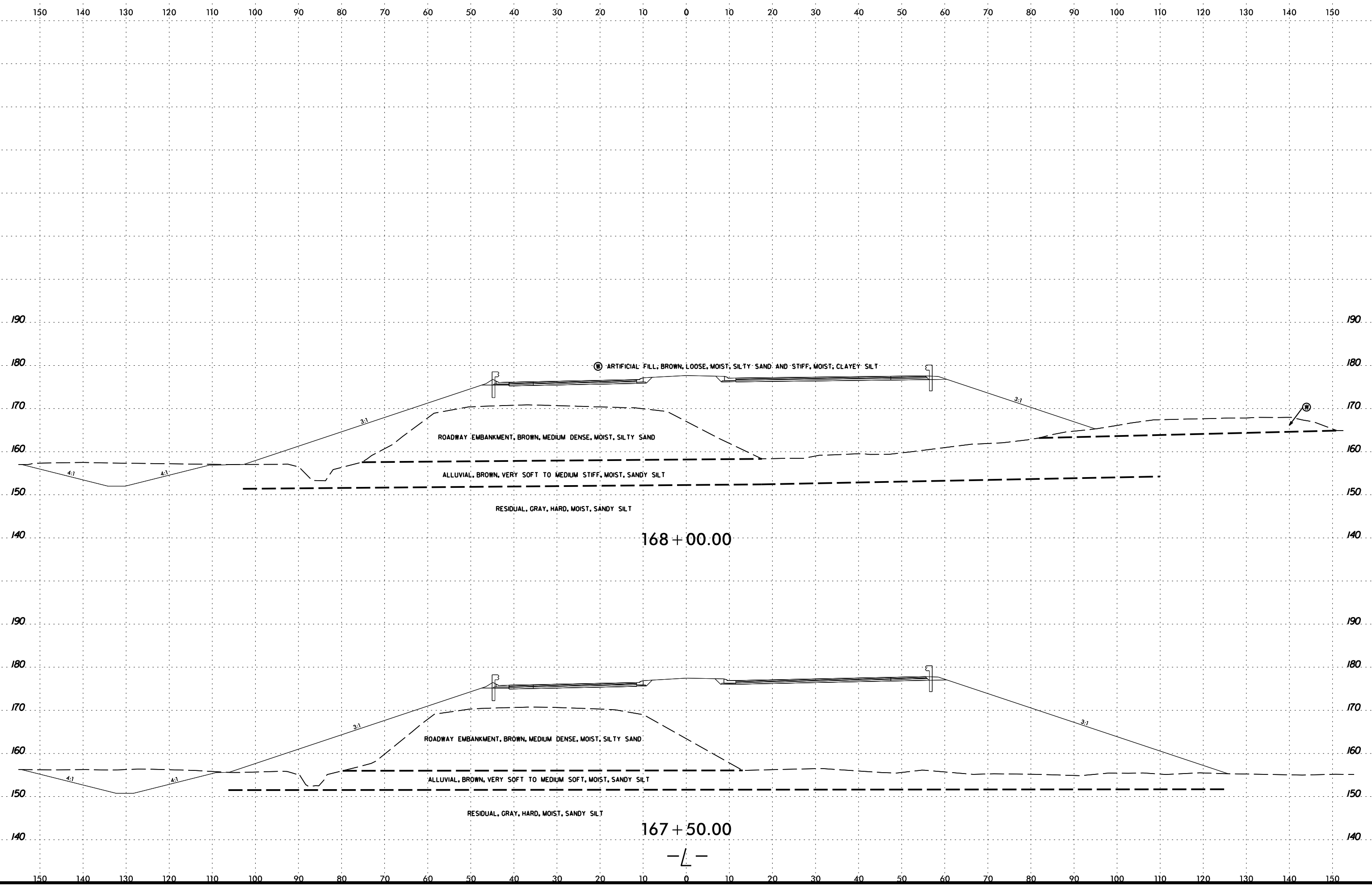
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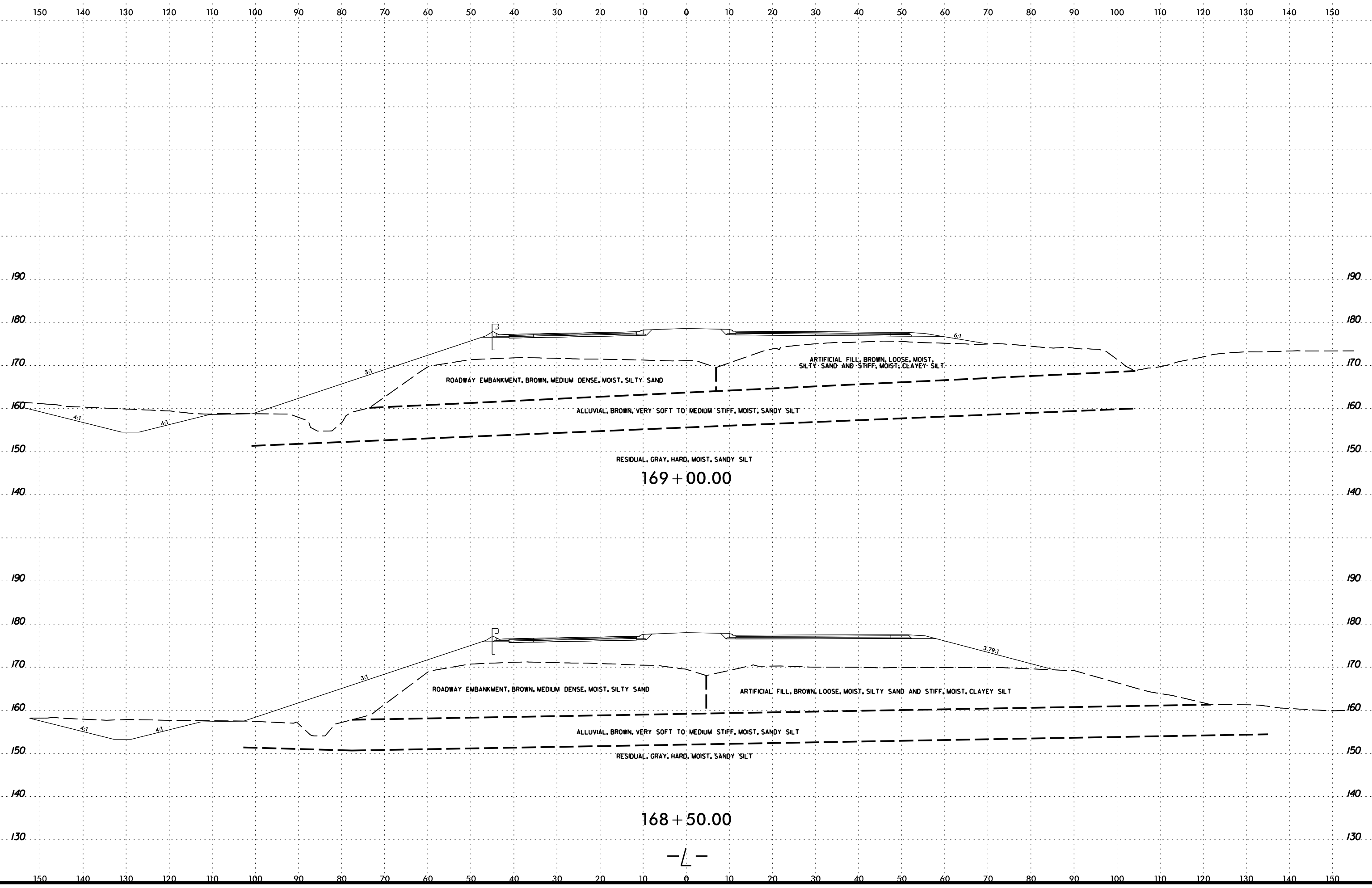
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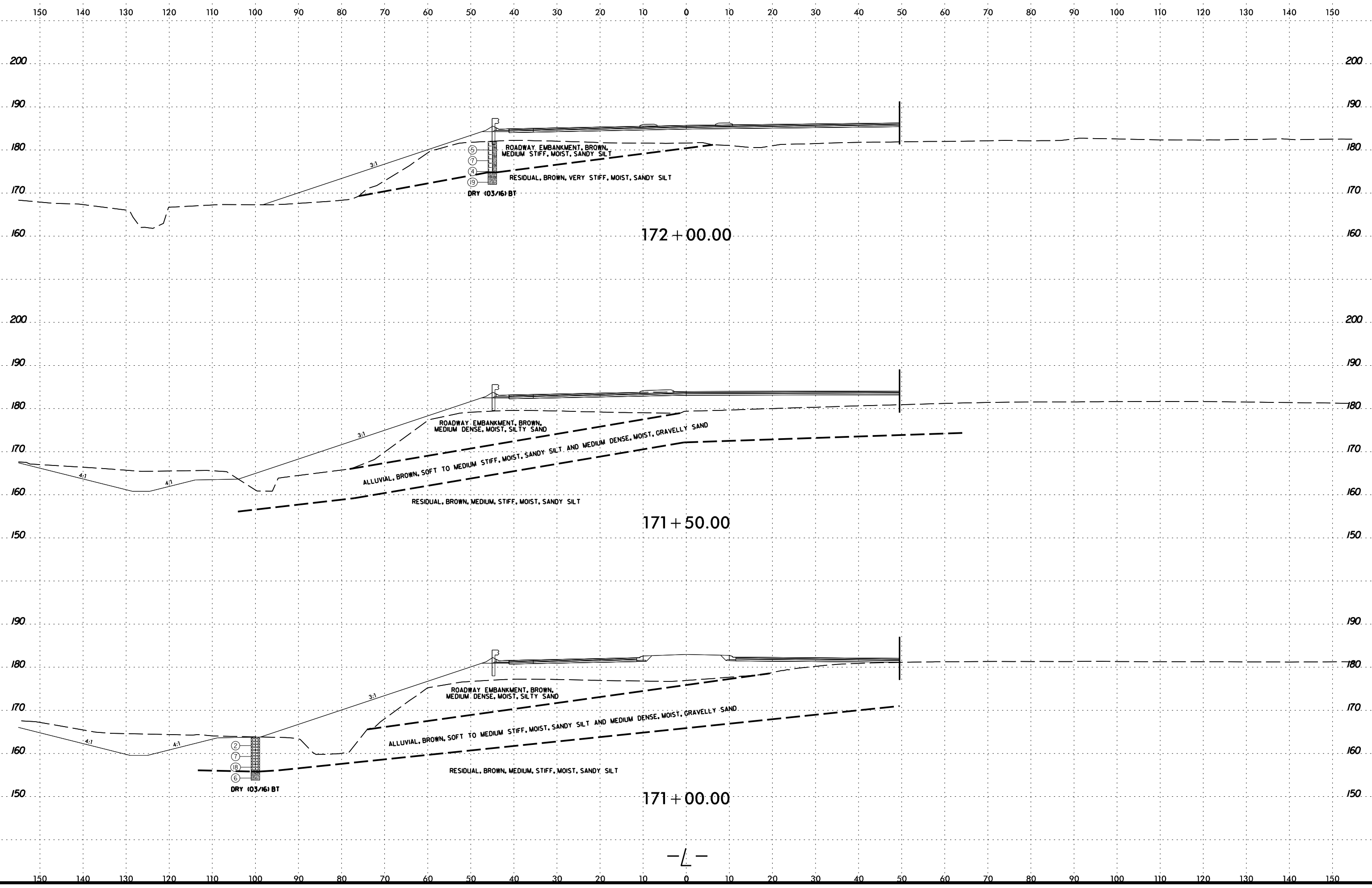
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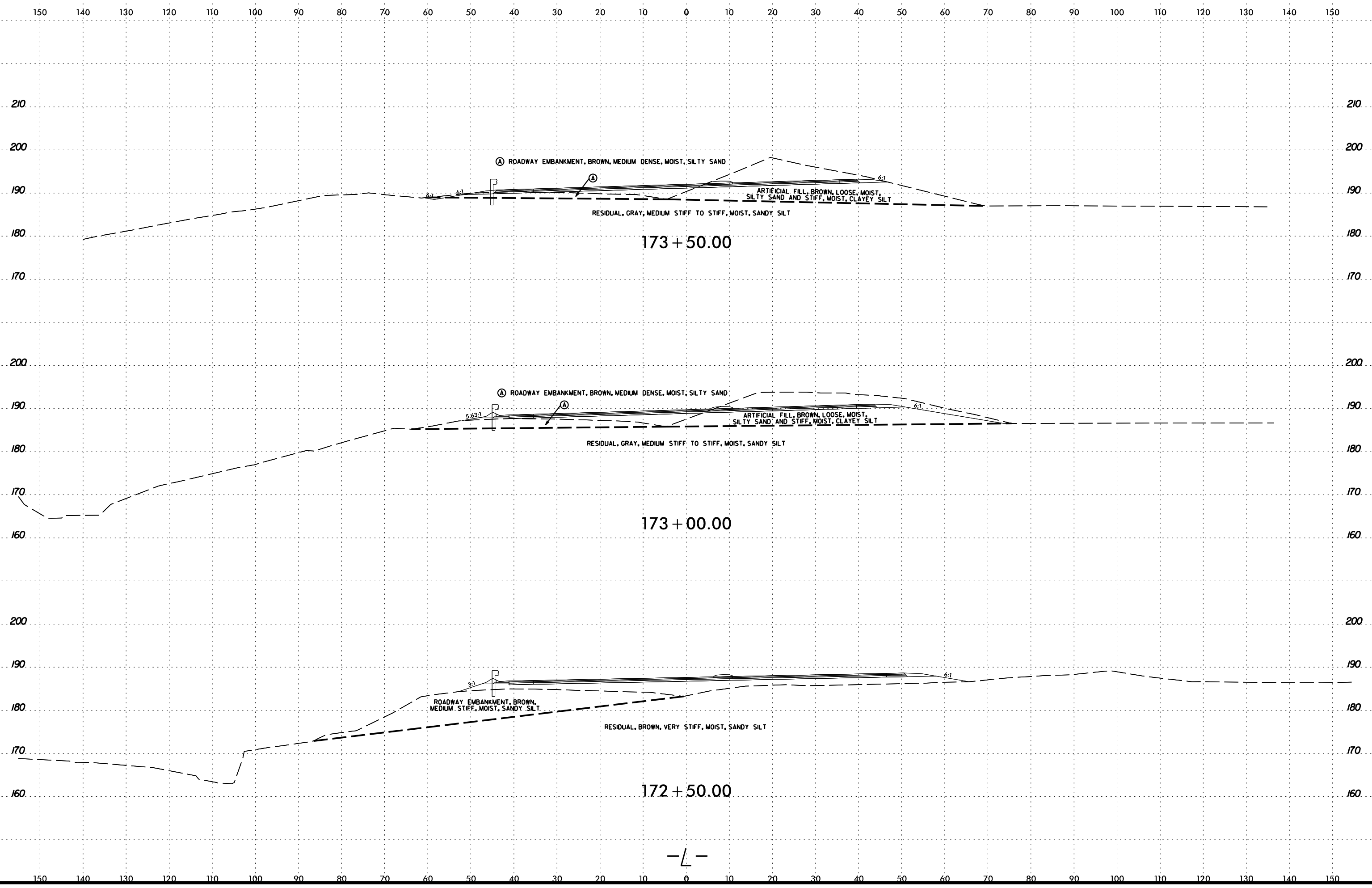
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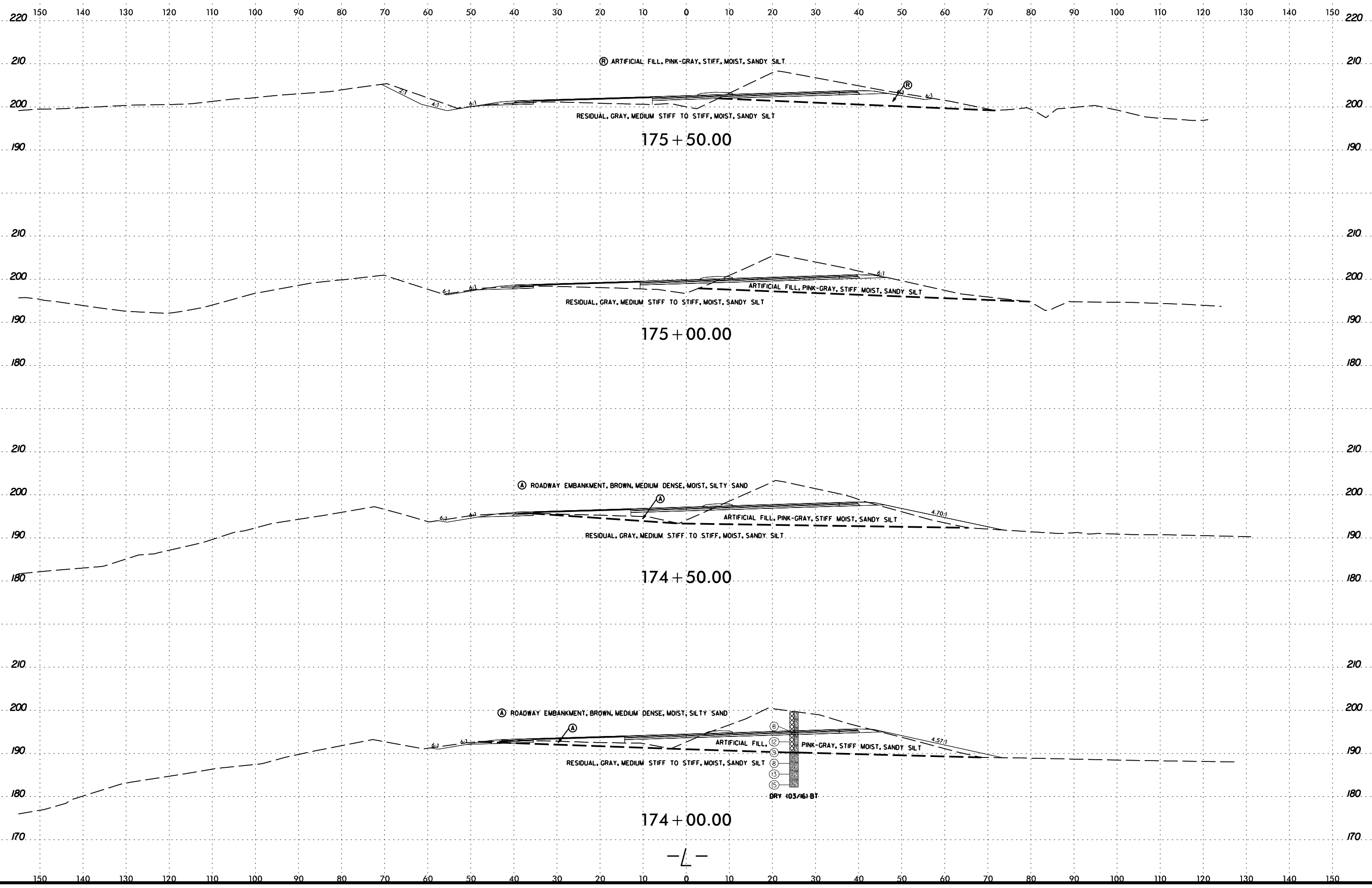
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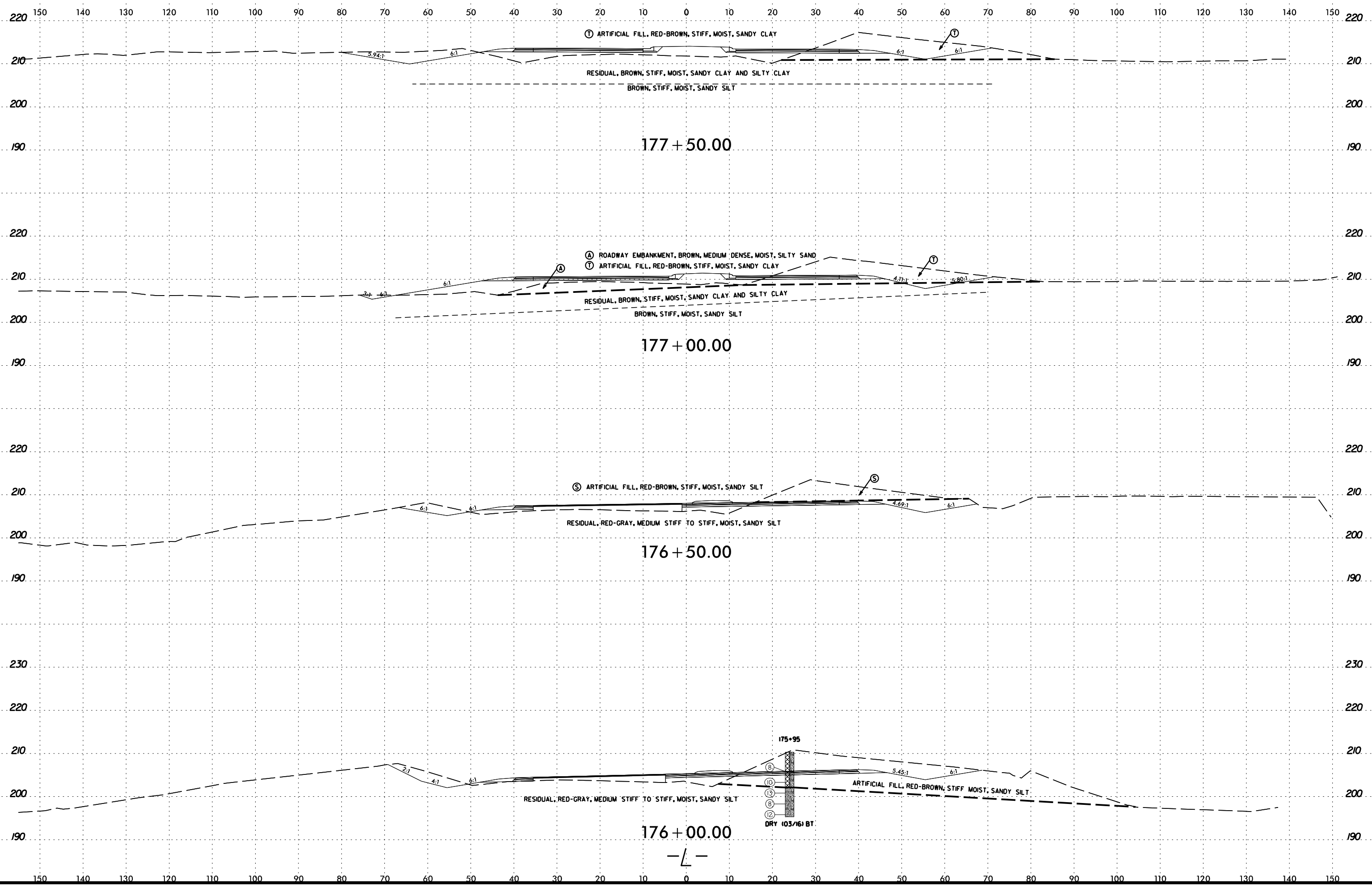


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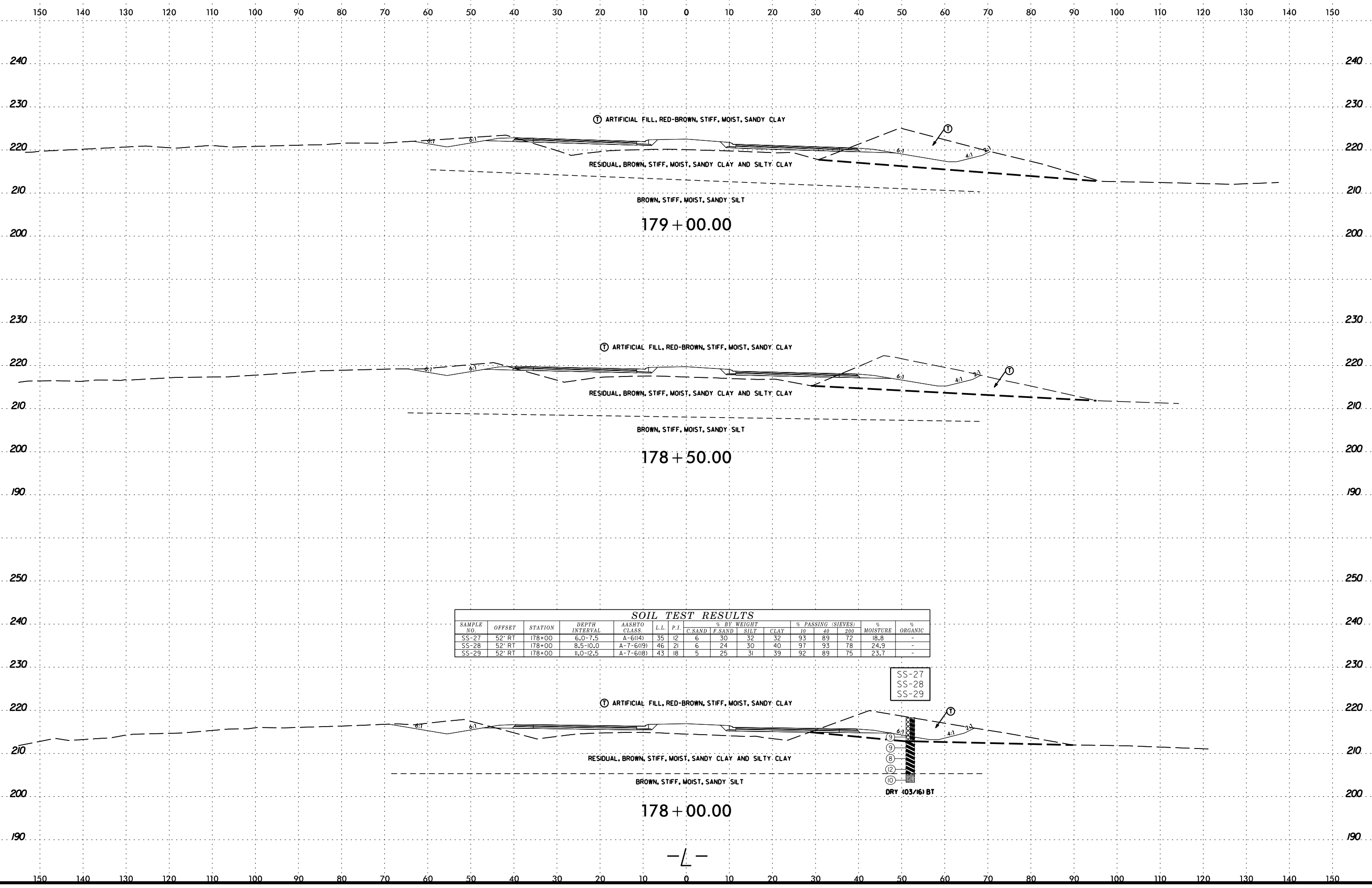
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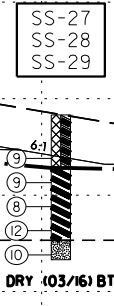
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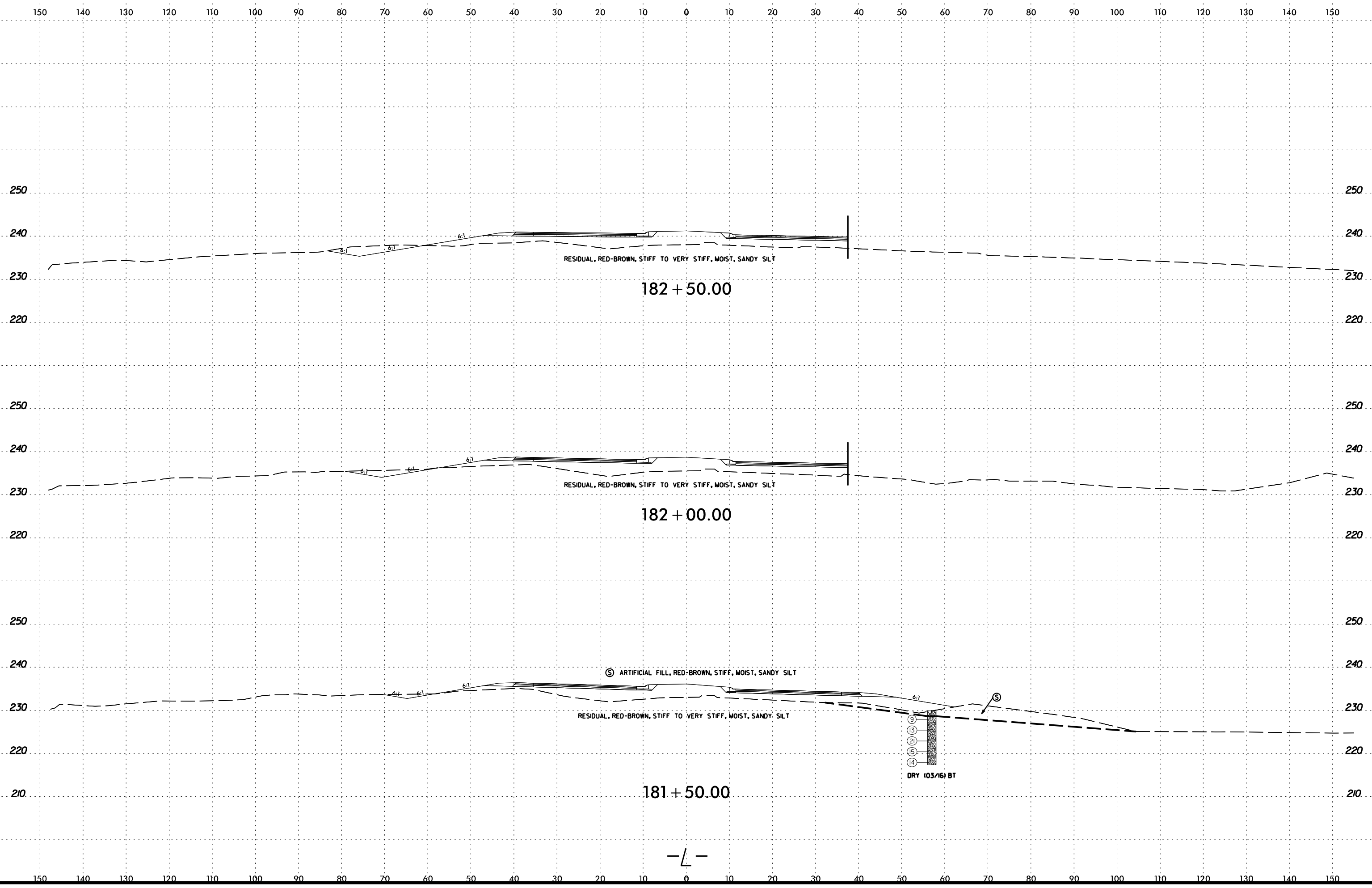
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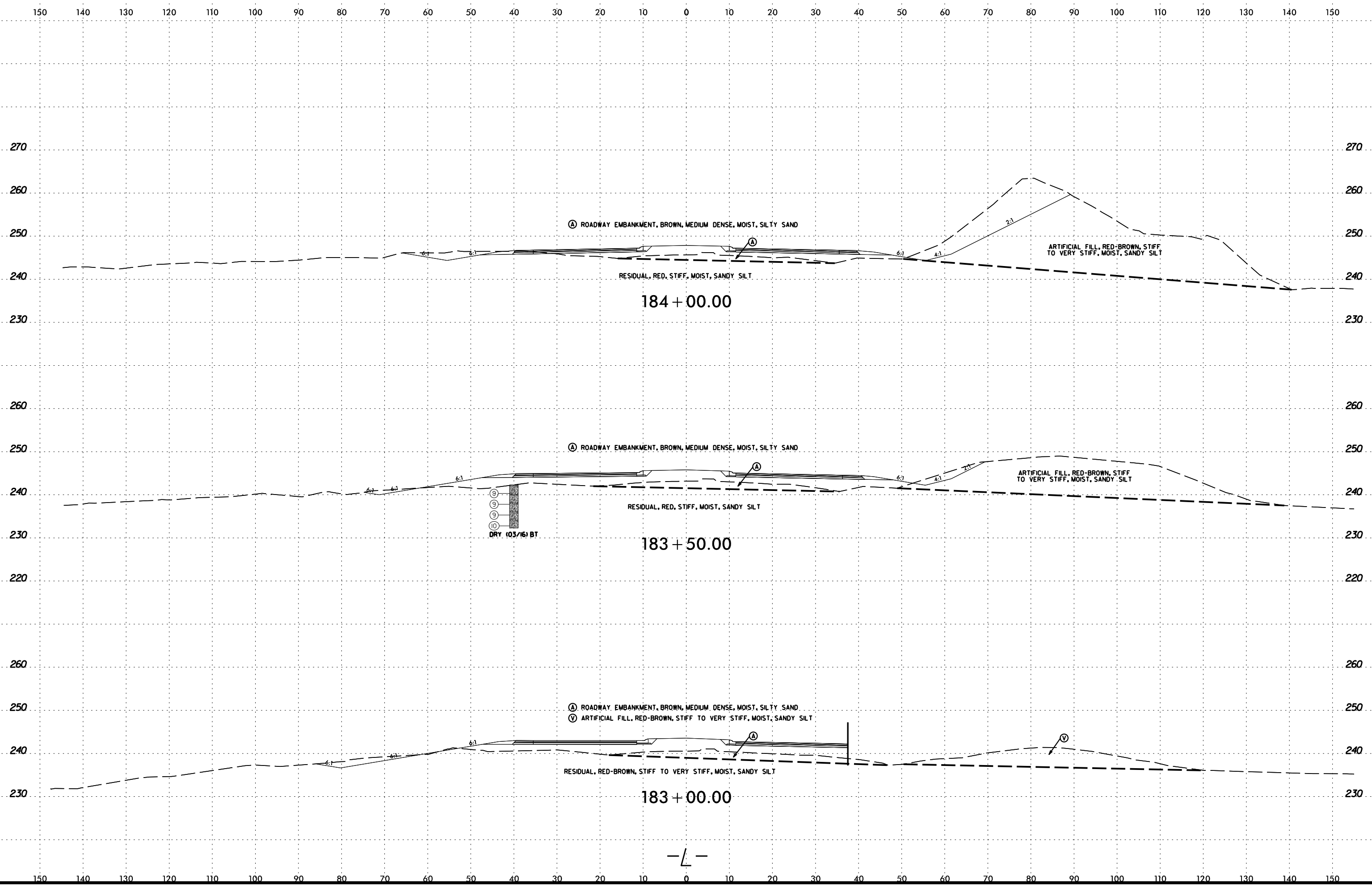
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SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC	
							C. SAND	F. SAND	SILT	CLAY	10	40	200			
SS-27	52' RT	178+00	6.0-7.5	A-6(14)	35	12	6	30	32	32	93	89	72	18.8	-	
SS-28	52' RT	178+00	8.5-10.0	A-7-6(19)	46	21	6	24	30	40	97	93	78	24.9	-	
SS-29	52' RT	178+00	11.0-12.5	A-7-6(18)	43	18	5	25	31	39	92	89	75	23.7	-	



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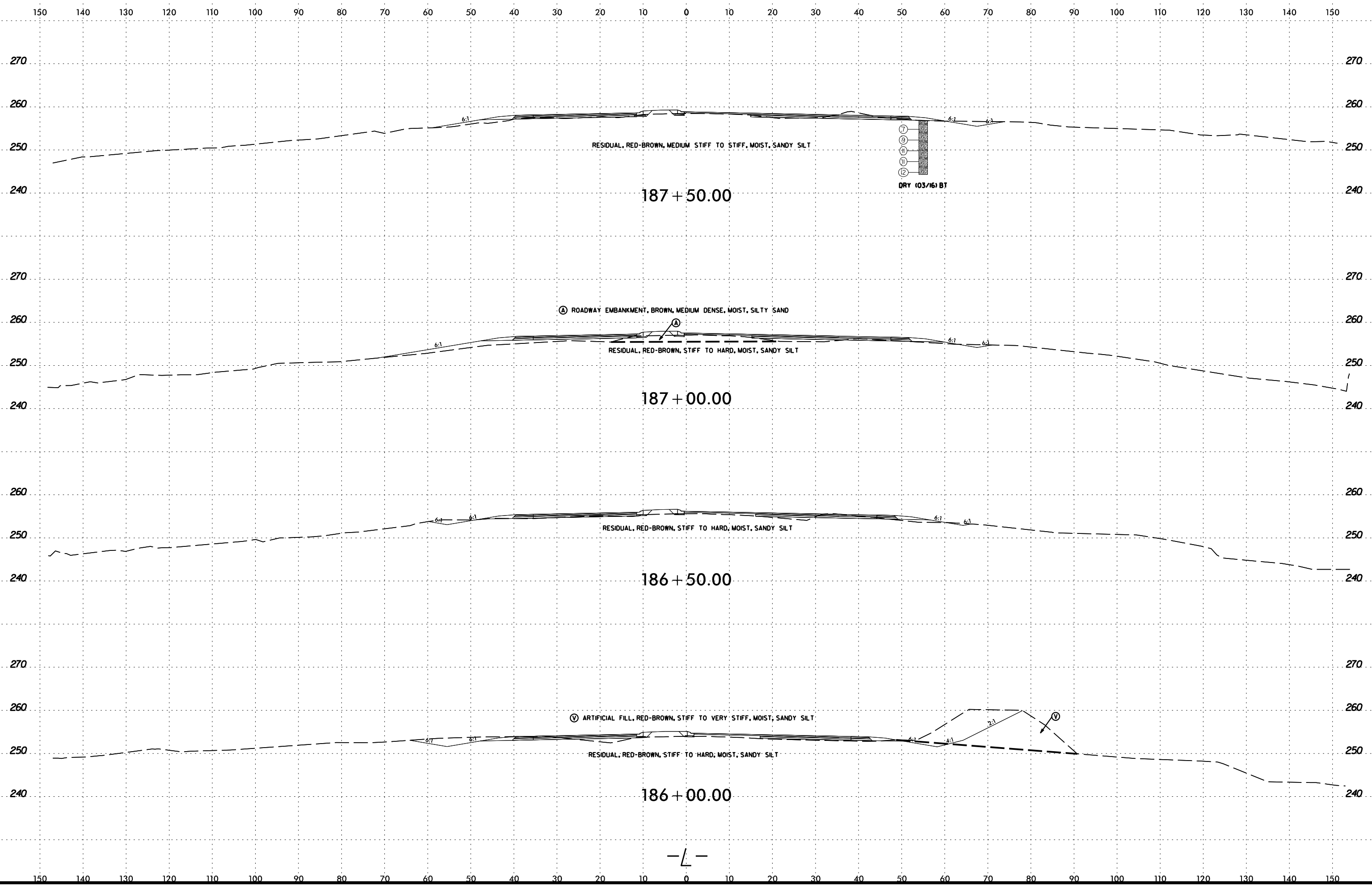


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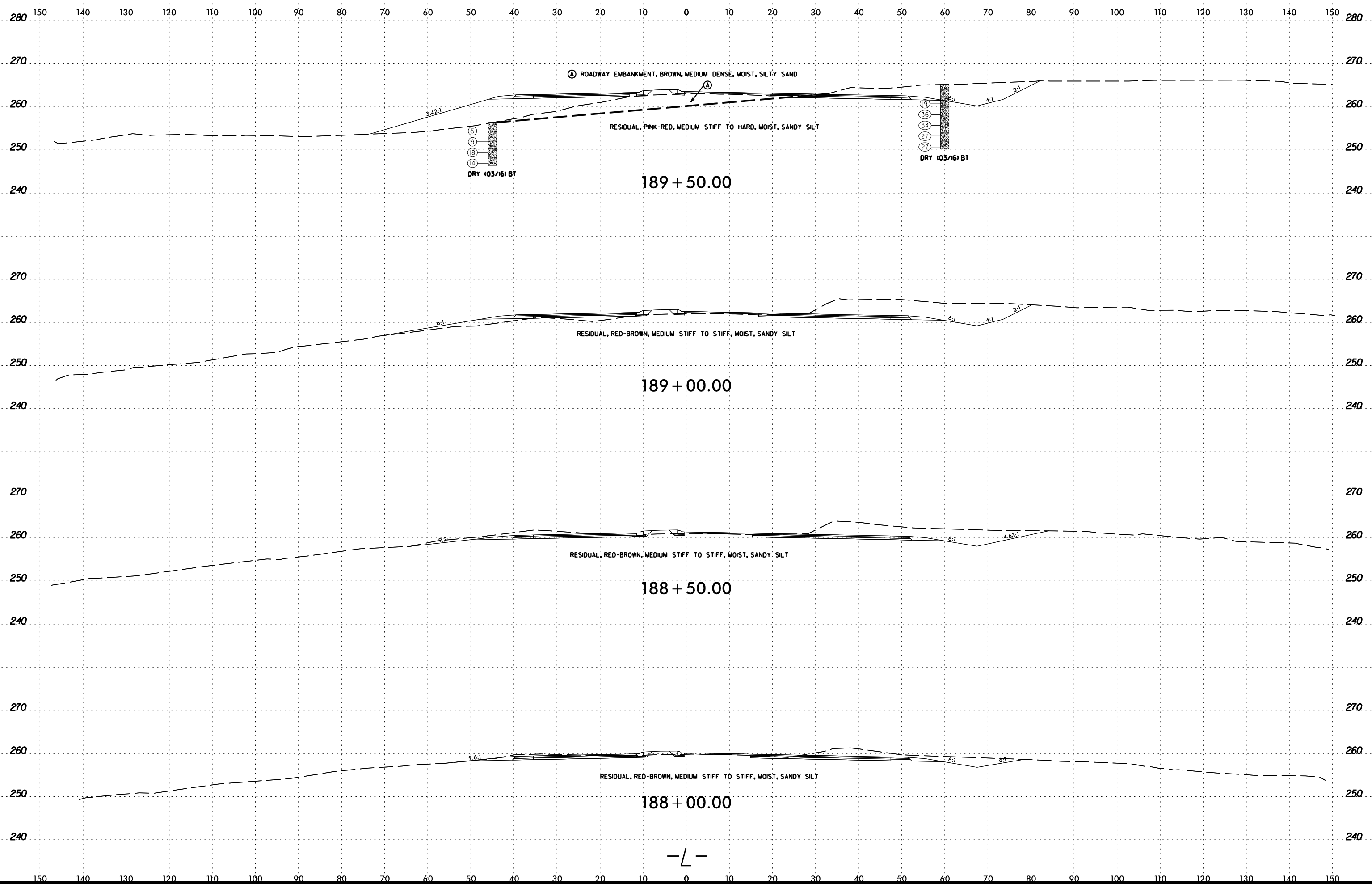
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6/23/16

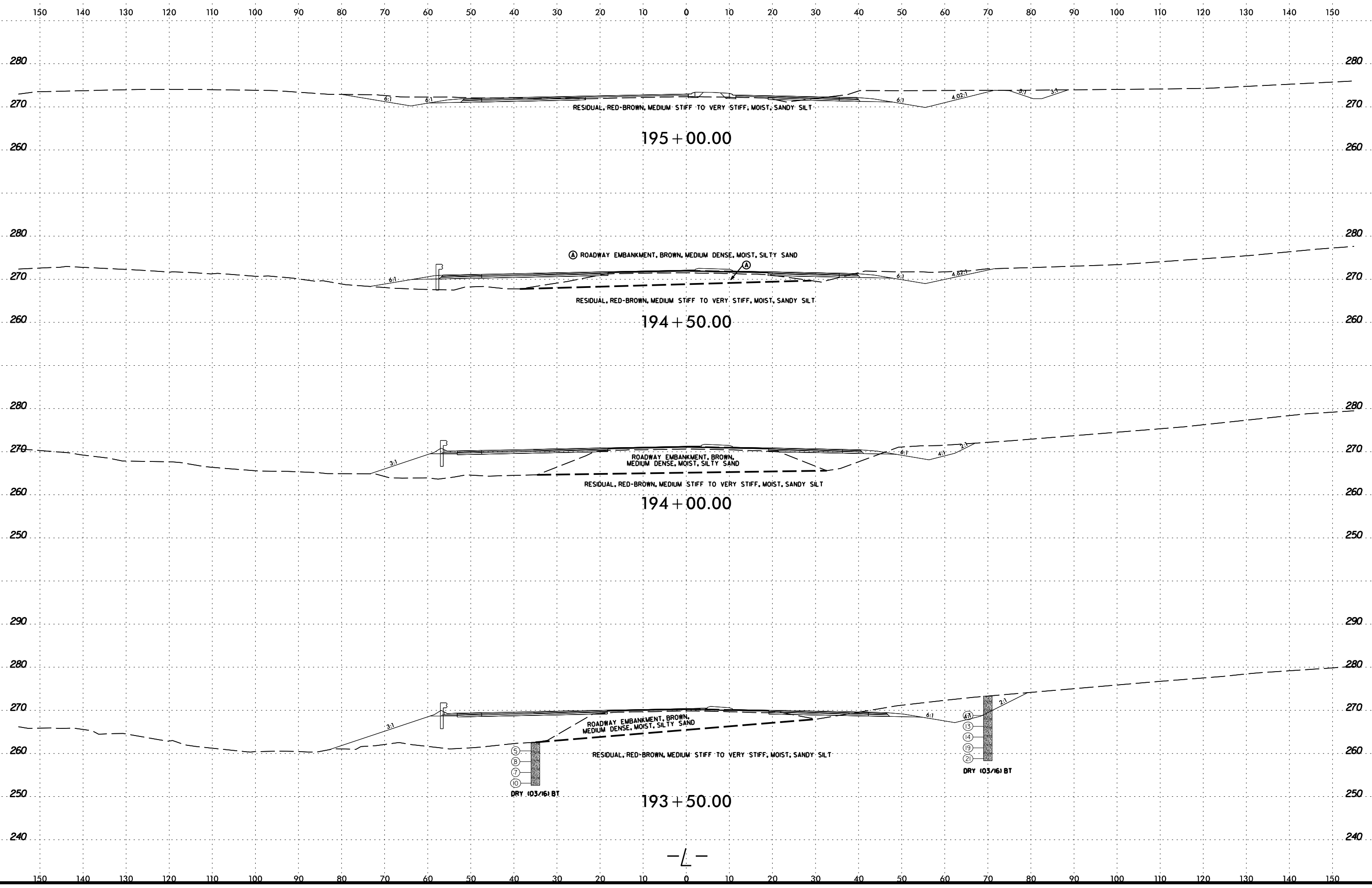


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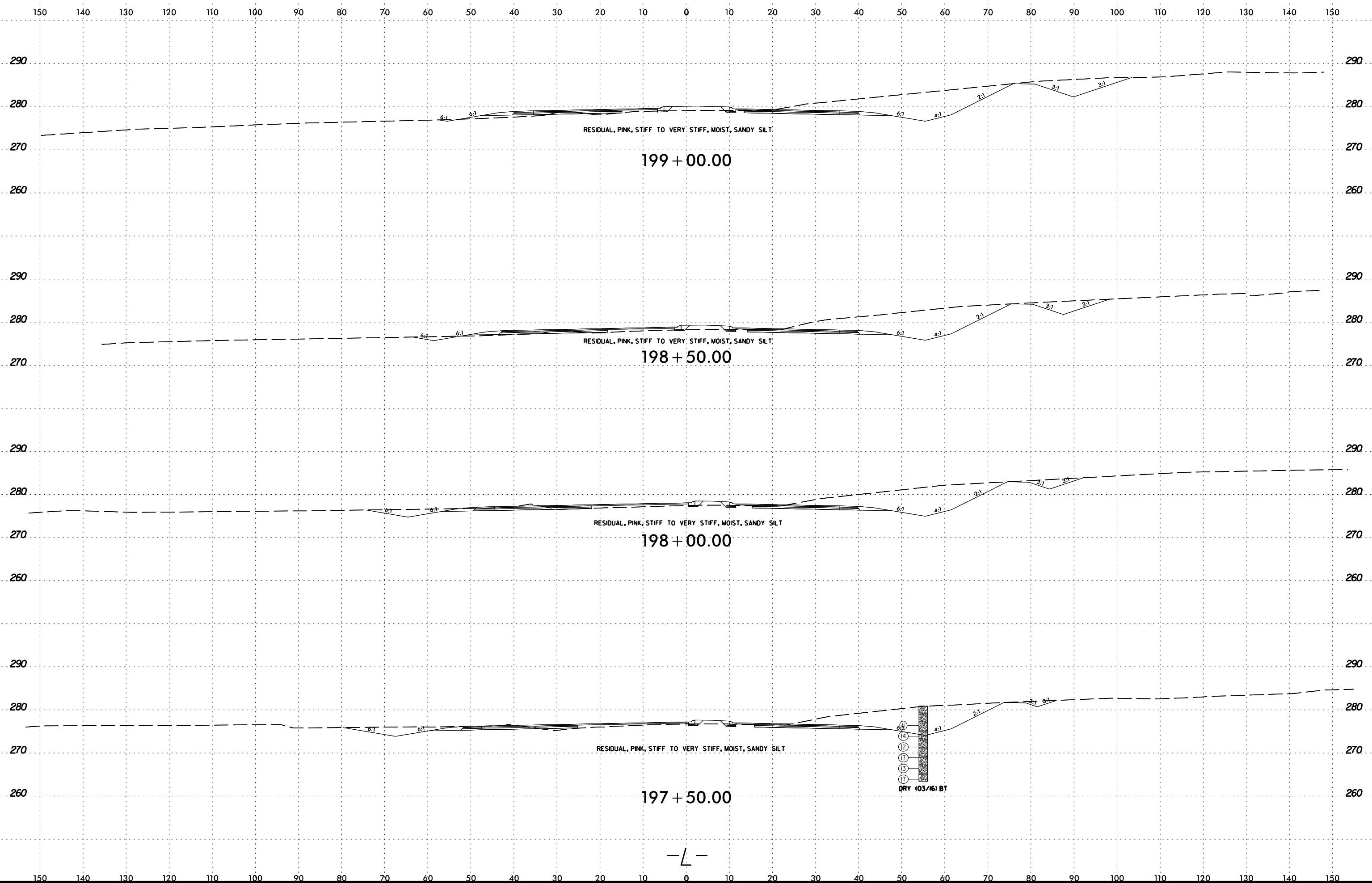
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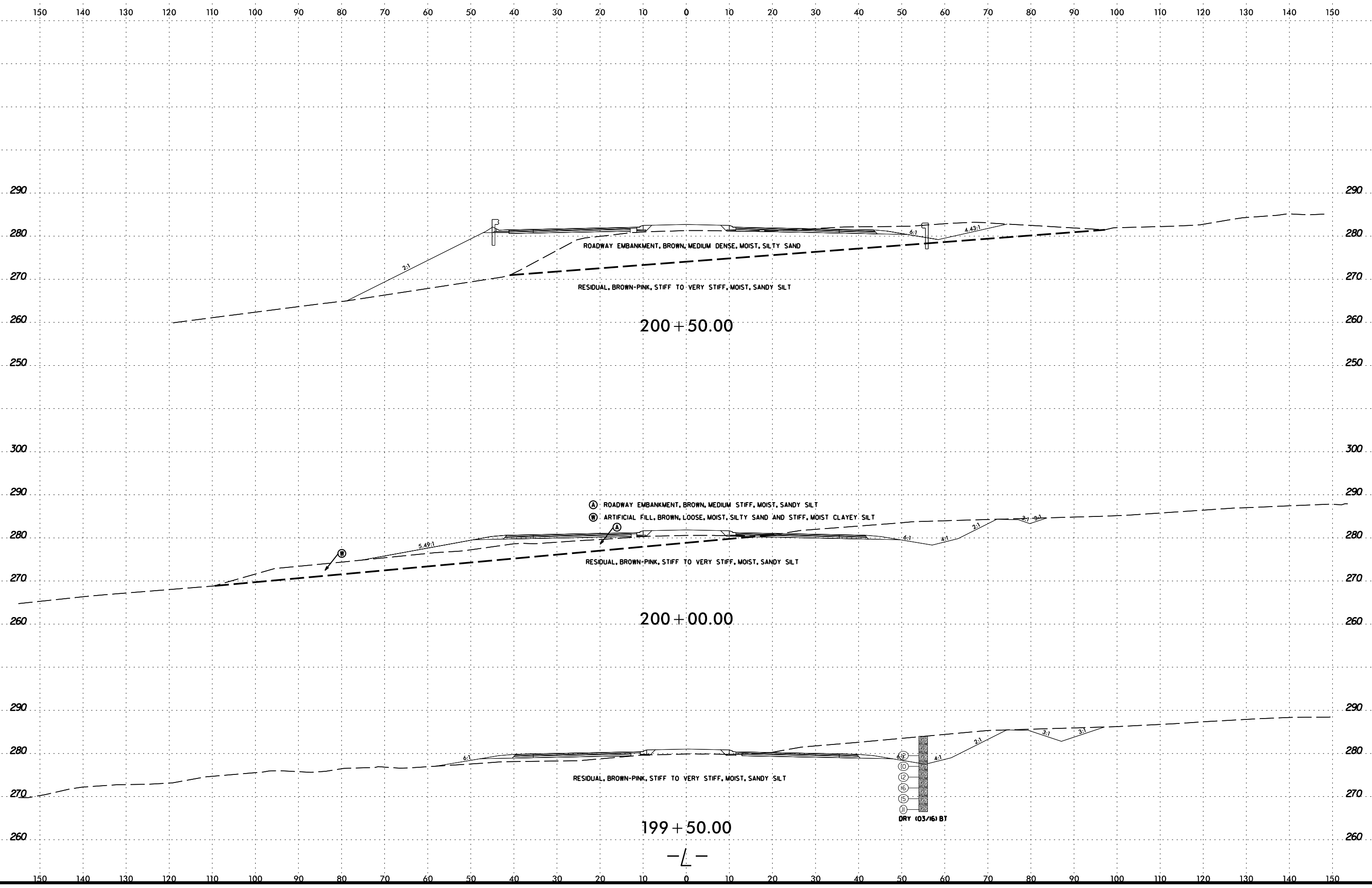
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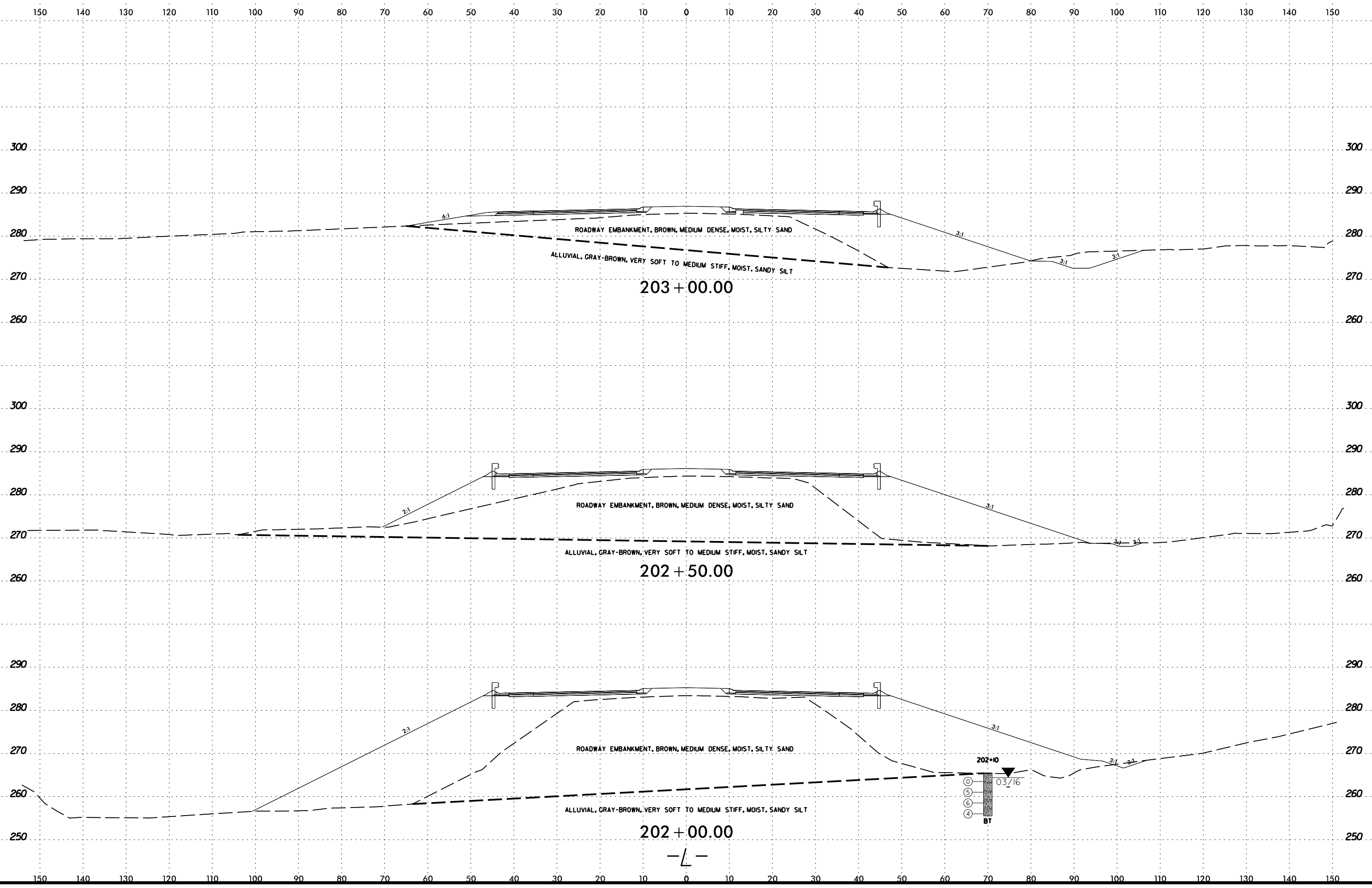


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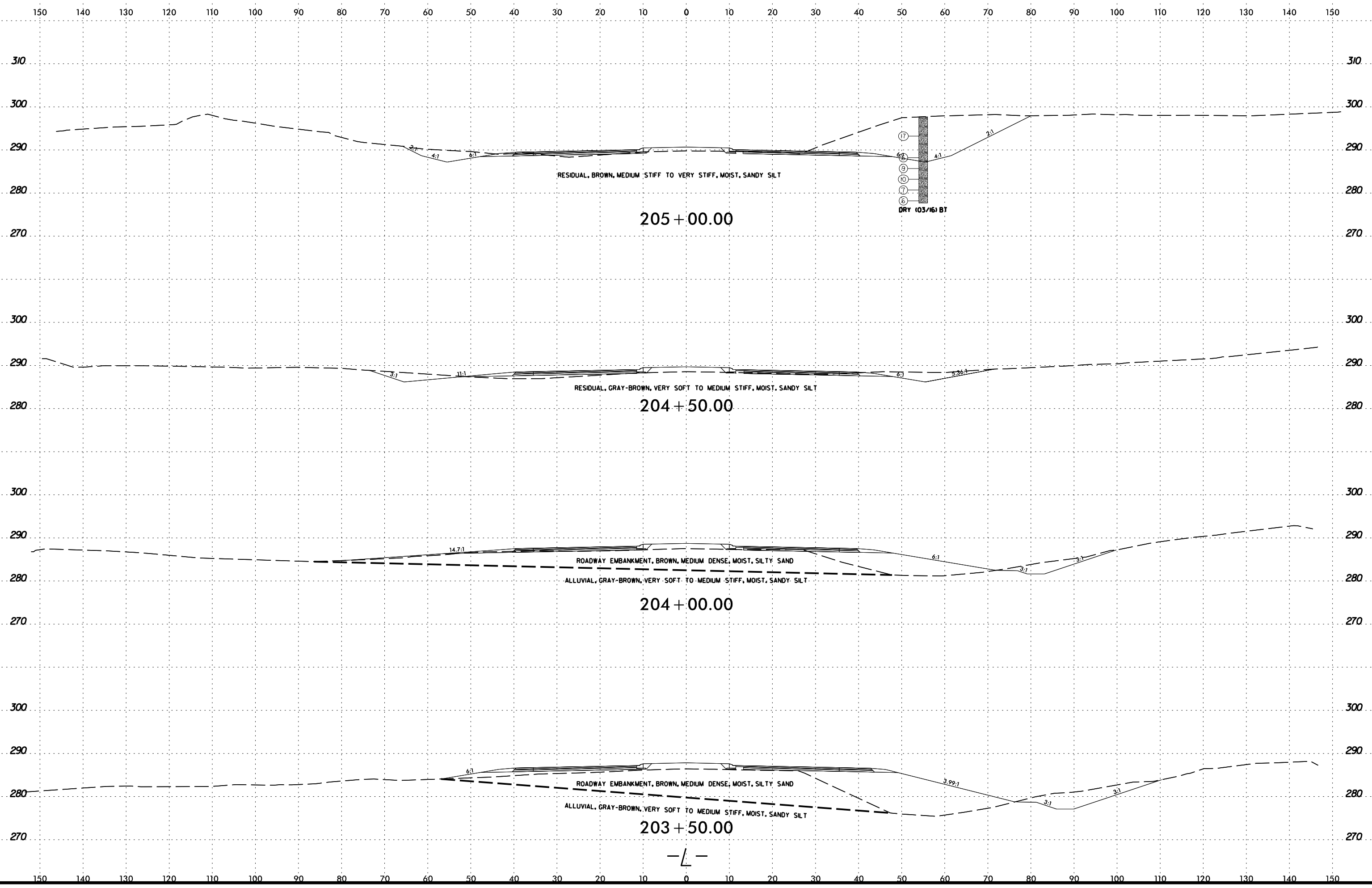


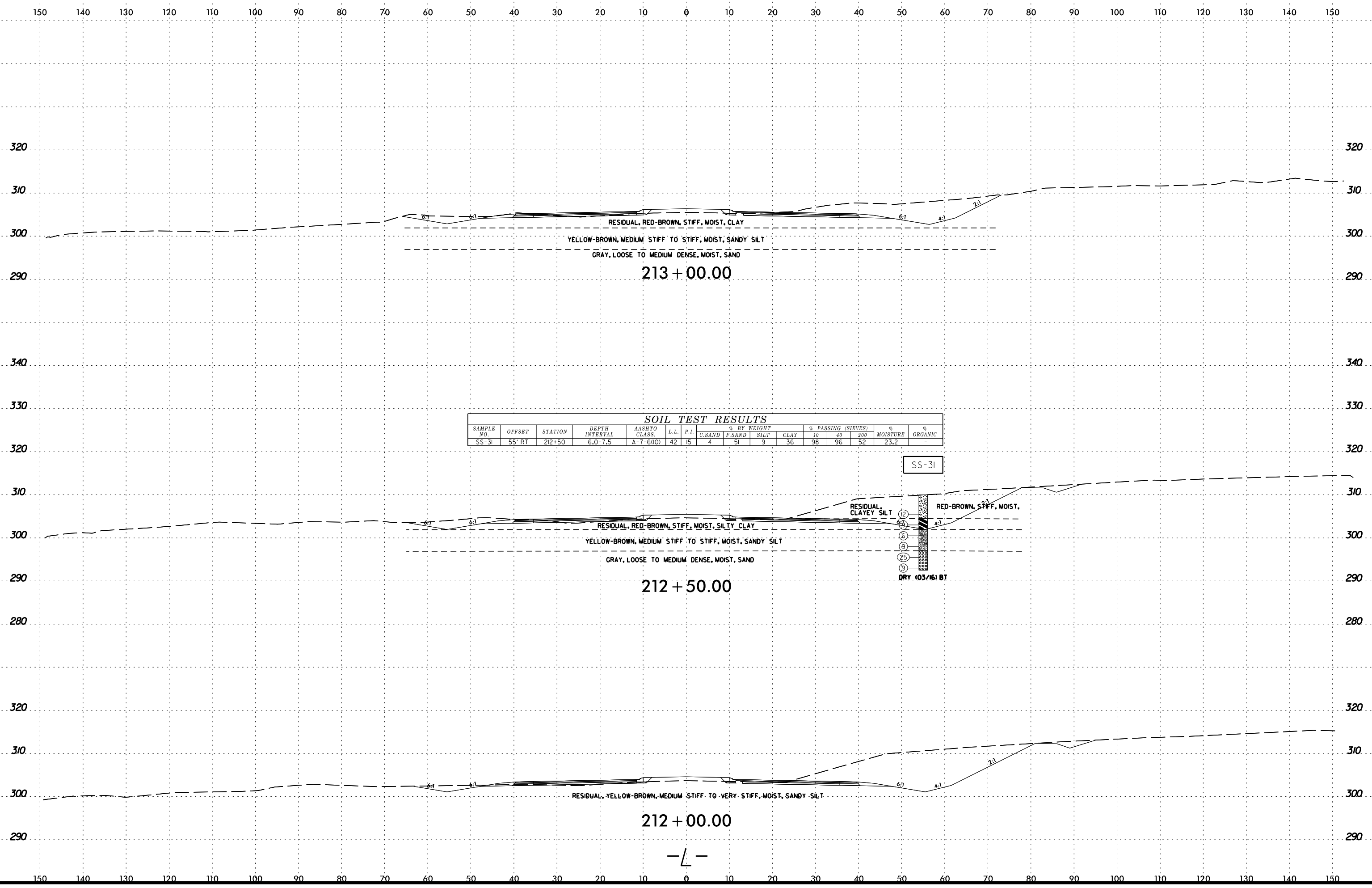
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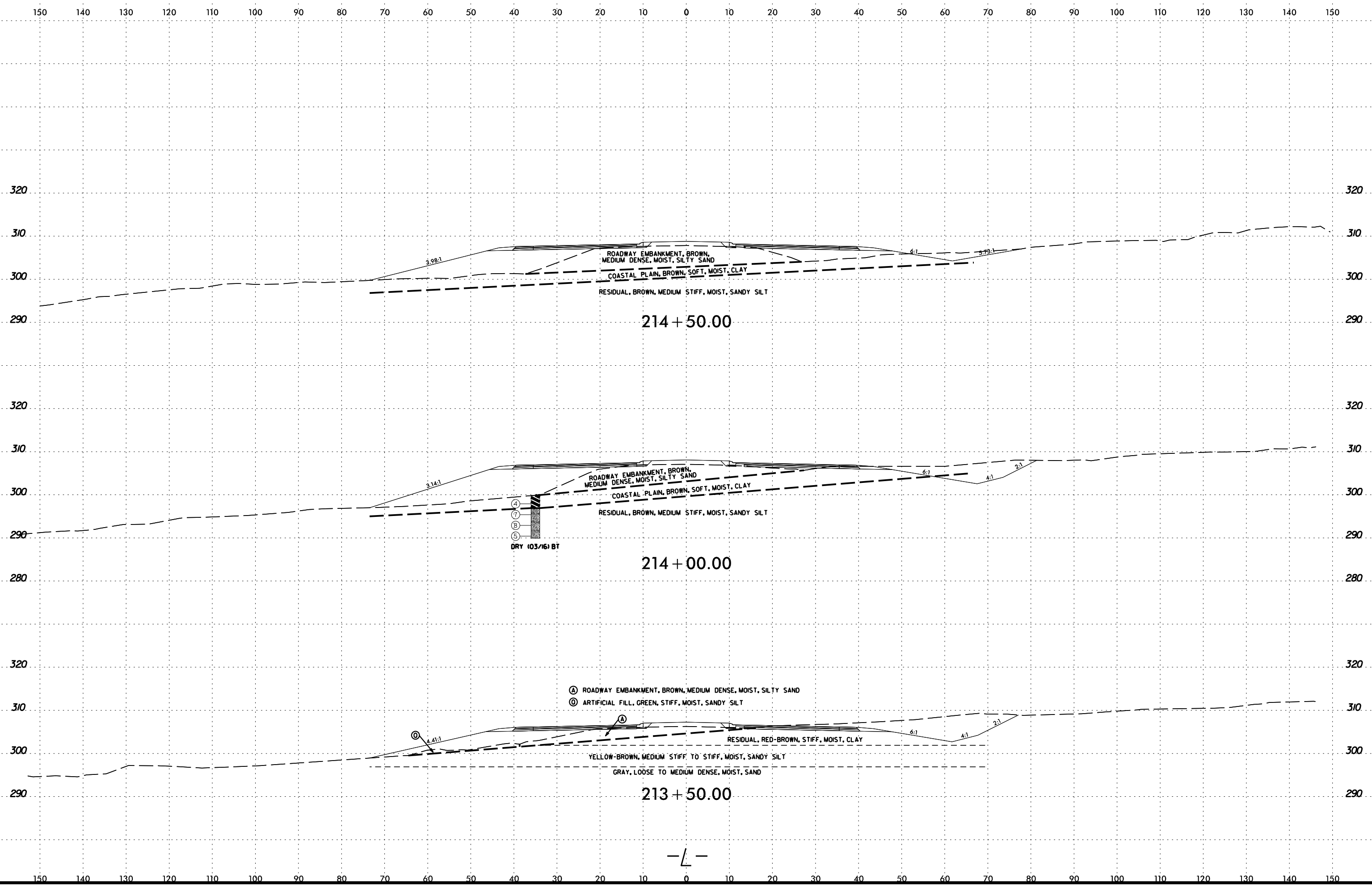


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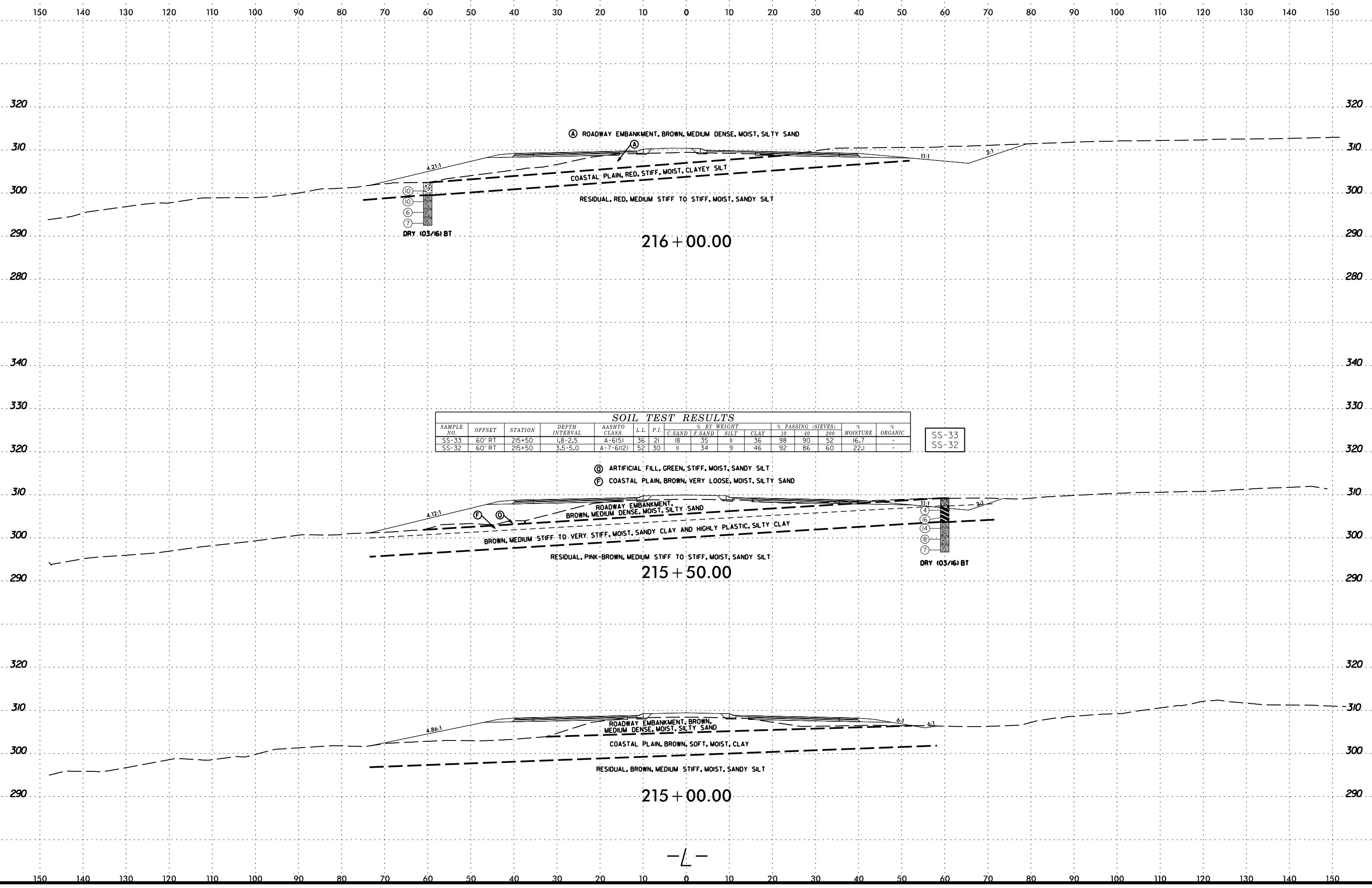




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SOIL TEST RESULTS

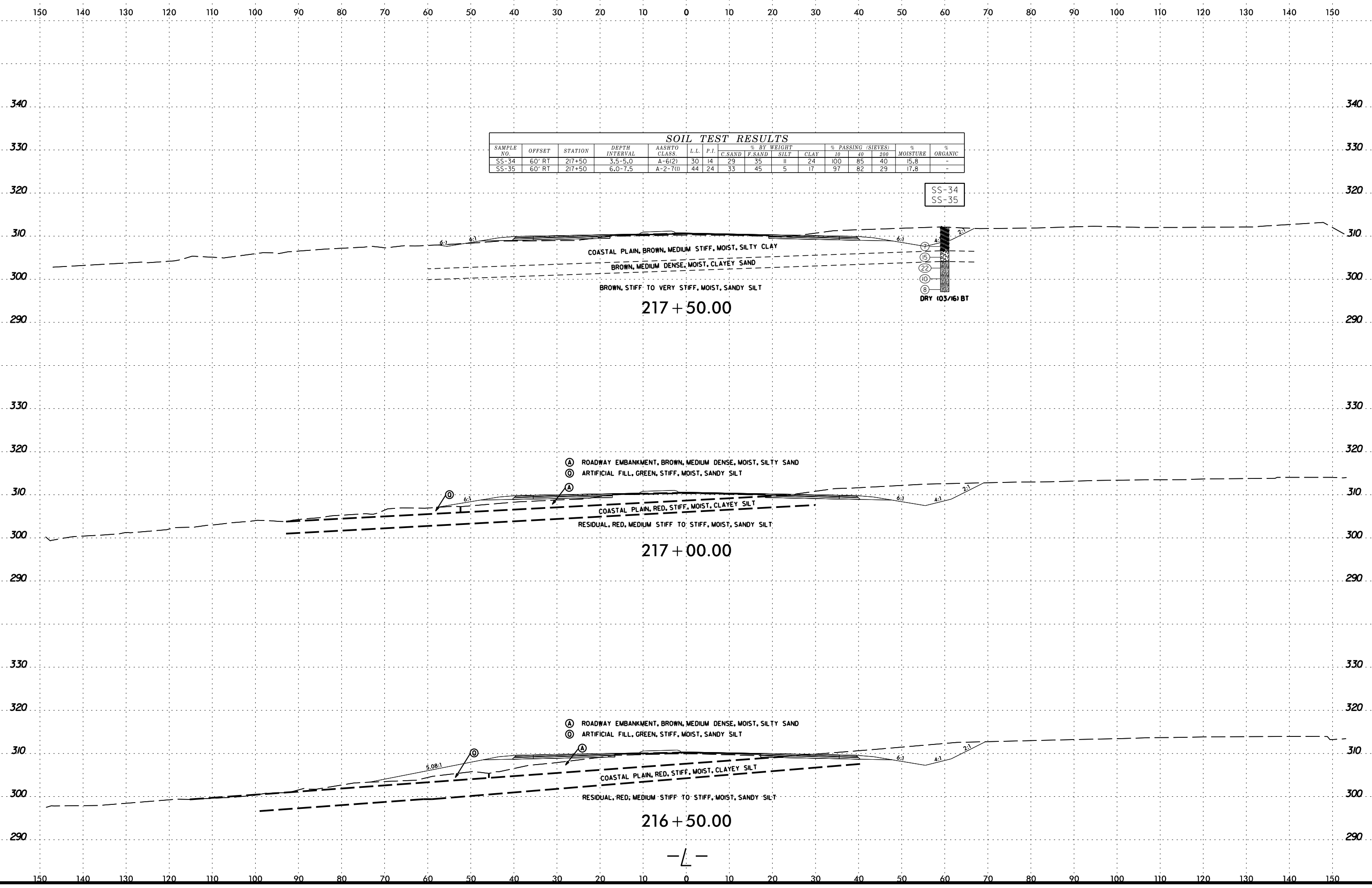
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							C.SAND	F.SAND	SILT	10	40	200			
SS-33	60' RT	215+50	1.8-2.5	A-6(5)	36	21	18	35	11	36	98	90	52	16.7	-
SS-32	60' RT	215+50	3.5-5.0	A-7-6(12)	52	30	11	34	9	46	92	86	60	22.1	-

SS-33
SS-32

- ⓐ ARTIFICIAL FILL, GREEN, STIFF, MOIST, SANDY SILT
- ⓑ COASTAL PLAIN, BROWN, VERY LOOSE, MOIST, SILTY SAND

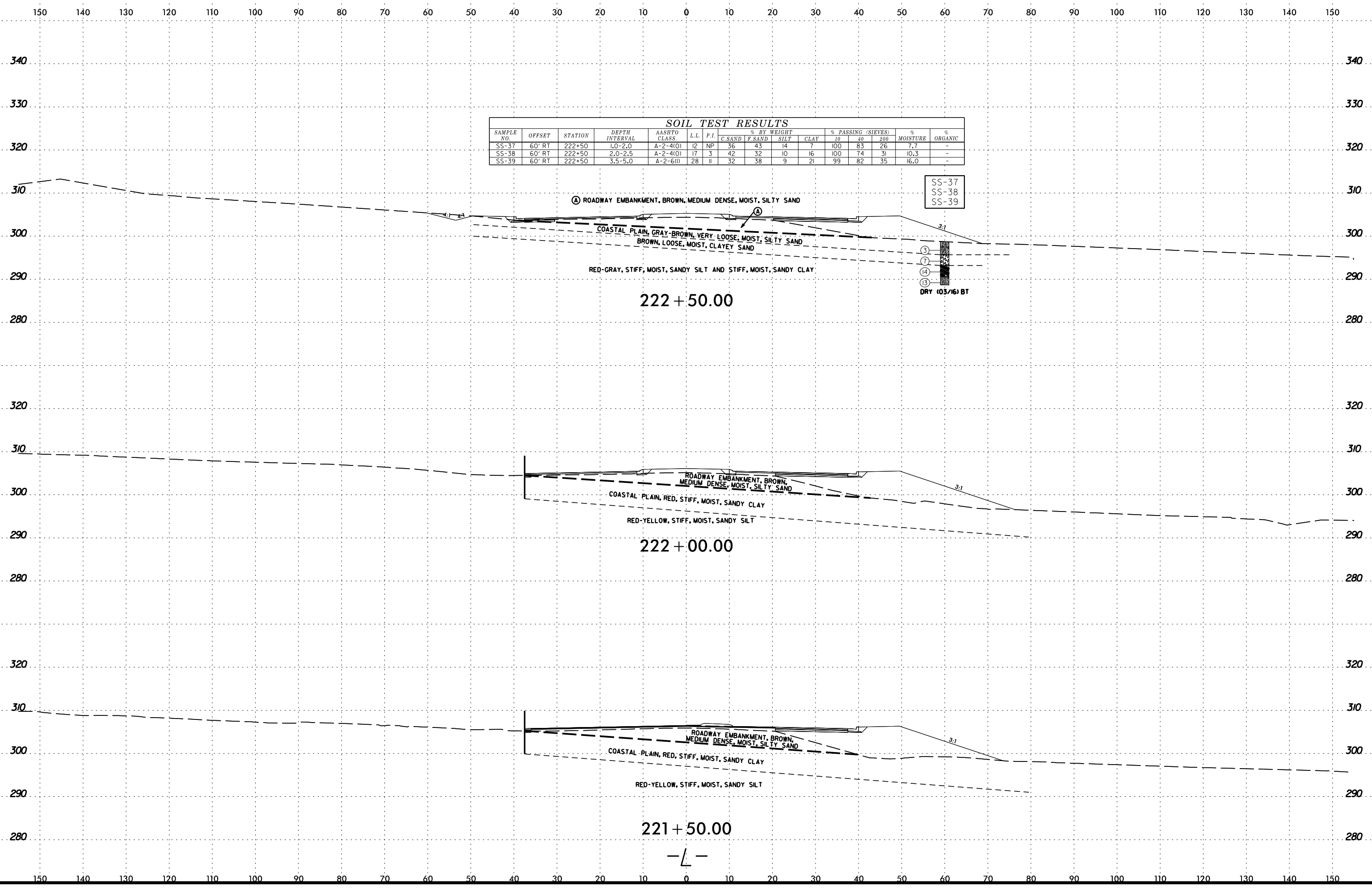
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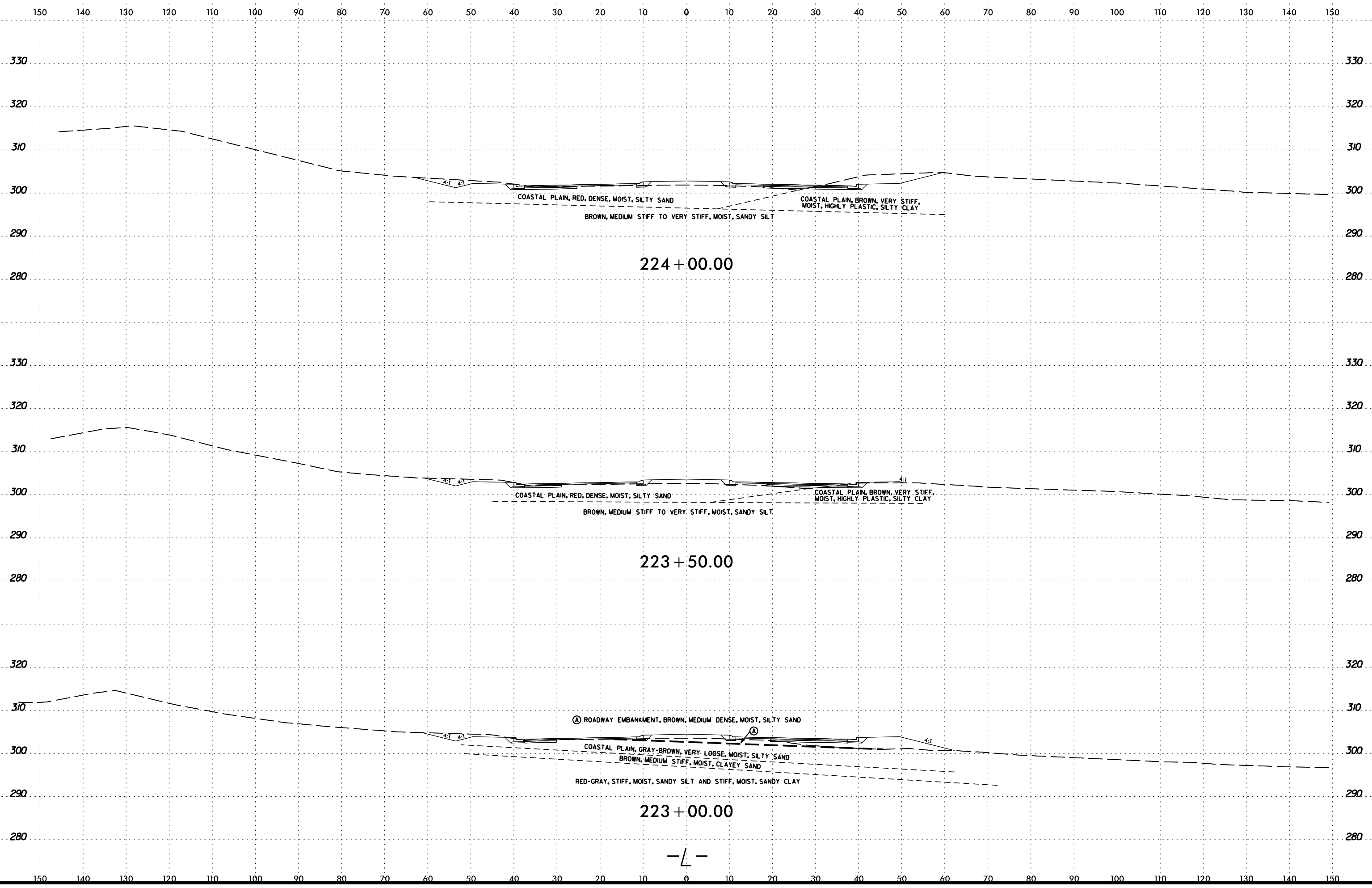
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							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-34	60' RT	217+50	3.5-5.0	A-6(2)	30	14	29	35	11	24	100	85	40	15.8	-
SS-35	60' RT	217+50	6.0-7.5	A-2-7(1)	44	24	33	45	5	17	97	82	29	17.8	-



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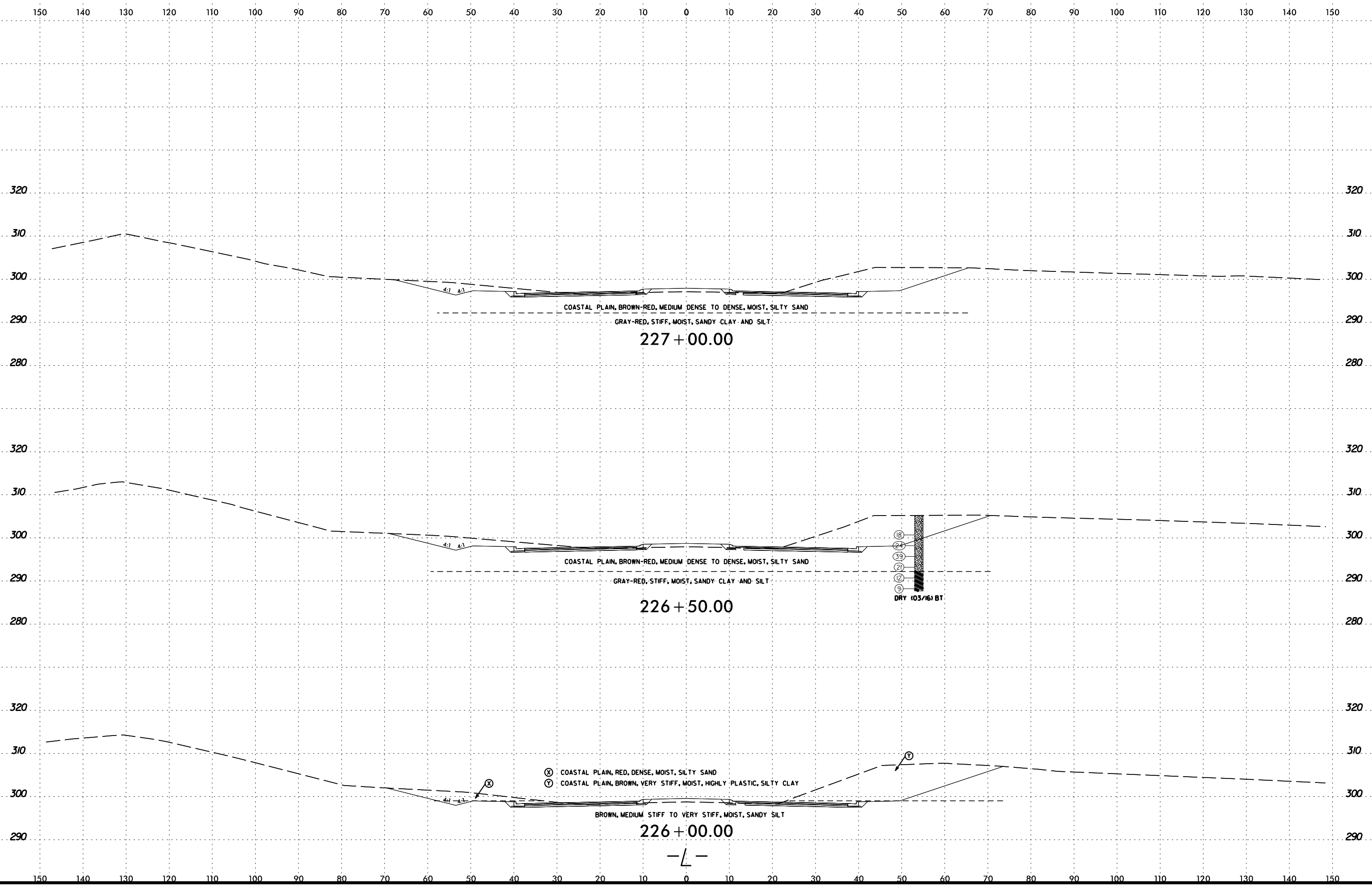
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							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-37	60' RT	222+50	1.0-2.0	A-2-4(10)	12	NP	36	43	14	7	100	83	26	7.7	-
SS-38	60' RT	222+50	2.0-2.5	A-2-4(10)	17	3	42	32	10	16	100	74	31	10.3	-
SS-39	60' RT	222+50	3.5-5.0	A-2-6(10)	28	11	32	38	9	21	99	82	35	16.0	-



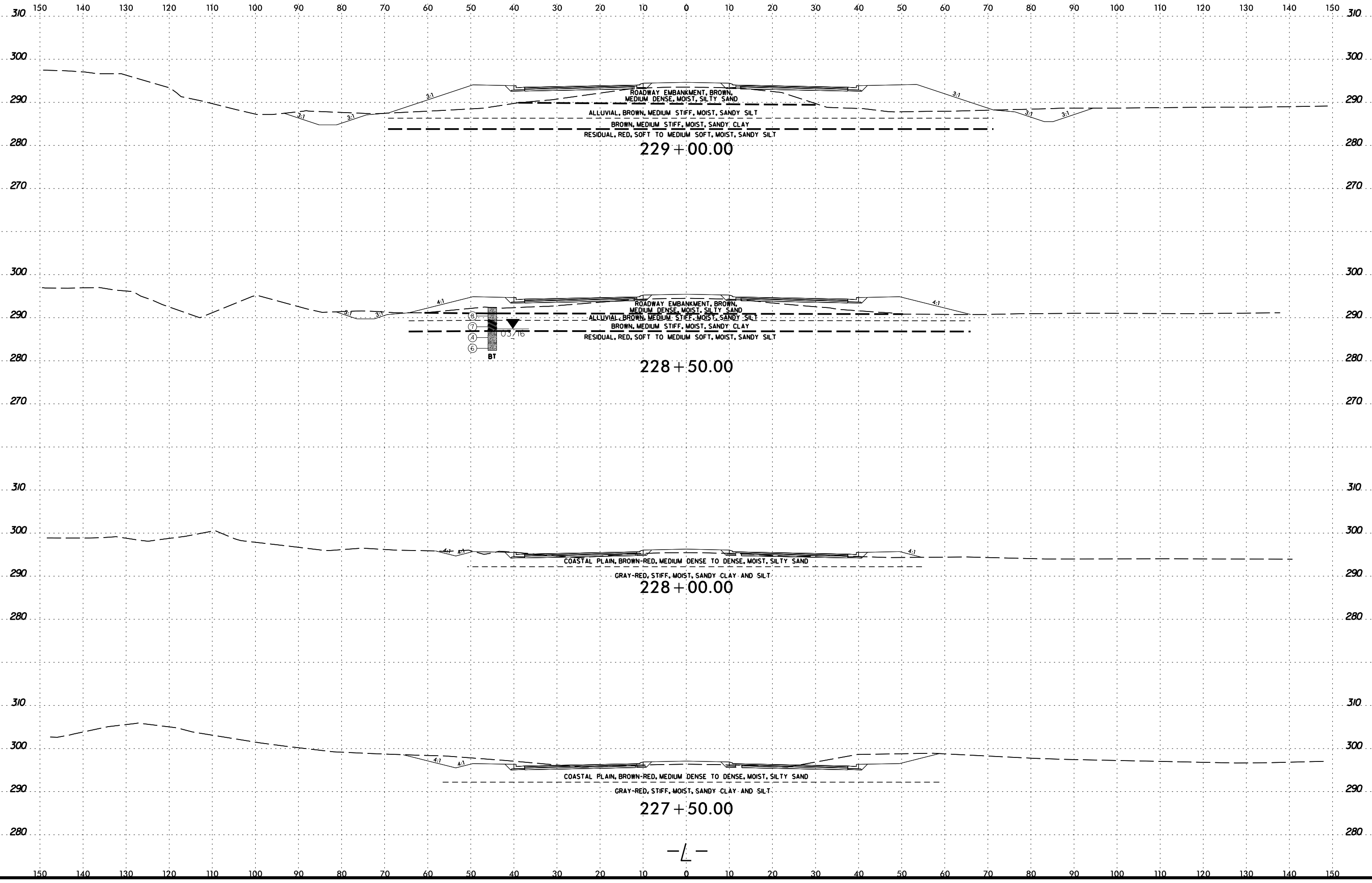


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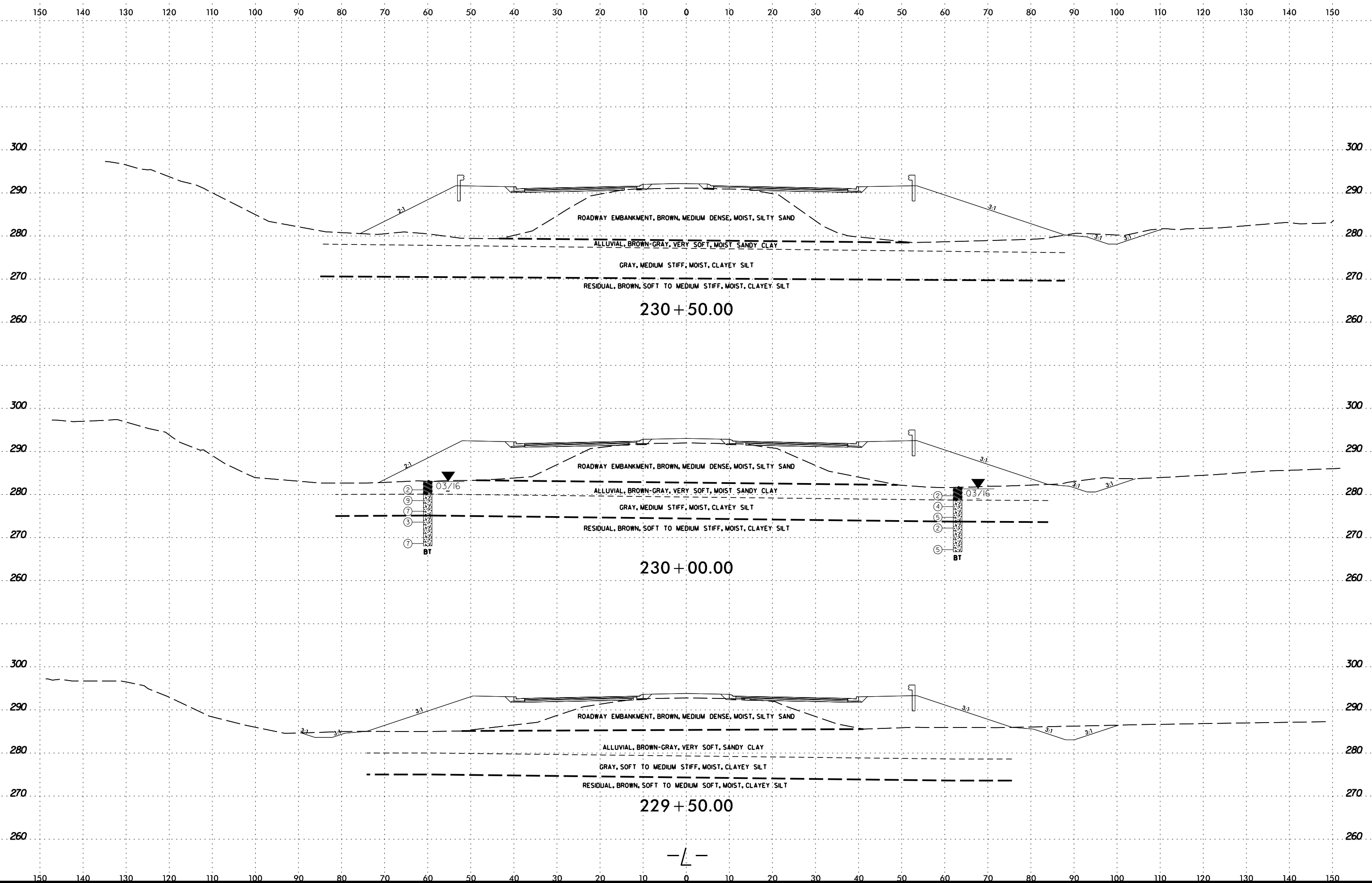
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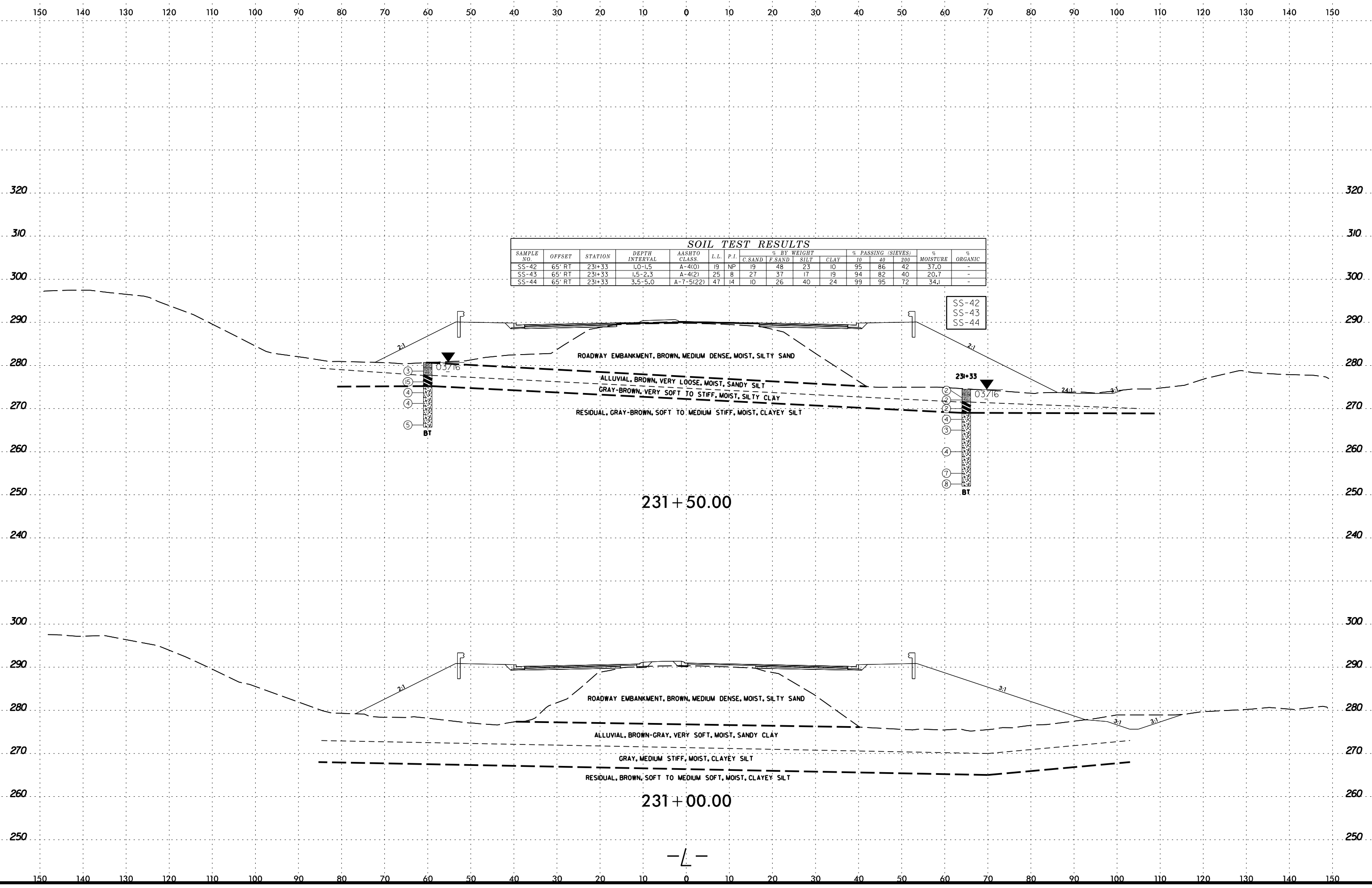
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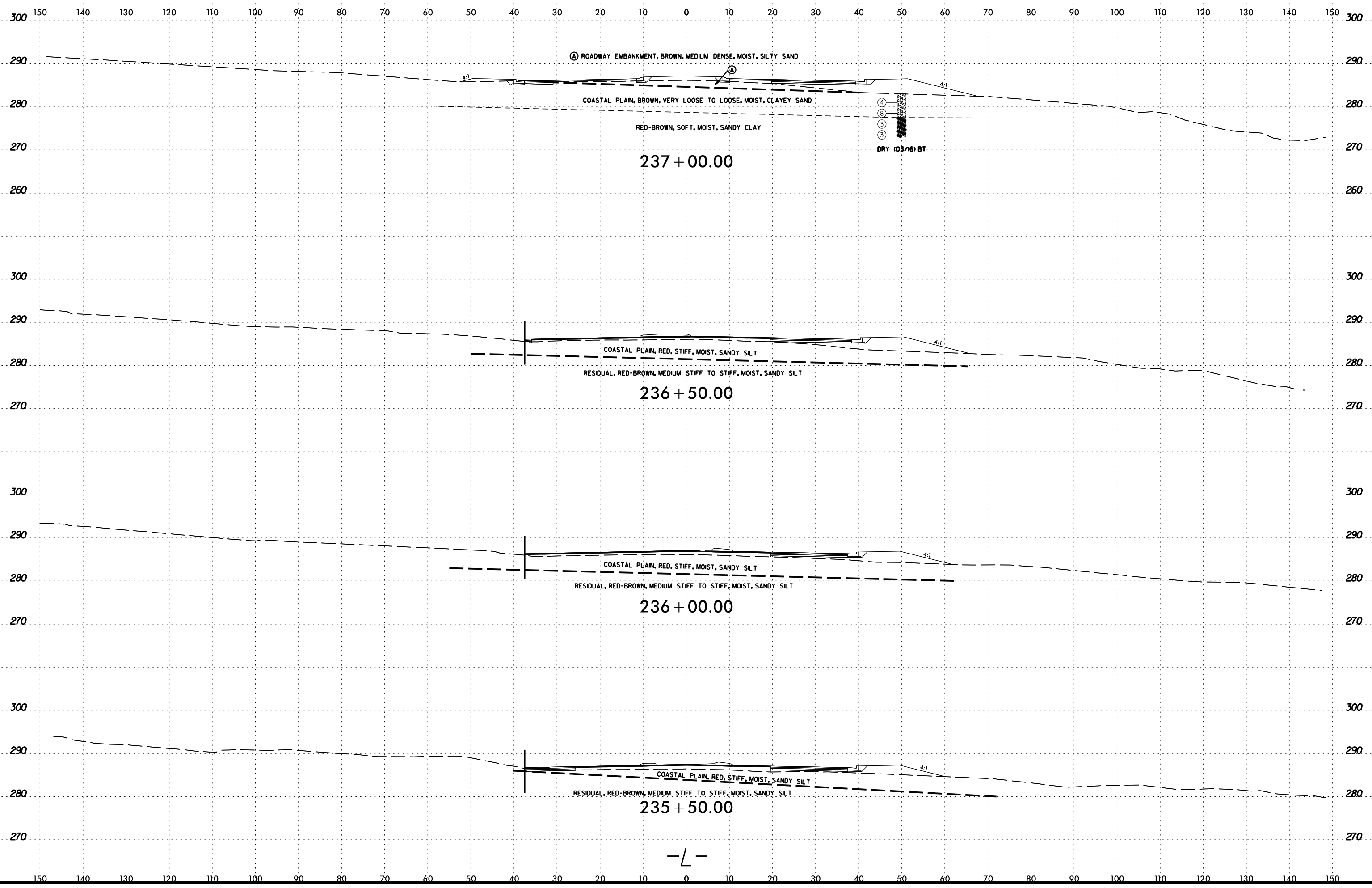
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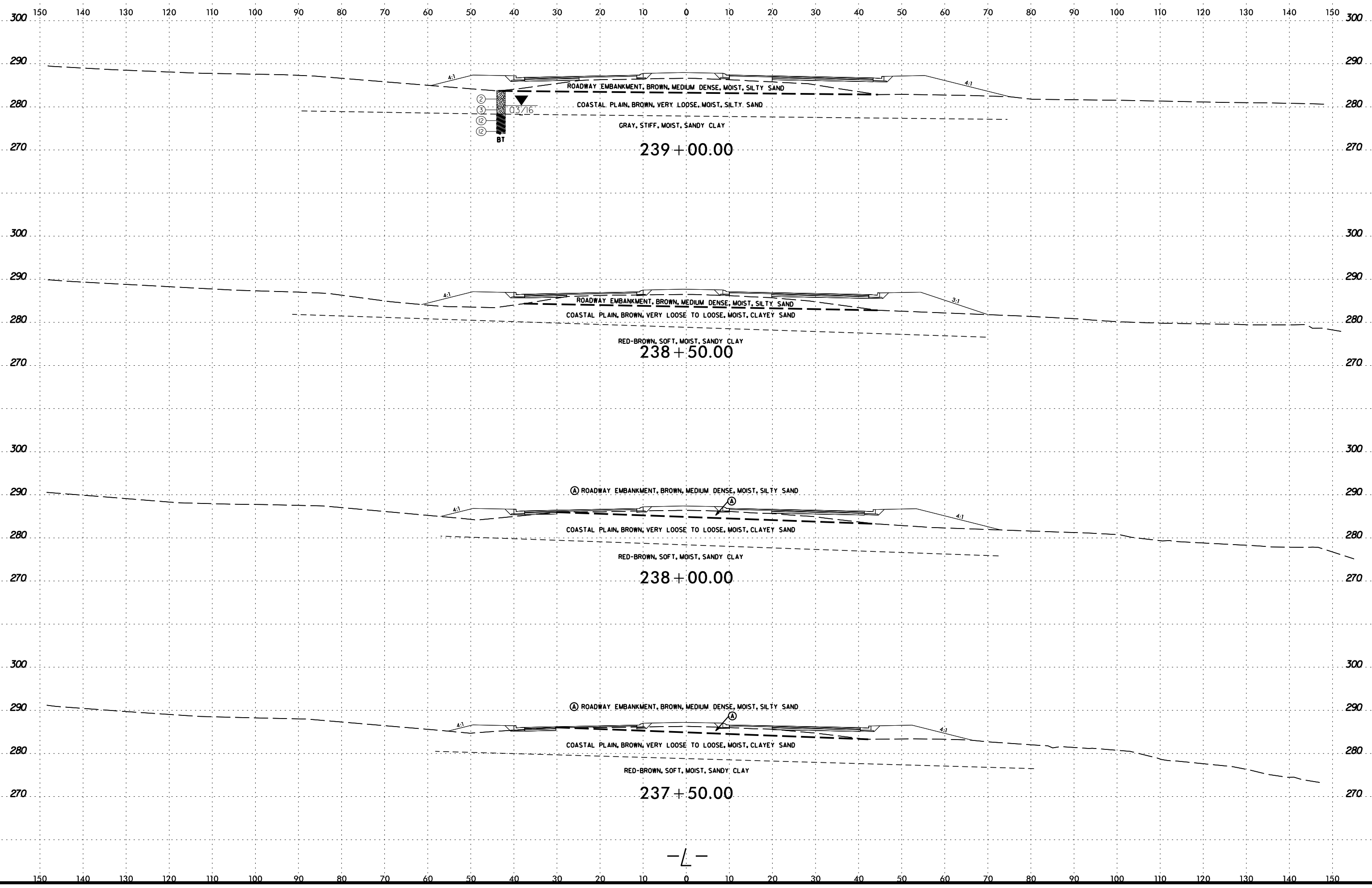
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SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-42	65' RT	231+33	1.0-1.5	A-4(0)	19	NP	19	48	23	10	95	86	42	37.0	-
SS-43	65' RT	231+33	1.5-2.3	A-4(2)	25	8	27	37	17	19	94	82	40	20.7	-
SS-44	65' RT	231+33	3.5-5.0	A-7-5(22)	47	14	10	26	40	24	99	95	72	34.1	-

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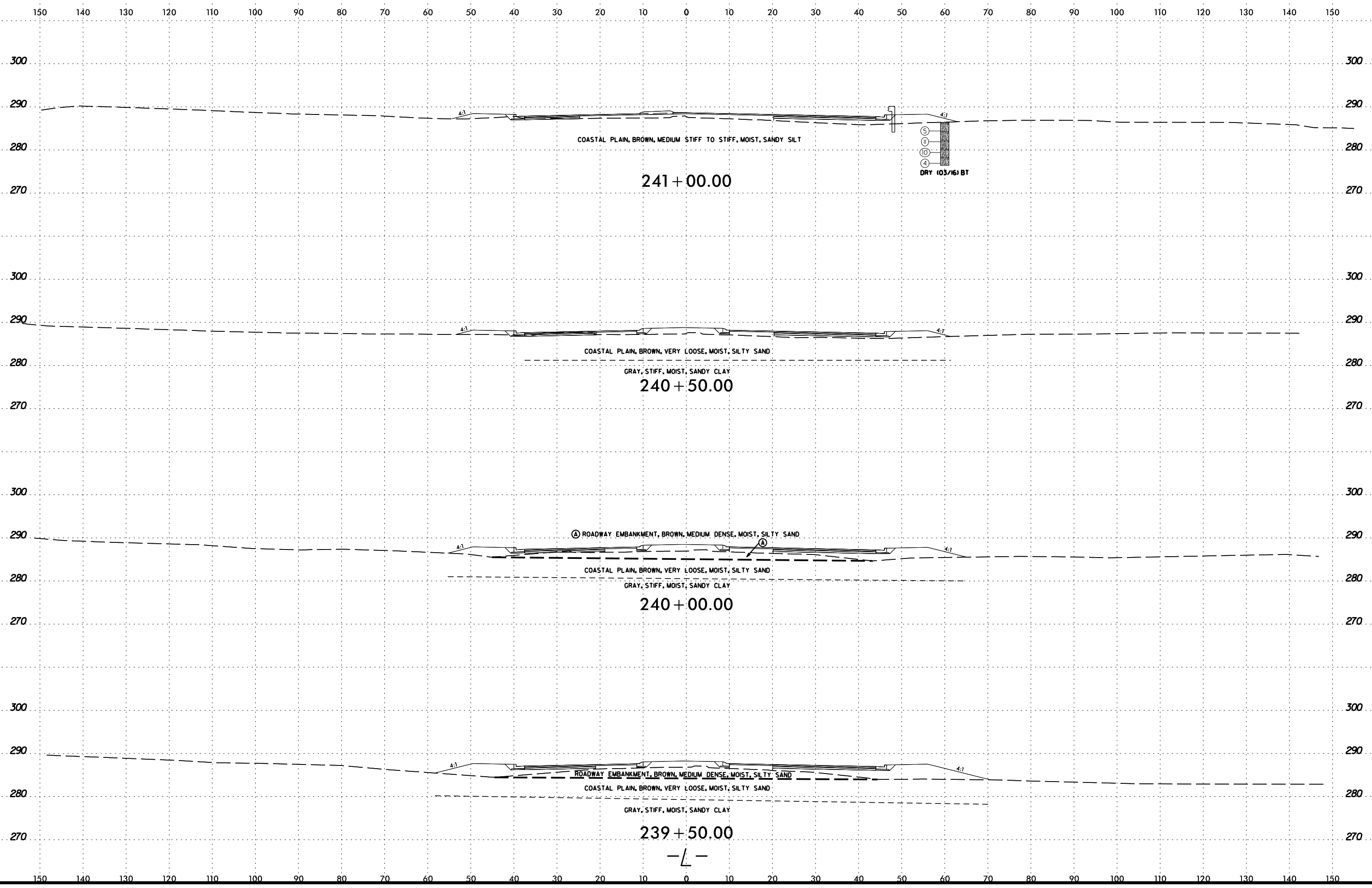


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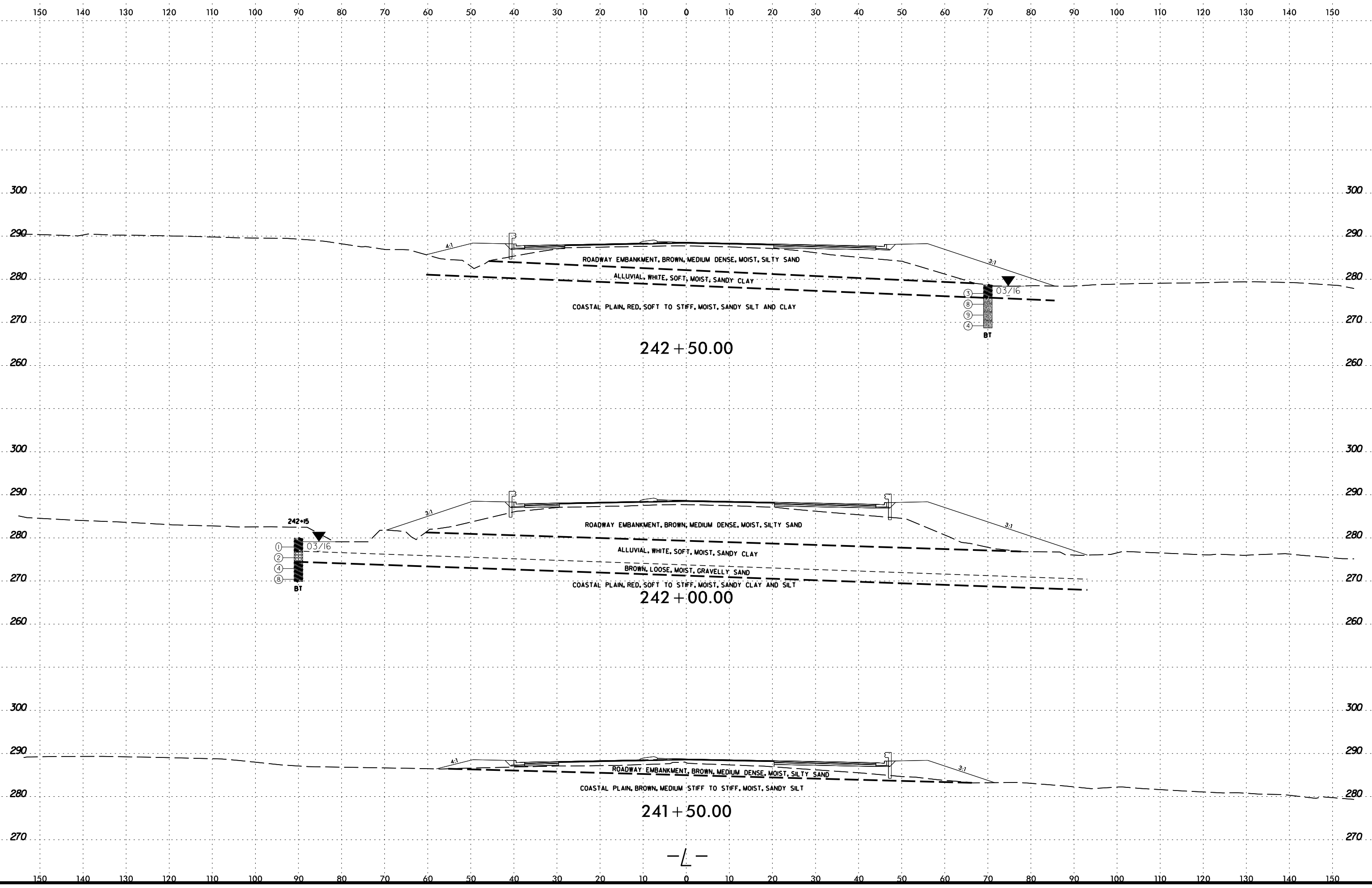
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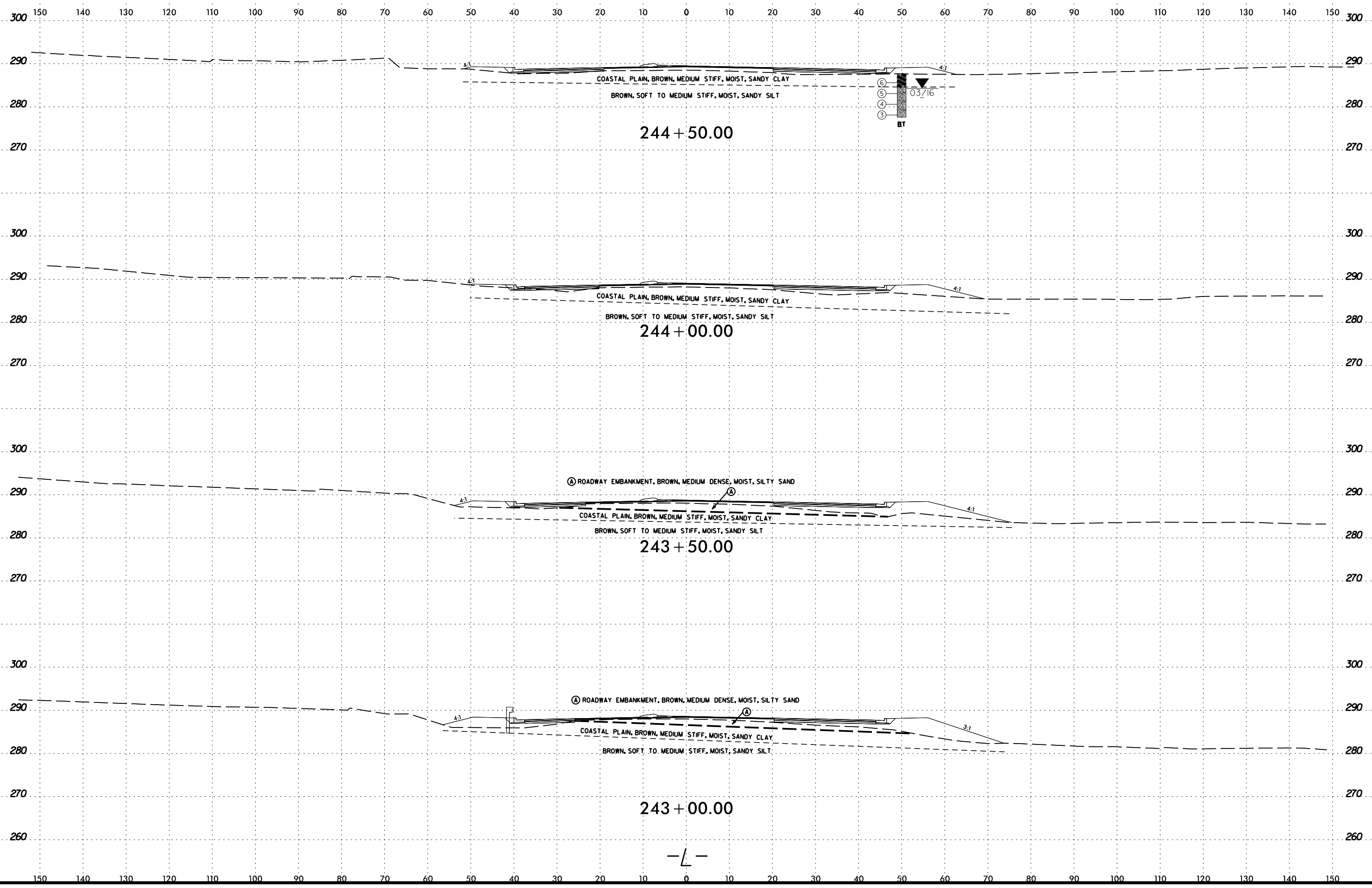
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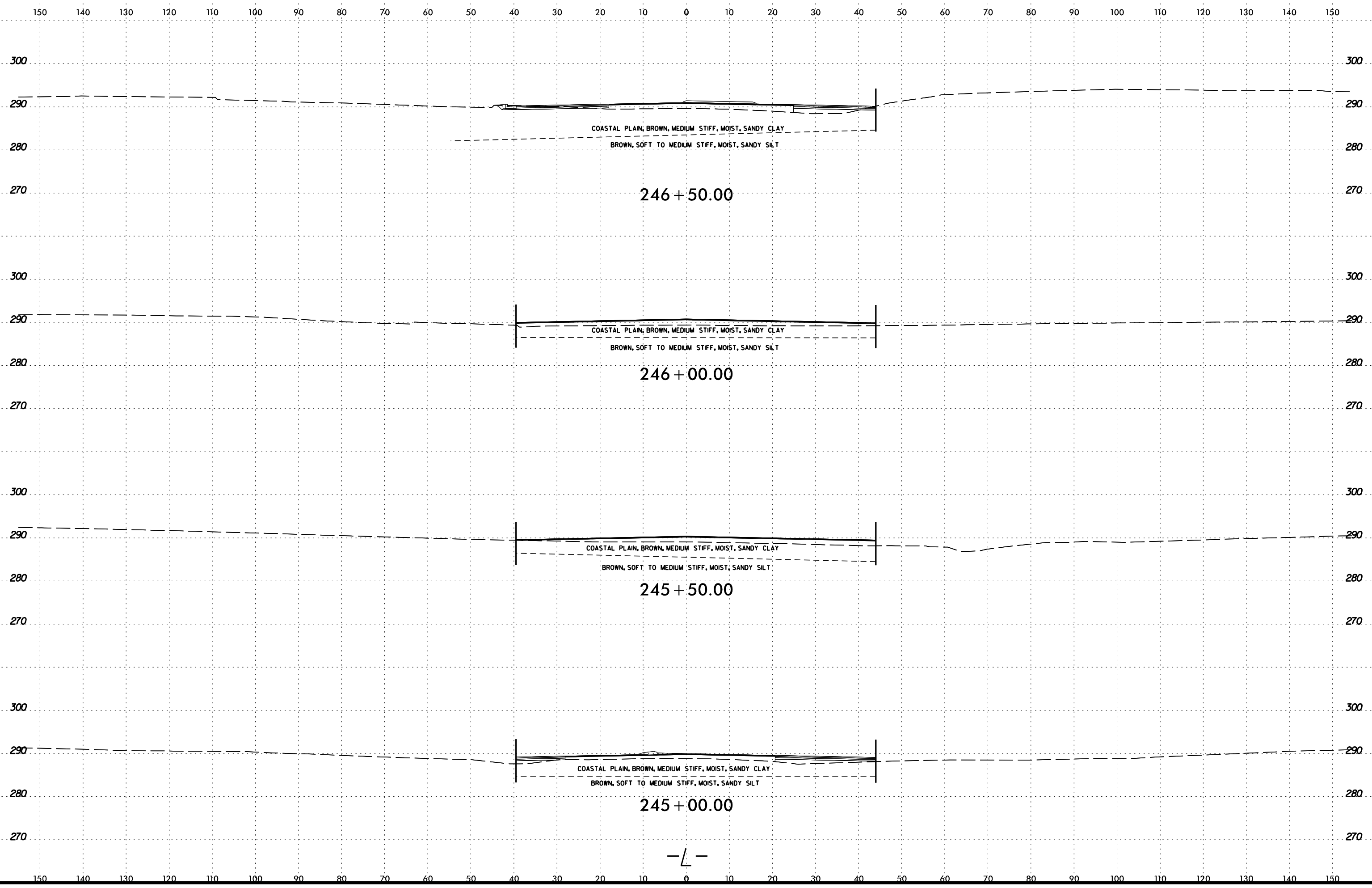
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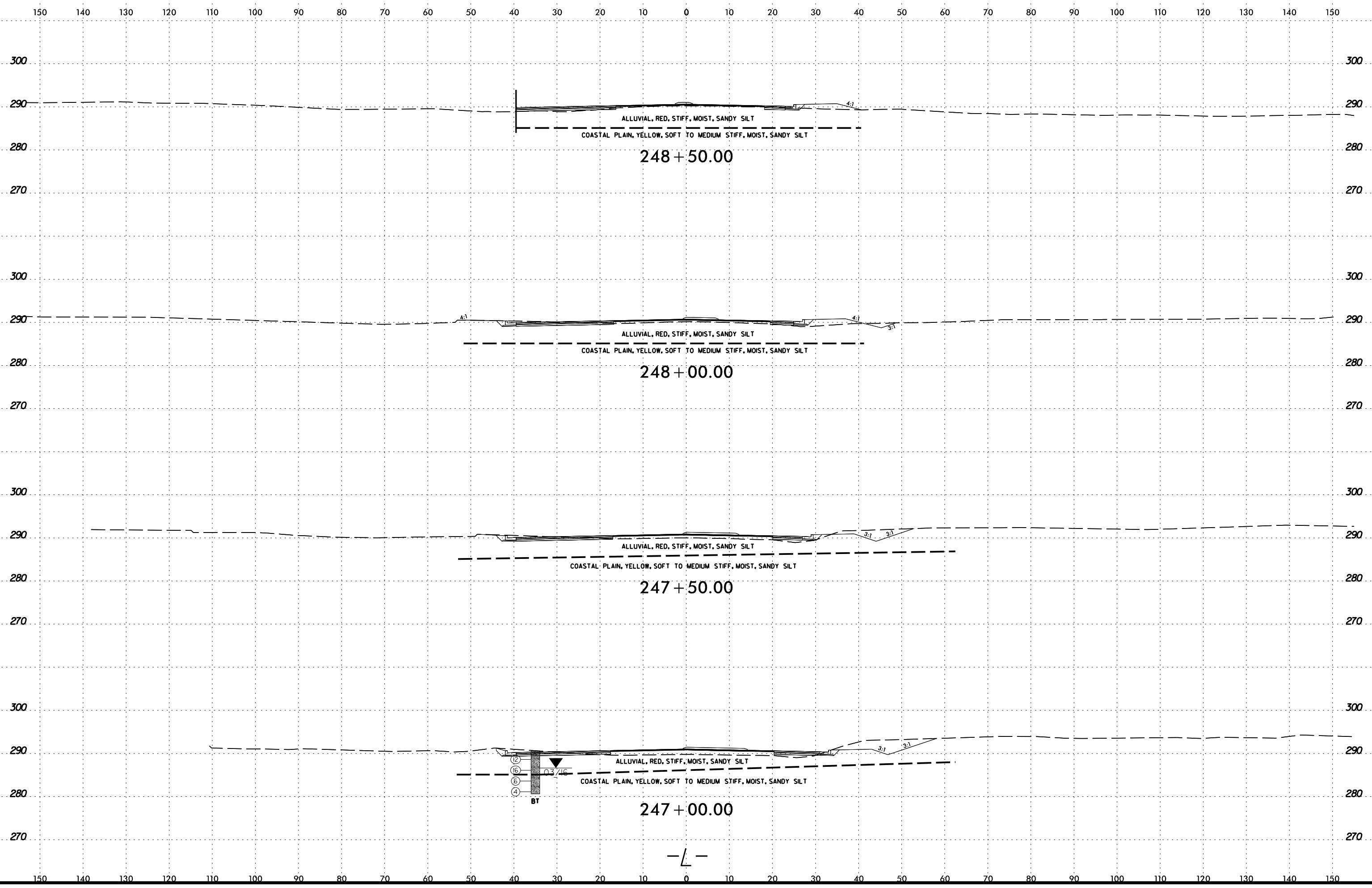
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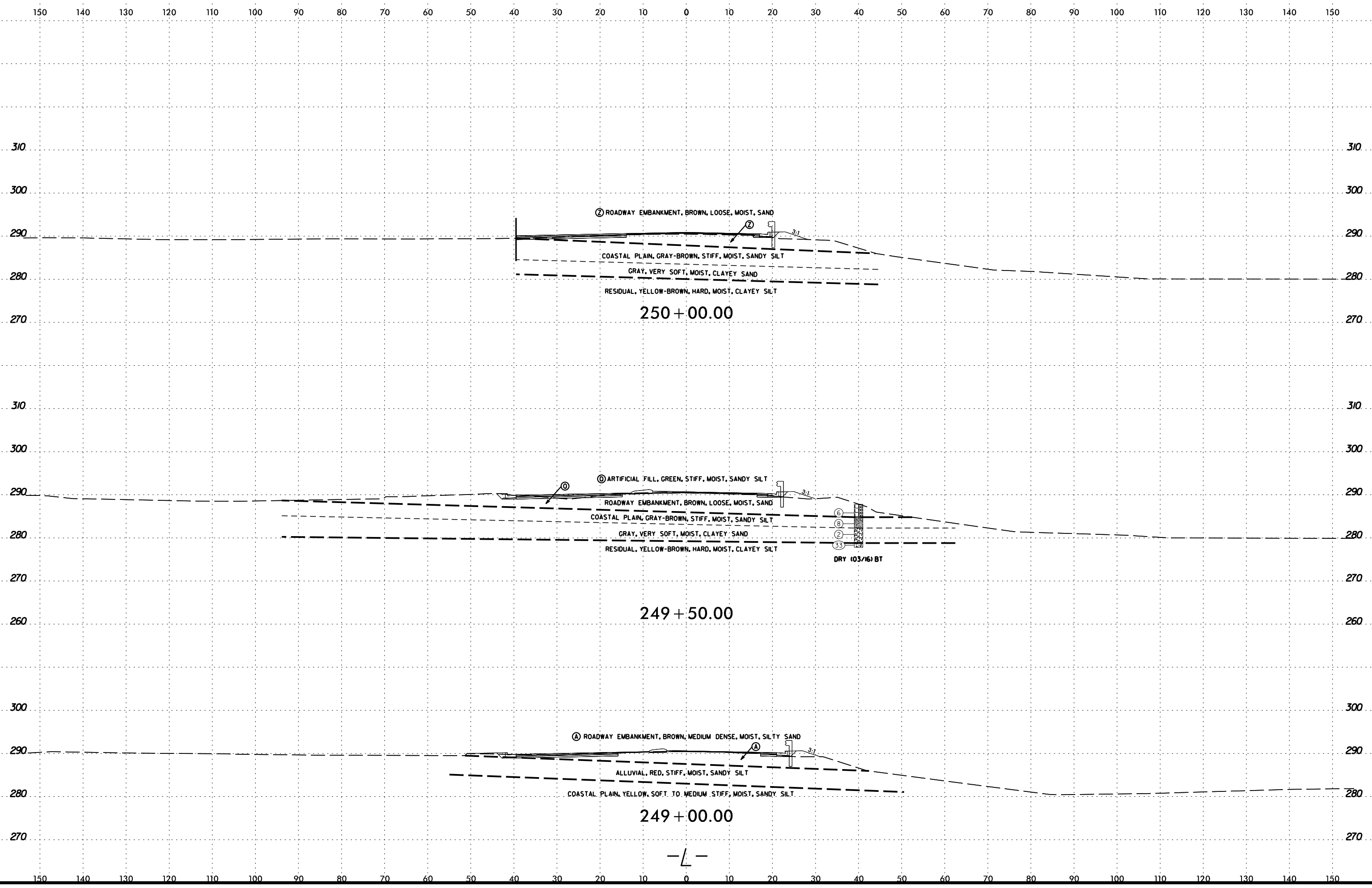
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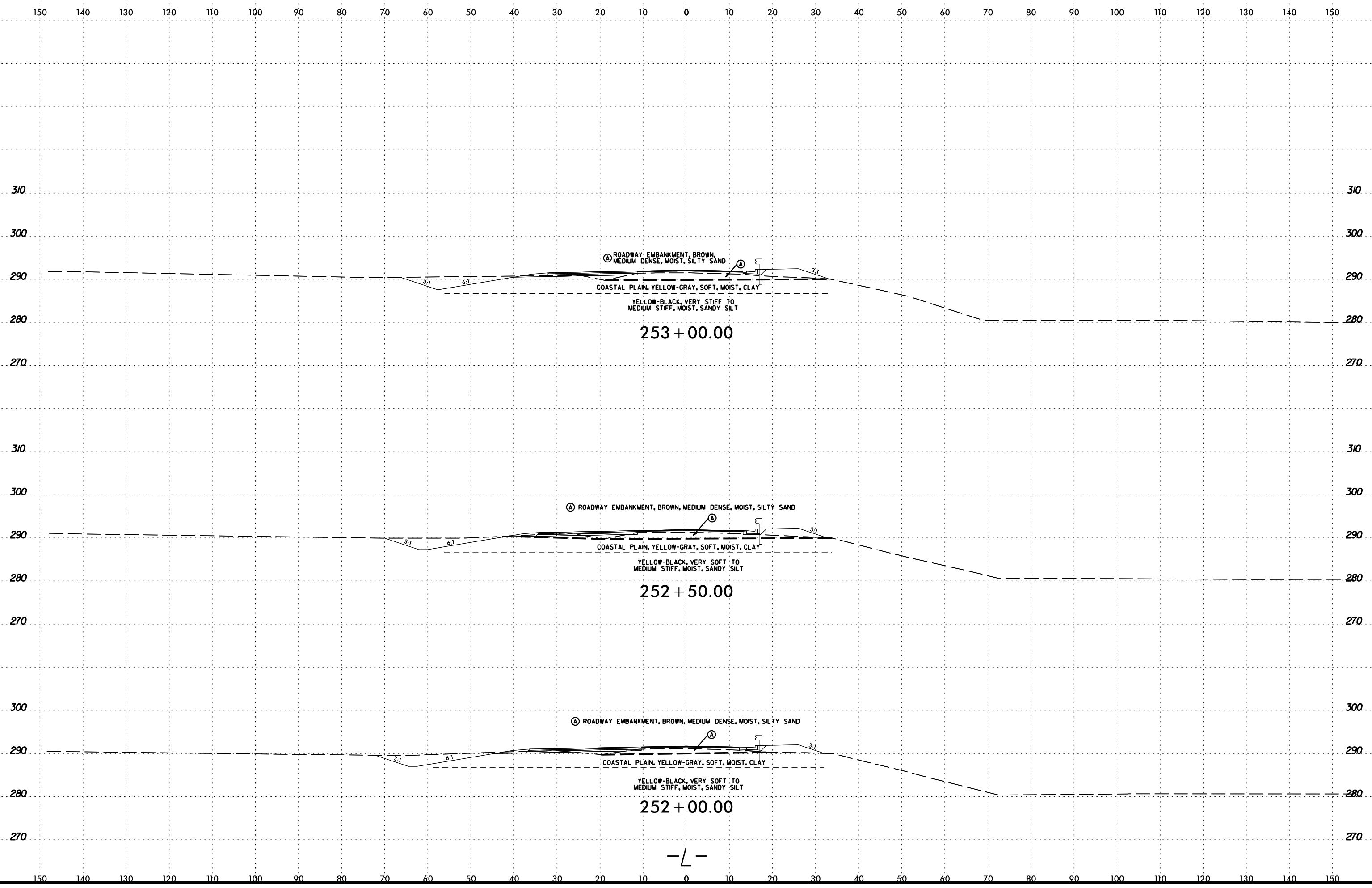
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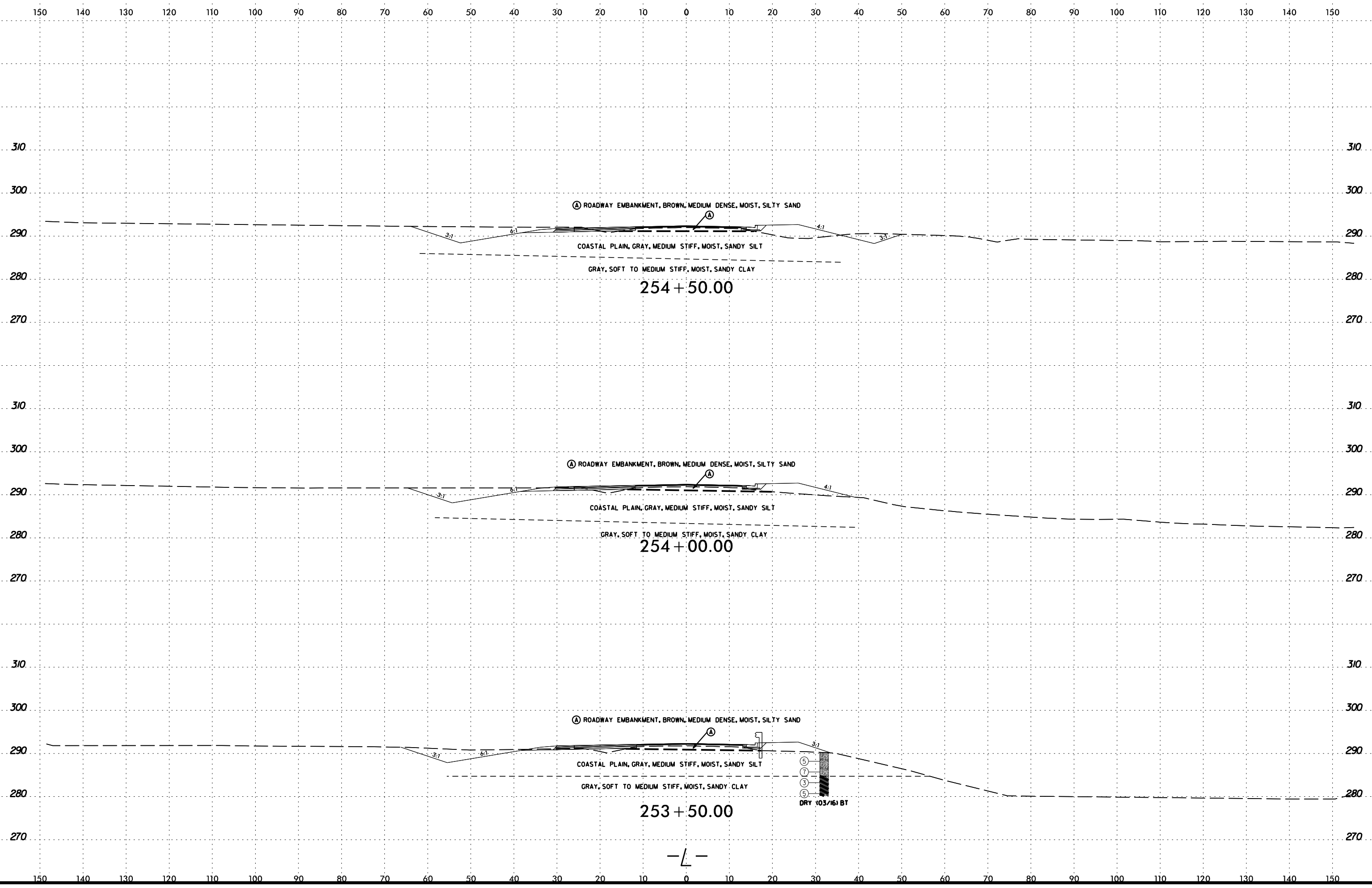
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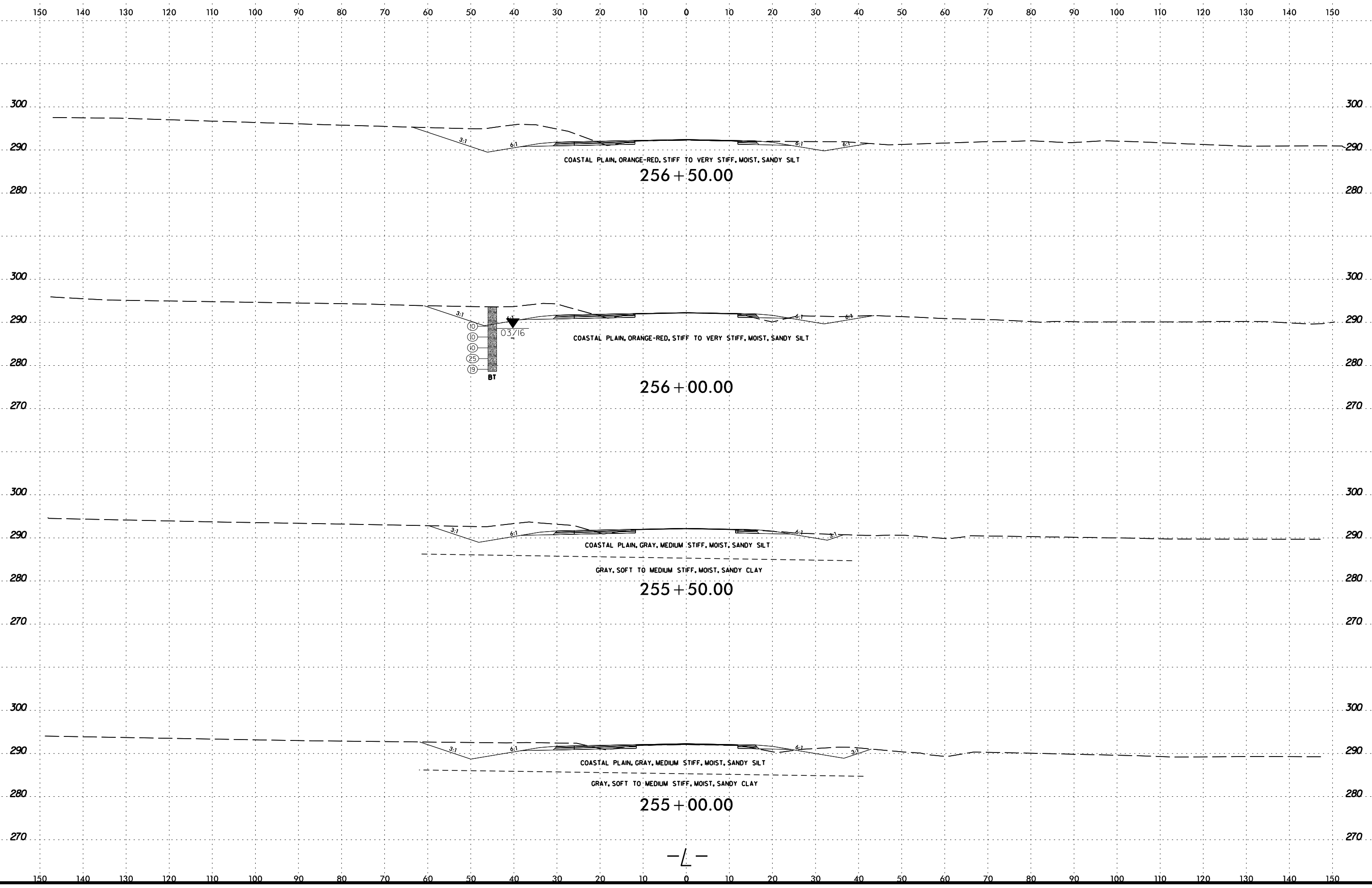
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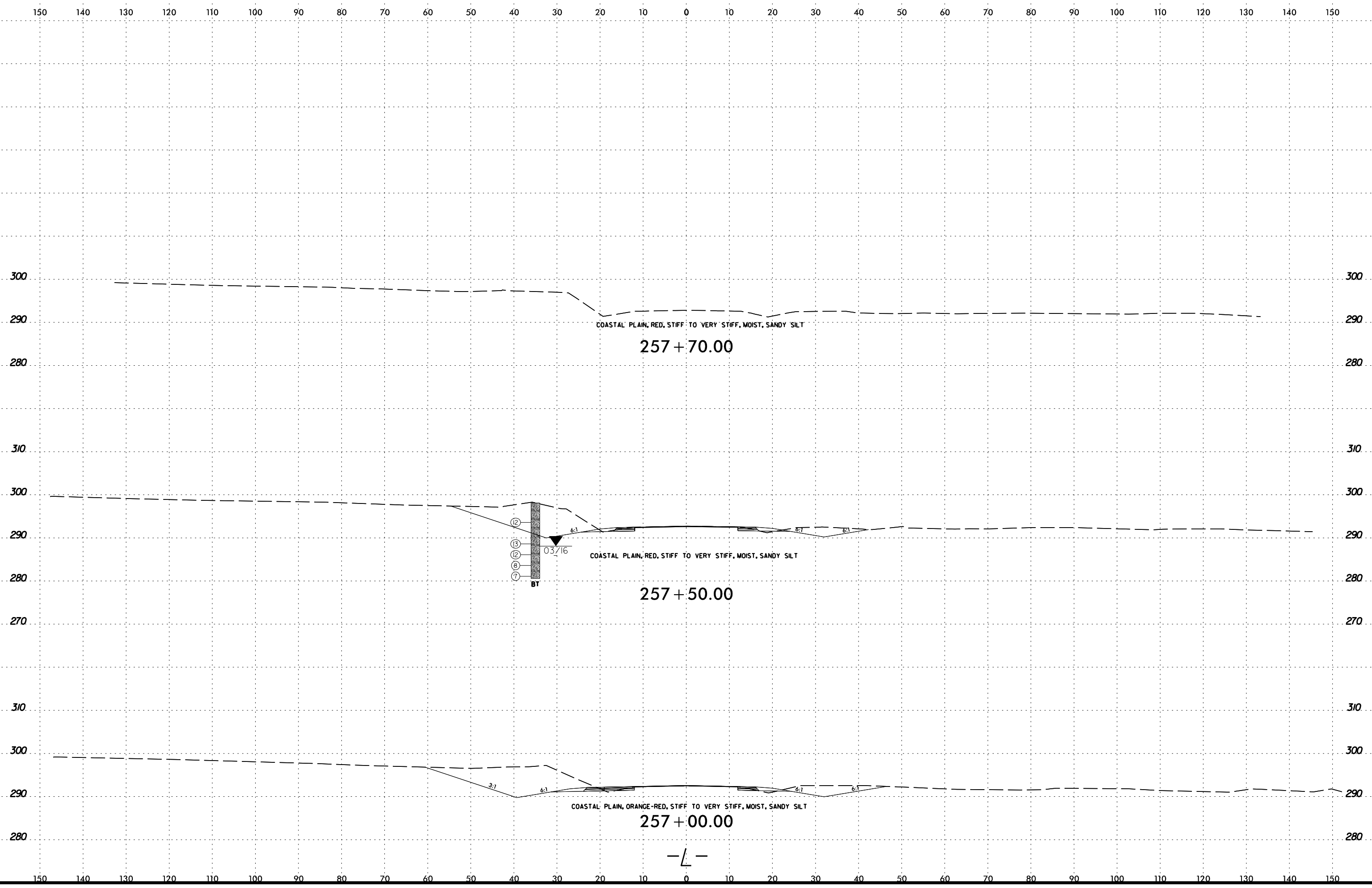
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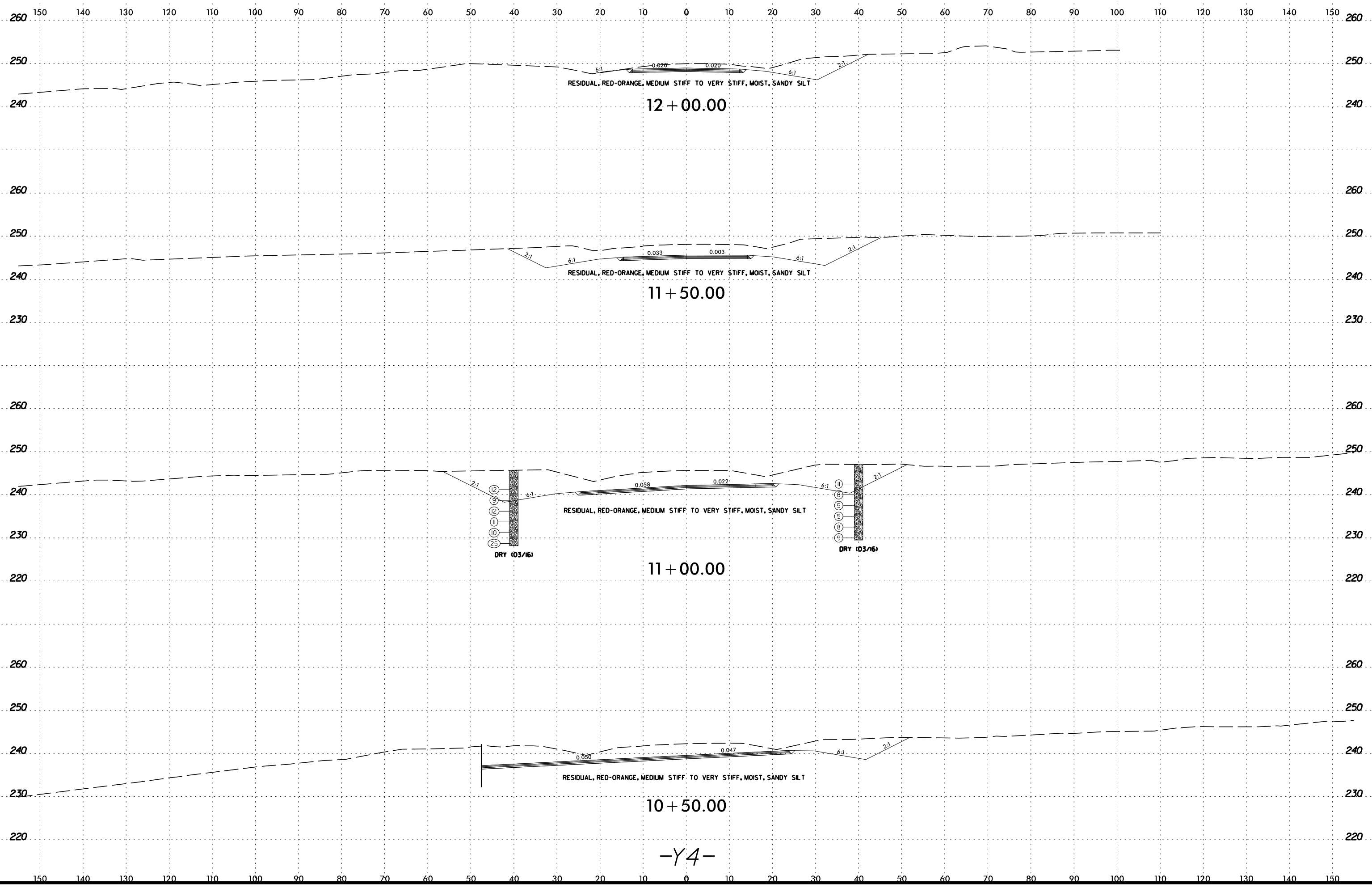




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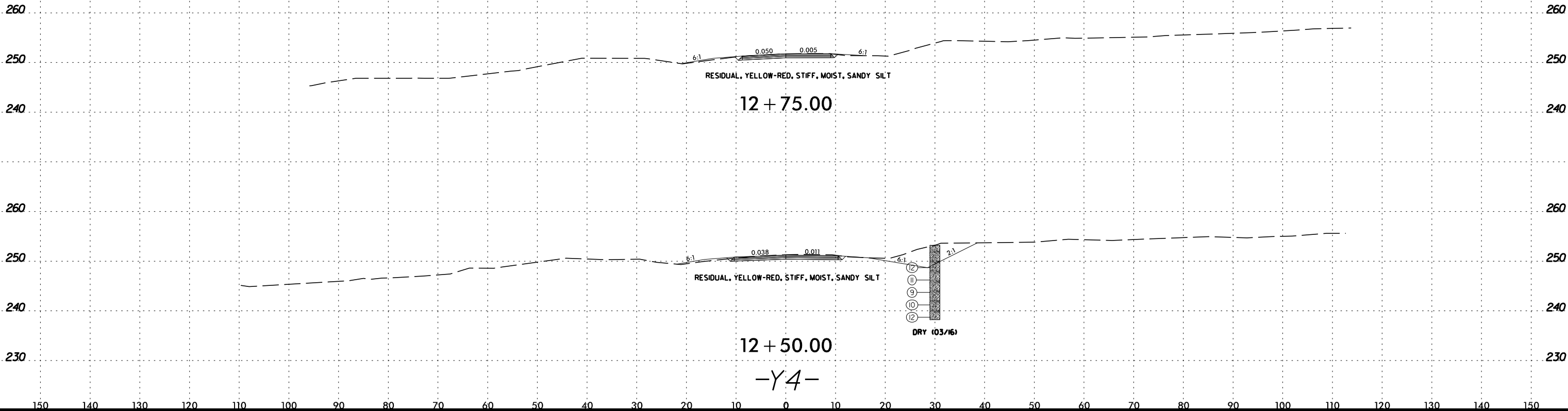


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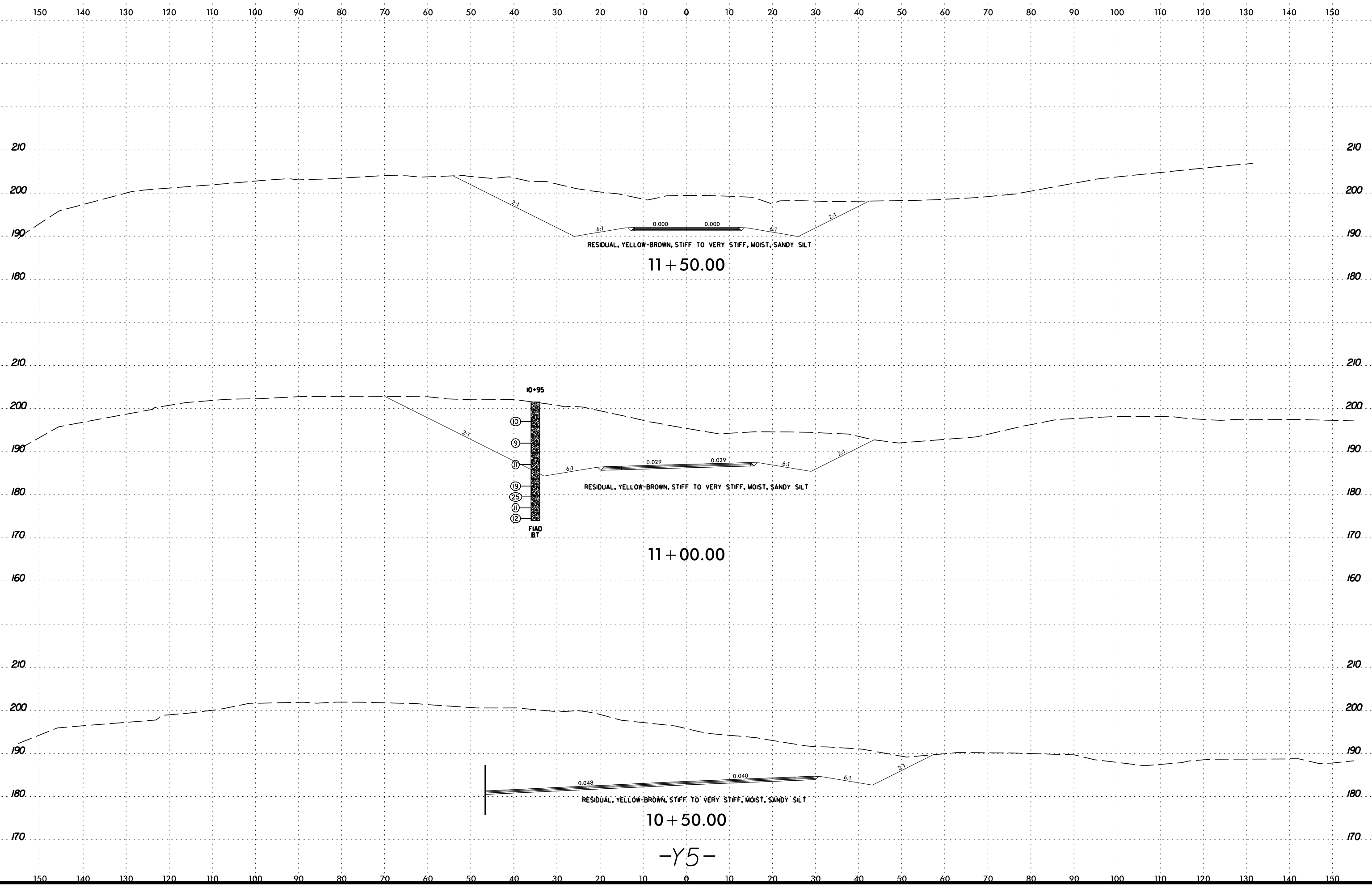
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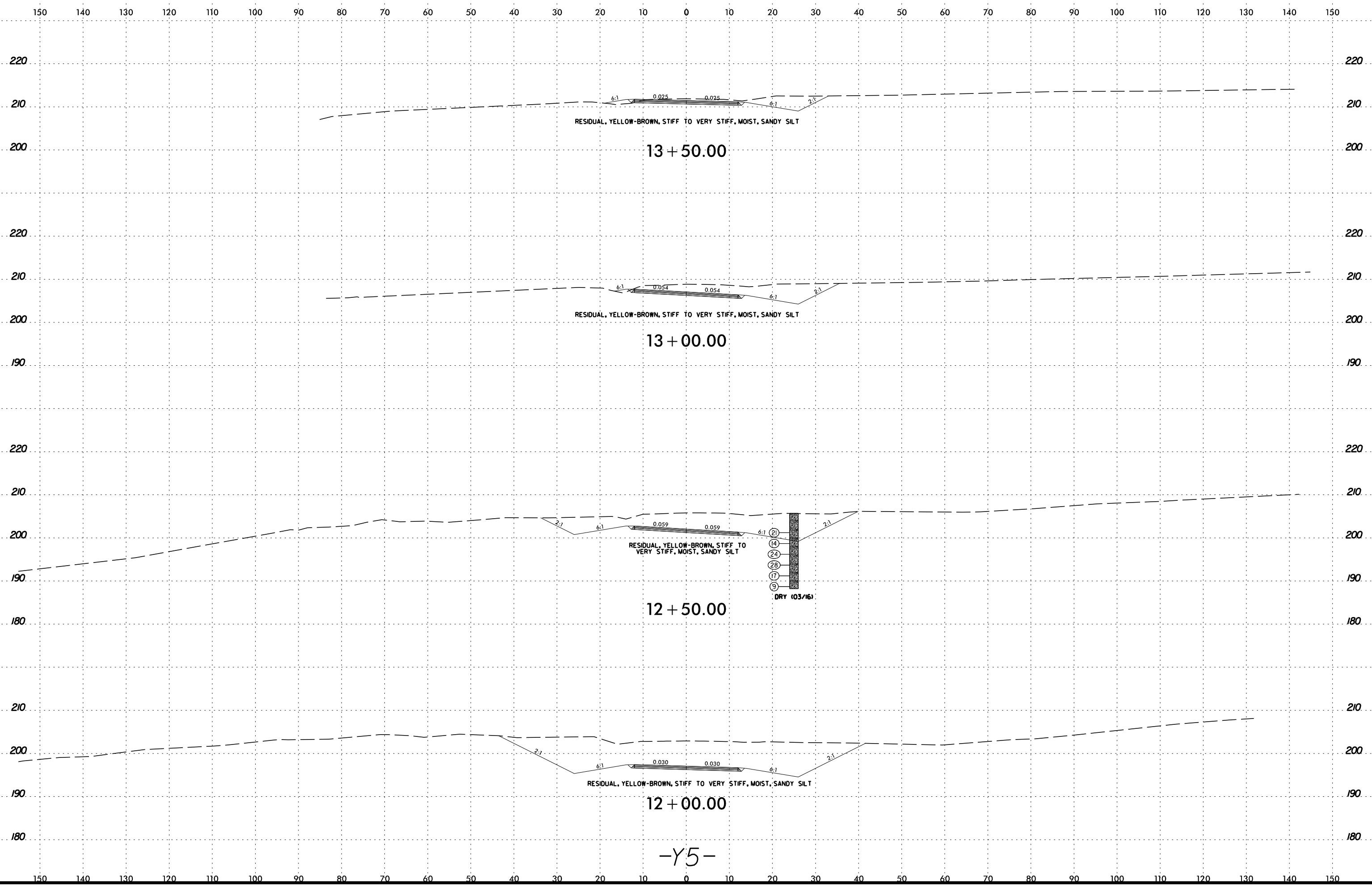
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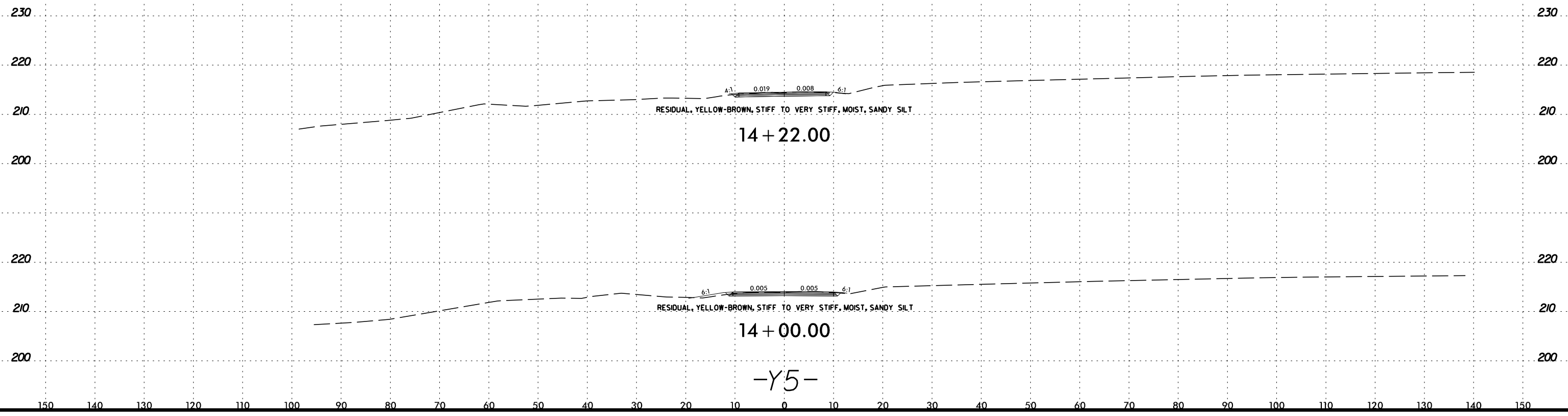
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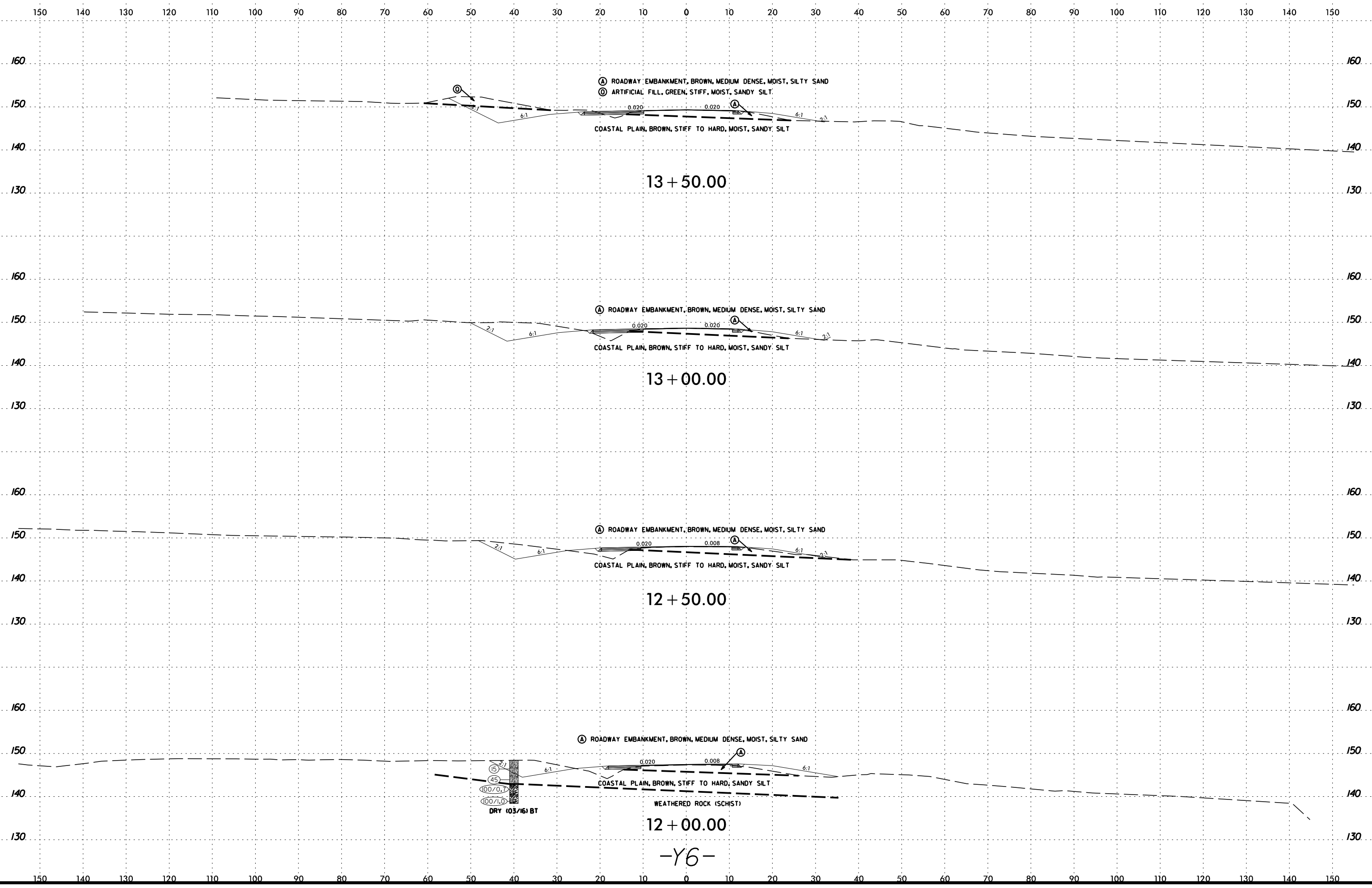


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Ⓐ ROADWAY EMBANKMENT, BROWN, MEDIUM DENSE, MOIST, SILTY SAND
 Ⓞ ARTIFICIAL FILL, GREEN, STIFF, MOIST, SANDY SILT

13 + 50.00

Ⓐ ROADWAY EMBANKMENT, BROWN, MEDIUM DENSE, MOIST, SILTY SAND
 Ⓞ ARTIFICIAL FILL, GREEN, STIFF, MOIST, SANDY SILT

13 + 00.00

Ⓐ ROADWAY EMBANKMENT, BROWN, MEDIUM DENSE, MOIST, SILTY SAND
 Ⓞ ARTIFICIAL FILL, GREEN, STIFF, MOIST, SANDY SILT

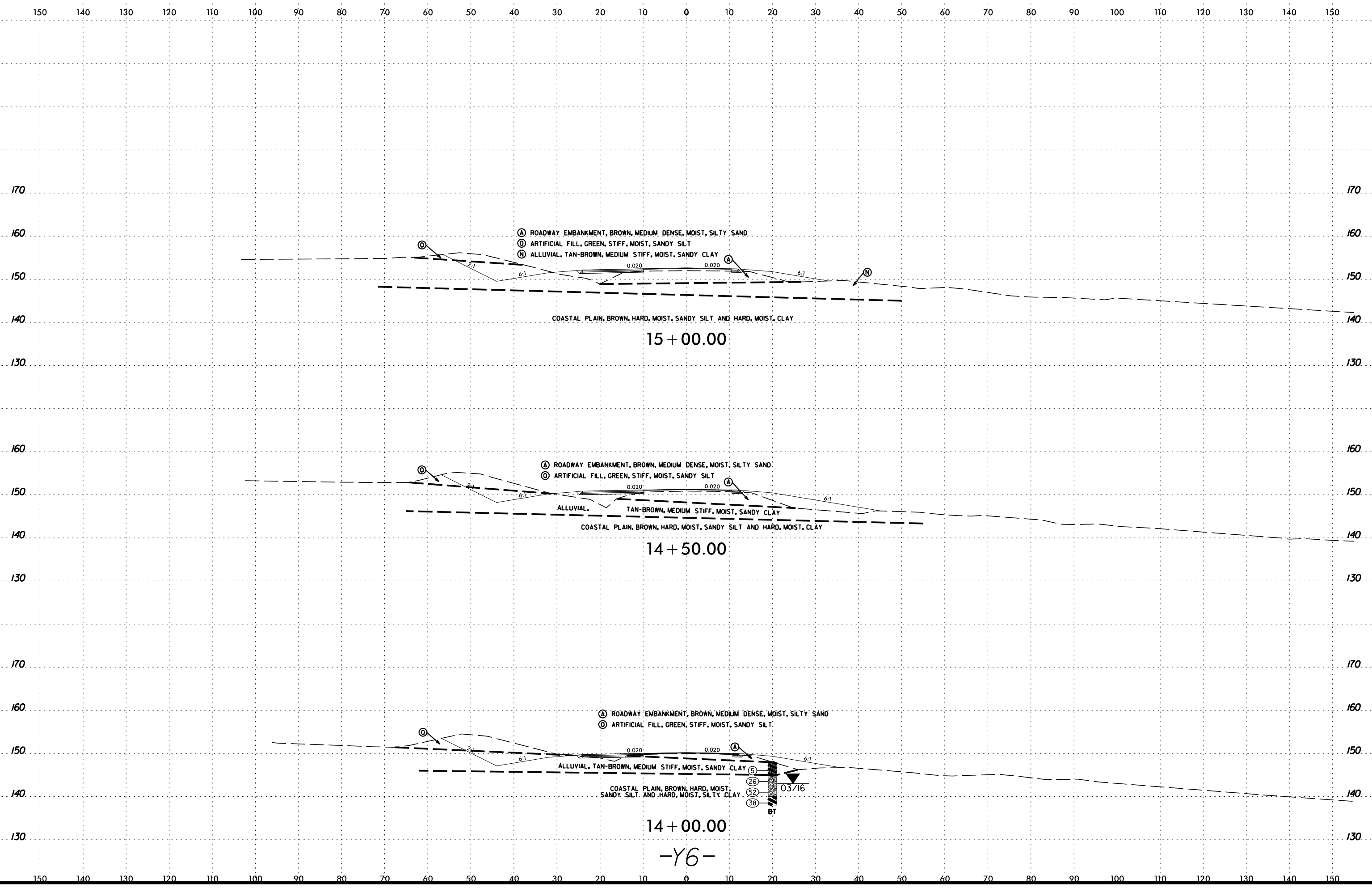
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Ⓐ ROADWAY EMBANKMENT, BROWN, MEDIUM DENSE, MOIST, SILTY SAND
 Ⓞ ARTIFICIAL FILL, GREEN, STIFF, MOIST, SANDY SILT

12 + 00.00

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6/23/16

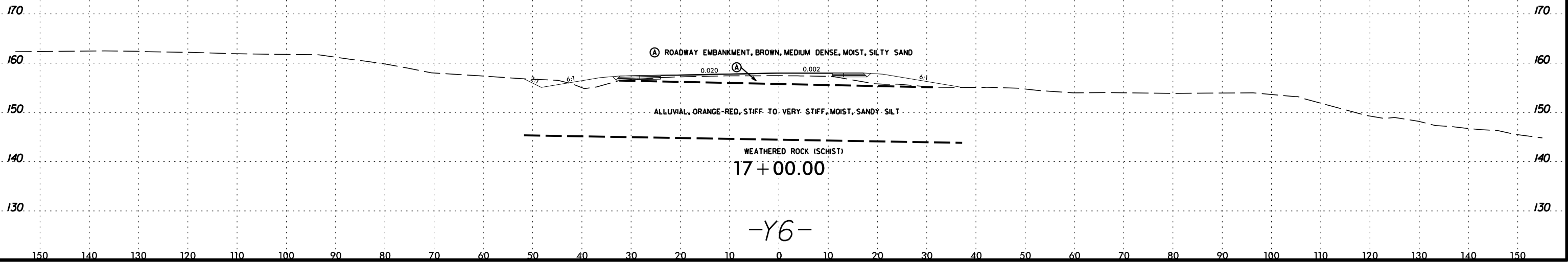


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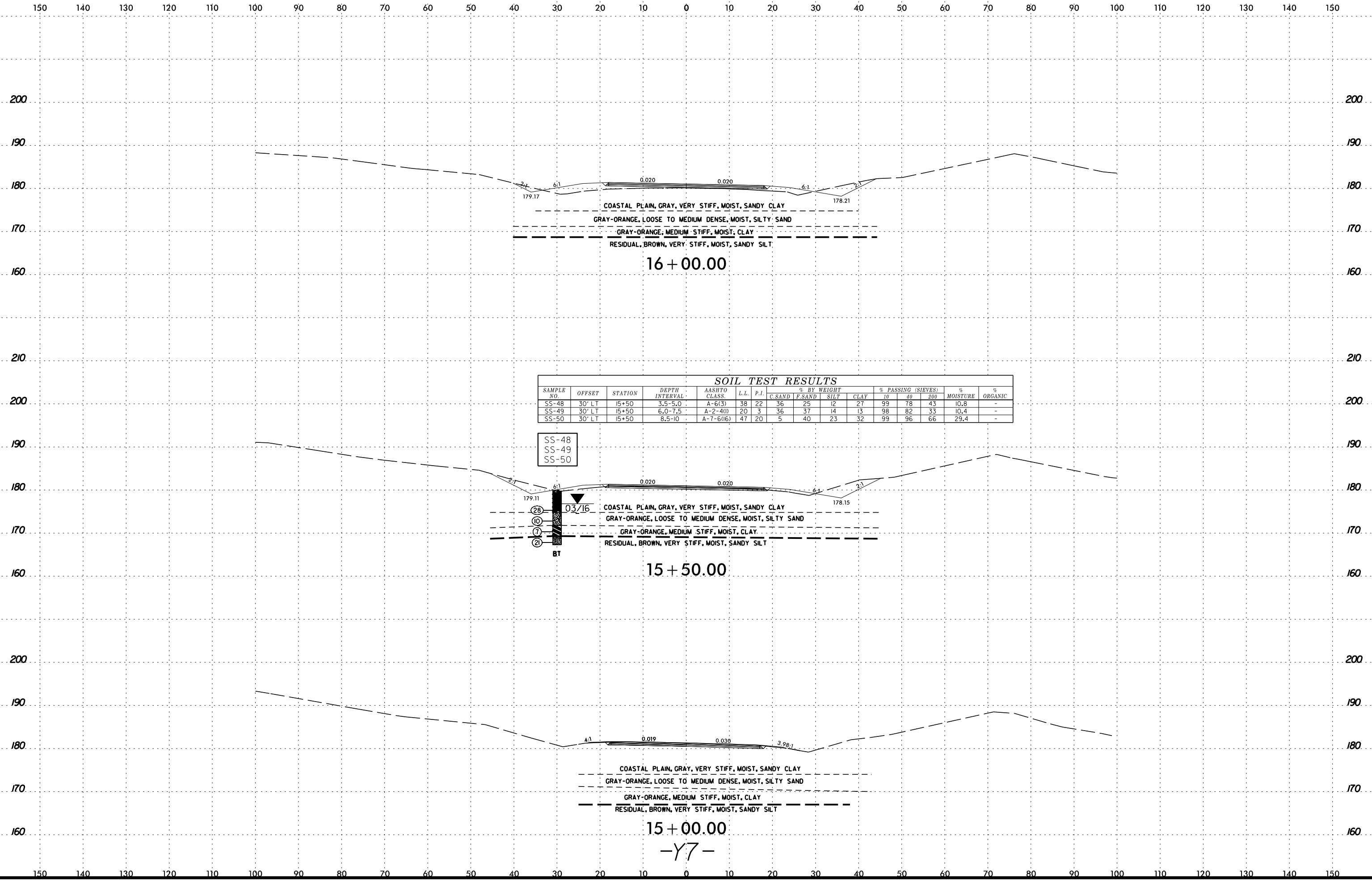
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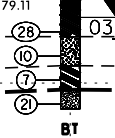
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SOIL TEST RESULTS

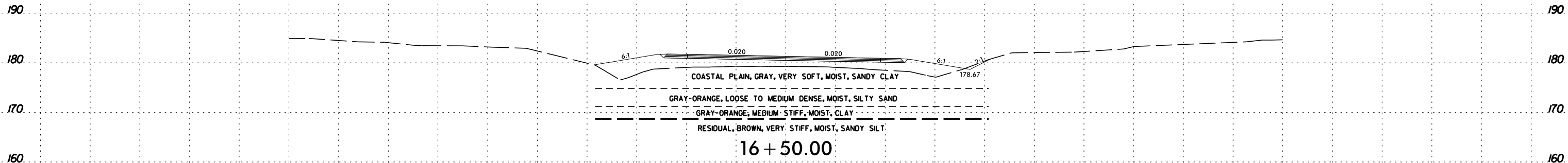
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							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-48	30' LT	15+50	3.5-5.0	A-6(3)	38	22	36	25	12	27	99	78	43	10.8	-
SS-49	30' LT	15+50	6.0-7.5	A-2-4(0)	20	3	36	37	14	13	98	82	33	10.4	-
SS-50	30' LT	15+50	8.5-10	A-7-6(16)	47	20	5	40	23	32	99	96	66	29.4	-

SS-48
SS-49
SS-50



6/23/16

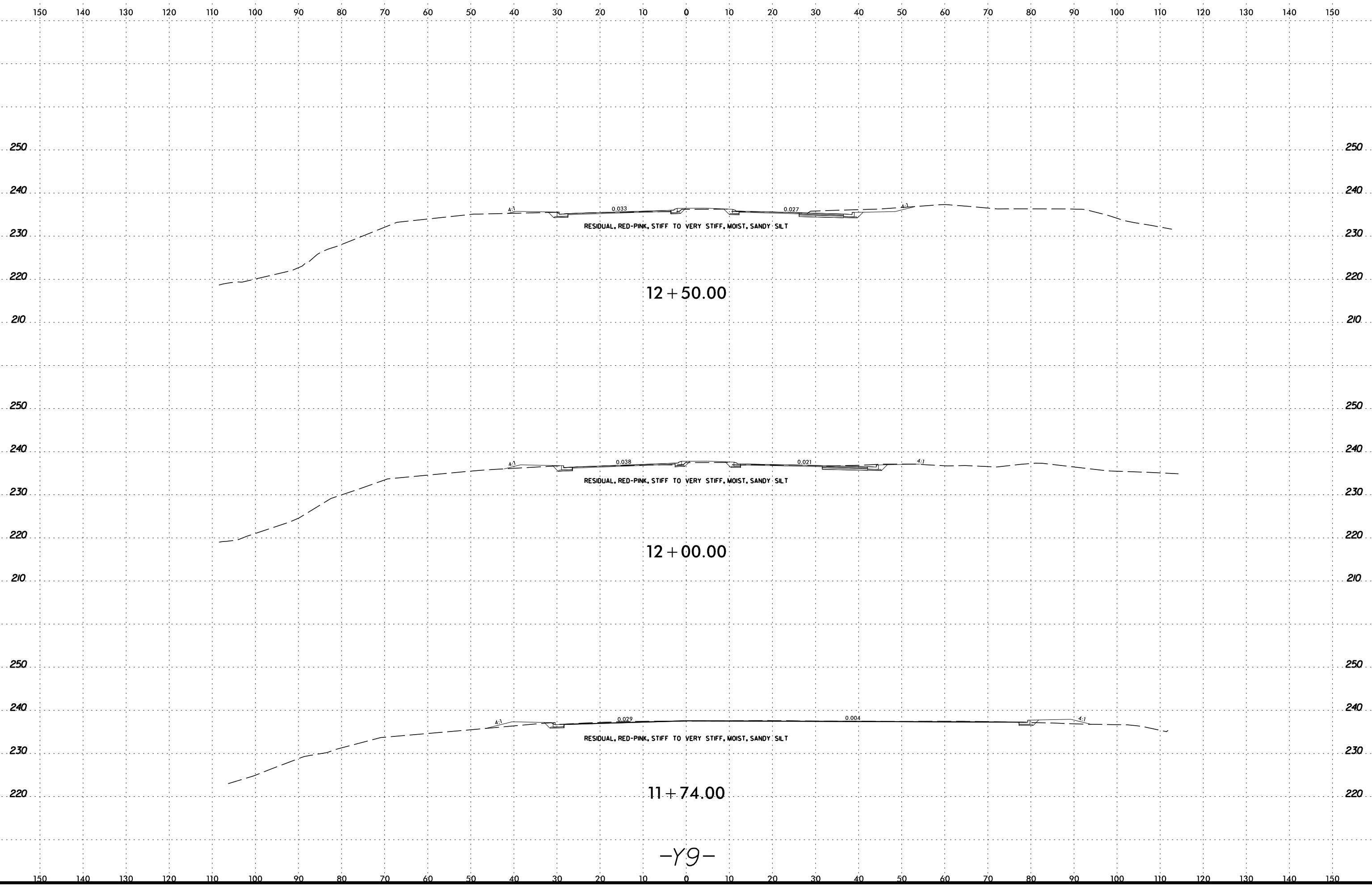
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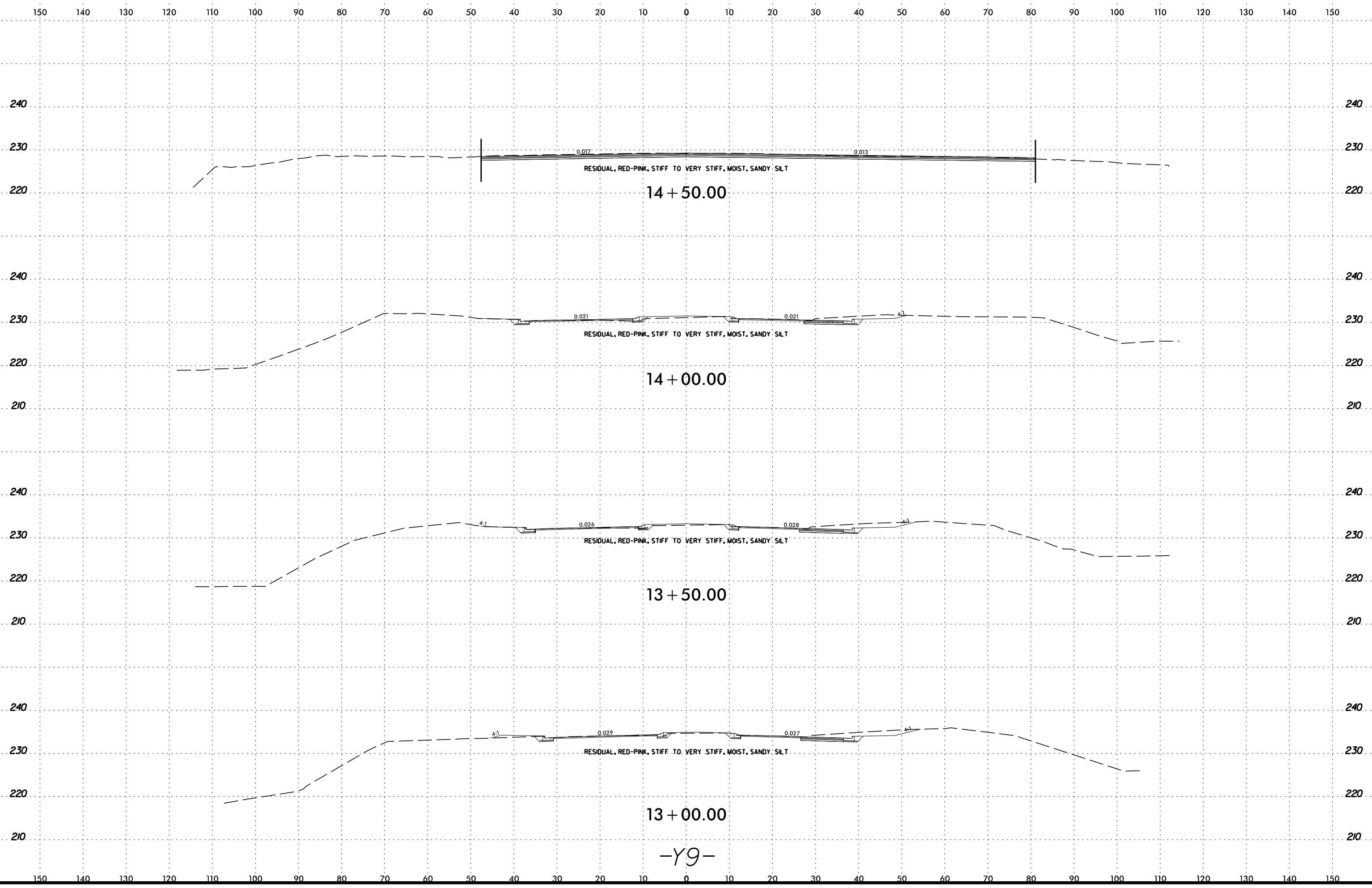
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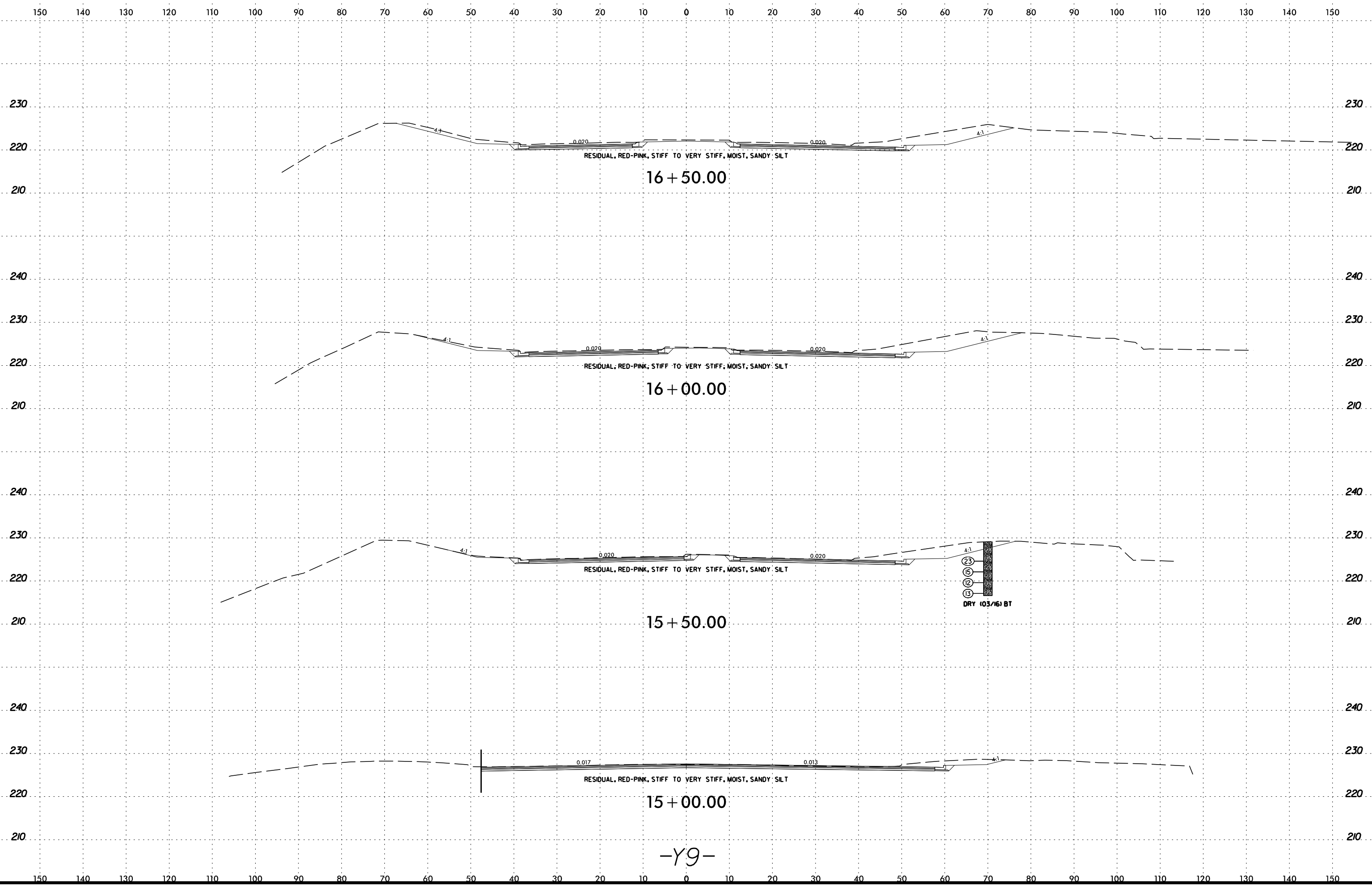
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6/23/16



16 + 50.00

16 + 00.00

15 + 50.00

15 + 00.00

-Y9-

RESIDUAL, RED-PINK, STIFF TO VERY STIFF, MOIST, SANDY SILT

RESIDUAL, RED-PINK, STIFF TO VERY STIFF, MOIST, SANDY SILT

RESIDUAL, RED-PINK, STIFF TO VERY STIFF, MOIST, SANDY SILT

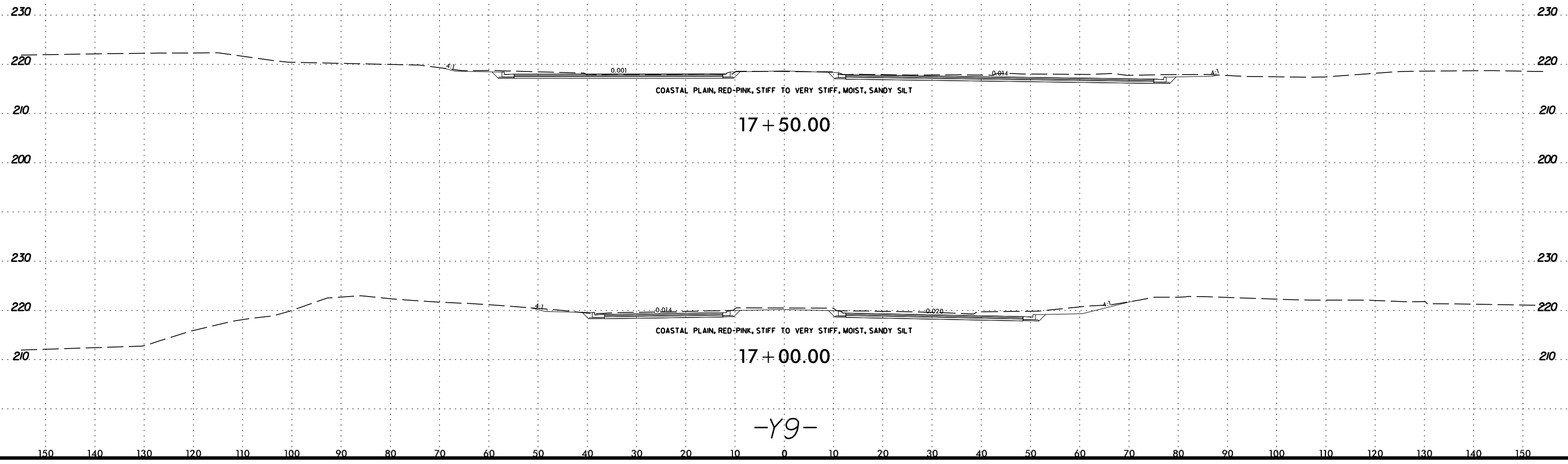
RESIDUAL, RED-PINK, STIFF TO VERY STIFF, MOIST, SANDY SILT

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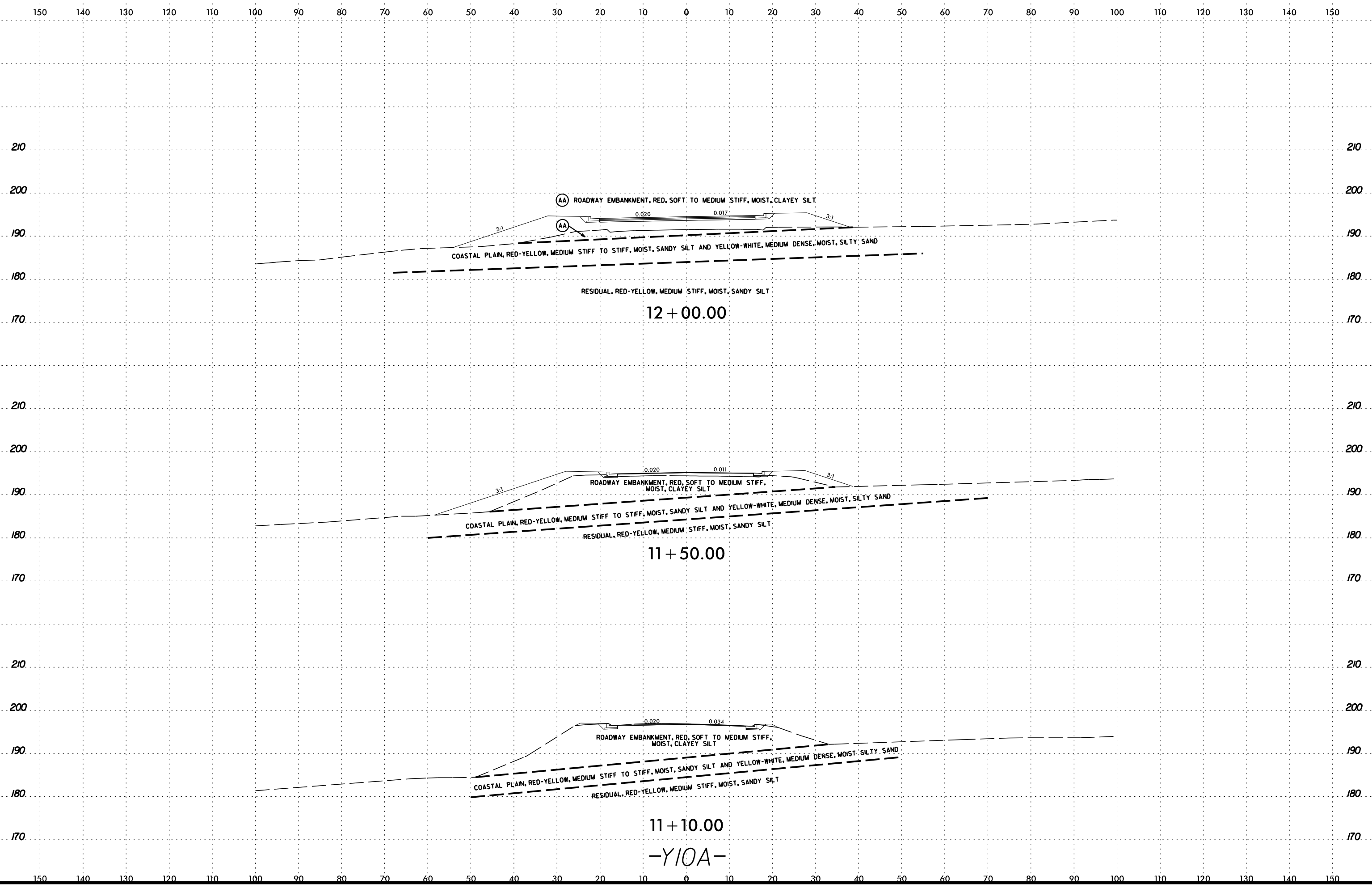
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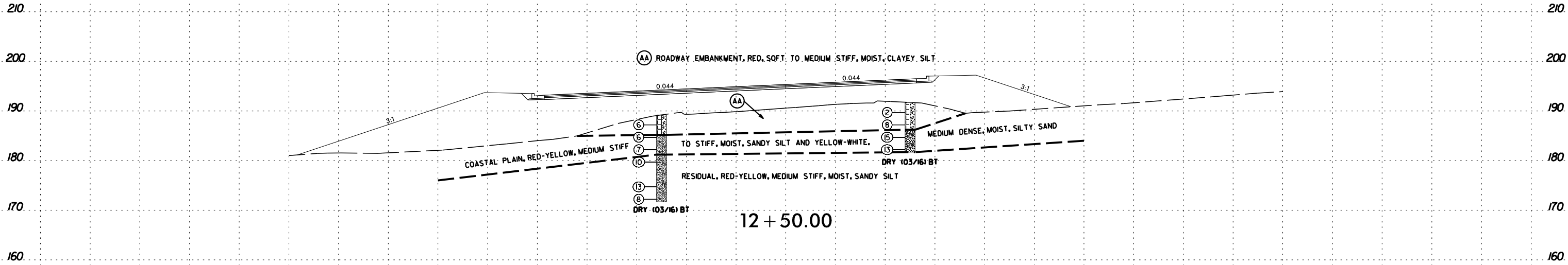
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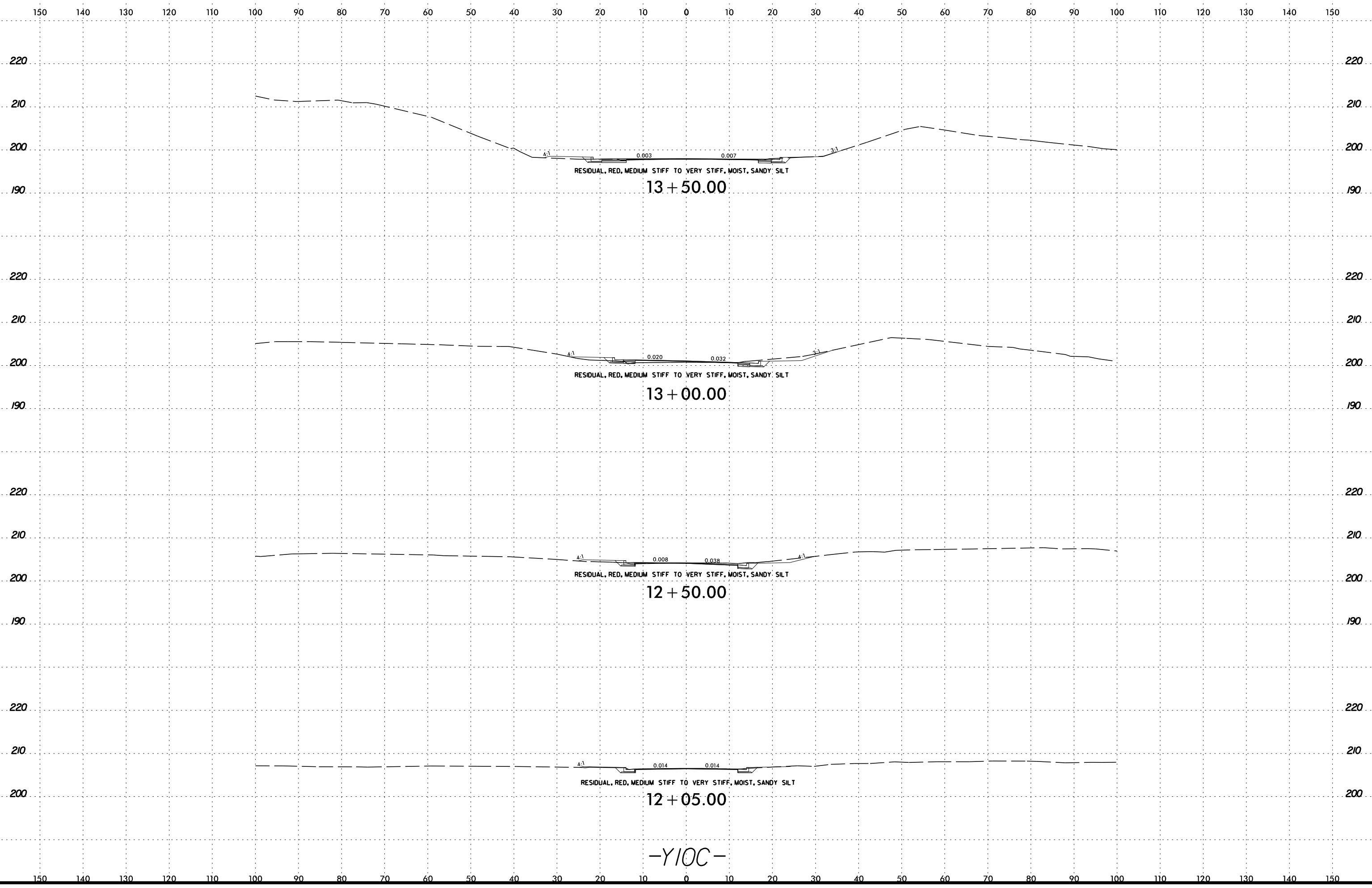
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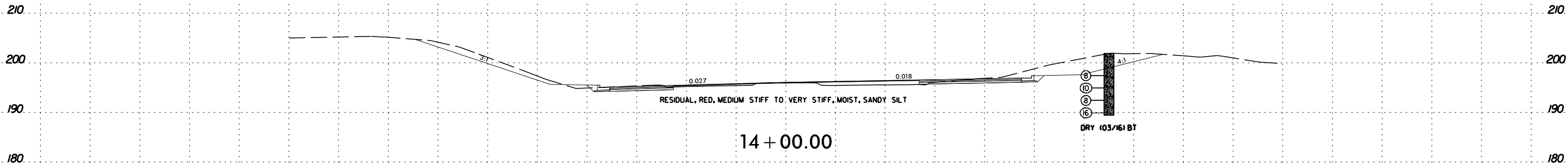
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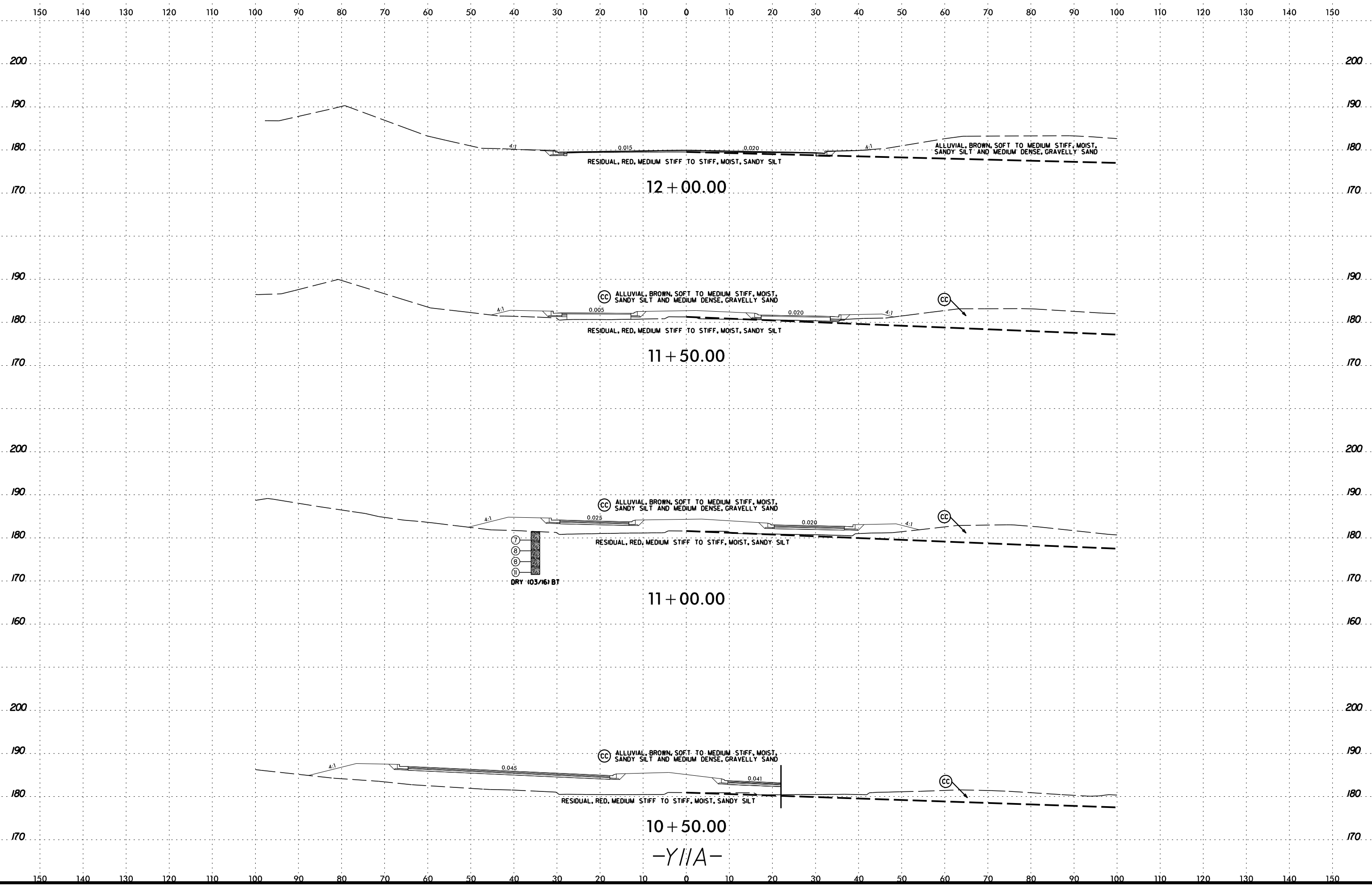
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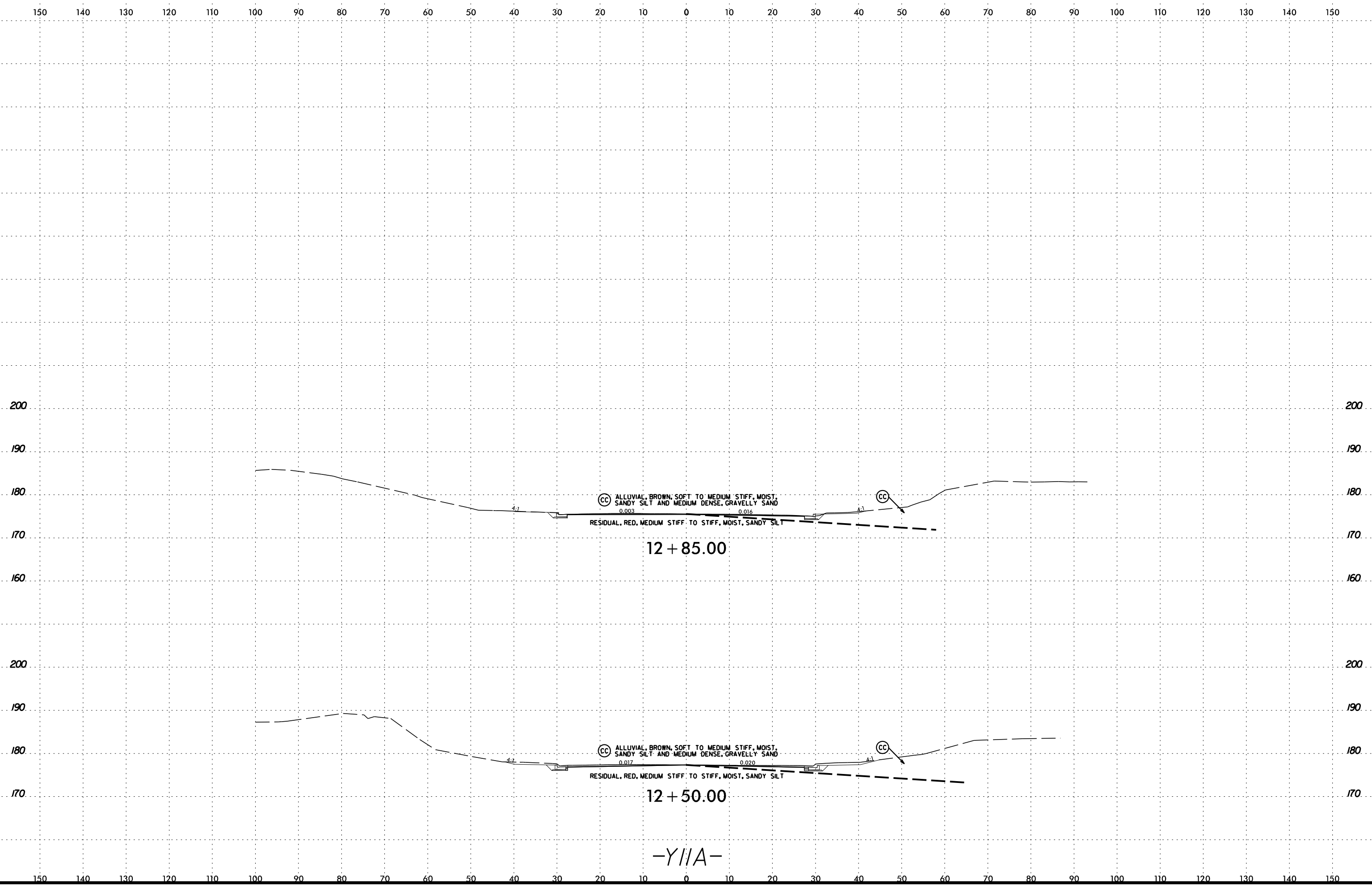
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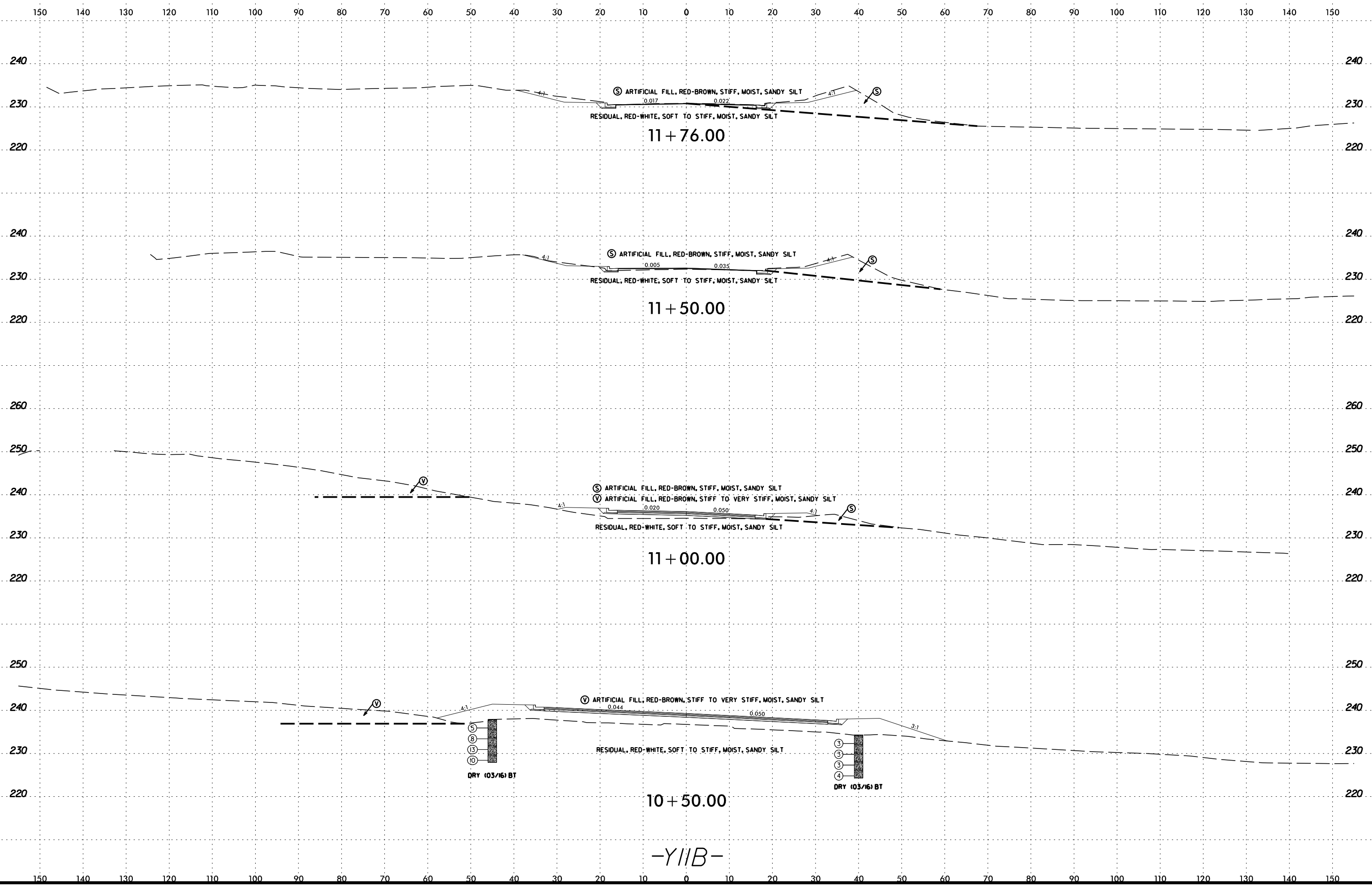
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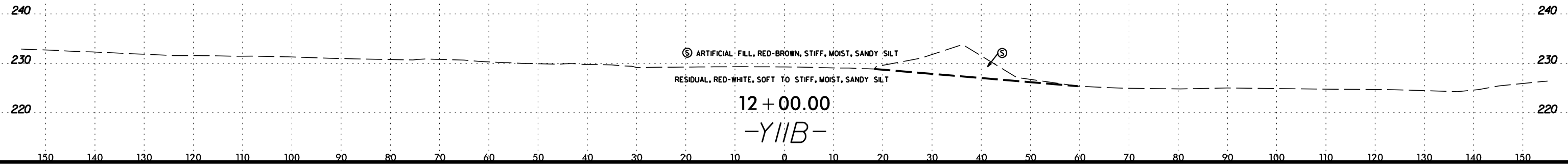


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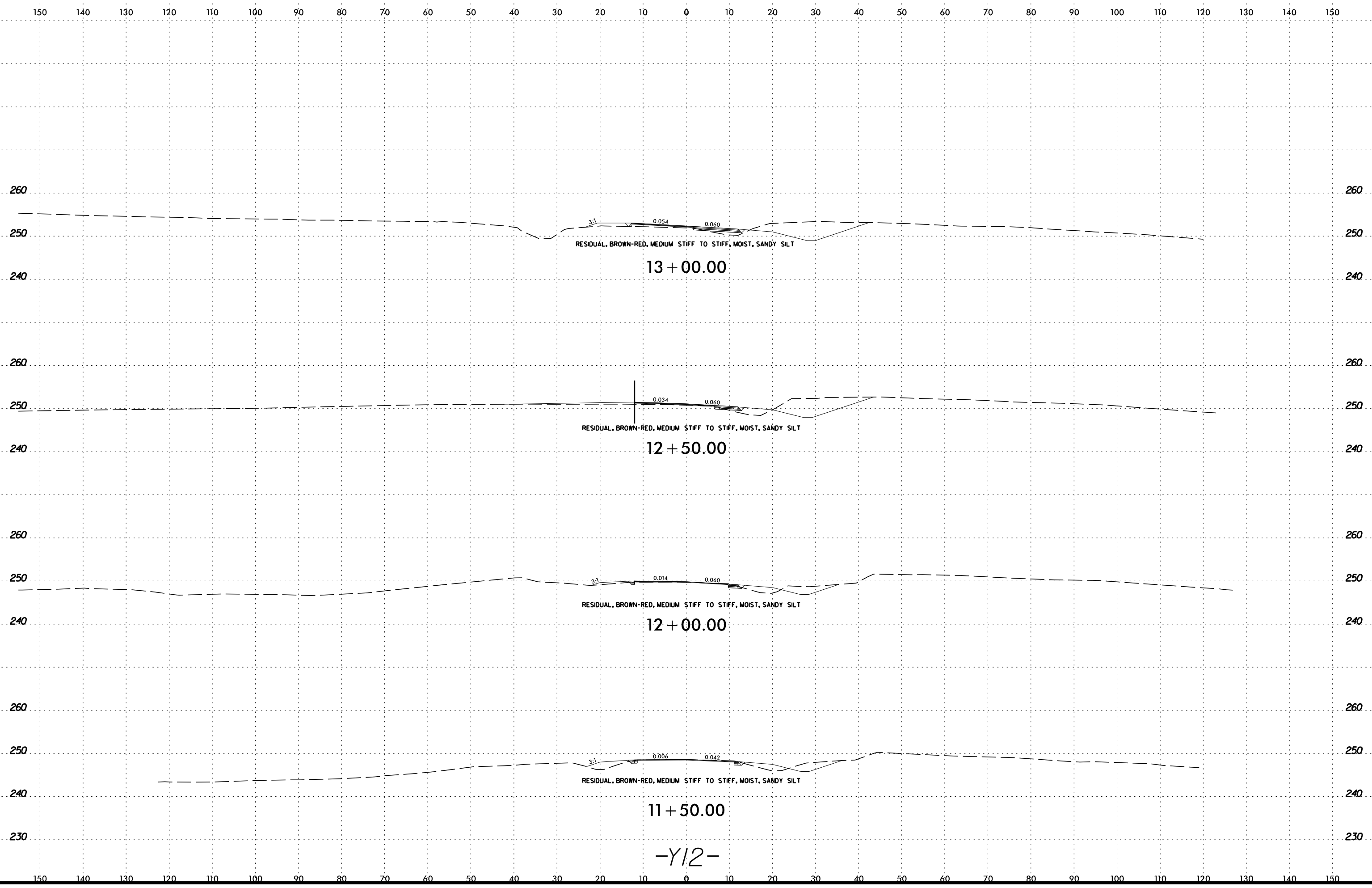
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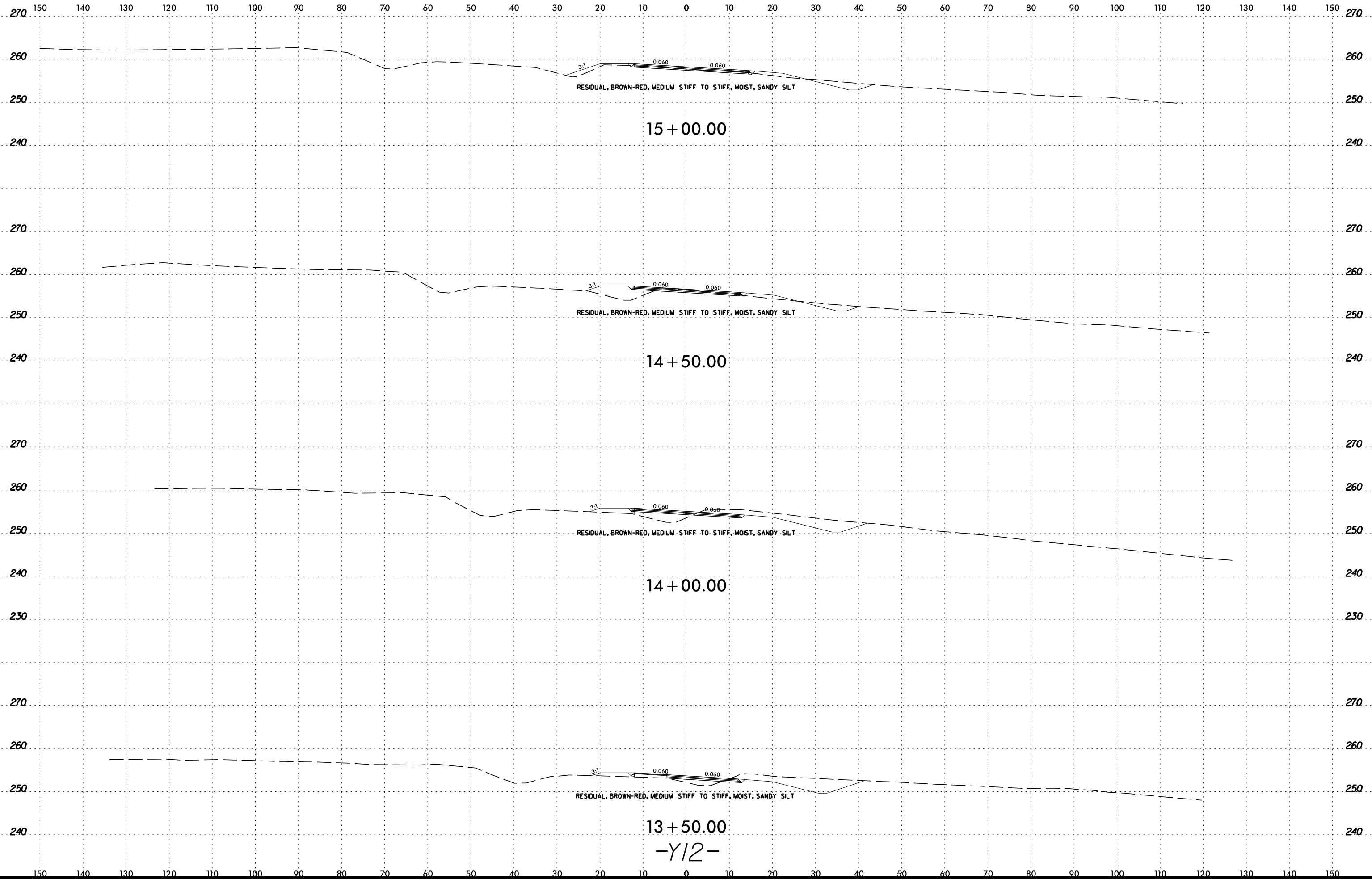
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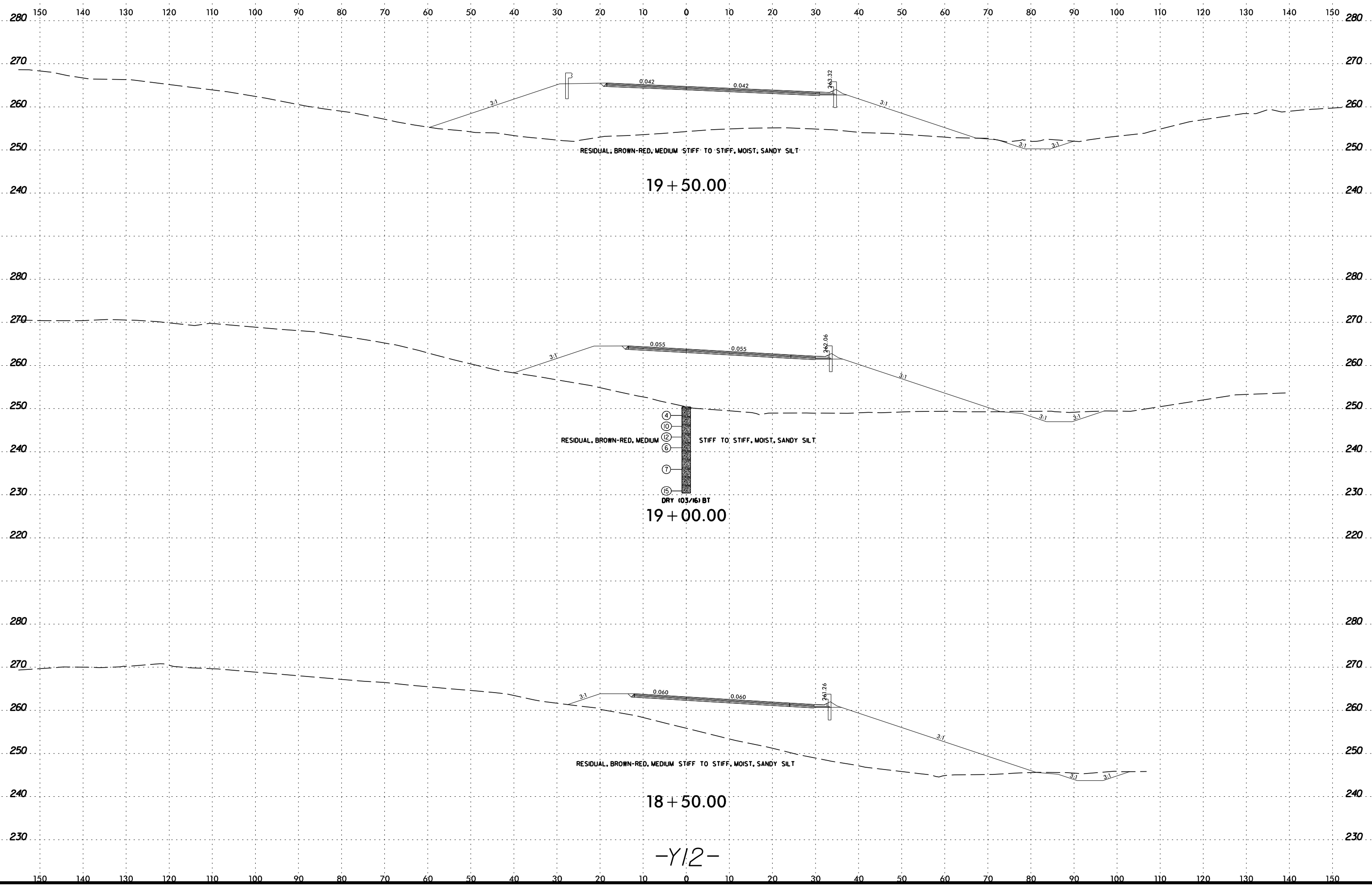


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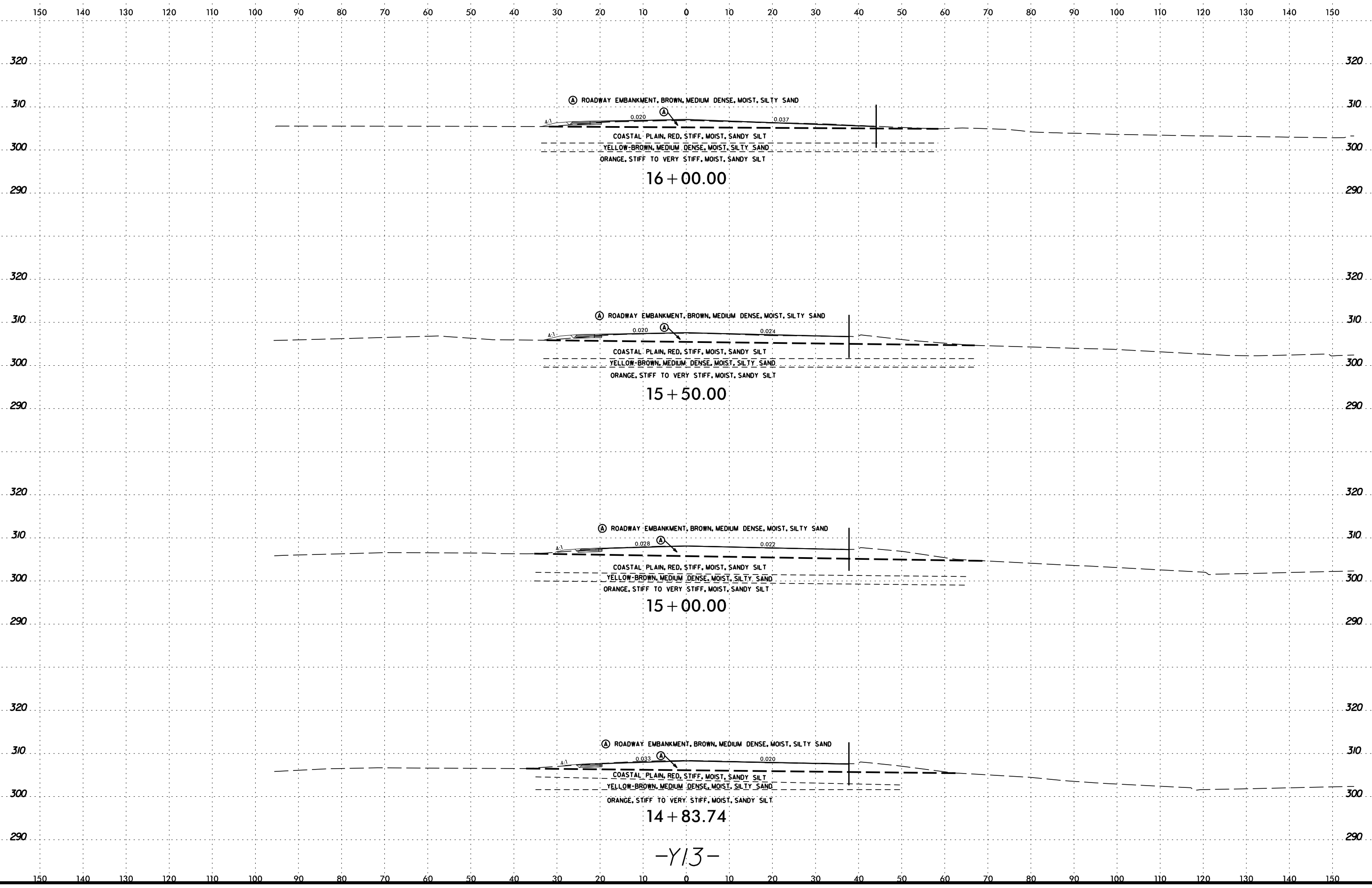
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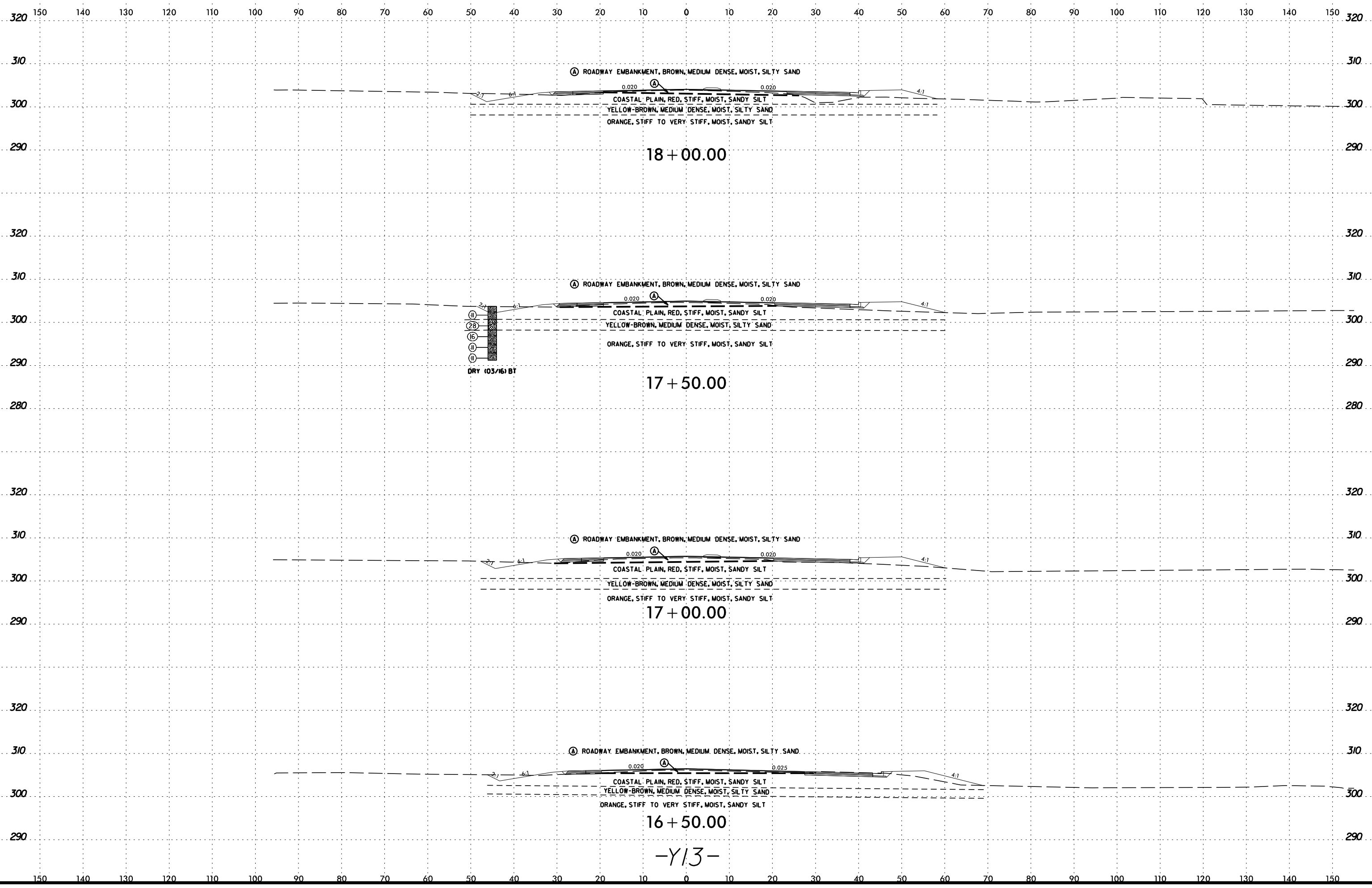
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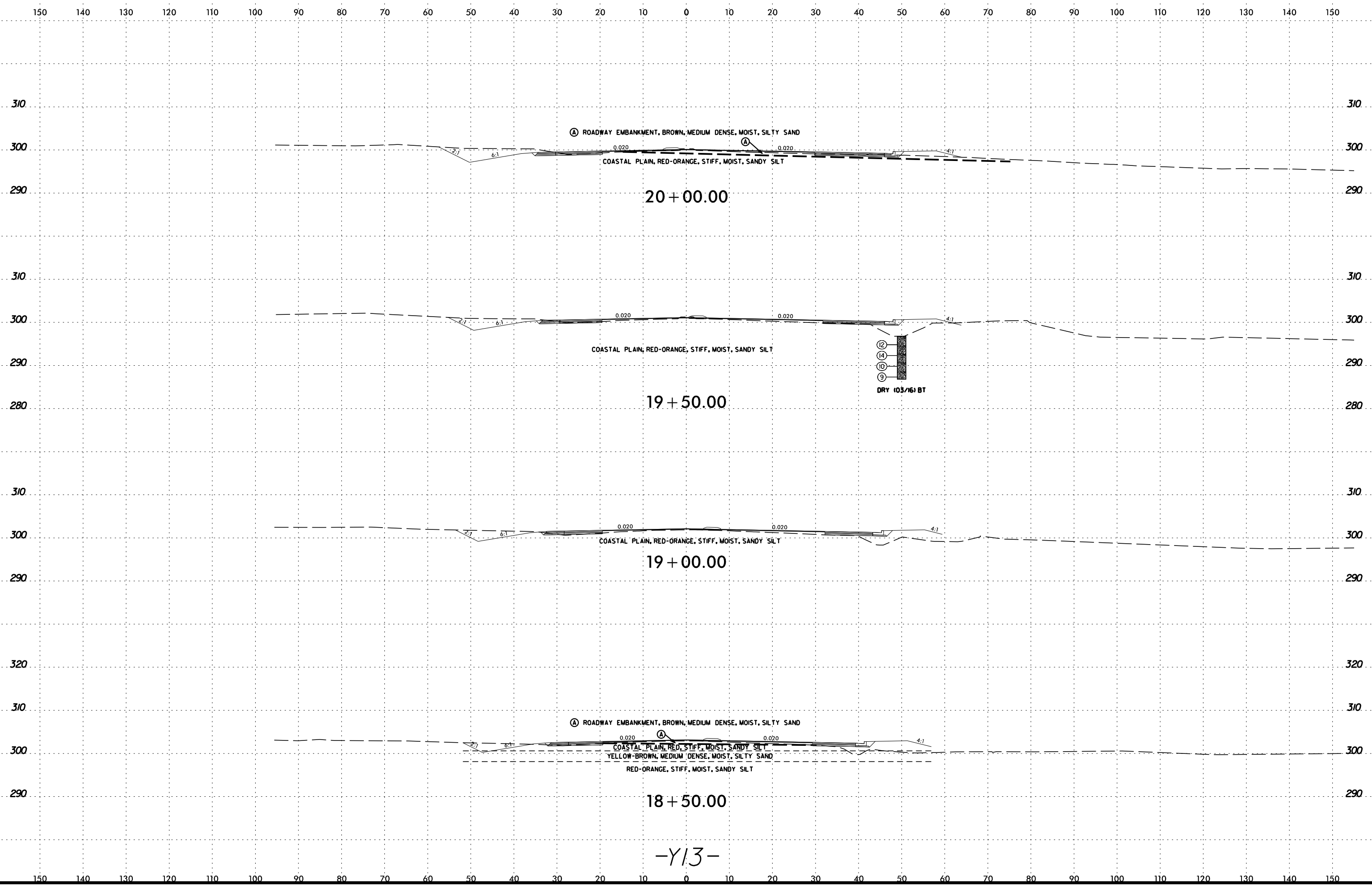
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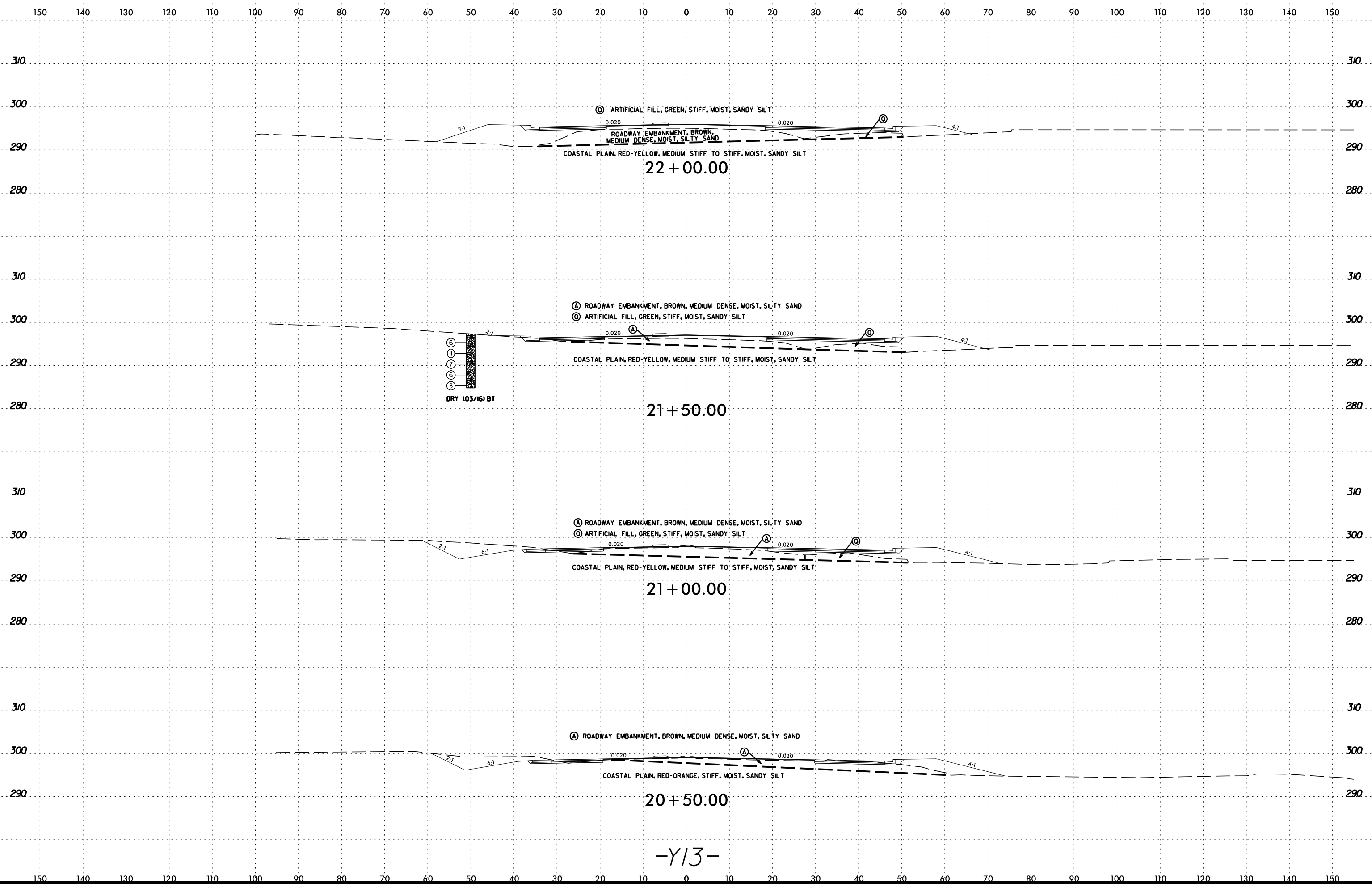
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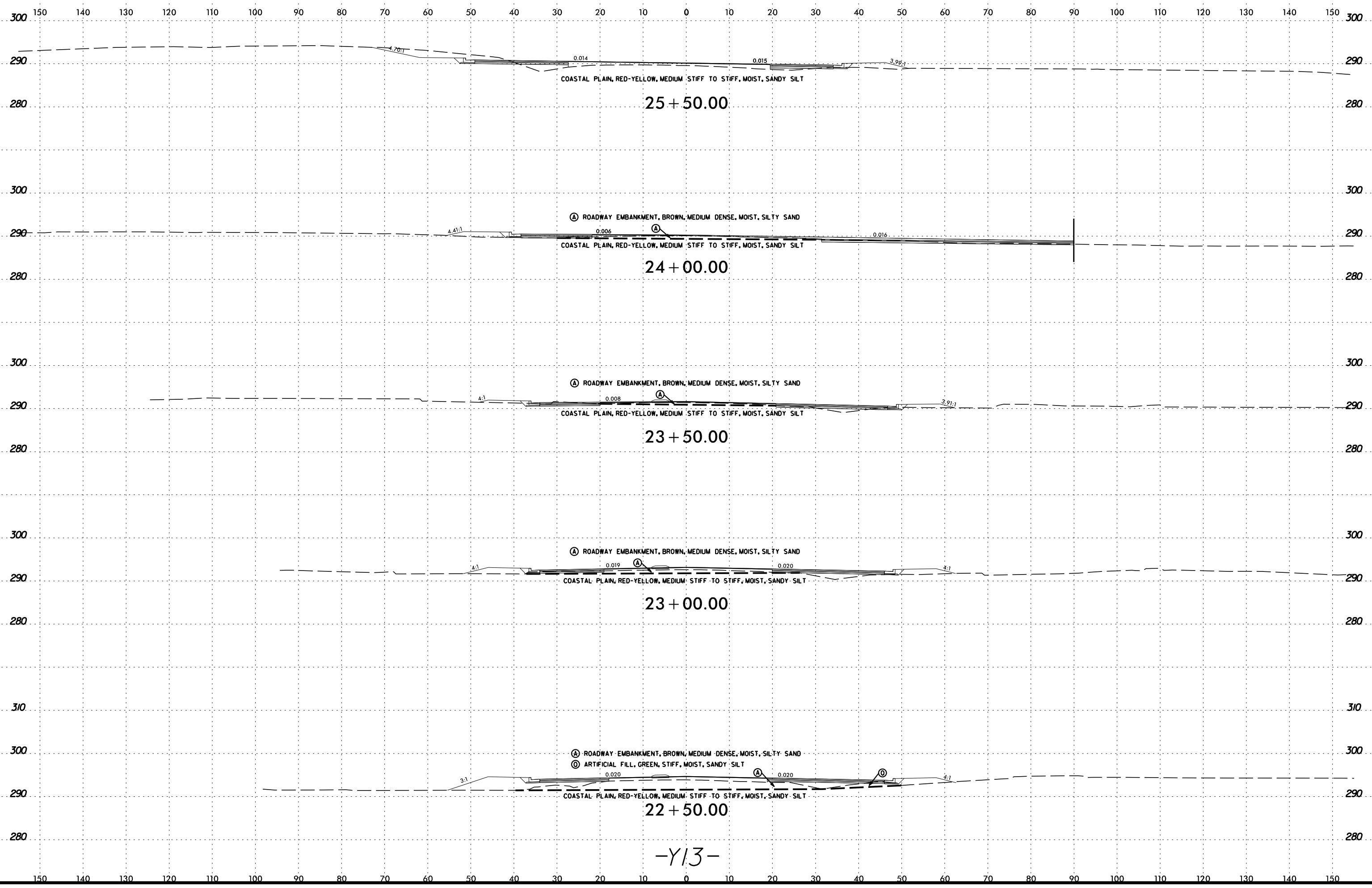
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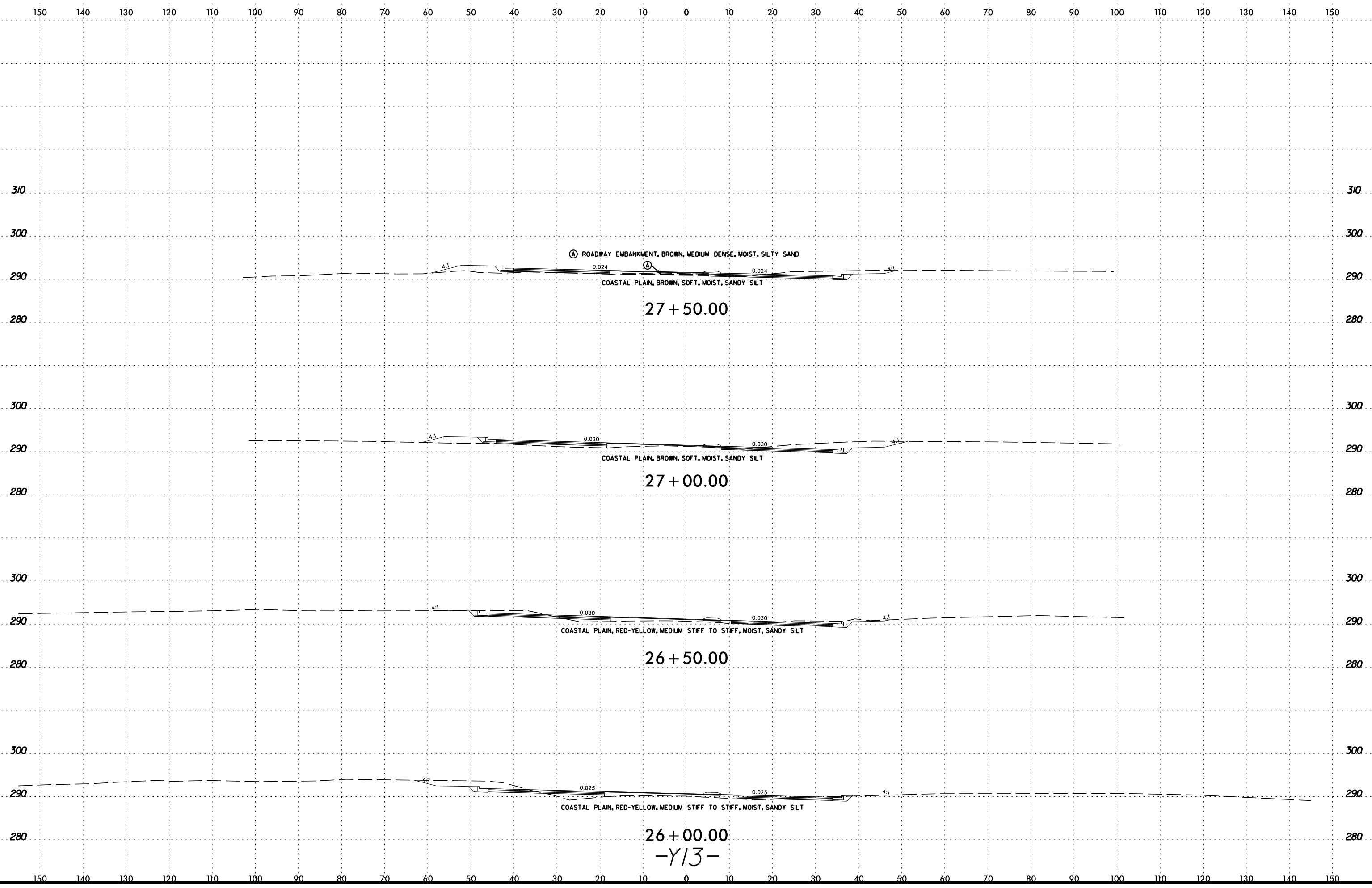
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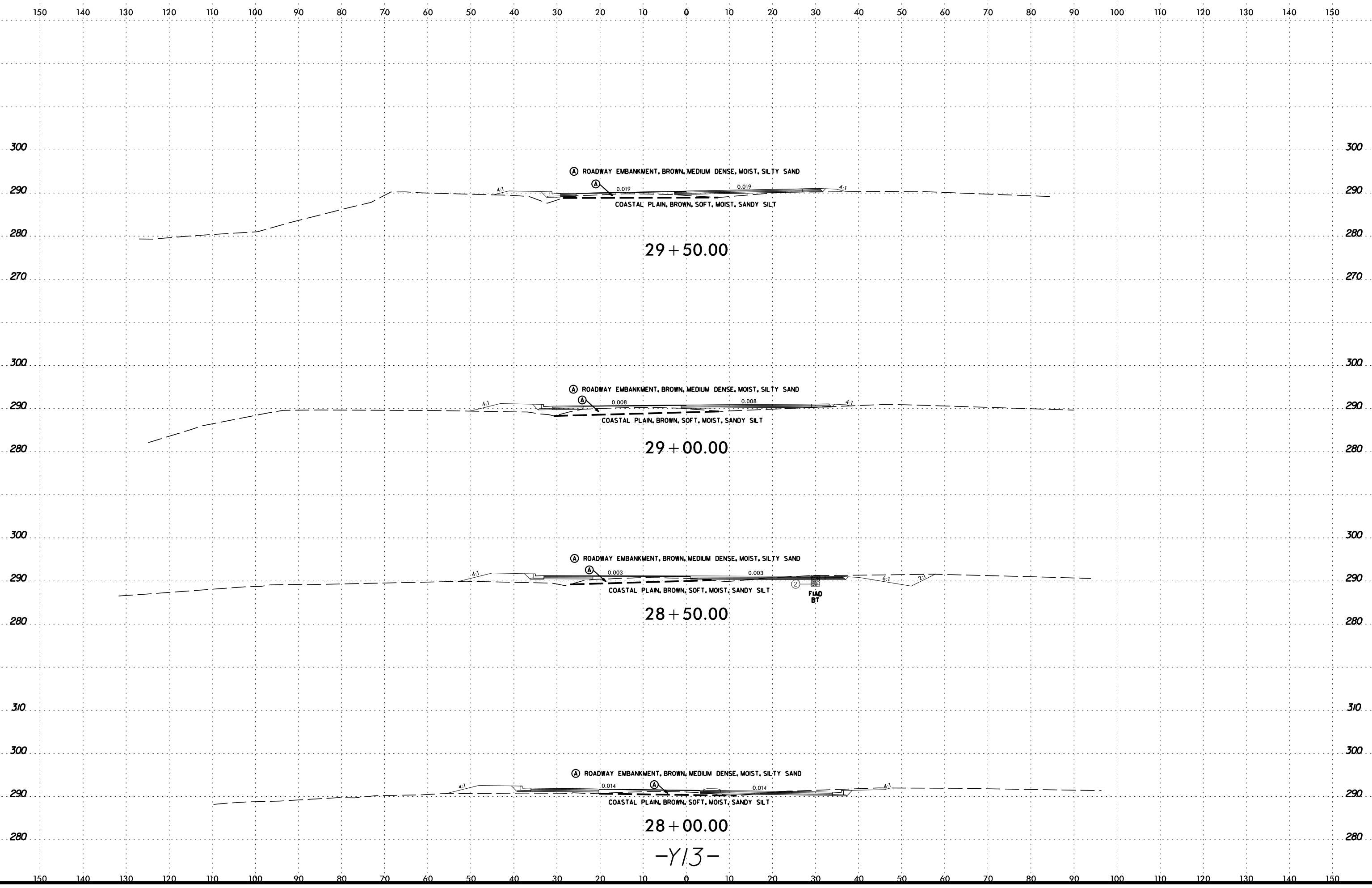
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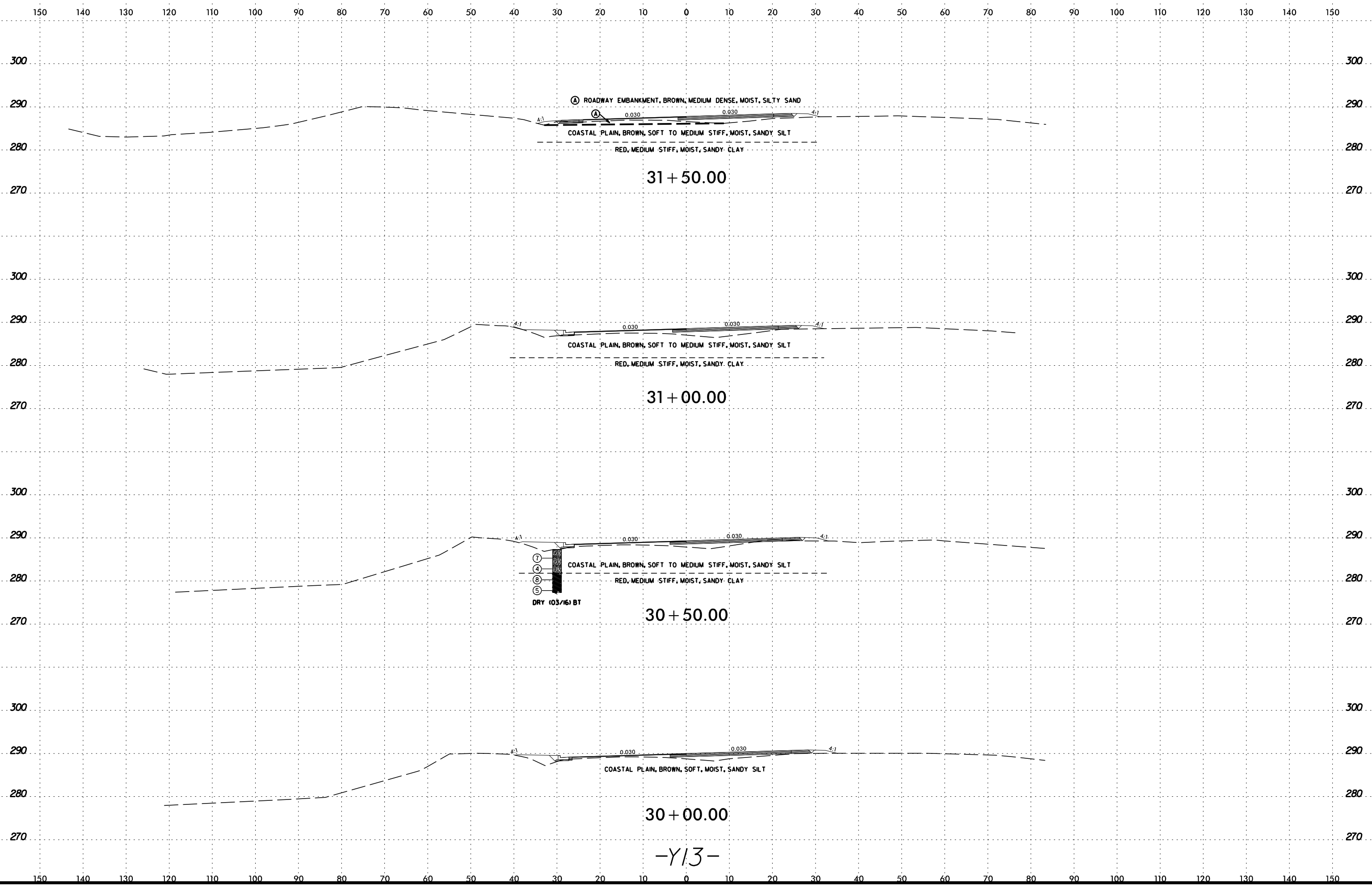
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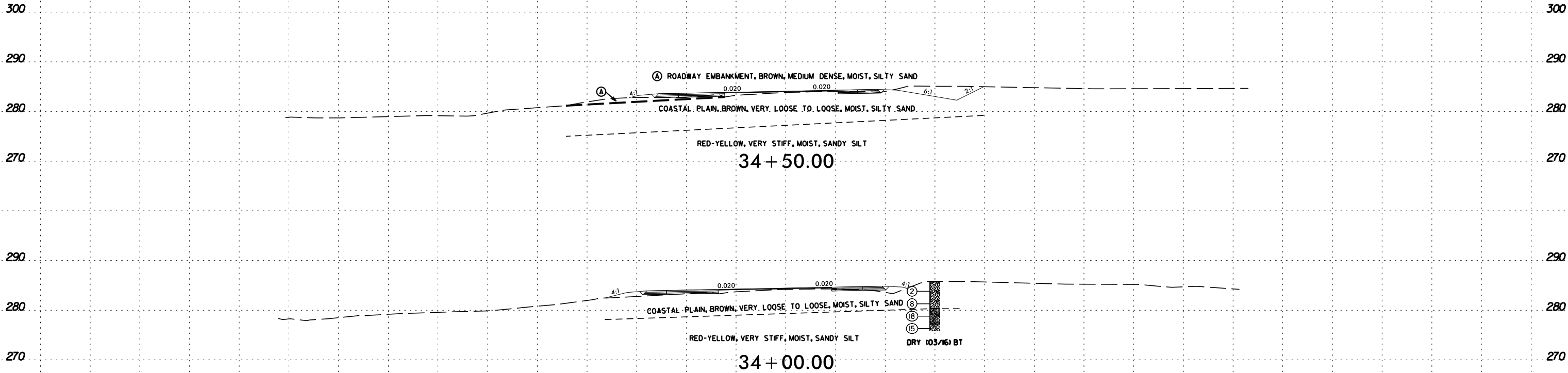
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REFERENCE: R-3825B

PROJECT: 34552

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-3825B	1	9

STRUCTURE
SUBSURFACE INVESTIGATION

CONTENTS

SHEET NO.	DESCRIPTION
1	TITLE SHEET
2	LEGEND (SOIL & ROCK)
2A	SUPPLEMENTAL LEGEND (GSI)
3	SITE PLAN
4-7	BORE LOG(S) & CORE REPORT(S)
8-9	CORE PHOTOGRAPHS)

COUNTY JOHNSTON
PROJECT DESCRIPTION NC 42 FROM EAST OF
SR 1902 (GLEN LAUREL RD) TO SR 1003
(BUFFALOE RD)
SITE DESCRIPTION PROPOSED UTILITIES EASEMENT

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

N. O. MOORE

A. N. KINTNER

D. G. PINTER

R. E. CLARKE

INVESTIGATED BY J. L. LOVE
DRAWN BY A. N. KINTNER
CHECKED BY C. A. KREIDER
SUBMITTED BY J. L. LOVE
DATE FEBRUARY 2018



DocuSigned by:
Jaime L. Love 2/15/2018

SIGNATURE DATE

**DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED**

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 208, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)						SILT-CLAY MATERIALS (> 35% PASSING #200)						ORGANIC MATERIALS		
	A-1	A-3	A-2		A-4	A-5	A-6	A-7	A-1, A-2	A-4, A-5	A-6, A-7				
GROUP CLASS.	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7		A-7-5	A-7-6						
SYMBOL															
% PASSING #10 #40 #200	50 MX 30 MX 15 MX	50 MX 25 MX 10 MX	51 MN 35 MX 35 MX	35 MX 35 MX	35 MX 35 MX	35 MX 35 MX	36 MN 36 MN	36 MN 36 MN	36 MN 36 MN	36 MN 36 MN					
MATERIAL PASSING #40 LL PI	-	6 MX	NP	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	40 MX 11 MN	40 MX 11 MN	40 MX 11 MN	40 MX 11 MN					
GROUP INDEX	0	0	0	4 MX	8 MX	12 MX	16 MX	NO MX							
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND	FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND	SILTY SOILS	CLAYEY SOILS										
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD			FAIR TO POOR			FAIR TO POOR	POOR	UNSUITABLE						
	PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30														

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE OPENING (MM)	4	10	40	60	200	270
	4.76	2.00	0.42	0.25	0.075	0.053
BOULDER (BLDR.)						
COBBLE (COB.)						
GRAVEL (GR.)						
COARSE SAND (CSE, SD.)						
FINE SAND (F SD.)						
SILT (SL.)						
CLAY (CL.)						
GRAIN SIZE	305	75	2.0	0.25	0.05	0.005
MM						
IN.	12	3				

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
LL - LIQUID LIMIT	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
PL - PLASTIC LIMIT	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
OM - OPTIMUM MOISTURE SHRINKAGE LIMIT	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
SL - SHRINKAGE LIMIT	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NON PLASTIC	0-5	VERY LOW
SLIGHTLY PLASTIC	6-15	SLIGHT
MODERATELY PLASTIC	16-25	MEDIUM
HIGHLY PLASTIC	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE LL < 31
MODERATELY COMPRESSIBLE LL = 31 - 50
HIGHLY COMPRESSIBLE LL > 50

PERCENTAGE OF MATERIAL

	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE 1 - 10%
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE 10 - 20%
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME 20 - 35%
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY 35% AND ABOVE

GROUND WATER

WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
 STATIC WATER LEVEL AFTER 24 HOURS
 PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
 SPRING OR SEEP

MISCELLANEOUS SYMBOLS

	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION		SPT DMT TEST BORING		SLOPE INDICATOR INSTALLATION
	SOIL SYMBOL		AUGER BORING		CONE PENETROMETER TEST
	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT		CORE BORING		SOUNDING ROD
	INFERRED SOIL BOUNDARY		MONITORING WELL		TEST BORING WITH CORE
	INFERRED ROCK LINE		PIEZOMETER INSTALLATION		SPT N-VALUE
	ALLUVIAL SOIL BOUNDARY				

RECOMMENDATION SYMBOLS

	UNDERCUT		UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE		UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK
	SHALLOW UNDERCUT				

ABBREVIATIONS

AR - AUGER REFUSAL	MED. - MEDIUM	VST - VANE SHEAR TEST
BT - BORING TERMINATED	MICA - MICACEOUS	WEA. - WEATHERED
CL - CLAY	MOD. - MODERATELY	W - UNIT WEIGHT
CPT - CONE PENETRATION TEST	NP - NON PLASTIC	W _d - DRY UNIT WEIGHT
CSE - COARSE	ORG. - ORGANIC	
DMT - DILATOMETER TEST	PMT - PRESSUREMETER TEST	SAMPLE ABBREVIATIONS
DPT - DYNAMIC PENETRATION TEST	SAP. - SAPROLITIC	S - BULK
e - VOID RATIO	SD. - SAND, SANDY	SS - SPLIT SPOON
F - FINE	SL. - SILT, SILTY	ST - SHELBY TUBE
FOSS. - FOSSILIFEROUS	SLI. - SLIGHTLY	RS - ROCK
FRAC. - FRACTURED, FRACTURES	TCR - TRICONE REFUSAL	RT - RECOMPACTED TRIAXIAL
FRAGS. - FRAGMENTS	w - MOISTURE CONTENT	CBR - CALIFORNIA BEARING RATIO
HI. - HIGHLY	V - VERY	

EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:	ADVANCING TOOLS:	HAMMER TYPE:
<input type="checkbox"/> CME-45C	<input type="checkbox"/> CLAY BITS	<input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL
<input checked="" type="checkbox"/> CME-55	<input type="checkbox"/> 6" CONTINUOUS FLIGHT AUGER	
<input type="checkbox"/> CME-550	<input type="checkbox"/> 8" HOLLOW AUGERS	CORE SIZE:
<input type="checkbox"/> VANE SHEAR TEST	<input type="checkbox"/> HARD FACED FINGER BITS	<input type="checkbox"/> -B <input type="checkbox"/> -H
<input type="checkbox"/> PORTABLE HOIST	<input type="checkbox"/> TUNG-CARBIDE INSERTS	<input checked="" type="checkbox"/> -N <input type="checkbox"/> -XL
<input type="checkbox"/>	<input checked="" type="checkbox"/> CASING <input checked="" type="checkbox"/> W/ ADVANCER	HAND TOOLS:
<input type="checkbox"/>	<input type="checkbox"/> TRICONE _____ *STEEL TEETH	<input type="checkbox"/> POST HOLE DIGGER
<input type="checkbox"/>	<input type="checkbox"/> TRICONE _____ *TUNG-CARB.	<input type="checkbox"/> HAND AUGER
<input type="checkbox"/>	<input checked="" type="checkbox"/> CORE BIT	<input type="checkbox"/> SOUNDING ROD
<input type="checkbox"/>		<input type="checkbox"/> VANE SHEAR TEST

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.

CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.

NON-CRYSTALLINE ROCK (NCR) FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.

COASTAL PLAIN SEDIMENTARY ROCK (CP) COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.

VERY SLIGHT (V SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.

SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.

MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.

MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL.

SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF.

VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF.

COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.

HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.

MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.

MEDIUM HARD CAN BE GROUDED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.

SOFT CAN BE GROUDED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.

VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.

MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.

INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.

EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL.

FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.

FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.

RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES
FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

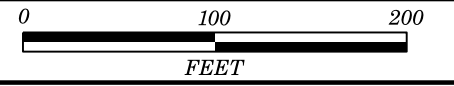
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)		SURFACE CONDITIONS					GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos, P and Hoek E., 2000)		SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)				
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.		VERY GOOD Very rough, fresh unweathered surfaces	GOOD Rough, slightly weathered, iron stained surfaces	FAIR Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments	VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings	From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.		VERY GOOD - Very Rough, fresh unweathered surfaces	GOOD - Rough, slightly weathered surfaces	FAIR - Smooth, moderately weathered and altered surfaces	POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments	VERY POOR - Very smooth, slickensided or highly weathered surfaces with soft clay coatings or fillings
STRUCTURE		DECREASING SURFACE QUALITY →					COMPOSITION AND STRUCTURE						
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities		90			N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.	70					
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets		80					B. Sandstone with thin inter-layers of siltstone	60					
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets			70				C. Sandstone and siltstone in similar amounts		50				
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity			60				D. Siltstone or silty shale with sandstone layers			40			
DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces				50			E. Weak siltstone or clayey shale with sandstone layers				30		
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes					40		F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure					20	
					30		G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers						10
					20		H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.						
					10								
		N/A	N/A										

→ Means deformation after tectonic disturbance

SITE PLAN



NAD 83/2011

60+00

65+00

70+00

BY3-140

-L- POT Sta. 68+11.45 =
-Y6- POT Sta. 17+91.45

BM#4
N: 691,147
E: 2,176,708
ELEV=140.97'

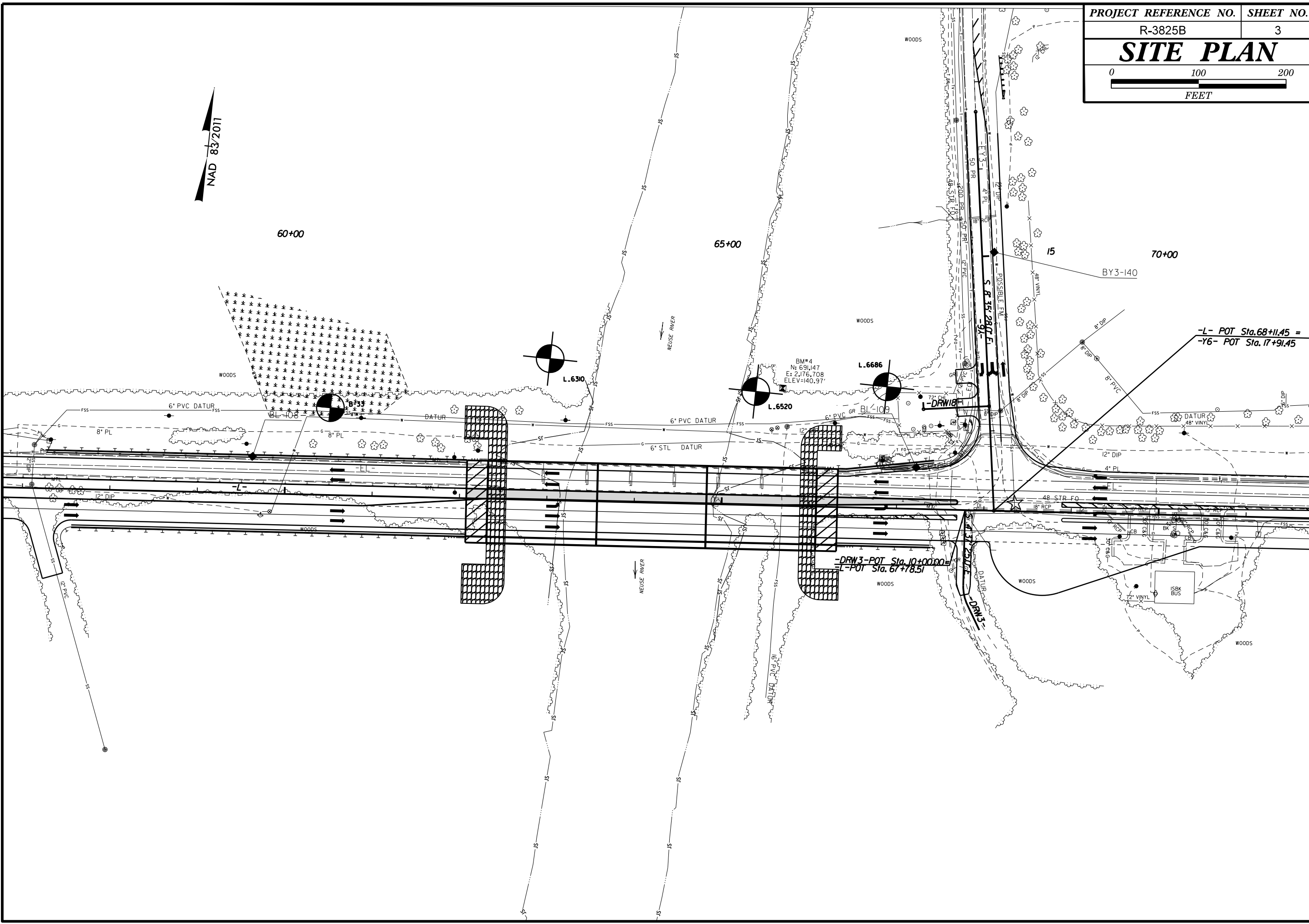
L.6310

L.6686

L.6520

BL-109

-DRW3-POT Sta. 10+00.00 =
-L-POT Sta. 67+78.51



GEOTECHNICAL BORING REPORT

BORE LOG

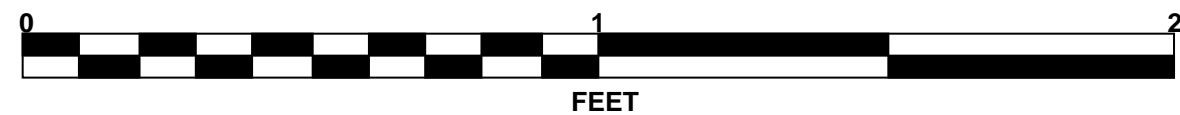
WBS 34552.1.FR3		TIP R-3825B		COUNTY JOHNSTON		GEOLOGIST MARCELO MEDEIROS										
SITE DESCRIPTION NC-42 From East of SR1902 to SR1003							GROUND WTR (ft)									
BORING NO. B-33		STATION 60+50		OFFSET 100 ft LT		ALIGNMENT -L-										
COLLAR ELEV. 139.7 ft		TOTAL DEPTH 14.3 ft		NORTHING 691,070		EASTING 2,176,196										
DRILL RIG/HAMMER EFF./DATE BRI9910 BK-51 75% 06/03/2015				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic										
DRILLER Contract Driller		START DATE 03/07/16		COMP. DATE 03/07/16		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100			ELEV. (ft)	DEPTH (ft)		
140														139.7	0.0	GROUND SURFACE
	138.7	1.0	2	2	4								M	136.7	3.0	ALLUVIAL BROWN, CLAYEY SILT WITH TRACE MICA
135	136.2	3.5	2	4	5								M			TAN-BROWN, SANDY CLAY
	133.7	6.0	2	2	3								M			
	131.2	8.5	2	2	3								M			
130	129.7	10.0	6	100/0.3										129.7	10.0	WEATHERED ROCK (SCHIST)
														125.4	14.3	
Boring Terminated at Elevation 125.4 ft IN WEATHERED ROCK (SCHIST) BORING DRILLED DURING ROADWAY INVESTIGATION BY GEOSYNTEC.																

NCDOT BORE DOUBLE R3825_GEO_UTILITIES.GPJ NC_DOT.GDT 2/15/18

CORE PHOTOGRAPHS

L_6310

BOXES 1 & 2: 18.8 - 30.5 FEET



L_6520

BOXES 1 & 2: 13.8 - 30.4 FEET



CORE PHOTOGRAPHS

L_6686
BOXES 1 & 2: 18.6 - 40.1 FEET

