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FILL FACE @ END BENT 1 CONSTRUCT T TON W_P. #1 STA. 20+02.43-L-NOTES: DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINE. INDICATES PILE BATTER 3:12 IN DIRECTION SHOWN. END BENT 1

FOUNDATION NOTES

1) FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
2) PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 100 TONS PER PILE.
3) DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 167 TONS PER PILE.
4)FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
5) DRILLED PIERS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 415 TONS/PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 30 TSF.
6) INSTALL DRILLED PIERS AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 746 FT.(#1) 748 FT.(#2), 751 FT.(#3), 754 FT.(#4), 755 FT.(#5), SATISFY THE REQUIRED TIP RESISTANCE AND HAVE A PENETRATION OF AT LEAST 7 FT.INTO ROCK AS DEFINED BY ARTICLE 411-1 OF THE STANDARD SPECIFICATIONS
7) PERMANENT STEEL CASINGS WILL BE REQUIRED FOR DRILLED PIERS AT BENT NO.1. DO NOT EXTEND PERMANENT CASINGS BELOW ELEVATION 765 FT.WITHOUT PRIOR APPROVAL FROM THE ENGINEER.
8) THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 IS 758 FT.(LT)AND 762 FT.(RT). THE SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.
9) DRILLED PIERS AT BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 415 TONS/PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 30 TSF.
10) INSTALL DRILLED PIERS AT BENT NO.2 TO A TIP ELEVATION NO HIGHER THAN 752 FT.(#1) 754 FT.(#2),756 FT.(#3),760 FT.(#4),763 FT.(#5),SATISFY THE REQUIRED TIP RESISTANCE AND HAVE A PENETRATION OF AT LEAST 7 FT.INTO ROCK AS DEFINED BY ARTICLE 411-1 OF THE STANDARD SPECIFICATIONS
DRAWN BY : RTJ DATE : 2/17 CHECKED BY : RDE DATE : 2/17 DESIGN ENGINEER OF RECORD : RDE DATE : 4/17

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RAWN BY :	NMW	DATE :	2/1
HECKED BY :	RDE	DATE :	2/1
ESIGN ENGINEER OF	RECORD : RDF	DATE :	4/1

[LL	L OF MATERIAL												
DIA. LED RS DIL	3'-6"DIA. DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 3'-6"DIA. DRILLED PIERS	CSL TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE						
FT.	LIN.FT.	LIN.FT.	EA.	LUMP SUM	SQ.FT.	SQ.FT.	CU.YDS.						
					13,068	14,524							
							90.6						
	43	55					95.9						
5	45	50					95.9						
							89.4						
5	88	105	1	LUMP SUM	13,068	14,524	371.8						
× 53 PILES	TWO BAR METAL RAIL	1'-2" × 3'-3" CONCRETE PARAPET	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	EXPANSION JOINT SEALS							
IN.FT.	LIN.FT.	LIN.FT.	TONS	SQ.YDS.	LUMP SUM	LUMP SUM							
	406.8	441.4					-						
510			189	210			-						
			203	325			4						
295			90	100			4						
			50	100			1						
805	406.8	441.4	887	985	LUMP SUM	LUMP SUM]						

	PROJECT NO. B-4982 IREDELL COUNTY							
	STATION: 21+10.00-L- SHEET 3 OF 4 4							
TH CAROL ROFESS IONAL NO THE	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH							
SEAL 22992 Docusigned by:	GENERAL DRAWING							
4/17/2018	FOR BRIDGE OVER THIRD CREEK ON US 21/NC 115 BETWEEN SR 2347 & SR 1491							
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	REVISIONS SHEET NO.							
TGS ENGINEERS	NO. BY: DATE: NO. BY: DATE: S-3							
SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	1 3 Intra Sheets 2 4 69							

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP USED IN THE CAUSEWAY MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS AT STATION 21+10.00-L-.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 21+10.00-L-."

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 40 FT EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS.FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

DRAWN BY :	NMW	DATE :	2/17
CHECKED BY :	RDE	DATE :	2/17
DESIGN ENGINEER	OF RECORD : RDE	DATE :	4/17

AFTER SERVING AS A TEMPORARY STRUCTURE THE EXISTING STRUCTURE CONSISTING OF THREE SPAN 2 @ 41'-9" & 1 @ 42'-4" WITH REINFORCED CONCRETE DECK SLAB SUPPORTED BY STEEL AND CONCRETE GIRDERS ON CONCRETE BENTS AND CONCRETE VERTICAL ABUTMENTS AND LOCATED 22'-O"± UPSTREAM SHALL BE REMOVED. BRIDGE IS PRESENTLY POSTED FOR LOAD LIMITS OF 30 TO 32 TONS. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THE LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE THE LIFE OF THE PROJECT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

THE SCOUR CRITICAL ELEVATION FOR BENT(S) NO.1 IS ELEVATION 758 FT.(LT.) AND 762 FT. (RT.). SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

THE SCOUR CRITICAL ELEVATION FOR BENT(S) NO. 2 IS ELEVATION 762 FT.(LT.) AND 770 FT. (RT.). SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

	PROJEC	T NO.		B-4982	2		
		CO	UNTY				
	STATIO	STATION: 21+10.00-					
	SHEET 4 OF	4					
OFFESS ION A	DEPAR	stat RTMENT	e of north car OF TRAI raleigh	OLINA NSPORTA	TION		
SEAL 22992 Docussigned by:	GE	NER	AL DR	AWIN	IG		
Kay D. Elliott MONEEP D. ELL 4/17/2018	FOR BR	IDGE JS 21, SR 23	OVER /NC 115 47 & S	THIRD 5 BETW 5R 1491	CREEK EEN		
DOCUMENT NOT CONSIDERED FINAL INLESS ALL SIGNATURES COMPLETED		DEVIT	TONS				
TGS ENGINEERS 804-C N. LAFAYETTE ST	NO. BY:	DATE:	NO. BY:	DATE:	S-4		
SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	1		3 4		total sheets 69		
= CORP. LICENSE NO.: C-0275	2		[쌱]		63		

		LOAD AN	ID RE	SIST	ANCE	FAC	TOR	RAT	ING	(LRF	R) SL	JMMA	RY F	OR F	PRES	TRES	SED	CON	CRET	E GI	RDEF	RS	
								STRENGTH I LIMIT STATE					SE	RVICE	III	LIMI	T STA	,ΤE					
										MOMENT					SHEAR						MOMENT		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)
		HL-93 (INVENTORY)	N/A	$\langle 1 \rangle$	1.40		1.75	0.69	1.40	А	ER	61.6	1.06	2.15	А	I	40.8	0.80	0.67	1.79	А	I	27.0
DESIGN		HL-93 (OPERATING)	NZA		1.81		1.35	0.69	1.81	А	ER	61.6	1.06	2.71	А	I	40.8	NZA					
RATING		HS-20 (INVENTORY)	36.000	2	2.36	84.9	1.75	0.69	2.56	А	ER	61.6	1.06	2.44	А	I	40.8	0.80	0.67	2.36	А	I	27.0
		HS-20 (OPERATING)	36.000		3.09	111.2	1.35	0.69	3.32	Α	ER	61.6	1.06	3.09	А	I	40.8	N/A					
		SNSH	13.500		5.28	71.2	1.40	0.69	8.09	А	ER	61.6	1.06	6.02	А	I	40.8	0.80	0.67	5.28	Α	I	27.0
		SNGARBS2	20.000		3.91	78.2	1.40	0.69	5.53	А	ER	61.6	1.06	4.59	А	I	40.8	0.80	0.67	3.91	Α	I	27.0
	ICLE	SNAGRIS2	22.000		3.69	81.1	1.40	0.69	5.04	А	ER	61.6	1.06	4.35	А	I	40.8	0.80	0.67	3.69	Α	I	27.0
	VEH.	SNCOTTS3	27.250		2.59	70.5	1.40	0.69	3.92	А	ER	61.6	1.06	3.24	А	I	40.8	0.80	0.67	2.59	Α	I	27.0
	S (S	SNAGGRS4	34.925		2.17	75.7	1.40	0.69	3.13	А	ER	61.6	1.06	2.82	А	I	40.8	0.80	0.67	2.17	Α	I	27.0
	ING	SNS5A	35.550		2.13	75.7	1.40	0.69	3.09	А	ER	61.6	1.06	2.91	А	I	40.8	0.80	0.67	2.13	Α	I	27.0
		SNS6A	39.950		1.98	79.1	1.40	0.69	2.79	А	ER	61.6	1.06	2.72	А	I	40.8	0.80	0.67	1.98	А	I	27.0
		SNS7B	42.000		1.88	78.9	1.40	0.69	2.66	А	ER	61.6	1.06	2.73	А	I	40.8	0.80	0.67	1.88	А	I	27.0
RATING	ER	TNAGRIT3	33.000		2.41	79.5	1.40	0.69	3.38	А	ER	61.6	1.06	3.20	А	I	40.8	0.80	0.67	2.41	А	I	27.0
	RAII	TNT4A	33.075		2.43	80.3	1.40	0.69	3.39	А	ER	61.6	1.06	3.06	А	I	40.8	0.80	0.67	2.43	А	I	33.9
	1 - IV	TNT6A	41.600		1.97	81.9	1.40	0.69	2.76	А	ER	61.6	1.06	2.95	А	I	40.8	0.80	0.67	1.97	Α	I	27.0
	SEA ST)	TNT7A	42.000		1.99	83.5	1.40	0.69	2.74	Α	ER	61.6	1.06	2.80	Α	I	40.8	0.80	0.67	1.99	Α	I	27.0
	TOR (TT)	TNT7B	42.000		2.04	85.6	1.40	0.69	2.78	Α	ER	61.6	1.06	2.62	Α	I	40.8	0.80	0.67	2.04	Α	I	27.0
	TRAC	TNAGRIT4	43.000		1.96	84.2	1.40	0.69	2.64	Α	ER	61.6	1.06	2.57	А	I	40.8	0.80	0.67	1.96	Α	I	27.0
	CK	TNAGT5A	45.000		1.85	83.2	1.40	0.69	2.54	А	ER	61.6	1.06	2.63	А	I	40.8	0.80	0.67	1.85	Α	I	27.0
	TRL	TNAGT5B	45.000	3	1.82	81.9	1.40	0.69	2.49	Α	ER	61.6	1.06	2.47	А	I	40.8	0.80	0.67	1.82	A	I	27.0

LRFR SUMMARY

(SPAN LENGTHS SHOWN @ 🖟 BEARING TO 🖟 BEARING)

ASSEMBLED BY : NMW	DATE :	2/17
CHECKED BY : RDE	DATE :	2/17
DESIGN ENGINEER OF RECORD	RDE DATE :	4/17
DRAWN BY : MAA 1/08 CHECKED BY : GM/DI 2/08	REV. II/12/08RR REV. 10/1/11	MAA/GM MAA/GM

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LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	γ_{DW}
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- 1. GIRDER DESIGNED WITH CONSPAN USING MULTI-SPAN CONTINUOUS OPTION.
- 2. DESIGN PRODUCES POSITIVE AND NEGATIVE MOMENTS FOR LIVE LOAD AND COMPOSITE DEAD LOADS.
- 3. STRENGTH I MOMENT CONTROLLED BY GIRDER #8 IN SPAN A FOR FINAL TYPICAL SECTION WITHOUT SIDEWALK.
- 4. STRENGTH I SHEAR CONTROLLED BY GIRDER #7 IN SPAN A FOR FINAL TYPICAL SECTION WITH SIDEWALK.
- 5. SERVICE III TENSION CONTROLLED BR GIRDER #7 IN SPAN A FOR FINAL TYPICAL SECTION WITH SIDEWALK.

(#) CONTROLLING LOAD RATING 1 DESIGN LOAD RATING (HL-93) 2 DESIGN LOAD RATING (HS-20) 3 LEGAL LOAD RATING ** *** *** SEE CHART FOR VEHICLE TYPE GIRDER LOCATION I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO	. <u> </u>	-4982
IREC	DELL	COUNTY
STATION:	21+10	.00-L-

SEAL 22992 Docusigned by: With NEBREHAME 4/17/2018			S RTMENT S FR PRE NCRE N-INTE	TA TA SUN ST ST ST	NORTH CARE TRAI NDAF MMA RES EG TATE	NSPORTA RY F SSED IRDEI TRAFI	TION OR RS TIC)
DOCUMENT NOT CONSIDERED FINAL JNLESS ALL SIGNATURES COMPLETED			REVI	SION	S		SHEET NO.
TGS ENGINEERS	NO.	BY:	DATE:	NO.	BY:	DATE:	S-5
SHELBY, NC 28150 PH (704) 476–0003	1			3			TOTAL SHEETS
CORP. LIĊENŚE NO.: C-0275	2			4			69
				STI	D.NO	LRFR1	

CONTRACTOR SHALL DEVISE A PLAN FOR ATTACHING THE TEMPORARY BARRIER RAIL TO THE EXISTING BRIDGE AND PROPOSED BRIDGE DECK. SEE TRAFFIC CONTROL PLANS FOR LOCATION AND PAY LIMITS OF THE ANCHORED PORTABLE CONCRETE BARRIER.

	PROJEC	T NO.		B-4982	2	
		IRED	ELL	CO	UNTY	
	STATIO	N:	21+1	21+10.00-L		
	SHEET 1 OF	2				
SEAL 22992 Docusigned by: 4/17/2018	DEPAR C BRID ON L	STMENT CONS SE OGE OV JS 21/ SR 23/	e of north car OF TRAI RALEIGH TRUC QUEN FOR /ER TH /NC 115 47 & S	NSPORTA TION CE IRD CF 5 BETW R 1491	TION REEK EEN	
DOCUMENT NOT CONSIDERED FINAL JNLESS ALL SIGNATURES COMPLETED	<u> </u>	REVIS	IONS		SHEET NO.	
804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	NO. BY: 1 2	DATE:	NO. BY: 3 4	DATE:	5-6 Total Sheets 69	

	PROJECT	NO		B-4982	<u> </u>		
	STATION	2	21+1(CO D.00-L			
SEAL 22992 Docusigned by: Hay D. HUGH 4/17/2018	DEPART C(BRIDG ON US SF	TION REEK EEN					
DOCUMENT NOT CONSIDERED FINAL JNLESS ALL SIGNATURES COMPLETED		REVISIO)NS		SHEET NO.		
TGS ENGINEERS 804–C N. LAFAYETTE ST	NO. BY:	DATE: NO	BY:	DATE:	S-7		
ORP. LICENSE NO.: C-0275	12	<u>. 3</u>	5		SHEETS 69		
	- · ·						

	PROJECT NO. <u>B-4982</u> IREDELL COUNTY STATION: <u>21+10.00-L-</u> SHEET 2 OF 3
SEAL 22992 Docusigned by: HIGH NEER ELLIGHT 4/17/2018	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE TYPICAL SECTIONS
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	REVISIONS SHEET NO.
TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	NO. BY: DATE: NO. BY: DATE: S-9 1 3 3 5 5 5 69 <td< th=""></td<>

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DRAWN BY :	NMW	DATE :	2/17
CHECKED BY :	RDE	DATE :	2/17
DESIGN ENGINEER C	F RECORD : RDE	DATE :	4/17

	0 . 6″Ø	L.R	.GRAD	E 270) ST	RANDS
	ARE	4	ULTI			PPLIED
	(SQUARE I	NCHES)		STRAND)	(LBS.	PER STRAND
	0.21	7	58,6	00	4	3,950
	REINFO	RCING	STEEI	L FOR	ONE	GIRDER
	BAR	NUMBER	SIZE	TYPE	LENGT	H WEIGHT
	S1 S2	73 12	#5 #6	1	8′-6 8′-6	<u> </u>
	S3	4	#4	3	8′-8	" 23
	S4	76	#4 #4	2	2'-9	" <u>140</u>
	* S6	4	#4 #5	STR	3'-6	· 15
TERIOR GDR.	S7	2	# 5	3	7′-2	.″ 15
TERIOR GDR.	57 58	4	#5 #⊿	3 STP	7'-2 7'-0	· 30
TERIOR GDR.	S9	5	*4 #4	STR	15'-1	1″ 53
	S10	10	# 5	STR	4'-0	9″ 42
	* NOT	E: S6 SHI	BARS SHA PMENT. H	ALL BE E IEAT BEN	BENT I NDING	BEFORE SHALL
		NOT	BE ALL(OWED.		
		A.I.I. = ··=	BAR T	YPES	T 74 -	
	· · · · · ·	ALL BAR	DIMENSIO	NS ARE OU	II-TO-0	
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	QUA	NTITI	ES FO	R ONE	GIF	RDER
		F	REINFORCIN STEEL	NG 8,000 CONCE	PSI RETE	0.6"ØL.R. STRANDS
			LB.	C.Y	(.	No.
	EXTERIOR G	IRDER	1,064 1,109	10		28 28
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& 7	GIRDER	S RE		- SPA	NA	AND C
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	DEPA	RTME	NT OF NOR	TRANSP	ORTA	TION
			RALE	IGH		
		AASI	HTO T	YPE	III	
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ED FINAL MDI ETED						
RS	NO. BY:	RI DATE:	NO. E	8Y: C)ATE:	STEEL NU.
3150 0003	1		3			TOTAL SHEETS
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	0.6″ 9	ØL.R	GRA	DE 2	270 S	TRANDS					
				MATE NGTH	E A H PF	APPLIED PRESTRESS (LBS. PER STRAND)					
	0 . 2	17	58,6	500		43,950					
	REINF	ORCING	STEE	L F	DR ON	E GIRDER					
	BAR	NUMBER	SIZE	TYP	E LEN	GTH WEIGHT					
	S1	73	# 5	1	8'-	6″ 647					
	S2	12	#6	1	8'-	6″ 153					
	S3	4	#4	3	8'-	8″ 23					
	S4	76	#4	2	2'-	9″ 140					
	* 56	8	#5			8″ 31					
DR.	<u> </u>	2	#5	3	<u> </u>	2″ 15					
DR.	S7	4	#5	3	7'-	2″ 30					
GDR.	S8	5	#4	STE	-׳־ א	0″ 23					
DR.	S9	5	#4	STE	R 15'-	·11″ 53					
	S10		<u>#5</u>	STE	<u> 4'-</u>	0″ 42					
	* NC) E: 56 SHI	PMENT.	ALL E HEAT	BENDIN	BEFORE G SHALL					
		NOT	BE ALL	OWED.							
			BAR	TYP	ES						
		ALL BAR	DIMENSI	ONS ARI	E OUT-TO-	•OUT					
	01/2"	6 ¹ /2"	01/2	5"93/6 <i>"</i>							
	578" 1]	8″ <u>53</u>	1'-3"					
	2'-7"		,, _,, _,, _	8/ C	<u>'-6" S5</u> 4" <u>S7</u>	A A					
	▼		<u>S1</u>		ی ۲. ۲.	4,-0 [*]					
	<u> 4"</u>		<u>S2</u>	I		S3 & S5					
	QU	ANTITI	ES FC	R C	NE GI	RDER					
			REINFORCI STEEL	NG 8. C	000 PSI ONCRETE	0.6″ØL.R. STRANDS					
			LB.		C.Y.	No.					
	EXTERIOR	GIRDER	1,074		10	28					
	INTERIOR	GIRDER	1,119		10	28					
			– –			–					
	GIF	RDERS	REQU	RED	<u>- SP</u>	AN B					
	NUM	BER	LEN	GTH	TO	TAL LENGTH					
	8	5	69'	-2″		553'-4"					
l					 B-498	32					
I		TRI									
		T 1 / 1			C	UUNIY					
(STAT		2	1+1(-00.0	L-					
	SHEET 2										
			STATE OF NO								
	DEF	PARTME	NT OF RAL		SPORT	ATION					
		AAS	HTO T	YPE		I					
	PRES	IKESS		UNC	≺E Ē	GIRDER					
	CO	NTINL	JOUS F	OR	LIVE	LOAD					
			SP	AN E	}						
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ע		R	EVISIONS		0475	SHEET NO.					
	^{NU} . ^{BY} :		: NO. হ	BY:	DATE:	TOTAL					
75	2		4			SHEETS 69					

STD. NO. PCG5

→ ¼″ BEVEL EDGE

SECTION "F"

(SEE NOTES)

ALL REINFORCING STEEL SHALL BE GRADE 60.

APPLY EPOXY PROTECTIVE COATING TO END OF GIRDER SURFACES INDICATED IN ELEVATION VIEW.

EMBEDDED PLATE ``B-1'' SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE ``B'' REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

ALL PRESTRESSED STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6,000 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, SHALL BE RAKED TO A DEPTH OF $\frac{1}{4}$ "EXCEPT IN THE AREA BETWEEN THE STIRRUP AND THE EDGE OF THE GIRDER.

OF 4500 LBS.

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD

	PROJEC	T NO.		B-4982	2
		IRED	ELL	CO	UNTY
	STATIO				
	SHEET 3 C)F 3			
SEAL 22992 DocuSigned by: 40 GAUEUFF490330 4/17/2018	DEPA PRESTI CON	STAT RTMENT S ^T AASHT RESSED TINUOL	e of north card OF TRAN RALEIGH TANDAR O TYPE O CONCI JS FOR	DINA NSPORTA D III RETE G LIVE L	TION IRDER OAD
DOCUMENT NOT CONSIDERED FINAL JNLESS ALL SIGNATURES COMPLETED		REVIS	SIONS		SHEET NO.
TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	№. вт: 1 2	DATE:	NO. BY: 3 4	DATE:	S-22 Total Sheets 69
			STD. N	O.PCG5)

FOR BOLT CONNECTION, -SEE TYPICAL BOLT WITH DTI ASSEMBLY DETAIL

STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE CHANNEL MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL $\frac{1}{4}$ TURN.

THE PLATES, BENT PLATES, CHANNELS, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE, WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

TABLE

GIRDER TYPE	CHANNEL SIZE	DIM ``A''	DIM ``B''	DIM ``L''		
III	MC 18 × 42.7	1′-5″	1'-2"	1'-6"		

B-4982 PROJECT NO.

COUNTY

21+10.00-L-STATION:

IREDELL

SEAL 22992 Docusigned by: Ray D. EULIAH	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD INTERMEDIATE STEEL DIAPHRAGMS											
D. EL prover	FOR TYPE III PRESTRESSED CONCRETE											
4/17/2018		GIRDERS										
NLESS ALL SIGNATURES COMPLETED		REVI	[SION	S		SHEET NO.						
TGS ENGINEERS 804–C N. LAFAYETTE ST	NO. BY:	DATE:	NO.	BY:	DATE:	S-23						
SHELBY, NC 28150 PH (704) 476–0003	1		3			TOTAL SHEETS						
CORP. LIĊENŚE NO.: C–0275	2		4			69						
			STE). NO.	PCG10							

						<u> </u>	TAGE	ΙC)EAD	LOA	D D	EFLE	10ITC	N TA	BLE	FOR	GIR	DERS												
0.6"Ø LOW RELAXATION STRANDS				SPA	ΝΑ	GIRD	ER 5							SPA	N B	GIRD	ER 5							SPAN	1 C	GIRD	ER 5			
TENTH POINTS		_	.2	.3	.4	.5	.6	.7	.8	.9			.2	.3	.4	.5	.6	.7	.8	.9			.2	.3	.4	.5	.6	.7	.8	_9
CAMBER (GIRDER ALONE IN PLACE)		.037	.069	.095	.111	.117	.111	.095	.069	.037	•	.037	.069	.095	.111	.117	.111	.095	.069	.037	ł	.037	.069	.095	.111	.117	.111	.095	.069	.037
* DEFLECTION DUE TO SUPERIMPOSED D.L.	+	.013	.026	.035	.041	.043	.041	.034	.025	.012	+	.011	.023	.032	.037	.040	.037	.032	.023	.011	ł	.012	.025	.034	.041	.043	.041	.035	.026	.013
FINAL CAMBER	ł	¹ /4″	1/2"	¹¹ /16″	13/16″	7⁄8″	7⁄8″	³ ⁄4″	⁹ /16″	5/16″	ł	5/16″	9/16″	³ ⁄4"	7⁄8"	15/16″	7⁄8"	3⁄4″	⁹ /16″	5/16"	ł	5⁄16″	⁹ /16″	3⁄4″	7⁄8″	7⁄8"	13/16″	¹¹ /16″	1/2″	۱/ ₄ "
0.6" Ø LOW RELAXATION STRANDS	T			SPAN	<u>۸</u>		56&	7					5	ΡΔΝΙ	3 G1		56&	7					<		G		568			
TENTH POINTS			_2					7	8	9			2		4			7	8	9			2		_4				8	q
CAMBER (GTRDER ALONE TN PLACE)		-0.37	.069	.095	.111	.117	.111	.095	-069	.037	▲	.037	.069	.095	.111	.117		.095	.069	.037	•	.037	.069	.095		.117	.111	.095	.069	.037
* DEFLECTION DUE TO SUPERIMPOSED D.L.		.020	.039	.053	.063	.066	.062	.053	.038	.019		.018	.036	.050	.059	.063	.059	.050	.036	.018	 ↓	.019	.038	.053	.062	.066	.063	.053	.039	.020
FINAL CAMBER		3/16″	3⁄8″	1/2"	9/16″	5⁄8″	9/16″	1/2"	3/8"	3/16″		1/4"	3⁄8″	⁹ /16"	5⁄8″	5⁄8″	5⁄8″	9/16″	3/8″	1/4"	ł	3/16″	3/8"	1/2"	9/16″	5/8″	9/16″	1/2"	3⁄8"	3/16
																											1			<u> </u>
0.6"Ø LOW RELAXATION STRANDS		-		SPA	NA	GIRD	ER 8							SPA	NB	GIRD	ER 8							SPAN	I C	GIRD	ER 8			
TENTH POINTS			. 2	.3	.4	.5	.6	.7	. 8	.9			.2	.3	.4	. 5	.6	<u>.</u> 7	.8	.9		_l	.2	.3	.4	.5	.6	.7	.8	9
CAMBER (GIRDER ALONE IN PLACE)	<u> </u>	.037	.069	.095	.111	.117	.111	.095	.069	.037	I ↑	.037	.069	. 095	.111	.117	.111	.095	.069	.037	ł	.037	.069	.095	.111	.117	.111	.095	.069	.037
* DEFLECTION DUE TO SUPERIMPOSED D.L.	+	.017	.033	.045	.053	.056	.053	.045	. 032	.016	↓	.015	.030	.042	.050	.052	.050	.042	.030	.015	•	.016	.032	.045	.053	.056	.053	.045	.033	.017
FINAL CAMBER	ł	1/4″	7/16″	⁵ ⁄8″	11/16″	3⁄4″	11/16″	5⁄/8″	7/16″	1/4″	I ↑	¹ /4″	1/2″	⁵ ⁄8″	3⁄4″	³ ⁄4″	3⁄4″	⁵ ⁄8″	1/2″	1/4″	ł	¹ /4″	7∕16″	5⁄8″	11/16″	3⁄4″	11/16″	5⁄8″	7⁄16″	1/4″
FINAL DEAD LOAD DEFLECTION TABLE FOR GIRDERS																														
0.6″Ø LOW RELAXATION STRANDS				SPA	N A	GIRD	ER 1							SPA	N B	GIRD	ER 1							SPAN	1 C	GIRD	ER 1			
TENTH POINTS			.2	.3	_4	.5	.6	.7	. 8	_9			.2	.3	.4	. 5	.6	.7	. 8	.9		<u>_</u>	.2	.3	.4	.5	.6	.7	.8	.9
CAMBER (GIRDER ALONE IN PLACE)	•	.037	.069	.095	.111	.117	.111	.095	.069	.037	•	.037	.069	.095	.111	.117	.111	.095	.069	.037	ł	.037	.069	.095	.111	.117	.111	.095	.069	.037
* DEFLECTION DUE TO SUPERIMPOSED D.L.	ł	.019	.036	.049	.056	.059	.055	.046	.033	.016	♦	.014	.028	.039	.046	.049	.046	.039	.028	.014	¥	.016	.033	.046	.055	.059	.056	.049	.036	.019
FINAL CAMBER	ł	3/16"	3⁄/8″	⁹ /16″	"/16″	11/16″	11/16″	%6″	7/16″	1/4″	ł	1/4″	1/2"	11/16″	3⁄4″	13/16″	3⁄4″	11/16″	1/2"	1/4″	ł	1/4″	7∕16″	9/16″	11/16″	11/16″	11/16″	9/16″	3⁄8″	3/16″
	. 					0.7.0.0																		60.00		0.7.0.0				
0.6" Ø LOW RELAXATION STRANDS		Τ.					ER 2							SPA	N B		ER 2	_									ER 2			
IENTH POINTS			.2	.3	.4	.5	. 6	.7	.8	<u>_9</u>			.2	.3	.4	.5	<u> </u>	.7	.8	.9			.2	.3	.4	.5	. 6	.7	<u>8</u>	.9
CAMBER (GIRDER ALONE IN PLACE)		.037	.069	.095			.111	_095	.069	.037		.037	.069	.095	.111		.111	.095	.069	.037	T I	.037	.069	.095	.111		.111	.095	.069	.037
* DEFLECTION DUE TO SUPERIMPOSED D.L.		.020	.039	.055	.062	.064	.060	1001	.036	<u>_</u> 018	▼	.016	.032	.045	.053	.056	.053	.045	.032	.016	*	.018	.036	.051	.060	.064	.062	.055	.039	.020
FINAL CAMBER	<u> </u>	3/16″	³ /8"	1/2"	5⁄8″	5⁄8″	5⁄8″	¹ /2″	3/8"	/4″	Î	1/4″	//16″	5⁄8″	1/16″	³ /4″	1/16″	5⁄8″	//16″	1/4″	Ť	1/4″	³ /8″	1/2"	5⁄8″	5⁄8″	5⁄8″	1/2"	3/8"	3/16″
0.6″Ø LOW RELAXATION STRANDS	Т			SPA	ΝΑ	GIRD	ER 3							SPA	N B	GIRD	ER 3							SPAN	1 C	GIRD	ER 3			
TENTH POINTS			.2	.3	.4	.5	.6	.7	. 8	.9		_	.2	.3	.4	.5	.6	<u>.</u> 7	.8	.9		<u>_</u>	.2	.3	.4	.5	.6	.7	.8	.9
CAMBER (GIRDER ALONE IN PLACE)	•	.037	.069	.095	.111	.117	.111	.095	.069	.037	•	.037	.069	.095	.111	.117	.111	.095	.069	.037	ł	.037	.069	.095	.111	.117	.111	.095	.069	.037
* DEFLECTION DUE TO SUPERIMPOSED D.L.	+	.020	.038	.051	.060	.063	.059	.050	.035	.018	•	.016	.032	.045	.053	.056	.053	.045	.032	.016	ł	.015	.029	.040	.047	.049	.046	.039	.027	.013
FINAL CAMBER	ł	3/16"	3⁄/8″	1/2″	⁵ ⁄8″	5⁄8″	5⁄8″	9/16	7⁄16	1/4″	ł	1/4″	7/16″	⁵ ⁄8″	11/16″	3⁄4″	"/16"	⁵ ⁄8″	7/16″	1/4″	ł	1/4″	۱/ ₂ "	5⁄8″	3⁄4″	13/16″	13/16″	11/16″	1/2"	5/16"
					NI 4	0 T D D										0 T D D										0100				
U.6" Ø LOW RELAXATION STRANDS	—	<u> </u>		5PA 								<u> </u>		5PA	N R	GIRD			-			<u> </u>		52AN - 1						
IENTH POINTS			.2	.3	.4	.5	. 6		.8	.9			.2	.3	4	<u>.</u> 5	<u> </u>	.7	.8	.9			.2	.3	.4	. 5	. 6	.7	.8	<u></u>
VANIDER (GIRDER ALUNE IN PLACE)		.031	.063	.035		.11 (.111	.UY5	.067	.03/		.03/	.05C	.095 077	.111	•11 (.111	.035	.05C	.US(⊺	.03/	.067	070	.111	.11 (.111	040	.067	.03/
FINAL CAMBER		CIU.	.029	.040	۵U40 ۲/	137	137		∎UZŎ	<u> </u>		<u> </u>	.020		137	aU40	13/		.020	<u> </u>	¥	.014	1/	"023	137	13/	3/ 10		1/	
		'/4″	'/2″	'716″	1⁄4″	'7/16″	'716″	'716″	'/2 [″]	'/4″	Ī	7/16″	'/2″	'716″	'7/16″	'⁄8″	'%16″	''/16″	'/2"	1/16″	T	'/4″	¹ ⁄2″	'716″	'716″	'7/16″	″4″	'716″	<u> </u> '/2″	'/4″

* INCLUDES FUTURE WEARING SURFACE

ALL VALUES ARE SHOWN IN FEET, EXCEPT ``FINAL CAMBER'' WHICH IS SHOWN IN INCHES.

DRAWN BY :		RTJ	DATE :	2/17
CHECKED BY :		RDE	DATE :	2/17
DESIGN ENGINEER	OF	RECORD : RDE	DATE :	4/17

+

+

PROJECT NO	B-4982
TOF	

IREDELL COUNTY

21+10.00-L-STATION:____

SHEET 1 OF 2

TH CAROL WAR	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH											
SEAL 22992 Pagustiened by:		SUPERSTRUCTURE										
KAN DEER HAN DEER ASTONEDY TASIONS	DEAD LOAD											
4/17/2018		DEF	LE	ECT	IONS							
OCUMENT NOT CONSIDERED FINAL												
NLESS ALL SIGNATURES COMPLETED		REVI	SION	S		SHEET NO.						
TGS ENGINEERS	NO. BY:	DATE:	NO.	BY:	DATE:	S-25						
SHELBY, NC 28150 PH (704) 476-0003	1		3			TOTAL SHEETS						
CORP. LICENSE NO.: C-0275	2		A			69						

						F	- INA	L DE	AD L	OAD	DEF	LECT	ION	TAB	LE F	OR (GIRD	ERS												
0.6"Ø LOW RELAXATION STRANDS				SPAI	N A	GIRD	ER 5							SPAI	NB	GIRD	ER 5					SPAN C GIRDER 5								
TENTH POINTS			.2	.3	.4	.5	.6	.7	.8	.9			.2	.3	_4	. 5	.6	.7	.8	.9		<u>_</u>	.2	.3	_ 4	.5	.6	_7	.8	.9
CAMBER (GIRDER ALONE IN PLACE)	ł	.037	.069	.095	.111	.117	.111	.095	.069	.037	ł	.037	.069	.095	.111	.117	.111	.095	.069	.037	ł	. 037	.069	.095	.111	.117	.111	.095	.069	.037
* DEFLECTION DUE TO SUPERIMPOSED D.L.	ł	.016	.031	.042	.050	.052	.049	.042	.030	.015	ł	.014	.028	.040	.047	.049	.047	.040	.028	.014	ł	. 015	.030	.042	.049	.052	.050	.042	.031	.016
FINAL CAMBER	ł	1/4″	7⁄16″	5⁄8″	3⁄4″	3⁄4″	3⁄4″	5⁄8″	1/2"	1/4″	t	1/4"	1/2"	11/16″	3⁄4″	13/16"	3⁄4″	11/16″	1/2"	1/4″	ł	/4″	1/2"	5⁄8″	3⁄4″	3⁄4″	3/4″	5⁄8″	7/16″	1/4"
							•																							
0.6"Ø LOW RELAXATION STRANDS				SPA	A V	GIRD	ER 6							SPA	NB	GIRD	ER 6							SPAN	I C	GIRD	ER 6			
TENTH POINTS		I	.2	.3	.4	. 5	.6	.7	. 8	_ 9		•	.2	.3	.4	. 5	.6	.7	. 8	_ 9		<u>_</u>	.2	.3	_ 4	.5	. 6	.7	.8	.9
CAMBER (GIRDER ALONE IN PLACE)	ł	.037	.069	.095	.111	.117	.111	.095	.069	.037	ł	.037	.069	.095	.111	.117	.111	.095	.069	.037	ł	.037	.069	.095	. 111	.117	.111	. 095	.069	.037
* DEFLECTION DUE TO SUPERIMPOSED D.L.	¥	.022	.042	.057	.067	.070	.066	.055	.039	.020	ł	.018	.036	.050	.059	.063	.059	.050	.036	.018	¥	. 017	.033	.046	. 053	.055	.052	.044	.031	.015
FINAL CAMBER	ł	3/16″	5/16″	7/16″	9/16″	⁹ /16″	9/16″	1/2"	3⁄8″	3/16″	ł	1/4″	3⁄8″	9/16″	5⁄8″	5⁄8″	5⁄8″	⁹ /16″	133″	۱/ ₄ ″	ł	/4″	7∕16″	5⁄8″	"/16″	3⁄4″	"/16″	⁵ ⁄8″	7∕16″	1/4"
											-																			
0.6"Ø LOW RELAXATION STRANDS				SPA	ΑΛ	GIRD	ER 7							SPA	NB	GIRD	ER 7							SPAN	I C	GIRD	ER 7			
TENTH POINTS			.2	.3	.4	. 5	.6	.7	. 8	.9			.2	.3	.4	. 5	.6	<u>.</u> 7	.8	.9			.2	.3	_ 4	.5	.6	.7	.8	.9
CAMBER (GIRDER ALONE IN PLACE)	ł	.037	.069	.095	.111	.117	.111	.095	.069	.037	ł	.037	.069	.095	.111	.117	.111	.095	.069	.037	ł	. 037	.069	.095	. 111	.117	.111	.095	.069	.037
* DEFLECTION DUE TO SUPERIMPOSED D.L.	ł	.022	.043	. 058	.068	.071	.067	.056	.040	.020	ł	.017	.036	.050	. 059	.063	. 059	.050	.036	.017	¥	.020	.040	.056	.067	. 071	.068	.058	.043	.022
FINAL CAMBER	ł	3/16″	5/16″	7/16″	1/2"	%6″	%6″	7/16″	³ /8"	3/16″	ł	1/4″	3⁄8″	⁹ /16″	⁵ ⁄8″	5⁄8″	5⁄8″	9/16″	3⁄8″	¹ /4″	ł	³ /16″	³ ⁄8″	7/16″	9/16 <i>"</i>	9/16″	1/2"	7/16″	5⁄16″	3/16″
											1										•									
0.6"Ø LOW RELAXATION STRANDS		<u>.</u>		SPAI	N A	GIRD	ER 8	-						SPAI	N B	GIRD	ER 8							SPAN		GIRD	ER 8			
TENTH POINTS		I	.2	.3	.4	. 5	.6	.7	. 8	.9			.2	.3	.4	. 5	.6	. 7	.8	. 9		.	.2	.3	_ 4	. 5	.6	.7	.8	.9
CAMBER (GIRDER ALONE IN PLACE)	ŧ	.037	.069	.095	.111	.117	.111	.095	.069	.037	ł	.037	.069	.095	.111	.117	.111	. 095	.069	.037	ł	. 037	.069	.095	.111	.117	.111	.095	.069	.037
* DEFLECTION DUE TO SUPERIMPOSED D.L.	•	.020	.038	.051	.060	.062	.058	.049	.035	.017	ł	.015	.030	.042	.050	.051	.050	.042	.030	.015	ł	. 017	.035	.049	.058	.062	.060	.051	.038	.020
FINAL CAMBER	ł	3/16″	3⁄8″	1/2"	5⁄8″	11/16″	5⁄8″	9/16″	7⁄16″	1/4″	ł	5/16″	1/2"	5⁄8"	3⁄4″	13/16"	3⁄4″	5⁄8″	1/2″	1/4″	ł	1/4″	7/16″	9/16″	5⁄8″	11/16″	5⁄8″	1/2"	3⁄8″	3⁄16″

* INCLUDES FUTURE WEARING SURFACE ALL VALUES ARE SHOWN IN FEET, EXCEPT ``FINAL CAMBER'' WHICH IS SHOWN IN INCHES.

DRAWN BY :		RTJ	DATE :	2/17
CHECKED BY :		RDE	DATE :	2/17
DESIGN ENGINEER	OF	RECORD : RD)E DATE :	4/17

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B-4982 PROJECT NO. IREDELL _ COUNTY 21+10.00-L-STATION: <u>SHEET 2 OF 2</u> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE DEAD LOAD DEFLECTIONS 4/17/2018 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED SHEET NO. REVISIONS TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275 S-26 DATE: NO. BY: DATE: NO. BY: total sheets 69 X

	PROJECT NO)	B-4982	2
	IRE	DELL	CO	UNTY
	STATION:	21+1	0.00-L	-
	SHEET 2 OF 3			
SEAL 22992 Docusigned by: HALL DEFENSE VICTOR AND ALTICONSIDERED FINAL	DEPARTMEN SUF PARAPE END P FOR 2-B	TATE OF NORTH CA TOFTRA RALEIGH PERSTRU T&N OSTE ARME	NSPORTA	TION IED LS RAIL
UNLESS ALL SIGNATURES COMPLETED	REV	ISIONS		SHEET NO.
804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	NO. BY: DATE: 1 2	NO. BY: 3 4	DATE:	S-28 TOTAL SHEETS 69

+

	TWO BAR METAL RAIL											
BIL AND	L OF FOU	MAT R EN	ERIAL D POS	FOR PA	ARAPET IFIED)							
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT							
B1	140	# 5	STR.	23'-7"	3444							
B2	20	# 5	STR.	21'-11"	457							
B3	20	# 5	STR.	21'-3"	443							
B4	20	#5	STR.	3'-6"	73							
B5	20	# 5	STR.	4'-3"	89							
E1	56	#7	STR.	5'-1"	582							
E2	8	#7	STR.	3'-0"	49							
E3	8	#7	STR.	3'-7'	59							
E4	8	#7	STR.	4'-0"	65							
E5	8	#7	STR.	4'-6"	74							
	10											
+1	12	# 6	SIR.	3'-6"	63							
+2	12	# 6	SIR.	4'-3"	((
+ 3	4	#6 #C	SIR.	3'-10"	23							
	4	#6 #C	SIR.	3'-1"	19							
F5	4	#6 #C	SIR.	2'-6"	15							
F 6	4	#6 #C	SIR.	2'-0"	12							
	4	#C		3-8	22							
ГО	4	<u>"0</u>		0 - C	Ζ1							
56	408	#5	1	6'-11"	2943							
<u> </u>	408	#5	2	7'-0"	2979							
58	72	#5	STR	3'-9"	282							
			5 T T		202							
TOTAL E	POXY-C	DATED F	REINFORC	ING STEEL	11,791 LBS.							
CLASS A	A CONC	RETE			64.6 C.Y.							
$1'-2'' \times 3$	3'-3" CO	NCRETE	PARAPET		441.40 L.F.							
L			B۵	R TYPF								
		ł	<u>−10″</u>		O″ ➡							

SHEET 3 OF 3 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE SEAL 22992 DocuSigned PARAPET & MODIFIED Kay RET END POST DETAILS FOR 2-BAR METAL RAIL 4/17/2018 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED SHEET NO. REVISIONS TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275 S-29 NO. BY: DATE: DATE: BY: TOTAL SHEETS 69

BY :		RTJ		DATE :	
ED BY :		RDE		DATE :	
I ENGINEER	OF	RECORD	: RDF	DATE :	

€ ¹³ / ₁₆	X 1" SLOTS	<u> </u>	★	Ę	.1 ¹ /2″ØHOLE- € SLO		
	³ / ₁₆ " × 1" SLOTS € 1 ¹ / ₂ " Ø 1 3 ³ / ₄ " TOP VIEN	V2'' P HOLE 7		RAIL SEC STANDARD CLAMP BAR	CTION	END V	
					SECTI	ON H-H	I (E
					EXPA	NSION	
	DETAILS	FOR	ATTA	CHING	METAL	RAIL	<u> </u>
ASSEMBLED BY : NMW (CHECKED BY : RDE (DESIGN ENGINEER OF RECORD : RDE (DRAWN BY : FCJ 1/88 REV. 5/7 REV. 5/1	DATE: 2/17 DATE: 2/17 DATE: 4/17 7/03 RWW/JTE 206 TLA/CM						
CHECKED BY : CRK 3/89 REV. 0/1	/II MAA/GM						

€ 1¹/₂'' Ø HOLE –

_<u>↓</u>Ľ ミ

11″

-(+)-

- THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:
- SHALL HAVE A MINIMUM LENGTH OF THREADS OF $1^{1}/_{2}$ ".
- SHALL BE APPROVED BY THE ENGINEER.)

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- SHALL HAVE N.C. THREADS.
- D. STANDARD CLAMP BARS (SEE METAL RAIL SHEET).
- E. $\frac{1}{2}$ " Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 1 OR 2 BAR METAL RAILS.

THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE $\frac{1}{2}$ " PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE 3/4" Ø X 15/8" BOLT WITH WASHER SHALL BE REPLACED WITH A $\frac{3}{4}$ "Ø X $\frac{6}{2}$ " BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE $\frac{3}{4}$ " Ø X 15%" BOLT SHALL APPLY TO THE $\frac{3}{4}$ " Ø X 6 $\frac{1}{2}$ " BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

<u>O END POST</u>

ANGLE TO BE MADE FROM '/2'' X 4'' X 11'' P AND -'/2'' X 4'' X 4'' P

2′′ 2′′

|--|

STRUCTURAL CONCRETE INSERT

A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND

B. 1 - 3/1' Ø X 15/8'' BOLT WITH WASHER.BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307.BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 15/8" GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE

C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A $\frac{7}{16}$ " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

NOTES

METAL RAIL TO END POST CONNECTION

'A. $\frac{1}{2}$ " PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.

B. $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A 3/10 X 15/8" BOLT WITH 2" O.D. WASHER IN PLACE. THE 3/1" Ø X 15/8" BOLT

C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.

CON	R.P.W.(TYP.ALL) *	CLOSED-END									
FER TRUCTURAL	RULE .375" Ø WIRE STRUT	APPROX.4''									
EINSERT	PLAN ELEVATION										
	STRUCTURAL CONCRETE										
	* EACH WELDED ATTACH FERRULE SHALL DEVE STRENGTH OF THE WI	HANT OF WIRE TO ELOP THE TENSILE IRE.									
	PROJECT NO.	B-4982									
	IREDELL	COUNTY									
	STATION: 2	1+10.00-L-									
	SHEET 2 OF 4										
TH CAROL WAR	STATE OF NOT DEPARTMENT OF RALL STAN	RTH CAROLINA TRANSPORTATION EIGH DARD									
SEAL 22992 Docusigned by: Hay DEER Horapy Habbar 4/17/2018	END OF RAIL DETAILS FOR TWO BAR METAL RAILS										
DOCUMENT NOT CONSIDERED FINAL											
NLESS ALL SIGNATURES COMPLETED TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	REVISIONS NO. BY: DATE: NO. 1 2 2	BY: DATE: SHEET NO. BY: DATE: S-31 TOTAL SHEETS 69									
	ST[D. NO. BMR2									

AT THE CONTRACTOR'S OPTION, METAL RAIL MAY BE EITHER ALUMINUM OR GALVANIZED STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF THE GENERAL NOTES AND THE FOLLOWING SPECIFICATIONS FOR THE ALTERNATE MATERIALS; HOWEVER, THE CONTRACTOR WILL BE REQUIRED TO USE THE SAME RAIL MATERIAL ON ALL STRUCTURES ON THE PROJECT FOR WHICH METAL RAIL IS DESIGNATED.

UNLESS OTHERWISE REQUIRED IN THE CONTRACT DOCUMENTS, THE CONTRACTOR HAS THE OPTION TO USE AN ALTERNATE TO THE 2 BAR METAL RAIL. THE ALTERNATE RAIL SHALL MEET THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AND MUST BE LISTED ON THE DEPARTMENT'S APPROVED PRODUCTS LIST (APL) UNDER ``2 BAR METAL RAIL ALTERNATE''. ADJUSTMENTS TO THE CONCRETE PARAPET WILL NOT BE ALLOWED.

MATERIAL FOR POSTS, BASES AND RAILS, EXPANSION BARS AND CLAMP BARS SHALL BE ASTM B-221 ALLOY 6061-T6. MATERIAL FOR RIVETS SHALL BE ASTM B316 ALLOY 6061-T6. RIVETS SHALL BE STANDARD BUTTON HEAD AND CONE POINT COLD DRIVEN AS PER DRAWING. THE BASE OF RAIL POSTS, OR ANY OTHER ALUMINUM SURFACE IN CONTACT WITH CONCRETE SHALL BE THOROUGHLY COATED WITH AN ALUMINUM IMPREGNATED CAULKING COMPOUND OF APPROVED QUALITY. MATERIAL FOR SHIMS TO BE ASTM B209 ALLOY 6061-T6.

MATERIAL AND GALVANIZING ARE TO CONFORM TO THE FOLLOWING SPECIFICATIONS: POST, POST BASES, RAILS, EXPANSION BARS AND CLAMP BARS: AASHTO M270 GRADE 36 STRUCTURAL STEEL -GALVANIZED TO AASHTO M111. RIVETS: RIVETS SHALL MEET THE REQUIREMENTS OF ASTM A502 FOR GRADE 1 RIVETS. THE CUT ENDS OF GALVANIZED STEEL RAILING, AFTER GRINDING SMOOTH SHALL BE GIVEN TWO COATS OF ZINC RICH PAINT MEETING THE REQUIREMENTS OF FEDERAL SPECIFICATION MIL-P-26915 USAF TYPE 1, OR OF FEDERAL

SHIMS: SHIMS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111. RAIL CAPS: RAIL CAPS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPLICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS. FOR END OF RAIL TO CLEAR FACE OF CONCRETE END POST DIMENSION, SEE STANDARD NO. BMR2. CAP SCREWS SHALL BE ASTM F593 ALLOY 305 STAINLESS STEEL. WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL. CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS AND POSTS. SHOP INSPECTION IS NOT REQUIRED. METAL RAIL POSTS SHALL BE SET NORMAL TO CURB GRADE. METHOD OF MEASUREMENT FOR METAL RAILS: FOR LENGTH OF METAL RAILS TO BE PAID FOR, SEE THE STANDARD

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE THE CONTRACTOR MAY, AT HIS OPTION, HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT, THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER. TO INSURE FUTURE IDENTIFICATION OF THE FABRICATOR, A PERMANENT IDENTIFYING MARK SHALL BE PLACED ON EACH POST. THE METHOD OF MARKING AND LOCATION SHALL BE SUCH THAT IT DOES NOT DETRACT FROM THE APPEARANCE OF THE POST, BUT REMAINS VISIBLE AFTER RAIL PLACEMENT. SHIMS SHALL BE USED AS NECESSARY FOR POST ALIGNMENT. ALLOY 6351-T5 MAY BE SUBSTITUTED FOR ALLOY 6061-T6 WHERE APPLICABLE. MINOR VARIATIONS IN DETAILS OF METAL RAIL WILL BE CONSIDERED. DETAILS OF SUCH VARIATIONS, IF DESIRED, SHALL BE SUBMITTED FOR APPROVAL.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

PAY LENGTH = 406.8 LIN.FT.

NOTES

ALUMINUM RAILS

GALVANIZED STEEL RAILS

GENERAL NOTES

	PROJEC	B-498 CC 0.00-L	1982 _ COUNTY 0-L-		
SEAL 22992 DocuSioned by:	DEPA	STAT ARTMENT	OF NORTH CAN OF TRA RALEIGH	NSPORTA	TION
4/17/2018	2	BAR	MET	AL RA	
TGS ENGINEERS		REVIS	STONS	0.75	SHEET NU.
804–C N. LAFAYETTE ST	NU. BY:	DATE:	NO. BY: ର	DATE:	
PH (704) 476–0003 CORP. LICENSE NO.: C–0275	2		୍ଚ ୟ		SHEETS 69

ASSEMBLED BY :	NMW	DATE :	2/17
CHECKED BY :	RDE	DATE :	2/17
DESIGN ENGINEER OF	RECORD :	RDE DATE :	<u>4</u> /17
DRAWN BY : MAA CHECKED BY : GM	5/10 RE 5/10 RE	V. 12/5/II V. 6/13	
	E RE	V. 1/15	

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THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED.AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " Ø GALVANIZED BOLTS, NUTS AND WASHERS.THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307.THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

SKETCH SHOWING POINTS OF ATTACHMENT

*LOCATION OF GUARDRAIL ATTACHMENT

B-4982 PROJECT NO. IREDELL COUNTY 21+10.00-L-STATION:___

SEAL 22992 Docusigned by: HMG NEER HUHH 1705 EUT (FragDate) 4/17/2018	(depa GUA F	RTMENT S ⁻ RDRA DI OR M		NORTH CAR TRAI RALEIGH NDAR AN AIL	NSPORTA D NCHOR S RAIL	TION RAGE .S
DOCUMENT NOT CONSIDERED FINAL INLESS ALL SIGNATURES COMPLETED			REVIS	STON	21		SHEET NO.
	NO.	BY:	DATE:		BY:	DATE:	S-34
804–C N. LAFAYETTE ST SHELBY, NC 28150	1			3	_ •		TOTAL SHEETS
CORP. LICENSE NO.: C-0275	2			4			69
		(SH	Т 5Ь)		STD	. NO. GF	RA3

GENERAL NOTES

1. FOR EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.

2. ALL PLATES AND ANGLES SHALL CONFORM TO AASHTO M270 GRADE 36 STEEL OR APPROVED EQUAL. ALL HOLD-DOWN BOLTS SHALL CONFORM TO ASTM F593 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL CONFORM TO ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL. ALL STUD ANCHORS SHALL CONFORM TO AASHTO M169, GRADES 1010 THRU 1020 OR APPROVED EQUAL. ALL CONCRETE INSERTS SHALL BE CLOSED END AND SHALL CONFORM TO AASHTO M169, GRADE 12L14. TENSILE CAPACITY SHALL BE 3000 LBS. MIN.

3. A PREMOLDED CORRUGATED OR NON-CORRUGATED GLAND SHALL BE USED FOR JOINTS SKEWED BETWEEN 50° THRU 130°.FOR JOINTS SKEWED LESS THAN 50° OR MORE THAN 130°, ONLY A CORRUGATED GLAND SHALL BE USED.

4. CLOSED END FERRULES AND STUD ANCHORS SHALL BE SHOP WELDED AND ALL HOLES SHALL BE SHOP DRILLED AS SHOWN ON PLANS. STUD ANCHORS SHALL BE ELECTRIC ARC END WELDED WITH COMPLETE FUSION.

5. SURFACES COMING IN CONTACT WITH NEOPRENE SHALL BE GROUND SMOOTH PRIOR TO METALLIZING.

6. UPON COMPLETION OF SHOP FABRICATION, THE HOLD DOWN PLATE AND BASE ANGLE ASSEMBLY, AS SHOWN IN THE "TYPICAL SECTION OF BASE ANGLE ASSEMBLY", SHALL BE METALLIZED. SEE SPECIAL PROVISION FOR THERMAL SPRAYED COATINGS (METALLIZATION).

7. BASE ANGLE ASSEMBLY SHALL BE CONTINUOUS FOR THE LENGTH OF THE JOINT. AT CROWN BREAKS, THE ENDS OF THE BASE ANGLE ASSEMBLY SHALL BE CUT PARALLEL TO THE BRIDGE CENTERLINE FOR SKEWS LESS THAN 80° AND GREATER THAN 100°. FINISHED WELD SHALL BE GROUND SMOOTH AND COATED WITH A MINIMUM THICKNESS OF 4 DRY MILS OF ZINC-RICH PAINT IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

8. FIELD SPLICES OF HOLD-DOWN PLATES SHALL BE KEPT TO A MINIMUM. CONTRACTOR SHALL FURNISH DETAILED PLANS SHOWING PROPOSED SPLICE LOCATIONS FOR APPROVAL. HOLD-DOWN PLATES SHALL NOT EXCEED 20' LENGTHS UNLESS APPROVED BY THE ENGINEER.

9. NO ALTERNATE JOINT DETAILS SHALL BE PERMITTED IN LIEU OF THOSE SHOWN ON THESE PLANS.

10. THE CONTRACTOR MAY, AT HIS OPTION, USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF CONCRETE INSERTS FOR COVER PLATES. THE YIELD LOAD OF THE $\frac{3}{4}$ "Ø BOLT IS 10 KIPS. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

	PROJECT I STATION SHEET 1 OF 2	NO REDEI	_L 21+1	B-4982 CO 0.00-L	2 OUNTY -			
SEAL 22992 Docusigned by: 4/17/2018	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD EXPANSION JOINT SEAL DETAILS							
DOCUMENT NOT CONSIDERED FINAL INLESS ALL SIGNATURES COMPLETED		REVISIO	NS		SHEET NO.			
804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	^{NO.} BY: [1 2	DATE: NO. 3 4	BY:	DATE:	TOTAL SHEETS 69			

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				RE	INF	ORC		IG	BAF	R SC	HEDI	JLE										
	DECK SLAB – THREE SPAN CONTINUOUS UNIT STAGE 1																					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT					
* A1	363	# 5	STR.	30'-5"	11,516	A200	8	# 6	STR.	5'-0"	60	₩ B1	42	#4	STR.	30'-0″	842					
A2	363	# 5	STR.	30'-5"	11,516	A201	4	#5	STR.	29'-6"	123	* B2	42	#6	STR.	60'-0"	3,785					
* A3	414	# 5	STR.	5'-4"	2,303	A202	4	# 5	STR.	28'-6"	119	* B3	42	#4	STR.	23'-4"	655					
Δ4	414	# 5	STR.	4'-8"	2,015	A203	4	# 5	STR.	27′-6″	115	* B4	80	# 6	STR.	25'-0″	3,004					
* A100	8	# 6	STR.	5'-0"	60	A204	4	# 5	STR.	26′-6″	111	B5	106	# 5	STR.	30'-0″	3,317					
* A101	4	# 5	STR.	29'-6"	123	A205	4	# 5	STR.	25′-6″	106	B6	106	# 6	STR.	60'-0"	9,553					
* A102	4	# 5	STR.	28'-6"	119	A206	4	# 5	STR.	24'-6"	102	B7	53	# 5	STR.	43′-8″	2,414					
* A103	4	# 5	STR.	27'-6"	115	A207	4	# 5	STR.	23′-6″	98											
* A104	4	# 5	STR.	26'-6"	111	A208	4	# 5	STR.	22'-6"	94	* G1	2	# 5	STR.	43'-0"	90					
* A105	4	# 5	STR.	25′-6″	106	A209	4	# 5	STR.	21′-6″	90											
* A106	4	# 5	STR.	24'-6"	102	A210	4	# 5	STR.	20'-6"	86	₩ J1	84	#4	1	1'-5″	79					
* A107	4	# 5	STR.	23'-6"	98	A211	4	# 5	STR.	19′-6″	81											
* A108	4	# 5	STR.	22'-6"	94	A212	4	# 5	STR.	18′-6″	77	₩ K1	12	*6	3	23'-10″	430					
* A109	4	# 5	STR.	21'-6"	90	A213	4	# 5	STR.	17'-6″	73	₩ K2	6	# 6	2	14'-4"	129					
* A110	4	# 5	STR.	20'-6"	86	A214	4	# 5	STR.	16′-6″	69	₩ K3	6	# 6	2	13'-4"	120					
* A111	4	# 5	STR.	19′-6″	81	A215	4	# 5	STR.	15′-6″	65	₩ K4	8	#8	3	25'-4″	541					
* A112	4	# 5	STR.	18'-6"	77	A216	4	# 5	STR.	14'-6"	60	₩ K5	4	#8	2	16′-6″	176					
* A113	4	# 5	STR.	17'-6"	73	A217	4	# 5	STR.	13'-6″	56	* K6	4	#8	2	13'-7"	145					
* A114	4	# 5	STR.	16'-6"	69	A218	4	# 5	STR.	12'-6″	52	₩ K20	12	#4	STR.	8'-10"	71					
* A115	4	# 5	STR.	15'-6"	65	A219	4	# 5	STR.	11'-6"	48	* K21	36	#4	STR.	10'-0"	240					
* A116	4	# 5	STR.	14'-6"	60	A220	4	# 5	STR.	10'-6"	44	₩ K22	16	#4	4	14'-11"	159					
* A117	4	# 5	STR.	13'-6"	56	A221	4	# 5	STR.	9'-6″	40	* K23	8	#4	5	16′-6″	88					
* A118	4	# 5	STR.	12'-6"	52	A222	4	# 5	STR.	8'-6"	35											
* A119	4	# 5	STR.	11'-6″	48	A223	4	# 5	STR.	7′-6″	31	* S1	60	# 5	6	4'-10"	302					
* A120	4	# 5	STR.	10'-6"	44	A224	4	#5	STR.	6'-6"	27	* S2	60	# 5	7	4'-8"	292					
* A121	4	# 5	STR.	9'-6"	40	A225	4	# 5	STR.	5′-6″	23	* S3	12	#4	8	4'-5″	35					
* A122	4	# 5	STR.	8'-6"	35	A226	4	# 5	STR.	4'-6"	19	* S4	36	#4	8	5'-2″	124					
* A123	4	# 5	STR.	7'-6"	31	A227	4	#5	STR.	3′-6″	15	* S5	144	#4	9	2'-9"	265					
* A124	4	# 5	STR.	6'-6"	27	A228	4	# 5	STR.	2'-6"	10											
* A125	4	# 5	STR.	5'-6"	23							₩ U1	36	#4	3	12'-4"	297					
* A126	4	# 5	STR.	4'-6"	19																	
* A127	4	# 5	STR.	3'-6"	15							TOTA	AL REIN	FORCIN	S STEEL:	30	,744 LBS					
* A128	4	# 5	STR.	2'-6"	10							TOTAL E	* A128 4 *5 STR. 2'-6" 10 TOTAL REINFORCING STEEL: 30,744 LB * A128 4 *5 STR. 2'-6" 10 TOTAL REINFORCING STEEL: 30,744 LB * A128 4 *5 STR. 2'-6" 10 TOTAL REINFORCING STEEL: 30,744 LB									

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STAGE	SUPERSTRUCTURE	QUANTITIES
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REINFORCING STEEL	30,744	LBS
EPOXY COATED REINFORCING STEEL	33,209	LBS
REINFORCED CONCRETE DECK SLAB	6,534	SF
CLASS AA CONCRETE	265.3	СҮ
GROOVING BRIDGE FLOORS	7,262	SF
2-BAR METAL RAIL	203.4	LF

DRAWN BY :		NMW		DATE :	2/17
CHECKED BY :		RDE		DATE :	2/17
DESIGN ENGINEER	OF	RECORD	: RDE	DATE :	4/17

REINFORCING STEEL SUMMARY								
	REINFORCING STEEL (LBS.)	EPOXY-COATED REINFORCING STEEL (LBS.)						
DECK SLAB	30,744	27,617						
PARAPET & END POSTS		5,592						
TOTAL	30,744	33,209						

CLASS AA CONCRE	TE SUMMARY			
	CUBIC YDS.			
DECK SLAB (POUR 1)	56.8			
(POUR 2)	78.8			
(POUR 3)	85.7			
(CLOSURE POUR)	12.9			
SUB-TOTAL	234.2			
PARAPET & END POSTS	31.1			
TOTAL	265.3			

GROOVING	BRIDGE FLOORS
BRIDGE DECK	5,902 SQ.FEET
APPROACH SLAB	1,360 SQ.FEET
TOTAL	7,262 SQ.FEET

	PROJECT I	NO REDEI	_L	B-4982 C0	2 UNTY	
	STATION:	l	21+10	0.00-L	-	
SEAL 22992 Docusigned by: 4/17/2018	DEPARTI SUF STAC	STATE O MENT O PERS LL OI GE 1 (F NORTH CARG F TRAN RALEIGH TRU F MAT	NSPORTA CTURI CRIAL RUCTIO	TION E DN	
INLESS ALL SIGNATURES COMPLETED	REVISIONS SHEET					
804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	^{NO.} BY: D 1 2	ате: NO. З Д	BY:	DATE:	S-38 TOTAL SHEETS 69	
			-		-	

	REINFORCING BAR SCHEDULE																
	DECK SLAB - THREE SPAN CONTINUOUS UNIT STAGE 2																
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	363	# 5	STR.	30'-5″	11,516	A200	8	#6	STR.	5'-0"	60	* B1	42	#4	STR.	30'-0"	842
Α2	363	#5	STR.	30'-5"	11,516	A201	4	#5	STR.	29′-6″	123	* B2	42	#6	STR.	60'-0"	3,785
* A3	414	#5	STR.	5'-4"	2,303	A202	4	#5	STR.	28'-6"	119	* B3	42	#4	STR.	23'-4"	655
Δ4	414	#5	STR.	4'-8"	2,015	A203	4	#5	STR.	27′-6″	115	* B4	80	#6	STR.	25'-0"	3,004
* A100	8	#6	STR.	5'-0"	60	A204	4	#5	STR.	26′-6″	111	B5	108	#5	STR.	30'-0"	33,79
* A101	4	#5	STR.	29'-6″	123	A205	4	#5	STR.	25′-6″	106	B6	108	#6	STR.	60'-0"	9,733
* A102	4	#5	STR.	28'-6"	119	A206	4	#5	STR.	24'-6"	102	B7	54	#5	STR.	43′-8″	2,459
* A103	4	#5	STR.	27'-6″	115	A207	4	#5	STR.	23′-6″	98						
* A104	4	#5	STR.	26'-6"	111	A208	4	*5	STR.	22′-6″	94	* G1	2	# 5	STR.	43'-0"	90
* A105	4	#5	STR.	25'-6"	106	A209	4	#5	STR.	21'-6″	90						
* A106	4	#5	STR.	24'-6"	102	A210	4	#5	STR.	20'-6"	86	* J1	84	#4	1	1'-5"	79
* A107	4	#5	STR.	23'-6"	98	A211	4	#5	STR.	19'-6"	81						
* A108	4	#5	STR.	22'-6"	94	A212	4	#5	STR.	18'-6"	77	₩ K7	12	#6	3	21'-0"	379
* A109	4	#5	STR.	21'-6"	90	A213	4	# 5	STR.	17'-6″	73	₩ K8	6	#6	2	14'-4"	129
* A110	4	#5	STR.	20'-6"	86	A214	4	#5	STR.	16'-6"	69	₩ K9	6	#6	2	14'-9"	133
* A111	4	#5	STR.	19'-6"	81	A215	4	#5	STR.	15'-6″	65	₩ K10	8	#8	3	22'-6"	481
* A112	4	#5	STR.	18'-6"	77	A216	4	#5	STR.	14'-6"	60	* K11	4	#8	2	16'-7"	177
* A113	4	#5	STR.	17'-6"	73	A217	4	#5	STR.	13'-6″	56	₩ K12	4	#8	2	15'-1"	161
* A114	4	#5	STR.	16'-6"	69	A218	4	#5	STR.	12'-6″	52	₩ K24	12	#4	STR.	7'-4"	59
* A115	4	#5	STR.	15'-6"	65	A219	4	#5	STR.	11'-6"	48	₩ K25	36	#4	STR.	8'-7"	206
* A116	4	#5	STR.	14'-6"	60	A220	4	#5	STR.	10'-6"	44	₩ K26	16	#4	4	13'-6"	144
* A117	4	#5	STR.	13'-6"	56	A221	4	#5	STR.	9'-6″	40	₩ K27	8	#4	5	15'-1"	81
* A118	4	#5	STR.	12'-6"	52	A222	4	#5	STR.	8′-6″	35						
* A119	4	#5	STR.	11'-6"	48	A223	4	#5	STR.	7'-6″	31	* S1	54	#5	6	4'-10"	272
* A120	4	#5	STR.	10'-6"	44	A224	4	#5	STR.	6'-6"	27	* S2	54	#5	7	4'-8"	263
* A121	4	#5	STR.	9'-6"	40	A225	4	#5	STR.	5′-6″	23	米 S3	12	#4	8	4'-5"	35
₩ A122	4	#5	STR.	8'-6"	35	A226	4	#5	STR.	4'-6"	19	* S4	36	#4	8	5'-2"	124
* A123	4	#5	STR.	7'-6"	31	A227	4	#5	STR.	3'-6"	15	* S5	126	#4	9	2'-9"	231
* A124	4	#5	STR.	6'-6"	27	A228	4	#5	STR.	2'-6"	10						
₩ A125	4	#5	STR.	5'-6"	23							* ∪1	30	#4	3	12'-4"	247
* A126	4	#5	STR.	4'-6"	19												
₩ A127	4	#5	STR.	3'-6"	15							тоти	AL REIN	FORCIN	G STEEL:	30	0,991 LBS
* A128	4	#5	STR.	2'-6"	10							TOTAL E	EPOXY-C	COATED I	REIN.STE	EEL: 27	,285 LBS

DRAWN BY :		NMW		DATE :	:	2/17
CHECKED BY :		RDE		DATE :	:	2/17
DESIGN ENGINEER	OF	RECORD	: RDE	DATE :	:	4/17

K12 5'-4" K11 6'-10" K9 5'-9" K8 5'-4" THIS LEG OVER GIRDER
$\frac{1}{1}$
2'-6%/6"

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE									
FOLLOWING MINIMUM SPLICE LENGTHS									
BAR SIZE	SUPERSTE EXCEPT A SLABS, P AND BARR	RUCTURE PPROACH ARAPET, IER RAIL	APPROACH SLABS BARRIER						
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL				
#4	2'-0"	1'-9″	2'-0"	1'-9″	2'-9"				
# 5	2'-6"	2'-2"	2'-6"	3'-5″					
# 6	3'-0"	2'-7″	3'-10"	2'-7"	4'-4"				
# 7	5'-3"	3'-6"							
# 8	6'-10"	4'-7"							

REINFORCING STEEL	30,991 LBS
EPOXY COATED REINFORCING STEEL	32,877 LBS
REINFORCED CONCRETE DECK SLAB	6,534 SF
CLASS AA CONCRETE	270.2 CY
GROOVING BRIDGE FLOORS	7,262 SF
2-BAR METAL RAIL	203 . 4 LF

DRAWN BY :		NMW		DATE :	2/17
CHECKED BY :		RDE		DATE :	2/17
DESIGN ENGINEER	OF	RECORD	: RDE	DATE :	4/17

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REINFORCING STEEL SUMMARY							
	REINFORCING STEEL (LBS.)	EPOXY-COATED REINFORCING STEEL (LBS.)					
DECK SLAB	30,991	27,285					
PARAPET & END POSTS		5,592					
TOTAL	30,991	32,877					

CLASS AA CONCRE	TE SUMMARY
	CUBIC YDS.
DECK SLAB (POUR 1)	57 . 8
(POUR 2)	80.7
(POUR 3)	87.5
(CLOSURE POUR)	13.1
SUB-TOTAL	239.1
PARAPET & END POSTS	31.1
TOTAL	270.2

GROOVING	BRIDGE FLOORS
BRIDGE DECK	5,902 SQ.FEET
APPROACH SLAB	1,360 SQ.FEET
TOTAL	7,262 SQ.FEET

	PR ST SHE	OJE (ATI)	2 OUNTY -				
SEAL 22992 Docusigned by: HAM NEER HUM NEER 4/17/2018		DEP#	ARTMEN	RS 0F 2 C	NORTH CARI F TRAN RALEIGH TRU MA1 ONST	NSPORTA	tion E ON
JNLESS ALL SIGNATURES COMPLETED			REV	ISION	٧S		SHEET NO.
TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	≥ 1	BY:	DATE:	3 4	BY:	DATE:	S-40 total sheets 69

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STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS. BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING. THE TOP SURFACE AREAS OF THE END BENT CAPS SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THE MEMBRANE CURING COMPOUND

THE TOP SURFACE OF THE END BENT CAP EXCEPT THE BRIDGE SEAT BUILDUPS SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE BACK FACE AT

		пт				
ВАК ПТРЕЗ		<u>ב ויט</u> חד			HIEKIA	
(1) нк си ой	RAR			TYPF	I JIAU	WFTCHT
	B1	2	#9	1	50'-3"	342
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	B2	1	#9	1	50'-8"	172
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	B3	1	#9 #0	1	51"-0'	173
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	B4 B5	<u> </u>	#9 #9	1	52'-0"	180
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	B6	1	#9	1	53'-3"	180
B6 52'-0" 1'-3"	B7	2	#9	1	53'-7"	364
B7 52'-4" 1'-3"	B19	36	#4 #4	STR	28'-2"	677
					<u> </u>	JZ
	Н3	12	#4	3	16′-9″	134
	H4	12	#4	3	17'-1"	137
()	К1	16	# ⊿	STR	26'-9"	286
	K3	2	#4	10	7'-9"	10
16'-1" НЗ						
16'-5" H4	S1	130	#4 #≀	4	12'-5"	1078
$1'-8'' \varnothing$	52 S4	32	#4	7	6'-6"	139
	S5	44	#4	6	3'-8"	108
$\frac{1}{2'-7''} = \frac{8''}{ -1-1 }$	S7	3	#6 #C	9	10'-9"	48
	58 < 9	12	#6 #⊿	8	5'-4" 6'-8"	<u> </u>
	S10	15	#4	6	5′-8″	57
	V1	88	#4	STR	7'-3"	426
		42	~4		<u> </u>	204
	- REI	NFORCI	ng ste	i Eel		
4 ¹ /2" 3'-8" 4 ¹ /2"	(ST)	AGE 1)			5,	,623 LBS.
χ_{1}^{*} χ_{2}^{*} ζ_{1}^{*}	CLAS	SS A CO	DNCRET	E BREA	AKDOWN	
		OR END	BENI	I SIA(GE 1) Not	79 C C V
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<u>َنْ 6</u> أَنْ		(5	STAGE	1 CONS	, T _)	
<u> </u>	POU	R#2B וו	ACKWAL PPFR F	L AND Part C))F WINGS	9.5 C.Y.
		(?	STAGE	1 CONS	(T_)	
ALL DAK UIMENSIUNS AKE UUT TU UUT.	—					
END BENI NO.1 STAGE 1						
NO: 9 I TN_ FT_= 270	ТОТ		55 1 1		TF	48-1 C Y
		AL ULA	JJ A U	UNCI\E	• •	
PILE DRIVING EQUIPMENT SETUP FOR HP 12×53 STEEL PIL	ES					
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		RTMEN	STATE OF N	IORTH CARC	NCDUBLY,	
TH UAROL WA				ALEIGH		
		SI	IRSTI		LIRF	
Z2992 Docusigned by:			– –			
Ranc REEFELINAT	~	END	BE	NI	No.1	
and	STA	GE	1_CC	NS T	RUCT	TON
4/17/2018			DET	AIL	S	
DOCUMENT NOT CONSIDERED FINAL						
UNLESS ALL SIGNATURES COMPLETED	NO 54	RE	VISIONS	5	0.175	SHEET NO. S-44
ENGINEERS 804-C N. LAFAYETTE ST SHELBY, NC 28150	1 BY:	DATE:	NO.	BY:	DATE:	
PH (704) 476–0003 CORP. LICENSE NO.: C–0275	2					5HEE 15

DRAWN BY :		CCC	DATE :	2/17
CHECKED BY :		RDE	DATE :	2/17
DESIGN ENGINEER	OF	RECORD : RDE	DATE :	4/17

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THE TOP SURFACE AREAS OF THE END BENT CAPS SHALL BE CURED IN ACCORDANCE

PILE DRIVING EQUIPMEN

3AR 1	[YPFS	1	RT		F M/	TERTA	1
			FND			STAC	- F 2
•	01.0"	RAD					WETCUT
	2'-8" S10	DAK		317E #0		LENGIA 47'-3"	
"		Ra	1	#9		46'-11"	160
"		R10	1	#a		<u></u> <u></u> <u></u>	160
			1	#0		<u></u> ΛΕ'_7"	155
				- " " + 0		40 - (" лел г."	
	$ (6) \langle \theta \theta \theta \theta \theta \theta \theta \theta \theta $	B12		#9 #2		45'-5"	154
<i>"</i>		B13		#9 #~		44'-4"	
"		B14	1	#9		43'-10"	149
		B15	1	#9	1	48'-1"	163
		B16	1	#9	1	46'-5"	158
	/1'-3'' LAP	B17	1	#9	1	44'-8"	152
		B18	5	#9	STR	14'-4"	244
	\checkmark	B20	36	# 4	STR	24'-9"	595
		B21	12	#4	STR	3'-8"	29
			4 -			4	A
	$\left(\begin{array}{c} \overline{} \end{array} \right)$	H1	12	#4	2	17'-8"	142
		H2	12	#4	2	17'-0"	136
_	\sim $/$		1.5			• • • • • •	005
		K2	16	#4	SIR	24'-10"	265
	1'-8" (7	<u>K3</u>	2	#4	10	7'-9"	10
H2						4 -	
		<u>S1</u>	42	#4	4	12'-5"	348
		S2	124	#4	5	4'-5"	366
	•• •	S3	82	#4	4	11'-8"	639
	< 2' - (" 8"	<u> </u>	28	#4	7	6'-6"	122
		S5	44	#4	6	3'-8"	108
		<u> </u>	3	#6	9	10'-9"	48
		S8	3	#6	8	5'-4"	24
		<u> </u>	8	#4	6	6'-8"	36
		<u>\$10</u>	10	#4	6	5'-8″	38
	2'-0" 1'-11" 2'-0"				0.7.0	.	40.0
		V1	88	#4	SIR	('-3"	426
<u> </u>		V2	42	#4	SIR	8'-11"	250
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∼					·		1
<u> </u>		REIN	NFORCI	NG STE	EL	F	390 I BC
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	₩' > z'_c / "		Ū	PPER F	PARTO	F WINGS	
			(5	STAGE	Z LUNS	1	
	NIS ARE OUT TO OUT						
	AND ANE UUT TU UUT.						
D BENT	No.1 STAGE 2						
12 X 5	53 STEEL PILES						
	LIN.FT.= 240	ΤΟΤΑ	L CLAS	SS A C	ONCRE	TE	42.5 C.Y.
NI SEL	UF FUR HF 12X53 SIEEL PILES						
ΕA	сн = 8						
	D			0	P	8-498	2
		LC					
			TKF	DEL	L	<u>C</u> O	UNTY
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	SH	eet 8 OI	- 8				
			ç	STATE OF N	ORTH CARC	LINA	
	ARALL CARAL	DEPA	RTMEN	IT OF	TRAN	ISPORTA	TION
	DFESS IDING			RA	LEIGH		
			۲	IBCTI		LIRF	
	SEAL 22992		JL	וונייי			
	Ray D. Flutt		FND	RF	NT	No. 1	
	ADVOINEER ADVOINTATA ADDISON	ст м) ()) ()) N I C -		
		SIA					TON
	4/17/2018			DET	AIL	S	
	ENT NOT CONCIDEDED FINIAL						
UNLESS	ALL SIGNATURES COMPLETED		RE	VISIONS) 		SHEET NO.
EERS 8	TGS ENGINEERS	BY:	DATE:	NO.	BY:	DATE:	S-48
	SHELBY, NC 28150 PH (704) 476–0003			3			TOTAL SHEETS
CO 📕	RP. LIČENŚE NO.: C-0275 2	I T		4			69

STATE C. P. OF. P.

<u>45-00'-00</u>" (TYP_)

€ BEARING

A126.

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.

ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL".

CONSTRUCTION JOINT DETAIL

	PROJECT NO. <u>B-4982</u> <u>IREDELL</u> COUNTY STATION: <u>21+10.00-L-</u> SHEET 2 OF 4
SEAL Doctusigned by: Kay D. Ellight D. ELLIGHT 4/17/2018	DEPARTMENT OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE BENT NO. 1 STAGE 2 CONSTRUCTION DETAILS
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TGS ENGINEERS 804-C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476-0003 CORP. LICENSE NO.: C-0275	REVISIONSSHEET NO.NO.BY:DATE:NO.BY:DATE:SHEET NO.130BY:DATE:TOTAL SHEETS 69

Pewwy Py . 1.8.28015 party : 2/.7				
TRANERY: 18.250015 19.17; 2/17				
Prover By : 18.20015 Date : 2/17				
DRAW DY	EXEMPTY I.2.1NV15 04T + 2/1 EXECUTE DEVELOPMENT OF ACTION - 47 04T - 2/17 EXECUTE DEVELOPMENT OF ACTION - 47 04T - 2/17			
Sken By : T.BENUIS ONE: 2/17	RAWN DY . T.2 FINIS DAT . 2/17 CROMENDED FLOOR DATE . 2/17 CROMENDED FLOOR DATE . 2/17			
Sken Br. L.J.LANUS Dati: 2/17	теминик т. Т.2. FINITS (241) - 2/17 переили и т. П.2. FINITS (241) - 2/17 переили и т. П.2. FINITS (241) - 2/17 переили и т. П.2. FINITS (241) - 2/17			
Deven s. T. I.S.EMMS DATE # 247	Rem by : LEENIIS DAT : 2/17 BSON BOOSD 5: R.D.A.RING DAT : 2/17 BSON BOOSD 5: R.D.A.RING DAT : 2/17			
DAMM BY. L.B.LINIS DATE: 2/17	Dawn Dr LEGNNIS DAT - 2/17 DESDN KONSTOR - R. DAT - 2/17 DESDN KONSTOR - R. DAT - 2/17			
PANN 57. T.B.EMNIS 1415. 2417	DARM BY 1 T.B.EWITS DATE 2/17 PROCEDURES OF PROJECT 2/17 PROCEDURES OF PROJECT 2/17			
DRAW, BY. T.B.EW.IS DATE: 2/17	итали ву т. Т.Я.ЕМИТS вите : 2/17 висателя и п. п. п. 18.19.19. вите : 2/17 весон такаето об недова : дела и п. 19.19.			
DRAN 57. T.B.ENNIS DAG: 2/17	Draw BY: J.B.EWIIS DIT: 2/17 Protoco BY: P.A.R.2VIOS JP DATE: 2/17 Protoco BY: P.A.R.2VIOS JP DATE: 2/17			
ENGINEY, T.B.E.NIS DIE, 2/17	Determ TU . T.B. EWNTS EXT: 2/17 Crease Bur P. A. RAYNOR JR EXT: 2/17 Crease Bur P. A. RAYNOR JR EXT: 2/17 Crease Bur P. A. RAYNOR JR EXT: 2/17			
DRAWLEY, TREINIS DIE, 2/17	THAM BY 1. T.B.EWITS DATE 2/17 PHERED DY F, A.B.KAYDOL JR DATE 2/17 PHERED DY F, A.B.KAYDOL JR DATE 2/17 PHERED DY F, A.B.KAYDOL JR DATE 2/17			
PRAVE 57 ; LB.ENVIS DATE ; 2/17	DRAWN BY			
PROMERY: TREENIS DATE: 2/17	DRAWL DV ; T.B.EDNYES DATE 2/17 CHECKED DV ; R.A.DAYNOR R. DATE 2/17 CHECKED DV ; R.A.DAYNOR R. DATE 2/17 DESIGN LAWAGE OF RECOM P.C. DATE 4/17			
DRAW. 67 ; T.B.E.NIS DATE ; 2/17	DRAWL BY - T.B.ENNIS DATE - 2/17 CHECKED BY - R.A.GAMMOR JR DATE - 2/17 CHECKED BY - R.A.GAMMOR JR DATE - 2/17 DESNE LEMALE OF RECORD JR DE DATE - 4/17			
PRAN BY : T.B.ENNIS DATE : 2/17	DRAWL BY, , T.B.EDNIS DATE, 2/17 DECKID BY, R. DATANOR R DATE, 2/17 DESIGN EXCEL OF RCOMP RE DATE, 2/17			
PRAM. BY: LEJENNIS PAG: 2/17	DRAWN BY ; T.G.E.NNTS DATE ; 2/17 CHOROED BY ; R.A.FAYNOR JR DATE ; 2/17 CESON ENGER PP FECON JR DATE ; 2/17 DESON ENGER PP FECON JR DATE ; 2/17			
PRANK BY : T.G.E.NNIS DATE : 2/1	DRAWN BY; T.B.ENNIS DATE: 2/17 CMCKCED BY; R.A.RAYNOR JR DATE: 2/17 CMCKCED BY; R.A.RAYNOR JR DATE: 2/17 CESOK ENGER BY FRCOM JR DD DATE: 4/17			
PRAVE BY : T.ELENNIS DATE : 2/17	DRAWN BY, T.B.ENNIS DATE, 2/17 DESCREMENT PROFILE DATE, 2/17 DESCREMENT PROFILE PATE, 2/17 DESCREMENT PROFILE PROFILE PATE, 2/17			
PRAN BY TEEENIS DATE 2/17	DRAWN BY : T.B.ENNIS DATE : 2/17 CMCKCKD BY : R.A.RAYNOR JR DATE : 2/17 CMCKCKD BY : R.A.RAYNOR JR DATE : 2/17 DESON FONCER OF FRCOM REQ DATE : 4/17			
PRAVN BY. T.B.EMVIS DATE: 2/17	DRAWN BY; T.B.ENNIS DATE: 2/17 CHECKED BY: R.A.RAYNOR JR DATE: 2/17 CHECKED DY: R.B.A.RAYNOR JR DATE: 2/17 DESKIR DAMEER DF RECOME: ROL EATE: 2/17			
DRAWN BY. T.E.ENNIS DATE: 2/17	DRAWN BY ; T.B.ENNIS DATE: 2/17 CHECKED BY : R.A.RAYNOR JE DATE: 2/17 DESCHE DATE: R OTE: 2/17 DESCHE DATE: R OTE: R OTE: 2/17			
DRAWN BY : T.B.ENNIS DATE (2/17	DRAWN BY ; T.B.ENNIS DATE : 2/17 CHECKED BY : R.A.RAYNOR JF. DATE : 2/17 DESIGN ENDER DO FREC DATE : 2/17			
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DRAWN BY 4 T.B.ENNIS DATE: 2/17	DRAWN BY ; T.B.ENNIS DATE ; 2/17 CHECKED BY ; R.J.A.RAYNOR JR DATE ; 2/17 DESIGN ENGRER OF REGORD ; RDE DATE ; 2/17			
DRAIN BY: T.B.E.NNIS DATE: 2/17	DRAWN BY: T.B.ENNIS DATE: 2/17 CHECKED BY: R.A.RAYNOR JR DATE: 2/17 DESIGN ENGRER OF REGO F. RED DATE: 2/17			
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	CHECKED BY : R.A.RAYNOR JR DATE : 2/17 DESIGN ENGINEER OF RECORD : RDE DATE : 4/17	ſ	DRAWN BY : T.B.ENNIS DATE : 2/1	.7

- * THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.
- ** THE SP-2, SP-3, SP-4, SP-5, AND SP-6 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR *5 PLAIN OR DEFORMED BAR.

BILI	BILL OF MATERIAL					BILI	LOF	MATEF	RIAL	
BEI	NT 1	STAGE	E 1			BEI	NT 1	STAGE	E 2	
NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
7	11	1 STR.	46'-11" 23'-3"	1,745 62	B6 B7	7 20	<u>11</u> 4	STR.	<u>38'-4"</u> 20'-10"	1,426 278
20	4	STR.	24'-7"	328	B8	7	11	1	40'-9"	1,516
	<u> </u>	STR. STR.	<u>45'-4"</u> 5'-7"	<u>1,686</u> 26	<u> </u>	7	4	STR. STR.	4'-11"	<u>51</u> 23
10	11	стр	34'-10"	1 051	B11	7	11	STR.	15'-3"	567
10	11	STR.	31'-10"	1,691	М1	10	11	STR.	39′-10″	2,116
10	11	STR.	30'-10"	1,638	M2	10	11	STR.	37'-10"	2,010
98	5	2	14'-9"	1,508	<u>\$1</u>	40	5	2	14'-9"	615
38	4	3	7'-2"	182	52	52	5		14'-0"	(59
5	4	3	8'-2"	27	U1	40	4	3	7'-2"	191
5	4	3	7'-0"	23	<u>U2</u>	5	4	3	<u>8'-2"</u>	27
30	11	1	13′-8″	2,178	05	5	4	5	1-0	23
7	*	Λ	75 4/ 10/	711	V1	20	11	1	13'-8"	1,452
<u> </u>	** **	4	501'-9"	523	SP-1	2	*	4	354'-10"	474
1	**	4	444'-2"	463	SP-2	1	**	4	600'-5"	626
1	* *	4	419'-6"	438	SP-3	1	**	4	559'-4"	583
		TTTFS						TTTFS		
CING SI	TEEL		LBS	12.945	REINFOF	RCING S	TEEL		LBS	. 11.054
				0.175						1.0.7
	REINFUR	ACTING 5	ILLL LDS	. 2,135	SPIRAL	CULUMIN	REINFUR	KUING S	IEEL LDS	1,665
	ETE			7.0	CLASS A		ETE			
<u>"2 CULU</u> #3 CAP	IMN5		<u>CU. YDS</u>	44.8	POUR	<u>#2 COLL</u> #3 CAP	JMNS		<u>CU. YDS</u>	<u> </u>
3 0/11			CU. YDS	52.7	TOTAL				CU. YDS	43.2
*1 DRILL	ED PIEF	RS	CU. YDS.	24.2	POUR	#1 DRILL	LED PIEF	RS	CU. YDS.	20.7
RTILED	PTFRS_	TN SOTI	I F	42.2	3′-6″Ø[PTFRS_	IN SOTI	I F	40.8
RILLED	PIERS, N	NOT IN	SOIL L.F	25.8	<u>3'-6"Ø</u> [DRILLED	PIERS, N	NOT IN	SOIL L.F	17.2
S			L.F.	290	CSL TUB	ES			L.F.	244
NT STE	EL CASI	NG	L.F.	33	PERMAN	ENT STEI	EL CASI	NG	L.F.	22

	PROJECT NO. <u>B-4982</u> IREDELL COUNTY						
	STATION: 21+10.00-L-						
SEAL 22992 Docusigned by: Ray D. ELLUH W. NEB Aronson 440000 D. ELLUH 4/17/2018	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE BENT No. 1 DETAILS						
OCUMENT NOT CONSIDERED FINAL NLESS ALL SIGNATURES COMPLETED	REVISIONS SHEET NO.						
TGS ENGINEERS 804-C N. LAFAYETTE ST	NO. BY: DATE: NO. BY: DATE: S-52						
SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	1 3 TOTAL SHEETS 2 4 69						

B-4982

DRAWN BY :	T.B.ENNIS	DATE :	2/17
CHECKED BY :	R.A.RAYNOR JR	DATE :	2/17
DESIGN ENGINEE	R OF RECORD : RDE	DATE :	4/17

Pewwy Py . 1.8.28015 party : 2/.7				
TRANERY: 18.250015 19.17; 2/17				
Prover By : 18.20015 Date : 2/17				
DRAW DY	EXEMPTY I.2.1NV15 04T + 2/1 EXECUTE DEVELOPMENT OF ACTION - 47 04T - 2/17 EXECUTE DEVELOPMENT OF ACTION - 47 04T - 2/17			
Sken By : T.BENUIS ONE: 2/17	RAWN DY . T.2 FINIS DAT . 2/17 CROMENDED FLOOR DATE . 2/17 CROMENDED FLOOR DATE . 2/17			
Sken Br. L.J.LANUS Dati: 2/17	теминик т. Т.2. FINITS (241) - 2/17 переили и т. П.2. FINITS (241) - 2/17 переили и т. П.2. FINITS (241) - 2/17 переили и т. П.2. FINITS (241) - 2/17			
Deven s. T. I.S.EMMS DATE # 247	Rem by : LEENIIS DAT : 2/17 BSON BOOSD 5: R.D.A.RING DAT : 2/17 BSON BOOSD 5: R.D.A.RING DAT : 2/17			
DAMM BY. L.B.LINIS DATE: 2/17	Dawn Dr LEGNNIS DAT - 2/17 DESDN KONSTOR - R. DAT - 2/17 DESDN KONSTOR - R. DAT - 2/17			
PANN 57. T.B.EMNIS 1415. 2417	DARM BY 1 T.B.EWITS DATE 2/17 PROCEDURES OF PROJECT 2/17 PROCEDURES OF PROJECT 2/17			
DRAW, BY. T.B.EW.IS DATE: 2/17	итали ву т. Т.Я.ЕМИТS вите : 2/17 висателя и п. п. п. 18.19.19. вите : 2/17 весон такаето об недова : дела и п. 19.19.			
DRAN 57. T.B.ENNIS DAG: 2/17	Draw BY: J.B.EWIIS DIT: 2/17 Protoco BY: P.A.R.2VIOS JP DATE: 2/17 Protoco BY: P.A.R.2VIOS JP DATE: 2/17			
ENGINEY, T.B.E.NIS DIE, 2/17	Determ TU . T.B. EWNTS EXT: 2/17 Crease Bur P. A. RAYNOR JR EXT: 2/17 Crease Bur P. A. RAYNOR JR EXT: 2/17 Crease Bur P. A. RAYNOR JR EXT: 2/17			
DRAWLEY, TREINIS DIE, 2/17	THAM BY 1. T.B.EWITS DATE 2/17 PHERED DY F, A.B.KAYDOL JR DATE 2/17 PHERED DY F, A.B.KAYDOL JR DATE 2/17 PHERED DY F, A.B.KAYDOL JR DATE 2/17			
PRAVE 57 ; LB.ENVIS DATE ; 2/17	DRAWN BY			
PROMERY: TREENIS DATE: 2/17	DRAWL DV ; T.B.EDNYES DATE 2/17 CHECKED DV ; R.A.DAYNOR R. DATE 2/17 CHECKED DV ; R.A.DAYNOR R. DATE 2/17 DESIGN LAWAGE OF RECOM P.C. DATE 4/17			
DRAW. 67 ; T.B.E.NIS DATE ; 2/17	DRAWL BY - T.B.ENNIS DATE - 2/17 CHECKED BY - R.A.GAMMOR JR DATE - 2/17 CHECKED BY - R.A.GAMMOR JR DATE - 2/17 DESNE LEMALE OF RECORD JR DE DATE - 4/17			
PRAN BY : T.B.ENNIS DATE : 2/17	DRAWL BY, , T.B.EDNIS DATE, 2/17 DECKID BY, R. DATANOR R DATE, 2/17 DESIGN EXCEL OF RCOMP RE DATE, 2/17			
PRAM. BY: LEJENNIS PAG: 2/17	DRAWN BY ; T.G.E.NNTS DATE ; 2/17 CHOROED BY ; R.A.FAYNOR JR: DATE ; 2/17 CESON ENGER PP FECON JR: DATE ; 2/17 DESON ENGER PP FECON JR: DATE ; 2/17			
PRANK BY : T.G.E.NNIS DATE : 2/1	DRAWN BY; T.B.ENNIS DATE: 2/17 CMECKED BY; R.A.RAYNOR JR DATE: 2/17 CESOK ENGER & FREGOR JR DATE: 2/17 CESOK ENGER & FREGOR JR DATE: 2/17			
PRAVE BY : T.ELENNIS DATE : 2/17	DRAWN BY, T.B.ENNIS DATE, 2/17 DESCREMENT PROFILE DATE, 2/17 DESCREMENT PROFILE PATE, 2/17 DESCREMENT PROFILE PROFILE PATE, 2/17			
PRAN BY TEEENIS DATE 2/17	DRAWN BY : T.B.ENNIS DATE : 2/17 CMCKCKD BY : R.A.RAYNOR JR DATE : 2/17 CMCKCKD BY : R.A.RAYNOR JR DATE : 2/17 DESON FONCER OF FRCOM REQ DATE : 4/17			
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DRAWN BY. T.E.ENNIS DATE: 2/17	DRAWN BY ; T.B.ENNIS DATE: 2/17 CHECKED BY : R.A.RAYNOR JE DATE: 2/17 DESCHE DATE: R OTE: 2/17 DESCHE DATE: R OTE: R OTE: 2/17			
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	CHECKED BY : R.A.RAYNOR JR DATE : 2/17 DESIGN ENGINEER OF RECORD : RDE DATE : 4/17	ſ	DRAWN BY : T.B.ENNIS DATE : 2/1	.7

- * THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.
- ** THE SP-2, SP-3, SP-4, SP-5, AND SP-6 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR *5 PLAIN OR DEFORMED BAR.

BILL OF MATERIAL BILL OF MATERIAL BENT 2 STAGE 1 BENT 2 STAGE 2 NO. SIZE TYPE LENGTH WEIGHT BAR NO. SIZE TYPE LENGTH 7 11 1 46'-11" 1,745 B6 7 11 SIR. 38'-4" 20 4 STR. 24'-7" 328 B8 7 11 1 40'-9" 20 4 STR. 24'-7" 328 B8 7 11 1 40'-9" 7 11 STR. 45'-4" 1,686 B9 7 4 STR. 10'-10" 10 11 STR. 23'-4" 1,612	
BENT 2 STAGE 1 BENT 2 STAGE 2 NO. SIZE TYPE LENGTH WEIGHT BAR NO. SIZE TYPE LENGTH 7 11 1 46'-11" 1,745 B6 7 11 STR. 38'-4" 4 4 STR. 23'-3" 62 B7 20 4 STR. 20'-10" 20 4 STR. 24'-7" 328 B8 7 11 1 40'-9" 7 11 STR. 45'-4" 1,686 B9 7 4 STR. 10'-10" 7 4 STR. 5'-7" 26 B10 7 4 STR. 15'-3" 10 11 STR. 26'-4" 1,612	
NO. SIZE TYPE LENGTH WEIGHT BAR NO. SIZE TYPE LENGTH 7 11 1 46'-11" 1,745 B6 7 11 STR. 38'-4" 4 4 STR. 23'-3" 62 B7 20 4 STR. 20'-10" 20 4 STR. 24'-7" 328 B8 7 11 1 40'-9" 7 11 STR. 45'-4" 1,686 B9 7 4 STR. 10'-10" 7 4 STR. 5'-7" 26 B10 7 4 STR. 4'-11" 10 11 STR. 26'-4" 1,399 M1 10 11 STR. 34'-4" 10 11 STR. 23'-4" 1,240 M2 10 11 STR. 32'-4" 98 5 2 14'-9" 1,508 S1 40 5 2 <td></td>	
7 11 1 46'-11" 1,745 B6 7 11 STR. 38'-4" 4 4 STR. 23'-3" 62 B7 20 4 STR. 20'-10" 20 4 STR. 24'-7" 328 B8 7 11 1 40'-9" 7 11 STR. 45'-4" 1,686 B9 7 4 STR. 10'-10" 7 4 STR. 5'-7" 26 B10 7 4 STR. 10'-10" 10 11 STR. 30'-4" 1,612 B11 7 11 STR. 34'-4" 10 11 STR. 23'-4" 1,240 M2 10 11 STR. 32'-4" 98 5 2 14'-9" 1,508 S1 40 5 2 14'-9" 5 4 3 7'-2" 182 - - - - - - - - - - - - - - -	WEIGHT
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7 11 STR. $45'-4''$ 1,686 B9 7 4 STR. $10'-10''$ 7 4 STR. $5'-7''$ 26 B10 7 4 STR. $10'-10''$ 10 11 STR. $30'-4''$ 1,612 11 STR. $34'-4''$ 10 11 STR. $26'-4''$ 1,399 M1 10 11 STR. $34'-4''$ 10 11 STR. $23'-4''$ 1,240 M2 10 11 STR. $32'-4''$ 98 5 2 $14'-9''$ 1,508 S1 40 5 2 $14'-9''$ 38 4 3 $7'-2''$ 182	1,516
1 4 5/1. 20 1 4 5/1. 4/1. 10 11 STR. 30'-4" 1,612 1 1 STR. 34'-4" 10 11 STR. 26'-4" 1,399 M1 10 11 STR. 34'-4" 10 11 STR. 23'-4" 1,240 M2 10 11 STR. 34'-4" 10 11 STR. 23'-4" 1,240 M2 10 11 STR. 34'-4" 10 11 STR. 23'-4" 1,240 M2 10 11 STR. 34'-4" 98 5 2 14'-9" 1,508 S1 40 5 2 14'-9" 5 4 3 7'-2" 182	<u>51</u>
10 11 STR. 30'-4" 1.612	<u> </u>
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10 11 STR. 23'-4" 1,240 M2 10 11 STR. 32'-4" 98 5 2 14'-9" 1,508 S1 40 5 2 14'-9" 38 4 3 7'-2" 182	1,824
98 5 2 14'-9" 1,508 S1 40 5 2 14'-9" 38 4 3 7'-2" 182	1,718
38 4 3 7'-2" 182 14'-0" 5 4 3 8'-2" 27 U1 40 4 3 7'-2" 5 4 3 8'-2" 27 U1 40 4 3 7'-2" 5 4 3 7'-0" 23 U2 5 4 3 8'-2" 0 11 1 13'-8" 2.178 03 5 4 3 7'-0" 30 11 1 13'-8" 2.178 03 11 1 13'-8" 1 ** 4 354'-10" 711 04 04 04 04'-0" 1 ** 4 329'-0" 343 SP-2 1 ** 4 493'-6" 1 ** 4 329'-0" 343 SP-2 1 ** 4 452'-5" QUANTITIES QUANTITIES QUANTITIES QUANTITIES Column REINFORCING STEEL LBS. 1.766 SPIRAL COLUMN REINFORCING STEEL LBS CL ASS A CONCRETE	615
38 4 3 7'-2" 182	759
3 4 3 7'-0" 21 U1 40 4 3 1'-2 5 4 3 7'-0" 23 U2 5 4 3 8'-2" 30 11 1 13'-8" 2,178	101
30 11 1 13'-8" 2,178 1 1 13'-8" 3 7'-0" 30 11 1 13'-8" 2,178 1 1 13'-8" 3 * 4 354'-10" 711 1 13'-8" 1 ** 4 411'-3" 429 SP-1 2 * 4 354'-10" 1 ** 4 411'-3" 429 SP-1 2 * 4 354'-10" 1 ** 4 329'-0" 343 SP-2 1 ** 4 493'-6" 1 ** 4 271'-5" 283 SP-3 1 ** 4 452'-5" QUANTITIES QUANTITIES QUANTITIES QUANTITIES QUANTITIES IBS ISS	27
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V1 20 11 1 13'-8" 3 * 4 354'-10" 711 1 1 13'-8" 1 ** 4 411'-3" 429 SP-1 2 * 4 354'-10" 1 ** 4 329'-0" 343 SP-2 1 ** 4 493'-6" 1 ** 4 271'-5" 283 SP-3 1 ** 4 452'-5" 0UANTITIES QUANTITIES QUANTITIES QUANTITIES OUANTITIES COLUMN REINFORCING STEEL LBS. 1.766 SPIRAL COLUMN REINFORCING STEEL LBS LBS CONCRETE CLASS A CONCRETE E E E E *2 COLUMNS CU. YDS. 7.9 POUR *2 COLUMNS CU. YDS E *3 CAP CU. YDS. 44.8 POUR *3 CAP CU. YDS CU. YDS *1 DRTILED PIERS CU. YDS 18.0 POUR *1 DRTILED PIERS CU. YDS	
3 ** 4 354'-10" /11 1 ** 4 411'-3" 429 SP-1 2 * 4 354'-10" 1 ** 4 329'-0" 343 SP-2 1 ** 4 493'-6" 1 ** 4 329'-0" 343 SP-2 1 ** 4 493'-6" 1 ** 4 271'-5" 283 SP-3 1 ** 4 452'-5" QUANTITIES QUANTITIES QUANTITIES COLUMN REINFORCING STEEL LBS. 12,016 REINFORCING STEEL LBS. COLUMN REINFORCING STEEL LBS. 12,016 COLUMN REINFORCING STEEL LBS. 1766 COLUMN REINFORCING STEEL LBS. COLOURN REINFORCING STEEL LBS. COL YDS. 7.9 POUR *2 COLUMNS CU. YDS. CU. YDS. 44.8 POUR *3 CAP CU. YDS. CU. YDS. 18.0 POUR *1 DRTILED PIERS	1,452
1 ** 4 411 - 3 429 3F - 1 2 * 4 354 - 10 1 ** 4 329' - 0" 343 SP - 2 1 ** 4 493' - 6" 1 ** 4 271' - 5" 283 SP - 3 1 ** 4 493' - 6" 1 ** 4 271' - 5" 283 SP - 3 1 ** 4 452' - 5" QUANTITIES QUANTITIES QUANTITIES QUANTITIES COLUMN REINFORCING STEEL LBS. 1,766 COLUMN REINFORCING STEEL LBS. 1,766 CONCRETE CLASS A CONCRETE *2 COLUMNS CU. YDS. 7.9 POUR *2 COLUMNS CU. YDS. *3 CAP CU. YDS. 44.8 POUR *3 CAP CU. YDS. CU. YDS. 18.0 POUR *1 DRTUED PIERS CU. YDS.	
1 ** 4 271'-5" 283 SP-3 1 ** 4 452'-5" QUANTITIES QUANTITIES QUANTITIES CING STEEL LBS. 12,016 REINFORCING STEEL LBS COLUMN REINFORCING STEEL LBS. 1,766 SPIRAL COLUMN REINFORCING STEEL LBS CONCRETE CLASS A CONCRETE *2 COLUMNS CU. YDS. 7.9 POUR *2 COLUMNS CU. YDS. *3 CAP CU. YDS. 44.8 CU. YDS. 52.7 TOTAL *1 DRILLED PIERS CU. YDS 18.0 POUR *1 DRILLED PIERS	<u> </u>
QUANTITIES QUANTITIES CING STEEL LBS. 12,016 REINFORCING STEEL LBS COLUMN REINFORCING STEEL LBS. 1,766 SPIRAL COLUMN REINFORCING STEEL LBS CONCRETE CLASS A CONCRETE *2 COLUMNS CU. YDS. 7.9 POUR *2 COLUMNS CU. YDS. *3 CAP CU. YDS. 44.8 CU. YDS. 52.7 TOTAL *1 DRTILLED PIERS CU. YDS. 18.0 POUR *1 DRTILED PIERS	472
QUANTITIES QUANTITIES CING STEEL LBS. 12,016 REINFORCING STEEL LBS COLUMN REINFORCING STEEL LBS. 1,766 SPIRAL COLUMN REINFORCING STEEL LBS CONCRETE CLASS A CONCRETE *2 COLUMNS CU. YDS. 7.9 POUR *2 COLUMNS CU. YDS. *3 CAP CU. YDS. 44.8 CU. YDS. 52.7 TOTAL *1 DRTILLED PIERS CU. YDS. 18.0 POUR *1 DRTILED PIERS	
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CONCRETE CLASS A CONCRETE #2 COLUMNS CU. YDS. 7.9 POUR #2 COLUMNS CU. YDS. 7.9 POUR #3 CAP CU. YDS. CU. YDS. 52.7 TOTAL CU. YDS. #1 DRTILLED PTERS CU. YDS. 18 0 POUR #1 DRTILLED PTERS	1 461
CONCRETE CLASS A CONCRETE #2 COLUMNS CU. YDS. 7.9 POUR #2 COLUMNS CU. YDS. #3 CAP CU. YDS. 44.8 POUR #3 CAP CU. YDS. CU. YDS. 52.7 TOTAL CU. YDS. #1 DRTILED PIERS CU. YDS. 18.0 POUR #1 DRTILED PIERS CU. YDS.	
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#1 DRTILED PTERS CILYDS 18.0 POUR #1 DRTILED PTERS CILYDS	38.0
	43.2
	16.7
RTILED PTERS TN SOTI LE 235 3'-6" @ DRTILED PTERS TN SOTI LE	29
RILLED PIERS, NOT IN SOIL L.F. 27 3'-6"Ø DRILLED PIERS, NOT IN SOIL L.F	18
LIF. 220 CSL TUBES LIF.	200
INT STEEL CASING L.F. 30 PERMANENT STEEL CASING L.F.	20

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BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL,CORRUGATED ALUMINUM ALLOY,OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

PILE SPLICE DETAILS

NOTES:

STIRRUPS IN CAP MAY BE SHIFTED AS NECESS BACKWALL SHALL BE PLACED BEFORE APPLYING

THE TOP SURFACE AREAS OF THE END BENT CAL WITH THE STANDARD SPECIFICATONS EXCEPT T METHOD SHALL NOT BE USED.

THE TOP SURFACE OF THE END BENT CAP EXCE SHALL BE SLOPES TRANSVERSELY FROM THE FI AT THE RATE OF 2%.

TYPES			BI	LL O	F MA	ATERIA	L
ARE OUT TO OUT.		Ε	ND	BEN	T 2	STAG	E 1
		BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
		B50	2	#9	1	53'-7"	364
2'-8" S10		B51	1	#9	1	53'-4"	181
		B52	2	#9 #0	1	52'-0"	354
		827 827		#9 #0	1 1	50'-Y"	115
		B54 B55	1	#9	1 1	<u> </u>	180
		B56	$\frac{1}{1}$	#9	1	<u> </u>	173
		B57	36	#4	STR	28'-3"	679
		B58	12	#4	STR.	3'-8"	29
/──1′-3′′ LAP		114.0				4 -7 , 4 4	170
		н10 µ11		#4 #∕	2	<u> </u>	152
\backslash $/$					۲	10 1	
		К1О	16	#4	STR	28'-3"	302
(\frown)		K11	2	#4	10	7'-9"	10
		C 1	174	# 4		10/ 5"	4 4 4 4
		51 52	134 134	-‴4 #⊿	4 5	12 -5" 2'-5"	1,111 395
		S4	32	#4	7	6′-6″	139
1'-8"Ø		S5	47	#4	_6	3'-8"	115
1		S7	3	# 6	9	10'-9"	48
. 2′-7″ 8″		S8	3	# 6	8	5'-4"	24
< нк.		59 C10	16	#4 #1	6	6'-8"	(1 70
		510		- 4	Ø	ు −రె	01
		V10 V11	94 44	#4 #4	STR STR	7'-7" 9'-6"	476 279
		REIN	FORCI	NG STE	EL		700
< 2'-0″ ► < 1'-11″ ► < 2'-0″ ► 		(STA(JE 1)			5,	190 LBS.
		CLASS			E BREA	KDOWN	
		יר UI סיי וחP	יי ביאט #1 י	ואו⊒ט אי∩ו AP	∠ ۲۵۱۵ ۱FR ۲۰	RT	39200
		. JUK	· C 0	F WINC		OLLARS	UJAC VATA
			(5	STAGE 1	CONS	1.)	
		POUR	#2 B	ACKWAL	L AND	F WINCS	9.1 C.Y.
3'-61/4"			U (S	STAGE 1		. ттисэ Т")	
		τοται	_ CLAS	SS A CO	ONCRE	TE	48.3 C.Y.
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			EN	שי BEN] 12 יי כו	וווס. 2 הז הדי	STAGE 1	
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		F	OR HP	12×53	STEEL	PILES	
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JANT TU ULEAK ANCHUR BOLTS.							
, THE EPOXY PROTECTIVE COATING.	I						
APS SHALL BE CURED IN ACCORDANC	CE						
THE MEMBRANE CURING COMPOUND							
PT THE ROTOCE SEAT DUTY DUDG							
LL FACE TO THE FRONT FACE							
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PH (704) 476-0003 CORP. LICENSE NO.: C-0275	12			<u>୍</u> ଅ 			SHEETS 69

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PLAN OF WING (W3)

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL,CORRUGATED ALUMINUM ALLOY,OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

CORROSION PROTECTION FOR STEEL PILES DETAIL

DRAWN BY :	JLA	DATE :	2/17
CHECKED BY :	RDE	DATE :	4/17
DESIGN ENGINEER	OF RECORD : RDE	DATE :	4/17

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NOTES:

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSA

BACKWALL SHALL BE PLACED BEFORE APPLYING THE TOP SURFACE AREAS OF THE END BENT CAPS WITH THE STANDARD SPECIFICATONS EXCEPT THE METHOD SHALL NOT BE USED.

THE TOP SURFACE OF THE END BENT CAP EXCEPT SHALL BE SLOPES TRANSVERSELY FROM THE FILL AT THE RATE OF 2%.

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ARE OUT TO OUT.				BEIN			
	┝	B2K B28	NU.	517F #1	I TPL	LENGIH 3'-8"	WEIGHI 27
<mark>< 2′-8″ ≤ S10</mark>	┣	B60	1	# <u>9</u>	1	43'-10"	149
<u>3′-8″</u> S9		B61	1	#9	1	44'-3"	150
<mark>≼ 8″ ⊳</mark> S5		B62	1	# 9	1	45′-6″	155
		B63	1	#9	1	46'-9"	159
	_	B64	1	#9 #0	1 СТР	47'-2"	160
	_	865 866	2 1	#9 #9	51R. 1	<u>15 -6</u> <u>44'-9"</u>	264
		B67	1	#9	1	45'-5"	152
1/_3// I AD		B68	1	# 9	1	46′-5″	158
	_	B69	1	#9	1	47'-5"	161
$\langle \cdot \rangle$	_	B70	1	#9 #4	1 СТР	48'-0"	163
		DTI	70		SIN.	24 -10	591
		H12	11	#4	2	15′-11″	117
((7))		H13	11	#4	2	16'-3"	119
			1.0		C T D		0.07
		K20	16	#4 #1	SIR 10	<u>25'-0"</u> 7'-9"	267
1′-8″Ø		NII.	2	- 4	10	1-9	10
	F	S1	46	#4	4	12'-5"	382
21-7" 0"		S2	120	#4	5	4'-5"	354
	Ľ	S3	74	#4	4	11'-8"	577
	F	S4	28	#4 #4		6'-6"	122
	_	<u>55</u> 57	40	#4 #6	6 9	10'-9"	98 48
	┣	S8	3	#6	8	5'-4"	24
		S9	16	#4	6	6'-8"	71
2'-0" 1'-11" 2'-0"		S10	20	#4	6	5′-8″	76
			10	+ 4		0/ 0//	0.41
	_	VIZ V13	40 80	+4 #⊿	SIR	9'-0" 7'-3"	241
	_	VIJ	00		511	I J	501
		RFTN	FORCI	NG STE	FL		
		(STA)	GE 2)			5,	,342 LBS.
3′-6 ¹ ∕4″ ►		CLASS	S A CC	NCRET	E BREA	KDOWN	
		(FOI	REND	BENT	2 STA	GE 2)	70 0 0 1
		POUR	#1 C.	AP,LOV F WIN(VER PA GS & C	RT COLLARS	32.8 C.Y.
			(S	STAGE	2 CONS	ST.)	
	1	POUR	#2 B	ACKWAL	L AND		8.3 C.Y.
3'-6'/4"			U (S	PPER F Stage	PART 0 2 CONS	F WINGS	
		τοτλ					41.1.C.Y
	_				UNCIL		
			EN	D BEN	No.2	STAGE 2	
ARY TO CLEAR ANCHOR BOLTS.			HP	12 X	53 STE	EL PILES	
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IL WEWDINAME CONTING COMPOUND		PILE	EXCA	VATION	I NOT	IN SOIL	40 L.F.
T THE BRIDGE SEAT BUILDUPS	-	P	ILE D	RIVINO	G EQUI	PMENT SE	TUP
L FACE TO THE FRONT FACE		F	OR HP	12×53	STEEL	PILES	
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804–C N. LAFAYETTE ST SHELBY, NC 28150	אט. B` 1	r:	DATE:	NO. ବ୍ଲ	BY:	DATE:	J-04 TOTAL
PH (704) 476-0003 CORP. LICENSE NO.: C-0275	2			 4			sheets 69
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ESTIMATED QUANTITIES						
GE @ 21+10.00-L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE				
	TONS	SQUARE YARDS				
) BENT 1	189	210				
BENT 1	315	350				
ENT 2	293	325				
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OFFESS DWAY HAT	ST/ DEPARTMEN	ATE OF NORTH CAROLINA TOFTRANSP RALEIGH	ORTATION			
SEAL 22992 Docusigned by: RAME NEER ELLIGHT		AP DET	AILS —			
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B-4982

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TGS ENGINEERS	NO.	BY:	DATE:	NO.	BY:	DATE:	S-65
SHELBY, NC 28150	1			3			TOTAL SHEETS
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	#5	STR	4'-5"	5	* B9	1	#5	STR	3'-5"	4
	# 6	STR	4'-5″	7	B10	1	# 6	STR	3′-5″	5
	#4	1	1'-5"	40	* J1	42	#4	STR	1'-5"	40
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)	#6	STR	24'-6"	2.208	B2	60	#6	STR	24'-6"	2.208
	#5	STR	3'-9"	4	* B3	1	#5	STR	4'-1"	4
	# 6	STR	3'-9"	6	B4	1	#6	STR	4'-1"	6
	#5	STR	3'-5"	4	* B5	1	# 5	STR	4'-5"	5
	#6	SIR	3'-5"	5	B6	1	*6	SIR	4'-5"	(
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DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SO.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SO.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SO.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $1\frac{1}{2}$ "RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

DRAWN BY :	NMW	DATE :	2/17
CHECKED BY :	RDE	DATE :	2/17
DESIGN ENGINEER	OF RECORD : RDF	DATE :	4/17

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{1}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 1/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{1}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION. SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

	PROJEC	T NO. IRED DN:	 ELL 21+1	B-4982 CC 10.00-L	2)UNTY 	
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TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275	№. вү: 1 22	DATE:	NO. ВҮ: З 4.	DATE:	S-69 total sheets 69	
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