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### NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

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### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

# ROADWAY SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. <u>34802.1.1 (U-2412A)</u> F.A. PROJ. <u>STP-4121(1)</u>			
COUNTY <b>GUILFORD</b>			
PROJECT DESCRIPTION <b>GREENSBORO/HIGH POINT - SR 4121</b>			
(GREENSBORO/HIGH POINT RD.) FROM THE PROPOSED			
US 311 BYPASS TO SR 1480 (VICKERY CHAPEL ROAD)			
INVENTORY			

N.C.	34802.	1.1 (U-2412A)	1	Ĺ	90
STATE	PROJ. NO.	P. A. PROJ. NO.	DES	CRIE	MOLL
U-2412A	STP-4121(1)	P.E.			
			R/W	&	UTIL,

### CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VANDIUS FELD BORNO LOSS, ROCK CORES, AND SOLI TEST DATA AVAILABLE MAY BE REVIEWED OR RISPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1993 250-4088. NETHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORNOL LOGS, ROCK CORES, OR SOLI TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A CEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSUMFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSUMFACE CONDITIONS BETWEEN BORNOS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATIORY SAMPLE DATA AND THE IN STU UN-PLACED TEST DATA CAN BE RELED ON ONLY TO THE DEGREE OF RELIABLITY INHERENT IN THE STANDARD TEST METHOD. THE OSSERVEW WATER LEVELS OR SOIL MOSITUME CONDITIONS INDICATED IN THE SUBSUMFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTUME CONDITIONS MOILT ONDITIONS TO CLUMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELMMARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT, THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MODE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSART ID SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAMA FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NCDOT PERSONNE

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\_\_\_\_\_

NOVEMBER, 2008



DRAWN BY: N. D. MOHS, W. D. FIELDS

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

### DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

	SOIL AND ROCK LEGEND	TERMS, SYMBOLS, AND ABBREVIATIONS	
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN	<u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <u>UNIFORM</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE, (ALSO POORLY GRADED)	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL AN INFERRED	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SDIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE.	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONG	AQUIFER - A WATER BEARING FORMATION OR STRATA.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:	ANGULARITY OF GRAINS  THE ANGULARITY OR ROUNDNESS OF BOIL GRAINS IS DESIGNATED BY THE TERMS; ANGULAR,	OF WEATHERED ROCK.  ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
VERY STIFF, GRAY, SATY CLAY, MOST WITH INTERBEDDED FINE SAND LATERS, HIGHLY PLASTIC, 4-7-6	SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT YOUR DIVISION OF A MALES NOR	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION	SEONS FER TOOL II TESTED,	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL  AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) DRGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	CRYSTALLINE ROCK (CR)  FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ET.	GROUND SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS, G-1-plants A-2-4-2-5[A-2-5] A-2-4-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-	COMPRESSIBILITY	NDN-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-0 A-1-b A-2-4 A-2-5 A-2-6 A-2-7 77-5 A-3 A-6, A-7	SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50	ROCK (NCR)  SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SANDSTONE, ETC.  CDASTAL PLAIN  CDASTAL PLA	CDLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
2 PASSING	HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANOSTONE, CEMENTED	CORE RECOVERY IREC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
# 10 50 HX GRANULAR SILT- MUCK,	PERCENTAGE OF MATERIAL  ORGANIC MATERIAL  GRANULAR SILT - CLAY  OTHER MATERIAL	CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
■ 40 38 MX 56 MX 51 MN	TRACE OF ORGANIC MATTER SOILS STILLS	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING BOCK PINCS UNDER	RICKS OR CUTS MASSIVE ROCK.
LIQUID LIMIT 48 MX 43 MN 48 MX 43 MN 48 MX 41 MN 48 MX 41 MN 800 0 1170	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX 10 MX MODERATE OR HIGHLY	HIGHLY ORGANIC	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF DPEN, (V SLIJ) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	<u>DIP DIRECTION (DIP AZIMUTH) -</u> THE DIRECTION OR BEARING OF THE HDRIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
USUAL TYPES STORE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY ORGANIC MATTER  SOILS SOILS SOILS MATTER	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER ORILLING	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLIL) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELOSPAR CRYSTAN SAFE DILL AND DESCRIPTION OF CONTAIN THE RESULT OF THE PROPERTY OF THE	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
GEN. RATING	— ·	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.  MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLDRATION AND WEATHERING EFFECTS. IN	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.  FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
AS A EXCELLENT TO GODD FAIR TO POOR POOR POOR UNSUITABLE	TENORED WATER, SHIORHIED ZORE, OR WHIER BERKING STRATA	(MOD.)  GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
P1 OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP	MITH FRESH ROCK.  MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
CONSISTENCY OR DENSENESS  COMPACTIVES OR RANGE OF STANDARD RANGE OF UNCONFINED	MISCELLANEOUS SYMBOLS	I SEVERE AND DISCOLORED AND A MAJORITY SHOW KADIINIZATION ROCK SHOWS SEVERE LOSS OF STREAMEN	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN
PRIMARY SOIL TYPE COMPACINESS OR CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (N-VALUE) (TONS/F12 )	ROADWAY EMBANKMENT (RE)  WITH SOIL DESCRIPTION  FOR CAPTURE OF THE TEST BORING  DESIGNATI  DESIGNATI	TE TECTED WOULD VIELD COT DESCRIPTION	THE FIELD.  JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
GENERALLY VERY LODSE (4	S - BULK SAMPLE  AUGER BORING  S - BULK SAMPLE	SEVERE ALL ROCK EXCEPT OUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
MATERIAL MEDIUM DENSE 10 TO 30 N/A	SS - 9PLIT SPOON ARTIFICIAL FILL (AF) OTHER  ARTIFICIAL FILL (AF) OTHER	EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.  IF TESTED. YIELDS SPY N VALUES > 180 BPF	ITS LATERAL EXTENT.
(NON-COHESIVE)  DENSE  90 TO 50  VERY DENSE  >50	THAN ROADWAY EMBANKMENT ————————————————————————————————————	VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK EARDIC ELEMENTS ARE DISCERNABLE DUT	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS, MOTTLED (MOT.) - TRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN
VERY SOFT	INFERRED SOIL BOUNDARY MONITORING WELL	(V SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT DNLY MINOR	SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.  PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
SILT-CLAY MEDIUM STIFF 4 TO 8 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.	INFERRED ROCK LINE  A PIEZOMETER  AS - HUCK SAMPLE	VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF	INTERVENING IMPERVIOUS STRATUM.
MATERIAL STIFF B TO 15 1 TO 2  (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	TTT ALLUVIAL SOIL BOUNDARY INSTALLATION RI - HECUMPACTER  SAMPLE  SLOPE INDICATOR	TRIAXIAL COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE DNLY IN SMALL AND SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
HARD >30 >4	25/025 DIP & DIP DIRECTION DF INSTALLATION CBR - CALIFORNIA	BEARING ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND
TEXTURE OR GRAIN SIZE	SPT N-VALUE	ROCK HARDNESS  VERY HARD CANNOT BE SCRATCHED BY KNIFE DR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	EXPRESSED AS A PERCENTAGE.  SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	SOUNDING ROD     REF  SPT REFUSAL	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	PARENT ROCK.
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY (BLDR.) (COB.) (GR.) (CSE.SD.) (F. CD.) (SL.) (CL.)	ABBREVIATIONS  AR - AUGER REFUSAL HI HIGHLY # - MOISTURE CON	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED  ENT  10 DETACH HAND SPECIMEN.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH 11S LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUCED ROCKS.
GRAIN MM 305 75 2.0 0.25 0.05 0.005 STZE IN. 12 3	BT - BORING TERMINATED MED MEDIUM V - VERY CL CLAY CL CLAY CPT - CONE PENETRATION TEST MOD MODERATELY MEA WEATHERED	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE TEST HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT REGULTS FROM FRICTION ALONG A FAULT DR SLIP PLANE.
SOIL MOISTURE - CORRELATION OF TERMS	CSE COARSE NP - NON PLASTIC 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF MALES OR BLCV POINT	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SOIL MOISTURE SCALE FIELD MOISTURE CHIPS FOR SITE P MOISTURE PERSONNELL	DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST	POINT OF A GEOLOGIST'S PICK.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS
(ATTERBERG LIMITS) DESCRIPTION GUIDE FOR FIELD MOISTORE DESCRIPTION	• - VOID RATIO SAP SAPROLITIC F - FINE SD SAND, SANDY	SUFT CAN BE BROVED ON BUGGED READILT BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	THAN 0.1 FOOT PER 60 BLOWS.  SIRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	FOSS FOSSILIFEROUS SL SILT, SILTY FRAC FRACTURED, FRACTURES SLI SLIGHTLY	PIECES CAN BE BROKEN BY FINGER PRESSURE.	OF STRAIDM AND EXPRESSED AS A PERCENTAGE.
PLASTIC LIDUID LIMIT	FRAGS FRAGMENTS TCR - TRICONE REFUSAL	VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	<u>STRATA ROCK QUALITY DESIGNATION (SROD) -</u> A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE
RANGE SEMISOLID: REDURES DRYING TO  (PI) STATE OF THE PROPERTY	EQUIPMENT USED ON SUBJECT PROJECT	FINGERNAIL. FRACTURE SPACING BEDDING	IDIAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  IDPSDIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
" PLL _ PLASTIC LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	TERM SPACING IERM THICKNESS	
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	AUTOMATIC [V]	MANUAL VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED > 4 FEET	BENCH MARK:
SL SHRINKAGE LIMIT	6' CONTINUOUS FLIGHT AUGER	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	ELEVATION: FT.
- DRY - (D) ATTAIN OPTIMUM MOISTURE	BK-51  B'-DONINGOUS FLIGHT AUGER  CORE SIZE:	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	NOTES:
PLASTICITY	X CME-45C X HARD FACED FINGER BITS -N	INDURATION	
PLASTICITY INDEX (P)) DRY STRENGTH NONPLASTIC 9-5 VERY LOW	TUNGCARBIDE INSERTS	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
LOW PLASTICITY 6-15 SLIGHT	CME-550	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS: GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH POST HOLE DIGGE	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE:	
COLOR	TRICONE TUNG,-CARB, HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	
		SAMPLE BREAKS ACROSS GRAINS.	

REVISED 02/23/06

PROJECT REFERENCE NO. 34802.I.I (U-24I2A)

SHEET NO.

2

**JAMESTOWN** HIGH POINT 412 BEGIN PROJECT U-2412A N END PROJECT U-2412A OIE VICINITY MAP K • • • DETOUR FOR OAKDALE ROAD BRIDGE CONSTRUCTION STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

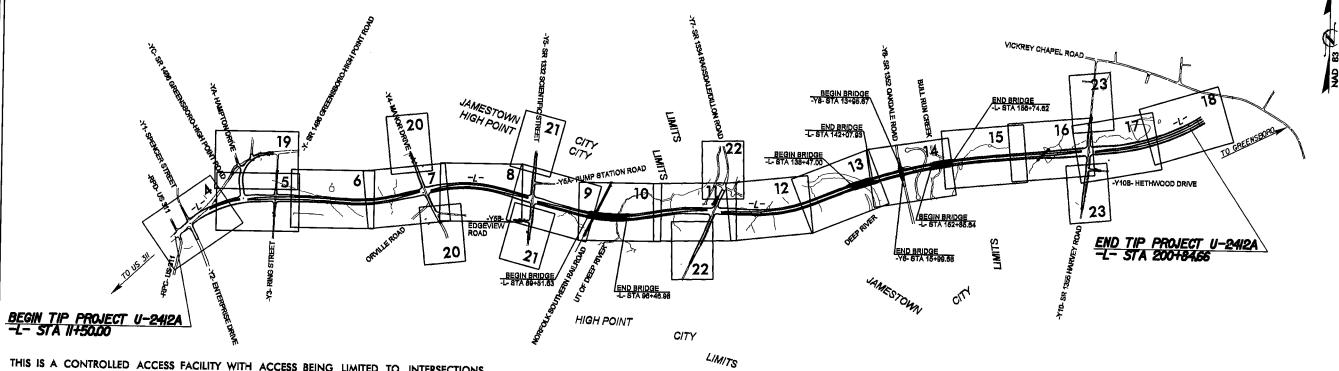
N.C. U-2412A 2A 90 P.A.PROLNO. 34802.1.1 STP-4121(1)

GREENSBORO/HIGH POINT ROAD FROM PROPOSED US 311 BYPASS LOCATION:

TO WEST OF SR 1480 (VICKREY CHAPEL ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, STRUCTURES, AND GUARDRAIL

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION INCOMPLETE PLANS
DO NOT USB FOR R/W ACQUISITION



THIS IS A CONTROLLED ACCESS FACILITY WITH ACCESS BEING LIMITED TO INTERSECTIONS. THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARIES OF HIGH POINT AND JAMESTOWN. CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III NCDOT CONTACT: B. DOUG TAYLOR, P.E. ENGINEERING COORDINATION, ROADWAY DESIGN UNIT

# GRAPHIC SCALES PROFILE (HORIZONTAL)

DESIGN DATA

ADT 2011 31,530 ADT 2031 = 48,870 11% 55% 3% ' 60 MPH ' (TTST 1% + DUAL 2%)

FUNC CLASS: ARTERIAL

### PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT U-2412A = 3.312 MILES LENGTH STRUCTURES TIP PROJECT U-2412A = .274 MILES TOTAL LENGTH OF TIP PROJECT U-2412A = 3.586 MILES

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RIGHT OF WAY DATE: FEBRUARY 20, 2009

LETTING DATE: OCTOBER 18, 2011 GLENDA M. GIBSON, PE

MICHAEL PEKAREK, PE

HYDRAULICS ENGINEER

ROADWAY DESIGN **ENGINEER** 

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

STATE HIGHWAY DESIGN ENGINEER



## STATE OF NORTH CAROLINA

### DEPARTMENT OF TRANSPORTATION

Michael F. Easley GOVERNOR

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Lyndo Tippett Secretary

November 20, 2008

STATE PROJECT:

34802.1.1 (U-2412A)

FEDERAL PROJECT:

STP-4121(1)

COUNTY:

Guilford

DESCRIPTION:

SR 4121 (Greensboro-High Point Road) from proposed US 311 Bypass to West

of SR 1480 (Vickery Chapel Road)

SUBJECT:

Geotechnical Report - Inventory

### **Project Description**

This project consists of a new four lane roadway (-L-, SR 4121, Greensboro-High Point Road) beginning to the east of High Point, and passing south of Jamestown. The project begins near the intersection of US 311 and Greensboro-High Point Rd. (SR 4121) south of High Point and extends 3.6 miles eastward toward Jamestown where it meets U-2412B toward Greensboro. The proposed realignment will begin at the intersection of Greensboro-High Point Rd (-L-), and Enterprise Dr. (-Y2-). An intersections is proposed with existing Greensboro-High Point Rd. (-Y-) at -L- 25+69. Additional intersections are planned at Enterprise DR. (-Y1-), Manor Dr. (-Y4-), Scientific St. (SR 1332, -Y5-), Dillon Rd. (SR 1334, -Y7-), and Harvey Rd. (-Y10-). Four Bridges are proposed on the alignment. These bridges will be at: -L- 89+52, over the Norfolk-Southern Railroad and an unnamed tributary to Deep River, -L- 138+47, over Deep River, -L- 152+85, over Bull Run Creek, and -Y8- 13+96, over -L-. One retaining wall is planned on -Y7- (Dillon Rd.) at 18+50.

The geotechnical field investigation was conducted during September 2008. Two Trigon/Kleinfelder drill crews were contracted to assist in investigating the subsurface. NCDOT Geotechnical Engineering Unit geologists sampled and logged the borings. ATV-mounted CME-45, and B-57 drill machines were used during field investigation. Standard Penetration Tests were performed in selected borings and additional borings were advanced using continuous flight augers. Representative soil samples were collected for visual classification in the field and submitted for laboratory analysis by NCDOT's Materials and Tests Unit.

The following alignments, totaling 5.0 miles, were investigated. Subsurface soil profiles, or cross-sections, of these alignments are included in this report:

<u>Line</u>		Station	
-L-	11+50	to	201+65
-Y-	10+00	to	22+00
-Y3-	10+50	to	19+00
<b>-</b> Y5-	10+25	to	26+00
<b>-</b> Y7-	17+00	to	31+75

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-Y8-	12+25	to	17+50
-Y10-	14+65	to	32+00
-Y10B-	10+00	to	12+50

### Areas of Special Geotechnical Interest

1) <u>Highly Plastic Clay Soils</u>: Occurrences of highly plastic clay soil (Plasticity Index greater than 25) are noted below:

<u>Alignment</u>	<u>Station</u>	Offset
-L-	18+75	65 RT
-L-	29+00	$\operatorname{CL}$
-L-	33+50	$\operatorname{CL}$
-L-	36+50	$\operatorname{CL}$
-L-	54+00	CL
-L-	58+50	$\operatorname{CL}$
-L-	61+50	$\operatorname{CL}$
-L-	64+00	$\operatorname{CL}$
<b>-</b> L-	67+00	CL
<b>-</b> L-	73+00	CL
<b>-</b> L-	75+50	$\operatorname{CL}$
-L-	78+00	80 RT
<b>-</b> L-	87+00	CL
<b>-</b> L-	90+00	20 RT
<b>-</b> L-	102+50	80 RT
<b>-</b> L-	104+00	$\operatorname{CL}$
<b>-</b> L-	106+50	$\operatorname{CL}$
-L-	111+50	$\operatorname{CL}$
-L-	152+80	55 RT
-L-	166+00	CL
-L-	167+50	CL
-L-	179+00	100 RT
-L-	183+00	$\operatorname{CL}$
<b>-</b> L-	186+00	$\operatorname{CL}$
<b>-</b> L-	194+50	$\operatorname{CL}$
-L-	197+00	$\mathbf{CL}$
-Y-	12+00	$\operatorname{CL}$
-Y-	15+00	55 LT
-Y3-	17+50	25 LT
-Y5-	15+00	40 RT
-Y5-	22+00	35 RT
-Y7-	20+00	42 RT

2) <u>Crystalline Rock</u>: Crystalline rock was encountered in the following continuous intervals:

Alignment	Station		
-L-	126+00	to	130+50
-L-	143+50	to	149+00
-L-	161+00	to	166+00

176+50

50 to 177+50

Additionally, crystalline rock was encountered in the following borings:

Alignment	Station	Offset
-L-	37+90	50 LT
-L-	40+30	35 RT
<b>-</b> L-	84+00	CL
-L-	90+00	$\operatorname{CL}$
-L-	106+50	$\operatorname{CL}$
<b>-</b> L-	109+00	80 RT
-L-	111+50	CL
-L-	116+50	40 LT
-L-	122+00	CL
-L-	138+00	20 LT
-L-	142+50	20 RT
-L-	152+80	55 RT
-L-	156+90	55 LT
-L-	160+00	CL
-L-	173+50	25 LT
-L-	199+00	CL

3) Artificial Fill: Artificial fill soil occurs in a landfill at the following location:

Alignment	<u>Station</u>		
-L-	81+25	to	85±00

Additionally, Artificial Fill was encountered in the following borings:

Alignment	<b>Station</b>	Offset
-L-	130+00	CL
-L-	130+50	50 RT
-L-	131+30	CL
-L-	142+50	20 RT

4) <u>Shallow Groundwater</u>: Shallow groundwater, which may cause problems during construction, was encountered in the following area:

<u>Alignment</u>	<u>Station</u>		
-L-	165±50	to	167+50

### Physiography and Geology

The project is located in the central Piedmont area of North Carolina. A mixture of woods, pastures, ponds, and agricultural fields are located along the project corridor. Single-family homes and businesses are located adjacent to the corridor. The terrain is moderately rolling with several steep slopes adjacent to several large creeks and Deep River that flow from left to right across the -L- alignment.

The entire project is underlain by metamorphosed granite of the Carolina Slate Belt. This granite is resistant to weathering and is often present very near the ground surface. Much of the rolling terrain along the project is due to north-south trending ridges of crystalline rock.

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### **Soil Properties**

Soils encountered at the project site include artificial fill, alluvial, roadway embankment, and residual soils.

Alluvial soil was encountered in one boring adjacent to a ditch line along -Y10-. This soil is present beneath proposed roadway embankment, and consists of gray, moist to wet, soft, silty clay (A-7-6).

Roadway embankment soil is present beneath the existing -L- and-Y- alignments. The embankment soil is generally less than three feet in thickness.

Artificial fill soil occurs within a landfill along the -L- alignment. The fill soil was sampled in 3 locations, and consists of 2 to 26 feet of brown, moist, stiff, sandy clay, silty clay and sandy silt (A-6, A-7-6, A-4). The landfill contains some construction debris, such as wood, cobble to boulder size concrete, and metal scraps. Other wood debris from tree stumps was also encountered. Other occurrences of artificial fill, (-L-, 130+00 to 131+50) are in areas of backfill over newly constructed sewer lines. These soils consist of brown, moist, stiff, sandy silt and sandy clay (A-4, A-6)

Residual soils are derived from the in-place weathering of the underlying granitic bedrock and are generally sandy and silty clays with good engineering properties. Silty clay (A-7-6) is the most common soil in the project area. Sandy soils are generally moist, loose to medium dense, silty sand (A-2-4) and coarse sand (A-1-b). Brown and tan, stiff to very stiff, sandy silt (A-4) is also present. Residual, highly plastic "cap" clays occur at the ground surface over several areas of the project. Areas containing highly plastic soils (plasticity indices of greater than 25) are listed above in the section "Areas of Special Geotechnical Interest".

### **Rock Properties**

Weathered rock and crystalline rock occur throughout the project. The weathered rock is derived from the underlying granite bedrock and ranges from inches to 10 feet or more in thickness. Crystalline rock occasionally occurs within five feet or more of the ground surface.

### **Groundwater**

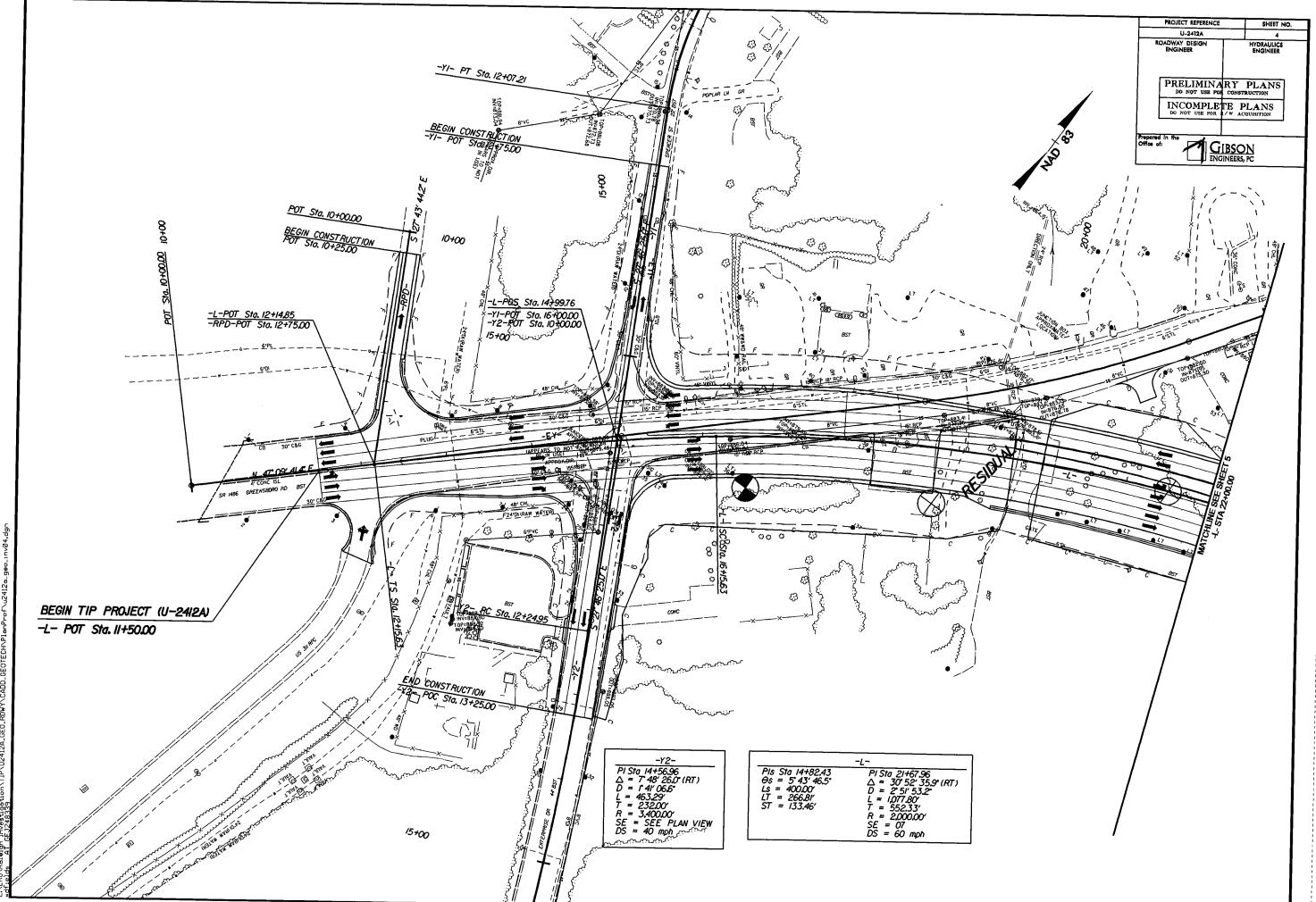
Groundwater was encountered in few borings completed on this project. Groundwater, when encountered, was variable across the project, ranging from 1.1 feet to 28.0 feet below the ground surface. Most groundwater occurrs in low areas which will be covered by roadway embankment during construction. The shallow groundwater located in a cut section at -L- 165+50 to 167+50 may cause problems during construction.

Nathan Mohs, LG

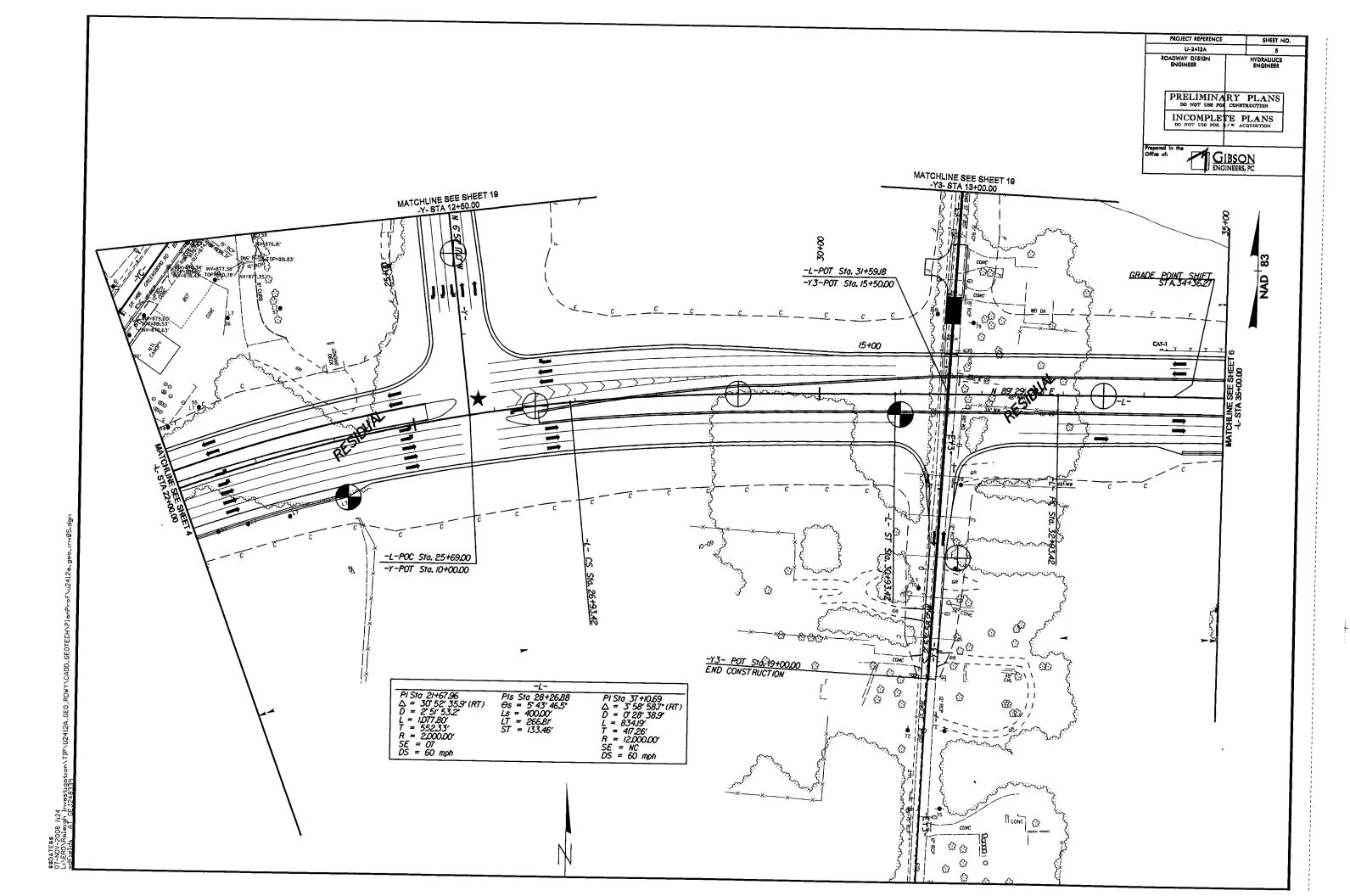
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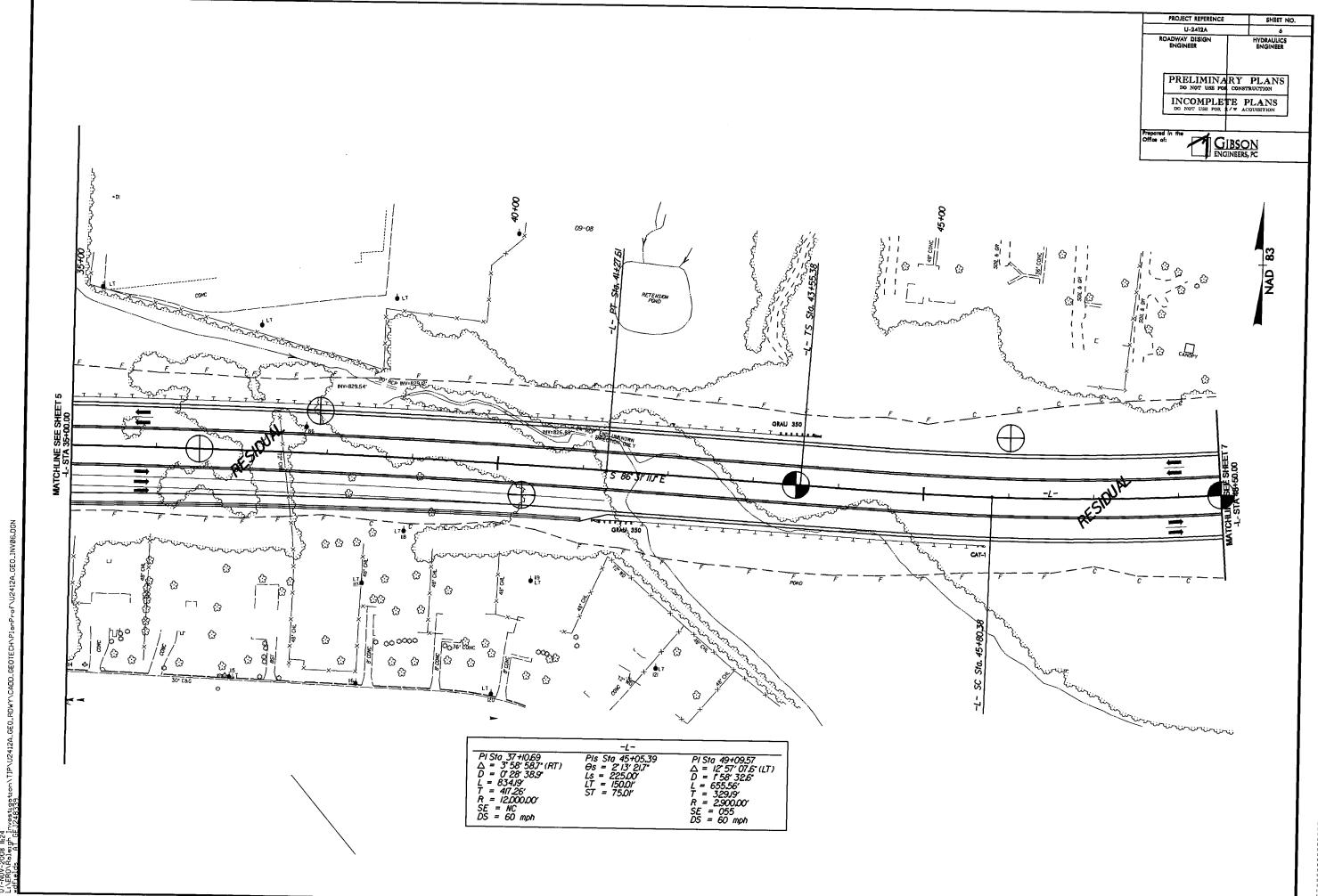
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# EARTHWORK BALANCE SHEET

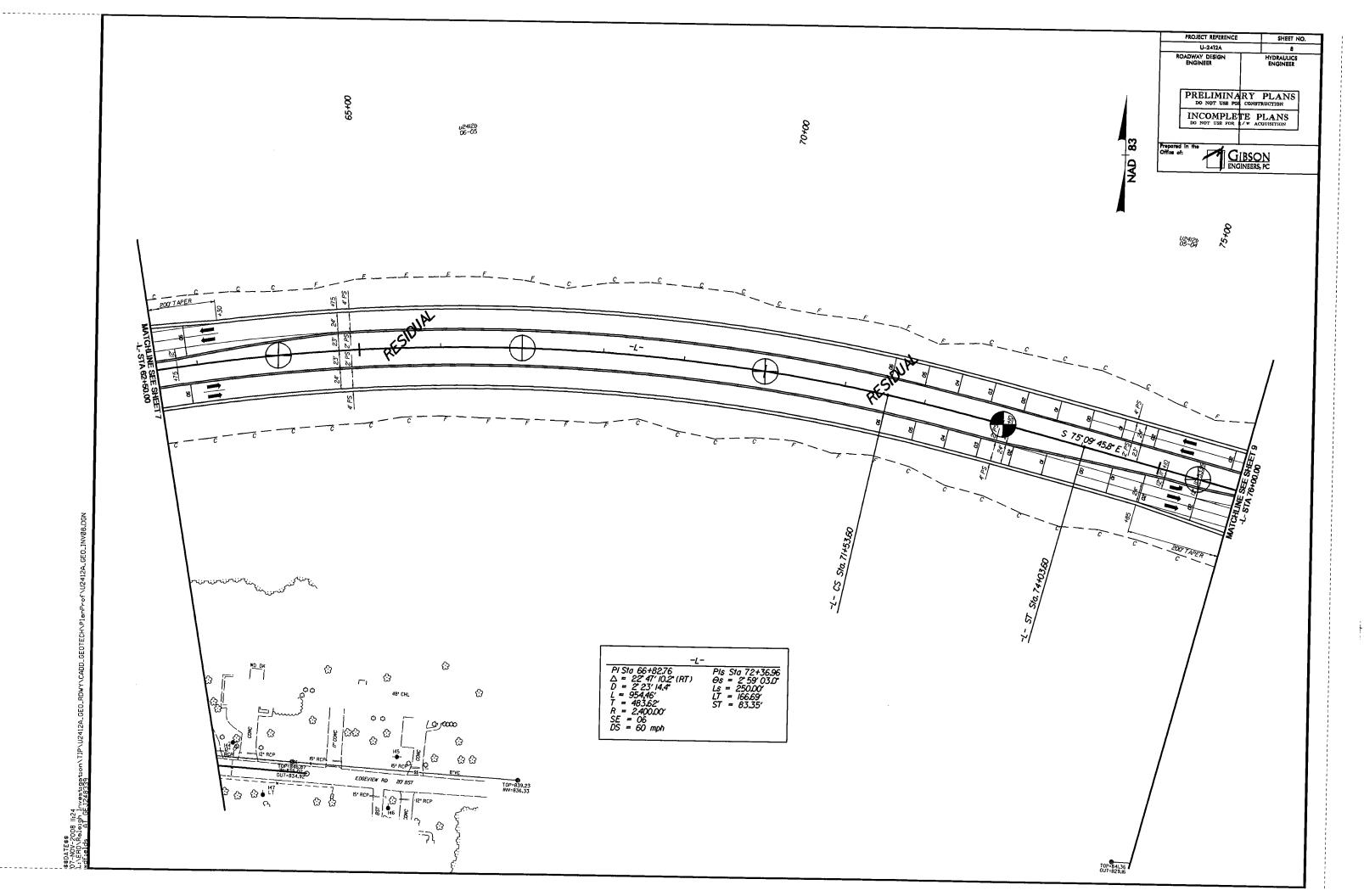


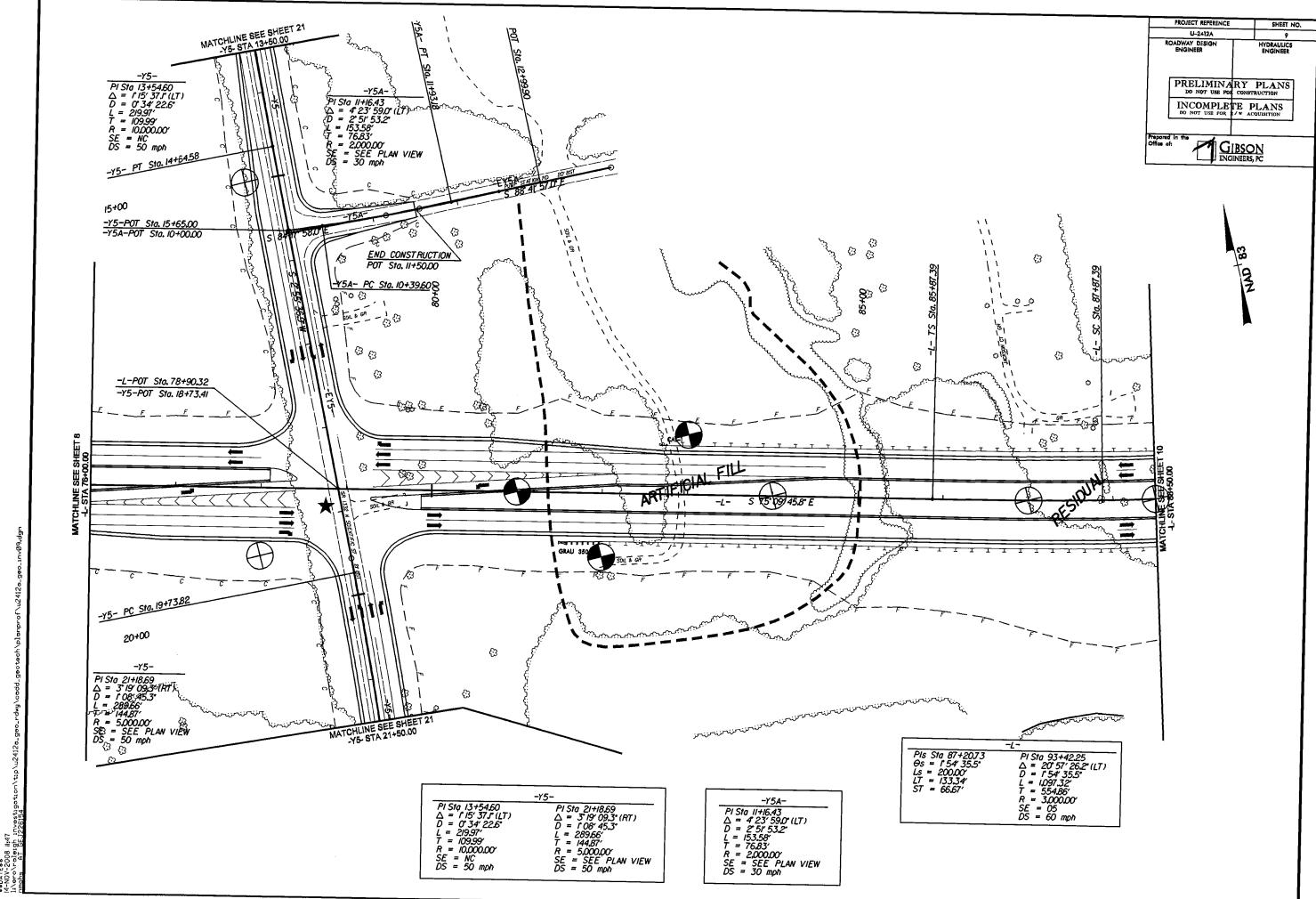
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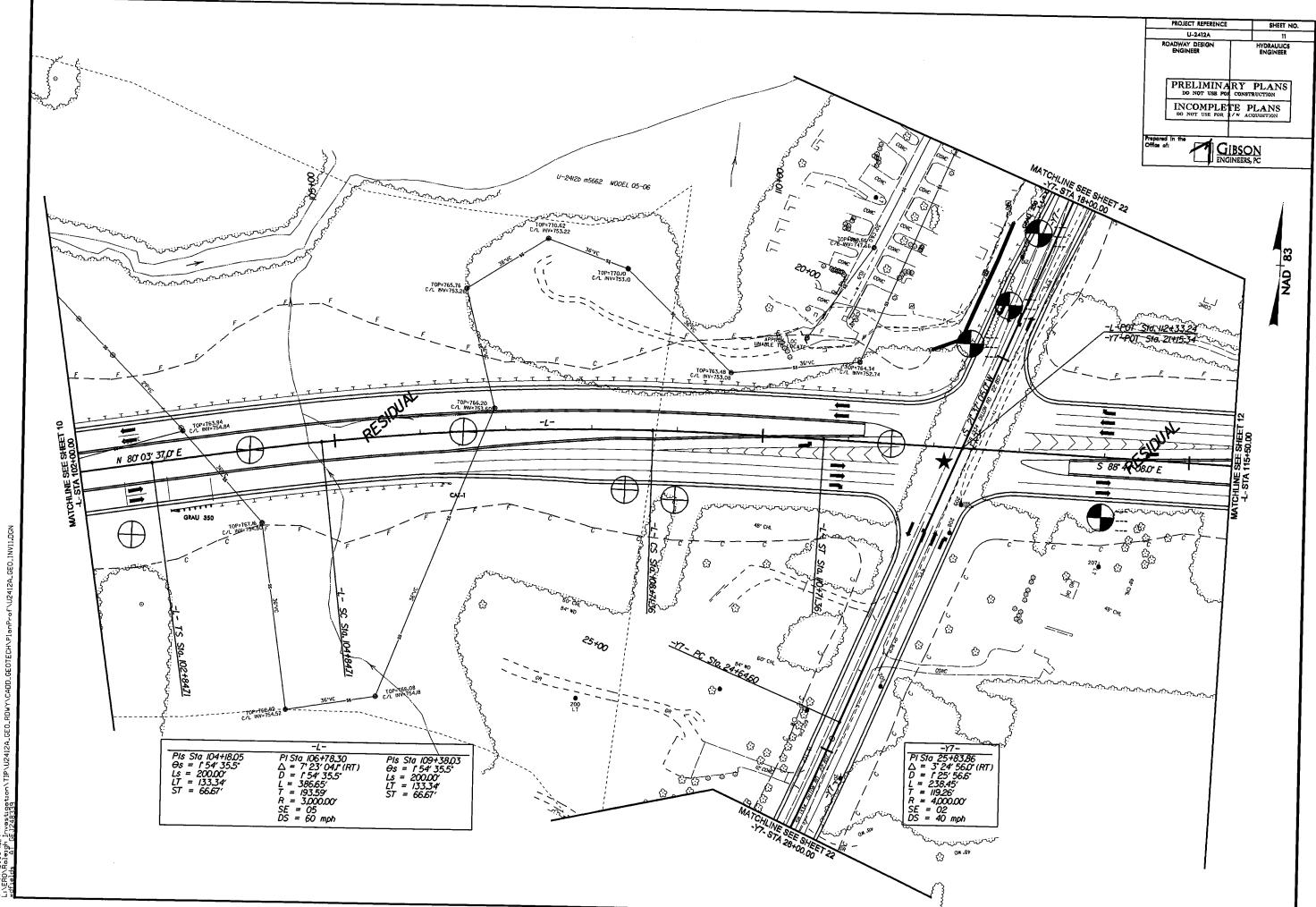


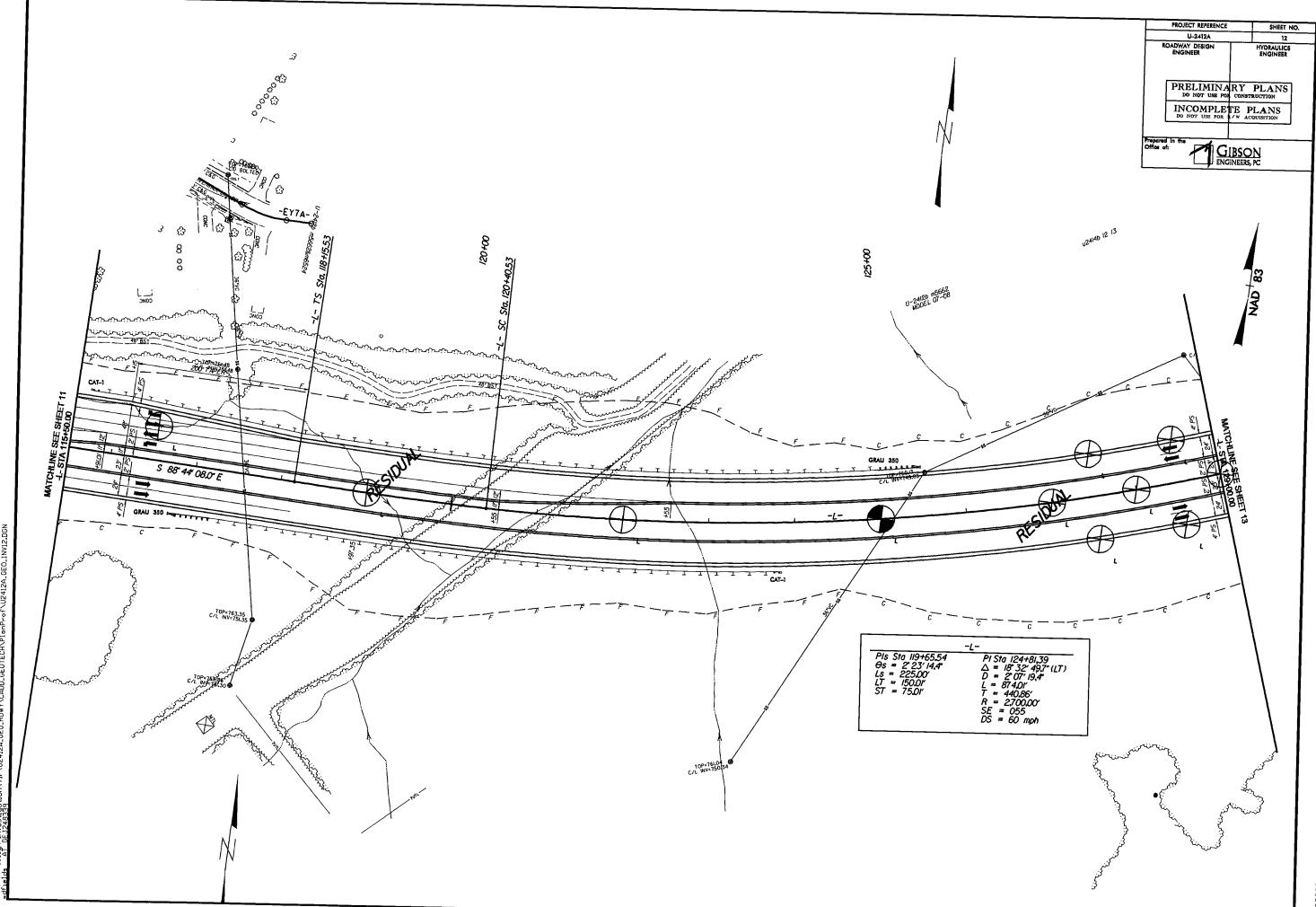


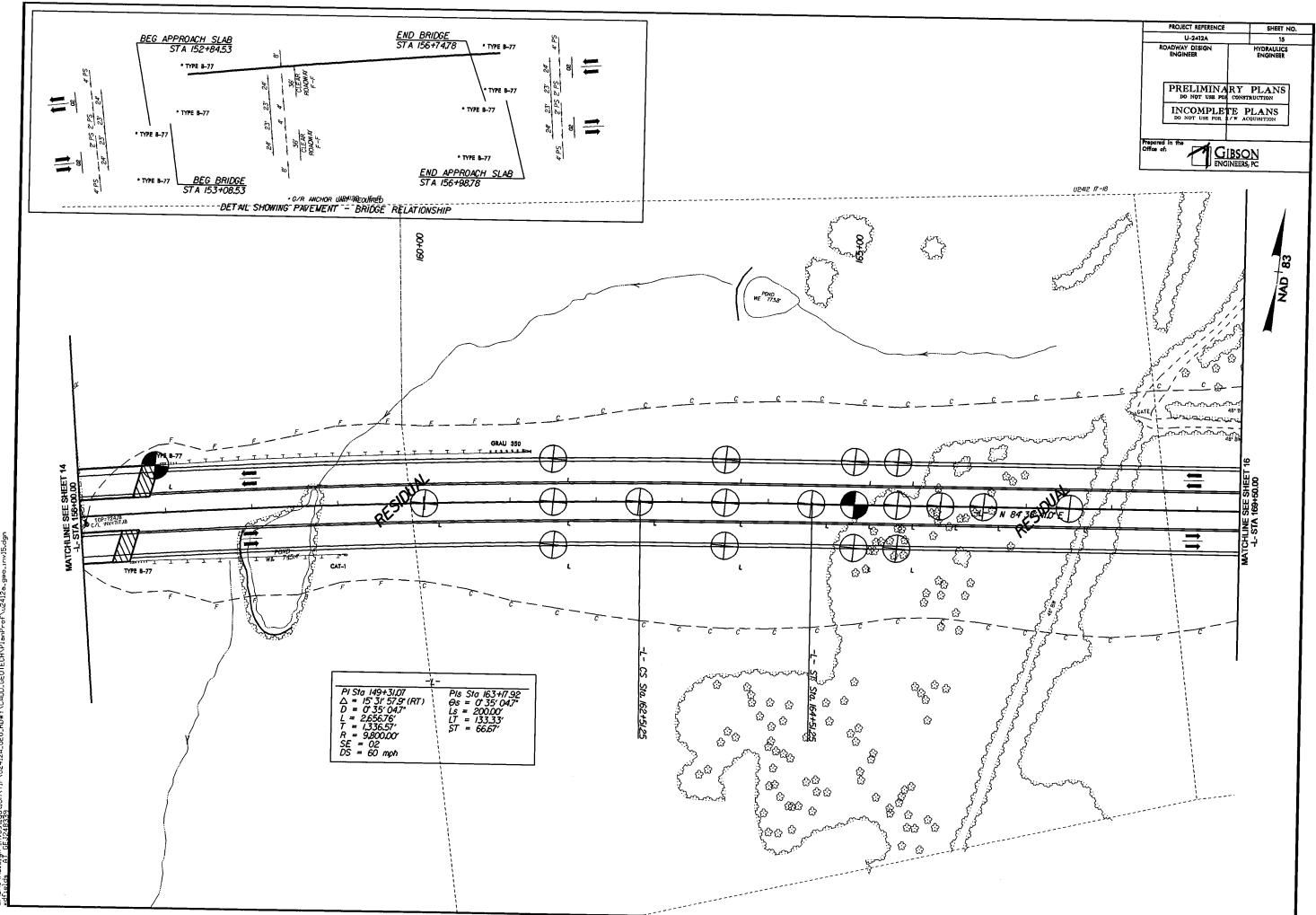
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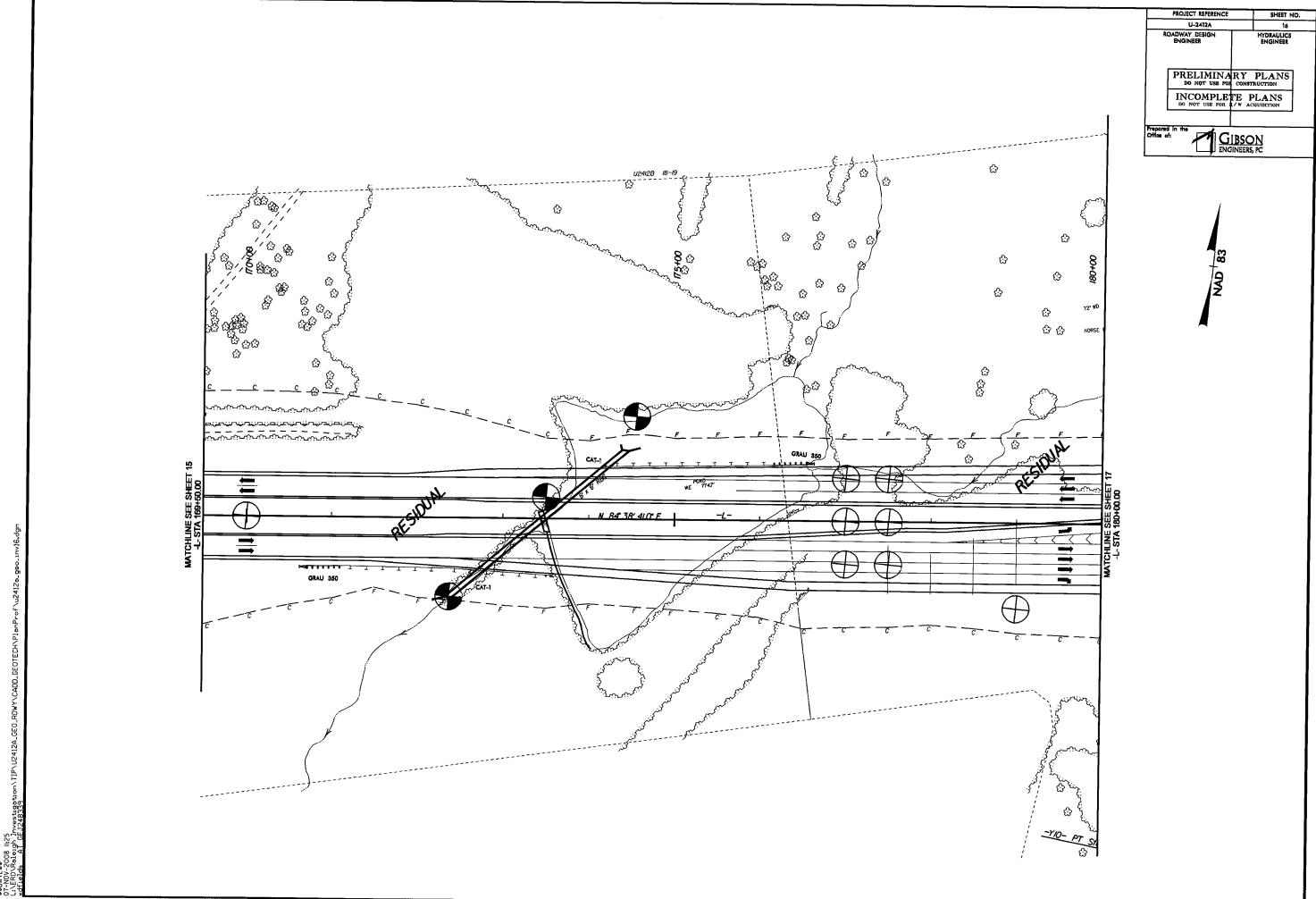


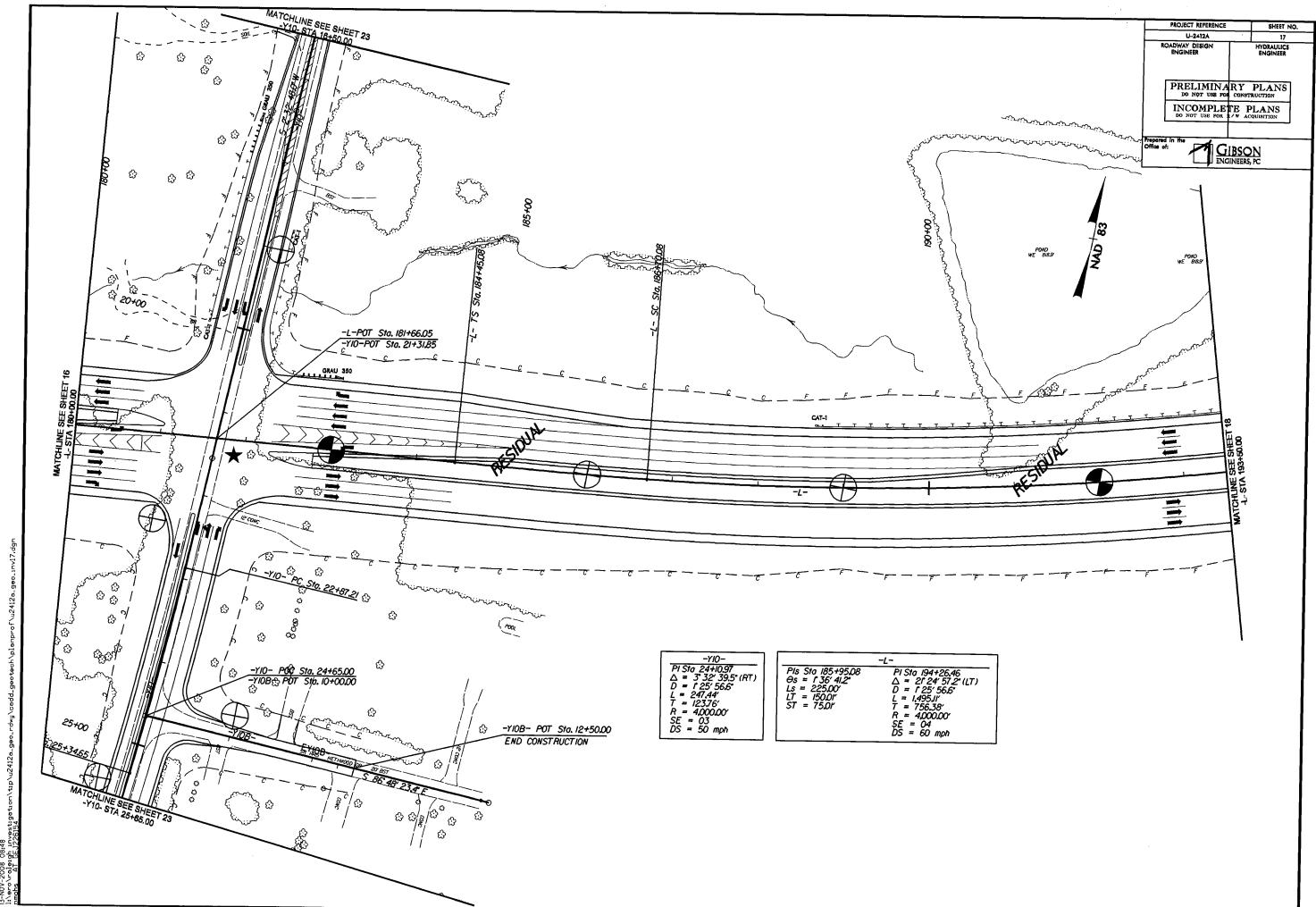


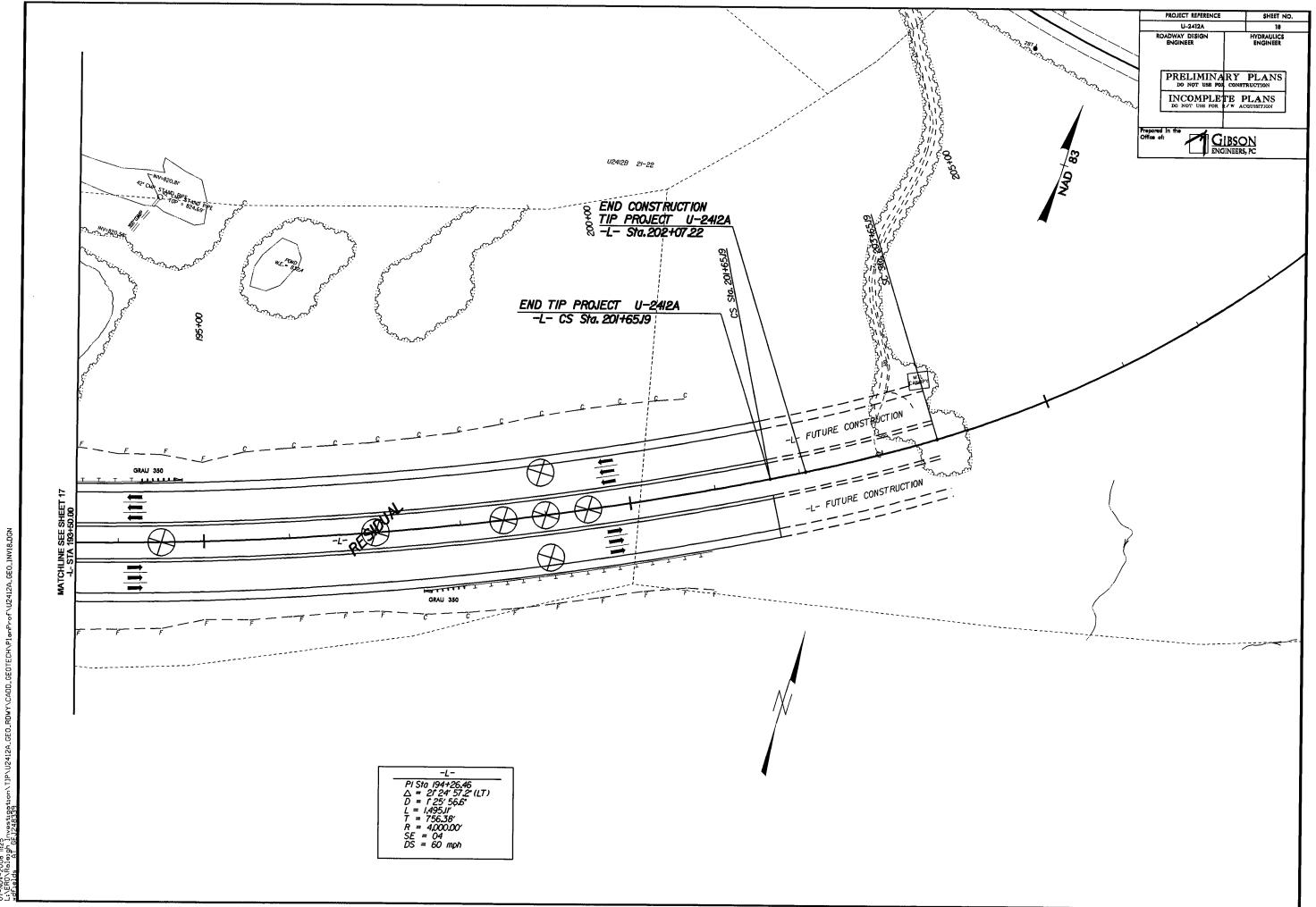


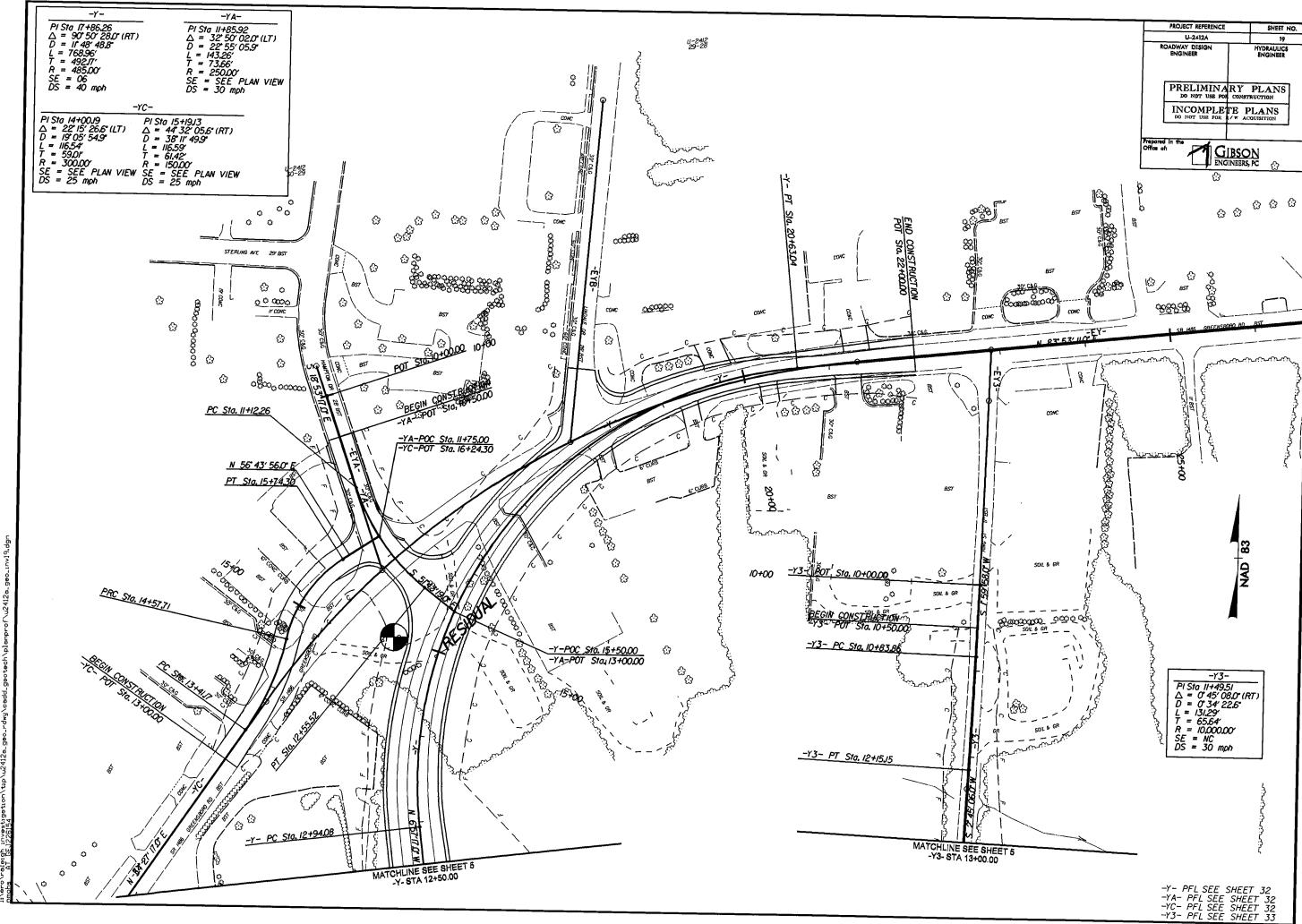
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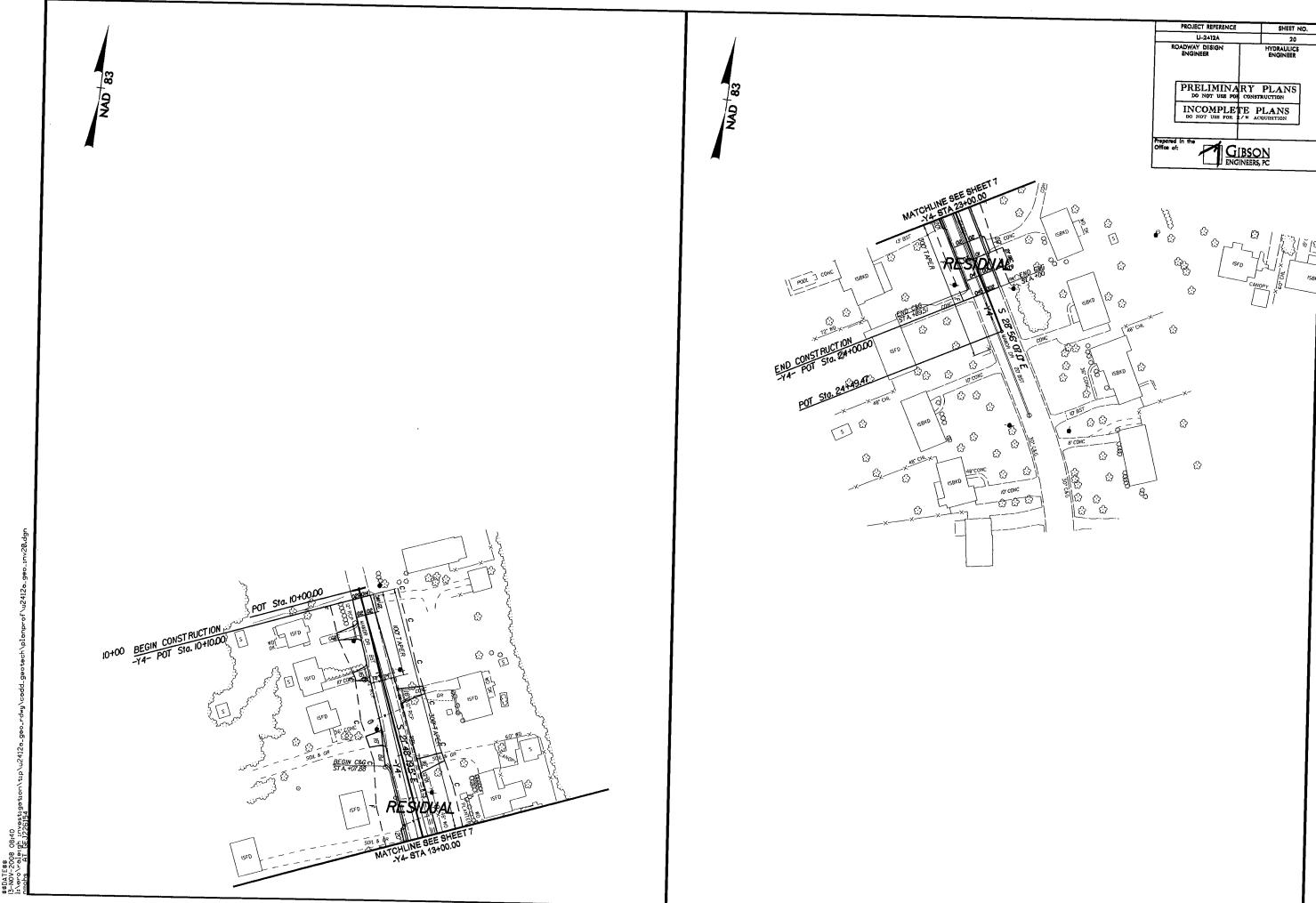
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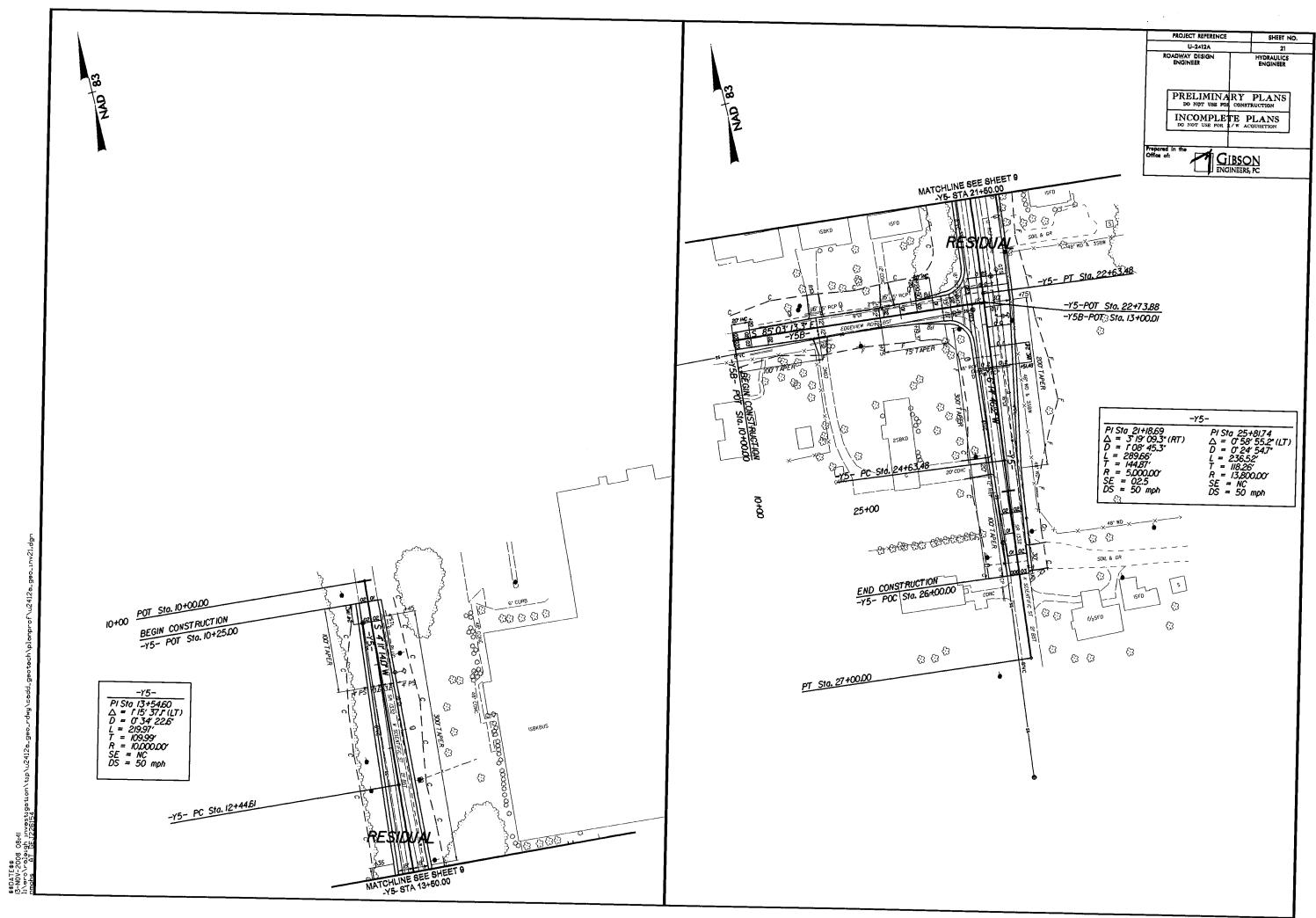


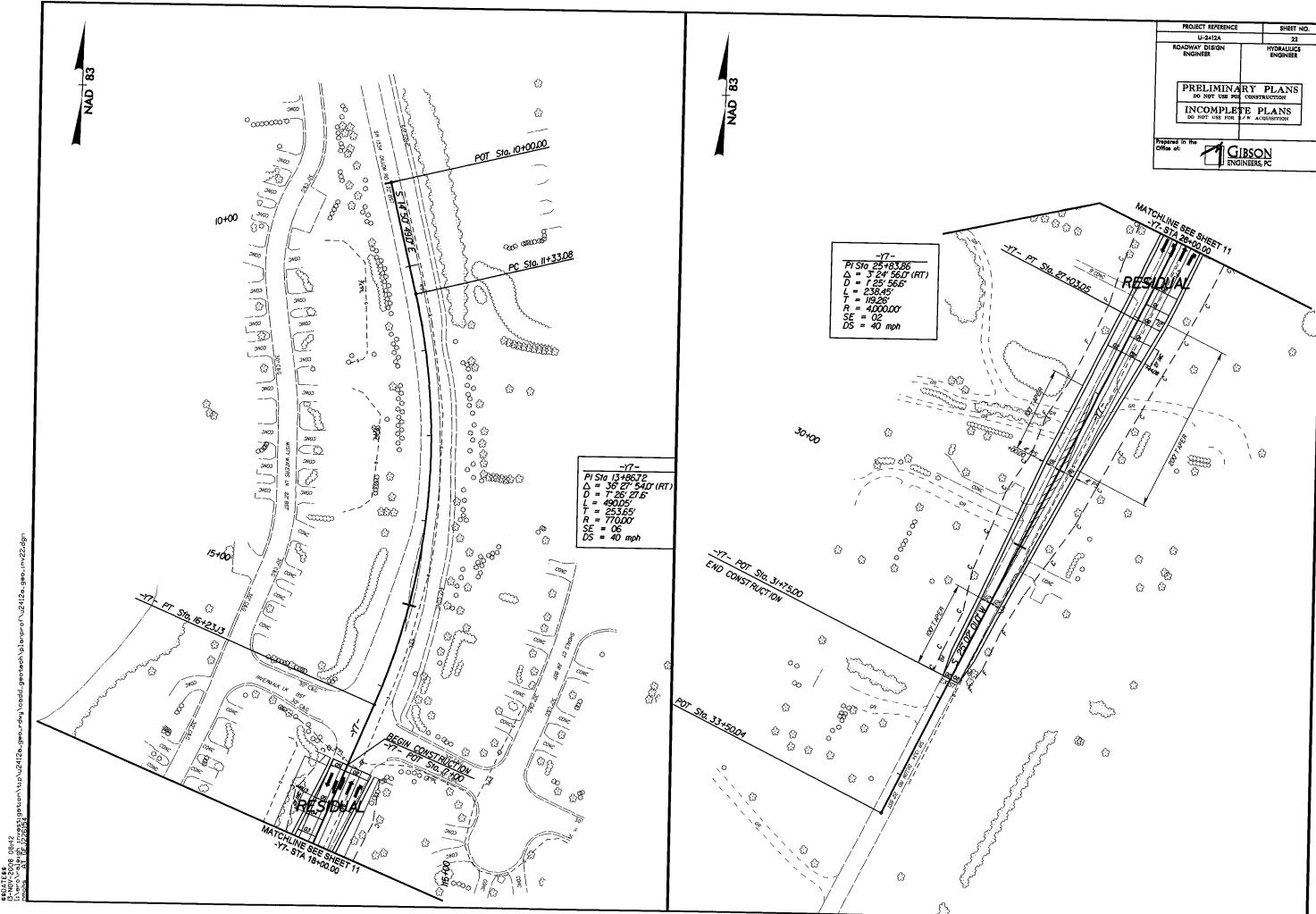


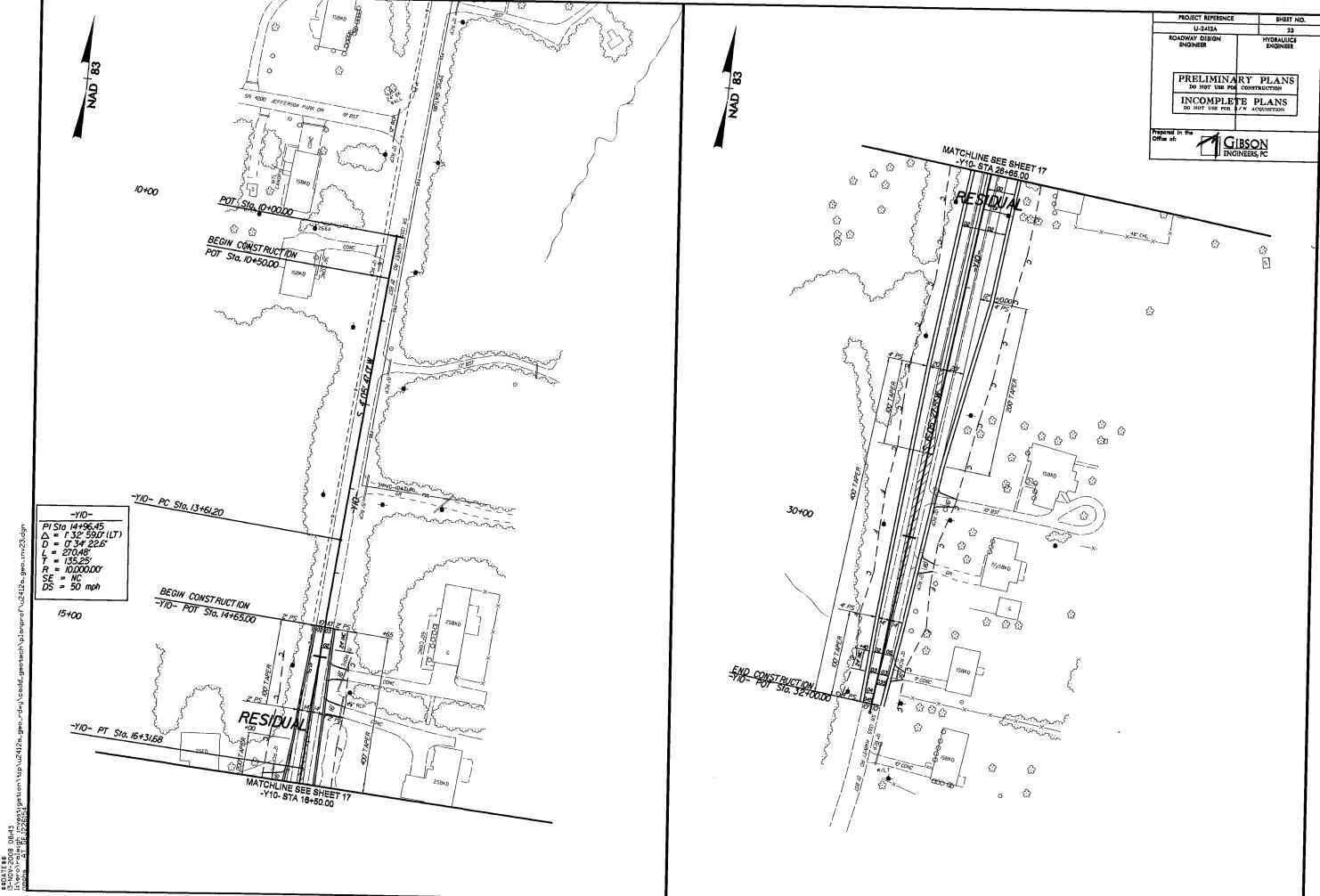


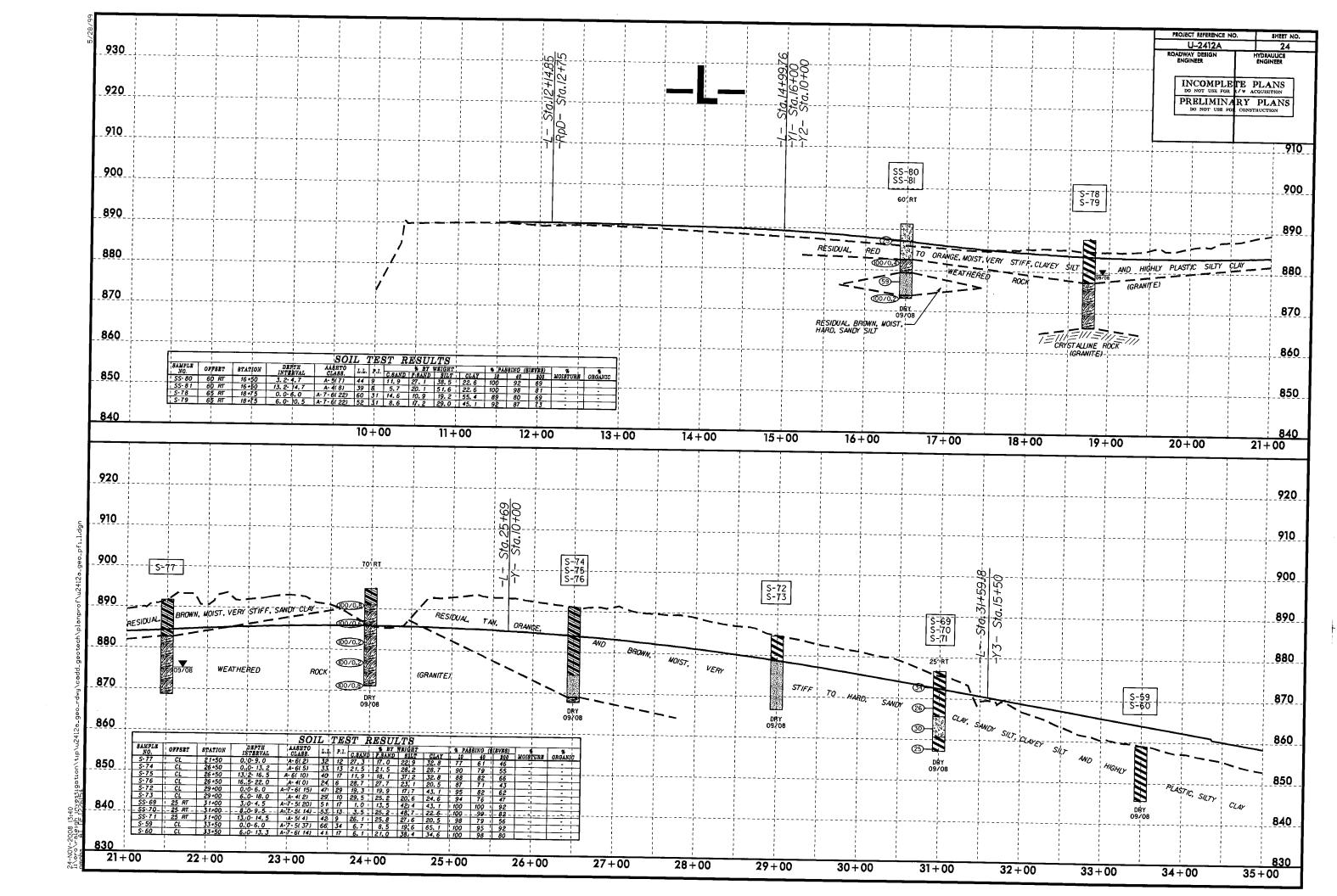


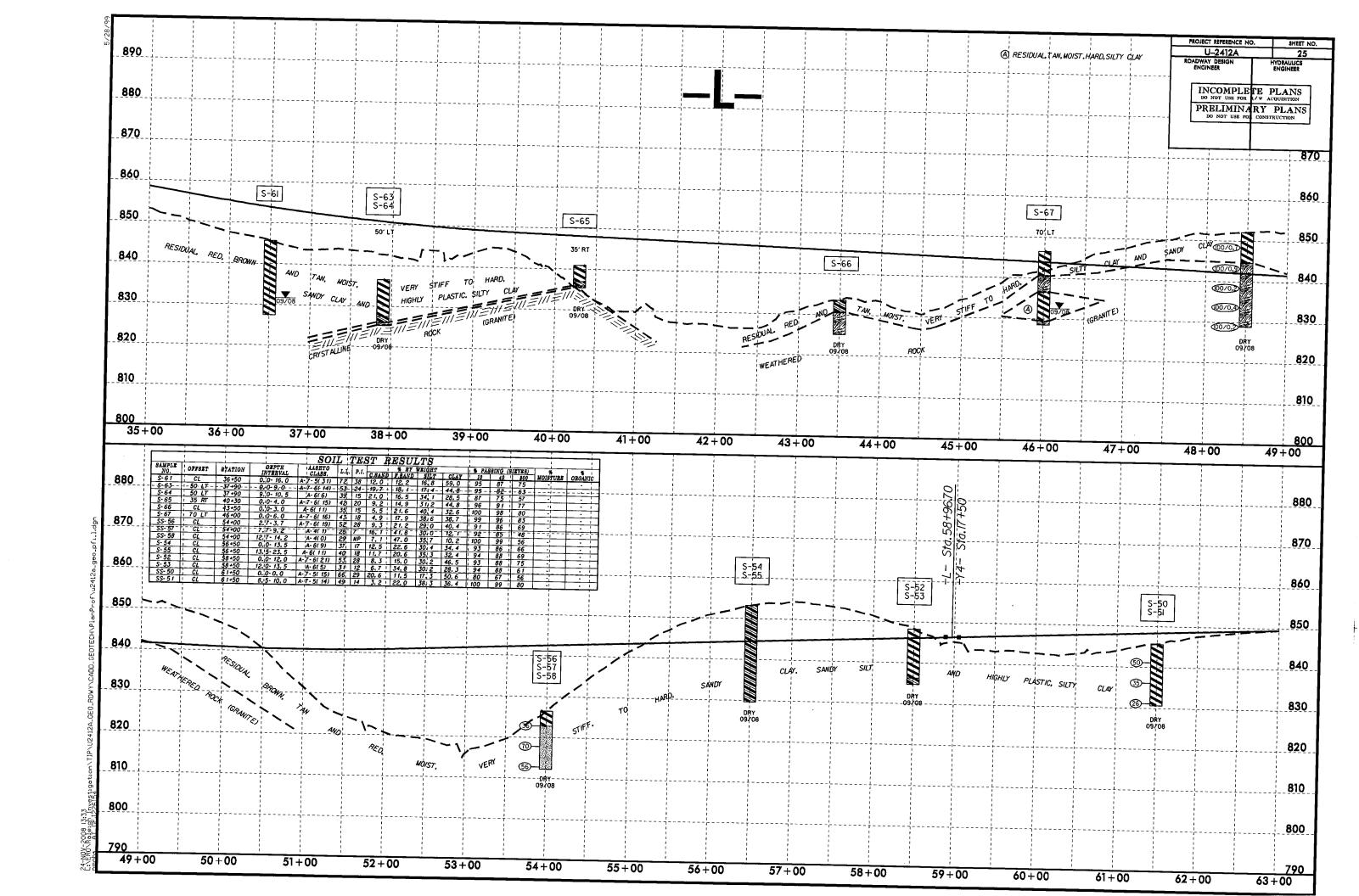


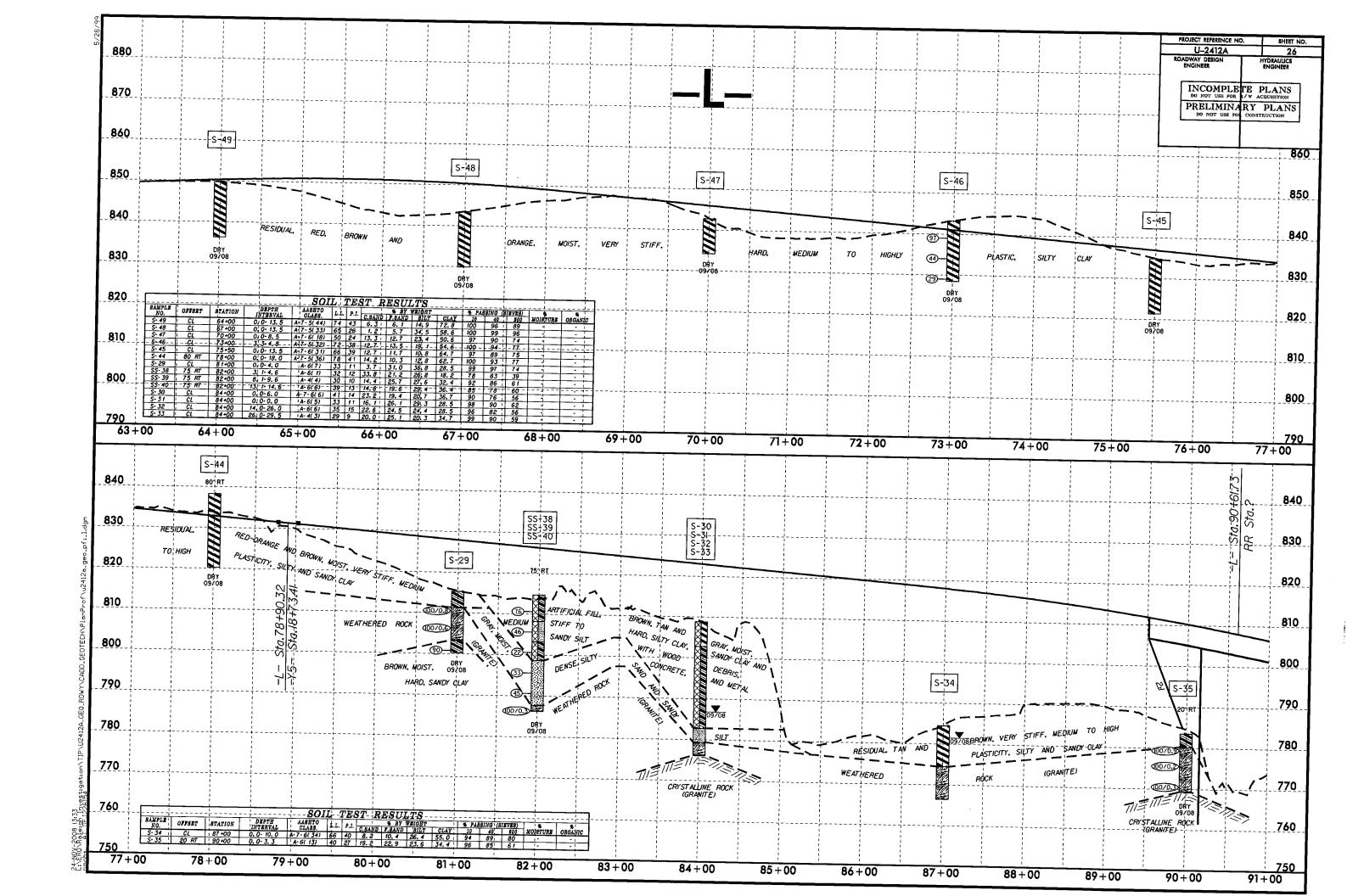


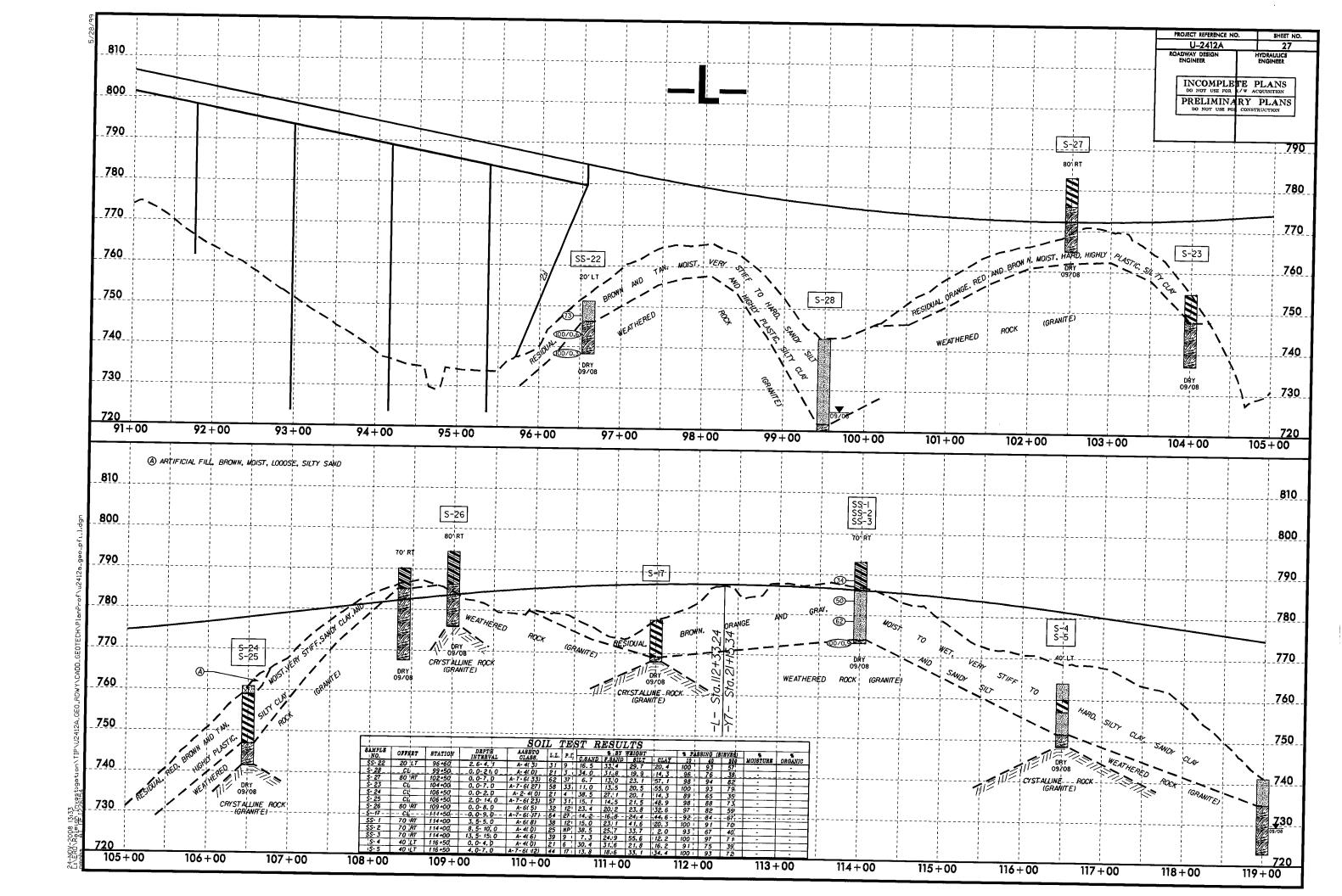


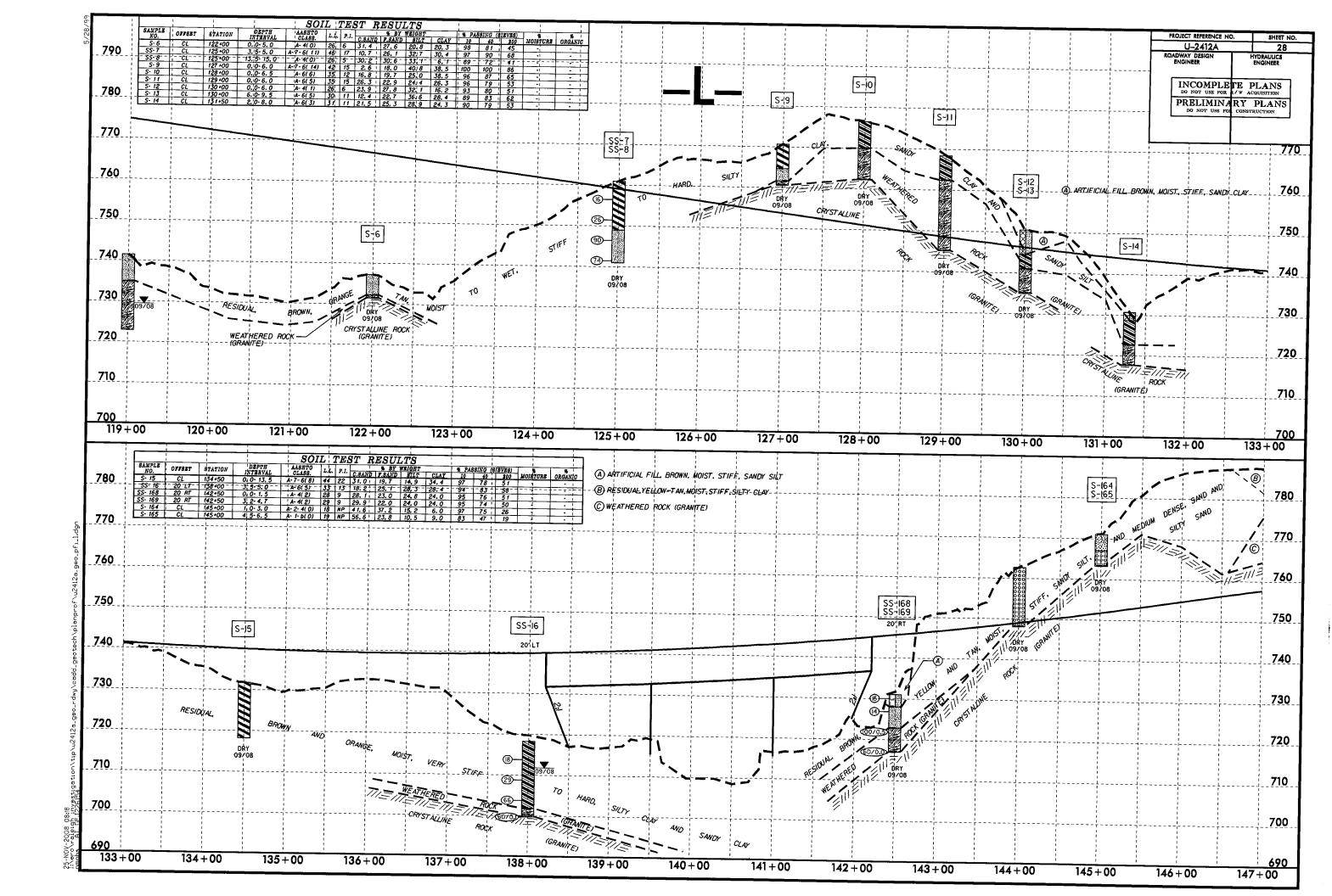


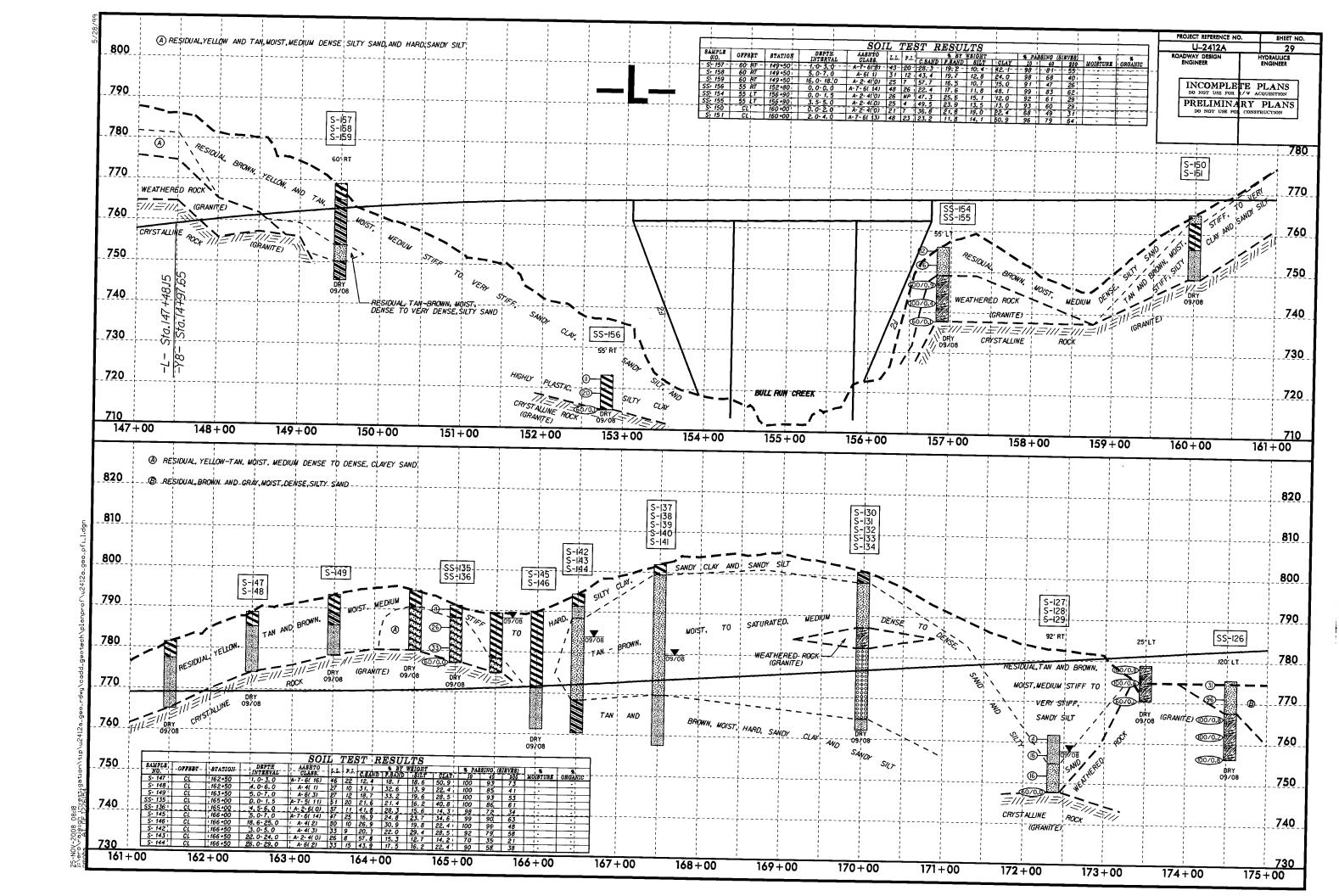


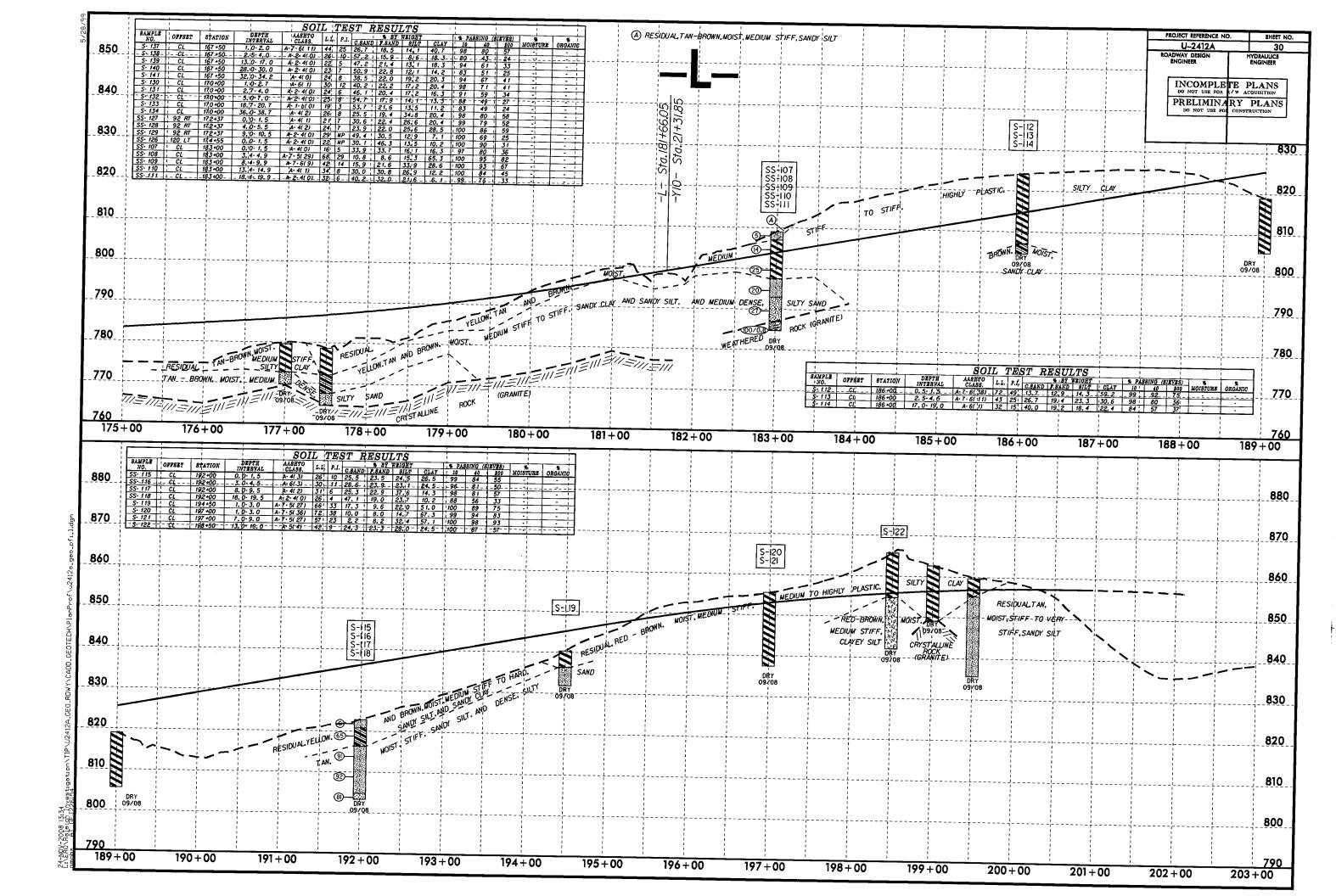


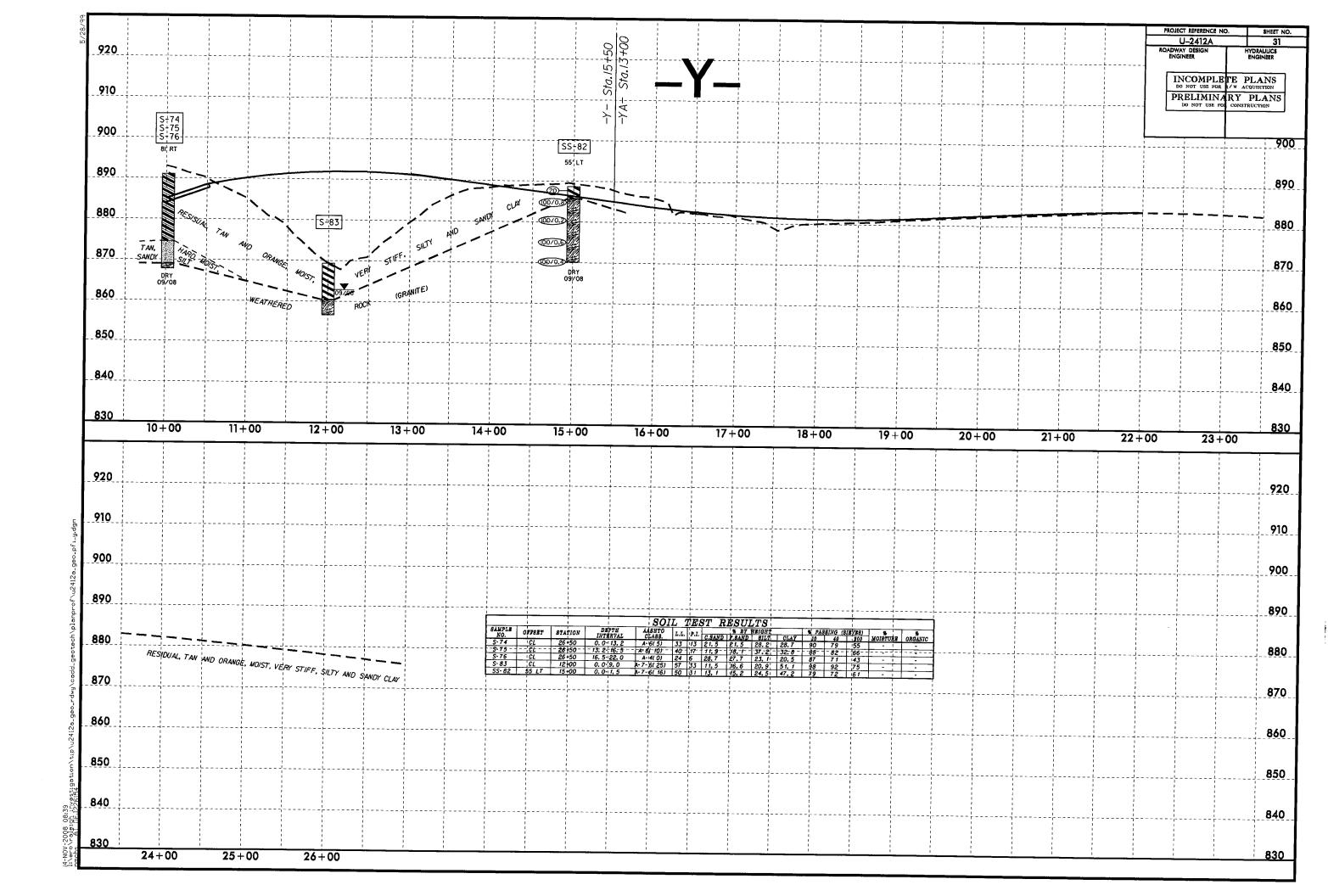


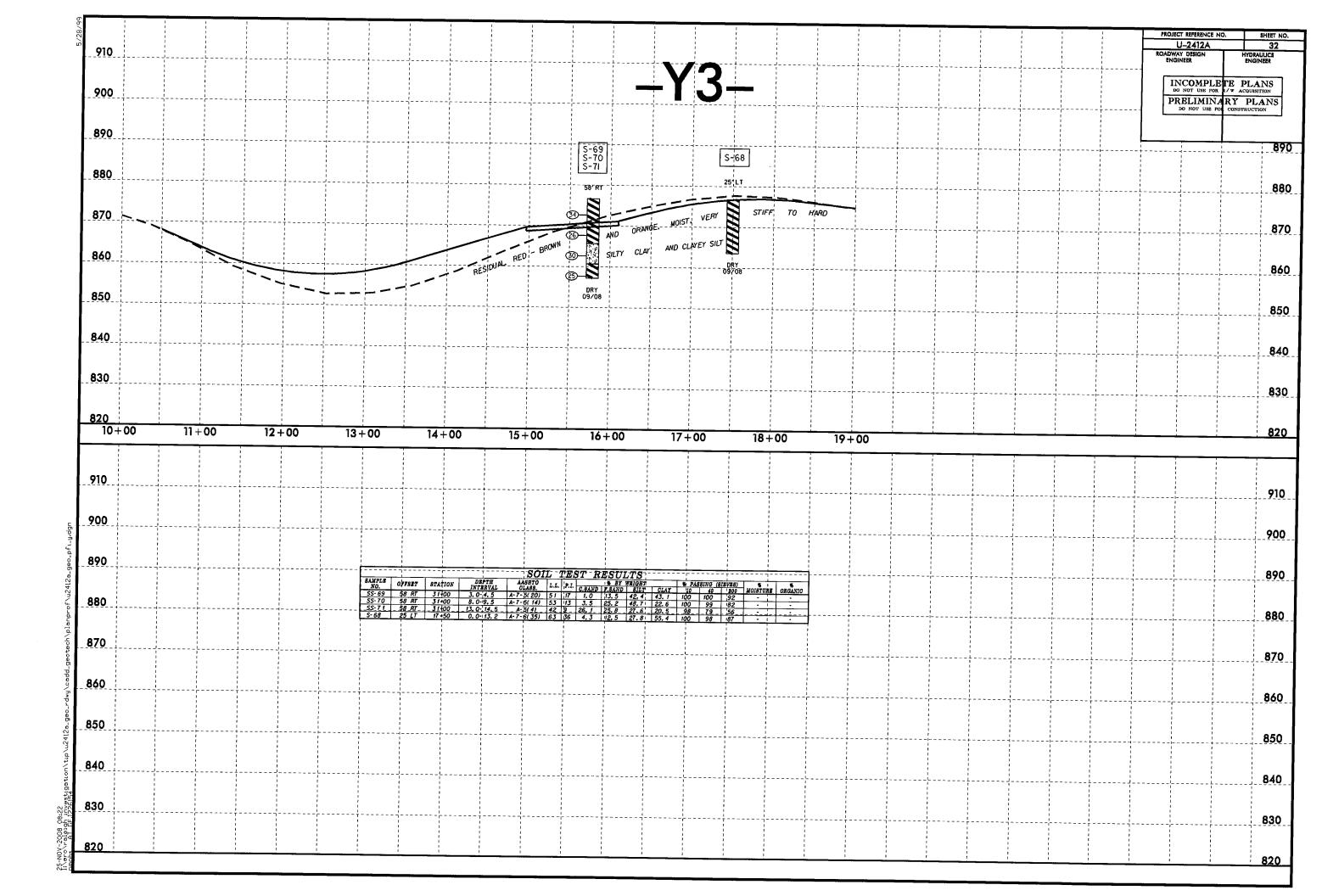


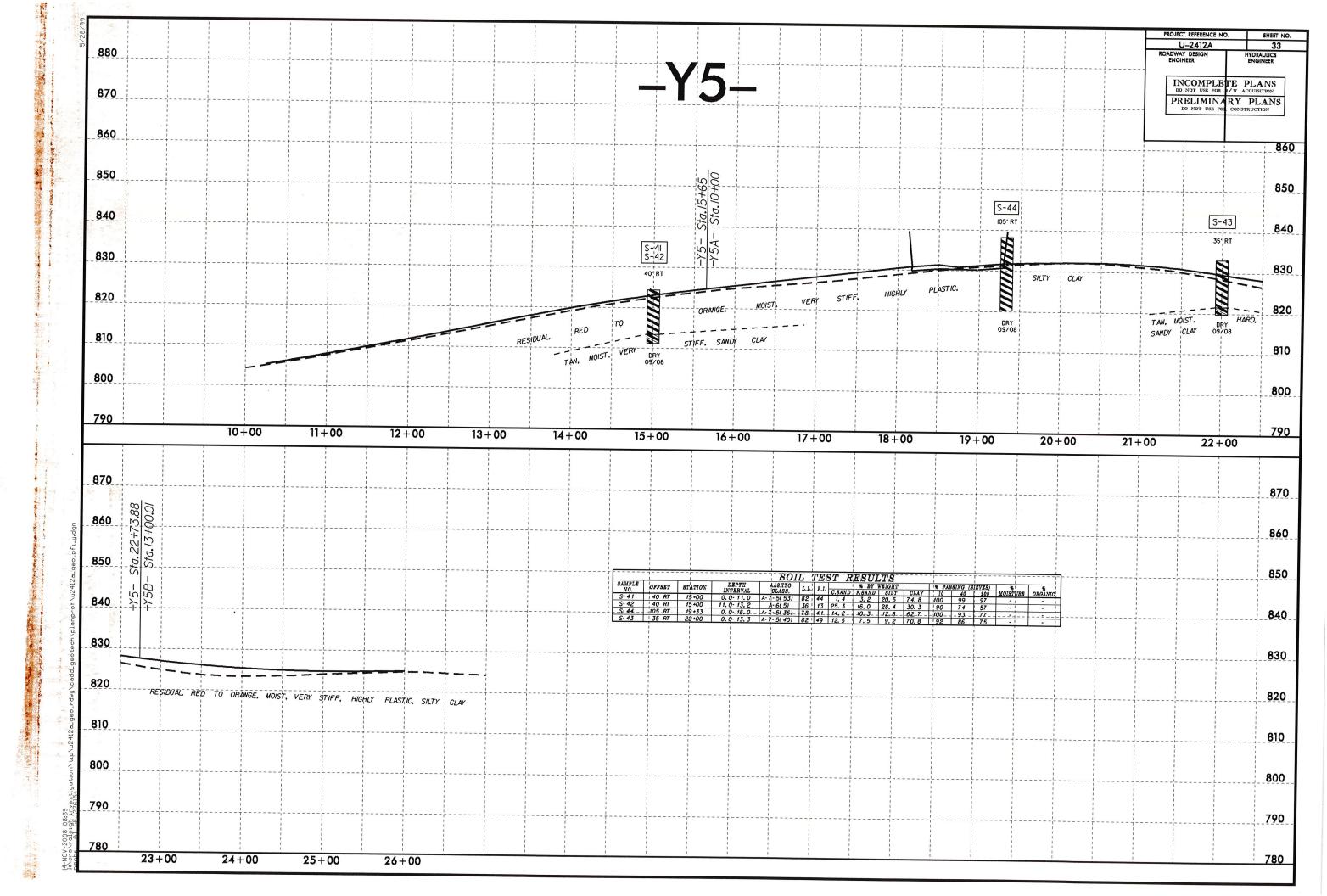


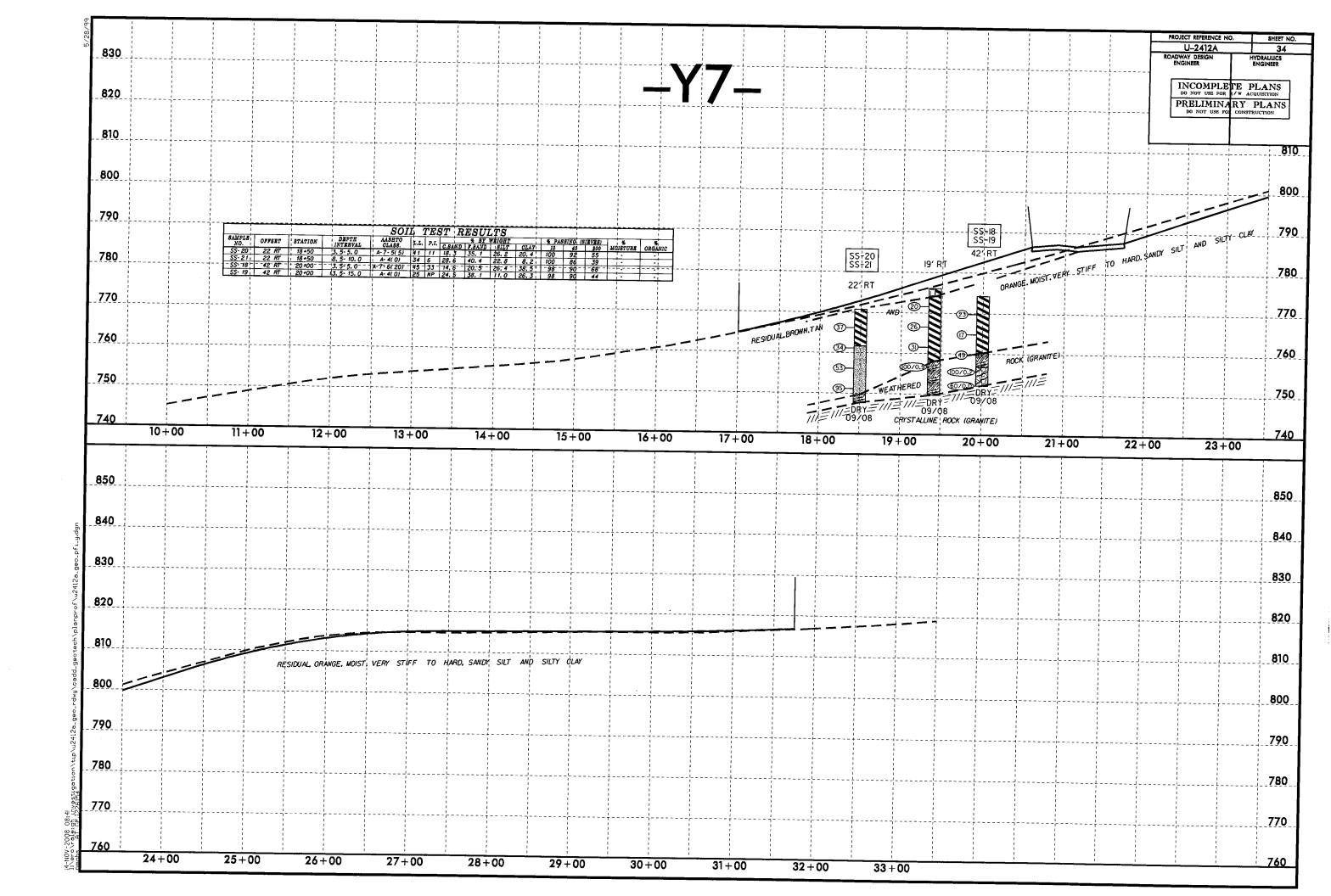


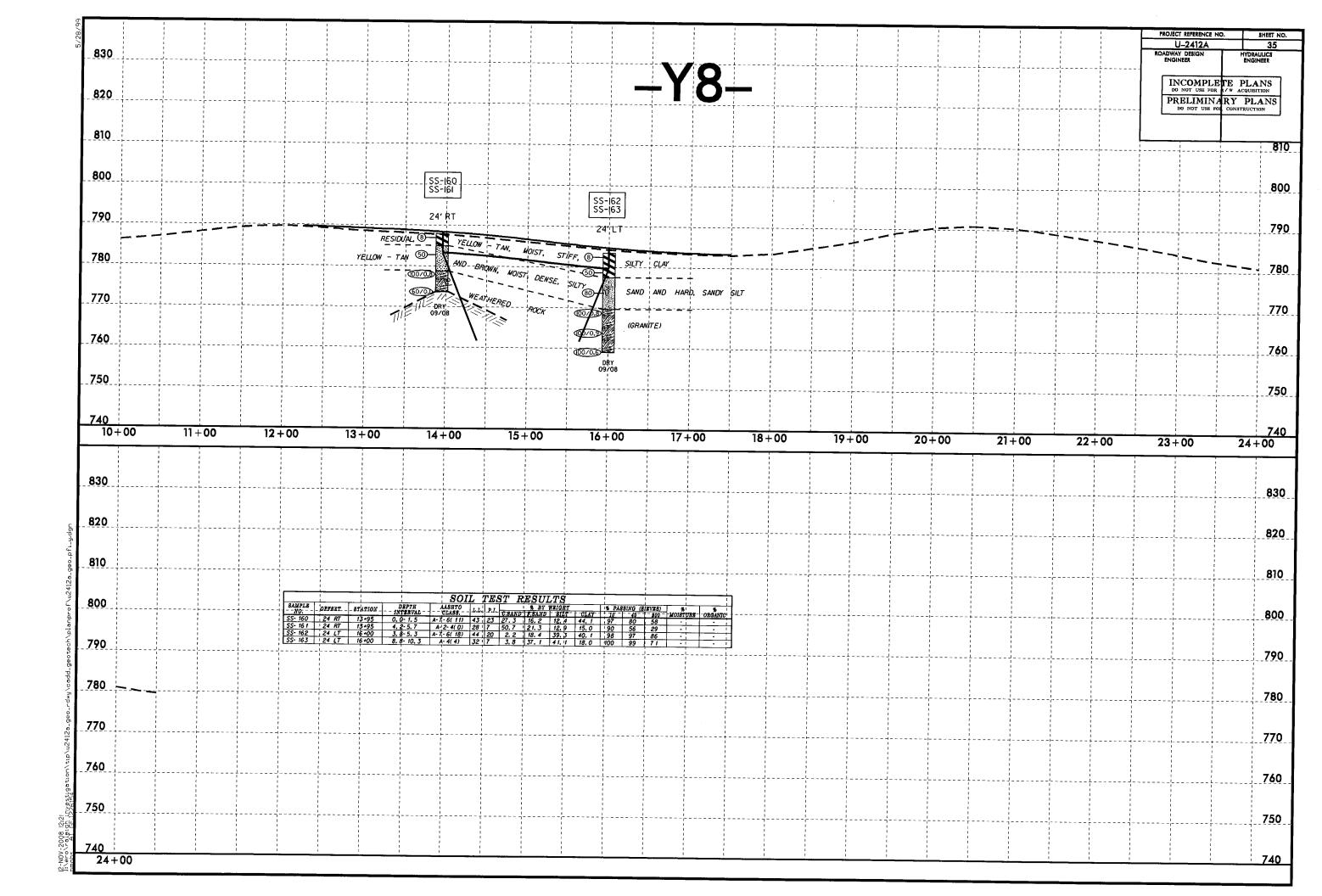


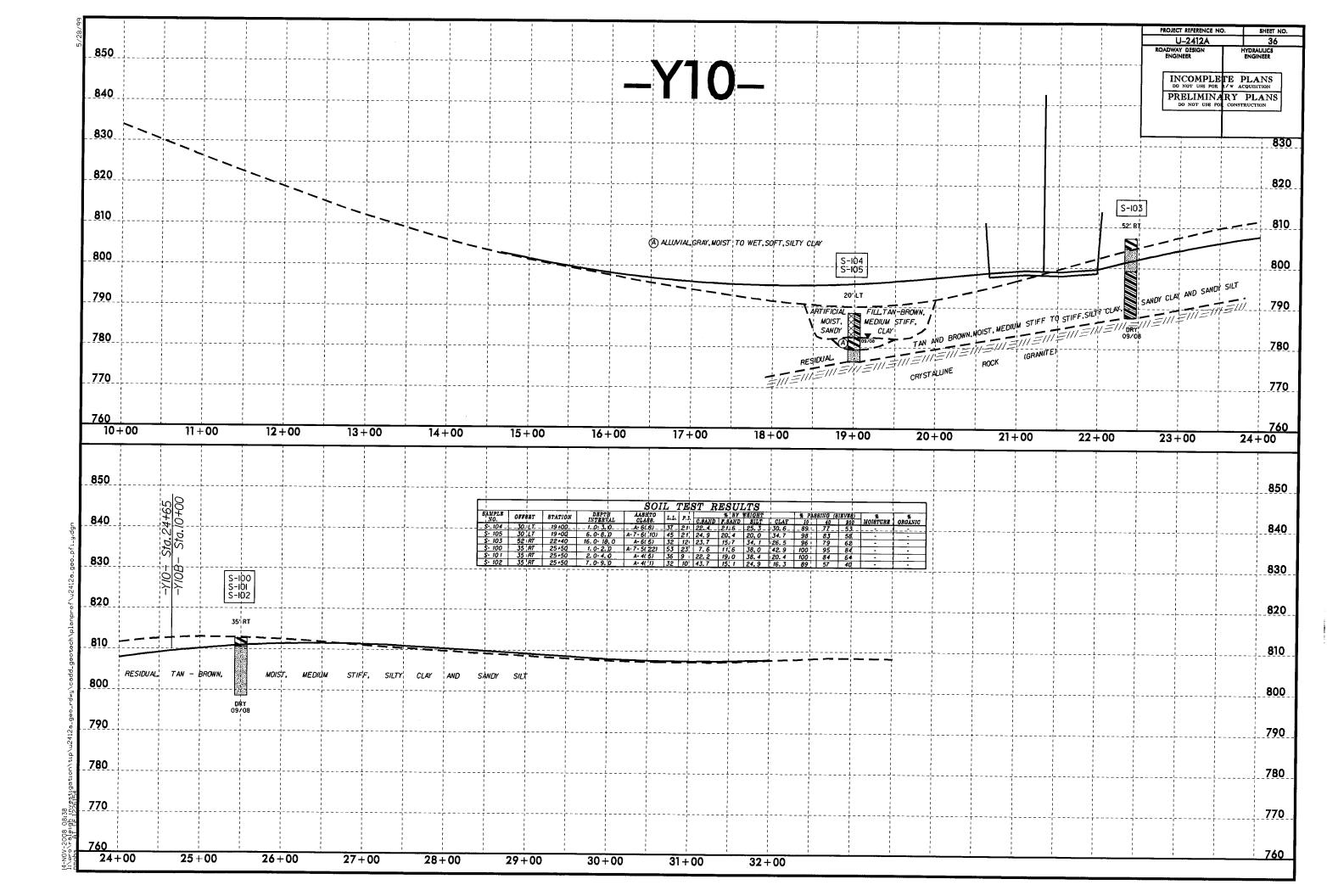




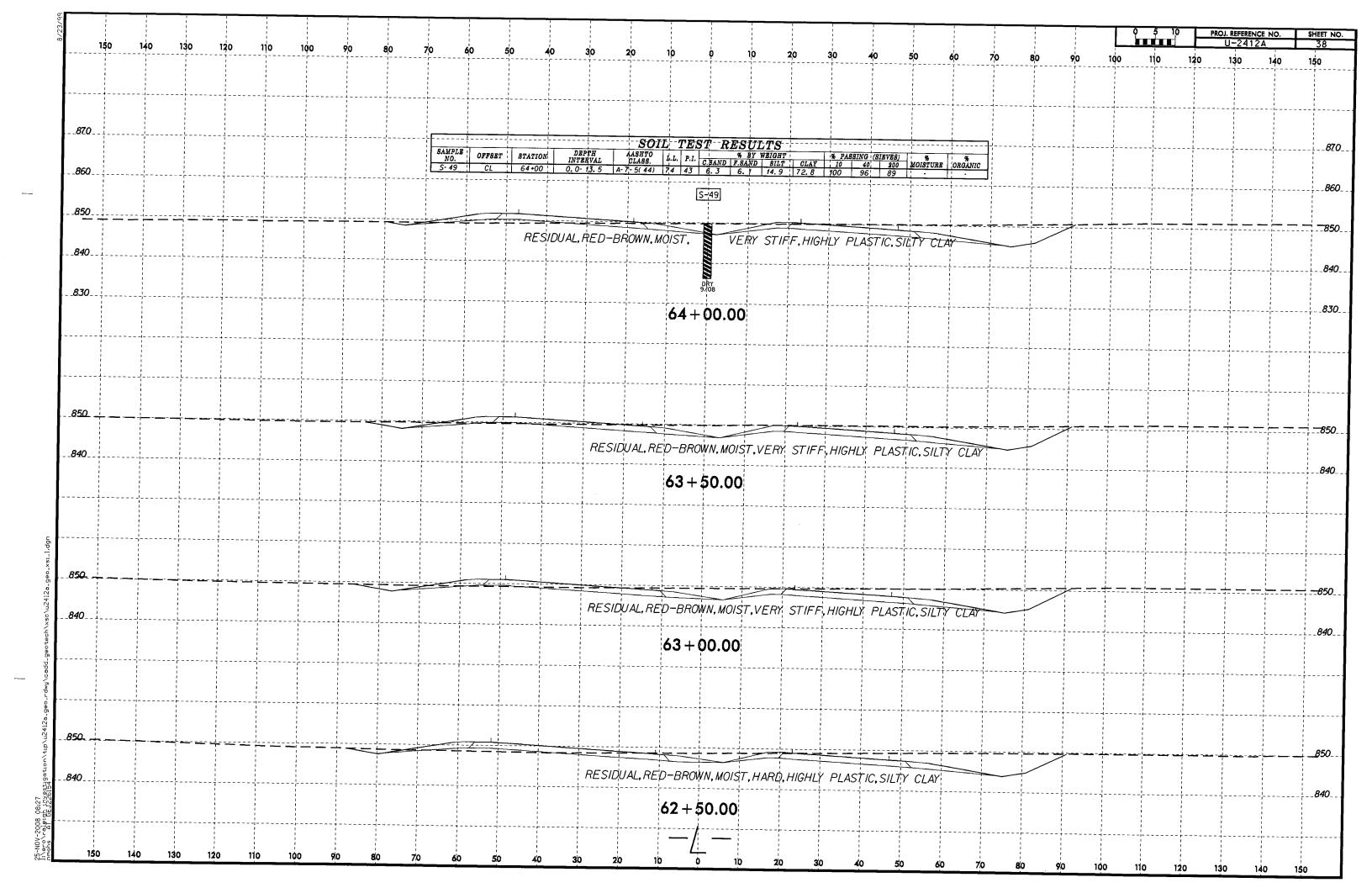


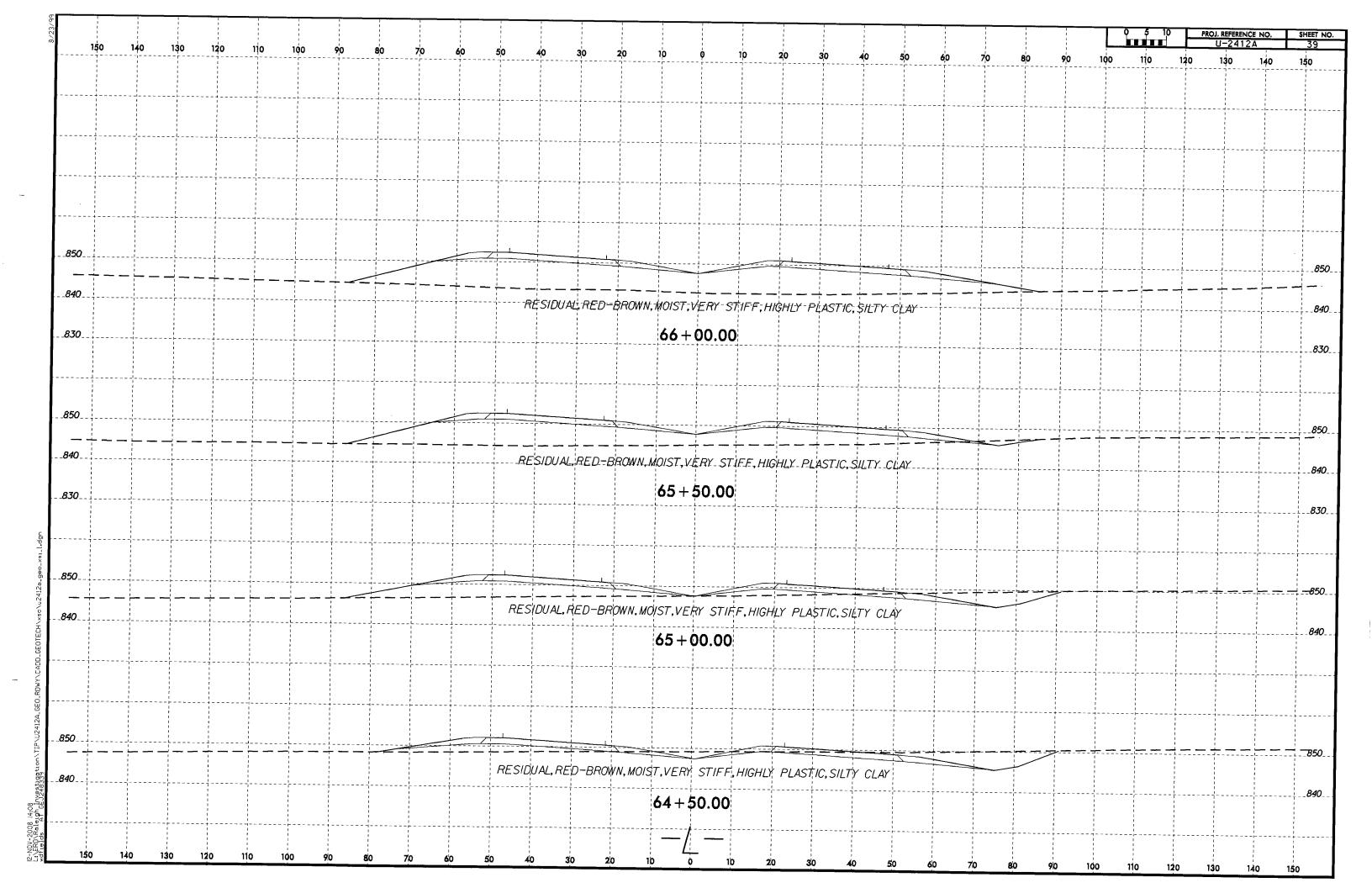


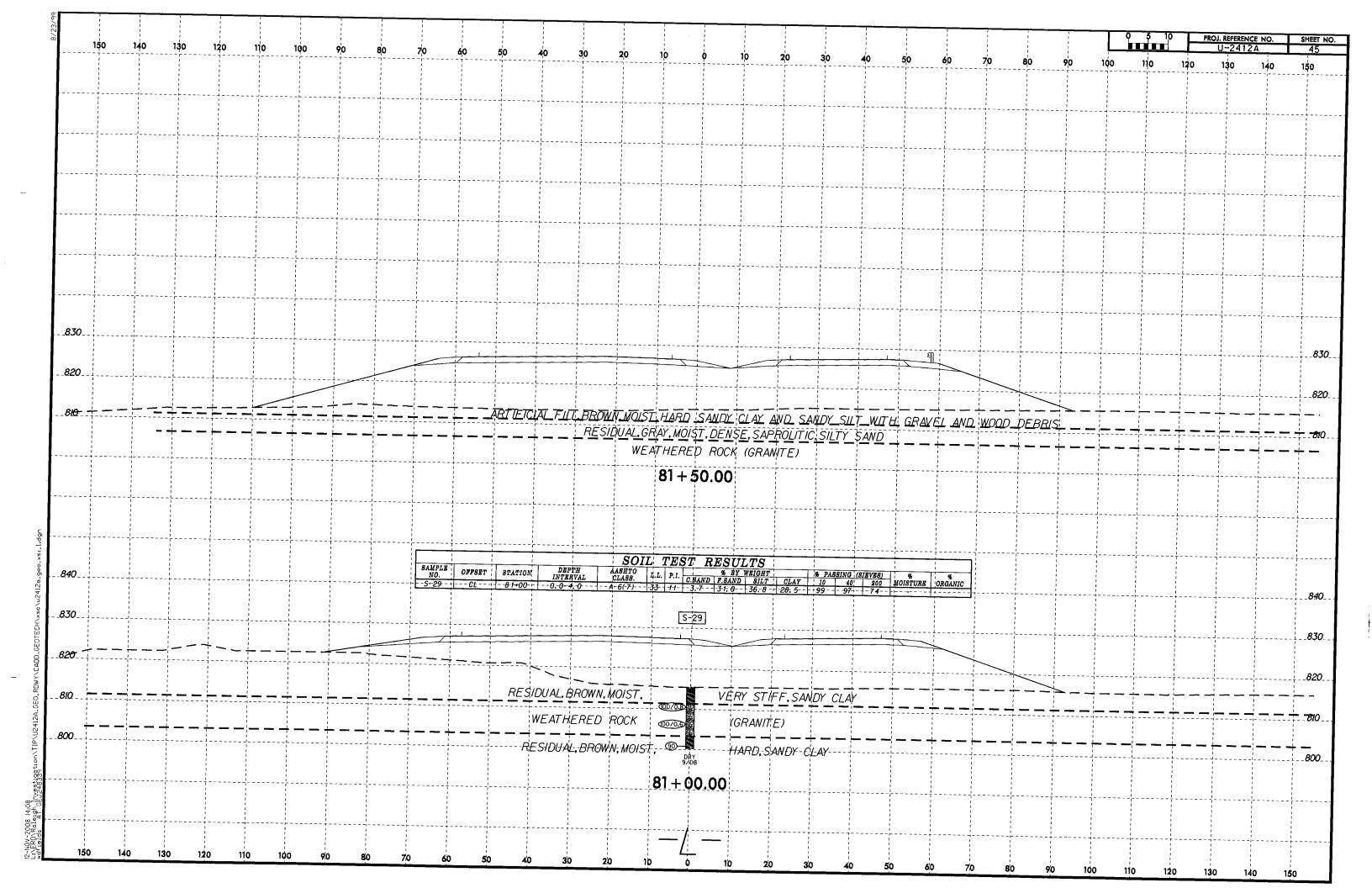


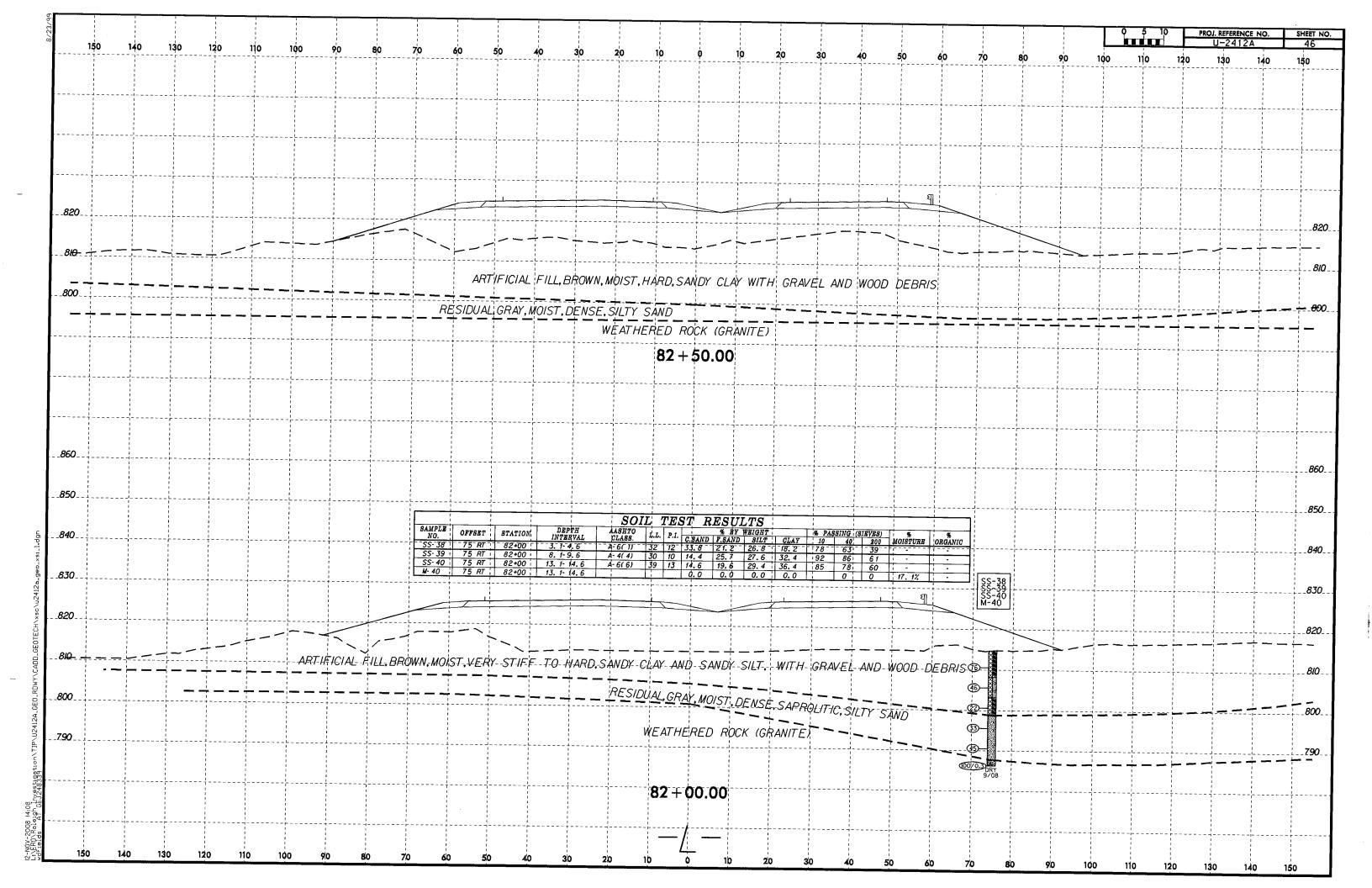


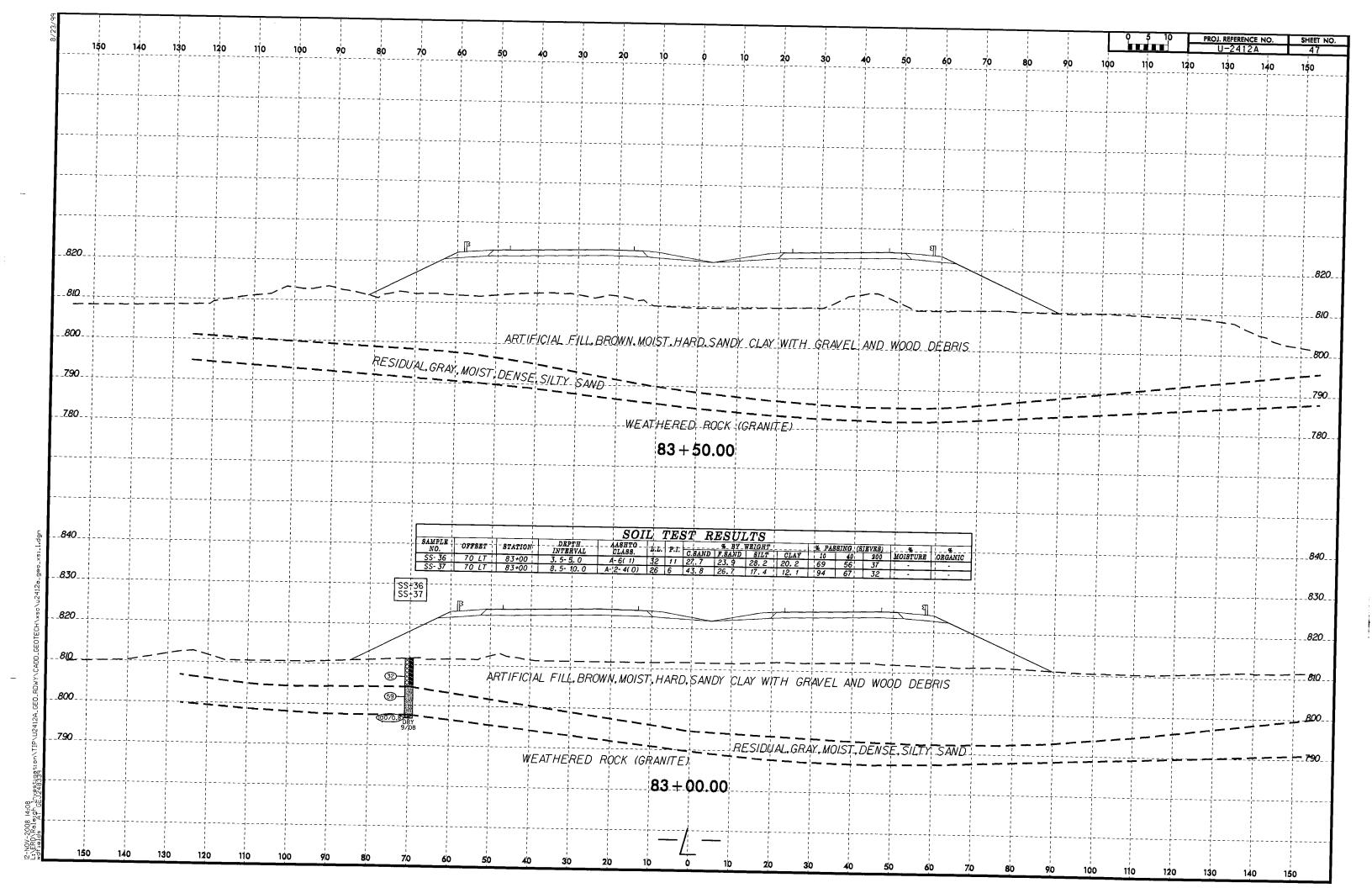
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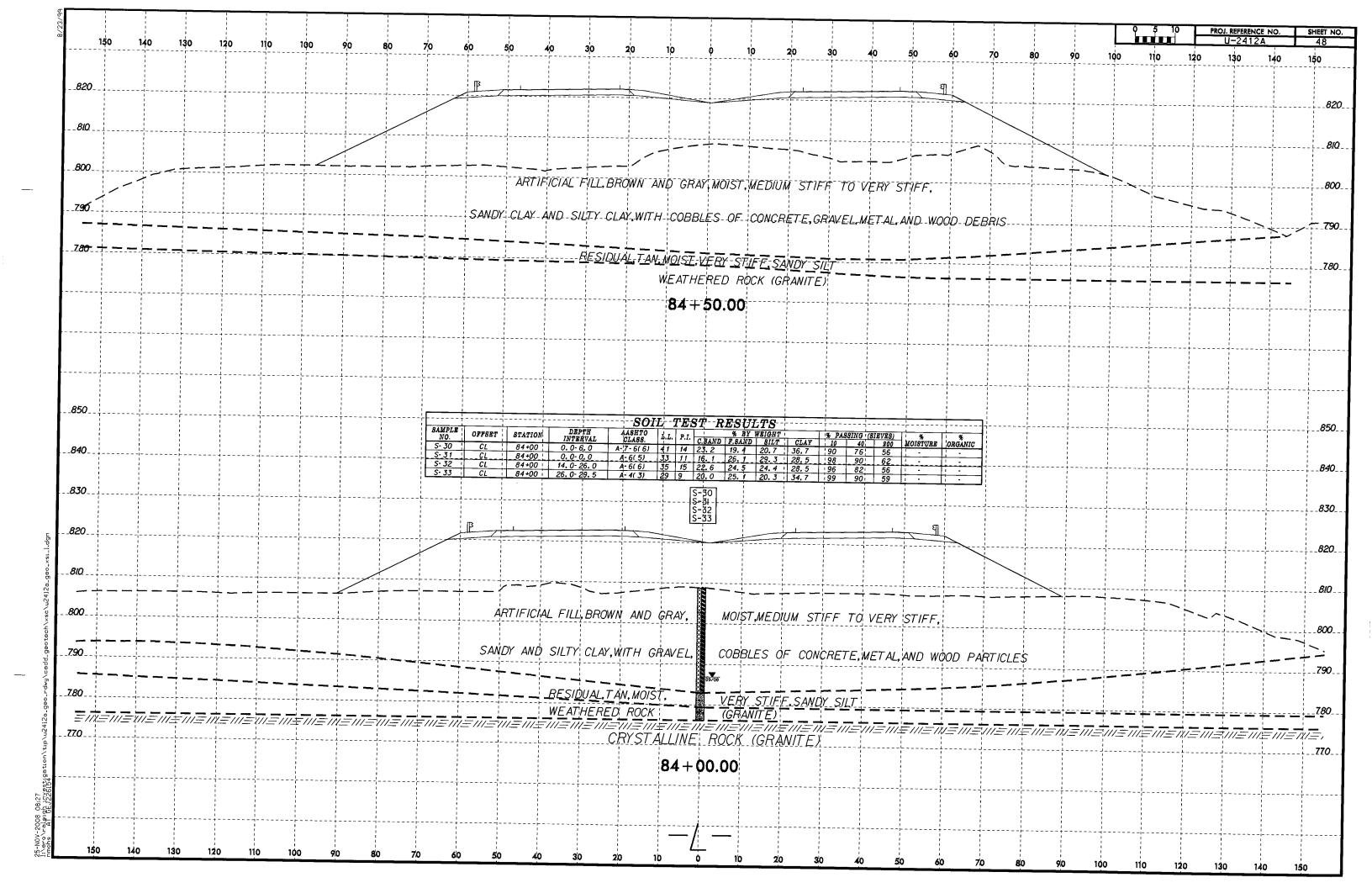


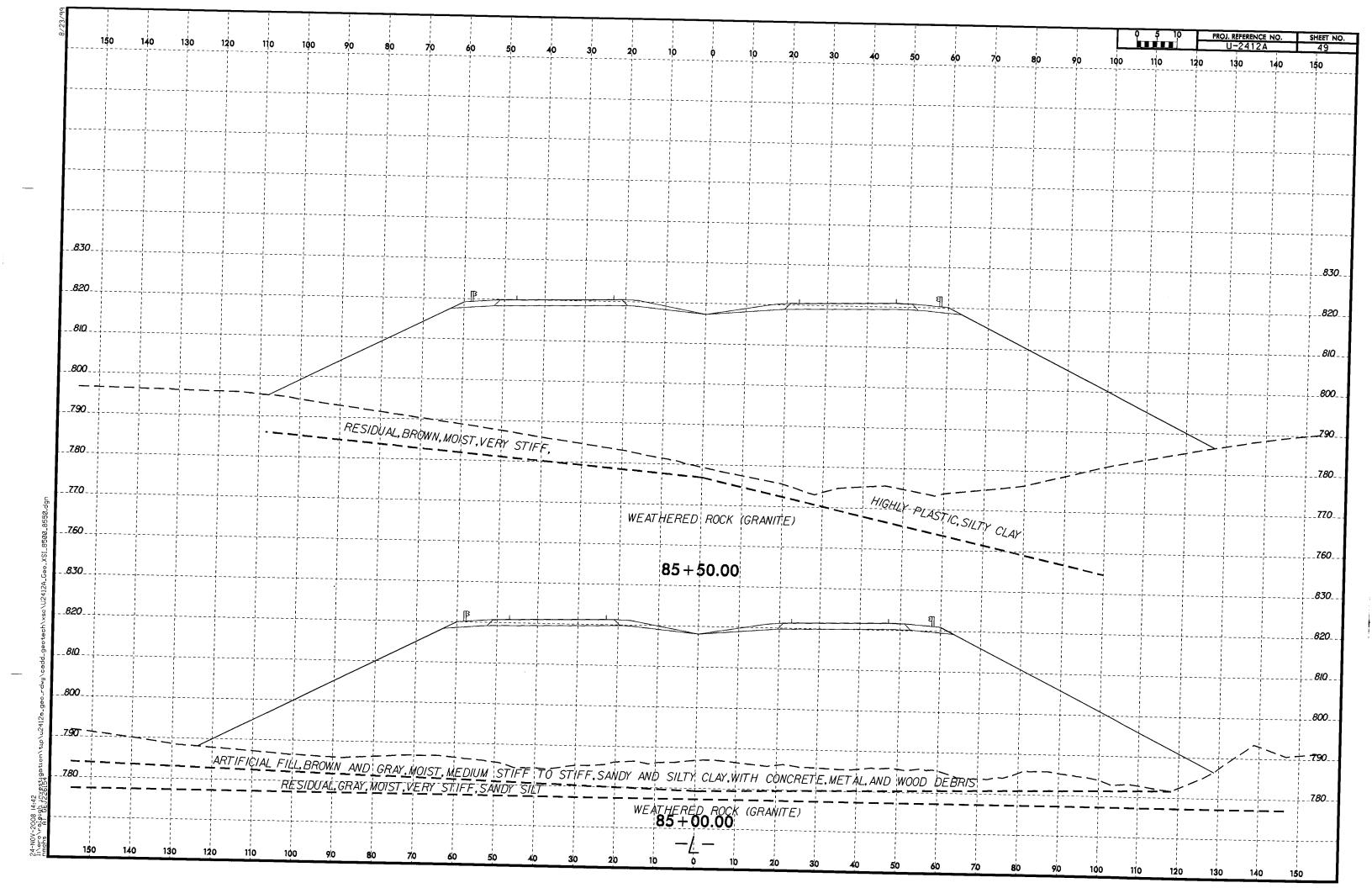


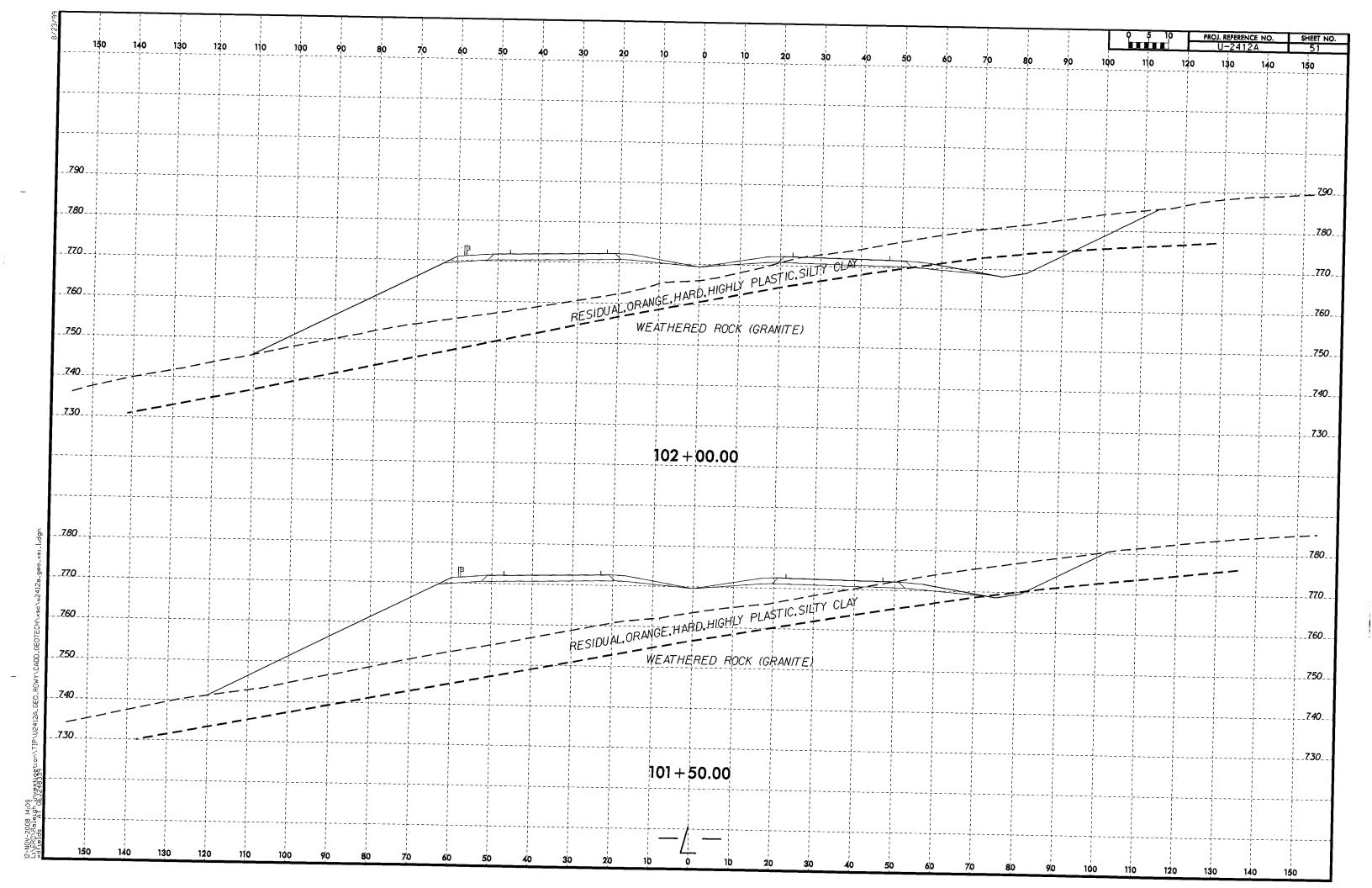


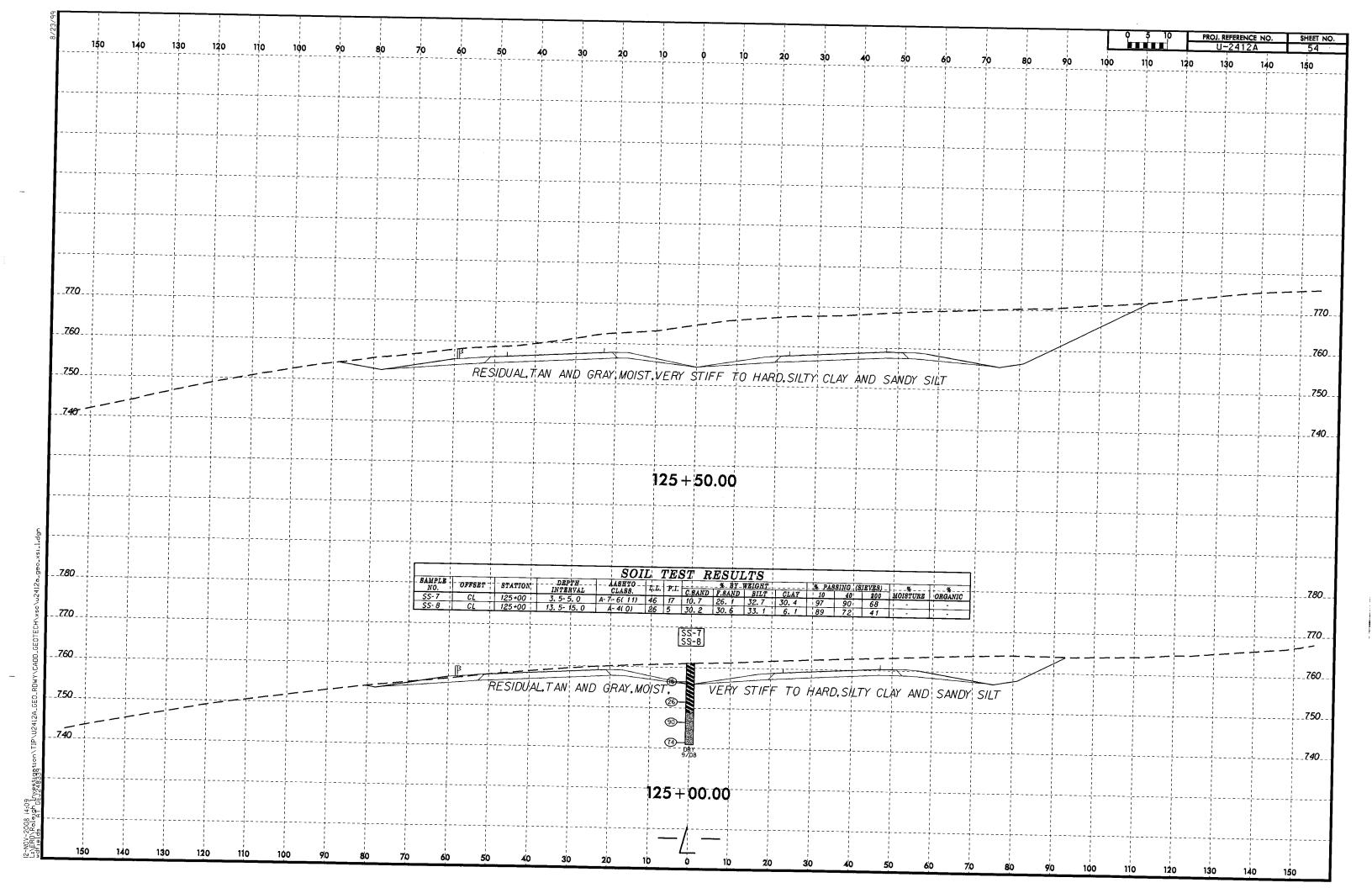


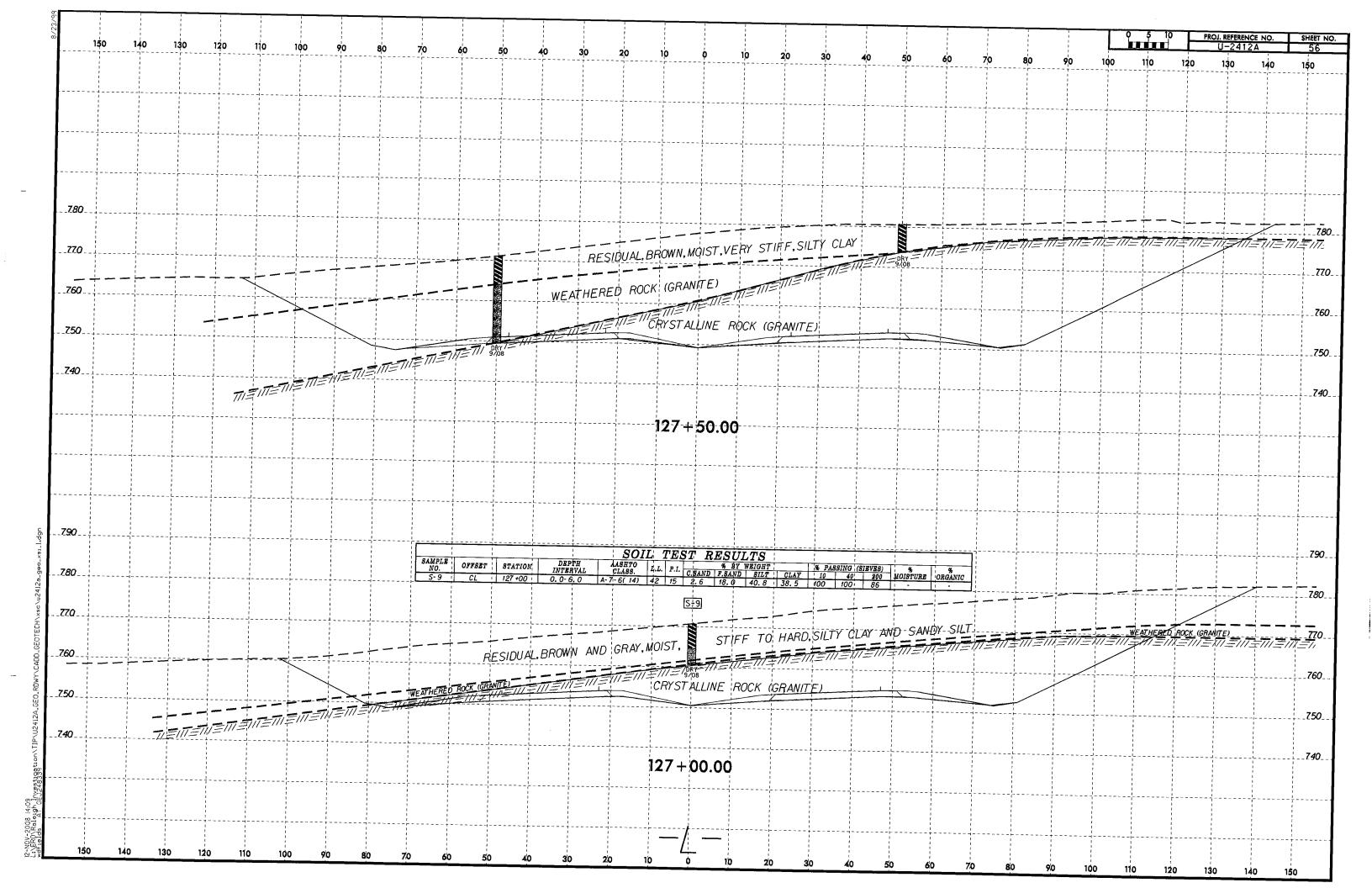


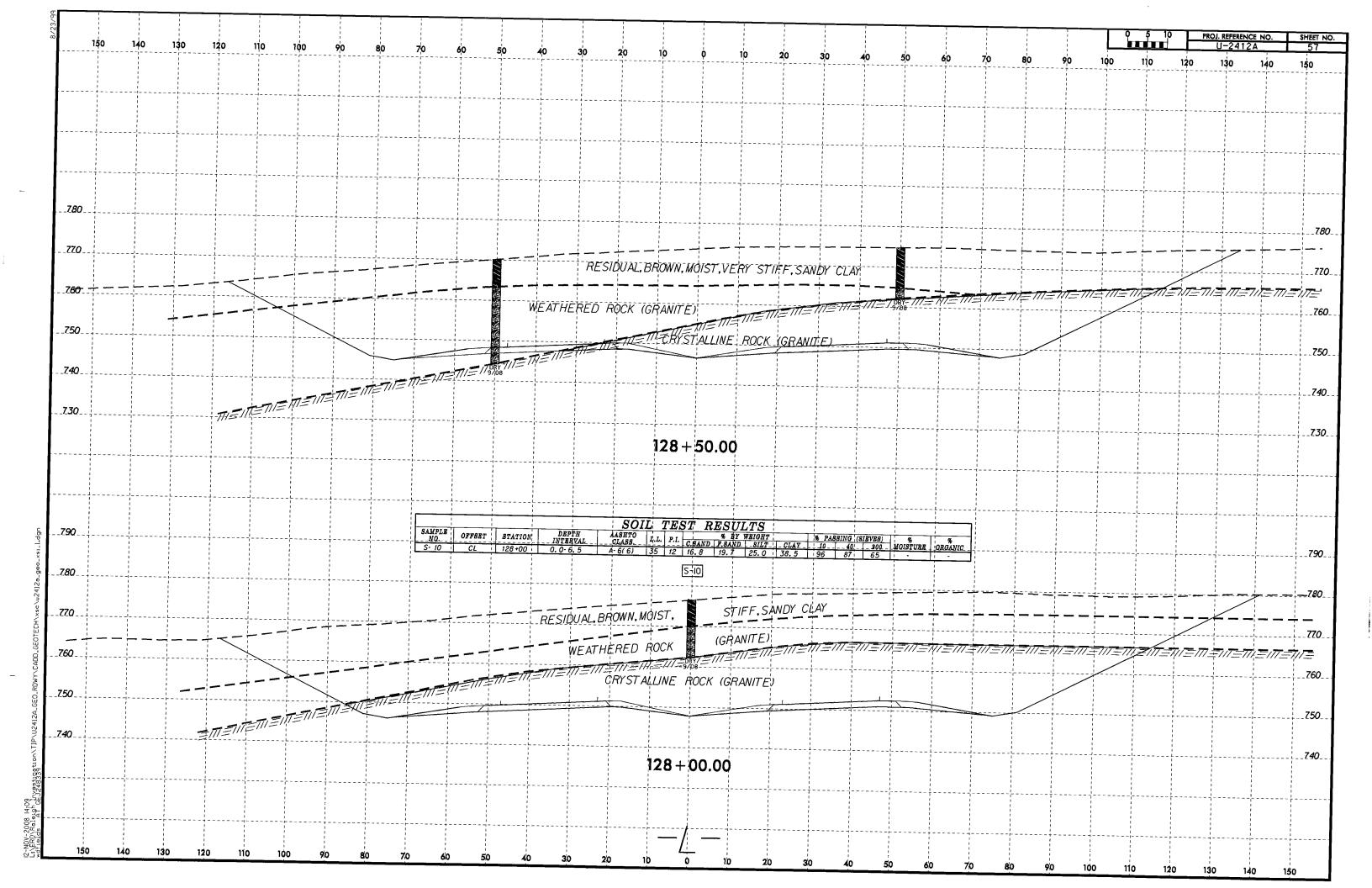


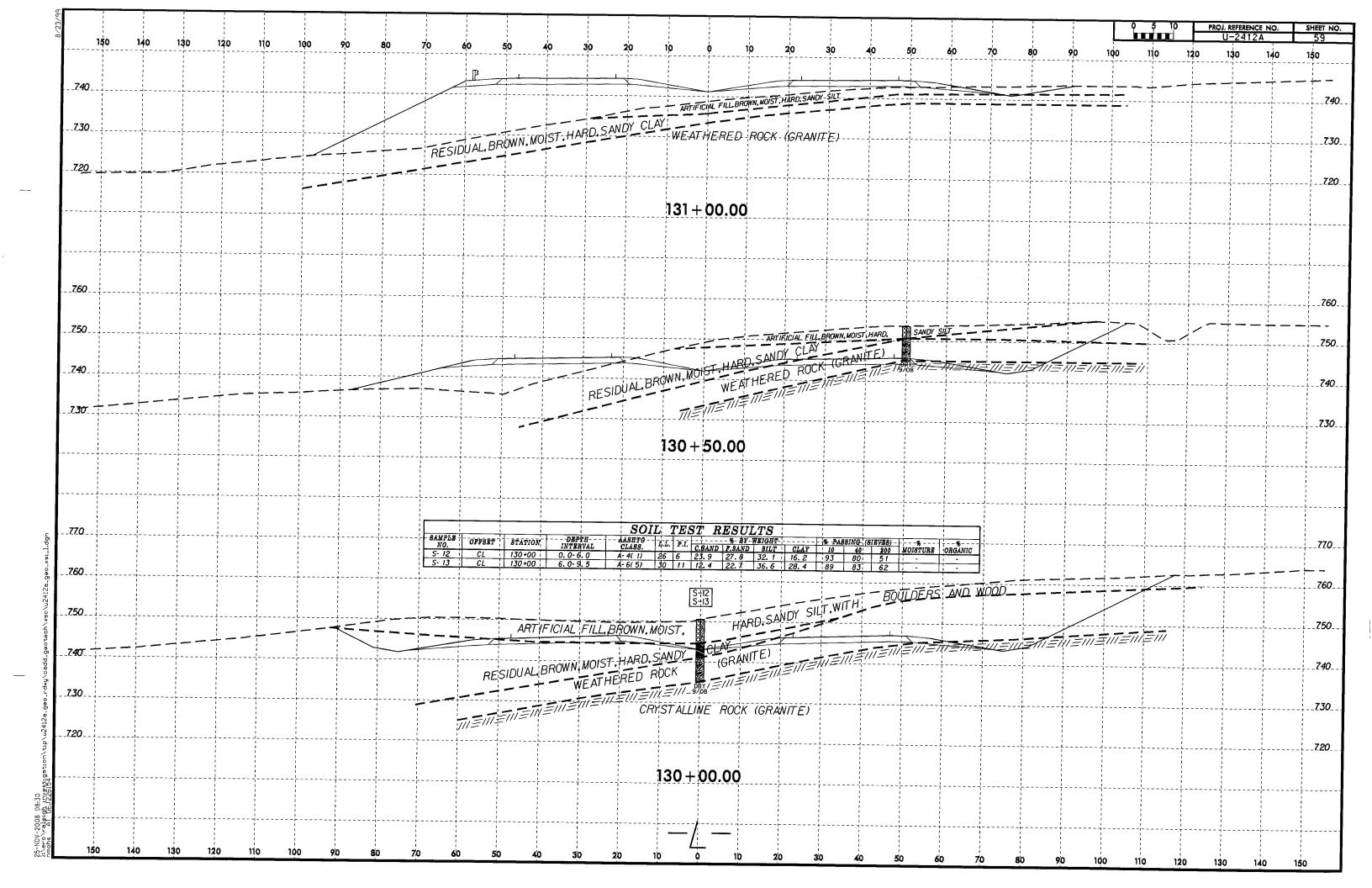


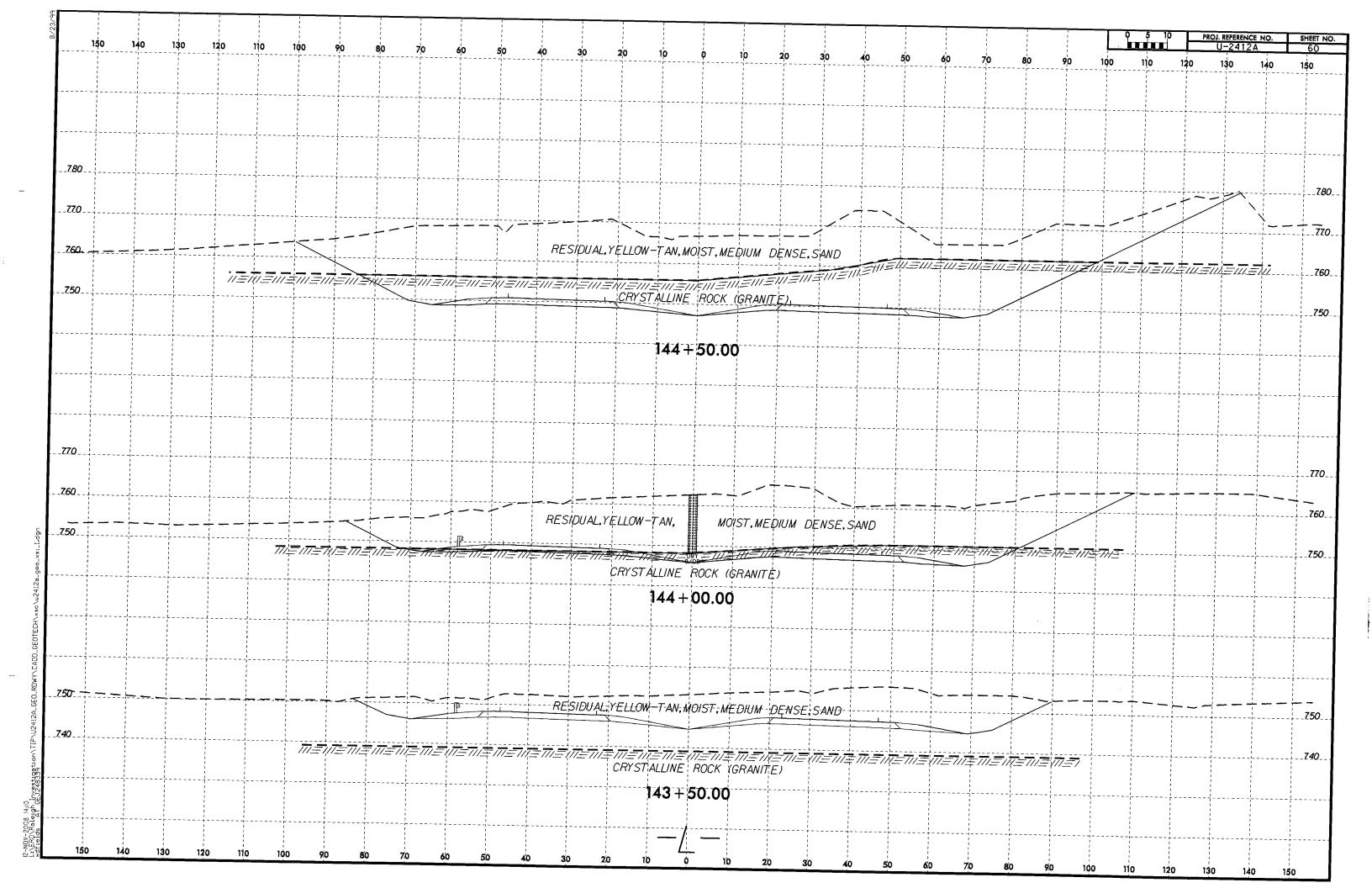


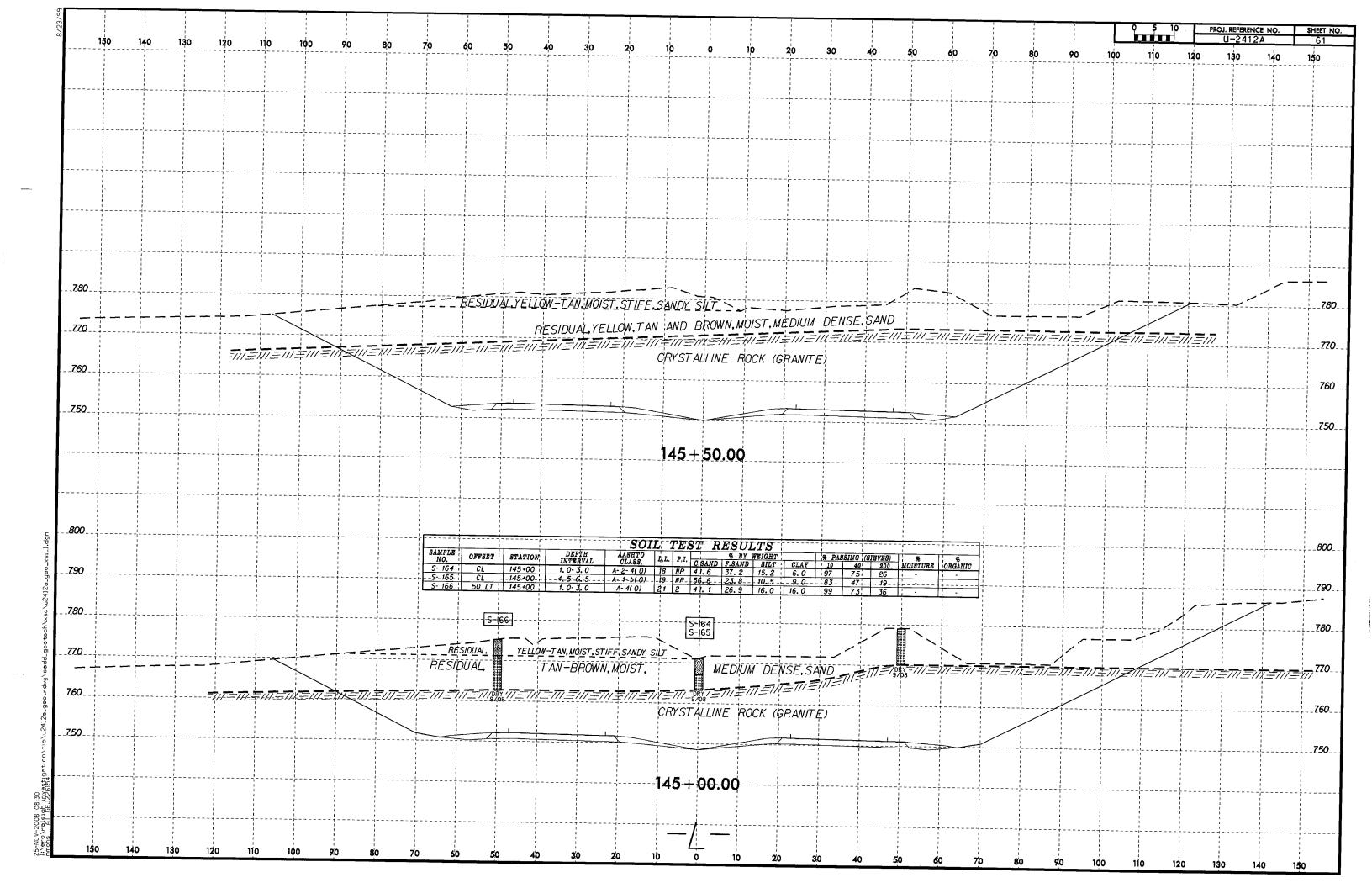


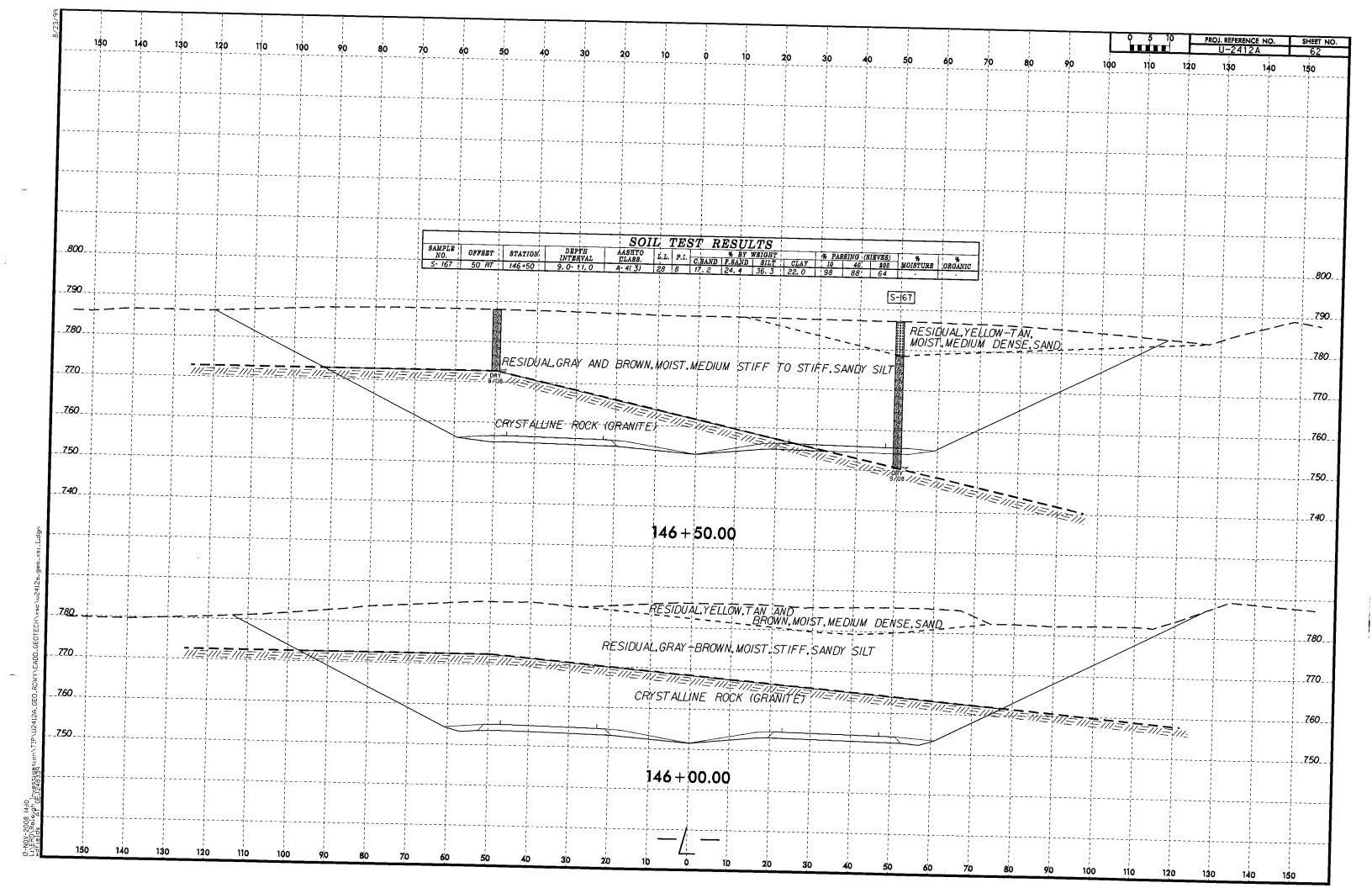


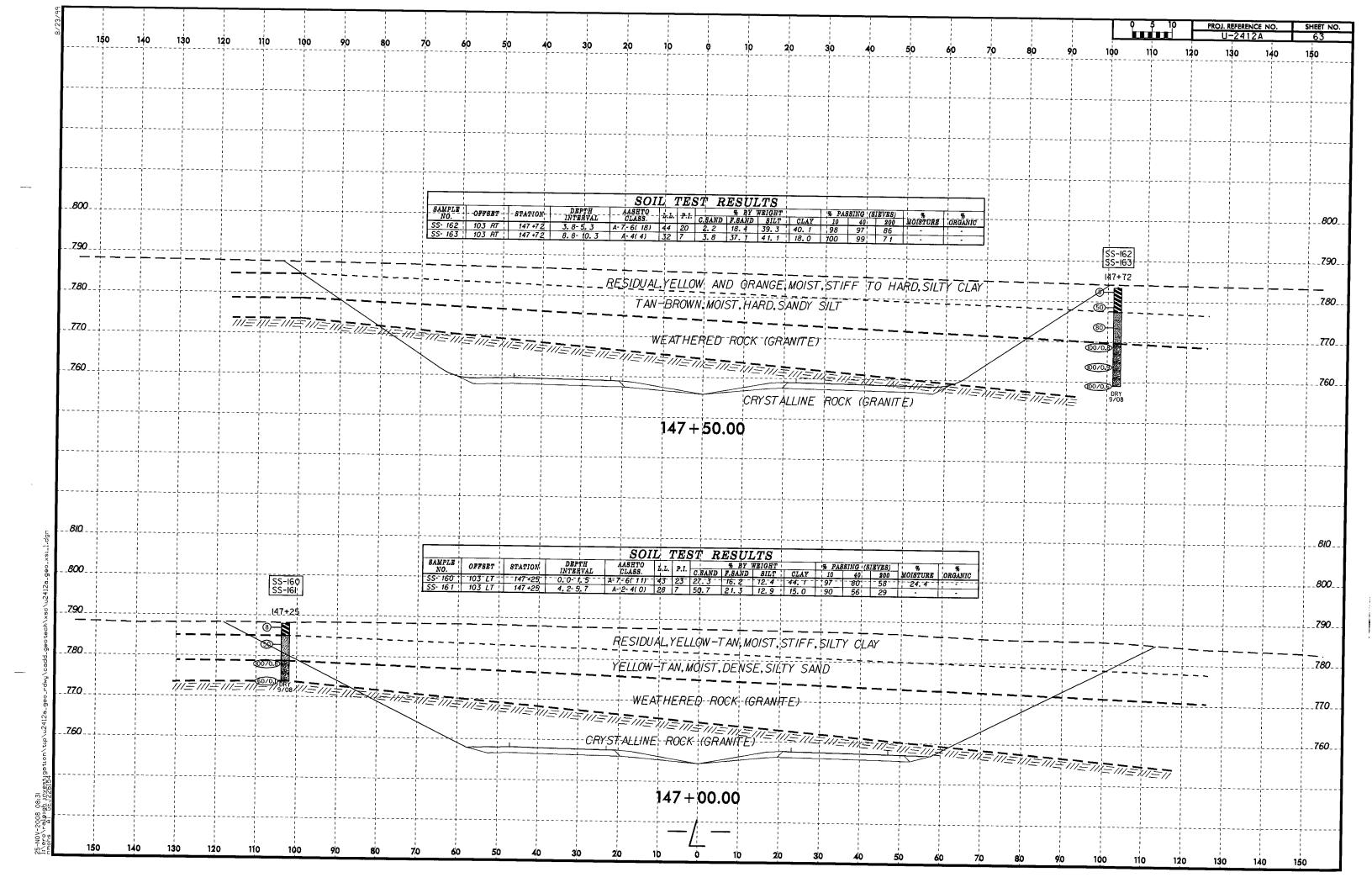












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