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SHORING HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2016 EDITION OF AREMA'S 'MANUAL FOR RAILWAY ENGINEERING, VOL. 2, STRUCTURES", AND "NORFOLK SOUTHERN GUIDELINES FOR DESIGN OF GRADE SEPARATION STRUCTURES, UNDERPASS GRADE SEPARATION DESIGN CRITERIA".

MAXIMUM WALL DEFLECTION LIMITED TO $1^{1}/_{2}$ ".

ALL SHORING MATERIAL SHALL BE IN "LIKE NEW" CONDITION.

SHEET PILING SHALL BE ASTM A572 GRADE 50 STEEL (HOT ROLLED) AND SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

SECTION 1: MOMENT OF INERTIA/FT. 656.2 in 4 SECTION MODULUS/FT. 66.8 in ³

SECTION 2: MOMENT OF INERTIA/FT. 156.9 in $\frac{4}{2}$ SECTION MODULUS/FT. 23.2 in ³

ASSUMED SOIL PARAMETERS COARSE GRAINED SOILS: SOIL MOIST UNIT WEIGHT - 120 PCF SOIL FRICTION ANGLE - 30 DEGREES UNDRAINED SHEAR STRENGTH - O PSF

* CONSTRUCTION SEQUENCE:

1. INSTALL SHORING TO REQUIRED TIP ELEVATION.

2. EXCAVATE TO EXCAVATION LIMIT SHOWN.

3. CONSTRUCT PROPOSED PIER.

4. BACKFILL PROPOSED PIER.

5. REMOVE SHORING.

* COMPLETE SHORING REMOVAL PRIOR TO ABUTMENT 2 EMBANKMENT CONSTRUCTION AND PILE INSTALLATION.

PROJECT N	O	P-5705BA

MECKLENBURG

STATION: POS STA. 28+12.88 -S2-

HINNING CAROL	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH									
SEAL 44886 556B3197C22B434 CRYAN RAMAN 2/21/2018	SUBSTRUCTURE									
ENT NOT CONSIDERED INAL UNLESS ALL NATURES COMPLETED	PIER 2 SHORING									
NORTH CAROLINA, P.C.			REVI:	SIONS			SHEET NO.			
ense No. C-1554 Six Forks Rd., Suite 200, Raleigh, N.C. 27609	NO.	BY	DATE	NO.	BY	DATE	S7-34			
DATE 10/17 DWG NO 74	1			3			TOTAL SHEETS			
DATE 10/17 DWG. NU. 34	2						51			





HOOKS ON 'Y' AND 'M' BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

TOP MAT OF FOOTING LONGITUDINAL AND TRANSVERSE REINFORCEMENT MAY BE SHIFTED AS

PROJECT I	NO	P-5705BA					
ME	CKLE	NBURC	<u>;</u> C	OUNTY			
STATION:	POS	STA.2	8+12.88	8 -S2-			

	SHEET	1 OF	4						
RTH CARO RTH CARO RTH CARO RTH MINING RTH ROFESSION IN THE REAL	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH								
SEAL 44886 556B3197C22B434 7/1/1/1/1/1/1/1/1/1/1/1/1/1/1/1/1/1/1/1	SUBSTRUCTURE								
	PIER 2								
ENT NOT CONSIDERED NAL UNLESS ALL ATURES COMPLETED									
NORTH CAROLINA, P.C.			REVIS	sions			SHEET NO.		
anse No. C-1554 Six Forks Rd., Suite 200, Raleigh, N.C. 27609	NO.	BY	DATE	NO.	BY	DATE	S7-35		
DATE	1			3			TOTAL SHEETS		
DATE0/17 DWG. NO. 35	2			4			51		

0012DEI

1'-1"

2¹/2″

(LOOKING NORTH TOWARDS EXISTING TRACK)

NOTES:

SHORING HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2016 EDITION OF AREMA'S "MANUAL FOR RAILWAY ENGINEERING, VOL. 2, STRUCTURES", AND "NORFOLK SOUTHERN GUIDELINES FOR DESIGN OF GRADE SEPARATION STRUCTURES, UNDERPASS GRADE SEPARATION DESIGN CRITERIA." MAXIMUM WALL DEFLECTION LIMITED TO $\frac{3}{8}$ ". ALL SHORING MATERIAL SHALL BE IN "LIKE NEW" CONDITION. SHEET PILING SHALL BE ASTM A572 GRADE 50 STEEL (HOT ROLLED) AND SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

MOMENT OF INERTIA/FT. 490.85 in⁴ SECTION MODULUS/FT. 60.7 in ³

ASSUMED SOIL PARAMETERS COARSE GRAINED SOILS: SOIL MOIST UNIT WEIGHT - 120 PCF SOIL FRICTION ANGLE - 30 DEGREES UNDRAINED SHEAR STRENGTH - O PSF

STANDARD SPECIFICATIONS.

REBAR IN THE PORTION OF THE EXISTING WINGWALL TO REMAIN SHALL BE CUT BACK TO A MINIMUM DEPTH OF 1"BELOW CONCRETE SURFACE AND PATCHED WITH AN EPOXY COMPOUND.

CONSTRUCTION SEQUENCE:

1. INSTALL SHORING TO REQUIRED TIP ELEVATION.

2. REMOVE EXISTING WINGWALL TO LIMITS SHOWN.

3. CONSTRUCT PROPOSED ABUTMENT.

4. BACKFILL PROPOSED ABUTMENT.

5. REMOVE SHORING.

EXISTING WINGWALL TO BE PARTIALLY REMOVED IN ACCORDANCE WITH SECTION 402 OF THE

SECTION A-A

 -	PROJE	ECT I ME(ON: .	N o Cklen Pos s	P- BUR TA. 2	570 G 28+12	5BA _ COL 2.88 -	JNTY -S2-		
ENT NOT CONSIDERED NAL UNLESS ALL ATURES COMPLETED		DEPA	state SUBS ABL SI	carolina Ranspo 1 ICTU INT ING	PORTATION JRE 2				
NORTH CAROLINA, P.C. onse No. C-1554 Ny Forks Rd. Suite 200 Raleian N.C. 27609	NO.	BY	REVISI DATE	ONS NO.	ВҮ	DATE	sheet no . S7-39		
	1 2			3 4			total sheets 51		

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001

51

DEPARTMENT OF TRANSPORTATION SHEET NO. S7-40 TOTAL SHEETS

	BAR TYPES						BILL	OF RE	INFOR	CING				
)″OR 1′-3″►	MARK B C D H1 12'-8" 1'-2" 1 ¹ / ₁₆ "	61	MARK	NO.	SIZE	TYPE	LENGTH	WEIGHT	MARK	NO.	SIZE	TYPE	LENGTH	WEIGHT
4'-3"	H2 12'-7" 10" 0 ³ / ₄ "							ABUTN	IENT 2					
	H3 $8'-4''$ $2'-5'' 2^{3}/_{16}''$	<u>IRU</u>	H1	21	7	2	13'-10″	594	T1	123	6	4	14'-8"	2710
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		H2	12	5	2	13′-5″	168	T2	123	7	4	14'-8"	3688
	$\begin{array}{c c c c c c c c c c c c c c c c c c c $	S1 11	H3	30	5	2	10'-9"	337	T3	2	6	4	7'-2″	22
	$\begin{array}{c c c c c c c c c c c c c c c c c c c $		H4	30	5	1	10'-9"	337	T4	2	7	4	7'-2″	30
	K9 = 3'-7'' = 10'' = 1/8	(4) 9^{-1}	H5	11	5	1	9'-8"	111	T5	46	5	5	48'-0"	2303
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		H6	12	7	1	9'-6"	234	T6	46	5	5	30'-4"	1456
	NOTE: "D" IS PARALLEL TO WINGWALL.		H7	1	5	1	9'-2"	10	T7	10	5	STR.	46'-5"	485
		11″ S1	H8	3	5	STR.	8'-5"	27	T8	10	5	STR.	28'-9"	300
		3'-11" S2	H9	3	(SIR.	8'-5"	52	19	5	5	SIR.	12'-1"	64
		3'-8" \$3	1/ 1	Λ		CTD	A/ 11//	01	110	5	5	SIR.	11'-6"	60
			KI K2	4	5	SIK.	$4^{-11^{"}}$)/1	EC	E	CTD	10/ 0//	740
\leq		<	KZ KZ	20	5	SIR. STR	28-6	<u> </u>		20 72	2 0	SIR. STD	12 - 8	1192
		<u>1'-5″ S5</u>	K J	25	5	SIR. STR	20-5	644	VZ VZ	 ع	о 5	SIR. STR	13 - 10 11' - 2''	91
		1'-4" S6	K4 K5	13	5	STR.	24 0	308	V J V A	<u>ט</u> גא	5	STR.	5'-5"	215
ION		1'-3" 57	K6	20	5	STR.	11'-6"	240	V5	1	5	STR.	12'-2"	13
			K7	10	5	3	5'-6"	58	V6	10	7	5	15'-2"	310
-		1 ¹ − 2 ⁿ 58	K8	10	5	3	5′-8″	60	V7	5	5	STR.	8'-10"	47
	B B 2'-0"	<u>1'-1" S9</u>	К9	8	5	2	4'-5"	37	V8	135	7	5	13'-3"	3657
		1'-0" S10	K10	8	5	1	4'-5″	37	V9	43	5	STR.	6'-11"	311
		11″ 511							V10	81	5	5	6′-3″	529
L S S			S1	74	5	4	3'-11"	194	V11	51	7	STR.	8'-3"	861
		<u>−1′−7″ S12</u>	S2	50	5	4	6'-11"	361	V12	29	5	STR.	10'-2"	308
		1'-6" S13	S3	23	5	4	6'-8"	160	V13	26	5	STR.	3'-11"	107
		1′-5″ S14	S4	1	5	4	4'-6"	5	V14	81	5	5	2'-9"	233
			S5	1	5	4	4'-5"	5	V15	18	5	STR.	10'-8"	201
11, -, -, -, -, -, -, -, -, -, -, -, -, -,			S6	1	5	4	4'-4"	5						
	<u>T5 46'-5"</u>	<u>1'-3″ S16</u>	S7	1	5	4	4'-3"	5						
	тб 28'-9″	1'-2" S17	S8	1	5	4	4'-2"	5			QUANT	TTTFS		
		1'-1" S18	59	1	5	4	4'-1"	5						TOTAL
	$\frac{\sqrt{6}}{\sqrt{1-2}}$		SIU SII	1	5	4	4' - 0''	4			LIEM			TUTAL
	<u>V8</u> 1'-2"		S11 \$12	1	5	4	3 - 11	<u>4</u> Б						05 710
,-0	V10 1'-0"	<u>11'-6″ T1</u>	S1Z S13	1	5	4	4 1 Δ'-6"	5	REINFUF	CING SI	EEL		LB2.	25,318
		11'-6″ T2	<u> </u>	1	5	4	4'-5"	5			ETE BREA			
		4'-О" ТЗ	S15	1	5	4	4'-4"	5		CONUR				120.1
-O" ABUT.STEM			S16	- 1	5	4	4'-3"	5	FUUK I	FUUIINC	, . <u></u>			123.1
		4 [°] −∪″ 14 →	S17	1	5	4	4'-2"	5	POUR 2	ABUTME	NT		cu. yds	50.8
			S18	1	5	4	4'-1"	5	SIEM 8	K BOIL.(JF WINGS			
	ALL BAR DIMENSIONS ARE OUT TO OUT		S19	1	5	4	4'-0"	5	BACKWA	ALL AND	WINGS		CU.YDS	21.4
									POUR 4 GTRDFR	PLATFO	RM All & W	TNGS	CU.YDS	33.5
									POUR 5		RM		CU. YDS	5.1
									GIRDER	BACKWA	ll and W	IINGS		
									TOTAL				CU.YDS	239.9
									HP 14 X	73 STEF	PTIFS		NO	.30
										, J UILL	_ ,			1.200

PROJECT NO. P-5705BA MECKLENBURG STATION: POS STA. 28+12.88 -S2-

STATE OF NORTH CAROLINA H CARO DEPARTMENT OF TRANSPORTATION STESSIO RALEIGH SEAL 44886 556B3197C22B434... SUBSTRUCTURE WGINEER 2/21/2018 RYAN ABUTMENT 2 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED HNTB NORTH CAROLINA, P.C. NC License No.C-1554 343 E.Six Forks Rd.,Suite 200,Raleigh,N.C.27609 SHEET NO. REVISIONS NO. BY DATE S7-45 DATE NO. BY total sheets 51 1 3 DATE <u>10/17</u> DATE <u>10/17</u> DWG.NO. 45 2 4

SHEET 6 OF 6

65'-0" SPAN B (© PIER 1 TO © PIER 2) 11'-2" 11'-2" 9'-11″ SEE THIS SHEET FOR CONTINUATION OF SPAN B 1'-10" P.L. 1'-7^{||}/₁₆"P.L. -11//₁₆" 6"Ø FLANGE AND FLARE /<u>6″ØD.I.PIPE (BELL</u> AND P.E.) TYP. - SEE (TYP.) - GRADE DETAIL "B" 4-------1.000% MIN. 9′-9¾″ P.L.\ 8′-6¾″P.L. DSA, SEE "STRUCTURE: DRAINAGE, INTERMEDIATE DETAILS" SHEET 2 OF 4 (TYP.) DIAPHRAGM (TYP.) 6″Ø D.I.PIPE 6″Ø FLANGED (FLANGED BOTH ENDS) TEE (TYP.) MAIN COLLECTOR (TYP.) PSA, SEE "STRUCTURE: DRAINAGE | DETAILS"SHEET 2 OF 4 (TYP.) 4'-107/₆"P.L.LINE 1 3'-9%6"P.L.LINE 2 SLOPE TOP OF DECK SLAB TO DRAIN

DRAINAGE DETAILS - SPAN A AND PORTION OF SPAN B

DRAWN BY <u>B.VAUGHN</u> CHECKED BY <u>R.RAPP</u>

NOTES:

ALL DIMENSIONS ARE SUBJECT TO ADJUSTMENTS TO FIT MEASUREMENTS TAKEN AFTER DECKS HAVE BEEN POURED. D.I. = DUCTILE IRON

P.L. = PIPE LENGTH

P.E. = PLAIN END

DRAINAGE LINE 1 IS LOCATED TO THE LEFT LOOKING AHEAD STATION AND DRAINAGE LINE 2 IS LOCATED TO THE RIGHT LOOKING AHEAD STATION. FOR ADDITIONAL NOTES AND DETAILS, SEE "STRUCTURE: DRAINAGE DETAILS" SHEET 2 OF 4.

		TATI	ME(ON : .	_ COL 2.88 -	JNTY -S2-					
		SHEET	1 OF	4						
CAROL MANUTALINA CAROL		STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION								
SEAL 44886 556B3197C22B434. 72/21/2018	.	STRUCTURE								
""/////IIII//I//		DRAINAGE								
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED		DETAILS								
		REVISIONS						SHEET NO.		
343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 2	27609	NO.	BY	DATE	NO.	BY	DATE	ンパー4の TOTAL		
AUGHN DATE <u>8/17</u> DWG. N APP DATE <u>9/17</u> DWG. N	10.46	2			4			sheets 51		

PROJECT NO. <u>P-5705</u>BA

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DOCUME FΙ SIGN HNTB N NC Licer 343 E. Si DRAWN BY <u>B.VAUGHN</u> CHECKED BY <u>R.RAPP</u>

NOTES:

• DUCTILE IRON PIPE AND FITTINGS AND CAST IRON GRATES O GASKETS AND PVC OR NEOPRENE COATED STRIPS O STEEL SUPPORT ANGLES AND PLATES O U-BOLTS AND H.S. BOLTS, WASHERS AND NUTS • OFFSET PIPE CLAMPS AND EXPANSION ANCHOR BOLTS • EXPANSION JOINT

ALL PIPES, FLANGES AND FITTINGS SHALL BE CLASS 53 DUCTILE IRON. ALL BENDS TO BE SHORT RADIUS, INCLUDING FLANGE & FLARE BENDS, UNLESS ALL BOLTED FLANGE CONNECTIONS. MAKE FINAL PIPE ALIGNMENT AND TIGHTEN U-BOLTS AFTER RAILROAD IN THE CONTRACT LUMP SUM PRICE BID FOR "STRUCTURE DRAINAGE SYSTEM". NO SEPARATE MEASUREMENT OR PAYMENT SHALL BE MADE FOR ANY COMPONENT OF THE STRUCTURE DRAINAGE SYSTEM INCLUDING, PROVIDE PVC OR NEOPRENE-COATED STRIPS, EPOXY-CEMENTED TO THE U-BOLT OR PIPE FOR STRAY CURRENT PROTECTION. THE OUTSIDE COATING FOR D.I. PIPE SHALL BE PAINTED WITH A SHOP

OTHERWISE NOTED. FOR LOCATIONS AND DESIGNATIONS OF DSA & PSA, SEE FRAMING PLAN SHEETS. PIPE LENGTHS SHOWN ALLOW FOR $\frac{1}{8}$ " THICK GASKETS TO BE USED AT TRACK HAS BEEN LAID ACROSS THE BRIDGE. PAYMENT FOR ALL MATERIALS, FABRICATION, INSTALLATION AND ADJUSTMENTS RELATED TO STRUCTURE DRAINAGE SHALL BE INCLUDED BUT NOT LIMITED TO:

PRIME COAT OF INORGANIC ZINC PRIMER AND A FINISH (FIELD) COATING OF ACRYLIC PAINT AS SPECIFIED FOR THE STRUCTURAL STEEL.

SUPERSTRUCTUR 6" I.D. DUCTILE IRON PIPE, CL 6" I.D. DUCTILE IRON PIPE, CL 6" I.D. DUCTILE IRON PIPE, CL 6" I.D. DUCTILE IRON PIPE FL 6" I.D. DUCTILE IRON BLIND | EXPANSION JOINT HEAVY DUTY CAST IRON GRAT PIPE SUPPORT ANGLES (PSA) DIAPHRAGM SUPPORT ANGLES

* INCLUDES U-BOLTS, ANGLES, NUTS, BOLTS, WASHERS AND PLATES.

FOR SUBSTRUCTURE DRAINAGE SYSTEM ESTIMATED QUANTITIES, SEE "STRUCTURE: DRAINAGE DETAILS" SHEET 3 OF 4.

FOR STRUCTURE DRAINAGE SYSTEM, SEE SPECIAL PROVISIONS.

RE DRAINAGE SYSTEM ESTIMATED QUANT		
ITEM	UNIT	TOTAL
LASS 53 (FLANGED BOTH ENDS)	FEET	168′-9 ¹⁵ ⁄16″
LASS 53 (FLANGE & P.E.)	FEET	7′-0 3⁄/ 8″
_ASS 53 (BELL & P.E.)	FEET	38′-7 ¹⁵ ⁄16″
ANGED FITTINGS,250 psiP.R.	LBS	3,070
FLANGES	LBS	200
	EA.	2
ES	EA.	22
*	EA.	12
(DSA) *	EA.	16

_		MEC	NO DKLEN POS S	P- BUR	5709 G 28+12	5BA _ COL 2.88 -	JNTY -S2-		
	SIAII	2 OF	4		_0 1				
SEAL	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH								
Software Ryan Ryan Ranner 2/21/2018	<u>учи д. дарр</u> -556B3197C22B434 /21/2018 DDATNACE								
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NORTH CAROLINA, P.C. nse No. C-1554		nv	REVISI	ONS	D) (0.175	SHEET NO. S7-47		
ix Forks Rd., Suite 200, Raleigh, N.C. 27609		ВҮ	DATE	NO.	BY	DATE	TOTAL		
DATE DWG.NO. 47	2			4			sheets 51		

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SUBSTRUCTURE DRAINAGE SYSTEM ESTIMATED QUANTI	TIES	
ITEM	UNIT	TOTAL
8"I.D. DUCTILE IRON PIPE, CLASS 53 (FLANGED BOTH ENDS)	FEET	207′-9 ¹⁵ / ₁₆ ″
8″I.D. DUCTILE IRON PIPE, CLASS 53 (FLANGE & P.E.)	FEET	12'-6"
8"I.D. DUCTILE IRON PIPE FLANGED FITTINGS, 250 psiP.R.	LBS	2,680
8"I.D. DUCTILE IRON BLIND FLANGES	LBS	126
BALL JOINT	EA.	1
8"I.D. OFFSET PIPE CLAMPS (9")	EA.	10
8"I.D. OFFSET PIPE CLAMPS (1'-0")	EA.	10
8"I.D. OFFSET PIPE CLAMPS (1'-3")	EA.	4

D.I. = DUCTILE IRON

P.L. = PIPE LENGTH

P.E. = PLAIN END

DETAILS

NORTH CAROLINA, P.C.			SHEET NO.				
nse No.C-1554 ix Forks Rd., Suite 200, Raleigh, N.C. 27609	NO.	BY	DATE	NO.	BY	DATE	S7-48
	1			3			TOTAL SHEETS
DATE DWG. NO. 48	2			4			51

* THE PROPOSED LIMITS OF SLOPE PROTECTION ADJACENT TO THE EXISTING SLOPE PROTECTION ARE BASED ON AN APPROXIMATE LOCATION OF THE CURRENT EDGE OF THE EXISTING SLOPE PROTECTION. THE CONTRACTOR SHALL ADJUST THE PROPOSED SLOPE PROTECTION LIMITS BASED ON THE ACTUAL LOCATION OF THE EXISTING SLOPE PROTECTION. IF THE EXISTING SLOPE PROTECTION IS DAMAGED OR PARTIALLY REMOVED DURING THE ABUTMENT OR PIER CONSTRUCTION OPERATIONS FOR THE PROPOSED BRIDGE, THAT EXISTING SLOPE PROTECTION SHALL BE REMOVED BACK TO THE NEAREST VERTICAL CONSTRUCTION JOINT FROM THE TOP OF THE EXISTING BERM TO THE TOE OF THE EXISTING SLOPE. THE LIMITS OF THE PROPOSED SLOPE PROTECTION WILL THEN BE EXTENDED TO MATCH AGAINST THE ADJUSTED EDGE OF EXISTING SLOPE PROTECTION.

TH CARO

SEAL 44886

WGINEER .

RYAN

DocuSigned b

2/21/2018

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NOTES: FOR BERM WIDTHS, ELEVATIONS AND HORIZONTAL DIMENSIONS, SEE GENERAL DRAWING.

FOR SECTIONS A-A, B-B, C-C, & D-D SEE SHEET 2 OF 2.

PROJECT NO. P-5705BA MECKLENBURG COUNTY

STATION: POS STA. 28+12.88 -S2-

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SLOPE PROTECTION

DETAILS

ATURES COMPLETED							
NORTH CAROLINA, P.C.			SHEET NO.				
nse No. C-1554 Six Forks Rd., Suite 200, Raleigh, N.C. 27609	NO.	BY	DATE	NO.	BY	DATE	S7-50
	1			3			TOTAL SHEETS
$\frac{10/17}{\text{DATE}} = \frac{10/17}{10}$	2			4			51

2'-O"LONG #4 BARS

5'-0" 5'-0" 5'-0"

PORTION.

SPA. @ 1'-6"CTS. MAX.

CONST. JT. TO BE NORMAL TO

END BENT CAP OR HORIZONTAL

STRIP WIDTHS MAY VARY IN CURVED

POURING DETAIL

SECTION C-C

 $6 \times 6 - W1.4 \times W1.4$

5

4

SECTION A-A

4″

OPTIONAL POURING DETAIL

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.FOR BERM WIDTH, SEE GENERAL DRAWING.

SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL WITH 2'-O'LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

ADDITIONAL SLOPE PROTECTION REQUIRED TO PROPERLY MATCH UP TO EXISTING SLOPE PROTECTION, AS DESCRIBED ON SHEET 1 OF 2, SHALL BE INCIDENTAL TO THE CONTRACT BID PRICE FOR THE PROVIDED QUANTITY.

BRIDGE

5′-0″

SECTION C'-C'

GENERAL NOTES:

- @ 3+12.88 -S2-	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX.L.F.
MENT 1	175	315
MENT 2	258	465

* QUANTITY SHOWN IS BASED ON 5'-O" POURS PER THE "POURING DETAIL".

PROJECT NO. _____P-5705BA MECKLENBURG STATION: POS STA. 28+12.88 -S2-

	SHEET	2 OF	2					
CAROKANA CAROKA	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH							
SEAL 44886 556B3197C22B434 2/21/2018 RYAN RANNING RANNING 2/21/2018	SLOPE PROTECTION							
· · ///ul/hull/vivi			D	ETA	ILS			
ENT NOT CONSIDERED INAL UNLESS ALL NATURES COMPLETED								
INAL UNLESS ALL NATURES COMPLETED NORTH CAROLINA, P.C. ense No. C-1554			REVIS	SIONS			SHEET NO.	
ense No.C-1554 Six Forks Rd.,Suite 200,Raleigh,N.C.27609	NO.	BY	DATE	NO.	BY	DATE	S7-51	
DATE10/17 DWC_NO51	1			3			TOTAL SHEETS	
DATE <u>10/17</u> DWG. NO. 51	2			4			51	

NOTES:

SEE "GENERAL DRAWING: GENERAL NOTES (SHEET 4 OF 4)" FOR GENERAL NOTES.

MVC = MINIMIM VERTICAL CLEARANCE

⇐ STRUCTURE DRAINAGE OUTLET LOCATION AND DIRECTION.

TOP OF DRILLED PIERS HAVE BEEN SET TO ACCOMODATE A FUTURE FILL SLOPE AND ROADWAY SECTION. SEE "SUBSTRUCTURE - COLUMN AND DRILLED PIER CONSTRUCTION SEQUENCE - PIER".

			PVI STA. EL.= 715. VC = 120	. = 42+ .13 .00′	50.00		
	(-)3 (.00007	E DAT	(-)1 <u>A</u> – A	.8518% <u>1-</u>		
JRVEY -A1- ° 14′ 25″ ₩ A							
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ORARY ROAD . ING	STATI	ME(ON:.	STA. 4	2+59	G 9.46	_ COL -A1-	JNTY =
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ENT NOT CONSIDERED	GENERAL DRAWING BRIDGE ON PROPOSED LEAD TRACK OVER P&N CORRIDOR BETWEEN W 4TH STREET						LEAD IDOR EET
NATURES COMPLETED NORTH CAROLINA, P.C. ense No. C-1554 Six Forks Rd., Suite 200, Raleigh, N.C. 27609 DATE 8/17 DWG. NO. 1	NO. 1	ND вү	W MO REVISI DATE	REH ons NO. 3	ead by	STR DATE	EET sheet no. S8-1 total sheets 39

FOUNDATION NOTES

FOR MICROPILES, SEE GEOTECHNICAL SPECIAL PROVISION.	AT
MICROPILES AT ABUTMENT NO.1 ARE DESIGNED FOR AN ALLOWABLE BEARING CAPACITY OF 70 TONS PER PILE.	MET
INSTALL REINFORCING CASINGS FOR MICROPILES AT ABUTMENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 669 FT AND WITH A PENETRATION OF AT LEAST 10 FT INTO ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS.	IF S AT BELC
USE REINFORCING CASINGS WITH YIELD STRENGTHS OF AT LEAST 45 KSI AND A MINIMUM WALL THICKNESS OF 0.5 IN FOR MICROPILES AT ABUTMENT NO.1.	SID INSI
ONE VERIFICATION TEST IS REQUIRED FOR MICROPILES INSTALLED AT ABUTMENT NO.1.	THEF FOR
FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.	CSL
DRILLED PIERS AT PIER NO.1 ARE DESIGNED FOR AN ALLOWABLE BEARING CAPACITY OF 405 TONS/PIER.CHECK FIELD CONDITIONS FOR THE REQUIRED TIP BEARING OF 30 TSF.	FOR
INSTALL DRILLED PIERS AT PIER NO.1 TO A TIP ELEVATION NO HIGHER THAN 671 FT (LT); 668 FT (CT); 668 FT (RT), SATISFY THE REQUIRED TIP BEARING AND HAVE A PENETRATION OF AT LEAST 9 FT INTO ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS.	PILE 55 ⁻ DRI
DRILLED PIERS AT PIER NO.2 ARE DESIGNED FOR AN ALLOWABLE BEARING CAPACITY OF 365 TONS/PIER.CHECK FIELD CONDITIONS FOR THE REQUIRED TIP BEARING OF 30 TSF.	PILI TES
INSTALL DRILLED PIERS AT PIER NO.2 TO A TIP ELEVATION NO HIGHER THAN 661 FT (LT); 656 FT (CT); 656 FT (RT), SATISFY THE REQUIRED TIP BEARING AND HAVE A PENETRATION OF AT LEAST 14 FT INTO WEATHERED ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS.	SEE
SPT ARE REQUIRED FOR DRILLED PIERS.FOR SPT TESTING,SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.THE REQUIRED N60 SPT VALUE FOR DRILLED PIERS AT PIER	

NOS.1 AND 2 IS 100 BLOWS WITH 12 INCHES OR LESS PENETRATION.

FOUNDATION LAYOUT

THE CONTRACTORS OPTION, SLURRY CONSTRUCTION MAY BE USED FOR THE STRUCTION OF DRILLED PIERS AT PIER NOS.1 AND 2. IF SLURRY CONSTRUCTION THODS ARE USED, THEN POLYMER SLURRY IS REQUIRED.

SLURRY CONSTRUCTION METHODS ARE USED TO CONSTRUCT THE DRILLED PIERS PIER NOS.1 AND 2, INSTALL A TEMPORARY CASING A MINIMUM OF 20 FT. _OW DRILLING GRADE PRIOR TO BEGINNING SLURRY EXCAVATION.

INSPECTIONS ARE REQUIRED FOR DRILLED PIERS AT PIER NOS.1 AND 2.FOR SID SPECTIONS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

RMAL INTEGRITY PROFILING IS REQUIRED FOR DRILLED PIERS AT PIER NOS.1 AND 2. THERMAL INTEGRITY PROFILING, SEE GEOTECHNICAL SPECIAL PROVISION.

TUBES AND CSL TESTING ARE REQUIRED FOR DRILLED PIERS.FOR CSL TESTING, SEE CTION 411 OF THE STANDARD SPECIFICATIONS.

PILES, SEE GEOTECHNICAL SPECIAL PROVISION.

LES AT ABUTMENT NO.2 ARE DESIGNED FOR AN ALLOWABLE BEARING CAPACITY OF TONS PER PILE.

IVE PILES AT ABUTMENT NO.2 TO A REQUIRED BEARING CAPACITY OF 110 TONS PER

ST THE FIRST PRODUCTION PILE AT ABUTMENT NO.2 WITH THE PDA.FOR PDA TESTING, GEOTECHNICAL SPECIAL PROVISION.

NOTES:

◄▷ INDICATES MICRO-PILE TO BE BATTERED IN DIRECTION OF ARROW AT THE RATE SHOWN.

◀IF INDICATES STEEL PILE TO BE BATTERED IN DIRECTION OF ARROW AT THE RATE SHOWN.

ALL DIMENSIONS ARE PARALLEL OR NORMAL TO FILL FACE ABUTMENTS, € PIERS, OR WORKLINE.

FOR FOUNDATION ELEVATIONS AND DETAILS, SEE PIER AND ABUTMENT DETAILS.

ALL PILES AT ABUTMENT 1 ARE 7"Ø MICRO-PILES. ALL PILES AT ABUTMENT 2 ARE STEEL HP 14×73.

ALL DIMENSIONS TO BATTERED PILES ARE AT BOTTOM OF CAP ELEVATION.

U.N.O. = UNLESS NOTED OTHERWISE

WORK POINT 4 STA. 43+24.96 -A1-

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David Hawkins	SHEET	2 OF	4				
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2/21/2018	FOUNDATION LAYOUT					Т	
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		OTAL BILL	OF MATERI	_ A L									
4'-6″DIA. DRILLED PIERS DT IN SOIL	PDA TESTING	SID INSPECTIONS	SPT TESTING	THERMAL INTEGRITY PROFILER	CSL TESTING	REINFORCED CONCRETE DECK SLAB	CLASS AA CONCRETE	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	APPROX. 231,350 LBS. STRUCTURAL STEEL	PAINTING OF STRUCTURAL STEEL	HP 14 Steel	x 73 PILES
L.F.	EACH	EACH	EACH	EACH	EACH	SQ. FEET	CU. YARDS	LBS.	LBS.	LUMP SUM	LUMP SUM	NO.	L.F.
						2,822.1				LUMP SUM	LUMP SUM		
							50.9	5,244					
53.0		3	3	3	3		38.9	19,254	6,054				
33.0		3	3	3	3		35.8	23,230	7,985				
							55.8	5,695				12	420.0
86.0	1	6	6	6	6	2,822.1	181.4	53,423	14,039	LUMP SUM	LUMP SUM	12	420.0
											•		

_	OF MATERIAL									
	CONCRETE PARAPET	RIP RAP CLASS II (2'-O" THICK)	GEOTEXTILE FOR DRAINAGE	SELF- LUBRICATING EXPANSION BEARING ASSEMBLIES	STRUCTURE DRAINAGE SYSTEM AT STA.42+59.46 -A1-	APPLICATION OF BRIDGE COATING	ABESTOS ASSESSMENT			
	L.F.	TONS	SQ. YDS.	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM			
	256.4			LUMP SUM	LUMP SUM	LUMP SUM				
		350	390			LUMP SUM				
						LUMP SUM				
						LUMP SUM				
		260	290			LUMP SUM				
	256.4	610	680	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM			

EXISTING SINGLE SPAN GIRDERS AND DECK AND EXISTIN AS SHOWN IN ITEMS SHALL BASIS AS "RE AT STATION

NG BACKWALLS SHALL BE REMOVED	PROJE		٥٧	P-	570	БВА	
N THE PLANS.REMOVAL OF THESE _ BE PAID FOR ON A LUMP SUM	MECKLENBURG COUNTY						JNTY
EMOVAL OF EXISTING STRUCTURE 42+59.46 -A1-".	STATI	ON:	STA.4	2+59	.46	-A1-	
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P28 P28

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GENERAL NOTES:

ASSUMED LIVE LOAD = AREMA E80 OR ALTERNATE LIVE LOAD

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 16TH EDITION OF AREMA'S MANUAL FOR RAILWAY ENGINEERING, VOL. 2, STRUCTURES", AND NORFOLK SOUTHERN CORPORATION'S "GUIDELINES FOR DESIGN OF GRADE SEPARATED STRUCTURES UNDERPASS GRADE SEPARATION DESIGN CRITERIA".

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AREMA MANUAL FOR RAILWAY ENGINEERING, VOL. 2, CH. 9, "SEISMIC DESIGN FOR RAILWAY".

REINFORCING STEEL SHALL BE ASTM DESIGNATION A615, GRADE 60. ALL DIMENSIONS RELATING TO BAR SPACING ARE TO BAR CENTERS UNLESS NOTED. FABRICATION TO BE IN ACCORDANCE WITH THE "MANUAL OF STANDARD PRACTICE", A.C.I. 315-80. ALL REINFORCING IN THE CONCRETE DECK SLAB AND PARAPETS SHALL BE EPOXY COATED.

EXPANSION JOINT MATERIAL SHALL BE EITHER RUBBER OR CORK CONFORMING WITH AASHTO SPECIFICATIONS M-153-84 EXCEPT AS SHOWN ON THE PLANS OR IN THE SPECIAL PROVISIONS.CELLULAR AND BULB TYPE WATERSTOPS AND RUBBER JOINT COMPOUNDS SHALL BE AS SHOWN ON THE PLANS AND IN THE SPECIAL PROVISIONS.

STRUCTURE DRAINAGE SYSTEM: METAL DRAINS BEHIND ABUTMENTS AND IN BALLAST TROUGH OF BRIDGE, INCLUDING DUCTILE IRON PIPE COLLECTOR SYSTEM, SHALL BE AS SHOWN ON THE PLANS AND OUTLINED IN THE SPECIAL PROVISIONS. DETAILS OF THE DRAINAGE SYSTEM SHALL BE SUBMITTED TO THE CHIEF ENGINEER BRIDGES AND STRUCTURES, NORFOLK SOUTHERN CORPORATION, ATLANTA, GA. FOR APPROVAL.

DAMPPROOFING: (PIER COLUMNS UP TO GROUND LINE, BACK OF BACKWALLS AND ABUTMENT SEATS, AND BACK OF WINGS SHALL BE DAMPPROOFED WITH METHOD "B" DAMPPROOFING.

WATERPROOFING: BRIDGE DECK AND ALL CONSTRUCTION JOINTS WHICH WILL BE COVERED BY FILL SHALL BE WATERPROOFED WITH A COLD LIQUID-APPLIED ELASTOMERIC MEMBRANE. FOR WATERPROOFING, SEE SPECIAL PROVISIONS.

WATERPROOFING IS REQUIRED AT THE FOLLOWING LOCATIONS: 1. BRIDGE DECK AND INSIDE OF CONCRETE PARAPET AS SHOWN ON "SUPERSTRUCTURE TYPICAL SECTION" SHEET.

SECTION" SHEET. 2. ALONG FULL CIRCUMFERENCE OF EACH BOTTOM OF COLUMN TO TOP OF DRILLED PIER

3. ALONG FILL FACE OF HORIZONTAL CONSTRUCTION JOINT AT TOP OF FOOTING ELEVATION AT EACH ABUTMENT.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES", JANUARY 2018, NORTH CAROLINA DEPARTMENT OF TRANSPORTATION (HEREIN CALLED STANDARD SPECIFICATIONS), EXCEPT AS NOTED HEREIN, ELSEWHERE ON PLANS, OR IN THE SPECIAL PROVISIONS (STRUCTURAL STEEL IN ACCORDANCE WITH CURRENT AREMA SPECIFICATIONS).

ALL CONCRETE SHALL BE 4,500 PSI CLASS AA CONCRETE WITH NO.57 OR 67 COARSE AGGREGATE AND SHALL BE AIR-ENTRAINED. MINIMUM CEMENT CONTENT PER CUBIC YARD OF CONCRETE SHALL BE 6.5 BAGS. NO SUBSTITUTION OF FLYASH, BLAST FURNACE SLAG OR OTHER MATERIAL WILL BE PERMITTED IN MEETING THIS MINIMUM CEMENT REQUIREMENT. CHAMFER ALL EXPOSED EDGES AND CORNERS 3/4 "EXCEPT AS NOTED. THE USE OF GROUND GRANULATED BLAST FURNACE SLAG IS NOT PERMITTED IN THIS STRUCTURE.

CONTROL OF WORK: ALL WORK INVOLVED IN THE CONSTRUCTION OF THE RAILWAY STRUCTURE SHALL BE PERFORMED SATISFACTORY TO THE ENGINEER AND COMPLIANT WITH THE DESIGN STANDARDS OF NORFOLK SOUTHERN RAILWAY COMPANY. ALL METHODS OF HANDLING THE WORK AFFECTING THE SAFETY OF RAIL OPERATIONS MUST BE APPROVED BY THE RAILWAY COMPANY, AS A SUBMITTAL THROUGH THE ENGINEER, AT LEAST 2 WEEKS BEFORE PROCEEDING WITH THAT PORTION OF THE WORK. RAIL TRAFFIC SHALL, AT ALL TIMES, BE MAINTAINED AND PROTECTED. THE CONTRACTOR SHALL NOT AT ANY TIME DELAY OR INTERFERE WITH RAIL OPERATIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

FOR SELF-LUBRICATING EXPANSION BEARING ASSEMBLIES, SEE SPECIAL PROVISIONS.

FOR CONDUIT IN PARAPETS, SEE SPECIAL PROVISIONS.

FOR PORTLAND CEMENT, SEE SPECIAL PROVISIONS.

FOR FINE AND COARSE AGGREGATE, SEE SPECIAL PROVISIONS.

SEE "STRUCTURAL STEEL DETAILS" SHEET FOR STRUCTURAL STEEL NOTES.

FOR BACKFILL BEHIND ABUTMENTS AND OTHER BACKFILL AROUND THE STRUCTURE, SEE SPECIAL PROVISION "BACKFILLING AROUND STRUCTURES".

FOR PAINTING STRUCTURAL STEEL, SEE SPECIAL PROVISONS.

FOR WATERSTOPS, SEE SPECIAL PROVISIONS.

FOR ELASTOMERIC FLASHING, SEE SPECIAL PROVISIONS.

FOR RUBBER JOINT COMPOUNDS, SEE SPECIAL PROVISIONS.

FOR STRUCTURE DRAINAGE SYSTEM, SEE SPECIAL PROVISIONS.

FOR STRUCTURAL STEEL, SEE SPECIAL PROVISIONS.

FOR RAILROAD TRACKWORK, SEE RAILROAD TRACKWORK PLANS.

FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR CONCRETE PARAPET, SEE SPECIAL PROVISIONS.

FOR WATERPROOFING, SEE SPECIAL PROVISIONS.

ALL BAR SUPPORTS AND ALL INCIDENTAL REINFORCING STEEL USED IN THE DECK AND PARAPET SHALL BE EPOXY COATED IN ACCORDANCE WITH THE NCDOT STANDARD SPECIFICATIONS.

THE CONCRETE DECK, STEEL GIRDERS AND CONCRETE BACKWALL OF THE EXISTING SINGLE SPAN BRIDGE SHALL BE REMOVED TO THE SEAT ELEVATION OF EXISTING ABUTMENT WALLS, APPROXIMATE EL. 703.0 ± AS SHOWN IN THE PLANS.

THE EXISTING STRUCTURE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE.SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON THE DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR 'REMOVAL OF EXISTING STRUCTURE AT STATION 42+64.46 -A1-".

FOR TEMPORARY RAILROAD SHORING, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT, SEE SPECIAL PROVISIONS.

FOR APPLICATION OF BRIDGE COATING, SEE SPECIAL PROVISIONS.

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RAWINGS

GENERAL PLAN & ELEVATION (SHEET 1 OF 4) FOUNDATION LAYOUT AND FOUNDATION NOTES (SHEET 2 OF 4) LOCATION SKETCH AND TOTAL BILL OF MATERIAL (SHEET 3 OF 4) GENERAL NOTES (SHEET 4 OF 4) TRUCTION YPICAL SECTION DECK DETAILS PLAN OF DECK - SPAN A PLAN OF DECK - SPAN B PLAN OF DECK - SPAN C FRAMING PLAN & GIRDER DETAILS - SPAN A AND SPAN C FRAMING PLAN & GIRDER DETAILS - SPAN B TRUCTURAL STEEL DETAILS BEARING DETAILS (SHEET 1 OF 2) BEARING DETAILS (SHEET 2 OF 2) XPANSION PLATE DETAILS ARAPET DETAILS (SHEET 1 OF 2) PARAPET DETAILS (SHEET 2 OF 2) METAL HANDRAIL DETAILS (SHEET 1 OF 2) METAL HANDRAIL DETAILS (SHEET 2 OF 2) BILL OF MATERIAL UTMENT 1 SHORING (SHEET 1 OF 2) UTMENT 1 SHORING (SHEET 2 OF 2) UTMENT 1 (SHEET 1 OF 3) UTMENT 1 (SHEET 2 OF 3) UTMENT 1 (SHEET 3 OF 3) ER 1 (SHEET 1 OF 2) ER 1 (SHEET 2 OF 2) LUMN AND DRILLED PIER CONSTRUCTION SEQUENCE - PIER ER 2 (SHEET 1 OF 2) ER 2 (SHEET 2 OF 2) UTMENT 2 SHORING UTMENT 2 (SHEET 1 OF 3) UTMENT 2 (SHEET 2 OF 3) UTMENT 2 (SHEET 3 OF 3) AGE DETAILS (SHEET 1 OF 3) AGE DETAILS (SHEET 2 OF 3) GE DETAILS (SHEET 3 OF 3)

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CONSTRUCTION SEQUENCE

- 1. CLOSE EXISTING SPUR TRACK TO RAIL TRAFFIC.
- 2. REMOVE EXISTING SPUR TRACK SUPERSTRUCTURE AND BACKWALL TO LIMITS SHOWN.
- 3. INSTALL TEMPORARY SHORING AT ABUTMENT 1 AND ABUTMENT 2.
- 4. ROUGH GRADE TO LIMITS SHOWN ON "RIP RAP DETAILS" SHEET.
- 5. INSTALL PROPOSED ABUTMENT AND PIER SUBSTRUCTURE.
- 6. BACKFILL TO PROPOSED GRADE AND REMOVE TEMPORARY SHORING.
- 7. FINISH GRADE, INSTALL SUPERSTRUCTURE AND RIP RAP SLOPE PROTECTION, TRACK CONTRACTOR TO CONSTRUCT TRACK AND OPEN TO RAIL TRAFFIC.

NOTES:

EXISTING BACKWALL SHALL BE REMOVED TO THE LIMITS SHOWN. REMOVAL SHALL NOT EXCEED THE LIMITS SHOWN IN THE PLANS AND SHALL NOT BE UNDERTAKEN UNTIL APPROVAL HAS BEEN GIVEN BY THE ENGINEER. REINFORCING WITHIN THE LIMITS OF REMOVAL SHALL BE REMOVED TO A MINIMUM DEPTH OF 4" BELOW PROPOSED BACKWALL LIMITS. HOLE SHALL BE FILLED WITH CLASS AA CONCRETE.

REMOVE EXISTING BACKWALL TO LIMITS SHOWN									
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22'-0"OUT TO OUT 11'-0" 11'-0″ 1'-0" 10'-0" 10'-0" LIMITS OF COLD LIQUID-APPLIED 6¹/2″ ELASTOMERIC WATERPROOFING MEMBRANE 28-#4 "B" BARS @ 9" = 20'-3" (TOP OF SLAB) Δ″ 1-#4 *"*B" BAR 9″MIN.(LIMIT OF (TOP OF SLAB) WATERPROOFING MEMBRANE. TYPICAL EACH PARAPET) € SURVEY -A1-9″WIDE END DAM @ ALUMINUM END OF EACH SPAN (SPAN B) HANDRAIL TOP OF RAIL AND PROFILE GRADE LINE -A1-1"END DAM #5 "A" BARS 9" MIN. BALLAST SEE DETAIL "A" 1″MIN.CLR. TO FLANGE 3″MIN. AND FORM CLR. 3'-0" (TYP.) 8" --+⊕ 1" SLOPE 1" SLOPE € ¾″TRIANGULAR DRIP NOTCH • FULL LENGTH OF BRIDGE • | • | • | (TYP.EA.SIDE) • i 48" WEB DEPTH (ASSUMED) (TYPICAL ALL SPANS) + 1 STAY-IN-PLACE € 6″Ø D.I.P. DRAINAGE COLLECTOR SYSTEM (TYP.) METAL FORM (TYP.) 1'-9" | 1'-9" 1'-9" 1'-9" € STEEL PLATE 8¹/4″ 8¹/4″ GIRDER (TYP.) € G3 € G1 € G2 € G4 € G5 4-#4 "B" BARS @ $8\frac{1}{2}$ " = $2^{-1}\frac{1}{2}$ " (TYP.BETWEEN GIRDERS BOTTOM OF SLAB) 5 SPACES @ 3'-6" = 17'-6" 2'-3" HALF SECTION HALF SECTION END DIAPHRAGM INTERMEDIATE DIAPHRAGM TYPICAL SECTION . . (ALL SPANS SIMILAR) 10″MIN.SPAN A & C 9″MIN.SPAN B € GIRDER TOP OF STAY-IN-PLACE FORM LEVEL WITH TOP OF GIRDER TOP FLANGE, 1"SPAN A & C 1″MIN. 1'-1" 2" SPAN B END DAM <u>detail "A"</u>

NOTES FOR SIGNAL CONDUIT IN PARAPET

SIGNAL CONDUIT TO BE 4"DIA. IN ACCORDANCE WITH UNDERWRITER'S LABORATORY SPECIFICATIONS.

PROVISIONS SHALL BE MADE FOR EXPANSION BETWEEN DECK SLABS AT EXPANSION JOINTS AT PIERS 1 & 2 (GALVANIZED EXPANSION FITTINGS).

COUPLING SHALL BE PROVIDED BEHIND BACKWALL OF ABUTMENT 1 AND 2 FOR CONNECTION TO 4"DIA.RIGID PIPE (RIGID PIPE BY RAILWAY COMPANY).

FOR SIGNAL CONDUIT IN PARAPET SEE PROJECT SPECIAL PROVISIONS.

NOTES:

ALL REINFORCING STEEL IN THE DECK AND PARAPETS SHALL BE EPOXY COATED. CLEAR COVER TO ALL REINFORCING IS 2" MINIMUM UNLESS NOTED OTHERWISE.

FOR ALUMINUM HANDRAIL DETAILS, SEE "METAL HANDRAIL DETAILS" SHEETS.

FOR CONCRETE PARAPET DETAILS, SEE "CONCRETE PARAPET DETAILS"SHEET.

DESIGN INCLUDES WEIGHT OF 6"ADDITIONAL BALLAST TO ACCOUNT FOR FUTURE RESURFACING OF TRACK.

PROJECT	NO	P-5705BA

MECKLENBURG

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STATION: STA. 42+59.46 -A1-

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anse No. 6-1554 Six Forks Rd., Suite 200, Raleigh, N.C. 27609	NO.	BY	DATE	NO.	BY	DATE	58-6		
	1			3			TOTAL SHEETS		

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NOTES:

ALL REINFORCING SHALL BE EPOXY COATED.

FOR SECTION A-A & B-B, SEE "SUPERSTRUCTURE DETAILS" SHEET.

FOR DECK DRAIN DETAIL, SEE DETAIL "B" ON "STRUCTURE DRAINAGE DETAILS" SHEET 3 OF 3.

FOR CONCRETE PARAPET DETAILS. SEE "CONCRETE PARAPET DETAILS" SHEETS.

P-5705BA PROJECT NO. MECKLENBURG ___COUNTY STATION: STA. 42+59.46 - A1-Docusigned by: David Hawkins TH CARO STATE OF NORTH CAROLINA WESS/ DEPARTMENT OF TRANSPORTATION SEAL 27812 RALEIGH SUPERSTRUCTURE W. HA PLAN OF DECK - SPAN A DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED HNTB NORTH CAROLINA, P.C. NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 SHEET NO. REVISIONS S8-8 BY DATE NO. BY DATE NO. total sheets 39 1 3 DWG.NO.8 2 4

		70′-6″ 🕻	JT.@PIER 1 TO 🕻 J	T.@PIER 2			
			86-#5A1 BARS @ 9″=	63'-9″			<u>9¹¹/16</u> ″
							<u>1'-07/16</u> "
			23'-6"			23'-6″	
	12'-0"		12'-0″		12'-0"	12'-0"	5′-3″
						FOR REINFORCING REQUIRED PARAPET, SEE "CONCRETE PAR DETAILS" SHEETS (TYP.)	FOR APET
			A				
			#4B3 (B0T.)	<u>L BG1</u>		DTSEE	#4B3 (B0T.)
				<u>€ вс2</u> /		6-#4B4 (BC TION) 26 *4B4/LINE	<u>2-#5A10</u> @ <u>6"(TOP)</u> (ALONG SKEW) <u>#5A1</u> #5A2
(TOP-SEE	 Щ ——			<u>© BG3</u>			
30-#4B4 ECTION) 3(#4B4/LI DWN 			<u>¢ BG4</u>		- T_PI 22-#4 ALT. ALT.	<u>#5A1</u> <u>#5A3</u>
-#4B3 & PICAL SI	#4B3 & 1 T. AS SH			O	<u>35</u> /		<u>2-#5A10</u> <u>@ 6" (BOT.)</u> (AL ONG SKEW)
€0 1 1 1	AL	# 4B3 (TOP)		<u>¢</u>	BC6 # 480 # 10b	(TYP. #4_"B")	++++++++++++++++++++++++++++++++++++++
						Q 1/2" OPEN JOINT IN PARAPET (TYP. EA.SIDE)	
				23'-6"		23	<u>′-6″</u>
					·		113/ //
			66-#5A1 BARS @	1'-0"= 65'-0"			
			70′-6″(W.P.2	TO W.P.3)			

<u> Plan - Span B</u>

-78°-00'-00″

NOTES:

ALL REINFORCING SHALL BE EPOXY COATED.

FOR SECTION A-A & B-B, SEE "SUPERSTRUCTURE DETAILS" SHEET.

FOR DECK DRAIN DETAIL, SEE DETAIL "B" ON "STRUCTURE DRAINAGE DETAILS" SHEET 3 OF 3.

FOR CONCRETE PARAPET DETAILS. SEE "CONCRETE PARAPET DETAILS" SHEETS.

PROJECT NO. _____P-5705BA MECKLENBURG COUNTY STATION: STA. 42+59.46 -A1-Docusigned by: David Hawkins TH CARO STATE OF NORTH CAROLINA · DEPARTMENT OF TRANSPORTATION SEAL 27812 RALEIGH SUPERSTRUCTURE VIO W. HA PLAN OF DECK - SPAN C DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED HNTB NORTH CAROLINA, P.C. NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 SHEET NO. REVISIONS S8-10 BY DATE NO. BY DATE NO. total sheets 39 1 3 DWG.NO. 10 2 4

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BOTTOM FLANGE GIRDER END DETAILS - SPAN C

28 Ω 0012DEL

BOTTOM FLANGE GIRDER END DETAILS - SPAN B

HNTB NC Lice 343 E. HNTB DRAWN BY J.BAYNE CHECKED BY V.KOLLIPARA

NOTES:

ALL DIMENSIONS ON THIS DRAWING ARE HORIZONTAL.

ABUTMENT FILL FACE, C PIER AND C BEARINGS ARE PARALLEL.

NO SHOP CAMBER REQUIRED.

REFER TO "STRUCTURAL STEEL DETAILS" SHEET FOR:

- STRUCTURAL STEEL NOTES
- DIAPHRAGM DETAILS
- STIFFENER AND CONNECTOR ₱ DETAILS
- SHEAR CONNECTOR DETAILS
- FLANGE TO WEB WELD DETAIL

FOR BEARINGS, SEE ``BEARING DETAILS" SHEET.

FOR DRAIN PIPE SUPPORT DETAILS, SEE "TYPICAL SECTION' SHEET.

FLANGE AND WEB SHOP SPLICES SHALL BE MADE WITH FULL PENETRATION GROOVE WELDS. SEE DETAILS ON "STRUCTURAL STEEL DETAILS" SHEET. FABRICATOR IS TO SHOW WELD CONFIGURATION AND JOINT PREPARATION ON SHOP DRAWINGS FOR APPROVAL.

FLANGE AND WEB SHOP SPLICE SHALL BE STAGGERED LONGITUDINALLY A MINIMUM OF 2'-O". SEE "STRUCTURAL STEEL DETAILS" SHEET FOR DETAIL.

FC = FRACTURE CRITICAL

INT = NON-FRACTURE CRITICAL MEMBERS OR COMPONENTS REQUIRING IMPROVED NOTCH TOUGHNESS.

U.O.N. = UNLESS OTHERWISE NOTED

DETAIL "A" (RIGHT SIDE OF WEB SHOWN, LEFT SIDE SIMILAR)

Docusigned by: David Hawkins David Hawkins CARO/	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH								
18	FRAMING PLAN								
ENT NOT CONSIDERE INAL UNLESS ALL NATURES COMPLETED	SPAN B								
NORTH CAROLINA, P.C.				REVISI	ONS			SHEET NO.	
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STRUCTURAL STEEL NOTES DESIGN DATA:

- GIRDER FLANGES AND WEB: A709, GRADE 50.
- MISCELLANEOUS MATERIAL: A709, GRADE 50.

ARE SHOWN AT 60°F.

GIRDER BOTTOM FLANGE PLATE INCLUDING WEB TO FLANGE WELD AND TENSION ZONE OF WEB PLATE ARE DESIGNED FRACTURE CRITICAL (FC) MEMBERS. THESE COMPONENTS SHALL MEET THE IMPACT TEST REQUIREMENTS SET FORTH IN THE FRACTURE CONTROL PLAN OF THE AREMA MANUAL FOR ZONE 2 REQUIREMENTS.

NORFOLK SOUTHERN CORPORATION SHALL BE FURNISHED COPIES OF MILL TEST REPORTS FOR ALL MATERIALS EXCEPT MISCELLANEOUS PLATES AND SHAPES. REPORTS SHALL INDICATE COMPLIANCE WITH ALL SPECIFIED REQUIREMENTS.

SHOP INSPECTION SHALL BE BY NORFOLK SOUTHERN CORPORATION OR ITS AUTHORIZED AGENT. SEE STRUCTURAL STEEL SPECIAL PROVISION FOR ADDITIONAL WELDING INSPECTION OF FLANGE PLATE TO WEB PLATE WELDS.

FOR PAINTING STRUCTURAL STEEL.SEE SPECIAL PROVISIONS.

BOLTED CONNECTIONS SHALL BE MADE WITH $\frac{7}{8}$ " Ø ASTM A325, TYPE 1 HIGH STRENGTH BOLTS WITH HEAVY HEX HEAD, HEAVY HEX NUT AND HARDENED WASHERS IN ACCORDANCE WITH A.R.E.M.A. SPECIFICATIONS USING THE TURN OF THE NUT METHOD. DIRECT TENSION INDICATORS SHALL NOT BE USED.

SHOP DRAWINGS SHALL BE APPROVED BY THE CHIEF ENGINEER - BRIDGES AND STRUCTURES, NORFOLK SOUTHERN CORPORATION. ATLANTA. G.A. SHOP DRAWINGS SHALL BE LABELED "NORFOLK SOUTHERN MP NS-378.00".

THE PLANS.

B695.

ANCHOR BOLTS SHALL BE 11/4 "Ø IN ACCORDANCE WITH A.R.E.M.A. SPECIFICATIONS AND SHALL BE GROUTED IN FORMED HOLES AFTER GIRDERS ARE ERECTED.

BEARING PADS SHALL BE USED WHENEVER STEEL MASONRY PLATE, OR OTHER STEEL BEARING PLATE, BEARS ON CONCRETE. PADS SHALL BE PREFORMED FABRIC BEARING PADS, 1/2 "THICK. FOR PAD REQUIREMENTS, SEE STRUCTURAL STEEL SPECIAL PROVISIONS.

SHEAR CONNECTORS ON GIRDERS MAY BE SHIFTED AS NECESSARY TO CLEAR FLANGE SPLICE WELDS.

WEB SHOP SPLICES ARE PERMITTED TO LIMIT THE MAXIMUM REQUIRED WEB PIECE LENGTHS TO 45'-O". PERMITTED WEB SHOP SPLICES SHALL NOT BE LOCATED WITHIN 15'-O" OF MAXIMUM DEAD LOAD DEFLECTION AND SHALL BE LOCATED 6"MIN.FROM CONNECTOR PLATE OR INTERMEDIATE STIFFENER WELDS.FLANGE AND WEB SHOP SPLICES SHALL CONFORM TO SHOP SPLICE DETAILS SHOWN ON THE PLANS.

STRUCTURAL STEEL ELEMENTS DENOTED AS "FC" ARE FRACTURE CRITICAL AND SHALL MEET IMPACT TEST REQUIREMENTS SET FORTH IN THE FRACTURE CONTROL PLAN OF THE AREMA MANUAL. CHAPTER 15. SECTION 1.14. NOTCH TOUGHNESS REQUIREMENTS AND TESTING SHALL BE BASED ON ZONE 2 REQUIREMENTS.

STRUCTURAL STEEL ELEMENTS DENOTED AS "INT" SHALL MEET IMPACT TEST REQUIREMENTS SET FORTH IN THE FRACTURE CONTROL PLAN OF THE AREMA MANUAL. CHAPTER 15. SECTION 1.2. TESTING SHALL BE BASED ON ZONE 2 REQUIREMENTS.

FOR DETAILS OF DIAPHRAGM SUPPORT ANGLES (DSA), SEE "STRUCTURE DRAINAGE DETAILS" SHEET 3 OF 3. FC = FRACTURE CRITICAL

INT = NON-FRACTURE CRITICAL MEMBERS OR COMPONENTS REQUIRING IMPROVED NOTCH TOUGHNESS.

2/21/201

STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS:

• ANCHOR BOLTS FOR BEARING DEVICES SHALL CONFORM TO ASTM F1554, GRADE 105. ANCHOR BOLTS, NUTS, AND PLATE WASHERS SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM F2329.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL UNLESS OTHERWISE NOTED. ALL DIMENSIONS

BOLT HOLES IN STRUCTURAL STEEL MEMBERS SHALL BE STANDARD SIZE UNLESS OTHERWISE INDICATED ON

HIGH STRENGTH BOLTS.NUTS & WASHERS SHALL BE MECHANICALLY GALVANIZED IN ACCORDANCE WITH ASTM

ALL WELDING CONNECTIONS SHALL BE MADE WITH SERIES E70 WELDING ELECTRODES.

P-5705BA PROJECT NO.

MECKLENBURG

David Hawkins									
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NORTH CAROLINA, P.C.			REVIS	sions			SHEET NO.		
ense No. C-1554 Six Forks Rd., Suite 200, Raleigh, N.C. 27609	NO.	BY	DATE	NO.	BY	DATE	S8-13		
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A DATE <u>9/17</u> DWG. NO. 13	2			4			39		

			В	EARING	PLATE	SCHEDL	ILE				
					DIMENS	EONS (in)					
GIRDER	FI	XED BEAF	RING (FBA	4-1)	EXPANSION BEARING (E						
	"A"	<i>"</i> B"	"C"	″D″	″E″	<i>"</i> F"	"G"	″H″	ر//		
AG1	11/2″	31/4″	51/4″	71/2″	11/2"	11/2"	23/4″	63⁄4″	91/2		
AG2	11/2"	3″	5″	71/2″	11/2"	11/2"	25/8″	65⁄8″	91/2		
AG3	1 ¹ /2″	23⁄4″	4 ³ ⁄ ₄ ″	71/2″	11/2"	11/2"	23⁄8″	6 ³ /8″	91/2		
AG4	1 ¹ /2″	21/2"	4 ¹ /2″	71/2″	11/2"	11/2"	21/8″	6 ¹ /8″	91/2		
AG5	1 ¹ /2″	21/4″	4 ¹ / ₄ "	71/2″	11/2"	11/2"	17⁄8″	57⁄8″	91/2		
AG6	11/2"	2″	4″	71/2″	11/2"	11/2"	15⁄8″	55⁄8″	91/2		
CG1	11/2″	23⁄4″	43⁄4″	71/2″	11/2"	11/2"	3 [!] /8″	7 ¹ /8″	91/2		
CG2	11/2″	25/8″	45⁄8″	71/2″	11/2"	11/2"	3″	7″	91/2		
CG3	11/2"	21/2"	4 ¹ /2″	71/2″	11/2"	11/2"	23⁄4″	6¾″	91/2		
CG4	11/2″	23/8″	4 ³ / ₈ ″	71/2″	11/2"	11/2"	25/8″	65⁄8″	91/2		
CG5	11/2"	21/8"	4 ¹ / ₈ ″	71/2"	11/2"	11/2"	21/2"	6 ¹ /2″	91/2		
CG6	11/2"	2″	4″	71/2"	11/2"	11/2"	21/4″	61/4″	91/2		

ANCHOR BOLT DETAIL

NOTES:

ANCHOR BOLTS, SLIDING PLATE (EXPANSION BEARING) AND MASONRY PLATE (FIXED AND EXPANSION BEARINGS) SHALL BE HOT DIPPED GALVANIZED.

ALL PLATE SURFACES SHALL BE PAINTED WITH A 3 COAT PAINT SYSTEM EXCEPT AS SPECIFIED BELOW.

- (A) THE SLIDING PLATE (EXPANSION BEARING) SHALL NOT BE PAINTED BUT SHALL RECEIVE A COAT OF LUBRICATION.
- (B) THE MASONRY PLATE (FIXED AND EXPANSION BEARINGS) SHALL NOT BE PAINTED.
- (C) THE BOTTOM SURFACE OF THE SOLE PLATE SHALL NOT BE PAINTED BUT SHALL RECEIVE A SINGLE COAT OF PRIMER APPLIED IN THE SHOP.
- (D) THE TOP SURFACE OF THE SOLE PLATE SHALL NOT BE PAINTED IN THE VICINITY OF THE WELD BETWEEN THE SOLE PLATE AND THE BOTTOM FLANGE.

FOR PAINTING STRUCTURAL STEEL, SEE SPECIAL PROVISIONS.

FOR SELF LUBRICATING EXPANSION BEARING ASSEMBLIES, SEE SPECIAL PROVISIONS.

BEARING PLATE SCHEDULE												
		DIMENSIONS (in)										
GIRDER	FI:	FIXED BEARING (FBA-2)			EXPANSION BEARING (EBA-2							
	"A"	<i>"</i> B"	"C "	″D″	"E"	<i>"</i> F <i>"</i>	"G"	<i>"</i> H"	"၂"			
BG1	11/2"	3 /8″	5 /8″	71/2″	1 ¹ /2″	11/2"	25⁄8″	65⁄8″	91/2″			
BG2	1 ¹ /2″	21⁄8″	41⁄8″	7 ¹ /2″	1 /2″	1 /2″	2 ³ /8″	6 ³ /8″	91/2″			
BG3	11/2″	2¾″	4 ³ ⁄4″	7 ¹ /2″	11/2"	1 /2″	2 ¹ /4″	6 ¹ /4″	91/2″			
BG4	11/2″	21/2″	4 ¹ /2″	7 ¹ /2″	11/2″	11/2″	2 /8″	6 ¹ /8″	91/2″			
BG5	11/2"	21/4″	4 ¹ /4	71/2″	11/2"	11/2"	1 7⁄8″	51⁄8″	91/2″			
BG6	11/2″	2″	4″	7 ¹ /2″	11/2″	11/2″	13⁄4″	53⁄4″	91/2″			

NOTE: PAYMENT FOR THE EXPANSION PLATE AND EXPANSION ANCHORS IS INCLUDED IN THE COST FOR STRUCTURE STEEL.

<u> Plan - expansion plate</u> (4 REQUIRED)

PLATE SHALL BE ANCHORED AT BACKWALL OF ABUTMENT 1 & 2, SPAN B SIDE OF PIER 1 AND SPAN B SIDE OF PIER 2.

2/21/2018

TH CARO,

SEAL 27812

V WGINEE

W. HAW

WFESS/

P-5705BA

MECKLENBURG COUNTY

STATION: STA. 42+59.46 -A1-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUPERSTRUCTURE

EXPANSION PLATE DETAILS

NORTH CAROLINA, P.C.		REVISIONS							
ense No. C-1554 Six Forks Rd., Suite 200, Raleigh, N.C. 27609	NO.	BY	DATE	NO.	BY	DATE	S8-16		
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<u> Plan - Span a parapets</u>

<u>PLAN - SPAN C PARAPETS</u>

SECTION THRU PARAPET

NOTES:

METAL HANDRAIL NOT SHOWN FOR CLARITY.

ALL HORIZONTAL DIMENSIONS SHOWN ARE MEASURED ALONG EXTERIOR FACE OF PARAPET.

ALL REINFORCING SHALL BE EPOXY COATED.

FOR LOCATION AND DETAILS OF SIGNAL CONDUIT IN CONCRETE PARAPET, SEE "TYPICAL SECTION" SHEET.

FOR ALUMINUM HANDRAIL DETAILS, SEE "METAL HANDRAIL DETAILS" SHEETS.

VERTICAL GROOVED CONTRACTION JOINTS, $\frac{1}{2}^{\prime\prime}$ IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF PARAPET AND IN ACCORDANCE WITH THE SPECIAL PROVISION FOR "CONCRETE PARAPET". THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS.

PAYMENT FOR CONCRETE PARAPET SHALL BE INCLUDED IN THE UNIT COST PAY ITEM FOR CONCRETE PARAPET.

PAY LENGTH = 256.4'

	F	PROJE		٩0	P-	570			
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	ç	STATI	TATION: <u>STA. 42+59.46</u> - A1-						
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<u> Plan - Span B parapets</u>

2/21/2018

NOTES: FOR NOTES, SEE SHEET "PARAPET DETAILS (1 OF 2)"

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METAL HANDRAIL POST SPACING - BRIDGE

<u>LEGEND</u>

- END POST
- INTERIOR POST

NOTE: FOR SECTION A-A, SEE SHEET 2 OF 2.

ALL DIMENSIONS SHOWN ARE ALONG EXTERIOR FACE OF PARAPET/WINGWALL.

<u> Pay length = 304.3'</u>

<u>ABUTMENT 1</u>

<u>ABUTMENT 2</u>

<u>METAL HANDRAIL POS</u>T SPACING - WINGWALLS

JST SFACING - WINGWA	<u>LLJ</u>								
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	1			3			SHEETS 39		

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ALUMINUM HANDRAIL AND POST

NOTES:

JOINTS IN RAILING (SPLICE GAP) SHALL BE LOCATED AS SHOWN IN POST SPACING PLAN.

ALUMINUM PIPE TO BE ASTM B-429, ALLOY 6061-T6 AND BASE PLATE TO BE ASTM B-209, ALLOY 6061-T6.

STAINLESS STEEL BOLTS, CAP SCREWS, AND NUTS TO BE ASTM A-276 TYPE 304. STAINLESS STEEL WASHERS TO BE ASTM A-276 TYPE 302.

POSTS TO BE SET PERPENDICULAR TO TOP OF PARAPET AND RAILS SHALL BE PLACED PARALLEL TO THE GRADE OF THE BRIDGE.

CERTIFIED MILL REPORTS ARE REQUIRED FOR RAIL AND POSTS. SHOP INSPECTIONS ARE NOT REQUIRED BY THE RAILROAD BUT MAY BE REQUIRED BY NCDOT.

AFTER ANCHOR ROD NUTS HAVE BEEN TIGHTENED, THREADS SHALL BE NICKED TO LOCK NUTS.

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURES THE CONTRACTOR MAY AT HIS OPTION HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER.

ANCHOR PLATES SHALL BE STEEL CONFORMING TO ASTM SPECIFICATION A36.

ANCHOR RODS SHALL CONFORM TO ASTM SPECIFICATION A276 TYPE 302 OR 304, STAINLESS STEEL AND THREADS SHALL BE ROLLED, NOT CUT.

UPPER ANCHOR ROD NUTS SHALL BE HEAVY HEX NUTS, PER ASTM A276 TYPE 302 OR 304 STAINLESS STEEL.

LOWER ANCHOR ROD NUTS SHALL BE HEAVY STEEL HEX NUTS, PER ASTM A563.

THE CENTERLINE OF ANY SPLICE AND/OR EXPANSION JOINT IS TO BE LOCATED AT LEAST 2'-O"AWAY FROM CENTERLINE OF POST. EXPANSION AND/OR SPLICE JOINTS FOR EACH RAIL OF TWO RAILINGS ARE TO BE PLACED IN THE SAME LOCATION AND IN THE SAME PANEL.

WELDING SHALL BE IN ACCORDANCE WITH THE CURRENT AWS STRUCTURAL WELDING CODE - ALUMINUM.

THE LENGTH OF METAL RAIL TO BE PAID FOR SHALL BE THE CONTINUOUS LENGTH MEASURED FROM END TO END OF RAIL, ALONG THE TOP RAIL.

SHOP DRAWINGS FOR RAILINGS ARE REQUIRED AND SHALL BE SUBMITTED FOR APPROVAL.

FOR METAL RAIL (ALUMINUM). SEE SPECIAL PROVISIONS.

SHEET 2 OF 2

P-5705BA PROJECT NO.

> MECKLENBURG

> > STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION RALEIGH

SUPERSTRUCTURE

METAL HANDRAIL

DETAILS

STATION: <u>STA. 42+59.46</u> -A1-

—Docusigned by: David Hawkins TH CARO, WESS/01 SEAL 27812 O FILL ENGINEER W. HA 2/21/2018

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SHEET NO. HNTB NORTH CAROLINA, P.C. NC License No. C-1554 REVISIONS S8-20 DATE NO. BY DATE NO. BY 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 total sheets 39 1 3 DWG.NO.20 2 4

		S	PAN A					S	PAN B					S	PAN C	
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENG
A1	73	5	STR.	21'-8"	1,650	A1	152	5	STR.	21'-8"	3,435	A1	38	5	STR.	21'-8
A2	2	5	STR.	21'-7"	45	A2	2	5	STR.	21'-7"	45	A2	2	5	STR.	21'-
A3	4	5	STR.	18'-0"	75	A3	4	5	STR.	18'-0"	75	A3	4	5	STR.	18'-0
Α4	2	5	STR.	14'-6"	30	A4	2	5	STR.	14'-6"	30	A4	2	5	STR.	14'-6
Α5	2	5	STR.	11'-0"	23	A5	2	5	STR.	11'-0"	23	A5	2	5	STR.	11'-0
A6	2	5	STR.	7′-5″	15	A6	2	5	STR.	7′-5″	15	A6	2	5	STR.	7'-5
A7	4	5	STR.	3'-11"	16	Α7	4	5	STR.	3'-11"	16	Α7	4	5	STR.	3'-1
A8	2	5	STR.	8'-7"	18	A8	2	5	STR.	8'-7"	18	A8	2	5	STR.	8′-7
Α9	2	5	STR.	13'-4"	28	A9	2	5	STR.	13'-4"	28	A9	2	5	STR.	13'-4
A10	8	5	STR.	22'-2"	185	A10	8	5	STR.	22'-2″	185	A10	8	5	STR.	22'-
B1	56	4	STR.	22'-0"	823	B3	112	4	STR.	30'-0"	2,244	B5	56	4	STR.	20'-
B2	56	4	STR.	16'-2"	605	B4	56	4	STR.	14'-0"	524	B14	12	5	STR.	20'-
B6	6	5	STR.	17'-9"	111	B10	12	5	STR.	23'-0″	288					
B7	6	5	STR.	17'-7"	110	B11	6	5	STR.	22'-10"	143	S1	44	5	1	6'-10
B8	6	5	STR.	17'-10"	112	B12	12	5	STR.	23'-1"	289					
B9	6	5	STR.	18'-0"	113	B13	6	5	STR.	23'-2″	145					
S1	76	5	1	6'-10"	542	S2	144	5	1	6′-8″	1,001					
				TOTAL	4,501					TOTAL	8,504					TOTA

EPOXY-COATED REINFORCING STEEL

QUANTITY BREAKDOWN BY SPAN									
	EPOXY COATED REINFORCING STEEL	CLASS AA (CU.`	CONCRETE YDS.)						
	(LBS.)	DECK SLAB	PARAPETS						
SPAN "A"	4,501	32.3	5.1						
SPAN "B"	8,504	60.5	10.1						
SPAN "C"	2,636	17.5	3.0						
TOTALS	15,641	110.3	18.2						

TOTAL SUPERSTRUCTURE QUANTITIES									
	REINFORCED CONCRETE DECK SLAB	EPOXY COATED REINFORCING STEEL	CLASS AA CONCRETE						
	SQ.FT.	LBS.	CU.YDS.						
DECK SLAB	2,822.1	12,215	110.3						
PARAPET —		3,426	18.2						
TOTALS 2,822.1		15,641	128.5						

PROJECT	NO. <u>P-57</u>	05BA
ME	CKLENBURG	
STATION:	STA. 42+59.46	5 -A1-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUPERSTRUCTURE

BILL OF MATERIAL

NORTH CAROLINA, P.C.		REVISIONS							
nse No. C-1554 Six Forks Rd., Suite 200, Raleigh, N.C. 27609	NO.	BY	DATE	NO.	BY	DATE	S8-21		
	1			3			TOTAL SHEETS		
A DATE <u>9/17</u> DWG. NO. 21	2			4			39		

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ANGLE ABOUT THE HORIZONTAL PLAN (DEGREES)
15
15
15
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15

NOTE: FOR DIRECTION OF ANGLE ABOUT THE VERTICAL PLANE, SEE "PLAN VIEW B-B".

> FOR DIRECTION OF ANGLE ABOUT THE HORIZONTAL PLANE, SEE "TYPICAL SECTION" ON SHEET 2 OF 2.

Ρ	l

NAIL NUMBER	ANGLE ABOUT THE HORIZONTAL PLANE (DEGREES)	ANGLE ABOUT THE VERTICAL PLANE (DEGREES)
16	15	0
17	15	0
18	15	0
19	15	8
20	15	5
21	15	0
22	15	0
23	15	0
24	15	0
25	15	0
26	15	8
27	15	8
28	15	5
29	15	8
30	15	0

2-#4 GRADE 60 VERTICAL 2-#4, GRADE 60 HORIZONTAL CONTINUOUS WALER BARS AT CONTINUOUS WALER BARS AT 8 INCHES CENTERED AROUND 8 INCHES CENTERED AROUND EACH NAIL ROW. EACH NAIL COLUMN MIN.LAP 12 INCHES MIN LAP 15 INCHES CONTINUOUS 6X6 W2.9×W2.9 WELDED WIRE FABRIC 25'-0"(TYP.) MIN LAP 8 INCHES PVC SCH.40 CENTRALIZER MIN 4 INCH THICK, 3,000 PSI @ 8'-0"SPA. SHOTCRETE FACING.EXTERIOR (TYP.) 3,000 PSI @ 28 FACE TO HAVE TROWLED FINISH DAYS GROUT (TYP.) 10" × 10" × 1" MIN. A36 BEVELED BEARING PLATE WITH WASHER AND NUT (TYP.) WEEP HOLE TO BE 6" ABOVE BOTTOM OF 15° EXCAVATION (SEE FOOT TYP.) DRAIN DETAIL) MIN.12 INCH WIDE COMPOSITE DRAIN STRIP 3 INCH DIA PVC CONNECTOR WITH DRAIN CONNECTOR PIPE WEEP HOLE EACH DRAIN \ MIN.6 INCH DIA. PIPE CENTERED BETWEEN BORE HOLE (TYP.) STRIP EACH OFFSET NAIL COLUMN TYPICAL SECTION COMPOSITE DRAIN STRIP 1'-0" MIN. 3″ØPVC CONNECTOR CONTINUOUS

FACING REINFORCEMENT AND DRAINAGE

DWG. NO. 23

2

2/21/2018

NOTES:

- 1. FOR PLAN VIEW OF TEMPORARY SHORING, SEE SHEET 1 OF 2.
- 2. MINIMUM SHOTCRETE COVER OF 2 INCHES (FRONT AND BACK) OVER STEEL REINFORCEMENT BAR AND MESH.
- 3. DESIGN LOAD = 33 KIPS, ULTIMATE CAPACITY = 66 KIPS.
- 4. ALL SOIL NAILS SHALL BE LEFT IN PLACE AFTER TEMPORARY WORK HAS FINISHED.

	PROJE	ECT I ME(ON: .	no Cklen Sta . 4	P- BUR 2+59	570 G 9.46	5BA _ COL -A1-	JNTY		
DocuSigned by: DocuSigned by:	SHEET 2 OF 2 state of North Carolina DEPARTMENT OF TRANSPORTATION								
O40442	SUBSTRUCTURE								
NT NOT CONSIDERED NAL UNLESS ALL ATURES COMPLETED	ABUTMENT 1 SHORING								
IORTH CAROLINA, P.C. nse No. C-1554		BY	REVISI	IONS	BV	DATE	sheet no. S8-23		
x FORKS Ka., SUITE 200, Kaleigh, N.C. 27609				,,,,,,		DATE	тота		

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39

NOTES:

SHEET 3 OF 3.

2/21/2018

FOR SECTION A-A, SECTION B-B AND WINGWALL DETAILS, SEE SHEET 2 OF 3. FOR LAYOUT AND DETAILS OF CONTINUOUS FRENCH DRAIN SYSTEM BETWEEN ABUTMENT WINGWALLS, SEE "STRUCTURE DRAINAGE DETAILS" SHEET 3 OF 3. \downarrow () INDICATES PILE BATTERED AT 3.5:12 IN DIRECTION SHOWN. CONDUIT TO BE 4"Ø IN ACCORDANCE WITH UNDERWRITERS LABORATORY SPECIFICATIONS. FOR FOOTING REINFORCING, SEE SECTION B-B SHEET 2 OF 3 AND FOOTING REINFORCING PLAN

	PRO. 	PROJECT NO. P-5705BA MECKLENBURG COUNTY STATION: STA. 42+59.46 -A1-									
DocuSigned by: David Hawkins ABED7524B855487 RTH CAROZ SEAL 27812 W. HAWKING MARKER	SHEE	T 1 OF DEP	3 artment SUBS	of north OF TF raleigh	carolina RANSPO 1 CTU	ortatic RE	N				
T NOT CONSIDERED L UNLESS ALL URES COMPLETED		ABUTMENT 1									
RTH CAROLINA, P.C.		REVISIONS					SHEET NO.				
Forks Rd., Suite 200, Raleigh, N.C. 276	09 NO.	BY	DATE	NO.	BY	DATE					
DATE <u>10/17</u> DWG. NO.	24 7			3			SHEETS 39				

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NOTES: FOR ABUTMENT AND WINGWALL DIMENSIONS AND ELEVATIONS, SEE SHEET 1 OF 3. FOR LOCATIONS OF HANDRAIL ANCHORAGES IN TOP OF WINGWALLS,

SEE "METAL HANDRAIL DETAILS" SHEETS.

$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		6		T2 T3 T4	40 4 4 20		7	6 6 STR	9'-9" 5'-2" 26'-6"	1,014 80 42 1,083
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$			-	V1 V2 V3 V4 V5 V6	41 21 12 46 21 12		7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	STR STR STR STR STR STR	8'-2" 8'-2" 10'-1" 10'-1" 7'-2" 8'-1"	684 179 247 484 157 101
ALL BAR DIMENSIC	NS ARE OU	T TO OUT							TOTAL	5,244
QUANTITIES			<u>NOTES</u> : "T" bars ma clear anch for footin	Y BE SH IOR BOLT IG LAYOL	IFTED S. JT AND	SLIGHTI DIMENS	LY AS SIONS,	NECES	SSARY TO) = 3.
ITEM REINFORCING STEEL LBS.	TOTAL 5,244									
CLASS AA CONCRETE BREAKDOWN: POUR 1 FOOTING CU.YDS	34.1									
BACKWALL & WINGS CU. YDS TOTAL CU. YDS	16.8 50.9			PROJE		IO	P-	-570	5BA	
7"ØMICROPILES EACH	12			STATI	MEC <u>0</u> n: <u>-</u>	KLEN sta.4	1BUF 42+5	≀G 9.46	_COU -A1-	NTY
		DocuSigned by: David Hawki	ns	SHEE	<u>T 3 OF</u>	3				
2/21/2018					DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE					
HNTE	DOCUMEN FINA SIGNAT HNTB NO NC License 343 E. Six	T NOT CONSIDE AL UNLESS ALL URES COMPLET RTH CAROLINA, P.C No. C-1554 Forks Rd., Suite 200, 1	ERED ED Raleigh, N.C. 27609	NO.	ВҮ	REVIS DATE	SIONS NO.	ВҮ	DATE	sheet no. S8-26
DRAWN BY CHECKED BY	M. WRIGHT J. WHEATLEY	DATE <u>10/17</u> DATE <u>10/17</u>	- DWG.NO. 26	1 2			3			total sheets 39

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- 2	2′-11″
<u>ی</u>	
2'-10"	2'-0"
2'-0"	1'-111/2"
8'-0"	<u>1'-2″</u>
7′-5″	HK. 1'-2″
2/ 10//	HK.

813/16

	BILL OF REINFORCING								
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT				
		ABUTM	ENT 1						
H1	8	5	1	12'-11"	108				
H2	14	6	1	13′-3″	279				
H3	4	5	2	6'-7"	27				
H4	16	5	STR	8'-4"	139				
K1	12	5	STR	26'-8″	334				
К2	6	5	3	14'-6"	91				
КЗ	6	5	4	6'-10"	43				
S1	21	5	5	3′-11″	86				
S2	1	5	5	4'-0"	4				
S3	1	5	5	4'-3"	4				
S4	1	5	5	4'-5"	5				
S5	1	5	5	4'-8"	5				
S6	1	5	5	4'-10"	5				
S7	1	5	5	5'-1"	5				
S8	1	5	5	5′-3″	5				
S9	1	5	5	5′-6″	6				
S10	1	5	5	5′-8″	6				
S11	1	5	5	12'-9"	13				
S12	1	5	5	8'-1"	8				
Τ1	48	7	6	10'-4"	1,014				
Τ2	4	7	6	9'-9"	80				
Т3	4	7	6	5′-2″	42				
T4	20	7	STR	26'-6″	1,083				
V1	41	7	STR	8'-2"	684				
V2	21	5	STR	8'-2"	179				
٧3	12	7	STR	10'-1"	247				
V4	46	5	STR	10'-1"	484				
V5	21	5	STR	7'-2″	157				
٧6	12	5	STR	8'-1"	101				
				TOTAL	5,244				

ALL DIMENSIONS SHOWN ARE PARALLEL OR NORMAL TO PIER UNLESS NOTED.

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY TOR PLACING REINFORCING STEEL.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCING IS DETAILED WITH 3 FT.OF EXTRA LENGTH.

ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL".

	F	PROJE		٩0	P-5705BA					
	_		ME	CKLEN	BUR	G	_COUNTY			
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Docusigned by: David Hawkins		SHEET	1 OF_2	2						
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W. HAWKING MUTUNIN		SUBSTRUCTURE								
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NORTH CAROLINA, P.C. nse No. C-1554			D) (REVISI	ONS			SHEET NO.		
ix Forks Rd., Suite 200, Raleig	jh, N.C. 27609	NO. 1	ВҮ	DATE	NO.	BY	DATE	TOTAL		
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NORTH CAROLINA, P.C.		REVISIONS							
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- 4. REMOVE COLUMN FORMS.
- 5. PROTECT THE COLUMN AND BACKFILL WITH CLEAN SAND.
- 6. EXTRACT TEMPORARY COLUMN CASING.
- 7. COMPLETE BRIDGE AND SHIFT RAILROAD TRAFFIC.

NOTES:

FOR COLUMN EXCAVATION SEE COLUMN EXCAVATION SPECIAL PROVISION.

THE CONTRACTOR SHALL DETERMINE THE DIAMETER OF TEMPORARY COLUMN CASING SUCH THAT THE WORK CAN BE COMPLETED. PROVIDE TEMPORARY COLUMN CASING CAP A MINIMUM DIAMETER 6"GREATER THAN THE LARGEST CASING USED FOR DRILLED PIER CONSTRUCTION, AND A FINISH GRADE MINIMUM THICKNESS IN ACCORDANCE WITH TABLE 411-1 OF THE NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES. THE CONTRACTOR HAS THE OPTION TO INSTALL THE COLUMN AND DRILLED PIER REINFORCING AT ONE TIME. THE ≡IIII≡IIII CONTRACTOR IS RESPONSIBLE FOR MAINTAINING THE ALIGNMENT AND POSITION OF THE REINFORCING CAGE. 4'-0"DIA. COLUMN CONTRACTOR SHALL ENSURE THAT THE TOP OF THE DRILLED PIER IS CLEAN AND FREE OF DEBRIS BEFORE PLACING CONST.JT. COLUMN CONCRETE. 4'-6"DIA. DRILLED PIER DocuSigned by David Hawkins SEAL 27812 SNGINEE! W. HAW 2/21/2018 P-5705BA PROJECT NO. MECKLENBURG STATION: STA. 42+59.46 -A1-STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE COLUMN AND DRILLED PIER CONSTRUCTION DOCUMENT NOT CONSIDERED SEQUENCE - PIERS FINAL UNLESS ALL SIGNATURES COMPLETED SHEET NO. HNTB NORTH CAROLINA, P.C. REVISIONS NC License No.C-1554 S8-29 NO. BY DATE NO. BY DATE 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 TOTAL SHEETS 1 3 ||/|7 ____ DATE DWG.NO. 29

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39

END ELEVATION

HNTB N NC Licen 343 E. Six J. BAYNE DRAWN BY J. BAYNE CHECKED BY J. WHEATLEY

ALL DIMENSIONS SHOWN ARE PARALLEL OR NORMAL TO € PIER UNLESS NOTED.

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCING IS DETAILED WITH 3 FT. OF EXTRA LENGTH.

ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL".

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IORTH CAROLINA, P.C.			REVISI	IONS			SHEET NO.		
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	1			3			TOTAL SHEETS 39		

		BILL OF MATERIAL									
H H	F	BAF	2	NO.	SIZE	. <u>er 2</u> Ty	'PE	LENGTH	WEIGHT		
	-	B1		12	9	S	TR.	26'-8"	1,088		
2 4'-7!		B2 B3		10 8	5	S	TR. 1	26'-8" 28'-6"	278 609		
	_	S1		44	5		2	13'-9″	631		
<u>3'-7"</u> <u>4'-2"</u>		S2 S3		53 14	4		3	6'-2" 6-0"	218 56		
		S4 S5		16 33	4		3 4	6'-6" 11'-7"	69 399		
	_	S6		24	5		2	14'-4"	359		
	-	M1 M2		24 48	11	S	TR. TR.	38′-7″ 43′-7″	4,920 11,115		
	-	V1		72	9		5	14'-3"	3,488		
3'-8"	-	SP-	1	2	**		6	943'-11″	1,969		
► ∧ 7 .	-	SP-	2 3	4	***		6	1095'-10" 461'-5"	4,572		
Sp ACERC	-										
	F		<u> </u>		QUAN	TITI	IES		1		
6	F				ITEM				TOTAL		
	F	REINF	ORCIN	G STEEI				LBS	23,230		
3'-8"Ø		SPIRA	L COLL	JMN RE	INFORCIN	G STEE	Ĺ	LBS	7,985		
$1\frac{1}{2}$ EXTR	A TURNS	CLASS		DNCRET				ΓY	13 3		
		POUR 2 C.Y.									
SP-1		4′-6″ [PTERS			NO			
×9-			ILLED	PIER I	N SOIL				70.0		
	- cc	DR: DR:	ILLED	PIER C	ONCRETE	POUR 1	l	C.Y.	60.7		
		PERMA	NENT S	STEEL (CASING			L.F.	60.0		
(BOTTOM OF SP-1									7		
ANU SP-2)											
			ESTIN	<u>.</u>							
									3		
	 	THERN 55 CT	rculai	R TTES	Y PROFIL	<u>er</u> Bf As'	TM DF	STGNATT	0N		
	A7C WIT *** THE BUN **** THE OR BAR)6,GR [H TH [SP-]]] [SP-]]]]]]]]]]]]]]]]]]]	ADE 6 IE "MAN 1 AND #5 PI 3 SPIF COLD D	O.FABI NUAL C SP-2 S LAIN C RAL RE DRAWN	RICATION F STAND SPIRAL R DR DEFOR INFORCI WIRE OR	N TO E ARD PI EINFO MED B NG STI #5 PL	BE IN RACTI RCING AR. EEL SH LAIN	ACCORD, CE", A.C. S STEEL HALL BE OR DEFO	ANCE E. 315.80. Shall BE W31 RMED		
		P	PROJE		٥٠	Ρ-	570	5BA			
		_		MEC	CKLEN	BUR	G	_cou	NTY		
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David Hawkins		, ſ	SHEET	2 OF	2						
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SEAL STAIL STAIL					SUBS	raleige STRL	¹ JC⊤l	JRE			
018							_				
MENT NOT CONSIDERE	D				F	PIEF	82				
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NOTES:

SHORING HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT EDITION OF AREMA'S MANUAL FOR RAILWAY ENGINEERING, VOL.2,STRUCTURES.

MAXIMUM WALL DEFLECTION LIMITED TO 0.5".

ALL SHORING MATERIAL SHALL BE IN "LIKE NEW" CONDITION.

SHEET PILING SHALL BE ASTM A572 GRADE 50 STEEL (HOT ROLLED) AND SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

MOMENT OF INERTIA/FT. 235.0 in⁴ SECTION MODULUS/FT. 10.0 in ³

<u>SECTION A-A</u>

- -	PROJE	5BA _ COL -A1-	JNTY				
DocuSigned by: David Hawkins ABED7524B855487 ABED7524B855487 ABED7524B855487 CARO TH CARO SEAL 27812 SEAL 27812		DEPA	artment	OF NORTH OF TH RALEIGH	carolina RANSPO H	ortatic IRE	ри
NT NOT CONSIDERED IAL UNLESS ALL TURES COMPLETED	ABUTMENT 2 SHORING						
ORTH CAROLINA, P.C. se No. C-1554 < Forks Rd., Suite 200, Raleigh, N.C. 27609	NO.	BY	REVISI DATE	ONS NO.	BY	DATE	sheet no. S8-32
DATEI/17DWG. NO. 32	1 2			3			total sheets 39

2/21/2018

FOR SECTION A-A, SECTION B-B AND WINGWALL DETAILS, SEE SHEET 2 OF 3. FOR LAYOUT AND DETAILS OF CONTINUOUS FRENCH DRAIN SYSTEM BETWEEN ABUTMENT WINGWALLS, SEE "STRUCTURE DRAINAGE DETAILS" SHEET 2 OF 3.

CONDUIT TO BE 4"Ø IN ACCORDANCE WITH UNDERWRITERS LABORATORY SPECIFICATIONS. FOR FOOTING REINFORCING, SEE SECTION B-B SHEET 2 OF 3 AND FOOTING REINFORCING PLAN SHEET 3 OF 3.

	F	PROJE	ROJECT NOP-5705BA MECKLENBURGCOU								
	S	STATI	ON: .	STA.4	2+59	.46	-A1-				
DocuSigned by: David Hawkins		SHEET	1 OF 3	3							
SEAL	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH										
The ENGINEER WITH THE STATE		SUBSTRUCTURE									
NT NOT CONSIDERE NAL UNLESS ALL ATURES COMPLETED	D	ABUTMENT 2									
IORTH CAROLINA, P.C. nse No. C-1554		DV	REVISI	ONS		D (75	SHEET NO.				
x Forks Rd., Suite 200, Ralei	gh, N.C. 27609	NO. 1	ВҮ	DAIE	NO. 3	ВҮ	DATE	TOTAL			
DATE	DWG.NO. 33	2			4			39			

NOTES: FOR ABUTMENT AND WINGWALL DIMENSIONS AND ELEVATIONS, SEE SHEET 1 OF 3. FOR LOCATIONS OF HANDRAIL ANCHORAGES IN TOP OF WINGWALLS,

SEE "METAL HANDRAIL DETAILS" SHEETS.

QUANTITIES ITEM TOTAL LBS. 5,695 REINFORCING STEEL CLASS AA CONCRETE BREAKDOWN: CU. YDS 38.9 FOOTING POUR 2 CU.YDS 16.9 BACKWALL & WINGS CU.YDS 55.8 HP 14 X 73 STEEL PILES NO. 12 L.F. 420.0

"T" BARS MAY BE SHIFTED SLIGHTLY AS NECESSARY TO CLEAR ANCHOR BOLTS.

FOR FOOTING LAYOUT AND DIMENSIONS, SEE SHEET 1 OF 3.

		PROJ	5BA							
			ME	CKLEN	BUR	G	_col	JNTY		
		STATI	STATION: STA. 42+59.46 - A1-							
Docusigned by: David Hawkins		SHEE	T 3 01	- 3						
SEAL 27812		STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH								
W. HAWMMUN				SUBS	STRL	ICTU	RE			
2/21/2018 " ^{####################################}		ABUTMENT 2								
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		ME	CKLEN	BUR	G	_COL	JNTY	
	STATI	ON: .	STA.4	2+59	9.46	-A1-		
Docusigned by: David Hawkins	SHEET	1 OF 3	3					
RTHESS ON THE SEAL	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH							
NGINEER WINNER	STRUCTURE							
	DRAINAGE							
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\sim Forks Ra., Suite 200, Raleigh, N.C. 27609 — DATE <u>$8/17$</u> DWG, NO, 36	1			3			TOTAL SHEETS	

SUBSTRUCTURE DRAINAGE SYSTEM ESTIMATED QUAN	TITIES	
ITEM	UNIT	TOTAL
DUCTILE IRON PIPE,CLASS 53 (FLANGED BOTH ENDS)	FEET	25.2
DUCTILE IRON PIPE,CLASS 53 (FLANGE & P.E.)	FEET	3.6
DUCTILE IRON PIPE FLANGED FITTINGS, 250 psiP.R.	LBS	540
DUCTILE IRON BLIND FLANGES	LBS	42
OFFSET PIPE CLAMPS	EA.	2
DUCTILE IRON PIPE, CLASS 53 (FLANGE & P.E.)	FEET	4.2

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ALL PIPES, FLANGES AND FITTINGS SHALL BE CLASS 53 DUCTILE IRON.

ALL BENDS TO BE SHORT RADIUS, INCLUDING FLANGE & FLARE BENDS, UNLESS OTHERWISE NOTED.

FOR LOCATIONS AND DESIGNATIONS OF DSA & PSA, SEE FRAMING PLAN.

PIPE LENGTHS SHOWN ALLOW FOR $1/_8$ " THICK GASKETS TO BE USED AT ALL BOLTED FLANGE CONNECTIONS.

MAKE FINAL PIPE ALIGNMENT AND TIGHTEN U-BOLTS AFTER RAILROAD TRACK HAS BEEN LAID ACROSS THE BRIDGE.

PAYMENT FOR ALL MATERIALS, FABRICATION, INSTALLATION AND ADJUSTMENTS RELATED TO STRUCTURE DRAINAGE SHALL BE INCLUDED IN THE CONTRACT LUMP SUM PRICE BID FOR "STRUCTURE DRAINAGE SYSTEM". NO SEPARATE MEASUREMENT OR PAYMENT SHALL BE MADE FOR ANY COMPONENT OF THE STRUCTURE DRAINAGE SYSTEM INCLUDING, BUT NOT LIMITED TO:

- DUCTILE IRON PIPE AND FITTINGS AND CAST IRON GRATES
- GASKETS AND PVC OR NEOPRENE COATED STRIPS
- STEEL SUPPORT ANGLES AND PLATES
- U-BOLTS AND H.S. BOLTS, WASHERS AND NUTS
- OFFSET PIPE CLAMPS AND EXPANSION ANCHOR BOLTS
 EXPANSION JOINT

PROVIDE PVC OR NEOPRENE-COATED STRIPS, EPOXY-CEMENTED TO THE U-BOLT OR PIPE FOR STRAY CURRENT PROTECTION.

THE OUTSIDE COATING FOR D.I. PIPE SHALL BE PAINTED WITH A SHOP PRIME COAT OF INORGANIC ZINC PRIMER AND A FINISH (FIELD) COATING OF ACRYLIC PAINT AS SPECIFIED FOR THE STRUCTURAL STEEL.

FOR STRUCTURE DRAINAGE SYSTEM, SEE SPECIAL PROVISIONS.

JPERSTRUCTURE DRAINAGE SYSTEM ESTIMATED QUANT	ITIES	
ITEM	UNIT	TOTAL
E IRON PIPE,CLASS 53 (FLANGED BOTH ENDS)	FEET	201′-7 /2″
E IRON PIPE, CLASS 53 (BELL & P.E.)	FEET	36′-9 ¾ ″
E IRON PIPE FLANGED FITTINGS,250 psiP.R.	LBS	3,520
E IRON BLIND FLANGES	LBS	150
DINT	EA.	4
CAST IRON GRATES	EA.	24
ANGLES (PSA) *	EA.	12
JPPORT ANGLES (DSA) *	EA.	16

* INCLUDES U-BOLTS, ANGLES, NUTS, BOLTS, WASHERS AND PLATES.

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NOTES: FOR BERM WIDTH DIMENSIONS AND BERM ELEVATIONS, SEE GENERAL DRAWING.

FOR TEMPORARY RAILROAD SHORING, SEE "ABUTMENT 1 SHORING" AND "ABUTMENT 2 SHORING" SHEETS.

FOR RETAINING WALL RW-9, SEE RETAINING WALL PLANS.

BRIDGE @ STA.42+59.46 -A1-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE			
	TONS	SQUARE YARDS			
ABUTMENT 1	350	390			
ABUTMENT 2	260	290			

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DESIGN DATA:

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SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES. ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS. ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " with the following exceptions: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES. DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION. HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 1/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 1/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE. THE CONTRACTOR MAY. AT HIS OPTION. SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{6}$ in thickness and DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

HANDRAILS AND POSTS:

STD. NO. SN