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**PROJECT: McCRIMMON PARKWAY EXTENSION**

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	012108004	1	60

**STATE OF NORTH CAROLINA**  
**DEPARTMENT OF TRANSPORTATION**  
**DIVISION OF HIGHWAYS**  
**GEOTECHNICAL ENGINEERING UNIT**

**ROADWAY**  
**SUBSURFACE INVESTIGATION**

COUNTY WAKE  
PROJECT DESCRIPTION McCRIMMON PARKWAY  
EXTENSION FROM NC 54 /CHAPEL HILL ROAD  
TO AVIATION PARKWAY

**INVENTORY**

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-Y7-	20+00 - 25+00	11	23

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THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF PREPARING THE SCOPE OF WORK TO BE INCLUDED IN THE REQUEST FOR PROPOSAL. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

SOIL AND ROCK BOUNDARIES WITHIN A BOREHOLE ARE BASED ON GEOTECHNICAL INTERPRETATION UNLESS ENCOUNTERED IN A SAMPLE. INTERPRETED BOUNDARIES MAY NOT NECESSARILY REFLECT ACTUAL SUBSURFACE CONDITIONS BETWEEN SAMPLED STRATA AND BOREHOLE INFORMATION MAY NOT NECESSARILY REFLECT ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

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1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

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DATE NOVEMBER 2014





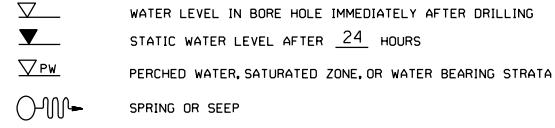
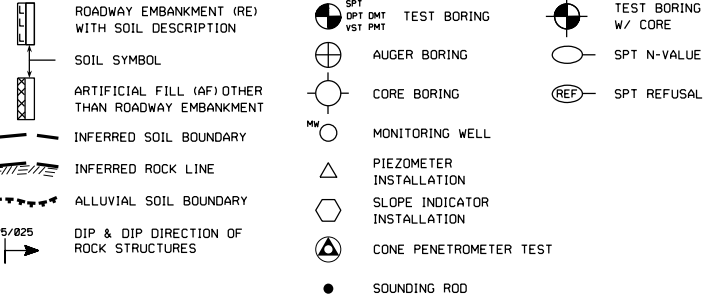


SIGNATURE \_\_\_\_\_ DATE \_\_\_\_\_

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION**  
**DIVISION OF HIGHWAYS**  
**GEOTECHNICAL ENGINEERING UNIT**

**SUBSURFACE INVESTIGATION**

**SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS					
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T208, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>		WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. <b>ANGULARITY OF GRAINS</b> THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.		HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:  WEATHERED ROCK (WR)  CRYSTALLINE ROCK (CR)  NON-CRYSTALLINE ROCK (NCR)  COASTAL PLAIN SEDIMENTARY ROCK (CPS)		ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.					
<b>SOIL LEGEND AND AASHTO CLASSIFICATION</b>		<b>MINERALOGICAL COMPOSITION</b>		<b>WEATHERING</b>							
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS		MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.		FRESH ROCK FRESH, CRYSTALLINE SHELL, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SL) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &gt; 100 BPF</i> VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &lt; 100 BPF</i> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.		<b>COMPRESSION</b> SLIGHTLY COMPRESSIBLE MODERATELY COMPRESSIBLE HIGHLY COMPRESSIBLE <b>PERCENTAGE OF MATERIAL</b> ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE		<b>GROUND WATER</b>  WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP			
<b>CONSISTENCY OR DENSENESS</b>		<b>MISCELLANEOUS SYMBOLS</b>		<b>ROCK HARDNESS</b>							
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> )		 ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION CONE PENETROMETER TEST SOUNDING ROD		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.							
<b>TEXTURE OR GRAIN SIZE</b>		<b>ABBREVIATIONS</b>		<b>FRACTURE SPACING</b>		<b>BEDDING</b>					
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST WEA. - WEATHERED UNIT WEIGHT DRY UNIT WEIGHT SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO		VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET		VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET					
<b>SOIL MOISTURE - CORRELATION OF TERMS</b>		<b>EQUIPMENT USED ON SUBJECT PROJECT</b>		<b>INDURATION</b>		<b>NOTES:</b>					
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION		DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: CORE SIZE: HAND TOOLS:		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.		BENCH MARK: BORING ELEVATIONS ESTIMATED USING PROJECT TIN FILE ELEVATION: N/A FT.					
LL LIQUID LIMIT PL PLASTIC LIMIT OM OPTIMUM MOISTURE SL SHRINKAGE LIMIT		<input type="checkbox"/> MOBILE B- <input type="checkbox"/> BK-51 <input type="checkbox"/> CME-45C <input type="checkbox"/> CME-550 <input type="checkbox"/> PORTABLE HOIST <input checked="" type="checkbox"/> D-50 (TER346) <input checked="" type="checkbox"/> D-50 (TER255)		<input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL <input type="checkbox"/> -B <input type="checkbox"/> -N <input type="checkbox"/> -H <input type="checkbox"/> POST HOLE DIGGER <input type="checkbox"/> HAND AUGER <input type="checkbox"/> SOUNDING ROD <input type="checkbox"/> VANE SHEAR TEST		- -					
<b>PLASTICITY</b>											
NONPLASTIC 0-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH											
<b>COLOR</b>											
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.											

Kimley-Horn & Associates, Inc.  
3001 Weston Parkway  
Cary, North Carolina 27513

Attn: Mr. Matthew West, PE, AVS

Re: **Geotechnical Report – Roadway Inventory**

County: Wake  
Description: McCrimmon Parkway Extension between NC 54 / Chapel Hill Road and Aviation Parkway

**Project Description**

This project consists of a proposed 2.1 mile, four-lane divided roadway (-L-) between NC 54 / Chapel Hill Road (-Y2-) and Aviation Parkway (-Y8-). Portions of the project have been previously constructed (between approximately -L- Stations 35+35 and 55+55), while the remainder of the alignment is on new location. The project is proposed to be constructed in phases as development progresses along the corridor. The initial phase will consist of the construction of a new four-lane roadway between NC 54 / Chapel Hill Road and the existing portion of McCrimmon Parkway and the construction of a new two-lane divided roadway between Airport Boulevard (-Y4-) and Aviation Parkway. Grading will be performed during the initial phase to accommodate the proposed ultimate design consisting of a four-lane divided roadway between NC 54 / Chapel Hill Road and Aviation Parkway.

All roadway design data presented in this report is based on the preliminary design provided to Terracon by Kimley-Horn & Associates in June through November of 2014. The geotechnical investigation of the -L- alignment was performed during June and July of 2014. Three borings were performed later along the Southport Drive (-Y7-) alignment in October 2014. Two track mounted Diedrich D-50T drill machines equipped with automatic hammers were used during the investigation. Standard Penetration Tests were performed in the boreholes and samples were collected for visual classification in the field. Select samples were submitted for laboratory analysis.

The following alignments were investigated. Subsurface profiles and selected cross sections of these alignments are included in this report.

<u>Line</u>	<u>Station</u>
-L-	23+75 to 35+35
-L-	55+55 to 143+00
-Y7-	20+00 to 24+84

**Areas of Special Geotechnical Interest**

1) Artificial Fills: Artificial fills were encountered on the project at the following locations:

<u>Line</u>	<u>Station</u>
-L-	26+41 to 27+52
-L-	28+85 to 29+88
-L-	30+12 to 34+74
-L-	68+24 to 70+96

Cross sections of the areas where artificial fills will impact construction are attached.

2) Alluvial Soils: Alluvial soils were encountered on the project at the following location:

<u>Line</u>	<u>Station</u>
-L-	27+06 to 28+64

3) High Plasticity Clays: High plasticity clays (PI > 25) were encountered on the project at the following locations:

<u>Line</u>	<u>Station</u>
-L-	69+60 to 72+95
-L-	93+55 to 98+35
-L-	112+50 to 114+00
-L-	119+70 to 121+85
-L-	124+50 to 126+50
-Y7-	19+75 to 22+85

A discussion of high plasticity clay soils is located in the section titled "Soil Properties." Cross sections of areas where high plasticity clays were encountered near proposed grades are attached.

4) Non-Crystalline Rock: The non-crystalline rock on this project includes Triassic siltstone and sandstone. Rock occurs within 6 feet below proposed grade at the following location:

<u>Line</u>	<u>Station</u>
-L-	92+00 to 93+00

5) Degradable Rock: Degradable rock is defined as rock material (including weathered rock) which exhibits high slaking characteristics when exposed to air and water (see Rock Properties for further explanation). Degradable rock occurs within the proposed unclassified excavation in the following areas:



<u>Line</u>	<u>Station</u>
-L-	59+70 to 62+85
-L-	72+10 to 76+50
-L-	81+40 to 89+25
-L-	90+75 to 95+60
-L-	123+75 to 128+60

Cross sections of areas where degradable rock was encountered near proposed grades are attached.

6) Groundwater: Groundwater was encountered near proposed grade at the following location:

<u>Line</u>	<u>Station</u>
-L-	91+00
-L-	69+00 to 70+00

6) Ponds: A pond was encountered within close proximity to the right of way at the proposed location:

<u>Line</u>	<u>Station</u>	<u>Offset (ft)</u>
-L-	68+40 to 70+80	80 RT

### **Physiography and Geology**

The terrain along the project corridor is comprised of gently rolling hills with steep-sided drainage areas. One flowing creek was observed along the sewer easement behind the landscape supply yard at approximately -L- Station 27+50. Lake Crabtree and several tributaries are also located nearby outside the project limits. No other flowing creeks or streams were observed during the investigation within the project limits. The corridor winds through wooded and recently logged areas located between business parks.

The project is located within the Piedmont Physiographic Province. More specifically, the project is located within the Durham Sub-Basin of the Triassic Basin. The soils along the project were created from weathering of the underlying sedimentary Triassic-age rock. The rock in the Triassic Basin consists of interbedded sandstone, siltstone, and mudstone with scattered diabase dike intrusions.

### **Soil Properties**

Soils within the project area include roadway embankment fill, artificial fill, alluvial, and Triassic residual soils.

Roadway embankment fill soils are present along the existing roadways on the project. These soils are similar to and derived from the Triassic residual soils encountered elsewhere on the project.

Artificial fill soils were encountered behind the existing landscape supply and construction laydown yards located approximately between -L- Stations 27+90 and 34+80. Artificial fill used as trench backfill should be anticipated along the sewer easement near -L- Station 28+00. Soils were observed piled in the wooded area at the rear of the landscape supply yard and at the rear of the construction laydown yard. The soil piles were observed to contain construction debris, topsoil, strippings, and a mix of sandy and clayey soils. Some artificial fill was placed at the rear of the construction laydown yard to level site grades. Where encountered in the borings, these soils consist of moist, sandy clay (A-6).

Alluvial soils are present near the surface in the creek along the sewer easement near -L- Station 28+00. These soils consist predominately of silts (A-4), although the bed of the creek appeared to be recently disturbed and lined with new rip rap prior to the field investigation.

Triassic residual soils are derived from the weathering of underlying sedimentary rock. These soils consist of orange, gray, and red-brown sandy silt (A-4), sandy clay (A-6), and silty clay (A-7). The near surface residual soils are generally soft to very stiff and become hard with depth. Most of the residual soils encountered along the project were moist when sampled although some near surface soils in the low lying areas were wet. Based on the laboratory testing performed, the clay soils at the site exhibit medium to high plasticity. The Triassic residual soils transition into weathered rock within the range of boring depths along a majority of the project.

### **Degradable Rock**

Weathered rock was encountered near proposed grades along the portions of the site mentioned herein. Triassic siltstone and weathered rock derived from Triassic siltstone typically exhibits high slaking characteristics when excavated or exposed to weather. Based on the weathered rock recovered in the split spoons taken along the project corridor, the weathered rock along the project is highly degradable.

Non-crystalline sedimentary Triassic rock underlies the project at depth, however none was encountered within 6 feet of proposed finished grade.

### **Groundwater**

A majority of the borings performed at the site remained open for a minimum of 24 hours to measure groundwater levels after drilling. The open boreholes were dry or groundwater was encountered well below proposed grades with the exception of the locations listed in "Areas of Special Geotechnical Interest." However, the potential for a perched water table is high along the project where soft soils are present above impermeable hard material such as weathered rock.

### **Ponds**

One pond is located adjacent to the project between approximately -L- Stations 68+40 and 70+80. The overflow is due northeast and drainage flows toward the project. The pond is up gradient from the natural surface and should not be affected by runoff from the project. No flowing water was observed draining from the pond. However, the soils down gradient from the dam between approximately -L-

Project Reference No.	Sheet No.
012108004	3B

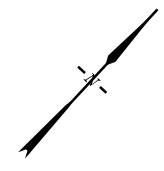
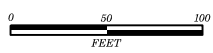
Stations 68+80 and 70+00 were very wet and soft possibly due to seepage beneath the existing earth dam.

Respectfully Submitted,  
**Terracon Consultants, Inc.**

Matthew J. Alexander, PE  
Geotechnical Project Engineer

R. L. "Levi" Denton, II, PE  
Principal, Geotechnical  
Department Manager

**INCOMPLETE PLANS**  
DO NOT USE FOR R/W ACQUISITION  
**PRELIMINARY PLANS**  
DO NOT USE FOR CONSTRUCTION



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JOSEPH M. WILKERSON  
PIN# 746838759  
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DB 2596 PG 696

N/F  
ATM OF NC, LLC  
PIN# 746837520  
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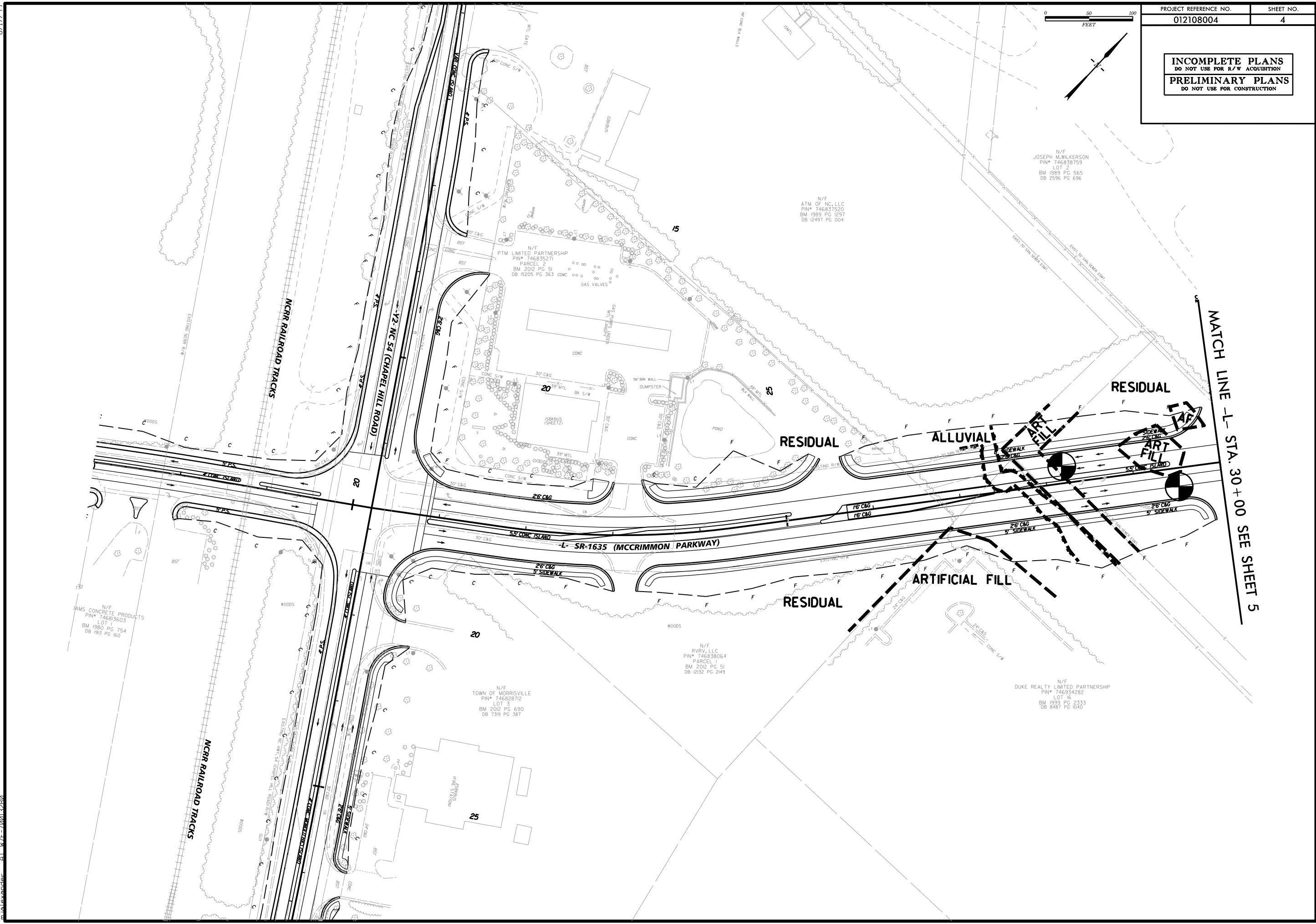
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LOT 1  
BM 1980 PG 754  
DB 1913 PG 160

N/F  
TOWN OF MORRISVILLE  
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LOT 3  
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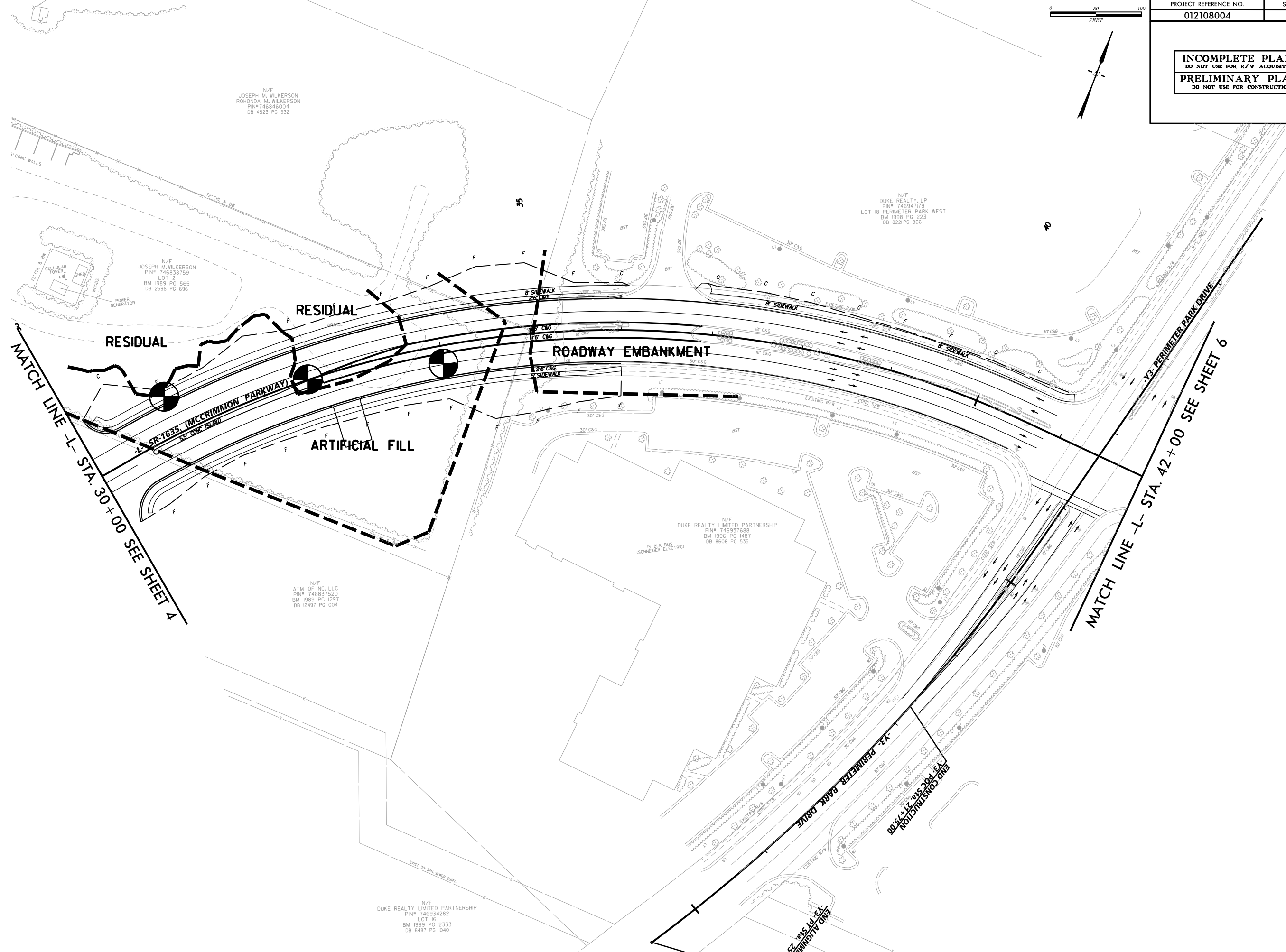
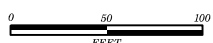
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**PRELIMINARY PLANS**  
DO NOT USE FOR CONSTRUCTION



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MATCH LINE -L- STA. 42+00 SEE SHEET 6

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ROHONDA M. WILKERSON  
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PIN# 746838759  
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PIN# 746947179  
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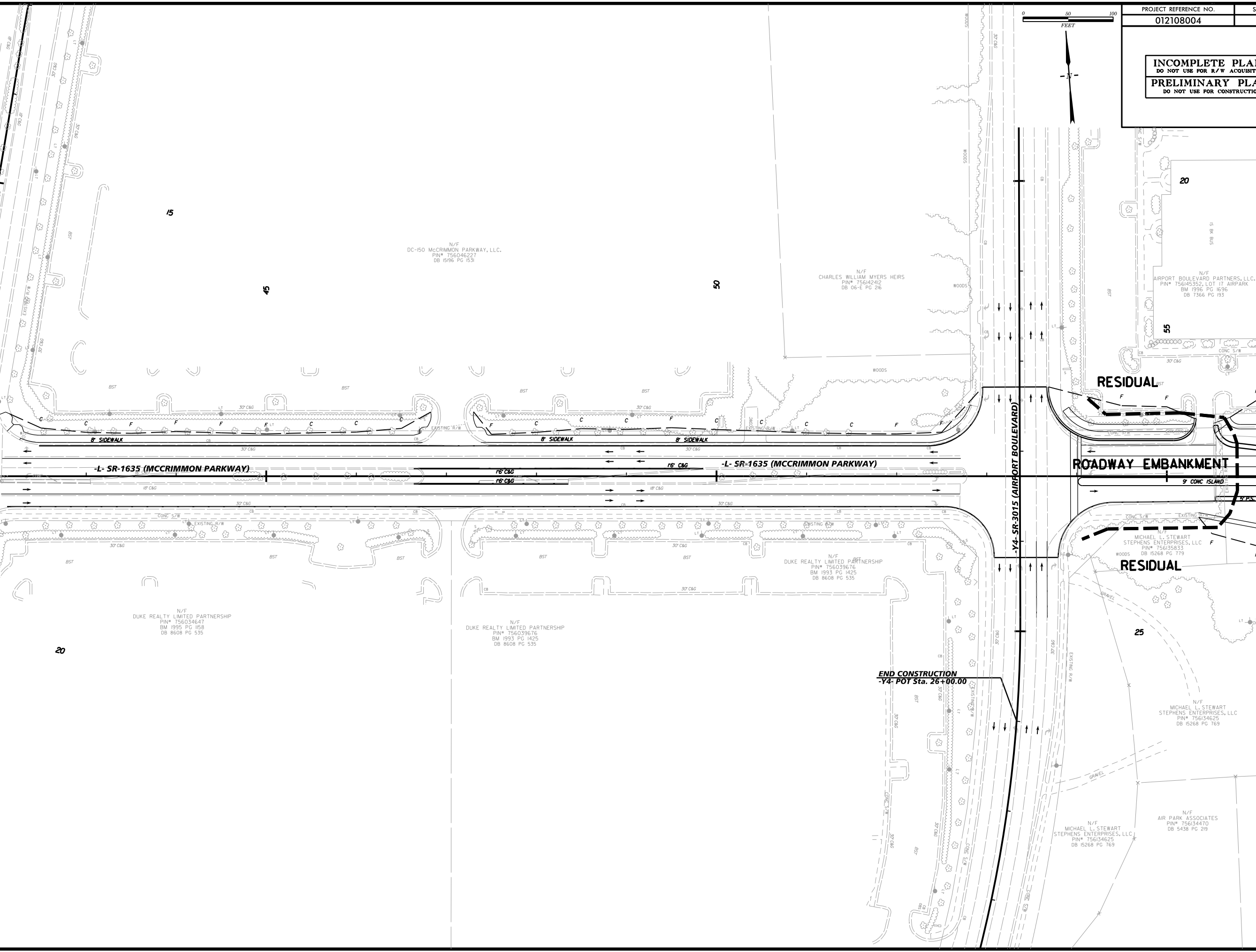
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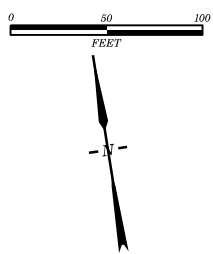
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MATCH LINE -L- STA. 69 + 00 SEE SHEET 8

RESIDUAL

RESIDUAL

RESIDUAL

RESIDUAL

RESIDUAL

RESIDUAL

N/F  
M.A.L. PROPERTIES, LLC  
CENTRAL CAROLINA REALTY, INC.  
MICHAEL L. STEWART  
STEPHENS ENTERPRISES, LLC.  
PIN# 756230220  
DB 15268 PG 759

N/F  
AIS FORESTRY & FARMING, LLC  
STEPHENS ENTERPRISES, LLC  
PIN# 756330565  
DB 15268 PG 759

N/F  
MICHAEL L. STEWART  
STEPHENS ENTERPRISES, LLC  
PIN# 756346225  
DB 15268 PG 769

N/F  
STEPHENS ENTERPRISES, LLC  
PIN# 756236139  
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AIR PARK ASSOCIATES  
PIN# 756235875  
FUTURE R/W  
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DB 5438 PG 219

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EXTER 120 TRANS AIR, LLC.  
PIN# 756241238  
LOT 16 AIRPARK  
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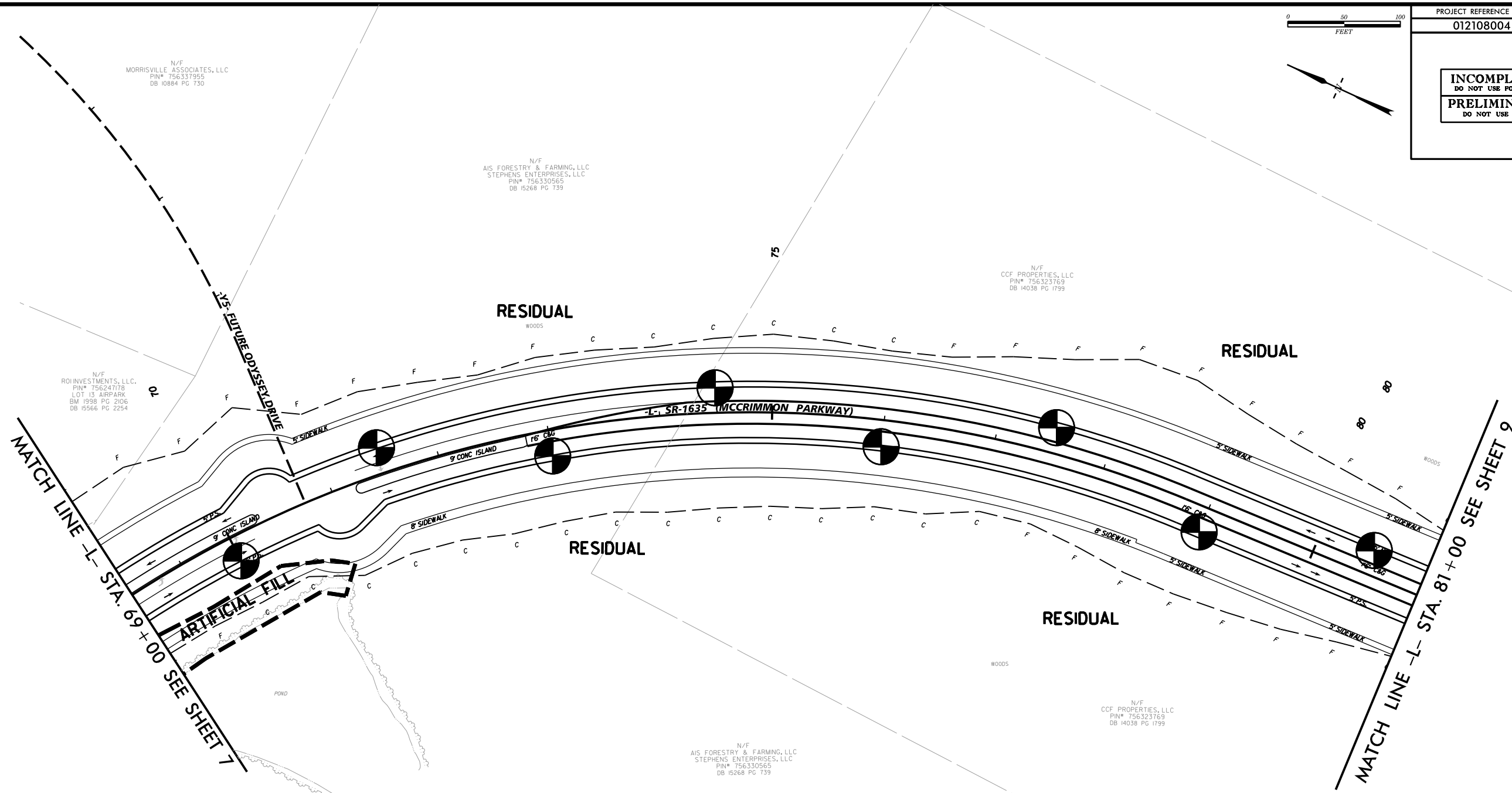
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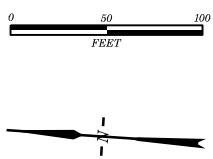


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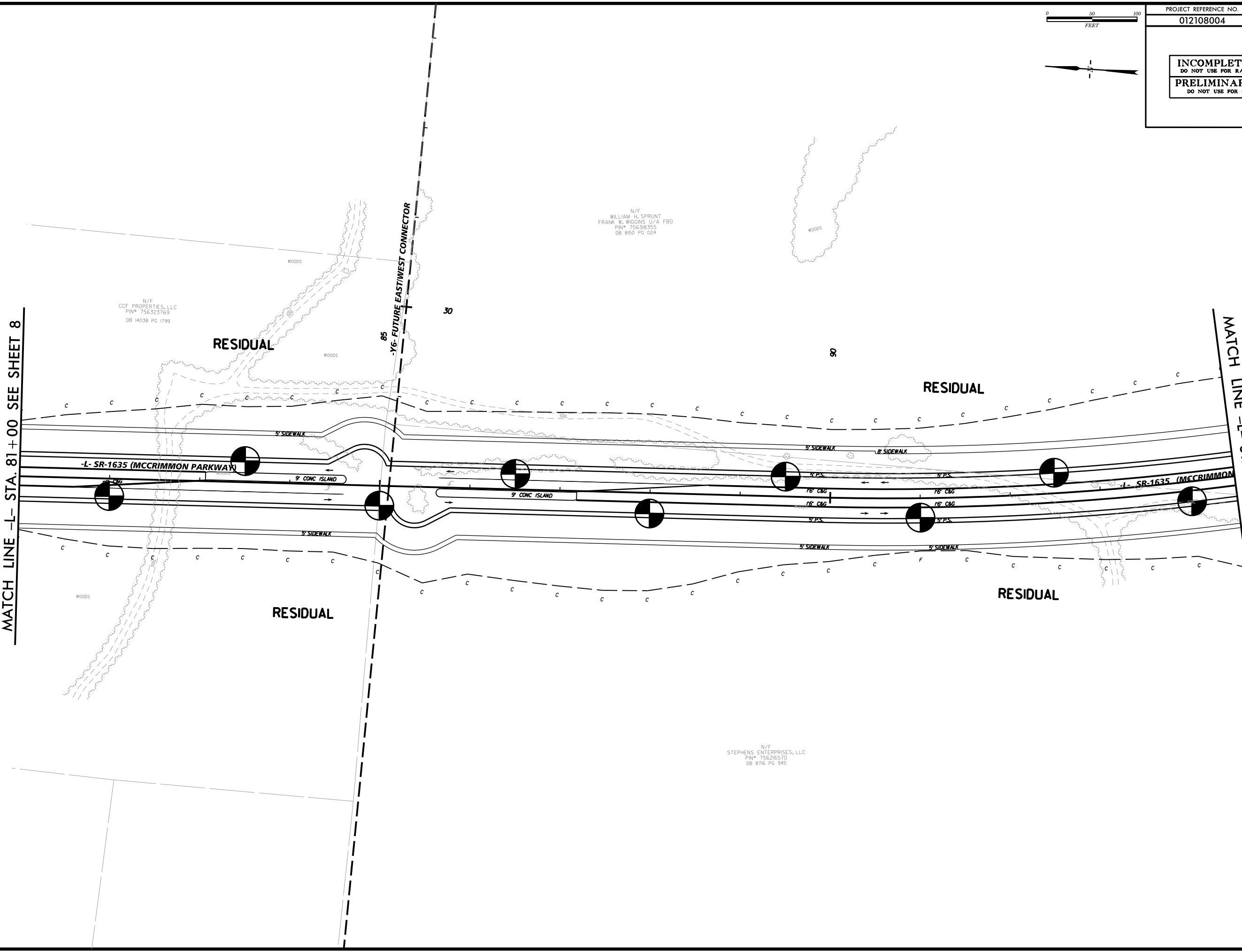
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PIN# 756323769  
DB 14038 PG 1799

N/F  
WILLIAM W. SPRUNT  
FRANK W. WIGGINS U/A FBO  
PIN# 756318355  
DB 8150 PG 024

N/F  
STEPHENS ENTERPRISES, LLC  
PIN# 756216570  
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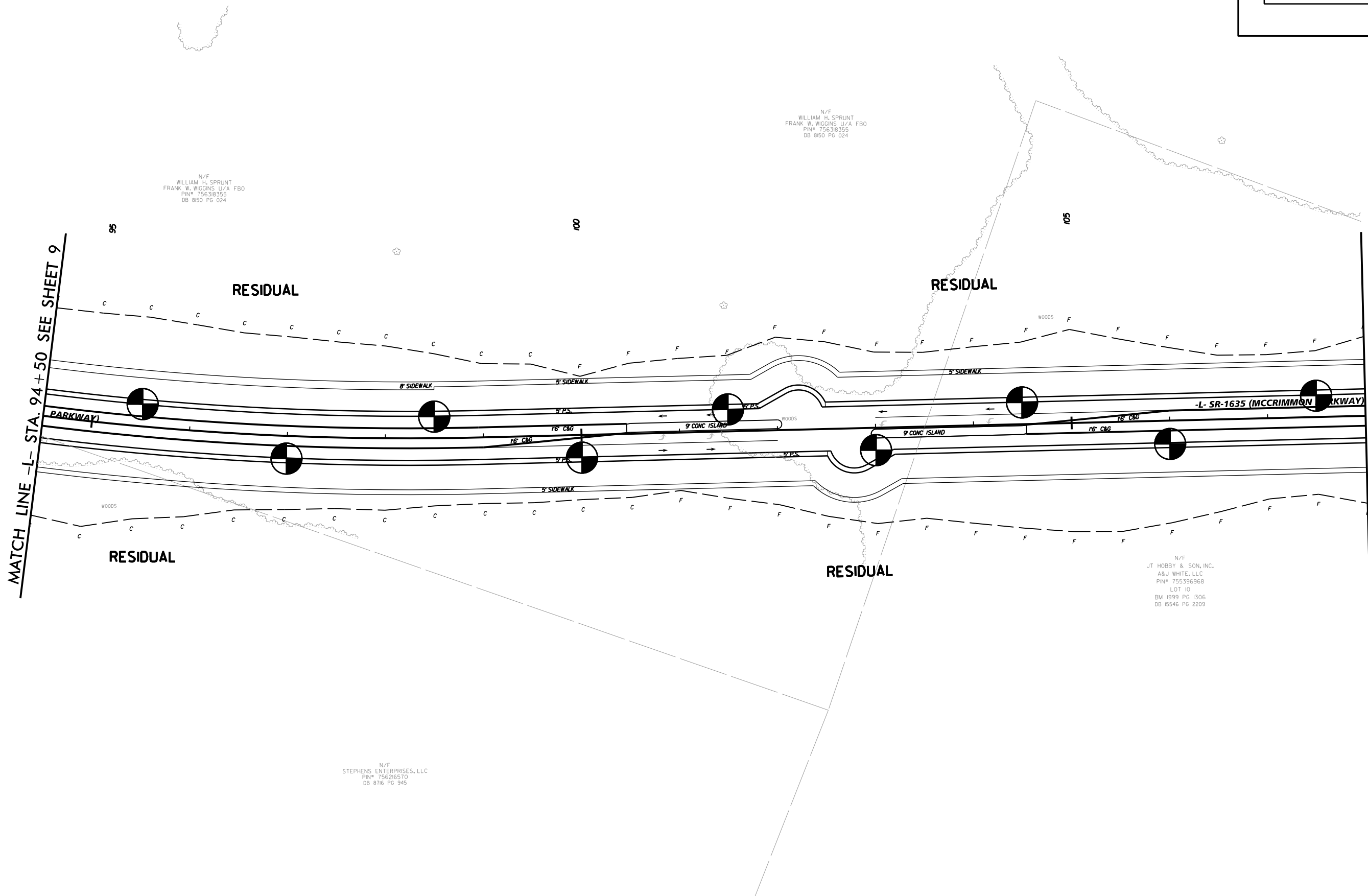


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MATCH LINE -L- STA. 94+50 SEE SHEET 9

MATCH LINE -L- STA. 108+00 SEE SHEET 11



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FRANK W. WIGGINS U/A FBO  
PIN# 756318355  
DB 850 PG 024

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FRANK W. WIGGINS U/A FBO  
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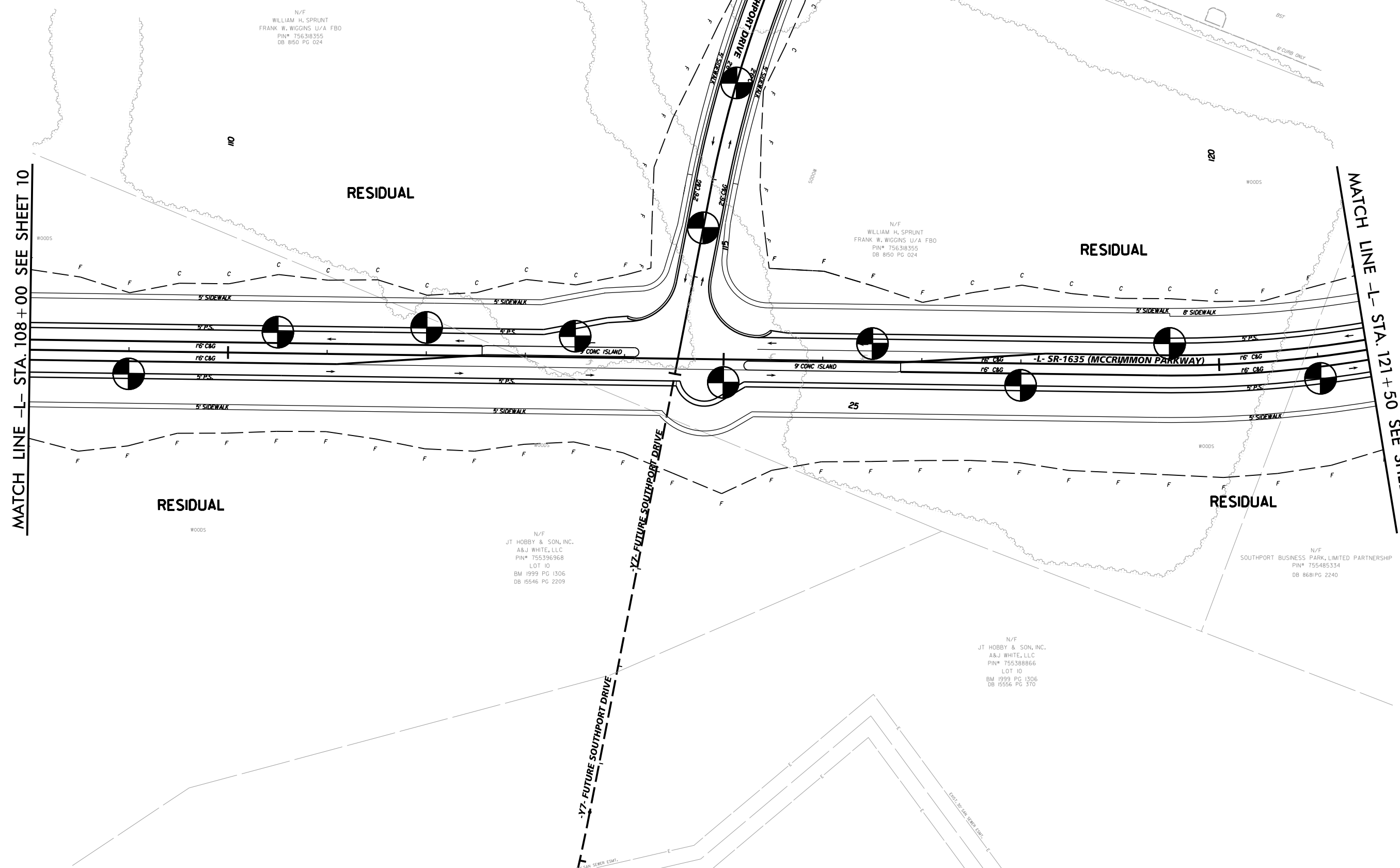
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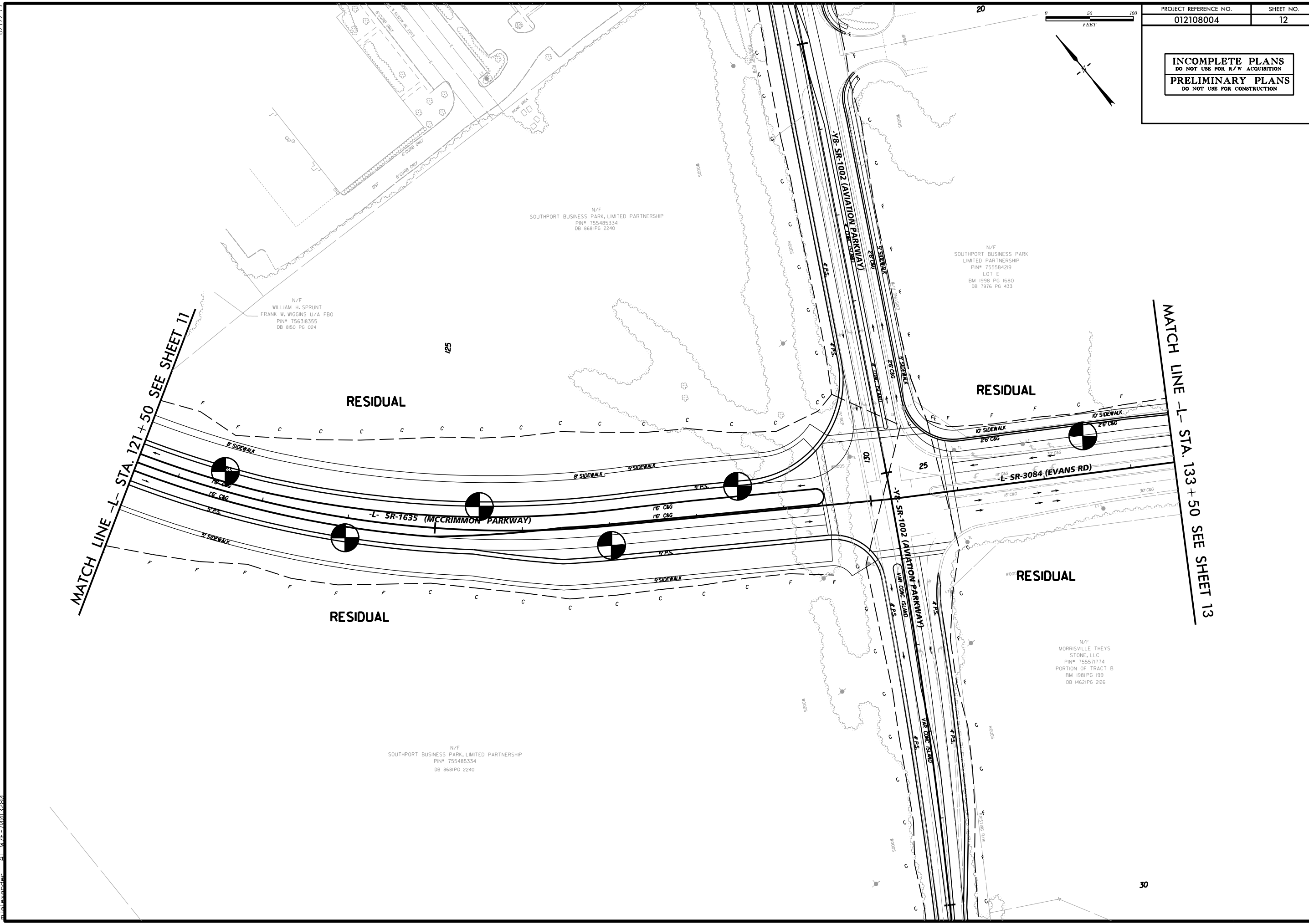


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FRANK W. WIGGINS U/A FBO  
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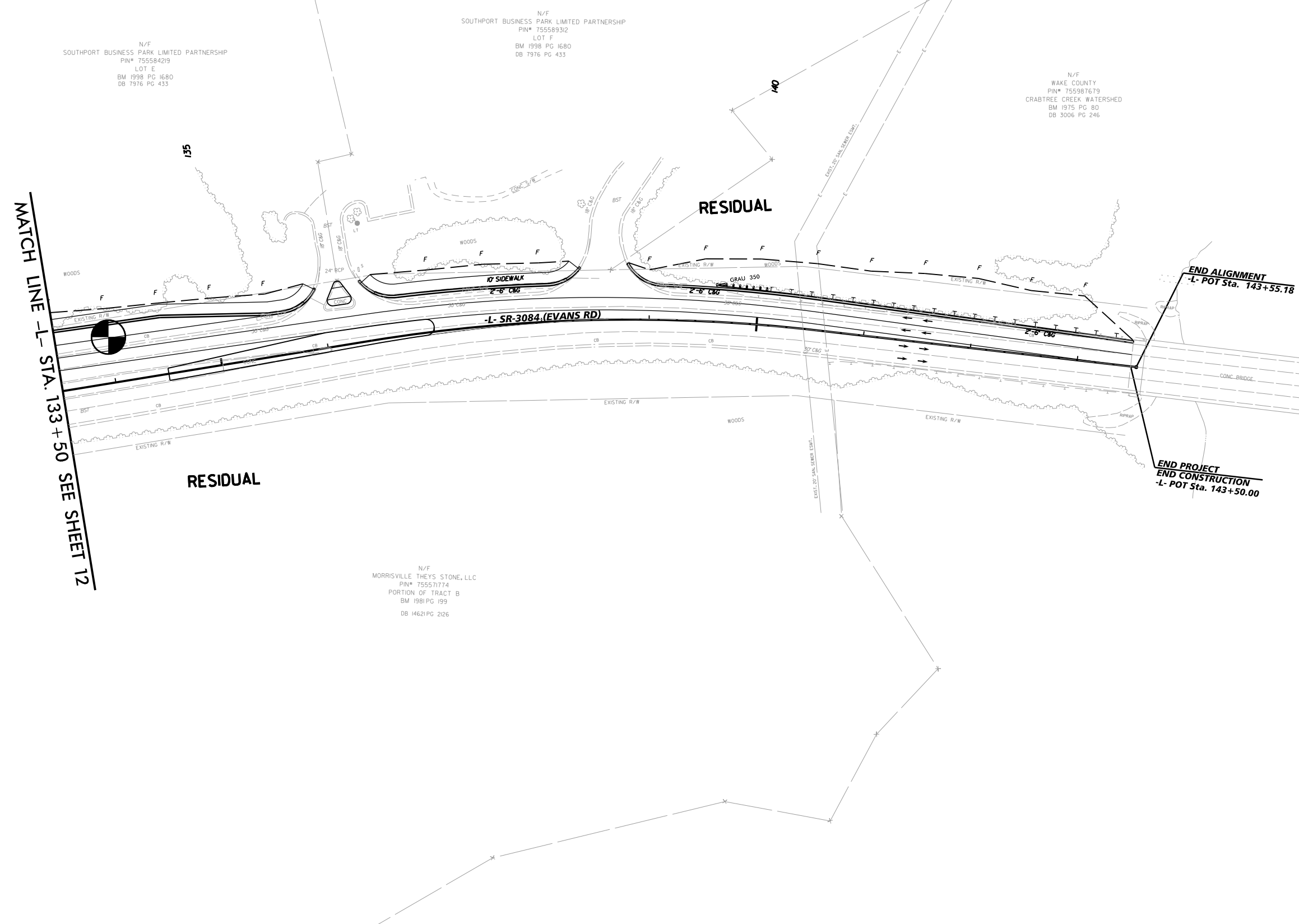
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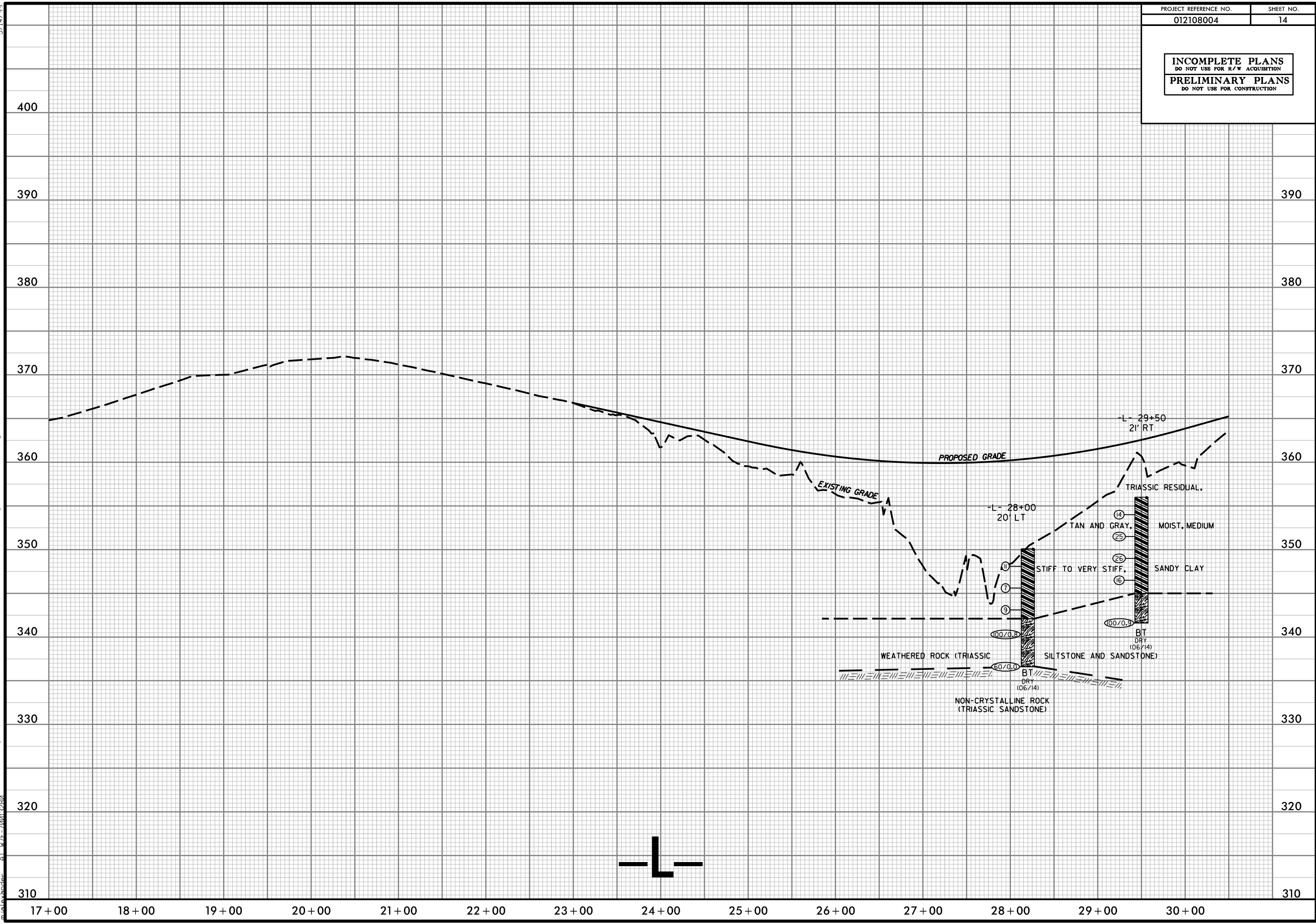
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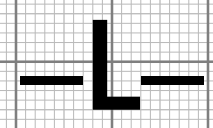


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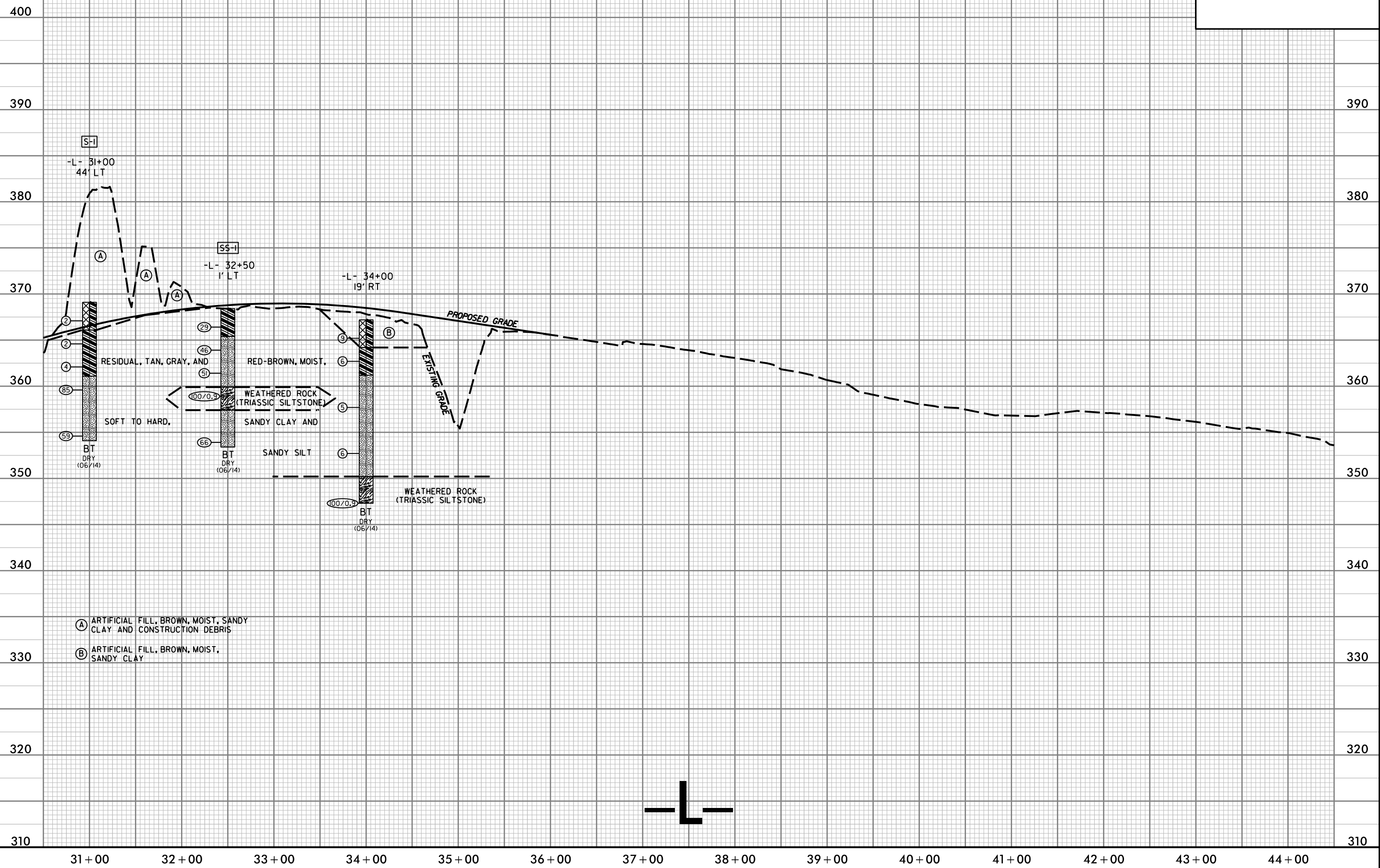
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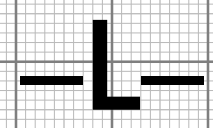
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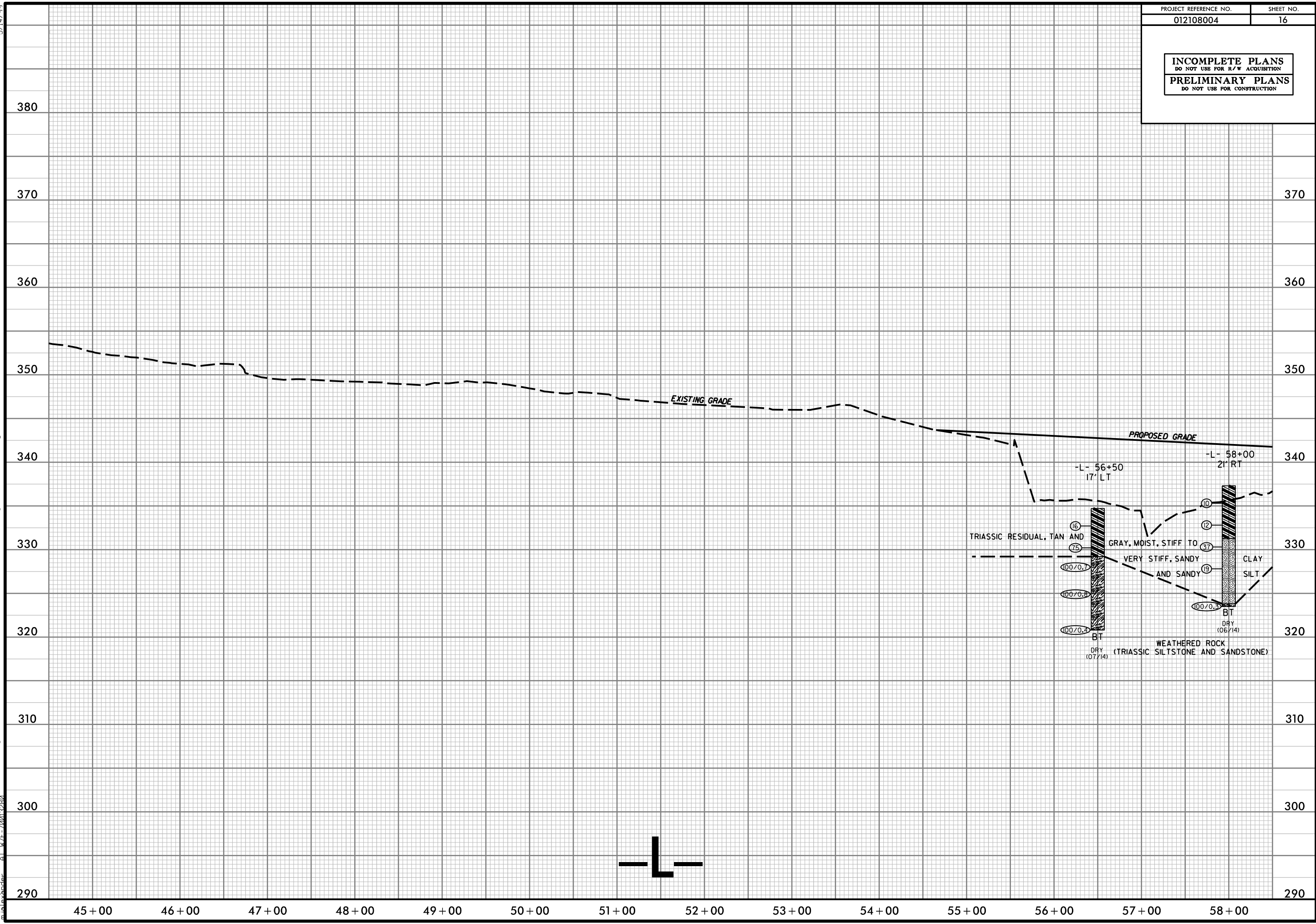


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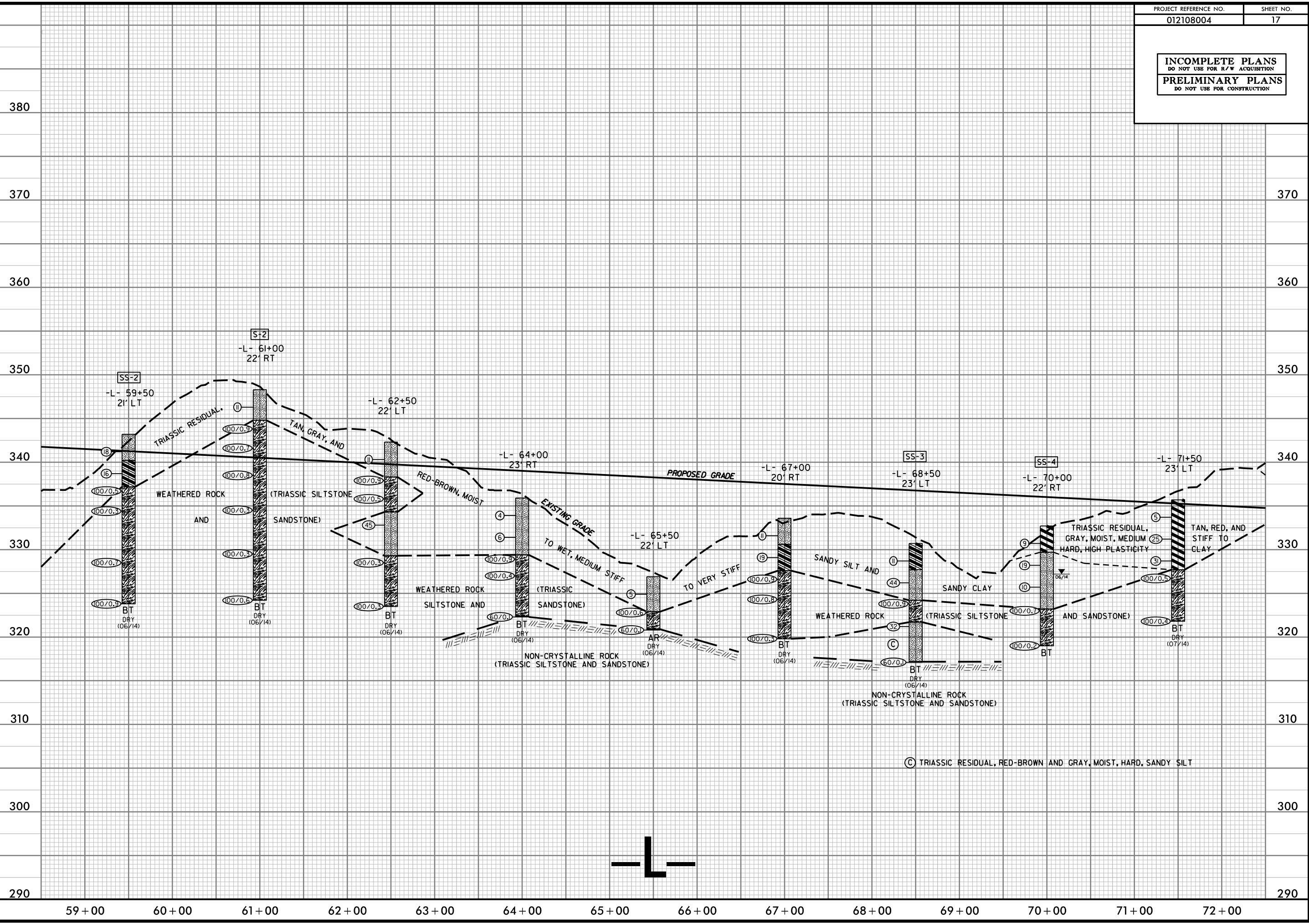
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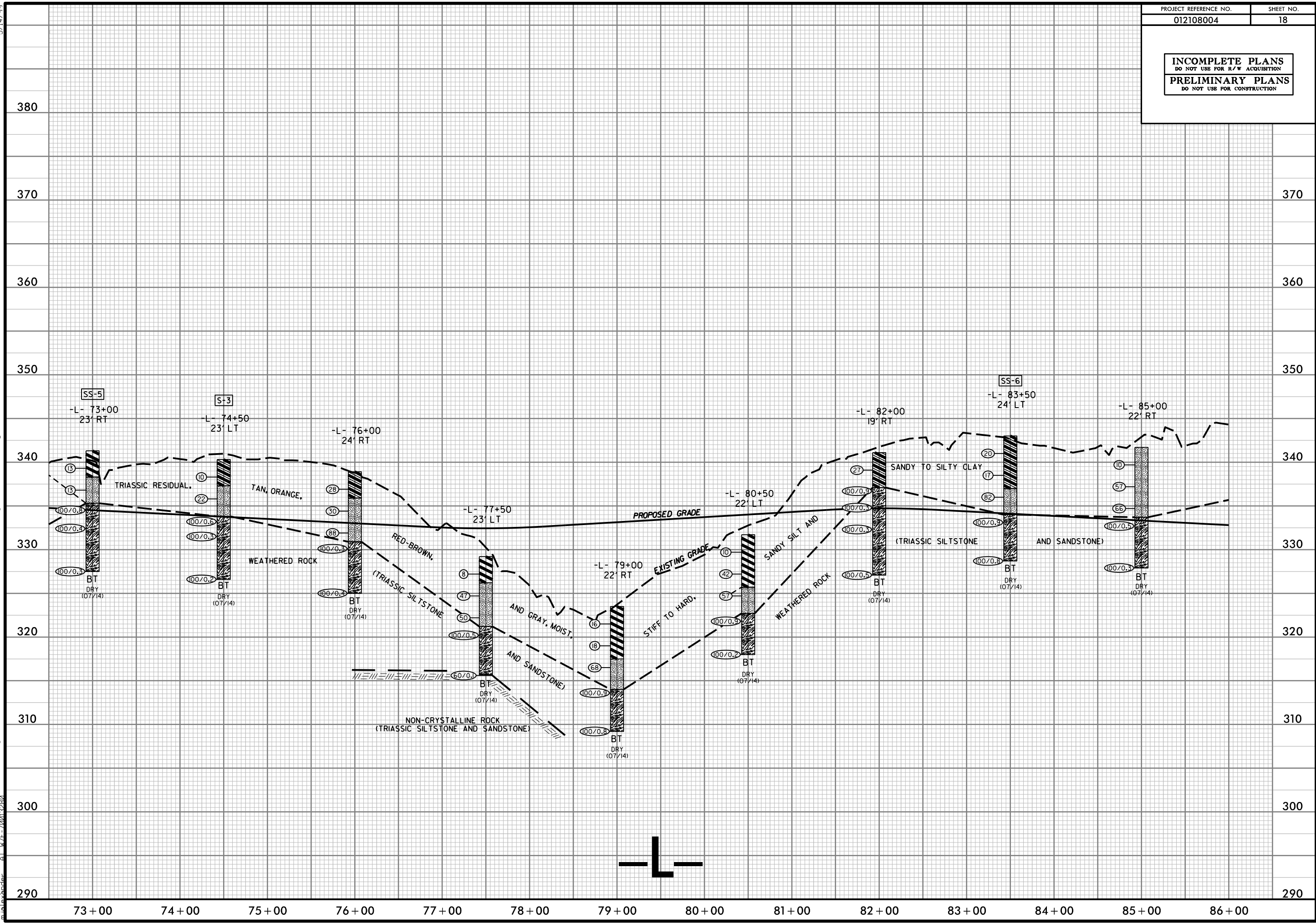
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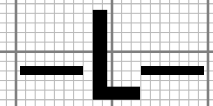
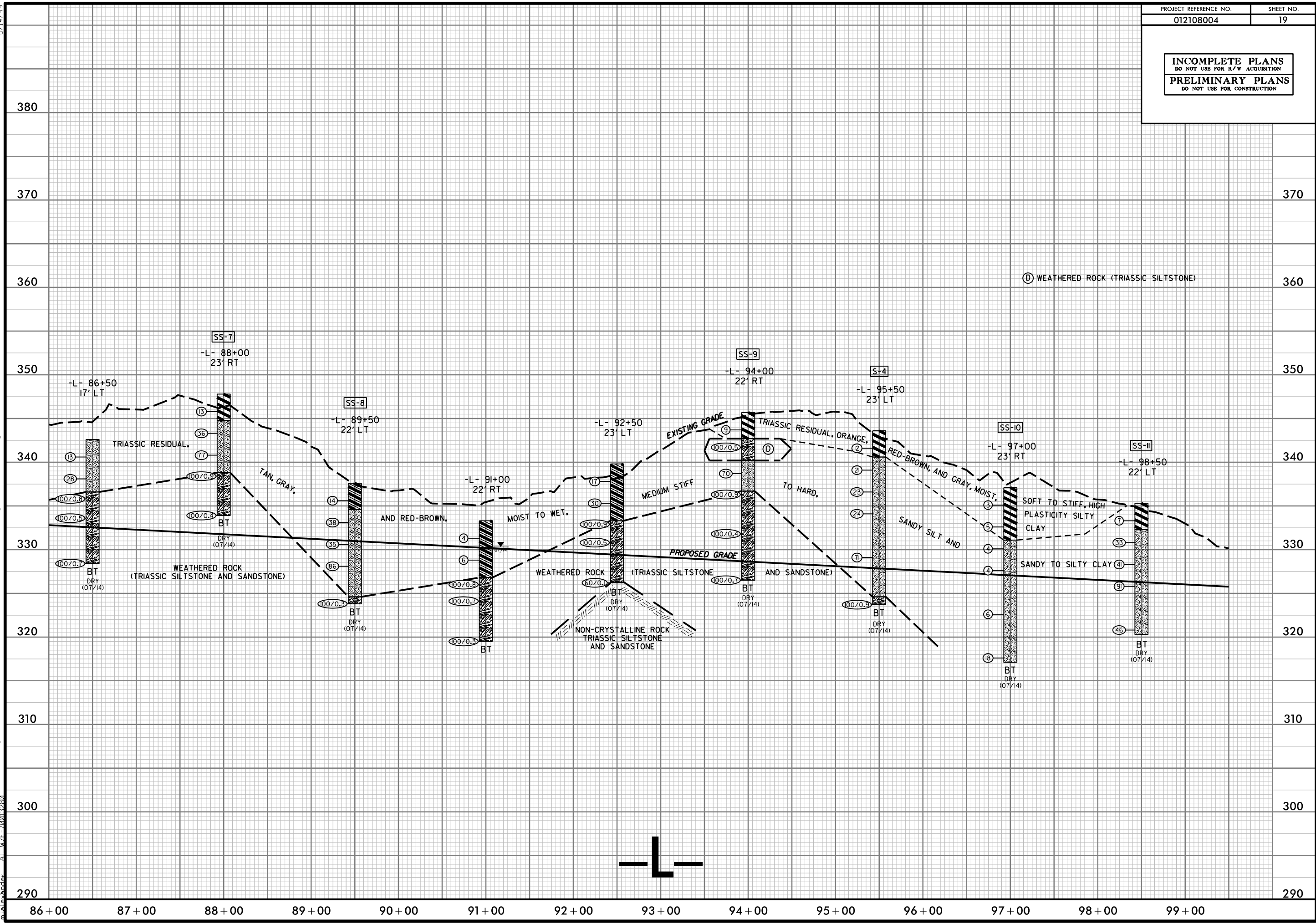
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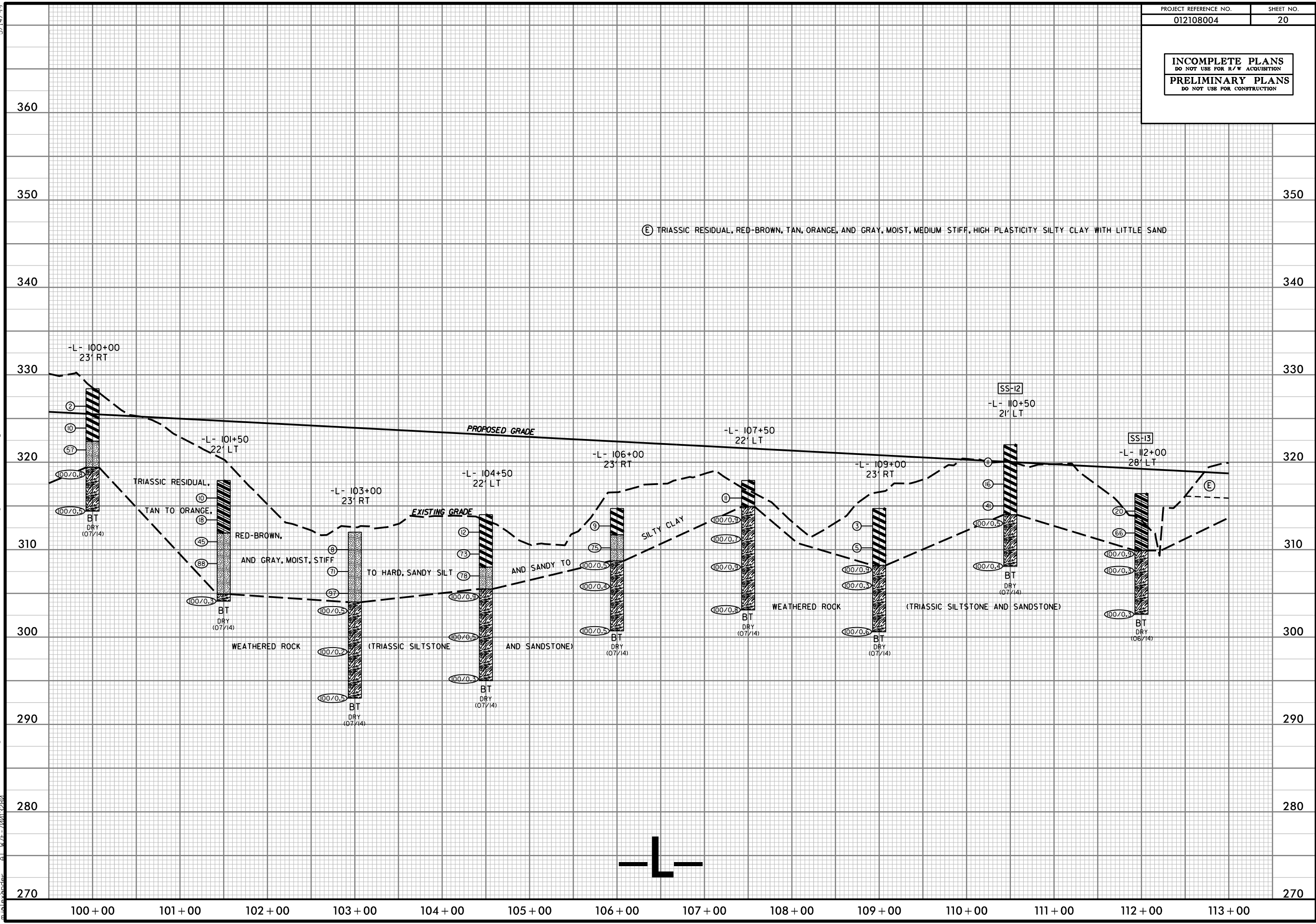
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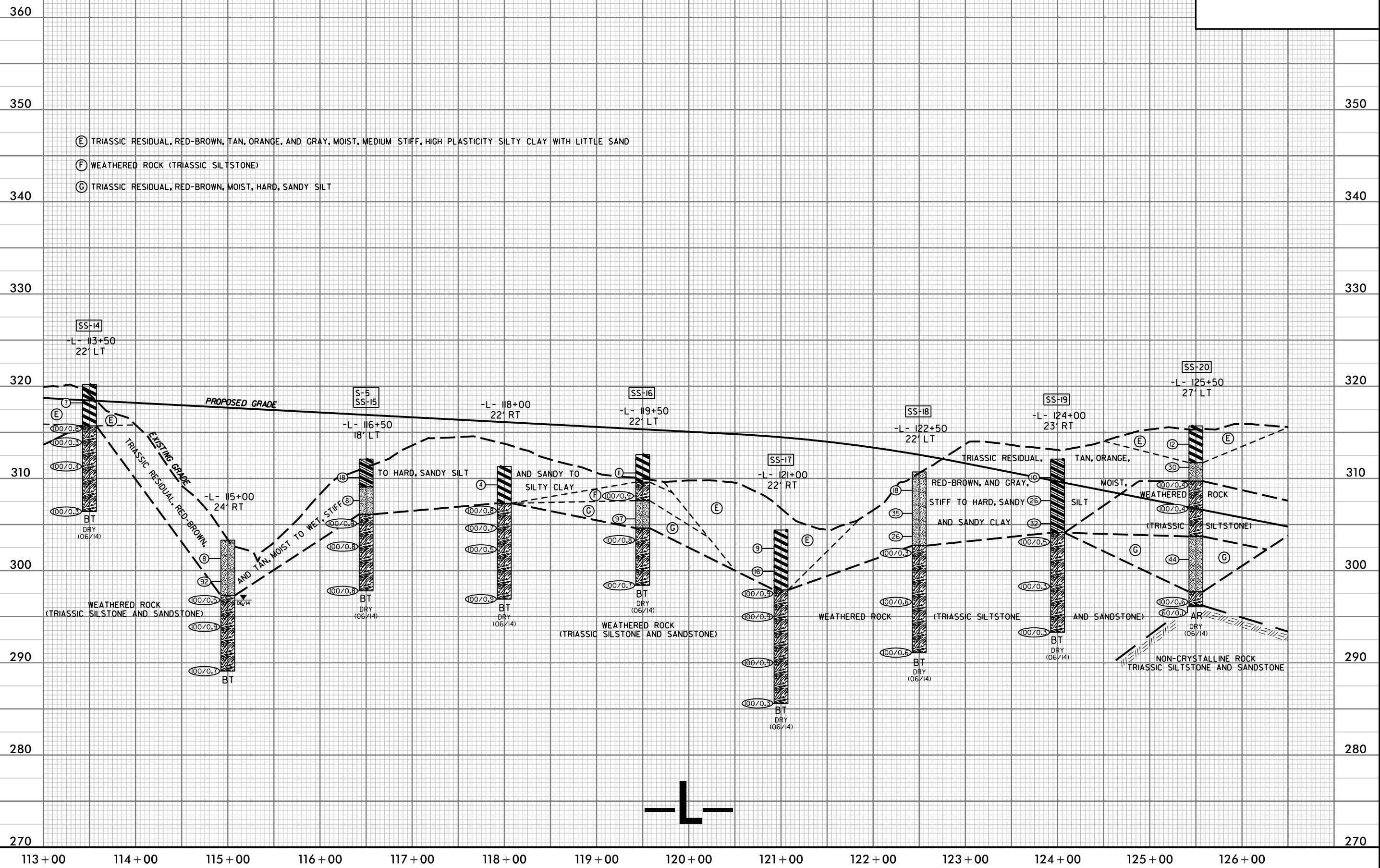
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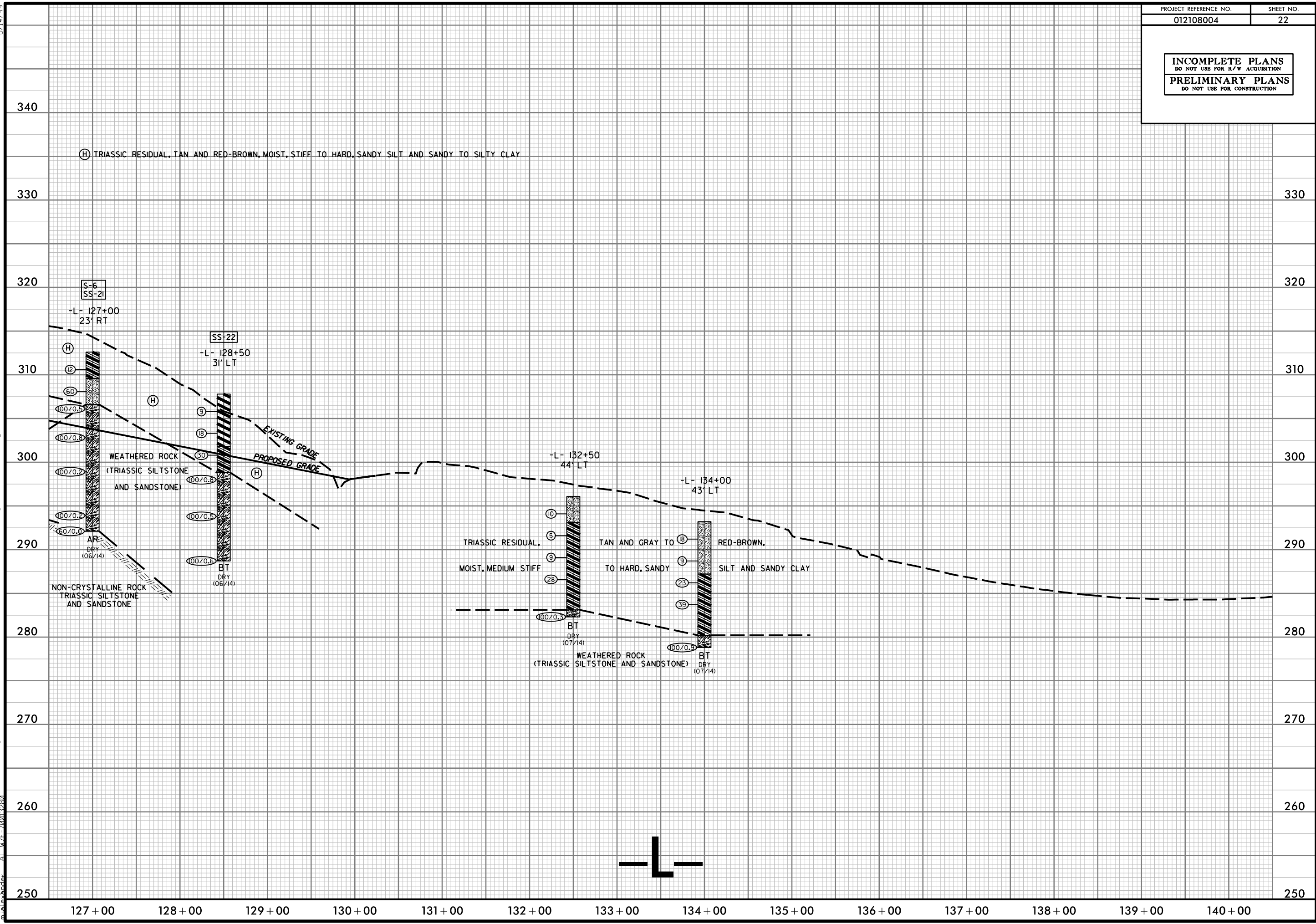
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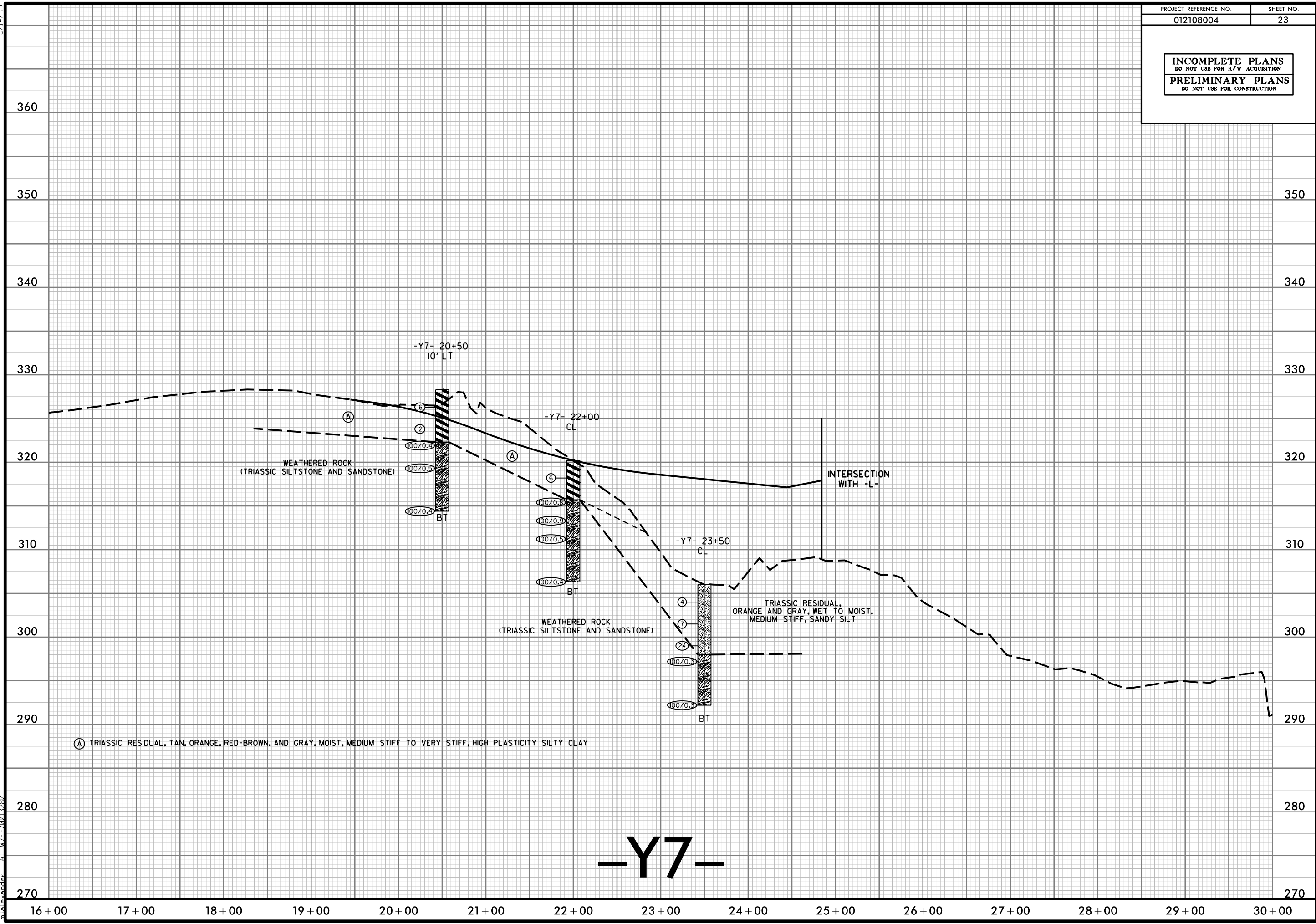
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**PRELIMINARY PLANS**  
DO NOT USE FOR CONSTRUCTION

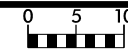
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(A) TRIASSIC RESIDUAL, TAN, ORANGE, RED-BROWN, AND GRAY, MOIST, MEDIUM STIFF TO VERY STIFF, HIGH PLASTICITY SILTY CLAY

**-Y7-**

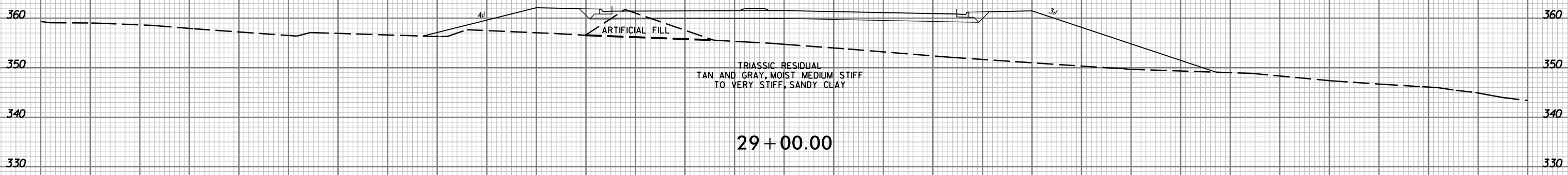
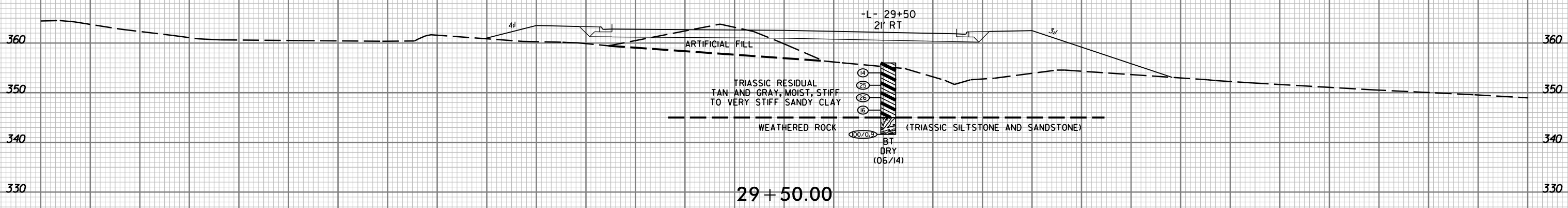
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PROJ. REFERENCE NO.  
012108004

SHEET NO.  
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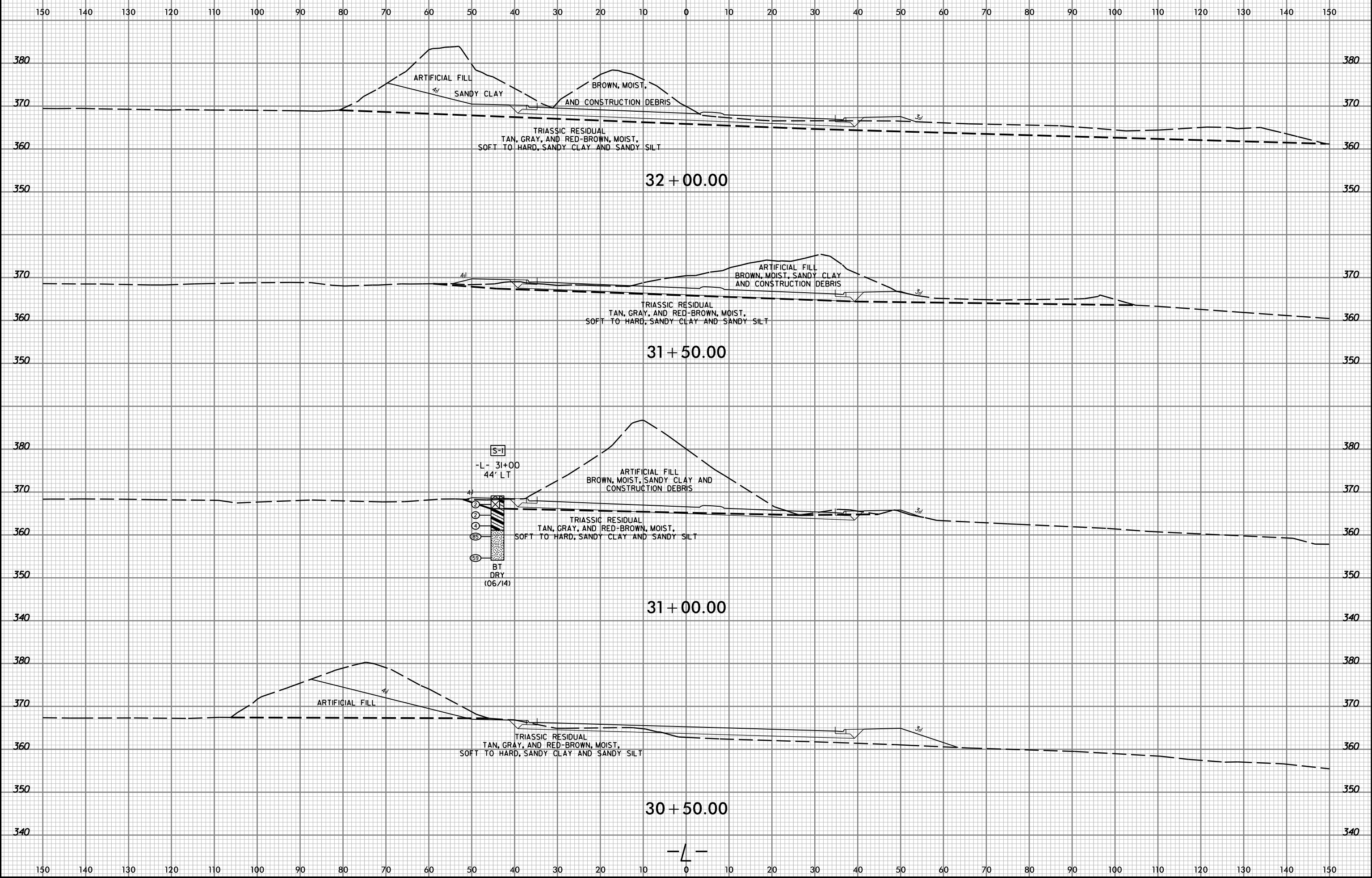


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8/23/99



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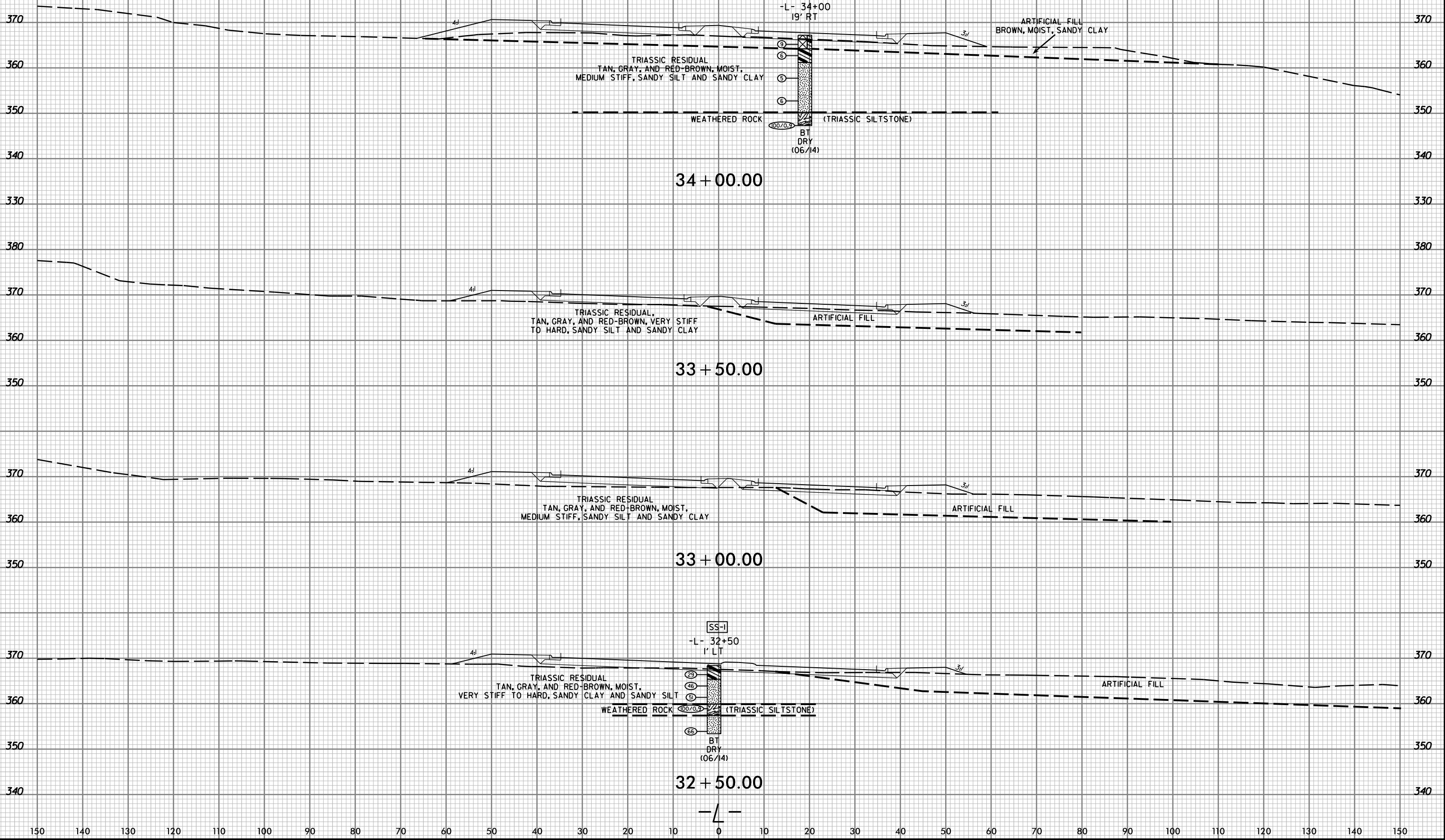
8/23/99



PROJ. REFERENCE NO.  
012108004

SHEET NO.  
26

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34 + 00.00

33 + 50.00

33 + 00.00

32 + 50.00

-L- 34+00  
19' RT

ARTIFICIAL FILL  
BROWN, MOIST, SANDY CLAY

TRIASSIC RESIDUAL  
TAN, GRAY, AND RED-BROWN, MOIST,  
MEDIUM STIFF, SANDY SILT AND SANDY CLAY

WEATHERED ROCK (TRIASSIC SILTSTONE)

BT  
DRY  
(06/14)

TRIASSIC RESIDUAL,  
TAN, GRAY, AND RED-BROWN, VERY STIFF  
TO HARD, SANDY SILT AND SANDY CLAY

ARTIFICIAL FILL

TRIASSIC RESIDUAL  
TAN, GRAY, AND RED-BROWN, MOIST,  
MEDIUM STIFF, SANDY SILT AND SANDY CLAY

ARTIFICIAL FILL

TRIASSIC RESIDUAL  
TAN, GRAY, AND RED-BROWN, MOIST,  
VERY STIFF TO HARD, SANDY CLAY AND SANDY SILT

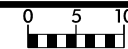
WEATHERED ROCK (TRIASSIC SILTSTONE)

BT  
DRY  
(06/14)

ARTIFICIAL FILL

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8/23/99

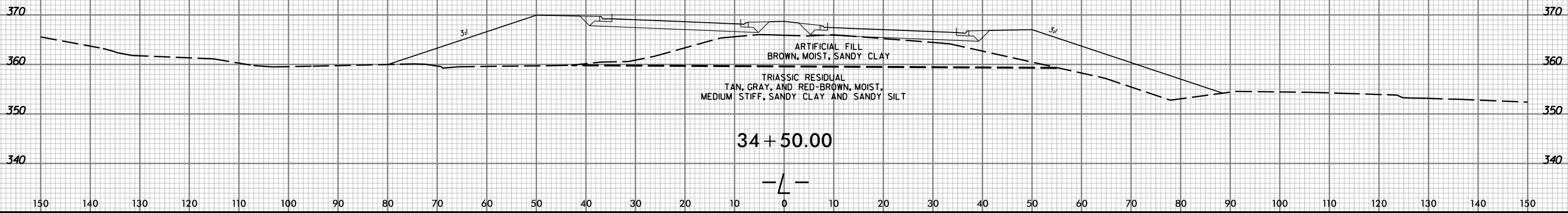


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SHEET NO.  
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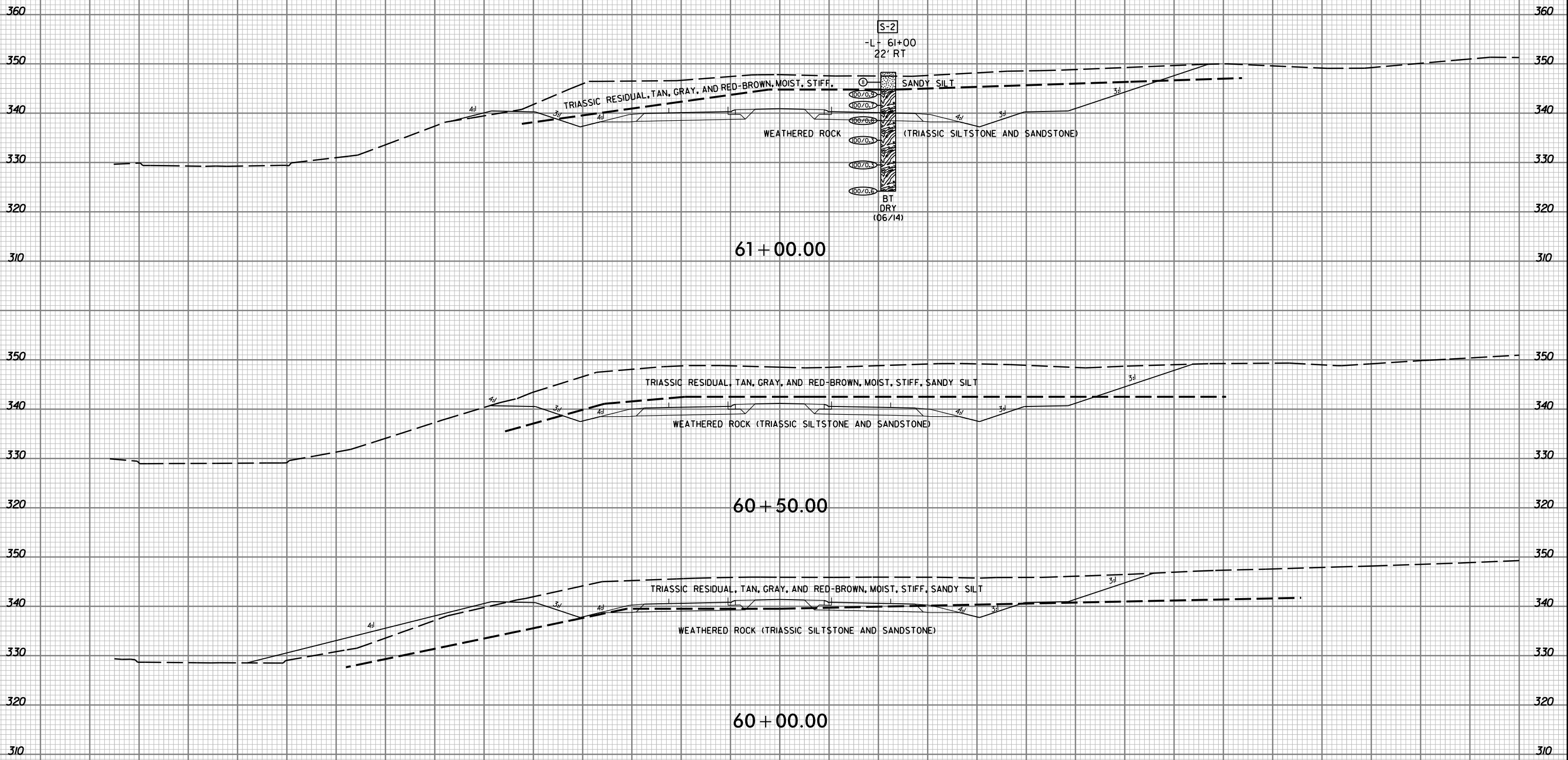
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miller&anderson PLW/E/2003/3580



8/23/99

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61 + 00.00

60 + 50.00

60 + 00.00

-L-

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miller.sander





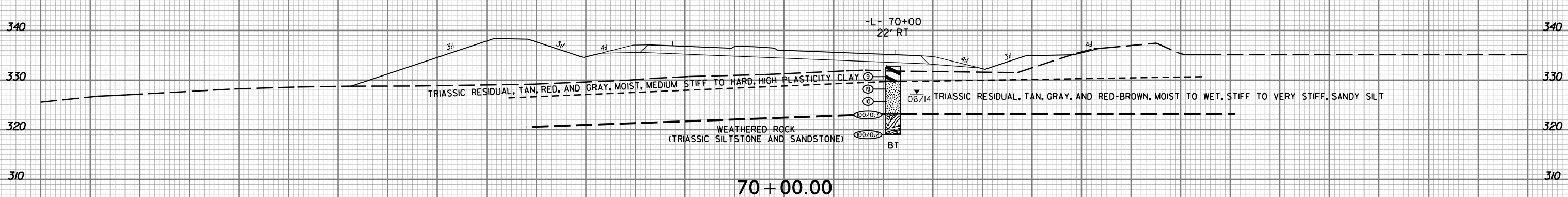
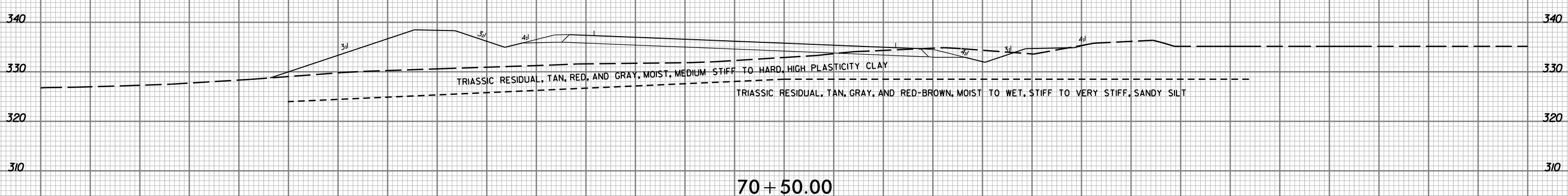
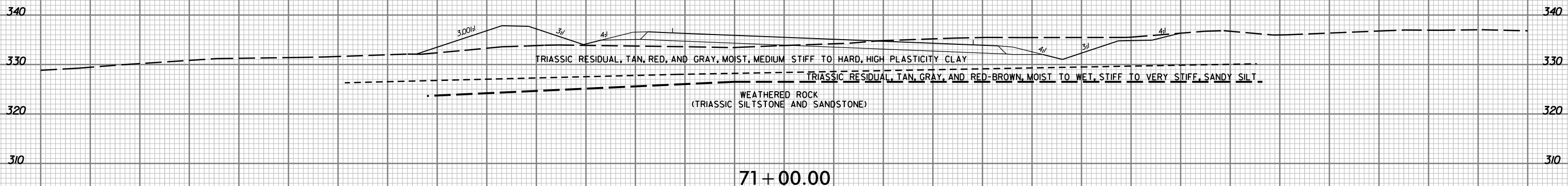
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PROJ. REFERENCE NO.  
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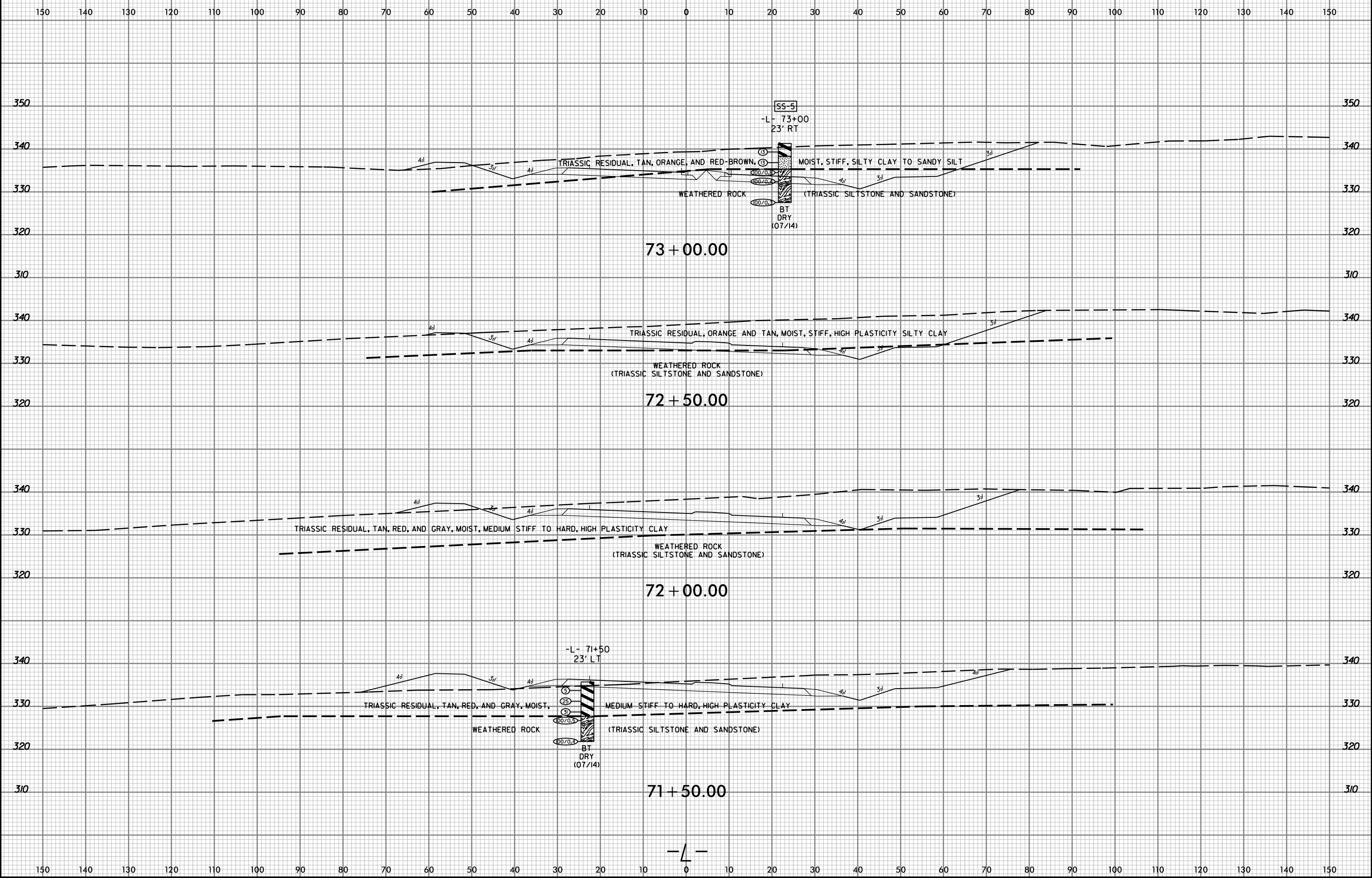
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8/23/99



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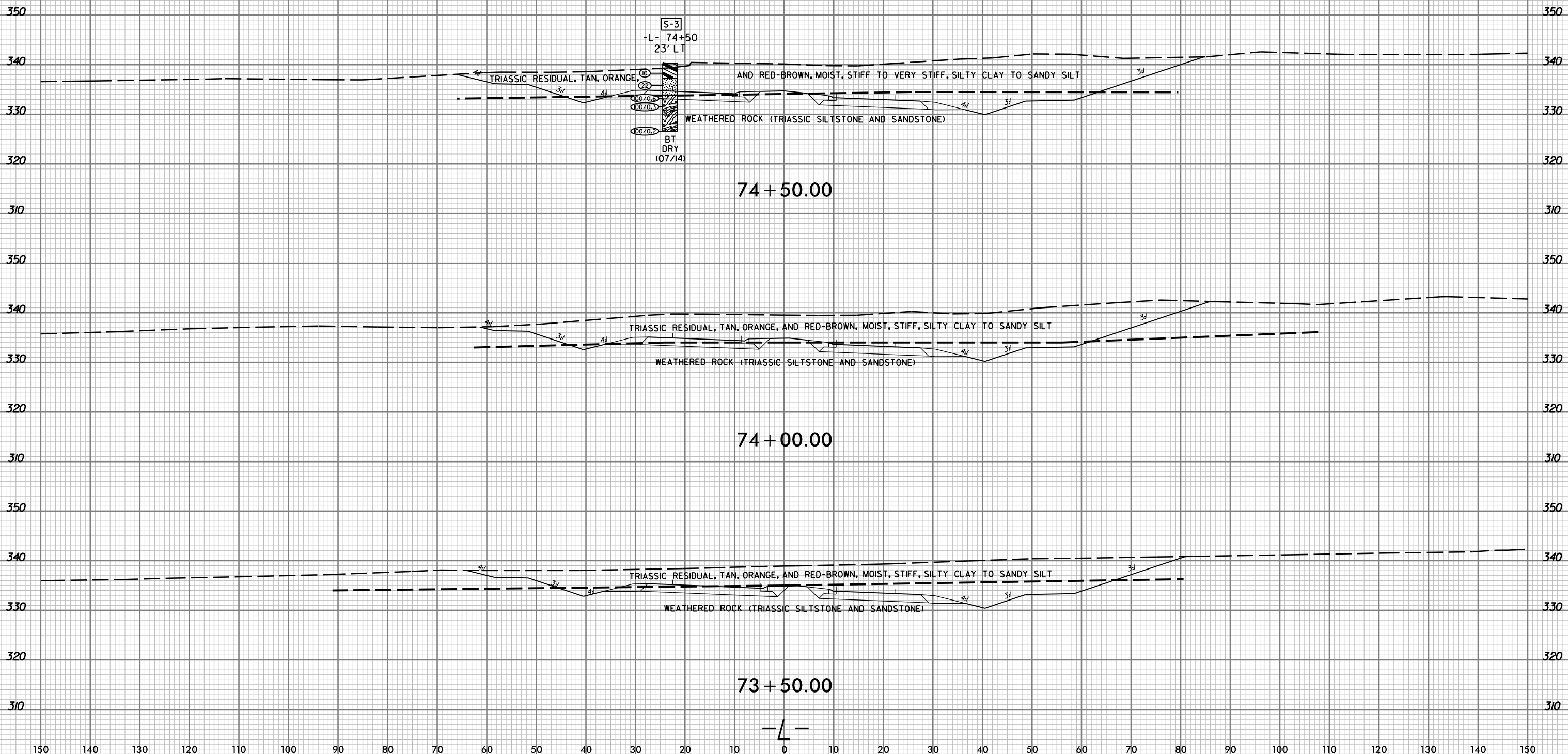
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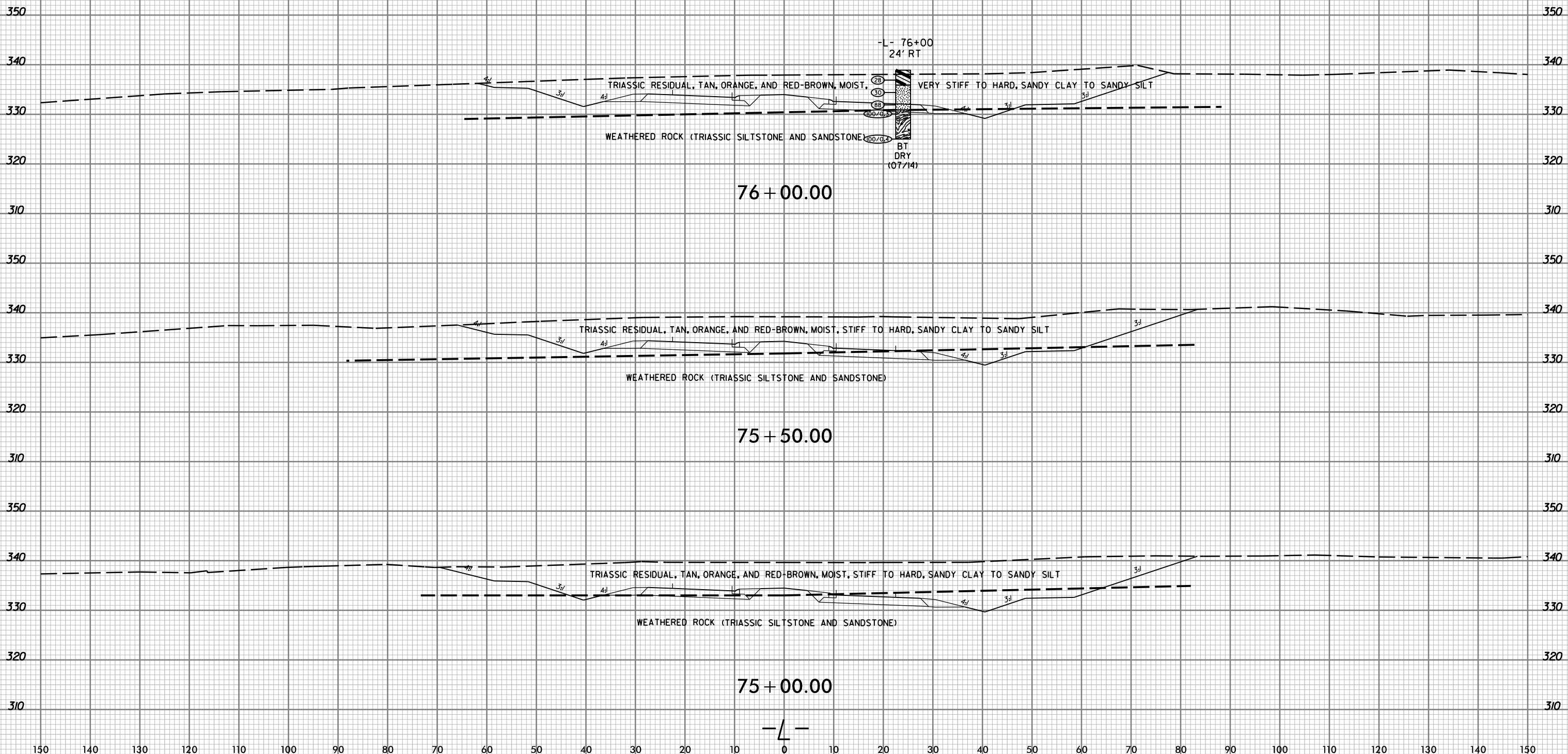
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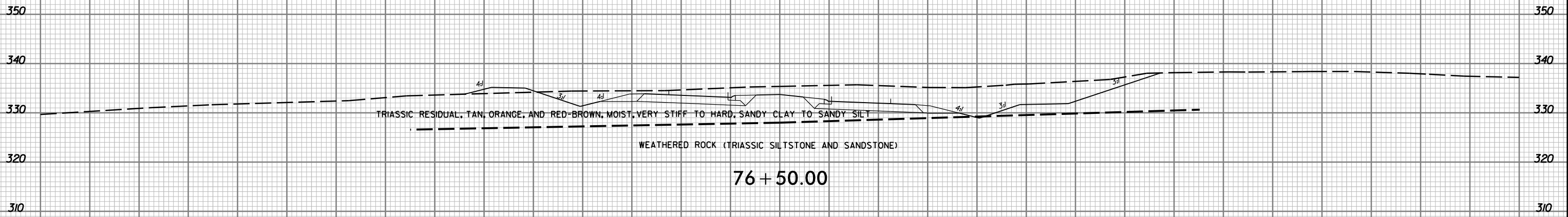


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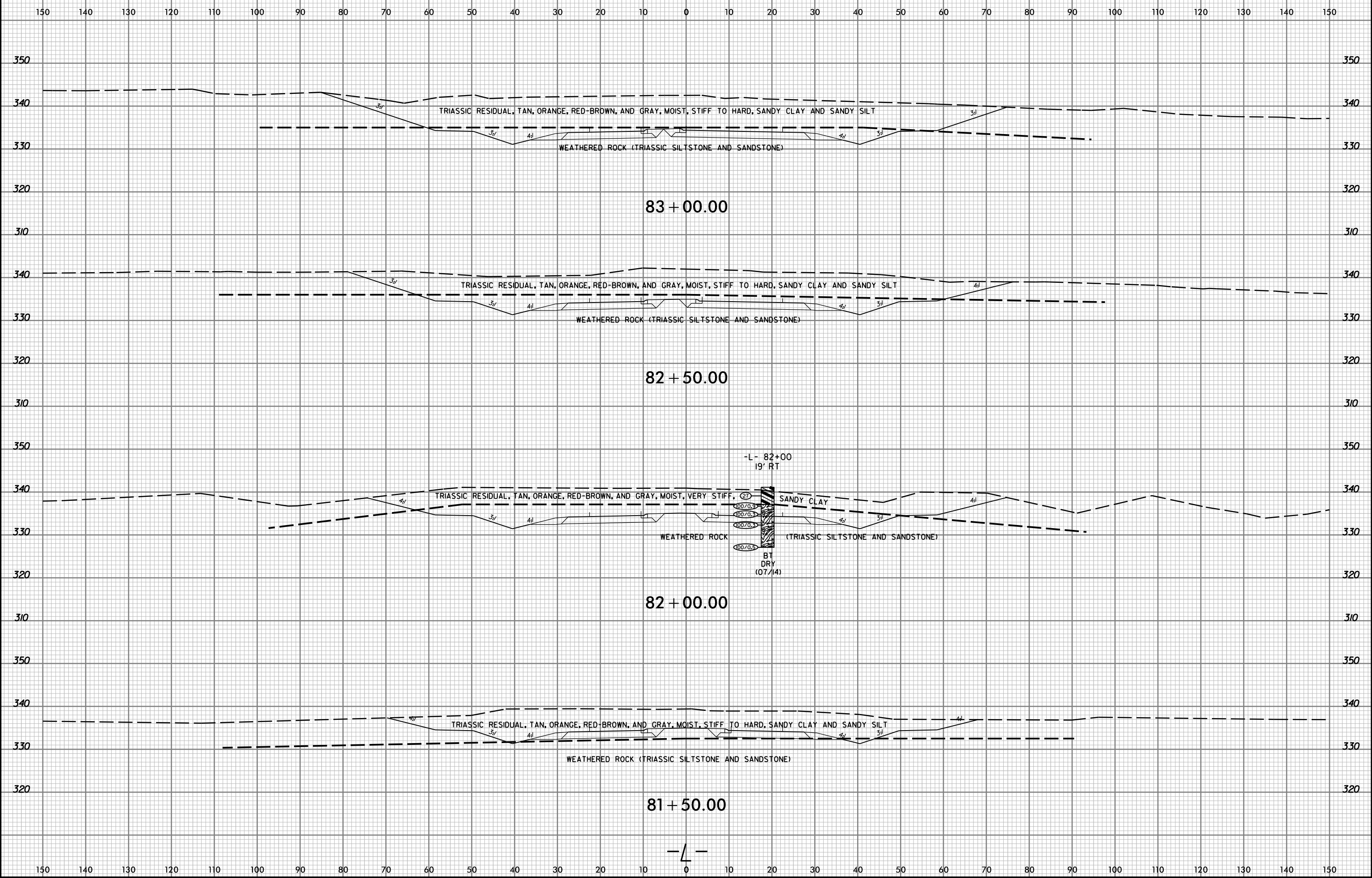
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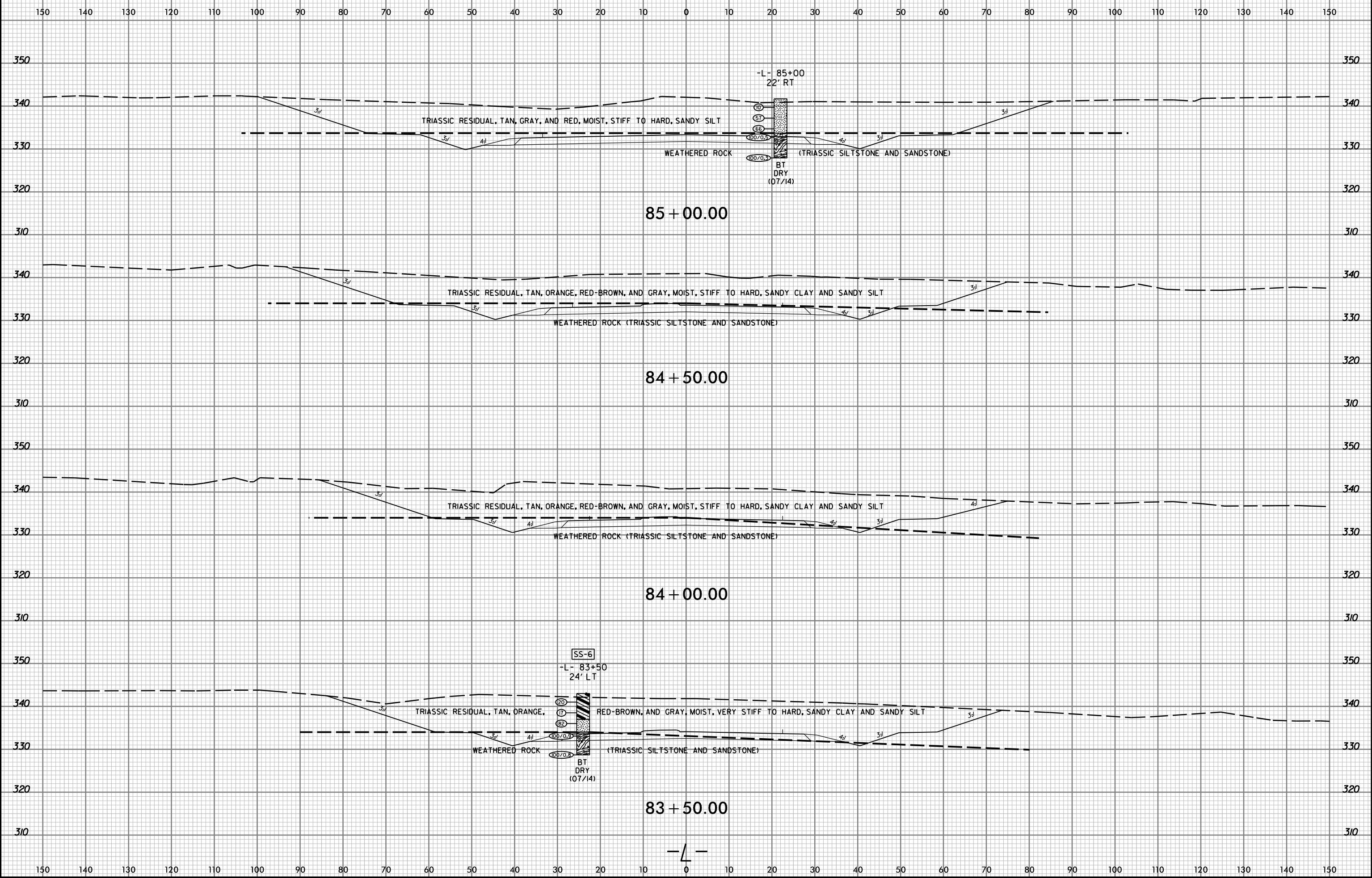
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8/23/99



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8/23/99



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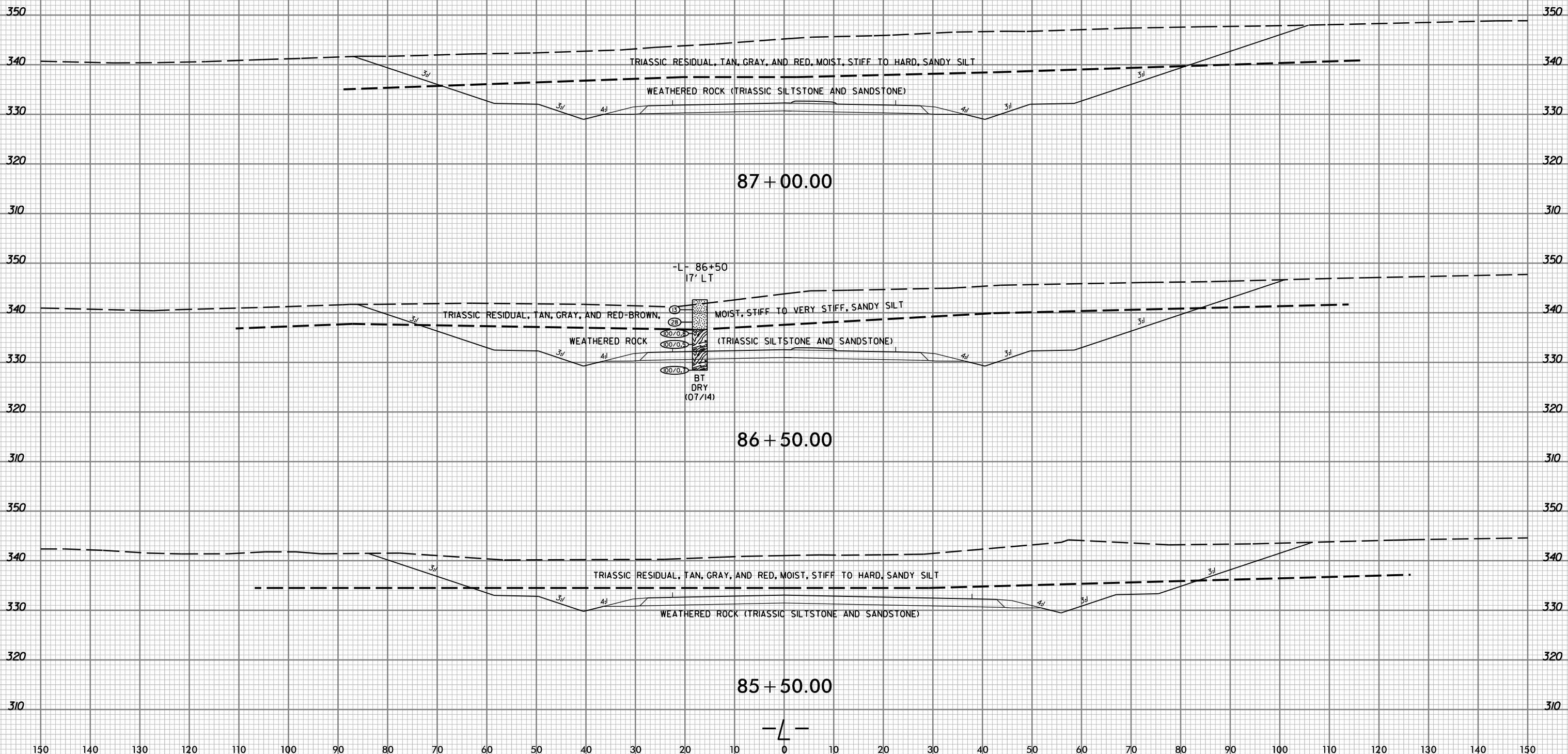
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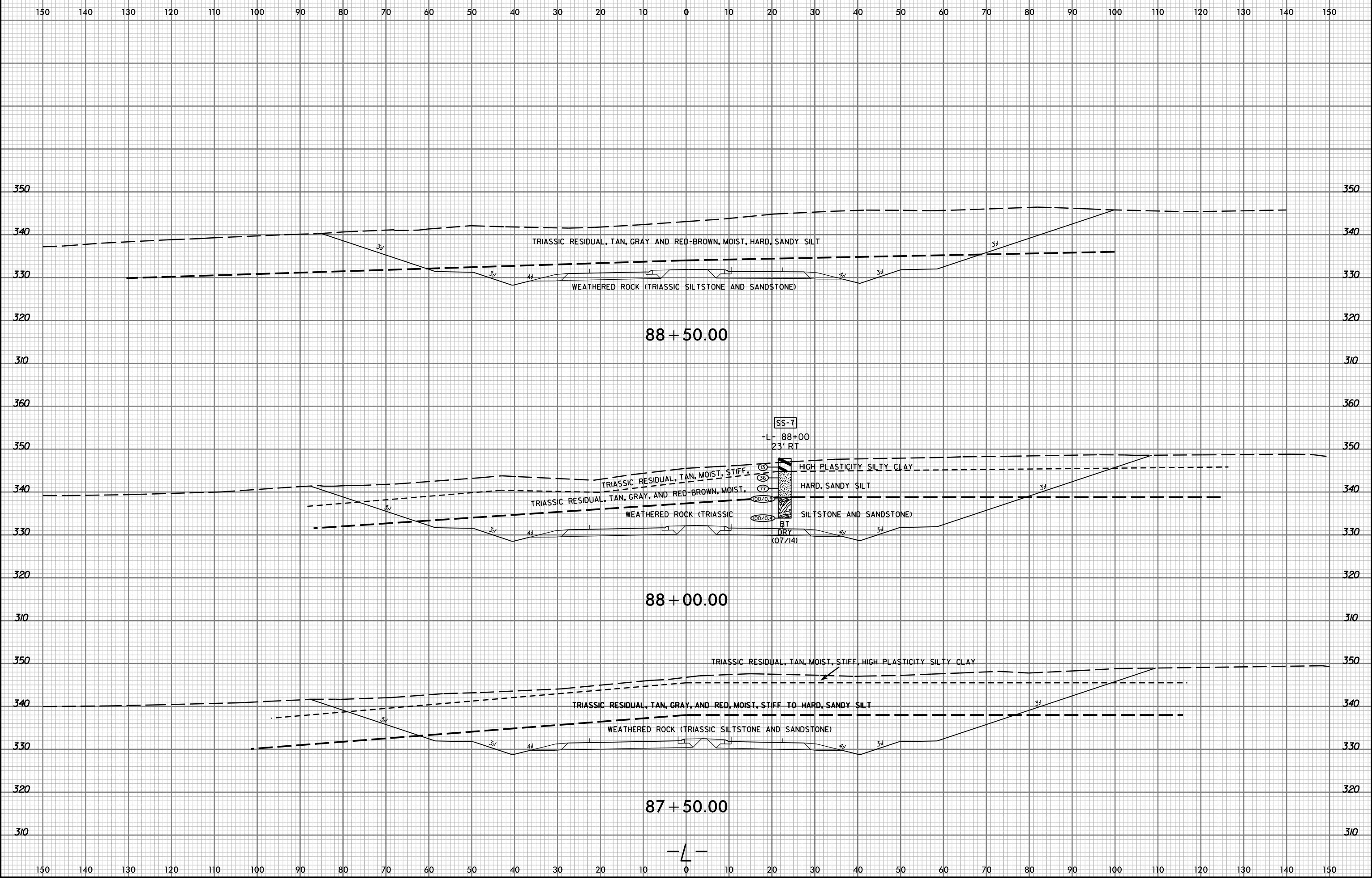


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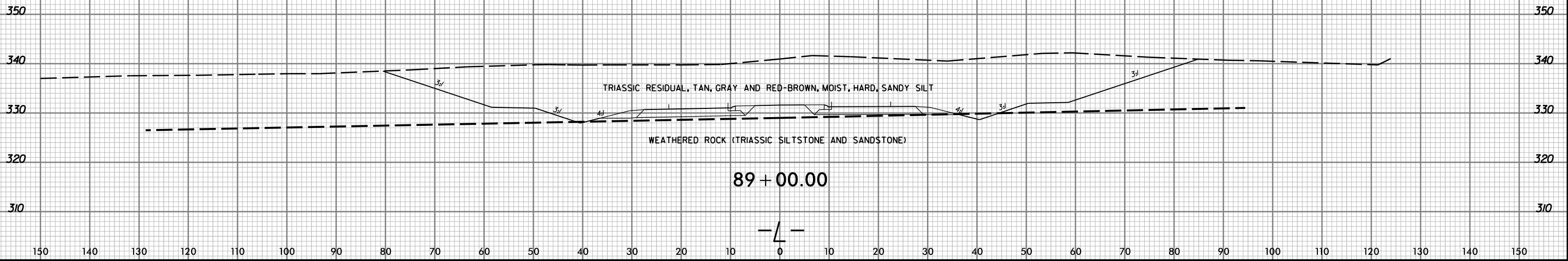
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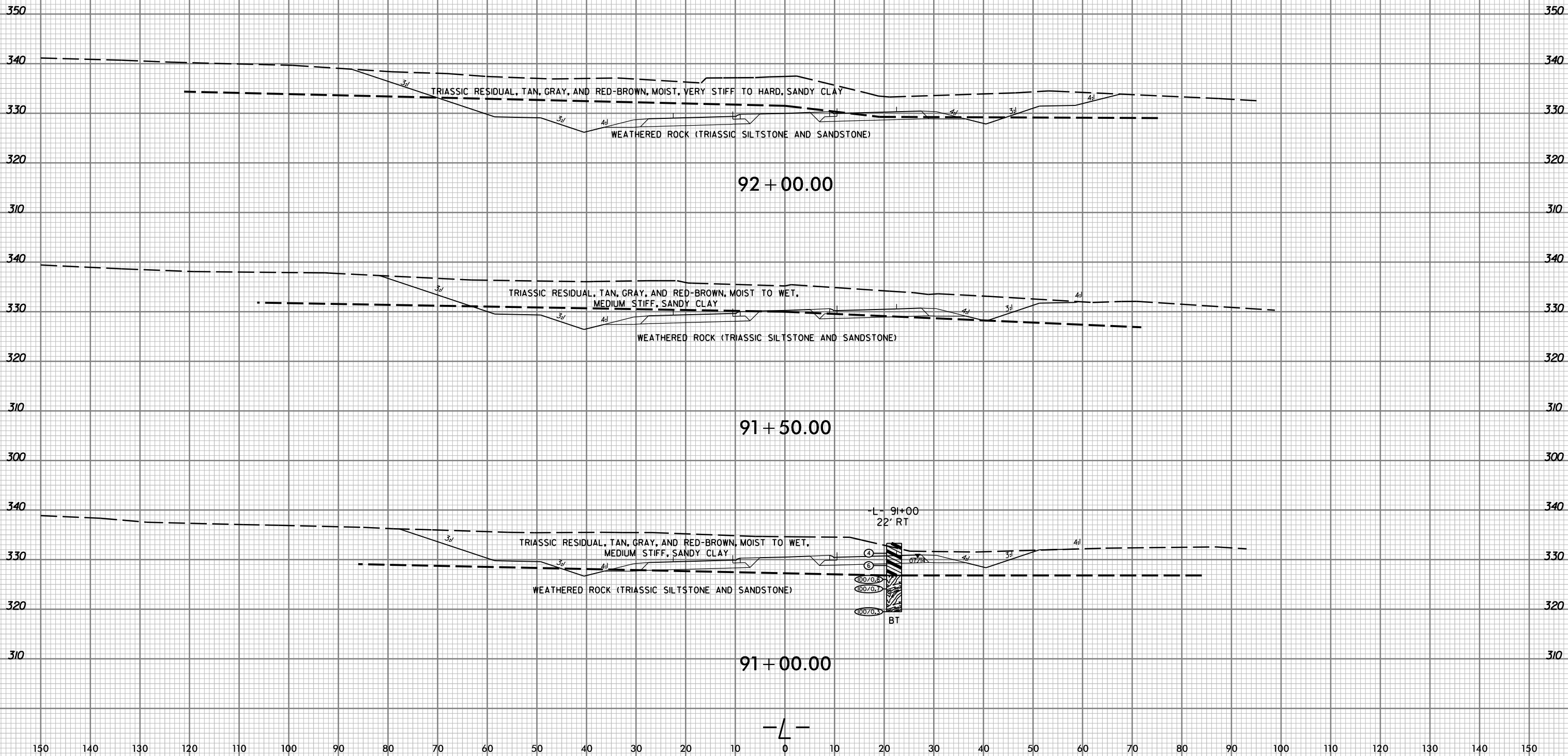
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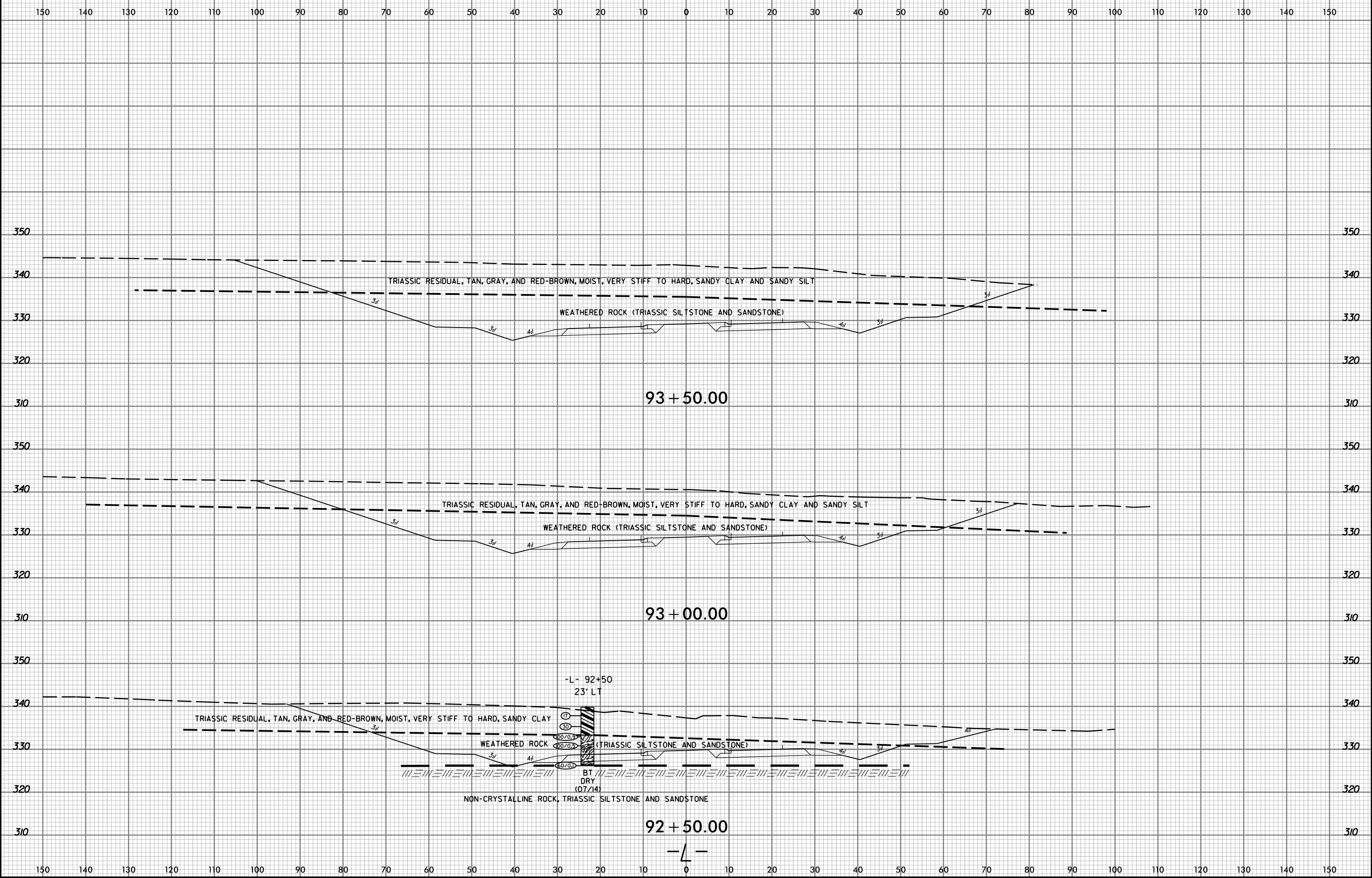


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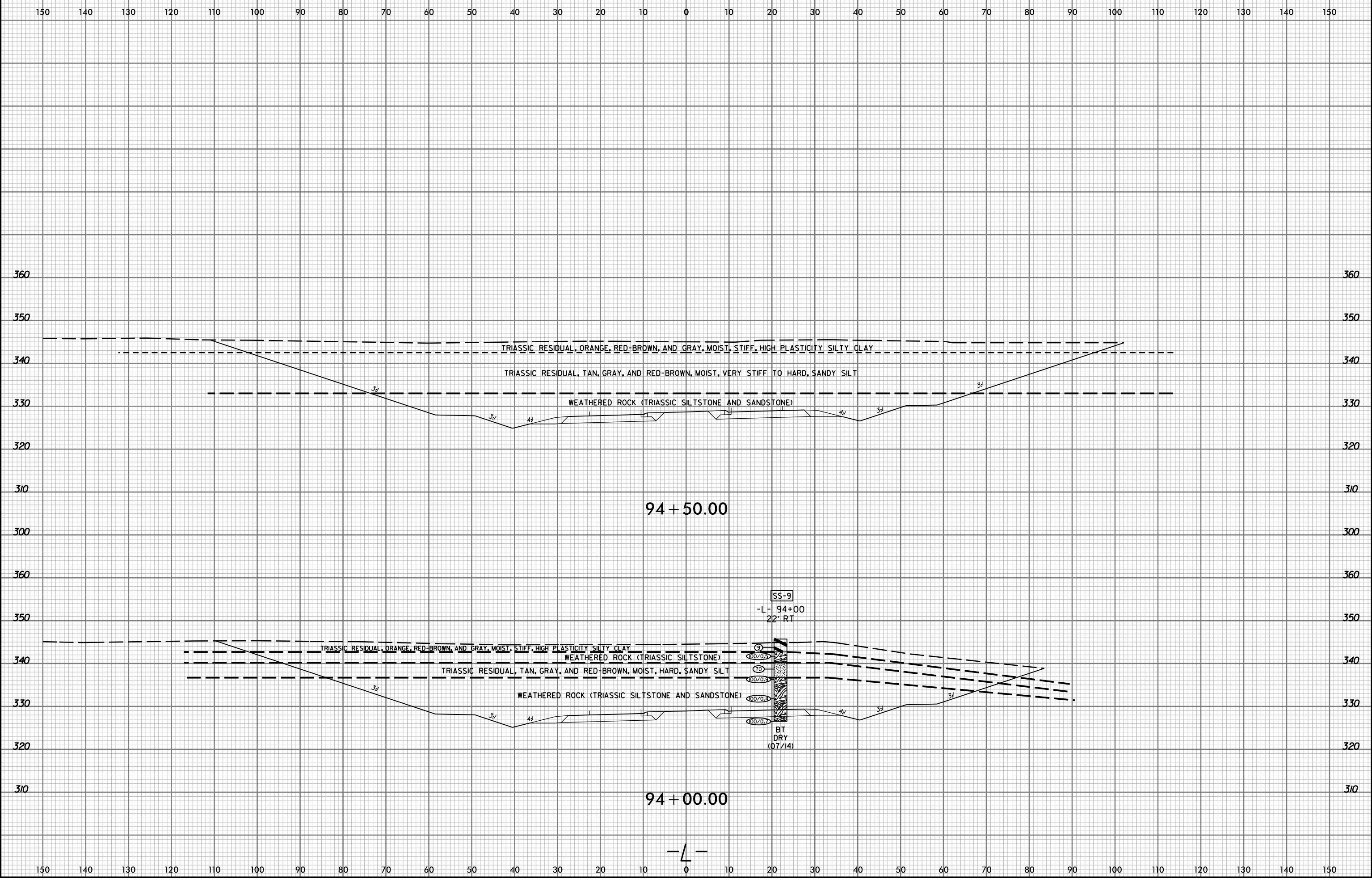
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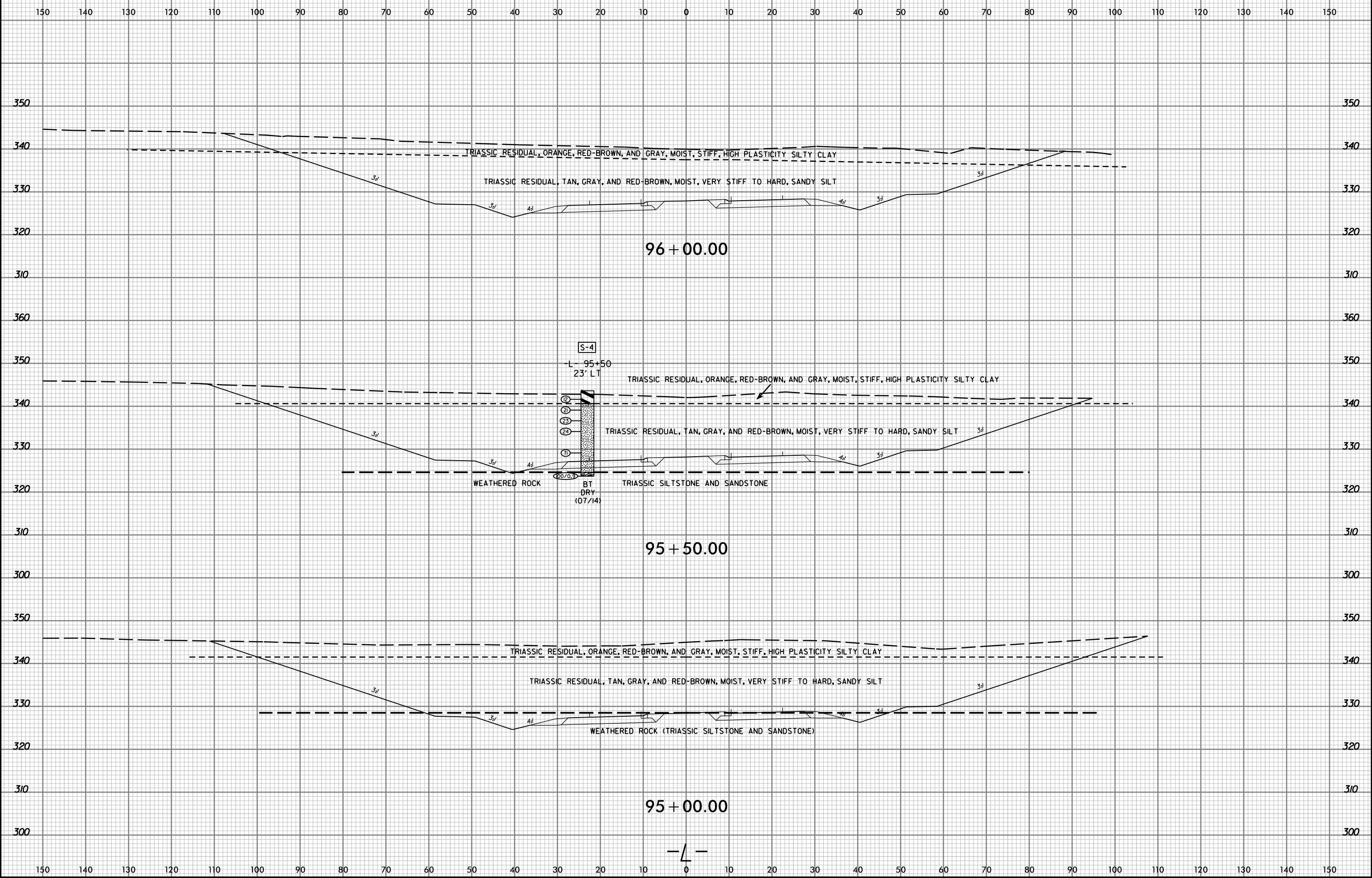


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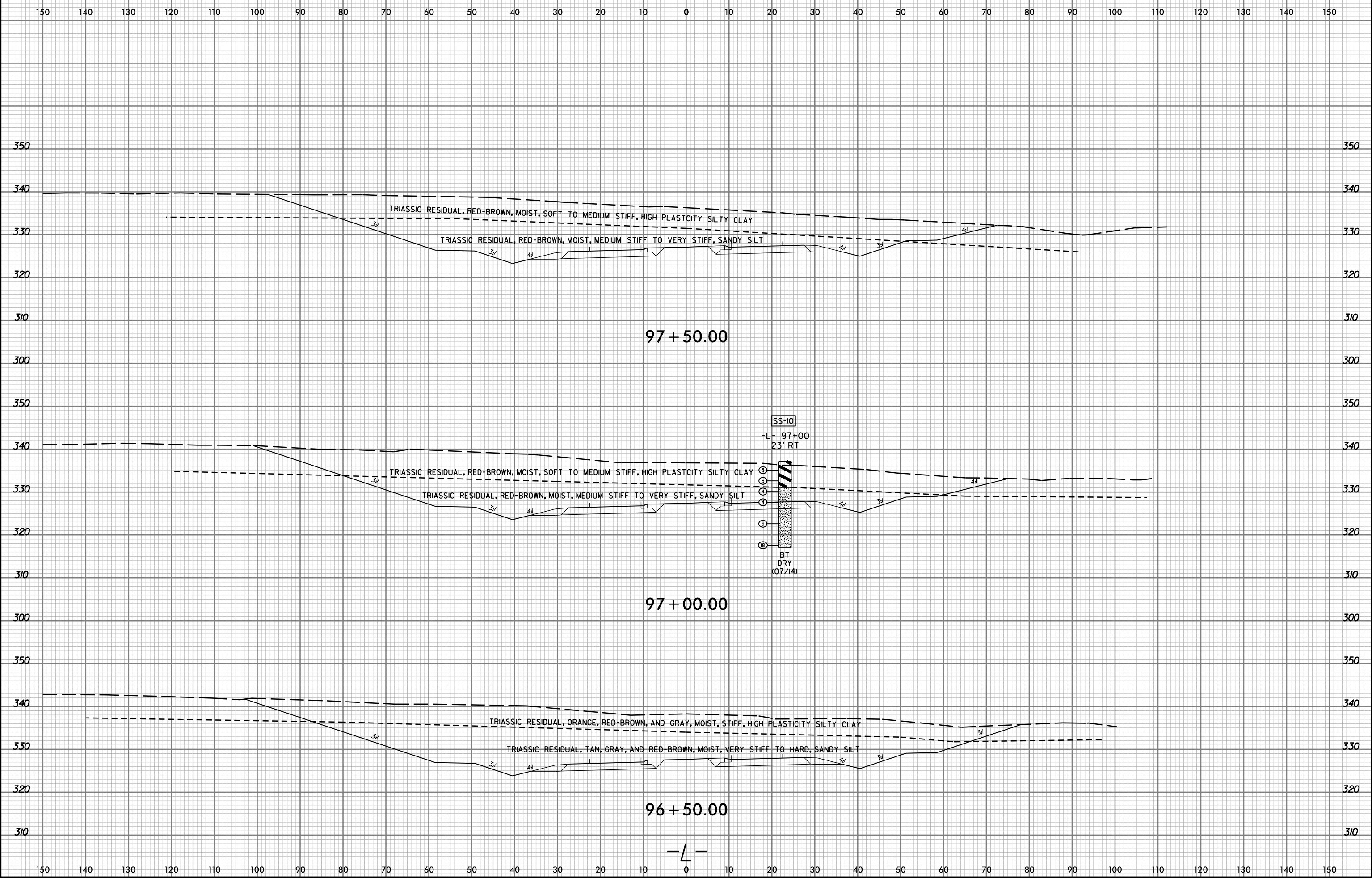
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8/23/99



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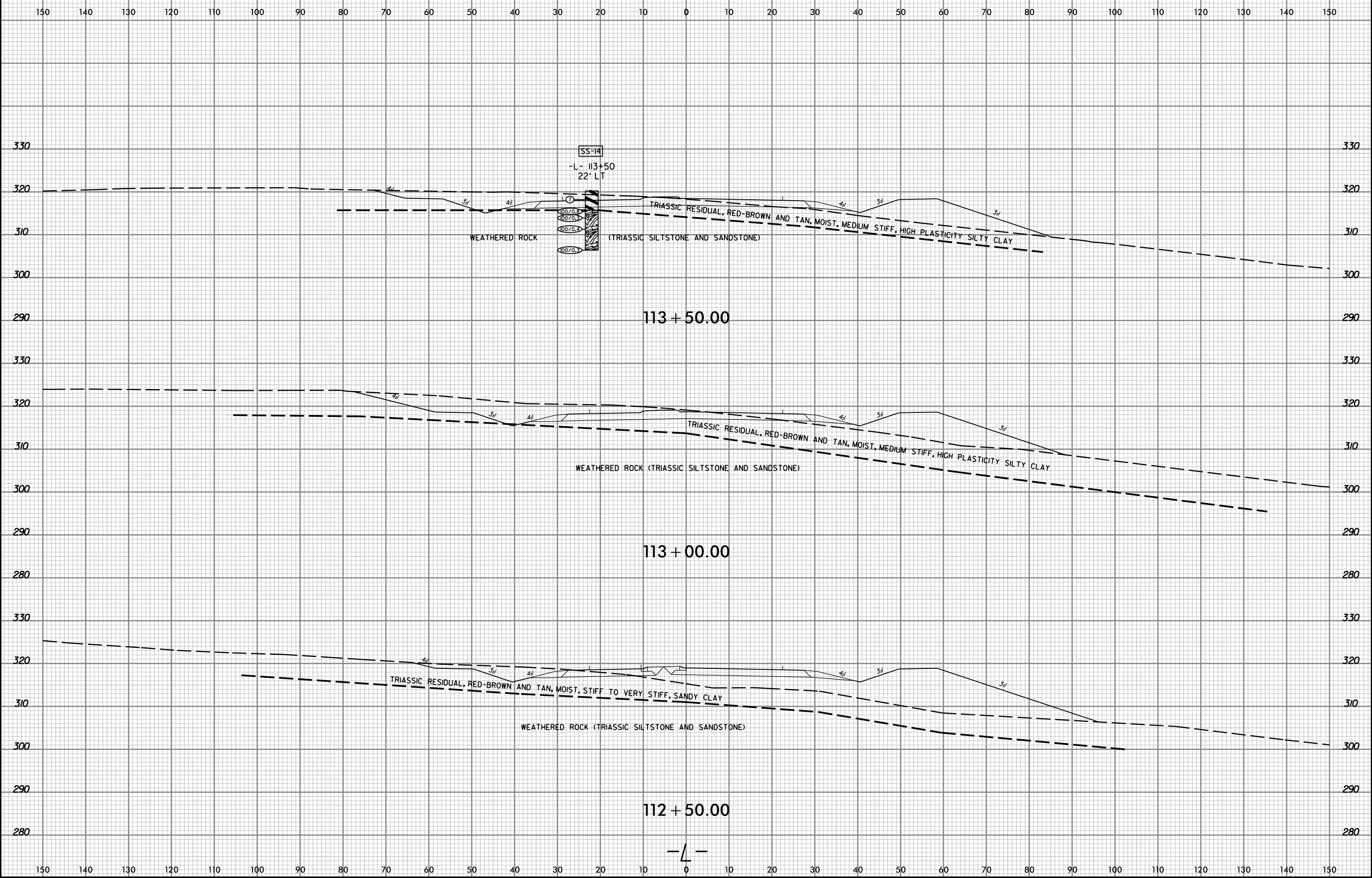


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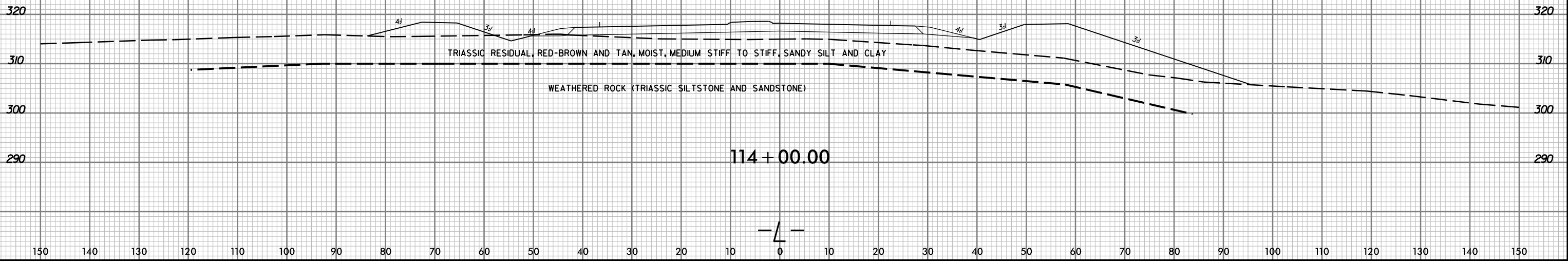


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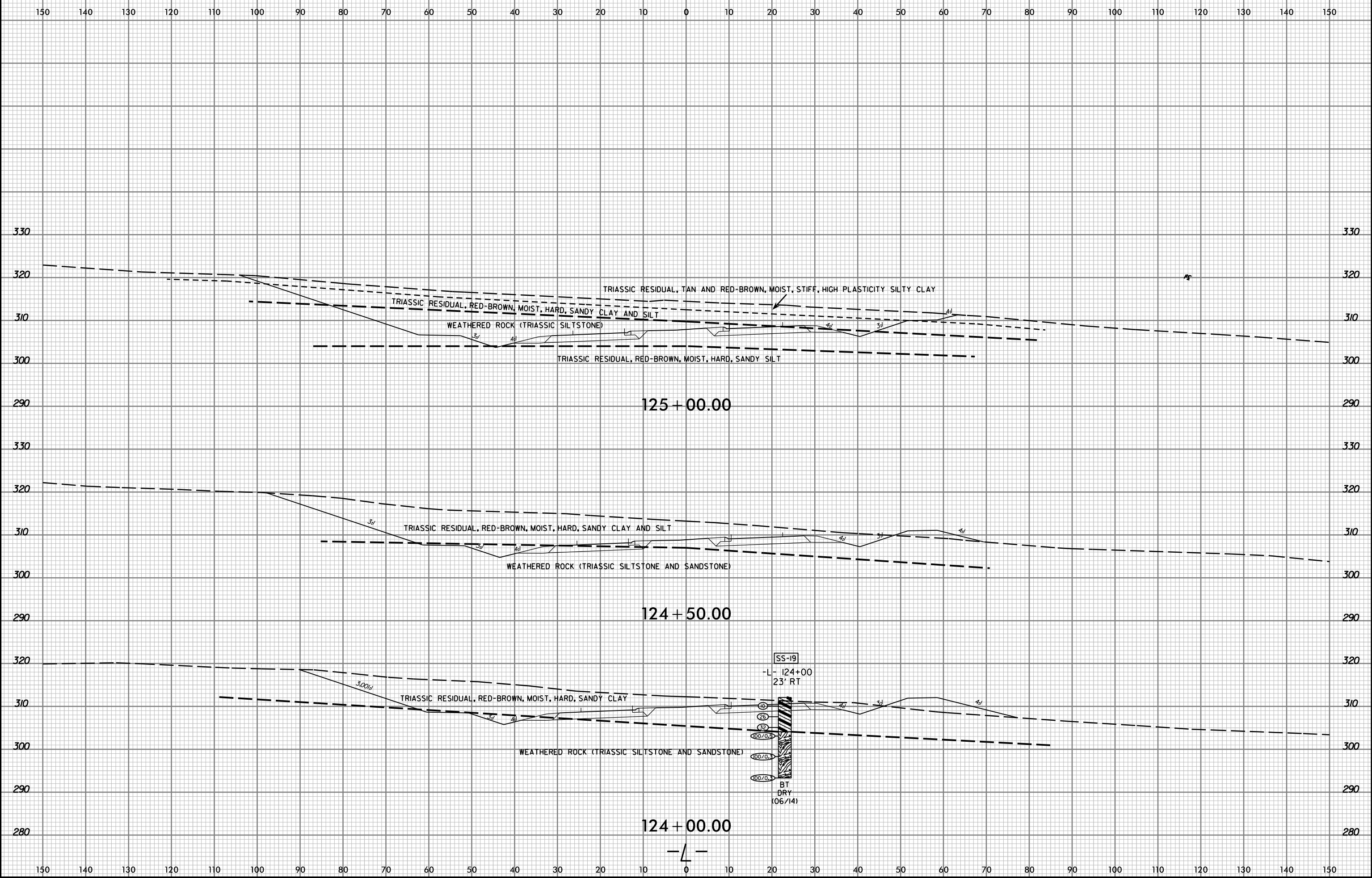


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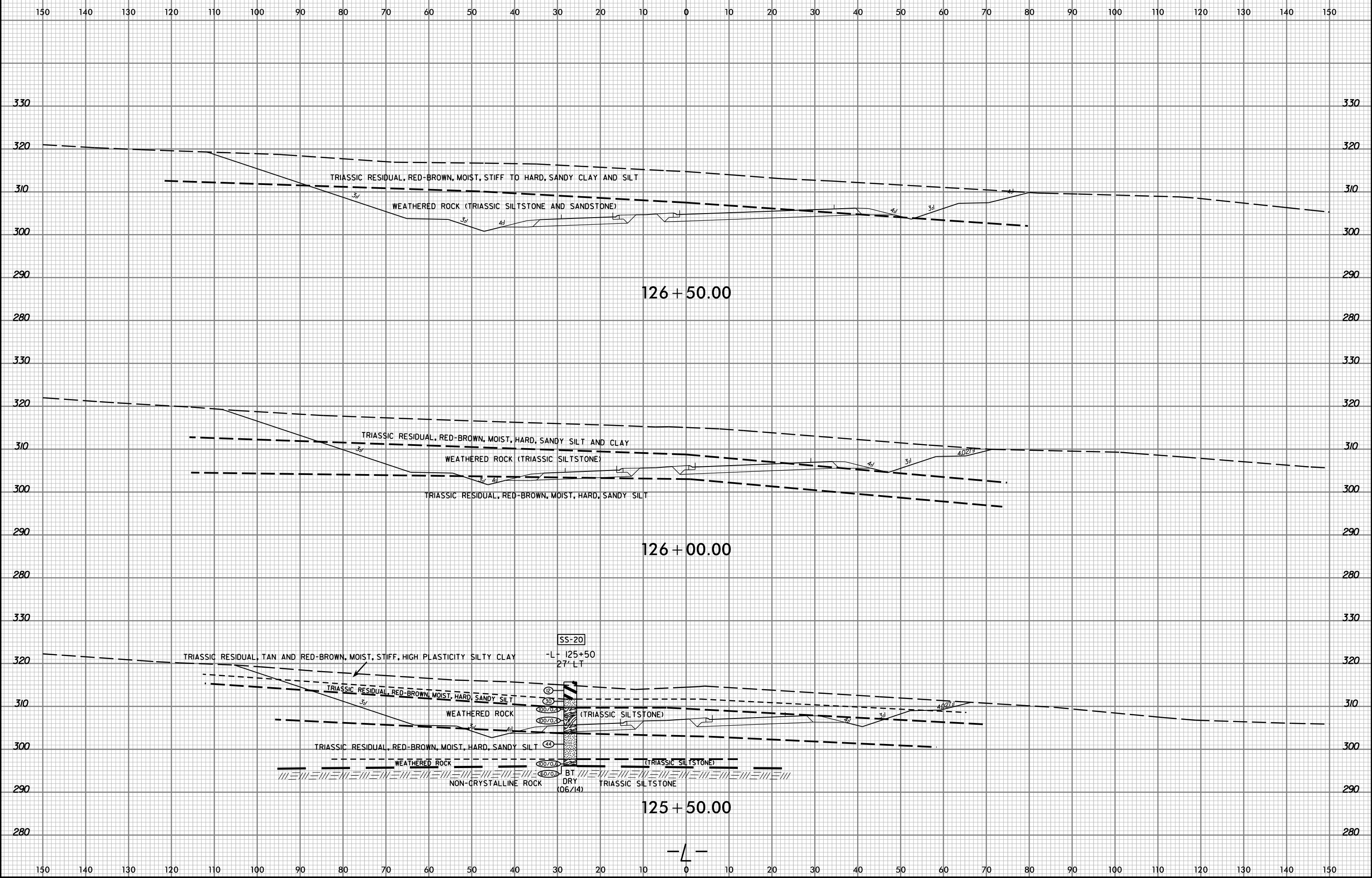


8/23/99



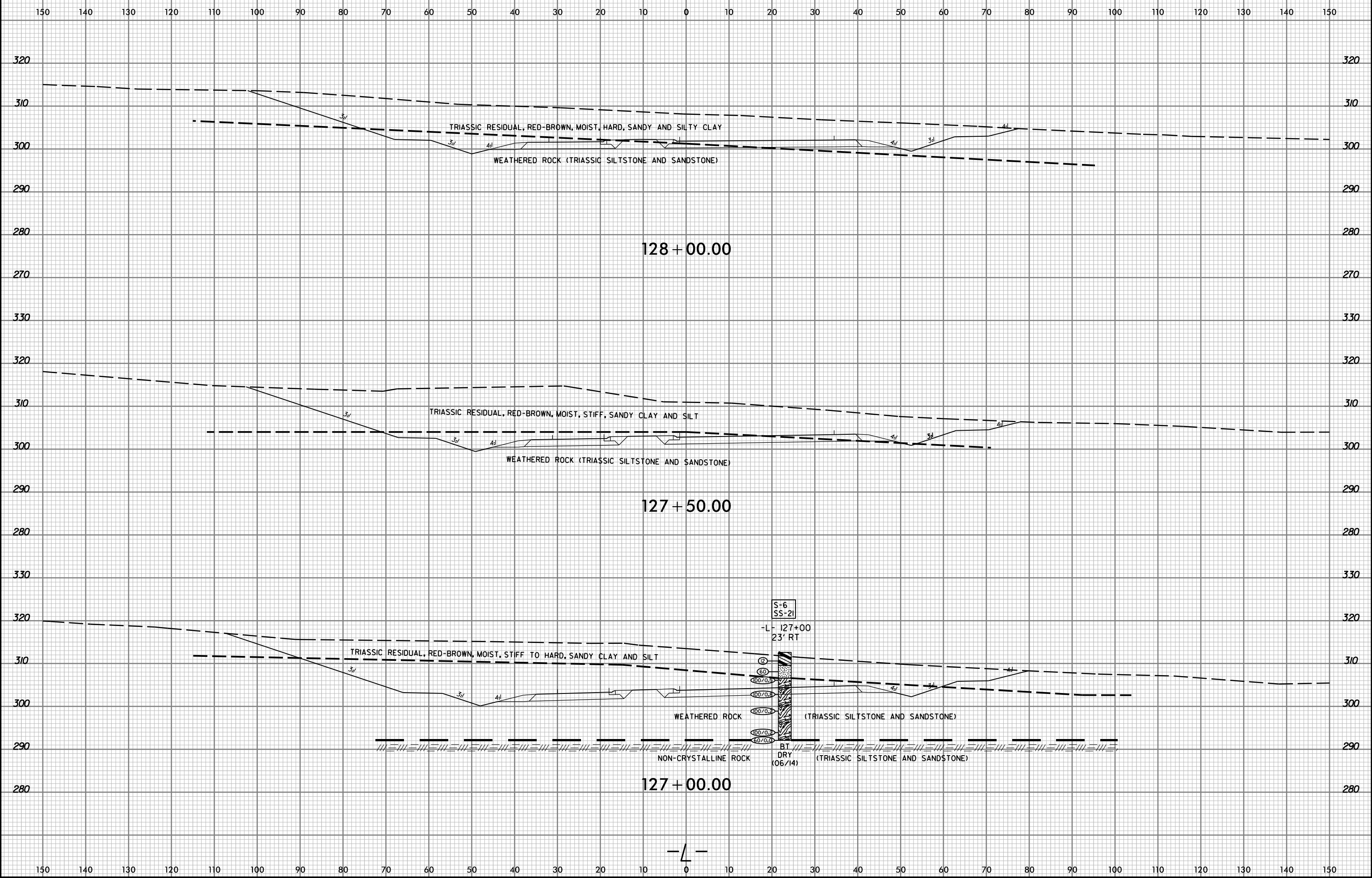
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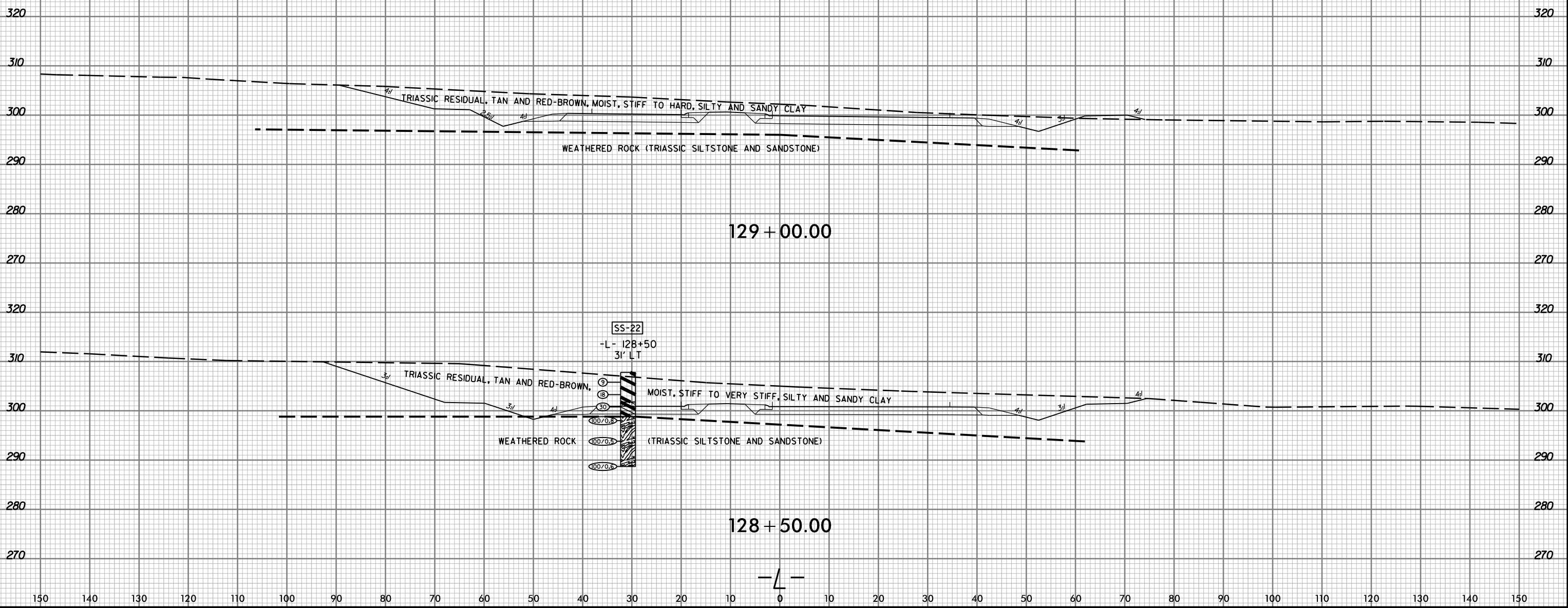
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8/23/99

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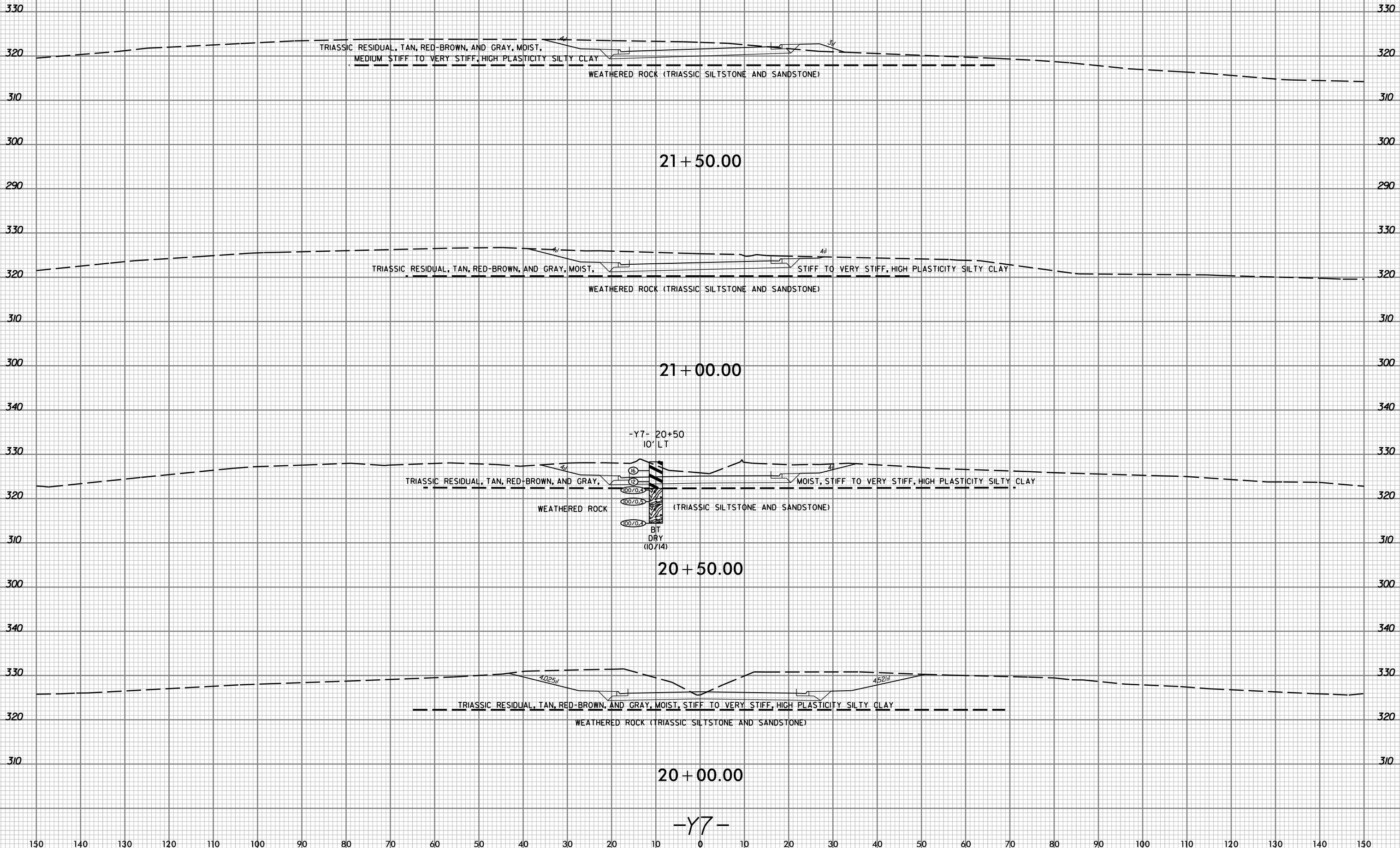
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PROJ. REFERENCE NO.  
012108004

SHEET NO.  
52

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TRIASSIC RESIDUAL, TAN, RED-BROWN, AND GRAY, MOIST,  
MEDIUM STIFF TO VERY STIFF, HIGH PLASTICITY SILTY CLAY

WEATHERED ROCK (TRIASSIC SILTSTONE AND SANDSTONE)

21 + 50.00

TRIASSIC RESIDUAL, TAN, RED-BROWN, AND GRAY, MOIST,

STIFF TO VERY STIFF, HIGH PLASTICITY SILTY CLAY

WEATHERED ROCK (TRIASSIC SILTSTONE AND SANDSTONE)

21 + 00.00

TRIASSIC RESIDUAL, TAN, RED-BROWN, AND GRAY,

MOIST, STIFF TO VERY STIFF, HIGH PLASTICITY SILTY CLAY

WEATHERED ROCK

(TRIASSIC SILTSTONE AND SANDSTONE)

-Y7- 20+50  
10' LT

(16)

(12)

(100/0.4)

(100/0.4)

(100/0.4)

BT  
DRY  
(10/14)

20 + 50.00

TRIASSIC RESIDUAL, TAN, RED-BROWN, AND GRAY, MOIST, STIFF TO VERY STIFF, HIGH PLASTICITY SILTY CLAY

WEATHERED ROCK (TRIASSIC SILTSTONE AND SANDSTONE)

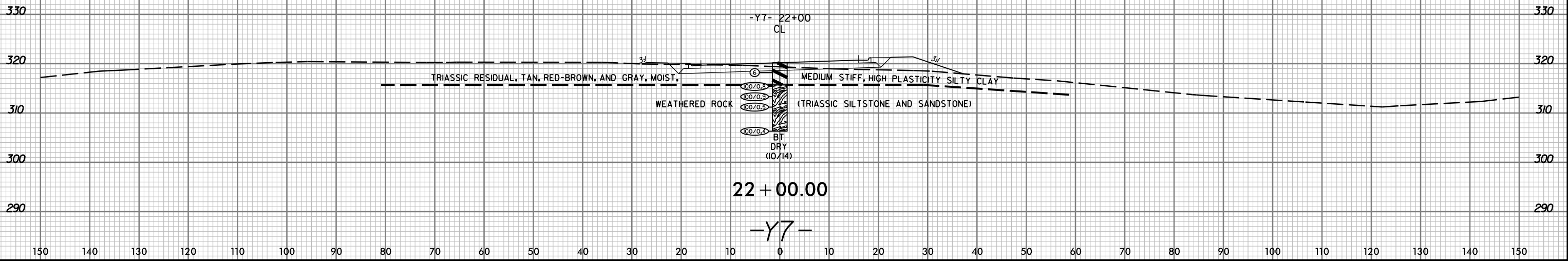
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-Y7-

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# MOISTURE-DENSITY RELATIONSHIP

ASTM D698/D1557

# REPORT FOR CALIFORNIA BEARING RATIO



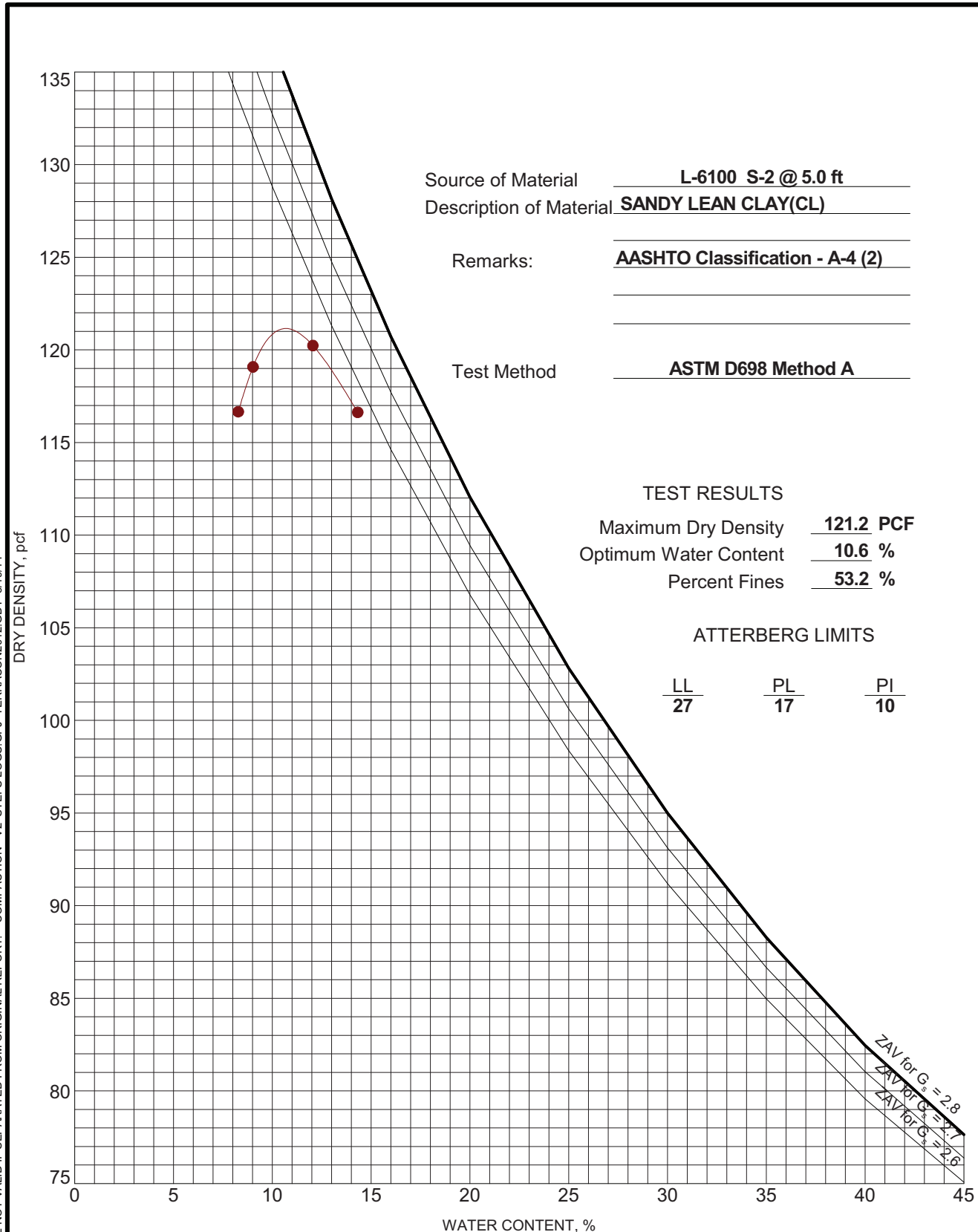
2401 Brentwood Road, Suite 107  
Raleigh, NC 27604  
919-873-2211

Report Number:  
Service Date: 08/04/14  
Report Date: 08/19/14

**Client**  
Kimley-Horn and Associates, Inc.  
Attn: Matthew West  
3001 Weston Parkway  
Cary, North Carolina 27513

**Project**  
McCrimmon Parkway Extension  
McCrimmon Parkway and Aviation Parkway  
Morrisville, North Carolina

Project No. 70135172



## SAMPLE INFORMATION

Sample Number:	<u>S-2</u>	Proctor Method:	<u>AASHTO T99 - Method A</u>
Boring Number:	<u>L-6100</u>	Maximum Dry Density (pcf):	<u>121.2</u>
Sample Location:	<u>N/A</u>	Optimum Moisture:	<u>10.6</u>
Depth:	<u>0-10'</u>	Liquid Limit:	<u>27</u>
Material Description:	<u>Sandy Lean Clay - AASHTO Classification A-4 (2)</u>	Plasticity Index:	<u>10</u>

## CBR TEST DATA

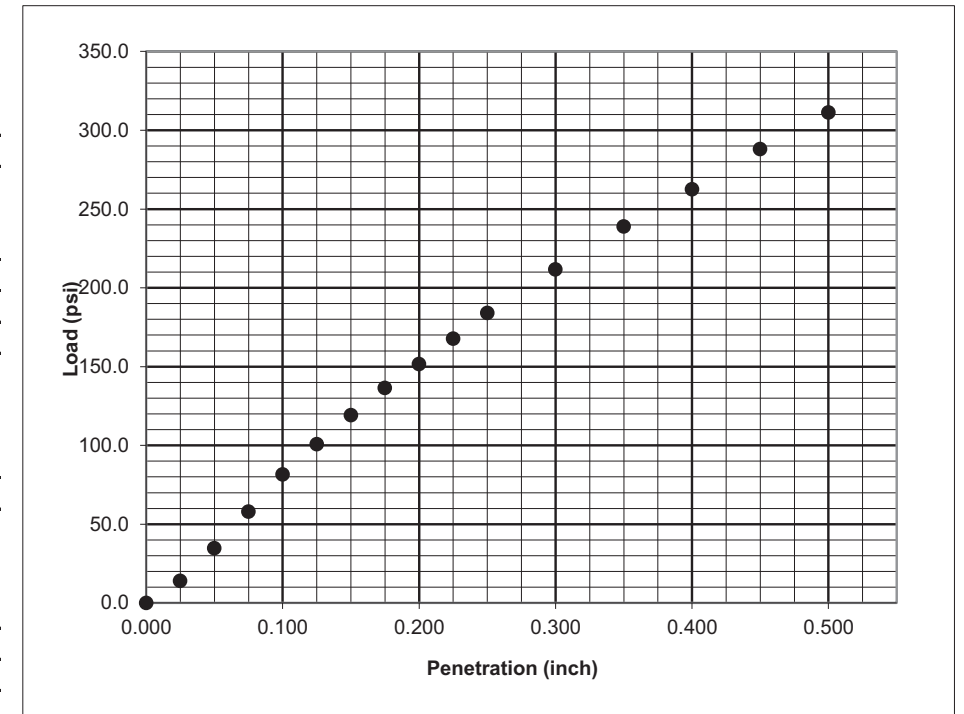
CBR Value at 0.100 inch	<u>8.2</u>
CBR Value at 0.200 inch	<u>10.1</u>
Surcharge Weight (lbs)	<u>10</u>
Soaking Condition	<u>Soaked</u>
Length of Soaking (hours)	<u>96</u>
Swell (%)	<u>1.1</u>

## DENSITY DATA

Dry Density Before Soaking (pcf)	<u>120.7</u>
Compaction of Proctor (%)	<u>99.6</u>

## MOISTURE DATA

Before Compaction (%)	<u>9.5</u>
After Compaction (%)	<u>9.6</u>
Top 1" After Soaking (%)	<u>14.0</u>
Average After Soaking (%)	<u>13.7</u>



## Comments:

Services: Obtain soil sample and test for California Bearing Ratio

Terracon Rep: Stephanie Huffman  
Reported To: Matthew J. Alexander, PE  
Contractor:

## Report Distribution

Reviewed by: Matthew J. Alexander, PE  
Geotechnical Project Engineer

## Test Methods:

ASTM D1883  
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LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. COMPACTION - V2 STEFFS LOGS.GPJ TERRACON2012.GDT 8/19/14

PROJECT: McCrimmon Parkway Extension

SITE: McCrimmon Parkway and Aviation Parkway  
Morrisville, North Carolina



PROJECT NUMBER: 70135172

CLIENT: Kimley-Horn and Associates, Inc.  
Cary, North Carolina

# MOISTURE-DENSITY RELATIONSHIP

ASTM D698/D1557

# REPORT FOR CALIFORNIA BEARING RATIO



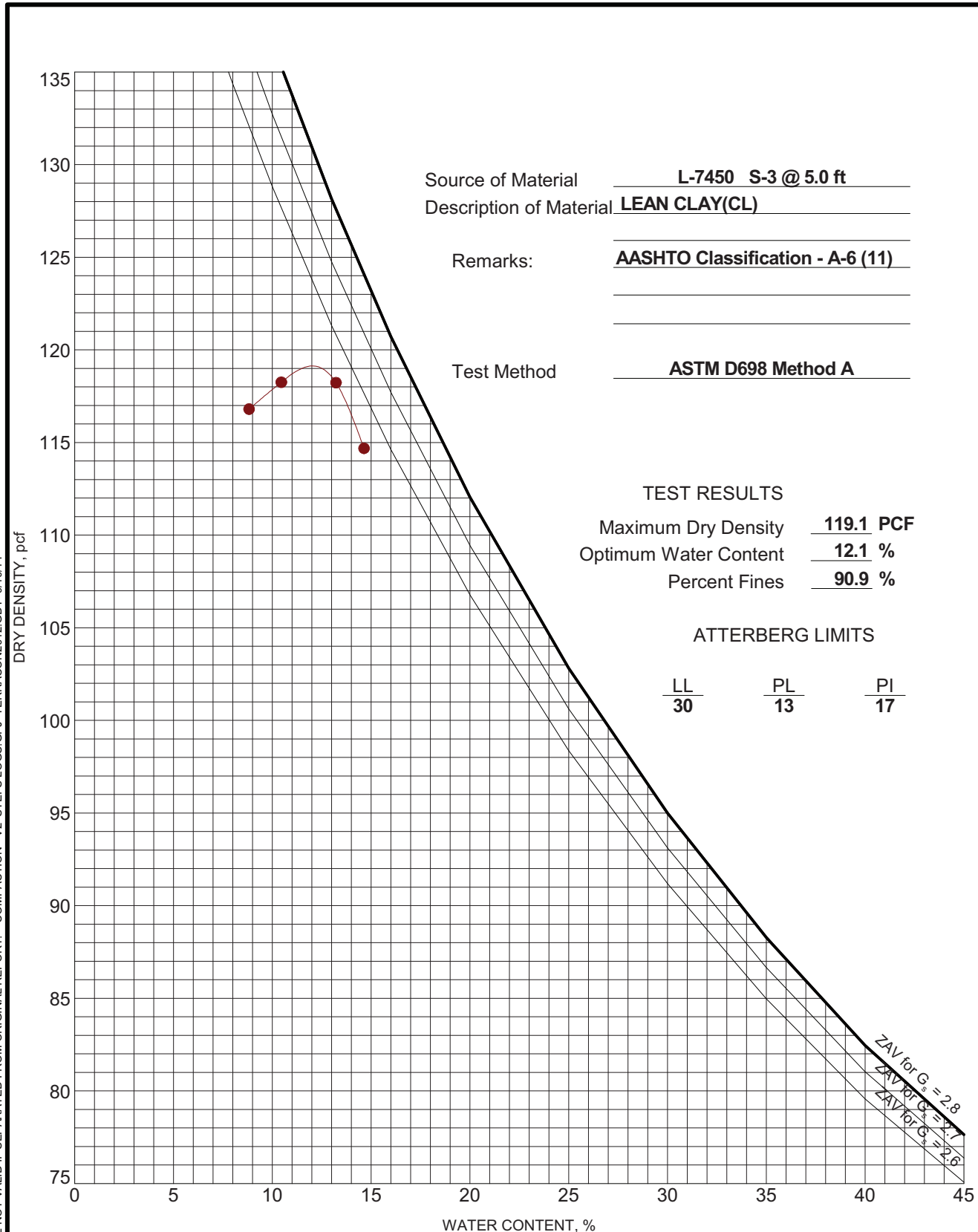
2401 Brentwood Road, Suite 107  
Raleigh, NC 27604  
919-873-2211

Report Number:  
Service Date: 09/04/14  
Report Date: 09/10/14

**Client**  
Kimley-Horn and Associates, Inc.  
Attn: Matthew West  
3001 Weston Parkway  
Cary, North Carolina 27513

**Project**  
McCrimmon Parkway Extension  
McCrimmon Parkway and Aviation Parkway  
Morrisville, North Carolina

Project No. 70135172



## SAMPLE INFORMATION

Sample Number:	<u>S-3</u>	Proctor Method:	<u>AASHTO T99 - Method A</u>
Boring Number:	<u>L-7450</u>	Maximum Dry Density (pcf):	<u>119.1</u>
Sample Location:	<u>N/A</u>	Optimum Moisture:	<u>12.1</u>
Depth:	<u>0-10'</u>	Liquid Limit:	<u>30</u>
Material Description:	<u>Lean Clay - AASHTO Classification A-6 (11)</u>	Plasticity Index:	<u>13</u>

## CBR TEST DATA

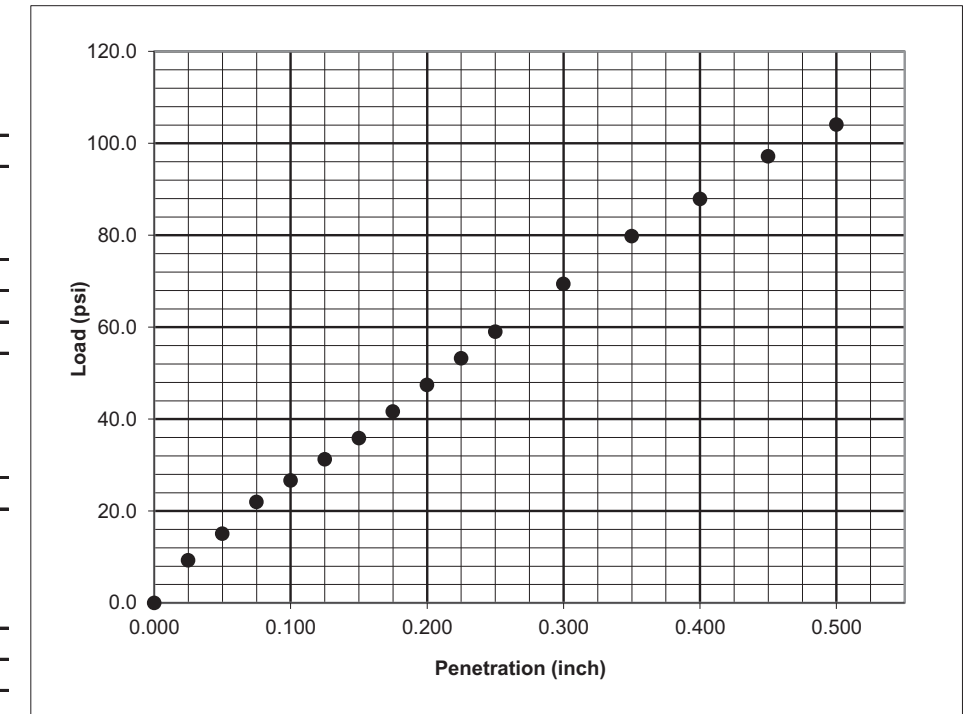
CBR Value at 0.100 inch	<u>2.0</u>
CBR Value at 0.200 inch	<u>3.2</u>
Surcharge Weight (lbs)	<u>10</u>
Soaking Condition	<u>Soaked</u>
Length of Soaking (hours)	<u>96</u>
Swell (%)	<u>1.4</u>

## DENSITY DATA

Dry Density Before Soaking (pcf)	<u>119.4</u>
Compaction of Proctor (%)	<u>100.2</u>

## MOISTURE DATA

Before Compaction (%)	<u>12.6</u>
After Compaction (%)	<u>12.9</u>
Top 1" After Soaking (%)	<u>16.6</u>
Average After Soaking (%)	<u>15.0</u>



## Comments:

Services: Obtain soil sample and test for California Bearing Ratio

Terracon Rep: Stephanie Huffman  
Reported To: Matthew J. Alexander, PE  
Contractor:

## Report Distribution

Reviewed by: Matthew J. Alexander, PE  
Geotechnical Project Engineer

## Test Methods:

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PROJECT: McCrimmon Parkway Extension

SITE: McCrimmon Parkway and Aviation Parkway  
Morrisville, North Carolina



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Cary, North Carolina

# MOISTURE-DENSITY RELATIONSHIP

ASTM D698/D1557

# REPORT FOR CALIFORNIA BEARING RATIO



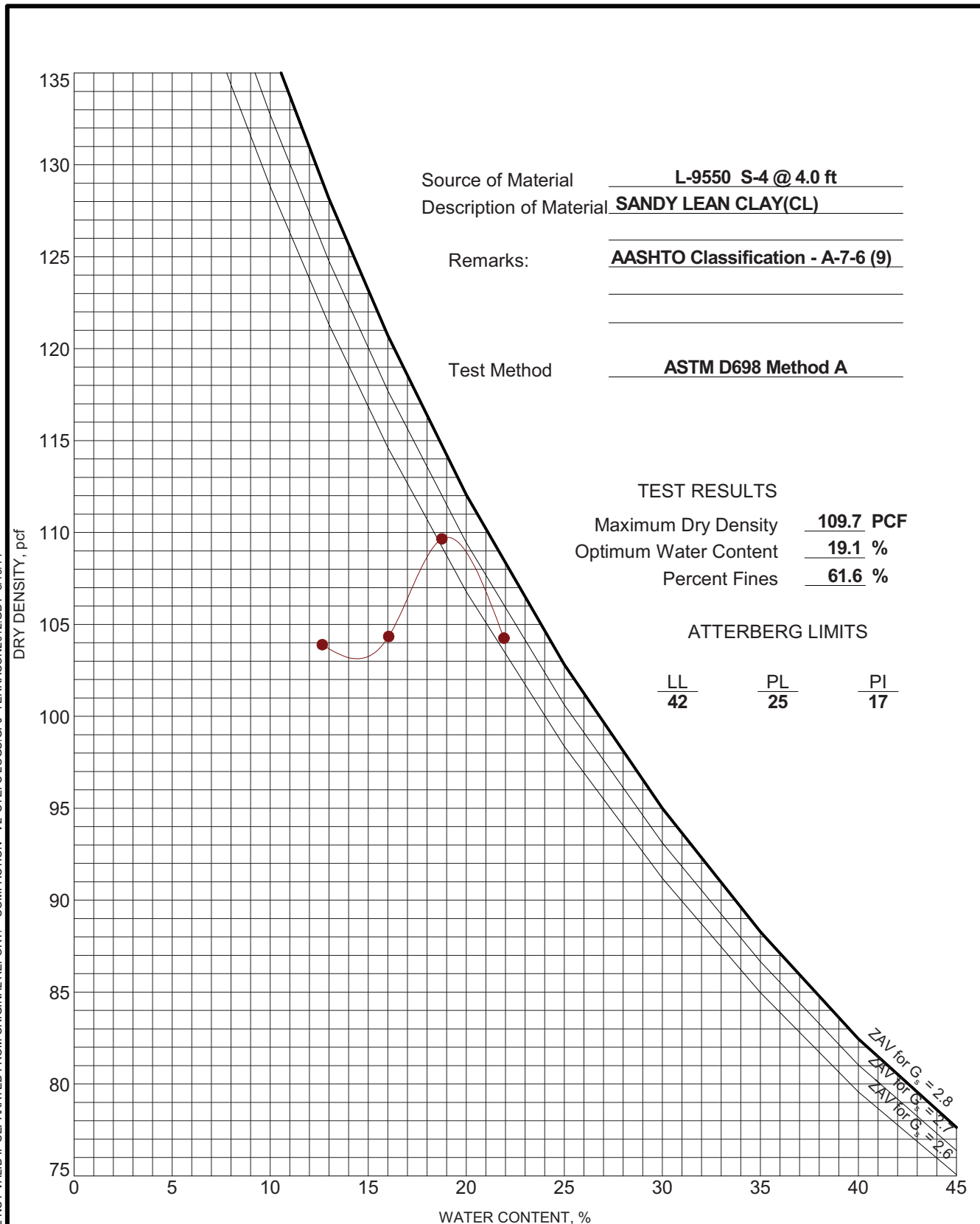
2401 Brentwood Road, Suite 107  
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919-873-2211

Report Number:  
Service Date: 08/04/14  
Report Date: 08/19/14

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3001 Weston Parkway  
Cary, North Carolina 27513

**Project**  
McCrimmon Parkway Extension  
McCrimmon Parkway and Aviation Parkway  
Morrisville, North Carolina

Project No. 70135172

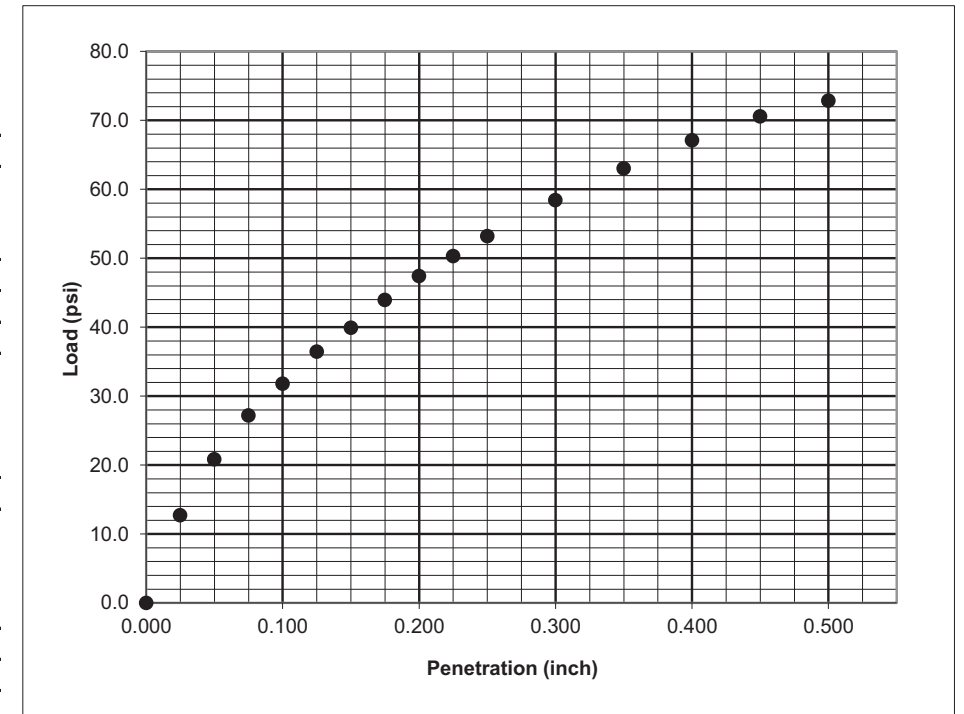


## SAMPLE INFORMATION

Sample Number:	S-4	Proctor Method:	AASHTO T99 - Method A
Boring Number:	L-9550	Maximum Dry Density (pcf):	109.7
Sample Location:	N/A	Optimum Moisture:	19.1
Depth:	0-8.5'	Liquid Limit:	42
Material Description:	Sandy Lean Clay - AASHTO Classification A-7-6 (9)	Plasticity Index:	17

## CBR TEST DATA

CBR Value at 0.100 inch	3.2
CBR Value at 0.200 inch	3.2
Surcharge Weight (lbs)	10
Soaking Condition	Soaked
Length of Soaking (hours)	96
Swell (%)	1.1



**DENSITY DATA**

Dry Density Before Soaking (pcf)	108.2
Compaction of Proctor (%)	98.6

**MOISTURE DATA**

Before Compaction (%)	20.0
After Compaction (%)	20.1
Top 1" After Soaking (%)	25.1
Average After Soaking (%)	24.0

**Comments:**  
**Services:** Obtain soil sample and test for California Bearing Ratio

**Terracon Rep:** Stephanie Huffman  
**Reported To:** Matthew J. Alexander, PE  
**Contractor:**

**Report Distribution**

**Reviewed by:** Matthew J. Alexander, PE  
Geotechnical Project Engineer

**Test Methods:** ASTM D1883  
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PROJECT: McCrimmon Parkway Extension

SITE: McCrimmon Parkway and Aviation Parkway  
Morrisville, North Carolina



PROJECT NUMBER: 70135172

CLIENT: Kimley-Horn and Associates, Inc.  
Cary, North Carolina

# MOISTURE-DENSITY RELATIONSHIP

ASTM D698/D1557

# REPORT FOR CALIFORNIA BEARING RATIO



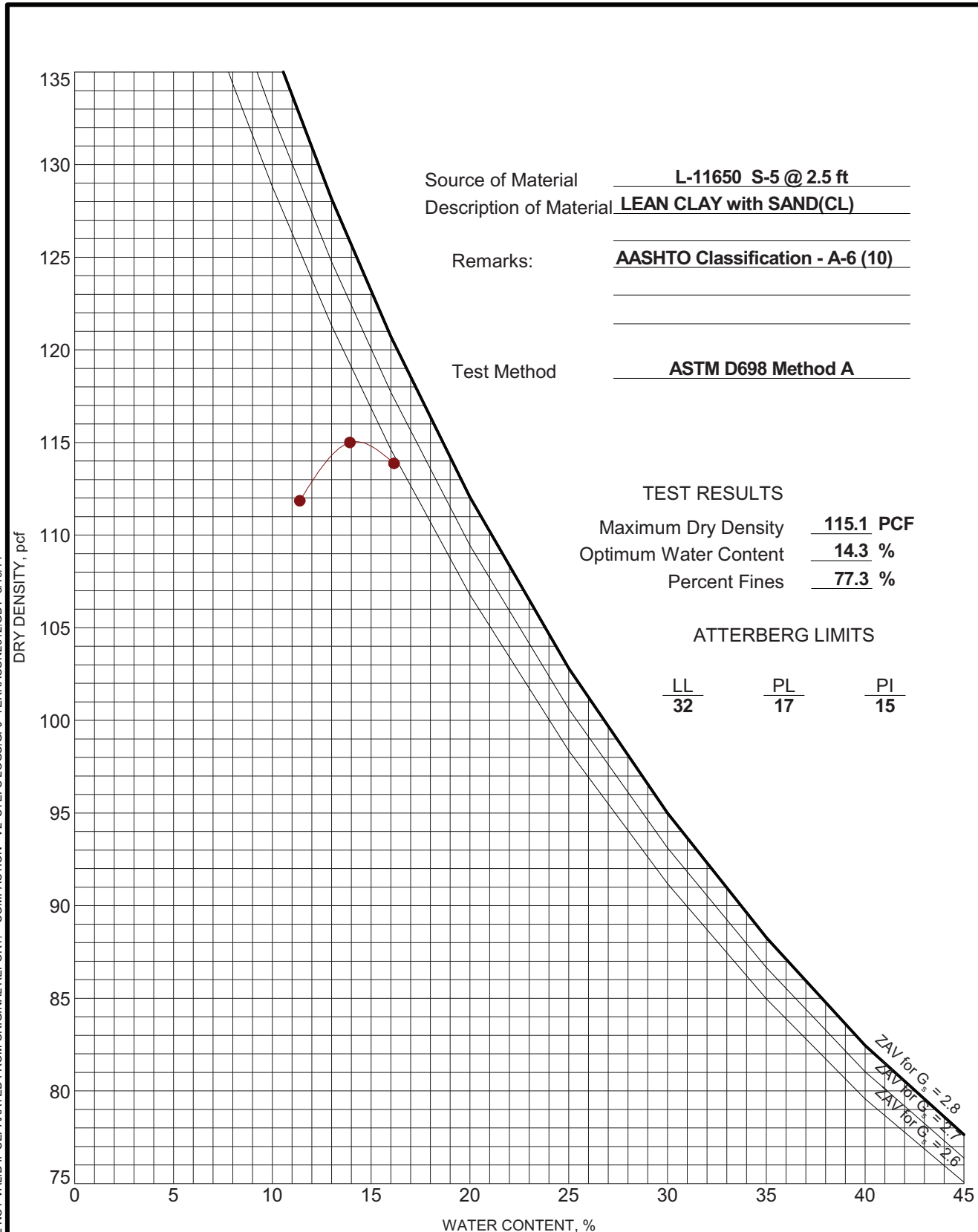
2401 Brentwood Road, Suite 107  
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Report Number:  
Service Date: 08/04/14  
Report Date: 08/19/14

**Client**  
Kimley-Horn and Associates, Inc.  
Attn: Matthew West  
3001 Weston Parkway  
Cary, North Carolina 27513

**Project**  
McCrimmon Parkway Extension  
McCrimmon Parkway and Aviation Parkway  
Morrisville, North Carolina

Project No. 70135172

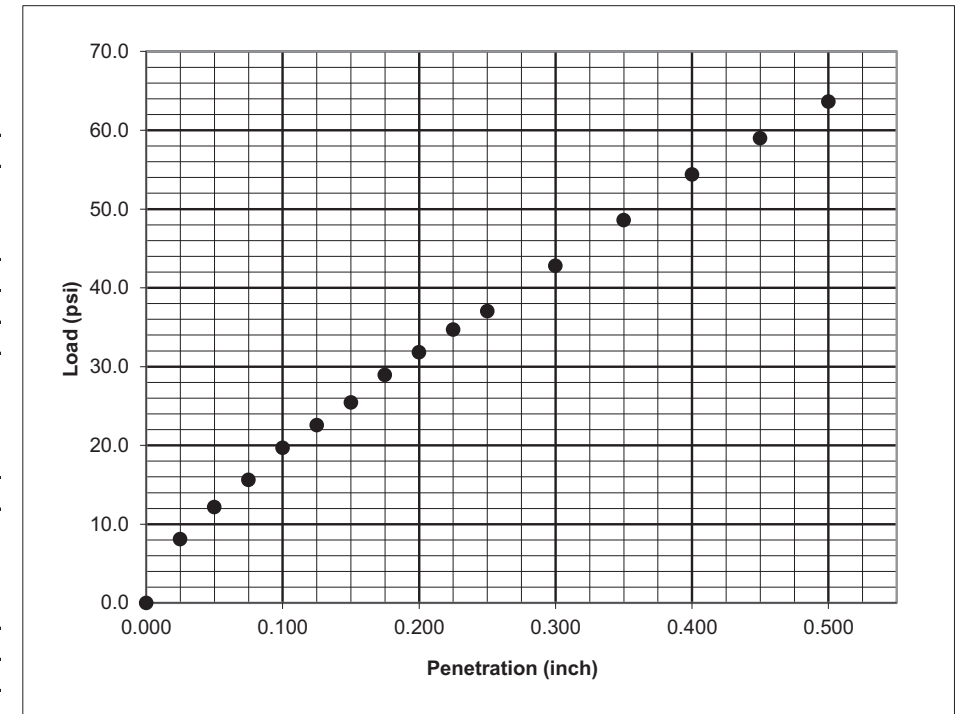


## SAMPLE INFORMATION

Sample Number:	S-5	Proctor Method:	AASHTO T99 - Method A
Boring Number:	L-11650	Maximum Dry Density (pcf):	115.1
Sample Location:	N/A	Optimum Moisture:	14.3
Depth:	1-5'	Liquid Limit:	32
Material Description:	Lean Clay with Sand - AASHTO Classification A-6 (10)	Plasticity Index:	15

## CBR TEST DATA

CBR Value at 0.100 inch	2.0
CBR Value at 0.200 inch	2.1
Surcharge Weight (lbs)	10
Soaking Condition	Soaked
Length of Soaking (hours)	96
Swell (%)	1.8



**DENSITY DATA**

Dry Density Before Soaking (pcf)	115.6
Compaction of Proctor (%)	100.4

**MOISTURE DATA**

Before Compaction (%)	14.4
After Compaction (%)	14.3
Top 1" After Soaking (%)	18.9
Average After Soaking (%)	17.7

**Comments:**  
**Services:** Obtain soil sample and test for California Bearing Ratio

**Terracon Rep:** Stephanie Huffman  
**Reported To:** Matthew J. Alexander, PE  
**Contractor:**

**Report Distribution**

**Reviewed by:** Matthew J. Alexander, PE  
Geotechnical Project Engineer

**Test Methods:** ASTM D1883  
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PROJECT: McCrimmon Parkway Extension	<p>2401 Brentwood Road, Suite 107 Raleigh, North Carolina</p>	PROJECT NUMBER: 70135172
SITE: McCrimmon Parkway and Aviation Parkway Morrisville, North Carolina		CLIENT: Kimley-Horn and Associates, Inc. Cary, North Carolina
		EXHIBIT: B-1



# MOISTURE-DENSITY RELATIONSHIP

ASTM D698/D1557

# REPORT FOR CALIFORNIA BEARING RATIO



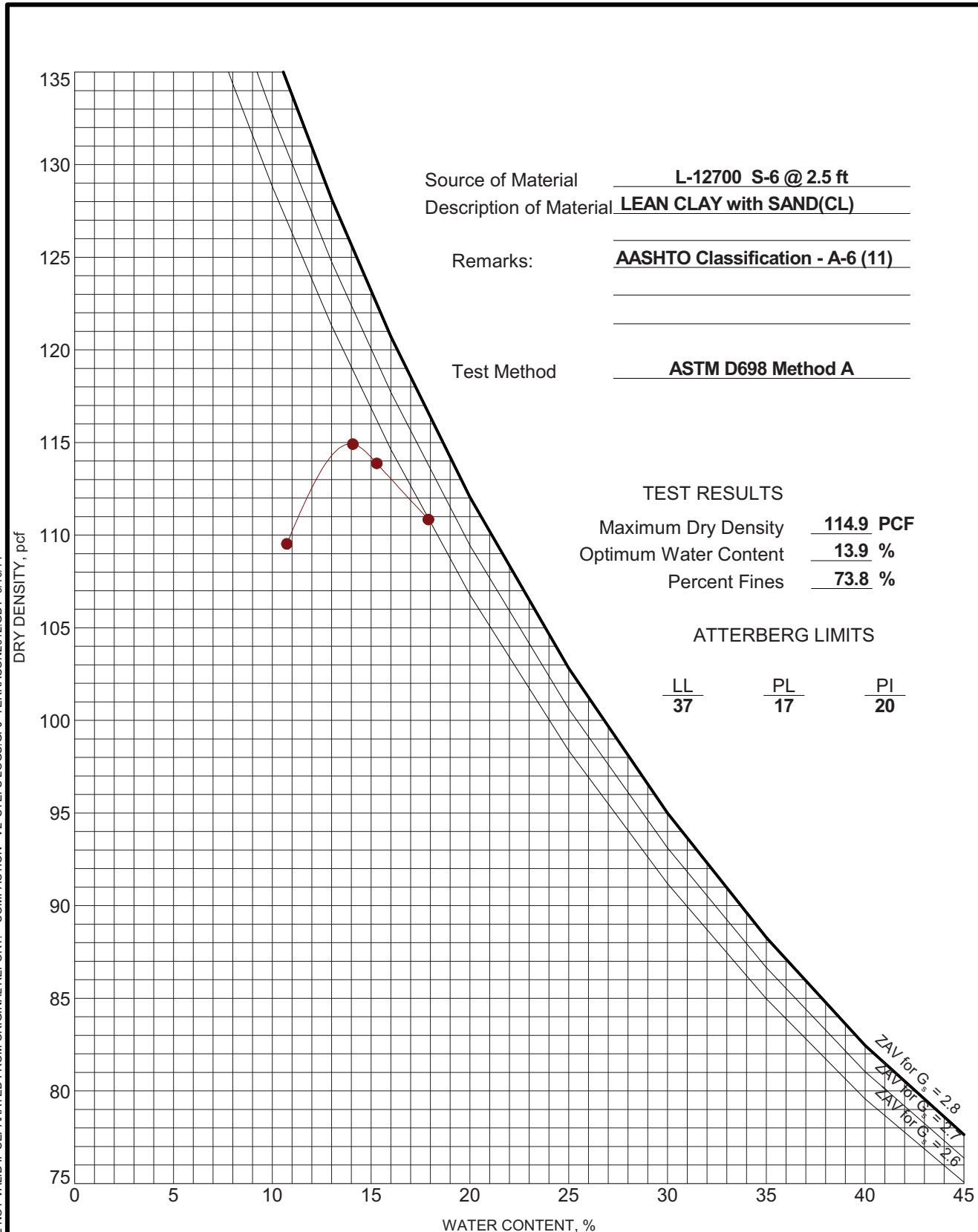
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Report Number:  
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**Client**  
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Attn: Matthew West  
3001 Weston Parkway  
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**Project**  
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Morrisville, North Carolina

Project No. 70135172

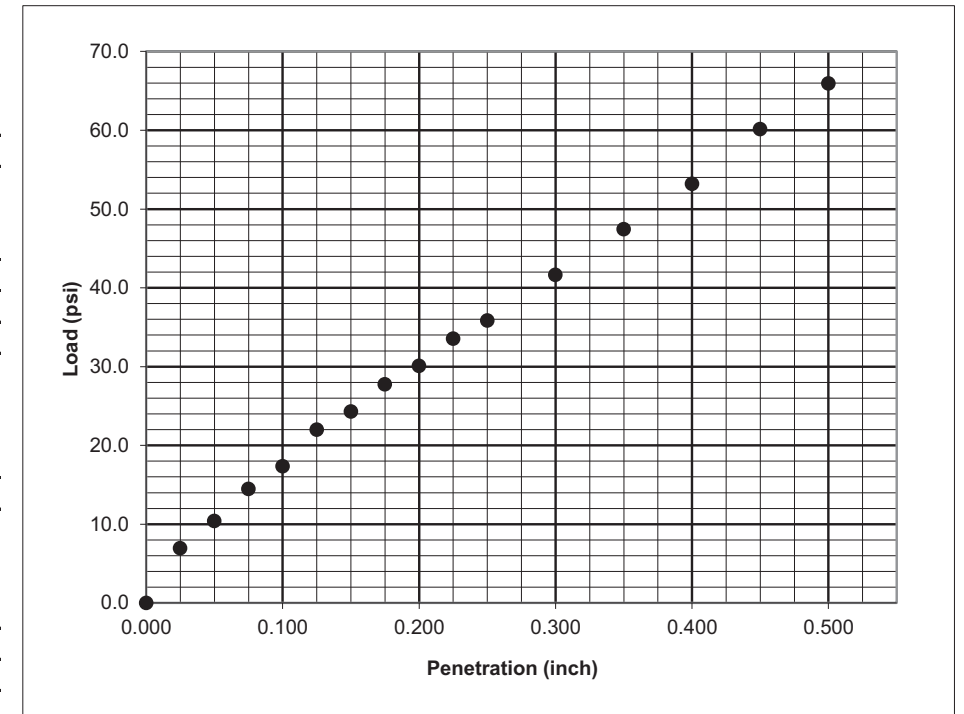


## SAMPLE INFORMATION

Sample Number:	S-6	Proctor Method:	AASHTO T99 - Method A
Boring Number:	L-12700	Maximum Dry Density (pcf):	114.9
Sample Location:	N/A	Optimum Moisture:	13.9
Depth:	1-5'	Liquid Limit:	37
Material Description:	Lean Clay with Sand - AASHTO Classification A-6 (11)	Plasticity Index:	17

## CBR TEST DATA

CBR Value at 0.100 inch	1.7
CBR Value at 0.200 inch	2.0
Surcharge Weight (lbs)	10
Soaking Condition	Soaked
Length of Soaking (hours)	96
Swell (%)	3.1



## DENSITY DATA

Dry Density Before Soaking (pcf)	114.2
Compaction of Proctor (%)	99.4

## MOISTURE DATA

Before Compaction (%)	14.0
After Compaction (%)	14.1
Top 1" After Soaking (%)	20.5
Average After Soaking (%)	16.3

## Comments:

Services: Obtain soil sample and test for California Bearing Ratio

Terracon Rep: Stephanie Huffman  
Reported To: Matthew J. Alexander, PE  
Contractor:

## Report Distribution

Reviewed by: Matthew J. Alexander, PE  
Geotechnical Project Engineer

## Test Methods:

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