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REFERENCE: I-5506

PROJECT: 43608

SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	I-5506	1	76

CONTENTS

LINE	STATION	PLAN
-L-	23+90 - 68+70	4-6
-Y-	60+21 - 115+72	7-10
-RPA-	13+35 - 26+51	5,10
-LPB-	10+00 - 19+66	5
-RPB-	10+00 - 26+94	5,9
-RPC-	24+90 - 28+82	5

CROSS SECTIONS

LINE	STATION	SHEETS
-L-	27+00 - 66+16	11 - 25
-Y-	62+00 - 81+50	26 - 30
-RPA-	15+50 - 25+50	31 - 46
-LPB-	12+02 - 18+00	47 - 51
-RPB-	11+84 - 24+16	52 - 74
-RPC-	25+80 - 27+50	75 - 76

ROADWAY SUBSURFACE INVESTIGATION

COUNTY WAKE
PROJECT DESCRIPTION I-40 AND SR 1002 (AVIATION PARKWAY)
INTERCHANGE

INVENTORY REVISION

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
- THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

B. KEANEY

C. JONES

C. MYERS

W. THOMPSON

S. DAVIS

HDR ENGINEERING, INC.

INVESTIGATED BY F&R, INC.

DRAWN BY CBJ

CHECKED BY BDK

SUBMITTED BY HDR ENGINEERING, INC.

DATE 1/2017

**DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED**



DocuSigned by:
Christopher B. Jones 1/19/2017
79CD97E4882C436...
SIGNATURE DATE



DocuSigned by:
Brian D. Keaney 1/20/2017
79CD97E4882C436...
SIGNATURE DATE

09.08/19

See Sheet 1-A For Index of Sheets
See Sheet 1-B For Conventional Plan Sheet Symbols
See Sheet 1-C For Survey Control Sheet

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

WAKE COUNTY

LOCATION: I-40 AND SR 1002 (AVIATION PARKWAY) INTERCHANGE

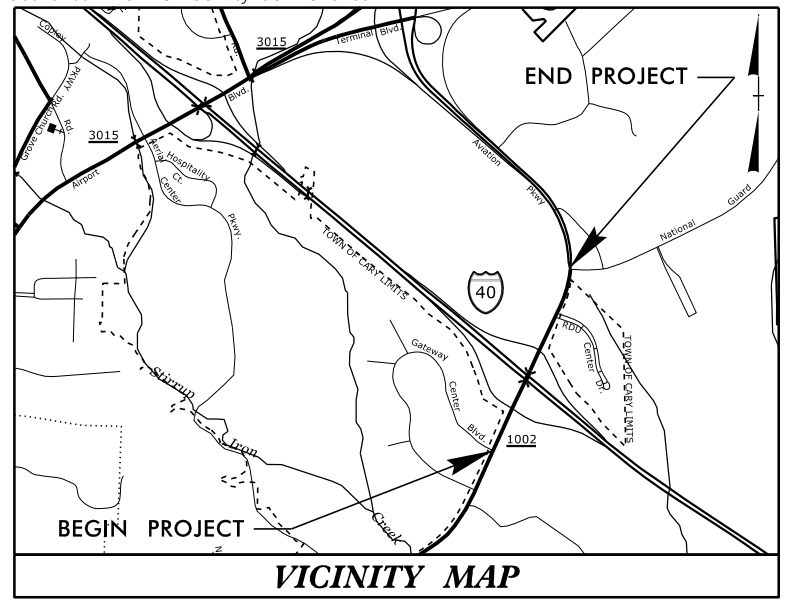
TYPE OF WORK: GRADING, DRAINAGE, PAVING, SIGNALS
CULVERTS AND STRUCTURES

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	I-5506	1	76
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
43608.1.1	NHPP-040-7(154)284	PE	
43608.2.FSI	NHPP-040-7(154)284	ROW/UTIL.	

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

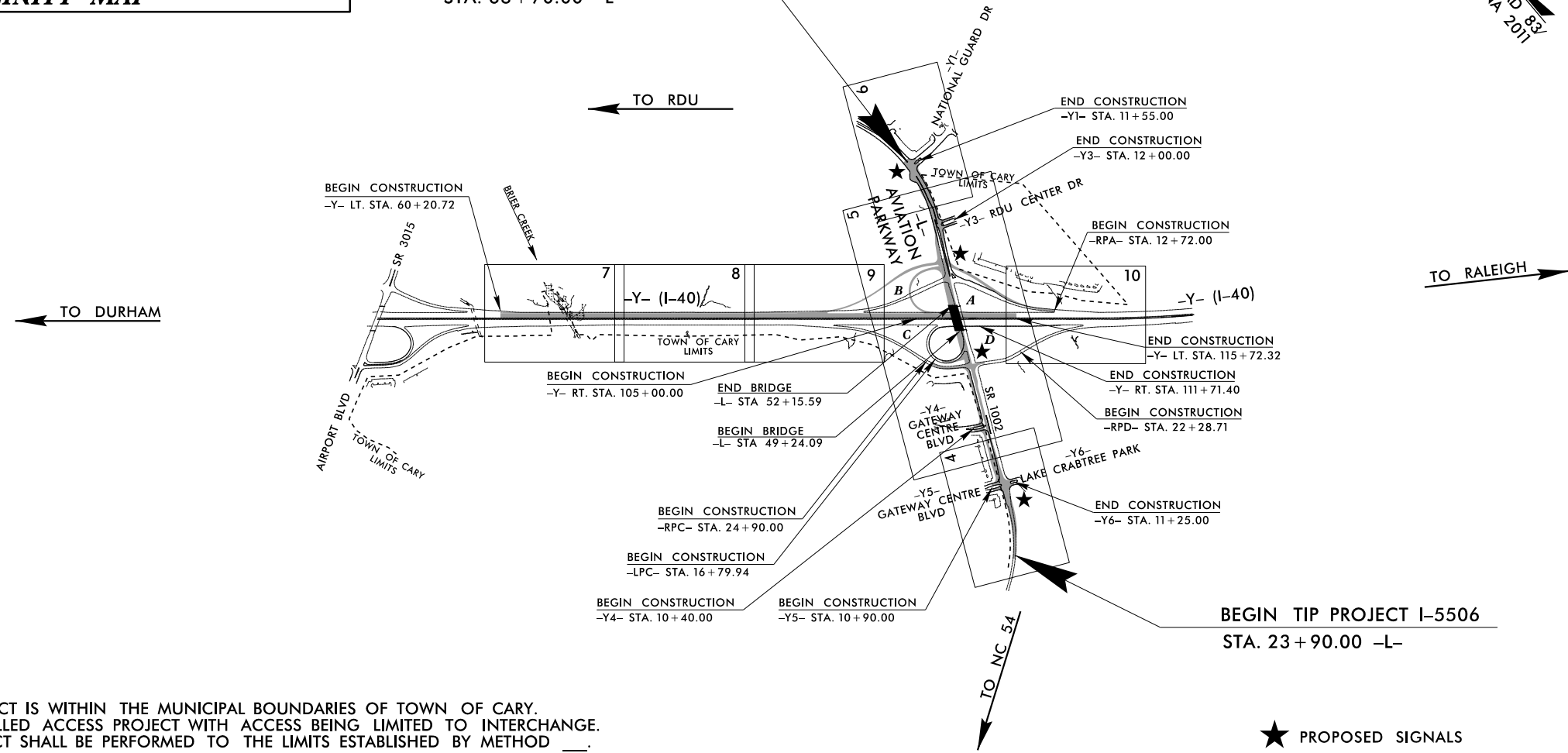
CFI PLANS
SUBMITTAL

TIP PROJECT: I-5506



VICINITY MAP

END TIP PROJECT I-5506
STA. 68 + 70.00 -L-



TO RALEIGH

TO DURHAM

TO RDU

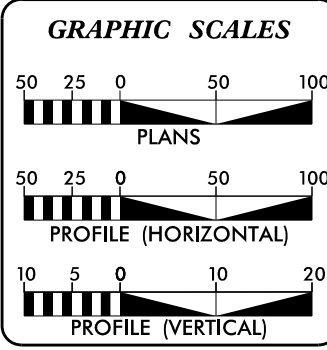
BEGIN TIP PROJECT I-5506
STA. 23 + 90.00 -L-

A PORTION OF THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARIES OF TOWN OF CARY.
THIS IS A PARTIAL CONTROLLED ACCESS PROJECT WITH ACCESS BEING LIMITED TO INTERCHANGE.
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD ____.

★ PROPOSED SIGNALS

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

CONTRACT:



DESIGN DATA

ADT 2018 =	28,555
ADT 2040 =	37,600
K =	55 %
D =	10 %
T =	9 % *
V =	50 MPH
* TTST =	2% DUAL = 5%
FUNC CLASS =	"MINOR ARTERIAL"

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT I-5506	=	0.793 MILES
LENGTH STRUCTURE TIP PROJECT I-5506	=	0.055 MILES
TOTAL LENGTH OF TIP PROJECT I-5506	=	0.848 MILES

Prepared for the North Carolina Department of Transportation in the Office of:

WETHERILL ENGINEERING
1223 JONES FRANKLIN ROAD
Raleigh, N.C. 27606
License No. F-0377
Bus: 919-855-5077
Fax: 919-855-8102

2012 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: **EDWARD G. WETHERILL, PE**
FEBRUARY 17, 2017
PROJECT ENGINEER

LETTING DATE: **BOB A. MAY, PE**
FEBRUARY 20, 2018
PROJECT DESIGN ENGINEER

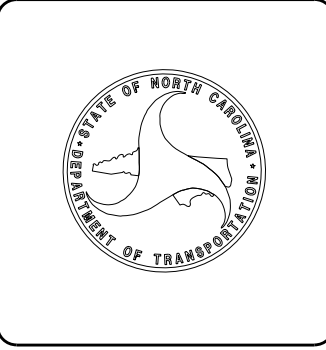
NCDOT CONTACT: **GARY LOVERING, PE**
ROADWAY DESIGN PROJECT ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.



10/13/2016 I:\Roadway\Proj\I5506.Rdy_tsh.dgn USER:fkf



January 5, 2016

STATE PROJECT: 43608
TIP NUMBER: I-5506
COUNTY: Wake
DESCRIPTION: I-40 and SR 1002 (Aviation Parkway) Interchange
SUBJECT: Geotechnical Roadway Inventory Report

PROJECT DESCRIPTION

The I-5506 project is designed to improve the flow of traffic around I-40 and Aviation Parkway near the town of Cary and Raleigh/Durham International Airport in Wake County. The I-5506 project consists of adding through and turn lanes to Aviation Parkway as well as a new bridge along this alignment. It also includes relocating Ramp A north of its current location, constructing Ramp B and Ramp C on a new alignment as well as Loops B and C. Partial widening of I-40 westbound shoulder widening and a culvert extension are also planned. The project length is approximately 0.85 miles along Aviation Parkway (-L-).

The field investigation was conducted in March 2016, using both a CME-55 rubber-tired and rubber-tracked rig with automatic hammer. Standard Penetration Tests (SPT) were performed at selected locations. Representative soil samples were collected and forwarded to an approved NCDOT M&T testing facility for soil quality analysis, moisture content, and AASHTO classification. Hand augers were performed in August 2016 to help identify rock outcropping along the -RPA- and -RPB- alignments.

The following alignments, totaling approximately 2.8 miles, were investigated

Line	Station		Length (ft)
-L-	23+90	to 68+70	4,480
-Y-	60+21	to 115+72	5,551
-RPA-	13+35	to 26+51	1,316
-LPB-	10+00	to 19+66	966
-RPB-	10+00	to 26+94	1694
-RPC-	24+90	to 27+61	271
-LPC	16+80	to 19+53	273
		Total =	14,551 (~2.8 miles)

PHYSIOGRAPHY AND GEOLOGY

Physiography and Geography

The project is located in the Piedmont Physiographic Province. Geologically, it is located in the Durham Triassic Basin. Soils in this region are deeply weathered into sandy silts, silty clays, and clays. The topography is rolling hills and valleys and the drainage from the area is poor. Sediments deposited in the Triassic Basin were eroded from sources including weathered metamorphic and igneous regimes.

The soil in the Durham Triassic Basin have historically proven to be unstable in steeper cut sections, especially those over ten feet high and those facing the south to east directions. Fractures and joints along this direction make cut slopes vulnerable to failure.

Soil Properties

Soils and rock encountered on the project site include roadway embankment, alluvial, Triassic Residual, weathered rock, and non-crystalline rock.

Roadway embankment soils are present in borings advanced on the shoulders of the existing roadways and typically consists of fine to coarse sand, silty sand (A-3, A-1-b, A-2-4), silt (A-4) and silty clay (A-7-6). These soils fluctuate in moisture content from dry to moist and range in thickness from a few feet to approximately 19 feet.

Alluvial soils were found in a few of the borings near the culvert. These soils were present in the upper 6 feet along the -Y- alignment from station 63+50 to 74+00 at approximately 90 to 115 feet LT and consist of fine to coarse silty sand (A-2-4) and sand (A-3).

The Triassic Residual deposits are present at all boring locations and consist of fine to coarse sand and clayey sand (A-1-b, A-3, A-2-4, A-2-6/7), clay (A-6, A-7-6), and silt (A-4). The consistency of the sands is generally loose to very dense while the consistency of the clays and silts is generally medium stiff to hard. The moisture content of the soils tested vary from dry to moist.

Weathered rock was also encountered along the project site at multiple areas. The weathered rock consists of a red-brown Triassic Sandstone and Siltstone, and was encountered at depths ranging from the surface to 11 feet.

Non-crystalline rock was found below the weathered rock in some of the borings. Although none of the rock was cored, it was identified by fragments in samples and by split spoon refusal. The non-crystalline rock consists of a red-brown Triassic Siltstone and Sandstone, characteristic of the region.

Groundwater

Groundwater varied over much of the site depending on the topography in which the boring was located. Generally in certain areas of the site, along lower natural ground areas, groundwater was encountered within several feet of the existing ground surface after 24 hours. Some of the water encountered on the site may be perched above impermeable zones.

Areas of Special Geotechnical Interest

Triassic weathered rock and non-crystalline rock was encountered above the proposed grade along some cut sections of the project. Table 1 below indicates the cut sections where the rock was encountered above grade or within 6' of proposed grade. Four rock soundings were completed in the vicinity of Ramp B to verify the presence of non-crystalline rock due to early termination of surrounding borings. SPT refusal tests in each rock sounding confirmed the non-crystalline rock.

Table 1 - Rock above Grade in Cut Sections

Alignment	Begin Station	End Station
-RPA-	16+50	21+50
-RPB-	12+10	18+00

The following sections were found to exhibit groundwater above or within 6' of proposed grade.

Alignment	Begin Station	End Station	Comment
-L-	29+50	30+50	Single boring, water at grade
-L-	36+50	37+50	Single boring, water at grade
-L-	57+50	62+50	Water at grade
-LPB-	13+30	18+37	Likely wet in cut section and near grade
-RPB-	15+80	17+32	Likely wet in cut section
-RPC-	27+16	27+61	Water at grade

Geotechnical Testing

The following samples were obtained to determine engineering properties of the soil and a list of each test completed for analysis are listed below.

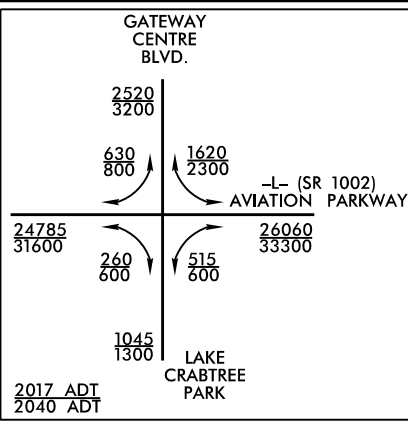
Boring	Sample No.	Depth (ft)	Tests
LPB_1815L	ST-1	1.0'-3.0'	Consolidation, Class, Moisture
RPA_1997R	BS #1	2.0'-4.0'	CBR, Std. Proctor, Class, Moisture
RPB_1733L	BS #2	2.0'-10.0'	CBR, Std. Proctor, Class, Moisture

Sincerely,
HDR ENGINEERING, INC.

Chris B. Jones, P.E.
Geotechnical Engineer

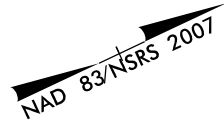
Brian D. Keane, P.E.
Geotechnical Project Manager

8/17/99



-L- (AVIATION PARKWAY)

PI Sta 26+25.79	PI Sta 34+81.13
$\Delta = 37' 15'' 07.8''$ (LT)	$\Delta = 5' 47'' 36.6''$ (RT)
D = 3' 40' 22.2"	D = 0' 47' 25.0"
L = 1,014.27'	L = 733.09'
T = 525.79'	T = 366.86'
R = 1,560.00'	R = 7,250.00'
SE = 0.04 FT/FT	SE = NC
DS = 50 MPH	DS = 50 MPH



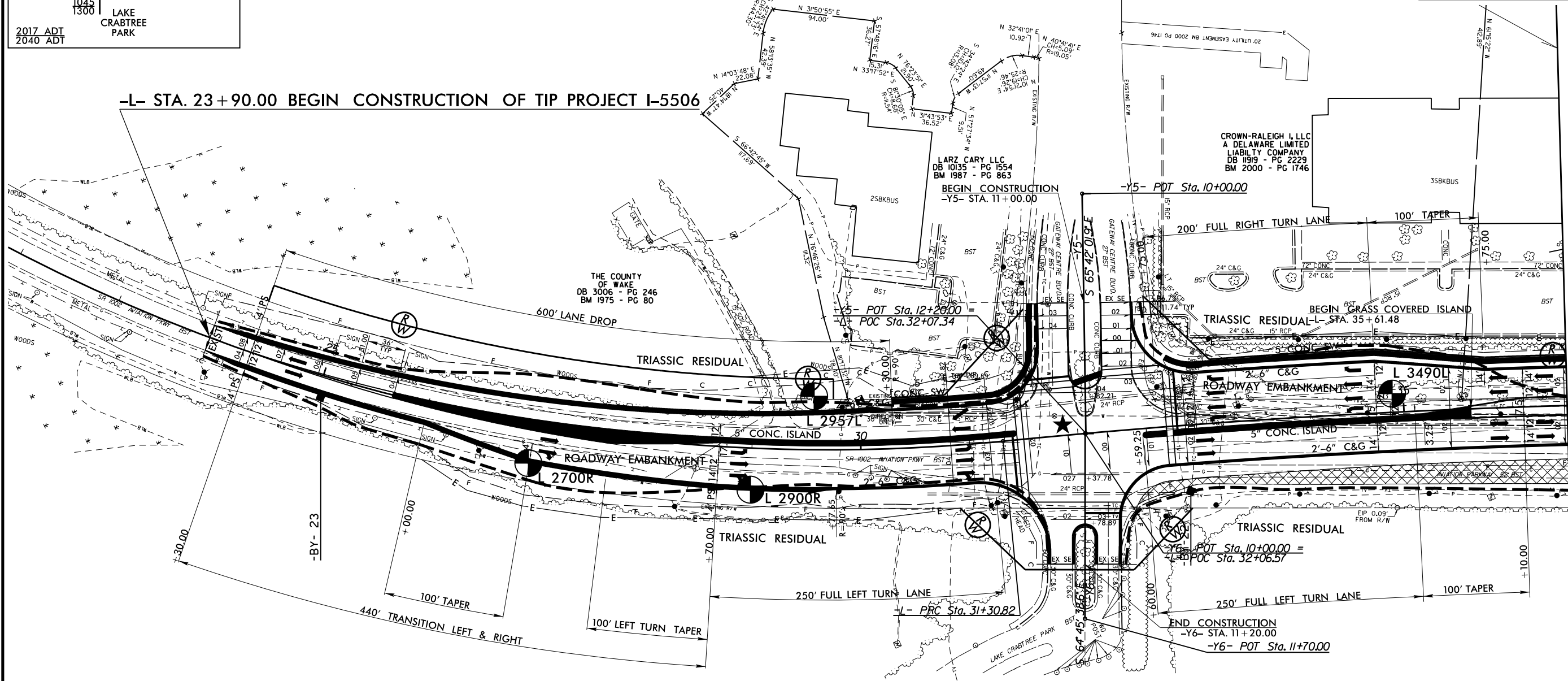
WETHERILL ENGINEERING
 1223 Jones Franklin Road
 Raleigh, N.C. 27606
 License No. E-4977
 Bus: 919 851 8077
 Fax: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. I-5506	SHEET NO. 4
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

-L- STA. 23+90.00 BEGIN CONSTRUCTION OF TIP PROJECT I-5506

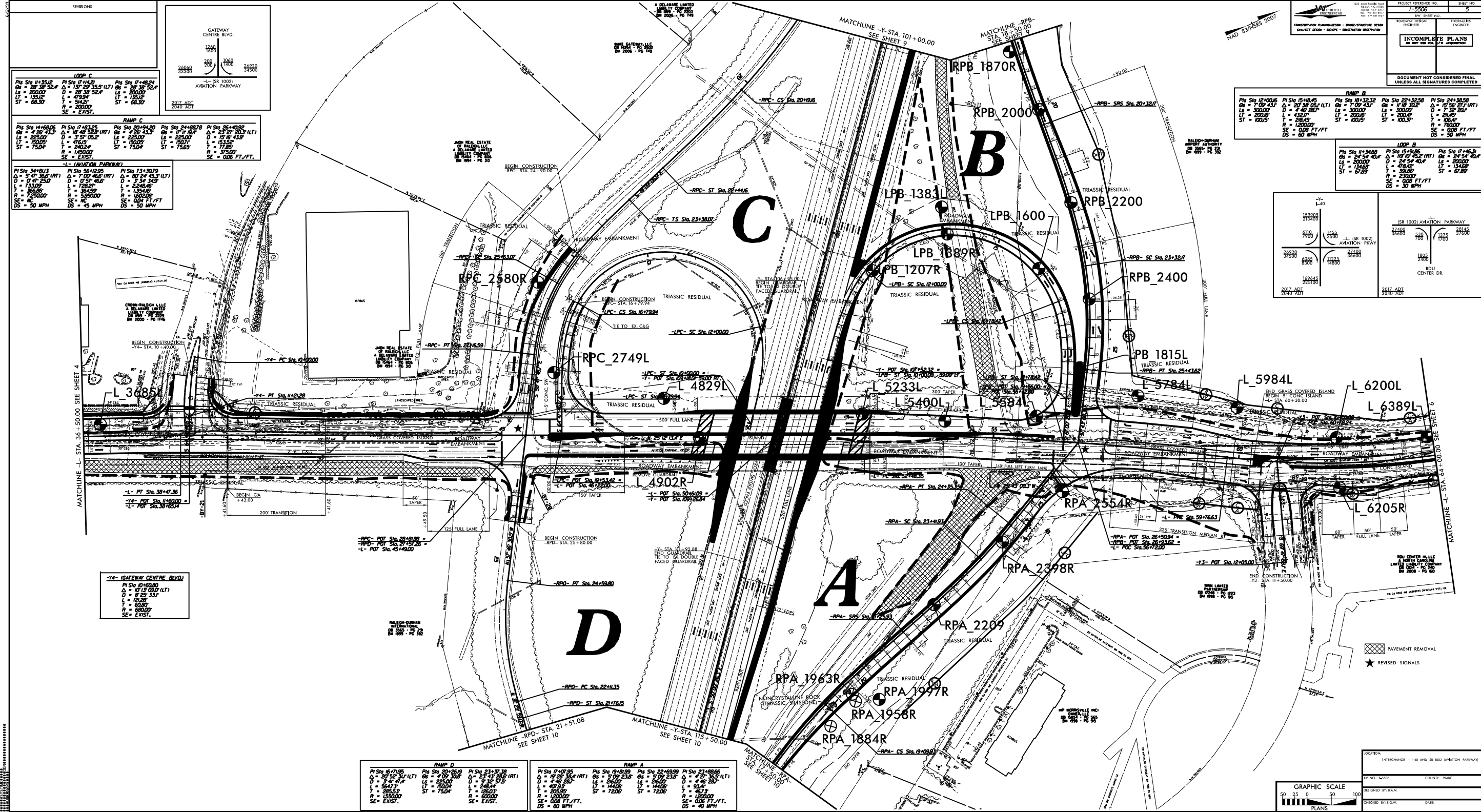
REVISIONS



MATCHLINE -L- STA. 36+50.00 SEE SHEET 5

PAVEMENT REMOVAL
 REVISED SIGNAL

SYSTEMS CONDITION



REVISIONS

LOOP C		
PI STA 11+35.05	PI STA 17+48.24	PI STA 17+48.24
BS = 28' 52" (20.000')	Δ = 131° 38' 52" (ILT)	BS = 28' 52" (20.000')
LS = 200.00'	D = 28' 38' 52"	LS = 200.00'
LT = 135.00'	L = 135.00'	LT = 135.00'
ST = 68.30'	T = 54.20'	ST = 68.30'
R = 2000.00'	R = 2000.00'	R = 2000.00'
SE = EXIST.	SE = EXIST.	SE = EXIST.

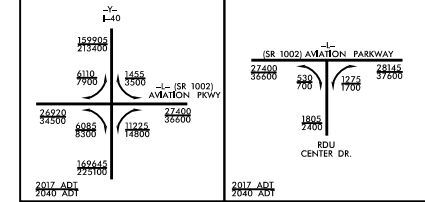
RAMP C				
PI STA 11+63.06	PI STA 17+83.25	PI STA 20+94.20	PI STA 24+82.78	PI STA 25+40.92
BS = 4' 20" (4.33')	Δ = 89° 46' 35" (RT)	BS = 4' 20" (4.33')	Δ = 17' 11" (R)	Δ = 23' 27" (ILT)
LS = 525.00'	D = 3' 05.2"	LS = 225.00'	LS = 525.00'	D = 2' 51" (3.33')
L = 525.00'	L = 75.00'	L = 150.00'	L = 507'	L = 513.50'
LT = 358.84'	T = 75.00'	LT = 150.00'	LT = 507'	T = 72.80'
R = 1650.00'	R = 175.00'	R = 1500.00'	R = 375.00'	R = 375.00'
SE = EXIST.	SE = EXIST.	SE = EXIST.	SE = EXIST.	SE = 0.06 FT./FT.

LAVATION PARKWAY		
PI STA 34+81.13	PI STA 56+42.95	PI STA 73+30.79
Δ = 5' 41" (5.67 RT)	Δ = 7' 00" (6.67 RT)	Δ = 60' 24" (53.17 LT)
D = 0' 41" (5.40')	D = 0' 31" (4.50')	D = 1' 34" (3.43')
L = 733.00'	L = 758.20'	L = 2,948.40'
L = 366.50'	L = 379.10'	L = 1,474.20'
R = 7200.00'	R = 3050.00'	R = 1602.00'
SE = 30 MPH	SE = 45 MPH	SE = 50 MPH

RAMP B				
PI STA 12+00.16	PI STA 15+18.45	PI STA 19+32.32	PI STA 22+32.58	PI STA 24+38.58
BS = 7' 09" (4.17')	Δ = 207' 39" (52.17 LT)	BS = 17' 28" (5.02')	Δ = 17' 52" (5.02')	Δ = 12' 50" (3.71 RT)
LS = 300.00'	D = 4' 46" (20.00')	LS = 300.00'	LS = 300.00'	D = 7' 35" (20.00')
L = 200.00'	L = 432.00'	L = 200.00'	L = 200.00'	L = 200.00'
LT = 200.00'	T = 284.65'	LT = 200.00'	LT = 200.00'	T = 264.90'
R = 1500.00'	R = 1500.00'	R = 1500.00'	R = 1500.00'	R = 1500.00'
SE = 60 MPH	SE = 60 MPH	SE = 60 MPH	SE = 60 MPH	SE = 60 MPH

LOOP B				
PI STA 11+34.68	PI STA 15+18.45	PI STA 17+48.24	PI STA 17+48.24	PI STA 17+48.24
BS = 24' 54" (40.00')	Δ = 197' 10" (45.21 LT)	BS = 24' 54" (40.00')	Δ = 12' 50" (3.71 RT)	Δ = 12' 50" (3.71 RT)
LS = 200.00'	D = 24' 54" (40.00')	LS = 200.00'	LS = 200.00'	LS = 200.00'
L = 124.60'	L = 476.85'	L = 476.85'	L = 476.85'	L = 476.85'
LT = 124.60'	T = 138.60'	LT = 124.60'	LT = 124.60'	T = 138.60'
R = 2300.00'	R = 2300.00'	R = 2300.00'	R = 2300.00'	R = 2300.00'
SE = 30 MPH	SE = 30 MPH	SE = 30 MPH	SE = 30 MPH	SE = 30 MPH

AVATION PARKWAY		
PI STA 10+60.80	PI STA 10+60.80	PI STA 10+60.80
Δ = 10' 13" (9.00 LT)	Δ = 10' 13" (9.00 LT)	Δ = 10' 13" (9.00 LT)
D = 0' 25" (3.33')	D = 0' 25" (3.33')	D = 0' 25" (3.33')
L = 60.00'	L = 60.00'	L = 60.00'
LT = 60.00'	LT = 60.00'	LT = 60.00'
R = 600.00'	R = 600.00'	R = 600.00'
SE = EXIST.	SE = EXIST.	SE = EXIST.



GRAPHIC SCALE
0 25 50 100
PLANS

LOCATION: INTERCHANGE - I-40 AND SR 102 (AVATION PARKWAY)

IF NO. 15235 COUNTY WAKE

DESIGNED BY E.G.W.

CHECKED BY E.G.W.

DATE: _____

PROJECT REFERENCE NO. 7-5506
SHEET NO. 5
ROADWAY DESIGN ENGINEER
ROADWAY DESIGN ENGINEER
INCOMPLETE PLANS
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

8/17/99

-L- (AVIATION PARKWAY)
 PI Sta 73+30.79
 $\Delta = 80^{\circ} 24' 45.3"$ (LT)
 $D = 3^{\circ} 34' 34.9"$
 $L = 2,248.46'$
 $T = 1,354.16'$
 $R = 1,602.08'$
 $SE = 0.04$ FT/FT
 $DS = 50$ MPH

-YI- (NATIONAL GUARD DR.)
 PI Sta 15+38.84
 $\Delta = 23^{\circ} 14' 21.4"$ (LT)
 $D = 4^{\circ} 38' 21.6"$
 $L = 500.92'$
 $T = 253.95'$
 $R = 1,235.00'$
 $SE = EXIST.$

10
 RALEIGH-DURHAM
 AIRPORT AUTHORITY
 DB 3565 - PG 231
 BM 1999 - PG 392

WETHERILL ENGINEERING
 TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO.	SHEET NO.
1-5506	6
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

10
 RALEIGH-DURHAM
 AIRPORT AUTHORITY
 DB 3565 - PG 231
 BM 1999 - PG 392

-L- STA. 68+70.00 END TIP PROJECT I-5506

TIE TO EX C&G
 END GRADE
 BEGIN MILLING & OVERLAY
 -L- STA. 65+80.00
 END CA
 +90.00

-YI- POT Sta. 10+00.00 =
 -L- POC Sta. 67+79.37

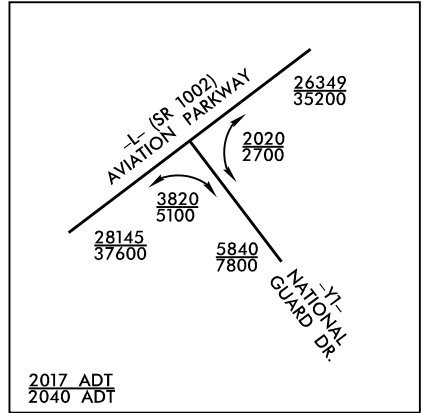
END MILLING & OVERLAY
 END CONSTRUCTION
 -L- STA. 68+70.00

END MILLING & OVERLAY
 END CONSTRUCTION
 -YI- STA. 11+45.00
 -YI- PC Sta. 12+84.89

18
 RALEIGH-DURHAM
 INTERNATIONAL
 DB 3565 - PG 231
 BM 1999 - PG 392

19
 RDU CENTER IV, LLC
 A NORTH CAROLINA
 LIMITED LIABILITY COMPANY
 DB 13014 - PG 240
 BM 2008 - PG 160

18
 RALEIGH-DURHAM
 INTERNATIONAL
 DB 3565 - PG 231
 BM 1999 - PG 392



REVISIONS

SYSTEMS TIME...

★ REVISED SIGNAL

8/17/99

WETHERILL ENGINEERING
 TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

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 Fax: 919 851 8107

PROJECT REFERENCE NO. 1-5506	SHEET NO. 7
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

RALEIGH-DURHAM
 AIRPORT AUTHORITY
 DB 8353 - PG 192
 BM 1999 - PG 392

J.B. WILKINSON &
 MILDRED BOYD WILKINSON
 DB 1100 - PG 337
 BM 1951 - PG 77

COUNTY OF WAKE
 DB 2337 - PG 379
 BM 1975 - PG 80

COUNTY OF WAKE
 DB 3057 - PG 297
 BM 1975 - PG 80

RALEIGH-DURHAM
 AIRPORT AUTHORITY
 DB 8875 - PG 1850
 BM 1975 - PG 80

BEGIN CONSTRUCTION
 -Y- STA. 60+20.72
 TIE TO EXIST. GUARDRAIL

TIE EOT TO EXIST. EOT

EXTEND EX. 3@9'x8' RCBC

EX. OVERHEAD SIGN

Y 6200L

Y 6673L

Y 7203L

ROADWAY EMBANKMENT

ROADWAY EMBANKMENT

DAN K MOORE FREEWAY 3' BST

EX. 3@9'x8' RCBC

ALLUVIAL

ALLUVIAL

AERIAL CENTER REALTY CORP.,
 A NORTH CAROLINA CORPORATION
 DB 1474 - PG 1846
 BM 1988 - PG 739

THE COUNTY
 OF WAKE
 DB 3944 - PG 922
 BM 1975 - PG 80

S 26°50'12" E
 174.0'

S 37°14'19" E
 141.71'

S 72°10'41" E
 187.88'

S 69°20'59" W
 85.26'

S 05°17'42" E
 189.83'

15 SOUTHERN BELL EASEMENT DB 513 PG 214

REVISIONS

SECTION
 TIME
 DATE
 BY
 CHECKED
 DATE
 BY

MATCHLINE -Y- STA. 73+00.00 SEE SHEET 8

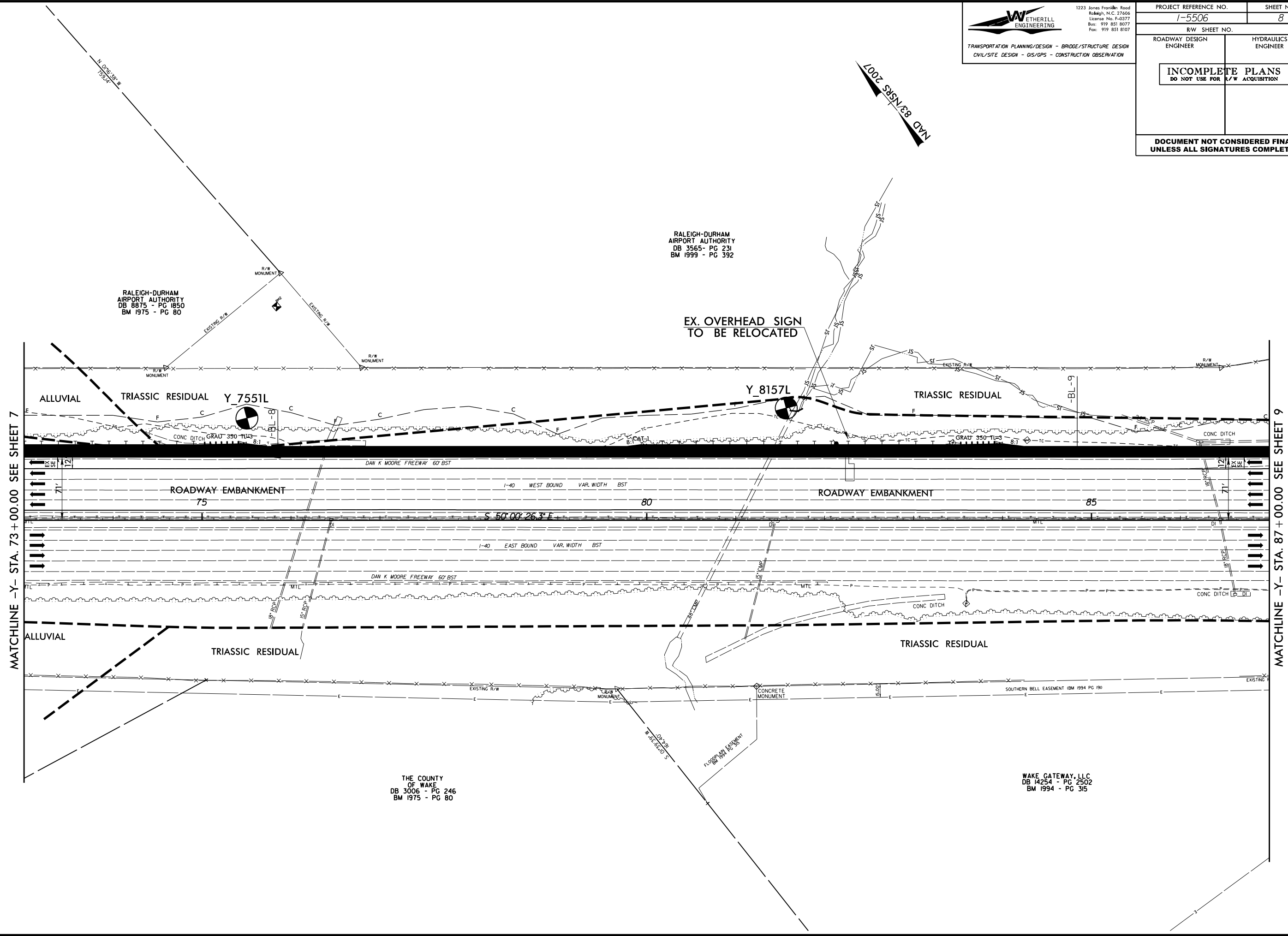
8/17/99

WETHERILL ENGINEERING
 1223 Jones Franklin Road
 Raleigh, N.C. 27606
 License No. E-49377
 Bus: 919 851 8077
 Fax: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. 1-5506	SHEET NO. 8
R/W SHEET NO. ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

REVISIONS



MATCHLINE -Y- STA. 73+00.00 SEE SHEET 7

MATCHLINE -Y- STA. 87+00.00 SEE SHEET 9

RALEIGH-DURHAM
AIRPORT AUTHORITY
DB 8875 - PG 1850
BM 1975 - PG 80

RALEIGH-DURHAM
AIRPORT AUTHORITY
DB 3565- PG 231
BM 1999 - PG 392

EX. OVERHEAD SIGN
TO BE RELOCATED

Y_8157L

TRIASSIC RESIDUAL

ROADWAY EMBANKMENT

ROADWAY EMBANKMENT

ALLUVIAL

TRIASSIC RESIDUAL

TRIASSIC RESIDUAL

THE COUNTY
OF WAKE
DB 3006 - PG 246
BM 1975 - PG 80

WAKE GATEWAY, LLC
DB 14254 - PG 2502
BM 1994 - PG 315

DATE PLOTTED: 08/17/99 10:00 AM

8.17/99

RAMP B				
PIs Sta 12+00.16	PI Sta 15+18.45	PIs Sta 18+32.32	PIs Sta 22+32.58	PI Sta 24+38.58
$\Theta_s = 7^{\circ} 09' 43.1''$	$\Delta = 20^{\circ} 38' 05.1''$ (LT)	$\Theta_s = 7^{\circ} 09' 43.1''$	$\Theta_s = 1^{\circ} 18' 30.2''$	$\Delta = 15^{\circ} 56' 27.1''$ (RT)
$L_s = 300.00'$	$D = 4^{\circ} 46' 28.7''$	$L_s = 300.00'$	$L_s = 300.00'$	$D = 7^{\circ} 32' 20.1''$
$LT = 200.16'$	$L = 432.17'$	$LT = 200.16'$	$LT = 200.41'$	$L = 211.45'$
$ST = 100.15'$	$T = 218.45'$	$ST = 100.15'$	$ST = 100.37'$	$T = 106.41'$
	$R = 1,200.00'$			$R = 760.00'$
	$SE = 0.08$ FT/FT			$SE = 0.08$ FT/FT
	$DS = 60$ MPH			$DS = 50$ MPH

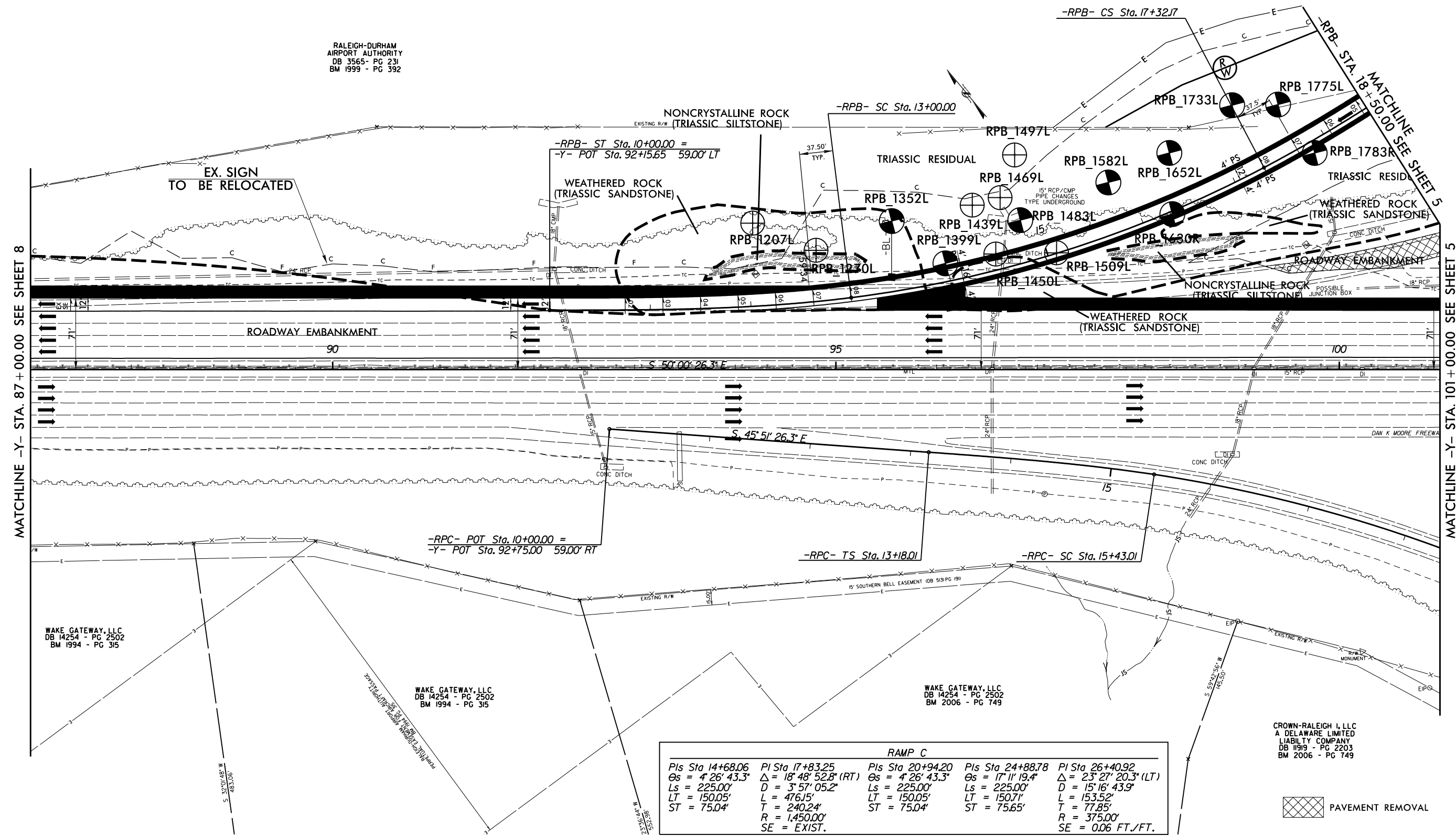
WETHERILL ENGINEERING
 1223 Jones Franklin Road
 Raleigh, N.C. 27606
 License No. F-0377
 Bus: 919 851 8077
 Fax: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. 1-5506	SHEET NO. 9
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

NAD 83/NSRS 2007

RALEIGH-DURHAM AIRPORT AUTHORITY
DB 3565 - PG 231
BM 1999 - PG 392



MATCHLINE -Y- STA. 87 + 00.00 SEE SHEET 8

MATCHLINE -Y- STA. 101 + 00.00 SEE SHEET 5

WAKE GATEWAY, LLC
DB 14254 - PG 2502
BM 1994 - PG 315

WAKE GATEWAY, LLC
DB 14254 - PG 2502
BM 1994 - PG 315

WAKE GATEWAY, LLC
DB 14254 - PG 2502
BM 2006 - PG 749

CROWN-RALEIGH I, LLC
A DELAWARE LIMITED LIABILITY COMPANY
DB 11919 - PG 2203
BM 2006 - PG 749

RAMP C				
PIs Sta 14+68.06	PI Sta 17+83.25	PIs Sta 20+94.20	PIs Sta 24+88.78	PI Sta 26+40.92
$\Theta_s = 4^{\circ} 26' 43.3''$	$\Delta = 18^{\circ} 48' 52.8''$ (RT)	$\Theta_s = 4^{\circ} 26' 43.3''$	$\Theta_s = 17^{\circ} 11' 19.4''$	$\Delta = 23^{\circ} 27' 20.3''$ (LT)
$L_s = 225.00'$	$D = 3^{\circ} 57' 05.2''$	$L_s = 225.00'$	$L_s = 225.00'$	$D = 15^{\circ} 16' 43.9''$
$LT = 150.05'$	$L = 476.15'$	$LT = 150.05'$	$LT = 150.71'$	$L = 153.52'$
$ST = 75.04'$	$T = 240.24'$	$ST = 75.04'$	$ST = 75.65'$	$T = 77.85'$
	$R = 1,450.00'$			$R = 375.00'$
	$SE = EXIST.$			$SE = 0.06$ FT/FT.

PAVEMENT REMOVAL

REVISIONS

8.17/99

8/17/99

WETHERILL ENGINEERING
 1223 Jones Franklin Road
 Raleigh, N.C. 27606
 License No. F-0377
 Bus: 919 851 8077
 Fax: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN
 CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

PROJECT REFERENCE NO. 1-5506	SHEET NO. 10
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

RAMP A

PIs Sta 14+30.06	PI Sta 17+07.95
Θs = 5° 09' 23.8"	Δ = 19° 28' 38.4" (RT)
Ls = 216.00'	D = 4' 46" 28.7"
LT = 144.06'	L = 407.93'
ST = 72.06'	T = 205.95'
	R = 1,200.00'
	SE = 0.08 FT./FT.
	DS = 60 MPH

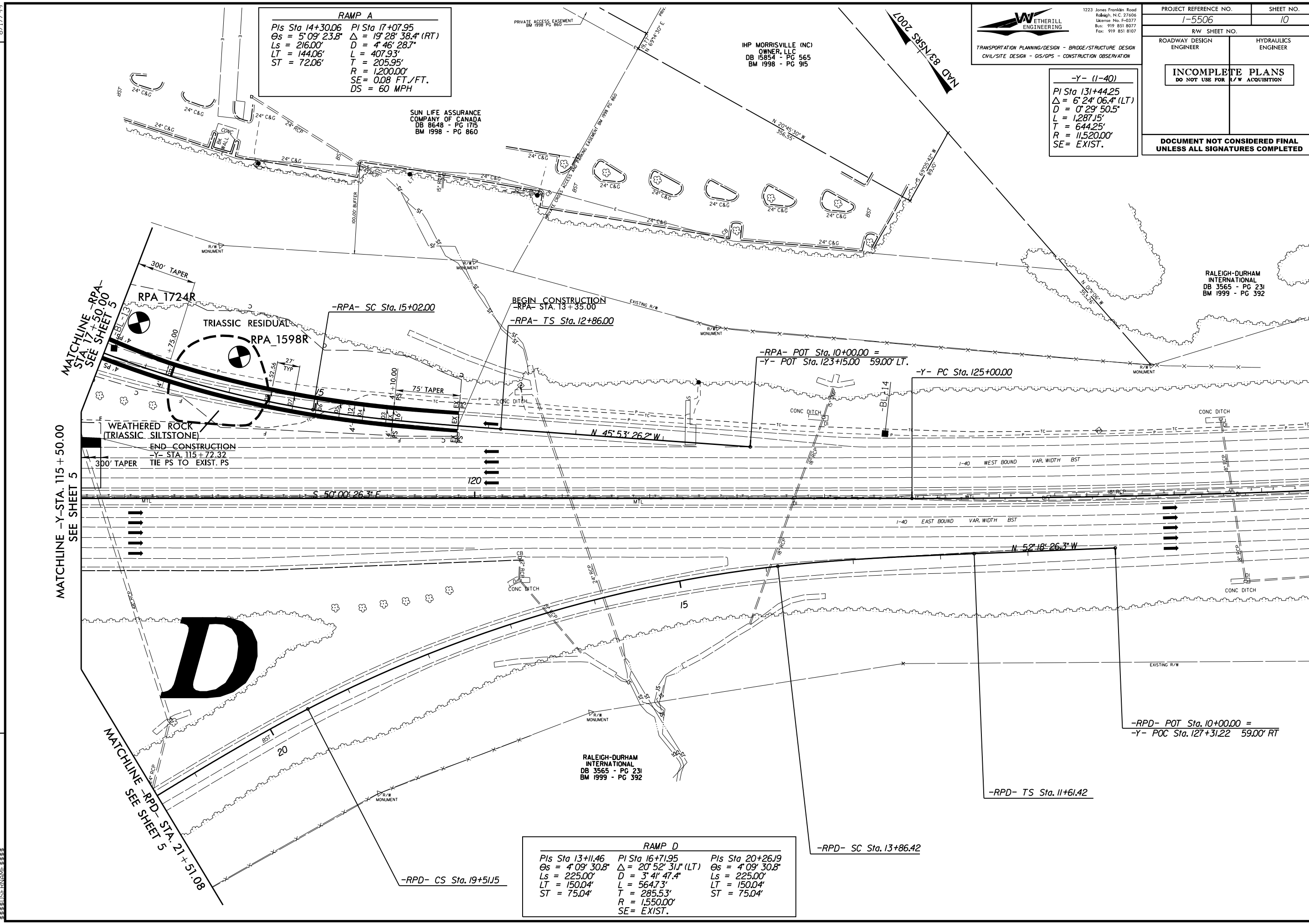
IHP MORRISVILLE (NC)
 OWNER, LLC
 DB 15854 - PG 565
 BM 1998 - PG 915

SUN LIFE ASSURANCE
 COMPANY OF CANADA
 DB 8648 - PG 175
 BM 1998 - PG 860

-Y- (1-40)

PI Sta 131+44.25
Δ = 6° 24' 06.4" (LT)
D = 0' 29' 50.5"
L = 1,287.15'
T = 644.25'
R = 11,520.00'
SE = EXIST.

RALEIGH-DURHAM
 INTERNATIONAL
 DB 3565 - PG 231
 BM 1999 - PG 392



RAMP D

PIs Sta 13+11.46	PI Sta 16+71.95	PIs Sta 20+26.19
Θs = 4° 09' 30.8"	Δ = 20° 52' 31.1" (LT)	Θs = 4° 09' 30.8"
Ls = 225.00'	D = 3' 41' 47.4"	Ls = 225.00'
LT = 150.04'	L = 564.73'	LT = 150.04'
ST = 75.04'	T = 285.53'	ST = 75.04'
	R = 1,550.00'	
	SE = EXIST.	

MATCHLINE -Y- STA. 115+50.00
SEE SHEET 5

MATCHLINE -RPA- STA. 17+50.00
SEE SHEET 5

MATCHLINE -RPD- STA. 21+51.08
SEE SHEET 5

REVISIONS

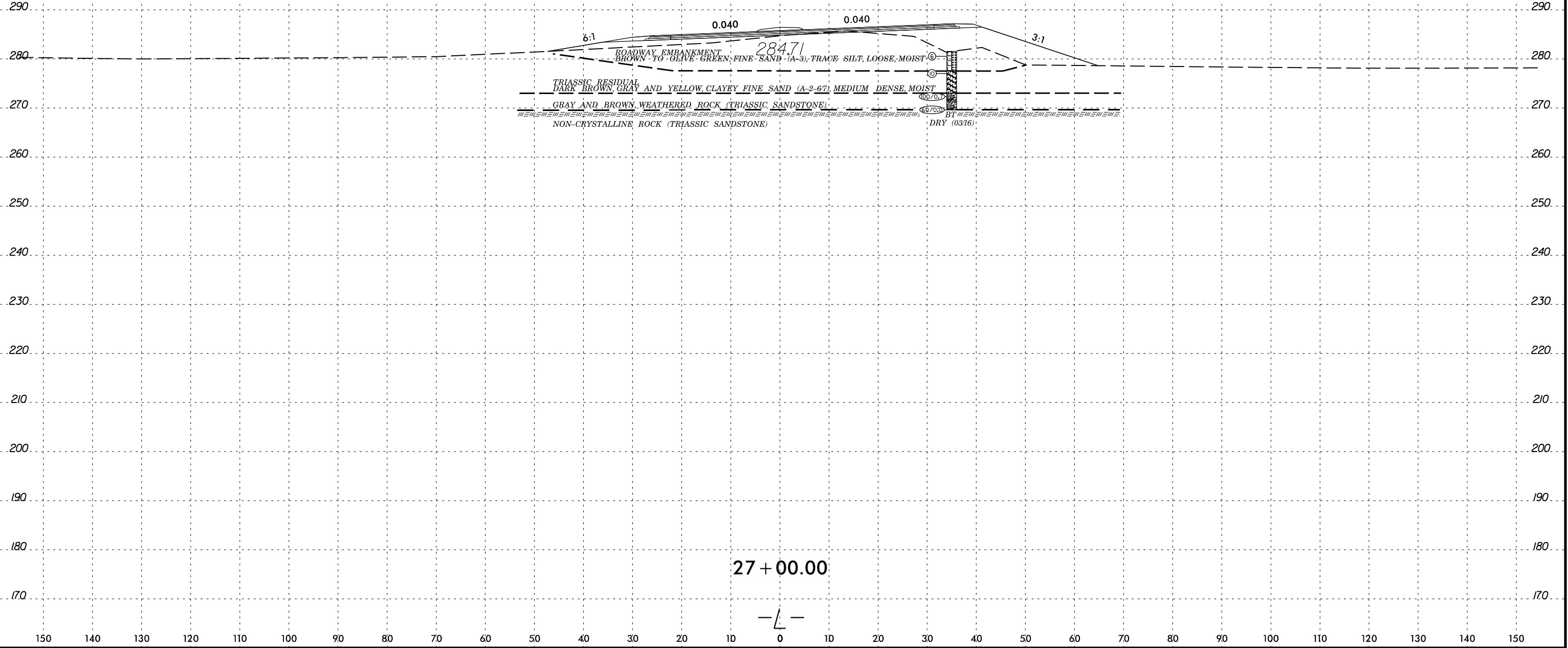
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 8/17/99 10:00 AM
 8/17/99 10:00 AM
 8/17/99 10:00 AM

8/23/99

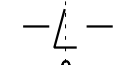


PROJ. REFERENCE NO.
I-5506

SHEET NO.
11



27 + 00.00

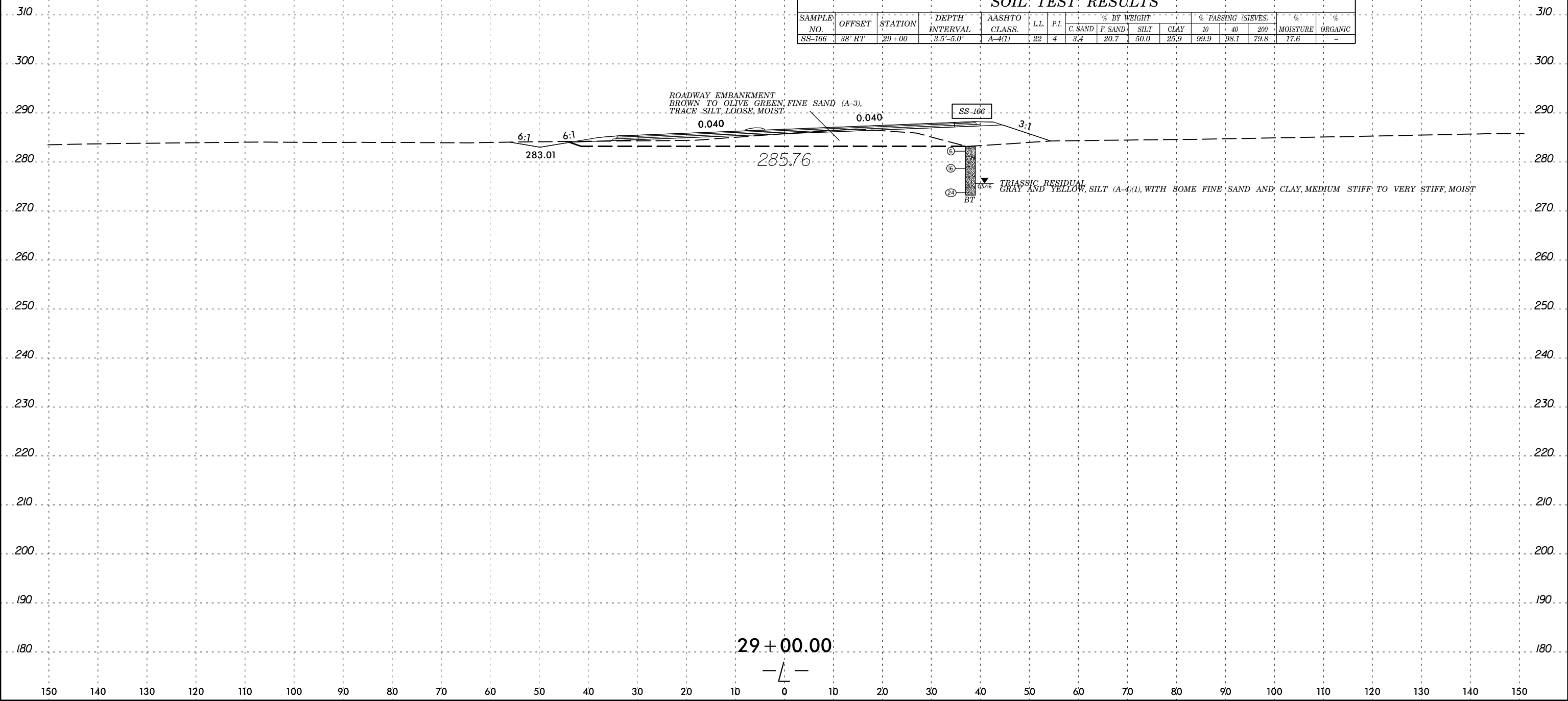


PLOT DRIVER: \$PLTRV\$
USER: \$USER\$
FILE: \$PWVAVULTPATHDESC\$
DATE: \$DATE\$
TIME: \$TIME\$
PENTABLE: \$PENTBL\$

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-166	38' RT	29+00	3.5'-5.0'	A-4(1)	22	4	3.4	20.7	50.0	25.9	99.9	98.1	79.8	17.6	-

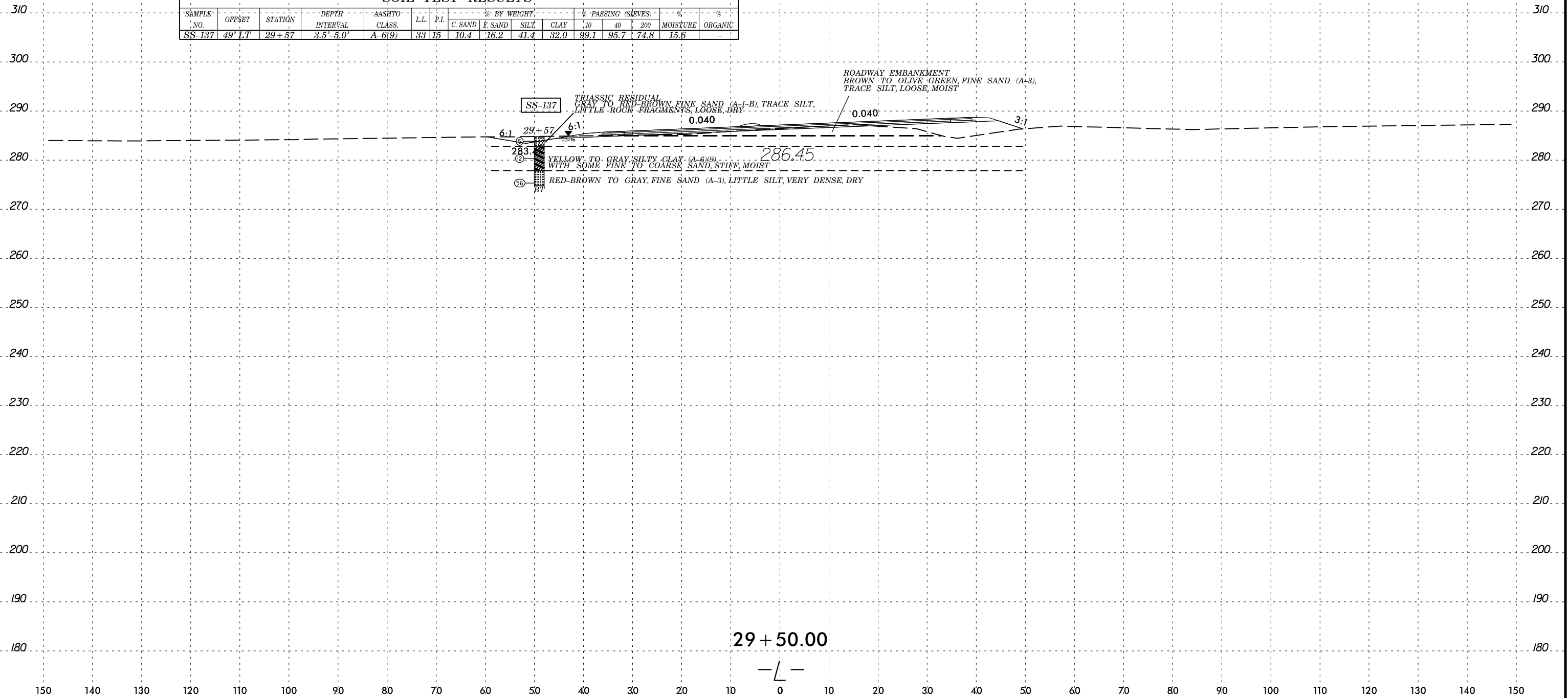
ROADWAY EMBANKMENT
BROWN TO OLIVE GREEN, FINE SAND (A-3),
TRACE SILT, LOOSE, MOIST

TRIASSIC RESIDUAL
GRAY AND YELLOW, SILT (A-4)(1), WITH SOME FINE SAND AND CLAY, MEDIUM STIFF TO VERY STIFF, MOIST

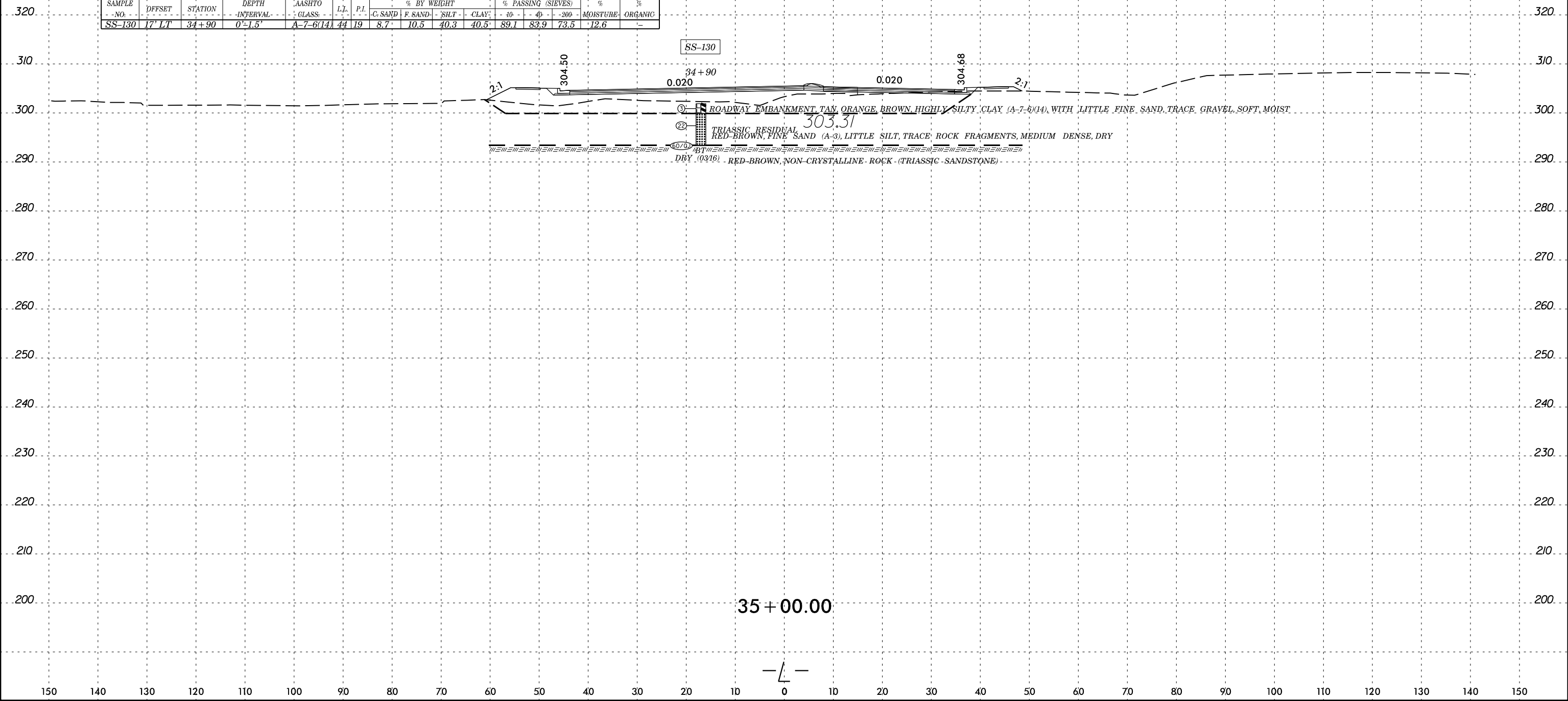


29+00.00
— 1 —

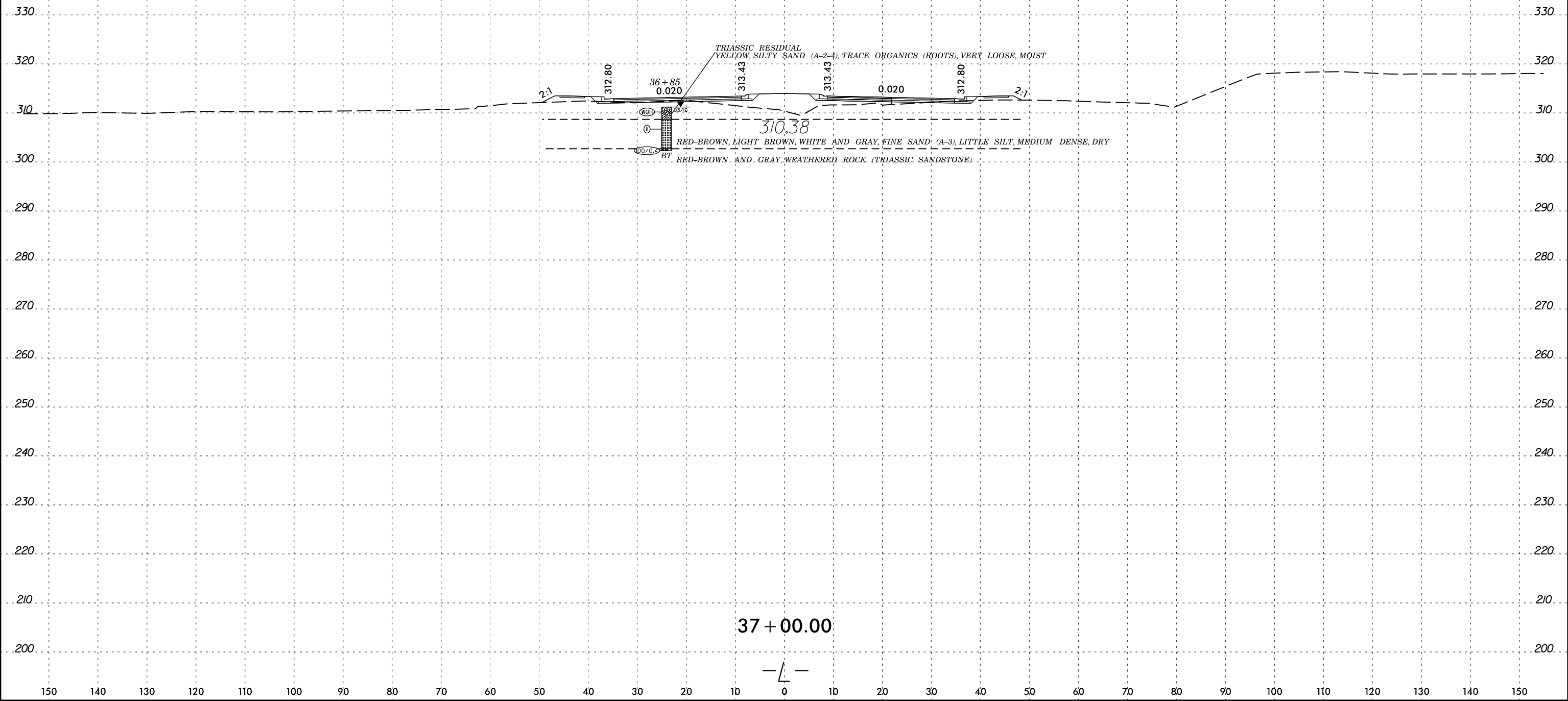
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	BY WEIGHT				PASSING (SIEVES)			MOISTURE	ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
SS-137	49' LT	29+57	3.5'-5.0'	A-6(9)	33	15	10.4	16.2	41.4	32.0	99.1	95.7	74.8	15.6	-

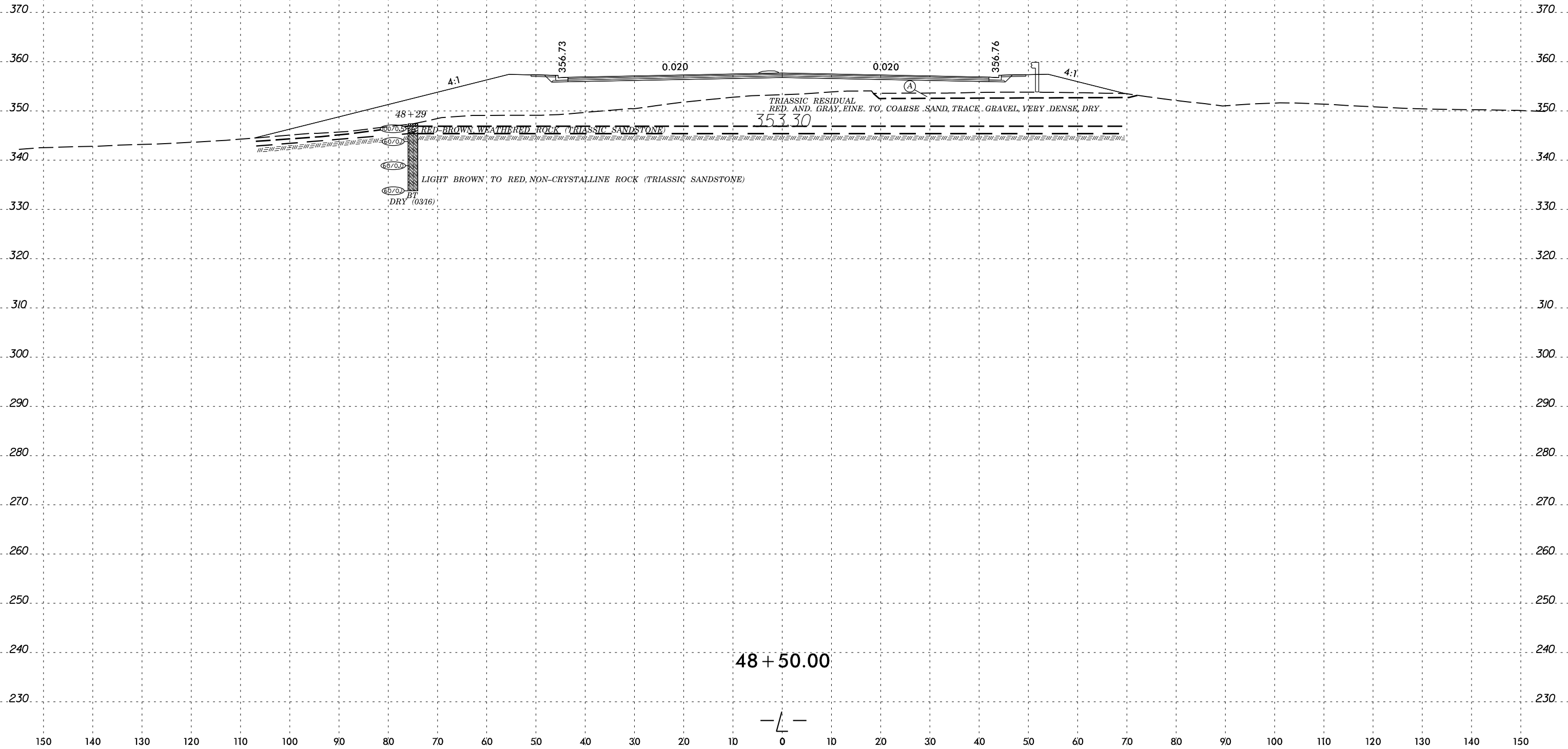


SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
SS-130	17' LT	34+90	0'-1.5'	A-7-6(14)	44	19	8.7	10.5	40.3	40.5	89.1	83.9	73.5	12.6	-

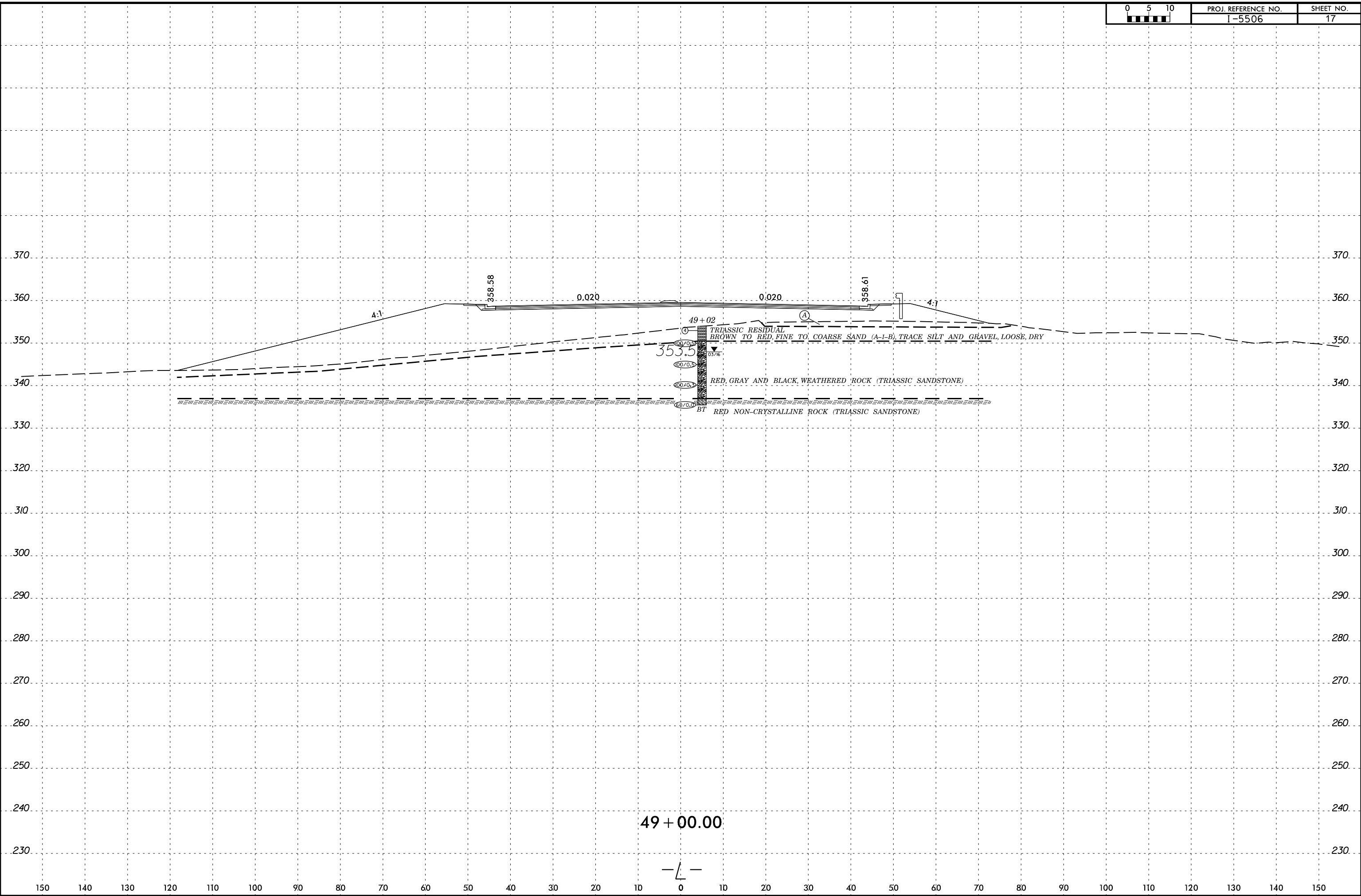


PLOT DRIVER: \$PLTRV\$
 USER: \$USER\$
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 DATE: \$DATE\$
 TIME: \$TIME\$
 PENTABLE: \$PENTBL\$



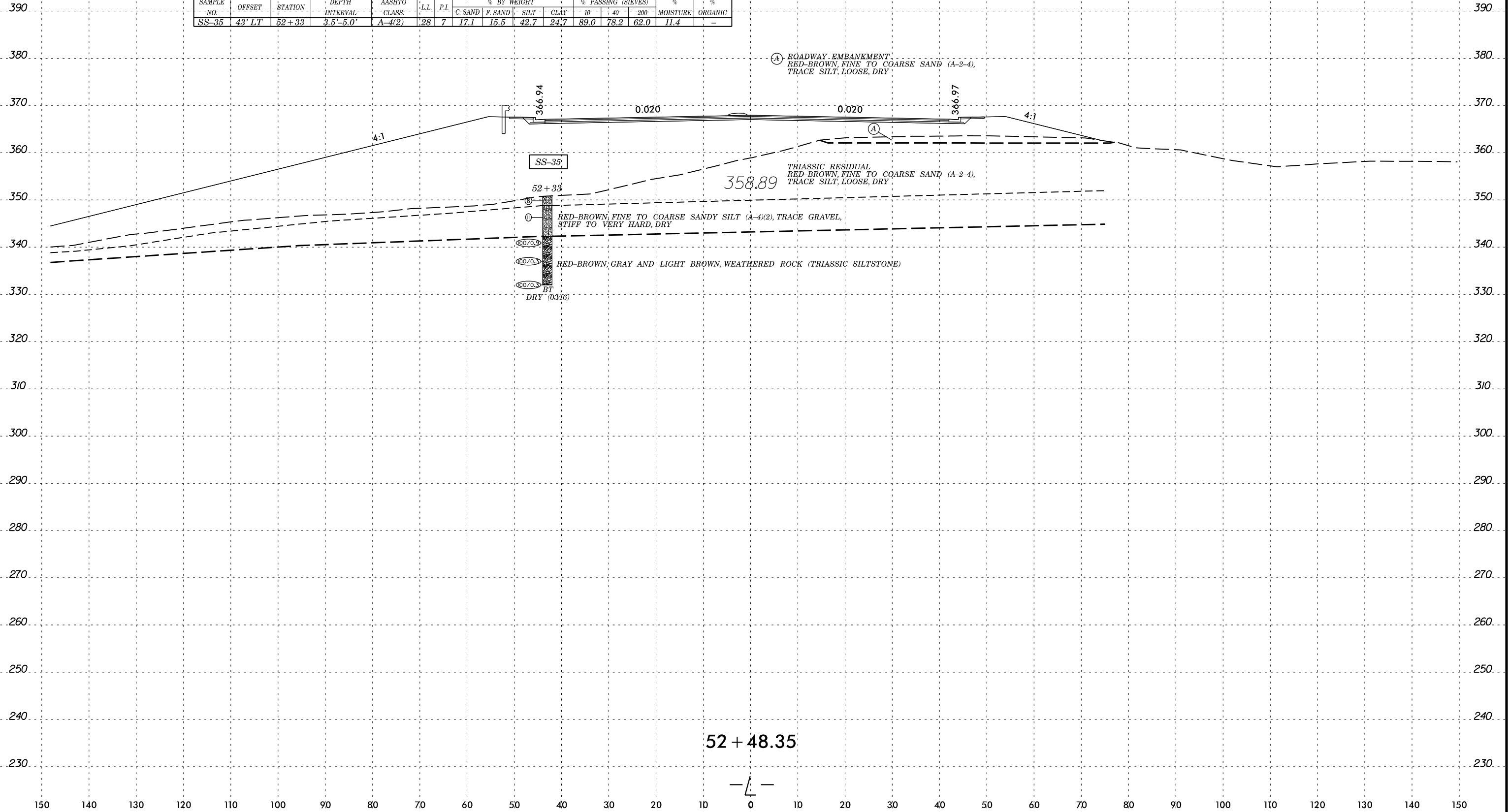


8/23/99



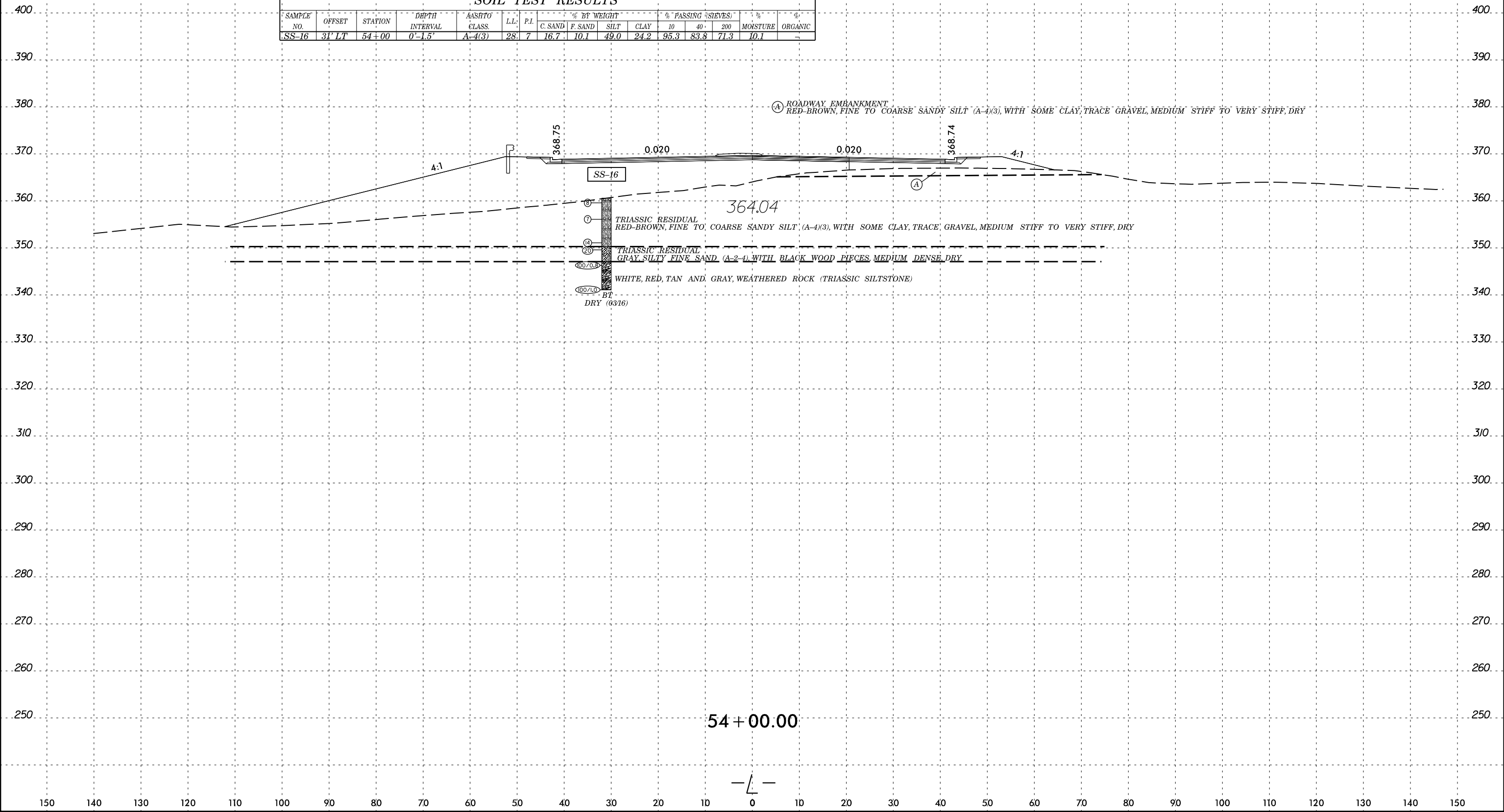
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 USER: \$USER\$
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 DATE: \$DATE\$
 TIME: \$TIME\$
 PENTABLE: \$PENTBL\$

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10'	40'	200'		
SS-35	43' LT	52+33	3.5'-5.0'	A-4(2)	28	7	17.1	15.5	42.7	24.7	89.0	78.2	62.0	11.4	-



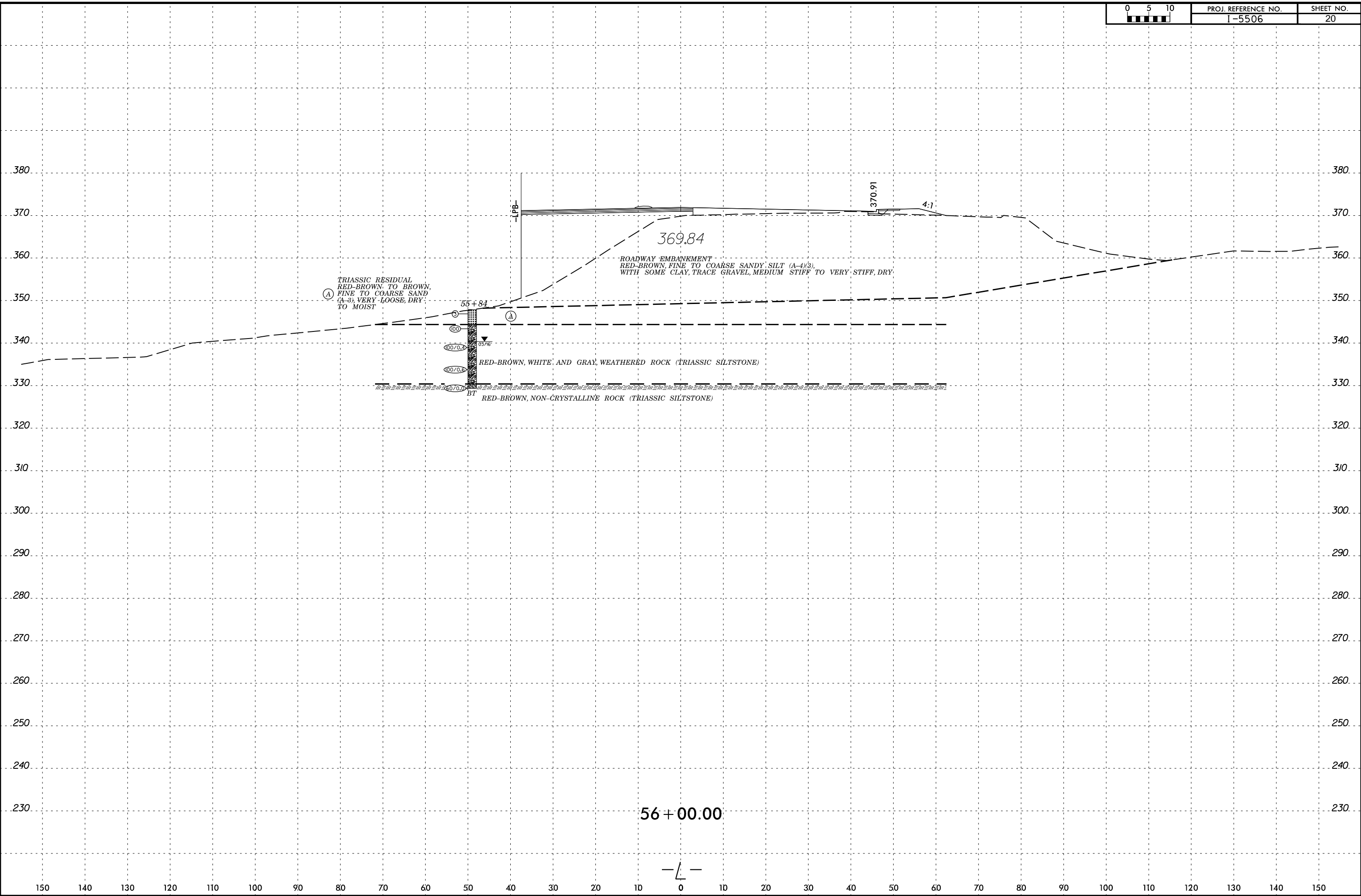
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 USER: \$USER\$
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 TIME: \$TIME\$
 PENTABLE: \$PENTBL\$

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE	ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-16	31' LT	54+00	0'-1.5'	A-4(3)	28	7	16.7	10.1	49.0	24.2	95.3	83.8	71.3	10.1	-

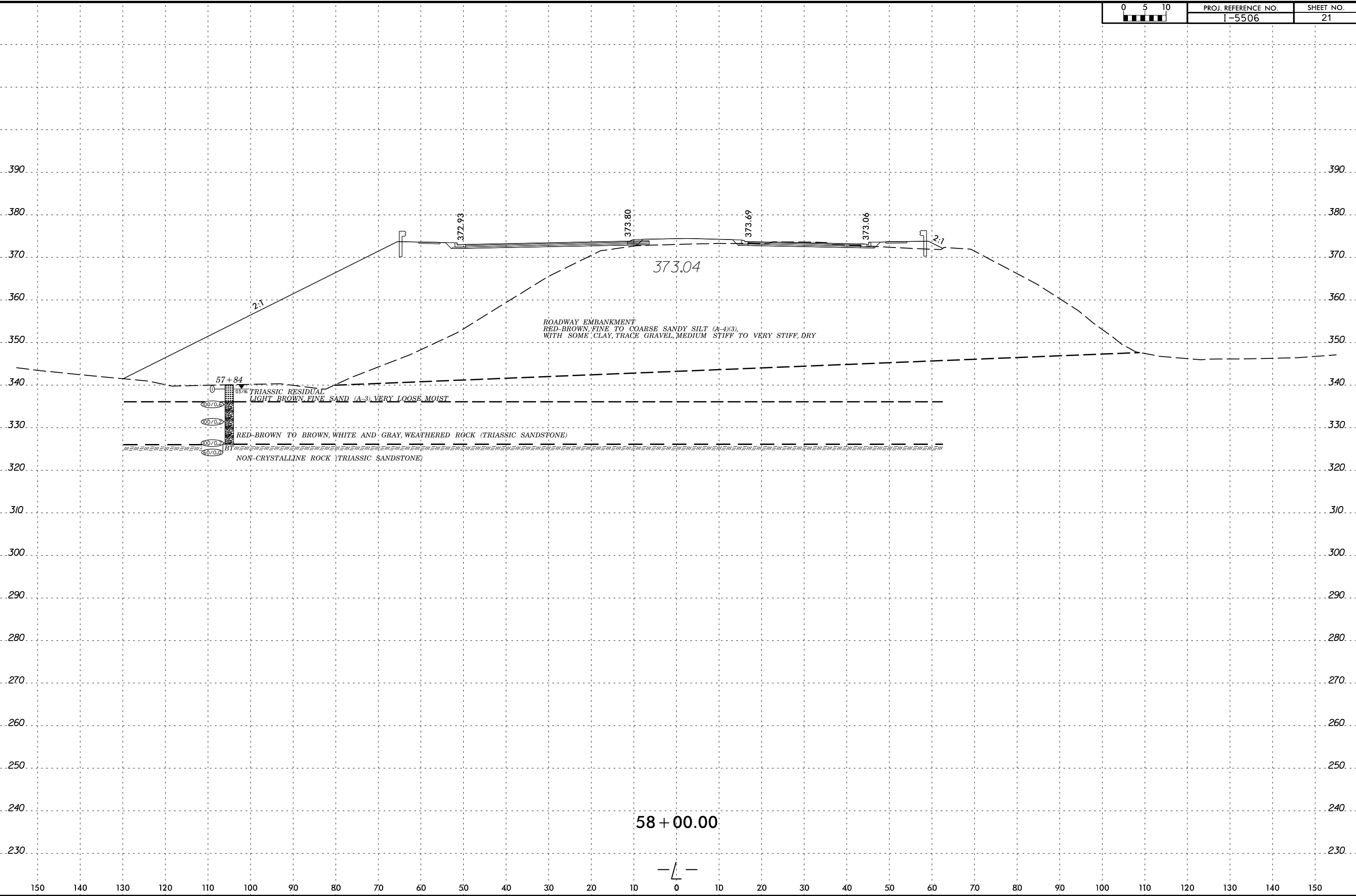


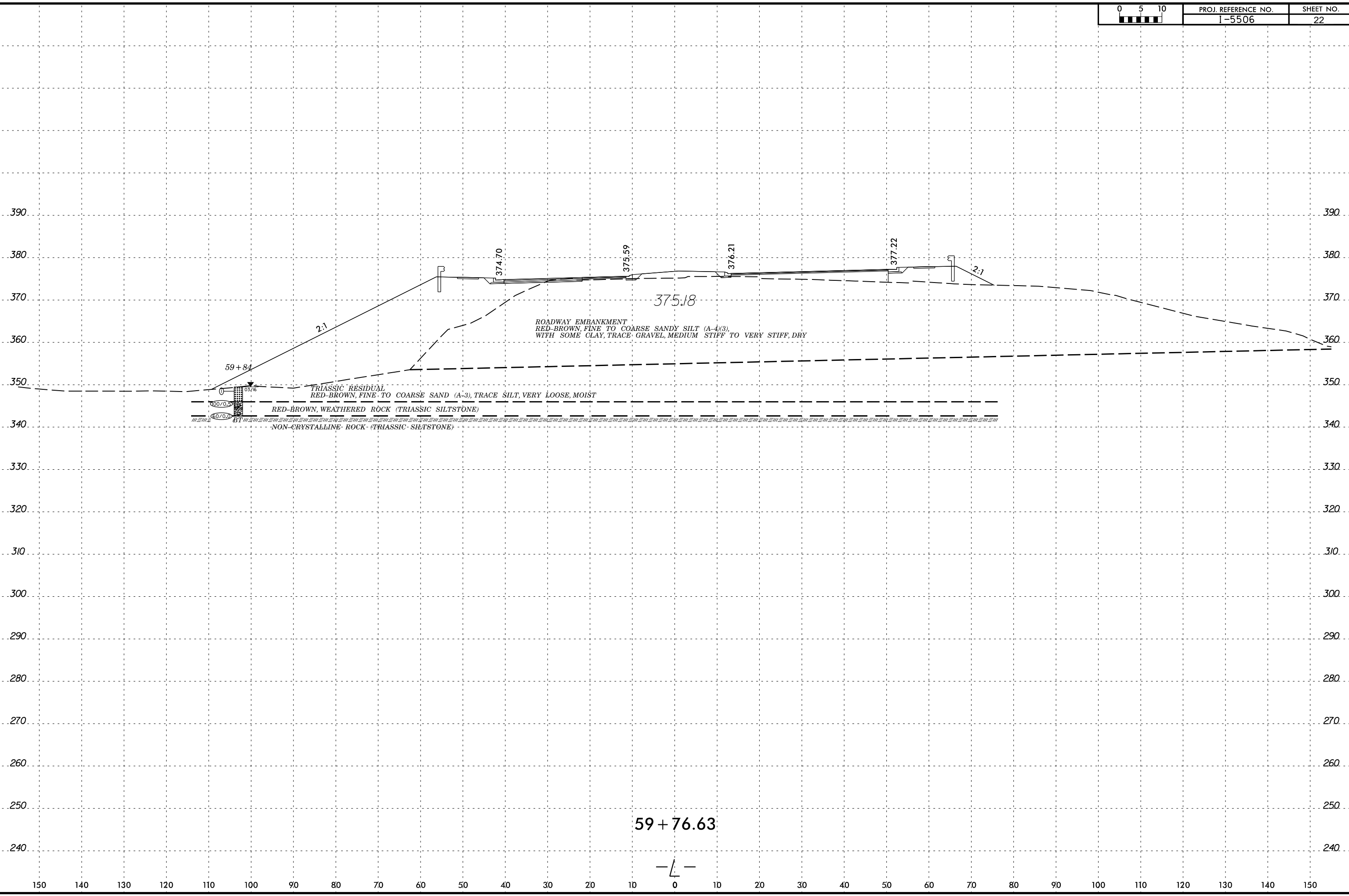
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 TIME: \$TIME\$
 PENTABLE: \$PENTBL\$

8/23/99



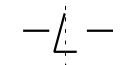
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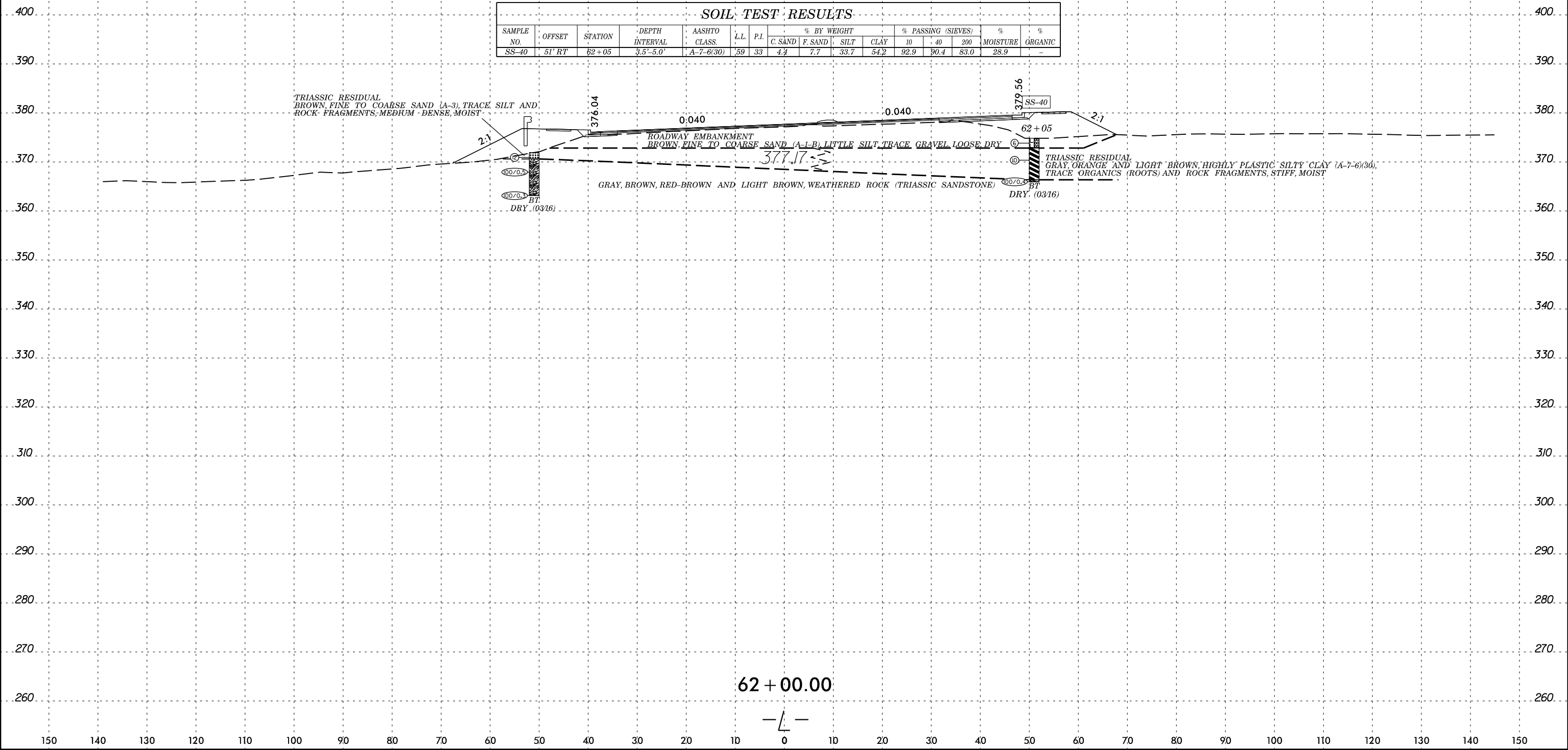


SECTION

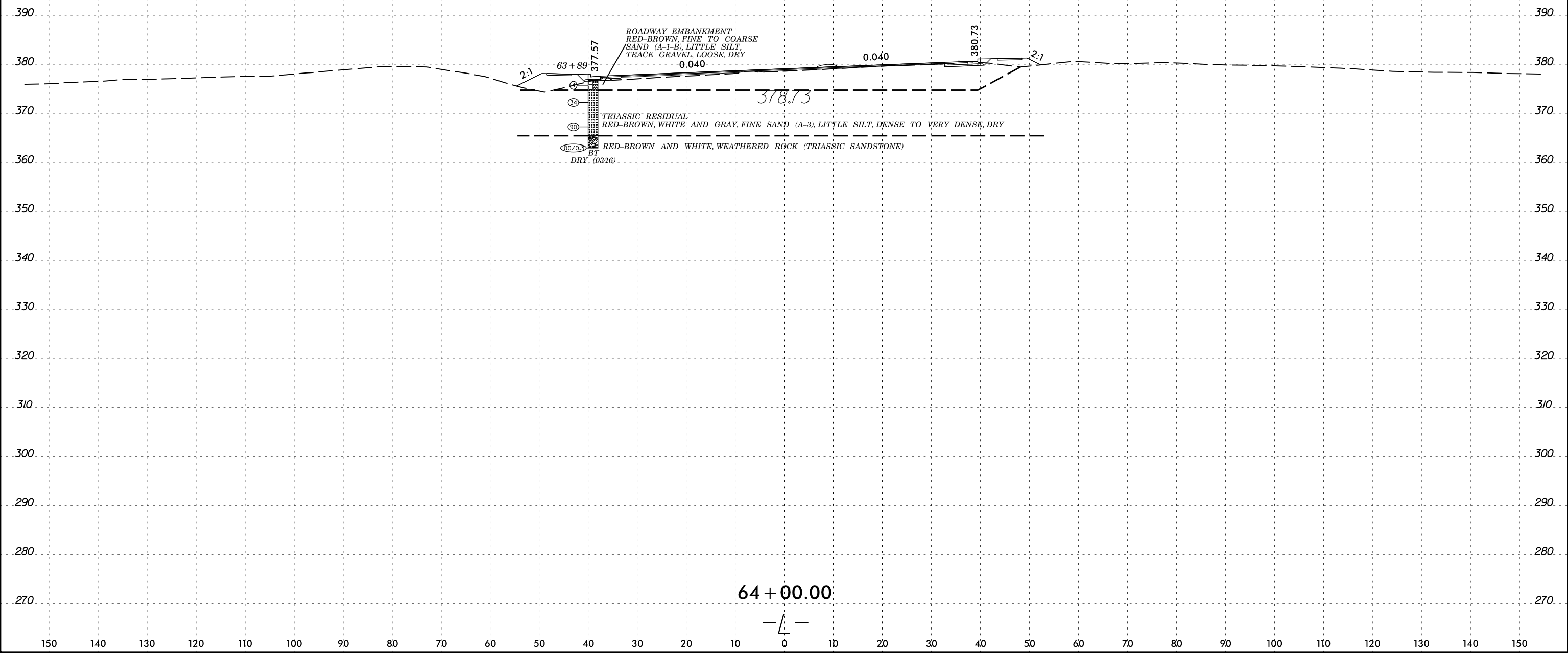
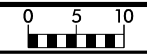
59 + 76.63

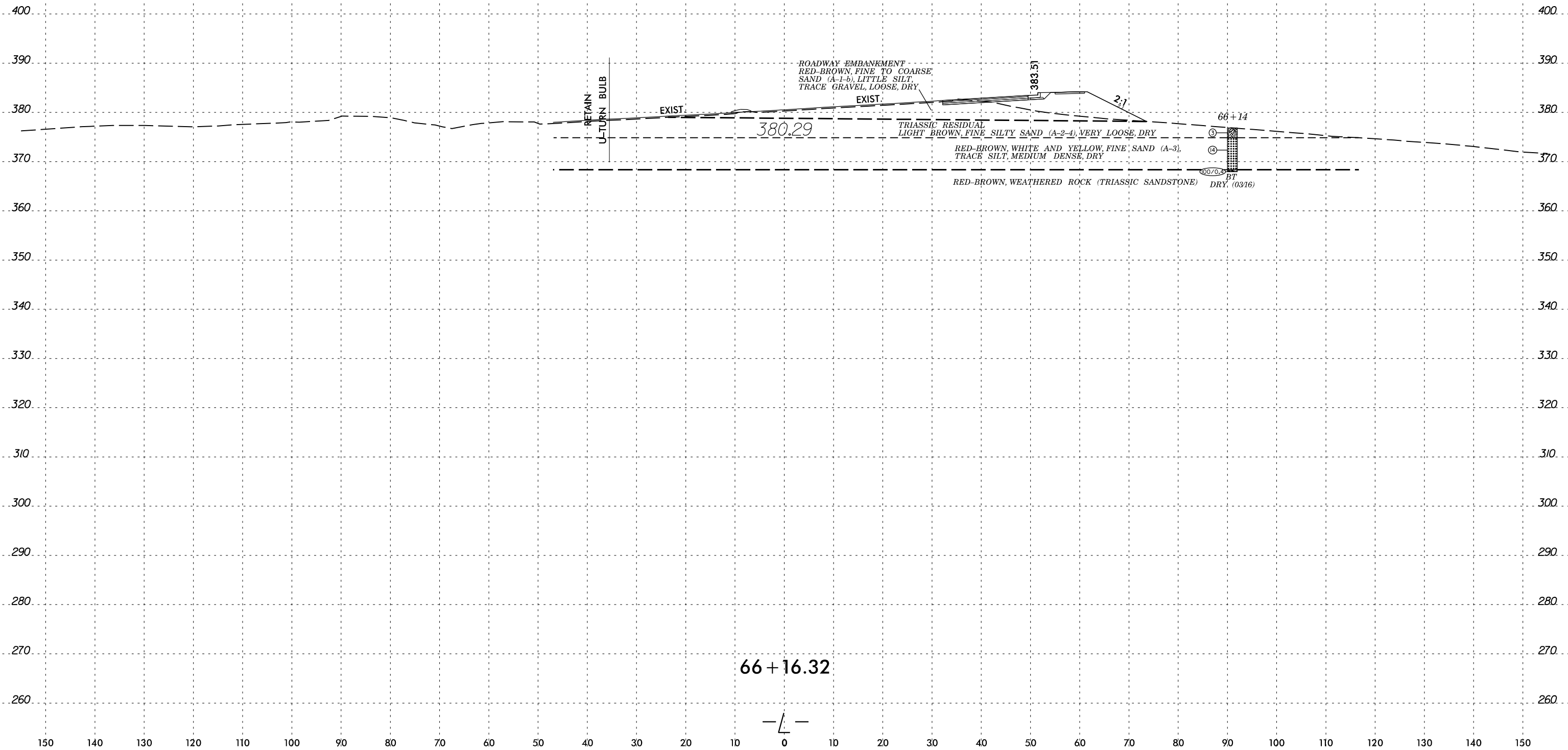


SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-40	51' RT	62+05	3.5'-5.0'	A-7-6(30)	59	33	4.4	7.7	33.7	54.2	92.9	90.4	83.0	28.9	-



8/23/99





RETAIN
U-TURN BULB

EXIST.

380.29

ROADWAY EMBANKMENT
RED-BROWN, FINE TO COARSE,
SAND (A-1-b), LITTLE SILT,
TRACE GRAVEL, LOOSE, DRY

EXIST.

TRIASSIC RESIDUAL
LIGHT BROWN, FINE SILTY SAND (A-2-4), VERY LOOSE, DRY

RED-BROWN, WHITE AND YELLOW, FINE SAND (A-3),
TRACE SILT, MEDIUM DENSE, DRY

RED-BROWN, WEATHERED ROCK (TRIASSIC SANDSTONE) DRY (0316)

383.51

2:1

66+14

③

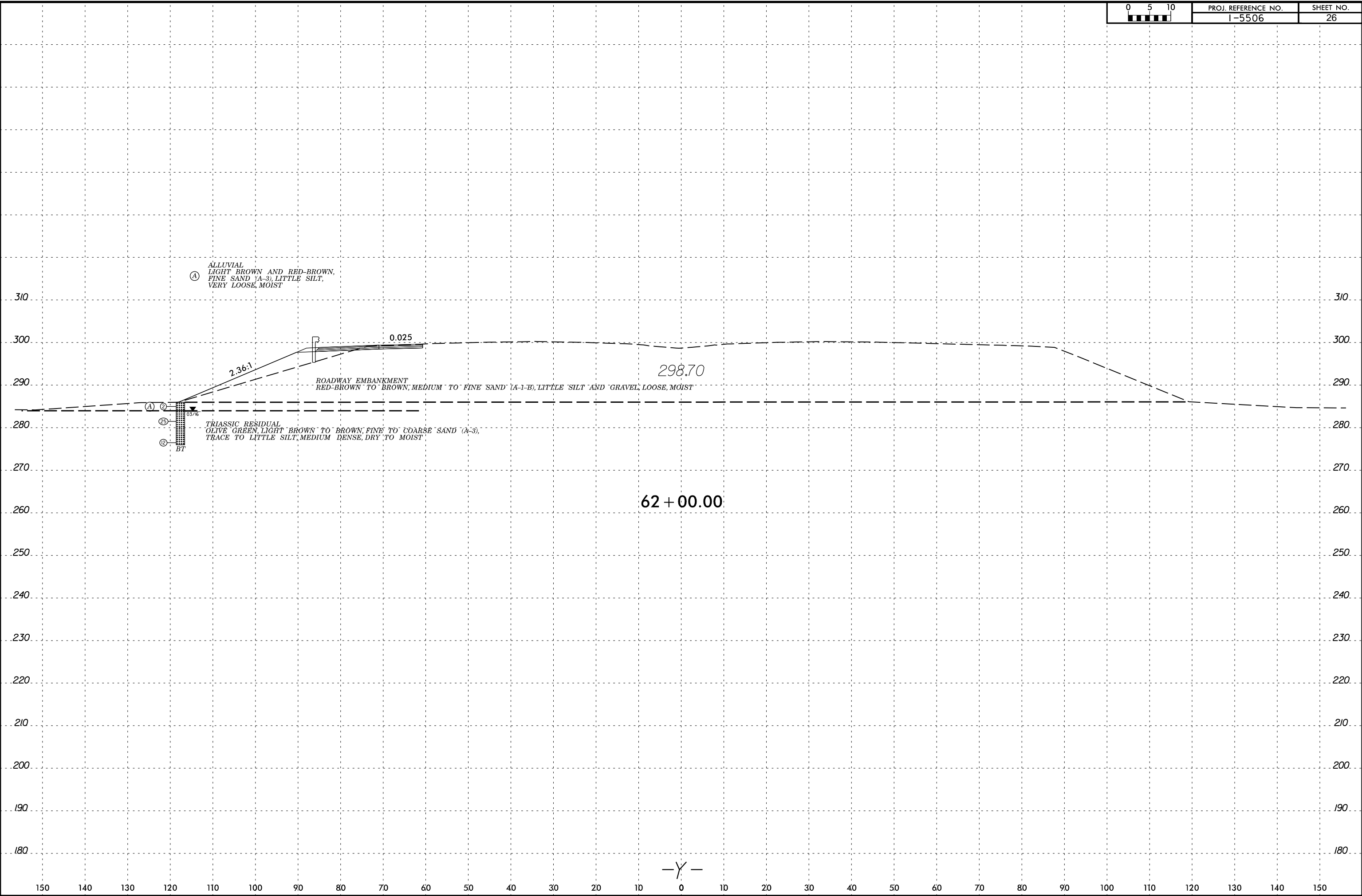
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BT

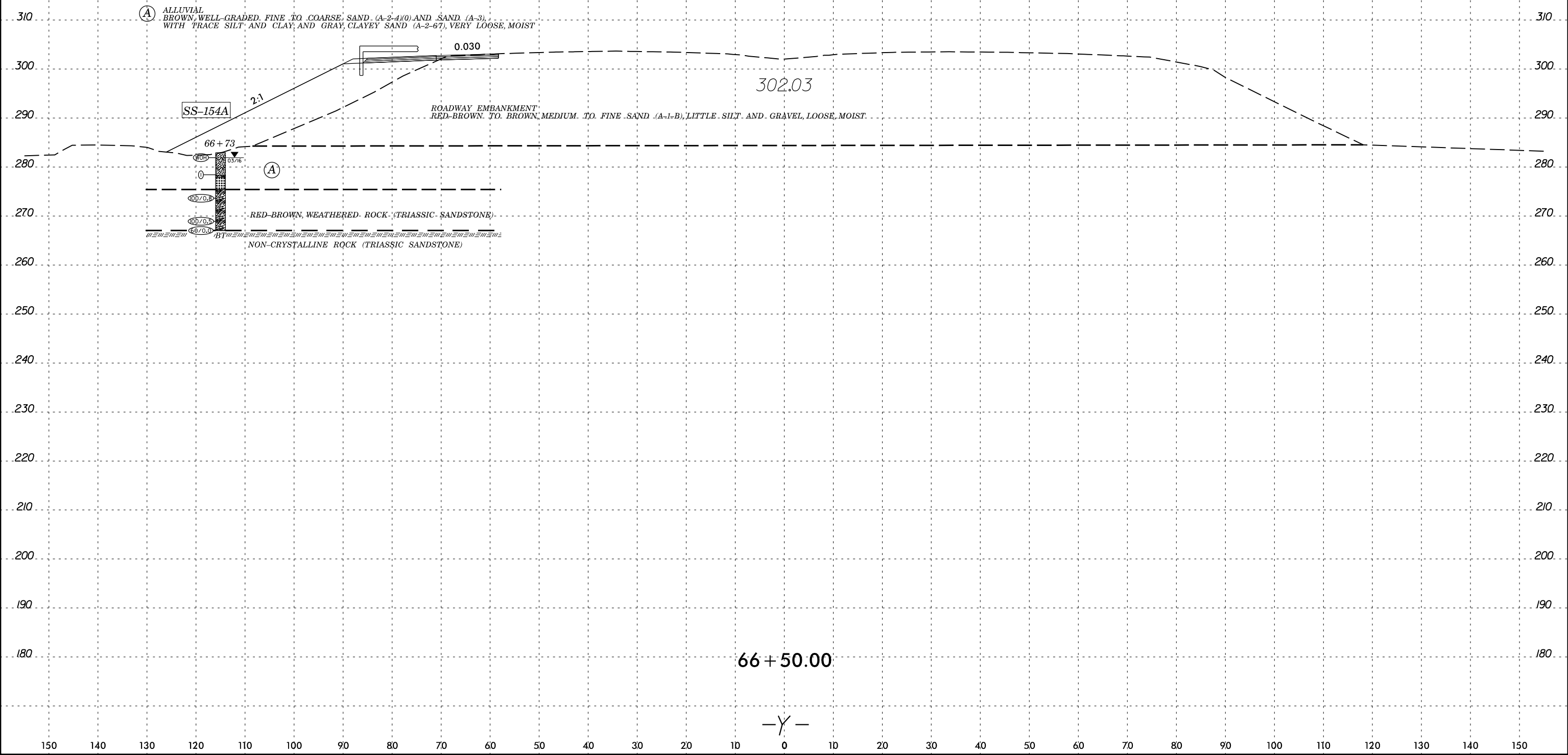
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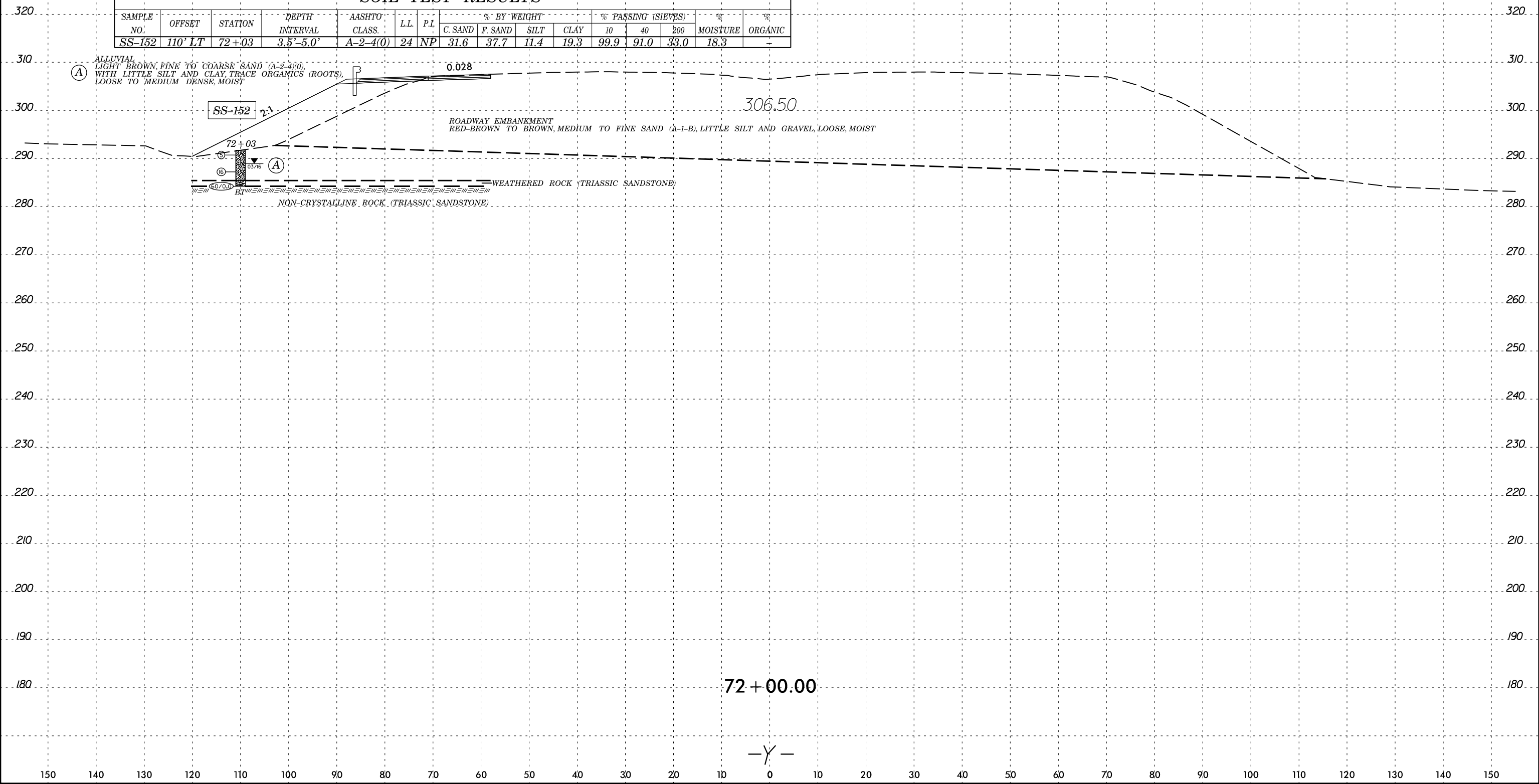
SECTION CONDITION
AS SHOWN
UNLESS NOTED OTHERWISE



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-154A	115' LT	66+73	3.5'-5.0'	A-2-4(0)	16	NP	73.9	12.1	8.4	5.6	98.2	45.3	14.4	25.7	



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.L.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-152	110' LT	72+03	3.5'-5.0'	A-2-4(0)	24	NP	31.6	37.7	11.4	19.3	99.9	91.0	33.0	18.3	-



PLOT DRIVER: \$PLTDVRS\$
 USER: \$USER\$
 FILE: \$PWVAVULTPATHDESC\$
 DATE: \$DATE\$
 TIME: \$TIME\$
 PENTABLE: \$PENTBL\$

72 + 00.00

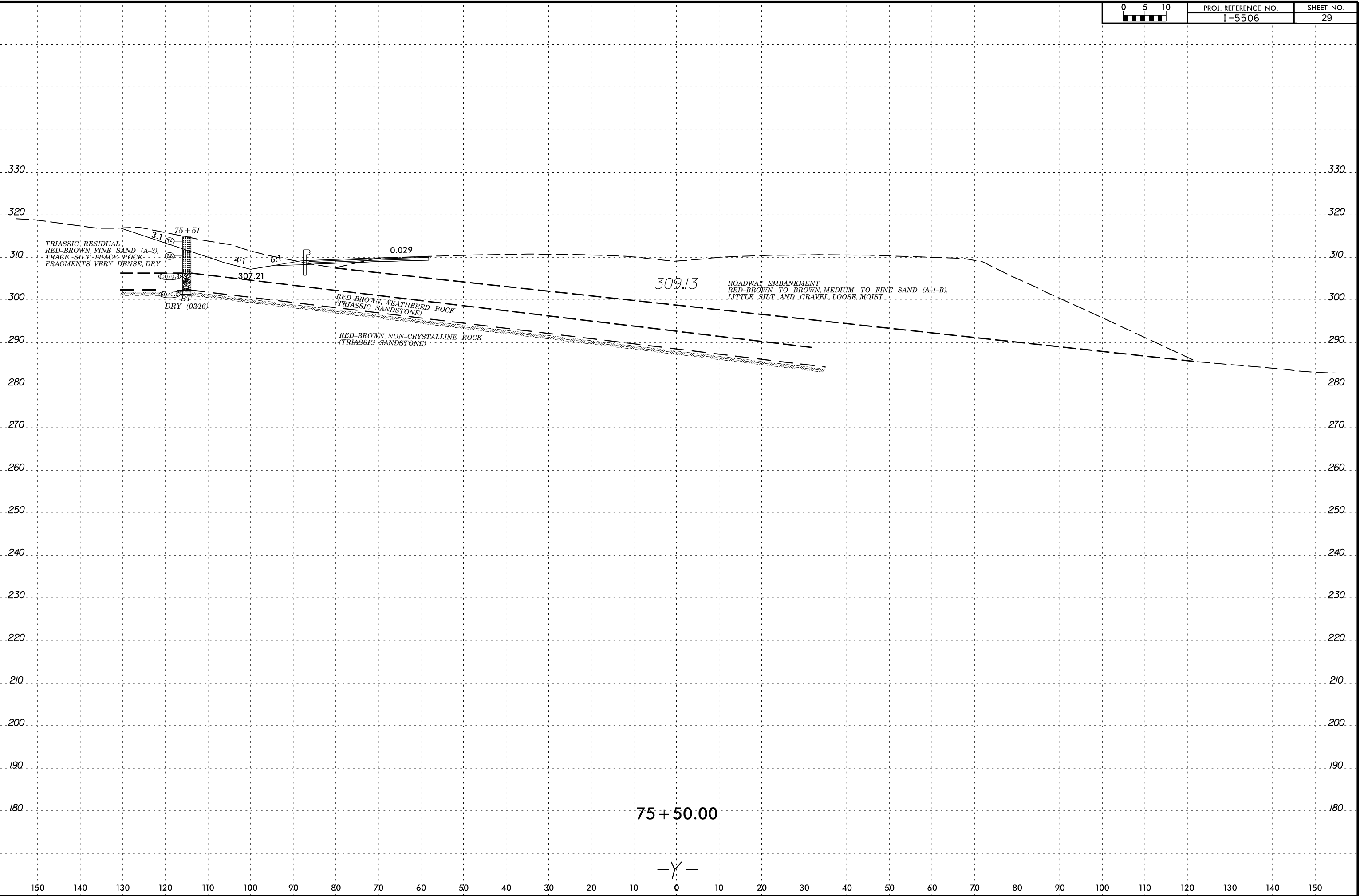
-Y-

8/23/99



PROJ. REFERENCE NO.
I-5506

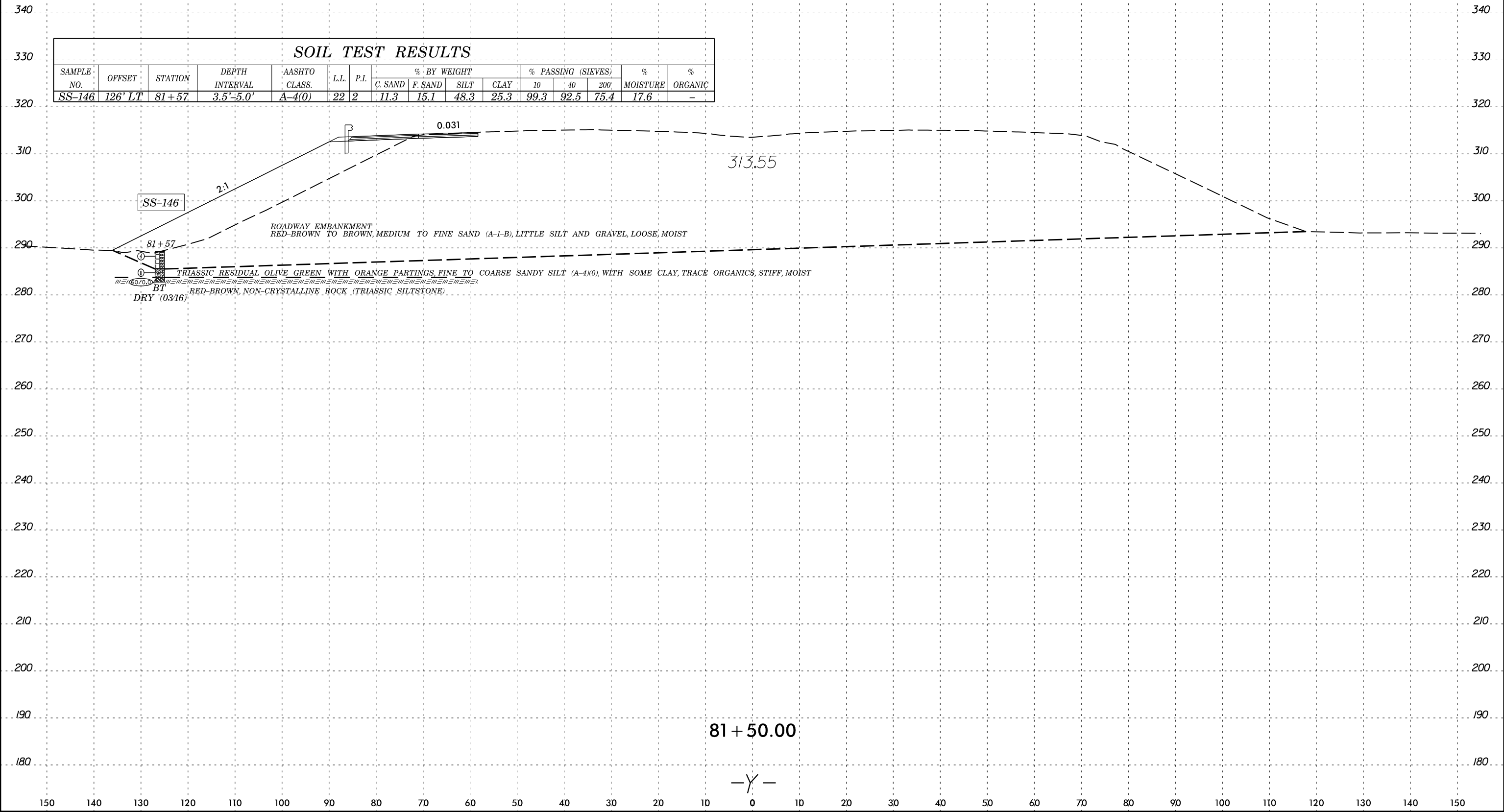
SHEET NO.
29



PLOT DRIVER: \$PLTDVRS\$
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DATE: \$DATE\$
TIME: \$TIME\$
PENTABLE: \$PENTBL\$

SOIL TEST RESULTS

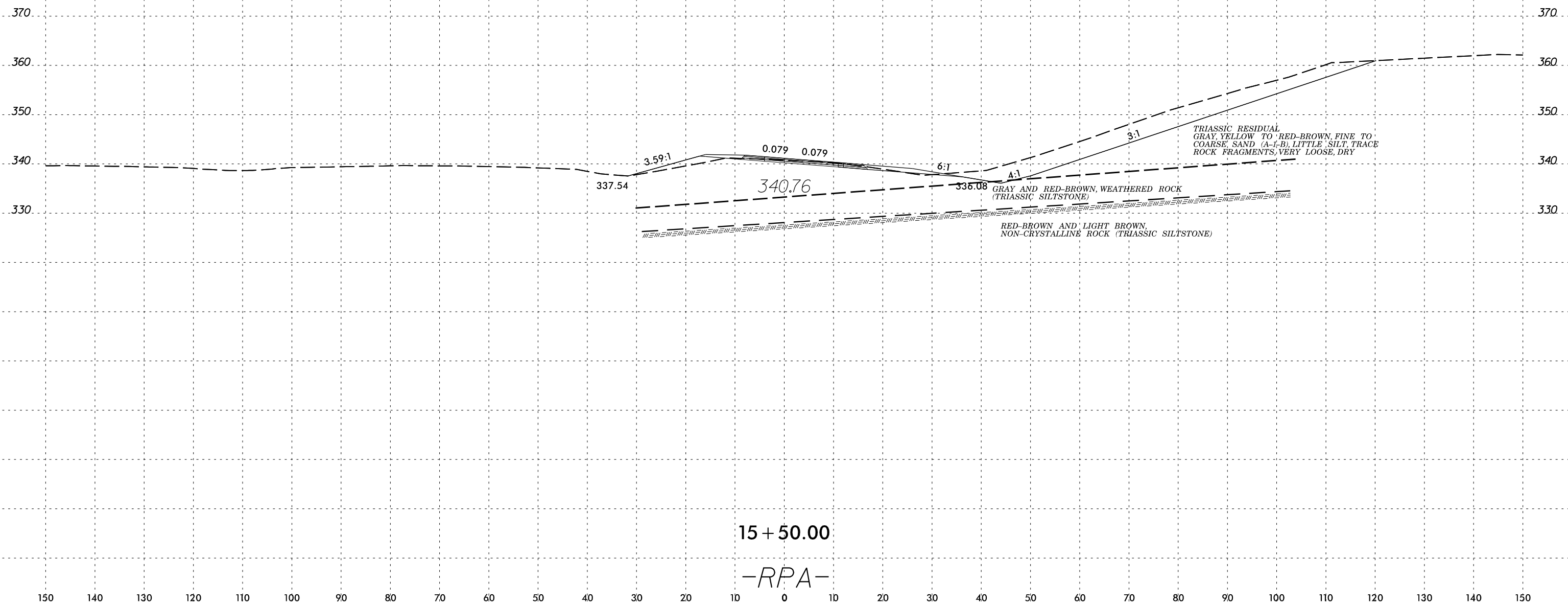
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-146	126' LT	81+57	3.5'-5.0'	A-4(0)	22	2	11.3	15.1	48.3	25.3	99.3	92.5	75.4	17.6	-



PLOT DRIVER: \$PLTRV\$
 USER: \$USER\$
 FILE: \$PWARYAULTPATHDESC\$
 DATE: \$DATE\$
 TIME: \$TIME\$
 PENTABLE: \$PENTBL\$

81 + 50.00

-Y-



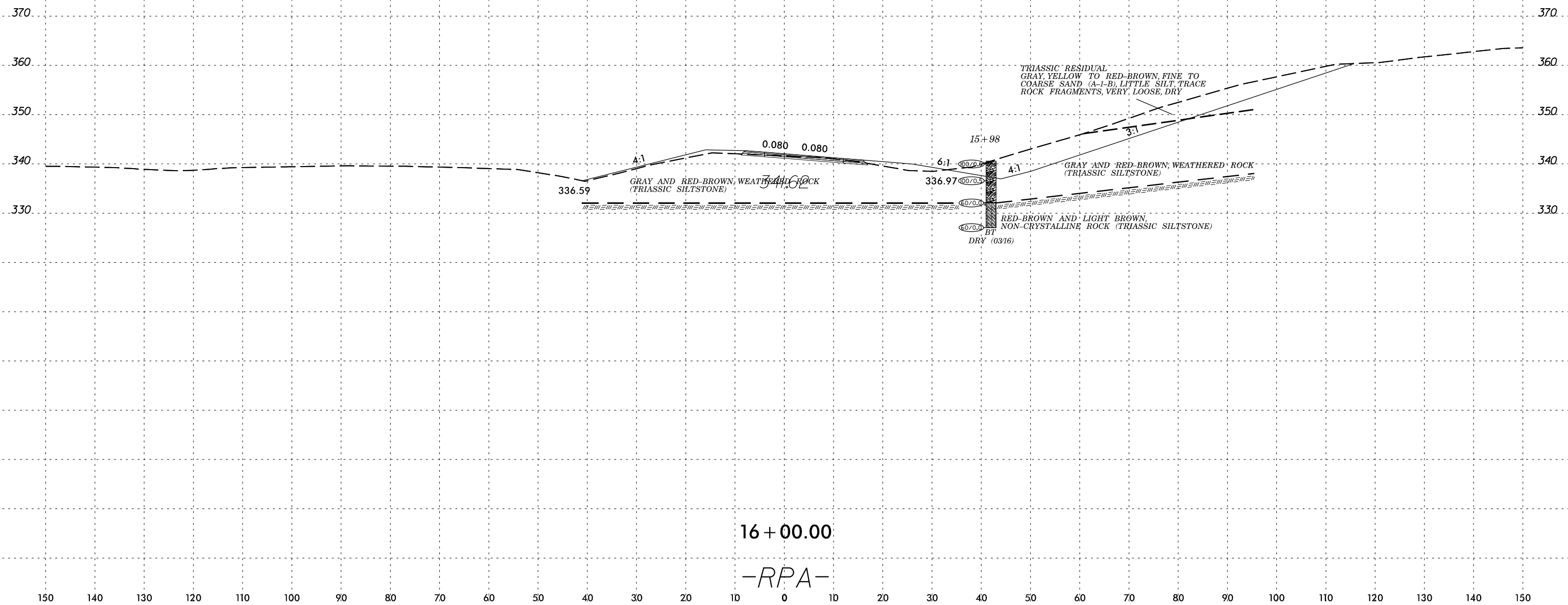
SECTION 15+50.00
RPA
8/23/99

15 + 50.00
-RPA-

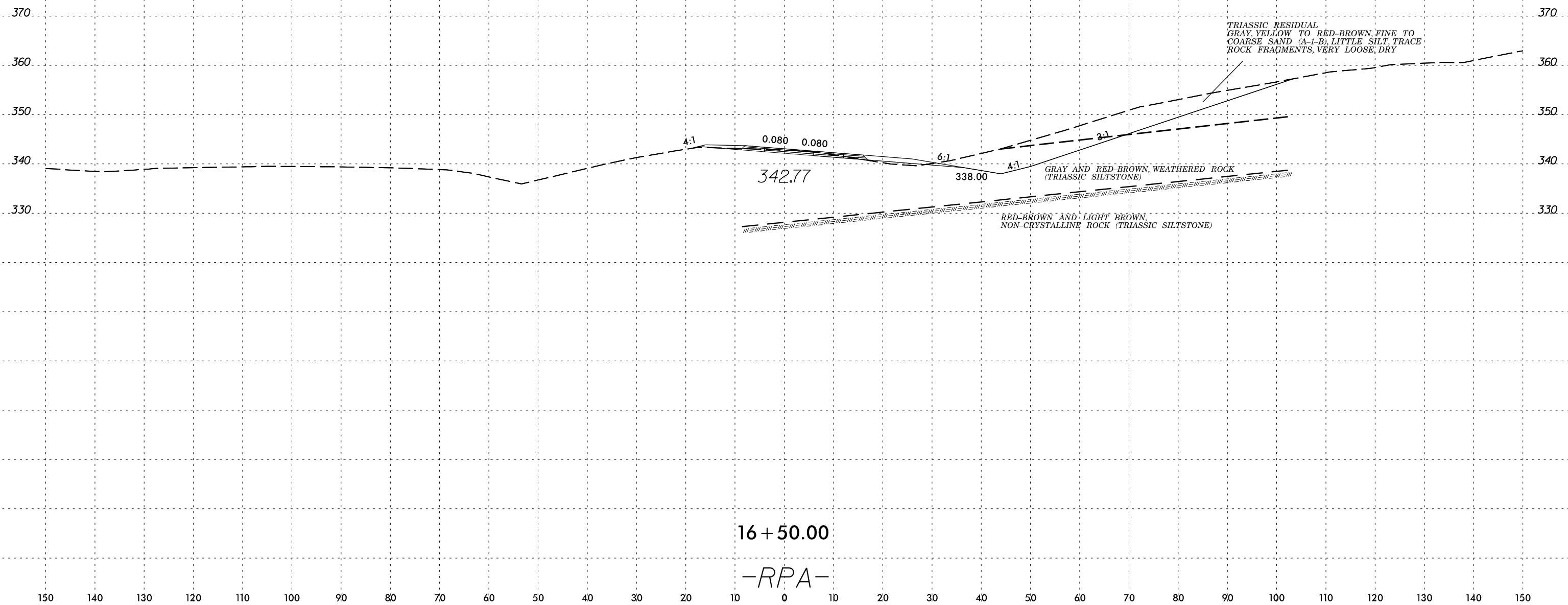
TRIASSIC RESIDUAL
GRAY, YELLOW TO RED-BROWN, FINE TO
COARSE SAND (A-F-B), LITTLE SILT, TRACE
ROCK FRAGMENTS, VERY LOOSE, DRY

GRAY AND RED-BROWN, WEATHERED ROCK
(TRIASSIC SILTSTONE)

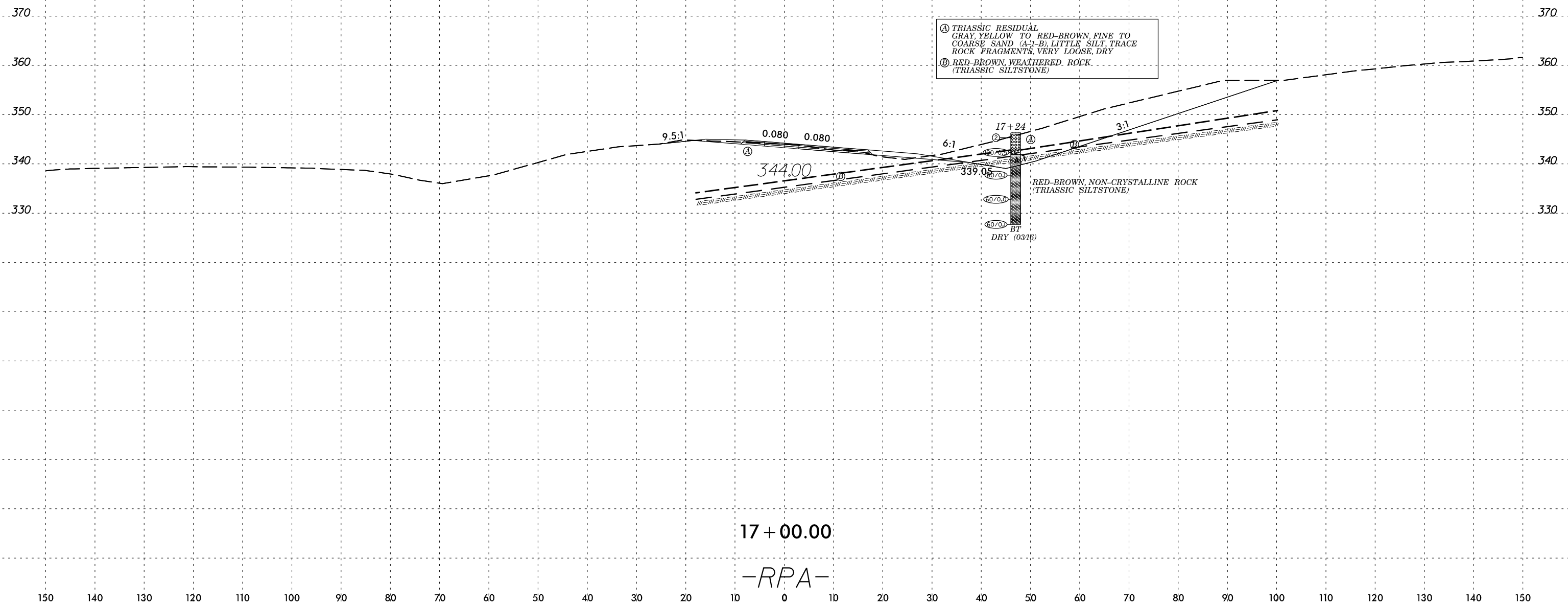
RED-BROWN AND LIGHT BROWN,
NON-CRYSTALLINE ROCK (TRIASSIC SILTSTONE)

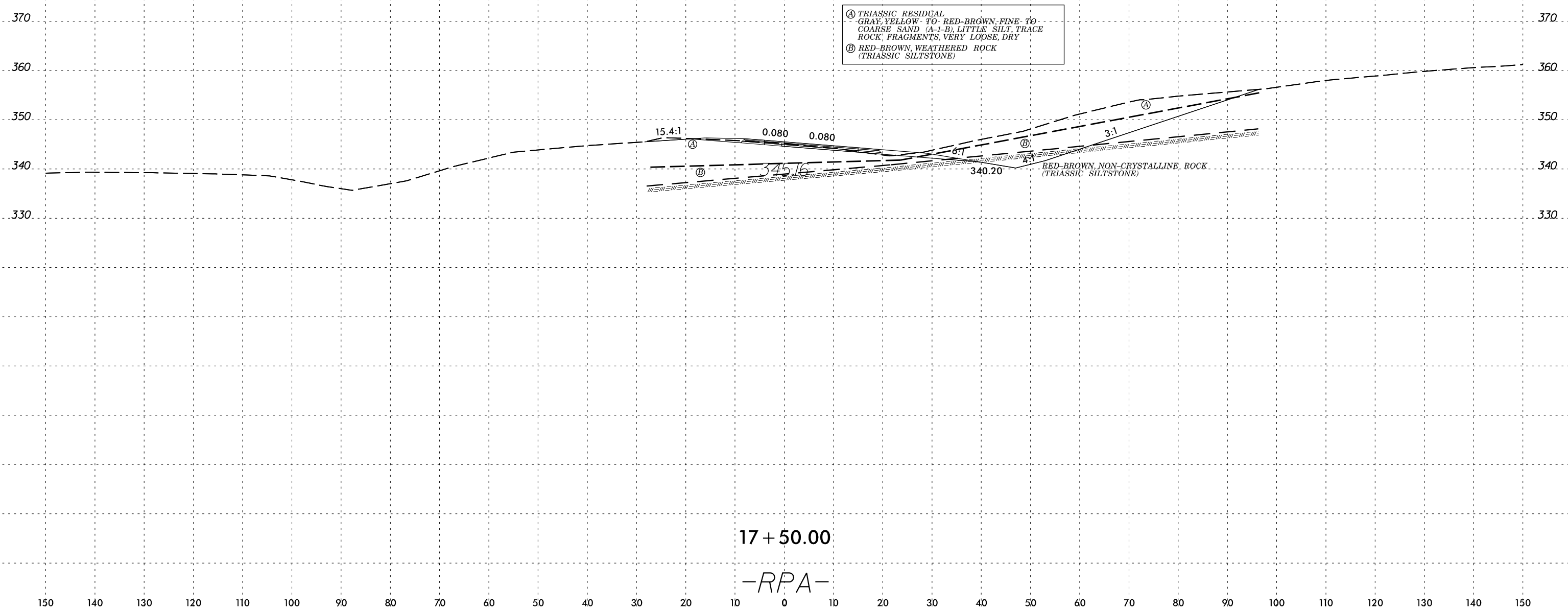


SECTION 16+00.00



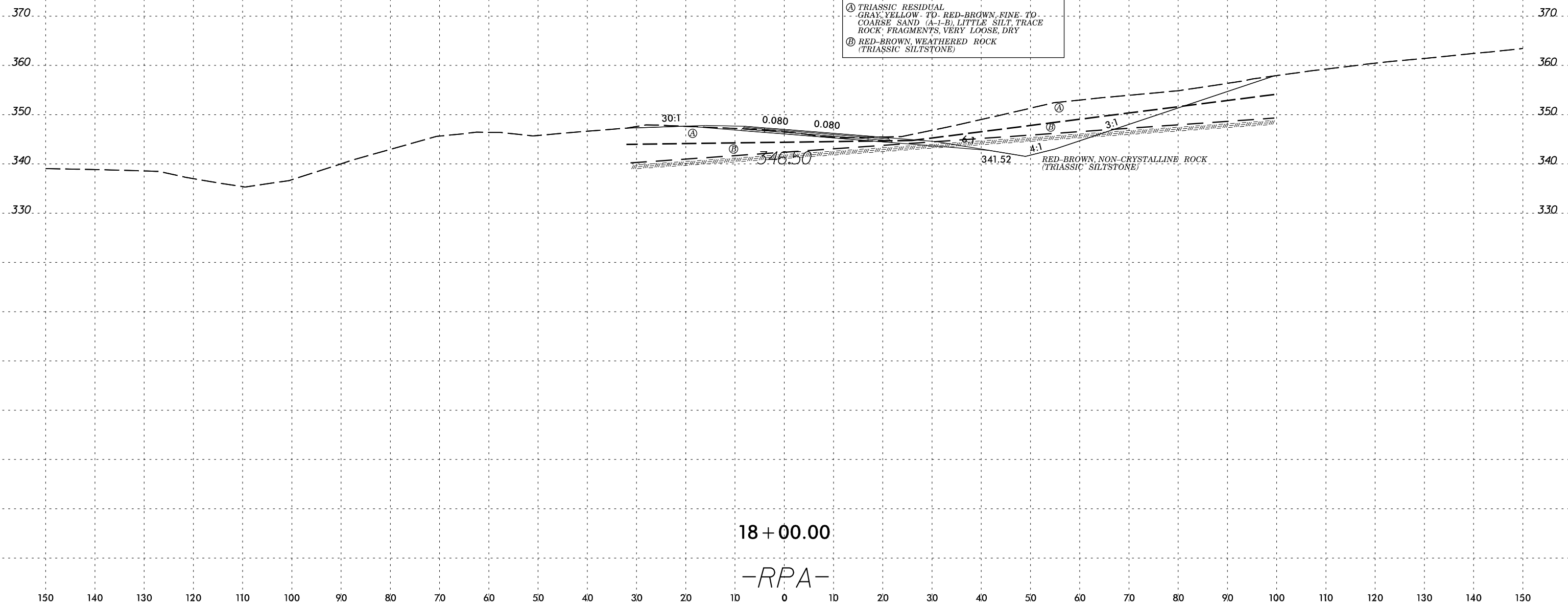
SECTION 16+50.00
CONSTRUCTION
PLAN
DATE
DRAWN
CHECKED
APPROVED



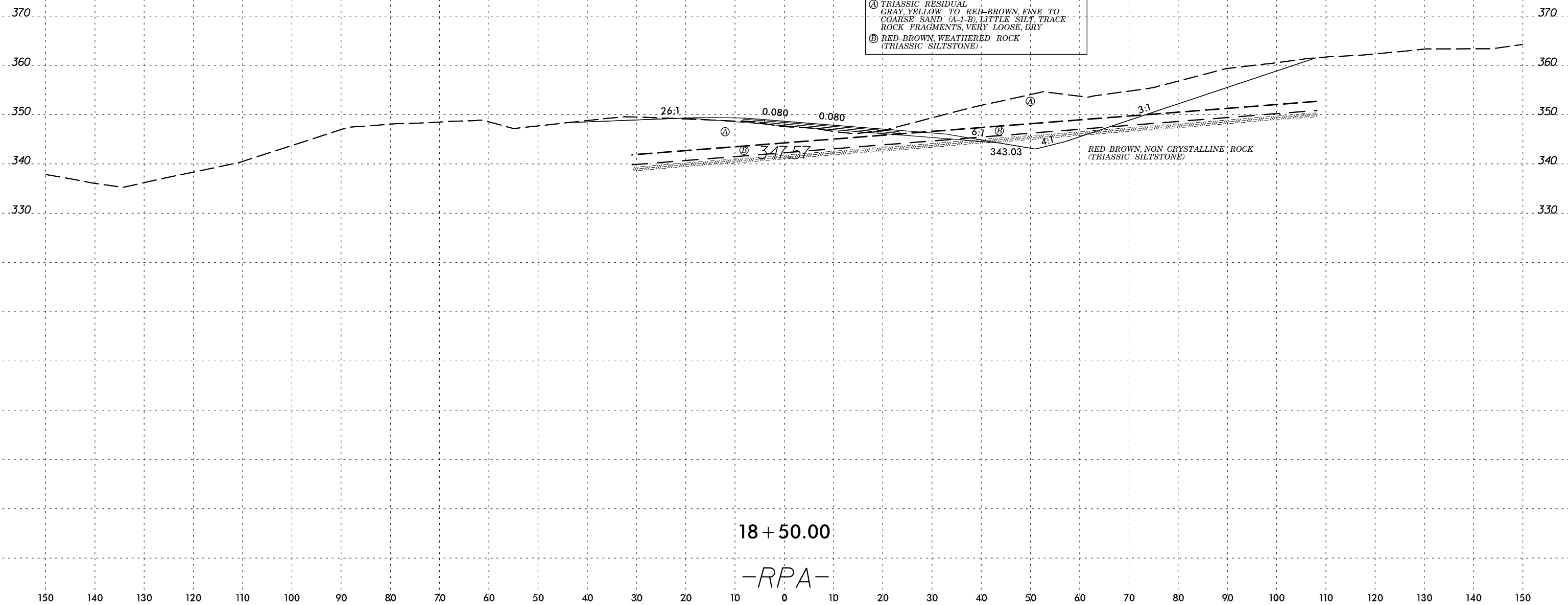


17 + 50.00
-RPA-

SECTION 17+50.00
 CONSTRUCTION
 DATE: 8/23/99



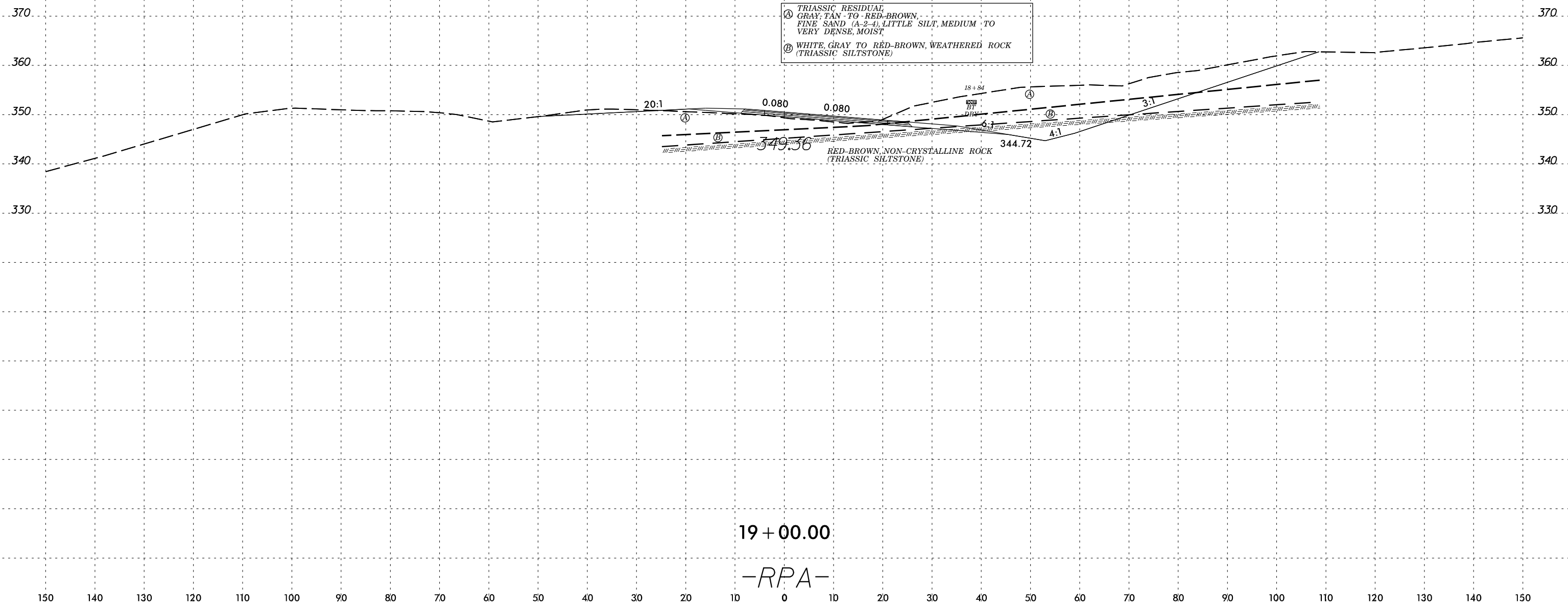
SECTION 18+00.00
CONSTRUCTION
PLAN
DATE
BY
CHECKED
APPROVED

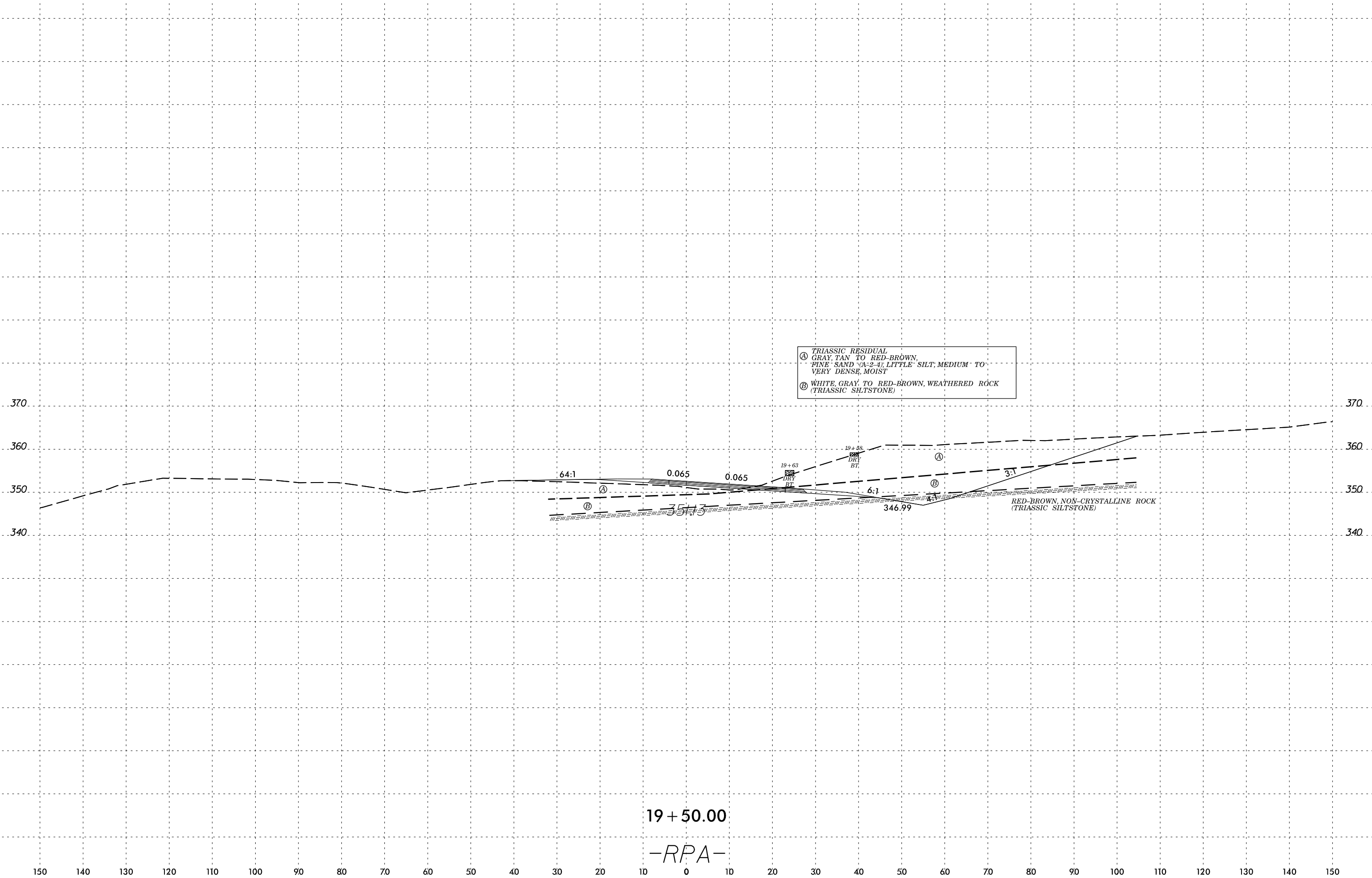


18 + 50.00

-RPA-

ELEVATION



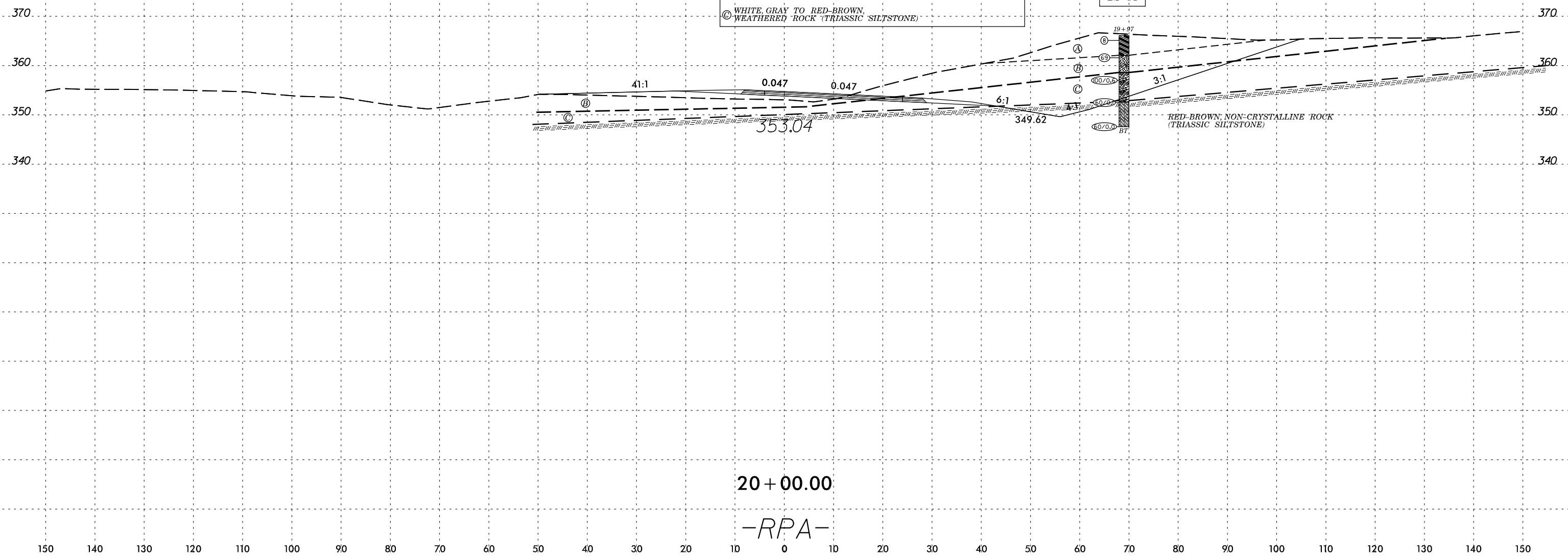


SECTION
 19+50.00
 RPA
 8/23/99

19 + 50.00
 -RPA-

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
BS-#1	69' RT	19+97	2.0'-4.0'	A-6(15)	39	16	5.9	5.7	41.1	47.3	97.6	93.7	87.2	14.9	-
SS-64	69' RT	19+97	8.5'-10.0'	A-4(0)	22	6	28.4	20.3	30.6	20.7	90.8	74.8	48.4	3.7	-

- TRIASSIC RESIDUAL
- Ⓐ ORANGE, LIGHT BROWN AND GRAY, HIGHLY SILTY CLAY (A-6(15), WITH LITTLE SAND, TRACE ORGANICS (ROOTS), STIFF, DRY
 - Ⓑ GRAY, TAN TO RED-BROWN, FINE SAND (A-2-4), LITTLE SILT, VERY DENSE, MOIST
 - Ⓒ WHITE, GRAY TO RED-BROWN, WEATHERED ROCK (TRIASSIC SILTSTONE)

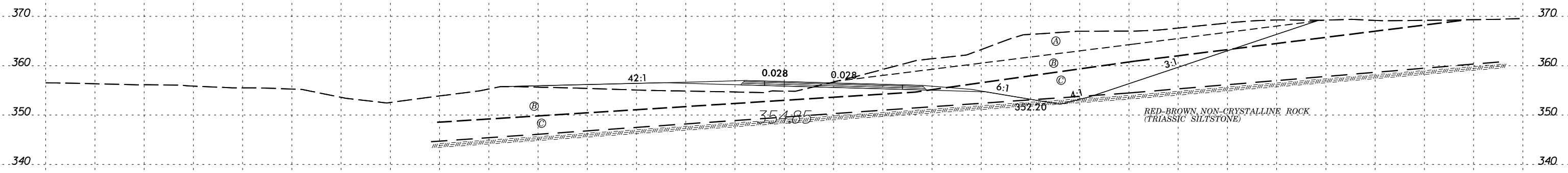


20 + 00.00

-RPA-

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- Ⓐ TRIASSIC RESIDUAL
ORANGE, LIGHT BROWN AND GRAY, HIGHLY SILTY CLAY, (A-6)(15),
WITH LITTLE SAND, TRACE ORGANICS (ROOTS), STIFF, DRY
- Ⓑ GRAY, TAN TO RED-BROWN, FINE SAND (A-2-4),
LITTLE SILT, VERY DENSE, MOIST
- Ⓒ WHITE, GRAY TO RED-BROWN,
WEATHERED ROCK (TRIASSIC SILTSTONE)



20 + 50.00

-RPA-

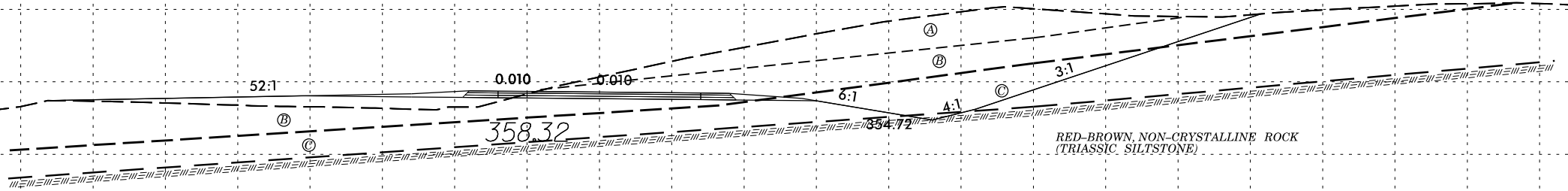
SYNTHETIC
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150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

380
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TRIASSIC RESIDUAL
 (A) ORANGE, LIGHT BROWN AND GRAY, HIGHLY SILTY CLAY (A-6/15),
 WITH LITTLE SAND, TRACE ORGANICS (ROOTS), STIFF, DRY
 (B) GRAY, TAN TO RED-BROWN, FINE SAND (A-2-4),
 LITTLE SILT, VERY DENSE, MOIST
 (C) WHITE, GRAY TO RED-BROWN,
 WEATHERED ROCK (TRIASSIC SILTSTONE)



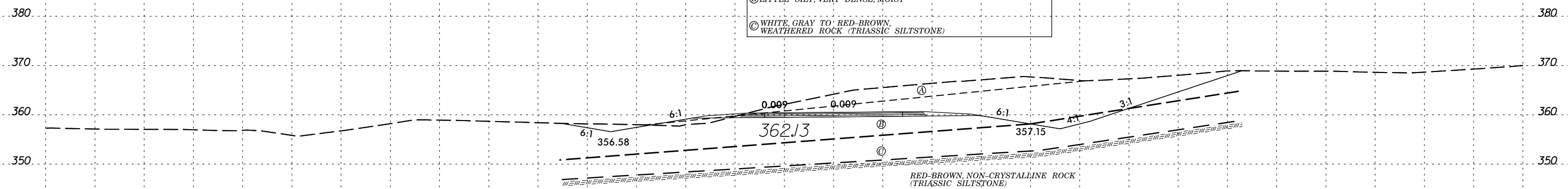
21+00.00

-RPA-

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

SECTION 21+00.00
 150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

TRIASSIC RESIDUAL
 (A) ORANGE, LIGHT BROWN AND GRAY, HIGHLY SILTY CLAY (A-6)(15),
 WITH LITTLE SAND, TRACE ORGANICS (ROOTS), STIFF, DRY
 GRAY, TAN TO RED-BROWN, FINE SAND (A-2-4),
 (B) LITTLE SILT, VERY DENSE, MOIST
 (C) WHITE, GRAY TO RED-BROWN,
 WEATHERED ROCK (TRIASSIC SILTSTONE)



RED-BROWN, NON-CRYSTALLINE ROCK
(TRIASSIC SILTSTONE)

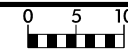
21+50.00

-RPA-

SECTION 6
 FROM
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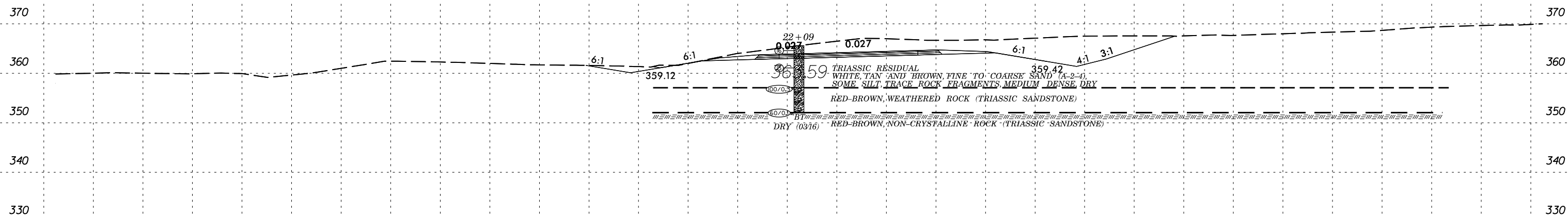
150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

8/23/99



PROJ. REFERENCE NO.
I-5506

SHEET NO.
44



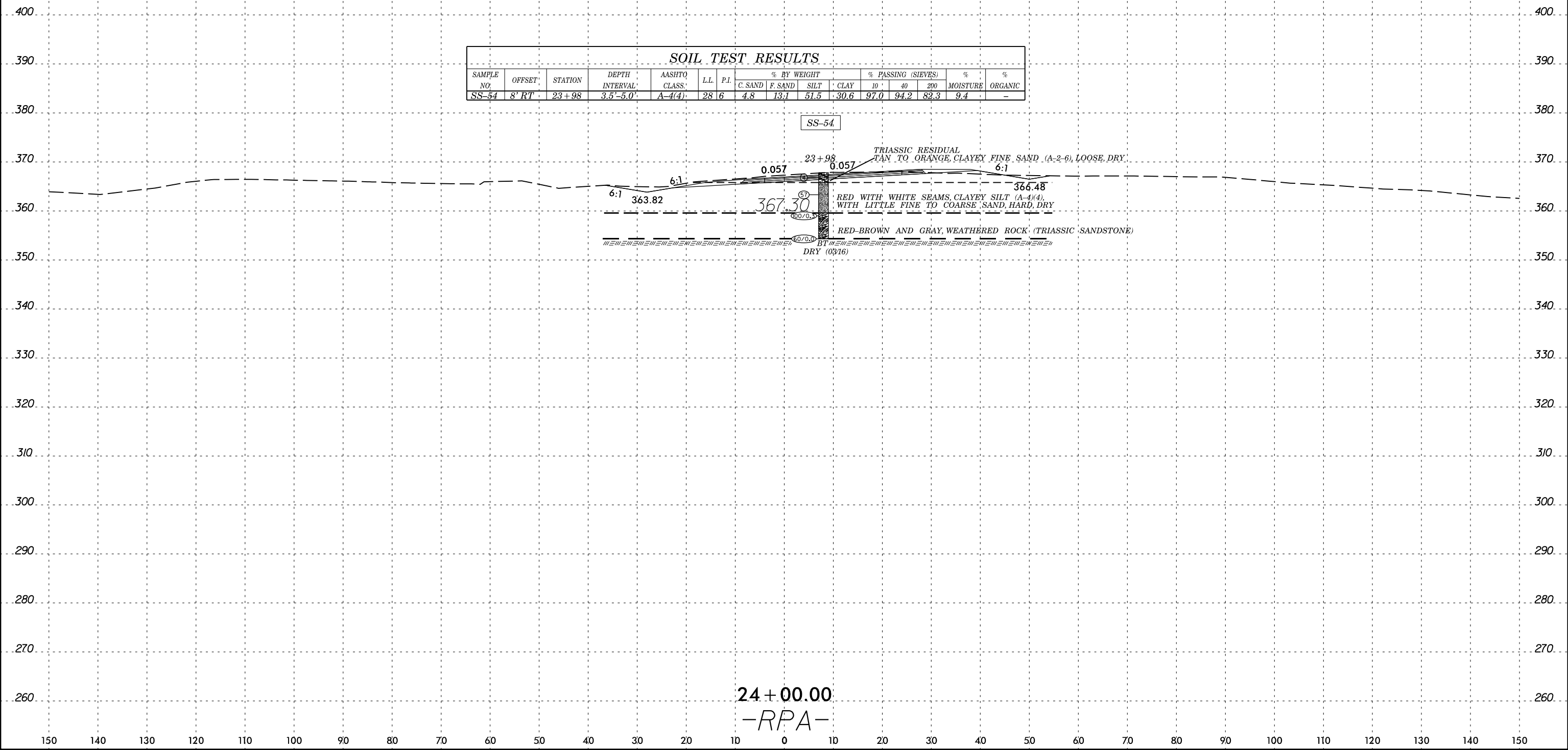
22+09
 0.027
 0.027
 359.12
 359.42
 6:1
 4:1
 3:1
 TRIASSIC RESIDUAL
 WHITE, TAN AND BROWN, FINE TO COARSE SAND (A-2-4)
 SOME SILT, TRACE ROCK FRAGMENTS, MEDIUM DENSE, DRY
 RED-BROWN, WEATHERED ROCK (TRIASSIC SANDSTONE)
 DRY (0316) RED-BROWN, NON-CRYSTALLINE ROCK (TRIASSIC SANDSTONE)

22 + 00.00
-RPA-

SYTIME
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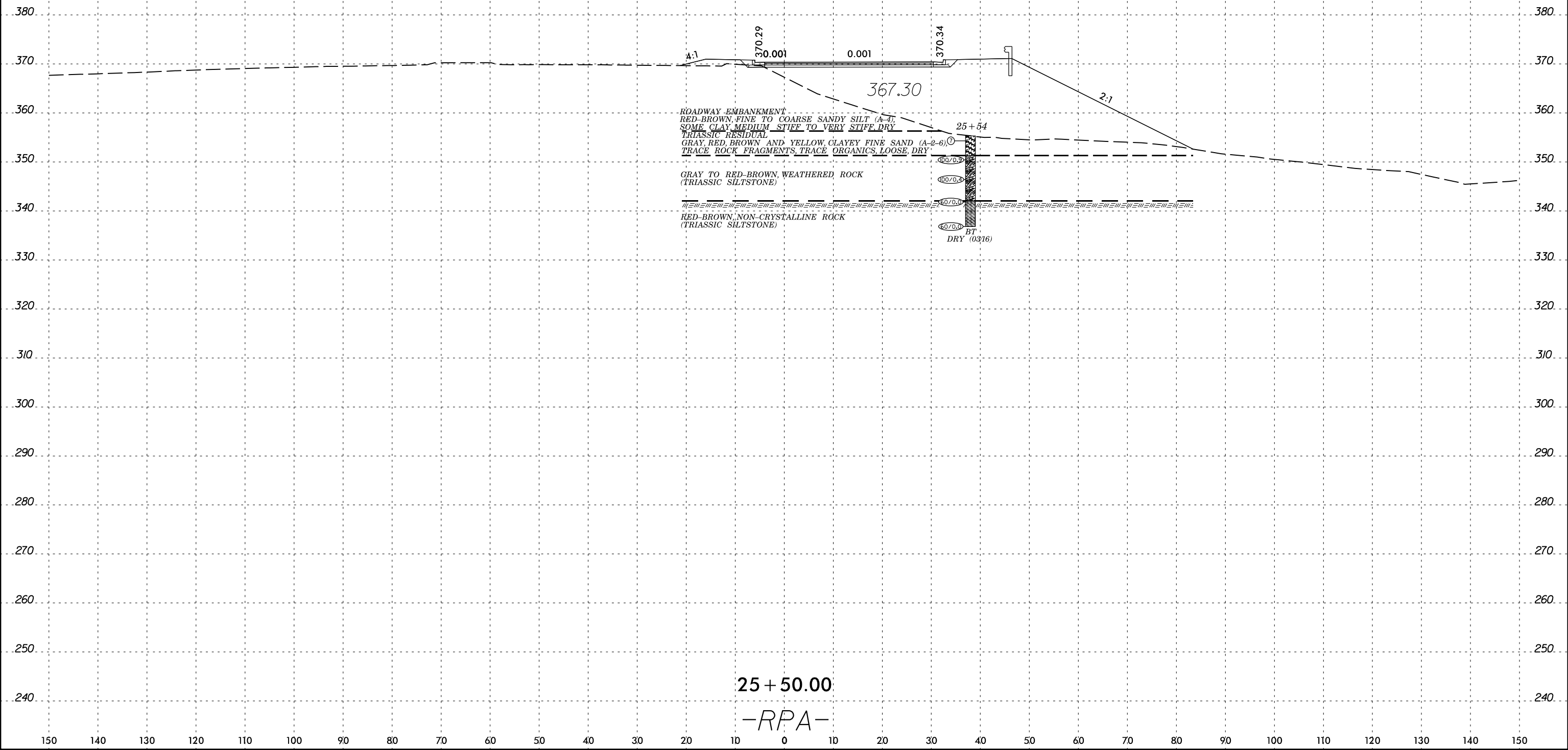
150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-54	8' RT	23+98	3.5'-5.0'	A-4(4)	28	16	4.8	13.1	51.5	30.6	97.0	94.2	82.3	9.4	-

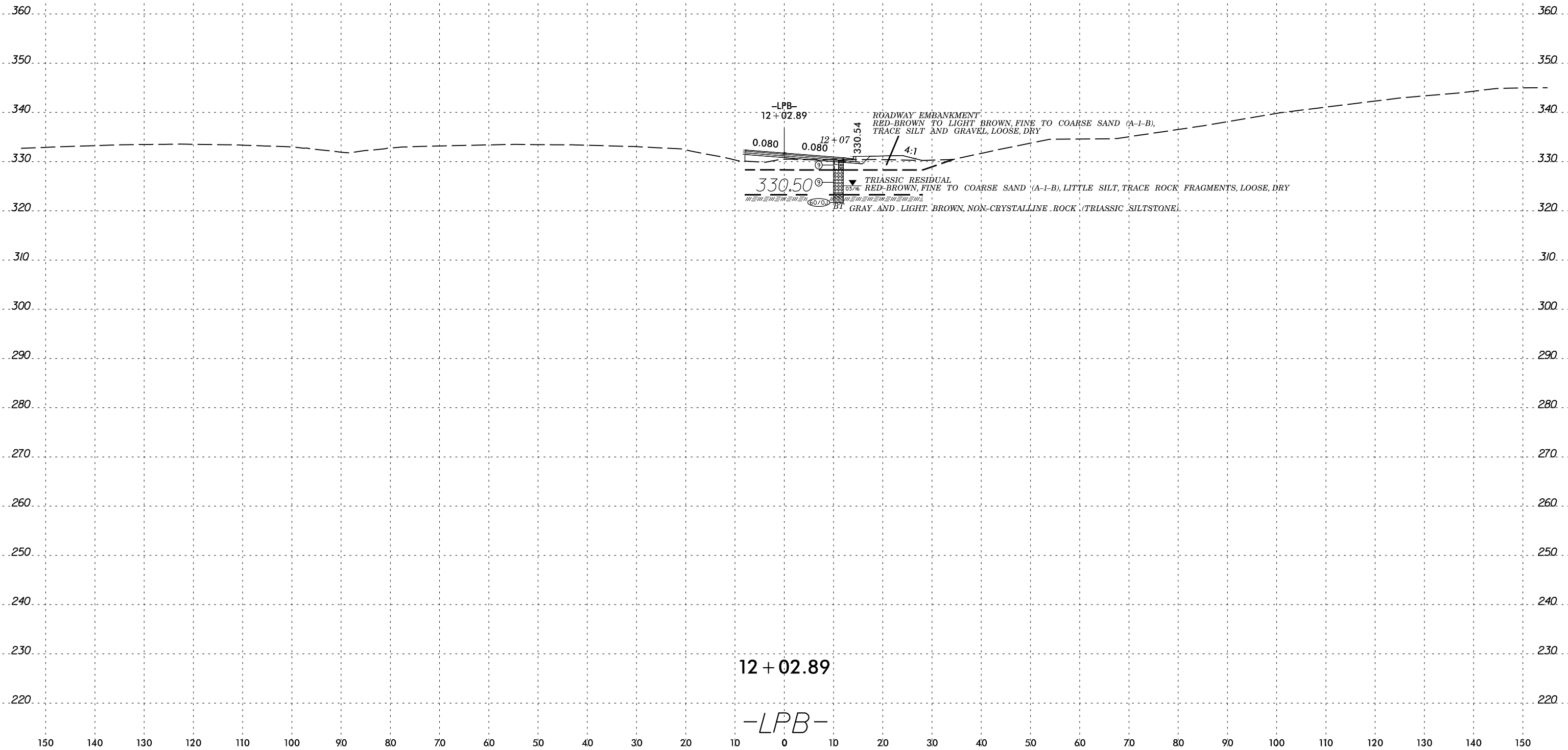


24+00.00
-RPA-

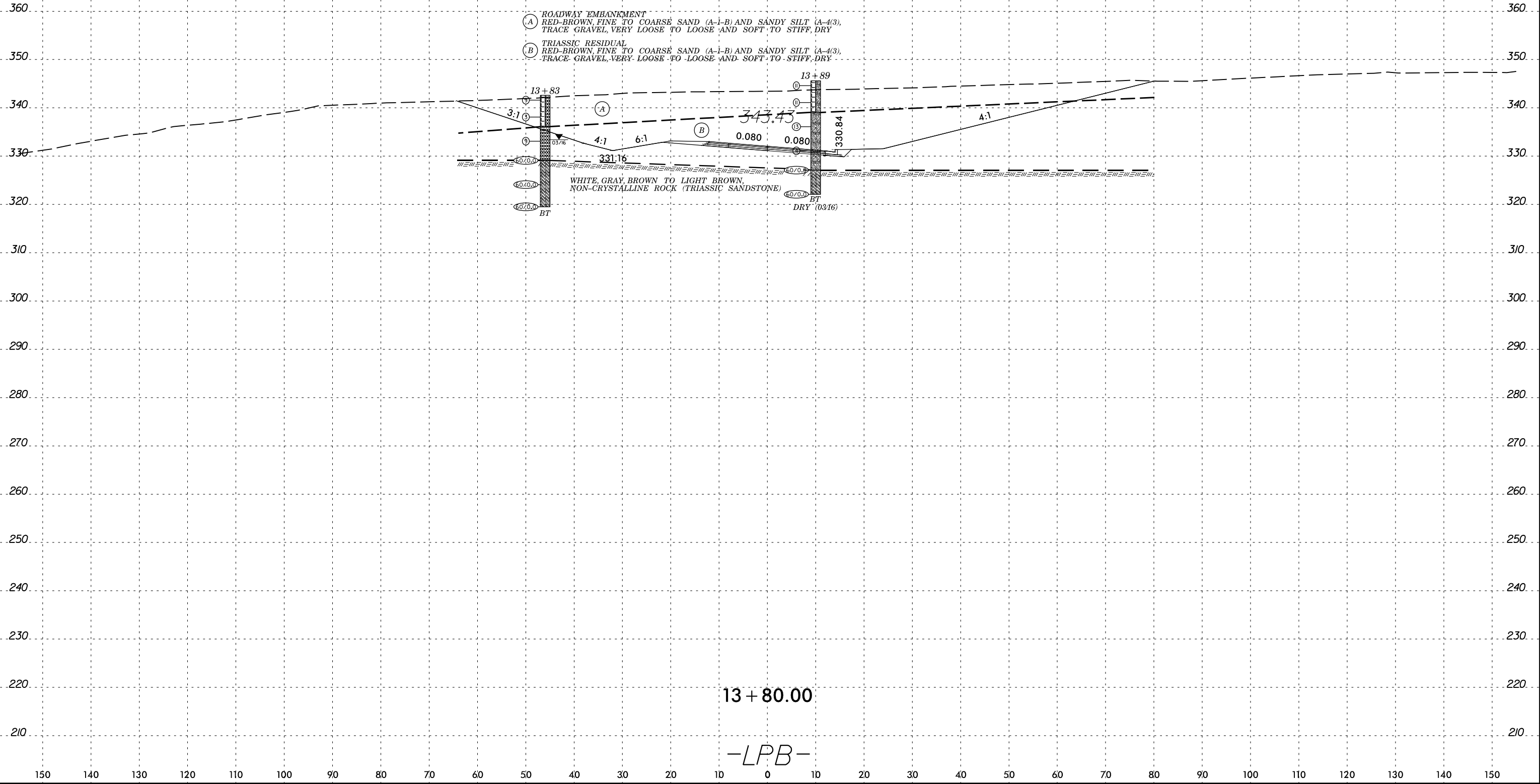
8/23/99



PLOT DRIVER: \$PLTRV\$
 USER: \$USER\$
 FILE: \$PWARYAULPATHDESC\$
 DATE: \$DATE\$
 TIME: \$TIME\$
 PENTABLE: \$PENTBL\$



SECTION CONDITION
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(A) ROADWAY EMBANKMENT
RED-BROWN, FINE TO COARSE SAND (A-1-B) AND SANDY SILT (A-4(3)),
TRACE GRAVEL, VERY LOOSE TO LOOSE AND SOFT TO STIFF, DRY

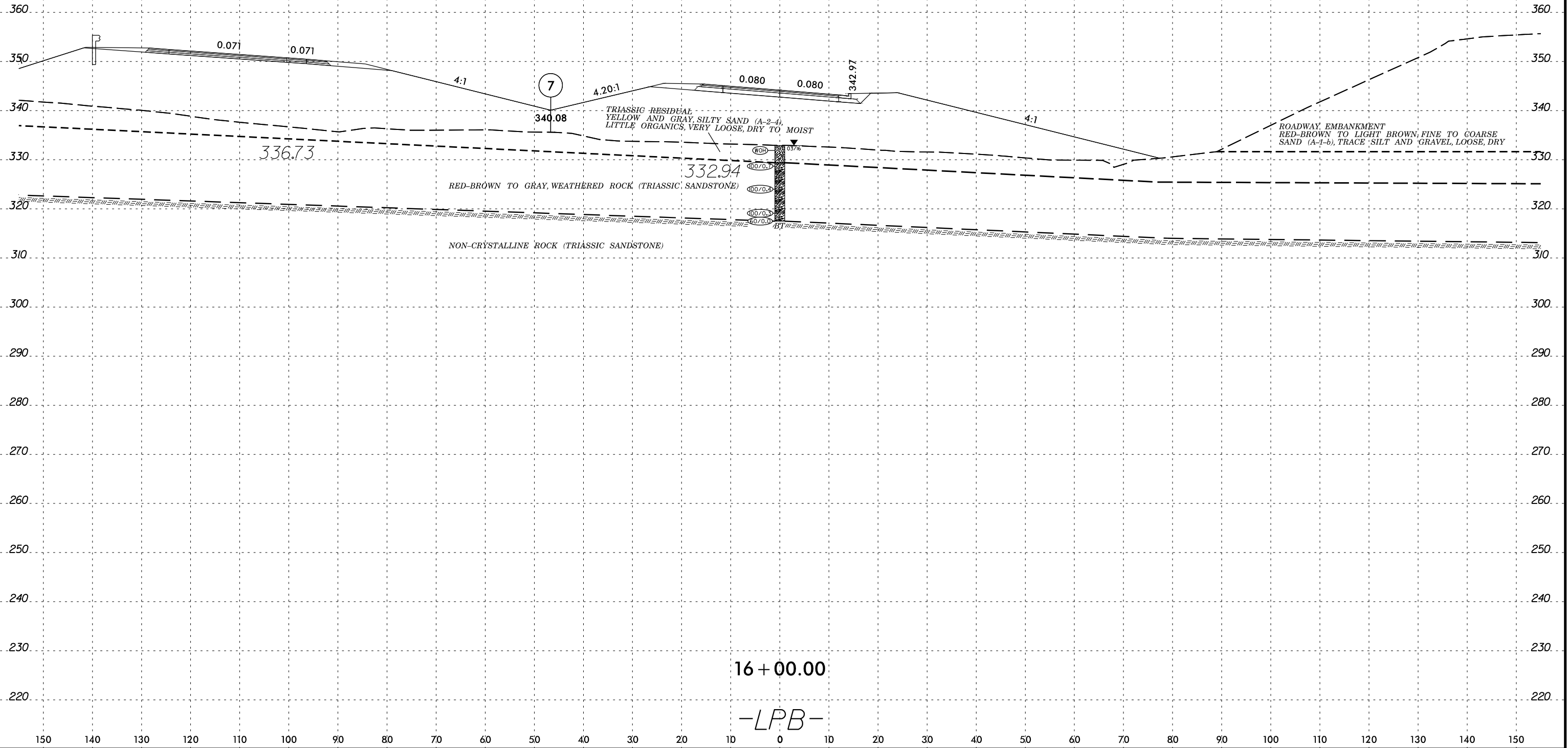
(B) TRIASSIC RESIDUAL
RED-BROWN, FINE TO COARSE SAND (A-1-B) AND SANDY SILT (A-4(3)),
TRACE GRAVEL, VERY LOOSE TO LOOSE AND SOFT TO STIFF, DRY

WHITE, GRAY BROWN TO LIGHT BROWN,
NON-CRYSTALLINE ROCK (TRIASSIC SANDSTONE)

13 + 80.00

-LPB-

SYTIME
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LOR
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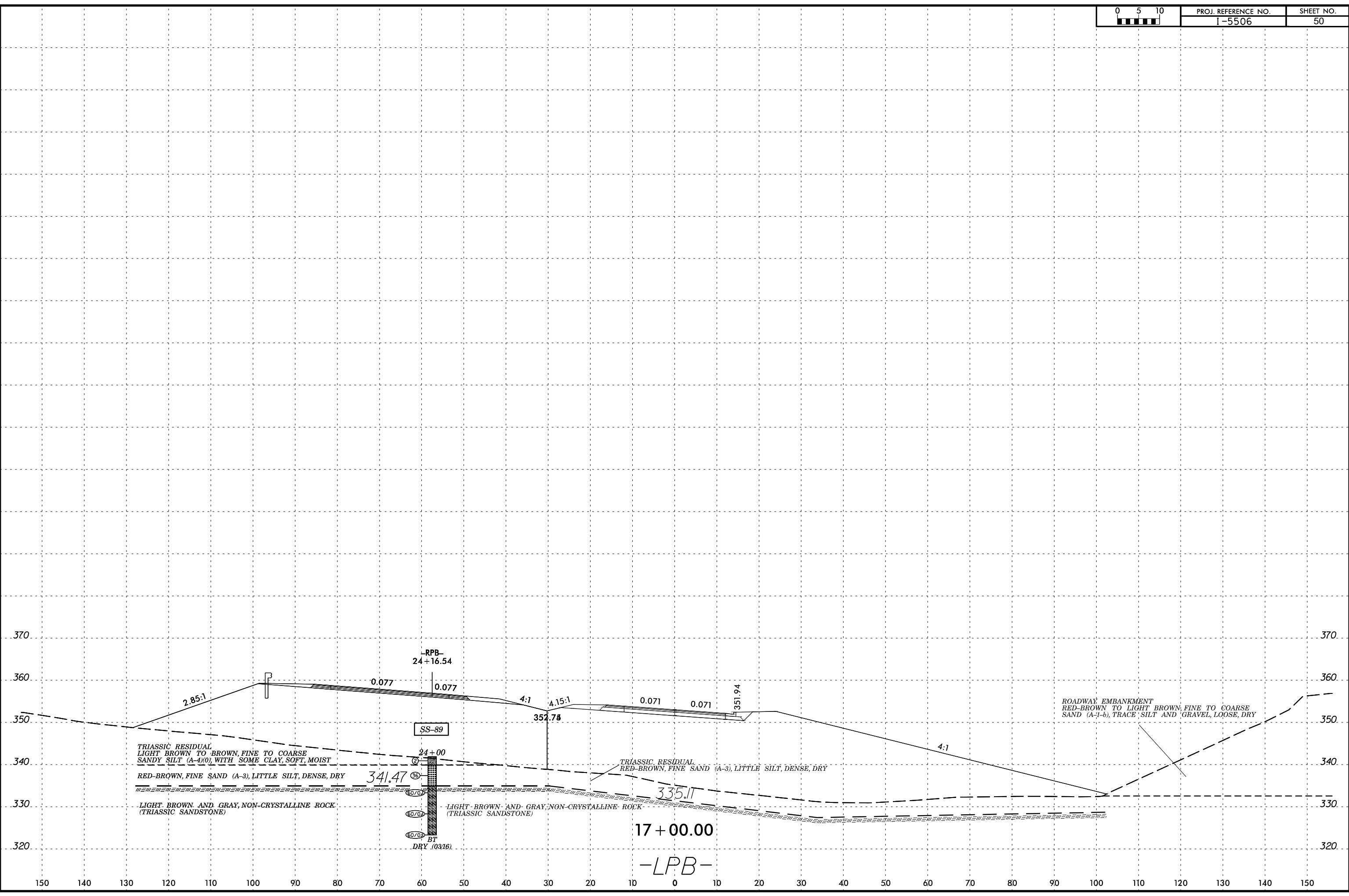
8/23/99



PROJ. REFERENCE NO.
-5506

SHEET NO.
50

PLOT DRIVER: \$PLTDVRS\$
USER: \$USER\$
FILE: \$PWARYAULTPATHDESC\$
DATE: \$DATE\$
TIME: \$TIME\$
PENTABLE: \$PENTBL\$



TRIASSIC RESIDUAL
LIGHT BROWN TO BROWN, FINE TO COARSE
SANDY SILT (A-4)(0), WITH SOME CLAY, SOFT, MOIST

RED-BROWN, FINE SAND (A-3), LITTLE SILT, DENSE, DRY

LIGHT BROWN AND GRAY, NON-CRYSTALLINE ROCK
(TRIASSIC SANDSTONE)

-RPB-
24+16.54

SS-89

24+00

341.47

60/00

60/00

BT

DRY (0316)

17+00.00

-LPB-

2.85:1

0.077

0.077

4:1

4.15:1

0.071

0.071

351.94

4:1

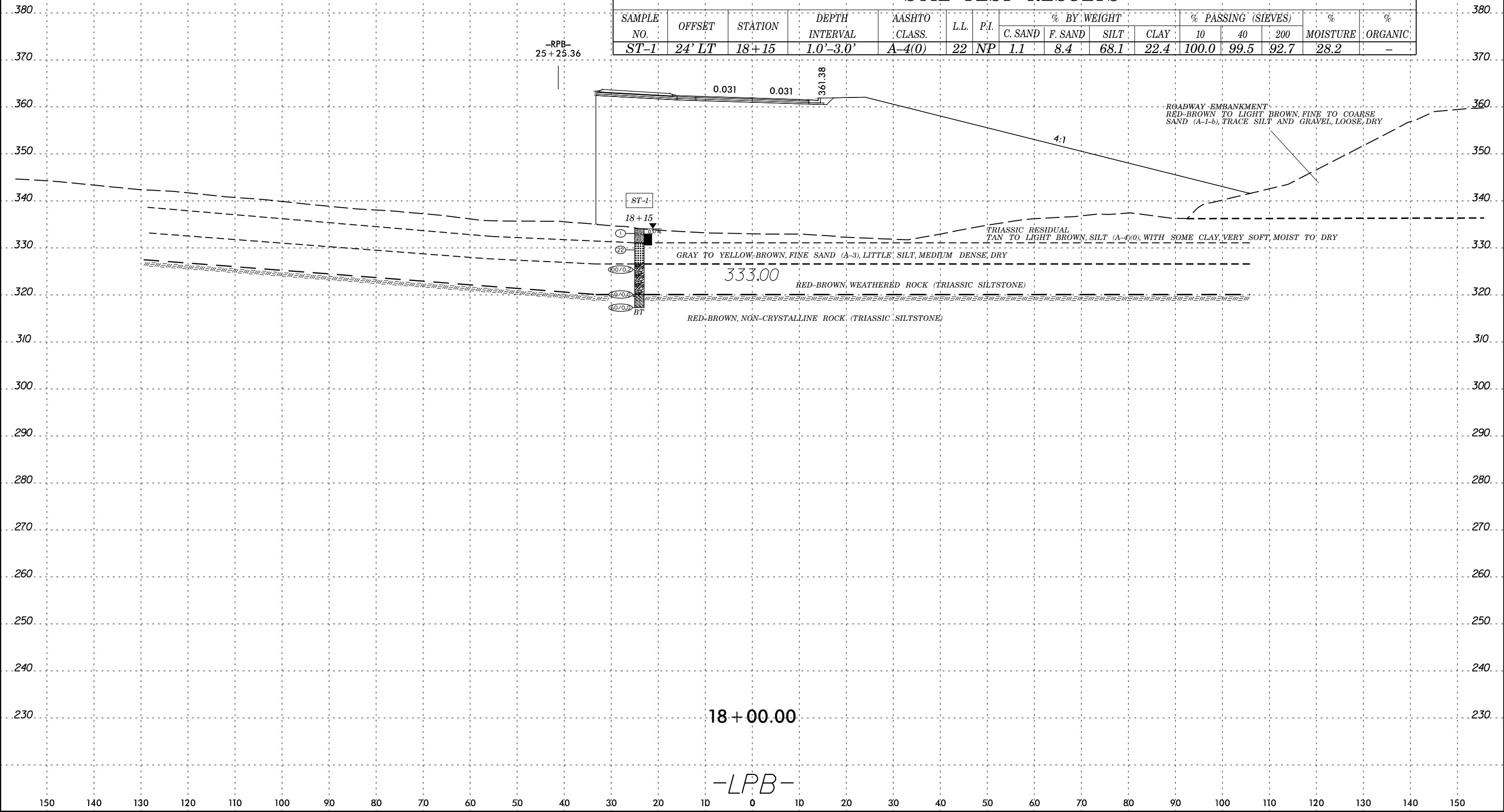
352.78

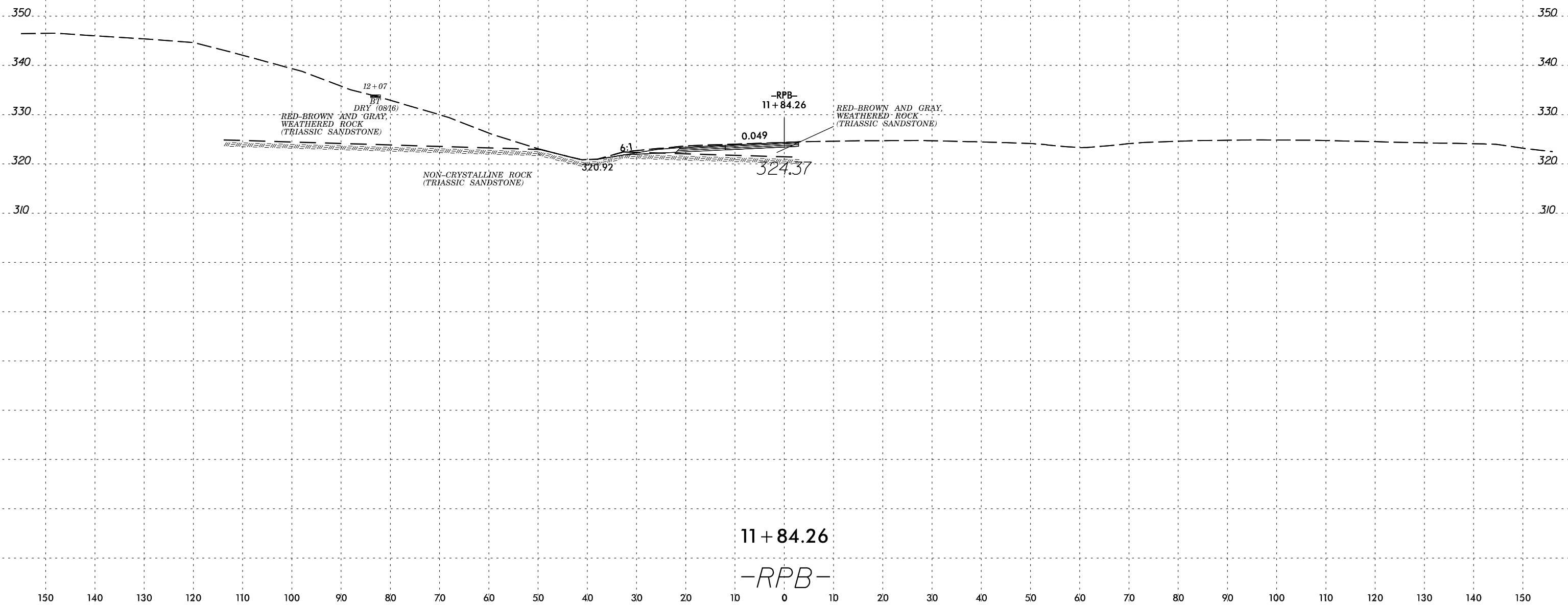
335.11

TRIASSIC RESIDUAL
RED-BROWN, FINE SAND (A-3), LITTLE SILT, DENSE, DRY

ROADWAY EMBANKMENT
RED-BROWN TO LIGHT BROWN, FINE TO COARSE
SAND (A-1-b), TRACE SILT AND GRAVEL, LOOSE, DRY

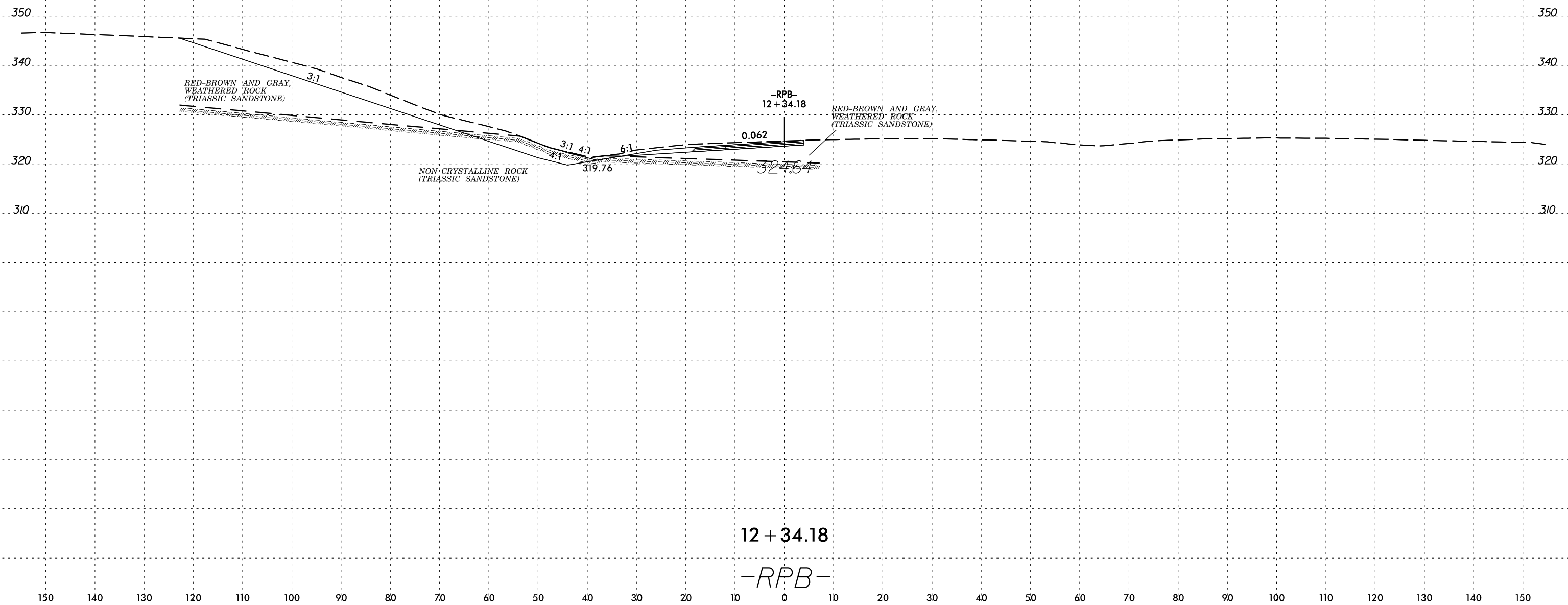
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-1	24' LT	18+15	1.0'-3.0'	A-4(0)	22	NP	1.1	8.4	68.1	22.4	100.0	99.5	92.7	28.2	-

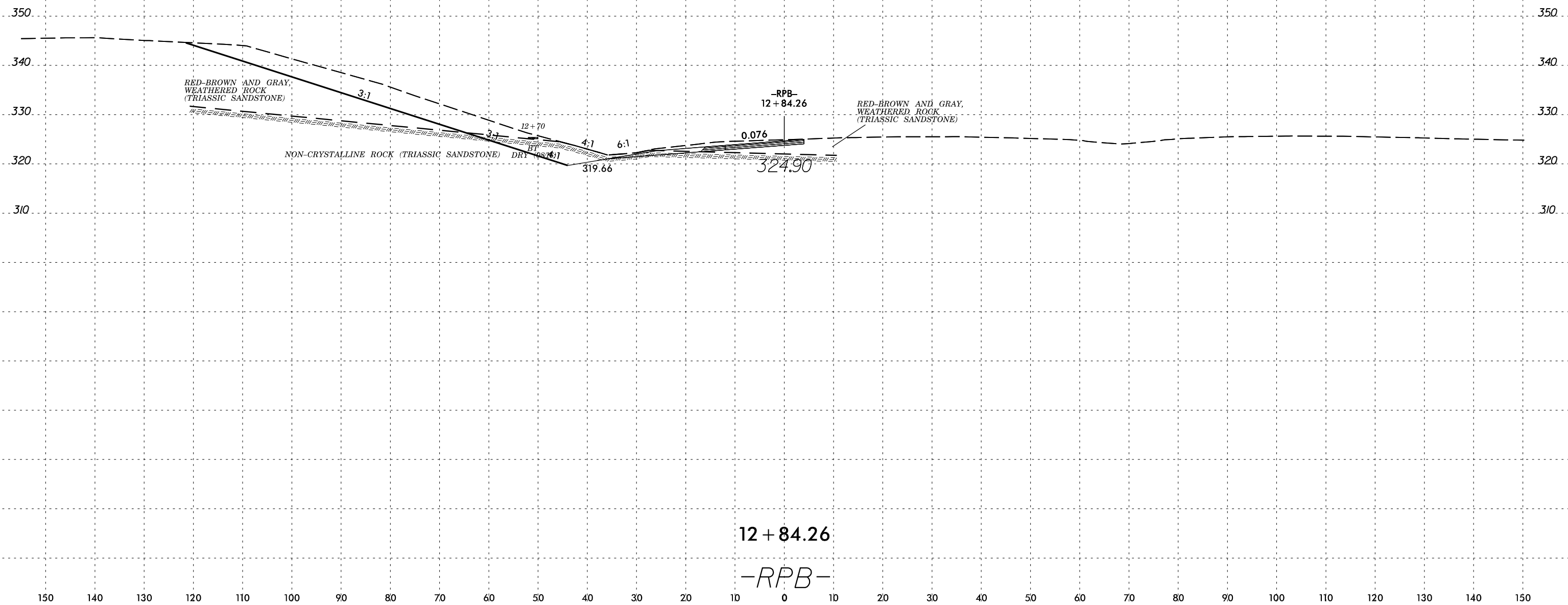




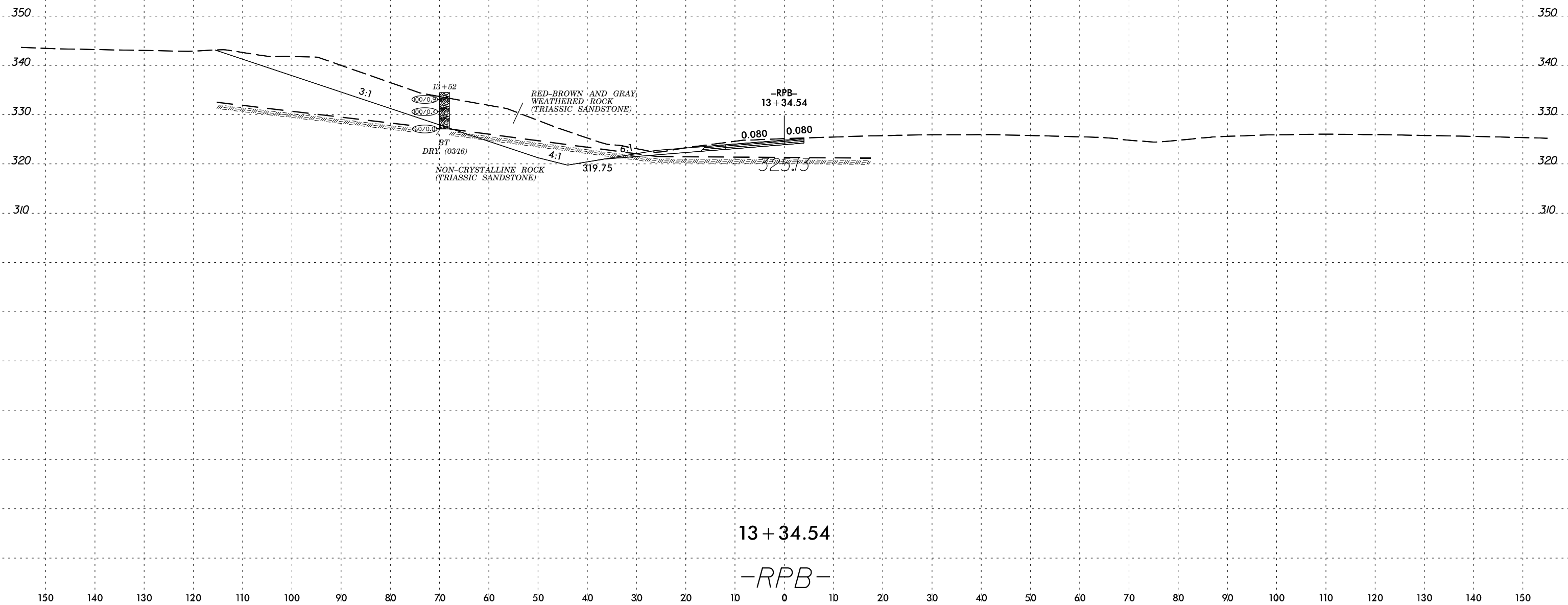
SECTION 11+84.26
PLAN
DATE 8/23/99

11 + 84.26
-RPB-

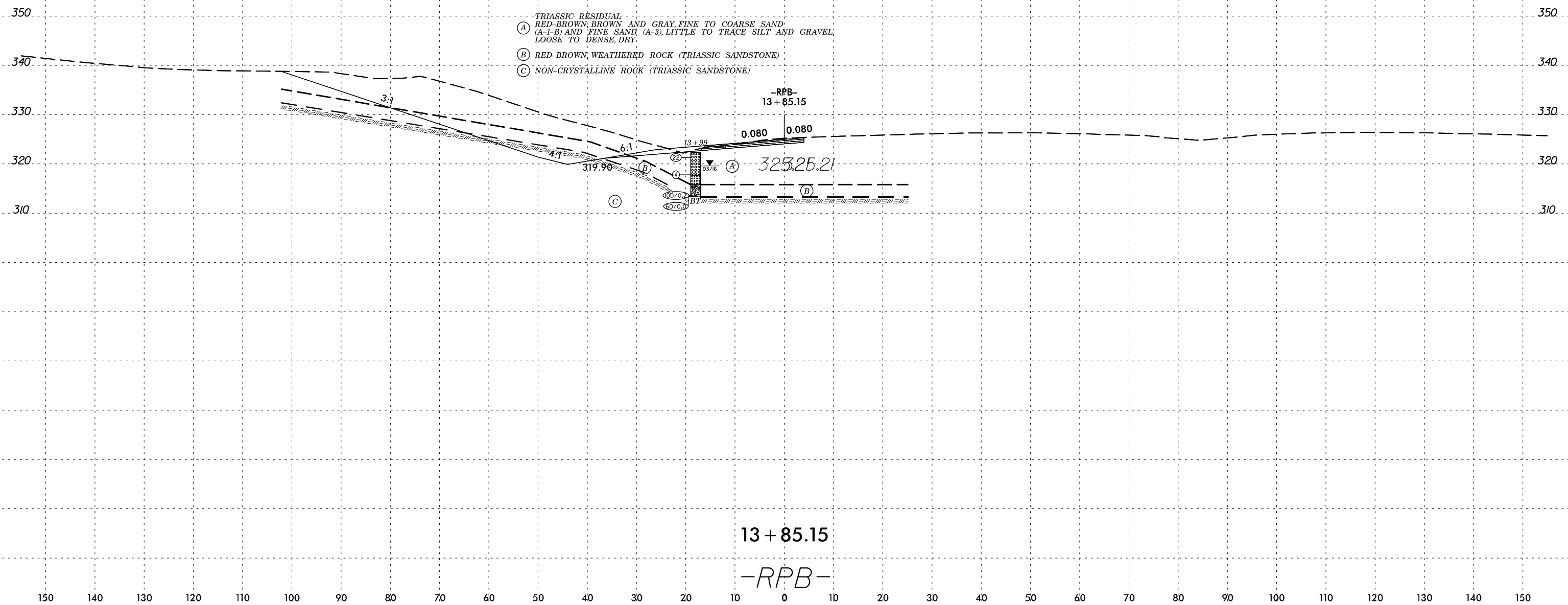




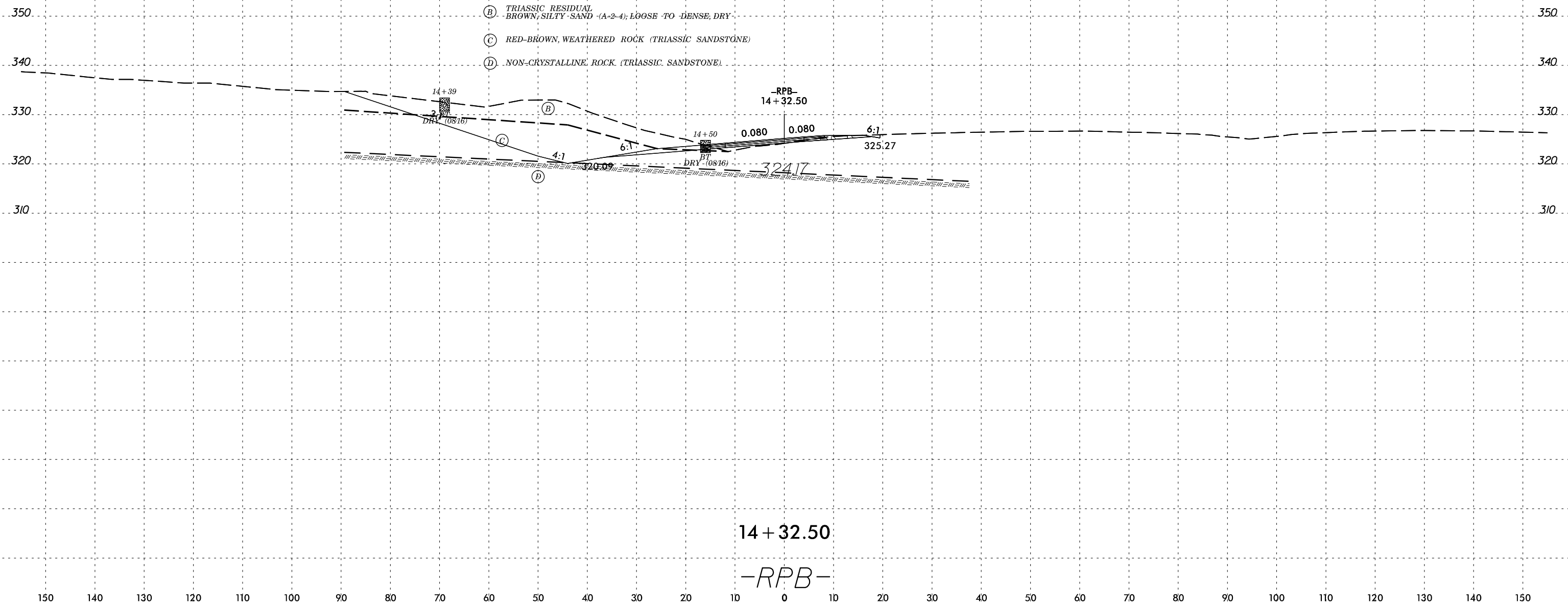
SECTION 12+84.26



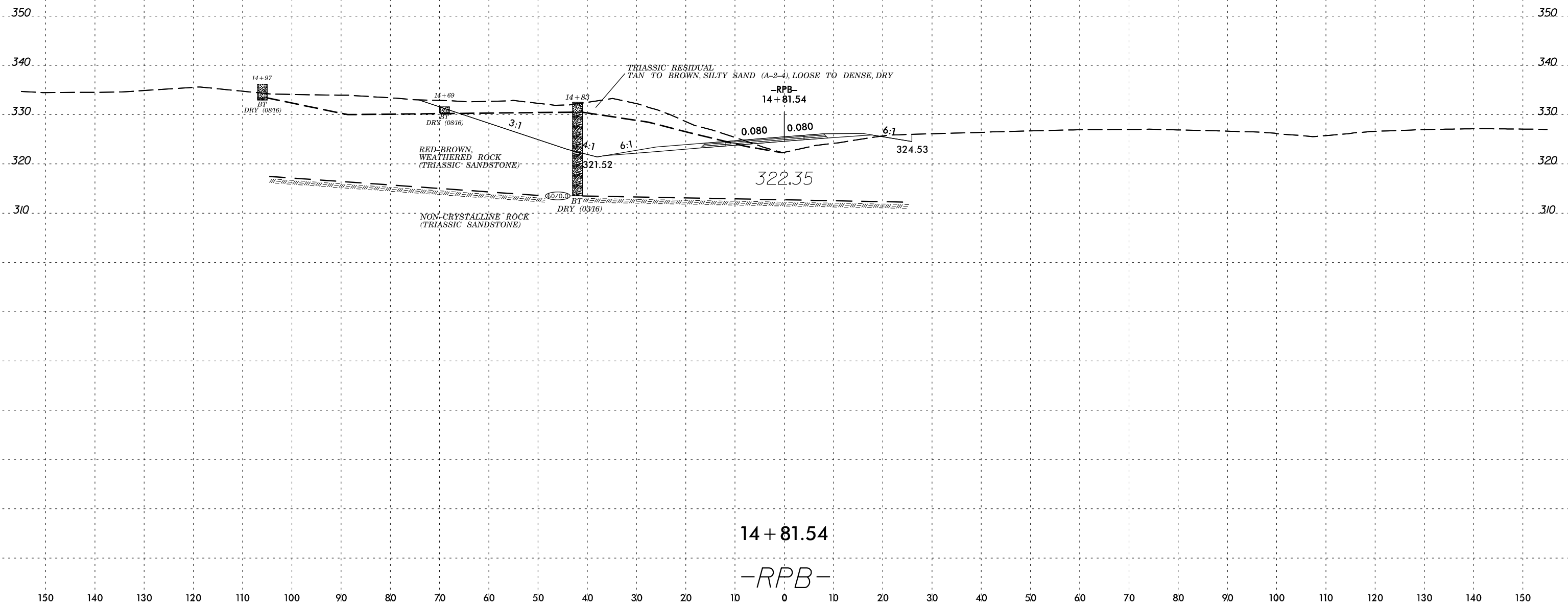
SYNTHETIC
SECTION
FOR
CONSTRUCTION
DRAWING

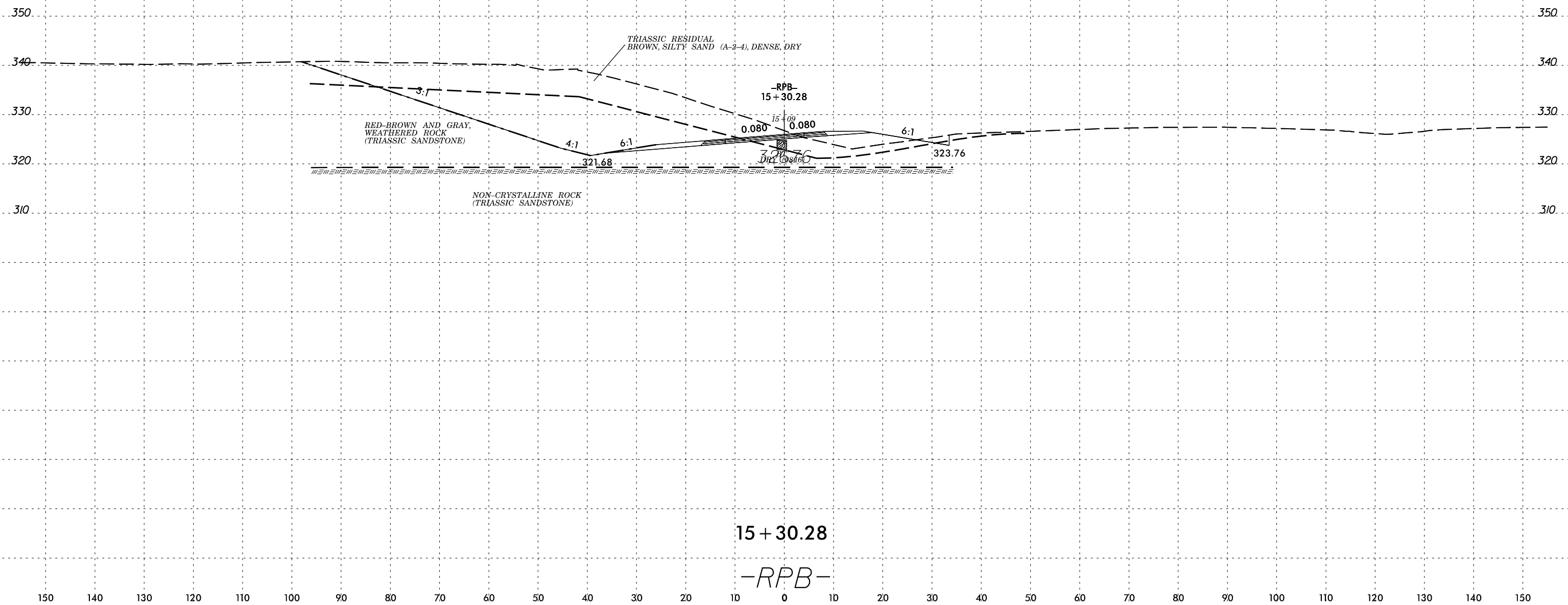


SYNTHETIC SECTION

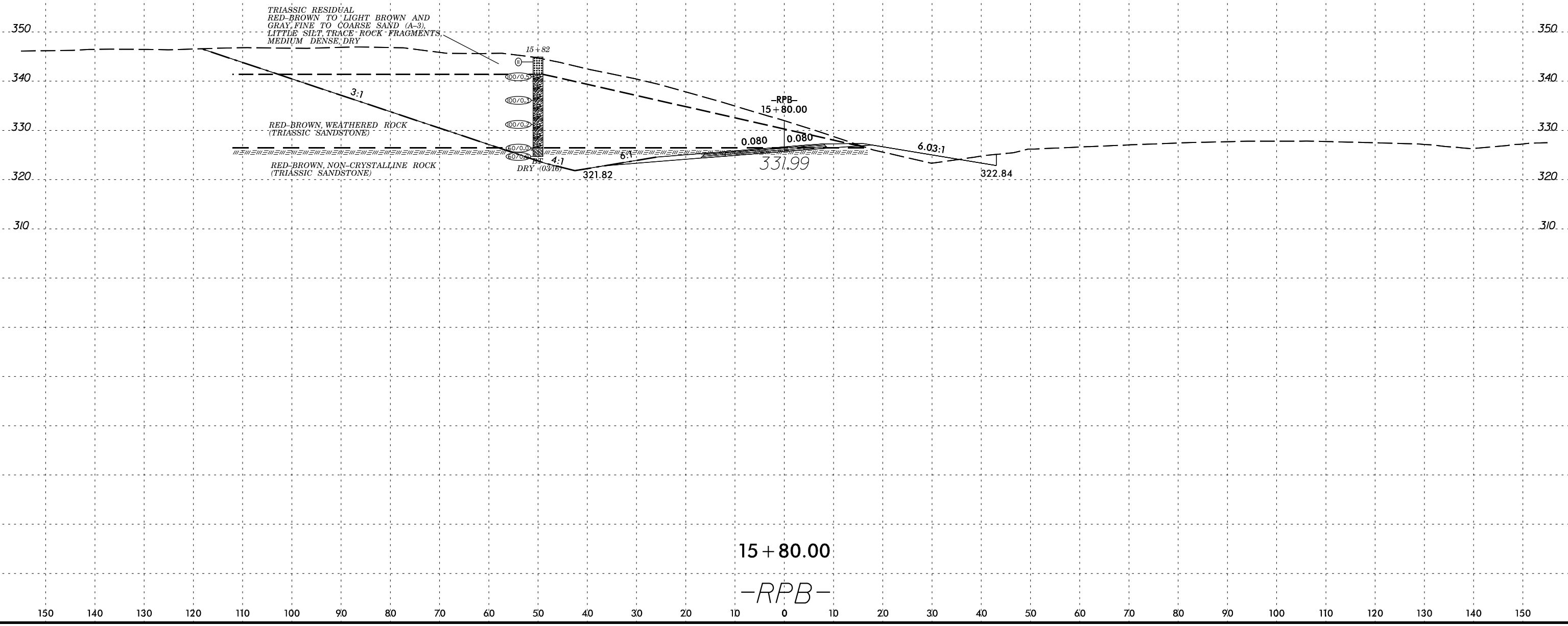


SECTION 14+32.50
CONSTRUCTION
PLAN
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SCALE
BY
CHECKED
APPROVED



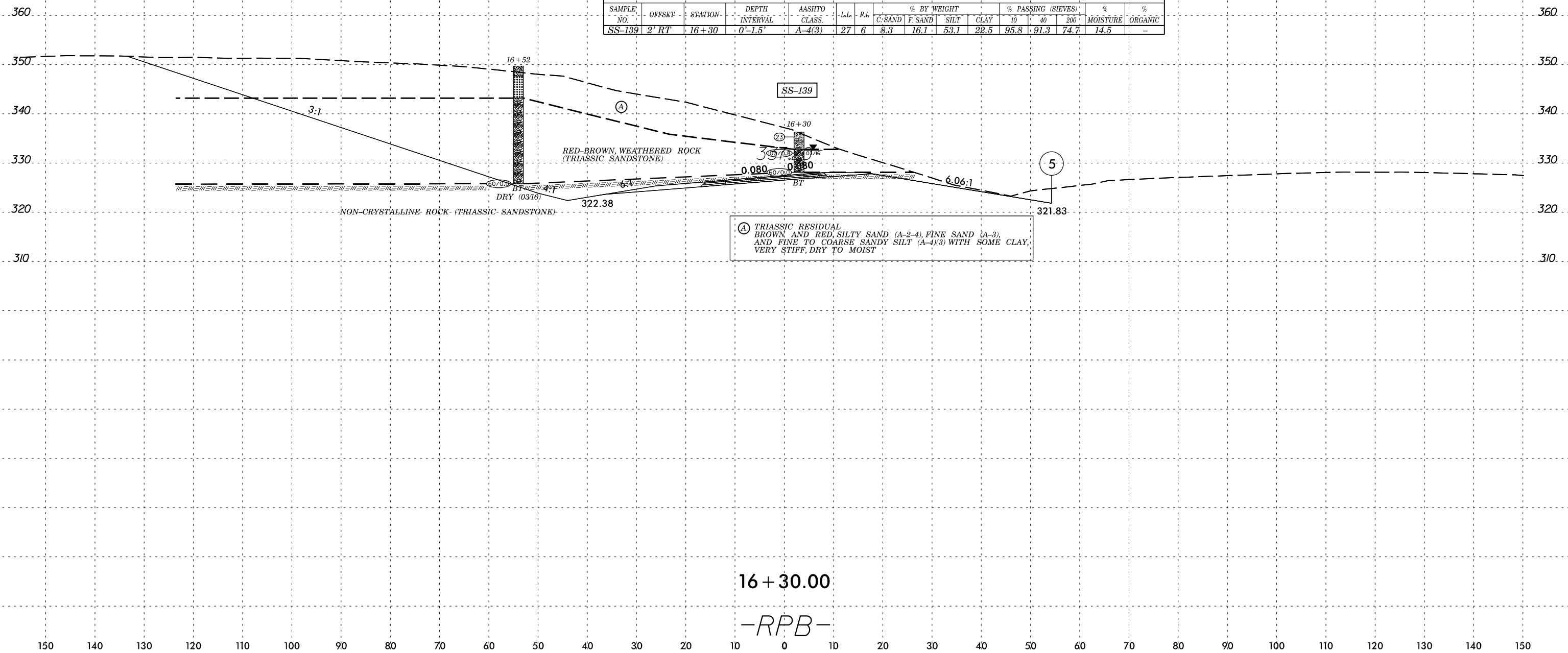


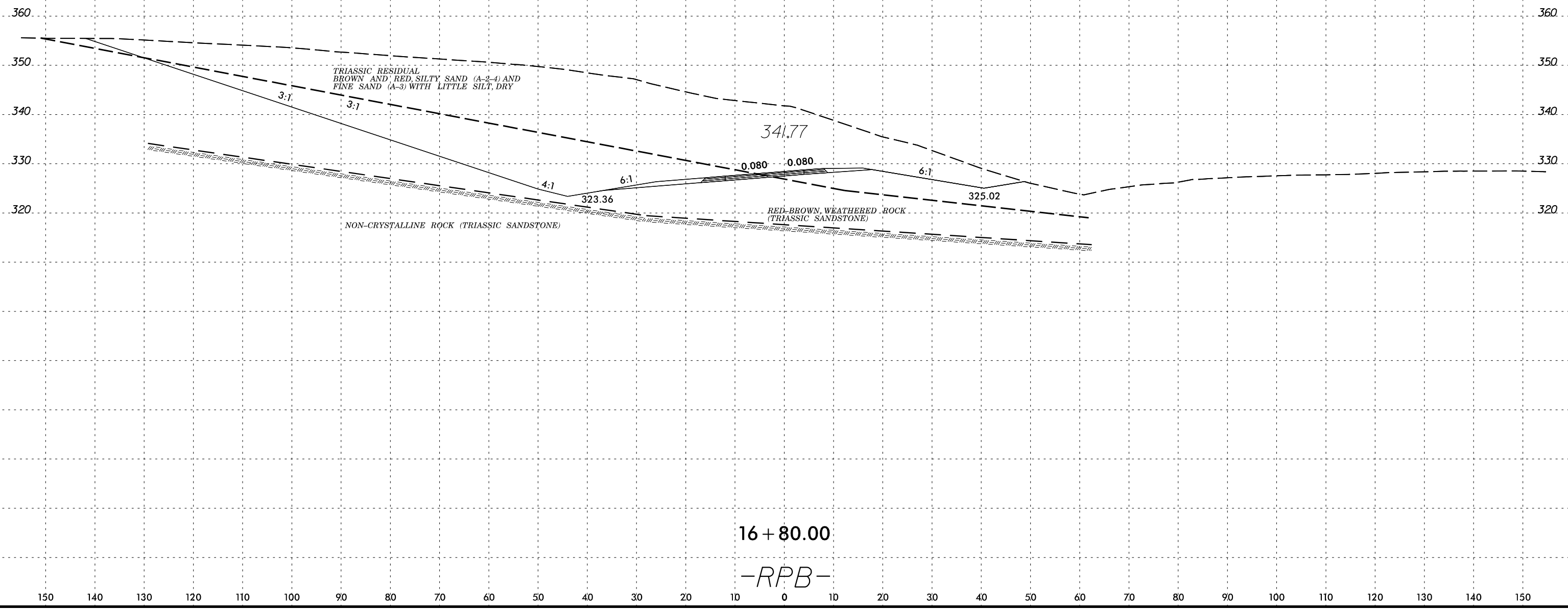
SECTION 15+30.28



SYNTHETIC
CONCRETE
PULVERIZED
FUELS
COURSE

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-139	2' RT	16+30	0'-1.5'	A-4(3)	27	6	8.3	16.1	53.1	22.5	95.8	91.3	74.7	14.5	-



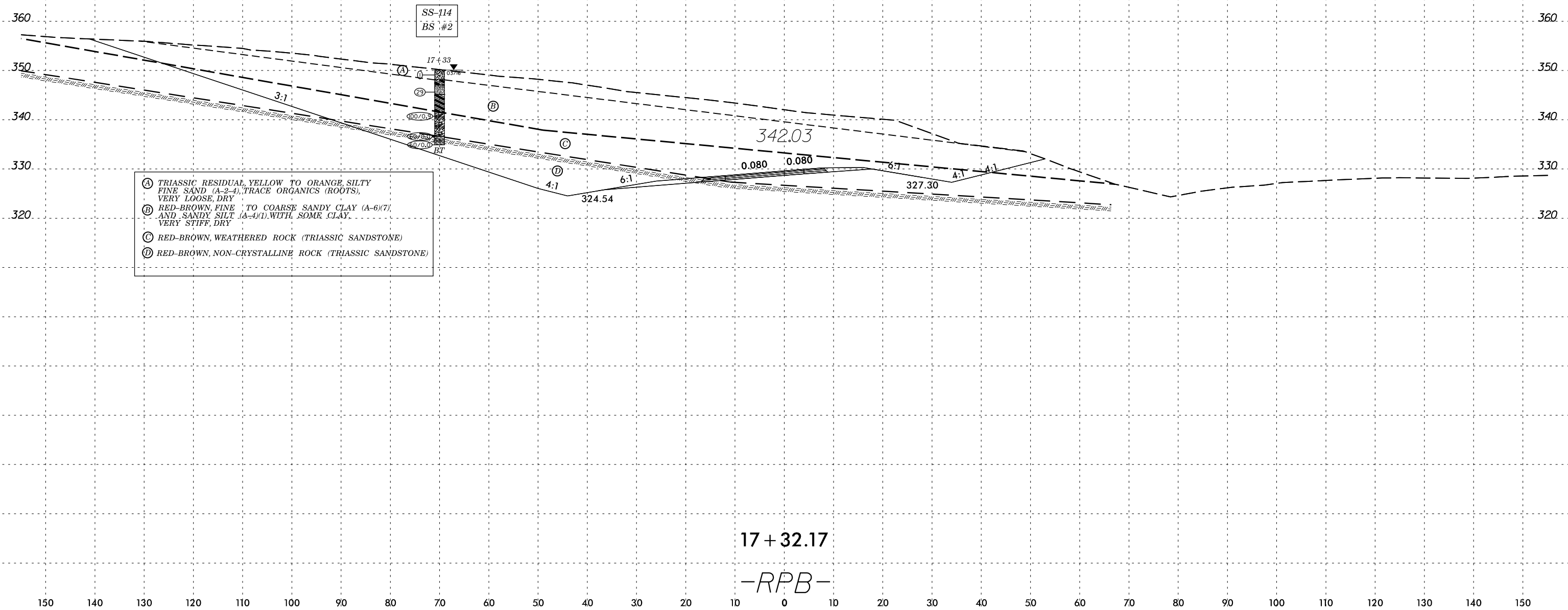


16 + 80.00

-RFB-

SECTION CUT

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-114	70' LT	17+33	3.5'-5.0'	A-4(1)	27	5	21.9	17.0	37.9	23.2	89.0	75.2	56.7	10.9	
BS #2	70' LT	17+33	2.0'-10.0'	A-6(7)	30	11	11.3	9.5	46.7	32.5	97.7	90.2	78.8	7.4	

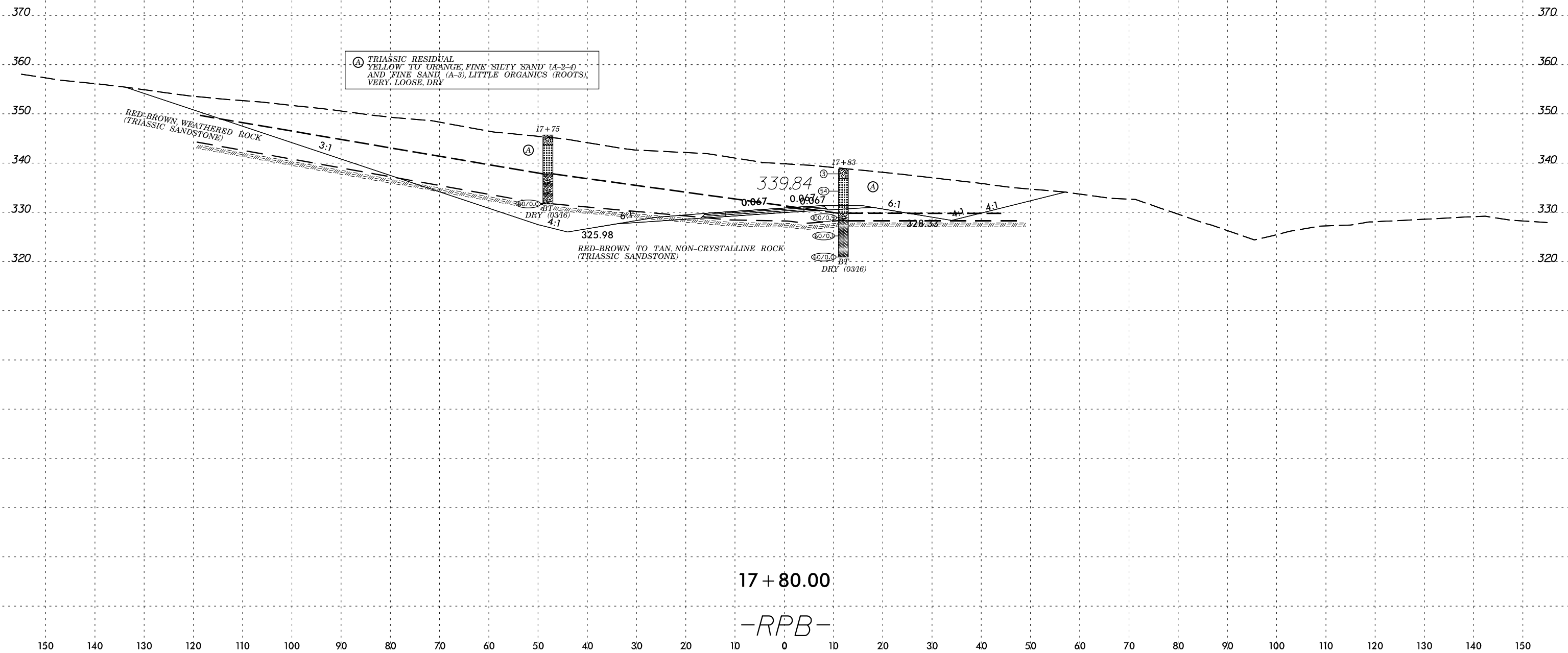


- (A) TRIASSIC RESIDUAL, YELLOW TO ORANGE SILTY FINE SAND (A-2-4), TRACE ORGANICS (ROOTS), VERY LOOSE, DRY
- (B) RED-BROWN, FINE TO COARSE SANDY CLAY (A-6(7)) AND SANDY SILT (A-4(1)) WITH SOME CLAY, VERY STIFF, DRY
- (C) RED-BROWN, WEATHERED ROCK (TRIASSIC SANDSTONE)
- (D) RED-BROWN, NON-CRYSTALLINE ROCK (TRIASSIC SANDSTONE)

17+32.17

-RPB-

SYNTHETIC SECTION



Ⓐ TRIASSIC RESIDUAL
YELLOW TO ORANGE, FINE SILTY SAND (A-2-4)
AND FINE SAND (A-3), LITTLE ORGANICS (ROOTS),
VERY LOOSE, DRY

RED-BROWN WEATHERED ROCK
(TRIASSIC SANDSTONE)

17+75

Ⓐ

4:1

325.98
RED-BROWN TO TAN, NON-CRYSTALLINE ROCK
(TRIASSIC SANDSTONE)

17+83

Ⓐ

6:1

0.067
DRY (0316)

339.84

4:1

4:1

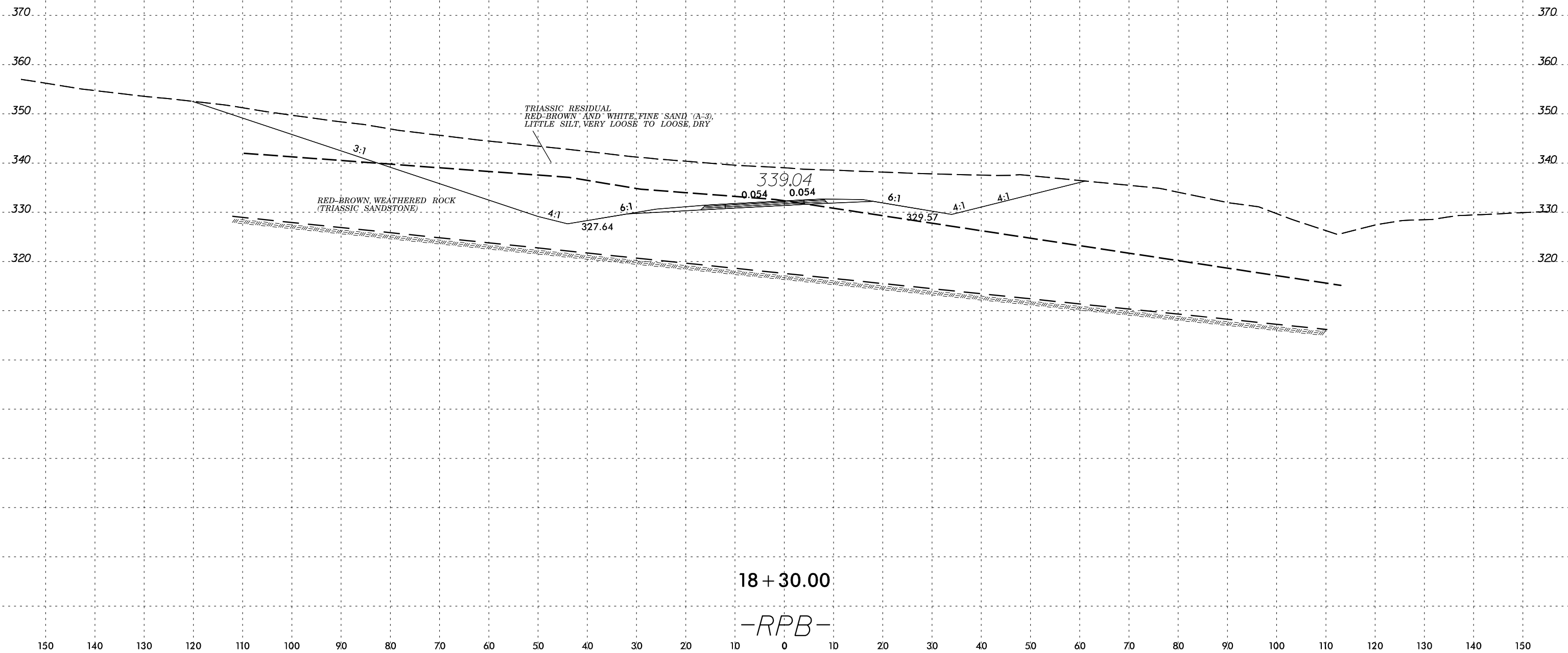
328.33

17+80.00

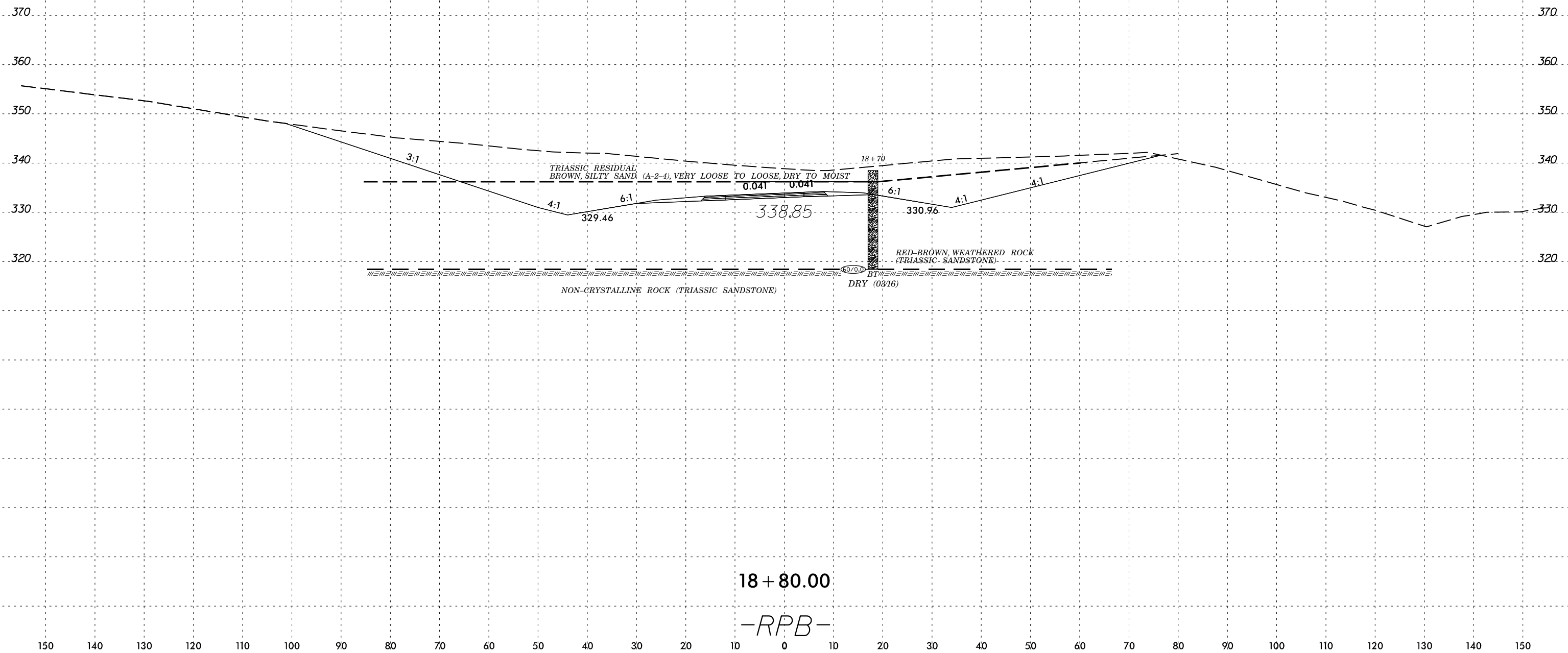
-RPB-

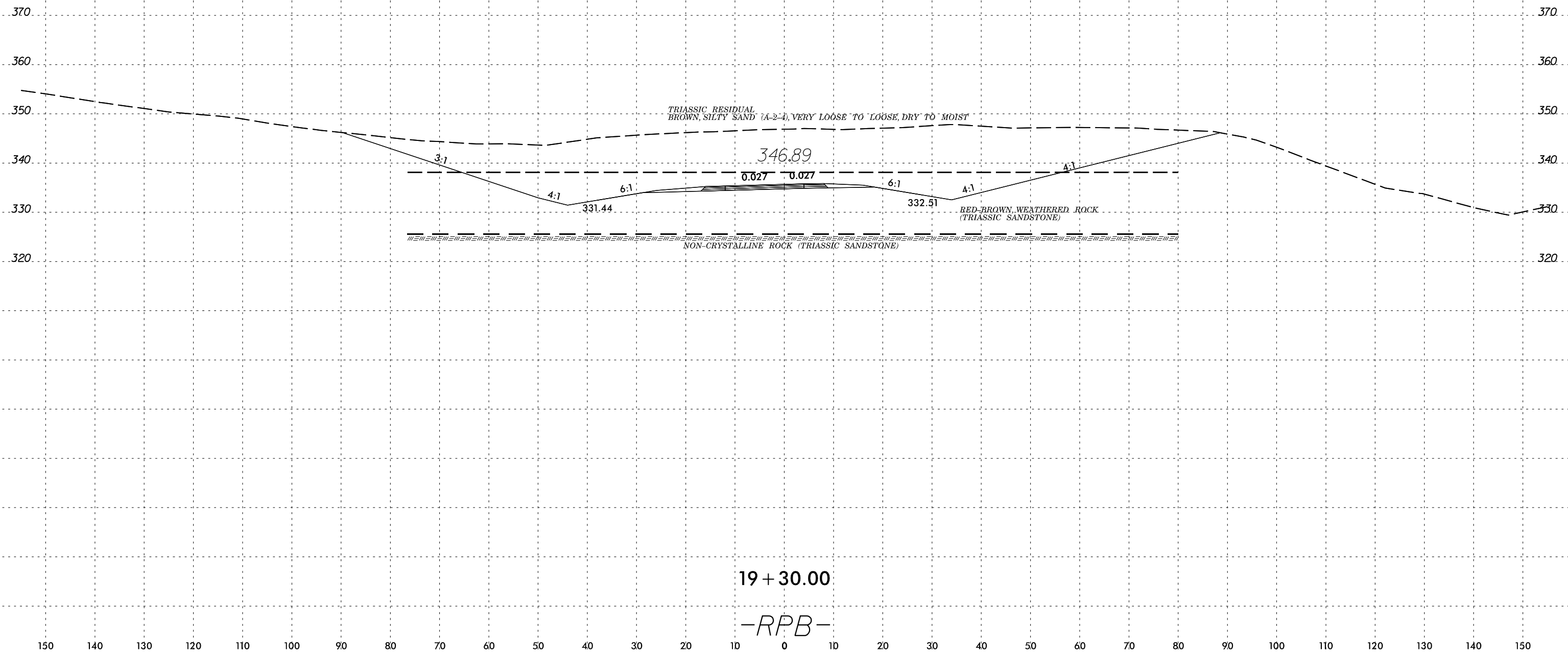
SECTION 17+80.00
CONSTRUCTION
PLAN
DATE 8/23/99

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



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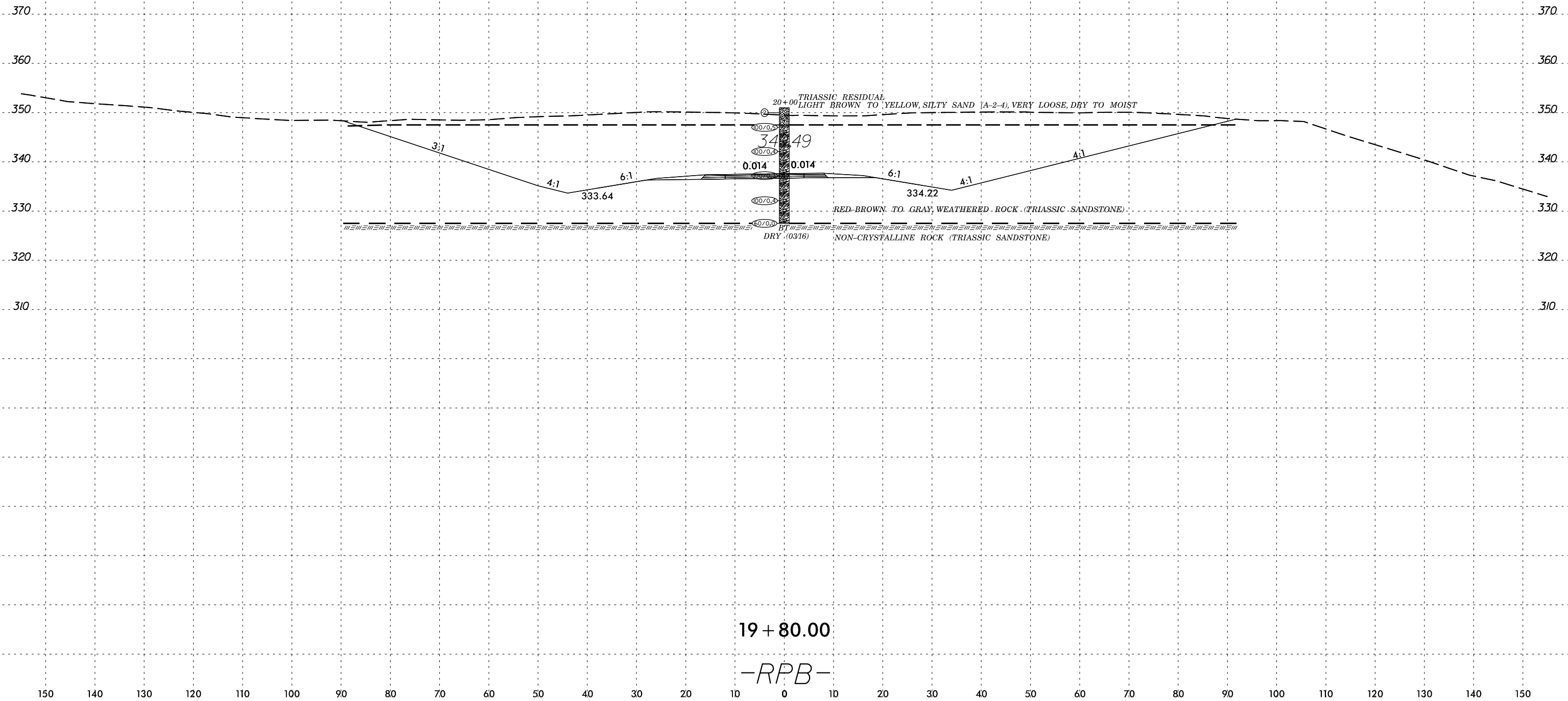




19 + 30.00

-RPB-

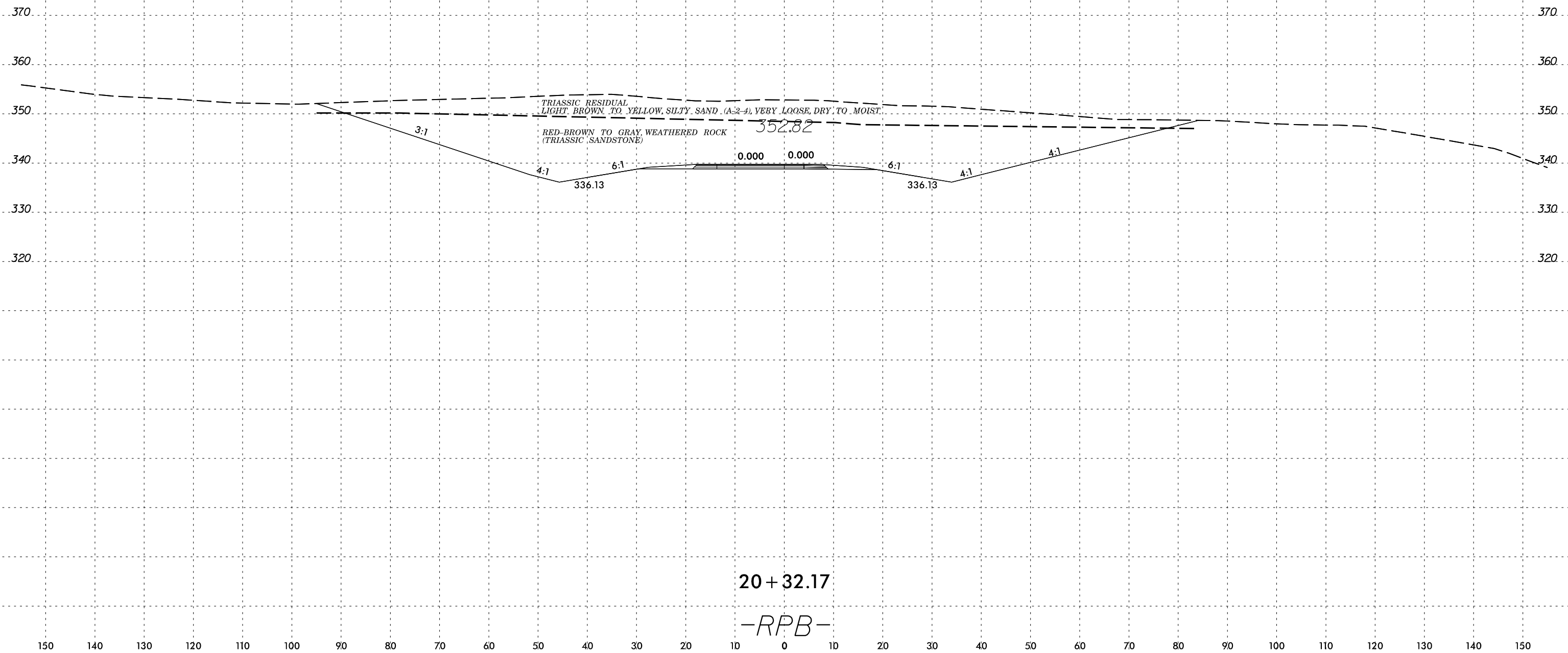
SECTION



19 + 80.00

-RPB-

SECTION 68
DRAWN BY
CHECKED BY
DATE



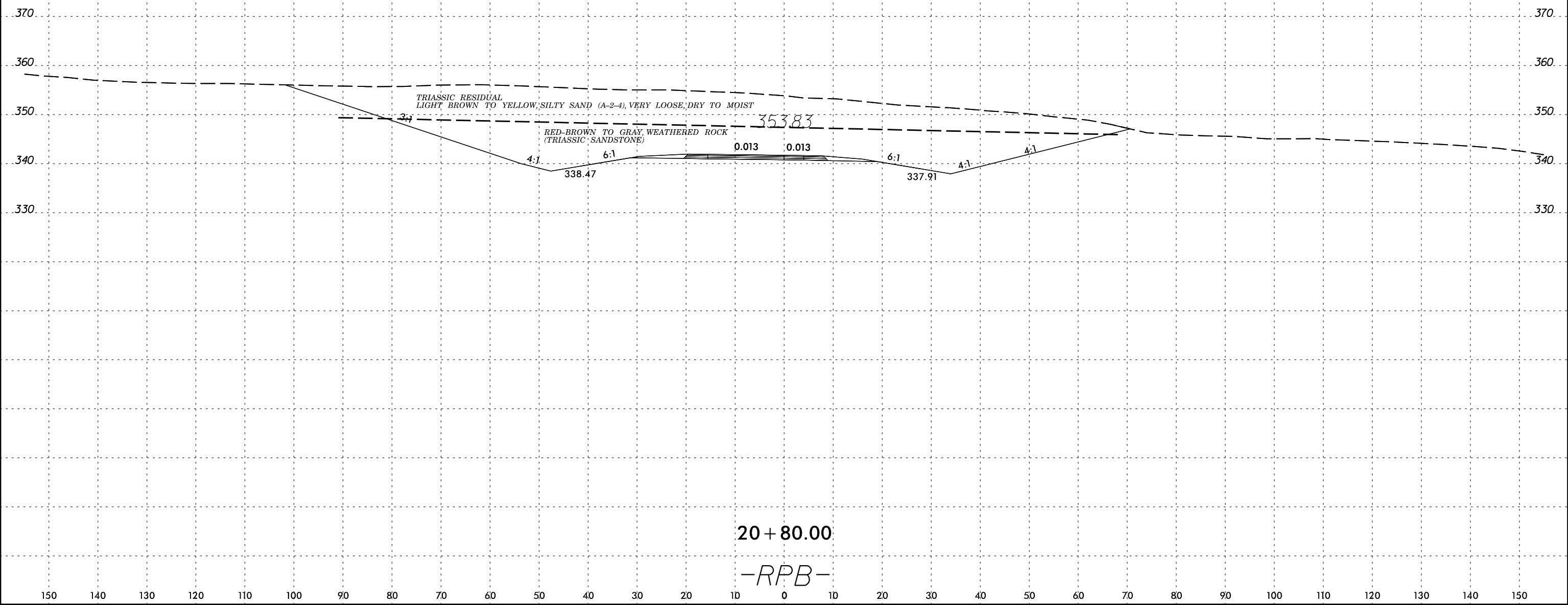
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8/23/99



PROJ. REFERENCE NO.
I-5506

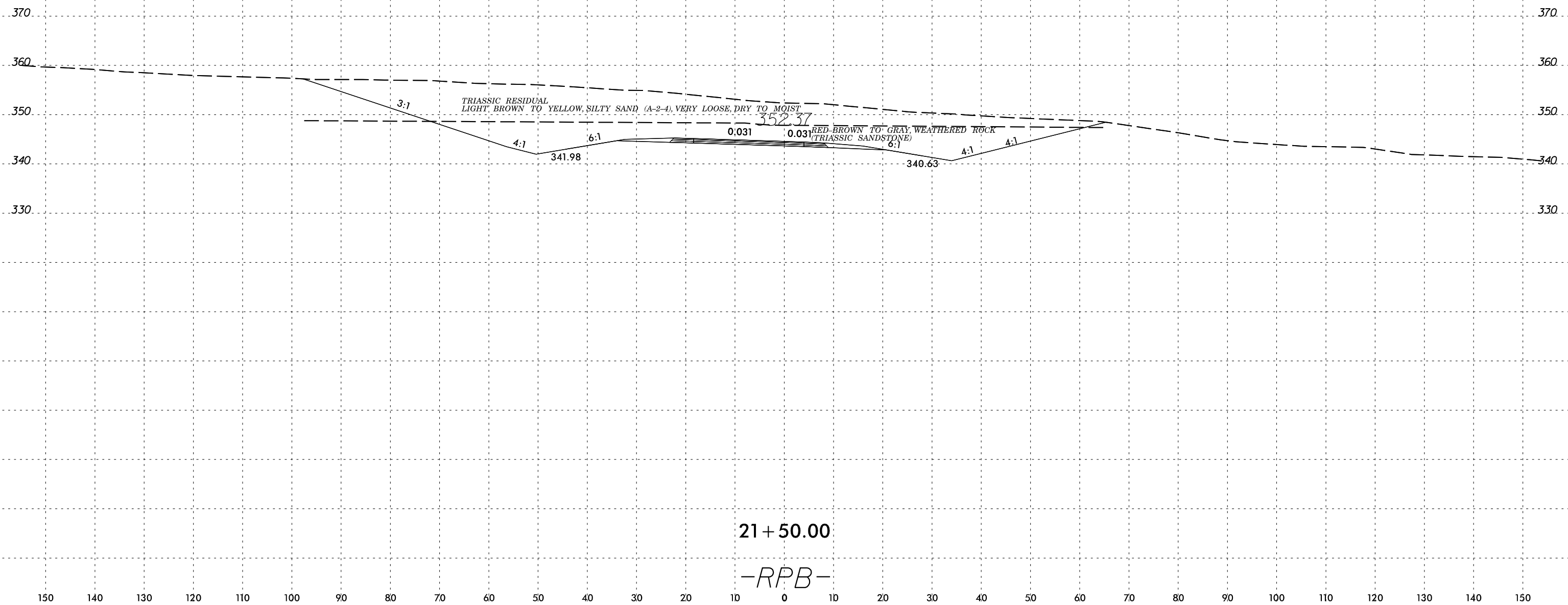
SHEET NO.
70



20+80.00

-RPB-

SYTIME
SHEET
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DRAWING

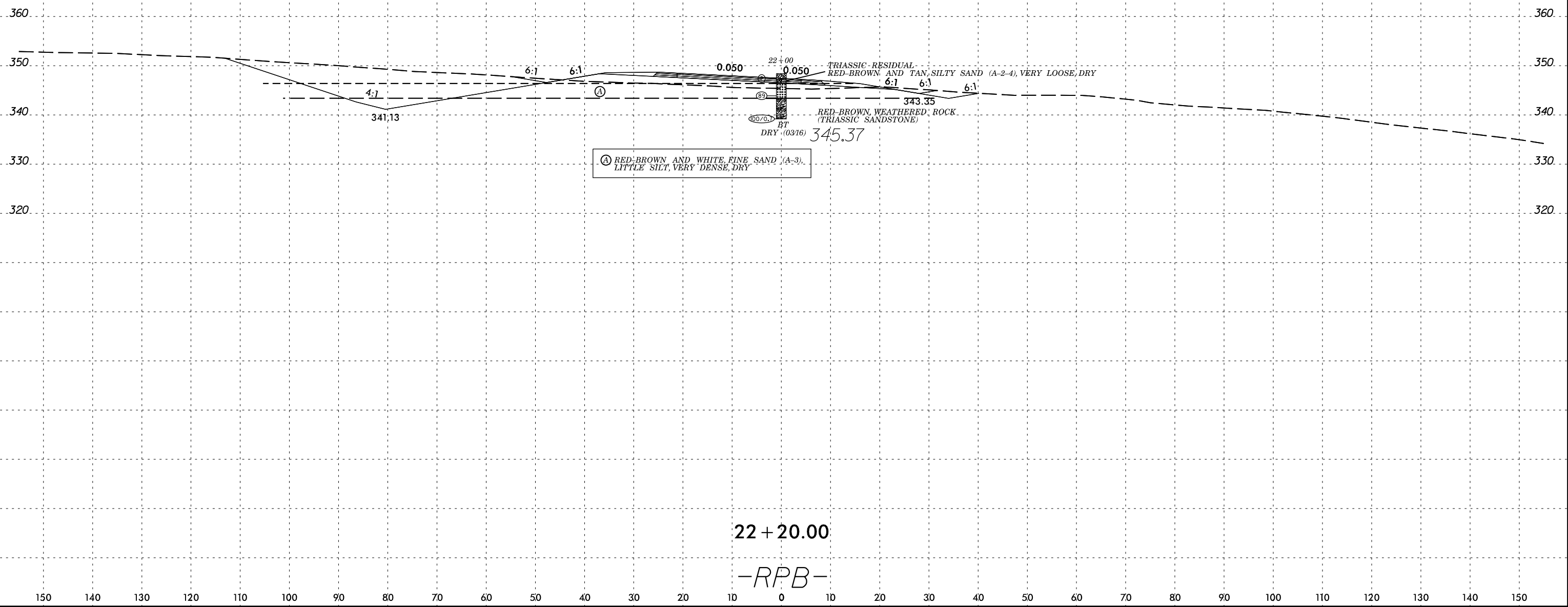


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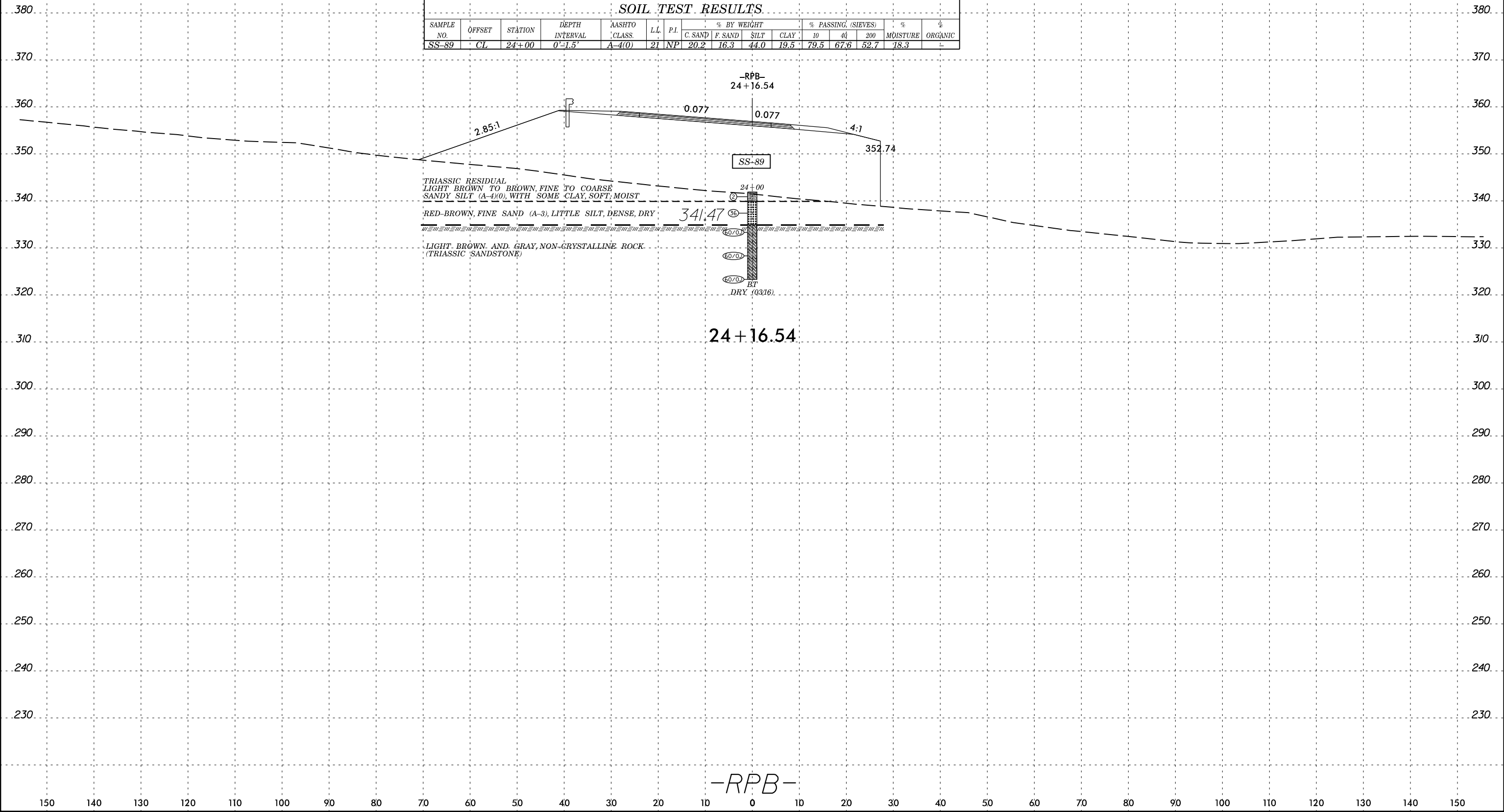
8/23/99



PROJ. REFERENCE NO.	SHEET NO.
I-5506	72

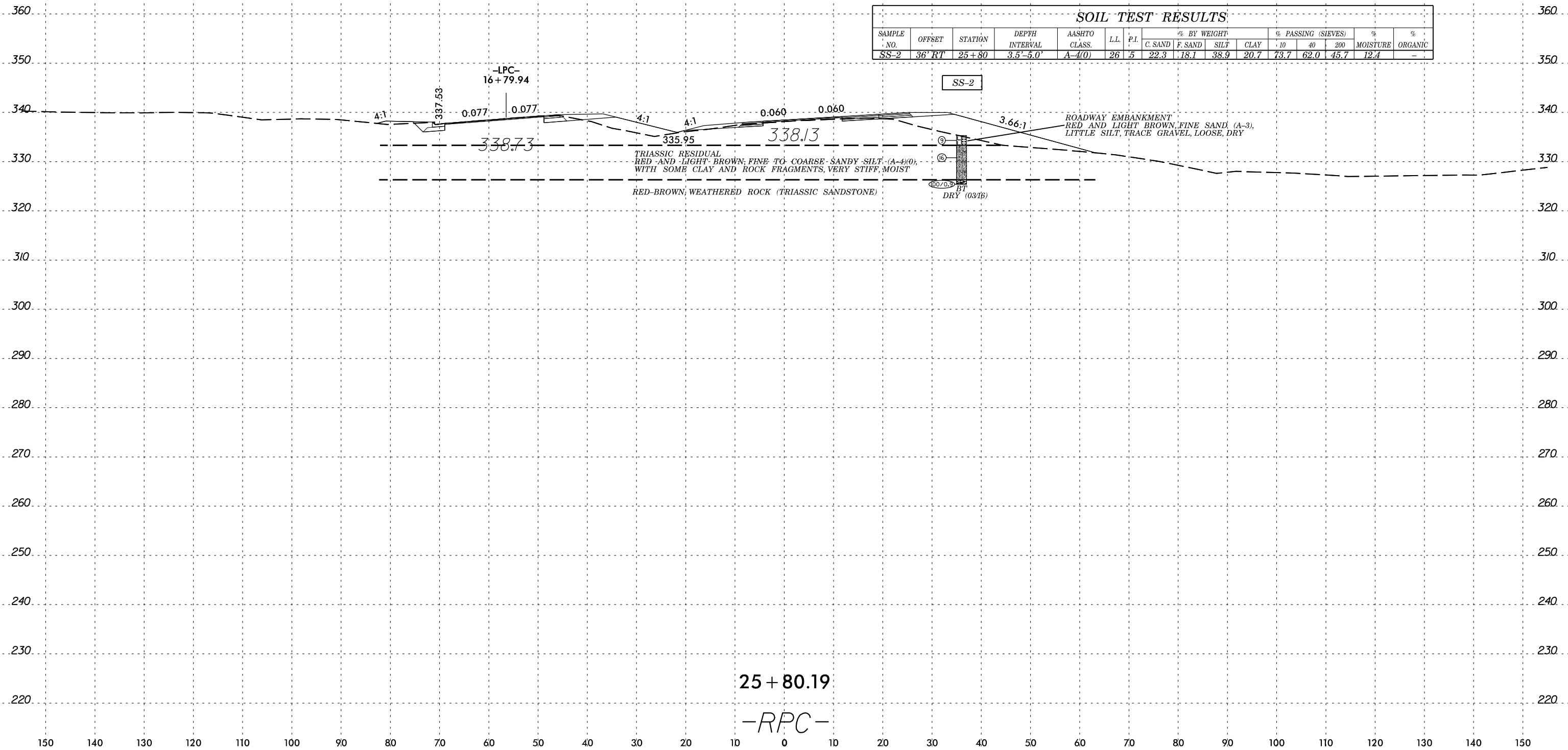


SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-89	CL	24+00	0'-1.5'	A-4(0)	21	NP	20.2	16.3	44.0	19.5	79.5	67.6	52.7	18.3	-



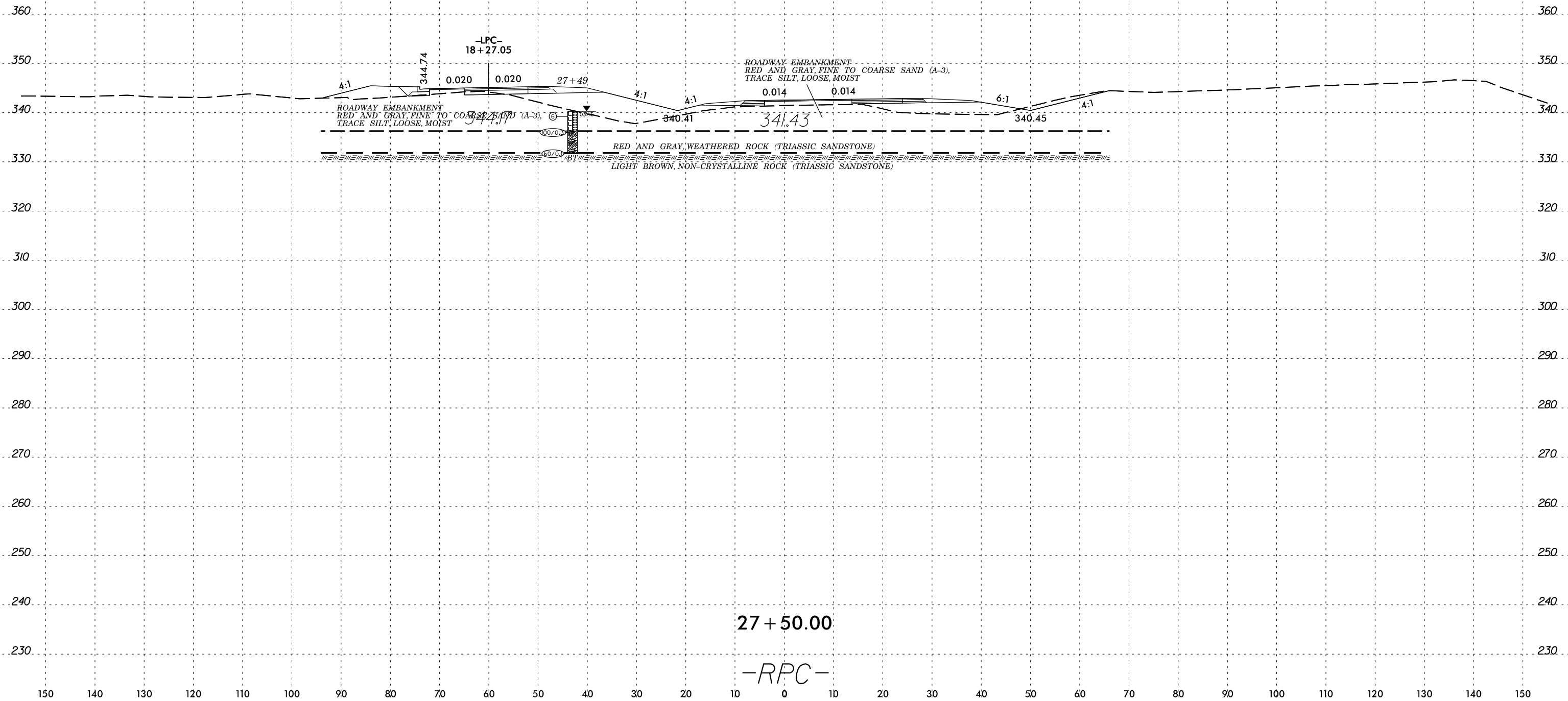
PLOT DRIVER: \$PLTDVRS\$
 USER: \$USER\$
 FILE: \$PWVAVULTPATHDESC\$
 DATE: \$DATE\$
 TIME: \$TIME\$
 PENTABLE: \$PENTBL\$

-RPB-



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
SS-2	36' RT	25+80	3.5'-5.0'	A-4(0)	26	5	22.3	18.1	38.9	20.7	73.7	62.0	45.7	12.4	-

SECTION 16.00
CONSTRUCTION
PLAN
GENERAL NOTES



SECTION \$\$\$\$\$\$
CONSTRUCTION \$\$\$\$\$\$
PLANNING \$\$\$\$\$\$
DESIGN \$\$\$\$\$\$
CONSTRUCTION \$\$\$\$\$\$
OPERATION \$\$\$\$\$\$