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REFERENCE: B-4943

PROJECT: 40110

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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY *DURHAM*

PROJECT DESCRIPTION *BRIDGE NO. 20 OVER DIAL CREEK ON SR 1616 (BAHAMA RD.)*

INVENTORY

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4943	1	17

CAUTION NOTICE

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2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

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DRAWN BY *J.R. SWARTLEY*

CHECKED BY *N.T. ROBERSON*

SUBMITTED BY *N.T. ROBERSON*

DATE *NOVEMBER 2016*

NORTH CAROLINA LICENSED SEAL 2296 GEOLOGIST JARETT SWARTLEY

DocuSigned by: Jarett Swartley 1/10/2017

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SIGNATURE DATE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)				ORGANIC MATERIALS		
GROUP CLASS.	A-1		A-3	A-2				A-4	A-5	A-6	A-7	A-1, A-2 A-3	A-4, A-5 A-6, A-7	
	A-1-a	A-1-b		A-2-4	A-2-5	A-2-6	A-2-7				A-7-5, A-7-6			
SYMBOL														
% PASSING #10 #40 #200	50 MX 30 MX 15 MX		50 MX 25 MX	51 MN 10 MX	35 MX	35 MX	35 MX	35 MX	36 MN	36 MN	36 MN	36 MN	GRANULAR SOILS	SILT-CLAY SOILS	MUCK, PEAT
MATERIAL PASSING #40 LL PI	— 6 MX			— NP	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN	SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER		HIGHLY ORGANIC SOILS
GROUP INDEX	0			0	0		4 MX		8 MX	12 MX	16 MX	NO MX			
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND		FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND					SILTY SOILS		CLAYEY SOILS				
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD								FAIR TO POOR				FAIR TO POOR	POOR	UNSUITABLE

PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTENCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH ($TONS/FT^2$)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE	4	10	40	60	200	270
OPENING (MM)	4.76	2.00	0.42	0.25	0.075	0.053

BOULDER (BLDR.)	COBBLE (COB.)	GRAVEL (GR.)	COARSE SAND (CSE. SD.)	FINE SAND (F SD.)	SILT (SL.)	CLAY (CL.)

GRAIN SIZE	MM	305	75	2.0	0.25	0.05	0.005
	IN.	12	3				

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
LL — LIQUID LIMIT		
PLASTIC RANGE (PI)	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
PL — PLASTIC LIMIT		
OM — OPTIMUM MOISTURE SL — SHRINKAGE LIMIT	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NON PLASTIC	0-5	VERY LOW
SLIGHTLY PLASTIC	6-15	SLIGHT
MODERATELY PLASTIC	16-25	MEDIUM
HIGHLY PLASTIC	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE	LL < 31
MODERATELY COMPRESSIBLE	LL = 31 - 50
HIGHLY COMPRESSIBLE	LL > 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL	
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE	1 - 10%
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE	10 - 20%
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME	20 - 35%
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY	35% AND ABOVE

GROUND WATER

- WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
- STATIC WATER LEVEL AFTER 24 HOURS
- PW — PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
- SPRING OR SEEP

MISCELLANEOUS SYMBOLS

- ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION
- SOIL SYMBOL
- ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT
- INFERRED SOIL BOUNDARY
- INFERRED ROCK LINE
- ALLUVIAL SOIL BOUNDARY
- 25/025 — DIP & DIP DIRECTION OF ROCK STRUCTURES
- SPT, DPT, DMT, VST, PMT — TEST BORING
- AUGER BORING
- CORE BORING
- MW — MONITORING WELL
- PIEZOMETER INSTALLATION
- SLOPE INDICATOR INSTALLATION
- CONE PENETROMETER TEST
- SOUNDING ROD
- TEST BORING WITH CORE
- SPT N-VALUE

RECOMMENDATION SYMBOLS

- UNDERCUT
- SHALLOW UNDERCUT
- UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE
- UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK
- UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL

ABBREVIATIONS

AR - AUGER REFUSAL
BT - BORING TERMINATED
CL. - CLAY
CPT - CONE PENETRATION TEST
CSE. - COARSE
DMT - DILATOMETER TEST
DPT - DYNAMIC PENETRATION TEST
e - VOID RATIO
F - FINE
FOSS. - FOSSILIFEROUS
FRAC. - FRACTURED, FRACTURES
FRAGS. - FRAGMENTS
HI. - HIGHLY
MED. - MEDIUM
MICA. - MICACEOUS
MOD. - MODERATELY
NP - NON PLASTIC
ORG. - ORGANIC
PMT - PRESSUREMETER TEST
SAP. - SAPROLITIC
SD. - SAND, SANDY
SL. - SILT, SILTY
SLI. - SLIGHTLY
TCR - TRICONE REFUSAL
w - MOISTURE CONTENT
V - VERY
VST - VANE SHEAR TEST
WEA. - WEATHERED
γ - UNIT WEIGHT
γ_d - DRY UNIT WEIGHT

SAMPLE ABBREVIATIONS

S - BULK
SS - SPLIT SPOON
ST - SHELBY TUBE
RS - ROCK
RT - RECOMPACTED TRIAXIAL
CBR - CALIFORNIA BEARING RATIO

EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:
- [X] CME-45C
- [X] CME-55
- [ ] CME-550
- [ ] VANE SHEAR TEST
- [ ] PORTABLE HOIST
- [X] BARGE
- [ ] _____

ADVANCING TOOLS:
- [ ] CLAY BITS
- [ ] 6" CONTINUOUS FLIGHT AUGER
- [X] 8" HOLLOW AUGERS
- [ ] HARD FACED FINGER BITS
- [ ] TUNG.-CARBIDE INSERTS
- [X] CASING [X] W/ ADVANCER
- [ ] TRICONE _____" STEEL TEETH
- [ ] TRICONE _____" TUNG.-CARB.
- [X] CORE BIT
- [ ] _____

HAMMER TYPE:
- [X] AUTOMATIC [ ] MANUAL

CORE SIZE:
- [ ] -B _____ [ ] -H _____
- [X] -N X

HAND TOOLS:
- [ ] POST HOLE DIGGER
- [ ] HAND AUGER
- [ ] SOUNDING ROD
- [ ] VANE SHEAR TEST
- [ ] _____

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.
ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)	NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
CRYSTALLINE ROCK (CR)	FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
NON-CRYSTALLINE ROCK (NCR)	FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTARY ROCK (CP)	COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH	ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
VERY SLIGHT (V SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
SLIGHT (SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
MODERATE (MOD.)	SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
MODERATELY SEVERE (MOD. SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*
SEVERE (SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*
VERY SEVERE (V SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*
COMPLETE	ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD	CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
MEDIUM HARD	CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
SOFT	CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
VERY SOFT	CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE	RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
MODERATELY INDURATED	GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
INDURATED	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
EXTREMELY INDURATED	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.

FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.

FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.

RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK: •see note

ELEVATION: FEET

NOTES:

- elevations derived from geopak and the .tin file 'B4943_ls_tin.tin' dated 1/27/2016

DATE: 8-15-14

PROJECT REFERENCE NO.	SHEET NO.
B-4943	2A

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

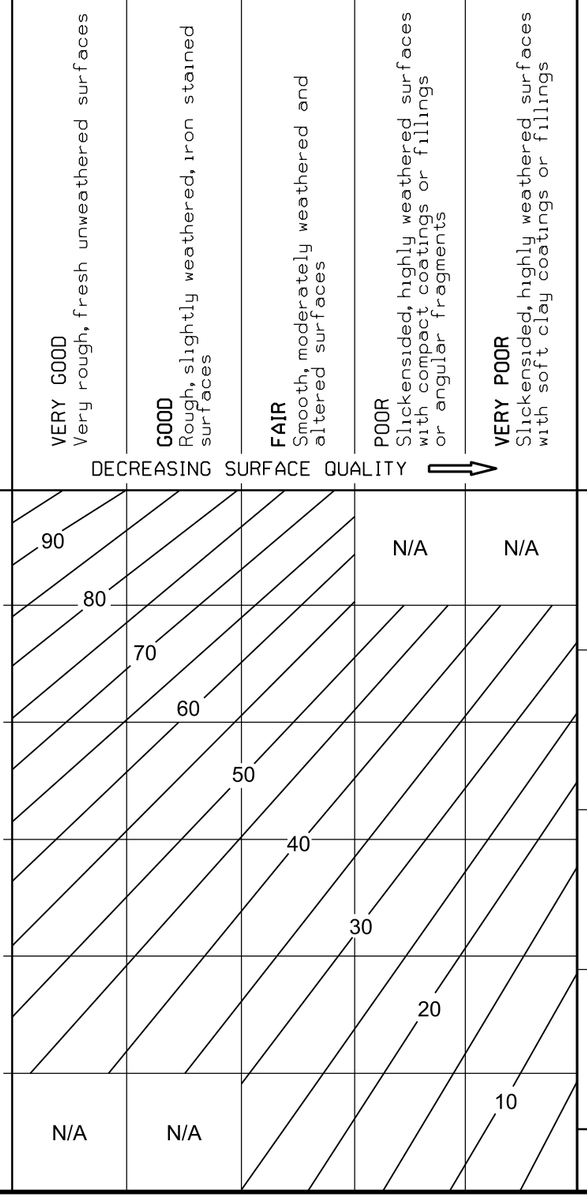
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)

From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.

STRUCTURE

- INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities
- BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets
- VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets
- BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity
- DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces
- LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes

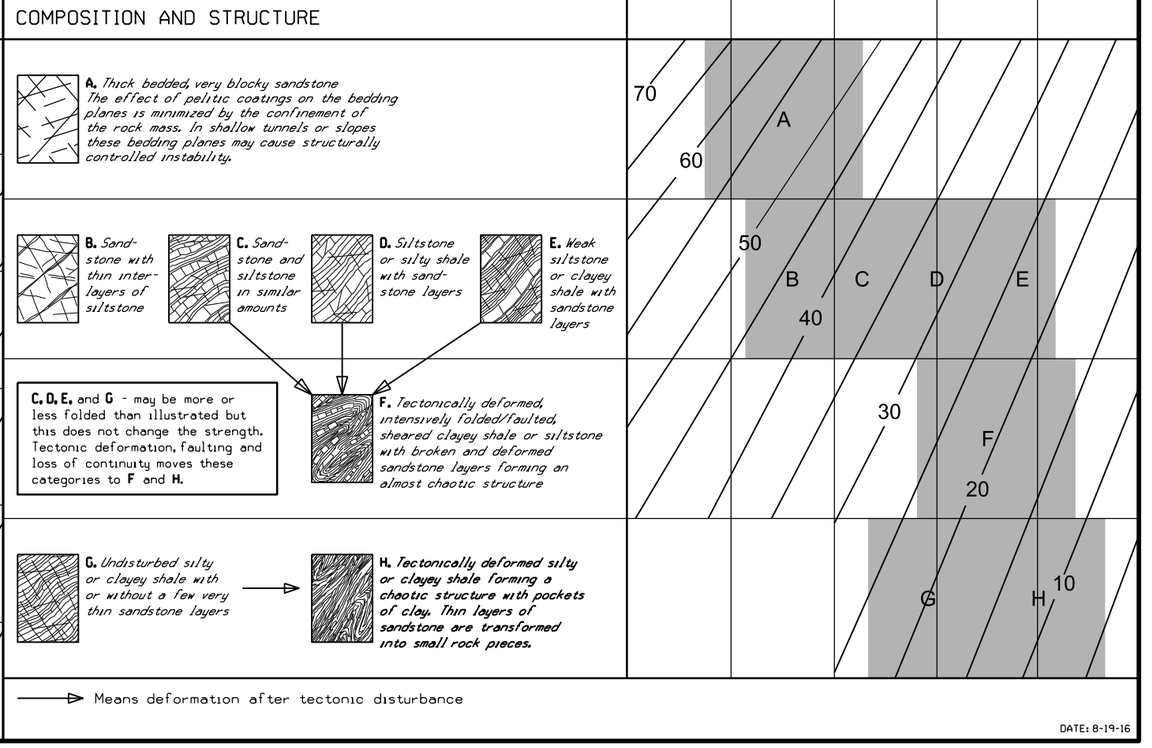


AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)

From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.

COMPOSITION AND STRUCTURE

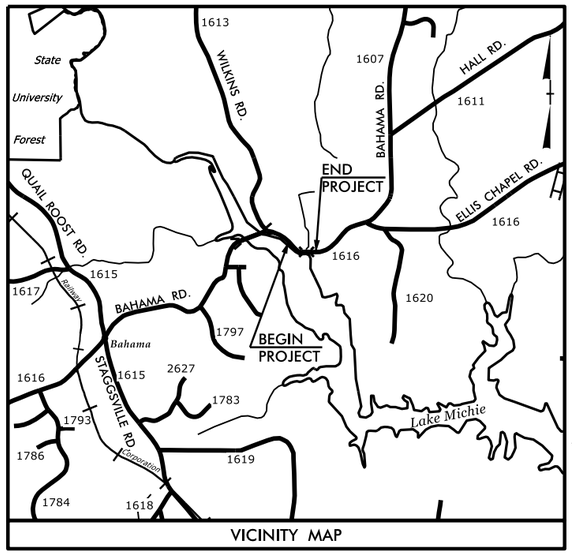


DATE: 8-19-16

09/08/99

T.I.P PROJECT: B-4943

CONTRACT:



25% PLANS SUBMITTAL

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

DURHAM COUNTY

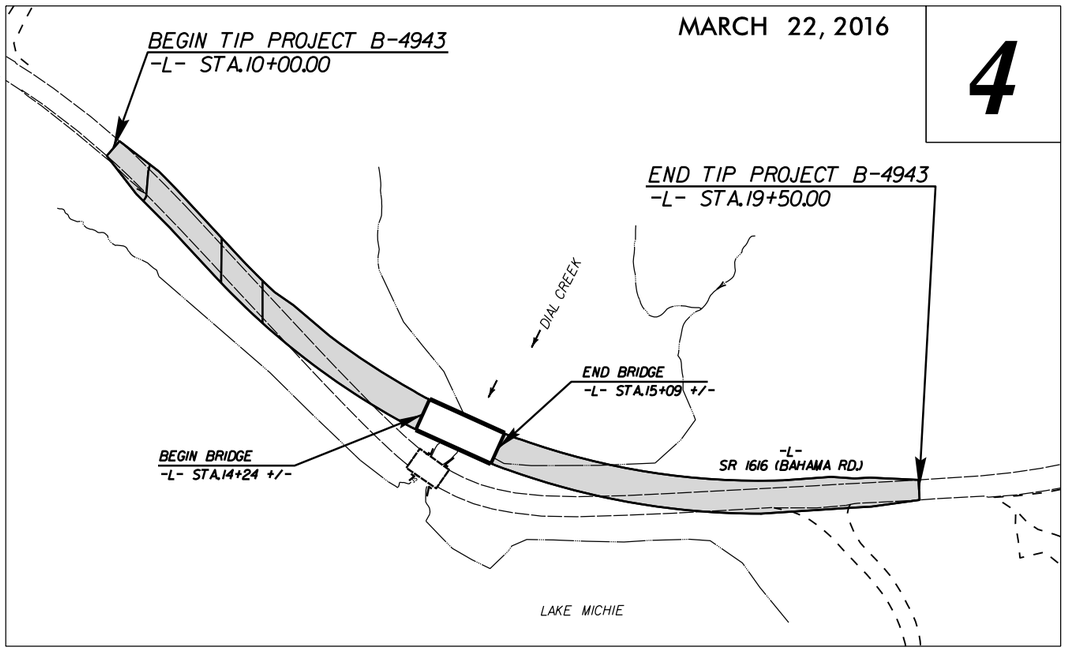
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4943	3	17
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
40110.1.1	BRZ-1616(10)	PE	

25% APPROVED PLANS

LOCATION: *BRIDGE NO. 20 OVER DIAL CREEK ON SR 1616 (BAHAMA RD.)*

TYPE OF WORK: *GRADING, DRAINAGE, PAVING, AND STRUCTURE*

NAD 83/NSRS 2007



INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

GRAPHIC SCALES

PLANS 50 25 0 50 100

PROFILE (HORIZONTAL) 50 25 0 50 100

PROFILE (VERTICAL) 10 5 0 10 20

DESIGN DATA

2017 ADT = 2738 VPD
2037 ADT = 3585 VPD
K = 10%
D = 85%
T = 6% *
V = 40 MPH
* (TTST 1% + DUAL 5%)
FUNC. CLASS. = RURAL MINOR COLLECTOR
SUBREGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-4943	=	0.145 mi.
LENGTH STRUCTURES TIP PROJECT B-4943	=	0.016 mi.
TOTAL LENGTH TIP PROJECT B-4943	=	0.161 mi.

Prepared in the Offices of:

STEWART
421 FAYETTEVILLE ST, STE 400
RALEIGH, NC 27601
T 919.380.8750

NC FIRM LICENSE No: F-1146
1151 SE Cary Parkway, Suite 101
Cary, NC 27518
(919) 55-COVER
Firm License #: C-1051
www.stewartpc.com
PROJECT #:#13013.00

ECOLOGICAL ENGINEERING

2012 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
DECEMBER 16, 2016

LETTING DATE:
DECEMBER 19, 2017

ANDY YOUNG, PE
PROJECT ENGINEER

MICHAEL BURNS, EI
PROJECT DESIGN ENGINEER

REKHA PATEL, PE
NCDOT CONTACT

HYDRAULICS ENGINEER

SIGNATURE: *P.E.*

ROADWAY DESIGN ENGINEER

SIGNATURE: *P.E.*

STATE OF NORTH CAROLINA • DEPARTMENT OF TRANSPORTATION •

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Transportation

PAT McCRORY
Governor

NICHOLAS J. TENNYSON
Secretary

December 5, 2016

STATE PROJECT: 40110.1.1 (B-4943)
FEDERAL PROJECT: BRZ-1616(10)
COUNTY: Durham
DESCRIPTION: Bridge No. 20 on SR 1616 (Bahama Rd.) over Dial Creek

SUBJECT: Geotechnical Report – Inventory

Project Description

This project lies just east of the town of Bahama in northern Durham County. The project consists of realigning Bahama Rd. (-L-) near the bridge over Dial Creek to reduce the severity of the curve. Along the east approach approximately 4 to 9 feet of surface water from Dial Creek covers the proposed alignment until it ties back in with existing Bahama road. The total roadway project length is 0.145 miles.

Twelve SPT borings were performed at various offset locations along the -L- alignment by the NCDOT Geotechnical Engineering Unit. The work was performed from August through November of 2016. A track-mounted CME-55 and a CME-45C mounted on a barge was used during the field investigation. ALL Carolina Crane & Equipment was contracted by the NCDOT to place the barge and rig into Lake Michie. Representative samples were collected for visual classification in the field and were submitted for laboratory analysis by the Materials and Tests Unit.

The following alignment, totaling 0.145 miles was investigated. Subsurface cross-sections of this alignment are included in this report.

Line	Stations
-L-	10+00 to 19+50

Areas of Special Geotechnical Interest

1) Soft Soils: Very soft, alluvial, wet clays were encountered on the project at the following locations:

Line	Station	Offset
-L-	14+75-16+75	Varies

Physiography and Geology

The project is located in the Piedmont physiographic province of North Carolina. The project corridor is rural residential. A mixture of woods and grassy fields lie along the project corridor. The terrain is rolling with some moderate existing cuts. Geologically the soils in this region are derived from the underlying meta-volcanics and meta-granites belonging to the Carolina Slate Belt. These rocks were formed by regional metamorphism in this area during the Permian Period.

Soil Properties

Soils encountered during this investigation are separated into 3 categories: Roadway Embankment, Alluvial and Residual soils.

Roadway Embankment soils are likely derived from nearby sources and are similar to Residual soils in composition. These soils generally consist of stiff, brown and tan, sandy silt (A-4).

The Alluvial soils are derived from the fluvial and lacustrine setting of this area where the mouth of Dial Creek meets Lake Michie. Clay and silt are common in this environment caused by the quiet settling of suspended particles. These soils consist of very soft, gray, silty clay (A-7-6), sandy clay (A-6) and loose, silty sand (A-2-4).

Residual soils are derived from the weathering of underlying rock in the area. These soils consist of gray, tan, and brown, stiff, saprolitic, sandy silt (A-4), sandy clay (A-6) and dense, saprolitic, silty sand (A-2-4).

Groundwater

Groundwater is near the surface water elevation of Dial Creek and Lake Michie. The water elevation of Dial Creek and Lake Michie was recorded during a period of above average rainfall.

Undisturbed Samples

An undisturbed thin wall Shelby tube sample was collected and submitted for testing at the following location:

Sample No.	Station (-L-)	Depth	Test
ST-1	15+75	2.0-4.0	Consolidation, Triaxial CU

Respectfully submitted,

Jarett Swartley, L.G.
Project Geological Engineer

PROJECT REFERENCE NO.	SHEET NO.
B-4943	4

SITE PLAN

0 100 200
FEET

10

15

20

NAD 83/
NSRS 2007

TO BAHAMA

16' GRAVEL

24" RCP

18" CMP

MTL

BL-101

GRAU 350 ±

WOODS

DIAL CREEK

4" WW

CONC HW

BL-102

LAKE MICHIE

GRAU 350 ±

EXISTING R/W

UNDEFINABLE

BAHAMA ROAD (SR 1616)
20.5' PAVED ROADWAY

TO ELLIS CHAPEL RD.

60.0'

EXISTING R/W

GRAVEL

GRAVEL & GRASS

ISBLKD

SAT. DISH

11' GRAVEL

BL-103

ATC

GATE

WOODS

13.5' GRAVEL

6/23/16

0 2.5 5	PROJ. REFERENCE NO.	SHEET NO.
	B-4943	5

RESIDUAL, TAN AND RED, MOIST, STIFF, SANDY CLAY AND SANDY SILT

WEATHERED ROCK (META-GRANITE)

RESIDUAL, TAN, MOIST, HARD, SANDY SILT

WEATHERED ROCK (META-GRANITE)

2:1

4:1

2:1

356.23

358.29

LAKE MICHIE/DIAL CREEK (10/19/16)

10+50.00

RESIDUAL, TAN AND RED, MOIST, STIFF, SANDY CLAY AND SANDY SILT

WEATHERED ROCK (META-GRANITE)

RESIDUAL, TAN, MOIST, HARD, SANDY SILT

WEATHERED ROCK (META-GRANITE)

2:1

4:1

4:1

2:1

355.68

357.71

355.11

LAKE MICHIE/DIAL CREEK (10/19/16)

10+00.00

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0 2.5 5	PROJ. REFERENCE NO.	SHEET NO.
	B-4943	6

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

390 385 380 375 370 365 360 355 350 345 340 335 330

SS-1
SS-2

8
12
100/0.6
72
100/1.0
100/0.4
100/0.2
DRY
8/16

RESIDUAL, TAN AND RED, MOIST, STIFF, SANDY CLAY AND SANDY SILT

WEATHERED ROCK (META-GRANITE)

RESIDUAL, TAN, MOIST, HARD, SANDY SILT

WEATHERED ROCK (META-GRANITE)

4:1

356.02

358.14

1.5:1

LAKE MICHIE/DIAL CREEK (10/19/16)

11+00.00

—L—

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0 2.5 5	PROJ. REFERENCE NO.	SHEET NO.
	B-4943	7

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

385 380 375 370 365 360 355 350 345 340 335 330

RESIDUAL, TAN AND RED, MOIST, STIFF, SANDY CLAY AND SANDY SILT

WEATHERED ROCK (META-GRANITE)

RESIDUAL, TAN, MOIST, HARD, SANDY SILT

WEATHERED ROCK (META-GRANITE)

4:1

355.64

Ⓐ ROADWAY EMBANKMENT, TAN, MOIST, MED. STIFF, SANDY SILT

357.38

Ⓐ

1.5:1

LAKE MICHIE/DIAL CREEK (10/19/16)

11+50.00

-L-

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0 2.5 5	PROJ. REFERENCE NO.	SHEET NO.
	B-4943	8

SS-3

12+70

5
32
44
100/0.7
100/0.6
100/0.5
100/0.3
DRY
8/16

2:1

Ⓐ ROADWAY EMBANKMENT, TAN, MOIST, MED. STIFF, SANDY SILT

RESIDUAL, TAN, MOIST, MED. STIFF TO HARD, SANDY SILT

2:1

351.20

355.67

Ⓐ

1.5:1

LAKE MICHIE/DIAL CREEK (10/19/16)

WEATHERED ROCK (META-GRANITE)

12+50.00

2:1

Ⓐ ROADWAY EMBANKMENT, TAN, MOIST, MED. STIFF, SANDY SILT

4:1

RESIDUAL, TAN, MOIST, MED. STIFF TO HARD, SANDY SILT

354.67

354.53

Ⓐ

1.5:1

LAKE MICHIE/DIAL CREEK (10/19/16)

WEATHERED ROCK (META-GRANITE)

12+00.00

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0 2.5 5	PROJ. REFERENCE NO.	SHEET NO.
	B-4943	9

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

370 365 360 355 350 345 340 335 330

LAKE MICHIE/DIAL CREEK (10/19/16)

6
62
36
100/0.8
60/0.1
DRY
8/16

2:1

Ⓐ ROADWAY EMBANKMENT, TAN, MOIST, MED. STIFF, SANDY SILT

RESIDUAL, TAN, MOIST, MED. STIFF TO HARD, SANDY SILT

348.94

354.64

350.48

WEATHERED ROCK (META-GRANITE)

CRYSTALLINE ROCK (META-GRANITE)

LAKE MICHIE/DIAL CREEK (10/19/16)

13+50.00

Ⓐ ROADWAY EMBANKMENT, TAN, MOIST, MED. STIFF, SANDY SILT

2:1

RESIDUAL, TAN, MOIST, MED. STIFF TO HARD, SANDY SILT

349.82

354.33

WEATHERED ROCK (META-GRANITE)

LAKE MICHIE/DIAL CREEK (10/19/16)

13+00.00

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75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

Ⓐ RESIDUAL, TAN, MOIST, MED. STIFF TO HARD, SANDY SILT

SS-4

14+24

14+24

355 350 345 340 335

LAKE MICHIE/DIAL CREEK (10/19/16)

2:1

353.42

Ⓐ

6 54 100/0.4 100/0.3 60/0.1

DRY 8/16

WEATHERED ROCK (META-GRANITE)

CRYSTALLINE ROCK, (META-GRANITE)

7 100/0.8 100/0.3 100/0.2 100/1.0 100/0.3 60/0.1

349.87

DRY 8/16

LAKE MICHIE/DIAL CREEK (10/19/16)

14+23.00

355 350 345 340 335 330

LAKE MICHIE/DIAL CREEK (10/19/16)

2:1

348.46

355.50

350.00

RESIDUAL, TAN, MOIST, MED. STIFF TO HARD, SANDY SILT

WEATHERED ROCK (META-GRANITE)

CRYSTALLINE ROCK (META-GRANITE)

LAKE MICHIE/DIAL CREEK (10/19/16)

330 325 320

14+00.00

—L—

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

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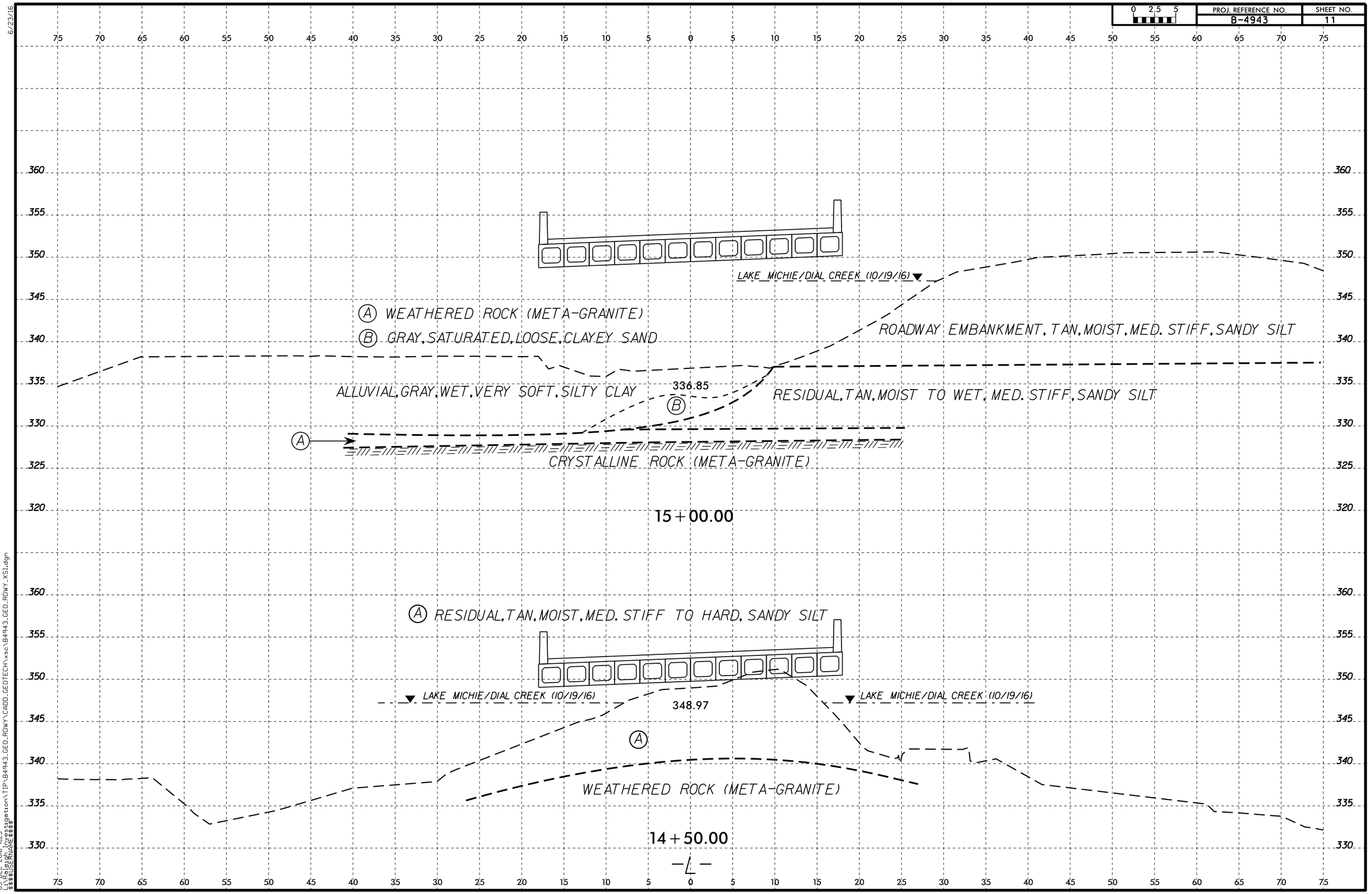

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0 2.5 5	PROJ. REFERENCE NO.	SHEET NO.
	B-4943	12

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

Ⓐ WEATHERED ROCK (META-GRANITE)
Ⓑ GRAY, SATURATED, LOOSE, CLAYEY SAND
Ⓒ (BOULDERS)

360 355 350 345 340 335 330 325 320 315 310 305 300

0.040 0.040

2:1

1.5:1

LAKE MICHIE/DIAL CREEK (10/19/16)

15+10

15+10

15+10

ROADWAY EMBANKMENT, TAN, MOIST, SANDY SILT

④ ⑤ ⑦

MED. STIFF, RESIDUAL, TAN, MOIST, MED. STIFF, SANDY SILT

ALLUVIAL, VERY SOFT, WOH ⑩

GRAY, WET, SILTY CLAY

338.02 WOH ⑭

60/0.0 60/0.0 60/0.1

Ⓐ Ⓑ Ⓒ

CRYSTALLINE ROCK, GRAY, TAN AND BLACK, MODERATELY WEATHERED, SOFT TO HARD, CLOSE FRACTURE SPACING, META-GRANITE

REC=86%
RQD=30%
GSI=28-32

REC=78%
RQD=23%
GSI=28-32

15+09.00

FIAD

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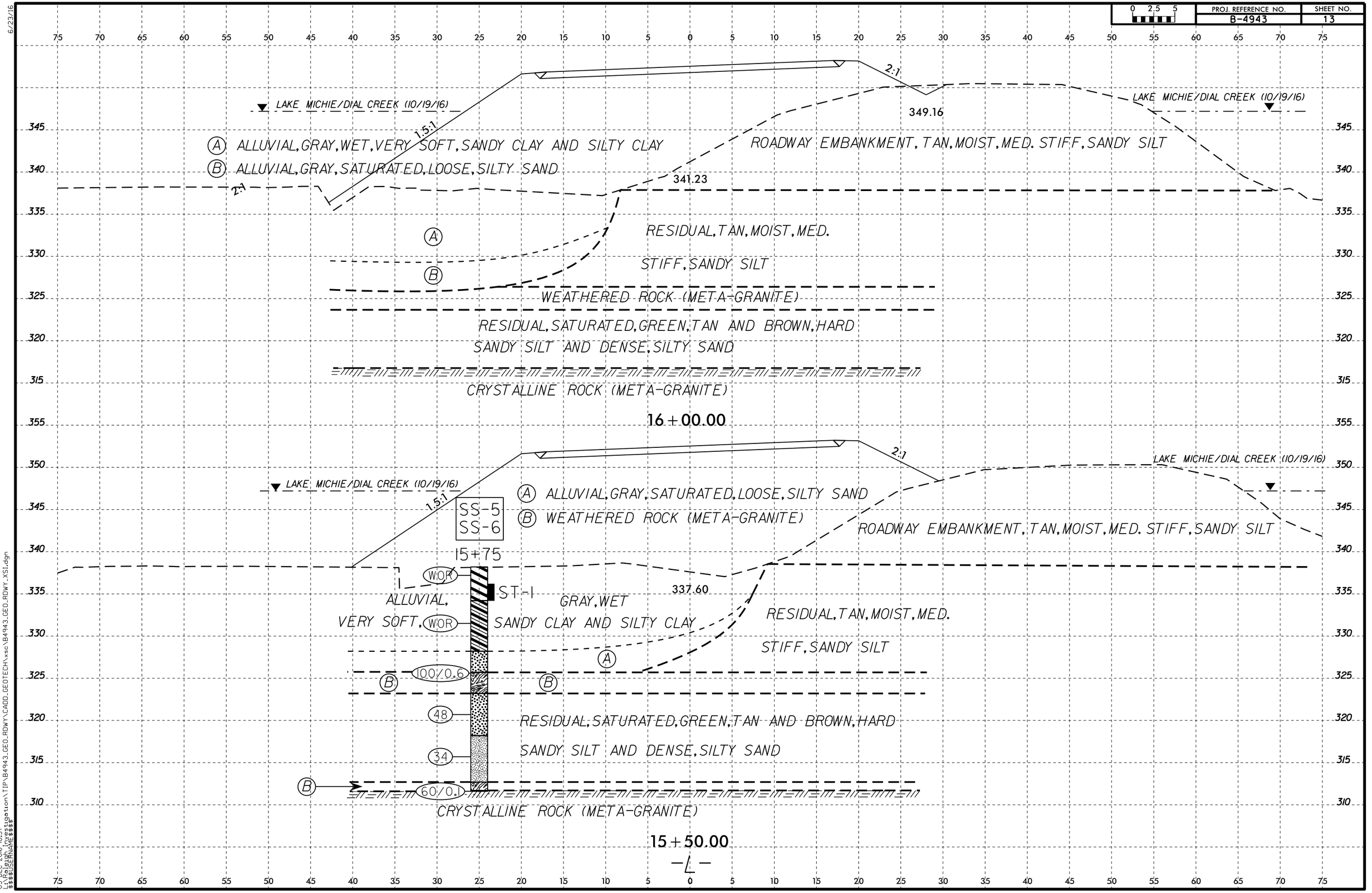

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0 2.5 5	PROJ. REFERENCE NO.	SHEET NO.
	B-4943	14

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

355 350 345 340 335 330 325 320

Ⓐ ALLUVIAL, GRAY AND TAN, WET, VERY SOFT TO MED. STIFF, SILTY CLAY AND SANDY SILT

Ⓑ GRAY, SATURATED, LOOSE, SILTY SAND

LAKE MICHIE/DIAL CREEK (10/19/16)

1.5:1

352.13

1.5:1

LAKE MICHIE/DIAL CREEK (10/19/16)

ROADWAY EMBANKMENT, TAN, MOIST, MED. STIFF, SANDY SILT

Ⓐ

RESIDUAL, TAN, MOIST, MED.

Ⓑ

STIFF, SANDY SILT

WEATHERED ROCK (META-GRANITE)

CRYSTALLINE ROCK (META-GRANITE)

17+00.00

335 360 355 350 345 340 335 330 325 320

Ⓐ ALLUVIAL, GRAY AND TAN, WET, VERY SOFT TO MED. STIFF, SILTY CLAY AND SANDY SILT

Ⓑ GRAY, SATURATED, LOOSE, SILTY SAND

LAKE MICHIE/DIAL CREEK (10/19/16)

1.5:1

2:1

348.57

2:1

349.67

LAKE MICHIE/DIAL CREEK (10/19/16)

ROADWAY EMBANKMENT, TAN, MOIST, MED. STIFF, SANDY SILT

WOR

6

Ⓐ

RESIDUAL, TAN, MOIST, MED.

5

Ⓑ

STIFF, SANDY SILT

100/0.3

60/0.1

WEATHERED ROCK (META-GRANITE)

16+50.00

—L—

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

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6/23/16

0 2.5 5	PROJ. REFERENCE NO.	SHEET NO.
	B-4943	15

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

355 350 345 340 335 330 325 320 315 310

Ⓐ ROADWAY EMBANKMENT, TAN, MOIST, MED. STIFF, SANDY SILT

18+20

2:1

356.91

Ⓐ

2:1

LAKE MICHIE/DIAL CREEK (10/19/16)

5
20
66
27
100/0.8
100/0.3
100/0.8
60/0.1

RESIDUAL, TAN AND BROWN, MOIST TO WET, MED. STIFF TO HARD, SANDY SILT

WEATHERED ROCK (META-GRANITE)

DRY 9/16

CRYSTALLINE ROCK (META-GRANITE)

18+00.00

Ⓐ (BOULDERS)

17+65

1.5:1

354.33

1.5:1

LAKE MICHIE/DIAL CREEK (10/19/16)

LAKE MICHIE/DIAL CREEK (10/19/16)

5
19
60/0.0

DRY 9/16

Ⓐ

ROADWAY EMBANKMENT, TAN, MOIST, MED. STIFF, SANDY SILT

RESIDUAL, TAN, MOIST, MED.

STIFF, SANDY SILT

WEATHERED ROCK (META-GRANITE)

CRYSTALLINE ROCK (META-GRANITE)

17+50.00

6/23/16

0 2.5 5	PROJ. REFERENCE NO.	SHEET NO.
	B-4943	16

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

370 365 360 355 350

2:1 4:1 4:1 2:1

363.99 366.44 364.01

RESIDUAL, TAN AND BROWN, MOIST TO WET, MED. STIFF TO HARD, SANDY SILT

19+50.00

365 360 355

3:1 3:1

362.92

RESIDUAL, TAN AND BROWN, MOIST TO WET, MED. STIFF TO HARD, SANDY SILT

19+00.00

350 345 340 335 330

2:1 3:1

359.69

RESIDUAL, TAN AND BROWN, MOIST TO WET, MED. STIFF TO HARD, SANDY SILT

18+50.00

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B-4943	17

SOIL TEST RESULTS

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-1	65' LT	11+00	0.0-1.5	A-6(9)	40	15	15.9	21.9	18.0	44.2	99	92	66	-	-
SS-2	65' LT	11+00	3.5-5.0	A-4(6)	37	10	16.1	24.5	25.2	34.2	100	93	65	14.5	-
SS-3	55' LT	12+70	3.6-5.1	A-4(0)	26	2	37.2	27.5	19.2	16.1	100	76	41	-	-
SS-4	9' LT	14+24	3.3-4.8	A-4(0)	28	7	39.4	26.1	14.4	20.1	98	73	39	-	-
SS-5	25' LT	15+75	5.7-7.2	A-6(5)	31	12	17.9	22.6	31.2	28.2	100	92	63	46.5	-
SS-6	25' LT	15+75	16.5-18.0	A-2-4(0)	35	NP	42.5	31.9	21.6	4.0	100	71	33	-	-