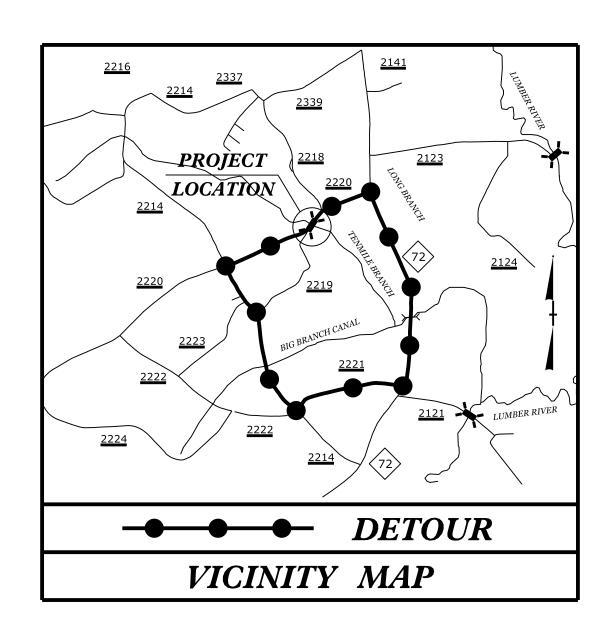
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STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

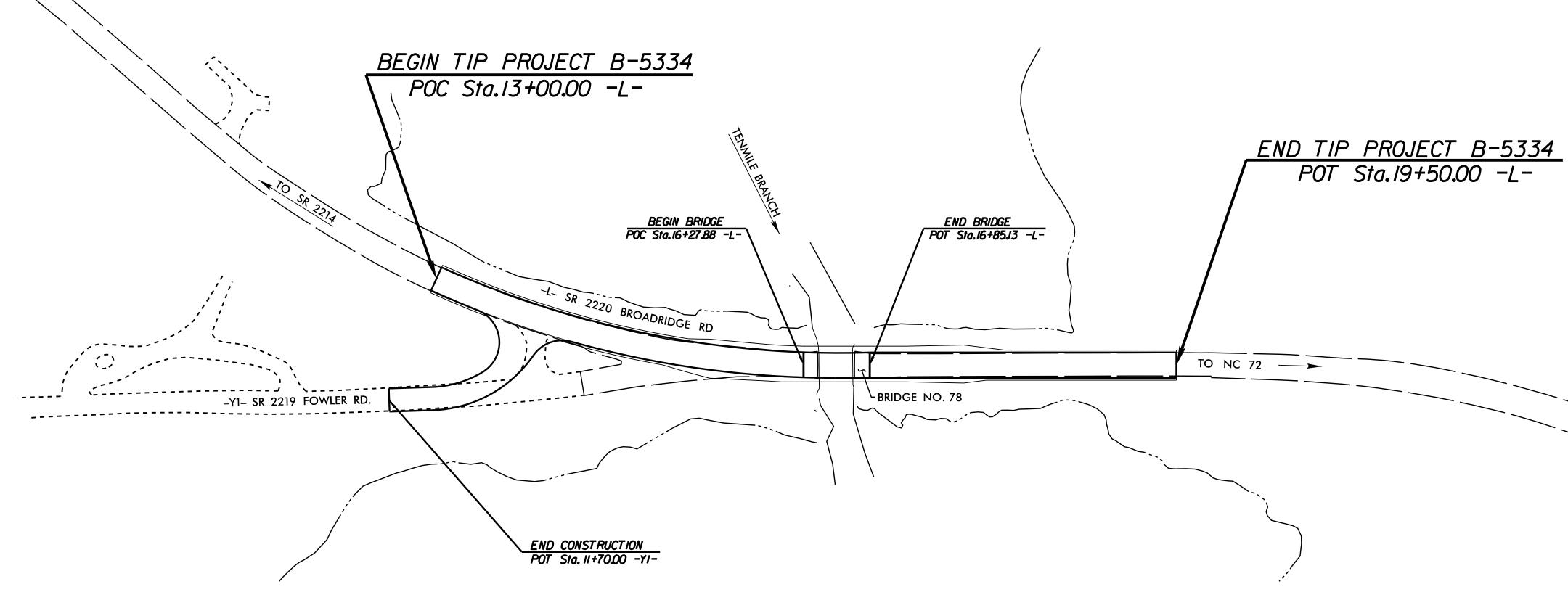
DIVISION OF HIGHWAYS

ROBESON COUNTY

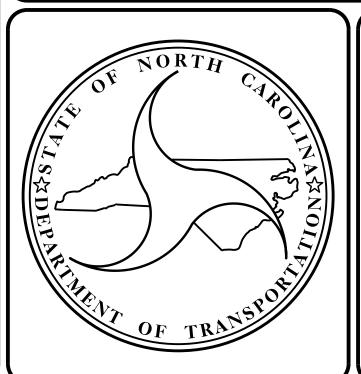
LOCATION: REPLACE BRIDGE 78 OVER TENMILE BRANCH
ON SR 2220 (NORTH BROADRIDGE ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE

STATE	STATE	PROJECT REFERENCE NO.		TAL EETS
N.C.	E			
STAT	E PROJ. NO.	F. A. PROJ. NO.	DESCRIPTION	
46	048.1.1	BRZ-2220(4)	P.E.	
46	048.2.1	_	ROW,UTIL	
46	048.3.1		CONST	



STRUCTURE



DESIGN DATA

ADT 2017 = 1245 ADT 2037 = 1973 K = 10 %

D = 60 %

T = 5 % *
V = 60 MPH
* (TTST 1 %, DUAL 4 %)

FUNC CLASS = LOCAL
SUB-REGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-5334 = 0.112 MILE LENGTH STRUCTURE TIP PROJECT B-5334 = 0.011 MILE

TOTAL LENGTH TIP PROJECT B-5334 = 0.123 MILE

Prepared in the Office of: DIVISION OF HIGHWAYS STRUCTURES MANAGEMENT UNIT

STRUCTURES MANAGEMENT UNIT 1000 BIRCH RIDGE DR. RALEIGH, N.C. 27610

2012 STANDARD SPECIFICATIONS

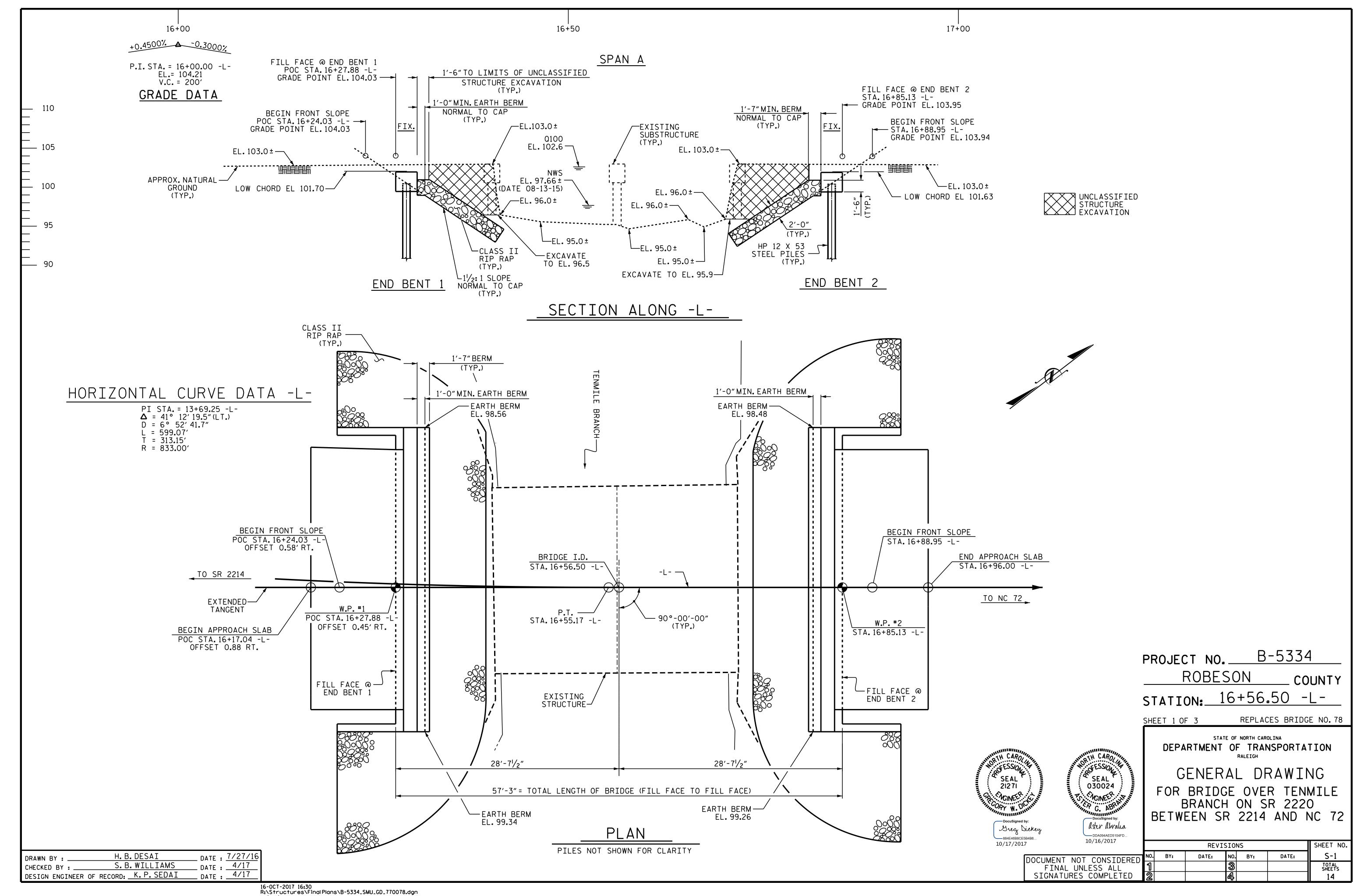
LETTING DATE: DECEMBER 19, 2017

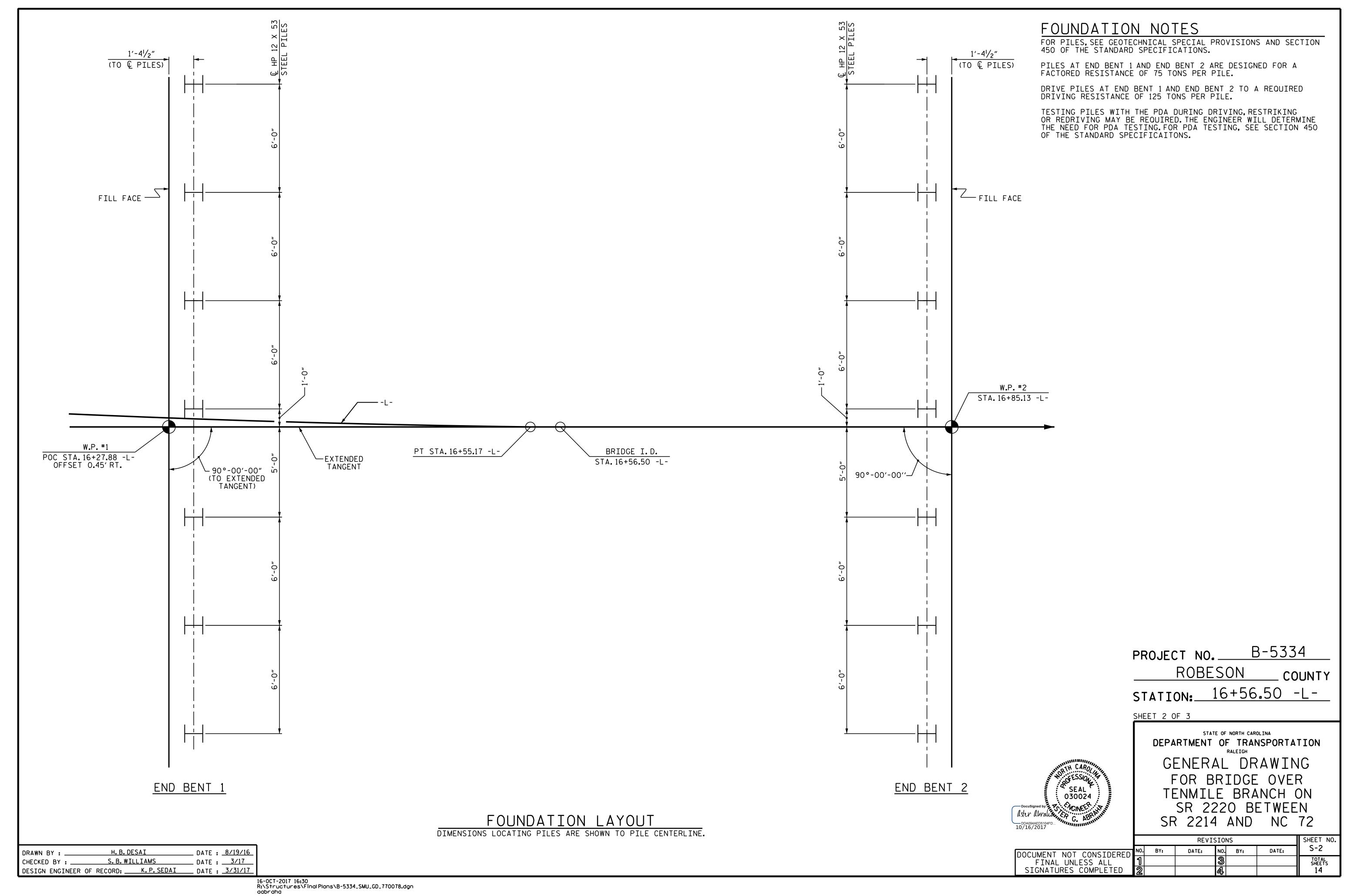
GREG W. DICKEY, P.E.

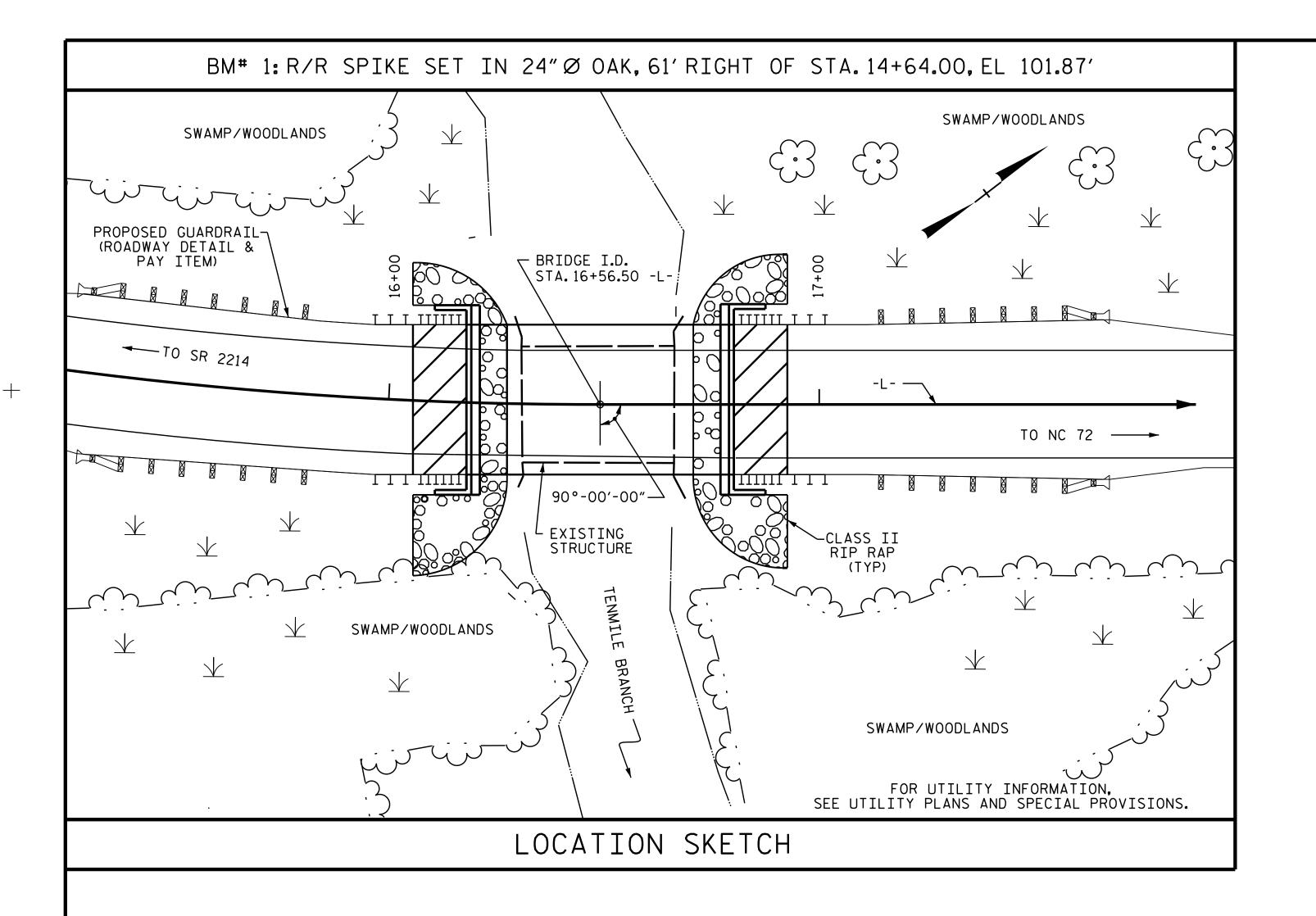
PROJECT ENGINEER

ASTER G. ABRAHA, P.E.

PROJECT DESIGN ENGINEER







NOTES

ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 2.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK & FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 40 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN @ 16'-2", 1 SPAN @ 15'-6", WITH A REINFORCED CONCRETE FLOOR ON TIMBER JOIST AND TIMBER CAPS AND PILES AT END BENTS AND INTERIOR BENTS WITH A CLEAR ROADWAY WIDTH OF 24'-1" LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED.

FOR STRUCTURE REMOVAL, SEE SPECIAL PROVISIONS.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES".

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

HYDRAULIC DATA

DESIGN DISCHARGE = 850 C.F.S. FREQUENCY OF DESIGN DISCHARGE = 25 YRS.

DESIGN HIGH WATER ELEVATION = 101.6

DRAINAGE AREA = 7.63 SQ.MI.

BASE DISCHARGE (Q100) = 1300 C.F.S.

BASE HIGH WATER ELEVATION = 102.6 C.F.S.

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE = 2475 C.F.S. FREQUENCY OF OVERTOPPING = 500+ YRS. OVERTOPPING ELEVATION = 103.7

	TOTAL BILL OF MATERIAL																
	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	HP STEE	12X53 EL PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0 PRES CO CORR	O"X 1'-9" STRESSED NCRETE RED SLABS	ASBESTOS ASSESSMENT
	LUMP SUM	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	EACH	NO.	LIN.FT.	EACH	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.	LUMP SUM
SUPERSTRUCTURE					LUMP SUM						110.25			LUMP SUM	11	605.00	
END BENT NO.1			LUMP SUM	14.2		2,115	7	7	385	7		185	195				
END BENT NO.2			LUMP SUM	14.2		2,115	7	7	385	7		205	215				
TOTAL	LUMP SUM	1	LUMP SUM	28.4	LUMP SUM	4,230	14	14	770	14	110.25	390	410	LUMP SUM	11	605.00	LUMP SUM

DocuSigned by:

Uster Abraha

DDA094AED5104FD...

10/16/2017

PROJECT NO. B-5334

ROBESON COUNTY

STATION: 16+56.50 -L-

SHEET 3 OF 3

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING
FOR BRIDGE OVER TENMILE
BRANCH ON SR 2220
BETWEEN SR 2214 AND NC 72

REVISIONS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2

REVISIONS

REVISIONS

SHEET NO. BY: DATE: NO. BY: DATE: S-3

TOTAL SHEETS

14

DRAWN BY: H.B.DESAI DATE: 8-19-16
CHECKED BY: S.B.WILLIAMS DATE: 3-17
DESIGN ENGINEER OF RECORD: K.P.SEDAI DATE: 3-31-17

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE MOMENT SHEAR MOMENT DISTRIBUTION FACTORS (DF) ROLLING RATING CONT DIST, LEFT SPAN DIST, LEFT SPAN DI: FA(GI| $\langle 1 \rangle$ 1.055 1.75 0.275 1.23 0.523 1.23 N/A 55′ EL 27 55′ 1.05 55′ 27 HL-93(Inv)5.4 0.80 0.275 1.591 0.523 1.59 HL-93(0pr) 1.35 1.59 55' EL 27 55′ EL 5.4 DESIGN $\langle 2 \rangle$ LOAD 36.000 1.322 47.585 0.523 1.47 1.32 55′ 55′ 55′ HS-20(Inv) 1.75 0.275 1.54 EL 27 5.4 0.80 0.275 27 RATING 0.523 1.90 5.4 HS-20(0pr) 36.000 1.900 68.396 1.35 0.275 1.99 55′ EL 27 55′ N/A EL 13.500 2.776 0.275 0.523 4.17 0.275 2.78 37.476 4.04 55' EL 27 5.4 55′ 27 SNSH EL 0.80 0.523 43.095 3.02 20.000 2.155 0.275 3.14 55′ 0.275 2.15 55′ SNGARBS2 EL 27 EL 5.4 0.80 27 0.523 2.83 22.000 2.079 45.734 0.275 3.03 55′ 0.275 2.08 55′ 27 SNAGRIS2 EL 27 55′ 5.4 0.80 EL 0.523 2.09 27.250 1.384 37.708 0.275 55′ 27 55′ 5.4 0.275 1.38 55′ 27 SNCOTTS3 2.01 EL 0.80 1.4 EL 34.925 1.189 41.527 0.275 1.73 55′ 27 0.523 1.77 55′ 5.4 0.275 55′ 27 SNAGGRS4 EL EL 0.80 1.19 35.550 41.255 0.275 1.69 0.523 1.82 1.16 55′ EL 27 55′ 55′ 27 SNS5A 1.160 EL 5.4 0.80 0.275 1.079 43.102 0.275 1.57 0.523 1.68 0.275 27 SNS6A 39.950 55′ EL 27 55′ EL 5.4 0.80 0.523 1.67 SNS7B 42.000 1.028 43.175 0.275 1.50 55′ EL 27 55′ 5.4 0.80 0.275 1.03 55′ 27 LEGAL LOAD 0.523 1.98 33.000 43.556 55′ 55′ 0.275 1.32 55′ TNAGRIT3 1.320 0.275 1.92 EL 27 EL 5.4 0.80 27 RATING 1.330 0.523 TNT4A 33.075 43.979 0.275 1.94 55' EL 27 1.91 55′ EL 5.4 0.80 0.275 1.33 55′ EL 27 0.523 1.83 TNT6A 41.600 1.101 45.811 0.275 1.60 55′ EL 27 55′ EL 5.4 0.80 0.275 1.10 55′ 27 42.000 1.114 46.804 0.275 1.62 55′ EL 27 0.523 1.71 55′ 5.4 0.275 55′ 27 TNT7A EL 0.80 1.11 1.69 55′ 0.523 1.62 0.275 55′ 42.000 1.163 48.848 0.275 27 5.4 0.80 1.16 27 TNT7B 1.4 EL EL 0.523 1.56 43.000 1.101 47.33 0.275 1.60 55′ 55′ 0.80 0.275 1.10 55′ 27 TNAGRIT4 EL 27 5.4 EL 1.58 1.031 46.405 0.275 1.50 0.523 55′ 1.03 TNAGT5A 45.000 55′ EL 27 0.80 0.275 EL 5.4 45.000 **(3)** 1.013 45.582 1.4 0.275 1.47 55′ EL **27** 55′ 27 0.523 EL 1.48 0.80 0.275 1.01 TNAGT5B

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

(3) LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

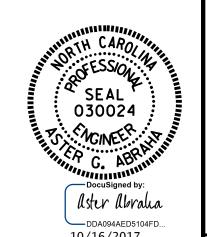
EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-5334

ROBESON COUNTY

STATION: 16+56.50 -L-



STATE OF NORTH CAROLINA

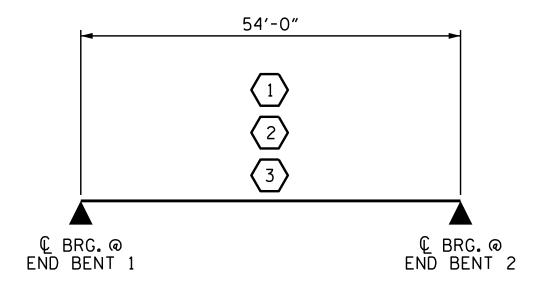
DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD SIMMAADY

LRFR SUMMARY FOR 55' CORED SLAB UNIT 90° SKEW (NON-INTERSTATE TRAFFIC)

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2 A SHEET NO.

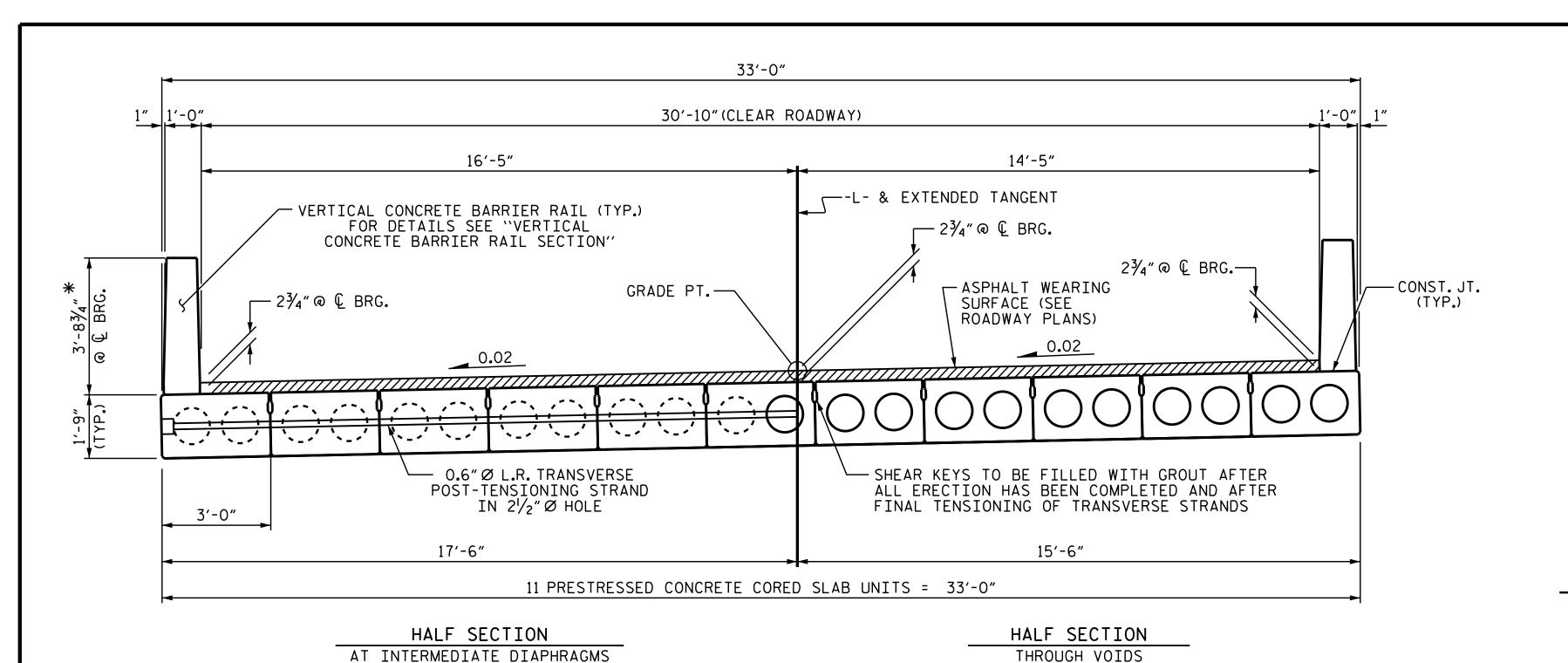


LRFR SUMMARY
FOR SPAN A

ASSEMBLED BY: R.P.PATEL DATE:08-02-16 CHECKED BY: J.P.MCCARTHA DATE:08-09-16 DRAWN BY: CVC 6/10

CHECKED BY : DNS 6/10

16-0CT-2017 16:30 R:\Structures\FinalPlans\B-5334_SMU_LRFR_770078.dgn



TYPICAL SECTION

*-THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT

THICKNESS SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

3'-0"

1'-6"
10"
1'-4"
10"
3"

11"
4"
4"
11"
3"
2 SPA.
② 2" CTS.
③ 2" CTS.
③ 2" CTS.

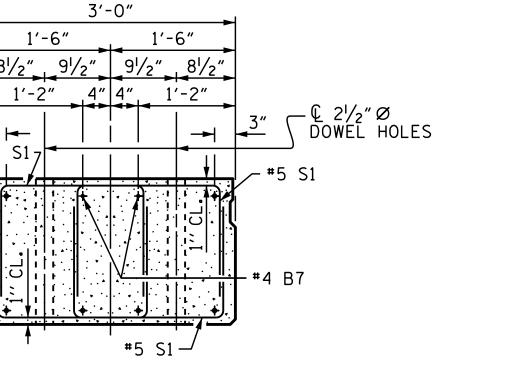
INTERIOR SLAB SECTION
(55' UNIT)

0.6" Ø LOW

RELAXATION STRAND LAYOUT

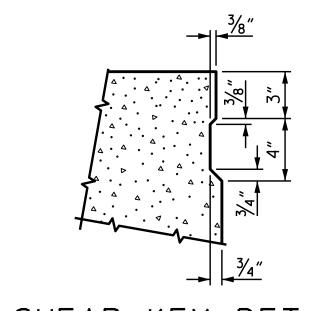
BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-O"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND



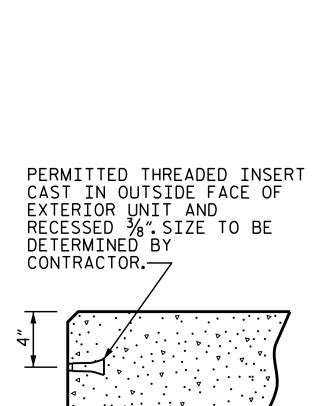
END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS
AND LOCATION OF DOWEL HOLES.
(STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB
UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.



SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.



3'-0''

1'-4''

-#5 S3

12" Ø VOIDS-

EXT. SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

3%'' CL.

10′′

THREADED INSERT DETAIL

HOLE FOR TRANSVERSE STRAND

A"

B

A"

B

B

A"

B

Q 0.6" Ø L.R. TRANSVERSE
POST-TENSIONING STRAND
SHEATHED WITH A
NON-CORROSIVE PIPE.

OUTSIDE FACE
OF EXTERIOR
CORED SLAB

ELEVATION VIEW

SECTION B-B

GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS

PROJECT NO. B-5334

ROBESON COUNTY

STATION: 16+56.50 -L-

SHEET 1 OF 3

SEAL 030024

Aster Abralia

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

3'-0" X 1'-9"
PRESTRESSED CONCRETE
CORED SLAB UNIT
90° SKEW

S-5

DATE:

10 /10 /2017						
10/16/2017			REV	ISIONS		
DOCUMENT NOT CONSIDERED	NO. 쉯	BY:	DATE:	NO.	BY:	
FINAL UNLESS ALL SIGNATURES COMPLETED	2			4		_

ASSEMBLED BY: R.P.PATEL DATE: 8-02-16
CHECKED BY: S.B.WILLIAMS DATE: 3-17

DRAWN BY: DGE 5/09
CHECKED BY: BCH 6/09

REV. 9/14

MAA/TMG

SEE "BRIDGE — APPROACH SLAB"
SHEET FOR DETAILS

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FIXED END

SECTION AT

 $\sim 2^{1/2}$ " Ø DOWEL HOLE

12"Ø7

ELASTOMERIC BEARING PAD

SEE "END BENT"
SHEETS FOR DETAILS

END BENT

ASPHALT

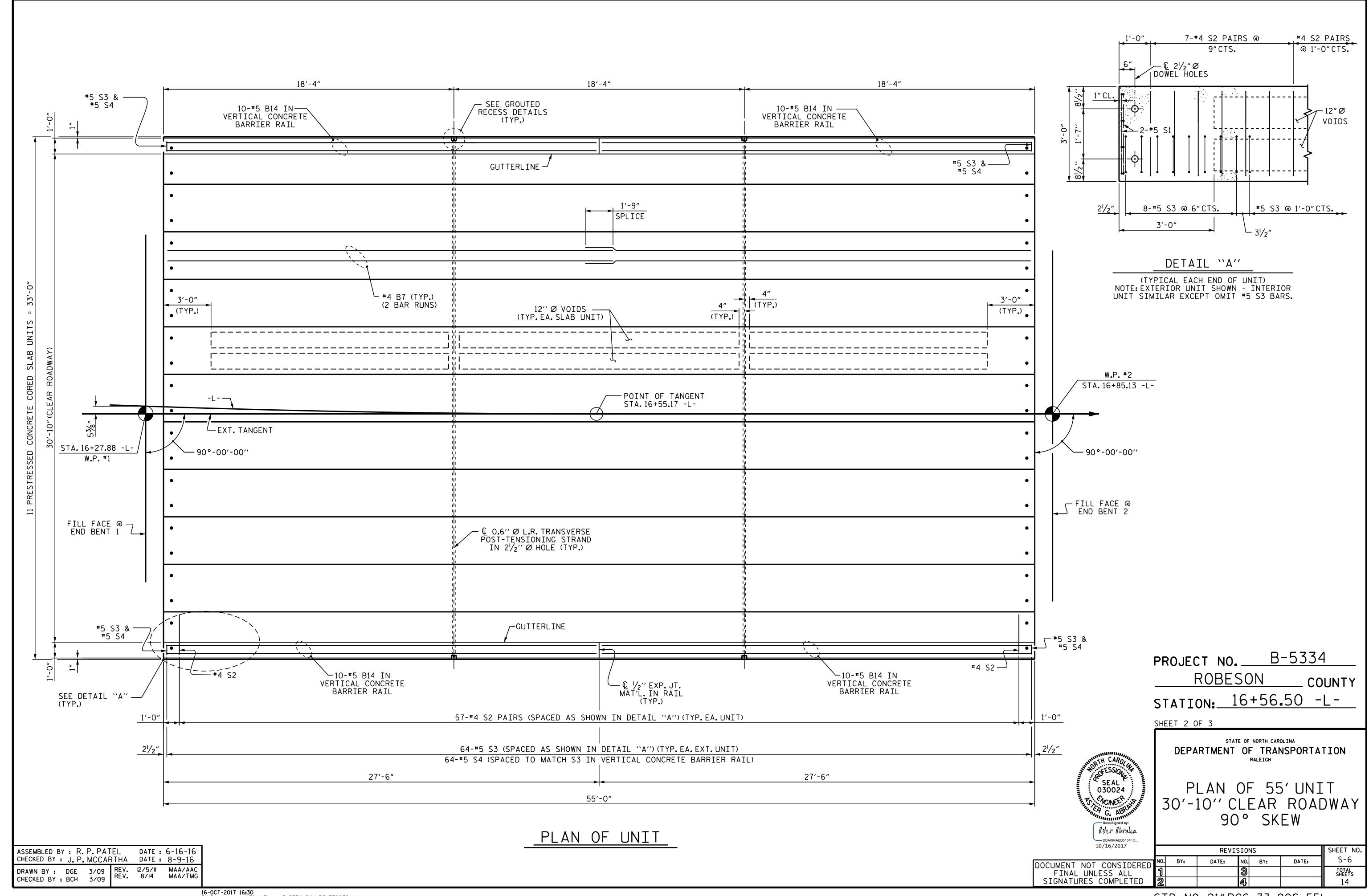
WEARING

2 LAYERS OF 30 LB.-ROOFING FELT TO PREVENT BOND.

11/2" Ø BACKER ROD-

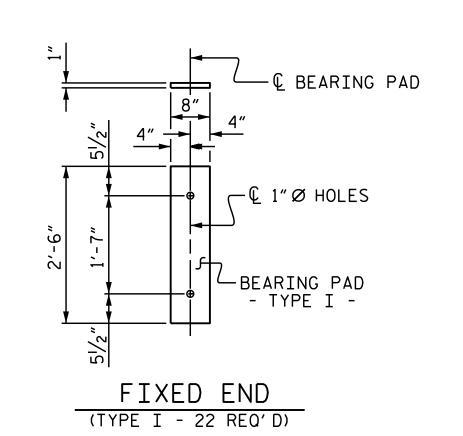
€ BEARING
& #6 DOWELS

SURFACE -



16-0CT-2017 16:30 R:\Structures\FinalPlans\B-5334_SMU_PS_770078.dgn aabraha

STD. NO. 21" PCS_33_90S_55L



ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

1'-0"

10"

— #5 S4

(TYP.) 3,-6, 21,0PED

VERTICAL DIM. VARIE

#5 S3 (SEE "PLAN OF

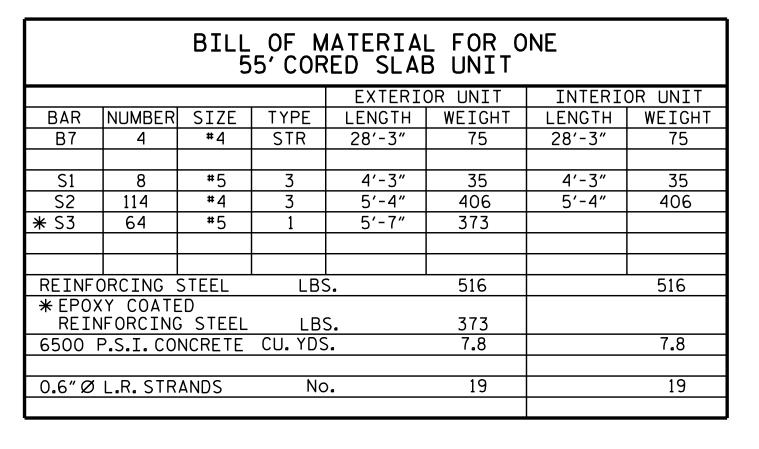
UNIT" FOR SPACING)

2¾"CL.

© 1/2"EXP.JT.MAT'L HELD IN PLACE WITH GALVANIZED NAILS. (NOTE: OMIT EXP.JT.MAT'L. WHEN SLIP FORM IS USED)

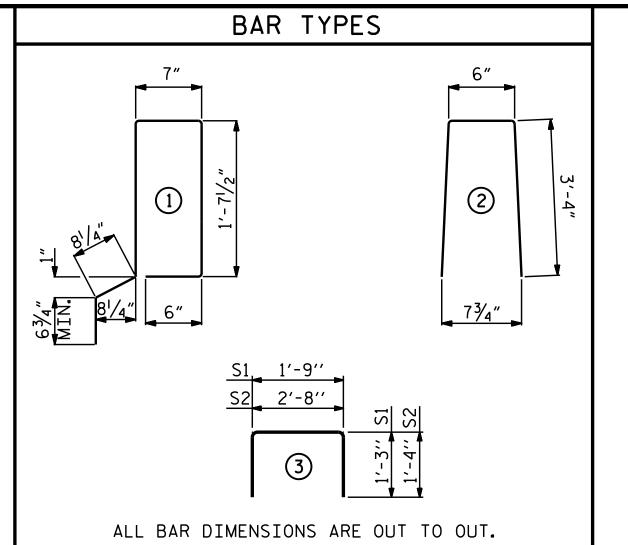
CONST. J

2"CL.MIN.

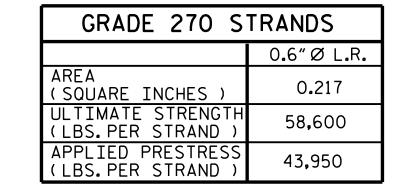


DEAD LOAD DEFLECTION AND CAMBER						
	3'-0" × 1'-9"					
55' CORED SLAB UNIT	0.6″Ø L.R. STRAND					
CAMBER (SLAB ALONE IN PLACE)	11/2"					
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	3⁄8″ ∤					
FINAL CAMBER	11/8″ Å					

** INCLUDES FUTURE WEARING SURFACE



CONCRETE REL	ASE STRENGTH
UNIT	PSI
55' UNIT	4900



_				
	CORED	SLAB!	S REQ	UIRED
		NUMBER	LENGTH	TOTAL LENGTH
	55' UNIT			
	EXTERIOR C.S	. 2	55′-0″	110'-0"
	INTERIOR C.S.	9	55′-0″	495'-0"
	TOTAL	11		605′-0"

BI	BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL										
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT					
	55' UNIT										
 ₩B14	40	40	#5	STR	27'-1"	1130					
* S4	128	128	#5	2	7′-2″	957					
★ EP0X	<u> </u>										
CLASS	AA CONCRETE			CU.YDS.)	14.1					
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN.FT.		110.25					

GUTTERLINE ASP	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
55' UNIT	15/8"	3′-75⁄8″

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS. 1/2" IN DEPTH. SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

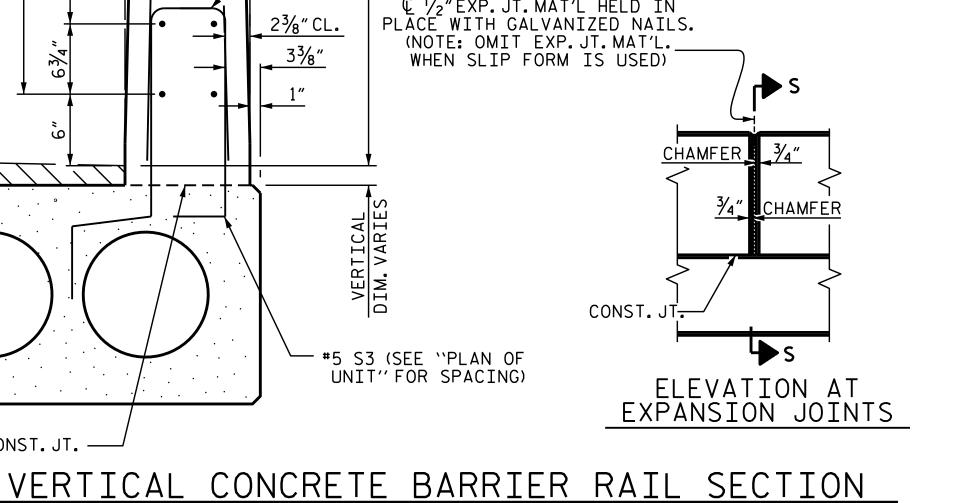
FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

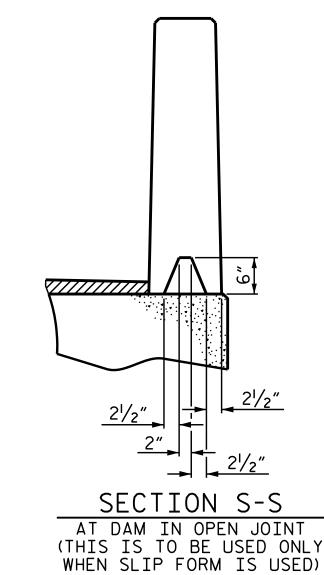
THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

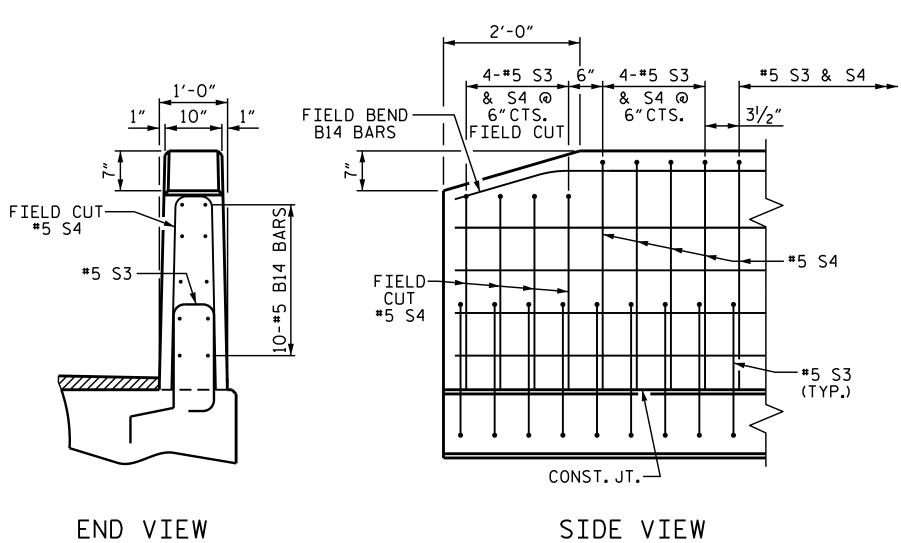
THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.







SIDE VIEW

END OF RAIL DETAILS

B-5334 PROJECT NO. _ ROBESON COUNTY STATION: 16+56.50 -L-SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

3'-0'' X 1'-9'' PRESTRESSÉD CONCRETE CORED SLAB UNIT 90° SKEW

10 (16 (2017									
10/16/2017		REVISIONS							
DCUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-7		
FINAL UNLESS ALL	1			3			TOTAL SHEETS		
SIGNATURES COMPLETED	2			4			14		

ASSEMBLED BY: R. P. PATEL DATE: 08-02-16 CHECKED BY: J. P. MCCARTHA DATE: 08-10-16 DRAWN BY: DGE 5/09 CHECKED BY: BCH 6/09

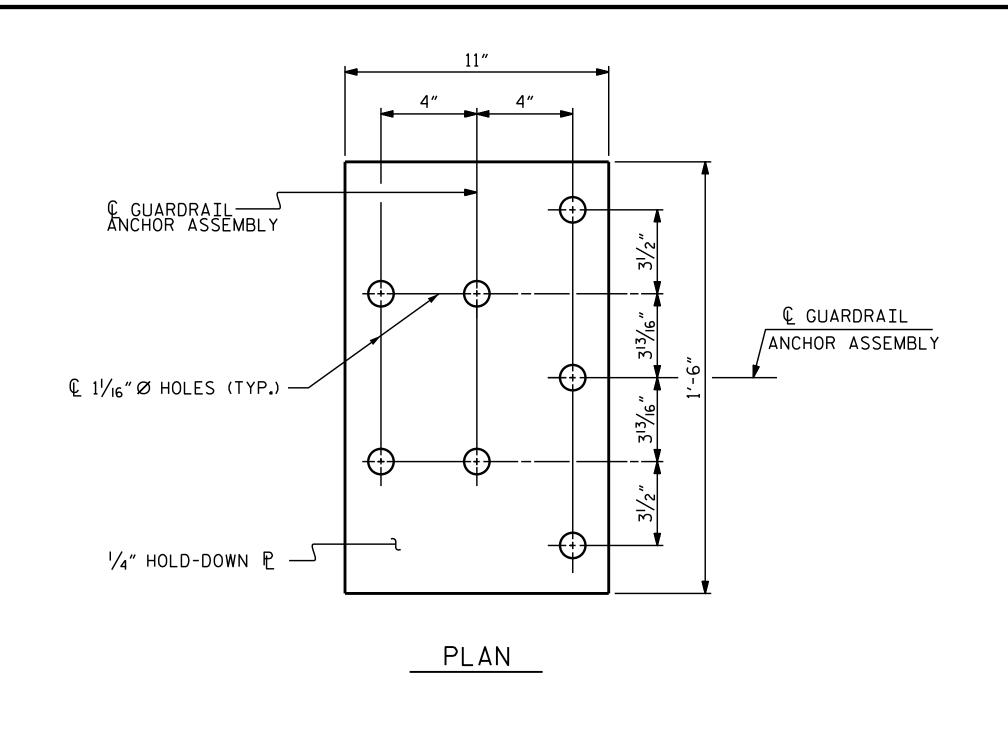
CONST. JT. —

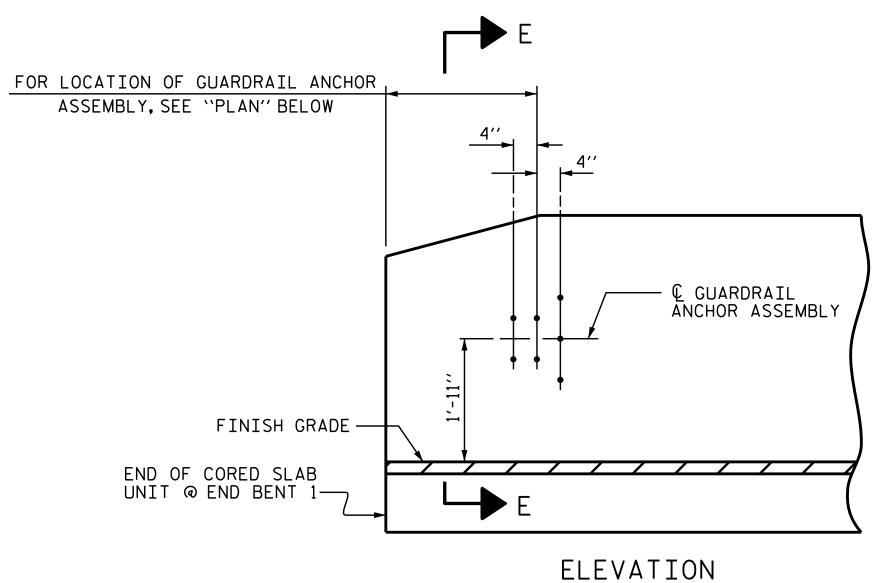
3'-8¾" VARIES (SEE "GUTTERLIN THICKNESS & RAIL HEIG

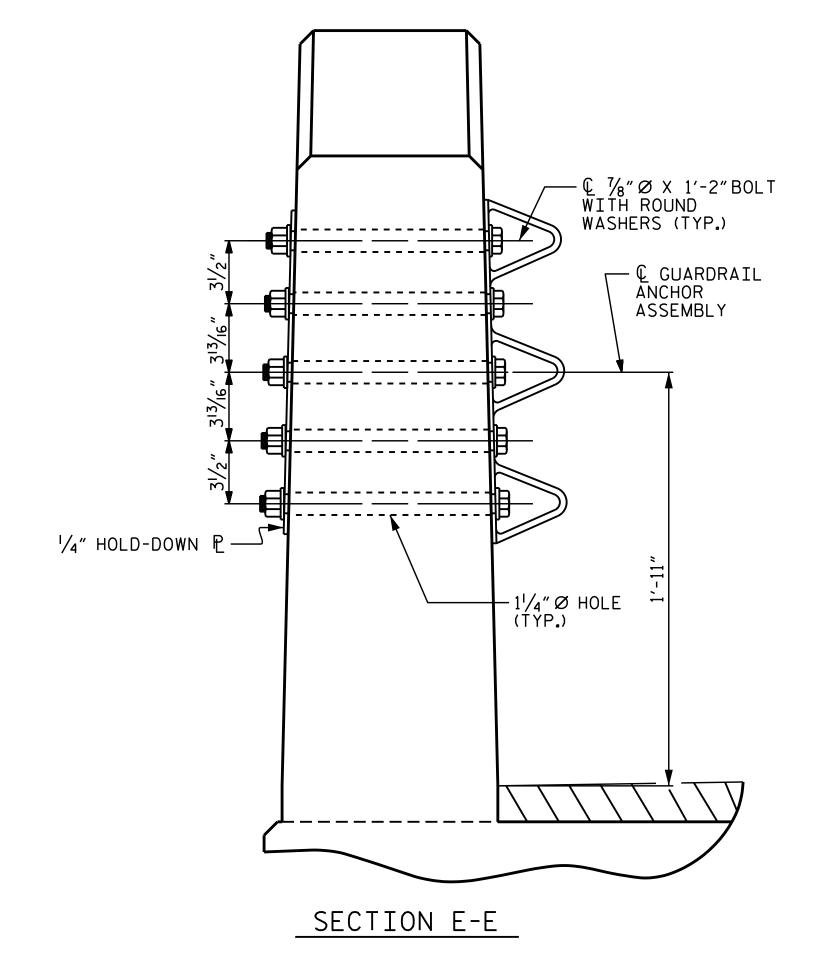
SEAL 3

S. CACINEER

aster abraba







GUARDRAIL ANCHOR ASSEMBLY DETAILS

ASSEMBLED BY : R.P.PATEL CHECKED BY : J.P.MCCARTHA

DRAWN BY : MAA 5/10

CHECKED BY : GM 5/10

DATE : 08-02-16

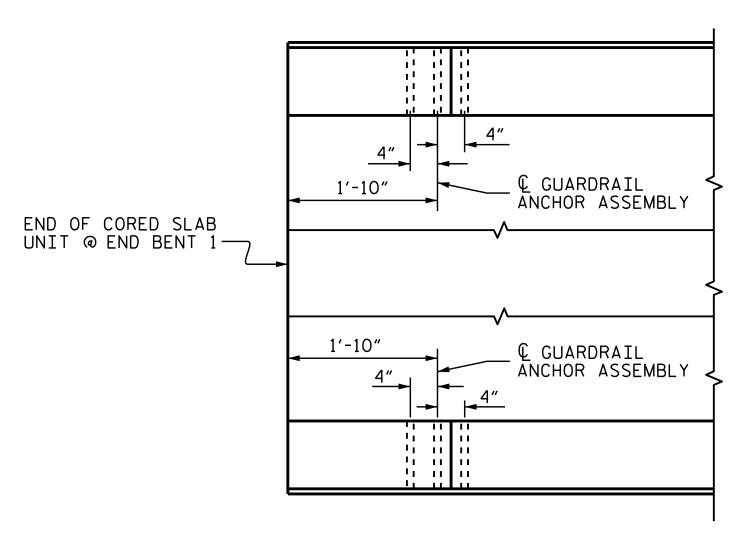
DATE : 08-10-16

MAA/GM

MAA/GM MAA/TMG

REV. 12/5/II

REV. 1/15



PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

END BENT 1 SHOWN, END BENT 2 SIMILAR

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

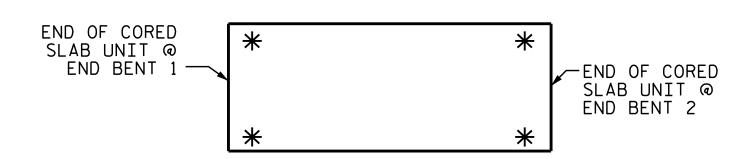
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



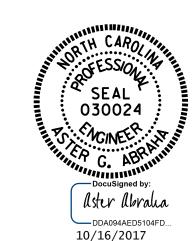
SKETCH SHOWING POINTS OF ATTACHMENT

★ DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-5334

ROBESON COUNTY

STATION: 16+56.50 -L-



DEPARTMENT OF TRANSPORTATION

STANDARD

GUARDRAIL ANCHORAGE

DETAILS

FOR VERTICAL CONCRETE

BARRIER RAIL

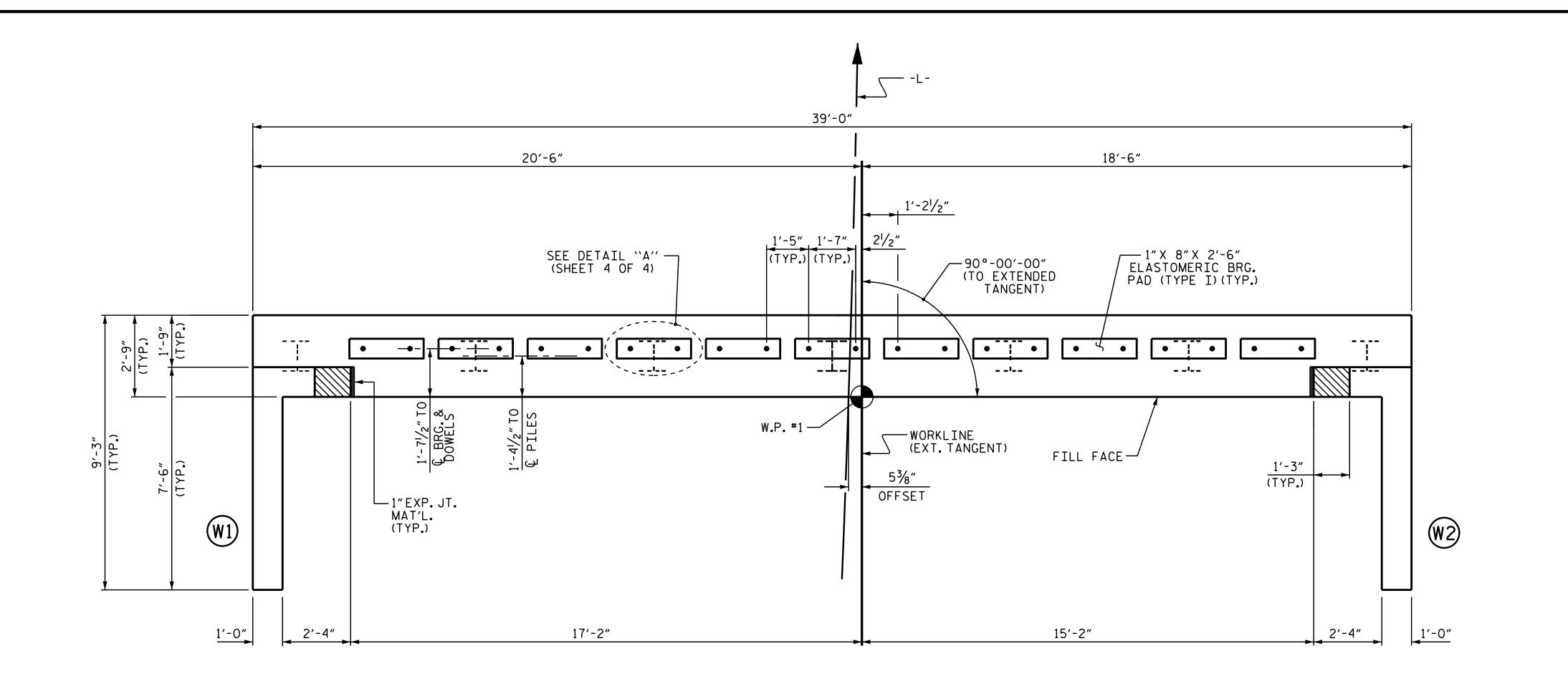
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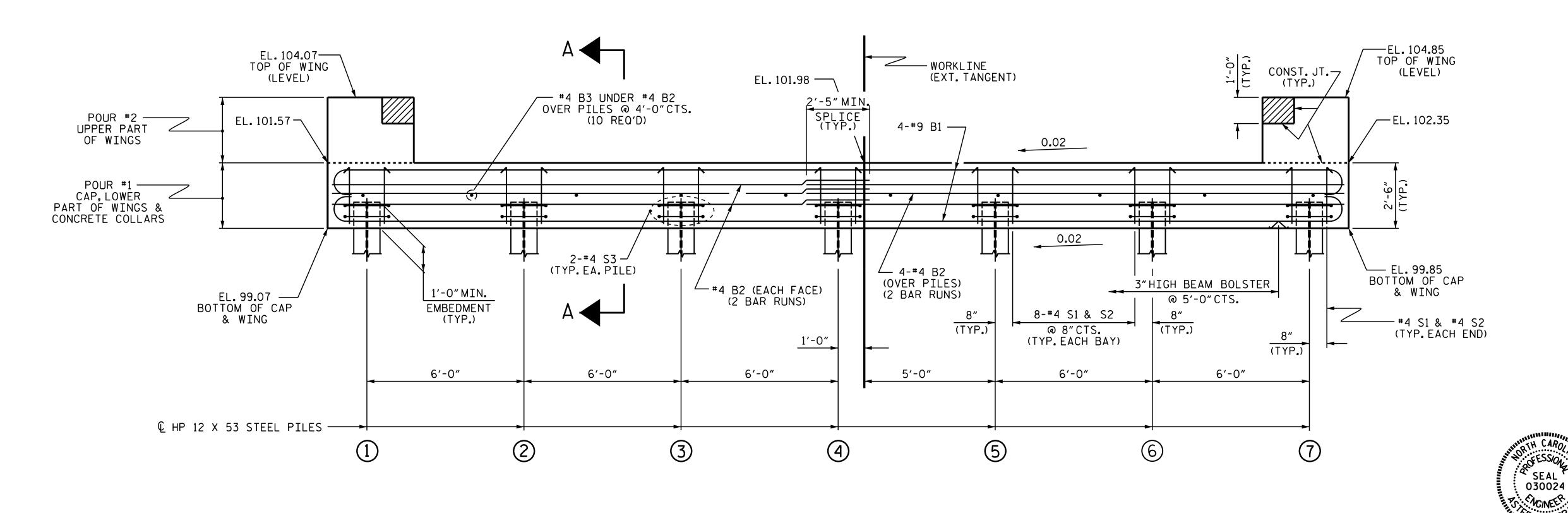
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TOTAL
SHEETS
14

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PLAN



ELEVATION

WINGS NOT SHOWN FOR CLARITY.

FOR SECTION A-A, SEE SHEET 4 OF 4.

CONCRETE COLLARS FOR STEEL PILES NOT SHOWN IN PLAN AND ELEVATION VIEWS FOR CLARITY.

SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL", SHEET 4 OF 4.

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

INSTALL THE 4"DIA. DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSASRY TO CLEAR THE DRAIN PIPE.

TOP OF PILE ELEVATIONS									
	100.10								
2	100.22								
3	100.34								
4	100.46								
(5)	100.58								
9	100.70								
7	100.82								

B-5334 PROJECT NO. ____ ROBESON _ COUNTY STATION: 16+56.50 -L-

SHEET 1 OF 4

Aster Abralia

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE

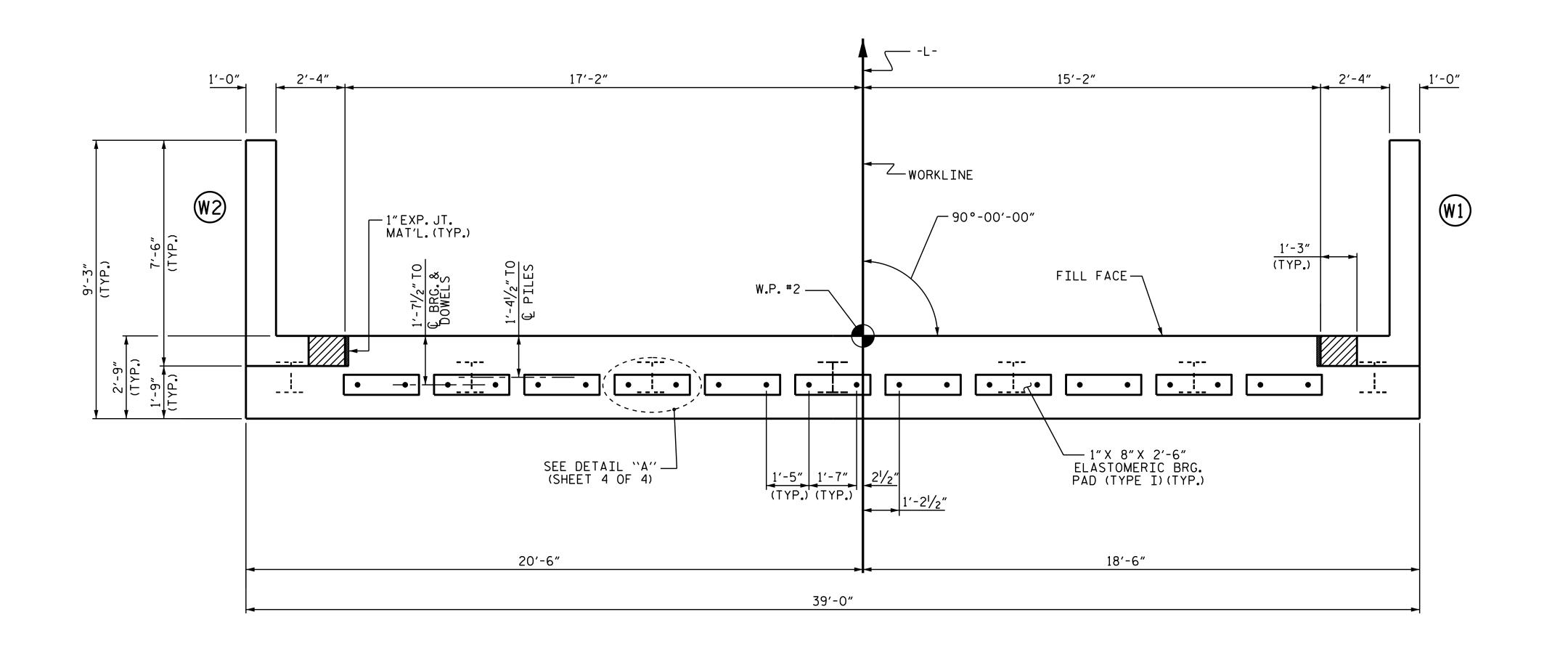
END BENT No. 1

	DDA094AED5104FD							
10/16/2017				SHEET NO.				
	DOCUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-9
	FINAL UNLESS ALL	1			3			TOTAL SHEETS
	SIGNATURES COMPLETED	2			4			14

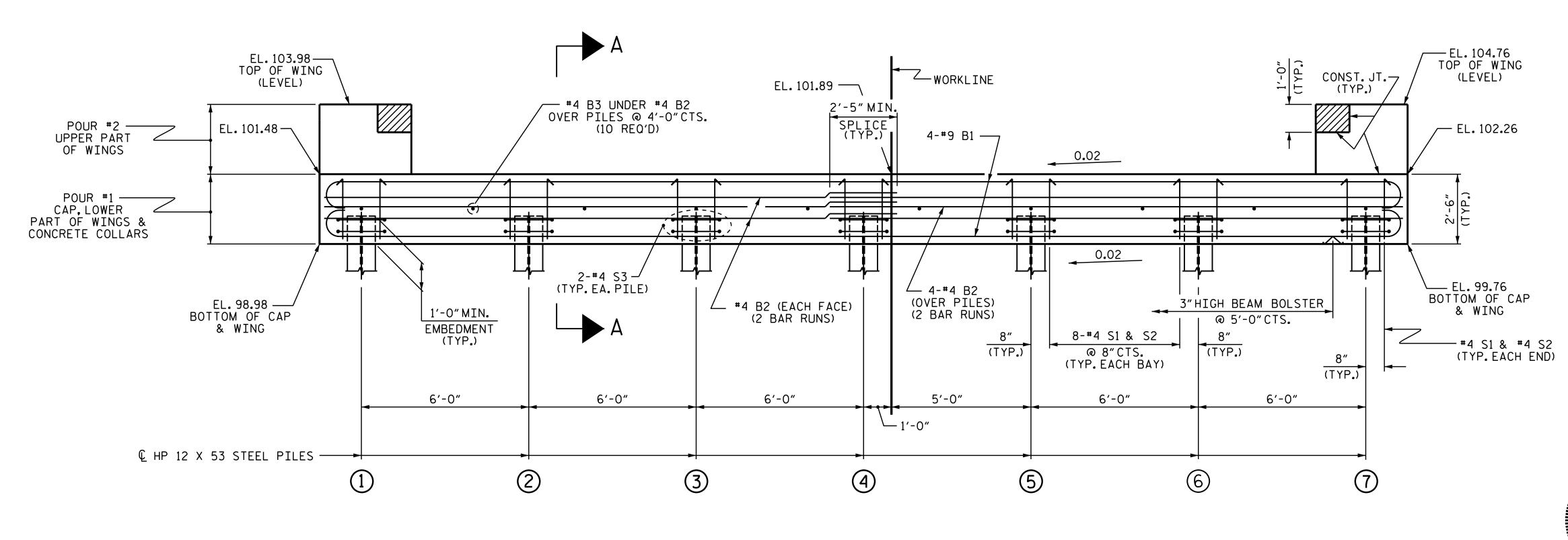
ASSEMBLED BY: R.P.PATEL DATE: 08-02-16 CHECKED BY: S.B.WILLIAMS DATE: 3/17

MAA/TMG

DRAWN BY: DGE OI/IO
CHECKED BY: MKT OI/IO
REV. 4/I5



PLAN



ELEVATION

WINGS NOT SHOWN FOR CLARITY.
FOR SECTION A-A, SEE SHEET 4 OF 4.
RETE COLLARS FOR STEEL PILES NOT SHOWN IN PLAN AND FLEX

CONCRETE COLLARS FOR STEEL PILES NOT SHOWN IN PLAN AND ELEVATION VIEWS FOR CLARITY.
SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL", SHEET 4 OF 4.

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

INSTALL THE 4"DIA.DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSASRY TO CLEAR THE DRAIN PIPE.

TOP OF PILE ELEVATIONS							
	100.02						
2	100.14						
3	100.26						
4	100.38						
(G)	100 . 50						
6	100.62						
7	100.74						

PROJECT NO. B-5334

ROBESON COUNTY

STATION: 16+56.50 -L-

SHEET 2 OF 4

SEAL 030024

Aster Abralia

J. MINEER

DEPARTMENT OF TRANSPORTATION
RALEIGH

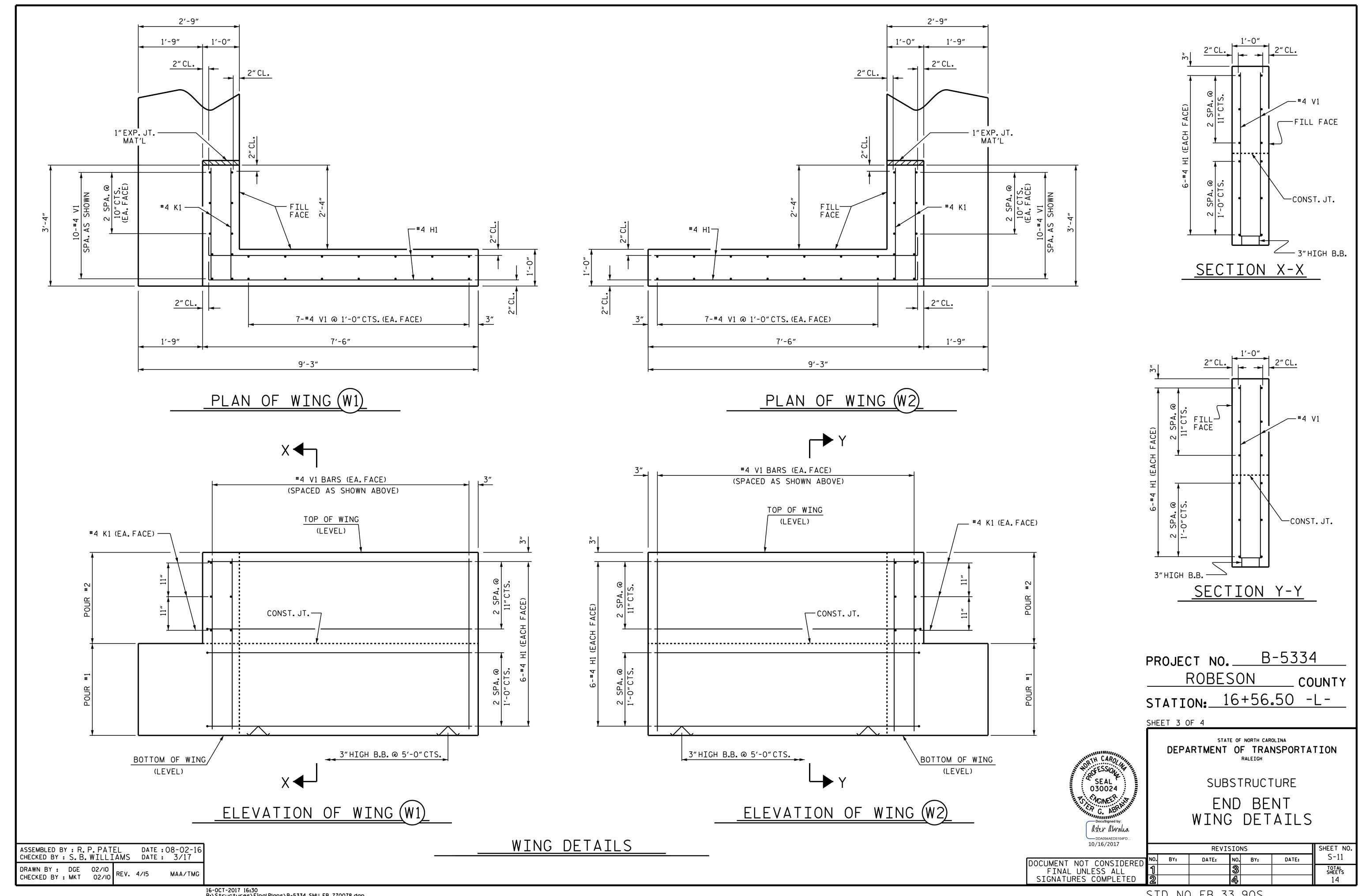
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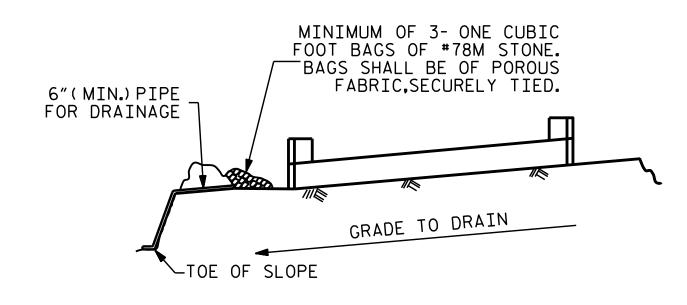
END BENT No. 2

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ASSEMBLED BY: R.P.PATEL DATE: 08-02-16 CHECKED BY: S.B. WILLIAMS DATE: 3/17

DRAWN BY: DGE OI/IO
CHECKED BY: MKT OI/IO
REV. 4/I5



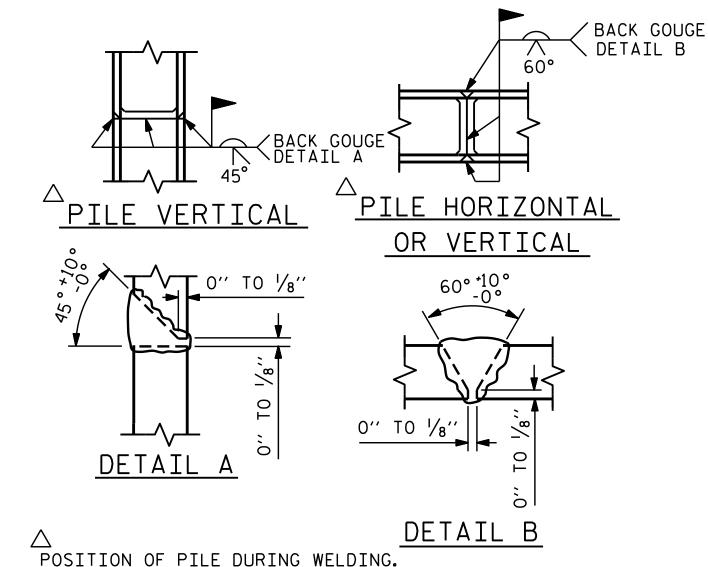


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

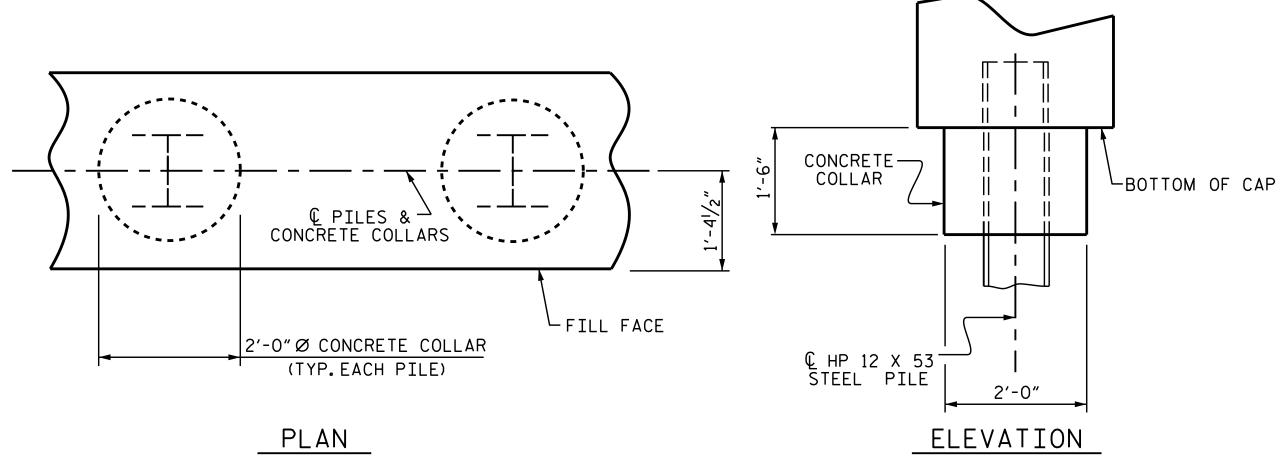
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



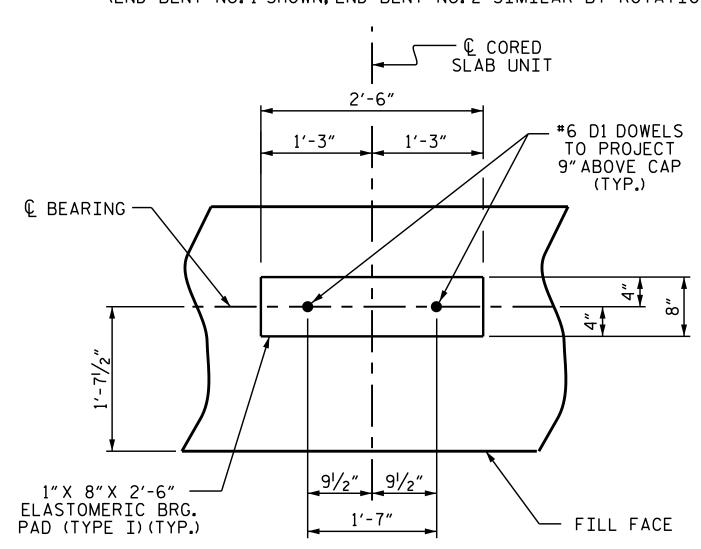
PILE SPLICE DETAILS



BAR TYPES BILL OF MATERIAL FOR ONE END BENT BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT #9 1 41'-0" 1115 38'-6" #4 STR 20'-7" 220 B2 16 7′-2" #4 | STR | 2'-5" 10 16 D1 | 22 | #6 | STR | 1'-6" 50 126 K1 | 12 | #4 | STR | 2'-11" 23 50 | #4 | 3 | 7'-5" 248 S2 50 #4 4 3'-2" 106 S3 | 14 | #4 5 | 6'-6" 61 ___1'-3'' LAP V1 | 48 | #4 | STR | 4'-8" 150 REINFORCING STEEL 2115 LBS. (FOR ONE END BENT) 2'-5" CLASS A CONCRETE BREAKDOWN 1′-8″ Ø (FOR ONE END BENT) POUR #1 CAP, LOWER PART 12.4 C.Y. OF WINGS & COLLARS ALL BAR DIMENSIONS ARE OUT TO OUT. POUR #2 UPPER PART OF 1.8 C.Y. END BENT No. 1 END BENT No. 2 WINGS HP 12 X 53 STEEL PILES HP 12 X 53 STEEL PILES NO: 7 LIN. FT.= 385 NO: 7 LIN. FT.= 385 TOTAL CLASS A CONCRETE 14.2 C.Y. PILE DRIVING EQUIPMENT PILE DRIVING EQUIPMENT SETUP FOR SETUP FOR HP 12 X 53 STEEL PILES HP 12 X 53 STEEL PILES NO: 7 NO: 7 PILE REDRIVES 7 EA. PILE REDRIVES 7 EA.

CORROSION PROTECTION FOR STEEL PILES DETAIL

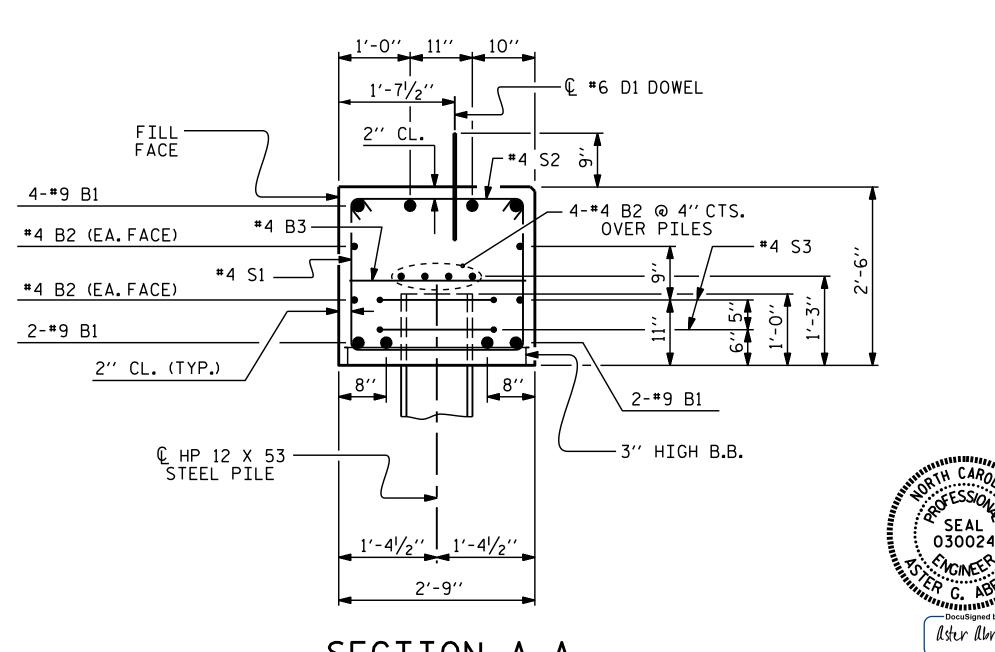
(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



DATE : 08-02-16 ASSEMBLED BY: R. P. PATEL DATE: 08-02-1 CHECKED BY: S. B. WILLIAMS DATE: 3/17 DRAWN BY: DGE 12/09 CHECKED BY: MKT 01/10

REV. II/I4

DETAIL "A" (END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

B-5334 PROJECT NO. ____ ROBESON _ COUNTY STATION: 16+56.50 -L-

SHEET 4 OF 4

SEAL F 030024

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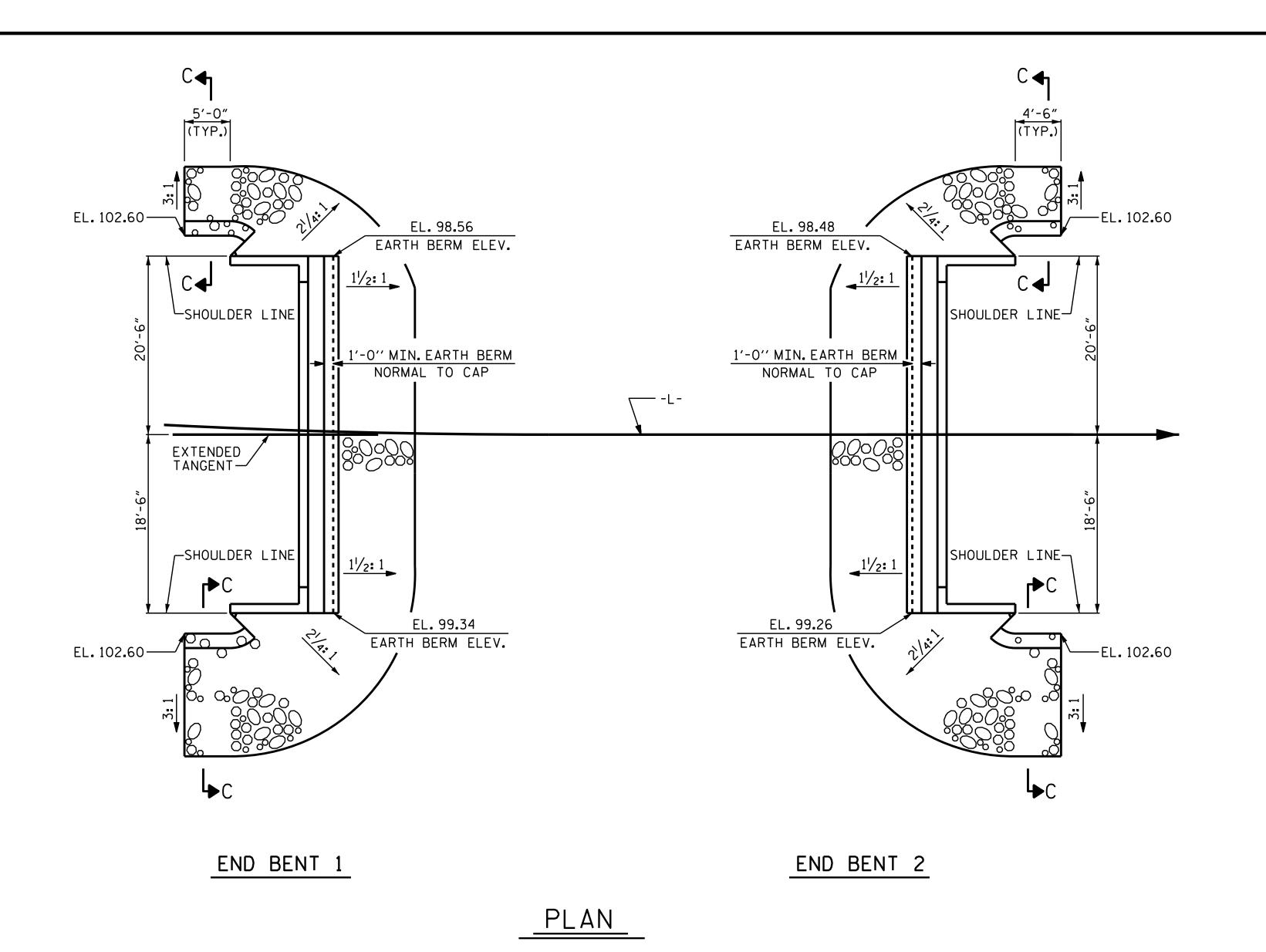
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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

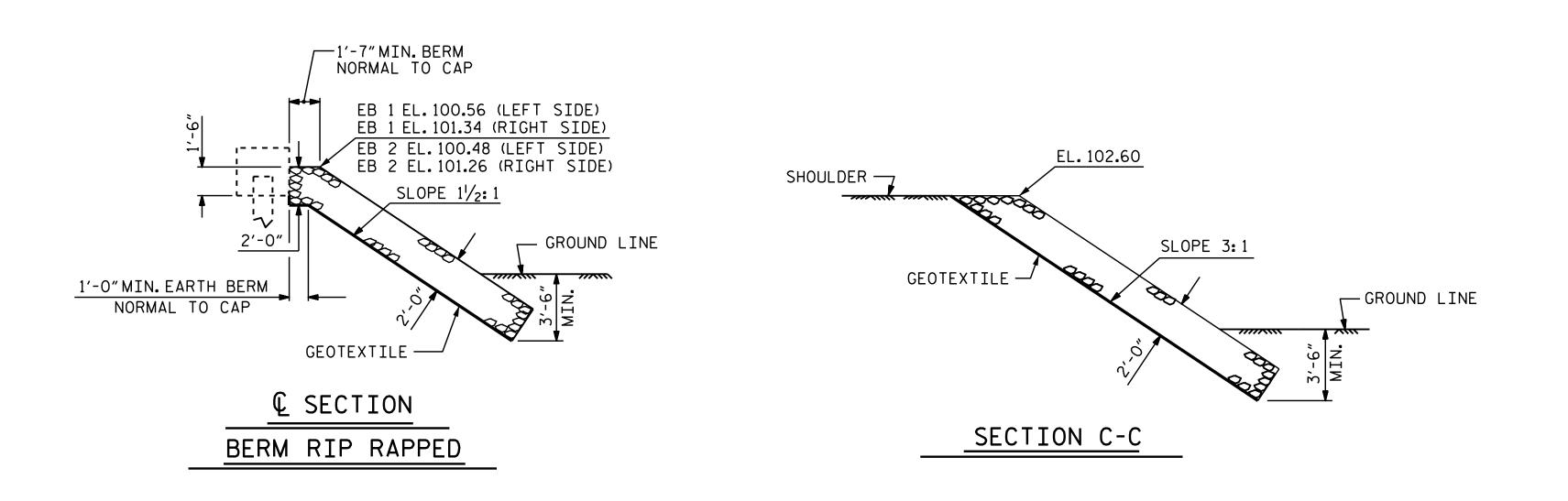
SUBSTRUCTURE

END BENT No.1 & 2 DETAILS

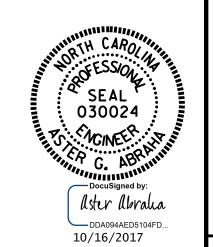
10/16/2017		REVISIONS					SHEET NO.	
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	FINAL UNLESS ALL	1			3			TOTAL SHEETS
	SIGNATURES COMPLETED	2			4			14



ESTIMATED QUANTITIES						
BRIDGE @ STA.16+56.50 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE				
	TONS	SQUARE YARDS				
END BENT 1	185	195				
END BENT 2	205	215				



PROJECT NO. B-5334 ROBESON _ COUNTY STATION: 16+56.50 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

RIP RAP DETAILS

SHEET NO.

S-13

DATE:

REVISIONS DATE: BY:

STD. NO. RR1

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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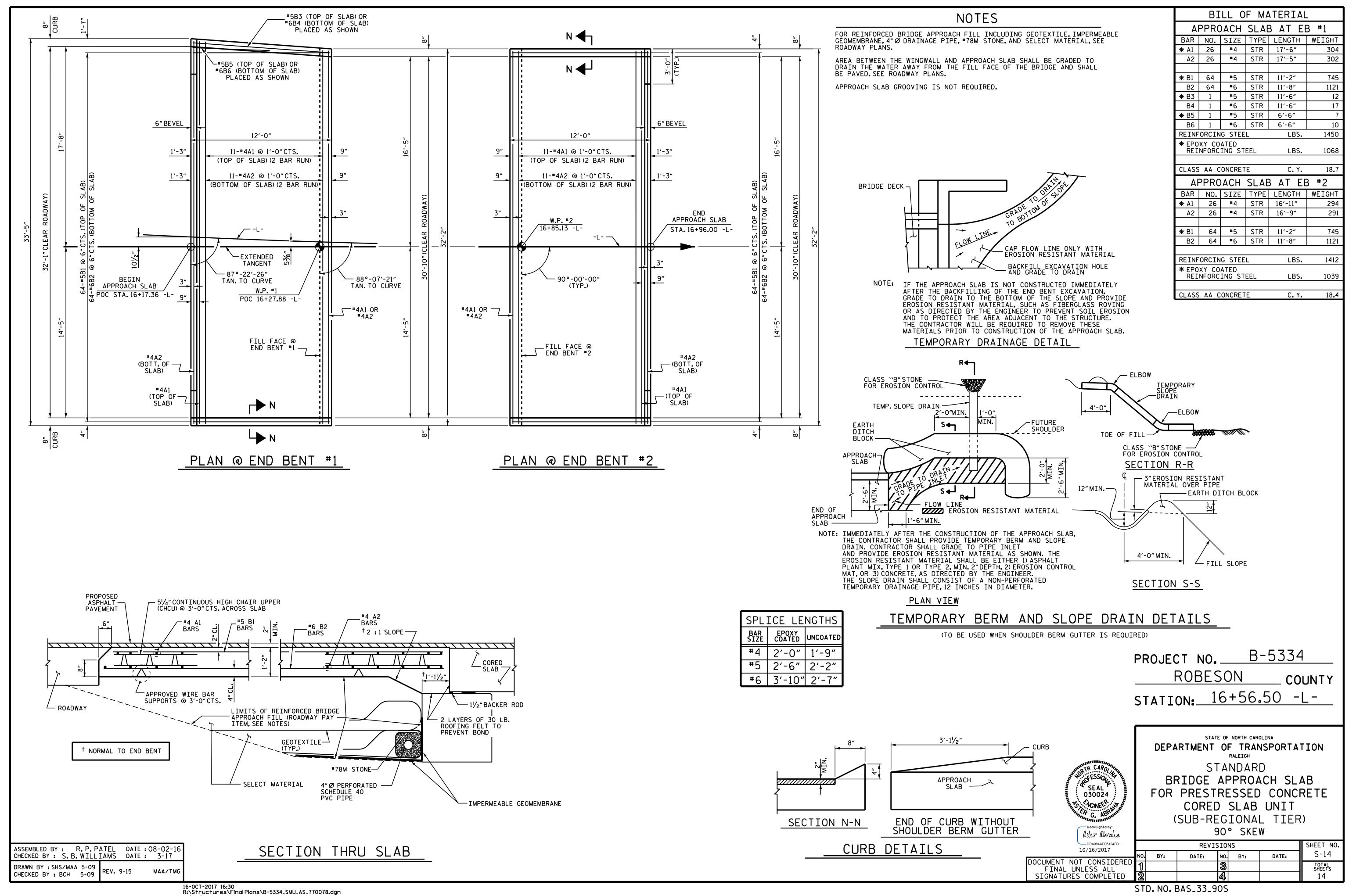
ASSEMBLED BY : M.K. BEARD CHECKED BY : S. B. WILLIAMS

DRAWN BY: REK 1/84 CHECKED BY: RDU 1/84

DATE : 8/11/16 DATE : 3/17

TLA/GM MAA/GM MAA/GM

REV. 5/I/06R REV. I0/I/II REV. I2/2I/II



STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ. IN.
- AASHTO M270 GRADE 50 -	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS. PER SO. IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS. PER SO. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS. SETTLEMENT OF FALSEWORK. AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $rac{3}{4}$ "Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990

REV. 6-16-95 EEM (J) RGW REV. 5-7-03 RWW (J) JTE REV. 10-1-11 MAA (/) GM REV. 8-16-99 RWW (x) LES REV. 5-1-06 TLA (x) GM