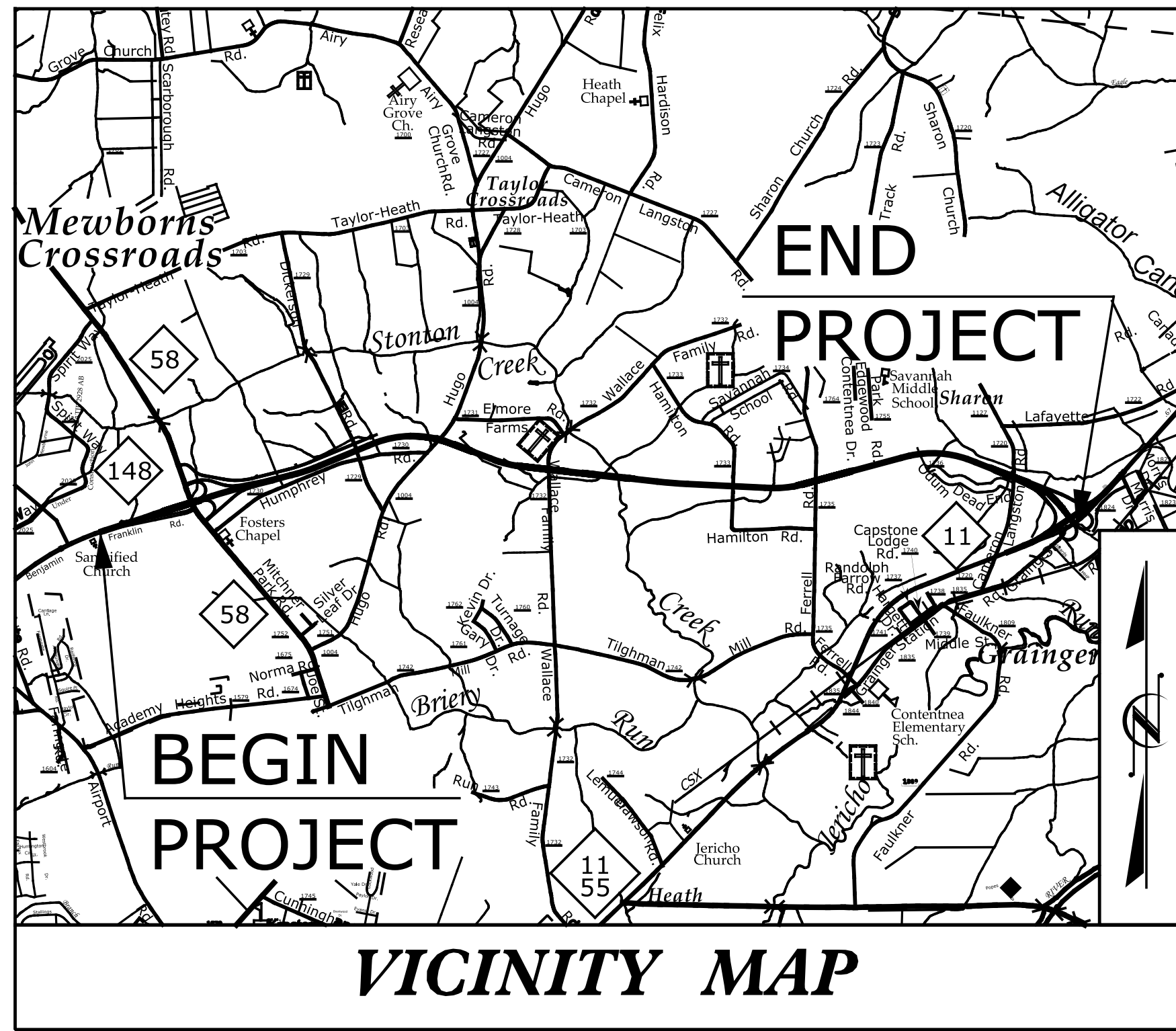


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TIP PROJECT: R-5703



STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

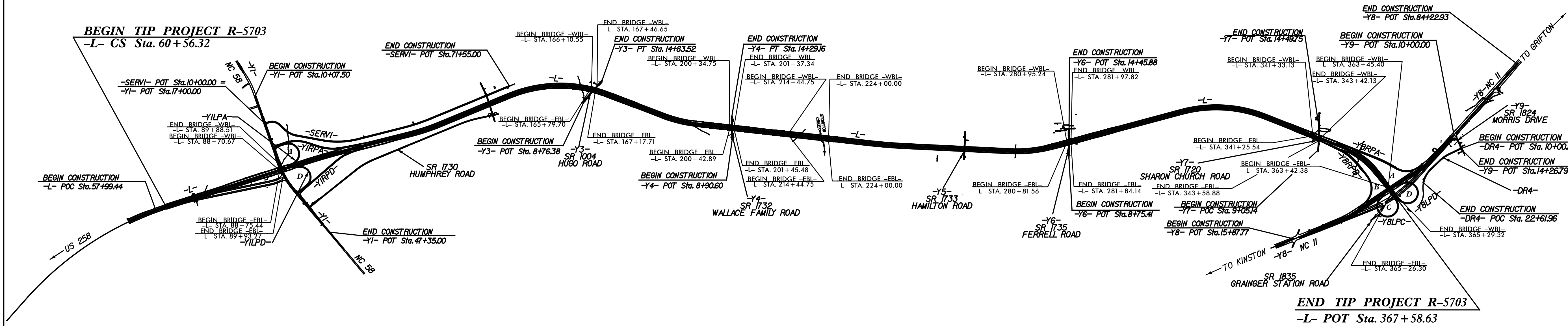
LENOIR COUNTY

LOCATION: NC 148 (HARVEY PARKWAY) FROM NC 58 TO NC 11

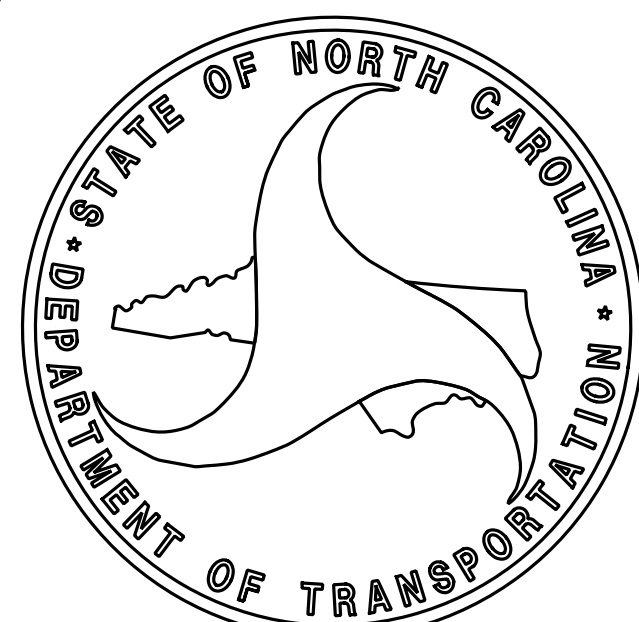
TYPE OF WORK: GRADING, PAVING, DRAINAGE, AND STRUCTURES

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-5703		
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
46375.1.1	N/A	PE	
46375.2.1	N/A	R/W	
46375.3.1	N/A	CONST.	

CONTRACT: C204014



STRUCTURES



DESIGN DATA

ADT 2017 =	4,000
ADT 2040 =	18,800
K =	9 %
D =	55 %
T =	8 % *
V =	70 MPH

* (TTST 4% + DUAL 4%)
FUNC CLASS =
FREEWAY/INTERSTATE

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT R-5703 =	5.473 MILES
LENGTH STRUCTURE TIP PROJECT R-5703 =	0.342 MILES
TOTAL LENGTH TIP PROJECT R-5703 =	5.815 MILES

Prepared In the Office of:

Michael Baker
Michael Baker Engineering, Inc.
8000 Regency Parkway, Suite 600
Cary, NC 27518
Professional Corporation License Number:
F-1084

2012 STANDARD SPECIFICATIONS

LETTING DATE:
DECEMBER 19, 2017

BRADLEY J. BELL, P.E.
PROJECT ENGINEER

PREPARED FOR:

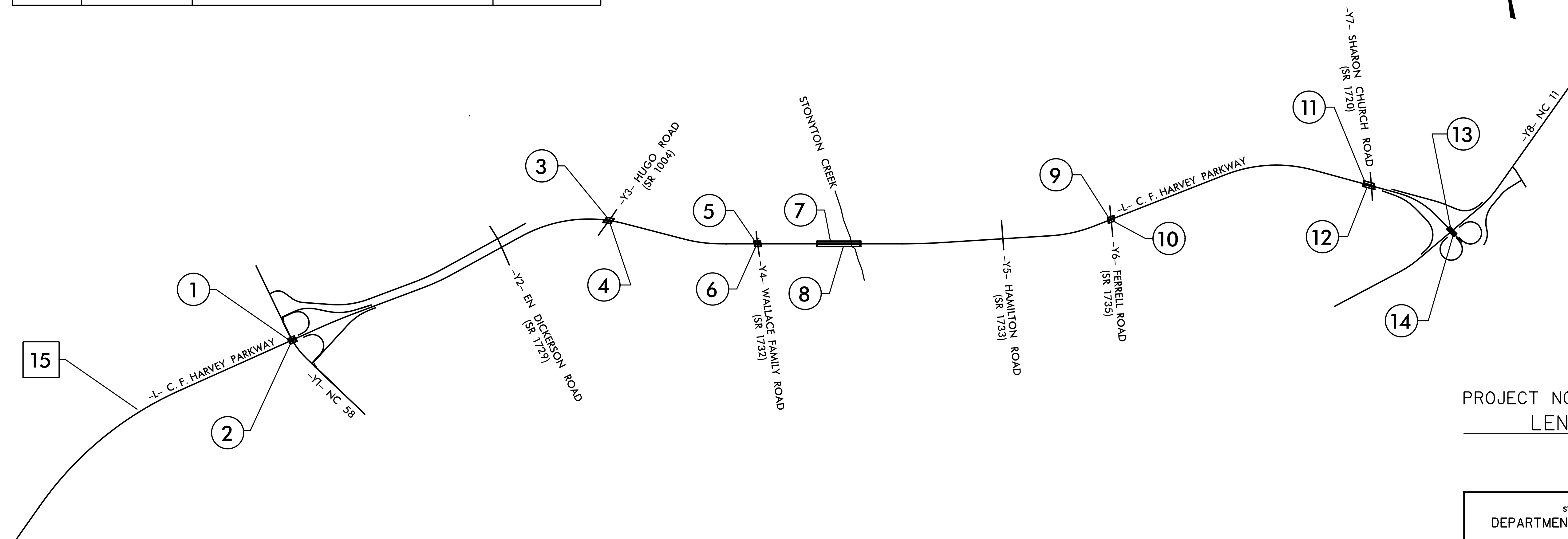
DIVISION OF HIGHWAYS
STRUCTURES MANAGEMENT UNIT
1000 BIRCH RIDGE DR.
RALEIGH, NC 27610

LEGEND

- # BRIDGES
- # CULVERT

PART 1			
STR. NO.	STATION	DESCRIPTION	SHEET NO.
1	89+28.52 -L- 28+62.92 -Y1-	LEFT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER NC 58 BETWEEN SR 1581 AND SR 1004	S1-1 TO S1-25
2	89+28.52 -L- 28+62.92 -Y1-	RIGHT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER NC 58 BETWEEN SR 1581 AND SR 1004	S2-1 TO S2-25
3	166+72.51 -L- 11+86.25 -Y3-	LEFT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER SR 1004 BETWEEN SR 1729 AND SR 1732	S3-1 TO S3-34
4	166+72.51 -L- 11+86.25 -Y3-	RIGHT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER SR 1004 BETWEEN SR 1729 AND SR 1732	S4-1 TO S4-31
5	200+91.89 -L- 11+61.86 -Y4-	LEFT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER SR 1732 BETWEEN SR 1004 AND SR 1733	S5-1 TO S5-25
6	200+91.89 -L- 11+61.86 -Y4-	RIGHT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER SR 1732 BETWEEN SR 1004 AND SR 1733	S6-1 TO S6-25
7	219+22.38 -L-	LEFT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER STONYTON CREEK BETWEEN SR 1732 AND SR 1735	S7-1 TO S7-50
8	219+22.38 -L-	RIGHT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER STONYTON CREEK BETWEEN SR 1732 AND SR 1735	S8-1 TO S8-52

PART 2			
STR. NO.	STATION	DESCRIPTION	SHEET NO.
9	281+39.19 -L- 11+64.76 -Y6-	LEFT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER SR 1735 BETWEEN SR 1733 AND SR 1720	S9-1 TO S9-25
10	281+39.19 -L- 11+64.76 -Y6-	RIGHT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER SR 1735 BETWEEN SR 1733 AND SR 1720	S10-1 TO S10-25
11	342+97.24 -L- 11+93.49 -Y7-	LEFT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER SR 1720 BETWEEN SR 1735 AND NC 11	S11-1 TO S11-29
12	342+97.24 -L- 11+93.49 -Y7-	RIGHT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER SR 1720 BETWEEN SR 1735 AND NC 11	S12-1 TO S12-29
13	364+28.98 -L- 41+56.43 -Y8-	LEFT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER NC 11 BETWEEN SR 1720 AND NC 11	S13-1 TO S13-29
14	364+28.98 -L- 41+56.43 -Y8-	RIGHT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER NC 11 BETWEEN SR 1720 AND NC 11	S14-1 TO S14-29
15	67+55.47 -L-	LEFT EXTENSION SINGLE 16 FT. X 9 FT. CONCRETE BOX CULVERT 93°-47'-47" SKEW	C15-1 TO C15-5



PART 1 OF 2

PROJECT NO. R-5703
LENOIR COUNTY

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

INDEX SHEET

DRAWN BY : C. E. MAYHEW DATE : 5-5-17
CHECKED BY : V. A. PATEL DATE : 7-19-17

REVISIONS						SHEET NO. TOTAL SHEETS
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			15
2			4			

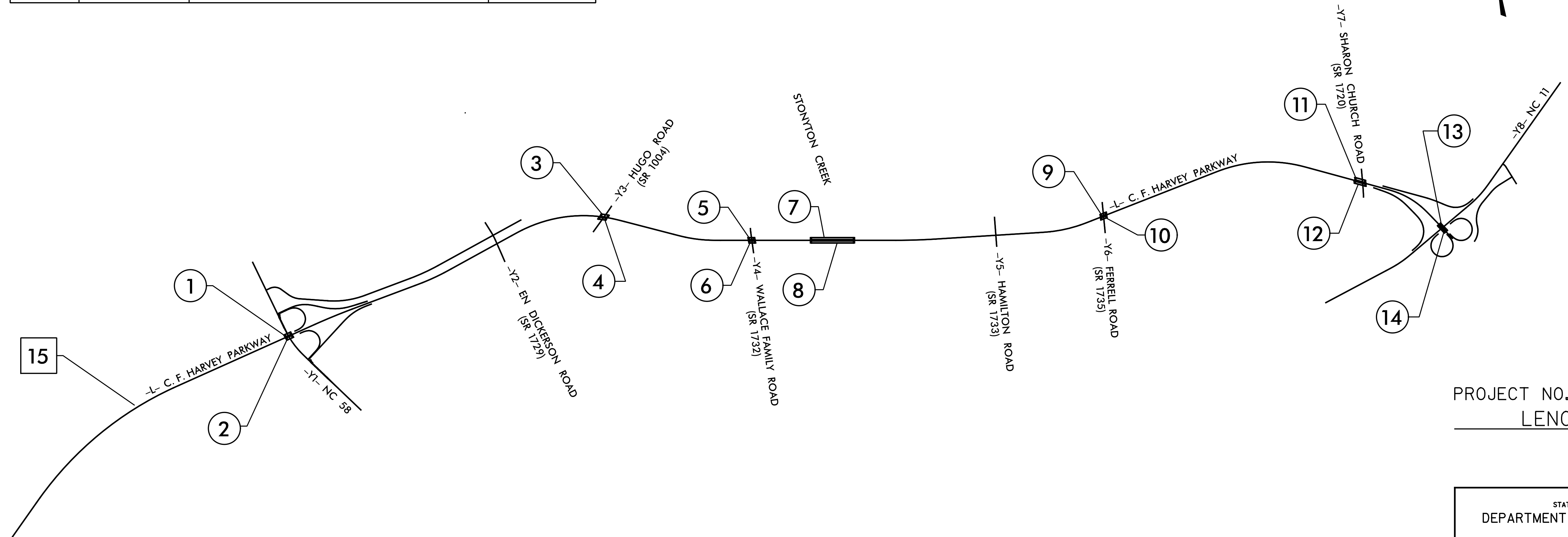
Michael Baker
INTERNATIONAL
Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084

LEGEND

- # BRIDGES
- # CULVERT

PART 1			
STR. NO.	STATION	DESCRIPTION	SHEET NO.
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7	219+22.38 -L-	LEFT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER STONYTON CREEK BETWEEN SR 1732 AND SR 1735	S7-1 TO S7-50
8	219+22.38 -L-	RIGHT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER STONYTON CREEK BETWEEN SR 1732 AND SR 1735	S8-1 TO S8-52

PART 2			
STR. NO.	STATION	DESCRIPTION	SHEET NO.
9	281+39.19 -L- 11+64.76 -Y6-	LEFT LANE BRIDGE ON C.F. HARVEY PARKWAY OVER SR 1735 BETWEEN SR 1733 AND SR 1720	S9-1 TO S9-25
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15	67+55.47 -L-	LEFT EXTENSION SINGLE 16 FT. X 9 FT. CONCRETE BOX CULVERT 93°-47'-47" SKEW	C15-1 TO C15-5



PROJECT NO. R-5703
LENOIR COUNTY

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

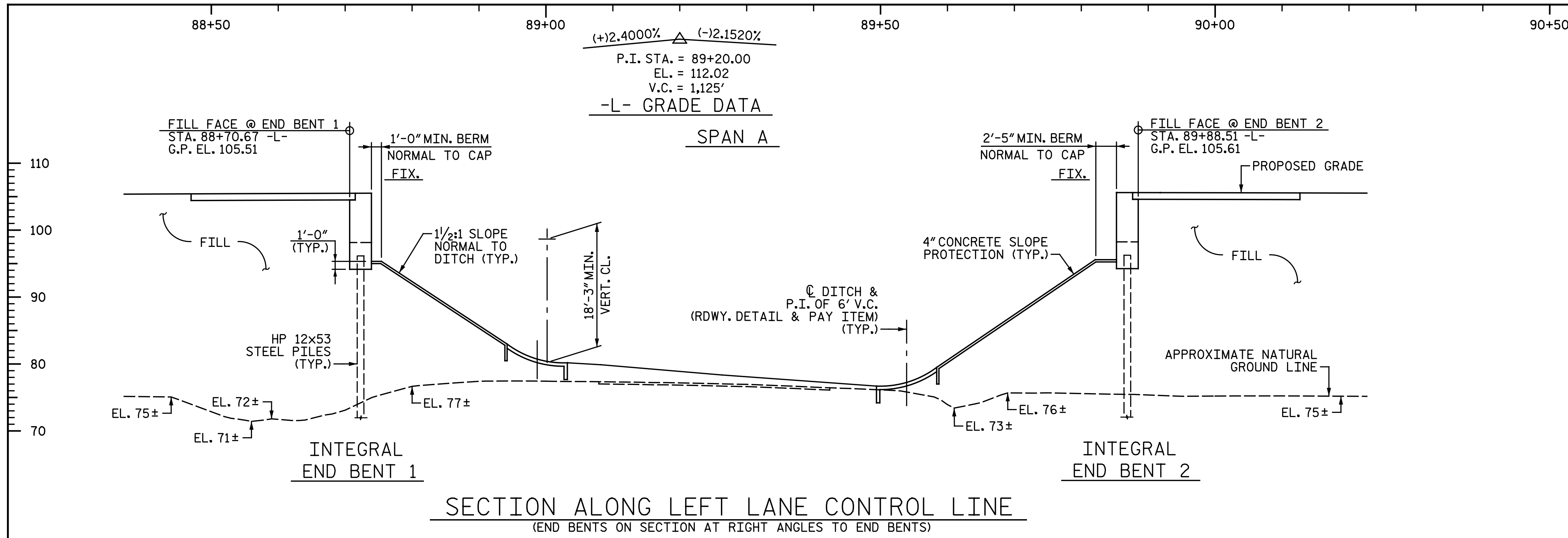
INDEX SHEET

DRAWN BY : C. E. MAYHEW DATE : 5-5-17
CHECKED BY : V. A. PATEL DATE : 7-19-17

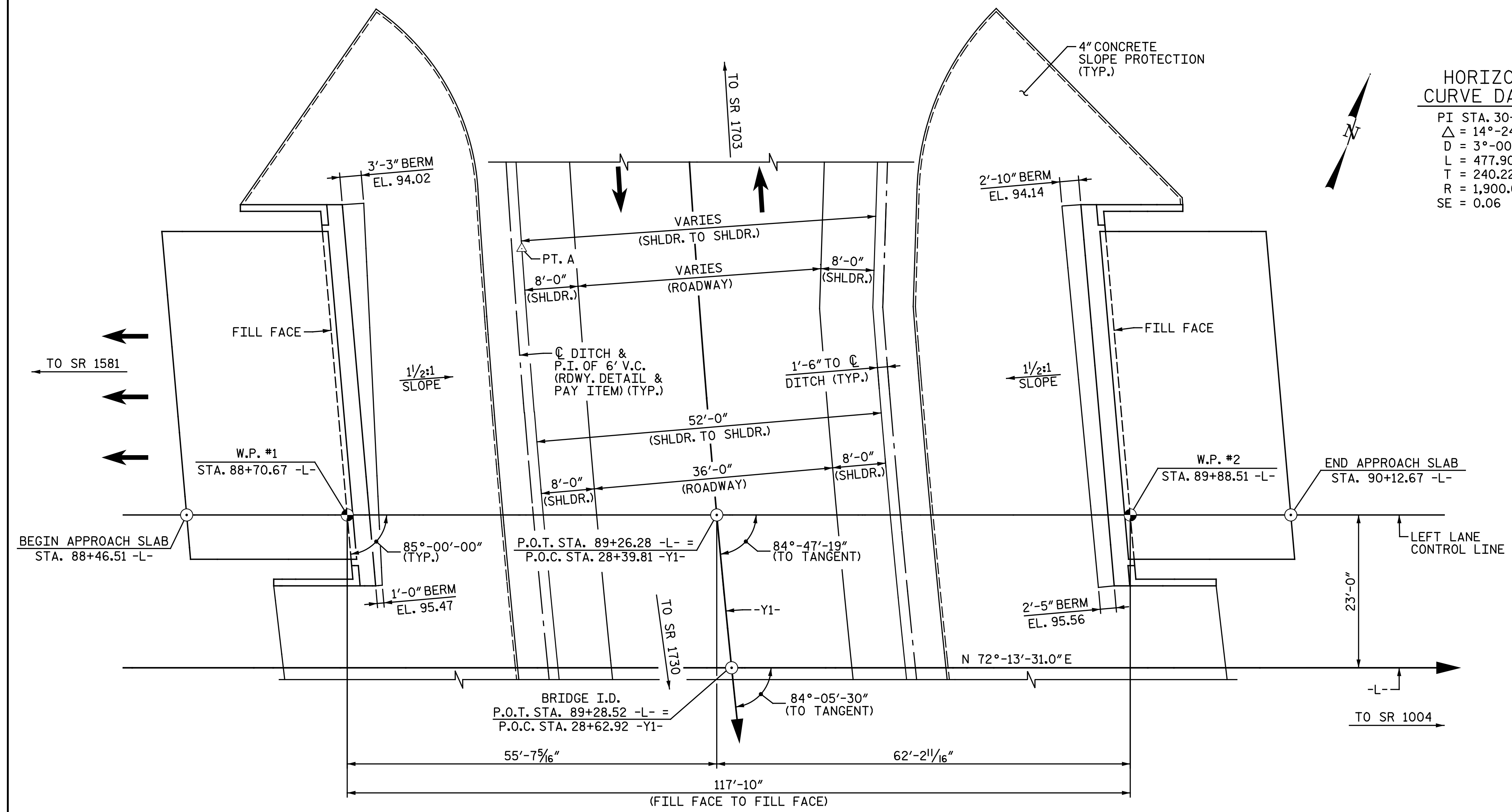
REVISIONS				SHEET NO.		
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			
2			4			

Michael Baker
INTERNATIONAL

Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084



SECTION ALONG LEFT LANE CONTROL LINE
(END BENTS ON SECTION AT RIGHT ANGLES TO END BENTS)



HORIZONTAL CURVE DATA -Y1-

PI STA. 30+62.34
 $\Delta = 14^\circ-24'-41.5''$ (LT)
 D = 3°-00'-56.0"
 L = 477.90'
 T = 240.22'
 R = 1,900.00'
 SE = 0.06

POINT	STATION ON -Y1-	OFFSET	ELEVATION ON -Y1-
A	27+97.78	26.0 RT	79.32

△ - POINT OF MINIMUM VERTICAL CLEARANCE.

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 89+28.52 -L-
 28+62.92 -Y1-
 SHEET 1 OF 3 BRIDGE No. 208



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER NC 58
 BETWEEN SR 1581 AND SR 1004

DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED

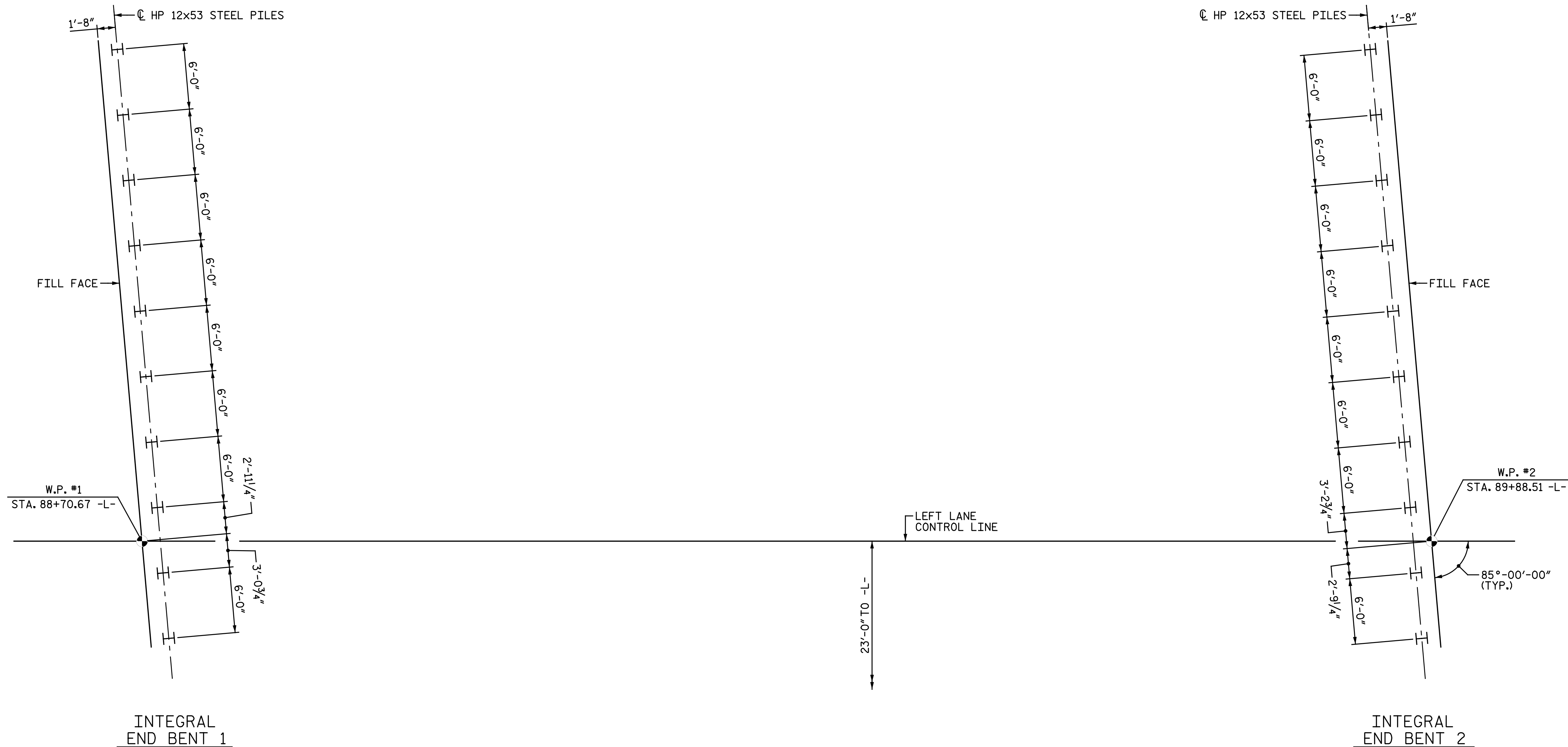
LEFT LANE

DRAWN BY : M. D. MAYHEW DATE : 4-13-17
 CHECKED BY : V. A. PATEL DATE : 5-18-17

BRIDGE I.D.
 P.O.T. STA. 89+28.52 -L- =
 P.O.C. STA. 28+62.92 -Y1-
 PLAN
 (PILES NOT SHOWN IN PLAN VIEW FOR CLARITY)

REVISIONS						SHEET NO. SI-1
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS
2			4			25

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 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No.: F-1084



FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINES.
 ALL PILES ARE VERTICAL.

NOTES:
 FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.
 PILES AT END BENT NO.1 AND END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 122.5 TONS PER PILE.
 DRIVE PILES AT END BENT NO.1 AND END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 205 TONS PER PILE.
 TESTING THE FIRST PRODUCTION PILE WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED AT END BENT NO.1 OR END BENT NO.2. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS AND FOR PILE DRIVING CRITERIA, SEE PILE DRIVING CRITERIA PROVISION.
 IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 40-50 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT NO.1 AND END BENT NO.2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3D(X2) OF THE STANDARD SPECIFICATIONS.

OBSERVE A 2 MONTH WAITING PERIOD AFTER CONSTRUCTING THE EMBANKMENT TO WITHIN 2 FT OF FINISHED GRADE BEFORE BEGINNING END BENT CONSTRUCTION AT END BENT NO.1 AND END BENT NO. 2. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SPECIAL PROVISIONS,

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-
28+62.92 -Y1-
 SHEET 2 OF 3



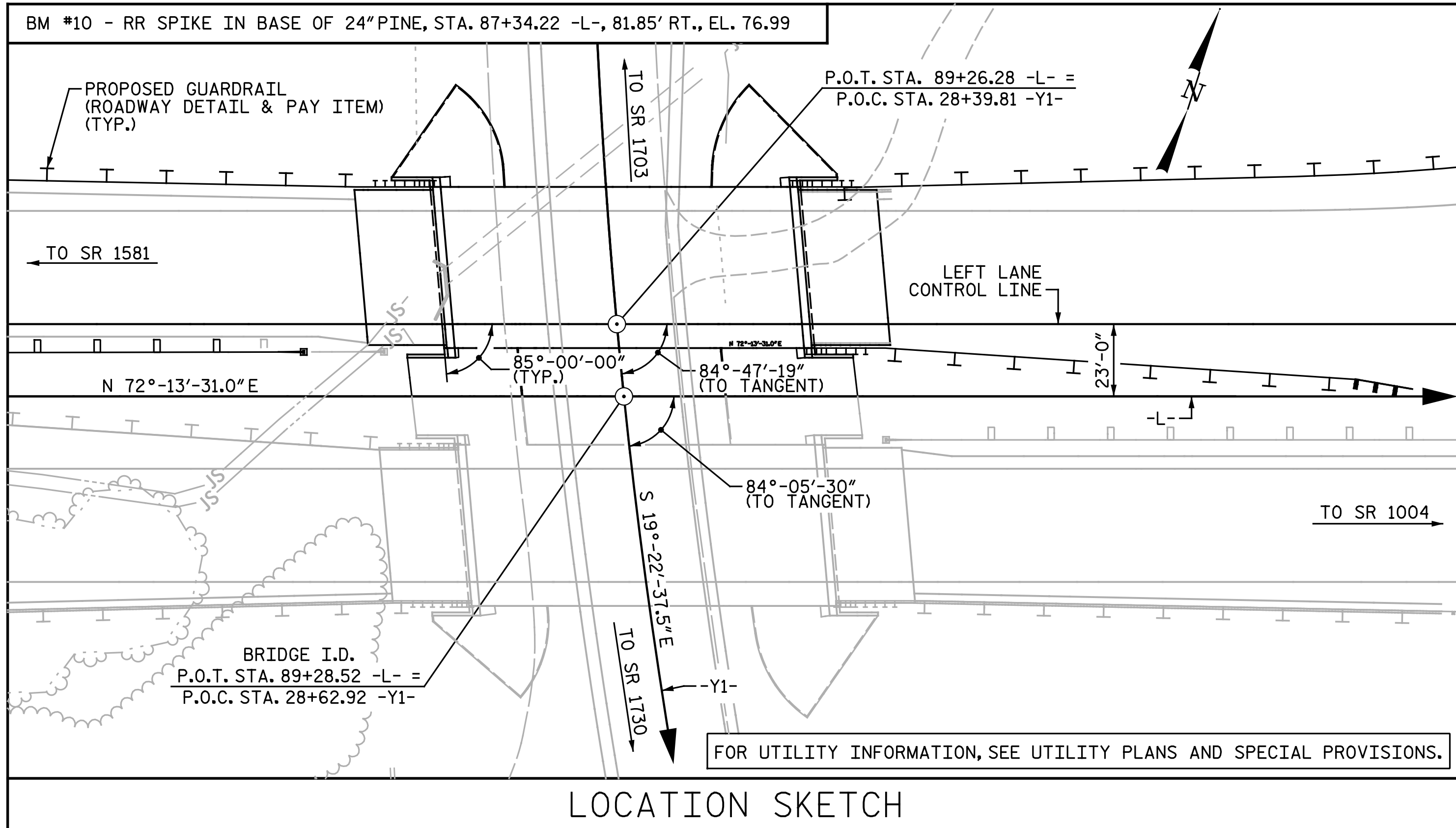
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER NC 58
 BETWEEN SR 1581 AND SR 1004

7/26/2017
 DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED

LEFT LANE

DRAWN BY : M. D. MAYHEW DATE : 4-17-17
 CHECKED BY : V. A. PATEL DATE : 5-18-17

NO.		BY:	DATE:	NO.		BY:	DATE:	SHEET NO.
1				3				SI-2
2				4				TOTAL SHEETS 25



NOTES:

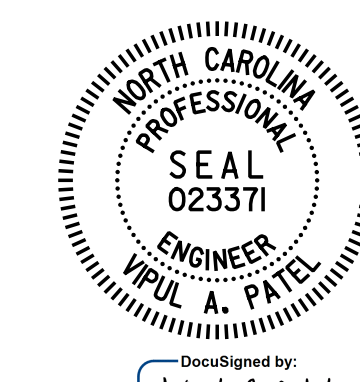
- ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
- THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.
- PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
- FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

LOCATION SKETCH

TOTAL BILL OF MATERIAL

LOCATION	PDA TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	MODIFIED 72\"/>								
	EA.	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	NO.	LIN. FT.	EA.	NO.	LIN. FT.	EA.	LIN. FT.	SQ. YDS.	LUMP SUM
SUPERSTRUCTURE		6,039	7,387				5	579.48					232.33		LUMP SUM
END BENT 1				42.8		6,893			10	10	800	5		350	
END BENT 2				42.8		6,893			10	10	800	5		430	
TOTAL	1	6,039	7,387	85.6	LUMP SUM	13,786	5	579.48	20	20	1,600	10	232.33	780	LUMP SUM

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-
28+62.92 -Y1-
 SHEET 3 OF 3



7/26/2017

Digitally signed by V. A. Patel

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER NC 58
 BETWEEN SR 1581 AND SR 1004

LEFT LANE

DRAWN BY : M. D. MAYHEW DATE : 4-17-17
 CHECKED BY : V. A. PATEL DATE : 5-18-17

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	SI-3
1			3			TOTAL SHEETS
2			4			25

Michael Baker INTERNATIONAL
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 Cary, North Carolina 27518
 NC License No. : F-1084

LOAD FACTORS:

DESIGN LOAD RATING SERVICE FACTORS	LIMIT STATE	γ_{DC}	γ_{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING #	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE						COMMENT NUMBER		
						LIVE-LOAD FACTORS (γ_{LL})	MOMENT					SHEAR					LIVE-LOAD FACTORS (γ_{LL})	MOMENT						
							DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)		DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION		DISTANCE FROM LEFT END OF SPAN (ft)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	①	1.06	--	1.75	0.971	1.41	A	5	57.24	0.984	1.19	A	5	80.42	1.00	0.971	1.06	A	5	57.24	1, 2	
	HL-93 (OPERATING)	N/A		1.74	--	1.35	0.971	1.83	A	5	57.24	1.012	1.74	A	3	92.01	N/A	-	-	-	-	-	-	2
	HS-20 (INVENTORY)	36.000	②	1.53	55.08	1.75	0.971	2.03	A	5	57.24	1.012	1.80	A	3	92.01	1.00	0.971	1.53	A	5	57.24	1, 2	
	HS-20 (OPERATING)	36.000		2.38	85.68	1.35	0.971	2.64	A	5	57.24	1.012	2.38	A	3	92.01	N/A	-	-	-	-	-	-	2
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500		3.70	49.95	1.40	0.971	6.14	A	5	57.24	1.012	5.90	A	3	92.01	1.00	0.971	3.70	A	5	57.24	1, 2
		SNGARBS2	20.000		2.66	53.20	1.40	0.971	4.40	A	5	57.24	1.012	4.08	A	3	92.01	1.00	0.971	2.66	A	5	57.24	1, 2
		SNAGRIS2	22.000		2.47	54.34	1.40	0.971	4.10	A	5	57.24	1.012	3.75	A	3	92.01	1.00	0.971	2.47	A	5	57.24	1, 2
		SNCOTTS3	27.250		1.84	50.14	1.40	0.971	3.05	A	5	57.24	1.012	2.85	A	3	92.01	1.00	0.971	1.84	A	5	57.24	1, 2
		SNAGGRS4	34.925		1.50	52.39	1.40	0.971	2.48	A	5	57.24	1.012	2.29	A	3	92.01	1.00	0.971	1.50	A	5	57.24	1, 2
		SNS5A	35.550		1.47	52.26	1.40	0.971	2.43	A	5	57.24	1.012	2.30	A	3	92.01	1.00	0.971	1.47	A	5	57.24	1, 2
		SNS6A	39.950		1.33	53.13	1.40	0.971	2.20	A	5	57.24	1.012	2.07	A	3	92.01	1.00	0.971	1.33	A	5	57.24	1, 2
		SNS7B	42.000		1.27	53.34	1.40	0.971	2.10	A	5	57.24	1.012	2.00	A	3	92.01	1.00	0.971	1.27	A	5	57.24	1, 2
	TRUCK TRACTOR SEMI-TRAILER (TST)	TNAGRIT3	33.000		1.62	53.46	1.40	0.971	2.68	A	5	57.24	1.012	2.51	A	3	92.01	1.00	0.971	1.62	A	5	57.24	1, 2
		TNT4A	33.075		1.62	53.58	1.40	0.971	2.68	A	5	57.24	1.012	2.46	A	3	92.01	1.00	0.971	1.62	A	5	57.24	1, 2
		TNT6A	41.600		1.31	54.50	1.40	0.971	2.17	A	5	57.24	1.012	2.10	A	3	92.01	1.00	0.971	1.31	A	5	57.24	1, 2
		TNT7A	42.000		1.31	55.02	1.40	0.971	2.16	A	5	57.24	1.012	2.07	A	3	92.01	1.00	0.971	1.31	A	5	57.24	1, 2
		TNT7B	42.000		1.33	55.86	1.40	0.971	2.21	A	5	57.24	1.012	1.97	A	3	92.01	1.00	0.971	1.33	A	5	57.24	1, 2
		TNAGRIT4	43.000		1.28	55.04	1.40	0.971	2.12	A	5	57.24	1.012	1.91	A	3	92.01	1.00	0.971	1.28	A	5	57.24	1, 2
		TNAGT5A	45.000		1.22	54.90	1.40	0.971	2.01	A	5	57.24	1.012	1.87	A	3	92.01	1.00	0.971	1.22	A	5	57.24	1, 2
		TNAGT5B	45.000	③	1.21	54.45	1.40	0.971	2.00	A	5	57.24	1.012	1.81	A	3	92.01	1.00	0.971	1.21	A	5	57.24	1, 2

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- A SERVICE III LIVE LOAD FACTOR OF 1.0 WAS USED TO BE CONSISTENT WITH THE VALUE USED DURING DESIGN.
- DISTANCE FROM LEFT END OF SPAN IS GIVEN WITH RESPECT TO CENTERLINE OF BEARING AND IS MEASURED ALONG THE CONTROLLING GIRDER.

CONTROLLING LOAD RATING

① DESIGN LOAD RATING (HL-93)

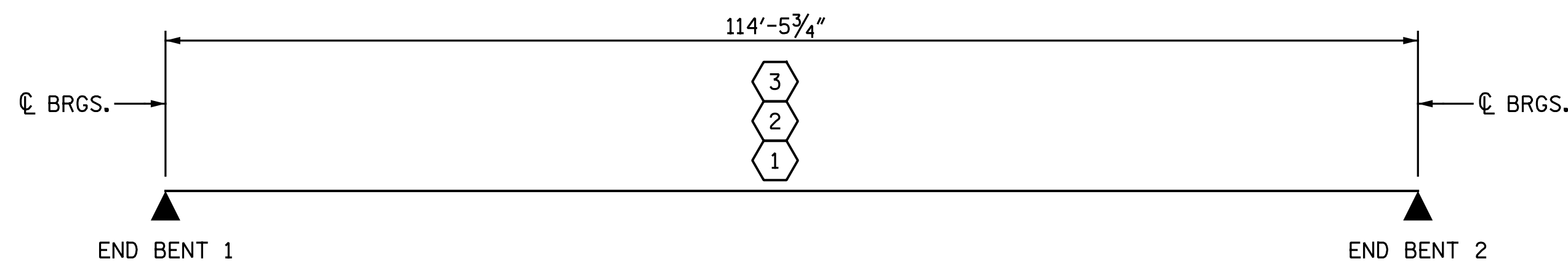
② DESIGN LOAD RATING (HS-20)

③ LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

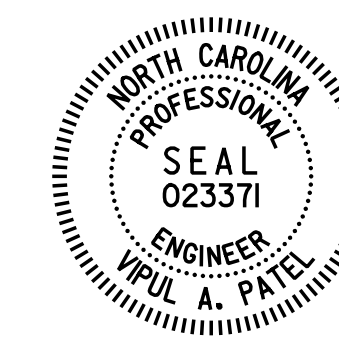
GIRDER LOCATION

GIRDER LOCATION IS PROVIDED USING GIRDER NUMBER, WHERE GIRDER 1 IS THE LEFT EXTERIOR GIRDER LOOKING AHEAD STATION. SEE "GIRDER LAYOUT" SHEET FOR ALL GIRDER LOCATIONS.



LRFR SUMMARY

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 89+28.52 -L-



7/26/2017

Designed by: V. A. Patel

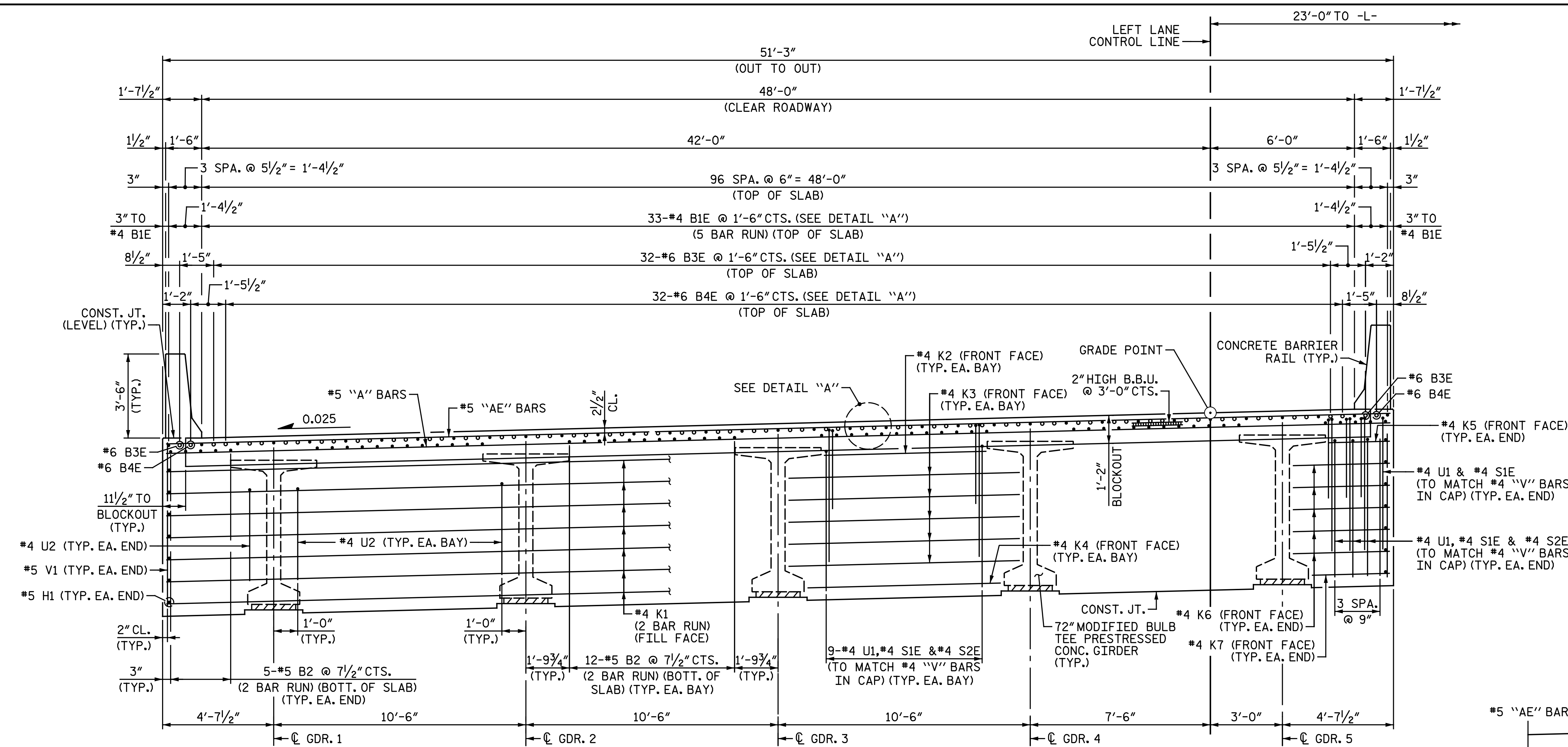
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD LRFR SUMMARY FOR PRESTRESSED CONCRETE GIRDERS (NON-INTERSTATE TRAFFIC) LEFT LANE

ASSEMBLED BY : M. D. MAYHEW	DATE : 5-11-17
CHECKED BY : V. A. PATEL	DATE : 5-18-17
DRAWN BY : MAA 1/08	REV. 11/2/08RR MAA/GM
CHECKED BY : GM/DI 2/08	REV. 10/1/11 MAA/GM

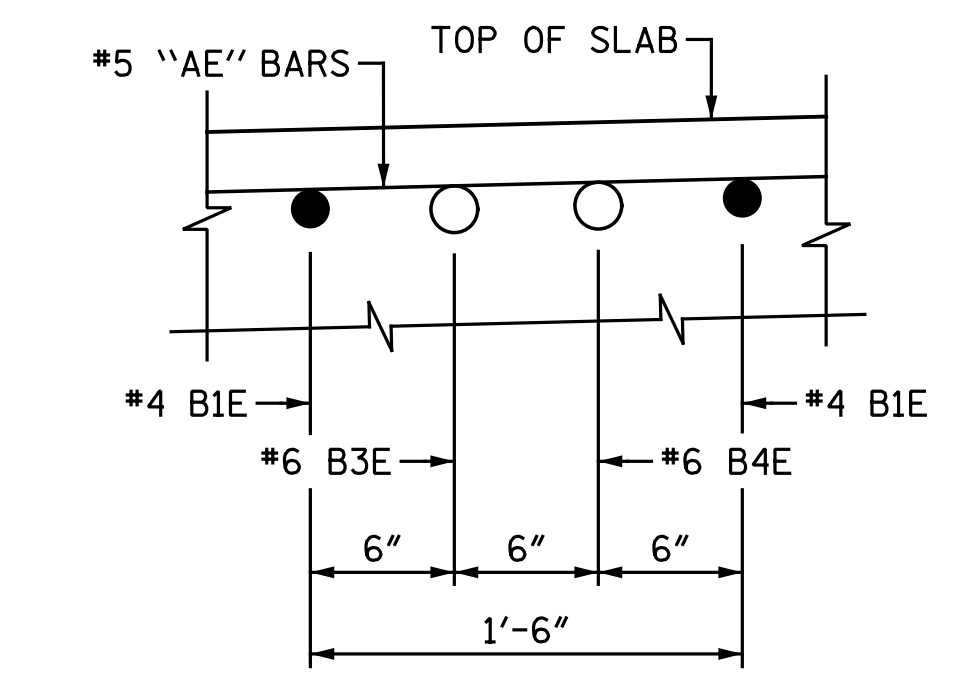
Michael Baker INTERNATIONAL
 Michael Baker Engineering
 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No. : F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	SI-4
1			3			TOTAL SHEETS
2			4			25

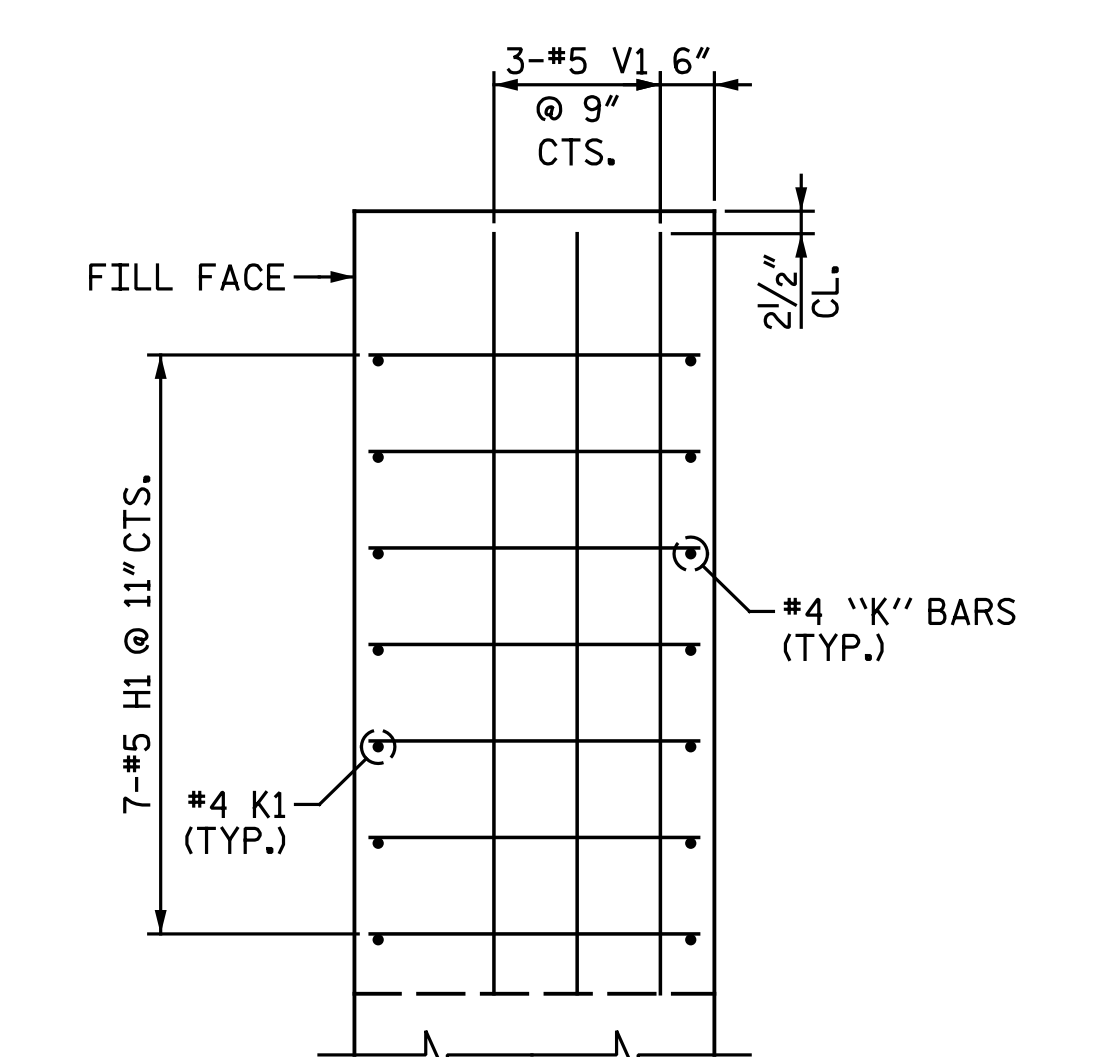


TYPICAL SECTION AT INTEGRAL END BENT

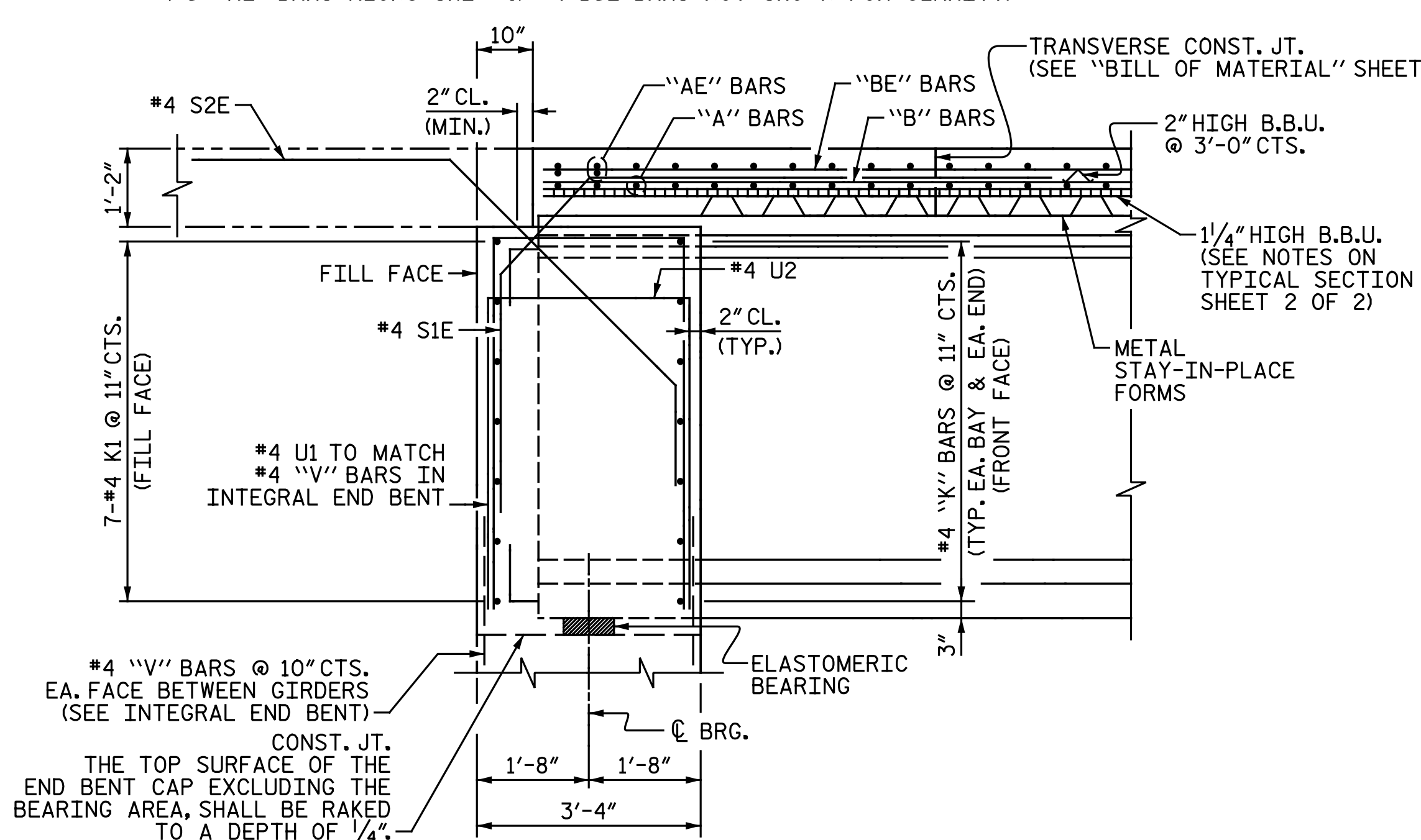
(END BENT 1 SHOWN, END BENT 2 SIMILAR)
 (#5 "AE" BARS ALONG SKEW & #4 B5E BARS NOT SHOWN FOR CLARITY)



DETAIL "A"



END OF DIAPHRAGM DETAIL
 (END BENT 1 SHOWN, END BENT 2 SIMILAR)



END OF GIRDER DETAIL AT INTEGRAL END BENT

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-

SHEET 1 OF 2



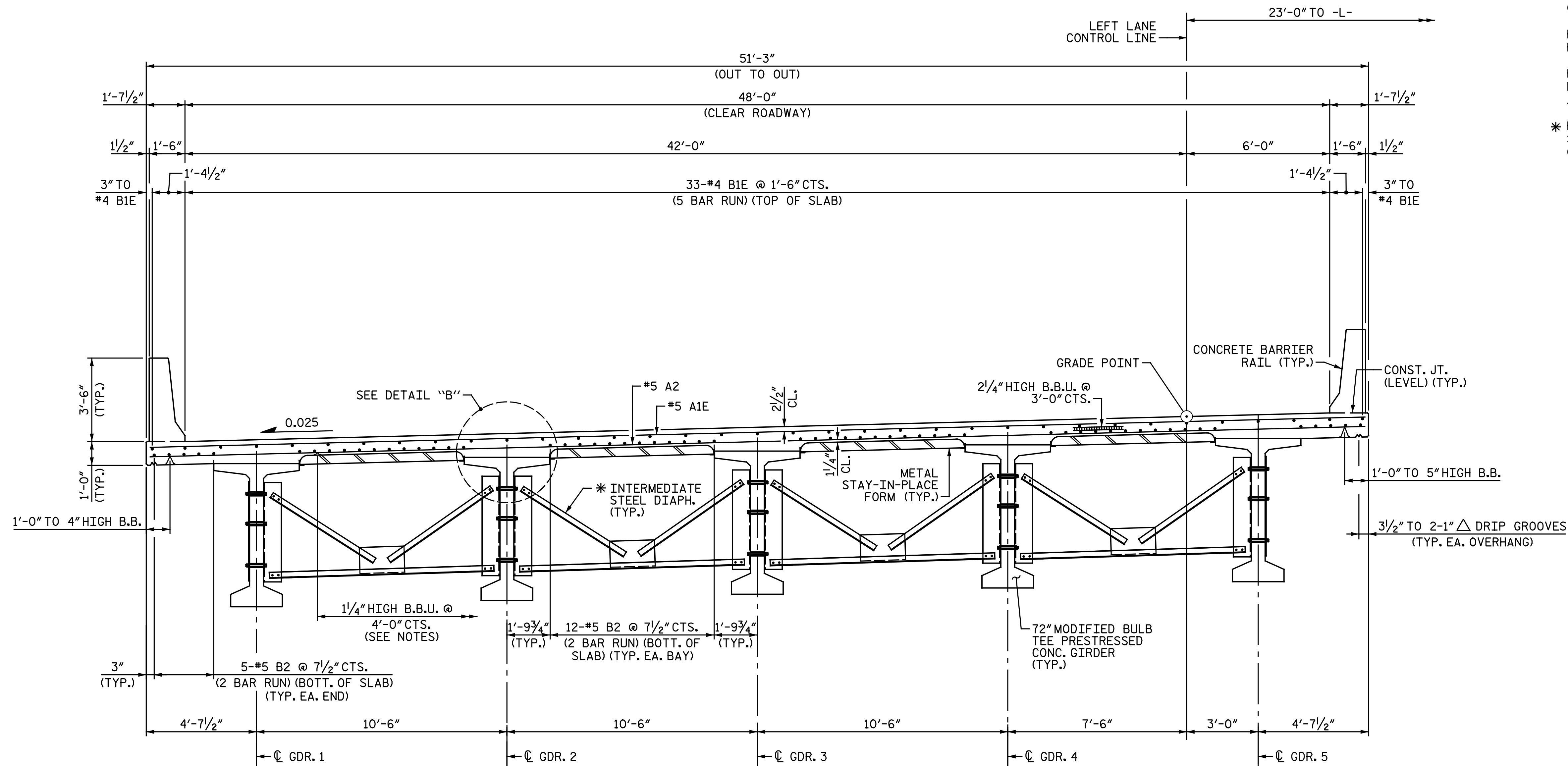
7/26/2017

DOCUMENT NOT CONSIDERED FINAL
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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE					
TYPICAL SECTION					
LEFT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO. SI-5					TOTAL SHEETS 25

DRAWN BY: M. D. MAYHEW DATE: 1-13-17
 CHECKED BY: V. A. PATEL DATE: 3-13-17



TYPICAL SECTION @ INTERMEDIATE DIAPHRAGM

NOTES:

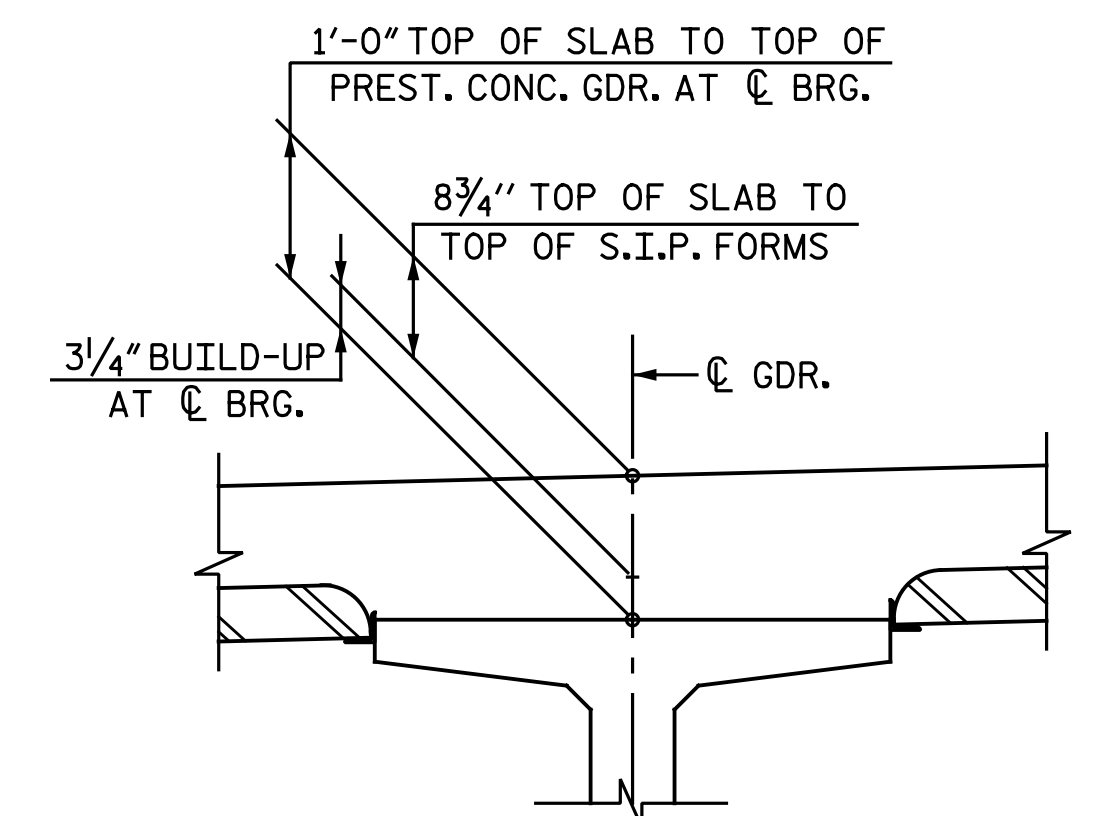
PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.

LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS AND TO FACILITATE INSTALLATION OF CONCRETE BARRIER RAIL REINFORCEMENT.

FOR CONCRETE BARRIER RAIL DETAILS, SEE "CONCRETE BARRIER RAIL" SHEET.

PREVIOUSLY CAST CONCRETE IN A SPAN SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE SPAN.

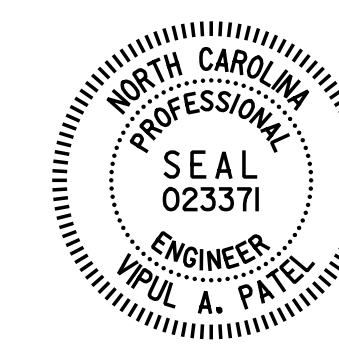
* FOR DETAILS OF INTERMEDIATE DIAPHRAGMS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 72" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS" SHEET.



DETAIL "B"

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-

SHEET 2 OF 2



7/26/2017

DocuSigned by:
Vipul A. Patel

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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 TYPICAL SECTION

LEFT LANE

REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			SI-6
2			4			TOTAL SHEETS 25

DRAWN BY : M. D. MAYHEW DATE : 1-17-17
 CHECKED BY : V. A. PATEL DATE : 3-13-17

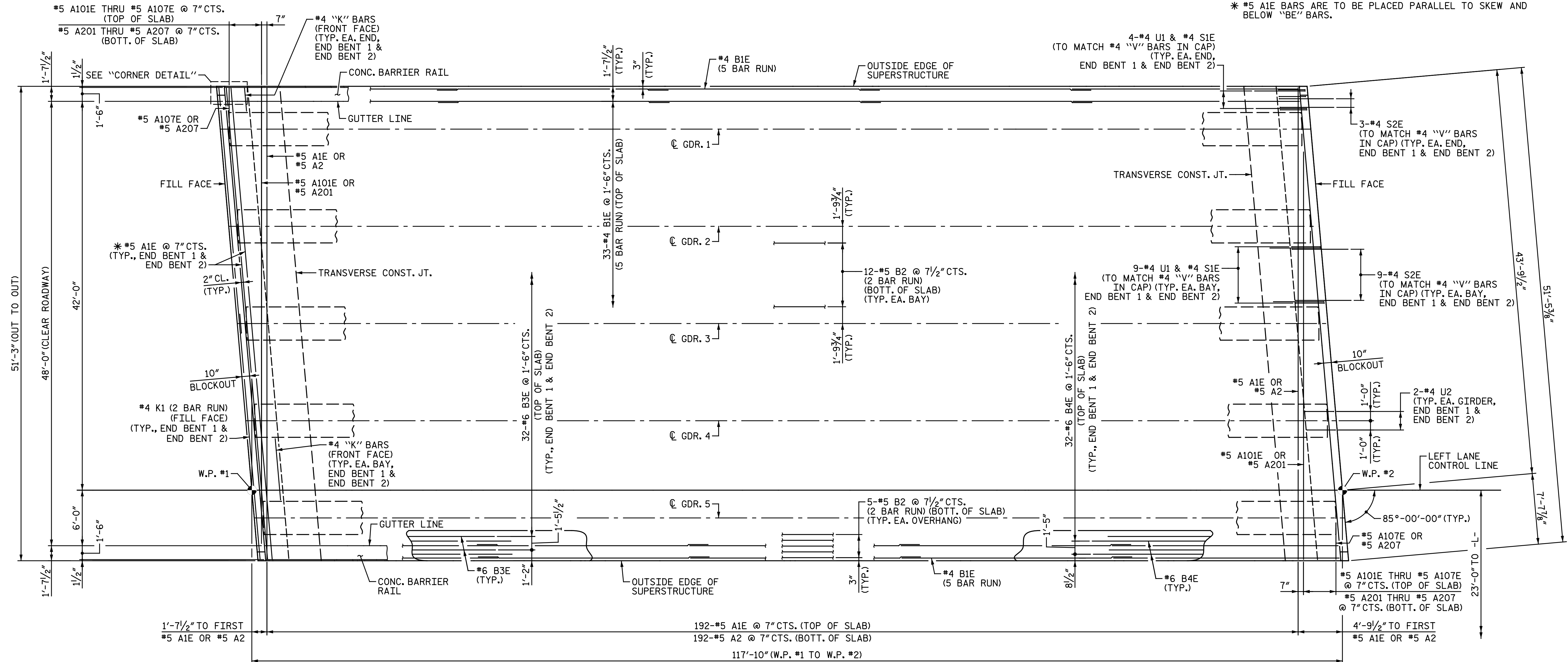
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 Michael Baker Engineering
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 Cary, North Carolina 27518
 NC License No. : F-1084

NOTES:

FOR REINFORCING STEEL IN CONCRETE BARRIER RAIL, SEE "CONCRETE BARRIER RAIL" SHEET.

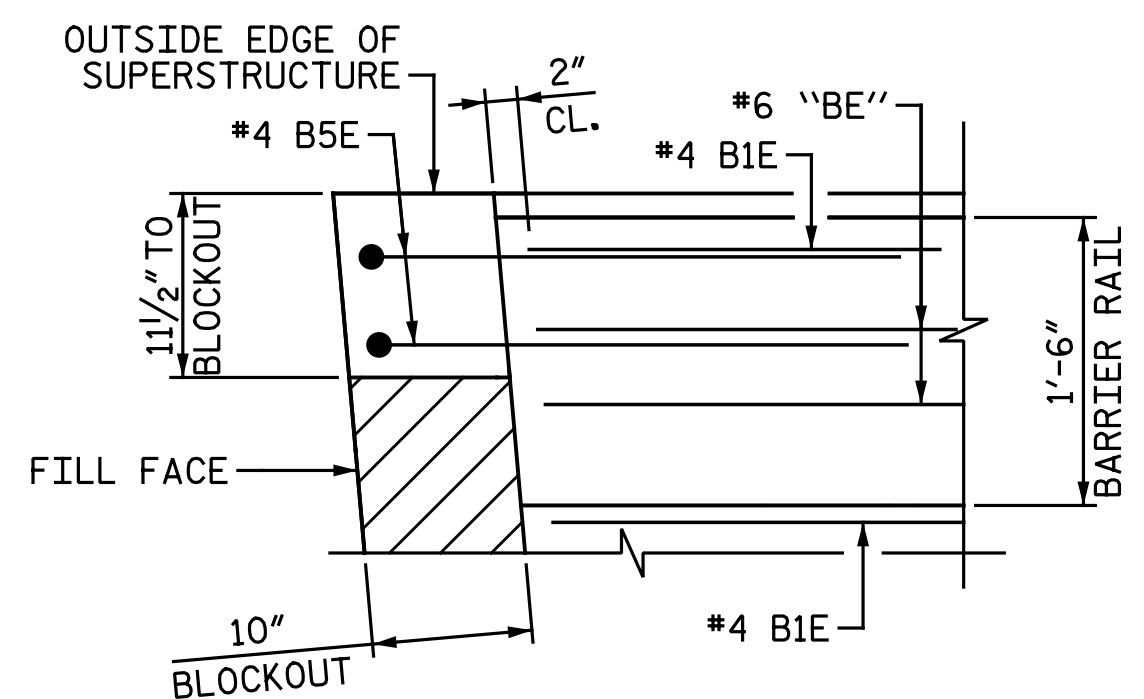
FOR DECK POURING SEQUENCE AND LOCATION OF TRANSVERSE CONSTRUCTION JOINTS, SEE "BILL OF MATERIAL" SHEET.

* #5 A1E BARS ARE TO BE PLACED PARALLEL TO SKEW AND BELOW "BE" BARS.



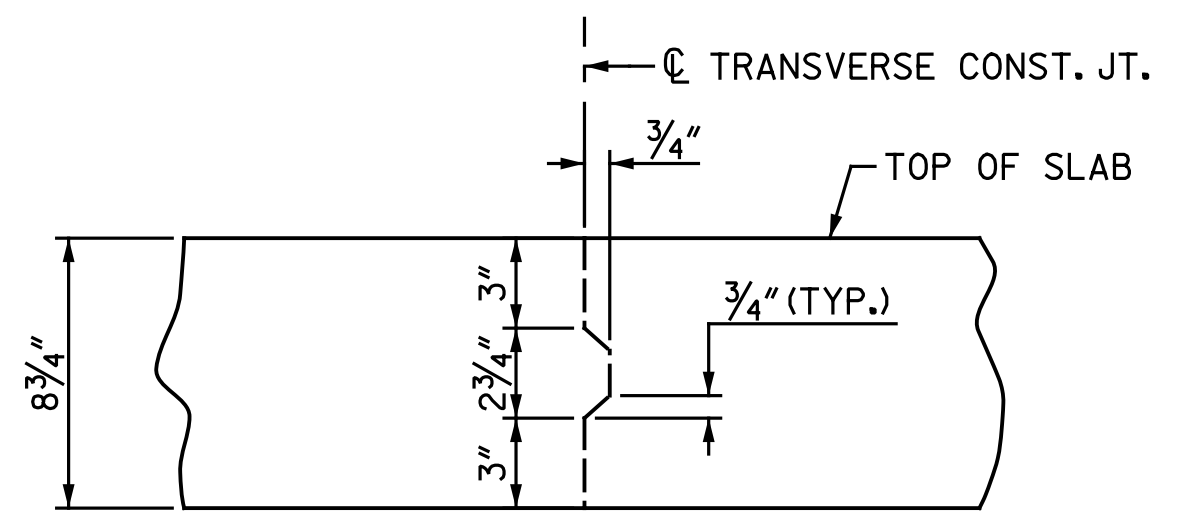
SPAN A
PLAN OF SPAN

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



CORNER DETAIL

ALL CORNERS SIMILAR.
 TRANSVERSE BARS NOT SHOWN FOR CLARITY.



TRANSVERSE CONST. JT. DETAIL

REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THROUGH JOINT.

SPLICE LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"
#5	2'-6"	2'-2"
#4 K1	-	2'-5"



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

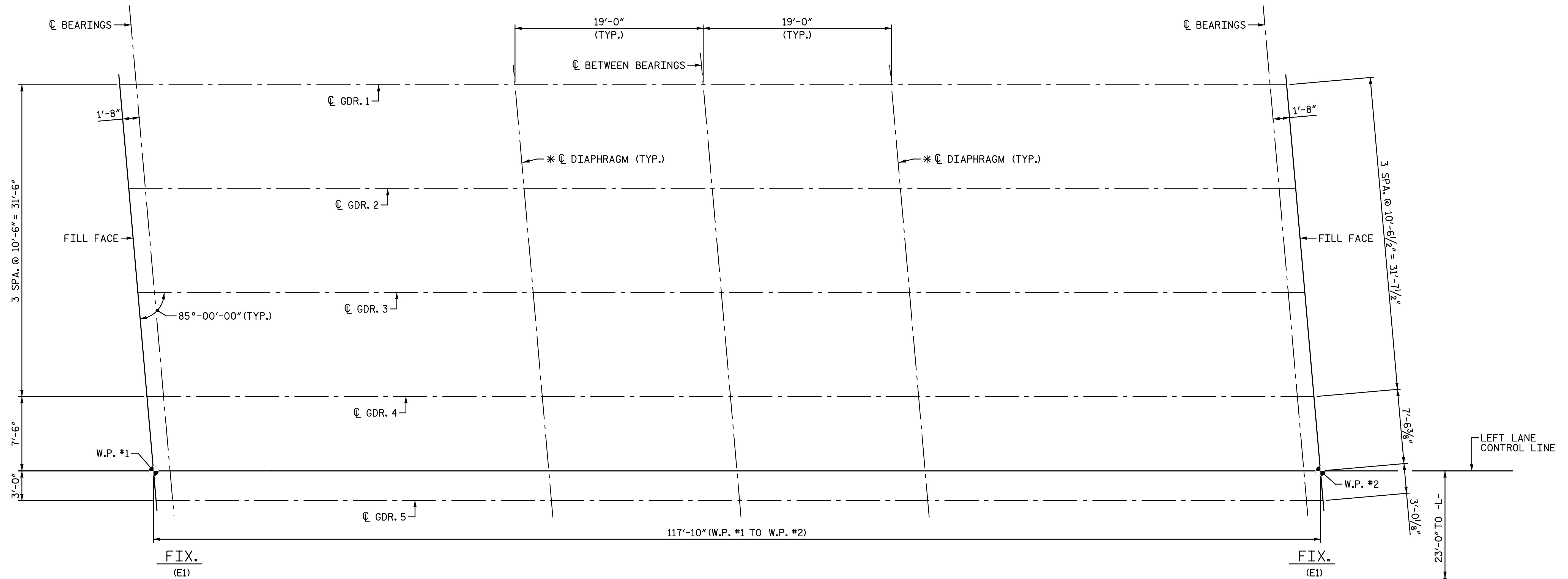
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 Michael Baker Engineering
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 Cary, North Carolina 27518
 NC License No.: F-1084

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 PLAN OF SPAN

LEFT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO.	SI-7
TOTAL SHEETS	25

DRAWN BY: M. D. MAYHEW DATE: 1-20-17
 CHECKED BY: V. A. PATEL DATE: 3-13-17

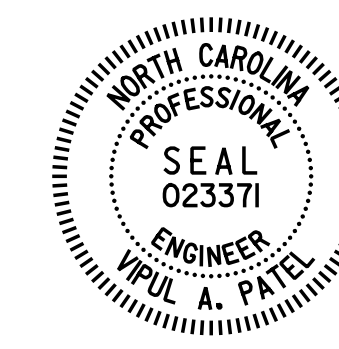


SPAN A

GIRDER LAYOUT

* INTERMEDIATE STEEL DIAPHRAGMS NOT SHOWN FOR CLARITY

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



7/26/2017

Designed by
Vipul A. Patel

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 GIRDER LAYOUT

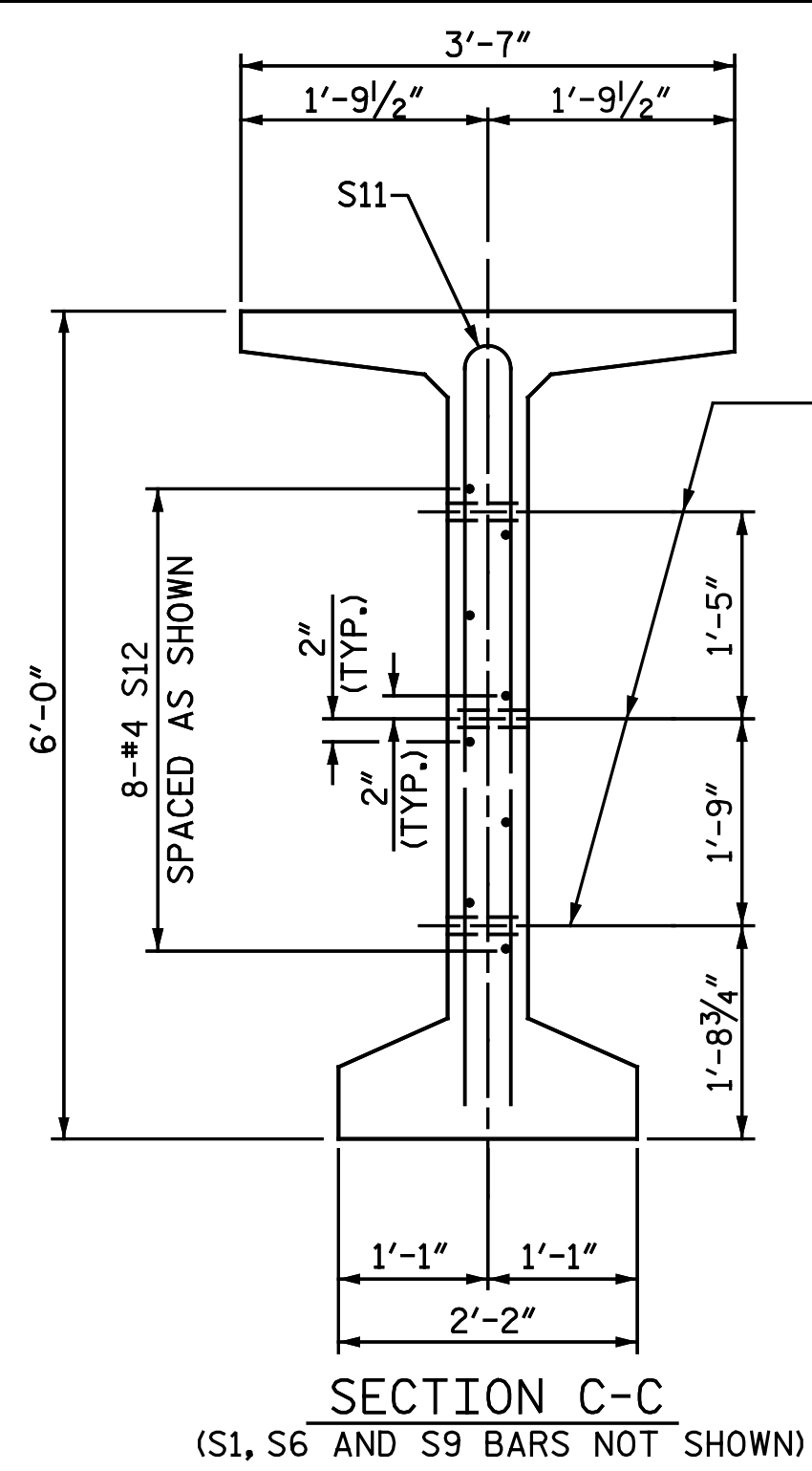
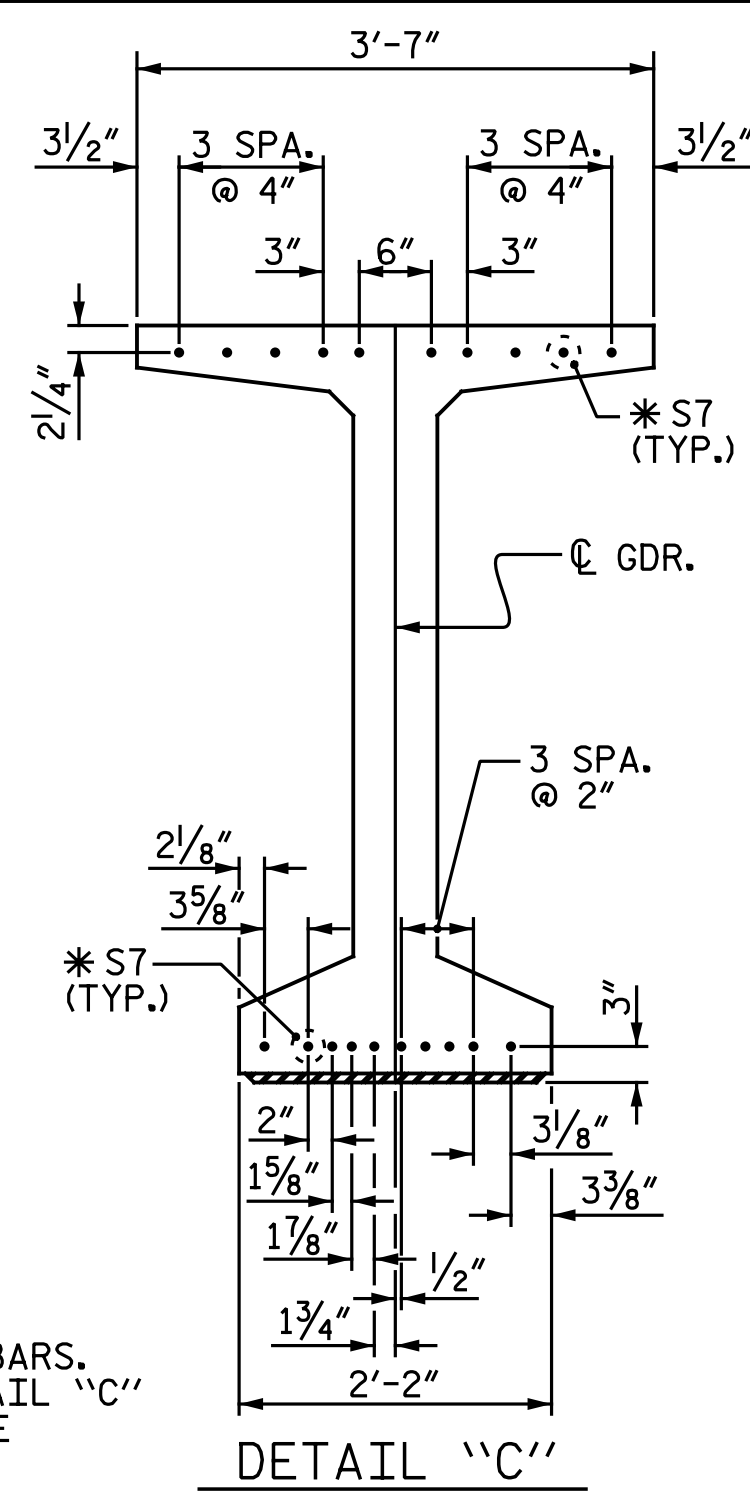
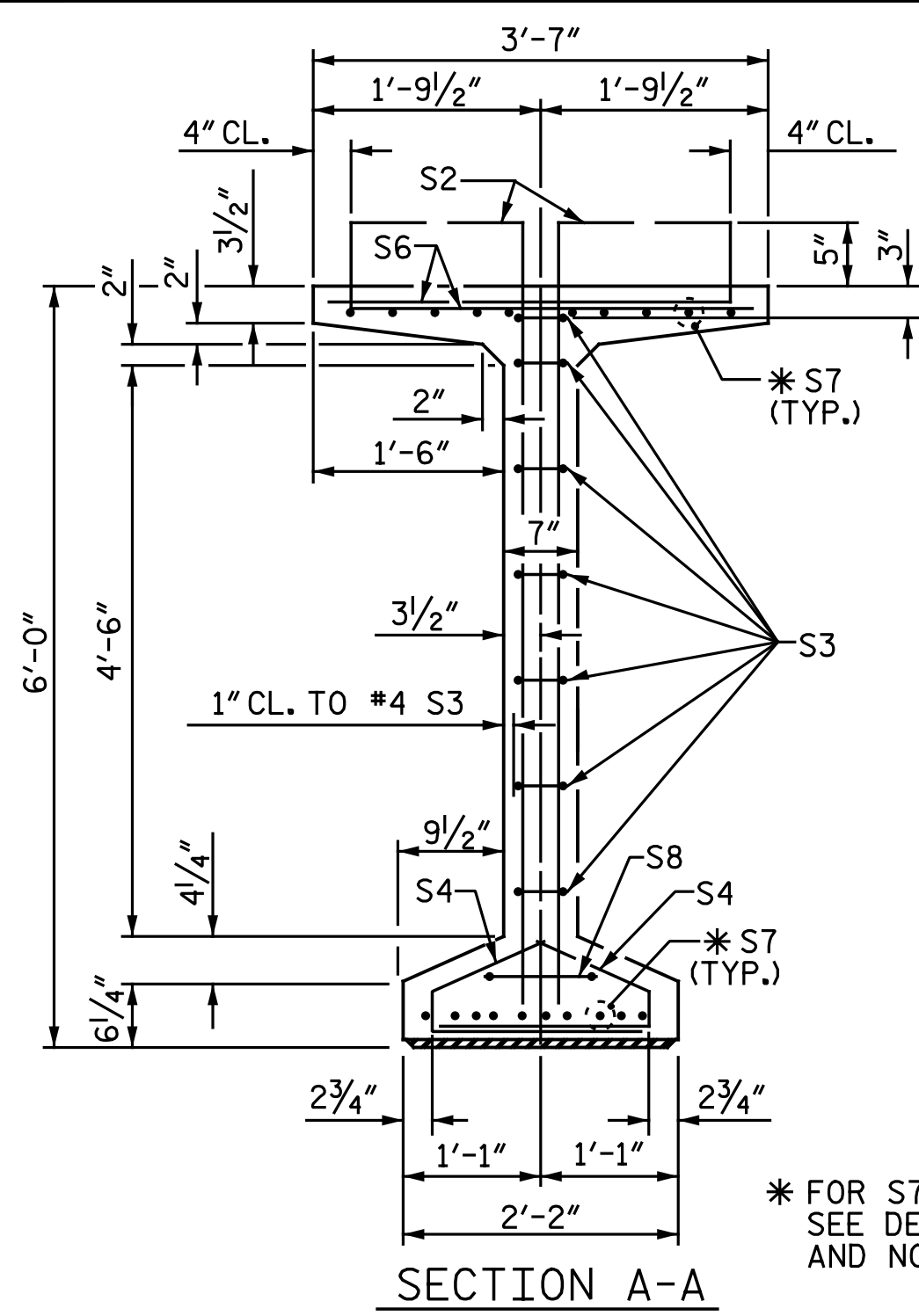
LEFT LANE

DOCUMENT NOT CONSIDERED FINAL
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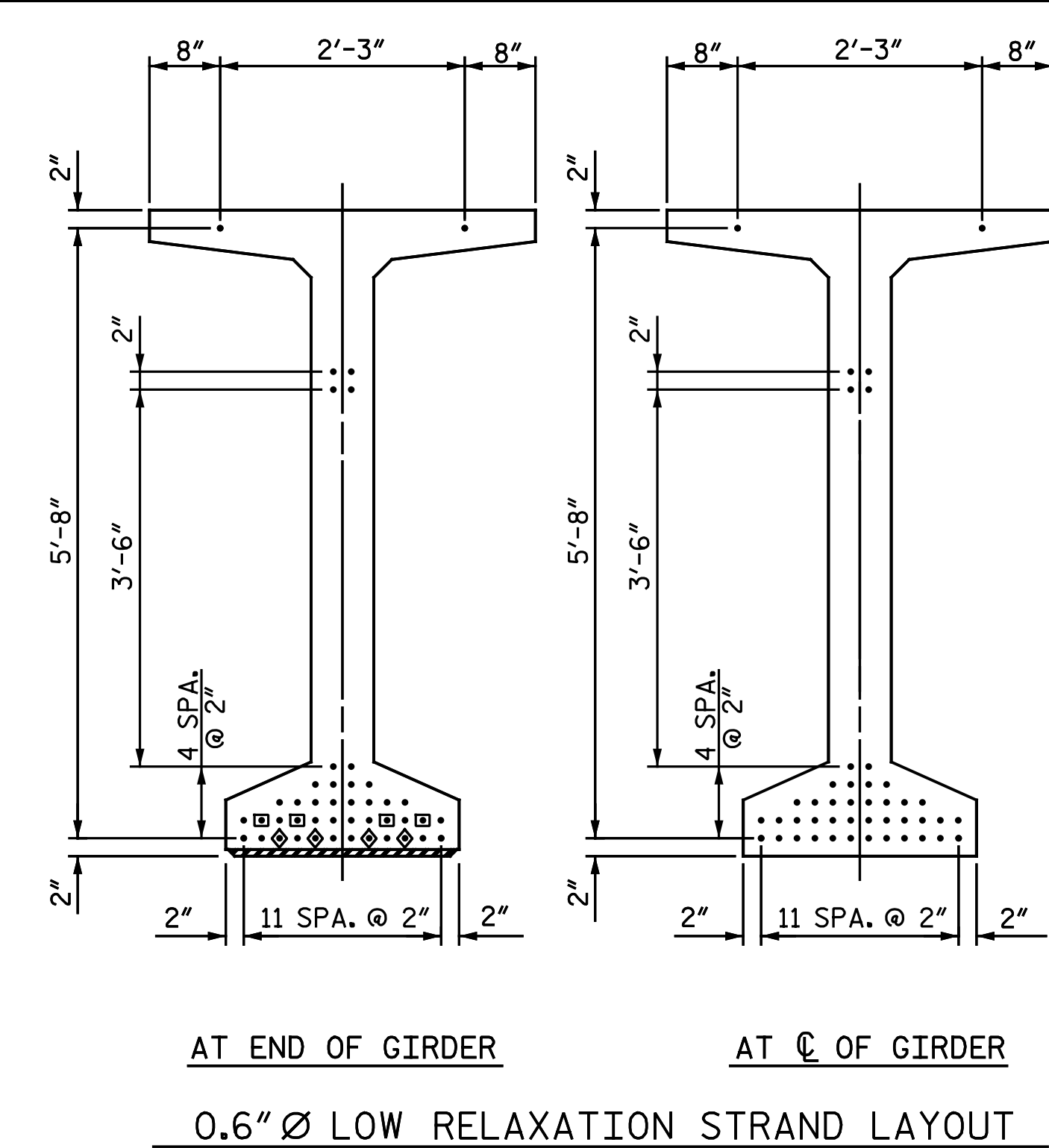
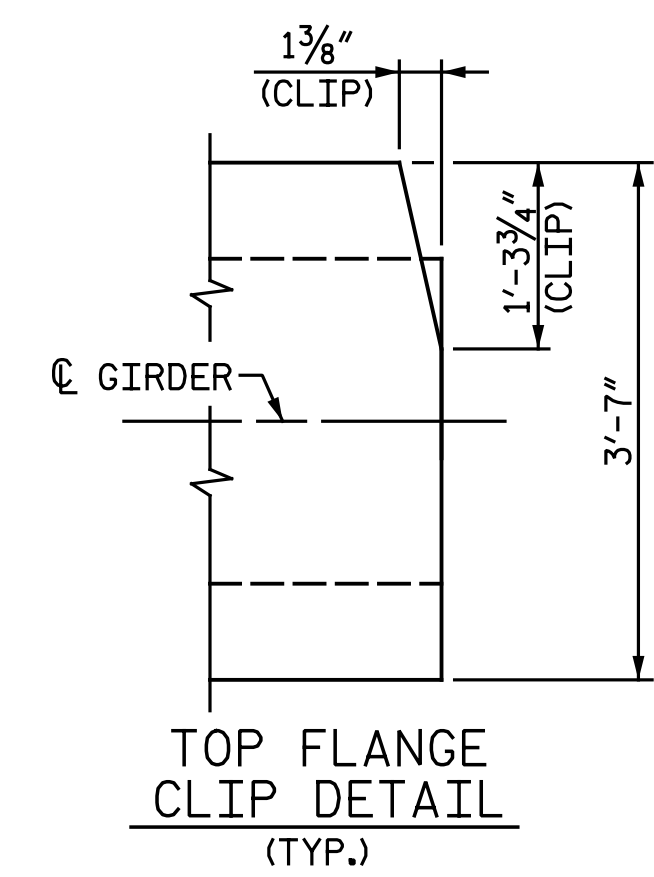
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	SI-8
1			3			TOTAL SHEETS
2			4			25

DRAWN BY : M. D. MAYHEW DATE : 1-20-17
 CHECKED BY : V. A. PATEL DATE : 3-13-17

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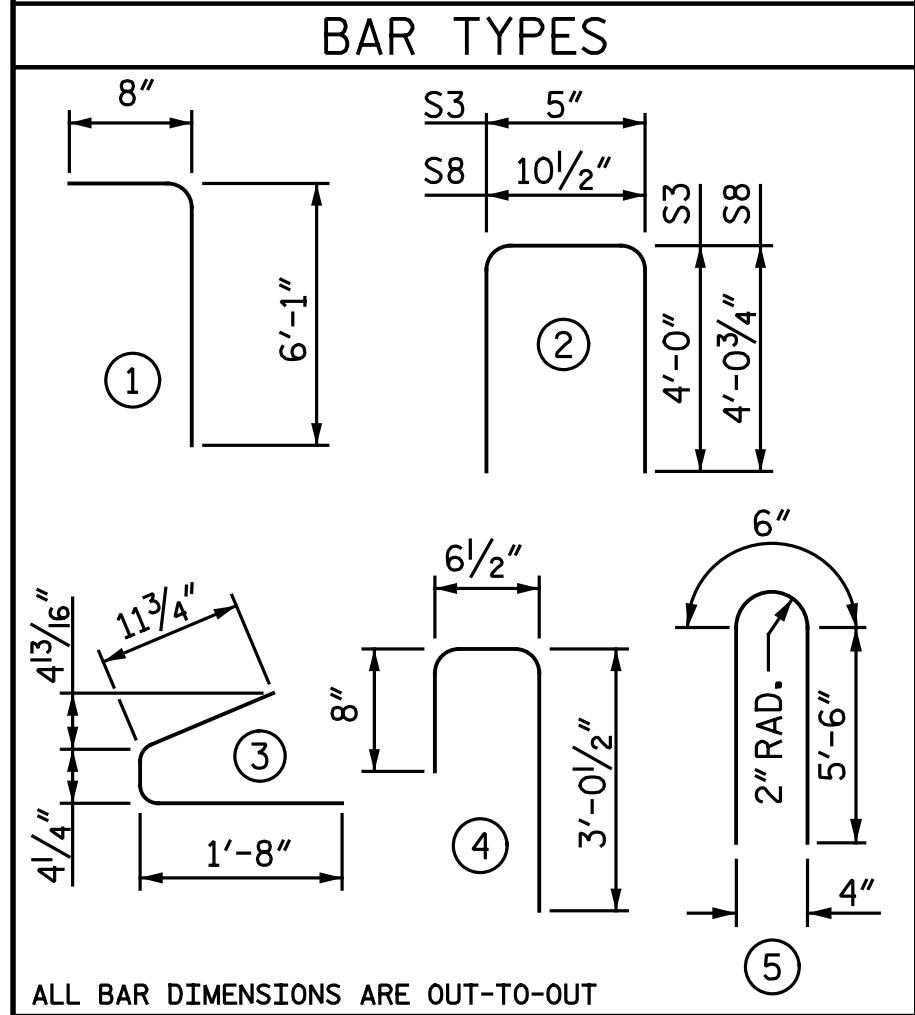
1/2" Ø FORMED HOLE. SEE GIRDER LAYOUT SHEET FOR LOCATION.



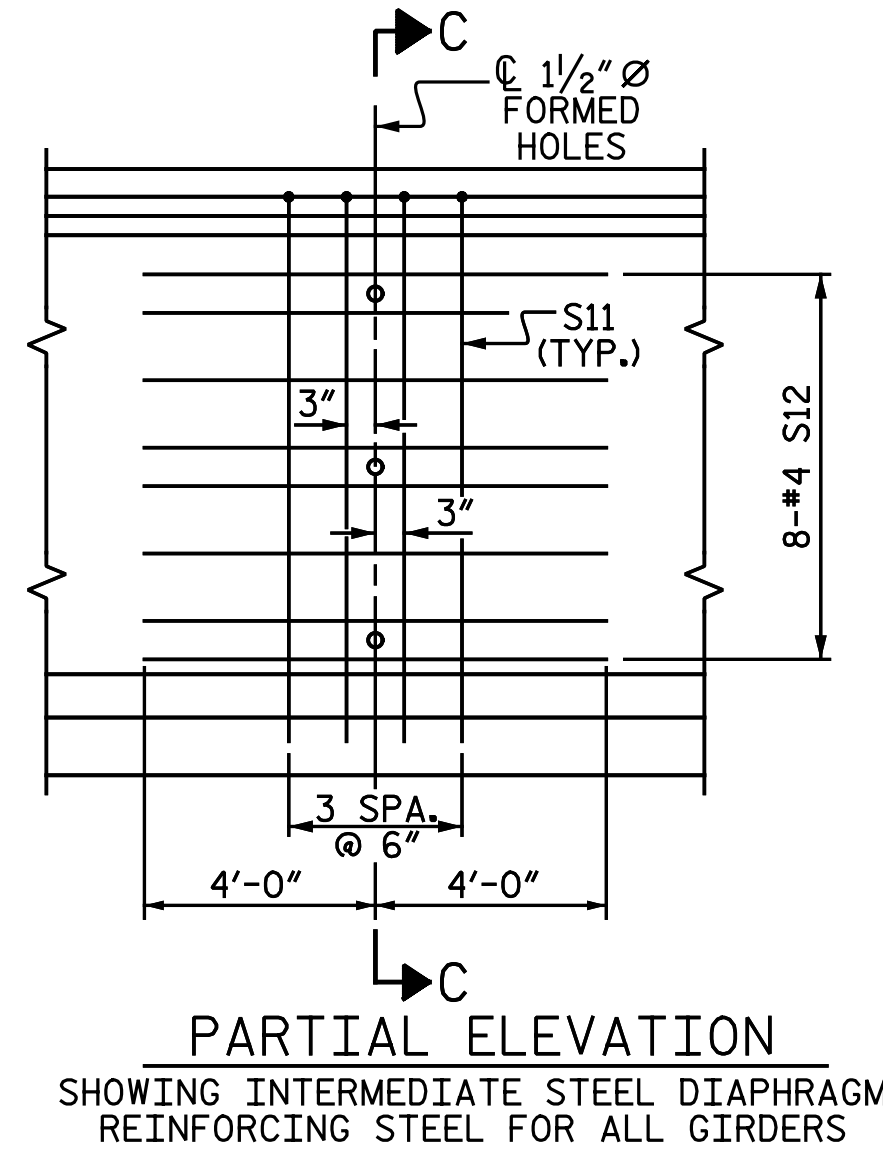
0.6" Ø L. R. GRADE 270 STRANDS		
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	174	#4	1	6'-9"	785
S2	36	#5	1	6'-9"	253
S3	14	#4	2	8'-5"	79
S4	120	#4	3	3'-0"	240
S6	210	#5	4	4'-3"	931
*S7	40	#5	STR	3'-8"	153
S8	2	#5	2	9'-0"	19
S9	44	#5	STR	3'-3"	149
S10	2	#3	STR	1'-10"	1
S11	8	#5	5	11'-6"	96
S12	16	#4	STR	8'-0"	86

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



- DEBONDING LEGEND**
- FULLY BONDED STRANDS
 - ◻ STRANDS DEBONDED FOR 4'-0" FROM END OF GIRDER
 - ◊ STRANDS DEBONDED FOR 10'-0" FROM END OF GIRDER



QUANTITIES FOR ONE GIRDER			
	REINFORCING STEEL (LB.)	9,500 PSI CONCRETE (C.Y.)	0.6" Ø L.R. STRANDS (No.)
GIRDER	2,792	24.8	44

GIRDERS REQUIRED		
NUMBER	LENGTH	TOTAL LENGTH
5	115'-10 3/4"	579.48'

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-



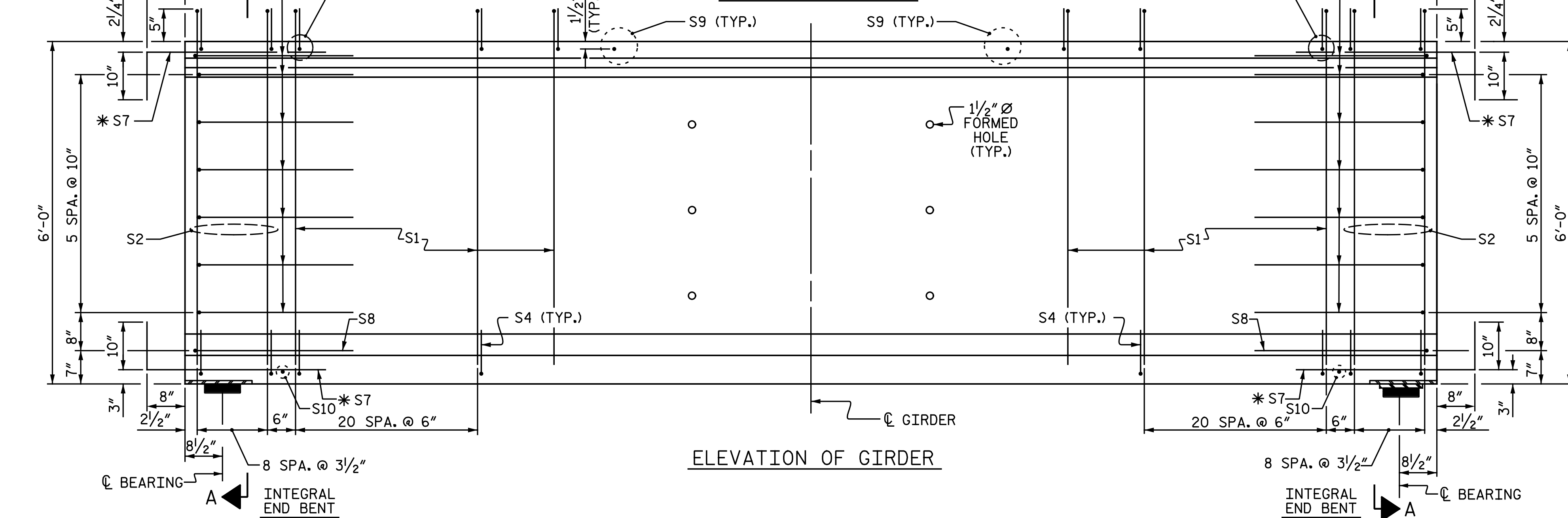
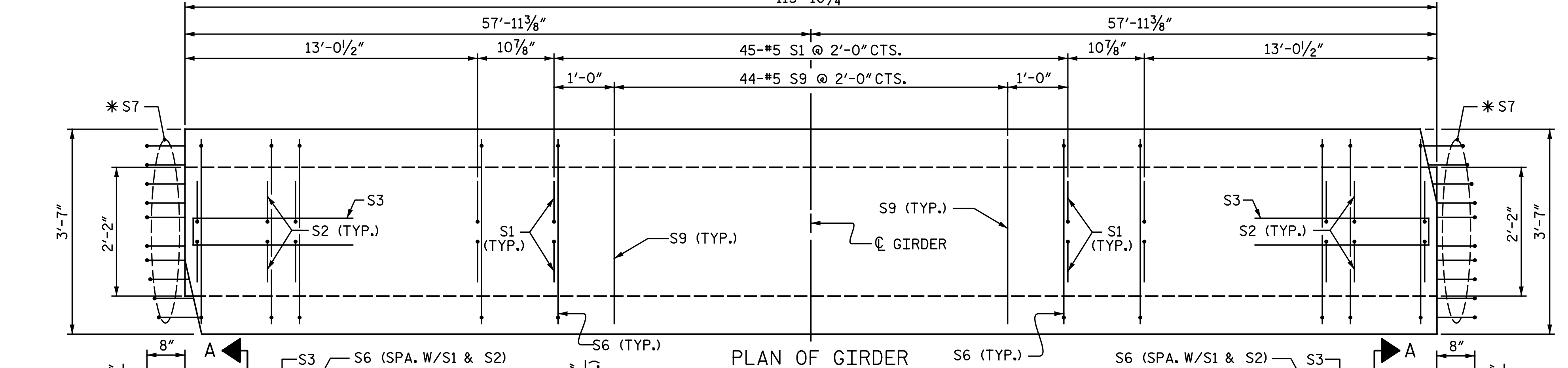
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
72" PRESTRESSED CONCRETE
MODIFIED BULB TEE

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

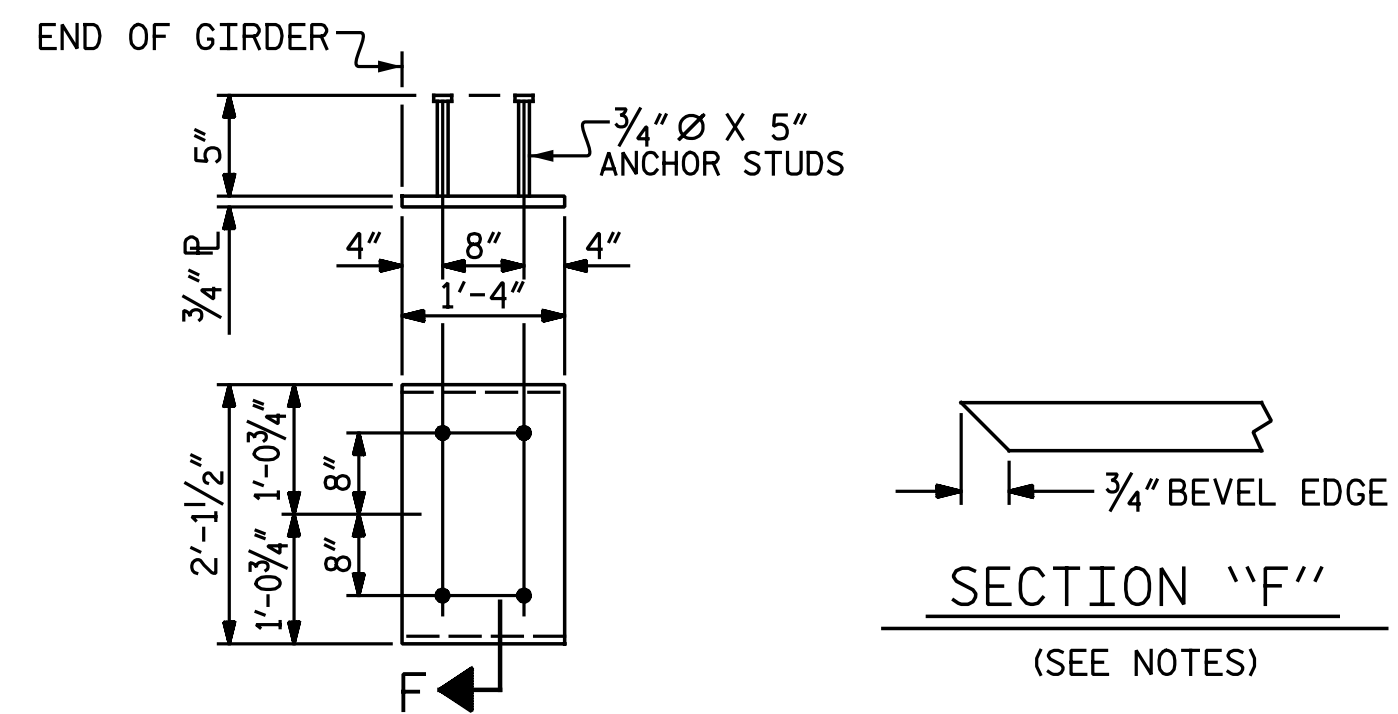
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

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SHEET NO. SI-9
TOTAL SHEETS 25
STD. NO. PCG8



ASSEMBLED BY : V. A. PATEL DATE : 3/7/2017
CHECKED BY : D. A. COLETTI DATE : 4/17/2017
DRAWN BY : EEM 2/6/97 REV. 10/1/11 MAA/GM
CHECKED BY : VAP 2/6/97 REV. 6/13 MAA/GM
REV. 1/15 MAA/TMG



EMBEDDED PLATE "B-1" DETAILS

TWO EMBEDDED PLATES "B-1" ARE REQUIRED FOR EACH GIRDER.

NOTES:

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 7,500 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

A 2" x 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 63" AND 72" MODIFIED BULB TEES ONLY.

THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD OF 4,500 LBS.

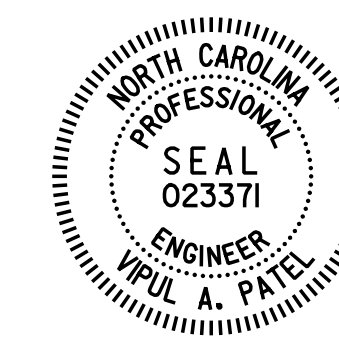
FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.

DEAD LOAD DEFLECTION TABLE FOR SPAN A

0.6 Ø LOW RELAXATION	GIRDERS 1 THRU 5																				
	0.00	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	1.00
TWENTIETH POINTS	0.00	0.043	0.084	0.123	0.159	0.191	0.217	0.239	0.255	0.264	0.267	0.264	0.255	0.239	0.217	0.191	0.159	0.123	0.084	0.043	0.000
CAMBER (GIRDER IN PLACE)	↑ 0.000	0.043	0.084	0.123	0.159	0.191	0.217	0.239	0.255	0.264	0.267	0.264	0.255	0.239	0.217	0.191	0.159	0.123	0.084	0.043	0.000
DEFLECTION DUE TO D.L. *	↓ 0.000	0.026	0.048	0.074	0.094	0.115	0.130	0.144	0.153	0.159	0.161	0.159	0.153	0.144	0.130	0.115	0.094	0.074	0.048	0.026	0.000
FINAL CAMBER	↑ 0"	3/16"	3/8"	9/16"	3/4"	7/8"	1"	1 1/8"	1 3/16"	1 1/4"	1 1/4"	1 1/4"	1 3/16"	1 1/8"	1"	7/8"	3/4"	9/16"	3/8"	3/16"	0"

* INCLUDES FUTURE WEARING SURFACE.
ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "FINAL CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM).

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-



7/26/2017

Digitally signed by Vipul A. Patel

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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

PRESTRESSED
CONCRETE GIRDER
DETAILS

LEFT LANE

DRAWN BY : C. E. MAYHEW DATE : 5-9-17
CHECKED BY : V. A. PATEL DATE : 5-22-17

NO.	BY:	DATE:	REVISIONS			SHEET NO.
			NO.	BY:	DATE:	
1			3			TOTAL SHEETS
2			4			25

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Cary, North Carolina 27518
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STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

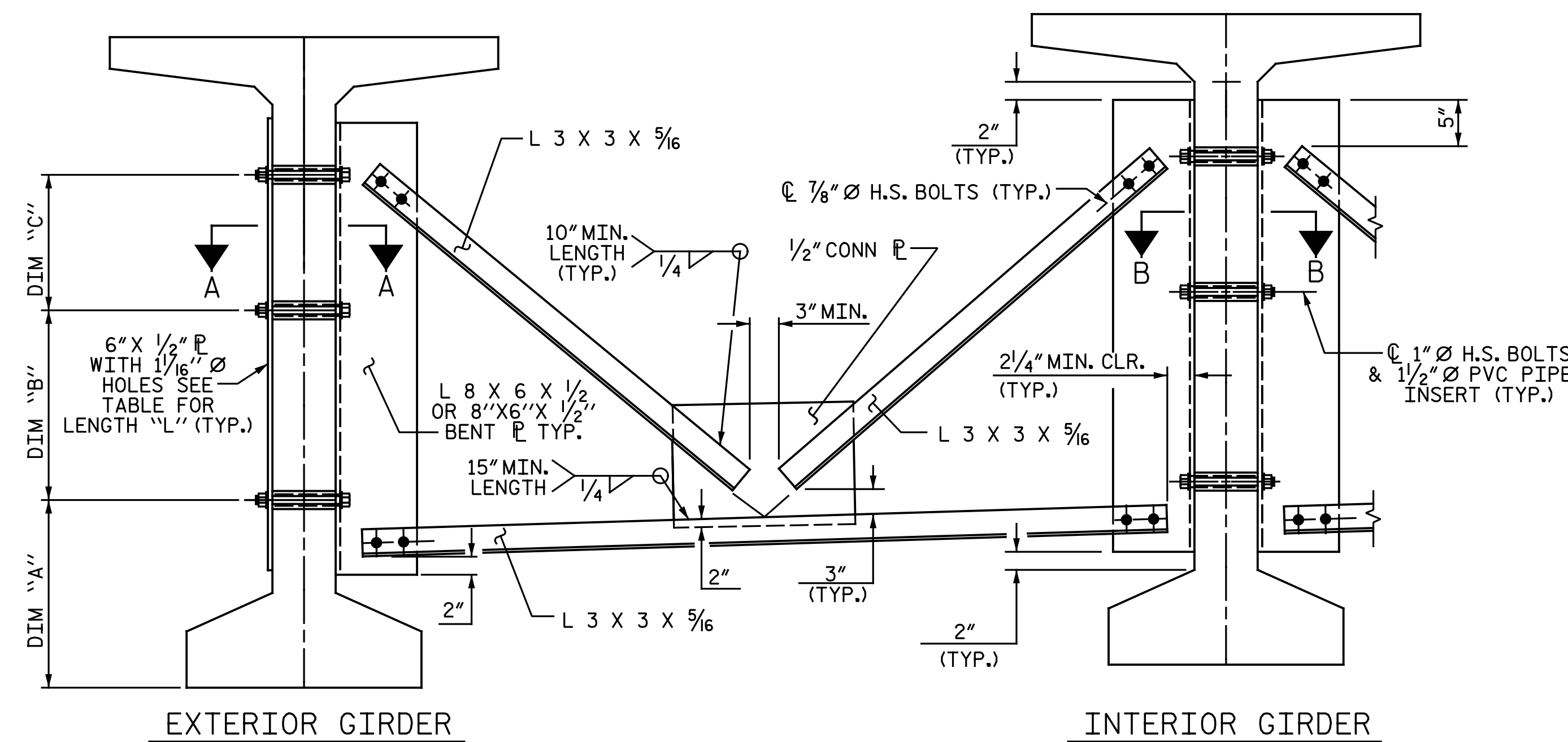
FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

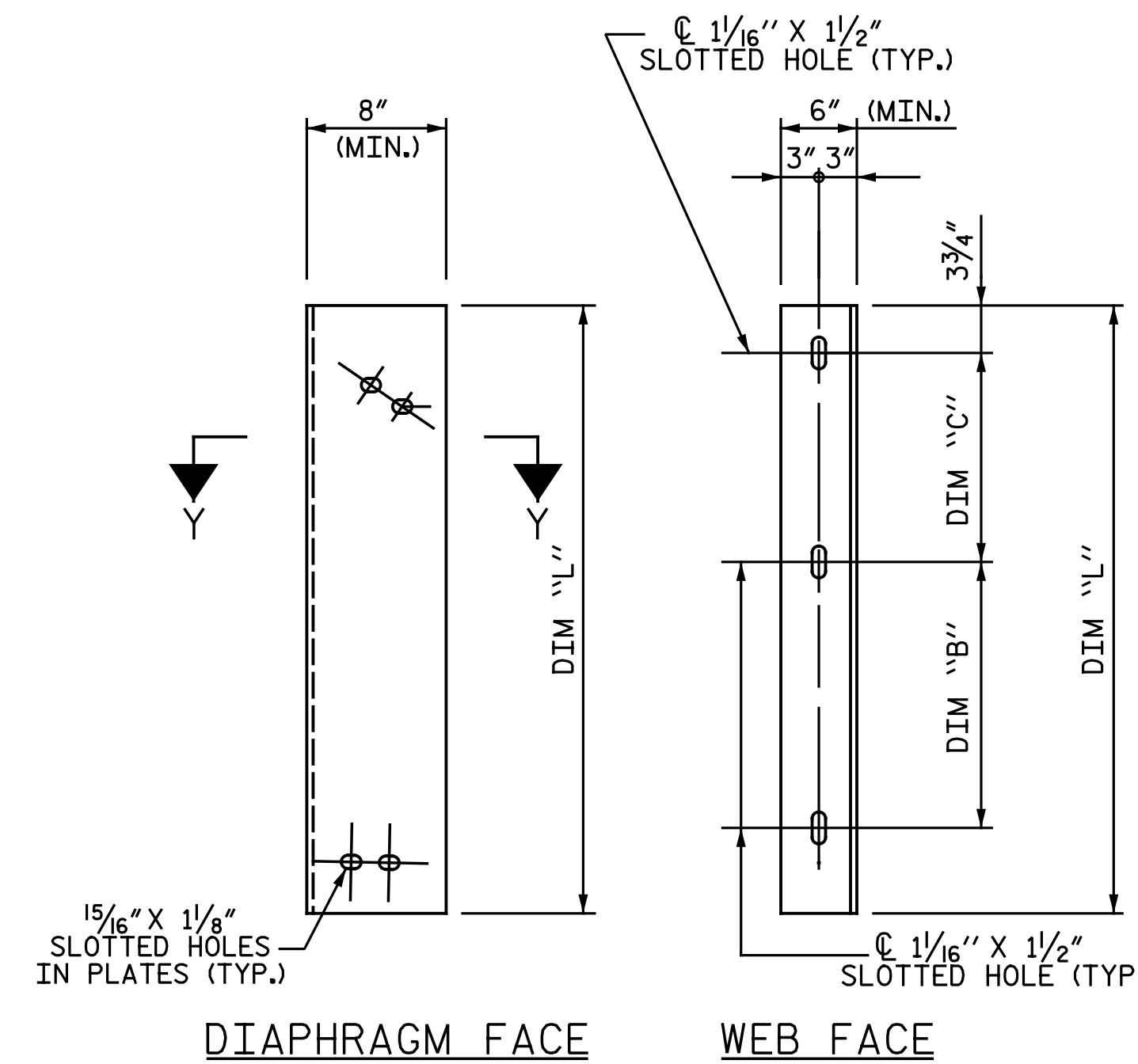
SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

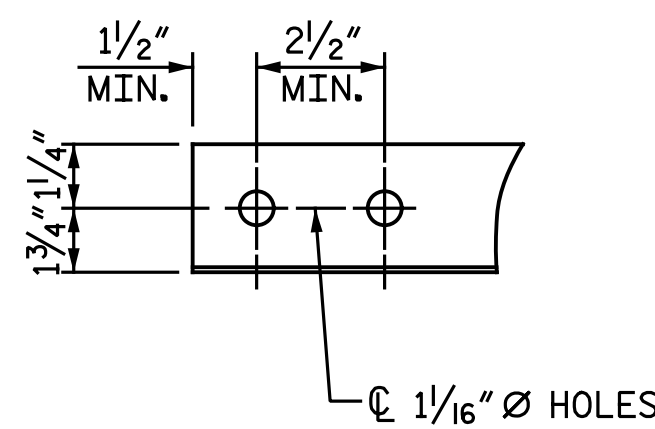
THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.



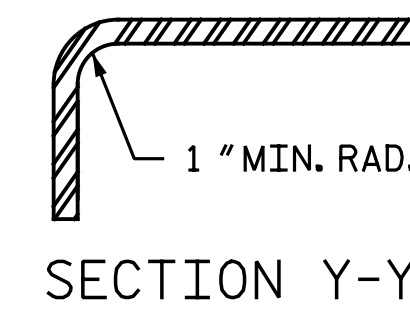
PART SECTION AT INTERMEDIATE DIAPHRAGM



DIAPHRAGM FACE WEB FACE



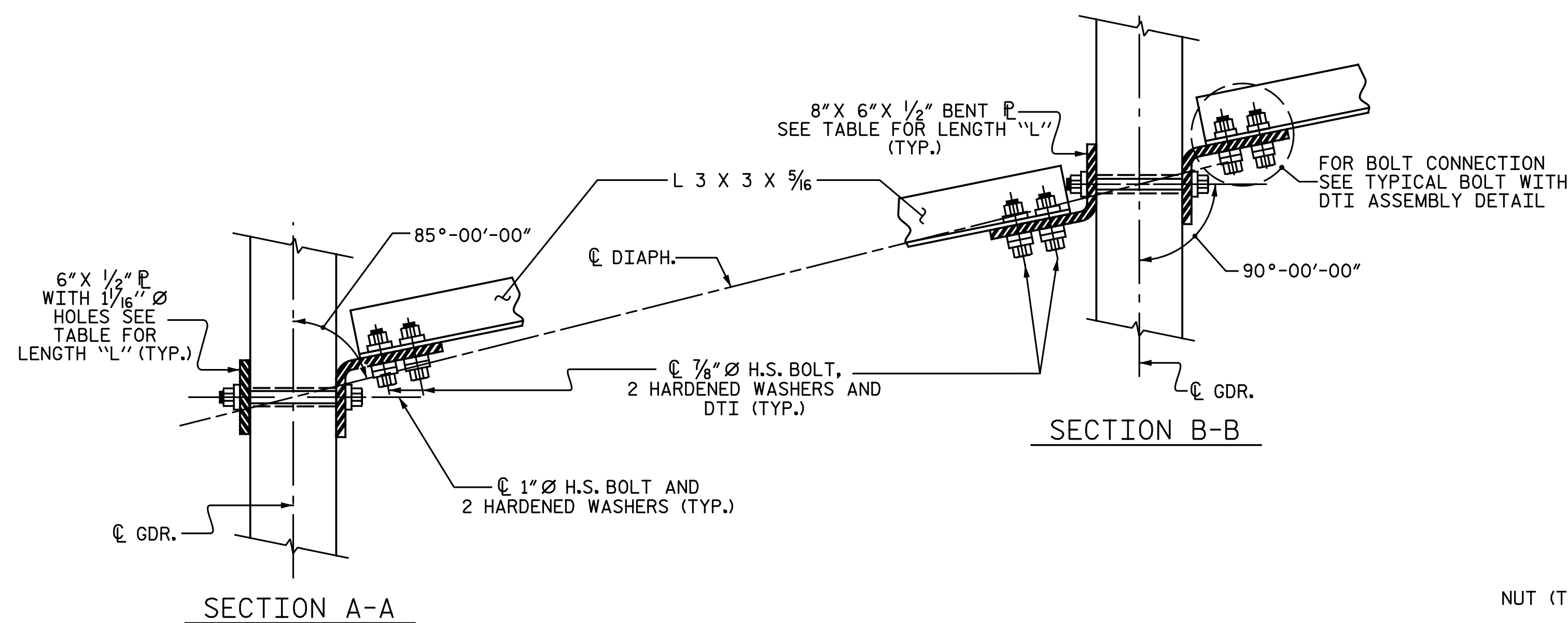
ANGLE END (L 3 x 3 x 5/16)



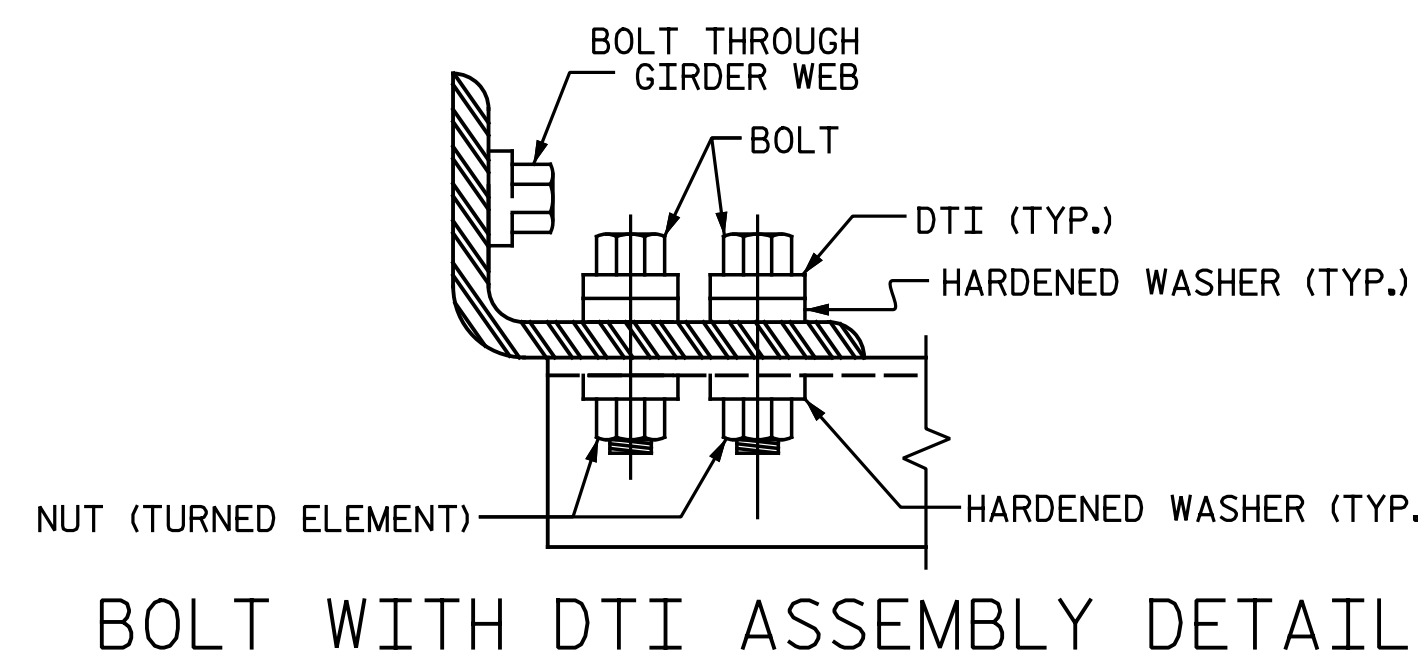
CONNECTOR PLATE DETAIL

TABLE

GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"	DIM "L"
72" BULB TEE	1'-8 3/4"	1'-9"	1'-5"	4'-2"

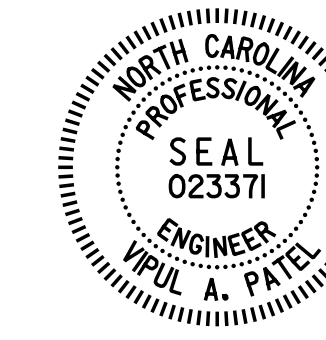


CONNECTION DETAILS



BOLT WITH DTI ASSEMBLY DETAIL

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



7/26/2017
 Digitally signed by Vipul A. Patel

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Michael Baker INTERNATIONAL
 Michael Baker Engineering
 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No.: F-1084

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR 72" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	SI-II
1			3			TOTAL SHEETS
2			4			25

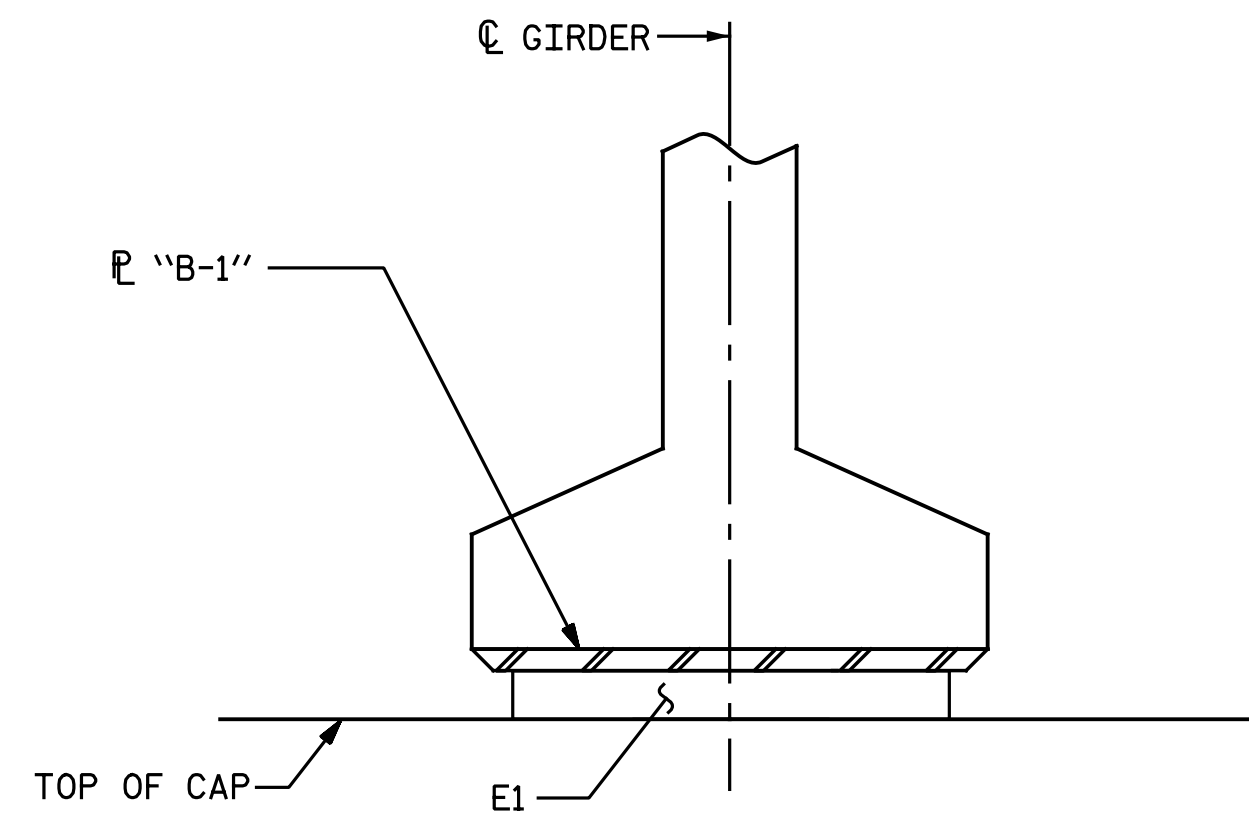
ASSEMBLED BY : M. D. MAYHEW DATE : 1-17-17
 CHECKED BY : V. A. PATEL DATE : 3-13-17
 DRAWN BY : RWW 11/09
 CHECKED BY : GM 11/09
 ADDED 11/23/09R
 REV. 10/11/11 MAA/GM

NOTES

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

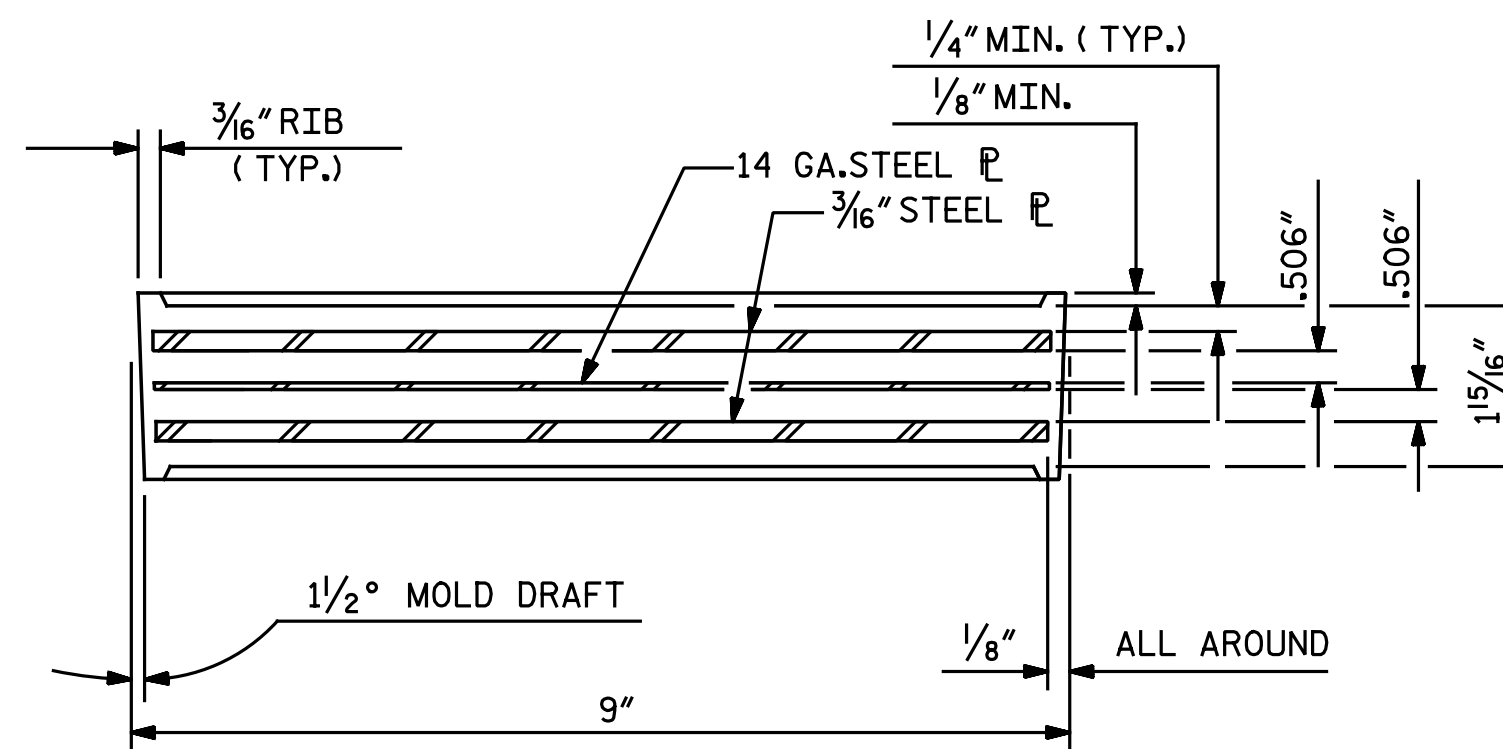
THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

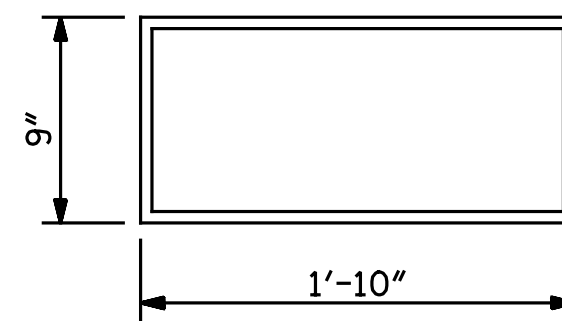


SECTION
(AT INTEGRAL END BENTS)

MAXIMUM ALLOWABLE SERVICE LOADS	
D.L.+L.L. (NO IMPACT)	
TYPE IV	225 k



TYPICAL SECTION OF ELASTOMERIC BEARING



E1 (10 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

TYPE IV

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



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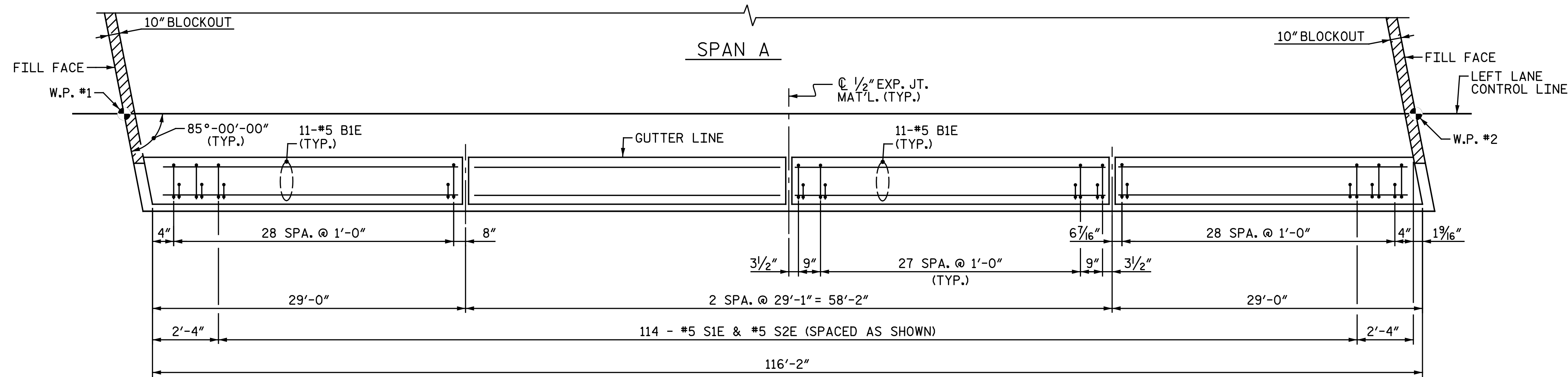
DocuSigned by:
Vipul A. Patel

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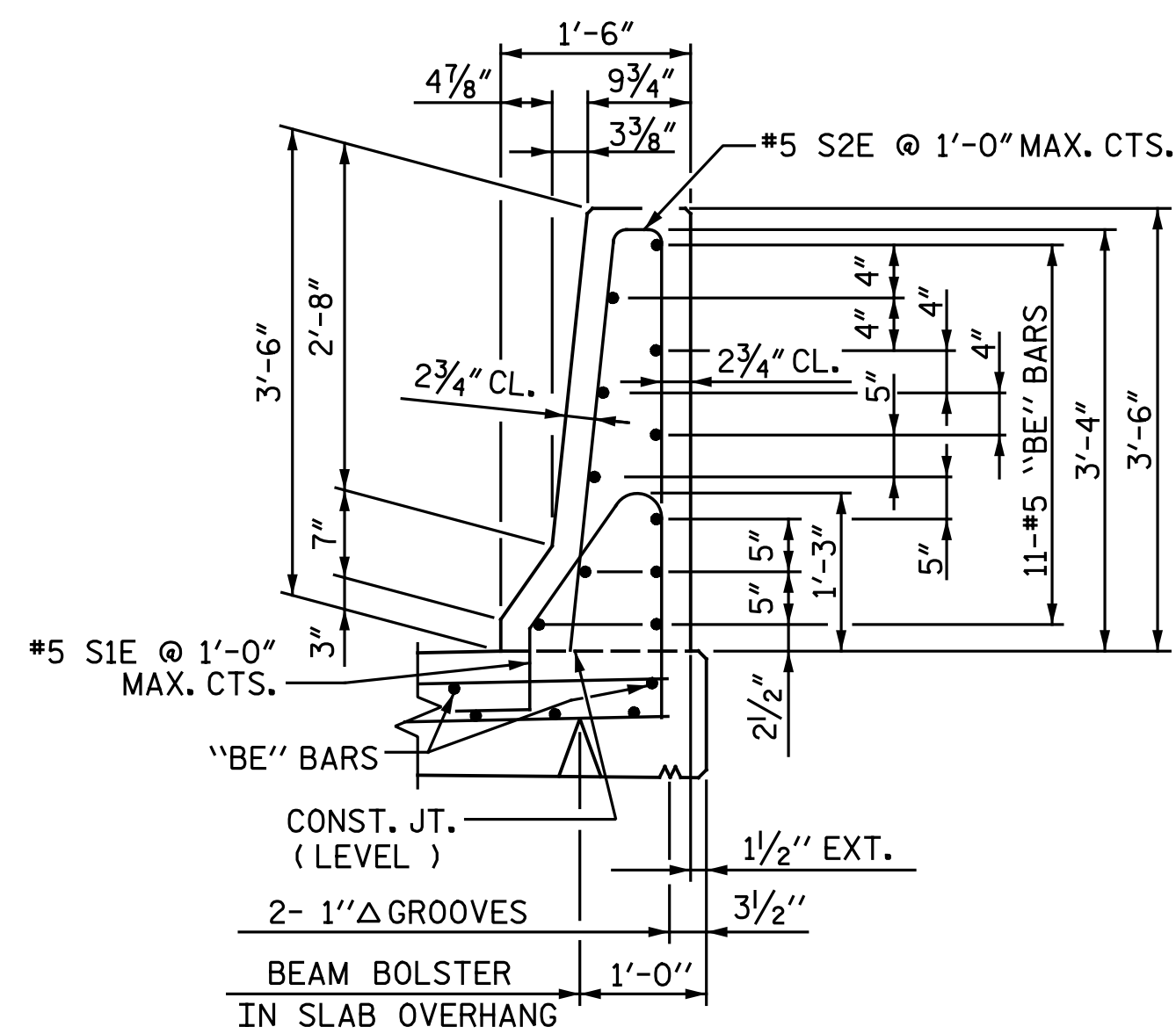
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 ELASTOMERIC BEARING
 DETAILS
 PRESTRESSED CONCRETE GIRDER
 SUPERSTRUCTURE
 LEFT LANE

ASSEMBLED BY : N.B.SPEAKS	DATE : 1-9-17
CHECKED BY : V. A. PATEL	DATE : 5-23-17
DRAWN BY : WJH 8/89	REV. 10/1/11 MAA/GM
CHECKED BY : CRK 8/89	REV. 6/13 AAC/MAA
	REV. 1/15 MAA/TMG

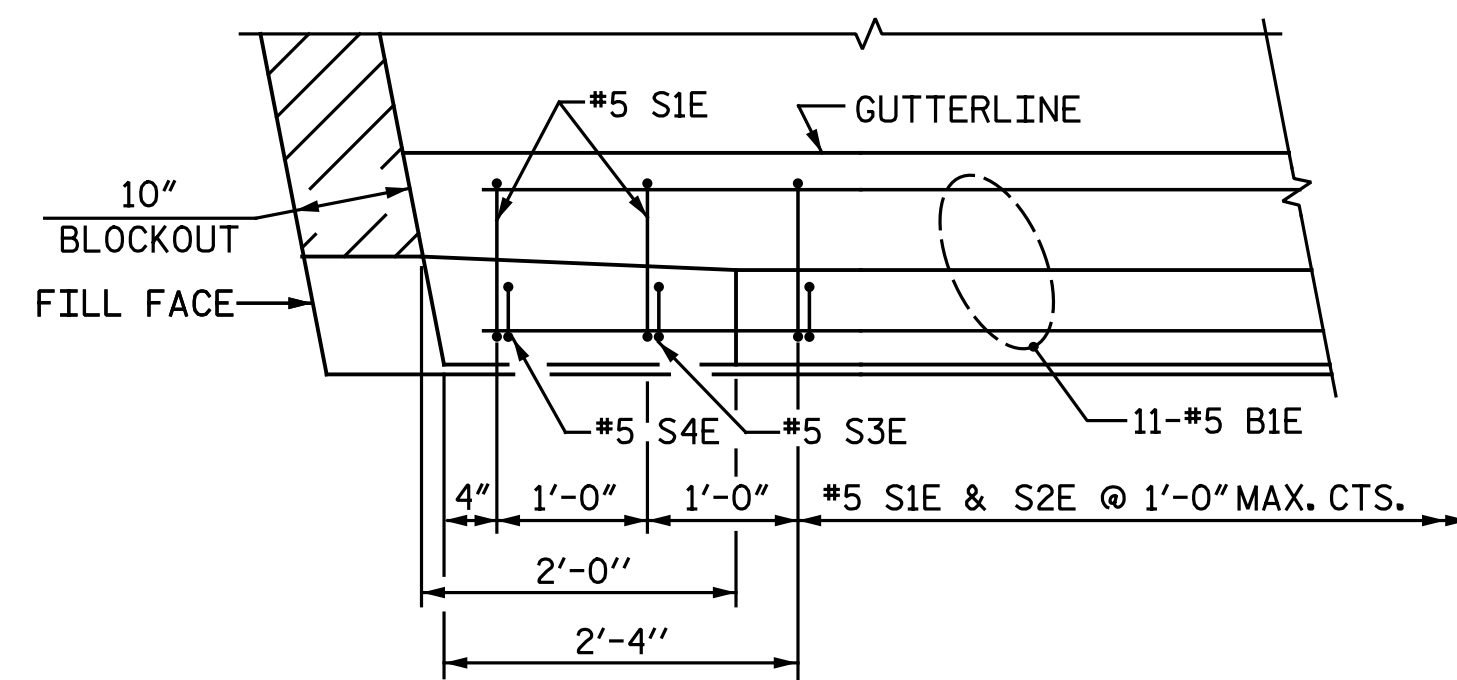
Michael Baker INTERNATIONAL Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 NC License No. : F-1084	REVISIONS				SHEET NO.		
	NO.	BY:	DATE:	NO.	BY:	DATE:	SI-12
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	2			4			25



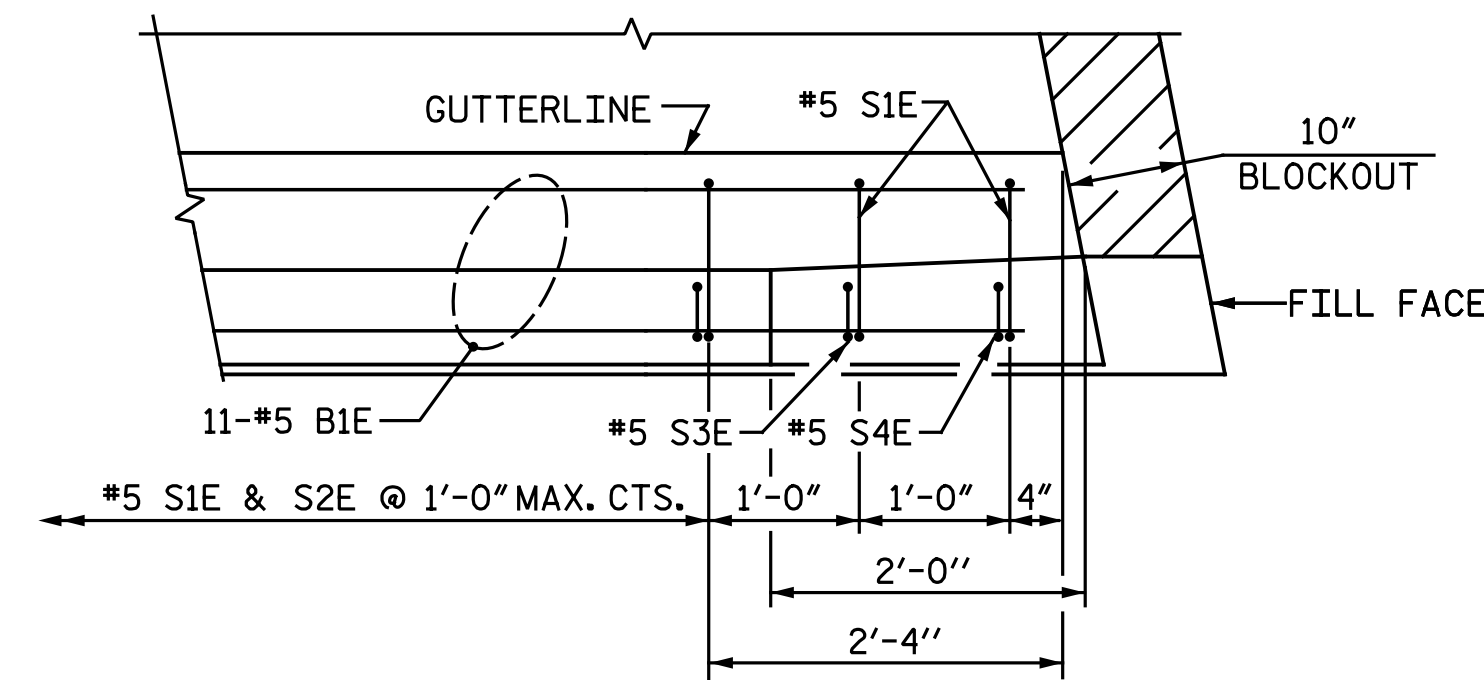
PLAN OF BARRIER RAIL
(RIGHT RAIL SHOWN, LEFT RAIL SIMILAR BY ROTATION)



SECTION THRU RAIL

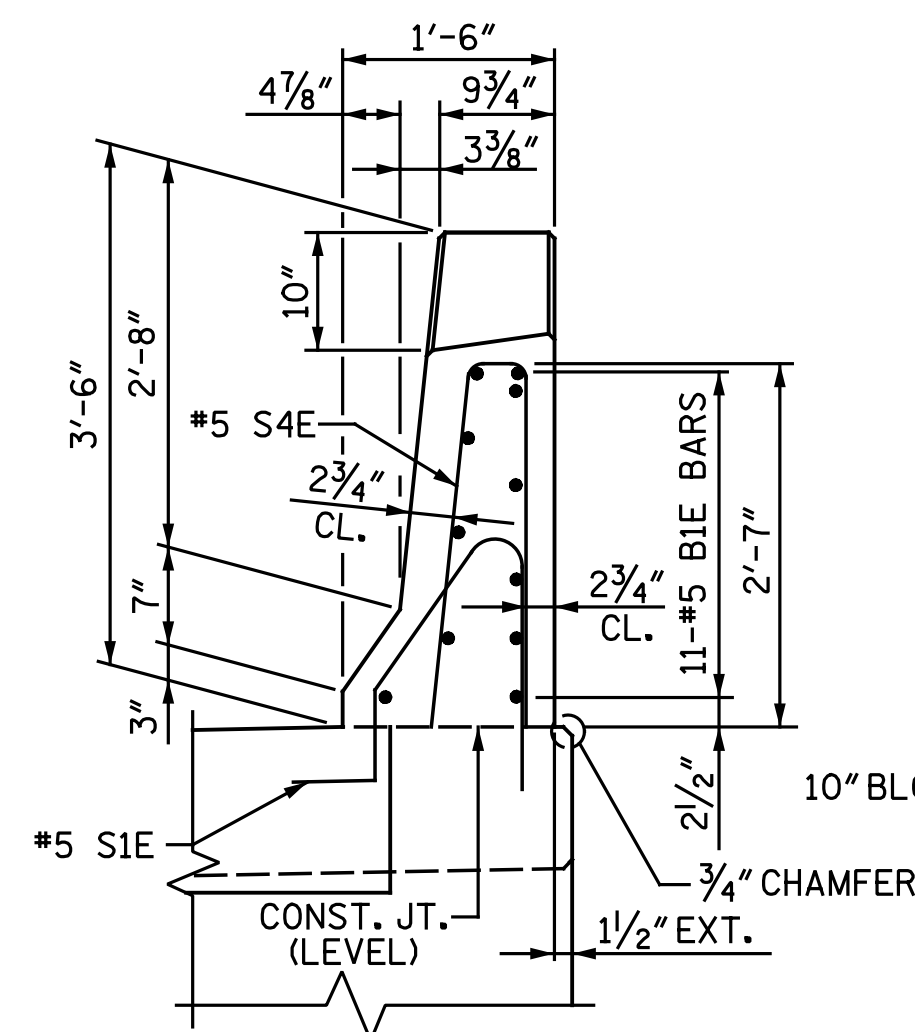


END BENT 1

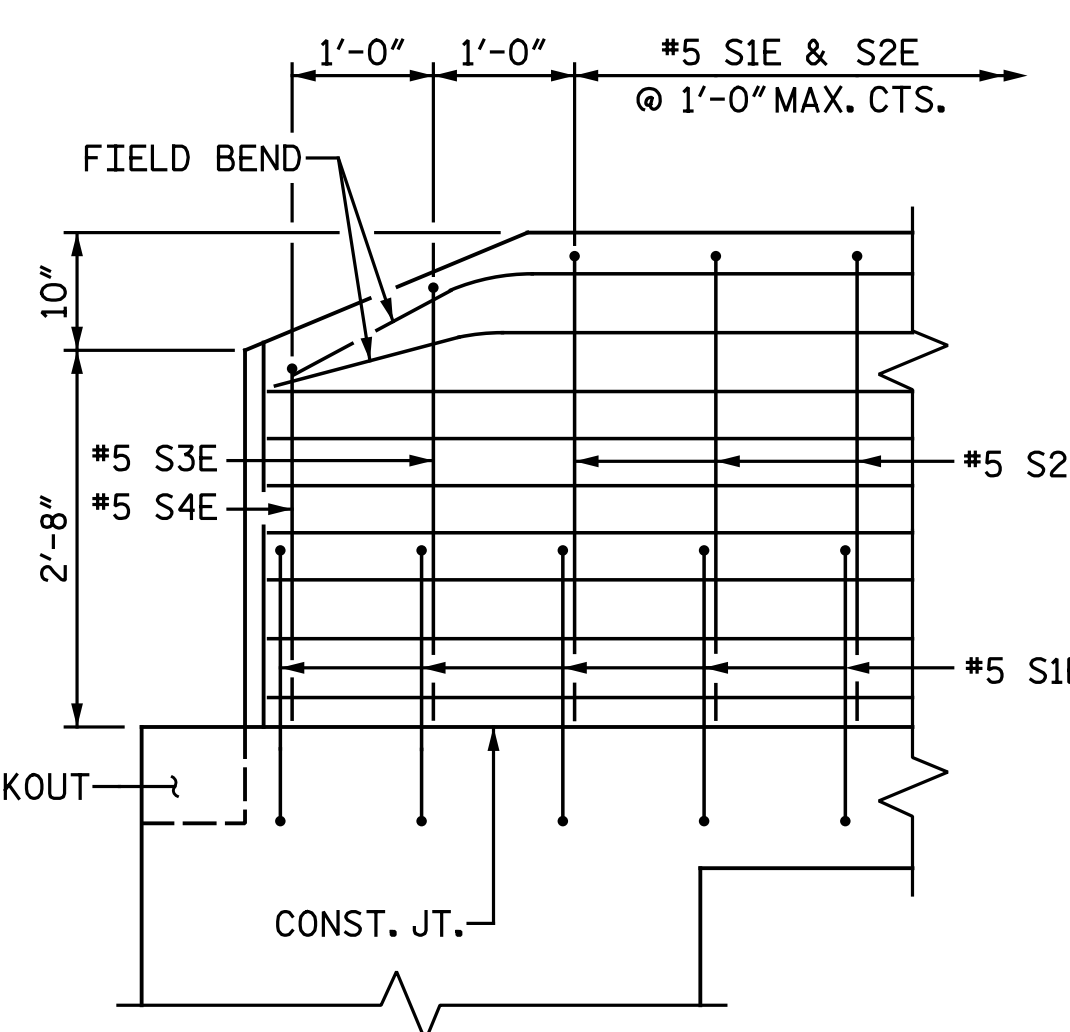


END BENT 2

PLAN
(RIGHT RAIL SHOWN, LEFT RAIL SIMILAR)

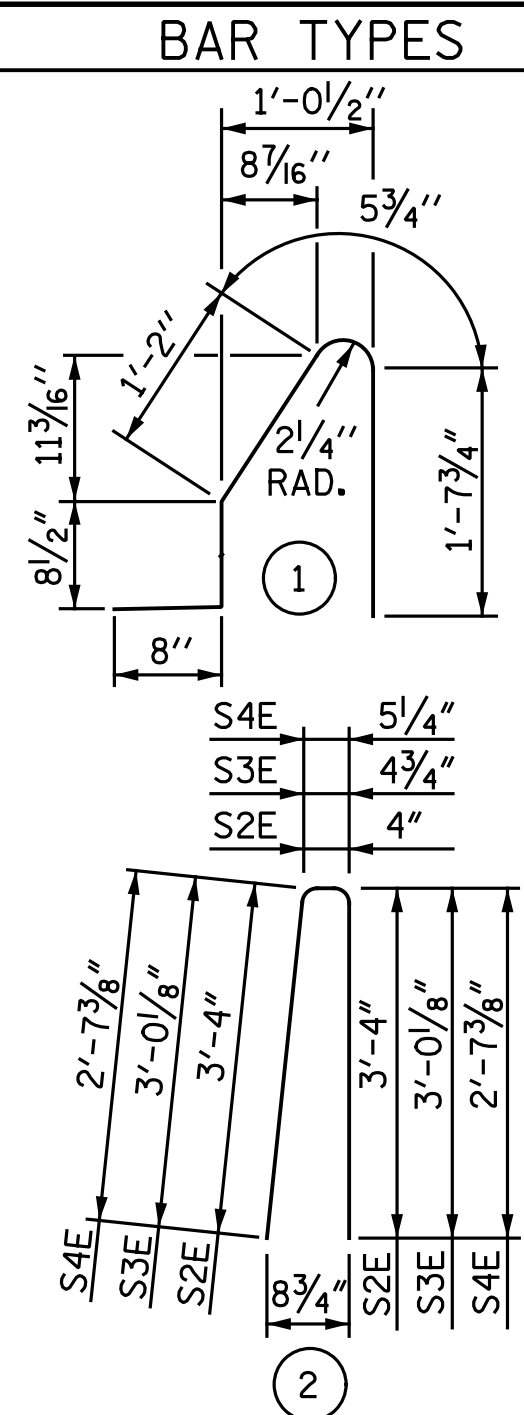


END VIEW



SIDE VIEW

END OF RAIL DETAILS



ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL					
FOR CONCRETE BARRIER RAIL ONLY					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1E	88	#5	STR.	28' - 8"	2,631
S1E	236	#5	1	4' - 8"	1,149
S2E	228	#5	2	7' - 0"	1,665
S3E	4	#5	2	6' - 5"	27
S4E	4	#5	2	5' - 8"	24
EPOXY COATED REINFORCING STEEL				LBS.	5,496
CLASS AA CONCRETE				C.Y.	31.6
CONCRETE BARRIER RAIL				L.F.	232.33

NOTES

THE BARRIER RAIL IN THE SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 CONCRETE
 BARRIER RAIL

LEFT LANE

9/12/2017
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NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

ASSEMBLED BY : N. B. SPEAKS	DATE : 9-11-17
CHECKED BY : V. A. PATEL	DATE : 9-12-17
DRAWN BY : ARB 5/87	REV. 10/1/11 MAA/GM
CHECKED BY : SJD 9/87	REV. 7/12 MAA/GM
	REV. 6/13 MAA/GM

BARRIER RAIL DETAILS

ELEVATION AT EXPANSION JOINTS

SECTION S-S
 AT DAM IN OPEN JOINT
 (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

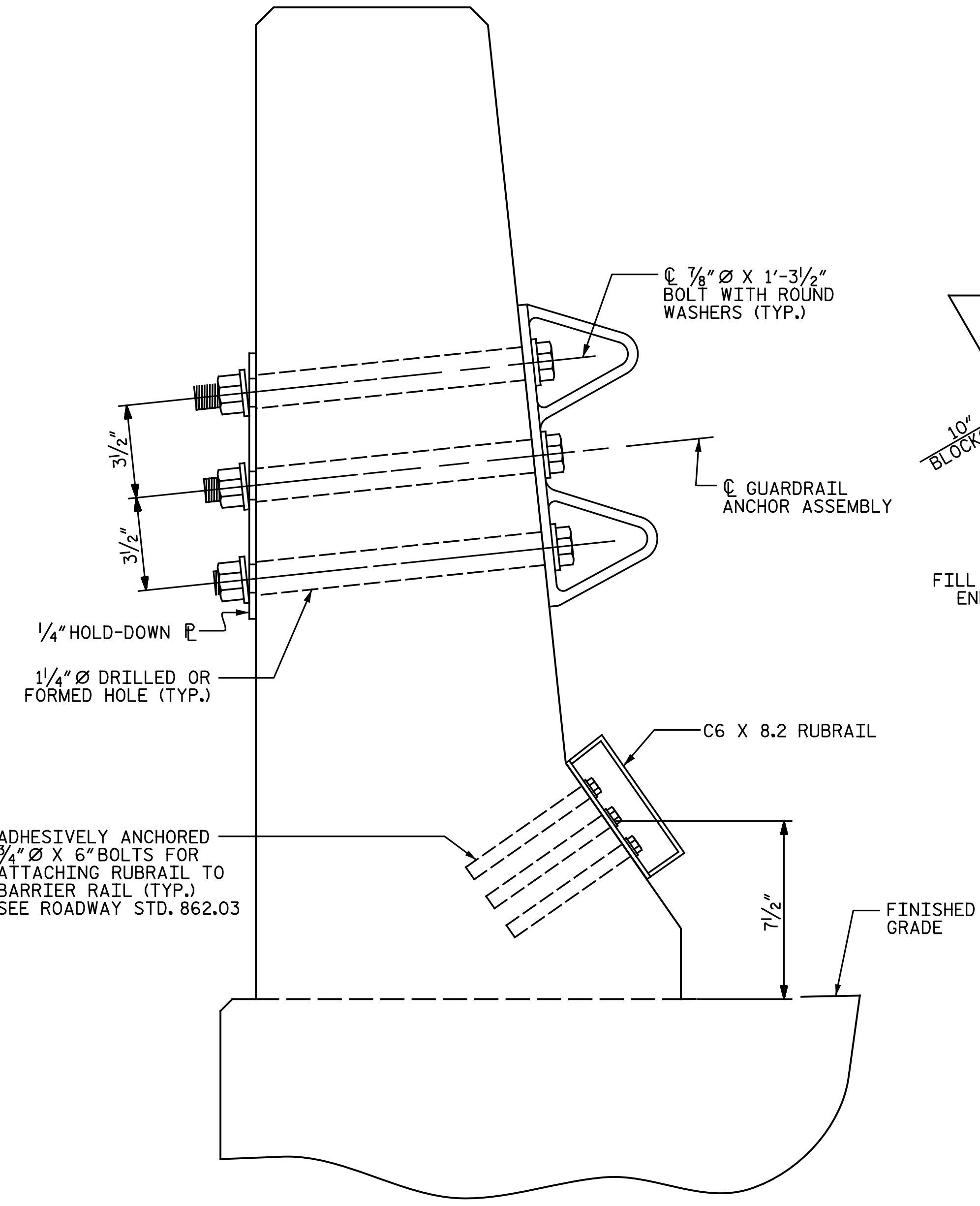
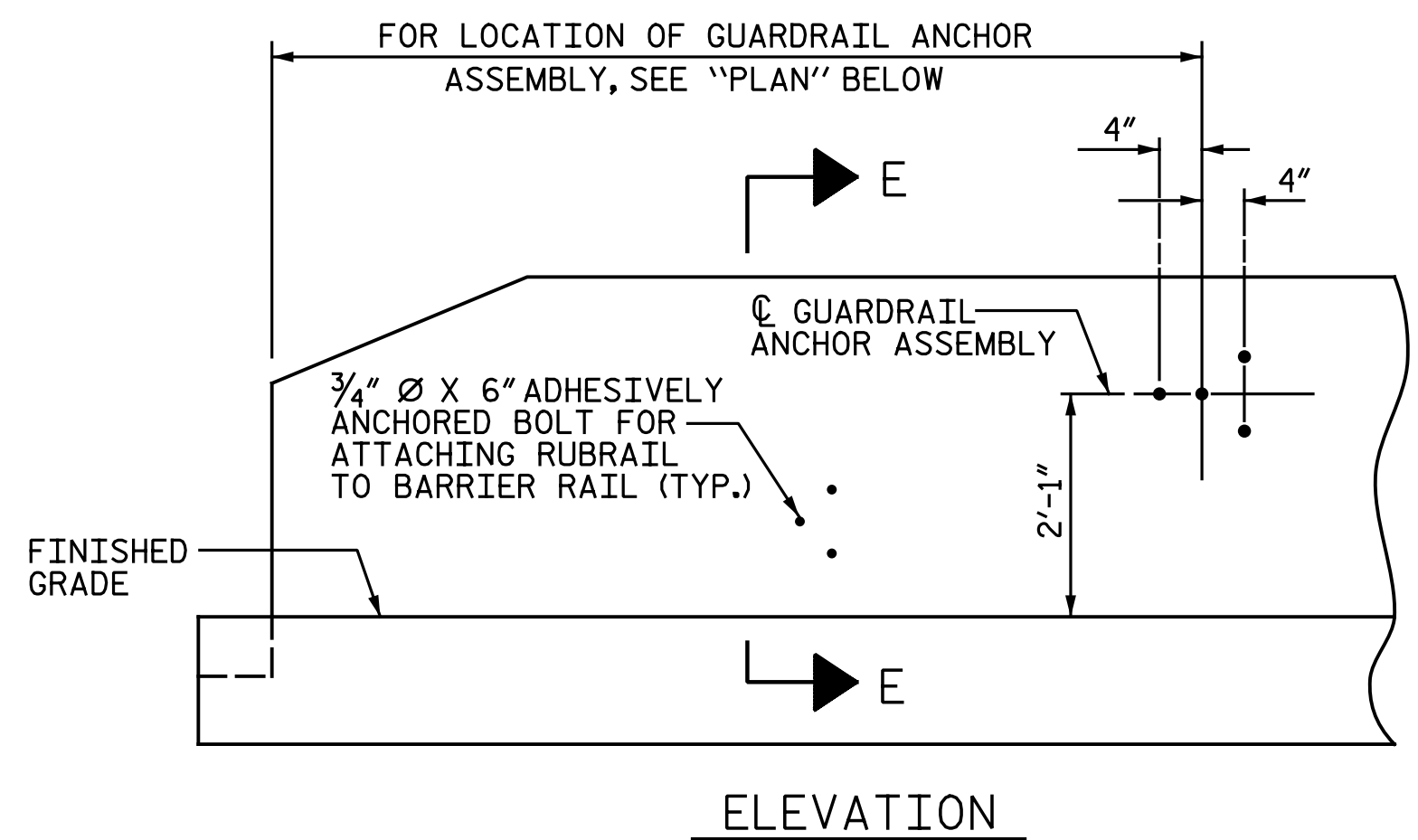
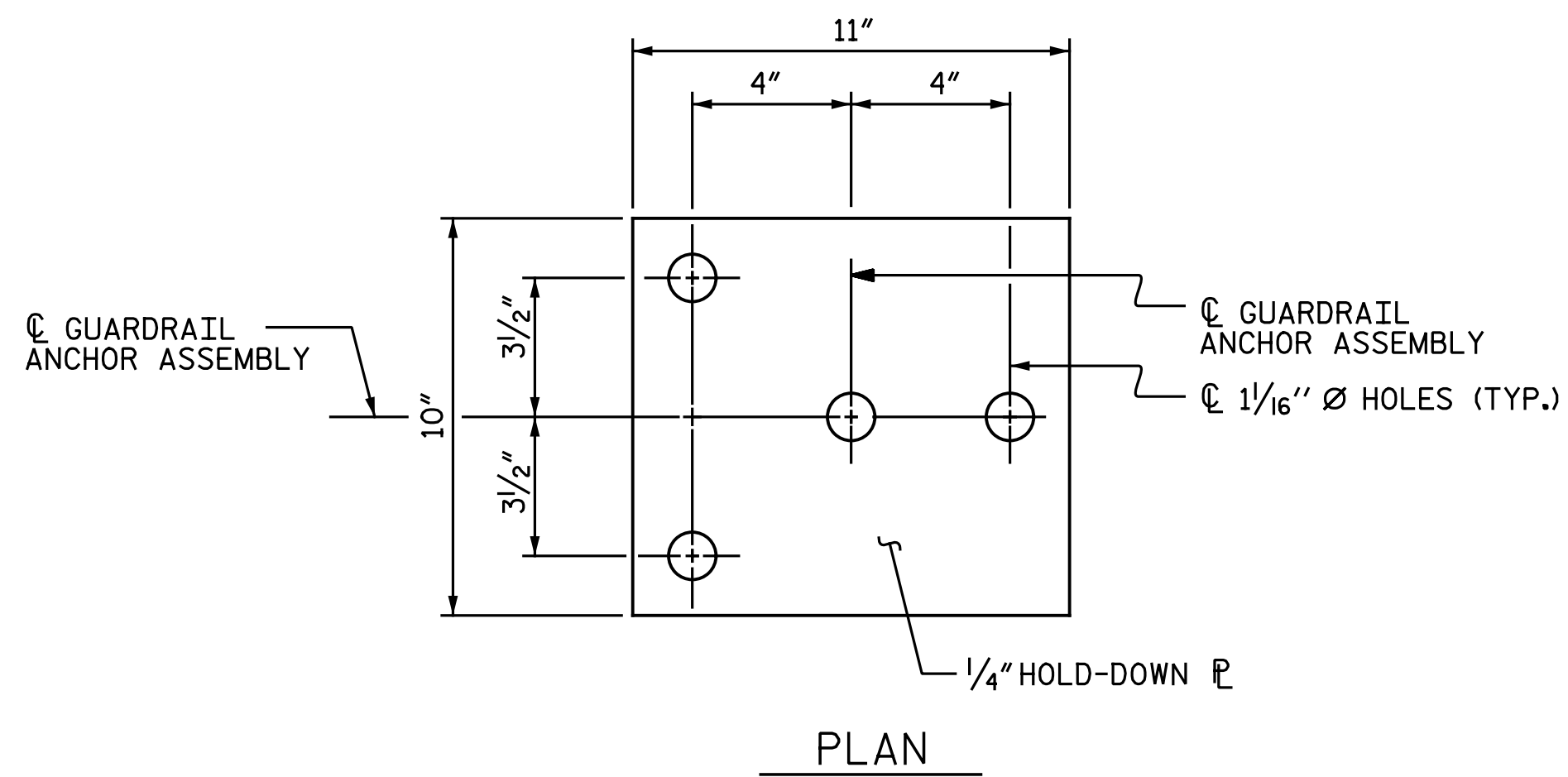
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

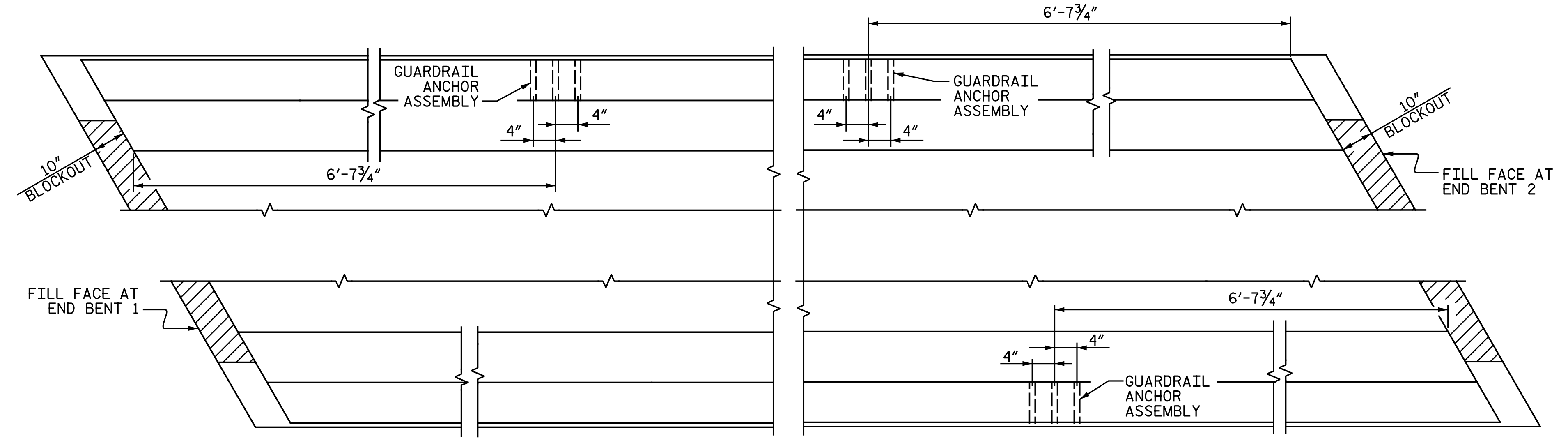
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

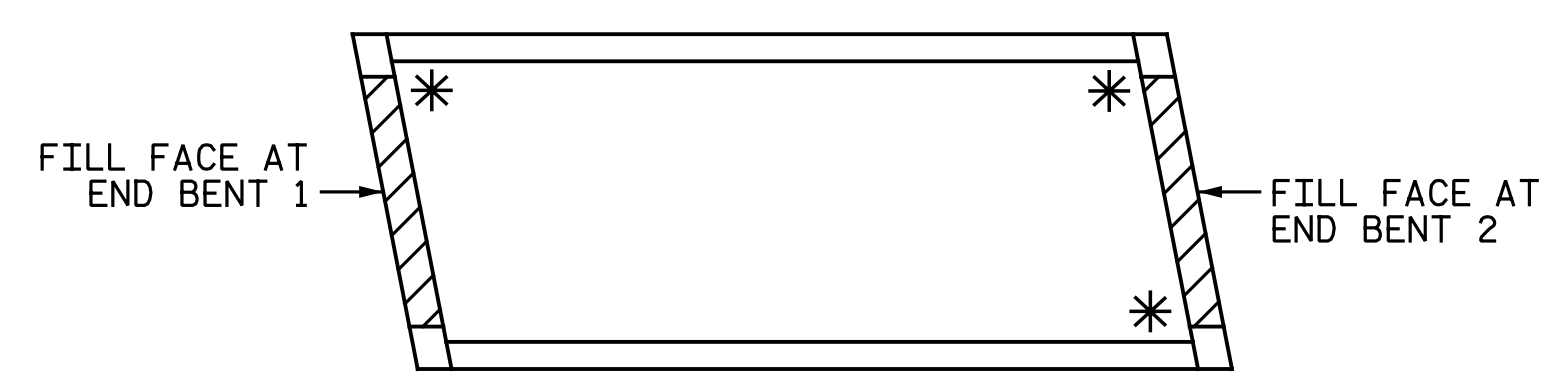
THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



SECTION E-E
GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF ANCHORS FOR GUARDRAIL



* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. R-5703
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STATION: 89+28.52 -L-



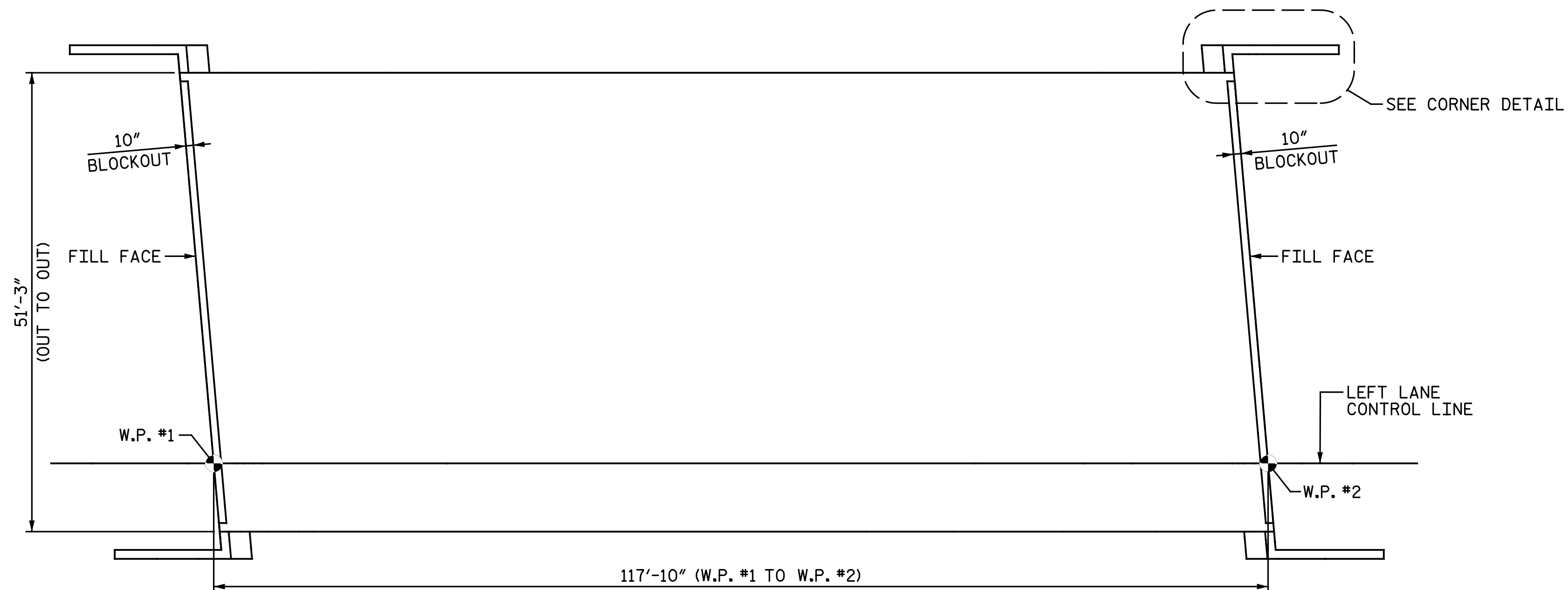
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
GUARDRAIL ANCHORAGE
FOR BARRIER RAIL
LEFT LANE

ASSEMBLED BY : N. B. SPEAKS	DATE : 9-11-17
CHECKED BY : V. A. PATEL	DATE : 9-12-17
DRAWN BY : TLA 5/06	REV. 10/1/11 MAA/GM
CHECKED BY : GM 5/06	REV. 7/12 MAA/GM
	REV. 6/13 MAA/GM

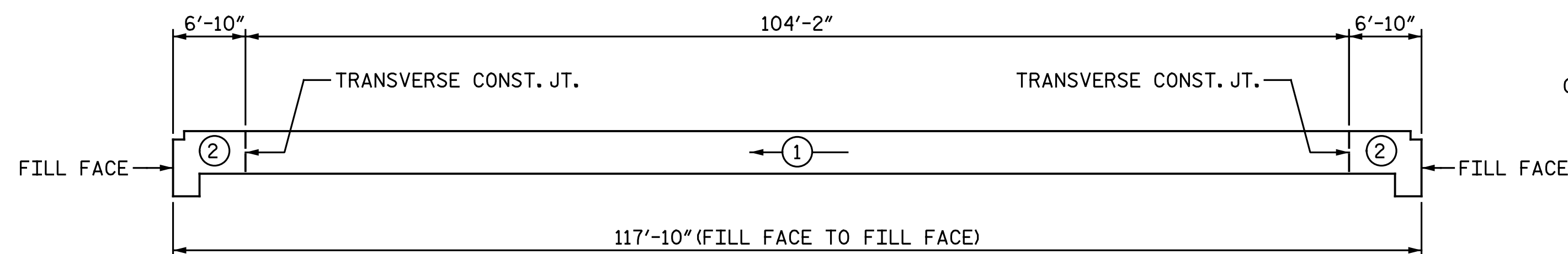
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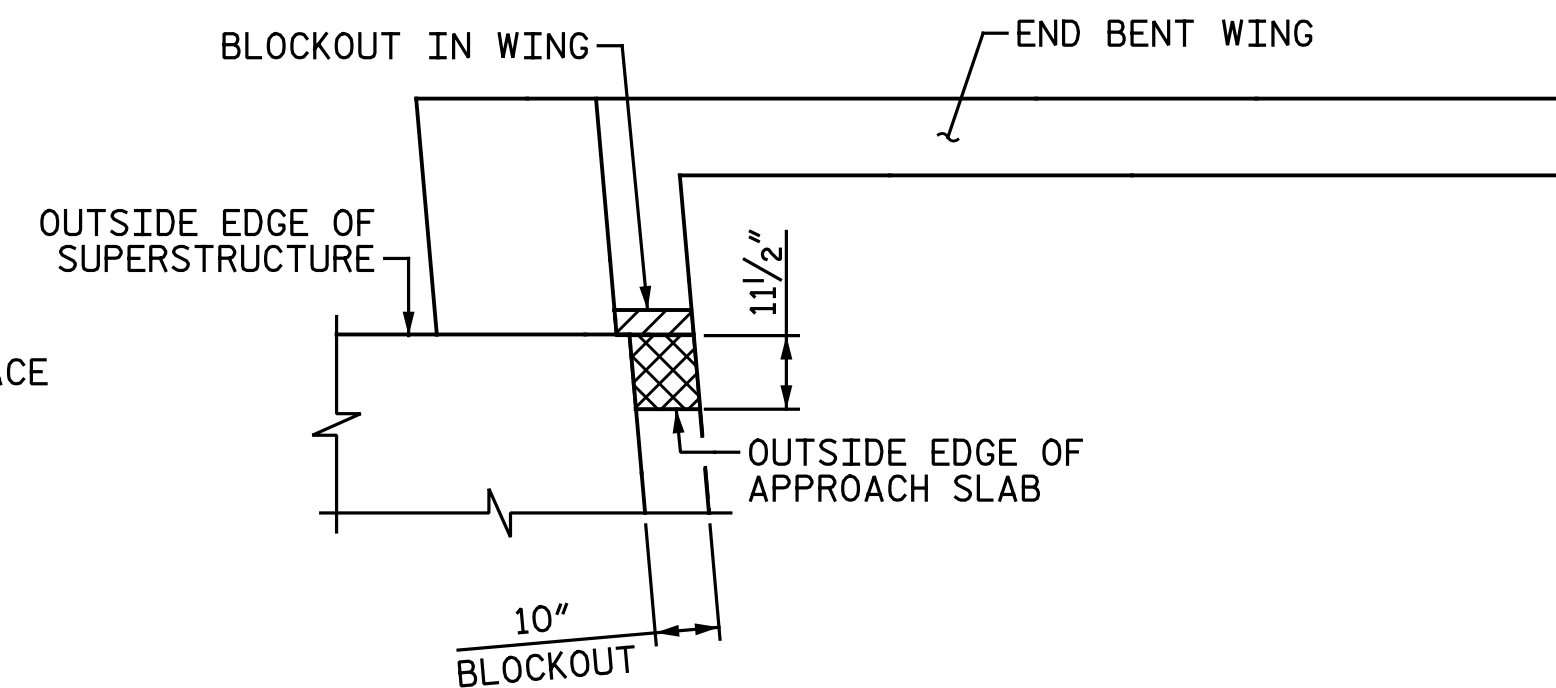
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NO.	BY:	DATE:	NO.	BY:	DATE:	SI-14
1			3			TOTAL SHEETS
2			4			25



LAYOUT FOR COMPUTING AREA OF REINFORCED CONCRETE DECK SLAB
(SQ. FT. = 6,039)

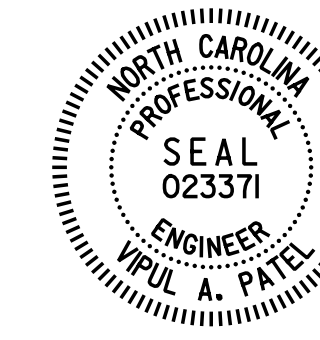


POURING SEQUENCE
⊕ DENOTES POUR NUMBER AND DIRECTION



CORNER DETAIL

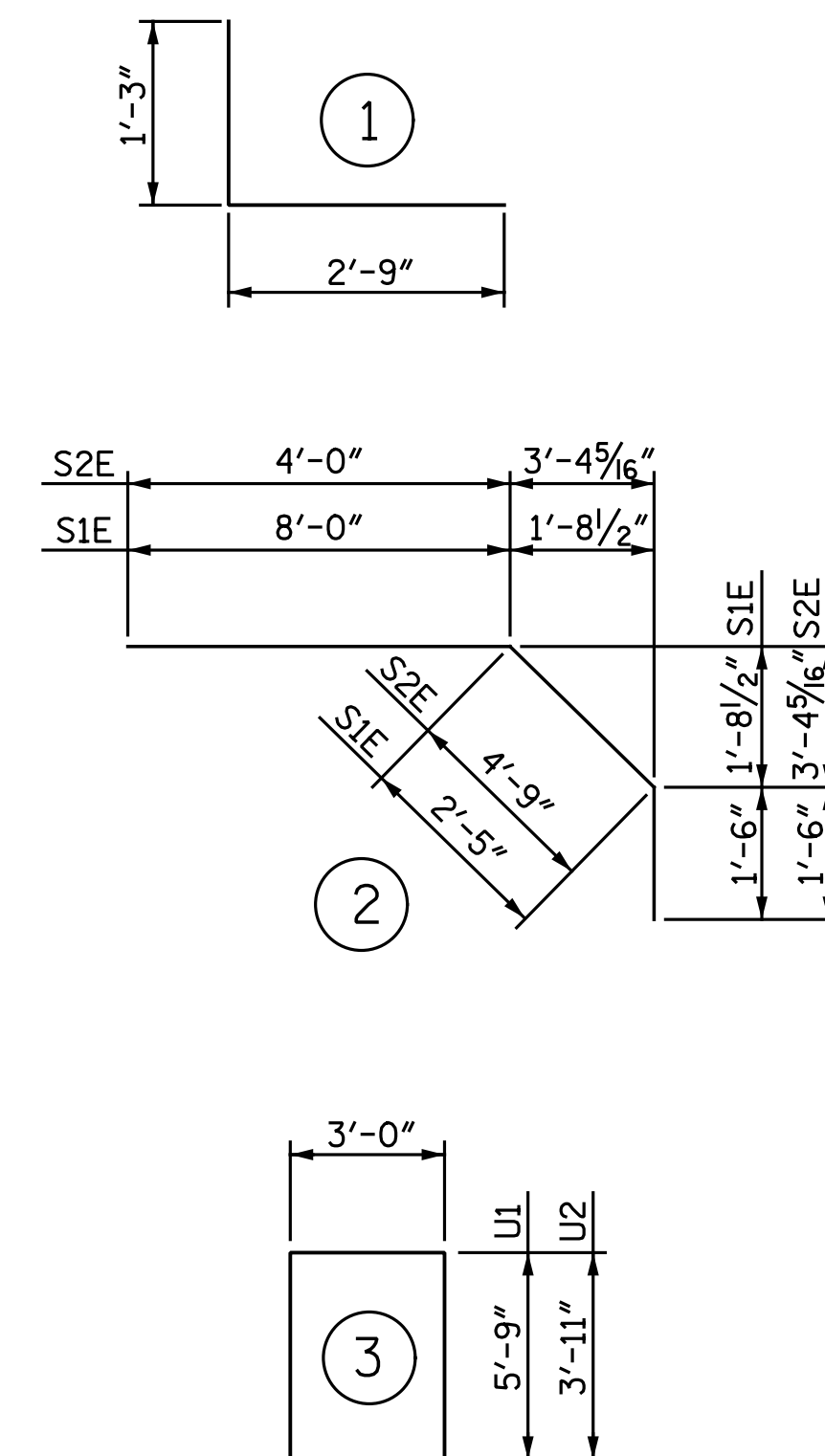
CONCRETE SHALL BE POURED IN THE CROSS-HATCHED AREA TO MATCH THE TOP OF END BENT WING ELEVATIONS. UNLESS OTHERWISE DIRECTED BY THE ENGINEER, THE CONCRETE IN THESE AREAS SHALL BE PLACED AT THE SAME TIME THE BLOCKOUTS IN THE END BENT WINGS ARE POURED AS NOTED ON SHEET 1 OF "INTEGRAL END BENT 1" AND SHEET 1 OF "INTEGRAL END BENT 2" SHEETS.



7/26/2017

DocuSigned by:
Vipul A. Patel

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT

SUPERSTRUCTURE BILL OF MATERIAL

	CLASS AA CONCRETE (CU. YDS.)	REINFORCING STEEL (LBS.)	EPOXY COATED REINFORCING STEEL (LBS.)
SPAN POUR 1	179.2	19,760	20,371
SPAN POUR 2	94.2		
TOTALS*	273.4	19,760	20,371

* QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

REINFORCING BAR SCHEDULE

SPAN A					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1E	196	#5	STR.	50' - 11"	10,409
A2	192	#5	STR.	50' - 11"	10,196
A101E	2	#5	STR.	43' - 10"	91
A102E	2	#5	STR.	37' - 2"	78
A103E	2	#5	STR.	30' - 6"	64
A104E	2	#5	STR.	23' - 10"	50
A105E	2	#5	STR.	17' - 2"	36
A106E	2	#5	STR.	10' - 6"	22
A107E	2	#5	STR.	3' - 10"	8
A201	2	#5	STR.	43' - 10"	91
A202	2	#5	STR.	37' - 2"	78
A203	2	#5	STR.	30' - 6"	64
A204	2	#5	STR.	23' - 10"	50
A205	2	#5	STR.	17' - 2"	36
A206	2	#5	STR.	10' - 6"	22
A207	2	#5	STR.	3' - 10"	8
B1E	175	#4	STR.	24' - 10"	2,903
B2	116	#5	STR.	59' - 0"	7,138
B3E	68	#6	STR.	25' - 5"	2,596
B4E	68	#6	STR.	27' - 7"	2,817
B5E	8	#4	1	4' - 0"	21
H1	28	#5	STR.	3' - 0"	88
K1	28	#4	STR.	26' - 10"	502
K2	8	#4	STR.	6' - 7"	35
K3	40	#4	STR.	9' - 7"	256
K4	8	#4	STR.	8' - 0"	43
K5	4	#4	STR.	2' - 6"	7
K6	20	#4	STR.	4' - 0"	53
K7	4	#4	STR.	3' - 2"	8
S1E	88	#4	2	11' - 11"	701
S2E	84	#4	2	10' - 3"	575
U1	88	#4	3	14' - 6"	852
U2	20	#4	3	10' - 10"	145
V1	12	#5	STR.	7' - 0"	88

	REINFORCING STEEL	LBS.
TOTALS*	19,760	19,760
	EPOXY COATED REINF. STEEL	LBS.
	20,371	20,371

"E" SUFFIX DENOTES EPOXY COATED REINFORCING STEEL

GROOVING BRIDGE FLOORS

APPROACH SLABS	2,175	SQ.FT.
BRIDGE DECK	5,212	SQ.FT.
TOTAL	7,387	SQ.FT.

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
BILL OF MATERIAL

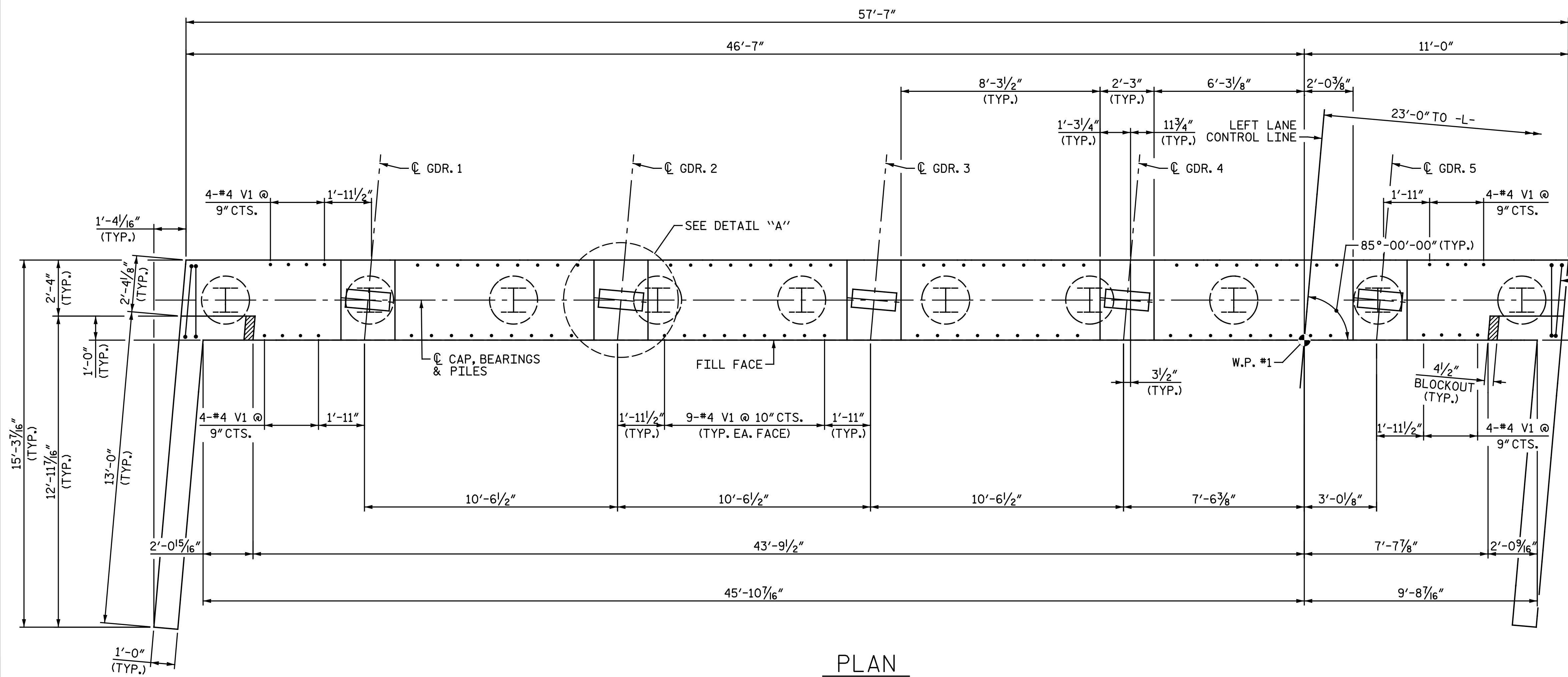
LEFT LANE

REVISIONS

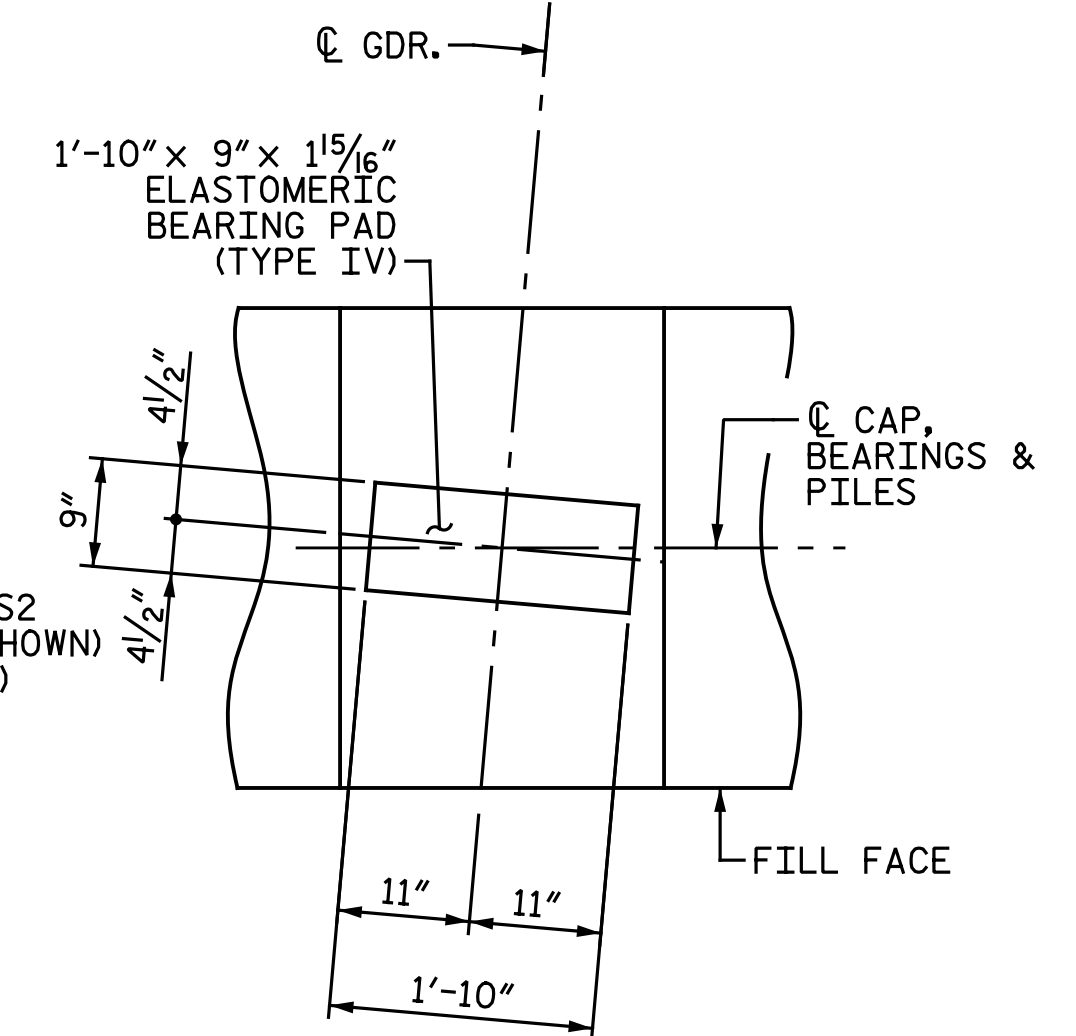
NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			SI-15
2			4			TOTAL SHEETS 25

DRAWN BY : M. D. MAYHEW DATE : 2-3-17
CHECKED BY : V. A. PATEL DATE : 3-13-17

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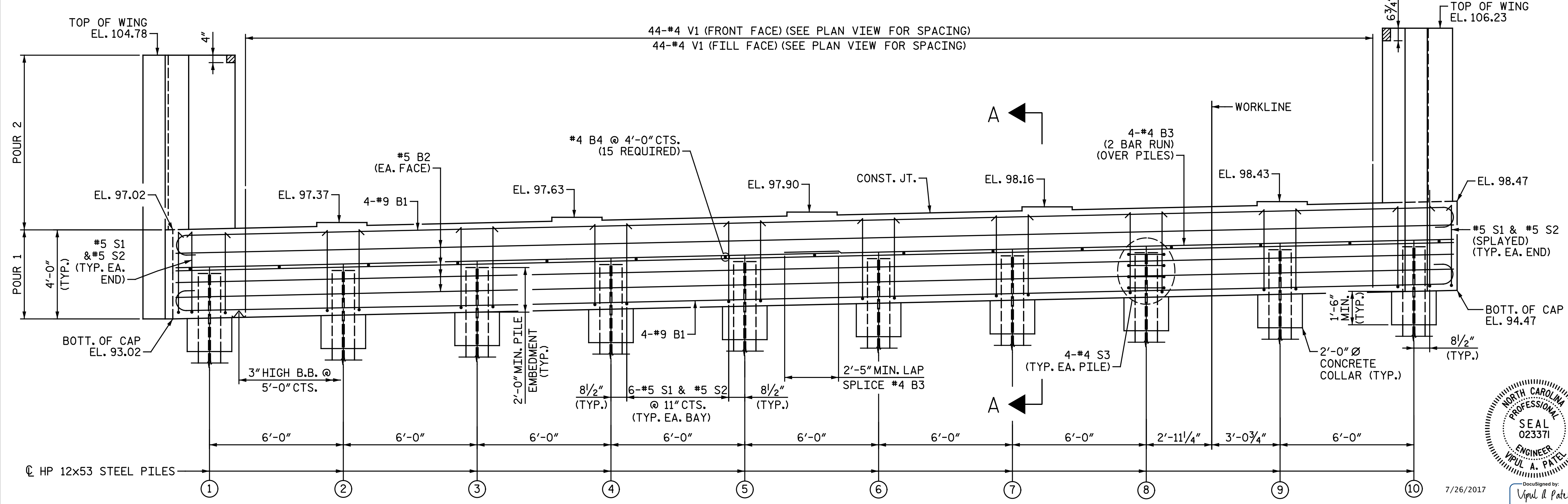


NOTES:
 FOR "SECTION A-A", SEE "INTEGRAL END BENT 1 DETAILS" SHEET.
 STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #4 V1 BARS.
 THE TOP SURFACE OF THE END BENT CAP, EXCLUDING THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4\"/>



DETAIL "A"
 ALL DIMENSIONS AND DETAILS SHOWN ARE TYPICAL FOR ALL BEARINGS AT EACH BRIDGE SEAT LOCATION.

PLAN



TOP OF PILE ELEVATIONS	
PILE	ELEVATION
①	95.08
②	95.23
③	95.38
④	95.53
⑤	95.68
⑥	95.83
⑦	95.98
⑧	96.14
⑨	96.29
⑩	96.44

PROJECT NO. R-5703
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 STATION: 89+28.52 -L-
 SHEET 1 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
INTEGRAL END BENT 1

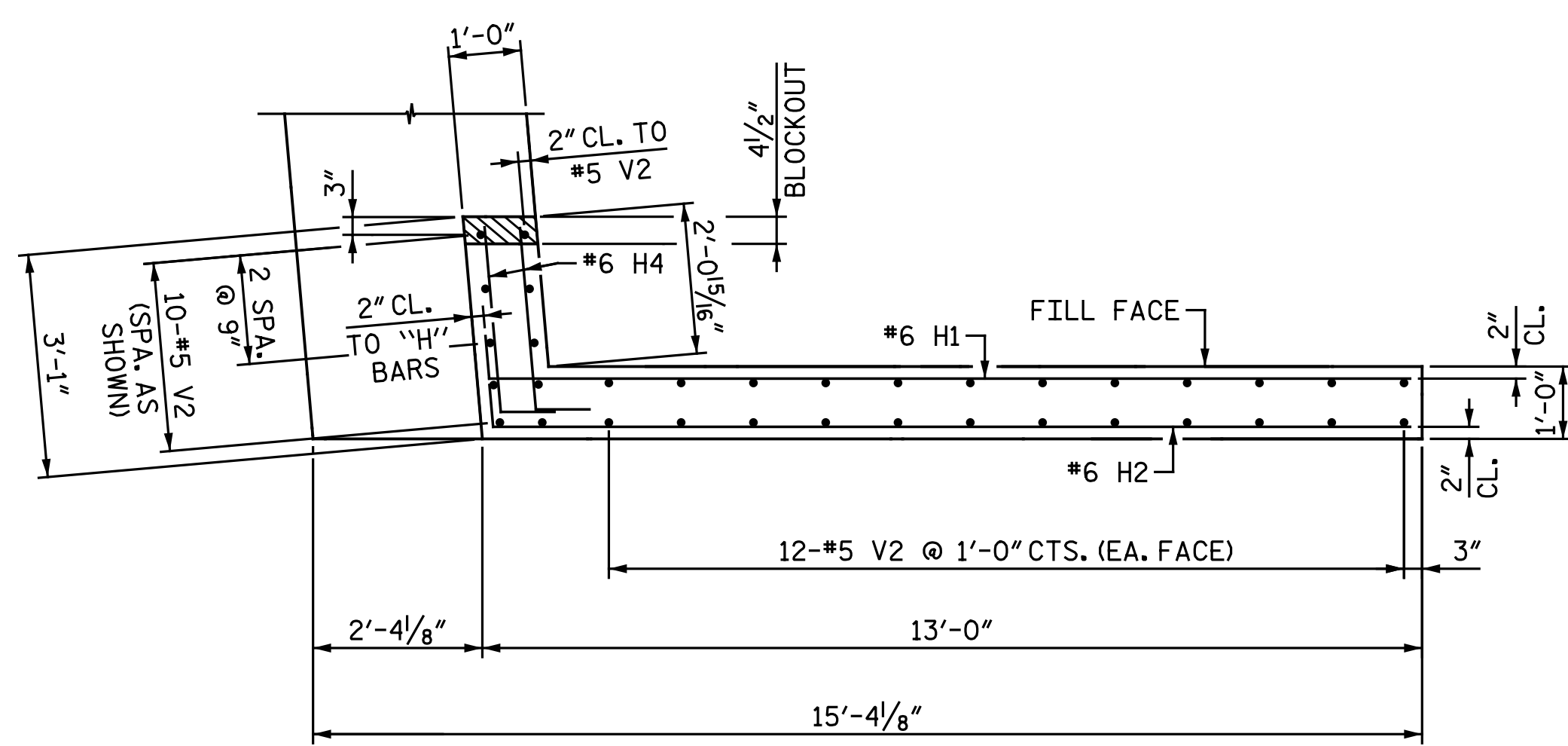
DRAWN BY: D. A. LAMAY DATE: 4-6-17
 CHECKED BY: V. A. PATEL DATE: 4-24-17

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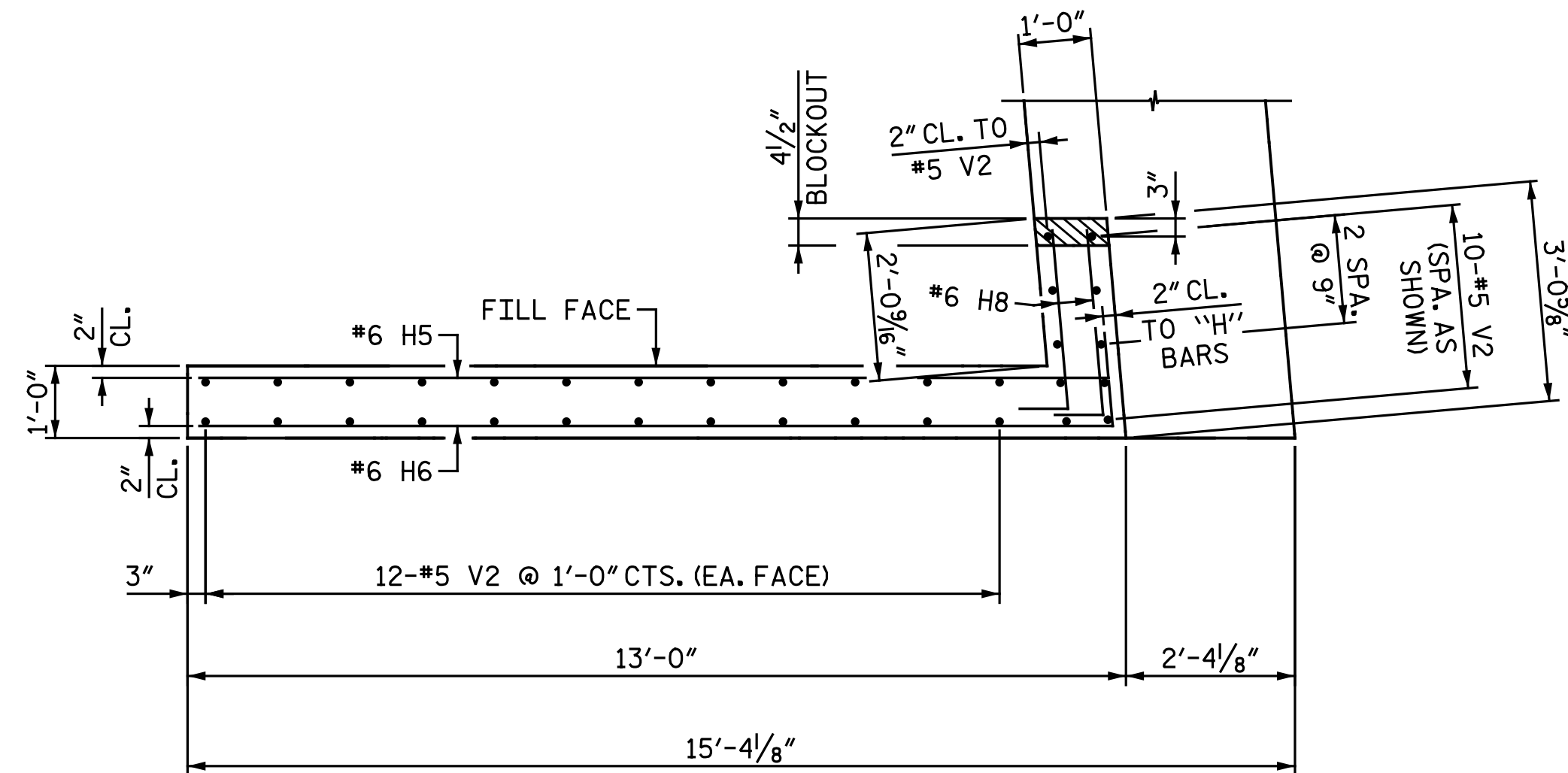
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NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 25
2			4			

LEFT LANE

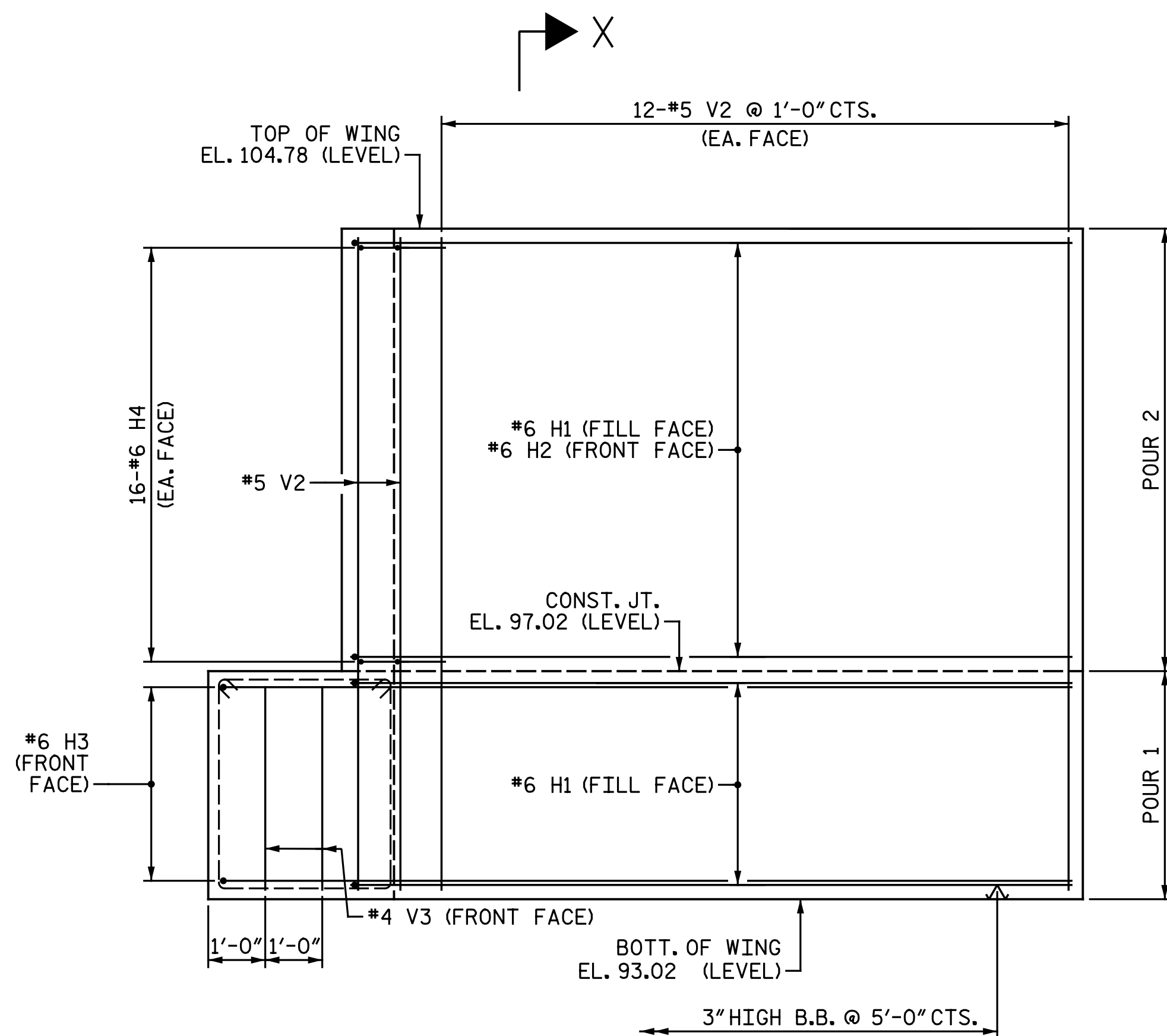
ELEVATION



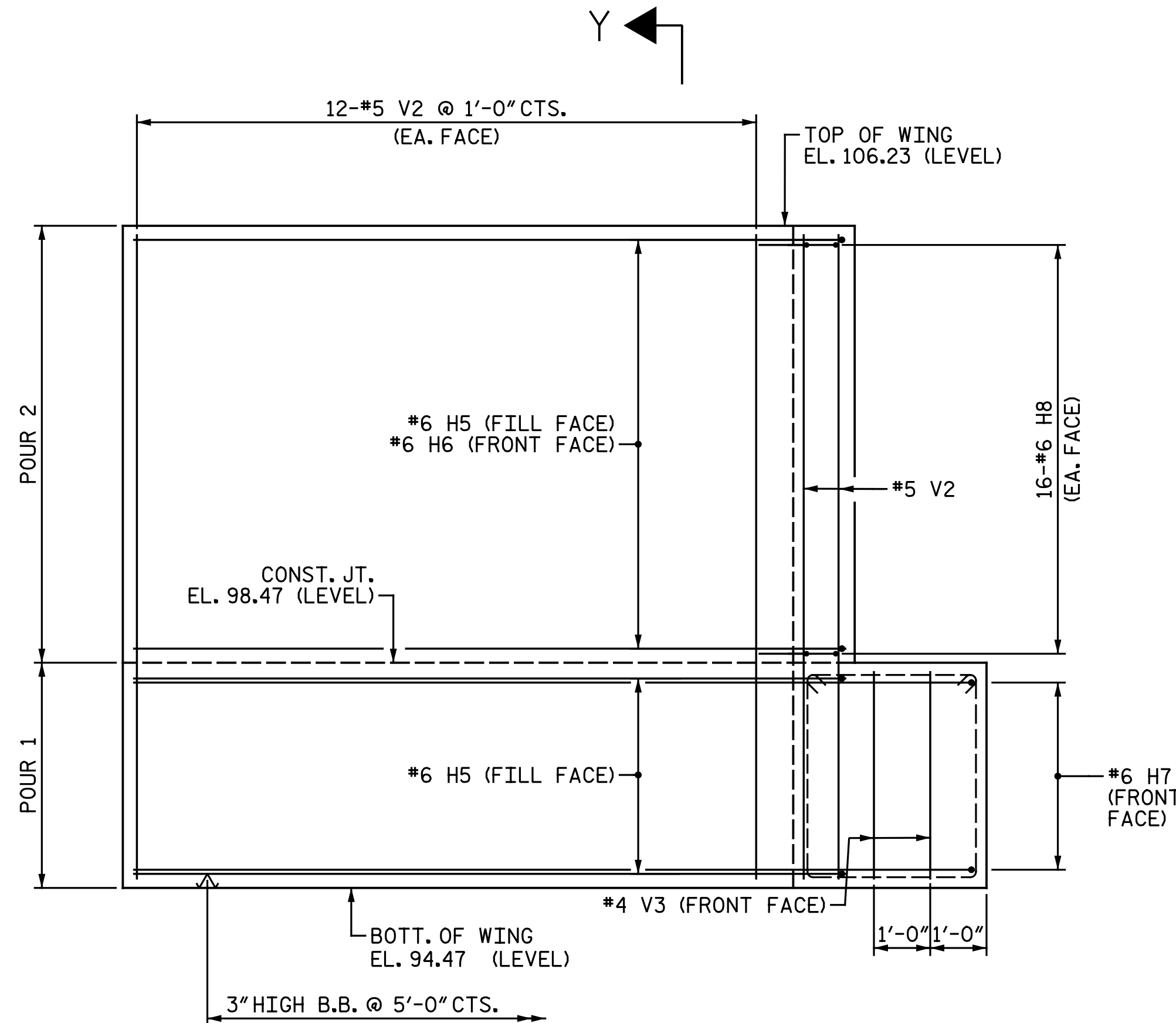
PLAN OF LEFT WING
(H3 BARS NOT SHOWN FOR CLARITY)



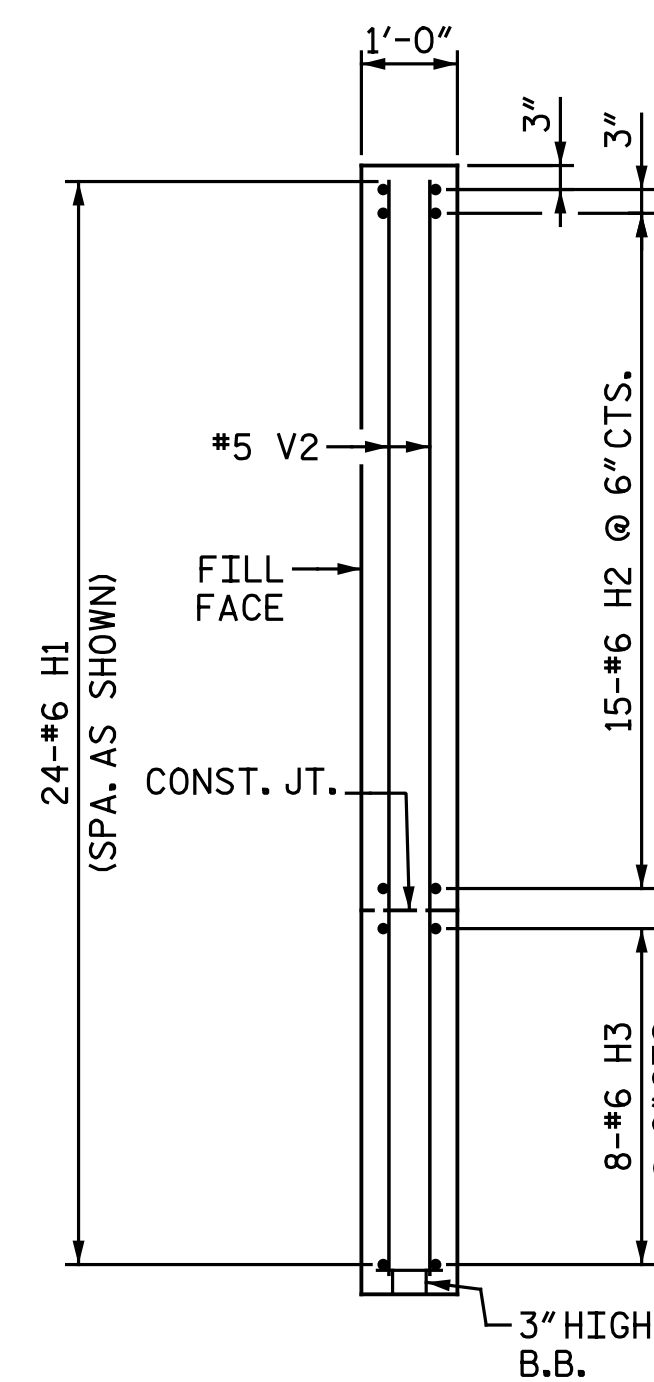
PLAN OF RIGHT WING
(H7 BARS NOT SHOWN FOR CLARITY)



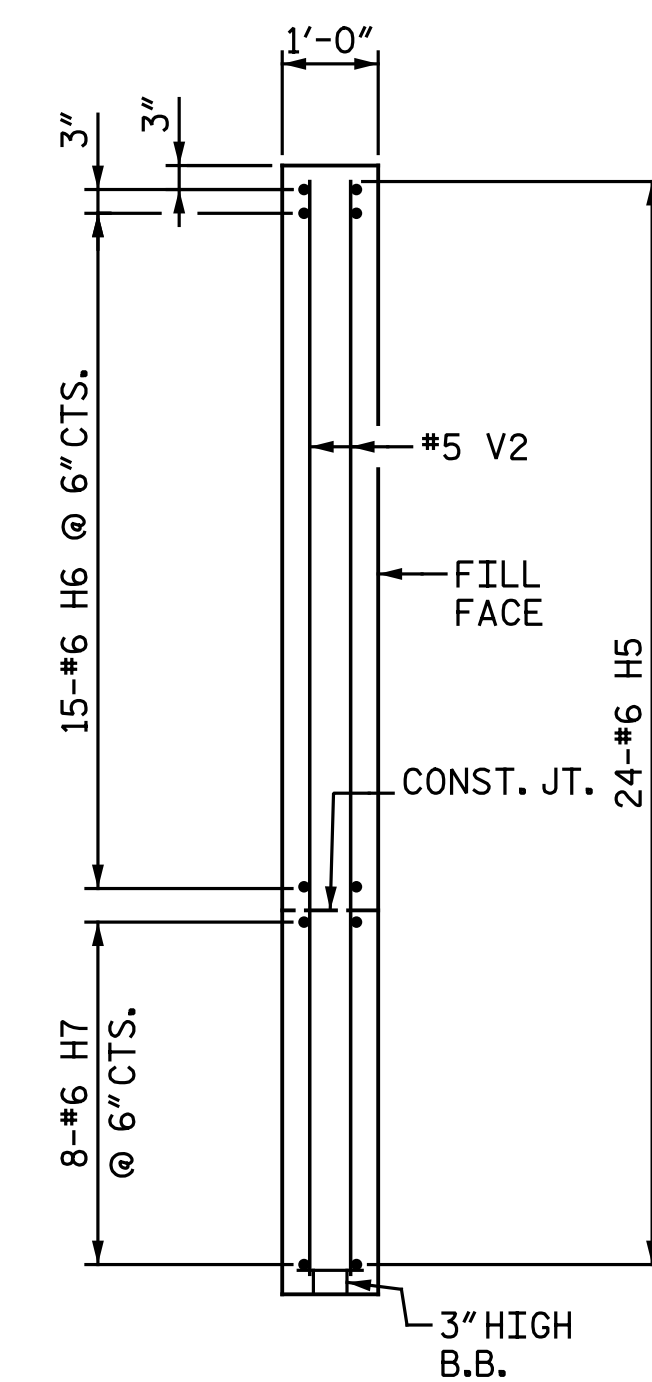
ELEVATION OF LEFT WING



ELEVATION OF RIGHT WING



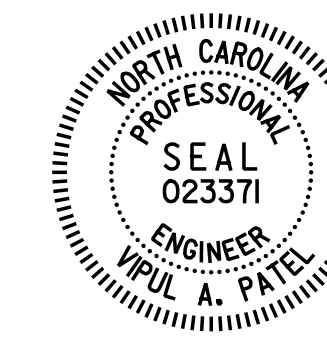
SECTION X-X



SECTION Y-Y

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-

SHEET 2 OF 2



7/26/2017

Designed by
Vipul A. Patel

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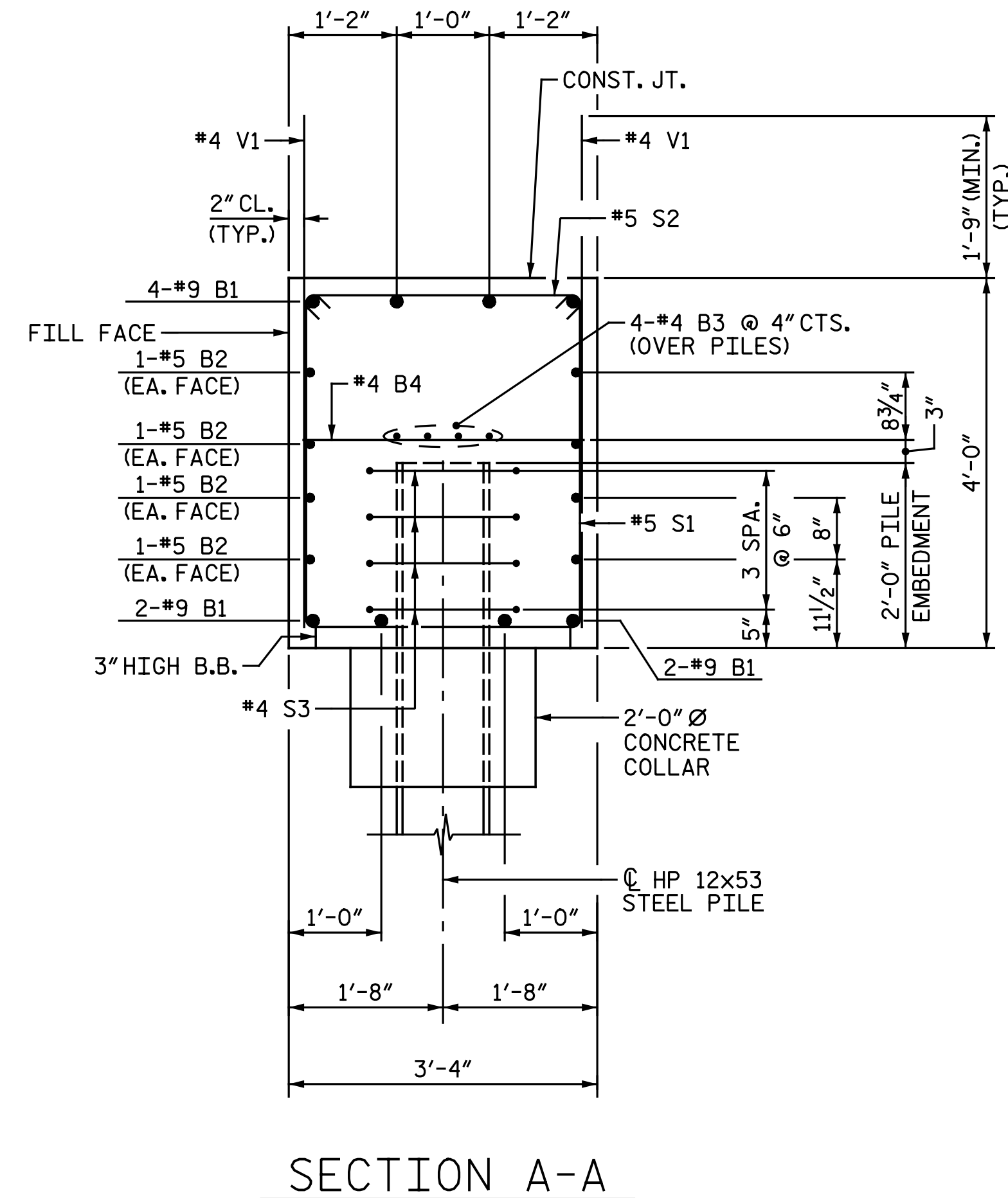
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 1

LEFT LANE

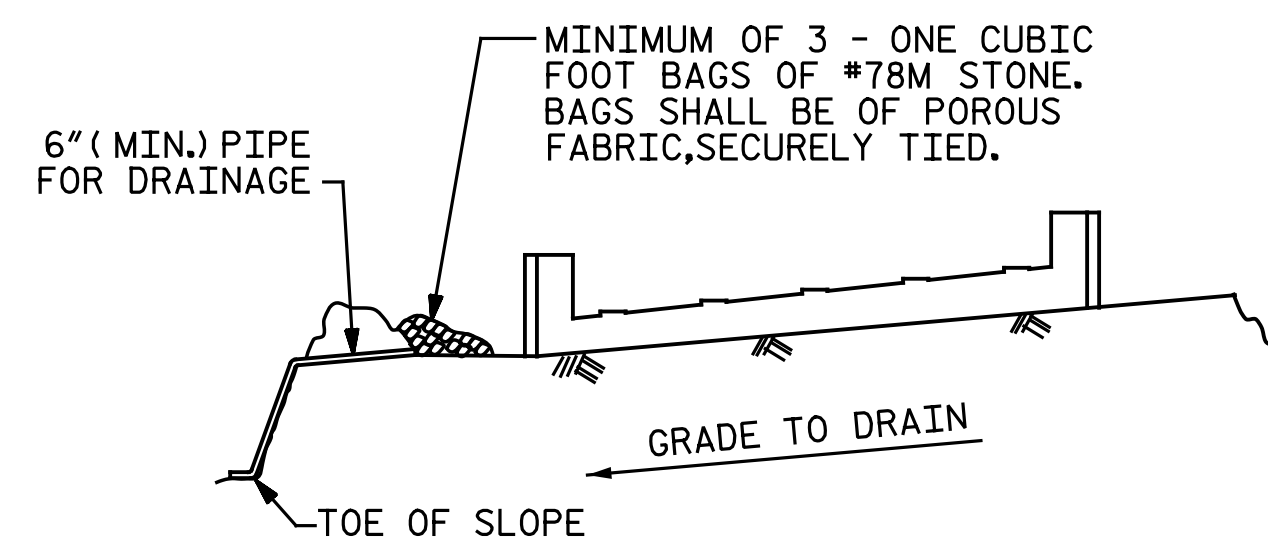
DRAWN BY : D. A. LAMAY DATE : 4-6-17
 CHECKED BY : V. A. PATEL DATE : 4-24-17

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SECTION A-A



BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

DRAWN BY : D. A. LAMAY DATE : 4-6-17
 CHECKED BY : V. A. PATEL DATE : 4-24-17

BILL OF MATERIAL

INTEGRAL END BENT 1

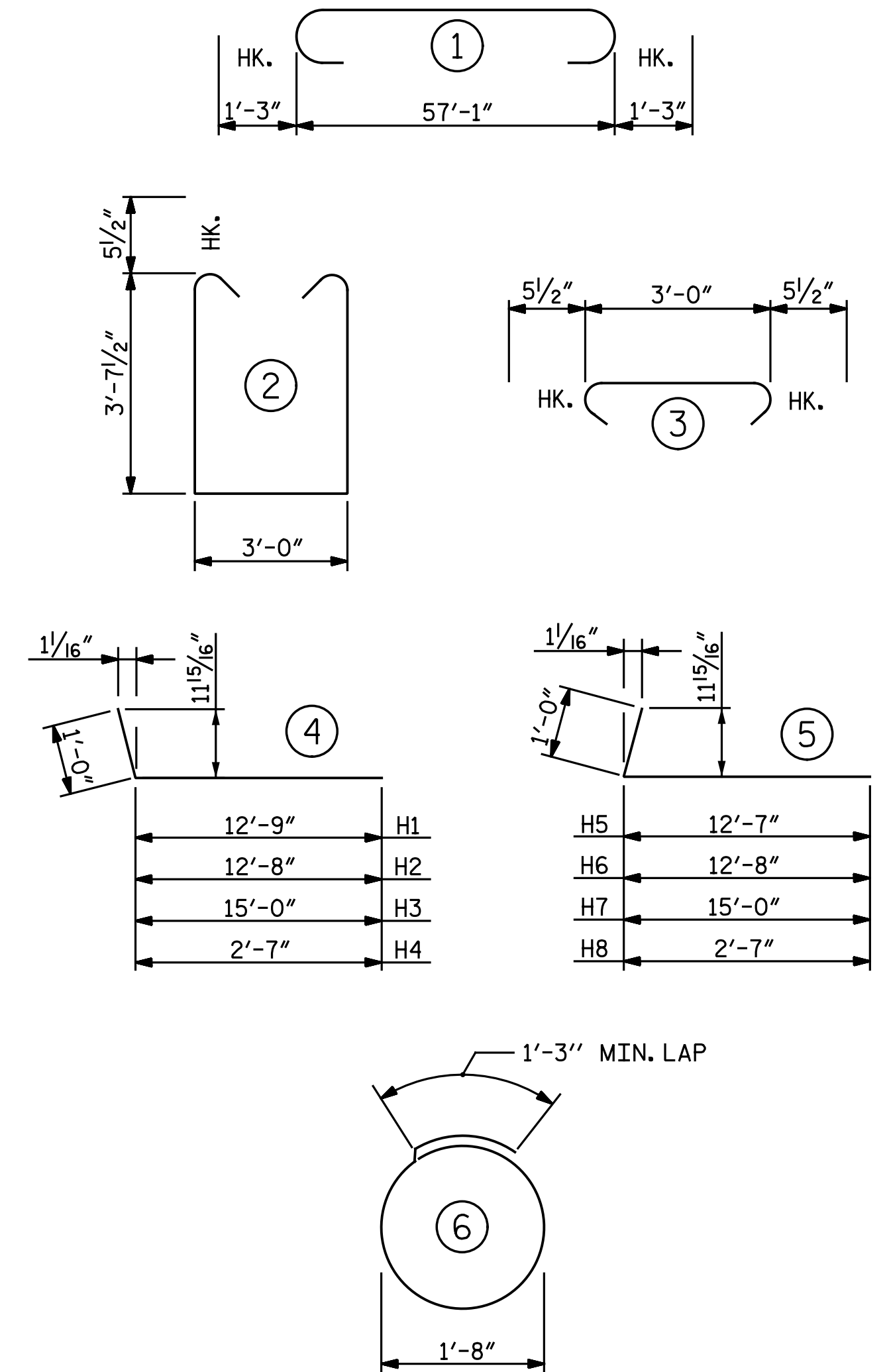
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#9	1	59' - 7"	1,621
B2	8	#5	STR.	57' - 3"	478
B3	8	#4	STR.	29' - 10"	159
B4	15	#4	STR.	3' - 0"	30
H1	24	#6	4	13' - 9"	496
H2	16	#6	4	13' - 8"	328
H3	8	#6	4	16' - 0"	192
H4	32	#6	4	3' - 7"	172
H5	24	#6	5	13' - 7"	490
H6	16	#6	5	13' - 8"	328
H7	8	#6	5	16' - 0"	192
H8	32	#6	5	3' - 7"	172
S1	58	#5	2	11' - 2"	676
S2	58	#5	3	3' - 11"	237
S3	40	#4	6	6' - 6"	174
V1	88	#4	STR.	5' - 7"	328
V2	68	#5	STR.	11' - 5"	810
V3	4	#4	STR.	3' - 7"	10

REINFORCING STEEL		LBS.	6,893
CLASS A CONCRETE			
POUR 1 -			
CAP, LOWER PART OF WINGS & COLLARS		C.Y.	34.2
POUR 2 -			
UPPER PART OF WINGS		C.Y.	8.6
TOTAL		C.Y.	42.8
PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES		EA.	10
HP 12x53 STEEL PILES			
NO. 10		L.F.	800
PILE REDRIVES		EA.	5

NOTE:

FOR PILE SPLICE DETAILS, SEE "INTEGRAL END BENT 2 DETAILS" SHEET.

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT.

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



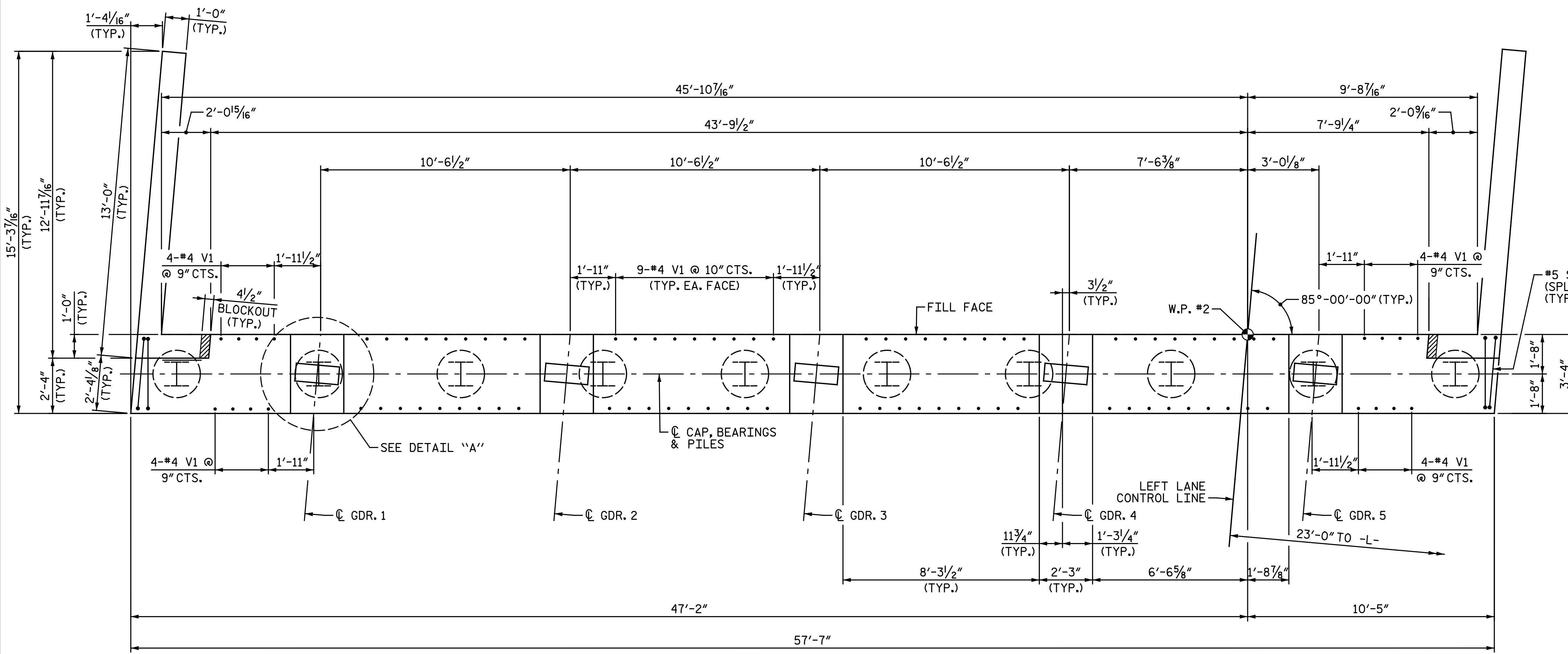
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 1
 DETAILS

DOCUMENT NOT CONSIDERED FINAL
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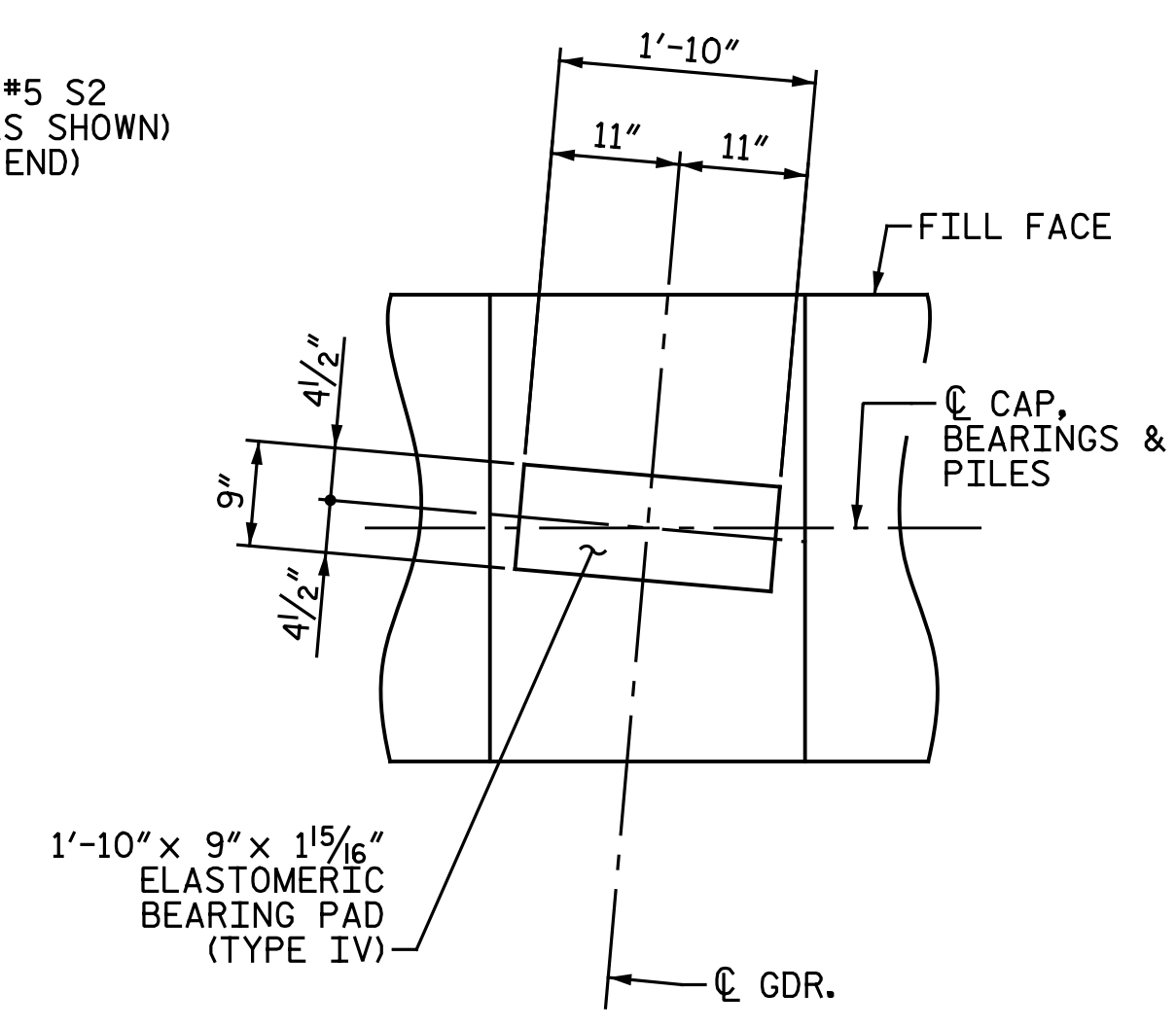
LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	SI-18
1			3			TOTAL SHEETS
2			4			25



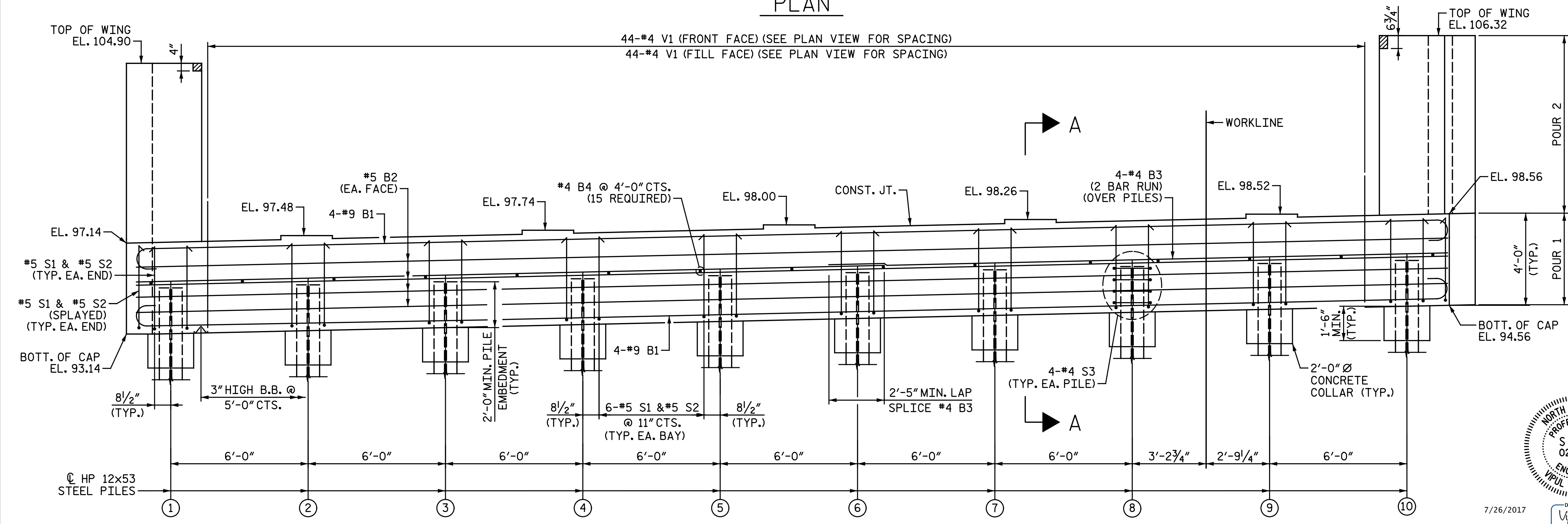
PLAN

NOTES:
 FOR "SECTION A-A", SEE "INTEGRAL END BENT 1 DETAILS" SHEET.
 STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #4 V1 BARS.
 THE TOP SURFACE OF THE END BENT CAP, EXCLUDING THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".
 THE CONCRETE IN THE HATCHED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.



DETAIL "A"

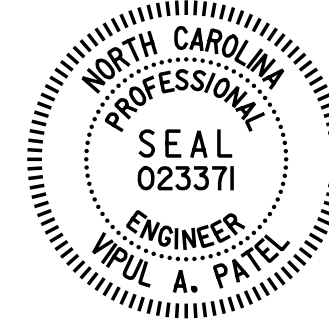
ALL DIMENSIONS AND DETAILS SHOWN ARE TYPICAL FOR ALL BEARINGS AT EACH BRIDGE SEAT LOCATION.



ELEVATION

TOP OF PILE ELEVATIONS	
PILE	ELEVATION
①	95.19
②	95.34
③	95.49
④	95.64
⑤	95.79
⑥	95.94
⑦	96.09
⑧	96.23
⑨	96.38
⑩	96.53

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 89+28.52 -L-
 SHEET 1 OF 2



7/26/2017
 Vipul A. Patel

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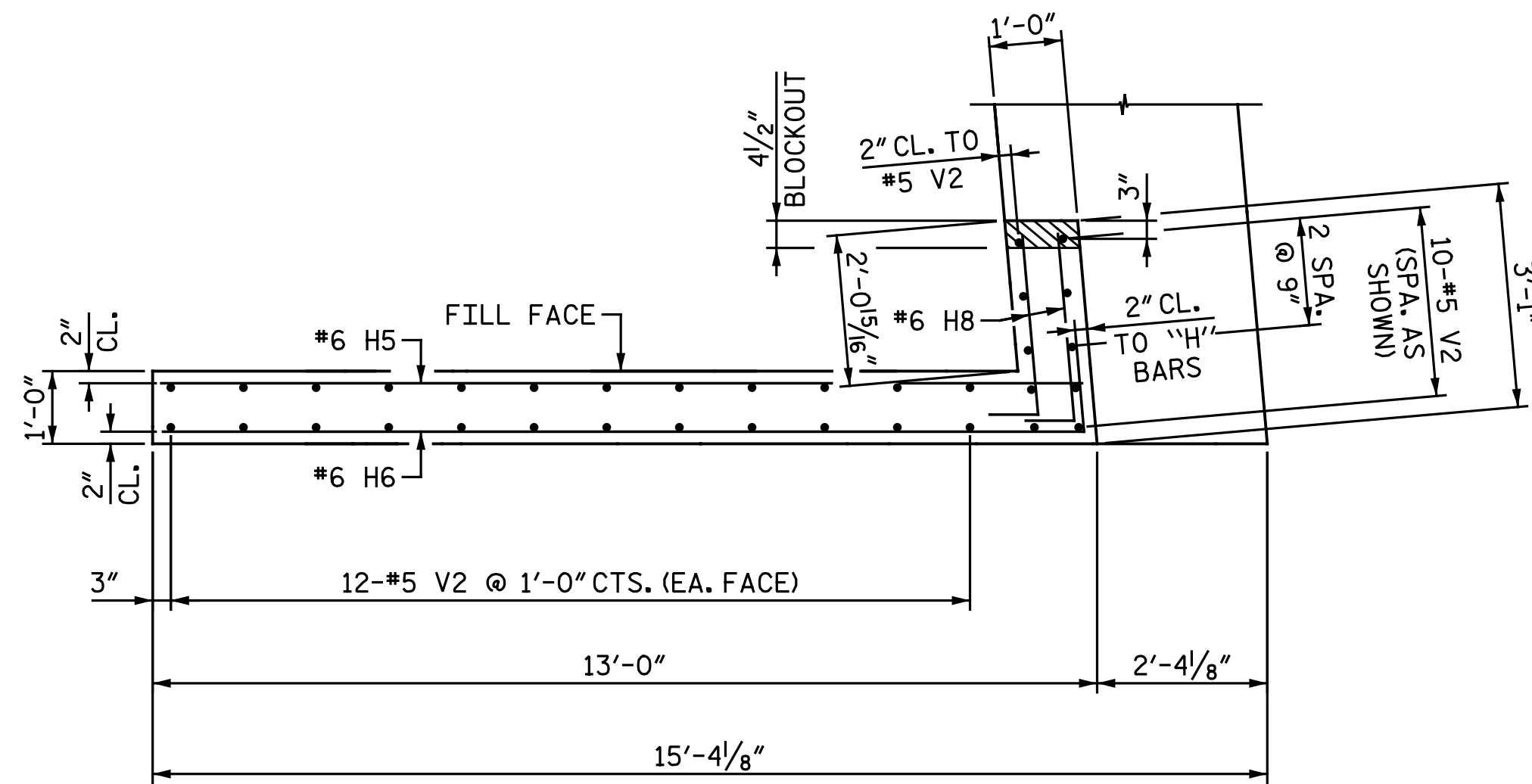
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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 2

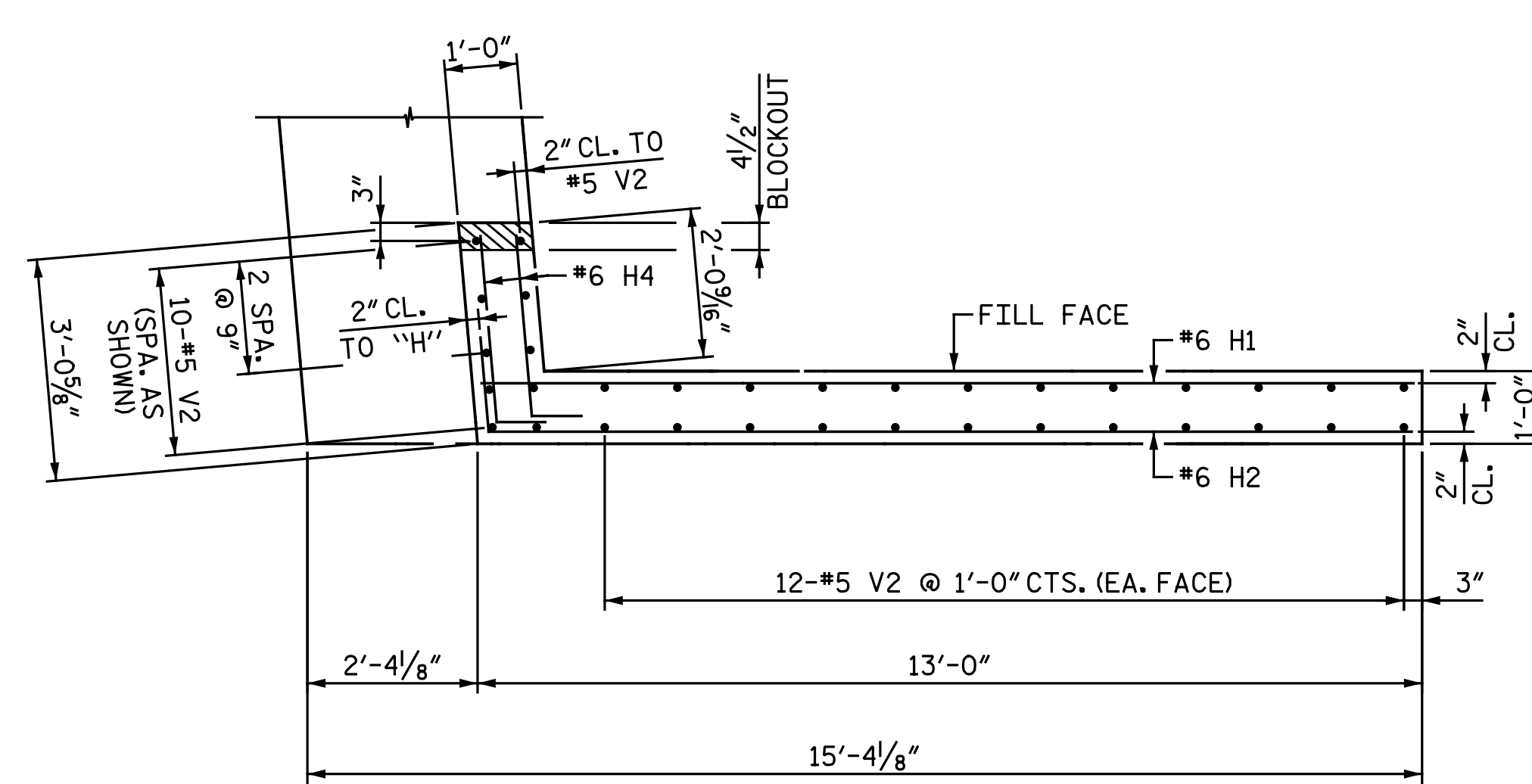
LEFT LANE

REVISIONS						SHEET NO. SI-19
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 25
2			4			

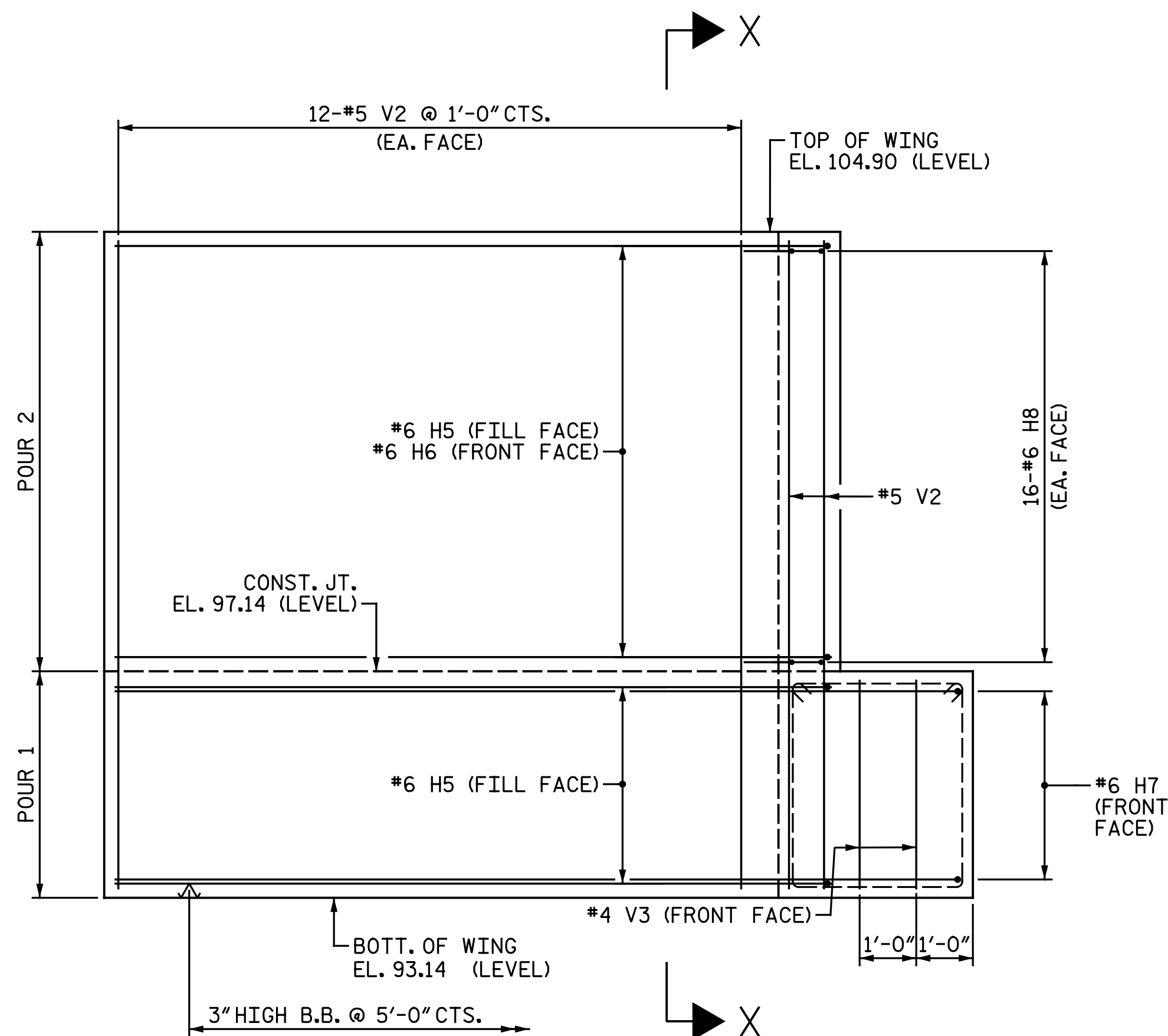
DRAWN BY: D. A. LAMAY DATE: 4-6-17
 CHECKED BY: V. A. PATEL DATE: 4-26-17



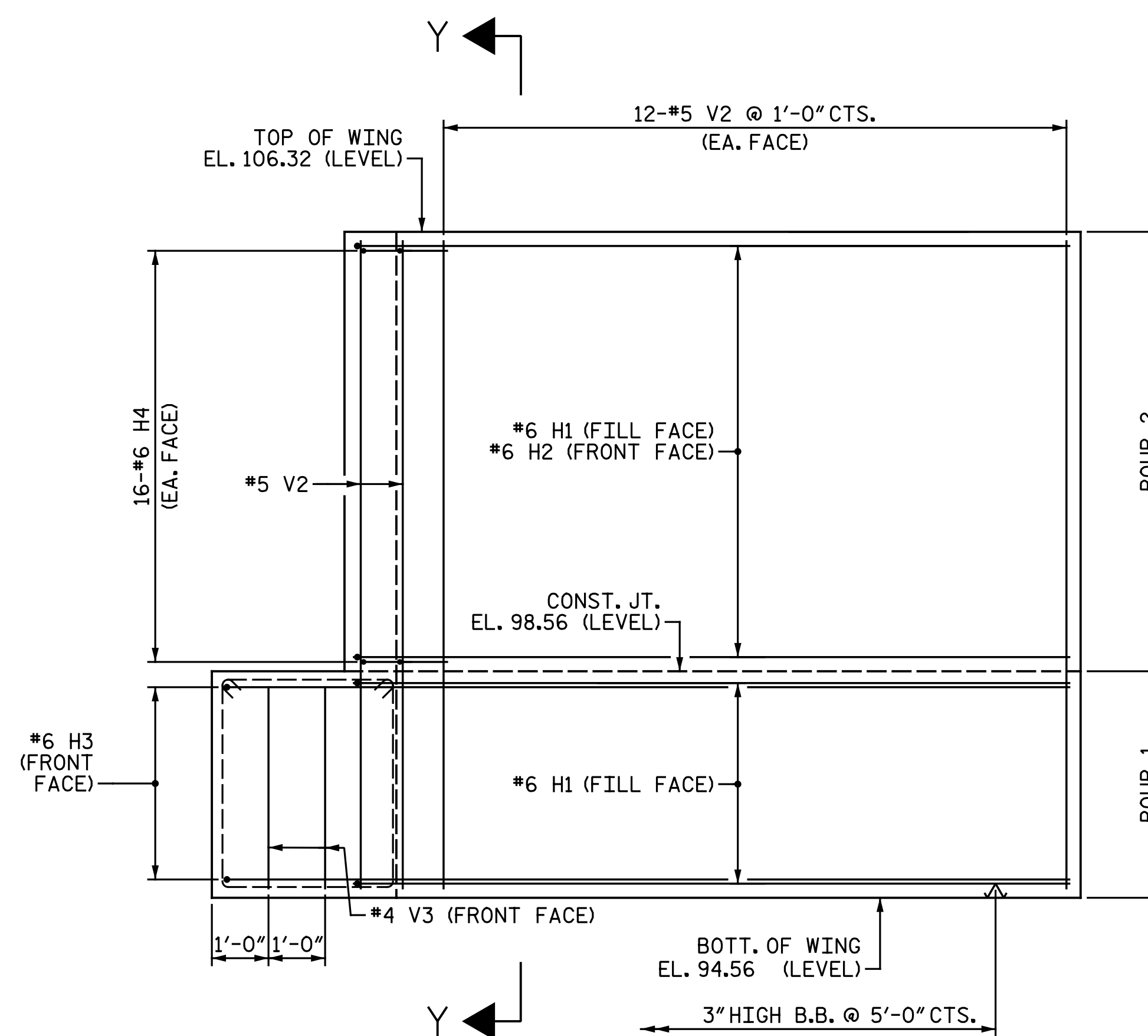
PLAN OF LEFT WING
(H7 BARS NOT SHOWN FOR CLARITY)



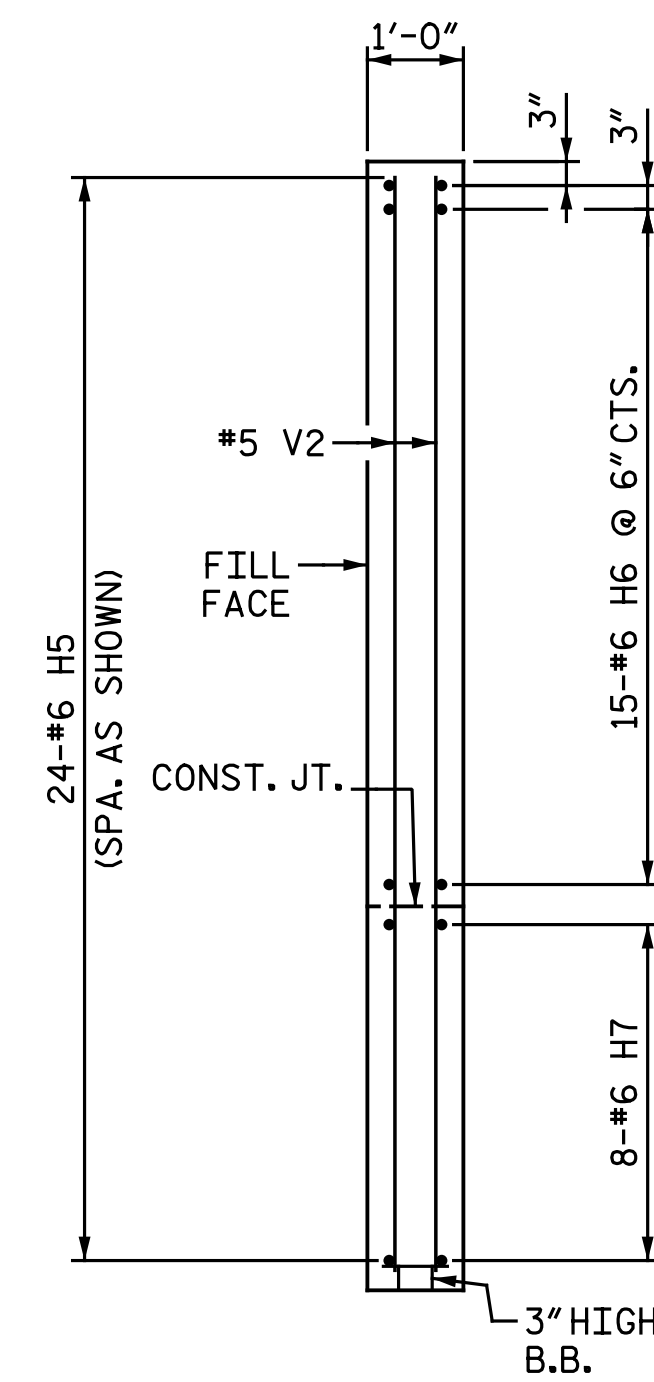
PLAN OF RIGHT WING
(H3 BARS NOT SHOWN FOR CLARITY)



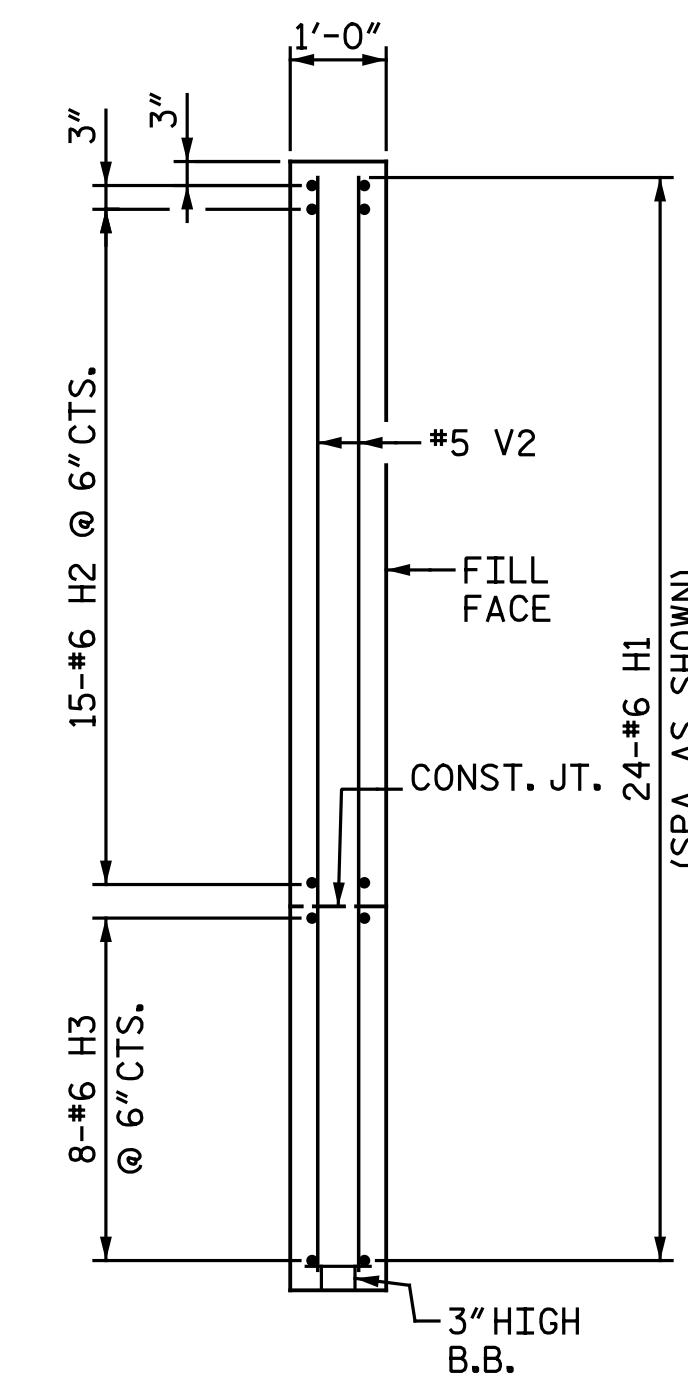
ELEVATION OF LEFT WING



ELEVATION OF RIGHT WING



SECTION X-X



SECTION Y-Y

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-

SHEET 2 OF 2



7/26/2017

Designed by
Vipul A. Patel

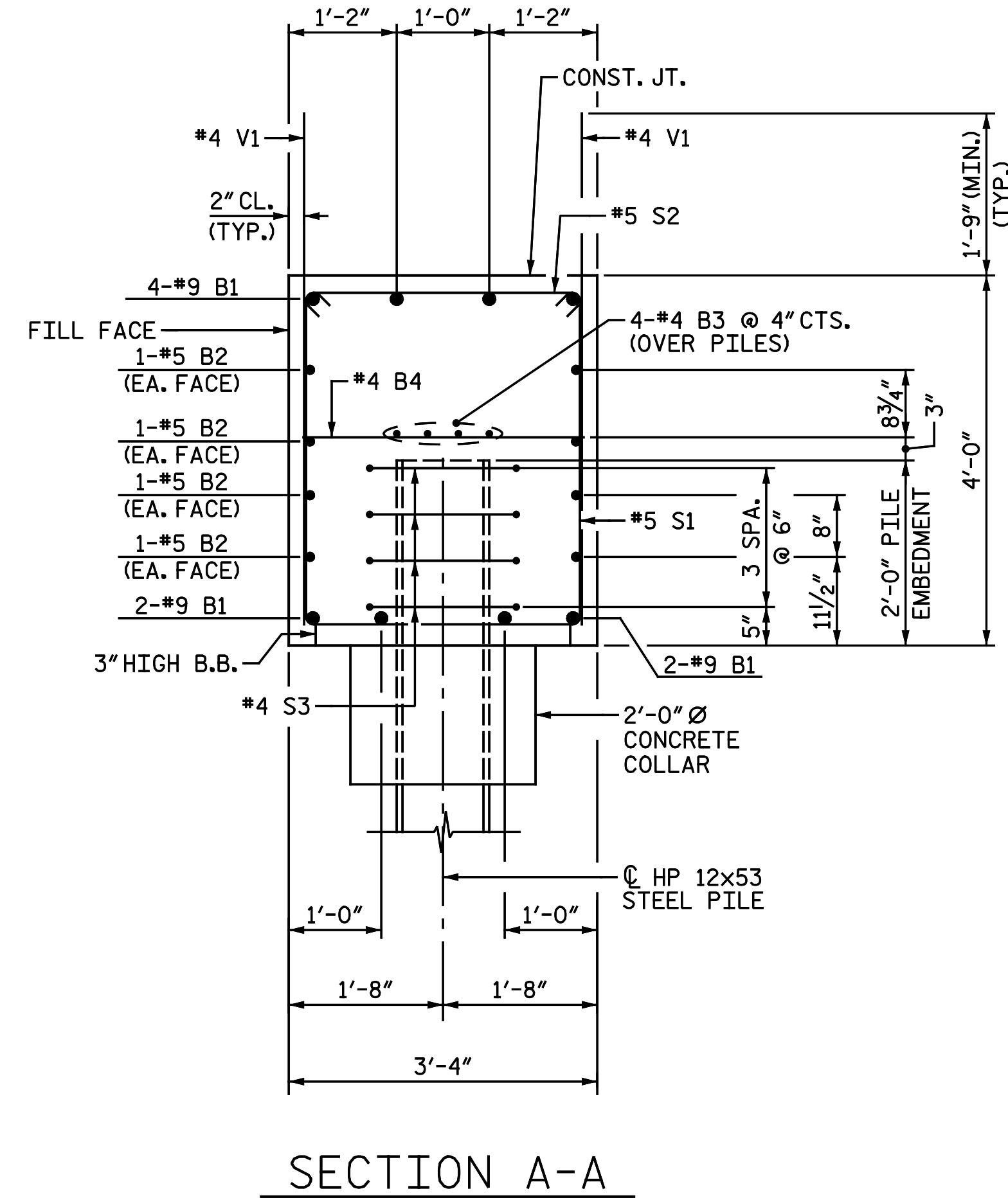
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 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 2

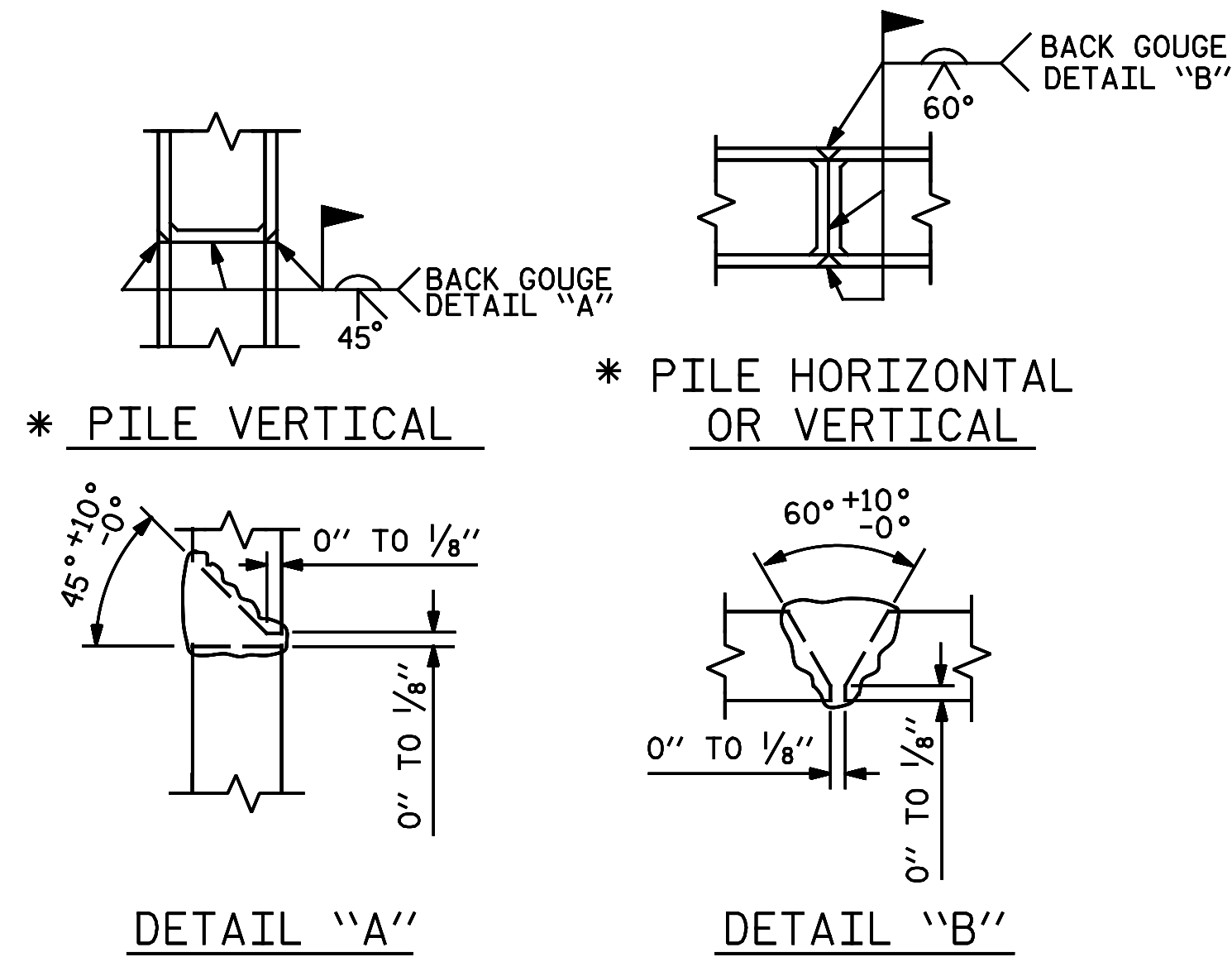
LEFT LANE

DRAWN BY : D. A. LAMAY DATE : 4-12-17
 CHECKED BY : V. A. PATEL DATE : 4-26-17

NO.	BY:	DATE:	REVISIONS			SHEET NO.
			NO.	BY:	DATE:	
1			3			TOTAL SHEETS
2			4			25



SECTION A-A



PILE SPLICE DETAILS

* POSITION OF PILE DURING WELDING

BILL OF MATERIAL

INTEGRAL END BENT 2

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#9		59' - 7"	1,621
B2	8	#5	STR.	57' - 3"	478
B3	8	#4	STR.	29' - 10"	159
B4	15	#4	STR.	3' - 0"	30
H1	24	#6	4	13' - 9"	496
H2	16	#6	4	13' - 8"	328
H3	8	#6	4	16' - 0"	192
H4	32	#6	4	3' - 7"	172
H5	24	#6	5	13' - 7"	490
H6	16	#6	5	13' - 8"	328
H7	8	#6	5	16' - 0"	192
H8	32	#6	5	3' - 7"	172
S1	58	#5	2	11' - 2"	676
S2	58	#5	3	3' - 11"	237
S3	40	#4	6	6' - 6"	174
V1	88	#4	STR.	5' - 7"	328
V2	68	#5	STR.	11' - 5"	810
V3	4	#4	STR.	3' - 7"	10

REINFORCING STEEL LBS. 6,893

CLASS A CONCRETE			
POUR 1 -			
CAP, LOWER PART OF WINGS & COLLARS		C.Y.	34.2
POUR 2 -			
UPPER PART OF WINGS		C.Y.	8.6
TOTAL		C.Y.	42.8

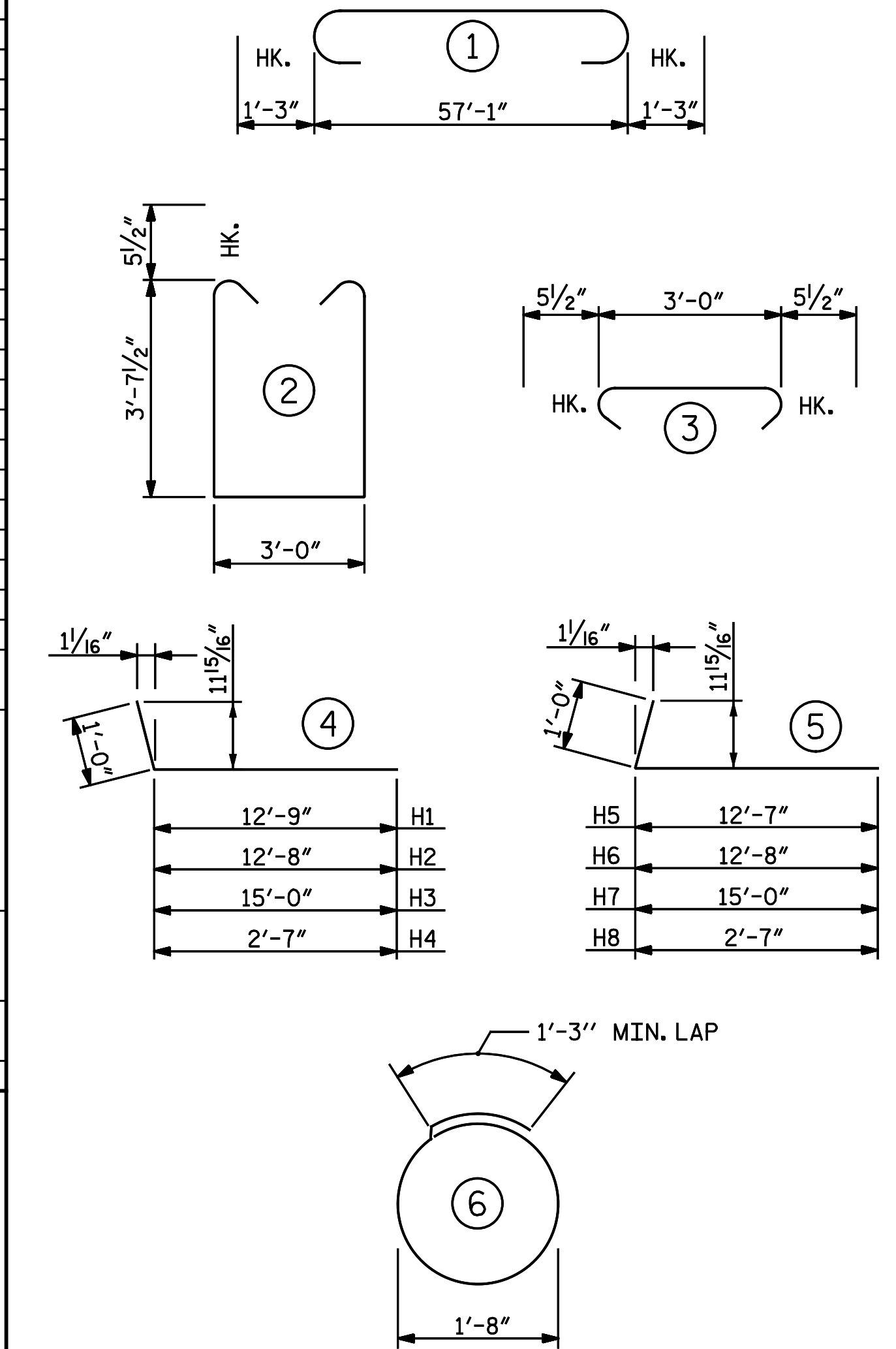
PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES			
		EA.	10

HP 12x53 STEEL PILES			
NO. 10		L.F.	800
PILE REDRIVES		EA.	5

NOTE:

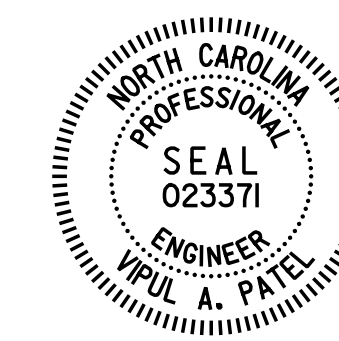
FOR TEMPORARY DRAINAGE AT END BENT, SEE "INTEGRAL END BENT 1 DETAILS" SHEET.

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT.

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-



7/26/2017

DocuSigned by Vipul A. Patel

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DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE
INTEGRAL END BENT 2
DETAILS

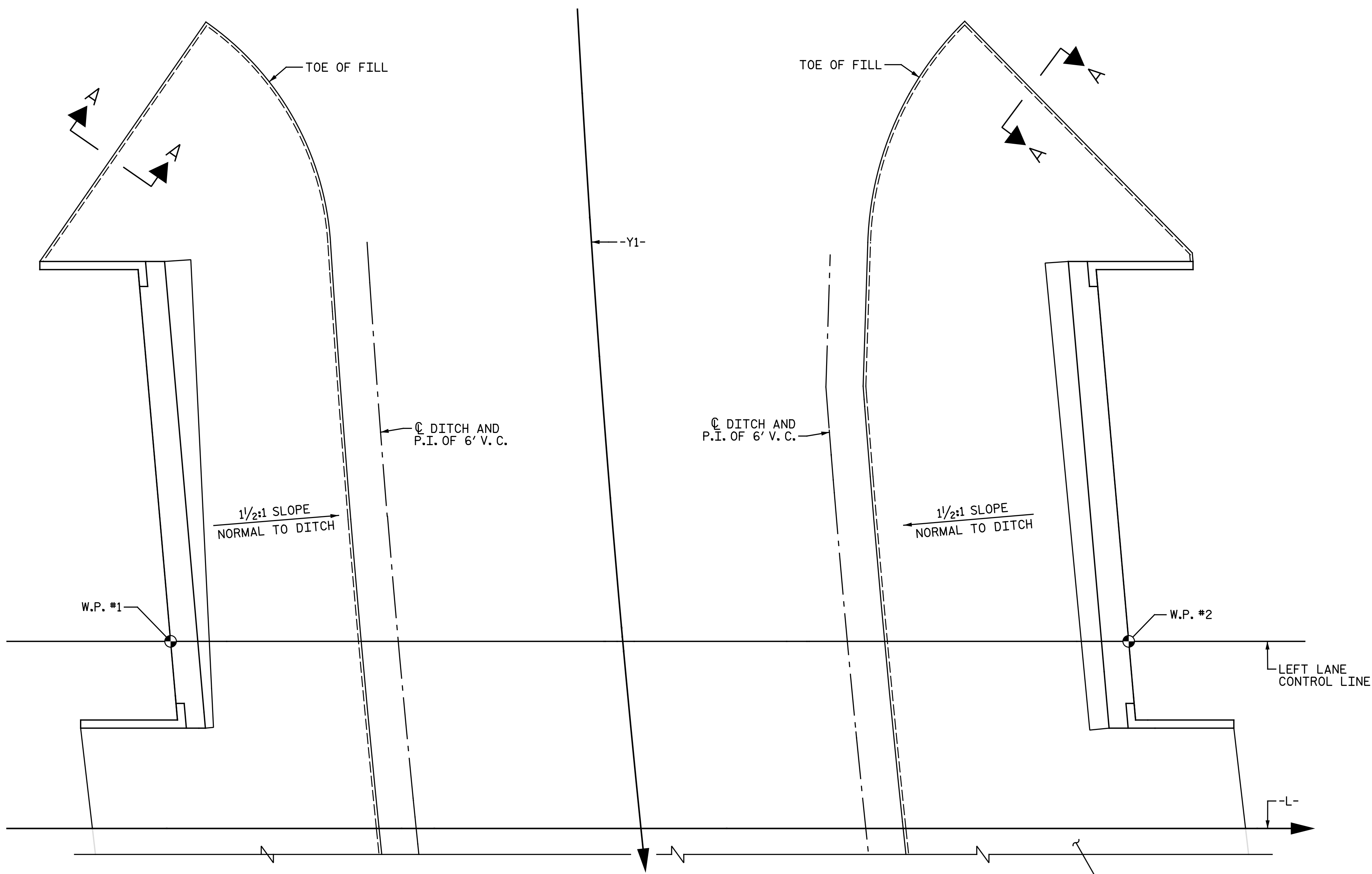
LEFT LANE

REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			SI-21
2			4			TOTAL SHEETS 25

DRAWN BY: D. A. LAMAY DATE: 4-6-17
CHECKED BY: V. A. PATEL DATE: 4-26-17

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Cary, North Carolina 27518
NC License No.: F-1084



PLAN

NOTES:

STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT.

MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.

FOR BERM WIDTHS AND ELEVATIONS, SEE GENERAL DRAWING AND "SLOPE PROTECTION DETAILS" SHEET 2 OF 2.

SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET.

CONCRETE SHALL BE CLASS "B".

THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED.

WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE.

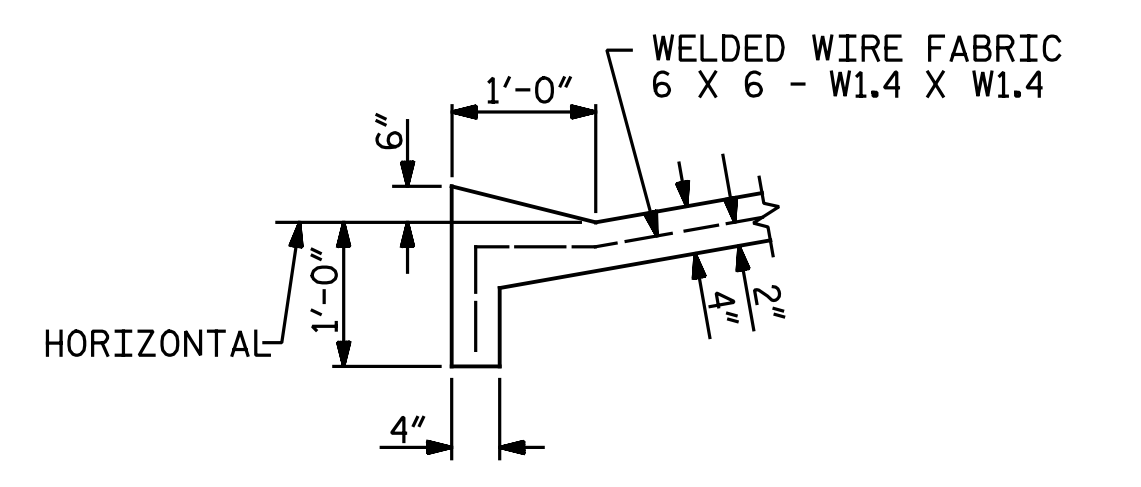
SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0" LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING.

SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6".

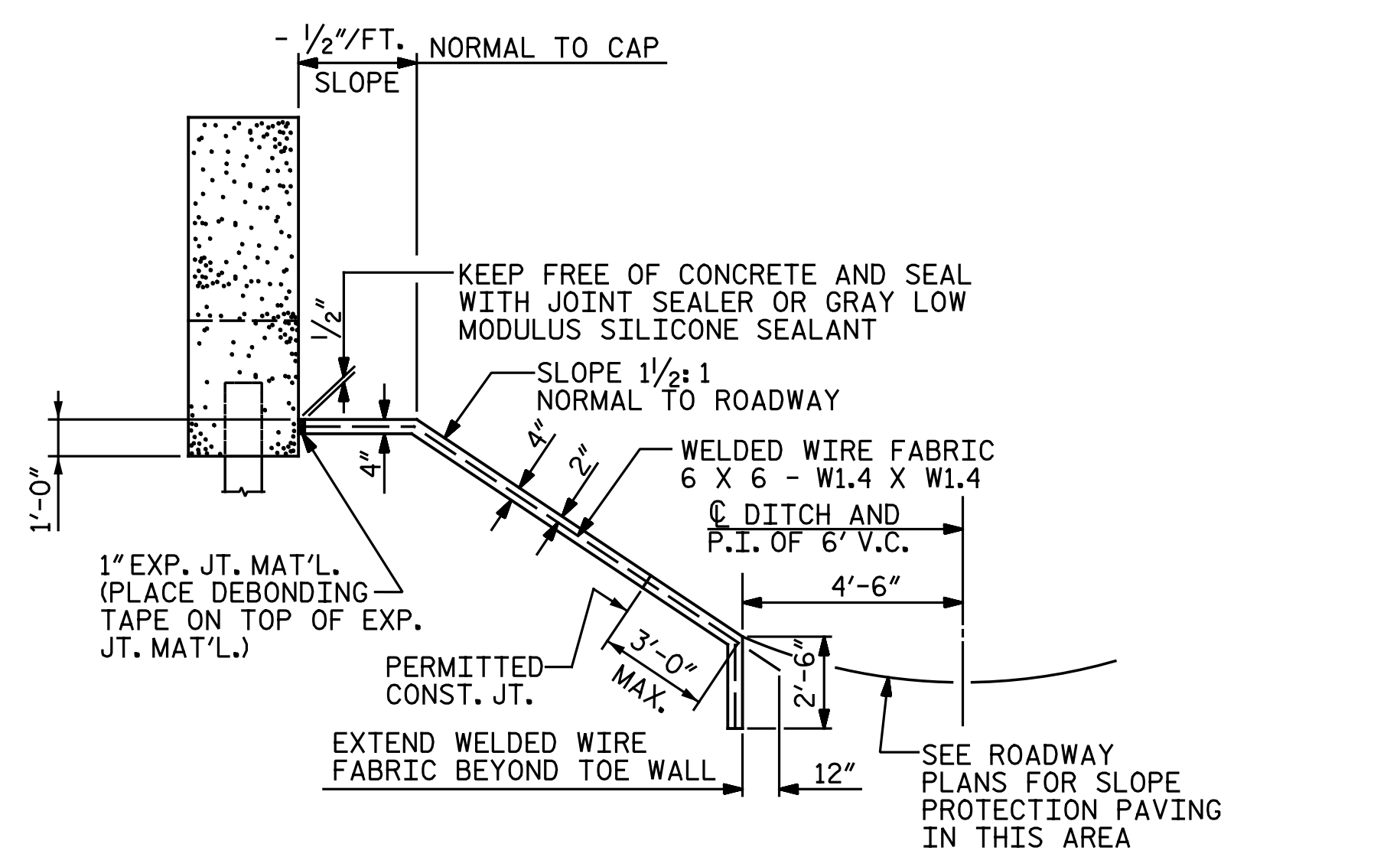
THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA. 89+28.52 -L- (LEFT LANE)	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX. L.F.
END BENT 1	350	630
END BENT 2	430	775

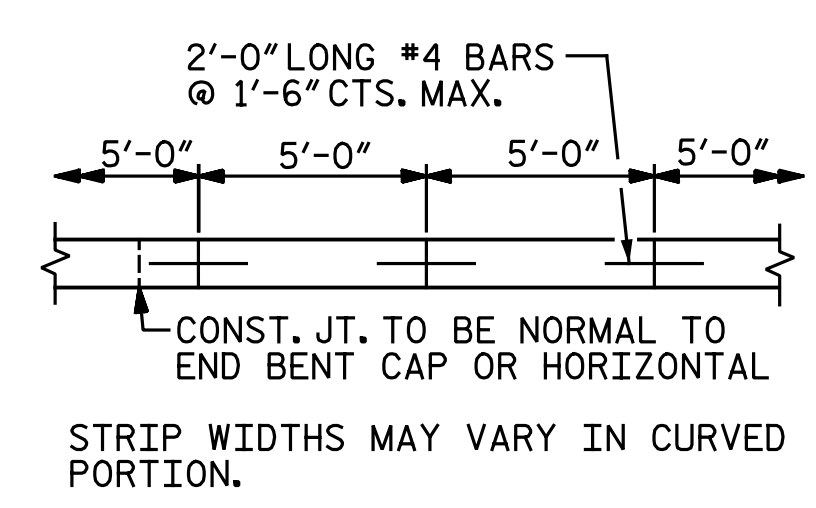
* QUANTITY SHOWN IS BASED ON 5' POURS.



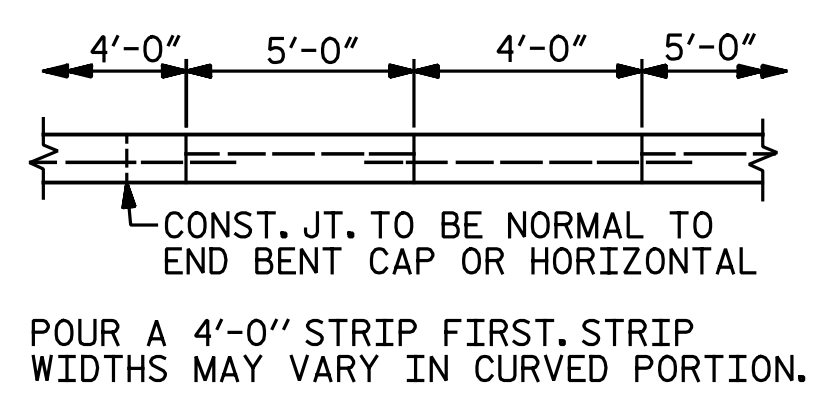
SECTION A-A



SECTION ALONG CONTROL LINE WHEN FILL CATCHES IN DITCH



POURING DETAIL



OPTIONAL POURING DETAIL

SLOPE PROTECTION DETAILS

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 89+28.52 -L-

SHEET 1 OF 2



7/26/2017

DocuSigned by:
Vipul A. Patel

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 SLOPE PROTECTION
 DETAILS
 LEFT LANE

ASSEMBLED BY : C. E. MAYHEW	DATE : 4-24-17
CHECKED BY : V. A. PATEL	DATE : 5-23-17
DRAWN BY : ELR 5/92	REV. 10/1/11 MAA/GM
CHECKED BY : GRP 6/92	REV. 12/21/11 MAA/GM
	REV. 1/16 MAA/TMG

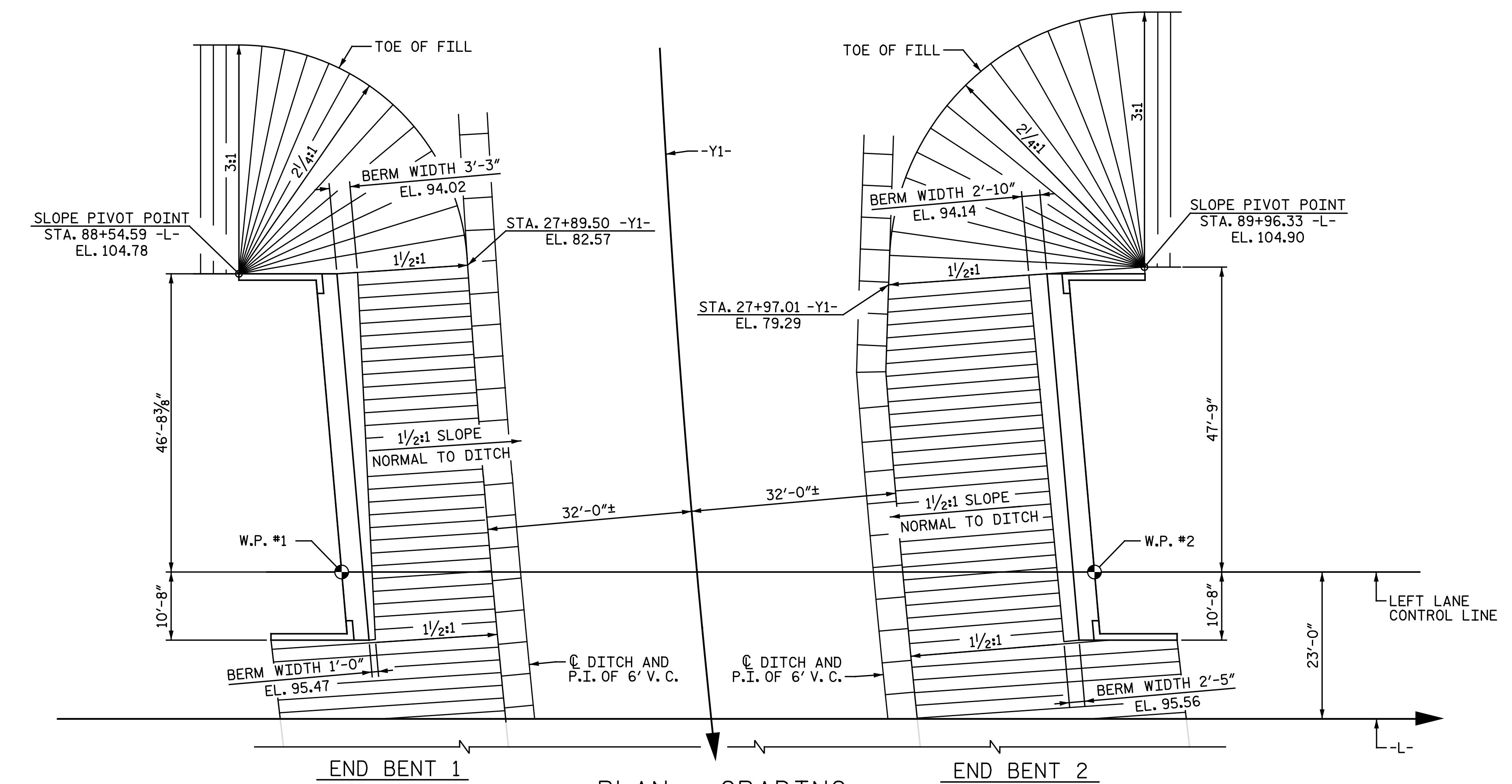
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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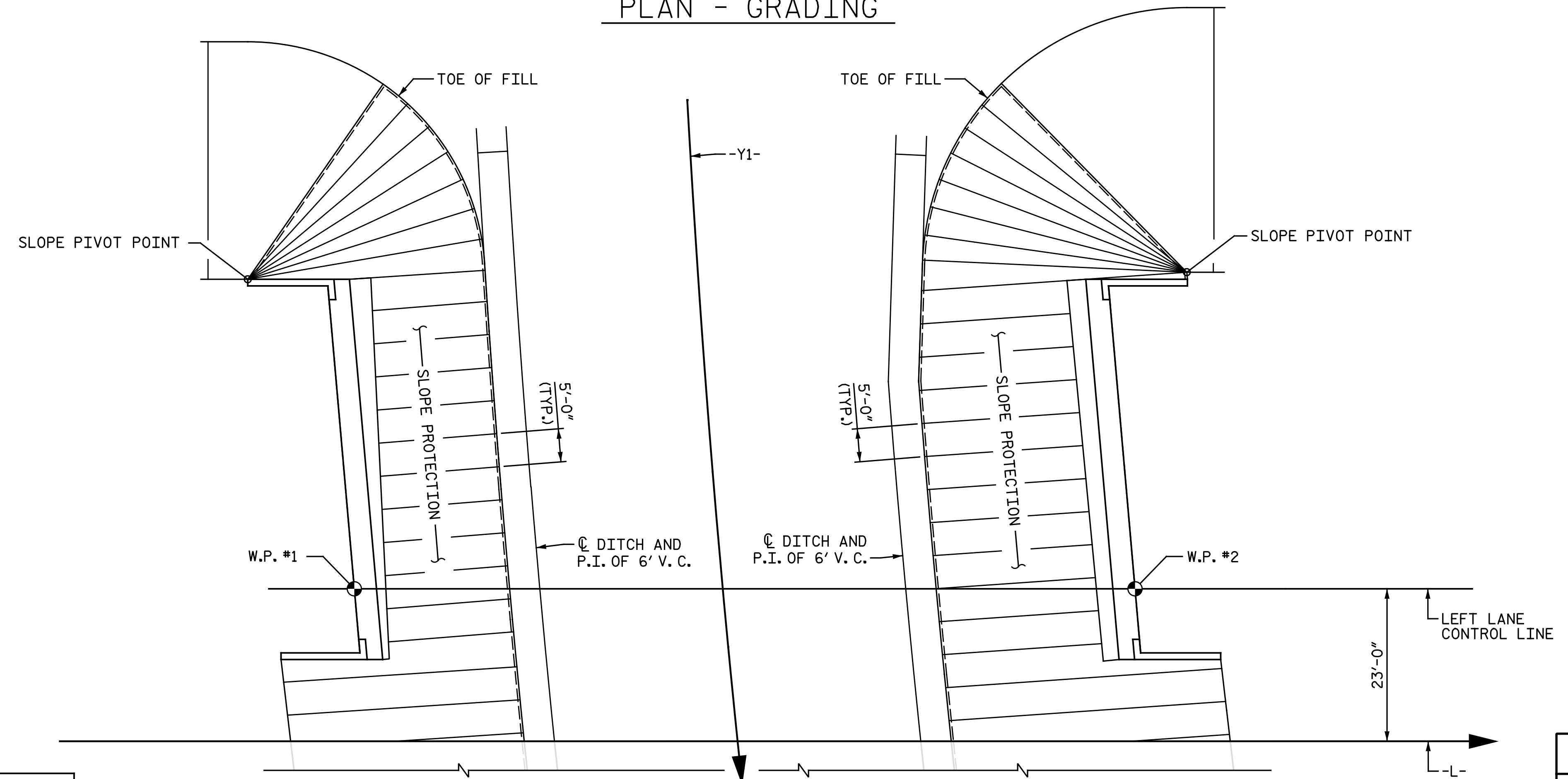
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NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 25
2			4			

NOTE:
ALL ELEVATIONS AND BERM WIDTHS ARE GIVEN AT THE TOP OF CONCRETE SLOPE PROTECTION.



PLAN - GRADING



PLAN - CONCRETE PLACEMENT

(1 1/2:1 SLOPE)

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-

SHEET 2 OF 2



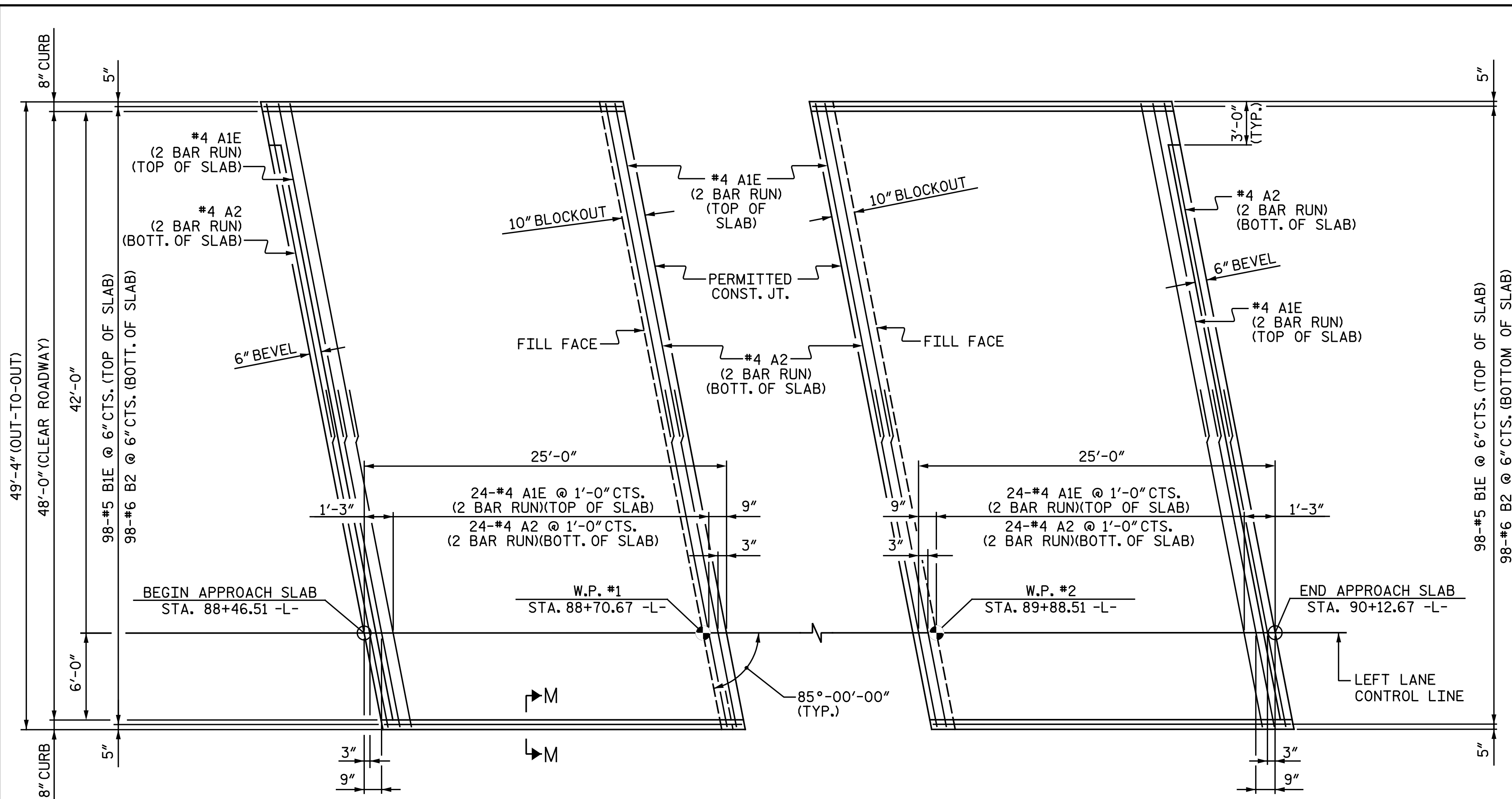
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
SLOPE PROTECTION
DETAILS
LEFT LANE

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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	SI-23
1			3			TOTAL SHEETS
2			4			25

ASSEMBLED BY : C. E. MAYHEW	DATE : 4-27-17
CHECKED BY : V. A. PATEL	DATE : 5-23-17
DRAWN BY : WJH 10/88	REV. 5/1/06 TLA/GM
CHECKED BY : FCJ 10/88	REV. 10/1/11 MAA/GM
	REV. 1/16 MAA/TMG



PLAN AT INTEGRAL END BENT 1 PLAN AT INTEGRAL END BENT 2

NOTES:

AT THE CONTRACTOR'S OPTION, THE APPROACH SLAB MAY BE CAST MONOLITHICALLY WITH THE INTEGRAL END BENT DIAPHRAGM AND THE END SECTION OF BRIDGE DECK. IF CAST WITH THE INTEGRAL DIAPHRAGM, THE LAYERS OF ROOFING FELT SHALL BE OMITTED. IF CAST SEPARATE FROM THE INTEGRAL DIAPHRAGM, APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

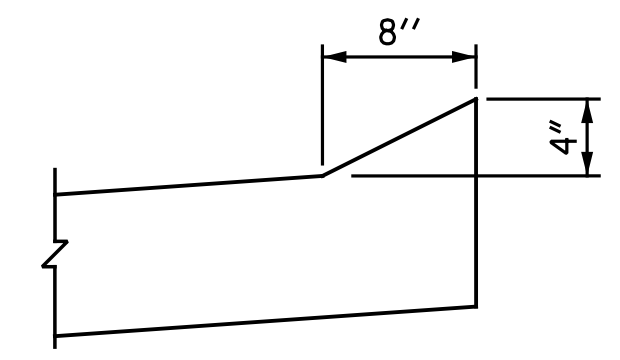
FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

#78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

#78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

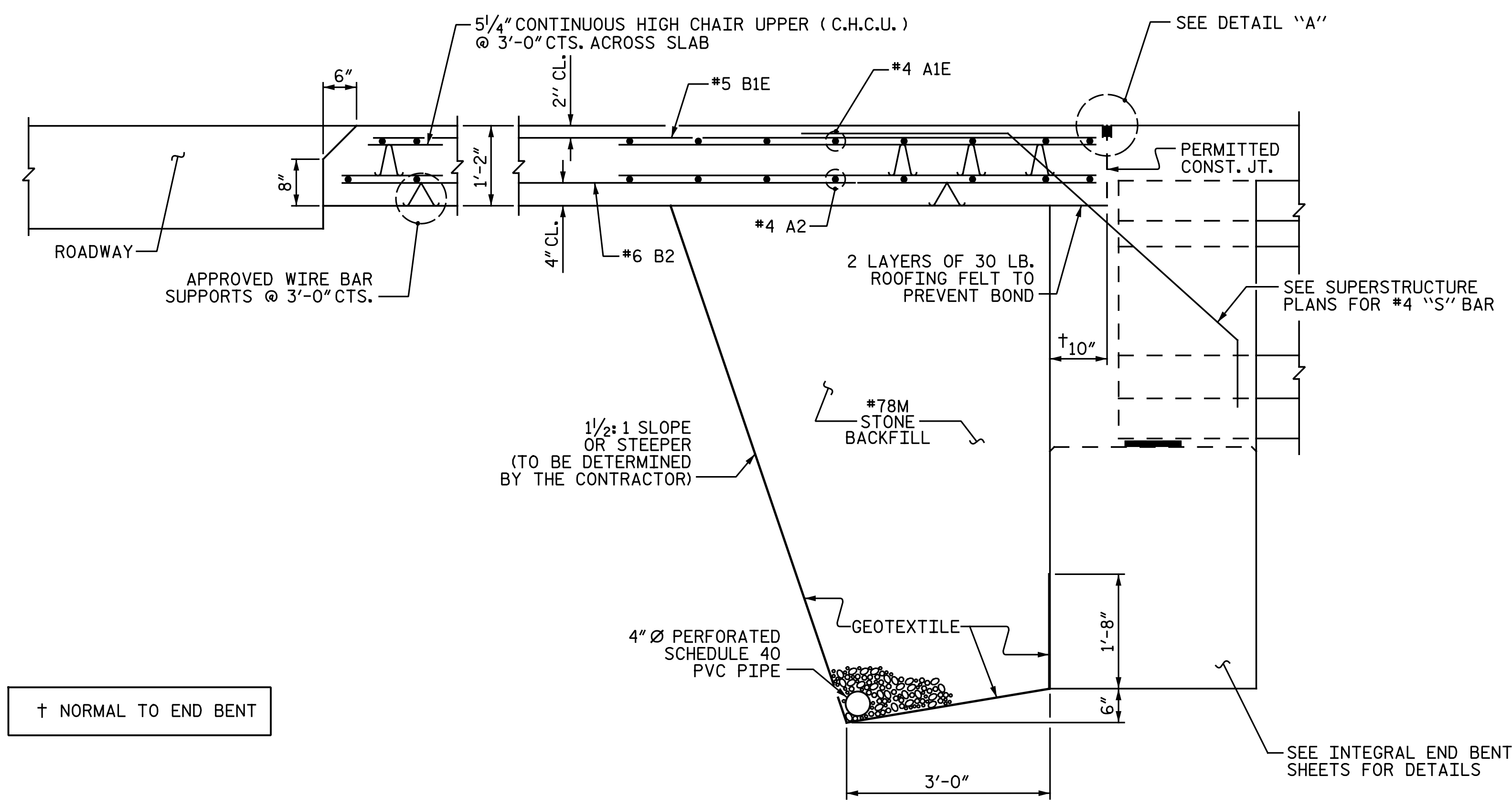


SECTION M-M

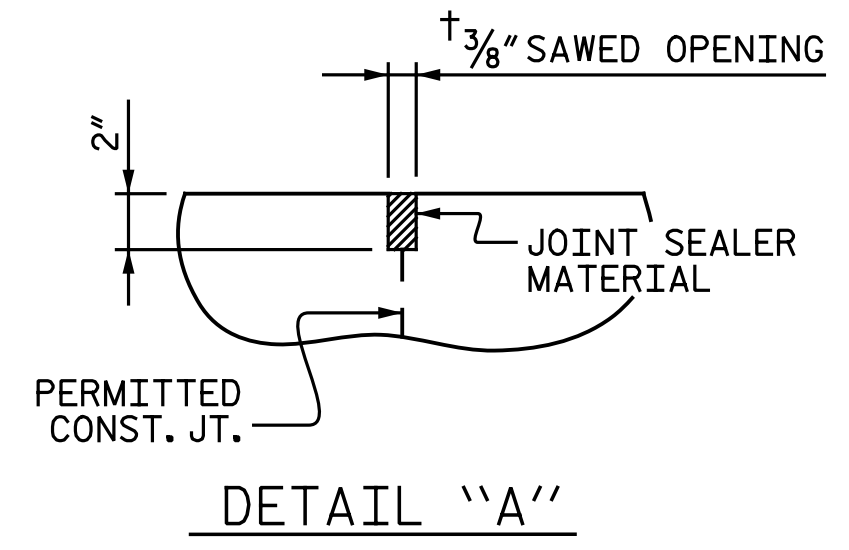
BILL OF MATERIAL					
APPROACH SLAB AT END BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1E	52	#4	STR.	25' - 8"	892
A2	52	#4	STR.	25' - 6"	886
B1E	98	#5	STR.	24' - 2"	2,470
B2	98	#6	STR.	24' - 8"	3,631
REINFORCING STEEL					LBS. 4,517
EPOXY COATED REINFORCING STEEL					LBS. 3,362
CLASS AA CONCRETE					C.Y. 53.3
BILL OF MATERIAL					
APPROACH SLAB AT END BENT 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1E	52	#4	STR.	25' - 8"	892
A2	52	#4	STR.	25' - 6"	886
B1E	98	#5	STR.	24' - 2"	2,470
B2	98	#6	STR.	24' - 8"	3,631
REINFORCING STEEL					LBS. 4,517
EPOXY COATED REINFORCING STEEL					LBS. 3,362
CLASS AA CONCRETE					C.Y. 53.3

SPLICE LENGTHS

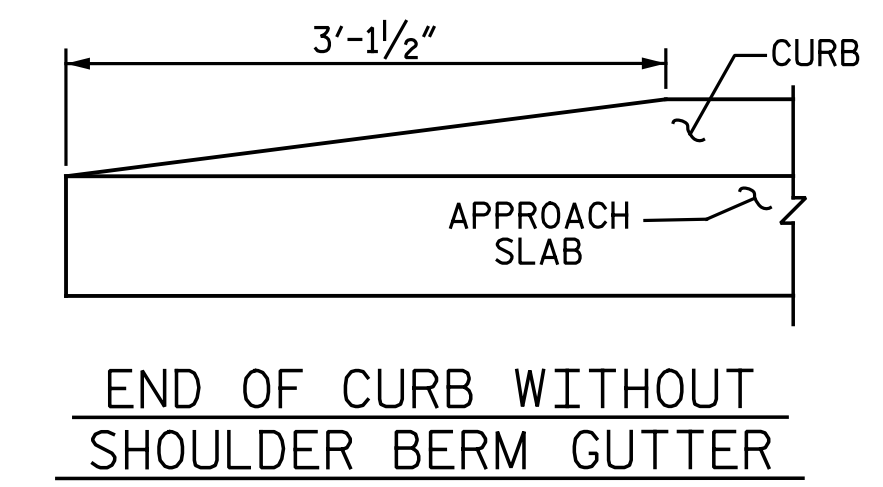
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"



SECTION THRU SLAB

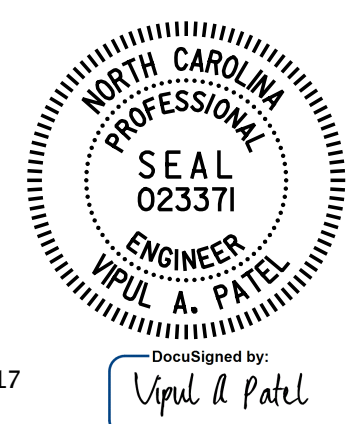


DETAIL "A"



END OF CURB WITHOUT SHOULDER BERM GUTTER

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT

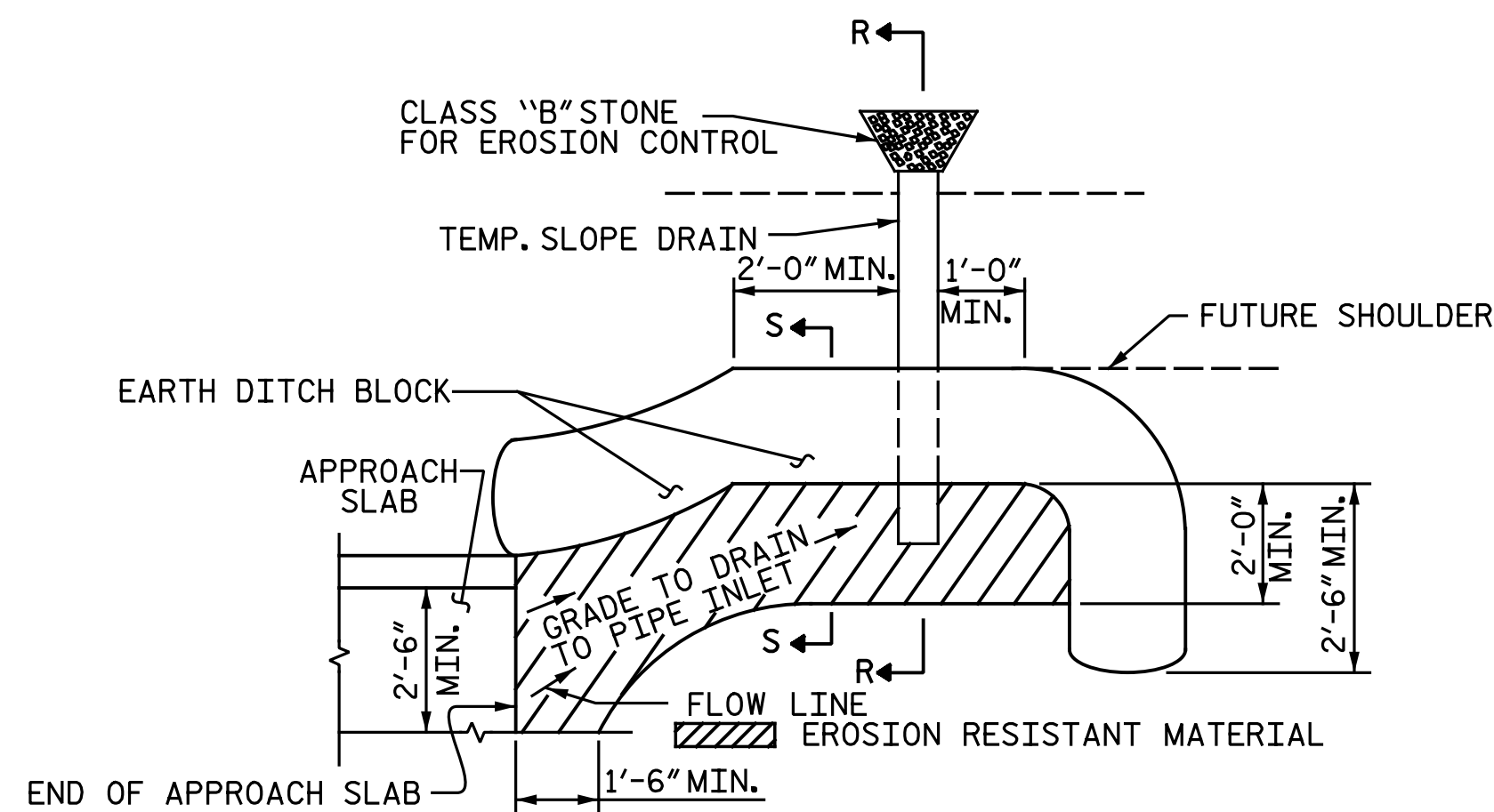
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



LEFT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

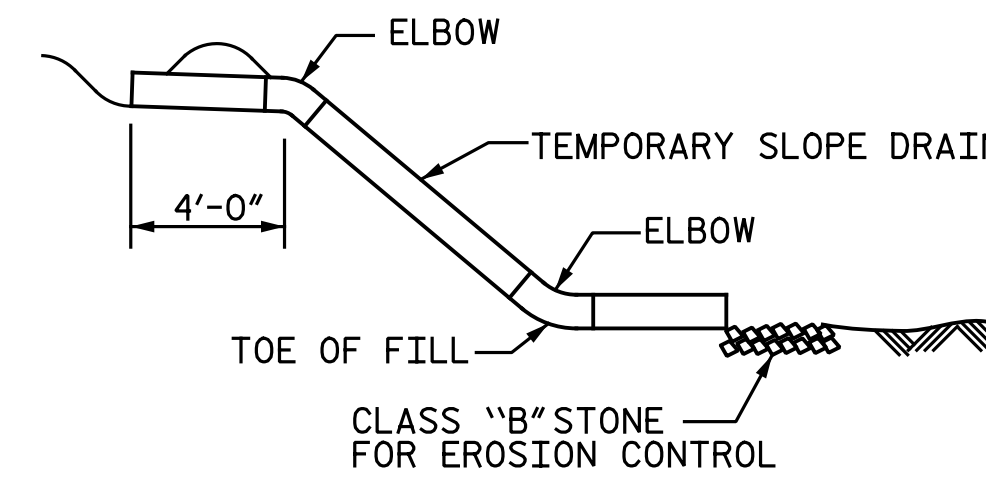
DRAWN BY: N. B. SPEAKS DATE: 3-1-17
 CHECKED BY: V. A. PATEL DATE: 5-23-17

SHEET NO. SI-24
 TOTAL SHEETS 25

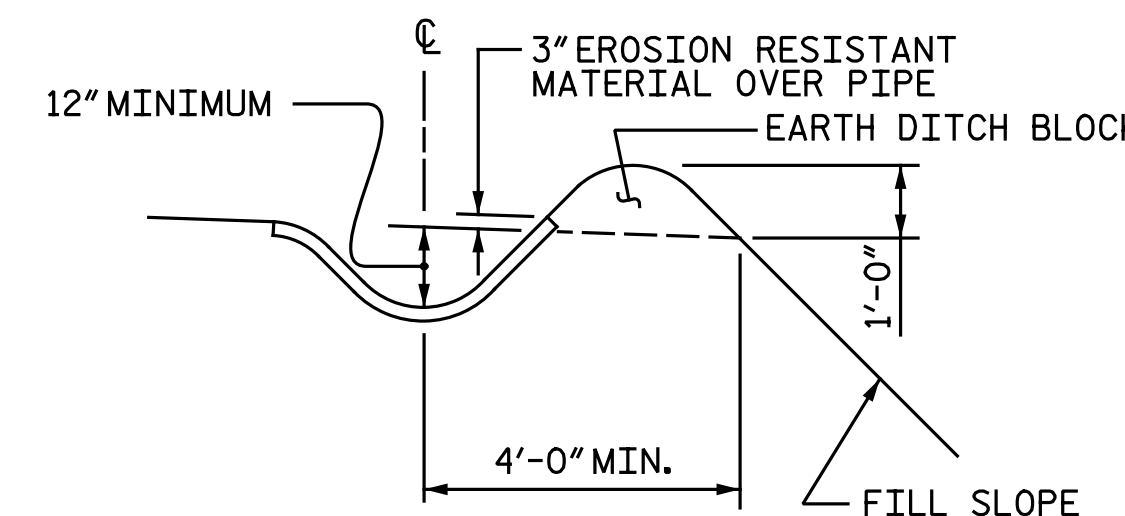


NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

PLAN VIEW



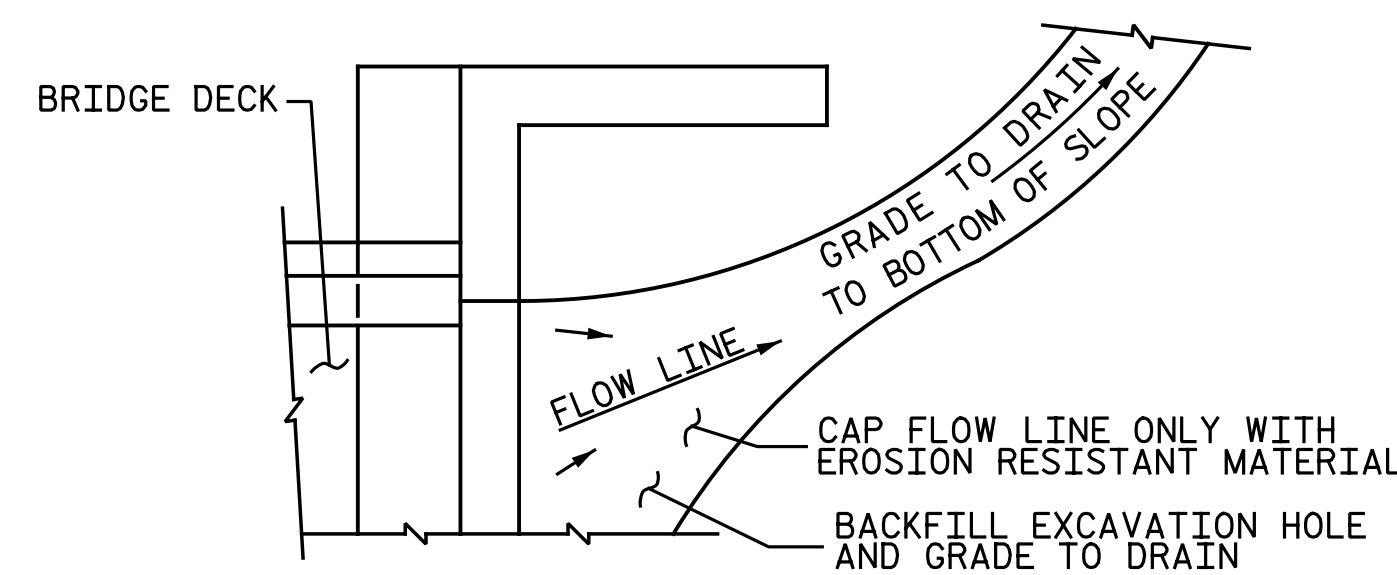
SECTION R-R



SECTION S-S

TEMPORARY BERM AND SLOPE DRAIN DETAILS

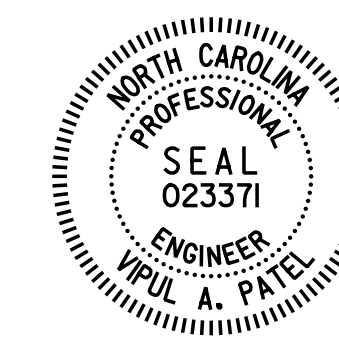
(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



7/26/2017

DocuSigned by:
Vipul A. Patel

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 BRIDGE APPROACH SLAB
 DETAILS

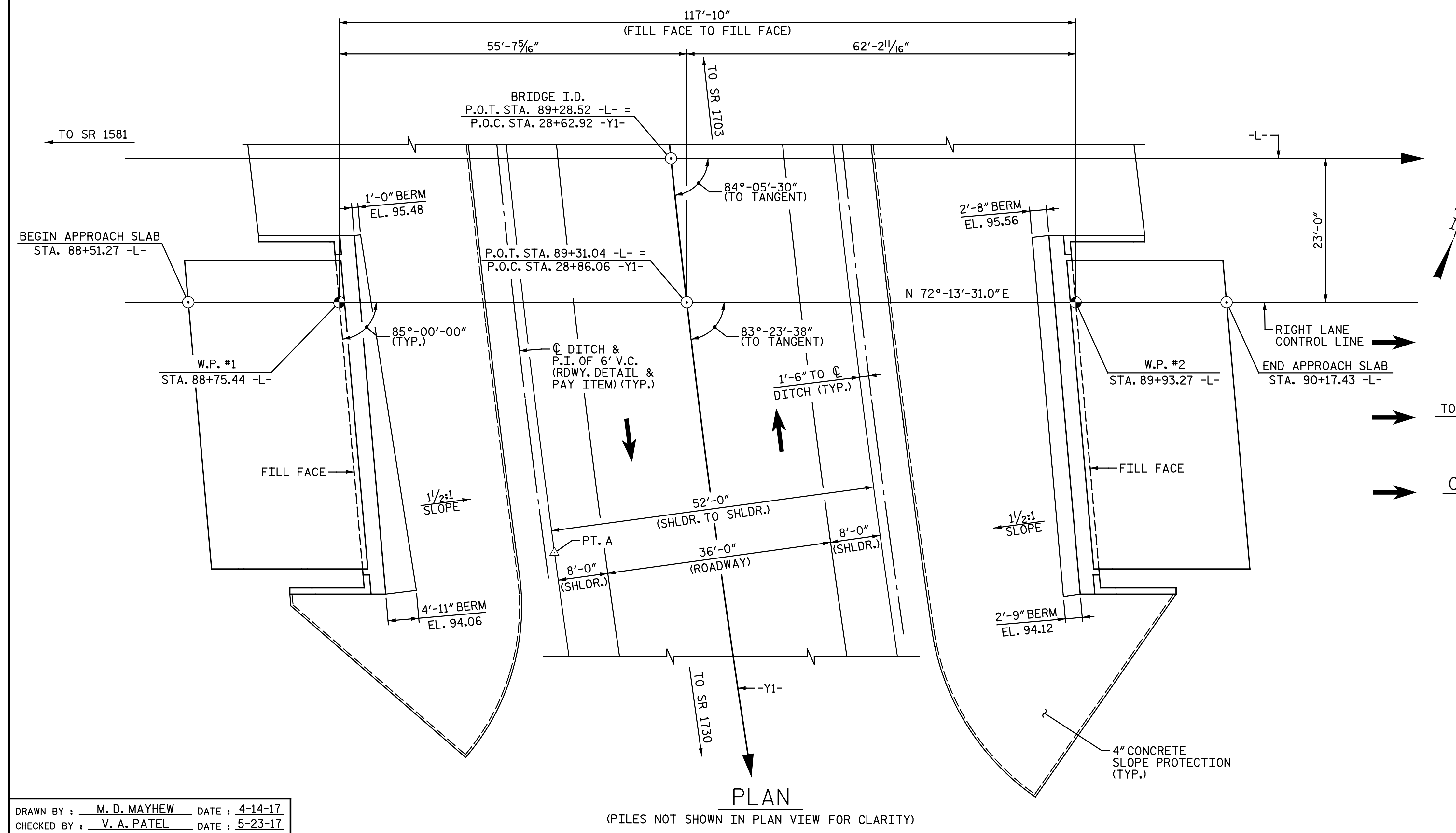
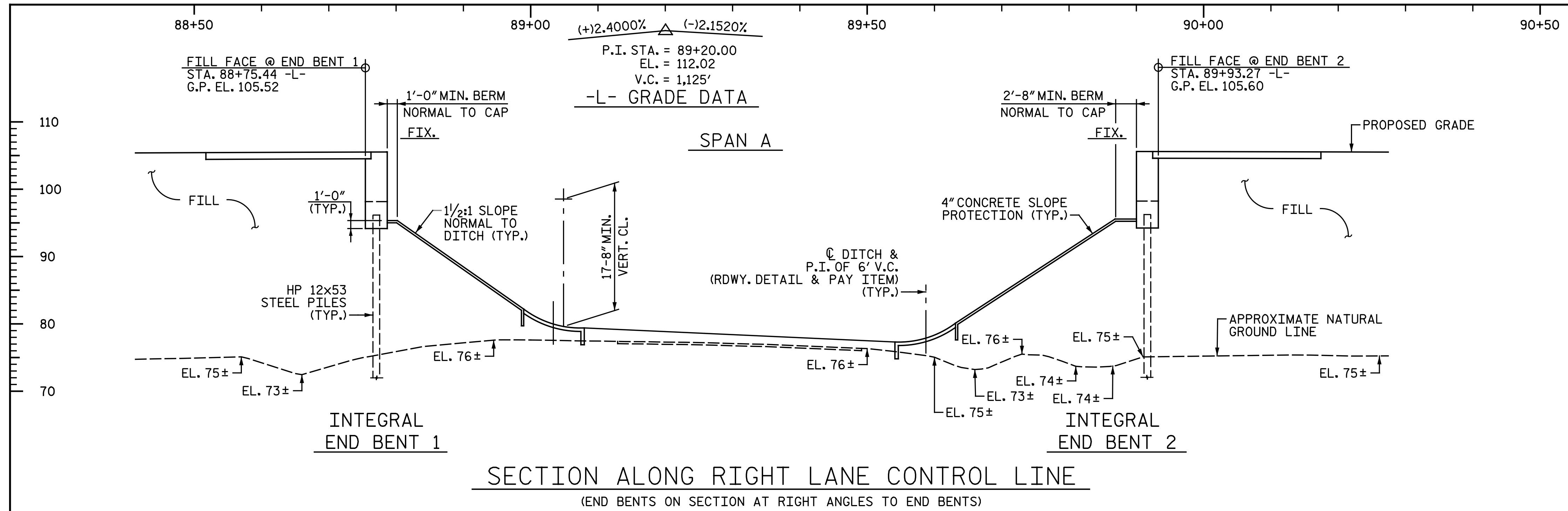
DOCUMENT NOT CONSIDERED FINAL
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LEFT LANE

ASSEMBLED BY : N. B. SPEAKS	DATE : 3-1-17
CHECKED BY : V. A. PATEL	DATE : 3-20-17
DRAWN BY : FCJ 11/88	REV. 10/11/11 MAA/GM
CHECKED BY : ARB 11/88	REV. 7/12 MAA/GM
	REV. 6/13 MAA/GM

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	SI-25
1			3			TOTAL SHEETS
2			4			25

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 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No. : F-1084



POINT	STATION ON -Y1-	OFFSET	ELEVATION ON -Y1-
A	29+22.94	26.0 RT	79.88

△ - POINT OF MINIMUM VERTICAL CLEARANCE.

HORIZONTAL CURVE DATA -Y1-
 PI STA. 30+62.34
 $\Delta = 14^\circ-24'-41.5''$ (LT)
 D = 3°-00'-56.0"
 L = 477.90'
 T = 240.22'
 R = 1,900.00'
 SE = 0.06

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-
28+62.92 -Y1-
 SHEET 1 OF 3 BRIDGE NO. 209



7/26/2017
 Digitally signed by V. A. Patel

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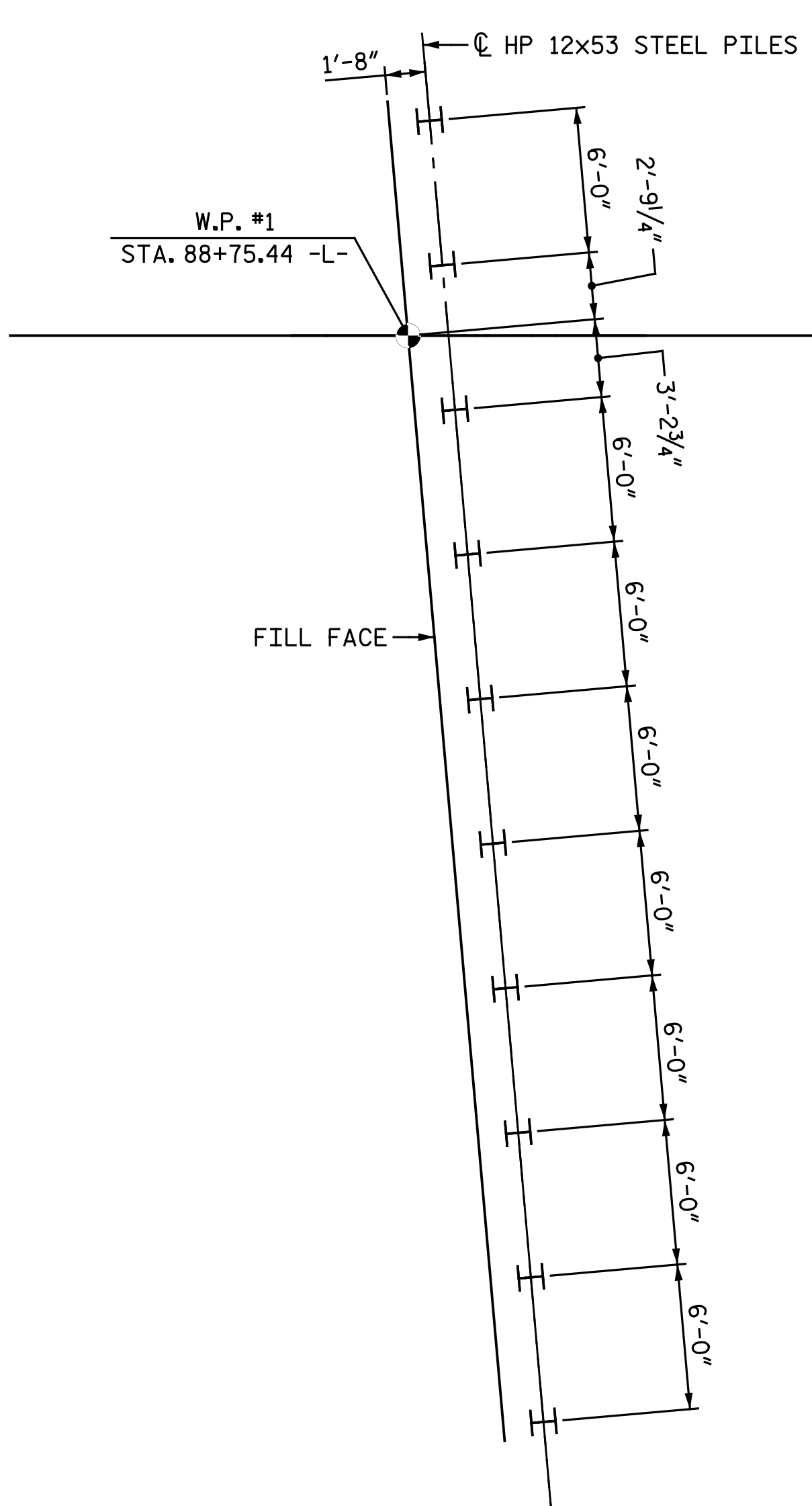
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER NC 58
 BETWEEN SR 1581 AND SR 1004

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-1
1			3			TOTAL SHEETS
2			4			25

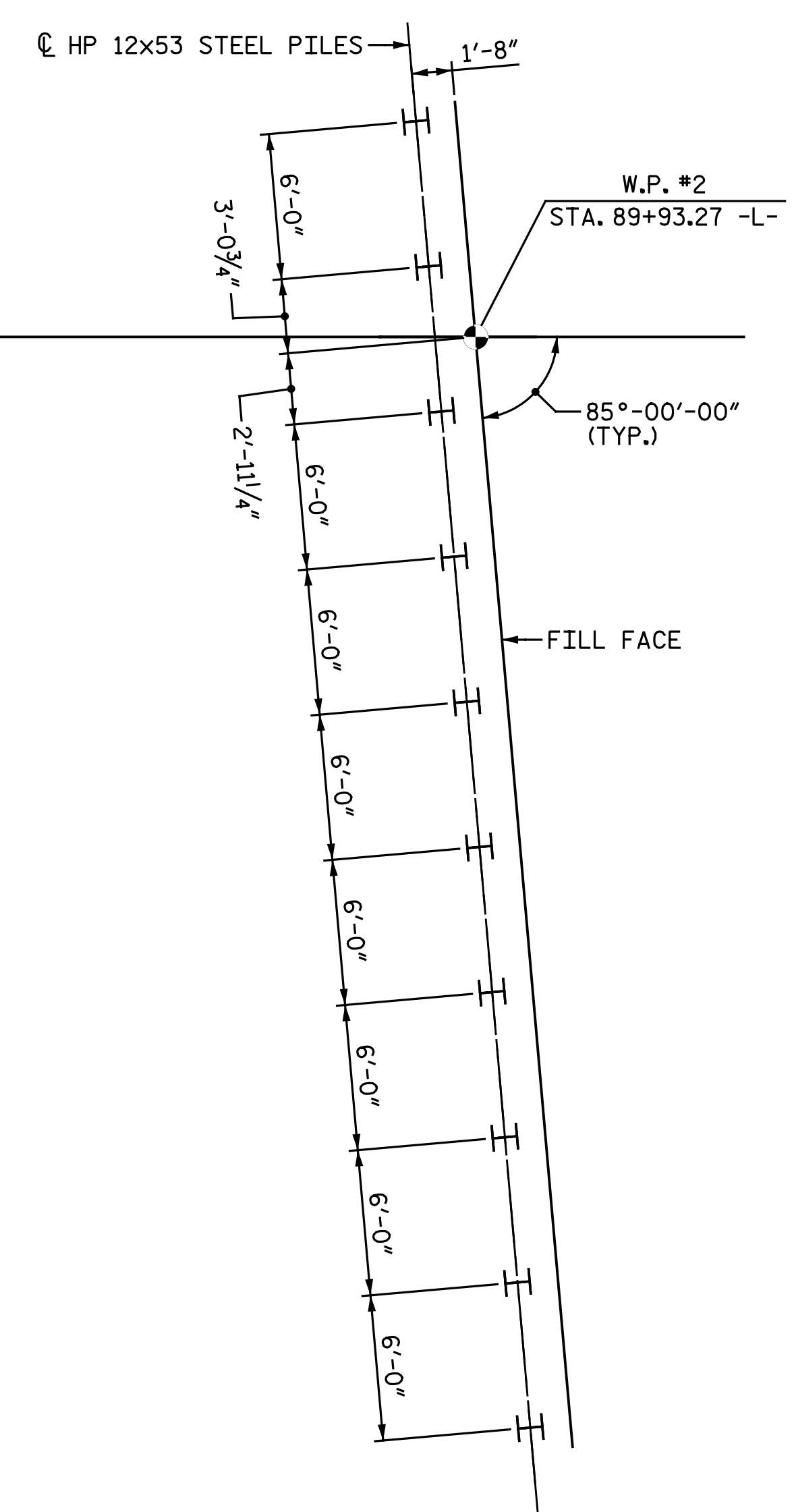
DRAWN BY: M. D. MAYHEW DATE: 4-14-17
 CHECKED BY: V. A. PATEL DATE: 5-23-17

PLAN
 (PILES NOT SHOWN IN PLAN VIEW FOR CLARITY)

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 NC License No.: F-1084



INTEGRAL
END BENT 1



INTEGRAL
END BENT 2

FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINES.
ALL PILES ARE VERTICAL.

NOTES:

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 AND END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 122.5 TONS PER PILE.

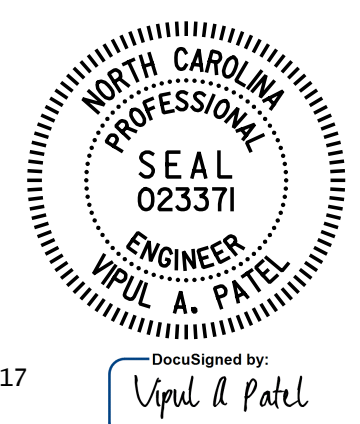
DRIVE PILES AT END BENT NO.1 AND END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 205 TONS PER PILE.

TESTING THE FIRST PRODUCTION PILE WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED AT END BENT NO.1 OR END BENT NO.2. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS AND FOR PILE DRIVING CRITERIA, SEE PILE DRIVING CRITERIA PROVISION.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 40-50 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT NO.1 AND END BENT NO.2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3D(X2) OF THE STANDARD SPECIFICATIONS.

OBSERVE A 2 MONTH WAITING PERIOD AFTER CONSTRUCTING THE EMBANKMENT TO WITHIN 2 FT OF FINISHED GRADE BEFORE BEGINNING END BENT CONSTRUCTION AT END BENT NO.1 AND END BENT NO.2. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SPECIAL PROVISIONS.

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-
28+62.92 -Y1-
SHEET 2 OF 3



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
GENERAL DRAWING
FOR BRIDGE ON C.F. HARVEY
PARKWAY OVER NC 58
BETWEEN SR 1581 AND SR 1004

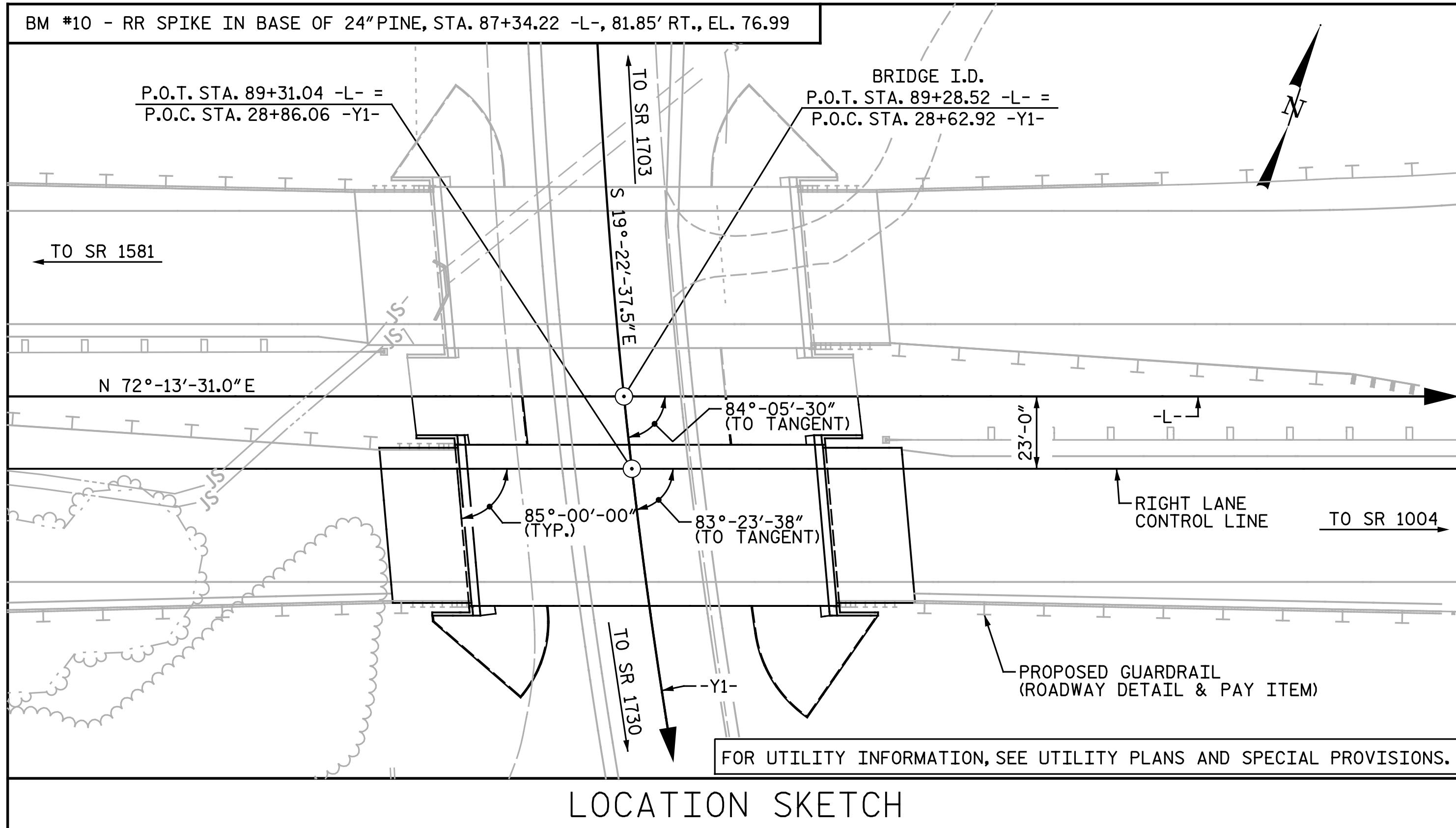
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RIGHT LANE

DRAWN BY : M. D. MAYHEW DATE : 4-17-17
CHECKED BY : V. A. PATEL DATE : 5-23-17

NO.		BY:	DATE:	NO.		BY:	DATE:	SHEET NO.
1				3				TOTAL SHEETS
2				4				25

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NOTES:

- ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
- THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.
- PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
- FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

TOTAL BILL OF MATERIAL

LOCATION	PDA TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	MODIFIED 72" PRESTRESSED CONCRETE GIRDERS		PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES		HP 12x53 STEEL PILES		PILE REDRIVES	CONCRETE BARRIER RAIL	4" SLOPE PROTECTION	ELASTOMERIC BEARINGS
							NO.	LIN. FT.	EA.	NO.	LIN. FT.	EA.				
SUPERSTRUCTURE		6,039	7,387				5	579.48						232.33		LUMP SUM
END BENT 1				42.8		6,893			10	10	800		5		350	
END BENT 2				42.8		6,893			10	10	800		5		430	
TOTAL	1	6,039	7,387	85.6	LUMP SUM	13,786	5	579.48	20	20	1,600		10	232.33	780	LUMP SUM

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-
28+62.92 -Y1-
 SHEET 3 OF 3



DocuSigned by:
 Bradley J. Bell
 8/8/2017 10:33:33 AM

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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER NC 58
 BETWEEN SR 1581 AND SR 1004

RIGHT LANE

REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			S2-3
2			4			TOTAL SHEETS 25

DRAWN BY: M. D. MAYHEW DATE: 4-17-17
 CHECKED BY: V. A. PATEL DATE: 5-23-17

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ_{DC}	γ_{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING #	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					COMMENT NUMBER			
						LIVE-LOAD FACTORS (γ_{LL})	MOMENT					SHEAR					LIVE-LOAD FACTORS (γ_{LL})	MOMENT						
							DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)		DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	①	1.06	--	1.75	0.971	1.41	A	1	57.24	0.984	1.19	A	1	80.42	1.00	0.971	1.06	A	1	57.24	1, 2	
	HL-93 (OPERATING)	N/A		1.74	--	1.35	0.971	1.83	A	1	57.24	1.012	1.74	A	3	92.01	N/A	-	-	-	-	-	-	2
	HS-20 (INVENTORY)	36.000	②	1.53	55.08	1.75	0.971	2.03	A	1	57.24	1.012	1.80	A	3	92.01	1.00	0.971	1.53	A	1	57.24	1, 2	
	HS-20 (OPERATING)	36.000		2.38	85.68	1.35	0.971	2.64	A	1	57.24	1.012	2.38	A	3	92.01	N/A	-	-	-	-	-	-	2
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500		3.70	49.95	1.40	0.971	6.14	A	1	57.24	1.012	5.90	A	3	92.01	1.00	0.971	3.70	A	1	57.24	1, 2
		SNGARBS2	20.000		2.66	53.20	1.40	0.971	4.40	A	1	57.24	1.012	4.08	A	3	92.01	1.00	0.971	2.66	A	1	57.24	1, 2
		SNAGRIS2	22.000		2.47	54.34	1.40	0.971	4.10	A	1	57.24	1.012	3.75	A	3	92.01	1.00	0.971	2.47	A	1	57.24	1, 2
		SNCOTTS3	27.250		1.84	50.14	1.40	0.971	3.05	A	1	57.24	1.012	2.85	A	3	92.01	1.00	0.971	1.84	A	1	57.24	1, 2
		SNAGGRS4	34.925		1.50	52.39	1.40	0.971	2.48	A	1	57.24	1.012	2.29	A	3	92.01	1.00	0.971	1.50	A	1	57.24	1, 2
		SNS5A	35.550		1.47	52.26	1.40	0.971	2.43	A	1	57.24	1.012	2.30	A	3	92.01	1.00	0.971	1.47	A	1	57.24	1, 2
		SNS6A	39.950		1.33	53.13	1.40	0.971	2.20	A	1	57.24	1.012	2.07	A	3	92.01	1.00	0.971	1.33	A	1	57.24	1, 2
		SNS7B	42.000		1.27	53.34	1.40	0.971	2.10	A	1	57.24	1.012	2.00	A	3	92.01	1.00	0.971	1.27	A	1	57.24	1, 2
	TRUCK TRACTOR SEMI-TRAILER (T/S/T)	TNAGRIT3	33.000		1.62	53.46	1.40	0.971	2.68	A	1	57.24	1.012	2.51	A	3	92.01	1.00	0.971	1.62	A	1	57.24	1, 2
		TNT4A	33.075		1.62	53.58	1.40	0.971	2.68	A	1	57.24	1.012	2.46	A	3	92.01	1.00	0.971	1.62	A	1	57.24	1, 2
		TNT6A	41.600		1.31	54.50	1.40	0.971	2.17	A	1	57.24	1.012	2.10	A	3	92.01	1.00	0.971	1.31	A	1	57.24	1, 2
		TNT7A	42.000		1.31	55.02	1.40	0.971	2.16	A	1	57.24	1.012	2.07	A	3	92.01	1.00	0.971	1.31	A	1	57.24	1, 2
		TNT7B	42.000		1.33	55.86	1.40	0.971	2.21	A	1	57.24	1.012	1.97	A	3	92.01	1.00	0.971	1.33	A	1	57.24	1, 2
		TNAGRIT4	43.000		1.28	55.04	1.40	0.971	2.12	A	1	57.24	1.012	1.91	A	3	92.01	1.00	0.971	1.28	A	1	57.24	1, 2
		TNAGT5A	45.000		1.22	54.90	1.40	0.971	2.01	A	1	57.24	1.012	1.87	A	3	92.01	1.00	0.971	1.22	A	1	57.24	1, 2
		TNAGT5B	45.000	③	1.21	54.45	1.40	0.971	2.00	A	1	57.24	1.012	1.81	A	3	92.01	1.00	0.971	1.21	A	1	57.24	1, 2

NOTES:
MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:
1. A SERVICE III LIVE LOAD FACTOR OF 1.0 WAS USED TO BE CONSISTENT WITH THE VALUE USED DURING DESIGN.
2. DISTANCE FROM LEFT END OF SPAN IS GIVEN WITH RESPECT TO CENTERLINE OF BEARING AND IS MEASURED ALONG THE CONTROLLING GIRDER.

CONTROLLING LOAD RATING

① DESIGN LOAD RATING (HL-93)

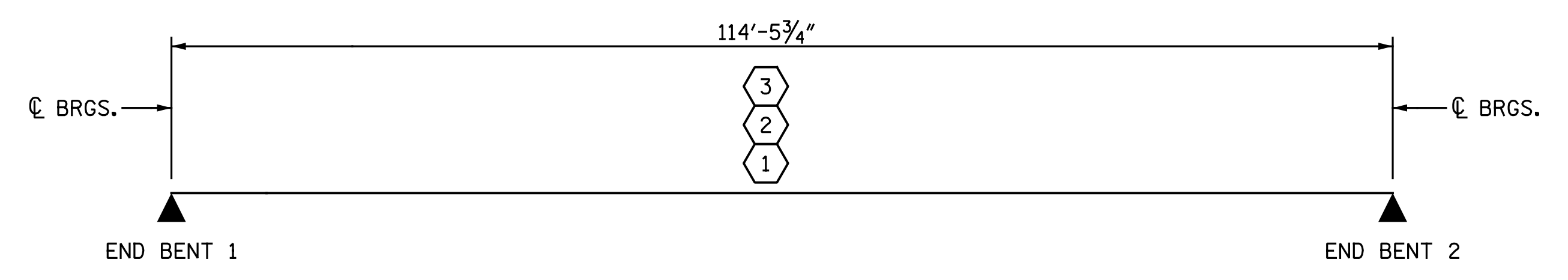
② DESIGN LOAD RATING (HS-20)

③ LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

GIRDER LOCATION IS PROVIDED USING GIRDER NUMBER, WHERE GIRDER 1 IS THE LEFT EXTERIOR GIRDER LOOKING AHEAD STATION. SEE "GIRDER LAYOUT" SHEET FOR ALL GIRDER LOCATIONS.



LRFR SUMMARY

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-

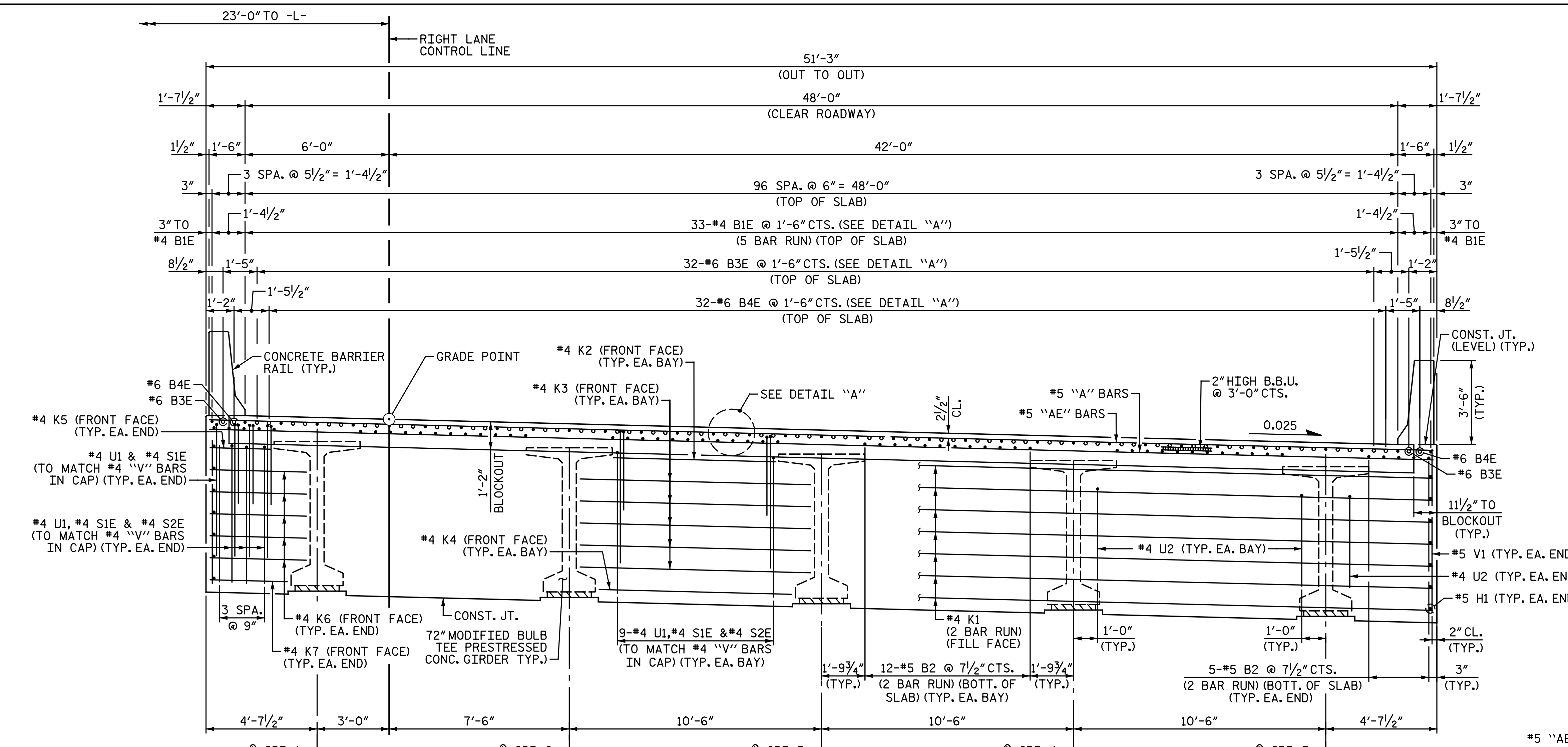


STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
LRFR SUMMARY FOR
PRESTRESSED
CONCRETE GIRDERS
(NON-INTERSTATE TRAFFIC)
RIGHT LANE

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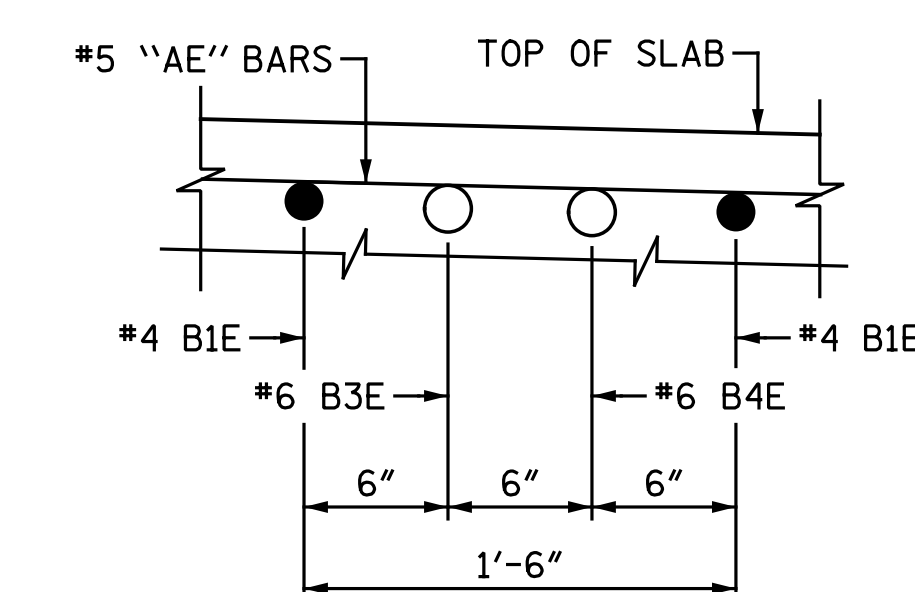
Michael Baker INTERNATIONAL	Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 NC License No.: F-1084		REVISIONS			SHEET NO. S2-4
	NO.	BY:	DATE:	NO.	BY:	DATE:
1				3		
2				4		
						TOTAL SHEETS 25

ASSEMBLED BY : M. D. MAYHEW	DATE : 5-11-17
CHECKED BY : V. A. PATEL	DATE : 5-18-17
DRAWN BY : MAA 1/08	REV. 11/2/08RR MAA/GM
CHECKED BY : GM/DI 2/08	REV. 10/1/11 MAA/GM

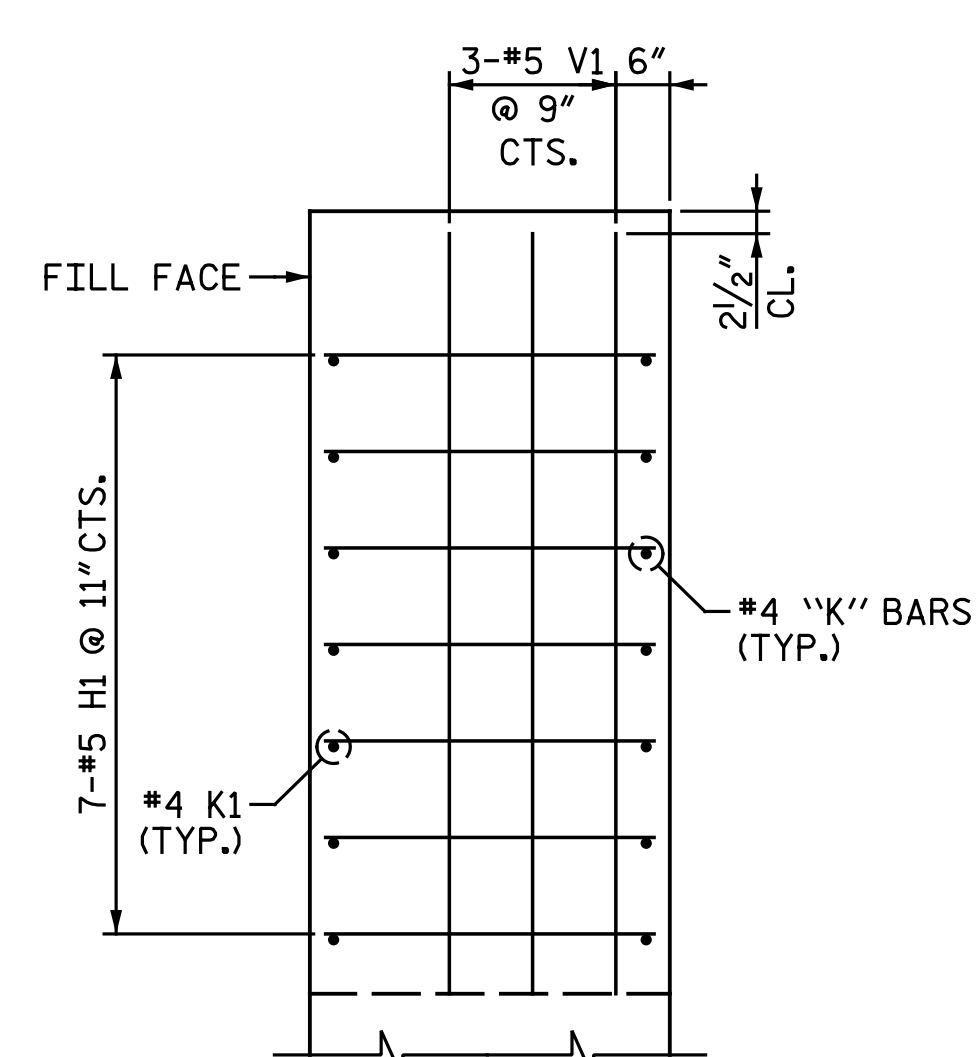


TYPICAL SECTION AT INTEGRAL END BENT

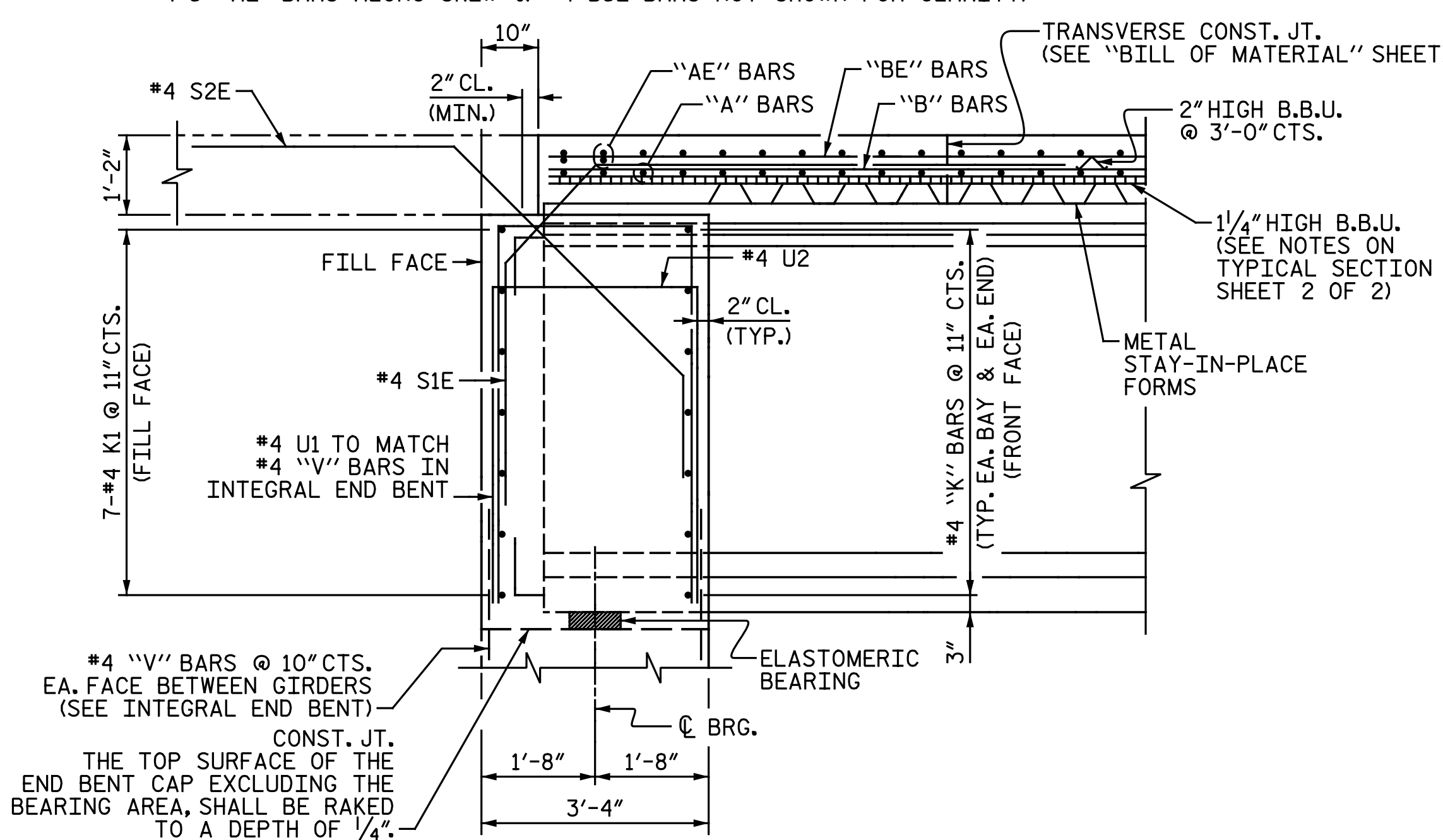
(END BENT 1 SHOWN, END BENT 2 SIMILAR)
 (#5 "AE" BARS ALONG SKEW & #4 B5E BARS NOT SHOWN FOR CLARITY)



DETAIL "A"



END OF DIAPHRAGM DETAIL
 (END BENT 1 SHOWN, END BENT 2 SIMILAR)



END OF GIRDER DETAIL
 AT INTEGRAL END BENT

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-

SHEET 1 OF 2



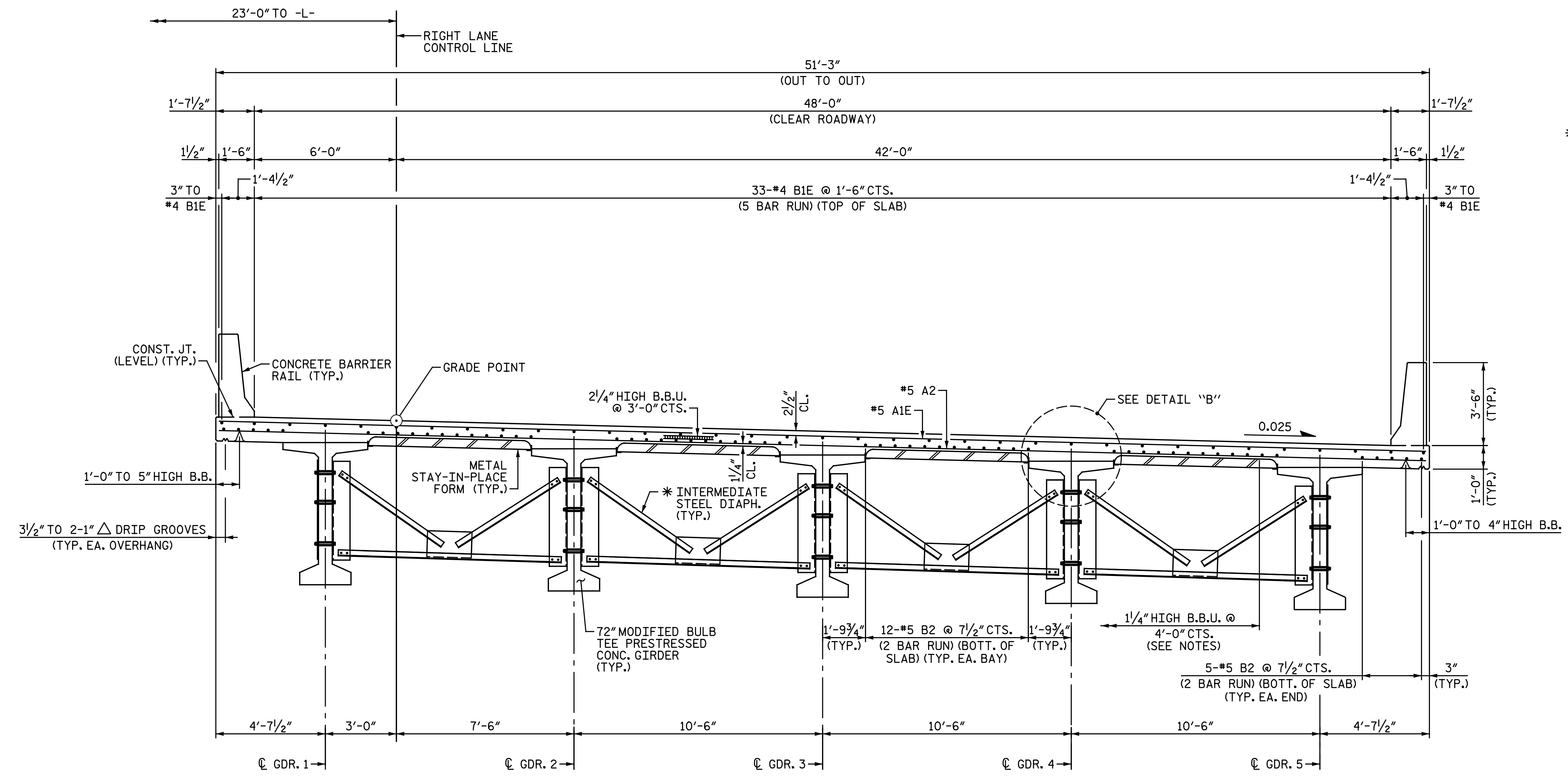
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 TYPICAL SECTION

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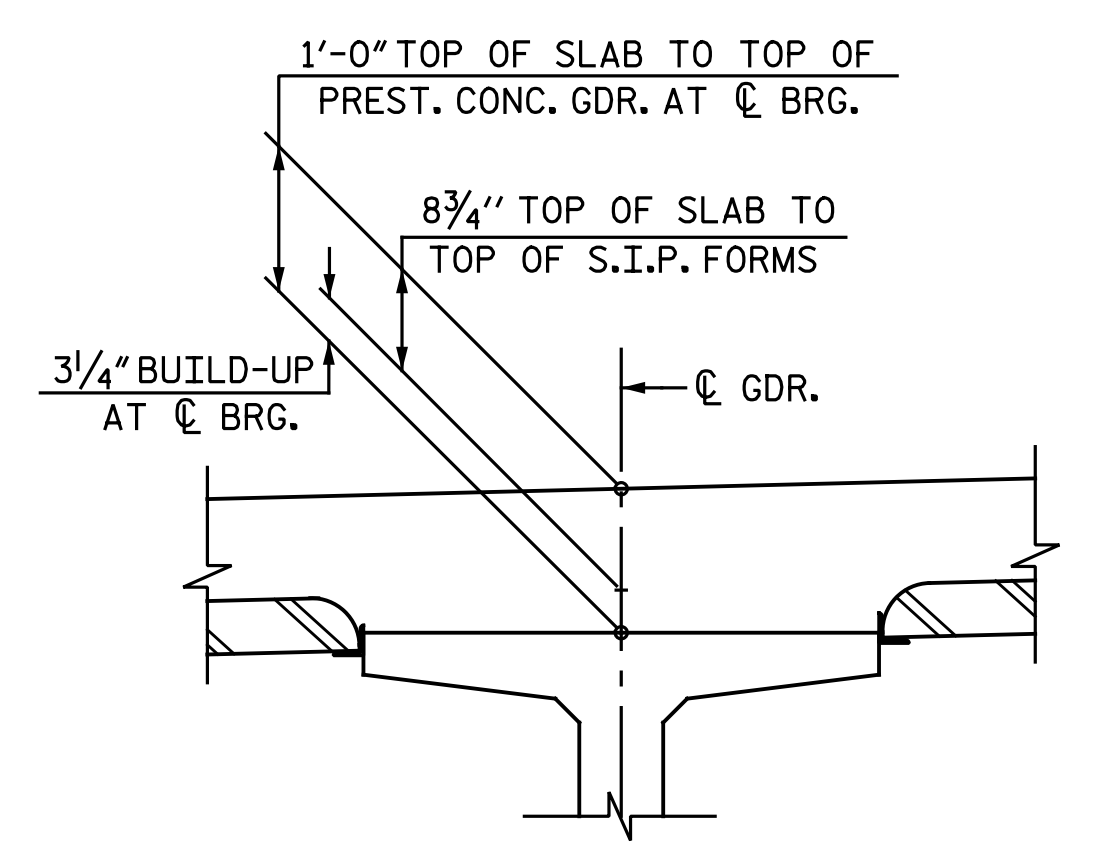
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NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 25
2			4			

DRAWN BY: M. D. MAYHEW DATE: 2-21-17
 CHECKED BY: V. A. PATEL DATE: 3-13-17



TYPICAL SECTION @ INTERMEDIATE DIAPHRAGM

NOTES:
 PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.
 LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS AND TO FACILITATE INSTALLATION OF CONCRETE BARRIER RAIL REINFORCEMENT.
 FOR CONCRETE BARRIER RAIL DETAILS, SEE "CONCRETE BARRIER RAIL" SHEET.
 PREVIOUSLY CAST CONCRETE IN A SPAN SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE SPAN.
 * FOR DETAILS OF INTERMEDIATE DIAPHRAGMS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 72" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS" SHEET.



DETAIL "B"

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 TYPICAL SECTION
 RIGHT LANE

REVISIONS						SHEET NO. S2-6
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2			4			

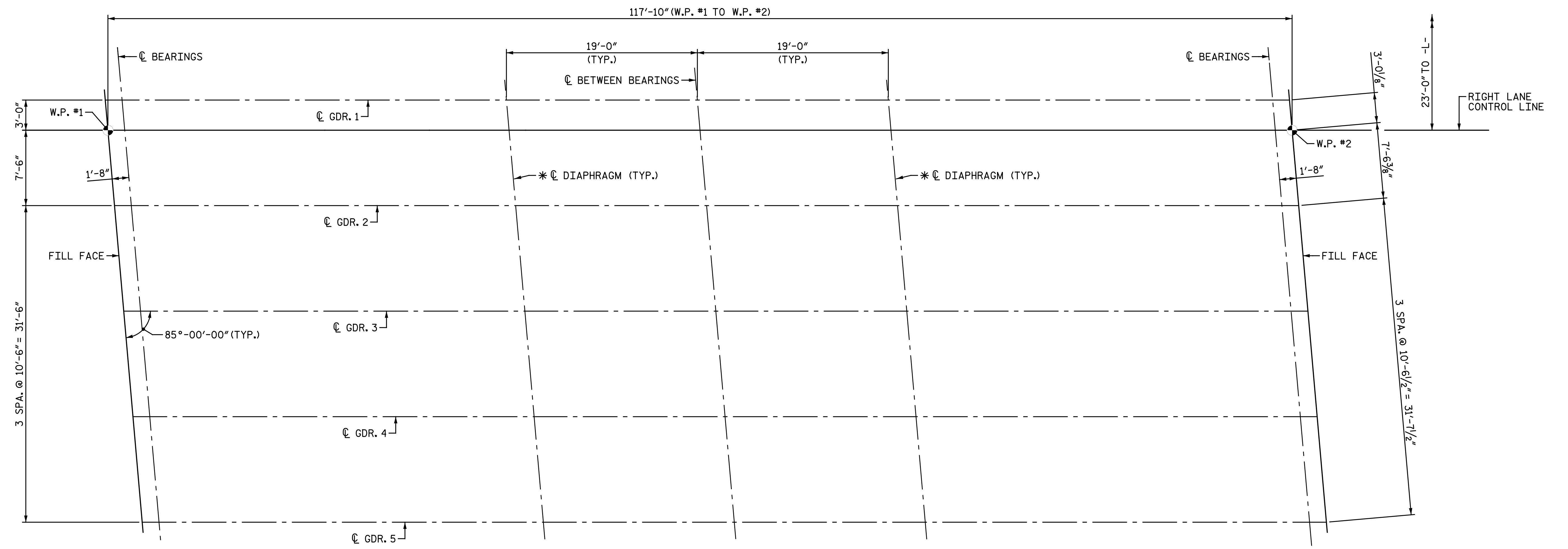


7/26/2017
 Documented by:
 Vipul A. Patel

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 NC License No.: F-1084

DRAWN BY: M. D. MAYHEW DATE: 2-21-17
 CHECKED BY: V. A. PATEL DATE: 3-13-17



FIX.
(E1)

FIX.
(E1)

SPAN A

GIRDER LAYOUT

* INTERMEDIATE STEEL DIAPHRAGMS NOT SHOWN FOR CLARITY

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



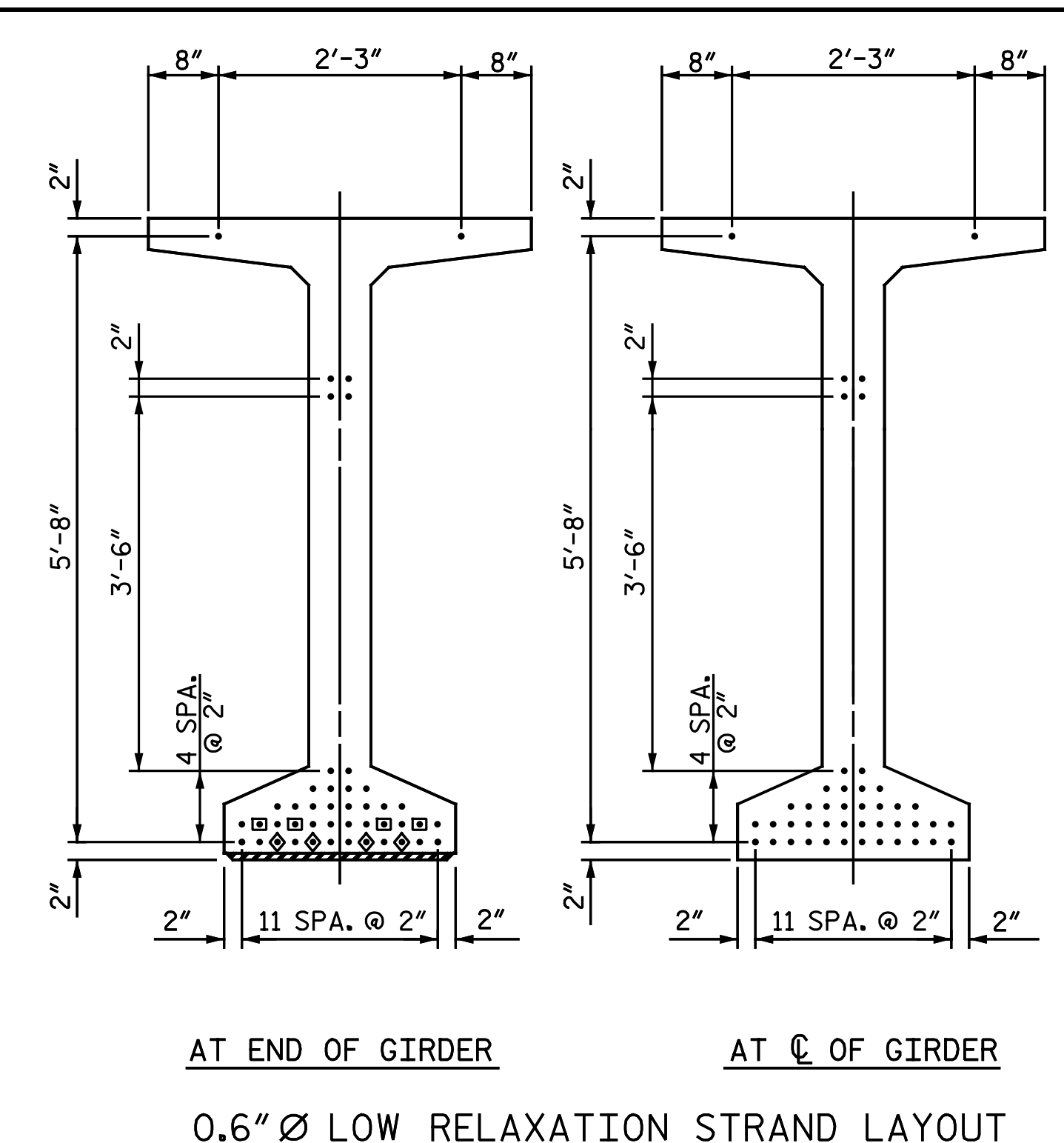
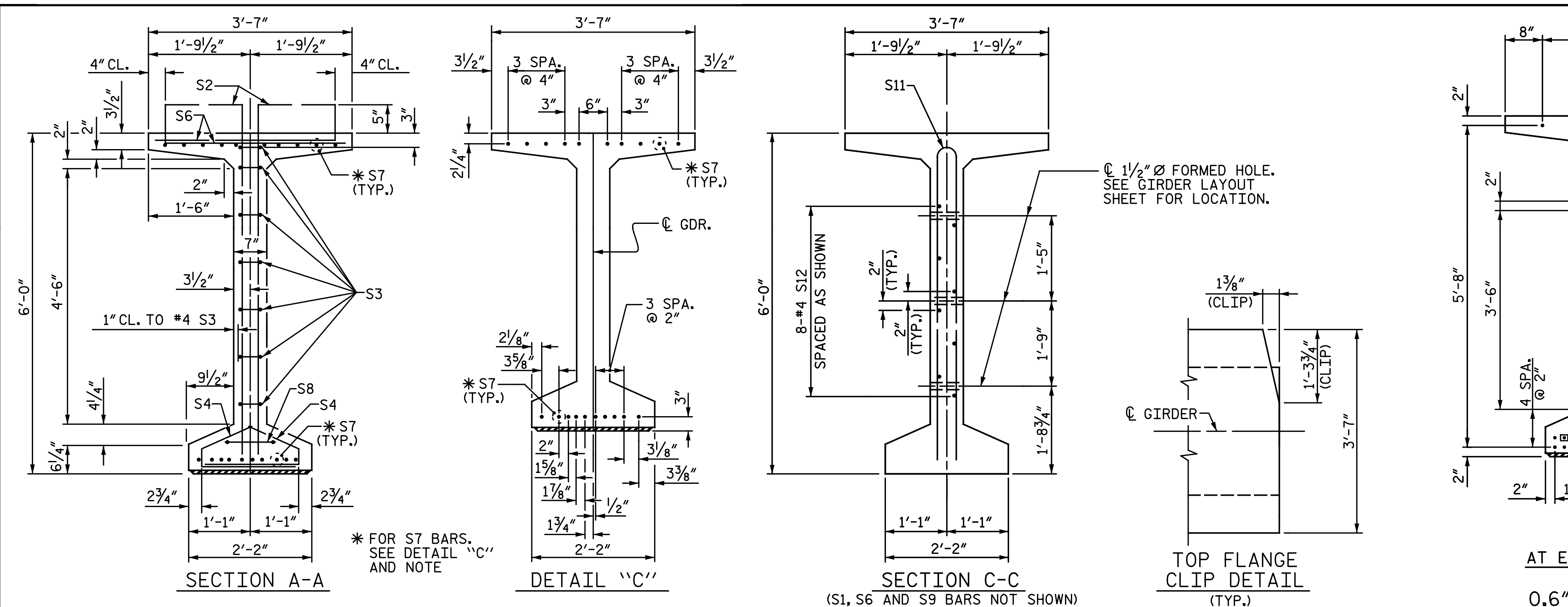
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 GIRDER LAYOUT
 RIGHT LANE

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1			3			TOTAL SHEETS
2			4			25

DRAWN BY : M. D. MAYHEW DATE : 2-21-17
 CHECKED BY : V. A. PATEL DATE : 3-13-17

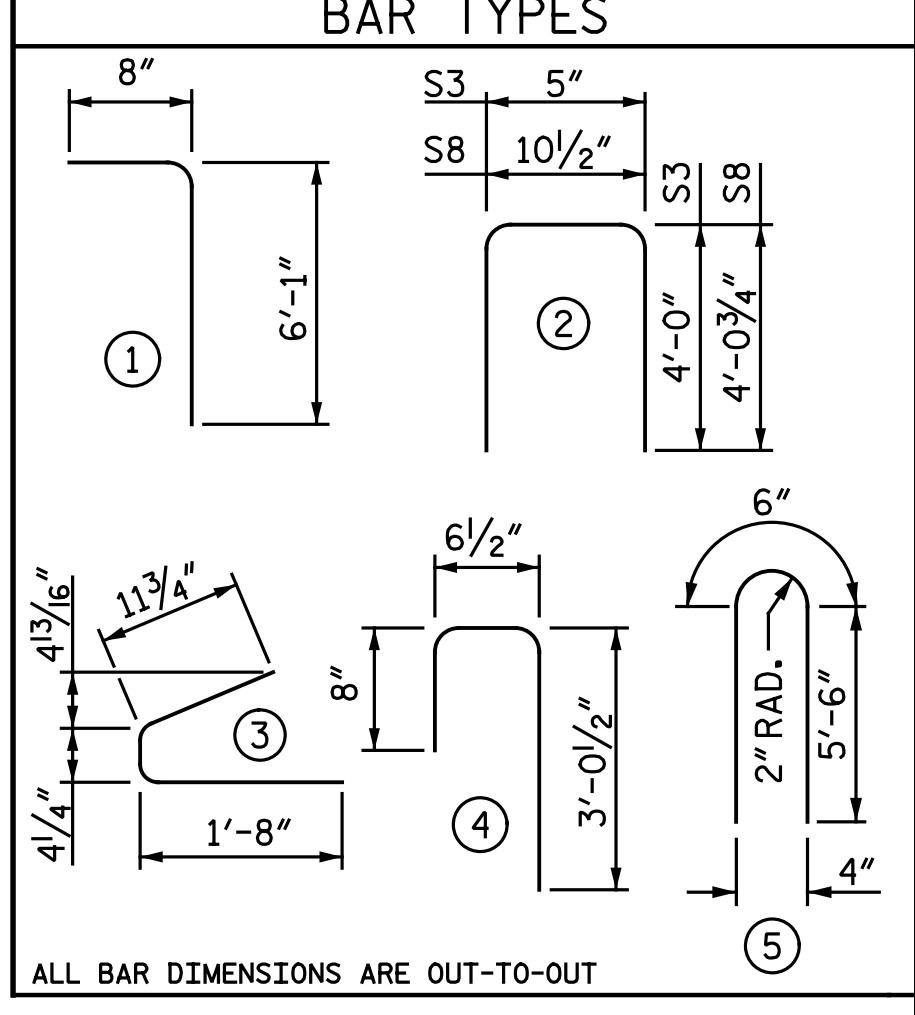
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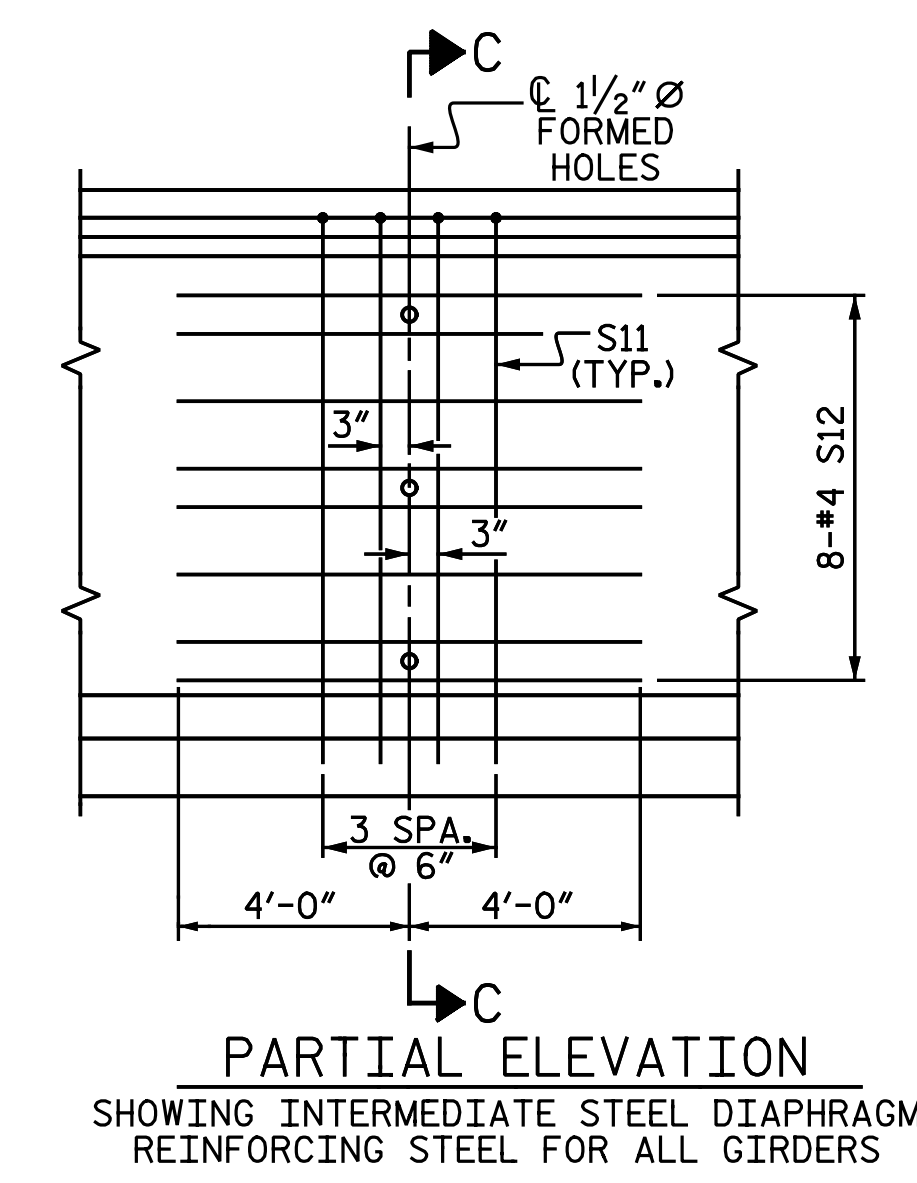
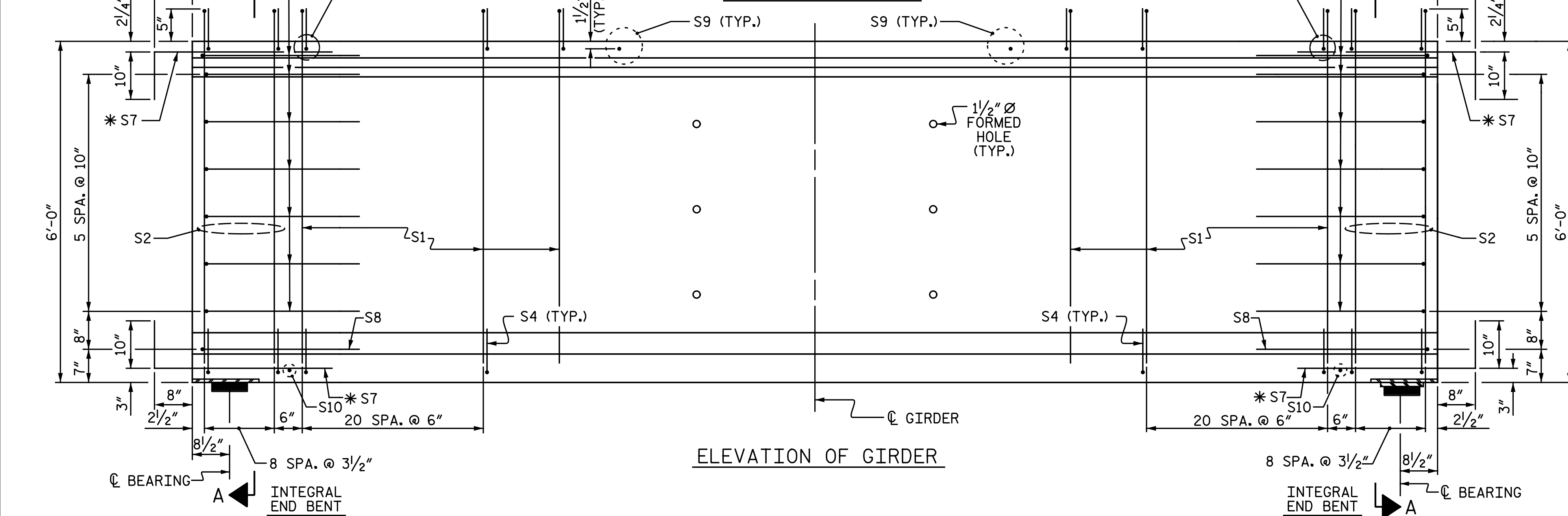
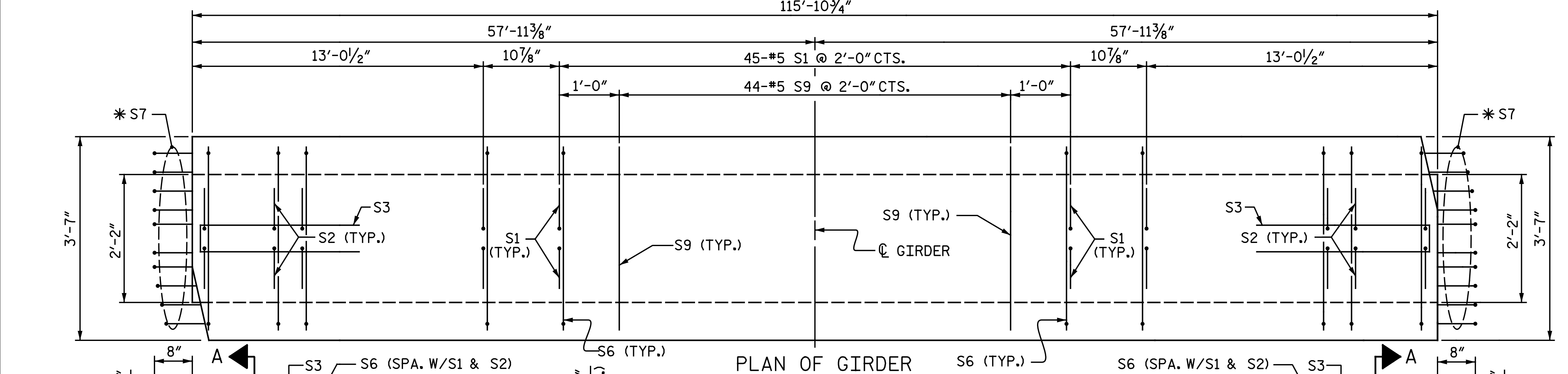
0.6" Ø L. R. GRADE 270 STRANDS		
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	174	#4	1	6'-9"	785
S2	36	#5	1	6'-9"	253
S3	14	#4	2	8'-5"	79
S4	120	#4	3	3'-0"	240
S6	210	#5	4	4'-3"	931
*S7	40	#5	STR	3'-8"	153
S8	2	#5	2	9'-0"	19
S9	44	#5	STR	3'-3"	149
S10	2	#3	STR	1'-10"	1
S11	8	#5	5	11'-6"	96
S12	16	#4	STR	8'-0"	86

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



- DEBONDING LEGEND**
- FULLY BONDED STRANDS
 - ◻ STRANDS DEBONDED FOR 4'-0" FROM END OF GIRDER
 - ◊ STRANDS DEBONDED FOR 10'-0" FROM END OF GIRDER



QUANTITIES FOR ONE GIRDER			
	REINFORCING STEEL	9,500 PSI CONCRETE	0.6" Ø L.R. STRANDS
	LB.	C.Y.	No.
GIRDER	2,792	24.8	44

GIRDERS REQUIRED		
NUMBER	LENGTH	TOTAL LENGTH
5	115'-10 3/4"	579.48'

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-



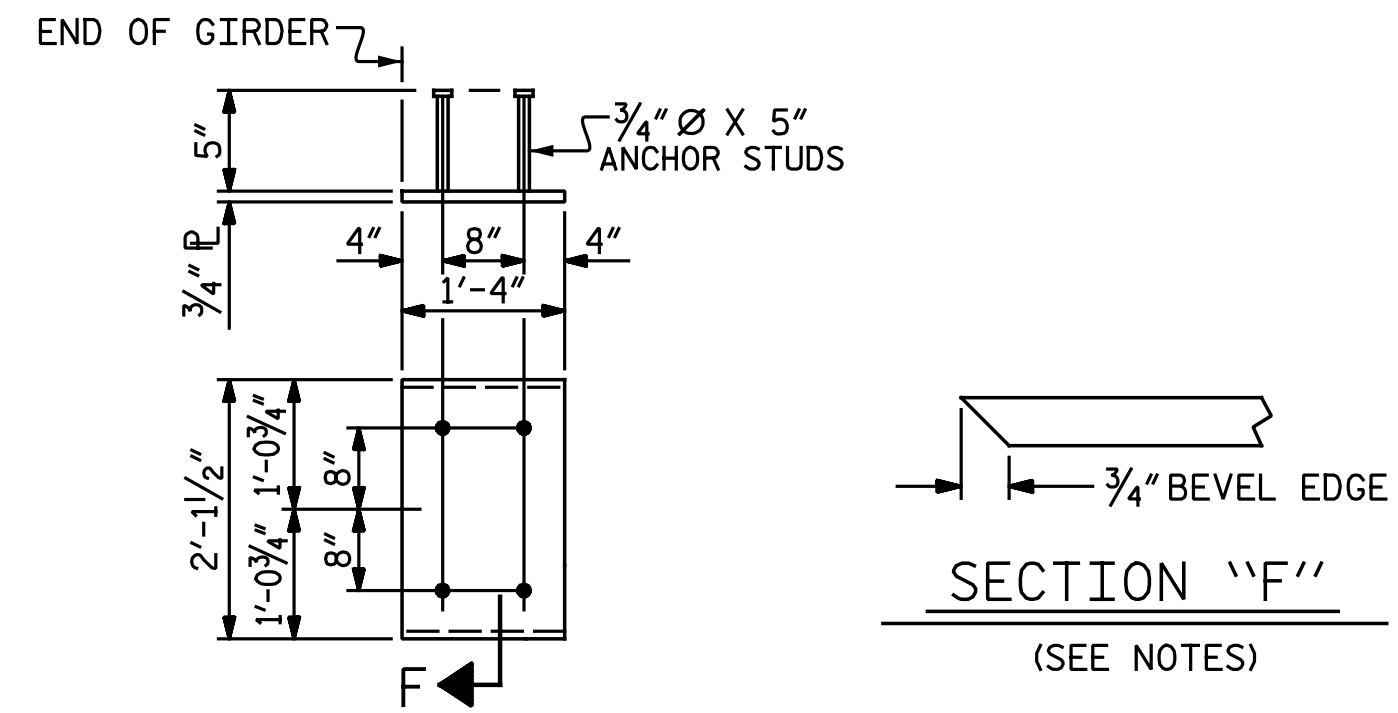
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Michael Baker Engineering
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Cary, North Carolina 27518
NC License No.: F-1084

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
STANDARD 72" PRESTRESSED CONCRETE MODIFIED BULB TEE					
RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO.	
S2-9	TOTAL SHEETS 25

ASSEMBLED BY: V. A. PATEL DATE: 3/7/2017
CHECKED BY: D. A. COLETTI DATE: 4/17/2017
DRAWN BY: EEM 2/6/97 REV. 10/1/11 MAA/GM
CHECKED BY: VAP 2/6/97 REV. 6/13 MAA/GM
REV. 1/15 MAA/TMG



EMBEDDED PLATE "B-1" DETAILS
TWO EMBEDDED PLATES "B-1" ARE REQUIRED FOR EACH GIRDER.

NOTES:

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 7,500 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

A 2" x 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 63" AND 72" MODIFIED BULB TEES ONLY.

THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD OF 4,500 LBS.

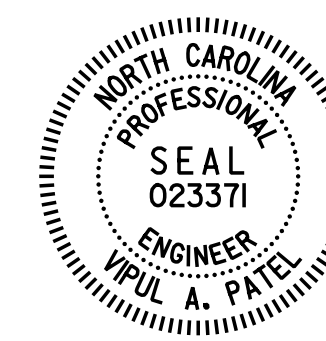
FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.

DEAD LOAD DEFLECTION TABLE FOR SPAN A

0.6 Ø LOW RELAXATION	GIRDERS 1 THRU 5																				
	0.00	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	1.00
TWENTIETH POINTS	0.00	0.043	0.084	0.123	0.159	0.191	0.217	0.239	0.255	0.264	0.267	0.264	0.255	0.239	0.217	0.191	0.159	0.123	0.084	0.043	0.000
CAMBER (GIRDER IN PLACE) ↑	0.000	0.043	0.084	0.123	0.159	0.191	0.217	0.239	0.255	0.264	0.267	0.264	0.255	0.239	0.217	0.191	0.159	0.123	0.084	0.043	0.000
DEFLECTION DUE TO D.L. * ↓	0.000	0.026	0.048	0.074	0.094	0.115	0.130	0.144	0.153	0.159	0.161	0.159	0.153	0.144	0.130	0.115	0.094	0.074	0.048	0.026	0.000
FINAL CAMBER ↑	0"	3/16"	3/8"	9/16"	3/4"	7/8"	1"	1 1/8"	1 3/16"	1 1/4"	1 1/4"	1 1/4"	1 3/16"	1 1/8"	1"	7/8"	3/4"	9/16"	3/8"	3/16"	0"

* INCLUDES FUTURE WEARING SURFACE.
ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "FINAL CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM).

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-



7/26/2017

Vipul A. Patel
7/26/2017

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

GIRDER DEFLECTIONS AND CAMBER

RIGHT LANE

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NO.	BY:	DATE:	NO.	BY:	DATE:	S2-10
1			3			TOTAL SHEETS
2			4			25

DRAWN BY : N. B. SPEAKS DATE : 3-13-17
CHECKED BY : V. A. PATEL DATE : 5-22-17

STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

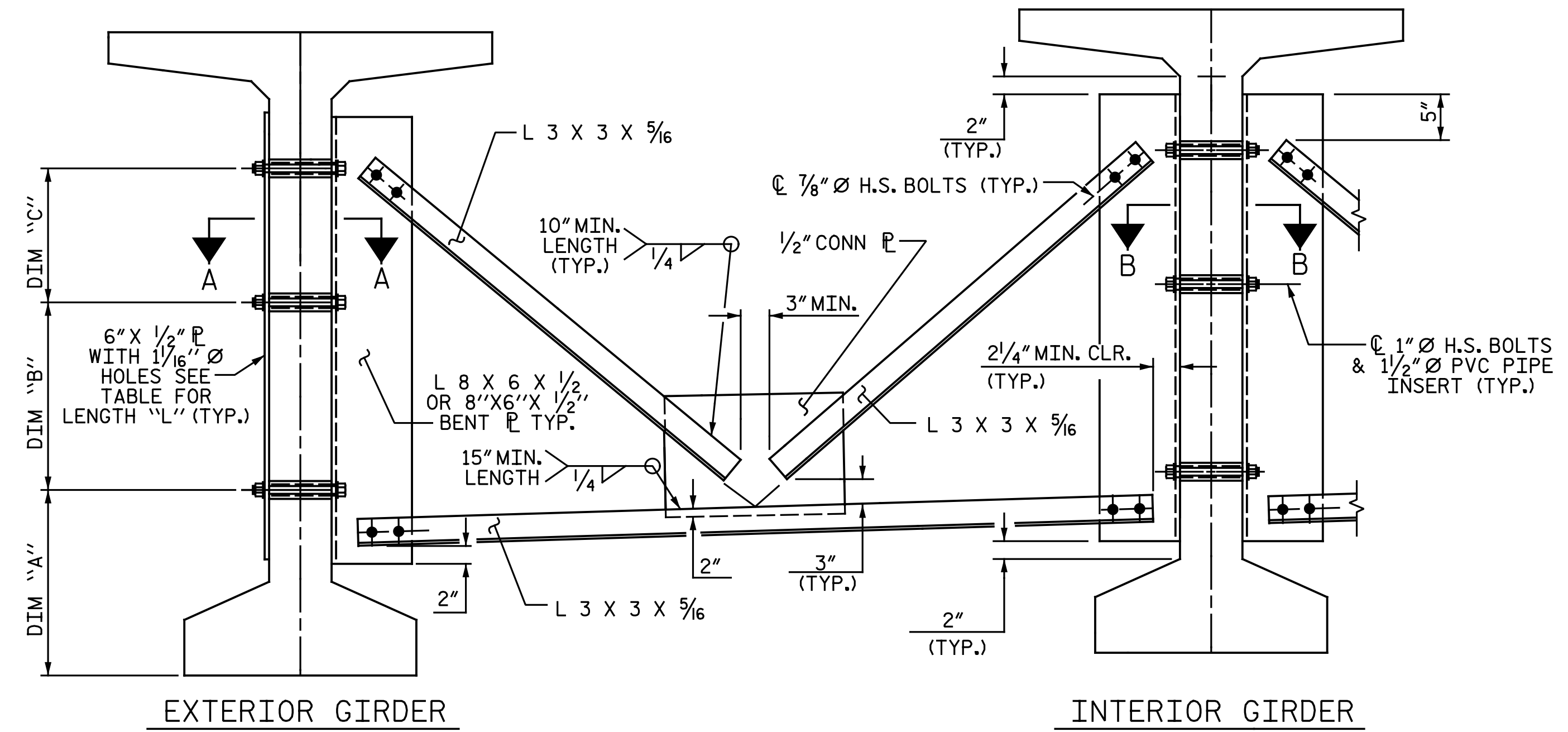
FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

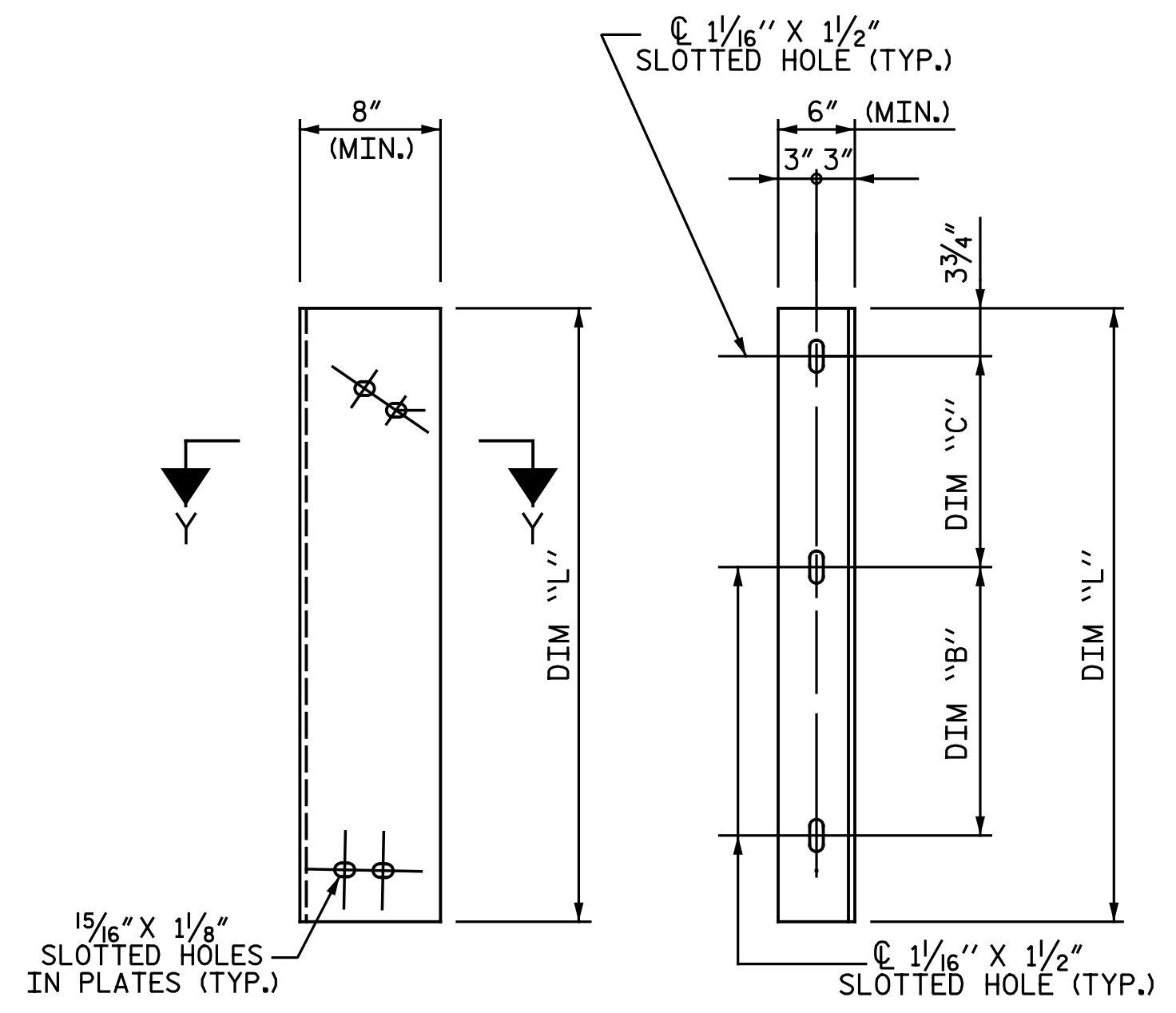
SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

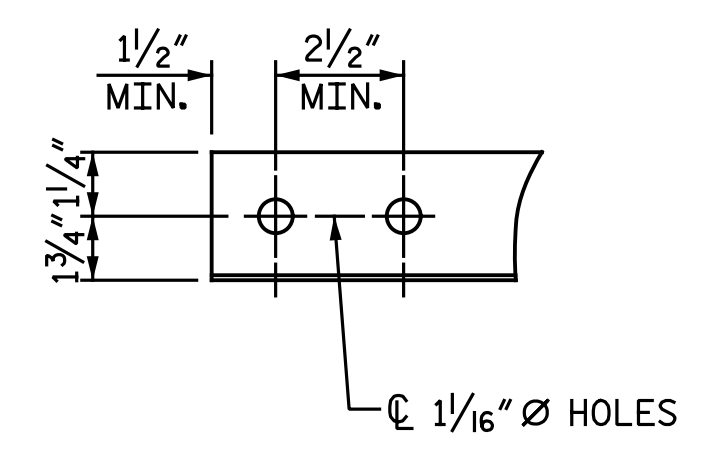
THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.



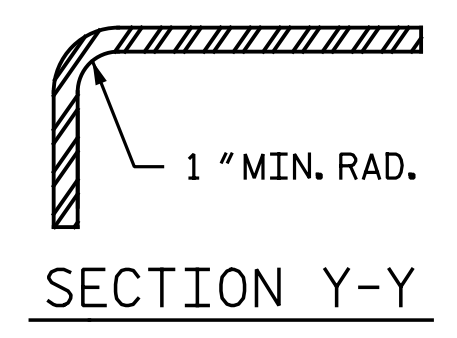
PART SECTION AT INTERMEDIATE DIAPHRAGM



DIAPHRAGM FACE WEB FACE



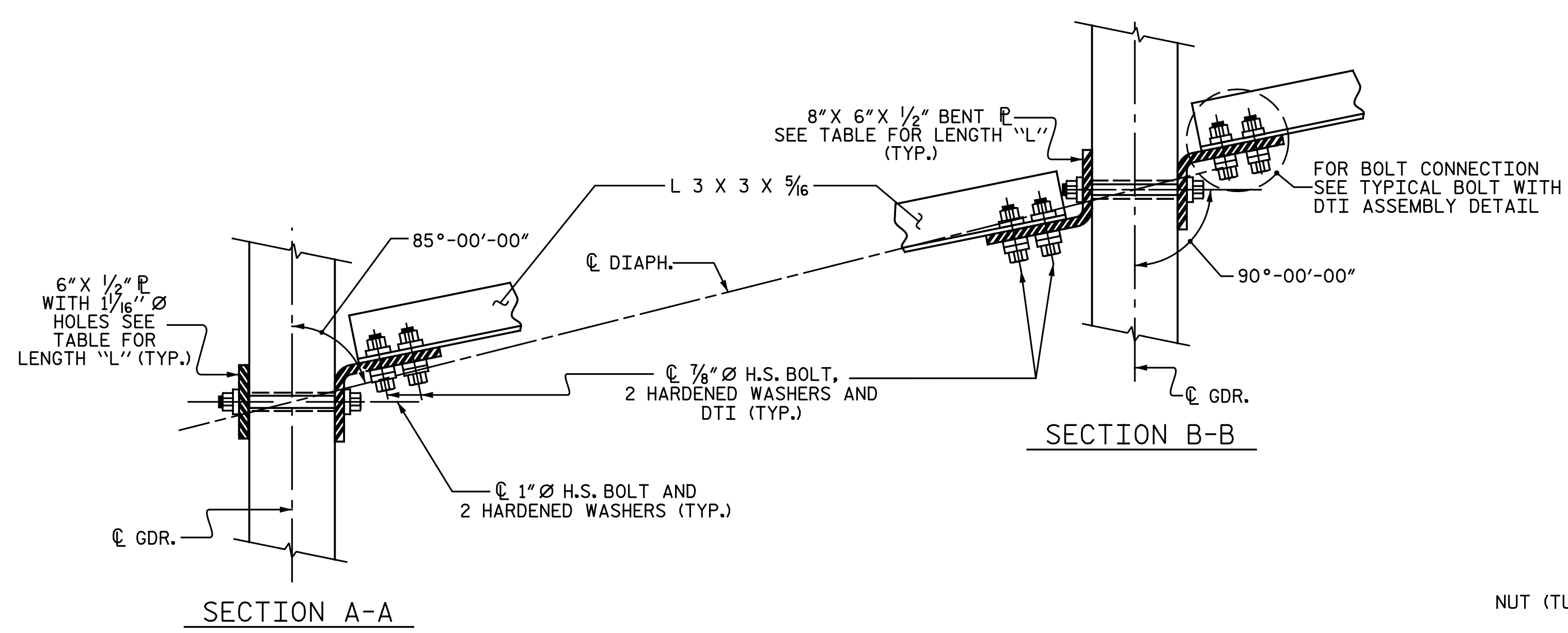
ANGLE END (L 3 X 3 X 5/16)



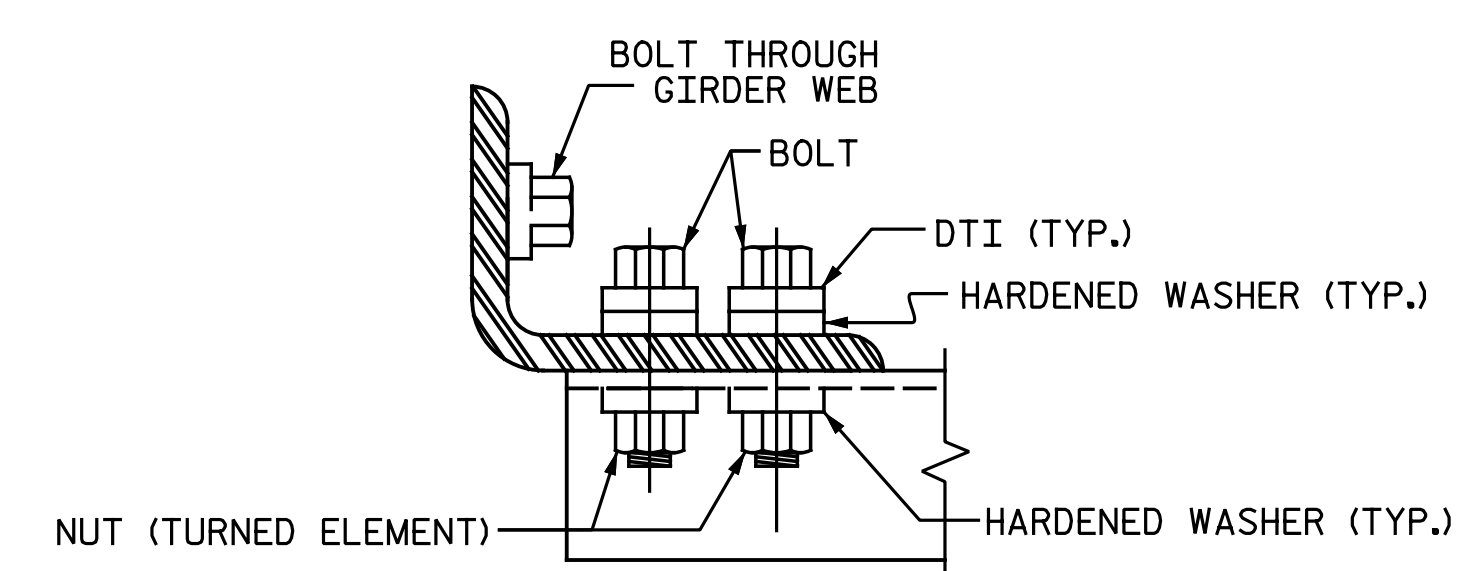
CONNECTOR PLATE DETAIL

TABLE

GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"	DIM "L"
72" BULB TEE	1'-8 3/4"	1'-9"	1'-5"	4'-2"



CONNECTION DETAILS



BOLT WITH DTI ASSEMBLY DETAIL

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR 72" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS RIGHT LANE

ASSEMBLED BY : M. D. MAYHEW DATE : 2-21-17
 CHECKED BY : V. A. PATEL DATE : 3-13-17
 DRAWN BY : RWW 11/09
 CHECKED BY : GM 11/09

ADDED 11/23/09R
 REV. 10/11/11
 MAA/GM

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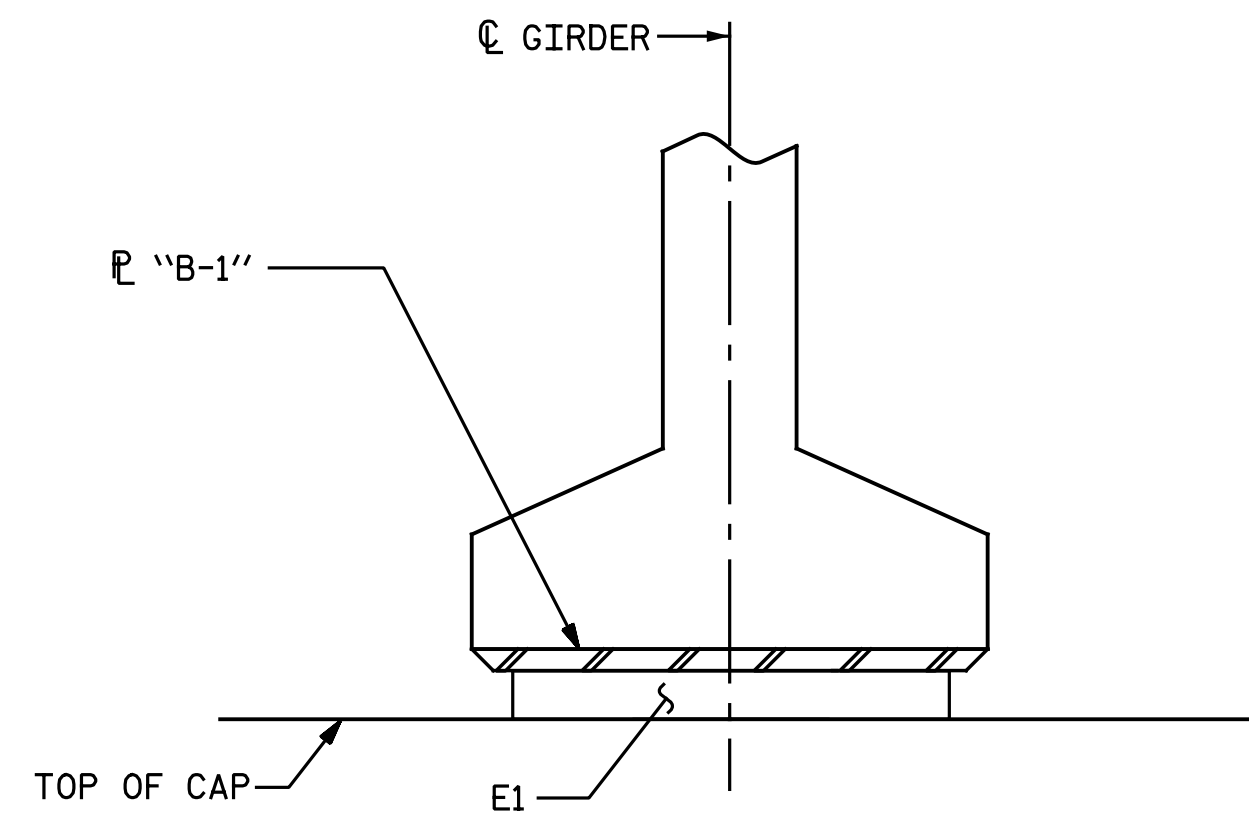
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-II
1			3			TOTAL SHEETS
2			4			25

NOTES

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

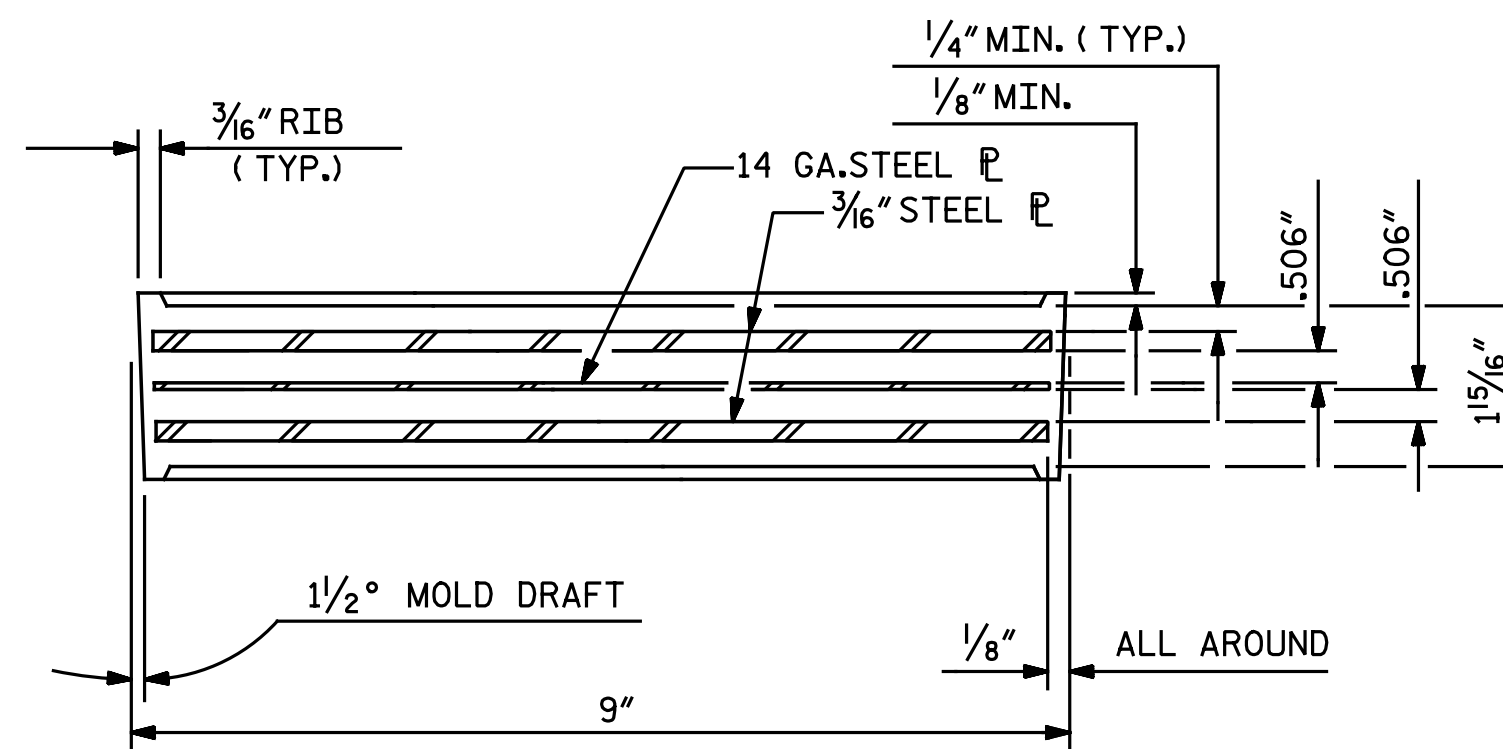
THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

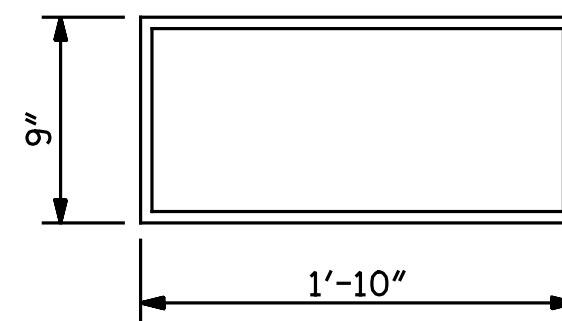


SECTION
(AT INTEGRAL END BENTS)

MAXIMUM ALLOWABLE SERVICE LOADS	
D.L.+L.L. (NO IMPACT)	
TYPE IV	225 k



TYPICAL SECTION OF ELASTOMERIC BEARING

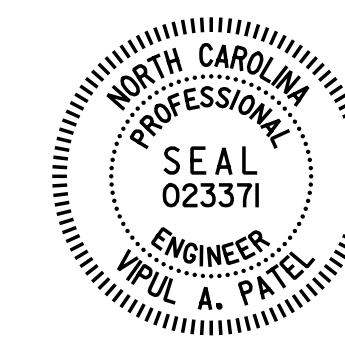


E1 (10 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

TYPE IV

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



7/26/2017

DocuSigned by:
Vipul A. Patel

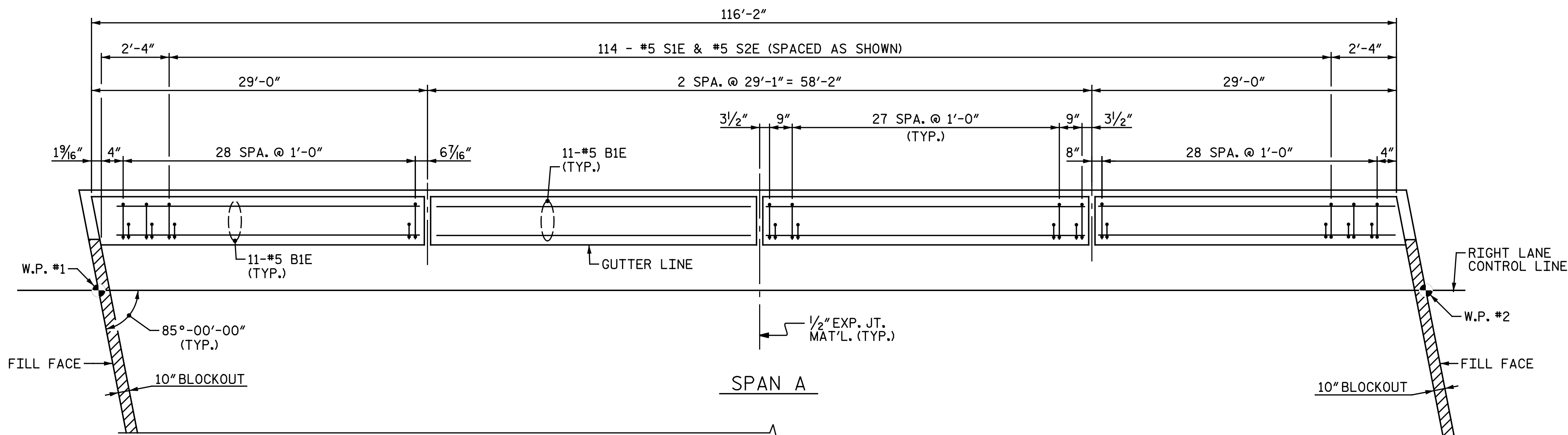
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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 ELASTOMERIC BEARING
 DETAILS
 PRESTRESSED CONCRETE GIRDER
 SUPERSTRUCTURE
 RIGHT LANE

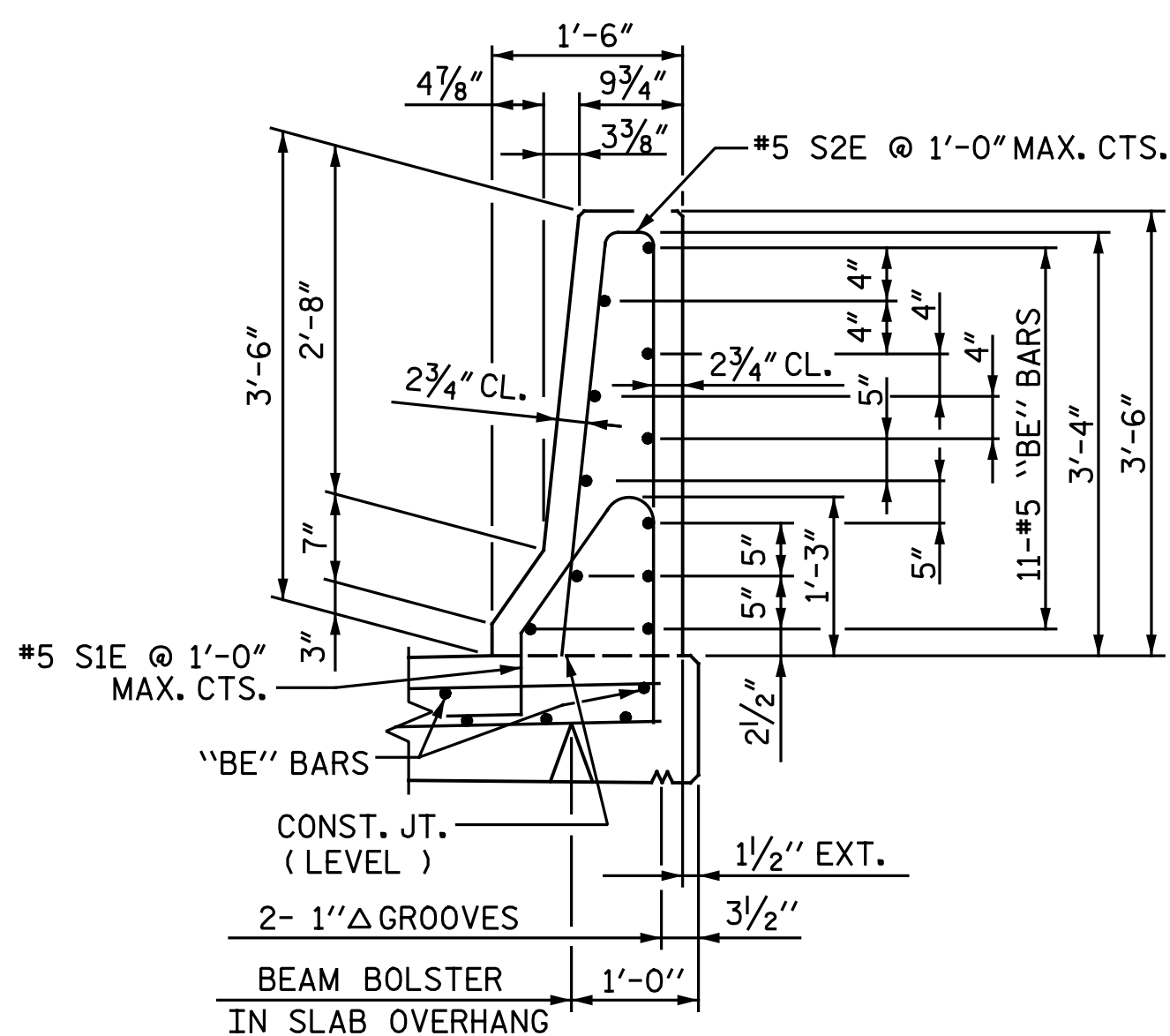
ASSEMBLED BY : N.B.SPEAKS	DATE : 1-9-17
CHECKED BY : V. A. PATEL	DATE : 5-23-17
DRAWN BY : WJH 8/89	REV. 10/1/11 MAA/GM
CHECKED BY : CRK 8/89	REV. 6/13 AAC/MAA
	REV. 1/15 MAA/TMG

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-12
1			3			TOTAL SHEETS
2			4			25

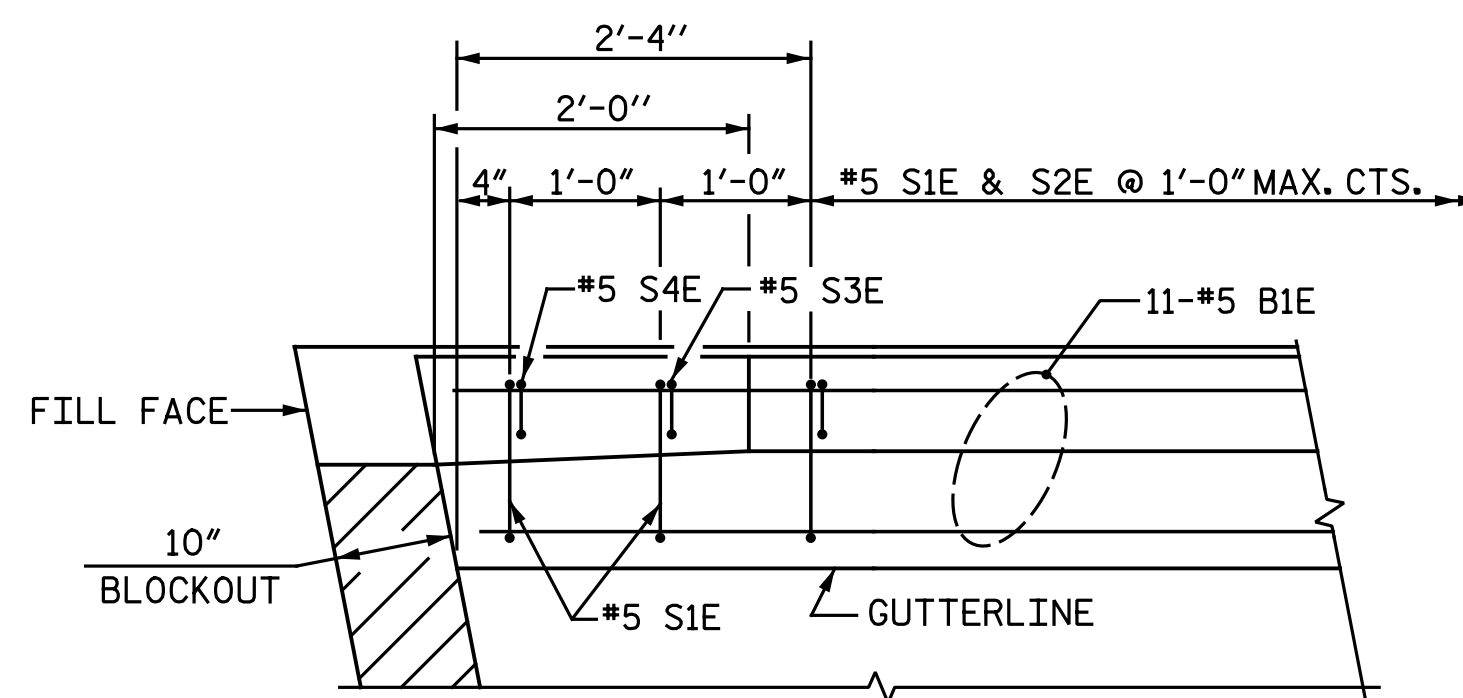
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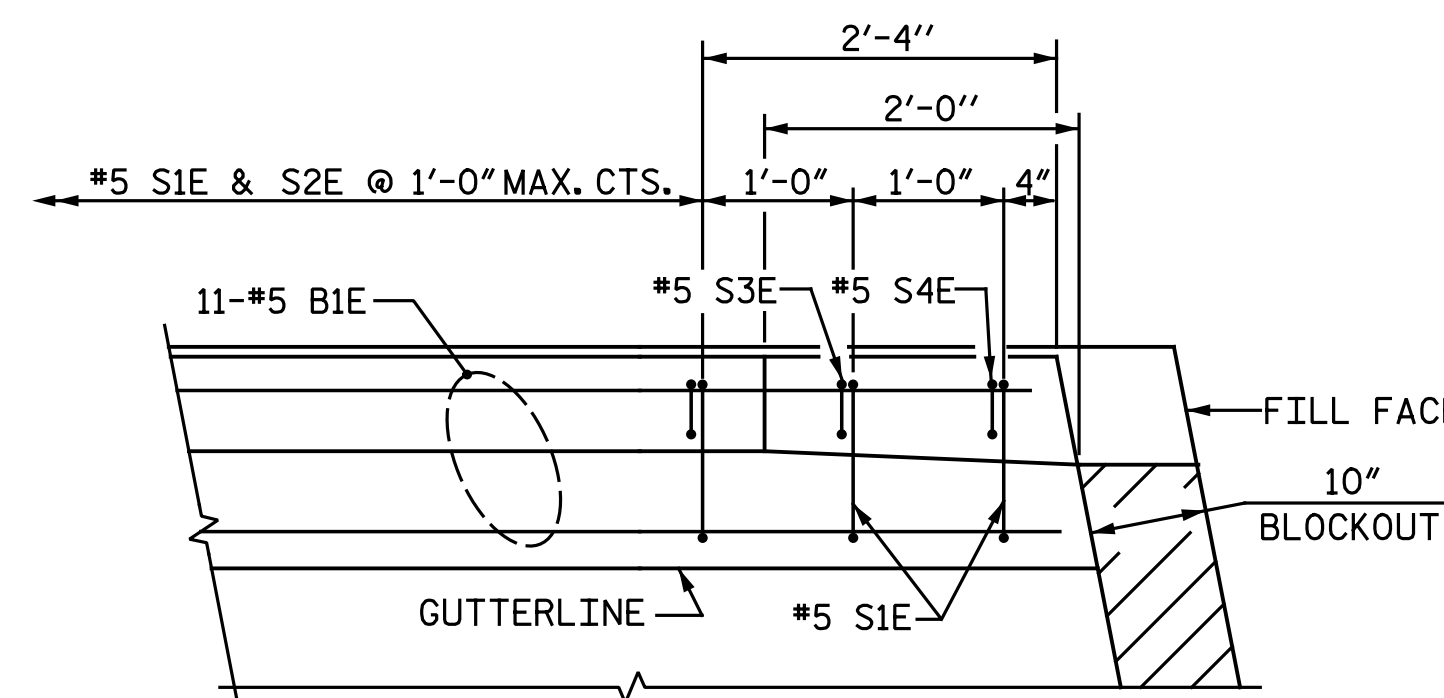
PLAN OF BARRIER RAIL
(LEFT RAIL SHOWN, RIGHT RAIL SIMILAR BY ROTATION)



SECTION THRU RAIL

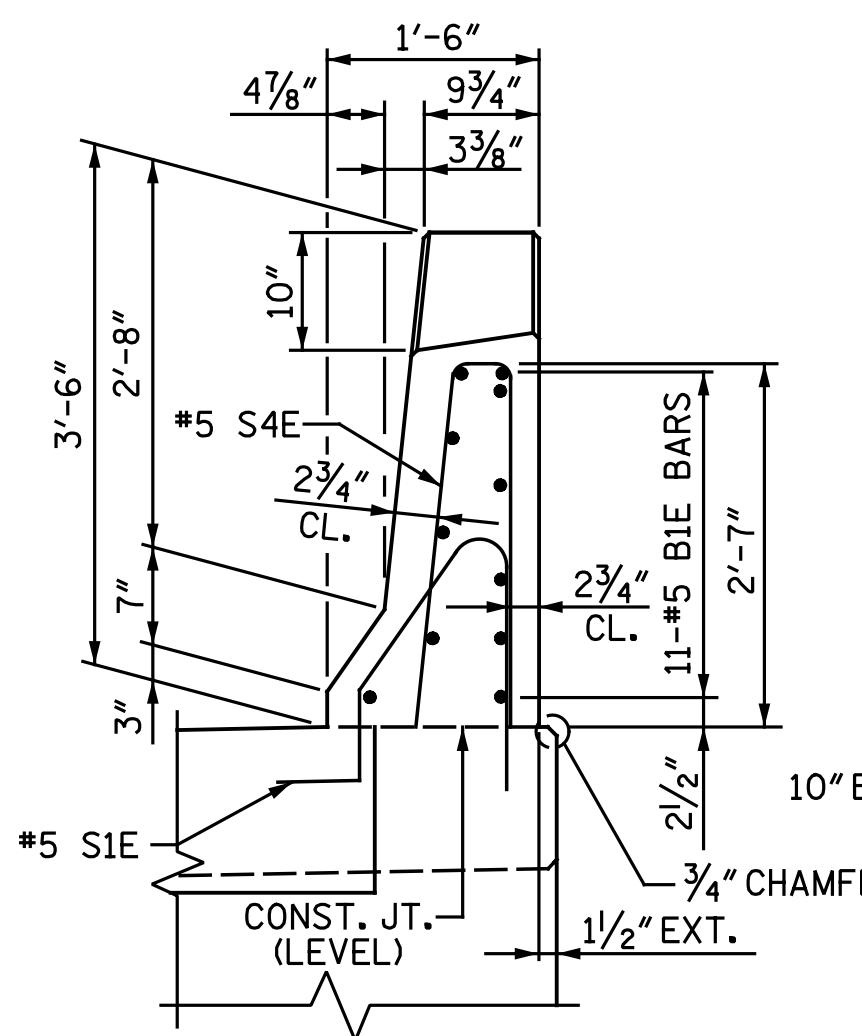


END BENT 1

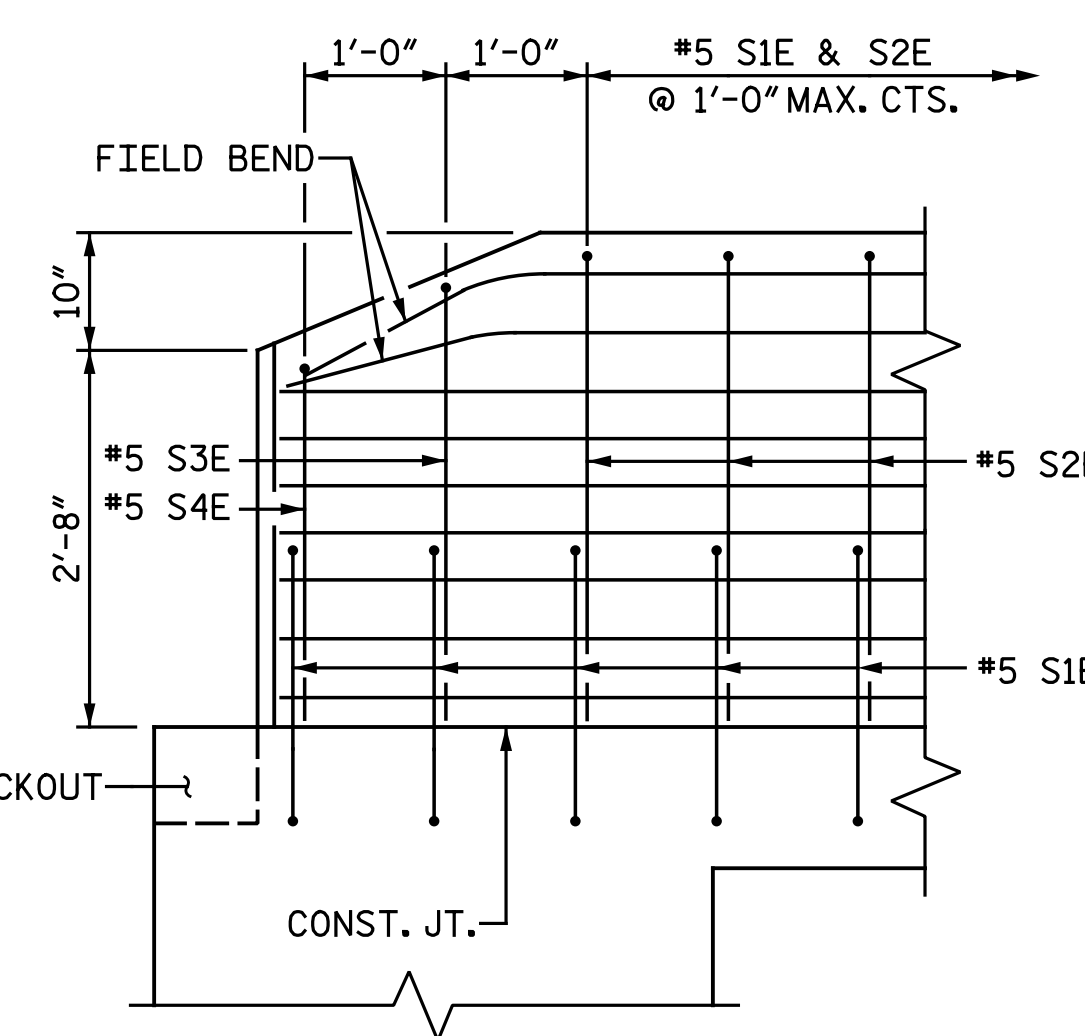


END BENT 2

PLAN
(LEFT RAIL SHOWN, RIGHT RAIL SIMILAR)

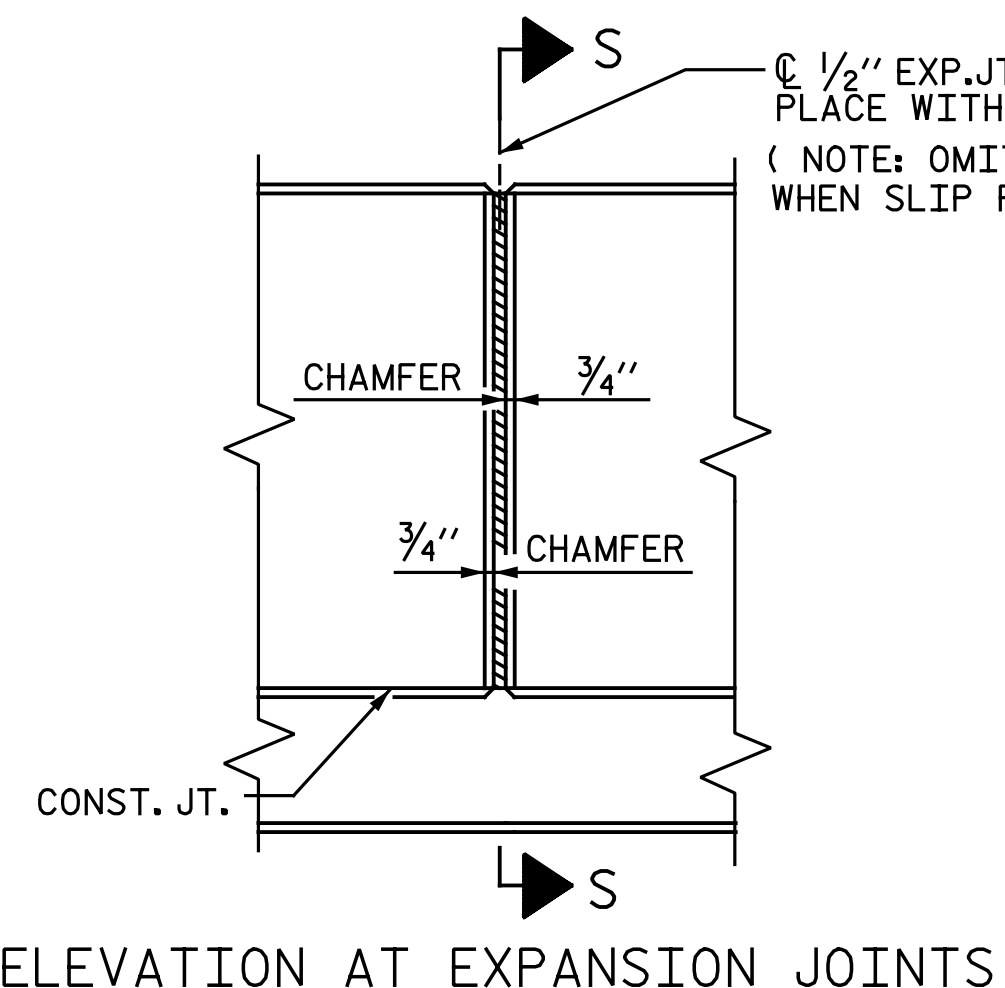


END VIEW

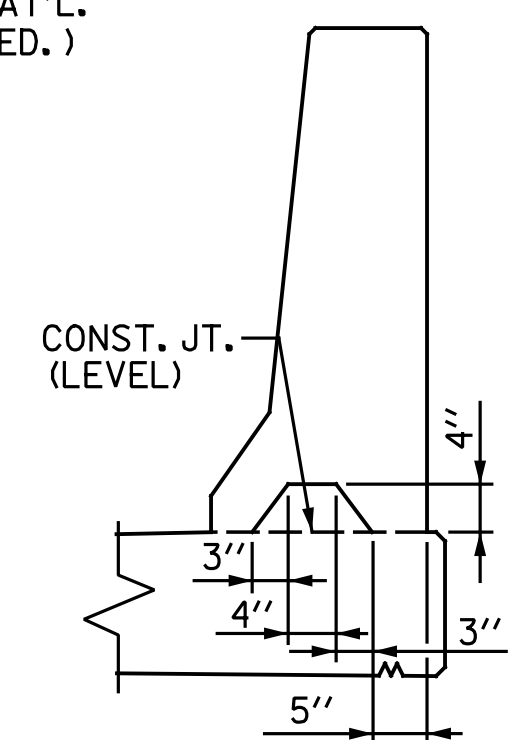


SIDE VIEW

END OF RAIL DETAILS



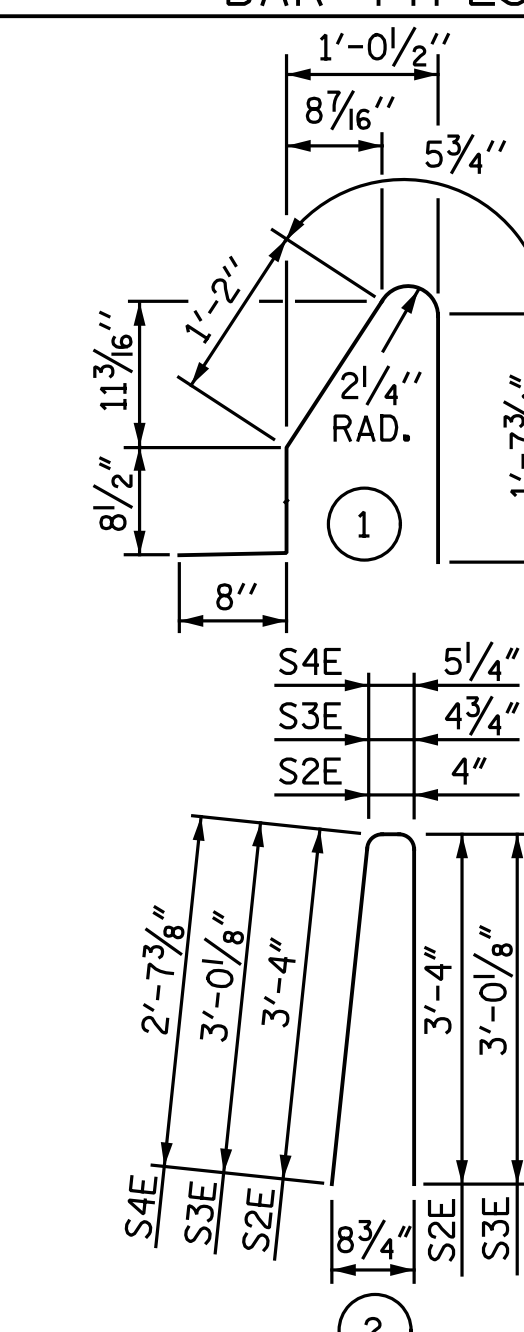
ELEVATION AT EXPANSION JOINTS



SECTION S-S
AT DAM IN OPEN JOINT
(THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

BARRIER RAIL DETAILS

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL					
FOR CONCRETE BARRIER RAIL ONLY					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1E	88	#5	STR.	28' - 8"	2,631
S1E	236	#5	1	4' - 8"	1,149
S2E	228	#5	2	7' - 0"	1,665
S3E	4	#5	2	6' - 5"	27
S4E	4	#5	2	5' - 8"	24
EPOXY COATED REINFORCING STEEL				LBS.	5,496
CLASS AA CONCRETE				C.Y.	31.6
CONCRETE BARRIER RAIL				L.F.	232.33

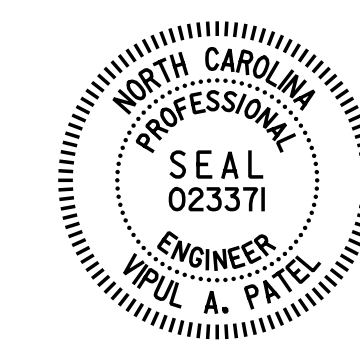
NOTES

THE BARRIER RAIL IN THE SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 CONCRETE
 BARRIER RAIL

RIGHT LANE

ASSEMBLED BY : N. B. SPEAKS	DATE : 9-11-17
CHECKED BY : V. A. PATEL	DATE : 9-12-17
DRAWN BY : ARB 5/87	REV. 10/1/11
CHECKED BY : SJD 9/87	REV. 7/12
	REV. 6/13

MAA/GM	
MAA/GM	
MAA/GM	

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1			3		
2			4		

SHEET NO.	S2-13
TOTAL SHEETS	25

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

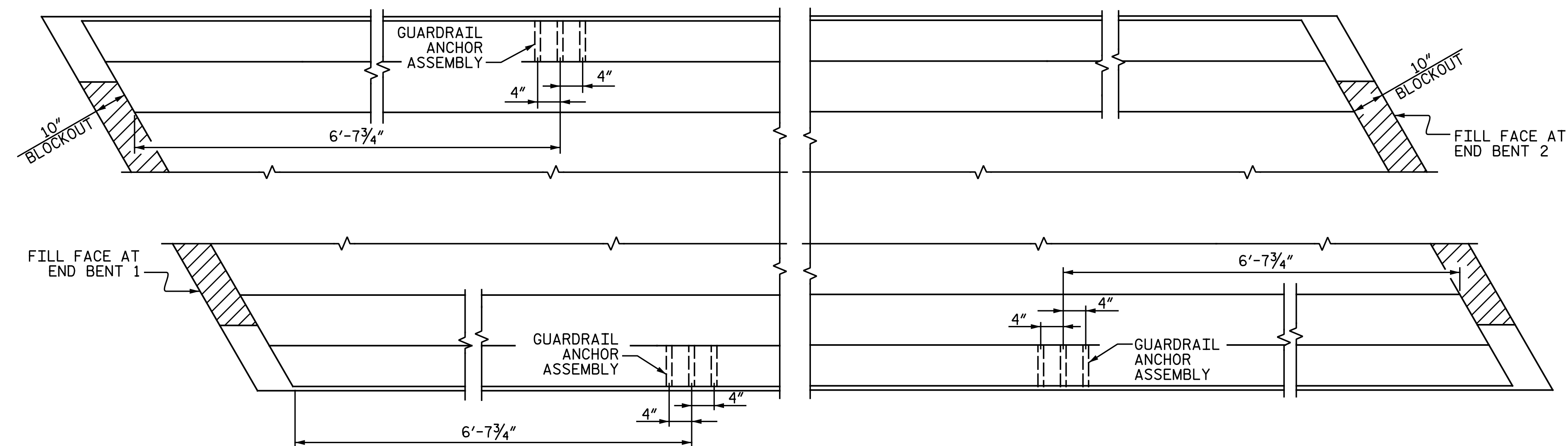
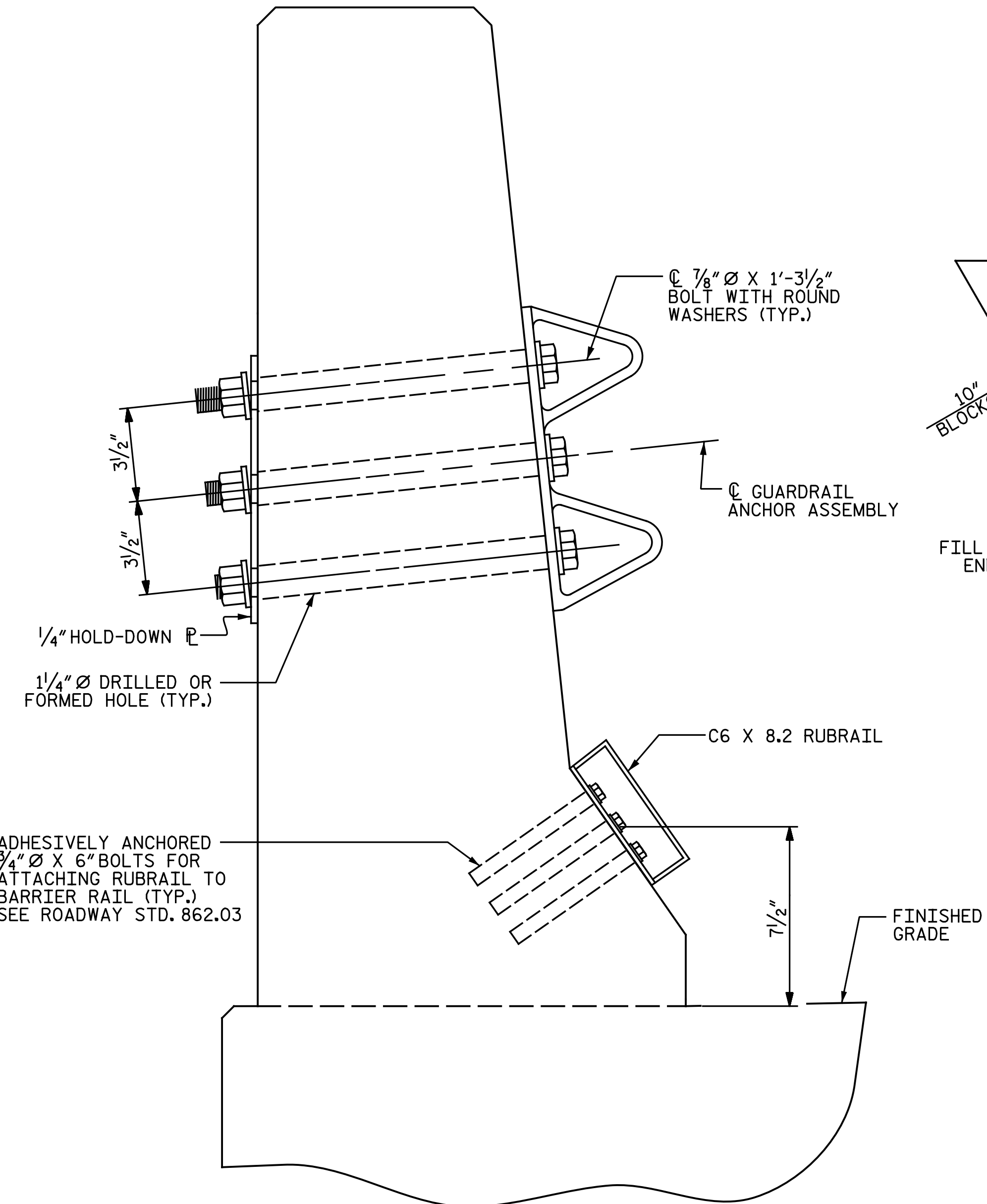
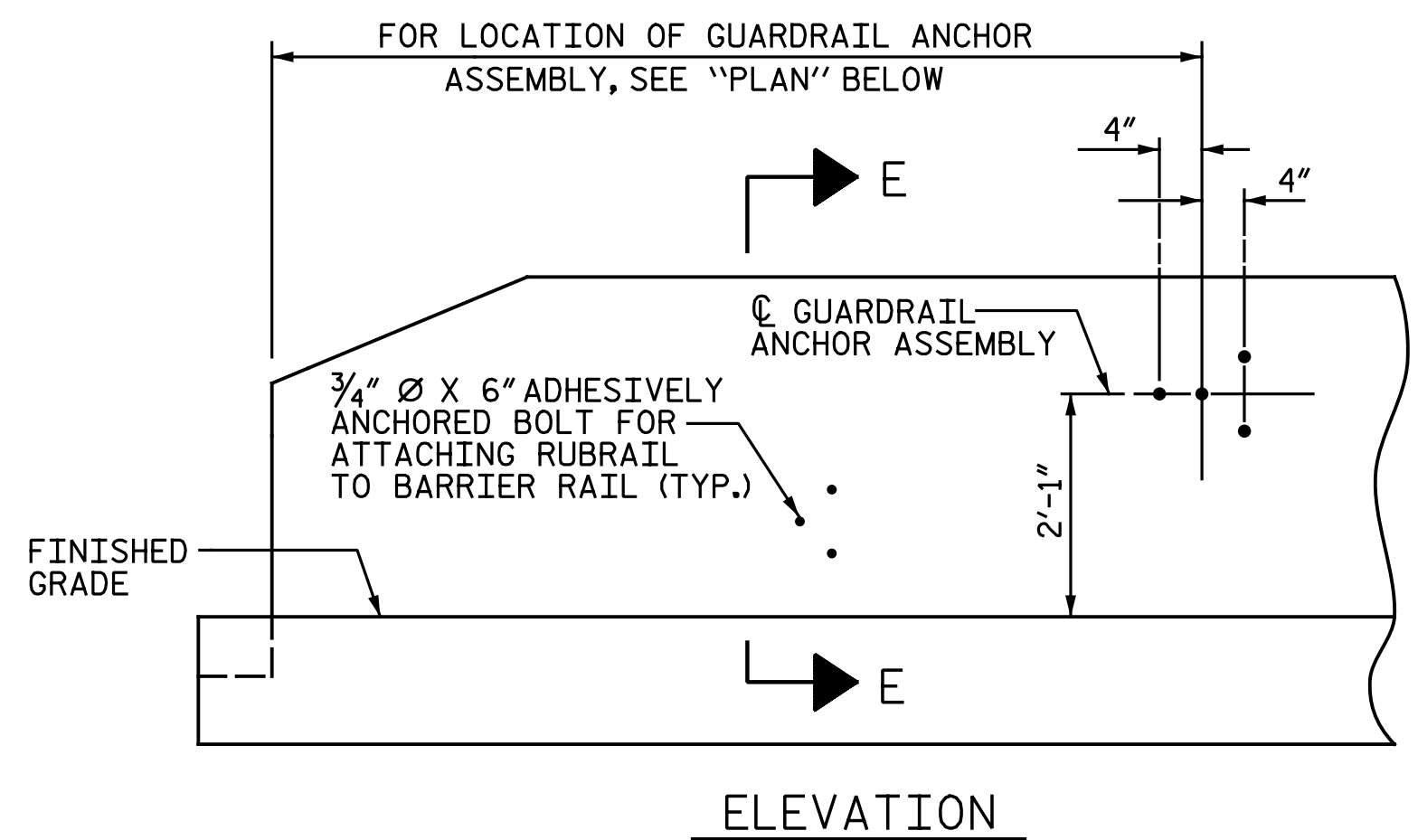
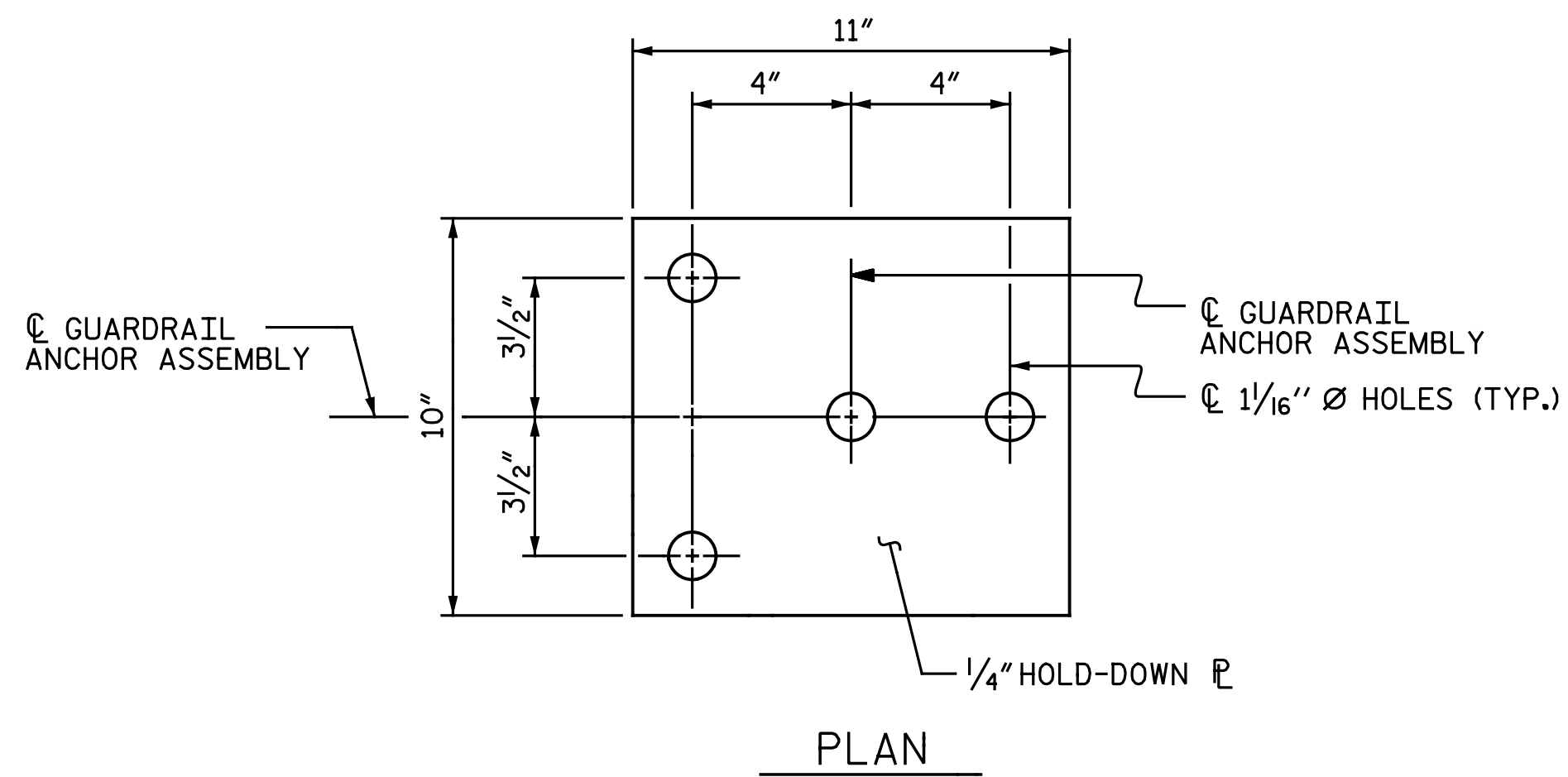
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

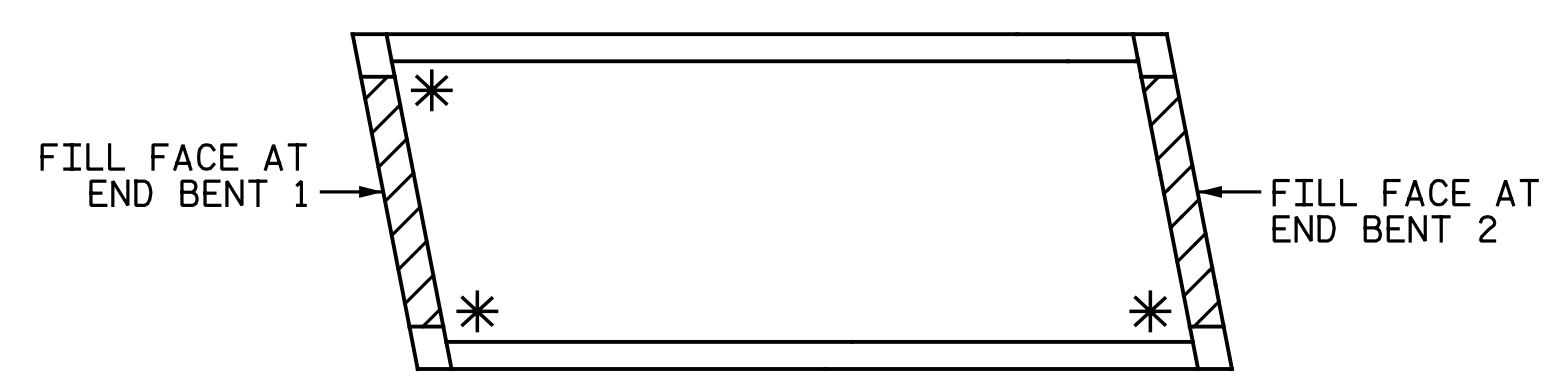
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



LOCATION OF ANCHORS FOR GUARDRAIL



* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
GUARDRAIL ANCHORAGE
FOR BARRIER RAIL
RIGHT LANE

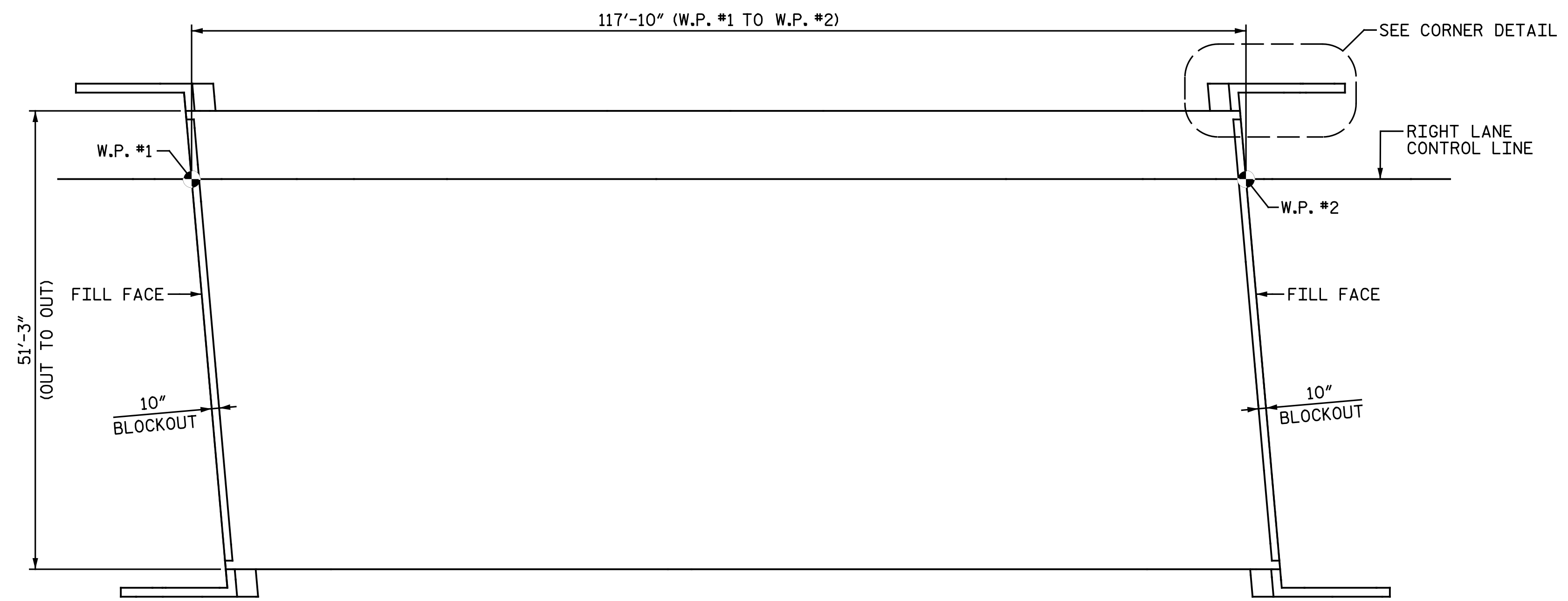
ASSEMBLED BY : N. B. SPEAKS	DATE : 9-11-17
CHECKED BY : V. A. PATEL	DATE : 9-12-17
DRAWN BY : TLA 5/06	REV. 10/1/11 MAA/GM
CHECKED BY : GM 5/06	REV. 7/12 MAA/GM
	REV. 6/13 MAA/GM

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NC License No. : F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			25
2			4			



LAYOUT FOR COMPUTING AREA OF REINFORCED CONCRETE DECK SLAB
(SQ. FT. = 6,039)

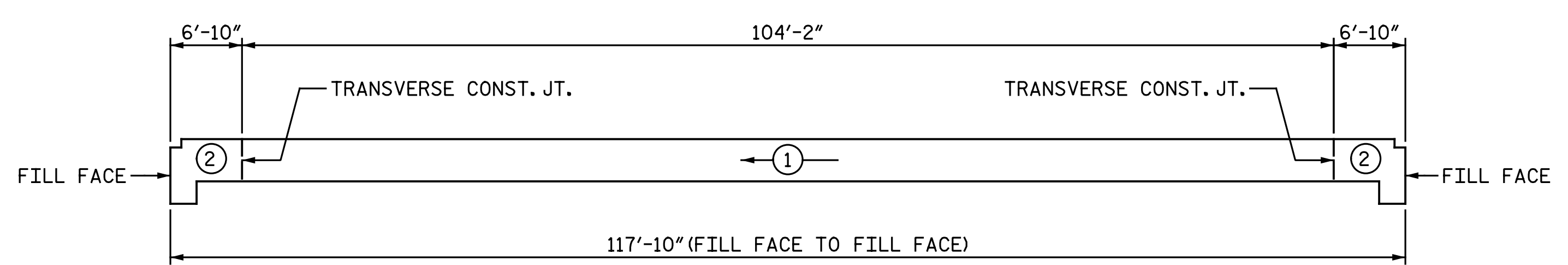
BAR TYPES				REINFORCING BAR SCHEDULE				
SPAN A								
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
A1E	196	#5	STR.	50' - 11"	10,409			
A2	192	#5	STR.	50' - 11"	10,196			
A101E	2	#5	STR.	43' - 10"	91			
A102E	2	#5	STR.	37' - 2"	78			
A103E	2	#5	STR.	30' - 6"	64			
A104E	2	#5	STR.	23' - 10"	50			
A105E	2	#5	STR.	17' - 2"	36			
A106E	2	#5	STR.	10' - 6"	22			
A107E	2	#5	STR.	3' - 10"	8			
A201	2	#5	STR.	43' - 10"	91			
A202	2	#5	STR.	37' - 2"	78			
A203	2	#5	STR.	30' - 6"	64			
A204	2	#5	STR.	23' - 10"	50			
A205	2	#5	STR.	17' - 2"	36			
A206	2	#5	STR.	10' - 6"	22			
A207	2	#5	STR.	3' - 10"	8			
B1E	175	#4	STR.	24' - 10"	2,903			
B2	116	#5	STR.	59' - 0"	7,138			
B3E	68	#6	STR.	25' - 5"	2,596			
B4E	68	#6	STR.	27' - 7"	2,817			
B5E	8	#4	1	4' - 0"	21			
H1	28	#5	STR.	3' - 0"	88			
K1	28	#4	STR.	26' - 10"	502			
K2	8	#4	STR.	6' - 7"	35			
K3	40	#4	STR.	9' - 7"	256			
K4	8	#4	STR.	8' - 0"	43			
K5	4	#4	STR.	2' - 6"	7			
K6	20	#4	STR.	4' - 0"	53			
K7	4	#4	STR.	3' - 2"	8			
S1E	88	#4	2	11' - 11"	701			
S2E	84	#4	2	10' - 3"	575			
U1	88	#4	3	14' - 6"	852			
U2	20	#4	3	10' - 10"	145			
V1	12	#5	STR.	7' - 0"	88			
REINFORCING STEEL					LBS.	19,760		
EPOXY COATED REINF. STEEL					LBS.	20,371		

—SUPERSTRUCTURE BILL OF MATERIAL—

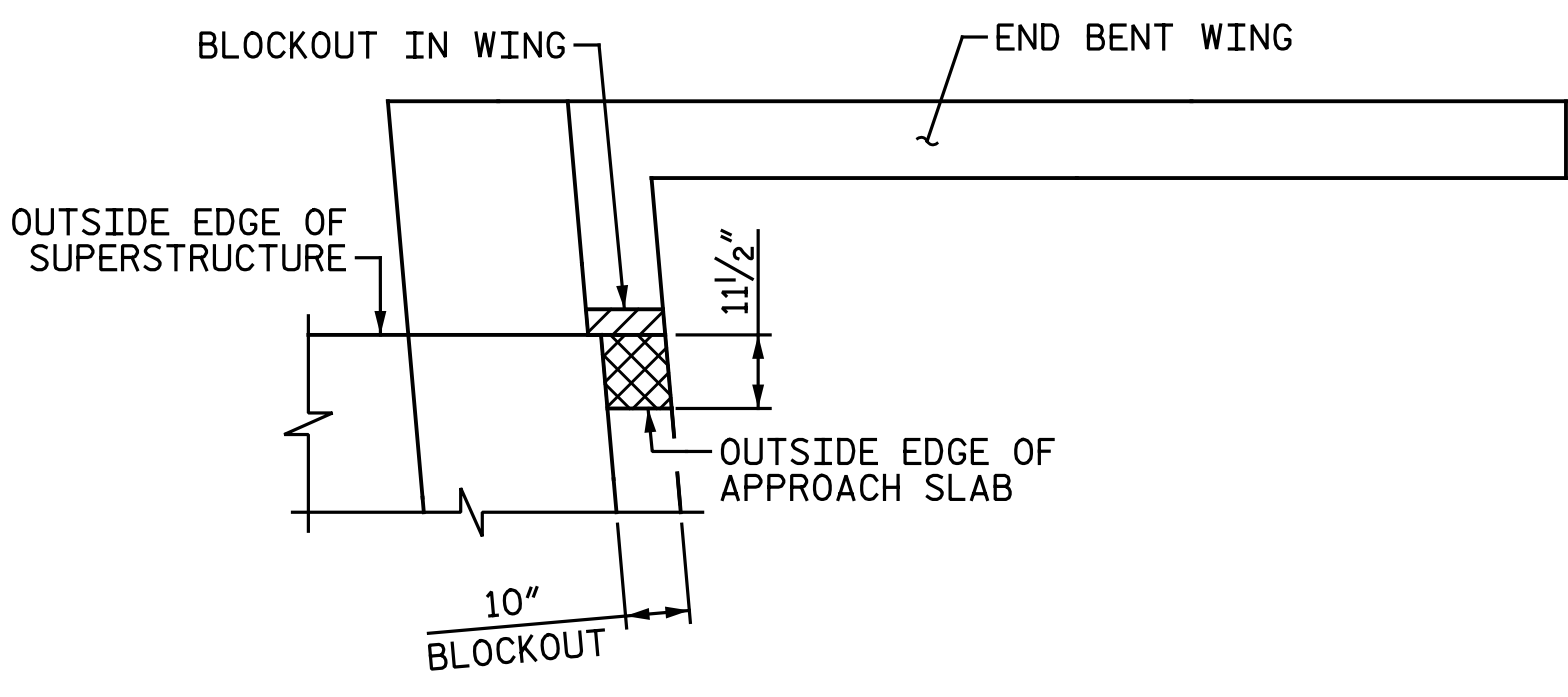
	CLASS AA CONCRETE (CU. YDS.)	REINFORCING STEEL (LBS.)	EPOXY COATED REINFORCING STEEL (LBS.)
SPAN POUR 1	179.2	19,760	20,371
SPAN POUR 2	94.2		
TOTALS *	273.4	19,760	20,371

* QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

"E" SUFFIX DENOTES EPOXY COATED REINFORCING STEEL



POURING SEQUENCE
⊙ DENOTES POUR NUMBER AND DIRECTION



CORNER DETAIL

CONCRETE SHALL BE POURED IN THE CROSS-HATCHED AREA TO MATCH THE TOP OF END BENT WING ELEVATIONS. UNLESS OTHERWISE DIRECTED BY THE ENGINEER, THE CONCRETE IN THESE AREAS SHALL BE PLACED AT THE SAME TIME THE BLOCKOUTS IN THE END BENT WINGS ARE POURED AS NOTED ON SHEET 1 OF "INTEGRAL END BENT 1" AND "INTEGRAL END BENT 2" SHEETS.

7/26/2017



GROOVING BRIDGE FLOORS		
APPROACH SLABS	2,175	SQ.FT.
BRIDGE DECK	5,212	SQ.FT.
TOTAL	7,387	SQ.FT.

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-

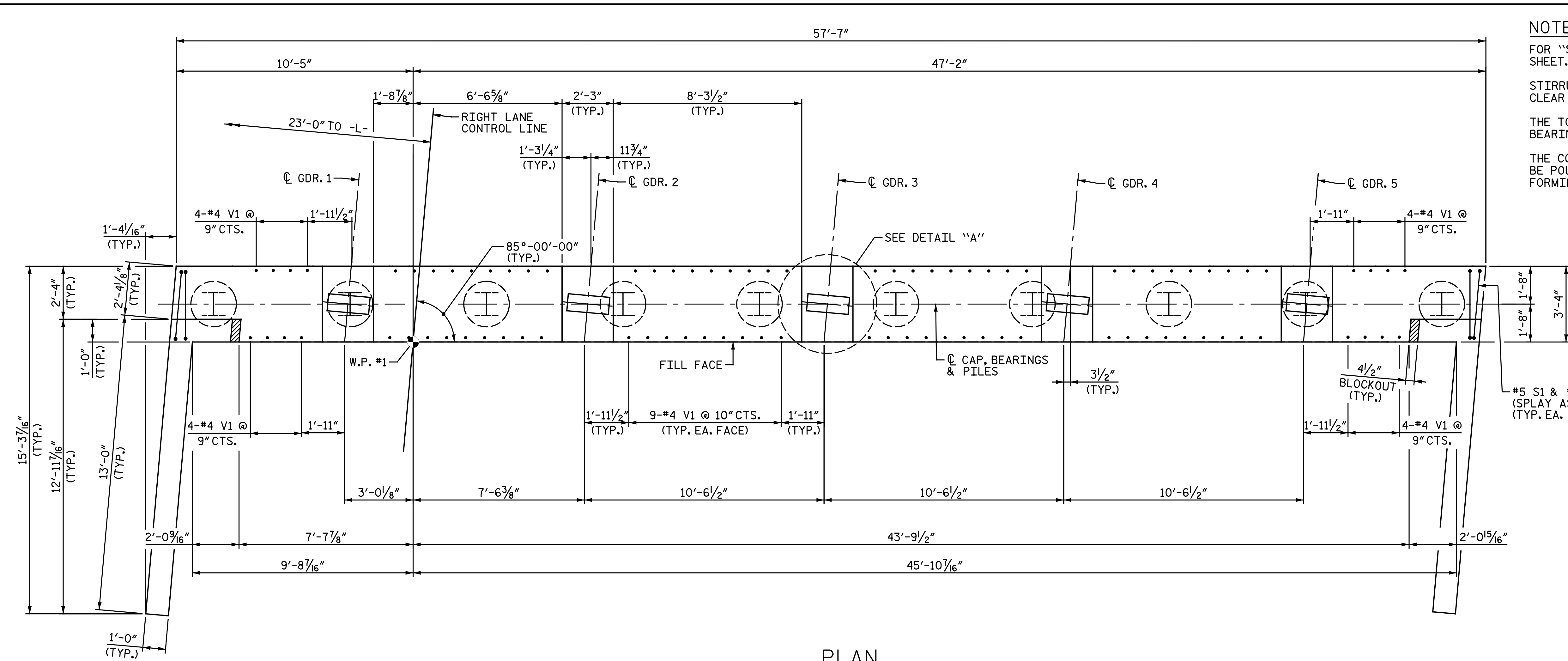
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
BILL OF MATERIAL
RIGHT LANE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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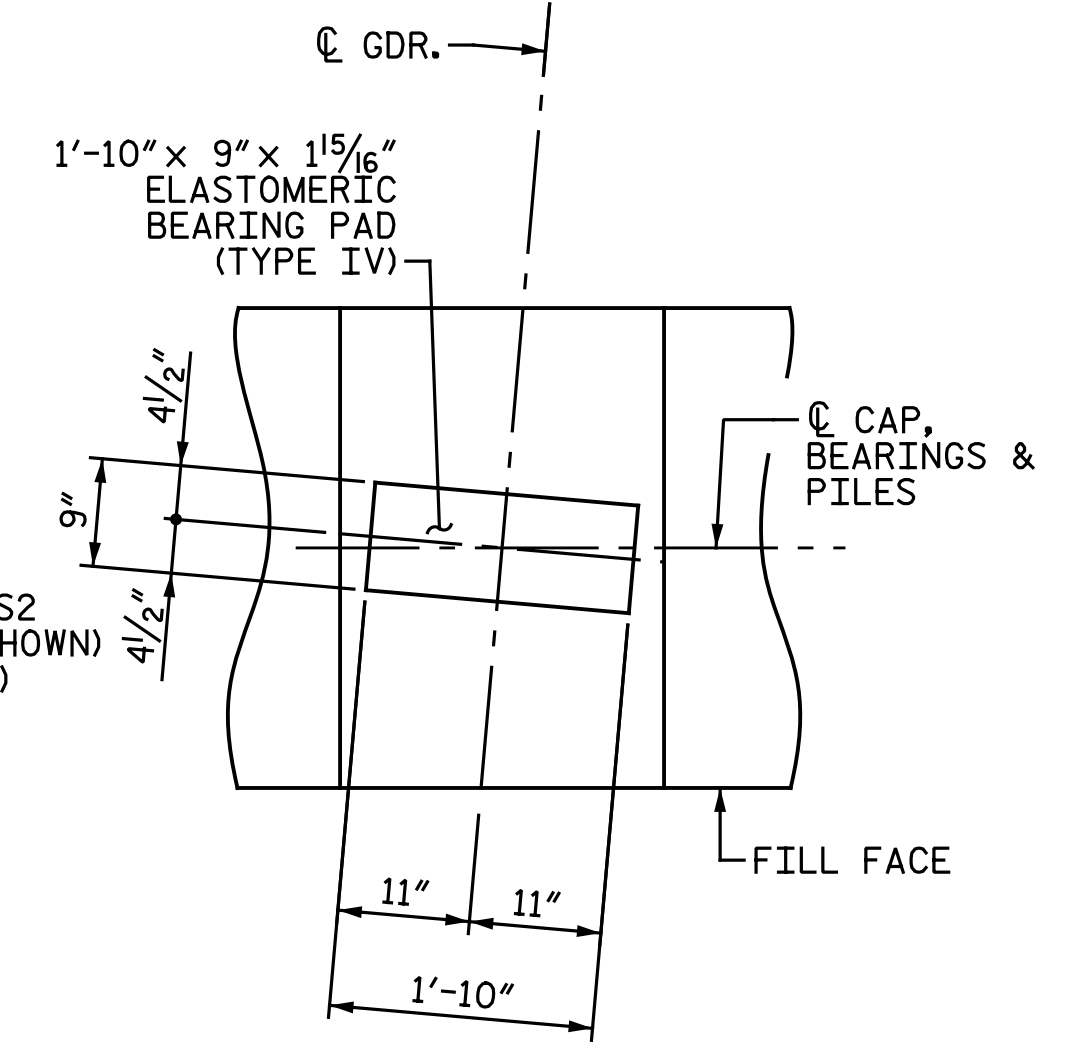
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-15
1			3			TOTAL SHEETS
2			4			25

DRAWN BY : M. D. MAYHEW DATE : 2-21-17
CHECKED BY : V. A. PATEL DATE : 3-13-17



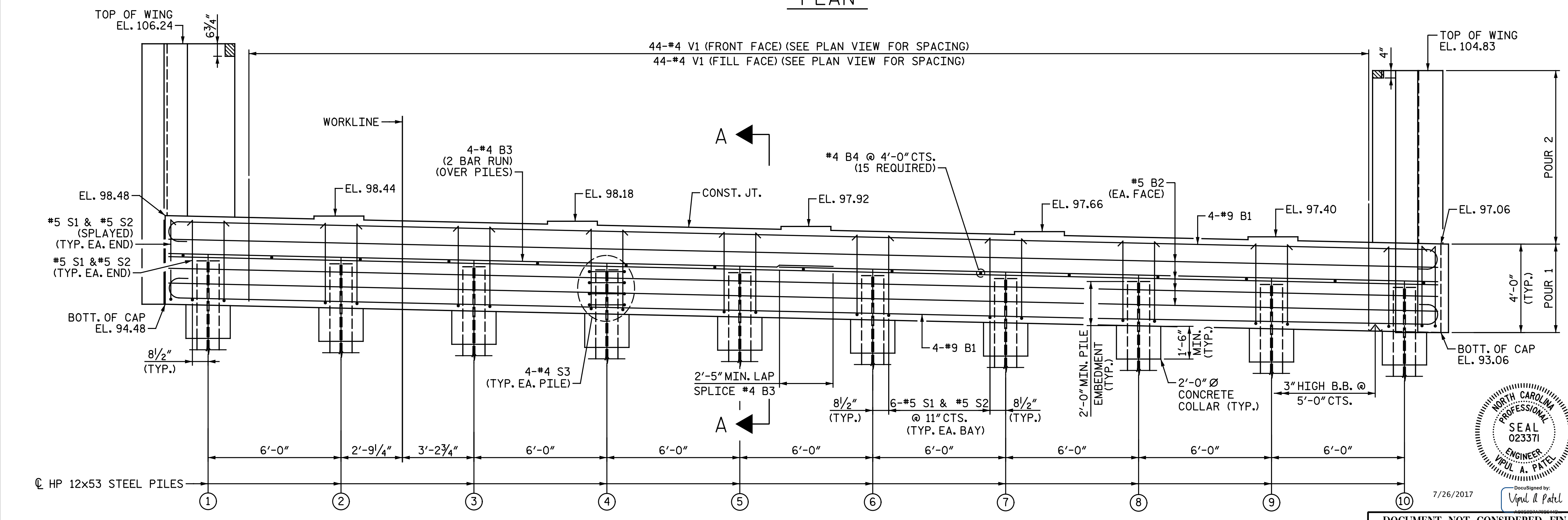
PLAN

NOTES:
 FOR "SECTION A-A", SEE "INTEGRAL END BENT 1 DETAILS" SHEET.
 STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #4 V1 BARS.
 THE TOP SURFACE OF THE END BENT CAP, EXCLUDING THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".
 THE CONCRETE IN THE HATCHED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.



DETAIL "A"

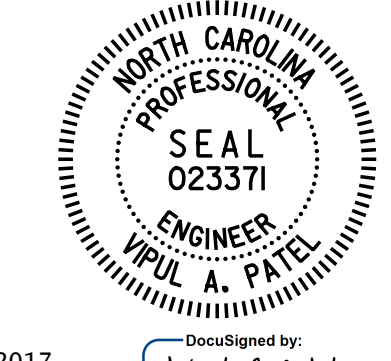
ALL DIMENSIONS AND DETAILS SHOWN ARE TYPICAL FOR ALL BEARINGS AT EACH BRIDGE SEAT LOCATION.



ELEVATION

TOP OF PILE ELEVATIONS	
PILE	ELEVATION
①	96.48
②	96.33
③	96.18
④	96.03
⑤	95.88
⑥	95.74
⑦	95.59
⑧	95.44
⑨	95.29
⑩	95.15

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-
 SHEET 1 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 1

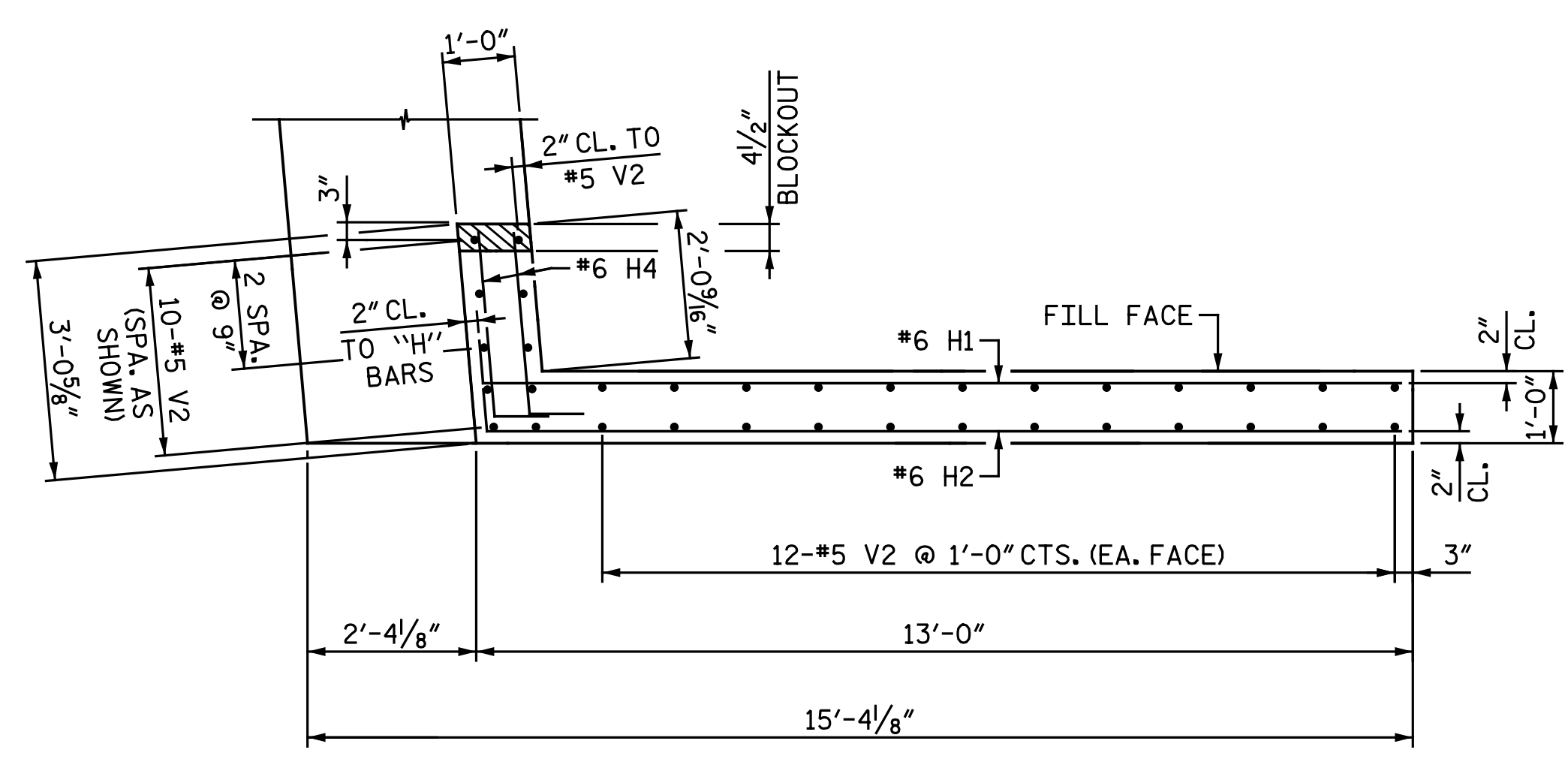
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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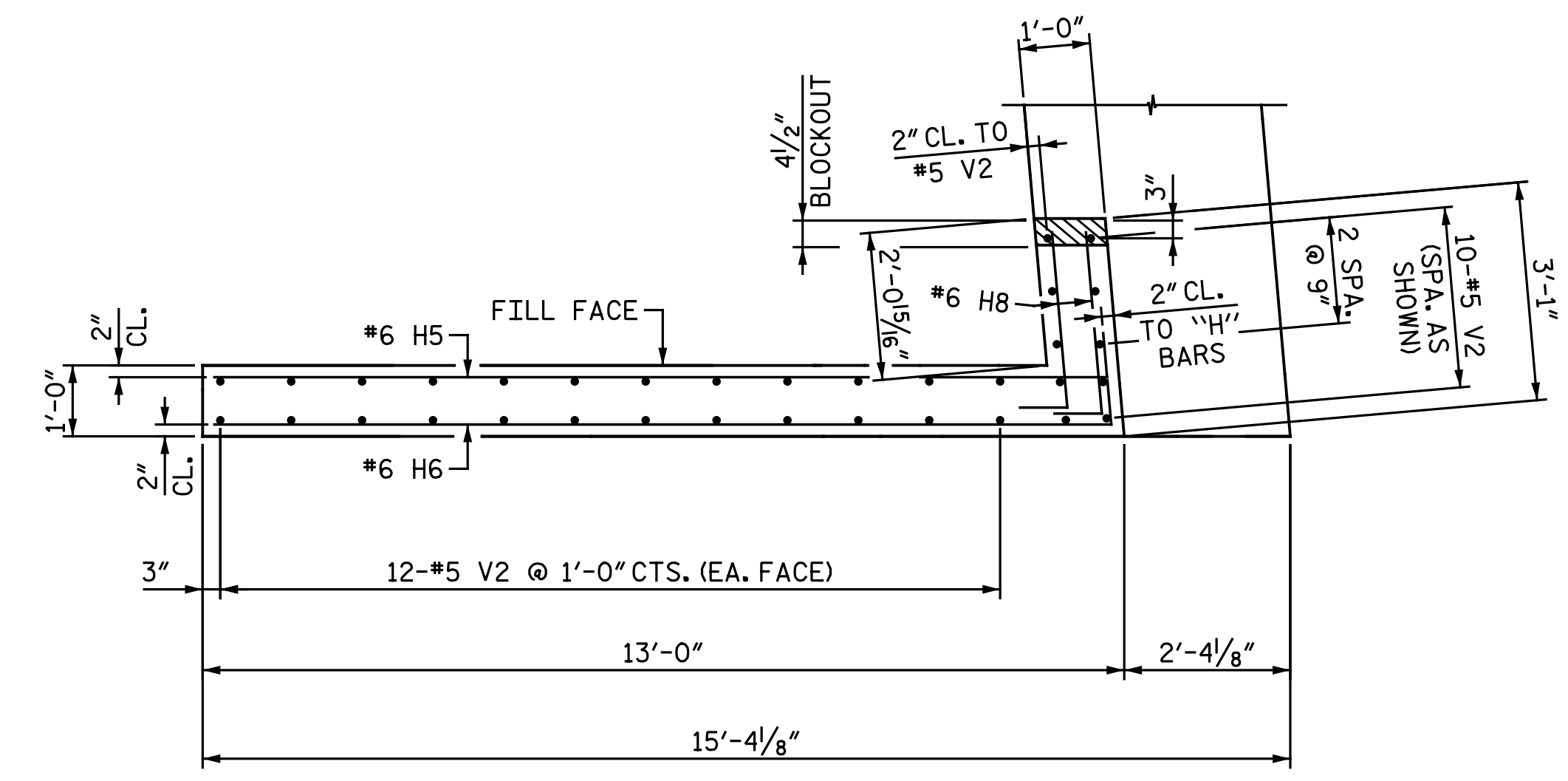
REVISIONS					SHEET NO. S2-16
NO.	BY:	DATE:	NO.	DATE:	
1			3		TOTAL SHEETS 25
2			4		

DRAWN BY: M. D. MAYHEW DATE: 4-18-17
 CHECKED BY: V. A. PATEL DATE: 5-15-17

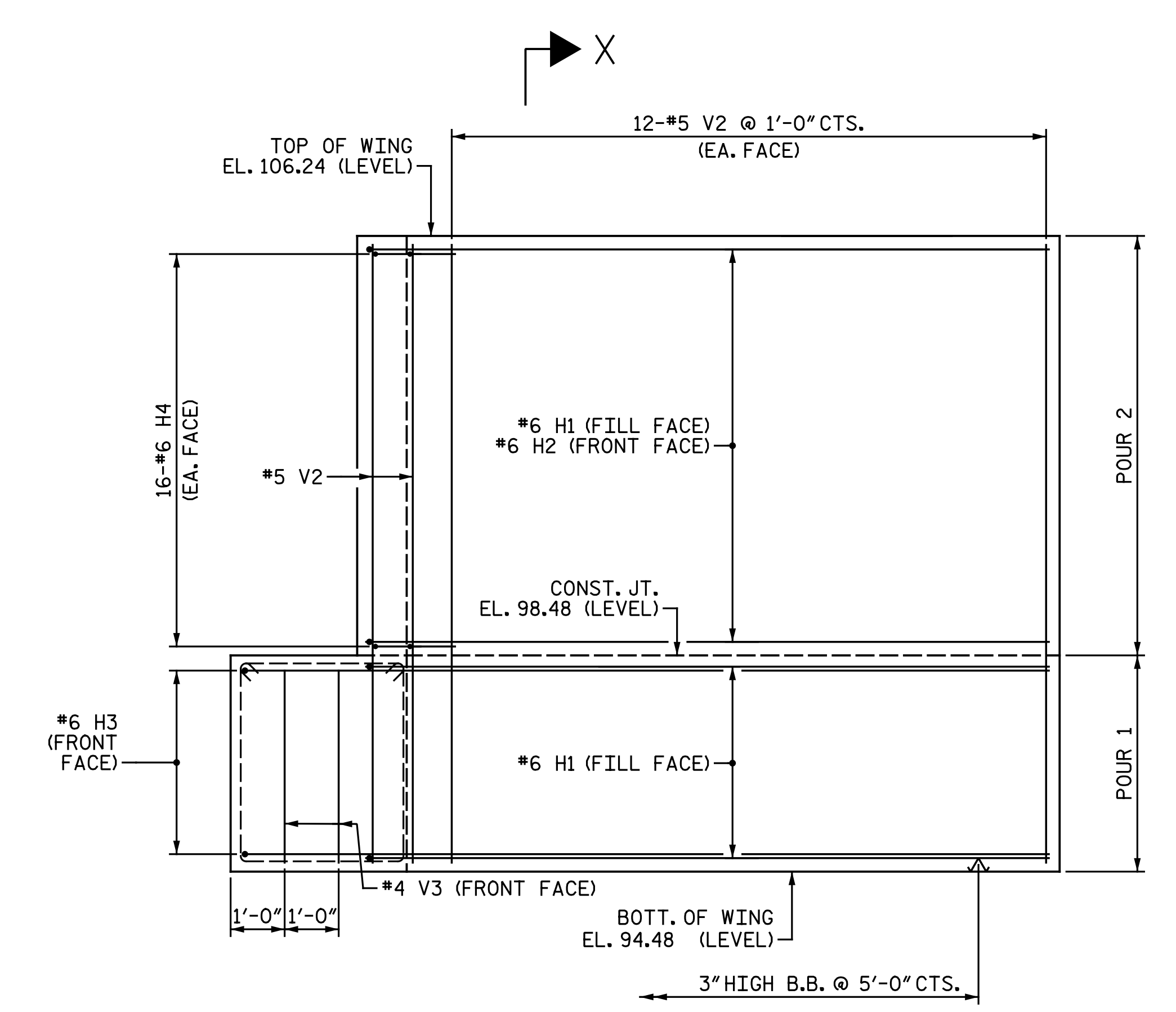
RIGHT LANE



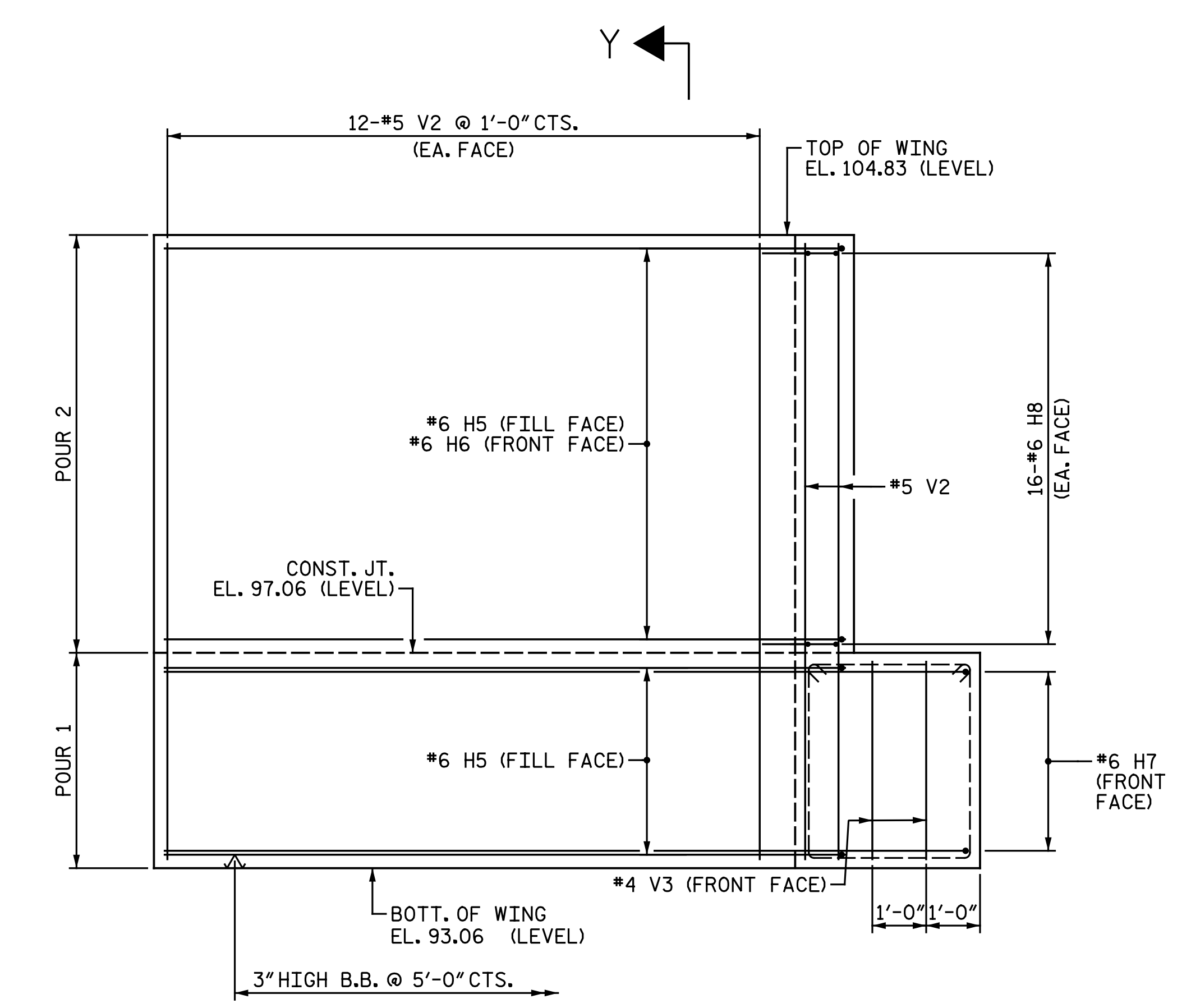
PLAN OF LEFT WING
(H3 BARS NOT SHOWN FOR CLARITY)



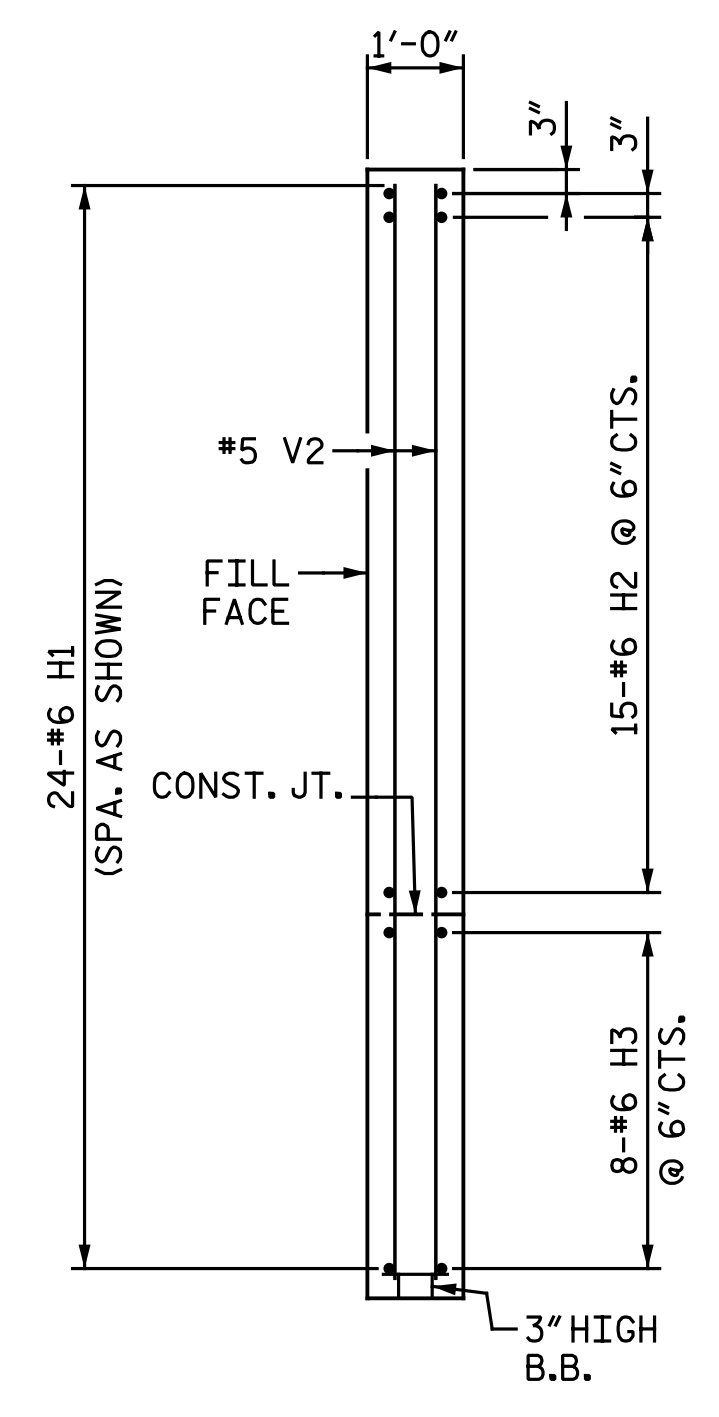
PLAN OF RIGHT WING
(H7 BARS NOT SHOWN FOR CLARITY)



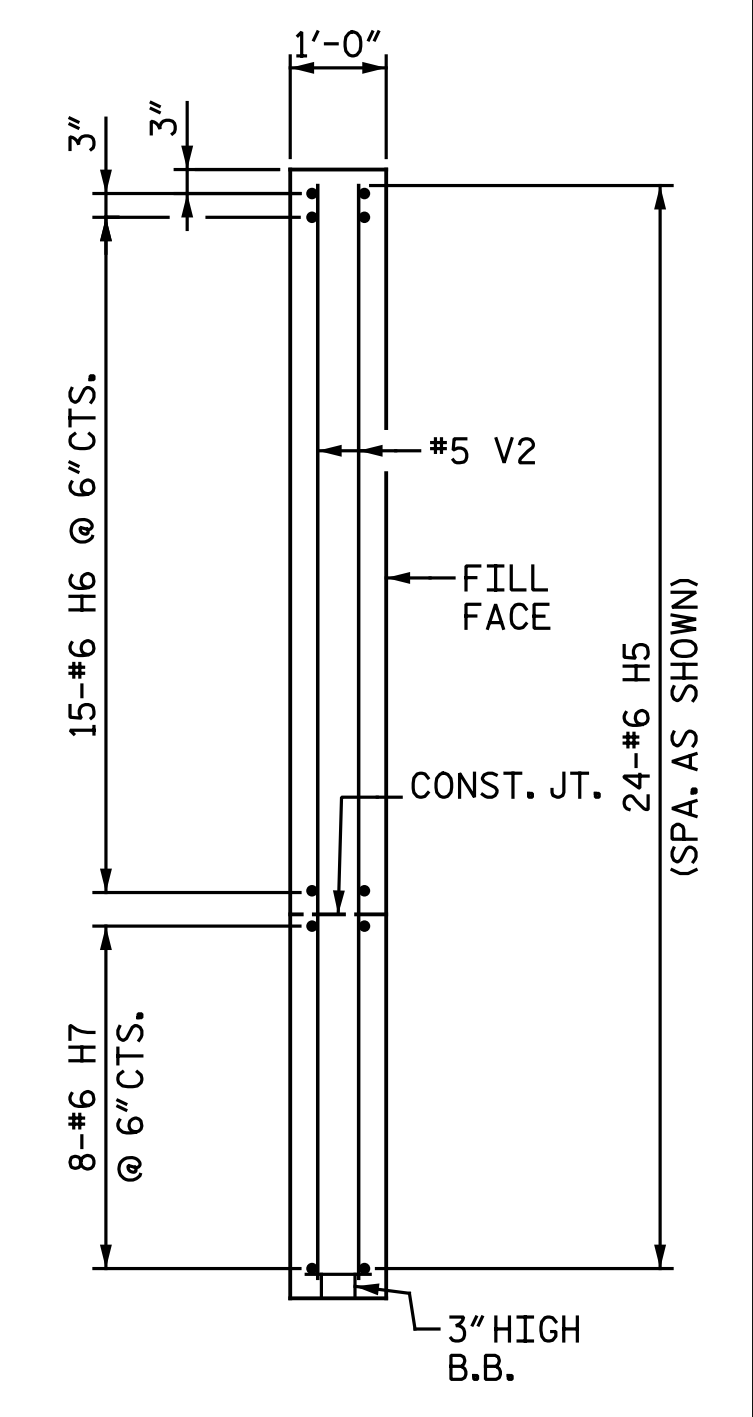
ELEVATION OF LEFT WING



ELEVATION OF RIGHT WING



SECTION X-X



SECTION Y-Y

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-
 SHEET 2 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
INTEGRAL END BENT 1

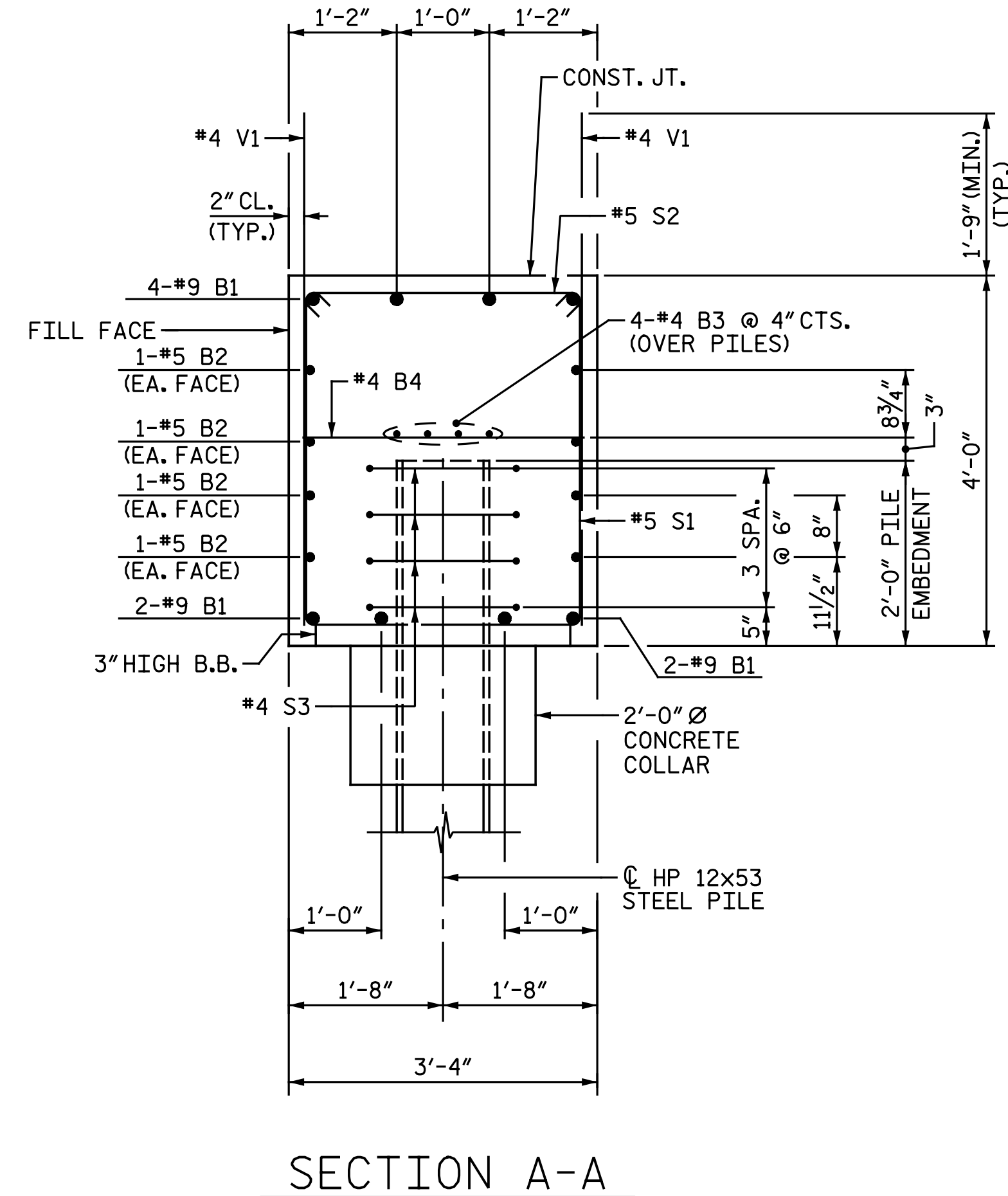
DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED

RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

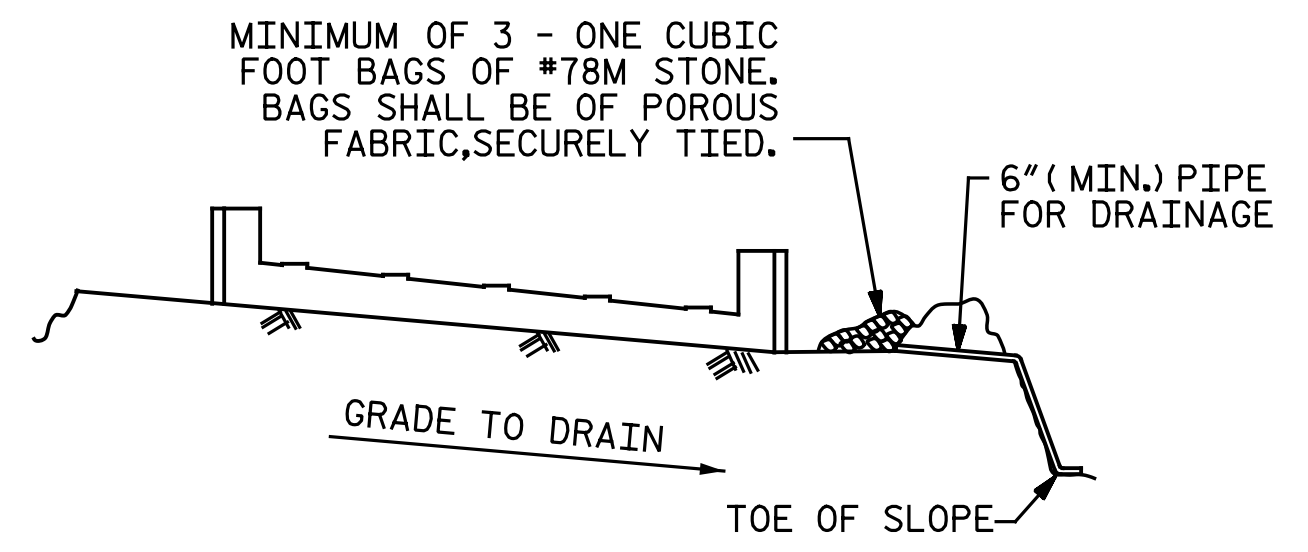
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 Cary, North Carolina 27518
 NC License No.: F-1084

DRAWN BY: M. D. MAYHEW DATE: 4-19-17
 CHECKED BY: V. A. PATEL DATE: 5-15-17

SHEET NO.
S2-17
 TOTAL SHEETS
25



SECTION A-A



BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

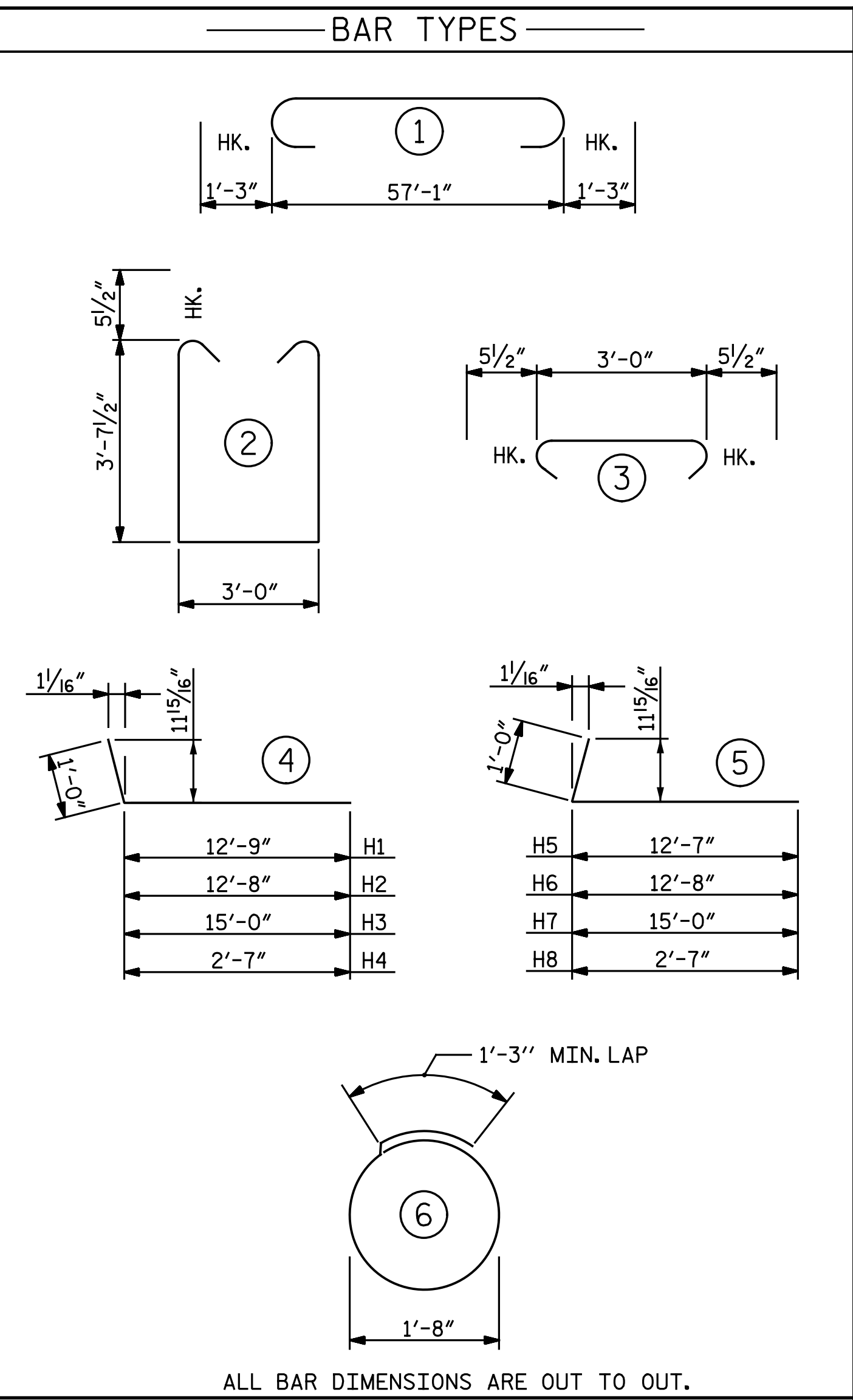
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

DRAWN BY : M. D. MAYHEW DATE : 4-19-17
 CHECKED BY : V. A. PATEL DATE : 5-15-17

BILL OF MATERIAL					
INTEGRAL END BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#9		59' - 7"	1,621
B2	8	#5	STR.	57' - 3"	478
B3	8	#4	STR.	29' - 10"	159
B4	15	#4	STR.	3' - 0"	30
H1	24	#6		13' - 9"	496
H2	16	#6		13' - 8"	328
H3	8	#6		16' - 0"	192
H4	32	#6		3' - 7"	172
H5	24	#6		13' - 7"	490
H6	16	#6		13' - 8"	328
H7	8	#6		16' - 0"	192
H8	32	#6		3' - 7"	172
S1	58	#5		11' - 2"	676
S2	58	#5		3' - 11"	237
S3	40	#4		6' - 6"	174
V1	88	#4	STR.	5' - 7"	328
V2	68	#5	STR.	11' - 5"	810
V3	4	#4	STR.	3' - 7"	10
REINFORCING STEEL				LBS.	6,893
CLASS A CONCRETE					
POUR 1 -					
CAP, LOWER PART OF WINGS & COLLARS					
				C.Y.	34.2
POUR 2 -					
UPPER PART OF WINGS					
				C.Y.	8.6
TOTAL				C.Y.	42.8
PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES					
				EA.	10
HP 12x53 STEEL PILES					
NO. 10				L.F.	800
PILE REDRIVES				EA.	5

NOTE:
 FOR PILE SPLICE DETAILS, SEE "INTEGRAL END BENT 2 DETAILS" SHEET.



ALL BAR DIMENSIONS ARE OUT TO OUT.

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 89+28.52 -L-

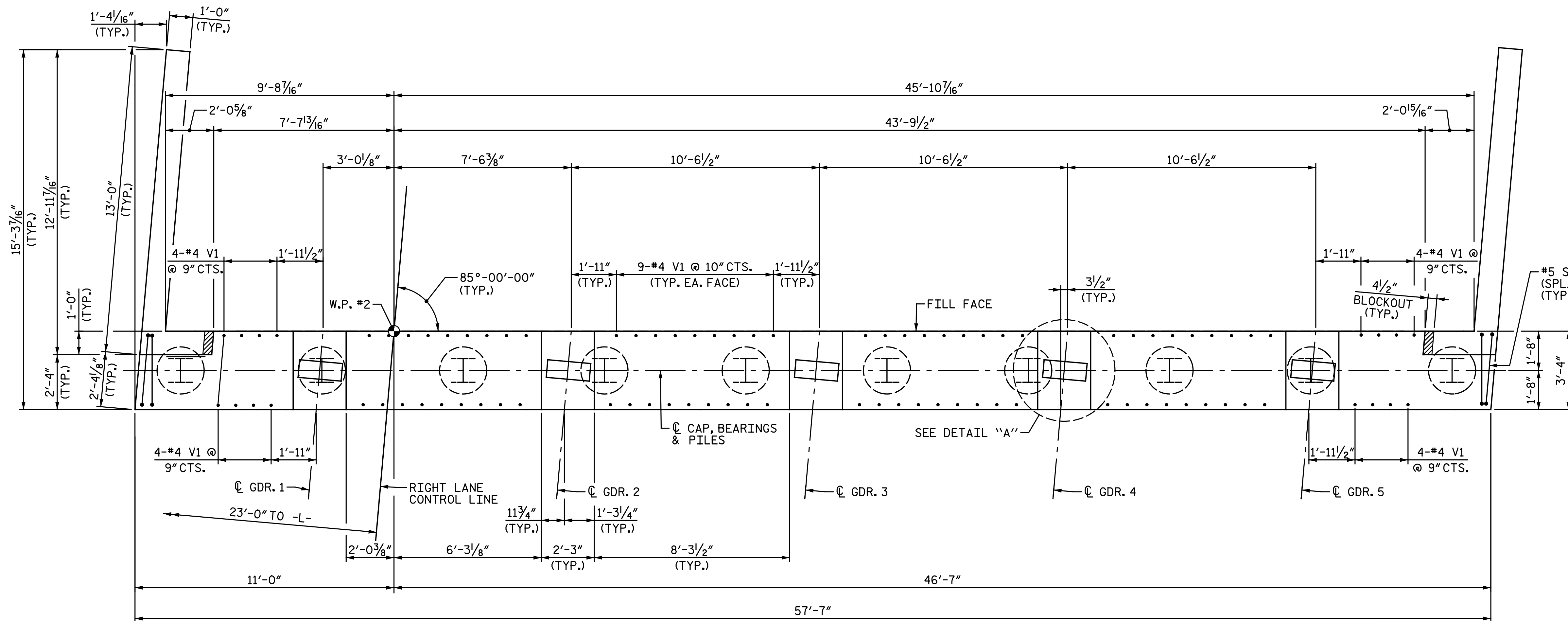


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 1
 DETAILS

DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED

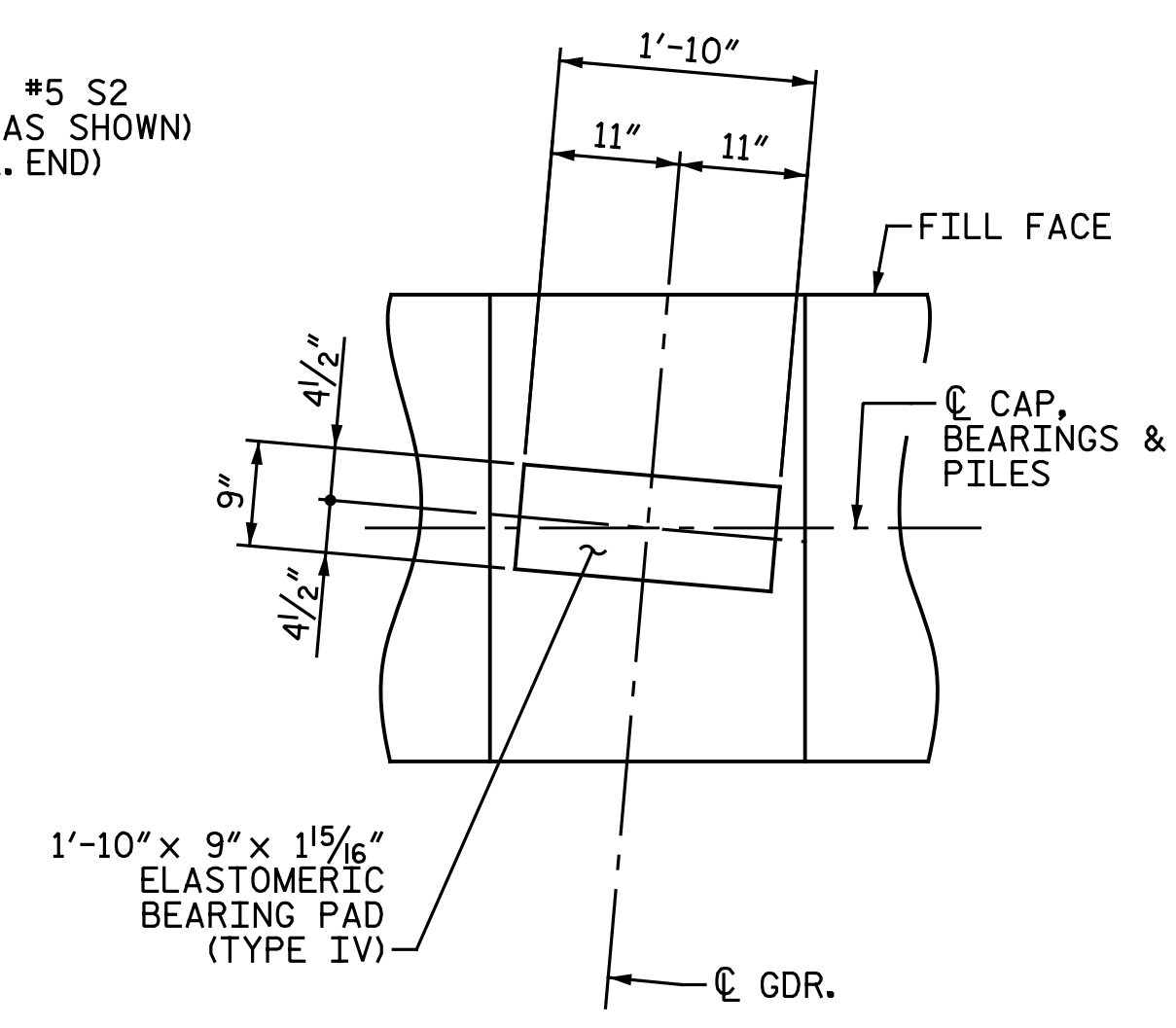
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NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS
2			4			25



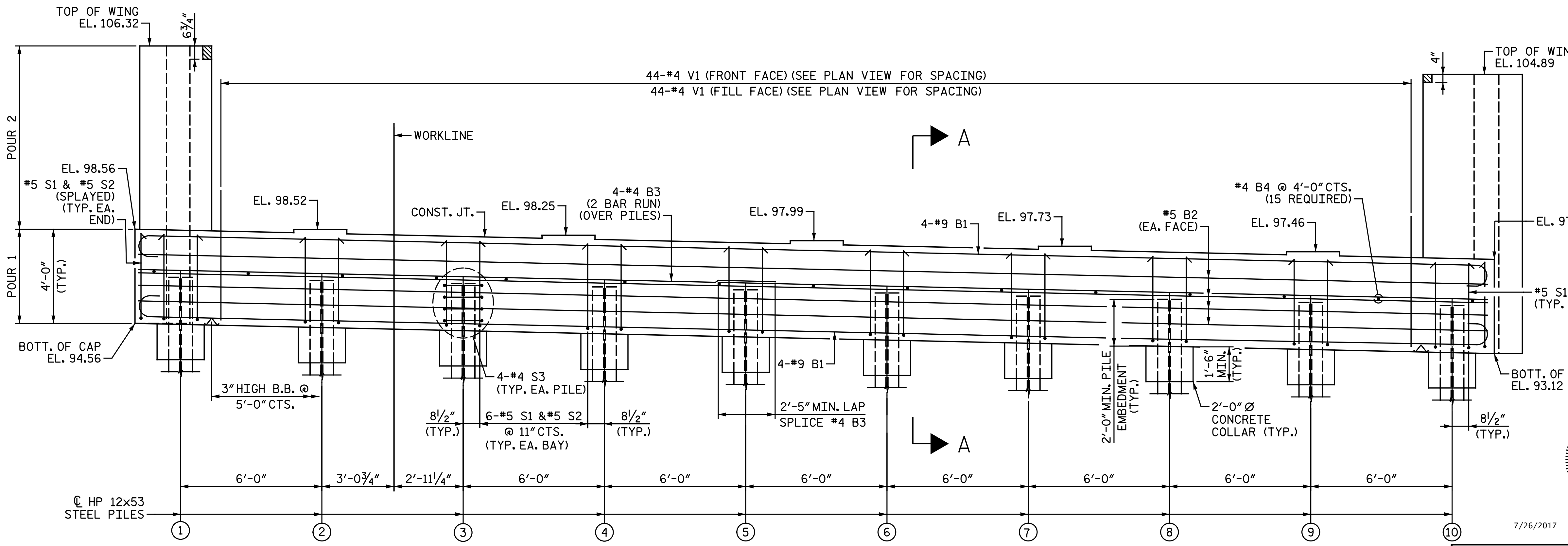
PLAN

NOTES:
 FOR "SECTION A-A", SEE "INTEGRAL END BENT 1 DETAILS" SHEET.
 STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #4 V1 BARS.
 THE TOP SURFACE OF THE END BENT CAP, EXCLUDING THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".
 THE CONCRETE IN THE HATCHED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.



DETAIL "A"

ALL DIMENSIONS AND DETAILS SHOWN ARE TYPICAL FOR ALL BEARINGS AT EACH BRIDGE SEAT LOCATION.



ELEVATION

TOP OF PILE ELEVATIONS	
PILE	ELEVATION
①	96.56
②	96.41
③	96.25
④	96.10
⑤	95.95
⑥	95.80
⑦	95.65
⑧	95.50
⑨	95.35
⑩	95.20

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-
 SHEET 1 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 2

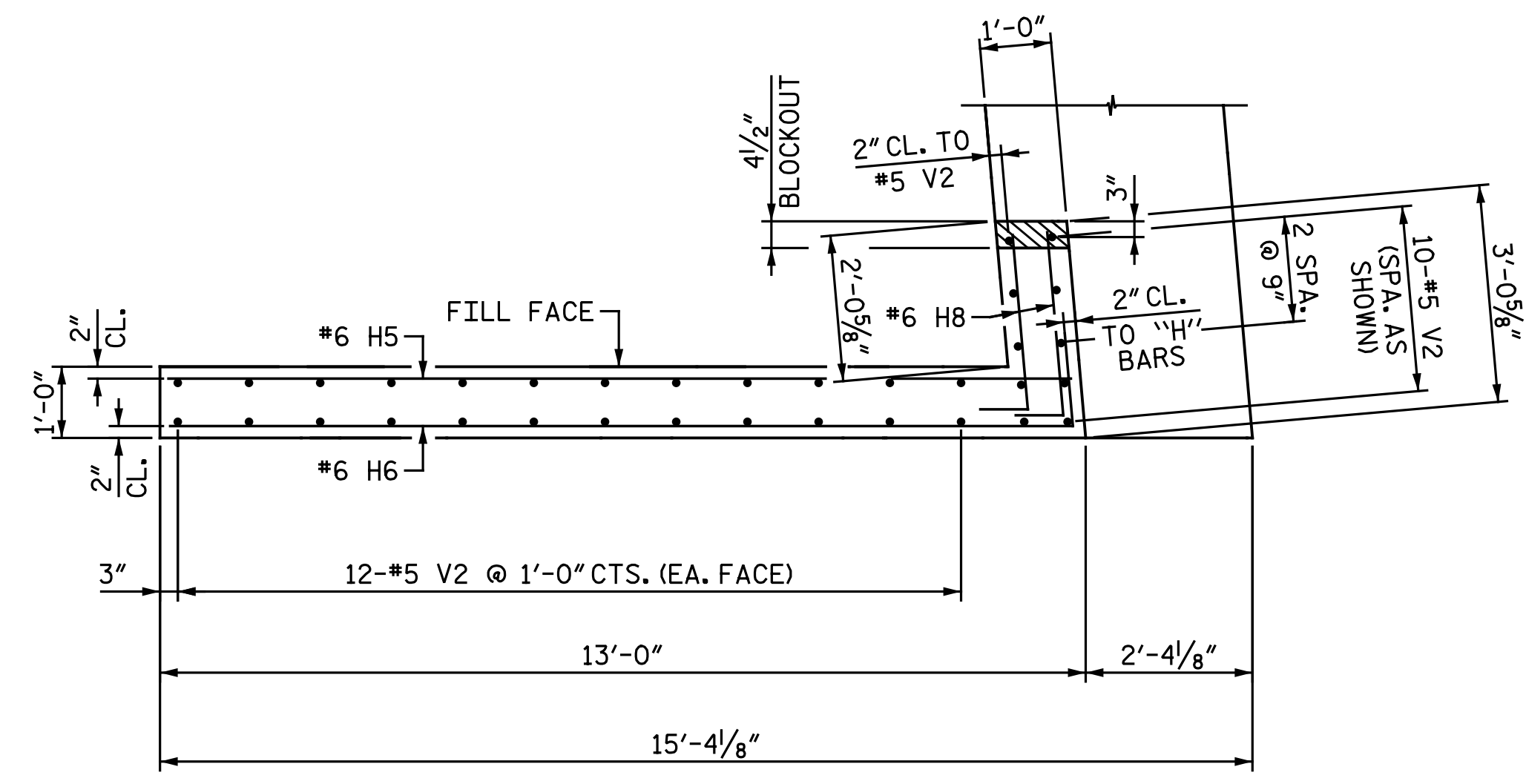
RIGHT LANE

DRAWN BY: M. D. MAYHEW DATE: 4-18-17
 CHECKED BY: V. A. PATEL DATE: 5-16-17

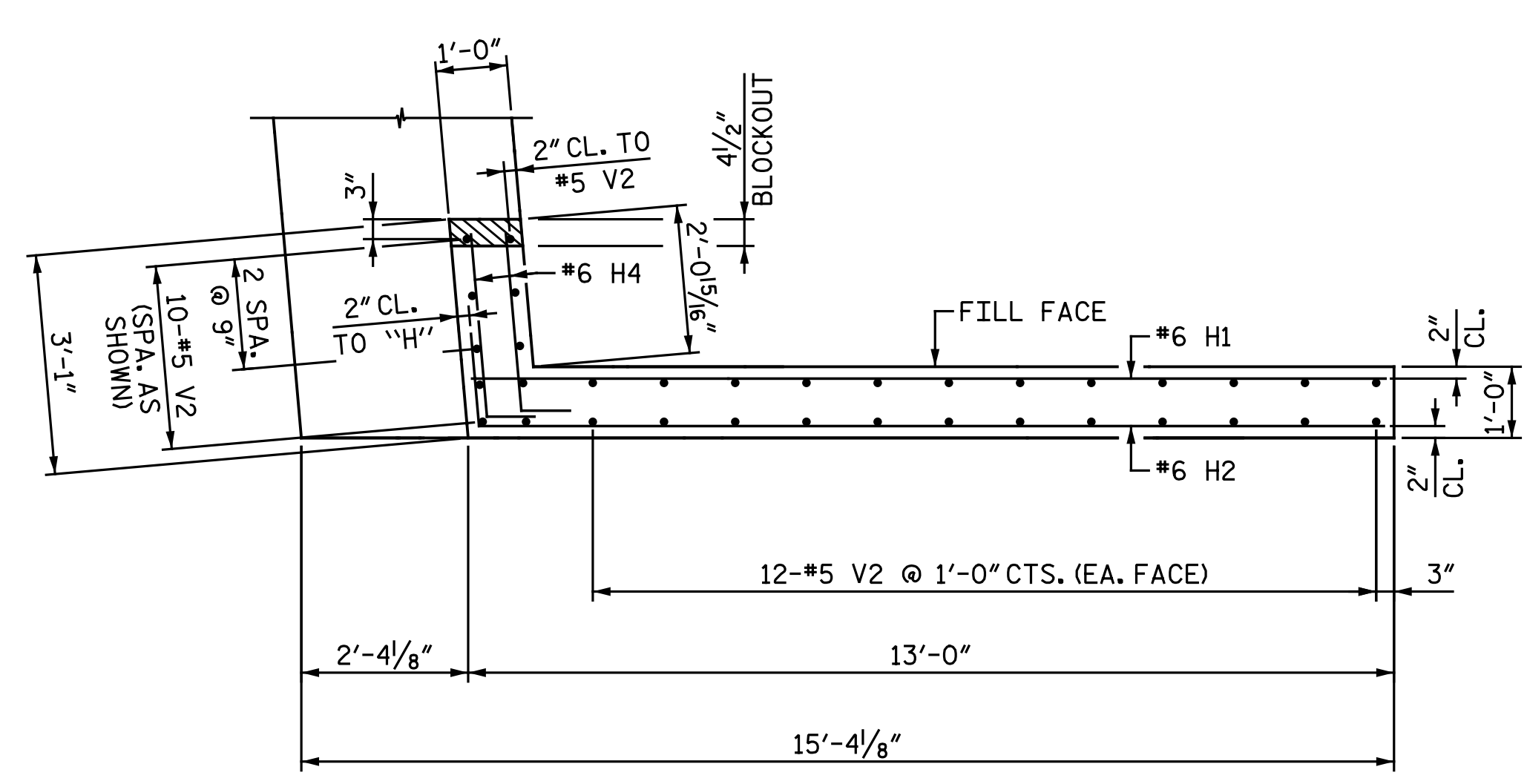
DOCUMENT NOT CONSIDERED FINAL
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NO.	BY:	DATE:	REVISIONS			SHEET NO.
			NO.	BY:	DATE:	
1			3			S2-19 TOTAL SHEETS 25
2			4			

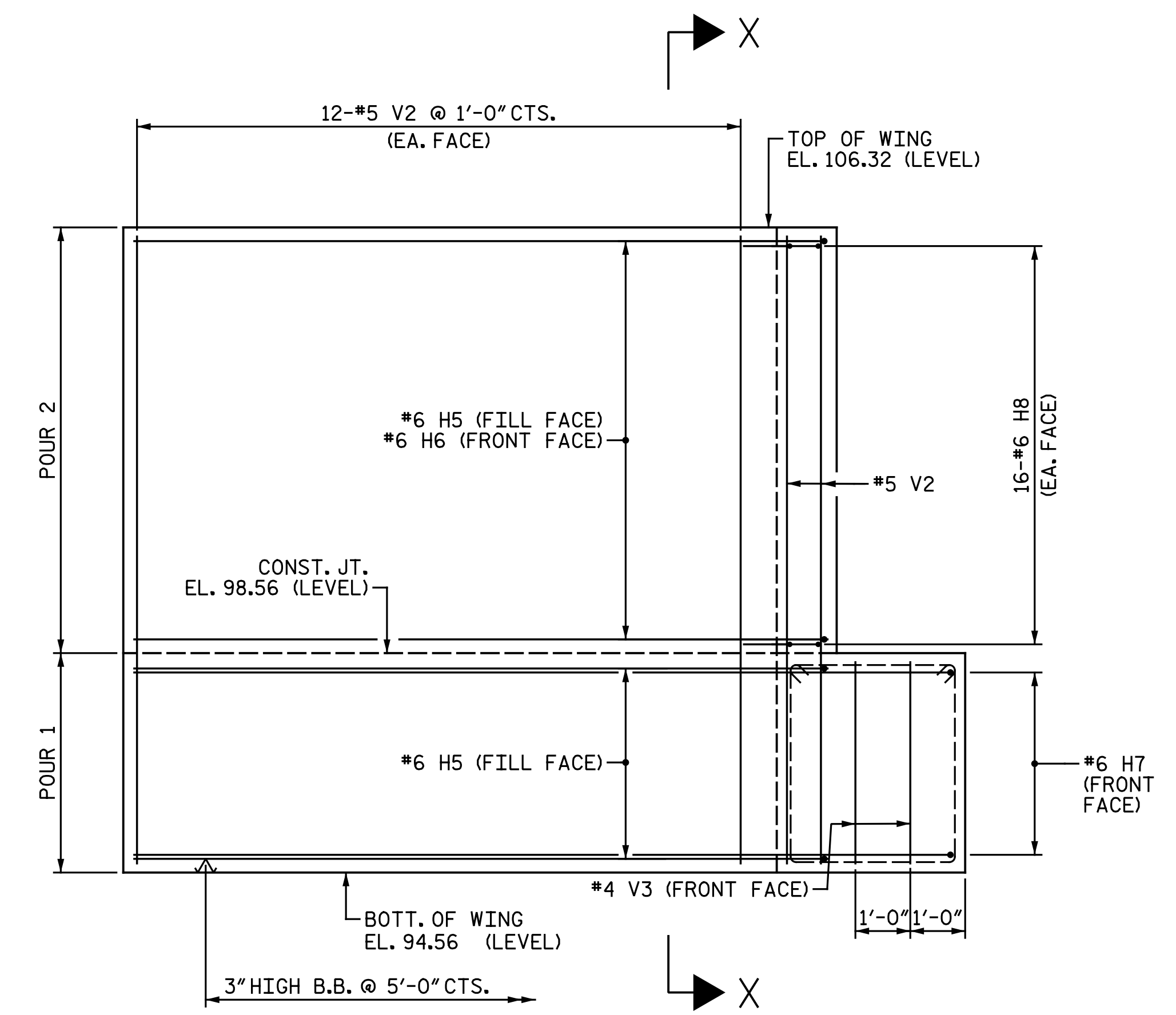
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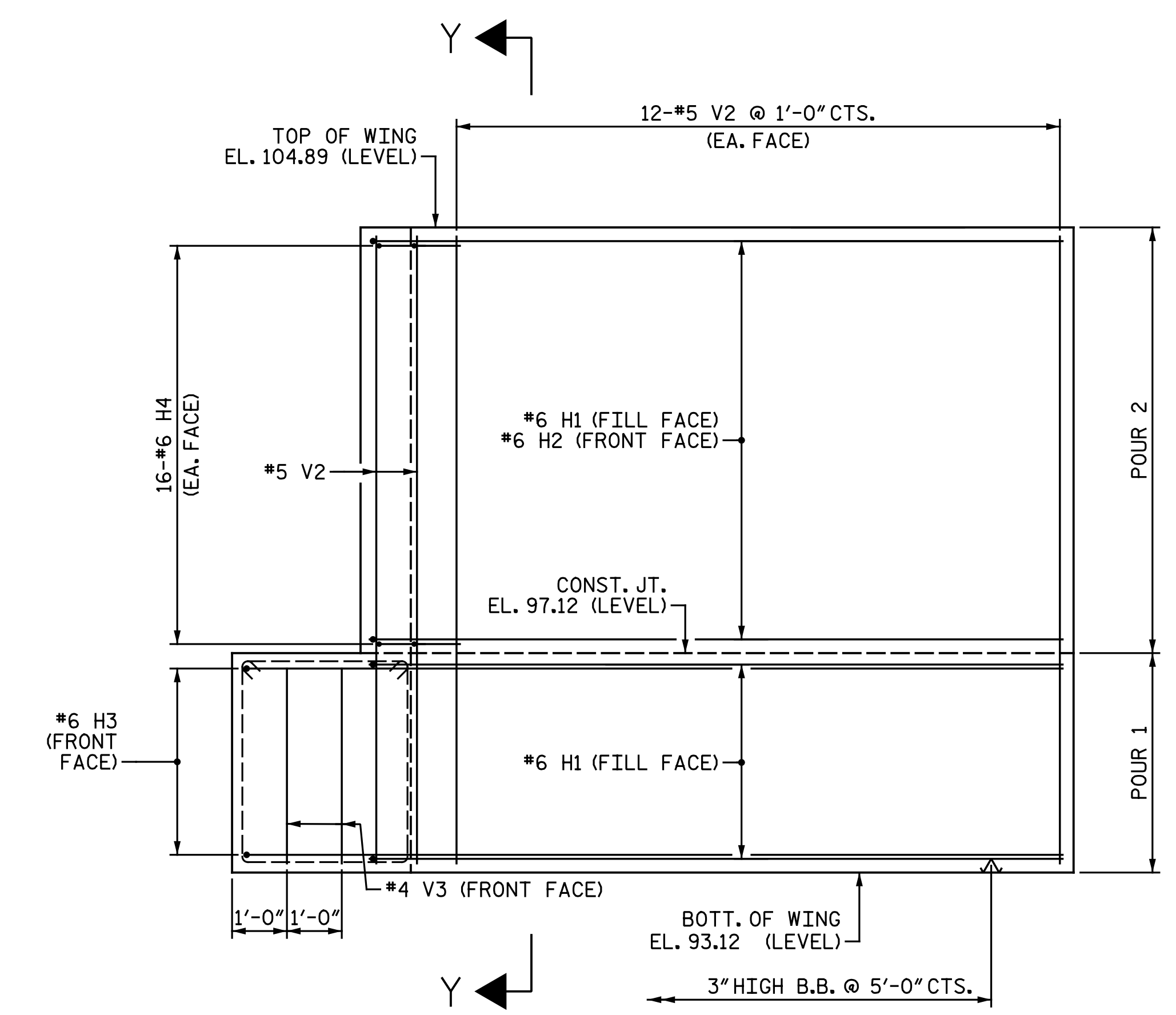
PLAN OF LEFT WING
(H7 BARS NOT SHOWN FOR CLARITY)



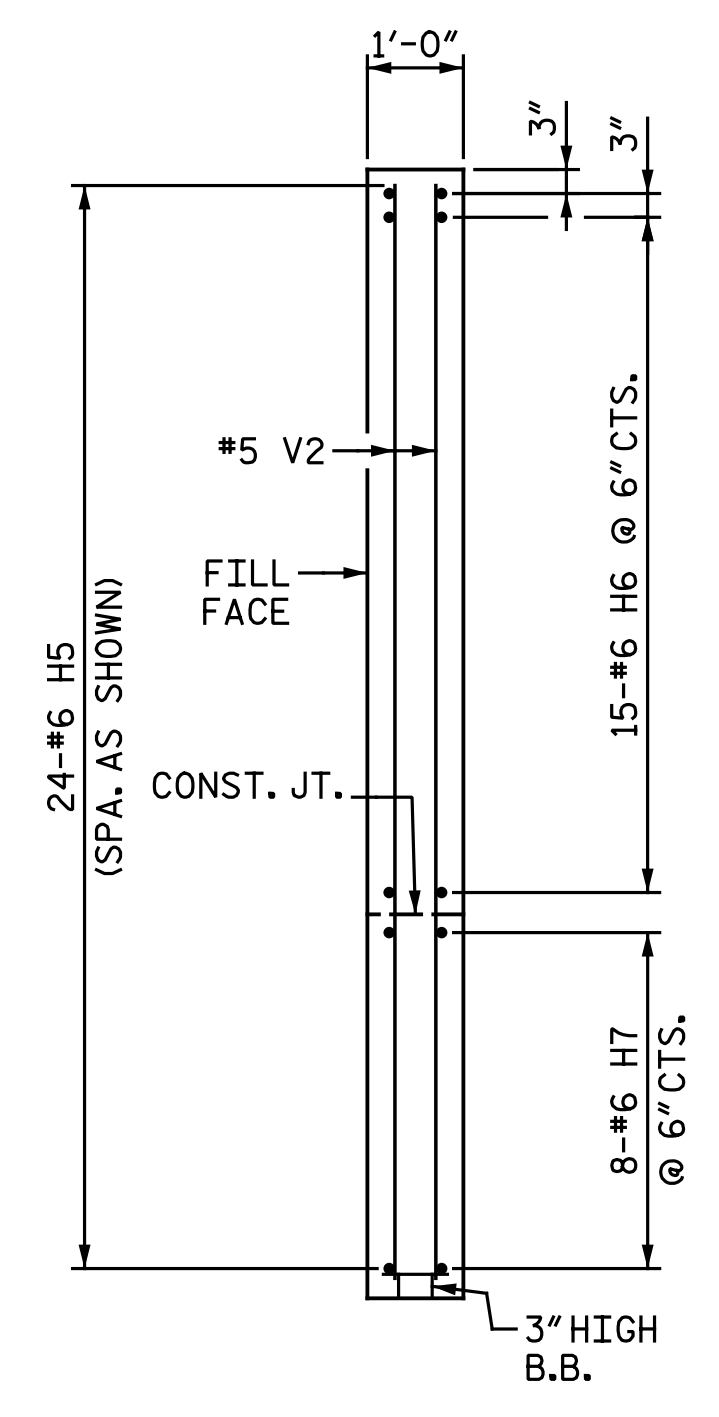
PLAN OF RIGHT WING
(H3 BARS NOT SHOWN FOR CLARITY)



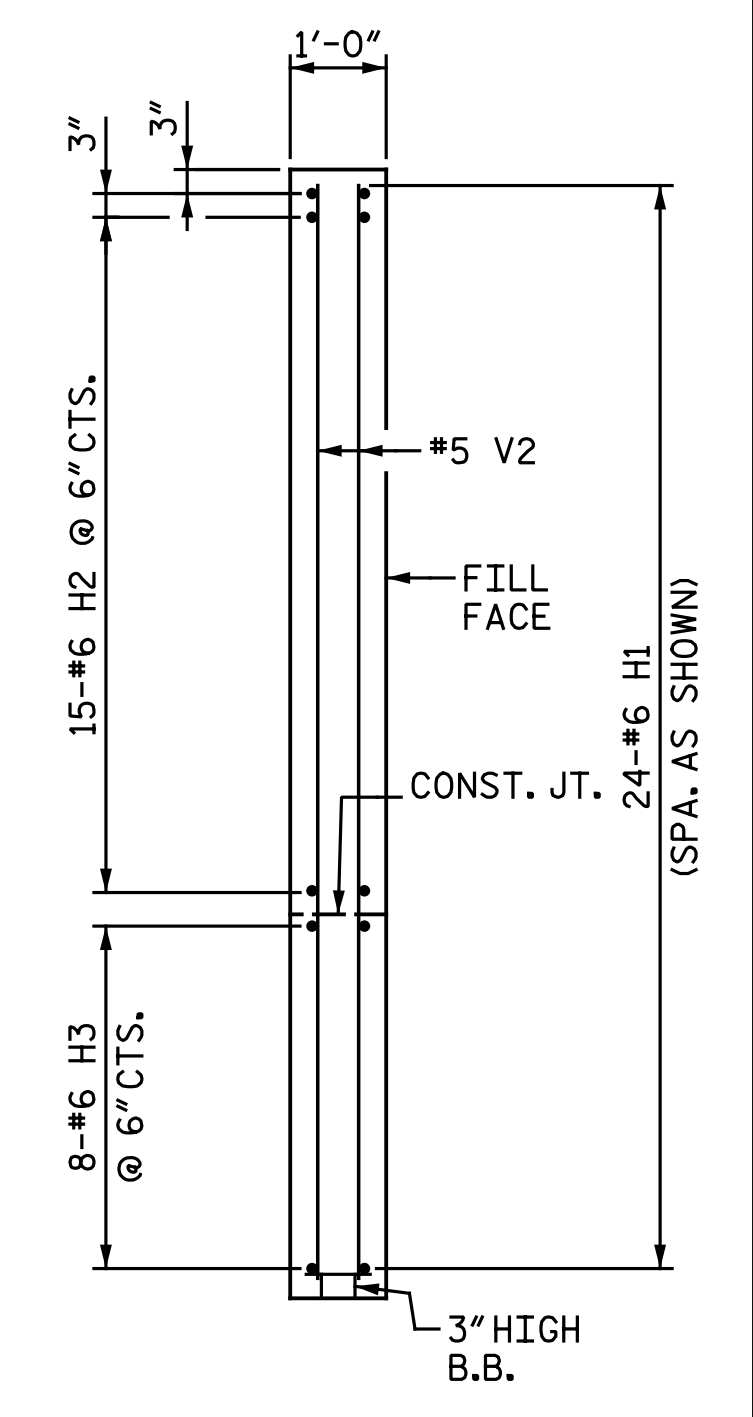
ELEVATION OF LEFT WING



ELEVATION OF RIGHT WING



SECTION X-X



SECTION Y-Y

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 89+28.52 -L-
 SHEET 2 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 2

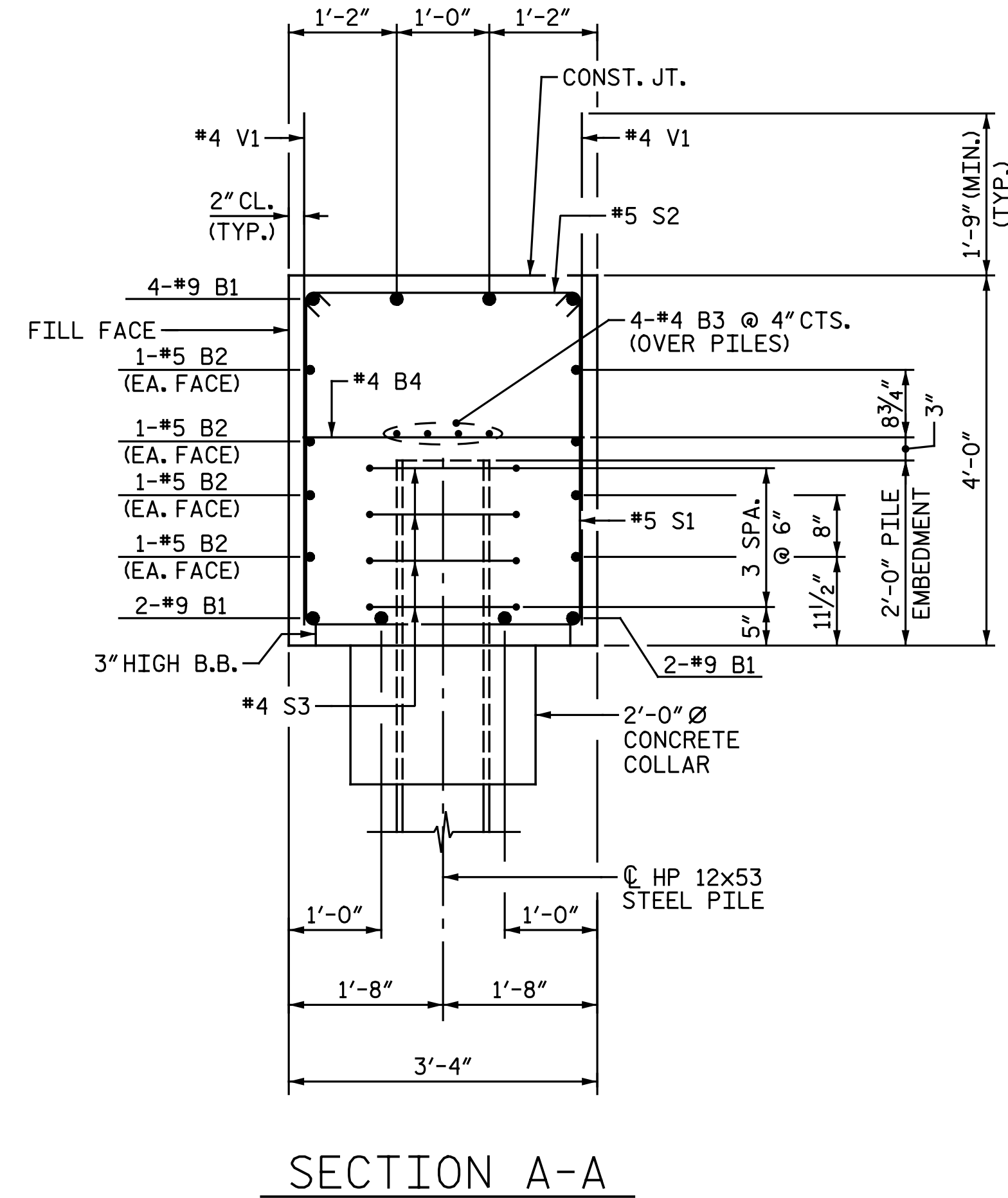
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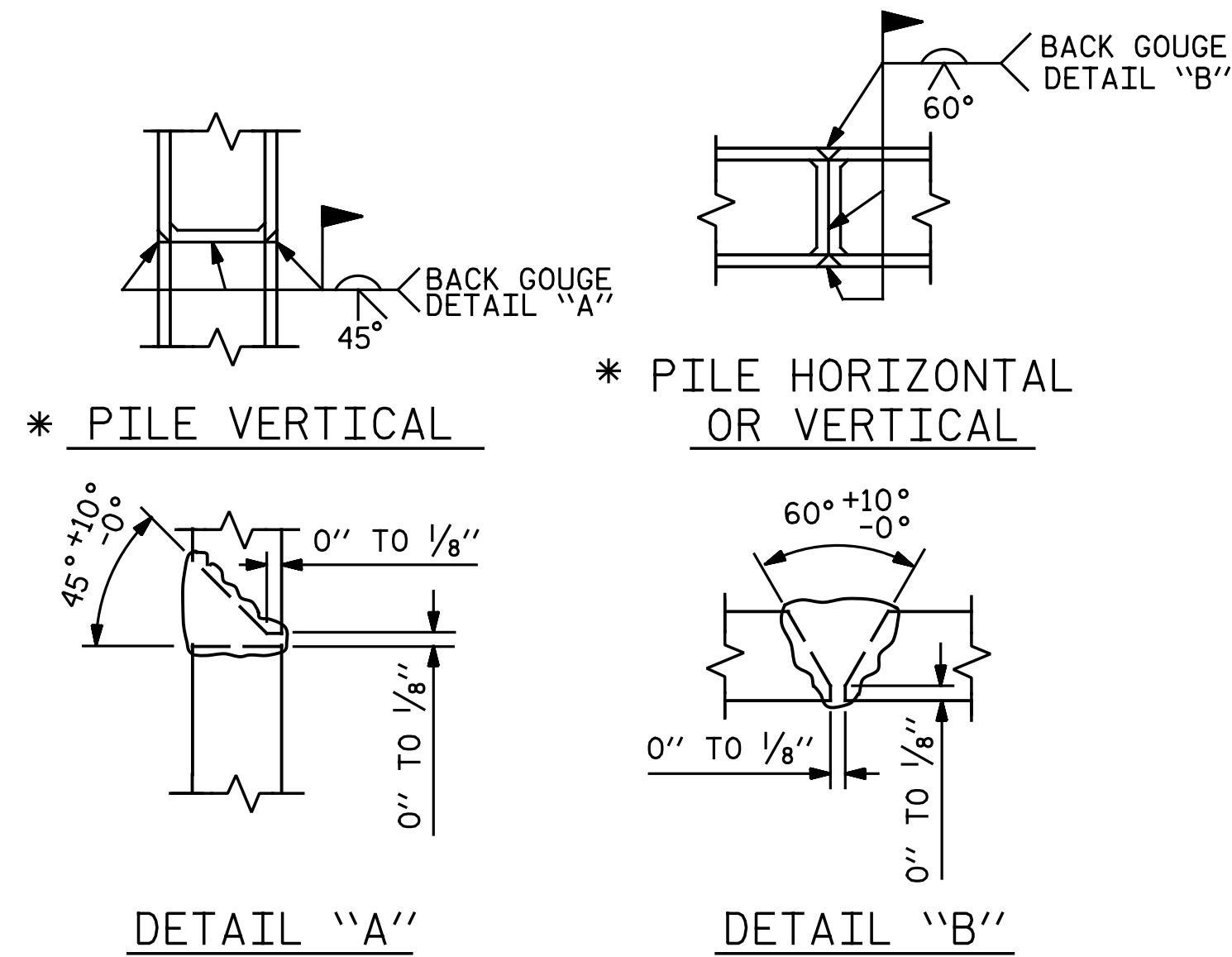
RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

DRAWN BY: M. D. MAYHEW DATE: 4-19-17
 CHECKED BY: V. A. PATEL DATE: 5-16-17

SHEET NO.
S2-20
 TOTAL SHEETS
25



SECTION A-A



PILE SPLICE DETAILS

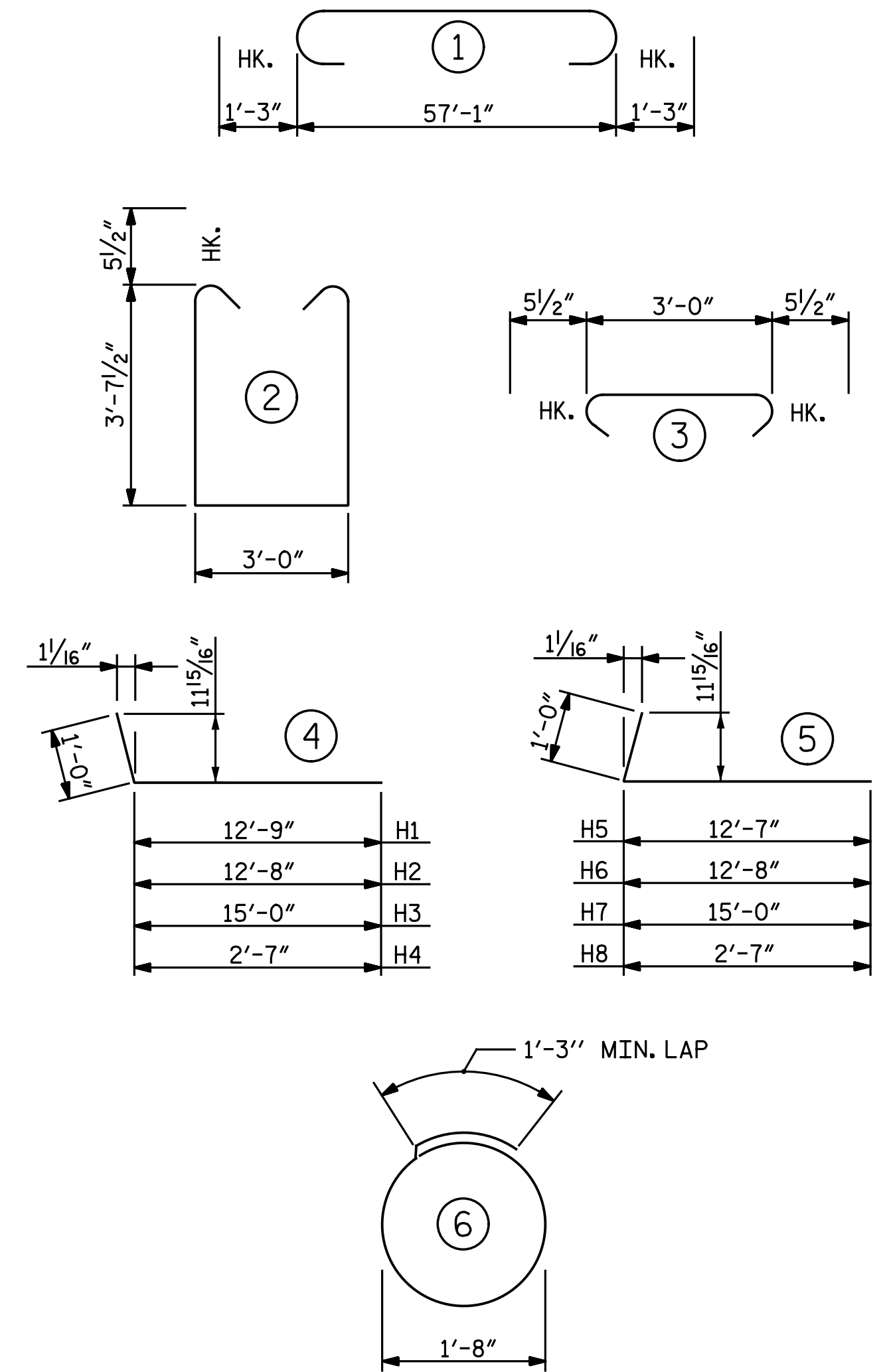
* POSITION OF PILE DURING WELDING

BILL OF MATERIAL					
INTEGRAL END BENT 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#9		59' - 7"	1,621
B2	8	#5	STR.	57' - 3"	478
B3	8	#4	STR.	29' - 10"	159
B4	15	#4	STR.	3' - 0"	30
H1	24	#6	4	13' - 9"	496
H2	16	#6	4	13' - 8"	328
H3	8	#6	4	16' - 0"	192
H4	32	#6	4	3' - 7"	172
H5	24	#6	5	13' - 7"	490
H6	16	#6	5	13' - 8"	328
H7	8	#6	5	16' - 0"	192
H8	32	#6	5	3' - 7"	172
S1	58	#5	2	11' - 2"	676
S2	58	#5	3	3' - 11"	237
S3	40	#4	6	6' - 6"	174
V1	88	#4	STR.	5' - 7"	328
V2	68	#5	STR.	11' - 5"	810
V3	4	#4	STR.	3' - 7"	10
REINFORCING STEEL				LBS. 6,893	
CLASS A CONCRETE					
POUR 1 -					
CAP, LOWER PART OF WINGS & COLLARS					
				C.Y.	34.2
POUR 2 -					
UPPER PART OF WINGS					
				C.Y.	8.6
TOTAL				C.Y.	42.8
PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES					
				EA.	10
HP 12x53 STEEL PILES					
NO. 10				L.F.	800
PILE REDRIVES				EA.	5

NOTE:

FOR TEMPORARY DRAINAGE AT END BENT, SEE "INTEGRAL END BENT 1 DETAILS" SHEET.

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT.

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-



DocuSigned by:
Bradley J. Bell

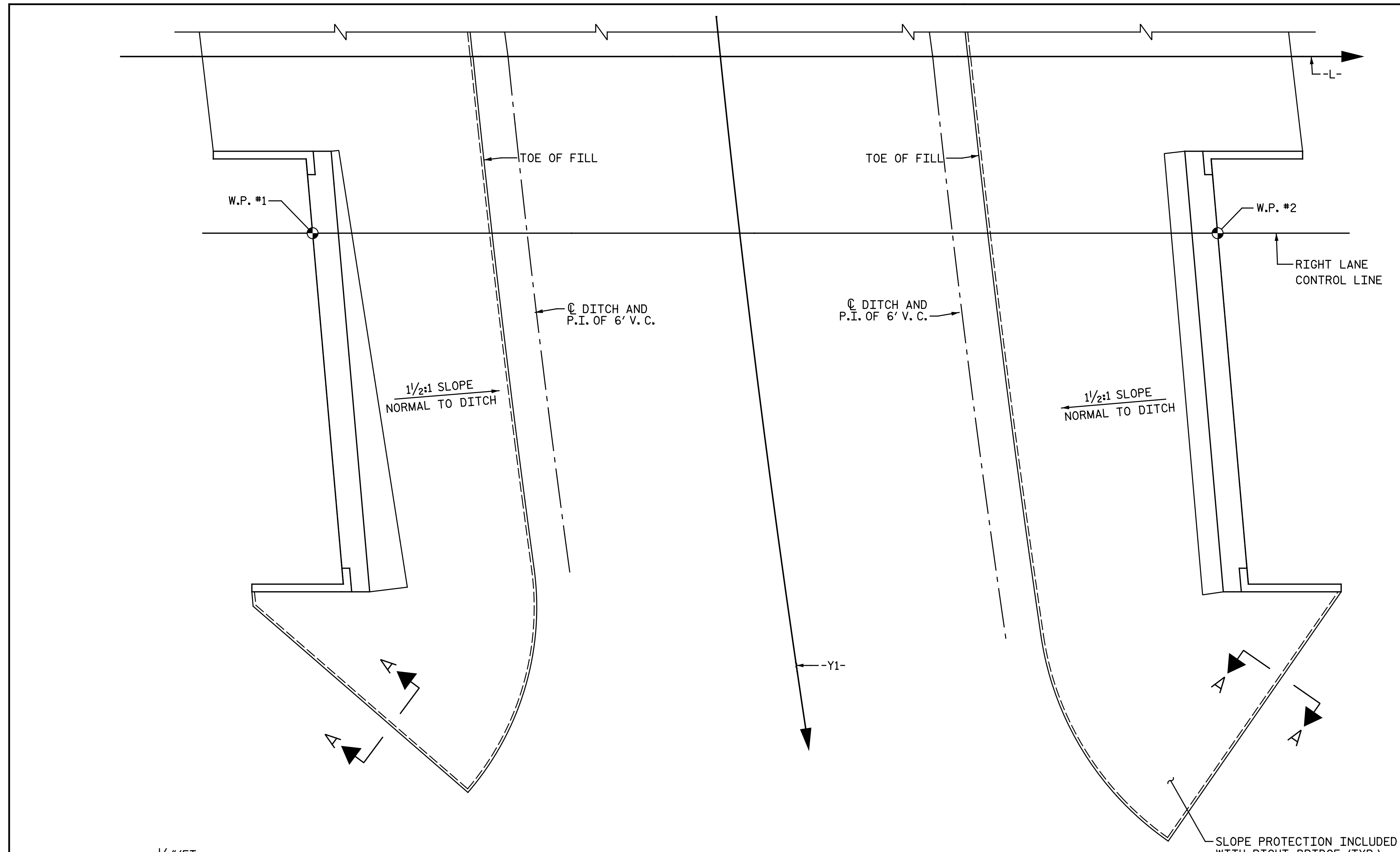
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DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE
INTEGRAL END BENT 2
DETAILS
RIGHT LANE

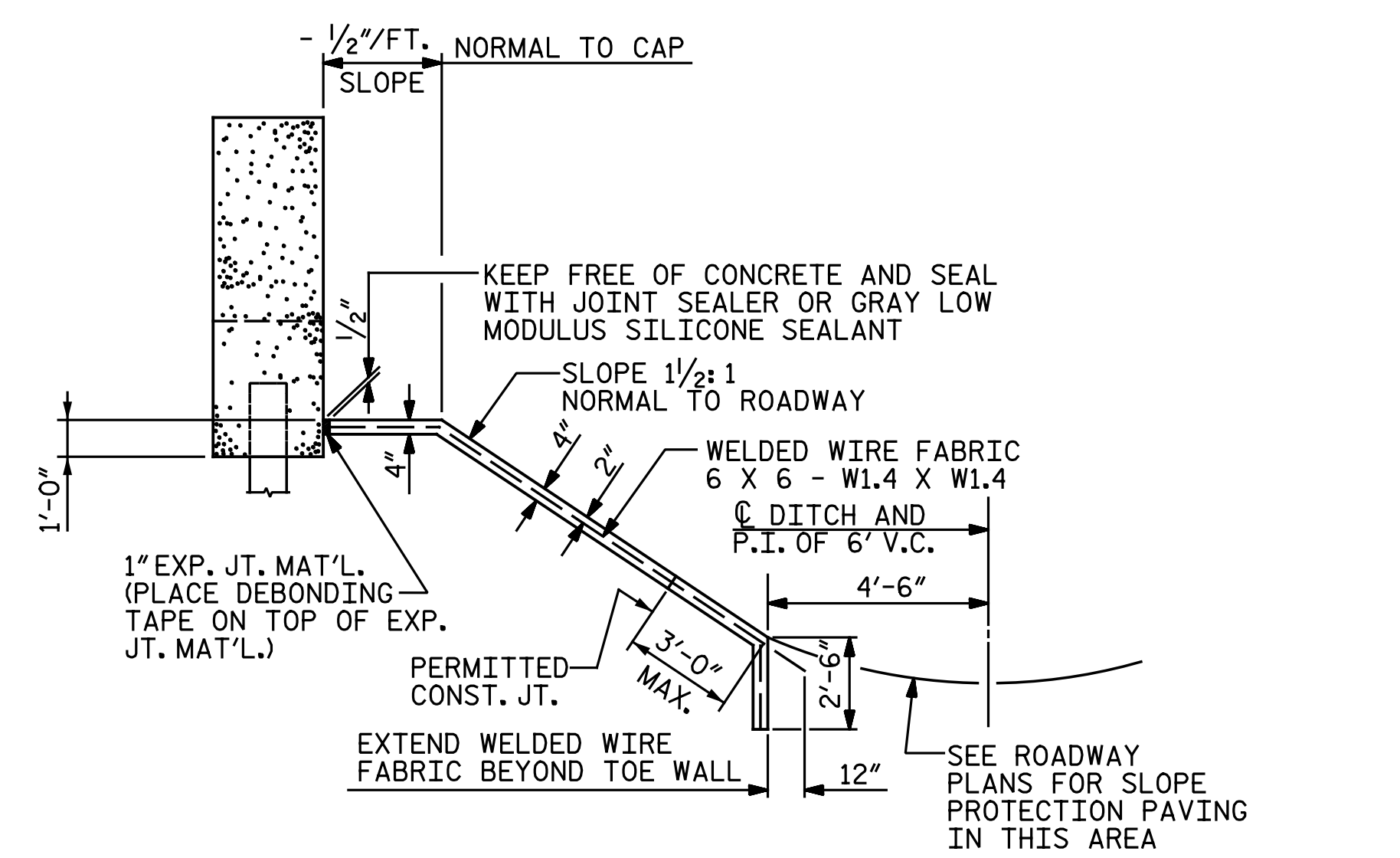
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-21
1			3			TOTAL SHEETS
2			4			25

DRAWN BY: M. D. MAYHEW DATE: 4-19-17
CHECKED BY: V. A. PATEL DATE: 5-16-17

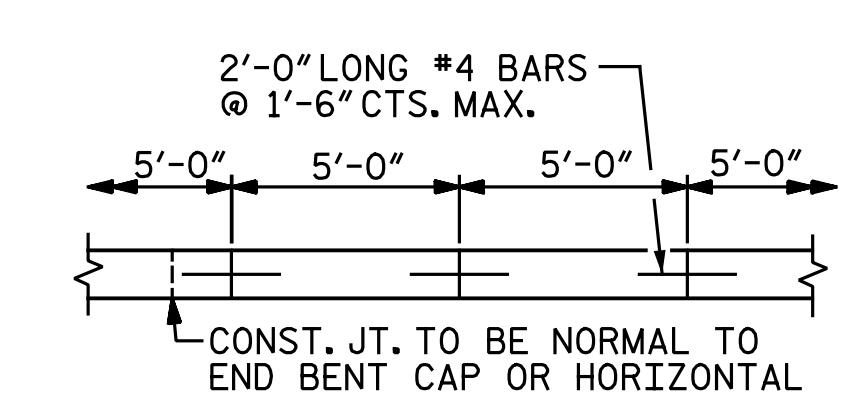
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Cary, North Carolina 27518
NC License No.: F-1084



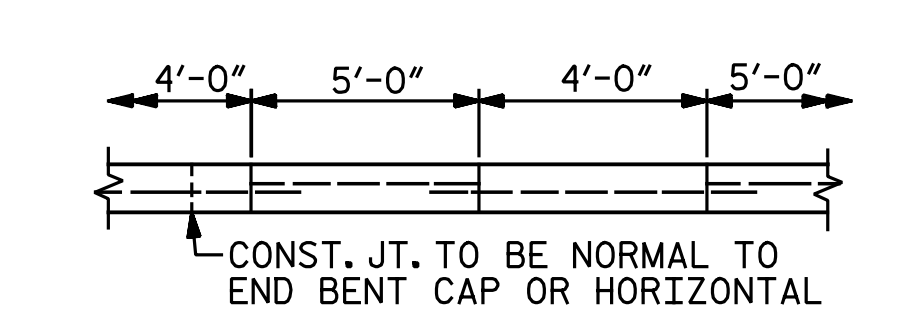
PLAN



SECTION ALONG CONTROL LINE WHEN FILL CATCHES IN DITCH



POURING DETAIL



OPTIONAL POURING DETAIL

SLOPE PROTECTION DETAILS

GENERAL NOTES:

STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT.

MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.

FOR BERM WIDTHS AND ELEVATIONS, SEE GENERAL DRAWING AND "SLOPE PROTECTION DETAILS" SHEET 2 OF 2.

SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET.

CONCRETE SHALL BE CLASS "B".

THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED.

WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE.

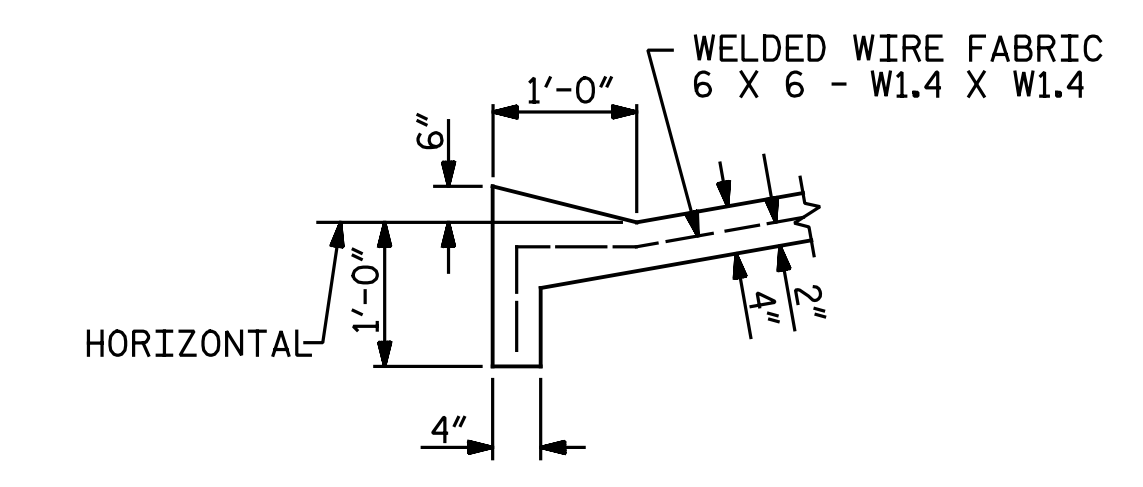
SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0" LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING.

SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6".

THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA. 89+28.52 -L- (LEFT LANE)	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX. L.F.
END BENT 1	350	630
END BENT 2	430	775

* QUANTITY SHOWN IS BASED ON 5' POURS.



SECTION A-A

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 89+28.52 -L-

SHEET 1 OF 2



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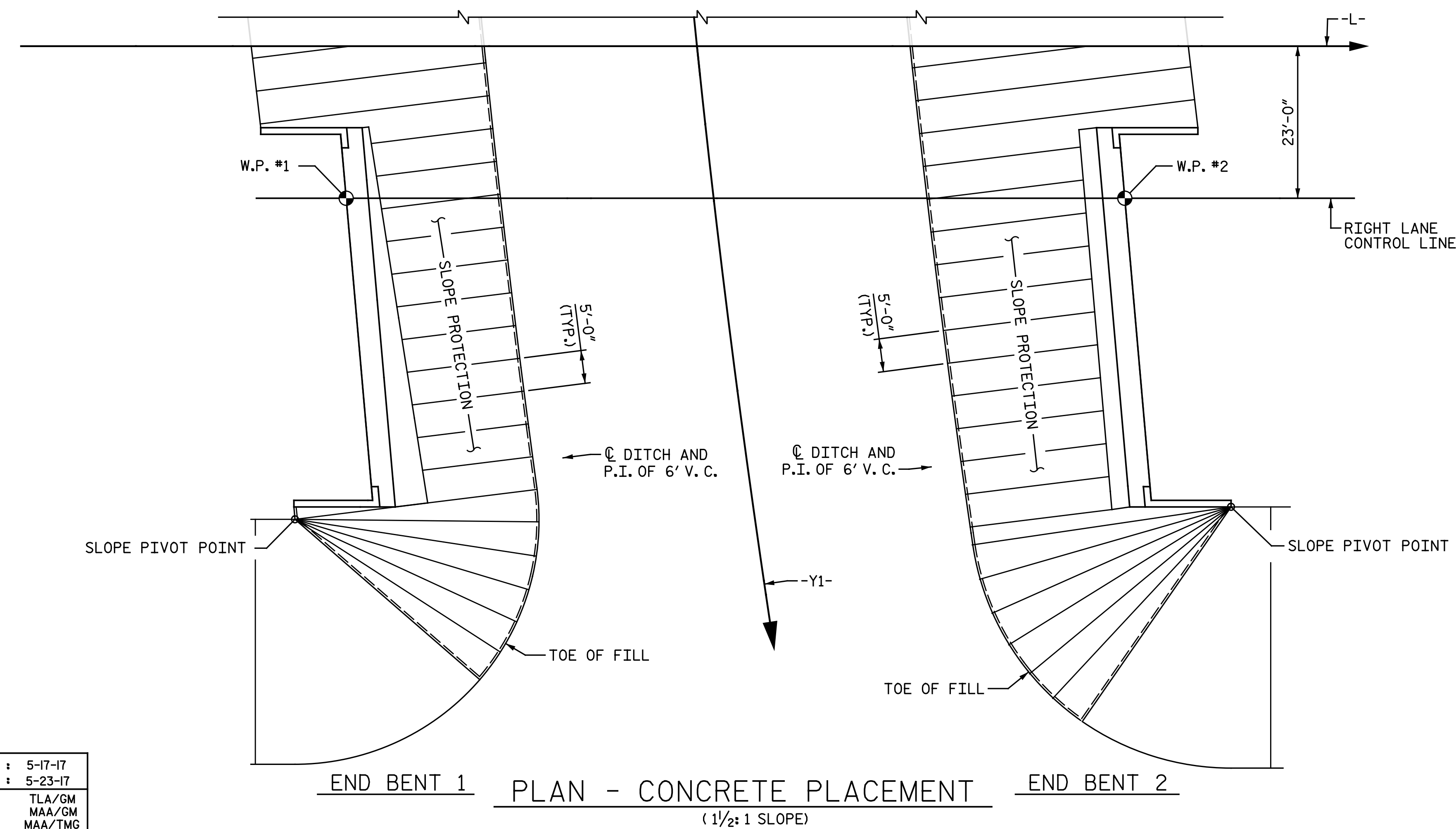
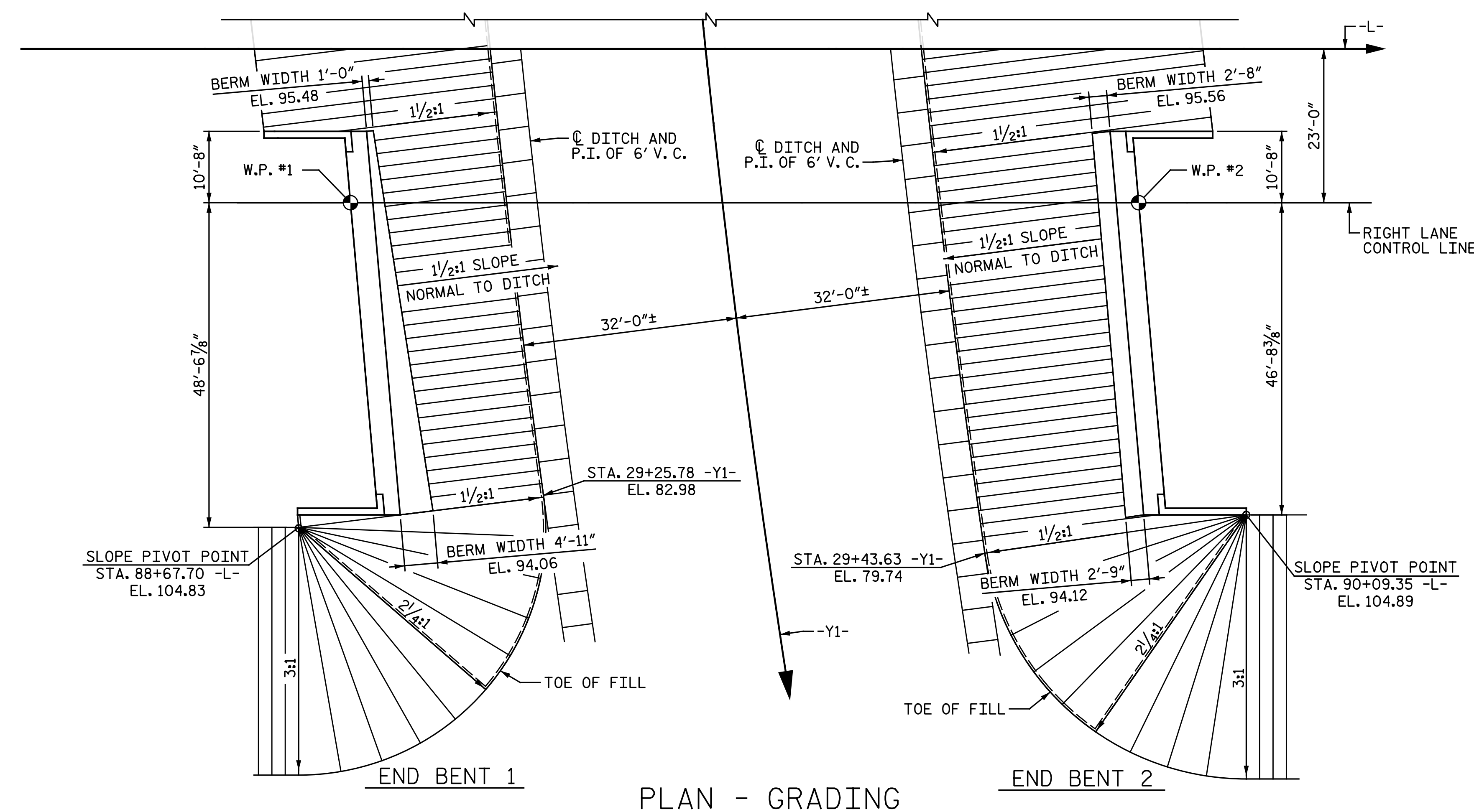
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 SLOPE PROTECTION
 DETAILS
 RIGHT LANE

ASSEMBLED BY : C. E. MAYHEW	DATE : 4-24-17
CHECKED BY : V. A. PATEL	DATE : 5-23-17
DRAWN BY : ELR 5/92	REV. 10/1/11 MAA/GM
CHECKED BY : GRP 6/92	REV. 12/21/11 MAA/GM
	REV. 1/16 MAA/TMG

NO.	BY:	DATE:	REVISIONS			SHEET NO.
			NO.	BY:	DATE:	
1			3			TOTAL SHEETS 25
2			4			

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 NC License No. : F-1084

NOTE:
ALL ELEVATIONS AND BERM WIDTHS ARE GIVEN AT THE TOP OF CONCRETE SLOPE PROTECTION.



ASSEMBLED BY : C. E. MAYHEW	DATE : 5-17-17
CHECKED BY : V. A. PATEL	DATE : 5-23-17
DRAWN BY : WJH 10/88	REV. 5/1/06 TLA/GM
CHECKED BY : FCJ 10/88	REV. 10/1/11 MAA/GM
	REV. 1/16 MAA/TMG

7/26/2017

Seal: NORTH CAROLINA PROFESSIONAL ENGINEER VIKAL A. PATEL 023371

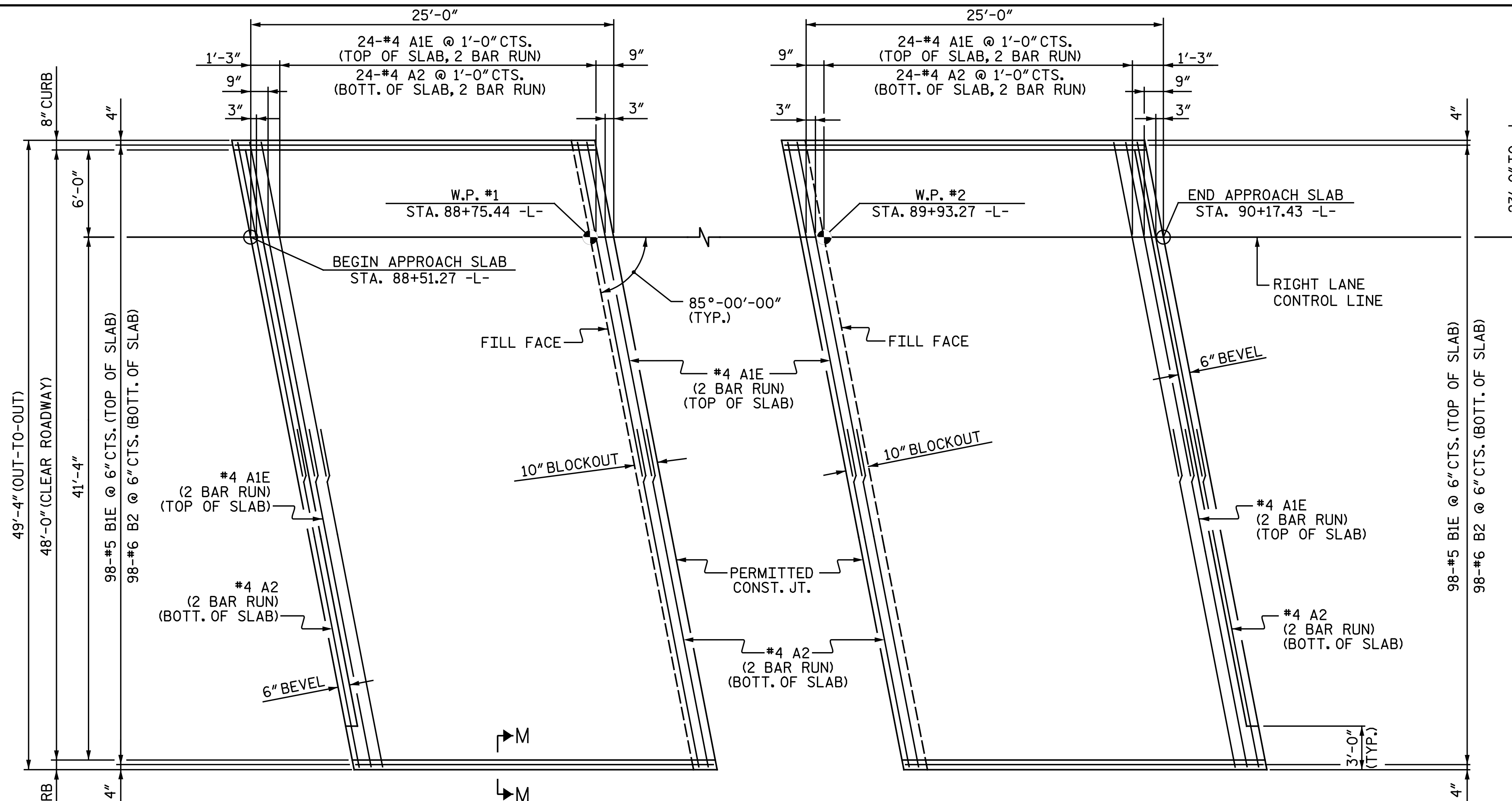
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NC License No.: F-1084

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PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-
SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD SLOPE PROTECTION DETAILS RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					SHEET NO. S2-23
					TOTAL SHEETS 25



PLAN @ INTEGRAL END BENT 1 PLAN @ INTEGRAL END BENT 2

NOTES:

AT THE CONTRACTOR'S OPTION, THE APPROACH SLAB MAY BE CAST MONOLITHICALLY WITH THE INTEGRAL END BENT DIAPHRAGM AND THE END SECTION OF BRIDGE DECK. IF CAST WITH THE INTEGRAL DIAPHRAGM, THE LAYERS OF ROOFING FELT SHALL BE OMITTED. IF CAST SEPARATE FROM THE INTEGRAL DIAPHRAGM, APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWS NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

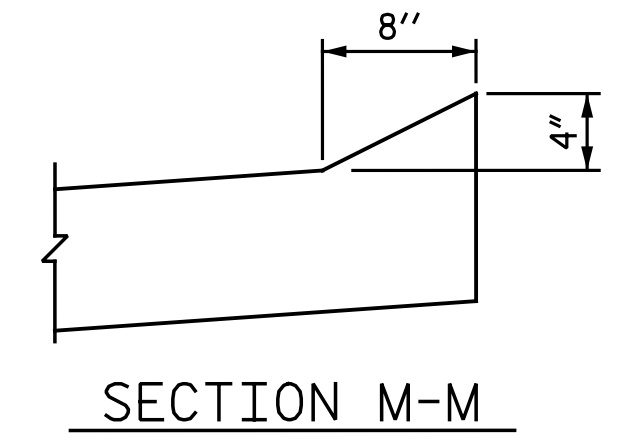
FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

#78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

#78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

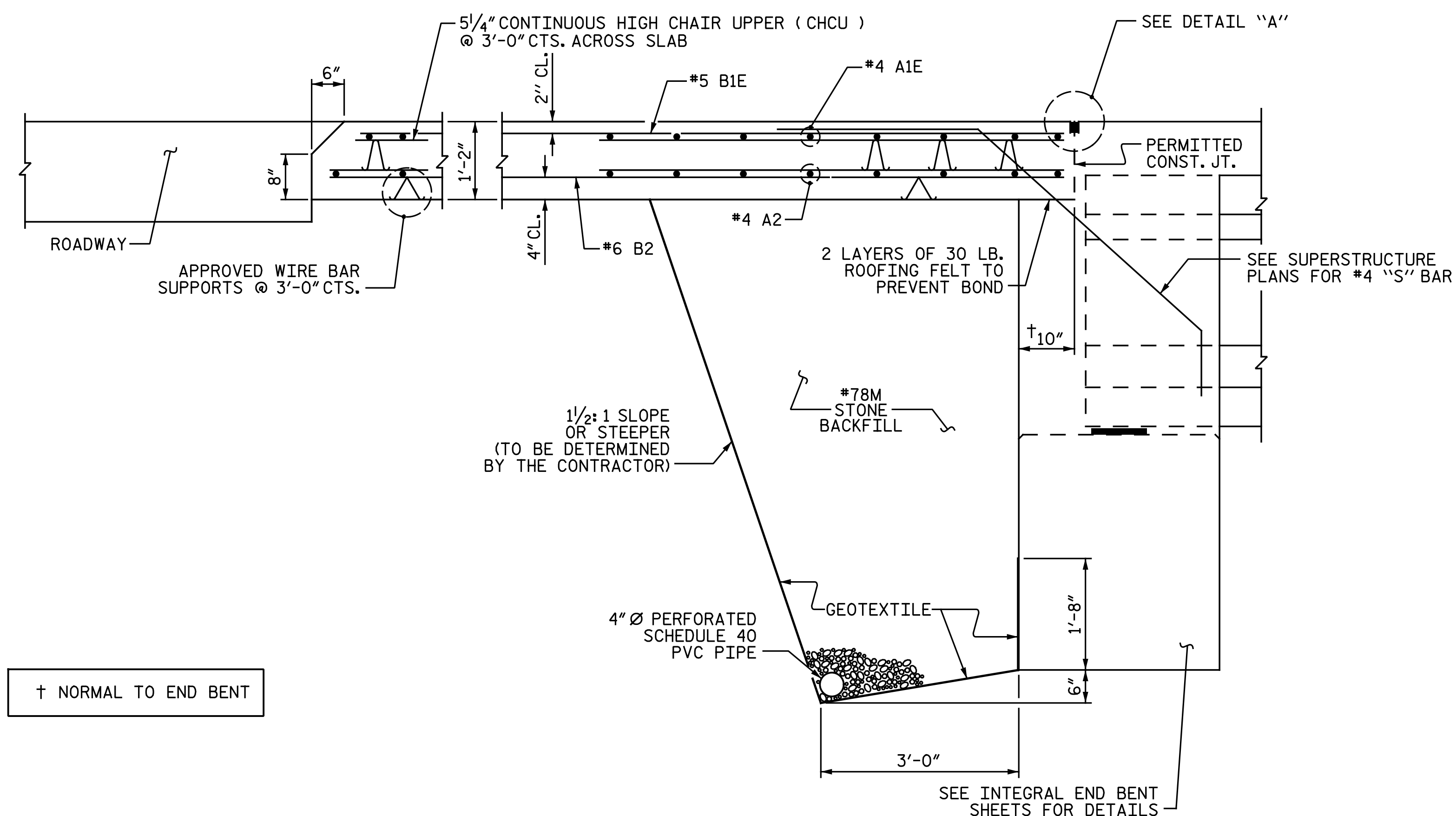


BILL OF MATERIAL					
APPROACH SLAB AT END BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1E	52	#4	STR.	25' - 8"	892
A2	52	#4	STR.	25' - 6"	886
B1E	98	#5	STR.	24' - 2"	2,470
B2	98	#6	STR.	24' - 8"	3,631
REINFORCING STEEL					LBS. 4,517
EPOXY COATED REINFORCING STEEL					LBS. 3,362
CLASS AA CONCRETE					C.Y. 53.3

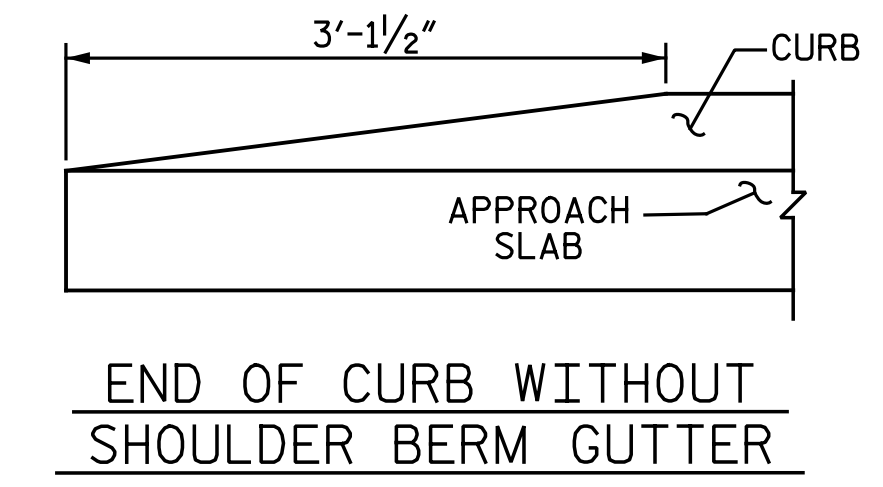
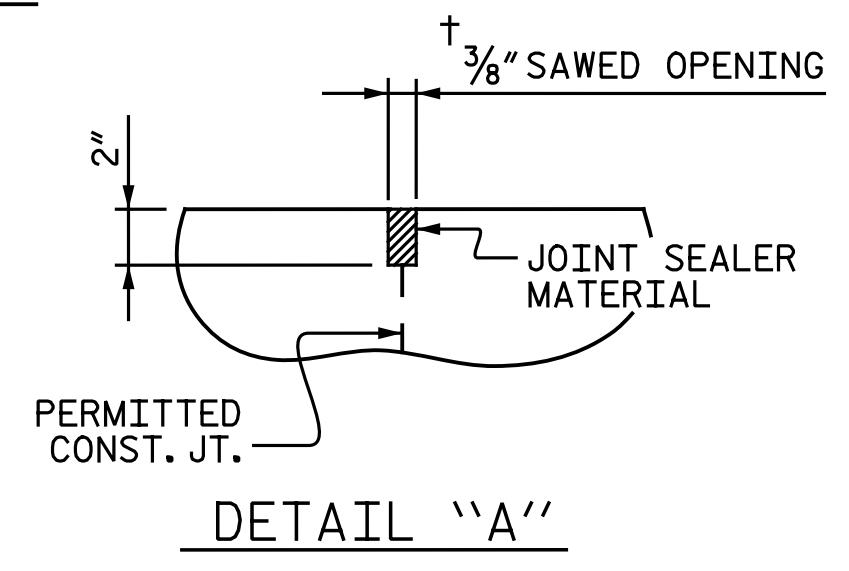
BILL OF MATERIAL					
APPROACH SLAB AT END BENT 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1E	52	#4	STR.	25' - 8"	892
A2	52	#4	STR.	25' - 6"	886
B1E	98	#5	STR.	24' - 2"	2,470
B2	98	#6	STR.	24' - 8"	3,631
REINFORCING STEEL					LBS. 4,517
EPOXY COATED REINFORCING STEEL					LBS. 3,362
CLASS AA CONCRETE					C.Y. 53.3

ALL BAR DIMENSIONS ARE OUT TO OUT.

SPLICE LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"



SECTION THRU SLAB



PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 89+28.52 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 BRIDGE APPROACH SLAB
 FOR INTEGRAL ABUTMENT

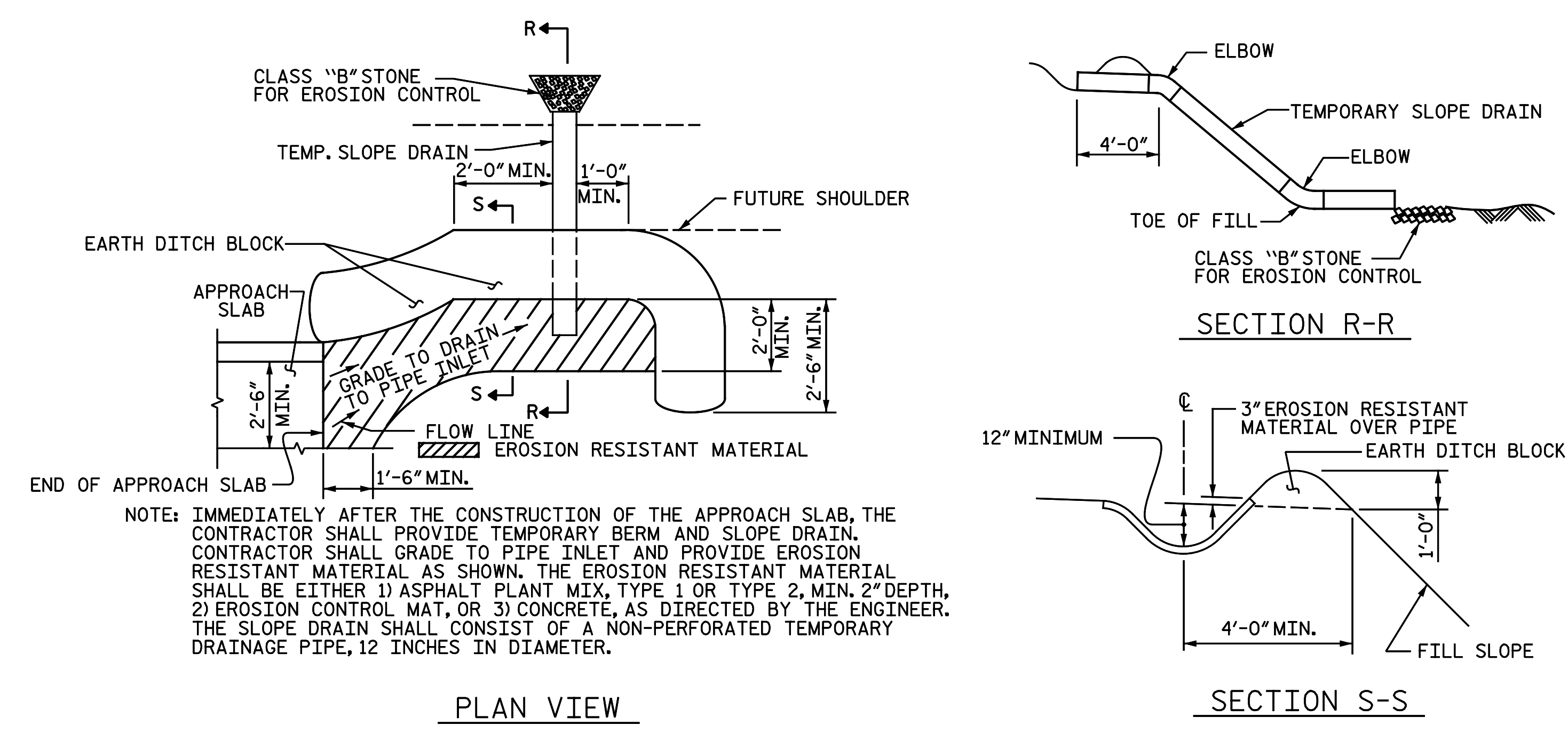
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 UNLESS ALL SIGNATURES COMPLETED

RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO. S2-24	
TOTAL SHEETS 25	

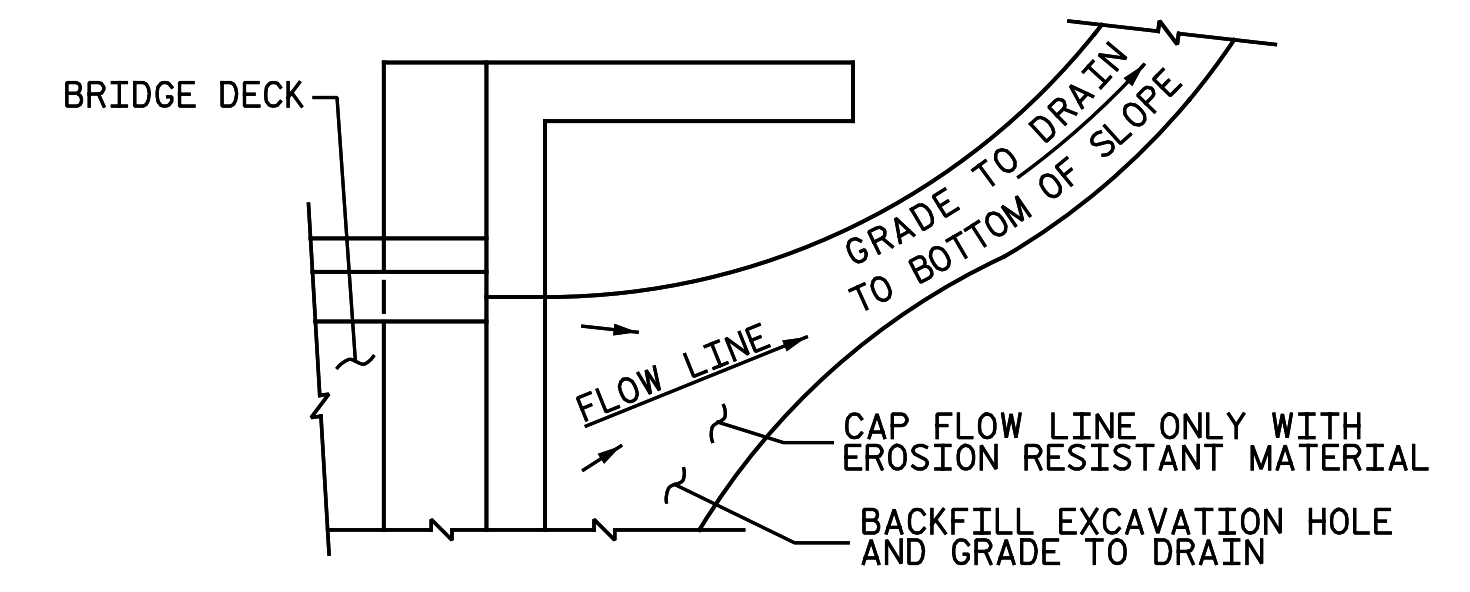
DRAWN BY: N. B. SPEAKS DATE: 3-6-17
 CHECKED BY: V. A. PATEL DATE: 5-23-17

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 Cary, North Carolina 27518
 NC License No.: F-1084



NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

TEMPORARY BERM AND SLOPE DRAIN DETAILS
(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 89+28.52 -L-



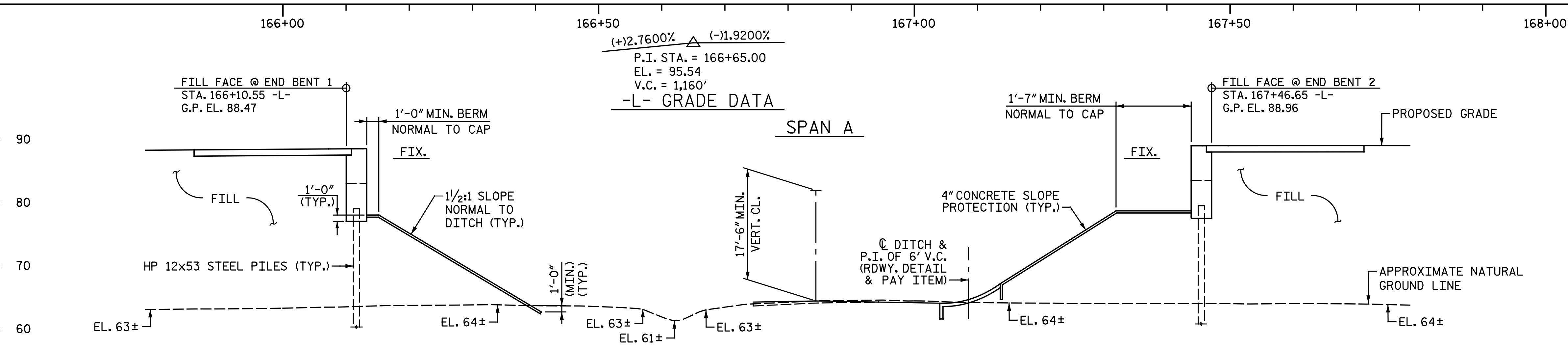
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
BRIDGE APPROACH SLAB
DETAILS
RIGHT LANE

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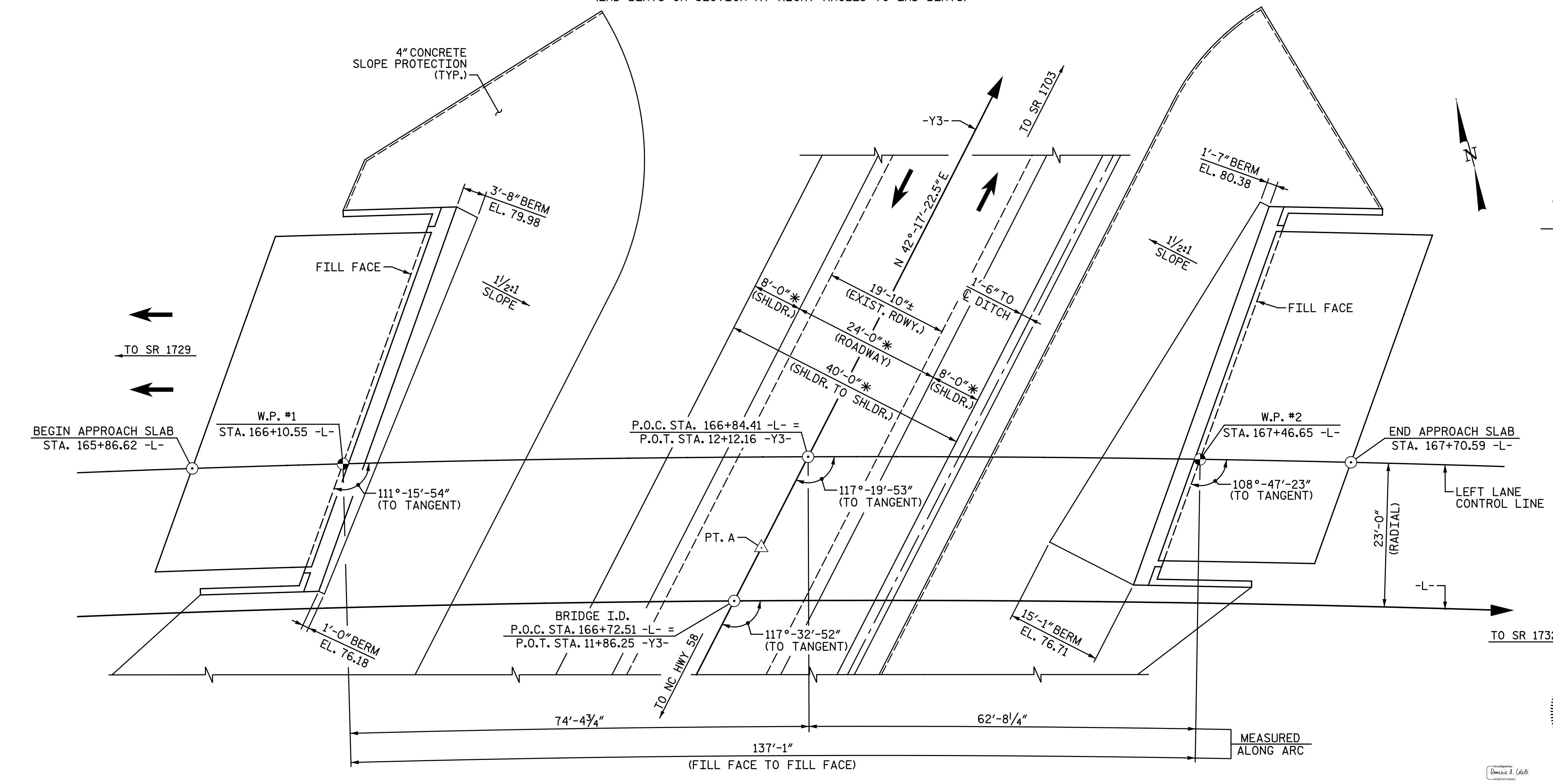
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			25
2			4			

ASSEMBLED BY : N. B. SPEAKS	DATE : 3-1-17
CHECKED BY : V. A. PATEL	DATE : 3-20-17
DRAWN BY : FCJ 11/88	REV. 10/11/11 MAA/GM
CHECKED BY : ARB 11/88	REV. 7/12 MAA/GM
	REV. 6/13 MAA/GM



SECTION ALONG LEFT LANE CONTROL LINE
(END BENTS ON SECTION AT RIGHT ANGLES TO END BENTS)



PLAN
(PILES NOT SHOWN IN PLAN VIEW FOR CLARITY)

-L- HORIZONTAL CURVE DATA
 P. I. STA. 158+61.41
 $\Delta = 38^\circ-24'-10.2''$ (RT)
 $D = 1^\circ-49'-08.1''$
 $L = 2,111.31'$
 $T = 1,097.03'$
 $R = 3,150.00'$

POINT	STATION ON -Y3-	OFFSET	ELEVATION ON -Y3-
A	11+95.83	0.00	64.36

Δ - DENOTES POINT OF MINIMUM VERTICAL CLEARANCE OVER EXISTING ROADWAY WITH 1/2" OVERLAY.
 * - FUTURE LANE CONFIGURATION

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
11+86.25 -Y3-
 SHEET 1 OF 4 BRIDGE NO. 210



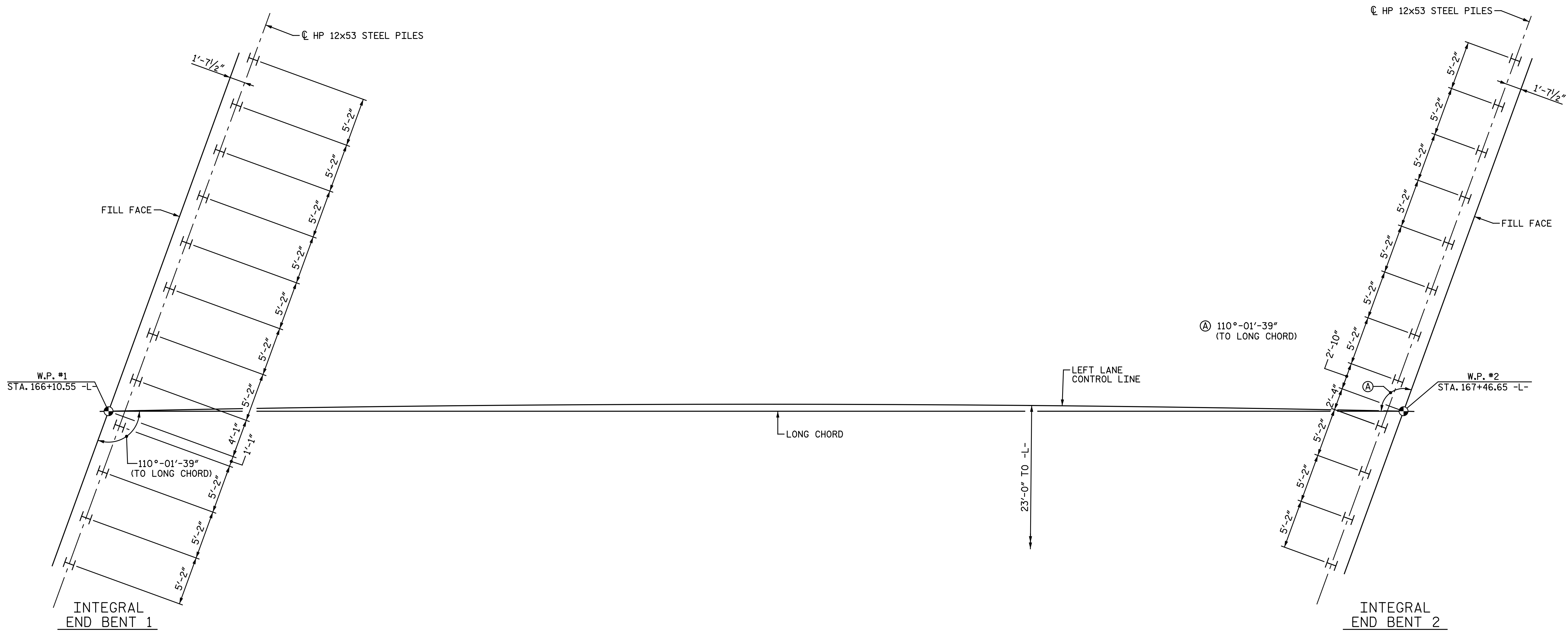
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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER SR 1004
 BETWEEN SR 1729 AND SR 1732
 LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-1
1			3			TOTAL SHEETS
2			4			34

DRAWN BY : C. E. MAYHEW DATE : 7-14-17
 CHECKED BY : D. A. COLETTI DATE : 7-14-17



FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINES.
 ALL HP 12x53 STEEL PILES ARE VERTICAL.

NOTES:

- FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.
- PILES AT END BENT NO. 1 AND END BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 125 TONS PER PILE.
- DRIVE PILES AT END BENT NO. 1 AND END BENT NO. 2 TO A REQUIRED DRIVING RESISTANCE OF 210 TONS PER PILE.
- TESTING THE FIRST PRODUCTION PILE WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED AT END BENT NO. 1 OR END BENT NO. 2. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS AND FOR PILE DRIVING CRITERIA, SEE PILE DRIVING CRITERIA PROVISION.
- IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 60-65 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT NO.1 AND 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.
- OBSERVE A 2 MONTH WAITING PERIOD AFTER CONSTRUCTING THE EMBANKMENT TO WITHIN 2 FT OF FINISHED GRADE BEFORE BEGINNING END BENT CONSTRUCTION AT END BENT NO.1 AND END BENT NO. 2. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SPECIAL PROVISIONS.

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
11+86.25 -Y3-
 SHEET 2 OF 4



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER SR 1004
 BETWEEN SR 1729 AND SR 1732
 LEFT LANE

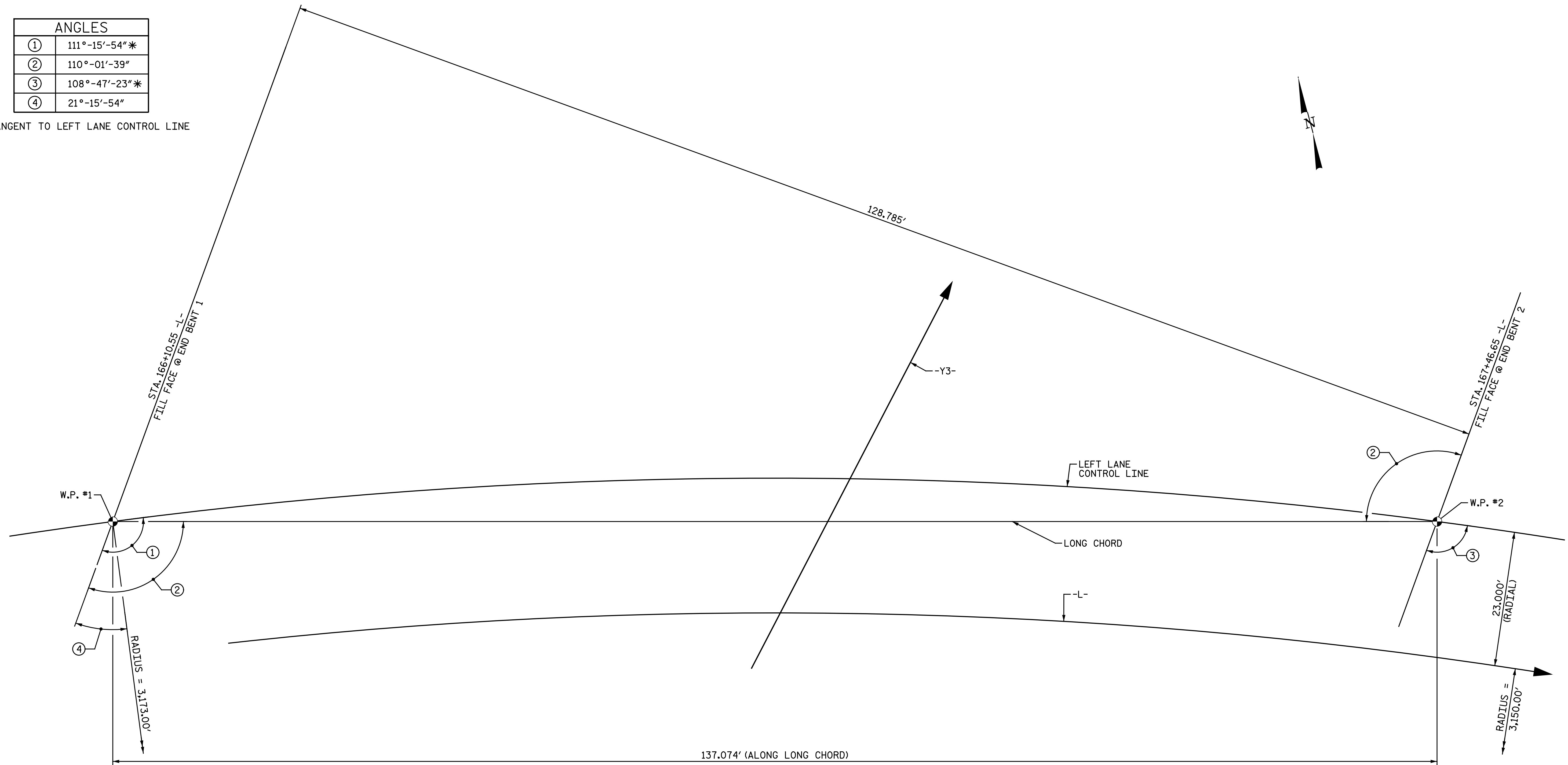
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-2
1			3			TOTAL SHEETS
2			4			34

DRAWN BY : C. E. MAYHEW DATE : 7-14-17
 CHECKED BY : D. A. COLETTI DATE : 7-14-17

ANGLES	
①	111°-15'-54"*
②	110°-01'-39"
③	108°-47'-23"*
④	21°-15'-54"

* TANGENT TO LEFT LANE CONTROL LINE

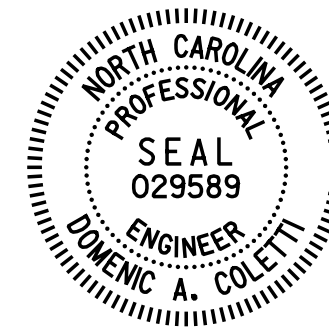


LONG CHORD LAYOUT
(END BENTS ARE PARALLEL)

-L- HORIZONTAL CURVE DATA

P. I. STA. 158+61.41
 $\Delta = 38^\circ-24'-10.2''$ (RT)
 $D = 1^\circ-49'-08.1''$
 $L = 2,111.31'$
 $T = 1,097.03'$
 $R = 3,150.00'$

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
11+86.25 -Y3-
 SHEET 3 OF 4

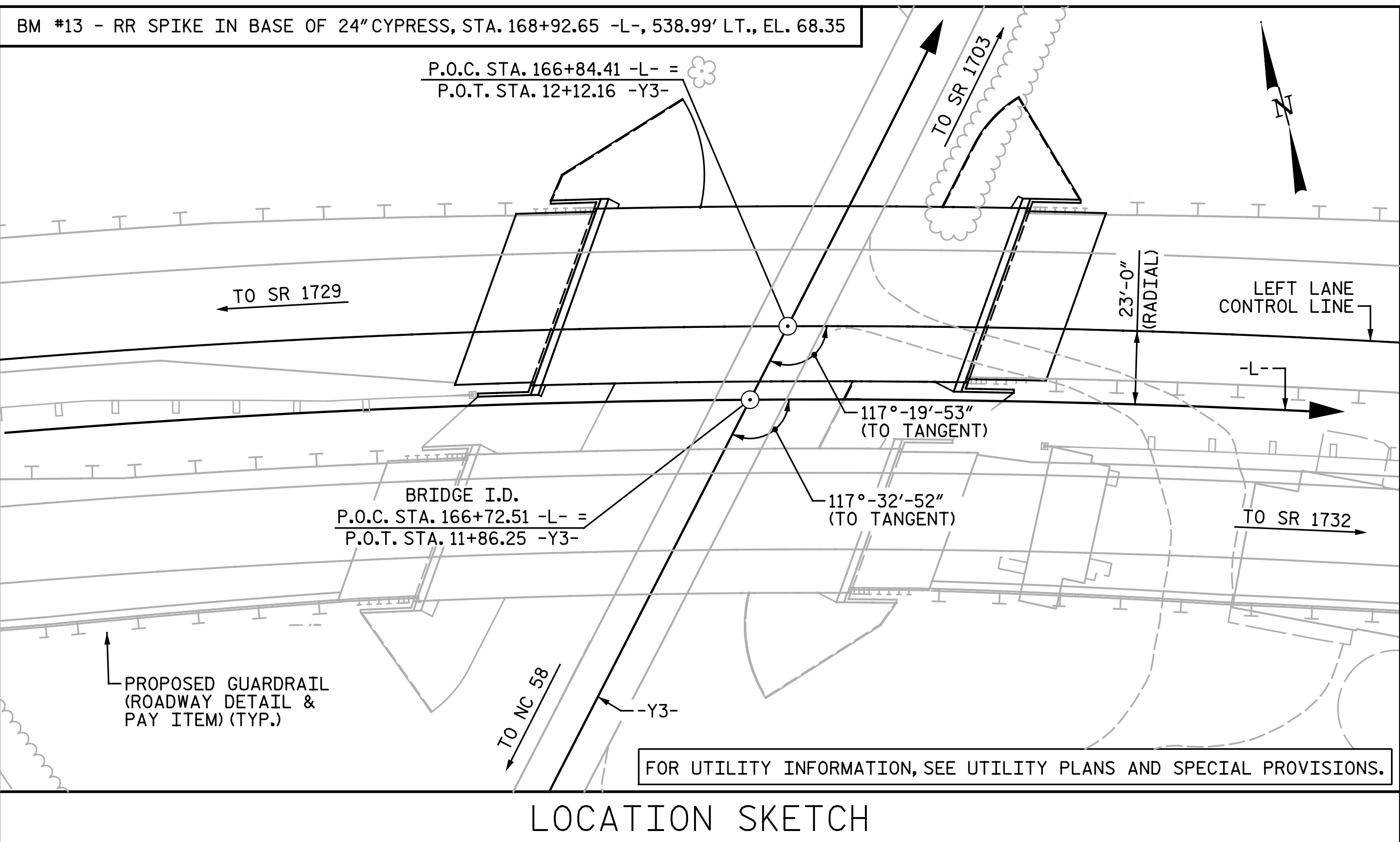


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER SR 1004
 BETWEEN SR 1729 AND SR 1732
 LEFT LANE

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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-3
1			3			TOTAL SHEETS
2			4			34

DRAWN BY : C. E. MAYHEW DATE : 1-17-17
 CHECKED BY : D. A. COLETTI DATE : 6-8-17



LOCATION SKETCH

NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE ELEVATION AND CLEARANCE SHOWN ON THE PLANS AT THE POINT OF MINIMUM VERTICAL CLEARANCE ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE ELEVATION ON THE EXISTING PAVEMENT AND CHECK THE CLEARANCE, REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.

FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH SYSTEM 4 OF ARTICLE 442-8 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

TOTAL BILL OF MATERIAL

LOCATION	PDA TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	APPROX. 289,500 LBS. STRUCTURAL STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES	HP 12x53 STEEL PILES		PILE REDRIVES	CONCRETE BARRIER RAIL	4" SLOPE PROTECTION	ELASTOMERIC BEARINGS
	EA.	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	LUMP SUM	EA.	NO.	LIN. FT.	EA.	LIN. FT.	SQ. YDS.	LUMP SUM
SUPERSTRUCTURE		7,525	8,929				LUMP SUM					270.51		LUMP SUM
END BENT 1				60.4		10,597		12	12	1,020	6		390	
END BENT 2				59.7		10,549		12	12	1,020	6		430	
TOTAL	1	7,525	8,929	120.1	LUMP SUM	21,146	LUMP SUM	24	24	2,040	12	270.51	820	LUMP SUM

PROJECT NO. R-5703

LENOIR COUNTY

STATION: 166+72.51 -L-
11+86.25 -Y3-

SHEET 4 OF 4



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
GENERAL DRAWING

FOR BRIDGE ON C.F. HARVEY PARKWAY OVER SR 1004 BETWEEN SR 1729 AND SR 1732
LEFT LANE

8/14/2017
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-4
1			3			TOTAL SHEETS
2			4			34

DRAWN BY : CEM / NBS DATE : 7-14-17
CHECKED BY : D. A. COLETTI DATE : 7-14-17

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR STEEL GIRDERS

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING #	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE II LIMIT STATE					COMMENT NUMBER			
						LIVE-LOAD FACTORS (%LL)	MOMENT					SHEAR					LIVE-LOAD FACTORS (%LL)	MOMENT						
							DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)		DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	1	1.16	--	1.75	0.83	1.27	A	1	66.81	1.00	1.16	A	3	26.72	1.30	0.83	1.36	A	1	66.81	1, 2	
	HL-93 (OPERATING)	N/A		1.50	--	1.35	0.83	1.64	A	1	66.81	1.00	1.50	A	3	26.72	1.00	0.83	1.76	A	1	66.81	1, 2	
	HS-20 (INVENTORY)	36.00	2	1.61	57.89	1.75	0.83	1.90	A	1	66.81	1.00	1.61	A	3	26.72	1.30	0.83	2.04	A	1	66.81	1, 2	
	HS-20 (OPERATING)	36.00		2.08	75.04	1.35	0.83	2.47	A	1	66.81	1.00	2.08	A	3	26.72	1.00	0.83	2.65	A	1	66.81	1, 2	
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500		5.01	67.64	1.40	0.83	5.84	A	1	66.81	1.00	5.02	A	3	26.72	1.30	0.83	5.01	A	1	66.81	1, 2
		SNGARBS2	20.000		3.49	69.80	1.40	0.83	4.15	A	1	66.81	1.00	3.49	A	3	26.72	1.30	0.83	3.56	A	1	66.81	1, 2
		SNAGRIS2	22.000		3.22	70.84	1.40	0.83	3.85	A	1	66.81	1.00	3.22	A	3	26.72	1.30	0.83	3.30	A	1	66.81	1, 2
		SNCOTTS3	27.250		2.49	67.85	1.40	0.83	2.89	A	1	66.81	1.00	2.50	A	3	26.72	1.30	0.83	2.49	A	1	66.81	1, 2
		SNAGGRS4	34.925		2.01	70.20	1.40	0.83	2.35	A	1	66.81	1.00	2.02	A	3	26.72	1.30	0.83	2.01	A	1	66.81	1, 2
		SNS5A	35.550		1.97	70.03	1.40	0.83	2.30	A	1	66.81	1.00	2.02	A	3	26.72	1.30	0.83	1.97	A	1	66.81	1, 2
		SNS6A	39.950		1.78	71.11	1.40	0.83	2.08	A	1	66.81	1.00	1.82	A	3	26.72	1.30	0.83	1.78	A	1	66.81	1, 2
		SNS7B	42.000		1.69	70.98	1.40	0.83	1.98	A	1	66.81	1.00	1.77	A	3	26.72	1.30	0.83	1.69	A	1	66.81	1, 2
	TRUCK TRACTOR SEMI-TRAILER (T/S)	TNAGRIT3	33.000		2.16	71.28	1.40	0.83	2.52	A	1	66.81	1.00	2.19	A	3	26.72	1.30	0.83	2.16	A	1	66.81	1, 2
		TNT4A	33.075		2.15	71.11	1.40	0.83	2.53	A	1	66.81	1.00	2.15	A	3	26.72	1.30	0.83	2.17	A	1	66.81	1, 2
		TNT6A	41.600		1.75	72.80	1.40	0.83	2.04	A	1	66.81	1.00	1.84	A	3	26.72	1.30	0.83	1.75	A	1	66.81	1, 2
		TNT7A	42.000		1.74	73.08	1.40	0.83	2.03	A	1	66.81	1.00	1.81	A	3	26.72	1.30	0.83	1.74	A	1	66.81	1, 2
		TNT7B	42.000		1.74	73.08	1.40	0.83	2.07	A	1	66.81	1.00	1.74	A	3	26.72	1.30	0.83	1.77	A	1	66.81	1, 2
		TNAGRIT4	43.000		1.69	72.67	1.40	0.83	1.99	A	1	66.81	1.00	1.69	A	3	26.72	1.30	0.83	1.71	A	1	66.81	1, 2
		TNAGT5A	45.000		1.62	72.90	1.40	0.83	1.89	A	1	66.81	1.00	1.66	A	3	26.72	1.30	0.83	1.62	A	1	66.81	1, 2
TNAGT5B	45.000		3	1.61	72.45	1.40	0.83	1.88	A	1	66.81	1.00	1.62	A	3	26.72	1.30	0.83	1.61	A	1	66.81	1, 2	
FATIGUE	HL-93 (INVENTORY)	γ _{LL} = 0.75																						

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ _{DC}	γ _{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE II LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE II LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- DISTANCE FROM LEFT END OF SPAN IS GIVEN WITH RESPECT TO CENTERLINE OF BEARING AND IS MEASURED ALONG THE CONTROLLING GIRDER.
- FATIGUE RATING IS NOT REQUIRED OR REPORTED SINCE GIRDER DOES NOT INCLUDE FATIGUE-PRONE DETAILS.

CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

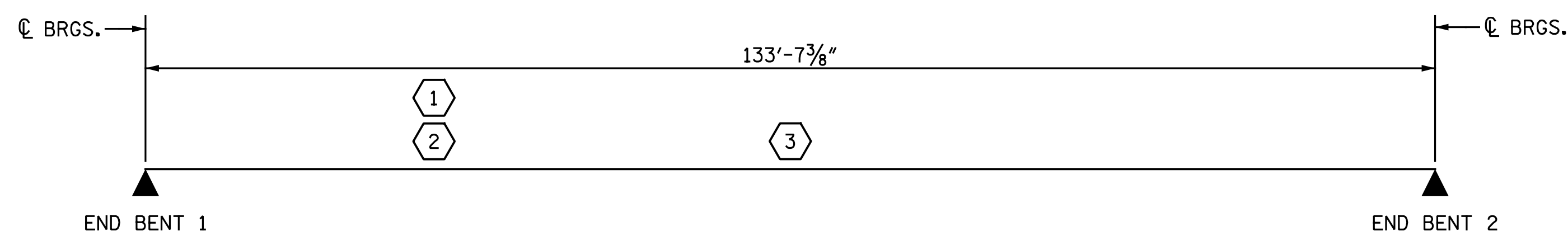
2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

GIRDER LOCATION IS PROVIDED USING GIRDER NUMBER, WHERE GIRDER 1 IS THE LEFT EXTERIOR GIRDER LOOKING AHEAD STATION. SEE "GIRDER LAYOUT" SHEET FOR ALL GIRDER LOCATIONS.



LRFR SUMMARY

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD

LRFR SUMMARY FOR
 STEEL GIRDERS
 (NON-INTERSTATE TRAFFIC)
 LEFT LANE

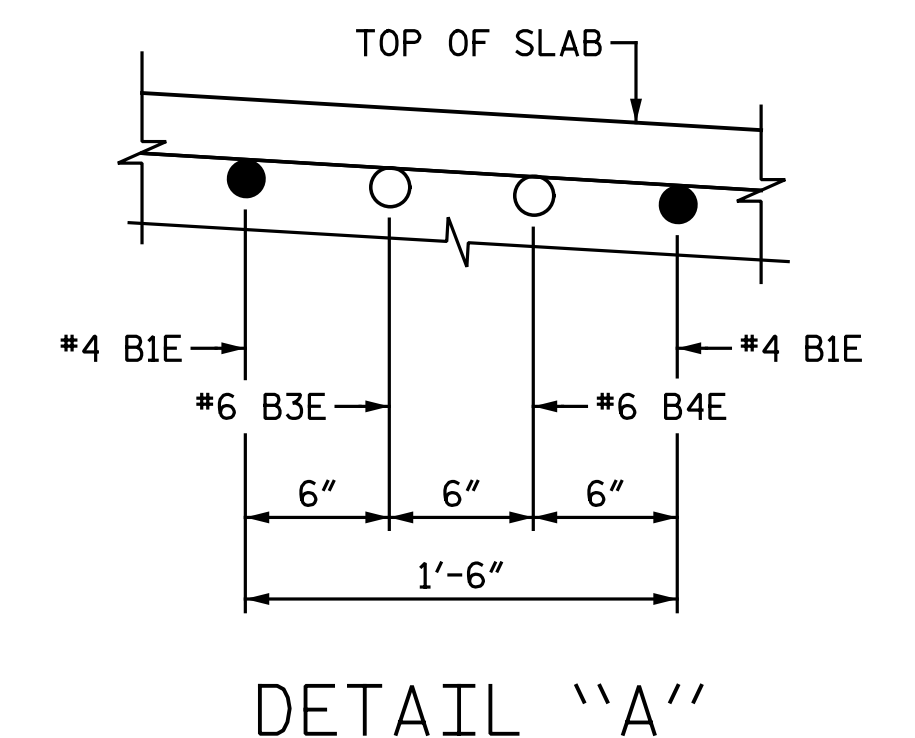
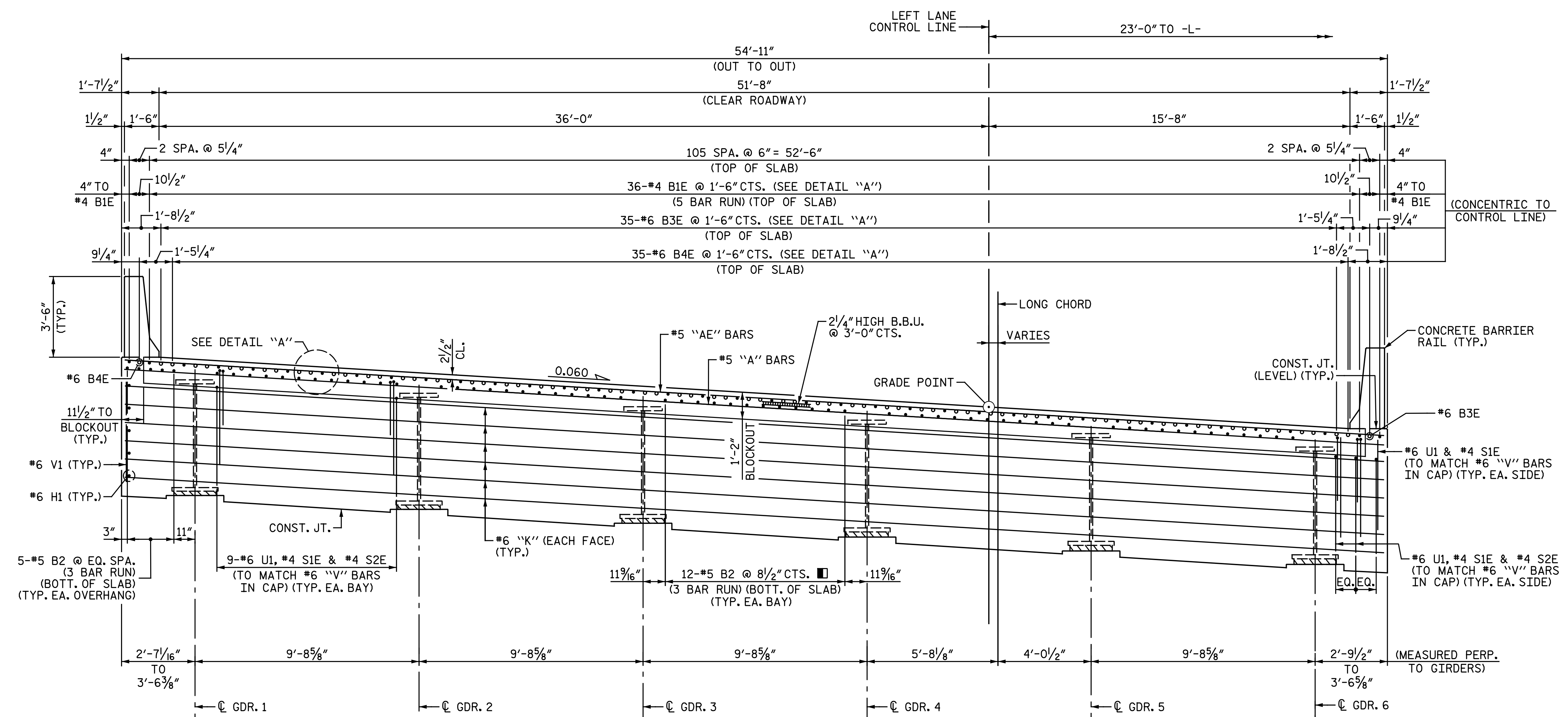
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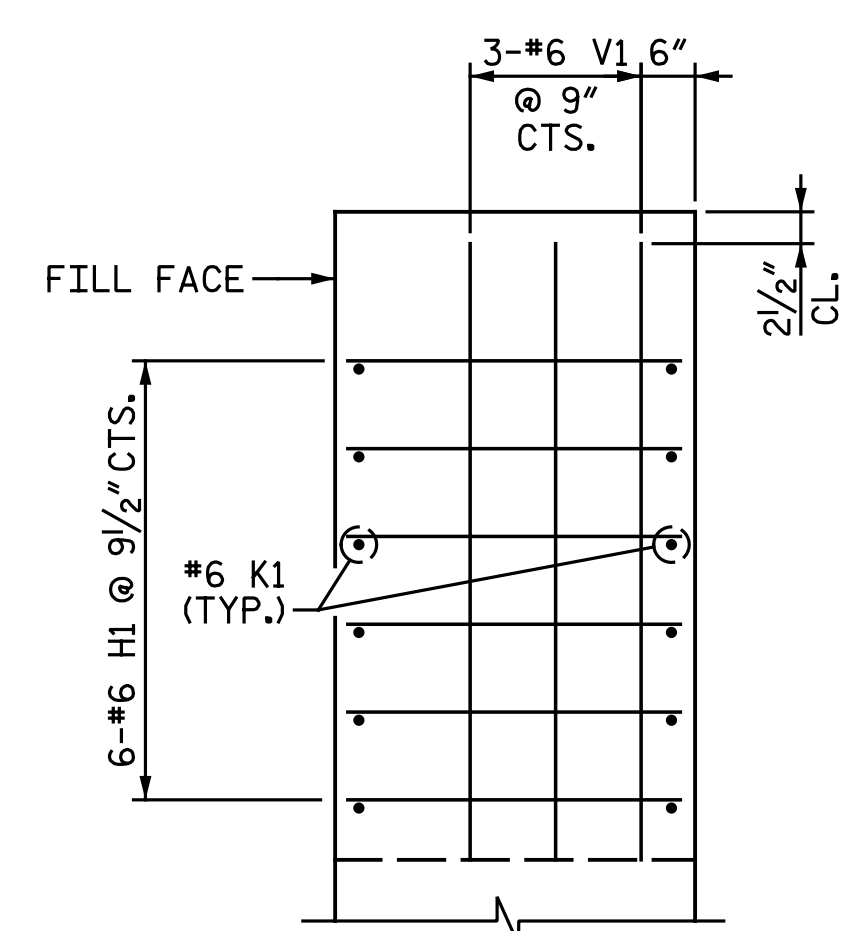
ASSEMBLED BY : C. E. MAYHEW	DATE : 3-10-17
CHECKED BY : D. A. COLETTI	DATE : 6-8-17
DRAWN BY : MAA	REV. 11/2/08RR MAA/GM
CHECKED BY : GM/DI	REV. 10/1/11 MAA/GM



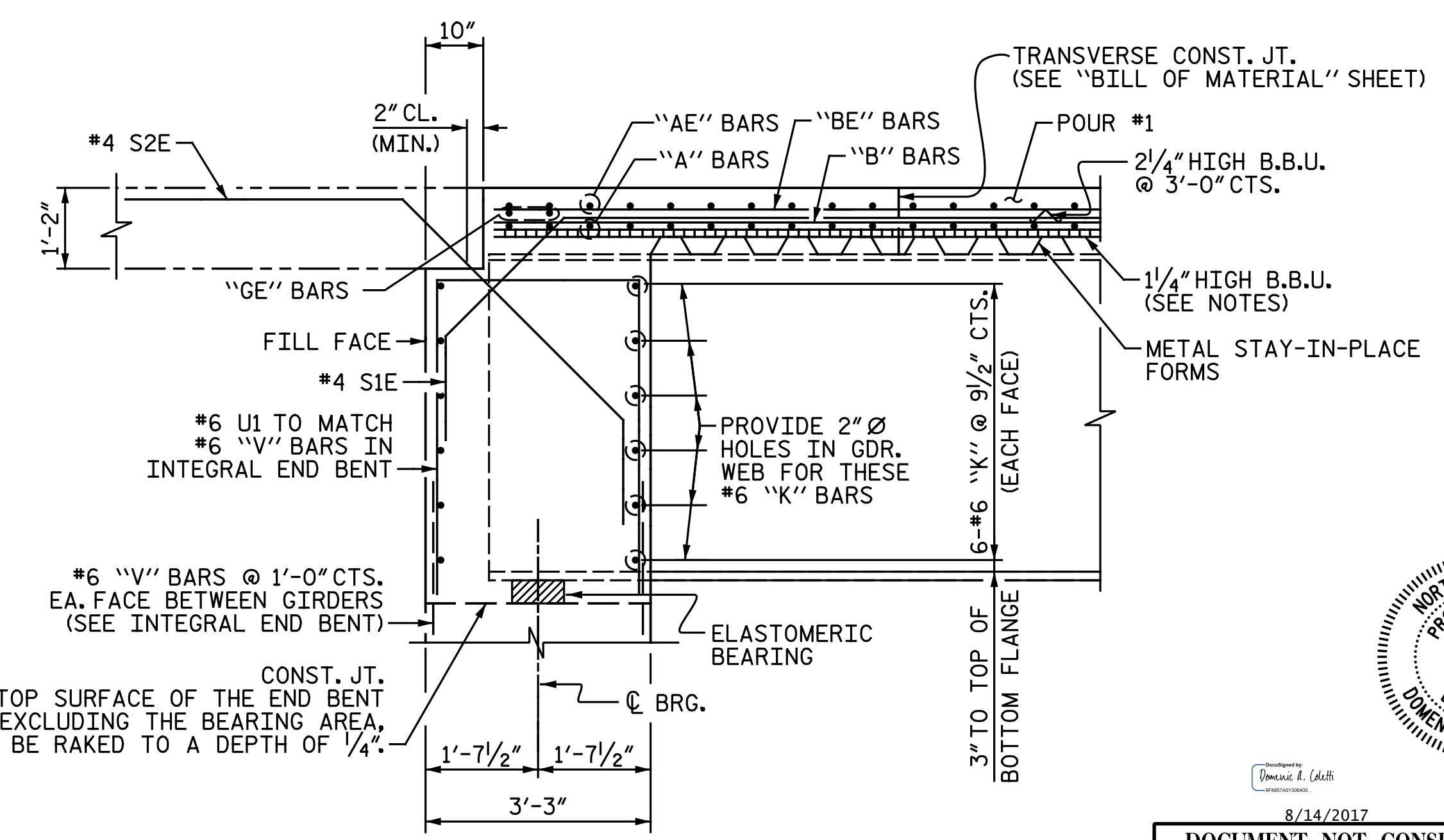
TYPICAL SECTION AT INTEGRAL END BENT

(ALL HORIZONTAL DIMENSIONS ARE RADIAL U.N.O.)
 (END BENT 1 SHOWN, END BENT 2 SIMILAR)
 (#5 "GE" BARS ALONG SKEW NOT SHOWN FOR CLARITY)
 (GIRDERS ARE PARALLEL TO LONG CHORD)

■ = PARALLEL TO GIRDERS



END OF DIAPHRAGM DETAIL
 (END BENT 1 SHOWN, END BENT 2 SIMILAR)



END OF GIRDER DETAIL AT INTEGRAL END BENT

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
 SHEET 1 OF 2



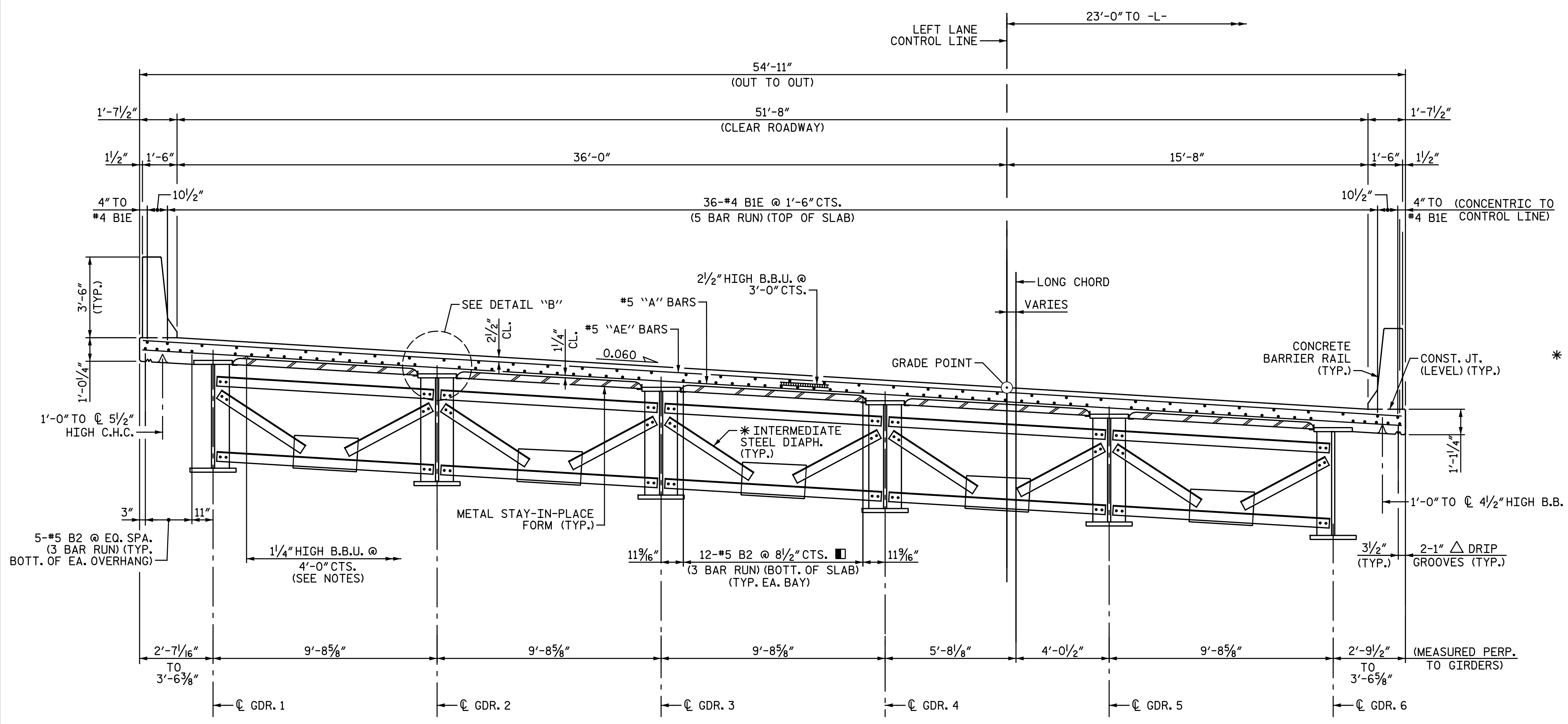
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
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 SUPERSTRUCTURE
 TYPICAL SECTION

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TYPICAL SECTION AT INTERMEDIATE DIAPHRAGMS
 (ALL HORIZONTAL DIMENSIONS ARE RADIAL U.N.O.)
 (GIRDERS ARE PARALLEL TO LONG CHORD)

■ = PARALLEL TO GIRDERS

NOTES:

PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2" ABOVE THE TOP OF THE REMOVABLE FORM.

FOR CONCRETE BARRIER RAIL DETAILS, SEE "CONCRETE BARRIER RAIL" SHEET.

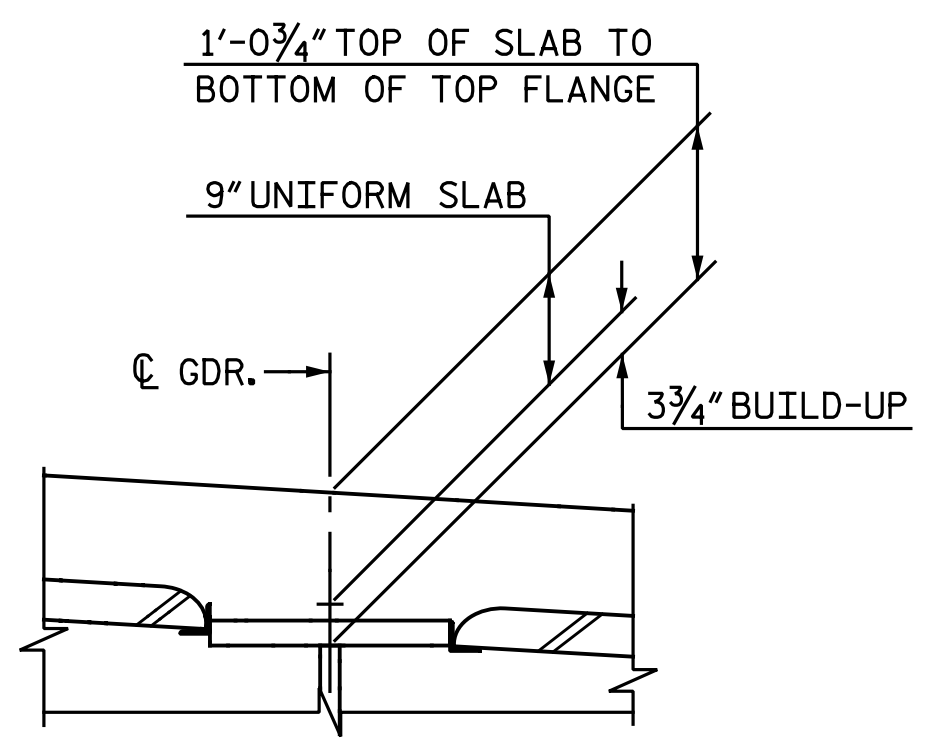
METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO BEAM OR GIRDER FLANGES IN THE ZONES REQUIRING CHARPY V-NOTCH TEST. SEE STRUCTURAL STEEL DETAIL SHEETS.

PREVIOUSLY CAST CONCRETE IN A SPAN SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE SPAN.

THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL STAY-IN-PLACE FORM SUPPORTS OR FORMS AND BEAM/GIRDER STIFFENERS OR CONNECTOR PLATES. THE PROPOSAL SHALL BE INDICATED, AS APPROPRIATE, ON EITHER THE STEEL WORKING DRAWINGS OR THE METAL STAY-IN-PLACE FORM WORKING DRAWINGS.

FOR ADDITIONAL INFORMATION ON DECK SLAB REINFORCING, SEE "PLAN OF SPAN" SHEETS.

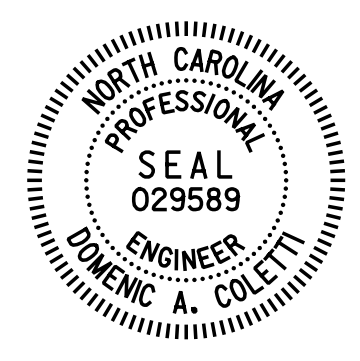
* FOR DETAILS OF INTERMEDIATE DIAPHRAGMS, SEE "STRUCTURAL STEEL DETAILS" SHEET 2 OF 2.



DETAIL "B"

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 2 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 TYPICAL SECTION

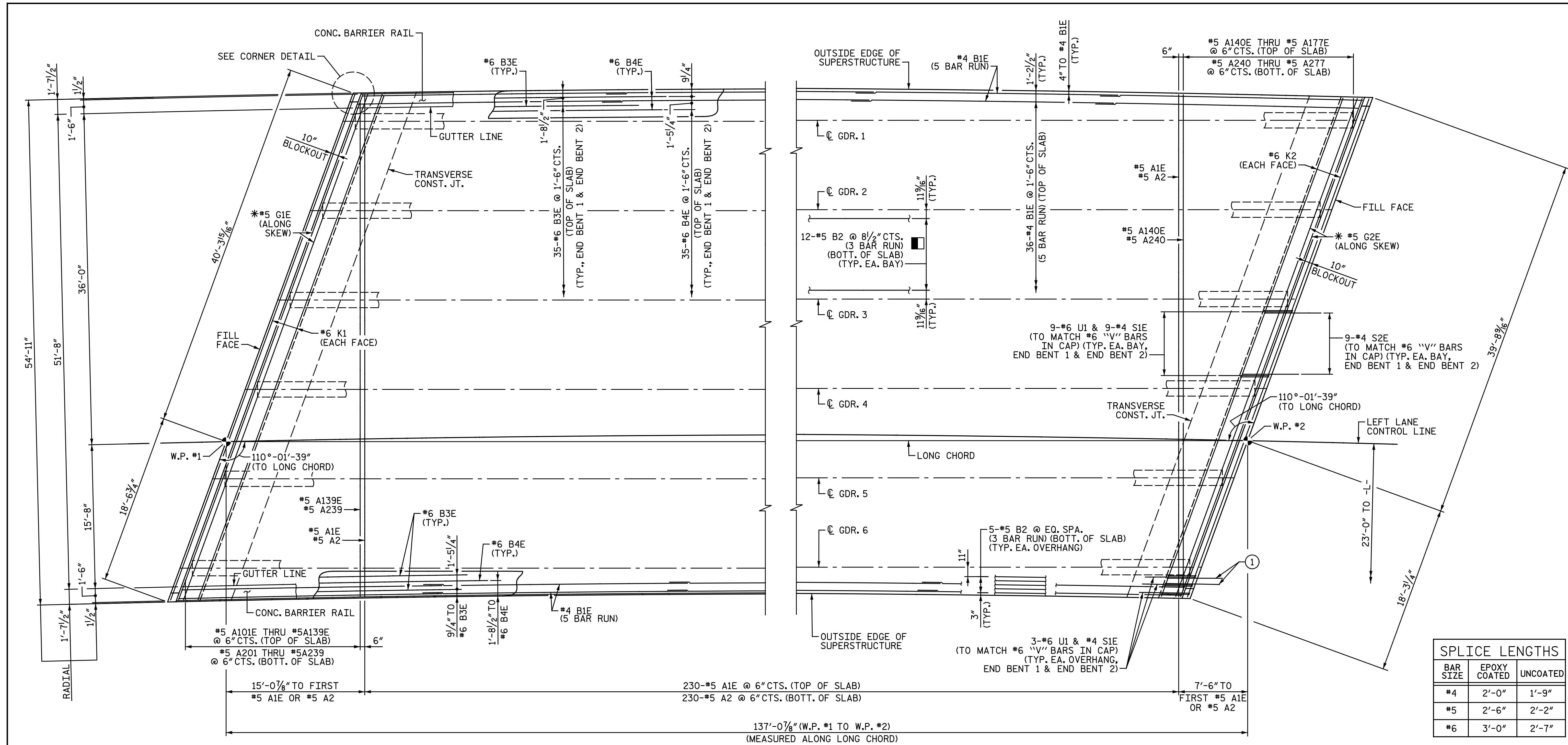
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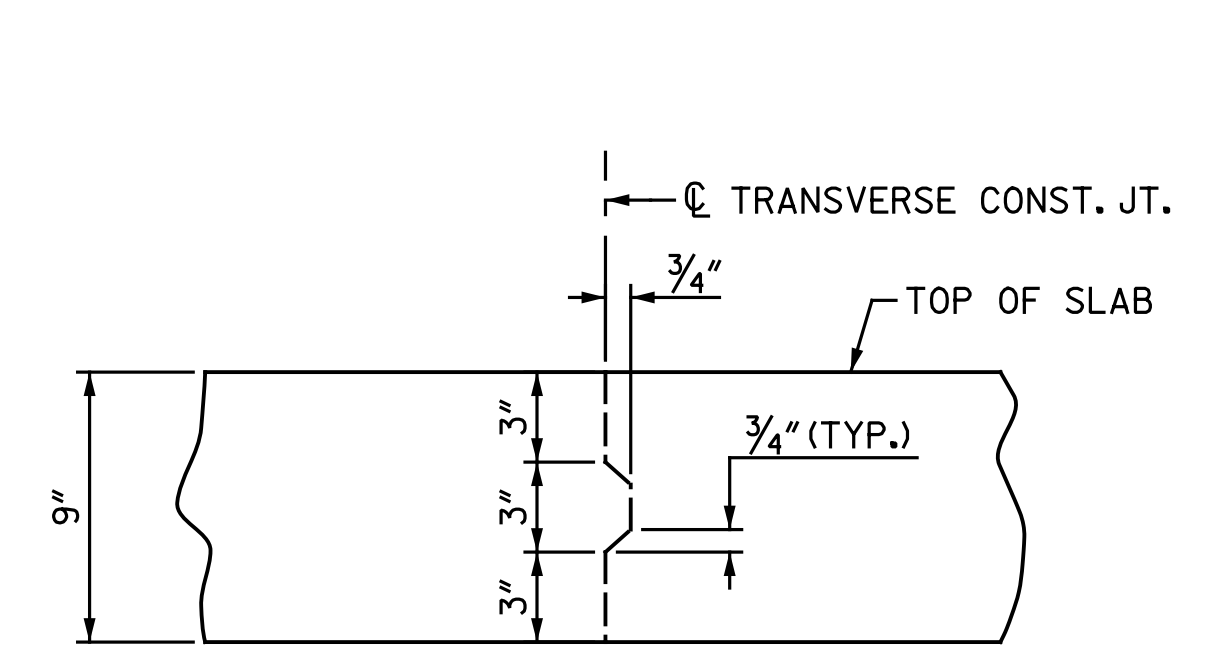
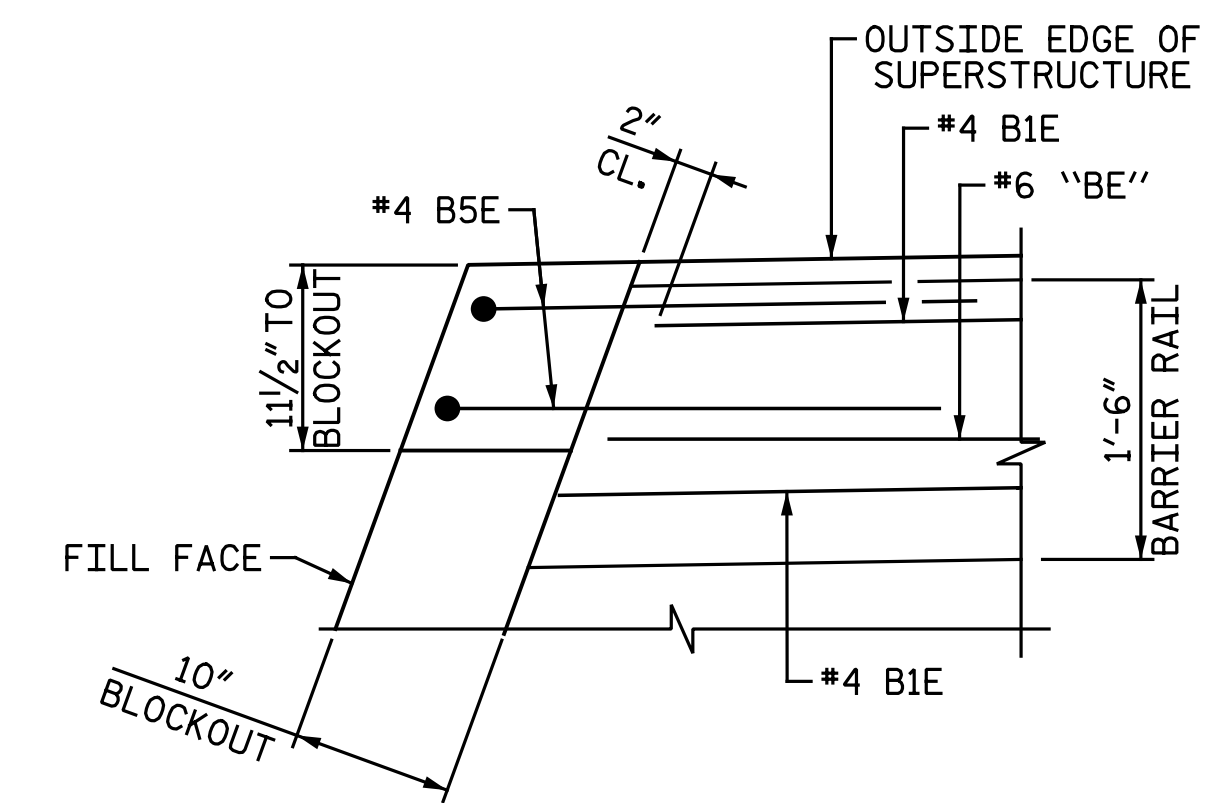
LEFT LANE

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1			3			TOTAL SHEETS
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SPlice LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"
#5	2'-6"	2'-2"
#6	3'-0"	2'-7"



**SPAN A
PLAN OF SPAN**

NOTES:
 * #5 "GE" BARS ARE TO BE PLACED PARALLEL TO SKEW AND BELOW "BE" BARS.
 FOR REINFORCING STEEL IN CONCRETE BARRIER RAIL, SEE "CONCRETE BARRIER RAIL" SHEET.
 FOR DECK POURING SEQUENCE AND LOCATION OF TRANSVERSE CONSTRUCTION JOINTS, SEE "BILL OF MATERIAL" SHEET.
 FOR ARC OFFSETS, SEE SHEET 2 OF 2.
 "A" AND "AE" BARS ARE TO BE PLACED PERPENDICULAR TO THE LONG CHORD.

- ① 2-#4 S2E (TO MATCH #6 "V" BARS IN CAP) (TYP. EA. OVERHANG, END BENT 1 & END BENT 2)
- ▣ PARALLEL TO GIRDERS

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
 SHEET 1 OF 2



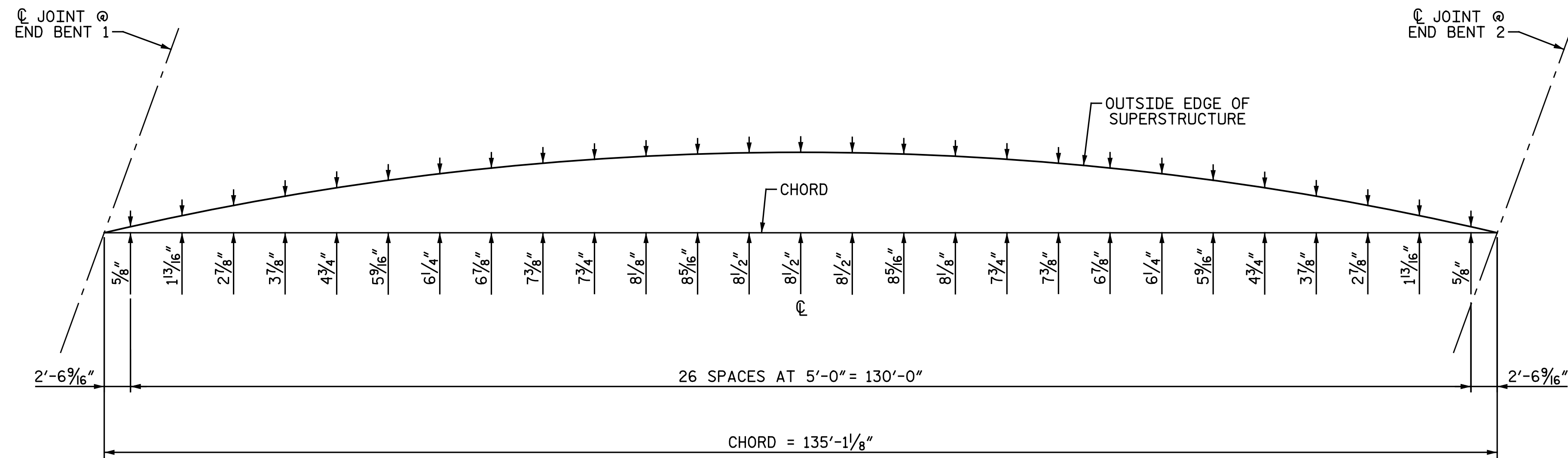
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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE PLAN OF SPAN					
LEFT LANE					
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2			4		

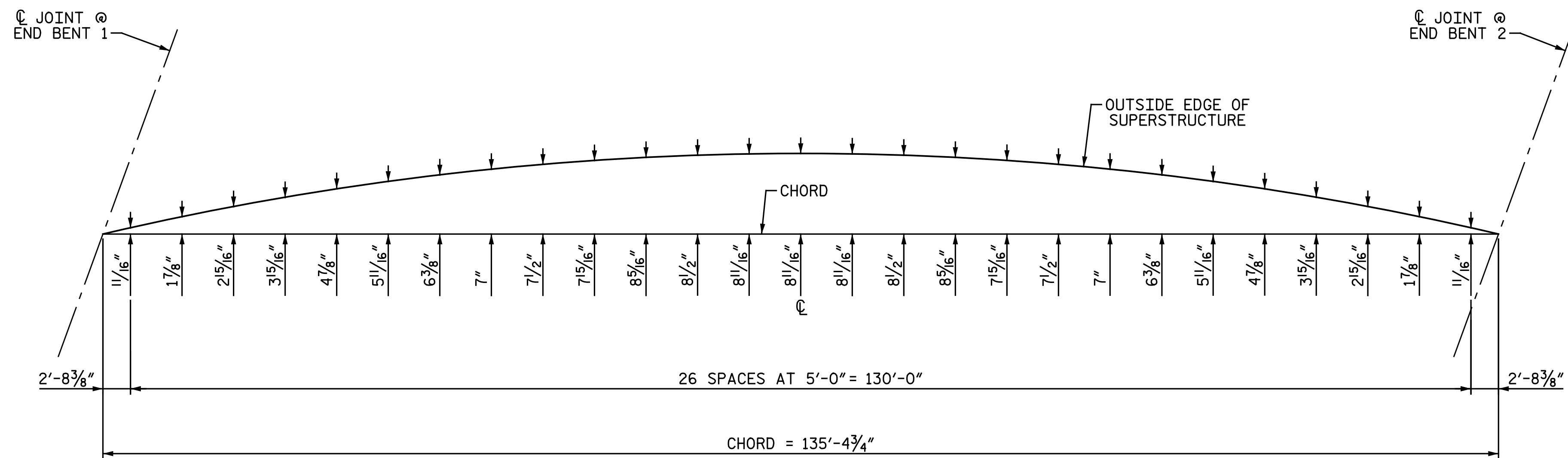
SHEET NO. S3-8	
TOTAL SHEETS 34	

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NOTES:
 Ⓢ JOINT INDICATES THE PERMITTED CONSTRUCTION JOINT BETWEEN APPROACH SLAB AND DECK.



LEFT SIDE



RIGHT SIDE

ARC OFFSETS - SPAN A

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 2 OF 2



STATE OF NORTH CAROLINA
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 PLAN OF SPANS

LEFT LANE

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STRUCTURAL STEEL NOTES:

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH SYSTEM 4 OF ARTICLE 442-8 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL, UNLESS OTHERWISE NOTED.

ALL FIELD CONNECTIONS TO BE 7/8" DIA. HIGH STRENGTH BOLTS UNLESS OTHERWISE NOTED. BOLTS SHALL BE DETAILED WITH THREADS EXCLUDED FROM SHEAR PLANES.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB IN THE FINAL CONDITION.

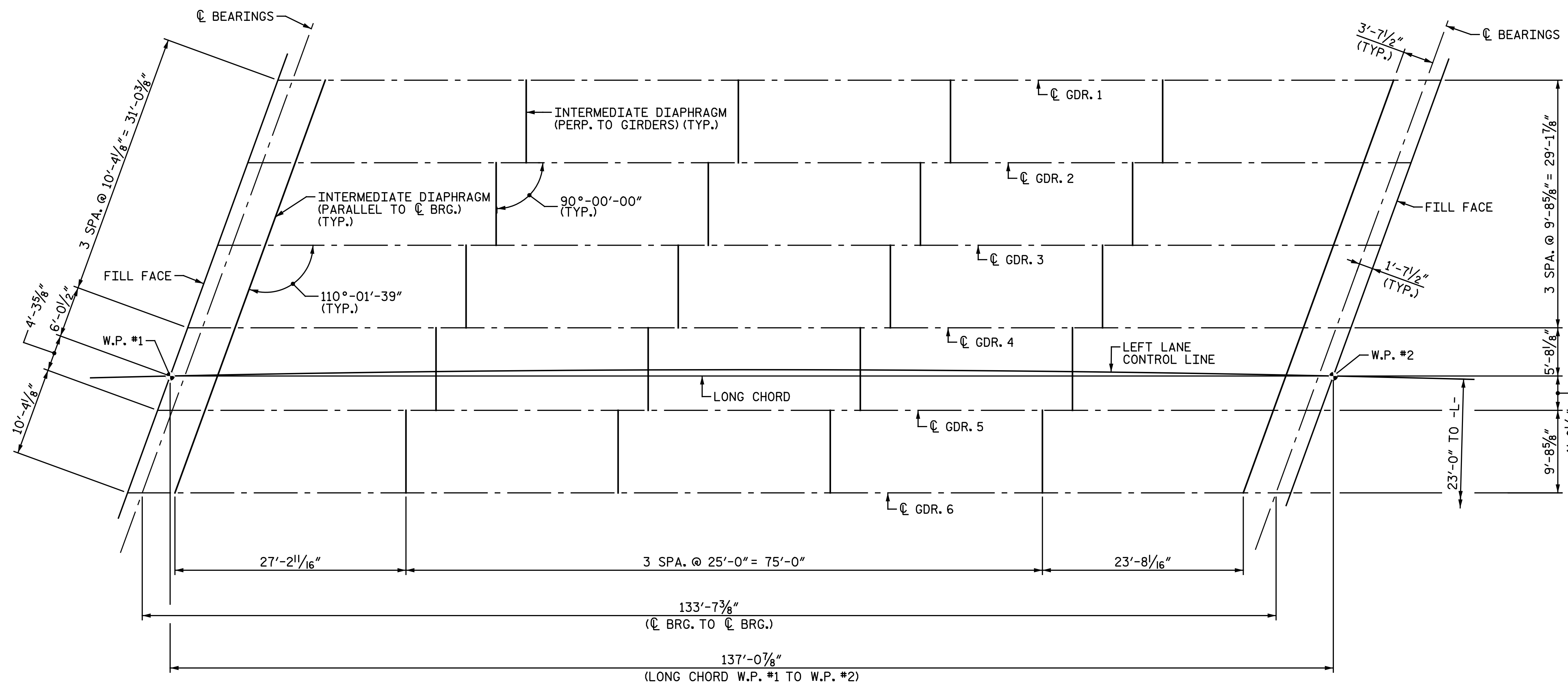
PERMITTED FLANGE AND WEB SHOP SPLICES SHALL NOT BE LOCATED WITHIN 15 FEET OF MAXIMUM DEAD LOAD DEFLECTION KEEP 2 FEET MINIMUM BETWEEN WEB AND FLANGE SHOP SPLICES. KEEP 6" MINIMUM BETWEEN CONNECTOR PLATE OR TRANSVERSE STIFFENER WELDS AND WEB OR FLANGE SHOP SPLICES.

STUDS ON GIRDERS MAY BE SHIFTED UP TO 1" IF NECESSARY TO CLEAR FLANGE SPLICE WELDS.

TENSION ON THE ASTM A325 BOLTS SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH ARTICLE 440-8 OF THE STANDARD SPECIFICATIONS.

ENDS OF GIRDERS SHALL BE PLUMB IN THE FINAL CONDITION.

FABRICATORS SHALL DETAIL DIAPHRAGM MEMBERS AND CONNECTIONS FOR FULL DEAD LOAD FIT UP. GIRDERS SHALL BE PLUMB AFTER THE FULL AMOUNT OF DEAD LOAD IS APPLIED.



INTEGRAL (E1)

SPAN A

INTEGRAL (E1)

GIRDER LAYOUT
(GIRDERS ARE PARALLEL TO LONG CHORD)

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 166+72.51 -L-

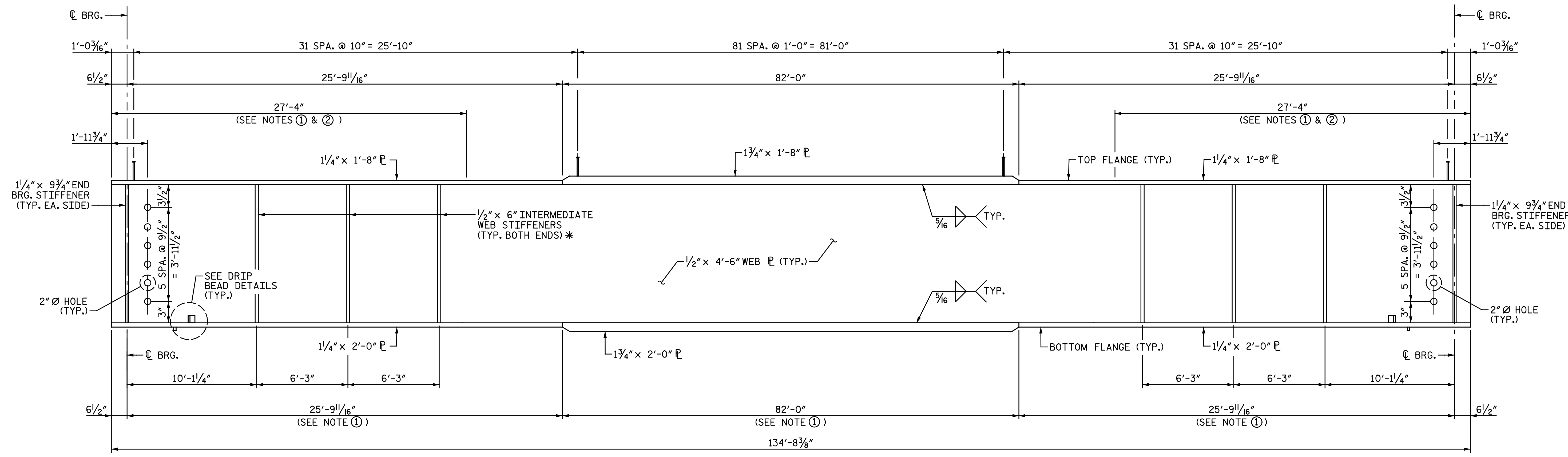


STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
FRAMING PLAN

8/14/2017
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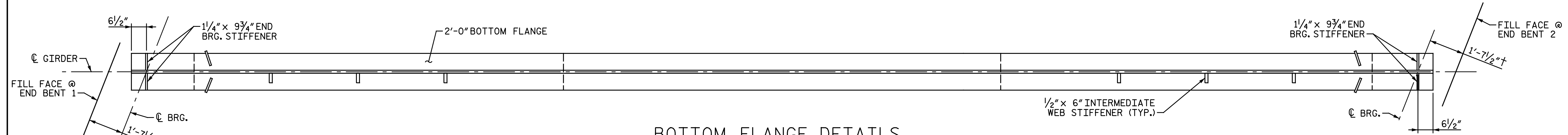
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* PLACE INTERMEDIATE WEB STIFFENERS ON ONLY ONE FACE OF THE WEB (INTERIOR FACE FOR EXTERIOR GIRDERS).

GIRDER ELEVATION

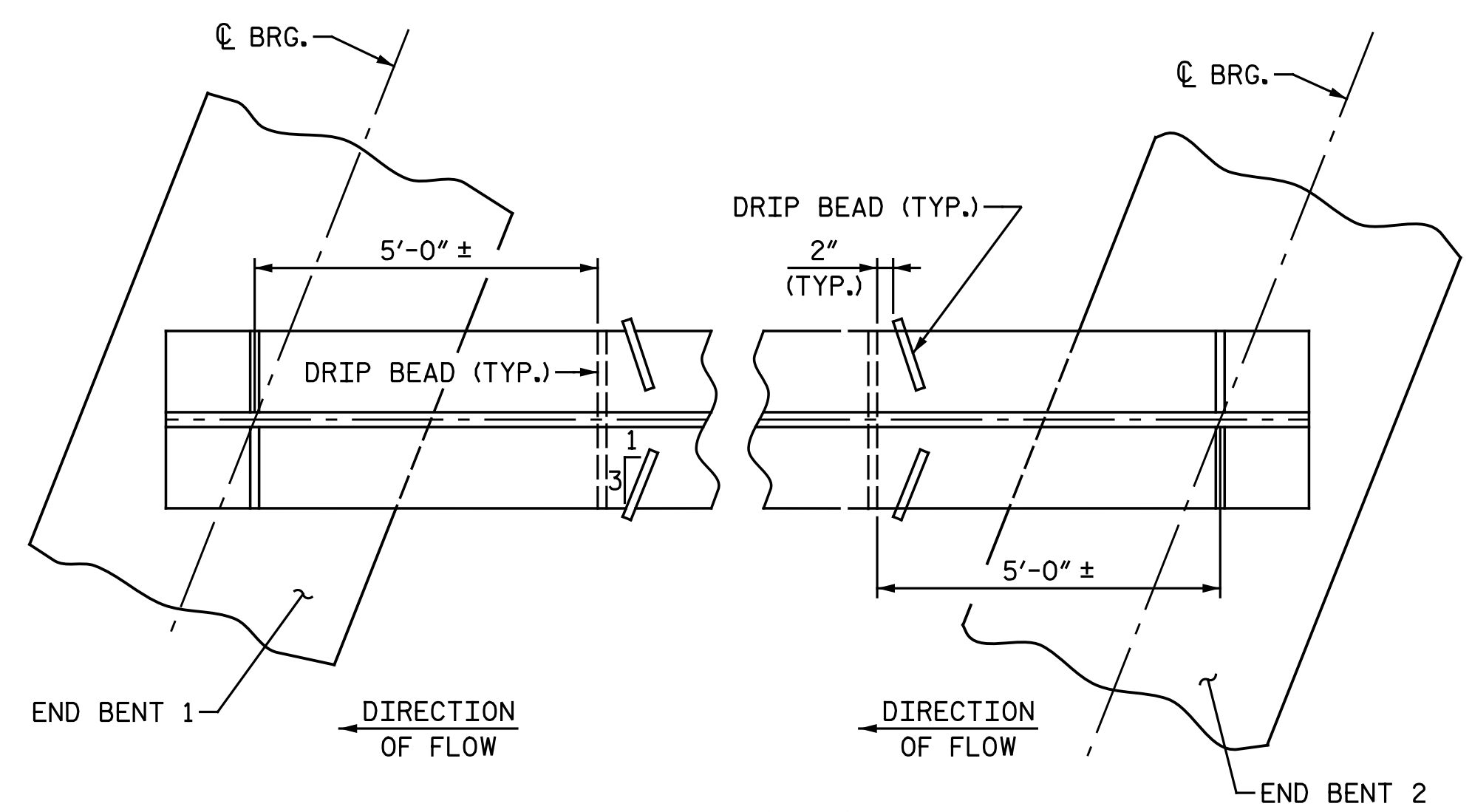


BOTTOM FLANGE DETAILS
† MEASURED PERPENDICULAR TO END BENT FILL FACE

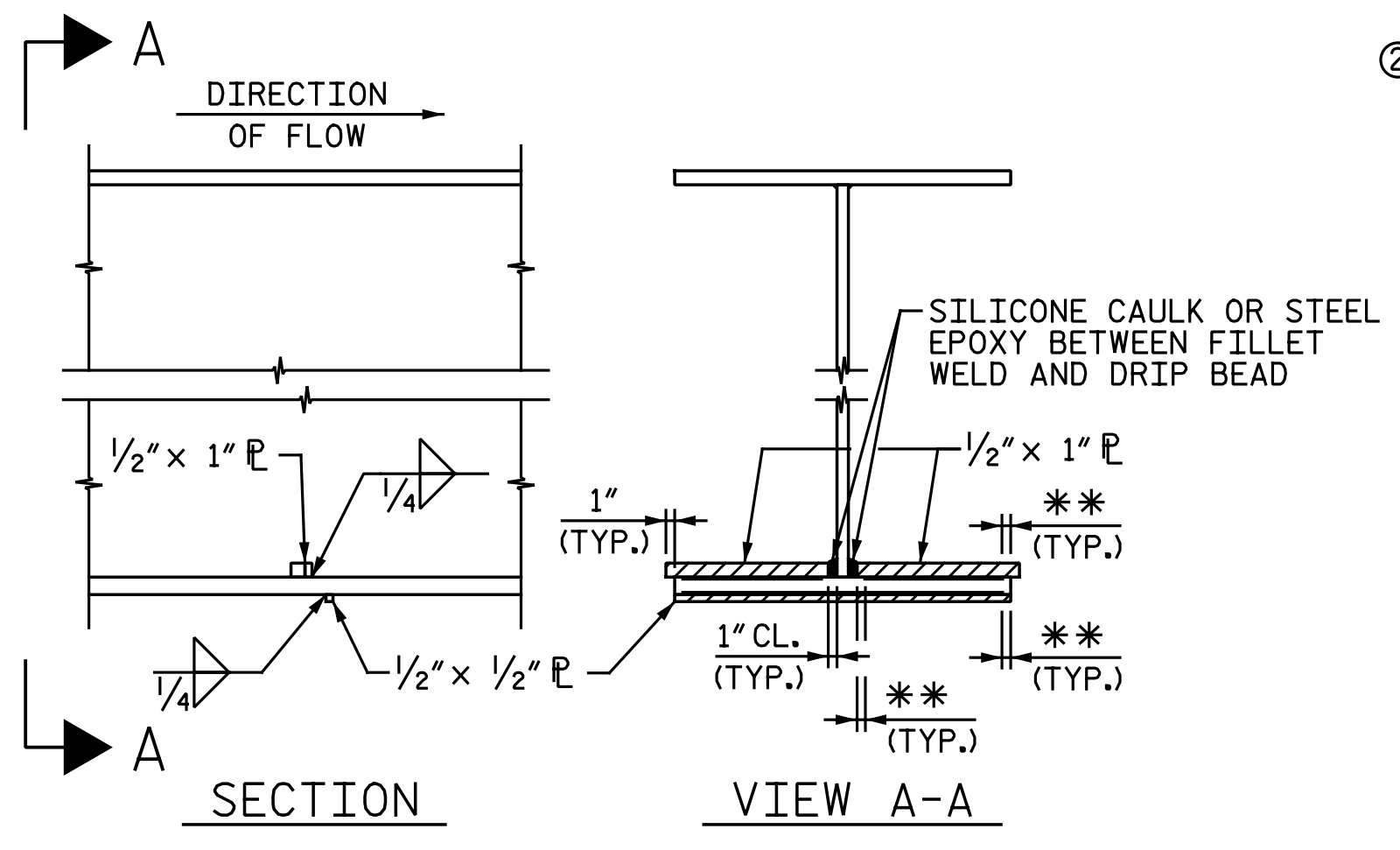
NOTES:

- ① CHARPY V-NOTCH TESTS ARE REQUIRED FOR ALL TOP OR BOTTOM FLANGE PLATES WHICH FALL WITHIN THESE LIMITS, ALL WEB PLATES, AND ALL SPLICE PLATES. IF A PERMITTED SHOP FLANGE SPLICE IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE FLANGE PLATE. FOR CHARPY V-NOTCH TESTS, SEE ARTICLE 1072-7 OF THE STANDARD SPECIFICATIONS.
 - ② NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.
- FOR SHEAR CONNECTOR TRANSVERSE SPACING, SEE "STRUCTURAL STEEL DETAILS", SHEET 2 OF 2.
- FOR STRUCTURAL STEEL NOTES, SEE "FRAMING PLAN" SHEET.

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
 SHEET 1 OF 2



PART PLAN - BOTTOM FLANGE



DRIP BEAD DETAILS

** SEE "WELD TERMINATION DETAILS" ON "STRUCTURAL STEEL DETAILS", SHEET 2 OF 3.

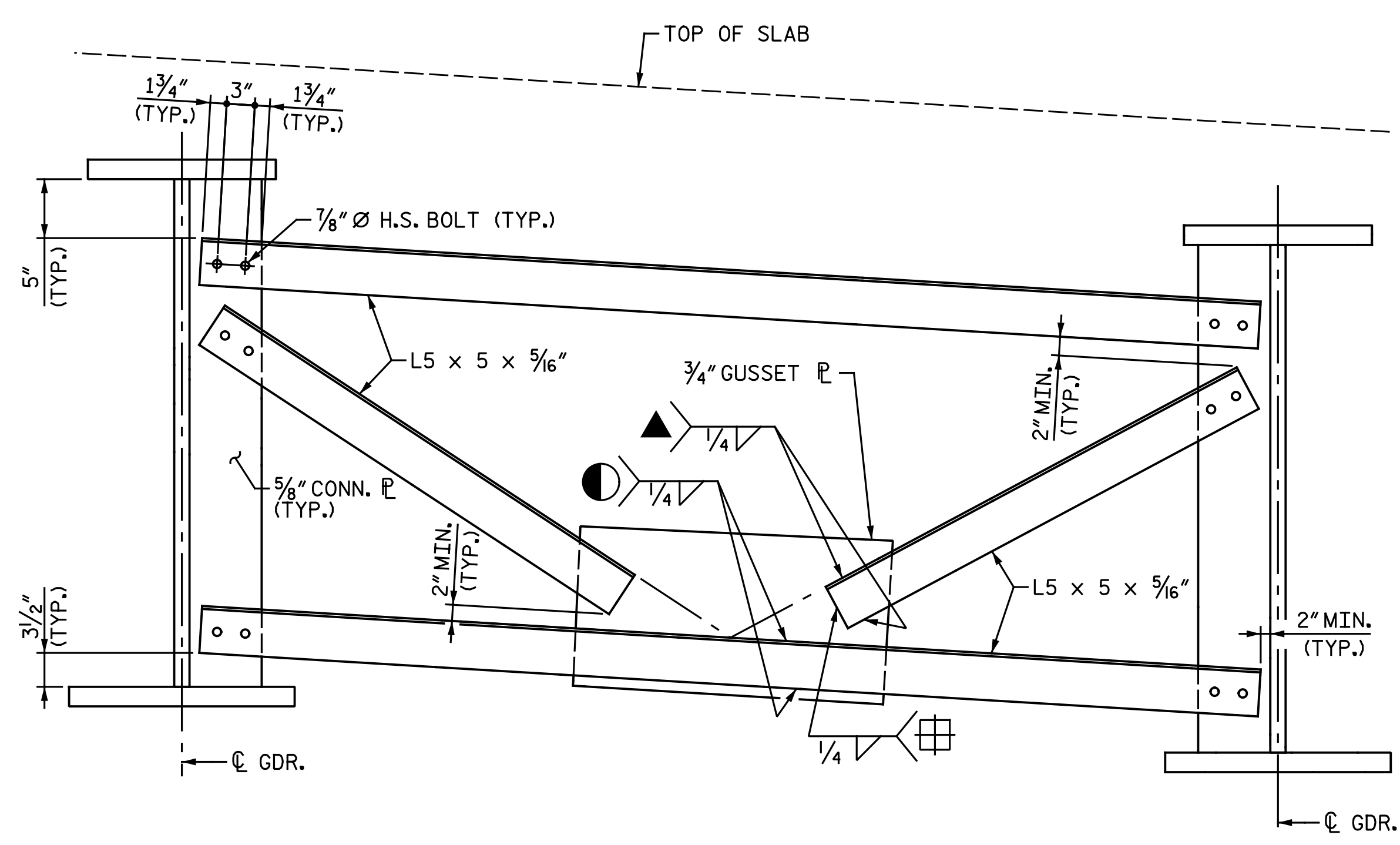


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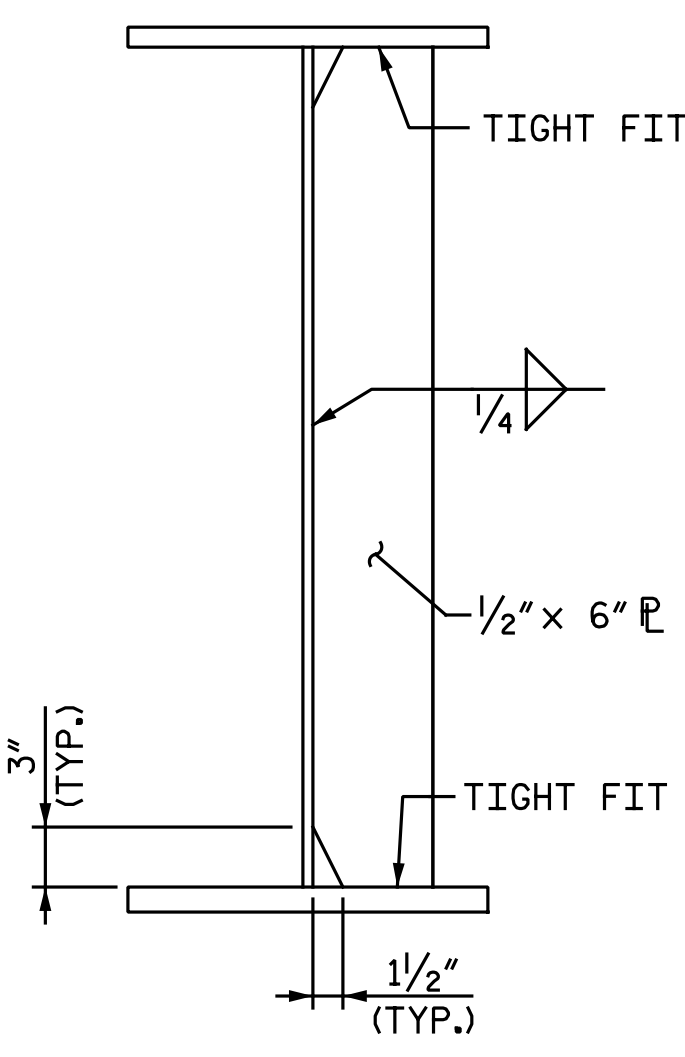
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SUPERSTRUCTURE					
STRUCTURAL STEEL DETAILS					
LEFT LANE					
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2			4		
SHEET NO. S3-II					TOTAL SHEETS 34

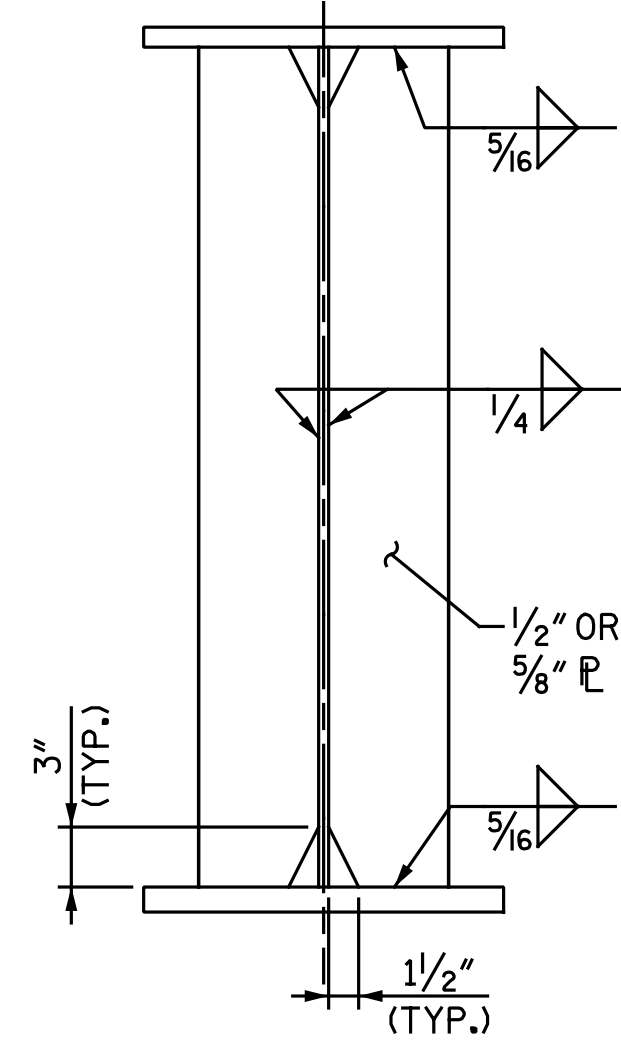
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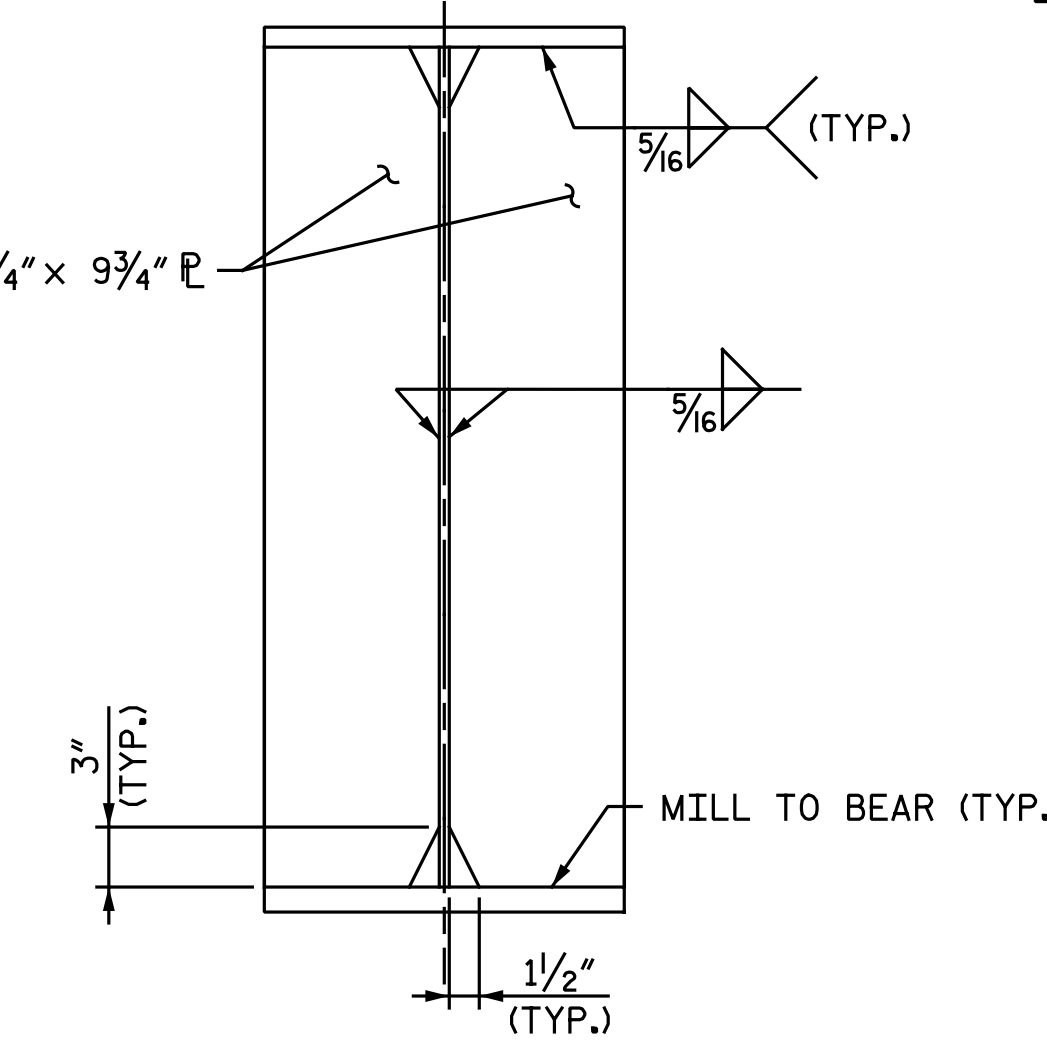
INTERMEDIATE DIAPHRAGM



INTERMEDIATE STIFFENER

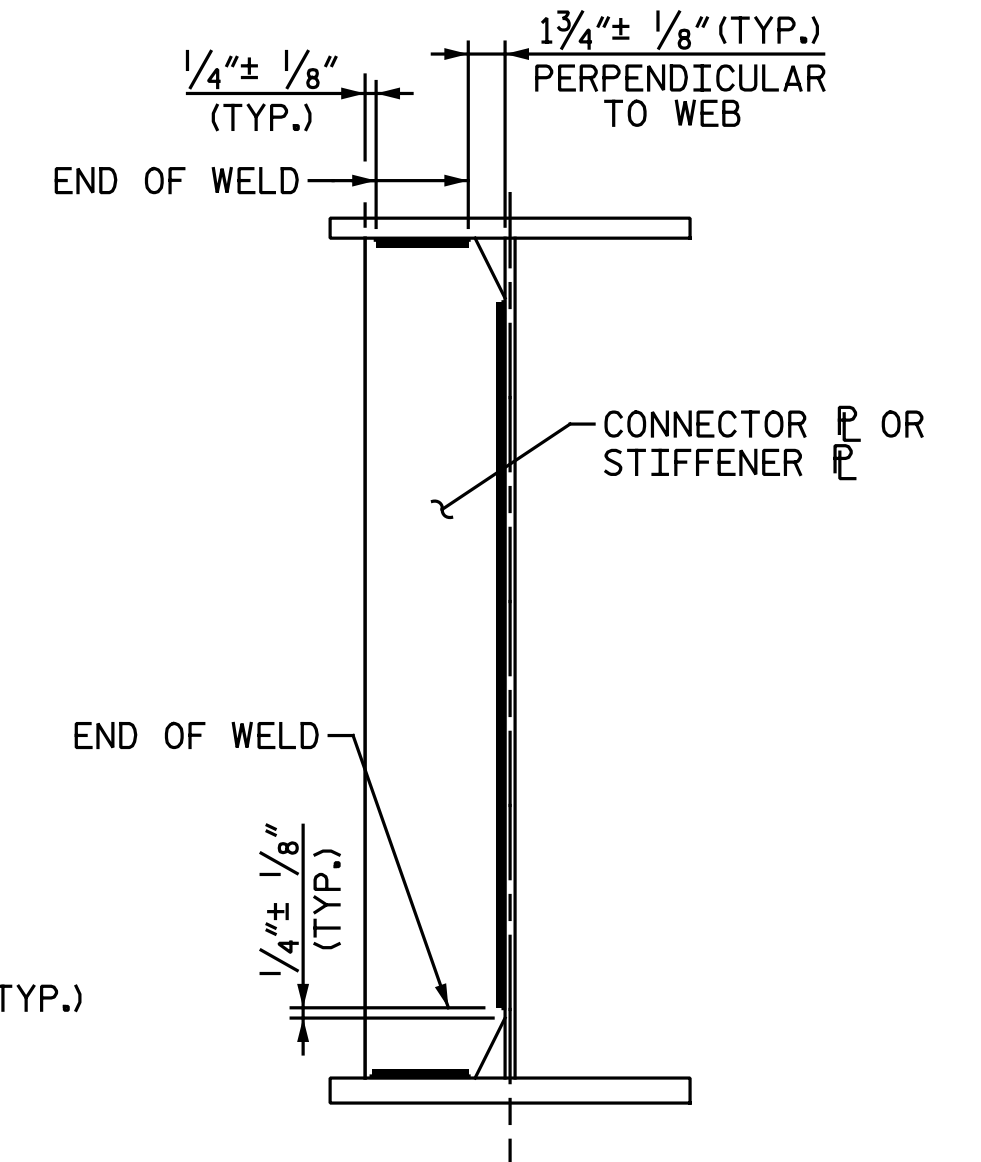


INTERMEDIATE DIAPHRAGM CONNECTOR



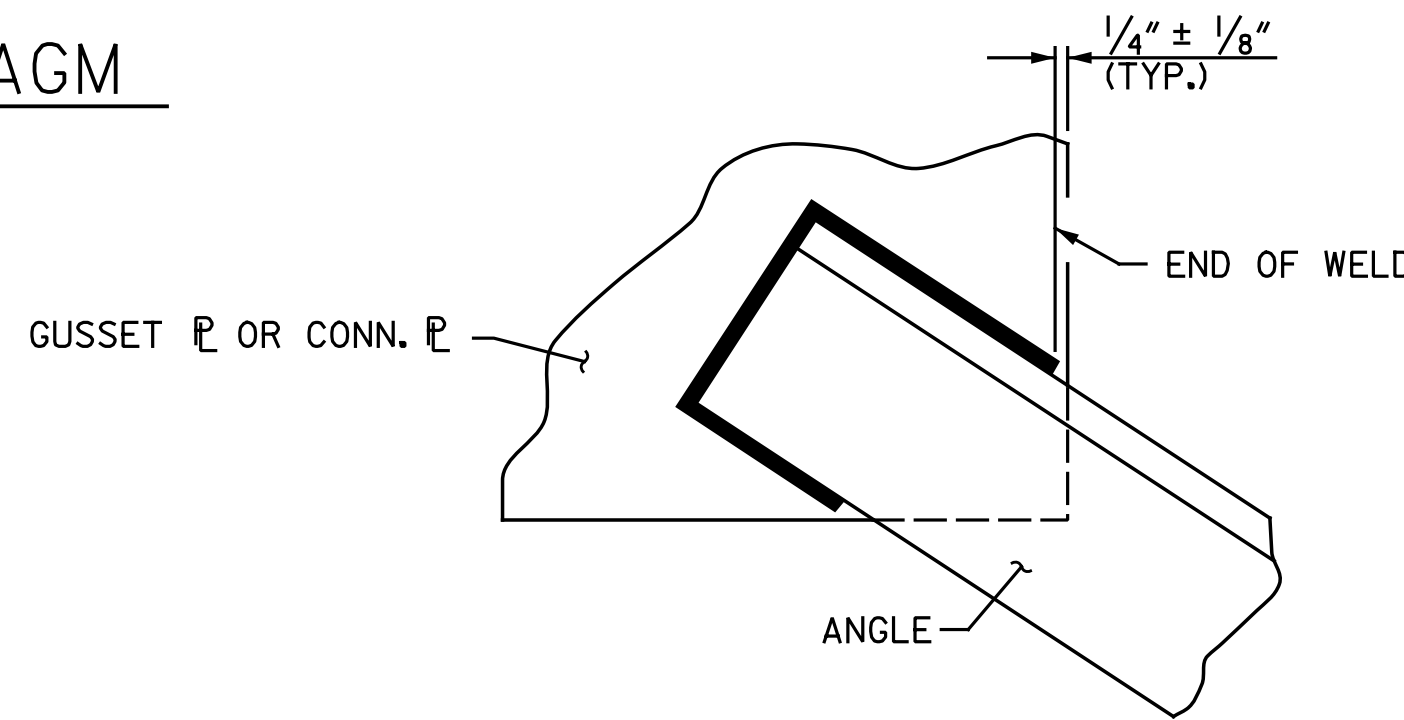
BEARING STIFFENER

BEARING STIFFENER MAY REQUIRE COPING IF WIDER THAN BOTTOM FLANGE.

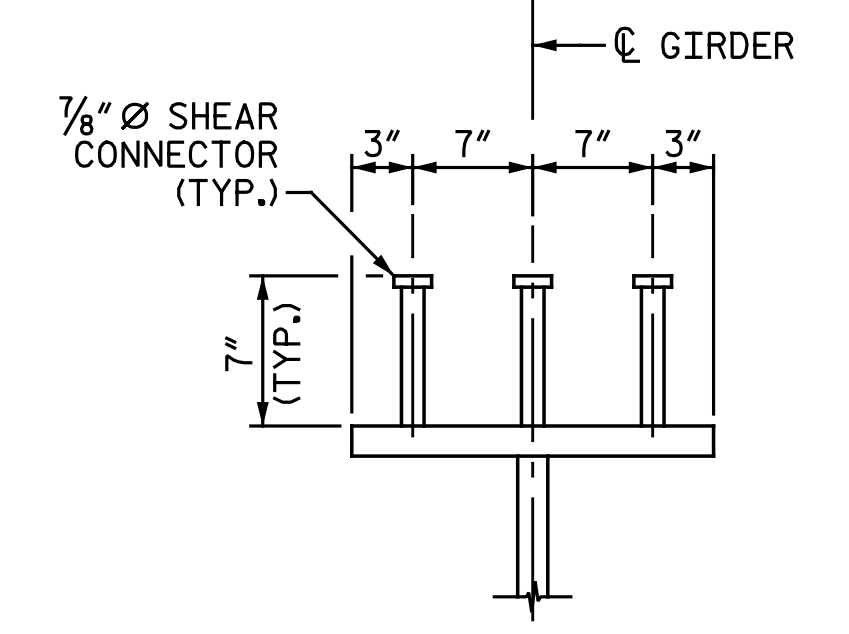
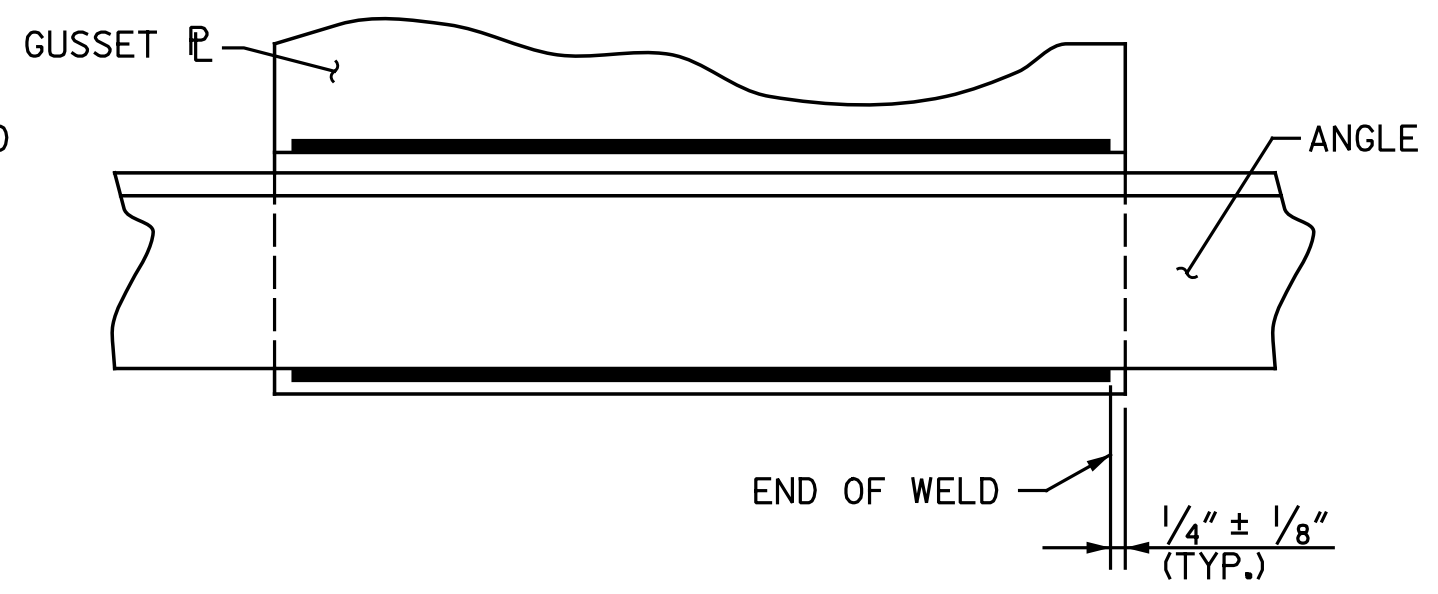


STIFFENER OR CONNECTOR PLATE CONNECTIONS WELD TERMINATION DETAILS

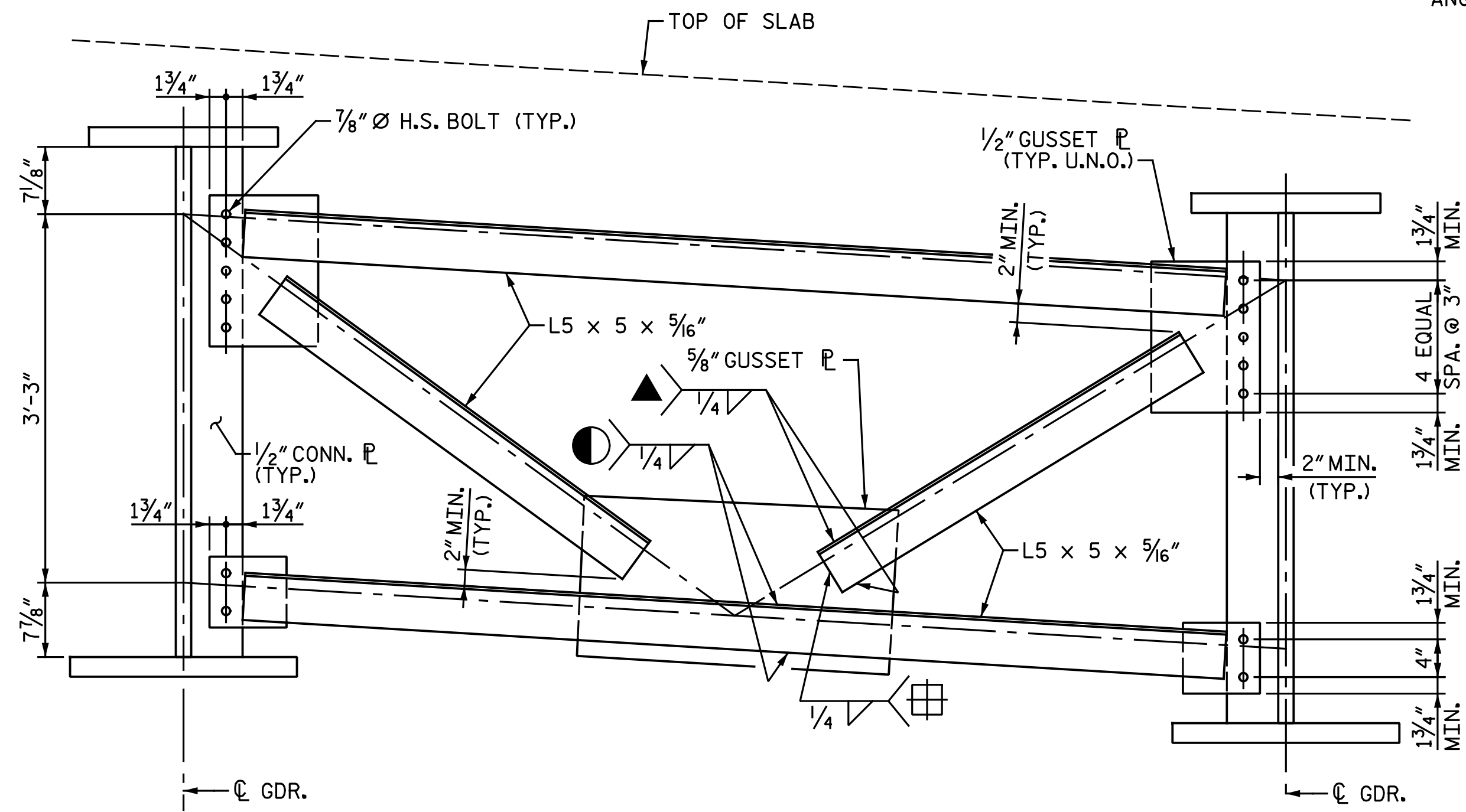
- ▲ 4" MIN. LENGTH EACH SIDE OF ANGLE, AT EACH ENDED WELDED END OF ANGLE
- ⊠ FULL WIDTH ACROSS END OF ANGLE, AT EACH WELDED END OF ANGLE
- 28" MIN. LENGTH EACH SIDE OF ANGLE, AT BOTTOM CHORD MID-LENGTH GUSSET PLATE CONN. SEE "ANGLE TO GUSSET PLATE CONNECTION WELD TERMINATION DETAILS"



ANGLE TO GUSSET PLATE CONNECTION WELD TERMINATION DETAILS

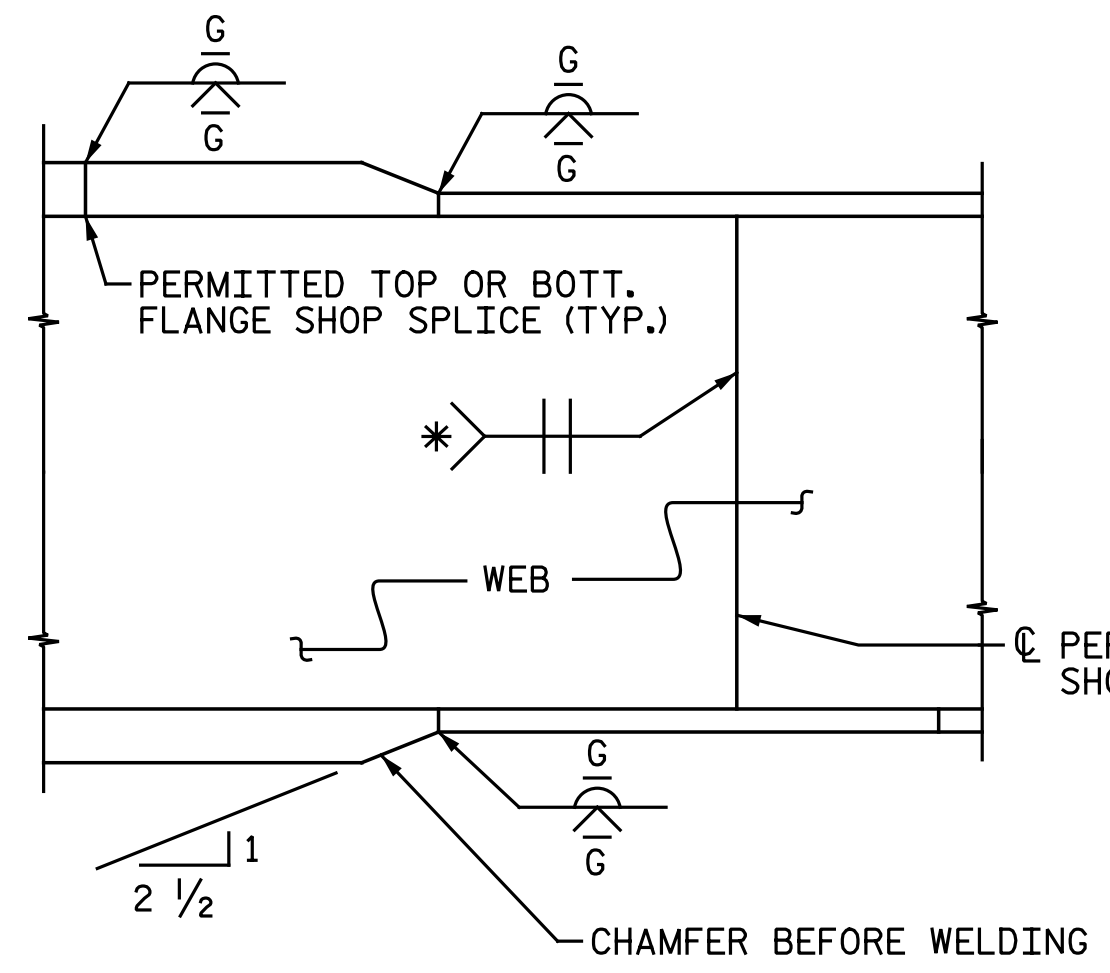


SHEAR CONNECTOR DETAIL



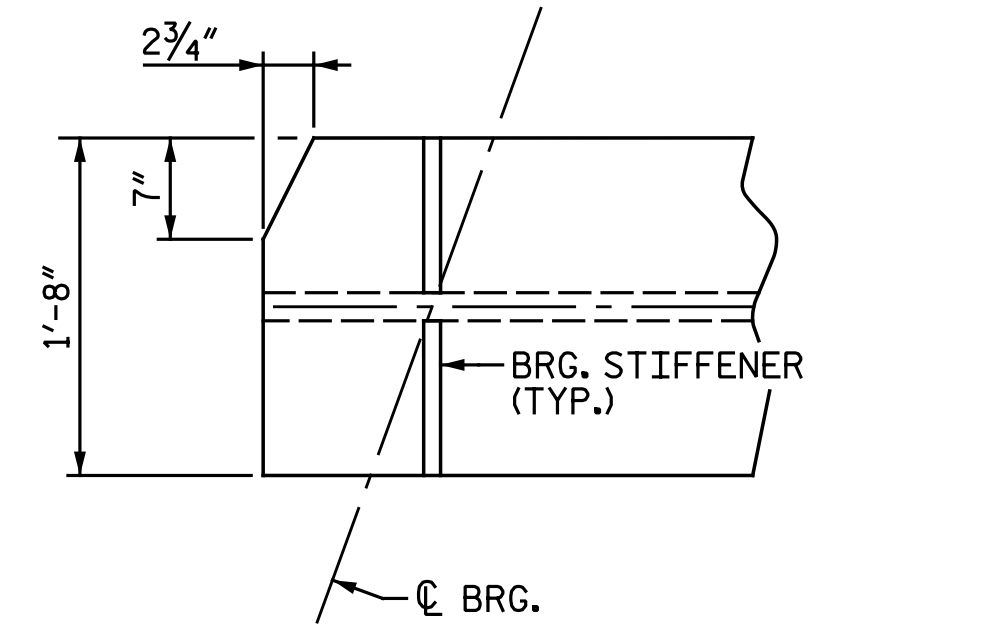
OPTIONAL INTERMEDIATE DIAPHRAGM

ANGLE TO GUSSET PLATE CONNECTION WELD TERMINATION DETAILS



TYPICAL FLANGE AND WEB BUTT JOINT DETAILS

* GRIND SMOOTH AND FLUSH ON OUTER FACE OF EXTERIOR GIRDERS



TOP FLANGE CLIP DETAIL

END BENT 1 SHOWN, END BENT 2 SIMILAR BY ROTATION



PROJECT NO. R-5703
LENOIR COUNTY
STATION: 166+72.51 -L-
SHEET 2 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
STRUCTURAL STEEL DETAILS

LEFT LANE

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CHECKED BY : D. A. COLETTI DATE : 6-8-17

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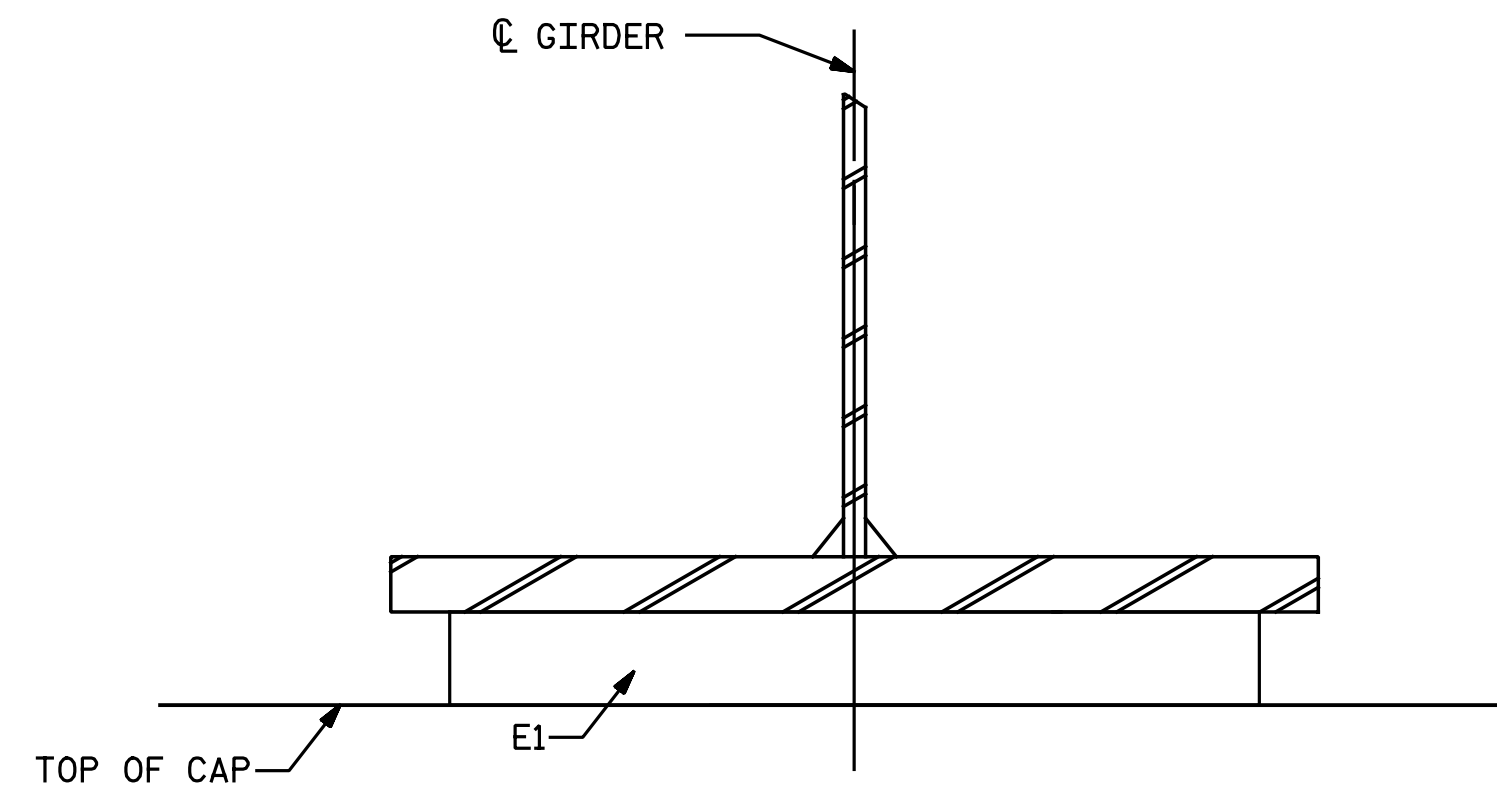
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1			3			TOTAL SHEETS 34
2			4			

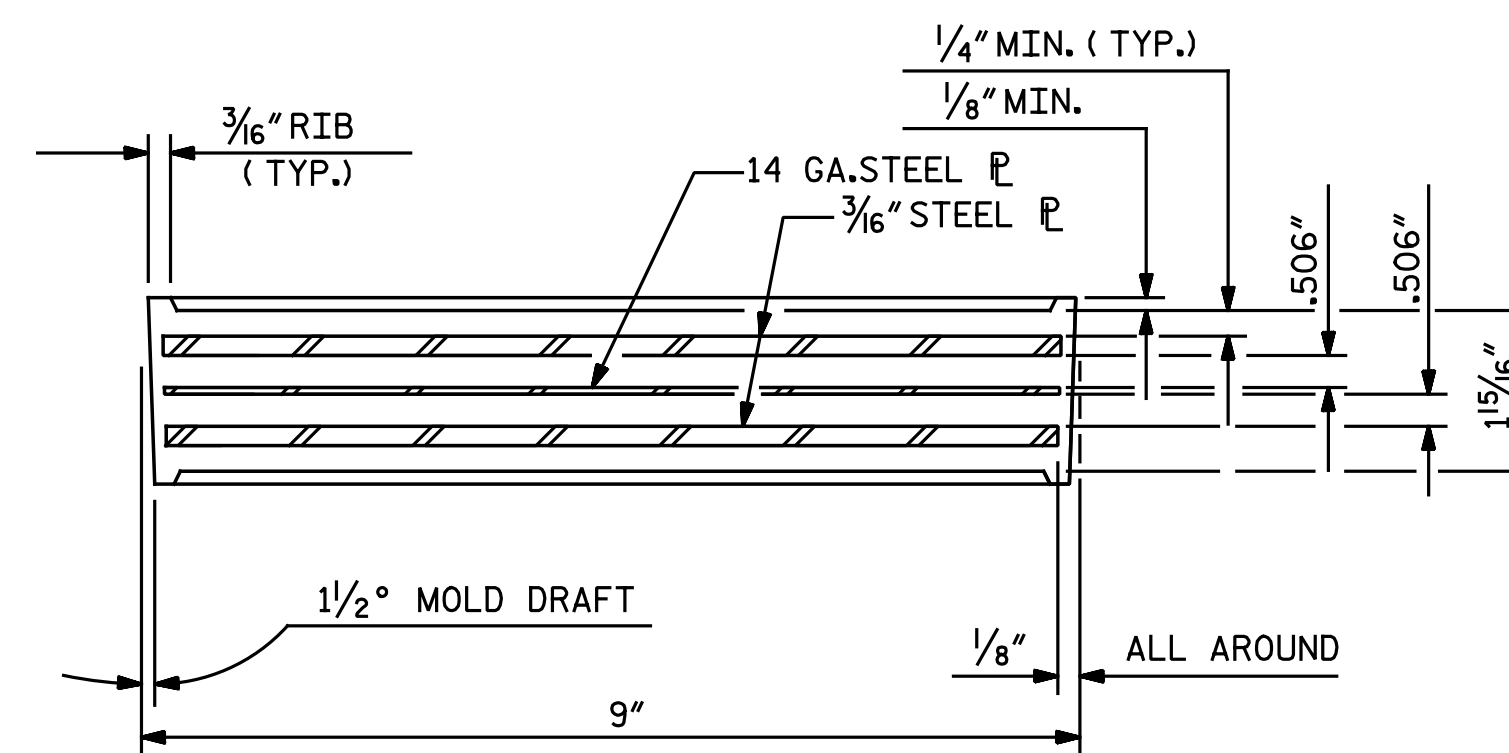
NOTES:

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

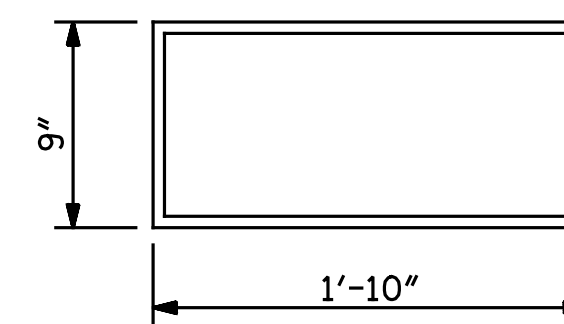


SECTION
AT INTEGRAL END BENTS



TYPICAL SECTION OF ELASTOMERIC BEARINGS

MAXIMUM ALLOWABLE SERVICE LOADS	
D.L.+L.L. (NO IMPACT)	
TYPE IV	225 k



E1 (12 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

TYPE IV

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 ELASTOMERIC BEARING
 DETAILS

LEFT LANE

8/14/2017
 DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED

Michael Baker INTERNATIONAL
 Michael Baker Engineering
 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No.: F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-13
1			3			TOTAL SHEETS
2			4			34

DRAWN BY : N. B. SPEAKS DATE : 7-19-17
 CHECKED BY : D. A. COLETTI DATE : 7-19-17

NOTES:
 VALUES GIVEN ARE AT TWENTIETH POINTS BETWEEN CENTERLINE OF BEARINGS.
 UPWARD DEFLECTIONS ARE INDICATED WITH A " - " SIGN.

DEAD LOAD DEFLECTION AND CAMBER ORDINATES FOR SPAN A

GIRDER 1

20TH POINTS	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019	0.037	0.054	0.069	0.083	0.094	0.103	0.109	0.113	0.115	0.113	0.109	0.103	0.094	0.083	0.069	0.054	0.037	0.019	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	0.063	0.125	0.182	0.233	0.277	0.315	0.345	0.367	0.381	0.385	0.381	0.367	0.345	0.315	0.277	0.233	0.182	0.125	0.063	0.000
DEFLECTION DUE TO WEIGHT OF RAIL	0.000	0.009	0.017	0.025	0.032	0.038	0.043	0.048	0.051	0.052	0.053	0.052	0.051	0.048	0.043	0.038	0.032	0.025	0.017	0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.091	0.179	0.261	0.334	0.398	0.452	0.496	0.527	0.547	0.553	0.547	0.527	0.496	0.452	0.398	0.334	0.261	0.179	0.091	0.000
VERTICAL CURVE ORDINATE	0.000	0.016	0.031	0.044	0.056	0.065	0.073	0.079	0.083	0.086	0.087	0.086	0.083	0.079	0.073	0.065	0.056	0.044	0.031	0.016	0.000
SUPERLEVATION ORDINATE	0.000	-0.008	-0.015	-0.021	-0.027	-0.031	-0.035	-0.038	-0.040	-0.041	-0.042	-0.041	-0.040	-0.038	-0.035	-0.031	-0.027	-0.021	-0.015	-0.008	0.000
REQUIRED CAMBER	0"	1 3/6"	2 5/6"	3 1/6"	4 3/8"	5 3/6"	5 7/8"	6 1/6"	6 7/8"	7 1/8"	7 3/6"	7 1/8"	6 7/8"	6 1/6"	5 7/8"	5 3/6"	4 3/8"	3 1/6"	2 5/6"	1 3/6"	0"

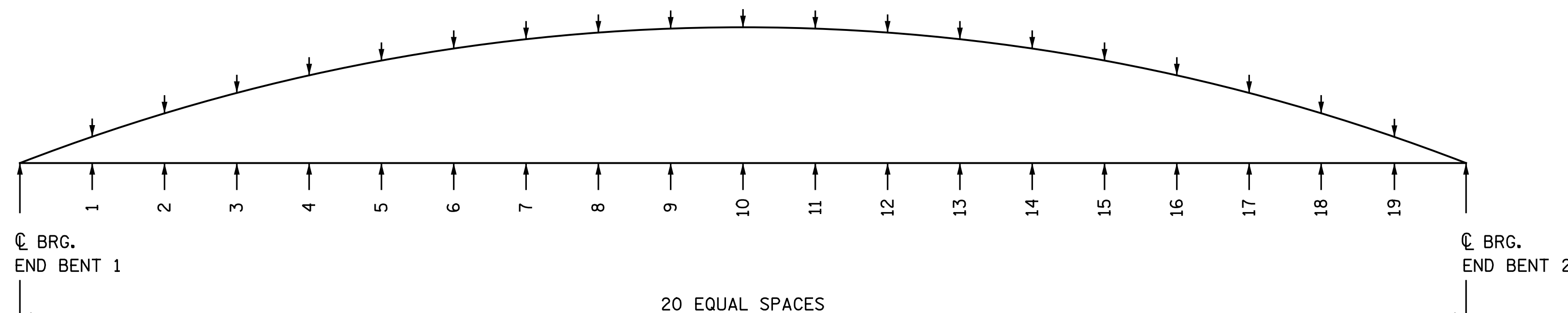
GIRDER 2

20TH POINTS	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019	0.037	0.054	0.069	0.083	0.094	0.103	0.109	0.113	0.115	0.113	0.109	0.103	0.094	0.083	0.069	0.054	0.037	0.019	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	0.063	0.124	0.181	0.232	0.277	0.314	0.344	0.366	0.380	0.384	0.380	0.366	0.344	0.314	0.277	0.232	0.181	0.124	0.063	0.000
DEFLECTION DUE TO WEIGHT OF RAIL	0.000	0.009	0.017	0.025	0.032	0.039	0.044	0.048	0.051	0.053	0.054	0.053	0.051	0.048	0.044	0.039	0.032	0.025	0.017	0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.091	0.179	0.261	0.334	0.398	0.452	0.495	0.527	0.546	0.553	0.546	0.527	0.495	0.452	0.398	0.334	0.261	0.179	0.091	0.000
VERTICAL CURVE ORDINATE	0.000	0.017	0.031	0.044	0.056	0.065	0.073	0.079	0.084	0.086	0.087	0.086	0.084	0.079	0.073	0.065	0.056	0.045	0.031	0.017	0.000
SUPERLEVATION ORDINATE	0.000	-0.008	-0.015	-0.021	-0.027	-0.031	-0.035	-0.038	-0.040	-0.041	-0.042	-0.041	-0.040	-0.038	-0.035	-0.031	-0.027	-0.021	-0.015	-0.008	0.000
REQUIRED CAMBER	0"	1 3/6"	2 3/8"	3 1/6"	4 3/8"	5 3/6"	5 7/8"	6 1/6"	6 7/8"	7 1/8"	7 3/6"	7 1/8"	6 7/8"	6 1/6"	5 7/8"	5 3/6"	4 3/8"	3 1/6"	2 5/6"	1 3/6"	0"

GIRDER 3

20TH POINTS	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019	0.037	0.054	0.069	0.083	0.094	0.103	0.109	0.113	0.115	0.113	0.109	0.103	0.094	0.083	0.069	0.054	0.037	0.019	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	0.063	0.124	0.181	0.231	0.276	0.313	0.343	0.365	0.379	0.383	0.379	0.365	0.343	0.313	0.276	0.231	0.181	0.124	0.063	0.000
DEFLECTION DUE TO WEIGHT OF RAIL	0.000	0.009	0.017	0.025	0.032	0.039	0.044	0.048	0.051	0.053	0.054	0.053	0.051	0.048	0.044	0.039	0.032	0.025	0.017	0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.090	0.179	0.260	0.333	0.397	0.451	0.494	0.526	0.545	0.552	0.545	0.526	0.494	0.451	0.397	0.333	0.260	0.179	0.090	0.000
VERTICAL CURVE ORDINATE	0.000	0.017	0.032	0.045	0.056	0.066	0.074	0.080	0.084	0.087	0.088	0.087	0.084	0.080	0.074	0.066	0.056	0.045	0.032	0.017	0.000
SUPERLEVATION ORDINATE	0.000	-0.008	-0.015	-0.021	-0.027	-0.032	-0.035	-0.038	-0.040	-0.042	-0.042	-0.042	-0.040	-0.038	-0.035	-0.031	-0.027	-0.021	-0.015	-0.008	0.000
REQUIRED CAMBER	0"	1 3/6"	2 5/6"	3 3/8"	4 3/8"	5 3/6"	5 7/8"	6 1/6"	6 13/6"	7 1/6"	7 3/6"	7 1/6"	6 13/6"	6 1/6"	5 7/8"	5 3/6"	4 3/8"	3 3/8"	2 5/6"	1 3/6"	0"

* INCLUDES SLAB, BUILDUPS, AND STAY-IN-PLACE FORMS. DEFLECTIONS BASED ON SLAB POUR SEQUENCE SHOWN ON "BILL OF MATERIAL" SHEET.
 ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "REQUIRED CAMBER" WHICH IS SHOWN IN INCHES (FRACTION FORM).



PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 1 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 DEAD LOAD DEFLECTION
 AND CAMBER ORDINATES

LEFT LANE

8/14/2017
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Michael Baker
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 NC License No.: F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-14
1			3			TOTAL SHEETS
2			4			34

DRAWN BY : C. E. MAYHEW DATE : 7-18-17
 CHECKED BY : D. A. COLETTI DATE : 7-19-17

SCHEMATIC CAMBER ORDINATES

NOTES:

VALUES GIVEN ARE AT TWENTIETH POINTS BETWEEN CENTERLINE OF BEARINGS.

UPWARD DEFLECTIONS ARE INDICATED WITH A " - " SIGN.

DEAD LOAD DEFLECTION AND CAMBER ORDINATES FOR SPAN A

GIRDER 4

20TH POINTS	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019	0.037	0.054	0.069	0.083	0.094	0.103	0.109	0.113	0.115	0.113	0.109	0.103	0.094	0.083	0.069	0.054	0.037	0.019	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	0.063	0.124	0.181	0.231	0.276	0.313	0.343	0.365	0.379	0.383	0.379	0.365	0.343	0.313	0.276	0.231	0.181	0.124	0.063	0.000
DEFLECTION DUE TO WEIGHT OF RAIL	0.000	0.009	0.017	0.025	0.032	0.039	0.044	0.048	0.051	0.053	0.054	0.053	0.051	0.048	0.044	0.039	0.032	0.025	0.017	0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.090	0.179	0.260	0.333	0.397	0.451	0.494	0.526	0.545	0.552	0.545	0.526	0.494	0.451	0.397	0.333	0.260	0.179	0.090	0.000
VERTICAL CURVE ORDINATE	0.000	0.017	0.032	0.045	0.057	0.066	0.074	0.081	0.085	0.088	0.088	0.088	0.085	0.080	0.074	0.066	0.057	0.045	0.032	0.017	0.000
SUPERLEVATION ORDINATE	0.000	-0.008	-0.015	-0.021	-0.027	-0.032	-0.035	-0.038	-0.040	-0.042	-0.042	-0.042	-0.040	-0.038	-0.035	-0.032	-0.027	-0.022	-0.015	-0.008	0.000
REQUIRED CAMBER	0"	1 3/16"	2 5/16"	3 1/16"	4 3/8"	5 1/16"	5 7/8"	6 1/16"	6 7/8"	7 1/16"	7 3/16"	7 1/16"	6 7/8"	6 1/16"	5 7/8"	5 3/16"	4 3/8"	3 1/16"	2 5/16"	1 3/16"	0"

GIRDER 5

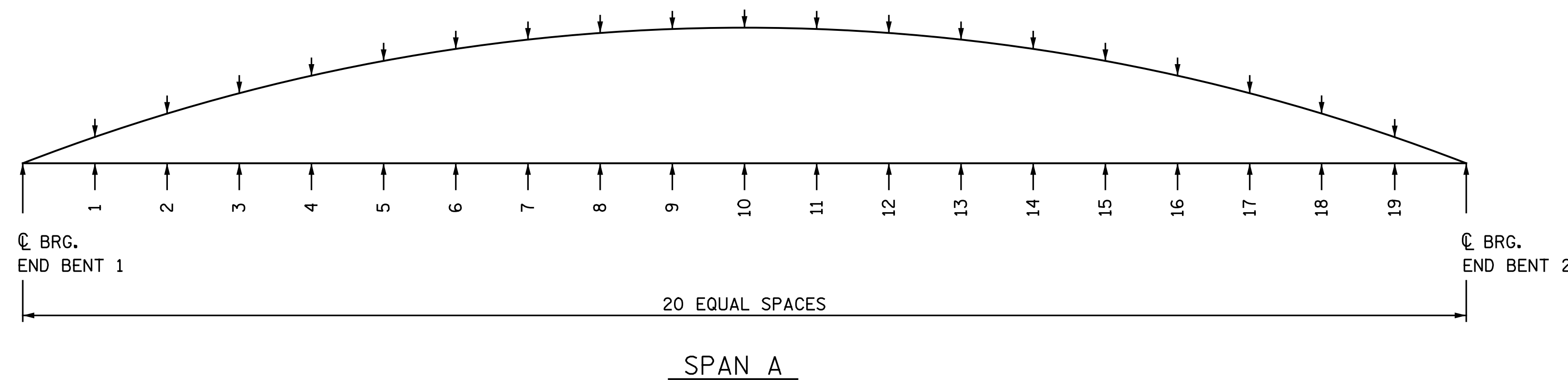
20TH POINTS	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019	0.037	0.054	0.069	0.083	0.094	0.103	0.109	0.113	0.115	0.113	0.109	0.103	0.094	0.083	0.069	0.054	0.037	0.019	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	0.063	0.124	0.181	0.232	0.277	0.314	0.344	0.366	0.380	0.384	0.380	0.366	0.344	0.314	0.277	0.232	0.181	0.124	0.063	0.000
DEFLECTION DUE TO WEIGHT OF RAIL	0.000	0.009	0.017	0.025	0.032	0.039	0.044	0.048	0.051	0.053	0.054	0.053	0.051	0.048	0.044	0.039	0.032	0.025	0.017	0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.091	0.179	0.261	0.334	0.398	0.452	0.495	0.527	0.546	0.553	0.546	0.527	0.495	0.452	0.398	0.334	0.261	0.179	0.091	0.000
VERTICAL CURVE ORDINATE	0.000	0.017	0.032	0.045	0.057	0.067	0.075	0.081	0.085	0.088	0.089	0.088	0.085	0.081	0.075	0.067	0.057	0.045	0.032	0.017	0.000
SUPERLEVATION ORDINATE	0.000	-0.008	-0.015	-0.022	-0.027	-0.032	-0.035	-0.038	-0.041	-0.042	-0.042	-0.042	-0.041	-0.038	-0.036	-0.032	-0.027	-0.022	-0.015	-0.008	0.000
REQUIRED CAMBER	0"	1 3/16"	2 3/8"	3 1/16"	4 3/8"	5 3/16"	5 7/8"	6 1/16"	6 7/8"	7 1/8"	7 3/16"	7 1/8"	6 7/8"	6 1/16"	5 7/8"	5 3/16"	4 3/8"	3 1/16"	2 3/8"	1 3/16"	0"

GIRDER 6

20TH POINTS	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019	0.037	0.054	0.069	0.083	0.094	0.103	0.109	0.113	0.115	0.113	0.109	0.103	0.094	0.083	0.069	0.054	0.037	0.019	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	0.063	0.125	0.182	0.233	0.277	0.315	0.345	0.367	0.381	0.385	0.381	0.367	0.345	0.315	0.277	0.233	0.182	0.125	0.063	0.000
DEFLECTION DUE TO WEIGHT OF RAIL	0.000	0.009	0.017	0.025	0.032	0.038	0.043	0.048	0.051	0.052	0.053	0.052	0.051	0.048	0.043	0.038	0.032	0.025	0.017	0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.091	0.179	0.261	0.334	0.398	0.452	0.496	0.527	0.547	0.553	0.547	0.527	0.496	0.452	0.398	0.334	0.261	0.179	0.091	0.000
VERTICAL CURVE ORDINATE	0.000	0.017	0.032	0.046	0.057	0.067	0.075	0.081	0.086	0.089	0.089	0.089	0.086	0.081	0.075	0.067	0.057	0.046	0.032	0.017	0.000
SUPERLEVATION ORDINATE	0.000	-0.008	-0.015	-0.022	-0.027	-0.032	-0.036	-0.039	-0.041	-0.042	-0.042	-0.042	-0.041	-0.039	-0.036	-0.032	-0.027	-0.022	-0.015	-0.008	0.000
REQUIRED CAMBER	0"	1 3/16"	2 3/8"	3 1/16"	4 3/8"	5 3/16"	5 7/8"	6 1/16"	6 7/8"	7 1/8"	7 3/16"	7 1/8"	6 7/8"	6 1/16"	5 7/8"	5 3/16"	4 3/8"	3 1/16"	2 3/8"	1 3/16"	0"

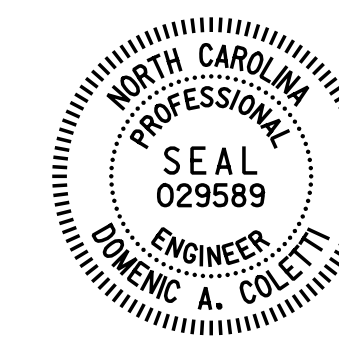
* INCLUDES SLAB, BUILDUPS, AND STAY-IN-PLACE FORMS. DEFLECTIONS BASED ON SLAB POUR SEQUENCE SHOWN ON "BILL OF MATERIAL" SHEET.

ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "REQUIRED CAMBER" WHICH IS SHOWN IN INCHES (FRACTION FORM).



PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 2 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 DEAD LOAD DEFLECTION
 AND CAMBER ORDINATES

LEFT LANE

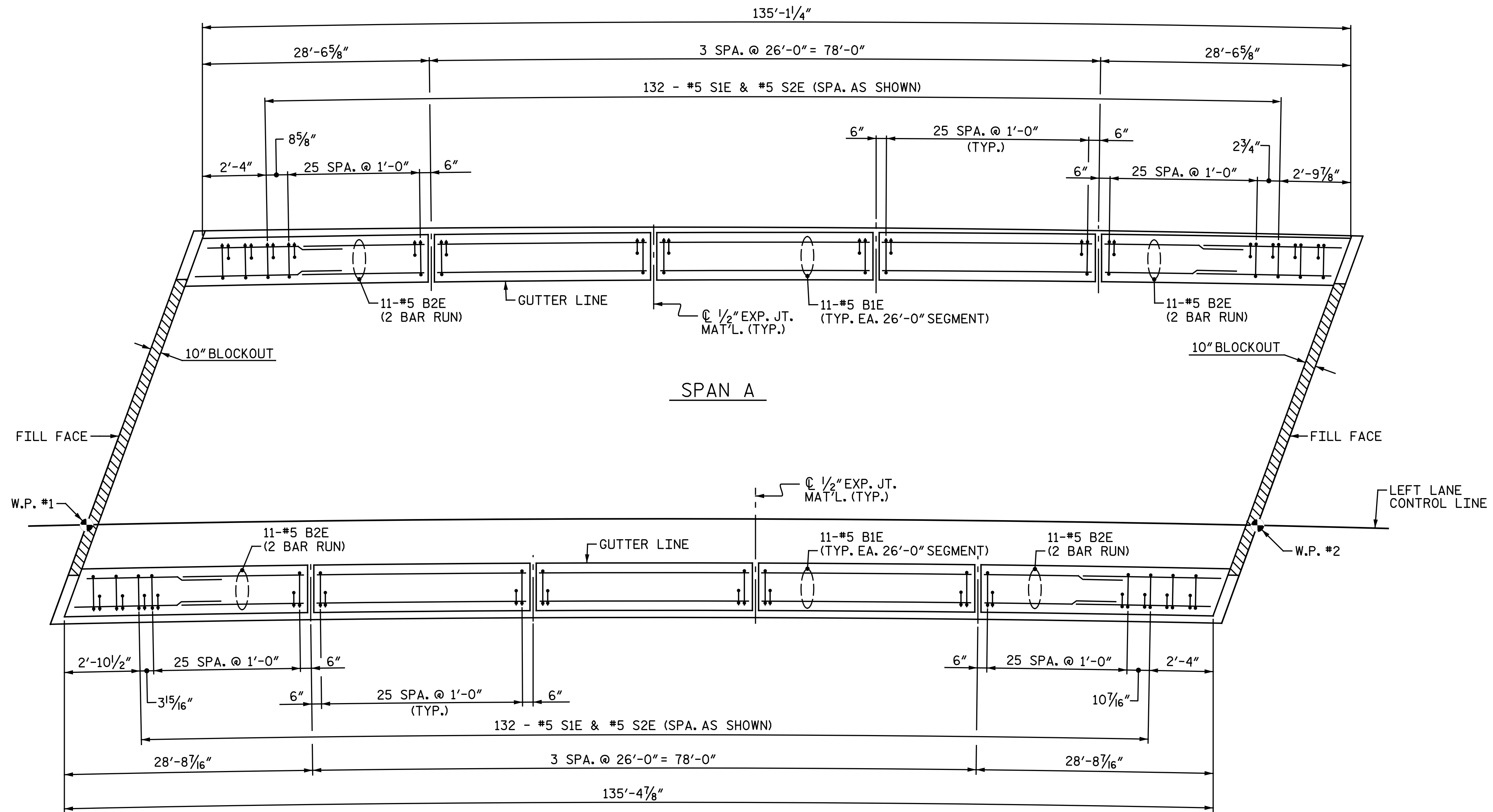
8/14/2017
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 NC License No.: F-1084

REVISIONS		SHEET NO.	
NO.	DATE	NO.	DATE
1		3	
2		4	

TOTAL SHEETS: 34

DRAWN BY : C. E. MAYHEW DATE : 7-18-17
 CHECKED BY : D. A. COLETTI DATE : 7-19-17

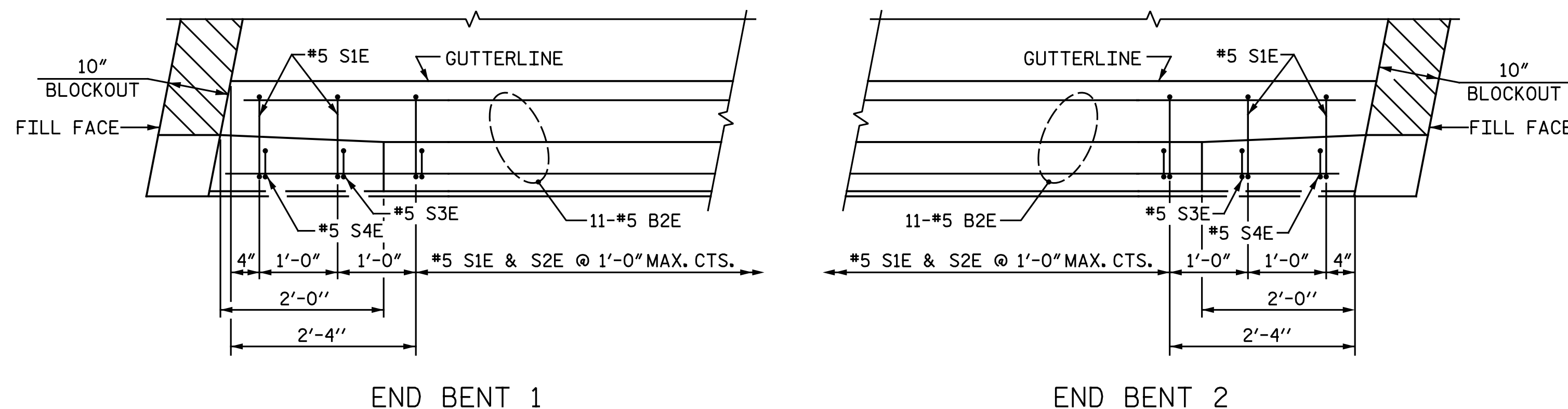
SCHEMATIC CAMBER ORDINATES



PLAN OF BARRIER RAIL

NOTE: ALL BARRIER RAIL DIMENSIONS ARE MEASURED ALONG THE ARC AT THE BACK FACE OF THE BARRIER RAIL.

SPLICE LENGTHS	
BAR SIZE	EPOXY COATED
#5	3'-5"



END BENT 1

END BENT 2

PLAN

(RIGHT RAIL SHOWN, LEFT RAIL SIMILAR)

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 1 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

CONCRETE BARRIER RAIL

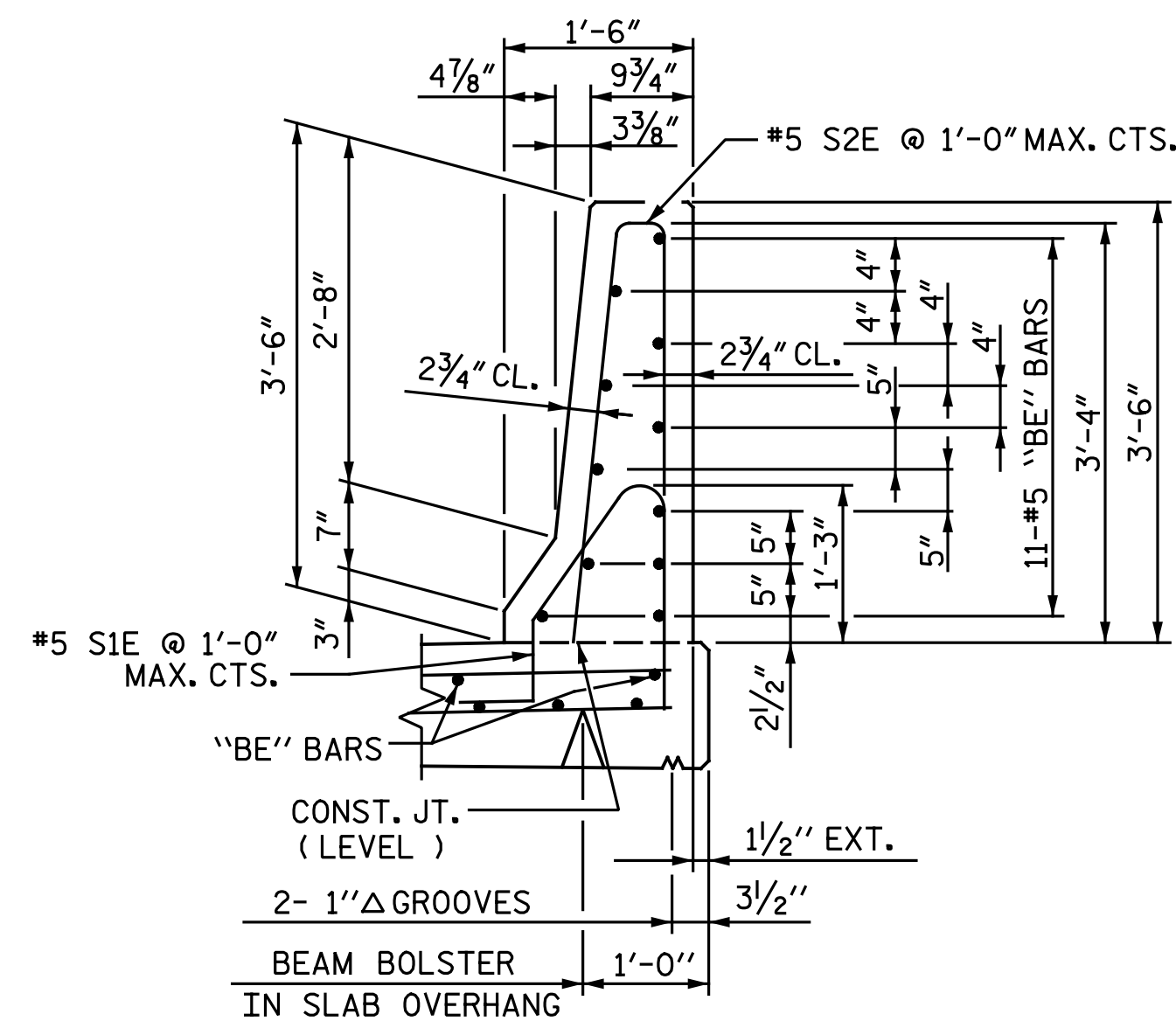
LEFT LANE

ASSEMBLED BY : N. B. SPEAKS	DATE : 3-29-17
CHECKED BY : D. A. COLETTI	DATE : 6-8-17
DRAWN BY : ARB 5/87	REV. 10/1/11 MAA/GM
CHECKED BY : SJD 9/87	REV. 7/12 MAA/GM
	REV. 6/13 MAA/GM

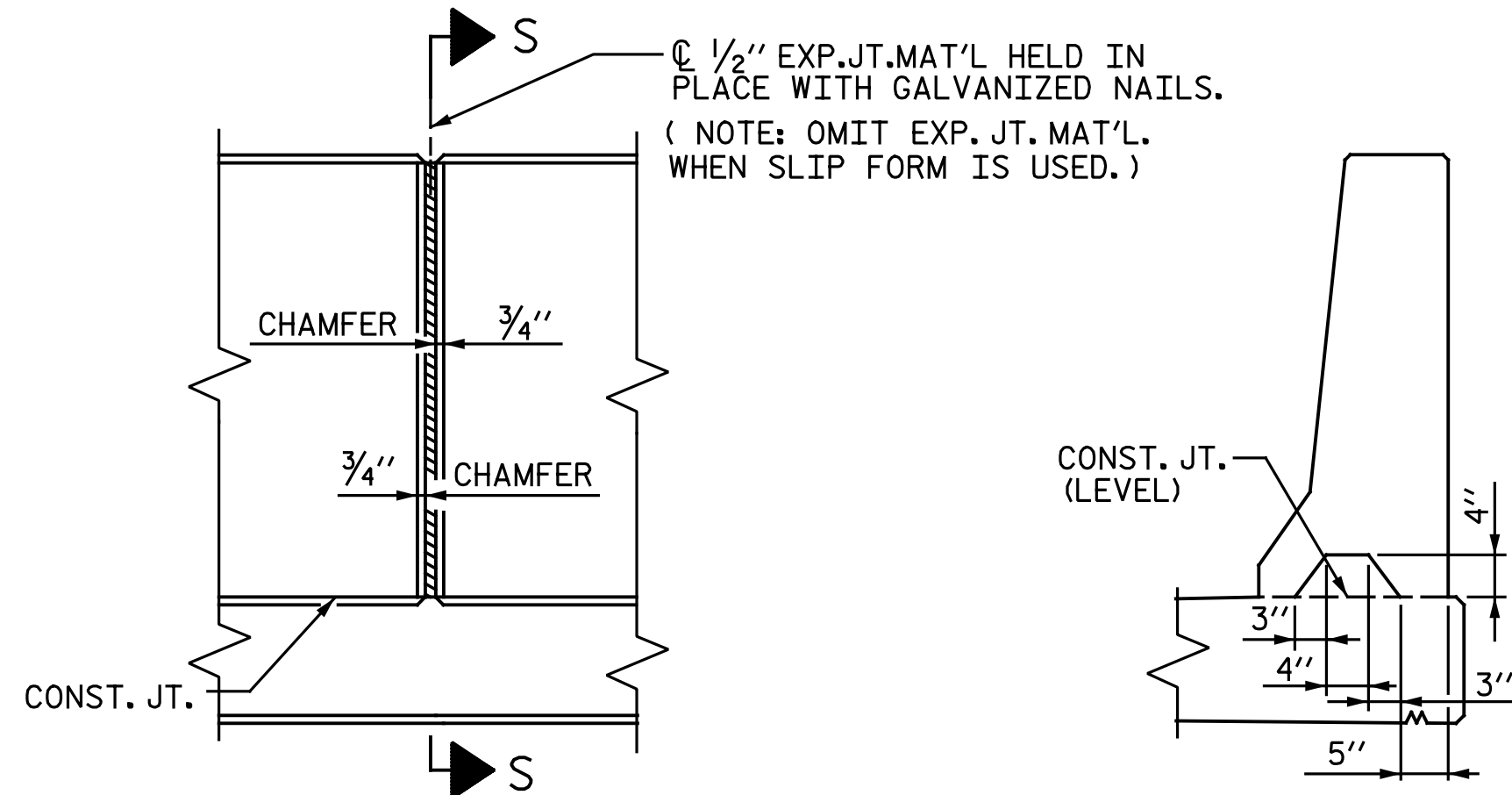
8/14/2017
 DOCUMENT NOT CONSIDERED FINAL
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Michael Baker INTERNATIONAL
 Michael Baker Engineering
 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No. : F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-16
1			3			TOTAL SHEETS
2			4			34



SECTION THRU RAIL



ELEVATION AT EXPANSION JOINTS

SECTION S-S
AT DAM IN OPEN JOINT
(THIS IS TO BE USED ONLY
WHEN SLIP FORM IS USED)

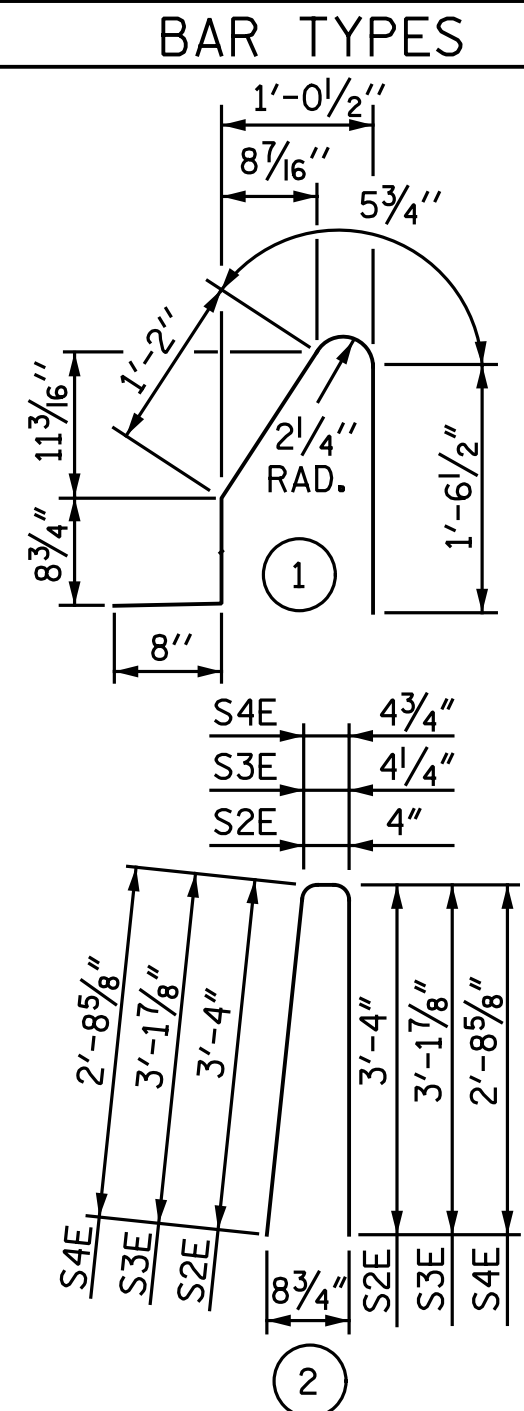
BARRIER RAIL DETAILS

NOTES:

THE BARRIER RAIL IN THE SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

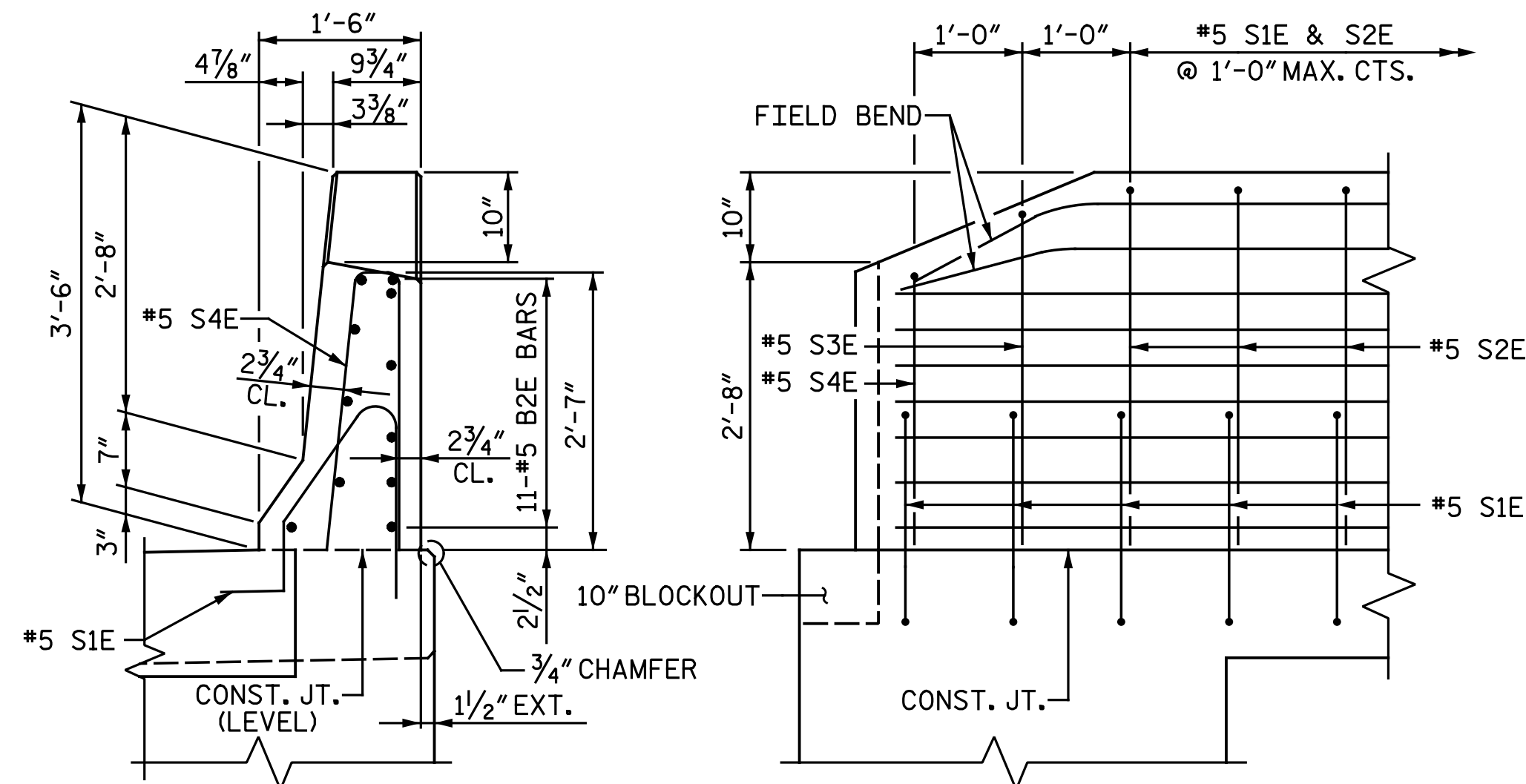
GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.



ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL

FOR CONCRETE BARRIER RAIL ONLY					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1E	66	#5	STR.	25' - 8"	1,767
B2E	88	#5	STR.	15' - 11"	1,461
S1E	272	#5	1	4' - 7"	1,300
S2E	264	#5	2	7' - 0"	1,927
S3E	4	#5	2	6' - 8"	28
S4E	4	#5	2	5' - 10"	24
EPOXY COATED REINFORCING STEEL				LBS.	6,507
CLASS AA CONCRETE				C.Y.	36.8
CONCRETE BARRIER RAIL				L.F.	270.51



END VIEW

SIDE VIEW

END OF RAIL DETAILS

ASSEMBLED BY : N. B. SPEAKS	DATE : 9-11-17
CHECKED BY : D. A. COLETTI	DATE : 9-12-17
DRAWN BY : ARB 5/87	REV. 10/1/11 MAA/GM
CHECKED BY : SJD 9/87	REV. 7/12 MAA/GM
	REV. 6/13 MAA/GM

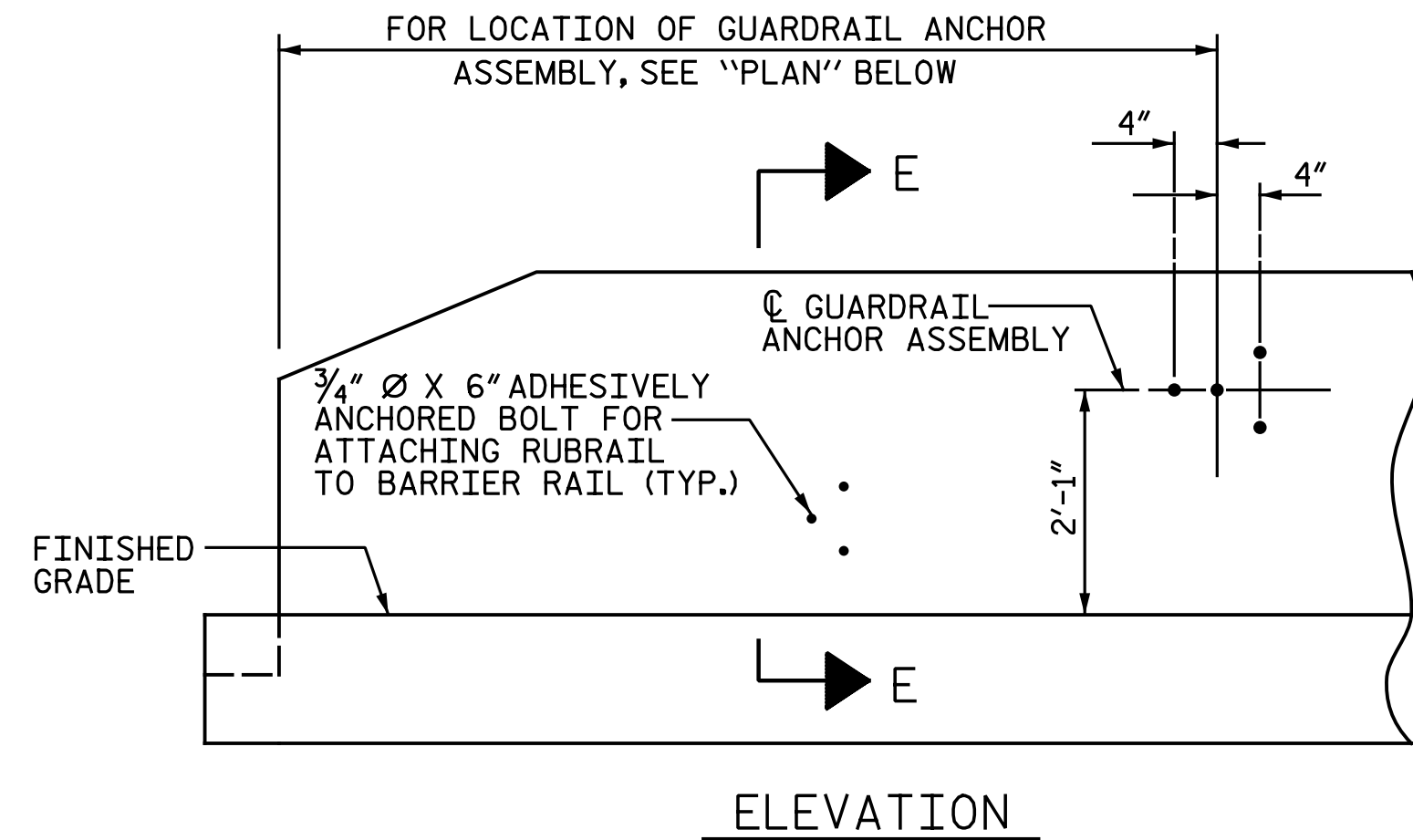
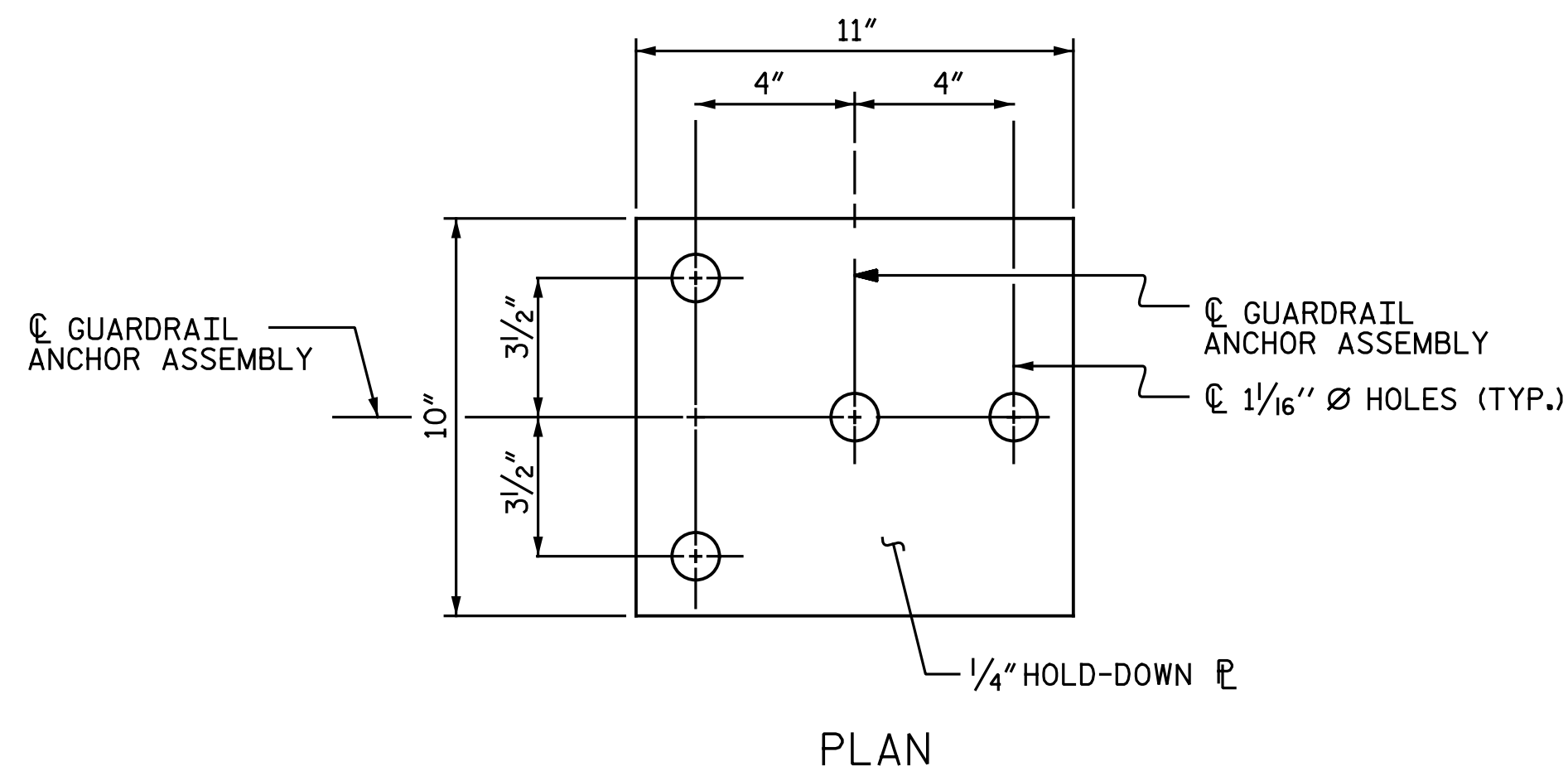
9/12/2017
 DOCUMENT NOT CONSIDERED FINAL
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 Michael Baker Engineering
 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No. : F-1084



PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
STANDARD CONCRETE BARRIER RAIL					
LEFT LANE					
REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS
					34



NOTES:

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

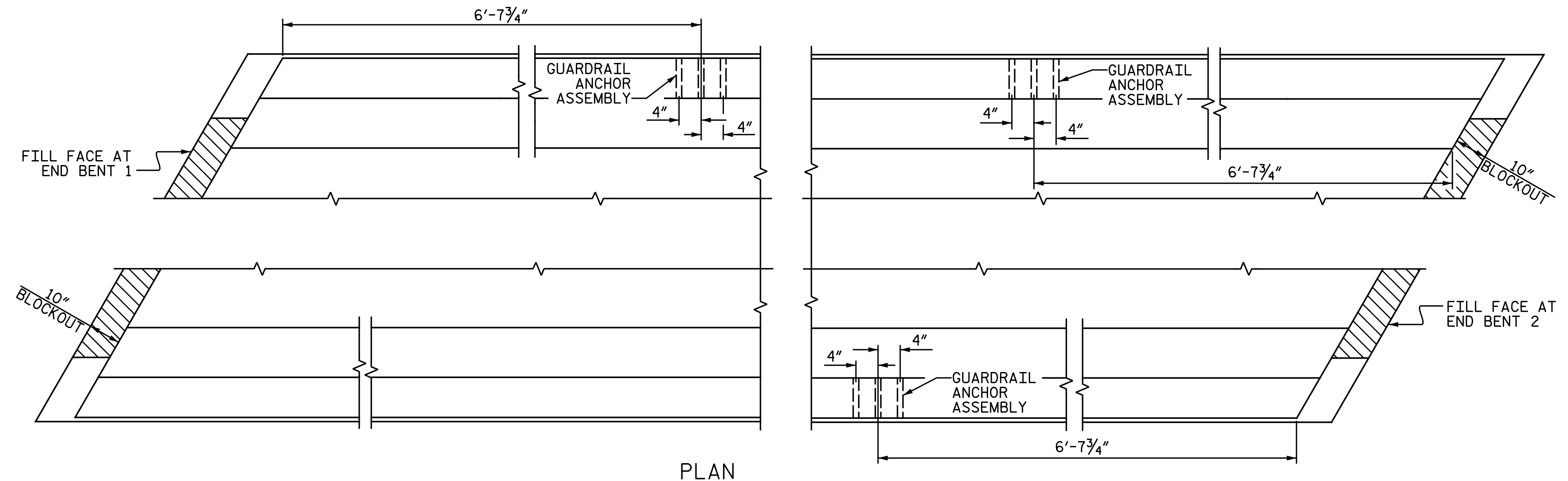
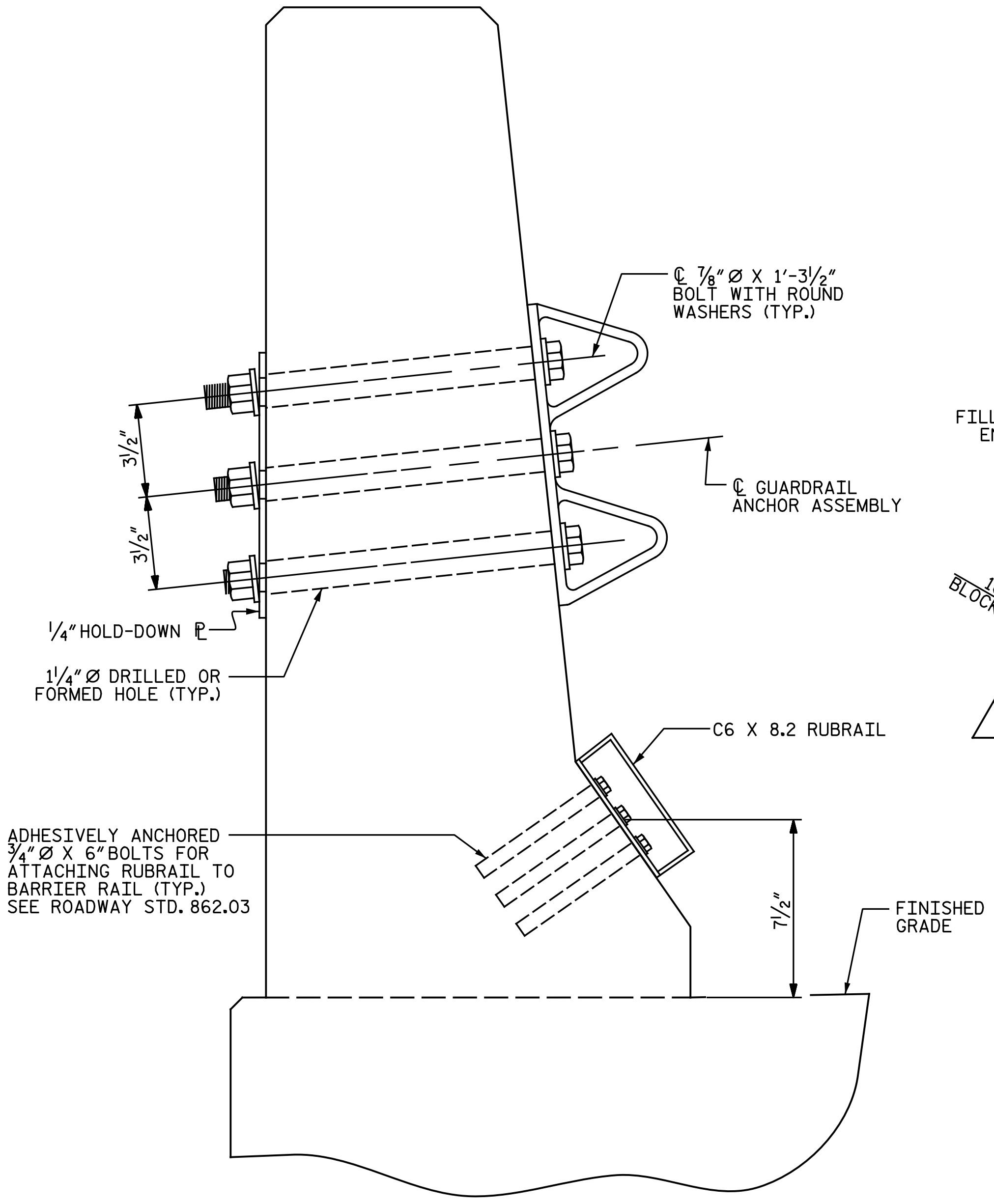
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

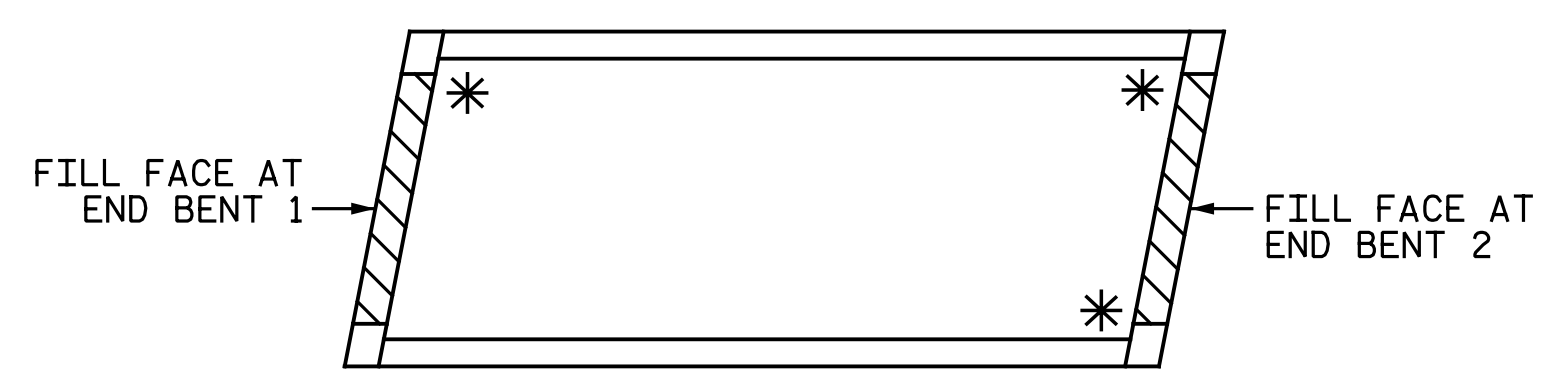
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



LOCATION OF ANCHORS FOR GUARDRAIL



* DENOTES GUARDRAIL ANCHOR ASSEMBLY

**SECTION E-E
GUARDRAIL ANCHOR ASSEMBLY DETAILS**

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 GUARDRAIL ANCHORAGE
 FOR BARRIER RAIL
 LEFT LANE

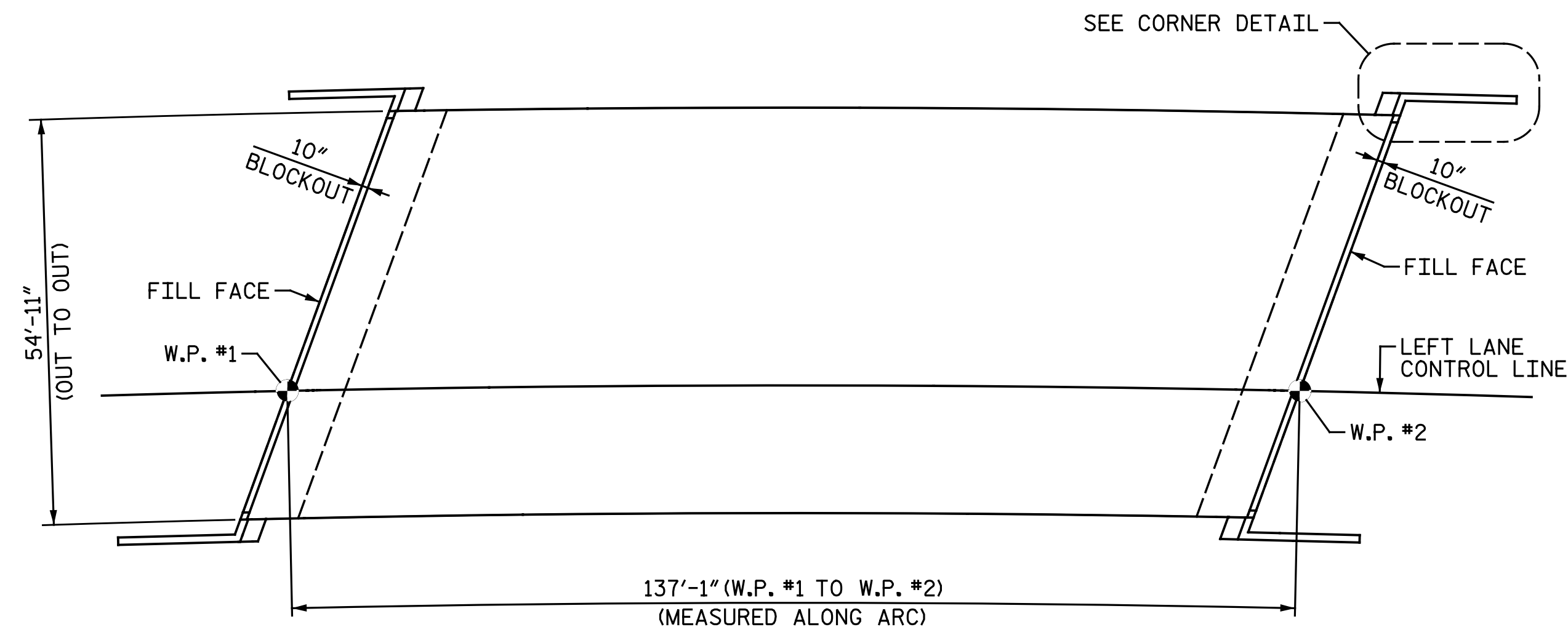
ASSEMBLED BY : N. B. SPEAKS	DATE : 9-11-17
CHECKED BY : D.A. COLETTI	DATE : 9-12-17
DRAWN BY : TLA 5/06	REV. 10/1/11 MAA/GM
CHECKED BY : GM 5/06	REV. 7/12 MAA/GM
	REV. 6/13 MAA/GM

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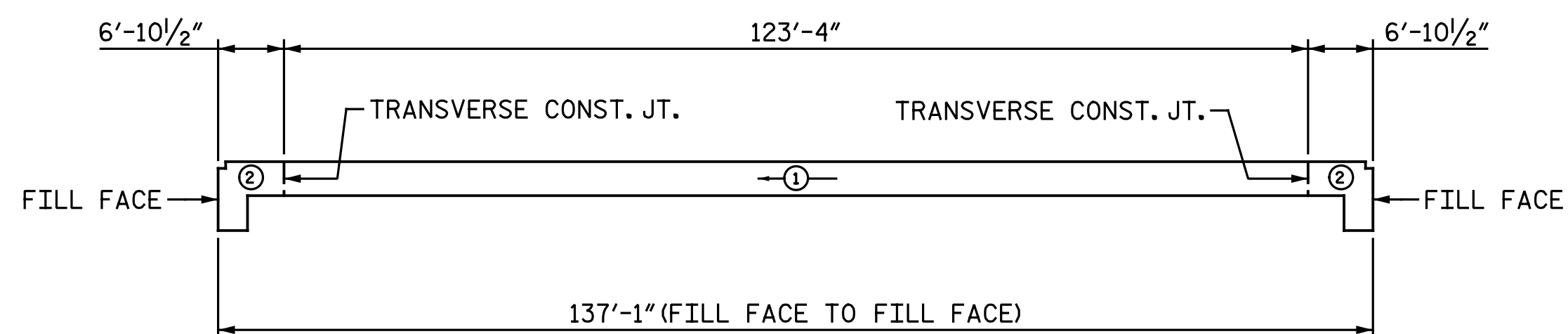
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1			3			34
2			4			

NOTES:
FOR BAR TYPES AND BAR SCHEDULE, SEE SHEET 2 OF 2.

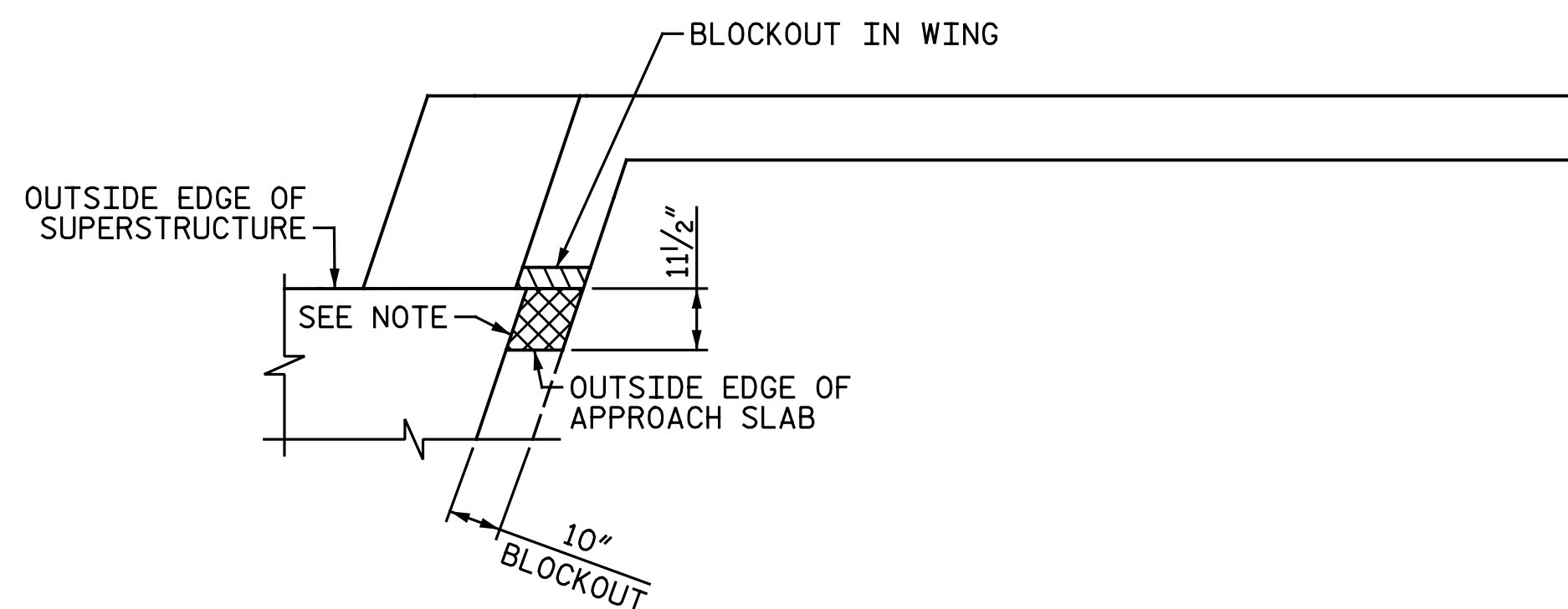


LAYOUT FOR COMPUTING AREA OF
REINFORCED CONCRETE DECK SLAB
(SQ. FT. = 7,525)



POURING SEQUENCE

⊕ DENOTES POUR NUMBER AND DIRECTION
(ALL DIMENSIONS ARE ALONG CONTROL LINE)



CORNER DETAIL

CONCRETE SHALL BE POURED IN THE CROSS-HATCHED AREA TO MATCH THE TOP OF END BENT WING ELEVATIONS. UNLESS OTHERWISE DIRECTED BY THE ENGINEER, THE CONCRETE IN THESE AREAS SHALL BE PLACED AT THE SAME TIME THE BLOCKOUTS IN THE END BENT WINGS ARE POURED AS NOTED ON SHEET 1 OF "INTEGRAL END BENT 1" AND "INTEGRAL END BENT 2" SHEETS.

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 166+72.51 -L-

SHEET 1 OF 2



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
BILL OF MATERIAL

LEFT LANE

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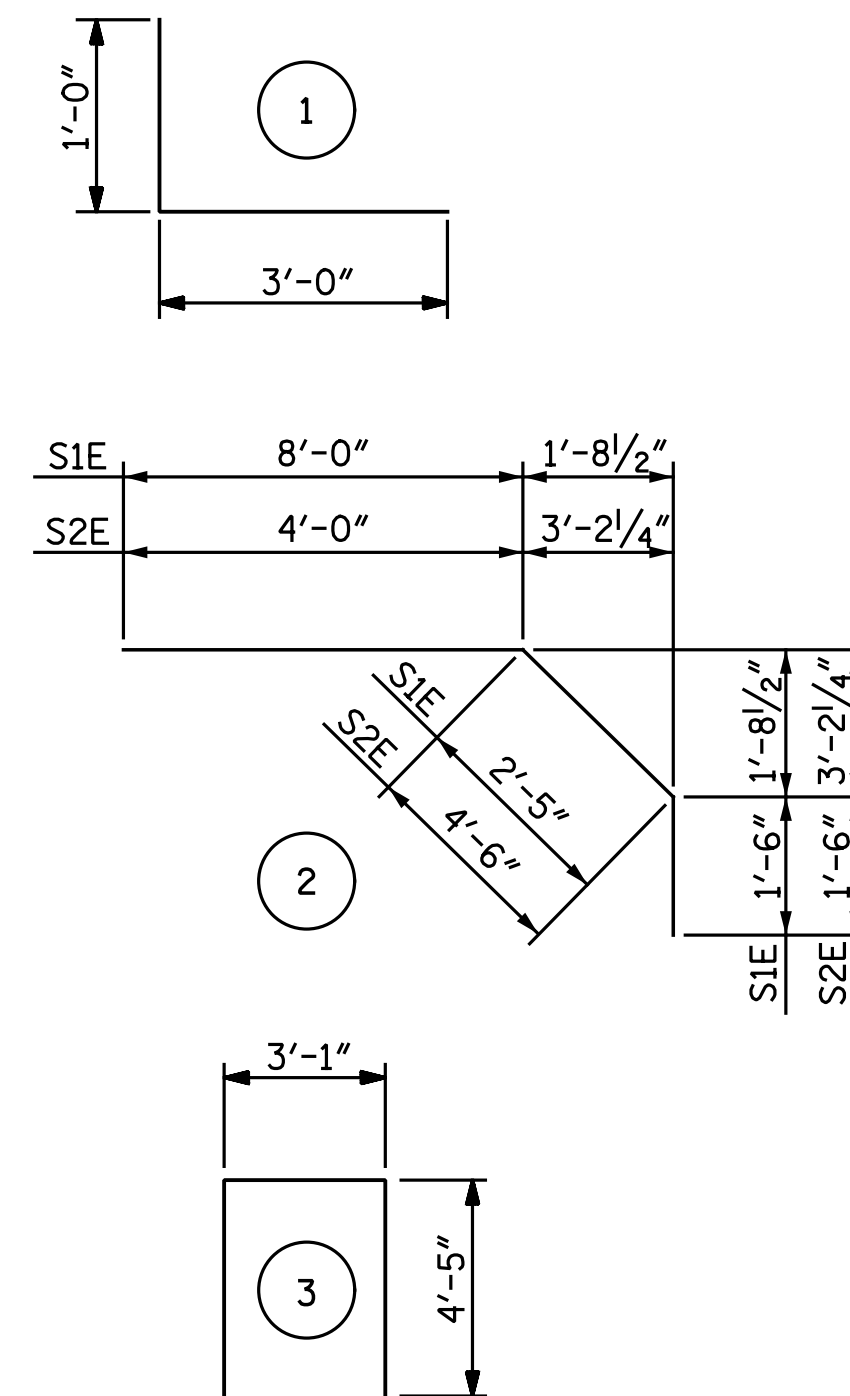
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-19
1			3			TOTAL SHEETS
2			4			34

DRAWN BY : C. E. MAYHEW DATE : 7-14-17
CHECKED BY : D. A. COLETTI DATE : 7-19-17

REINFORCING BAR SCHEDULE

SPAN A						SPAN A (CONT'D)						SPAN A (CONT'D)						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
A1E	230	#5	STR.	54' - 7"	13,094	A201	1	#5	STR.	2' - 2"	2	B1E	190	#4	STR.	28' - 8"	3,638	
A2	230	#5	STR.	54' - 7"	13,094	A202	1	#5	STR.	3' - 6"	4	B2	210	#5	STR.	46' - 6"	10,185	
A101E	1	#5	STR.	2' - 2"	2	A203	1	#5	STR.	4' - 10"	5	B3E	72	#6	STR.	29' - 2"	3,154	
A102E	1	#5	STR.	3' - 6"	4	A204	1	#5	STR.	6' - 3"	7	B4E	72	#6	STR.	31' - 4"	3,389	
A103E	1	#5	STR.	4' - 10"	5	A205	1	#5	STR.	7' - 7"	8	B5E	8	#4	1	4' - 0"	21	
A104E	1	#5	STR.	6' - 3"	7	A206	1	#5	STR.	8' - 11"	9	G1E	2	#5	STR.	58' - 5"	122	
A105E	1	#5	STR.	7' - 7"	8	A207	1	#5	STR.	10' - 3"	11	G2E	2	#5	STR.	57' - 7"	120	
A106E	1	#5	STR.	8' - 11"	9	A208	1	#5	STR.	11' - 8"	12	H1	24	#6	STR.	2' - 11"	105	
A107E	1	#5	STR.	10' - 3"	11	A209	1	#5	STR.	13' - 0"	14	K1	12	#6	STR.	58' - 5"	1,053	
A108E	1	#5	STR.	11' - 8"	12	A210	1	#5	STR.	14' - 4"	15	K2	12	#6	STR.	57' - 7"	1,038	
A109E	1	#5	STR.	13' - 0"	14	A211	1	#5	STR.	15' - 9"	16	S1E	102	#4	2	11' - 11"	812	
A110E	1	#5	STR.	14' - 4"	15	A212	1	#5	STR.	17' - 1"	18	S2E	98	#4	2	10' - 0"	655	
A111E	1	#5	STR.	15' - 9"	16	A213	1	#5	STR.	18' - 5"	19	U1	102	#6	3	11' - 11"	1,826	
A112E	1	#5	STR.	17' - 1"	18	A214	1	#5	STR.	19' - 10"	21	V1	12	#4	STR.	5' - 8"	45	
A113E	1	#5	STR.	18' - 5"	19	A215	1	#5	STR.	21' - 2"	22	REINFORCING STEEL					LBS.	29,605
A114E	1	#5	STR.	19' - 10"	21	A216	1	#5	STR.	22' - 6"	23	EPOXY COATED REINF. STEEL					LBS.	27,264
A115E	1	#5	STR.	21' - 2"	22	A217	1	#5	STR.	23' - 11"	25	"E" SUFFIX DENOTES EPOXY COATED REINFORCING STEEL						
A116E	1	#5	STR.	22' - 6"	23	A218	1	#5	STR.	25' - 3"	26							
A117E	1	#5	STR.	23' - 11"	25	A219	1	#5	STR.	26' - 7"	28							
A118E	1	#5	STR.	25' - 3"	26	A220	1	#5	STR.	28' - 0"	29							
A119E	1	#5	STR.	26' - 7"	28	A221	1	#5	STR.	29' - 4"	31							
A120E	1	#5	STR.	28' - 0"	29	A222	1	#5	STR.	30' - 9"	32							
A121E	1	#5	STR.	29' - 4"	31	A223	1	#5	STR.	32' - 1"	33							
A122E	1	#5	STR.	30' - 9"	32	A224	1	#5	STR.	33' - 5"	35							
A123E	1	#5	STR.	32' - 1"	33	A225	1	#5	STR.	34' - 10"	36							
A124E	1	#5	STR.	33' - 5"	35	A226	1	#5	STR.	36' - 2"	38							
A125E	1	#5	STR.	34' - 10"	36	A227	1	#5	STR.	37' - 6"	39							
A126E	1	#5	STR.	36' - 2"	38	A228	1	#5	STR.	38' - 11"	41							
A127E	1	#5	STR.	37' - 6"	39	A229	1	#5	STR.	40' - 3"	42							
A128E	1	#5	STR.	38' - 11"	41	A230	1	#5	STR.	41' - 7"	43							
A129E	1	#5	STR.	40' - 3"	42	A231	1	#5	STR.	43' - 0"	45							
A130E	1	#5	STR.	41' - 7"	43	A232	1	#5	STR.	44' - 4"	46							
A131E	1	#5	STR.	43' - 0"	45	A233	1	#5	STR.	45' - 8"	48							
A132E	1	#5	STR.	44' - 4"	46	A234	1	#5	STR.	47' - 1"	49							
A133E	1	#5	STR.	45' - 8"	48	A235	1	#5	STR.	48' - 5"	50							
A134E	1	#5	STR.	47' - 1"	49	A236	1	#5	STR.	49' - 9"	52							
A135E	1	#5	STR.	48' - 5"	50	A237	1	#5	STR.	51' - 2"	53							
A136E	1	#5	STR.	49' - 9"	52	A238	1	#5	STR.	52' - 6"	55							
A137E	1	#5	STR.	51' - 2"	53	A239	1	#5	STR.	53' - 10"	56							
A138E	1	#5	STR.	52' - 6"	55	A240	1	#5	STR.	53' - 10"	56							
A139E	1	#5	STR.	53' - 10"	56	A241	1	#5	STR.	52' - 6"	55							
A140E	1	#5	STR.	53' - 10"	56	A242	1	#5	STR.	51' - 1"	53							
A141E	1	#5	STR.	52' - 6"	55	A243	1	#5	STR.	49' - 8"	52							
A142E	1	#5	STR.	51' - 1"	53	A244	1	#5	STR.	48' - 4"	50							
A143E	1	#5	STR.	49' - 8"	52	A245	1	#5	STR.	46' - 11"	49							
A144E	1	#5	STR.	48' - 4"	50	A246	1	#5	STR.	45' - 7"	48							
A145E	1	#5	STR.	46' - 11"	49	A247	1	#5	STR.	44' - 2"	46							
A146E	1	#5	STR.	45' - 7"	48	A248	1	#5	STR.	42' - 10"	45							
A147E	1	#5	STR.	44' - 2"	46	A249	1	#5	STR.	41' - 5"	43							
A148E	1	#5	STR.	42' - 10"	45	A250	1	#5	STR.	40' - 0"	42							
A149E	1	#5	STR.	41' - 5"	43	A251	1	#5	STR.	38' - 8"	40							
A150E	1	#5	STR.	40' - 0"	42	A252	1	#5	STR.	37' - 3"	39							
A151E	1	#5	STR.	38' - 8"	40	A253	1	#5	STR.	35' - 11"	37							
A152E	1	#5	STR.	37' - 3"	39	A254	1	#5	STR.	34' - 6"	36							
A153E	1	#5	STR.	35' - 11"	37	A255	1	#5	STR.	33' - 1"	35							
A154E	1	#5	STR.	34' - 6"	36	A256	1	#5	STR.	31' - 9"	33							
A155E	1	#5	STR.	33' - 1"	35	A257	1	#5	STR.	30' - 4"	32							
A156E	1	#5	STR.	31' - 9"	33	A258	1	#5	STR.	29' - 0"	30							
A157E	1	#5	STR.	30' - 4"	32	A259	1	#5	STR.	27' - 7"	29							
A158E	1	#5	STR.	29' - 0"	30	A260	1	#5	STR.	26' - 3"	27							
A159E	1	#5	STR.	27' - 7"	29	A261	1	#5	STR.	24' - 10"	26							
A160E	1	#5	STR.	26' - 3"	27	A262	1	#5	STR.	23' - 5"	24							
A161E	1	#5	STR.	24' - 10"	26	A263	1	#5	STR.	22' - 1"	23							
A162E	1	#5	STR.	23' - 5"	24	A264	1	#5	STR.	20' - 8"	22							
A163E	1	#5	STR.	22' - 1"	23	A265	1	#5	STR.	19' - 4"	20							
A164E	1	#5	STR.	20' - 8"	22	A266	1	#5	STR.	17' - 11"	19							
A165E	1	#5	STR.	19' - 4"	20	A267	1	#5	STR.	16' - 6"	17							
A166E	1	#5	STR.	17' - 11"	19	A268	1	#5	STR.	15' - 2"	16							
A167E	1	#5	STR.	16' - 6"	17	A269	1	#5	STR.	13' - 9"	14							
A168E	1	#5	STR.	15' - 2"	16	A270	1	#5	STR.	12' - 5"	13							
A169E	1	#5	STR.	13' - 9"	14	A271	1	#5	STR.	11' - 0"	11							
A170E	1	#5	STR.	12' - 5"	13	A272	1	#5	STR.	9' - 7"	10							
A171E	1	#5	STR.	11' - 0"	11	A273	1	#5	STR.	8' - 3"	9							
A172E	1	#5	STR.	9' - 7"	10	A274	1	#5	STR.	6' - 10"	7							
A173E	1	#5	STR.	8' - 3"	9	A275	1	#5	STR.	5' - 6"	6							
A174E	1	#5	STR.	6' - 10"	7	A276	1	#5	STR.	4' - 1"	4							
A175E	1	#5	STR.	5' - 6"	6	A277	1	#5	STR.	2' - 8"	3							
A176E	1	#5	STR.	4' - 1"	4													
A177E	1	#5	STR.	2' - 8"	3													

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT

SUPERSTRUCTURE BILL OF MATERIAL

	CLASS AA CONCRETE (CU. YDS.)	REINFORCING STEEL (LBS.)	EPOXY COATED REINFORCING STEEL (LBS.)
SPAN A		29,605	27,264
POUR 1	218.3		
POUR 2	93.1		
TOTALS *	311.4	29,605	27,264

QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

GROOVING BRIDGE FLOORS

APPROACH SLABS	2,364	SQ.FT.
BRIDGE DECK	6,565	SQ.FT.
TOTAL	8,929	SQ.FT.

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SHEET 2 OF 2



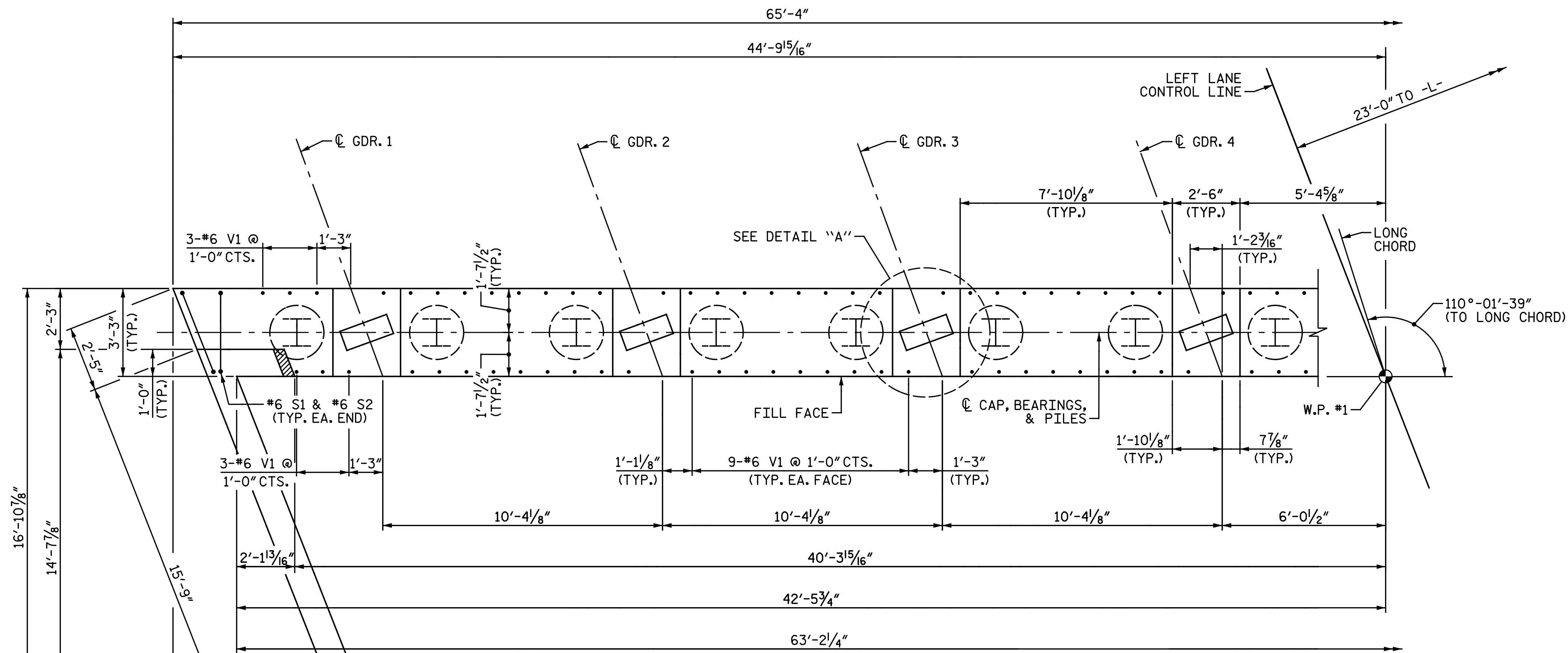
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 RALEIGH
 SUPERSTRUCTURE
 BILL OF MATERIAL

LEFT LANE

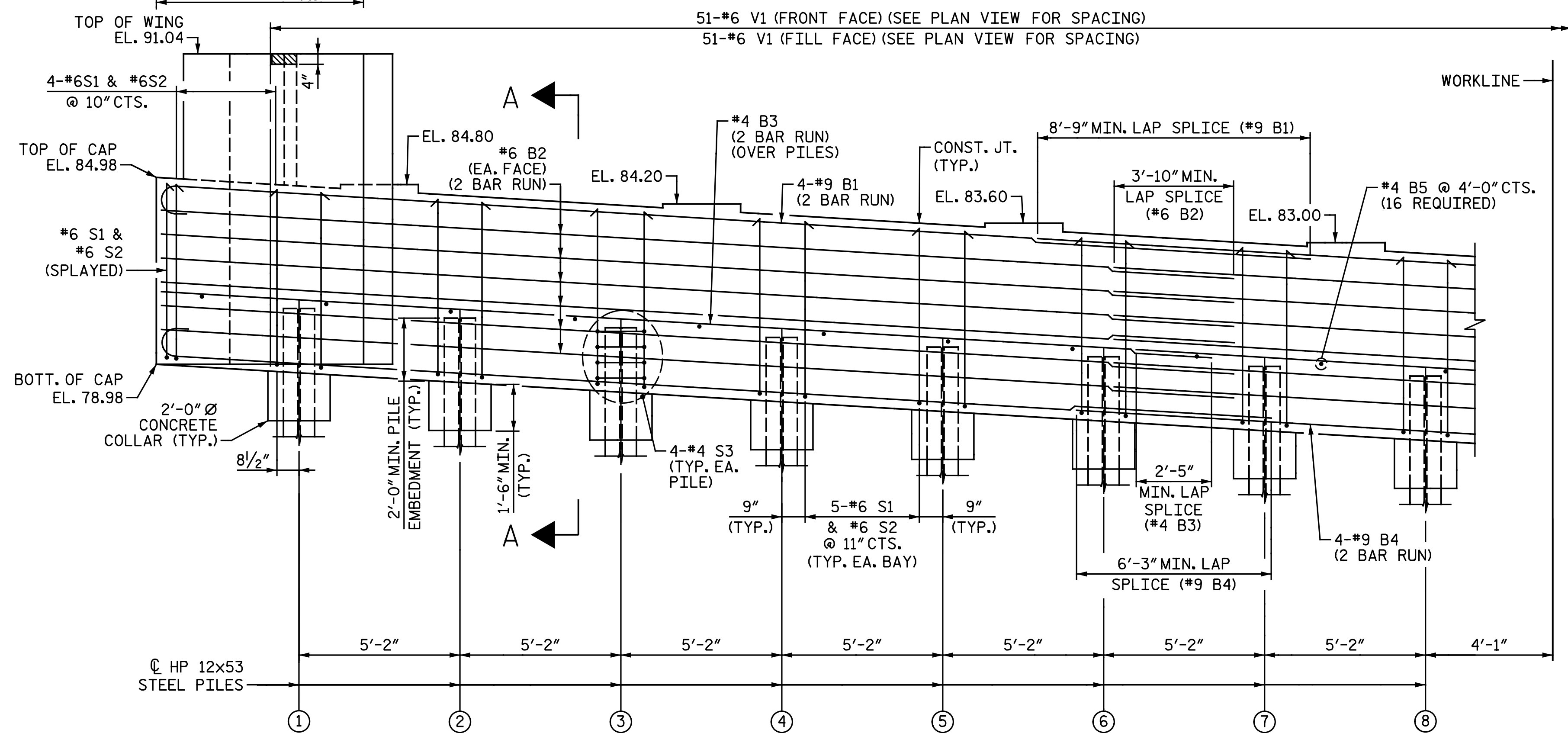
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1			3			34
2			4			

DRAWN BY : C. E. MAYHEW DATE : 7-14-17
 CHECKED BY : D. A. COLETTI DATE : 7-19-17



PARTIAL PLAN



PARTIAL ELEVATION

NOTES:

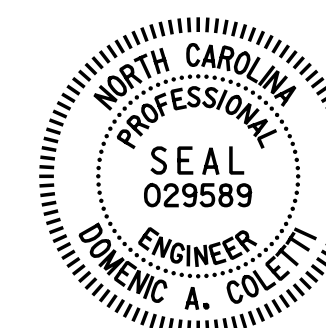
FOR NOTES, SEE SHEET 2 OF 4.

FOR "DETAIL A", SEE SHEET 2 OF 4.

TOP OF PILE ELEVATIONS	
PILE	ELEVATION
①	80.75
②	80.45
③	80.15
④	79.85
⑤	79.55
⑥	79.25
⑦	78.95
⑧	78.65
⑨	78.35
⑩	78.05
⑪	77.75
⑫	77.45

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 1 OF 4

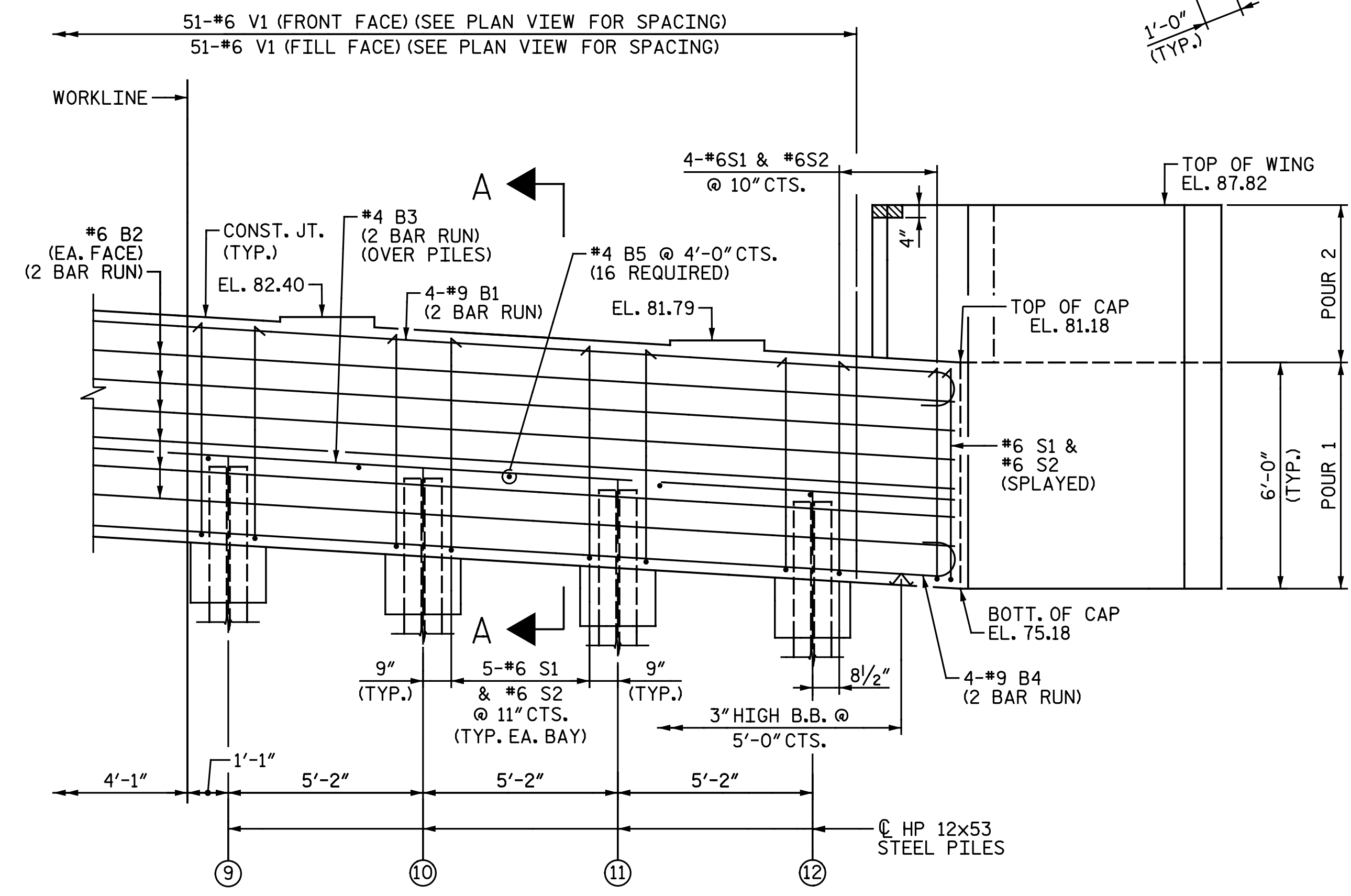
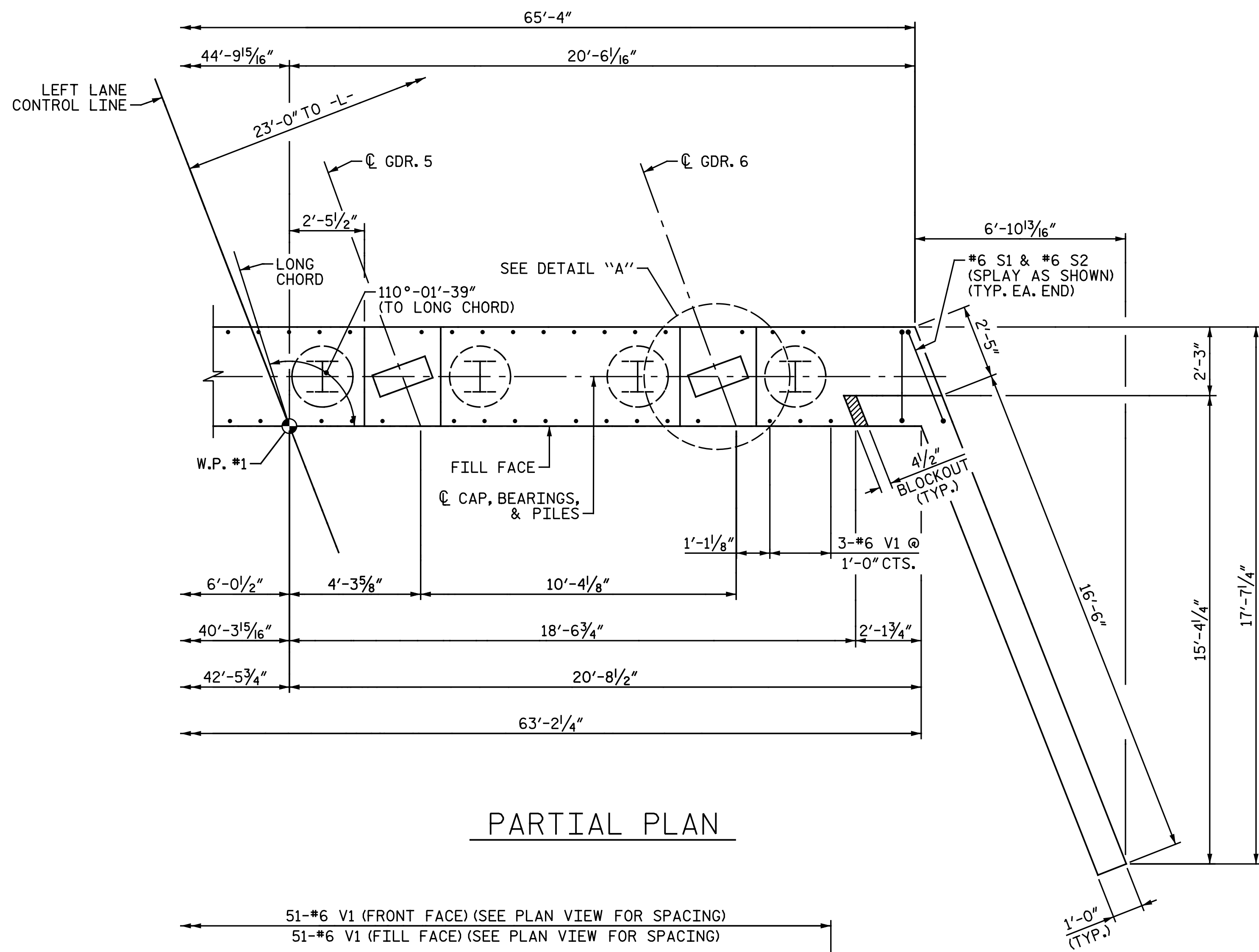


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 1
 LEFT LANE

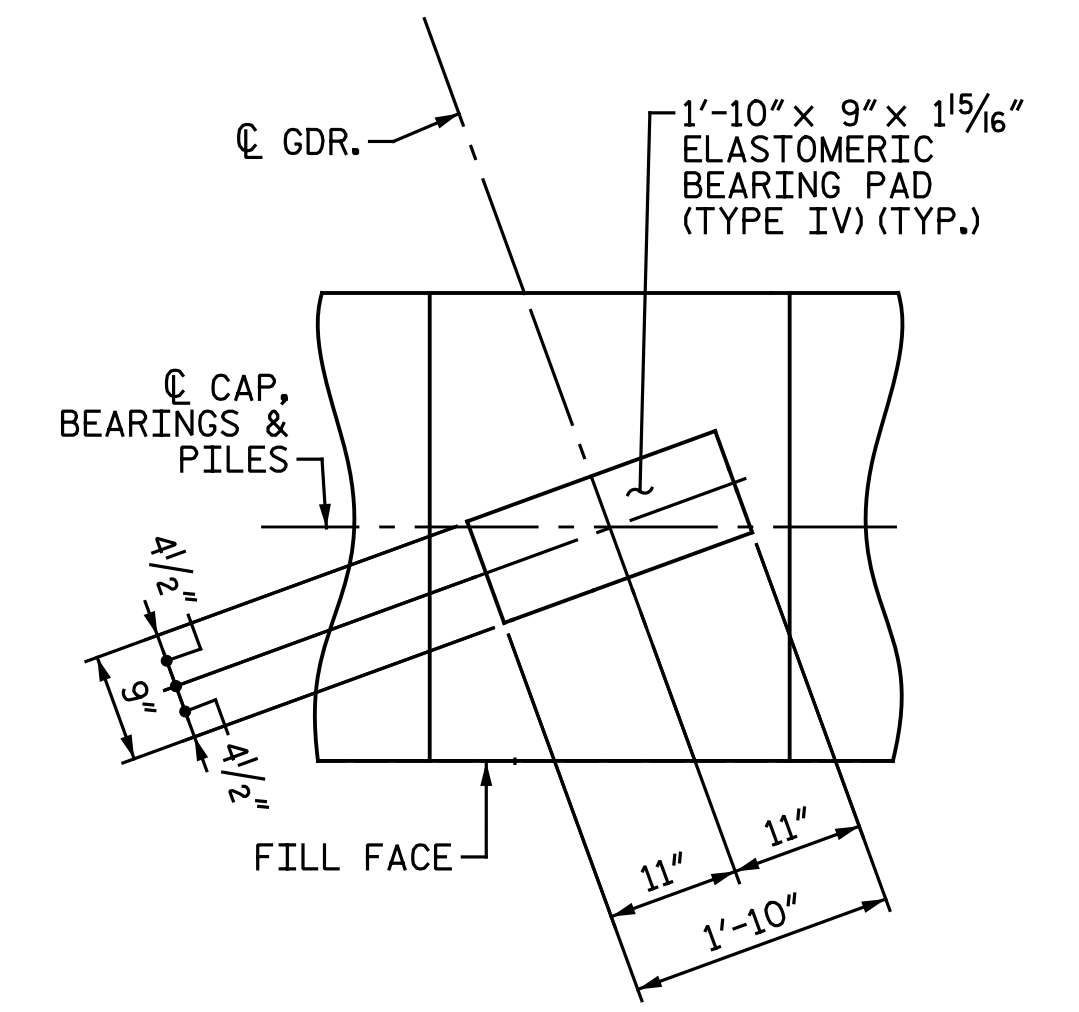
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NO.	BY:	DATE:	NO.	BY:	DATE:	S3-21
1			3			TOTAL SHEETS
2			4			34

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 CHECKED BY: D. A. COLETTI DATE: 7-14-17



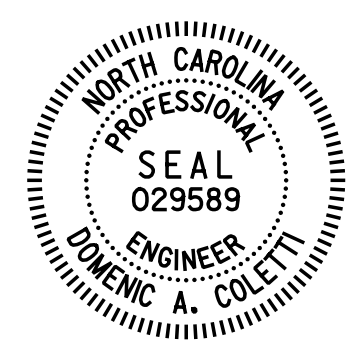
NOTES:
 FOR "SECTION A-A", SEE "INTEGRAL END BENT 1 DETAILS" SHEET.
 FOR "TOP OF PILE ELEVATIONS", SEE SHEET 1 OF 4.
 STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #4 V1 BARS.
 THE TOP SURFACE OF THE END BENT CAP, EXCLUDING THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".
 THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.



DETAIL "A"
 ALL DIMENSIONS AND DETAILS SHOWN ARE TYPICAL FOR ALL BEARINGS AT EACH BRIDGE SEAT LOCATION.

PROJECT NO. R-5703
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 STATION: 166+72.51 -L-

SHEET 2 OF 4



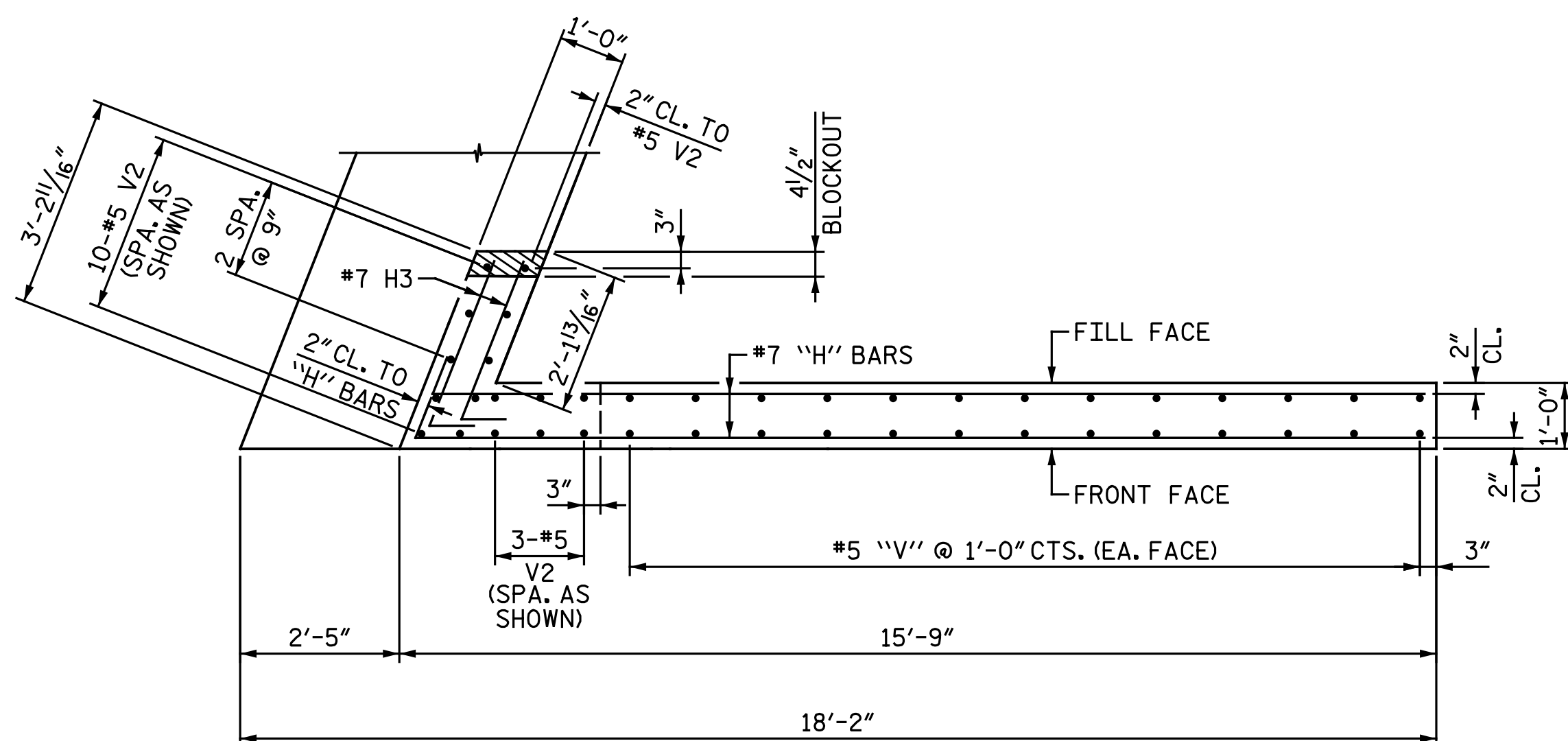
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 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 1
 LEFT LANE

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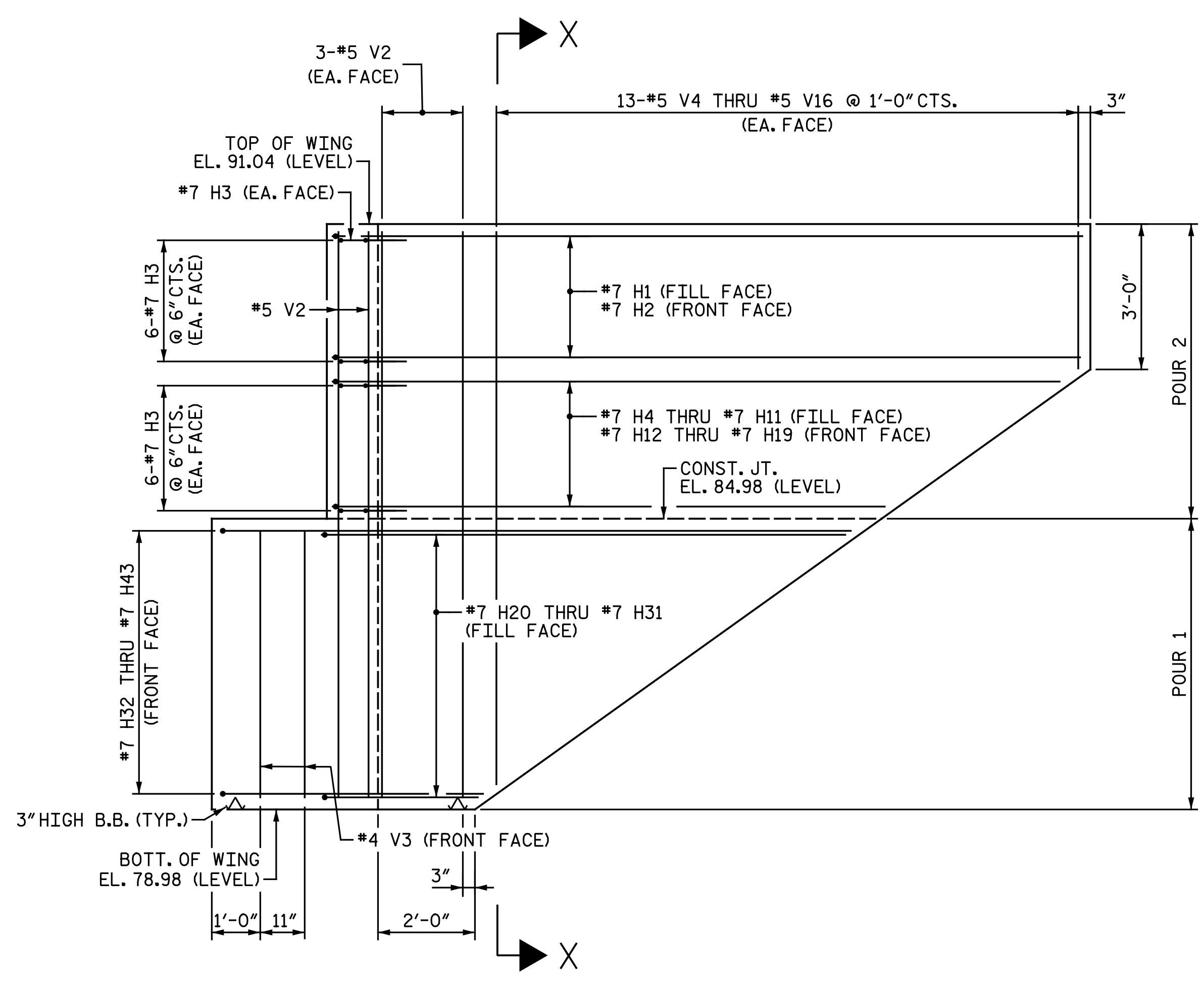
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1			3			TOTAL SHEETS
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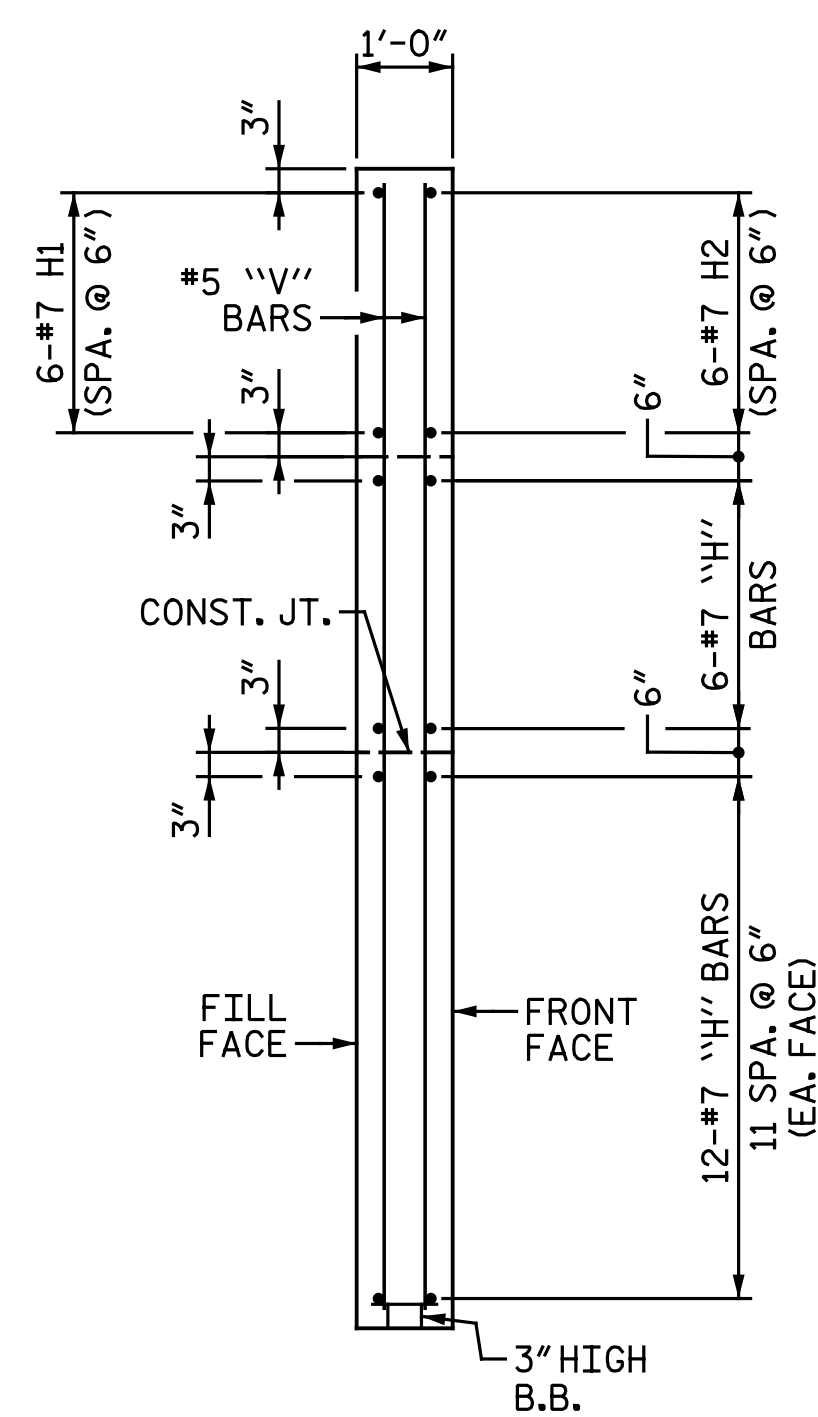
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 CHECKED BY: D. A. COLETTI DATE: 7-14-17



PLAN OF LEFT WING



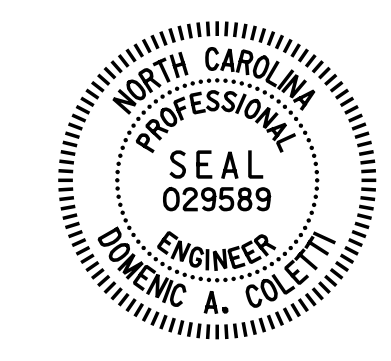
ELEVATION OF LEFT WING



SECTION X-X

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 3 OF 4



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 1

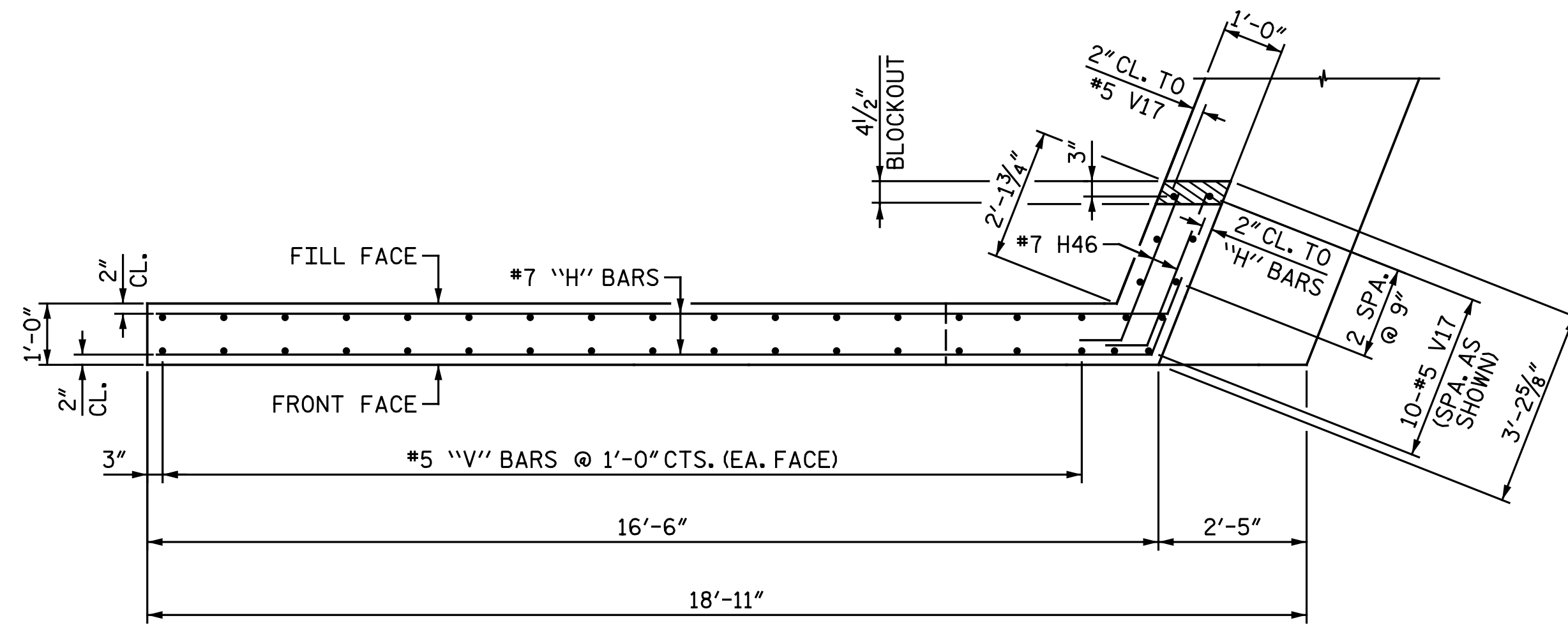
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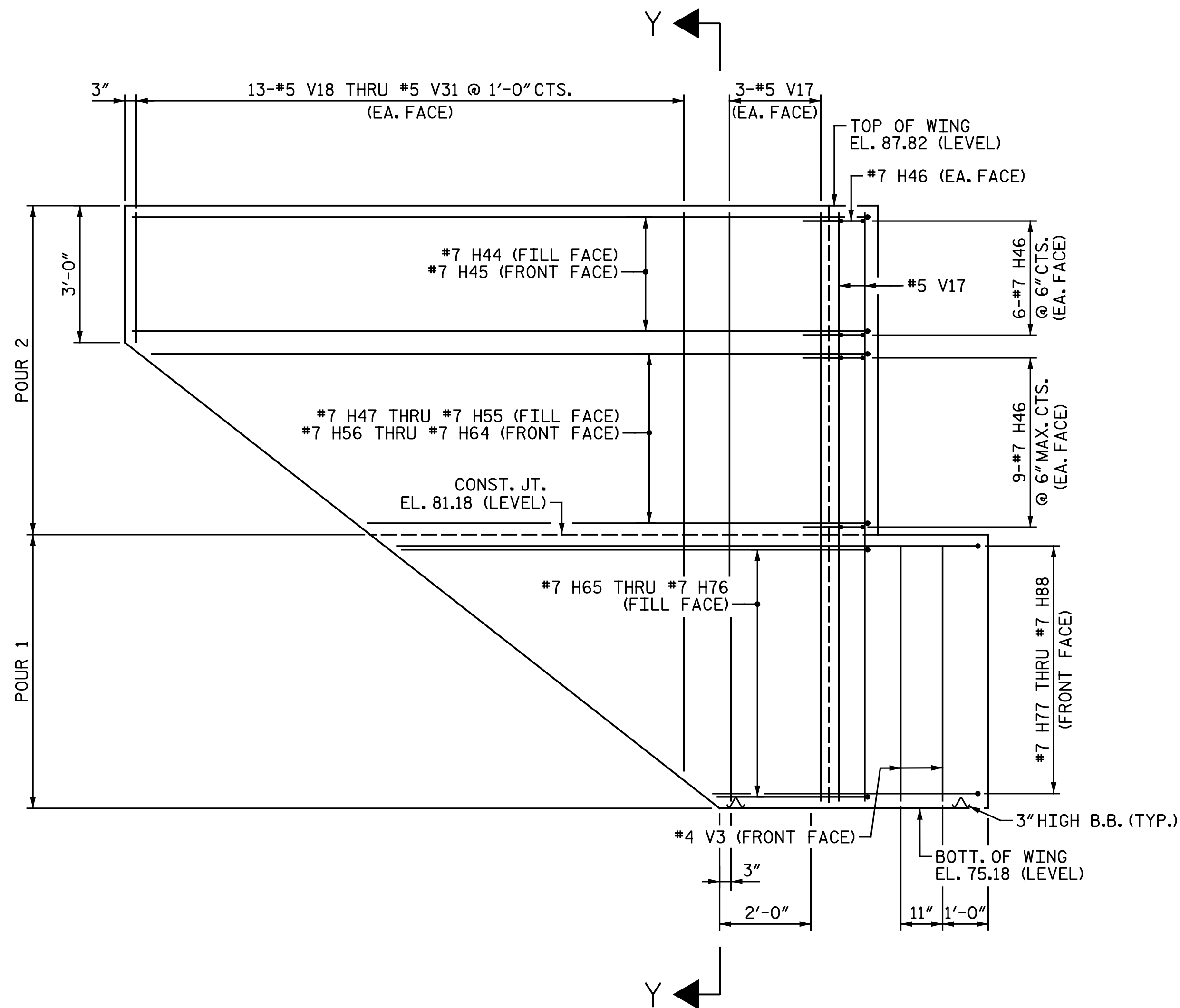
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REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO.	S3-23
TOTAL SHEETS	34

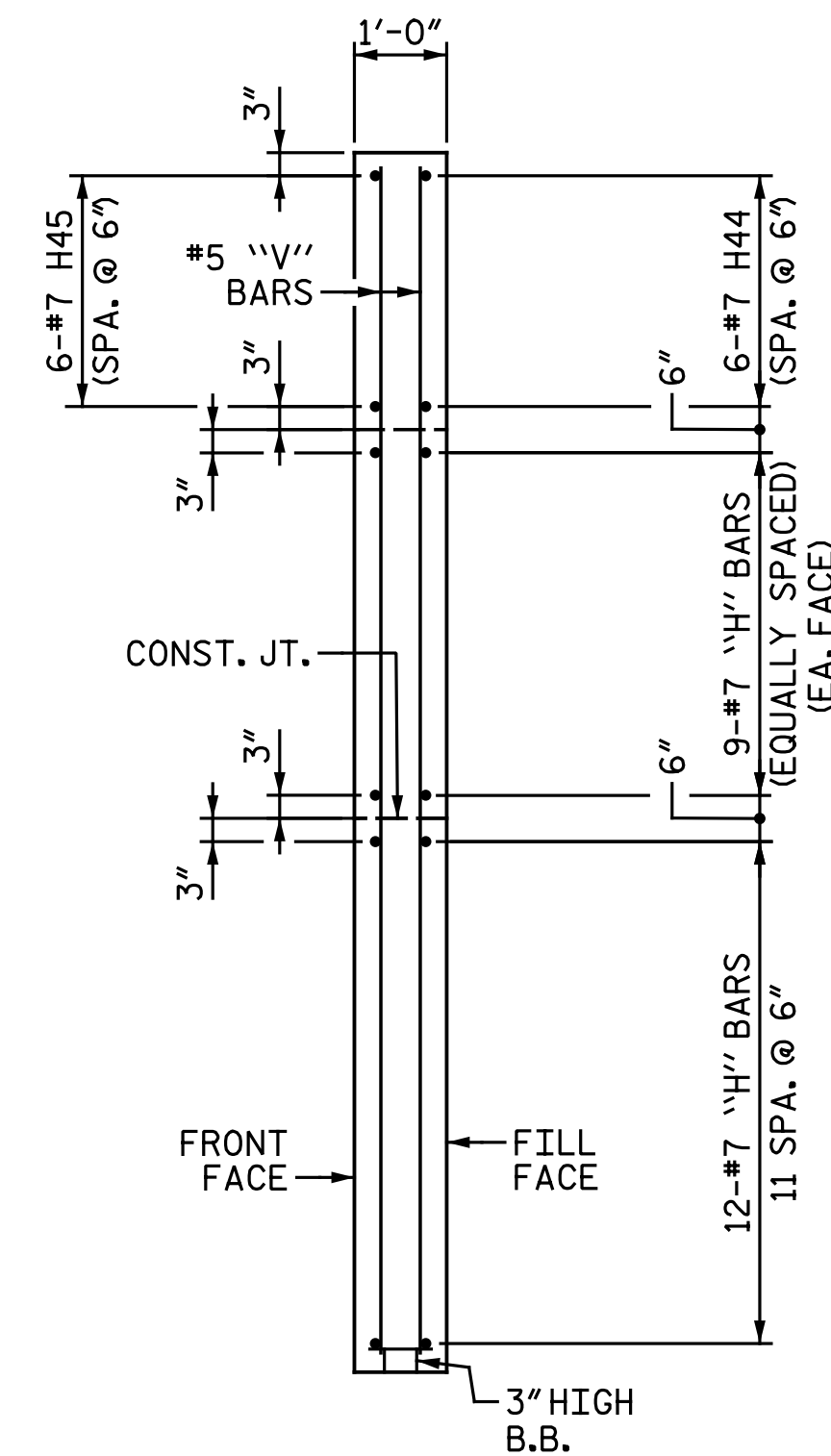
DRAWN BY : C. E. MAYHEW DATE : 7-14-17
 CHECKED BY : D. A. COLETTI DATE : 7-14-17



PLAN OF RIGHT WING



ELEVATION OF RIGHT WING



SECTION Y-Y

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 4 OF 4



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 1

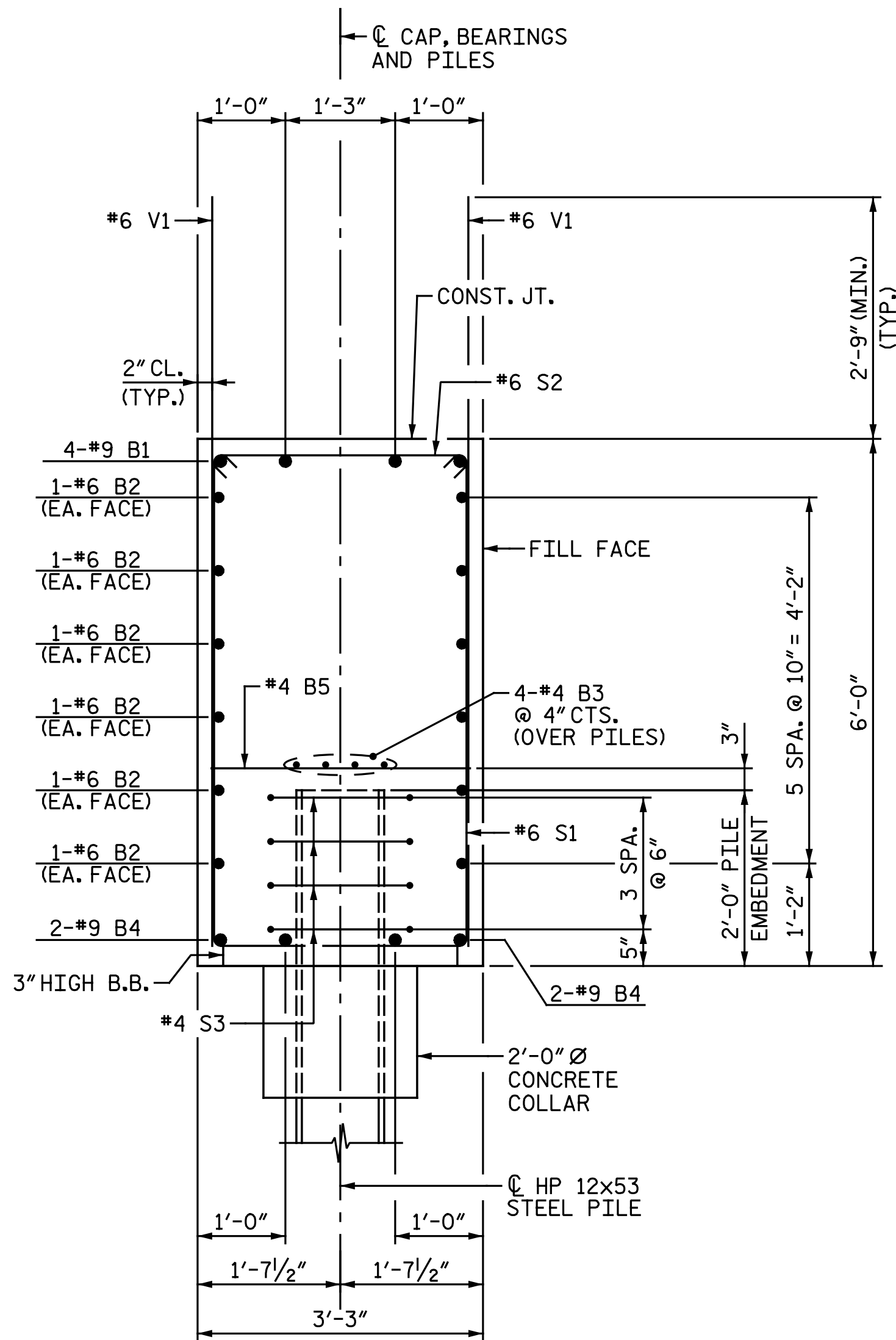
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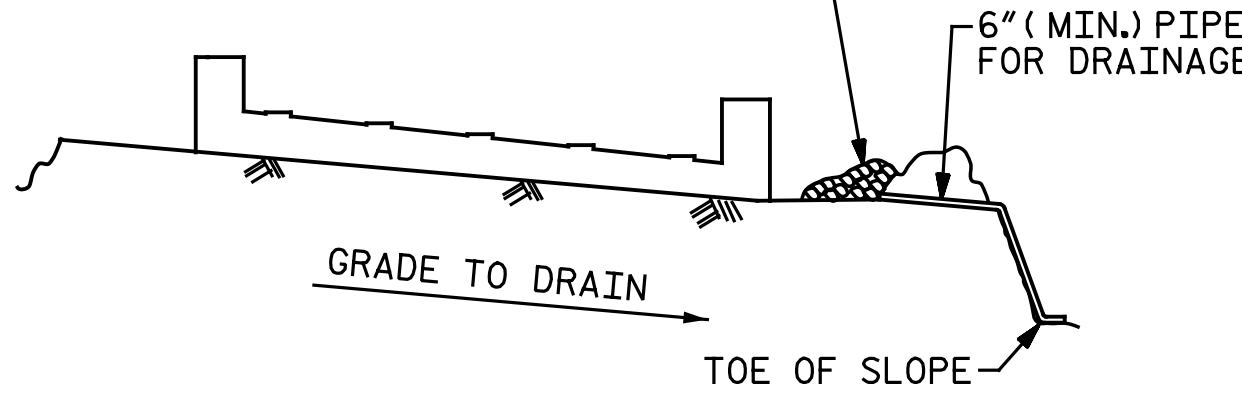
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			34
2			4			

DRAWN BY : C. E. MAYHEW DATE : 7-14-17
 CHECKED BY : D. A. COLETTI DATE : 7-14-17



SECTION A-A

MINIMUM OF 3- ONE CUBIC FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED.



BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS.

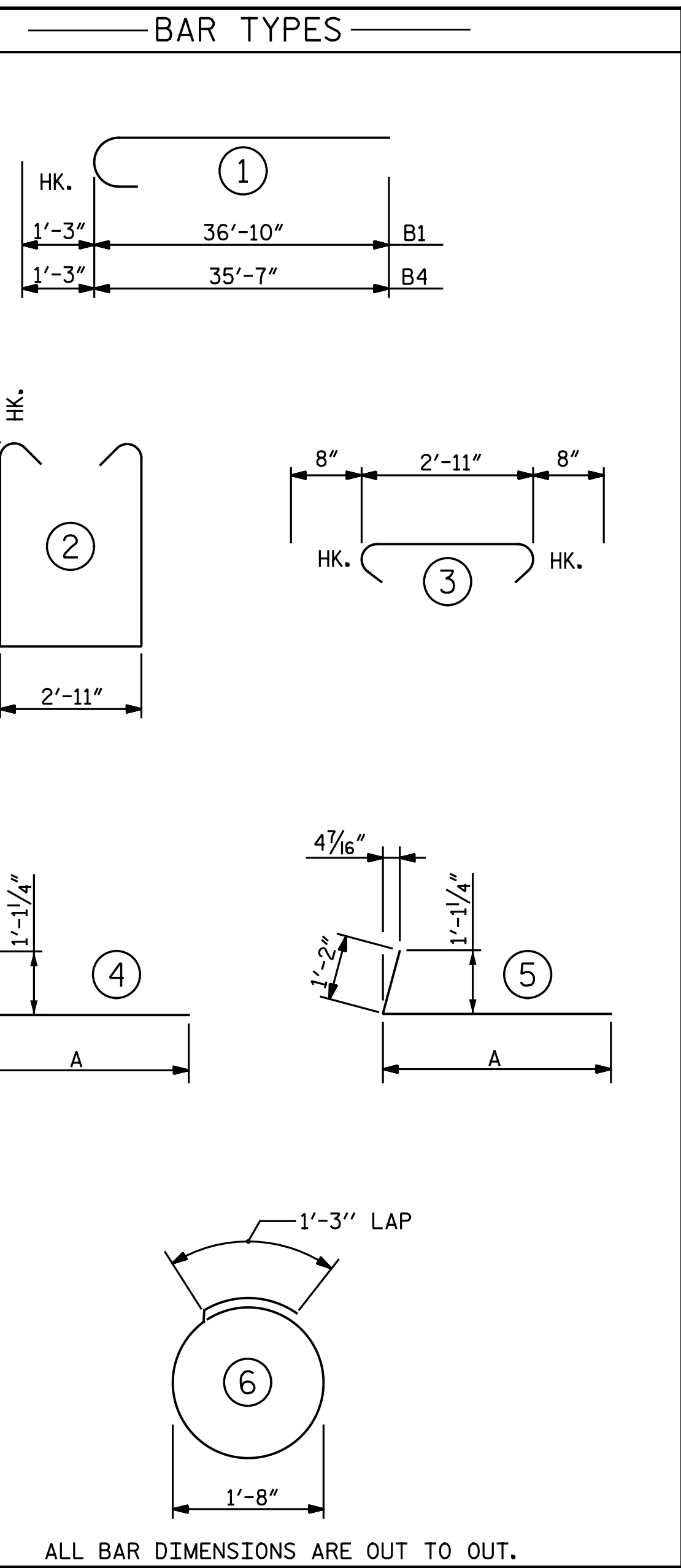
TEMPORARY DRAINAGE AT END BENT

BAR	A	BAR	A
H1	15' - 8"	H41	16' - 1"
H2	15' - 5"	H42	2' - 6"
H3	2' - 6"	H43	15' - 4"
H4	15' - 2"	H44	14' - 7"
H5	14' - 5"	H45	13' - 10"
H6	13' - 8"	H46	13' - 1"
H7	12' - 12"	H47	12' - 4"
H8	12' - 3"	H48	11' - 7"
H9	11' - 7"	H49	10' - 11"
H10	14' - 11"	H50	15' - 6"
H11	14' - 2"	H51	14' - 10"
H12	13' - 5"	H52	14' - 1"
H13	12' - 9"	H53	13' - 4"
H14	11' - 12"	H54	12' - 7"
H15	11' - 4"	H55	11' - 10"
H16	10' - 9"	H56	11' - 2"
H17	10' - 1"	H57	10' - 3"
H18	9' - 5"	H58	9' - 6"
H19	8' - 8"	H59	8' - 10"
H20	7' - 12"	H60	8' - 1"
H21	7' - 4"	H61	7' - 5"
H22	6' - 7"	H62	6' - 9"
H23	5' - 11"	H63	5' - 12"
H24	5' - 3"	H64	5' - 4"
H25	4' - 6"	H65	4' - 8"
H26	3' - 10"	H66	3' - 11"
H27	3' - 2"	H67	3' - 3"
H28	12' - 11"	H68	2' - 7"
H29	12' - 2"	H69	12' - 9"
H30	11' - 6"	H70	12' - 1"
H31	10' - 9"	H71	11' - 5"
H32	10' - 1"	H72	10' - 8"
H33	9' - 5"	H73	9' - 12"
H34	8' - 8"	H74	9' - 4"
H35	7' - 12"	H75	8' - 7"
H36	7' - 4"	H76	7' - 11"
H37	6' - 7"	H77	7' - 3"
H38	5' - 11"	H78	6' - 6"
H39	5' - 3"	H79	5' - 10"
H40	15' - 10"	H80	5' - 2"

NOTE: FOR PILE SPLICE DETAILS, SEE "INTEGRAL END BENT 2 DETAILS" SHEET.

BILL OF MATERIAL					BILL OF MATERIAL						
INTEGRAL END BENT 1					INTEGRAL END BENT 1						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#9		38' - 1"	1,036	H64	1	#7	4	6' - 6"	13
B2	24	#6	STR.	34' - 5"	1,241	H65	1	#7	4	5' - 10"	12
B3	8	#4	STR.	33' - 9"	180	H66	1	#7	4	5' - 1"	10
B4	8	#9		36' - 10"	1,002	H67	1	#7	4	4' - 5"	9
B5	16	#4	STR.	2' - 11"	31	H68	1	#7	4	3' - 9"	8
						H69	1	#7	4	13' - 11"	28
H1	6	#7		16' - 9"	206	H70	1	#7	4	13' - 3"	27
H2	6	#7		16' - 7"	203	H71	1	#7	4	12' - 7"	26
H3	24	#7		3' - 8"	180	H72	1	#7	4	11' - 10"	24
H4	1	#7		5' - 16' - 4"	33	H73	1	#7	4	11' - 2"	23
H5	1	#7		5' - 15' - 7"	32	H74	1	#7	4	10' - 6"	21
H6	1	#7		5' - 14' - 10"	30	H75	1	#7	4	9' - 9"	20
H7	1	#7		5' - 14' - 2"	29	H76	1	#7	4	9' - 1"	19
H8	1	#7		5' - 13' - 5"	27	H77	1	#7	4	8' - 5"	17
H9	1	#7		5' - 12' - 9"	26	H78	1	#7	4	7' - 8"	16
H10	1	#7		5' - 16' - 1"	33	H79	1	#7	4	7' - 0"	14
H11	1	#7		5' - 15' - 4"	31	H80	1	#7	4	6' - 4"	13
H12	1	#7		5' - 14' - 7"	30						
H13	1	#7		5' - 13' - 11"	28	S1	65	#6	2	15' - 6"	1,513
H14	1	#7		5' - 13' - 2"	27	S2	65	#6	3	4' - 3"	415
H15	1	#7		5' - 12' - 6"	26	S3	48	#4	6	6' - 6"	208
H16	1	#7		5' - 11' - 11"	24						
H17	1	#7		5' - 11' - 3"	23	V1	102	#6	STR.	8' - 6"	1,302
H18	1	#7		5' - 10' - 7"	22	V2	16	#5	STR.	11' - 7"	193
H19	1	#7		5' - 9' - 10"	20	V3	4	#4	STR.	5' - 7"	15
H20	1	#7		5' - 9' - 2"	19	V4	2	#5	STR.	11' - 3"	23
H21	1	#7		5' - 8' - 6"	17	V5	2	#5	STR.	10' - 7"	22
H22	1	#7		5' - 7' - 9"	16	V6	2	#5	STR.	9' - 10"	21
H23	1	#7		5' - 7' - 1"	14	V7	2	#5	STR.	9' - 2"	19
H24	1	#7		5' - 6' - 5"	13	V8	2	#5	STR.	8' - 5"	18
H25	1	#7		5' - 5' - 8"	12	V9	2	#5	STR.	7' - 9"	16
H26	1	#7		5' - 5' - 0"	10	V10	2	#5	STR.	7' - 0"	15
H27	1	#7		5' - 4' - 4"	9	V11	2	#5	STR.	6' - 3"	13
H28	1	#7		5' - 14' - 1"	29	V12	2	#5	STR.	5' - 7"	12
H29	1	#7		5' - 13' - 4"	27	V13	2	#5	STR.	4' - 10"	10
H30	1	#7		5' - 12' - 8"	26	V14	2	#5	STR.	4' - 2"	9
H31	1	#7		5' - 11' - 11"	24	V15	2	#5	STR.	3' - 5"	7
H32	1	#7		5' - 11' - 3"	23	V16	2	#5	STR.	2' - 8"	6
H33	1	#7		5' - 10' - 7"	22	V17	16	#5	STR.	12' - 3"	204
H34	1	#7		5' - 9' - 10"	20	V18	2	#5	STR.	12' - 1"	25
H35	1	#7		5' - 9' - 2"	19	V19	2	#5	STR.	11' - 4"	24
H36	1	#7		5' - 8' - 6"	17	V20	2	#5	STR.	10' - 7"	22
H37	1	#7		5' - 7' - 9"	16	V21	2	#5	STR.	9' - 11"	21
H38	1	#7		5' - 7' - 1"	14	V22	2	#5	STR.	9' - 2"	19
H39	1	#7		5' - 6' - 5"	13	V23	2	#5	STR.	8' - 5"	18
H40	6	#7		5' - 17' - 0"	208	V24	2	#5	STR.	7' - 9"	16
H41	6	#7		5' - 17' - 3"	212	V25	2	#5	STR.	7' - 0"	15
H42	26	#7		5' - 3' - 8"	195	V26	2	#5	STR.	6' - 4"	13
H43	1	#7		5' - 16' - 6"	34	V27	2	#5	STR.	5' - 7"	12
H44	1	#7		4' - 15' - 9"	32	V28	2	#5	STR.	4' - 10"	10
H45	1	#7		4' - 15' - 0"	31	V29	2	#5	STR.	4' - 2"	9
H46	1	#7		4' - 14' - 3"	29	V30	2	#5	STR.	3' - 5"	7
H47	1	#7		4' - 13' - 6"	28	V31	2	#4	STR.	2' - 8"	4
H48	1	#7		4' - 12' - 9"	26						
H49	1	#7		4' - 12' - 1"	25						
H50	1	#7		4' - 16' - 8"	34						
H51	1	#7		4' - 16' - 0"	33						
H52	1	#7		4' - 15' - 3"	31						
H53	1	#7		4' - 14' - 6"	30						
H54	1	#7		4' - 13' - 9"	28						
H55	1	#7		4' - 13' - 0"	27						
H56	1	#7		4' - 12' - 4"	25						
H57	1	#7		4' - 11' - 5"	23						
H58	1	#7		4' - 10' - 8"	22						
H59	1	#7		4' - 10' - 0"	20						
H60	1	#7		4' - 9' - 3"	19						
H61	1	#7		4' - 8' - 7"	18						
H62	1	#7		4' - 7' - 11"	16						
H63	1	#7		4' - 7' - 2"	15						

REINFORCING STEEL	LBS.	10,597
CLASS A CONCRETE		
POUR 1 - CAP, LOWER PART OF WINGS & COLLARS	C.Y.	52.3
POUR 2 - UPPER PART OF WINGS	C.Y.	8.1
TOTAL	C.Y.	60.4
PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES	EA.	12
HP 12x53 STEEL PILES		
NO. 12	L.F.	1,020
PILE REDRIVES	EA.	6



ALL BAR DIMENSIONS ARE OUT TO OUT.

PROJECT NO. R-5703
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RALEIGH
SUBSTRUCTURE
INTEGRAL END BENT 1
DETAILS

LEFT LANE

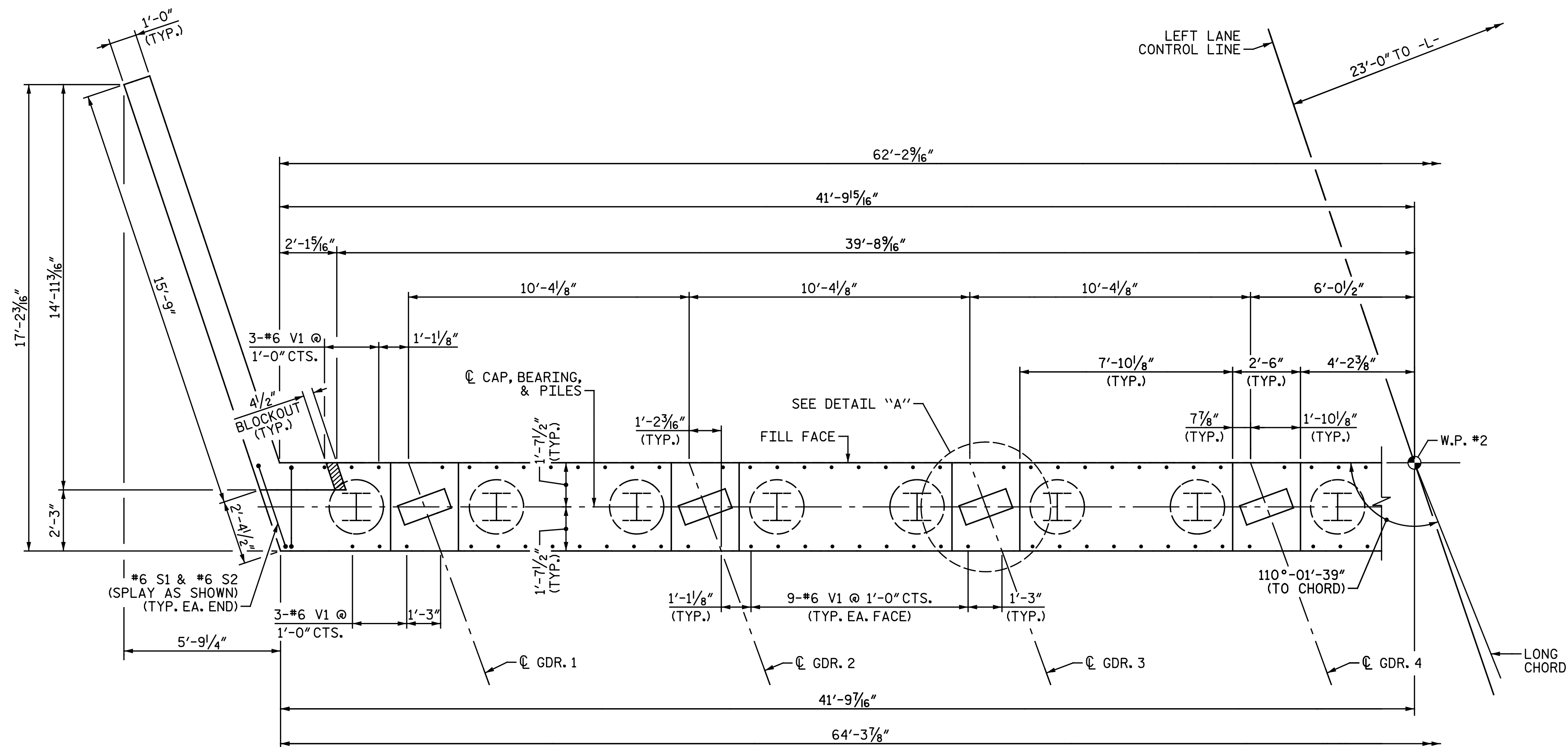
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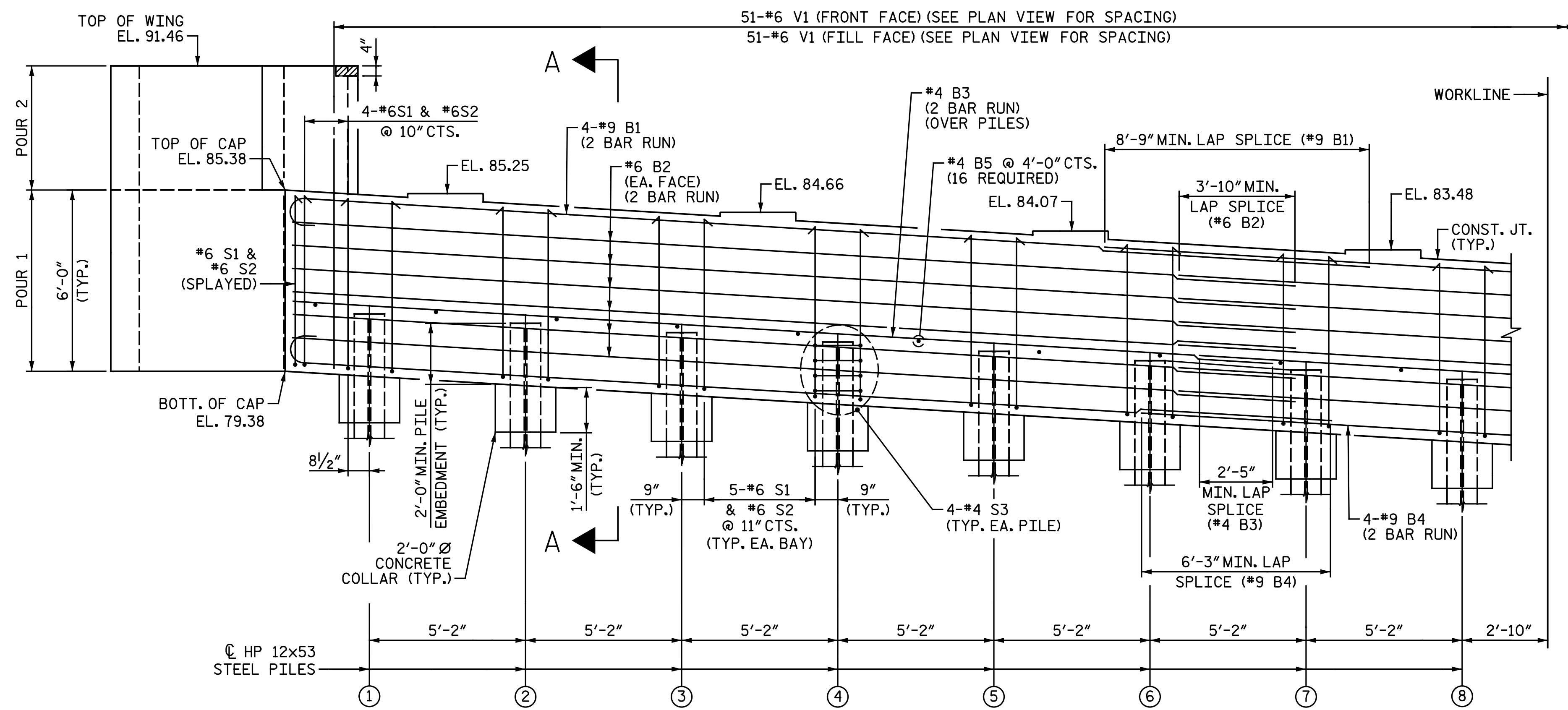
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NO.	DATE	NO.	DATE
1		3	
2		4	

TOTAL SHEETS: 34

DRAWN BY: CEM / MDM DATE: 7-13-17
CHECKED BY: D. A. COLETTI DATE: 7-14-17



PARTIAL PLAN



PARTIAL ELEVATION

NOTES:
 FOR NOTES, SEE, SHEET 2 OF 4.
 FOR "DETAIL A", SEE SHEET 2 OF 4.

TOP OF PILE ELEVATIONS	
PILE	ELEVATION
①	81.19
②	80.90
③	80.60
④	80.31
⑤	80.01
⑥	79.72
⑦	79.42
⑧	79.13
⑨	78.83
⑩	78.54
⑪	78.24
⑫	77.95

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 1 OF 4

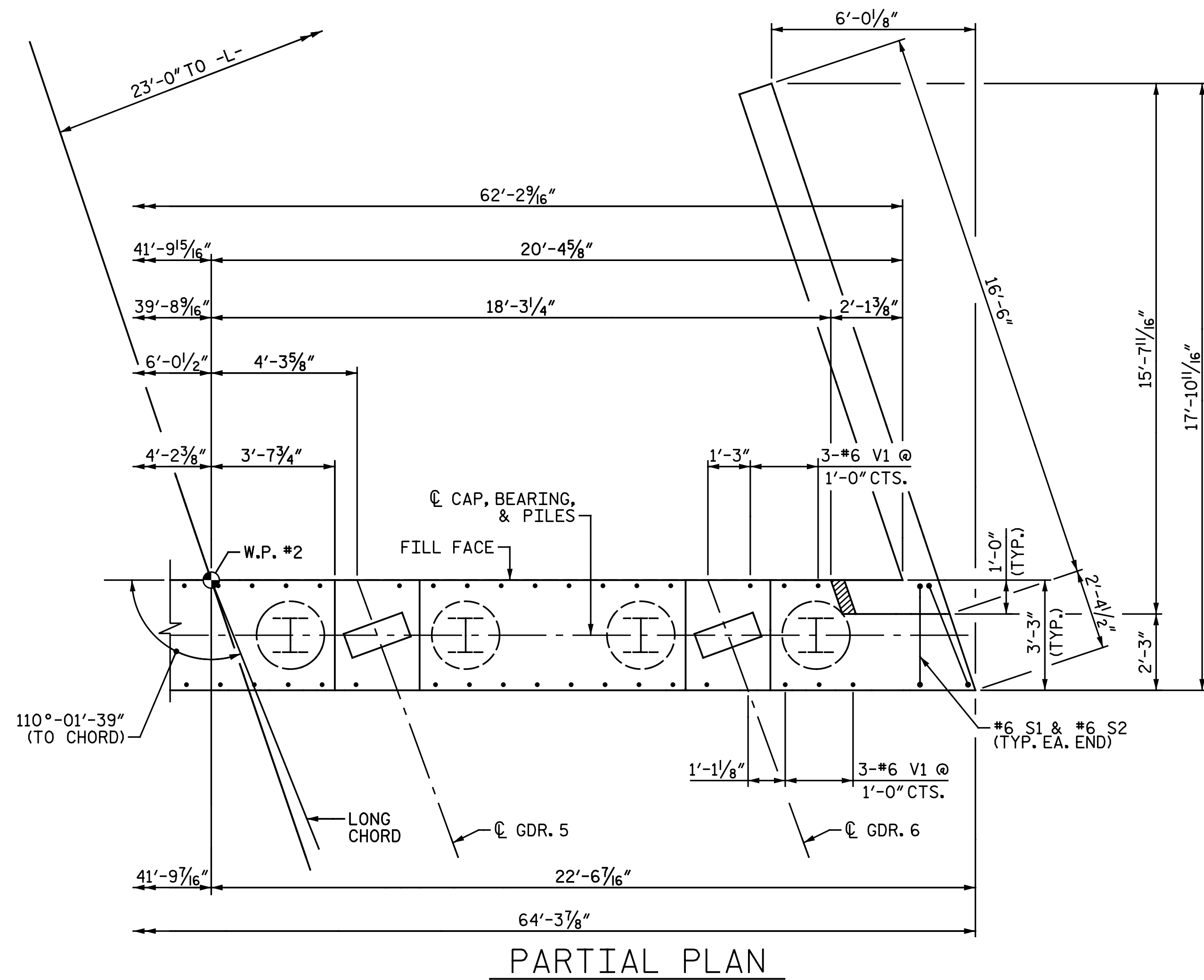


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 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 2
 LEFT LANE

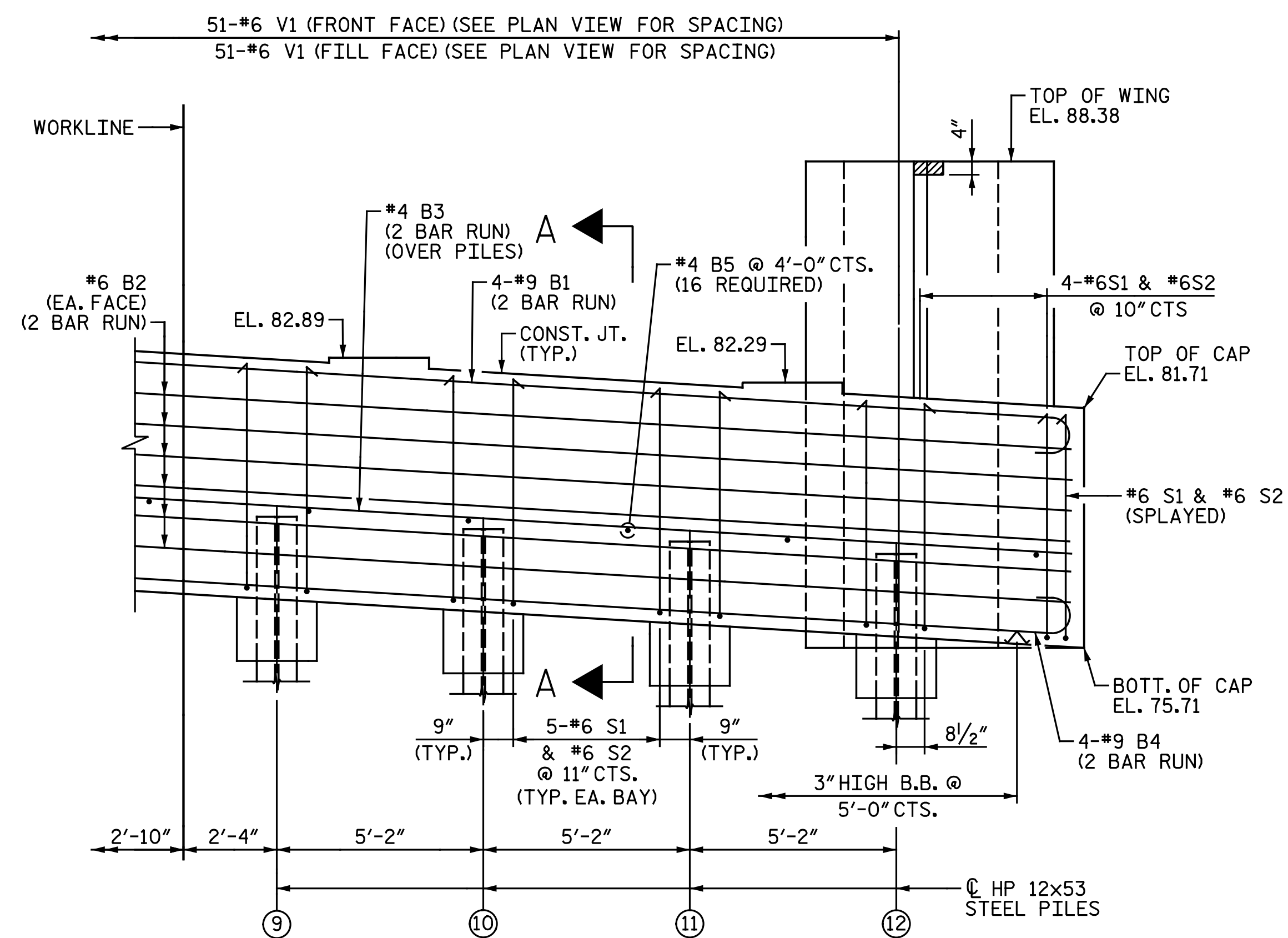
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-26
1			3			TOTAL SHEETS
2			4			34

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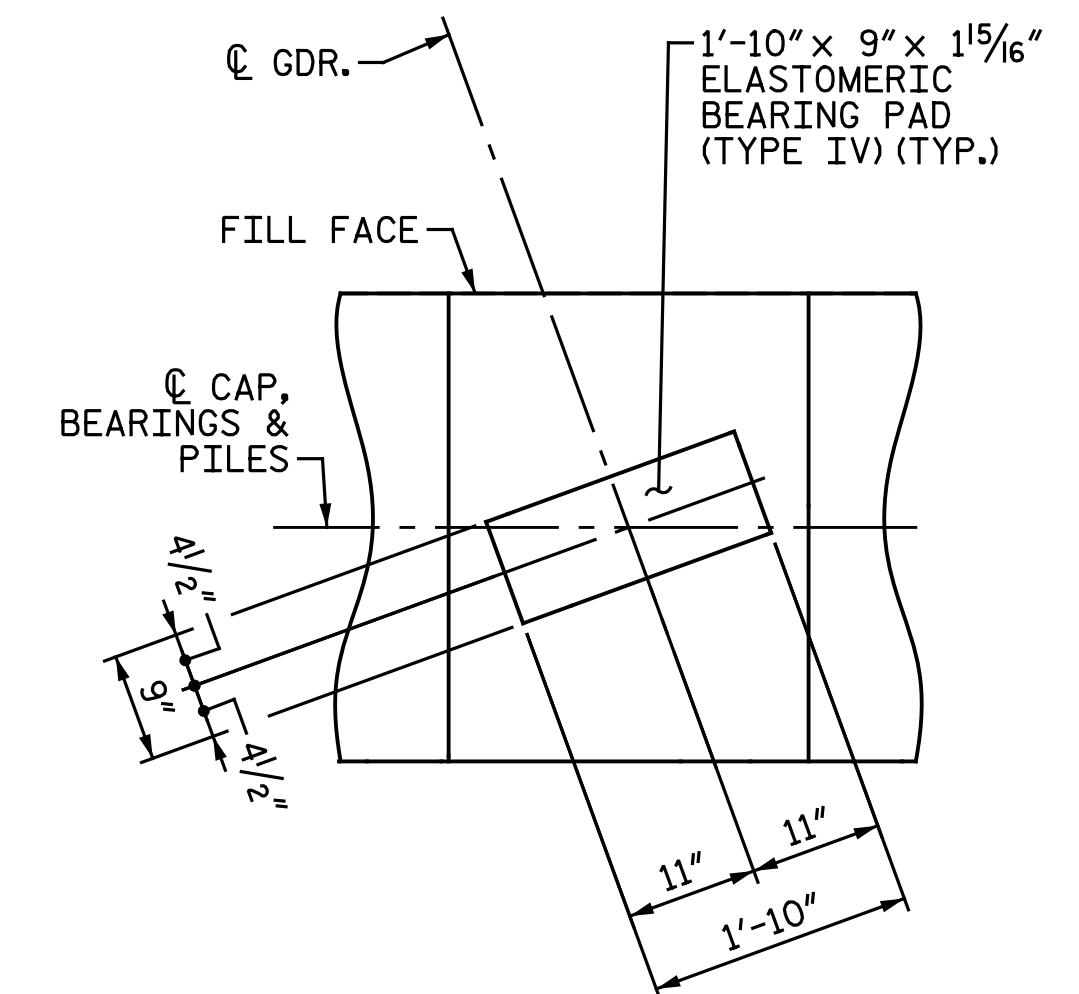
PARTIAL PLAN



PARTIAL ELEVATION

NOTES:

- FOR "SECTION A-A", SEE "INTEGRAL END BENT 2 DETAILS" SHEET.
- FOR "TOP OF PILE ELEVATIONS", SEE SHEET 1 OF 4.
- STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #6 V1 BARS.
- THE TOP SURFACE OF THE END BENT CAP, EXCLUDING THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4\".
- THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.



DETAIL "A"

ALL DIMENSIONS AND DETAILS SHOWN ARE TYPICAL FOR ALL BEARINGS AT EACH BRIDGE SEAT LOCATION.

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 2 OF 4



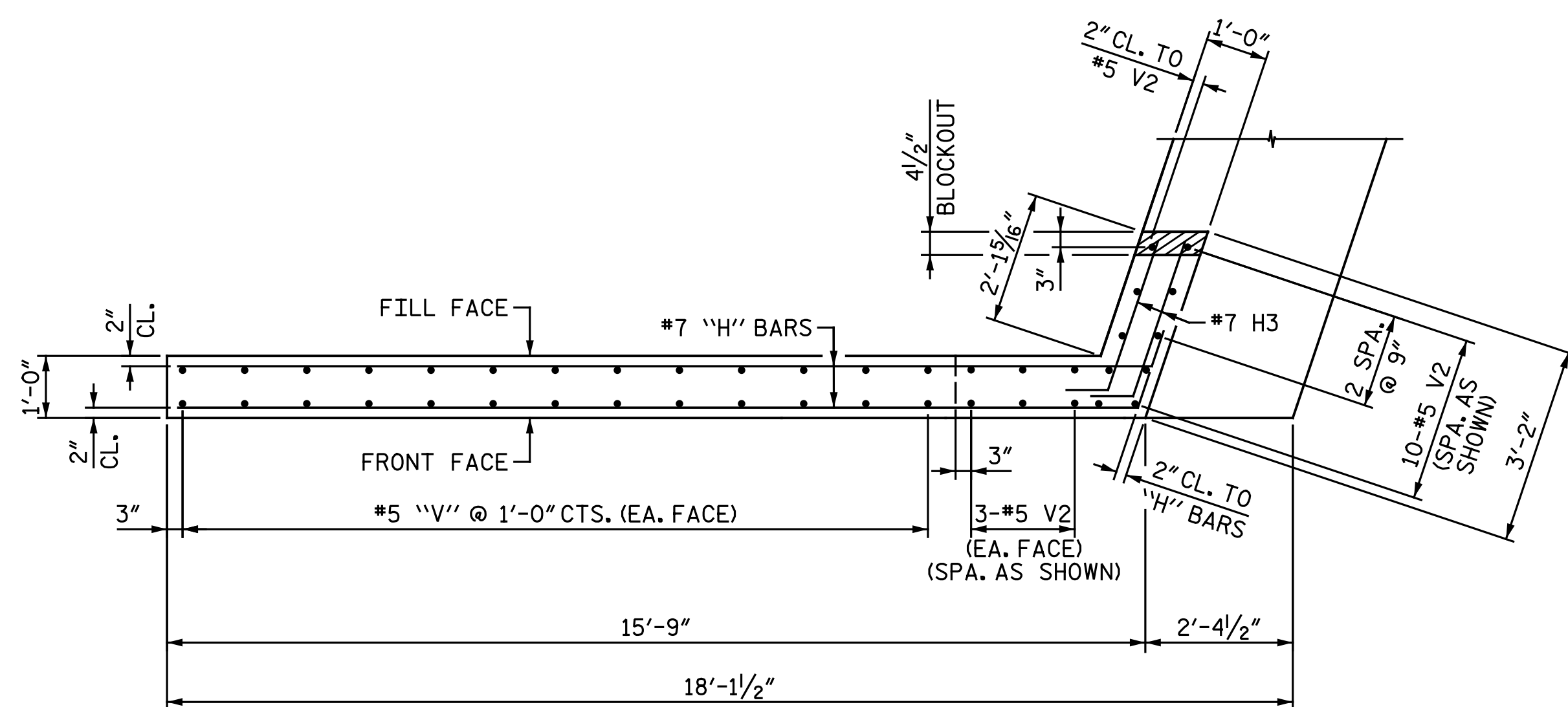
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 2

LEFT LANE

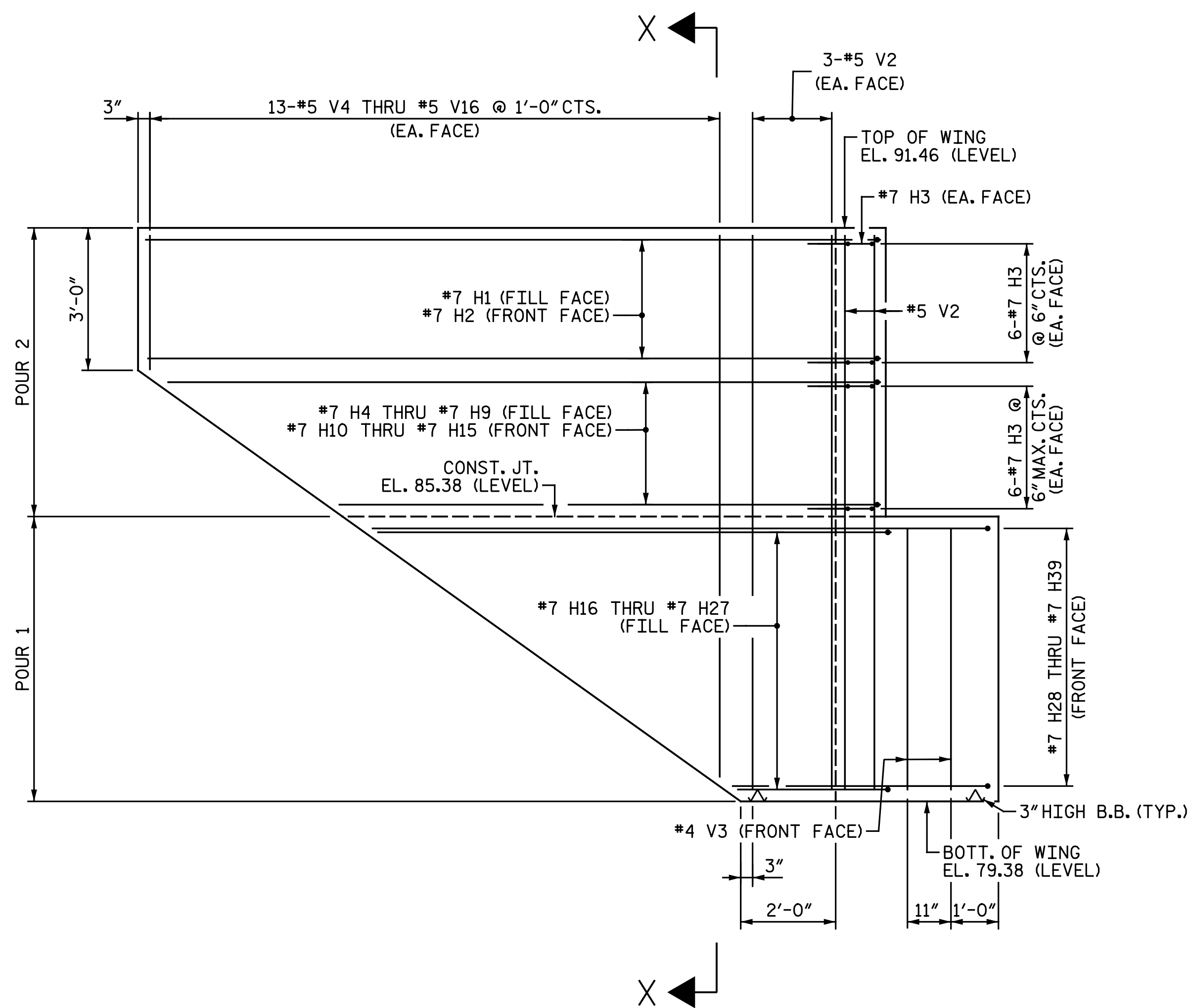
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REVISIONS						SHEET NO. S3-27
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 34
2			4			

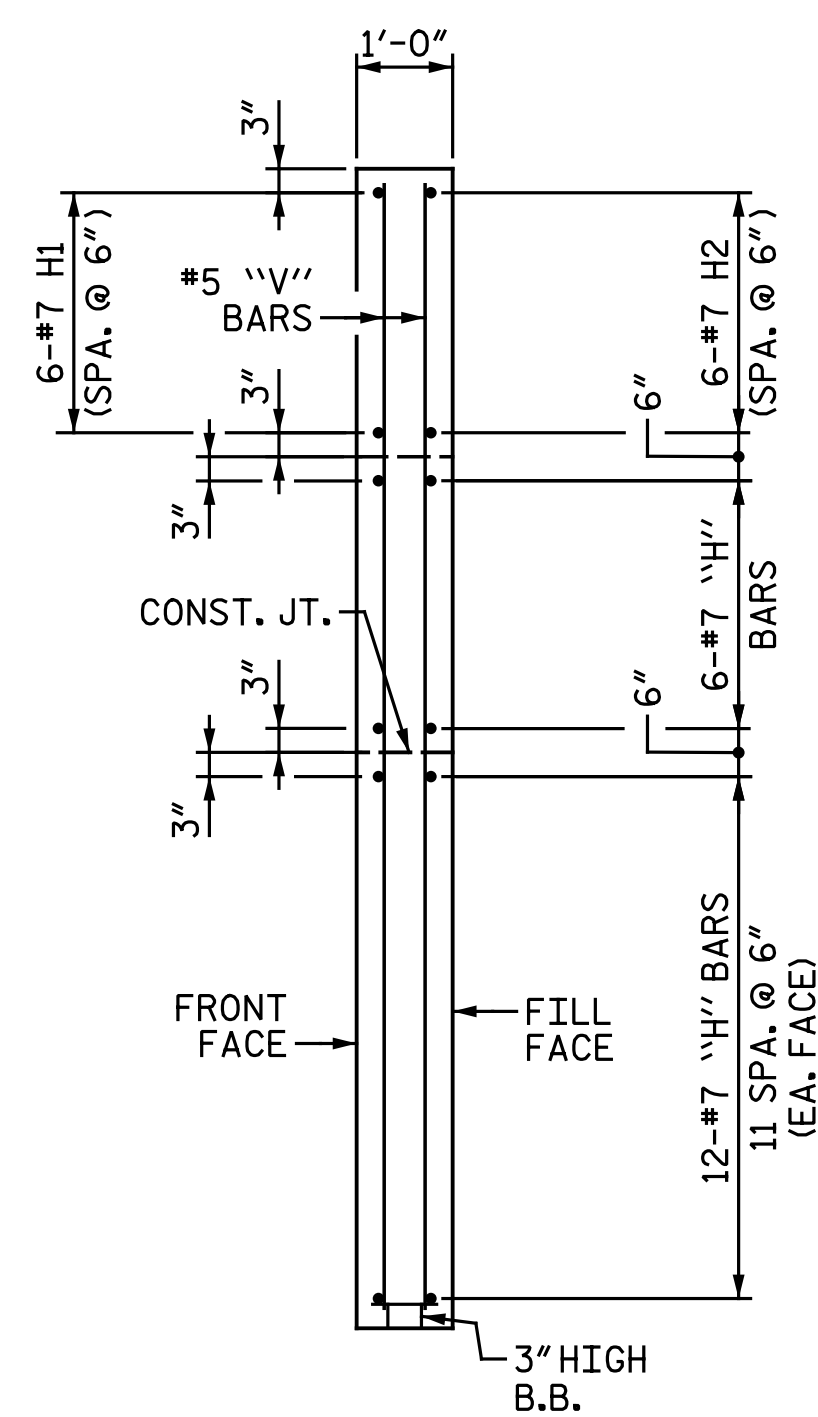
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PLAN OF LEFT WING



ELEVATION OF LEFT WING



SECTION X-X

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 3 OF 4

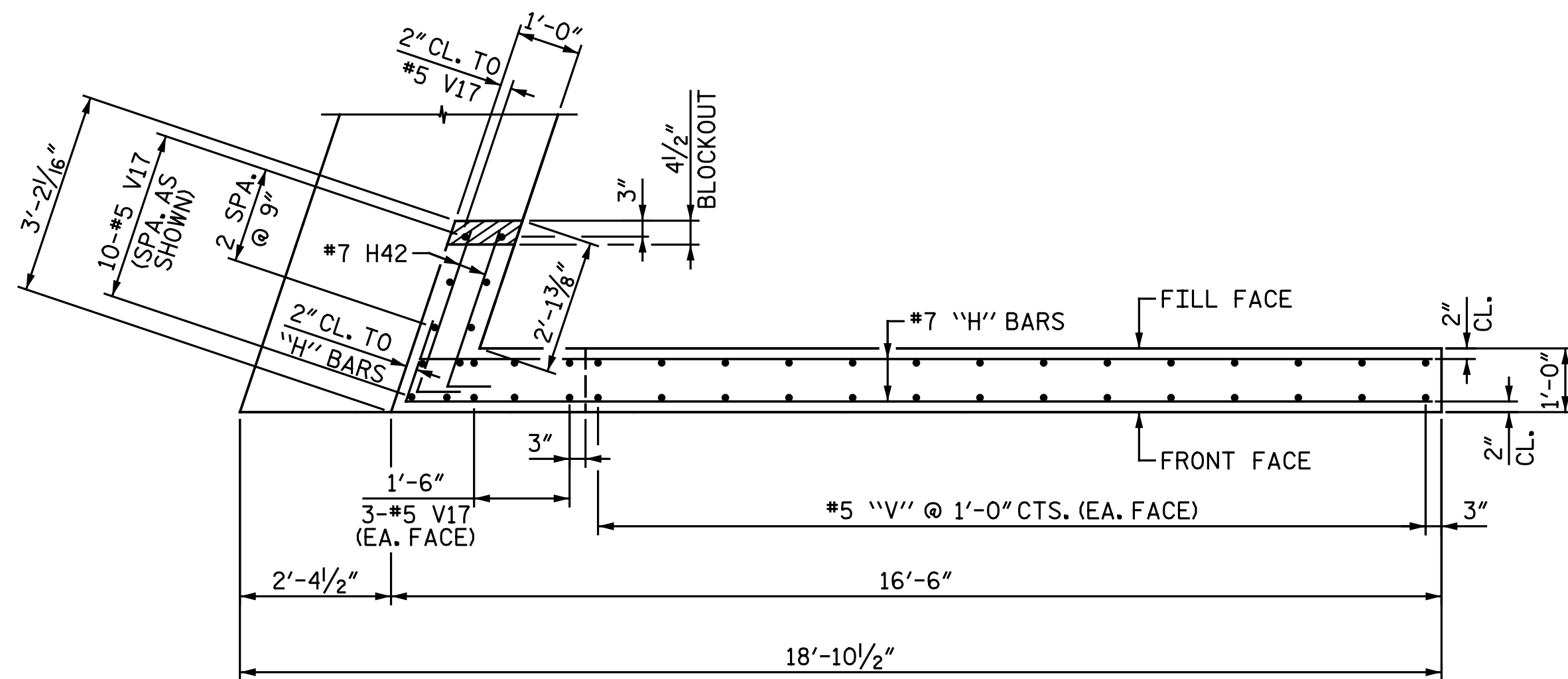


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 2
 LEFT LANE

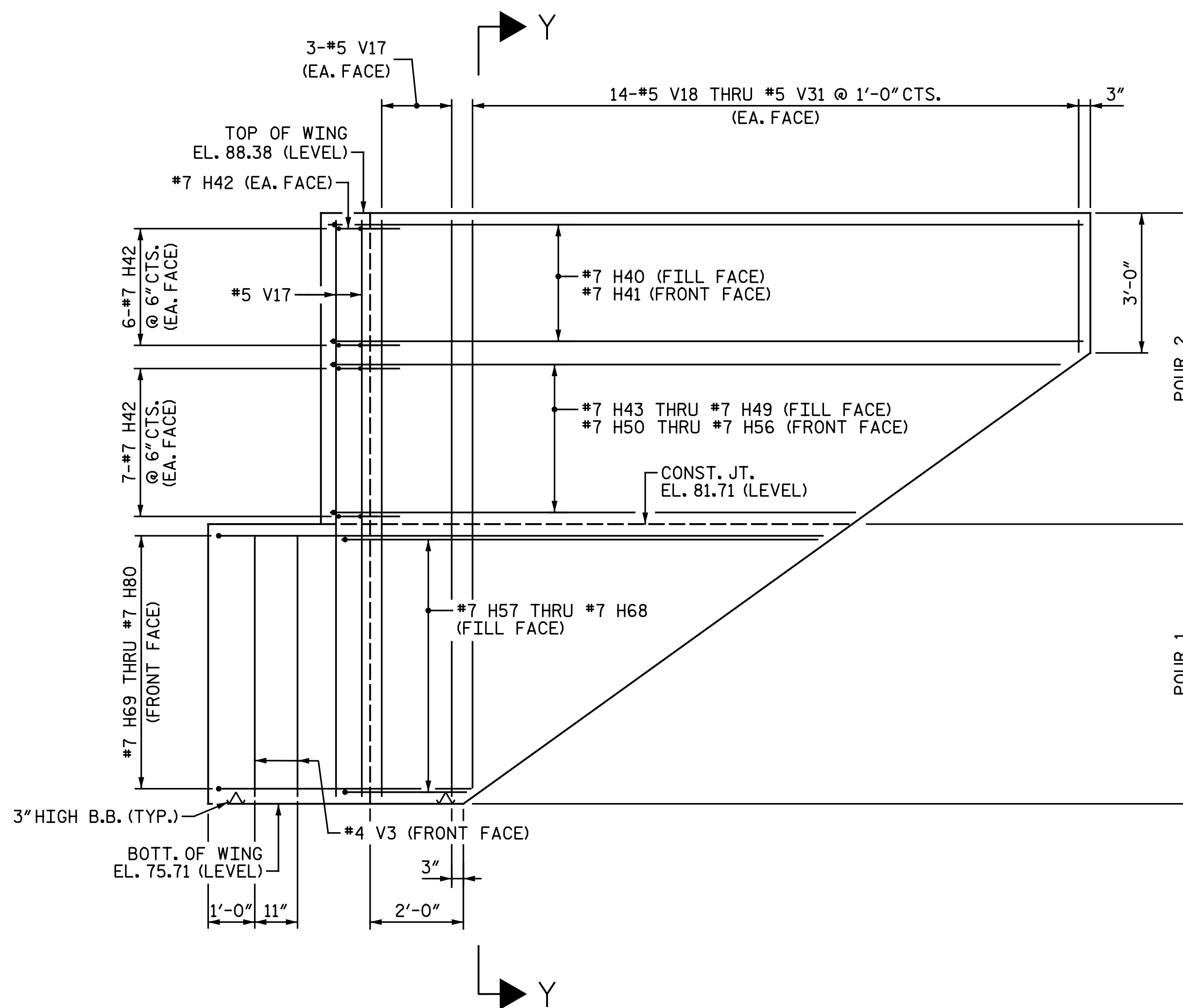
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-28
1			3			TOTAL SHEETS
2			4			34

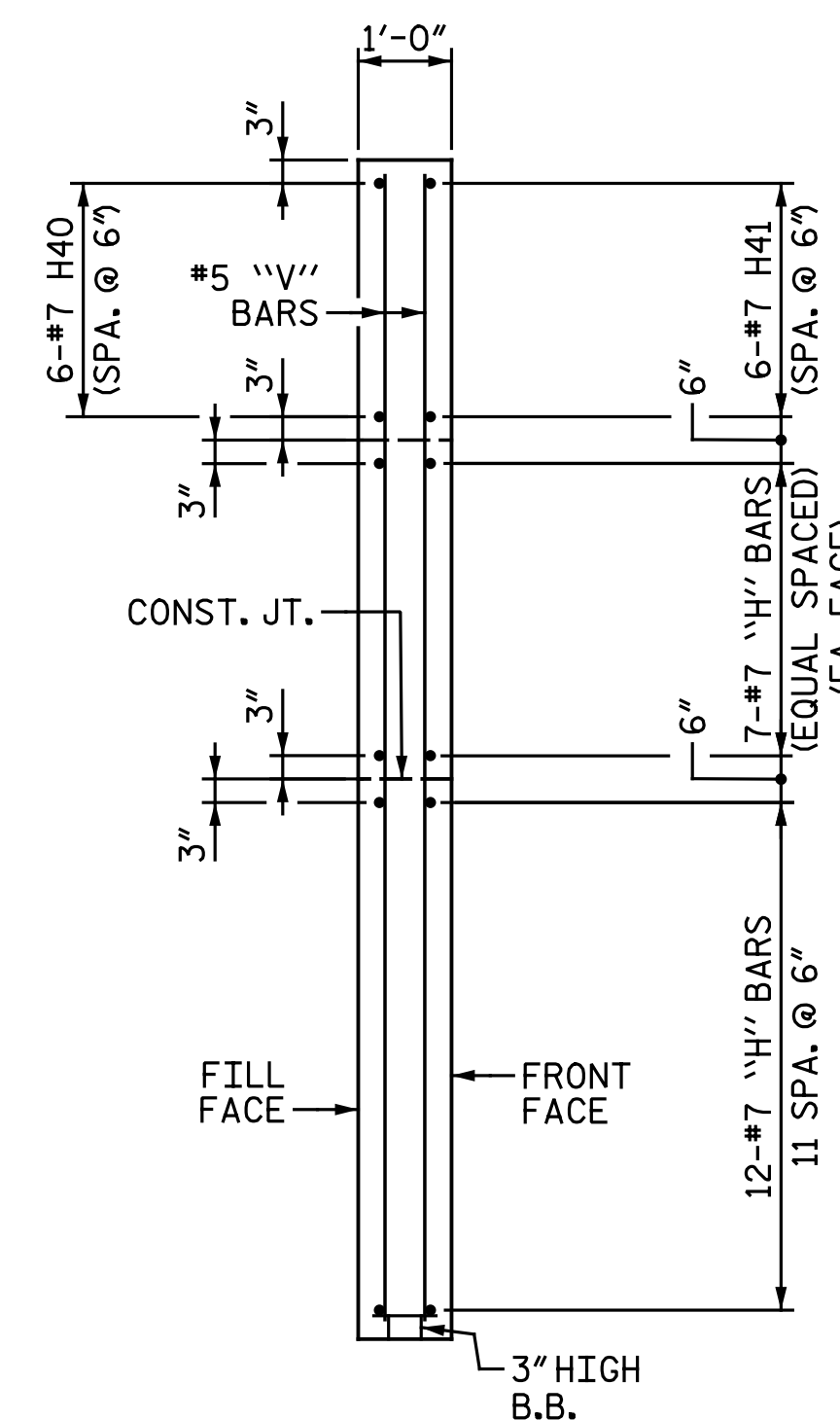
DRAWN BY : C. E. MAYHEW DATE : 4-14-17
 CHECKED BY : D. A. COLETTI DATE : 6-9-17



PLAN OF RIGHT WING



ELEVATION OF RIGHT WING



SECTION Y-Y

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 4 OF 4



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 2

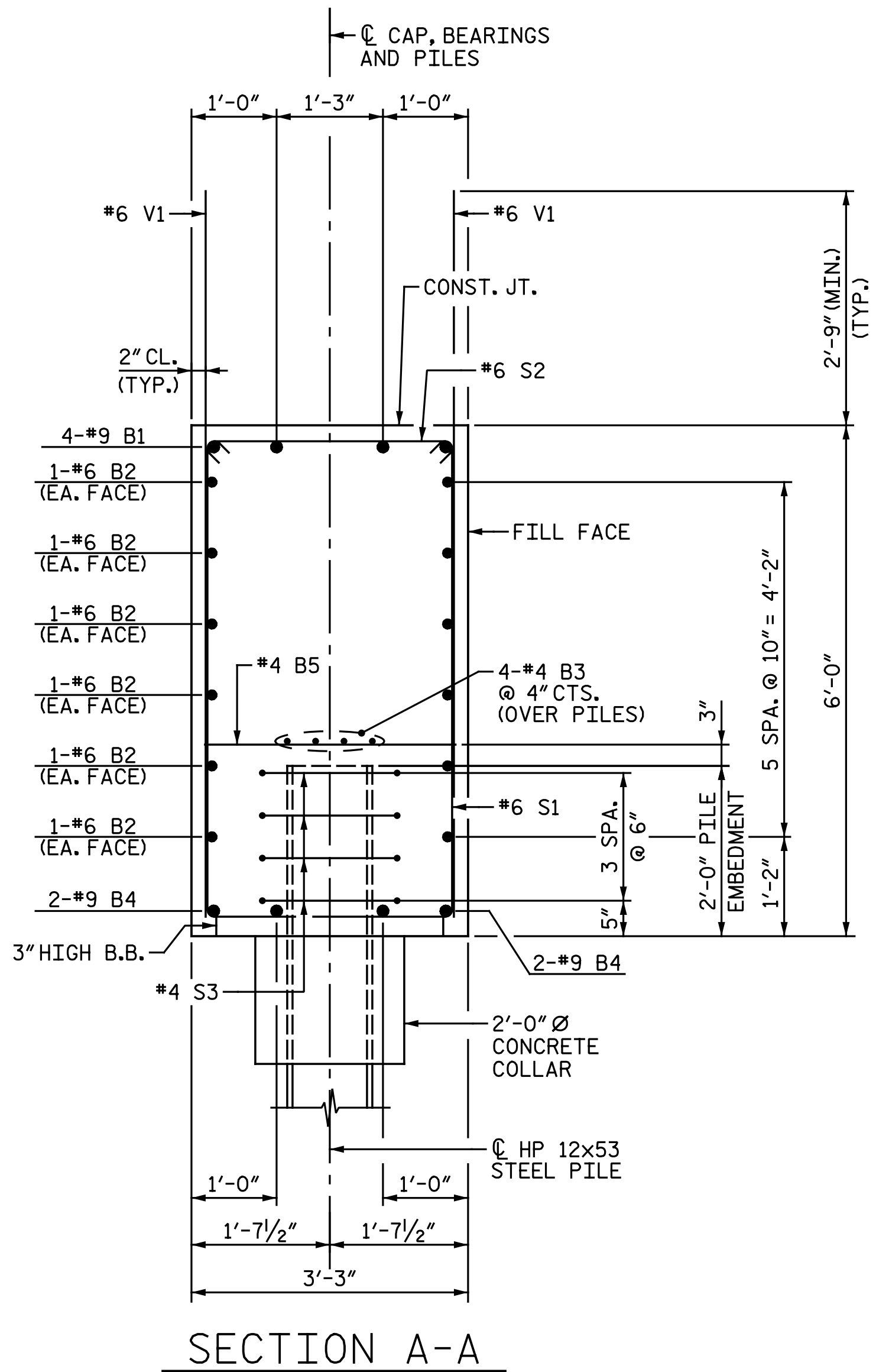
LEFT LANE

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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-29
1			3			TOTAL SHEETS
2			4			34

DRAWN BY : C. E. MAYHEW DATE : 7-14-17
 CHECKED BY : D. A. COLETTI DATE : 7-14-17

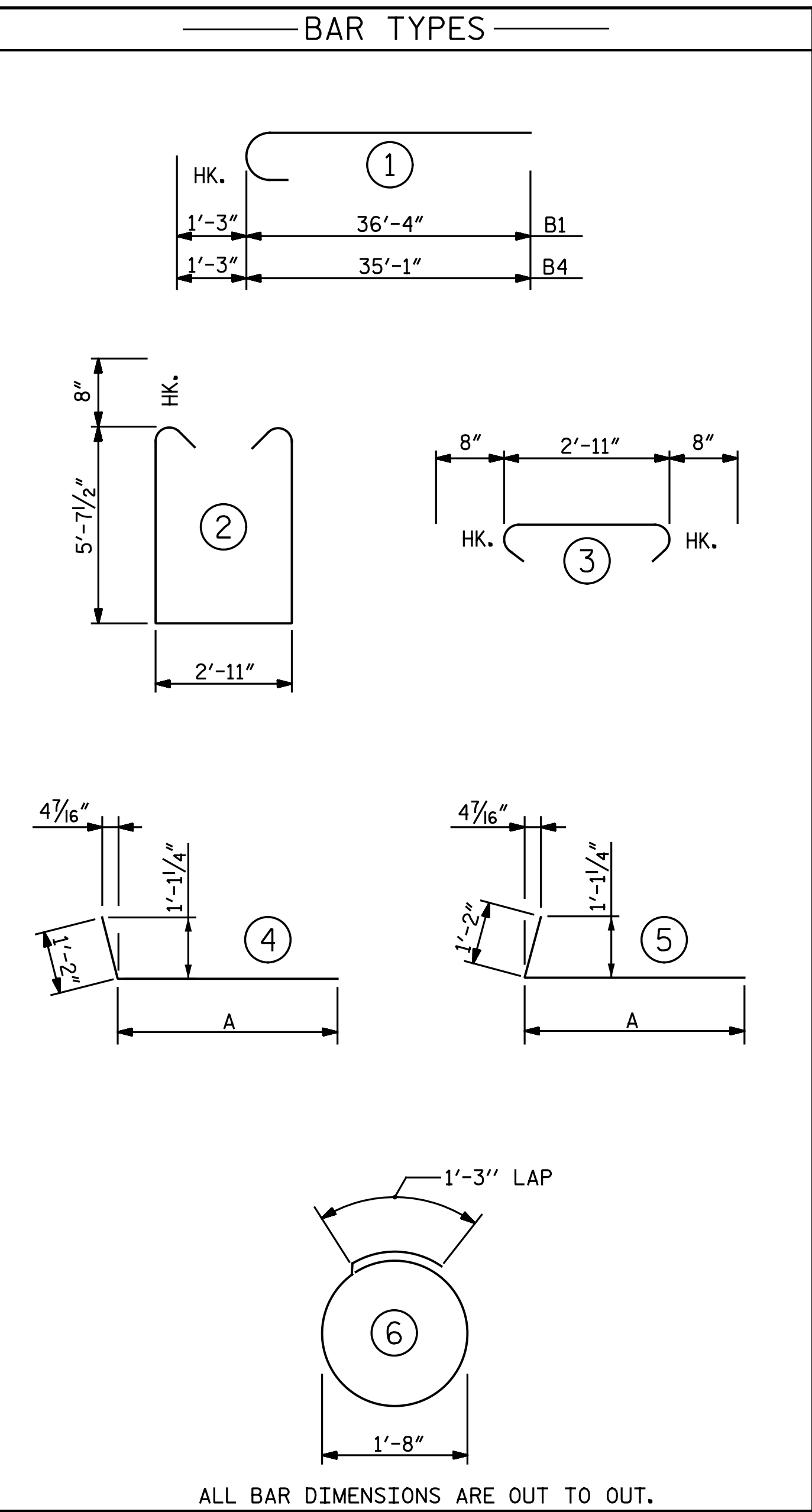


BAR	A	BAR	A
H1	15' - 8"	H41	16' - 1"
H2	15' - 5"	H42	2' - 6"
H3	2' - 6"	H43	15' - 4"
H4	15' - 2"	H44	14' - 7"
H5	14' - 5"	H45	13' - 10"
H6	13' - 8"	H46	13' - 1"
H7	12' - 12"	H47	12' - 4"
H8	12' - 3"	H48	11' - 7"
H9	11' - 7"	H49	10' - 11"
H10	14' - 11"	H50	15' - 6"
H11	14' - 2"	H51	14' - 10"
H12	13' - 5"	H52	14' - 1"
H13	12' - 9"	H53	13' - 4"
H14	11' - 12"	H54	12' - 7"
H15	11' - 4"	H55	11' - 10"
H16	10' - 9"	H56	11' - 2"
H17	10' - 1"	H57	10' - 3"
H18	9' - 5"	H58	9' - 6"
H19	8' - 8"	H59	8' - 10"
H20	7' - 12"	H60	8' - 1"
H21	7' - 4"	H61	7' - 5"
H22	6' - 7"	H62	6' - 9"
H23	5' - 11"	H63	5' - 12"
H24	5' - 3"	H64	5' - 4"
H25	4' - 6"	H65	4' - 8"
H26	3' - 10"	H66	3' - 11"
H27	3' - 2"	H67	3' - 3"
H28	12' - 11"	H68	2' - 7"
H29	12' - 2"	H69	12' - 9"
H30	11' - 6"	H70	12' - 1"
H31	10' - 9"	H71	11' - 5"
H32	10' - 1"	H72	10' - 8"
H33	9' - 5"	H73	9' - 12"
H34	8' - 8"	H74	9' - 4"
H35	7' - 12"	H75	8' - 7"
H36	7' - 4"	H76	7' - 11"
H37	6' - 7"	H77	7' - 3"
H38	5' - 11"	H78	6' - 6"
H39	5' - 3"	H79	5' - 10"
H40	15' - 10"	H80	5' - 2"

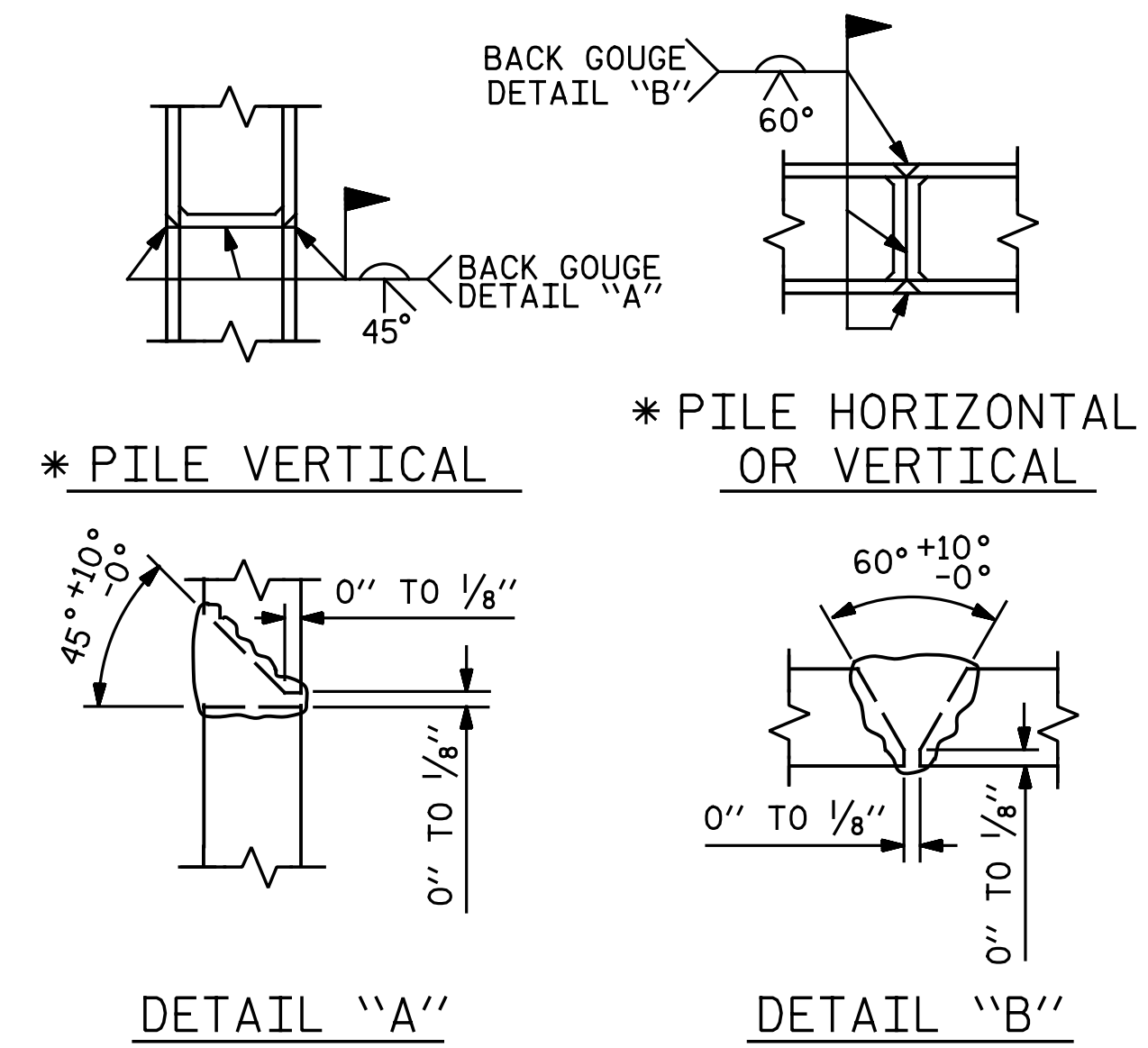
NOTE:
FOR TEMPORARY DRAINAGE AT END BENT DETAILS, SEE "INTEGRAL END BENT 1 DETAILS" SHEET.

BILL OF MATERIAL						BILL OF MATERIAL					
INTEGRAL END BENT 2						INTEGRAL END BENT 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#9		37' - 7"	1,022	H64	1	#7	4	6' - 6"	13
B2	24	#6	STR.	33' - 11"	1,223	H65	1	#7	4	5' - 10"	12
B3	8	#4	STR.	33' - 3"	178	H66	1	#7	4	5' - 1"	10
B4	8	#9	1	36' - 4"	988	H67	1	#7	4	4' - 5"	9
B5	16	#4	STR.	2' - 11"	31	H68	1	#7	4	3' - 9"	8
H1	6	#7	5	16' - 9"	206	H69	1	#7	4	13' - 11"	28
H2	6	#7	5	16' - 7"	203	H70	1	#7	4	13' - 3"	27
H3	24	#7	5	3' - 8"	180	H71	1	#7	4	12' - 7"	26
H4	1	#7	5	16' - 4"	33	H72	1	#7	4	11' - 10"	24
H5	1	#7	5	15' - 7"	32	H73	1	#7	4	11' - 2"	23
H6	1	#7	5	14' - 10"	30	H74	1	#7	4	10' - 6"	21
H7	1	#7	5	14' - 2"	29	H75	1	#7	4	9' - 9"	20
H8	1	#7	5	13' - 5"	27	H76	1	#7	4	9' - 1"	19
H9	1	#7	5	12' - 9"	26	H77	1	#7	4	8' - 5"	17
H10	1	#7	5	16' - 1"	33	H78	1	#7	4	7' - 8"	16
H11	1	#7	5	15' - 4"	31	H79	1	#7	4	7' - 0"	14
H12	1	#7	5	14' - 7"	30	H80	1	#7	4	6' - 4"	13
H13	1	#7	5	13' - 11"	28	S1	65	#6	2	15' - 6"	1,513
H14	1	#7	5	13' - 2"	27	S2	65	#6	3	4' - 3"	415
H15	1	#7	5	12' - 6"	26	S3	48	#4	6	6' - 6"	208
H16	1	#7	5	11' - 11"	24	V1	102	#6	STR.	8' - 6"	1,302
H17	1	#7	5	11' - 3"	23	V2	16	#5	STR.	11' - 7"	193
H18	1	#7	5	10' - 7"	22	V3	4	#4	STR.	5' - 7"	15
H19	1	#7	5	9' - 10"	20	V4	2	#5	STR.	11' - 3"	23
H20	1	#7	5	9' - 2"	19	V5	2	#5	STR.	10' - 7"	22
H21	1	#7	5	8' - 6"	17	V6	2	#5	STR.	9' - 10"	21
H22	1	#7	5	7' - 9"	16	V7	2	#5	STR.	9' - 2"	19
H23	1	#7	5	7' - 1"	14	V8	2	#5	STR.	8' - 5"	18
H24	1	#7	5	6' - 5"	13	V9	2	#5	STR.	7' - 9"	16
H25	1	#7	5	5' - 8"	12	V10	2	#5	STR.	7' - 0"	15
H26	1	#7	5	5' - 0"	10	V11	2	#5	STR.	6' - 3"	13
H27	1	#7	5	4' - 4"	9	V12	2	#5	STR.	5' - 7"	12
H28	1	#7	5	14' - 1"	29	V13	2	#5	STR.	4' - 10"	10
H29	1	#7	5	13' - 4"	27	V14	2	#5	STR.	4' - 2"	9
H30	1	#7	5	12' - 8"	26	V15	2	#5	STR.	3' - 5"	7
H31	1	#7	5	11' - 11"	24	V16	2	#5	STR.	2' - 8"	6
H32	1	#7	5	11' - 3"	23	V17	16	#5	STR.	12' - 3"	204
H33	1	#7	5	10' - 7"	22	V18	2	#5	STR.	12' - 1"	25
H34	1	#7	5	9' - 10"	20	V19	2	#5	STR.	11' - 4"	24
H35	1	#7	5	9' - 2"	19	V20	2	#5	STR.	10' - 7"	22
H36	1	#7	5	8' - 6"	17	V21	2	#5	STR.	9' - 11"	21
H37	1	#7	5	7' - 9"	16	V22	2	#5	STR.	9' - 2"	19
H38	1	#7	5	7' - 1"	14	V23	2	#5	STR.	8' - 5"	18
H39	1	#7	5	6' - 5"	13	V24	2	#5	STR.	7' - 9"	16
H40	6	#7	5	17' - 0"	208	V25	2	#5	STR.	7' - 0"	15
H41	6	#7	5	17' - 3"	212	V26	2	#5	STR.	6' - 4"	13
H42	26	#7	5	3' - 8"	195	V27	2	#5	STR.	5' - 7"	12
H43	1	#7	5	16' - 6"	34	V28	2	#5	STR.	4' - 10"	10
H44	1	#7	4	15' - 9"	32	V29	2	#5	STR.	4' - 2"	9
H45	1	#7	4	15' - 0"	31	V30	2	#5	STR.	3' - 5"	7
H46	1	#7	4	14' - 3"	29	V31	2	#4	STR.	2' - 8"	4
H47	1	#7	4	13' - 6"	28						
H48	1	#7	4	12' - 9"	26						
H49	1	#7	4	12' - 1"	25						
H50	1	#7	4	16' - 8"	34						
H51	1	#7	4	16' - 0"	33						
H52	1	#7	4	15' - 3"	31						
H53	1	#7	4	14' - 6"	30						
H54	1	#7	4	13' - 9"	28						
H55	1	#7	4	13' - 0"	27						
H56	1	#7	4	12' - 4"	25						
H57	1	#7	4	11' - 5"	23						
H58	1	#7	4	10' - 8"	22						
H59	1	#7	4	10' - 0"	20						
H60	1	#7	4	9' - 3"	19						
H61	1	#7	4	8' - 7"	18						
H62	1	#7	4	7' - 11"	16						
H63	1	#7	4	7' - 2"	15						

REINFORCING STEEL	LBS.	10,549
CLASS A CONCRETE		
POUR 1 -		
CAP, LOWER PART OF WINGS & COLLARS	C.Y.	51.6
POUR 2 -		
UPPER PART OF WINGS	C.Y.	8.1
TOTAL	C.Y.	59.7
PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES	EA.	12
HP 12x53 STEEL PILES		
NO. 12	L.F.	1020
PILE REDRIVES	EA.	6



ALL BAR DIMENSIONS ARE OUT TO OUT.



PILE SPLICE DETAILS
* POSITION OF PILE DURING WELDING

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 166+72.51 -L-



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE
INTEGRAL END BENT 2
DETAILS

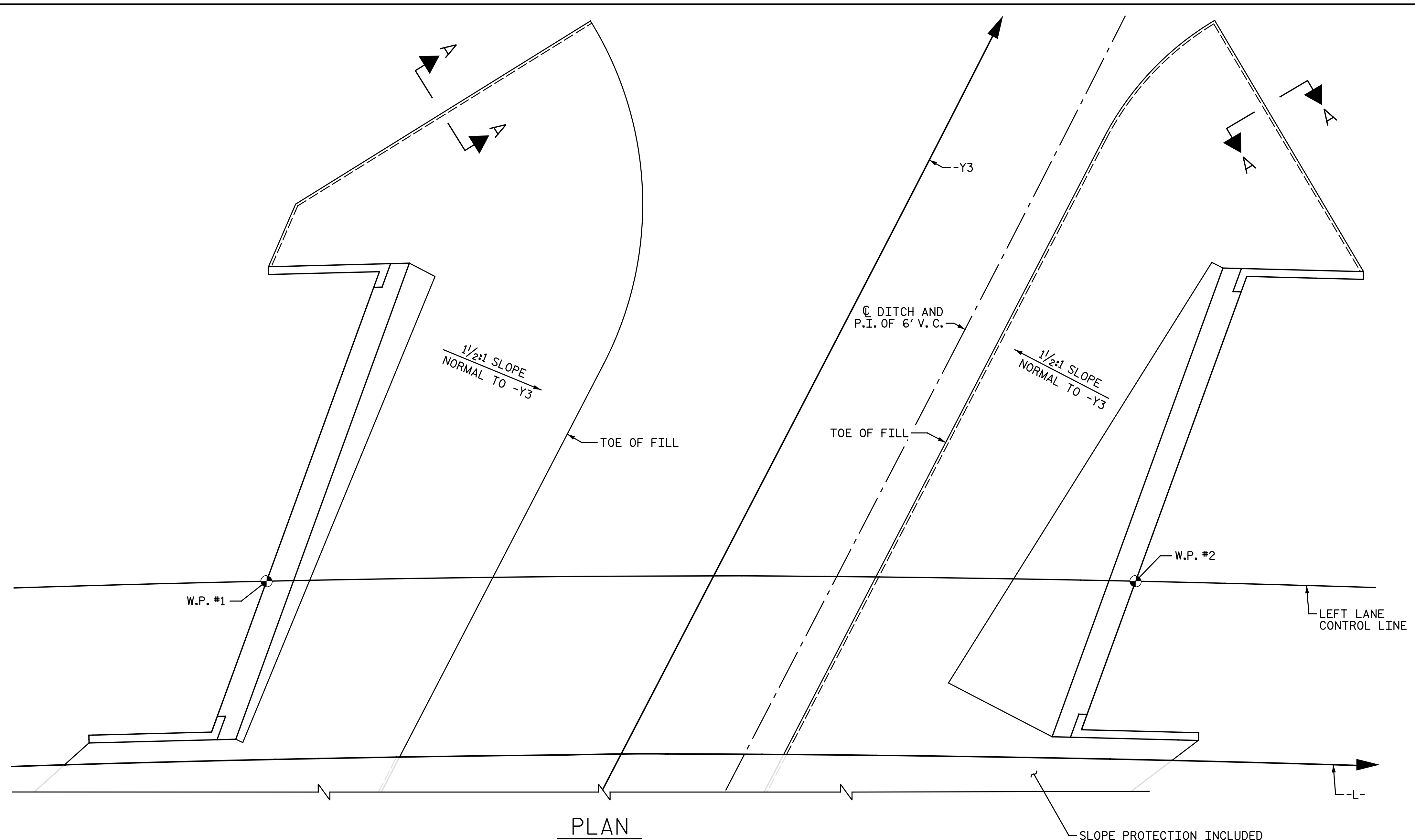
LEFT LANE

8/14/2017
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UNLESS ALL SIGNATURES COMPLETED

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Michael Baker Engineering
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Cary, North Carolina 27518
NC License No.: F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS
2			4			34

DRAWN BY: CEM / MDM DATE: 7-14-17
CHECKED BY: D. A. COLETTI DATE: 7-14-17

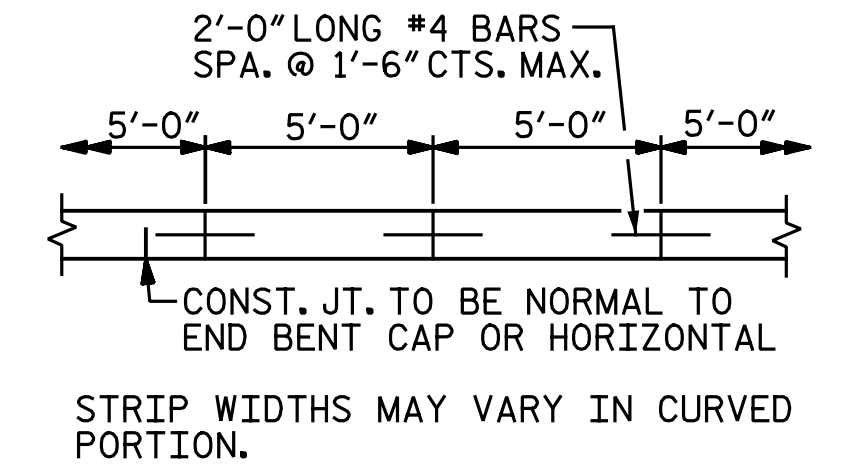


PLAN

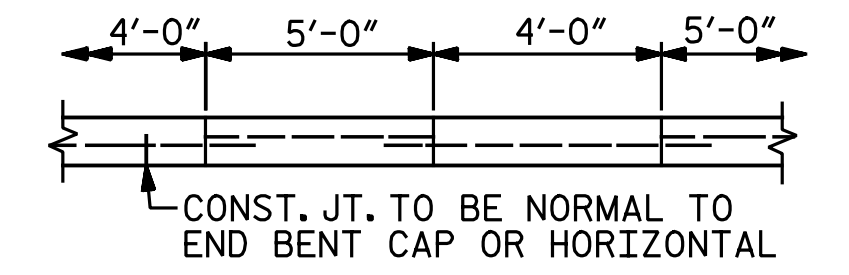
GENERAL NOTES:
 STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT.
 MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.
 FOR BERM WIDTHS AND ELEVATIONS, SEE GENERAL DRAWING AND "SLOPE PROTECTION DETAILS" SHEET 2 OF 2.
 SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET.
 CONCRETE SHALL BE CLASS "B".
 THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED.
 WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE.
 SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0" LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING.
 SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6".
 THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA. 166+72.51 -L- (LEFT LANE)	4 INCH SLOPE PROTECTION	WELDED WIRE FABRIC * 60 INCHES WIDE
	SQUARE YARDS	APPROX. L.F.
END BENT 1	390	705
END BENT 2	430	775

* QUANTITY SHOWN IS BASED ON 5' POURS.

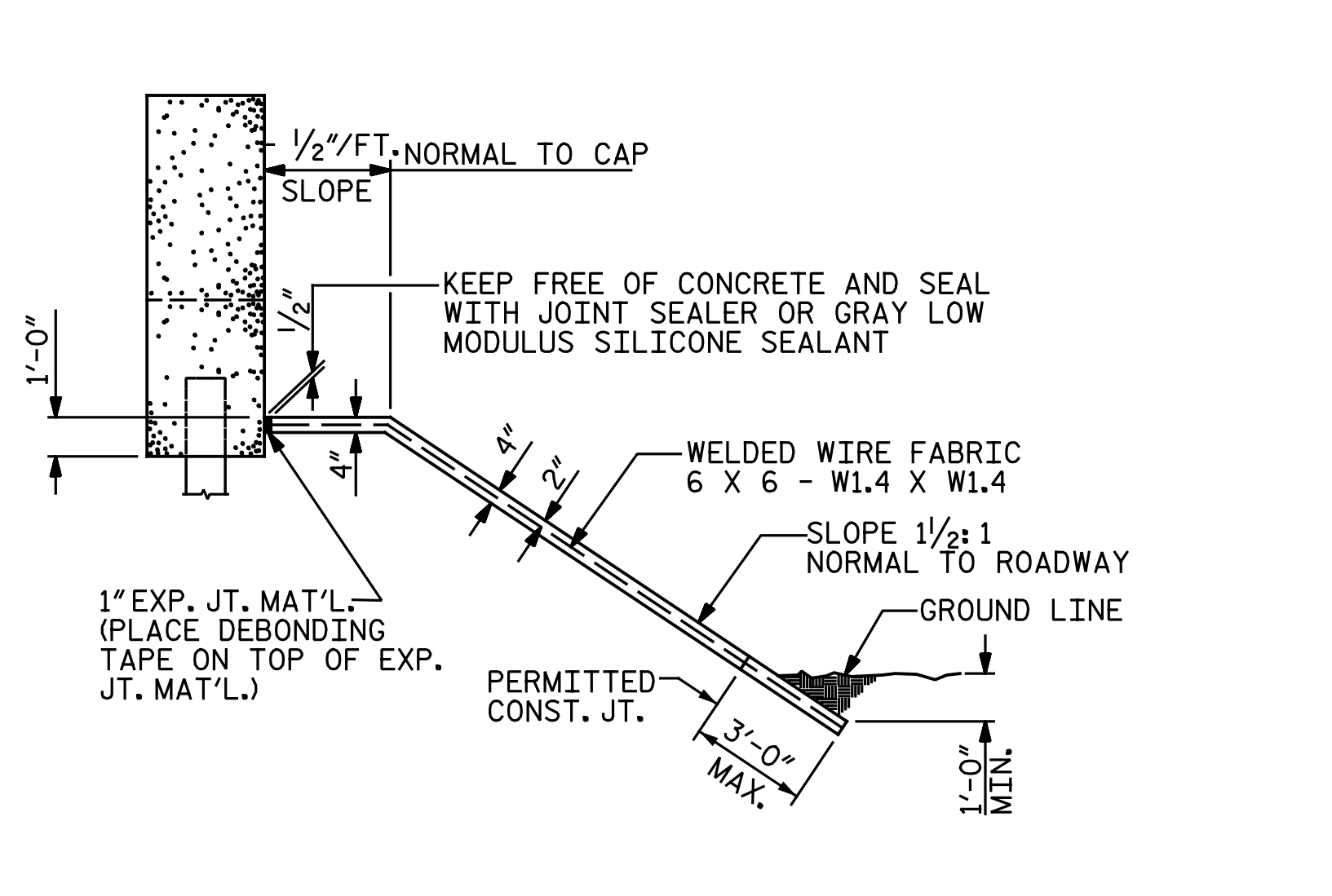


POURING DETAIL

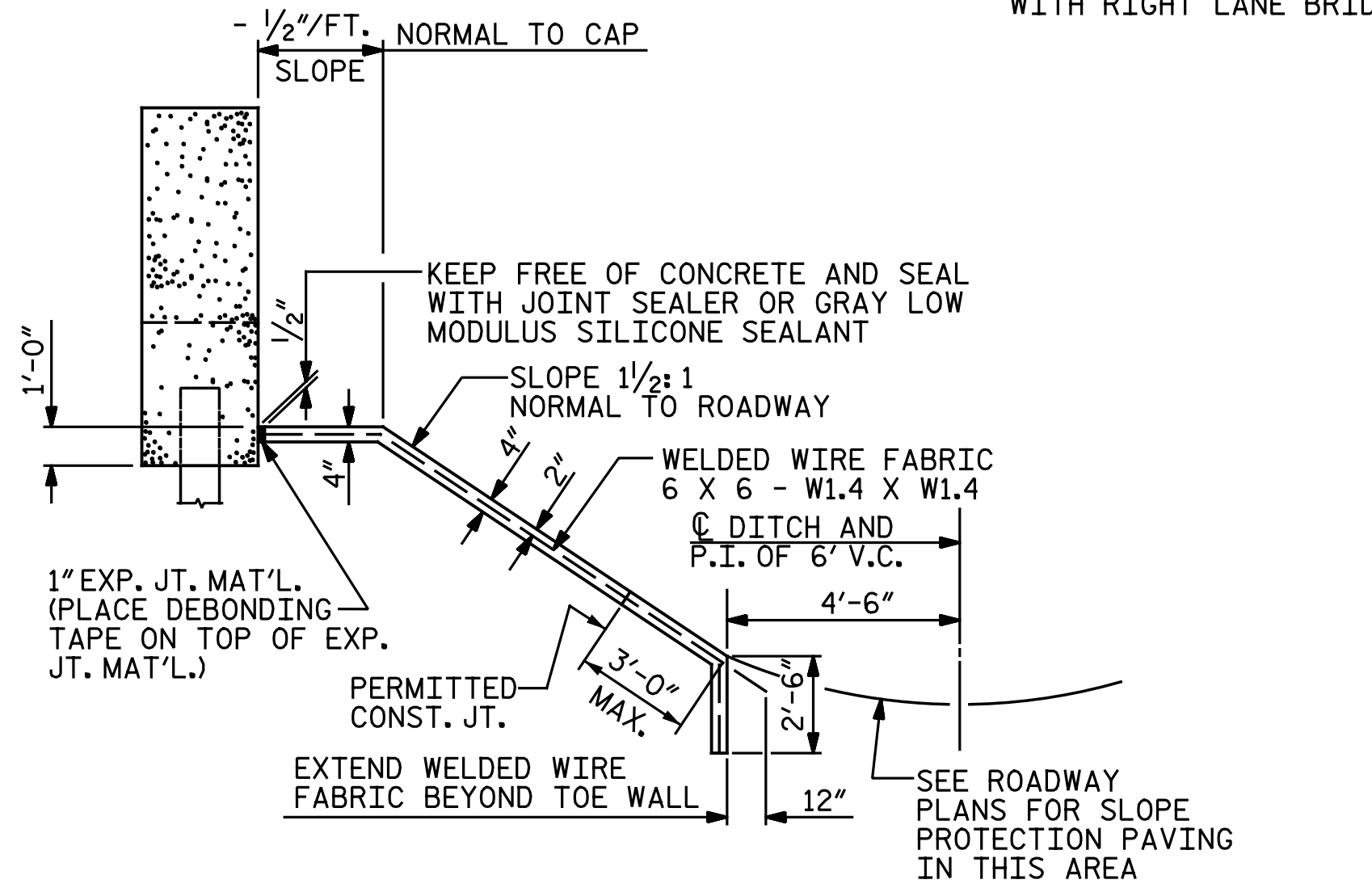


POUR A 4'-0" STRIP FIRST. STRIP WIDTHS MAY VARY IN CURVED PORTION.

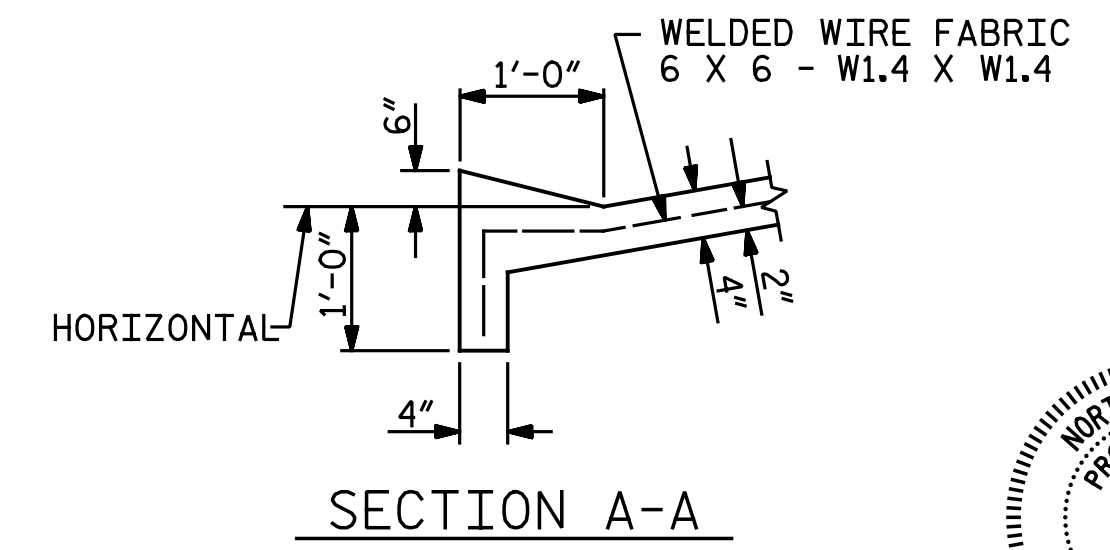
OPTIONAL POURING DETAIL



SECTION A-A SURVEY WHEN DITCH IS NOT PROVIDED



SECTION A-A SURVEY WHEN FILL CATCHES IN DITCH



SECTION A-A



PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-
 SHEET 1 OF 2

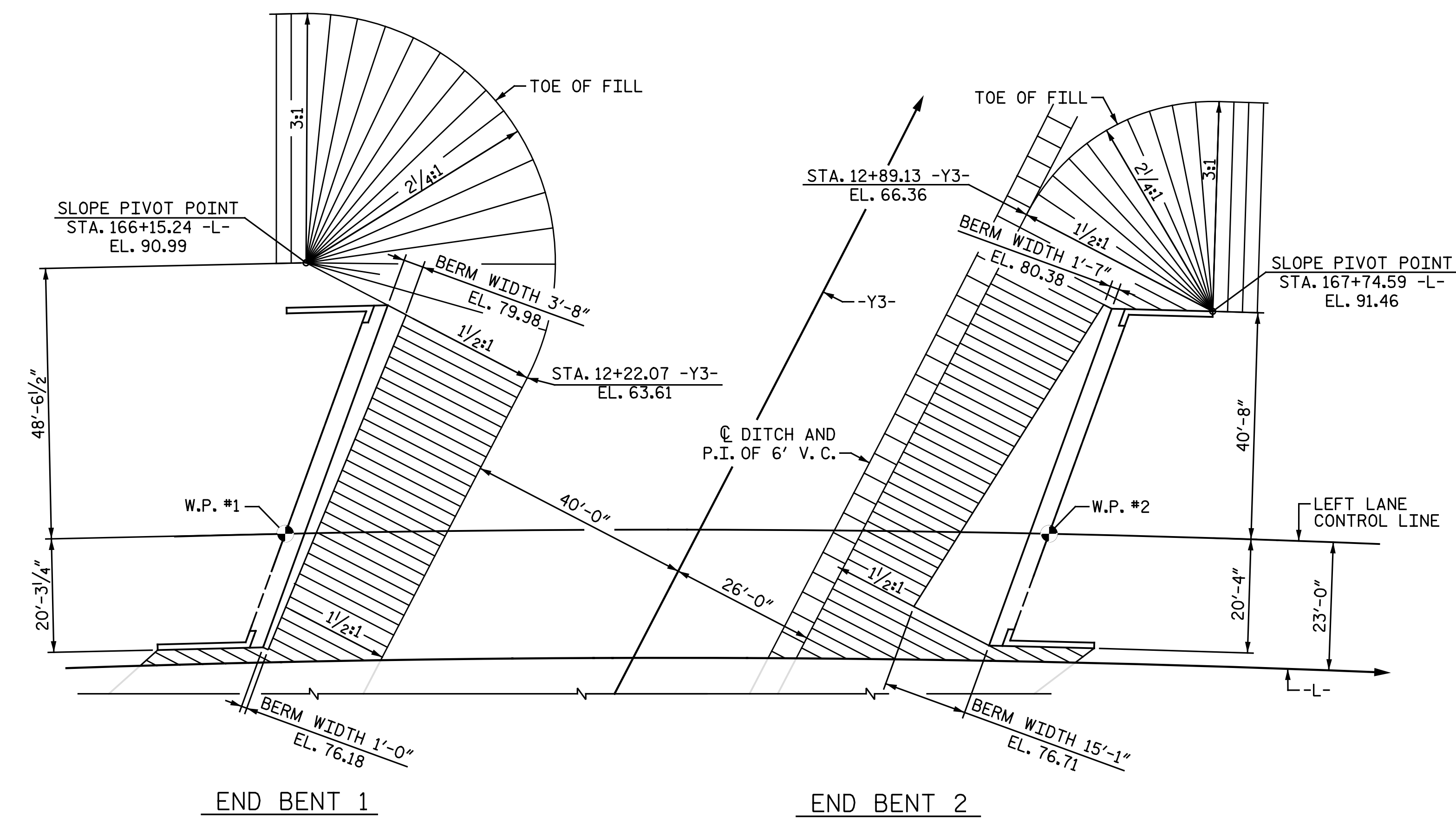
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 SLOPE PROTECTION
 DETAILS
 LEFT LANE

ASSEMBLED BY : M. D. MAYHEW	DATE : 7-14-17
CHECKED BY : D. A. COLETTI	DATE : 7-18-17
DRAWN BY : ELR 5/92	REV. 10/1/11 MAA/GM
CHECKED BY : GRP 6/92	REV. 12/21/11 MAA/GM
	REV. 1/16 MAA/TMG

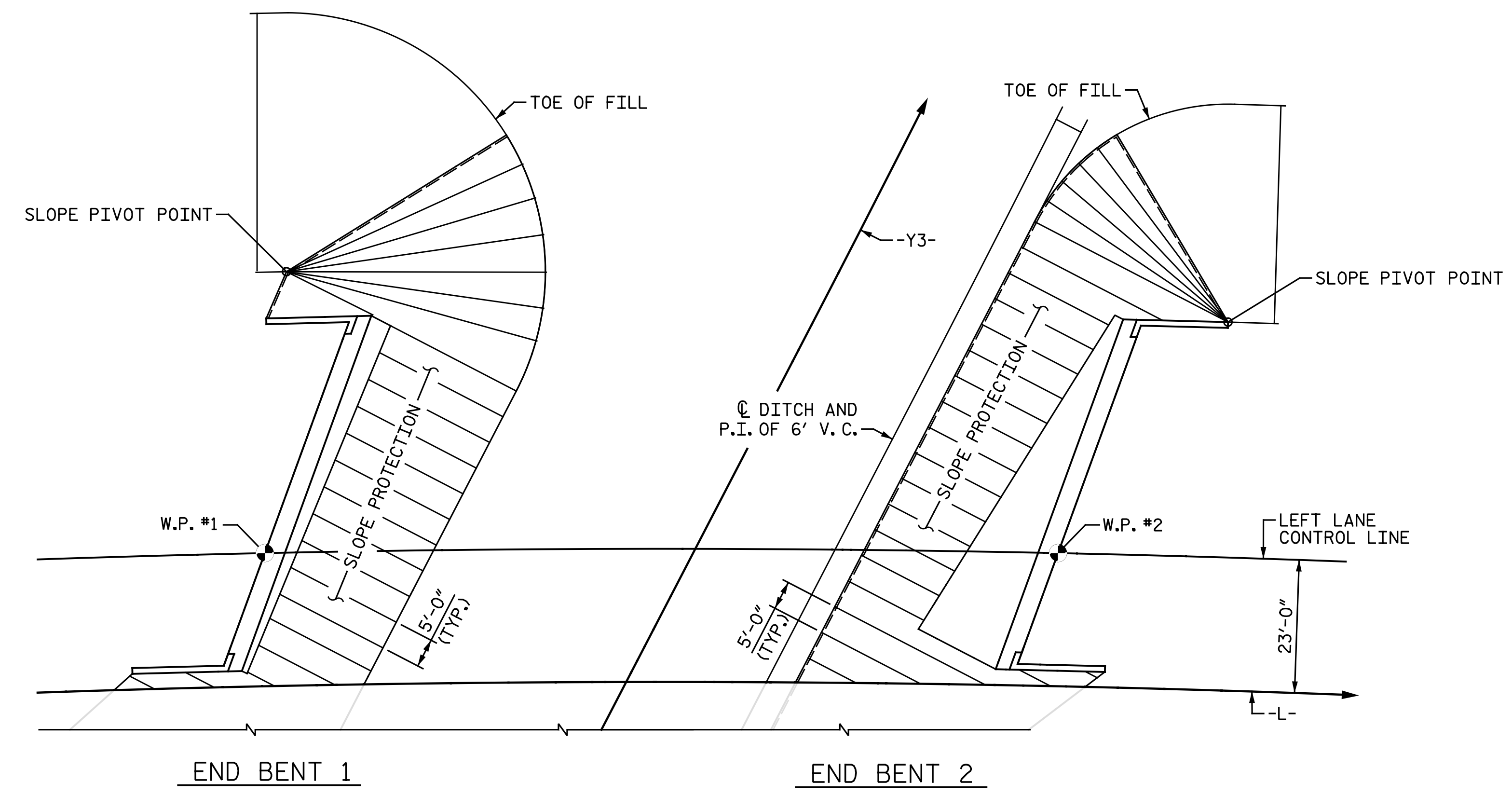
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 NC License No. : F-1084

REVISIONS						SHEET NO. S3-31
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 34
2			4			

NOTE:
ALL ELEVATIONS AND BERM WIDTHS ARE GIVEN AT THE TOP OF CONCRETE SLOPE PROTECTION.



PLAN - GRADING



PLAN - CONCRETE PLACEMENT
(1/2:1 SLOPE)

ASSEMBLED BY : M. D. MAYHEW	DATE : 7-14-17
CHECKED BY : D. A. COLETTI	DATE : 7-18-17
DRAWN BY : WJH 10/88	REV. 5/1/06 TLA/GM
CHECKED BY : FCJ 10/88	REV. 10/1/11 MAA/GM
	REV. 1/16 MAA/TMG

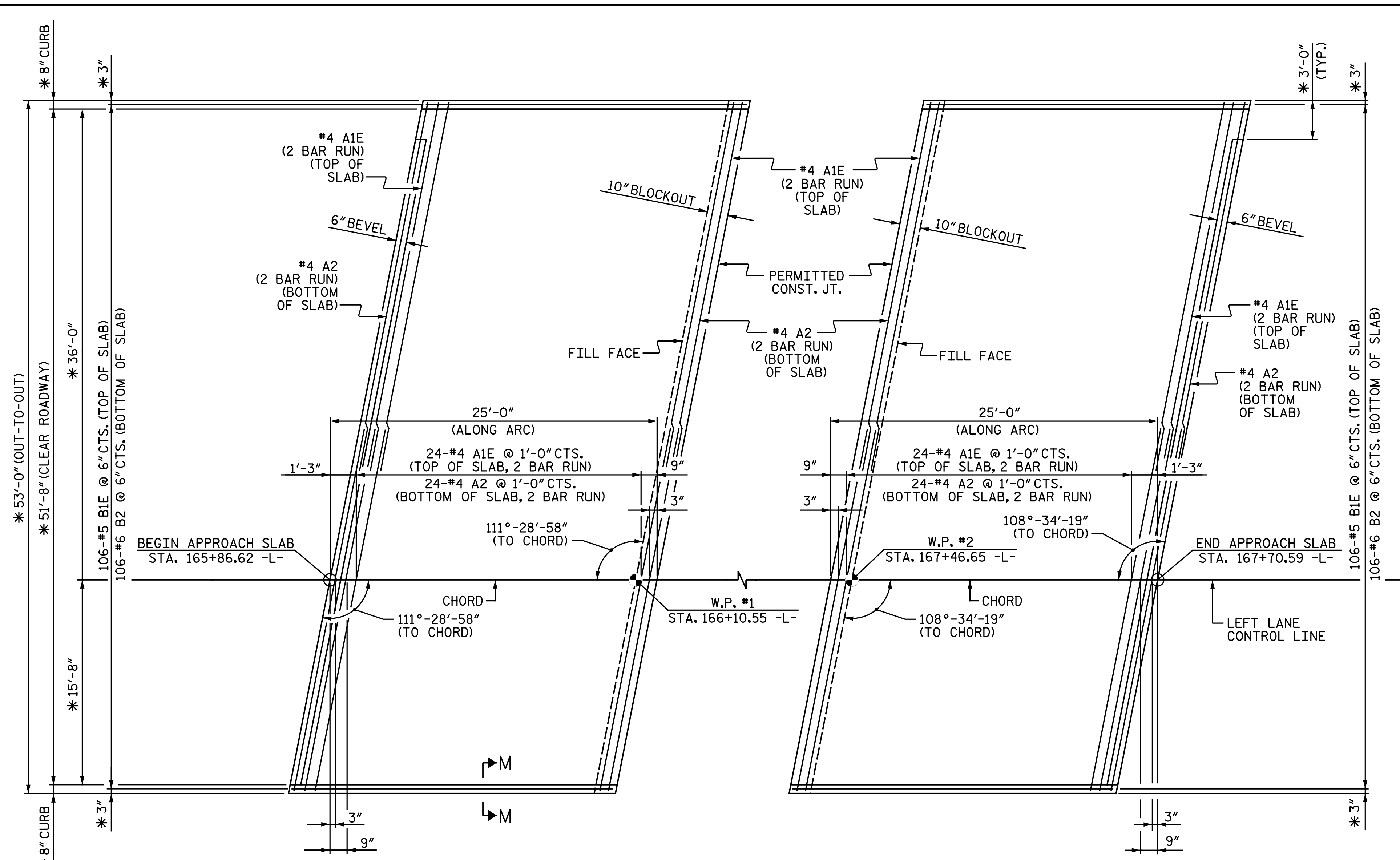


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PROJECT NO. R-5703
LENOIR COUNTY
STATION: 166+72.51 -L-
SHEET 2 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
SLOPE PROTECTION
DETAILS
LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			34
2			4			



PLAN @ INTEGRAL END BENT 1

PLAN @ INTEGRAL END BENT 2

NOTES:

AT THE CONTRACTOR'S OPTION, THE APPROACH SLAB MAY BE CAST MONOLITHICALLY WITH THE INTEGRAL END BENT DIAPHRAGM AND THE END SECTION OF BRIDGE DECK. IF CAST WITH THE INTEGRAL DIAPHRAGM, THE LAYERS OF ROOFING FELT SHALL BE OMITTED. IF CAST SEPERATE FROM THE INTEGRAL DIAPHRAGM, APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

#78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

#78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

BILL OF MATERIAL

APPROACH SLAB AT END BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1E	52	#4	STR.	29' - 4"	1,019
A2	52	#4	STR.	29' - 3"	1,016
B1E	106	#5	STR.	24' - 2"	2,672
B2	106	#6	STR.	24' - 8"	3,927
REINFORCING STEEL					LBS. 4,943

EPOXY COATED REINFORCING STEEL					LBS. 3,691
CLASS AA CONCRETE					C.Y. 57.2

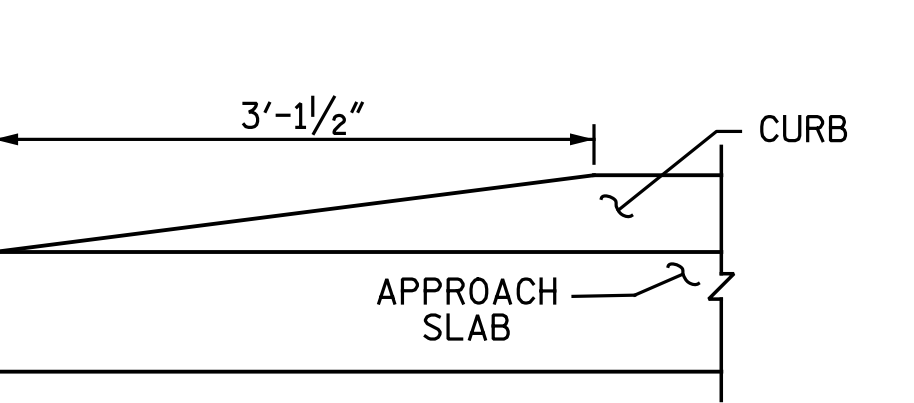
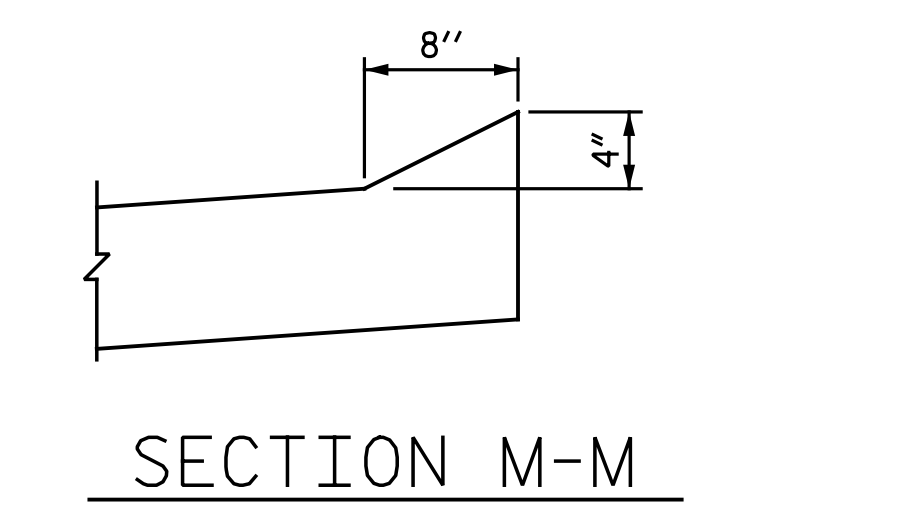
BILL OF MATERIAL

APPROACH SLAB AT END BENT 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1E	52	#4	STR.	29' - 4"	1,019
A2	52	#4	STR.	29' - 3"	1,016
B1E	106	#5	STR.	24' - 2"	2,672
B2	106	#6	STR.	24' - 8"	3,927
REINFORCING STEEL					LBS. 4,943

EPOXY COATED REINFORCING STEEL					LBS. 3,691
CLASS AA CONCRETE					C.Y. 57.2

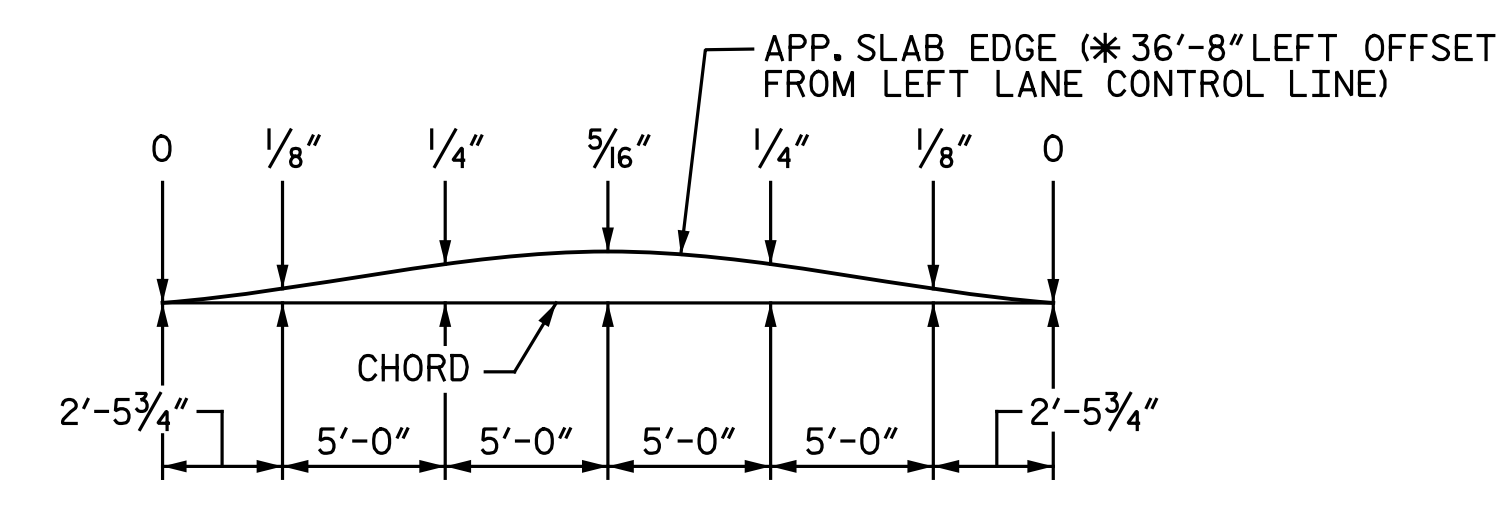
ALL BAR DIMENSIONS ARE OUT TO OUT.

SPLICE LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"

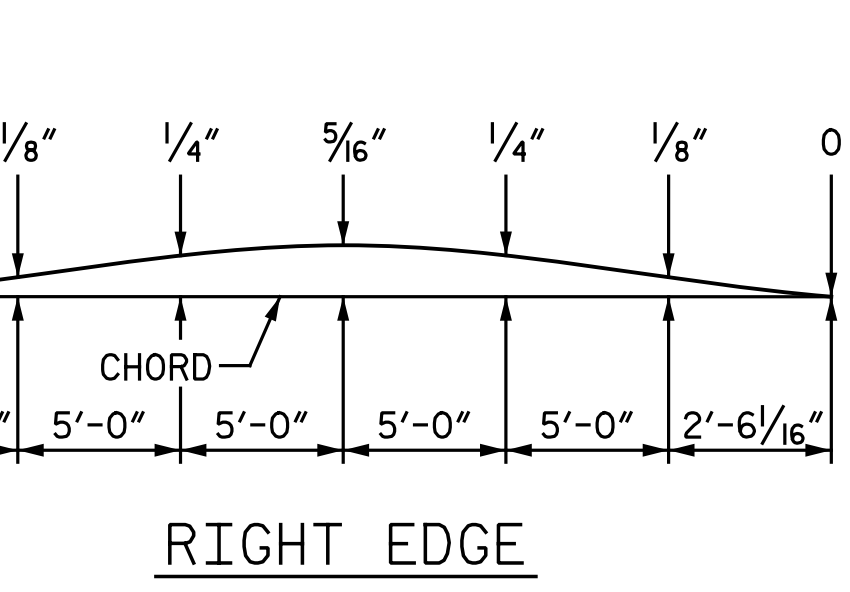


END OF CURB WITHOUT SHOULDER BERM GUTTER

* RADIAL DIMENSIONS
+ NORMAL TO END BENT

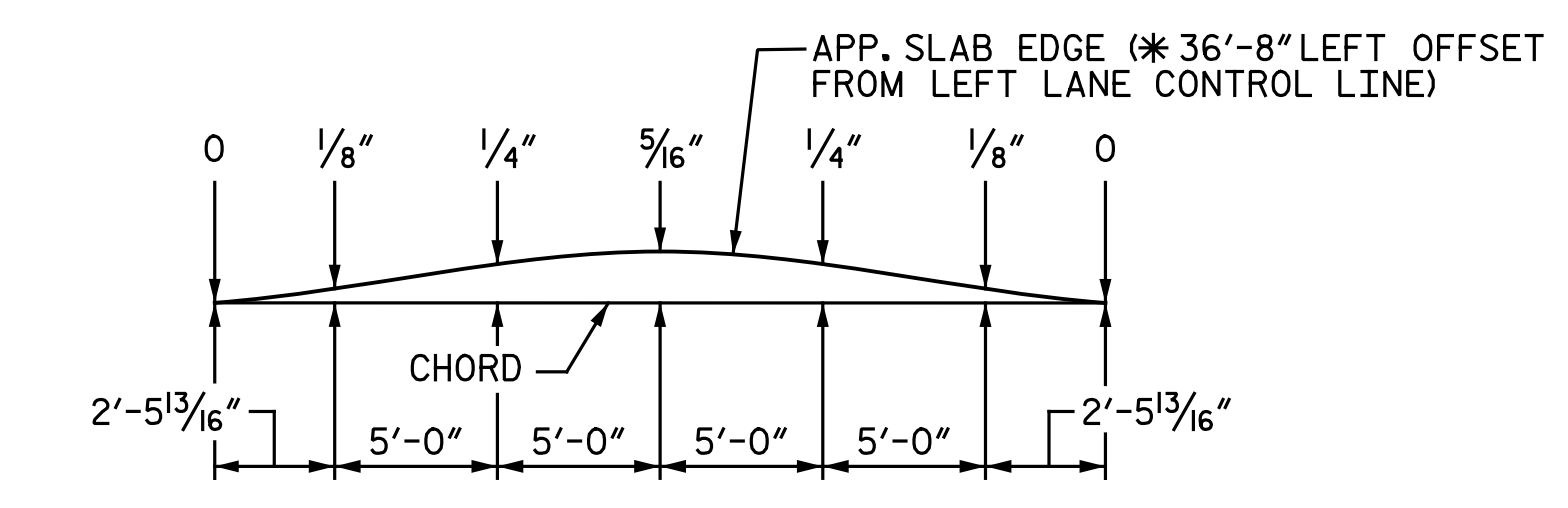


LEFT EDGE

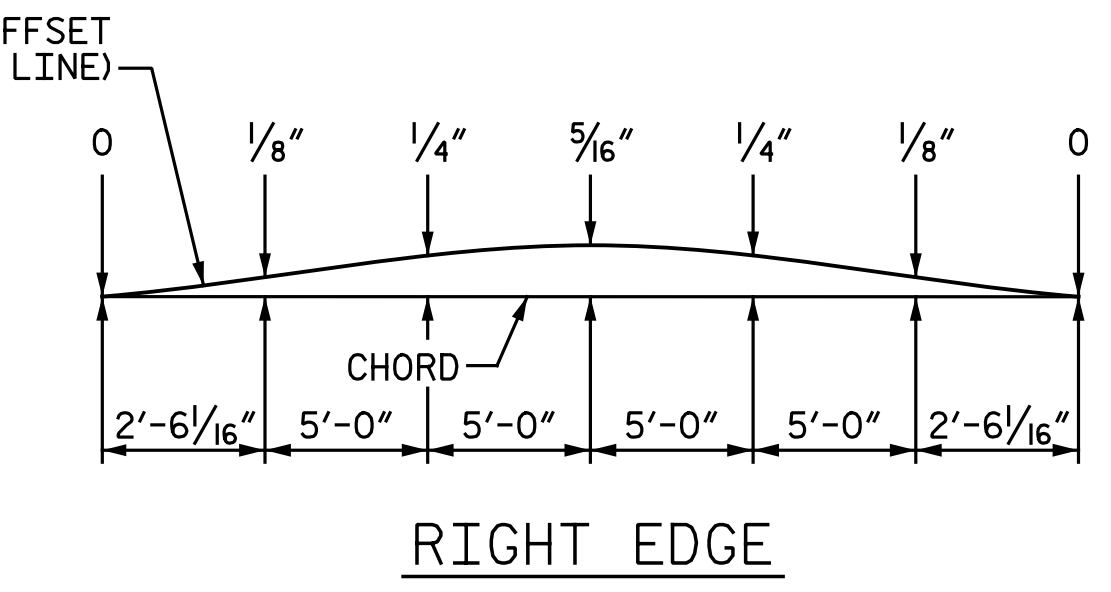


RIGHT EDGE

APPROACH SLAB @ END BENT 1



LEFT EDGE



RIGHT EDGE

APPROACH SLAB @ END BENT 2

ARC OFFSETS

DRAWN BY : N. B. SPEAKS DATE : 7-14-17
CHECKED BY : D. A. COLETTI DATE : 7-14-17

8/14/2017
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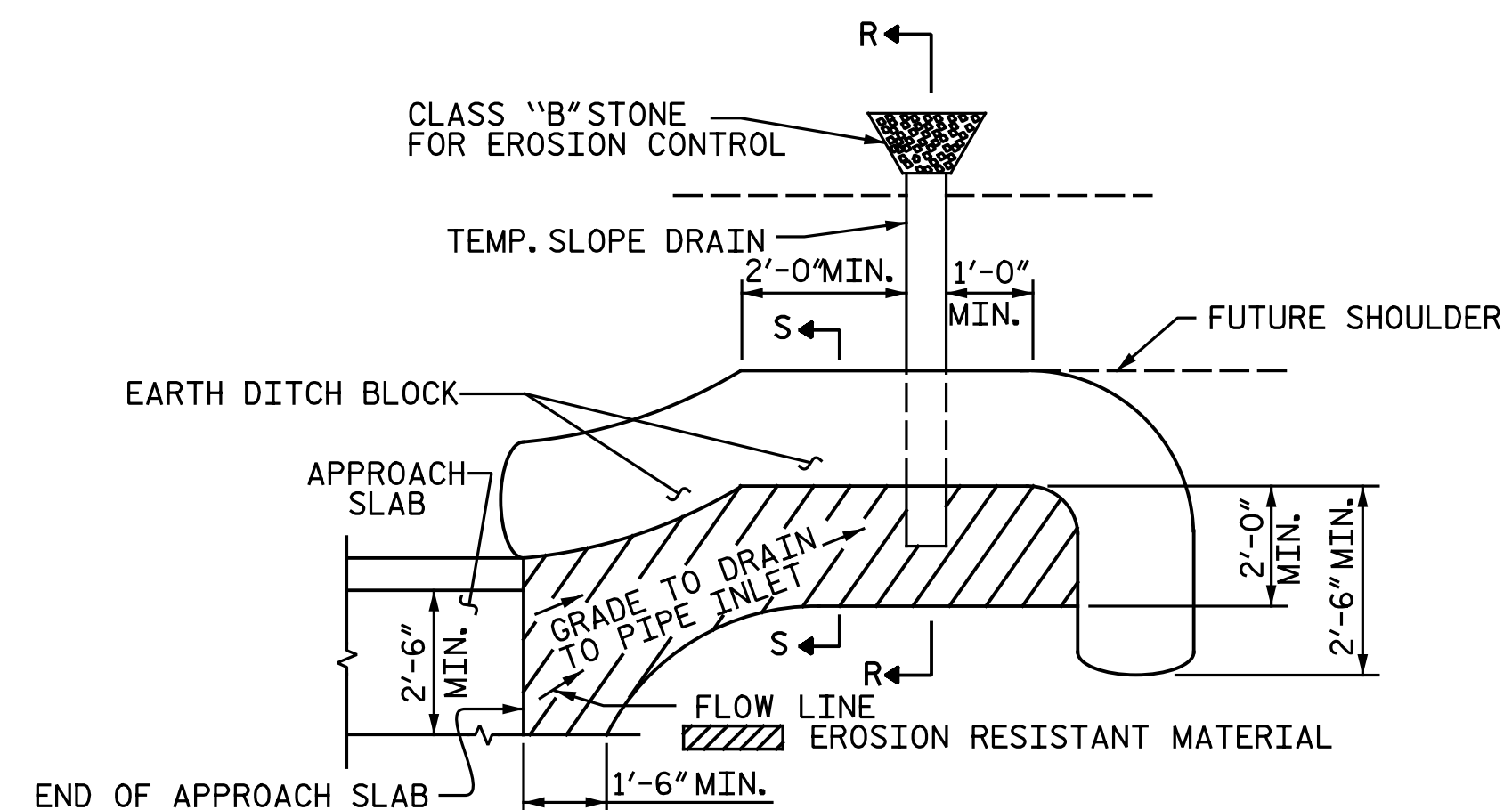
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PROJECT NO. R-5703
LENOIR COUNTY
STATION: 166+72.51 -L-

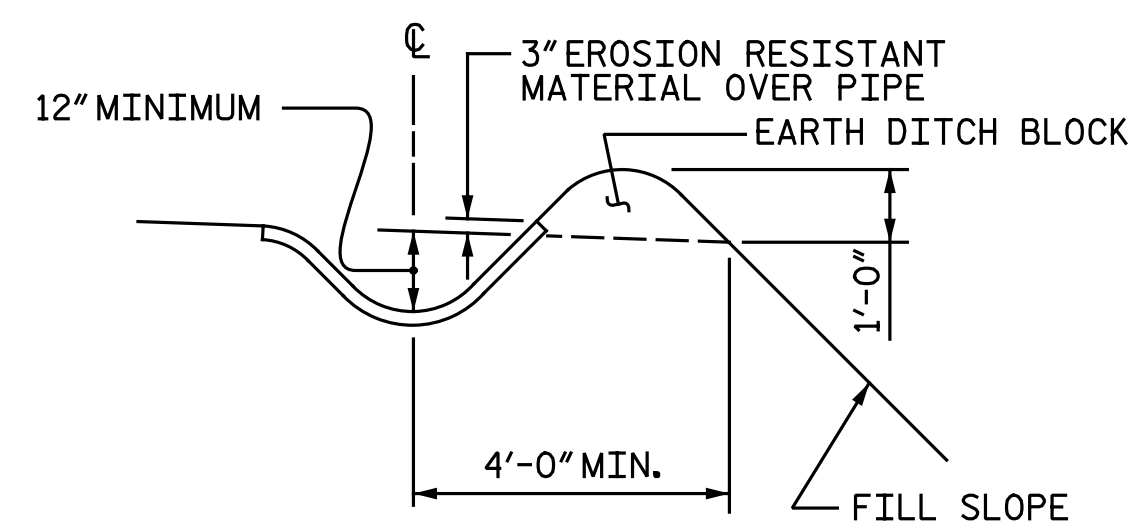
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT					
LEFT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO. S3-33
TOTAL SHEETS 34

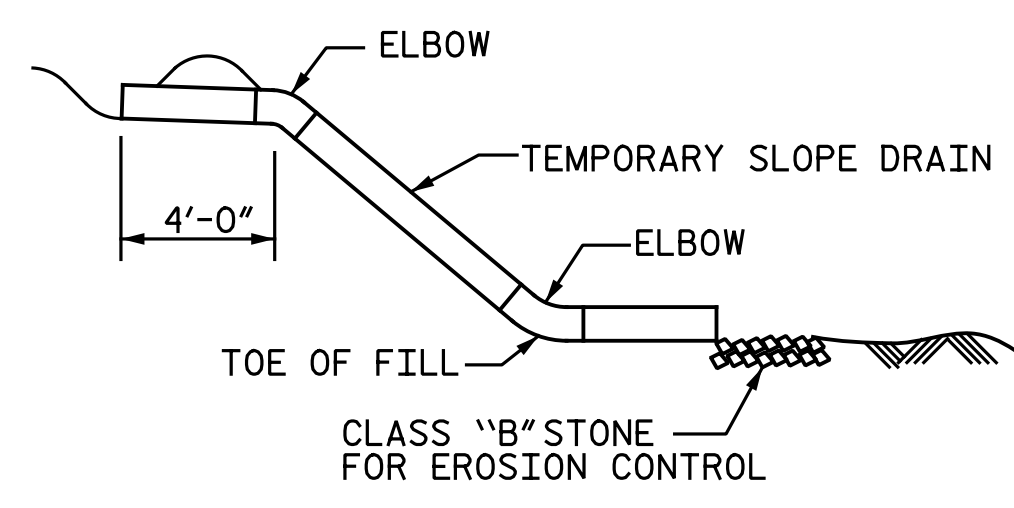


NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

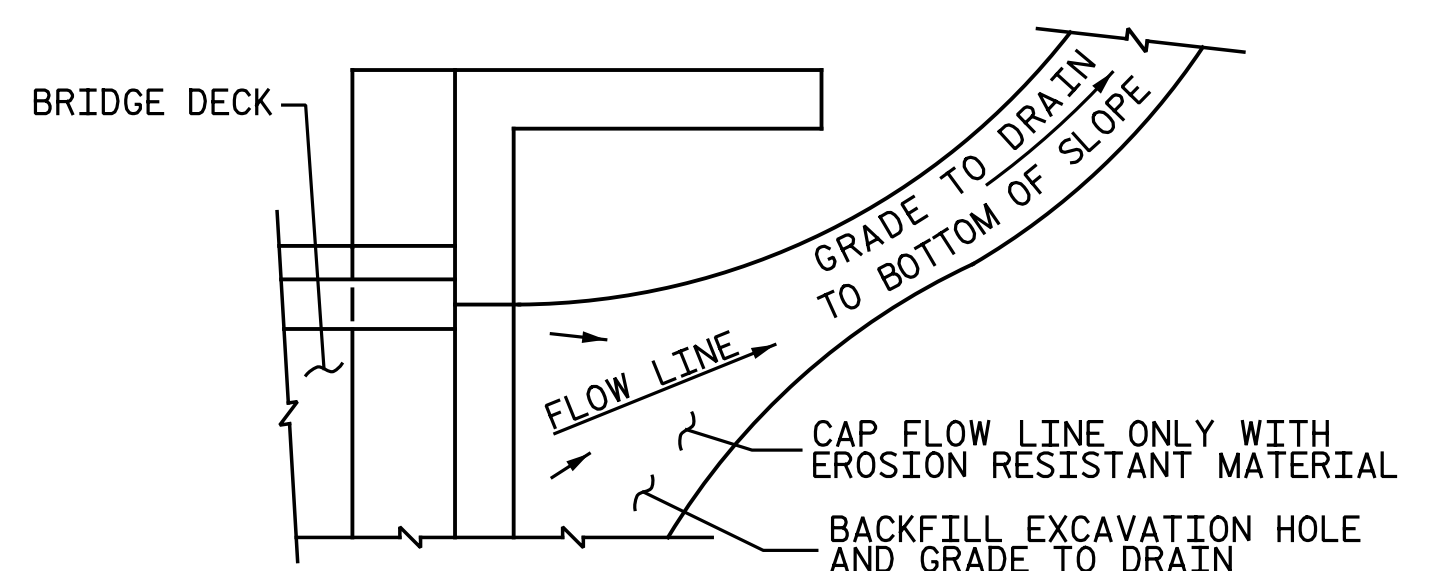
PLAN VIEW



SECTION S-S



SECTION R-R

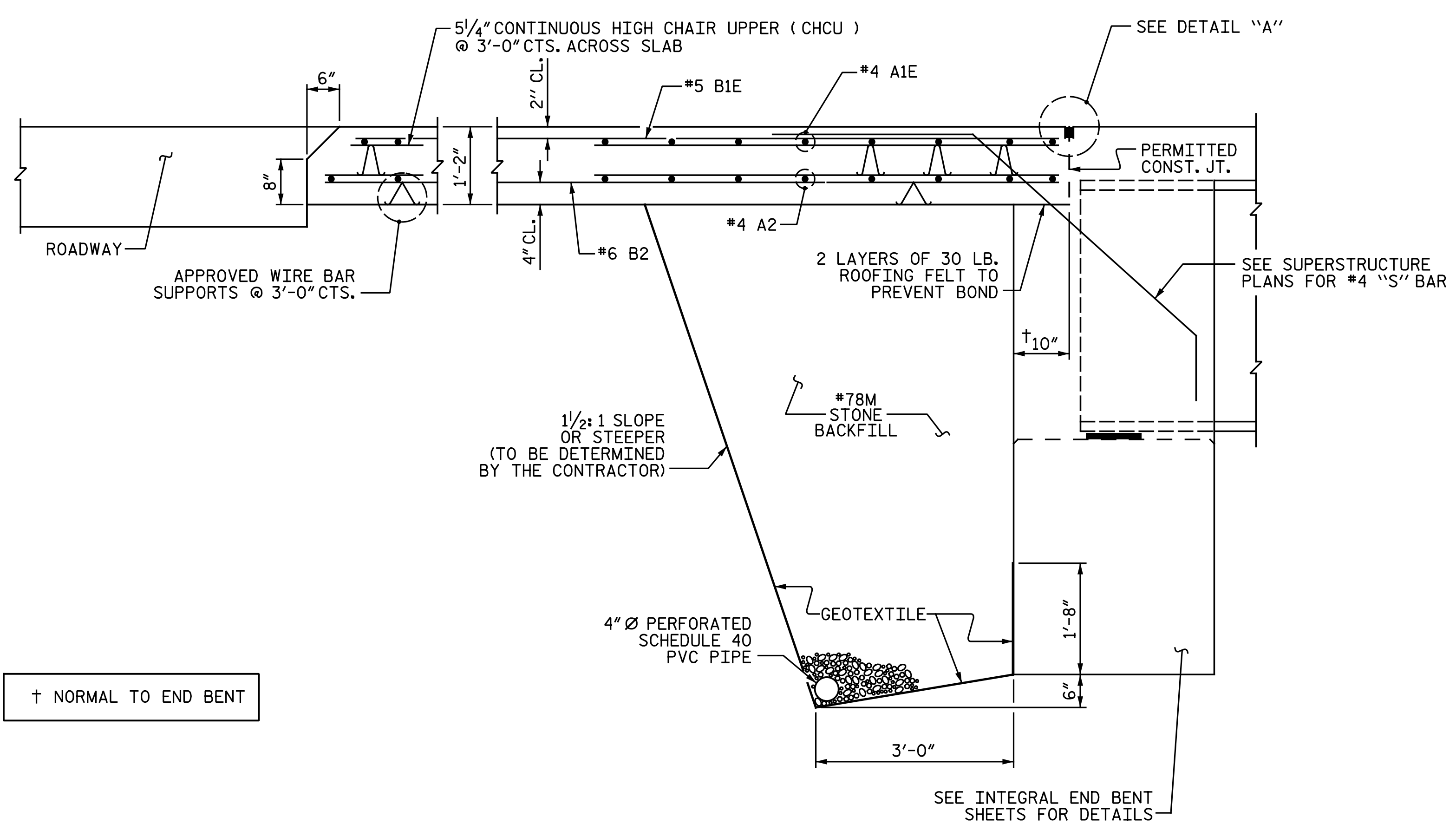


NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

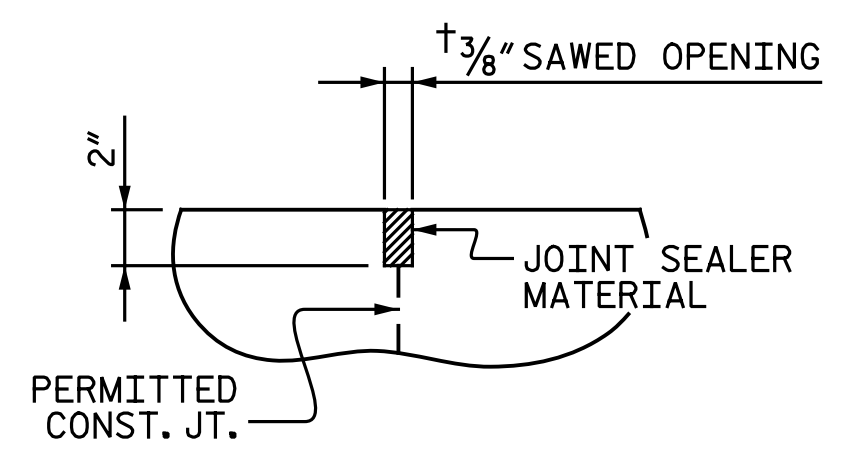
TEMPORARY DRAINAGE DETAIL

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



SECTION THRU SLAB



DETAIL "A"

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 BRIDGE APPROACH SLAB
 DETAILS

LEFT LANE

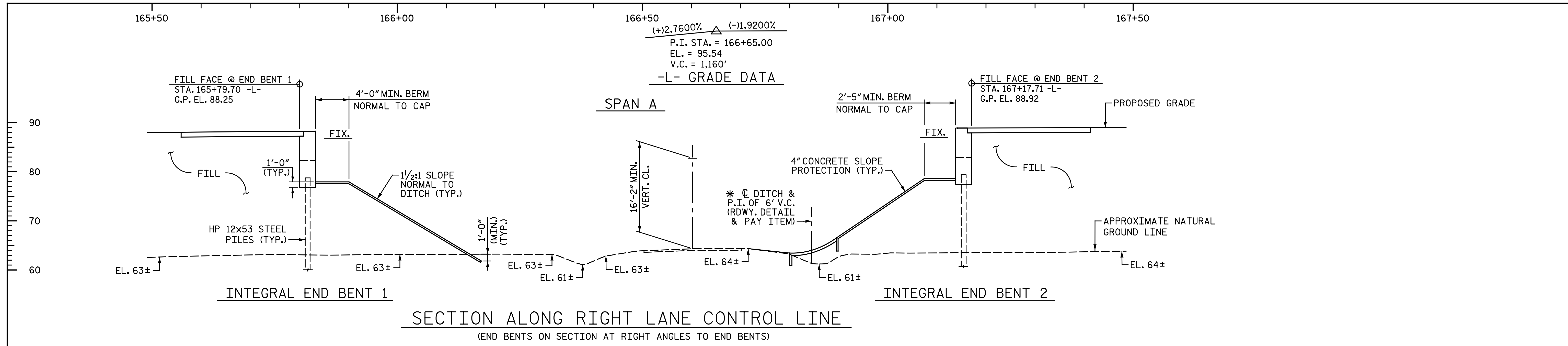
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 NC License No.: F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-34
1			3			TOTAL SHEETS
2			4			34

DRAWN BY: N. B. SPEAKS DATE: 3-6-17
 CHECKED BY: D. A. COLETTI DATE: 6-8-17

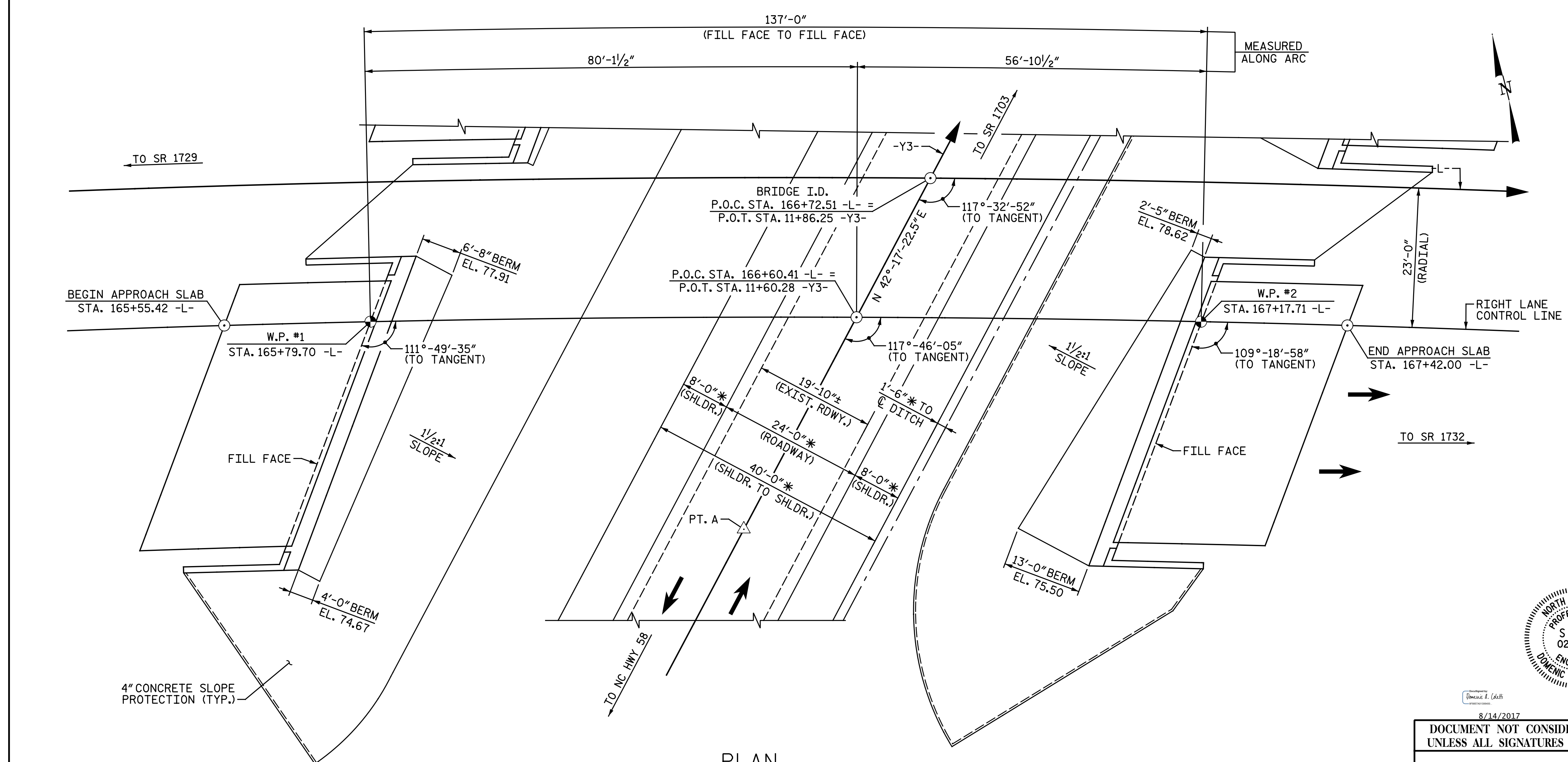
† NORMAL TO END BENT



-L- HORIZONTAL CURVE DATA
 P. I. STA. 158+61.41
 $\Delta = 38^\circ-24'-10.2''$ (RT)
 $D = 1^\circ-49'-08.1''$
 $L = 2,111.31'$
 $T = 1,097.03'$
 $R = 3,150.00'$

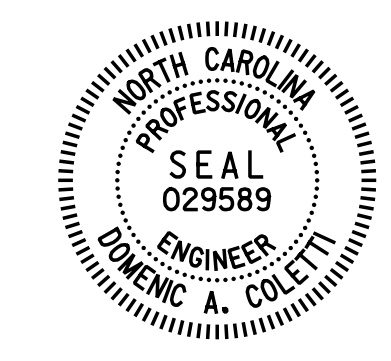
POINT	STATION ON -Y3-	OFFSET	ELEVATION ON -Y3-
A	11+20.71	0.00	64.24

Δ - DENOTES POINT OF MINIMUM VERTICAL CLEARANCE OVER EXISTING ROADWAY WITH 1/2" OVERLAY.
 * - FUTURE CONSTRUCTION



PLAN
 (PILES NOT SHOWN IN PLAN VIEW FOR CLARITY)

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
11+86.25 -Y3-
 SHEET 1 OF 4 BRIDGE NO. 211

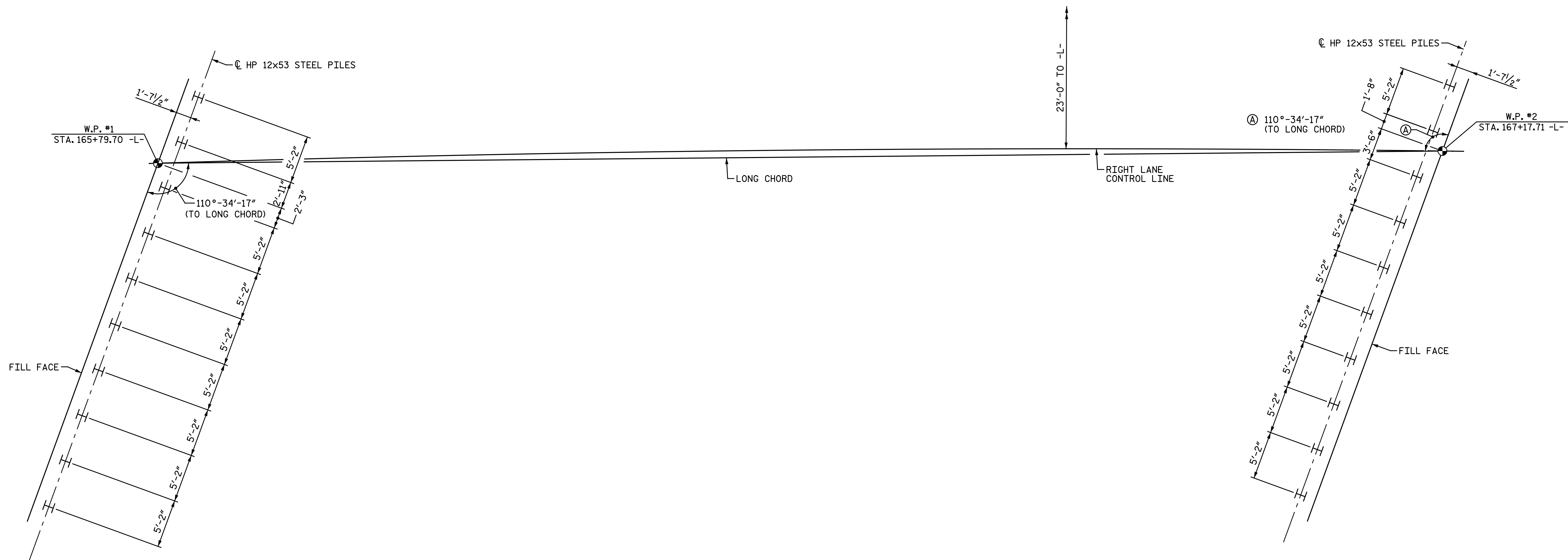


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER SR 1004
 BETWEEN SR 1729 AND SR 1732
 RIGHT LANE

8/14/2017
 DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED

Michael Baker INTERNATIONAL Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 NC License No.: F-1084	REVISIONS				SHEET NO. S4-1 TOTAL SHEETS 31	
	NO.	BY:	DATE:	NO.		BY:
	1			3		
	2			4		

DRAWN BY : C. E. MAYHEW DATE : 4-24-17
 CHECKED BY : D. A. COLETTI DATE : 6-8-17



INTEGRAL
END BENT 1

INTEGRAL
END BENT 2

FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINES.

ALL HP 12x53 STEEL PILES ARE VERTICAL.

NOTES:

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 AND END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 125 TONS PER PILE.

DRIVE PILES AT END BENT NO.1 AND END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 210 TONS PER PILE.

TESTING THE FIRST PRODUCTION PILE WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED AT END BENT NO.1 OR END BENT NO.2. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS AND FOR PILE DRIVING CRITERIA, SEE PILE DRIVING CRITERIA PROVISION.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 60-65 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT NO.1 AND 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

OBSERVE A 2 MONTH WAITING PERIOD AFTER CONSTRUCTING THE EMBANKMENT TO WITHIN 2 FT OF FINISHED GRADE BEFORE BEGINNING END BENT CONSTRUCTION AT END BENT NO.1 AND END BENT NO.2. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SPECIAL PROVISIONS.

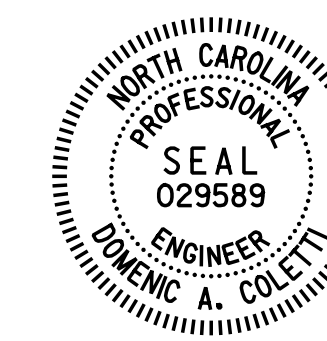
PROJECT NO. R-5703

LENOIR COUNTY

STATION: 166+72.51 -L-

11+86.25 -Y3-

SHEET 2 OF 4



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
GENERAL DRAWING

FOR BRIDGE ON C.F. HARVEY
PARKWAY OVER SR 1004
BETWEEN SR 1729 AND SR 1732
RIGHT LANE

8/14/2017
DOCUMENT NOT CONSIDERED FINAL
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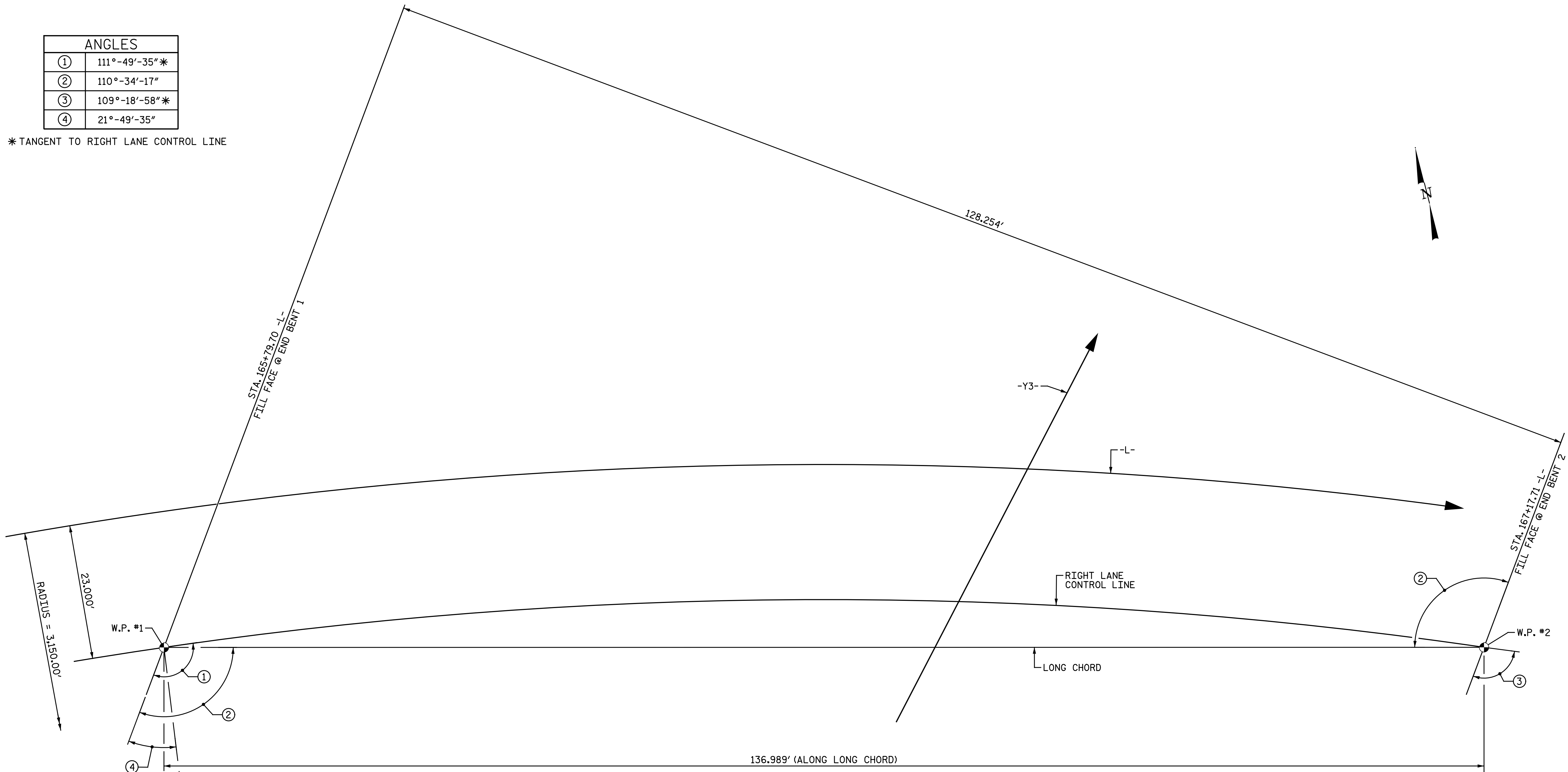
Michael Baker INTERNATIONAL
Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 27518
NC License No.: F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-2
1			3			TOTAL SHEETS
2			4			31

DRAWN BY : M. D. MAYHEW DATE : 5-16-17
CHECKED BY : D. A. COLETTI DATE : 6-8-17

ANGLES	
①	111°-49'-35"*
②	110°-34'-17"
③	109°-18'-58"*
④	21°-49'-35"

* TANGENT TO RIGHT LANE CONTROL LINE



LONG CHORD LAYOUT
(END BENTS ARE PARALLEL)

-L- HORIZONTAL CURVE DATA

P. I. STA. 158+61.41
 $\Delta = 38^\circ-24'-10.2''$ (RT)
 D = $1^\circ-49'-08.1''$
 L = 2,111.31'
 T = 1,097.03'
 R = 3,150.00'

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
11+86.25 -Y3-
 SHEET 3 OF 4

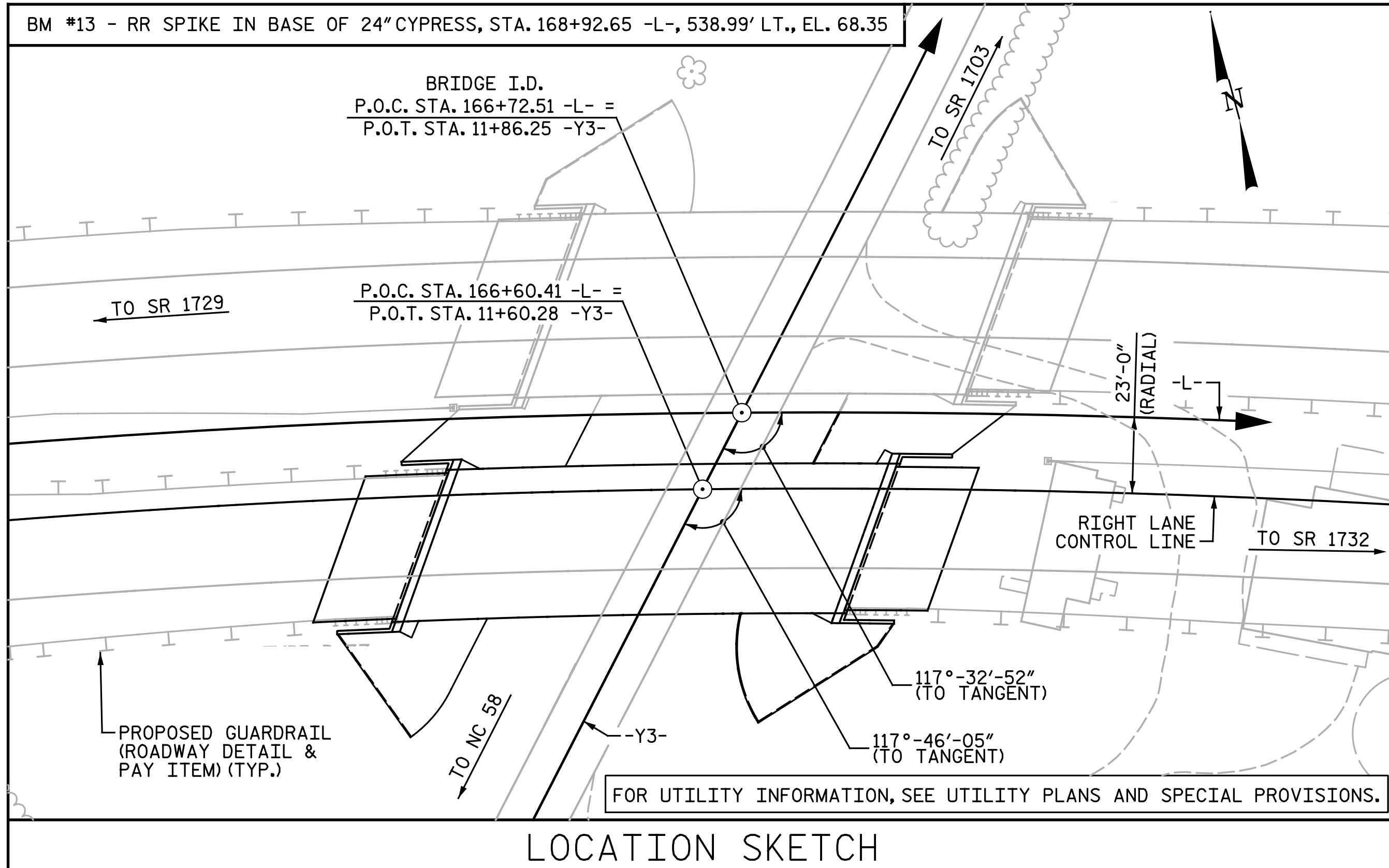


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER SR 1004
 BETWEEN SR 1729 AND SR 1732
 RIGHT LANE

8/14/2017
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 UNLESS ALL SIGNATURES COMPLETED
Michael Baker International
 Michael Baker Engineering
 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No.: F-1084

DRAWN BY : C. E. MAYHEW DATE : 4-24-17
 CHECKED BY : D. A. COLETTI DATE : 6-8-17

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-3
1			3			TOTAL SHEETS
2			4			31



LOCATION SKETCH

NOTES:

- ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
- THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- THE ELEVATION AND CLEARANCE SHOWN ON THE PLANS AT THE POINT OF MINIMUM VERTICAL CLEARANCE ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE ELEVATION ON THE EXISTING PAVEMENT AND CHECK THE CLEARANCE. REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.
- FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.
- REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
- ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH SYSTEM 4 OF ARTICLE 442-8 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.
- FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

TOTAL BILL OF MATERIAL

LOCATION	PDA TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	APPROX. 240,600 LBS. STRUCTURAL STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES	HP 12x53 STEEL PILES		PILE REDRIVES	CONCRETE BARRIER RAIL	4" SLOPE PROTECTION	ELASTOMERIC BEARINGS
	EA.	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	LUMP SUM	EA.	NO.	LIN. FT.	EA.	LIN. FT.	SQ. YDS.	LUMP SUM
SUPERSTRUCTURE		6,204	7,159				LUMP SUM					270.63		LUMP SUM
END BENT 1				52.70		9,269		10	10	850	5		470	
END BENT 2				52.10		9,226		10	10	850	5		400	
TOTAL	1	6,204	7,159	104.80	LUMP SUM	18,495	LUMP SUM	20	20	1,700	10	270.63	870	LUMP SUM

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
11+86.25 -Y3-
 SHEET 4 OF 4



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-4
1			3			TOTAL SHEETS
2			4			31

DRAWN BY: C.E.M. / M.D.M. DATE: 4-24-17
 CHECKED BY: D. A. COLETTI DATE: 6-8-17

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ_{DC}	γ_{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR STEEL GIRDERS																								
LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING #	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE II LIMIT STATE						COMMENT NUMBER		
						LIVE-LOAD FACTORS (γ_{LL})	MOMENT					SHEAR					LIVE-LOAD FACTORS (γ_{LL})	MOMENT						
							DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)		DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION		DISTANCE FROM LEFT END OF SPAN (ft)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	1	1.16	--	1.75	0.83	1.27	A	1	66.81	1.00	1.16	A	3	26.72	1.30	0.83	1.36	A	1	66.81	1, 2	
	HL-93 (OPERATING)	N/A		1.50	--	1.35	0.83	1.64	A	1	66.81	1.00	1.50	A	3	26.72	1.00	0.83	1.76	A	1	66.81	1, 2	
	HS-20 (INVENTORY)	36.00	2	1.61	57.89	1.75	0.83	1.90	A	1	66.81	1.00	1.61	A	3	26.72	1.30	0.83	2.04	A	1	66.81	1, 2	
	HS-20 (OPERATING)	36.00		2.08	75.04	1.35	0.83	2.47	A	1	66.81	1.00	2.08	A	3	26.72	1.00	0.83	2.65	A	1	66.81	1, 2	
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500		5.01	67.64	1.40	0.83	5.84	A	1	66.81	1.00	5.02	A	3	26.72	1.30	0.83	5.01	A	1	66.81	1, 2
		SNGARBS2	20.000		3.49	69.80	1.40	0.83	4.15	A	1	66.81	1.00	3.49	A	3	26.72	1.30	0.83	3.56	A	1	66.81	1, 2
		SNAGRIS2	22.000		3.22	70.84	1.40	0.83	3.85	A	1	66.81	1.00	3.22	A	3	26.72	1.30	0.83	3.30	A	1	66.81	1, 2
		SNCOTTS3	27.250		2.49	67.85	1.40	0.83	2.89	A	1	66.81	1.00	2.50	A	3	26.72	1.30	0.83	2.49	A	1	66.81	1, 2
		SNAGGRS4	34.925		2.01	70.20	1.40	0.83	2.35	A	1	66.81	1.00	2.02	A	3	26.72	1.30	0.83	2.01	A	1	66.81	1, 2
		SNS5A	35.550		1.97	70.03	1.40	0.83	2.30	A	1	66.81	1.00	2.02	A	3	26.72	1.30	0.83	1.97	A	1	66.81	1, 2
		SNS6A	39.950		1.78	71.11	1.40	0.83	2.08	A	1	66.81	1.00	1.82	A	3	26.72	1.30	0.83	1.78	A	1	66.81	1, 2
		SNS7B	42.000		1.69	70.98	1.40	0.83	1.98	A	1	66.81	1.00	1.77	A	3	26.72	1.30	0.83	1.69	A	1	66.81	1, 2
	TRUCK TRACTOR SEMI-TRAILER (T/S)	TNAGRIT3	33.000		2.16	71.28	1.40	0.83	2.52	A	1	66.81	1.00	2.19	A	3	26.72	1.30	0.83	2.16	A	1	66.81	1, 2
		TNT4A	33.075		2.15	71.11	1.40	0.83	2.53	A	1	66.81	1.00	2.15	A	3	26.72	1.30	0.83	2.17	A	1	66.81	1, 2
		TNT6A	41.600		1.75	72.80	1.40	0.83	2.04	A	1	66.81	1.00	1.84	A	3	26.72	1.30	0.83	1.75	A	1	66.81	1, 2
		TNT7A	42.000		1.74	73.08	1.40	0.83	2.03	A	1	66.81	1.00	1.81	A	3	26.72	1.30	0.83	1.74	A	1	66.81	1, 2
		TNT7B	42.000		1.74	73.08	1.40	0.83	2.07	A	1	66.81	1.00	1.74	A	3	26.72	1.30	0.83	1.77	A	1	66.81	1, 2
		TNAGRIT4	43.000		1.69	72.67	1.40	0.83	1.99	A	1	66.81	1.00	1.69	A	3	26.72	1.30	0.83	1.71	A	1	66.81	1, 2
TNAGT5A	45.000		1.62	72.90	1.40	0.83	1.89	A	1	66.81	1.00	1.66	A	3	26.72	1.30	0.83	1.62	A	1	66.81	1, 2		
TNAGT5B	45.000		3	1.61	72.45	1.40	0.83	1.88	A	1	66.81	1.00	1.62	A	3	26.72	1.30	0.83	1.61	A	1	66.81	1, 2	
FATIGUE	HL-93 (INVENTORY)	$\gamma_{LL} = 0.75$																						

NOTES:

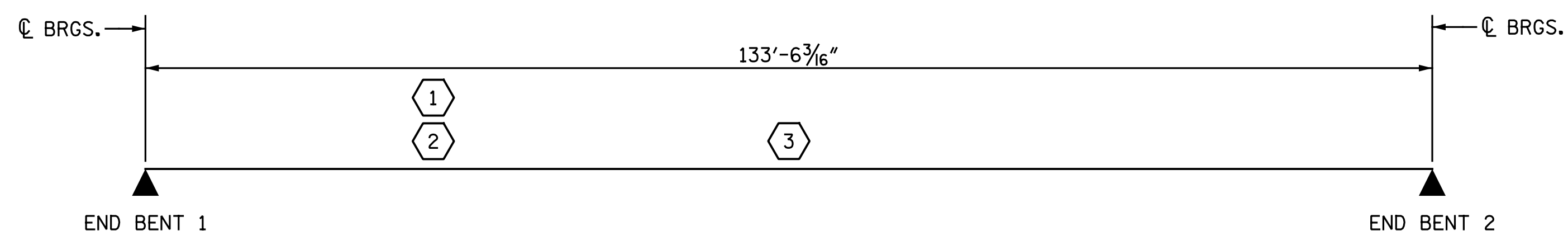
MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE II LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE II LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- DISTANCE FROM LEFT END OF SPAN IS GIVEN WITH RESPECT TO CENTERLINE OF BEARING AND IS MEASURED ALONG THE CONTROLLING GIRDER.
- FATIGUE RATING IS NOT REQUIRED OR REPORTED SINCE GIRDER DOES NOT INCLUDE FATIGUE-PRONE DETAILS.

#	CONTROLLING LOAD RATING
1	DESIGN LOAD RATING (HL-93)
2	DESIGN LOAD RATING (HS-20)
3	LEGAL LOAD RATING **
** SEE CHART FOR VEHICLE TYPE	
GIRDER LOCATION	
GIRDER LOCATION IS PROVIDED USING GIRDER NUMBER, WHERE GIRDER 1 IS THE LEFT EXTERIOR GIRDER LOOKING AHEAD STATION. SEE "GIRDER LAYOUT" SHEET FOR ALL GIRDER LOCATIONS.	



LRFR SUMMARY

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD

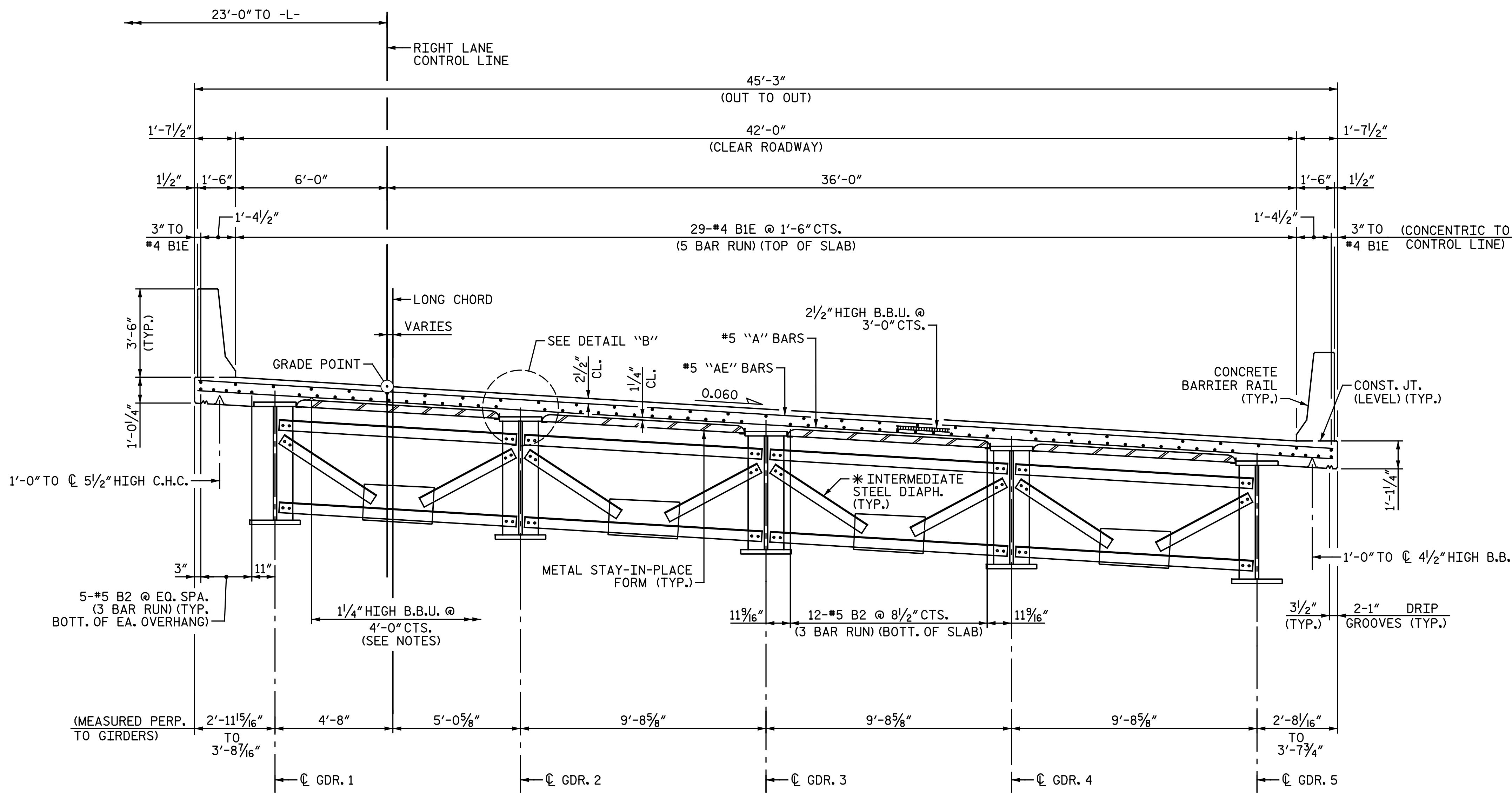
LRFR SUMMARY FOR
 STEEL GIRDERS
 (NON-INTERSTATE TRAFFIC)
 RIGHT LANE

8/14/2017
 DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED

Michael Baker INTERNATIONAL
 Michael Baker Engineering
 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No.: F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-5
1			3			TOTAL SHEETS 31
2			4			

ASSEMBLED BY : C. E. MAYHEW	DATE : 3-10-17
CHECKED BY : D. A. COLETTI	DATE : 6-8-17
DRAWN BY : MAA 1/08	REV. 11/2/08RR MAA/GM
CHECKED BY : GM/DI 2/08	REV. 10/1/11 MAA/GM



TYPICAL SECTION AT INTERMEDIATE DIAPHRAGMS

(ALL HORIZONTAL DIMENSIONS ARE RADIAL U.N.O.)
(GIRDERS ARE PARALLEL TO LONG CHORD)

■ = PARALLEL TO GIRDERS

NOTES:

PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2" ABOVE THE TOP OF THE REMOVABLE FORM.

FOR CONCRETE BARRIER RAIL DETAILS, SEE "CONCRETE BARRIER RAIL" SHEET.

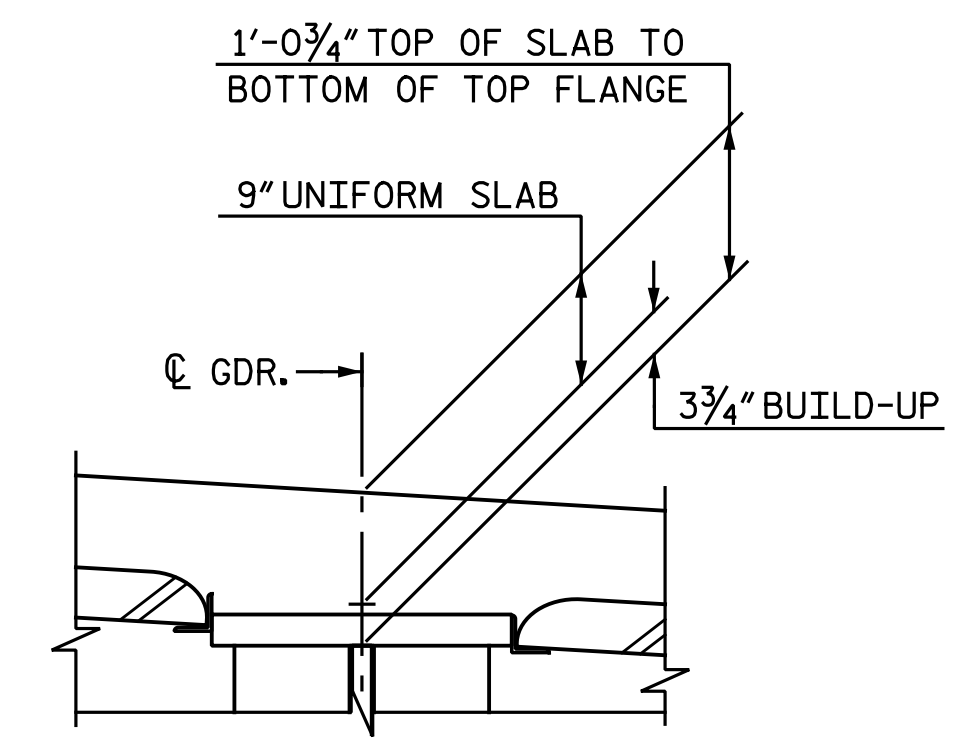
METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO BEAM OR GIRDER FLANGES IN THE ZONES REQUIRING CHARPY V-NOTCH TEST. SEE STRUCTURAL STEEL DETAIL SHEETS.

PREVIOUSLY CAST CONCRETE IN A SPAN SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE SPAN.

THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL STAY-IN-PLACE FORM SUPPORTS OR FORMS AND BEAM/GIRDER STIFFENERS OR CONNECTOR PLATES. THE PROPOSAL SHALL BE INDICATED, AS APPROPRIATE, ON EITHER THE STEEL WORKING DRAWINGS OR THE METAL STAY-IN-PLACE FORM WORKING DRAWINGS.

FOR ADDITIONAL INFORMATION ON DECK SLAB REINFORCING, SEE "PLAN OF SPAN" SHEETS.

* FOR DETAILS OF INTERMEDIATE DIAPHRAGMS, SEE "STRUCTURAL STEEL DETAILS" SHEET 2 OF 2.



DETAIL "B"

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 166+72.51 -L-

SHEET 2 OF 2



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
TYPICAL SECTION

RIGHT LANE

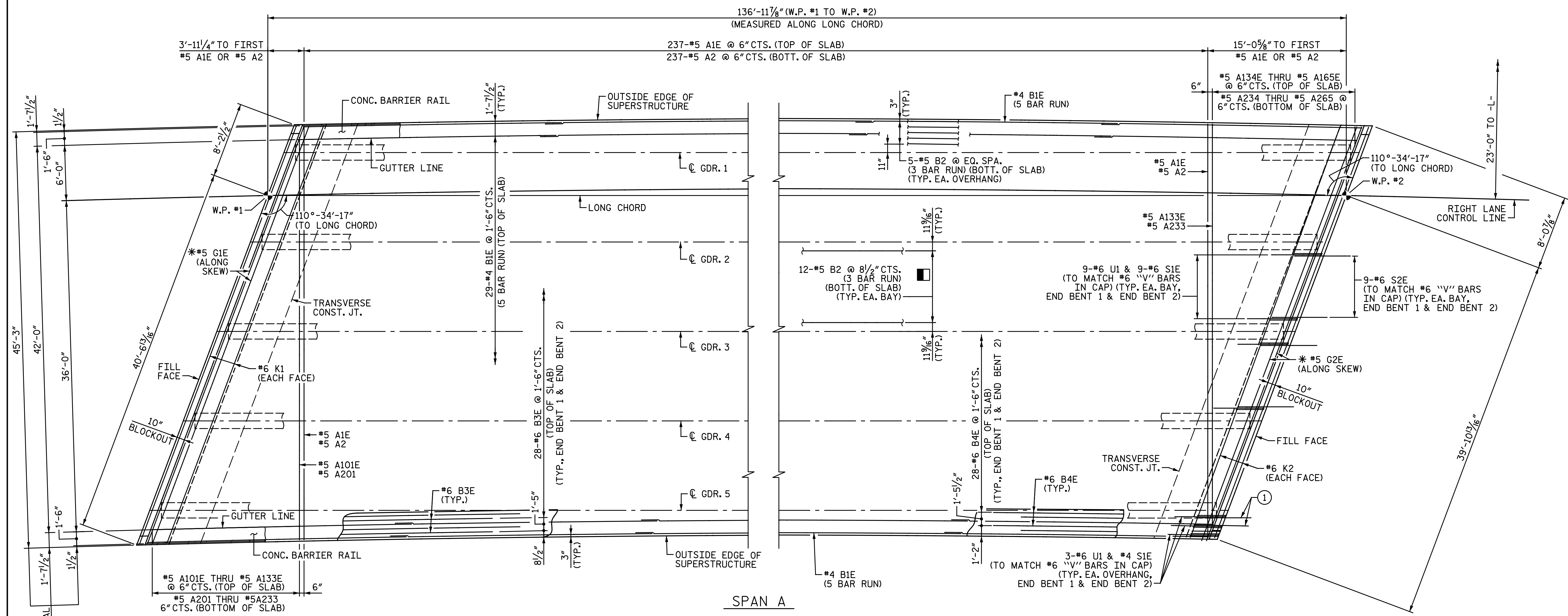
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NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

DRAWN BY: M. D. MAYHEW DATE: 5-16-17
CHECKED BY: D. A. COLETTI DATE: 6-8-17

SHEET NO.
S4-7
TOTAL SHEETS
31

NOTES:
 * #5 "GE" BARS ARE TO BE PLACED PARALLEL TO SKEW AND BELOW "BE" BARS.
 FOR REINFORCING STEEL IN CONCRETE BARRIER RAIL, SEE "CONCRETE BARRIER RAIL" SHEET.
 FOR DECK POURING SEQUENCE AND LOCATION OF TRANSVERSE CONSTRUCTION JOINTS, SEE "BILL OF MATERIAL" SHEET.
 FOR ARC OFFSETS, SEE SHEET 2 OF 2.
 "A" AND "AE" BARS ARE TO BE PLACED PERPENDICULAR TO THE LONG CHORD.

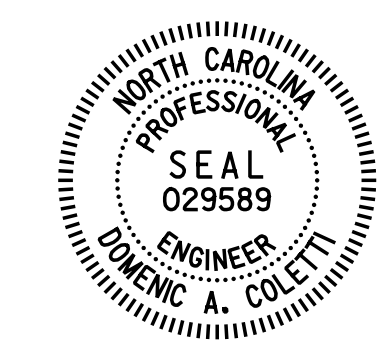


SPAN A
 PLAN OF SPAN

- ① 2-#4 S2E (TO MATCH #6 "V" BARS IN CAP) (TYP. EA. OVERHANG, END BENT 1 & END BENT 2)
- PARALLEL TO GIRDERS

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
 SHEET 1 OF 2

SPLICE LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"
#5	2'-6"	2'-2"
#6	3'-0"	2'-7"

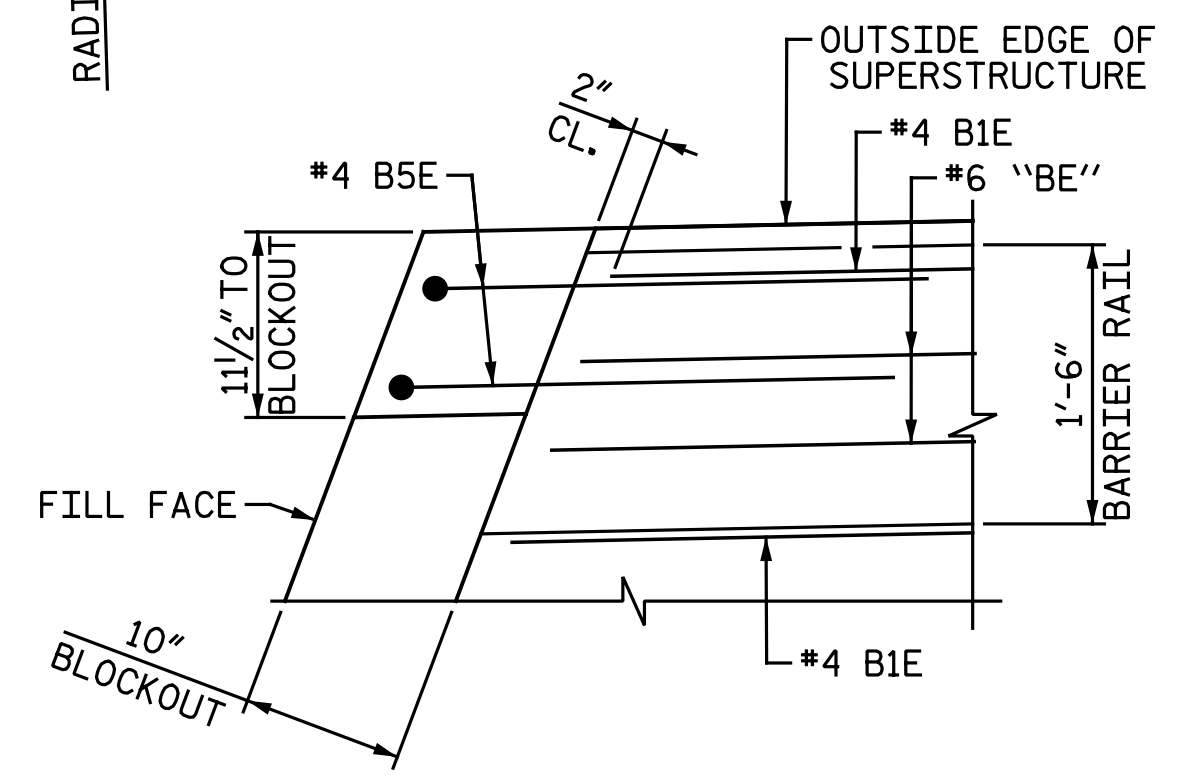


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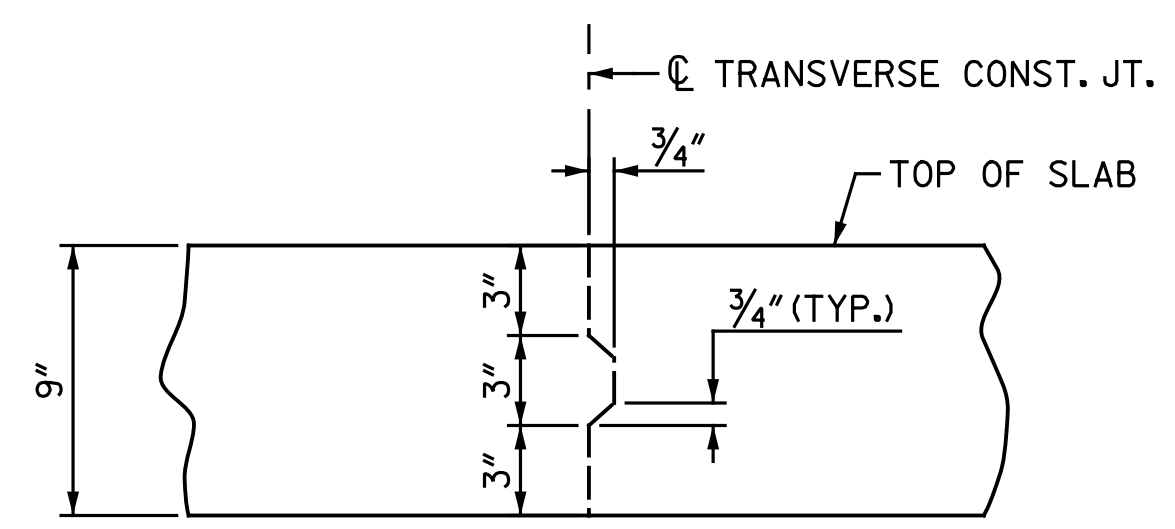
Michael Baker INTERNATIONAL		Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 27518 NC License No.: F-1084			
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 PLAN OF SPAN
 RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-8
1			3			TOTAL SHEETS
2			4			31



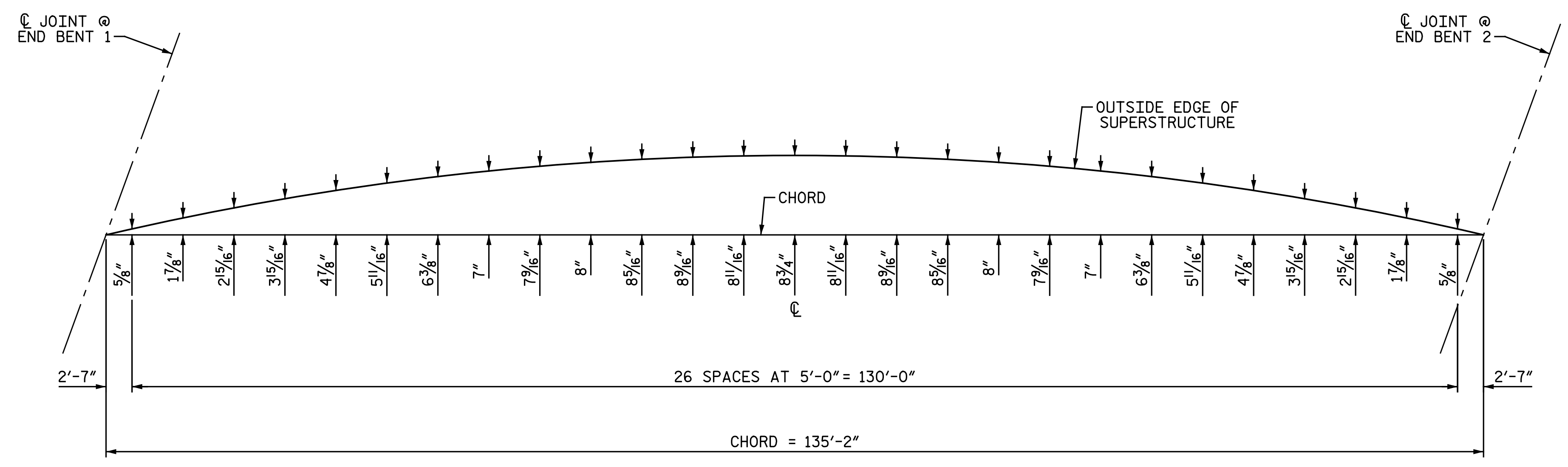
CORNER DETAIL
 ALL CORNERS SIMILAR.
 TRANSVERSE BARS NOT SHOWN FOR CLARITY.



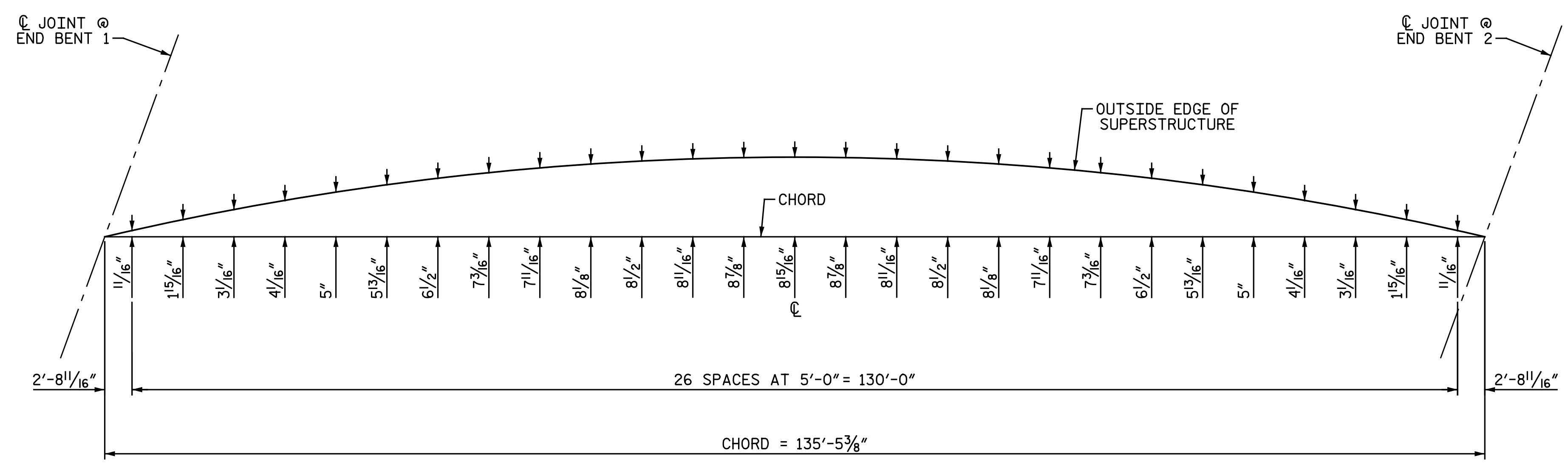
TRANSVERSE CONST. JT. DETAIL
 REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THROUGH JOINT.

DRAWN BY: C. E. MAYHEW DATE: 6-9-17
 CHECKED BY: D. A. COLETTI DATE: 6-8-17

NOTES:
 Ⓢ JOINT INDICATES THE PERMITTED CONSTRUCTION JOINT BETWEEN APPROACH SLAB AND DECK.



LEFT SIDE



RIGHT SIDE

ARC OFFSETS - SPAN A

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 2 OF 2



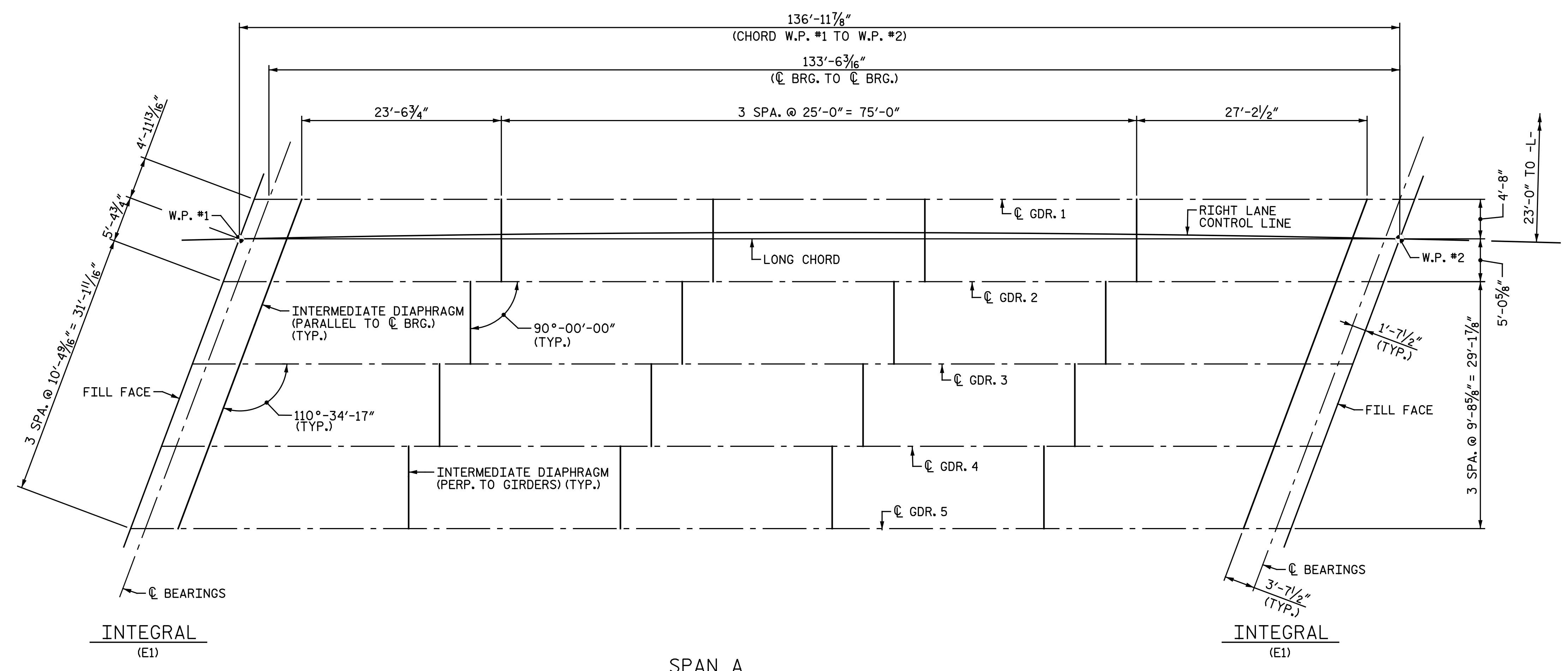
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 PLAN OF SPANS

RIGHT LANE

8/14/2017
 DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED
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 Michael Baker Engineering
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 NC License No.: F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-9
1			3			TOTAL SHEETS
2			4			31

DRAWN BY : D. A. LAMAY DATE : 6-6-17
 CHECKED BY : D. A. COLETTI DATE : 6-8-17



GIRDER LAYOUT
(GIRDERS ARE PARALLEL TO LONG CHORD)

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH SYSTEM 4 OF ARTICLE 442-8 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL, UNLESS OTHERWISE NOTED.

ALL FIELD CONNECTIONS TO BE 7/8" DIA. HIGH STRENGTH BOLTS UNLESS OTHERWISE NOTED. BOLTS SHALL BE DETAILED WITH THREADS EXCLUDED FROM SHEAR PLANES.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB IN THE FINAL CONDITION.

PERMITTED FLANGE AND WEB SHOP SPLICES SHALL NOT BE LOCATED WITHIN 15 FEET OF MAXIMUM DEAD LOAD DEFLECTION KEEP 2 FEET MINIMUM BETWEEN WEB AND FLANGE SHOP SPLICES. KEEP 6" MINIMUM BETWEEN CONNECTOR PLATE OR TRANSVERSE STIFFENER WELDS AND WEB OR FLANGE SHOP SPLICES.

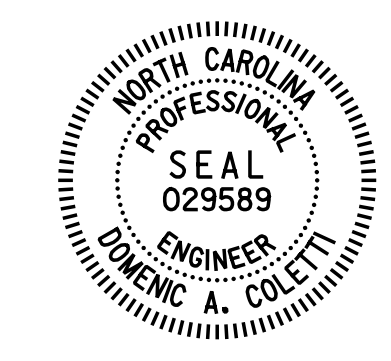
STUDS ON GIRDERS MAY BE SHIFTED UP TO 1" IF NECESSARY TO CLEAR FLANGE SPLICE WELDS.

TENSION ON THE ASTM A325 BOLTS SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH ARTICLE 440-8 OF THE STANDARD SPECIFICATIONS.

ENDS OF GIRDERS SHALL BE PLUMB IN THE FINAL CONDITION.

FABRICATORS SHALL DETAIL DIAPHRAGM MEMBERS AND CONNECTIONS FOR FULL DEAD LOAD FIT UP. GIRDERS SHALL BE PLUMB AFTER THE FULL AMOUNT OF DEAD LOAD IS APPLIED.

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-



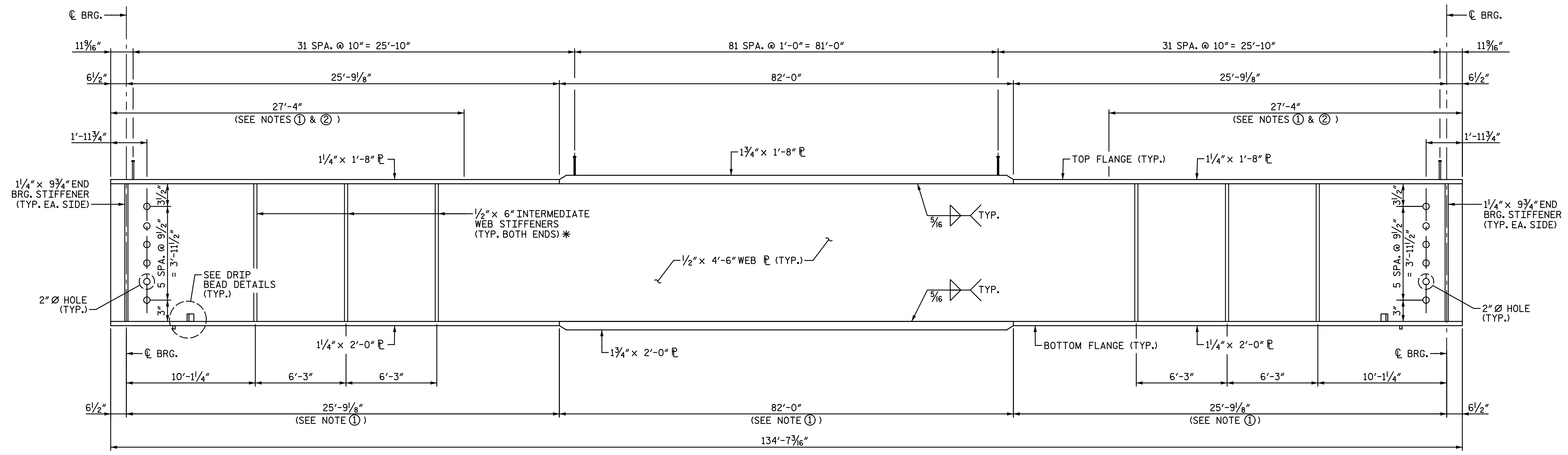
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 FRAMING PLAN

8/14/2017
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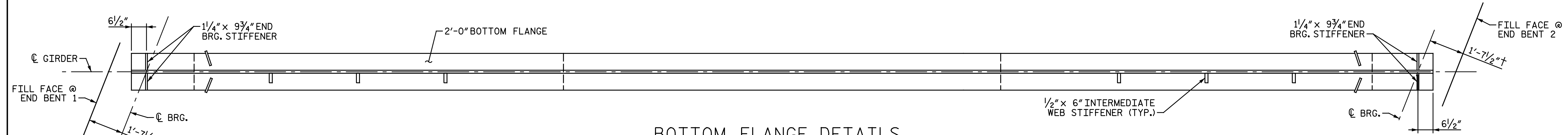
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-10
1			3			TOTAL SHEETS
2			4			31

DRAWN BY : M. D. MAYHEW DATE : 5-17-17
 CHECKED BY : D. A. COLETTI DATE : 6-8-17



* PLACE INTERMEDIATE WEB STIFFENERS ON ONLY ONE FACE OF THE WEB (INTERIOR FACE FOR EXTERIOR GIRDERS).

GIRDER ELEVATION



BOTTOM FLANGE DETAILS

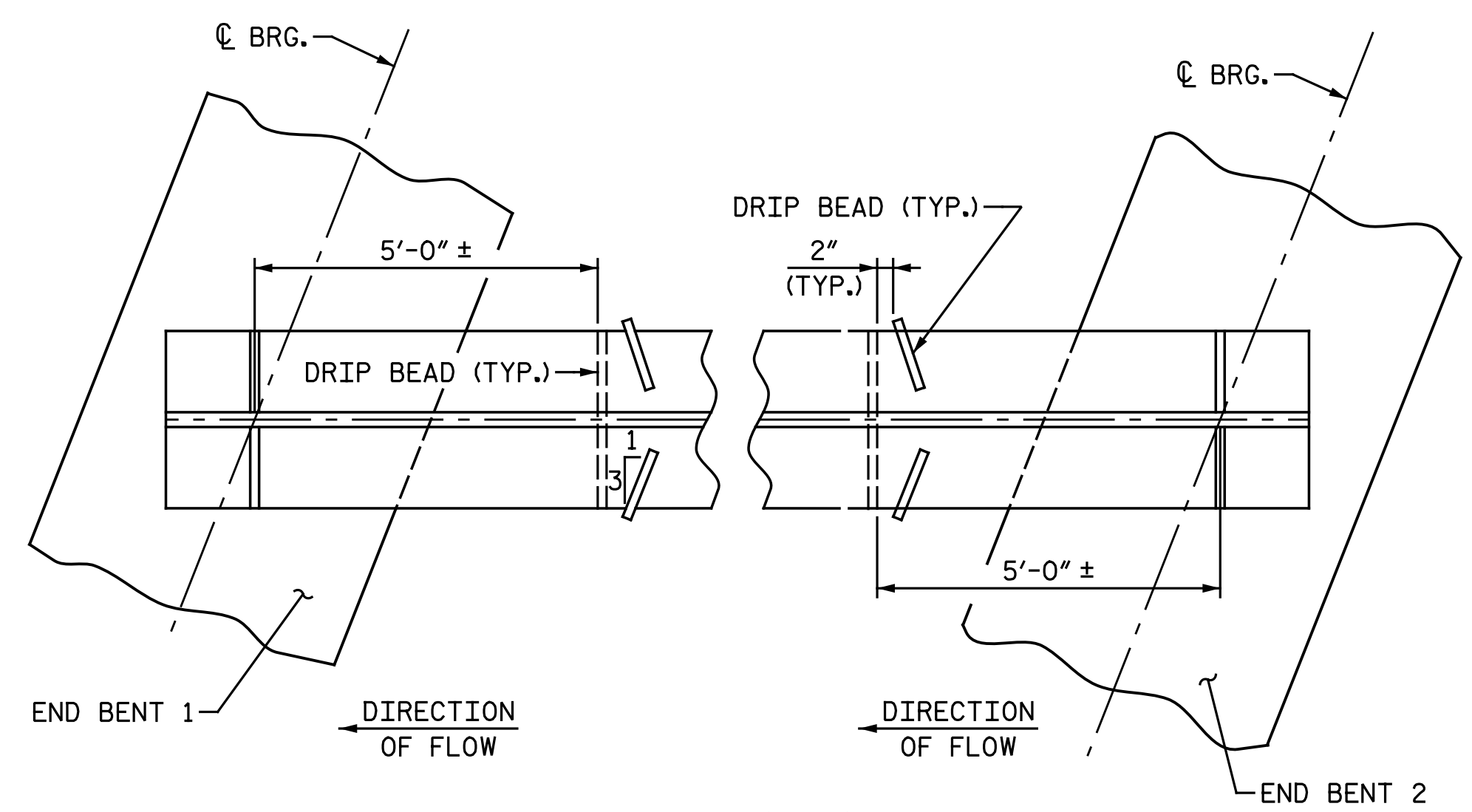
† MEASURED PERPENDICULAR TO END BENT FILL FACE

NOTES:

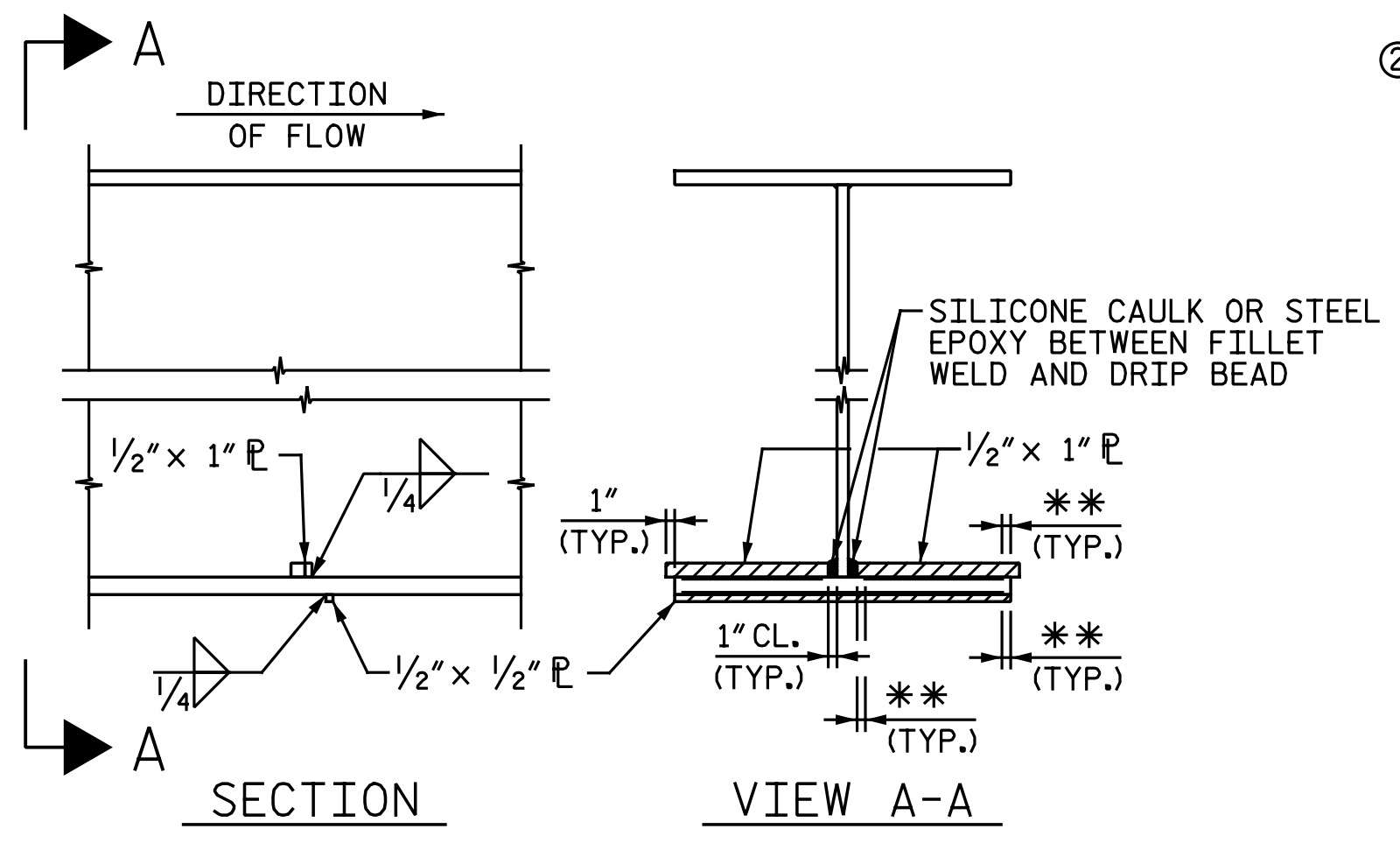
- ① CHARPY V-NOTCH TESTS ARE REQUIRED FOR ALL TOP OR BOTTOM FLANGE PLATES WHICH FALL WITHIN THESE LIMITS, ALL WEB PLATES, AND ALL SPLICE PLATES. IF A PERMITTED SHOP FLANGE SPLICE IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE FLANGE PLATE. FOR CHARPY V-NOTCH TESTS, SEE ARTICLE 1072-7 OF THE STANDARD SPECIFICATIONS.
 - ② NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.
- FOR SHEAR CONNECTOR TRANSVERSE SPACING, SEE "STRUCTURAL STEEL DETAILS", SHEET 2 OF 2.
- FOR STRUCTURAL STEEL NOTES, SEE "FRAMING PLAN" SHEET.

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 1 OF 2



PART PLAN - BOTTOM FLANGE



DRIP BEAD DETAILS

** SEE "WELD TERMINATION DETAILS" ON "STRUCTURAL STEEL DETAILS", SHEET 2 OF 3.



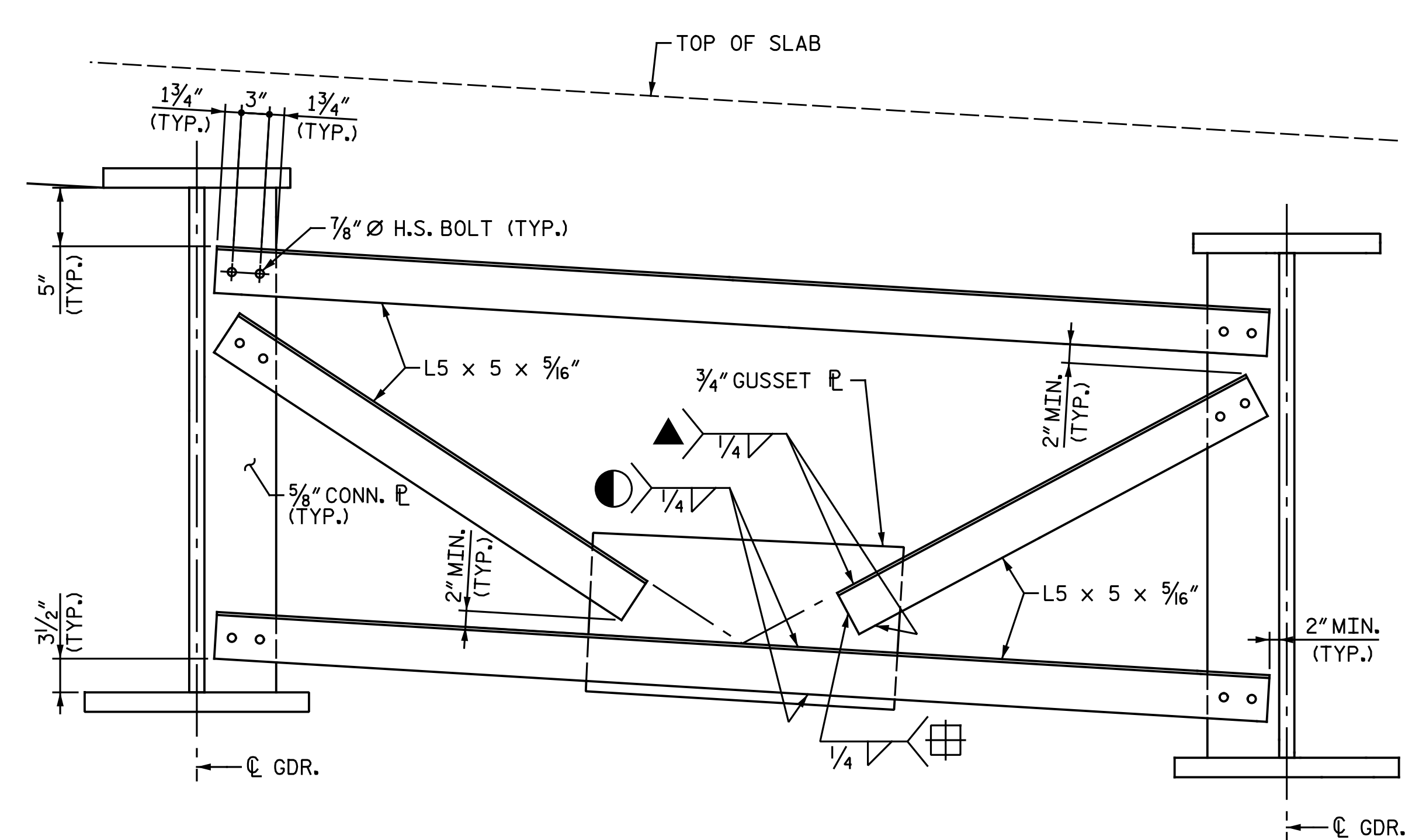
8/14/2017
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Michael Baker INTERNATIONAL

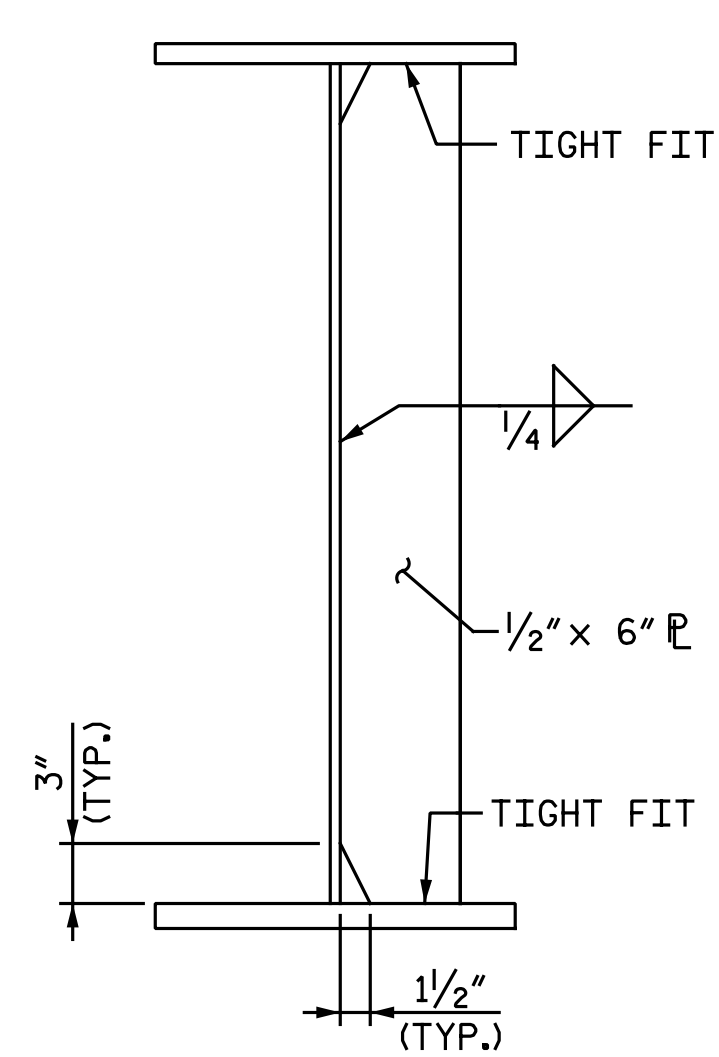
Michael Baker Engineering
 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No.: F-1084

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE					
STRUCTURAL STEEL DETAILS					
RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO. S4-II					TOTAL SHEETS 31

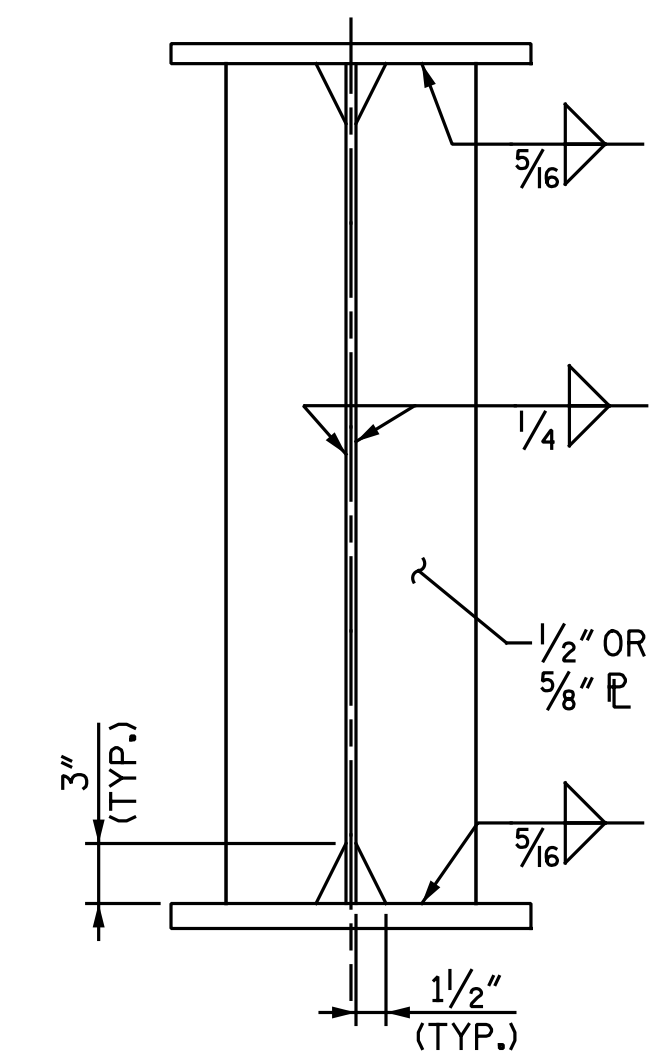
DRAWN BY: C. E. MAYHEW DATE: 3-10-17
 CHECKED BY: D. A. COLETTI DATE: 6-8-17



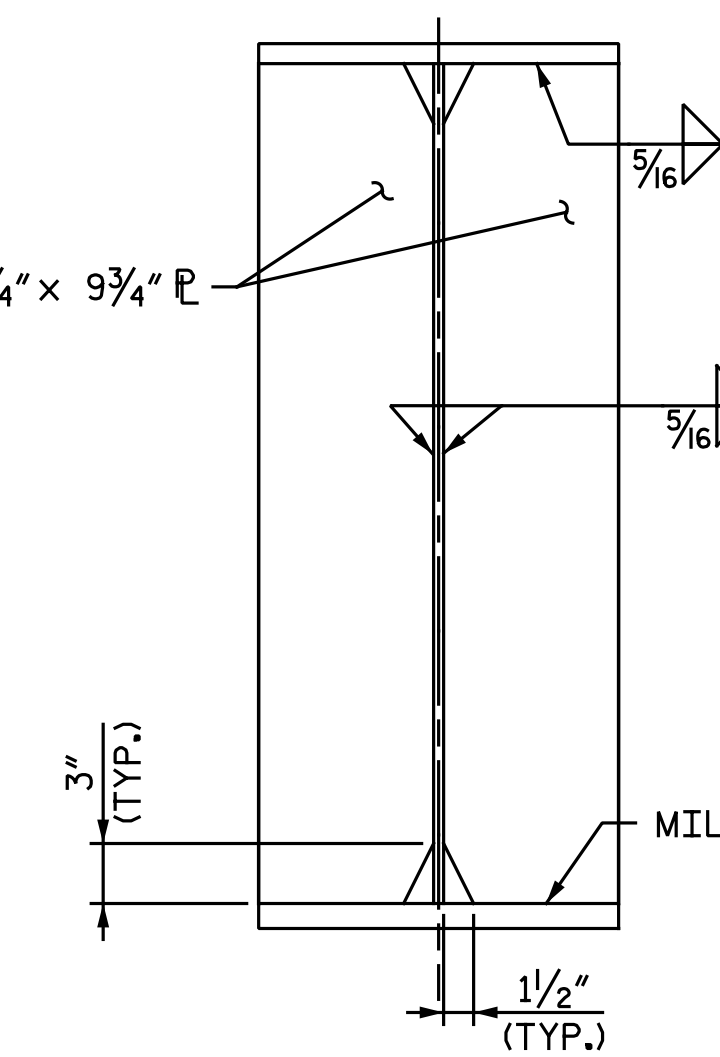
INTERMEDIATE DIAPHRAGM



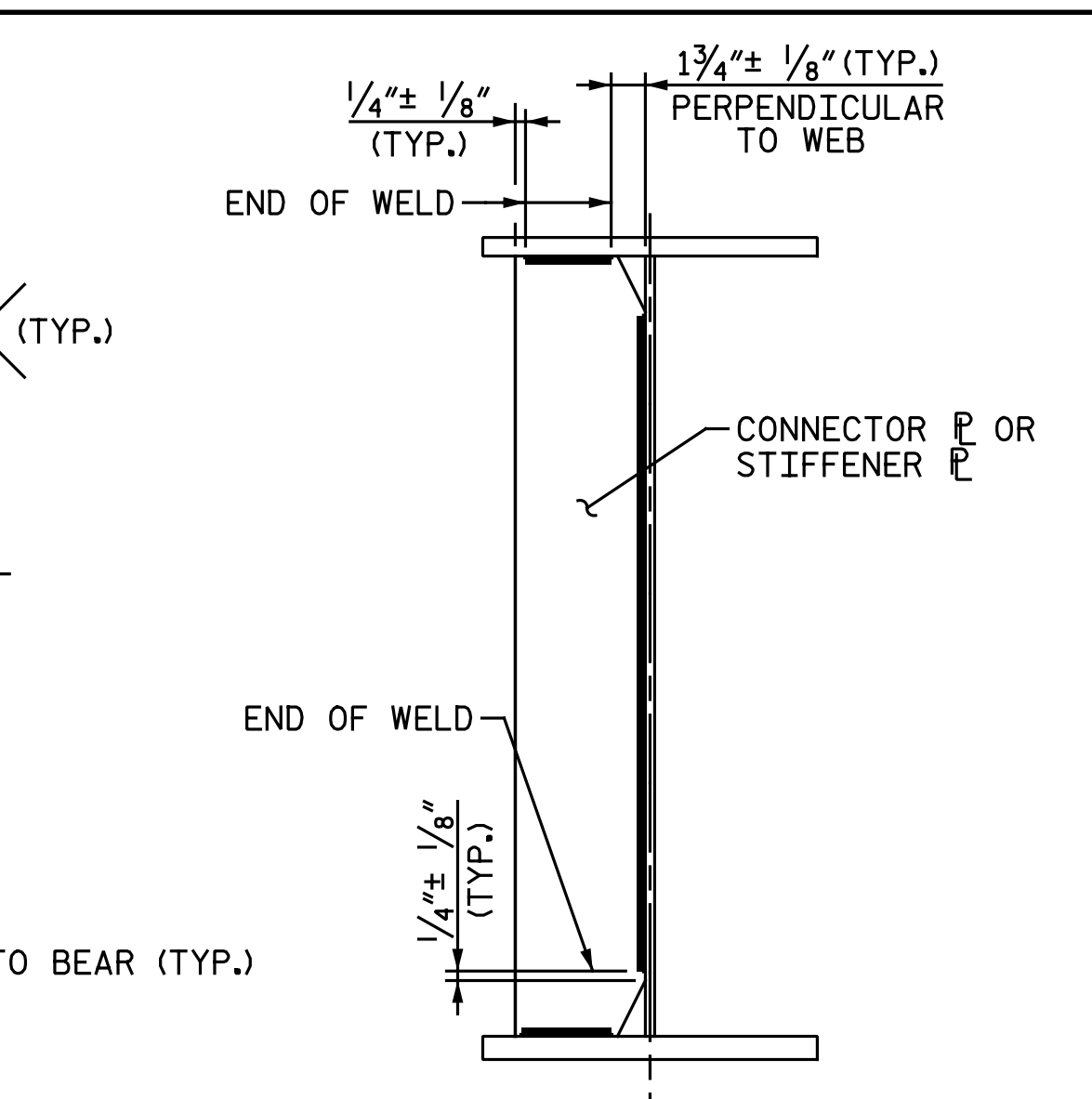
INTERMEDIATE STIFFENER



INTERMEDIATE DIAPHRAGM CONNECTOR

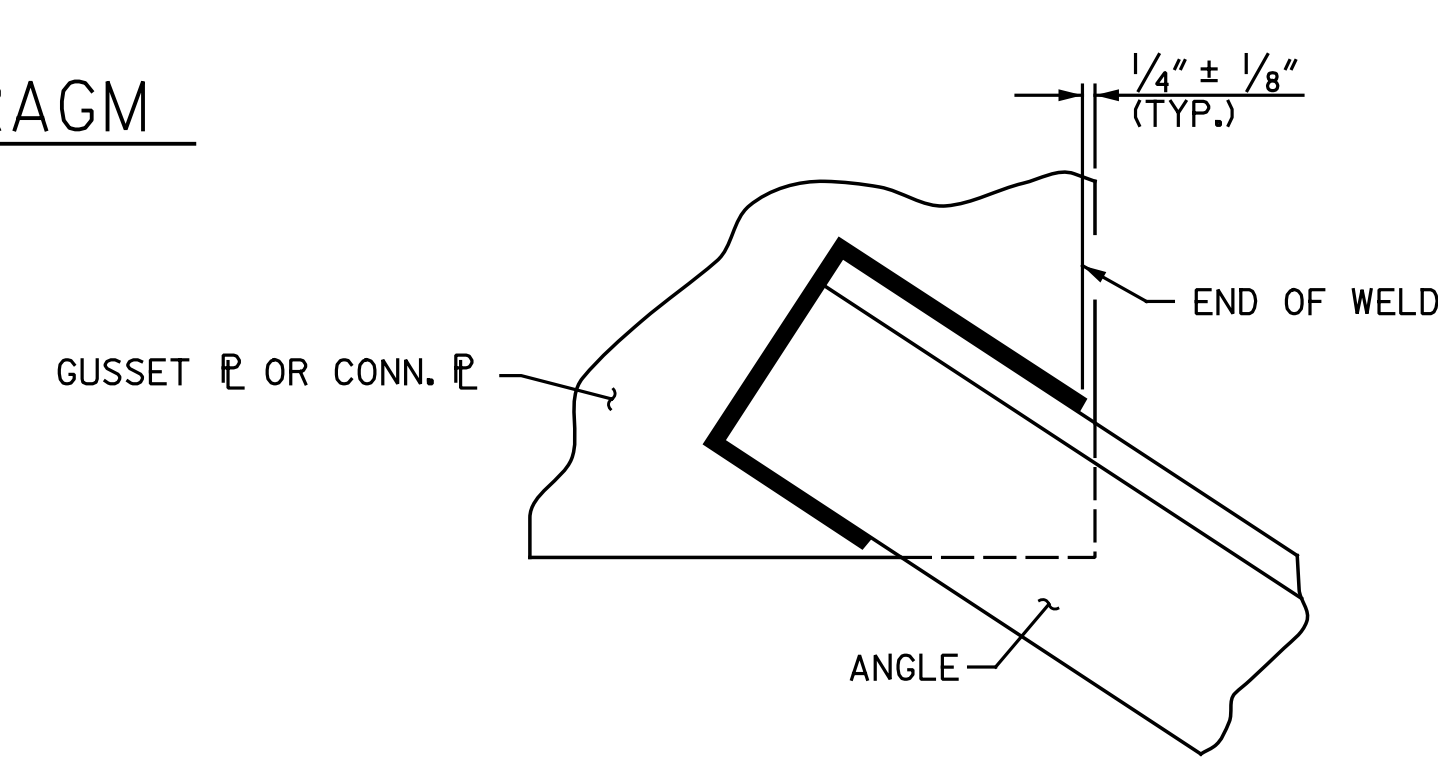


BEARING STIFFENER

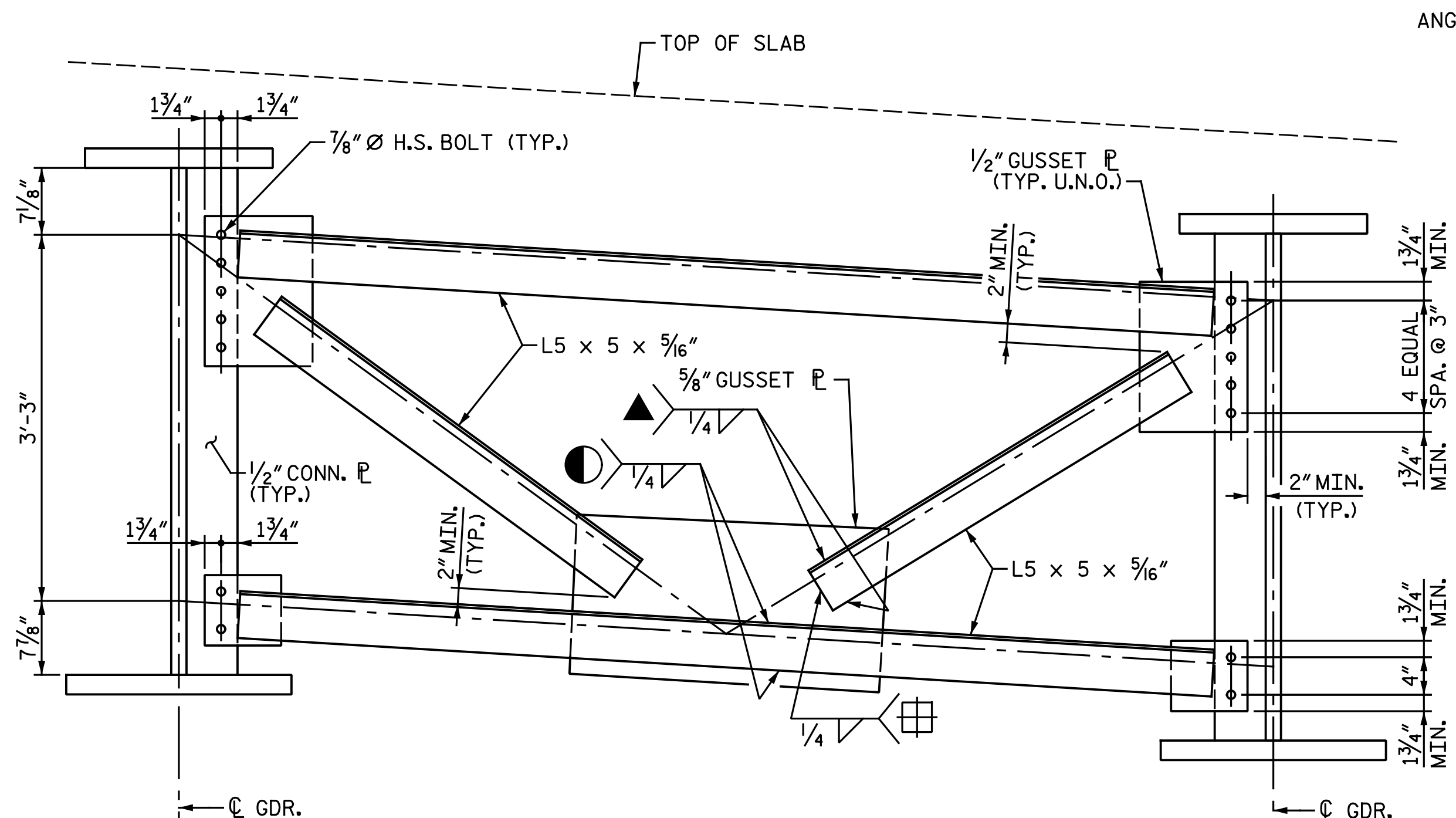


STIFFENER OR CONNECTOR PLATE CONNECTIONS WELD TERMINATION DETAILS

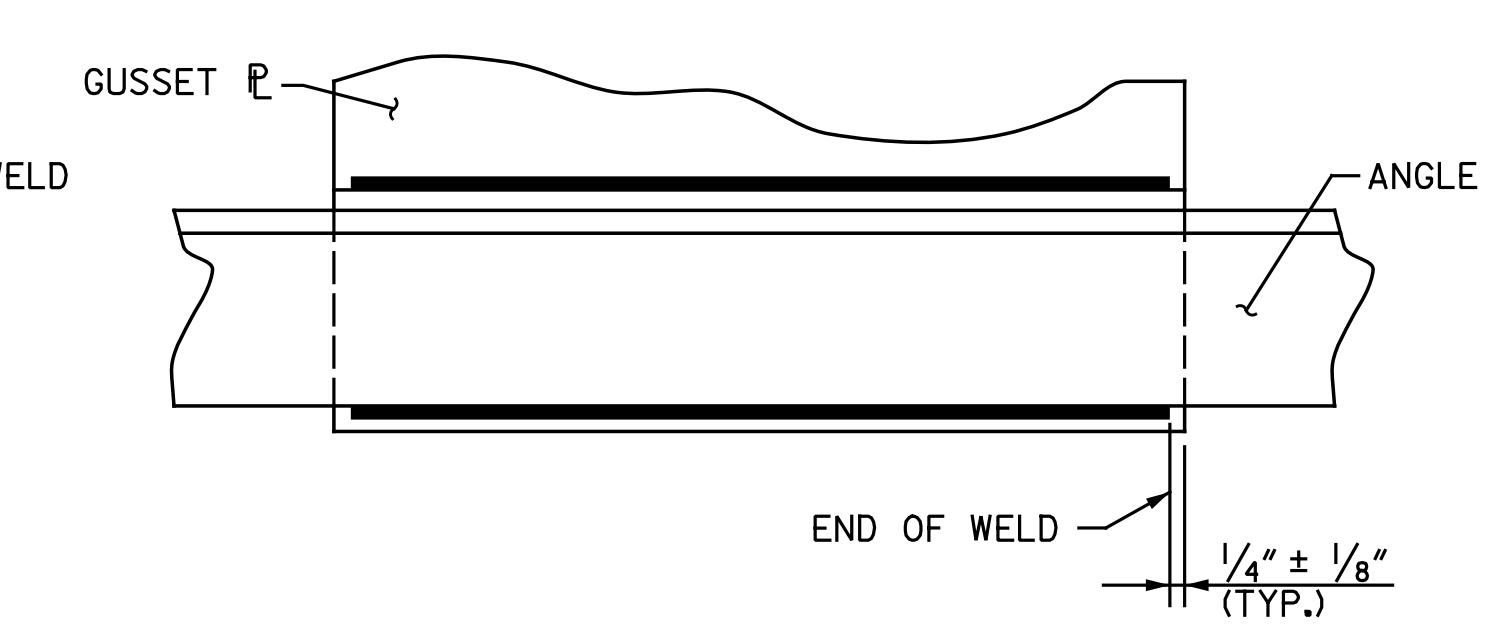
- ▲ 4" MIN. LENGTH EACH SIDE OF ANGLE, AT EACH ENDED WELDED END OF ANGLE
- ⊞ FULL WIDTH ACROSS END OF ANGLE, AT EACH WELDED END OF ANGLE
- 28" MIN. LENGTH EACH SIDE OF ANGLE, AT BOTTOM CHORD MID-LENGTH GUSSET PLATE CONN. SEE "ANGLE TO GUSSET PLATE CONNECTION WELD TERMINATION DETAILS"



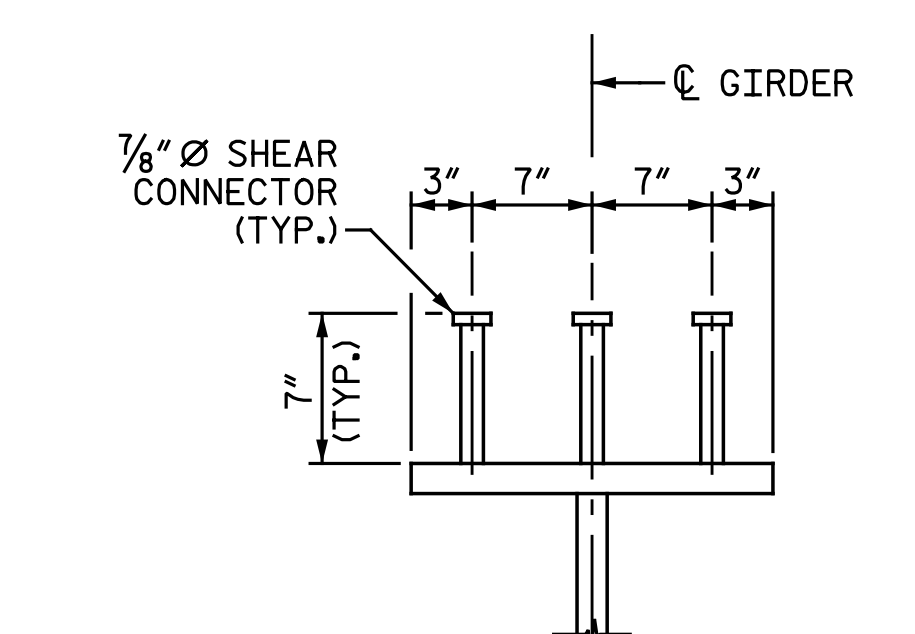
ANGLE TO GUSSET PLATE CONNECTION WELD TERMINATION DETAILS



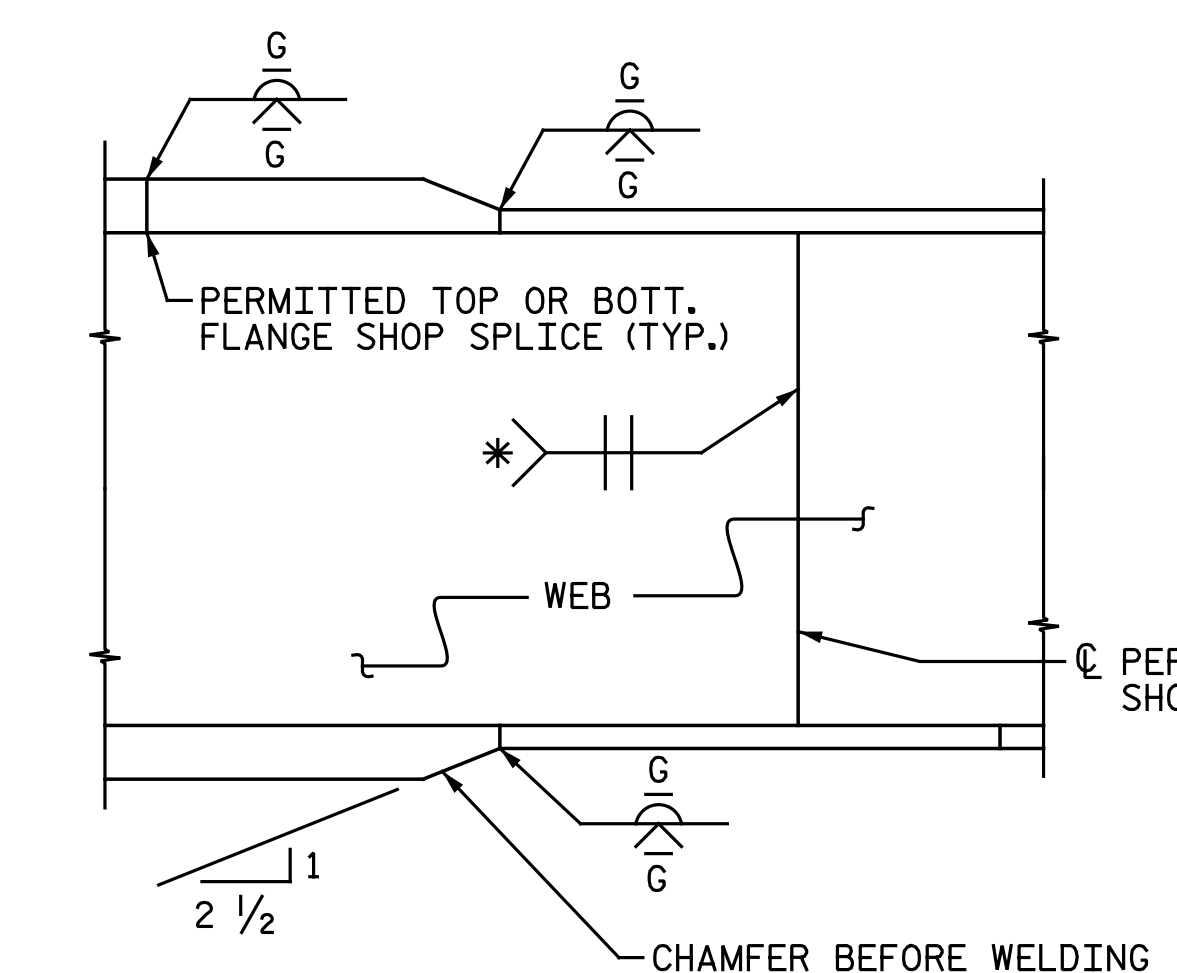
OPTIONAL INTERMEDIATE DIAPHRAGM



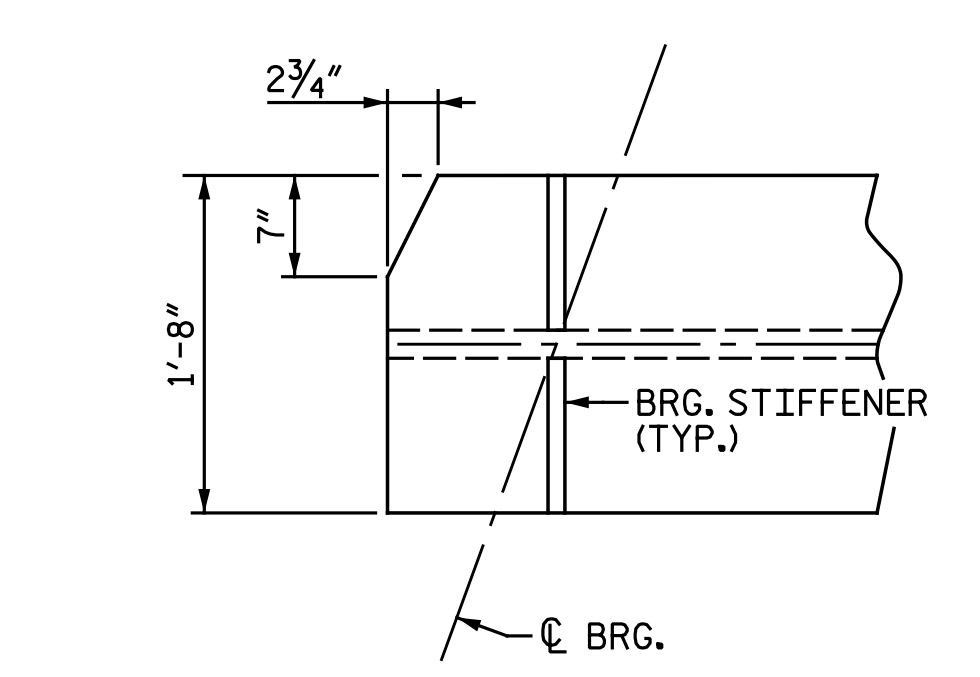
GUSSET PLATE CONNECTION WELD TERMINATION DETAILS



SHEAR CONNECTOR DETAIL



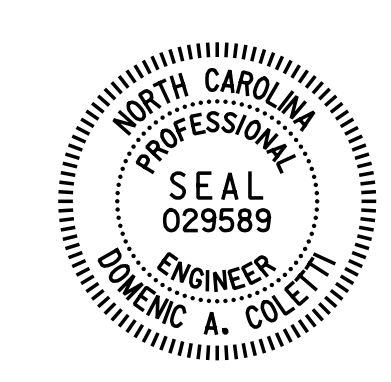
TYPICAL FLANGE AND WEB BUTT JOINT DETAILS



TOP FLANGE CLIP DETAIL

DRAWN BY : C. E. M. / N. B. S. DATE : 3-10-17
 CHECKED BY : D. A. COLETTI DATE : 6-8-17

8/14/2017
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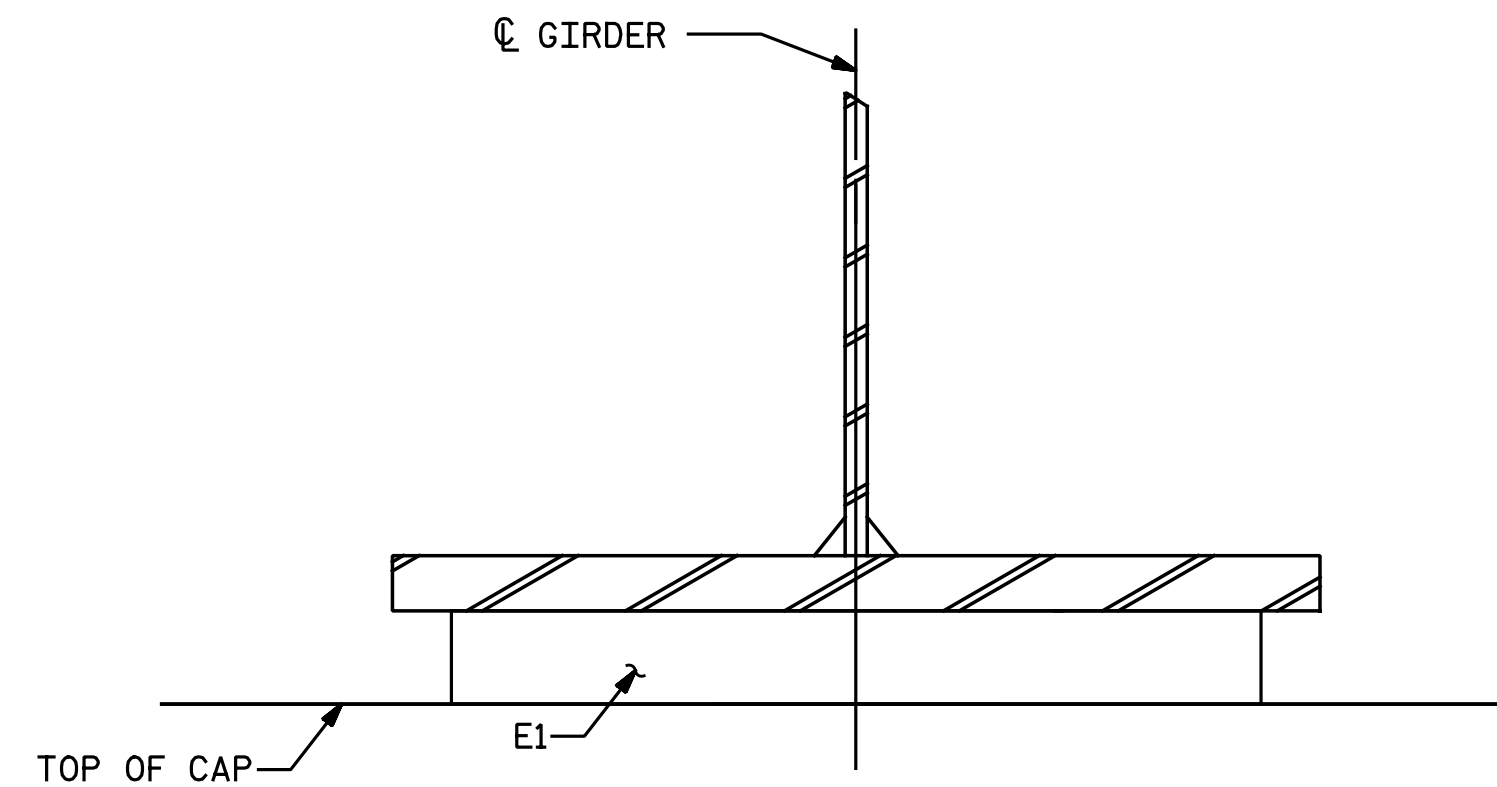
PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-
 SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE					
STRUCTURAL STEEL DETAILS					
RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO. S4-12					TOTAL SHEETS 31

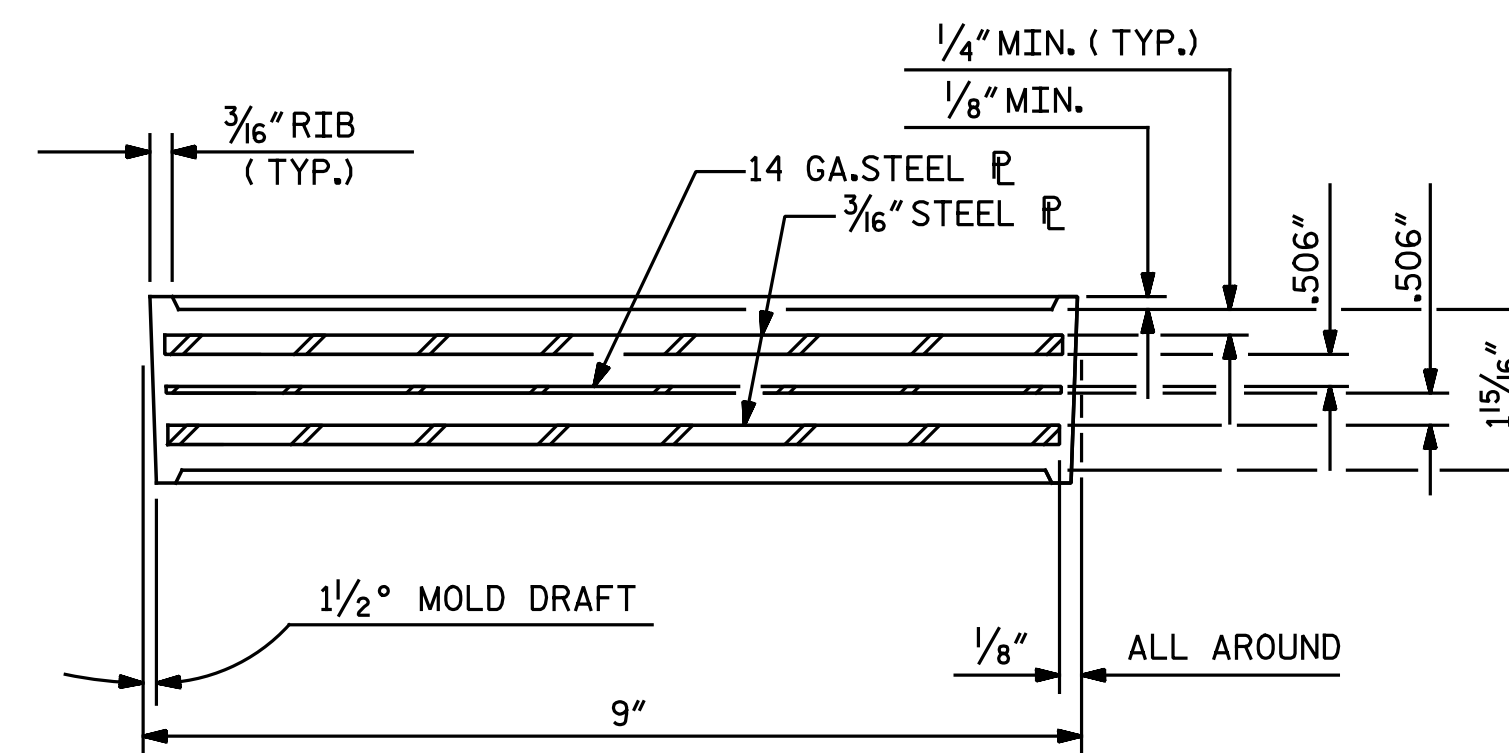
NOTES:

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

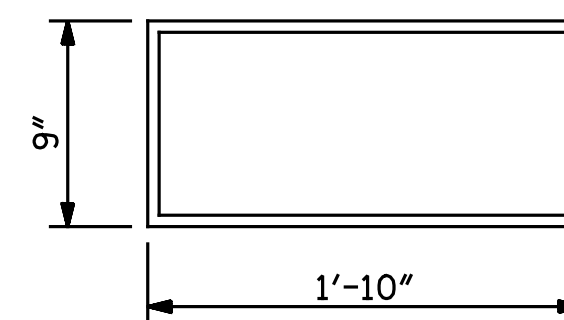
FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.



SECTION
AT INTEGRAL END BENTS



TYPICAL SECTION OF ELASTOMERIC BEARINGS



E1 (10 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

TYPE IV

MAXIMUM ALLOWABLE SERVICE LOADS	
D.L.+L.L. (NO IMPACT)	
TYPE IV	225 k

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 ELASTOMERIC BEARING
 DETAILS
 RIGHT LANE

8/14/2017
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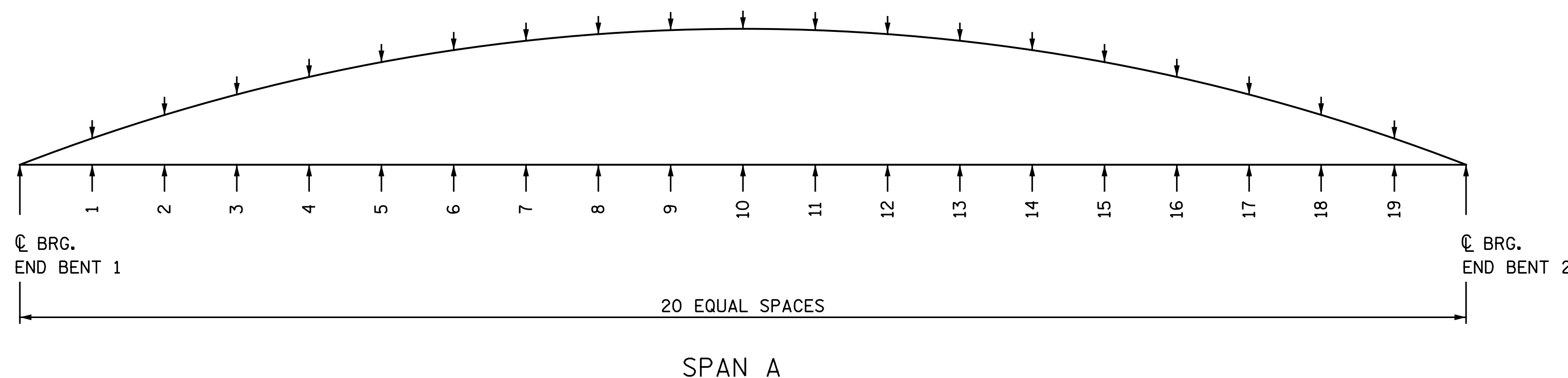
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-13
1			3			TOTAL SHEETS
2			4			31

DRAWN BY : N. B. SPEAKS DATE : 4-5-17
 CHECKED BY : D. A. COLETTI DATE : 6-8-17

NOTES:
 VALUES GIVEN ARE AT TWENTIETH POINTS BETWEEN CENTERLINE OF BEARINGS.
 UPWARD DEFLECTIONS ARE INDICATED WITH A " - " SIGN.

DEAD LOAD DEFLECTION AND CAMBER ORDINATES FOR SPAN A																					
GIRDER 1																					
20TH POINTS	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019	0.037	0.054	0.069	0.083	0.094	0.103	0.109	0.113	0.115	0.113	0.109	0.103	0.094	0.083	0.069	0.054	0.037	0.019	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	0.063	0.125	0.182	0.233	0.277	0.315	0.345	0.367	0.381	0.385	0.381	0.367	0.345	0.315	0.277	0.233	0.182	0.125	0.063	0.000
DEFLECTION DUE TO WEIGHT OF RAIL	0.000	0.009	0.017	0.025	0.032	0.038	0.043	0.048	0.051	0.052	0.053	0.052	0.051	0.048	0.043	0.038	0.032	0.025	0.017	0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.091	0.179	0.261	0.334	0.398	0.452	0.496	0.527	0.547	0.553	0.547	0.527	0.496	0.452	0.398	0.334	0.261	0.179	0.091	0.000
VERTICAL CURVE ORDINATE	0.000	0.017	0.033	0.046	0.058	0.068	0.076	0.083	0.087	0.090	0.091	0.090	0.087	0.083	0.076	0.068	0.058	0.046	0.033	0.017	0.000
SUPERLEVATION ORDINATE	0.000	-0.008	-0.015	-0.022	-0.027	-0.032	-0.036	-0.039	-0.041	-0.042	-0.043	-0.042	-0.041	-0.039	-0.036	-0.032	-0.027	-0.022	-0.015	-0.008	0.000
REQUIRED CAMBER	0"	1 3/16'	2 3/8 "	3 1/6'	4 3/8 "	5 3/16'	5 15/16'	6 1/2 "	6 7/8 "	7 1/8 "	7 3/16'	7 1/8 "	6 7/8 "	6 1/2 "	5 15/16'	5 3/16'	4 3/8 "	3 1/6'	2 3/8 "	1 3/16'	0"
GIRDER 2																					
20TH POINTS	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019	0.037	0.054	0.069	0.083	0.094	0.103	0.109	0.113	0.115	0.113	0.109	0.103	0.094	0.083	0.069	0.054	0.037	0.019	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	0.063	0.124	0.181	0.232	0.277	0.314	0.344	0.366	0.380	0.384	0.380	0.366	0.344	0.314	0.277	0.232	0.181	0.124	0.063	0.000
DEFLECTION DUE TO WEIGHT OF RAIL	0.000	0.009	0.017	0.025	0.032	0.039	0.044	0.048	0.051	0.053	0.054	0.053	0.051	0.048	0.044	0.039	0.032	0.025	0.017	0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.091	0.179	0.261	0.334	0.398	0.452	0.495	0.527	0.546	0.553	0.546	0.527	0.495	0.452	0.398	0.334	0.261	0.179	0.091	0.000
VERTICAL CURVE ORDINATE	0.000	0.017	0.033	0.047	0.059	0.069	0.077	0.083	0.088	0.091	0.091	0.091	0.088	0.083	0.077	0.069	0.059	0.047	0.033	0.017	0.000
SUPERLEVATION ORDINATE	0.000	-0.008	-0.015	-0.022	-0.027	-0.032	-0.036	-0.039	-0.041	-0.042	-0.043	-0.042	-0.041	-0.039	-0.036	-0.032	-0.027	-0.022	-0.015	-0.008	0.000
REQUIRED CAMBER	0"	1 3/16'	2 3/8 "	3 1/6'	4 3/8 "	5 3/16'	5 15/16'	6 1/2 "	6 7/8 "	7 1/8 "	7 3/16'	7 1/8 "	6 7/8 "	6 1/2 "	5 15/16'	5 3/16'	4 3/8 "	3 1/6'	2 3/8 "	1 3/16'	0"
GIRDER 3																					
20TH POINTS	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019	0.037	0.054	0.069	0.083	0.094	0.103	0.109	0.113	0.115	0.113	0.109	0.103	0.094	0.083	0.069	0.054	0.037	0.019	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	0.063	0.124	0.181	0.231	0.276	0.313	0.343	0.365	0.379	0.383	0.379	0.365	0.343	0.313	0.276	0.231	0.181	0.124	0.063	0.000
DEFLECTION DUE TO WEIGHT OF RAIL	0.000	0.009	0.017	0.025	0.032	0.039	0.044	0.048	0.051	0.053	0.054	0.053	0.051	0.048	0.044	0.039	0.032	0.025	0.017	0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.090	0.179	0.260	0.333	0.397	0.451	0.494	0.526	0.545	0.552	0.545	0.526	0.494	0.451	0.397	0.333	0.260	0.179	0.090	0.000
VERTICAL CURVE ORDINATE	0.000	0.017	0.033	0.047	0.059	0.069	0.077	0.084	0.088	0.091	0.092	0.091	0.088	0.084	0.077	0.069	0.059	0.047	0.033	0.018	0.000
SUPERLEVATION ORDINATE	0.000	-0.008	-0.015	-0.022	-0.027	-0.032	-0.036	-0.039	-0.041	-0.043	-0.043	-0.043	-0.041	-0.039	-0.036	-0.032	-0.028	-0.022	-0.016	-0.008	0.000
REQUIRED CAMBER	0"	1 3/16'	2 3/8 "	3 1/6'	4 3/8 "	5 3/16'	5 15/16'	6 1/2 "	6 7/8 "	7 1/8 "	7 3/16'	7 1/8 "	6 7/8 "	6 1/2 "	5 15/16'	5 3/16'	4 3/8 "	3 1/6'	2 3/8 "	1 3/16'	0"

* INCLUDES SLAB, BUILDUPS, AND STAY-IN-PLACE FORMS. DEFLECTIONS BASED ON SLAB POUR SEQUENCE SHOWN ON "BILL OF MATERIAL" SHEET.
 ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "REQUIRED CAMBER" WHICH IS SHOWN IN INCHES (FRACTION FORM).



PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 1 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

DEAD LOAD DEFLECTION
 AND CAMBER ORDINATES

RIGHT LANE

8/14/2017
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Michael Baker
 INTERNATIONAL

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 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No.: F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-14
1			3			TOTAL SHEETS
2			4			31

DRAWN BY : C. E. MAYHEW DATE : 6-1-17
 CHECKED BY : D. A. COLETTI DATE : 6-8-17

NOTES:

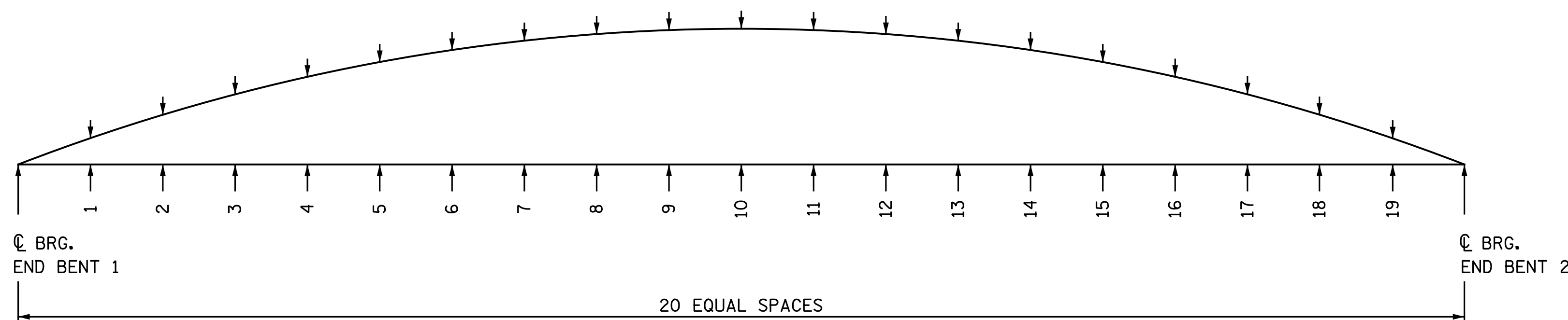
VALUES GIVEN ARE AT TWENTIETH POINTS BETWEEN CENTERLINE OF BEARINGS.

UPWARD DEFLECTIONS ARE INDICATED WITH A " - " SIGN.

DEAD LOAD DEFLECTION AND CAMBER ORDINATES FOR SPAN A																					
GIRDER 4																					
20TH POINTS	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019	0.037	0.054	0.069	0.083	0.094	0.103	0.109	0.113	0.115	0.113	0.109	0.103	0.094	0.083	0.069	0.054	0.037	0.019	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	0.063	0.124	0.181	0.232	0.277	0.314	0.344	0.366	0.380	0.384	0.380	0.366	0.344	0.314	0.277	0.232	0.181	0.124	0.063	0.000
DEFLECTION DUE TO WEIGHT OF RAIL	0.000	0.009	0.017	0.025	0.032	0.039	0.044	0.048	0.051	0.053	0.054	0.053	0.051	0.048	0.044	0.039	0.032	0.025	0.017	0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.091	0.179	0.261	0.334	0.398	0.452	0.495	0.527	0.546	0.553	0.546	0.527	0.495	0.452	0.398	0.334	0.261	0.179	0.091	0.000
VERTICAL CURVE ORDINATE	0.000	0.018	0.033	0.047	0.059	0.069	0.078	0.084	0.089	0.092	0.093	0.092	0.089	0.084	0.078	0.070	0.059	0.047	0.033	0.018	0.000
SUPERLEVATION ORDINATE	0.000	-0.008	-0.016	-0.022	-0.028	-0.032	-0.036	-0.039	-0.041	-0.043	-0.043	-0.043	-0.041	-0.039	-0.036	-0.032	-0.028	-0.022	-0.016	-0.008	0.000
REQUIRED CAMBER	0"	1 3/16"	2 3/8"	3 1/6"	4 3/8"	5 1/4"	5 15/16"	6 1/2"	6 7/8"	7 1/8"	7 1/4"	7 1/8"	6 7/8"	6 1/2"	5 15/16"	5 1/4"	4 3/8"	3 1/6"	2 3/8"	1 3/16"	0"

GIRDER 5																					
20TH POINTS	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019	0.037	0.054	0.069	0.083	0.094	0.103	0.109	0.113	0.115	0.113	0.109	0.103	0.094	0.083	0.069	0.054	0.037	0.019	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	0.063	0.125	0.182	0.233	0.277	0.315	0.345	0.367	0.381	0.385	0.381	0.367	0.345	0.315	0.277	0.233	0.182	0.125	0.063	0.000
DEFLECTION DUE TO WEIGHT OF RAIL	0.000	0.009	0.017	0.025	0.032	0.038	0.043	0.048	0.051	0.052	0.053	0.052	0.051	0.048	0.043	0.038	0.032	0.025	0.017	0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.091	0.179	0.261	0.334	0.398	0.452	0.496	0.527	0.547	0.553	0.547	0.527	0.496	0.452	0.398	0.334	0.261	0.179	0.091	0.000
VERTICAL CURVE ORDINATE	0.000	0.018	0.034	0.047	0.060	0.070	0.078	0.085	0.089	0.092	0.093	0.092	0.090	0.085	0.078	0.070	0.060	0.048	0.034	0.018	0.000
SUPERLEVATION ORDINATE	0.000	-0.008	-0.015	-0.022	-0.028	-0.032	-0.036	-0.039	-0.042	-0.043	-0.043	-0.043	-0.041	-0.039	-0.036	-0.032	-0.028	-0.022	-0.015	-0.008	0.000
REQUIRED CAMBER	0"	1 3/16"	2 3/8"	3 1/6"	4 3/8"	5 1/4"	5 15/16"	6 1/2"	6 7/8"	7 1/8"	7 1/4"	7 1/8"	6 7/8"	6 1/2"	5 15/16"	5 1/4"	4 3/8"	3 1/6"	2 3/8"	1 3/16"	0"

* INCLUDES SLAB, BUILDUPS, AND STAY-IN-PLACE FORMS. DEFLECTIONS BASED ON SLAB POUR SEQUENCE SHOWN ON "BILL OF MATERIAL" SHEET.
 ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "REQUIRED CAMBER" WHICH IS SHOWN IN INCHES (FRACTION FORM).



PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 2 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

DEAD LOAD DEFLECTION
 AND CAMBER ORDINATES

RIGHT LANE

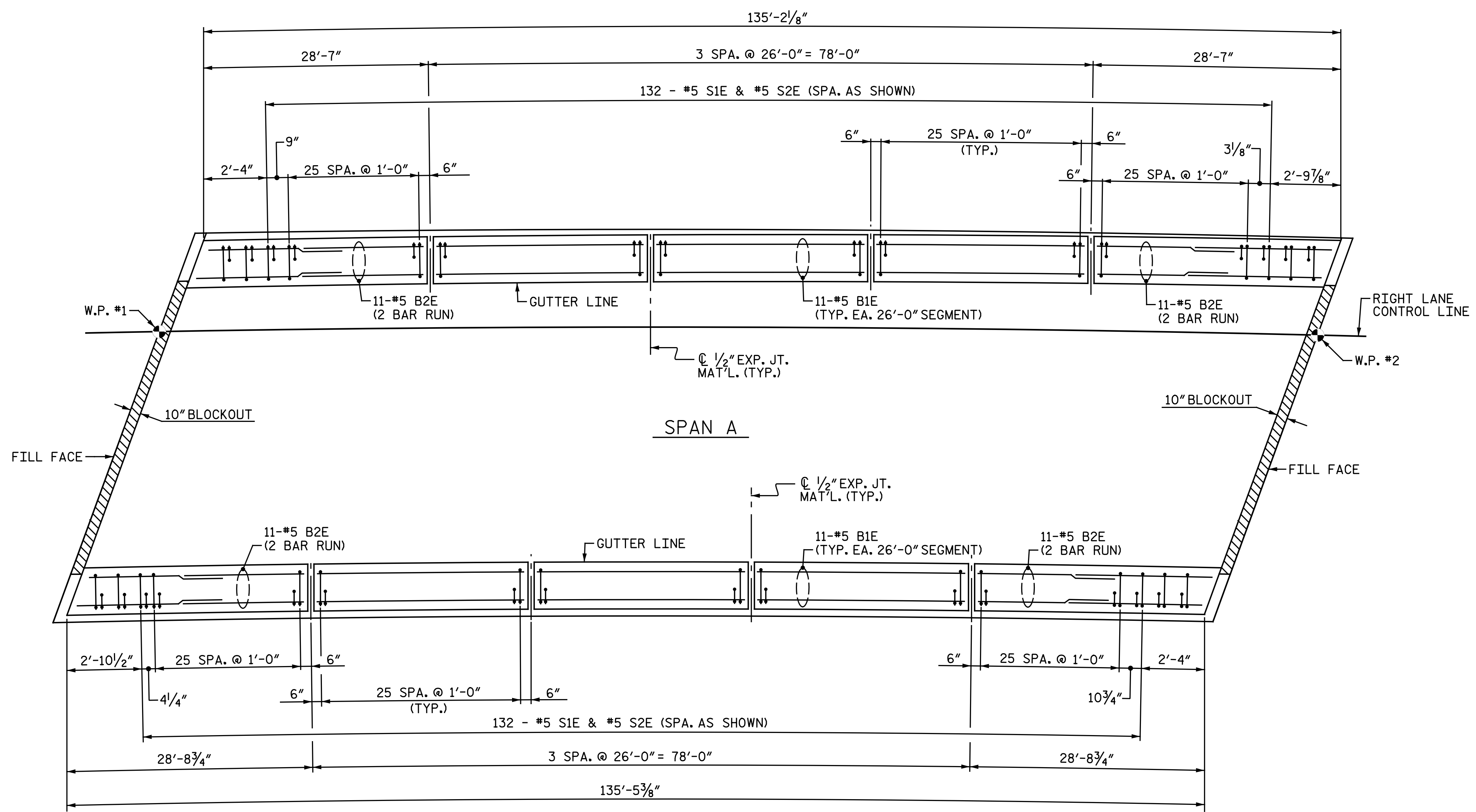
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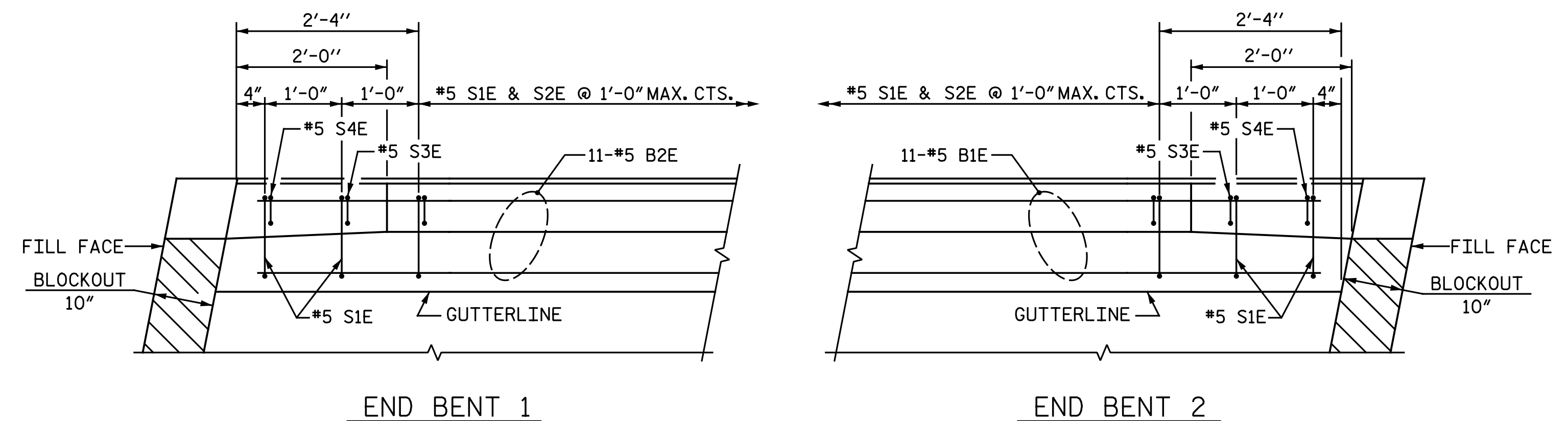
DRAWN BY : C. E. MAYHEW DATE : 6-1-17
 CHECKED BY : D. A. COLETTI DATE : 6-8-17



PLAN OF BARRIER RAIL

NOTE: ALL BARRIER RAIL DIMENSIONS ARE MEASURED ALONG THE ARC AT THE BACK FACE OF THE BARRIER RAIL.

SPLICE LENGTHS	
BAR SIZE	EPOXY COATED
#5	3'-5"



END BENT 1

END BENT 2

PLAN
(RIGHT RAIL SHOWN, LEFT RAIL SIMILAR)

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
 SHEET 1 OF 2



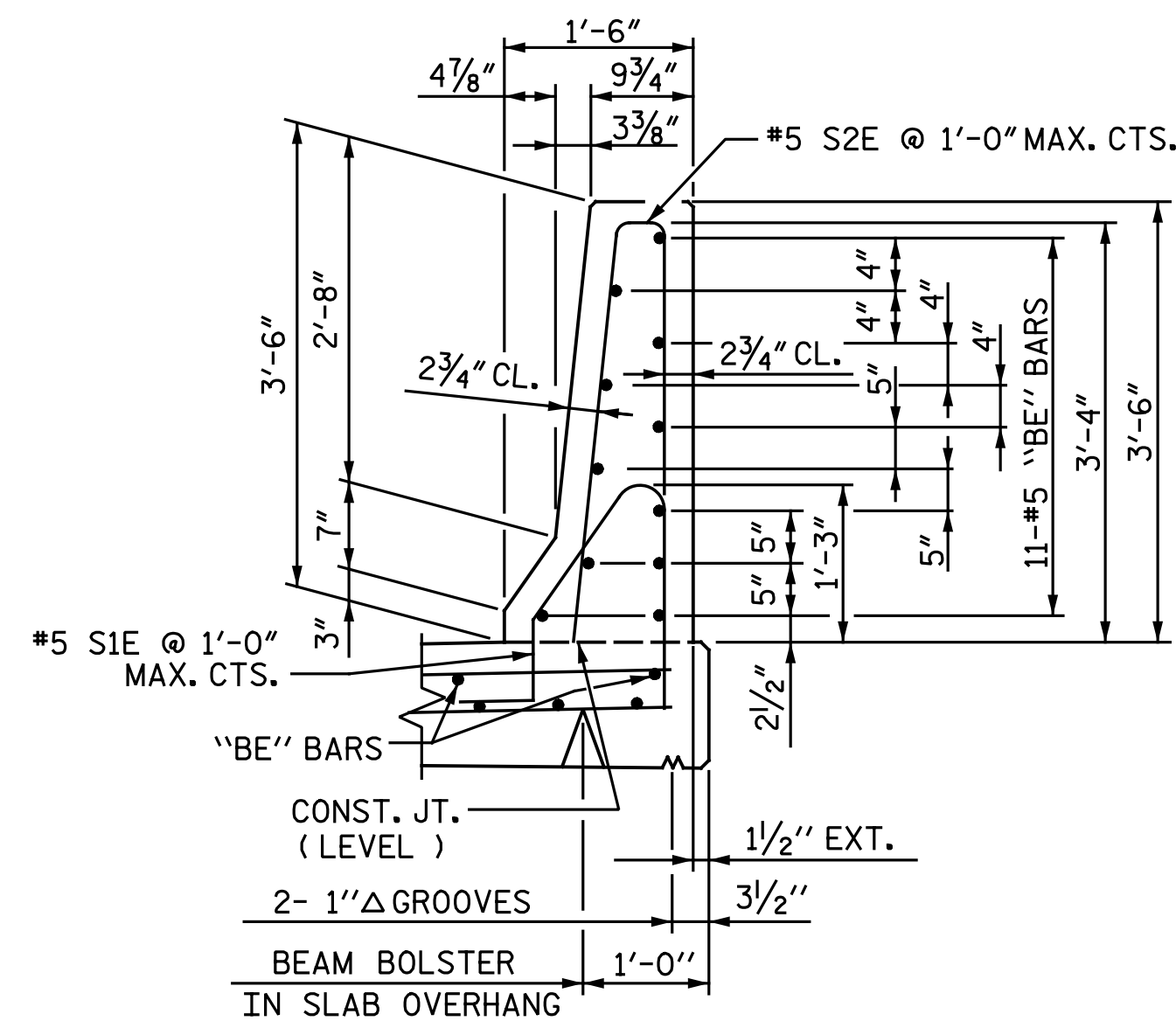
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
CONCRETE BARRIER RAIL

RIGHT LANE

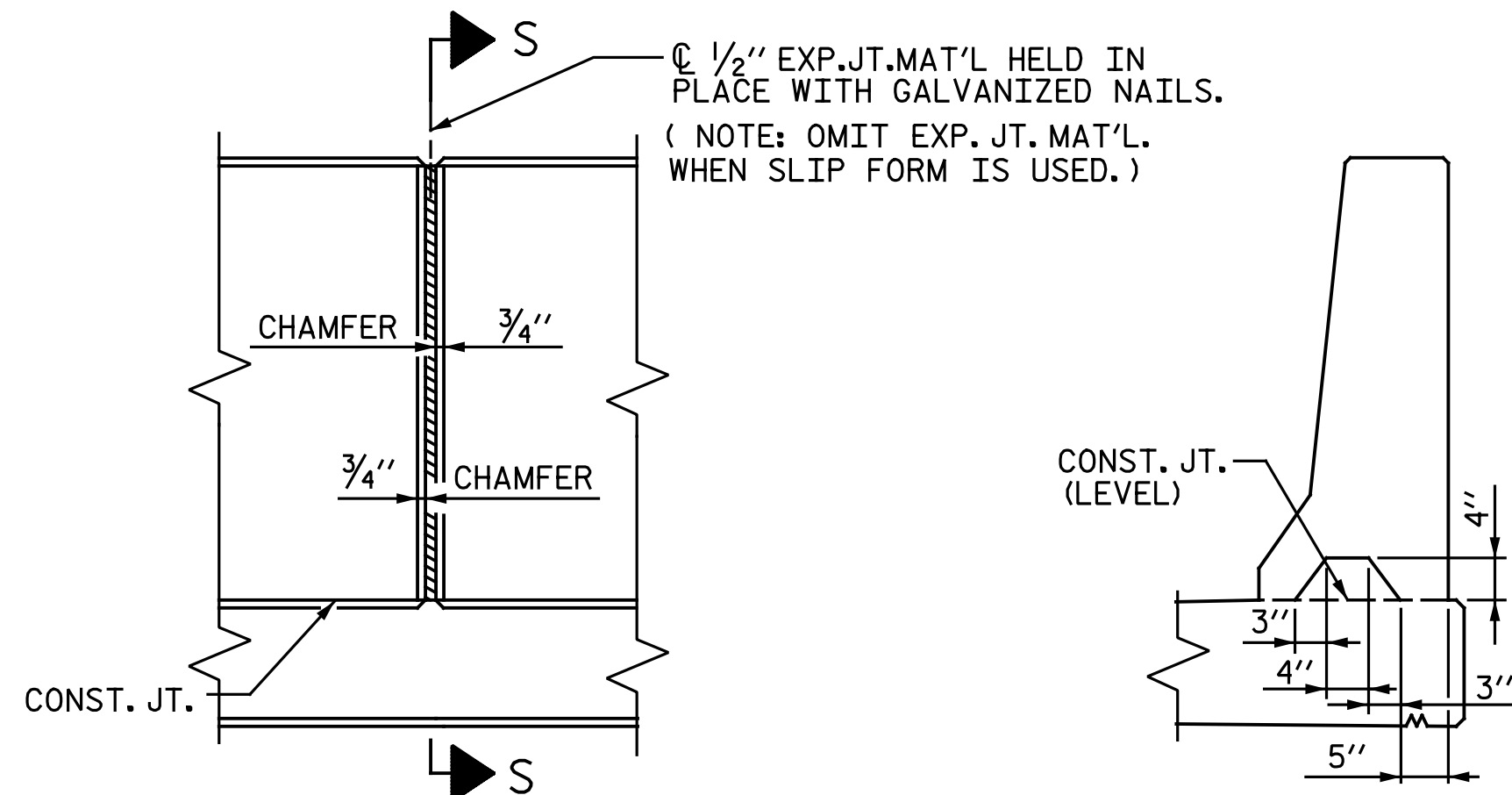
ASSEMBLED BY : N. B. SPEAKS	DATE : 3-29-17
CHECKED BY : D. A. COLETTI	DATE : 6-8-17
DRAWN BY : ARB 5/87	REV. 10/1/11
CHECKED BY : SJD 9/87	REV. 7/12
	REV. 6/13
MAA/GM	MAA/GM
MAA/GM	MAA/GM

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2			4			31



SECTION THRU RAIL



ELEVATION AT EXPANSION JOINTS

SECTION S-S
AT DAM IN OPEN JOINT
(THIS IS TO BE USED ONLY
WHEN SLIP FORM IS USED)

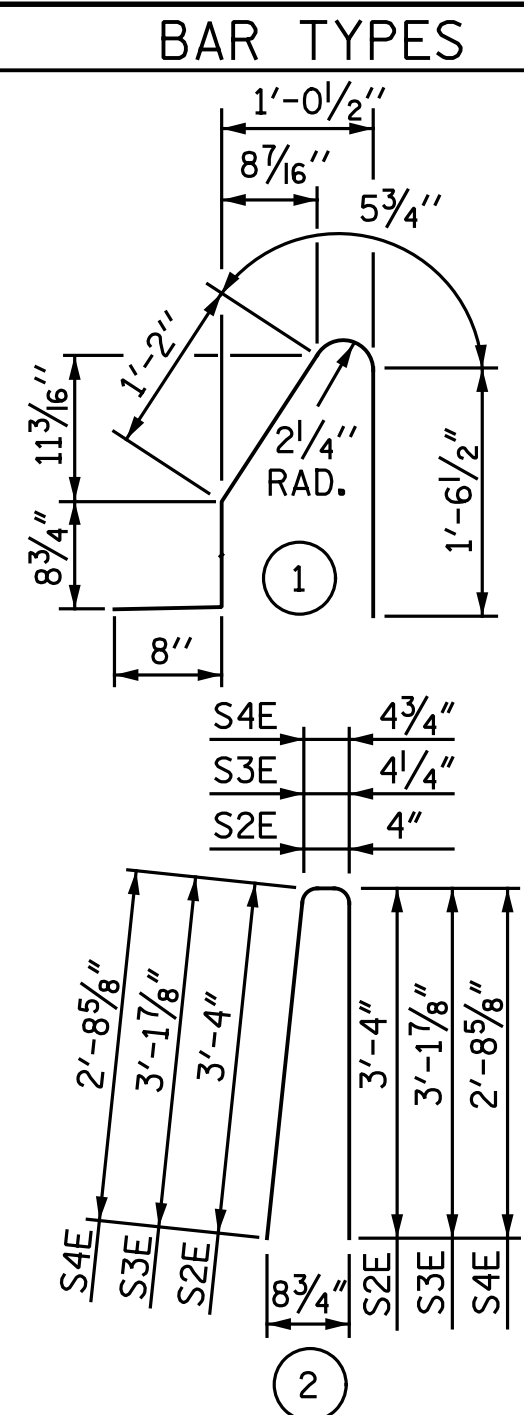
BARRIER RAIL DETAILS

NOTES:

THE BARRIER RAIL IN THE SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.



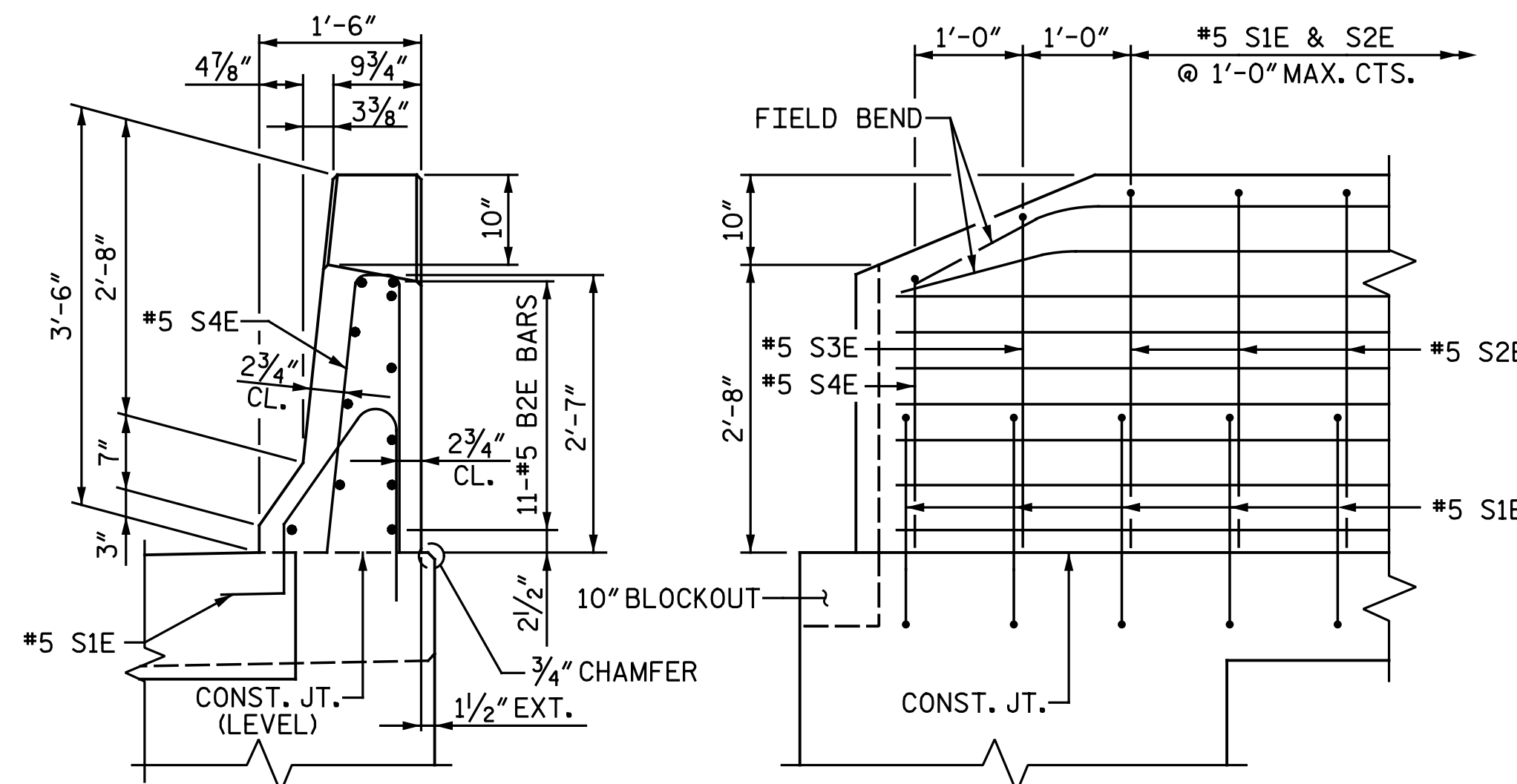
ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL

FOR CONCRETE BARRIER RAIL ONLY

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1E	66	#5	STR.	25'-8"	1,767
B2E	88	#5	STR.	15'-11"	1,461
S1E	272	#5	1	3'-11"	1,111
S2E	264	#5	2	7'-0"	1,927
S3E	4	#5	2	6'-8"	28
S4E	4	#5	2	5'-10"	24

EPOXY COATED REINFORCING STEEL	LBS.	6,318
CLASS AA CONCRETE	C.Y.	36.8
CONCRETE BARRIER RAIL	L.F.	270.60



END VIEW

SIDE VIEW

END OF RAIL DETAILS

ASSEMBLED BY : N. B. SPEAKS	DATE : 9-11-17
CHECKED BY : D. A. COLETTI	DATE : 9-12-17
DRAWN BY : ARB 5/87	REV. 10/1/11
CHECKED BY : SJD 9/87	REV. 7/12
	REV. 6/13

MAA/GM
MAA/GM
MAA/GM

9/12/2017
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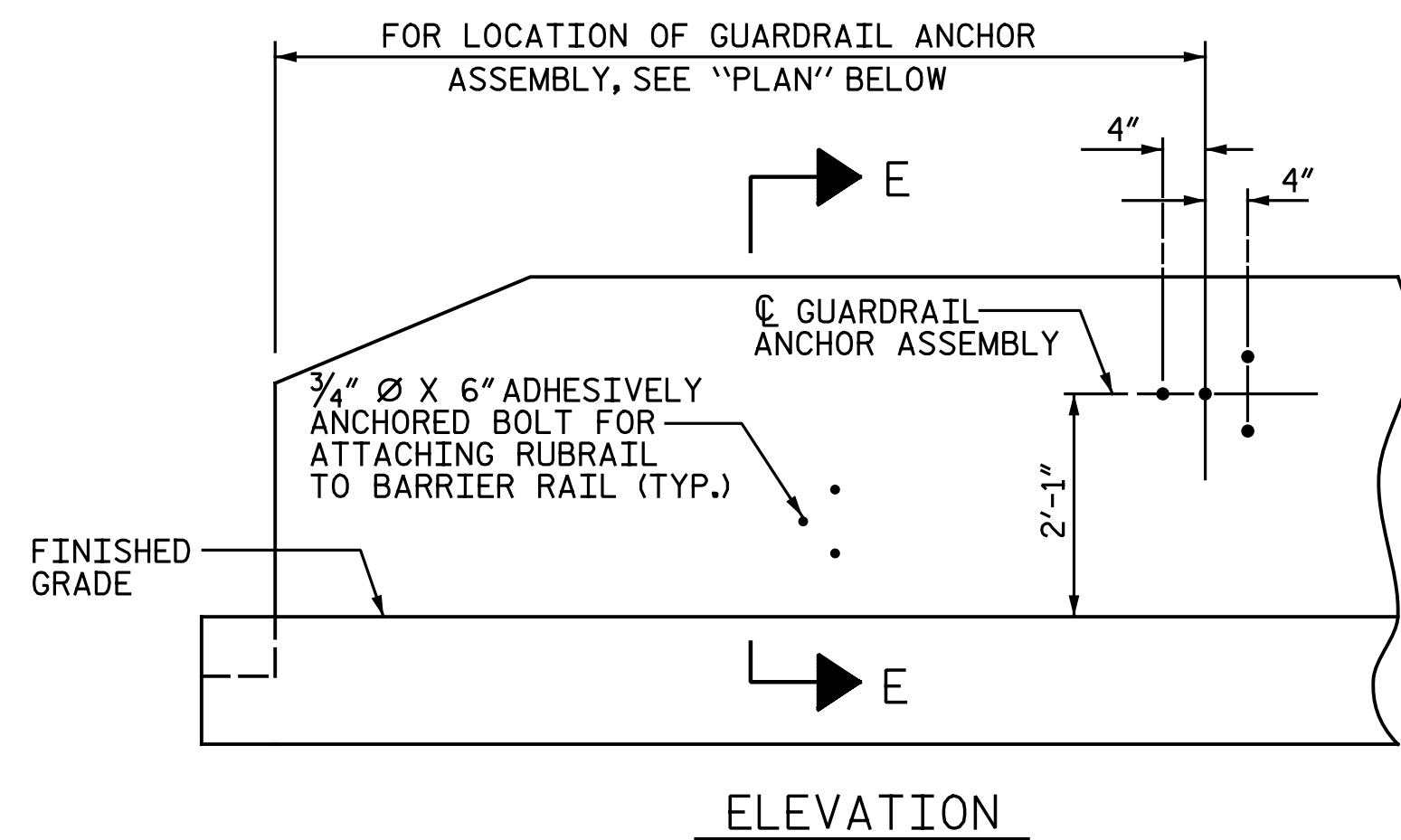
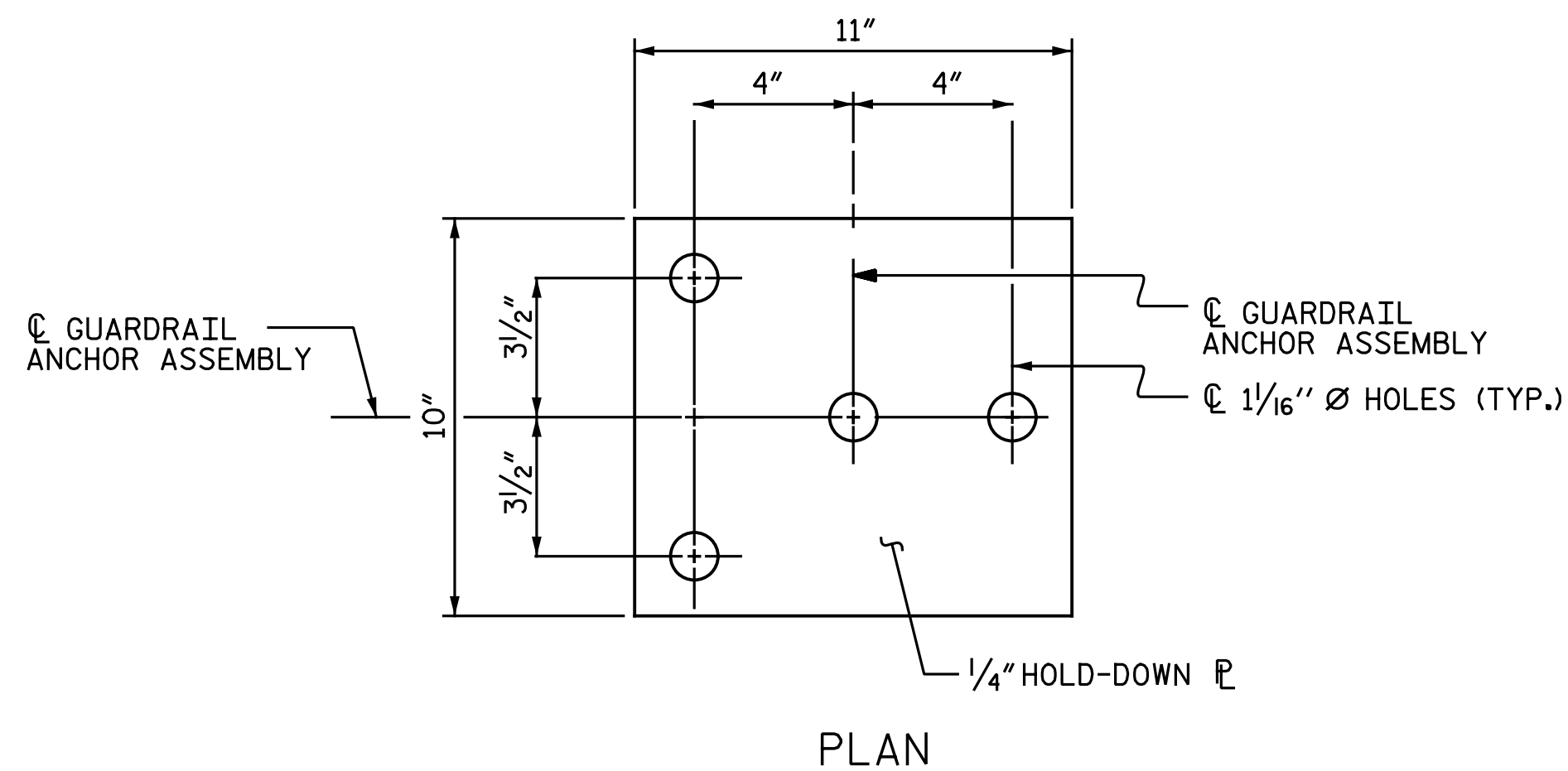


PROJECT NO. R-5703
LENOIR COUNTY
STATION: 166+72.51 -L-

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
CONCRETE
BARRIER RAIL

RIGHT LANE

REVISIONS						SHEET NO.
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NOTES:

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

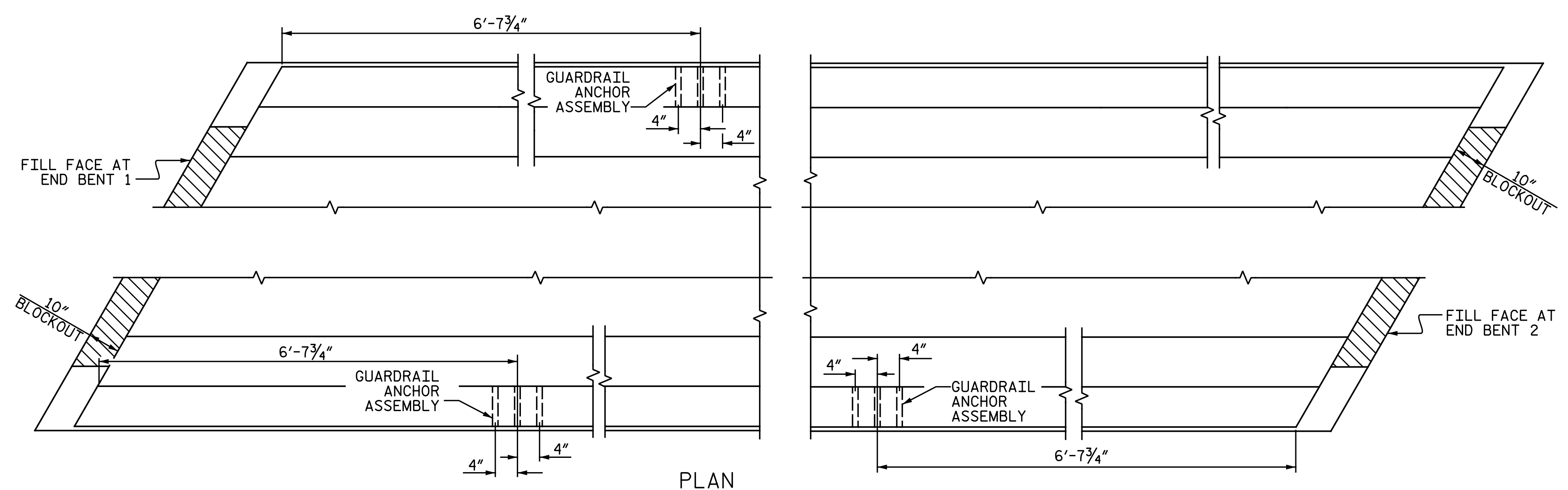
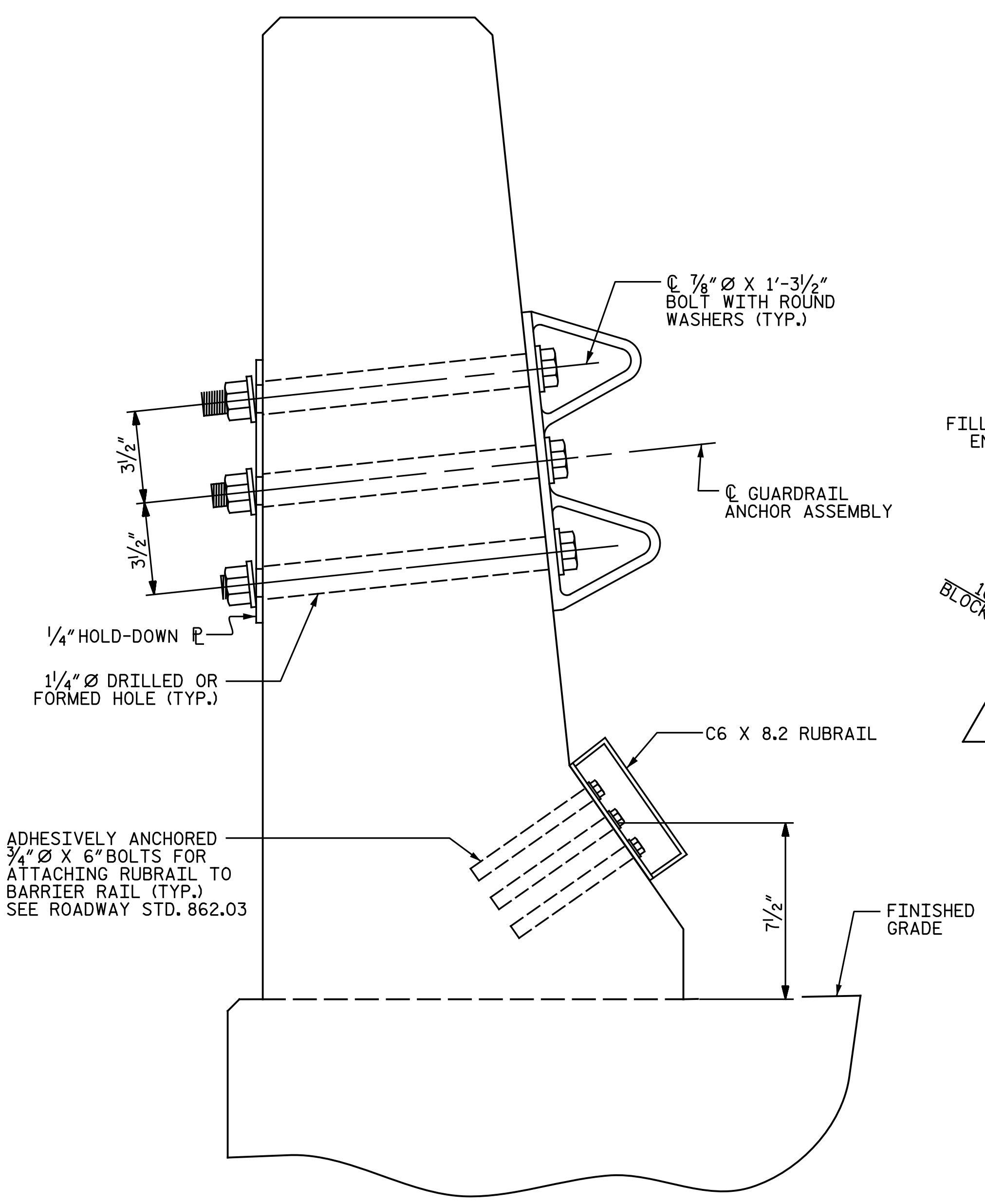
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

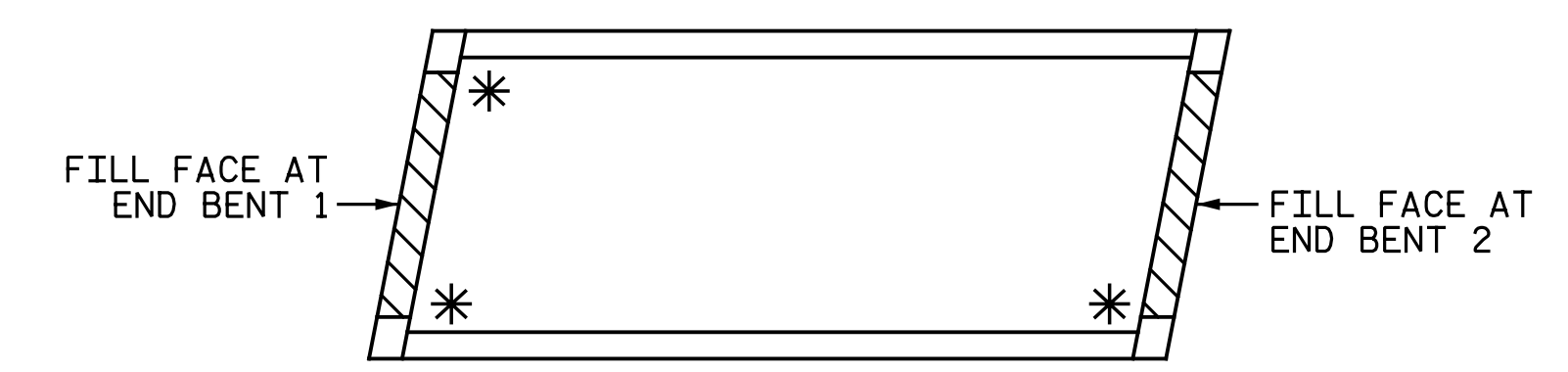
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



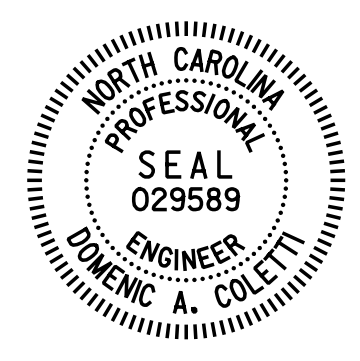
LOCATION OF ANCHORS FOR GUARDRAIL



* DENOTES GUARDRAIL ANCHOR ASSEMBLY

**SECTION E-E
GUARDRAIL ANCHOR ASSEMBLY DETAILS**

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-



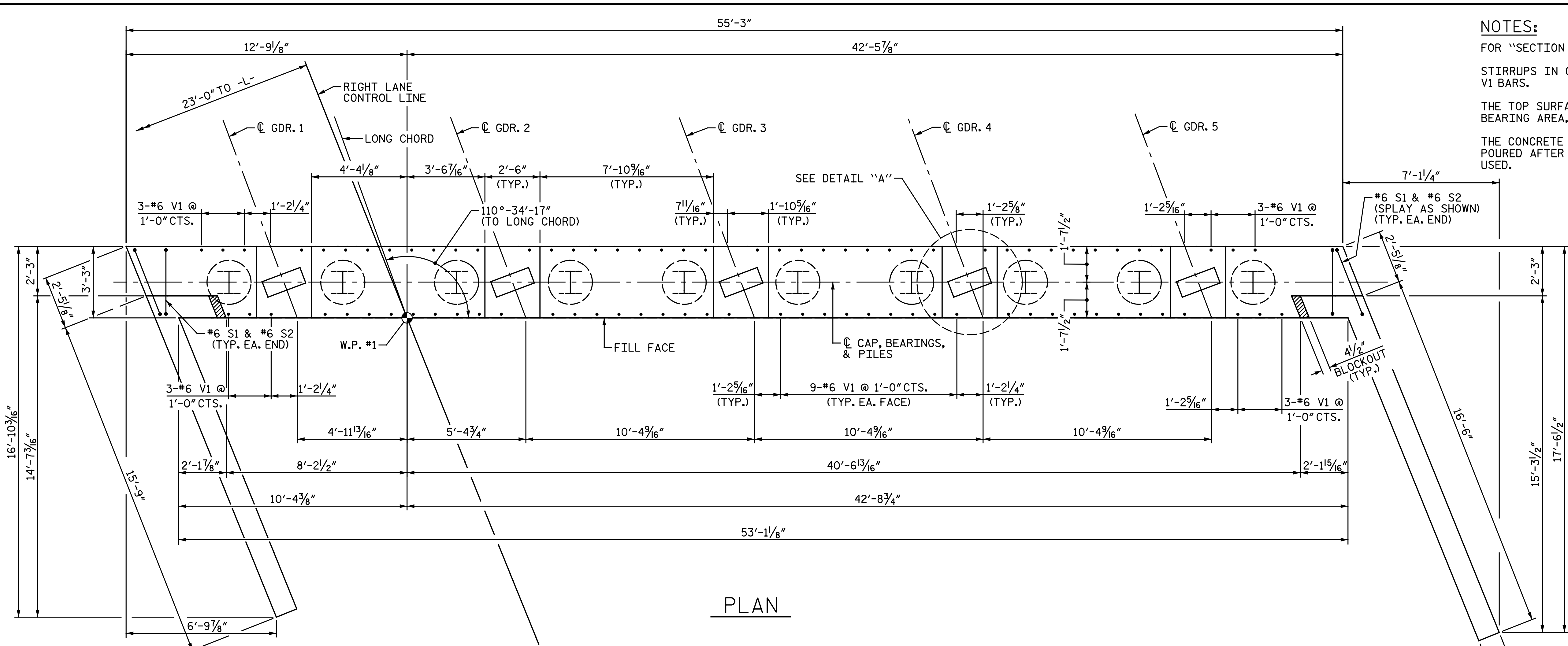
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 GUARDRAIL ANCHORAGE
 FOR BARRIER RAIL
 RIGHT LANE

ASSEMBLED BY : N. B. SPEAKS	DATE : 9-11-17
CHECKED BY : D.A. COLETTI	DATE : 9-12-17
DRAWN BY : TLA 5/06	REV. 10/1/11 MAA/GM
CHECKED BY : GM 5/06	REV. 7/12 MAA/GM
	REV. 6/13 MAA/GM

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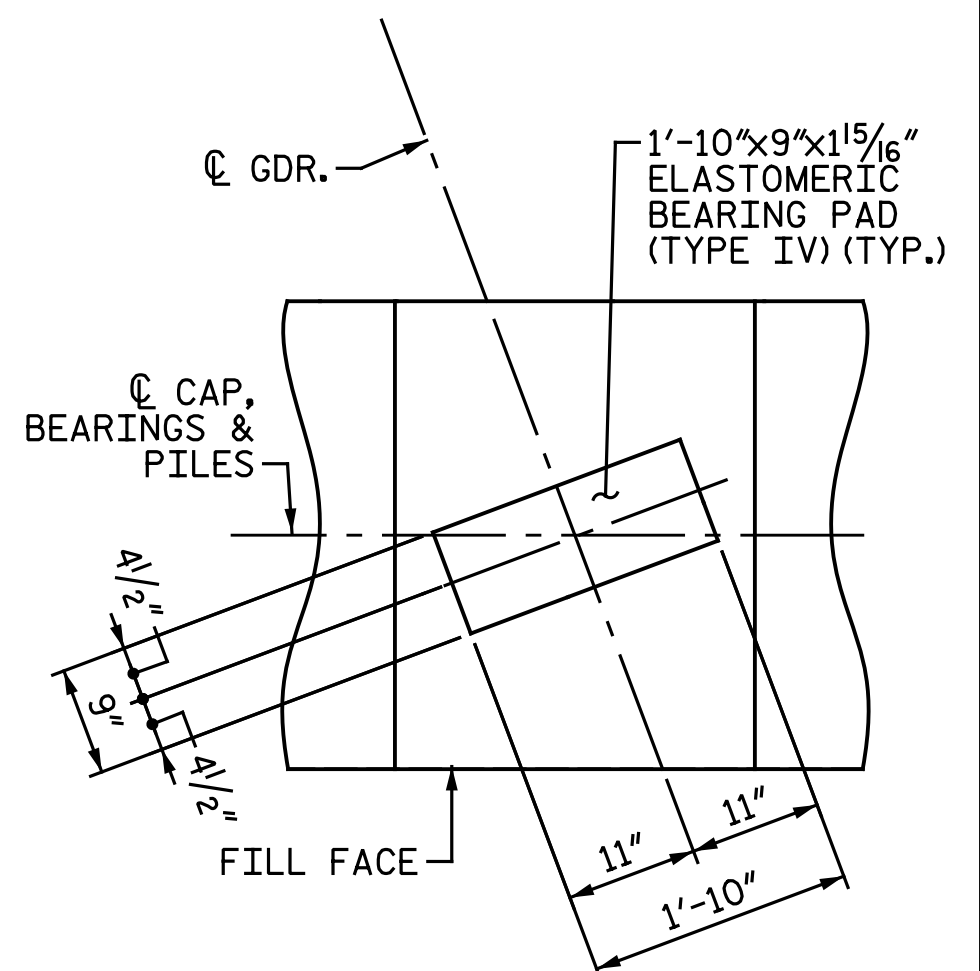
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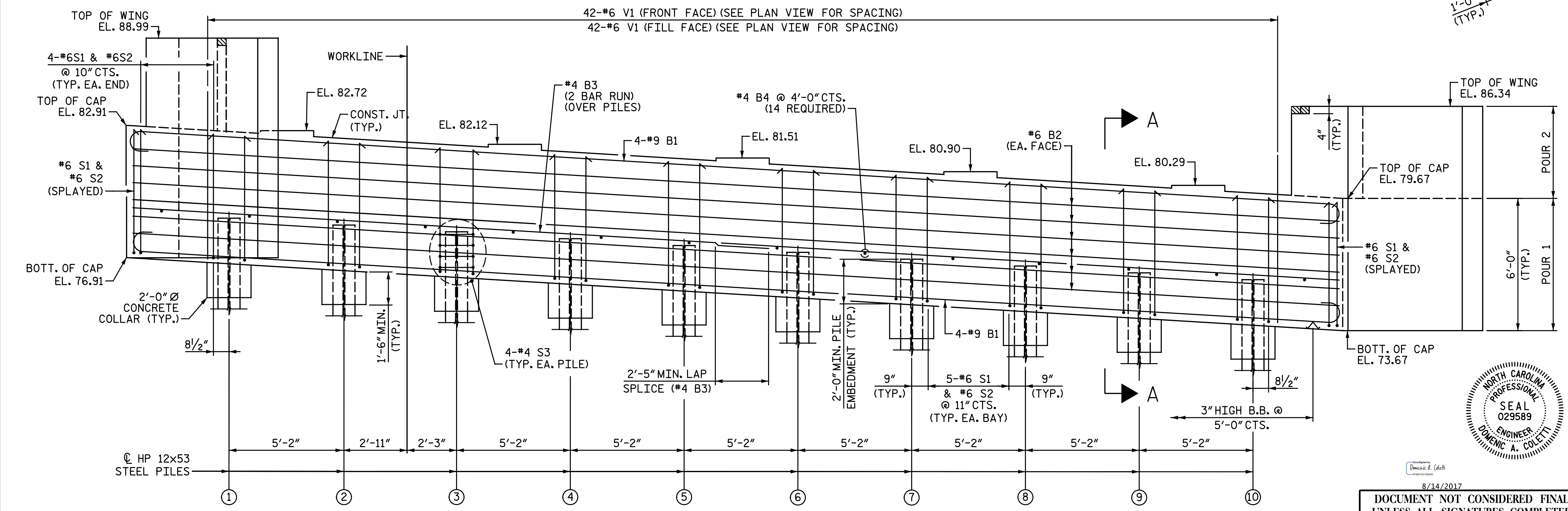


PLAN

NOTES:
 FOR "SECTION A-A", SEE "INTEGRAL END BENT 1 DETAILS" SHEET.
 STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #6 V1 BARS.
 THE TOP SURFACE OF THE END BENT CAP, EXCLUDING THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".
 THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.



DETAIL "A"
 ALL DIMENSIONS AND DETAILS SHOWN ARE TYPICAL FOR ALL BEARINGS AT EACH BRIDGE SEAT LOCATION.



ELEVATION

TOP OF PILE ELEVATIONS	
PILE	ELEVATION
1	78.67
2	78.37
3	78.07
4	77.76
5	77.46
6	77.16
7	76.85
8	76.55
9	76.25
10	75.95

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 1 OF 3



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
INTEGRAL END BENT 1

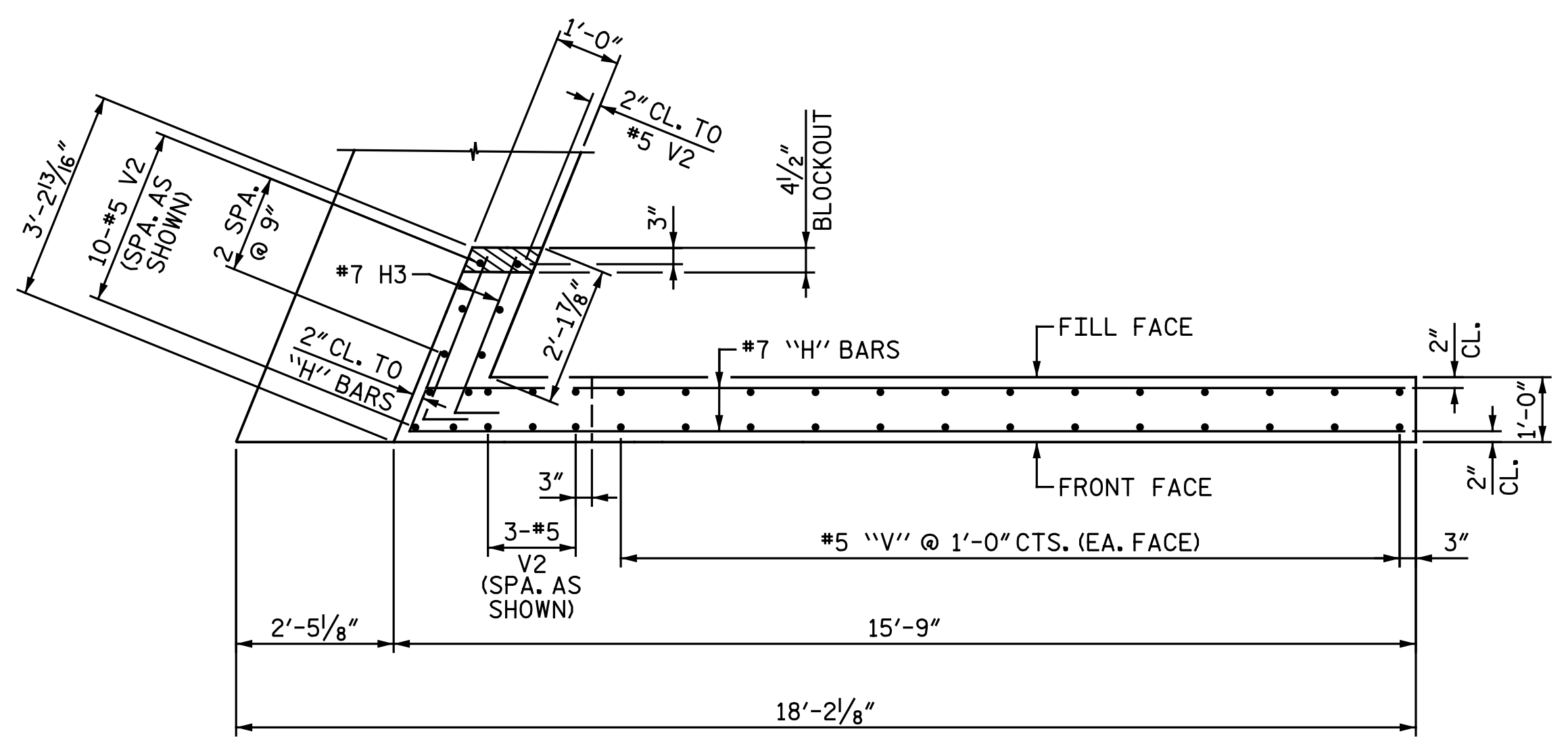
RIGHT LANE

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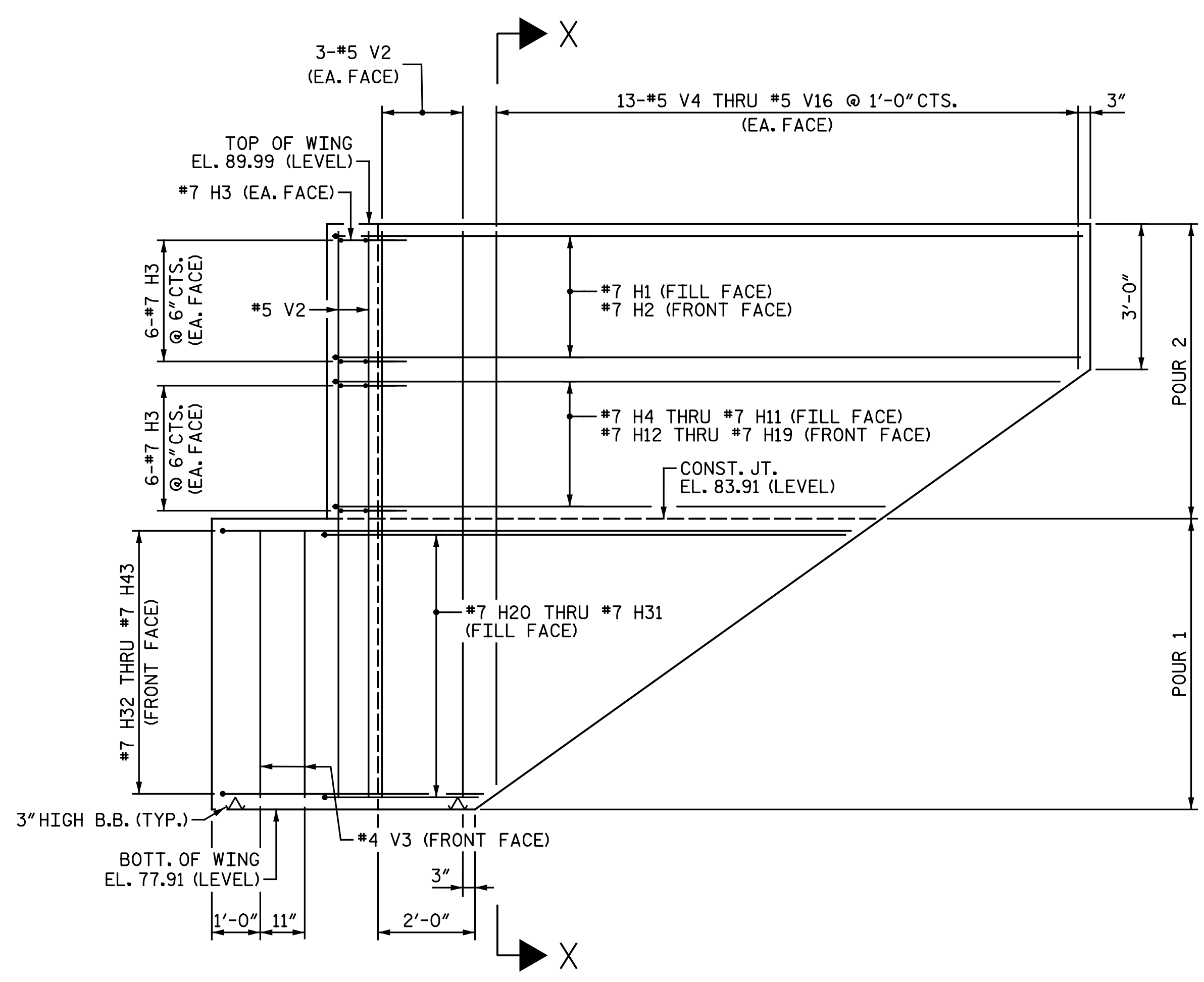
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2			4			

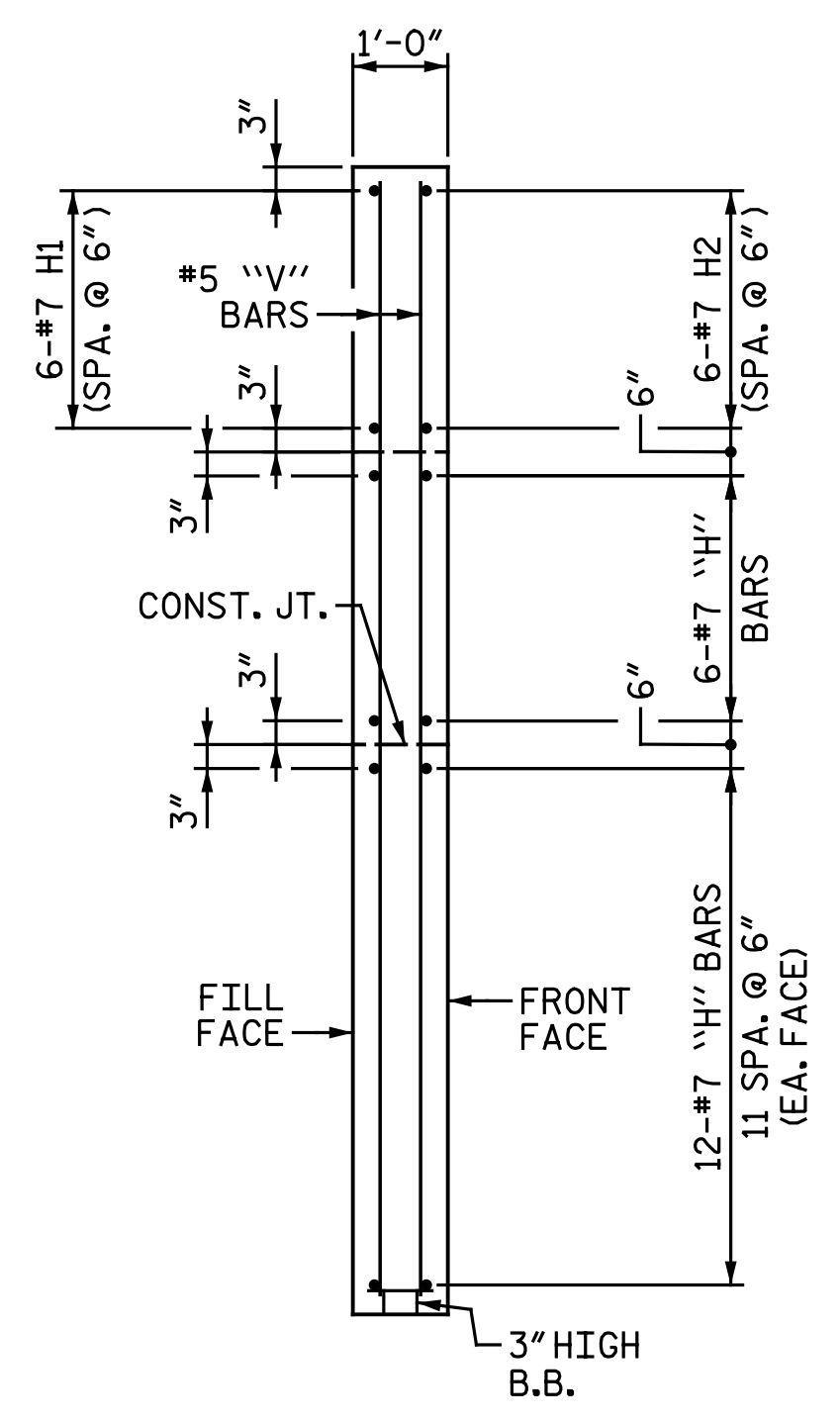
DRAWN BY: C. E. MAYHEW DATE: 5-25-17
 CHECKED BY: D. A. COLETTI DATE: 6-8-17



PLAN OF LEFT WING



ELEVATION OF LEFT WING



SECTION X-X

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
 SHEET 2 OF 3



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 1

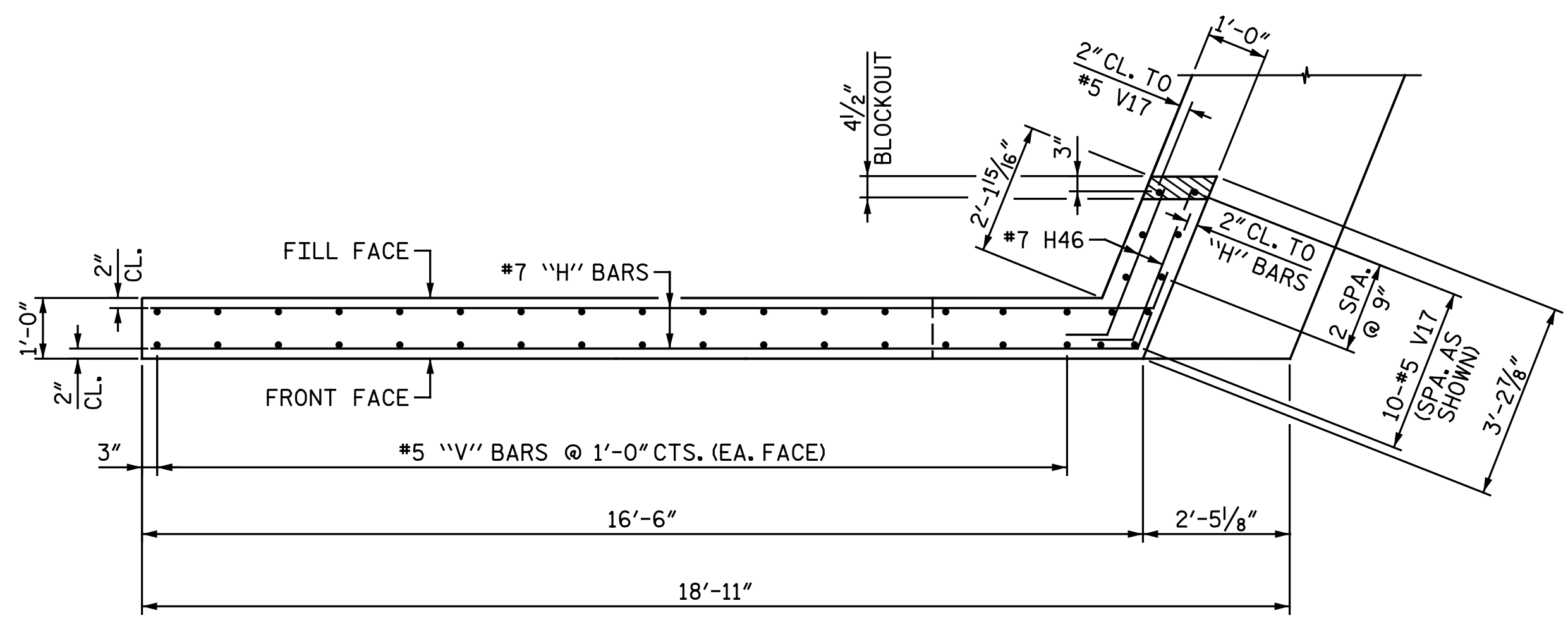
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RIGHT LANE

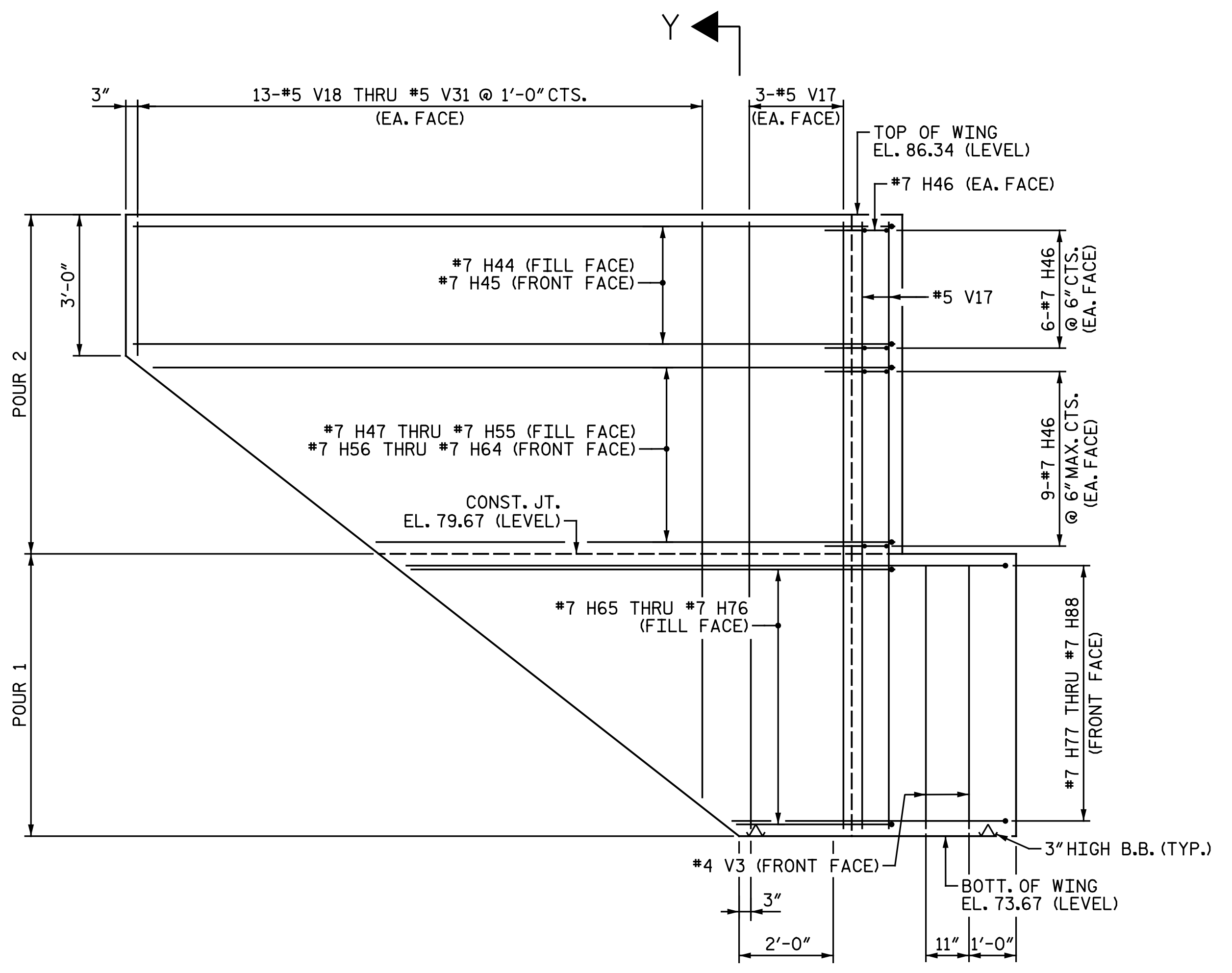
DRAWN BY : C. E. MAYHEW DATE : 4-14-17
 CHECKED BY : D. A. COLETTI DATE : 6-9-17

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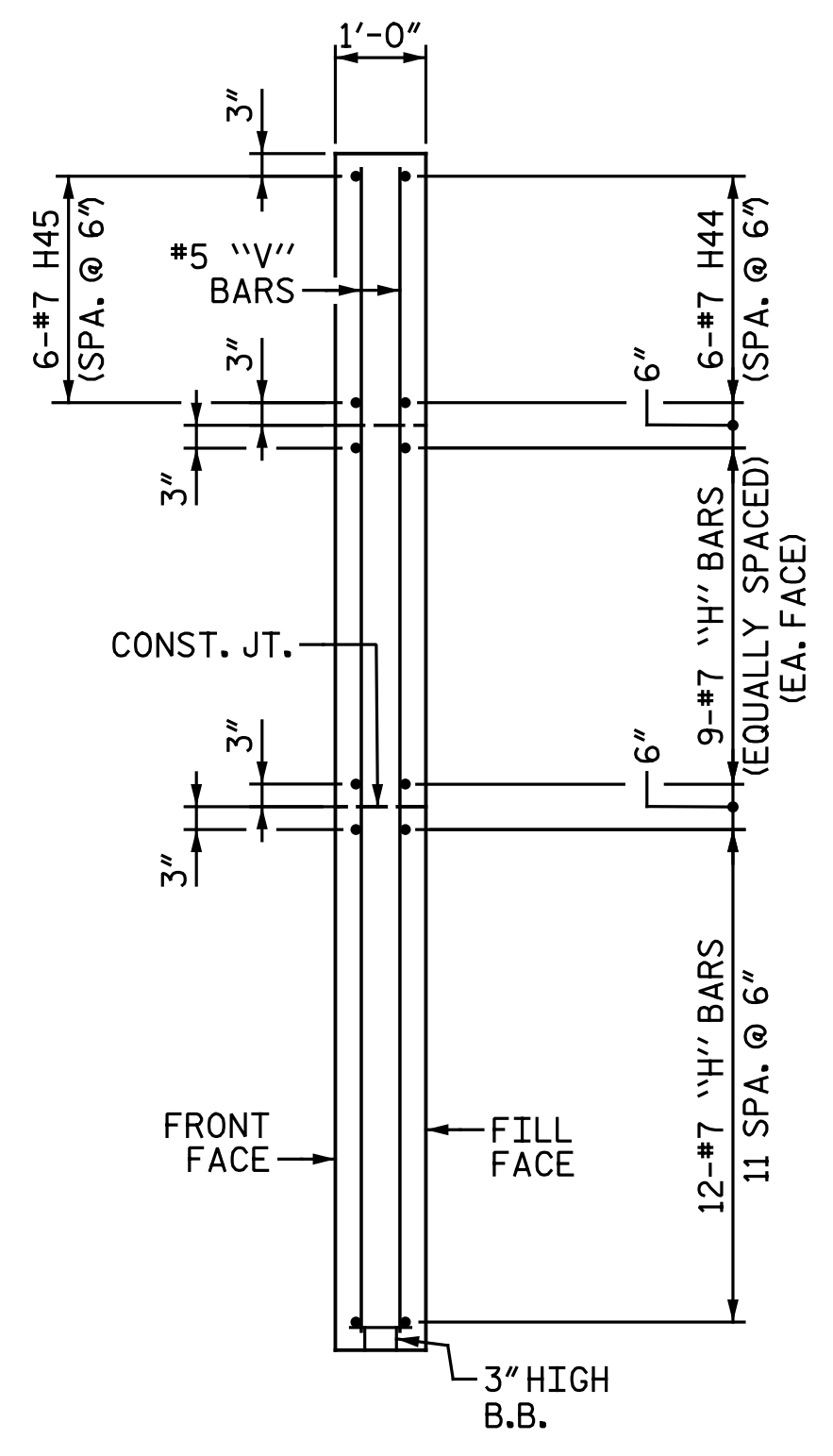
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1			3			TOTAL SHEETS
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PLAN OF RIGHT WING



ELEVATION OF RIGHT WING



SECTION Y-Y

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
 SHEET 3 OF 3



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL END BENT 1

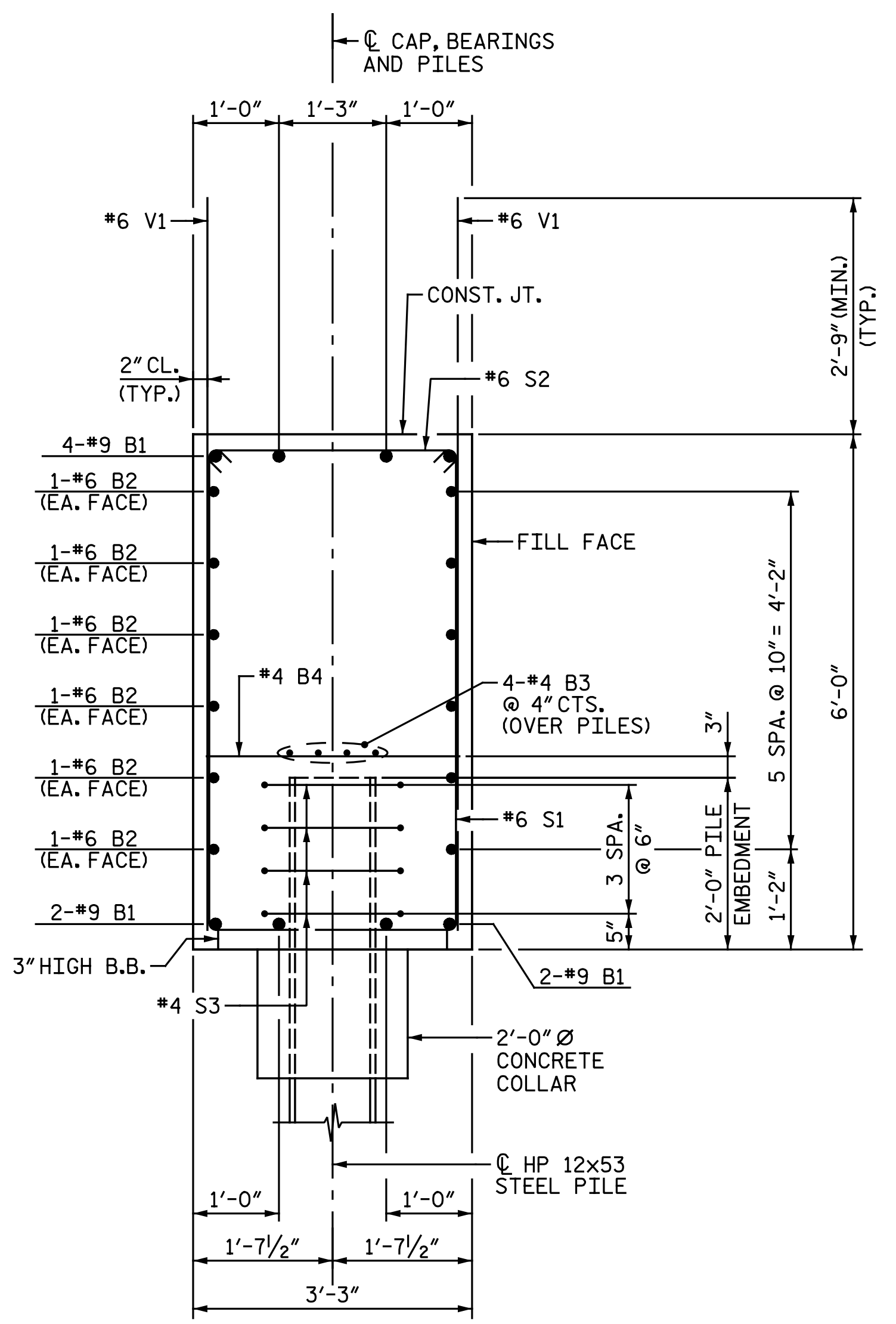
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RIGHT LANE

DRAWN BY : C. E. MAYHEW DATE : 4-17-17
 CHECKED BY : D. A. COLETTI DATE : 6-9-17

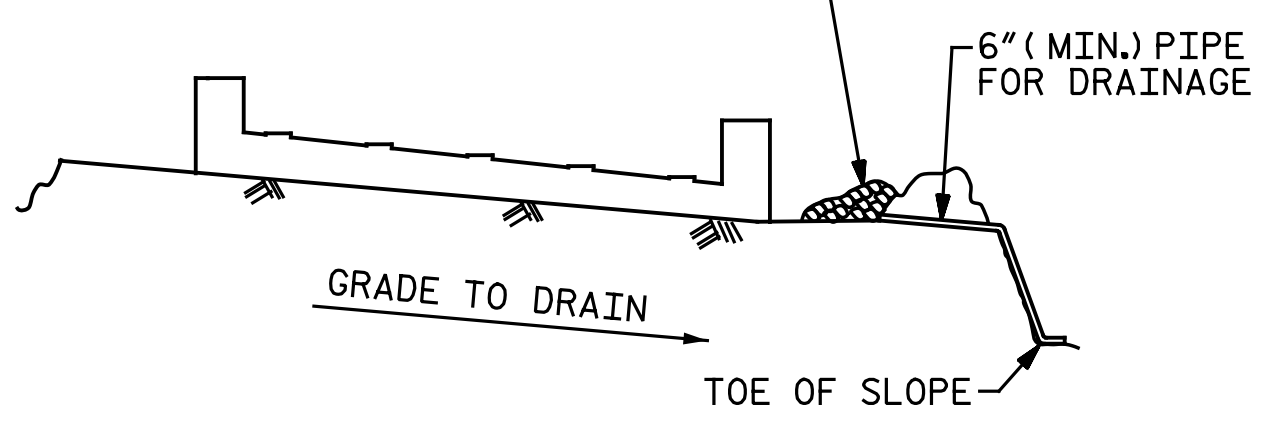
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SECTION A-A

MINIMUM OF 3- ONE CUBIC FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED.



BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS.

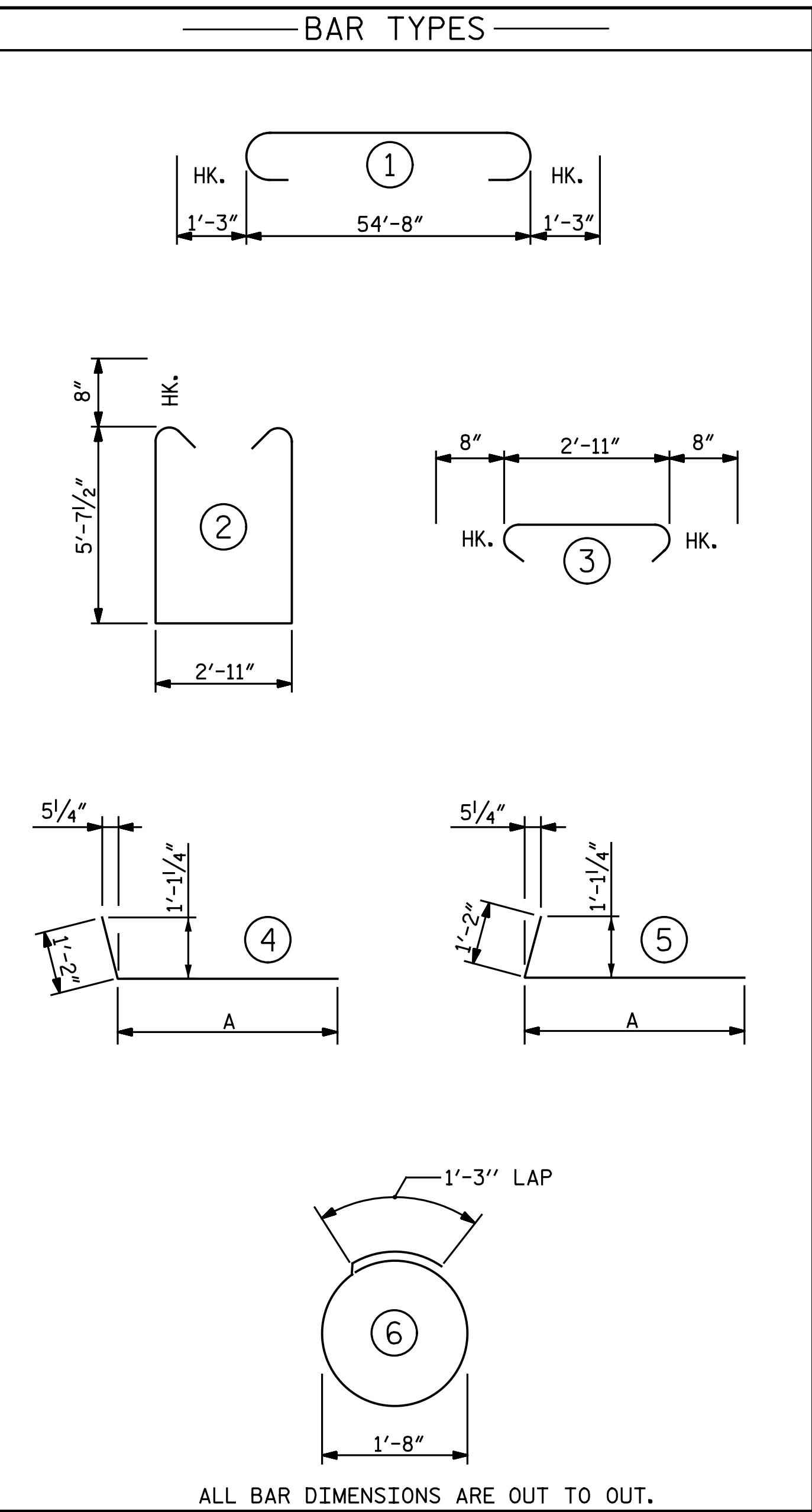
TEMPORARY DRAINAGE AT END BENT

BAR	A	BAR	A
H1	15' - 8"	H41	16' - 1"
H2	15' - 5"	H42	2' - 6"
H3	2' - 6"	H43	15' - 4"
H4	15' - 2"	H44	14' - 7"
H5	14' - 5"	H45	13' - 10"
H6	13' - 8"	H46	13' - 1"
H7	12' - 12"	H47	12' - 4"
H8	12' - 3"	H48	11' - 7"
H9	11' - 7"	H49	10' - 11"
H10	14' - 11"	H50	15' - 6"
H11	14' - 2"	H51	14' - 10"
H12	13' - 5"	H52	14' - 1"
H13	12' - 9"	H53	13' - 4"
H14	11' - 12"	H54	12' - 7"
H15	11' - 4"	H55	11' - 10"
H16	10' - 9"	H56	11' - 2"
H17	10' - 1"	H57	10' - 3"
H18	9' - 5"	H58	9' - 6"
H19	8' - 8"	H59	8' - 10"
H20	7' - 12"	H60	8' - 1"
H21	7' - 4"	H61	7' - 5"
H22	6' - 7"	H62	6' - 9"
H23	5' - 11"	H63	5' - 12"
H24	5' - 3"	H64	5' - 4"
H25	4' - 6"	H65	4' - 8"
H26	3' - 10"	H66	3' - 11"
H27	3' - 2"	H67	3' - 3"
H28	12' - 11"	H68	2' - 7"
H29	12' - 2"	H69	12' - 9"
H30	11' - 6"	H70	12' - 1"
H31	10' - 9"	H71	11' - 5"
H32	10' - 1"	H72	10' - 8"
H33	9' - 5"	H73	9' - 12"
H34	8' - 8"	H74	9' - 4"
H35	7' - 12"	H75	8' - 7"
H36	7' - 4"	H76	7' - 11"
H37	6' - 7"	H77	7' - 3"
H38	5' - 11"	H78	6' - 6"
H39	5' - 3"	H79	5' - 10"
H40	15' - 10"	H80	5' - 2"

NOTE: FOR PILE SPLICE DETAILS, SEE "INTEGRAL END BENT 2 DETAILS" SHEET.

BILL OF MATERIAL						BILL OF MATERIAL					
INTEGRAL END BENT 1						INTEGRAL END BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#9		57' - 2"	1,555	H64	1	#7	4	6' - 6"	13
B2	12	#6	STR.	54' - 10"	988	H65	1	#7	4	5' - 10"	12
B3	8	#4	STR.	28' - 8"	153	H66	1	#7	4	5' - 1"	10
B4	14	#4	STR.	2' - 11"	27	H67	1	#7	4	4' - 5"	9
						H68	1	#7	4	3' - 9"	8
H1	6	#7		16' - 9"	206	H69	1	#7	4	13' - 11"	28
H2	6	#7		16' - 7"	203	H70	1	#7	4	13' - 3"	27
H3	24	#7		3' - 8"	180	H71	1	#7	4	12' - 7"	26
H4	1	#7		16' - 4"	33	H72	1	#7	4	11' - 10"	24
H5	1	#7		15' - 7"	32	H73	1	#7	4	11' - 2"	23
H6	1	#7		14' - 10"	30	H74	1	#7	4	10' - 6"	21
H7	1	#7		14' - 2"	29	H75	1	#7	4	9' - 9"	20
H8	1	#7		13' - 5"	27	H76	1	#7	4	9' - 1"	19
H9	1	#7		12' - 9"	26	H77	1	#7	4	8' - 5"	17
H10	1	#7		16' - 1"	33	H78	1	#7	4	7' - 8"	16
H11	1	#7		15' - 4"	31	H79	1	#7	4	7' - 0"	14
H12	1	#7		14' - 7"	30	H80	1	#7	4	6' - 4"	13
H13	1	#7		13' - 11"	28						
H14	1	#7		13' - 2"	27	S1	55	#6	2	15' - 6"	1,280
H15	1	#7		12' - 6"	26	S2	55	#6	3	4' - 3"	351
H16	1	#7		11' - 11"	24	S3	40	#4	6	6' - 6"	174
H17	1	#7		11' - 3"	23						
H18	1	#7		10' - 7"	22	V1	84	#6	STR.	8' - 6"	1,072
H19	1	#7		9' - 10"	20	V2	16	#5	STR.	11' - 7"	193
H20	1	#7		9' - 2"	19	V3	4	#4	STR.	5' - 7"	15
H21	1	#7		8' - 6"	17	V4	2	#5	STR.	11' - 3"	23
H22	1	#7		7' - 9"	16	V5	2	#5	STR.	10' - 7"	22
H23	1	#7		7' - 1"	14	V6	2	#5	STR.	9' - 10"	21
H24	1	#7		6' - 5"	13	V7	2	#5	STR.	9' - 2"	19
H25	1	#7		5' - 8"	12	V8	2	#5	STR.	8' - 5"	18
H26	1	#7		5' - 0"	10	V9	2	#5	STR.	7' - 9"	16
H27	1	#7		4' - 4"	9	V10	2	#5	STR.	7' - 0"	15
H28	1	#7		14' - 1"	29	V11	2	#5	STR.	6' - 3"	13
H29	1	#7		13' - 4"	27	V12	2	#5	STR.	5' - 7"	12
H30	1	#7		12' - 8"	26	V13	2	#5	STR.	4' - 10"	10
H31	1	#7		11' - 11"	24	V14	2	#5	STR.	4' - 2"	9
H32	1	#7		11' - 3"	23	V15	2	#5	STR.	3' - 5"	7
H33	1	#7		10' - 7"	22	V16	2	#5	STR.	2' - 8"	6
H34	1	#7		9' - 10"	20	V17	16	#5	STR.	12' - 3"	204
H35	1	#7		9' - 2"	19	V18	2	#5	STR.	12' - 1"	25
H36	1	#7		8' - 6"	17	V19	2	#5	STR.	11' - 4"	24
H37	1	#7		7' - 9"	16	V20	2	#5	STR.	10' - 7"	22
H38	1	#7		7' - 1"	14	V21	2	#5	STR.	9' - 11"	21
H39	1	#7		6' - 5"	13	V22	2	#5	STR.	9' - 2"	19
H40	6	#7		17' - 0"	208	V23	2	#5	STR.	8' - 5"	18
H41	6	#7		17' - 3"	212	V24	2	#5	STR.	7' - 9"	16
H42	26	#7		3' - 8"	195	V25	2	#5	STR.	7' - 0"	15
H43	1	#7		16' - 6"	34	V26	2	#5	STR.	6' - 4"	13
H44	1	#7		15' - 9"	32	V27	2	#5	STR.	5' - 7"	12
H45	1	#7		15' - 0"	31	V28	2	#5	STR.	4' - 10"	10
H46	1	#7		14' - 3"	29	V29	2	#5	STR.	4' - 2"	9
H47	1	#7		13' - 6"	28	V30	2	#5	STR.	3' - 5"	7
H48	1	#7		12' - 9"	26	V31	2	#4	STR.	2' - 8"	4
H49	1	#7		12' - 1"	25						
H50	1	#7		16' - 8"	34						

REINFORCING STEEL	LBS.	9,269
CLASS A CONCRETE		
POUR 1 - CAP, LOWER PART OF WINGS & COLLARS	C.Y.	44.6
POUR 2 - UPPER PART OF WINGS	C.Y.	8.1
TOTAL	C.Y.	52.7
PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES	EA.	10
HP 12x53 STEEL PILES		
NO. 10	L.F.	850
PILE REDRIVES	EA.	5



ALL BAR DIMENSIONS ARE OUT TO OUT.

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 166+72.51 -L-



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE
INTEGRAL END BENT 1
DETAILS

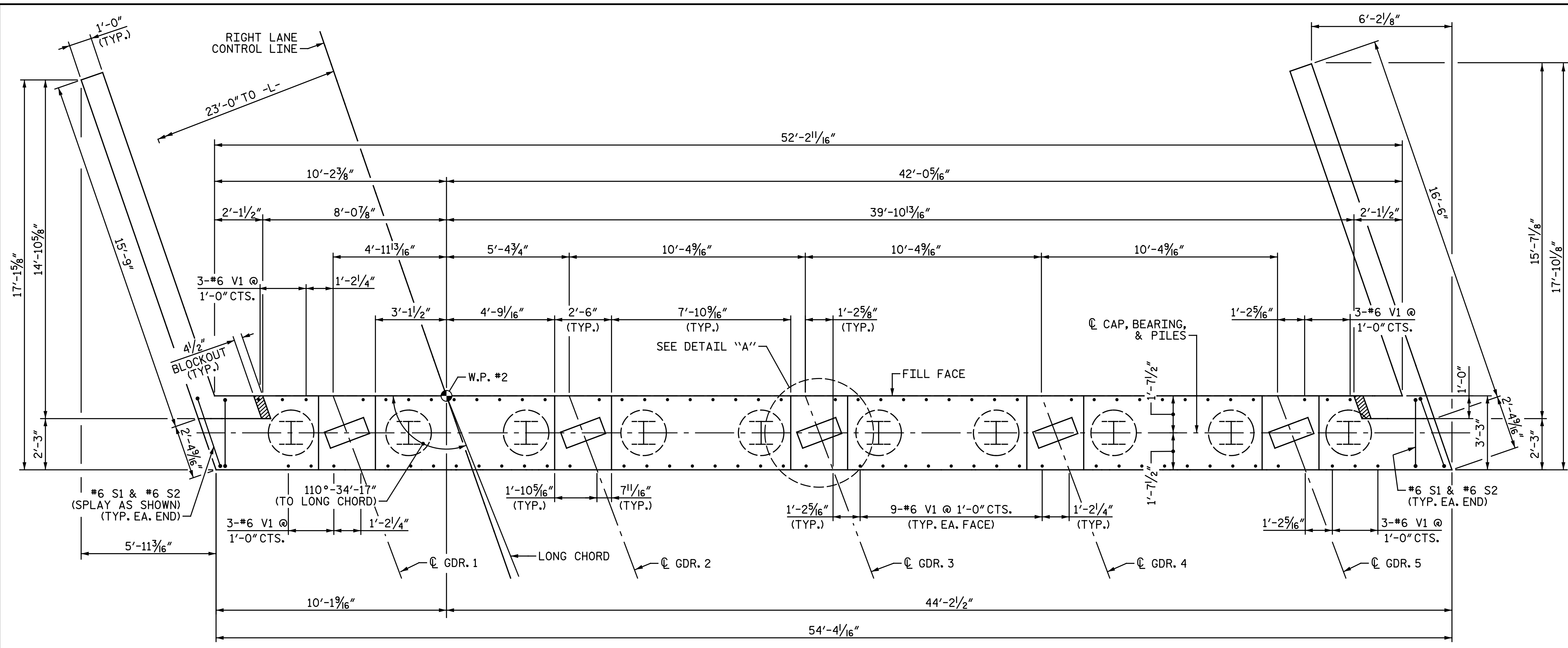
RIGHT LANE

8/14/2017
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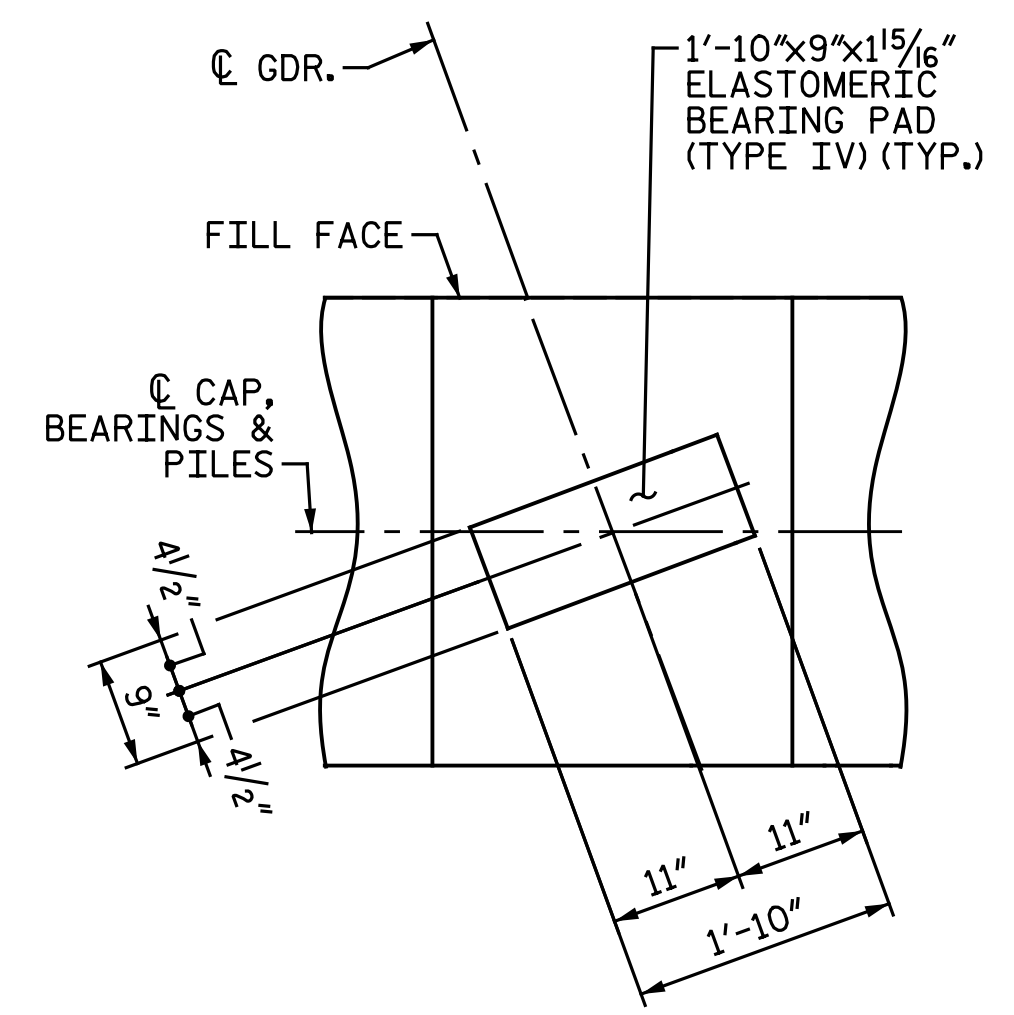
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NC License No.: F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-23
1			3			TOTAL SHEETS
2			4			31

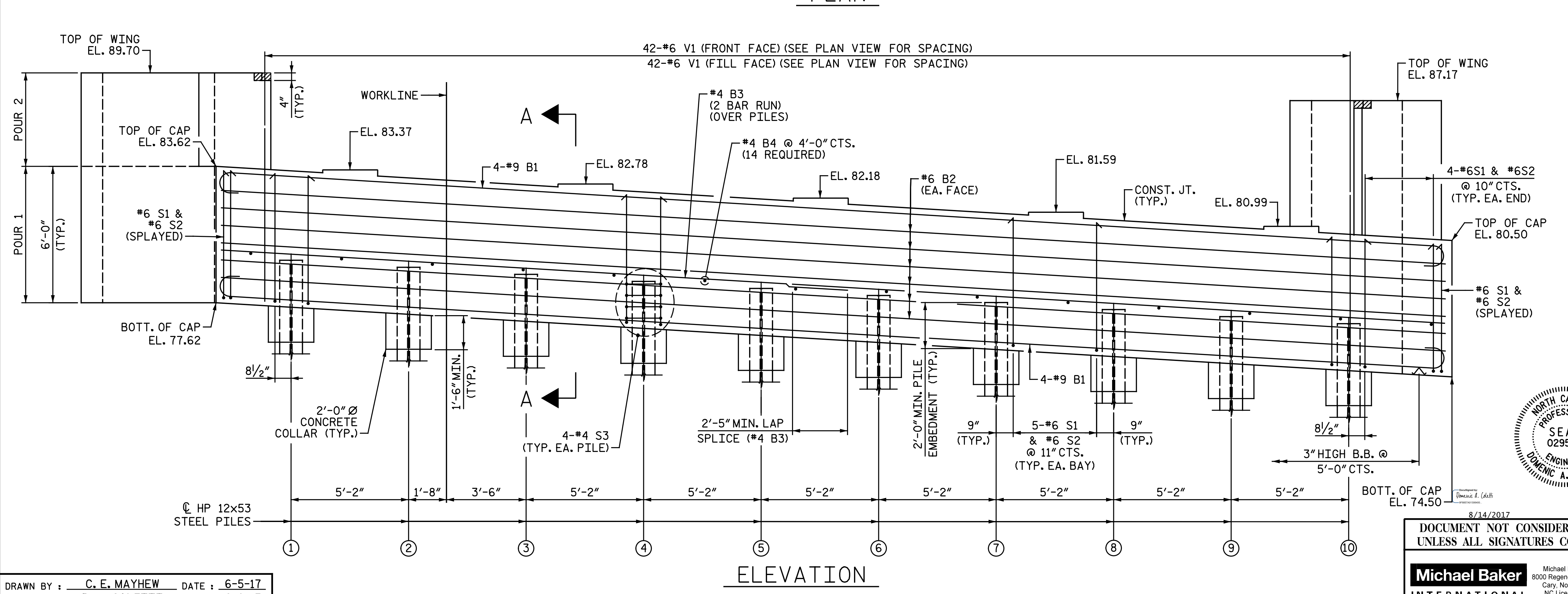
DRAWN BY: C. E. MAYHEW DATE: 6-6-17
CHECKED BY: D. A. COLETTI DATE: 6-9-17



NOTES:
 FOR "SECTION A-A", SEE "INTEGRAL END BENT 1 DETAILS" SHEET.
 STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #6 V1 BARS.
 THE TOP SURFACE OF THE END BENT CAP, EXCLUDING THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4\"/>

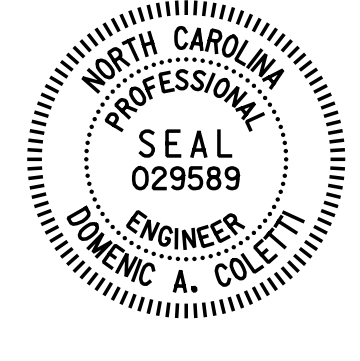


DETAIL "A"
 ALL DIMENSIONS AND DETAILS SHOWN ARE TYPICAL FOR ALL BEARINGS AT EACH BRIDGE SEAT LOCATION.



TOP OF PILE ELEVATIONS	
PILE	ELEVATION
①	79.40
②	79.10
③	78.81
④	78.51
⑤	78.21
⑥	77.92
⑦	77.62
⑧	77.32
⑨	77.03
⑩	76.73

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
 SHEET 1 OF 3



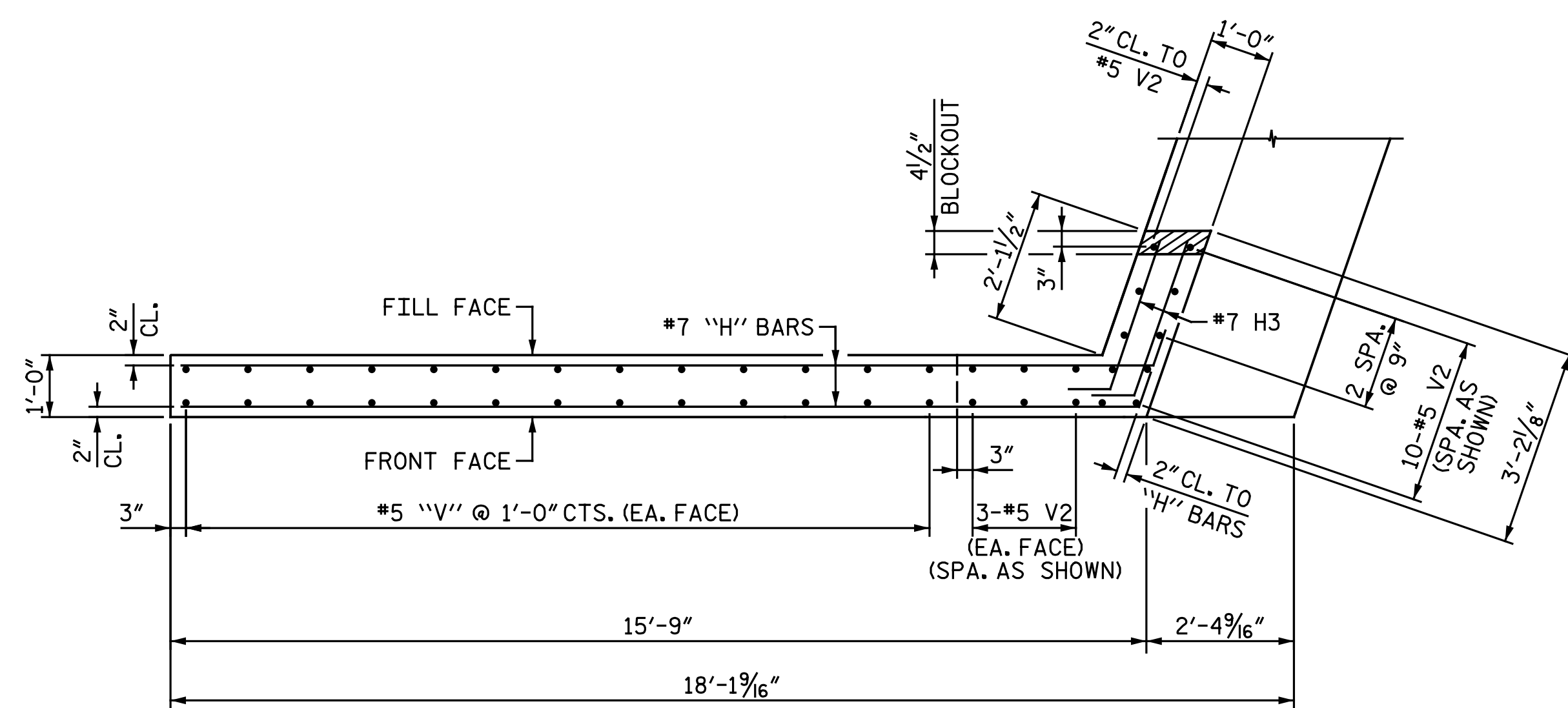
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
INTEGRAL END BENT 2
 RIGHT LANE

DRAWN BY: C. E. MAYHEW DATE: 6-5-17
 CHECKED BY: D. A. COLETTI DATE: 6-9-17

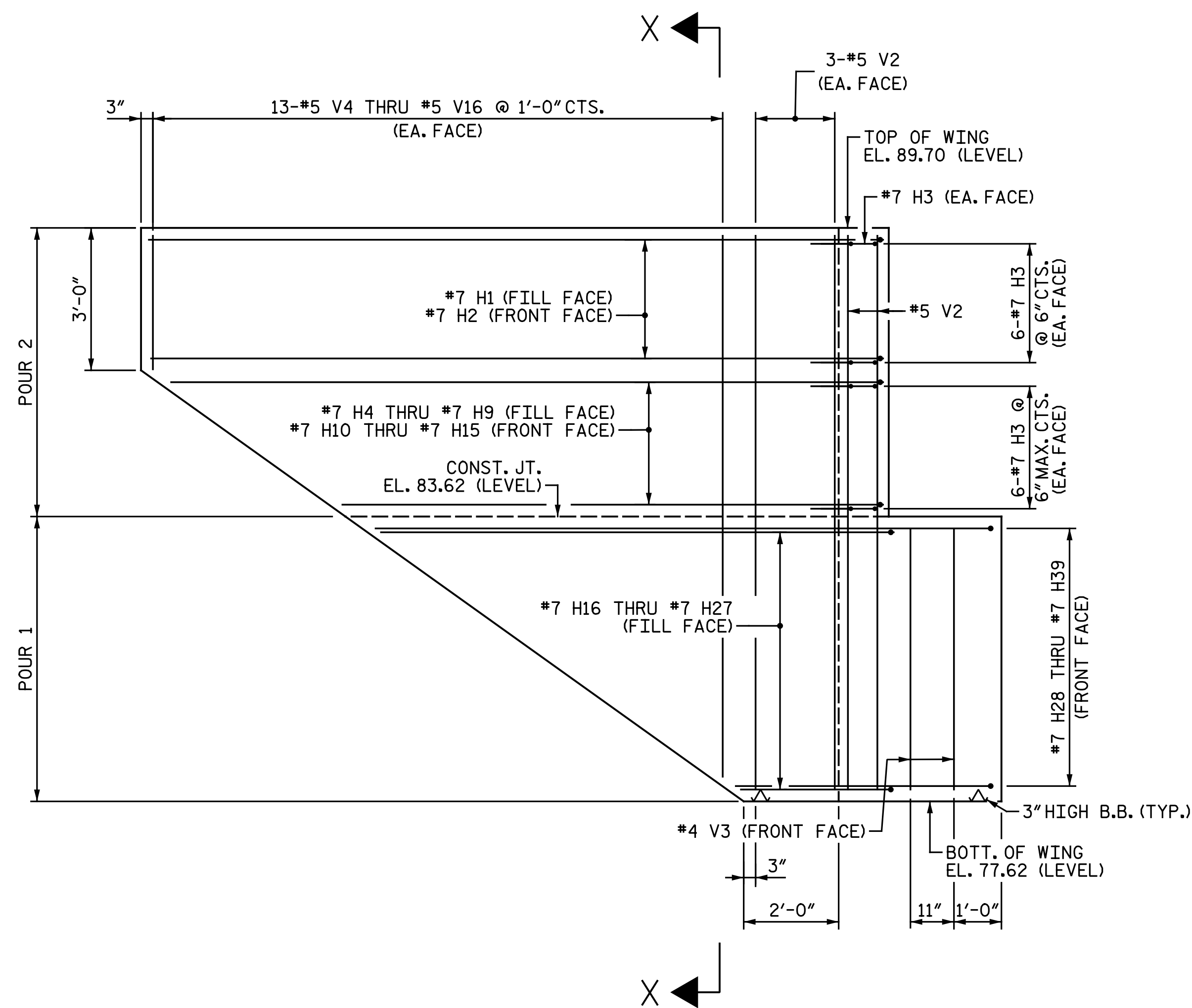
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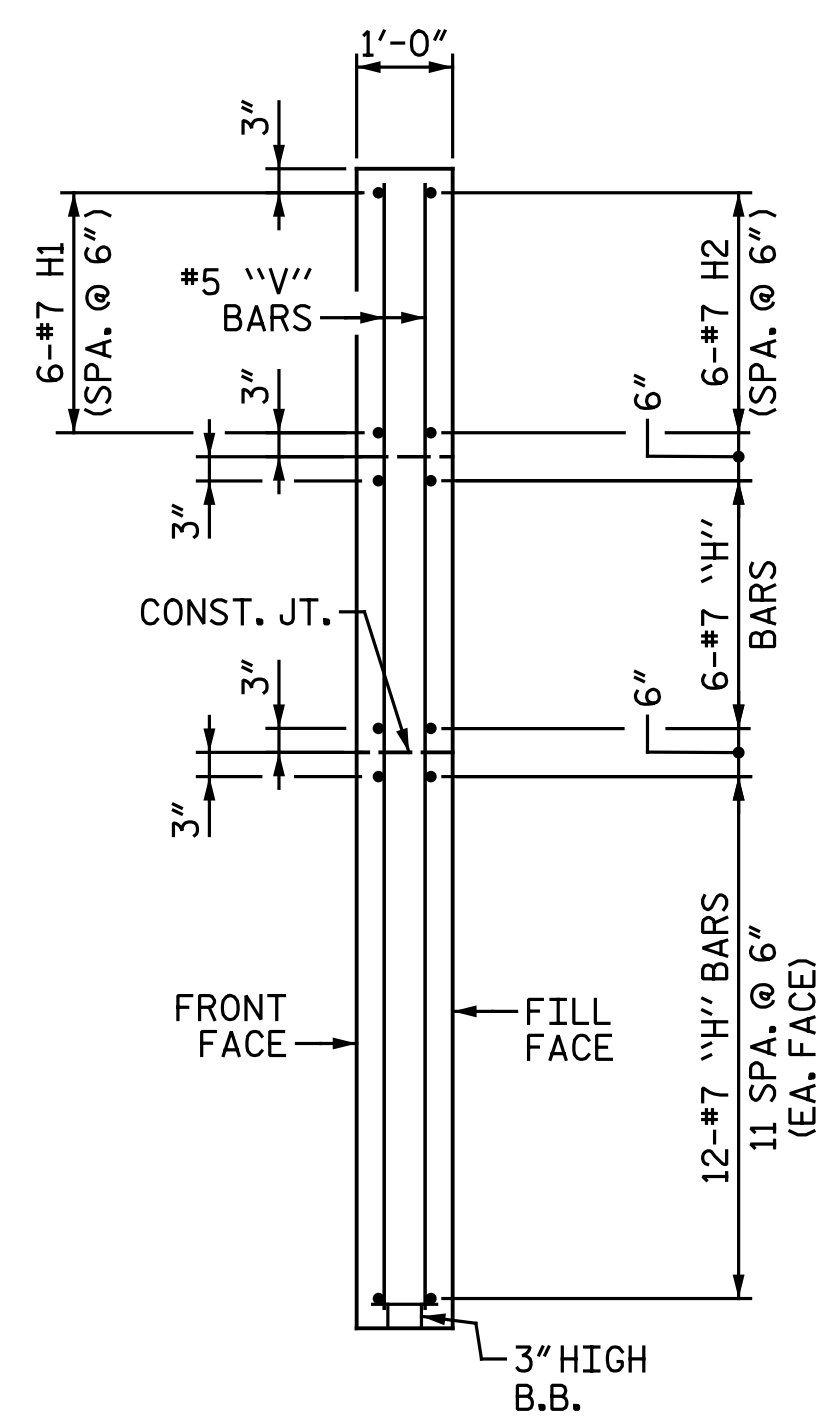
REVISIONS						SHEET NO. S4-24
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 31
2			4			



PLAN OF LEFT WING



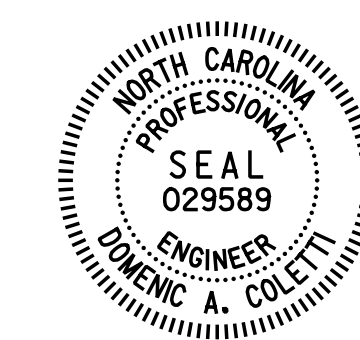
ELEVATION OF LEFT WING



SECTION X-X

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 2 OF 3



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
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 INTEGRAL END BENT 2

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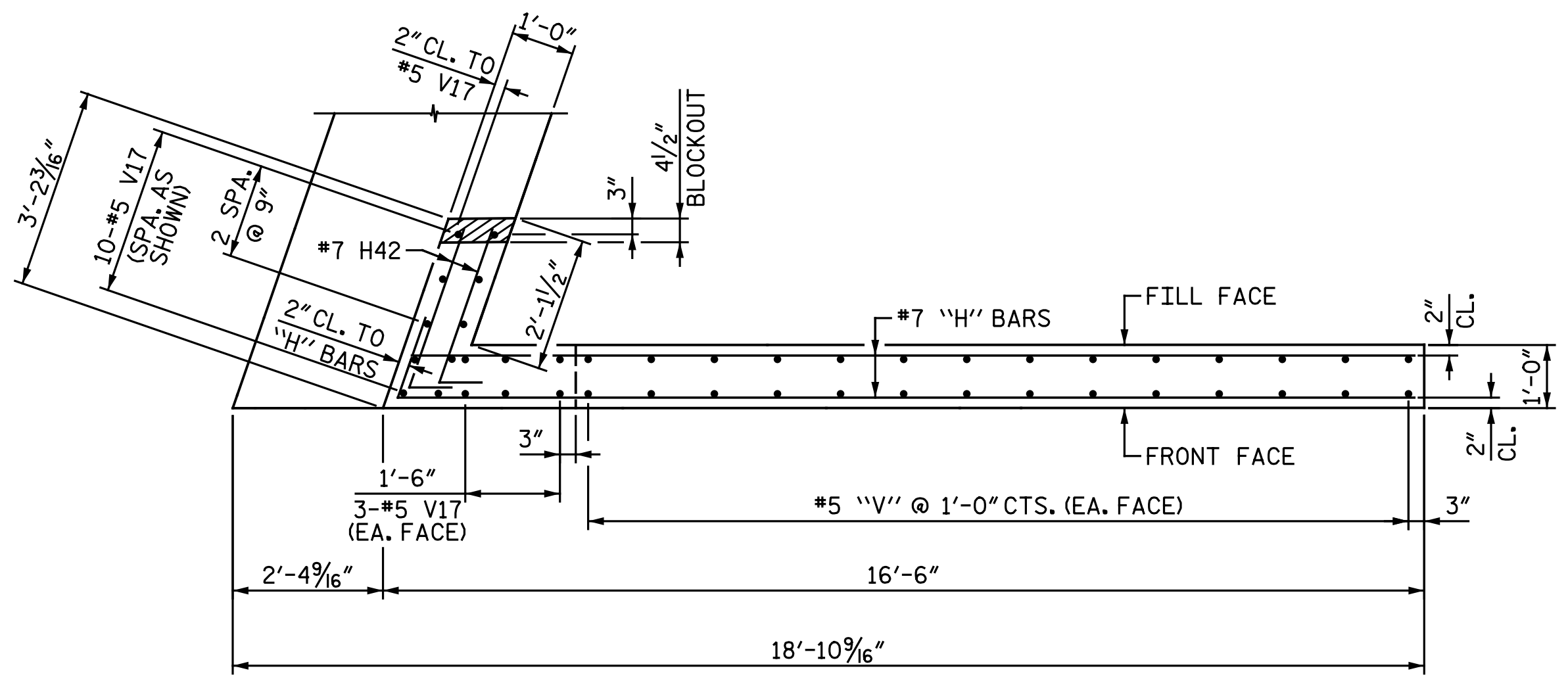
RIGHT LANE

DRAWN BY : C. E. MAYHEW DATE : 4-14-17
 CHECKED BY : D. A. COLETTI DATE : 6-9-17

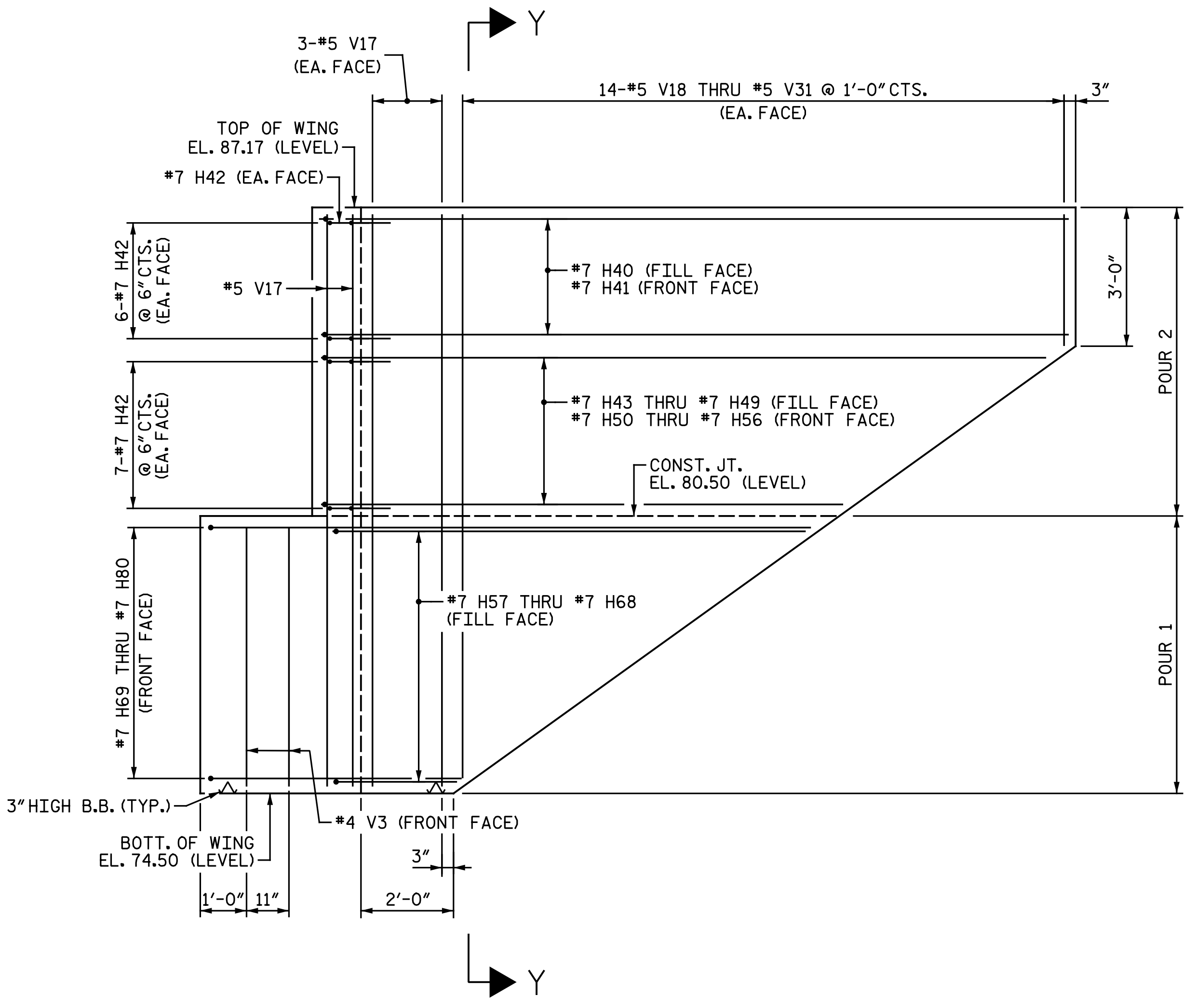
REVISIONS		SHEET NO.	
NO.	DATE	NO.	DATE
1		3	
2		4	

TOTAL SHEETS: 31

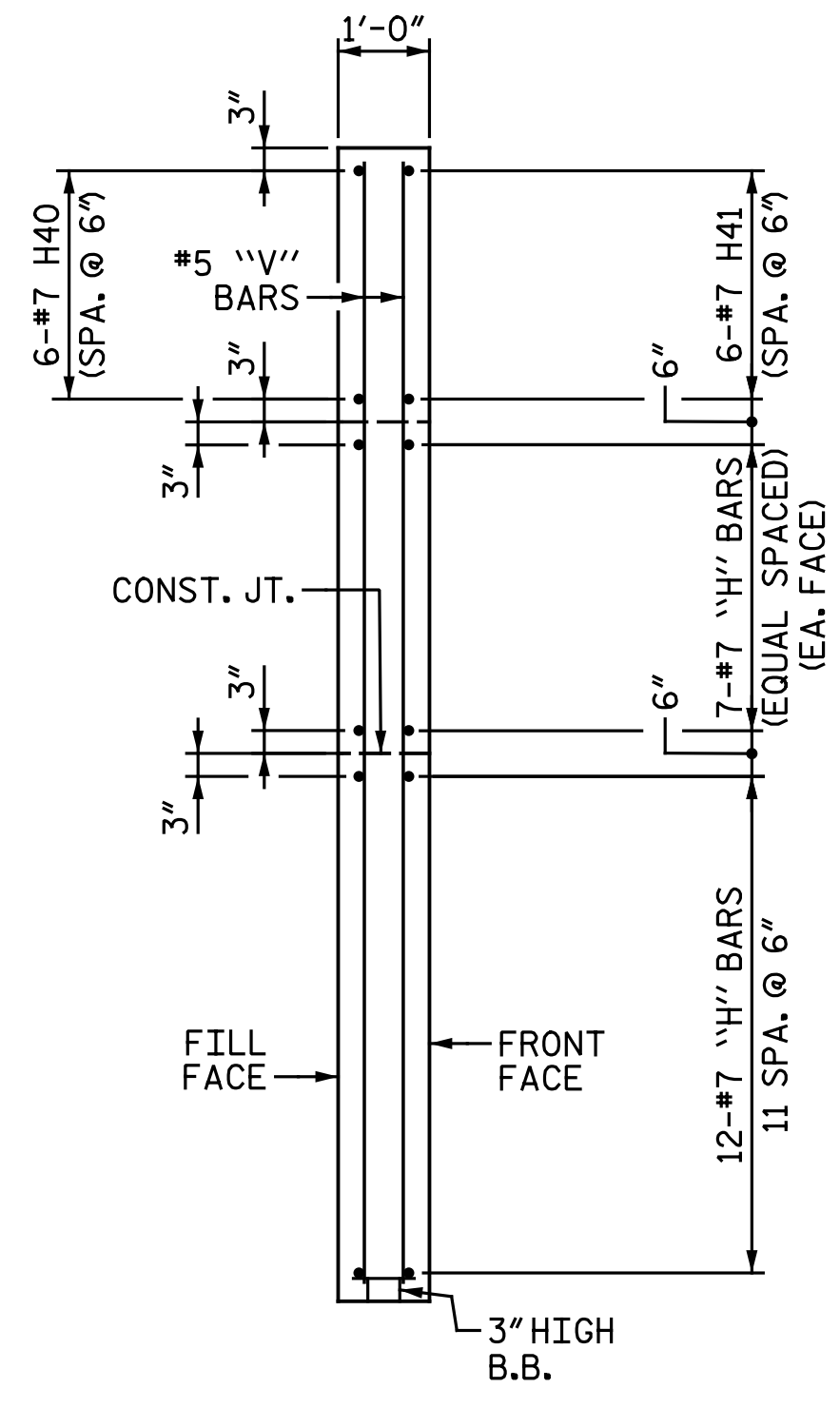
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 Cary, North Carolina 27518
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PLAN OF RIGHT WING



ELEVATION OF RIGHT WING



SECTION Y-Y

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-
 SHEET 3 OF 3



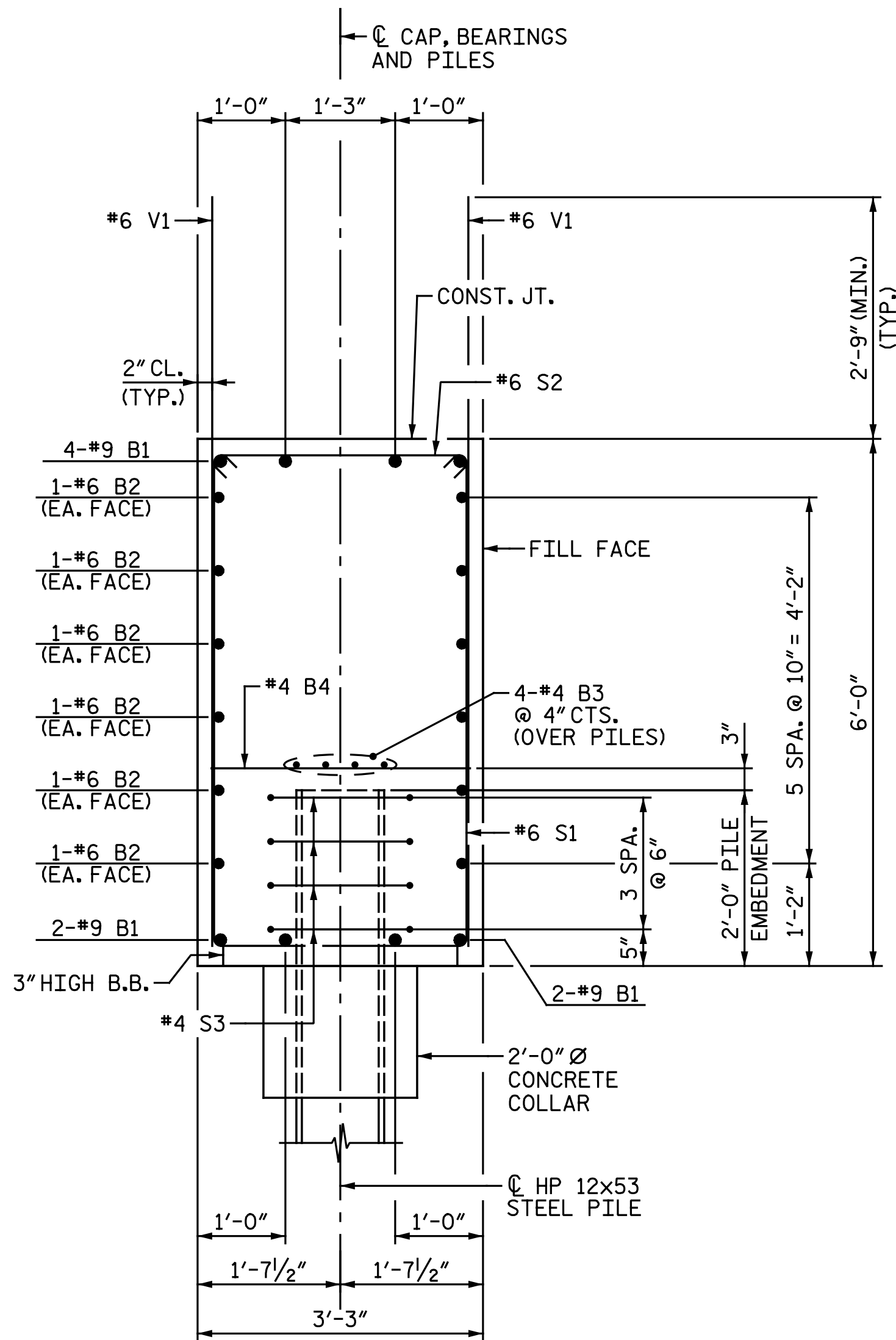
STATE OF NORTH CAROLINA
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 INTEGRAL END BENT 2

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 Cary, North Carolina 27518
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RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

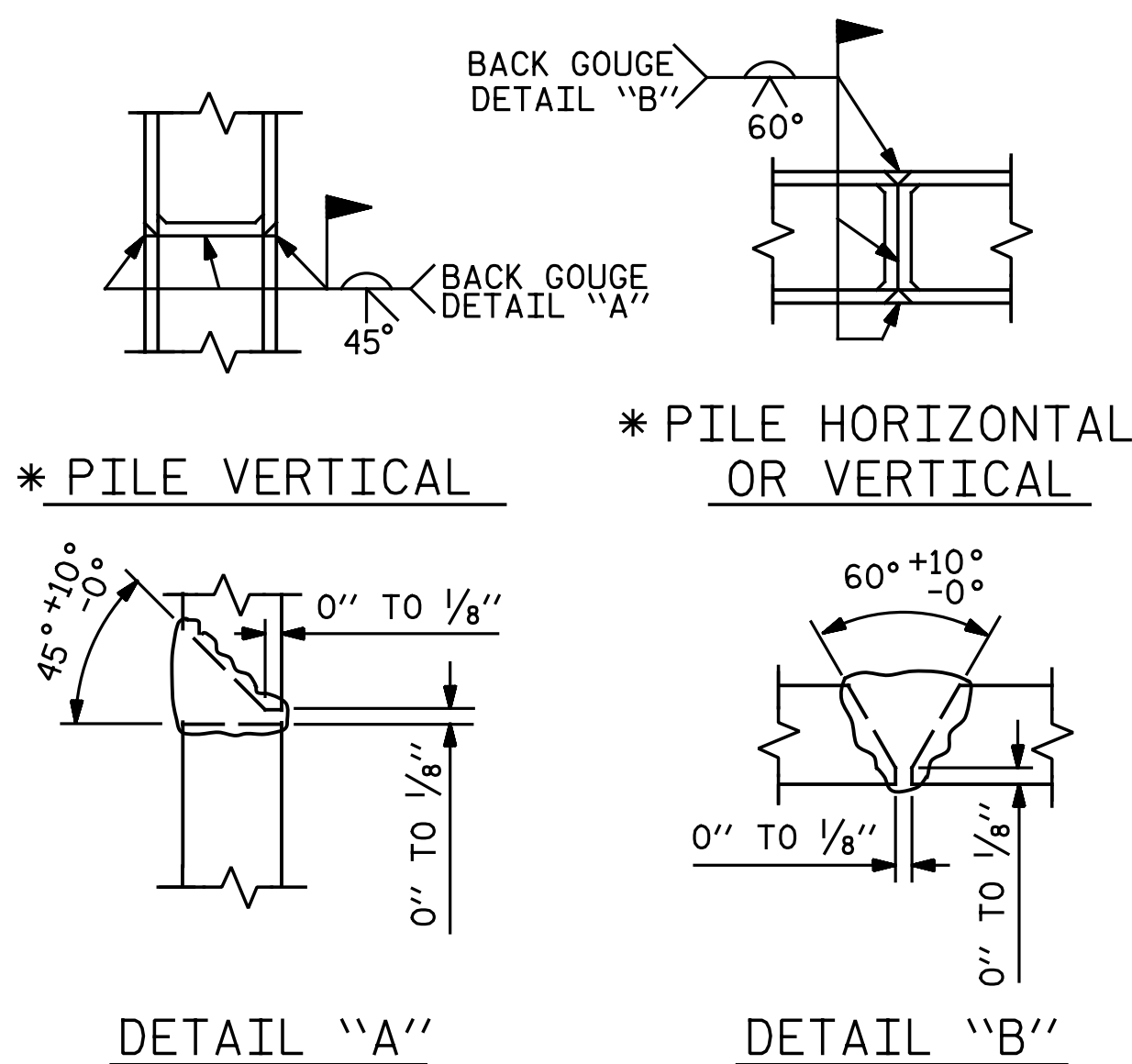
SHEET NO.	S4-26
TOTAL SHEETS	31

DRAWN BY : C. E. MAYHEW DATE : 6-7-17
 CHECKED BY : D. A. COLETTI DATE : 6-9-17



SECTION A-A

NOTE:
FOR TEMPORARY DRAINAGE AT END BENT DETAILS, SEE "INTEGRAL END BENT 1 DETAILS" SHEET.



PILE SPLICE DETAILS

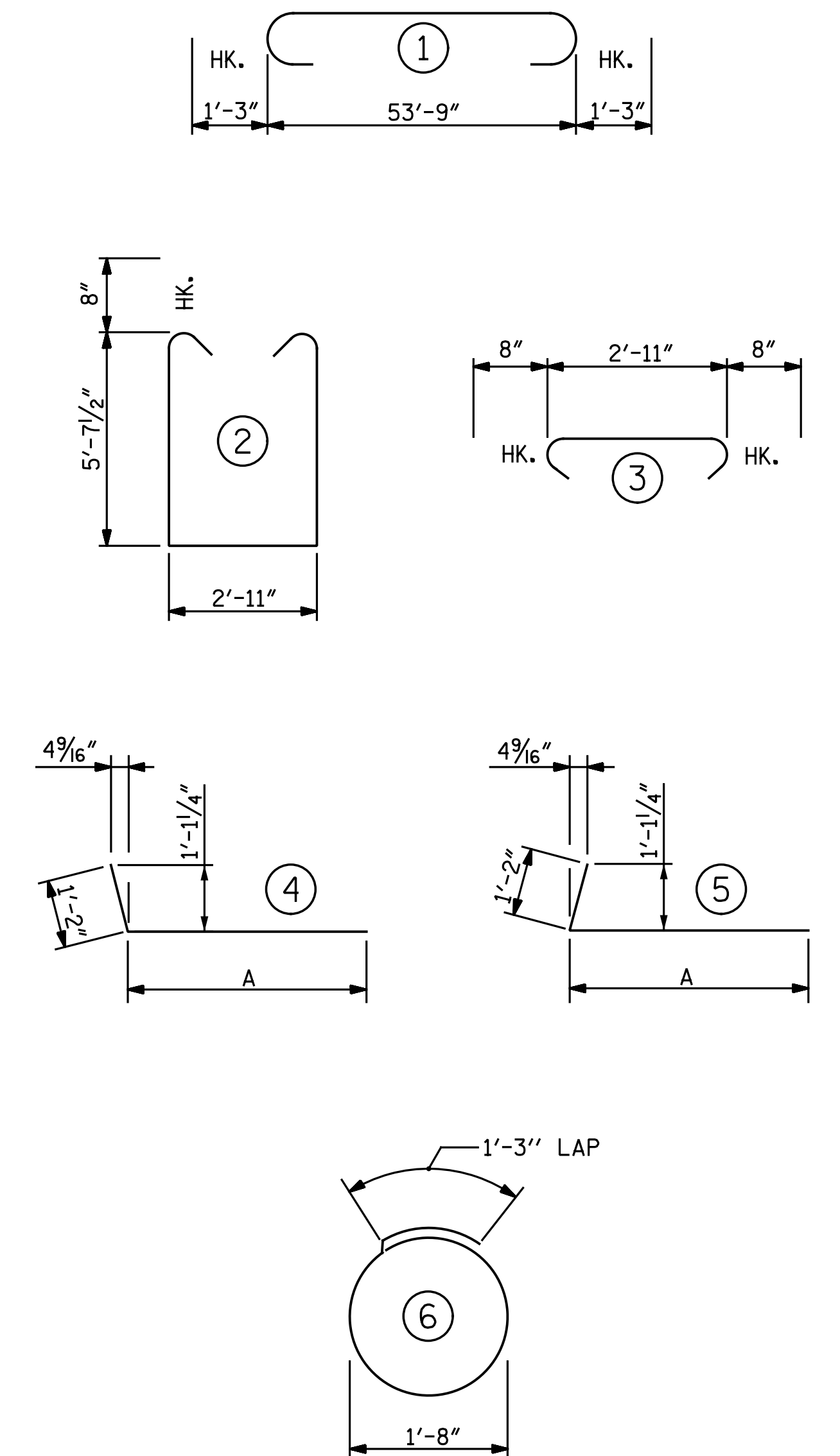
* POSITION OF PILE DURING WELDING

BAR	A	BAR	A
H1	15' - 8"	H41	16' - 1"
H2	15' - 5"	H42	2' - 6"
H3	2' - 6"	H43	15' - 4"
H4	15' - 2"	H44	14' - 7"
H5	14' - 5"	H45	13' - 10"
H6	13' - 8"	H46	13' - 1"
H7	12' - 12"	H47	12' - 4"
H8	12' - 3"	H48	11' - 7"
H9	11' - 7"	H49	10' - 11"
H10	14' - 11"	H50	15' - 6"
H11	14' - 2"	H51	14' - 10"
H12	13' - 5"	H52	14' - 1"
H13	12' - 9"	H53	13' - 4"
H14	11' - 12"	H54	12' - 7"
H15	11' - 4"	H55	11' - 10"
H16	10' - 9"	H56	11' - 2"
H17	10' - 1"	H57	10' - 3"
H18	9' - 5"	H58	9' - 6"
H19	8' - 8"	H59	8' - 10"
H20	7' - 12"	H60	8' - 1"
H21	7' - 4"	H61	7' - 5"
H22	6' - 7"	H62	6' - 9"
H23	5' - 11"	H63	5' - 12"
H24	5' - 3"	H64	5' - 4"
H25	4' - 6"	H65	4' - 8"
H26	3' - 10"	H66	3' - 11"
H27	3' - 2"	H67	3' - 3"
H28	12' - 11"	H68	2' - 7"
H29	12' - 2"	H69	12' - 9"
H30	11' - 6"	H70	12' - 1"
H31	10' - 9"	H71	11' - 5"
H32	10' - 1"	H72	10' - 8"
H33	9' - 5"	H73	9' - 12"
H34	8' - 8"	H74	9' - 4"
H35	7' - 12"	H75	8' - 7"
H36	7' - 4"	H76	7' - 11"
H37	6' - 7"	H77	7' - 3"
H38	5' - 11"	H78	6' - 6"
H39	5' - 3"	H79	5' - 10"
H40	15' - 10"	H80	5' - 2"

BILL OF MATERIAL						BILL OF MATERIAL					
INTEGRAL END BENT 2						INTEGRAL END BENT 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#9	1	56' - 3"	1,530	H64	1	#7	4	6' - 6"	13
B2	12	#6	STR.	53' - 11"	972	H65	1	#7	4	5' - 10"	12
B3	8	#4	STR.	28' - 3"	151	H66	1	#7	4	5' - 1"	10
B4	14	#4	STR.	2' - 11"	27	H67	1	#7	4	4' - 5"	9
						H68	1	#7	4	3' - 9"	8
H1	6	#7	5	16' - 9"	206	H69	1	#7	4	13' - 11"	28
H2	6	#7	5	16' - 7"	203	H70	1	#7	4	13' - 3"	27
H3	24	#7	5	3' - 8"	180	H71	1	#7	4	12' - 7"	26
H4	1	#7	5	16' - 4"	33	H72	1	#7	4	11' - 10"	24
H5	1	#7	5	15' - 7"	32	H73	1	#7	4	11' - 2"	23
H6	1	#7	5	14' - 10"	30	H74	1	#7	4	10' - 6"	21
H7	1	#7	5	14' - 2"	29	H75	1	#7	4	9' - 9"	20
H8	1	#7	5	13' - 5"	27	H76	1	#7	4	9' - 1"	19
H9	1	#7	5	12' - 9"	26	H77	1	#7	4	8' - 5"	17
H10	1	#7	5	16' - 1"	33	H78	1	#7	4	7' - 8"	16
H11	1	#7	5	15' - 4"	31	H79	1	#7	4	7' - 0"	14
H12	1	#7	5	14' - 7"	30	H80	1	#7	4	6' - 4"	13
H13	1	#7	5	13' - 11"	28						
H14	1	#7	5	13' - 2"	27	S1	55	#6	2	15' - 6"	1,280
H15	1	#7	5	12' - 6"	26	S2	55	#6	3	4' - 3"	351
H16	1	#7	5	11' - 11"	24	S3	40	#4	6	6' - 6"	174
H17	1	#7	5	11' - 3"	23						
H18	1	#7	5	10' - 7"	22	V1	84	#6	STR.	8' - 6"	1,072
H19	1	#7	5	9' - 10"	20	V2	16	#5	STR.	11' - 7"	193
H20	1	#7	5	9' - 2"	19	V3	4	#4	STR.	5' - 7"	15
H21	1	#7	5	8' - 6"	17	V4	2	#5	STR.	11' - 3"	23
H22	1	#7	5	7' - 9"	16	V5	2	#5	STR.	10' - 7"	22
H23	1	#7	5	7' - 1"	14	V6	2	#5	STR.	9' - 10"	21
H24	1	#7	5	6' - 5"	13	V7	2	#5	STR.	9' - 2"	19
H25	1	#7	5	5' - 8"	12	V8	2	#5	STR.	8' - 5"	18
H26	1	#7	5	5' - 0"	10	V9	2	#5	STR.	7' - 9"	16
H27	1	#7	5	4' - 4"	9	V10	2	#5	STR.	7' - 0"	15
H28	1	#7	5	14' - 1"	29	V11	2	#5	STR.	6' - 3"	13
H29	1	#7	5	13' - 4"	27	V12	2	#5	STR.	5' - 7"	12
H30	1	#7	5	12' - 8"	26	V13	2	#5	STR.	4' - 10"	10
H31	1	#7	5	11' - 11"	24	V14	2	#5	STR.	4' - 2"	9
H32	1	#7	5	11' - 3"	23	V15	2	#5	STR.	3' - 5"	7
H33	1	#7	5	10' - 7"	22	V16	2	#5	STR.	2' - 8"	6
H34	1	#7	5	9' - 10"	20	V17	16	#5	STR.	12' - 3"	204
H35	1	#7	5	9' - 2"	19	V18	2	#5	STR.	12' - 1"	25
H36	1	#7	5	8' - 6"	17	V19	2	#5	STR.	11' - 4"	24
H37	1	#7	5	7' - 9"	16	V20	2	#5	STR.	10' - 7"	22
H38	1	#7	5	7' - 1"	14	V21	2	#5	STR.	9' - 11"	21
H39	1	#7	5	6' - 5"	13	V22	2	#5	STR.	9' - 2"	19
H40	6	#7	5	17' - 0"	208	V23	2	#5	STR.	8' - 5"	18
H41	6	#7	5	17' - 3"	212	V24	2	#5	STR.	7' - 9"	16
H42	26	#7	5	3' - 8"	195	V25	2	#5	STR.	7' - 0"	15
H43	1	#7	5	16' - 6"	34	V26	2	#5	STR.	6' - 4"	13
H44	1	#7	4	15' - 9"	32	V27	2	#5	STR.	5' - 7"	12
H45	1	#7	4	15' - 0"	31	V28	2	#5	STR.	4' - 10"	10
H46	1	#7	4	14' - 3"	29	V29	2	#5	STR.	4' - 2"	9
H47	1	#7	4	13' - 6"	28	V30	2	#5	STR.	3' - 5"	7
H48	1	#7	4	12' - 9"	26	V31	2	#4	STR.	2' - 8"	4
H49	1	#7	4	12' - 1"	25						
H50	1	#7	4	16' - 8"	34						
H51	1	#7	4	16' - 0"	33						
H52	1	#7	4	15' - 3"	31						
H53	1	#7	4	14' - 6"	30						
H54	1	#7	4	13' - 9"	28						
H55	1	#7	4	13' - 0"	27						
H56	1	#7	4	12' - 4"	25						
H57	1	#7	4	11' - 5"	23						
H58	1	#7	4	10' - 8"	22						
H59	1	#7	4	10' - 0"	20						
H60	1	#7	4	9' - 3"	19						
H61	1	#7	4	8' - 7"	18						
H62	1	#7	4	7' - 11"	16						
H63	1	#7	4	7' - 2"	15						

REINFORCING STEEL	LBS.	9,226
CLASS A CONCRETE		
POUR 1 -		
CAP, LOWER PART OF WINGS & COLLARS	C.Y.	44.0
POUR 2 -		
UPPER PART OF WINGS	C.Y.	8.1
TOTAL	C.Y.	52.1
PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES	EA.	10
HP 12x53 STEEL PILES NO. 10	L.F.	850
PILE REDRIVES	EA.	5

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT.

PROJECT NO. R-5703
LENOIR COUNTY
STATION: 166+72.51 -L-



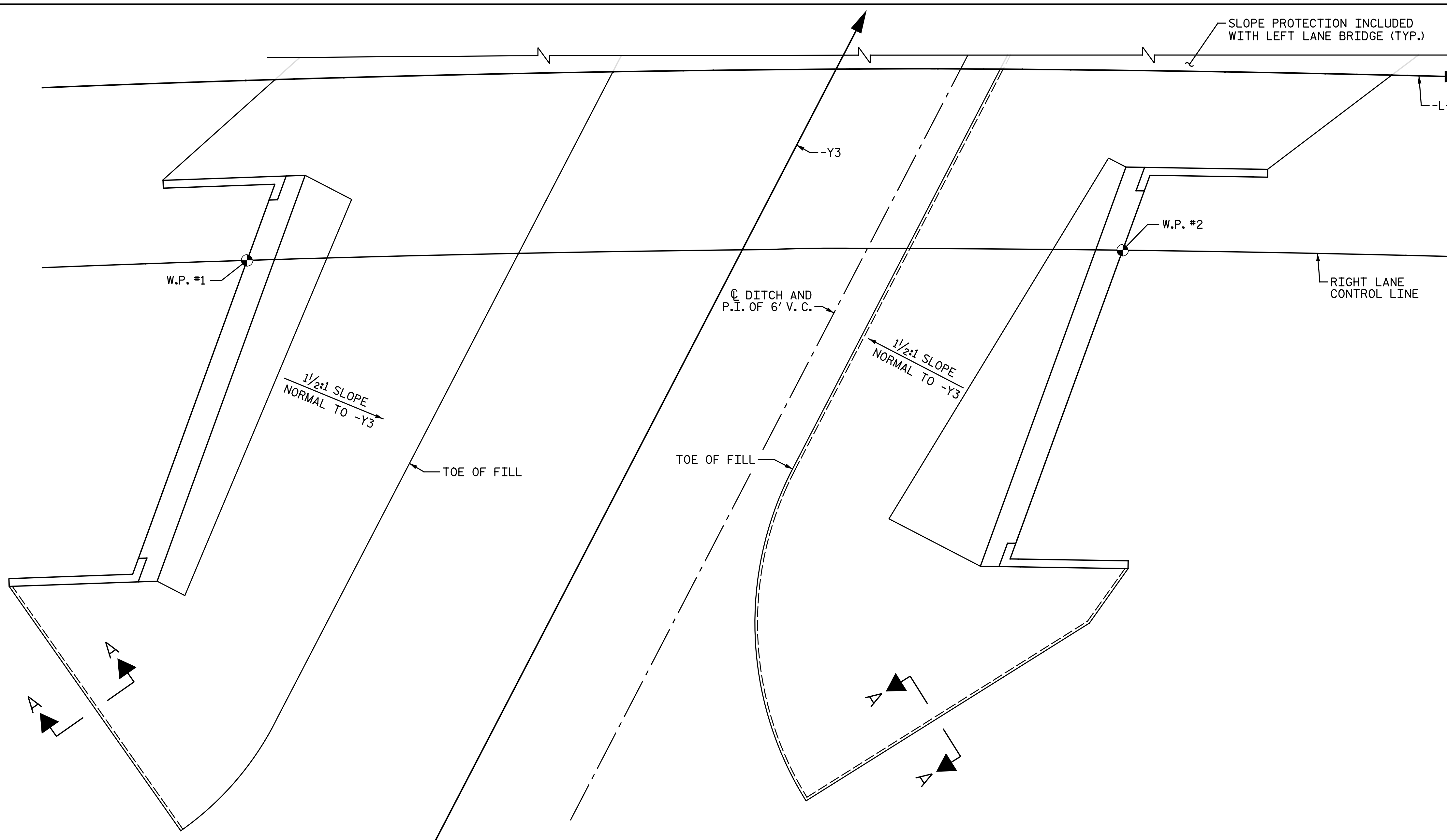
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE
INTEGRAL END BENT 2
DETAILS

RIGHT LANE

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NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS
2			4			31

DRAWN BY: C. E. MAYHEW DATE: 4-14-17
CHECKED BY: D. A. COLETTI DATE: 6-9-17



PLAN

GENERAL NOTES:

STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT.

MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.

FOR BERM WIDTHS AND ELEVATIONS, SEE GENERAL DRAWING AND "SLOPE PROTECTION DETAILS" SHEET 2 OF 2.

SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET.

CONCRETE SHALL BE CLASS "B".

THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED.

WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE.

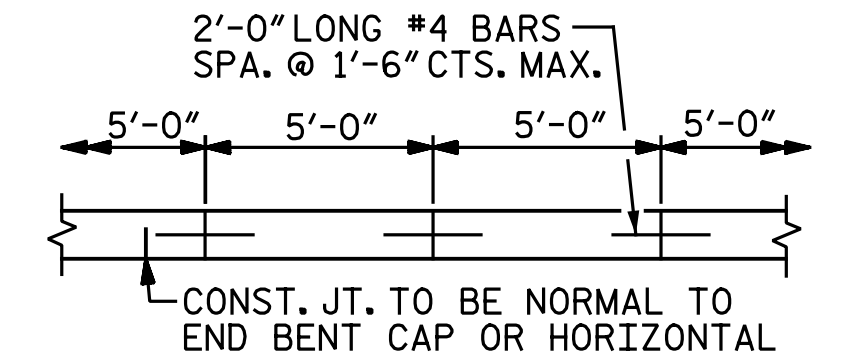
SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0" LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING.

SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6".

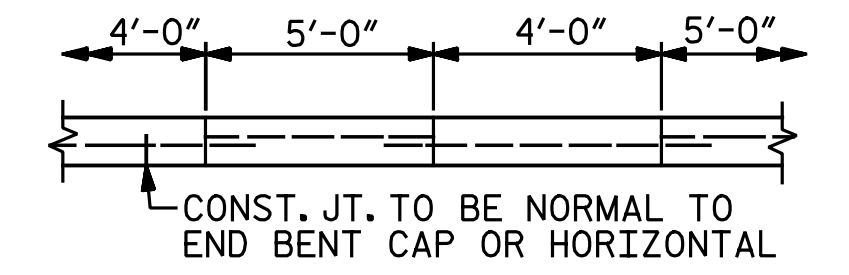
THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA. 166+72.51 -L- (RIGHT LANE)	4 INCH SLOPE PROTECTION	WELDED WIRE FABRIC 60 INCHES WIDE *
	SQUARE YARDS	APPROX. L.F.
END BENT 1	470	850
END BENT 2	400	720

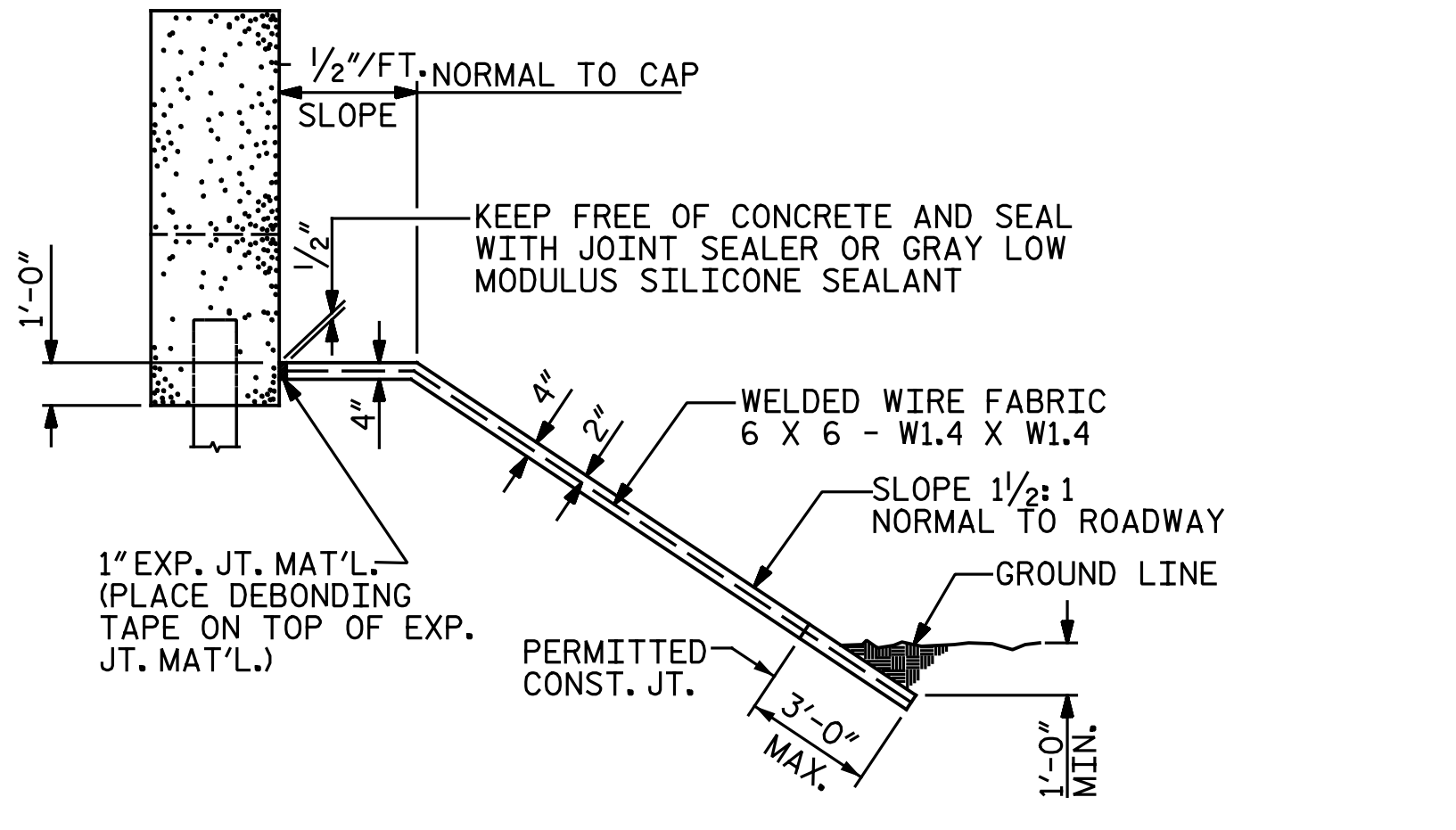
* QUANTITY SHOWN IS BASED ON 5' POURS.



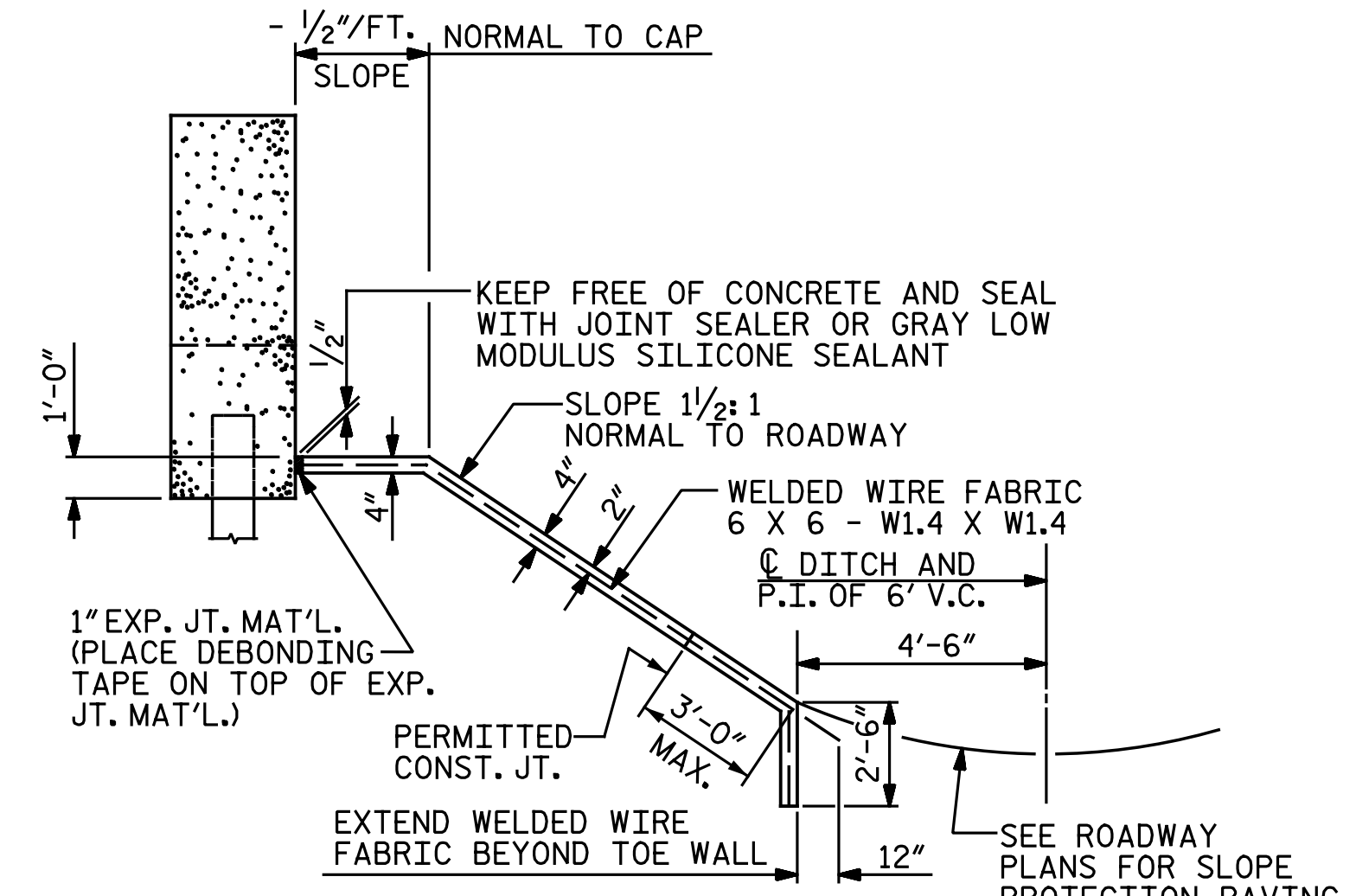
POURING DETAIL



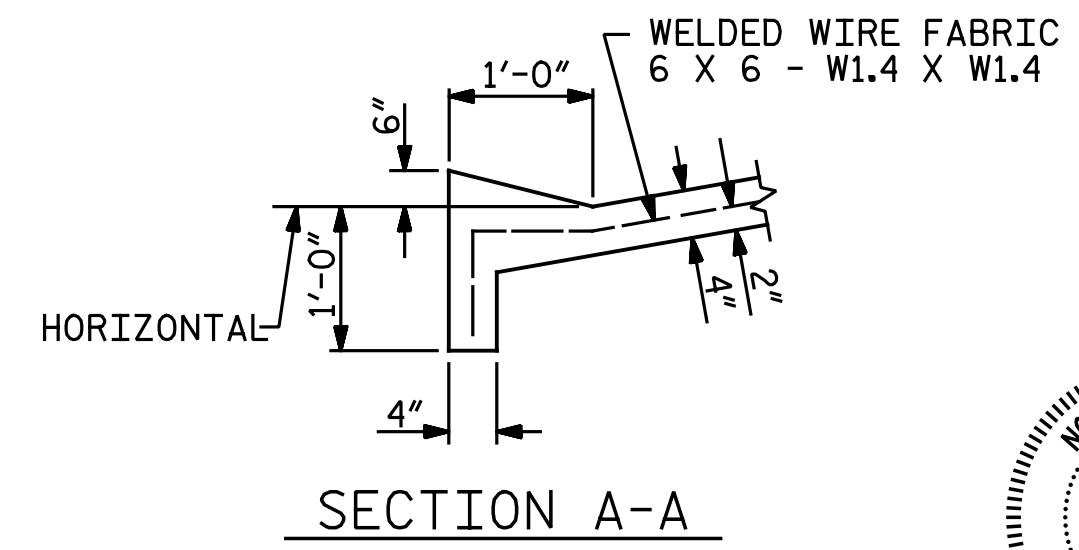
OPTIONAL POURING DETAIL



SECTION A-A ALONG C SURVEY WHEN DITCH IS NOT PROVIDED



SECTION A-A ALONG C SURVEY WHEN FILL CATCHES IN DITCH



SECTION A-A



PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 166+72.51 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

STANDARD
 SLOPE PROTECTION
 DETAILS

RIGHT LANE

ASSEMBLED BY : M. D. MAYHEW	DATE : 6-15-17
CHECKED BY : D. A. COLETTI	DATE : 6-16-17
DRAWN BY : ELR 5/92	REV. 10/1/11 MAA/GM
CHECKED BY : GRP 6/92	REV. 12/21/11 MAA/GM
	REV. 1/16 MAA/TMG

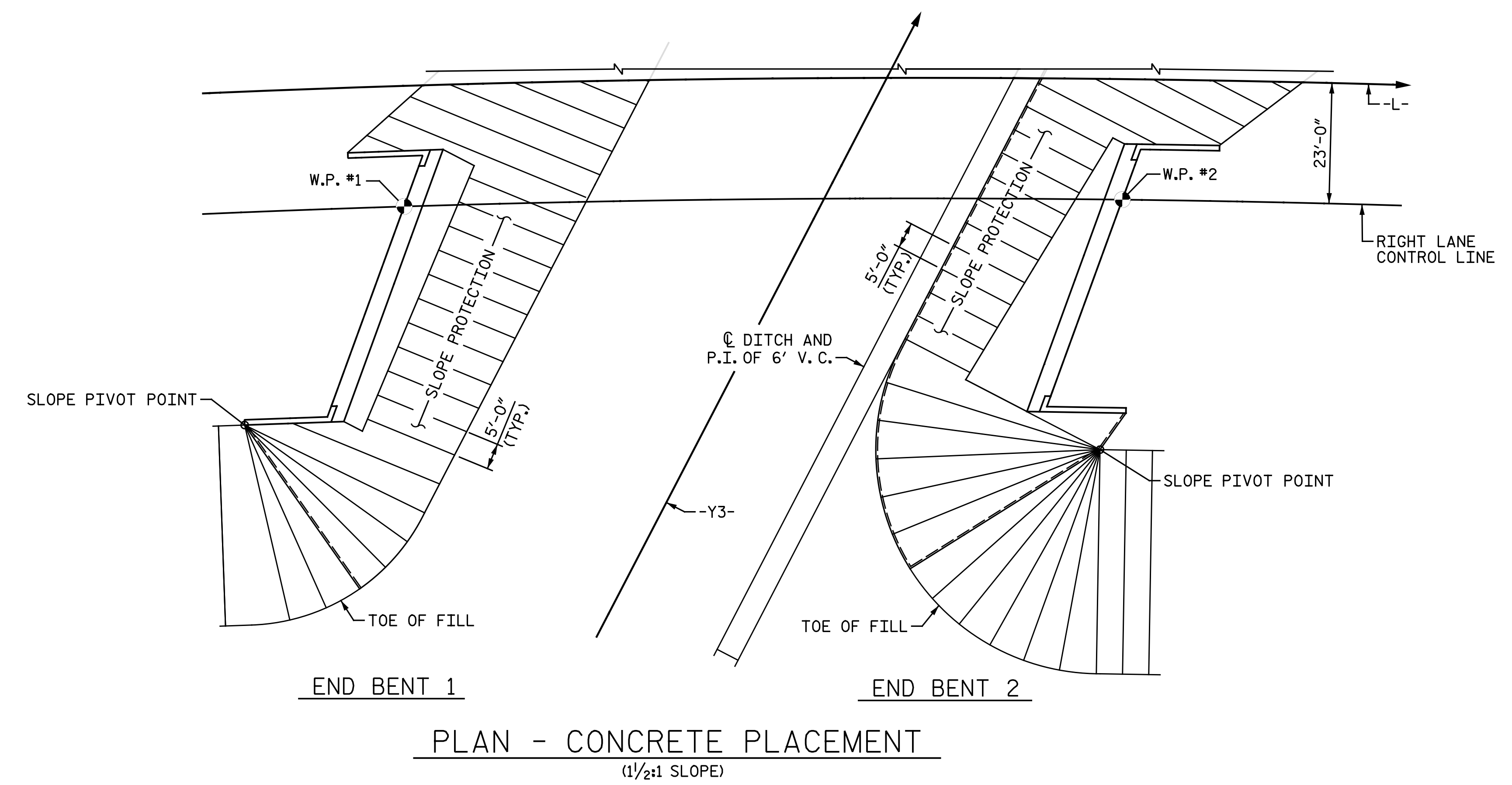
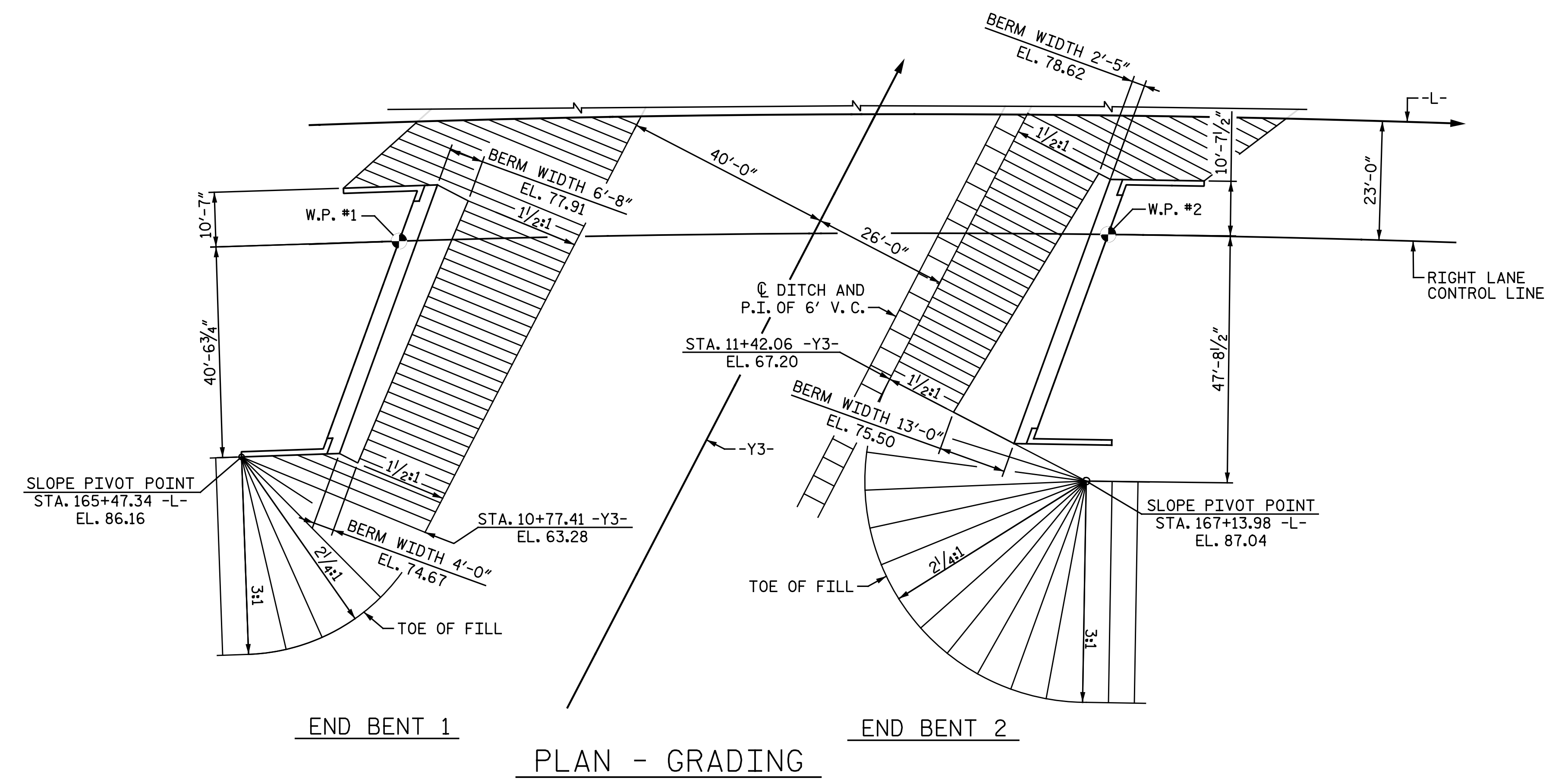
8/14/2017
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 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No. : F-1084

REVISIONS						SHEET NO. S4-28
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 31
2			4			

NOTE:
ALL ELEVATIONS AND BERM WIDTHS ARE GIVEN AT THE TOP OF CONCRETE SLOPE PROTECTION.



PROJECT NO. R-5703
LENOIR COUNTY
STATION: 166+72.51 -L-
SHEET 2 OF 2

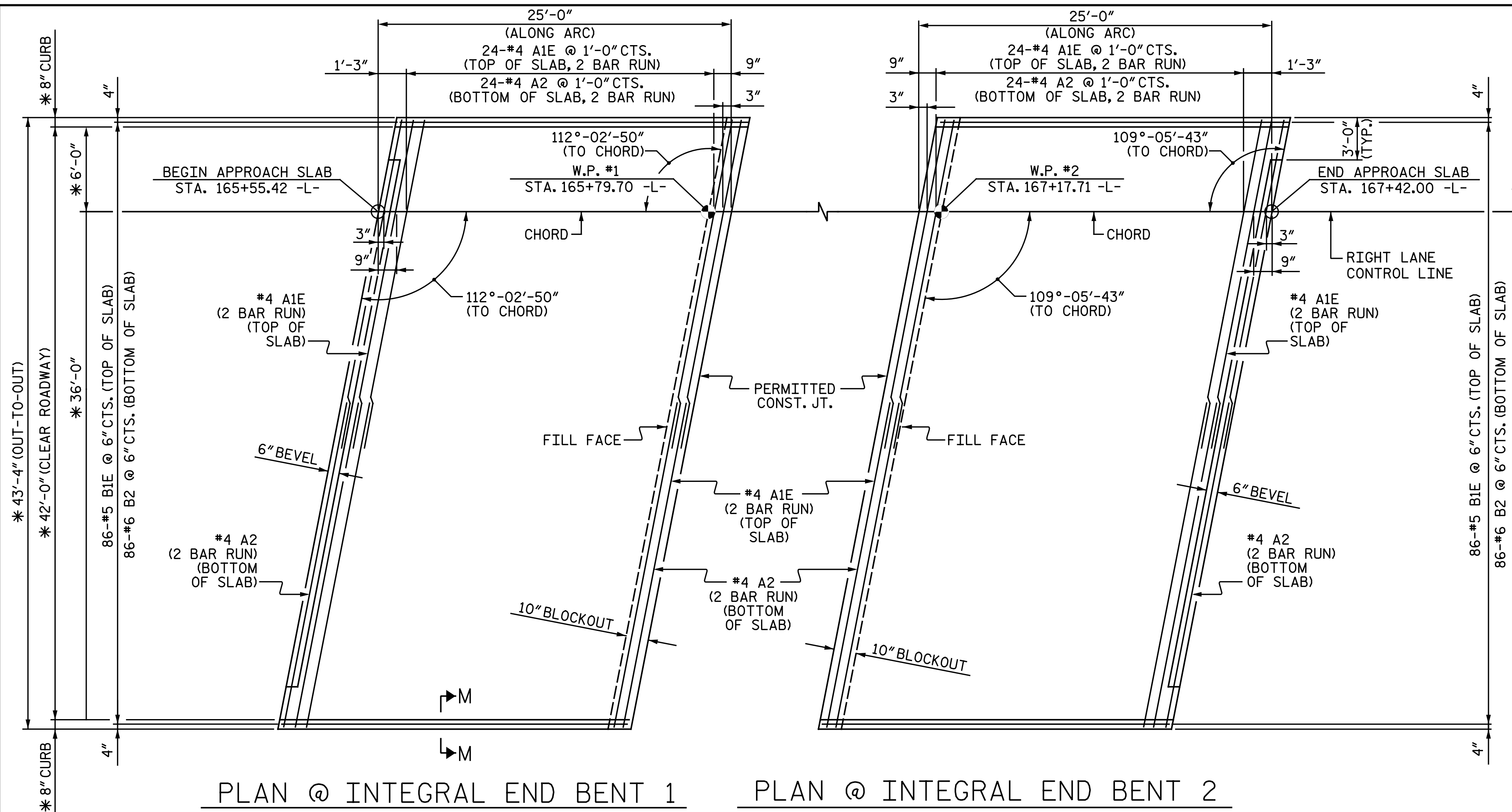


STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
SLOPE PROTECTION
DETAILS
RIGHT LANE

8/14/2017
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NC License No.: F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-29
1			3			TOTAL SHEETS
2			4			31

ASSEMBLED BY : M. D. MAYHEW	DATE : 6-15-17
CHECKED BY : D. A. COLETTI	DATE : 6-16-17
DRAWN BY : WJH 10/88	REV. 5/1/06 TLA/GM
CHECKED BY : FCJ 10/88	REV. 10/1/11 MAA/GM
	REV. 1/16 MAA/TMG



PLAN @ INTEGRAL END BENT 1

PLAN @ INTEGRAL END BENT 2

NOTES:

AT THE CONTRACTOR'S OPTION, THE APPROACH SLAB MAY BE CAST MONOLITHICALLY WITH THE INTEGRAL END BENT DIAPHRAGM AND THE END SECTION OF BRIDGE DECK. IF CAST WITH THE INTEGRAL DIAPHRAGM, THE LAYERS OF ROOFING FELT SHALL BE OMITTED. IF CAST SEPARATE FROM THE INTEGRAL DIAPHRAGM, APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

#78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

#78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

BILL OF MATERIAL

APPROACH SLAB AT END BENT 1

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1E	52	#4	STR.	24' - 2"	839
A2	52	#4	STR.	24' - 0"	834
B1E	86	#5	STR.	24' - 2"	2,168
B2	86	#6	STR.	24' - 8"	3,186

REINFORCING STEEL	LBS.	4,020
EPOXY COATED REINFORCING STEEL	LBS.	3,007
CLASS AA CONCRETE	C.Y.	46.8

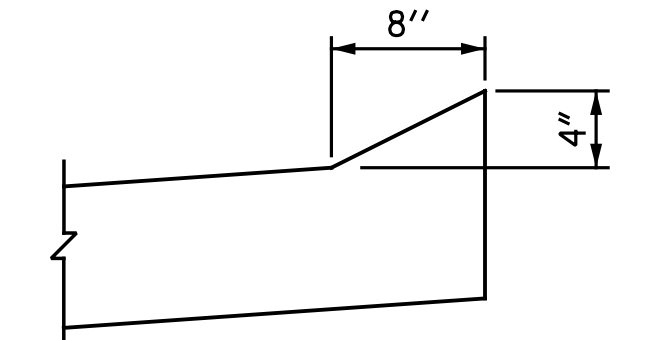
APPROACH SLAB AT END BENT 2

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1E	52	#4	STR.	24' - 2"	839
A2	52	#4	STR.	24' - 0"	834
B1E	86	#5	STR.	24' - 2"	2,168
B2	86	#6	STR.	24' - 8"	3,186

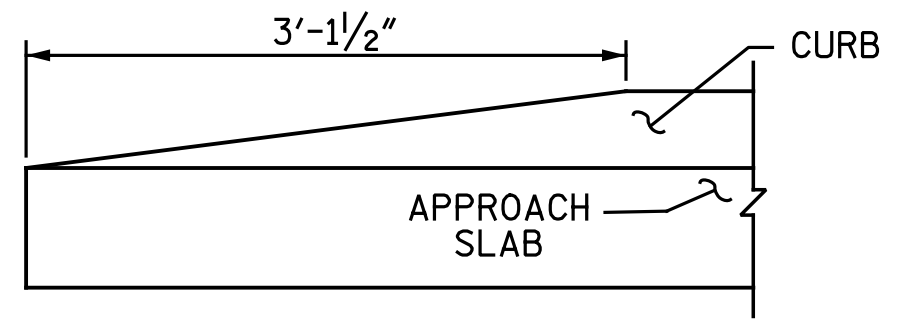
REINFORCING STEEL	LBS.	4,020
EPOXY COATED REINFORCING STEEL	LBS.	3,007
CLASS AA CONCRETE	C.Y.	46.8

SPLICE LENGTHS

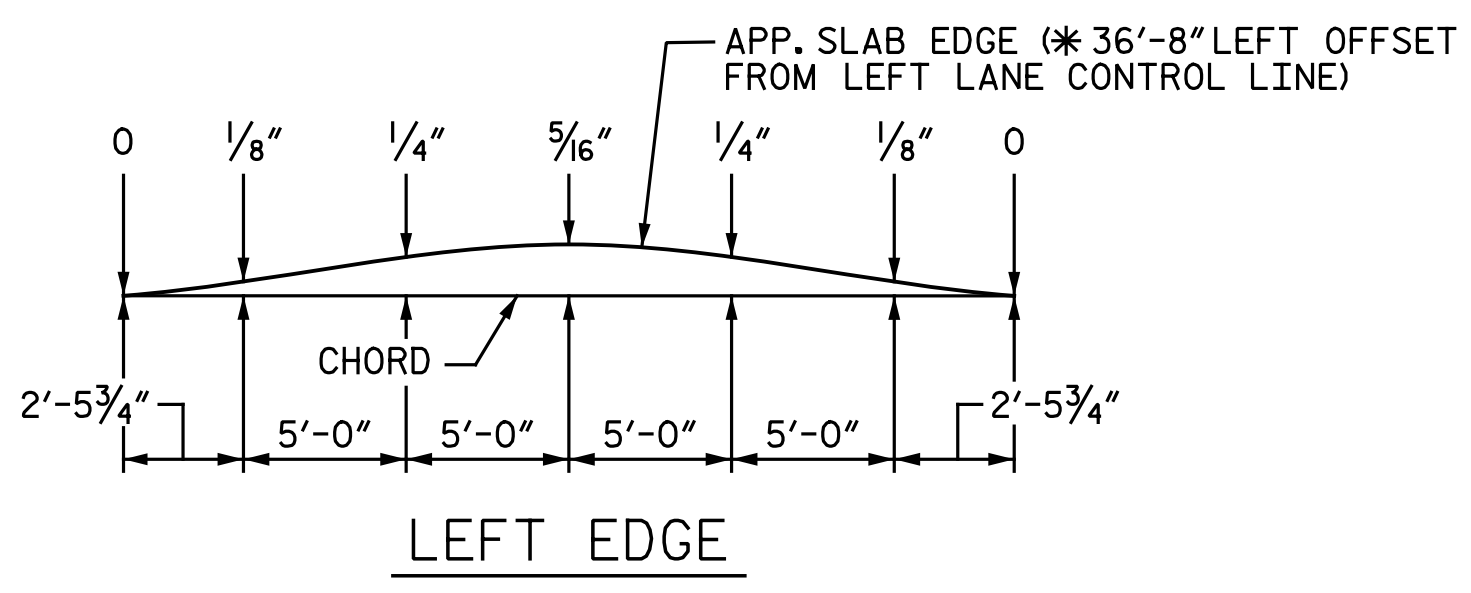
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"



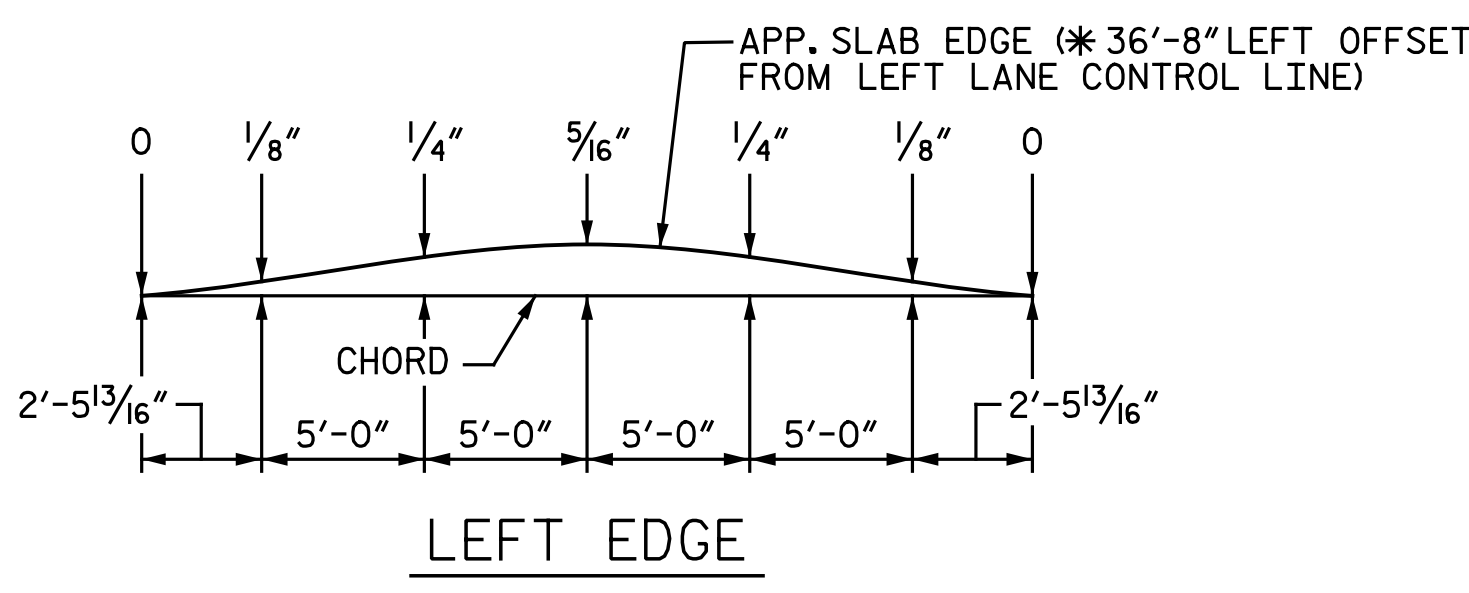
SECTION M-M



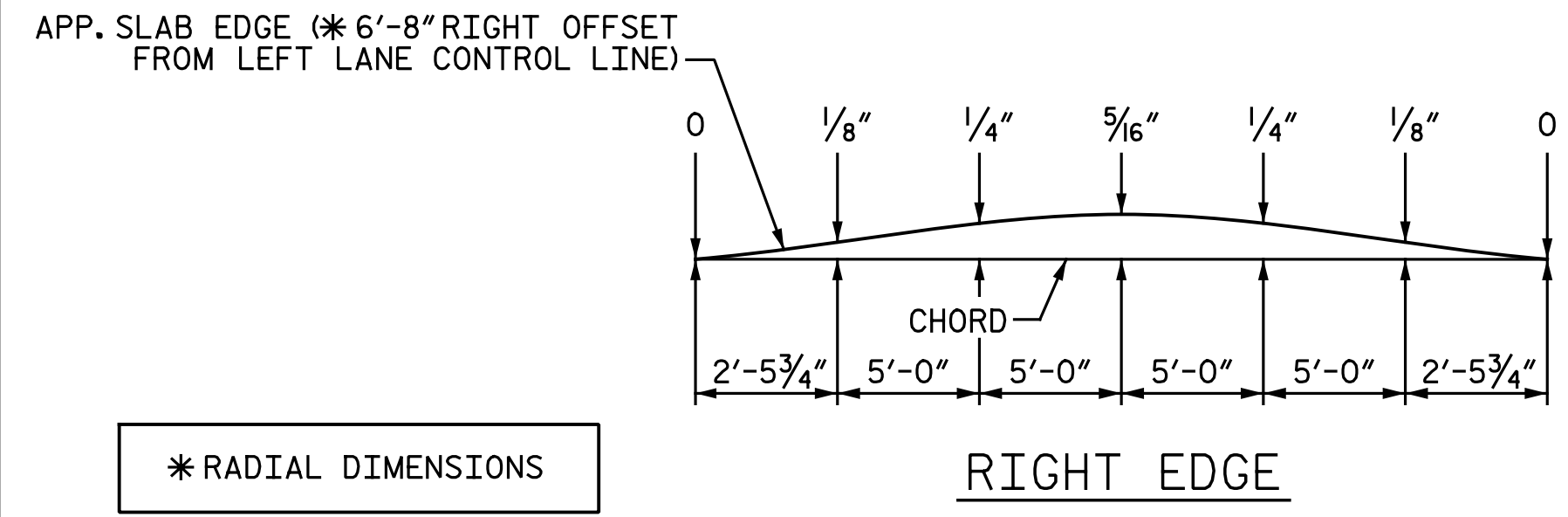
END OF CURB WITHOUT SHOULDER BERM GUTTER



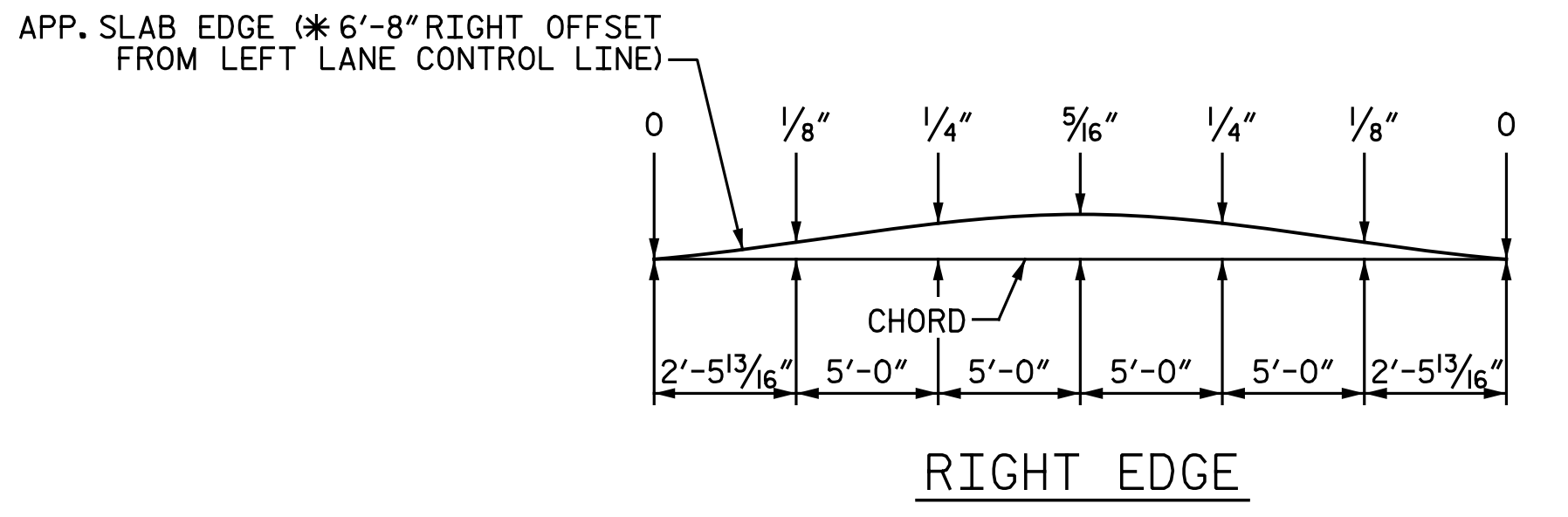
LEFT EDGE



LEFT EDGE



RIGHT EDGE



RIGHT EDGE

APPROACH SLAB @ END BENT 1

APPROACH SLAB @ END BENT 2

* RADIAL DIMENSIONS

ARC OFFSETS

DRAWN BY : N. B. SPEAKS DATE : 3-6-17
 CHECKED BY : D. A. COLETTI DATE : 6-8-17



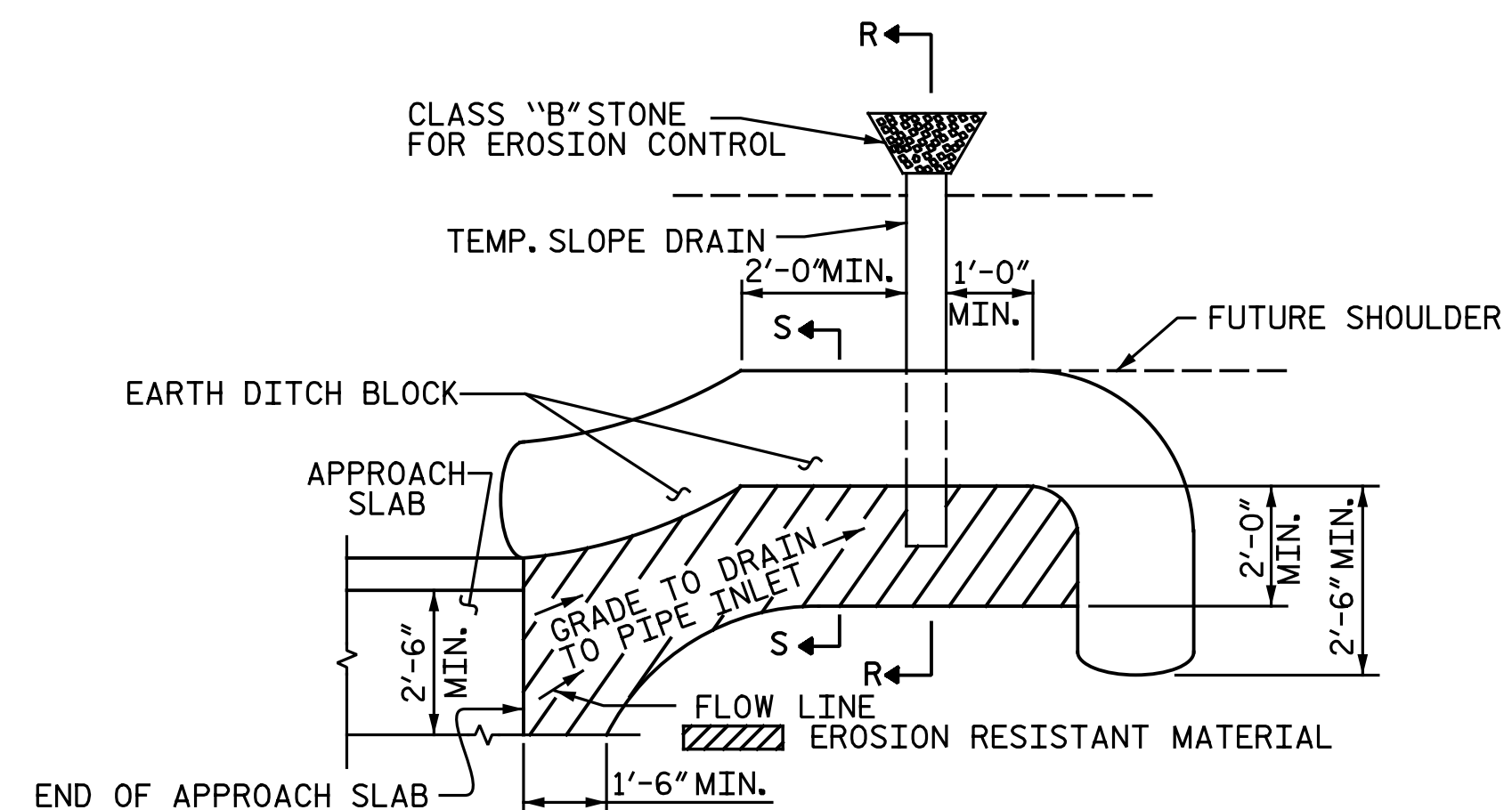
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 NC License No.: F-1084

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-

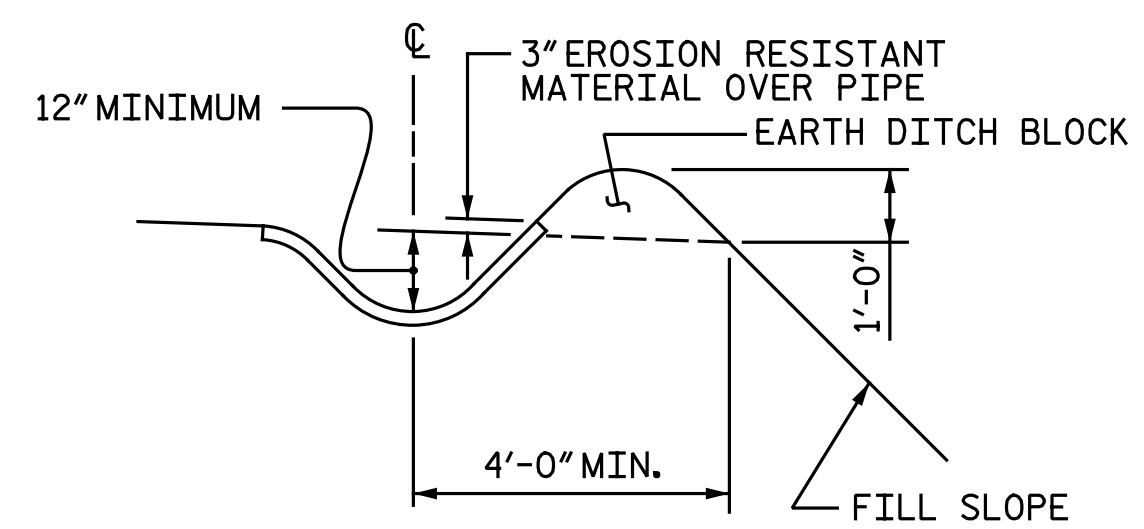
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT
 RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-30
1			3			TOTAL SHEETS
2			4			31

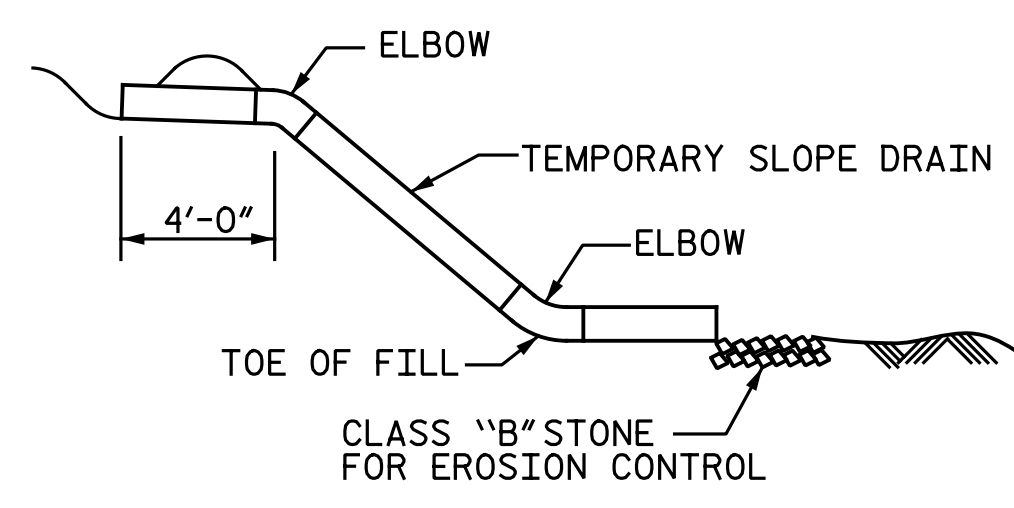


NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

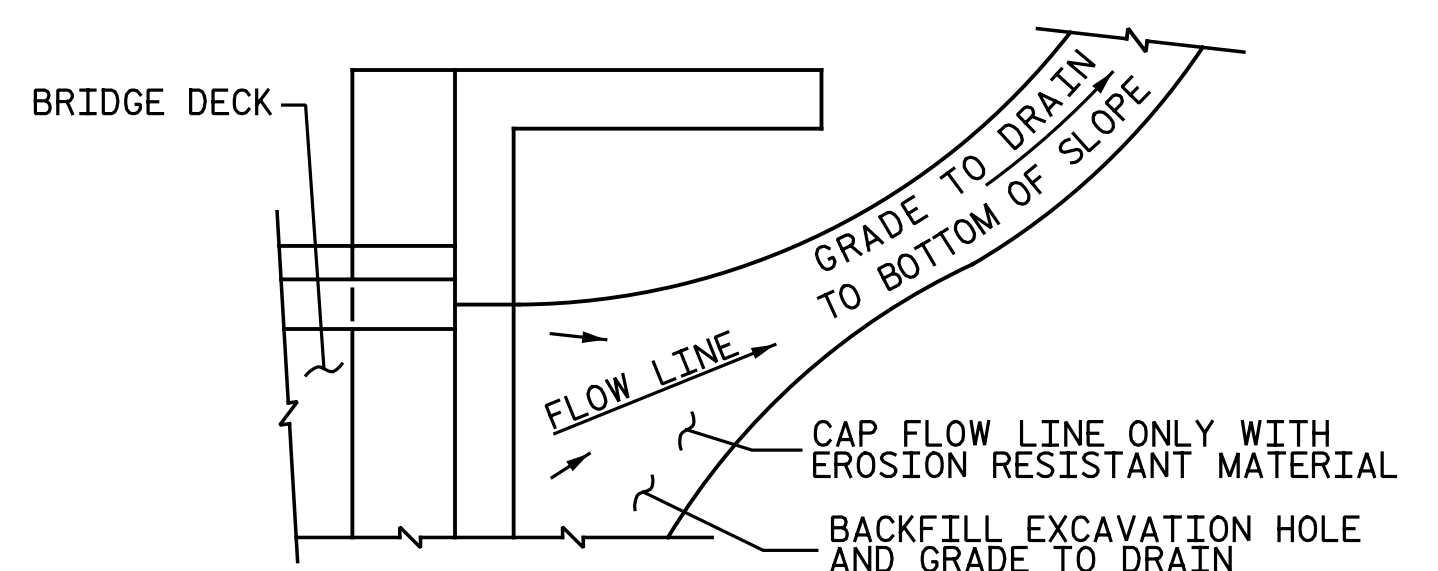
PLAN VIEW



SECTION S-S



SECTION R-R

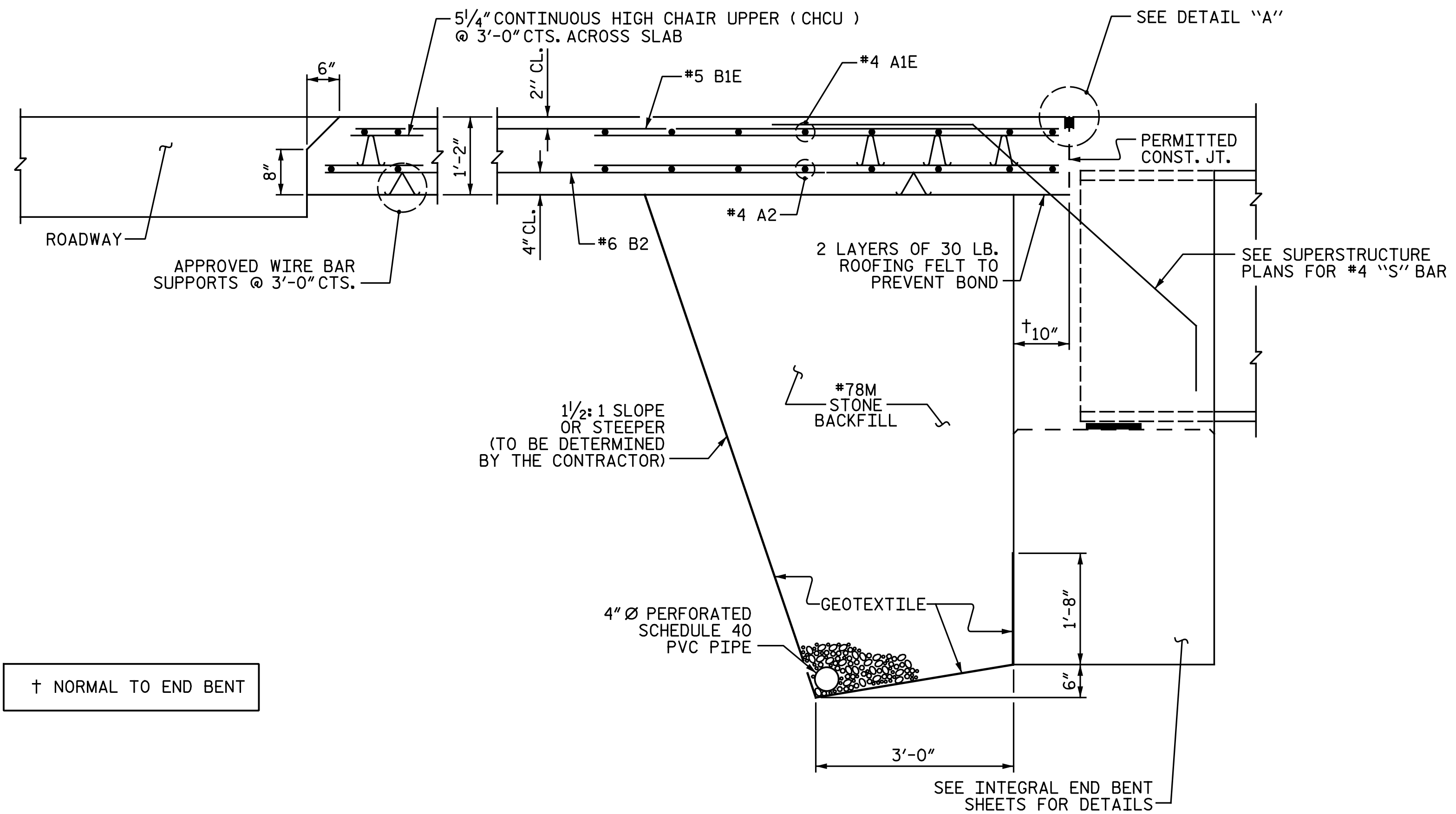


NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

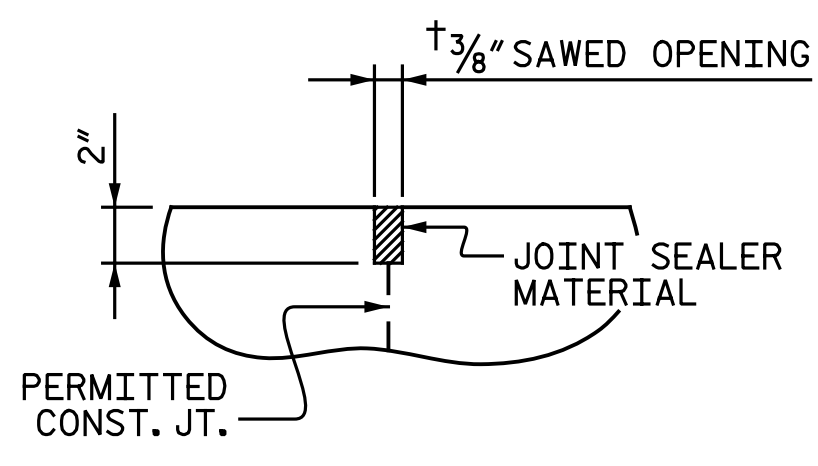
TEMPORARY DRAINAGE DETAIL

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



SECTION THRU SLAB



DETAIL "A"

† NORMAL TO END BENT

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 166+72.51 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 BRIDGE APPROACH SLAB
 DETAILS

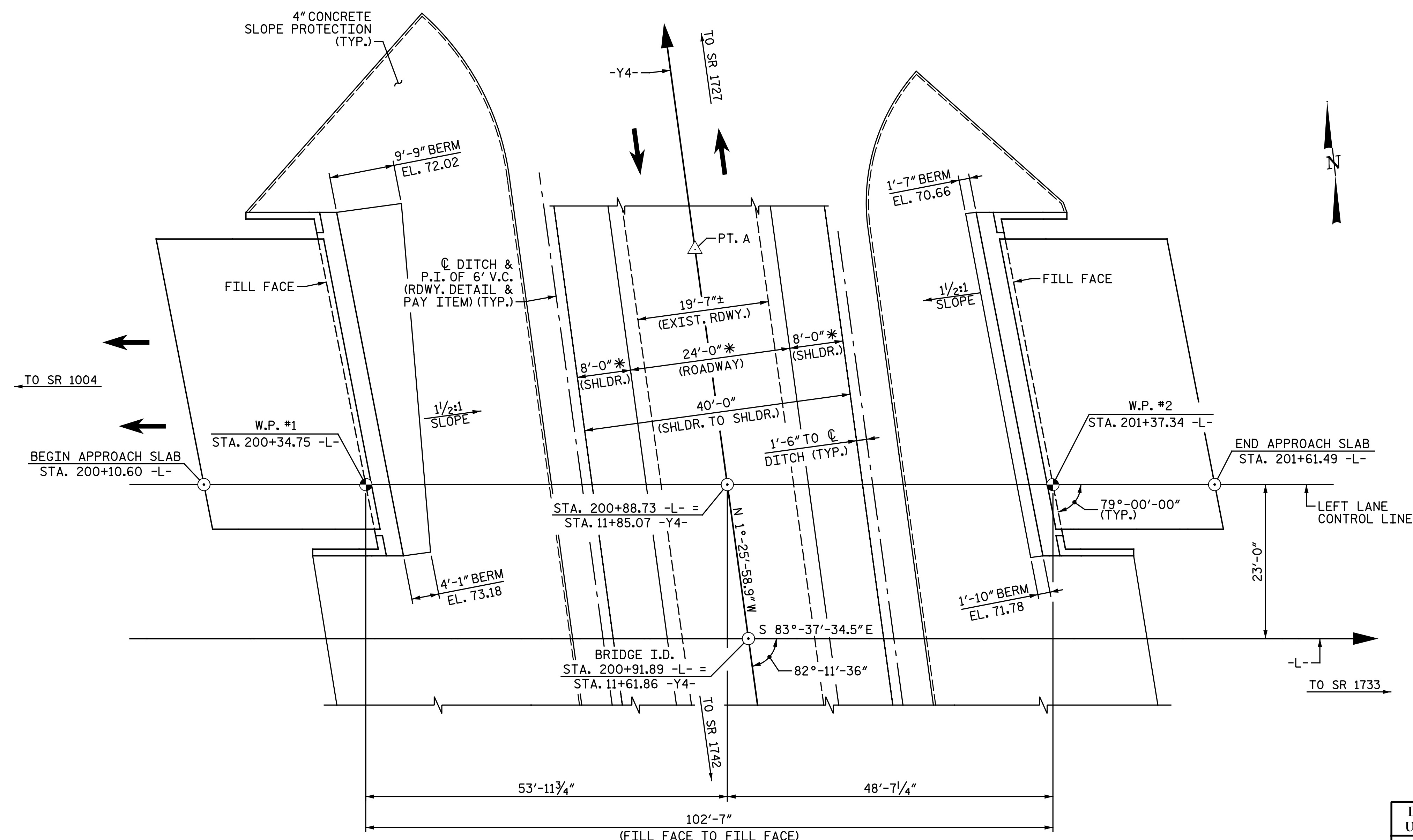
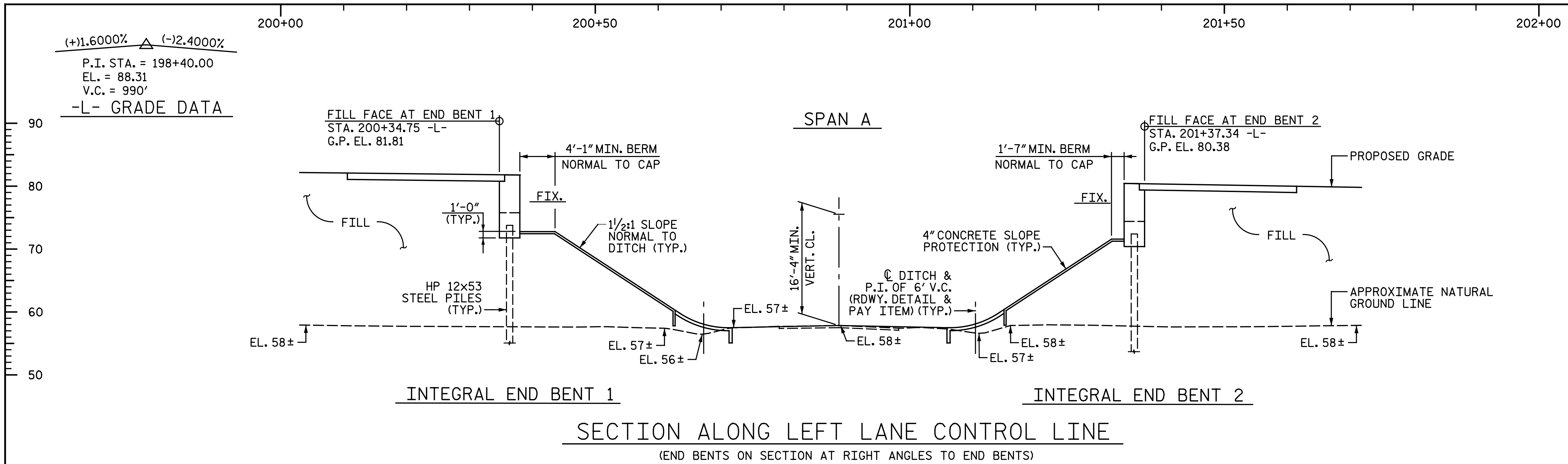
RIGHT LANE

8/14/2017
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-31
1			3			TOTAL SHEETS
2			4			31

DRAWN BY: N. B. SPEAKS DATE: 5-10-17
 CHECKED BY: D. A. COLETTI DATE: 6-8-17



POINT	STATION ON -Y4-	OFFSET	ELEVATION ON -Y4-
A	12+20.48	0.00	58.44

△ - POINT OF MINIMUM VERTICAL CLEARANCE OVER EXISTING ROADWAY WITH 1/2" OVERLAY
 * - FUTURE LANE CONFIGURATION

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 200+91.89 -L-
11+61.86 -Y4-
 SHEET 1 OF 3 BRIDGE NO. 212

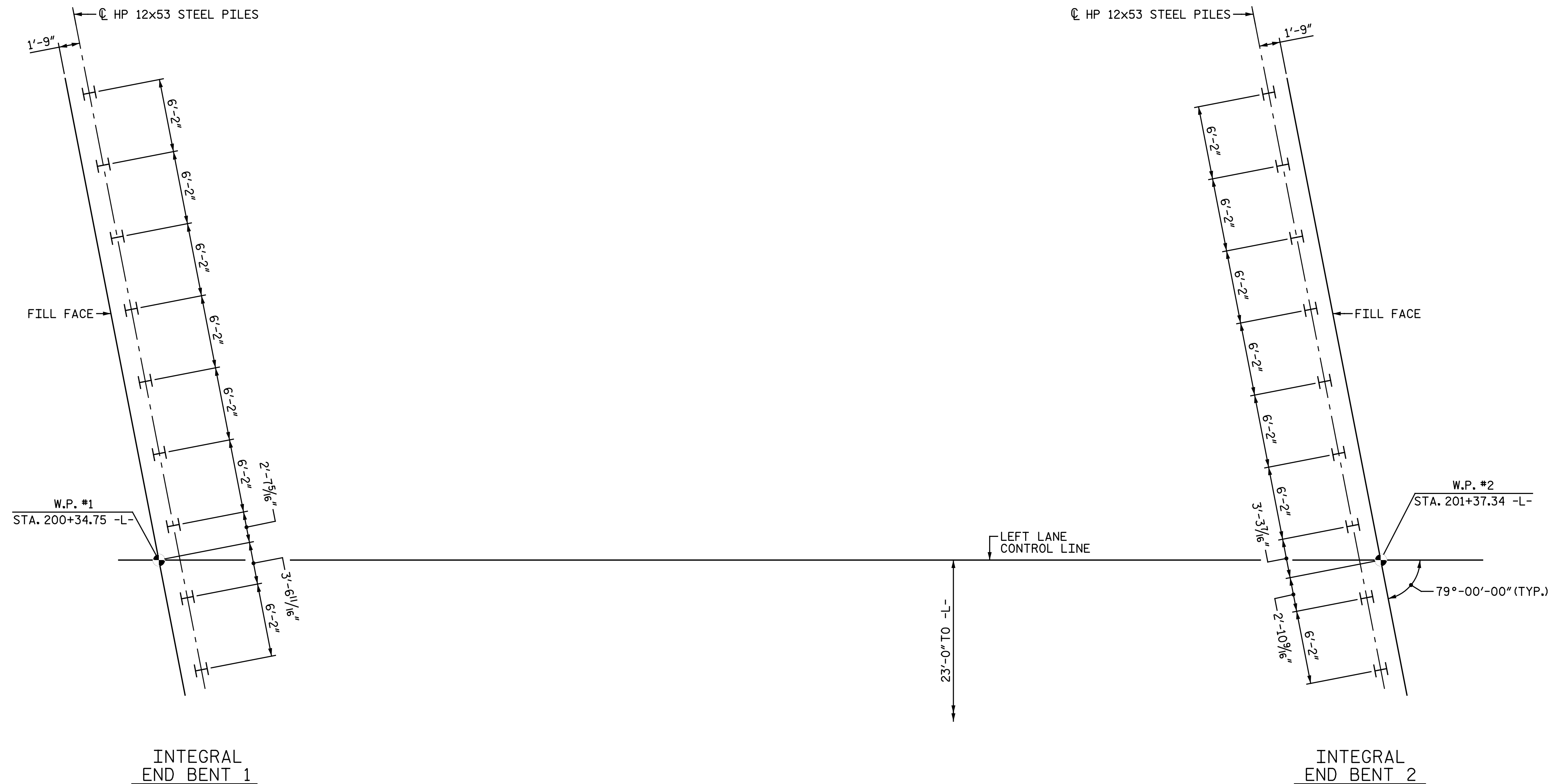


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REVISIONS						SHEET NO. S5-1
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 25
2			4			

DRAWN BY : C. E. MAYHEW DATE : 3-23-17
 CHECKED BY : B. J. BELL DATE : 4-10-17

PLAN
 (PILES NOT SHOWN FOR CLARITY)



FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINES.

ALL PILES ARE VERTICAL.

NOTES:

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 AND END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 109 TONS PER PILE.

DRIVE PILES AT END BENT NO.1 AND END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 185 TONS PER PILE.

TESTING THE FIRST PRODUCTION PILE WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED AT END BENT NO.1 OR END BENT NO.2. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS AND FOR PILE DRIVING CRITERIA, SEE PILE DRIVING CRITERIA PROVISION.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 40-50 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT NO.1 AND END BENT NO.2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

OBSERVE A 1 MONTH WAITING PERIOD AFTER CONSTRUCTING THE EMBANKMENT, END BENT AND REINFORCED BRIDGE APPROACH FILL, IF APPLICABLE, BEFORE BEGINNING APPROACH SLAB CONSTRUCTION AT END BENT NO.1 AND END BENT NO.2.

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 200+91.89 -L-
11+61.86 -Y4-
 SHEET 2 OF 3

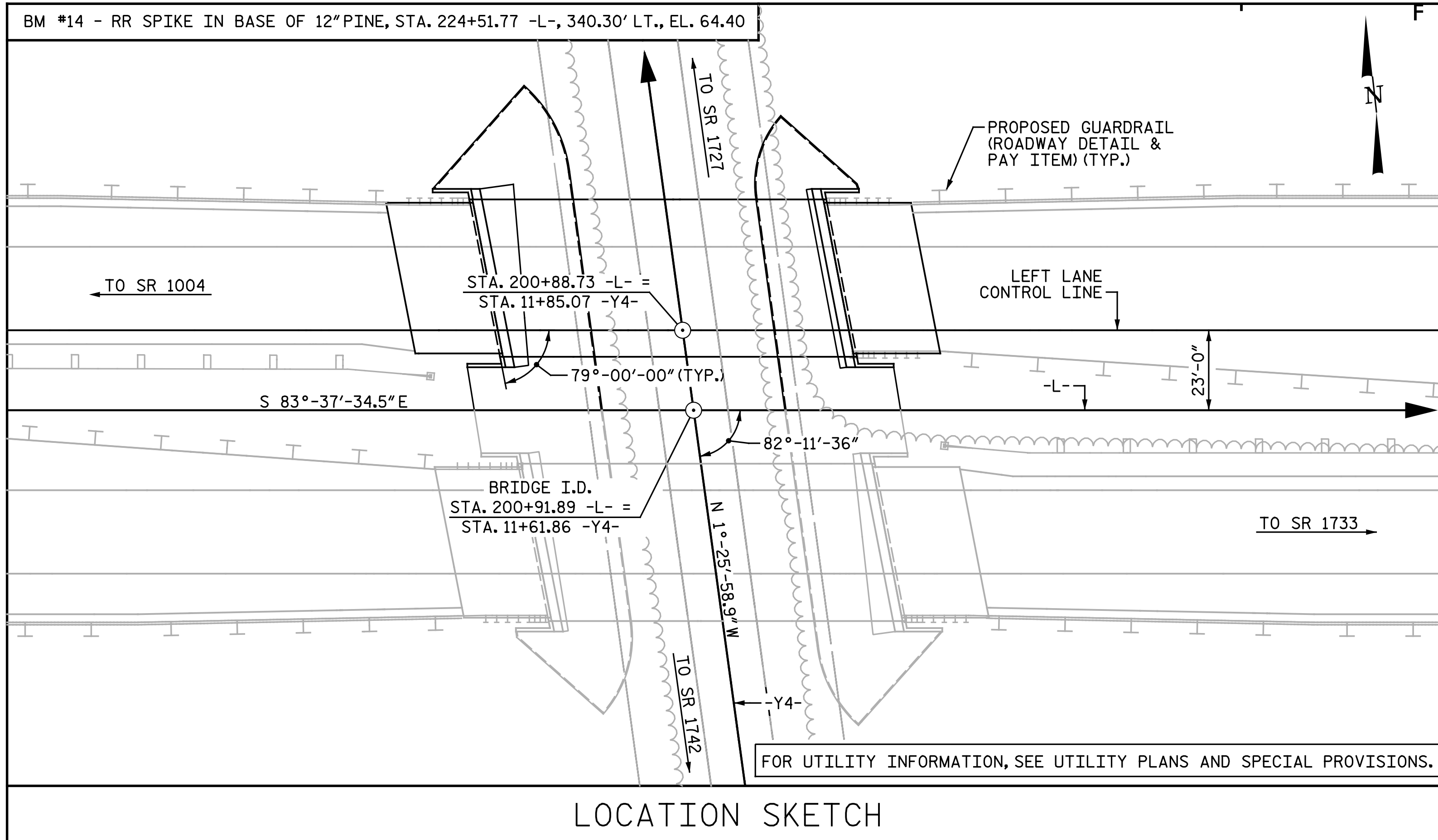


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER SR 1732
 BETWEEN SR 1004 AND SR 1733

8/10/2017
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S5-2
1			3			TOTAL SHEETS
2			4			25

DRAWN BY : M. D. MAYHEW DATE : 3-13-17
 CHECKED BY : B. J. BELL DATE : 4-10-17



NOTES:

- ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
- THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- THE ELEVATION AND CLEARANCE SHOWN ON THE PLANS AT THE POINT OF MINIMUM VERTICAL CLEARANCE ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE ELEVATION ON THE EXISTING PAVEMENT AND CHECK THE CLEARANCE. REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.
- FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.
- PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
- FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

LOCATION SKETCH

TOTAL BILL OF MATERIAL

LOCATION	PDA TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	54" PRESTRESSED CONCRETE GIRDERS		PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES	HP 12x53 STEEL PILES	PILE REDRIVES	CONCRETE BARRIER RAIL	4" SLOPE PROTECTION	ELASTOMERIC BEARINGS	
	EA.	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	NO.	LIN. FT.	EA.	NO.	LIN. FT.	EA.	LIN. FT.	SQ. YDS.	LUMP SUM
SUPERSTRUCTURE		4,642	5,805				5	502.19				201.77		LUMP SUM	
END BENT 1				38.1		5,230			9	9	495	5	400		
END BENT 2				38.1		5,230			9	9	540	5	270		
TOTAL	1	4,642	5,805	76.2	LUMP SUM	10,460	5	502.19	18	18	1,035	10	201.77	670	LUMP SUM

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 200+91.89 -L-
11+61.86 -Y4-
 SHEET 3 OF 3



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 FOR BRIDGE ON C.F. HARVEY
 PARKWAY OVER SR 1732
 BETWEEN SR 1004 AND SR 1733

LEFT LANE

8/10/2017
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S5-3
1			3			TOTAL SHEETS
2			4			25

DRAWN BY : C. E. MAYHEW DATE : 3-17-17
 CHECKED BY : B. J. BELL DATE : 4-10-17

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ_{DC}	γ_{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING #	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					COMMENT NUMBER			
						LIVE-LOAD FACTORS (%LL)	MOMENT					SHEAR					LIVE-LOAD FACTORS (%LL)	MOMENT						
							DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)		DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	1	1.07	--	1.75	0.821	1.58	A	1	49.50	0.960	1.08	A	3	19.40	1.00	0.821	1.07	A	1	49.50	1,2	
	HL-93 (OPERATING)	N/A		1.45	--	1.35	0.821	2.04	A	1	49.50	0.960	1.45	A	3	19.40	N/A	-	-	-	-	-	-	2
	HS-20 (INVENTORY)	36.000	2	1.45	52.20	1.75	0.821	2.20	A	1	49.50	0.960	1.45	A	3	19.40	1.00	0.821	1.49	A	1	49.50	1,2	
	HS-20 (OPERATING)	36.000		1.93	69.48	1.35	0.821	2.85	A	1	49.50	0.960	1.93	A	3	19.40	N/A	-	-	-	-	-	-	2
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500		3.53	47.66	1.40	0.821	6.51	A	1	49.50	0.960	4.75	A	3	19.40	1.00	0.821	3.53	A	1	49.50	1,2
		SNGARBS2	20.000		2.56	51.20	1.40	0.821	4.72	A	1	49.50	0.960	3.30	A	3	19.40	1.00	0.821	2.56	A	1	49.50	1,2
		SNAGRIS2	22.000		2.39	52.58	1.40	0.821	4.41	A	1	49.50	0.960	3.03	A	3	19.40	1.00	0.821	2.39	A	1	49.50	1,2
		SNCOTTS3	27.250		1.75	47.69	1.40	0.821	3.24	A	1	49.50	0.960	2.28	A	3	19.40	1.00	0.821	1.75	A	1	49.50	1,2
		SNAGGRS4	34.925		1.44	50.29	1.40	0.821	2.65	A	1	49.50	0.960	1.84	A	3	19.40	1.00	0.821	1.44	A	1	49.50	1,2
		SNS5A	35.550		1.41	50.13	1.40	0.821	2.60	A	1	49.50	0.960	1.86	A	3	19.40	1.00	0.821	1.41	A	1	49.50	1,2
		SNS6A	39.950		1.28	51.14	1.40	0.821	2.36	A	1	49.50	0.960	1.67	A	3	19.40	1.00	0.821	1.28	A	1	49.50	1,2
		SNS7B	42.000		1.22	51.24	1.40	0.821	2.25	A	1	49.50	0.960	1.62	A	3	19.40	1.00	0.821	1.22	A	1	49.50	1,2
	TRUCK TRACTOR SEMI-TRAILER (T/S)	TNAGRIT3	33.000		1.56	51.48	1.40	0.821	2.87	A	1	49.50	0.960	2.03	A	3	19.40	1.00	0.821	1.56	A	1	49.50	1,2
		TNT4A	33.075		1.56	51.60	1.40	0.821	2.88	A	1	49.50	0.960	1.98	A	3	19.40	1.00	0.821	1.56	A	1	49.50	1,2
		TNT6A	41.600		1.27	52.83	1.40	0.821	2.33	A	1	49.50	0.960	1.72	A	3	19.40	1.00	0.821	1.27	A	1	49.50	1,2
		TNT7A	42.000		1.27	53.34	1.40	0.821	2.34	A	1	49.50	0.960	1.69	A	3	19.40	1.00	0.821	1.27	A	1	49.50	1,2
		TNT7B	42.000		1.30	54.60	1.40	0.821	2.39	A	1	49.50	0.960	1.60	A	3	19.40	1.00	0.821	1.30	A	1	49.50	1,2
		TNAGRIT4	43.000		1.24	53.32	1.40	0.821	2.29	A	1	49.50	0.960	1.54	A	3	19.40	1.00	0.821	1.24	A	1	49.50	1,2
		TNAGT5A	45.000		1.18	53.10	1.40	0.821	2.17	A	1	49.50	0.960	1.52	A	3	19.40	1.00	0.821	1.18	A	1	49.50	1,2
		TNAGT5B	45.000	3	1.17	52.65	1.40	0.821	2.15	A	1	49.50	0.960	1.46	A	3	19.40	1.00	0.821	1.17	A	1	49.50	1,2

NOTES:

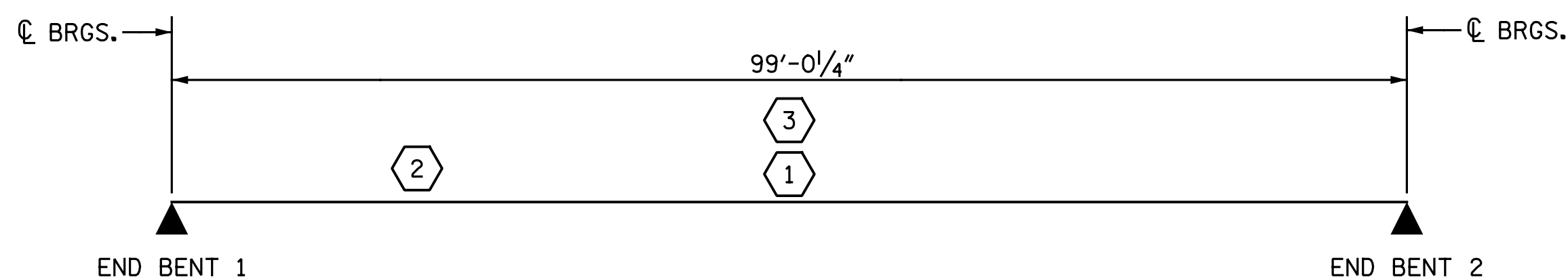
MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- A SERVICE III LIVE LOAD FACTOR OF 1.0 WAS USED TO BE CONSISTENT WITH THE VALUE USED DURING DESIGN.
- DISTANCE FROM LEFT END OF SPAN IS GIVEN WITH RESPECT TO CENTERLINE OF BEARING AND IS MEASURED ALONG THE CONTROLLING GIRDER.

#	CONTROLLING LOAD RATING
1	DESIGN LOAD RATING (HL-93)
2	DESIGN LOAD RATING (HS-20)
3	LEGAL LOAD RATING **
**	SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION	
GIRDER LOCATION IS PROVIDED USING GIRDER NUMBER, WHERE GIRDER 1 IS THE LEFT EXTERIOR GIRDER LOOKING AHEAD STATION. SEE "GIRDER LAYOUT" SHEET FOR ALL GIRDER LOCATIONS.	



LRFR SUMMARY

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 200+91.89 -L-



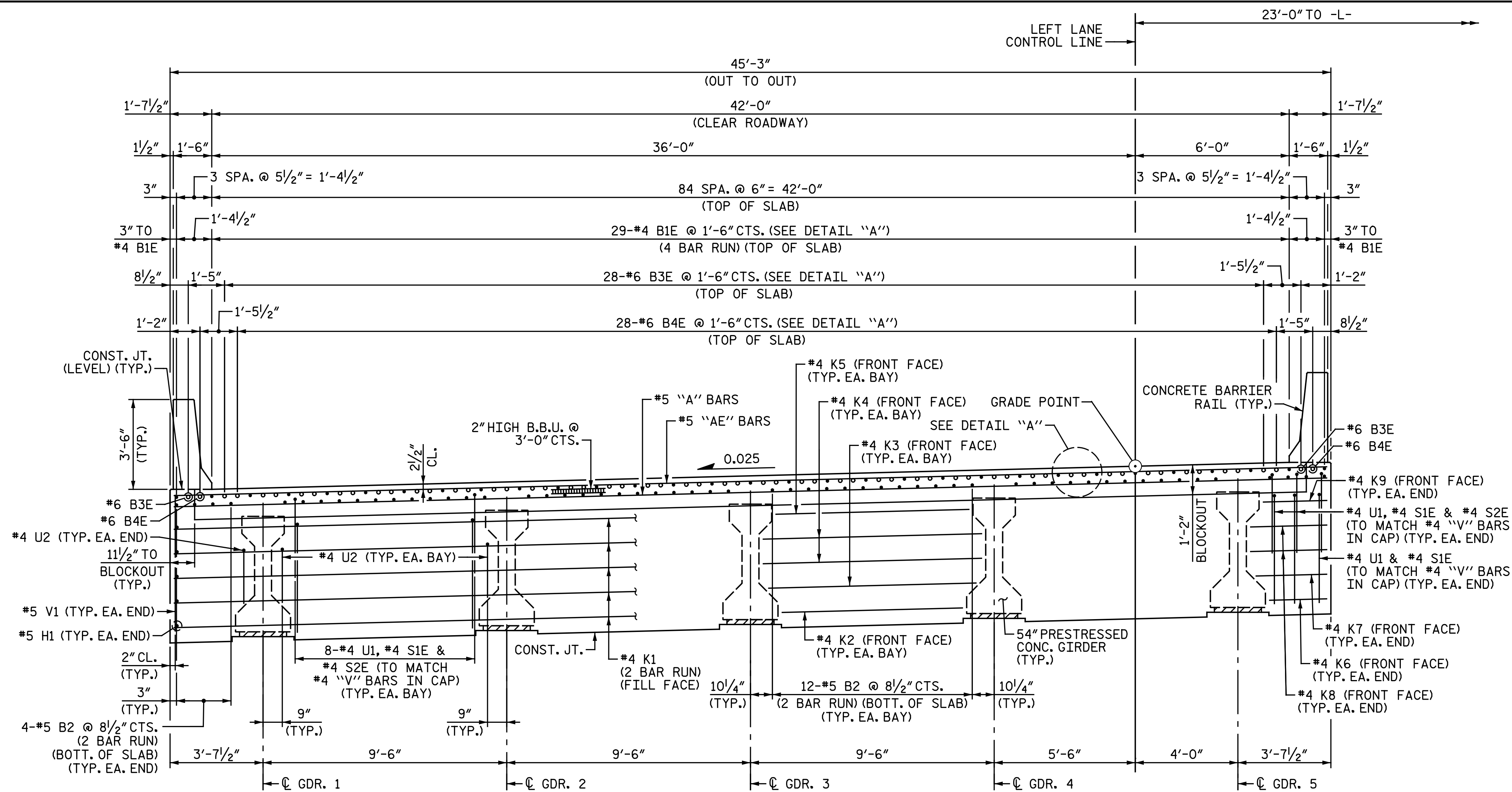
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 LRFR SUMMARY FOR
 PRESTRESSED
 CONCRETE GIRDERS
 (NON-INTERSTATE TRAFFIC)
 LEFT LANE

8/10/2017
 DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED

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 Michael Baker Engineering
 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No.: F-1084

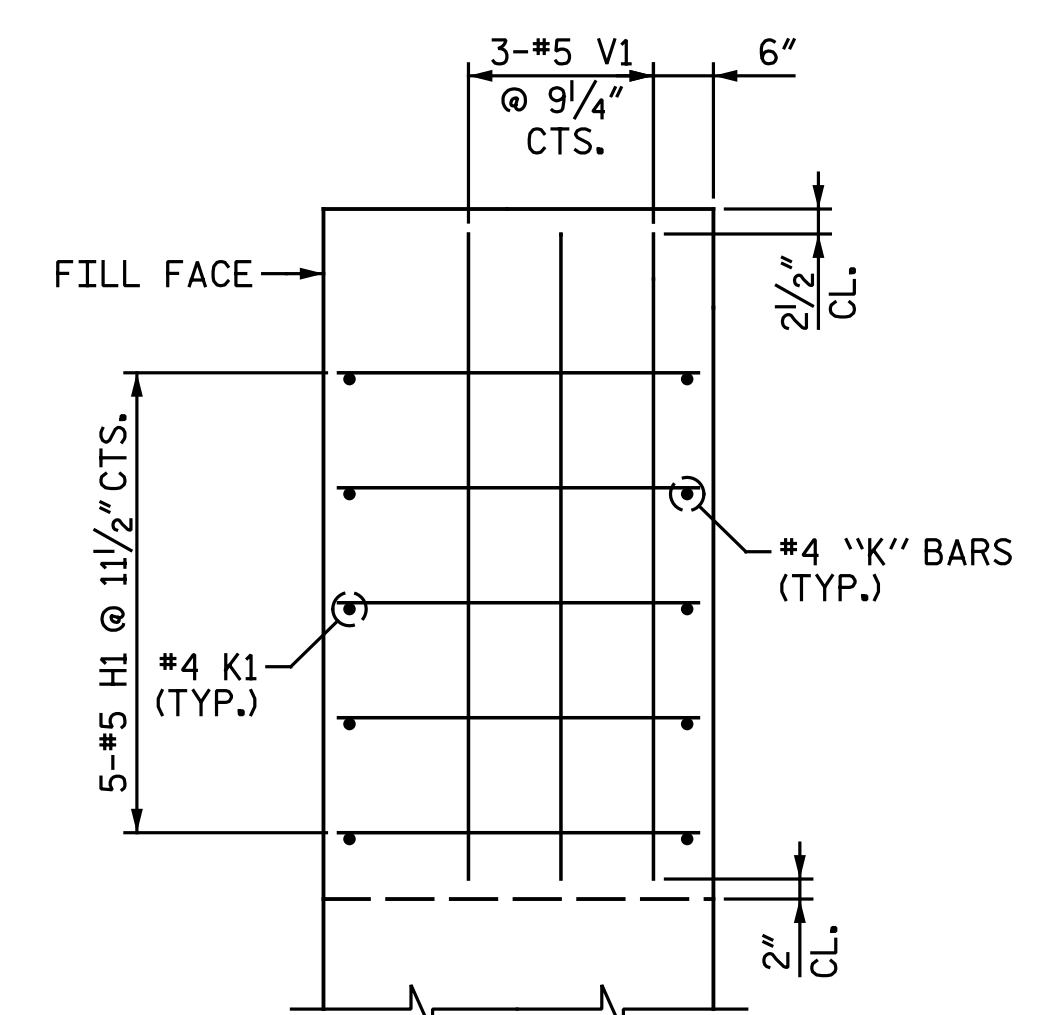
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S5-4
1			3			TOTAL SHEETS
2			4			25

ASSEMBLED BY : N. B. SPEAKS	DATE : 2-27-17
CHECKED BY : B. J. BELL	DATE : 4-6-17
DRAWN BY : MAA 1/08	REV. 11/2/08RR MAA/GM
CHECKED BY : GM/DI 2/08	REV. 10/1/11 MAA/GM

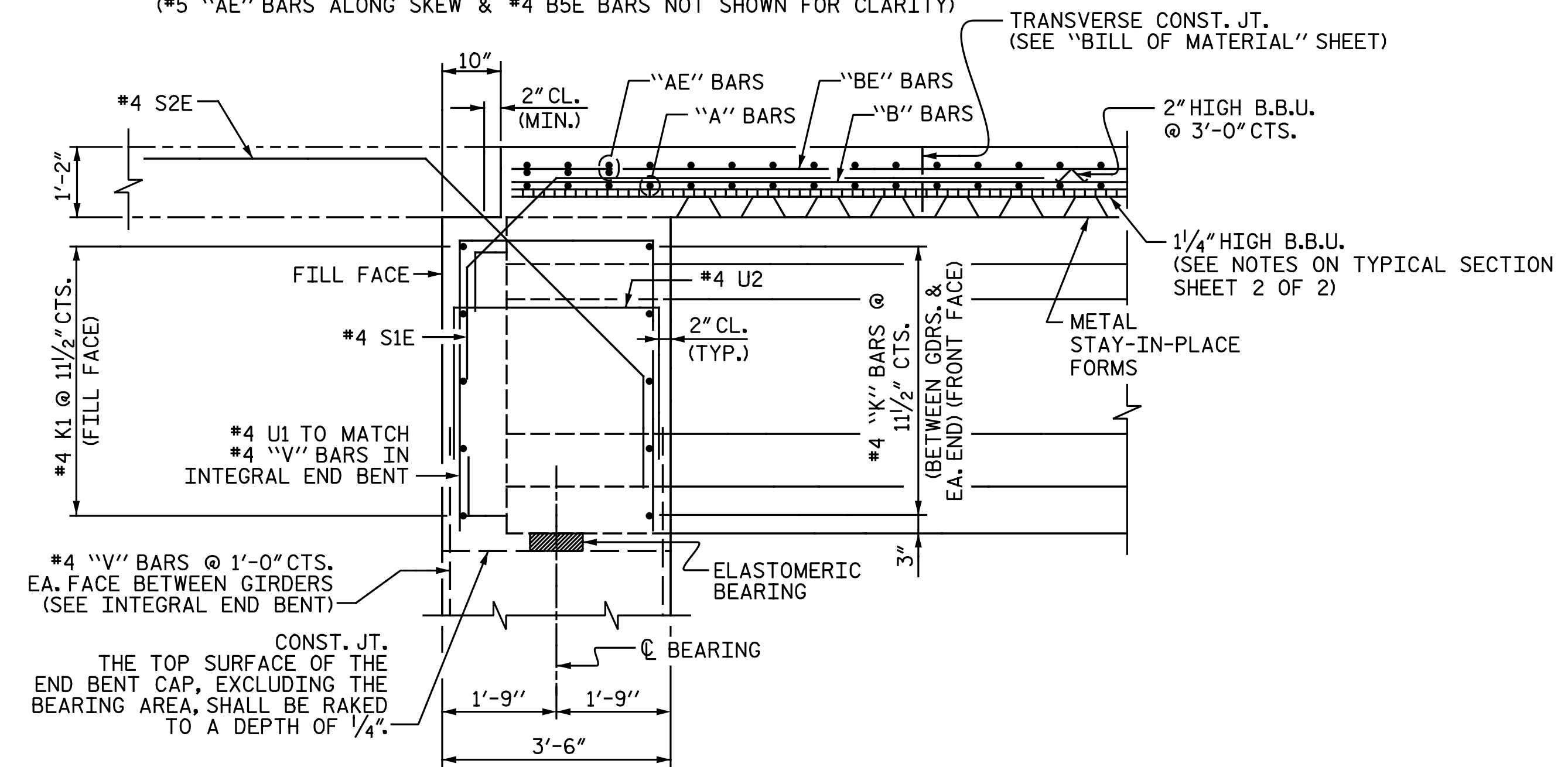


TYPICAL SECTION AT INTEGRAL END BENT

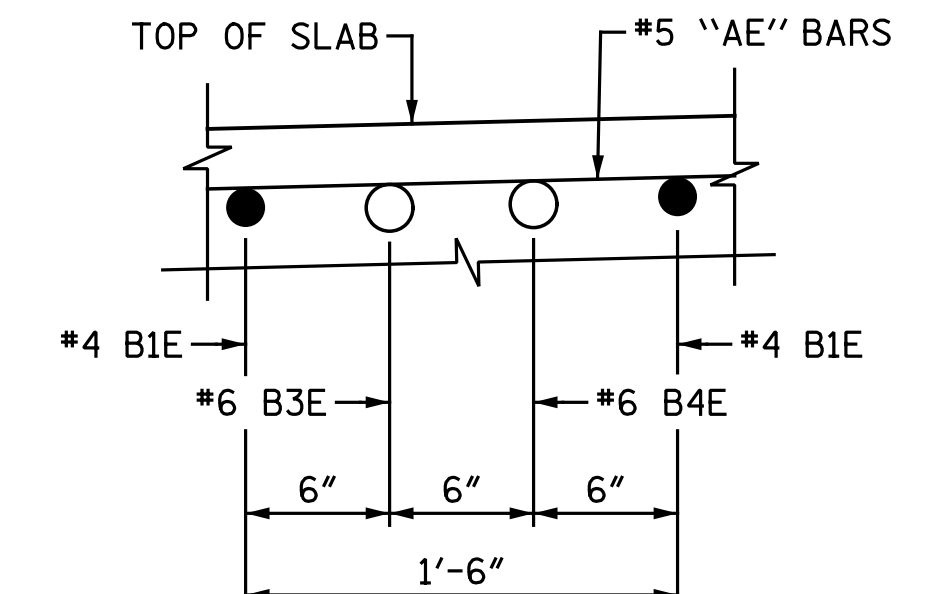
(END BENT 1 SHOWN, END BENT 2 SIMILAR)
 (#5 "AE" BARS ALONG SKEW & #4 B5E BARS NOT SHOWN FOR CLARITY)



END OF DIAPHRAGM DETAIL
 (END BENT 1 SHOWN, END BENT 2 SIMILAR)



END OF GIRDER DETAIL AT INTEGRAL END BENT



DETAIL "A"

PROJECT NO. R-5703
LENOIR COUNTY
 STATION: 200+91.89 -L-

SHEET 1 OF 2

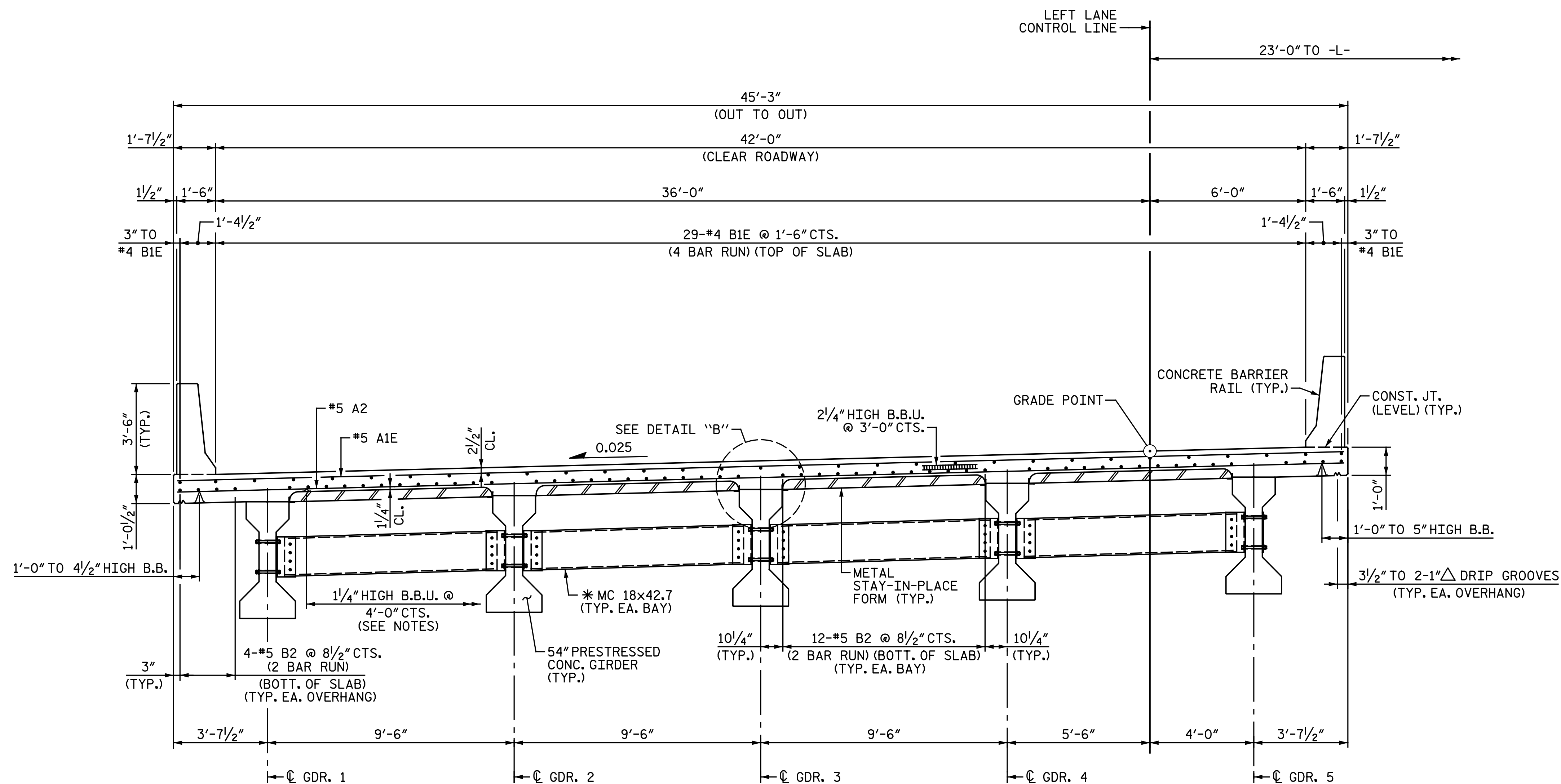


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 TYPICAL SECTION

8/10/2017
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 UNLESS ALL SIGNATURES COMPLETED
Michael Baker INTERNATIONAL
 Michael Baker Engineering
 8000 Regency Parkway, Suite 600
 Cary, North Carolina 27518
 NC License No.: F-1084

REVISIONS						SHEET NO. S5-5
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 25
2			4			

DRAWN BY: M. D. MAYHEW DATE: 3-15-17
 CHECKED BY: B. J. BELL DATE: 3-24-17



TYPICAL SECTION AT INTERMEDIATE DIAPHRAGM

NOTES:

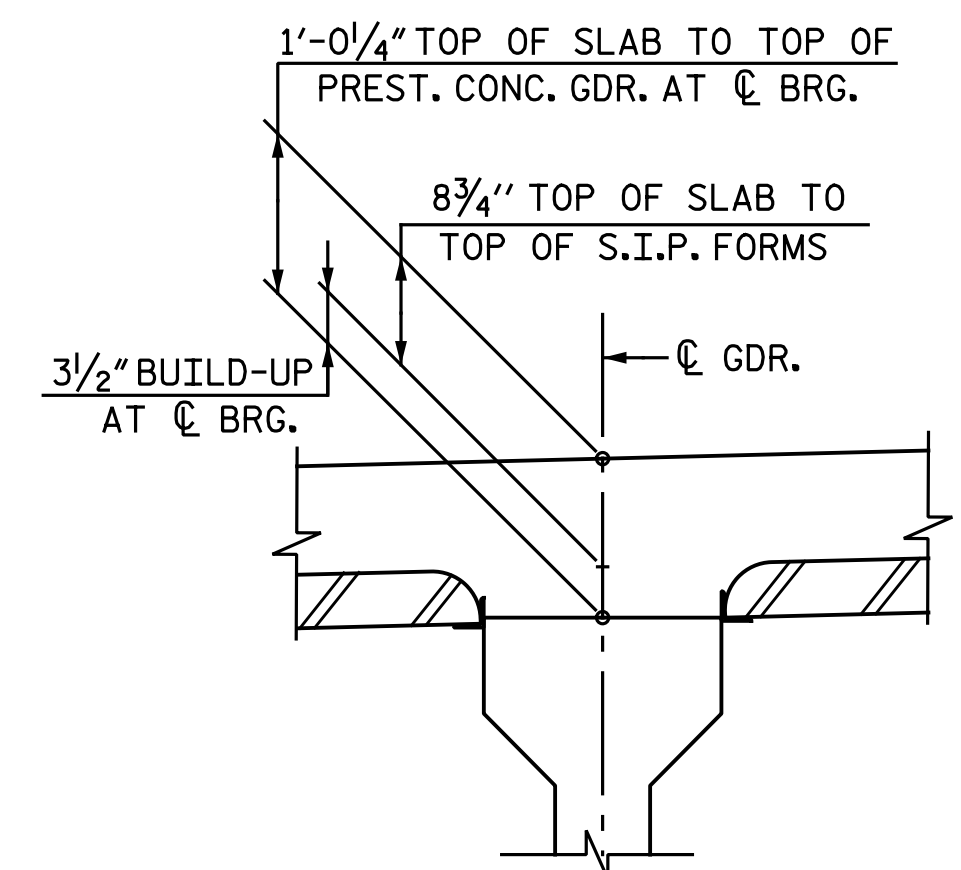
PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.

LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS AND TO FACILITATE INSTALLATION OF CONCRETE BARRIER RAIL REINFORCEMENT.

PREVIOUSLY CAST CONCRETE IN A SPAN SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE SPAN.

FOR CONCRETE BARRIER RAIL DETAILS, SEE "CONCRETE BARRIER RAIL" SHEET.

* FOR DETAILS OF INTERMEDIATE DIAPHRAGMS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR TYPE IV PRESTRESSED CONCRETE GIRDERS" SHEET.



DETAIL "B"

PROJECT NO. R-5703
 LENOIR COUNTY
 STATION: 200+91.89 -L-

SHEET 2 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 TYPICAL SECTION

LEFT LANE

8/10/2017
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 Michael Baker Engineering
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 Cary, North Carolina 27518
 NC License No.: F-1084

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S5-6
1			3			TOTAL SHEETS
2			4			25

DRAWN BY: M. D. MAYHEW DATE: 3-15-17
 CHECKED BY: B. J. BELL DATE: 3-20-17

