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				Prepared	1 In the Office of:
PROJECT LENGTH				PLANS PREPARED BY:	2610 Wycliff Road Suite 102
ECT BRIDGE 008:				🎽 Gannett	Fleming Raleigh NC 27607-3073 (919) 420-7660
ROADWAY	=	0.148	MI.	Excellence Deliv	vered As Promised NC Lic. No. F-0270
STRUCTURE	=	0.019	MI.	2012 STANDARD SPECIFICATIONS	
ENGTH OF PROJECT	=	0.167	MI.	LETTINC DATE.	
ECT BRIDGES 129 & 130:				DECEMBER 19, 2017	PROJECT ENGINEER
ROADWAY (EACH)	=	0.151	MI.		
STRUCTURE (EACH)	=	0.019	MI.		RICHARD F. WERTMAN, PE
ENGTH OF PROJECT (EACH)	=	0.170	MI.		PROJECT DESIGN ENGINEER
			J		



		INDEX	
STRUCTURE	STATION	DESCRIPTION	SHEE
1	21+57.23 -L- 109+69.94 -L2-	BRIDGE ON US 301 OVER I-95 BUSINESS LOOP SBL BETWEEN I-95 BUSINESS OFF RAMP AND I-95 BUSINESS	S01-1
2	106+59.75 -L1- 14+51.19 -Y-	BRIDGE ON I-95 BUSINESS LOOP NBL OVER US 301 BETWEEN DOBBIN HOLMES ROAD AND I-95	S02-1
3	107+16.84 -L2- 13+69.76 - Y-	BRIDGE ON I-95 BUSINESS LOOP SBL OVER US 301 BETWEEN DOBBIN HOLMES ROAD AND I-95	S03-1
4	21+57.23 -L- 109+69.94 -L2-	MSE RETAINING WALL NO.1 AT END BENT 1 OF STR.#1	W-1
5	21+57.23 -L- 109+69.94 -L2-	MSE RETAINING WALL NO.2 AT END BENT 2 OF STR.#1	W-1
6	14+10.22 -Y-	MSE RETAINING WALL NO.3 AT END BENT 1 OF STR.#2 AND STR.#3	W-5
7	14+10.22 -Y-	MSE RETAINING WALL NO.4 AT END BENT 2 OF STR. #2 AND STR. #3	W-5

-L2_ 		
NO.	S. 129 & 130	
ET NUMBERS	PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> COUN [.] STATION:	 TY
THRU SO1-24	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH	N
THRU S02-24		
1 THRU W-4	INDEX	
5 THRU W-9		
5 THRU W-9	REVISIONS SHEE NO. BY: DATE: NO. BY: DATE: TO 1 3 TO TO	ET NO.
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		CUMDERLAND COUNTY
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		SHEET 1 OF 3 REDIACES RETORE NO 0
		NEILAGES DITUGE NULO
		STATE OF NORTH CAROLINA
	WHITH CAROL	
	SESSION THE	
	SEAL F	GENERAL DRAWING
	R. WOMFER	BRIDGE ON US 301 OVER
	F WERMIN	RETWEEN I-05 RICTNESS OFF T-25 RUSTNESS LUUH SRL
		RAMP AND I-95 BUSINESS
	Richard Fo Weitma	
f Road	08901A86EBF6470 11/7/2017	REVISIONS SHEET NO.
27607-3073	CUMENT NOT CONSTDERED	NO. BY: DATE: NO. BY: DATE: SO1-1
560 F-0270	FINAL UNLESS ALL	1 TOTAL SHEETS
	JIGNATURES CUMPLETED	<u> 後</u> 24 24



END BENT #1

NOTES:

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FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICAT
PILES AT END BENTS NO.1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 100 TONS PER PILE
DRIVE PILES AT END BENTS NO.1 AND 2 TO A REQUIRED DRIVING RESISTANCE OF 140 TONS PER PILE
OBSERVE A ONE MONTH WAITING PERIOD AFTER CONSTRUCTING THE MSE WALLS AND BEFORE DRIVING F AT END BENTS NO.1 AND 2. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SPECIAL PROVISI
FOR WAITING PERIODS,SEE SECTION 235 OF THE STANDARD SPECIFICATIONS AND THE SURCHARGE AND WAITING PERIODS PROVISION.
TESTING PILES WITH THE PDA DURING DRIVING,RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING,SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
PILE SLEEVES AT END BENTS NO.1 AND 2 ARE TO BE PLACED DURING CONSTRUCTION IN THE REINFORC BACKFILL OF THE MSE WALLS. SEE MSE WALL PLANS.
PILES AT END BENTS NO.1 AND 2 ARE TO BE DRIVEN AFTER CONSTRUCTION AND RELEASE OF THE MSE WALLS PENDING THE SETTLEMENT MONITORING PROGRAM.
THE MSE WALL MAY NEED TO BE INSTALLED USING TWO-STAGED CONSTRUCTION TECHNIQUES,TO ALLOW SETTLEMENT TO OCCUR. THE ANTICIPATED SETTLEMENT WAS APPROXIMATELY 3.5-INCHES FOR BOTH ENE BENTS NO.1 AND 2.
<pre># END BENT NO.2 PILES LOCATED NEAR THE EXISTING FOOTING CAN BE SHIFTED TO AVOID INTERFEREN PILE SPACING NOT TO EXCEED 8'-6". THE CONTRACTOR IS RESPONSIBLE FOR LOCATING EXISTING FOOTINGS BEFORE DRIVING NEW PILES.</pre>
DRAWN BY : J.A. BOYER DATE : 09/25/17 CHECKED BY : R.F. WERTMAN DATE : 10/09/17 DESIGN ENGINEER OF RECORD : R.F. WERTMAN DATE : 11/06/17

END BENT #2

FOUNDATION LAYOUT DIMENSIONS LOCATING PILES ARE SHOWN TO PILE CENTERLINE. TIONS. _ • PILES IONS. CED THE ICE.



STR.NO.1



DRAWN BY : B.A. WHITE	_ DATE : <u>09/22/17</u>
CHECKED BY : _R.F. WERTMAN	DATE: <u>10/08/17</u>
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE: 11/06/17

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ΒI	ILL OF	MATER	$\Box \land$				-			
CING L	STRUCTURAL STEEL APPROX. 81650 LBS.	PILE DRIVEN EQUIPMENT SETUP FOR HP 14 X 73 STEEL PILES	HP Stee	14 X 73 El PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	4″SLOPE PROTECTION	ELASTOMERIC BEARINGS	FOAM JOINT SEALS	
1	LUMP SUM	EACH	N0.	LIN.FT.	EACH	LIN.FT.	SQ.YDS.	LUMP SUM	LUMP SUM	
	LUMP SUM					192.5		LUMP SUM	LUMP SUM	ſ
6		6	6	492	6		12			ſ
1		7	7	504	7		12			Γ
C	LUMP SUM	13	13	996	13	192.5	24	LUMP SUM	LUMP SUM	Γ



			NOTES:				
ASSUME	D LIVE	LOAD =	HL-93 OR ALTERNAT	FE LOADIN	G.		
THIS B LRFD B	RIDGE H RIDGE D	AS BEE ESIGN	EN DESIGNED IN ACC SPECIFICATIONS.	ORDANCE V	VITH THE AASHTO		
THIS B	RIDGE I	S LOCA	ATED IN SEISMIC ZO)NE 1.			
FOR OT	HER DES	IGN DA	ATA AND GENERAL NO	TES,SEE S	SHEET SN.		
FOR SU	BMITTAL	OF W	ORKING DRAWINGS, SE	EE SPECIA	L PROVISIONS.		
FOR FA	LSEWORK	and f	FORMWORK, SEE SPECI	AL PROVIS	SIONS.		
FOR CR	ANE SAF	ETY,SE	EE SPECIAL PROVISI	ONS.			
FOR GR	OUT FOR	STRU	CTURES, SEE SPECIAL	PROVISI	DNS.		
THE EL OF MIN AVAILA ELEVAT REPORT NECESS WILL B	EVATION IIMUM VE BLE, PRI ION ON ANY VA ARY TO E PROVI	AND (ERTICA OR TO THE EX RIATI ACHIEN DED BY	CLEARANCE SHOWN ON L CLEARANCE ARE FR BEGINNING BRIDGE (ISTING PAVEMENT A ONS TO THE ENGINEE /E THE REQUIRED MI (THE DEPARTMENT.	THE PLAN COM THE B CONSTRUC ND CHECK R.ANY PL NIMUM VE	IS AT THE POINT EST INFORMATION TION, VERIFY THE THE CLEARANCE. AN REVISIONS RTICAL CLEARANCE		
FOR MA STRUCT	INTENAN URE, SEE	ICE ANI SPECI	D PROTECTION OF TR TAL PROVISIONS.	RAFFIC BE	NEATH PROPOSED		
REMOVA FORMS SPECIF	BLE FOR IN ACCC ICATION	MS MA DRDANCI	Y BE USED IN LIEU E WITH ARTICLE 420	OF METAL -3 OF THE	STAY-IN-PLACE STANDARD		
NEEDLE FOR ON	BEAMS THE PL	WILL N ANS OF	NOT BE ALLOWED UNL R APPROVED BY THE	ESS OTHEF Engineer.	WISE CALLED		
ALL ST AND PA THE ST PLANS.	RUCTURA INTED I ANDARD	L STEE N ACCO SPECIF	EL SHALL BE AASHTO ORDANCE WITH SYSTE FICATIONS UNLESS O	M270 GRA M 4 OF A THERWISE	DE 50W RTICLE 442-8 OF NOTED ON THE		
INASML STEEL	JCH AS T CONTAIN	HE PAI S LEAD	INT SYSTEM ON THE D, THE CONTRACTOR'S	EXISTING ATTENTIC	STRUCTURAL N IS DIRECTED		
TO ART RESULT	ICLE 10 ING FRO	7-1 OF M_COM	THE STANDARD SPEC PLIANCE WITH APPLI	IFICATIO	NS.ANY COSTS ATE OR FEDERAL		
REGULA LEAD B "REMOV"	IIONS P ASED PA AL OF E>	'ERIAII INT SH KISTIN	NING TO HANDLING C HALL BE INCLUDED II G STRUCTURE AT STA)F MATERI N THE BIC ATION 21+	ALS CONTAINING PRICE FOR 57.23 -L-".		
THE EX	ISTING	STRUC	TURE CONSISTING OF	TWO SPA	NS_@_45'-0", ONE_SP	AN	
@ 75'-C REINFO)″, AND T RCED CO	WO SP	ANS @ 40'-O", WITH A E DECK ON STEEL I-E	A CLEAR R BEAMS ON	OADWAY OF 24'-1"AN END BENTS WITH	ND	
REINFO REINFO THE PR	RCED CO RCED C		E CAPS ON STEEL PI TE POSTS AND BEAMS TURE SHALL BE REMO'	ON PILE	FOOTINGS LOCATED	AT	
PRESEN	TLY NOT	POSTE	ED FOR LOAD LIMIT.			-	
THE SU PLANS	BSTRUCT IS FROM	URE OF	F THE EXISTING BRI BEST INFORMATION A	DGE INDIC	ATED ON THE SINCE THIS		
THE CO	NTRACTO	ls shu DR shal Trang	WN FUR THE CUNVENI _L HAVE NO CLAIM W Sportation for ANN	HATSOEVE	R AGAINST THE		
COST I BRIDGE		BASEI	D ON DIFFERENCES B E SHOWN ON THE PLA	ETWEEN TH	HE EXISTING HE ACTUAL		
CONDIT	IONS AT	THE F	PROJECT SITE.				
FOR ER	OSION C	ONTROL	L MEASURES, SEE ERO	SION CON	IROL PLANS.		
FOR AS	BESTOS	ASSES	SMENT FOR BRIDGE D	FMOLTTTO	N AND RENOVATION		
ACTIVI	TIES, SE	E SPE	CIAL PROVISIONS.				
				PROJE	CT NO. <u>41</u>	665.7	А
				CL	JMBERLAND	CO	
МАС	ASBES	STOS			21+57	CO 23 _l	_
INT EALS	ASSESS	MENT		SIAIL	109+6	<u>23 L</u> 9_94 -	- 2-
P SUM	LUMP	SUM		SHEET 3	OF 3		
P SUM	LUMP	SUM		DFP	STATE OF NORTH CA	ROLINA NSPORTA	ΓΤΟΝ
			WRTH CAROL		RALEIGH		
			SEAL 037/80	G	ENERAL DF	RAWIN	IG
P SUM	LUMP	SUM	REF. NGINEE	BF	RIDGE ON US	301 OV	ER
			THAD F WERMIN	BETV	VEEN I-95 BU	SINESS	OFF
			Docusigned by: Auchan F. Duitma	ΓA	WIF AIND 1-95	DUSTINE	
2610 Wycli Suite 102	iff Road		11/14/2017	NO. RV.	REVISIONS	ΝΑΤΕ •	SHEET NO. SO1-3
Raleigh NG (919) 420-2 NC Lic. No	5 27607-3073 7660 5. F-0270	DOCUME FI SIGN	NT NOT CONSIDERED NAL UNLESS ALL ATURES COMPLETED	1	3 4		TOTAL SHEETS 24

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			LOAD	ANE) res	ISTA	NCE	FAC	TOR	RAT	ING	(LRF	R) SI	JMMA	RY F	FOR	STEE	LG]	IRDEF	RS				
										STRE	NGTH	I LIM	IIT ST	ΓΑΤΕ			SERVICE II LIMIT STATE							
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	NZA	$\langle 1 \rangle$	1.48		1.75	0.648	1.48	А	EL	45.86	0.959	1.85	А	I	0.00	1.30	0.648	1.65	А	EL	20.00	
DESIGN		HL-93 (OPERATING)	N/A		1.92		1.35	0.648	1.92	А	EL	45.86	0.959	2.40	А	I	0.00	1.00	0.648	2.14	А	EL	20.00	
RATING		HS-20 (INVENTORY)	36.00	$\langle 2 \rangle$	2.03	73.08	1.75	0.648	2.03	А	EL	45.86	0.959	2.49	А	I	0.00	1.30	0.648	2.24	А	EL	20.00	
		HS-20 (OPERATING)	36.00		2.63	94.68	1.35	0.648	2.63	А	EL	45.86	0.959	3.22	А	I	0.00	1.00	0.648	2.91	А	EL	20.00	
		SNSH	13.500		5.38	72.63	1.40	0.648	6.03	А	EL	45.86	0.959	7.65	А	I	0.00	1.30	0.648	5.38	А	EL	20.00	
		SNGARBS2	20.000		3.81	76.20	1.40	0.648	4.35	А	EL	45.86	0.959	5.35	А	I	0.00	1.30	0.648	3.81	А	EL	20.00	
	ICLE	SNAGRIS2	22.000		3.53	77.66	1.40	0.648	4.04	А	EL	45.86	0.959	4.94	А	I	0.00	1.30	0.648	3.53	А	EL	20.00	
	<pre>// CHH</pre>	SNCOTTS3	27.250		2.68	73.03	1.40	0.648	2.95	А	EL	45.86	0.959	3.81	А	I	0.00	1.30	0.648	2.68	А	EL	20.00	
	C (S	SNAGGRS4	34.925		2.19	76.49	1.40	0.648	2.43	А	EL	45.86	0.959	3.11	А	I	0.00	1.30	0.648	2.19	А	EL	20.00	
	ING	SNS5A	35.550		2.16	76.79	1.40	0.648	2.39	А	EL	45.86	0.959	3.11	А	I	0.00	1.30	0.648	2.16	А	EL	20.00	
		SNS6A	39.950		1.96	78.30	1.40	0.648	2.17	А	EL	45.86	0.959	2.82	А	I	0.00	1.30	0.648	1.96	А	EL	20.00	
LEGAL		SNS7B	42.000		1.89	79.38	1.40	0.648	2.06	А	EL	45.86	0.959	2.74	А	I	0.00	1.30	0.648	1.89	А	EL	20.00	
RATING	ER	TNAGRIT3	33.000		2.42	79.86	1.40	0.648	2.65	А	EL	45.86	0.959	3.38	А	I	0.00	1.30	0.648	2.42	А	EL	20.00	
	RAII	TNT4A	33.075		2.35	77.73	1.40	0.648	2.65	А	EL	45.86	0.959	3.31	А	I	0.00	1.30	0.648	2.35	А	EL	20.00	
	L-IV	TNT6A	41.600		1.98	82.37	1.40	0.648	2.15	А	EL	45.86	0.959	2.87	А	I	0.00	1.30	0.648	1.98	А	EL	45.86	
	SEN ST)	TNT7A	42.000		1.94	81.48	1.40	0.648	2.16	А	EL	45.86	0.959	2.82	А	I	0.00	1.30	0.648	1.94	А	EL	20.00	
	TOR (TT)	TNT7B	42.000		1.93	81.06	1.40	0.648	2.20	А	EL	45.86	0.959	2.70	А	I	0.00	1.30	0.648	1.93	А	EL	20.00	
	TRAC	TNAGRIT4	43.000		1.87	80.41	1.40	0.648	2.11	A	EL	45.86	0.959	2.62	A	I	0.00	1.30	0.648	1.87	А	EL	20.00	
	JCK	TNAGT5A	45.000		1.84	82.80	1.40	0.648	2.00	A	EL	45.86	0.959	2.57	A	I	0.00	1.30	0.648	1.84	A	EL	45.86	
	TRL	TNAGT5B	45.000	$\langle 3 \rangle$	1.78	80.10	1.40	0.648	1.98	А	EL	45.86	0.959	2.50	A	I	0.00	1.30	0.648	1.78	Α	EL	20.00	
FATIGUE		HL-93 (INVENTORY)	γ _{LL} =0.75																					



ASSEMBLED BY : B.A. WHITE	DATE : 09/20/17
CHECKED BY : R.F. WERTMAN	DATE : 09/24/17
DRAWN BY : MAA 1/08	REV.II/I2/08RR MAA/GM
CHECKED BY : GM/DI 2/08	REV.IO/I/II MAA/GM





LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\rm DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE II	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE II LIMIT STATES. ALLOWABLE STRESS FOR SERVICE II LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93) **
$\langle 2 \rangle$ design load rating (HS-20) **
$\sqrt{3}$ Legal load rating $**$
** SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER – EXTERIOR RIGHT GIRDER

	PROJE <u>Cl</u> Stati	CT NO. 4 JMBERLANI ON: 21+57 109+6	<u>1665.7</u>) co .23 -L .9.94 -	<u>-</u> -L2-
DocuSigned by	DEP ROLING DEP DEP NER WERNING	STATE OF NORTH C ARTMENT OF TR RALEIGH STANDA LRFR SUMMA STEEL GI NON-INTERSTAT	ARD ARY FOF RDERS E TRAFFI	TION R C)
d 08901A86EBF64 11/7/2017	70	REVISIONS		SHEET NO.
7-3073 DOCUMENT NOT CON FINAL UNLESS SIGNATURES COM	ISIDERED NO. BY: All 1 Pleted 2	DATE: NO. BY: 3 4	DATE:	SO1-4 TOTAL SHEETS 24
	STR. NO. 1	STD. N	D.LRFR3)



YPICAL SECTION

STEEL PLATE GIRDER 34"WEB DEPTH

____€ JOINT /---#5 ``K'' BARS NOTES: $3^{1}/_{2}^{\prime\prime}$ CL. TO S1 BAR -#4 S1 BARS PROVIDE 11/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO — "A" BARS BENT, SEE PLANS FOR BRIDGE SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING APPROACH SLABS. REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS *"B"* BARS FOR METAL DECK (C.H.C.M.) @ 4'-O"CTS. WITH A $-1\frac{1}{2}$ "HIGH B.B.U. HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF $2^{1}/_{2}$ " Above the top of the PARALLEL REMOVABLE FORM. TO JT. BARRIER RAIL SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE HAS BEEN CAST AND HAS REACHED A MINIMUM $\frac{3}{4}'' \oslash X 4''$ 2″CL.TO COMPRESSIVE STRENGTH OF 3,000 PSI. SHEAR STUDS $-0^{1}/2^{\prime}$ ``K'' BARS @ 1'-0"CTS. - N/ THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A 2"HIGH B.B. ◄── 2″ CL. TO S1 BAR SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL AT 5'-0"CTS. STAY-IN-PLACE FORM SUPPORTS OR FORMS AND ₩ #5 G1 BA BEAM/GIRDER STIFFENERS OR CONNECTOR PLATES. THE CONN. P - $2'-5\frac{1}{2}''$ SHIFTE PROPOSAL SHALL BE INDICATED, AS APPROPRIATE, ON AS NEC EITHER THE STEEL WORKING DRAWINGS OR THE METAL CLEAR STAY-IN-PLACE FORM WORKING DRAWINGS. -BRG.STIFFENER STEEL FOR VERTICAL CONCRETE BARRIER RAIL REINFORCING STEEL AND DETAILS, SEE "VERTICAL CONCRETE BARRIER RAIL PLAN & DETAIL"SHEET. FILL FACE-----METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO BEAM OR GIRDER FLANGES IN THE ZONES REQUIRING CHARPY V-NOTCH TEST. SEE STRUCTURAL STEEL DETAIL SHEETS. END OF GIRDER DETAIL AT END BENT

PREVIOUSLY CAST CONCRETE IN A SPAN SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE SPAN.

drawn by : <u>B</u> .A. WHITE	DATE :	09/11/17
CHECKED BY : R.F. WERTMAN	DATE :	09/24/17
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	. DATE :	11/06/17

INTERMEDIATE DIAPHRAGMS



 $10\frac{1}{2}$ TOP OF SLAB TO BOTTOM OF TOP FLANGE @ 🕻 BRG.



⁄₄″HIGH B.B.U. EE NOTES TAY-IN-PLACE ETAL FORMS	PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> COUN	TY
AR MAY BE D SLIGHTLY, ESSARY,TO REINFORCING & STIRRUPS-	STATION: 21+57.23 -L- 109+69.94 -L2 SHEET 1 OF 3	2-
NUMBER OF THE CAROLINA	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATIO RALEIGH	N
SEAL 037180	SUPERSTRUCTURE	
F WERMUN	TYPICAL SECTION	
DocuSigned by: Dicheen F. Vuitmo 1108901A866BF6470		ET NO
		01-5
F-0270 DOCUMENT NOT CONSIDERED F-0270 SIGNATURES COMPLETED		otal Heets 24





DRAWN BY : B.A. WHITE	DATE: 09/18/17
CHECKED BY : R.F. WERTMAN	DATE: 09/25/17
DESTGN ENGINEER OF RECORD : R.F. WERTMAN	DATE: 11/06/17

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<u>PLAN OF SPAN A</u>





DRAWN BY : B.A. WHITE	DATE : <u>09/12/17</u>
CHECKED BY : <u>R.F. WERTMAN</u>	DATE : <u>09/26/17</u>
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE : <u>11/06/17</u>





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NOTES:

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH SYSTEM 4 OF ARTICLE 442-8 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL, UNLESS OTHERWISE NOTED.

ALL FIELD CONNECTIONS TO BE $\frac{7}{8}$ "DIA. HIGH STRENGTH BOLTS UNLESS OTHERWISE NOTED.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB.

A CHARPY V-NOTCH TEST IS REQUIRED FOR WEB PLATES AND BOTTOM FLANGE PLATES FOR ALL GIRDERS AND IN ACCORDANCE WITH ARTICLE 1072-7 OF THE STANDARD SPECIFICATIONS.

PERMITTED FLANGE AND WEB SHOP SLICES SHALL NOT BE LOCATED WITHIN 15 FEET OF MAXIMUM DEAD LOAD DEFLECTION (NOR WITHIN 15 FEET OF INTERMEDIATE BEARINGS OF CONTINUOUS UNITS). KEEP 2 FEET MINIMUM BETWEEN WEB AND FLANGE SHOP SLICES. KEEP 6" MINIMUM BETWEEN CONNECTOR PLATE OR TRANSVERSE STIFFENER WELDS AND WEB OR FLANGE SHOP SPLICES.

STUDS ON GIRDERS MAY BE SHIFTED UP TO 1"IF NECESSARY TO CLEAR FLANGE SPLICE WELD.

TENSION ON THE ASTM A325 BOLTS SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH ARTICLE 440-8 OF THE STANDARD SPECIFICATIONS.



CONNECTOR PLATE DETAILS

) AND	DRIP	PROJECT	NO	410	665.7	A
, 2'' X 1'	′ P	CUM	BERL	AND	C0	UNTY
<mark>- * *</mark> (TYP. 2)	STATION	l: <u>21</u> 10	+57.2 9+69	<u>23 -L</u>).94 -	<u>-</u> -L2-
		SHEET 2 OF :	2			
<mark>∢ * *</mark> (TYP.)	DEPART	state C	OF NORTH CARC DF TRAN RALEIGH	ISPORTA	TION
AILS''	ROFESSION F WERTING	s STF	super: RUCTI	struc URAL	ture . STE	EL
Road	Docusigned by: Cuchen F. Vuitor 08901A86EBF6470 11/7/2017		REVISIO	DNS		SHEET NO.
27607-3073 60 F-0270	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL STGNATURES COMPLETED	NO. BY:	DATE: NO	BY:	DATE:	SO1-9 TOTAL SHEETS
	STORATORES COMILETED					24



ASSEMBLED BY : B.A. WHITE CHECKED BY : R.F. WERTMA	DATE : 09/14/17 N DATE : 09/26/17
DRAWN BY: EEM 2/97 CHECKED BY:VAP 2/97	REV. 10/1/11 MAA/GM REV. 6/13 AAC/MAA REV. 1/15 MAA/TMG









NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF $^{1\!/}_{2}$ TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

THE PAYMENT FOR THE PIPE SLEEVES SHALL BE INCLUDED IN THE SEVERAL PAY ITEMS.

FOR PAINTED STRUCTURAL STEEL (EXCLUDING AASHTO M270 GRADE 50W), SOLE PLATES, ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

FOR AASHTO M270 GRADE 50W STRUCTURAL STEEL, SOLE PLATE SHALL BE AASHTO M270 GRADE 50W AND SHALL NOT BE GALVANIZED. ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

WHEN FIELD WELDING THE SOLE PLATE TO THE GIRDER FLANGE, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

—LOAD RATING—							
	MAX.D.L.+L.L.						
TYPE IV	310 K						

-	PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> county STATION: <u>21+57.23</u> -L- 109+69.94 -L2-
DocuSigned by:	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD ELASTOMERIC BEARING DETAILS — DETAILS (STEEL SUPERSTRUCTURE)
09901A86EBF6470 11/7/2017	REVISIONS SHEET NO.
3073 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: SOI-IO 1 3 TOTAL SHEETS 24
	STR. NO. 1 STD. NO. EB2

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DEAD LOAD DEFLECTION TABLE FOR GIRDERS																						
				GI	RDER	1 & G	IRDER	4				GIRDER 2 & GIRDER 3										
TENTH POINTS	0	.1	. 2	. 3	.4	. 5	.6	. 7	. 8	.9	0	0	.1	.2	.3	" 4	. 5	. 6	. 7	. 8	.9	0
DEFLECTION DUE TO WEIGHT OF GIRDER	0	0.023	0.043	0.059	0.069	0.072	0.069	0.059	0.043	0.023	0	0	0.023	0.043	0.059	0.069	0.072	0.069	0.059	0.043	0.023	0
DEFLECTION DUE TO WEIGHT OF SLAB 米	0	0.060	0.118	0.162	0.190	0.200	0.190	0.162	0.118	0.060	0	0	0.051	0.102	0.140	0.164	0.173	0.164	0.140	0.102	0.051	0
DEFLECTION DUE TO WEIGHT OF BARRIER RAIL	0	0.013	0.026	0.033	0.038	0.040	0.038	0.033	0.026	0.013	0	0	0.013	0.026	0.033	0.038	0.040	0.038	0.033	0.026	0.013	0
TOTAL DEAD LOAD DEFLECTION	0	0.096	0.187	0.254	0.297	0.312	0.297	0.254	0.187	0.096	0	0	0.087	0.171	0.232	0.271	0.285	0.271	0.232	0.171	0.087	0
VERTICAL CURVE ORDINATE	0	0.086	0.152	0.200	0.228	0.238	0.228	0.200	0.152	0.086	0	0	0.086	0.152	0.200	0.228	0.238	0.228	0.200	0.152	0.086	0
REQUIRED CAMBER	0	2 ³ / ₁₆ ″	4 / ₁₆ ″	57⁄16″	6 ⁵ /16″	6 ⁵ ⁄8″	6 ⁵ /16″	57⁄16″	4 / ₁₆ ″	2 ³ / ₁₆ ″	0	0	2 / ₁₆ ″	37⁄8″	5 ³ /16″	6″	6 /4″	6″	5 ³ /16″	31⁄8″	2 / ₁₆ ″	0

* INCLUDES SLAB, BUILDUPS & STAY-IN-PLACE FORMS. ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT `` REQUIRED CAMBER '', WHICH IS GIVEN IN INCHES (FRACTION FORM). FABRICATORS SHALL DETAIL DIAPHRAGM MEMBERS AND CONNECTIONS FOR FULL DEAD LOAD FIT UP.GIRDERS SHALL BE PLUMB AFTER THE FULL AMOUNT OF DEAD LOAD IS APPLIED.

DRAWN BY : B.A. WHITE	DATE : <u>09/15/17</u>
CHECKED BY : R.F. WERTMAN	date : <u>09/27/17</u>
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE : <u>11/06/17</u>



SCHEMATIC OF CAMBER ORDINATES



		PROJEC CU STATIC	X NO. MBER DN: <u>2</u> 1(<u>41</u> LAND 1+57.1 09+69	<u>665.7</u> co 23 -L).94 -	<u>A</u> UNTY
	SEAL 037180	DEPA	RTMENT SUPEF	OF NORTH CARI OF TRAN RALEIGH RSTRUC	NSPORTA TURE	TION
	DocuSigned by:	DE	FLEC	TION	TABL	ES
Road	08901A86EBF6470 11/7/2017		REVIS	IONS		SHEET NO.
7607-3073 50 -0270	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL STGNATURES COMPLETED	NO. ВҮ: 1 の	DATE:	NO. ВҮ: З	DATE:	SO1-11 TOTAL SHEETS

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** SEE "END OF RAIL DETAILS" ON SHEET 2 OF 2 FOR ADDITIONAL REINFORCING STEEL.

DIMENSIONS ARE SHOWN FROM 🕻 JOINT AT BACK FACE OF BARRIER RAIL

DRAWN BY : B.A. WHITE	DATE :	09/11/17
CHECKED BY : T.M. FORD	DATE :	09/27/17
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE :	11/06/17

96'-3 $^{\rm l}\!/_{\rm 8}''$ C jt end bent 1 to C jt end bent 2

PLAN OF BARRIER RAIL



		PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> county STATION: <u>21+57.23</u> -L- 109+69.94 -L2- SHEET 1 OF 2
	TH CAROL	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
	SEAL 037180 F WER	VERTICAL CONCRETE BARRIER RATL
Road	DocuSigned by: Dichen F. Vuitno 08901A86EBF6470 11/7/2017	REVISIONS SHEET NO.
7607-3073 50 -0270	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: SO1-12 1 3 3 TOTAL SHEETS 24

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NOTES

THE BARRIER RAIL IN EACH SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE

WHEN FOAM JOINT SEAL IS REQUIRED, THE JOINT IN THE DECK SHALL BE SAWED PRIOR TO THE CASTING OF VERTICAL CONCRETE BARRIER RAIL.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

THE #5 S3 & S4 BARS SHALL BE INSTALLED, USING AN ADHESIVE ANCHORING SYSTEM, AFTER SAWING THE JOINT. THE YIELD LOAD FOR THE #5 S3 & S4 BARS IS 18.6 KIPS. FIELD TESTING FOR THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS



	PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> COUNTY STATION: <u>21+57.23</u> -L- 109+69.94 -L2-
OFESSION SEAL 037180	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD
THE F WERMIN	CONCRETE
DocuSigned by: Richard F. Vuitors	BARRIER RAIL
11/7/2017	REVISIONS SHEET NO.
7-3073 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL	NO. BY: DATE: SOT TS 1 3 TOTAL SHEETS
SIGNATURES COMPLETED	<u>2</u> (4) 24

STR.NO.1

STD. NO. CBR2

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THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $1/4^{\prime\prime}$ HOLD DOWN PLATE AND 7 - $1/8^{\prime\prime}$ Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/811 Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL

THE 1 $\frac{1}{4}$ // Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED

		PROJEC CUI STATIC	T NO. MBER 0N: <u>2</u> 1(<u>41</u> LAND 1+57. 09+69	<u>665.7</u> co 23 -L 9.94 -	<u>A</u> UNTY
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SEAL 037180 GUARDRAIL ANCHORAGE FOR BARRIER RAIL						
Road	Uchen & Vuitma 08901A86EBF6470 11/7/2017		REVIS	SIONS	D 1 T 5	SHEET NO.
7607-3073 60 0270	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	вү: 1 2	DATE:	мо, вү: 3 4	DATE:	TOTAL SHEETS 24
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STR.NO.1

SID. NU. GRAS





			RF		TNG B	AR S	CHEDI	ΠF		
BAR	NO.	ST7F	TYPE	LENGTH	WEIGHT			ST7F	TYPF	
* A1	1.31	#5	STR	25'-8"	3507	A201	4	#5	STR	Г
A2	131	#5	STR	25'-8"	3507	A202	4	#5	STR	┢
* A3	6	#6	STR	15'-0"	136	A203	4	#5	STR	t
* A101	4	#5	STR	24'-10"	104	A204	4	#5	STR	t
* A102	4	#5	STR	24'-0"	101	A205	4	#5	STR	t
* A103	4	#5	STR	23″-2″	97	A206	4	#5	STR	T
* A104	4	#5	STR	22'-4″	94	A207	4	#5	STR	T
* A105	4	#5	STR	21'-5″	90	A208	4	#5	STR	t
* A106	4	#5	STR	20′-7″	86	A209	4	#5	STR	T
₩ A107	4	#5	STR	19′-9″	83	A210	4	#5	STR	Γ
₩ A108	4	#5	STR	18'-11″	79	A211	4	#5	STR	Γ
₩ A109	4	#5	STR	18'-1"	76	A212	4	#5	STR	Γ
★ A110	4	#5	STR	17'-3″	72	A213	4	#5	STR	T
* A111	4	#5	STR	16′-5″	69	A214	4	#5	STR	Γ
★ A112	4	#5	STR	15′-7″	65	A215	4	#5	STR	Γ
* A113	4	#5	STR	14'-9"	62	A216	4	#5	STR	Γ
★ A114	4	#5	STR	13′-11″	58	A217	4	#5	STR	
米 A115	4	#5	STR	13'-1"	55	A218	4	#5	STR	
₩ A116	4	#5	STR	12′-3″	52	A219	4	#5	STR	
米 A117	4	#5	STR	11′-5″	48	A220	4	#5	STR	
米 A118	4	#5	STR	10'-7"	45	A221	4	#5	STR	
₩ A119	4	#5	STR	9′-9″	41	A222	4	#5	STR	
₩ A120	4	#5	STR	8'-10"	37	A223	4	#5	STR	
米 A121	4	#5	STR	8'-0"	34	A224	4	#5	STR	
米 A122	4	#5	STR	7′-2″	30	A225	4	#5	STR	
米 A123	4	#5	STR	6'-4"	27	A226	4	#5	STR	
米 A124	4	#5	STR	5′-6″	23	A227	4	#5	STR	L
米 A125	4	#5	STR	4'-8"	20	A228	4	#5	STR	
₩ A126	4	#5	STR	3'-10"	16	A229	4	#5	STR	L
米 A127	4	#5	STR	3'-0"	13					L
米 A128	4	#5	STR	2'-2"	9	米 B1	72	#4	STR	
₩ A129	4	#5	STR	1'-4"	6	B2	66	#5	STR	╞
						₩ G1	2	#5	STR	┞
										t
GRO	OVIN	G BF	RIDG	E FLOC	RS	米 K1	12	#5	1	L
			,			₩ K2	12	#5	2	
RKID	GE DEC	K	4	2016 SQ.F	- ₀	₩ K3	18	#5	STR	Ļ
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ΤΟΤΑ	L			3052 SQ.F	Τ.	REINF	DRCING	STEFI		L
						* EPOX	Y COAT	ED RE	INF.S	Τſ

SUPERSTRUCTURE BILL OF MATERIAL							
	CLASS AA CONCRETE	REINFORCING STEEL	*EPOXY COATED REINFORCING STEEL				
	(CU.YDS.)	(LBS.)	(LBS.)				
TOTALS	69.7	8532	6,995				

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS								
BAR SIZE	SUPERST EXCEPT A SLABS, P AND BARR	RUCTURE APPROACH ARAPET, IER RAIL	APPROAC	PARAPET AND BARRIER				
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED				
#4	2'-0″	1'-9″	2'-0″	1'-9″	2′-9″			
#5	2'-6″	2'-2″	2'-6″	2'-2″	3′-5″			
#6	3'-0"	2′-7″	3'-10″	2'-7″	4'-4"			



STD. NO. BOM1 STR.NO.1





DATE • 09/27/17
DATE: 10/03/17
DATE: 11/06/17
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STR.NO.1



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BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



1'-0" 1'-0" 1'-0" 1'-0" __2″CL. CONST. JT.— 5-#9 B8----5-#9 B1 — #5 S2-FILL FACE — -#5 S4 2″ CL. (TYP.)

PARTIAL SECTION C-C



DRAWN BY : J.A. BOYER	DATE: 09/26/17
CHECKED BY : R.F. WERTMAN	DATE: 10/07/17
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE: 11/06/17



STR.NO.1

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STR.NO.1

STD. NO. BAS2

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NC

			BILL OF MATERIAL						
DTES:	F	FOR ONE APPROACH SLAB							
E CONSTRUCTED PRIOR TO DECK.			BAR	NO.	SI:	ι` ΖΕ		LENGTH	WEIGHT
"MSE RETAINING WALL" PLA	NS.		* A1 A2	50 52	#	4 4	STR STR	21'-2" 21'-0"	707 730
SPECIAL PROVISIONS.				51	#	5	STR	24'-2"	1286
SEAL WIDTH OF THE FOAM J		B2 51 #6 STR 24'-8" 1890							
SEE SPECIAL PROVISIONS.		REINFORCING STEEL LBS. 2620							
			* EP RE	OXY CO INFORO	DATE CINC	ED G S	TEEL	LBS.	1993
			CLAS	S AA	CON	CRE	ETE	C.Y.	27.9
				_					
CONCRETE				Sf BA	PLIC R E	<u>)е</u> ЕРО	LENGTH XY	S CHART	
STOMERIC NCRETE *				SI #	ZE (2′-	.TED U -0"	NCOATED 1'-9"	
CU.FT.)				#	5	2'- 3'-	-6″	2'-2"	
6.8 17.0					0	0	10	<u> </u>	
INIMUM BLOCKOUT SHOWN.									
R◀	-								
	2000000			SS \\B'	′ ST(ONF			
		FC	DR EROS	ION C	onte	ROL	-		
TEMP. SLOPE DRAIN	1'-0"								
	MIN.	7		IRE					
		<u> </u>	\longrightarrow						
DRAIN	-///	$\langle \rangle$		2'-0"	MIN				
GRADE TO INLE	<u>////</u> 	4			2'-6"				
FLOW LINE		(\smile		<u> </u>				
LI'-6" MIN. EROSI	ON RES	IST	ANT MA	TERIA	L				
ATELY AFTER THE CONSTRUCT	TTON OF	- т				R			
ITRACTOR SHALL PROVIDE THE CONSTRUCT CONTRACTOR SHALL GRADE T	EMPORAF	' Y? NI	BERM A	ND SLO)PE	D,			
OVIDE EROSION RESISTANT N RESISTANT MATERIAL SHA	MATERIA LL BE E	AL ITI	AS SHO HER 1) A	WN. TH SPHAL	IE T				
AIX, TYPE 1 OR TYPE 2, MIN. 3) CONCRETE, AS DIRECTED	2" DEPT BY THE	H, 2 EN	2) EROSI GINEER.	ON CO	NTR	ЭL			
ARY DRAINAGE PIPE, 12 INC	HES IN	DI	AMETER.	IEU					
PLAN VIEW									
		PF	ROJE	CT N	0.		41	665./	Α
			CL	MBE	<u> </u>		and	CO	UNTY
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				_	STATE	. OF	NORTH CAR	DLINA	
WILLING CAR			DEPA	RTME	NT	OF ₅	TRAN RALEIGH	NSPORTA	TION
A CFESSIO					ST	Άľ	NDAR	D	
037180 Distance of the second			В	RID	GE	_	APF	PROAC	CH
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iff Road 11/7/2017	Weitma	E		R	EVIS	ION	S		SHEET NO.
TEREST CONSTRUCTIONS	IDERED	ุ№0. ป	BY:	DATE		NO.	BY:	DATE:	SU1-23 TOTAL SHEETS
SIGNATURES COMPL	_ETED	2	_	_		4			24

STR.NO.1

STD.NO.BAS4

drawn by : B.A. WHITE	DATE : 10/03/17
снескер ву : Т.М. FORD	DATE: 10/05/17
DESIGN ENGINEER OF RECORD :	DATE : 11/06/17



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NOTES

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS, MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.

SLOPE PROTECTION SHALL CONSIST OF 4"POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET.CONCRETE SHALL BE CLASS "B".THE CONCRETE SURFACE SHALL BE FINISHED TO THE SATISFACTION OF THE ENGINEER. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 20" WIDE. THE COST OF THE WELDED WIRE FABRIC SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.21+57.23 -L- STA.109+69.94 -L2-	4 INCH SLOPE PROTECTION	* Welded Wire Fabric 60 inches Wide
	SQUARE YARDS	APPROX.L.F.
END BENT 1	12	22
END BENT 2	12	22

* QUANTITY SHOWN IS BASED ON 5' POURS.

— MSE	: WALL	PF 	ROJEC CU Fatio	CT I <u>ME</u> ON:	NO. BER <u>2</u> 1(1+)	41 <u>AND</u> - <u>57.</u>)+69	<u>665.7</u> cc 23 -l 3.94	<u>^</u> -L2-
	SEAL 037180 F WERMIN		depa SL	OF	MENT S ⁻ DE DE		NORTH CARG TRAN RALEIGH NDAR ROTI	D D ECTI	tion ON
ad	DocuSigned by: Dichew F. Vuitor 08901A86EBF6470 11/7/2017				REVIS	SION	S		SHEET NO.
07-3073	DOCUMENT NOT CONSTDERED	N0.	BY:	D	ATE:	NO.	BY:	DATE:	S01-24
270	FINAL UNLESS ALL SIGNATURES COMPLETED	1 2				3 4			TOTAL SHEETS 24







DRAWN RY - 1 A BOYER	DATE . 10/11/17
DRAWN BI : ORA: DOTEN	10/12/17
CHECKED BY : T.F. WERTMAN	DATE : 10/12/17
DESIGN ENGINEER OF RECORD : <u>R.F. WERIMAN</u>	. date : <u>11/06/17</u>





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	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	STRUCTURAL STEEL APPROX. 122,200 LBS.	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	HP STE	12 X 53 El PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	4″SLOPE PROTECTION	ELASTOMERIC BEARINGS	FOAM JOINT SEALS
	LUMP SUM	EACH	SQ.FT.	SQ.FT.	CU.YDS.	LUMP SUM	LBS.	LUMP SUM	EACH	NO.	LIN.FT.	EACH	LIN.FT.	SQ.YDS.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE			3866	4939		LUMP SUM		LUMP SUM					192.1		LUMP SUM	LUMP SUM
END BENT NO.1					51.3		7053		8	8	576	8		24		
END BENT NO.2					51.9		7162		8	8	576	8		49		
TOTAL	LUMP SUM	1	3866	4939	103.2	LUMP SUM	14203	LUMP SUM	16	16	1152	16	192.1	73	LUMP SUM	LUMP SUM

DRAWN BY : J.A. BOYER	DATE : 09/03/17
CHECKED BY : _R.F. WERTMAN	DATE: 10/16/17
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE: <u>11/06/17</u>

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NEEDL FOR (LE B DN T
ALL S AND F THE S PLANS	STRU PAIN STAN S.
INASI STEEL TO AI RESUL RESUL LEAD "REMC	MUCH _ CO RTIC _TIN _ATI BAS)VAL
THE E SPAN DECK ON PE REINE LOCA BRID(EXIS @ 6 ON RECA FORC TED GE I
THE S PLANS INFOR THE (SUBS S IS RMAT CONT



		NOTES:				
ASSUME	D LIVE LOAD	= HL-93 OR ALTERNA	TE LOADIN	G.		
THIS B LRFD B	RIDGE HAS BEE RIDGE DESIGN	EN DESIGNED IN ACC SPECIFICATIONS.	ORDANCE W	/ITH THE AASHTO		
THIS B	RIDGE IS LOCA	ATED IN SEISMIC ZO	DNE 1.			
FOR OT	HER DESIGN DA	ATA AND GENERAL NO)TES,SEE S	SHEET SN.		
FOR SU	BMITTAL OF W	ORKING DRAWINGS, S	EE SPECIAI	_ PROVISIONS.		
FOR CR	ANE SAFETY. SE	FF SPFCTAL PROVIST	ONS.	STONS.		
FOR GR	OUT FOR STRU	CTURES, SEE SPECIAL	PROVISIO	DNS.		
THE EL OF MIN AVAILA ELEVAT REPORT NECESS WILL B	EVATION AND (IMUM VERTICA BLE.PRIOR TO ION ON THE EX ANY VARIATI ARY TO ACHIEY E PROVIDED BY	CLEARANCE SHOWN ON L CLEARANCE ARE FO BEGINNING BRIDGE (ISTING PAVEMENT A ONS TO THE ENGINED VE THE REQUIRED MI (THE DEPARTMENT.	N THE PLAN ROM THE B CONSTRUC AND CHECK ER.ANY PL INIMUM VE	IS AT THE POINT EST INFORMATION TION, VERIFY THE THE CLEARANCE. AN REVISIONS RTICAL CLEARANCE		
FOR MA STRUCT	INTENANCE AN URE, SEE SPEC	D PROTECTION OF TH IAL PROVISIONS.	RAFFIC BEN	NEATH PROPOSED		
REMOVA FORMS SPECIF	BLE FORMS MA IN ACCORDANC ICATIONS.	Y BE USED IN LIEU E WITH ARTICLE 420	OF METAL)-3 OF THE	STAY-IN-PLACE STANDARD		
NEEDLE FOR ON	BEAMS WILL N THE PLANS OF	NOT BE ALLOWED UNL R APPROVED BY THE	ESS OTHER ENGINEER.	WISE CALLED		
ALL ST AND PA THE ST PLANS.	RUCTURAL STEE INTED IN ACC ANDARD SPECIE	ORDANCE WITH SYSTE	M270 GRA EM 4 OF AI DTHERWISE	DE 50W RTICLE 442-8 OF NOTED ON THE		
INASML STEEL	ICH AS THE PAT CONTAINS LEAD	INT SYSTEM ON THE), THE CONTRACTOR'S	EXISTING ATTENTIO	STRUCTURAL N IS DIRECTED		
RESULT	ICLE IO7-I OF ING FROM COM ITONS PERTAT	THE STANDARD SPEC PLIANCE WITH APPLI NTNG TO HANDITNG (IFICATION ICABLE STA DF MATERTA	ATE OR FEDERAL		
LEAD B "REMOV,	ASED PAINT S AL OF EXISTIN	HALL BE INCLUDED I G STRUCTURE AT ST	N THE BID ATION 106	PRICE FOR +59.74 -L-",		
THE EX	ISTING STRUC	TURE CONSISTING OF	TWO SPAI	NS @ 60'-0" AND ON	IE	
DECK O ON PRE	N STEEL I-BEA CAST PRESTRES	MS ON END BENTS W SSED CONCRETE PILE	ITH REINF	ORCED CONCRETE C RIOR BENTS WITH	APS	
REINFO LOCATE	RCED CONCRETE	E CAPS ON PRECAST Posed structure sh / Not posted for 1	PRESTRESS	SED CONCRETE PILE MOVED.THE EXISTI	S NG	
THE SU	BSTRUCTURE OF	THE EXISTING BRI	DGE INDIC	ATED ON THE		
PLANS INFORM THF CO	IS FROM THE ATION IS SHO NTRACTOR SHAL	BEST INFORMATION . WN FOR THE CONVEN: I HAVF NO CLATM W	AVAILABLE IENCE OF ⁻ VHATSOFVEF	SINCE THIS The contractor, r against the		
DEPART COST I	MENT OF TRAN	SPORTATION FOR AN O ON DIFFERENCES E	Y DELAYS Between th	OR ADDITIONAL E EXISTING		
BRIDGE CONDIT	SUBSIRUCIURI IONS AT THE	- SHOWN ON THE PLA Project site.	NS AND IF	IE ACIUAL		
FOR ER	OSION CONTRO	_ MEASURES, SEE ERC	SION CON	FROL PLANS.		
FOR PL	ACING LOAD O	N SIRUCIURE MEMBER Sment for bridge f	RS, SEE SPE Demoi titoi	CIAL PROVISIONS. N AND RENOVATION		
ACTIVI	TIES, SEE SPE	CIAL PROVISIONS.		AND RENOVATION		
			PROJE	CT NO. <u>4</u>	1665.7	Α
			CL	JMBERLAND) CO	UNTY
OAM DINT	ASBESTOS ASSESSMENT		STATI	ON: 106+5	9.74 -	1-
LALS			SHFFT 2 (14+51. DF 2	19 -Y	_
IP SUM	LUMP SUM			STATE OF NORTH C	ROLINA	
		HINNING CAROLING	DEP	ARTMENT OF TRA	ANSPORTA [®]	TION
		SEAL 037/B0	G	eneral di	RAWIN	IG
P SUM	LUMP SUM	ROW CINER ST	RDT			FCC
		DocuSigned by:		NBL OVER U	, <u>βοστη</u> S301 BE)δη ληγ	TWEEN
2010 111	iff Doc-	Dichaul Fo Vuitoro 08901A86EBF6470 11/14/2017			ANL	SHEET NO
2610 Wycli Suite 102 Raleigh NC	с 27607-3073 DOCUME	ENT NOT CONSIDERED	NO. BY:	DATE: NO. BY:	DATE:	SO2-3
(919) 420-2 ed NC Lic. No	7660 FI 5. F-0270 SIGN	NAL UNLESS ALL ATURES COMPLETED	า 2	3 4		SHEETS 24

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		Т	LUAD) ANL	J KES	\bot	NCE	FAC	IUK	KAI.	LNG	(LKF	K) Sl	JMMA	KY F	- UK	SIEE	LG.	KUE	72				
										STRE	NGTH	I LIM	IIT ST	ATE				S	ERVIC	E II	LIMIT	- STA	TE	
									-	MOMENT	-	-			SHEAR	-					MOMENT			
I F V F I	1 - -	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING #	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD Factors (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	NZA	$\langle 1 \rangle$	1,66		1.75	0.649	1.91	А	EL	45.97	0,926	2.24	А	I	0.00	1.30	0.649	1.66	А	EL	20,00	
DESI	IGN	HL-93 (OPERATING)	NZA		2.16		1.35	0.649	2.48	А	EL	45.97	0.926	2.90	А	I	0.00	1.00	0.649	2.16	А	EL	20.00	
RAT	ING	HS-20 (INVENTORY)	36.00	2	2.26	81.36	1.75	0.649	2.62	А	EL	45.97	0.926	3.00	А	I	0.00	1.30	0.649	2.26	А	EL	20.00	
		HS-20 (OPERATING)	36.00		2.93	105.48	1.35	0.649	3.40	А	EL	45.97	0.926	3.88	А	I	0.00	1.00	0.649	2.93	А	EL	20,00	
		SH	12.500		5.89	73.63	1.40	0.649	8.45	А	EL	45.97	0.926	9.99	А	I	0.00	1.30	0.649	5.89	А	EL	20.00	
		S3C	21.500		3.45	74.18	1.40	0.649	4.88	А	EL	45.97	0.926	5.85	А	I	0.00	1.30	0.649	3.45	А	EL	20.00	
	ICLE	S3A	22.750		3.27	74.39	1.40	0.649	4.63	А	EL	45.97	0.926	5.55	А	I	0.00	1.30	0.649	3.27	А	EL	20.00	
	<pre></pre>	S4A	26.750		2.85	76.24	1.40	0.649	4.08	А	EL	45.97	0.926	4.82	А	I	0.00	1.30	0.649	2.85	А	EL	20.00	
	SLE (S	S5A	30.500		2.54	77.47	1.40	0.649	3.60	А	EL	45.97	0.926	4.35	А	I	0.00	1.30	0.649	2.54	А	EL	20.00	
	DNIS	S6A	34.500		2.29	79.01	1.40	0.649	3.25	А	EL	45.97	0.926	3.93	А	I	0.00	1.30	0.649	2.29	А	EL	20.00	
LOAD		S7B	38.500		2.10	80.85	1.40	0.649	2.96	А	EL	45.97	0.926	3.65	А	I	0.00	1.30	0.649	2.10	А	EL	20.00	
RAIING		S7A	40.000	3	2.09	83.60	1.40	0.649	2.91	А	EL	45.97	0.926	3.70	А	I	0.00	1.30	0.649	2.09	А	EL	45.97	
	۲ _۲	Т4А	28.250		2.76	77.97	1.40	0.649	3.99	А	EL	45.97	0.926	4.66	А	I	0.00	1.30	0.649	2.76	А	EL	20.00	
	ACTO ILEF)	Т5В	32.000		2.50	80.00	1.40	0.649	3.51	А	EL	45.97	0.926	4.37	А	I	0.00	1.30	0.649	2.50	А	EL	20,00	
	TR/ TRA	Т6А	36.000		2.30	82.80	1.40	0.649	3.21	А	EL	45.97	0.926	3.99	А	I	0.00	1.30	0.649	2.30	А	EL	45.97	
	RUCK EMI- (1	Т7А	40.000		2.13	85,20	1.40	0.649	2.97	А	EL	45.97	0.926	3.69	A	I	0.00	1.30	0.649	2.13	A	EL	45.97	
		Т7В	40.000		2.11	84.40	1.40	0.649	3.15	А	EL	45.97	0.926	3.51	А	I	0.00	1.30	0.649	2.11	А	EL	20.00	
FATIGL	JE	HL-93 (INVENTORY)	γ _{LL} =0.75																					



END BENT 1

ASSEMBLED BY : B.A. WHITE	DATE : 10/12/17
CHECKED BY : R.F. WERTMAN	DATE : 10/13/17
DRAWN BY : MAA 1/08	REV.II/I2/O8RR MAA/GM
CHECKED BY : GM/DI 2/08	REV.IO/I/II MAA/GM







LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE II	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE II LIMIT STATES. ALLOWABLE STRESS FOR SERVICE II LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING
1 design load rating (HL-93) **
$\langle 2 \rangle$ design load rating (HS-20) **
$\left< \frac{3}{3} \right>$ Legal load rating $*$ *
** SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER - EXTERIOR RIGHT GIRDER

<u>CUMBERLAND</u> STATION: 106+59. 14+51.19	<u>COUNTY</u> 74 -L1- 9 -Y-
SEAL OSTIBO CHARGER ACTIONA	SPORTATION) Y FOR ERS AFFIC)
pocuSigned by: <i>Queckeul & Juethanianov 11/7/2017</i> REVISIONS REVISIONS 7-3073 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2 1 2 4 4 1 2 1 2 1 2 1 2 1 1 2 1 1 1 1 1 1 1 1	DATE: SHEET NO. SO2-4 TOTAL SHEETS 24



drawn by : <u>B</u> .A. WHITE	date : <u>10/10/17</u>
CHECKED BY : R.F. WERTMAN	DATE : <u>10/12/17</u>
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE : <u>11/06/17</u>

DocuSign Envelope ID: 3A4D0F7A-B2E8-4708-A87D-4003DA39697E

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STR.NO.2





DRAWN BY : B.A. WHITE	DATE: 10/04/17
CHECKED BY : R.F. WERTMAN	DATE: 10/14/17
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE : <u>11/06/17</u>







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NOTES:

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH SYSTEM 4 OF ARTICLE 442-8 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL, UNLESS OTHERWISE NOTED.

ALL FIELD CONNECTIONS TO BE $\frac{7}{8}$ "DIA. HIGH STRENGTH BOLTS UNLESS OTHERWISE NOTED.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB.

A CHARPY V-NOTCH TEST IS REQUIRED FOR WEB PLATES AND BOTTOM FLANGE PLATES FOR ALL GIRDERS AND IN ACCORDANCE WITH ARTICLE 1072-7 OF THE STANDARD SPECIFICATIONS.

PERMITTED FLANGE AND WEB SHOP SLICES SHALL NOT BE LOCATED WITHIN 15 FEET OF MAXIMUM DEAD LOAD DEFLECTION (NOR WITHIN 15 FEET OF INTERMEDIATE BEARINGS OF CONTINUOUS UNITS). KEEP 2 FEET MINIMUM BETWEEN WEB AND FLANGE SHOP SLICES. KEEP 6" MINIMUM BETWEEN CONNECTOR PLATE OR TRANSVERSE STIFFENER WELDS AND WEB OR FLANGE SHOP SPLICES.

STUDS ON GIRDERS MAY BE SHIFTED UP TO 1"IF NECESSARY TO CLEAR FLANGE SPLICE WELD.

TENSION ON THE ASTM A325 BOLTS SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH ARTICLE 440-8 OF THE STANDARD SPECIFICATIONS.



CONNECTOR PLATE DETAILS

WELD AND DRIP BEAD		PROJEC	T NO.	41	665.7	Α
- ∕₂′′X 1′′₽		CU	MBER	LAND	C0	UNTY
** (TYP.)		STATIC	DN: <u>1(</u>)6+59 1+51.1).74 - _9 -Y	- <u>L1-</u> -
		SHEET 2 OF	- 2			
$\rightarrow = \frac{\pi \pi}{(\text{TYP})}$	HUNDELSSO	DEPA	state RTMENT	OF NORTH CARO OF TRAN RALEIGH	NSPORTA	TION
DETAILS''	SEAL 037180 F WER	ST	supef RUCT	struc URAL	ture . STE	EL
	-DocuSigned by: Dichem F. Vuitro -08901A86EBF6470					
Road 1	1/7/2017		REVIS	IONS		SHEET NO. SO2-9
7607-3073 DOCUMENT 60 FINAL 5-0270 SIGNATUF	NOT CONSIDERED UNLESS ALL RES COMPLETED	1 2	DATE:	NO. BY: 3 4	DATE	TOTAL SHEETS 24

STR.NO.2
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ASSEMBLED BY : B.A. WHITE CHECKED BY : R.F. WERTMA	DATE : 10/04/17 N DATE : 10/14/17
DRAWN BY : EEM 2/97 CHECKED BY : VAP 2/97	REV. 10/1/11 MAA/GM REV. 6/13 AAC/MAA REV. 1/15 MAA/TMG





- 4″ THREAD (TYP.)

NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

THE PAYMENT FOR THE PIPE SLEEVES SHALL BE INCLUDED IN THE SEVERAL PAY ITEMS.

FOR PAINTED STRUCTURAL STEEL (EXCLUDING AASHTO M270 GRADE 50W), SOLE PLATES, ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

FOR AASHTO M270 GRADE 50W STRUCTURAL STEEL, SOLE PLATE SHALL BE AASHTO M270 GRADE 50W AND SHALL NOT BE GALVANIZED. ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

WHEN FIELD WELDING THE SOLE PLATE TO THE GIRDER FLANGE, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

—LOAD RATING—								
	MAX.D.L.+L.L.							
TYPE IV	310 K							

-	PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> county Station: <u>106+59.74</u> -L1- 14+51.19 -Y-
Bocusigned by:	DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD ELASTOMERIC BEARING DETAILS (STEEL SUPERSTRUCTURE)
08901A86EBF6470 11/7/2017	REVISIONS SHEET NO.
073 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: SU2-10 1 3 TOTAL SHEETS 2 4 24
	STR. NO. 2 STD. NO. EB2

		GIRDER 1 & GIRDER 6								GIRDER 2 & GIRDER 5					GIRDER 3 & GIRDER 4																		
TENTH POINTS	0	.1	.2	.3	.4	.5	.6	.7	.8	.9	0	0	.1	.2	.3	. 4	.5	.6	.7	.8	.9	0	0	.1	.2	.3	.4	. 5	.6	.7	•8	.9	0
DEFLECTION DUE TO WEIGHT OF GIRDER	0	0.023	0.044	0.059	0.069	0.072	0.069	0.059	0.044	0.023	0	0	0.023	0.044	0.059	0.069	0.072	0.069	0.059	0.044	0.023	0	0	0.023	0.044	0.059	0.069	0.072	0.069	0.059	0.044	0.023	0
DEFLECTION DUE TO WEIGHT OF SLAB *	0	0.062	0.124	0.171	0.201	0.211	0.201	0.171	0.124	0.062	0	0	0.055	0.111	0.154	0.181	0.190	0.181	0.154	0.111	0.055	0	0	0.048	0.098	0.136	0.160	0.168	0.160	0.136	0.098	0.048	0
DEFLECTION DUE TO WEIGHT OF BARRIER RAIL	0	0.011	0.020	0.028	0.032	0.033	0.032	0.028	0.020	0.011	0	0	0.011	0.020	0.028	0.032	0.033	0.032	0.028	0.020	0.011	0	0	0.011	0.020	0.028	0.032	0.033	0.032	0.028	0.020	0.011	0
TOTAL DEAD LOAD DEFLECTION	0	0.096	0.188	0.258	0.302	0.316	0.302	0.258	0.188	0.096	0	0	0.089	0.175	0.241	0.282	0.295	0.282	0.241	0.175	0.089	0	0	0.082	0.162	0.223	0.261	0.273	0.261	0.223	0.162	0.082	0
VERTICAL CURVE ORDINATE	0	0.025	0.045	0.058	0.067	0.070	0.067	0.058	0.045	0.025	0	0	0.025	0.045	0.058	0.067	0.070	0.067	0.058	0.045	0.025	0	0	0.025	0.045	0.058	0.067	0.070	0.067	0.058	0.045	0.025	0
REQUIRED CAMBER	0	17⁄16″	213/16″	3 ¹³ /16″	47⁄16″	45/8″	47⁄ ₁₆ ″	3 ¹³ /16″	213/16″	17/16″	0	0	13⁄8″	25⁄8″	3%6″	4 ¹³ / ₁₆ ″	4 ³ / ₈ ″	4 ¹³ / ₁₆ ″	3%6″	25⁄8″	13⁄8″	0	0	1 /4″	2 ¹ /2″	33⁄8″	3 ¹⁵ /16″	4 ¹ / ₈ ″	3 ¹⁵ /16″	33⁄8″	21/2"	1 /4″	0

* INCLUDES SLAB, BUILDUPS & STAY-IN-PLACE FORMS.

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ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT `` REQUIRED CAMBER '', WHICH IS GIVEN IN INCHES (FRACTION FORM). FABRICATORS SHALL DETAIL DIAPHRAGM MEMBERS AND CONNECTIONS FOR FULL DEAD LOAD FIT UP.GIRDERS SHALL BE PLUMB AFTER THE FULL AMOUNT OF DEAD LOAD IS APPLIED.

DRAWN BY : T.M. FORD	DATE :	10/09/17
CHECKED BY : R.F. WERTMAN	DATE :	10/14/17
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE :	11/06/17



SCHEMATIC OF CAMBER ORDINATES



		PROJEC CU STATIC	CT NO. MBER ON: <u>1</u>	<u>LAN</u> <u>26+</u> 4+51	116 ID 59. 1.19	<u>65.7</u> co 74 - } -Y	<u>A</u> UNTY - <u>L1-</u> -
	REFERENCE FOR SEAL	DEPA	SUPE	e of north OF TF raleig RSTRU	LOA	Sporta URE	LION
Road	DocuSigned by: Dechaul F. Vuitors 11/7/2017						SHEET NO.
7607-3073 60 0270	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. ВҮ: 1 2	DATE:	NO. ВҮ: 33 44		DATE:	S02-11 Total Sheets 24

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** SEE "END OF RAIL DETAILS" ON SHEET 2 OF 2 FOR ADDITIONAL REINFORCING STEEL. DIMENSIONS ARE SHOWN FROM 🕻 JOINT AT BACK FACE OF BARRIER RAIL

DRAWN BY : B.A. WHITE	DATE : 10/12/17
CHECKED BY : R.F. WERTMAN	DATE : 10/14/17
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE : <u>11/06/17</u>

96'-05%″ € JT END BENT 1 TO € JT END BENT 2

PLAN OF BARRIER RAIL



	PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> <u>COUNTY</u> STATION: <u>106+59.74</u> -L1- 14+51.19 -Y- SHEET 1 OF 2									
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH										
SEAL 037180 F WERMIN	VERTICAL CONCRETE BARRIER RAIL									
Occusigned by: Dichaul For Vietors										
Road 11/7/2017	REVISIONSSHEET NO.NO.BY:DATE:NO.SO2-12									
F-0270 FINAL UNLESS ALL SIGNATURES COMPLETED	1 3 TOTAL SHEETS 2 4 24									



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THE BARRIER RAIL IN EACH SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE

WHEN FOAM JOINT SEAL IS REQUIRED, THE JOINT IN THE DECK SHALL BE SAWED PRIOR TO THE CASTING OF VERTICAL CONCRETE BARRIER RAIL.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

THE #5 S3 & S4 BARS SHALL BE INSTALLED, USING AN ADHESIVE ANCHORING SYSTEM, AFTER SAWING THE JOINT. THE YIELD LOAD FOR THE #5 S3 & S4 BARS IS 18.6 KIPS. FIELD TESTING FOR THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS



	PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> COUNTY STATION: <u>106+59.74</u> -L1- 14+51.19 -Y- SHEET 2 OF 2
POFESSION SEAL	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD
O37180 O37180 F WERNING	VERTICAL CONCRETE BARRIER RAIL
ad 21/7/2017	REVISIONS SHEET NO.
77-3073 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	1 3 TOTAL SHEETS 2 4 24

STR.NO.2

STD.NO.CBR2



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BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/811 Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL

THE 1 $\frac{1}{4}$ // Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $1/4^{\prime\prime}$ HOLD DOWN PLATE AND 7 - $1/8^{\prime\prime}$ Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE

	PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> COUNTY STATION: <u>106+59.74</u> -L1- 14+51.19 -Y-
REPART OF THE PROPERTY OF THE	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD GUARDRAIL ANCHORAGE FOR BARRIER RAIL
Road DocuSigned by: Dicheast For United 08901A86EBF6470 11/7/2017	REVISIONS SHEET NO.
7607-3073 50 DOCUMENT NOT CONSIDERED 50 FINAL UNLESS ALL 50270 SIGNATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: SO2-14 1 3 TOTAL SHEETS TOTAL SHEETS 24
	STR. NO. 2 STD. NO. GRA3



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		CTTE	RE		ING B	AR SCH						-	BAR	IYPES —		
* A1	113	*5	STR	39'-11"	4705	A201	4	#5	STR	38'-7"	161	-	4'-3"			
A2	113	#5	STR	39'-11"	4705	A202	4	#5 	STR	37'-7"	157	┨				
* A3	6	#6	STR	15'-0"	136	A203	4	#5 #5	STR STR	36'-7"	153		1		▲	
★ A101	4	#5	STR	38′-7″	161	A204	4	#5	STR	34'-7"	140	- тнтя і	FG	(1) š		
₩ A102	4	#5	STR	37′-7″	157	A206	4	#5	STR	33′-7″	140	OVER GI	IRDER	\bigcirc		
₩ A103	4	#5 #5	STR	36'-7"	153	A207	4	#5 #5	STR	32'-7"	136	-			_	
* A104 * A105	4	#5	STR	34'-7"	140	A208 A209	4	#5	STR	30'-7"	132	-	-	1'-6"		
∗ A106	4	#5	STR	33'-7"	140	A210	4	#5	STR	29'-7"	123					
₩ A107	4	#5	STR	32'-7"	136	A211	4	#5 #5	STR	28'-7"	119	_				
₩ A108 ₩ A109	4	#5 #5	SIR STR	31'- ("	132	A212	4	#5 #5	SIR	26'-7"	115	-		2'-2"		
* A100	4	#5	STR	29'-7"	123	A213	4	#5	STR	25'-7"	107	1	-			
₩ A111	4	#5	STR	28'-7"	119	A215	4	#5	STR	24'-7"	103					
+ A112 ★ A113	4	#5 #5	STR	27'-7"	115	A216	4	#5 #5	STR	23'-7"	98	- [*] ₀		(2)		
* A113 * A114	4	#5	STR STR	26 - 7	107	A217 A218	4	#5	STR	22 - 1	94					
★ A115	4	#5	STR	24'-7"	103	A219	4	#5	STR	20'-7"	86					
* A116	4	#5 #5	STR	23'-7"	98	A220	4	#5 #5	STR	19'-7"	82	_	1'-6"	-	1'-6"	
* Α11 / * Δ118	4	#5 #5	SIR STR	22'-7"	94	A221	4	#5 #5	SIR	18'-1"	(8	-				
* A110 * A119	4	#5	STR	20'-7"	86	A223	4	#5	STR	16'-7"	69					
₩ A120	4	#5	STR	19′-7″	82	A224	4	#5	STR	15′-7″	65		6″	6″	1	
* A121	4	#5 #5	STR	18'-7"	78	A225	4	#5 #5	STR	14'-7"	61	_				
* A122 * A123	4	#5	STR	16'-7"	69	A226 A227	4	#5	STR	12'-7"	53	-				
* A124	4	#5	STR	15'-7"	65	A228	4	#5	STR	11'-7"	48			\frown		
₩ A125	4	#5	STR	14'-7"	61	A229	4	#5 #5	STR	10'-7"	44	_		(3)	ق	
* Δ126 * Δ127	4	#5 #5	SIR STR	13'-1"	51	A230	4	#5 #5	SIR	9'-/" 8'-7"	40	-		_		
* A121	4	#5	STR	11'-7"	48	A231	4	#5	STR	7'-7"	32					
₩ A129	4	#5	STR	10'-7"	44	A233	4	#5	STR	6'-7"	27			2'-1"		
¥ A130	4	#5 #5	STR	9'-7"	40	A234	4	#5 #5	STR	5'-7"	23	_				
* A131 * A132	4	#5	STR	7'-7"	32	A235 A236	4	#5	STR	3'-7"	15		BAR DIMENST	ONS ARE OUT T	ΤΠΟ ΟΠΤ	
∗ A133	4	#5	STR	6'-7"	27	A237	4	#5	STR	2'-7"	11					
* A134	4	#5	STR	5'-7"	23	A238	4	#5	STR	1'-7"	7	_				
* A135 * A136	4	#5 #5	STR STR	4'- <i>1"</i> 3'-7"	19	* B1 '	08	#4	STR	25'-5"	1834	-				
₩ A137	4	#5	STR	2'-7"	10	B2 1	02	#5	STR	49'-0"	5213	-				
₩ A138	4	#5	STR	1'-7"	7											
						* G1	2	#5	SIR	56'-5"	118	-				
					F	* K1	12	#5	1	6'-6"	81					
						米 K2	24	#5	2	6'-8"	167					
GRO	OVIN	G BH	KTD(-	E FLOC		<u>₩K3</u>	30	#5	STR	7'-11"	248	-				
BRID	GE DECK	K		3360 SQ.F	-T.	* S1	60	#4	3	4'-7"	184	-				
APPR	OACH SL	ABS		1579 SQ.F	- T.											
ΤΟΤΑ	L			4939 SQ.F	=⊤.	REINFOR	CING S	TEEL		=	13,103					
						* EPOXY	COATED	RE:	INF.S	TEEL =	10,658					
									_,]					NO /1	665 7	Λ
				CIURE NDE	RETIN	ON T	NG 5 ЦЕ	IEt					PRUJECI	NO. <u>¬</u>	000.1	<u>A</u>
	FOLI		TNG	MININ	MIM S			GTI	нςΙ				<u> </u>	<u>Berland</u>	C0	UNTY
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		EXCE	PT A	PROACH			PAR	APEI	г				STATION:	$-100 \cdot 5$	$\frac{1}{10}$ \vee	
	SIZE	AND	BS, PA BARRI	ER RAIL		ST SEADS	BAR	ND RIEF	R					14 ')1	LJI	
		EPC	XY		EPOXY			AIL								
		COA	ΓED		COATED									STATE OF NORTH CAR	OLINA VCPORTAT	
	#4	2'-0	"	1′-9″	2'-0"	1'-9"	_ 2	2'-9"				WINNING TH CARO		RALEIGH		
	#5	2′-6	"	2'-2"	2'-6"	2'-2"	3	6′-5″				ROFESSION THE				
	#6	3'-0	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	2'-7″	3′-10″	2'-7"	4	/-4″				SEAL '	5	ULEKZIKU(JUKE	
	L		I			1	<u> </u>					A GINER AND			TEDT	ΛI
												THIN F WEIMIN				ΗL
												DocuSigned by:				
									2610 11	liff Dood		08901A86EBF6470 11/7/2017	i	REVISIONS		SHEET NO
				PLA PLA	ANS PREPARED BY	/: 			2010 Wycl Suite 102	иј коаа С 27607-2072 Гр			NO. BY: DA	ATE: NO. BY:	DATE:	S02-15
						nce Delivered		ng	(919) 420-	7660 0. F-0270		INGT CONSTDERED	1	3		TOTAL SHEETS
					LAUCHE	ILL DELIVEIEU		JUC U			JUNALL	JACJ VUNILLIEU		1(45)		. /4

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS												
BAR SIZE	SUPERST EXCEPT A SLABS, P AND BARR	RUCTURE APPROACH ARAPET, IER RAIL	APPROAC	PARAPET AND BARRIER								
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL							
#4	2'-0″	1'-9″	2'-0″	1'-9″	2′-9″							
#5	2'-6″	2'-2"	2'-6″	2'-2"	3′-5″							
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"							



STD. NO. BOM1 STR.NO.2



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STR.NO.2



DocuSign Envelope ID: 3A4D0F7A-B2E8-4708-A87D-4003DA39697E

		PROJEC	CT NO. MBER DN: <u>1</u>	<u>410</u> LAND 26+59 4+51.1	<u>665.7</u> co).74 - .9 -Y	<u>A</u> UNTY - <u>L1-</u> -
	RCH CAROL RCH CAROL	DEPA	RTMENT SUB	e of north card OF TRAN RALEIGH STRUCT BEN	ISPORTA URE T #1	TION
Road 7607-3073	DOCUMENT NOT CONSIDERFD	NO. BY:	REVIS DATE:	SIONS	DATE:	SHEET NO. SO2-17
60 -0270	FINAL UNLESS ALL SIGNATURES COMPLETED	1 2 STR NO 2		3 4		total sheets 24



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STR.NO.2



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DRAWN BY . T.M. FORD	DATE • 10/16/17
CHECKED BY : R.F. WERTMAN	DATE: $\frac{10/18/17}{10/18/17}$
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE : <u>11/06/17</u>



<u>PLAN</u>

SEE "SECTION A-A" ON SHEET 3 OF 3 FOR 24" Ø C.S. PIPE DETAILS





STR.NO.2



drawn by : <u>T.M. FORD</u>	DATE : <u>10/17/17</u>	
снескед ву : <u>R</u> .F. WERTMAN	DATE : <u>10/18/17</u>	
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE : 11/06/17	

		PROJEC [.] CUM	T NO. Mbfr	41 AND	<u>665.7</u>	
		STATIO	N: <u>1</u> (<u>)6+59</u> 4+51_1	<u></u> 20).74 - 9 -Y	<u>-L1-</u>
		SHEET 2 OF	= 3			
	THUM CAROL	DEPAR	stati RTMENT	E OF NORTH CARG OF TRAN RALEIGH	NSPORTA	TION
	SEAL		SUB	STRUCT	URE	
	037180 PO F WERNING		END	BENT	- #2	
	DocuSigned by: Auchan F. Wurtons					
Road	08901A86EBF6470 11/7/2017		REVIS	SIONS		SHEET NO.
7607-3073 60 0270	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. вү: 1 2	DATE:	NO. ВҮ: 3 4	DATE:	SU2-20 TOTAL SHEETS 24
		STR. NO. 2				



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TEMPORARY DRAINAGE AT END BENT



YPES -				В	ILL O	F MATE	ERIAL	
71/	_ //				END	BENT	#2	
_(7)		F	BAR	No.	SIZE	TYPE	LENGTH	WEIGHT
<i>\</i> 0'	$\mathbf{\lambda}$		B1	10	#9	1	38′-6″	1309
, k		Ļ	B2	8	#10	1	38'-2"	1314
		F	В Л	12	<u> </u>	SIR ctd	34'-4"	430
	<u>`Z</u>	—_;	<u>в</u> 5	12	#4	STR	23'-7"	189
_	H1 13'-9"	▶	B6	20	#4	STR	2'-2"	29
_	H2 13'-7"	▶ [Β7	10	#4	STR	7'-6"	51
				ļ				
			H1	17	#5	2	14'-7"	259
		F	ΠΖ			Z	14 -5	200
		F	K1	24	#4	STR	23'-7"	378
	1'-3" L		К2	6	#4	STR	5'-0"	20
			K3	6	#4	STR	4'-0"	16
		F	<u>۲</u>	78	#5		11/_10″	963
		F		78	#5	4	4'-7"	373
	$\left(\begin{array}{c} \end{array}\right)$	F	S3	32	#4	6	6'-6"	139
		L	U1	56	#4	7	3'-8"	138
		F	U2	11	#4	(6'-8"	/6
	<u> </u>	F	V1	112	 #5	STR	6'-7"	769
		F	V2	12	#5	STR	8'-2"	103
al	3'-8"	U2	V3	36	#5	STR	8'-3"	310
	8″							
1 &			RET	.NFORC.	ING STEE			/162 LBS.
		│ ┐ ┠			CLASS	A CONCE	FTF	
		F	OUR ·	#1 (CAP	& LOWE	R WINGS)		41.9 C.Y.
1,-6		F	°OUR [:]	#2 (UPF	PER WING	S & BACH	(WALL)	10.0 C.Y.
							AL	51.9 C.Y.
-	I	' F	No	- 8	HP IZ X	53 SIEEL	PILES 576	SITN FT
		-		- 0			510	
DIMEN	ISIONS ARE OUT ⁻	TO OUT.	PILE	DRIVI	NG EQUIF	MENT SET	UP	8 5104
			FOR H	P 12 X	53 STE	EL PILES		0 EACH
_		_			/FS			8 5104
		L		NEDIT				0 LACH
ET	AILS		PRC STA	DJEC CUN TIO	Γ ΝΟ. <u>/ΒΕR</u> Ν: <u>1</u> (14	<u>41</u> LAND)6+59 1+51.1	<u>665.7</u> co <u>}.74</u> - 9 -Y	<u>A</u> UNTY - <u>L1-</u> -
ACK R NT)	RESTRAINT					OF NORTH CAR		TTON
		CAROL MIL		UEMAK	IMENI	CF IKAN RALEIGH	NSPURIA	
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		TI80			SURS	SIRUCI	UKE	
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		WERNIN	1		END	RFN	#2	
	DocuSigned b	/////// py:						
	Richard	E Vuitro	1					
oad	08901A86EBF 11/14/201	7			REVIS	IONS		SHEET NO.
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)TES:	FC	DR C	<u> 0</u>)NE / (2 R		OACH S	SLAB
E CONSTRUCTED PRIOR TO DECK.	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
"MSE RETAINING WALL" PLANS.	* A1 A2	50 52	#4 #4	STR STR	29'-3" 29'-1"	977 1010
SPECIAL PROVISIONS.	 	79	#5	СТР	21/-2"	1002
SEAL WIDTH OF THE FOAM JOINT	B2	79	#6	STR	24'-8"	2928
SEE SPECIAL PROVISIONS.	REINF * EPO REI	ORCII XY CO	NG STE DATED CING S	TEEL	LBS.	3938 2969
				TF	CΥ	42.6
		SF BA SI	PLICE I R EPO ZE COA	LENGTH XY TED U	S CHART NCOATED	
UNCRETE ERIC E * F.)		#Ę #Ę	+ 2'- 5 2'- 5 3'-	-6″ -10″	<u>1-9</u> 2'-2" 2'-7"	
JM BLOCKOUT SHOWN.						
TEMP. SLOPE DRAIN 2'-0'MIN BLOCK CH CH CH CH CH CH CH CH CH CH	ERNANT HE APPROBERM AN ILET ANT MAT HE APPROBERM AN ILET AS SHOW HER 1) AS PERFORAT AMETER.	S ``B" ON CO RE DER DER N. TH CON ED	SLAB, PE SLAB, NTROL			
PF ST SH	ROJEC CUN FATIO	ΓΝ ΔΒΕ Ν:	0. ERL# 106 14+	41 AND 5+59 51.1	<u>665.7</u> co <u>}.74</u> - _9 -Y	<u>A</u> UNTY - <u>L1-</u> -
Road	depar	RTMEN RID SLA	STATE OF NT OF STAN GE AB	NORTH CAR TRAN ALEIGH NDAR APF DET	DIINA NSPORTA D PROAC AILS	TION
27607-3073 DOCUMENT NOT CONSIDERED 100. 660 FINAL UNLESS ALL 1	BY:	DATE:	NO.	BY:	DATE:	SO2-23 TOTAL SHEFTS
F-0270 SIGNATURES COMPLETED						24
511	VEINOR C					

DRAWN BY : B.A. WHITE	DATE : 10/12/17
CHECKED BY : R.F. WERTMAN	DATE : <u>10/16/17</u>
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE : <u>11/06/17</u>

. B.A. WHITE	_ DATE : <u>10/12/17</u> _
BY : R.F. WERTMAN	DATE: 10/16/17
NGINEER OF RECORD : <u>R.F. WERTMAN</u>	_ DATE : <u>11/06/17</u>





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PLAN





NOTES

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.

SLOPE PROTECTION SHALL CONSIST OF 4"POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET.CONCRETE SHALL BE CLASS "B".THE CONCRETE SURFACE SHALL BE FINISHED TO THE SATISFACTION OF THE ENGINEER. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 20" WIDE. THE COST OF THE WELDED WIRE FABRIC SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.106+59.74 -L1- STA.14+51.19 -Y-	4 INCH SLOPE PROTECTION	* Welded Wire Fabric 60 inches Wide
	SQUARE YARDS	APPROX. L.F.
END BENT 1	24	44
END BENT 2	49	78

* QUANTITY SHOWN IS BASED ON 5' POURS.







STR.NO.3



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MIRTH

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FOR UTILITY INFORMATION, SEE UTILITY

PLANS AND SPECIAL PROVISIONS

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						$ \top$ 0^{-}	TAL BI	ILL OF	MATER	$\Box \triangle$				-			
	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	STRUCTURAL STEEL APPROX. 122,200 LBS.	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	HP Stee	12 X 53 El PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	4″SLOPE PROTECTION	ELASTOMERIC BEARINGS	FOAM JOINT SEALS	Γ
	LUMP SUM	EACH	SQ.FT.	SQ.FT.	CU. YDS.	LUMP SUM	LBS.	LUMP SUM	EACH	NO.	LIN.FT.	EACH	LIN.FT.	SQ.YDS.	LUMP SUM	LUMP SUM	
SUPERSTRUCTURE			3866	4939		LUMP SUM		LUMP SUM					192.1		LUMP SUM	LUMP SUM	Γ
END BENT NO.1					49.9		6872		8	8	576	8		19			Γ
END BENT NO.2					52.5		7391		8	8	576	8		34			
TOTAL	LUMP SUM	1	3866	4939	102.4	LUMP SUM	14263	LUMP SUM	16	16	1152	16	192.1	53	LUMP SUM	LUMP SUM	Γ

DRAWN BY : J.A. BOYER	DATE: 09/03/17
CHECKED BY : R.F. WERTMAN	DATE: 10/16/17
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	_ DATE : <u>11/06/17</u>

ASSUMED
THIS BRI LRFD BRI
THIS BRI
FOR OTHE
FOR SUBM
FOR FALS
FOR CRAN
FOR GROU
THE ELEV OF MINIM AVAILABL ELEVATIO REPORT A NECESSAR WILL BE
FOR MAIN Structur
REMOVABL FORMS IN SPECIFIC
NEEDLE BI FOR ON T
ALL STRU PAINTED STANDARD
INASMUCH STEEL CO TO ARTIC RESULTIN REGULATI

THE EXISTING STRUCTURE CONSISTING OF TWO SPANS @ 60'-O" AND ONE SPAN @ 65'-0", WITH A CLEAR ROADWAY OF 28' AND REINFORCED CONCRETE DECK ON STEEL I-BEAMS ON END BENTS WITH REINFORCED CONCRETE CAPS ON PRECAST PRESTRESSED CONCRETE PILES & INTERIOR BENTS WITH REINFORCED CONCRETE CAPS ON PRECAST PRESTRESSED CONCRETE PILES LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

LIVE LOAD = HL-93 OR ALTERNATE LOADING.

IDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO IDGE DESIGN SPECIFICATIONS.

IDGE IS LOCATED IN SEISMIC ZONE 1.

ER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

MITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

SEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

NE SAFETY, SEE SPECIAL PROVISIONS.

JT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

ATION AND CLEARANCE SHOWN ON THE PLANS AT THE POINT MUM VERTICAL CLEARANCE ARE FROM THE BEST INFORMATION _E.PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE ON ON THE EXISTING PAVEMENT AND CHECK THE CLEARANCE. ANY VARIATIONS TO THE ENGINEER, ANY PLAN REVISIONS Y TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE PROVIDED BY THE DEPARTMENT.

NTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED RE, SEE SPECIAL PROVISIONS.

LE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD CATIONS.

BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED THE PLANS OR APPROVED BY THE ENGINEER.

ICTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND IN ACCORDANCE WITH SYSTEM 4 OF ARTICLE 442-8 OF THE SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL ONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED CLE 107-1 OF THE STANDARD SPECIFICATIONS.ANY COSTS NG FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL IONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 107+16.84 -L2-".

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.

ASBESTOS ASSESSMENT		PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> COUNTY STATION: <u>107+16.84</u> -L2- 13+69.76 -Y-
		SHEET 3 OF 3
LUMP SUM		
LUMP SUM		STATE OF NORTH CAROLINA
LUMP SUM	BR CFESSION SEAL 037180 FOR SEAL 037180 FWER HILL DocuSigned by: Chichaul & Ductors	GENERAL DRAWING BRIDGE ON I-95 BUSINESS LOOP SBL OVER US301 BETWEEN DOBBIN HOLMES ROAD AND I-95
Road	11/14/2017	REVISIONS SHEET NO.
7607-3073 DOCUM	ENT NOT CONSIDERED	NO. BY: DATE: NO. BY: DATE: SO3-3
50 FI 5-0270 SIGN	NAL UNLESS ALL IATURES COMPLETED	1 3 IOTAL SHEETS 2 4 24

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			LOAD	ANE) res	SISTA	NCE	FAC	TOR	RAT	ING	(LRF	R) SI	JMMA	RY F	FOR	STEE	IL GI		7 5				
								STRENGTH I LIMIT STATE					S	SERVICE II LIMIT STATE										
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING #	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVE-LOAD Factors (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	COMMENT NUMBER
		HL-93 (INVENTORY)	NZA	$\langle 1 \rangle$	1.66		1.75	0.649	1.91	А	EL	45.97	0.926	2.24	А	I	0.00	1.30	0.649	1.66	А	EL	20.00	
	GN	HL-93 (OPERATING)	NZA		2.16		1.35	0.649	2.48	А	EL	45.97	0.926	2.90	А	I	0.00	1.00	0.649	2.16	А	EL	20.00	
RAT]	NG	HS-20 (INVENTORY)	36.00	$\langle 2 \rangle$	2.26	81.36	1.75	0.649	2.62	А	EL	45.97	0.926	3.00	А	I	0.00	1.30	0.649	2.26	А	EL	20.00	
		HS-20 (OPERATING)	36.00		2.93	105.48	1.35	0.649	3.40	А	EL	45.97	0.926	3.88	А	I	0.00	1.00	0.649	2.93	А	EL	20.00	
		SH	12.500		5.89	73.63	1.40	0.649	8.45	А	EL	45.97	0.926	9.99	А	I	0.00	1.30	0.649	5.89	А	EL	20.00	
		S3C	21.500		3.45	74.18	1.40	0.649	4.88	А	EL	45.97	0.926	5.85	А	I	0.00	1.30	0.649	3.45	А	EL	20.00	
	ICL	S3A	22.750		3.27	74.39	1.40	0.649	4.63	А	EL	45.97	0.926	5.55	А	I	0.00	1.30	0.649	3.27	А	EL	20.00	
	<pre>H < C < C </pre>	S4A	26.750		2.85	76.24	1.40	0.649	4.08	А	EL	45.97	0.926	4.82	А	I	0.00	1.30	0.649	2.85	А	EL	20.00	
	CLE (S	S5A	30.500		2.54	77.47	1.40	0.649	3.60	А	EL	45.97	0.926	4.35	А	I	0.00	1.30	0.649	2.54	А	EL	20.00	
		S6A	34.500		2.29	79.01	1.40	0.649	3.25	А	EL	45.97	0.926	3.93	А	I	0.00	1.30	0.649	2.29	А	EL	20.00	
LOAD		S7B	38.500		2.10	80.85	1.40	0.649	2.96	А	EL	45.97	0.926	3.65	А	I	0.00	1.30	0.649	2.10	А	EL	20.00	
		S7A	40.000	3	2.09	83.60	1.40	0.649	2.91	А	EL	45.97	0.926	3.70	А	I	0.00	1.30	0.649	2.09	А	EL	45.97	
	L L L	Т4А	28.250		2.76	77.97	1.40	0.649	3.99	А	EL	45.97	0.926	4.66	А	I	0.00	1.30	0.649	2.76	А	EL	20.00	
	ACTC ALEI ALEI	T5B	32.000		2.50	80.00	1.40	0.649	3.51	А	EL	45.97	0.926	4.37	А	I	0.00	1.30	0.649	2.50	А	EL	20.00	
	TR/-TR/-TR/-T	ТбА	36.000		2.30	82.80	1.40	0.649	3.21	А	EL	45.97	0.926	3.99	А	I	0.00	1.30	0.649	2.30	А	EL	45.97	
	SEMT.	Т7А	40.000		2.13	85.20	1.40	0.649	2.97	А	EL	45.97	0.926	3.69	А	I	0.00	1.30	0.649	2.13	А	EL	45.97	
		Т7В	40.000		2.11	84.40	1.40	0.649	3.15	А	EL	45.97	0.926	3.51	А	I	0.00	1.30	0.649	2.11	А	EL	20.00	1
FATIGUE	-	HL-93 (INVENTORY)	$\gamma_{LL}=0.75$																					

END BENT 1

ASSEMBLED BY : B.A. WHITE	DATE : 10/13/17
CHECKED BY : R.F. WERTMAN	DATE : 10/16/17
DRAWN BY : MAA 1/08	REV.II/I2/08RR MAA/GM
Checked by : GM/DI 2/08	REV.IO/I/II MAA/GM

<u>LRFR SUMMARY</u>

LOAD FACTORS:

	I TMTT STATE	$\gamma_{\rm DC}$	γ_{DW}
DESIGN		• 00	• 🗆 🗤
LOAD Rating	STRENGTH I	1.25	1.50
FACTORS	SERVICE II	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE II LIMIT STATES. ALLOWABLE STRESS FOR SERVICE II LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING
$\left<1\right>$ design load rating (HL-93) **
$\langle 2 \rangle$ design load rating (HS-20) $*$ $*$
<pre>3 LEGAL LOAD RATING **</pre>
* * SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I – INTERIOR GIRDER
EL – EXTERIOR LEFT GIRDER
ER – EXTERIOR RIGHT GIRDER

		PROJEC <u>CU</u> Static	CT NO. I <u>MBER</u> ON: <u>1</u>	<u>41</u> <u>ALAND</u> 07+16 3+69.	<u>665.7</u> C0 <u>.84 -</u> 76 -`	A UNTY L2- (-
	RTH CAROLAR SEAL 037180 F WERMIN	DEPA	RTMENT S LRFR STEE (INTER	TANDAF SUMMAF STATE TI	NSPORTA NSPORTA RY FOF DERS RAFFIC)	TION {
	08901A86EBF6470 11/7/2017		REVIS	SIONS	DATE	SHEET NO.
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STR.NO.3

STD. NO. LRFR4

BEFORE ADDITIONAL CONCRETE IS CAST IN THE SPAN.

drawn by : <u>B</u> .A. WHITE	DATE : 10/13/17
CHECKED BY : <u>R.F. WERTMAN</u>	DATE: <u>10/16/17</u>
design engineer of record : <u>R.F. WERTMAN</u>	DATE: <u>11/06/17</u>

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10 ¹ /	[′] 2″TOP OF SLAB TO BOTTOM OF TOP FLANGE @ € BRG.
2 ¹ / ₂ " BUILD-UP AT & GRD. STAY-IN- METAL FORMS 4" HIGH B.B.	8" TOP OF SLAB TO TOP OF S.I.P. FORMS @ & BRG. C GIRDER
	DETAIL ``A''
RS 1 ^I / ₂ ″HIGH B.B.U.	
<pre>1¼"HIGH B.B.U. SEE NOTES STAY-IN-PLACE METAL FORMS * #5 G1 BAR MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO CLEAR REINFORCING STEEL & STEPPUPS</pre>	PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> county station: <u>107+16.84</u> -L2- 13+69.76 -Y-
DocuSigned by:	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE TYPICAL SECTION
nd 11/7/2017 17-3073 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	REVISIONSSHEET NO.NO.BY:DATE:NO.SO3-513TOTAL SHEETSTOTAL SHEETS2424STR. NO. 3

drawn by : B.A. WHITE	DATE: 10/12/17
CHECKED BY : R.F. WERTMAN	DATE: 10/16/17
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	date: <u>11/06/17</u>

NOTES:

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH SYSTEM 4 OF ARTICLE 442-8 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL, UNLESS OTHERWISE NOTED.

ALL FIELD CONNECTIONS TO BE $\frac{7}{8}$ "DIA. HIGH STRENGTH BOLTS UNLESS OTHERWISE NOTED.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB.

A CHARPY V-NOTCH TEST IS REQUIRED FOR WEB PLATES AND BOTTOM FLANGE PLATES FOR ALL GIRDERS AND IN ACCORDANCE WITH ARTICLE 1072-7 OF THE STANDARD SPECIFICATIONS.

PERMITTED FLANGE AND WEB SHOP SLICES SHALL NOT BE LOCATED WITHIN 15 FEET OF MAXIMUM DEAD LOAD DEFLECTION (NOR WITHIN 15 FEET OF INTERMEDIATE BEARINGS OF CONTINUOUS UNITS). KEEP 2 FEET MINIMUM BETWEEN WEB AND FLANGE SHOP SLICES. KEEP 6" MINIMUM BETWEEN CONNECTOR PLATE OR TRANSVERSE STIFFENER WELDS AND WEB OR FLANGE SHOP SPLICES.

STUDS ON GIRDERS MAY BE SHIFTED UP TO 1" IF NECESSARY TO CLEAR FLANGE SPLICE WELD.

TENSION ON THE ASTM A325 BOLTS SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH ARTICLE 440-8 OF THE STANDARD SPECIFICATIONS.

CONNECTOR PLATE DETAILS

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₂ '' X 1'	ŕ P		\Box	MBER		AND	CO	UNTY
► <u>**</u> (TYP. 2)	S	TATI	ON: <u>1(</u>	<u>)</u> 3 +	⁷ +16 -69.	<u>84</u> -	<u> </u>
		SH	<u>-EI 2 C</u>)F 2				
4 ★ ★ (TYP.	RTH CAROL		DEPA	stat RTMENT	e of OF	NORTH CARC TRAN Raleigh	ISPORTA	TION
AILS''	SEAL 037180 F WER		S	supef TRUC ⁻	rs Fl	truc JRAL	ture . STE	EL
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STR.NO.3

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ASSEMBLED BY : B.A.WHITE Checked by : R.F.Wertma	DATE : N DATE :	10/04/17 10/16/17
DRAWN BY : EEM 2/97 Checked by : Vap 2/97	REV.10/1/11 REV.6/13 REV.1/15	MAA/GM AAC/MAA MAA/TMG

– 4″ THREAD

NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

THE PAYMENT FOR THE PIPE SLEEVES SHALL BE INCLUDED IN THE SEVERAL PAY ITEMS.

FOR PAINTED STRUCTURAL STEEL (EXCLUDING AASHTO M270 GRADE 50W), SOLE PLATES, ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

FOR AASHTO M270 GRADE 50W STRUCTURAL STEEL, SOLE PLATE SHALL BE AASHTO M270 GRADE 50W AND SHALL NOT BE GALVANIZED. ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

WHEN FIELD WELDING THE SOLE PLATE TO THE GIRDER FLANGE, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

— LOAD R	ATING-
	MAX. D.L.+L.L.
TYPE IV	310 k

		PROJEC <u>CU</u> Static	CT NO. <u>MBER</u> DN: <u>1</u>	<u>41</u> LAND 07+16 3+69.	<u>665.7</u> C0 <u>.84 -</u> 76 -`	<u>A</u> UNTY L2- (-
	Docusigned by:	depa ELA (S	stat RTMENT STOM D STEEL	e of north card OF TRAN raleigh TANDAR ERIC ETAIL SUPERS	olina NSPORTA D BEAR _S ==== TRUCTU	TION ING RE)
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DEAD LOAD DEFLECTION TABLE FOR GIRDERS																																	
		GIRDER 1 & GIRDER 6					GIRDER 2 & GIRDER 5				GIRDER 3 & GIRDER 4																						
TENTH POINTS	0	.1	.2	.3	.4	.5	.6	7	.8	.9	0	0	.1	.2	.3	<u> </u>	.5	.6	.7	.8	.9	0	0	.1	.2	.3	. 4	.5	.6	.7	.8	.9	0
DEFLECTION DUE TO WEIGHT OF GIRDER	0	0.023	0.044	0.059	0.069	0.072	0.069	0.059	0.044	0.023	0	0	0.023	0.044	0.059	0.069	0.072	2 0.06	9 0.05	9 0.04	4 0.023	0	0	0.023	0.044	0.059	9 0.069	0.072	0.069	0.059	0.044	0.023	0
DEFLECTION DUE TO WEIGHT OF SLAB *	0	0.062	0.124	0.171	0.201	0.211	0.201	0.171	0.124	0.062	0	0	0.055	0.111	0.154	0.181	0.190	0.18	1 0.15	1 0.111	0.055	0	0	0.048	0.098	0.136	5 0 . 16C	0.168	0.160	0.136	0.098	0.048	0
DEFLECTION DUE TO WEIGHT OF BARRIER RAIL	0	0.011	0.020	0.028	0.032	0.033	0.032	0.028	0.020	0.011	0	0	0.011	0.020	0.028	0.032	0.033	3 0.03	2 0.02	3 0.02	0.011	0	0	0.011	0.020	0.028	8 0.032	0.033	0.032	0.028	0.020	0.011	0
TOTAL DEAD LOAD DEFLECTION	0	0.096	0.188	0.258	0.302	0.316	0.302	0.258	0.188	0.096	0	0	0.089	0.175	0.241	0.282	0.295	5 0.28	2 0.24	0.175	5 0.089	0	0	0.082	0.162	0.223	3 0.261	0.273	0.261	0.223	0.162	0.082	0
VERTICAL CURVE ORDINATE	0	0.025	0.045	0.058	0.067	0.070	0.067	0.058	0.045	0.025	0	0	0.025	0.045	0.058	0.067	0.070	0.06	7 0.05	3 0.04	5 0.025	0	0	0.025	0.045	0.058	3 0.067	0.070	0.067	0.058	0.045	0.025	0
											_																						
REQUIRED CAMBER	0	17/16″	2 ¹³ /16″	3 ¹³ /16″	47/16″	45/8″	47/16″	3 ¹³ /16″	2 ¹³ /16″	17/ ₁₆ ″	0	0	13/8″	25/8″	3%6″	4 ¹³ /16″	4 3/8"	413/16	" 3 ⁹ /16	25/81	13/8″	0	0	11/4″	21/2"	33/8″	3 ¹⁵ /16"	41/8"	315/16″	33/8″	21/2"	11/4″	0

* INCLUDES SLAB, BUILDUPS & STAY-IN-PLACE FORMS.

ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT `` REQUIRED CAMBER '', WHICH IS GIVEN IN INCHES (FRACTION FORM). FABRICATORS SHALL DETAIL DIAPHRAGM MEMBERS AND CONNECTIONS FOR FULL DEAD LOAD FIT UP.GIRDERS SHALL BE PLUMB AFTER THE FULL AMOUNT OF DEAD LOAD IS APPLIED.

DRAWN BY : T.M. FORD	DATE : <u>10/09/17</u>
CHECKED BY : R.F. WERTMAN	date: <u>10/17/17</u>
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE : <u>11/06/17</u>

SCHEMATIC OF CAMBER ORDINATES

		PROJE(<u>CL</u> STATI	CT NO. I <u>MBER</u> ON: <u>1(</u> 1]		<u>1665.7</u>) co <u>6.84 -</u> .76 -	<u>Z</u> 2007 2007 2007 2007 2007			
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SEAL 037180 DEAD LOAD								
Road	DocuSigned by:	REVISIONS SHEET N							
7607-3073 60 F-0270	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. ВҮ: 1 2	DATE:	NO. ВҮ: З	DATE:	TOTAL SHEETS 24			

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* * SEE "END OF RAIL DETAILS" ON SHEET 2 OF 2 FOR ADDITIONAL REINFORCING STEEL. DIMENSIONS ARE SHOWN FROM 🕻 JOINT AT BACK FACE OF BARRIER RAIL

DRAWN BY : B.A. WHITE	date : <u>10/12/17</u>
CHECKED BY : R.F. WERTMAN	date : <u>10/17/17</u>
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	date: <u>11/06/17</u>

96′-05⁄8″ € JT END BENT 1 TO € JT END BENT 2

PLAN OF BARRIER RAIL

	PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> COUNTY STATION: <u>107+16.84</u> -L2- 13+69.76 -Y- SHEET 1 OF 2							
NININA CAROL	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH							
SEAL 037180 FWGINEER	VERTICAL CONCRETE BARRIER RAIL							
Road DocuSigned by: Dichent F. Vutno 08901A86EBF6470 11/7/2017	REVISIONS SHEET NO.							
27607-3073 60 FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: SO3-12 1 3 3 TOTAL SHEETS 24 24							

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NOTES

THE BARRIER RAIL IN EACH SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE

WHEN FOAM JOINT SEAL IS REQUIRED, THE JOINT IN THE DECK SHALL BE SAWED PRIOR TO THE CASTING OF VERTICAL CONCRETE BARRIER RAIL.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

THE #5 S3 & S4 BARS SHALL BE INSTALLED, USING AN ADHESIVE ANCHORING SYSTEM, AFTER SAWING THE JOINT. THE YIELD LOAD FOR THE #5 S3 & S4 BARS IS 18.6 KIPS. FIELD TESTING FOR THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS

		PROJECT NO41665.7A
		<u> </u>
		STATION: 107+16.84 -L2- 13+69 76 -Y-
		SHEET 2 OF 2
	CAROUNI	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
	SFAL	STANDARD
	037180 POLY GINEER	VERTICAL
	Docusigned by: Achaul F. Vuitma	BARRIER RAIL
d	08901A86EBF6470 11/7/2017	REVISIONS SHEET NO.
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BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL

THE 1 $\frac{1}{4}$ " \varnothing holes shall be formed or drilled with a core bit. Impact tools WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " Hold down plate and

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE

		PF 	ROJEC <u>CU</u> Ati	CT NO. MBER ON: <u>1</u>	2 0 7 3 +	41 <u>AND</u> 7+16 -69.	<u>665.7</u> CC <u>.84 -</u> 76 -`	<u>A</u> UNTY - <u>L2-</u> Y-
	SEAL 037180 F WERMIN		depa GUA F C	stat RTMENT S RDRA)R BA	T A	NORTH CAR TRAN RALEIGH NDAF _ AN RIE	olina NSPORTA RD NCHOF R RA	TION RAGE IL
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			RE]	enforc	ING B	AR S	CHEDI	JLE		
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENG
* A1	113	#5	STR	39′-11″	4705	A201	4	#5	STR	38′-7
A2	113	#5	STR	39′-11″	4705	A202	4	#5	STR	37′-7
₩ A3	6	#6	STR	15'-0"	136	A203	4	#5	STR	36′-7
						A204	4	#5	STR	35′-7
* A101	4	#5	STR	38′-7″	161	A205	4	#5	STR	34′-7
* A102	4	#5	STR	37'-7"	157	A206	4	#5	STR	33′-7
* A103	4	#5	STR	36′-7″	153	A207	4	#5	STR	32'-7
* A104	4	#5	STR	35′-7″	148	A208	4	#5	STR	31'-7
₩ A105	4	#5	STR	34'-7"	144	A209	4	#5	STR	30'-7
* A106	4	#5	STR	33′-7″	140	A210	4	#5	STR	29'-7
* A107	4	#5	STR	32'-7"	136	A211	4	#5	STR	28′-7
* A108	4	#5	STR	31'-7"	132	A212	4	#5	STR	27'-7
* A109	4	#5	STR	30'-7"	128	A213	4	#5	STR	26'-7
* A110	4	#5	STR	29'-7"	123	A214	4	#5	STR	25′-7
* A111	4	#5	STR	28'-7"	119	A215	4	#5	STR	24'-7
* A112	4	#5	STR	27'-7"	115	A216	4	#5	STR	23'-7
* A113	4	#5	STR	26'-7"	111	A217	4	#5	STR	22'-7
* A114	4	#5	STR	25'-7"	107	A218	4	#5	STR	21'-7
* A115	4	#5	STR	24'-7"	103	A219	4	#5	STR	20'-7
* A116	4	#5	STR	23'-7"	98	A220	4	#5	STR	19'-7
* A117	4	#5	STR	22'-7"	94	A221	4	#5	STR	18′-7
* A118	4	#5	STR	21'-7"	90	A222	4	#5	STR	17'-7
* A119	4	#5	STR	20'-7"	86	A223	4	#5	STR	16'-7
* A120	4	#5	STR	19'-7"	82	A224	4	#5	STR	15'-7
* A121	4	#5	STR	18'-7"	78	A225	4	#5	STR	14′-7
* A122	4	#5	STR	17'-7"	73	A226	4	#5	STR	13′-7
* A123	4	#5	STR	16'-7"	69	A227	4	#5	STR	12'-7
* A124	4	#5	STR	15′-7″	65	A228	4	#5	STR	11′-7
₩ A125	4	#5	STR	14'-7"	61	A229	4	#5	STR	10′-7
* A126	4	#5	STR	13′-7″	57	A230	4	#5	STR	9'-7
* A127	4	#5	STR	12'-7"	53	A231	4	#5	STR	8'-7
* A128	4	#5	STR	11'-7"	48	A232	4	#5	STR	7'-7
* A129	4	#5	STR	10'-7"	44	A233	4	#5	STR	6'-7
₩ A130	4	#5	STR	9′-7″	40	A234	4	#5	STR	5'-7
₩ A131	4	#5	STR	8′-7″	36	A235	4	#5	STR	4′-7
₩ A132	4	#5	STR	7'-7"	32	A236	4	#5	STR	3'-7
₩ A133	4	#5	STR	6'-7"	27	A237	4	#5	STR	2'-7
* A134	4	#5	STR	5′-7″	23	A238	4	#5	STR	1'-7'
₩ A135	4	#5	STR	4′-7″	19					
₩ A136	4	#5	STR	3′-7″	15	米 B1	108	#4	STR	25′-5
* A137	4	#5	STR	2'-7"	11	B2	102	#5	STR	49′-0
* A138	4	#5	STR	1'-7"	7					
						★ G1	2	#5	STR	56′-5
					•					
						∗ K1	12	#5	1	6'-6
						∗ K2	24	#5	2	6'-8
GRC) OVIN	G BF	RIDG	E FLOC)RS	∗ K3	30	#5	STR	7'-11
BRI	DGE DEC	K		3360 SQ.F	- T.	★ S1	60	#4	3	4'-7

GROUVING	DRIDGE F	LUURS
BRIDGE DECK	3360	SQ.FT.
APPROACH SLAB	S 1579	SQ.FT.
TOTAL	4939	SQ.FT.

REINF	DRCING	STEEL		
∗ EPOX	Y COAT	ED RE	INF. ST	TEEL

E BILL OF	MATERIAL
REINFORCING STEEL	* EPOXY COATED REINFORCING STEEL
(LBS.)	(LBS.)
13103	10658

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS					
BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET, AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3′-5″
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"

		<u>_</u>
		BAR TYPES
LENGTH	WEIGHT	
38′-7″	161	4'-3"
37'-7"	157	
36′-7″	153	
35′-7″	148	
34′-7″	144	$THTS LFG (1) \delta$
33′-7″	140	OVER GIRDER
32′-7″	136	
31′-7″	132] 1'-6"
30′-7″	128	
29'-7"	123	
28'-7"	119	
27'-7"	115	
26'-7"	111	2'-2"
25'-7"	107	
24'-7"	103	
23'-7"	98	
22'-7"	94	
21'-7"	90	
20'-7"	86	
19'-7"	82	1'-6''
18'-7"	78	
17'-7"	73	
16'-7"	69	
15'-7"	65	- 6"
14'-7"	61	
13'-7"	57	
12'-7"	53	
11'-7"	48	
10'-7"	44	
9'-("	40	
8'-("	36	
('-("	32	
6'-("	27	
5' - 1''	23	
$\frac{4^{2} - 7^{2}}{277}$	19	
3 = 1	15	ALL BAR DIMENSIONS ARE OUT TO OUT
2 - 1	7	-
1 1		-
25'-5"	1834	-
49'-0"	5213	-
13 0	0210	-
56'-5"	118	
	110	
6'-6"	81	
6'-8"	167	1
7'-11"	248	
4'-7"	184	
=	13103	
FI =	10,658	
	10,000	
		$\square \square $
		PROJECI NO. <u>41000.1A</u>
		CONDLILANDCOUNTY
		(7.11)
		SIAILON: 10110.04
		$13 + 69 \cdot (6 - Y -$
		STATE OF NORTH CAROLINA DEDARTMENT OF TRANSDOTATION
		RALEIGH
		ESSION FESSION
	:	SUPERSTRUCTURE
		037180
		DILL UT WAIERIAL
		Richard F. Duiton
Poed		08901A86EBF6470 11/7/2017 DEVICTONIC SUBJECT NO
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drawn by : <u>T.M. Ford</u>

CHECKED BY : R.F. WERTMAN

DESIGN ENGINEER OF RECORD : R.F. WERTMAN DATE : 11/06/17

DATE : <u>10/18/17</u>

____ DATE : <u>10/19/17</u>

SEE "SECTION A-A" ON SHEET 3 OF 3 FOR 24" Ø C.S. PIPE DETAILS

<u>PLAN</u>

56-#5 V1 @ 1'-0"CTS.(EA.FACE)

STR.NO.3

	PROJECT NO. <u>41665.7A</u> <u>CUMBERLAND</u> COUNTY STATION: <u>107+16.84</u> -L2- 13+69.76 -Y- SHEET 2 OF 3
TH CAROLINI	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
SEAL 037180	SUBSTRUCTURE
AD F WERMIN	END BENT #1
DocuSigned by: Dicheul F. Vuitma 08901A86EBF6470	
Road 11/7/2017	REVISIONS SHEET NO.
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BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

CONST.JT.-

drawn by : T.M. FORD	DATE: 10/16/17
CHECKED BY : R.F. WERTMAN	DATE: 10/19/17
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE: 11/06/17

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	PROJECT NO. <u>41665.7</u> <u>CUMBERLAND</u> CO STATION: <u>107+16.84</u> 13+69.76 - Y	A UNTY L2- /-
	SHEET 2 OF 3	
NUMBER CAROLIN	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTA RALEIGH	FION
SEAL 037180	SUBSTRUCTURE	
F WERNIN	END BENT #2	
DocuSigned by: Dechem F V	ectino	
Road 11/7/2017	REVISIONS	SHEET NO. SA3-2A
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	T			
	BIL	L OF MA	A TERIAL	-
TES:	FOR ONE APPROACH SLAB			
CONSTRUCTED PRIOR TO ECK.	BAR NO	ST7F TYPF	LINCO /	WETGHT
'MSE RETAINING WALL″PLANS.	* A1 50	#4 STR	29'-3"	977
PECTAL PROVISIONS	A2 52	#4 STR	29'-1"	1010
FAL WIDTH OF THE FOAM JOINT	*B1 79	#5 STR	24'-2"	1992
LAL WIDTH OF THE FOAM OUTH	B2 79	#6 STR	24'-8"	2928
EE SPECIAL PROVISIONS.	REINFORCIN * EPOXY CO REINFORC	I <u>g steel</u> Ated Ing steel	LBS.	3938 2969
	CLASS AA C	CONCRETE	C.Y.	42.6
	SP	LICE LENGTH	IS CHART	
TOMERIC CONCRETE	SIZ	LE COATED U	NCOATED	
ELASTOMERIC CONCRETE * (CLL FT.)	#4 #5 #6	2'-0" 2'-6" 3'-10"	1'-9" 2'-2" 2'-7"	
9,2				
9.2				
18.4				
) UN THE MINIMUM BLUCKUUT SHOWN.				
R 🖛				
FOF	— CLASS ``B'' R erosion co	STONE NTROL		
TEMP. SLOPE DRAIN				
2'-0"MIN. 1'-0"				
BLOCK S S	SHOULDER			
DRAIN	<u>2'-0"</u>			
GRADE TO INLE		− 5′−6″		

FLOW LINE END OF 1'-6'' MIN.

NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

PLAN VIEW

	PROJECT NO. <u>41665.7A</u> CUMBERLAND COUNTY					
	STATION: <u>107+16.84</u> -L2- 13+69.76 -Y-					
	SHEET 2 OF 2					
BEAL 037180 F WERNING	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD BRIDGE APPROACH					
FRoad	REVISIONS SHEET NO					
27607-3073 560 FINAL UNLESS ALL F-0270 SIGNATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: SO3-23 1 3 TOTAL SHEETS TOTAL 24 24					

STR.NO.3

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NOTES

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.

SLOPE PROTECTION SHALL CONSIST OF 4"POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET.CONCRETE SHALL BE CLASS "B".THE CONCRETE SURFACE SHALL BE FINISHED TO THE SATISFACTION OF THE ENGINEER. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 20" WIDE. THE COST OF THE WELDED WIRE FABRIC SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.107+16.84 -L2- STA.13+69.76 -Y-	4 INCH SLOPE PROTECTION	* Welded Wire Fabric 60 inches Wide
	SQUARE YARDS	APPROX. L.F.
END BENT 1	19	34
END BENT 2	34	60

* QUANTITY SHOWN IS BASED ON 5' POURS.



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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

SLOPE PROTECTION
DETAILS

DocuSigned by: Dichard For Vietono 08901A86EBF6470 11/7/2017			REVIS	SIO	NS		SHEET NO
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INAL UNLESS ALL	1			ß			TOTAL SHEETS
NATURES COMPLETED	2			Ą			24
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² SEAL ⁶ 037180

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SO.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN	
UF IIMDER	JIJ LOS. PER SU. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FI.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

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DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS,AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER

SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

