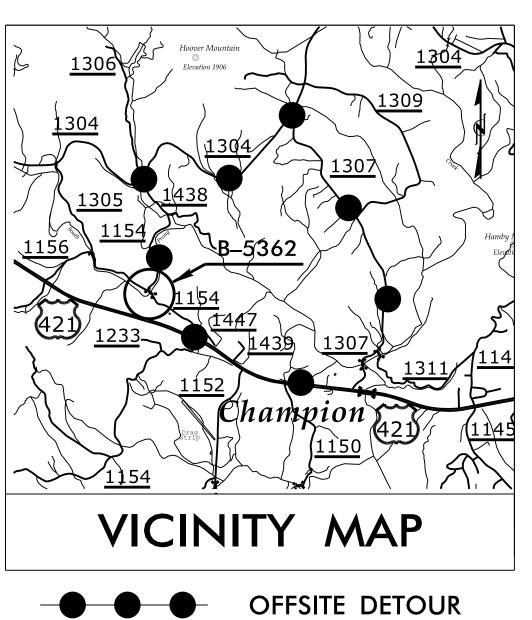
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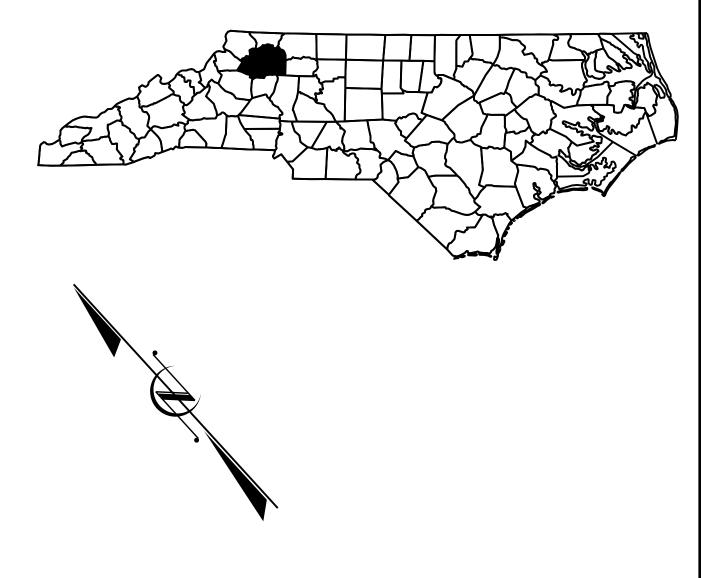
# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# WILKES COUNTY

LOCATION: BRIDGE 82 OVER SOUTH PRONG LEWIS FORK CREEK ON SR 1154

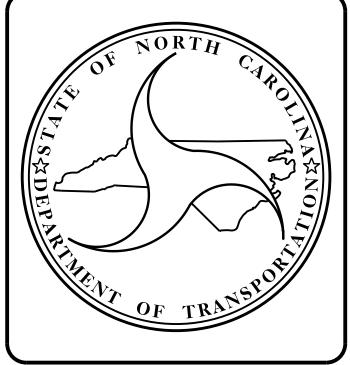
TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE

STATE	STATE		SHEET NO.	TOTAL SHEETS			
N.C.	В	<b>-4978</b>					
STATE PRO	l. NO.	F. A. PROJ. NO.		DESCRIPT	TON		
39897	.1.1	BRZ-1154(6)	P. E.				
39897.	1.2			R/W	/		
39897.	1.3		CON	<b>NSTRU</b>	ICTION		



BEGIN TIP PROJECT B-4 STA. 10+45.00	F	SOUTH PRONG LEWIS FORK CREEK	END TIP PROJECT B-4978 STA.13+75.00 -L-
TO OLD HWY 421  HACKETT TOWN RD. (SR 1305)  BEGIN CONST.  STA. 10+15.00 -Y-	BEGIN BRIDGE STA. 11+83.62 -L-		END BRIDGE STA. 12+88.62 -L-

# STRUCTURE



# DESIGN DATA

ADT 2017 = 218 ADT

ADT 2037 = 309 ADT

K = 10 %

D = 60 %

T = 5 % \*

V = 15 MPH

\* TTST = 2% DUAL 3%

FUNC CLASS = LOCAL

SUB REGIONAL TIER

PROJECT LENGTH

LENGTH OF ROADWAY TIP PROJECT B-4978 = 0.043 MI.

LENGTH OF STRUCTURES TIP PROJECT B-4978 = 0.020 MI.

TOTAL LENGTH OF TIP PROJECT B-4978 = 0.063 MI.

Prepared in the Office of:

# DIVISION OF HIGHWAYS

STRUCTURES MANAGEMENT UNIT
1000 BIRCH RIDGE DR.
RALEIGH, N.C. 27610

2012 STANDARD SPECIFICATIONS

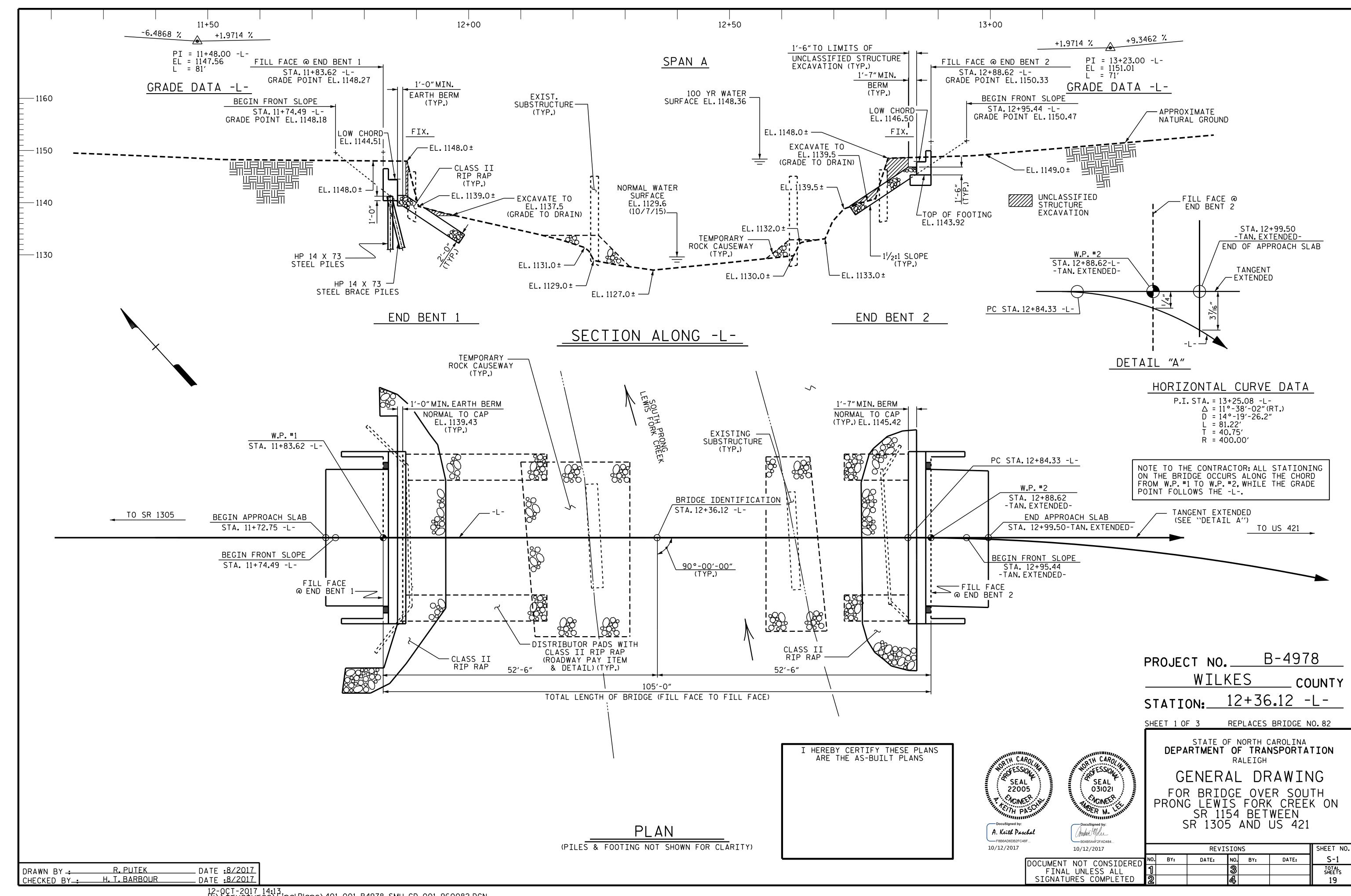
LETTING DATE: NOVEMBER 21, 2017

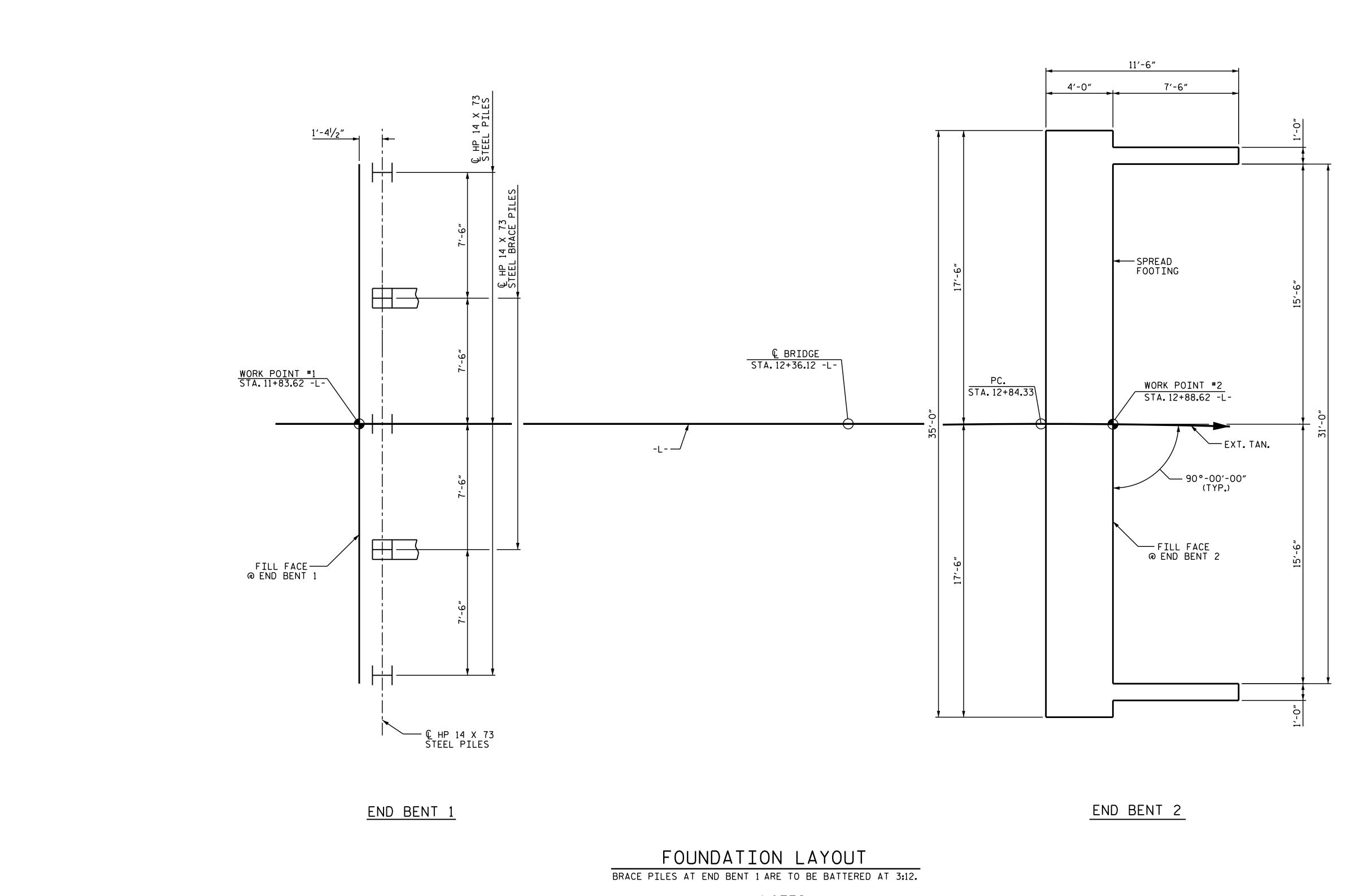
A. KEITH PASCHAL, P.E.

PROJECT ENGINEER

AMBER M. LEE, P.E.

PROJECT DESIGN ENGINEER





# NOTES

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT 1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 135 TONS PER PILE.

DRIVE PILES AT END BENT 1 TO A REQUIRED DRIVING RESISTANCE OF 225 TONS PER PILE.

THE SPREAD FOOTING AT END BENT 2 IS DESIGNED FOR A FACTORED RESISTANCE OF 8 TSF. CHECK FIELD CONDITIONS FOR THE REQUIRED RESISTANCE OF 18 TSF JUST BEFORE PLACING CONCRETE.

KEY IN FOOTING AT END BENT 2 AT LEAST 6"INTO ROCK WITH MINIMUM THICKNESS AS SHOWN ON THE PLANS.

PROJECT NO. B-4978

WILKES

LKES COUNTY

STATION: 12+36.12 -L-

SHEET 2 OF 3

amber Male

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER SOUTH PRONG LEWIS FORK CREEK ON SR 1154 BETWEEN SR 1305 AND US 421

BOABSAAFZFAD484...
10/12/2017

REVISIONS

SHEET NO.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

REVISIONS

SHEET NO.

SHEET NO.

SHEET NO.

10/12/2017

REVISIONS

SHEET NO.

BY:

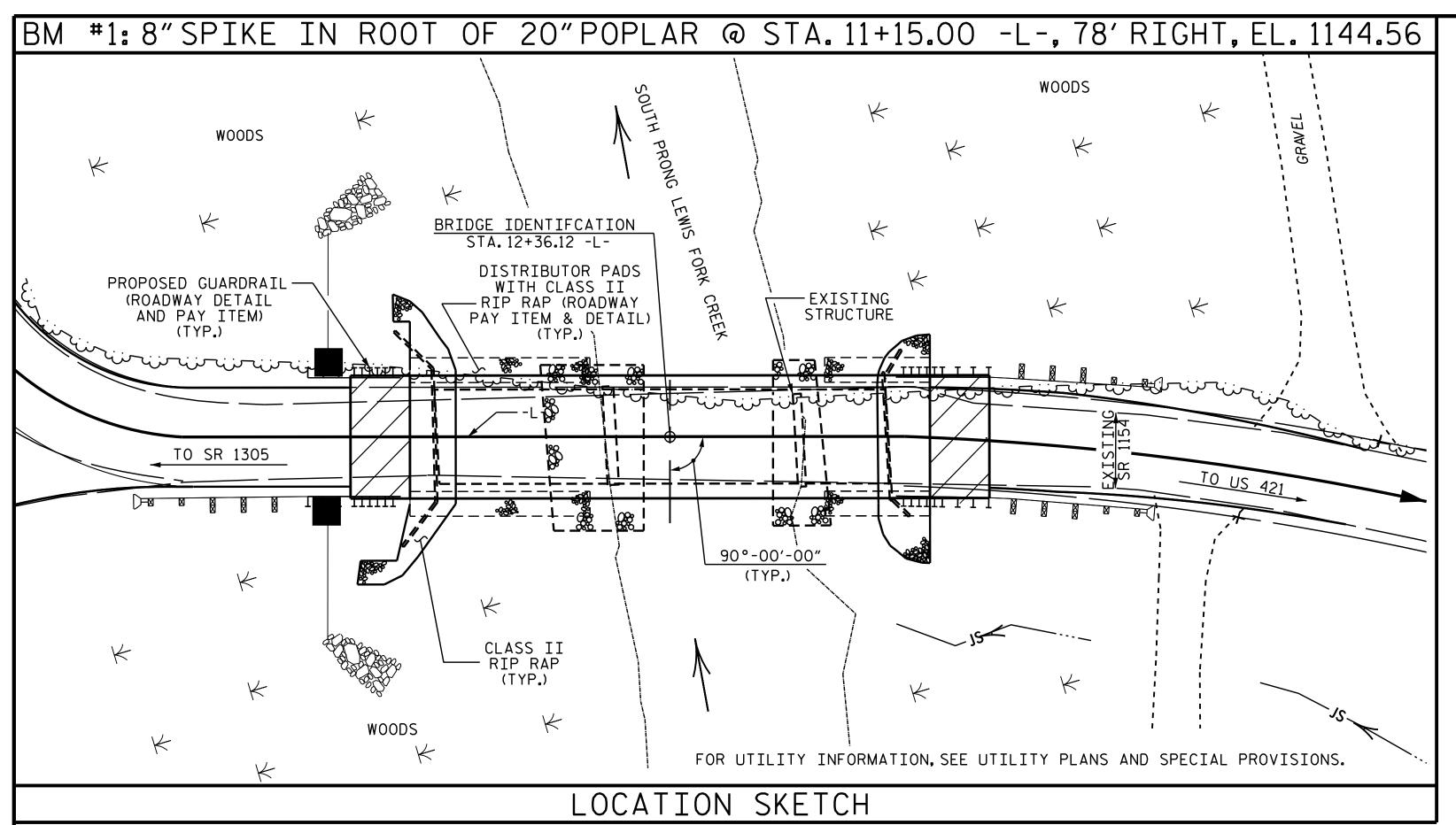
DATE:

S-2

TOTAL SHEETS

19

DRAWN BY: M. WELDON DATE: 07-2017
CHECKED BY: H. T. BARBOUR DATE: 08-2017



# NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP USED IN THE CAUSEWAY MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS AT STATION 12+36.12 -L-.

TEMPORARY CAUSEWAY ACCESS WILL BE REMOVED TO A WIDTH OF APPROXIMATELY THIRTY (30) FEET, ONCE EXISTING BENTS ARE REMOVED TO FACILITATE THE SETTING OF THE BOX BEAMS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 12+36.12 -L-."

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 15 FT EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 3 SPANS, 1 @ 17'-10", 1 @ 38'-8" AND 1 @ 35'-0" ON I BEAMS AND DOUBLE CHANNELS ON 4 X 8 TIMBER FLOOR; WITH A CLEAR ROADWAY WIDTH OF 19.083 FT. ON END BENTS AND BENTS WITH TIMBER CAP, PILES AND SILLS WITH CONCRETE FOOTINGS AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT. FOR REMOVAL OF EXISTING STRUCTURE, SEE SPECIAL PROVISIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR ASBESTOS ASSESEMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

# HYDRAULIC DATA

DESIGN DISCHARGE \_\_\_\_ = 4,400 C.F.S. FREQUENCY OF DESIGN FLOOD \_\_\_\_\_ = 25 YRS. DESIGN HIGH WATER ELEVATION \_\_\_\_ = 1139.2 FT. DRAINAGE AREA \_\_\_\_\_ = 32.4 SQ. MI BASE DISCHARGE (Q100) = 13,806 C.F.S BASE HIGH WATER ELEVATION = 1148.36 FT.

# OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE \_\_\_\_ = 11,763+ C.F.S. FREQUENCY OF OVERTOPPING FLOOD \_\_\_\_ = 100- YRS. OVERTOPPING FLOOD ELEVATION \_\_\_\_ = 1148.16 FT.

	TOTAL BILL OF MATERIAL																
	CONSTRUCTION MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS	REMOVAL OF EXISTING STRUCTURE	FOUNDATION EXCAVATION FOR END BENT 2	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 14 X 73 STEEL PILES	HP STEI	14 X 73 EL PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS		O"X 3'-3" STRESSED ONCRETE X BEAMS	ASBESTOS ASSESSMENT
	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	EA.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.	LUMP SUM
SUPERSTRUCTURE											205.5				9	924.75	
END BENT 1				LUMP SUM	26.8		4,185	5	5	115.0		75	80				
END BENT 2			LUMP SUM	LUMP SUM	26.9		5,776					150	165				
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM	53.7	LUMP SUM	9,961	5	5	115.0	205 <b>.</b> 5	225	245	LUMP SUM	9	924.75	LUMP SUM

B-4978 PROJECT NO.\_ WILKES COUNTY STATION: 12+36.12 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING FOR BRIDGE OVER SOUTH PRONG LEWIS FORK CREEK ON SR 1154 BETWEEN SR 1305 AND US 421

10/12/2017 SHEET NO REVISIONS DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

R. PUTEK DATE : 08-2017 DRAWN BY DATE : 08-2017 H. T. BARBOUR CHECKED BY : \_

### LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE MOMENT SHEAR MOMENT DISTRIBUTION FACTORS (DF) ROLLING RATING MINIMUN RATING (RF) GIRDER CONT DIST/ LEFT SPAN DIST/ LEFT SPAN 1.75 0.272 1.07 50.625 0.487 1.33 5.063 0.272 50.625 N/A 1.04 EL 1.04 HL-93(Inv) Α 0.80 1.39 1.35 0.487 1.73 50.625 HL-93(0pr) N/A 0.272 1.39 EL 5.063 N/A **DESIGN** LOAD 36.000 52.339 1.75 0.272 50.625 0.487 5.063 0.272 EL 1.8 1.45 50.625 HS-20(Inv) 1.45 1.5 0.80 RATING 36.000 50.625 2.33 HS-20(0pr) 1.94 69.971 1.35 0.272 1.94 EL 0.487 5.063 N/A Α EL 50.625 5.56 13.500 3.46 46.689 0.272 50.625 0.487 0.80 0.272 3.46 4.46 EL SNSH EL 5.063 3.22 0.487 3.88 50.625 20.000 50.002 0.272 EL 50.625 5.063 0.80 0.272 2.50 SNGARBS2 2.5 EL 2.34 3.58 0.272 50.625 22.000 51.406 0.272 3.01 50.625 0.487 5.063 0.80 2.34 SNAGRIS2 EL EL 50.625 27.250 46.836 2.77 1.72 50.625 1.72 0.272 2.22 EL 0.487 5.063 0.272 SNCOTTS3 0.80 EL SNAGGRS4 34.925 49.124 0.272 EL 50.625 0.487 2.25 5.063 0.80 0.272 50.625 1.41 1.81 1.41 EL 35.550 1.38 48.969 0.272 1.78 50.625 0.487 2.25 5.063 0.272 1.38 50.625 EL SNS5A EL 0.80 Α 1.25 50.625 1.25 50.006 0.272 50.625 0.487 2.04 5.063 0.80 0.272 SNS6A 39.950 1.61 EL EL 50.625 SNS7B 42.000 1.19 50.046 0.272 1.54 EL 50.625 0.487 1.98 5.063 0.80 0.272 1.19 LEGAL LOAD 50.254 50.625 TNAGRIT3 33.000 1.52 0.272 EL 0.487 2.44 5.063 0.80 0.272 1.52 50.625 1.96 RATING 1.53 0.272 50.625 0.487 0.80 0.272 50.625 TNT4A 33.075 50.483 1.97 Α EL 2.4 EL 5.063 1.53 EL 50.625 0.487 2.07 50.625 TNT6A 41.600 1.24 51.446 0.272 1.59 EL 5.063 0.80 0.272 1.24 50.625 42.000 1.24 51.951 0.272 1.59 EL 50.625 0.487 2.03 5.063 0.80 0.272 1.24 TNT7A EL 1.27 53.138 0.272 1.63 50.625 0.487 1.95 5.063 0.272 1.27 50.625 42.000 EL 0.80 TNT7B 1.4 EL 50.625 1.89 0.272 43.000 1.21 52.214 0.272 1.57 0.487 5.063 0.80 50.625 TNAGRIT4 EL 1.21 EL 50.625 1.15 51.749 0.272 50.625 0.487 1.85 0.272 1.15 45.000 1.48 EL 5.063 0.80 TNAGT5A

EL 50.625 0.487

# LOAD FACTORS:

	DESIGN	LIMIT STATE	$\gamma_{\sf DC}$	$\gamma_{\sf DW}$
	LOAD RATING FACTORS	STRENGTH I	1.25	1.50
		SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

# COMMENTS:

- 1.
- 3.
- 4.

EL 50.625

0.80 | 0.272 | 1.14 |

5.063

- (#) CONTROLLING LOAD RATING
- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- (3) LEGAL LOAD RATING \*\*
- \*\* SEE CHART FOR VEHICLE TYPE

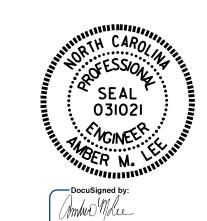
# GIRDER LOCATION

- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER

PROJECT NO. B-4978

WILKES COUNTY

STATION: 12+36.12 -L-



STANDARD

LRFR SUMMARY FOR
PRESTRESSED
CONCRETE GIRDERS
(NON-INTERSTATE TRAFFIC)

STATE OF NORTH CAROLINA

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2 SHEET NO. BY: DATE: SHEET'S 19

0.272 1.47

51.328

45.000

TNAGT5B

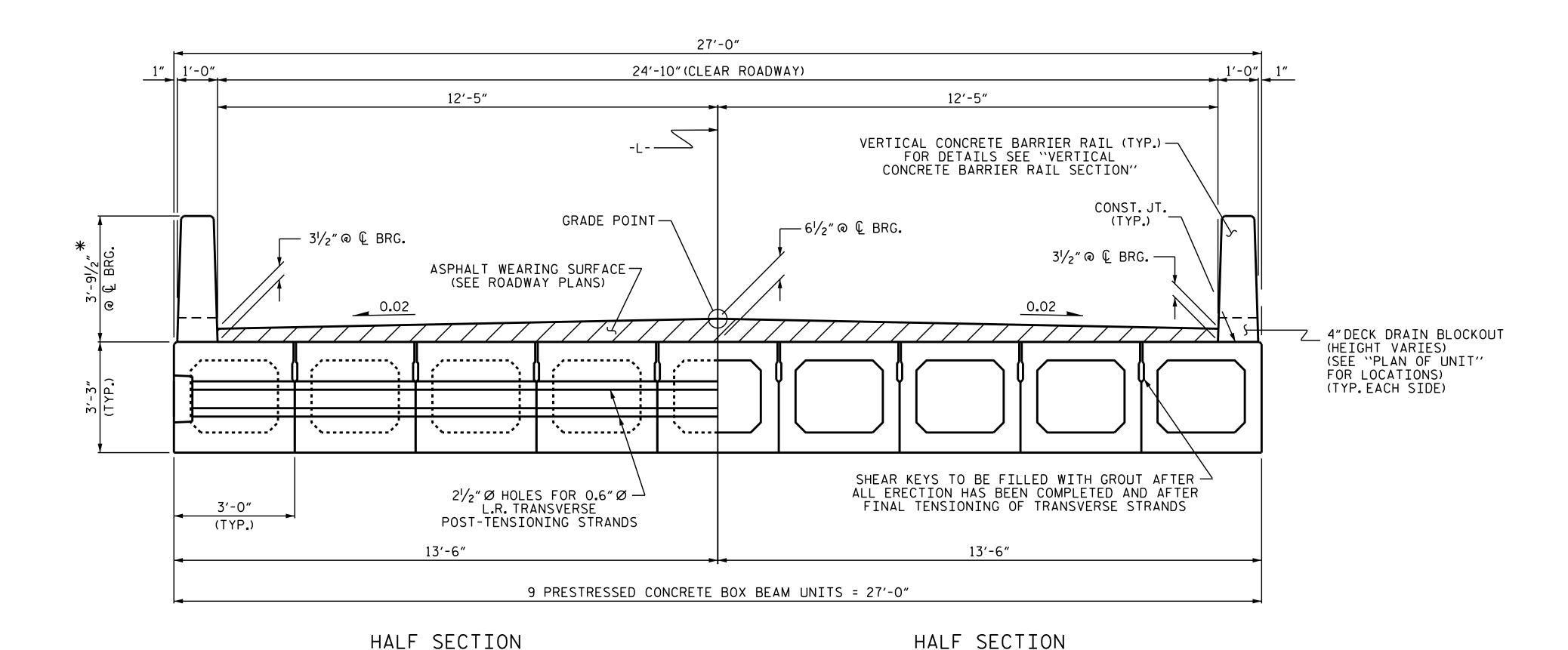
LRFR SUMMARY

ASSEMBLED BY : B. A. DUKE
CHECKED BY : A. M. LEE

DATE : IO-28-I6
DATE : 8-20I7

DRAWN BY : MAA I/08
CHECKED BY : GM/DI 2/08

REV. II/I2/08RR MAA/GM
REV. IO/I/II MAA/GM

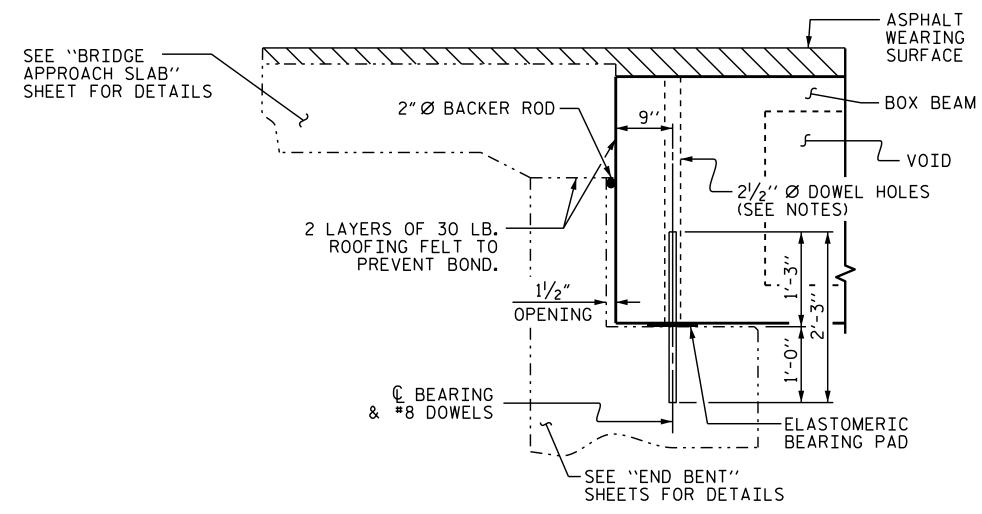


THROUGH VOIDS

\* THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

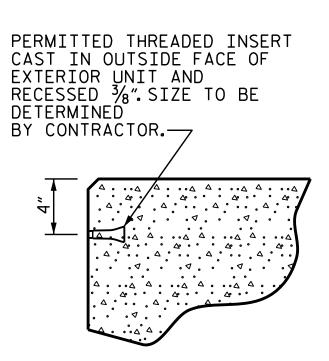
TYPICAL SECTION

# FIXED END



AT INTERMEDIATE DIAPHRAGMS

SECTION AT END BENT



THREADED INSERT DETAIL

# NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE  $2\frac{1}{2}$ " Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6,000 PSI.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 4" X 5". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE BOX BEAM UNIT TO THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR BOX BEAM UNITS THAT REQUIRE DRAINS IN THE BARRIER RAIL.

> B-4978 PROJECT NO. WILKES COUNTY STATION: 12+36.12 -L-

SHEET 1 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT

9/26/ DOCUMENT NOT FINAL UNL SIGNATURES

031021

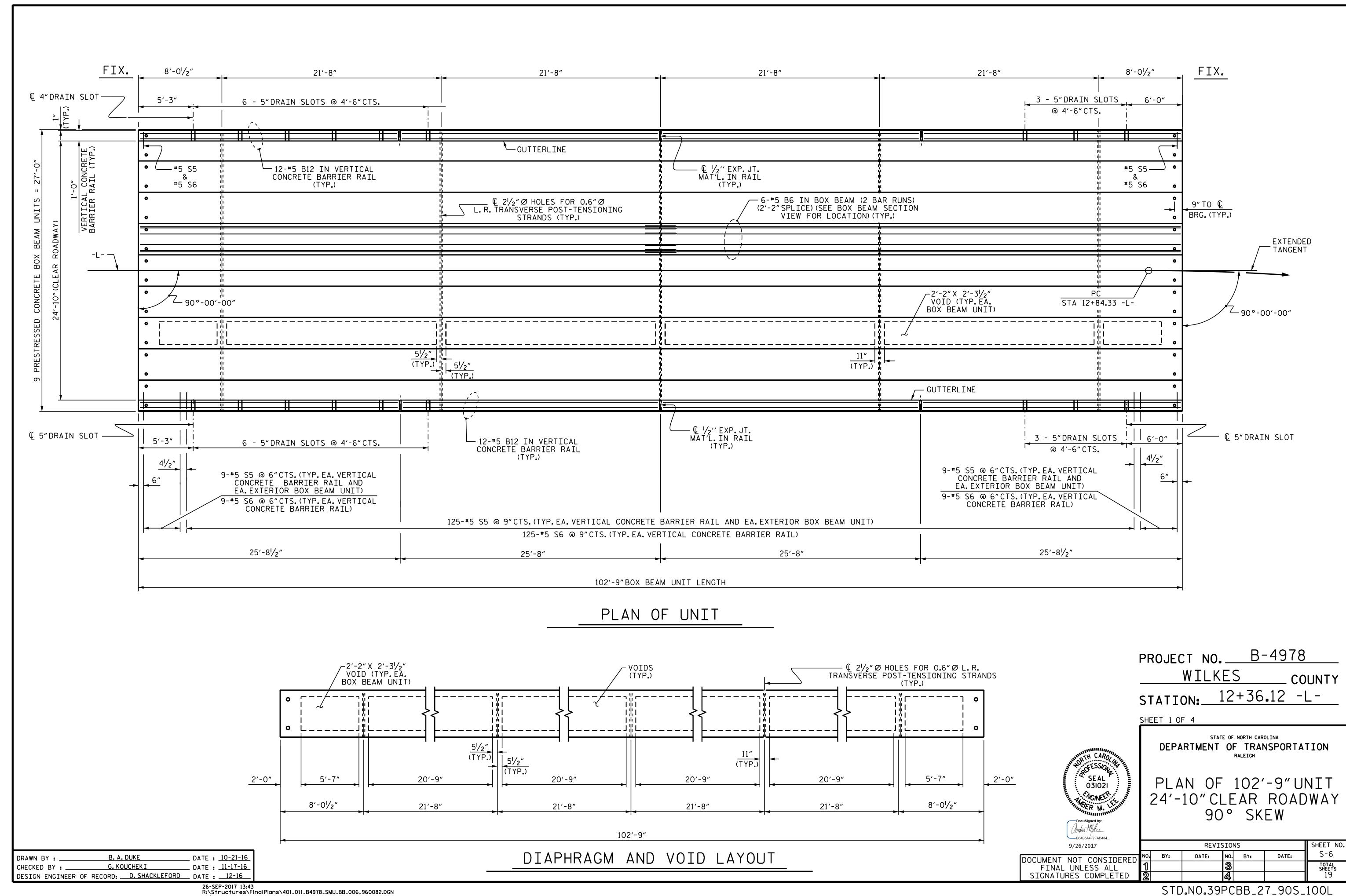
CHCINEER

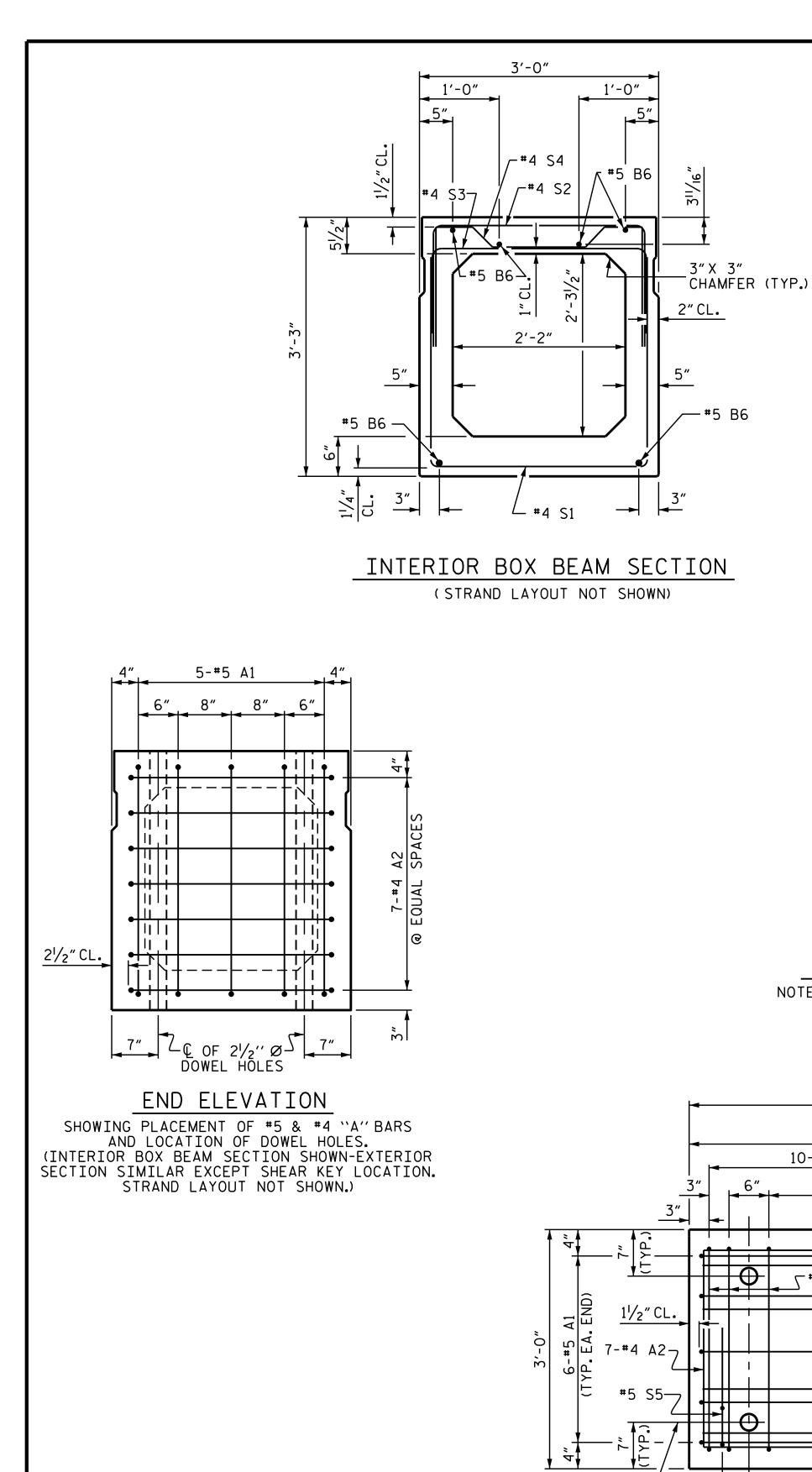
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9/26/2017		SHEET NO.					
NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-5
. UNLESS ALL	1			3			TOTAL SHEETS
IRES COMPLETED	2			4			19

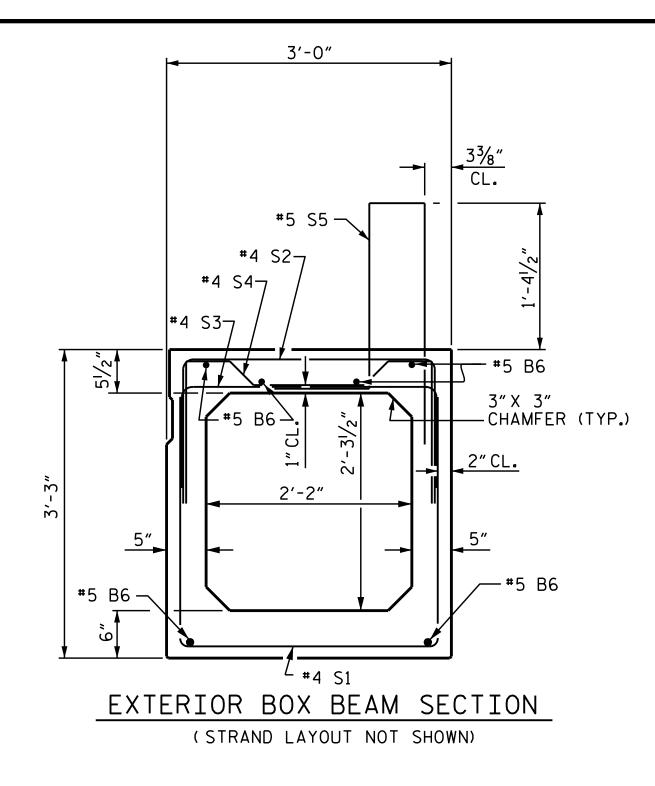
CHECKED BY: G. KOUCHEKI DATE: 11-2-16 DRAWN BY : DGE 8/II REV. 10/15 MAA/TMG CHECKED BY : TMG II/II

DATE :10-20-16

ASSEMBLED BY : B. A. DUKE







GRADE 270 STRANDS

(SQUARE INCHES)

ULTIMATE STRENGTH (LBS.PER STRAND)

APPLIED PRESTRESS (LBS.PER STRAND )

SHEAR KEY DETAIL

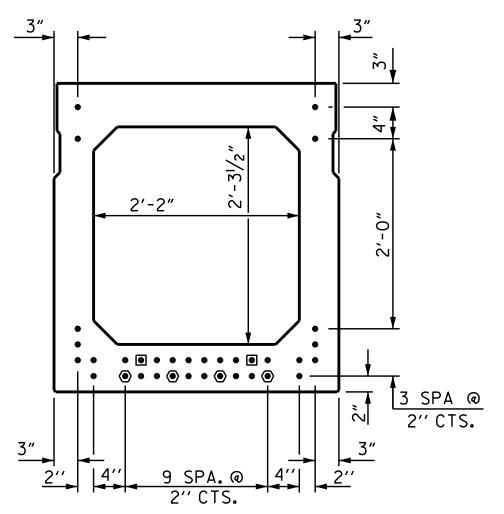
NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR BOX BEAMS.

0.6" Ø L.R.

0.217

58,600





TYPICAL STRAND LOCATION (34 STRANDS REQUIRED)

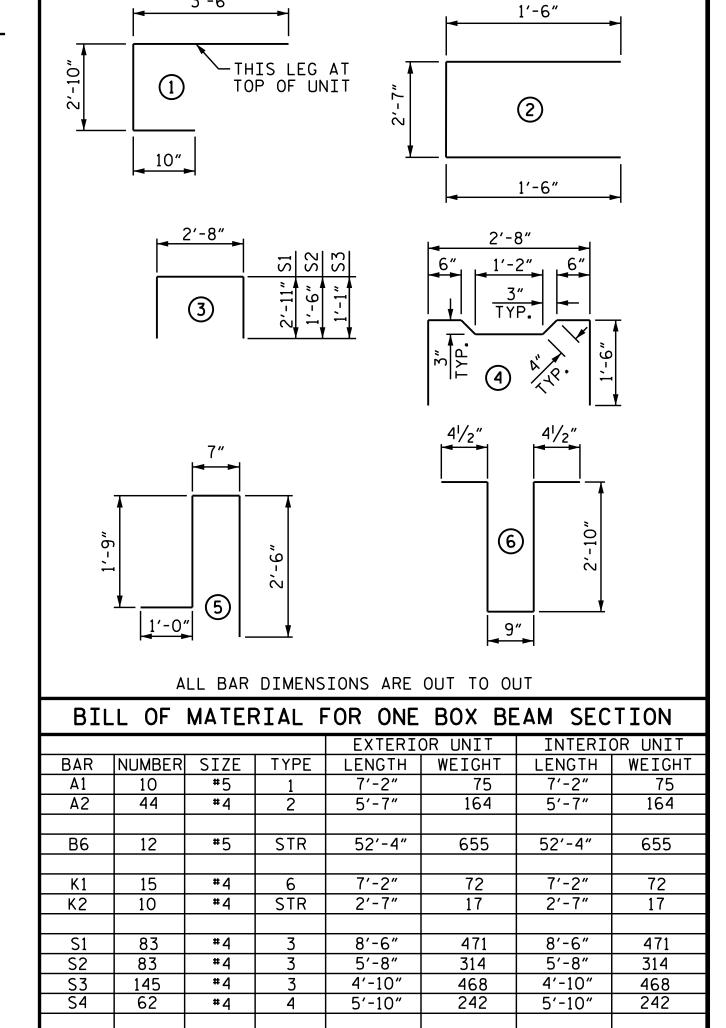
DEBONDING LEGEND

FULLY BONDED STRANDS

STRANDS DEBONDED FOR 4'-0"FROM END OF GIRDER

STRANDS DEBONDED FOR 12'-0"FROM END OF GIRDER

BOND SHALL BE BROKEN ON STRANDS AS SHOWN FOR THE SPECIFIED LENGTH FROM EACH END OF THE BOX BEAM. SEE STANDARD SPECIFICATIONS ARTICLE 1078-7.



5′-10″

20.1

2478

870

No. 34

870

LBS.

CU. YDS. 19.9

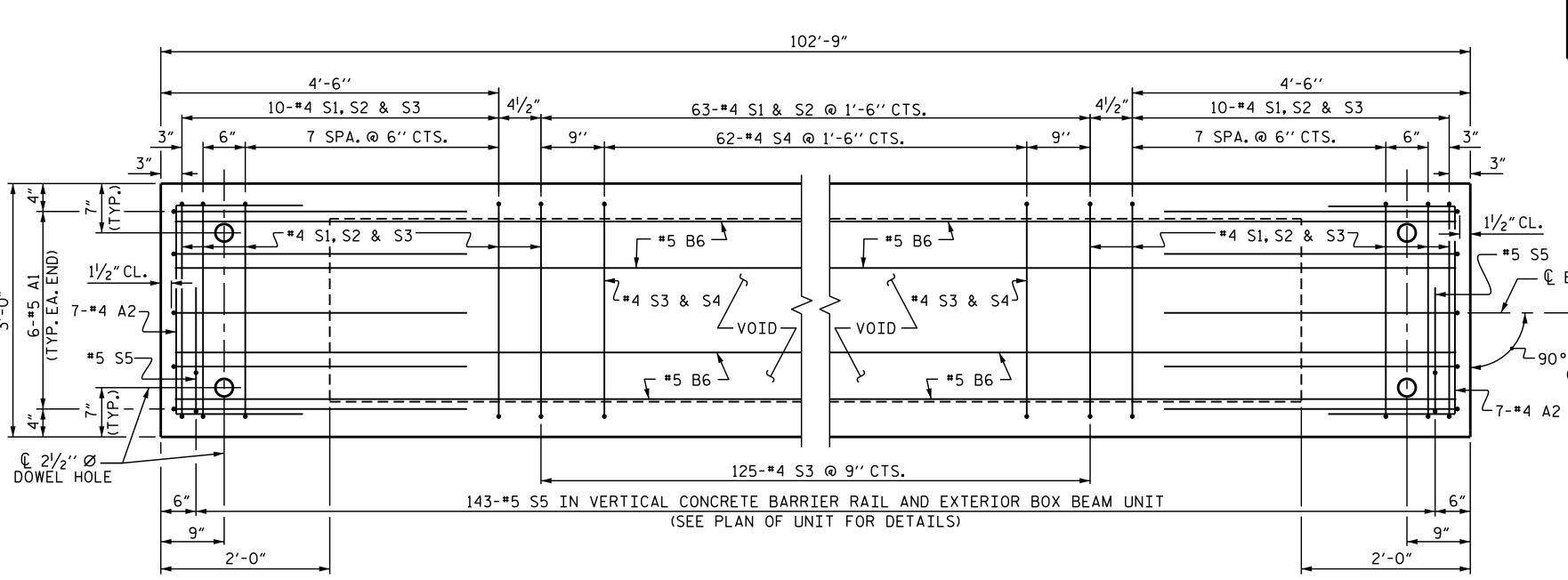
2478

No. 34

CU. YDS

BAR TYPES

3′-6″



B-4978 PROJECT NO. \_\_\_ WILKES COUNTY STATION: 12+36.12 -L-

SHEET 2 OF 4

\* S5 143

\_\_ € BOX BEAM

-90°-00′-00″

(TYP.)

031021

NOINEER

Ambut Mile

REINFORCING STEEL

7500 P.S.I. CONCRETE

0.6" Ø L.R. STRANDS

\* EPOXY COATED REINF. STEEL

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

3'-0" X 3'-3"
PRESTRESSED CONCRETE
BOX BEAM UNIT

SHEET NO. REVISIONS 9/26/2017 S-7 DATE: DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED BY: TOTAL SHEETS

PLAN OF BOX BEAM

EXTERIOR UNIT SHOWN, INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S5 BARS.

FOR LOCATION OF DIAPHRAGMS, SEE "PLAN OF UNIT".

FOR THREADED INSERTS, SEE "THREADED INSERT DETAIL". FOR REINFORCING STEEL IN DIAPHRAGMS, SEE "DOUBLE DIAPHRAGM DETAILS".

\_ DATE : 10-26-16

\_ DATE : <u>11-17-16</u>

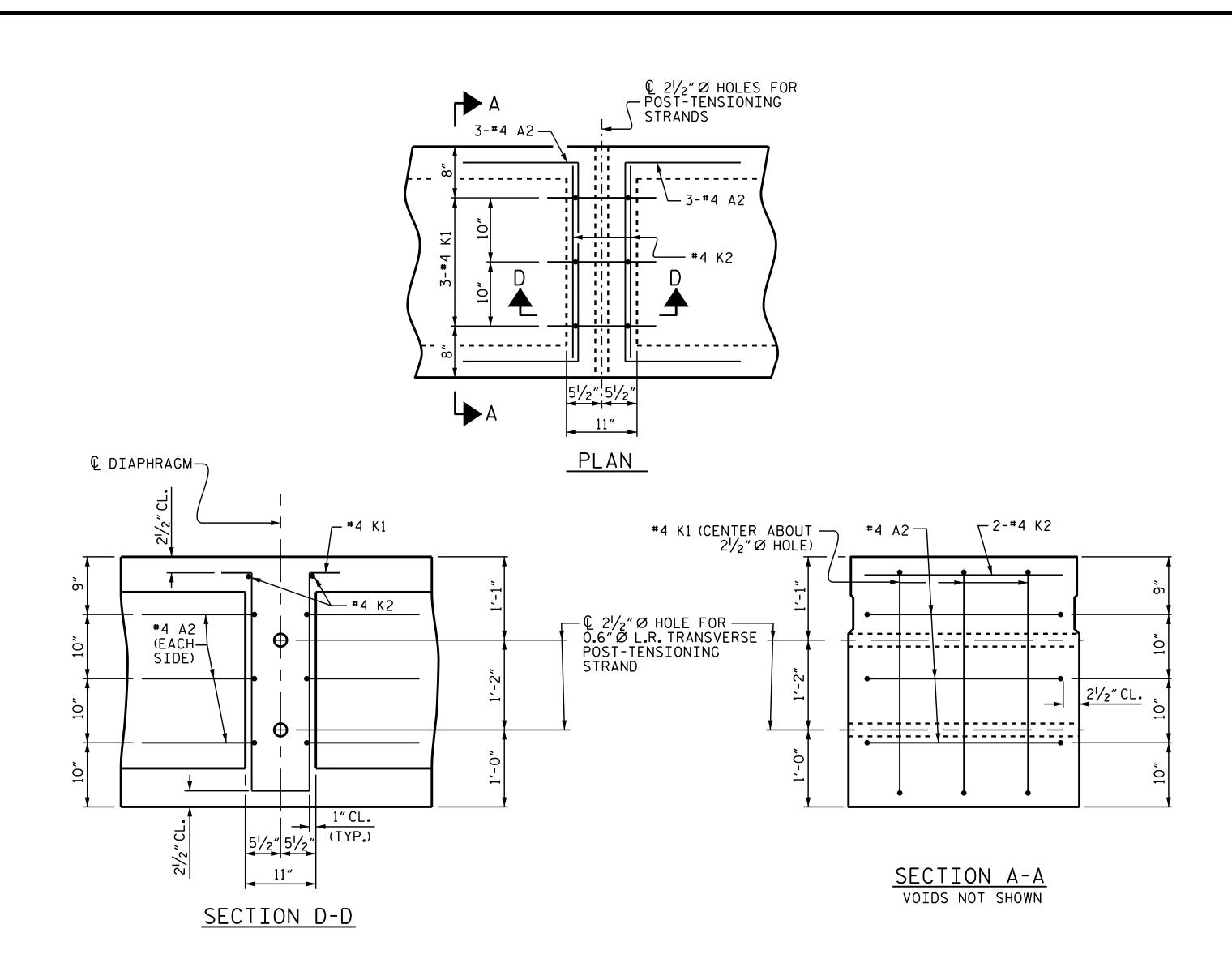
B. A. DUKE

G. KOUCHEKI

DESIGN ENGINEER OF RECORD: D. SHACKLEFORD DATE: 12-2016

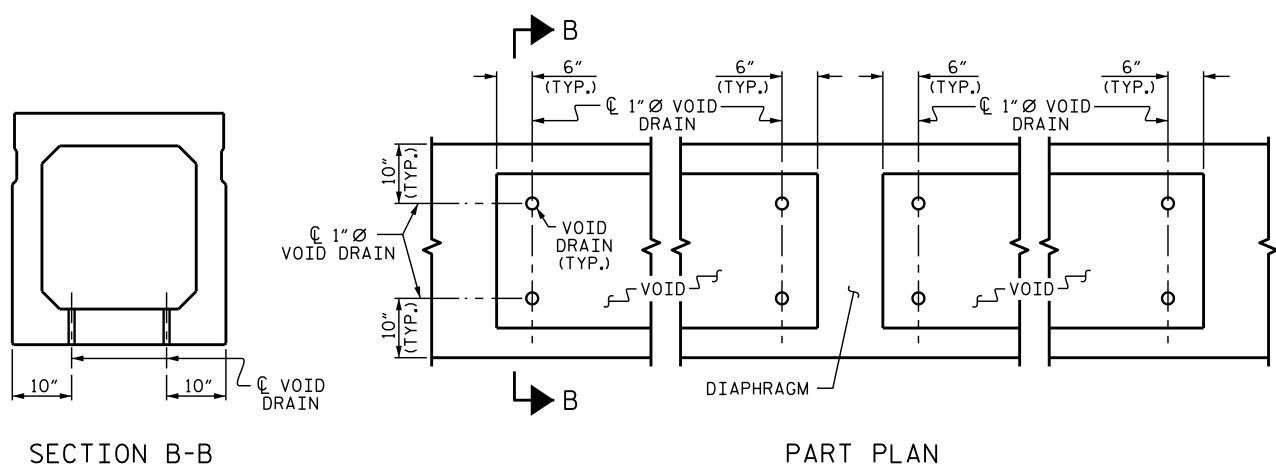
DRAWN BY :

CHECKED BY :



# DOUBLE DIAPHRAGM DETAILS

#4 "S" BARS NOT SHOWN. #4 "S" BARS MAY BE SHIFTED SLIGHTLY TO CLEAR  $2\frac{1}{2}$ " Ø HOLE.



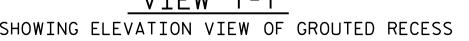
B. A. DUKE \_ DATE : <u>10-24-16</u> DRAWN BY : \_\_ DATE : <u>11-17-16</u> G. KOUCHEKI DESIGN ENGINEER OF RECORD: D. SHACKLEFORD DATE: 12-2016

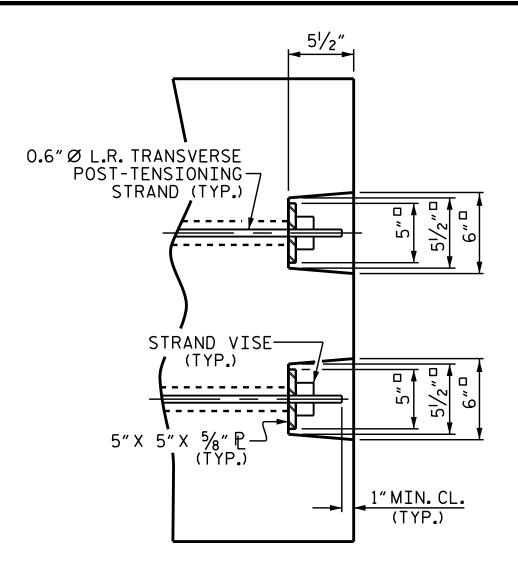
PART PLAN

# VOID DRAIN DETAILS

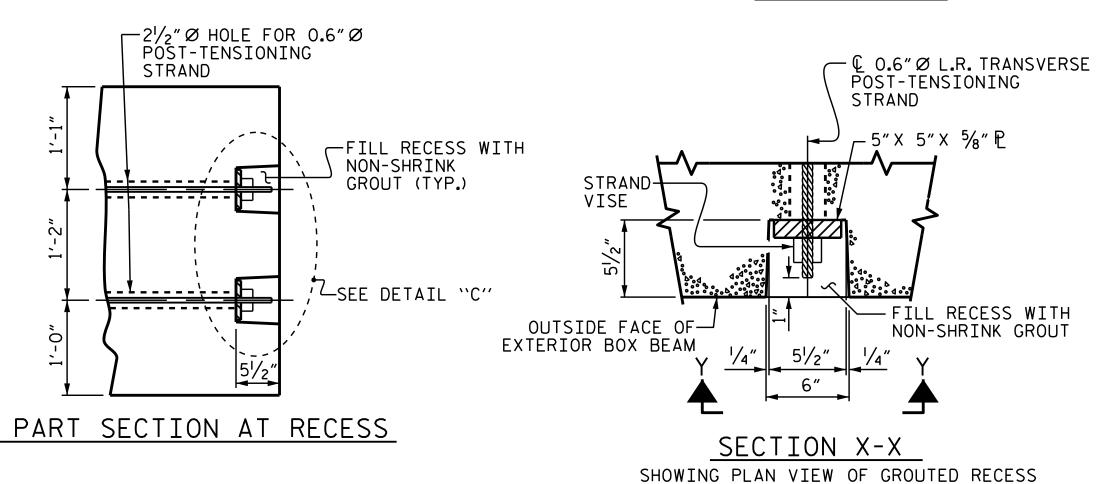
(DIMENSIONS SHOWN ARE TYPICAL FOR EACH VOID)

 $2\frac{1}{2}$ "Ø HOLE FOR 0.6"Ø POST-TENSIONING-STRAND (TYP.) VIEW Y-Y SHOWING ELEVATION VIEW OF GROUTED RECESS





DETAIL "C"



GROUTED RECESS DETAIL AT OF POST-TENSIONED STRANDS OF EXTERIOR BOX BEAM

DEAD LOAD DEFLECTION AND	CAMBER
	3'-0" × 3'-3"
102'-9"BOX BEAM UNIT (NC)	0.6"Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	2"
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	11/4" ∤
FINAL CAMBER	3⁄4″ ∳
ME THELLINES CHITHDE WEADTHE SHIDEA	CE

\*\* INCLUDES FUTURE WEARING SURFACE

PROJECT NO. B-4978 WILKES \_\_\_ COUNTY

STATION: 12+36.12 -L-

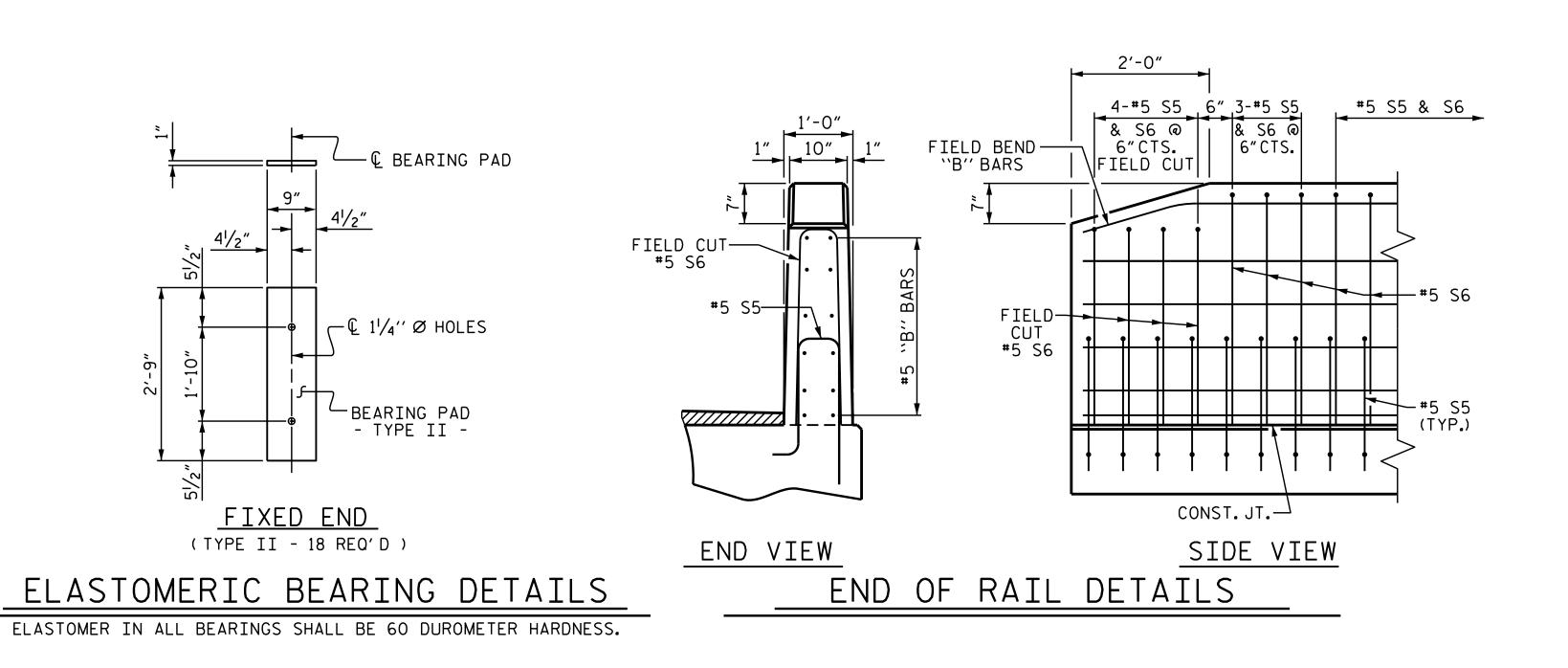
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

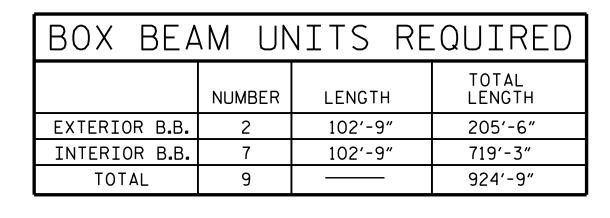
3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT

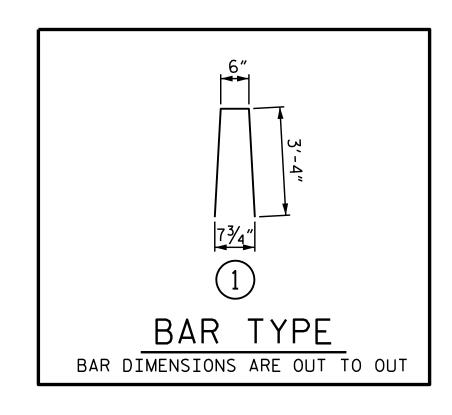
SHEET 3 OF 4

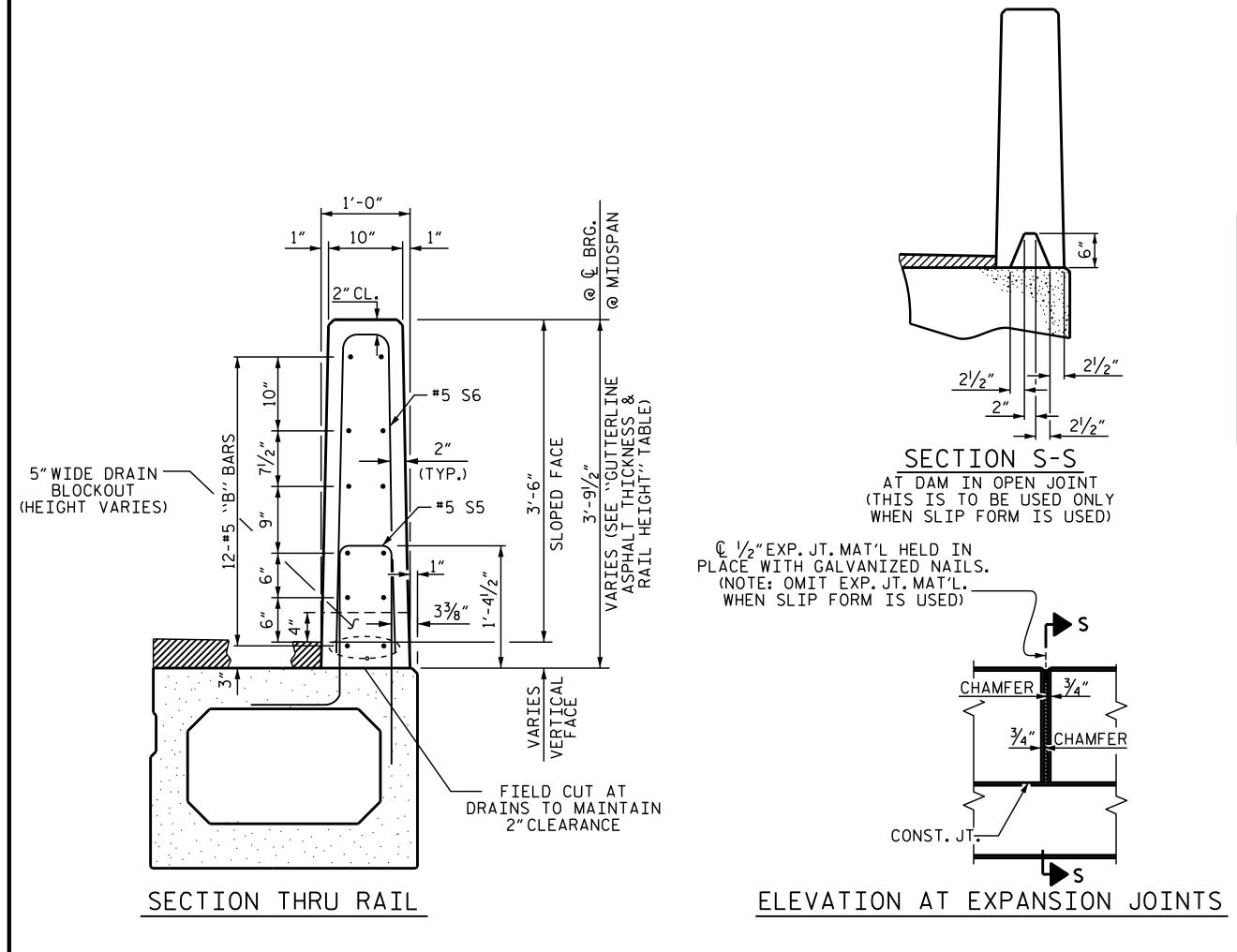
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SHEET NO. REVISIONS S-8 DATE: DATE:









BII	L OF MATERIAL FOR VERTICAL CON	CRETE B	ARR:	IER F	RAIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
	102'-9" UNIT				
<b></b> ₩ B12	96	#5	STR	25'-4"	2537
	000		4	7, 0,,	0170
<del>*</del> \$6	286	#5	1	7′-2″	2138
* EPOX	Y COATED REINFORCING STEEL	l	LBS.		1 <u>4675</u>
CLASS	AA CONCRETE		CU.YDS.		26.5
TOTAL	VERTICAL CONCRETE BARRIER RAIL		LN.FT.		205.5

GUTTERLINE ASPHALT THICKNESS & RAIL HEIGHT

ASPHALT OVERLAY THICKNESS RAIL HEIGHT
@ MID-SPAN @ MID-SPAN

102'-9"UNITS

2¾"

3'-8¾"

PROJECT NO. B-4978

WILKES COUNTY

STATION: 12+36.12 -L-

SHEET 4 OF 4

SEAL 031021

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

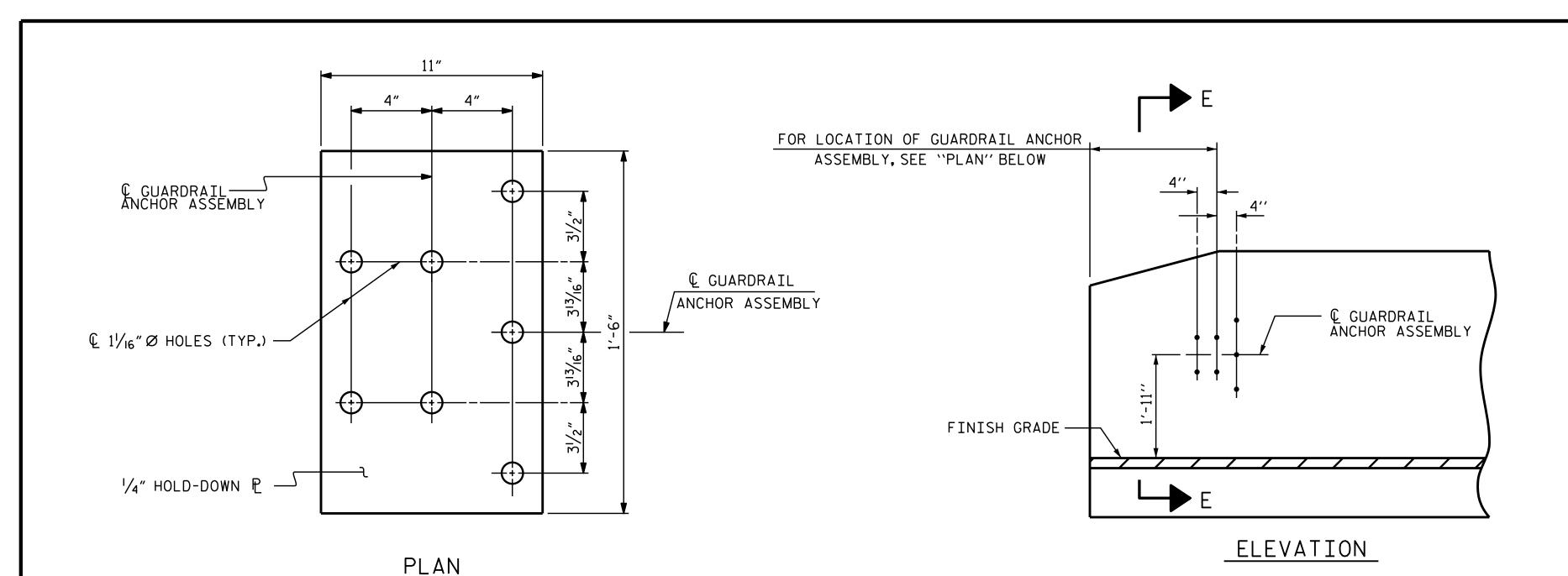
RALEIGH

3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT

B04B5A4F2FAD484							
9/26/2017			SHEET NO.				
DOCUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-9
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			19

VERTICAL CONCRETE BARRIER RAIL DETAILS

DRAWN BY: \_\_\_\_\_\_B.A.DUKE DATE: 10-24-16
CHECKED BY: \_\_\_\_\_\_G.KOUCHEKI DATE: 11-17-16
DESIGN ENGINEER OF RECORD: \_\_\_\_\_D.SHACKLEFORD DATE: 12-2016



# NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 -  $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE  $7/8^{\prime\prime}$  Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

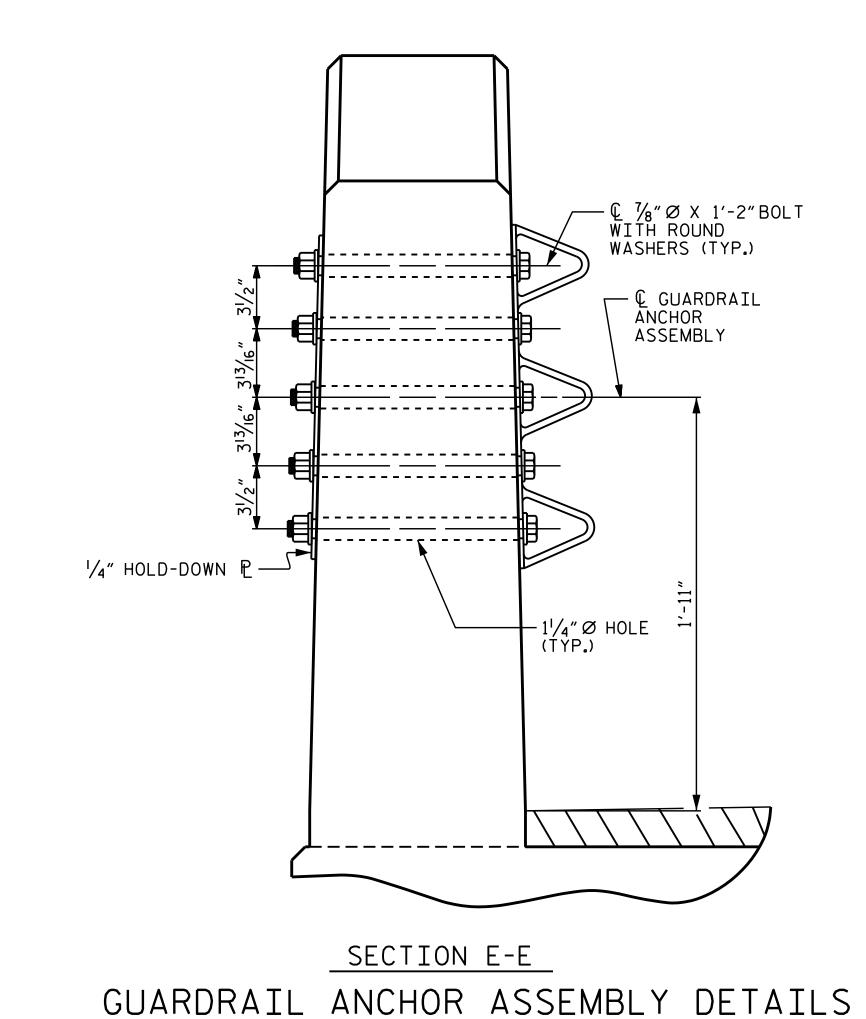
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

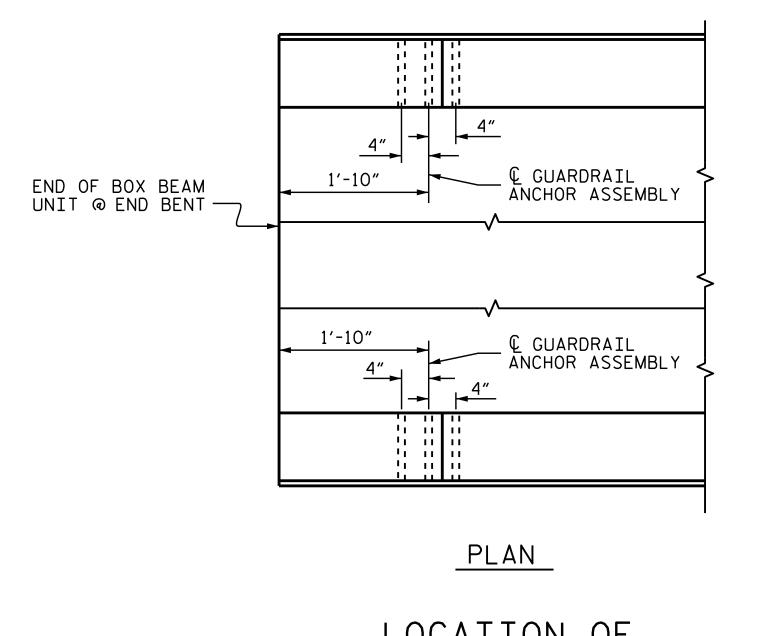
AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1  $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.







END OF BOX BEAM UNIT @ END BENT #1 — END OF BOX BEAM
UNIT @ END BENT #2

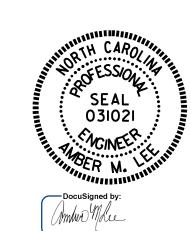
> SKETCH SHOWING POINTS OF ATTACHMENT

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

B-4978 PROJECT NO.\_ WILKES COUNTY

12+36.12 -L-STATION:\_

STATE OF NORTH CAROLINA

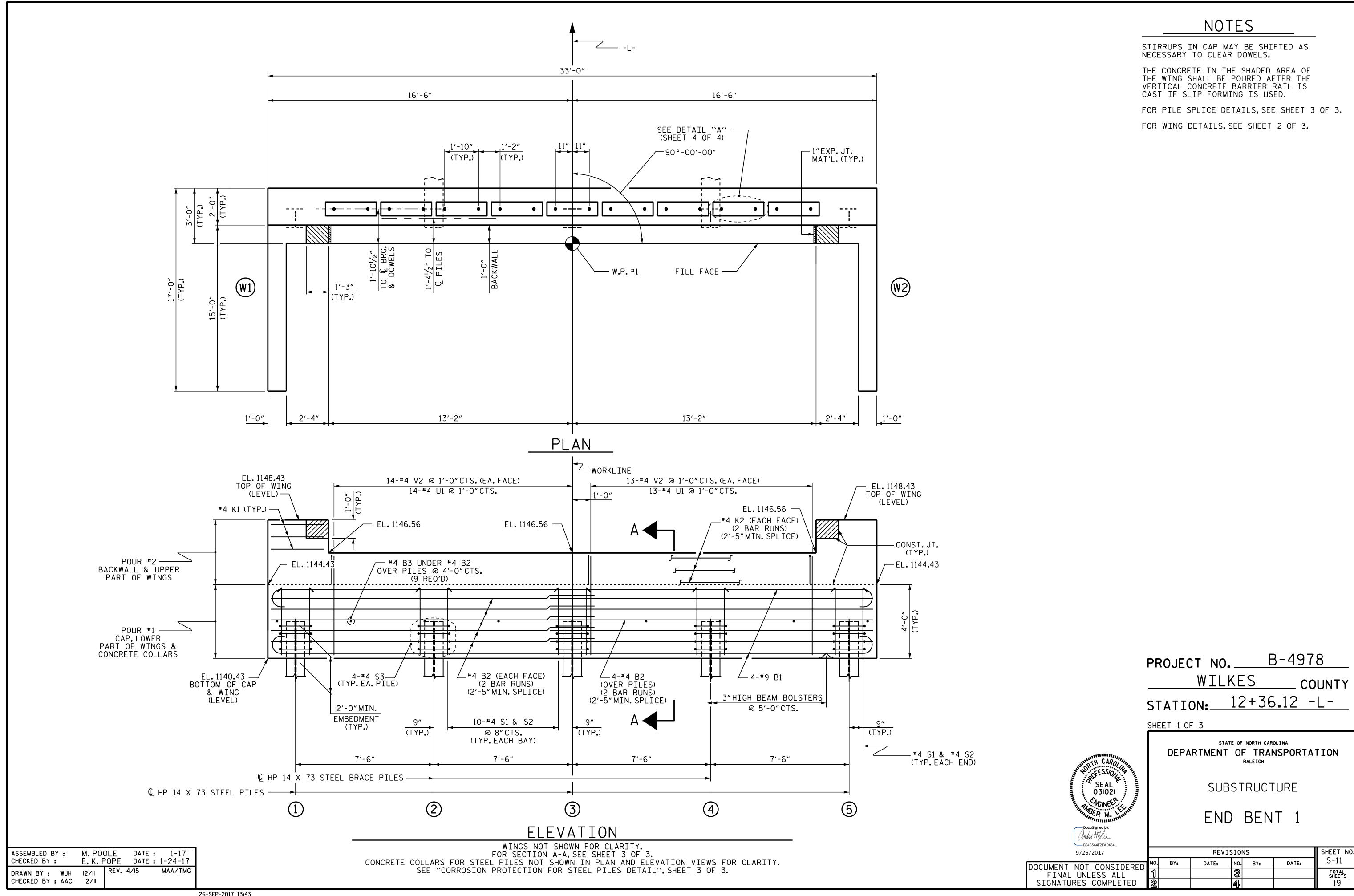


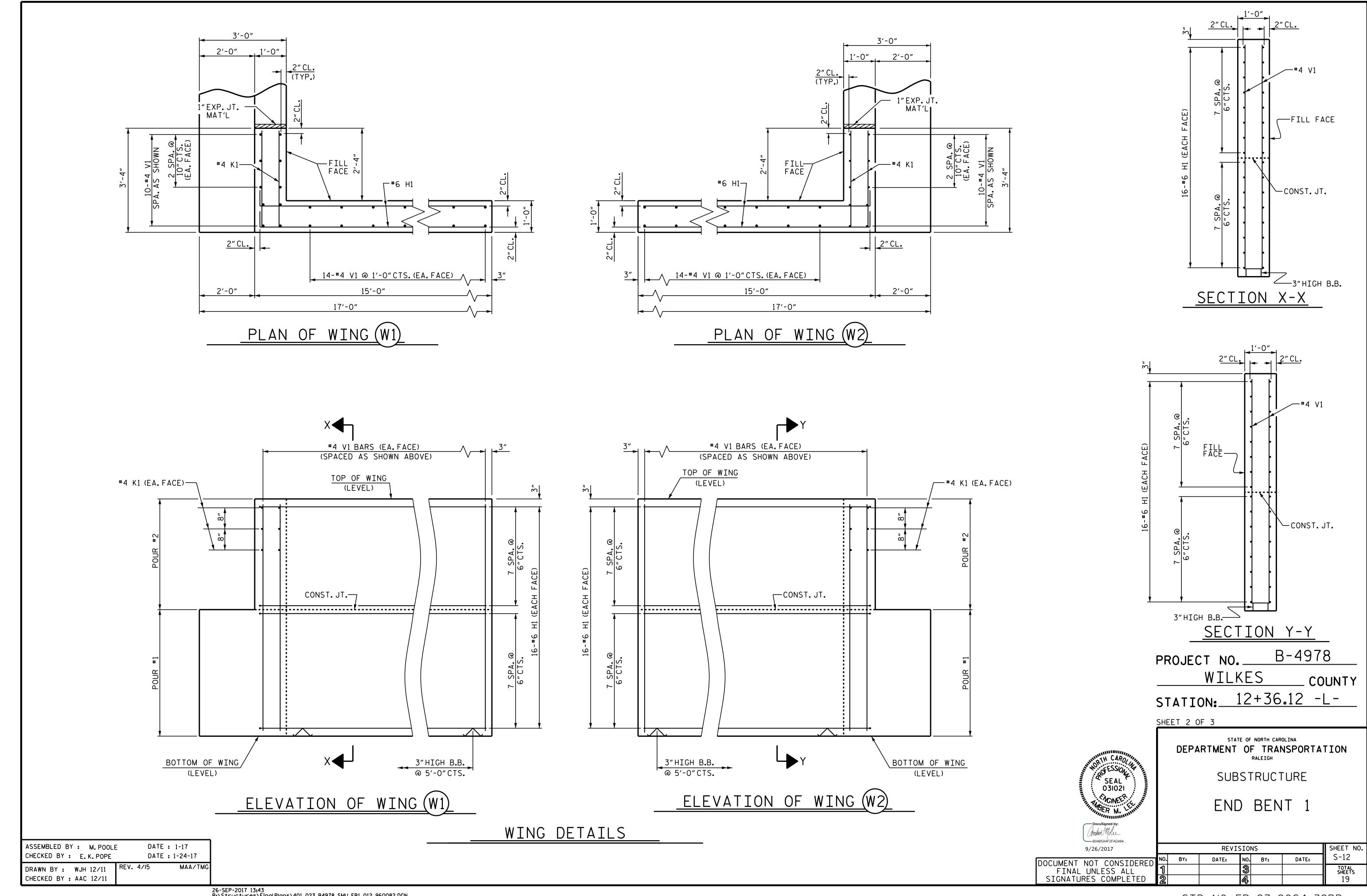
9/26/2017

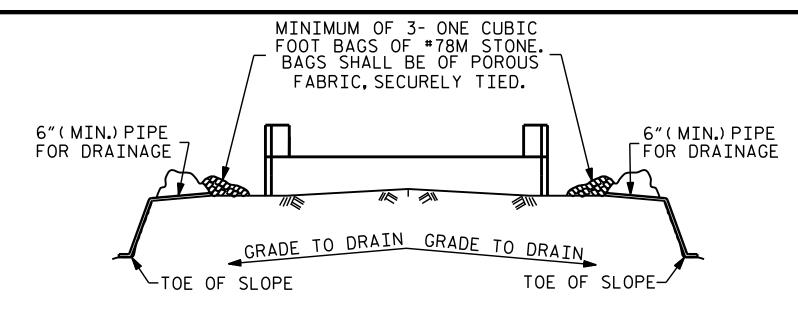
DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE BARRIER RAIL

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

REVISIONS S-10 DATE:





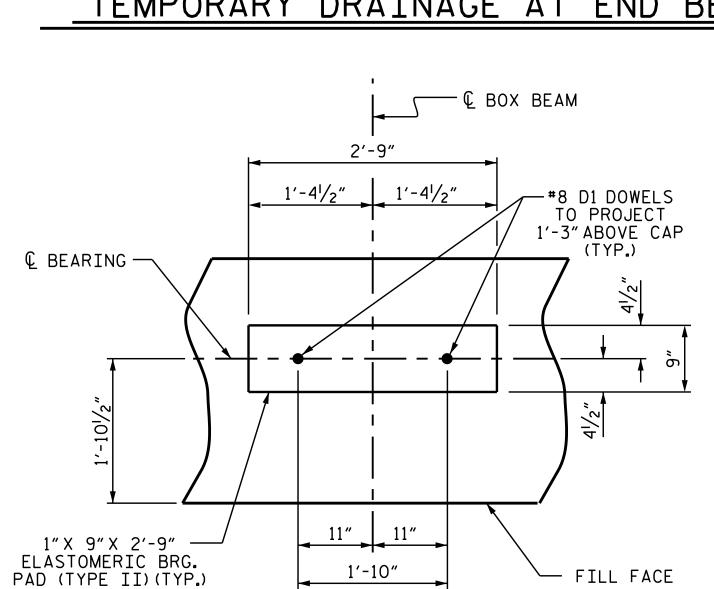


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

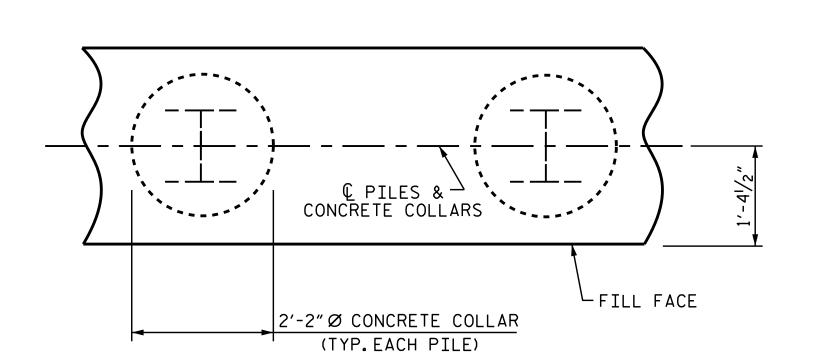
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

# TEMPORARY DRAINAGE AT END BENT



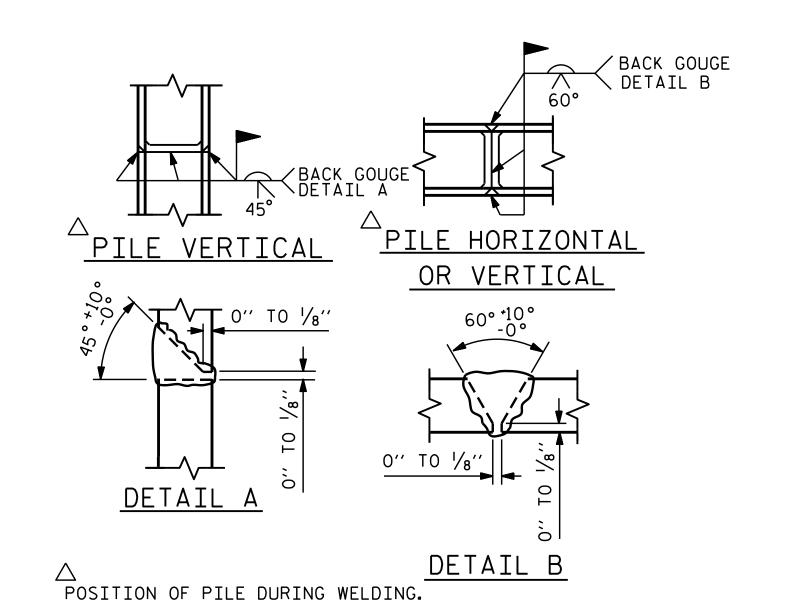
DETAIL "A" (END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

1'-10"

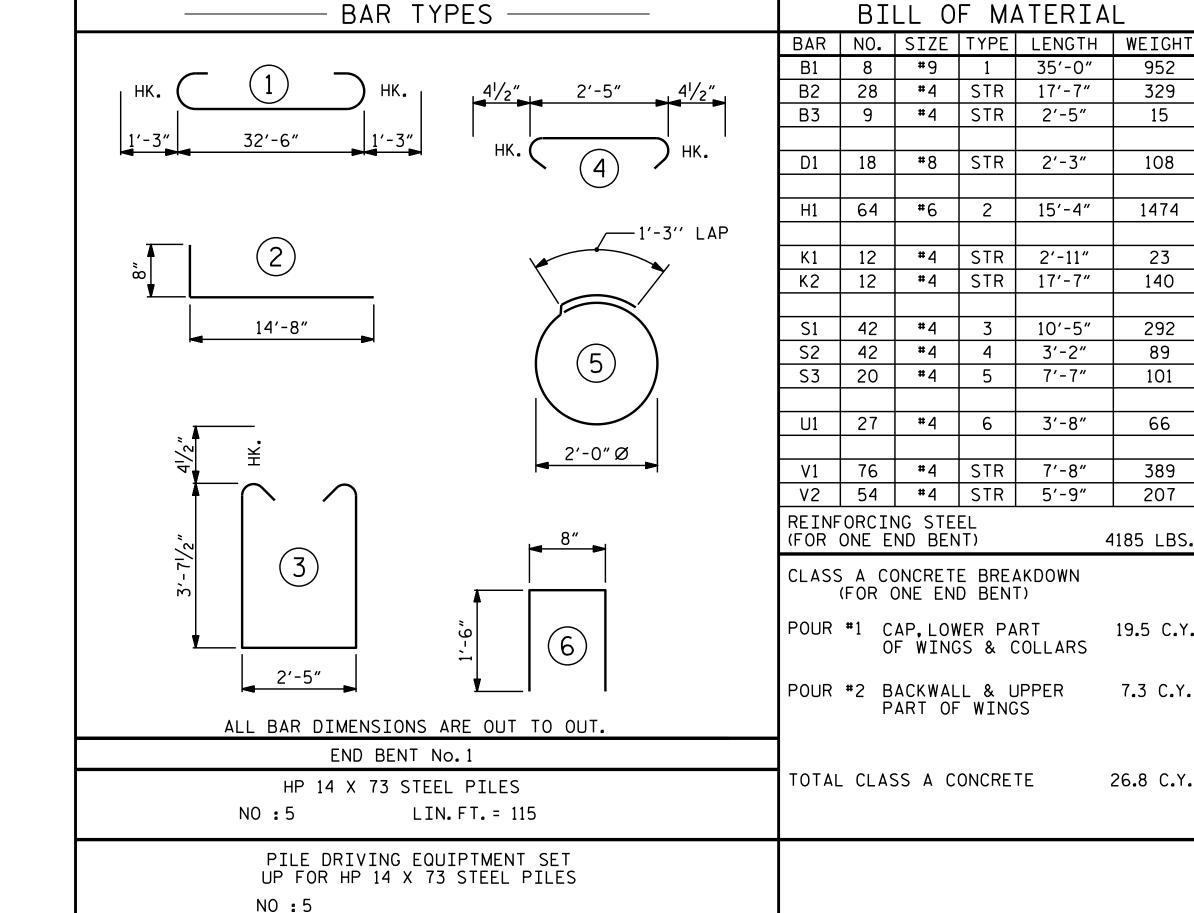


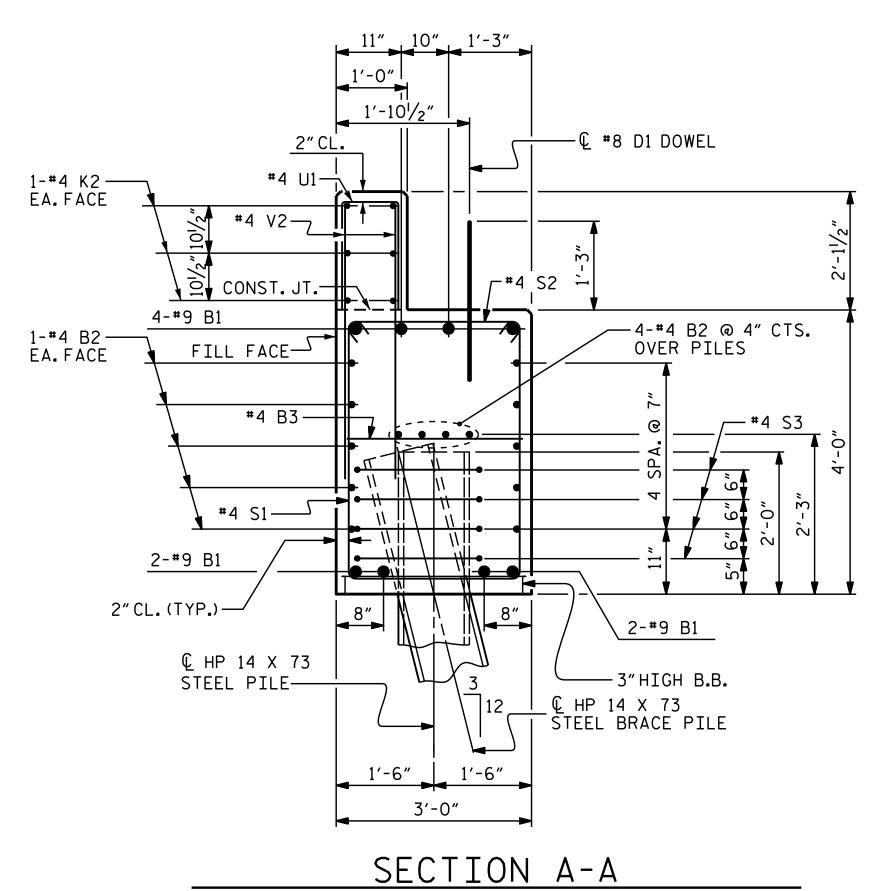
PLAN

CONCRETE — COLLAR BOTTOM OF CAP © HP 14 X 73 STEEL PILE 2'-2" ELEVATION CORROSION PROTECTION FOR STEEL PILES DETAIL



PILE SPLICE DETAILS





(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

031021 CONEE 'Amhor Male 9/26/2017

B-4978 PROJECT NO. WILKES COUNTY 12+36.12 -L-STATION:\_\_

BILL OF MATERIAL

9

12

42

42

#4

#4

#4

(FOR ONE END BENT)

OF WINGS & COLLARS

PART OF WINGS

#4 | STR | 17'-7"

#4 | STR | 2'-5"

#8 | STR | 2'-3"

#6 | 2 | 15'-4"

#4 | STR | 2'-11"

#4 | STR | 17'-7"

3 | 10'-5"

5 7'-7"

6 | 3'-8"

3'-2"

1 35'-0"

329

15

108

1474

23

140

292

89

101

66

389

207

4185 LBS.

19.5 C.Y.

7.3 C.Y.

26.8 C.Y.

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

END BENT 1

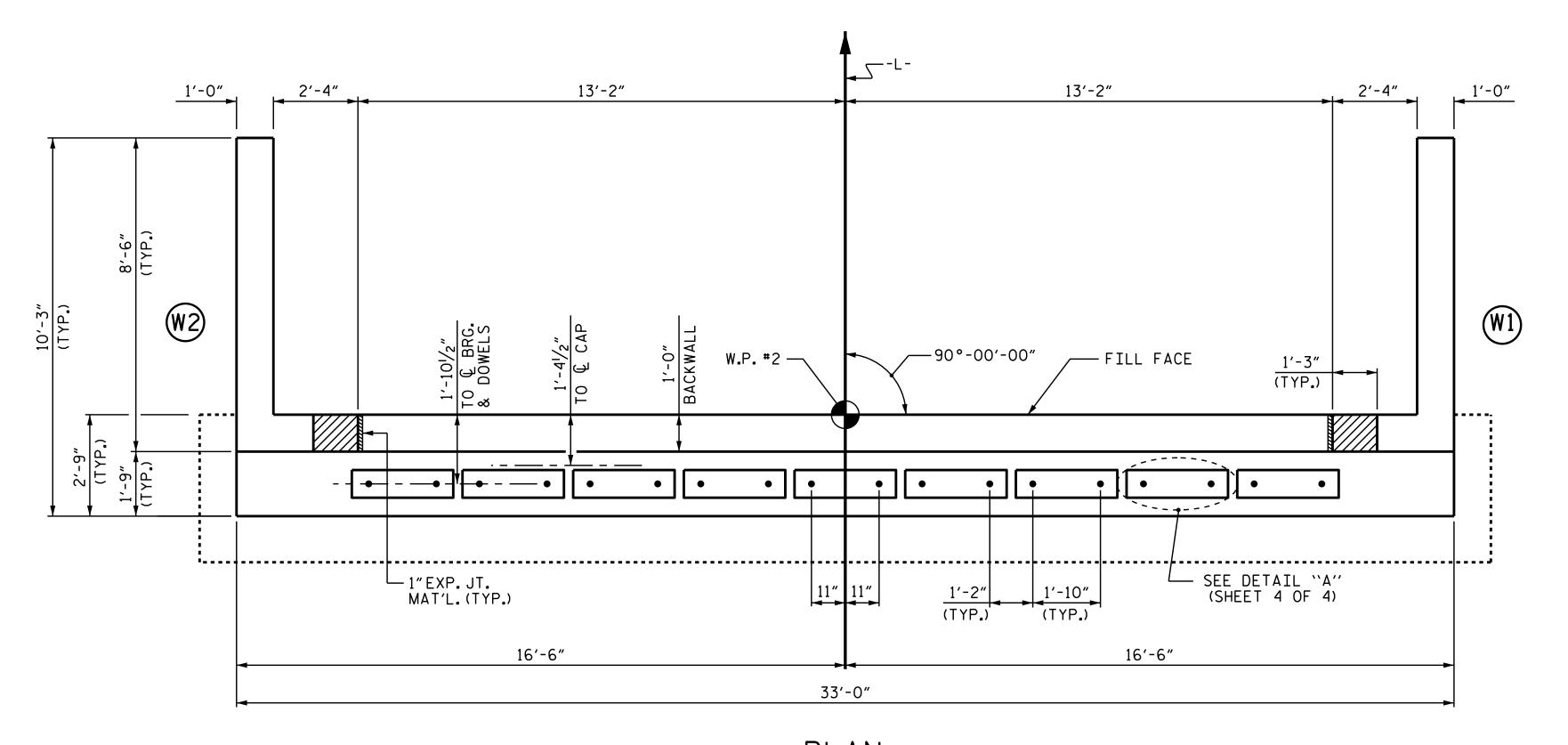
SHEET NO REVISIONS S-13 DATE: DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED BY:

ASSEMBLED BY : M. POOLE DATE : 1-17 DATE : 1-24-17 CHECKED BY : E.K. POPE REV. 8/14 DRAWN BY: WJH 12/11 CHECKED BY : AAC 12/11

26-SEP-2017 13:43 R:\Structures\FinalPlans\401\_025\_B4978\_SMU\_EB1\_013\_960082.DGN

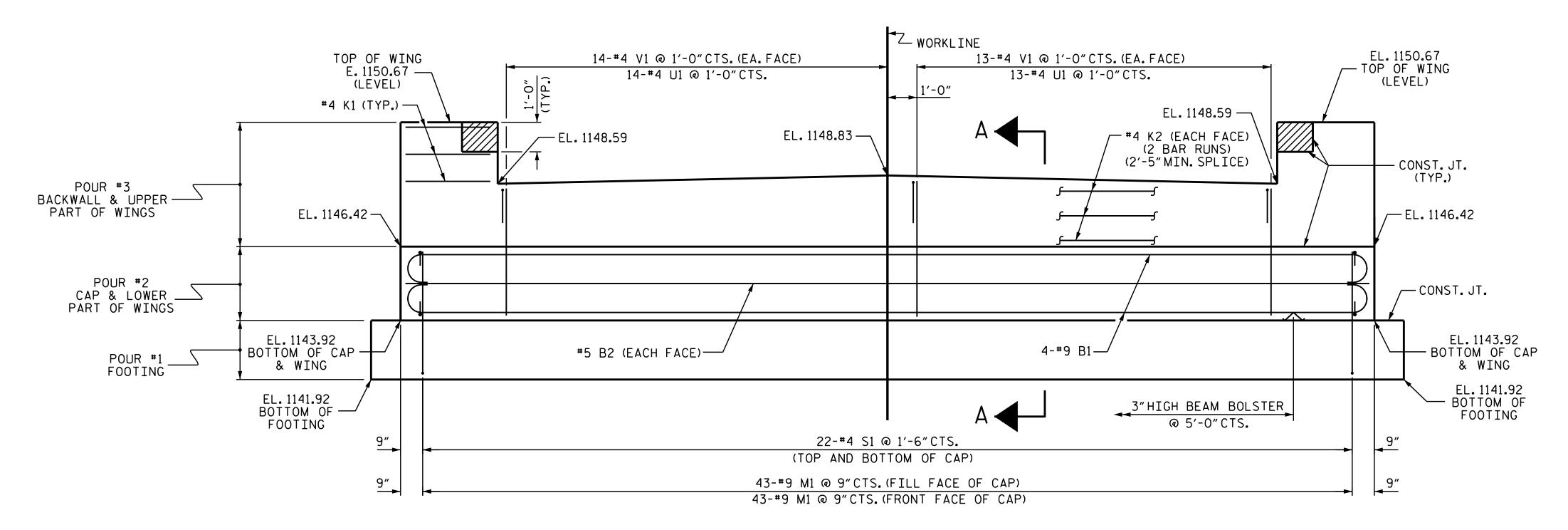
FILL FACE

STD. NO. EB\_27\_90S4\_39BB



PLAN

FOR FOOTING DIMENSIONS AND REINFORCING STEEL, SEE SHEET 2 OF 4



ELEVATION

WINGS NOT SHOWN FOR CLARITY.
FOR SECTION A-A, SEE SHEET 4 OF 4.

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR WING DETAILS, SEE SHEET 3 OF 4.

INSTALL THE 4"Ø DRAIN PIPE THROUGH WINGWALL AS REQUIRED FOR SUBREGIONAL TIER BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WINGWALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

PROJECT NO. B-4978

WILKES COUNTY

STATION: 12+36.12 -L-

SHEET 1 OF 4

SEAL 031021

CHOINEER

Amber Male

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

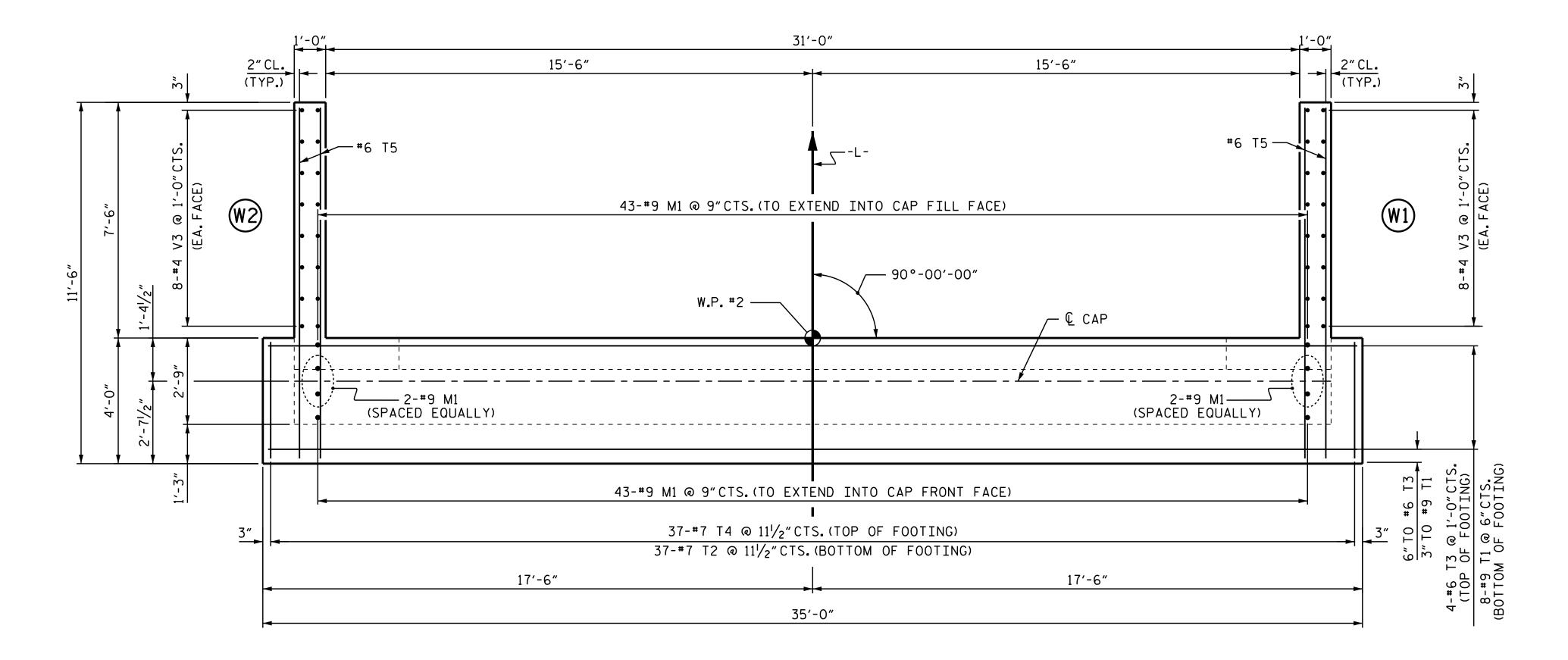
RALEIGH

SUBSTRUCTURE

END BENT 2

BOUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2 REVISIONS SHEET NO. S-14

DRAWN BY: \_\_\_\_\_\_H.T.BARBOUR DATE: 10-10-17
CHECKED BY: \_\_\_\_\_\_H.T.BARBOUR DATE: 10-11-17
DESIGN ENGINEER OF RECORD: \_\_\_\_\_A.SORSENGINH DATE: 10-11-17



PLAN OF FOOTING

PROJECT NO. B-4978

WILKES COUNTY

STATION: 12+36.12 -L-

SEAL O31021

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE

END BENT 2

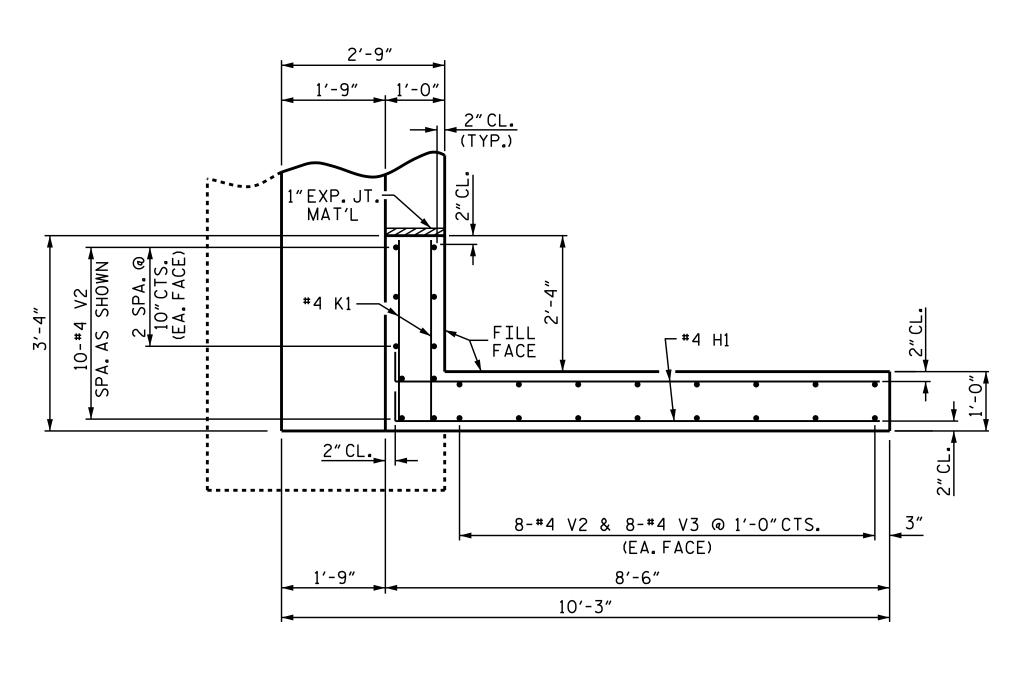
SHEET NO. S-15

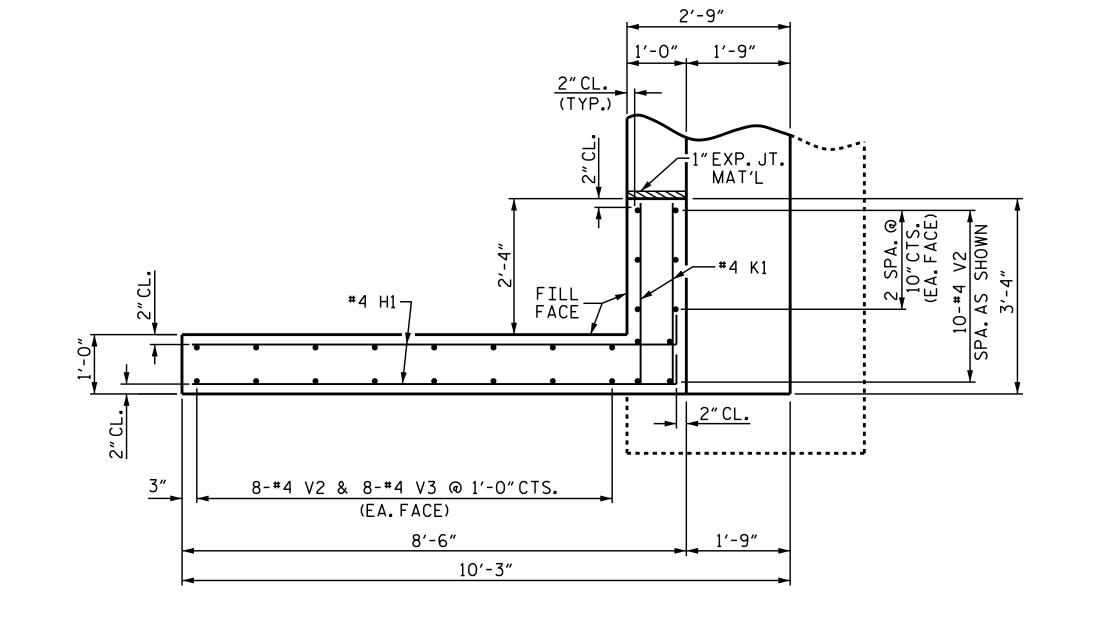
DATE:

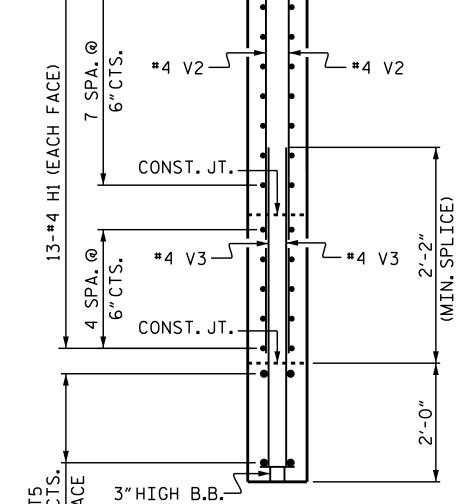
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2

SHEET 2 OF 4

DRAWN BY: A. SORSENGINH DATE: 10-9-17
CHECKED BY: H. T. BARBOUR DATE: 10-11-17
DESIGN ENGINEER OF RECORD: A. SORSENGINH DATE: 10-11-17

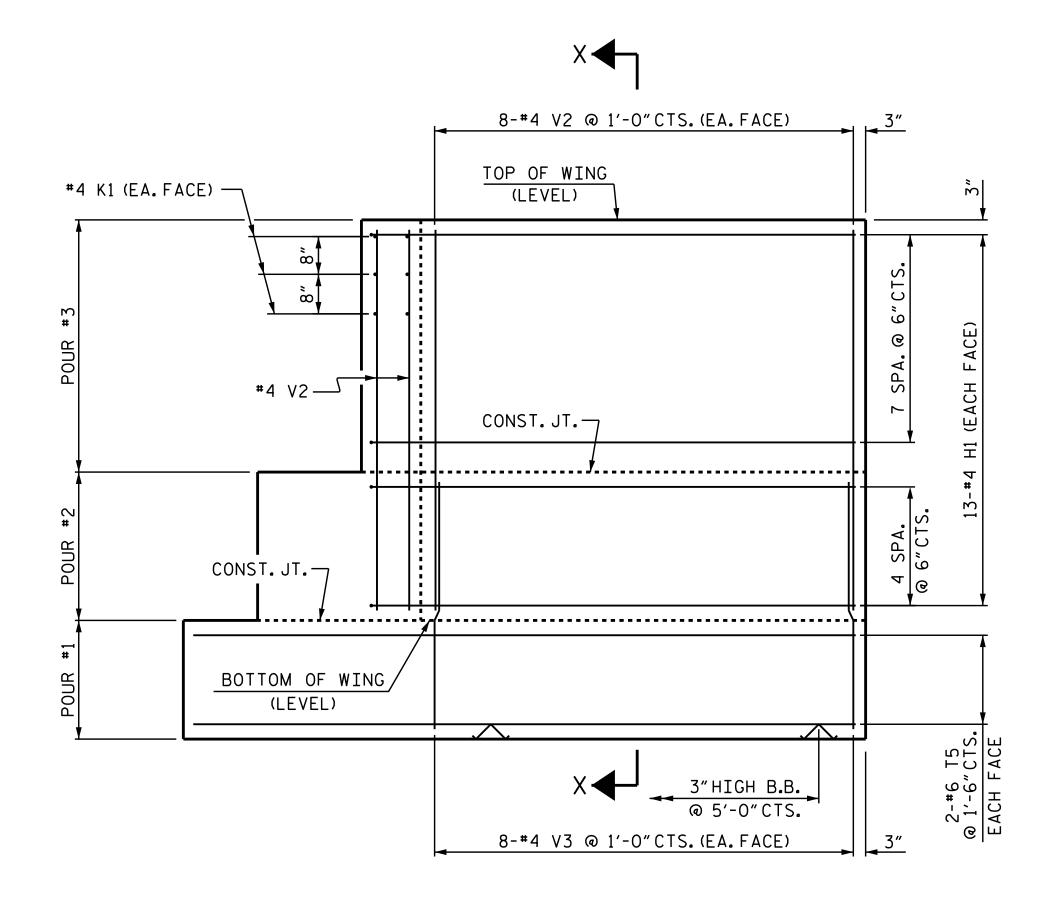






SECTION X-X

PLAN OF WING (W1)



8-#4 V2 @ 1'-0"CTS.(EA.FACE) TOP OF WING (LEVEL) - #4 K1 (EA. FACE) CONST. JT. CONST. JT. BOTTOM OF WING (LEVEL) 8-#4 V3 @ 1'-0"CTS.(EA.FACE)

ELEVATION OF WING (W2)

PLAN OF WING (W2)

STATION: 12+36.12 -L-SHEET 3 OF 4 STATE OF NORTH CAROLINA

WILKES

PROJECT NO.\_\_\_\_

B-4978

\_ COUNTY

SHEET NO.

S-16

SUBSTRUCTURE

DEPARTMENT OF TRANSPORTATION

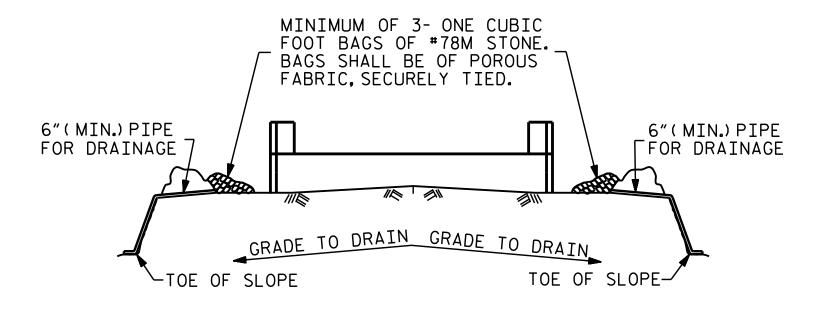
END BENT 2

REVISIONS DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

ELEVATION OF WING (W1)

A. SORSENGINH \_ DATE : <u>10-11-17</u> H. T. BARBOUR

DESIGN ENGINEER OF RECORD: A. SORSENGINH DATE: 10-11-17

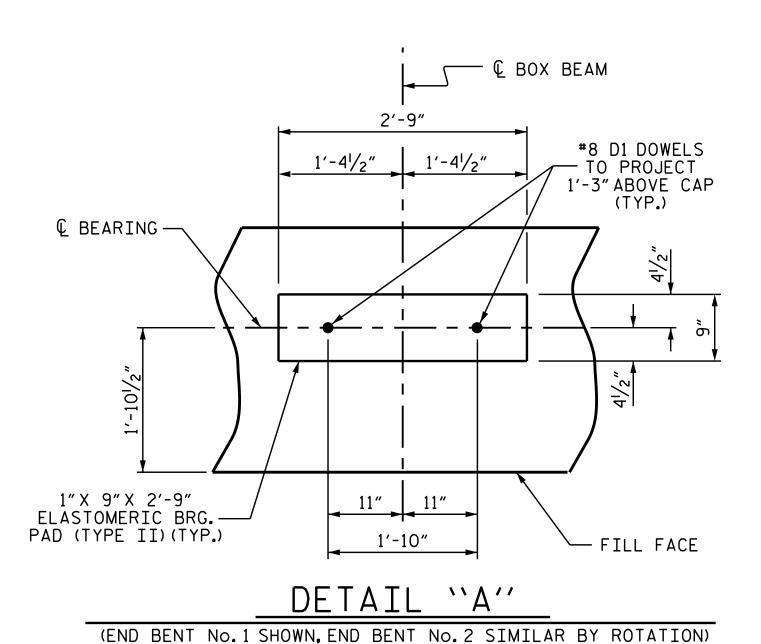


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# TEMPORARY DRAINAGE AT END BENT



SECTION A-A

4'-0"

 $1'-4\frac{1}{2}''$   $1'-4\frac{1}{2}''$ 

2'-9"

3"HIGH B.B. —

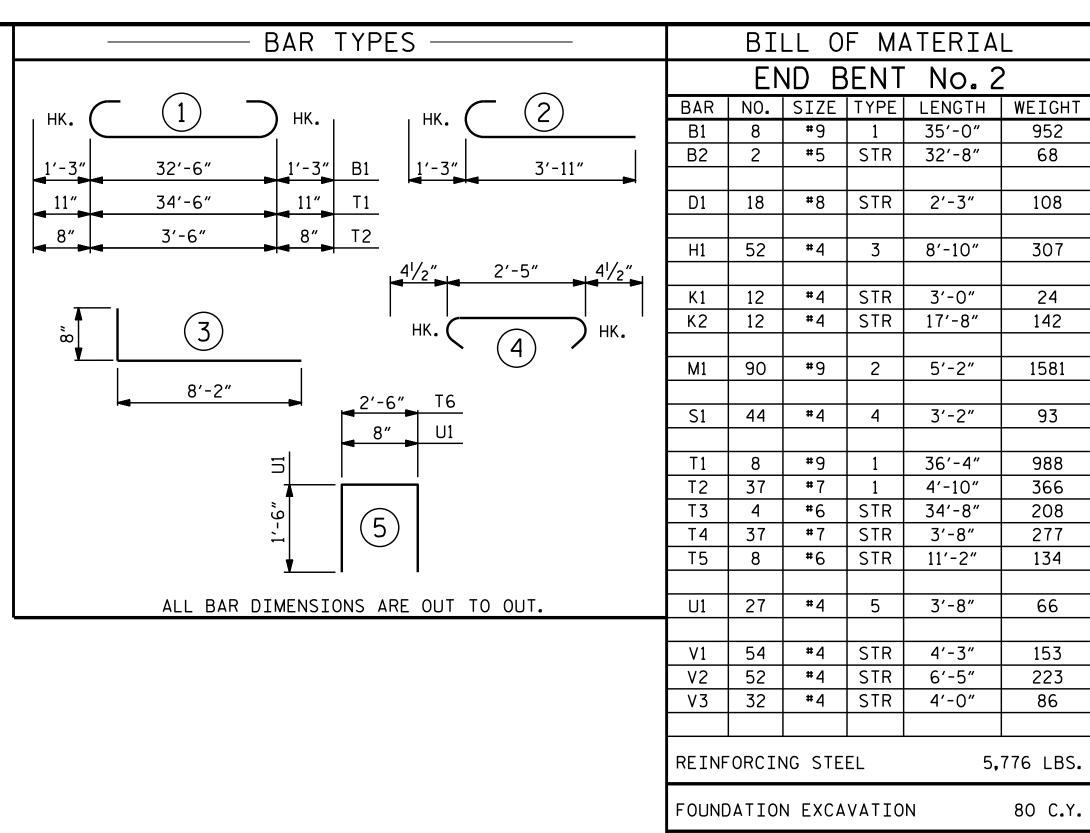
#7 T2 —

2'-71/2"

1'-3"

\_(TYP.)

11"



1'-10" 1'-0" -#4 U1 CLASS A CONCRETE BREAKDOWN #4 K2 (EA.FACE) 2"CL. POUR #1 FOOTING 11.5 C.Y. (TYP.) **⊢** #4 V1 #4 K2 (EA.FACE) POUR #2 CAP & LOWER 9.8 C.Y. PART OF WINGS #4 K2 (EA.FACE) POUR #3 BACKWALL & UPPER 5.6 C.Y. PART OF WINGS 4-#9 B1 — CONST. JT. #4 S1 ← FILL FACE TOTAL CLASS A CONCRETE 26.9 C.Y. #5 B2 (EA. FACE) **~#**8 D1 2" CL. (TYP.) ─ 3″HIGH B.B. — CONST. JT. +6 T3 (TYP.)

PROJECT NO. B-4978

WILKES COUNTY

STATION: 12+36.12 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE

END BENT 2

BOUND BY: DATE: NO. BY: DATE: NO. BY: DATE: SIGNATURES COMPLETED

REVISIONS

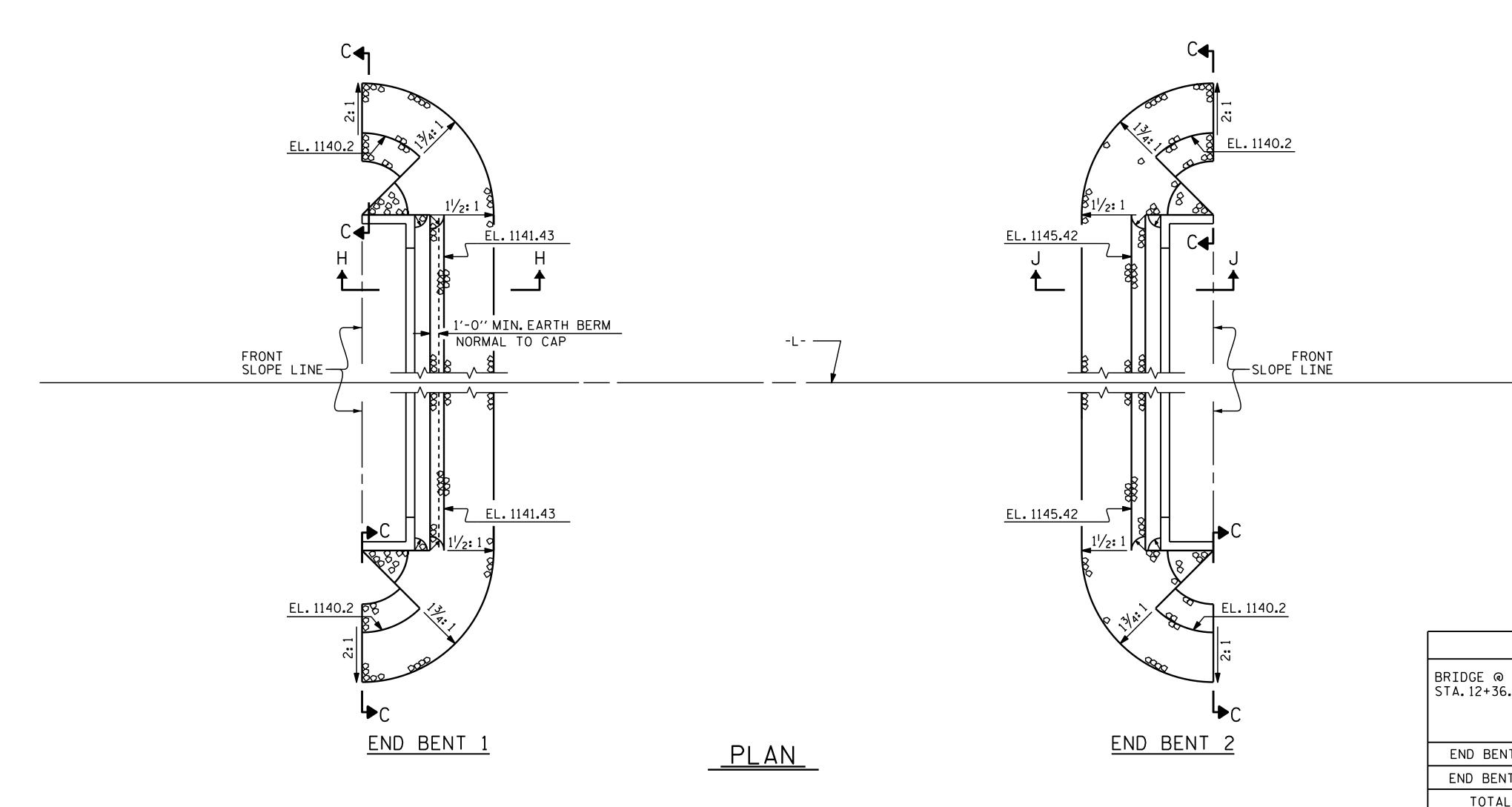
REVISIONS

SHEET NO. S-17

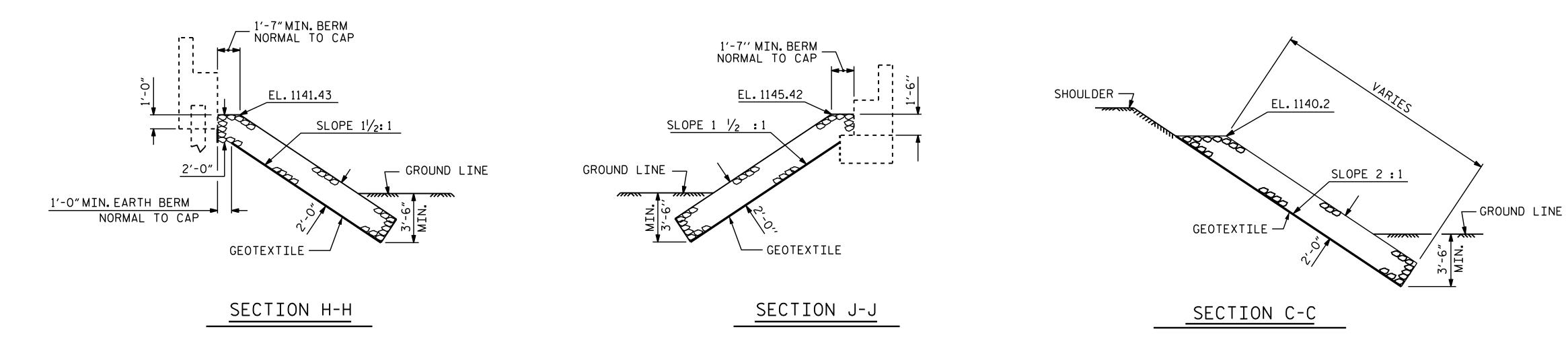
TOTAL SHEETS

19

DRAWN BY: A. SORSENGINH DATE: 10-10-17
CHECKED BY: H. T. BARBOUR DATE: 10-11-17
DESIGN ENGINEER OF RECORD: A. SORSENGINH DATE: 10-11-17



ESTIMATED QUANTITIES							
BRIDGE @ STA.12+36.12 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE					
	TONS	SQUARE YARDS					
END BENT 1	75	80					
END BENT 2	150	165					
TOTAL	225	245					



B-4978 PROJECT NO.\_\_\_\_ WILKES \_ COUNTY

STATION: 12+36.12 -L-

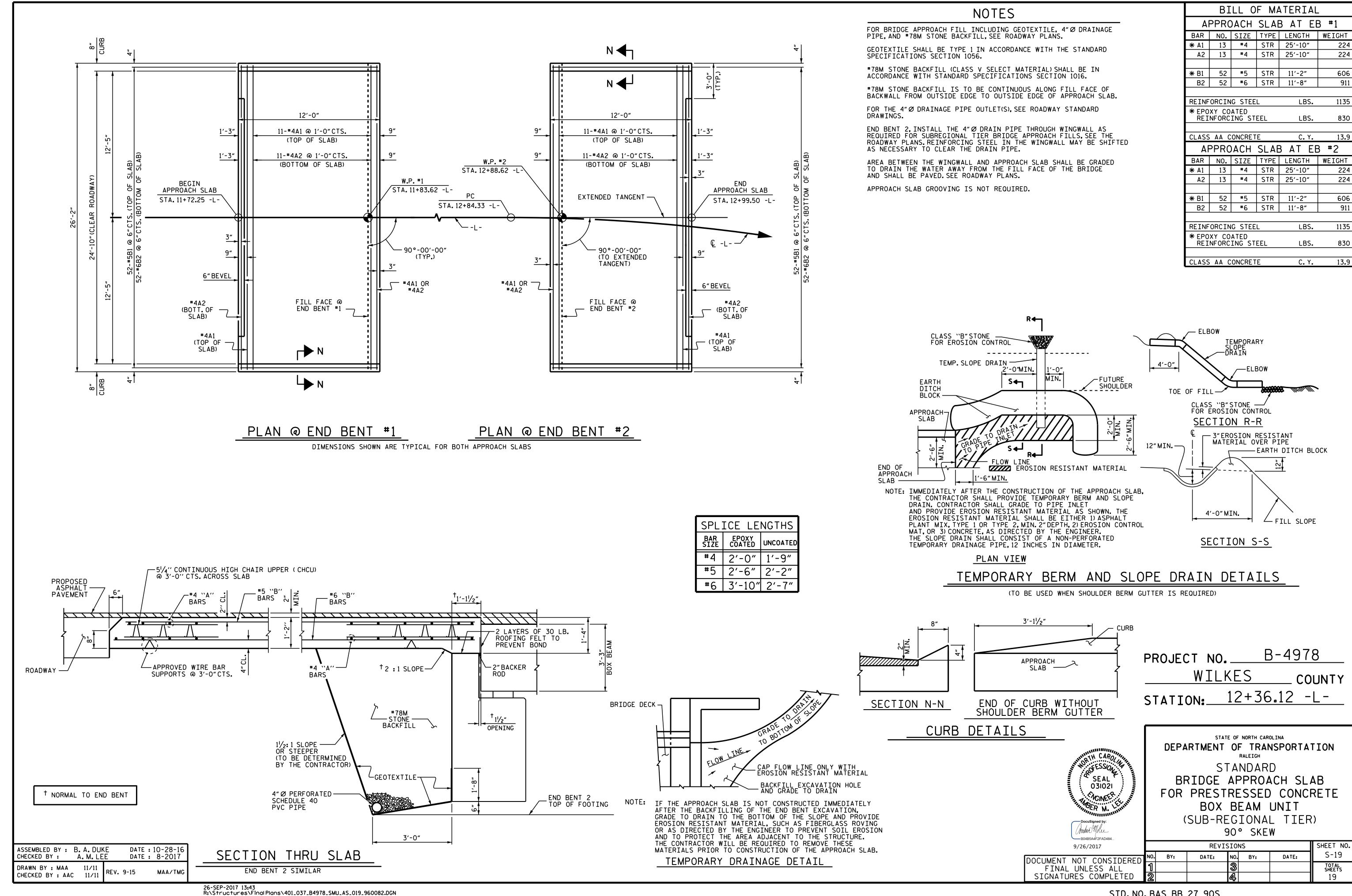
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH STANDARD

-RIP RAP DETAILS-

MWU Mice B04B5A4F2FAD484... 10/12/2017 SHEET NO. REVISIONS S-18 DATE: DATE: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

ASSEMBLED BY: T.L. AVERETTE DATE: 7/2017 CHECKED BY: E.K. POPE DATE: 7/2017 REV. 5/I/06R REV. I0/I/II REV. I2/2I/II TLA/GM MAA/GM MAA/GM DRAWN BY: REK 1/84 CHECKED BY: RDU 1/84

SEAL 031021



# STANDARD NOTES

# DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W -	27,000 LBS. PER SO. IN.
- AASHTO M270 GRADE 50 -	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.

## MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

# CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.
SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

# REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

# HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

# SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990