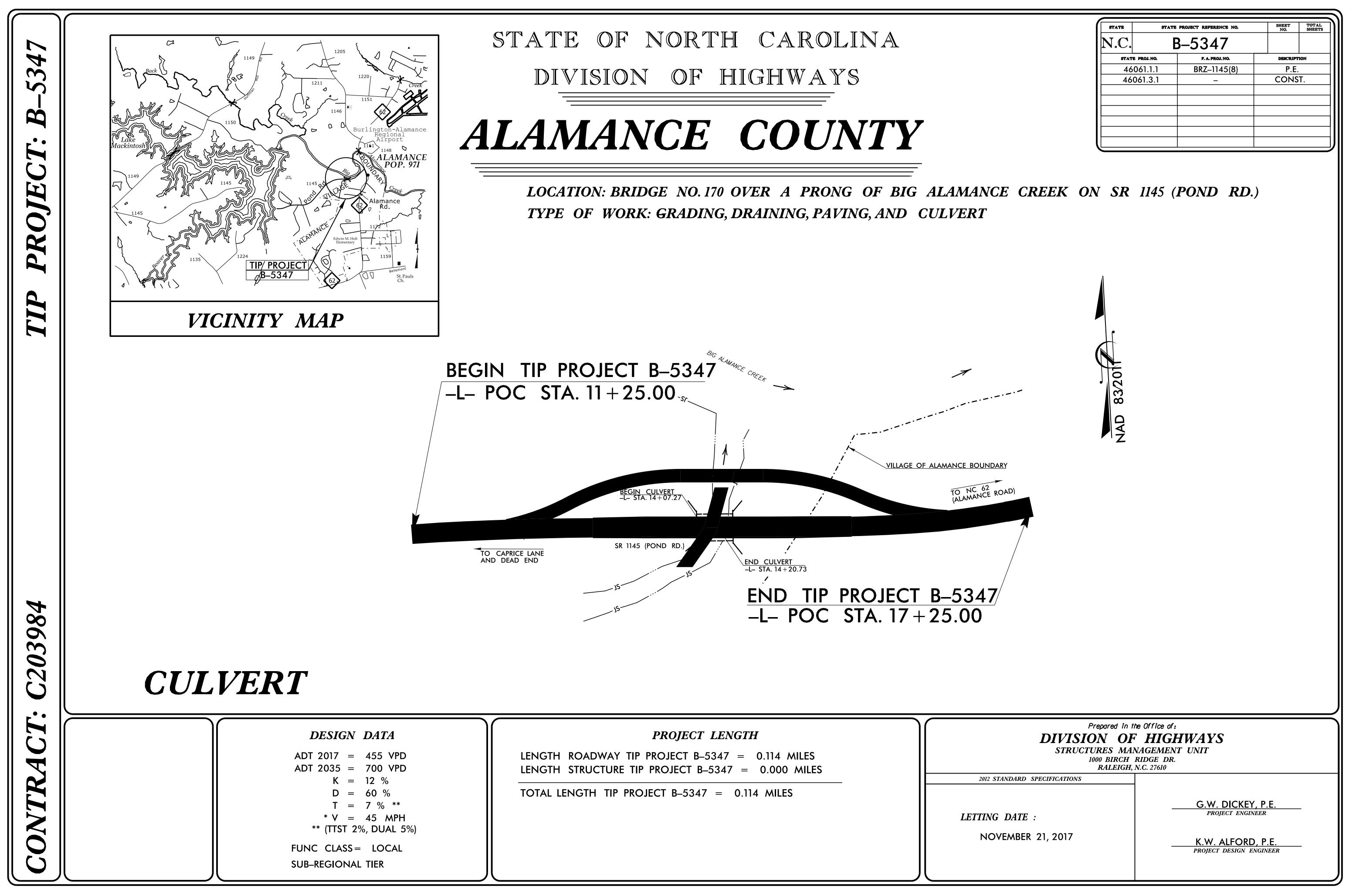
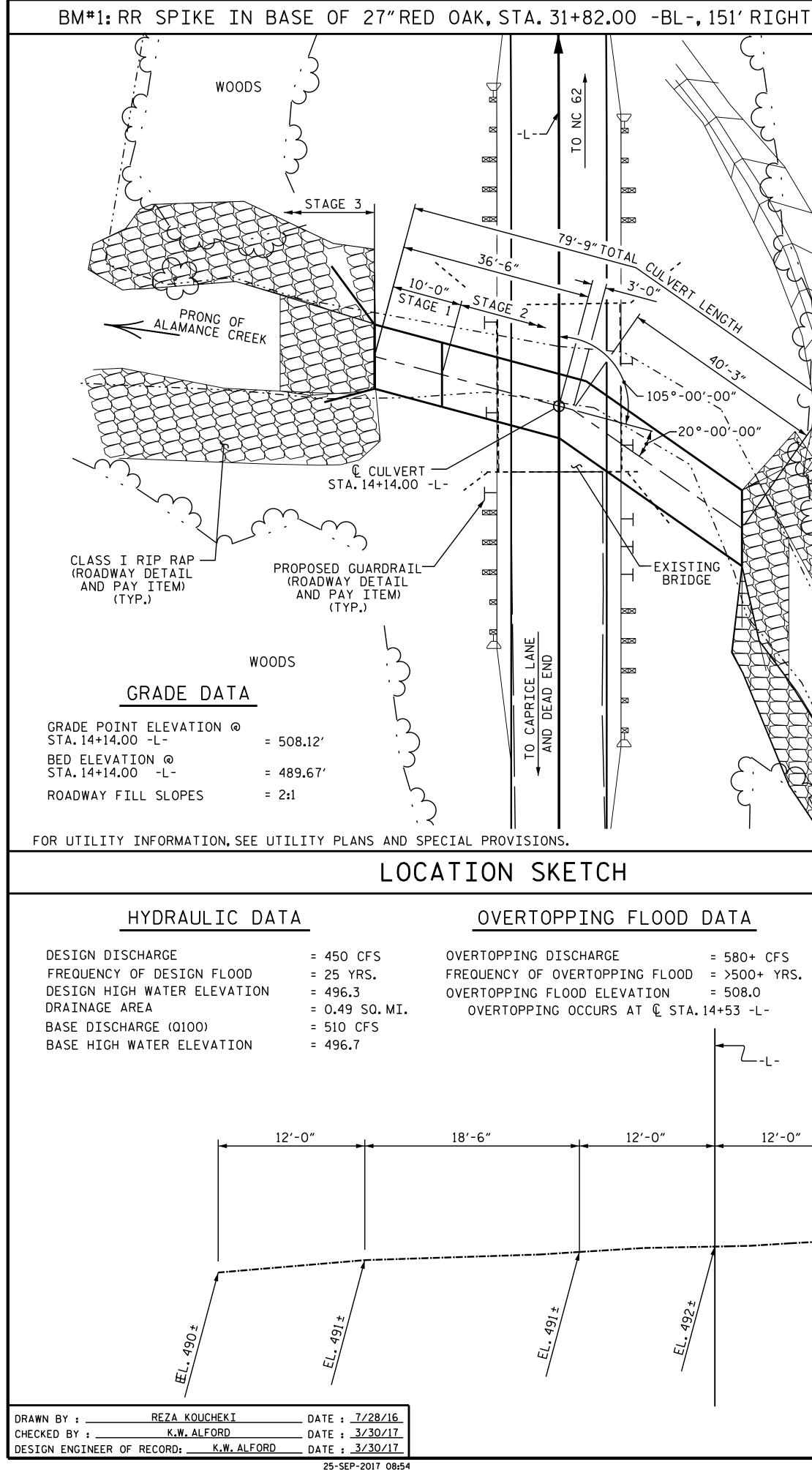
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<b>PROJECT LENGTH</b>	
LENGTH ROADWAY TIP PROJECT B $-5347 = 0.114$ MILES LENGTH STRUCTURE TIP PROJECT B $-5347 = 0.000$ MILES	
$\frac{1}{2} = \frac{1}{2} = \frac{1}$	2012 STAN
TOTAL LENGTH TIP PROJECT $B_{-5347} = 0.114$ MILES	
	LETTING 1
	NOVE

STATE         STATE PROJECT REFERENCE NO.         SHEET NO.         TOTAL SHEETS								
N.C.	B-5347							
STAT	B PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	10N				
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GHT,ELEV.515.66		NOTES	
WOODS	<ul> <li>A 3 FOOT STRIP OF FILTER FABRIC FACE OF THE WING COVERING THE ENJOINT.</li> <li>3"Ø WEEP HOLES INDICATED TO BE SPECIFICATIONS.</li> <li>CONCRETE IN CULVERTS TO BE POURE STAGE I</li> <li>1. FLOOR SLAB INCLUDING 4" OF AT OUTLET END.</li> <li>2. THE REMAINING PORTIONS OF BY ROOF SLAB AND HEADWALL</li> <li>STAGE II</li> <li>1. THE INLET WING FOOTINGS AN FLOOR SLAB INCLUDING 4" OF</li> <li>2. THE REMAINING PORTIONS OF HEIGHT FOLLOWED BY ROOF SI STAGE III</li> <li>1. THE OUTLET WING FOOTINGS SI WALLS.</li> <li>2. THE REMAINING PORTIONS OF THE RESIDENT ENGINEER SHALL CHEC STAKING IT OUT TO MAKE CERTAIN CARE OF THE FILL.</li> <li>AT THE CONTRACTORS OPTION, HE MA REINFORCING STEEL IN THE INTERIC LOWER WALL CONSTRUCTION JOINT. TI PROVIDED IN THE SPLICE LENGTH CH WEIGHT OF STEEL DUE TO THE SPLIC CONTRACTOR.</li> <li>DIMENSIONS FOR WING LAYOUT AS W STEEL EMBEDDED IN BARREL ARE SHO INASMUCH AS THE PAINT SYSTEM ON CONTAINS LEAD, THE CONTRACTOR'S A IO7-1 OF THE STANDARD SPECIFICATION</li> </ul>	SEE STANDARD NOTE SHEET. SHALL BE ATTACHED TO THE FILL NTIRE LENGTH OF THE EXPANSION IN ACCORDANCE WITH THE ED IN THE FOLLOWING ORDER: ALL VERTICAL WALLS FOR 10' THE WALLS FULL HEIGHT FOLLOWED S. ND THE REMAINING PORTION OF THE ALL VERTICAL WALLS. THE WALLS AND INLET WINGS FULL LAB AND HEADWALLS. INCLUDING 4" OF ALL VERTICAL THE OUTLET WINGS FULL HEIGHT. EX THE LENGTH OF CULVERT BEFORE THAT IT WILL PROPERLY TAKE Y SPLICE THE VERTICAL DR FACE OF EXTERIOR WALL ABOVE HE SPLICE LENGTH SHALL BE AS HART SHOWN ON THE PLANS. EXTRA CES SHALL BE PAID FOR BY THE VELL AS ADDITIONAL REINFORCING DWN ON WING SHEET. I THE EXISTING STRUCTURAL STEEL ATTENTION IS DIRECTED TO ARTICLE IONS. ANY COSTS RESULTING FROM E OR FEDERAL REGULATIONS IALS CONTAINING LEAD BASED PAINT ICE FOR TREMOVAL OF EXISTING	PERMIT COND FOR CULVERT PLANS. FOR GROUT FO FOR MAINTENA FOR SUBMITTA FOR FALSEWOF FOR CRANE SA THE EXISTING CLEAR ROADWA
		TOTAL STRUCTURE QU	ANTITIES
		CLASS A CONCRETE STAGE I STAGE II STAGE III TOTAL	9.1 C.Y.
FS YRS.		REINFORCING STEEL STAGE I STAGE II STAGE III TOTAL	2,540 LBS. 16,651 LBS. 639 LBS. 19,830 LBS.
<u>''-0" 8'-0'</u>	<u>4'-0" 5'-0" 12'-0"</u>	FOUNDATION COND.MAT'L. STAGE I STAGE II TOTAL	<u>13</u> TONS <u>90</u> TONS <u>103</u> TONS
/		CULVERT EXCAVATION	LUMP SUM

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REMOVAL OF EXISTING STRUCTURE

ASBESTOS ASSESSMENT

LUMP SUM

LUMP SUM

NTRACTOR SHALL FILL THE PROPOSED CULVERT WITH NATIVE AL TO A DEPTH OF 1 FOOT.NATIVE MATERIAL CONSISTS OF AL THAT IS EXCAVATED FROM THE STREAM OR FLOOD PLAIN AT DJECT SITE DURING CONSTRUCTION. NATIVE MATERIAL IS TO APPROVAL BY THE ENGINEER AND MAY BE SUBJECT TO CONDITIONS.

LVERT DIVERSION DETAILS & PAY ITEM, SEE EROSION CONTROL

OUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

INTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS.

BMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

LSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

ANE SAFETY, SEE SPECIAL PROVISIONS.

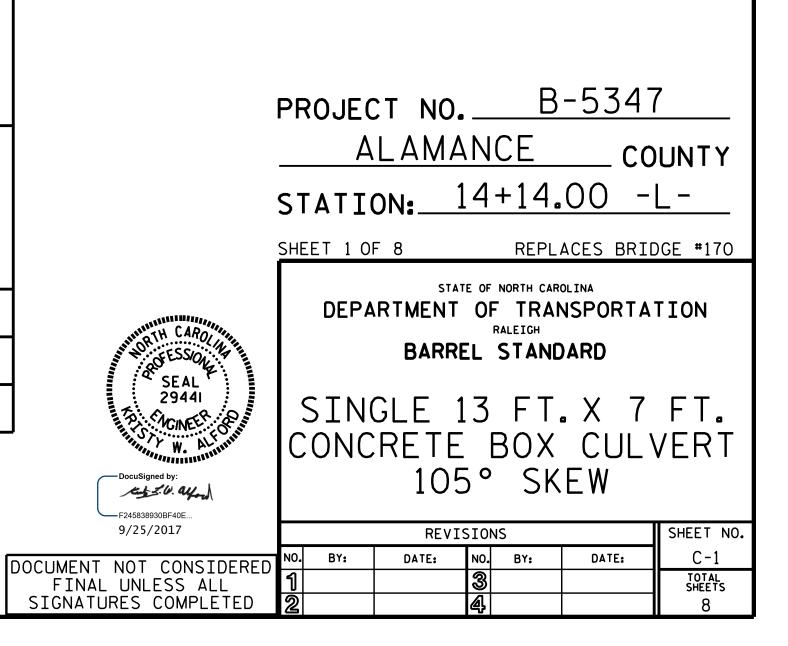
ISTING STRUCTURE CONSISTING OF ONE 35'-5"SPAN WITH A ROADWAY WIDTH OF 25'-O",WITH A TIMBER DECK ON I-BEAMS BER CAPS AND PILE END BENTS WITH STEEL PLANK BULKHEADS CATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE NG BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT.

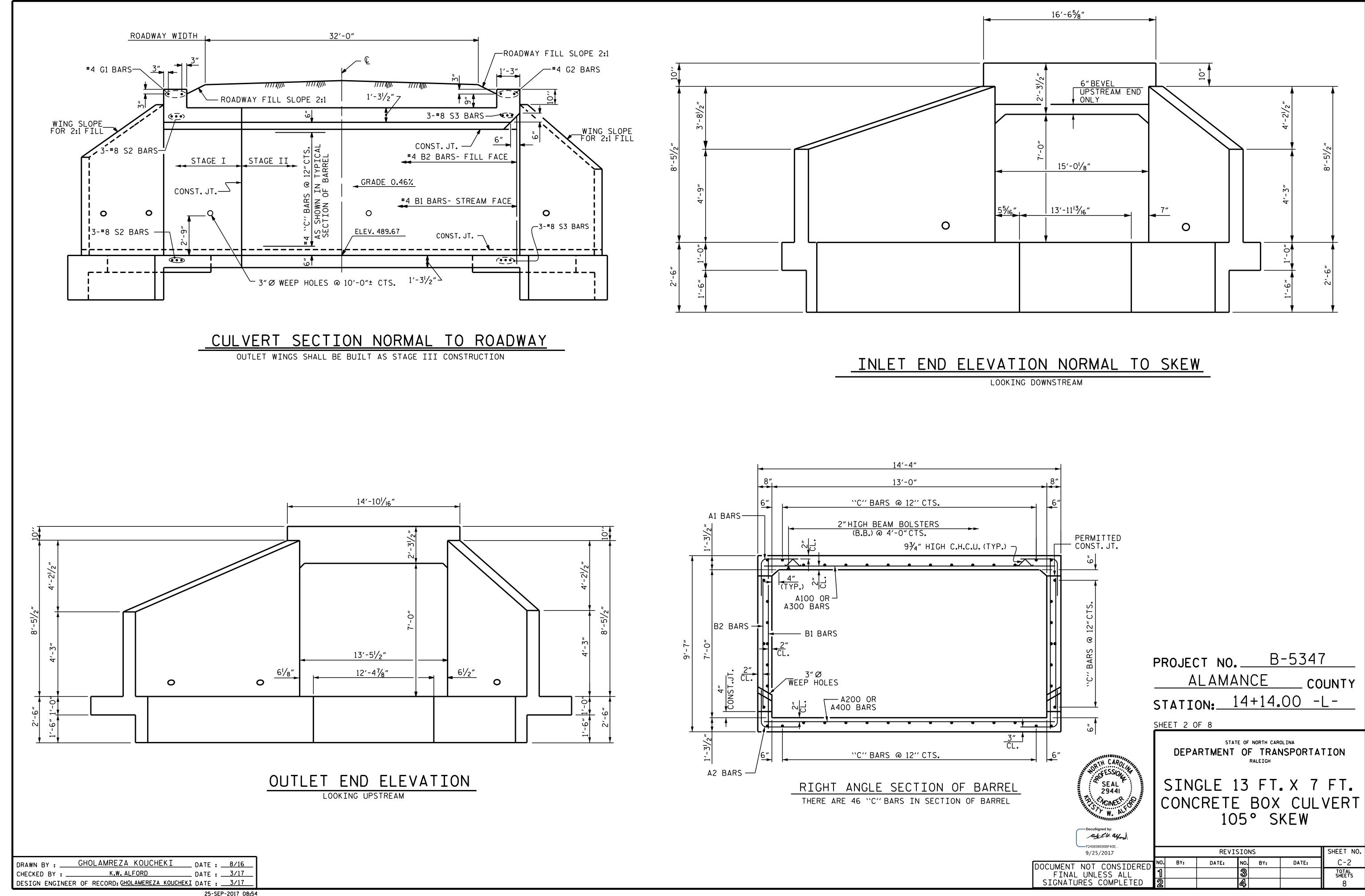
BSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS M THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION WN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CTOR SHALL HAVE NO CLAIMS WHATSOEVER AGAINST THE MENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST ED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE UCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT OJECT SITE.

L OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER REVENTS DEBRIS FROM FALLING INTO THE WATER.THE CTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE IDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD ICATIONS.

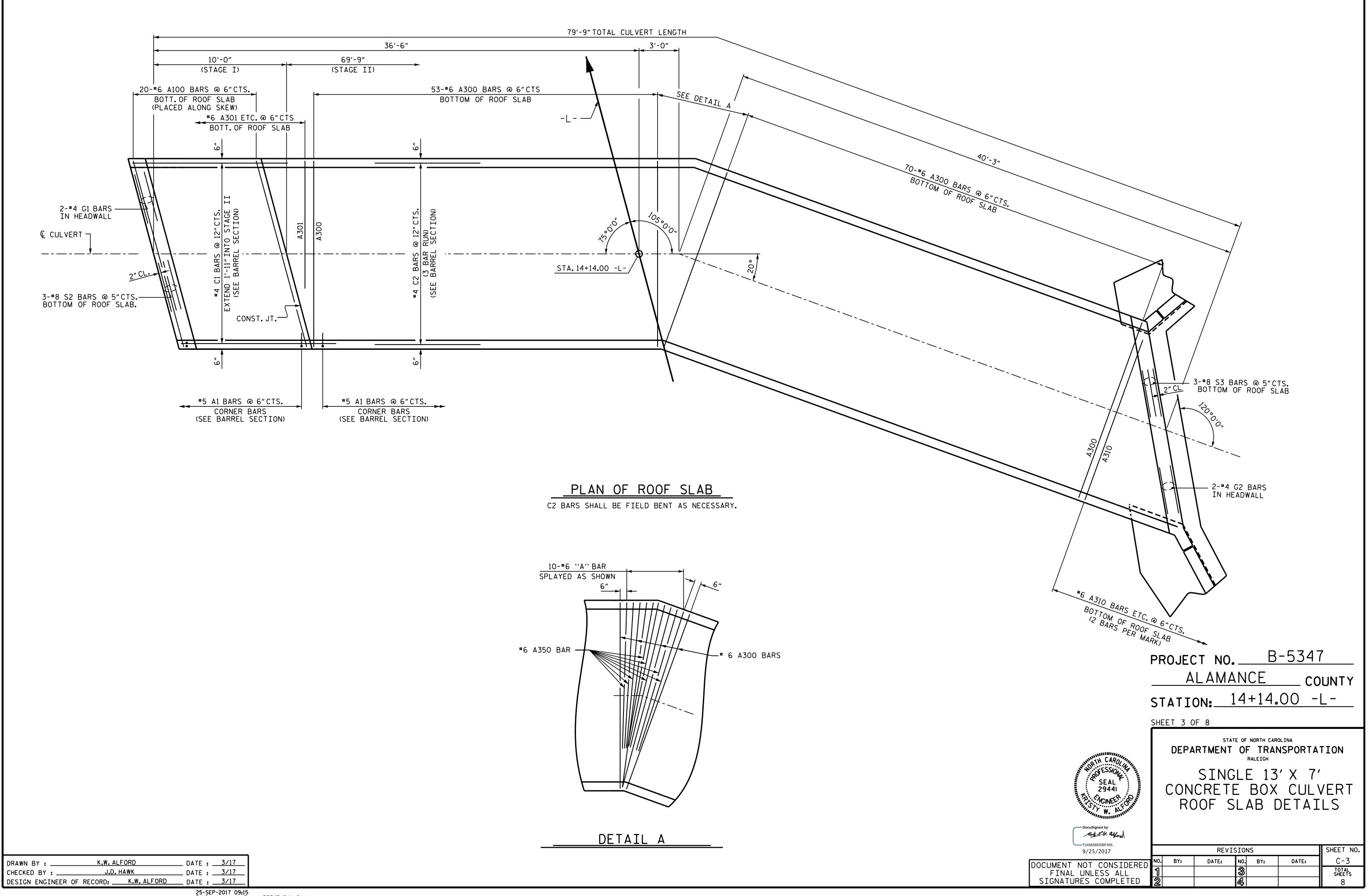
BESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION TES, SEE SPECIAL PROVISIONS.

I HEREBY CERTIFY THESE PLANS ARE THE AS BUILT PLANS



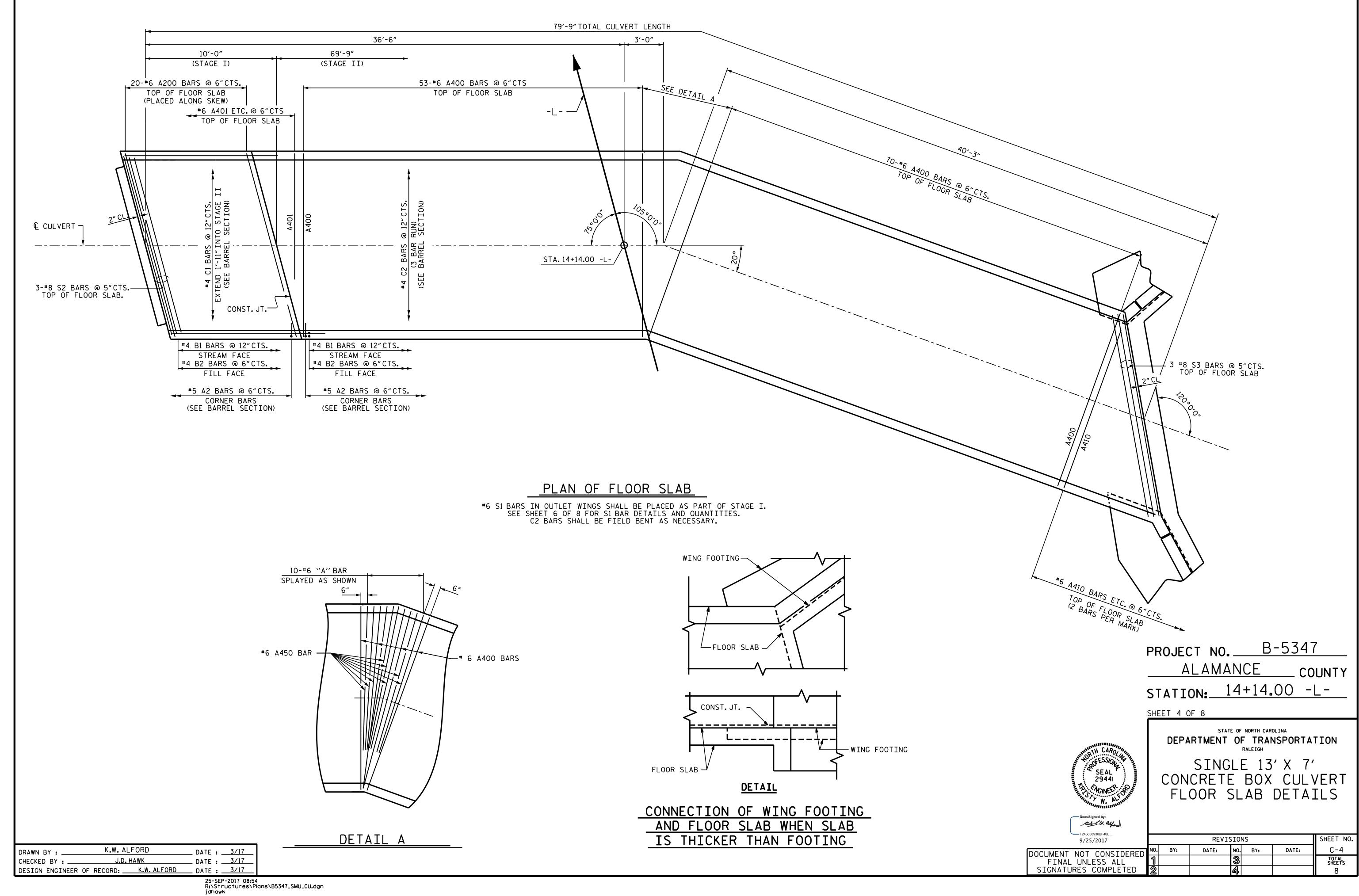


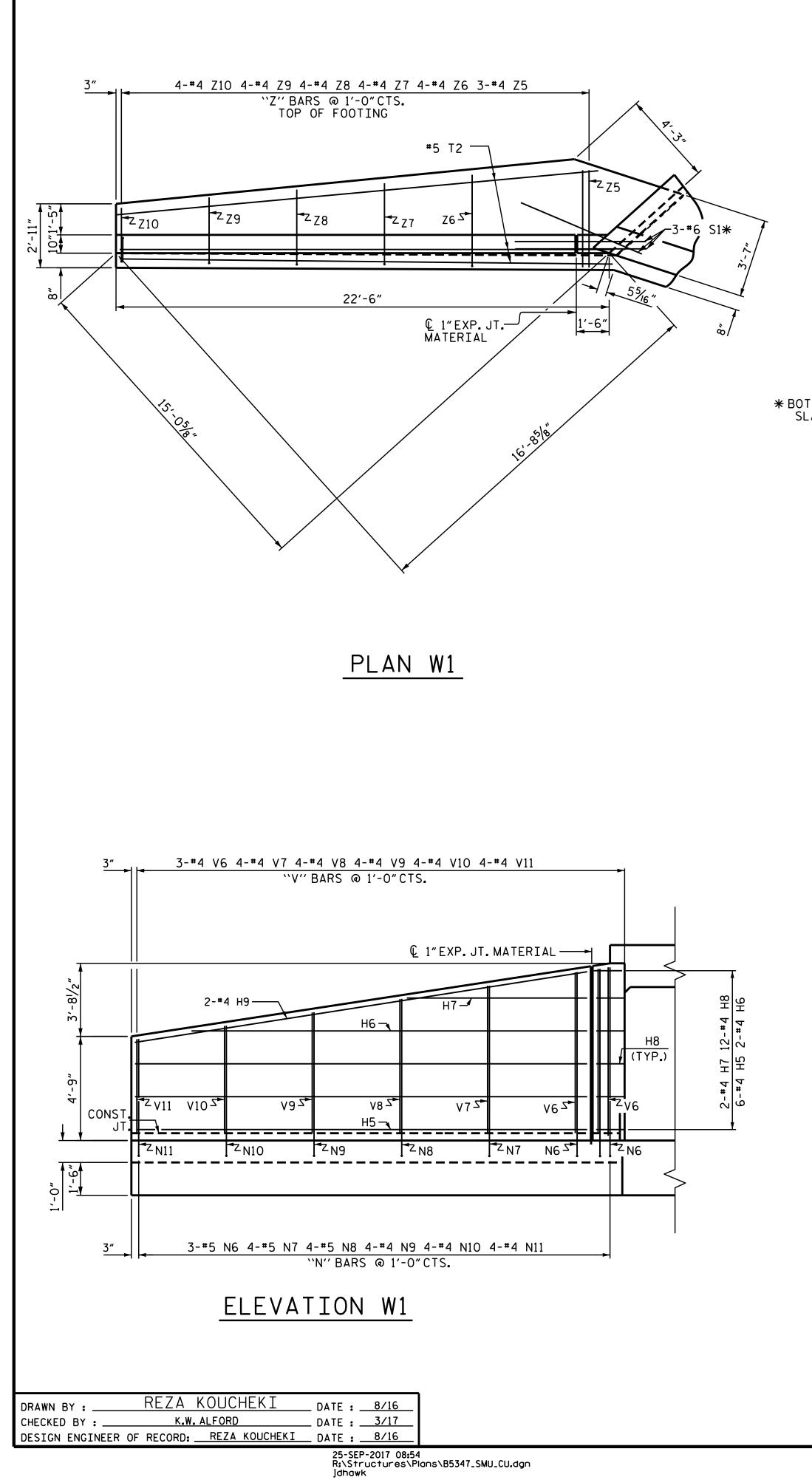
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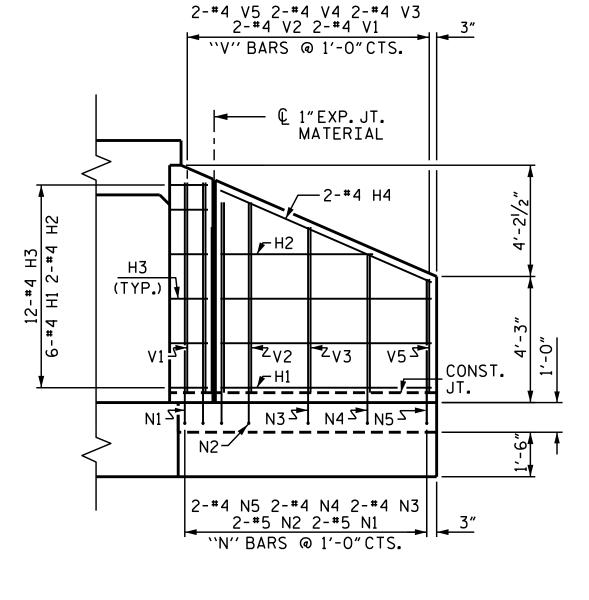
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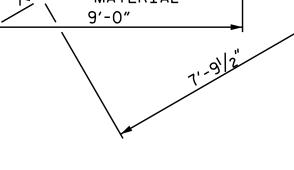




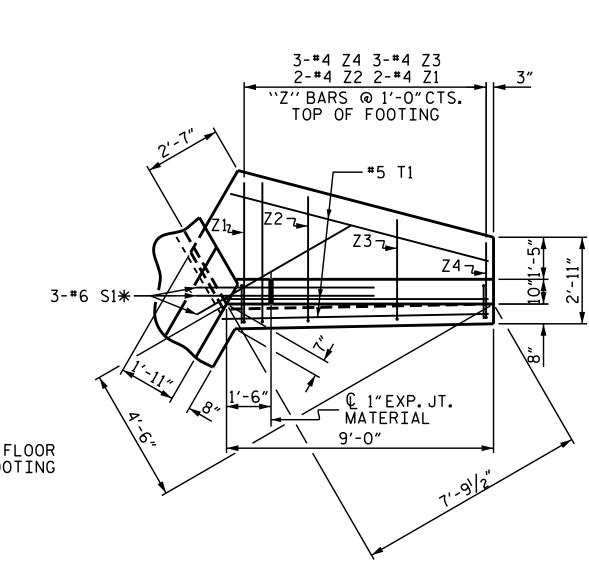
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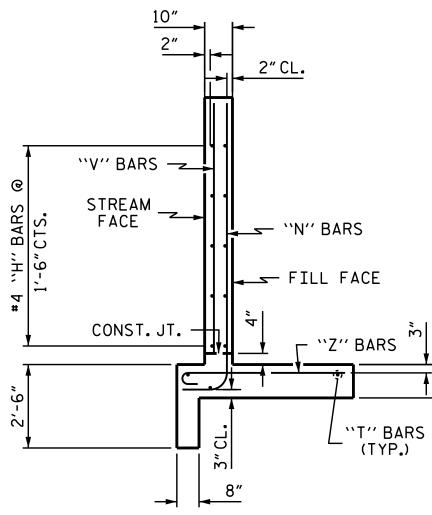








PLAN W2



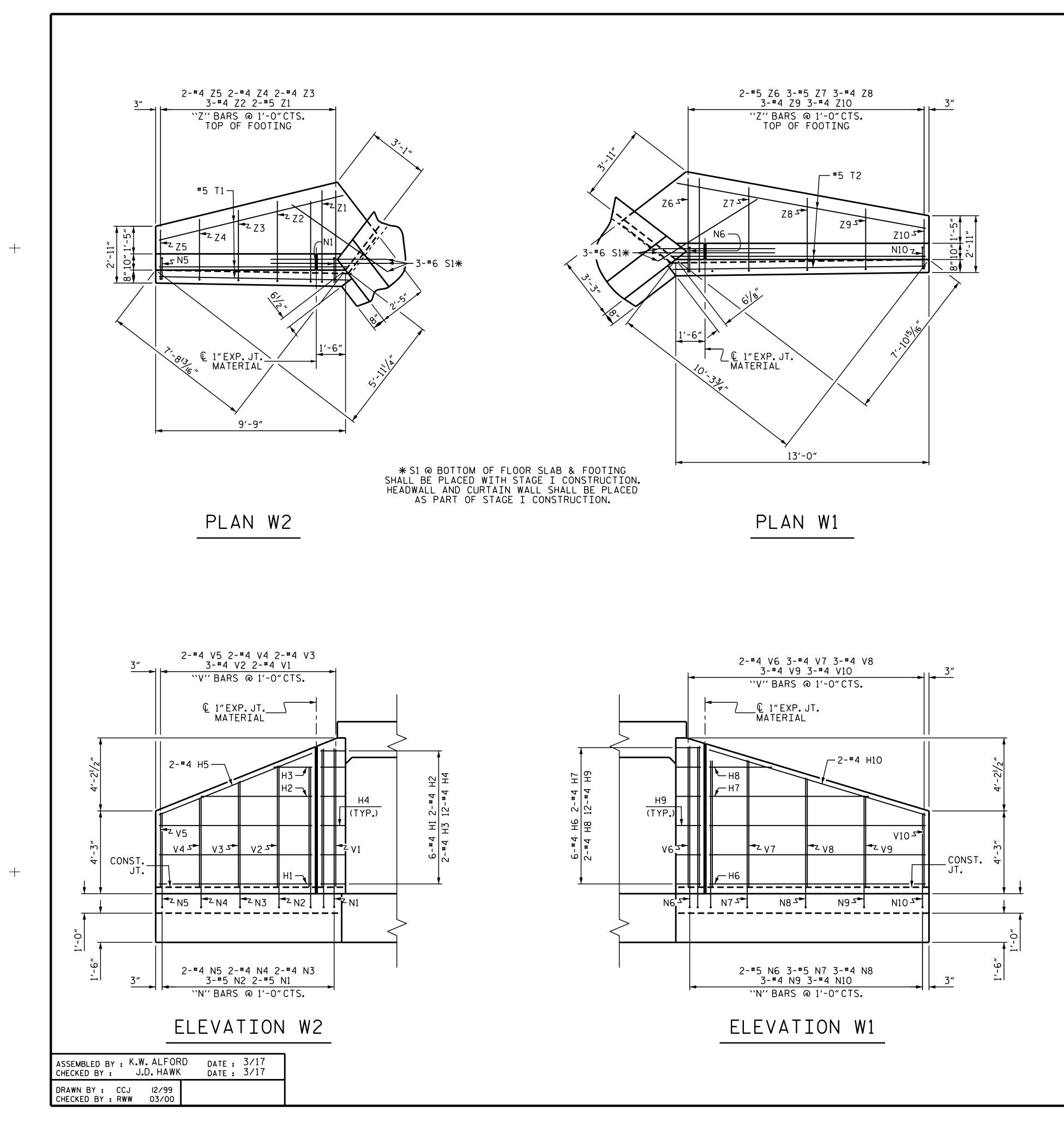
TYPICAL WING SECTION



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ALL

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BAR TYPES	BAR	BILL No.	OF	MA TYPE	TERIA	AL WEIGHT
ALL BAR DIMENSIONS ARE OUT TO OUT.	H1 H2	6	#4 #4	STR STR	7'-1" 5'-2"	28 7
	Н3	6	#4 #4	1 STR	3'-3" 7'-9"	13
	H4 H5	26	#4	STR	20'-7"	10 82
	H6 H7	2 2	#4 #4	STR STR	18'-0" 8'-3"	24 11
	H8 H9	6 2	#4 #4	2 STR	3'-3" 20'-11"	13 28
	N1	2	<b>#</b> 5	3	9'-1"	19
(2) $2^{-0^{\prime}}$	N2 N3	2	#5 #4	3	8'-6" 7'-7"	18 10
	N4 N5	2	#4 #4	3	6'-9" 5'-10"	9 8
	N6 N7	3	*5 *5	3	9'-4" 8'-7"	29 36
<u>1'-3" 1'-8¾"</u>	N8	4	#5 #4	3 3 3	8'-1" 7'-6"	34
N N N N N N N N N N N N N N N N N N N	N9 N10	4	#4	3	6'-10"	20 18
	N11	4	#4	3	6'-4"	17
-71/2" -01/2" -31/2" -11/2" -11/2" -11/2" -1/2" -11/2"	S1	6	*6	STR	6'-0"	54
7'-7/2" 7'-01/2" 5'-31/2" 5'-31/2" 6'-11/2" 6'-11/2" 6'-11/2" 6'-1/2" 1'-10/2" 1'-10/2"	T1 T2	3 3	#5 #5	STR STR	9'-0" 22'-6"	28 70
	V1	2	#4	STR	7'-1″	10
	V2 V3	2	#4 #4	STR STR	6′-5″ 5′-7″	9 7
3/12	V4 V5	2	#4 #4	STR STR	4'-8" 3'-10"	6 5
	V6 V7	3 4	#4 #4	STR STR	7'-4" 6'-9"	15 18
	V8 V9	4	#4 #4	STR STR	6'-1" 5'-6"	16 15
	V10 V11	4	#4 #4	STR STR STR	4'-11" 4'-3"	13 13 11
	Z1	2	+4	4	5'-3"	7
Z2     4'-3"     6"       Z3     3'-5"     6"	Z2 Z3	2	#4 #4	4	4'-9" 3'-11"	6
Z4 2'-7" 6"	Z4	3	#4	4	3'-1"	86
Z5     4'-5"     6"       Z6     4'-2"     6"	Z5 Z6	3	#4 #4	4	4'-11" 4'-8"	10 12
Z7 3'-9" 6"	<u> </u>	4	#4 #4	4	4'-3" 3'-11"	11 10
Z8     3'-5"     6"       Z9     3'-0"     6"	Z9 Z10	4	#4 #4	4	3'-6" 3'-0"	9 8
Z10 2'-6" 6"	REIN	  FORC]	ENG S	TEEL	7	98 LBS
		2 WIN 55 A (		FTF		
<u>(</u> 4)нк.	ULAS	2 WI 1 HEA	NGS DWALL	S	(	2.7 CY D.8 CY
		1 END	CURI	TAIN	WALLS ( TOTAL 14	D.5 CY 4.0 CY
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TYPICAL WING SECTION

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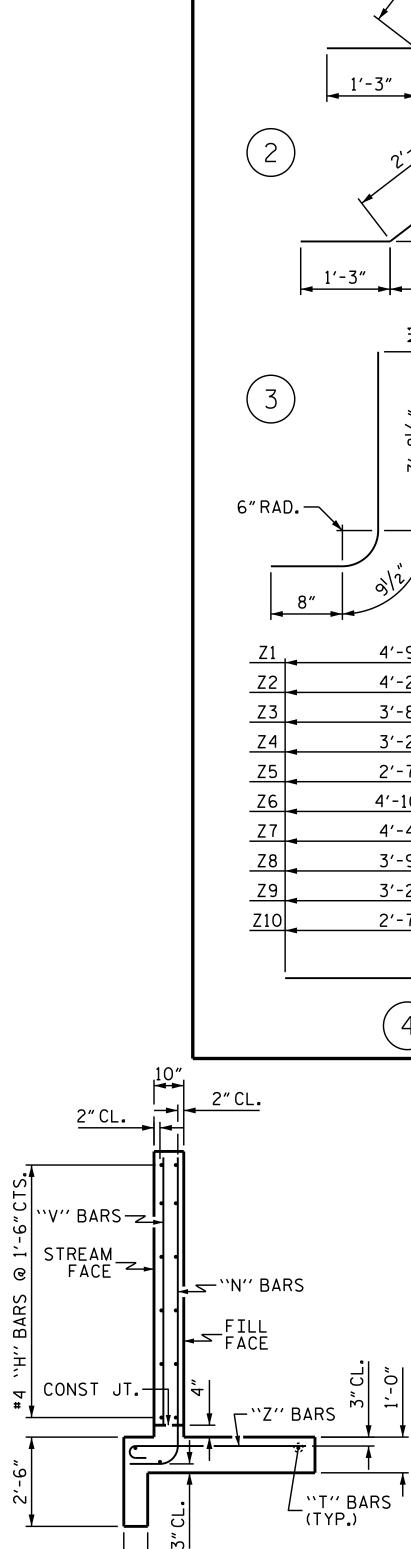
BARS

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BAR TYPES		BIL	L OF	MA	TERIAL	
	BAR	NO.	SIZE	TYPE	LENGTH	- WEIGHT
ALL BAR DIMENSIONS ARE OUT TO OUT.	H1	6	#4	STR	7'-10"	31
$\mathbf{\lambda}$	H2	2	#4	STR	5'-8"	8
	H3	2	#4	STR	2'-0"	3
	H4	12	#4	1	3'-3"	26
	H5	2	+4	STR	8′-5″	11
	H5 H6	6	#4 #4	STR	11'-1"	44
	H7	2	#4	STR	8'-2"	
			#4 #4			11
	H8	2		STR	3'-3"	4
<u>1'-3"</u> <u>1'-25/8"</u>	H9	12	#4 #4	2	3'-3"	26
-	H10	2	#4	STR	11'-7"	15
$\begin{pmatrix} 2 \end{pmatrix}$ $2^{t}$		-				
	N1	2	<b>#</b> 5	3	9'-2"	19
1,-25%	N2	3	<b>#</b> 5	3	8'-3"	26
	N3	2	#4	3	7'-6"	10
	N4	2	#4	3	6'-8"	9
	N5	2	#4	3	5'-10"	8
1'-3" 1'-7"	N6	2	<b>#</b> 5	3	9′-3″	19
	N7	3	<b>#</b> 5	3	8'-7"	27
- 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	N8	3	#4	3	7'-8″	15
N10 N10 N10 N10 N10 N10 N10 N10 N10 N10	N9	3	#4	3	6'-9"	14
	N10	3	#4	3	5'-10"	12
3		<u> </u>	· · ·	~		
	S1	6	#6	STR	6'-0"	54
				511		
7'-81/2' 6'-91/2' 6'-91/2' 7'-91/2' 7'-91/2' 7'-1/2'' 7'-1/2'' 7'-1/2''	T1	3	<b>#</b> 5	STR	9'-9"	31
9" RAD. —	T2	3	<b>#</b> 5	STR	13'-0"	41
				C T C	<b>-7</b> ,	
	V1	2	#4	STR	7'-1"	10
	V2	3	#4	STR	6'-3"	13
	٧3	2	#4	STR	5′-5″	7
8"	٧4	2	#4	STR	4'-7"	6
	٧5	2	#4	STR	3'-10"	5
<u>Z1 4'-9" 7"  </u>	٧6	2	#4	STR	7'-3"	10
Z2 4'-2" _6"	٧7	3	#4	STR	6′-6″	13
Z3 3'-8" 6"	V8	3	#4	STR	5′-7″	11
	٧9	3	#4	STR	4'-8"	9
	V10	3	#4	STR	3'-10"	8
Z5 2'-7"6"						-
Z6 4'-10" 7"	Z1	2	<b>#</b> 5	4	5′-4″	11
	Z1 Z2	3	#4	4	4'-8"	9
<u>Z7</u> <u>4'-4"</u> <u>7"</u>			#4 #4		4'-8"	
Z8 3'-9" 6"	Z3	2		4		6
Z9 3'-2" 6"	<u>Z4</u>	2	#4 #4	4	3'-8"	5
	Z5	2	#4 #5	4	3'-1"	4
<u>Z10</u> <u>2'-7"</u> <u>6"</u>	Z6	2	#5 #5	4	5'-5"	11
	Z7	3	<b>#</b> 5	4	4'-11"	15
	Z8	3	#4	4	4'-3"	9
) нк. '	Z9	3	#4	4	3'-8"	7
	Z10	3	#4	4	3'-1"	6
(4)	REIN	FORCIN	NG STE	EL	6	39 LBS
<u> </u>		2 WING			5	
<u>L.</u>	2	S A C( WING OTAL		E		9.1 CY 9.1 CY
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r' BARS	OJECT	NO	•	B	-5347	7
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····z‴ bars <sup>™</sup>   <sup>→</sup>   ST	ATION	N:	14+	14.(	<u> 10 -l</u>	
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		ST	AGE I	[]	
BAR	N0.	SIZE	TYPE	LENGTH	WEIGHT
A1	278	<b>#</b> 5	1	9'-3"	2682
A2	278	<b>#</b> 5	1	8'-1"	2344
A300	125	#6	STR	13'-11"	2613
A301	125	#6	STR	12'-3"	18
A302	1	#6	STR	12'5	16
A303	1	#6	STR	8′-6″	13
A304	1	#6	STR	6'-8"	10
A305	1	#6	STR	4'-9"	7
	1	#6			4
A306	2	#6	STR	2'-11"	
A310			STR	12'-4" 10'-7"	37
A311	2	#6 #C	STR		32
A312		#6	STR	8'-10"	27
A313	2	#6	STR	7'-2"	22
A314	2	<b>#</b> 6	STR	5'-5"	16
A315	2	<b>#</b> 6	STR	3'-8"	11
A350	8	*6	STR	11'-0"	132
A400	125	<b>#</b> 6	STR	13'-11"	2613
A401	1	<b>#</b> 6	STR	12'-3"	18
A402	1	#6	STR	10'-4"	16
A403	1	#6	STR	8′-6″	13
A404	1	<b>#</b> 6	STR	6′-8″	10
A405	1	<b>#</b> 6	STR	4'-9"	7
A406	1	<b>#</b> 6	STR	2'-11"	4
A410	2	#6	STR	12'-4"	37
A411	2	<b>#</b> 6	STR	10′-7″	32
A412	2	<b>#</b> 6	STR	8′-10″	27
A413	2	<b>#</b> 6	STR	7′-2″	22
A414	2	<b>#</b> 6	STR	5'-5"	16
A415	2	<b>#</b> 6	STR	3'-8"	11
A450	8	<b>#</b> 6	STR	11'-0"	132
	140	++ A	670	0/ 7"	0.05
B1	140	#4	STR	9'-3"	865
B2	278	#4	STR	8'-1"	1501
C2	138	#4	STR	24'-7″	2266
G2	2	#4	STR	16′-1″	21
S2	6	#8	STR	16'-1"	258

STAGE I							
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
A1	40	#5	1	9'-3″	386		
Α2	40	#5	1	8'-1"	337		
A100	20	#6	STR	14'-5"	433		
A200	20	#6	STR	14'-5"	433		
B1	20	#4	STR	9'-3"	124		
B2	40	#4	STR	8'-1"	216		
C1	46	#4	STR	11'-9″	361		
G1	2	#4	STR	14'-5"	19		
S1	6	#8	STR	14'-5"	231		

BAR TYPE						
VERTICAL LEG 1 6" R. 4'-10 <sup>1</sup> /2" 1 2 2 2 2 2 2 2 2 2 2 2 2 2						
BAR DIMENSIONS ARE OUT TO OUT						
SPLICE LENGTH CHART						
BAR SIZE SPLICE LENGTH						
**C'' <b>#</b> 4 1'-11"						
``B'' <b>#</b> 4 1'-5"						

DRAWN BY :	K.W. ALFORD		DATE :	3/17
CHECKED BY :	J.D. HAWK		DATE : _	3/17
DESIGN ENGINEER	OF RECORD: K.	ALFORD	DATE :	3/17

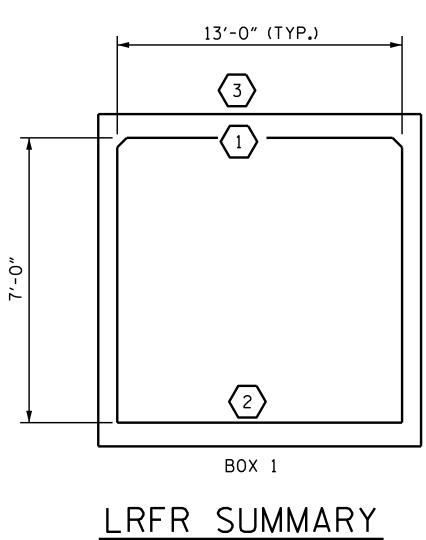
STAGE I	QUANTI	TIES	
CLASS A CONCRETE			
BARREL @ 1.721	CY./FT	17.2	C.Y.
HEADWALL		0.7	C.Y.
CURTAIN WALL		0.5	C.Y.
TOTAL		18.4	C.Y.
REINFORCING STEEL			
BARREL		2,540	LBS.
TOTAL		2,540	LBS.
FOUNDATION CONDITION	ING MATERIAL	13	TONS

STAGE II QUANTI	TIES	)
CLASS A CONCRETE		
BARREL @ 1.721 CY./FT.	120.0	C.Y.
INLET WINGS	14.0	C.Y.
TOTAL	134.0	C.Y.
REINFORCING STEEL		
BARREL	15,853	LBS.
INLET WINGS	798	C.Y.
TOTAL	16,651	LBS.
FOUNDATION CONDITIONING MATERIAL	90	TONS

STAGE III	QUANTITIES
CLASS A CONCRETE	
OUTLET WINGS	9.1 C.Y.
TOTAL	9.1 C.Y.
REINFORCING STEEL	
OUTLET WINGS	639 C.Y.
TOTAL	639 LBS.

	PROJECT NO. <u>B-5347</u> <u>ALAMANCE</u> COUNTY STATION: <u>14+14.00</u> -L-
ACTION NOT THE CAROLAND AND AND AND AND AND AND AND AND AND	SHEET 7 OF 8 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SINGLE 13' X 7' CONCRETE BOX CULVERT
DocuSigned by: F245838930BF40E 9/25/2017 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	REVISIONS       SHEET NO.         NO.       BY:       DATE:       C - 7         1       3       DATE:       TOTAL SHEETS         2       4       8

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS																
STRENGTH I LIMIT STATE																
					MOMENT SHEAR											
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y <sub>LL</sub> )	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f+)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f+)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	$\langle 1 \rangle$	1.14		1.75	1.14	1	TOP SLAB	6.83	4.64	1	Top Slab	1.32	
DESIGN		HL-93 (OPERATING)	N⁄A		1.48		1.35	1.48	1	TOP SLAB	6.83	6.02	1	Top Slab	1.32	
LOAD RATING		HS-20 (INVENTORY)	36.000	2	1.63	58.64	1.75	1.63	1	BOTTOM SLAB	6.83	6.86	1	Top Slab	1.32	
		HS-20 (OPERATING)	36.000		2.11	76.02	1.35	2.11	1	BOTTOM SLAB	6.83	8.89	1	Top Slab	1.32	
		SNSH	13.500		2.97	40.11	1.40	2.97	1	BOTTOM SLAB	6.83	11.57	1	Exterior Wall	7.17	
		SNGARBS2	20.000		2.78	55 <b>.</b> 62	1.40	2.78	1	BOTTOM SLAB	6.83	11.57	1	Exterior Wall	7.17	
	ICLE	SNAGRIS2	22.000		2.97	65.36	1.40	2.97	1	BOTTOM SLAB	6.83	11.57	1	Exterior Wall	7.17	
	VEHICLE	SNCOTTS3	27.250		1.43	38.89	1.40	1.43	1	TOP SLAB	6.83	5.8	1	Top Slab	1.32	
	C (S	SNAGGRS4	34.925		1.44	50.34	1.40	1.44	1	TOP SLAB	6.83	6.08	1	Top Slab	1.32	
	SINGLE (S)	SNS5A	35.550		1.37	48.86	1.40	1.37	1	TOP SLAB	6.83	5.76	1	Top Slab	12.35	
		SNS6A	39.950	3	1.37	54.8	1.40	1.37	1	TOP SLAB	6.83	5.76	1	Top Slab	1.32	
		SNS7B	42.000		1.37	57 <b>.</b> 61	1.40	1.37	1	TOP SLAB	6.83	5.76	1	Top Slab	1.32	
LOAD RATING	ER	TNAGRIT3	33.000		2.06	68.1	1.40	2.06	1	TOP SLAB	6.83	8.87	1	Top Slab	1.32	
	-TRAILER	TNT4A	33.075		1.70	56.24	1.40	1.7	1	TOP SLAB	6.83	6.92	1	Top Slab	1.32	
	L-IN	TNT6A	41.600		1.45	60.48	1.40	1.45	1	TOP SLAB	6.83	6.03	1	Top Slab	1.32	
	SEMI-	TNT7A	42.000		1.60	67.37	1.40	1.6	1	TOP SLAB	6.83	6.62	1	Top Slab	12.35	
	TRACTOR (TTS	TNT7B	42.000		1.47	61.76	1.40	1.47	1	BOTTOM SLAB	6.83	6.13	1	Top Slab	12.35	
	TRA(	TNAGRIT4	43.000		1.57	67.67	1.40	1.57	1	BOTTOM SLAB	6.83	6.62	1	Top Slab	1.32	
	TRUCK	TNAGT5A	45.000		1.61	72.31	1.40	1.61	1	BOTTOM SLAB	6.83	6.73	1	Top Slab	1.32	
	TRI	TNAGT5B	45.000		1.62	72.89	1.40	1.62	1	BOTTOM SLAB	6.83	6.81	1	Bottom Slab	1.32	



(LOOKING DOWNSTREAM)

ASSEMBLED BY REZA KO CHECKED BY : K.W. ALFO	DUCHEKIDATE :8/23/16 DRD DATE : 3/17
DRAWN BY : WMC 7/II CHECKED BY : GM 7/II	REV.IO/I/II MAA/GM

## LOAD FACTORS:

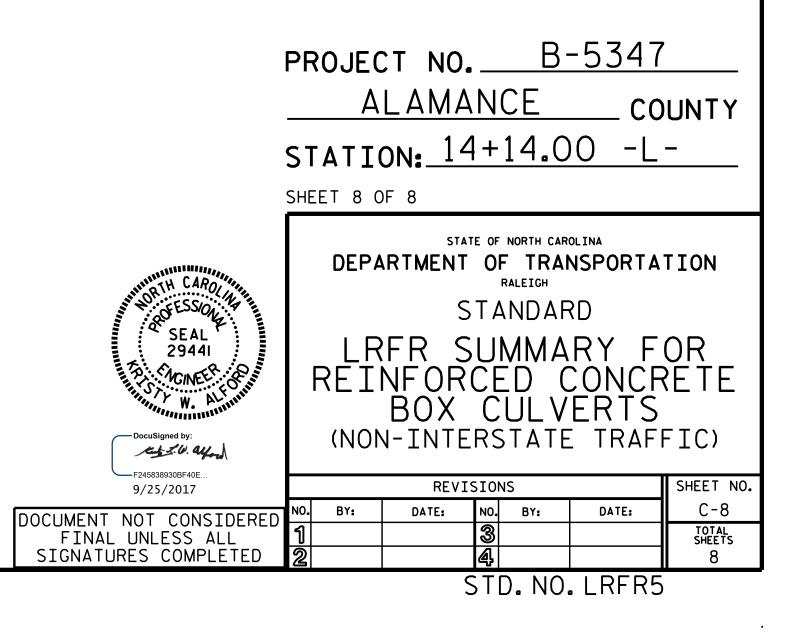
DESIGN LOAD RATING FACTORS					
LOAD TYPE	MAX FACTOR	MIN FACTOR			
DC	1.25	0.90			
DW	1.50	0.65			
EV	1.30	0.90			
ЕН	1.35	0.90			
ES	1.35	0.90			
LS	1.75				
WA	1.00				

## NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

CONTROLLING LOAD RATING 1 DESIGN LOAD RATING (HL-93) 2 DESIGN LOAD RATING (HS-20) 3 LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE



DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

## MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

## CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

#### DOWELS:

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DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

## STANDARD NOTES

## ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

## **REINFORCING STEEL:**

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

## STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR

EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

## HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

## SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

# ENGLISH JANUARY, 1990

STD. NO. SN