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October 3, 2016

MEMORANDUM TO:	Brice Bell, P.E.
	Division Bridge Program Manager - Division 6
FROM:	K. J. Kim, Ph.D., P.E. K.J. Kim Eastern Regional Geotechriftean Manager
DDAIECT.	
PROJECT: COUNTY:	53088.1.FD1 (R-5752) Robeson
DESCRIPTION:	Bridge on -Y- SR 2220 over -L- (US 74) at Sta. 30+39 -Y-
SUBJECT:	Bridge Foundation Recommendations

The Geotechnical Engineering Unit has reviewed and presents the subsurface investigation and foundation recommendations prepared by Geosyntec Consultants of NC, for the above referenced structure and present the following project data:

<u>X</u> Bridge Inventory (8) pages

<u>X</u> Foundation Design Recommendations (3) pages

_____ Design Calculations () pages

X Special Provisions (2) pages

Please call Majid Khazaei, P.E. or Jamie L. Pedro, L.G. at (919) 662-4710 if there are any questions concerning this memorandum.

KJK/CAK/MK Attachment

✓Nothing Compares[™]√

		FO	UN	NDATIO	N RE	COM	MENDATIONS
WB	S	53	088	.1.FD1	Ι	DESCRIPT	ION Bridge On -Y- (SR 2220) Over -L-
TIP NO	Э.		R-5	5752	_	(US 74)	
COUNT	Y		Rob	eson			
STATIO	N	3	0+3	9 -Y-			
	-	INITIAL	S	DATE			P.E. SEAL
DESIG	N	NWW	5	9/5/16			NG INE LANG
CHECH	-	WS		9/19/16			DocuSigned by:
							Moroge WI Wainging 2016 AECOCGE0A2E14E2 SIGNATURE
		BENT TATION	FO	OUNDATION TYPE		TORED TANCE	ADDITIONAL INFORMATION
END BENT 1	29	+ 46.23 -Y-	, ,	Cap on HP 12 x 53 Steel H-Piles	110 Te	ons/Pile	Bottom of Cap Elev. = 136.5 ft Average Estimated Pile Length = 55 ft Number of Piles/Cap = 8
BENT 1	30	+ 39.23 -Y-	• 1	Footing on HP x 53 Steel H-Piles	100 Te	ons/Pile	Bottom of Footing Elev. = 117.9 ft Average Estimated Pile Length = 55 ft Number of Piles/Footing = 6
END BENT 2	31	1+32.23 -Y-		Cap on HP12 x 53 Steel H-Piles	110 T	ons/pile	Bottom of Cap Elev. = 136.3 ft Average Estimated Pile Length = 55 ft. Number of Piles/Cap = 8
		(SEE NO	TE	S ON PLANS A	AND CO	MMENTS	ON FOLLOWING PAGES.)

FOUNDATION RECOMMENDATIONS NOTES ON PLANS

- 1. FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.
- 2. PILES AT END BENT NO. 1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 110 TONS PER PILE.
- 3. PILES AT END BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 110 TONS PER PILE.
- 4. PILES AT BENT NO. 1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 100 TONS PER PILE.
- 5. DRIVE PILES AT END BENT NO. 1 TO A REQUIRED DRIVING RESISTANCE OF 183 TONS PER PILE.
- 6. DRIVE PILES AT END BENT NO. 2 TO A REQUIRED DRIVING RESISTANCE OF 183 TONS PER PILE.
- 7. DRIVE PILES AT BENT NO. 1 TO A REQUIRED DRIVING RESISTANCE OF 167 TONS PER PILE.
- 8. TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

— ds MWW

FOUNDATION RECOMMENDATIONS NOTES ON PLANS

- 9. OBSERVE A 4 MONTH WAITING PERIOD AFTER CONSTRUCTING THE EMBANKMENT TO WITHIN 2 FT OF FINISHED GRADE BEFORE BEGINNING END BENT CONSTRUCTION AT END BENT NO 1. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SPECIAL PROVISIONS
- 10. OBSERVE A 4 MONTH WAITING PERIOD AFTER CONSTRUCTING THE EMBANKMENT TO WITHIN 2 FT OF FINISHED GRADE BEFORE BEGINNING END BENT CONSTRUCTION AT END BENT NO 2. FOR BRIDGE WAITING PERIODS, SEE SEE ROADWAY PLANS AND SPECIAL PROVISIONS



FOUNDATION RECOMMENDATIONS COMMENTS

- 1. Bridge end bent slopes of 1.5:1.0 (H:V) are ok with concrete slope protection.
- 2. Bridge approach fills for subregional tier bridges are required for both end bents.
- 3. The embankments to be constructed at both end bents will be instrumented to monitor settlement. See Roadway plans for detail.
- 4. The factored axial load for End Bent No. 1 is 110 tons per pile.
- 5. The factored axial load for End Bent No. 2 is 110 tons per pile.
- 6. The factored axial load for Bent No. 1 is 100 tons per pile.



PILE PAY ITEMS

(Revised 8/11/15)

WBS ELEMENT		53088.1.FD1	1			DATE	9/13/2017				
TIP NO.		R-5752			DESIC	GNED BY	NWW				
COUNTY		Robeson		CHECKED BY WS							
STATION		30+39 -Y-									
DESCRIPTION		Bridg	e On -Y- (SR 222	- 20) Over -L-	(US 74))					
NUM NUMBER OF	IBER OF F F END BEN	TS WITH PILES PILES PER BENT TS WITH PILES PER END BENT		for	uired for Piles" & vation" pa						
	1	D.			TE C						
		Ľ.	ILE PAY ITEM	QUANIII							
	Steel Pile				l Exca	Pile avation inear ft)	PDA				
Bent # or		Pipe Pile Plates	Predrilling For Piles	Pile Redrives	l Exca		PDA Testing				
End Bent #	Pile	Pipe Pile	Predrilling For Piles	Pile Redrives (per each)	l Exca (per l	avation inear ft)					
End Bent # Bent # 1	Pile Points (yes/no) No	Pipe Pile Plates (yes/no/maybe) No	Predrilling For Piles (per linear ft) No	Pile Redrives (per each) 9	l Exca (per l In Soil No	avation inear ft) Not In Soil No	Testing				
End Bent # Bent # 1 End Bent # 1	Pile Points (yes/no) No	Pipe Pile Plates (yes/no/maybe) No No	Predrilling For Piles (per linear ft) No No	Pile Redrives (per each) 9 4	I Exca (per l In Soil No No	avation inear ft) Not In Soil No No	Testing				
End Bent # Bent # 1 End Bent # 1 End Bent # 2	Pile Points (yes/no) No No	Pipe Pile Plates (yes/no/maybe) No	Predrilling For Piles (per linear ft) No	Pile Redrives (per each) 9	l Exca (per l In Soil No	avation inear ft) Not In Soil No	Testing				
End Bent # Bent # 1 End Bent # 1	Pile Points (yes/no) No No	Pipe Pile Plates (yes/no/maybe) No No	Predrilling For Piles (per linear ft) No No	Pile Redrives (per each) 9 4	I Exca (per l In Soil No	avation inear ft) Not In Soil No No	Testing				

Blanks or "no" represent quantity of zero.

If steel pile points are required, calculate quantity of "Steel Pile Points" as equal to the number of steel piles.

If pipe pile plates are or may be required, calculate the quantity of "Pipe Pile Plates" as equal to the number of pipe piles.

Show quantity of "PDA Testing" on the plans as total only.

PILES

Revise the 2012 Standard Specifications as follows:

Page 4-70, Article 450-2, Materials, line 2, in the materials table, replace "Neat Cement Grout, Nonshrink" with "Neat Cement Grout, Type 1".

Page 4-70, Article 450-2, Materials, line 8, in the last sentence of the second paragraph, replace "approved by the Materials and Tests Unit." with "that are on the NCDOT Approved Products List."

Page 4-72, Subarticle 450-3(D)(3), Required Driving Resistance, lines 26-30, replace first paragraph with the following:

The Engineer will determine if the proposed pile driving methods and equipment are acceptable and provide the blows/ft and equivalent set for the required driving resistance noted in the plans, i.e., "pile driving criteria" except for structures with pile driving analyzer (PDA) testing. For structures with PDA testing, provide pile driving criteria for any bents and end bents with piles in accordance with Subarticle 450-3(F)(4).

Page 4-73, Subarticle 450-3(E)(1), Pile Excavation, lines 19-20, in the third sentence of the second paragraph, replace "use smooth or corrugated clean watertight steel casings" with "use smooth non-corrugated clean watertight steel casings".

Page 4-73, Subarticle 450-3(F), Pile Driving Analyzer, lines 45-48, replace third paragraph with the following:

The Engineer will complete the review of the proposed pile driving methods and equipment within 7 days of receiving PDA reports and pile driving criteria. Do not place concrete for caps or footings on piles until PDA reports and pile driving criteria have been accepted.

Page 4-75, Subarticle 450-3(F), Pile Driving Analyzer, line 21, add the following to the end of Article 450-3:

(4) Pile Driving Criteria

Analyze pile driving with the GRL Wave Equation Analysis Program (GRLWEAP) manufactured by Pile Dynamics, Inc. Use the same PDA Consultant that provides PDA reports to perform GRLWEAP analyses and develop pile driving criteria. Provide driving criteria sealed by an engineer approved as a Project Engineer (key person) for the same PDA Consultant.

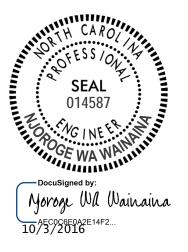
Analyze pile driving so driving stresses, energy transfer, ram stroke and blows/ft from PDA testing and resistances from CAPWAP analyses correlate to GRLWEAP models. Provide pile driving criteria for each combination of required driving resistance and pile length installed for all pile types and sizes. Submit 2 copies of pile driving criteria with

PDA reports. Include the following for driving criteria:

- (a) Project information in accordance with Subarticle 450-3(F)(3)(a)
- (b) Table showing blows/ft and equivalent set vs. either stroke for multiple strokes in increments of 6" or bounce chamber pressure for multiple pressures in increments of 1 psi
- (c) Maximum stroke or blows/ft or pile cushion requirements to prevent overstressing piles as needed
- (d) GRLWEAP software version information
- (e) PDF copy of all pile driving criteria and executable GRLWEAP input and output files

Page 4-76, Article 450-4, Measurement and Payment, lines 27-29, replace third sentence of the sixth paragraph with the following:

The contract unit price for *PDA Testing* will be full compensation for performing PDA testing the first time a pile is tested, performing CAPWAP analysis on data collected during initial drive, restrikes and redrives, providing PDA reports, performing GRLWEAP analysis and developing and providing pile driving criteria.



CONTENTS

	<u>SHEET NO.</u>
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REFERENCE

DESCRIPTION TITLE SHEET LEGEND (SOIL & ROCK) SITE PLAN PROFILE BORE LOGS

SOIL TEST RESULTS

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY **ROBESON**

PROJECT DESCRIPTION PROPOSED BRIDGE ON -Y- (SR 2220) AT STA. 30+39.23 OVER

-L- (US-74) AT STA. 55+06.59

3088 う PROIEC

STATE PROJECT REFERENCE NO. STATE NO. SHEETS N.C.R-5752 8 1

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOLI TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6860. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CENERAL SOL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UN-PLACED TEST DATA CAN BE RELIED ON ONLY TO THE DECREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOLL MOISTURE CONDITIONS. NOICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOLL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

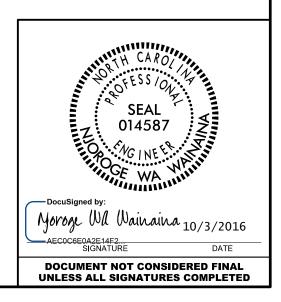
THE BIDDER OR CONTRACTOR IS CALIDINED THAT DETAILS SHOWN ON THE VIDSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPNION OF THE DEPARTMENT AS TO THE TYPE AND THE INTERPRETATIONS MADE, OR THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OF FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDENSATION.

- NOTES: I. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

GEOSVNTEC

GEOSIMIEC
CAROLINA DRILLING
INVESTIGATED BY ROHIT WARRIER
DRAWN BY CHUCK TURLINGTON
CHECKED BY
SUBMITTED BY NJOROGE WAINAINA
DATE SEPTEMBER 2016



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

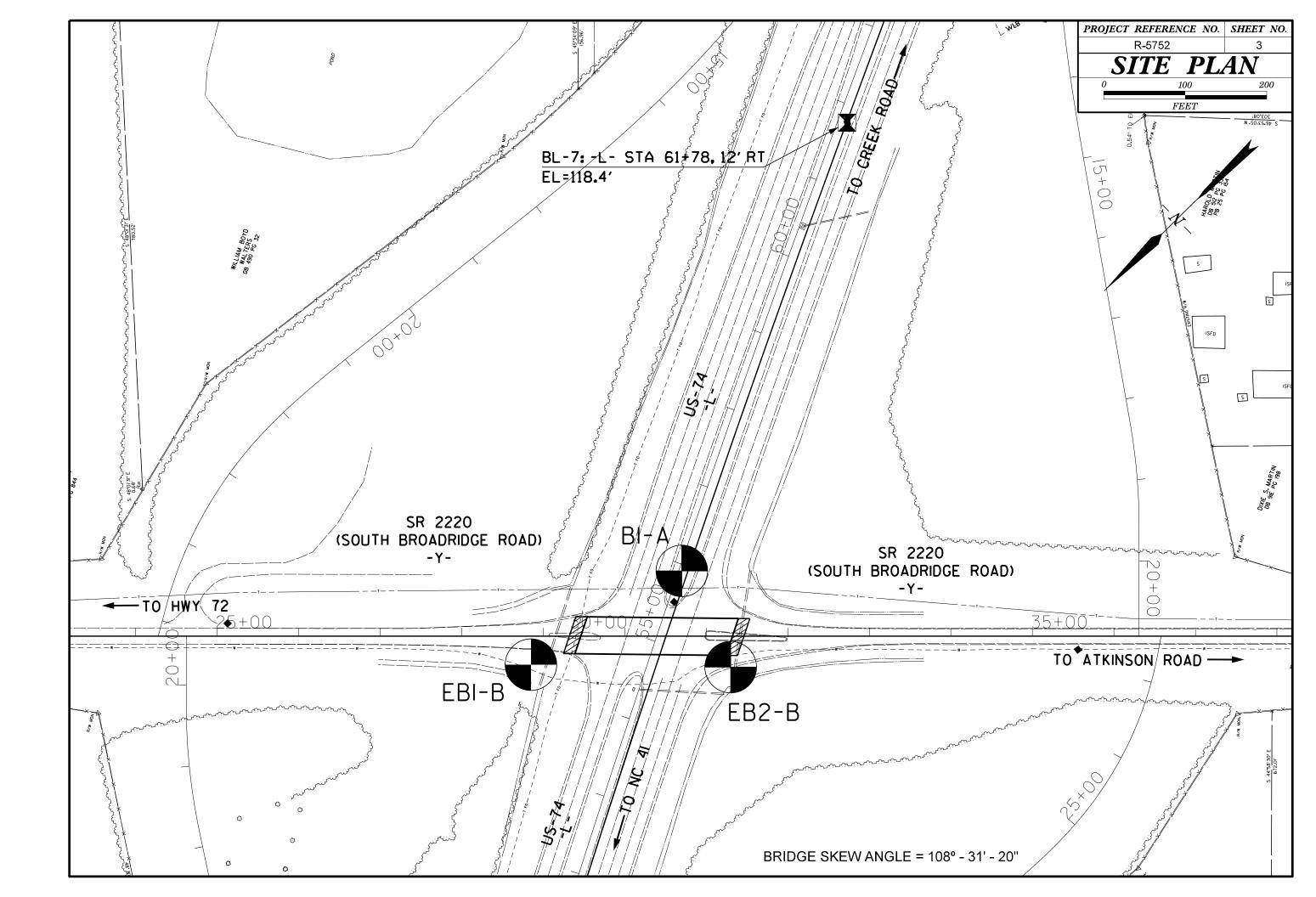
			SOIL D	ESCR	IPTION						GRADATION					ROCK DE	SCRIPTION
BE PENET ACCORDI	RATED WITH NG TO THE	UNCONSOLIDAT A CONTINUOUS STANDARD PENE E AASHTO SYS	FLIGHT POW TRATION TES	ER AUG	ER AND YIEL HTO T 206,4	LD LESS THAN ASTM D1586).	100 BLOWS F SOIL CLASSIF	ER FOOT ICATION	UNIFORMLY GRADED - IN	NDICATES THAT	T SOIL PARTICLES ARE ALL	E SIZES FROM FINE TO COA APPROXIMATELY THE SAME ES OF TWO OR MORE SIZES.	SIZE. ROCK LIN SPT REFU	E INDICATE SAL IS PE	ES THE LEVE NETRATION E	L AT WHICH NON-COA BY A SPLIT SPOON S	WOULD YIELD SPT REFUSAL IF TESTE ASTAL PLAIN MATERIAL WOULD YIELD AMPLER EQUAL TO OR LESS THAN 0.1 ANSITION BETWEEN SOIL AND ROCK
CONSISTE	NCY, COLOR,	TEXTURE, MOIS ICAL COMPOSIT	URE, AASHTO	CLASSI	FICATION, AN	ND OTHER PER	FINENT FACTO	RS SUCH		ANG	ULARITY OF GRAIN	IS	REPRESEN	TED BY A	ZONE OF WE	ATHERED ROCK. Y DIVIDED AS FOLLO	
	/ERY STIFF.GF	DIL LEGEN	IST WITH INTE	RBEDDE	D FINE SAND) LAYERS, HIGHLY	PLASTIC, A-7-6	••		NGULAR, <u>SUBROU</u>	ESS OF SOIL GRAINS IS DE UNDED, OR <u>ROUNDED</u> .		WEATHERE ROCK (WR)	D		4	IN MATERIAL THAT WOULD YIELD SPT
GENERAL		RANULAR MATERIA	LS	SIL	T-CLAY MATERI	IALS	ORGANIC MATER		-		ALOGICAL COMPOSI		CRYSTALL	INF	J. J.		GRAIN IGNEOUS AND METAMORPHIC RO
CLASS.		≤ 35% PASSING #2			35% PASSING *	1					OUARTZ, FELDSPAR, MICA, TA		ROCK (CR)			GNEISS, GABBRO, S	REFUSAL IF TESTED. ROCK TYPE IN CHIST, ETC.
GROUP CLASS.	A-1 A-1-a A-1-b	A-3 A-2-4 A-2	A-2 5 A-2-6 A-2-		8-9 H-6	A-7 A-1.A A-7-5 A-3 A-7-6 A-3				(COMPRESSIBILITY		NON-CRYS				GRAIN METAMORPHIC AND NON-COASTA K THAT WOULD YEILD SPT REFUSAL
SYMBOL	0000000000			1						HTLY COMPRES		LL < 31 LL = 31 - 50	ROCK (NCF COASTAL			ROCK TYPE INCLU	DES PHYLLITE, SLATE, SANDSTONE, ETO EDIMENTS CEMENTED INTO ROCK, BUT
○ % PASSING	0000000000		<u></u>	5423222					HIGH	ILY COMPRESSIE	BLE	LL > 50	SEDIMENT			SPT REFUSAL. RO	CK TYPE INCLUDES LIMESTONE, SANDS
*10 *40	50 MX 30 MX 50 MX 9	51 MN				GRANUL		MUCK, PEAT			ENTAGE OF MATER	IAL	(CP)				HERING
		Ø MX 35 MX 35	1X 35 MX 35 M	X 36 MN	36 MN 36 MN		SOILS		ORGANIC MATERIAL	GRAN <u>SOI</u>	IULAR SILT - CLAY ILS <u>SOILS</u>	OTHER MATERIAL	FRESH	ROCK	FRESH, CRYST	ALS BRIGHT, FEW JOIN	ITS MAY SHOW SLIGHT STAINING. ROCK
MATERIAL PASSING #40									TRACE OF ORGANIC M LITTLE ORGANIC MAT		- 3% 3 - 5% - 5% 5 - 12%	TRACE 1 - 10% LITTLE 10 - 20%			R IF CRYSTA		
LL	-				41 MN 40 MX	41 MN	OILS WITH .ITTLE OR		MODERATELY ORGANIC HIGHLY ORGANIC		10% 12 - 20%	SOME 20 - 35% HIGHLY 35% AND AB	VERY SLIG (V SLI.)				,SOME JOINTS MAY SHOW THIN CLAY C SHINE BRIGHTLY. ROCK RINGS UNDER H
PI	6 MX	NP 10 MX 10 1	11 MN 11 MP	+	10 MX 11 MN		MODERATE	HIGHLY ORGANIC	HIGHET ONOHNIC		10% > 20% GROUND WATER	HIGHET 30% HIGHB			CRYSTALLINE		
GROUP INDEX	STONE FRAGS				12 MX 16 MX		MOUNTS OF ORGANIC	SOILS			EL IN BORE HOLE IMMEDIA	TELY AFTER DRULING	SLIGHT (SLI.)	1 INCH.	. OPEN JOINT	S MAY CONTAIN CLAY.	AND DISCOLORATION EXTENDS INTO RO IN GRANITOID ROCKS SOME OCCASIONAL
OF MAJOR	GRAVEL, AND		or Clayey AND Sand			AYEY DILS	MATTER		▼		TER LEVEL AFTER <u>24</u> H		10050475				RYSTALLINE ROCKS RING UNDER HAMMER
MATERIALS GEN. RATING	SAND					FAIR	rn l		 		ATER, SATURATED ZONE, OR		MODERATE (MOD.)	GRANIT	TOID ROCKS, M	10ST FELDSPARS ARE	SCOLORATION AND WEATHERING EFFECTS DULL AND DISCOLORED, SOME SHOW CLA
AS SUBGRADE	E	XCELLENT TO GOO	D		FAIR TO POOR	POOF	POOR	UNSUITABLE		SPRING OR					SOUND UNDER FRESH ROCK.	HAMMER BLOWS AND	SHOWS SIGNIFICANT LOSS OF STRENGTH
	Р	I OF A-7-5 SUBGR					30		0 00-				MODERATE				OR STAINED. IN GRANITOID ROCKS,ALL F
		LUN	SISTENU		DENSEN		ANGE OF UN			MISU	ELLANEOUS SYMBO	LS	SEVERE (MOD, SEV.)				KAOLINIZATION. ROCK SHOWS SEVERE L ST'S PICK, ROCK GIVES "CLUNK" SOUND "
PRIMARY S	OIL TYPE	COMPACIN CONSIST			RATION RESIS	STENCE C	OMPRESSIVE (TONS/F	STRENGTH	L ROADWAY EMB	BANKMENT (RE)	DIP & DIP DIRE					YIELD SPT REFUSAL	
		VERY L	IOSE		(N-VALUE)		(TUNS/F	1-7	1 4		697		SEVERE (SEV.)				OR STAINED. ROCK FABRIC CLEAR AND E IN GRANITOID ROCKS ALL FELDSPARS #
GENERAL GRANULA		L009	E		4 TO 10				SOIL SYMBOL		OPT DMT TEST BOR	ING 📿 INSTALLATION				SOME FRAGMENTS OF S YIELD SPT N VALUES	STRONG ROCK USUALLY REMAIN.
MATERIA (NON-CO		MEDIUM DENS	E		10 TO 30 30 TO 50		N/A			ILL (AF) OTHER		CONE PENETR	DMETER VERY				DR STAINED. ROCK FABRIC ELEMENTS AR
		VERY D			> 50		(_	I INFERRED SO		- CORE BORING	SOUNDING ROL	SEVERE (V SEV.)				SOIL STATUS, WITH ONLY FRAGMENTS OF F ROCK WEATHERED TO A DEGREE THAT
GENERAL		VERY S	r i		< 2 2 TO 4		< 0.25 0.25 TO	0.5			MW ~		,				IAIN. <u>IF TESTED, WOULD YIELD SPT N V</u>
SILT-CL MATERIA		MEDIUM STIF			4 TO 8 8 TO 15		0.5 TO 1 TO 2		INFERRED ROOM	CK LINE	MONITORING WE		COMPLETE				OT DISCERNIBLE, OR DISCERNIBLE ONLY Y BE PRESENT AS DIKES OR STRINGERS
(COHESI)		VERY S HAR			15 TO 30 > 30		2 TO > 4	4	TTTTT ALLUVIAL SO	IL BOUNDARY	△ PIEZOMETER INSTALLATION	O- SPT N-VALUE			AN EXAMPLE.		
				DR GI	RAIN SI	ZE				RECON	MMENDATION SYMB	DLS					IARDNESS
U.S. STD. SIE	EVE SIZE		4 10	40	60	200 27	Ø				FIED EXCAVATION -	UNCLASSIFIED EXCAVATI				HED BY KNIFE OR SHA WS OF THE GEOLOGIST	ARP PICK. BREAKING OF HAND SPECIMEN N'S PICK.
OPENING (MM	1)	4	76 2.00	0.4		0.075 0.0	53		SHALLOW		LE WASTE Læ FIED EXCAVATION -	ACCEPTABLE, BUT NOT T USED IN THE TOP 3 FEE	T OF HARD				NLY WITH DIFFICULTY. HARD HAMMER B
BOULDER (BLDR.)			VEL	COAR SAN		FINE SAND	SILT (SL.)	CLAY (CL.)		АССЕРТАВ	BLE DEGRADABLE ROCK	EMBANKMENT OR BACKFI	_L MODERATEI		TACH HAND S E SCRATCHED		GOUGES OR GROOVES TO 0.25 INCHES DE
				(CSE.		(F SD.)					ABBREVIATIONS		HARD		ATED BY HAR DERATE BLOW		IST'S PICK. HAND SPECIMENS CAN BE D
GRAIN MM SIZE IN.		75 3	2.0		0.25	0.0	0.00	5	AR - AUGER REFUSAL BT - BORING TERMINATE		MED MEDIUM MICA MICACEOUS	VST - VANE SHEAR TE WEA WEATHERED	MEDIUM				S DEEP BY FIRM PRESSURE OF KNIFE O
	SI	DIL MOIS	URE - 0	ORRE	ELATION	OF TER	4S		CL CLAY CPT - CONE PENETRATIO		MOD MODERATELY NP - NON PLASTIC	γ - UNIT WEIGHT $\gamma_{\rm d}$ - DRY UNIT WEIGHT	HARD		E EXCAVATED OF A GEOLOG		PEICES 1 INCH MAXIMUM SIZE BY HARD
	MOISTURE S		FIELD MO		GUIDE	FOR FIELD	MOISTURE DE	SCRIPTION	CSE COARSE		ORG ORGANIC	_	SOFT				KNIFE OR PICK. CAN BE EXCAVATED IN
(ATT	ERBERG LIM	115)	DESCRIF						DMT - DILATOMETER TES DPT - DYNAMIC PENETRA		PMT - PRESSUREMETER TE SAP SAPROLITIC	ST <u>SAMPLE ABBREVIATI</u> S - BULK	0115			VERAL INCHES IN SIZE DKEN BY FINGER PRES	E BY MODERATE BLOWS OF A PICK POIN SURE.
			- SATURA (SAT.)	TED -		LLY LIQUID; W BELOW THE			e – VOID RATIO F – FINE		SD SAND, SANDY SL SILT, SILTY	SS - SPLIT SPOON ST - SHELBY TUBE	VERY				CAVATED READILY WITH POINT OF PICK.
PLASTIC		_IMIT _							FOSS FOSSILIFEROUS		SLI SLIGHTLY	RS - ROCK	SOFT	OR MO FINGEF		NESS CAN BE BROKEN	BY FINGER PRESSURE. CAN BE SCRATCH
RANGE <			- WET -	W)		SOLID:REQUIR IN OPTIMUM I		D	FRAC FRACTURED, FRAC FRAGS FRAGMENTS		<pre>TCR - TRICONE REFUSAL ₩ - MOISTURE CONTENT</pre>	RT - RECOMPACTED TF CBR - CALIFORNIA BEA		FRAC	TURE SP	ACING	BEDDING
(PI) PL	PLASTIC	LIMIT _							HI HIGHLY		V - VERY	RATIO	TEF			SPACING	TERM
ОМ	OPTIMUN	MOISTURE	- MOIST	- (M)	SOLID	; AT OR NEAR	OPTIMUM M	OISTURE	DRILL UNITS:	ADVANCING	USED ON SUBJECT	PROJECT HAMMER TYPE:	WIDE		з	E THAN 10 FEET 3 TO 10 FEET	VERY THICKLY BEDDED THICKLY BEDDED 1.
SL		GE LIMIT _							X CME-45C				NUAL CLOSE	ATELY CLC		1 TO 3 FEET .16 TO 1 FOOT	THINLY BEDDED 0.1 VERY THINLY BEDDED 0.0
			- DRY - (D)		IRES ADDITIO IN OPTIMUM N		0			ITINUOUS FLIGHT AUGER		VERY (CLOSE		THAN 0.16 FEET	THICKLY LAMINATED 0.00 THINLY LAMINATED <
				STIC					CME-55		LOW AUGERS	СОRE SIZE:				INDU	RATION
					NDEX (PI)		DRY STREN	бтн	CME-550	HARD	FACED FINGER BITS			MENTARY F	OCKS, INDUR	ATION IS THE HARDE	NING OF MATERIAL BY CEMENTING, HE
	PLASTIC		<u>. ENOTI</u>	0-5	11		VERY LO	W		TUNG	-CARBIDE INSERTS		FR	ABLE			FINGER FREES NUMEROUS GRAINS; BY HAMMER DISINTEGRATES SAMPLE.
MOD	GHTLY PLAS ERATELY PL	ASTIC		6-15 16-25			SLIGHT MEDIUM		VANE SHEAR TEST	X CASING	G W/ ADVANCER	HAND TOOLS:					E SEPARATED FROM SAMPLE WITH ST
HIGH	HLY PLASTIC			OR MI			HIGH		PORTABLE HOIST	TRICOM	NE STEEL TEETH	HAND_AUGER	MOI	DERATELY	INDURATED		Y WHEN HIT WITH HAMMER.
				OLOF	{					TRICON		SOUNDING ROD	IND	URATED			IFFICULT TO SEPARATE WITH STEEL BREAK WITH HAMMER.
		NCLUDE COLOF										VANE SHEAR TEST					R BLOWS REQUIRED TO BREAK SAMPLE
MU	DILIER2 200	CH AS LIGHT,	HAK, SIREA	CU, EII	C. ARE USED	U DESCRIE	E AFFEARANU	L .		X 27/8	AND 31% DRAG BIT		EX.	FREMELY I	NUURATED		S ACROSS GRAINS.



DATE: 8-15-14



	TERMS AND DEFINITIONS
ED. AN INFERRED D SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
1 FOOT PER 60 IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.
	<u>ARENACEOUS</u> - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. <u>ARGILLACEOUS</u> - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
T N VALUES >	A NOTABLE PROPORTION OF CLAT IN THEIR COMPOSITION, SOUT AS SHALE, SLATE, END ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
OCK THAT NCLUDES GRANITE,	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
AL PLAIN IF TESTED. C.	<u>COLLUVIDM</u> -ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
MAY NOT YIELD STONE,CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
211/22 11/252	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
RINGS UNDER	$\overline{\text{DIP}}$ - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
COATINGS IF OPEN, HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
DCK UP TO AL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
R BLOWS. IS. IN	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
AY. ROCK HAS H AS COMPARED	PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
FELDSPARS DULL LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
EVIDENT BUT ARE KAOLINIZED	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS
RE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
OF STRONG ROCK T ONLY MINOR	OF AN INTERVENING IMPERVIOUS STRATUM.
<u>VALUES < 100 BPF</u> IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
S. SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
NS REQUIRES	$\underline{SAPROLITE}$ - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
BLOWS REQUIRED	<u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
DEEP CAN BE DETACHED	$\underline{\text{SLICKENSIDE}}$ - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
OR PICK POINT. BLOWS OF THE	STANDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPI) - NUMBER OF BLOWS (N OR BPF)OF A 140 LB.HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
N FRAGMENTS NT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
. PIECES 1 INCH HED READILY BY	STRATA ROCK DUALITY DESIGNATION (SROD) - A MEASURE OF ROCK DUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SECMENTS WITHIN A STRATUM EDUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
THICKNESS	BENCH MARK: BL-7: -L- STA: 6I+78, OFFSET: 12' RT
THICKNESS 4 FEET	NORTHING: 273,021 EASTING: 2,000,517 ELEVATION: 118.4 FEET
1.5 - 4 FEET .16 - 1.5 FEET	
03 - 0.16 FEET 08 - 0.03 FEET	<u>NOTES:</u> TIN FILE NAME "r5752_ls_tnl.tin" WITH FILE DATE 2/23/2016, WAS
< 0.008 FEET	USED TO GENERATE BORING PROFILE GROUND LINE.
EAT, PRESSURE, ETC.	N/A:NOT APPLICABLE WOH:WEIGHT OF HAMMER
TEEL PROBE;	
PROBE:	
Е;	



130					0	50	100 PROJECT REFE	RENCE NO. SHEET N
	EBI-B		BI-A	EB2-B		FEET	R-575	52 4
	ASPHALT 28+85		30+70			VE = 5:1	BORIN ALONG -Y-	IGS PROJECTED CENTERLINE PROFILE
· — — — — –	<u>35' RT</u> <u>35' RT_</u>		80′LT	<u>38' RT</u>				
120		/	[<u>\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</u>			ASPHALT		
		$(A) \qquad \qquad I \qquad $		8	→			
		//			7/16			
110	COASTAL CLAYEY SAND	AIN,GRAY,LOOSE TO MEDIUM DE (YORKTOWN FORMATION)			SATURATED, FINE	SAND AND		1
	CLAIEI SAWD							
	9							
		ST -1			—-ST-2			
.100	GRAY-TAN. 2	SOFT.SATURATED. SAND LAYERS			SILTY CLAY.WIT	ГН		1
			24	(12)				
	COAST AL PLAIN,	AY-TAN,LOOSE TO MEDIUM DEN (BLACK CREEK FORMATION)	'SE,		SATURATED, FINE	SAND		
90					· ·] · · · · · · · · · · · · · · · · · · ·			
	DARK GRAY.	STIFF TO VERY STIFF.			WET, SILTY CLA	47		
.80		······································	(26)			······	•••••••••••••••••••••••••••••••••••••••	د ز
70	GRAY, LOOSE	TO MEDIUM DENSE.			WET,CLAYEYSA	WD		
60	(24)		(24)	38-				
	DARK GRAY, 36	VERY STIFF TO HARD, SANDY CLAY		30-	WET, SILTY CLA	47		
50			23	47				Ę
	39-2		38)>26-				
						IBANKMENT, RED-BROWN, I		
.40	27 - 5			(28)		AIN, RED-BROWN AND GRA	AY,STIFF,MOIST,SAND	Y SILT AND 2
						(YORKTOWN FORMATION)		
NOTE:				(29)	C CUAST AL PL	AIN,GRAY,DENSE,WET,SILT	I JANU	
	NE TAKEN FROM "r5752_ls_tnl.tin",FILE D	AI E 2/23/2016						
30		······	(43)	(60)				
+50	28+00 +50 29+00) +50 30+00	+50	31+00	+50 32+00) +50 33	3+00 +50	34+00

GEOTECHNICAL BORING REPORT BORE LOG

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WBS	53088	3.1.FD1			TI	P R-575	52	C	COUNTY	r ROBES	SON			GE	OLOGIST Rohit	Warrier			WBS	5308	3.1.FD1			TI	P R-57	52	COUN	ΓY
SITE	DESCR	IPTION	I PRO	OPOSI			N -Y- (S	R 2220	0) AT S	TA. 30+39	.23 OVE	R -L- (US 7	(4) AT 3	STA. 55+06.59		GROUN	D WTR (ft)	SITE	DESCR	RIPTION	PR	OPOSE	ED BR	IDGE O	N -Y- (SR 2	220) AT \$	ST/
BOR	ING NO.	. EB1-	В		S	TATION	28+85			OFFSET	35 ft RT	-		ALI	GNMENT -Y-		0 HR.	5.0	BOR	ING NO	. EB1	-В		ST	ATION	28+85		0
COLI		EV. 12	20.1 ft		т	OTAL DE	PTH 8	0.0 ft		NORTHIN	IG 273,	765		EAS	STING 2,000,319	9	24 HR.	5.0	COL	LAR EL	EV. 12	20.1 ft		тс	DTAL DE	PTH 80.0	ft	N
DRILL	RIG/HAI	MMER E	FF./DA	TE BF	R18284	CME-45C 8	89% 02/2	6/2016			DRILL	METHO	DD N	/lud Rota	iry	HAMM	IER TYPE	Automatic	DRILI	RIG/HA	MMER E	FF./DA	TE BR	R18284 (CME-45C	89% 02/26/20	16	
DRIL	LER A				S	TART DA	TE 07	/20/16		COMP. D	ATE 07	/20/16	6	SUF	RFACE WATER D	EPTH N	/A		DRIL	LER A	nderso	n, J.		ST	ART DA	TE 07/20/	′16	C
ELEV	DRIVE	DEPTH	BLC	W COL	JNT		BLC	WS PE	R FOOT		SAMP	. 💙/	L		SOIL AND				ELEV	DRIVE ELEV	DEPTH	BLC	ow cor	JNT		BLOWS	PER FOO	л
(ft)	ELEV (ft)	(ft)		0.5ft	0.5ft	0	25	50		75 10	NO.	мо	DI G	ELEV.		ROCK DES	CRIFTION	DEPTH (ft)	(ft)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0	25	50	75
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SHEET 5

ROBESOI	N			GEOLOGIST Rohit War	rier		
FA. 30+39.23	8 OVER	l -L- (l	JS 74) AT STA. 55+06.59		GROUN	D WTR (ft)
OFFSET 3	5 ft RT			ALIGNMENT -Y-		0 HR.	5.0
NORTHING	273,7	65		EASTING 2,000,319		24 HR.	5.0
	DRILL N	IETHO	D Mu	d Rotary	НАММЕ	R TYPE	Automatic
COMP. DAT	E 07/2	20/16		SURFACE WATER DEPT	H N//	4	
75 100	SAMP. NO.	моі	L O G	SOIL AND ROCI	K DESC	RIPTION	
				DARK GRAY, SILT	Y CLAY	(continue	a)
		w		40.1			80.0
<u> </u>				Boring Terminated	at Eleva	ation 40.1	ft
				CÕASTAL PLA	IIN, SIL I	Y CLAY	
				<u>Other Samples:</u> ST-1 (17.0 - 19.0)			
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GEOTECHNICAL BORING REPORT BORE LOG

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		8.1.FD1				P R-57				r ROBES				GEOLOGIST Rohit Warrier 74) AT STA. 55+06.59 GROUND WTR (ft)					53088					TIP R-5752 COUNT ED BRIDGE ON -Y- (SR 2220) AT S COUNT COUNT			
				OPOS					· ·				(US 7	4) AT STA	55+06.59		ID WTR (ft)					OPOS				220) AT ST	
BOR	ING NO	. B1-A			S	TATION	30+70)		OFFSET	80 ft LT	•		ALIGNN	IENT -Y-	0 HR.	5.5	BOR	ING NO.	. B1-A	۸		ST	TATION 3	30+70		
COL	LAR EL	EV. 12	0.2 ft		Т	OTAL DE	PTH 9	90.0 ft		NORTHIN	G 273,	553		EASTIN	G 2,000,270	24 HR.	4.9	COL		E V. 12	20.2 ft		тс)TAL DEP	TH 90.01	ft l	
DRILL	RIG/HA	MMER E	FF./DA	TE BF	R18284	CME-45C	89% 02/	26/2016	; 		DRILL	METH	OD N	/lud Rotary	HAMI	MER TYPE	Automatic	DRILI	L RIG/HAMMER EFF./DATE BR1828					CME-45C 89	9% 02/26/201	16	
DRIL	LER A	Anderso	n, J.		S	TART DA	TE 07	7/22/16	6	COMP. D	ATE 07	/22/16	6	SURFA	CE WATER DEPTH	N/A		DRIL	ORILLER Anderson, J.					ART DAT	E 07/22/ ⁻	16	
ELEV	DRIVE ELEV	DEPIR	BLC	W COL	JNT		BL	OWS P	ER FOOT		SAMP				SOIL AND ROCK DES	SCRIPTION		ELEV	DRIVE ELEV	DEPTH	BLC	ow co	UNT		BLOWS	PER FOOT	
(ft)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0	25	5	0	75 100) NO.	Имс	DI G	ELEV. (ft)			DEPTH (ft)	(ft)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0	25	50 7	
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		ŧ												- 100.0			0.0		41.7 -	- 78.5	10	14	18		. <i>I</i>		
120	119.2	1.0									+1			120.2	GROUND SURF COASTAL PL	AIN	0.0	40	-	Ł					• 32		
	116 7	- - 3.5	2	3	5							M		117.2	RED-BROWN, SAN (YORKTOWN FOR		3.0		36.7 -	83.5							
115	116.7	+ <u>3.5</u> +	2	3	5	• • • • • • • • • • • • • • • • • • •		· · · ·				M	Ĺ	114.7	GRAY, SILTY (CLAY		35		- 03.5 -	15	21	30			• • • • • • •	
	114.2	<u>+ 6.0</u>	2	6	4						1	Sat.		- 114.7	GRAY, FINE S	AND			-	ŧ						1	
	111.7	+ - 8.5					· · ·	· · · · · ·	· · · · ·					-					- 31.7 -	- 88.5					· · · /	/	
110	-	‡	3	5	5	• • 10	• •					Sat.	· • • • • • •	-					-	<u> </u>	12	19	24		•••	43 • • • •	
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5	71.7	1 48 5				$\left \left \begin{array}{c} \cdot \cdot \cdot \\ \cdot \cdot \end{array} \right \right $	1 :	· · ·						13.2	DARK GRAY, CLAY	EY SAND	47.0		-	ŧ							
65 60 55		+	6	8	9	• • •	17					w	/./.						-	Ł							
		Ŧ]		///	68.2			52.0		-	Ŧ							
2	66.7	53.5		4	F	:;/:		· · · ·						F	DARK GRAY, SILT	Y CLAY			-	Ŧ							
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45								-38		1				4						L							

SHEET 6

ROBESON		GEOLOGIST Rohit Warrier							
ra. 30+39.23 Ove	ER -L- (US 74) AT STA. 55+06.59	GROUND WTR (ft)						
OFFSET 80 ft L	Т	ALIGNMENT -Y-	0 HR. 5.5						
NORTHING 273	,553	EASTING 2,000,270	24 HR. 4.9						
DRILL	METHOD Mu	d Rotary	IAMMER TYPE Automatic						
COMP. DATE 0	7/22/16	SURFACE WATER DEPTH	I N/A						
SAM	P. L	SOIL AND ROCK							
75 100 NO.	MOI G	SOIL AND ROCK	DESCRIPTION						
│ · · · · │ │ ─ ─		43.2 GRAY, SILTY SA	//.0						
		DARK GRAY,	SILTY CLAY						
		38.2 DARK GRAY, S	82.0 SANDY CLAY						
	w 🕅								
· · · ·									
	W	30.2	90.0						
		Boring Terminated a COASTAL PLAIN	I, SANDY CLAY						
	F								
	F								
	<u> </u>								

GEOTECHNICAL BORING REPORT BORE LOG

OCLUME ELV: 1027.1 DTFAL DEPTH: 0.01 MORTHMO 270.500 PARTINE TWO: 102.4 TOTAL DEPTH: 100.1 No DBILL BOXAMDERTICATION HUMBLY CHARGE WARKING BULL PARAGEMENT ALL START DATE: 072.10 BULL PARAGEMENT ALL START DATE: 0.00	BORE LOG																												
DRNIN D. ED-3 STATION 1:4:0 OPFSET 30:1 RT ALIONEMT																	-												
COLLAR ELEV. 100 / LAR ELE						STA. 30+39.23 OVER -L- (US 74)					AT STA. 55+06.59 GROUND WTR (f		WTR (ft)	SITE	DESCR	IPTION	PRC	POS					STA.						
DBILL REMAININGE EFF.DATE BRIVESH CXE-450 UPK 12282216 DBILL METHOD MAIMER TYPE Autmine DBILL REMAININGE EFF.DATE BRIVESH CXE-450 UPK 12282216 COMP. DATE 07221/6 SURFACE WATER DEPTH NA DBILL REMAINER BRIVESH CXE-450 UPK 12282216 COMP. DATE 07221/6 SURFACE WATER DEPTH NA DBILL REMAINER BILL METHOD Multiple Multiple SURFACE WATER DEPTH NA UPK BILL METHOD Multiple SURFACE WATER DEPTH NA UPK BILL METHOD Multiple SURFACE OPTHON UPK FILM METHOD BILL METHOD SURFACE OPTHON UPK FILM METHOD SURFACE OPTHON SURFACE OPTHON UPK FILM METHOD SURFACE OPTHON SURFACE OPTHON SURFACE OPTHON UPK FILM METHOD SURFACE OPTHON SURFACE OPTHON SURFACE OPTHON SURFACE OPTHON UPK SURFACE SURFACE OPTHON SURFACE OPTHON						OFFSET 38 ft RT					ALIGNMENT -Y- 0 HR. 3			3.4	BORING NO. EB2-B									OF					
DRILLER Anderson. J. START DATE 07/23/16 COMP. DATE 07/23/16 SURFACE WATER DEPTH NA ELEV Direction BLOWS FERFOOT BLOWS FERFOOT <th colspan="5">COLLAR ELEV. 120.2 ft TOTAL DEPTH 90.0 ft</th> <th>NORTH</th> <th colspan="5">NORTHING 273,593</th> <th colspan="3">EASTING 2,000,144 24 HR. 3.</th> <th>COLL</th> <th>AR ELE</th> <th>IV. 12</th> <th>0.2 ft</th> <th></th> <th colspan="3">TOTAL DEPTH 90.0 ft</th> <th>NC</th>	COLLAR ELEV. 120.2 ft TOTAL DEPTH 90.0 ft					NORTH	NORTHING 273,593					EASTING 2,000,144 24 HR. 3.			COLL	AR ELE	IV. 12	0.2 ft		TOTAL DEPTH 90.0 ft			NC						
Inc.U Device (1) Device (1) <thdevice (1) Device (1) Device (1)</thdevice 	DRILL	RIG/HA	MMER E	FF./DA	TE BF	R18284	CME-45C	89% 02/	/26/2016	6		DRI	LL ME	ETHOD	D Mu	ud Rotary	HAMN	IER TYPE AU	utomatic	DRILL	. RIG/HAI	MMER E	FF./DA	TE BF	R18284	CME-45C 89	% 02/26/20	16	
Image: Application of the content o	DRIL	LER A	nderso	n, J.		S	TART D	ATE 0	7/23/1	6	COMP.	DATE	07/2	3/16		SURFA	CE WATER DEPTH N	/A		DRIL	LER A	ndersoi	n, J.		S		E 07/23/	16	CC
(i) (IDEFIN					BL	LOWS F	PER FOOT			MP.	▼∕			SOIL AND ROCK DES	CRIPTION			DRIVE		BLC	w col	JNT		BLOWS	PER FOC	т
120 102 1	(ft)		(ft)	0.5ft	0.5ft	0.5ft	0	25	5	50	75 1	00 N	0.			ELEV. (ft)			DEPTH (ft)	(ft)		(ft)	0.5ft	0.5ft	0.5ft	0 :	25	50	75
120 102 1																													
100 100 <td>125</td> <td></td> <td>Ļ</td> <td></td> <td>_</td> <td></td> <td></td> <td></td> <td>45</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td> </td> <td>Mate</td> <td>ch Line</td> <td></td>	125		Ļ													_				45							Mate	ch Line	
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International constraints Intern	100		‡												þ	120.2	GROUND SURF	ACE	0.0	10	41.7 -	- 78.5 -	10	14	14		$1 \cdot \cdot$	· · · ·	
1167 3.6 </td <td>120</td> <td>119.2</td> <td>1.0</td> <td>2</td> <td>4</td> <td>4</td> <td> </td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>-</td> <td>COASTAL PLA</td> <td>AIN</td> <td>0.0</td> <td>40</td> <td>-</td> <td>-</td> <td></td> <td></td> <td></td> <td> </td> <td></td> <td>+</td> <td></td>	120	119.2	1.0	2	4	4	 									-	COASTAL PLA	AIN	0.0	40	-	-						+	
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112 6.0 1 5 5 1117 8.6 1 5 5 110 1 5 5 5 5 100 1 5	115		ŧ	2	3	6	· • • 9 ·						5-19		N	114.7	GRAY-RED, SILTY	CLAY	5.5	35	-	-	6	8	21		4 29 · ·		
1117 8.5 - <td></td> <td>114.2</td> <td>1 6.0 I</td> <td>1</td> <td>5</td> <td>5</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>Sat.</td> <td></td> <td></td> <td>GRAY, FINE SA</td> <td>ND</td> <td></td> <td></td> <td>-</td> <td>_</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>: :</td>		114.2	1 6.0 I	1	5	5								Sat.			GRAY, FINE SA	ND			-	_							: :
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100 96.7 22.5 96.7 22.5 90 90.7 22.6 90.7 22.6 90.7 22.6 90.7 22.6 90.7 23.5 91.7 28.5 92.0 91.7 91.7 28.5 92.0 91.7 <t< td=""><td>105</td><td>100.7 ·</td><td>+ 13.5 +</td><td>4</td><td>2</td><td>2</td><td>$\left \left \begin{array}{c} \mathbf{I} \\ \mathbf{\Phi}_{4} \\$</td><td></td><td></td><td></td><td></td><td>. </td><td></td><td>Sat.</td><td></td><td></td><td></td><td></td><td></td><td></td><td>-</td><td>-</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	105	100.7 ·	+ 13.5 +	4	2	2	$\left \left \begin{array}{c} \mathbf{I} \\ \mathbf{\Phi}_{4} \\ $.		Sat.							-	-							
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100 -			Ŧ						· · · ·		.	.			N						-	-							
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86.7 33.5 - </td <td>90</td> <td></td> <td>‡</td> <td>6</td> <td>7</td> <td>3</td> <td>• •10</td> <td>, · ·</td> <td></td> <td></td> <td>· · · ·</td> <td>· </td> <td></td> <td>Sat.</td> <td></td> <td>—</td> <td></td> <td></td> <td></td> <td></td> <td>-</td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	90		‡	6	7	3	• •10	, · ·			· · · ·	·		Sat.		—					-	-							
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		76.7	43.5	4	10	10	$\left \left \begin{array}{c} \cdot \cdot \end{array} \right \right $	<u>\.</u>			.	:		101	\sim		GRAY-TAN, CLAYE	Y SAND			-								
70 71.7 48.5 2 2 6 66.7 53.5 3 4 7 66.7 53.5 3 4 7 61.9 58.3 9 16 22 56.7 63.5 10 13 17 56.7 63.5 10 13 17 60.7 73.5 10 10 13 17 60.7 73.5 10 10 16 22 57.0 60.7 73.5 10 10 18 17 60.5 15 61.7 73.5 10 10 16 7.7 7.7 7.7 46.7 73.5 10 10 16 7.7 7.7 7.7 7.7		-	Ŧ					P 20 /						vv	\sim	-					_	Ŀ							
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	45		+	10	10	16	<u>] </u>	· •26				·		W	\mathbf{Y}						-								

SHEET 7

NT	r ROBESC	ON			GEOLOGIS	T Rohit Wa	rrier									
S	TA. 30+39.2	3 OVER	R -L- (l	JS 74)	AT STA. 55	+06.59		GROUN	D WTR (ft)							
	OFFSET 3	38 ft RT			ALIGNMEN	IT -Y-		0 HR.	3.4							
	NORTHING	3 273,5	93		EASTING	2,000,144		24 HR.	3.2							
		DRILL N	IETHO	D Mud	Rotary HAMMER TYPE Automatic											
	COMP. DA	TE 07/2	23/16		SURFACE	WATER DEP	TH N//	4								
ОТ		SAMP.	\square	L O		SOIL AND ROCK DESCRIPTION										
	75 100	NO.	моі													
			L													
•••					DA	RK GRAY, SILT	TY CLAY	(continue	d)							
· ·			w	N												
	<u> </u>		~~	\mathbb{N}												
					38.2	DARK GRAY	, SANDY	CLAY	82.0							
•••			w													
· ·																
0.	••••		W		30.2 Bi	oring Terminated	d at Eleva	ation 30.2	90.0 ft							
						COASTAL PLA	IN, SANI	DY CLAY								
					Other	<u>Samples:</u> -2 (18.0 - 20.0)										
					51	-2 (10.0 - 20.0)										
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SOIL TEST RESULTS

EB1–B

	SOIL TEST RESULTS														
SAMPLE NO.							C.SAND	% BY V F.SAND	VEIGHT SILT	CLAY	% PAS 10	SING (S 40	IEVES) 200	% MOISTURE	% ORGANIC
ST-I	35' RT	28+85	17.0-19.0	A-2-4(0)	23	NP	28	44	7	21	100	76	28	23.0	-

EB2-B

	SOIL TEST RESULTS														
SAMPLE	OFFSET	STATION	DEPTH	AASHTO CLASS.	ττ	<i>P.I.</i>		% BY	VEIGHT		% PASSING (SIEVES)			%	%
NO.	OFFSEI	STATION	INTERVAL		L, L.		C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-19	38' RT	31+30	3.5-5.0	A-7-6(53)	84	62		20	13	66	100	99	80	37.8	-
ST-2	38' RT	31+30	18.0-20.0	A-2-4(0)	21	NP	25	63	5	7	100	78	13	71.4	-

SHEET 8 (R-5752)