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QUANTIT	IES
LUMP	SUM
644	TONS
1,001.2	_C.Y.
21.4	_C.Y.
3.0	_C.Y.
1,025.6	_C.Y.
78,974	LBS.
1,321	_LBS.
80,295	LBS.
· · · ·	QUANTIT LUMP 644 1,001.2 21.4 3.0 1,025.6 78,974 1,321 80,295

FOR OTHER DESIGN DATA AND NOTES, SEE STANDARD NOTES SHEET. 3" Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS. CONCRETE IN CULVERTS TO BE POURED IN THE FOLLOWING ORDER: 1. WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL

HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALLS.

2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED IN THE BARREL, SPACED TO LIMIT THE POURS TO A MAXIMUM OF 70 FT.LOCATION OF JOINTS SHALL BE SUBJECT TO

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS.EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR. NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED. FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS. FOR CULVERT EXCAVATION, SEE SECTION 414 OF THE STANDARD SPECIFICATIONS.

THE REINFORCED CONCRETE BOX CULVERT SHALL BE PLACED ON THE STANDARD 1.0 FOOT BLANKET OF FOUNDATION CONDITIONING MATERIAL.

THE REQUIRED BEARING CAPACITY AT THE BASE OF THE CULVERT IS 3 TSF. THE REQUIRED BEARING CAPACITY SHALL BE VERIFIED.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

THE REINFORCED CONCRETE BOX CULVERT SHALL BE CONSTRUCTED WITH 6 INCHES OF CAMBER TO ACCOUNT FOR ANTICIPATED SETTLEMENT.

F. A. PROJECT No.: NHP-0918(062)

A Kuith Dawhal	PROJEC	T NO. FORS DN: <u>39</u>	<u>U</u> YTH 97+73	-2579 co .00 -)C UNTY -L-
F3B6AD6DB2FC48F F3B6AD6DB2FC48F F3B6AD6DB2FC48F F3B6AD6DB2FC48F F3B6AD6DB2FC48F F3B6AD6DB2FC48F F3B6AD6DB2FC48F	DEPA	RTMENT	TE OF NORTH CAN OF TRAN RALEIGH	ISPORTAT	TON FT.
Docusingente: G. CHLINN Marshall G. Chlinn 6549D6EBAA3B405 7/27/2017	CONC	RETE	BOX	CUL	VERT
	NO. BY:	DATE:	NO. BY:	DATE:	C1-1
FINAL UNLESS ALL SIGNATURES COMPLETED	1		3		TOTAL SHEETS 5
	STR.#5)			

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS																
										STRENGTH	I LIM	IT ST	ATE			
								MOMENT SHEAR					SHEAR			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f+)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f+)	COMMENT NUMBER
		HL-93 (INVENTORY)	NZA	$\langle 1 \rangle$	3.52		1.75	5.35	1	EXTERIOR WALL	4.19	3.52	1	EXTERIOR WALL	6.99	
DESIGN		HL-93 (OPERATING)	N/A		4.56		1.35	6.93	1	EXTERIOR WALL	4.19	4.56	1	EXTERIOR WALL	6.99	
RATING	HS-20 (INVENTORY)	36.000	2	3.52	126.61	1.75	5.35	1	EXTERIOR WALL	4.19	3.52	1	EXTERIOR WALL	6.99		
		HS-20 (OPERATING)	36.000		4.56	164.13	1.35	6.93	1	EXTERIOR WALL	4.19	4.56	1	EXTERIOR WALL	6.99	
		SH	12 . 500	3	4.40	54.95	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
		S3C	21.500		4.40	94.52	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
	ICLE	S3A	22.750		4.40	100.01	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
	H A A A A A A A A A A A A A A A A A A A	S4A	26.750		4.40	117.60	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
	C S C C S	S5A	30.500		4.40	134.09	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
	DNI	S6A	34.500		4.40	151.67	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
		S7B	38.500		4.40	169.26	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
NATING		S7A	40.000		4.40	175.85	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
	Ra	Τ4Δ	28.250		4.40	124.19	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
		T5B	32.000		4.40	140.68	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
	TR/ -TR/ TTS1	Т6А	36.000		4.40	158.27	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
	RUCK SEMI	Τ7Α	40.000		4.40	175.85	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	
		Т7В	40.000		4.40	175.85	1.40	6.69	1	EXTERIOR WALL	4.19	4.40	1	EXTERIOR WALL	6.99	

ASSEMBLED BY : N.D'AIUTO CHECKED BY : H.A.LOCKLEAR	DATE DATE	: 3/31/16 : 4/6/16
DRAWN BY : WMC 7/II CHECKED BY : GM 7/II	REV. 10/1/11	MAA/GN

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LRFR SUMMARY

LOAD FACTORS

DESIGN LOAD	RAIING	FACIORS
LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	
WA	1.00	

DESTON LOAD DATTNE EACTORS

NOTE

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

CONTROLLING LOAD RATING 1 DESIGN LOAD RATING (HL-93) 2 DESIGN LOAD RATING (HS-20) 3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE





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DETAIL										
NECTION	OF WING	FOOTING								
ND FLOOR	SLAB WH	EN SLAB								
S THICKE	r than f	OOTING								







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NOTES MATERIAL EXCAVATED FROM THE EXISTING BED SHALL BE STOCKPILED FOR USE IN THE PROPOSED CULVERT AS SHOWN IN THE "PLAN VIEW".

THE ENTIRE COST OF WORK REQUIRED TO CONSTRUCT THE SILLS SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.



VERTICAL LEG 1 6'' R. $A_{1, A2}$ $2'-3^{1/2''}$ BAR DIMENSIONS ARE OUT TO OUT.										
	BII	_L 0	F MA	ATERIA	L					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT					
A1	2164	#4	1	6'-3"	9035					
Δ2	2164	#4	1	5'-5"	7830					
A100	1306	# 5	STR.	7′-6″	10216					
A101	4	#5	STR.	5′-7″	23					
A102	4	# 5	STR.	3'-7"	15					
A200	1306	# 5	STR.	7'-6″	10216					
A201	4	# 5	STR.	5'-7"	23					
A202	4	# 5	STR.	3'-7"	15					
R1	1532	#⊿	STR	9'-3"	9466					
B2	2164	# ⊿	STR	<u> </u>	9155					
02	2104		511.	0 1	5155					
C1	1120	#4	STR.	29'-7"	22133					
01										
D1	54	#6	STR.	1'-8"	135					
F1	153	#4	STR.	4'-0"	409					
G1	4	#4	STR.	8′-9″	23					
S2	12	# 8	STR.	8′-9″	280					
REIN	NFORCI	NG ST	EEL	LBS.	78,974					

BAR TYPE

SPLICE LENGTH CHART									
BAR	SIZE	SPLICE LENGTH							
B1	#4	1′-5″							
C1	#4	1'-11"							

	PF - S1		CT NO. FORS N: 39		<u>U</u> TH '+73	- <u>2579</u> cc	<u>9C</u> DUNTY -L-				
	SH	EET 4 C)F 5								
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH										
	C	SIN Conc	IGLE CRETE	6	FΤ. BOX	X 7 CUL	FT. VERT				
7	REVISIONS SHEET NO										
RED	NO.	BY:	DATE:	NO.	BY:	DATE:	C1-4				
- D	1 2			3 A			TOTAL SHEETS 5				

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A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE



TYPICAL WIN SECTION



RAR TYPES		ΒTI	I OF	ΜΔ	τερτω				
JAR THES						-			
	BAR	<u>NO.</u>	SIZE	ΙΥΡΕ	LENGTH	WEIGHT			
\sim	H1	12	#4	STR	7'-1"	57			
	H2	4	#4	STR	5'-2"	14			
	Н3	24	#4	1	3'-3"	52			
√/ / <u>×</u>	μΔ	<u>_</u> . Δ	#⊿	- STP	7'-9"	21			
		12	# ∕		· J 17/_10″	111			
	CTT		+ 4 ++ 4		10/ ///				
\sim / $ $	Hb	4	#4	SIK	10'-4"	28			
	H7	4	#4	SIR	4'-3"	11			
· 1'-0"	H8	24	#4	2	3'-3"	52			
	Н9	4	#4	STR	14'-3"	38			
N									
0"	N1	4	#5	٦	9'-1"	38			
2'-0	NIC		<u> </u>		9'_6"	30			
		4		ر -	0-0	35			
, İ	N3	4	#4	3	('-("	20			
	N4	4	#4	3	6'-9"	18			
	N5	4	#4	3	5′-10″	16			
1/ 03/ //	N6	4	#5	3	9'-4"	39			
	N7	6	#5	3	8′-10″	55			
	N8	8	#4	ч	7'-10"	42			
	NQ		, #∕	<u></u>	6'-10"	37			
			# 4	J 7	6 -10 E(10"	- J1 - 71			
† † † † † † † † † †			<u></u>	3	01- C				
	S1	12	#6	STR	6'-0"	108			
	T1	6	#5	STR	9′-0″	56			
	T2	6	# 5	STR	15′-9″	99			
		1							
	١/1	Δ	#⊿	STP	7'-1"	10			
					۲ I ۲ - ۲ - ۲ - ۲	17			
× /	V Z	4	··· 4						
12/	V 3	4	*4	SIK) - ("	15			
	V 4	4	#4	SIR	4'-8"	12			
	V5	4	#4	STR	3'-10"	10			
· • • · · · · · · · · · · · · · · · · ·	V6	4	#4	STR	7'-4"	20			
-9 0	٧7	6	#4	STR	6'-9"	27			
<u>′-3″6″ _</u>	V8	8	#4	STR	5′-9″	31			
·-5" 6"	V9	8	#4	STR	4'-9"	25			
	V10	8	# 4	STR	3'-10"	20			
<u>'-("</u>	V10		1	511	5 10	20			
′-6″ 6″	71		# 4	1	Г/ 7//	1.4			
	<u> </u>	4	#4	4	5'-3"	14			
	Z2	4	#4	4	4'-9"	13			
′-3″ 6″	Z3	6	#4	4	3'-11"	16			
	Z4	6	#4	4	3'-1"	12			
	Z5	10	#4	4	5′-0″	33			
	76	8	#4	4	4'-4"	23			
	77		, #∕	1	3'_9"	20			
) нк	70			- Т - <	J J Z/ 1//	20			
	20	0	- 4	4	5-1	01			
	REIN	IFORCI	NG STE	EL	LBS	5. 1,321			
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IENSIONS ARE OUT TO OUT.	CLAS			E		10.0			
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		2 HEAL	JWALLS			. 0.9			
		Z END	CURIA	TN WA		0.7			
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<u>SHEE</u>	150F	5							
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DEPARIMENT OF TRANSPORTATION									
STAN OF CESSION NA TH	.) RALEIGH								
	STANDARD WINGS								
SEAL 20125				١R					
20125 FOR									
THE STAN MOINE	CONCRETE ROX CHIVER								
DocuSigned by		· — · L ~ "							
U Marshall G. Churk, Jr. Ⅰ H	= ('-	υ"			SLUPE	= 2:1			
6549D6EBAA3B405	G	$\sim \sim$		20°	<kum Kum</kum 				
7/27/2017	Ċ			<u> </u>	JNEW				
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FINAL UNLESS ALL			3		IF	TOTAL SHEETS			
						5			

STR.#5 STD. NO. CW6007



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	ROADWAY DATA	NOTES
	GRADE POINT ELEV. @ STA. 437+96.50 -L- = 901.03 BED ELEV. @ STA. 437+96.50 -L- = 893.57 ROADWAY SLOPES = 2:1	ASSUMED LIVE L DESIGN FILL = -
	HYDRAULIC DATA	3"Ø WEEP HOLES
OODS	DESIGN DISCHARGE = 385 C.F.S. FREQUENCY OF DESIGN FLOOD = 50 YR. DESIGN HIGH WATER ELEV. = 902.3 DRAINAGE AREA = 0.259 SQ. MI. BASE DISCHARGE (Q100) = 445 C.F.S BASE HIGH WATER ELEV. = 902.9	CONCRETE IN CL 1.WING FOO OF ALL N 2.THE REMAI HEIGHT FO
	OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING FLOOD OVERTOPPING FLOOD ELEV. SOUTH STREAM STR	THE RESIDENT E IT OUT TO MAKE DIMENSIONS FOR EMBEDDED IN BA
		TRANSVERSE CON THE POURS TO A APPROVAL OF TH
II RIP RAP AY DETAIL Y ITEM)	TOTAL STRUCTURE QUANTITIESCULVERT EXCAVATIONLUMP SUM	IN THE INTERIC ABOVE LOWER WA IN THE SPLICE TO THE SPLICES
WOODS	FOUNDATION CONDITIONING MATERIAL	NO PRECAST REI FOR CULVERT DI
	CLASS A CONCRETE BARREL @ 1.677 CY/FT 560.1 C.Y. WINGS, ETC. 23.1 C.Y. SILLS 0.6 C.Y. TOTAL 583.8 C.Y.	FOR SUBMITTAL FOR FALSEWORK FOR CRANE SAFE FOR GROUT FOR
SIONS.	REINFORCING STEEL BARREL @ <u>48,618</u> LBS. WINGS,ETC. <u>1,463</u> LBS. TOTAL <u>50,081</u> LBS.	THE CONTRACTOF REINFORCING ST OF REINFORCING FOR PROJECTS R INCH SAMPLES C ARE TAKEN MUST LENGTH OF THF



_OAD = HL-93 OR ALTERNATE LOADING. 46.87 FEET. IGN DATA AND NOTES, SEE STANDARD NOTE SHEET. S INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS. CULVERTS TO BE POURED IN THE FOLLOWING ORDER: OTINGS AND FLOOR SLAB INCLUDING 4" VERTICAL WALLS. INING PORTIONS OF THE WALLS AND WINGS FULL OLLOWED BY ROOF SLAB AND HEADWALLS. INGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING KE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL. R WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL ARREL ARE SHOWN ON WING SHEET. INSTRUCTION JOINTS SHALL BE USED IN THE BARREL, SPACED TO LIMIT A MAXIMUM OF 70 FT.LOCATION OF JOINTS SHALL BE SUBJECT TO HE ENGINEER. CTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL OR FACE OF EXTERIOR WALL AND BOTH FACES OF INTERIOR WALLS VALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED LENGTH CHART SHOWN ON THE PLANS.EXTRA WEIGHT OF STEEL DUE SHALL BE PAID FOR BY THE CONTRACTOR. INFORCED BOX CULVERT OPTION WILL BE ALLOWED. IVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS. OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS. AND FORMWORK, SEE SPECIAL PROVISIONS. ETY, SEE SPECIAL PROVISIONS. STRUCTURES, SEE SPECIAL PROVISIONS. R SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF TEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS S STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND EQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS. THE REINFORCED CONCRETE BOX CULVERT SHALL BE PLACED ON THE STANDARD 1.0 FOOT BLANKET OF FOUNDATION CONDITIONING MATERIAL. SEE SECTION 414 OF THE STANDARD SPECIFICATIONS. CONSTRUCT THE REINFORCED CONCRETE BOX CULVERT WITH 9 INCHES OF CAMBER TO ACCOUNT FOR ANTICIPATED SETTLEMENT. UNDERCUT SOFT/LOOSE SOILS THAT MAY BE ENCOUNTERED BENEATH THE BOTTOM OF THE FOUNDATION CONDITIONING MATERIAL. BACKFILL UNDERCUT AREAS WITH FOUNDATION CONDITIONING MATERIAL. 25'-0"± PROJECT NO. U-2579C FORSYTH COUNTY 437+96.50 -L-STATION: SHEET 1 OF 5 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SEAL SINGLE 8 FT.X 8 FT. 20125 CONCRETE BOX CULVERT VCINE Marshall G. 7/27/2017 90° SKEW 3549D6FBAA3B40 7/27/2017 SHEET NO REVISIONS C2-1 DATE: DATE: NO. BY: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS 5

STR #6

STD. NO. CB11A

F.A. PROJECT No.: NHP-0918(062)

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS																
										STRENGTH	I LIM	IT ST	ATE			
								MOMENT				SHEAR				
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f+)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f+)	COMMENT NUMBER
		HL-93 (INVENTORY)	NZA		4.16		1.75	5.44	1	BOT.CORNER WALL	8.75	4.16	1	EXTERIOR WALL	7.98	
DESIGN		HL-93 (OPERATING)	NZA		5.39		1.35	7.05	1	BOT.CORNER WALL	8.75	5.39	1	EXTERIOR WALL	7.98	
RATING	HS-20 (INVENTORY)	36.000	2	4.16	149.81	1.75	5.44	1	BOT.CORNER WALL	8.75	4.16	1	EXTERIOR WALL	7.98		
		HS-20 (OPERATING)	36.000		5.39	194.20	1.35	7.05	1	BOT.CORNER WALL	8.75	5.39	1	EXTERIOR WALL	7.98	
		SH	12.500	3	5.20	65 . 02	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
	ш	S3C	21.500		5.20	111.84	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
	ICLI	S3A	22.750		5.20	118.34	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
	AEH (\	S4A	26.750		5.20	139 . 15	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
	C) (S)	S5A	30.500		5.20	158.66	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
	INC	S6A	34.500		5.20	179.46	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
		S7B	38.500		5.20	200.27	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
		S7A	40.000		5.20	208.07	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
	ж ^ж	Τ4Α	28.250		5.20	146.95	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
	ACT(AILE [)	Т5В	32.000		5.20	166.46	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
	 TR/ TR/ TS1 	ТбА	36.000		5.20	187.27	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
	RUCK SEMI	Τ7Α	40.000		5.20	208.07	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	
		Т7В	40.000		5.20	208.07	1.40	6.80	1	BOT.CORNER WALL	8.75	5.20	1	EXTERIOR WALL	7.98	

ASSEMBLED BY : N.D'AIUTO		: 3/30/16
CHECKED DI : H.A.LUCKLEAN	DATE	: 4/0/10
DRAWN BY : WMC 7/II	REV. 10/1/11	MAA/GN
CHECKED BY : GM 7/II		

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(LOOKING DOWNSTREAM)

LOAD FACTORS

DESIGN LOAD RAIING FACTORS				
LOAD TYPE	MAX FACTOR	MIN FACTOR		
DC	1.25	0.90		
DW	1.50	0.65		
EV	1.30	0.90		
EH	1.35	0.90		
ES	1.35	0.90		
LS	1.75			
WA	1.00			

DESTON LOAD DATTNE EACTORS

NOTE

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

CONTROLLING LOAD RATING 1 DESIGN LOAD RATING (HL-93) 2 DESIGN LOAD RATING (HS-20) 3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE





CULVERT SECTION NORMAL TO ROADWAY



BY BY					
E.L.R. CHECKED A.R.B. CHECKED					
SED 8-28-92 BY SED 8-22-89 BY AWN 8-22-1989					
EVIS EVIS EDR	DRAWN BY :	N.D'AI	DATE : .	3/30/16	
~ ~ ~ ~	CHECKED BY :	H.A. LO(CKLEAR	DATE :.	4/6/16
	DESIGN ENGINEER	OF RECORD:	J.K. BOWLES	DATE :	4/6/16

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G.R.P. C.R.K.





END ELEVATION





mcheek

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	BAR TYPE					
VERTICAL LEG 1 6'' R. A1, A2 $3'-1^{1/2''}$ $3^{1/2}$ 7 7 7 7 7 7 7 7						
			$\frac{S ARE}{F M}$	ATERTA	iui.	
BAD					WETCHT	
	944	# <u></u>	1	7'-2"	4519	
A2	944	#4	1	6'-4"	3994	
A100	890	# 5	STR.	9′-7″	8896	
A200	1002	# 5	STR.	9'-7"	10015	
					46.40	
	668	#4	SIR.	10'-5"	4648	
	942	* 4	SIR.	('-4"	4615	
	588	#⊿	STR	29'-9"	11685	
	U 300 "4 SIK. 29'-9" 11685					
D1	6	# 6	STR.	2'-1"	19	
F1	68	#4	STR.	4'-5"	201	
G1	G1 4 # 4 STR. 9'-8" 26					
REIN	NFORCI	NG ST	EEL	LBS.	48,618	

Ν	0	Т	Ε	S

SPLICE LENGTH

1′-5″

1'-11"

MATERIAL EXCAVATED FROM THE EXISTING BED SHALL BE STOCKPILED FOR USE IN THE PROPOSED CULVERT AS SHOWN IN THE "PLAN VIEW". BED MATERIAL SHALL BE SUPPLEMENTED WITH CLASS "B" RIP RAP AS NECESSARY. IF CLASS "B" RIP RAP IS USED, NATURAL MATERIAL SHALL BE PLACED ON TOP AND LEVELED TO FACILITATE ANIMAL PASSAGE. BED MATERIAL SHALL BE SUBJECT TO APPROVAL BY THE ENGINEER. THE ENTIRE COST OF WORK REQUIRED TO PLACE THE EXCAVATED MATERIAL OR SUPPLEMENTAL MATERIAL AS SHOWN ON THE PLANS SHALL BE INCLUDED IN THE CONTRACT LUMP SUM PRICE BID FOR CULVERT EXCAVATION. THE ENTIRE COST OF WORK REQUIRED TO CONSTRUCT THE SILLS SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.





R TYPES		BIL	L OF	MA	IERIAL	-
STONS ARE OUT TO OUT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
SIONS ARE OUT TO OUT.	H1	24	#4	STR	10'-10"	174
λ.	H2	8	#4	STR	7'-8"	41
	Н3	8	#4	STR	4'-1"	22
2'-0'	H4	48	#4	1	3'-3"	104
	Н5	8	#4	STR	11'-9"	63
	N1	8	#5	2	10'-2"	85
	N2	12	# 5	2	9'-2"	115
1'-93' "	N3	12	#4	2	7'-11"	63
	N4	12	#4	2	6'-7"	53
	N5	12	#4	2	5'-4"	43
	<u>S1</u>	12	#6	SIR	6'-0"	108
† † † † †					10/ 0/	100
		12	#5	SIR	12'-9"	160
					01.47	47
		8	#4	SIR	8'-1"	43
0//2	V2	12	#4	SIR	('-1"	51
	V3	12	#4	SIR	5'-10"	47
	V4	12	#4	STR	4'-7"	37
	V5	12	#4	STR	3'-4"	27
<u> </u>	Z1	8	#5	<u>3</u> -	6'-0"	50
<u>,</u>	Z2	12	# 5	3	5'-5"	68
	<u> </u>	12	#4	<u>5</u>	4'-("	51
		12	#4	<u>5</u>	5'-10"	<u>51</u>
·	25	12	# 4	ا ک	51.	25
	REINF	ORCIN	IG STFI	EL		
	FOR 4	1 WING	is		LBS	5. 1,453
<u> </u>				_		
4'-10" 7"	CLASS	S A CU 4 WINC		<u>-</u>	CΥ	21.4
		2 HEAD	WALLS		C.Y	. 0.9
4'-1" 6"		2 END	CURTA	IN WAI	LS C.Y	. 0.8
7/ ///				тот	AL C.Y	. 23.1
2'-7"						
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DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SO.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN	
UF IIMDER	JIJ LOS. PER SU. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FI.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

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DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS,AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER

SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

