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STATE	STATE	PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS		
N.C.		<u>B-4964</u>	1			
STAT	E PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	10N		
402	242.1.1_	BRSTP-2600(1)	PE			
402	242.2.1	BRSTP-2600(1)	R/W			
402	242.3.1 _	BRSTP-2600(1)	CONST.			

Prepared in the Office of: DIVISION OF HIGHWAYS STRUCTURES MANAGEMENT UNIT 1000 BIRCH RIDGE DR. RALEIGH, N.C. 27610							
ING DATE : August 15, 2017							





F.A. PROJECT NO. BRSTP-2600(1)

+1.5693% -1.5513%

P.I.STA.= 21+88.00 EL.= 768.00 V.C.= 401′

<u>GRADE DATA -L-</u>

- APPROXIMATE NATURAL GROUND LINE

└─EL.759 ±

 $\rightarrow \swarrow \checkmark \checkmark \checkmark \checkmark \checkmark$

	Т)P OF	RAIL	ELEV	ATION		
	-RATI - STATION	1st RATI	2 nd	RATI	Zrd DATI	⊿th ₽∧т	
	12+25_00	733.33	73	3.24	733.20	7,33,14	· <u> </u>
	12+50.00	733.21	73	3.11	733.07	732.98	
	12+75.00	733.19	73	3.04	733.00	732.89	
	13+00.00	733.10	73	2.90	732.90	732.80	
	13+25.00	733.06	73	2.83	732.83	732.70	
	THE -RAIL-	STATION	GIVEN.	JATDED	ARE PERPEN	DICULAR I	U
_	¢ EXISTING	- 	EXISTIN	<u>G_</u>		┍┛ _■ FILL	FACE @
40'-0	3/16″		4	0'-2 <mark>3/</mark> 16″	F	LII END I	BENT 1
WALL YP.)				APPRO NATURAI	XIMATE L GROUND		
NIM	UM CLEAF	ANCE	- F	RAIL	ROAD	_	
(L	OOKING STATION SPAN LENGTHS BAS	AHEAD ALC SED ON TH	NG RAII IS SEC ⁻	_ROAD) FION			
2 IND BEN .94 -L-	<u>NT 2</u>						
	TO NC 87	<u>HOF</u>	RIZON	TAL PI ST Δ = 3 D = 5 L = T = R =	CURVE D A. 22+10.67 2°01′59.2″ 5°50′47.4″ 547.90′ 281.32′ 980.00′	<u>ATA -L</u>	
SLAB -L-					_		_
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	133 Fayetteville Stre Suite 210	eet				H TRA	
	Raleigh, NC 2760 919-714-8670			UL O	REPLA MILE STATE OF NORTH CAR	POST 265. Rolina	<u>≥ NU.8</u> 20
	meadhunt.com NC License No. F-12	235	DEPA	ARTMEN	NT OF TRA	NSPORTA	TION
	SEAL 043177 Docusigned by:		G ov RA:	ENE BRID 'ER N ILROA BUSIN	RAL DF ge on s orfolk ad betw ness ani	RAWIN R 2600 SOUTHE EEN US D NC 8	IG) IRN 29 7
_	Jack Hobson 6/1,	/2017		RE	VISIONS		SHEET
[DOCUMENT NOT CONS FINAL LINEFSS A		BY:	DATE:	NO. BY:	DATE:	S-01
L	SIGNATURES COMPL	eted 1)		<u>- জ</u> নি		SHEETS
			1		<u> </u>		UL 10



FOUNDATION NOTES:





	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	HP Stee	12 X 53 L PILES	VERTICAL CONCRETE BARRIER RAIL	4″ CONCRETE SLOPE PROTECTION	ELASTOMERIC BEARINGS	3'-(PRE C(B0	D'' X 3'-3" STRESSED DNCRETE X BEAMS	ASBESTOS ASSESSMENT
	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	EACH	NO.	LIN.FT.	LIN.FT.	SQ. YDS.	LUMP SUM	NO.	LIN.FT.	LUMP SUM
SUPERSTRUCTURE				LUMP SUM					206.8		LUMP SUM	11	1,137.58	LUMP SUM
END BENT NO. 1		LUMP SUM	21.5		3,192	8	8	456		10				
END BENT NO. 2		LUMP SUM	21.5		3,192	8	8	496		10				
TOTAL	LUMP SUM	LUMP SUM	43.0	LUMP SUM	6,384	16	16	952	206.8	20	LUMP SUM	11	1,137.58	LUMP SUM

DRAWN BY :	J.S. HOBSON	DATE : 07/07/16
CHECKED BY :	J.A. LEE	DATE : 07/11/16
DESIGN ENGINEER	OF RECORD :	. DATE : 07/11/16

NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK. SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

THE RAILROAD TRACK TOP OF RAIL ELEVATIONS SHOWN ON THE PLANS ARE FROM THE BEST INFORMATION AVAILABLE.PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE TOP OF RAIL ELEVATIONS AND REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 83 ± FT AT END BENT 1, AND 78 ± FT AT END BENT 2. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 18+73.69".

THE EXISTING STRUCTURE CONSISTING OF 6 REINFORCED CONCRETE DECK SPANS, 1 @ 21'-3", 2 @ 21'-0", 1 @ 35'-0", 1 @ 21'-0", AND 1 @ 21'-3", ON STEEL I-BEAMS AND SUPPORTED ON END BENTS AND INTERIOR BENTS CONSISTING OF REINFORCED CONCRETE CAPS ON TIMBER PILES, AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION, SEE SPECIAL PROVISIONS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITIES ON ROADWAY PLANS.

FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.

Mead Section133 Fayetteville Street Suite 210Palaiah NG 27601	PROJEC RO STATIC	CT NO. <u>CKIN</u> DN: <u>1</u> 12+	<u>B</u> <u>GHAM</u> 8+73. -70.54	-4964 co .69 -l 4 -RA	1 OUNTY _IL-		
919-714-8670 meadhunt.com NC License No. F-1235	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						
SEAL 043177 0, MG INE FR. OTHER DocuSigned by:	G ov raj E	ENERA BRIDGE ER NOF ELROAD BUSINE	AL DF E ON S RFOLK BETWI SS ANI	RAWIN R 2600 SOUTHE EEN US D NC 8	IG) IRN 29 7		
Jack Hobson 6/1/2017		REVIS	SIONS		SHEET NO.		
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SIGNATURES COMPLETED	<u>ป</u>		3		TOTAL SHEETS		

		LOAD AN	ID RE	SIST	ANCE	FAC	TOR	RAT	ING	(LRF	R) Sl	JMMA	RY F	OR F	PRES	TRES	SED	CON	CRET	E GI	RDE	RS						
										STRE	NGTH	I LIN	IIT ST	ΓΑΤΕ				SE	RVICE	III	LIMI	t sta	ι ΤΕ					
							_	MOMENT	-	_		_	SHEAR		_			-	MOMENT	_		-						
LEVEL	WEIGHT (W) (TONS) CONTROLLING	WEIGHT (W) (TONS)	(TONS) (TONS) CONTROLLING #	CONTROLLING # LOAD RATING # MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NILIMBER					
		HL-93 (INVENTORY)	N⁄A	$\langle 1 \rangle$	1.45		1.75	0.221	1.45	А	EL	50.96	0.466	2.74	А	EL	9.59	0.80	0.221	1.45	А	EL	50.96					
DESIGN		HL-93 (OPERATING)	N⁄A		1.88		1.35	0.221	1.88	А	EL	50.96	0.466	3.62	А	EL	9.59	N/A										
RATING		HS-20 (INVENTORY)	36.000	$\langle 2 \rangle$	2.04	73.440	1.75	0.221	2.04	А	EL	50.96	0.466	3.76	А	EL	9.59	0.80	0.221	2.04	А	EL	50.96					
		HS-20 (OPERATING)	36.000		2.64	95.040	1.35	0.221	2.64	А	EL	50.96	0.466	4.94	А	EL	9.59	N/A										
		SNSH	13.500		4.85	65.475	1.40	0.221	6.07	А	EL	50.96	0.466	12.07	А	EL	9.59	0.80	0.221	4.85	А	EL	50.96					
		SNGARBS2	20.000		3.51	70.200	1.40	0.221	4.38	А	EL	50.96	0.466	8.38	А	EL	9.59	0.80	0.221	3.51	А	EL	50.96					
	ICLE	SNAGRIS2	22.000		3.28	72.160	1.40	0.221	4.10	А	EL	50.96	0.466	7.71	А	EL	9.59	0.80	0.221	3.28	А	EL	50.96					
	<u <td>SNCOTTS3</td><td>27.250</td><td></td><td>2.41</td><td>65.673</td><td>1.40</td><td>0.221</td><td>3.01</td><td>А</td><td>EL</td><td>50.96</td><td>0.466</td><td>5.89</td><td>А</td><td>EL</td><td>9.59</td><td>0.80</td><td>0.221</td><td>2.41</td><td>А</td><td>EL</td><td>50.96</td><td></td></u 	SNCOTTS3	27.250		2.41	65.673	1.40	0.221	3.01	А	EL	50.96	0.466	5.89	А	EL	9.59	0.80	0.221	2.41	А	EL	50.96					
	C (S	SNAGGRS4	34.925		1.97	68.802	1.40	0.221	2.47	А	EL	50.96	0.466	4.76	А	EL	9.59	0.80	0.221	1.97	А	EL	50.96					
	SINC	SNS5A	35.550		1.93	68.612	1.40	0.221	2.41	А	EL	50.96	0.466	4.77	А	EL	9.59	0.80	0.221	1.93	А	EL	50.96					
		SNS6A	39.950		1.76	70.312	1.40	0.221	2.19	А	EL	50.96	0.466	4.30	А	EL	9.59	0.80	0.221	1.76	А	EL	50.96					
LEGAL		SNS7B	42.000		1.67	70.140	1.40	0.221	2.09	А	EL	50.96	0.466	4.17	А	EL	9.59	0.80	0.221	1.67	А	EL	50.96					
RATING	LER	TNAGRIT3	33.000		2.14	70.620	1.40	0.221	2.67	А	EL	50.96	0.466	5.19	А	EL	9.59	0.80	0.221	2.14	А	EL	50.96					
	RAI	TNT4A	33.075		2.14	70.781	1.40	0.221	2.68	А	EL	50.96	0.466	5.09	А	EL	9.59	0.80	0.221	2.14	А	EL	50.96					
	L-IN	TNT6A	41.600		1.73	71.968	1.40	0.221	2.17	А	EL	50.96	0.466	4.37	А	EL	9.59	0.80	0.221	1.73	А	EL	50.96					
	SEI ST)	TNT7A	42.000		1.73	72.660	1.40	0.221	2.17	А	EL	50.96	0.466	4.30	А	EL	9.59	0.80	0.221	1.73	А	EL	50.96					
	CTOF (TT	TNT7B	42.000		1.77	74.340	1.40	0.221	2.22	А	EL	50.96	0.466	4.10	А	EL	9.59	0.80	0.221	1.77	А	EL	50.96	\square				
	TRA	TNAGRIT4	43.000		1.70	73.100	1.40	0.221	2.13	А	EL	50.96	0.466	3.97	А	EL	9.59	0.80	0.221	1.70	А	EL	50.96					
	JCK	TNAGT5A	45.000		1.61	72.450	1.40	0.221	2.02	А	EL	50.96	0.466	3.90	А	EL	9.59	0.80	0.221	1.61	А	EL	50.96					
	TRI	TNAGT5B	45.000	3	1.60	72.000	1.40	0.221	2.00	А	EL	50.96	0.466	3.78	A	EL	9.59	0.80	0.221	1.60	А	EL	50.96					



DRAWN BY :	J.S. HOBSON	DATE: 06/28/16
CHECKED BY :	J.A. LEE	DATE: 07/20/16
DESIGN ENGIN	EER OF RECORD :J.S. HOBSON	_ DATE : <u>07/20/16</u>



LRFR SUMMARY

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

COI

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

1. 2.

3.

4.

(#) CONTROLLING LOAD RATING 1 design load rating (HL-93) $\left< 2 \right>$ design load rating (HS-20) 3 LEGAL LOAD RATING ** ** SEE CHART FOR VEHICLE TYPE GIRDER LOCATION I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER



PROJEC <u>RC</u> STATI	CT NO. O <u>CKIN</u> ON: <u>1</u> 12+	<u>B</u> <u>GHAM</u> 8+73. 70.54	-4964 co 69 -l 1 -RA	1 UNTY IL-						
DEPA	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD									
LR [(NOI	LRFR SUMMARY FOR BOX BEAM UNIT (NON-INTERSTATE TRAFFIC)									
	REVISIONS									
№0. ВY: 1	DATE:	NU. BY:	DATE:	TOTAL						
2		4 4		16						

STD.NO.LRFR1



CHECKED BY : TMG II/II



NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 5.500 PSI.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " in depth, shall be tooled in all exposed faces of the barrier rail and IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS, A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK. THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

Mead Section133 Fayetteville Street Suite 210Delaiab NC 27601	PROJECT NO. <u>B-4964</u> <u>ROCKINGHAM</u> co STATION: <u>18+73.69</u> -L 12+70.54 -RA	 UNTY IL-					
NC 27601 919-714-8670 meadhunt.com NC License No. F-1235	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD						
SEAL 043177 0, NG INEE 0, HOB	3'-O"X 3'-3" PRESTRESSED CONCI BOX BEAM UNIT	RETE					
Jack Holson 6/1/2017 5000000000000000000000000000000000000	REVISIONS NO. BY: DATE: NO. BY: DATE: 1 3	SHEET NO. S-05 TOTAL SHEETS					



DocuSign Envelope ID: A84E61CE-AC3F-4550-9839-48A58ED204C9





2 SPA @ 2″ CTS.

			B	AR TYPE	ES		
2'-10"		3'-6"	IS LEG P OF UN	AT "-'," "		1'-6"	• - •
		3	2'-11" S1, S6 1'-6" S2, S ⁻	1'-1" S3, S8		8" 2" P. () () () () () () () () () () () () ()	
1 - 91/s "	1'-0		5'-6 ¹ /2"	TONS ADE		4 ¹ / ₂ "	
рті			DIMINS				TTON
DTL	L UF		(IAL F	TYTEDT	DUA DE	TNTEDT	
BAR		ST7F	TYPF		WETGHT		WETGHT
A1	10	#5	1	7'-2"	75	7'-2"	75
A2	44	#4	2	5'-7"	164	5'-7"	164
B6	12	#5	STR	52′-8″	659	52′-8″	659
K1	15	#4	6	7'-2″	72	7'-2″	72
K2	10	#4	STR	2'-7"	17	2'-7"	17
S1	63	#4	3	8′-6″	358	8′-6″	358
S2	63	#4	3	5′-8″	238	5′-8″	238
S3	125	#4	3	4'-10"	404	4'-10"	404
S4	62	#4	4	5′-10″	242	5′-10″	242
K S5	144	#5	5	5′-11″	889		
S6	20	#5	3	8'-6"	177	8′-6″	177
<u> </u>	20	#5	<u>,</u>	5'-8"	118	5′-8″	118
	20	#5	<u>ק</u>	4'-10"	101	4'-10"	101
50	_ 20		J		101	1 10	101
REINFO	DRCING	L STEEL		2625	LBS.	262	5 LBS

889

No. 32

20.2

LBS.

CU.YDS. 20.0

No. 32

CU.YDS

Mead Select133 Fayetteville Street Suite 210Delaieb NC 22601	PROJECT NO. <u>B-4964</u> <u>ROCKINGHAM</u> CO STATION: <u>18+73.69</u> -L 12+70.54 -RA	UNTY IL-
NC 27601 919-714-8670 meadhunt.com NC License No. F-1235	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTAT RALEIGH	ION
SEAL 043177 DocuSigned by:	3'-O"X 3'-3" PRESTRESSED CONCF BOX BEAM UNIT	RETE
Jack Hobson 6/1/2017	REVISIONS	SHEET NO.
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL	NO. BY: DATE: NO. BY: DATE:	S-07 total
SIGNATURES COMPLETED	U 2 4	SHEETS 16

* EPOXY COATED REINF.STEEL

7500 P.S.I. CONCRETE

0.6″ØL.R. STRANDS



ASSEMBLED BY :	J.S. HOE	BSON	DATE	:06/03/16
CHECKED BY :	J.A. LI	EE	DATE	:07/05/16
DRAWN BY : DGE CHECKED BY : TMC	/ G /	REV.8/	14	MAA/TMG





DEAD LOAD DEFLECTION AND CAMBER 103'-5"BOX BEAM UNIT (NC & SE) CAMBER (SLAB ALONE IN PLACE) DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD*** FINAL CAMBER ** INCLUDES FUTURE WEARING SURFACE

STD.NO.39PCBB7_90S



DRAWN BY :	J.S. HOBSON	DATE : 06/03/16
CHECKED BY :	J.A. LEE	DATE : 07/05/16
DESIGN ENGINEER	OF RECORD : J.S. HOBSON	DATE : <u>07/05/16</u>

BOX BEA	AM UN	NITS RE	-
	NUMBER	LENGTH	
EXTERIOR B.B.	2	103′-5″	
INTERIOR B.B.	9	103′-5″	
TOTAL	11		

BII	LL OF MATERIAL FOR VERTICAL CONCRE	ΤΕ Β	ARR	IER F	RAIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
	103'-5" UNIT				
米 B12	96	#5	STR	25'-5"	2545
米 S9	288	#5	1	7'-2"	2153
米 EPOX	Y COATED REINFORCING STEEL		LBS.		4698
CLASS	AA CONCRETE		CU.YDS.		27.0
TOTAL	VERTICAL CONCRETE BARRIER RAIL		LN.FT.		206.8

BII	L OF MATERIAL FOR VERTICAL CONCRET	ΓΕ Β	ARR	IER R	CAIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
	103'-5" UNIT				
₩ B12	96	#5	STR	25′-5″	2545
* S9	288	#5	1	7'-2″	2153
★ EPOX	COATED REINFORCING STEEL		LBS.		4698
CLASS	AA CONCRETE		CU.YDS.		27.0
TOTAL	/ERTICAL CONCRETE BARRIER RAIL		LN.FT.		206.8

GUTTERLINE ASPI	HALT THICKNESS	& RAI	L HEIGHT
	ASPHALT OVERLAY THI @ MID-SPAN	ICKNESS	RAIL HEIGHT @ MID-SPAN
103'-5" UNITS	1 ¹⁵ / ₁₆ ″		3′-7 ¹⁵ ⁄16′′





Mead Section133 Fayetteville Street Suite 210Delaigh NC 27601	PROJECT NO ROCKINGHA STATION:18+7 12+70.	<u>B-4964</u> MCOUNTY 3.69 -L- 54 -RAIL-
919-714-8670 meadhunt.com NC License No. F-1235	STATE OF NORT	H CAROLINA RANSPORTATION GH
SEAL 043177 DocuSigned by:	3'-0"X PRESTRESSED BOX BEA	3'-3") concrete M unit
Jack Hobson 6/1/2017	REVISIONS	SHEET NO.
OCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: DATE: NO. BY 1 3 2 4	DATE: S-09 TOTAL SHEETS 16



4″ 4″ € GUARDRAIL ANCHOR ASSEMBLY € GUARDRAIL ANCHOR ASSEMBLY PLAN

> LOCATION OF ANCHORS FOR GUARDRAIL END BENT #1 SHOWN, END BENT #2 SIMILAR.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " hold down plate and 7 - $\frac{7}{8}$ " Ø Bolts with nuts and washers.

•

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

<u>B-</u>4964 PROJECT NO.___ Mead & lunt ROCKINGHAM _ COUNTY STATION: <u>18+73.69</u> -L-12+70.54 -RAIL-133 Fayetteville Street Suite 210 SHEET 1 OF 4 Raleigh, NC 27601 919-714-8670 STATE OF NORTH CAROLINA meadhunt.com DEPARTMENT OF TRANSPORTATION NC License No. F-1235 RALEIGH SUBSTRUCTURE FESS/ON SEAL 043177 END BENT No.1 AGINES DocuSigned by: Jack Hobson 6/1/2017 SHEET NO. REVISIONS DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED NO. BY: S-11 NO. BY: DATE: DATE: TOTAL SHEETS 16

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

Mead Section133 Fayetteville Street Suite 210Baleigh, NC 27601	PROJECT NO. <u>B-4964</u> <u>ROCKINGHAM</u> COUNTY STATION: <u>18+73.69</u> -L- <u>12+70.54</u> -RAIL- SHEET 2 OF 4
NC License No. F-1235	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
SEAL 043177	SUBSTRUCTURE
DocuSigned by:	END BENT No.2
Jack Hobson 6/1/2017	REVISIONS SHEET NO.
DOCUMENT NOT CONSIDERED	NO. BY: DATE: NO. BY: DATE: S-12
SIGNATURES COMPLETED	<u>1</u> 2 4 16

Mead Select133 Fayetteville Street Suite 210 Baleigh NC 27601	PROJECT NO. <u>B-4964</u> <u>ROCKINGHAM</u> COUNTY STATION: <u>18+73.69</u> -L- <u>12+70.54</u> -RAIL- SHEET 4 OF 4
919-714-8670 meadhunt.com NC License No. F-1235	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
SEAL 043177 DocuSigned by:	SUBSTRUCTURE END BENT No.1 & 2 DETAILS
Jack Hobson 6/1/2017	REVISIONS SHEET NO.
FINAL UNLESS ALL SIGNATURES COMPLETED	1 3 TOTAL SHEETS 2 4 16

SID. NO. EB_33_9054_39BB

GENERAL NOTES

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS.

SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-O"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.18+73.69 -L-	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE		
	SQUARE YARDS	APPROX.L.F.		
END BENT 1	10	19		
END BENT 2	10	19		

* QUANTITY SHOWN IS BASED ON 5' POURS.

OPTIONAL POURING DETAIL

Mead Sector Street Suite 210	PROJEC <u>RO</u> STATIO	CT NO. CKIN DN: <u>1</u> 124	I <u>GHA</u> 8+7 +70.	B- AM 3.6 .54	-4964 co <u>59 -l</u> -RA	1 UNTY IL-
919-714-8670 meadhunt.com NC License No. F-1235	DEPA	STA-	TE OF NORT OF T RALEI	TH CAROL RAN	SPORTA	TION
SEAL 043177 Docusigned by:	SL	.OPE D	PRC Eta)te Il	ECTI(S	NC
Jack Hobson 6/1/2017		REVI	SIONS			SHEET NO.
DOCUMENT NOT CONSIDERED	NO. BY:	DATE:	NO. BY	Y:	DATE:	S-15
SIGNATURES COMPLETED	1		3			TOTAL SHEETS

16

SPLICE LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9″
#5	2'-6"	2'-2"
#6	3′-10″	2'-7"

STD. NO. BAS_BB_33_90S

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SO.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

+

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR

EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH JANUARY, 1990

STD. NO. SN