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REFERENCE

5047

SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

<u>LINE</u>	STATION	<u>PLAN</u>	PROFILE
-L-	IO+00 TO 68+00	4 TO 8	II TO 15
-RAMP A-	IO+00 TO 25+I2	7, 9	16
-RAMP B-	10+00 TO 29+94	5, 6, 9	17
-RAMP C-	10+00 TO 26+24	5, 6, 10	18
-RAMP D-	10+00 TO 29+55	7, 8, 10	19
-Y I-	IO+OO TO 26+35	6. 9. 10	20

CROSS SECTIONS

<u>LINE</u>	<u>STATION</u>	<u>SHEETS</u>
-L-	10+00 TO 25+00	2I TO 28
-L-	37+00	29
-L-	38+00	30
-L-	40+00	31
-L-	42+00	32
-L-	57+50 TO 68+00	32 TO 38
-RAMP A-	23+50 TO 24+50	39
-RAMP B-	10+00 TO 14+50	40 TO 43
-RAMP C-	10+00 TO 14+00	44 TO 47
-RAMP C-	23+50 TO 25+50	48 TO 49
-RAMP D-	26+50 TO 29+00	50 TO 52
-Y I-	II+00 TO I4+50	53 TO 55
-Y I-	16+00 TO 19+50	55 TO 58
-Y I-	24+00 TO 26+00	59 TO 60

APPENDICES

<u> APPENDIX</u>	<u>TITLE</u>	<u>SHEETS</u>
Α	LABORATORY RESULTS	6I TO 62
В	CPT LOGS	63 TO 65

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY	WAYNE	
PROJECT DESCRIP	TIONDIVISION 4 - US 117 AND	
	NTRY CLUB ROAD) INTERCHANGE	
INVENTORY		

STATE PROJECT REFERENCE NO. 65 R = 5719

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-680. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS OF THE INVESTIGATION. THE STATEM LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE INVESTIGATION. THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AS WELL AS A CALLED NOW CHARTSE CACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- IES:
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 OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS
 OR CONTRACT FOR THE PROJECT.
 BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
 FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
 CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

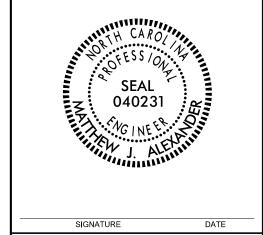
DUGGINS, W. T. EKLUND, M. A. SMITH, E. H. LEE, D. K. LEE, S.

ALEXANDER, M. J.

PERSONNEL

INVESTIGATED BY TERRACON CONSULTANTS ALEXANDER, M. J. DRAWN BY RIGGS, A. F. CHECKED BY SUBMITTED BY TERRACON CONSULTANTS

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2401 BRENTWOOD ROAD, SUITE 107 RALEIGH, NORTH CAROLINA 27604

PHONE: (919) 873–2211 FAX: (919) 873–9555 NC REGISTERED FIRM: F-0869

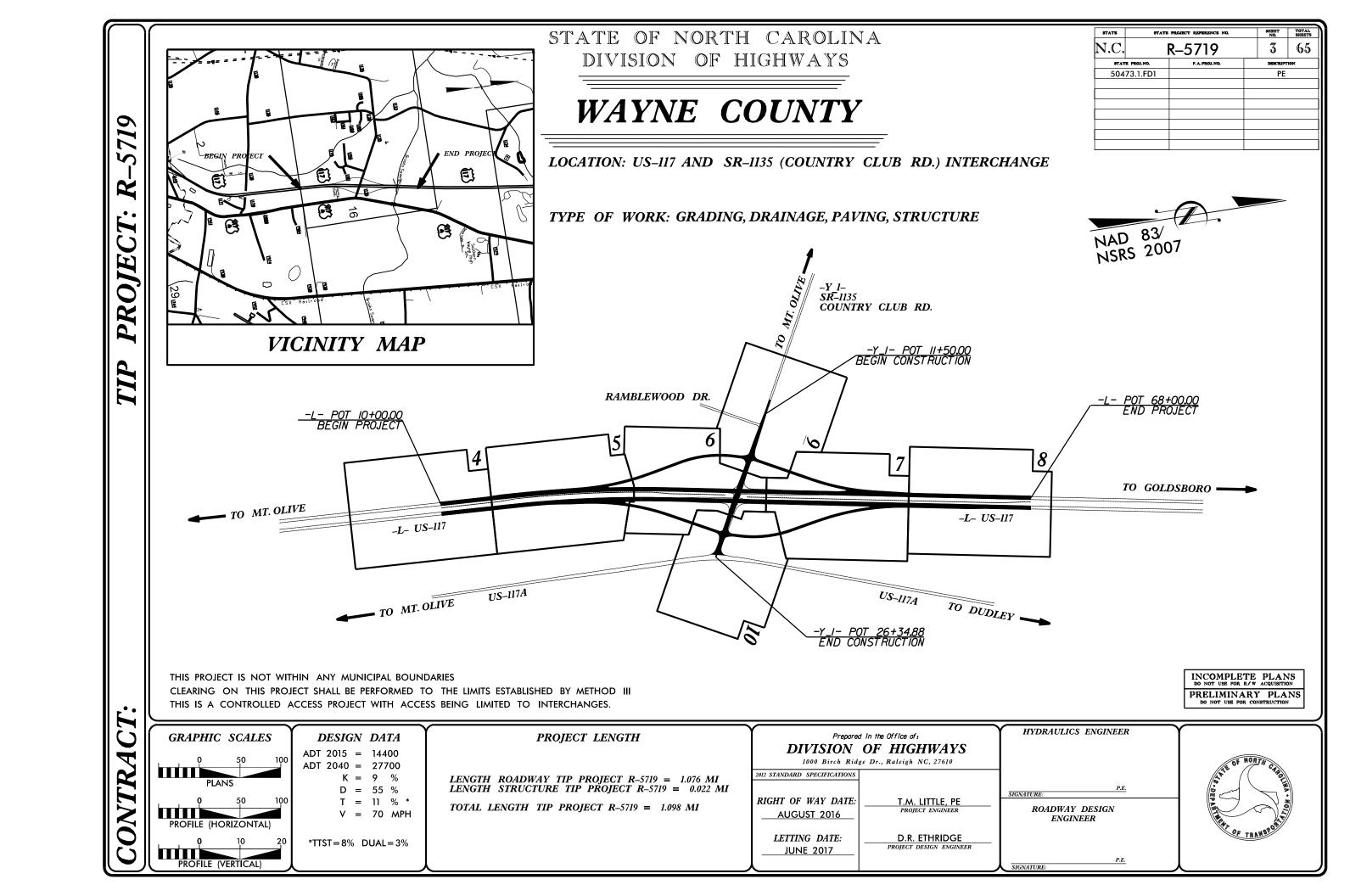
PROJECT REFERENCE NO. SHEET NO. 2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF.GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
CENERAL CRANIII AR MATERIALS SILT-CLAY MATERIALS	MINERALOGICAL COMPOSITION	FINE TO COARSE CRAIN ICNEOUS AND METAMORPHIC POCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	ROCK (CP) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ONEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN FOR TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN FOR TOWN AND THE PROPERTY OF THE PROPERTY OF THE PROPERTY OF T	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-3 A-6, A-7	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LL < 31	ROCK (NCR) SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 0000d00000	MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
7. PASSING GRANULAR SILT- MUCK,	HIGHLY COMPRESSIBLE LL > 50 PERCENTAGE OF MATERIAL	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 59 MX GRANULAR CLAY PEAT GRANULAR CLAY PEAT GRANULAR GLAY PEAT SOILS S	GRANULAR SILT - CLAY	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
א פל אוים פל	ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL PASSING *40	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE. VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	HORIZONTAL.
LL 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 501L5 WITH	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	<u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
P! 6 MX NP 18 MX 18 MX 11 MN 11 MN 18 MX 18 MX 11 MN 11 MN MODERATE OPERATE	GROUND WATER	OF A CRYSTALLINE NATURE.	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
GROUP INDEX W W 4 MX 8 MX 12 MX 16 MX NU MX AMUUN 15 UF SOILS		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TITES STUME FRAUS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN, RATING EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE		(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	- SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.
COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED	FD 05:005	(MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTINESS OR PENETRATION RESISTENCE COMPRESSIVE STRENGTH (N-VALUE) (TONS/FT ²)	ROADWAY EMBANKMENT (RE) DIP & DIP DIRECTION WITH SOIL DESCRIPTION OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
VERY LOOSE (4	SPT C SLOPE INDICATOR	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANILAR LOOSE 4 TO 10	SOIL SYMBOL DOT ONT TEST BORING SCORE INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS
MATERIAL MEDIUM DENSE 10 10 30 N/A	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT AUGER BORING CONE PENETROMETER	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50		SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.5	— INFERRED SOIL BOUNDARY — CORE BORING SOUNDING ROD	(V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u>	OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	A ALLUMIAL SOIL BOUNDARY A PIEZOMETER	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4	INSTALLATION SPIN NOTHER	ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	USED IN THE TOP 2 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN. MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.005 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{ m d}$ - DRY UNIT WEIGHT	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION (ATTERBERG LIMITS) DESCRIPTION	CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
CATHRATER HOURS VERY MET HOUSE V	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	TENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC PLASTIC PROMITE AND A	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE / - WET - (W) SEMISULID; REQUIRES DRYING TO	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: N/A - SEE NOTES
(PI) PL PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	DENOTI PIANK, N/ A SEE NOTES
- MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: N/A FEET
OM OPTIMUM MOISTURE SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	FIAD - FILLED IMMEDIATELY AFTER DRILLING
ATTAIN OPTIMUM MOISTURE	CME-55 6* CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	
PLASTICITY	8* HOLLOW AUGERS	INDURATION	REPORT PREPARED USING PROVIDED TIN FILE (r5719_ls_tnl.tin)
PLASTICITY INDEX (PI) DRY STRENGTH	L CME-550	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. RUBBING WITH FINGER FREES NUMEROUS GRAINS:	
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST TUNGCARBIDE INSERTS	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM	X CASING W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST X TRICONE 215/16 STEEL TEETH X HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	X DIEDRICH D-50 TRICONE TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	X 31/4" HOLLOW STEM AUGERS VANE SHEAR TEST	CHARP HAMMER BLOWS REQUIRED TO RREAK SAMPLE.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	X ACKER RENEGADE X TRICONE 37/8 STEEL TEETH	EXTREMELY INDURATED SHARP HAMMER BLOWS REDUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1-





Date: January 2017

WBS Number: 50473
TIP Number: R-5719
County: Wayne

Description: US 117 and SR 1135 (Country Club Road) Interchange Improvements

Subject: Roadway Geotechnical Report - Inventory

Project Description

The project is located along the existing US 117 corridor between Goldsboro and Mt. Olive in Wayne County. On US 117 (-L-), the project begins approximately 0.6 miles south of the existing at-grade intersection of US 117 and SR 1135 (-Y1-) and continues north along US 117 for approximately 1.1 miles. The -L- alignment is proposed to cross over the -Y1- alignment on dual single-span structures. The proposed interchange also includes ramps in all four quadrants of the interchange on new location. On SR 1135 (-Y1-), the project begins approximately 0.2 miles west of the existing intersection and continues east for approximately 0.3 miles. The -Y1- alignment is proposed to remain in the same location and at nearly the same grade. The project corridor is in a rural setting and much of the surrounding land is undeveloped.

The geotechnical subsurface investigation was performed from January through May of 2016. Standard penetration test (SPT) borings were advanced using Diedrich D-50 and Acker Renegade rotary drill rigs equipped with recently calibrated automatic hammers. SPT borings were advanced utilizing wash boring and hollow stem auger drilling techniques to advance the borings to the necessary depths. Representative soil samples were collected in the field for visual classification and selected samples were submitted for laboratory analysis by Terracon's soil testing laboratory. Two Shelby tube samples were taken near the proposed structures for consolidation testing. Laboratory testing was performed in general accordance with the AASHTO specifications. In addition to the soil test borings performed along the corridor, a Pagani TG73-200 rig was utilized to perform four cone penetrometer (CPT) soundings. Supplemental hand auger borings were performed during October 2016. Excavated soils were visually classified in the field by a trained geologist. Select representative samples were submitted for laboratory analysis by Terracon's soil testing laboratory.

The following alignments were investigated by soil testing and visual reconnaissance:

<u>Alignment</u>	<u>Stations</u>
-L-	10+00 to 68+00
-RAMP A-	10+00 to 25+12
-RAMP B-	10+00 to 29+94
-RAMP C-	10+00 to 26+24
-RAMP D-	10+00 to 29+55
-Y 1-	10+00 to 26+35

Terracon Consultants, Inc. 2401 Brentwood Road Raleigh, NC 27604
P [919] 873 2211 F [919] 873 9555 terracon.com NC Registered Firm F-0869

Geotechnical Envi

Environmental

Construction Materials

Facilities

PROJECT REFERENCE NO.	SHEET NO.
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Physiography and Geology

The site is located within the Inner Coastal Plain Physiographic and Geologic Province of North Carolina in Wayne County. The Coastal Plain Province is characterized by subdued topographic features. The existing elevations along the investigated corridor range from approximately 125 feet to 150 feet. In general, the topography at this site is slightly rolling with gentle slopes.

The project is located in the Inner Coastal Plain Physiographic Province with geology consisting of a wedge of unconsolidated sands, silt, and clays interbedded with occasional limestone strata, which rests atop crystalline basement rocks. Based on previous mapping (N.C. Geologic Map 1985) and our knowledge of the local geology, the site falls within the Cretaceous age Black Creek Formation. However, based on our site visit and subsurface conditions encountered, the near surface soils appear to be recent Coastal Plain deposits of alluvial origin and are consistent with interbedded sands, clayey sands and clays typical of Undivided Coastal Plain soils. This type of deposition has resulted in a highly variable subsurface profile along the project alignments. The Undivided Coastal Plain deposits overlie the denser, darker soils belonging to the Black Creek Formation. The Black Creek Formation soils are described as gray to black lignitic clay with thin beds and laminae of fine-grained micaceous sand and contains thin beds and laminae of fine-grained micaceous sand and thick lenses of cross-bedded sand. Glauconitic, fossiliferous clayey sand lenses are common in the upper part.

Soil Properties

Soils encountered during this investigation are separated into three categories based on their origin. The soils encountered consist of roadway embankment fill, Undivided Coastal Plain deposited soils and Formational soils.

Roadway embankment soils were encountered at the following approximate locations:

<u>Alignment</u>	<u>Stations</u>
-L-	10+00 to 16+50
-L-	18+23 to 21+26
-L-	48+79 to 68+00
-RAMP A-	10+00 to 14+40
-RAMP B-	10+00 to 10+30
-RAMP D-	10+00 to 15+79

The roadway embankment soils encountered appear to be derived from the nearby existing cuts along the -L- alignment. The roadway embankment soils predominately consist of medium stiff to very stiff, moist to wet, sandy clay (A-6) and medium stiff, moist, highly plastic silty clay (A-7-6). Some loose, moist, silty sand (A-2-4) was also encountered in the borings advanced through the existing roadway embankments.

Roadway embankment fill consisting of moderately plastic sandy clay (A-6) and moderately to highly plastic silty clay (A-7-6) was encountered along -L- between 10+00 and 16+50, 18+20 and 21+35, and from 57+50 to 68+00. In general, the cohesive roadway embankment soil extends to boring termination depths ranging

PROJECT REFERENCE NO.	SHEET NO.
R-5719	3B

from 4 to 10 feet. The plasticity indices of the cohesive roadway embankment soils range from 16 to 31 based on laboratory testing.

A majority of existing US 117 through the project was cut below natural ground. Therefore, Undivided Coastal Plain deposits are present at the surface and beneath the roadway embankment soils and asphalt pavement sections. The Undivided Coastal Plain soils can be generalized as alternating layers of silt, clay, and sand extending to the maximum depths of exploration. The near surface Undivided Coastal Plain deposits along the -L-, -RAMP-, and -Y 1- alignments are generally medium stiff to stiff and consists of moist to wet, sandy silt (A-4), sandy clay (A-6) and moderately to highly plastic silty clay (A-7-6). These cohesive soils were encountered at or near the existing ground surface on a majority of the project. The cohesive soils are interbedded with laterally discontinuous, saturated, very loose to medium dense, silty and clayey sand (A-2-4 and A-2-6, respectively). The Undivided Coastal Plain soils encountered deeper than approximately 10 feet below existing grades were generally very soft to soft cohesive soils and very loose to loose granular soils which extend to the top of the Black Creek Formation.

Undivided Coastal Plain soils consisting of moderately plastic sandy clay (A-6) and moderately to highly plastic silty clay (A-7-6) were encountered at or near the existing ground surface between -L- 16+50 and 24+30. The plasticity indices of the cohesive soils range from 17 to 23 based on laboratory testing. The cohesive soils extend to approximately 10 feet below existing grades and are underlain by saturated, very loose to medium dense silty to clayey sand (A-2-4 and A-2-6, respectively) and fine sand (A-3) based on borings advanced to 15 feet below the existing ground surface.

Undivided Coastal Plain soils consisting of moderately plastic sandy clay (A-6) was encountered near the existing ground surface on -RAMP A- between 23+55 and 25+12 or existing SR 1135. The moderately plastic sandy clay extends to a depth of approximately 2 feet below existing grades and is underlain by moist and loose fine sand (A-3).

The Undivided Coastal Plain soils consisting of moderately plastic sandy clay encountered on the -L-alignment continue along the -RAMP B- and -RAMP C- alignments. Moderately plastic sandy clay was encountered near the existing ground surface between -RAMP B- 10+00 and 14+50 and on -RAMP C-between 10+00 and 14+00. Plasticity indices for these soils ranged from 20 to 23 based on laboratory testing.

Undivided Coastal Plain soils consisting of slightly to moderately plastic sandy clay (A-6) were encountered near proposed grade on -RAMP C- between 23+50 and 25+15. The slightly to moderately plastic sandy clay is underlain by soft to medium stiff, moist to wet, sandy silt (A-4).

Undivided Coastal Plain soils consisting of slightly to moderately plastic sandy clay (A-6) and highly plastic silty clay (A-7-6) were encountered on -RAMP D- between 26+70 and 29+30 near existing SR 1135. The sandy clay was encountered near the surface and transitions to highly plastic silty clay at depths of approximately 3 to 5 feet below existing grades between -RAMP D- 26+66 and 29+30. The plasticity indices of the cohesive soils range from 15 to 35 based on laboratory testing.

Undivided Coastal Plain soils consisting of moderately plastic sandy clay (A-6) was encountered along the -Y 1- alignment between 10+00 and 14+05. The cohesive soils extended to hand auger termination depth of 4 feet below the existing ground surface. Moderately plastic sandy clay (A-6) and highly plastic silty clay (A-7-6) were encountered along -Y 1- between 17+00 and 21+00. The cohesive soils extend to hand auger termination depth of 4 feet along -Y 1- although the borings along -L- encountered very loose to loose, saturated silty sand (A-2-4) approximately 18 feet below the existing ground surface. Moderately plastic sandy clay (A-6) and highly plastic silty clay (A-7-6) were encountered near the existing ground surface between 23+50 and 25+50. Some of this area coincides with the -RAMP C- and -RAMP D- alignments. The cohesive soils extend to boring termination depths ranging from 4 to 10 feet. The moderately plastic sandy clay and highly plastic silty clay are interbedded with sandy silt (A-4).

Formational soils of the Black Creek Formation were encountered in the deep borings along the -L-alignment between Elevation 94 and 60 feet. These soils are characterized by their black to gray color and consist of layers of loose to dense, saturated, silty to clayey sands (A-2-4 and A-2-6).

Groundwater

In general, the corridor drains to Brooks Swamp to the north and Lee Branch to the west. Groundwater was encountered during drilling and sampling along the -L- and -RAMP- alignments at depths ranging from 4 to 13 feet below existing grades. Groundwater was not encountered within 6 feet of proposed grades in the borings performed at the site. The depth of groundwater, beneath the ground surface, will fluctuate with seasonal precipitation and may occur a higher levels at other times of the year above less permeable clayey soils.

Areas of Special Geotechnical Interest

1) <u>Plastic Soils</u> - Moderately to highly plastic soils with plastic indices (PI) of 16 and greater were encountered near proposed subgrade at the following locations:

<u>Alignment</u>	<u>Stations</u>
-L-	10+00 to 24+30
-L-	57+75 to 68+00
-RAMP A-	23+75 to 24+92
-RAMP B-	10+00 to 13+15
-RAMP C-	10+00 to 13+70
-RAMP C-	23+90 to 25+05
-RAMP D-	26+70 to 29+30
-Y 1-	11+50 to 14+00
-Y 1-	17+00 to 21+00
-Y 1-	23+50 to 25+50

A discussion of these plastic soils is located above in the section titled "Soil Properties".

UNDISTRUBED SAMPLES

The following "Shelby" tube samples were taken to provide data for the in-situ strength of the soil.

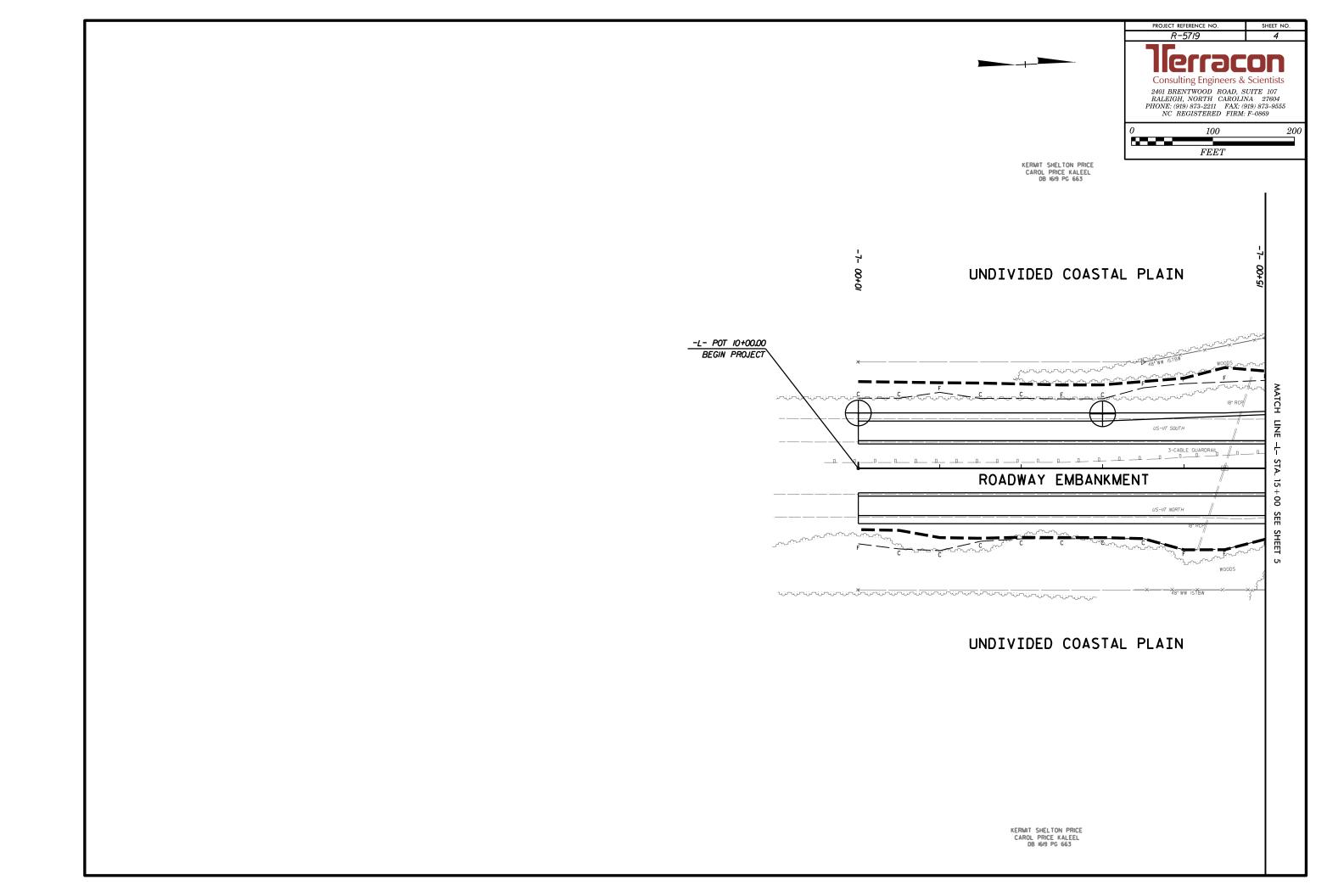
Sample No.	Location	<u>Offset</u>	Depth (ft.)	<u>Test</u>
ST-1	38+83 -L-	68 ft. Left	13.5 - 15.0	Consolidation
ST-2	39+50 -L-	80 ft. Right	15.0 - 17.4	Consolidation

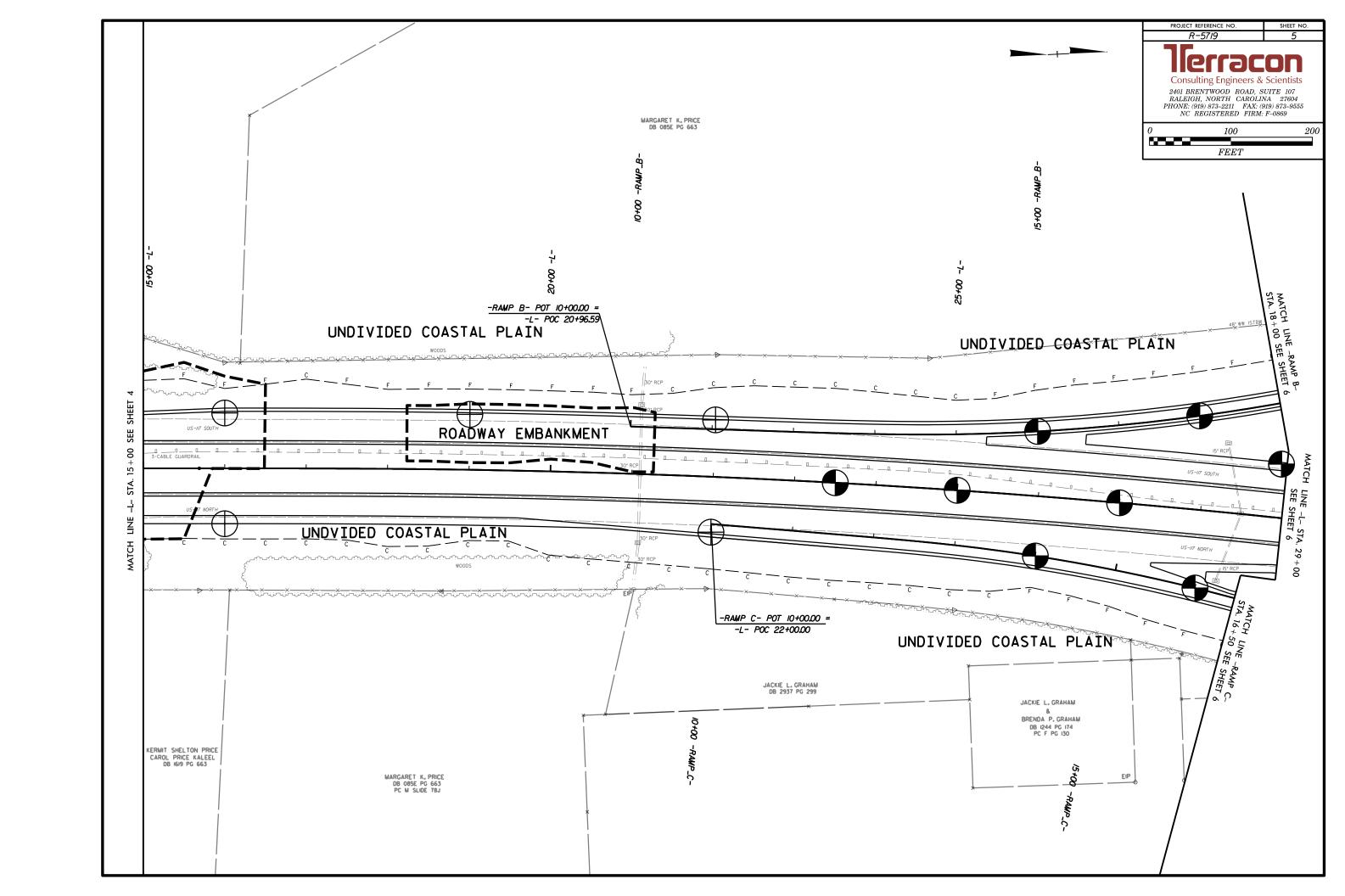
Sincerely,

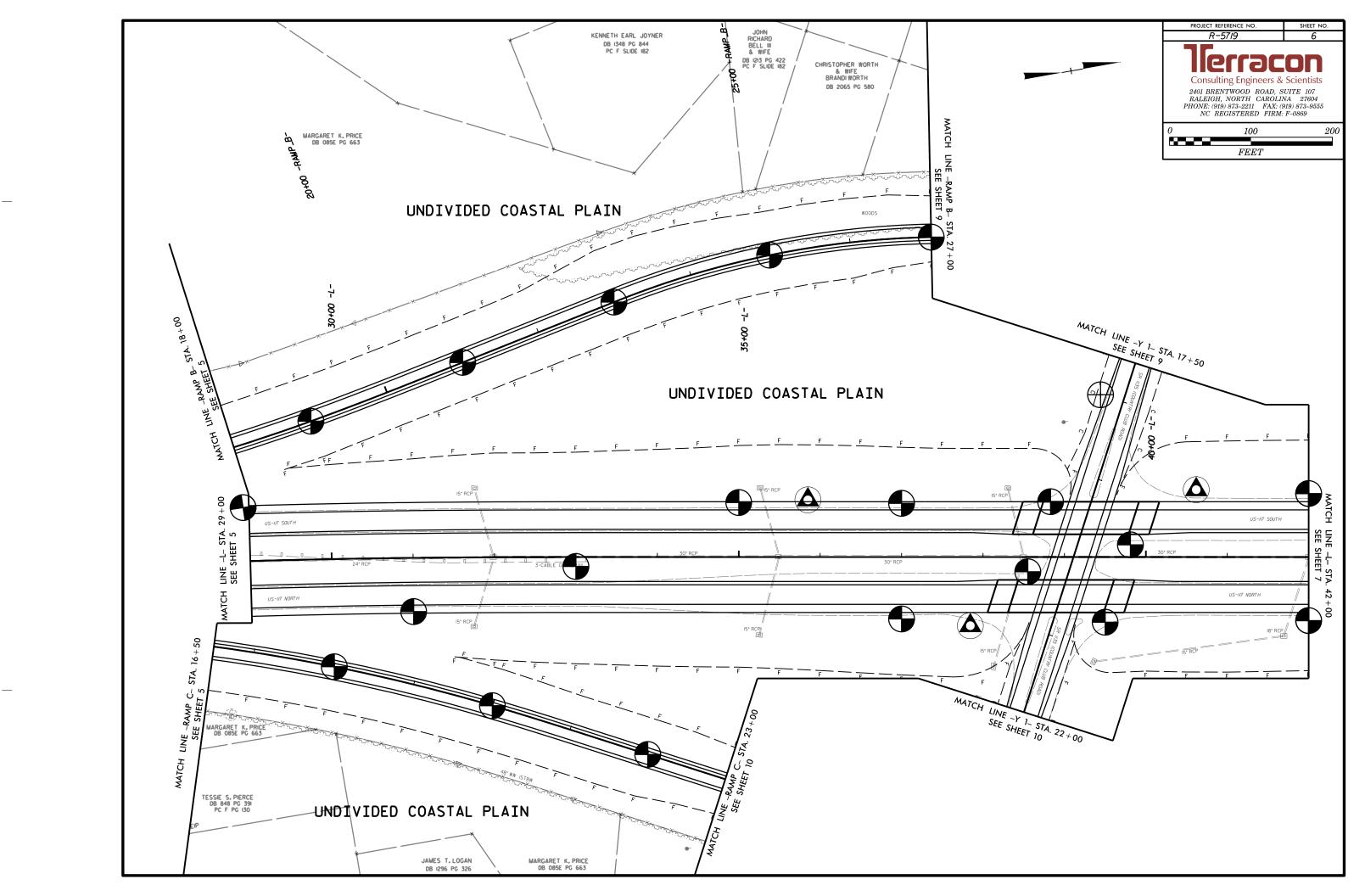
Terracon Consultants, Inc.

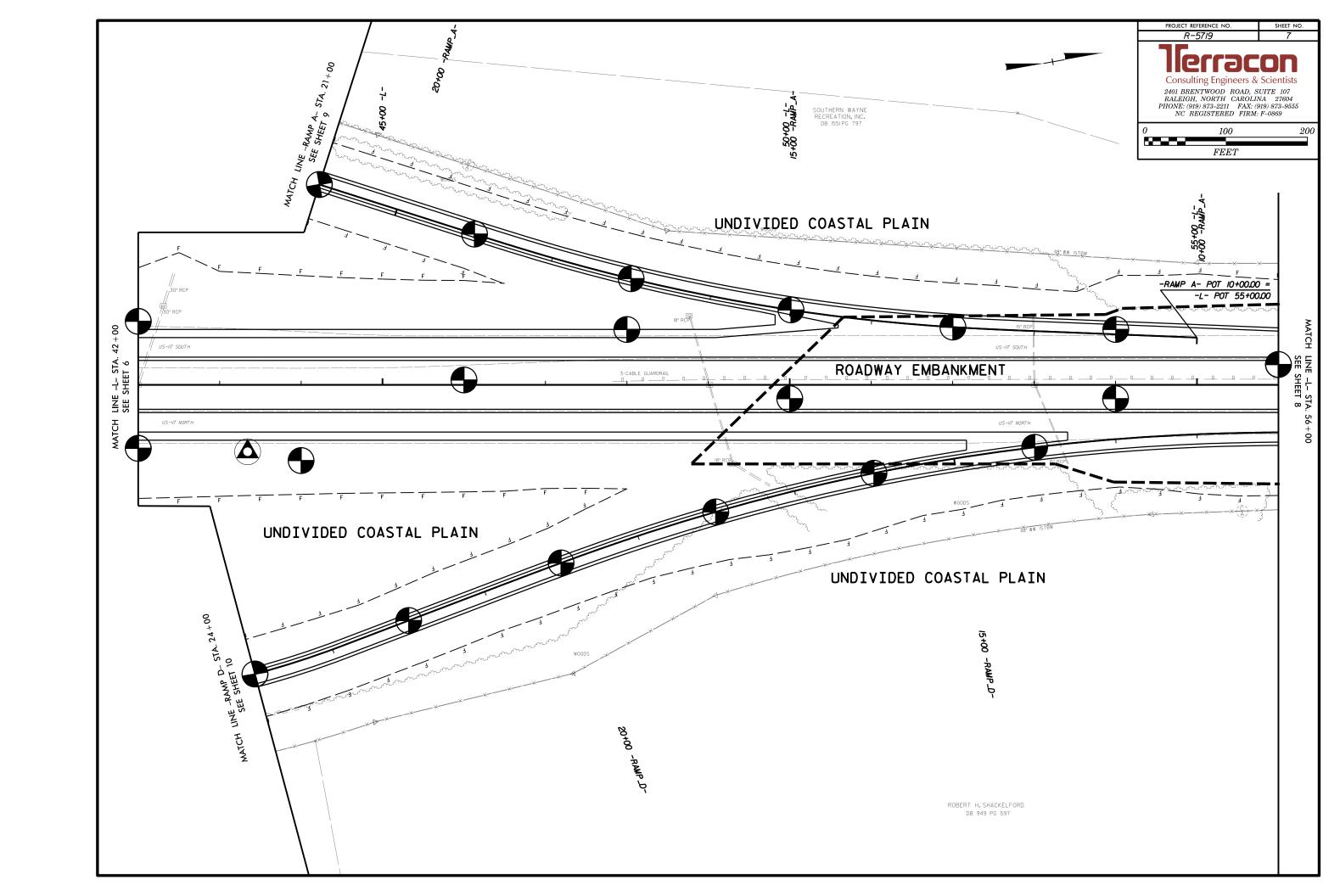
Matthew J. Alexander, PE Project Geotechnical Engineer Abner F. Riggs, Jr., PE Senior Geotechnical Engineer

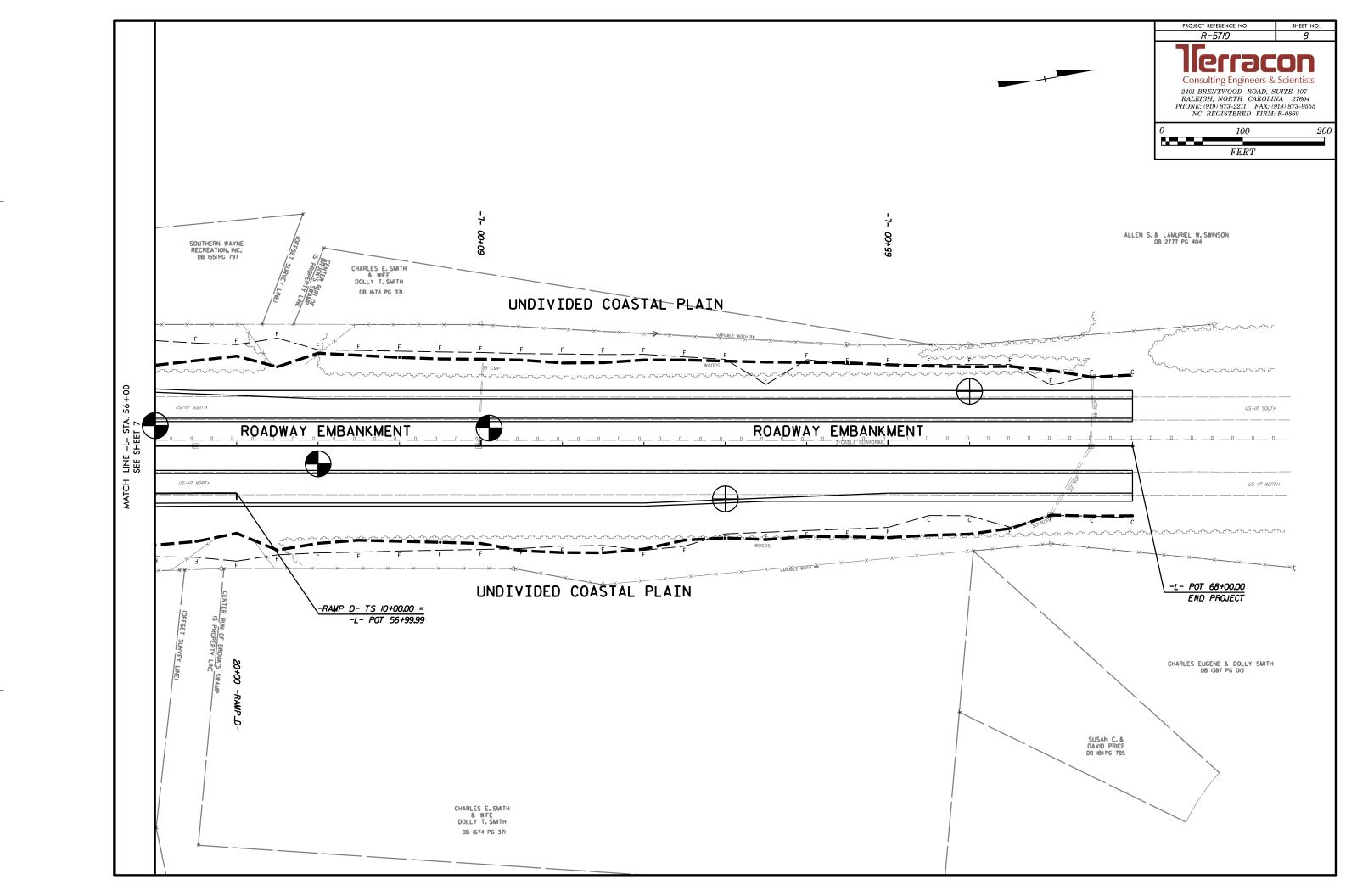
PROJECT REFERENCE NO.	SHEET NO.
R-5719	3C

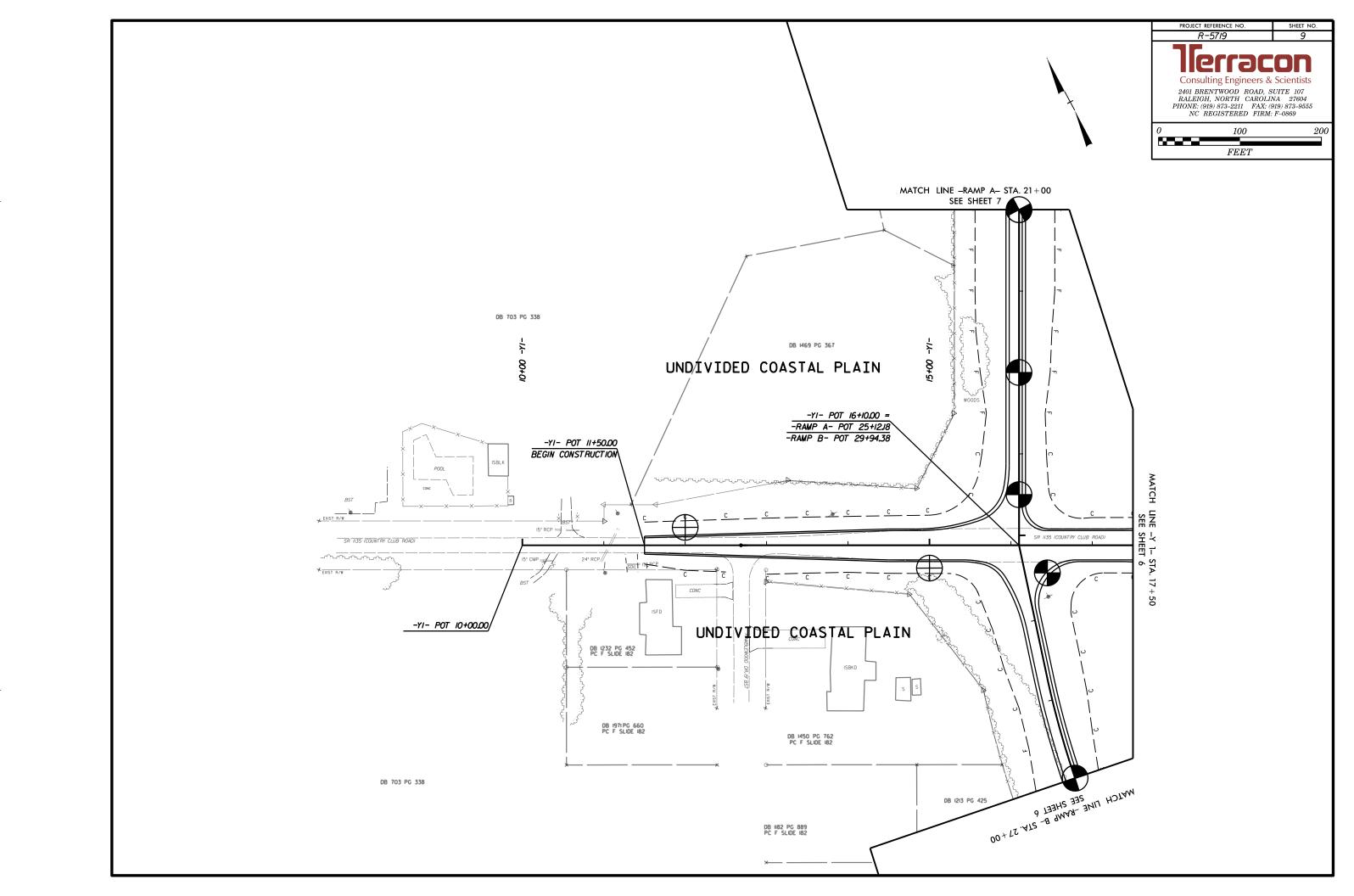


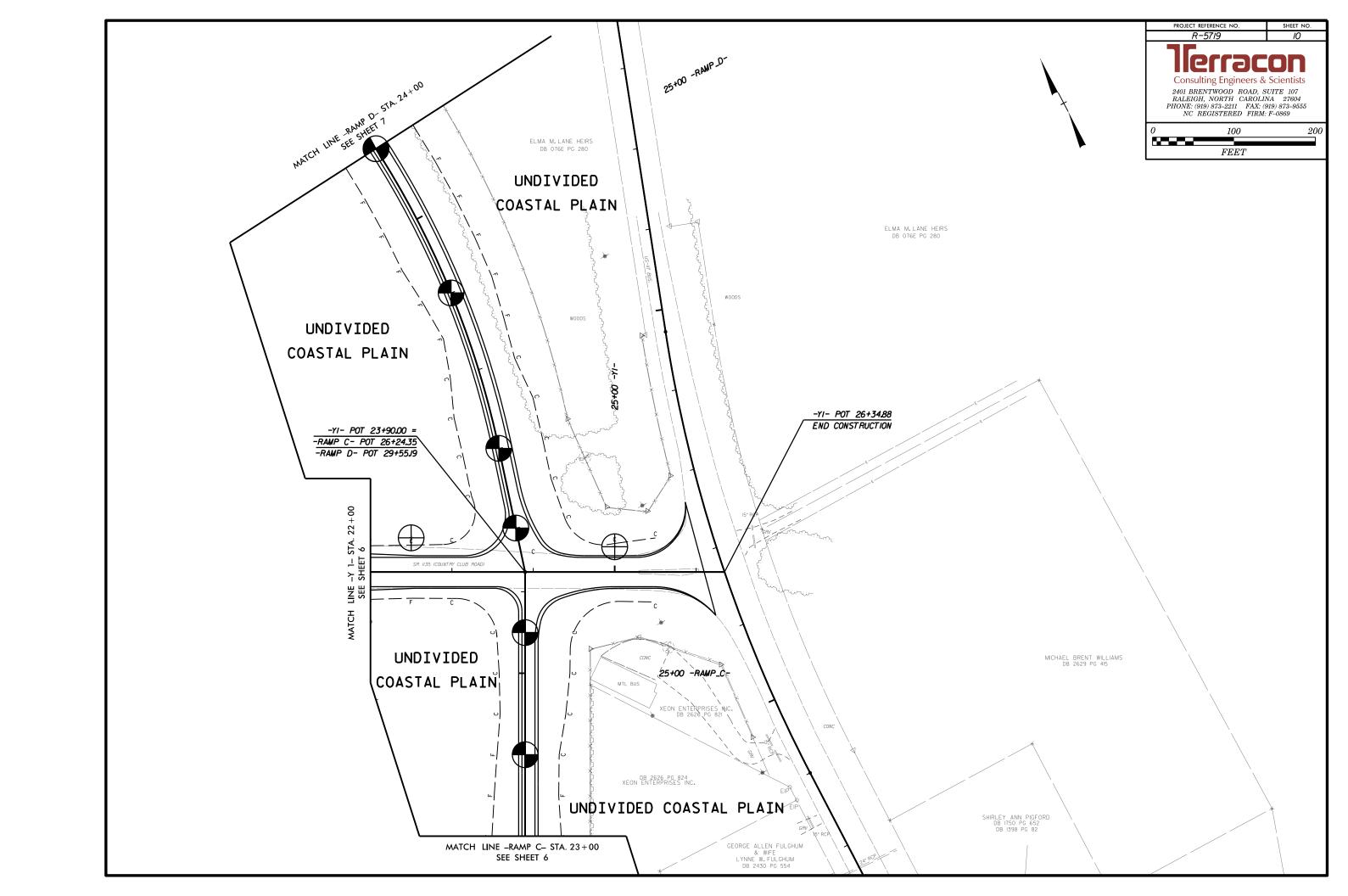


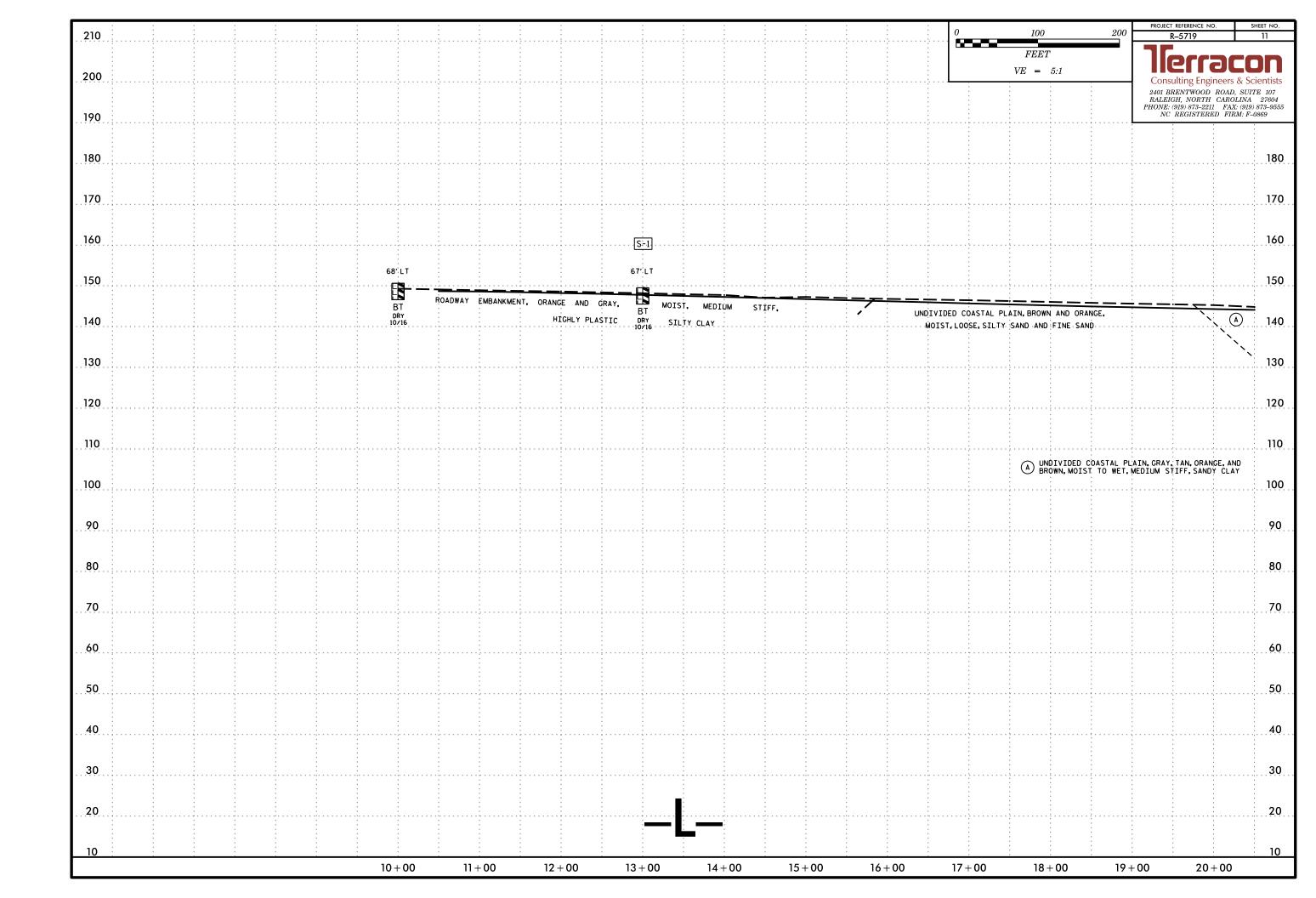


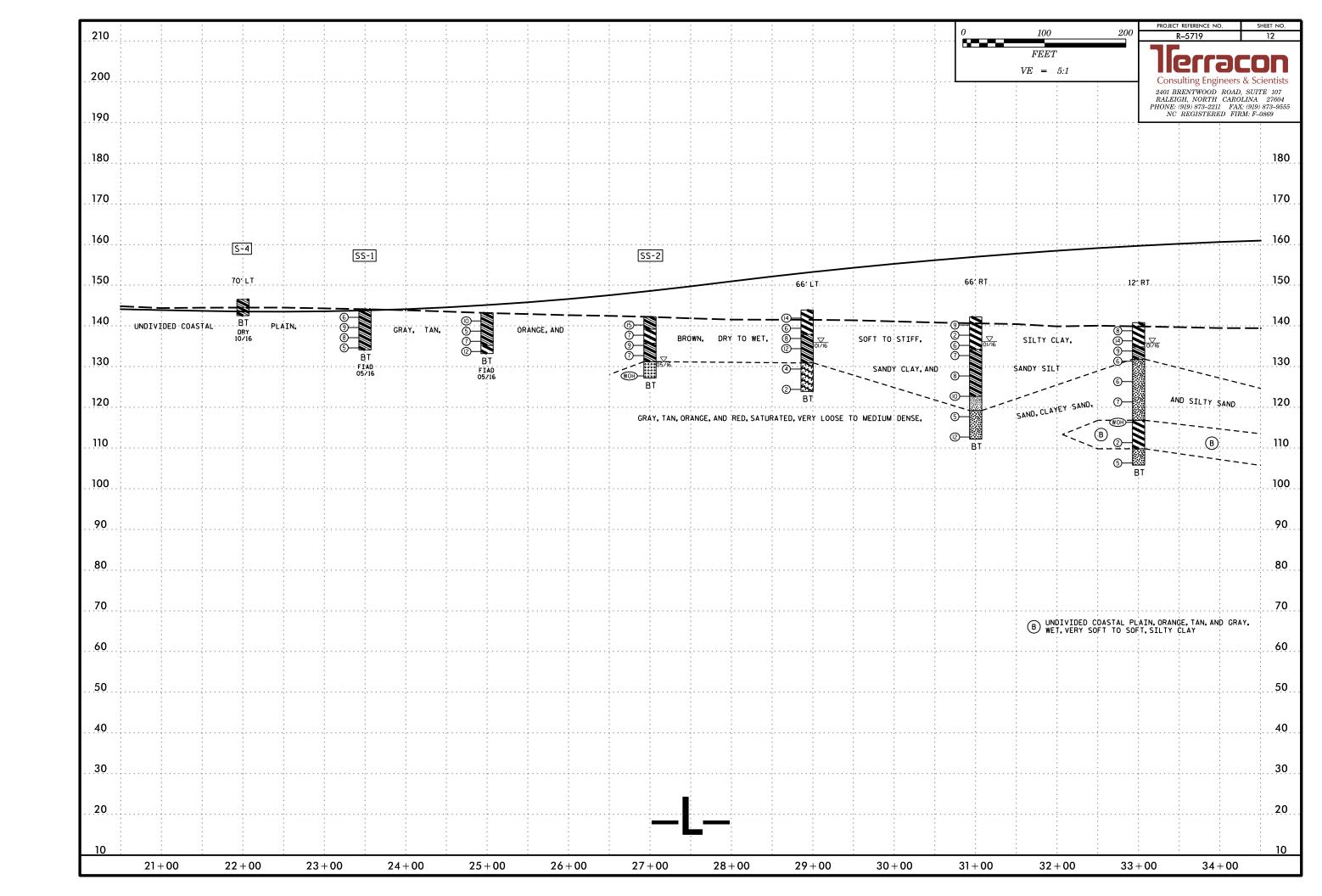


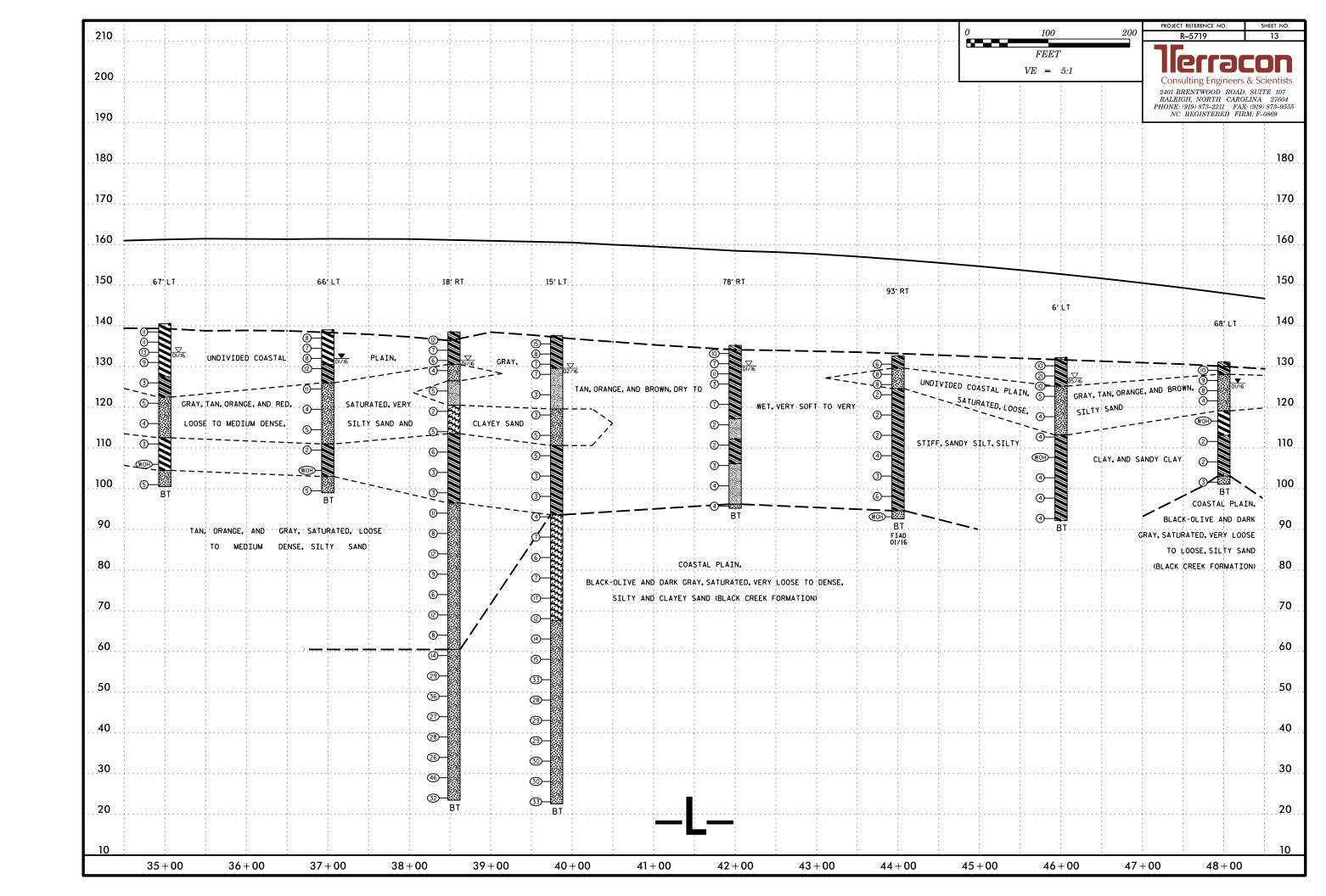


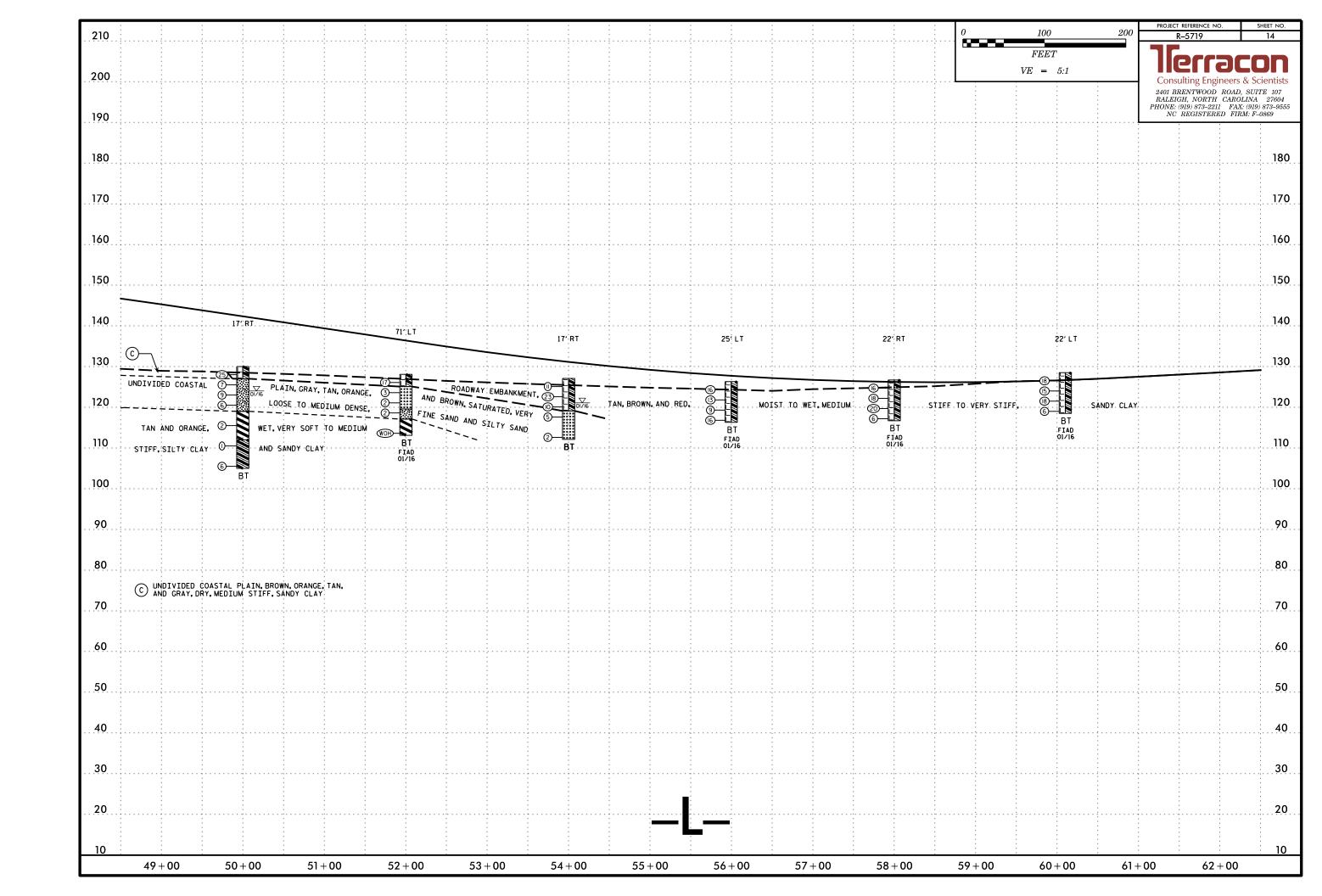


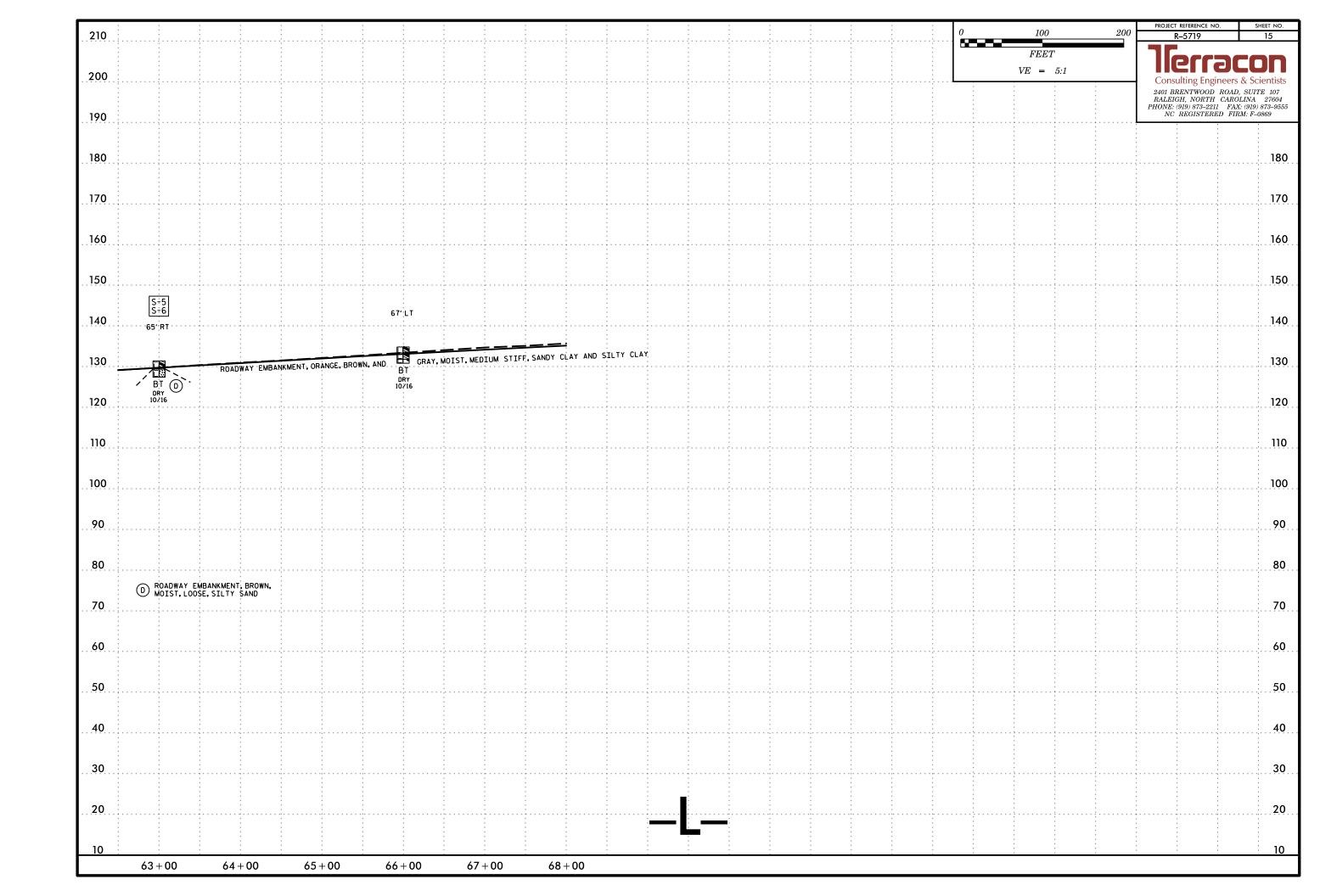


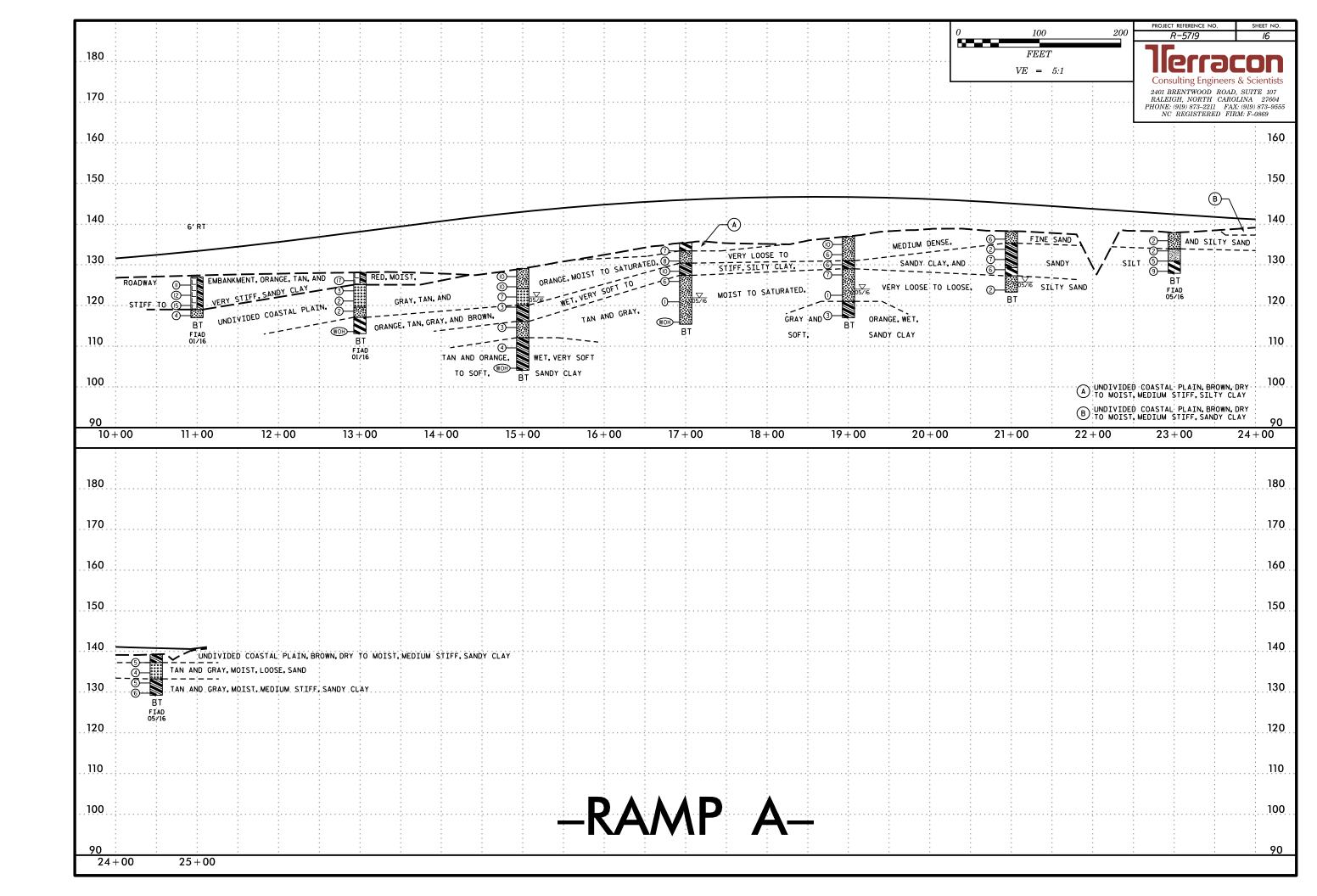


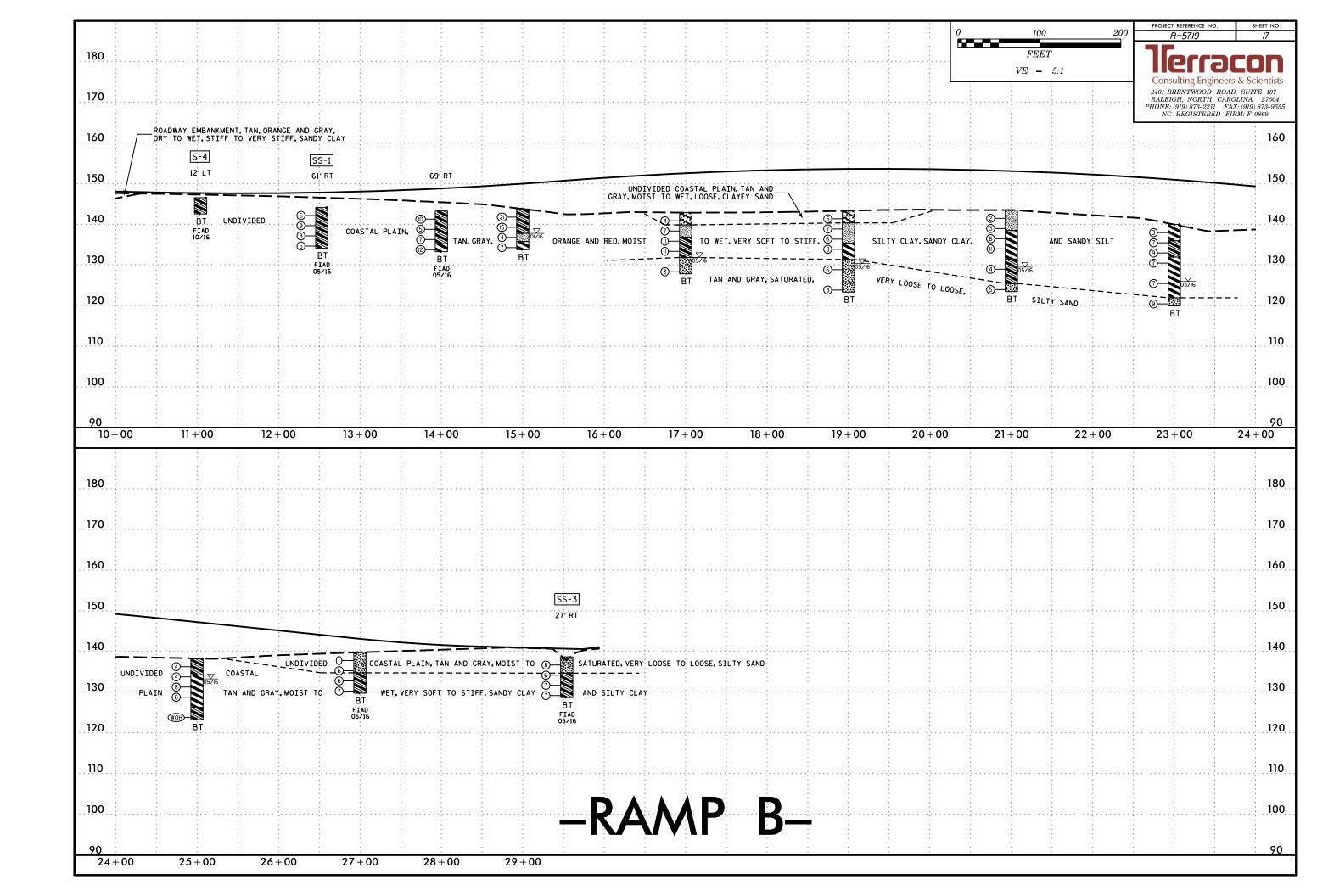


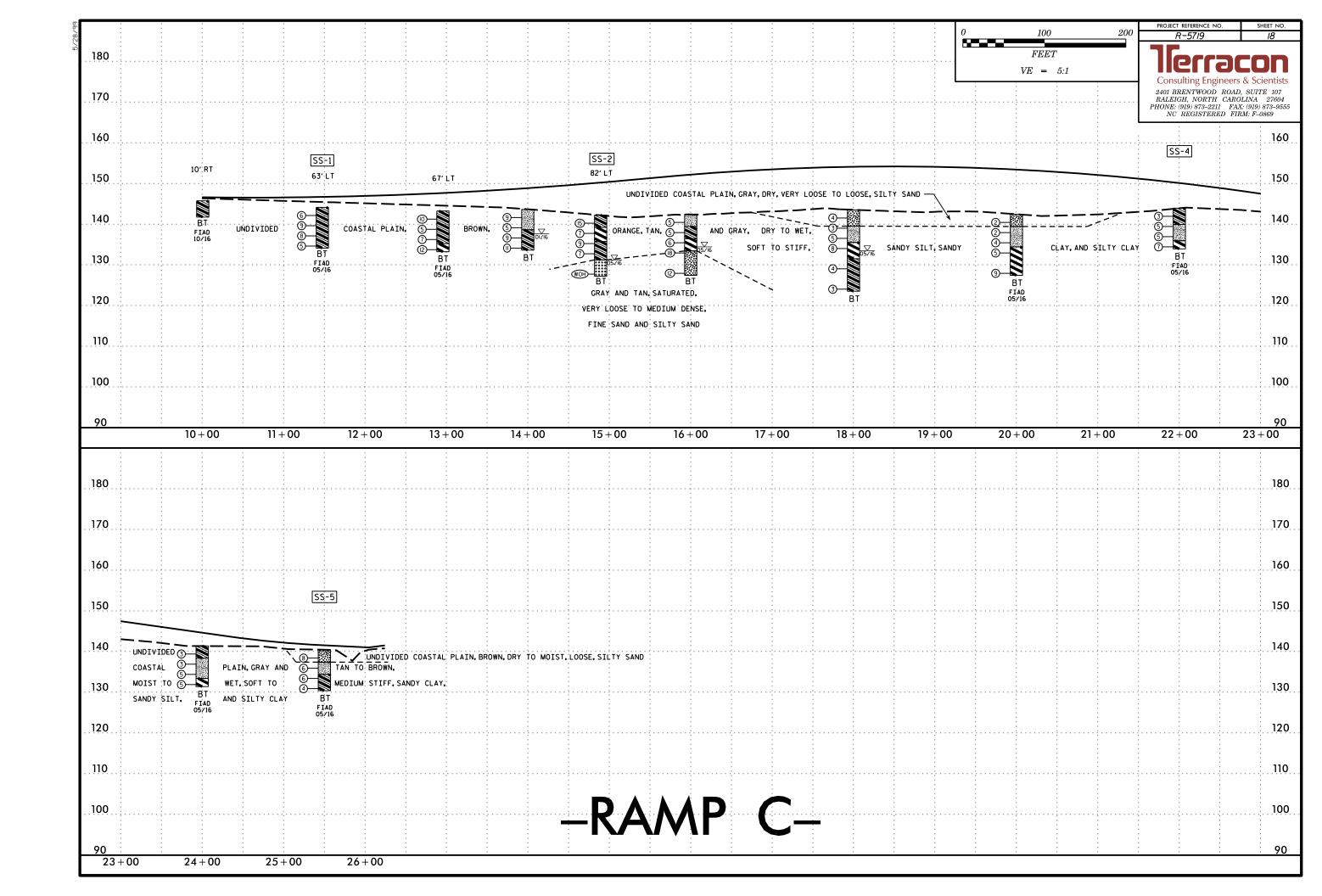


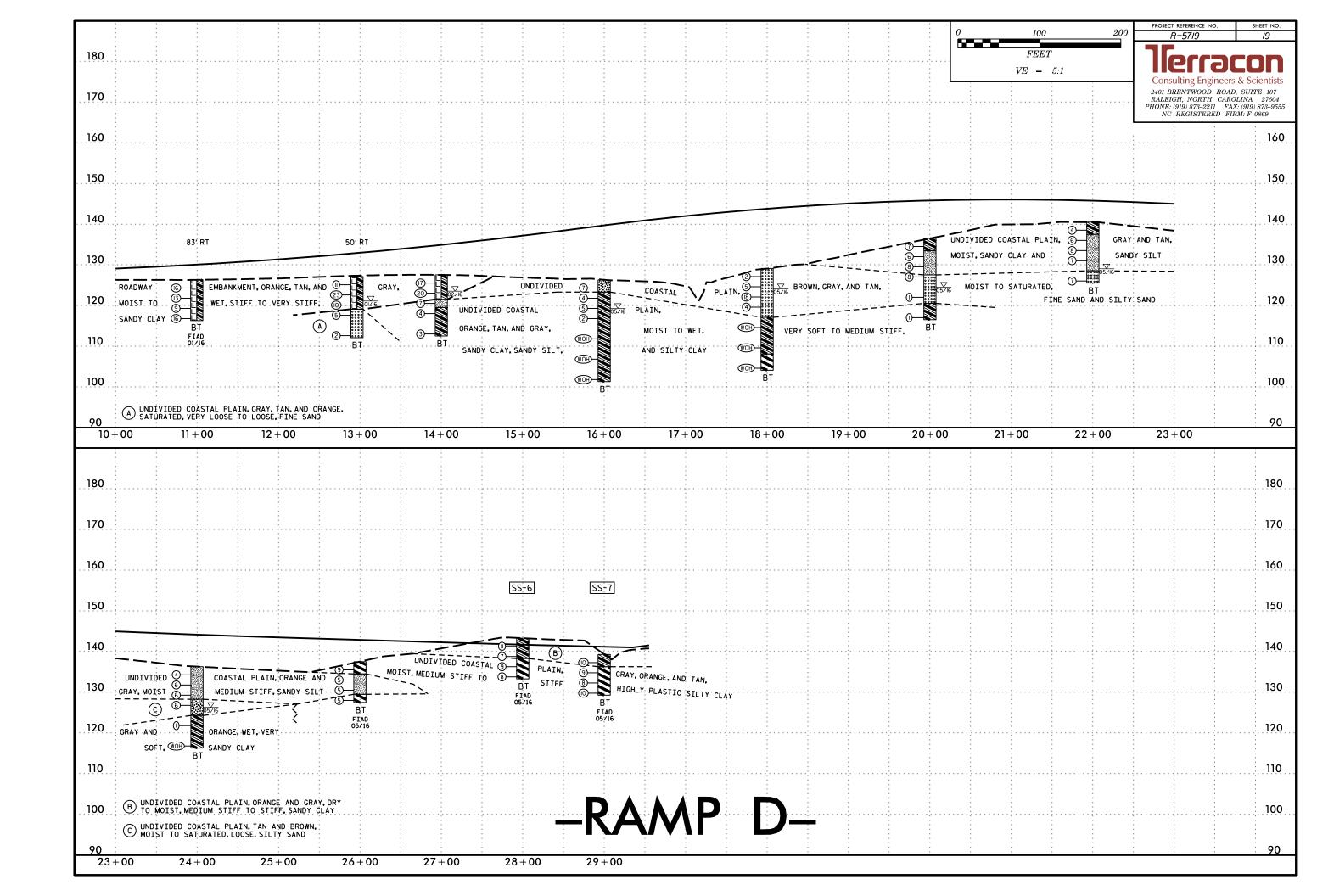


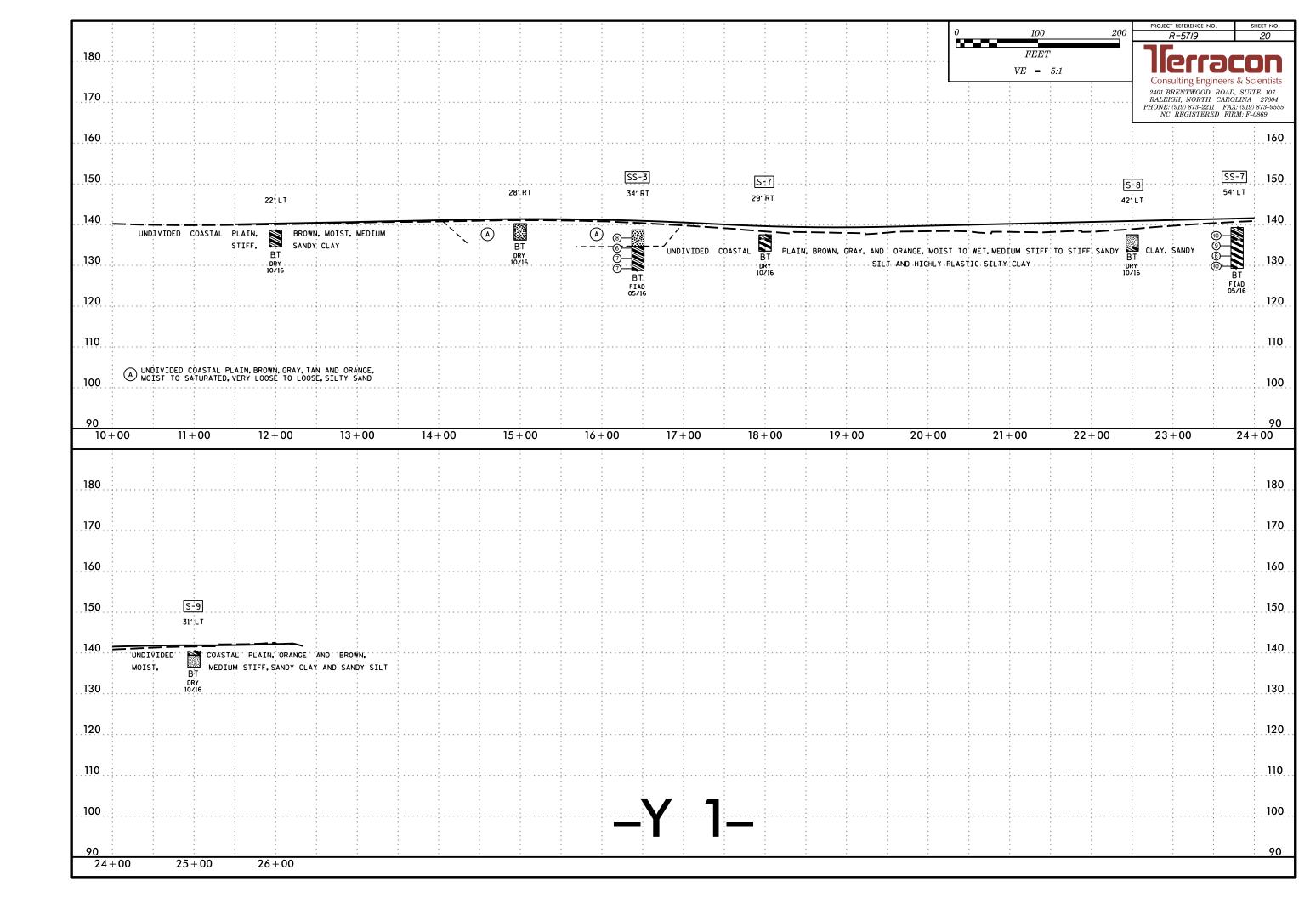


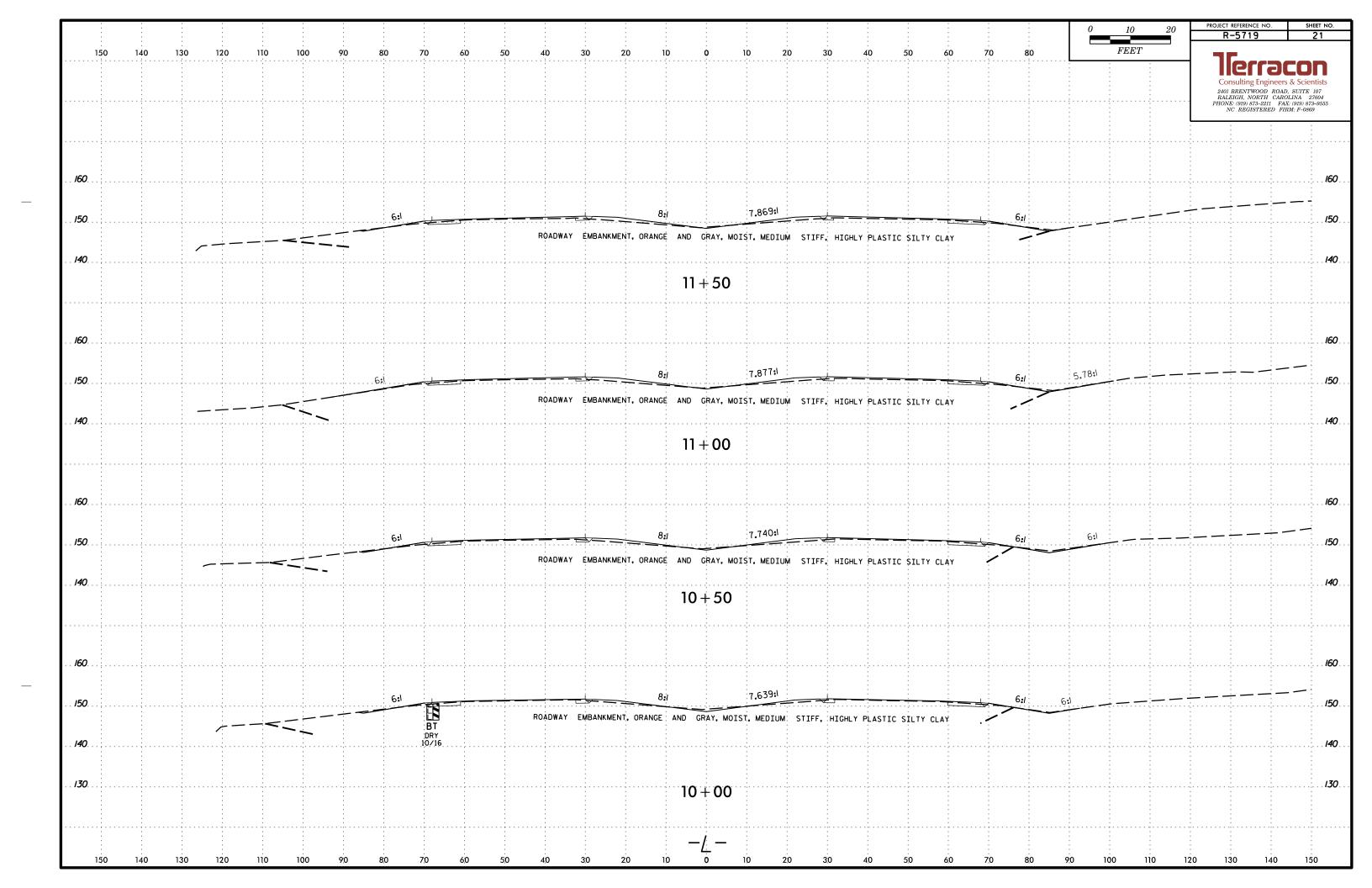


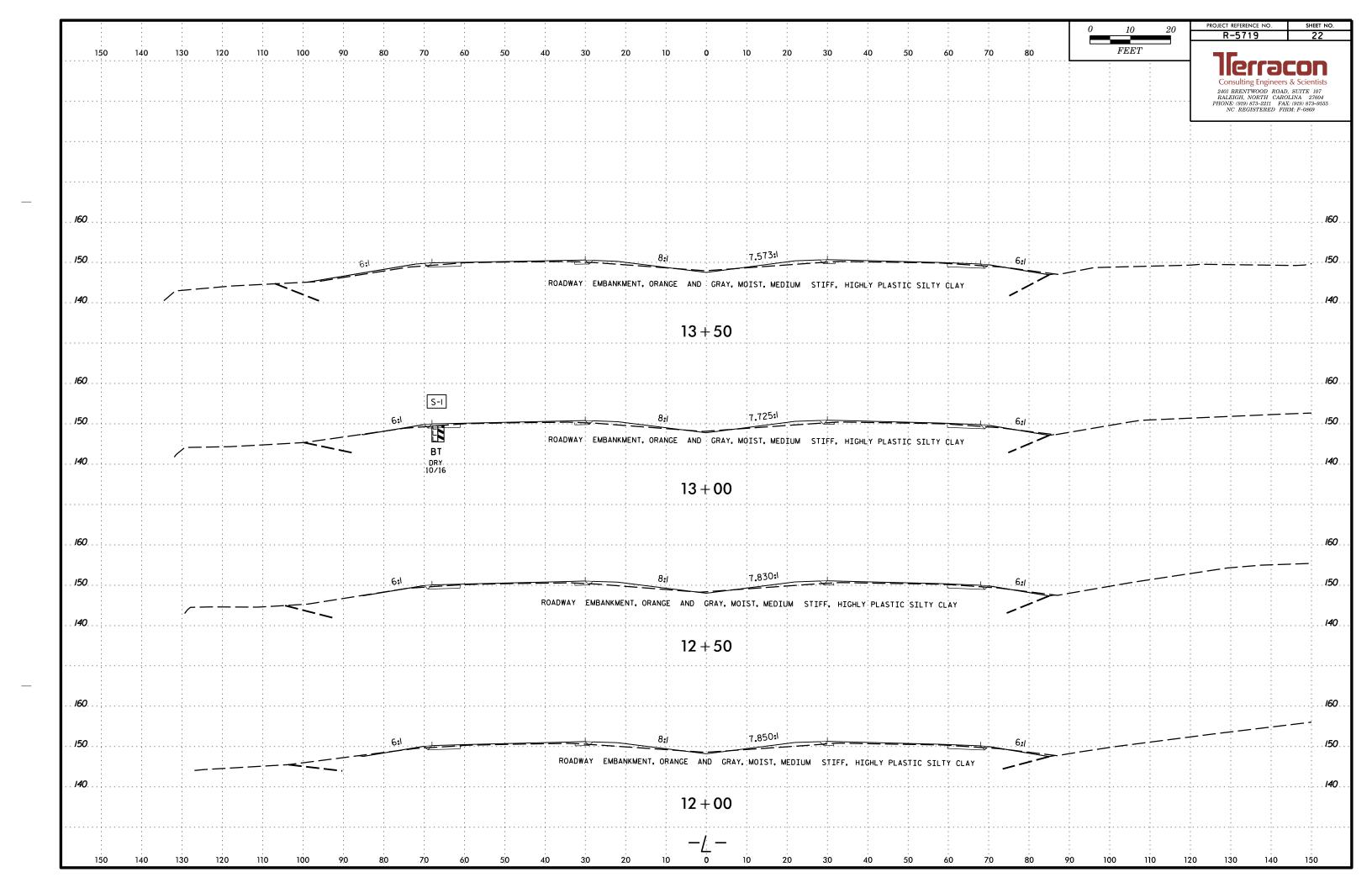


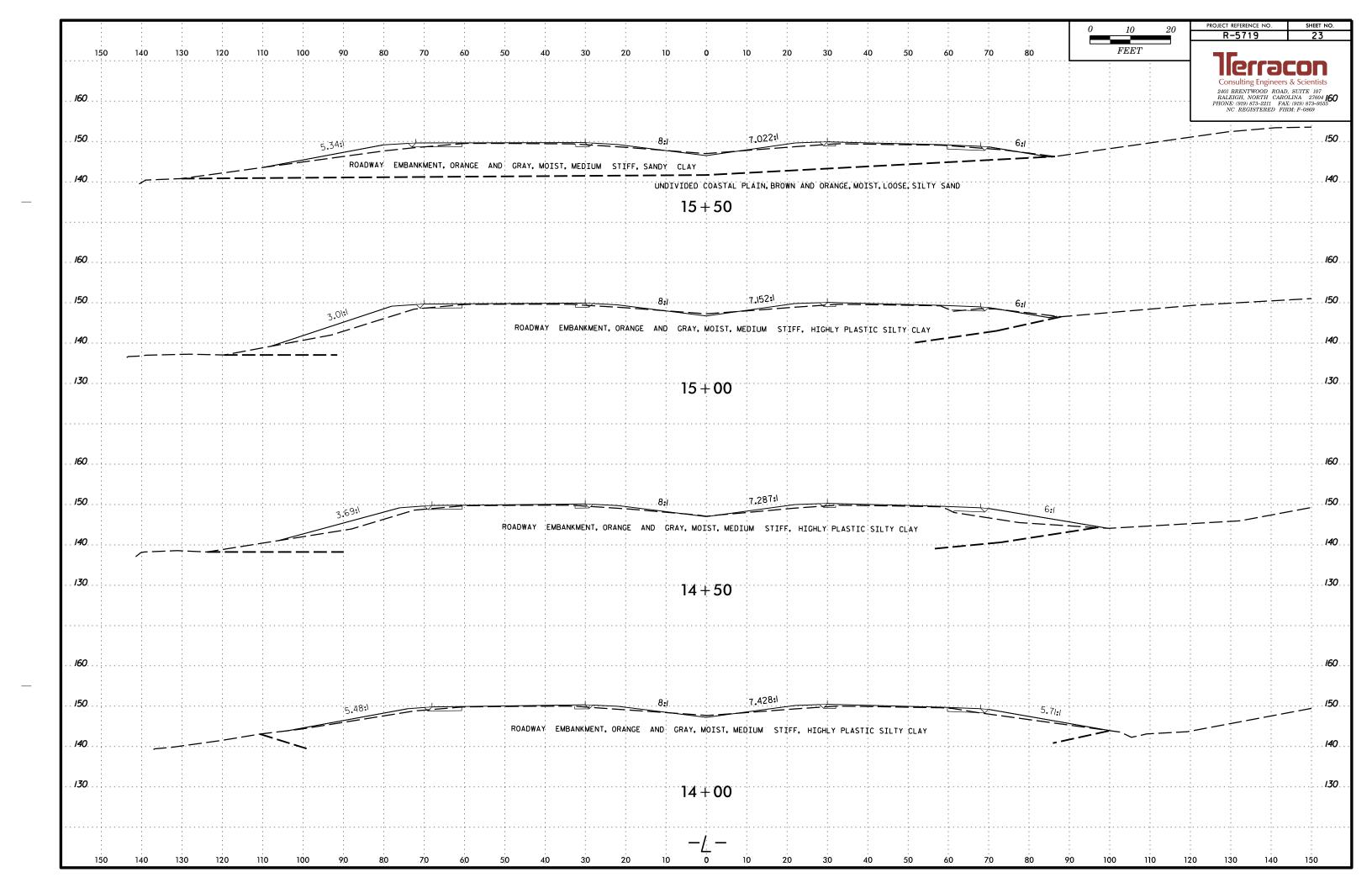


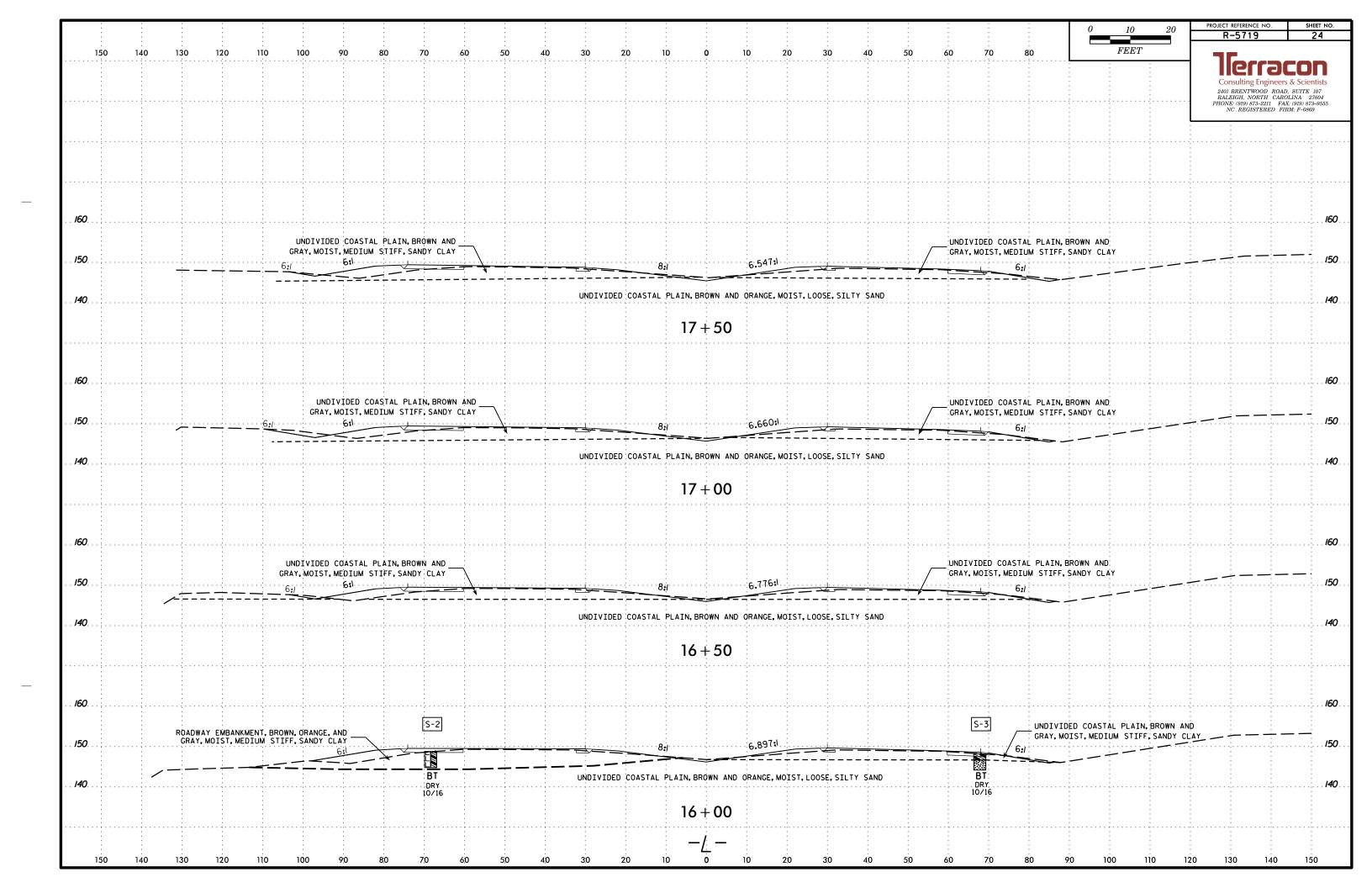




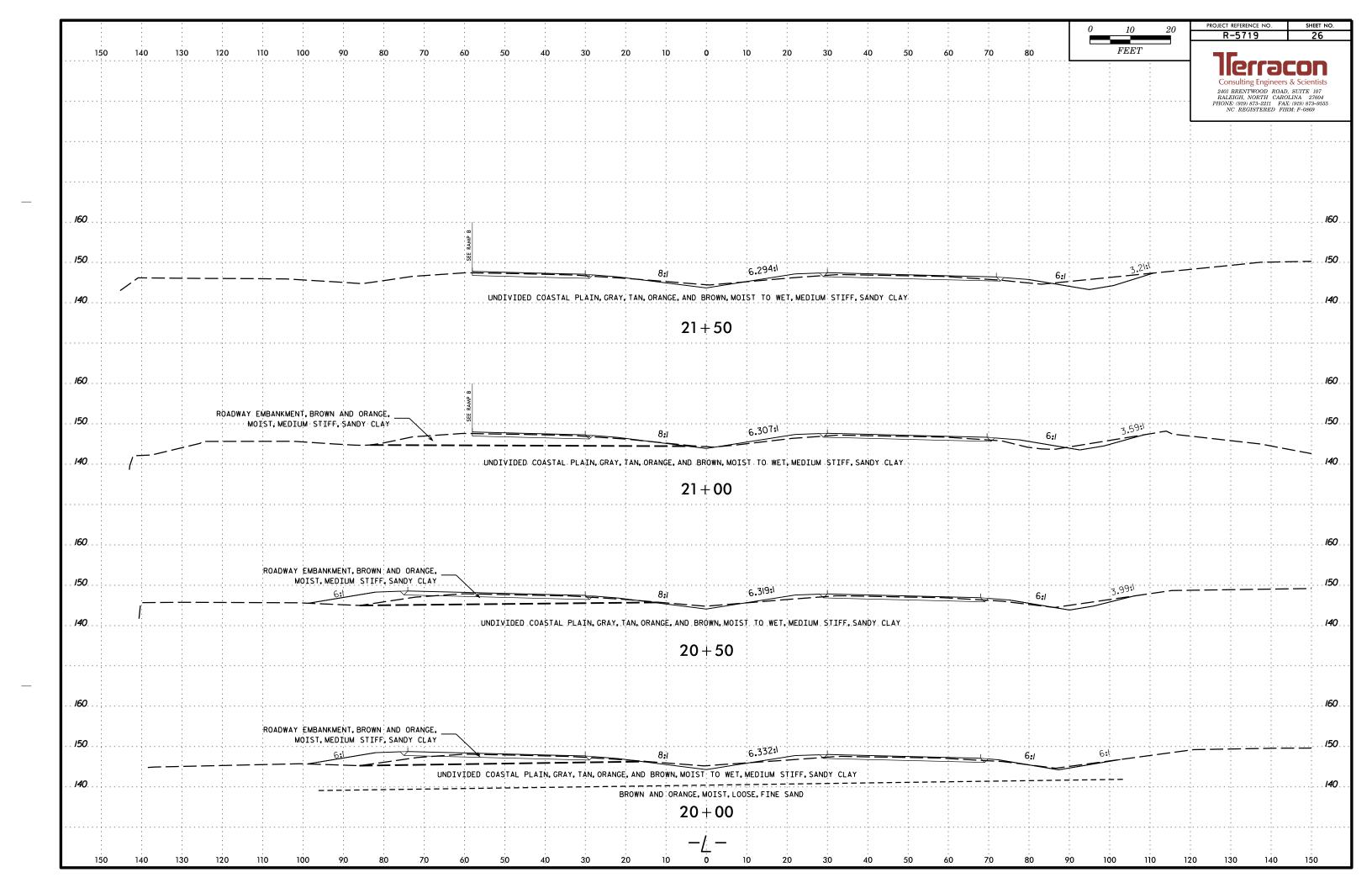


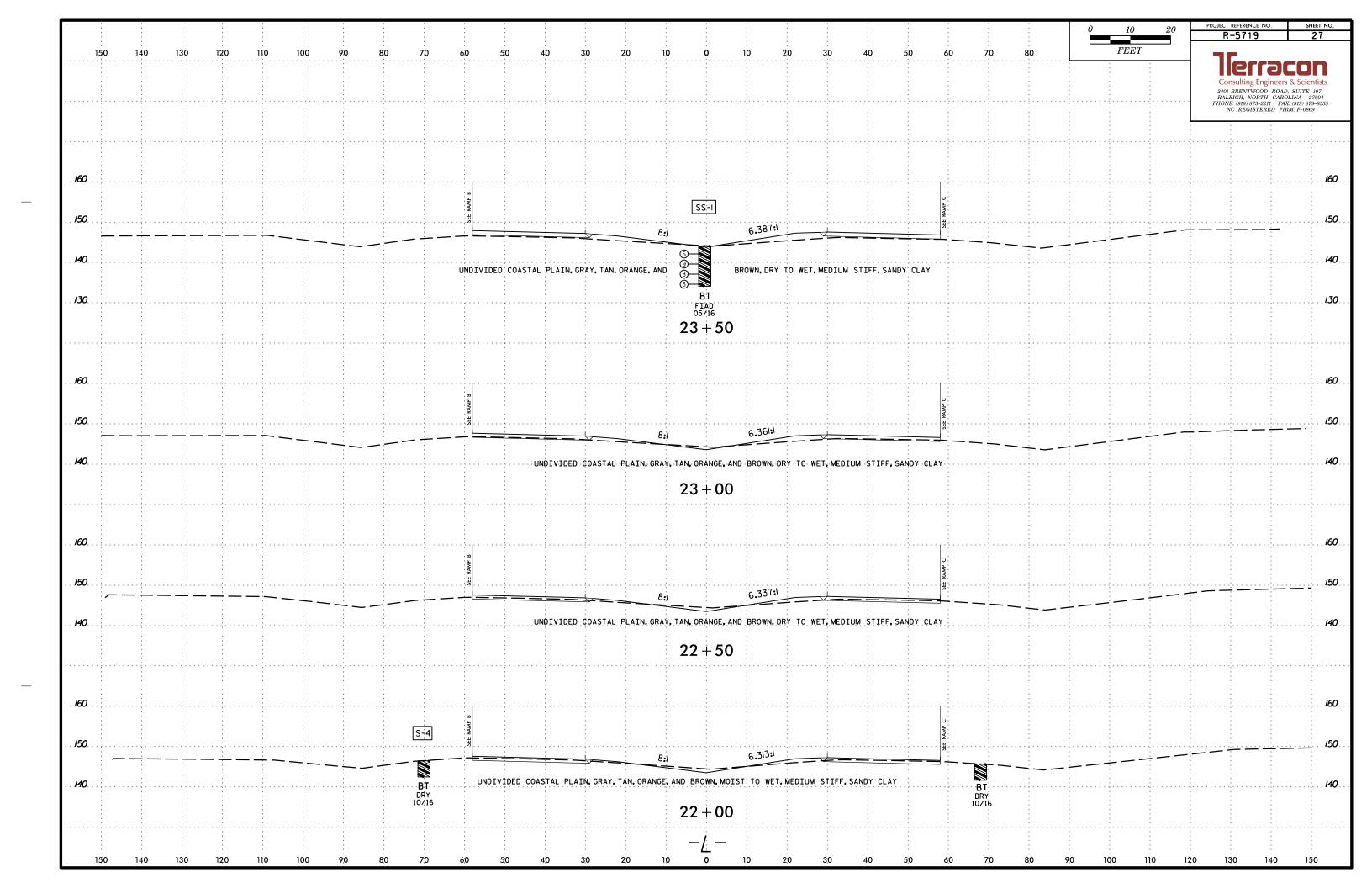


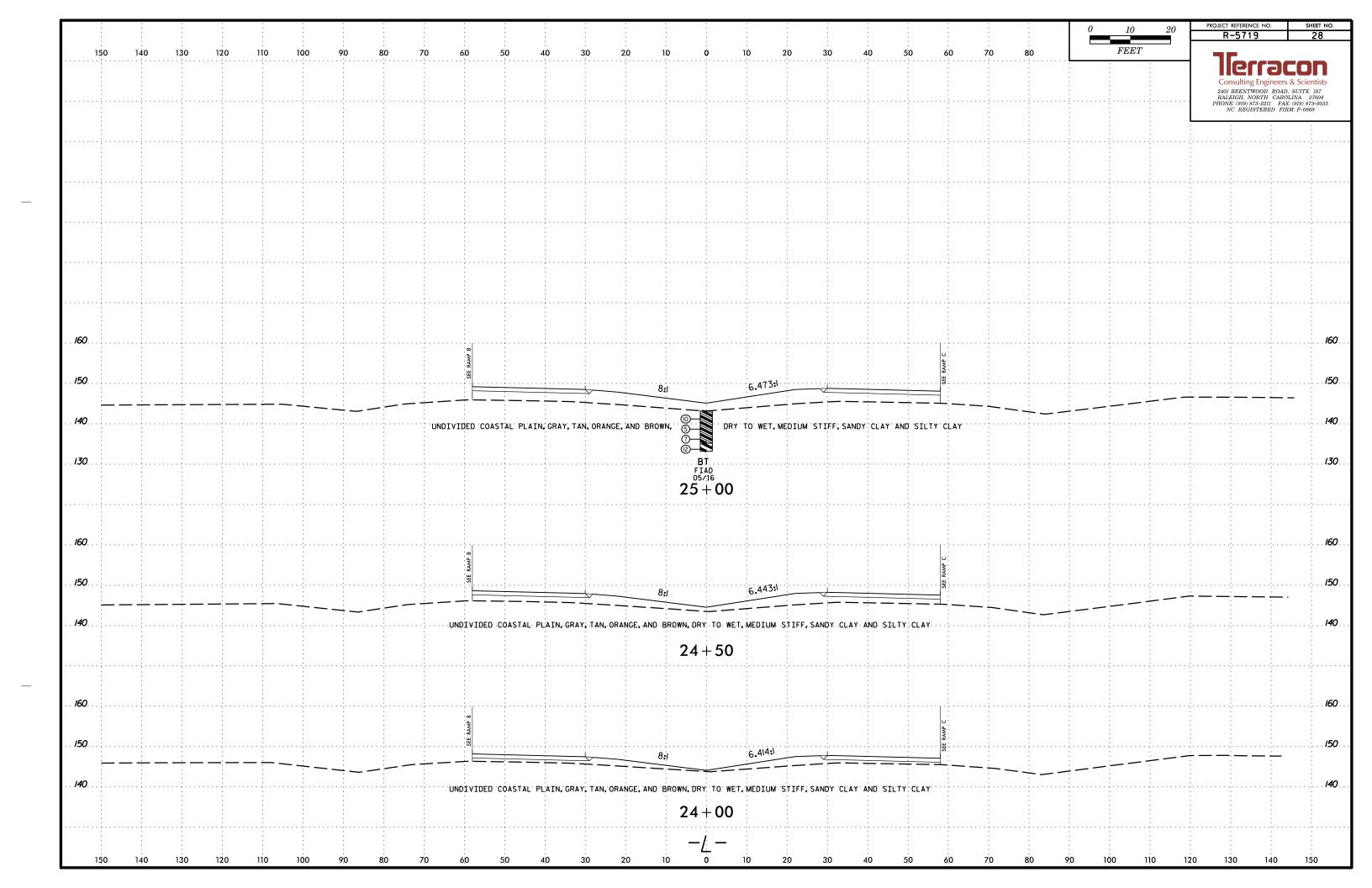


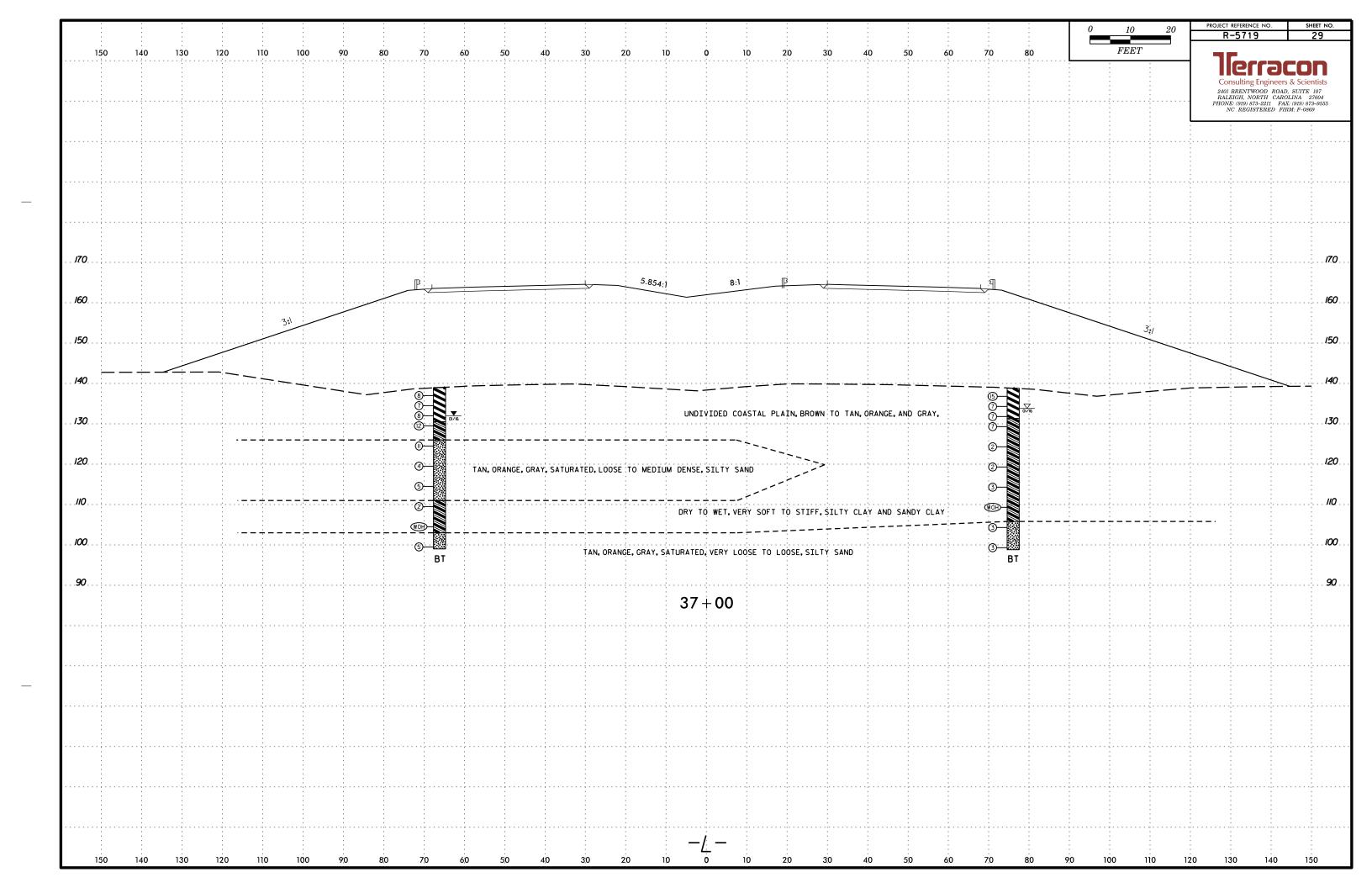


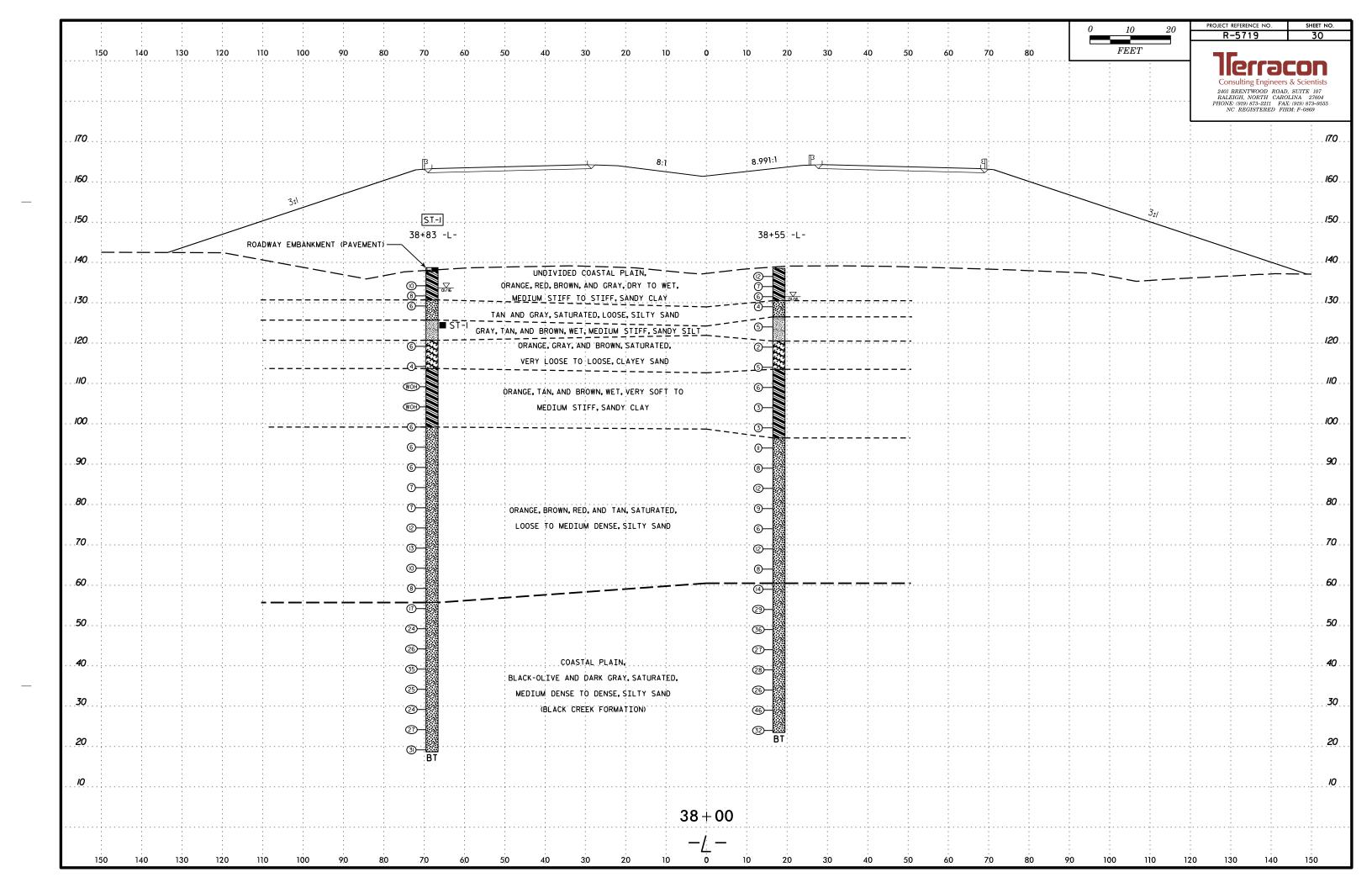
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				Consulting Engineers & Scientist 2401 BRENTWOOD ROAD, SUITE 107 RALEIGH, NORTH CAROLINA 27604
				PHONE: (919) 873–2211 FAX: (919) 873–95 NC REGISTERED FIRM: F-0869
50	ROADWAY EMBANKMENT, BROWN AND ORANGE,		UNDIVIDED COASTAL PLAIN, BROWN AND	
50	MOIST, MEDIUM STIFF, SANDY CLAY	8:/ 6.345:1	GRAY, MOIST, MEDIUM STIFF, SANDY CLAY 6:/	<u> </u>
10.		UNDIVIDED COASTAL PLAIN, BROWN AND ORANGE, MOIST, LOOSE, FINE SAND		
		19 + 50		
o				
50	ROADWAY EMBANKMENT, BROWN AND ORANGE,		UNDIVIDED COASTAL PLAIN, BROWN AND GRAY, MOIST, MEDIUM STIFF, SANDY CLAY	
<u> </u>	6:1	8:1 6.358:1	6:1	
10	BT DRY 10/16	UNDIVIDED COASTAL PLAIN, BROWN AND ORANGE, MOIST, LOOSE, FINE SAND		
		19 + 00		
50	ROADWAY EMBANKMENT, BROWN AND ORANGE,		UNDIVIDED COASTAL PLAIN, BROWN AND	
50	MOIST, MEDIUM STIFF, SANDY CLAY	8:/ 6.382:	GRAY, MOIST, MEDIUM STIFF, SANDY CLAY	
10.		UNDIVIDED COASTAL PLAIN, BROWN AND ORANGE, MOIST, LOOSE, SILTY SAND TO FINE SAND		
		18 + 50		
50				
20	ROADWAY EMBANKMENT, BROWN AND ORANGE,		UNDIVIDED COASTAL PLAIN, BROWN AND GRAY, MOIST, MEDIUM STIFF, SANDY CLAY	
io	6:1	8:1 6.438:1	6:1 6:1	
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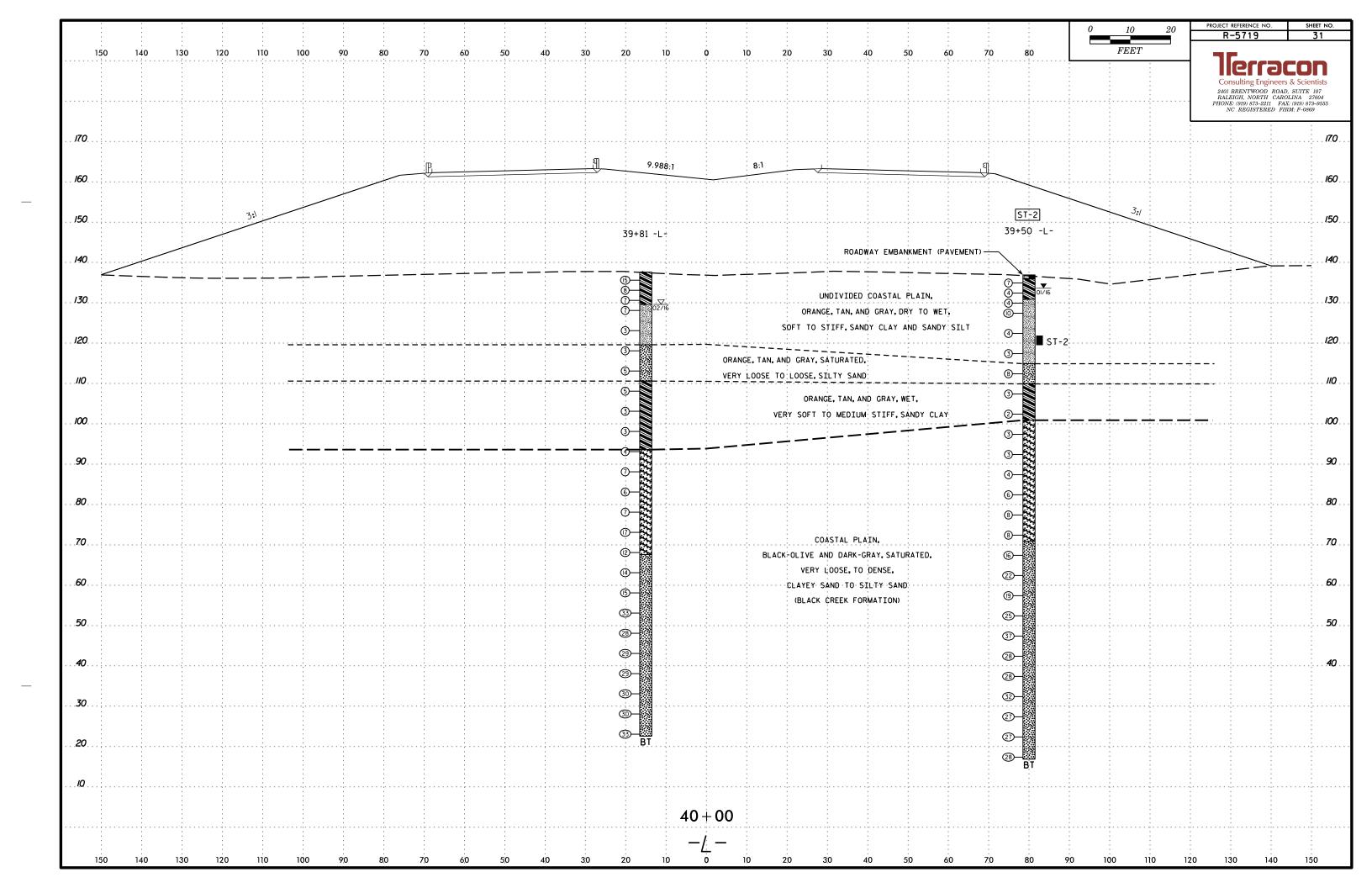


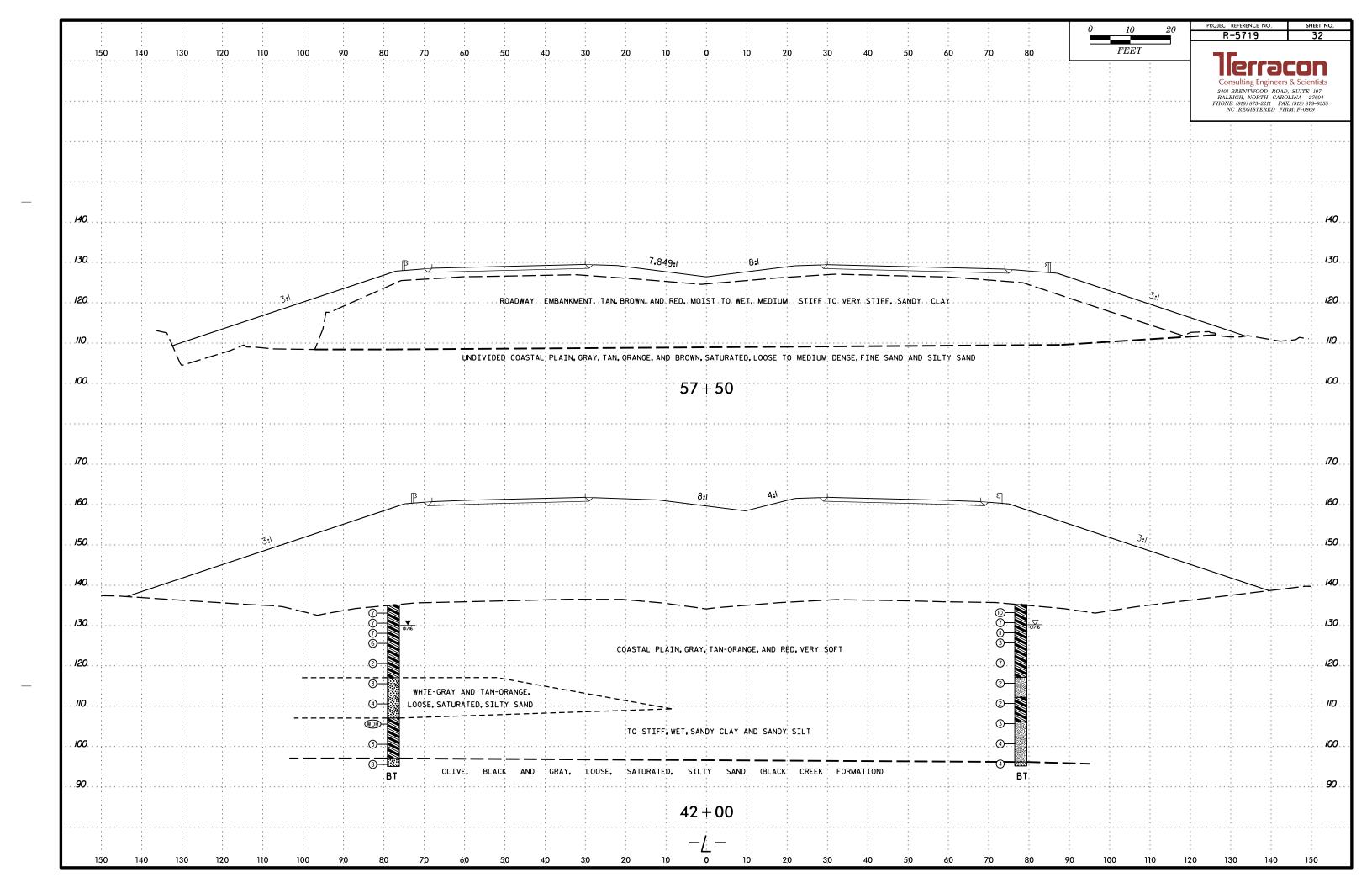


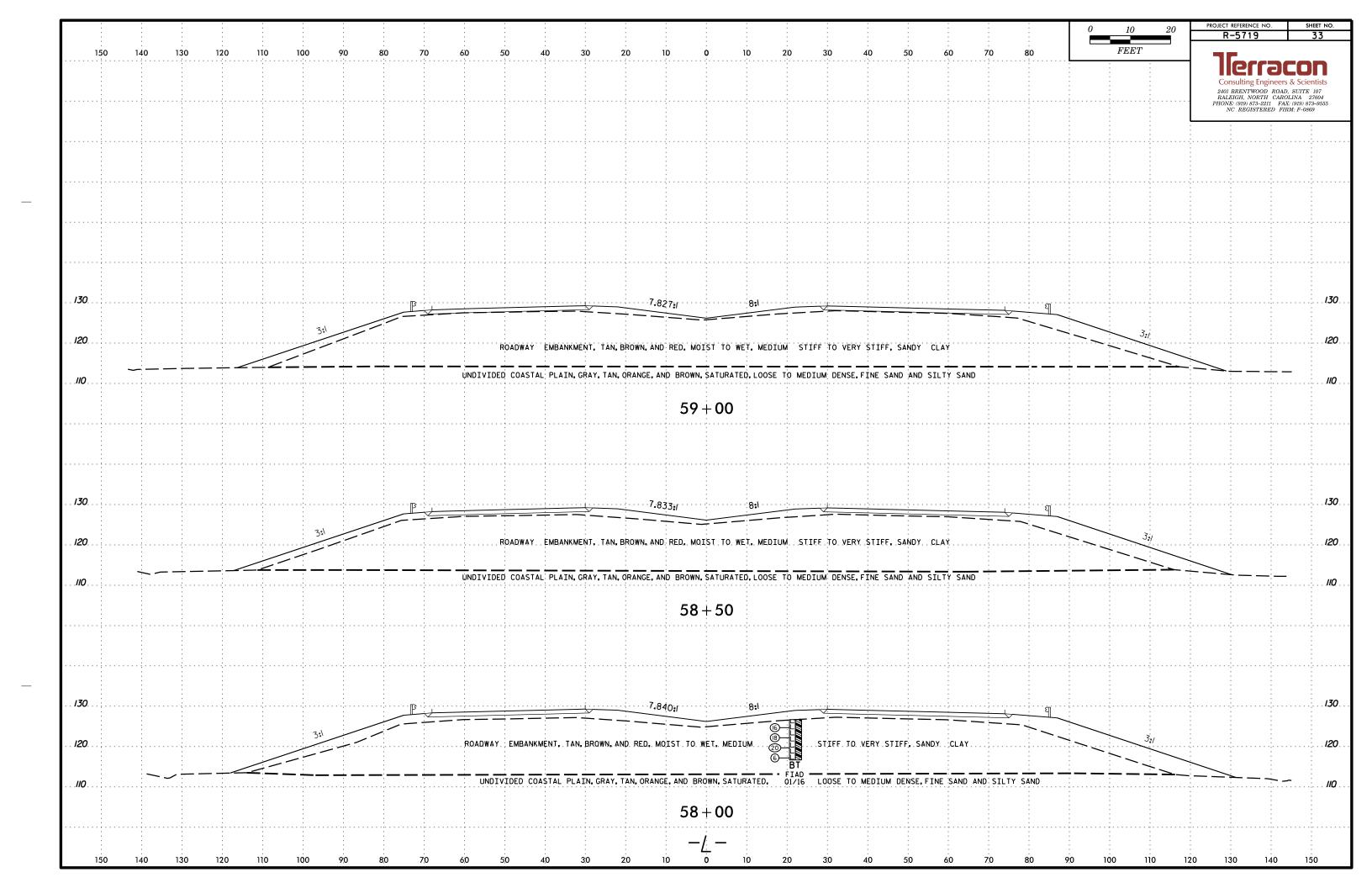


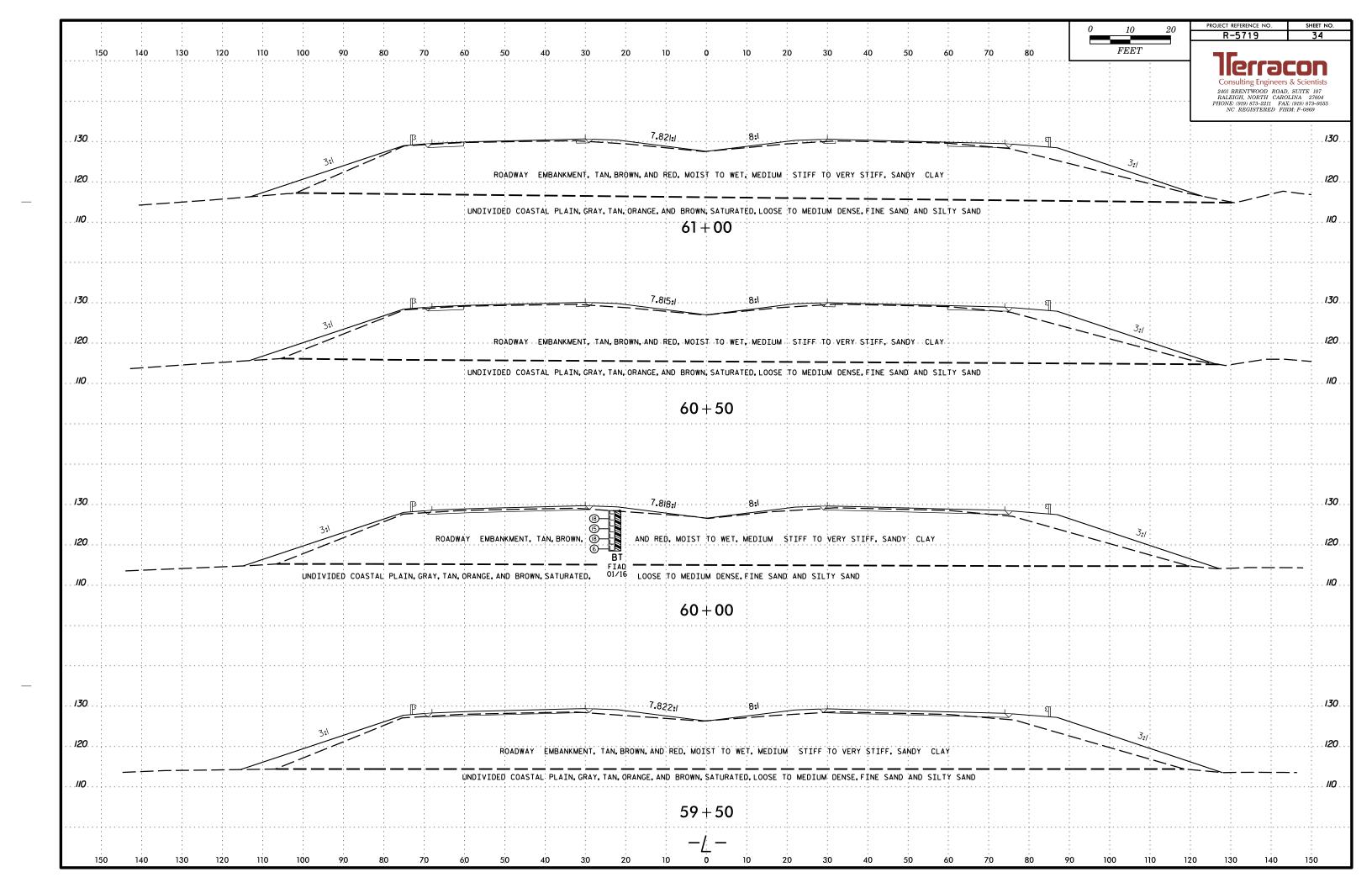


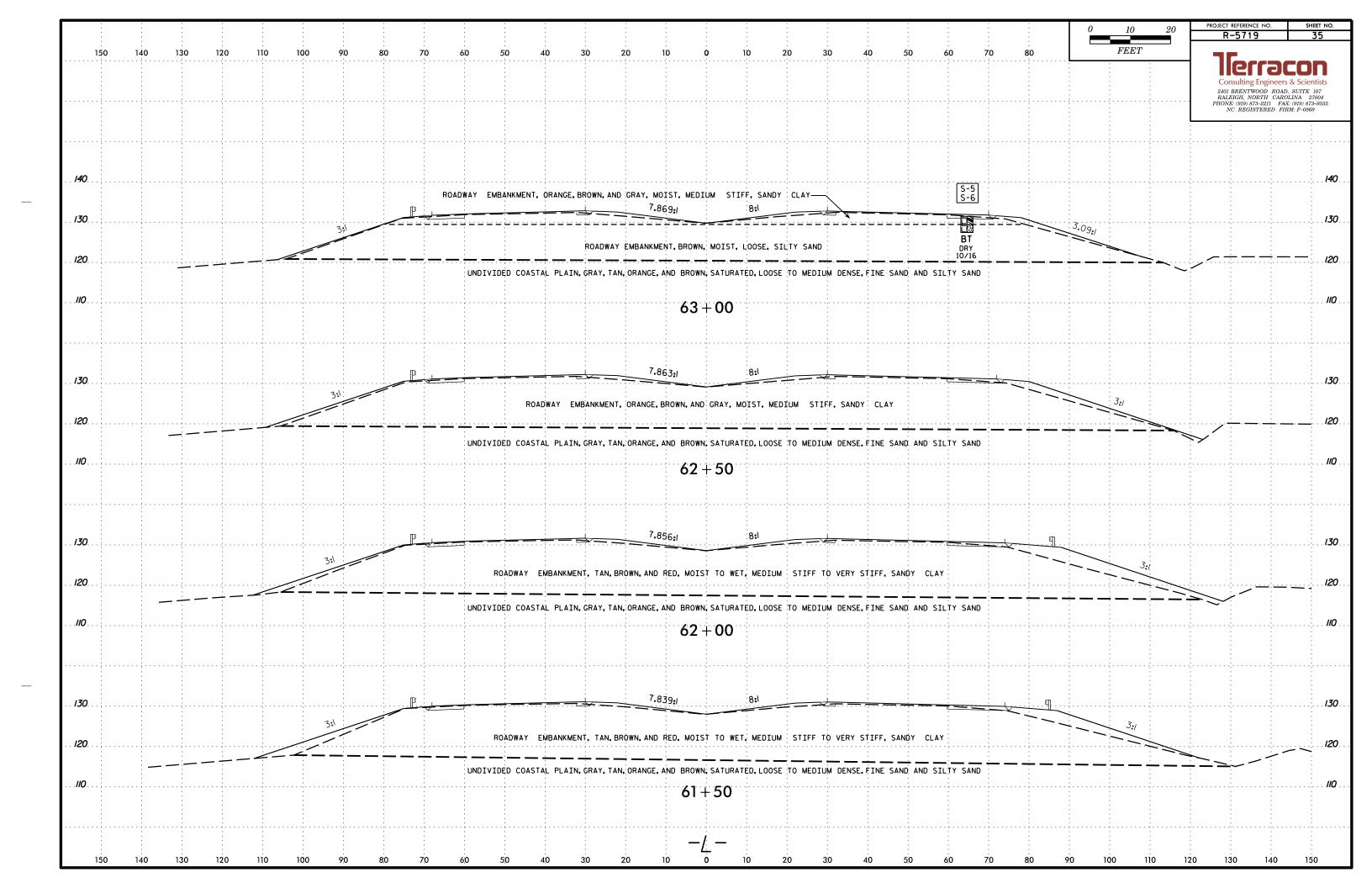


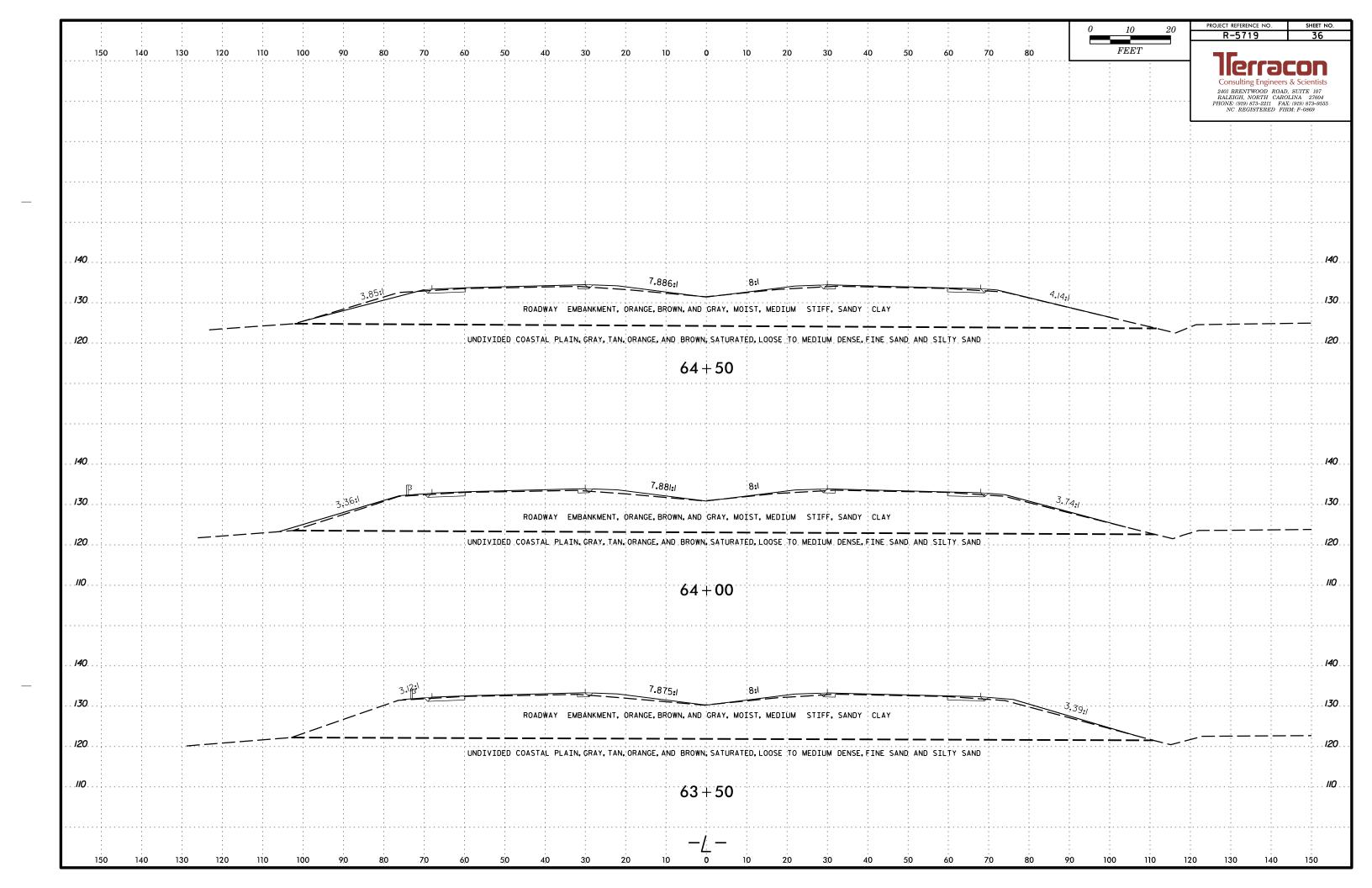


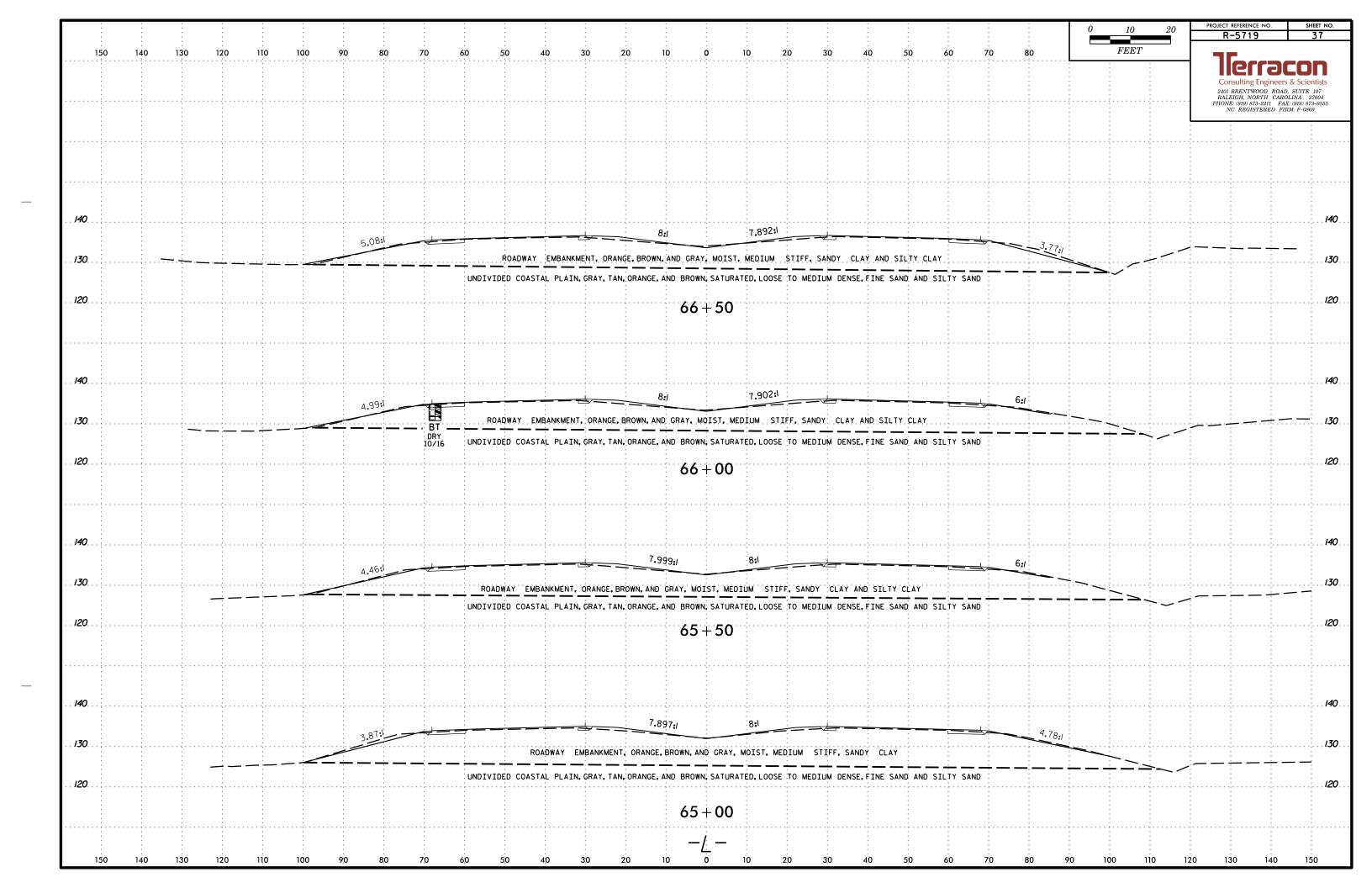


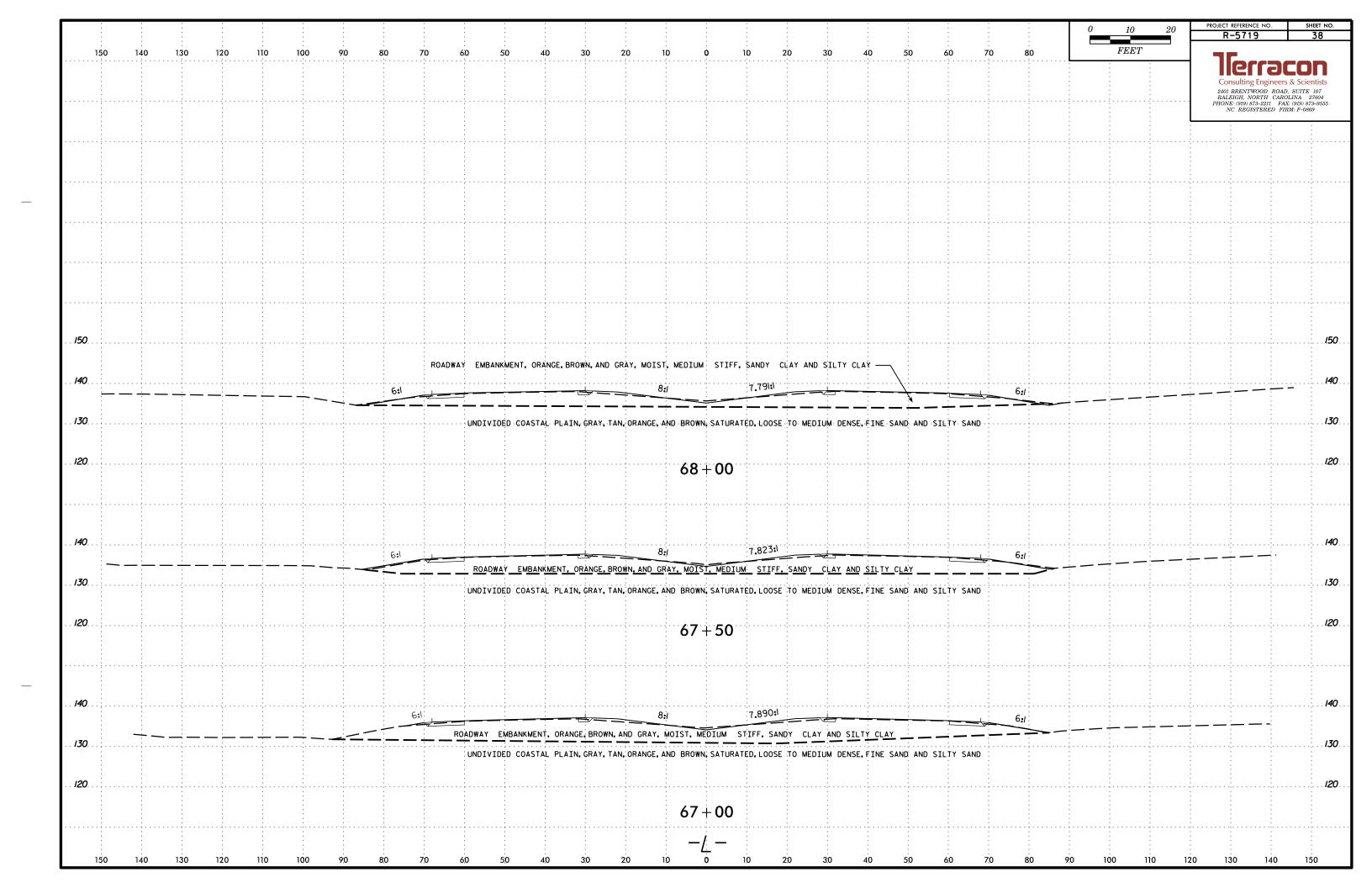


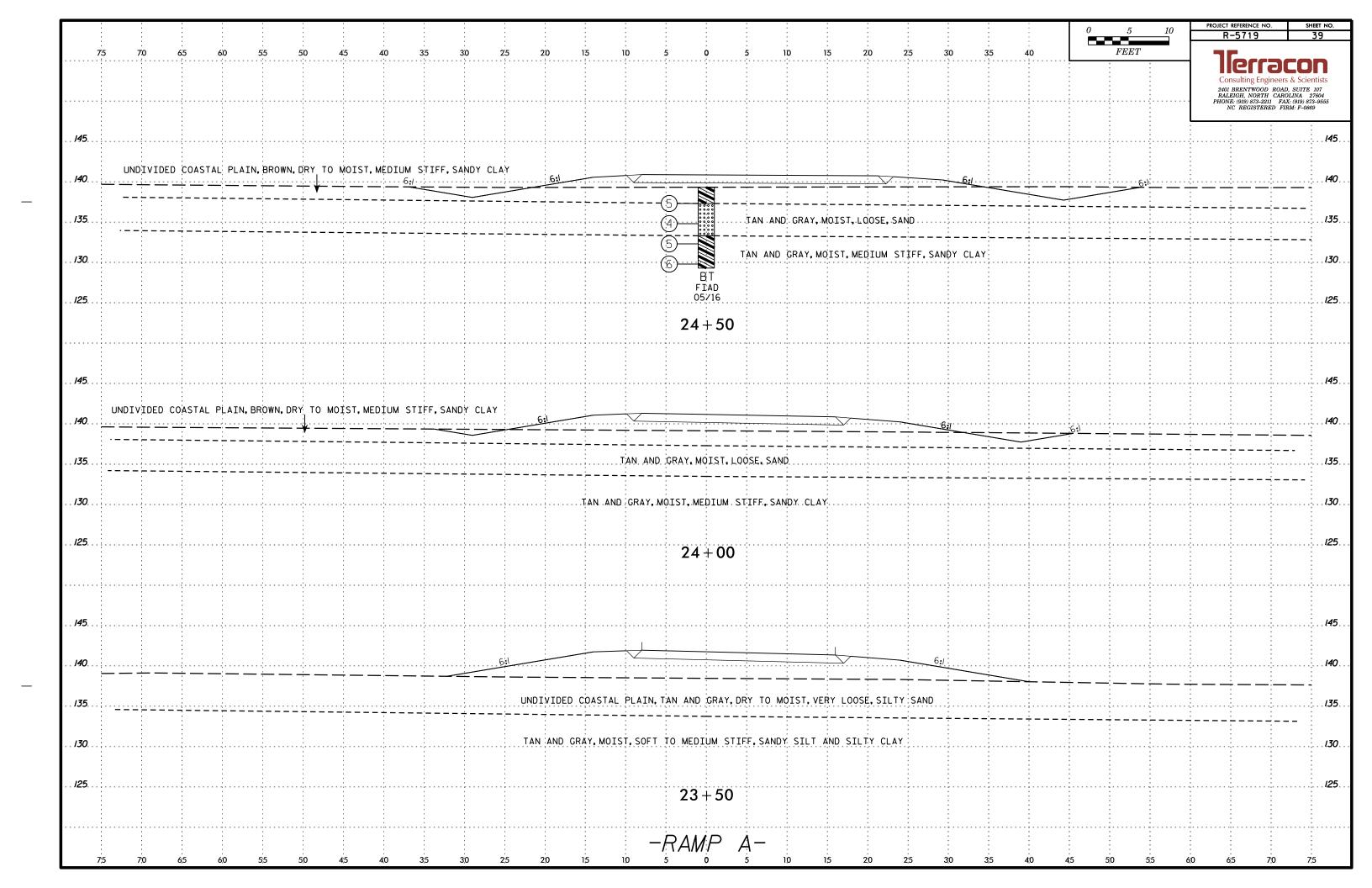


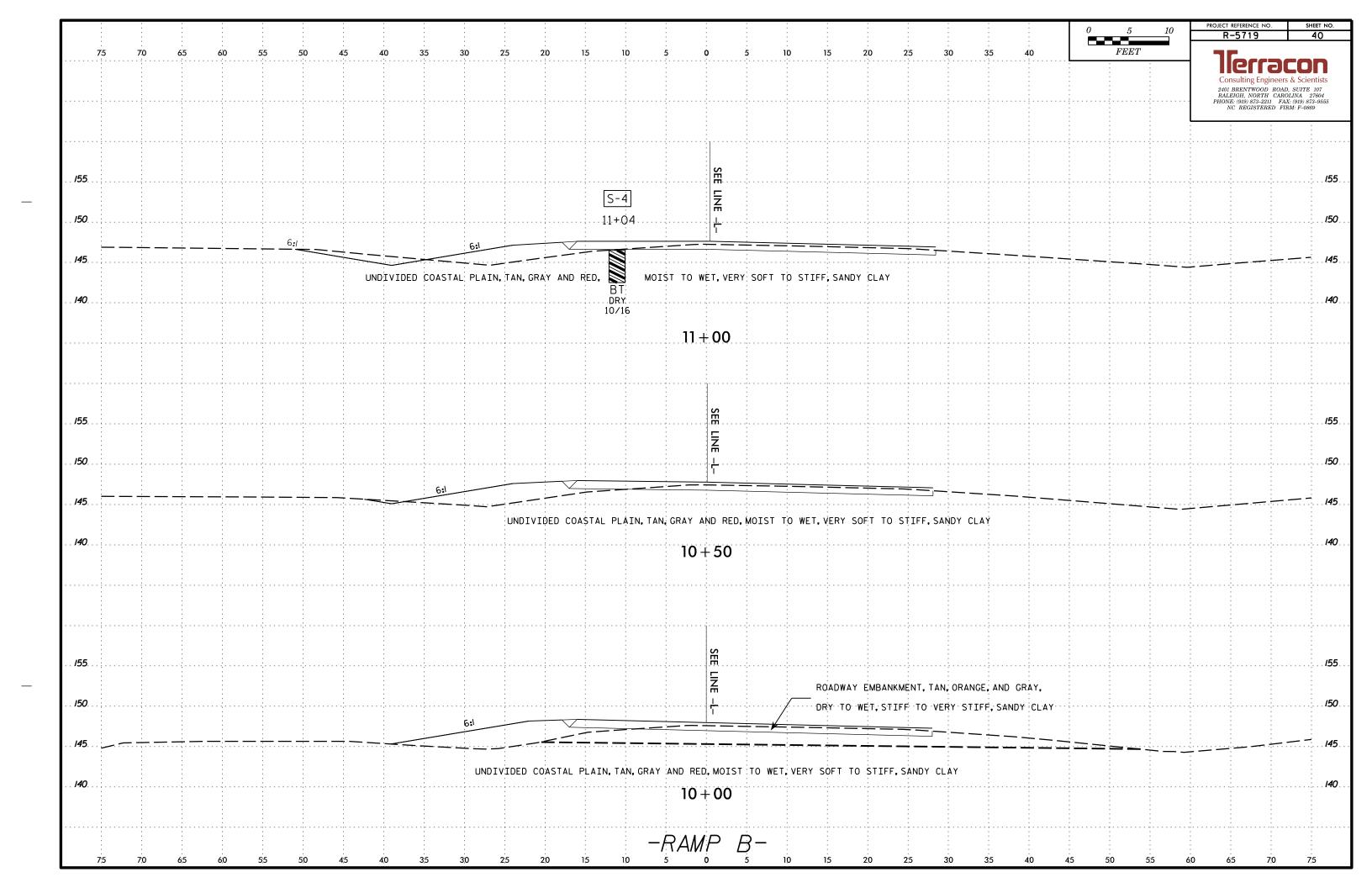


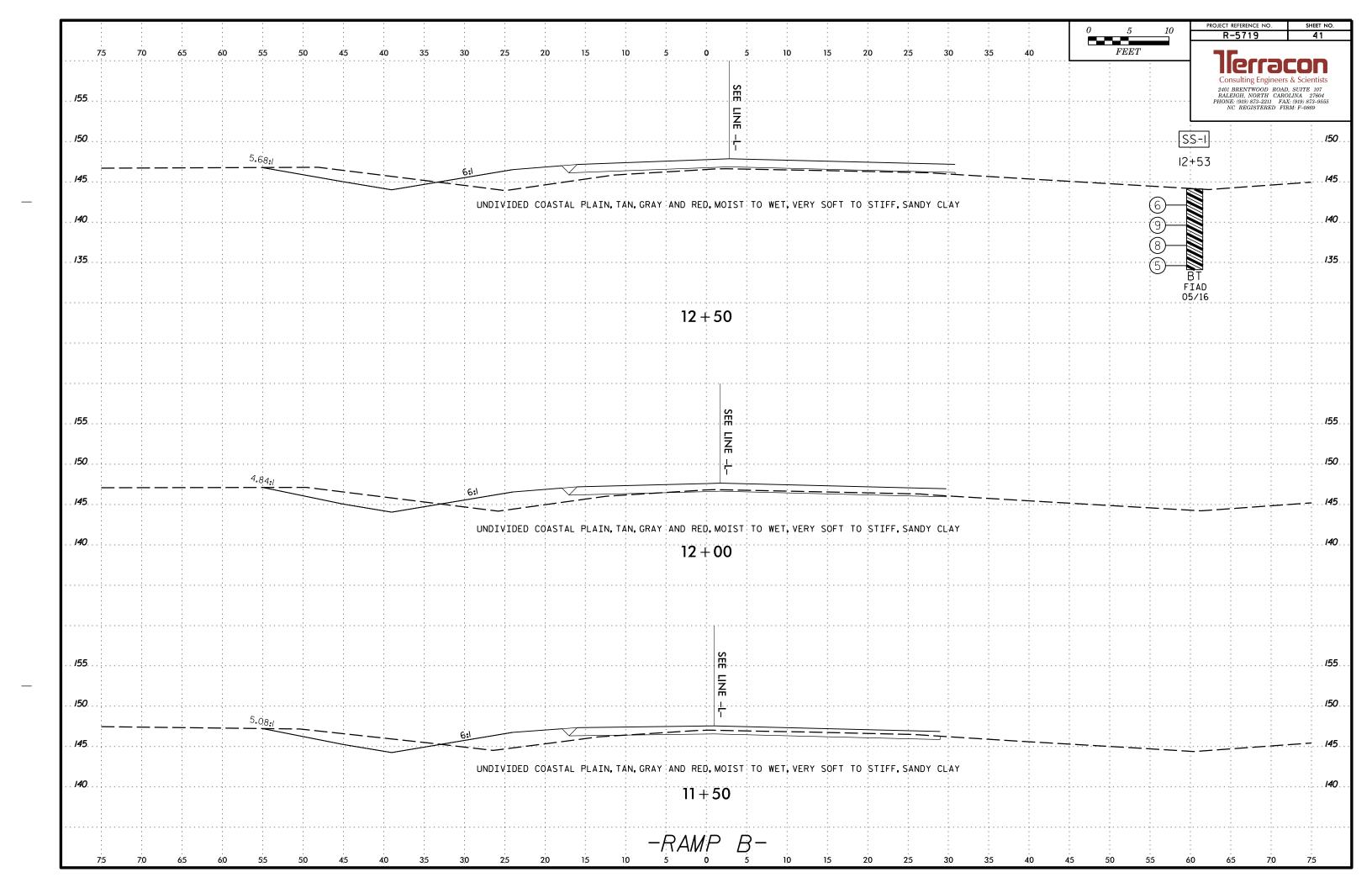


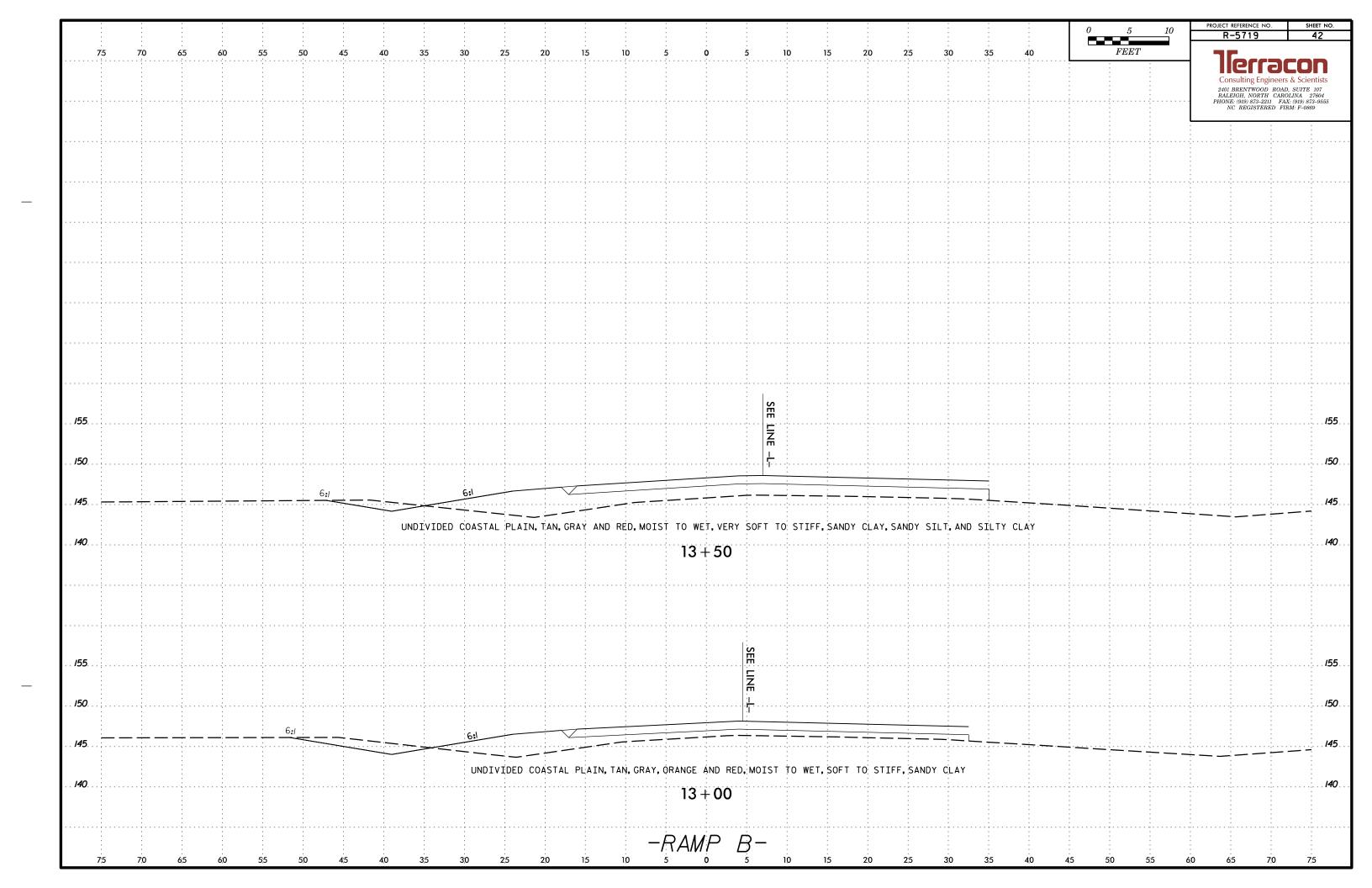


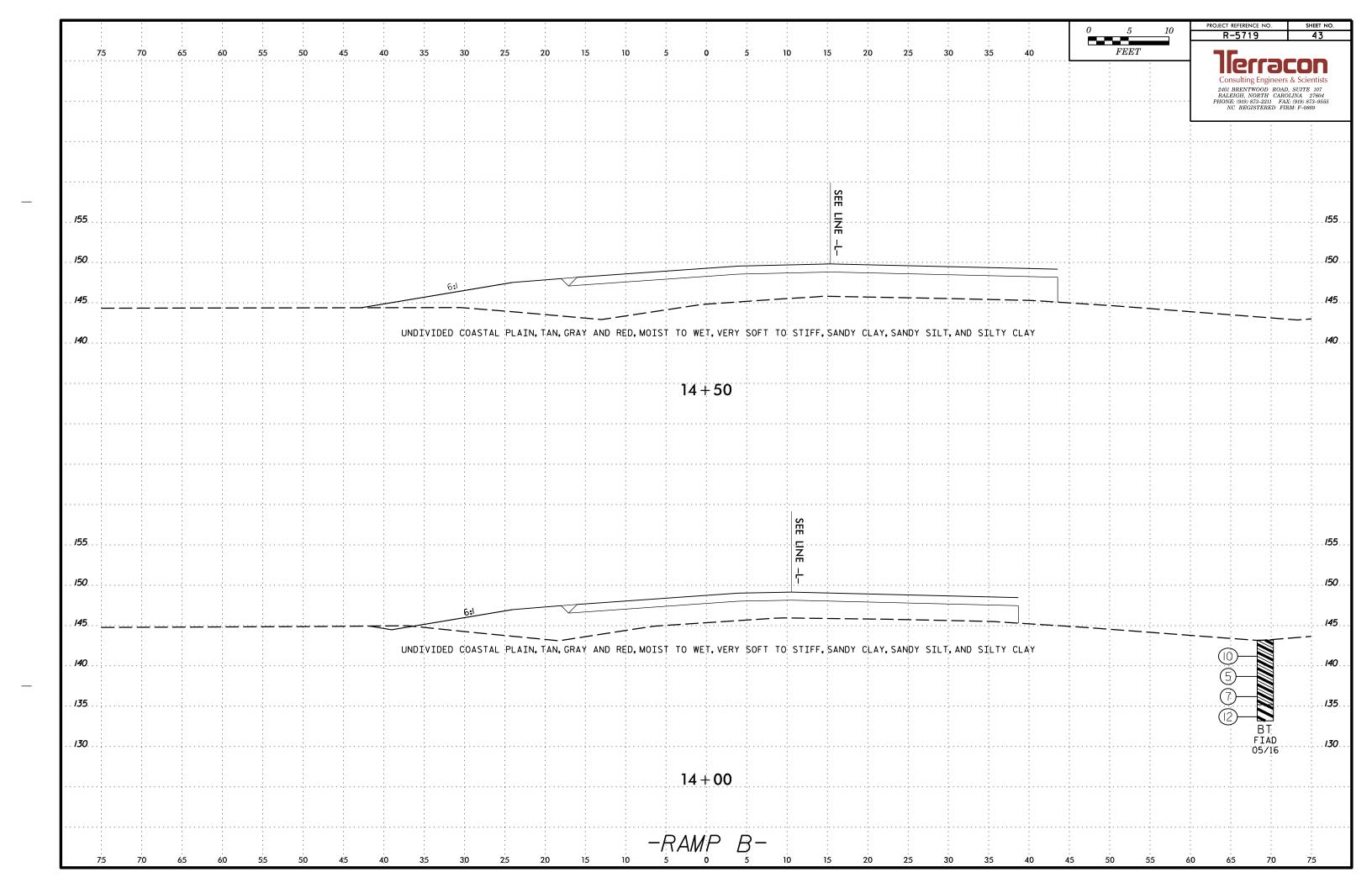


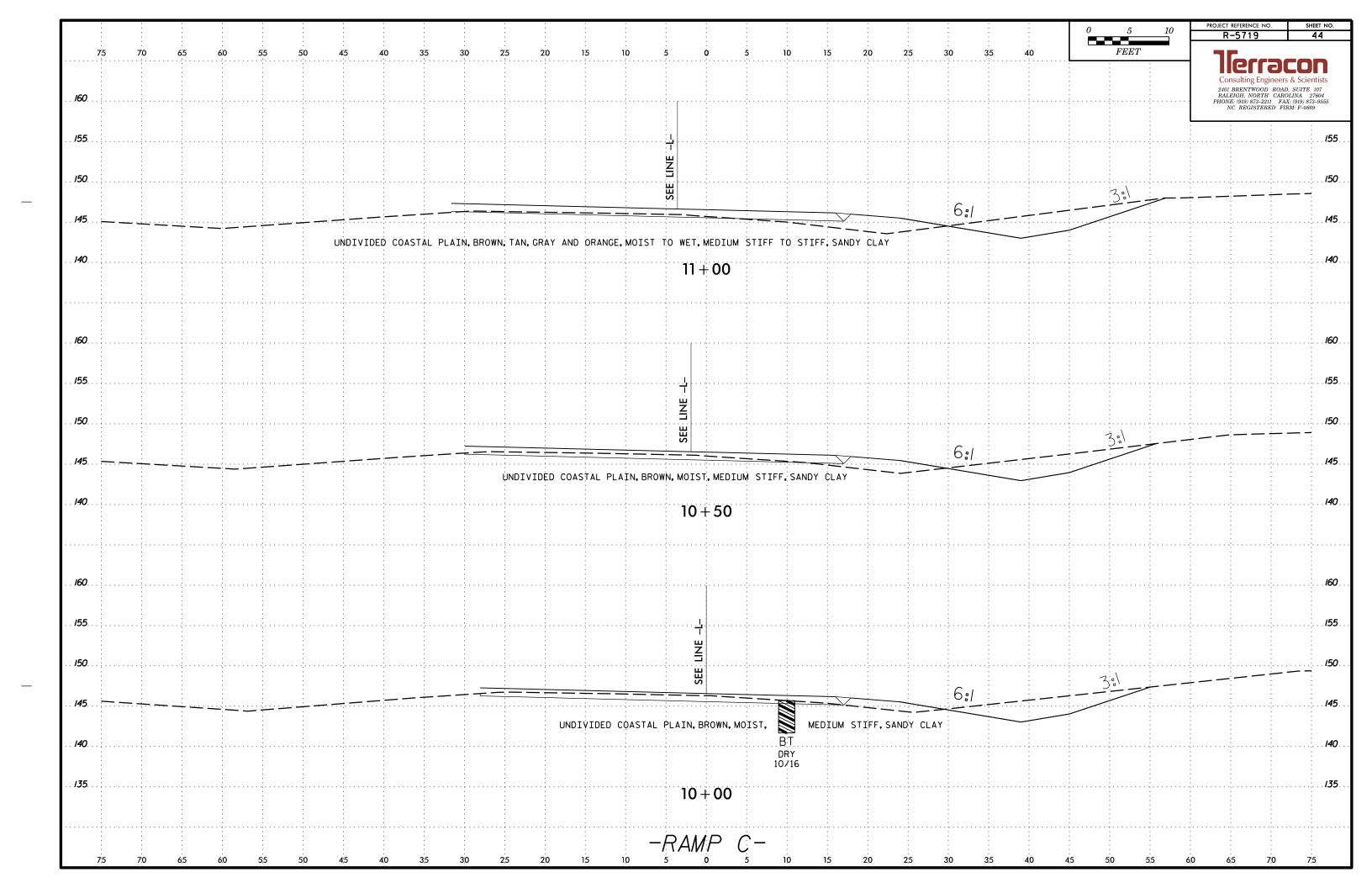


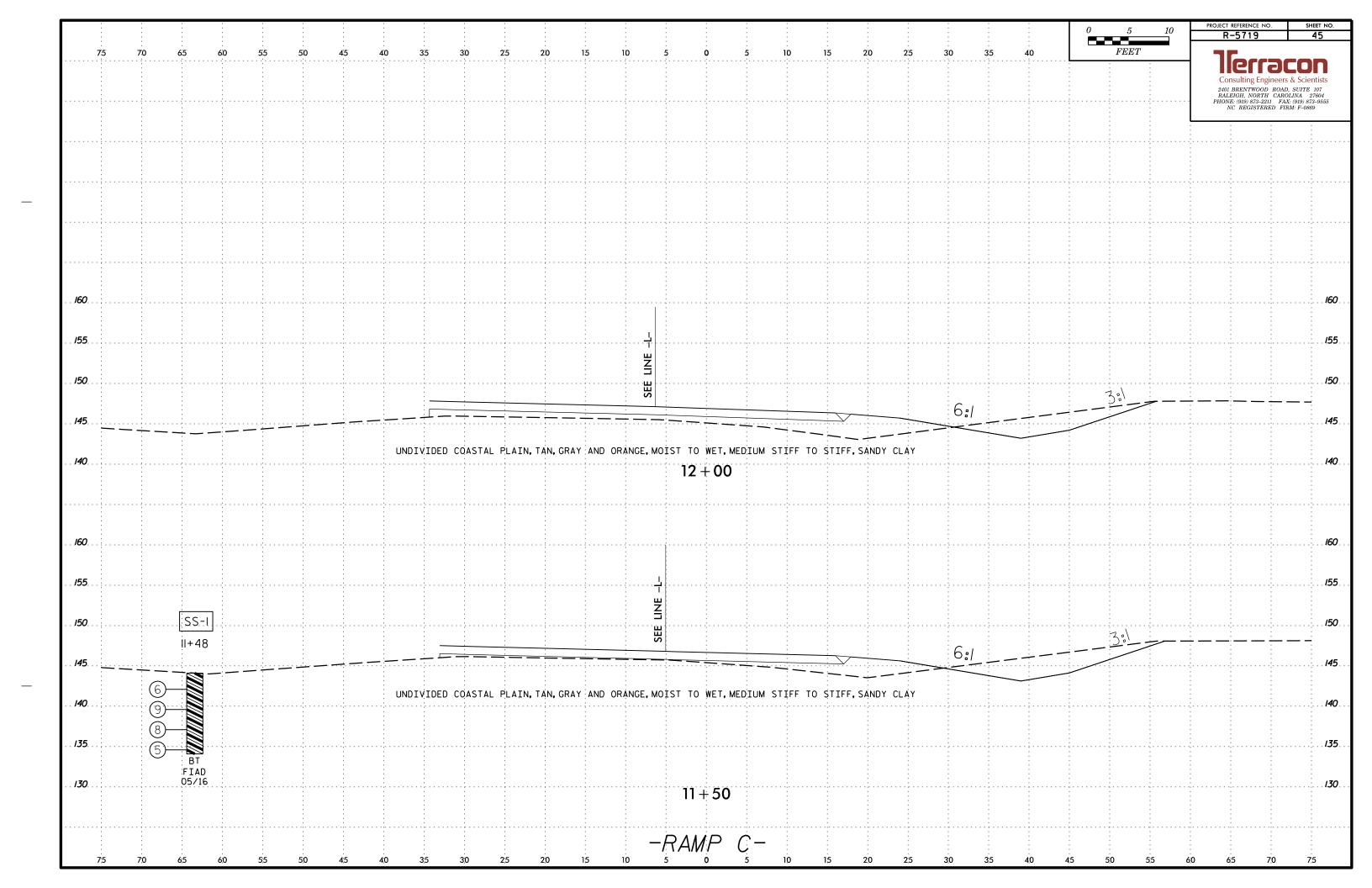


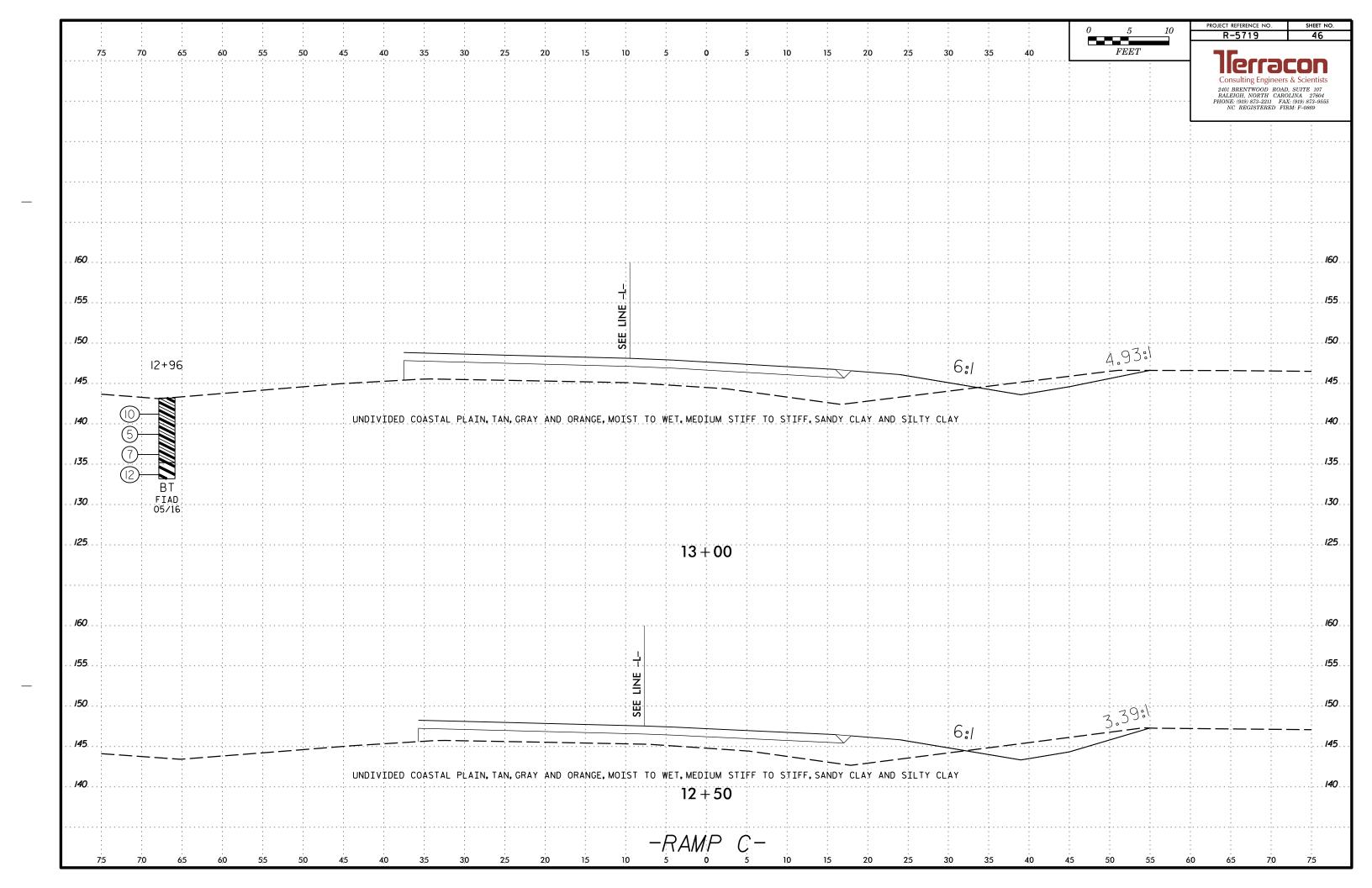


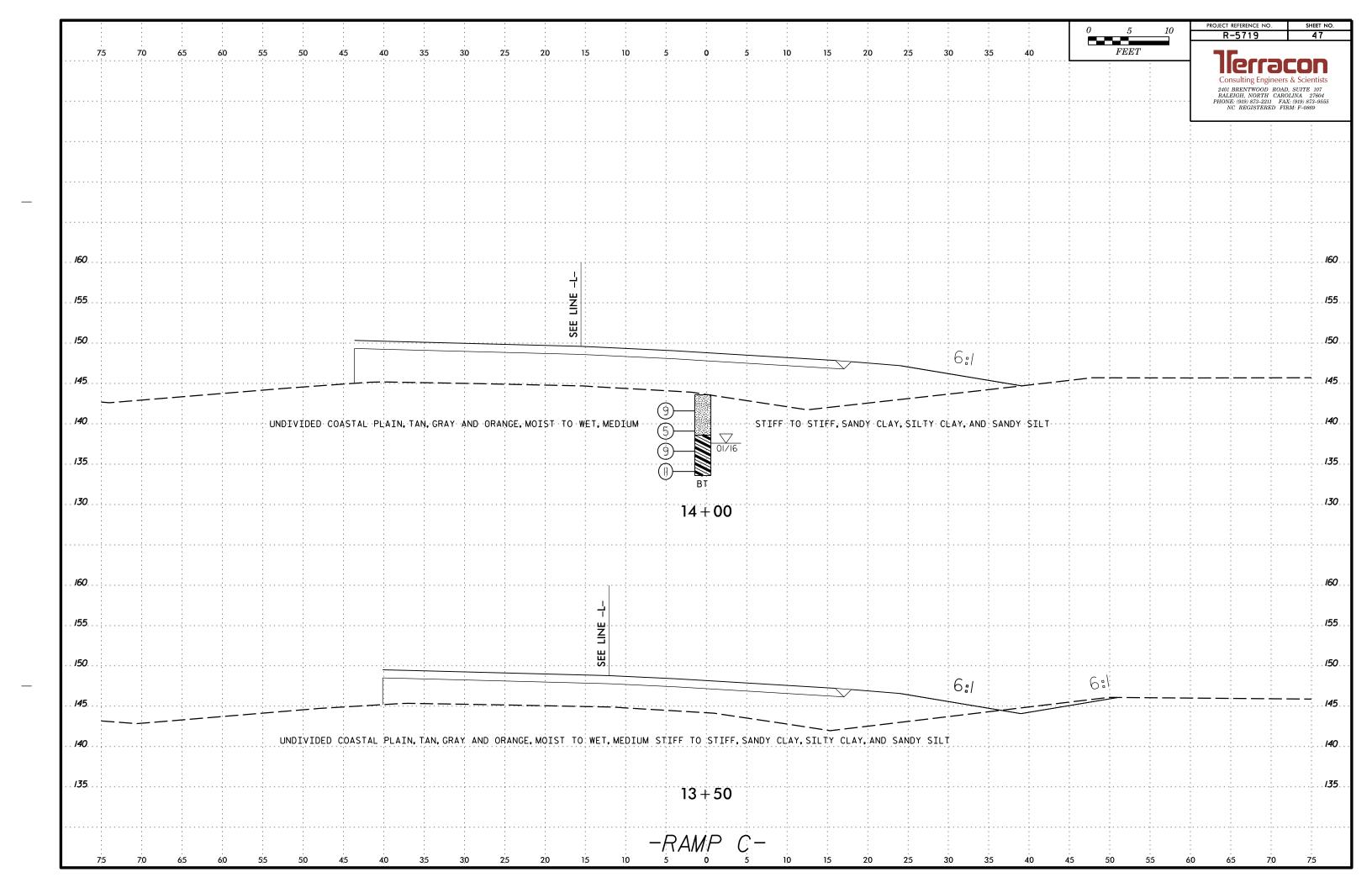


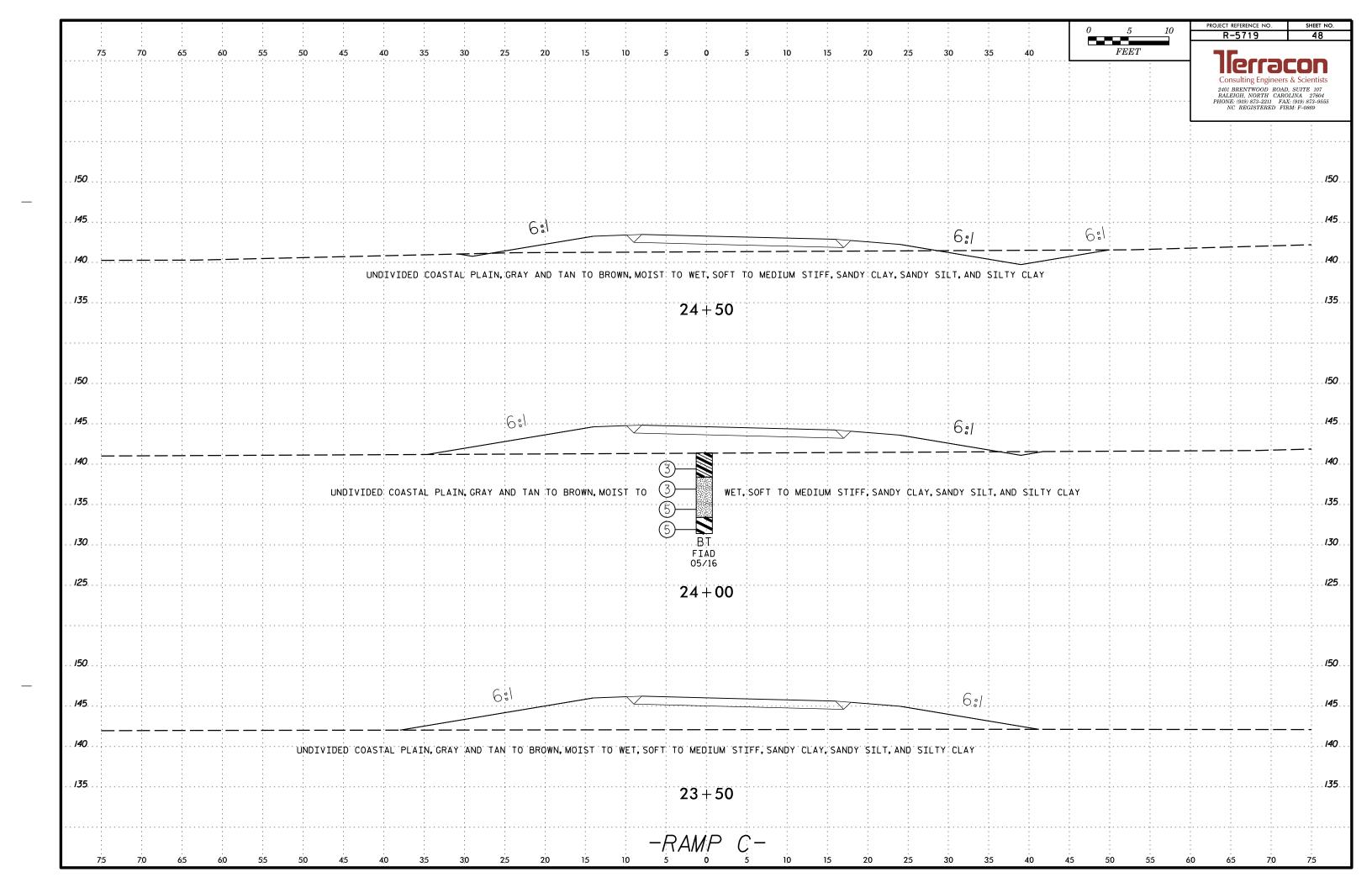


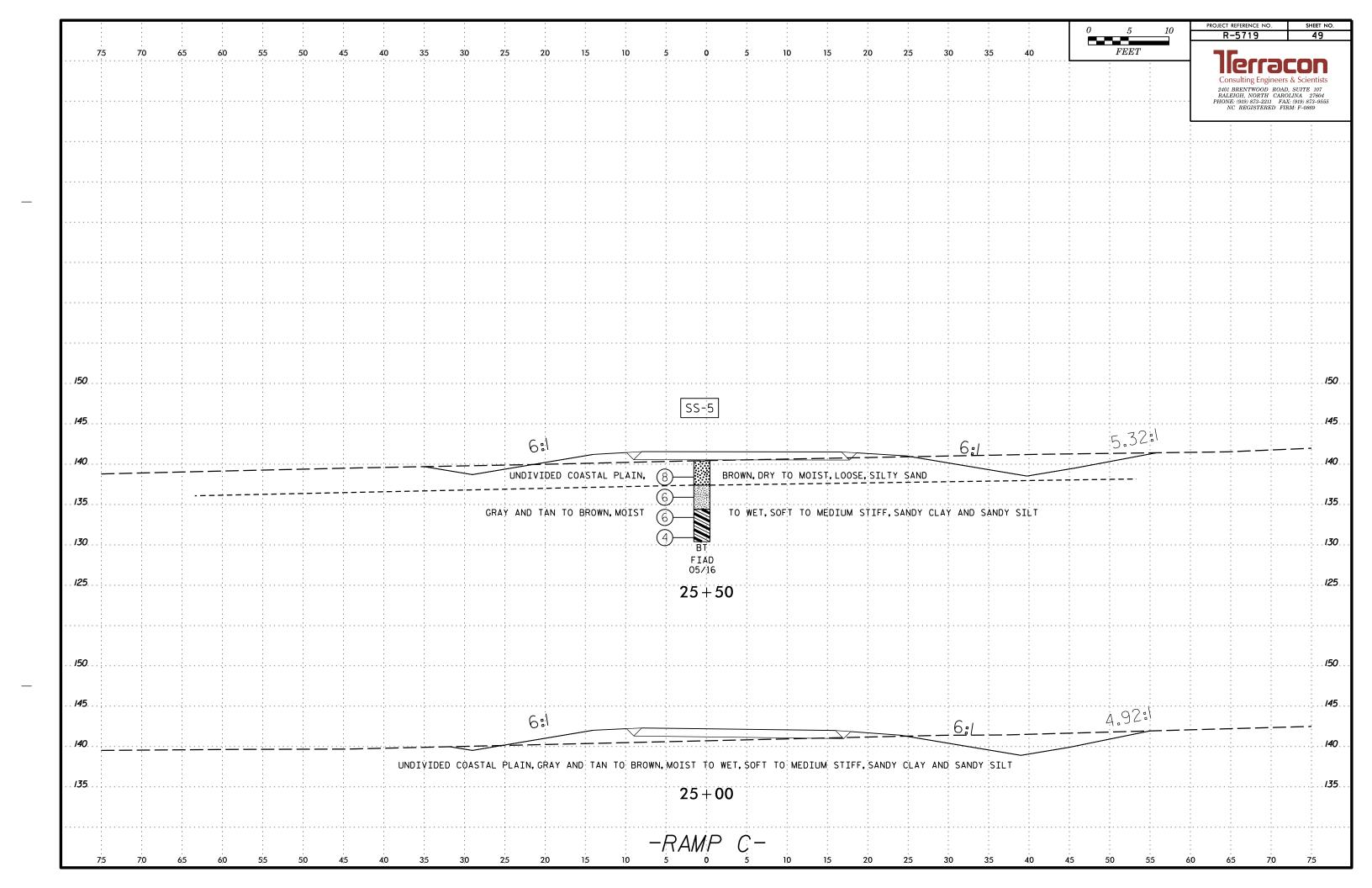


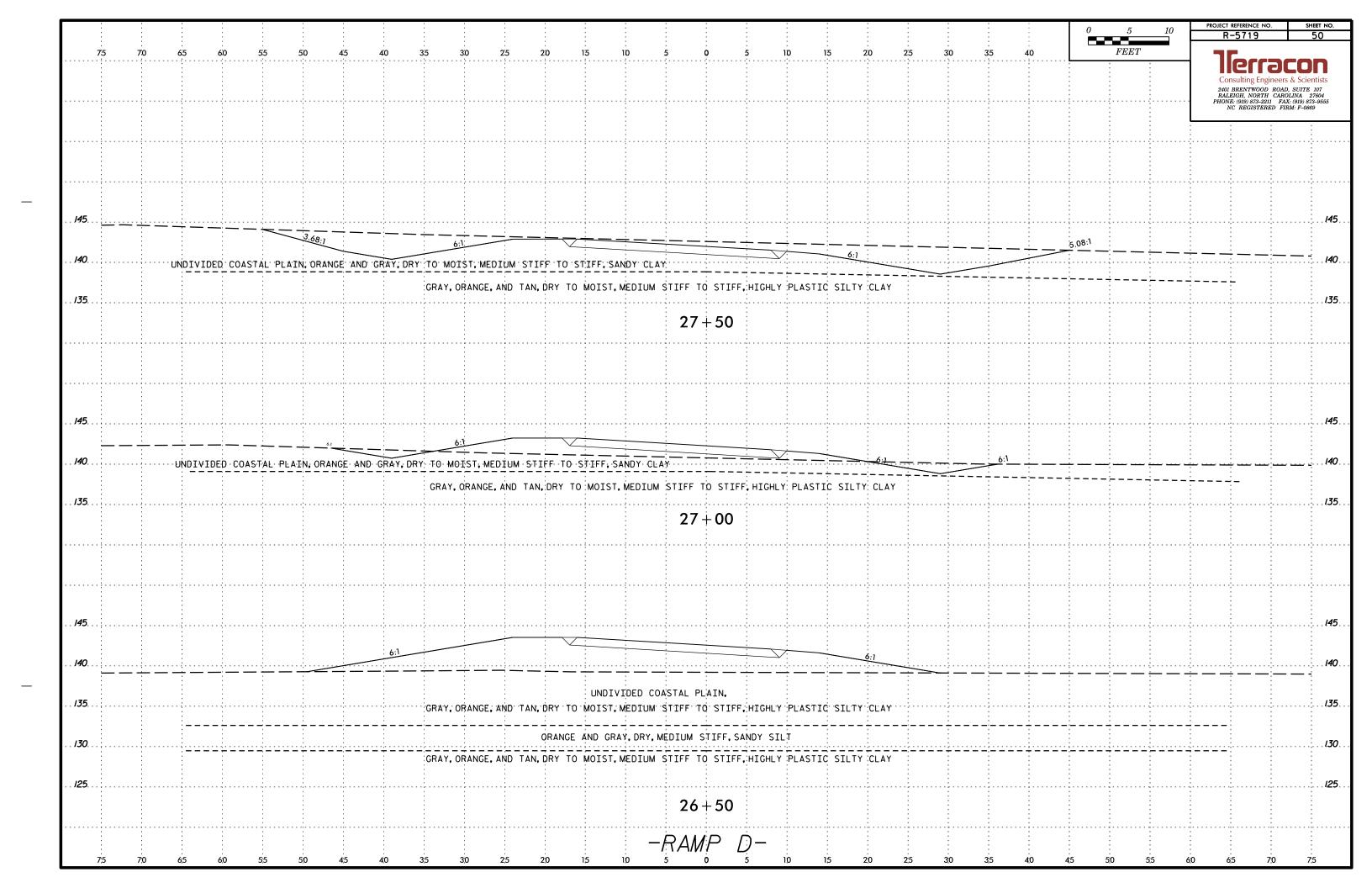


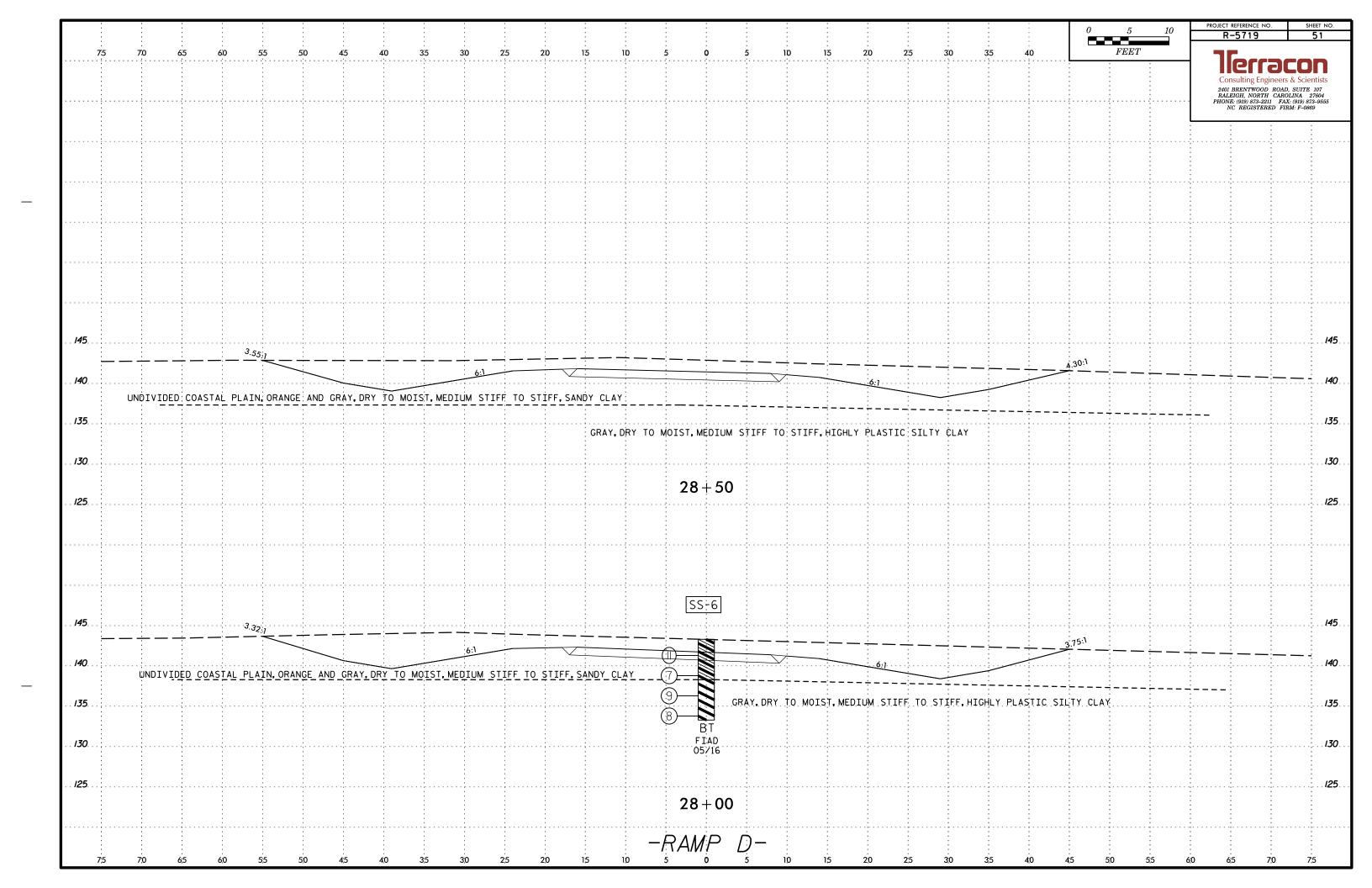


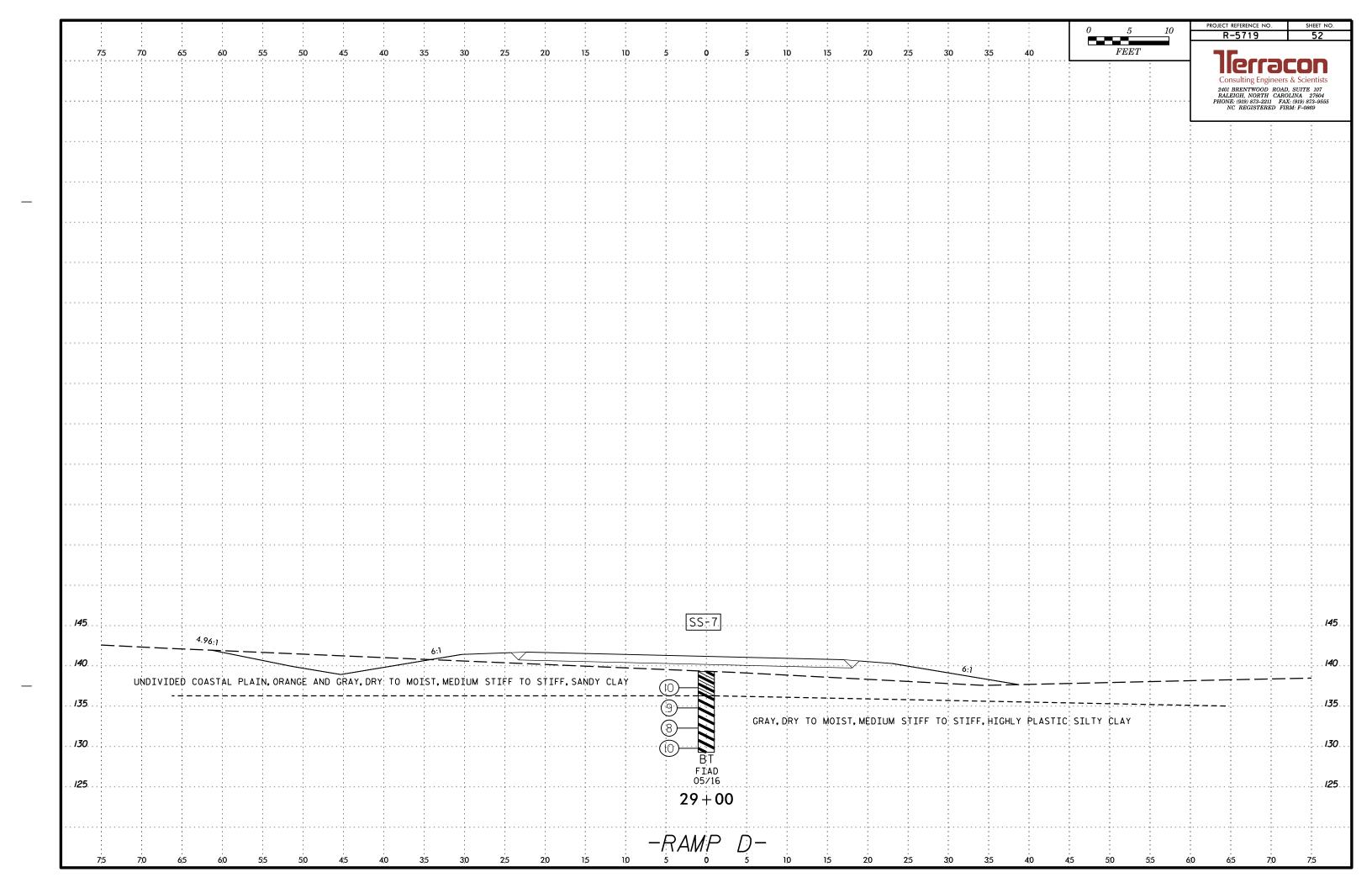


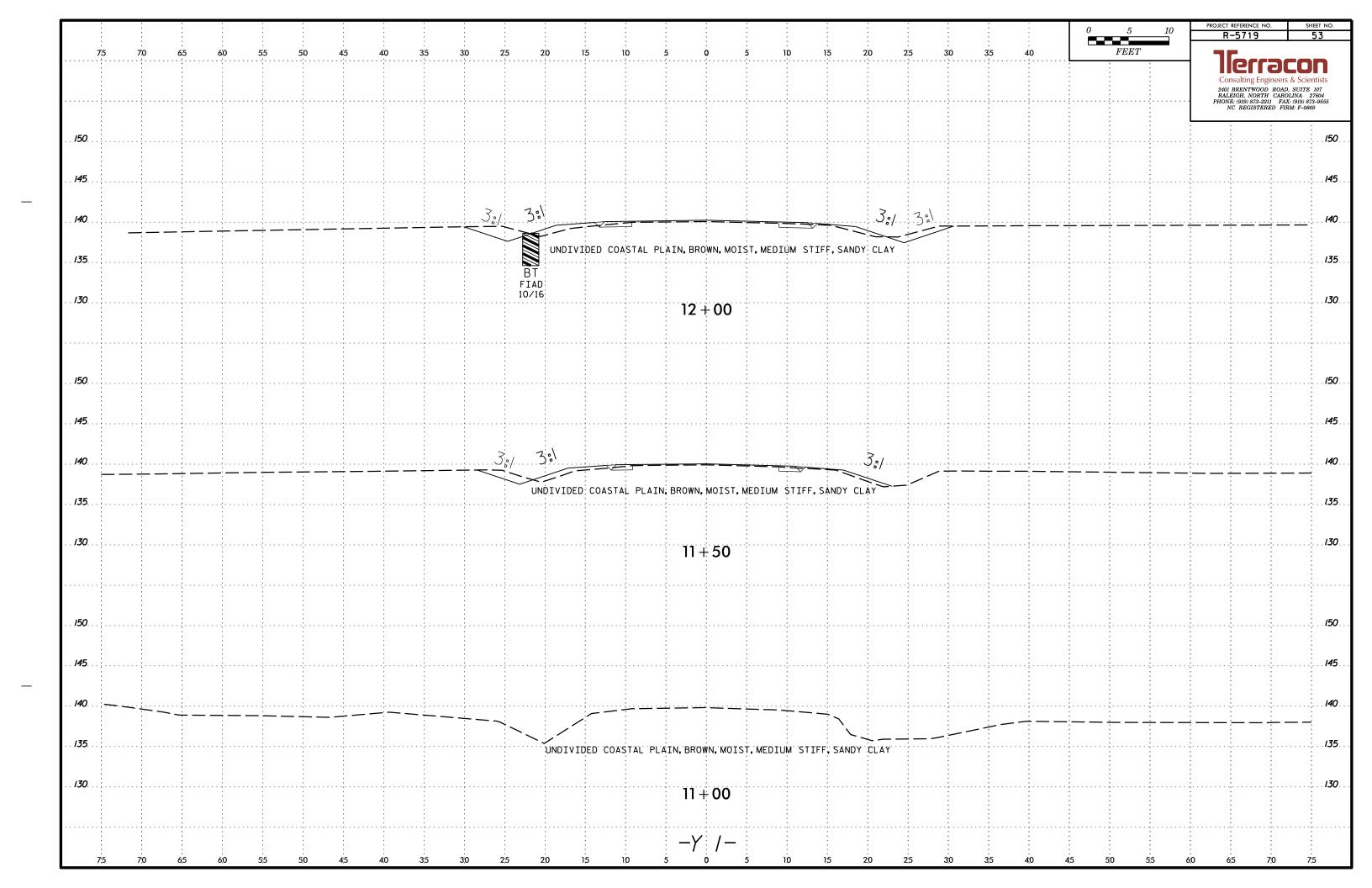


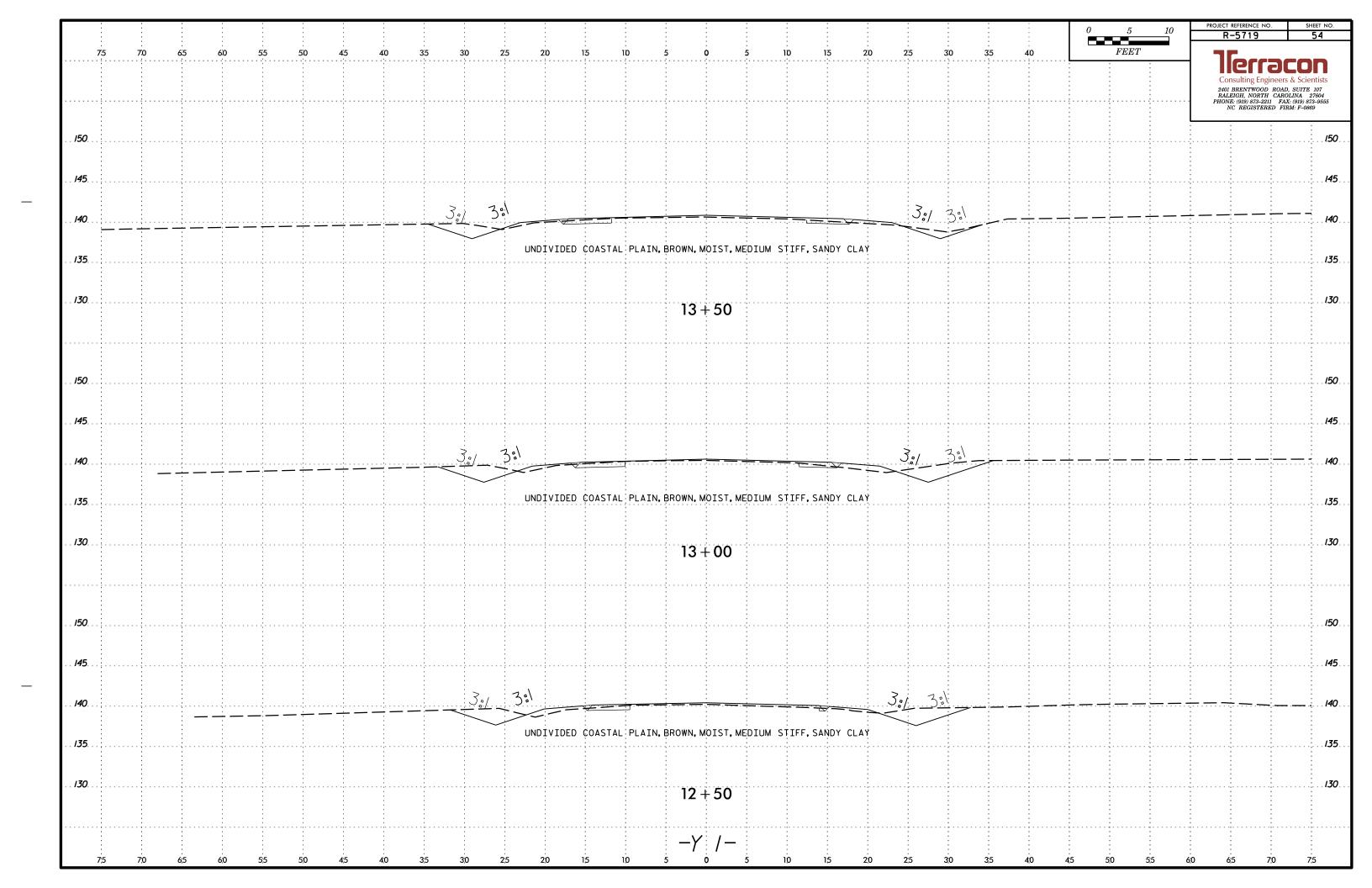


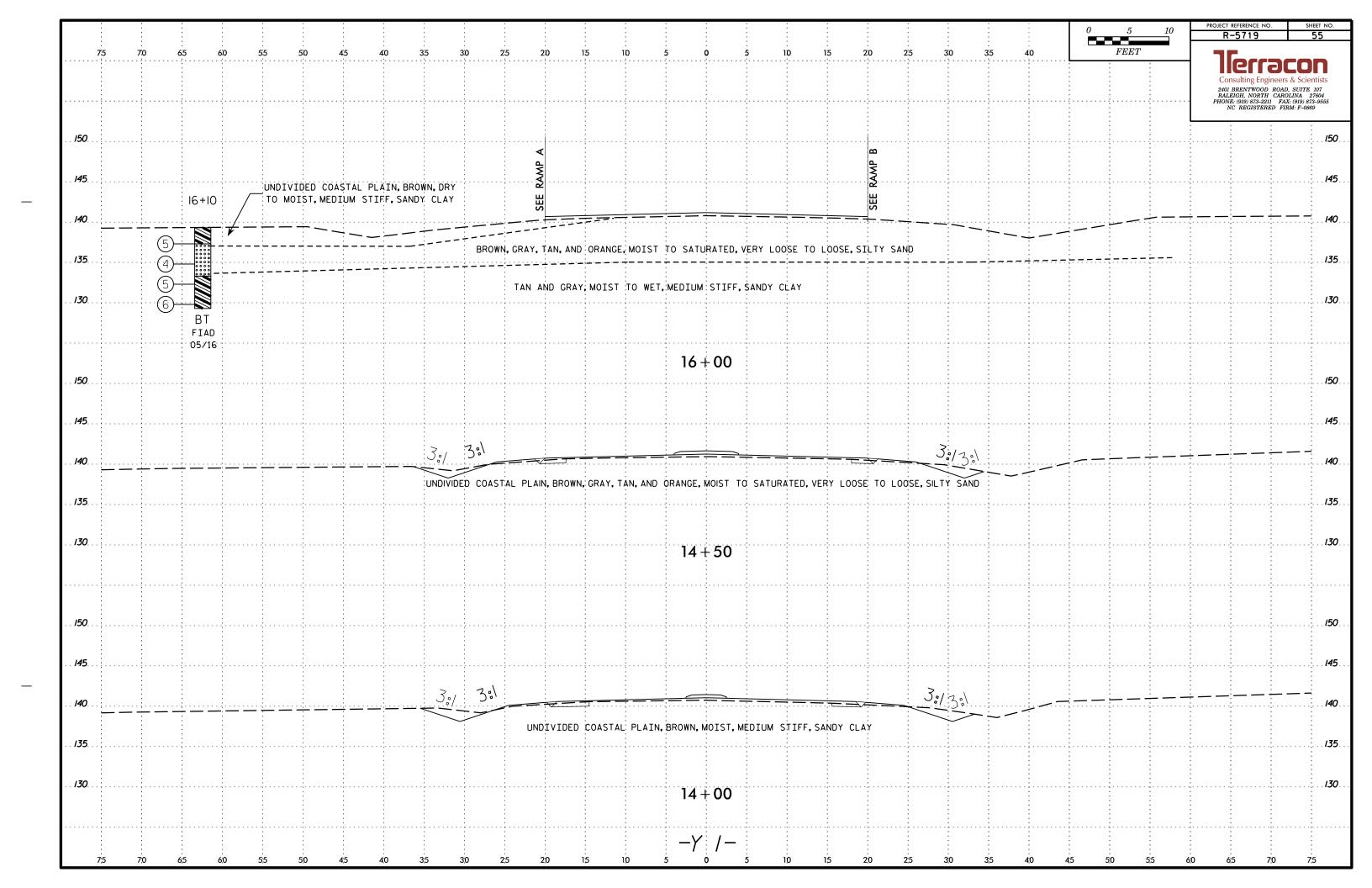


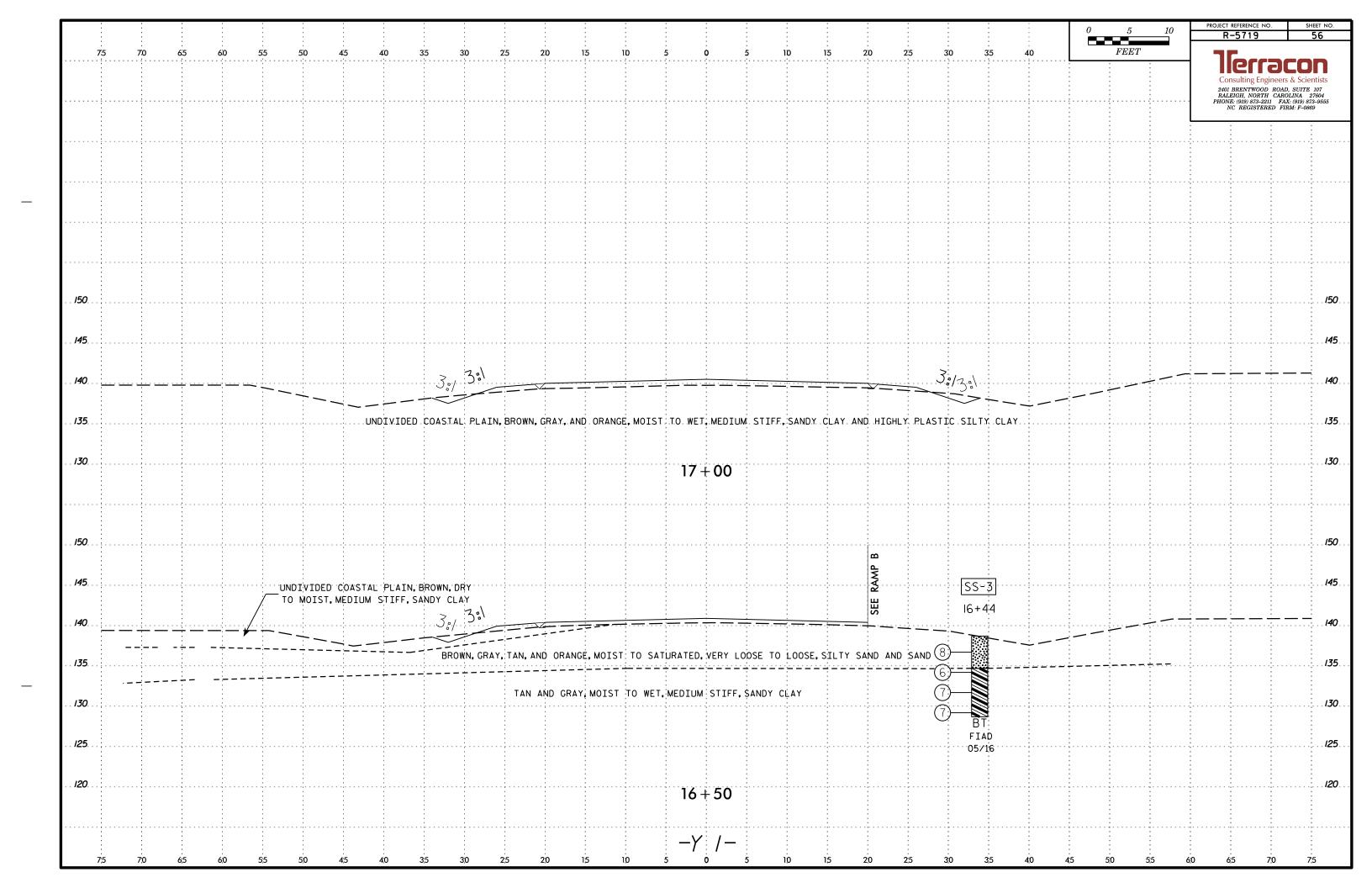


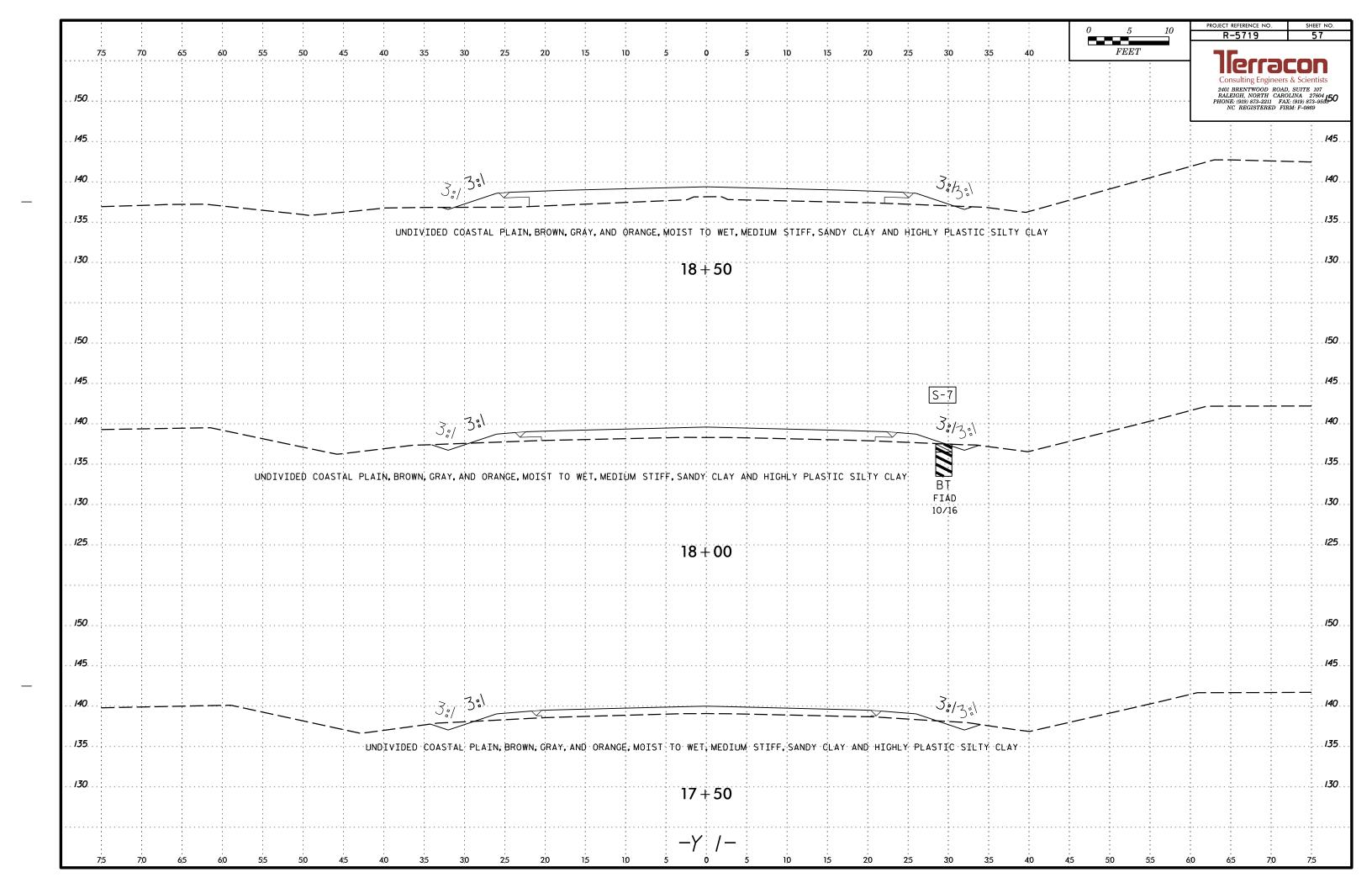


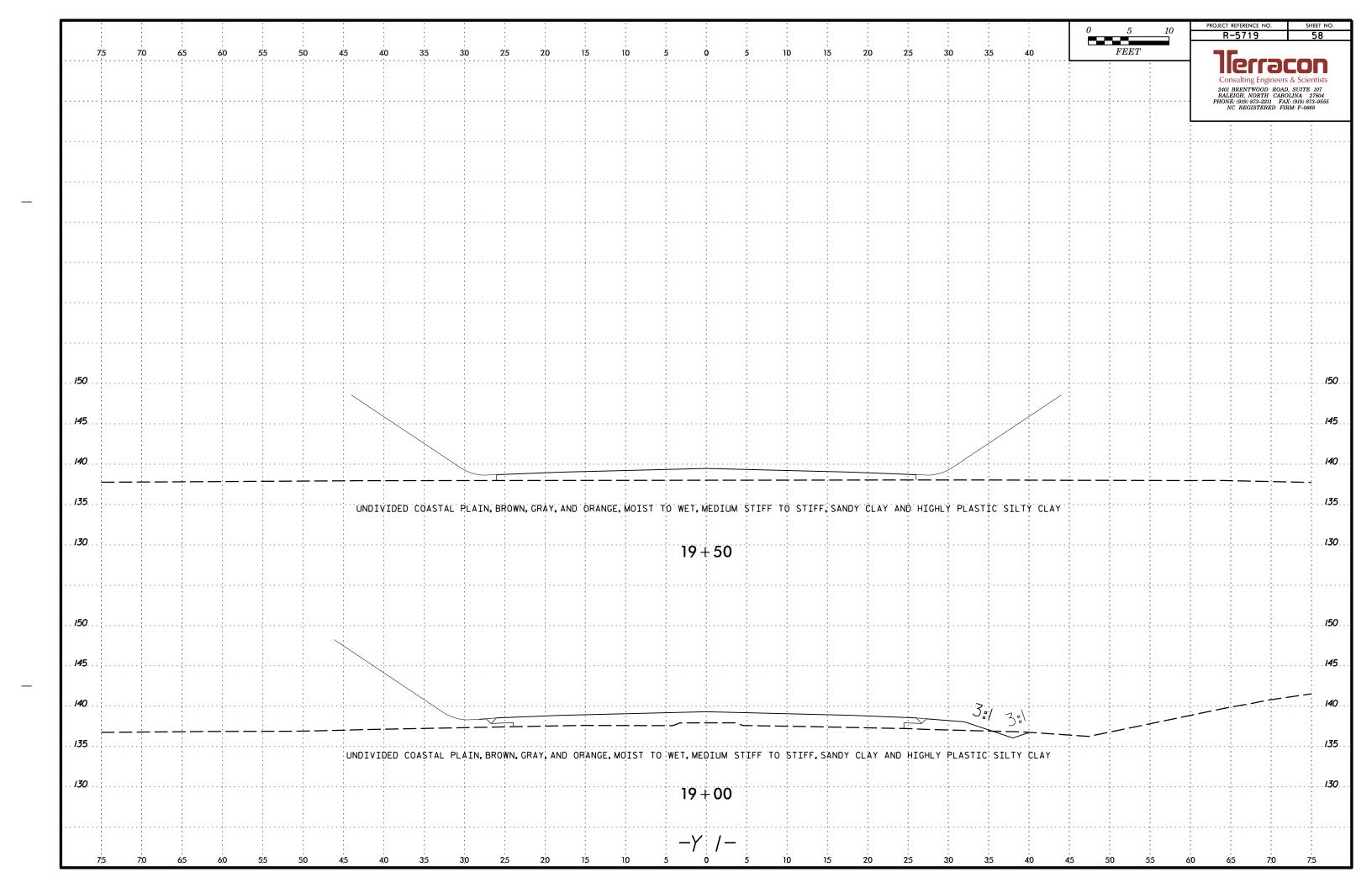


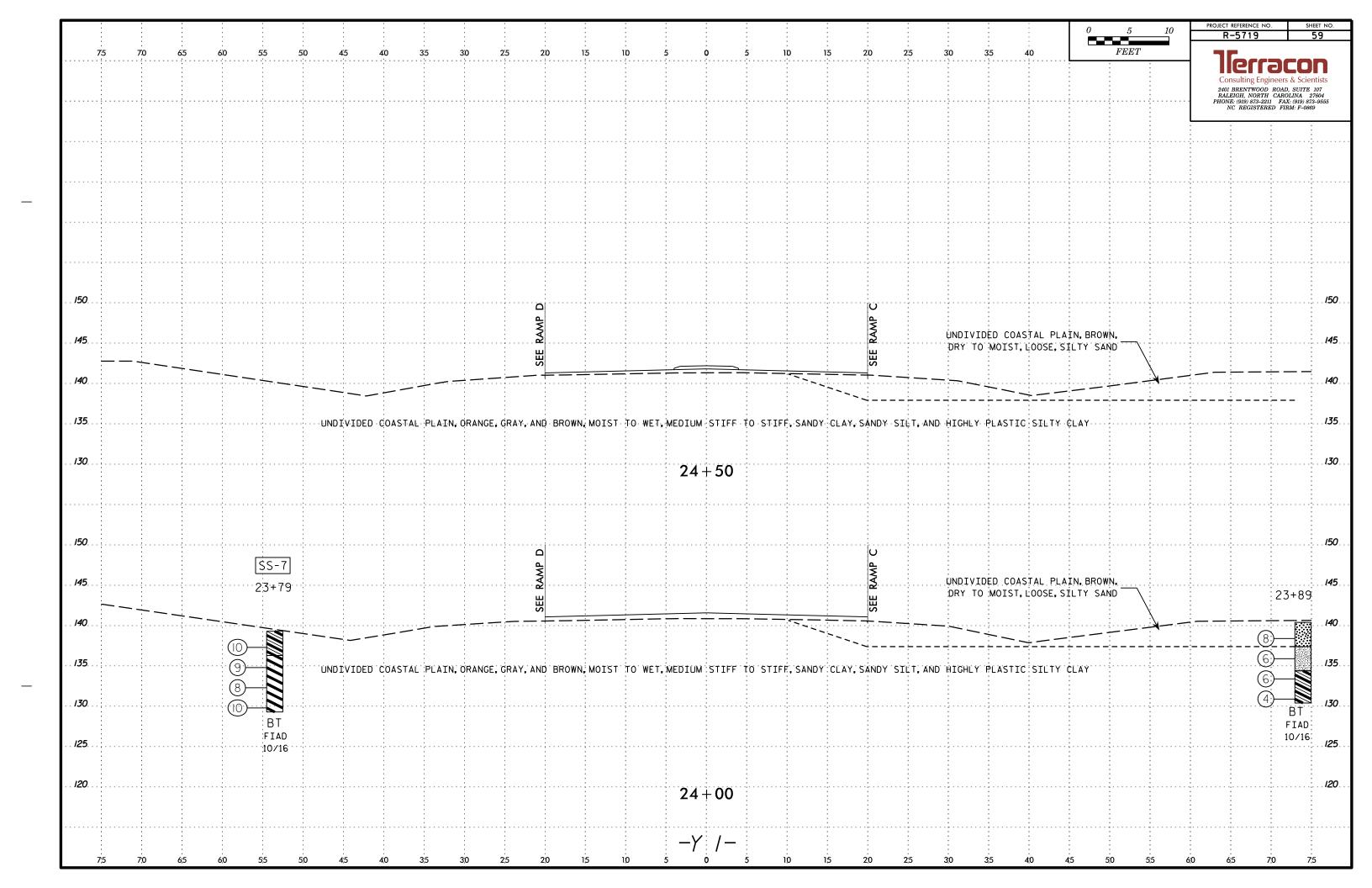


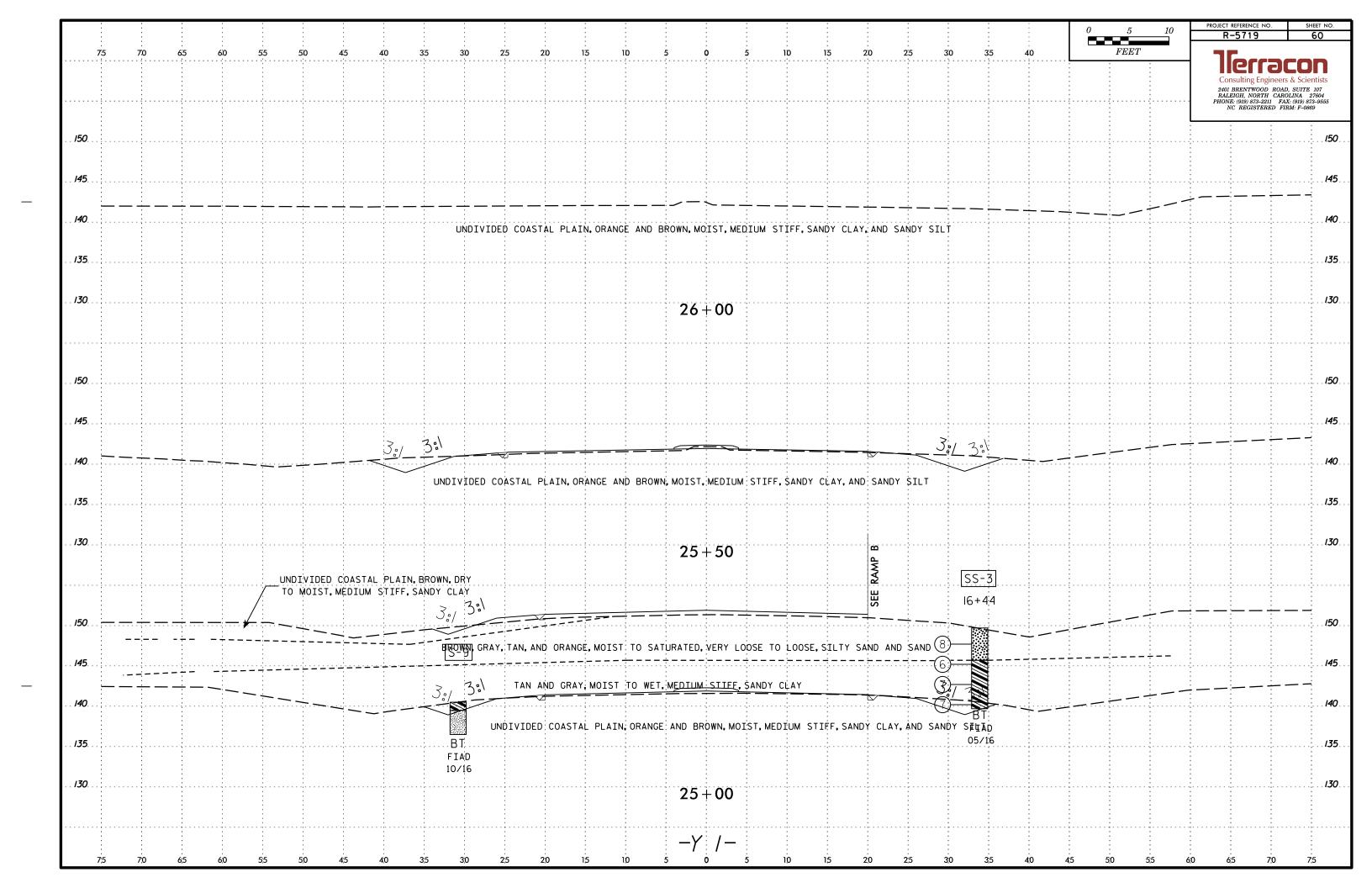












PROJECT REFERENCE NO. SHEET NO. 61

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

APPENDIX A LABORATORY RESULTS

REFERENCE: R.

PROJECT: 50473

Consulting Engineers & Scientists
2401 BRENTWOOD ROAD, SUITE 107
RALEIGH, NORTH CAROLINA 27604
PHONE: (919) 873–2211 FAX: (919) 873–9555
NC REGISTERED FIRM: F-0869

INITIALS DATE

LABORATORY TESTING SUMMARY

PROJECT NUMBER:	50473	TIP:	R-5719	COUNTY:	WAYNE

DESCRIPTION: US 117 AND SR 1135 (COUNTRY CLUB ROAD) INTERCHANGE

Sample No. Alignm		nt Station	Offset (feet)	Depth Interval (feet)	AASHTO Class.	L.L.	P.I.	% by Weight				%	% Passing (sieves)				0/
	Alignment							Coarse Sand	Fine Sand	Silt	Clay	Retained #4 Sieve	#10	#40	#200	% Moisture	% Organic
SS-1	-L-	23+50	CL	1.0 - 2.5	A-6 (8)	36	20	8.6	44.7	19.3	27.4	0	100	97	58	19.1	ND
SS-2	-L-	27+00	CL	1.0 - 2.5	A-6 (2)	26	11	9.3	50.7	16.7	23.3	0	100	97	49	12.0	ND
SS-3	-RAMP B-	29+50	27 RT	6.0 - 7.5	A-6 (11)	40	25	5.4	45.9	18.8	29.9	0	100	98	59	22.9	ND
SS-4	-RAMP C-	22+00	CL	1.0 - 2.5	A-6 (4)	26	14	8.0	50.8	15.8	25.4	0	100	97	53	18.2	ND
SS-5	-RAMP C-	25+50	CL	3.5 - 5.0	A-4 (0)	19	5	7.7	56.2	16.7	19.4	0	100	98	48	13.2	ND
SS-6	-RAMP D-	28+00	CL	3.5 - 5.0	A-6 (3)	31	15	8.7	56.3	10.9	24.1	1	98	95	44	16.1	ND
SS-7	-RAMP D-	29+00	CL	3.5 - 5.0	A-7-6 (18)	53	35	6.8	40.6	14.6	38.0	0	100	98	61	20.1	ND
OT 4		20.02	CO I T	40.5.45.0	A 4 (O)	0.5	NP	0.4	00.4	00.0	40.4		100	400	F4	24.4	ND
ST-1	-L-	38+83	68 LT	13.5 - 15.0	A-4 (0)	25		0.4	66.4	20.8	12.4	0		100	51	31.4	
ST-2	-L-	39+50	80 RT	15.0 - 17.4	A-4 (0)	24	6	21.5	52.3	12.1	14.1	0	100	91	38	19.2	ND
S-1	-L-	13+00	67 LT	0 - 4.0	A-7-6 (11)	45	31	4.3	49.9	10.9	34.9	0	100	100	50	21.1	ND
S-2	-L-	16+00	68 LT	0 - 4.0	A-6 (4)	33	17	4.9	55.6	17.1	22.4	0	100	98	48	15.4	ND
S-3	-L-	16+00	68 RT	1.5 - 4.0	A-2-4 (0)	30	10	26.7	52.8	1.6	18.9	0	97	85	22	ND	ND
S-4	-L-	22+00	70 LT	0 - 4.0	A-6 (5)	38	23	26.3	36.6	4.5	32.6	0	100	90	41	13.7	ND
S-5	-L-	63+00	65 RT	0 - 2.0	A-6 (5)	28	16	7.8	50.3	12.6	29.3	0	100	97	52	13.7	ND
S-6	-L-	63+00	65 RT	2.0 - 4.0	A-2-4 (0)	26	9	15.8	58.2	6.4	19.6	0	100	94	31	ND	ND
S-7	-Y1-	18+00	29 RT	1.0 - 3.0	A-7-6 (30)	59	43	5.1	31.0	16.4	47.5	0	100	99	72	23.0	ND
S-8	-Y1-	22+50	42 LT	3.0 - 4.0	A-6 (4)	28	16	8.6	50.6	12.7	28.1	0	100	97	50	15.2	ND
S-9	-Y1-	25+00	31 LT	1.0 - 4.0	A-4 (1)	26	9	5.8	61.4	12.4	20.4	0	100	99	45	14.1	ND
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NP - NONPLASTIC

ND - NOT DETERMINED

LABORATORY TESTING OF SHELBY TUBE SAMPLES, ST-1 AND ST-2, WAS PERFORMED BY GEOTECHNICS.

Stephanie H. Huffman

Certified Lab Technician Signature

114-01-1203

Certification Number

PROJECT REFERENCE NO. SHEET NO. 63

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

APPENDIX B CPT LOGS

REFERENCE: 50473 PROJECT:

R-

Consulting Engineers & Scientists
2401 BRENTWOOD ROAD, SUITE 107
RALEIGH, NORTH CAROLINA 27604
PHONE: (919) 873-2211 FAX: (919) 873-9555
NC REGISTERED FIRM: F-0869

INITIALS DATE

