

CONTRACT: ID: W-5313

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	46136.1.1 (W-5313)	1	132
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
46136.1.1	STP-1221(15)	P.E.	
		RW & UTIL.	

CONTENTS

LINE	STATION	PLAN	PROFILE	XSECT
-L-	10+25 TO 295+35.81	4-25	N/A	35-92
-L-	318+35.84 TO 420+00	26-34	N/A	93-120
-Y1-	10+00 TO 14+20	4	N/A	N/A
-Y2-	10+75 TO 12+60.70	6	N/A	N/A
-Y3-	10+50 TO 12+24.08	14	N/A	N/A
-Y4-	10+00 TO 13+75	14	N/A	121-124
-Y5-	11+90 TO 13+33.49	20	N/A	125
-Y6-	10+00 TO 15+15	20	N/A	126-129
-Y7-	10+00 TO 11+40	33	N/A	N/A
-Y8-	11+25 TO 12+84.99	34	N/A	N/A

ROADWAY
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 46136.1.1 (W-5313) F.A. PROJ. STP-1221(15)
 COUNTY ROWAN
 PROJECT DESCRIPTION SR 1221 (OLD BEATTY FORD ROAD) FROM
SR 1337 (LENTZ ROAD) TO SR 2335 (LOWER STONE
CHURCH ROAD)

INVENTORY

CAUTION NOTICE

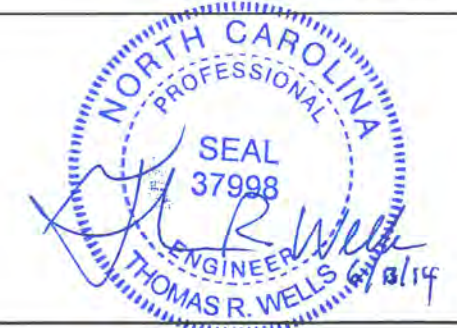
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6850. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE, THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PERSONNEL
R. TOOTHMAN
G. LOWDERMILK

INVESTIGATED BY D. GOODNIGHT
 CHECKED BY T. WELLS
 SUBMITTED BY KLEINFELDER
 DATE JUNE 2014



DRAWN BY: W. FELDER

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

PROJECT REFERENCE NO. 46136.1 (W-5313)	SHEET NO. 2 OF 132
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SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS	
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>		WELL-GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.		HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) CRYSTALLINE ROCK (CR) NON-CRYSTALLINE ROCK (NCR) COASTAL PLAIN SEDIMENTARY ROCK (CP)		ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOTJ.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROQ) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROQ) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.	
SOIL LEGEND AND AASHTO CLASSIFICATION							
GENERAL CLASS.		GRANULAR MATERIALS (≤ 35% PASSING #200)		SILT-CLAY MATERIALS (> 35% PASSING #200)		ORGANIC MATERIALS	
GROUP CLASS.	A-1, A-2, A-3	A-1, A-2, A-3	A-2, A-4, A-5, A-6, A-7	A-4, A-5, A-6, A-7	A-1, A-2, A-3	A-4, A-5, A-6, A-7	
SYMBOL	[Symbol]		[Symbol]		[Symbol]		
% PASSING	10 40 200	10 40 200	10 40 200	10 40 200	10 40 200	10 40 200	
LIQUID LIMIT PLASTIC INDEX	6 MX		NP		40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN		
GROUP INDEX	0		0		4 MX 8 MX 12 MX 16 MX No MX		
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS, GRAVEL, AND SAND		FINE SAND		SILTY OR CLAYEY GRAVEL AND SAND		
GEN. RATING AS A SUBGRADE	EXCELLENT TO GOOD		FAIR TO POOR		FAIR TO POOR		POOR UNSUITABLE
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS ≥ LL - 30							
CONSISTENCY OR DENSENESS							
PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)				
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	<4 4 TO 10 10 TO 30 30 TO 50 >50	N/A				
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	<2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 >30	<0.25 0.25 TO 0.50 0.5 TO 1.0 1 TO 2 2 TO 4 >4				
TEXTURE OR GRAIN SIZE							
U.S. STD. SIEVE SIZE OPENING (MM)	4	10	40	60	200	270	
	4.76	2.00	0.42	0.25	0.075	0.053	
BOULDER (BLDR.)	COBBLE (COB.)	GRAVEL (GR.)	COARSE SAND (CSE, SD.)	FINE SAND (F SD.)	SILT (SL.)	CLAY (CL.)	
GRAIN SIZE MM	305	75	2.0	0.25	0.05	0.005	
GRAIN SIZE IN.	12	3					
SOIL MOISTURE - CORRELATION OF TERMS							
SOIL MOISTURE SCALE (ATTERBERG LIMITS)		FIELD MOISTURE DESCRIPTION		GUIDE FOR FIELD MOISTURE DESCRIPTION			
PLASTIC RANGE (PI)	LL	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE				
	PL						
	OM	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE				
SL	OPTIMUM MOISTURE	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE				
	SHRINKAGE LIMIT	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE				
PLASTICITY							
NONPLASTIC		PLASTICITY INDEX (PI)		DRY STRENGTH			
LOW PLASTICITY		0-5		VERY LOW			
MED. PLASTICITY		6-15		SLIGHT			
HIGH PLASTICITY		16-25		MEDIUM			
		26 OR MORE		HIGH			
COLOR							
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.							
MINERALOGICAL COMPOSITION							
MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.							
COMPRESSION							
SLIGHTLY COMPRESSIBLE				LIQUID LIMIT LESS THAN 31			
MODERATELY COMPRESSIBLE				LIQUID LIMIT EQUAL TO 31-50			
HIGHLY COMPRESSIBLE				LIQUID LIMIT GREATER THAN 50			
PERCENTAGE OF MATERIAL							
ORGANIC MATERIAL		GRANULAR SOILS		SILT-CLAY SOILS		OTHER MATERIAL	
TRACE OF ORGANIC MATTER		2 - 3%		3 - 5%		TRACE 1 - 10%	
LITTLE ORGANIC MATTER		3 - 5%		5 - 12%		LITTLE 10 - 20%	
MODERATELY ORGANIC		5 - 10%		12 - 20%		SOME 20 - 35%	
HIGHLY ORGANIC		>10%		>20%		HIGHLY 35% AND ABOVE	
GROUND WATER							
WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING							
STATIC WATER LEVEL AFTER 24 HOURS							
PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA							
SPRING OR SEEP							
MISCELLANEOUS SYMBOLS							
ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION		TEST BORING		TEST BORING W/ CORE			
SOIL SYMBOL		AUGER BORING		SPT N-VALUE			
ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT		CORE BORING		SPT REFUSAL			
INFERRED SOIL BOUNDARY		MONITORING WELL					
INFERRED ROCK LINE		PIEZOMETER INSTALLATION					
ALLUVIAL SOIL BOUNDARY		SLOPE INDICATOR INSTALLATION					
DIP & DIP DIRECTION OF ROCK STRUCTURES		CONE PENETROMETER TEST					
		SOUNDING ROD					
ABBREVIATIONS							
AR - AUGER REFUSAL		MED. - MEDIUM		VST - VANE SHEAR TEST			
BT - BORING TERMINATED		MICA - MICACEOUS		WEA. - WEATHERED			
CL. - CLAY		MOD. - MODERATELY		UNIT WEIGHT			
CPT - CONE PENETRATION TEST		NP - NON PLASTIC		γ _d - DRY UNIT WEIGHT			
CSE. - COARSE		ORG. - ORGANIC		SAMPLE ABBREVIATIONS			
DMT - DILATOMETER TEST		PMT - PRESSUREMETER TEST		S - BULK			
DPT - DYNAMIC PENETRATION TEST		SAP. - SAPROLITIC		SS - SPLIT SPOON			
e - VOID RATIO		SD. - SAND, SANDY		ST - SHELBY TUBE			
F - FINE		SL. - SILT, SILTY		RS - ROCK			
FOSS. - FOSSILIFEROUS		SLI. - SLIGHTLY		RT - RECOMPACTED TRIAXIAL			
FRAC. - FRACTURED, FRACTURES		TCR - TRICONE REFUSAL		CBR - CALIFORNIA BEARING RATIO			
FRAGS. - FRAGMENTS		w - MOISTURE CONTENT					
HI. - HIGHLY		V - VERY					
EQUIPMENT USED ON SUBJECT PROJECT							
DRILL UNITS:		ADVANCING TOOLS:		HAMMER TYPE:			
<input type="checkbox"/> MOBILE B- _____		<input type="checkbox"/> CLAY BITS		<input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL			
<input type="checkbox"/> BK-51		<input type="checkbox"/> 6" CONTINUOUS FLIGHT AUGER		CORE SIZE:			
<input type="checkbox"/> CME-45C		<input checked="" type="checkbox"/> 8" HOLLOW AUGERS		<input type="checkbox"/> -B _____			
<input type="checkbox"/> CME-550		<input type="checkbox"/> HARD FACED FINGER BITS		<input type="checkbox"/> -N _____			
<input type="checkbox"/> PORTABLE HOIST		<input type="checkbox"/> TUNG.-CARBIDE INSERTS		<input type="checkbox"/> -H _____			
<input checked="" type="checkbox"/> CME-55		<input type="checkbox"/> CASING <input type="checkbox"/> W/ ADVANCER		HAND TOOLS:			
		<input type="checkbox"/> TRICONE _____ * STEEL TEETH		<input type="checkbox"/> POST HOLE DIGGER			
		<input type="checkbox"/> TRICONE _____ * TUNG.-CARB.		<input type="checkbox"/> HAND AUGER			
		<input type="checkbox"/> CORE BIT		<input type="checkbox"/> SOUNDING ROD			
				<input type="checkbox"/> VANE SHEAR TEST			
FRACTURE SPACING				BEDDING			
TERM		SPACING		TERM		THICKNESS	
VERY WIDE		MORE THAN 10 FEET		VERY THICKLY BEDDED		> 4 FEET	
WIDE		3 TO 10 FEET		THICKLY BEDDED		1.5 - 4 FEET	
MODERATELY CLOSE		1 TO 3 FEET		THINLY BEDDED		0.16 - 1.5 FEET	
CLOSE		0.16 TO 1 FEET		VERY THINLY BEDDED		0.03 - 0.16 FEET	
VERY CLOSE		LESS THAN 0.16 FEET		THICKLY LAMINATED		0.008 - 0.03 FEET	
				THINLY LAMINATED		< 0.008 FEET	
INDURATION							
FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.							
FRIABLE				RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.			
MODERATELY INDURATED				GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.			
INDURATED				GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.			
EXTREMELY INDURATED				SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.			
BENCH MARK: BORING ELEVATIONS OBTAINED USING W5313_LS.TIN.TIN DATED 12/19/13 ELEVATION: N/A FT.							
NOTES: FIAD - FILLED IN AFTER DRILLING							

09/08/09

TIP PROJECT: W-5313

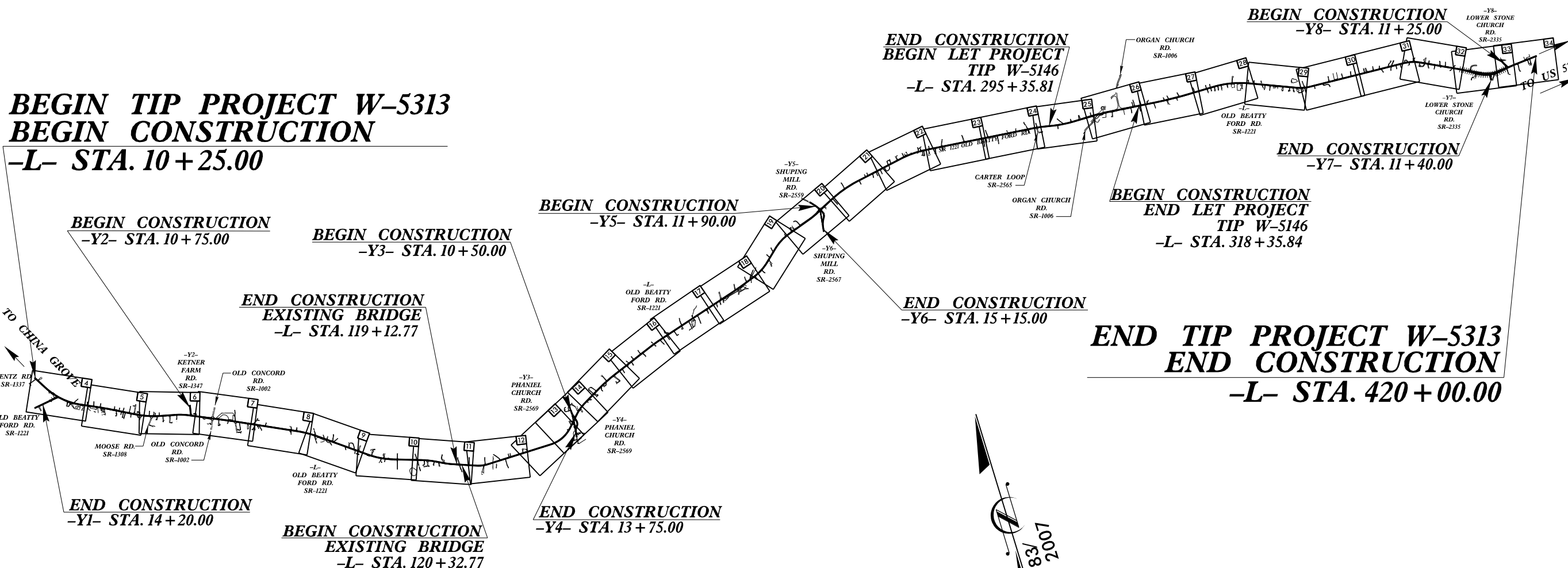
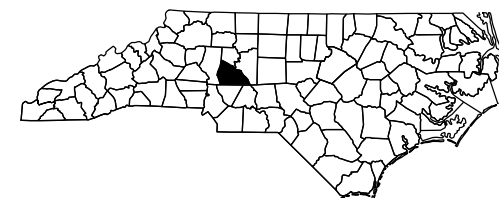
CONTRACT: 46136.1.1

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

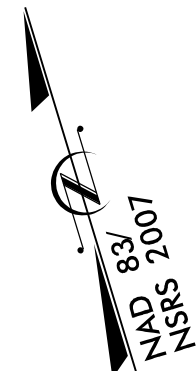
ROWAN COUNTY

**LOCATION: SR 1221 (OLD BEATTY FORD ROAD)
FROM SR 1337 (LENTZ ROAD) TO
SR 2335 (LOWER STONE CHURCH ROAD)**
TYPE OF WORK: WIDENING, RESURFACING, DRAINAGE, SIGNING

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	W-5313	2A	132
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
46136.1.1	STP-1221(15)	P.E.	



THIS PROJECT IS NOT WITHIN ANY MUNICIPAL BOUNDARIES.
THERE IS NO CONTROL OF ACCESS ON THIS PROJECT.
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD



INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

<p>GRAPHIC SCALES</p> <p>50 25 0 50 100 PLANS</p> <p>50 25 0 50 100 PROFILE (HORIZONTAL)</p> <p>10 5 0 10 20 PROFILE (VERTICAL)</p>	<p>DESIGN DATA</p> <p>ADT 2015 = 4334 ADT 2035 = 5667 DHV = 10 % D = 65 % T = 8 % * V = 50 MPH * TTST = 3 DUAL = 5 FUNC CLASS = RUAL COLLECTOR SUB REGIONAL TIER</p>	<p>PROJECT LENGTH</p> <p>LENGTH OF ROADWAY TIP PROJECT W-5313 = 7.30 MILES LENGTH OF EXISTING STRUCTURE TIP PROJECT W-5313 = 0.02 MILES LENGTH OF LET TIP PROJECT W-5146 = 0.44 MILES TOTAL LENGTH OF TIP PROJECT W-5313 = 7.76 MILES</p>	<p>Prepared in the Office of: DIVISION OF HIGHWAYS 1000 Birch Ridge Dr., Raleigh NC, 27610</p>	<p>HYDRAULICS ENGINEER</p> <p>SIGNATURE: _____ P.E.</p>	
			<p>2012 STANDARD SPECIFICATIONS</p> <p>RIGHT OF WAY DATE: MARCH 21, 2014 LETTING DATE: SEPTEMBER 15, 2015</p>	<p>TONY HOUSER, PE. PROJECT ENGINEER</p> <p>LEE ANN MOORE PROJECT DESIGN ENGINEER</p> <p>SIGNATURE: _____ P.E.</p>	



June 13, 2104
File No. 138804 | GSO14R0146

STATE PROJECT: 46136.1.1 (W-5313)
FEDERAL PROJECT: STP-1221(15)
COUNTY: Rowan
DESCRIPTION: SR 1221 (Old Beatty Ford Road) from SR 1337 (Lentz Road) to SR 2335 (Lower Stone Church Road)

SUBJECT: Geotechnical Report - Inventory

Project Description

This project consists of the proposed 7.4 mile widening of Old Beatty Ford Road (-L-) from Lentz Road to Lower Stone Church Road. This project includes of the reconstruction of portions of the intersections with Lentz Road (-Y1-), Ketner Farm Road (-Y2-), Phaniel Church Road (-Y3- and -Y4-), Shuping Mill Road (-Y5- and -Y6-), and Lower Stone Church Road (-Y7- and -Y8-).

The geotechnical investigation was conducted during January of 2014. One drill machine, a CME-55, with automatic hammers, was used during the investigation. Standard Penetration Tests were performed at each boring location. Representative soil samples were collected for visual classification in the field and selected samples were submitted for laboratory analysis by Kleinfelder, Inc.

The following alignments, totaling 7.8 miles, were investigated.

<u>Line</u>	<u>Stations</u>
-L-	10+25 to 295+35.81
-L-	318+35.84 to 420+00
-Y1-	10+00 to 14+20
-Y2-	10+75 to 12+60.70
-Y3-	10+50 to 12+24.08
-Y4-	10+00 to 13+75
-Y5-	11+90 to 13+33.49
-Y6-	10+00 to 15+15
-Y7-	10+00 to 11+40
-Y8-	11+25 to 12+84.99

Areas of Special Geotechnical Interest

1) Highly Plastic Clays: Highly plastic clays (PI > 25) were encountered on the project at the following locations:

<u>Line</u>	<u>Stations</u>	<u>Offsets</u>
-L-	27+30 to 32+70	LT to RT
-L-	37+30 to 47+70	LT to RT
-L-	92+60 to 97+60	LT to RT
-L-	162+30 to 167+70	LT to RT
-L-	179+20 to 180+20	LT to RT
-L-	208+50 to 209+30	LT to RT
-L-	257+30 to 277+60	LT to RT
-L-	282+30 to 287+60	LT to RT
-L-	318+36 to 337+70	LT to RT
-L-	350+60 to 351+60	LT to RT
-L-	367+60 to 377+60	LT to RT
-L-	392+00 to 402+60	LT to RT
-Y4-	11+80 to 13+75	LT to RT
-Y6-	10+00 to 15+15	LT to RT

A discussion of these highly plastic clay soils is located below in the section titled "Soil Properties".

2) Artificial Fill: Several areas of artificial fill occur at the following locations.

<u>Line</u>	<u>Station</u>	<u>Offset (ft)</u>
-L-	84+50	26 RT to 38 RT
-L-	154+79	19 RT to 31 RT
-L-	189+86	16 RT to 28 RT

3) Alluvial: The following locations were found to have very soft to soft alluvial soils.

<u>Line</u>	<u>Station</u>	<u>Offset (ft)</u>
-L-	179+20 to 180+20	LT to RT
-L-	208+60 to 209+70	LT to RT
-L-	397+20 to 398+60	LT to RT

4) Ponds: One pond is located within close proximity of the right of way on this project. This was noted at the following location:

<u>Line</u>	<u>Station</u>	<u>Offset (ft)</u>
-L-	190+27 to 191+55	30 RT to 185 RT

5) Rock Outcrops: Several rock outcrops were observed along the existing roadway at the following loactions:

<u>Line</u>	<u>Stations</u>	<u>Offsets</u>
-L-	227+50 to 233+00	LT
-L-	228+30 to 229+00	RT
-Y5-	10+00 to 13+00	RT

Physiography and Geology

The project is located in the Piedmont Physiographic Province. The project corridor is comprised primarily of residential and agricultural properties. The general topography of the site consists of rolling hills with flat to moderate slopes along the existing roadways.

Geologically, the project is located within the Charlotte Belt. Soils are derived from the underlying metamorphic bedrock primarily consisting of metavolcanic rock with areas of intrusive granite in the eastern portion of the project.

Soil Properties

Soils encountered during this investigation are separated into four categories based on origin. They consist of roadway embankment, artificial fill, alluvial, and residual soils.

Roadway Embankment soils are present along the existing roadways (-L-) on the project. These soils consist of moist, medium stiff to very stiff, medium plasticity, brown, tan-red, and brown-green, coarse to fine sandy clay (A-6) and moist, medium stiff, high plasticity, brown to brown-green, coarse to fine sandy, silty clay (A-7-5). The plasticity index of the roadway embankment soils tested ranged from 29 to 32.

Artificial Fill soils are present in several small isolated areas throughout the project adjacent to the existing roadway (-L-). The artificial soils encountered consist of moist, stiff, non-plastic, tan, fine sandy silt (A-4), moist, non-plastic, tan, fine to coarse sand with gravel (A-1-b), and moist, stiff, medium plasticity, gray and brown, silty, coarse to fine sandy clay (A-6). The plasticity index of the artificial fill soils tested was 16.

Alluvial soils are present along several streams that cross the existing roadway (-L-). The alluvial soils encountered consist of wet, very soft to stiff, non-plastic, coarse to fine sandy silt (A-4), wet, soft to stiff, high plasticity, coarse to fine sandy, silty clay (A-7-5), wet, soft, low plasticity, fine sandy, clayey silt (A-5), and wet, very loose to loose, non-plastic, tan, silty, coarse to fine sand (A-2-4). Alluvial soils are also present in a pond near the project right of way and are further discussed in the "Ponds" section of this report. The plasticity index of the alluvial soils tested was 54.

Residual soils are derived from the weathering of underlying metavolcanic rock. These majority of the residual soils encountered consist of moist to wet, medium stiff to hard, non-plastic, fine sandy silt (A-4), moist, stiff to very stiff, low plasticity, coarse to fine sandy, clayey silt (A-5), moist, stiff to hard, medium plasticity, silty, fine sandy clay (A-6), and moist to wet, medium stiff to very stiff, medium to high plasticity, coarse to fine sandy, silty clay (A-7-5 and A-7-6). Minor amounts of moist to wet, loose to very dense, non-plastic to low plasticity, clayey, silty, coarse to fine sand (A-2-4 and A-2-5) are also present. The plasticity index of the residual soils tested ranged from 17 to 68.

Rock Properties

Weathered rock was encountered along the existing roadways (-L-) at elevations ranging from 688.5 to 796.0 feet (MSL). The majority of the weathered rock consists of tan and gray-green metavolcanic with some isolated areas of granite.

Crystalline rock was encountered along the existing roadways (-L-) at elevations ranging from 687.1 to 749.9 feet (MSL). The crystalline rock consists of metavolcanic.

Several rock outcrops were observed near the intersection of Old Beatty Ford Road (-L-) and Shuping Mill Road (-Y5-).

Groundwater

Groundwater generally occurs well below the ground surface with the exception of several isolated locations along the existing roadways (-L-) of the project. Groundwater was encountered at depths ranging from 0.1 to 12 feet below the existing ground surface. Areas with shallow groundwater were influenced by the weather during the investigation.

Ponds

One pond is located near the project right of way. This pond is listed by alignment, station, and offsets in the "Areas of Special Geotechnical Interest." This pond was investigated. Alluvial soils consist of brown and gray, sandy silts (A-4) and silty clays (A-7-5).

Prepared by,



Thomas R. Wells, P.E.
Senior Professional



Xavier C. Barrett, P.E.
Principal Professional

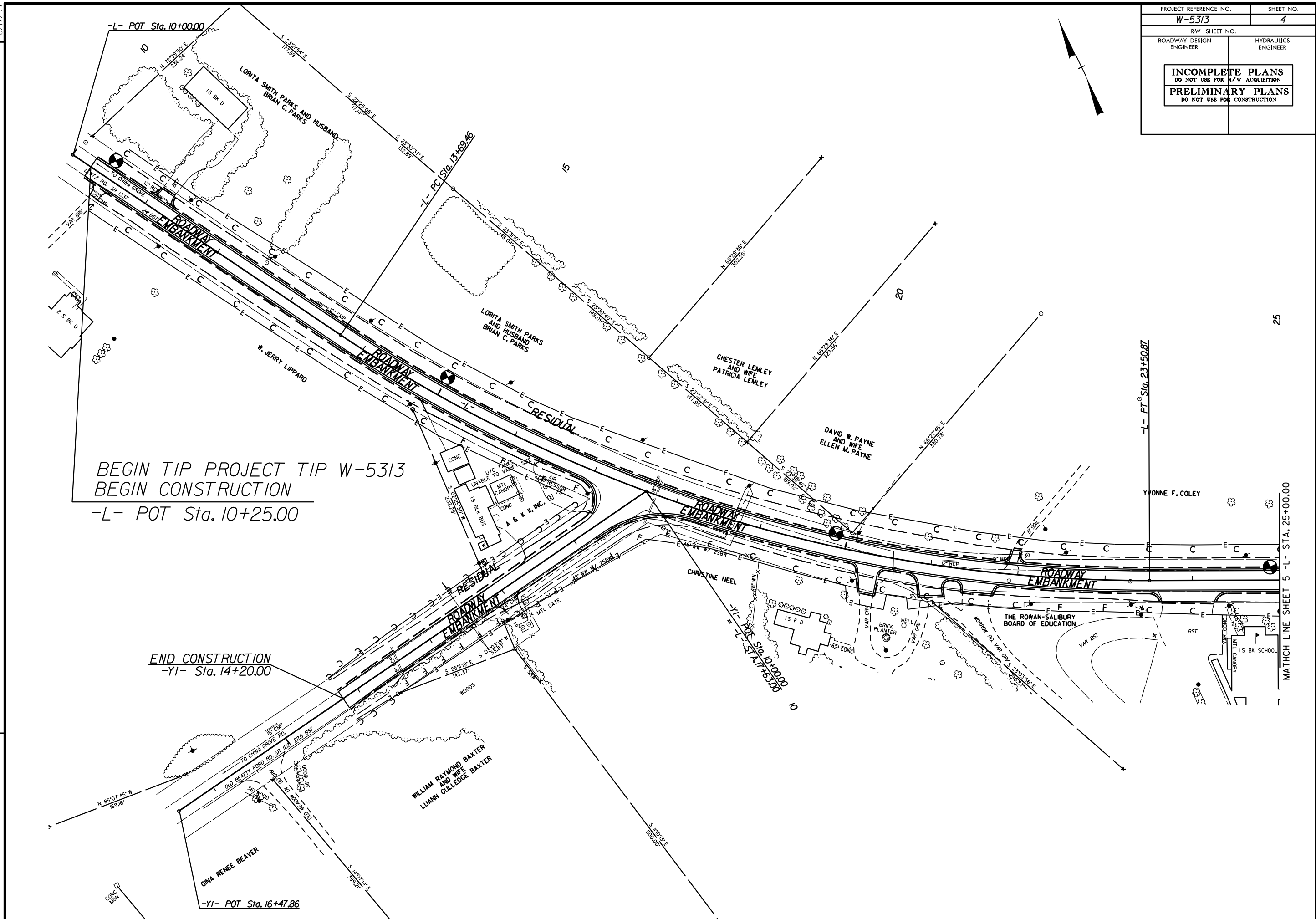
TRW/XCB:cas

PROJECT REFERENCE NO. W-5313	SHEET NO. 4
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



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BEGIN TIP PROJECT TIP W-5313
 BEGIN CONSTRUCTION
 -L- POT Sta. 10+25.00

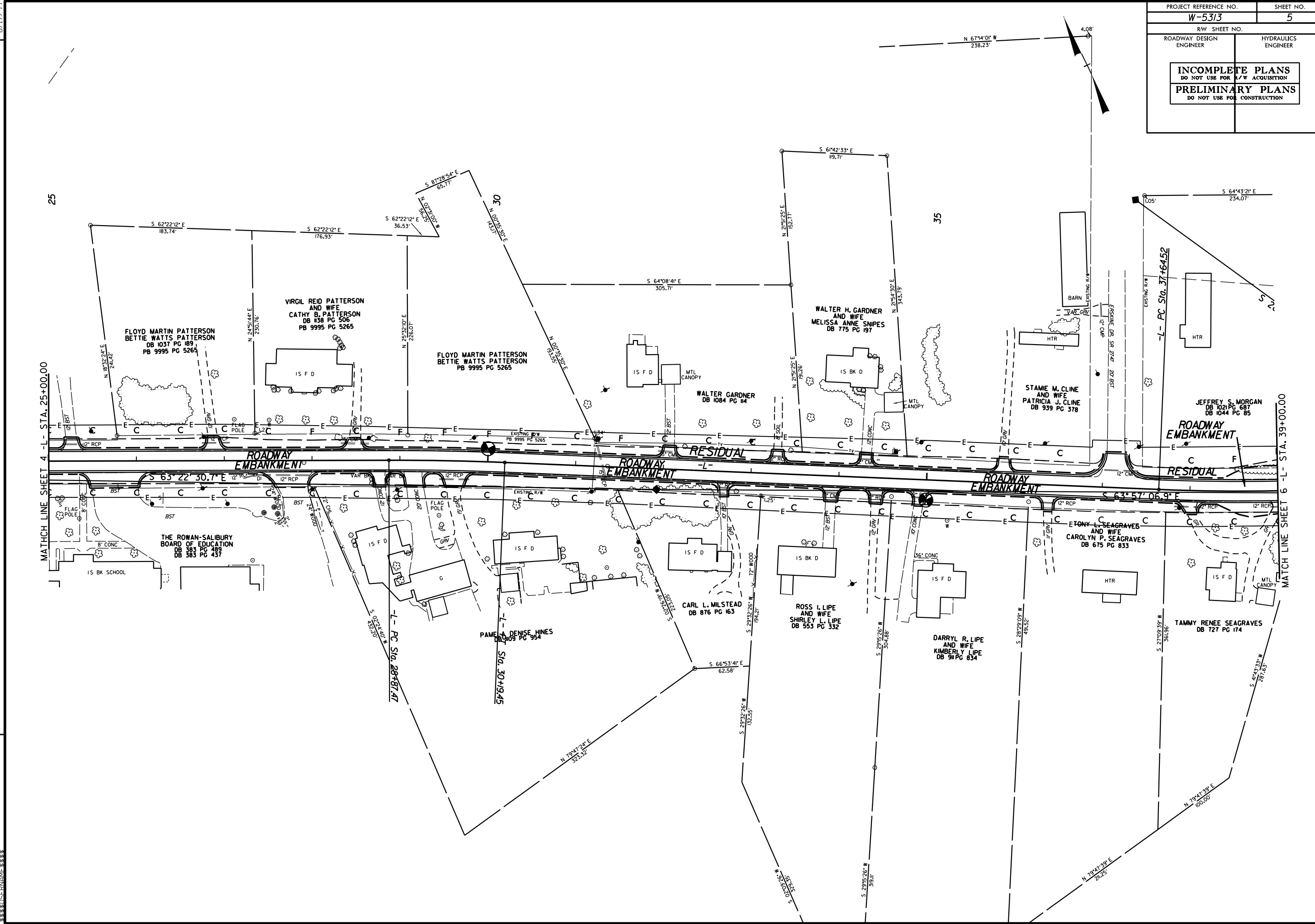
END CONSTRUCTION
 -YI- Sta. 14+20.00

25

MATCH LINE SHEET 5 -L- STA. 25+00.00

REVISIONS

PROJECT REFERENCE NO. W-5313	SHEET NO. 5
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS	
DO NOT USE FOR A/W ACQUISITION	
PRELIMINARY PLANS	
DO NOT USE FOR CONSTRUCTION	



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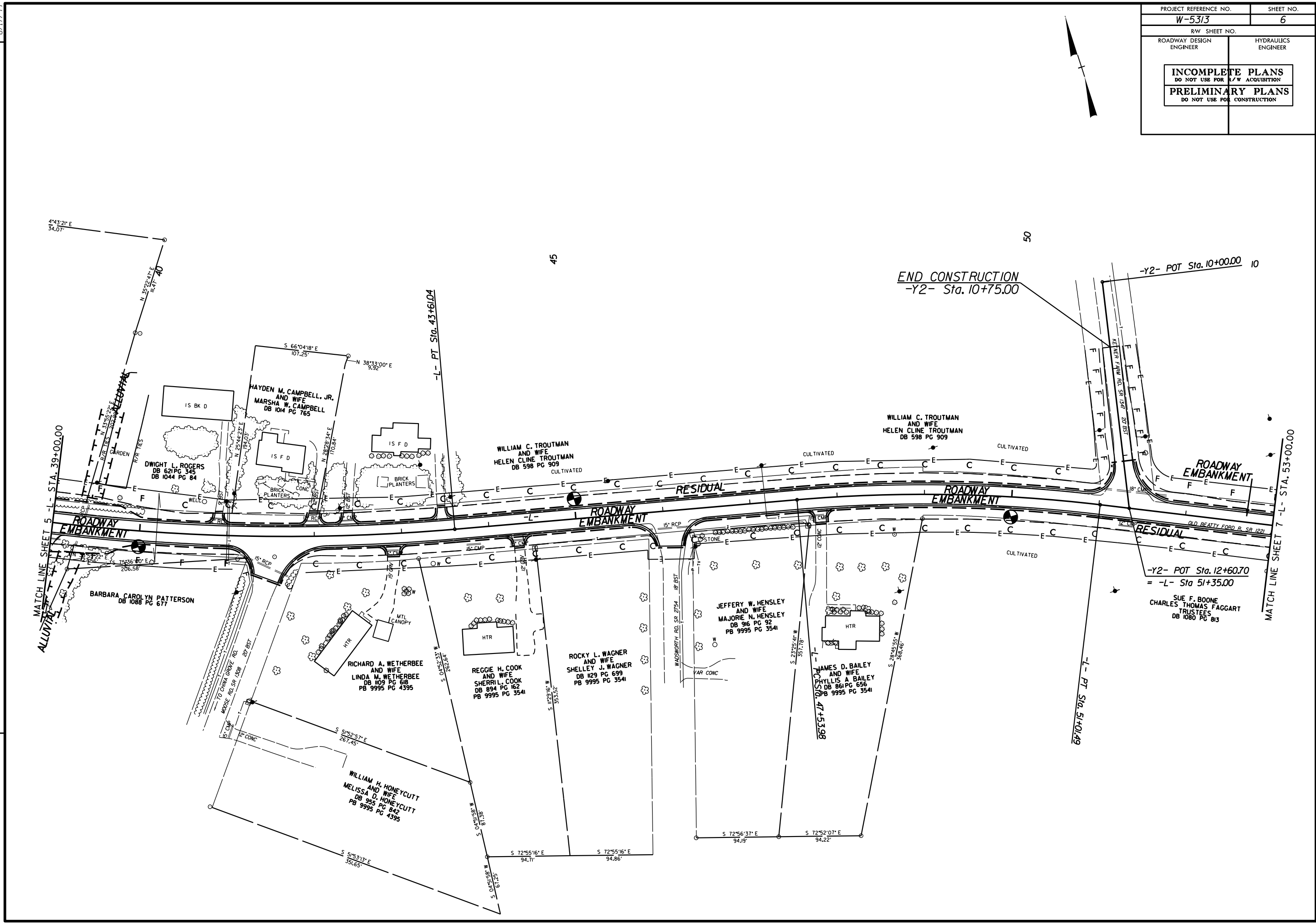
MATCH LINE SHEET 4 - L - STA. 25+00.00

MATCH LINE SHEET 6 - L - STA. 39+00.00

PROJECT REFERENCE NO. W-5313	SHEET NO. 6
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS
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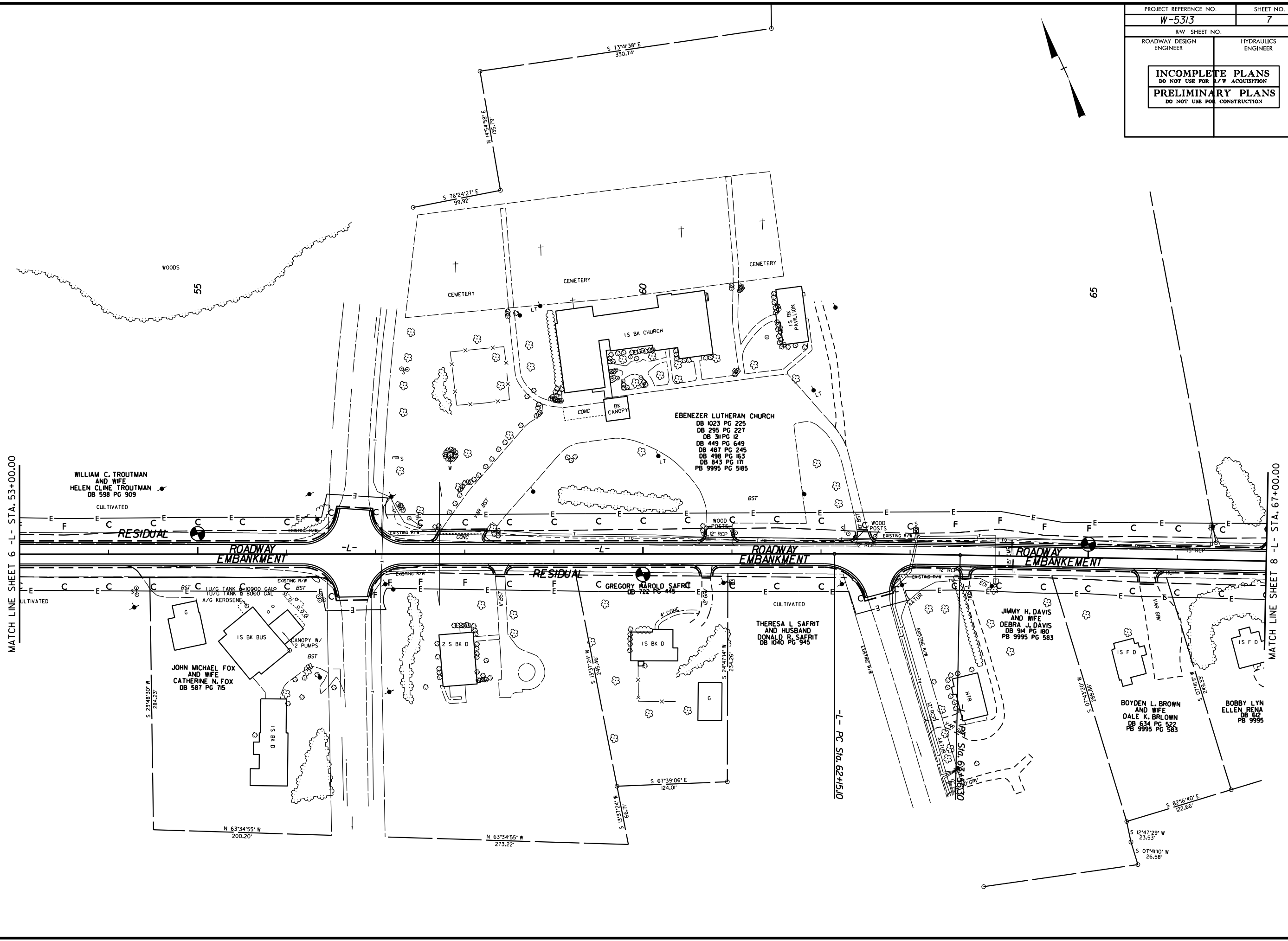


PROJECT REFERENCE NO.	SHEET NO.
W-5313	7
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



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REVISIONS



MATCH LINE SHEET 6 - L - STA. 53+00.00

MATCH LINE SHEET 8 - L - STA. 67+00.00

L - PC Sta. 62+51.0

WILLIAM C. TROUTMAN
AND WIFE
HELEN CLINE TROUTMAN
DB 598 PG 909
CULTIVATED

EBENEZER LUTHERAN CHURCH
DB 1023 PG 225
DB 295 PG 227
DB 311 PG 12
DB 449 PG 649
DB 487 PG 245
DB 498 PG 163
DB 843 PG 171
PB 9995 PG 5185

JOHN MICHAEL FOX
AND WIFE
CATHERINE N. FOX
DB 587 PG 715

GREGORY HAROLD SAFRIT
DB 445 PG 445

THERESA L SAFRIT
AND HUSBAND
DONALD R. SAFRIT
DB 1040 PG 945

JIMMY H. DAVIS
AND WIFE
DEBRA J. DAVIS
DB 914 PG 180
PB 9995 PG 583

BOYDEN L. BROWN
AND WIFE
DALE K. BROWN
DB 634 PG 522
PB 9995 PG 583

BOBBY LYN
ELLEN RENA
DB 612
PB 9995

S 23°46'30" W
284.23

N 63°34'55" W
200.20

N 63°34'55" W
273.22

S 67°39'06" E
124.00

S 13°37'24" W
245.46

S 24°47'14" W
234.26

S 01°41'10" W
153.33

S 82°16'40" E
122.66

S 12°47'29" W
23.53

S 07°41'10" W
26.58

S 73°41'38" E
330.74

S 76°24'27" E
99.92

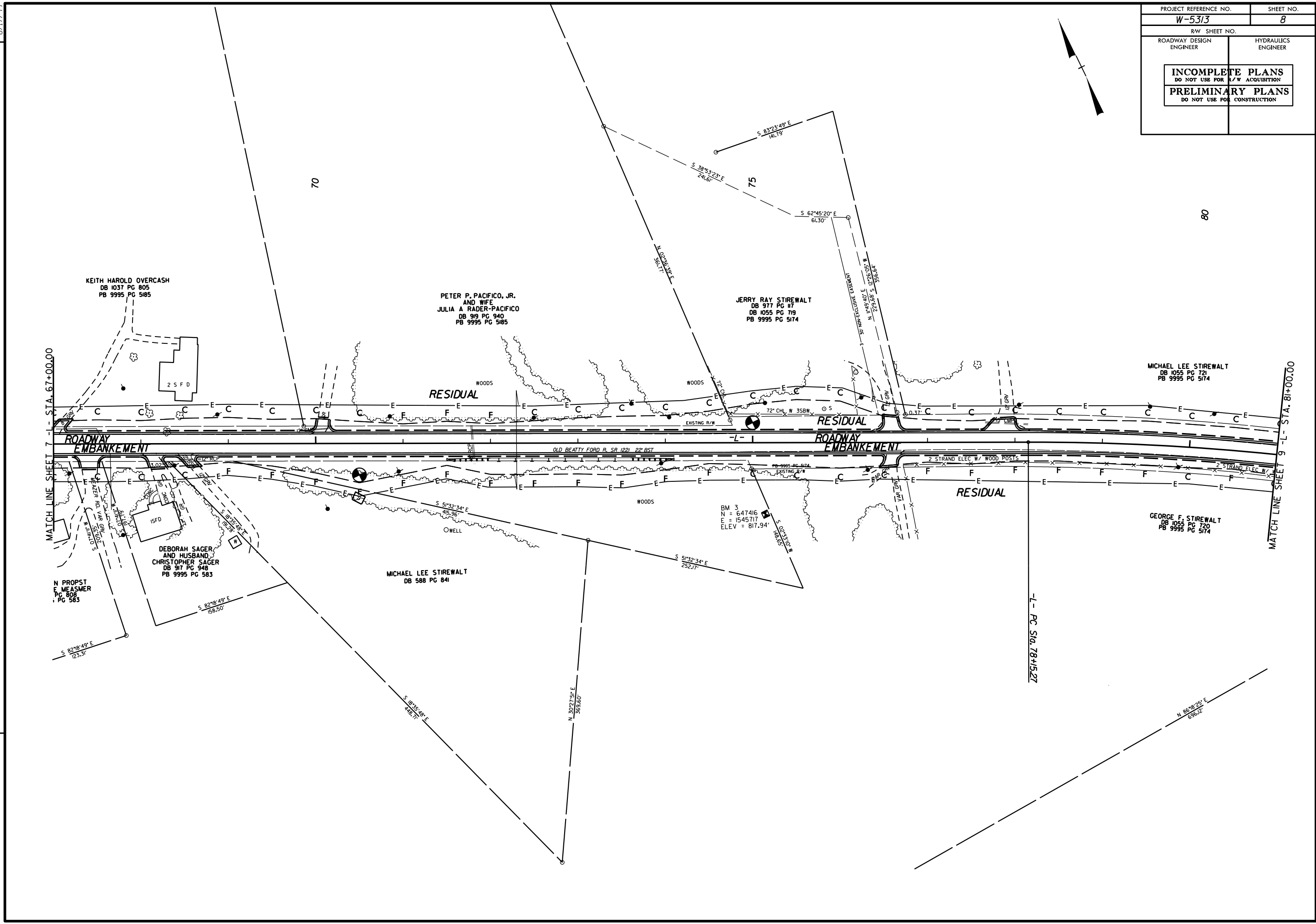
N 14°54'38" E
135.19

8/17/99

PROJECT REFERENCE NO. W-5313	SHEET NO. 8
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS



80

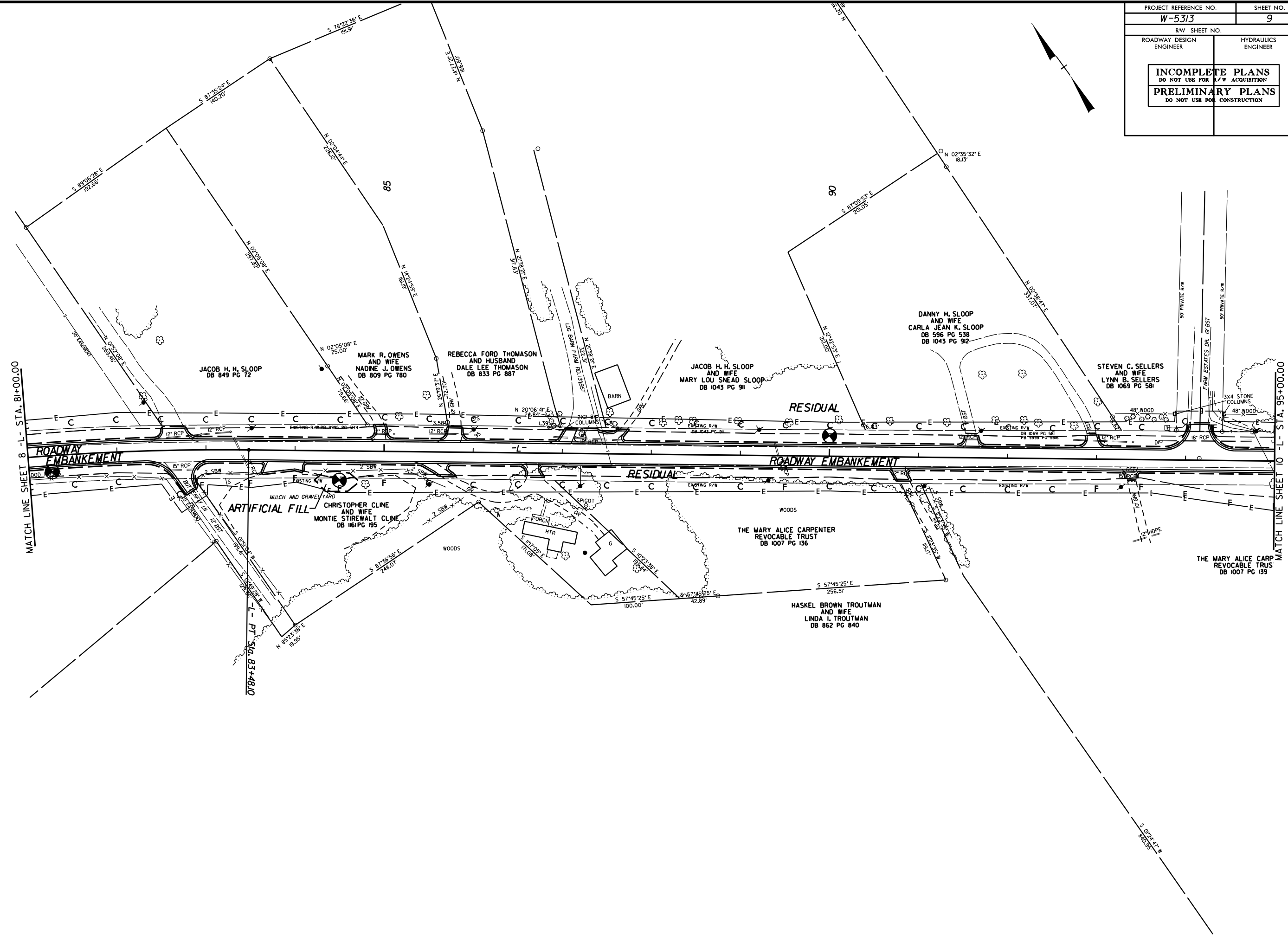
MATCH LINE SHEET 7

MATCH LINE SHEET 9

-L- PC Sta. 78+15.27

PROJECT REFERENCE NO. W-5313	SHEET NO. 9
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

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REVISIONS

MATCH LINE SHEET 8 - L - STA. 81+00.00

MATCH LINE SHEET 10 - L - STA. 95+00.00

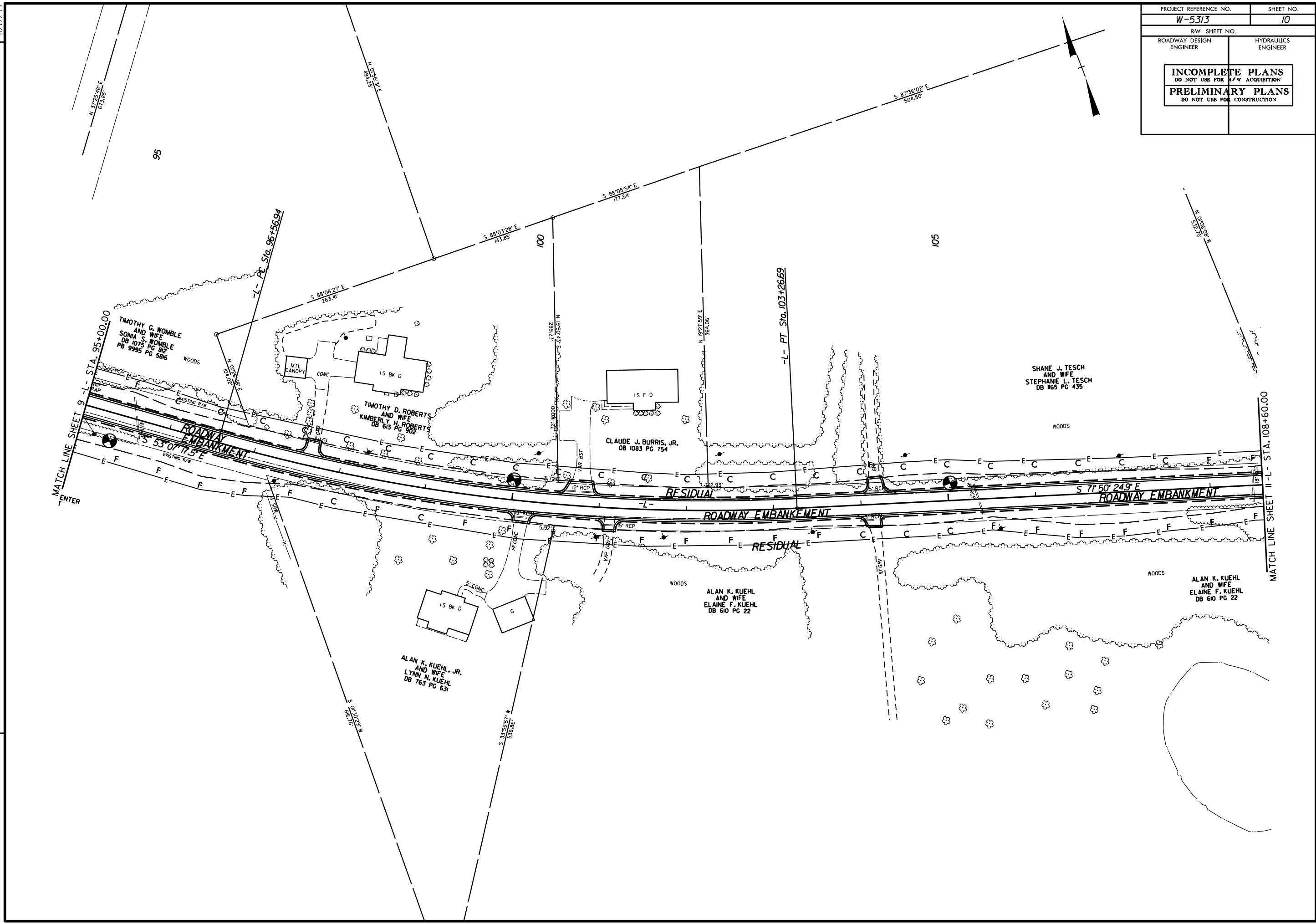
L-7 - PT STA. 83+48.10

S 0°24'47" E 100.00'

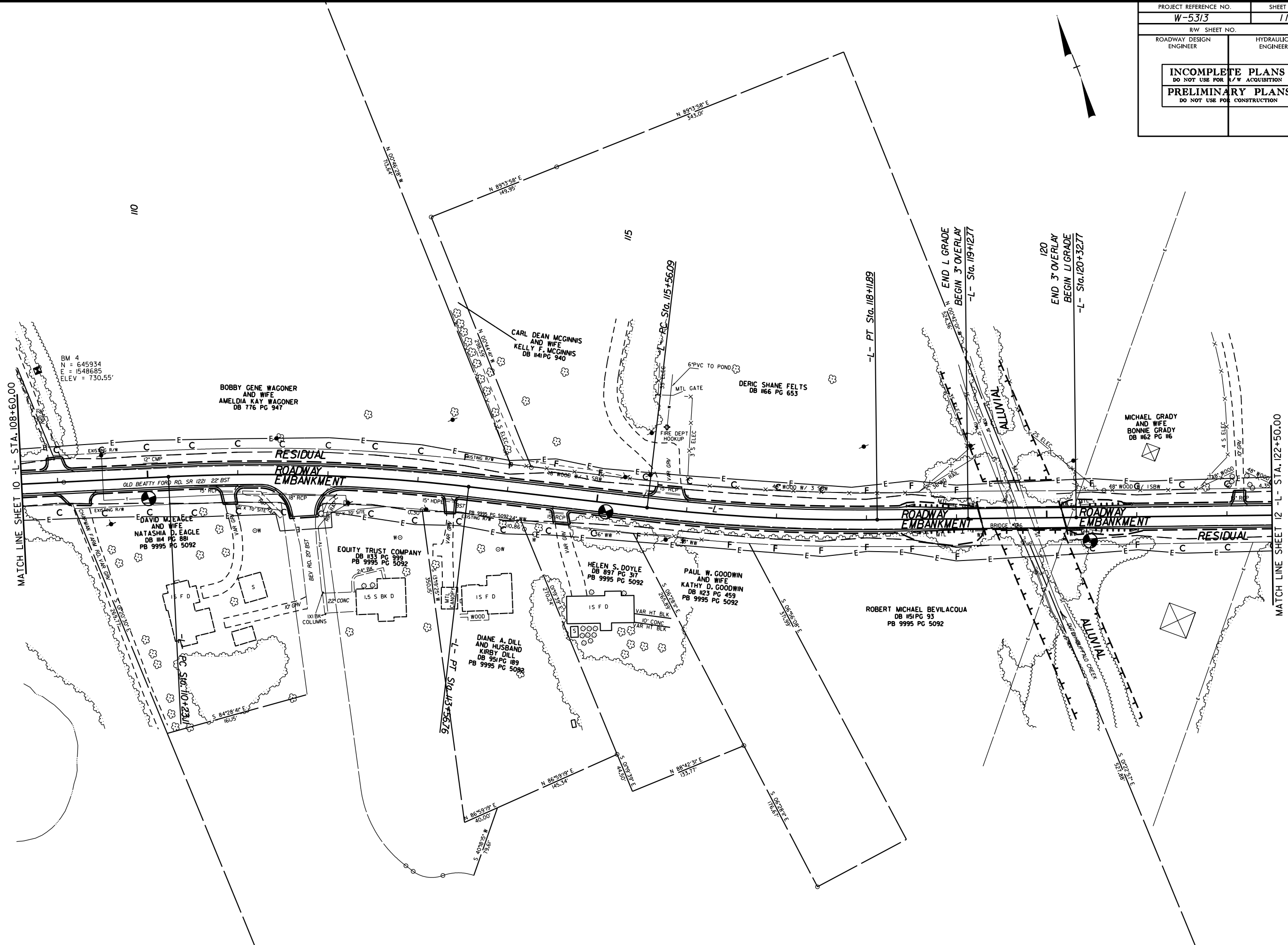
8/17/99

PROJECT REFERENCE NO. W-5313	SHEET NO. 10
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

REVISIONS



PROJECT REFERENCE NO.	SHEET NO.
W-5313	11
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS

MATCH LINE SHEET 10 - L - STA. 108+60.00

MATCH LINE SHEET 12 - L - STA. 122+50.00

BM 4
 N = 645934
 E = 1548685
 ELEV = 730.55'

BOBBY GENE WAGONER
 AND WIFE
 AMELDIA KAY WAGONER
 DB 776 PG 947

CARL DEAN MCGINNIS
 AND WIFE
 KELLY F. MCGINNIS
 DB 114 PG 940

DERIC SHANE FELTS
 DB #66 PG 653

MICHAEL GRADY
 AND WIFE
 BONNIE GRADY
 DB #62 PG #6

DAVID M. EAGLE
 AND WIFE
 NATASHA D. EAGLE
 DB #114 PG 881
 PB 9995 PG 5092

EQUITY TRUST COMPANY
 DB #133 PG 999
 PB 9995 PG 5092

HELEN S. DOYLE
 DB 897 PG 317
 PB 9995 PG 5092

PAUL W. GOODWIN
 AND WIFE
 KATHY D. GOODWIN
 DB #23 PG 459
 PB 9995 PG 5092

ROBERT MICHAEL BEVILACQUA
 DB #51 PG 93
 PB 9995 PG 5092

DIANE A. DILL
 AND HUSBAND
 KIRBY DILL
 DB 951 PG 189
 PB 9995 PG 5082

110

115

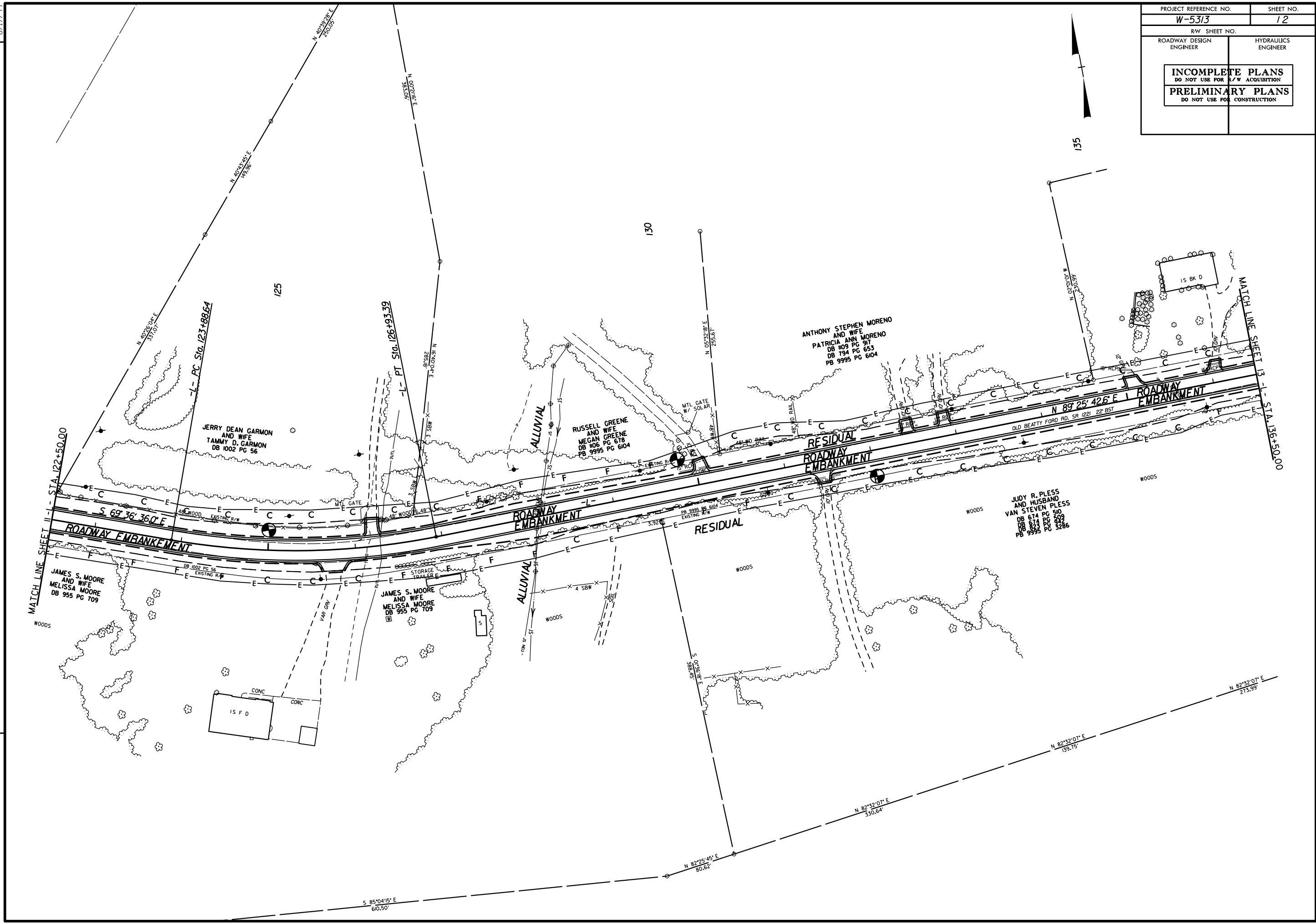
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8/17/99

PROJECT REFERENCE NO.	SHEET NO.
W-5313	12
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INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



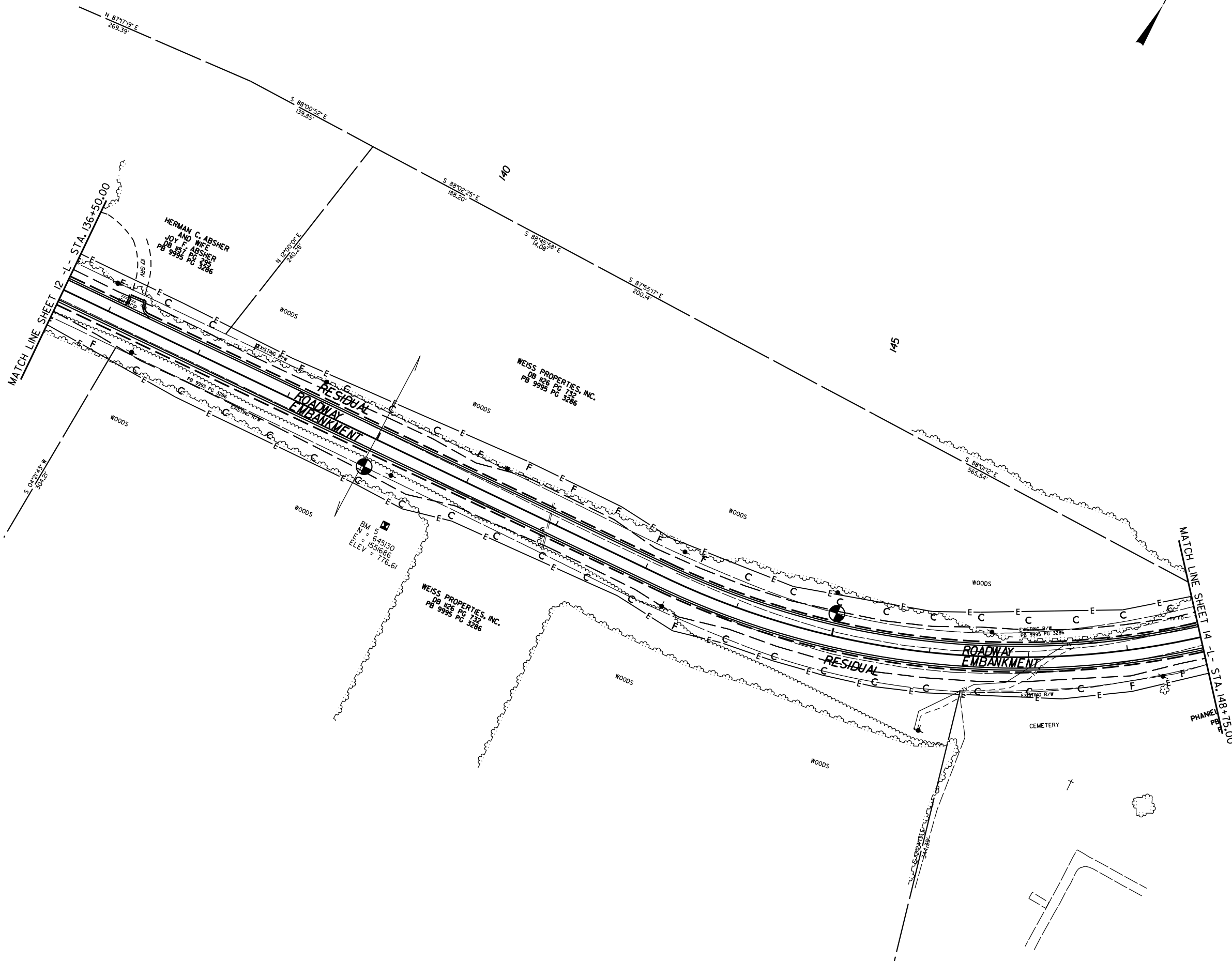
REVISIONS



B.17/99

REVISIONS

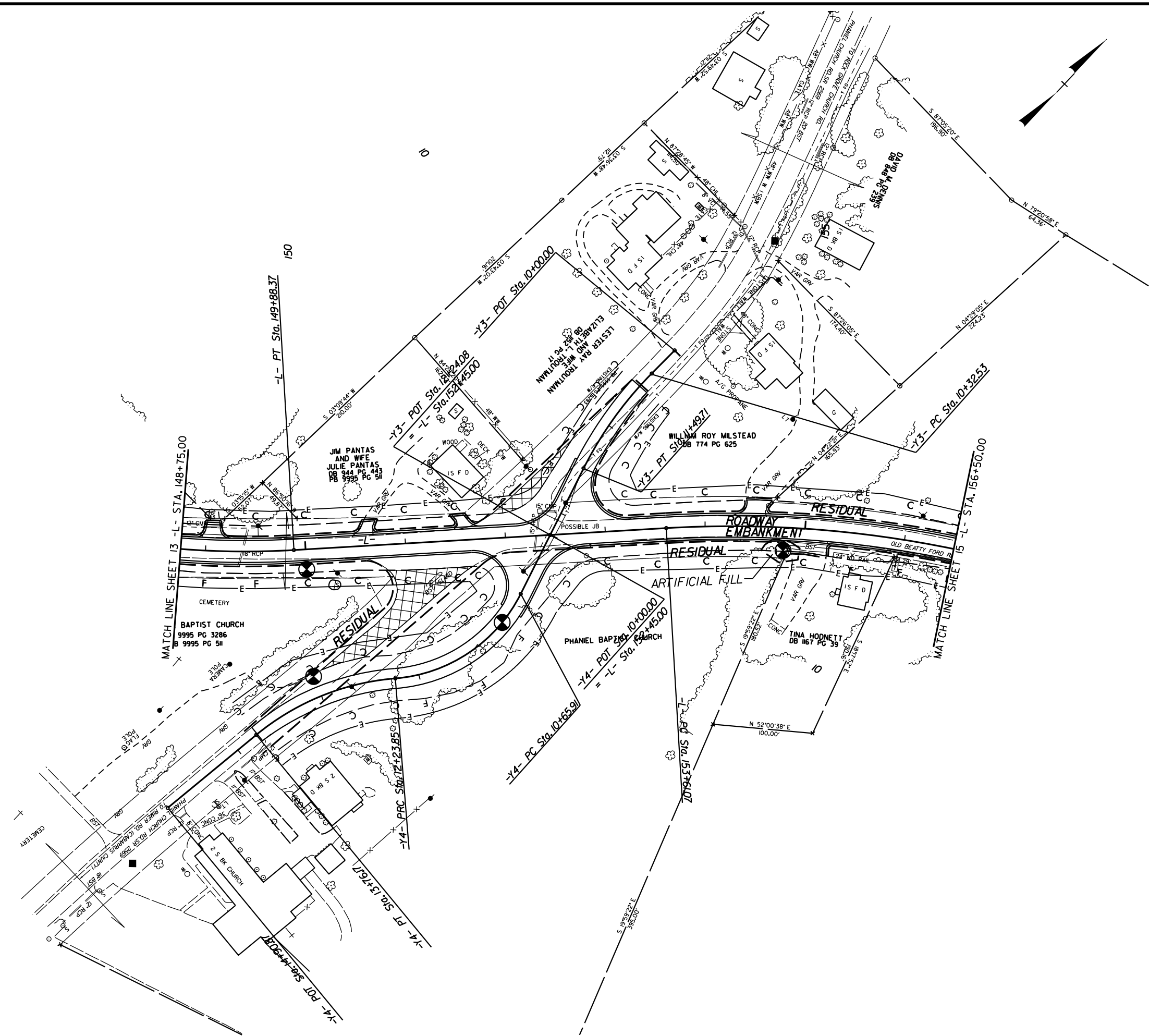
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RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



B.17/99

5/14/99

PROJECT REFERENCE NO.	SHEET NO.
W-5313	14
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INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

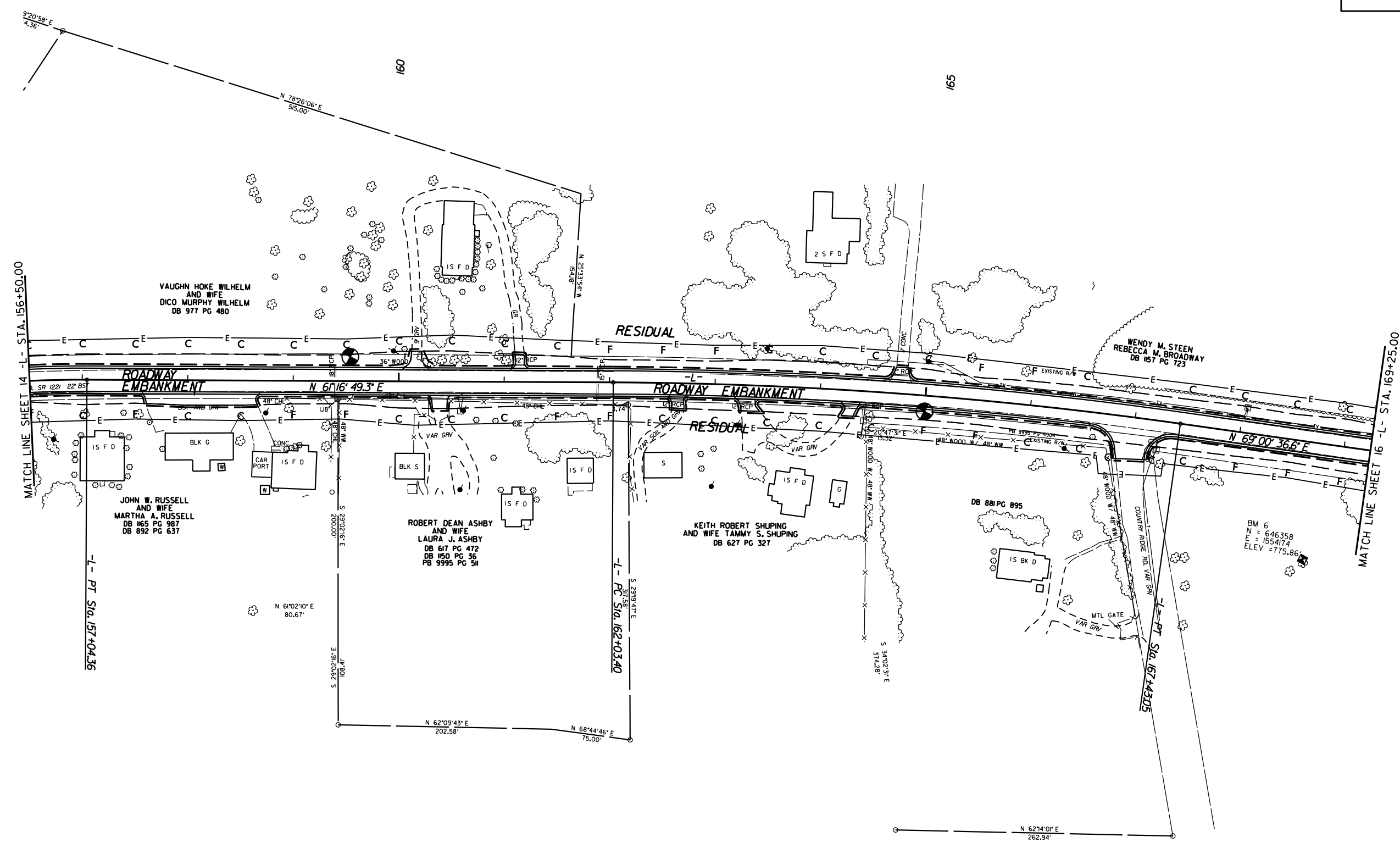


8/17/99

PROJECT REFERENCE NO. W-5313	SHEET NO. 15
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ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



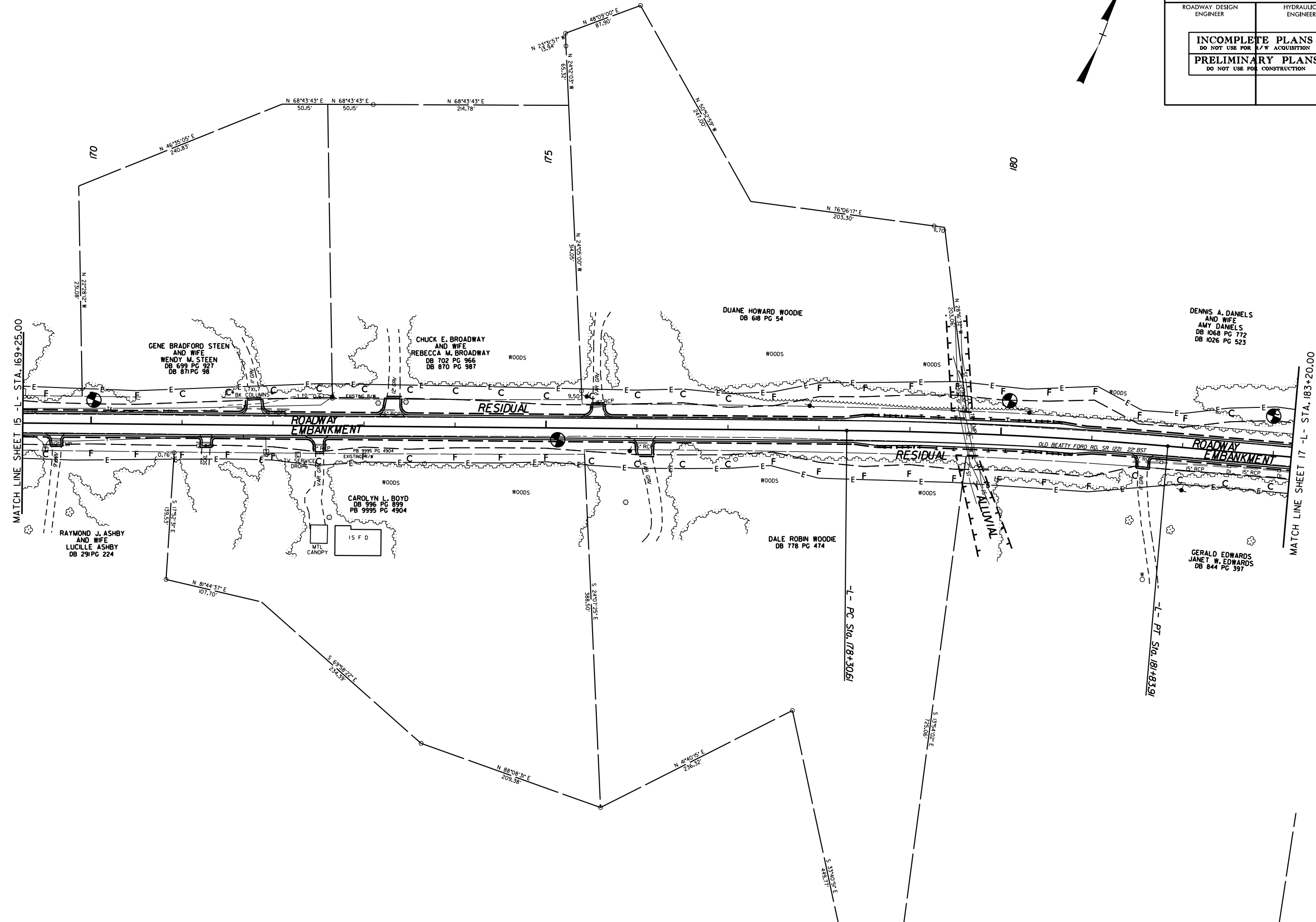
REVISIONS



262.94'

8/17/99

PROJECT REFERENCE NO. W-5313	SHEET NO. 16
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS

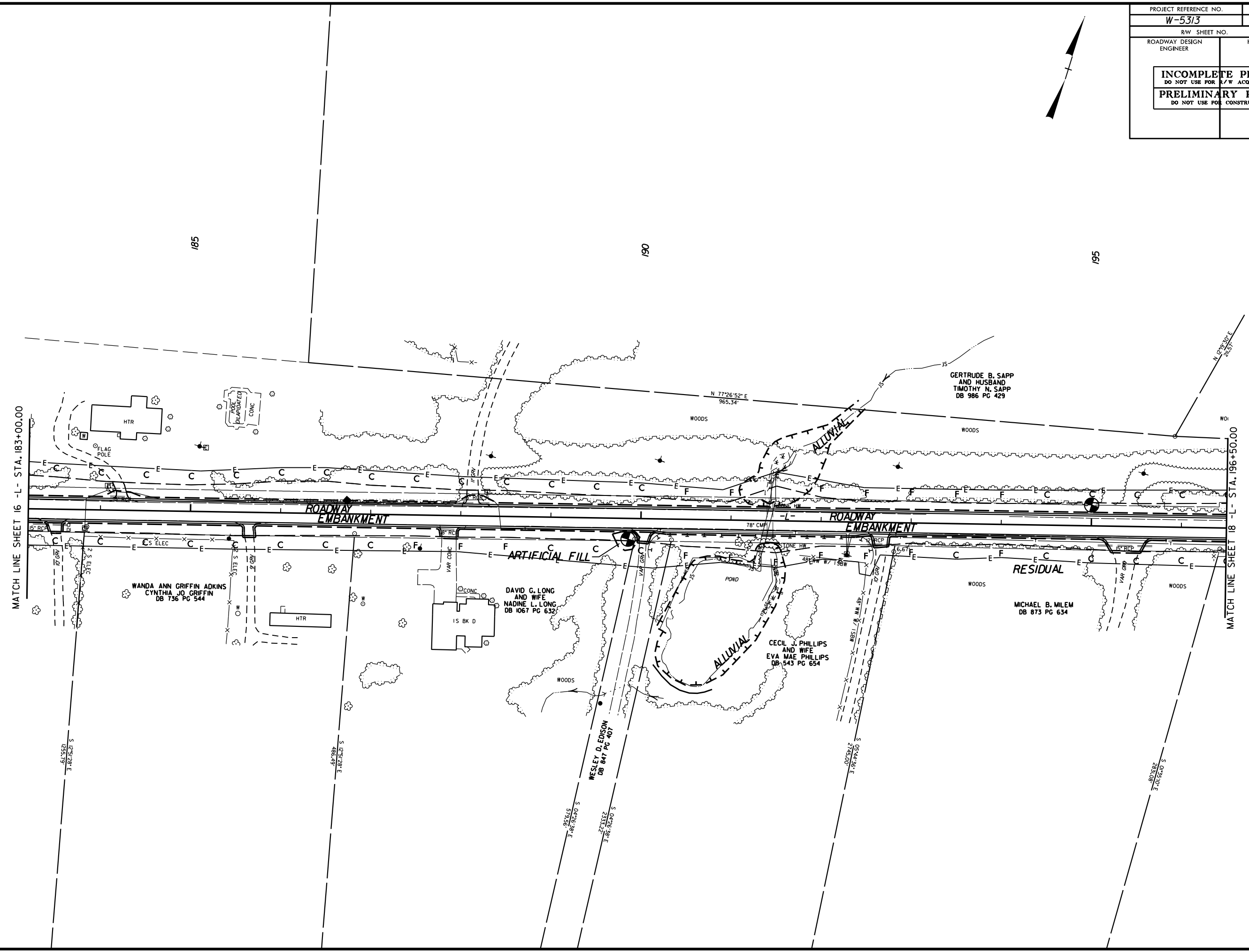
MATCH LINE SHEET 15 - L - STA. 169+25.00

MATCH LINE SHEET 17 - L - STA. 183+20.00

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REVISIONS

PROJECT REFERENCE NO. W-5313	SHEET NO. 17
RW SHEET NO.	
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INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



185

190

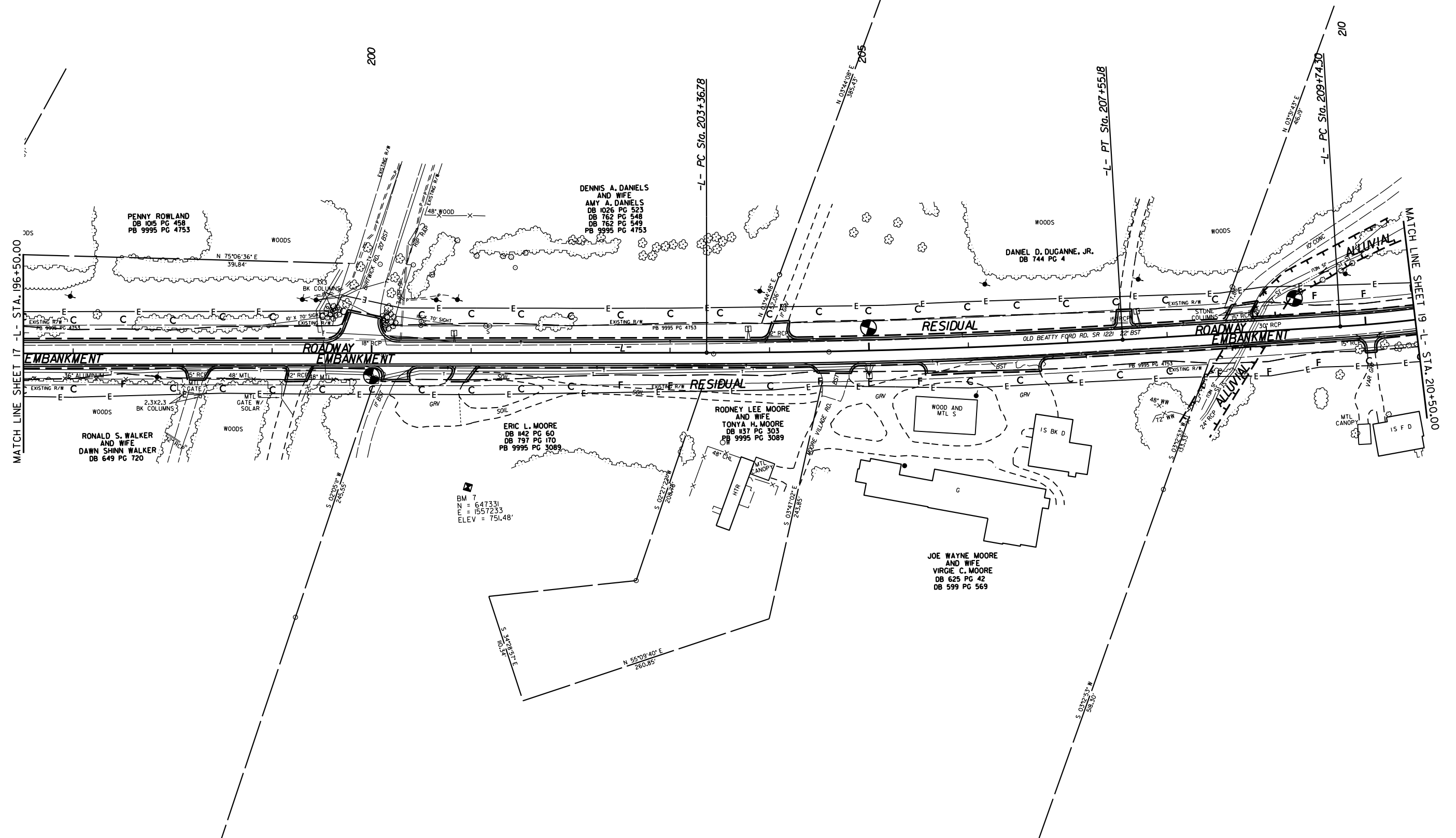
195

8/17/99

PROJECT REFERENCE NO. W-5313	SHEET NO. 18
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS



MATCH LINE SHEET 17 - L - STA. 196+50.00

MATCH LINE SHEET 19 - L - STA. 210+50.00

200

205

210

-L- PC Sta. 203+36.78

-L- PT Sta. 207+55.18

-L- PC Sta. 209+74.30

RESIDUAL

ROADWAY EMBANKMENT

EMBANKMENT

ROADWAY EMBANKMENT

RESIDUAL

RONALD S. WALKER
AND WIFE
DAWN SHINN WALKER
DB 649 PG 720

PENNY ROWLAND
DB 1005 PG 458
PB 9995 PG 4753

DENNIS A. DANIELS
AND WIFE
AMY A. DANIELS
DB 1026 PG 523
DB 762 PG 548
DB 762 PG 549
PB 9995 PG 4753

DANIEL D. DUGANNE, JR.
DB 744 PG 4

ERIC L. MOORE
DB 142 PG 60
DB 797 PG 170
PB 9995 PG 3089

RODNEY LEE MOORE
AND WIFE
TONYA H. MOORE
DB 137 PG 303
PB 9995 PG 3089

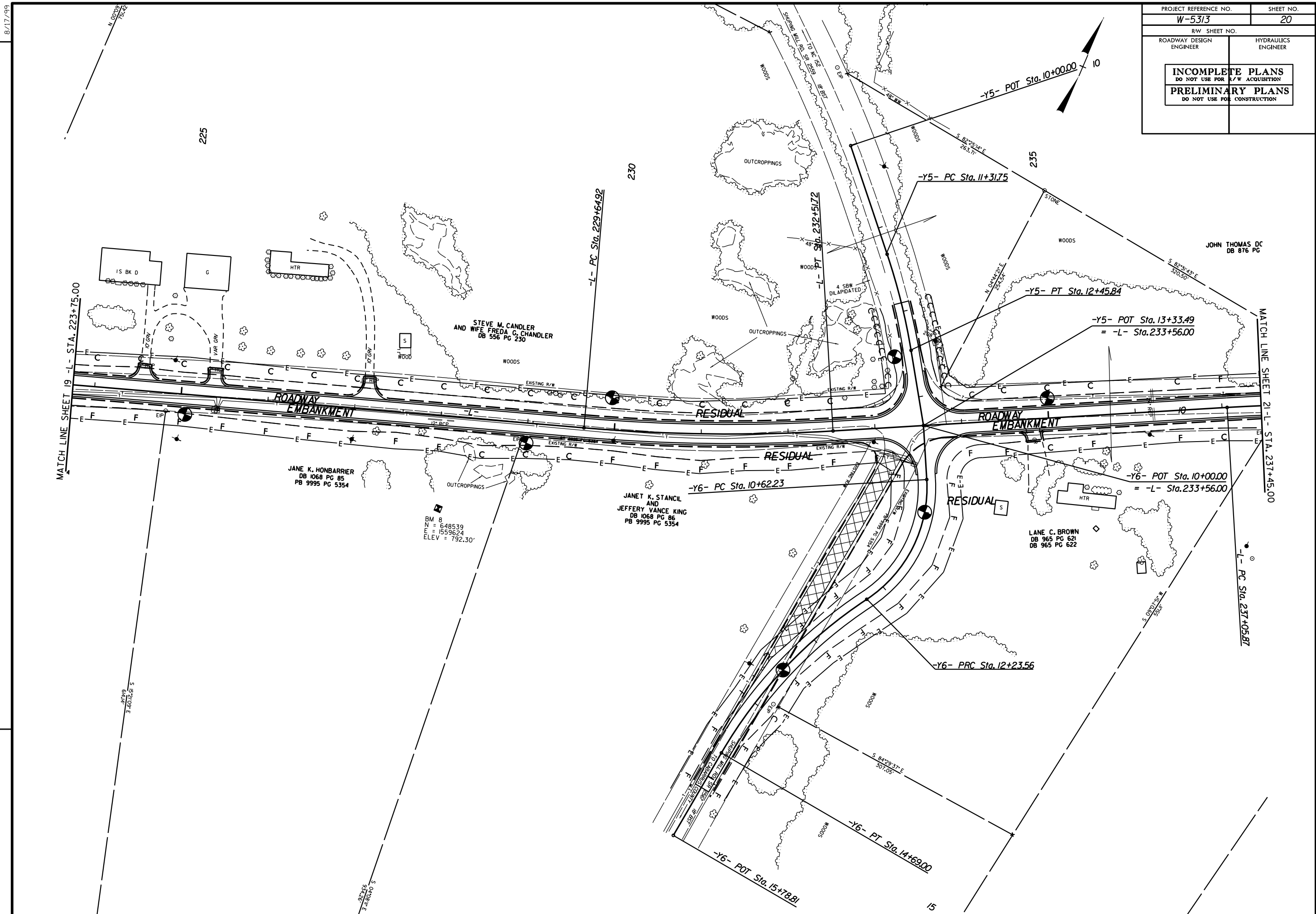
JOE WAYNE MOORE
AND WIFE
VIRGIE C. MOORE
DB 625 PG 42
DB 599 PG 569

BM 7
N = 647331
E = 1557233
ELEV = 751.48'

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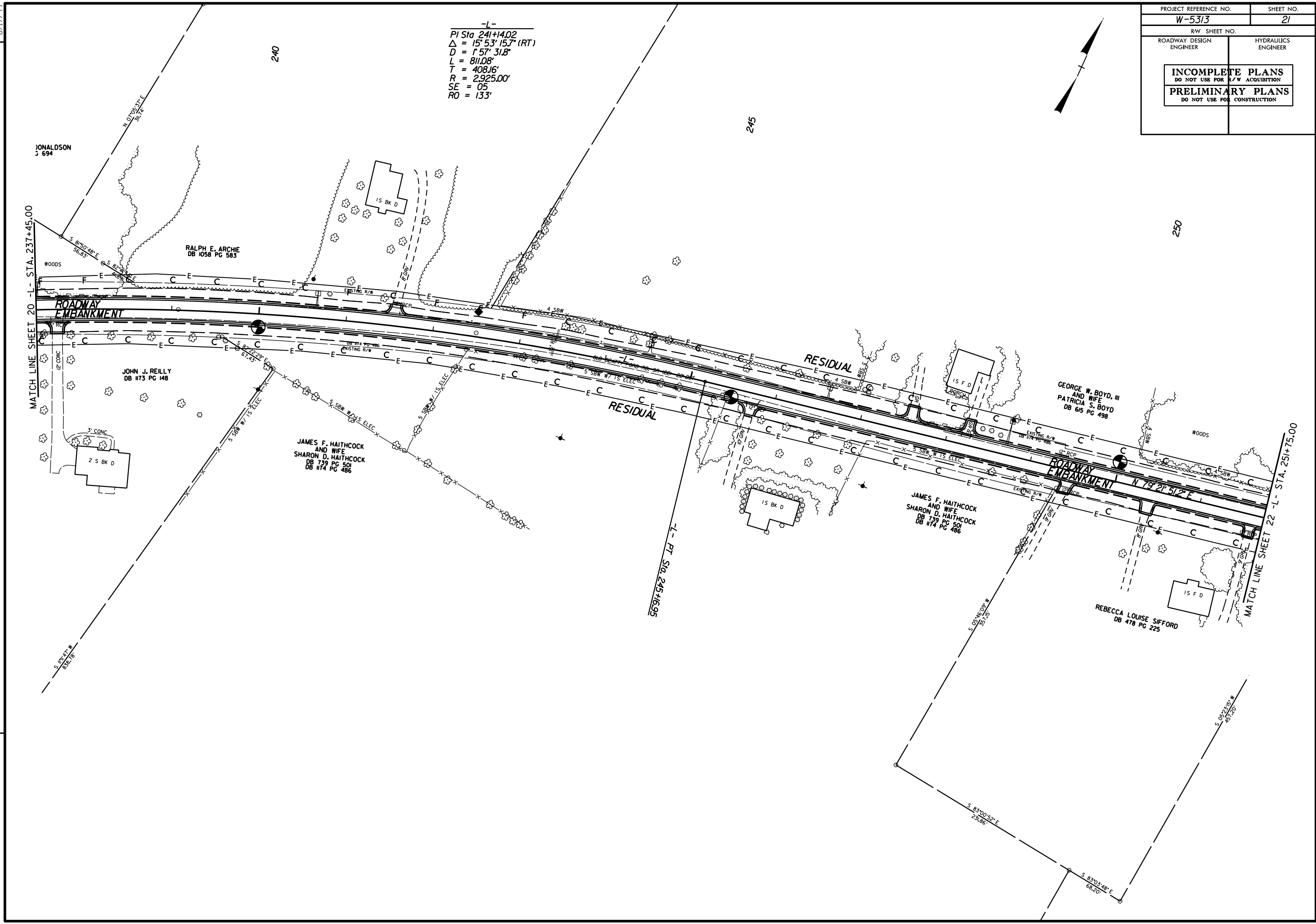
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W-5313	20
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

REVISIONS



PROJECT REFERENCE NO.	SHEET NO.
W-5313	21
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

-L-
 PI Sta 241+14.02
 $\Delta = 15^{\circ} 53' 15.7" (RT)$
 $D = 1^{\circ} 57' 31.8"$
 $L = 811.08'$
 $T = 408.16'$
 $R = 2925.00'$
 $SE = 05$
 $RO = 133'$



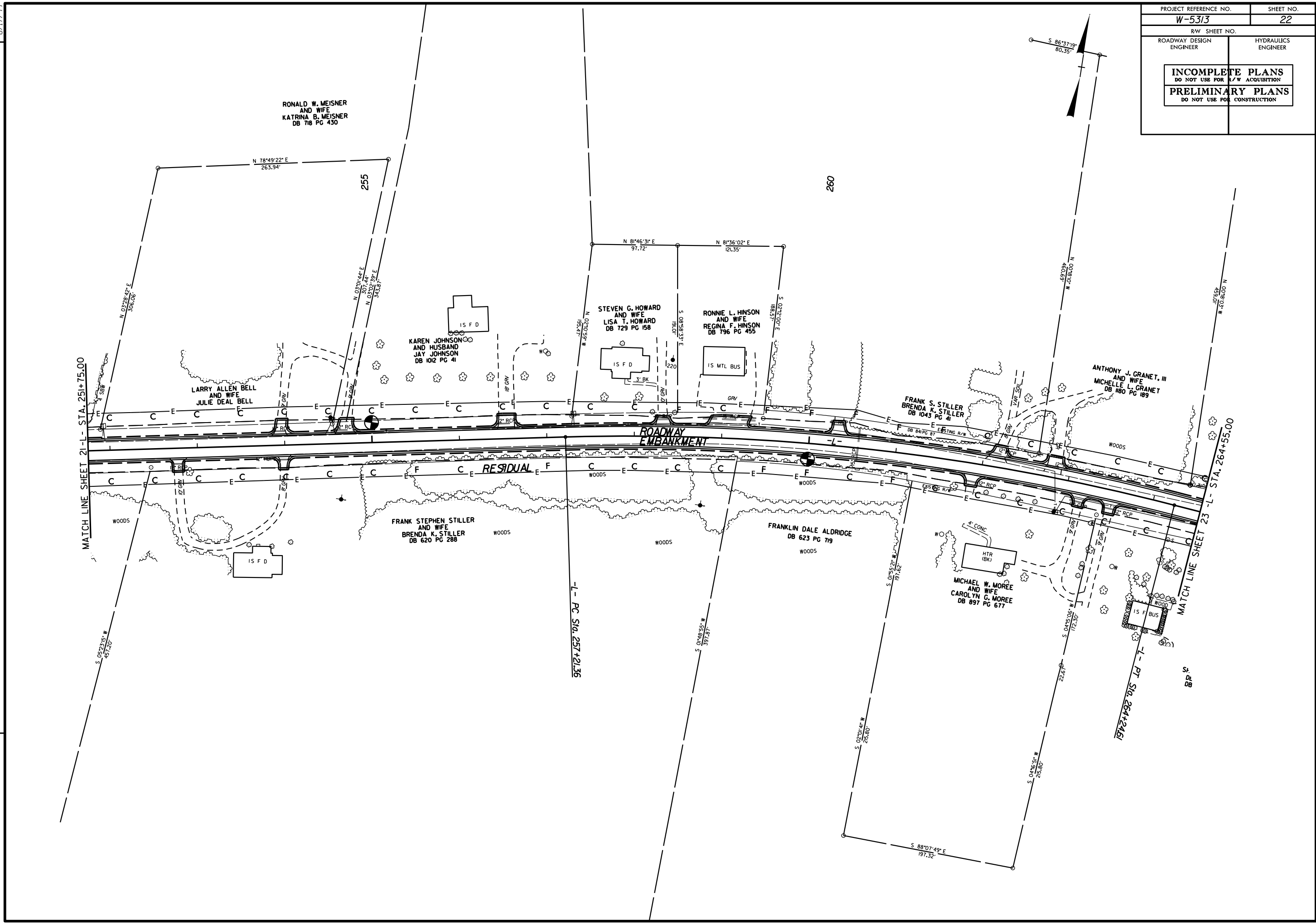
REVISIONS

8/17/99

8/17/99

REVISIONS

PROJECT REFERENCE NO.	SHEET NO.
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RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

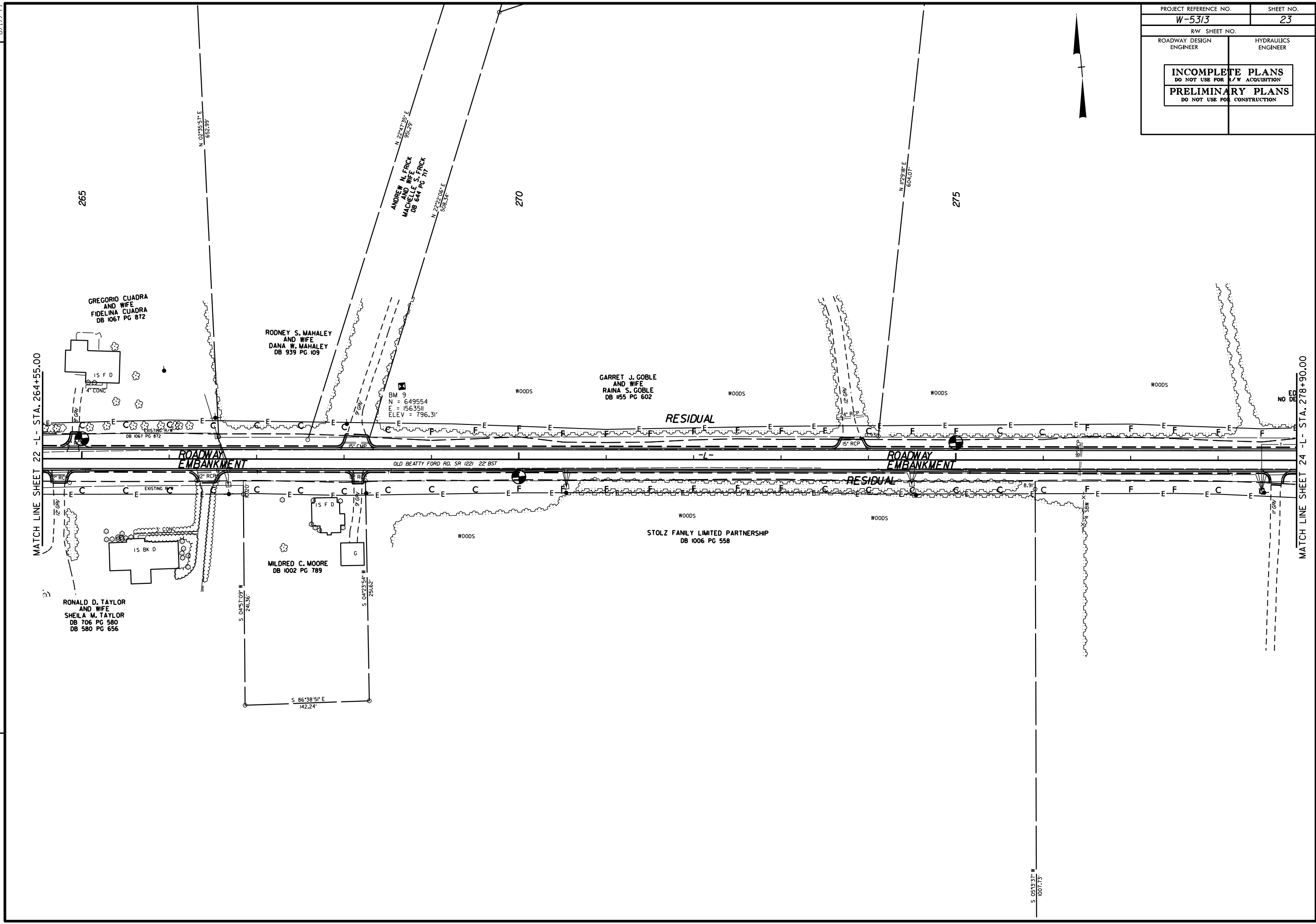


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PROJECT REFERENCE NO. W-5313	SHEET NO. 23
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



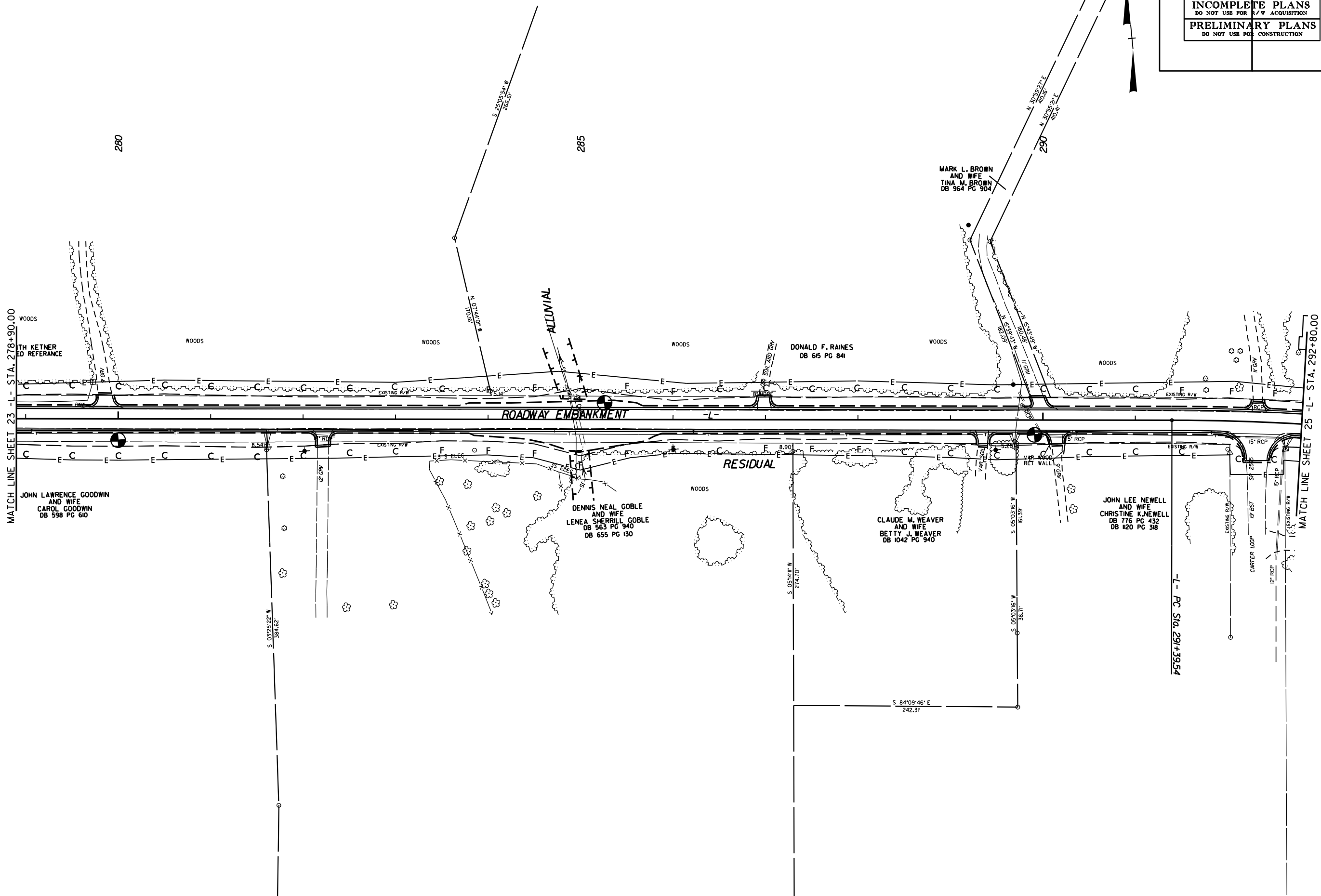
REVISIONS



S 05°53'37" W 1001.73'

8/17/99

PROJECT REFERENCE NO. W-5313	SHEET NO. 24
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

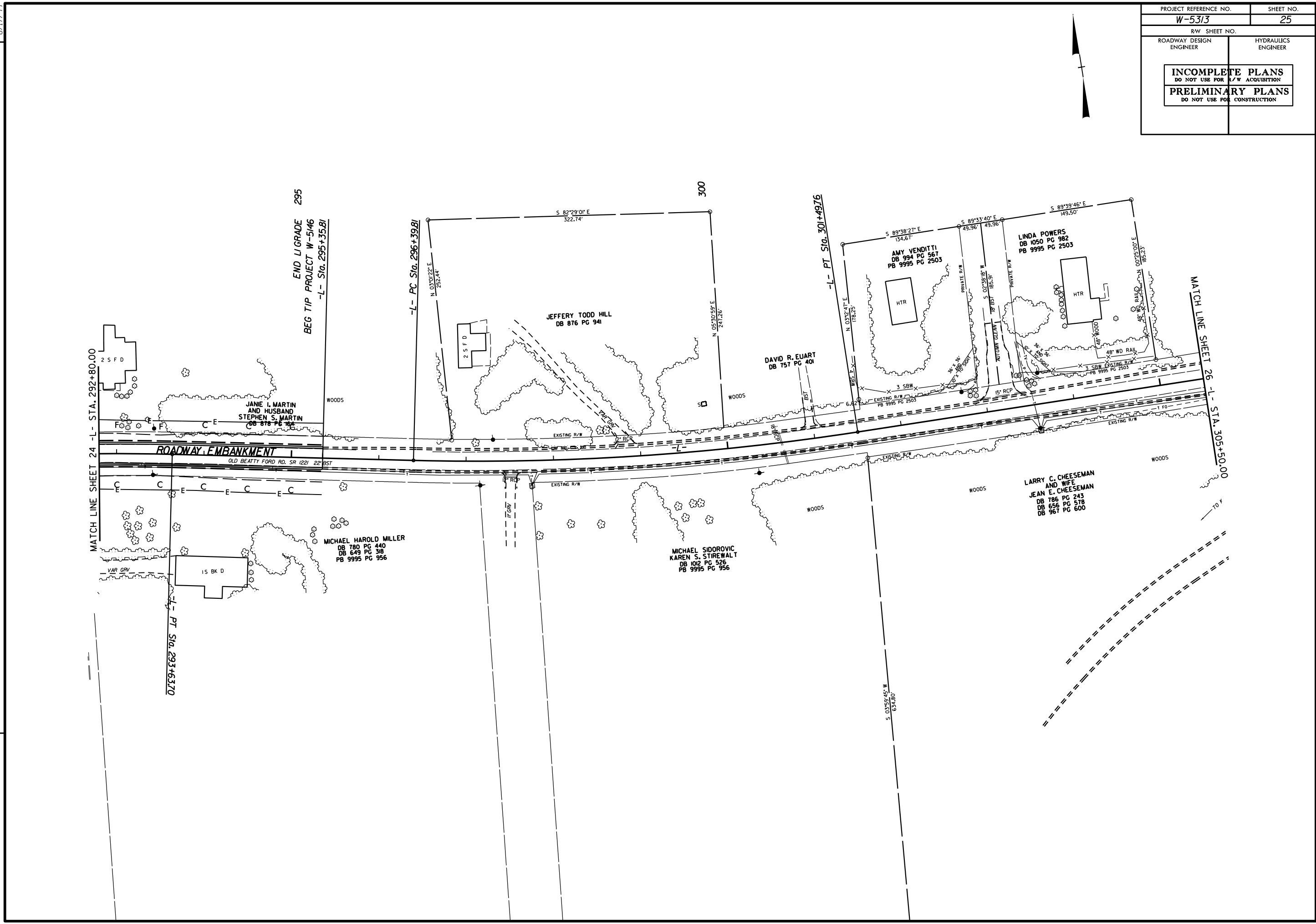


REVISIONS

PROJECT REFERENCE NO. W-5313	SHEET NO. 25
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS

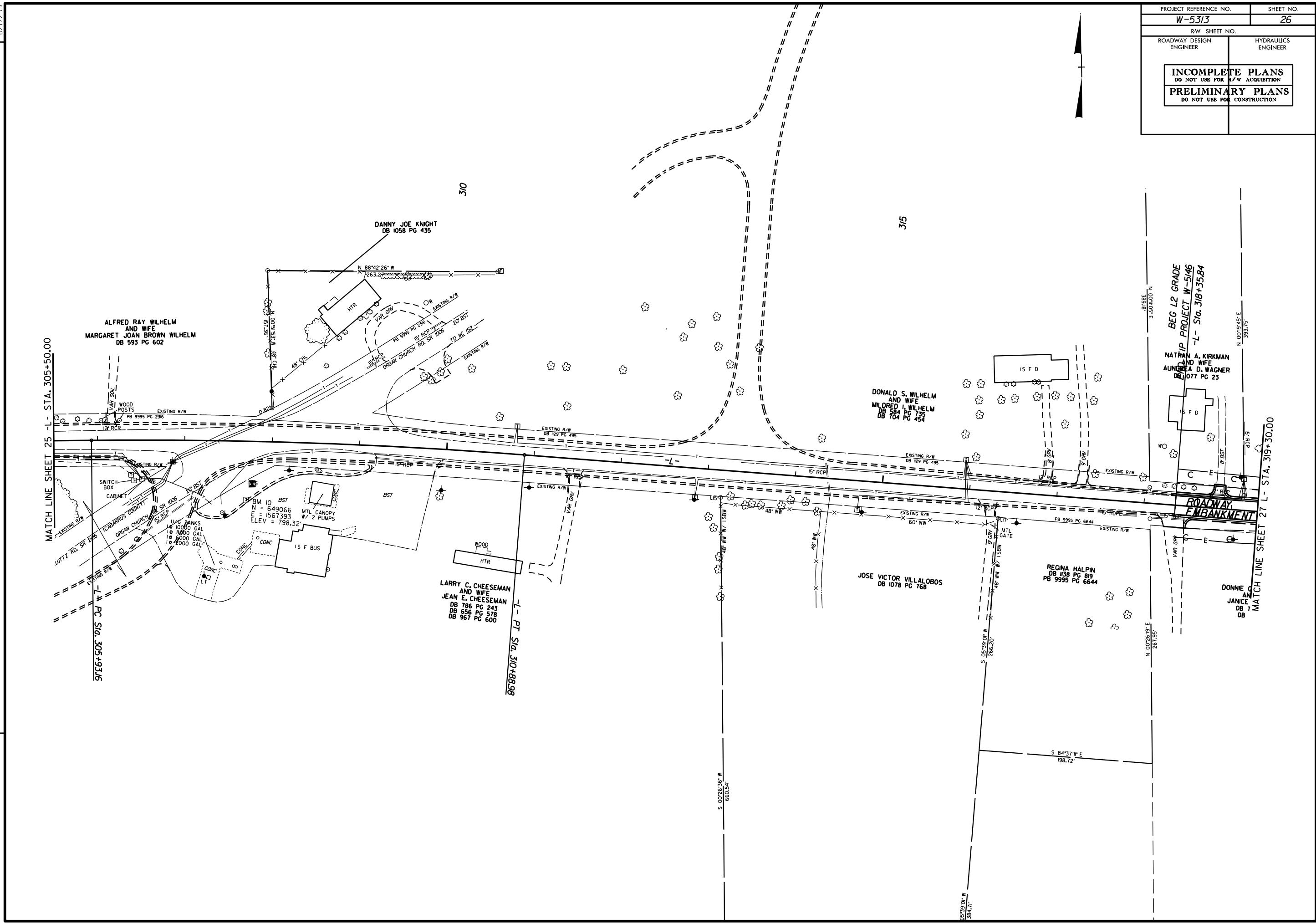


8/17/99

PROJECT REFERENCE NO. W-5313		SHEET NO. 26	
RW SHEET NO.			
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INCOMPLETE PLANS DO NOT USE FOR ACQUISITION			
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			



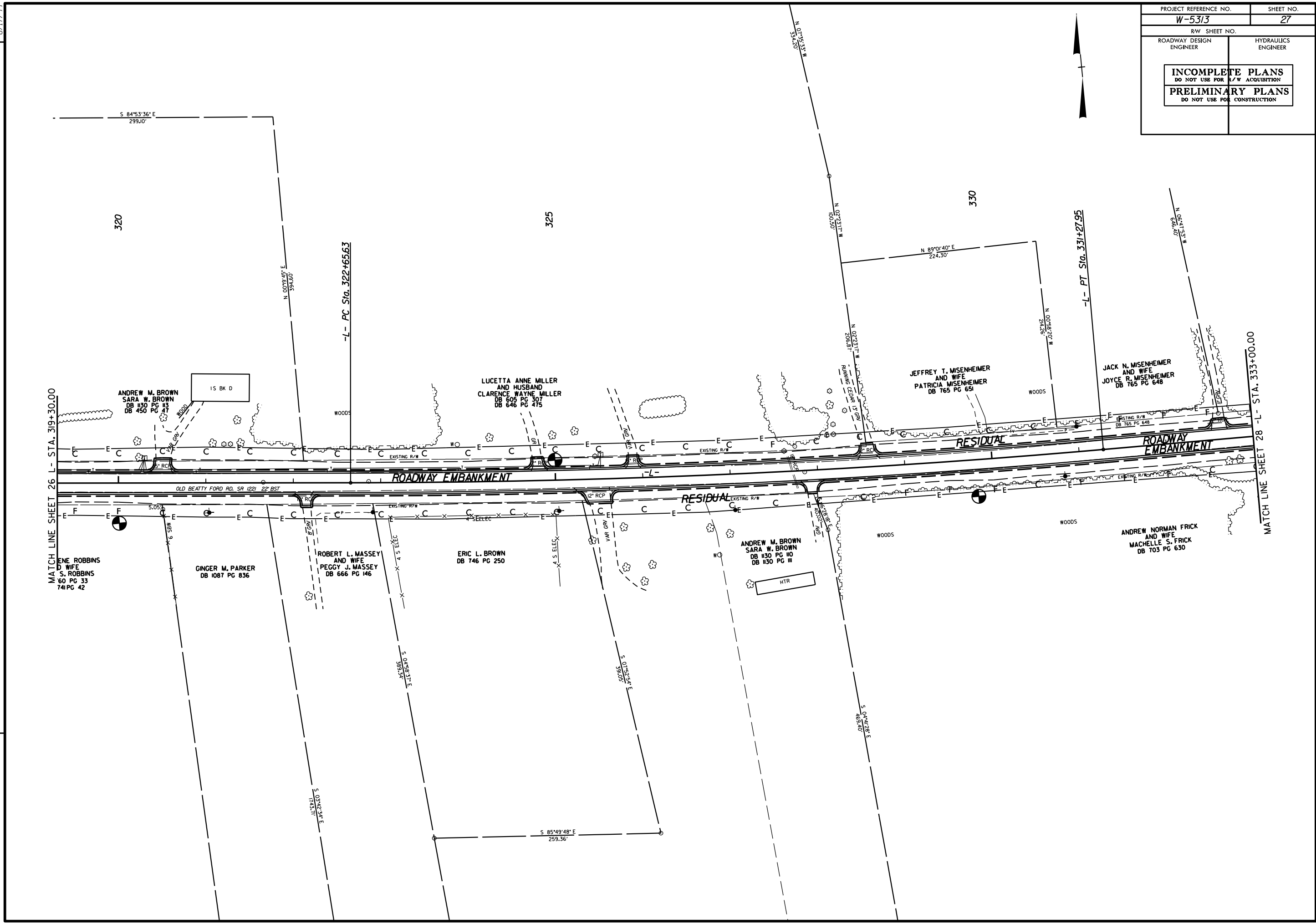
REVISIONS



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REVISIONS

PROJECT REFERENCE NO. W-5313	SHEET NO. 27
RW SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



MATCH LINE SHEET 26 -L- STA. 319+30.00

MATCH LINE SHEET 28 -L- STA. 333+00.00

ANDREW M. BROWN
SARA W. BROWN
DB 130 PG 13
DB 450 PG 47

LUCETTA ANNE MILLER
AND HUSBAND
CLARENCE WAYNE MILLER
DB 605 PG 307
DB 646 PG 475

JEFFREY T. MISENHEIMER
AND WIFE
PATRICIA MISENHEIMER
DB 765 PG 651

JACK N. MISENHEIMER
AND WIFE
JOYCE R. MISENHEIMER
DB 765 PG 648

ENE ROBBINS
D. WIFE
S. ROBBINS
60 PG 33
741 PG 42

GINGER M. PARKER
DB 1087 PG 836

ROBERT L. MASSEY
AND WIFE
PEGGY J. MASSEY
DB 666 PG 146

ERIC L. BROWN
DB 746 PG 250

ANDREW M. BROWN
SARA W. BROWN
DB 130 PG 13
DB 450 PG 47

ANDREW NORMAN FRICK
AND WIFE
MACHELLE S. FRICK
DB 703 PG 630

S 84°53'36" E
299.10'

N 00°19'45" E
394.60'

-L- PC Sta. 322+65.63

325

330

-L- PT Sta. 331+27.95

N 89°01'40" E
224.30'

N 0°19'30" E
119.90'

OLD BEAUTY FORD RD. SR 1221 22' BST

ROADWAY EMBANKMENT

RESIDUAL

ROADWAY EMBANKMENT

S 85°49'48" E
259.36'

S 04°42'28" E
1465.40'

S 03°42'34" E
1135.11'

S 11°28'50" S
311.85'

S 01°52'54" E
312.05'

S 4°5' ELEC.

S 4°5' ELEC.

HTR

WOODS

WOODS

WOODS

WOODS

WOODS

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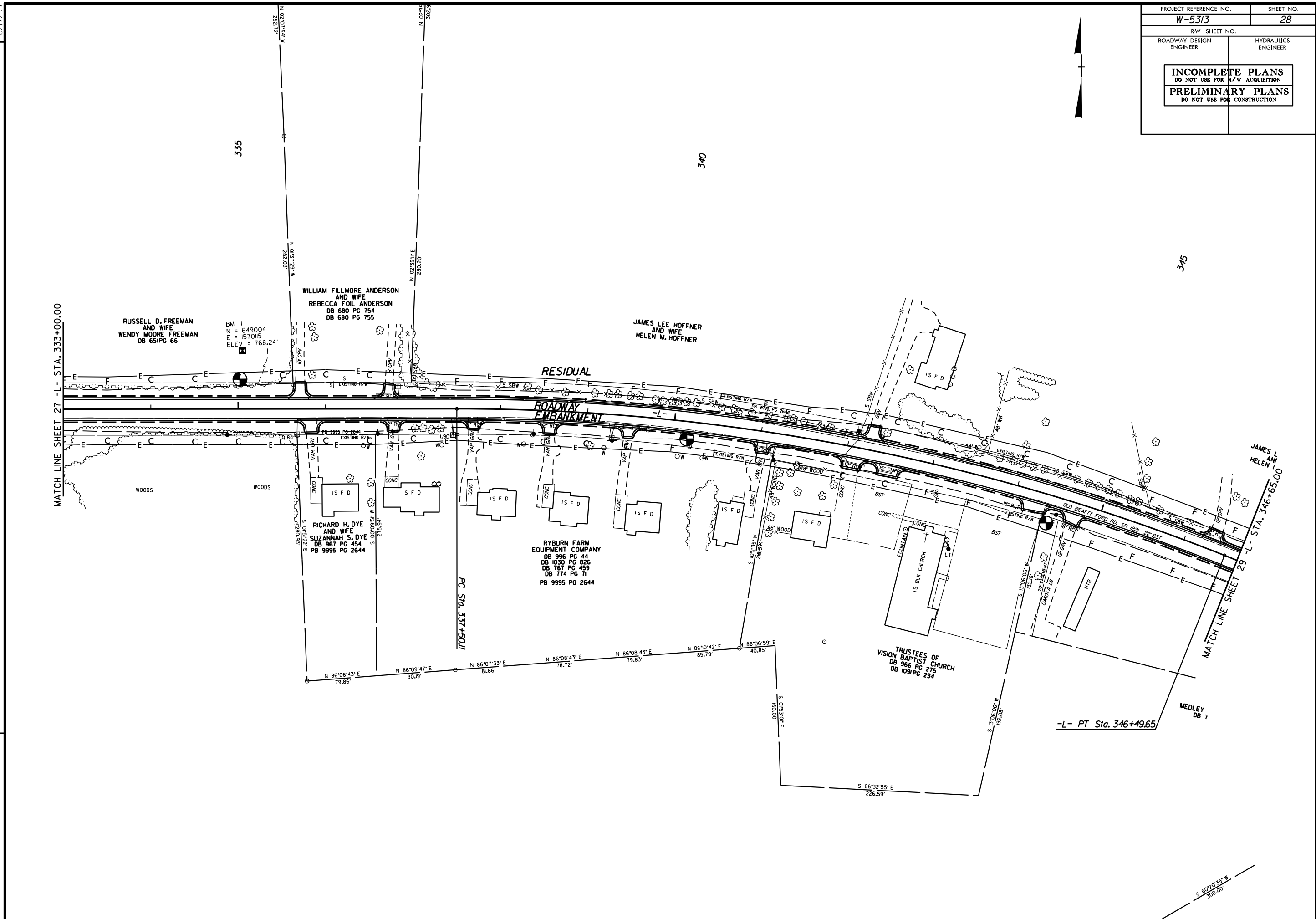
E C

PROJECT REFERENCE NO. W-5313	SHEET NO. 28
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



8/17/99

REVISIONS



345

-L- PT Sta. 346+49.65

S 60°20'35" W
300.00

8/17/99

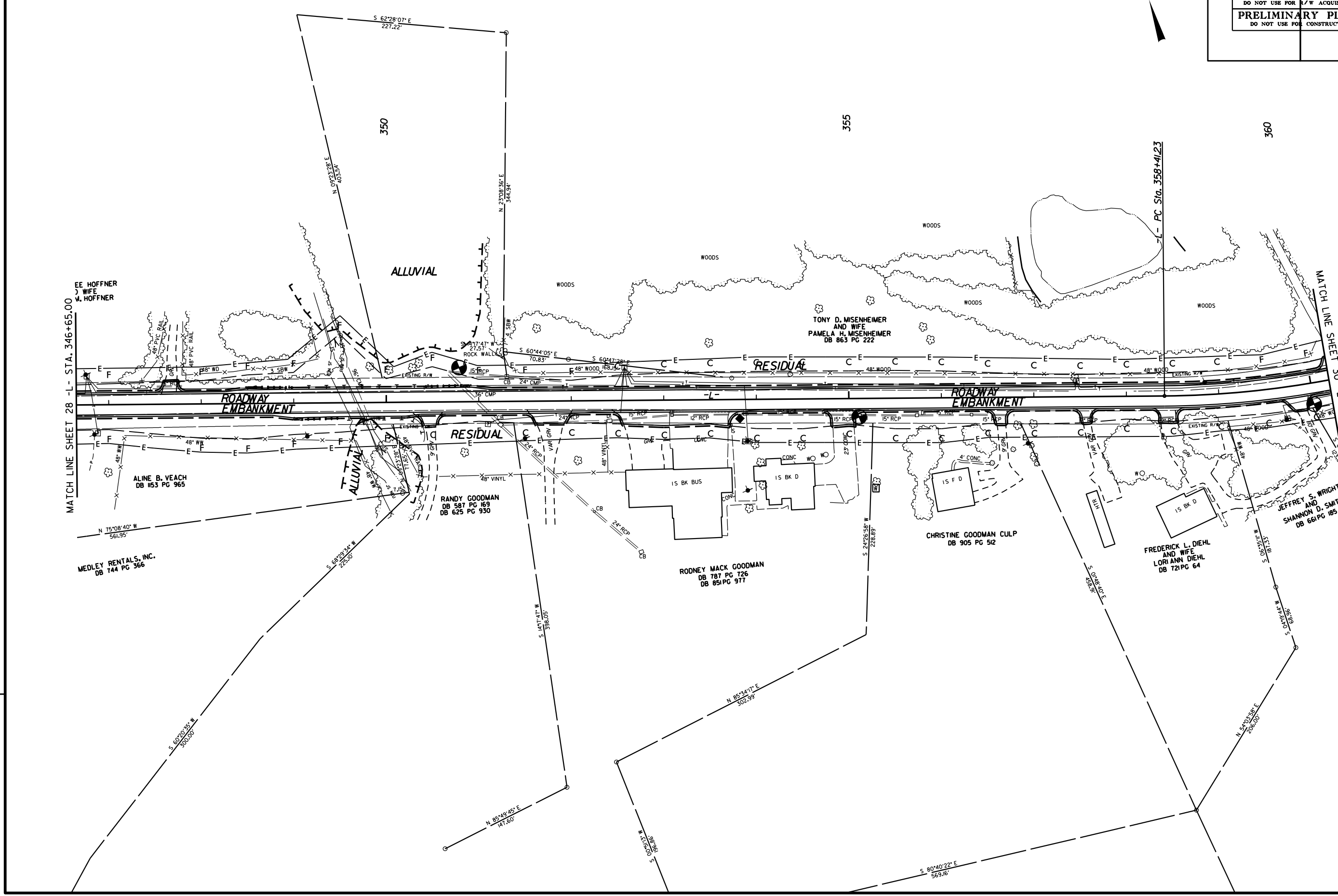
PROJECT REFERENCE NO. W-5313	SHEET NO. 29
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS

MATCH LINE SHEET 28 - L - STA. 346+65.00

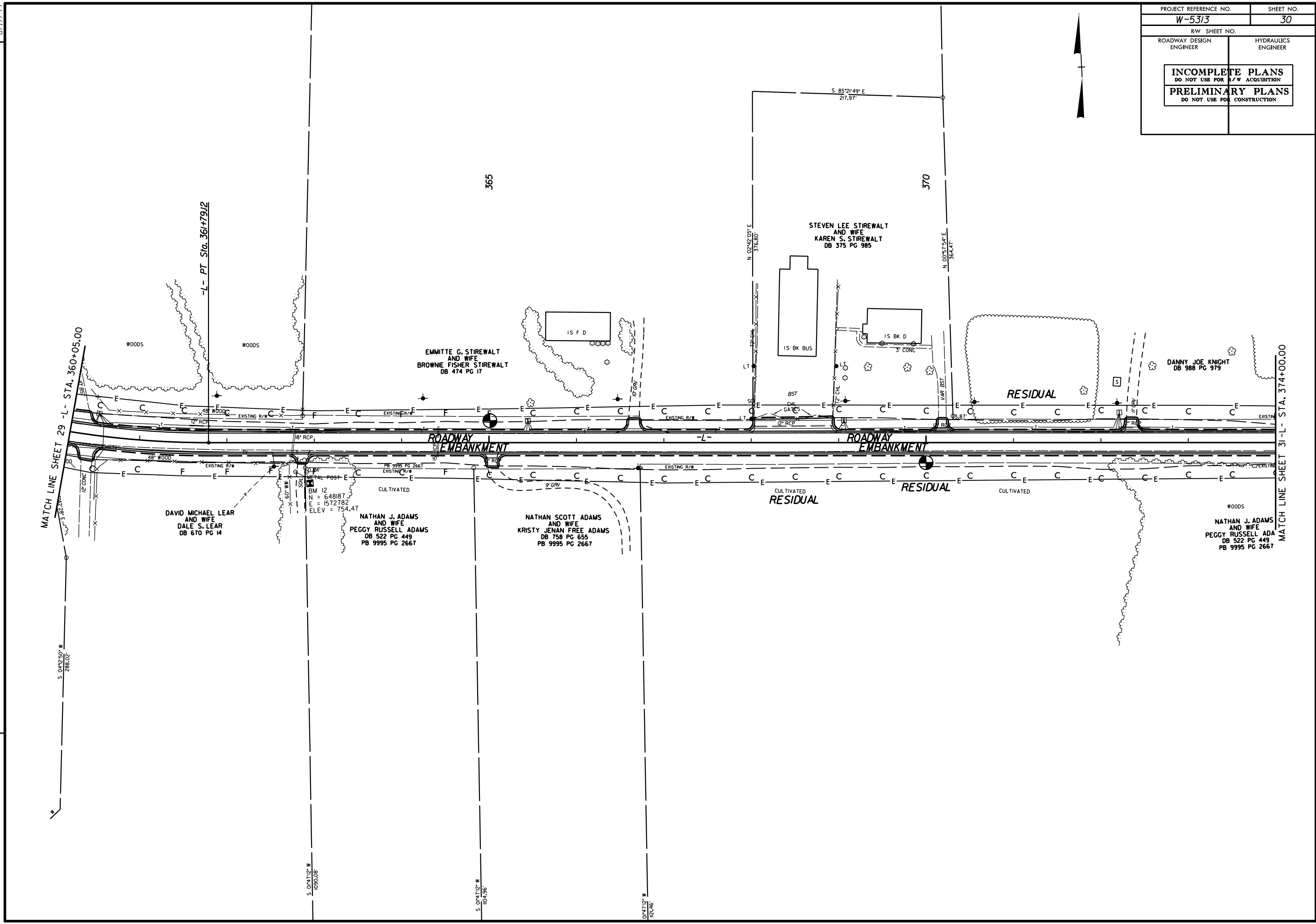
MATCH LINE SHEET 30 - L - STA. 360+20.00



8/17/99

REVISIONS

PROJECT REFERENCE NO. W-5313	SHEET NO. 30
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



MATCH LINE SHEET 29 -L- STA. 360+05.00

MATCH LINE SHEET 31 -L- STA. 374+00.00

S. 0°15'50" W
288.02'

S. 01°47'12" W
1090.08'

S. 01°47'12" W
104.96'

01°47'12" W
1121.46'

N. 02°45'05" E
376.80'

N. 00°57'54" E
364.47'

S. 85°21'49" E
217.97'

-L- PT Sta. 361+79.12

ROADWAY EMBANKMENT

ROADWAY EMBANKMENT

RESIDUAL

RESIDUAL

CULTIVATED
RESIDUAL

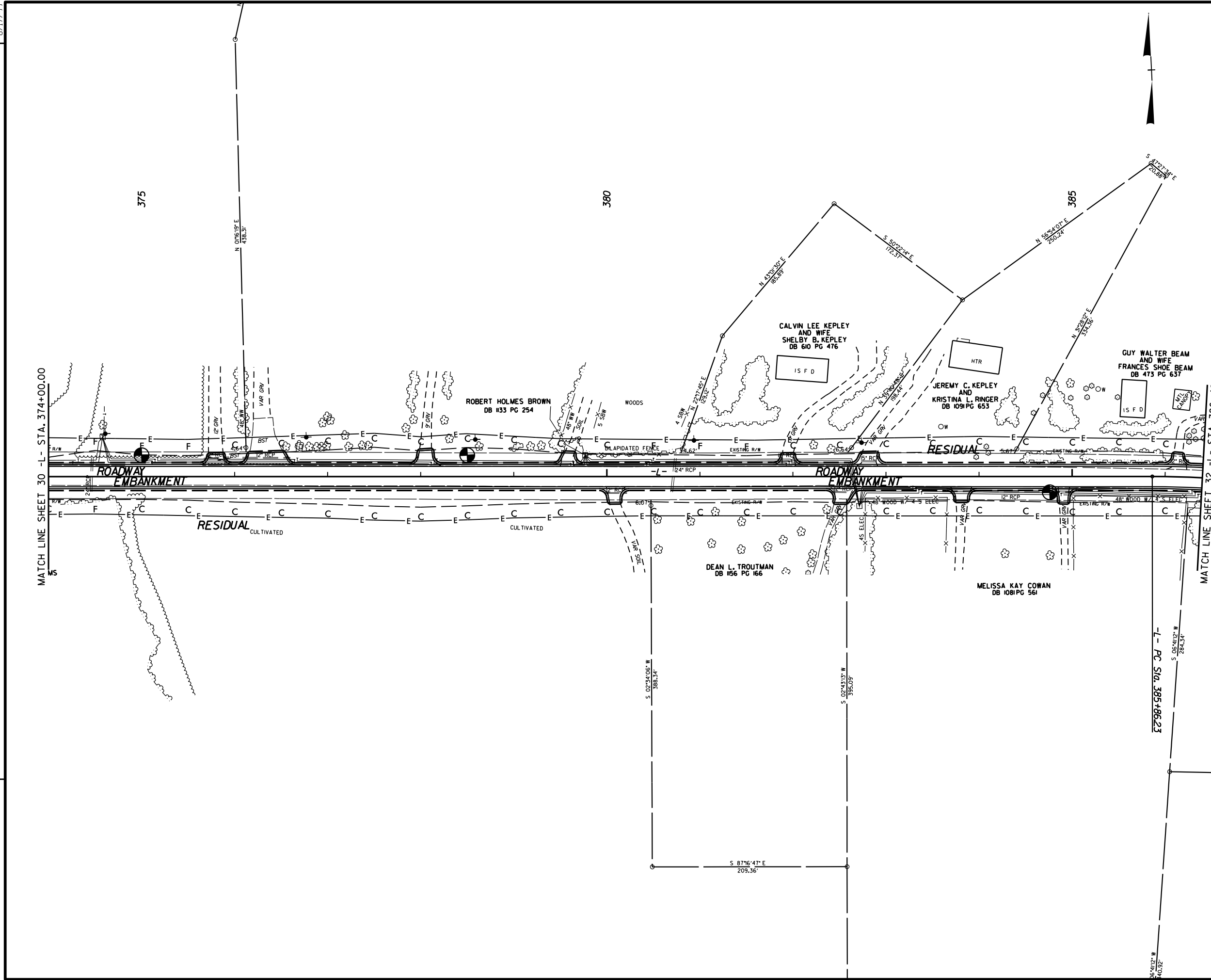
CULTIVATED

WOODS
NATHAN J. ADAMS AND WIFE
PEGGY RUSSELL ADA
DB 522 PG 449
PB 9995 PG 2667

PROJECT REFERENCE NO. W-5313	SHEET NO. 31
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

8/17/99

REVISIONS



MATCH LINE SHEET 30 -L- STA. 374+00.00

MATCH LINE SHEET 32 -L- STA. 386+40.00

-L- PC Sta. 385+86.23

375

380

385

S. 02°34'06" W
388.34'

S. 02°43'13" W
395.09'

S. 87°16'47" E
209.36'

S. 05°41'12" W
284.34'

S. 47°12' W
40.32'

N. 01°16'09" E
438.31'

S. 02°34'06" W
388.34'

S. 02°43'13" W
395.09'

S. 87°16'47" E
209.36'

S. 05°41'12" W
284.34'

S. 47°12' W
40.32'

N. 01°16'09" E
438.31'

S. 02°34'06" W
388.34'

S. 02°43'13" W
395.09'

S. 87°16'47" E
209.36'

S. 05°41'12" W
284.34'

S. 47°12' W
40.32'

N. 01°16'09" E
438.31'

S. 02°34'06" W
388.34'

S. 02°43'13" W
395.09'

S. 87°16'47" E
209.36'

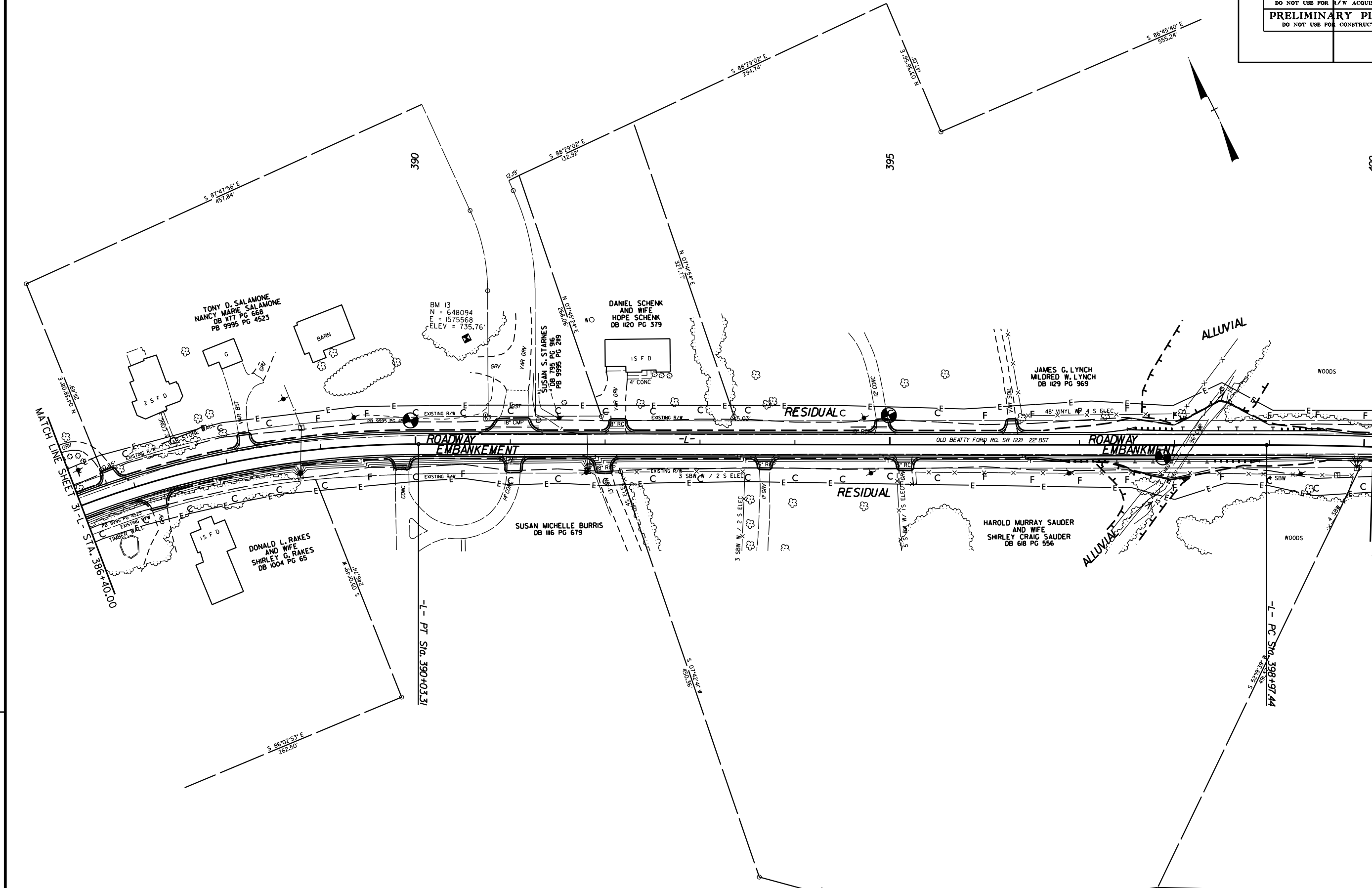
S. 05°41'12" W
284.34'

S. 47°12' W
40.32'

PROJECT REFERENCE NO.	SHEET NO.
W-5313	32
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

8/17/99

REVISIONS



MATCH LINE SHEET 31 - L - STA. 386+10.00

MATCH LINE SHEET 33 - L - STA. 400+10.00

-L- PT Sta. 390+03.31

-L- PC Sta. 398+97.44

OLD BEATTY FORD RD. SR 1221 22' BST

ALLUVIAL

RESIDUAL

RESIDUAL

ROADWAY EMBANKMENT

ROADWAY EMBANKMENT

TONY D. SALAMONE
NANCY MARIE SALAMONE
DB 177 PG 668
PB 9995 PG 4523

DANIEL SCHENK
AND WIFE
HOPE SCHENK
DB 120 PG 379

SUSAN S. STARNES
DB 795 PG 916
PB 9995 PG 219

SUSAN MICHELLE BURRIS
DB 116 PG 679

DONALD L. RAKES
AND WIFE
SHIRLEY G. RAKES
DB 104 PG 65

JAMES G. LYNCH
MILDRED W. LYNCH
DB 129 PG 969

HAROLD MURRAY SAUDER
AND WIFE
SHIRLEY CRAIG SAUDER
DB 618 PG 556

BM 13
N = 648094
E = 1575568
ELEV = 735.76'

S 86°02'53" E
262.50'

S 88°29'02" E
294.74'

S 86°45'40" E
555.24'

S 48°19'43" E
173.16'

S 71°03'07" E
255.05'

390

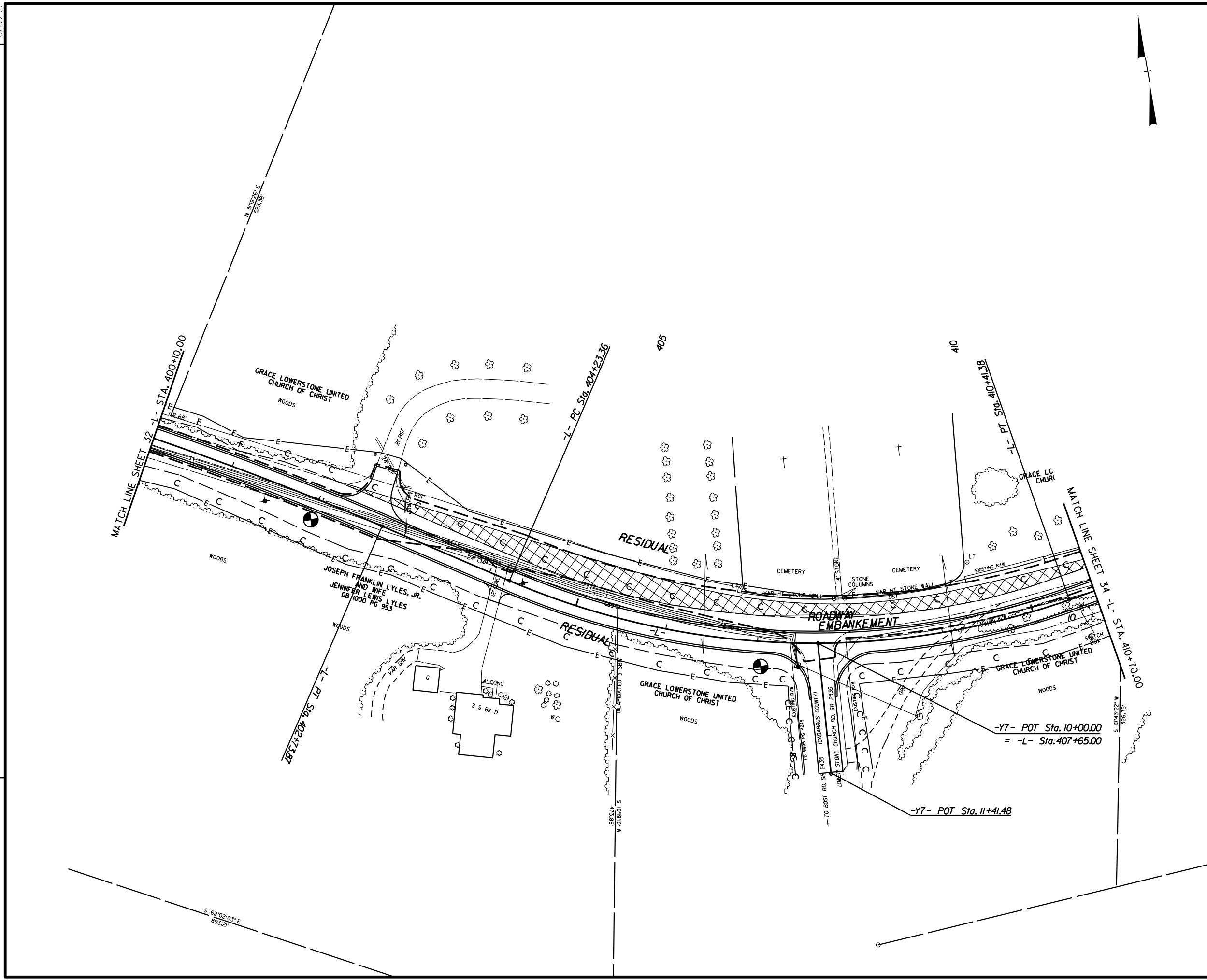
395

400

8/17/99

REVISIONS

PROJECT REFERENCE NO. W-5313	SHEET NO. 33
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



S 62°02'03" E 893.21'

S 10°43'22" W 326.75'

-Y7- POT Sta. 11+41.48

-Y7- POT Sta. 10+00.00 = -L- Sta. 407+65.00

MATCH LINE SHEET 32 -L- STA. 400+10.00

MATCH LINE SHEET 34 -L- STA. 410+10.00

GRACE LOWERSTONE UNITED CHURCH OF CHRIST

GRACE LC CHURCH

JOSEPH FRANKLIN LYLES, JR. AND WIFE JENNIFER LEWIS LYLES DB 1000 PC 953

GRACE LOWERSTONE UNITED CHURCH OF CHRIST

GRACE LOWERSTONE UNITED CHURCH OF CHRIST

RESIDUAL

RESIDUAL

ROADWAY EMBANKMENT

CEMETERY

CEMETERY

WOODS

WOODS

WOODS

WOODS

WOODS

WOODS

UNL. IMPROVED CURB

TO BRST RD. SR 2435 (CABARRUS COUNTY)

STONE CHURCH RD. SR 2335

TO BRST RD. SR 4843

EXISTING R/W

EXISTING R/W

EXISTING R/W

EXISTING R/W

EXISTING R/W

EXISTING R/W

EXISTING R/W

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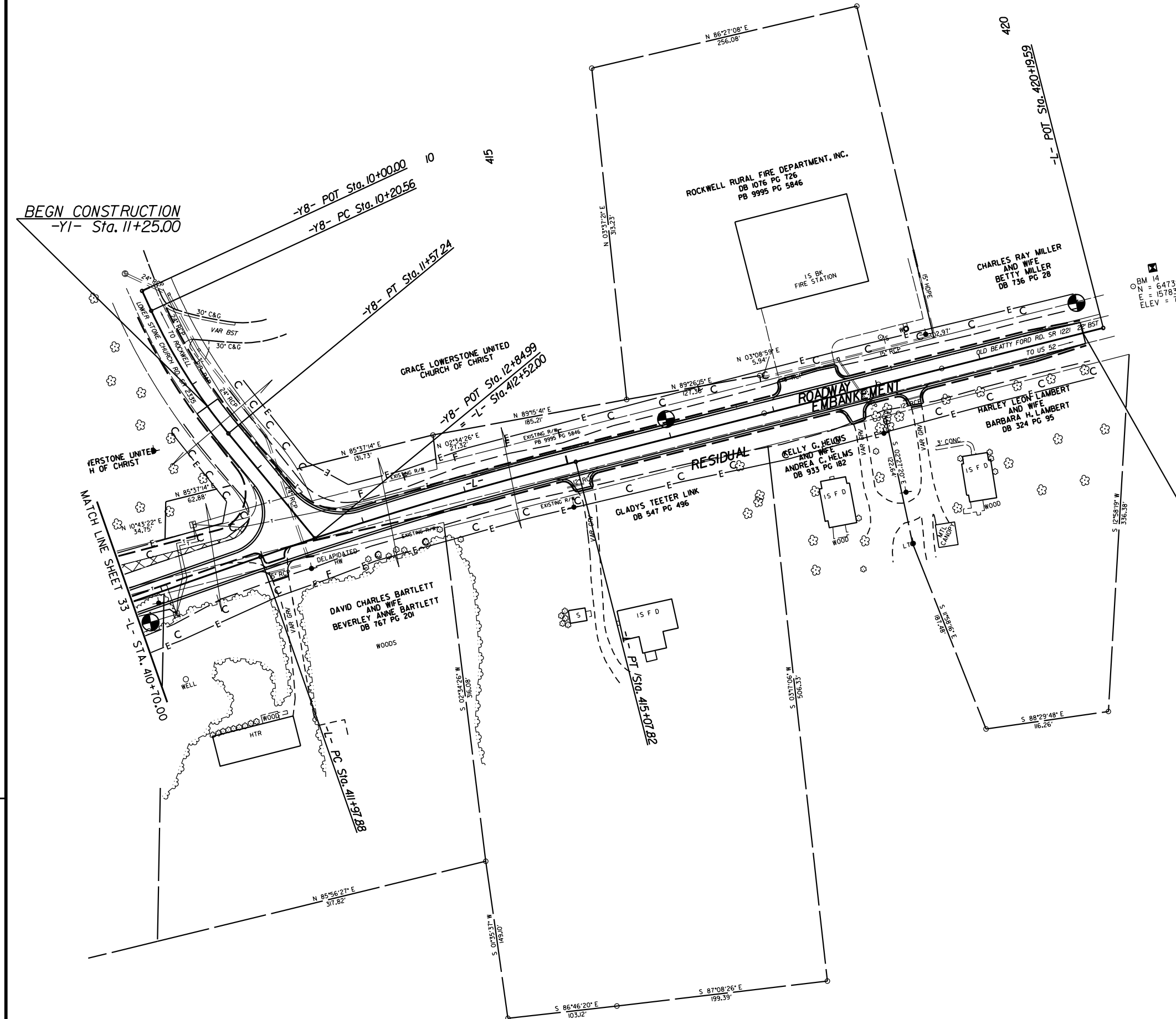
EXISTING R/W

EXISTING R/W

8/17/99

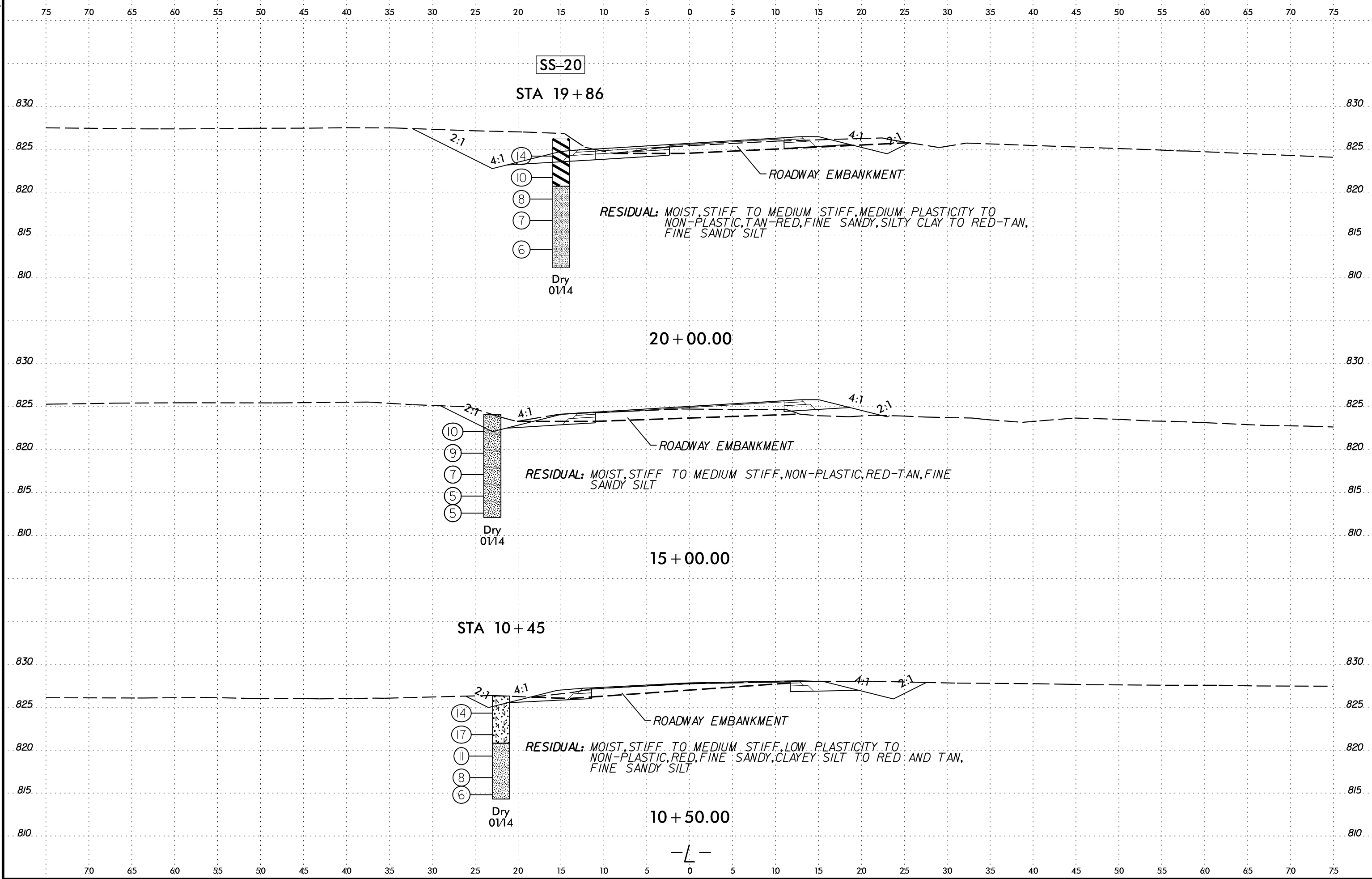
REVISIONS

PROJECT REFERENCE NO. W-5313	SHEET NO. 34
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

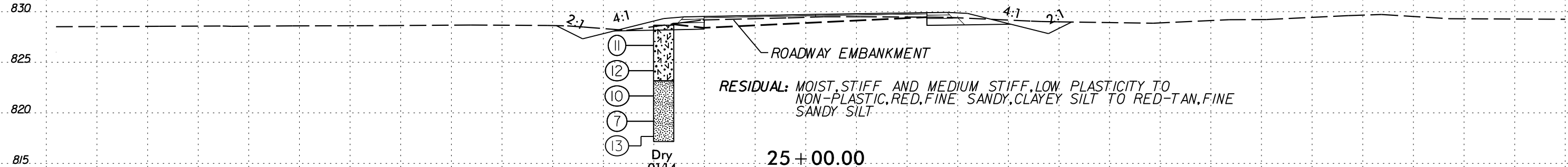


BM 14
 N = 647380
 E = 1578343
 ELEV = 758.84'

END TIP PROJECT W-5313
 END CONSTRUCTION
 -L- POT Sta. 420+00.00



75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75



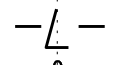
STA 24+89

ROADWAY EMBANKMENT

RESIDUAL: MOIST, STIFF AND MEDIUM STIFF, LOW PLASTICITY TO NON-PLASTIC, RED, FINE SANDY, CLAYEY SILT TO RED-TAN, FINE SANDY SILT

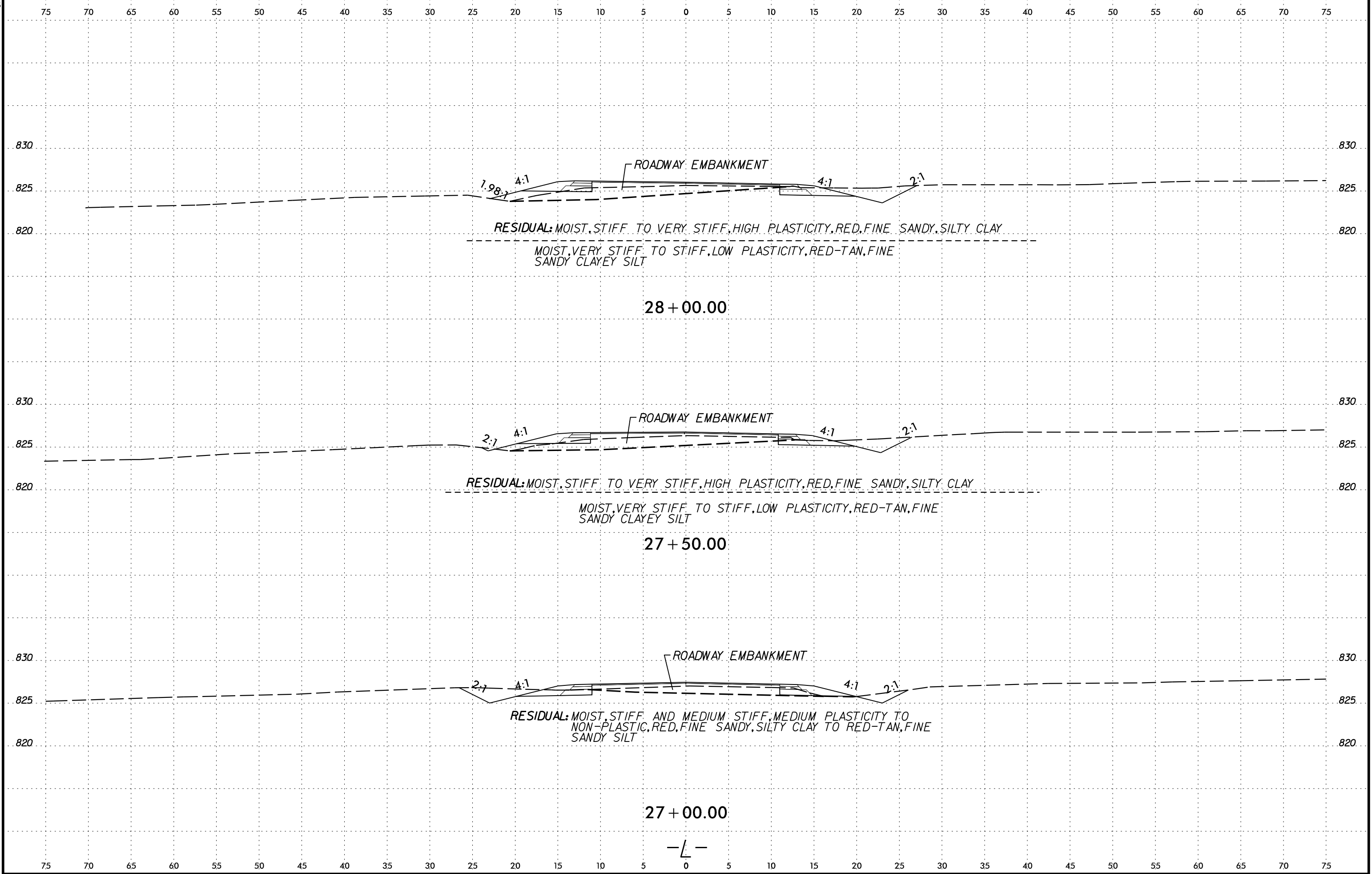
25+00.00

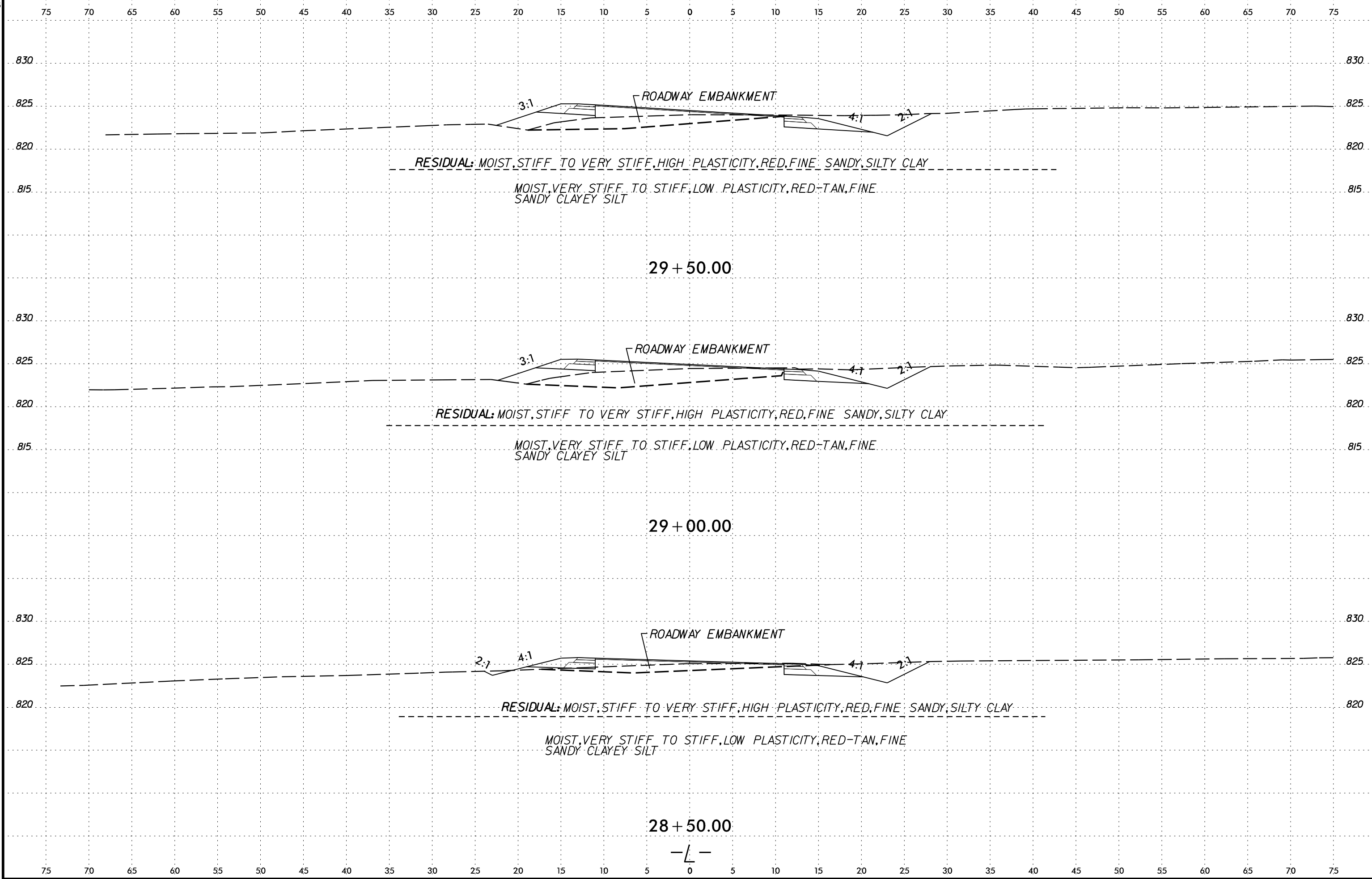
Dry 01/14

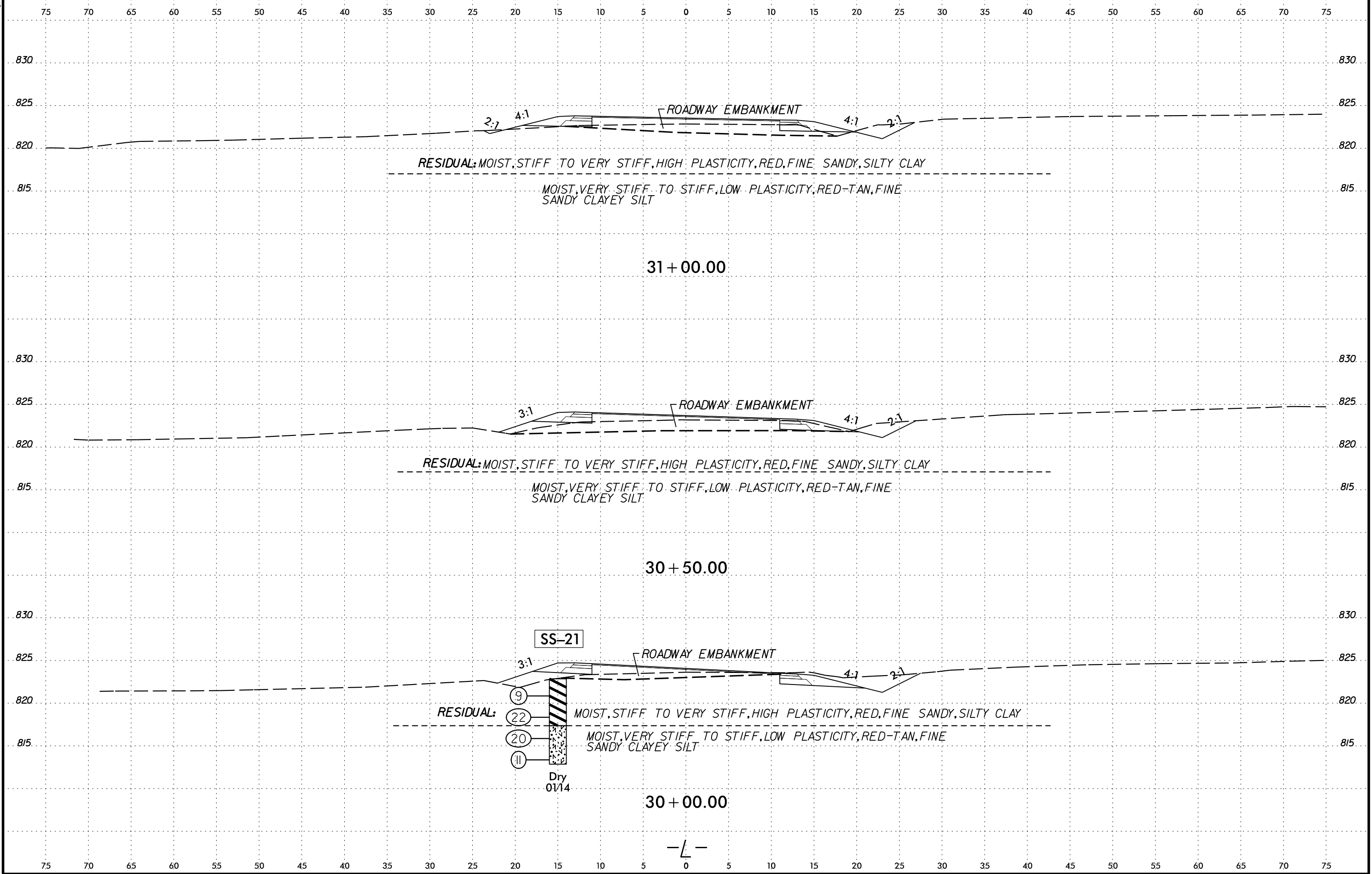


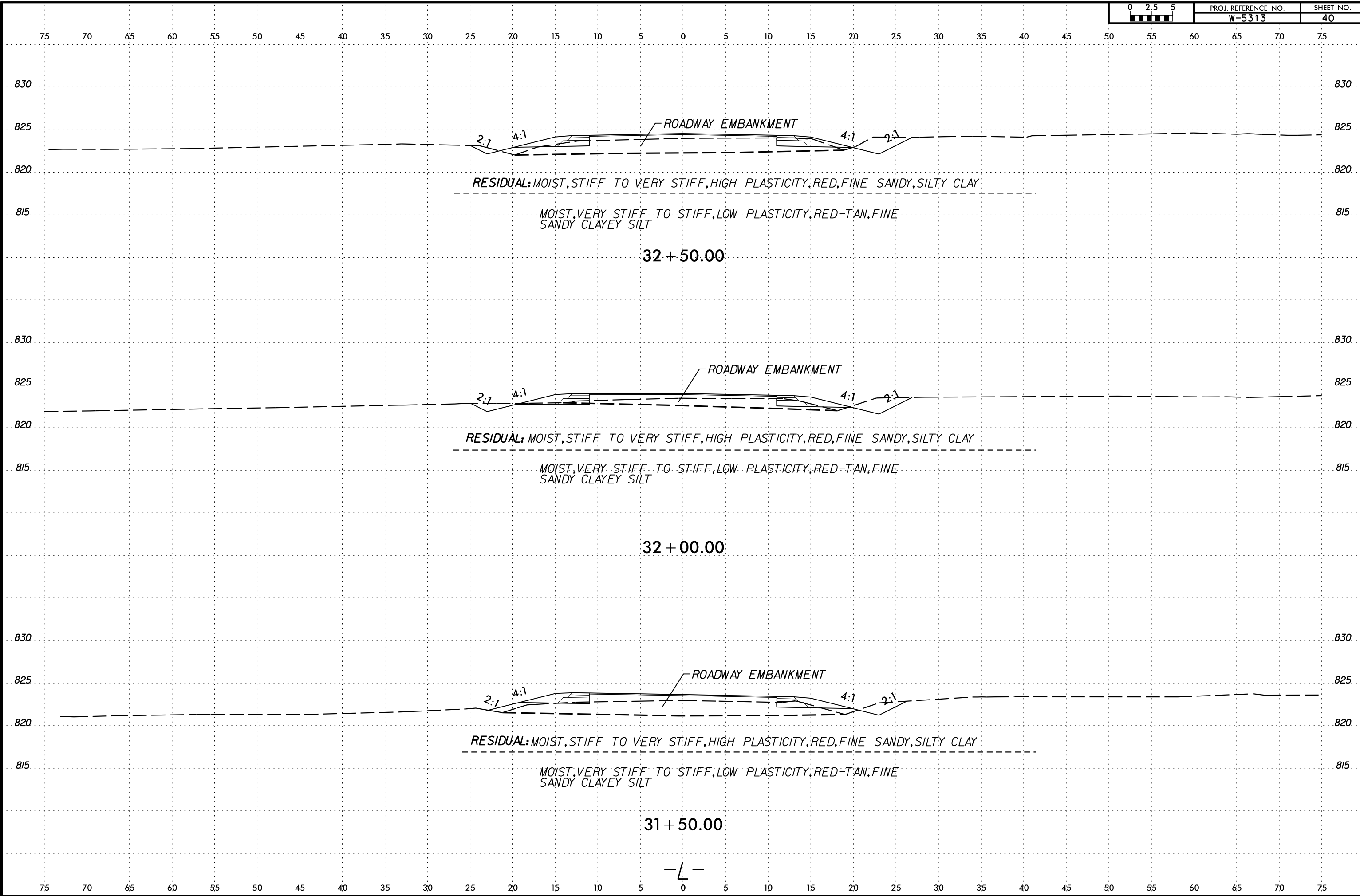
75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

830
825
820
815

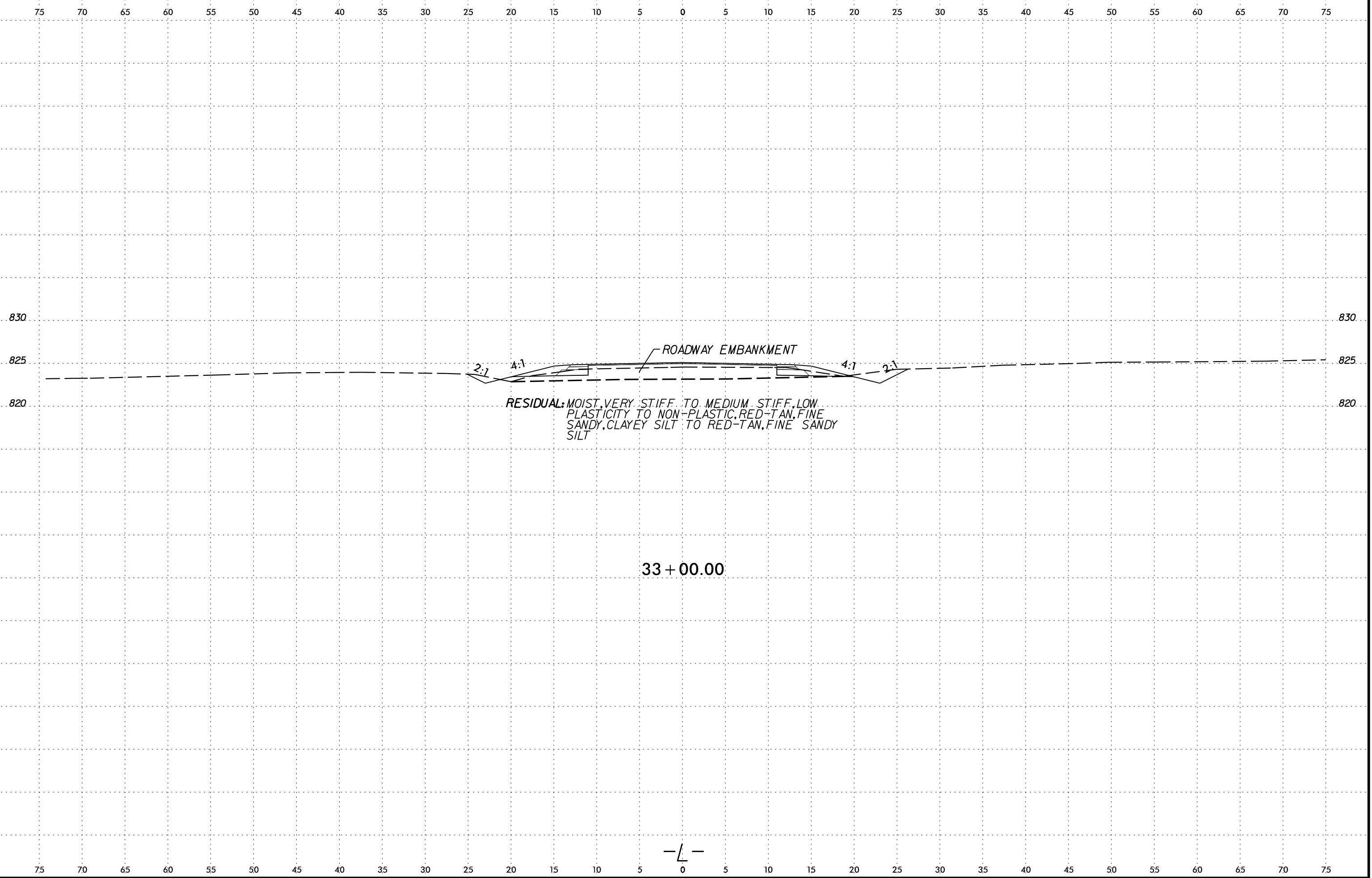








8/23/99



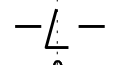
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830 825 820

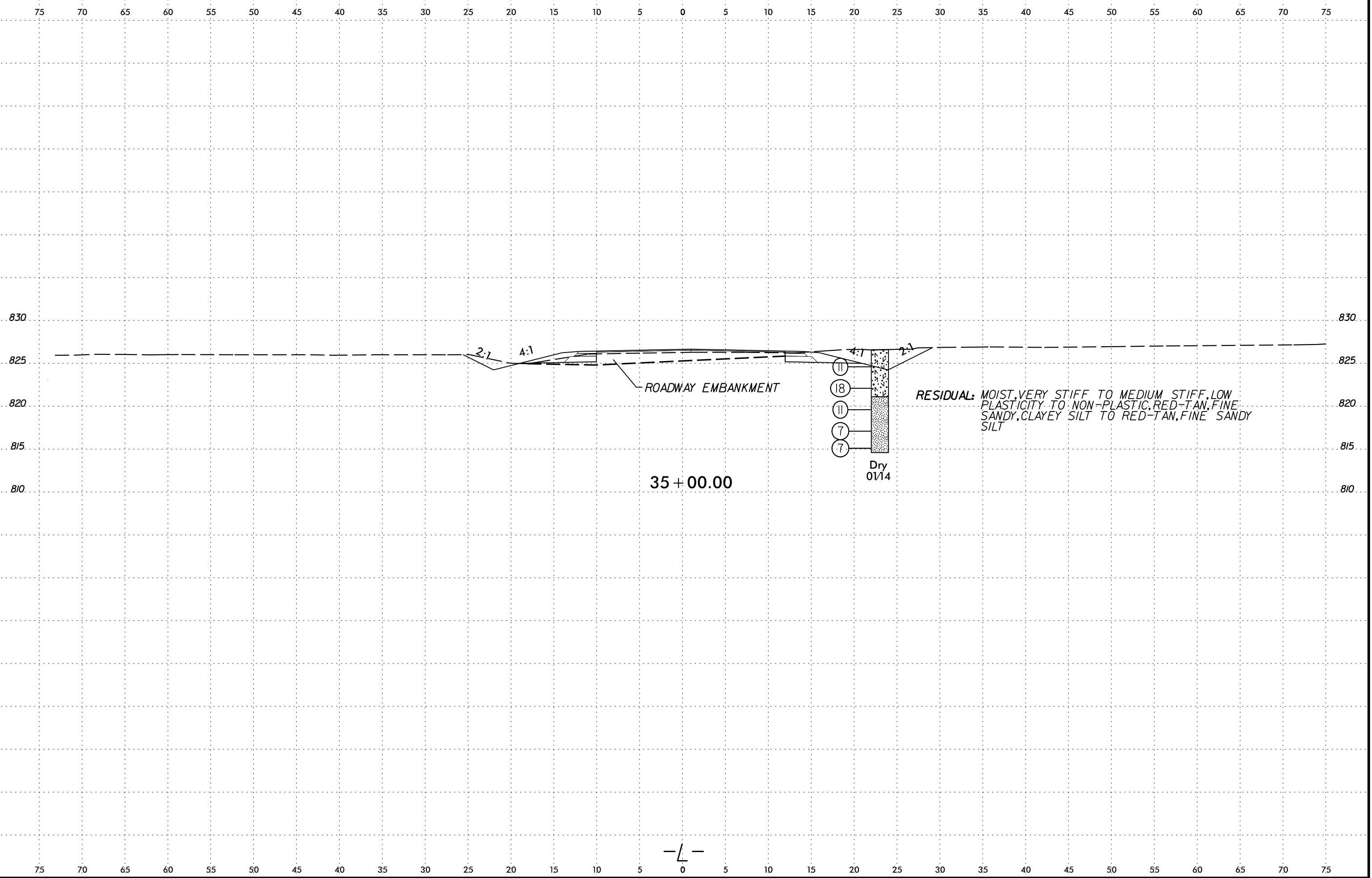
33 + 00.00

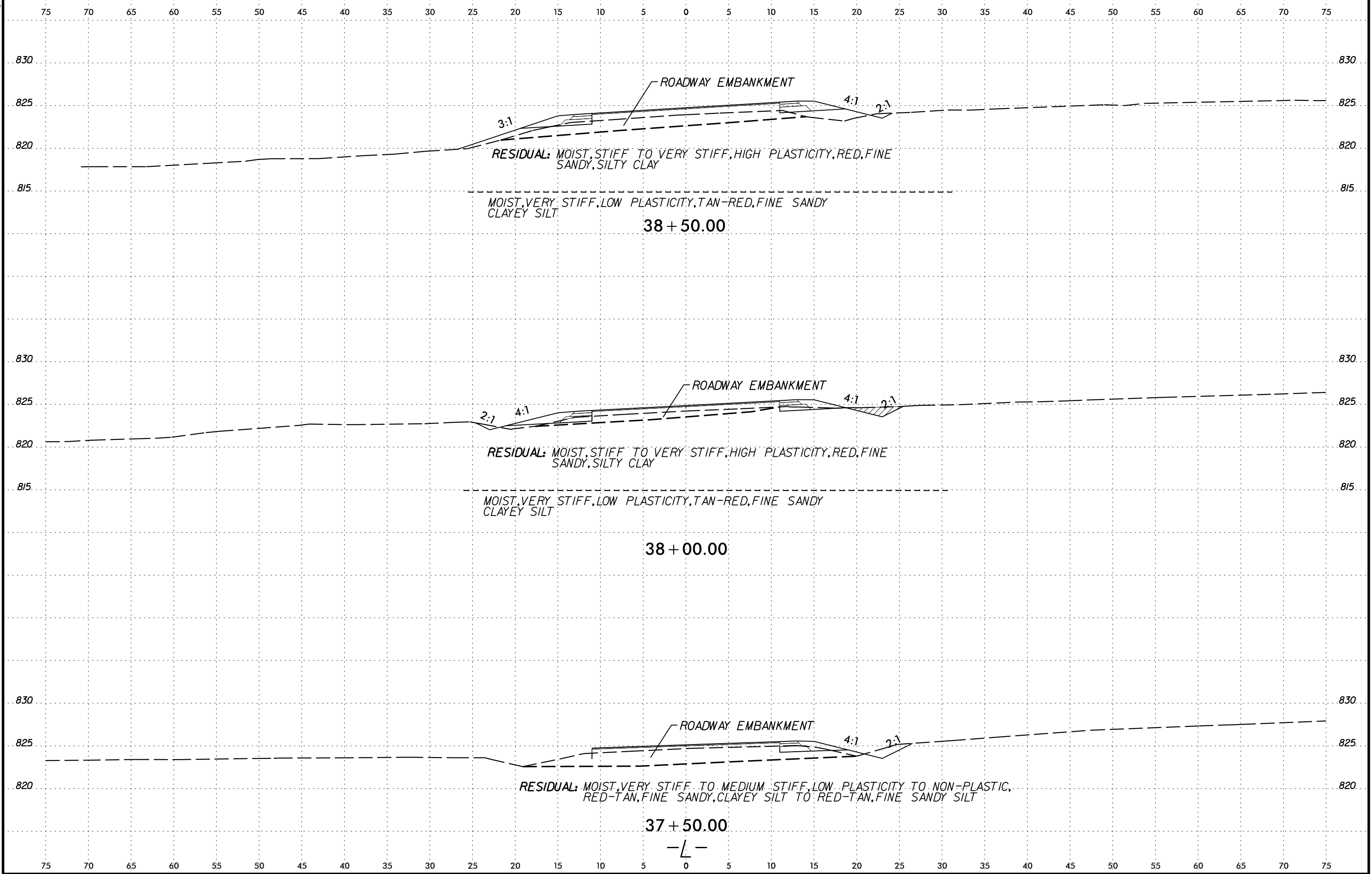
ROADWAY EMBANKMENT

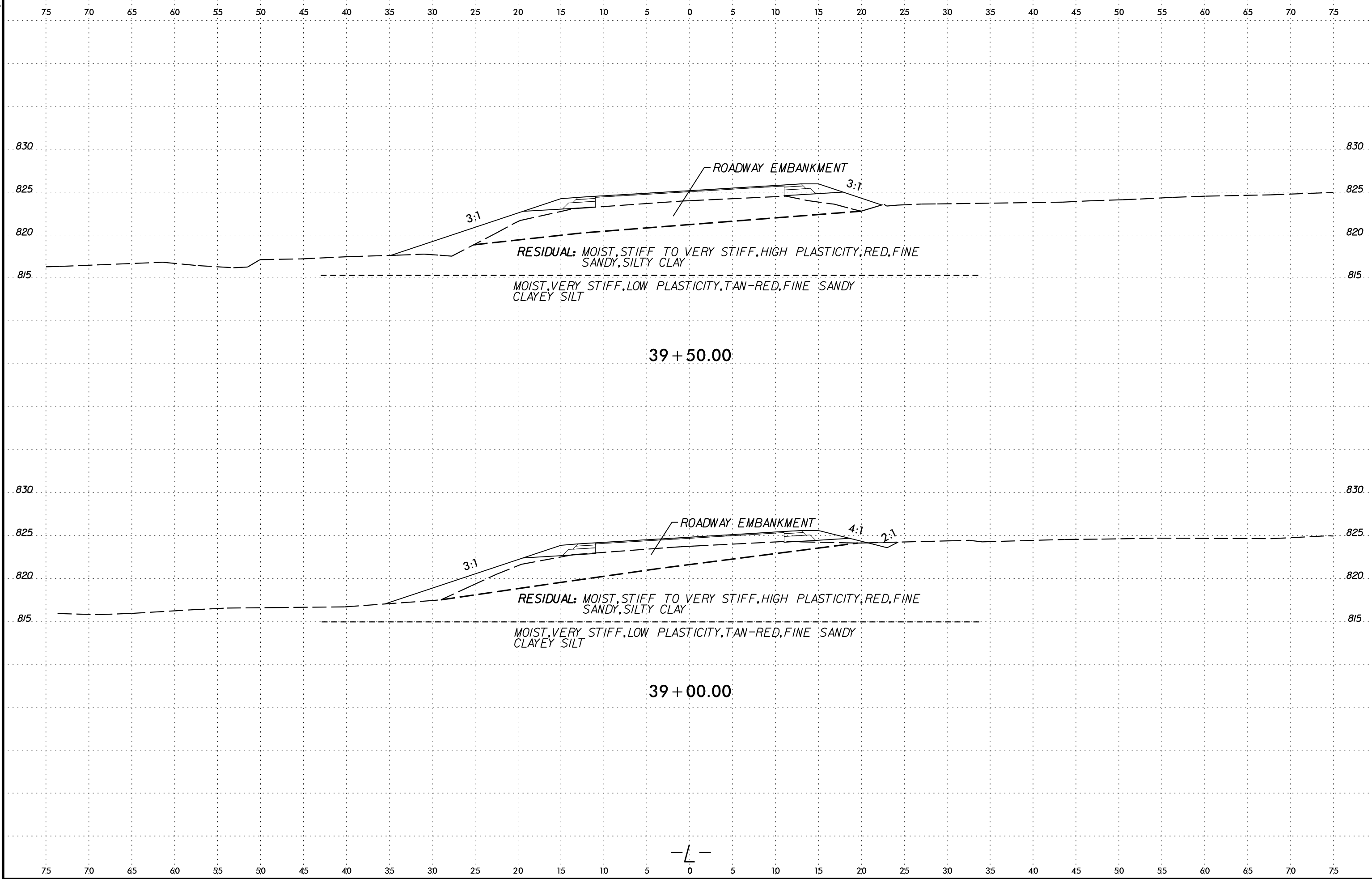
RESIDUAL: MOIST, VERY STIFF TO MEDIUM STIFF, LOW PLASTICITY TO NON-PLASTIC, RED-TAN, FINE SANDY, CLAYEY SILT TO RED-TAN, FINE SANDY SILT

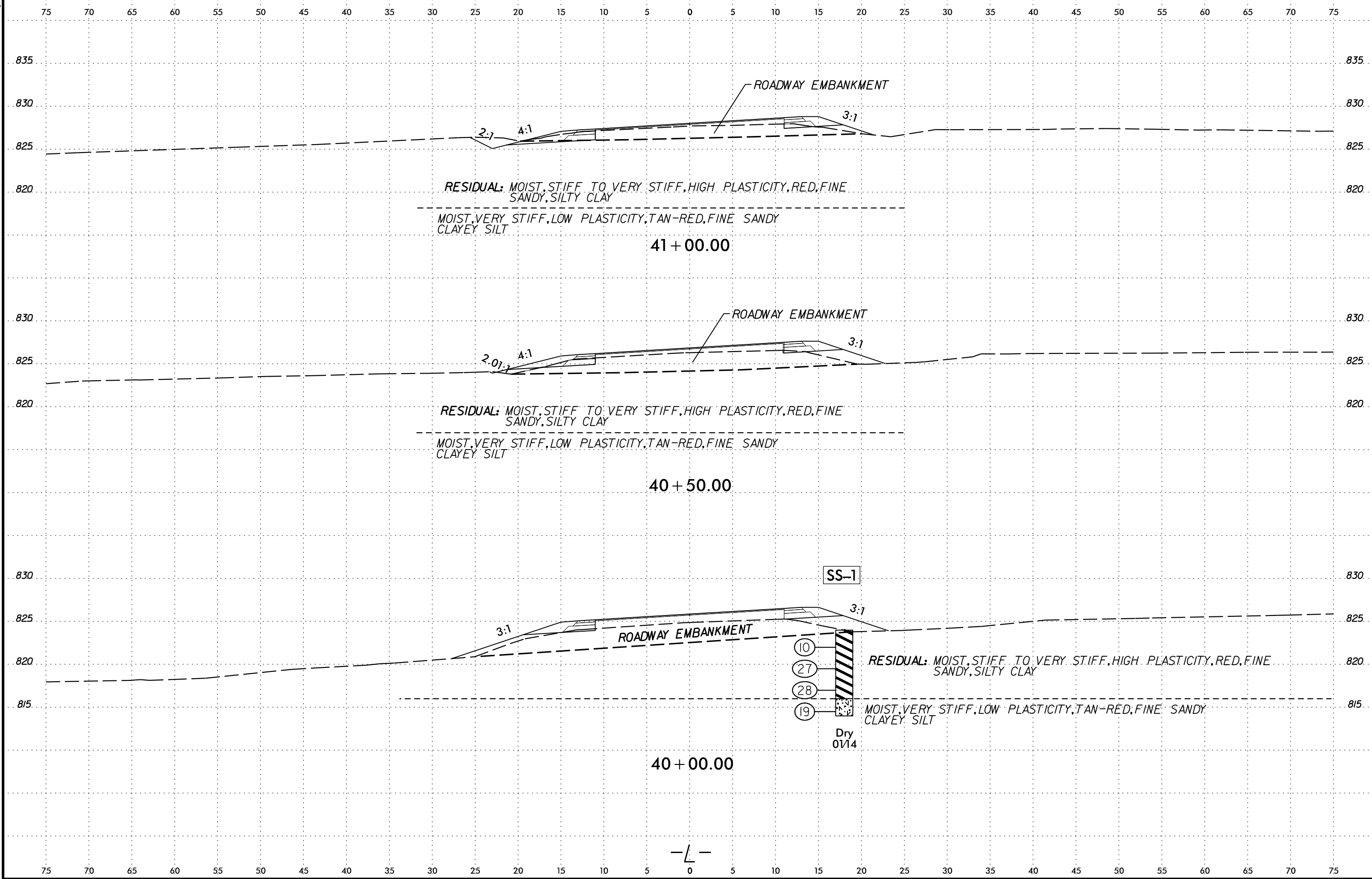


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41 + 00.00

40 + 50.00

40 + 00.00

SS-1

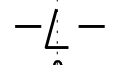
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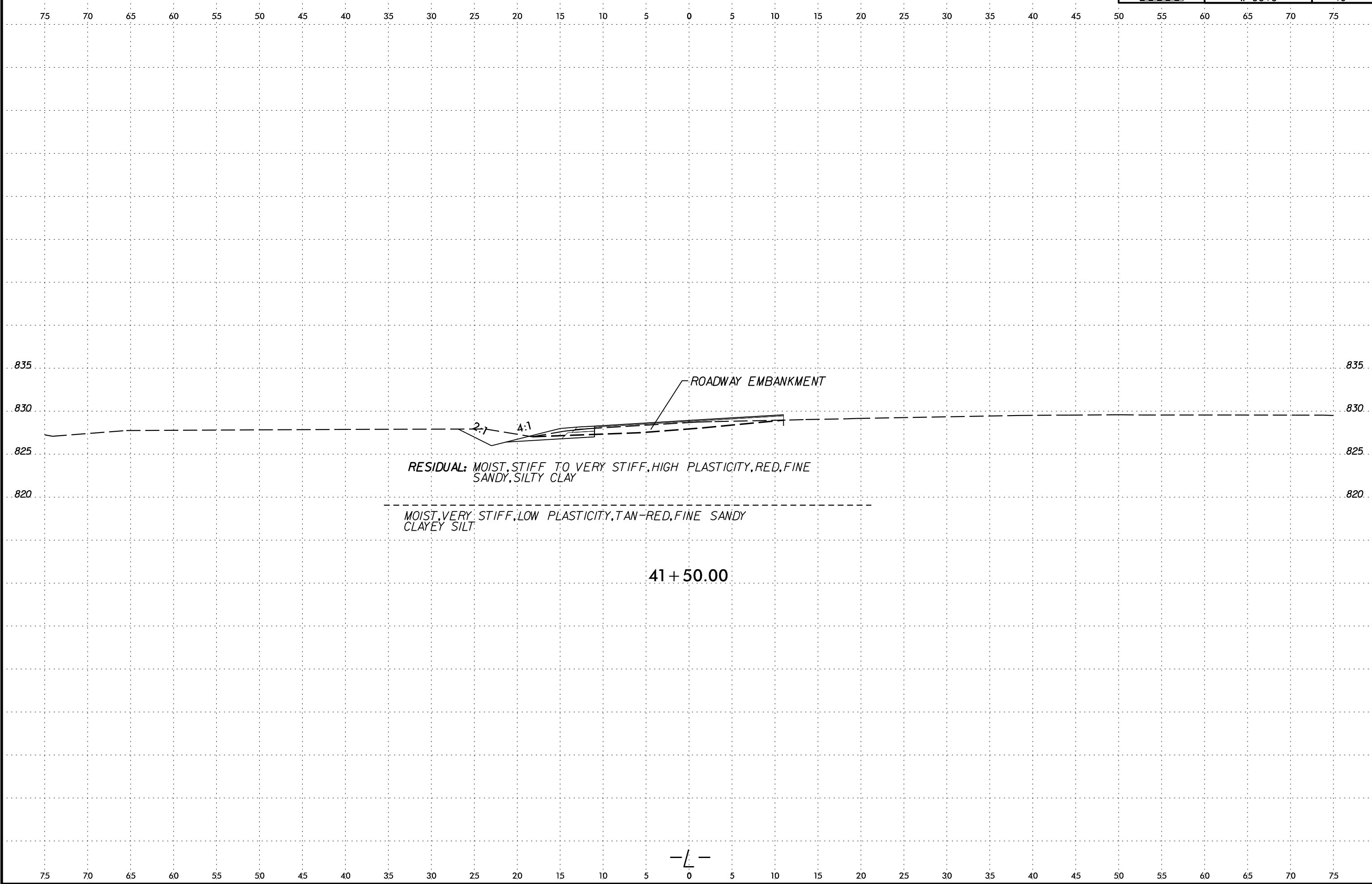
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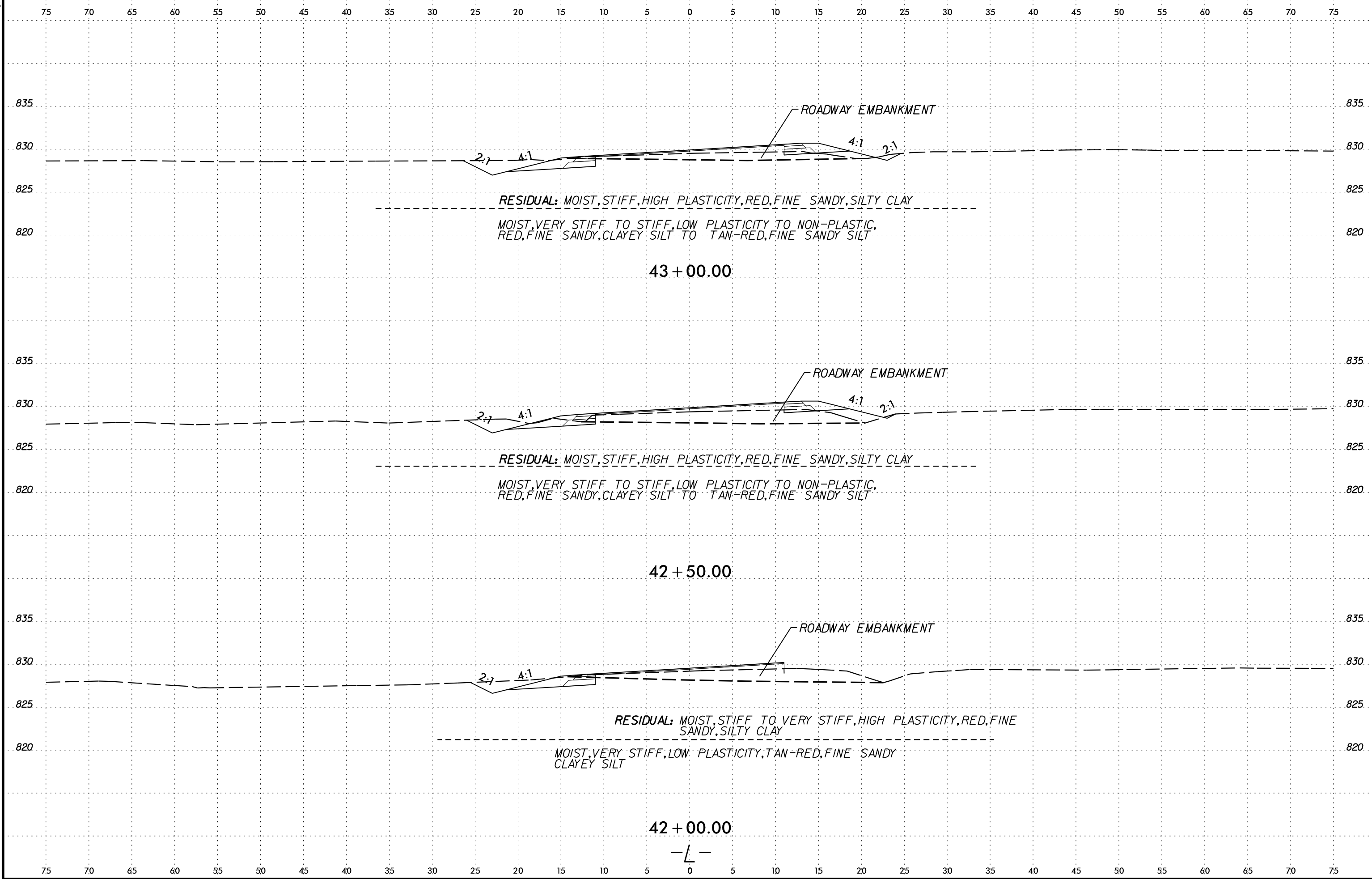
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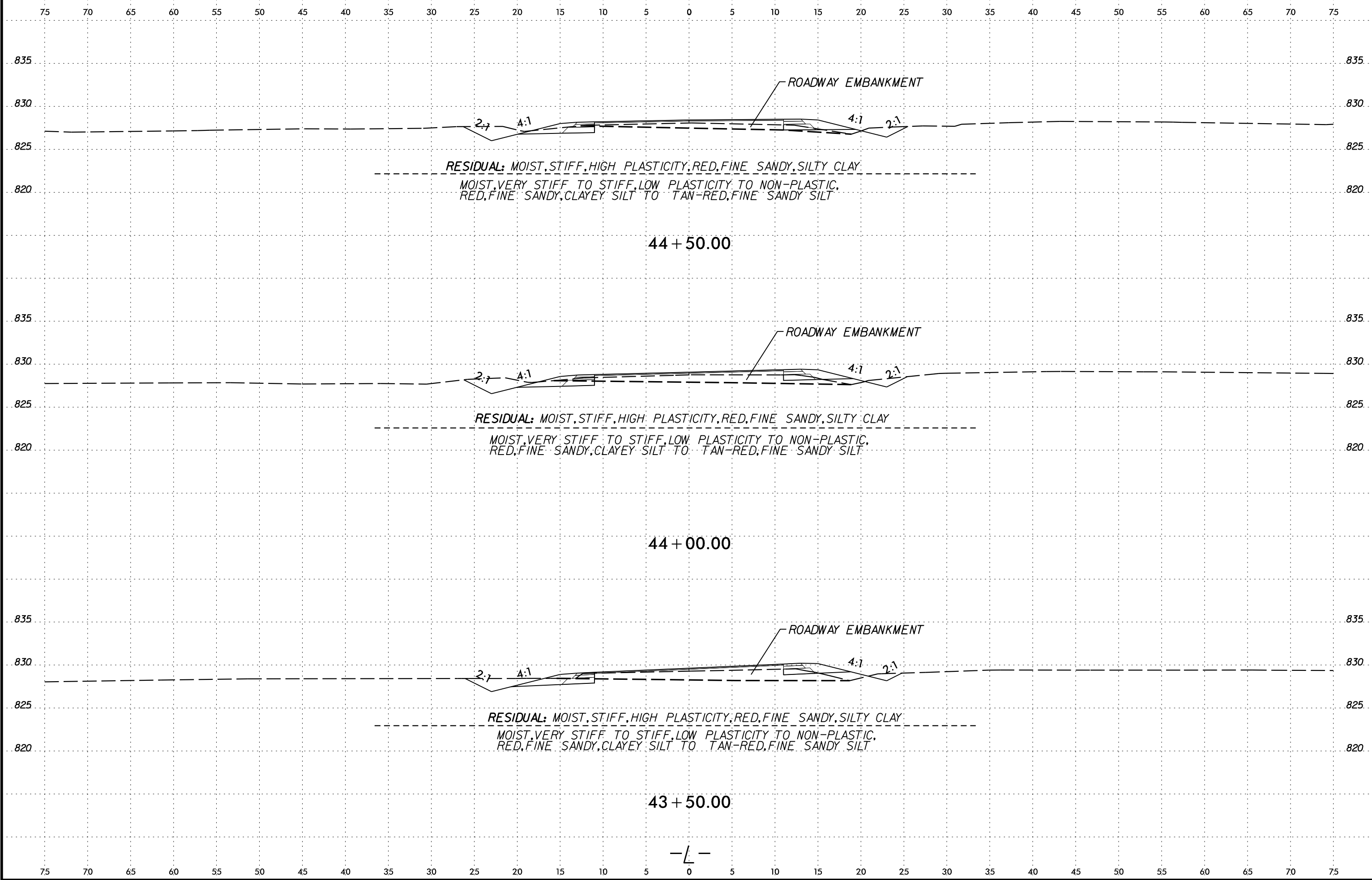
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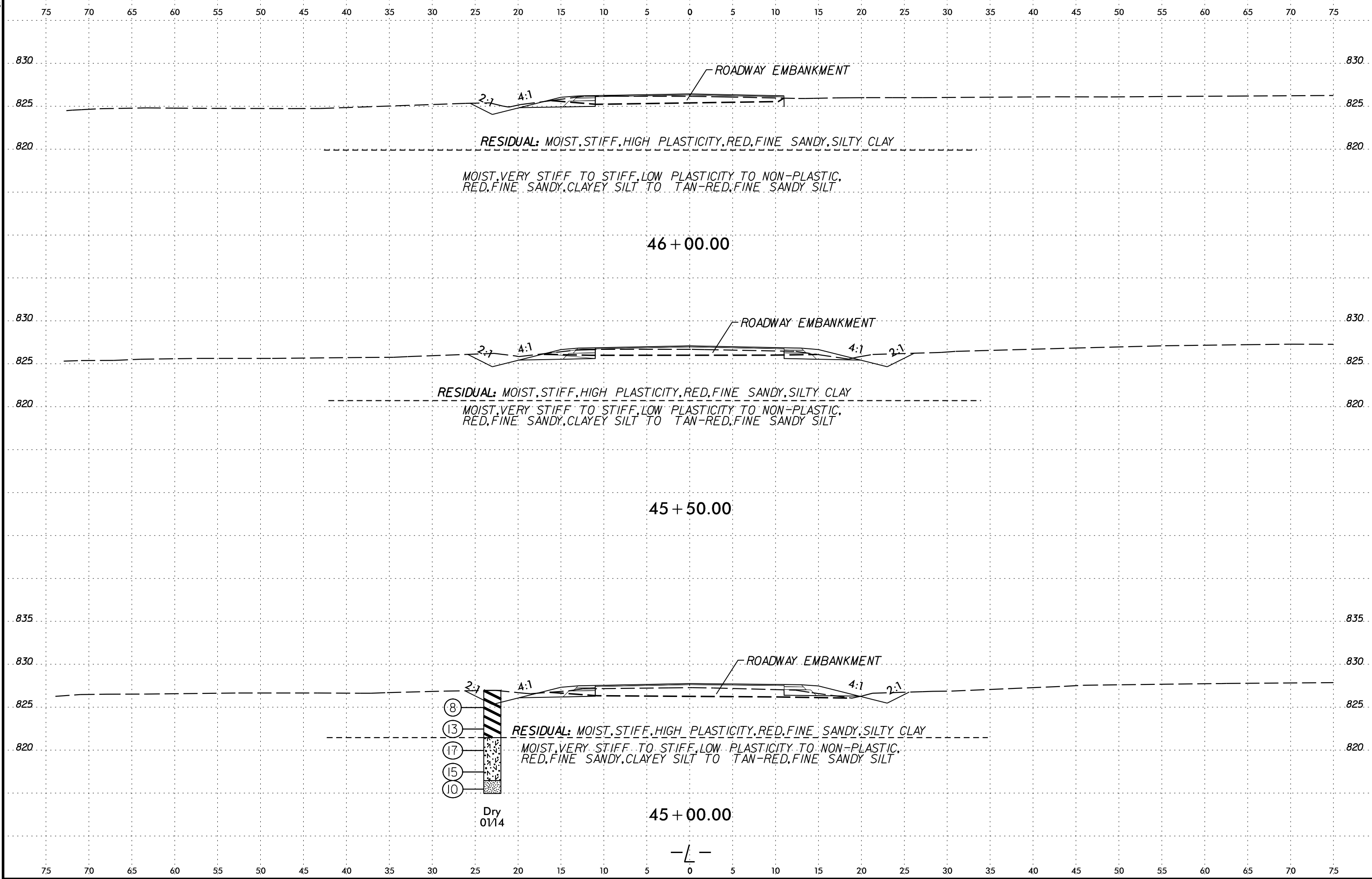
Dry 01/14

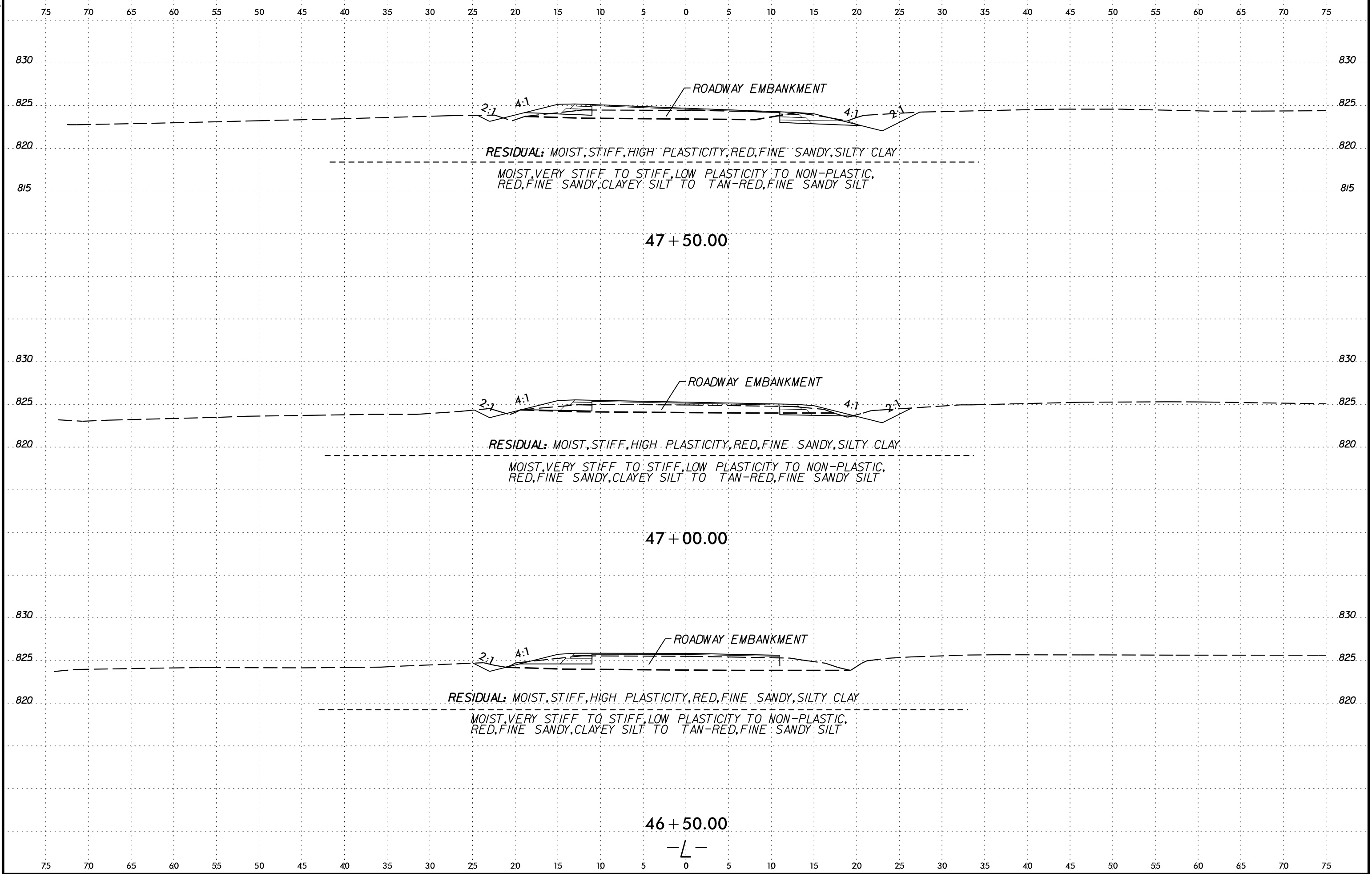




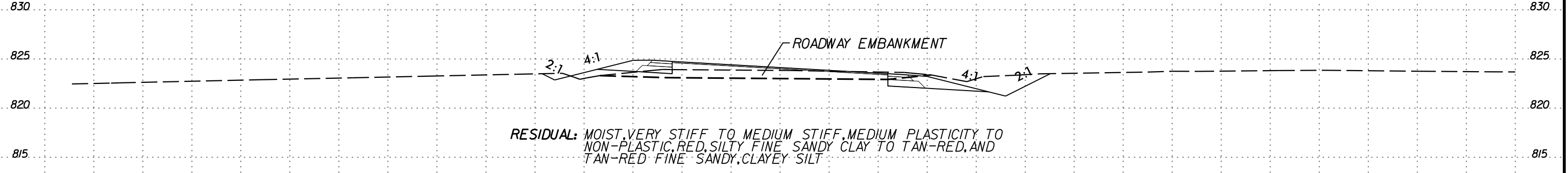




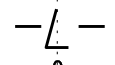




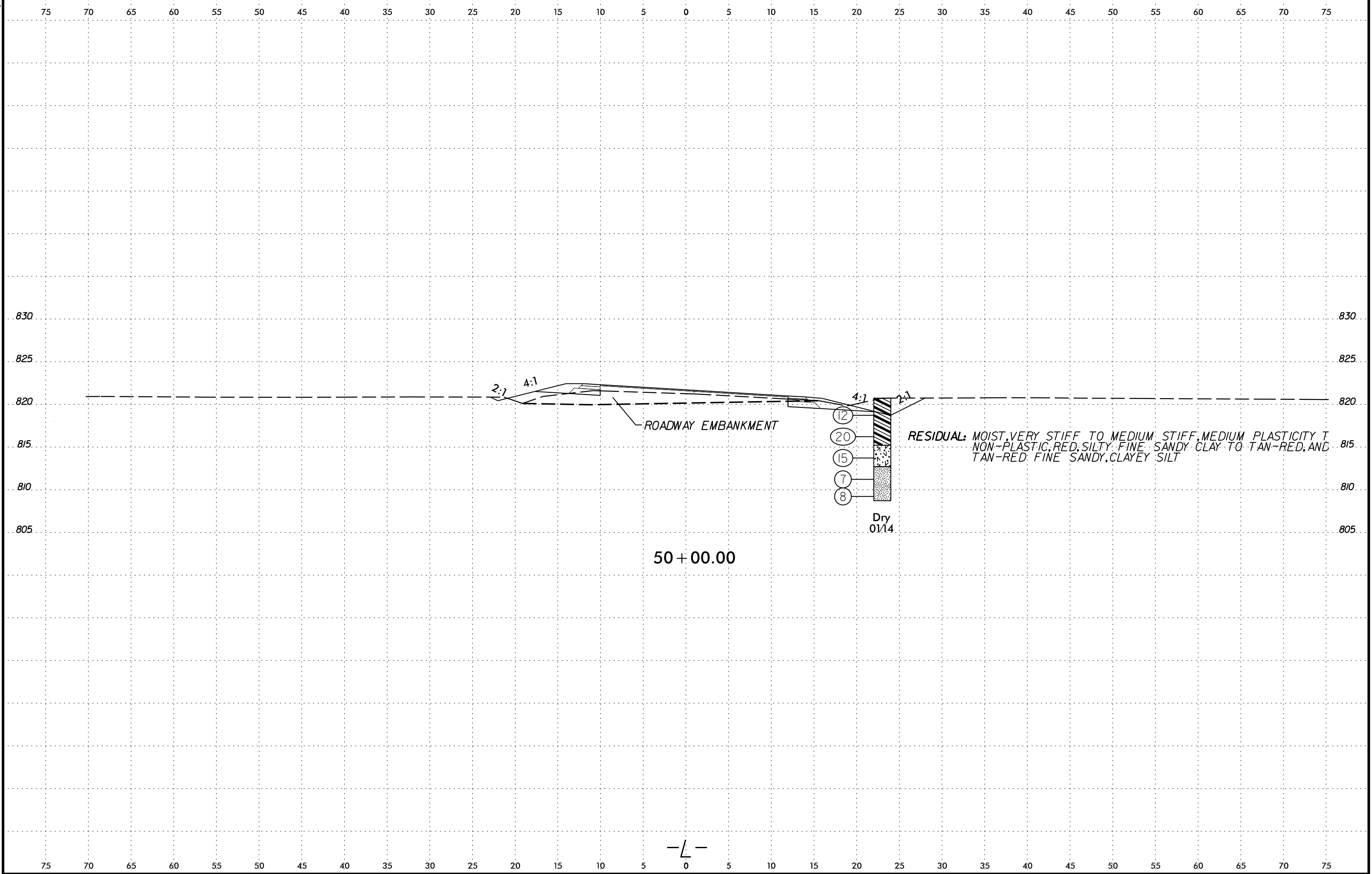
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48 + 00.00

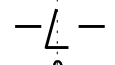


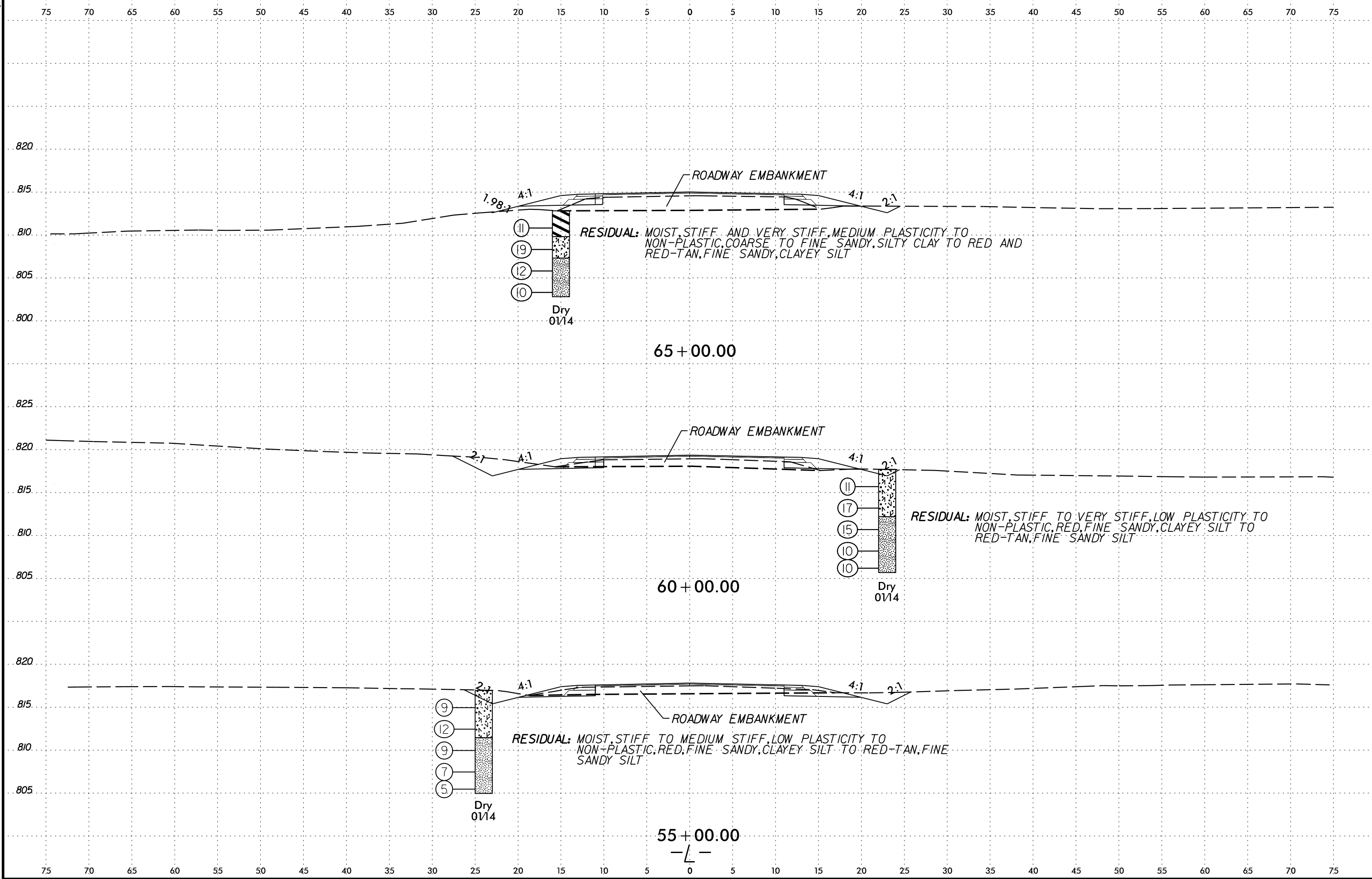
75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75



RESIDUAL: MOIST, VERY STIFF TO MEDIUM STIFF, MEDIUM PLASTICITY TO
 NON-PLASTIC; RED, SILTY FINE SANDY CLAY TO TAN-RED, AND
 TAN-RED, FINE SANDY, CLAYEY SILT

50+00.00





1.98.2

4:1

4:1

2:1

- 11
- 19
- 12
- 10

Dry 01/14

RESIDUAL: MOIST, STIFF, AND VERY STIFF, MEDIUM PLASTICITY TO NON-PLASTIC, COARSE TO FINE SANDY, SILTY CLAY TO RED AND RED-TAN, FINE SANDY, CLAYEY SILT

65 + 00.00

2:1

4:1

4:1

2:1

- 11
- 17
- 15
- 10
- 10

Dry 01/14

RESIDUAL: MOIST, STIFF TO VERY STIFF, LOW PLASTICITY TO NON-PLASTIC, RED, FINE SANDY, CLAYEY SILT TO RED-TAN, FINE SANDY SILT

60 + 00.00

2:1

4:1

4:1

2:1

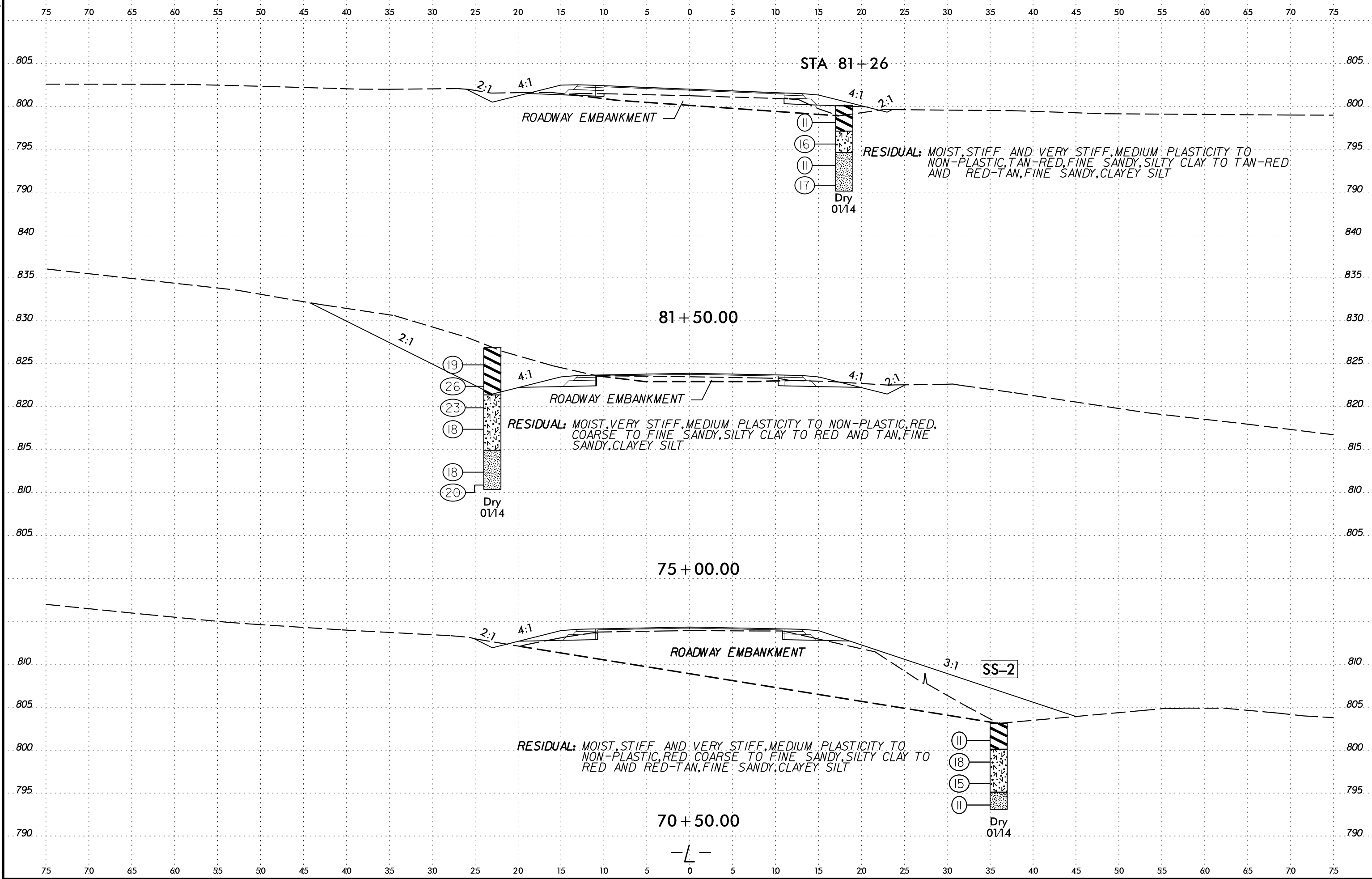
- 9
- 12
- 9
- 7
- 5

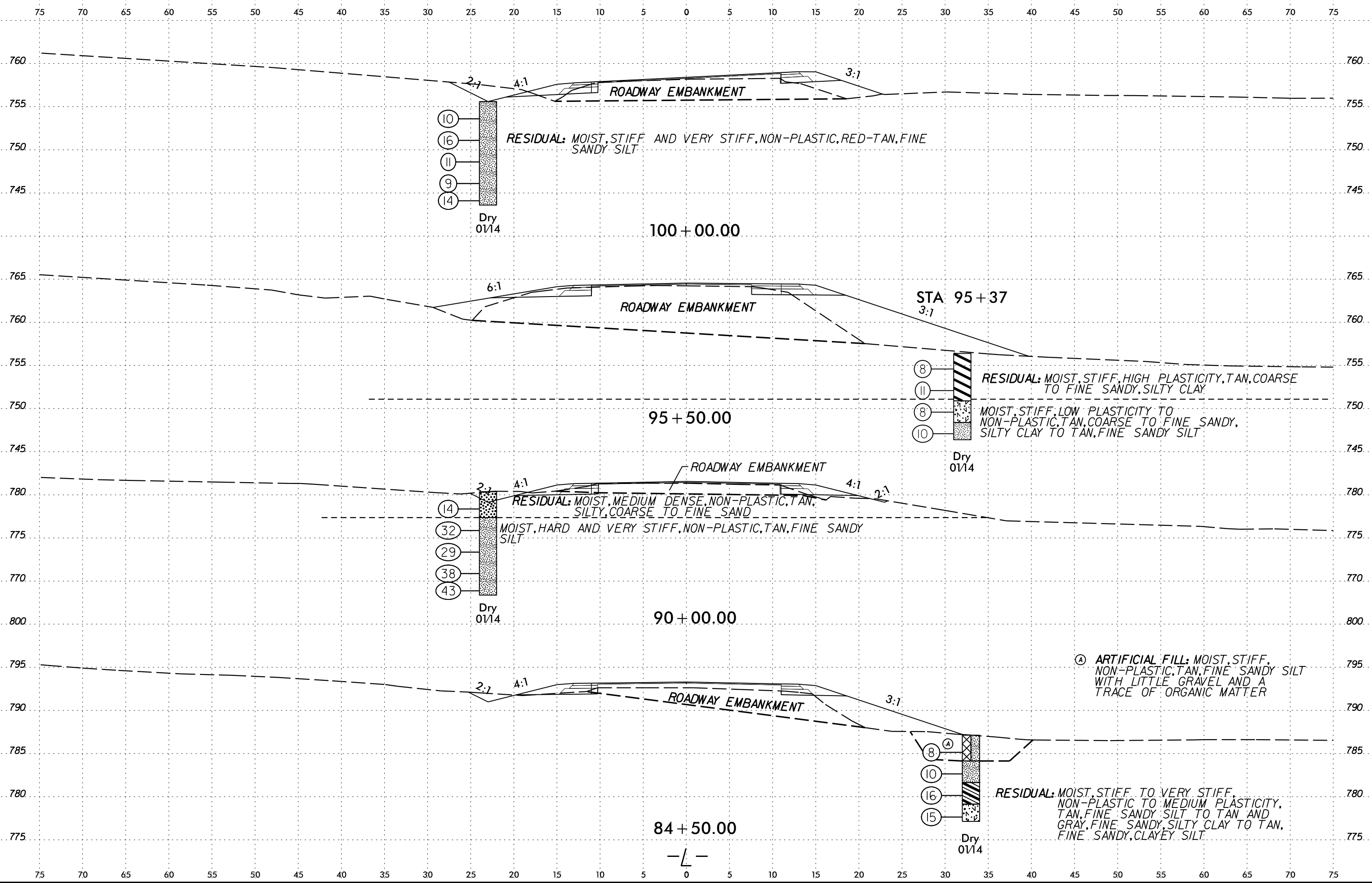
Dry 01/14

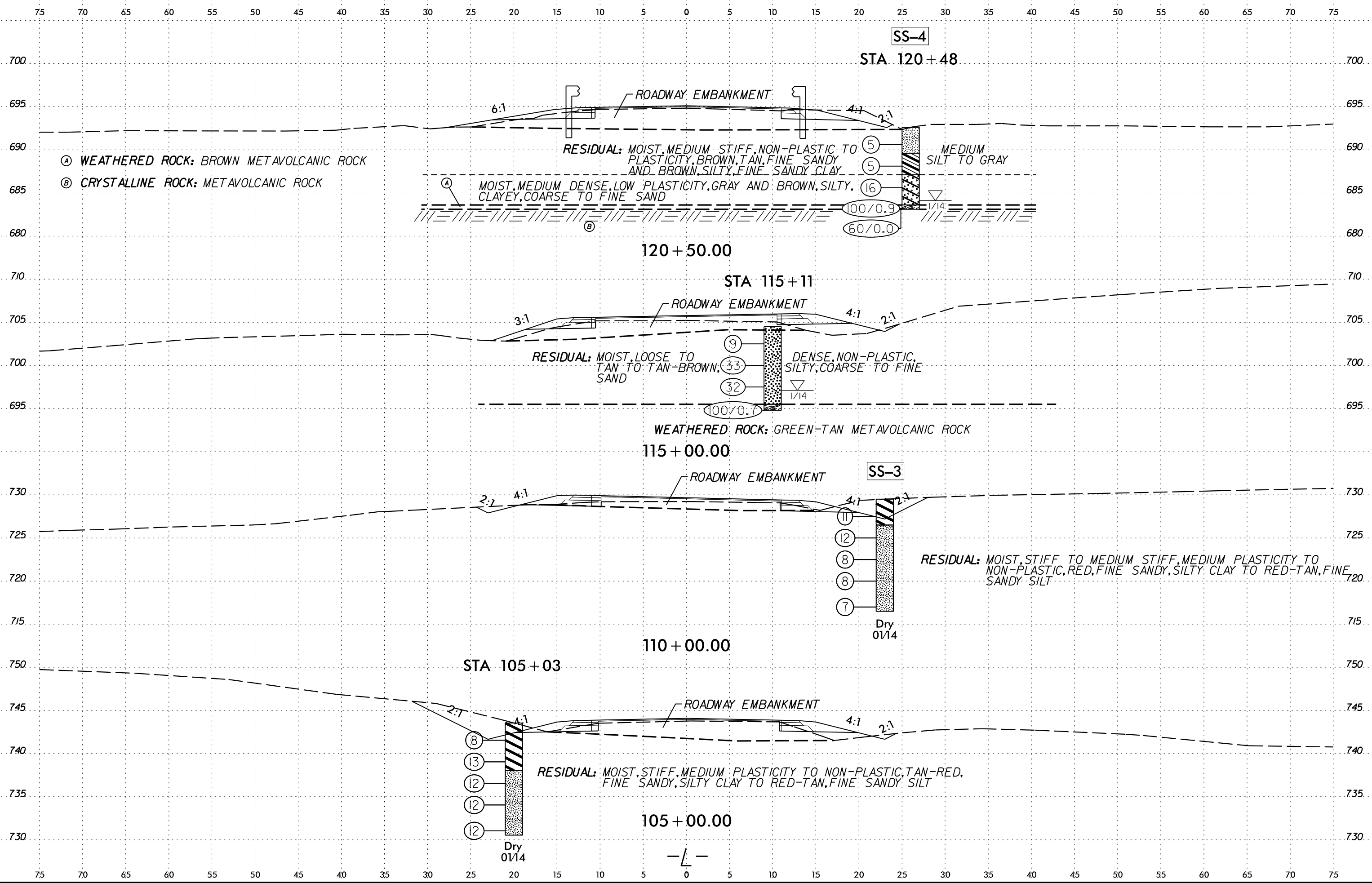
RESIDUAL: MOIST, STIFF TO MEDIUM STIFF, LOW PLASTICITY TO NON-PLASTIC, RED, FINE SANDY, CLAYEY SILT TO RED-TAN, FINE SANDY SILT

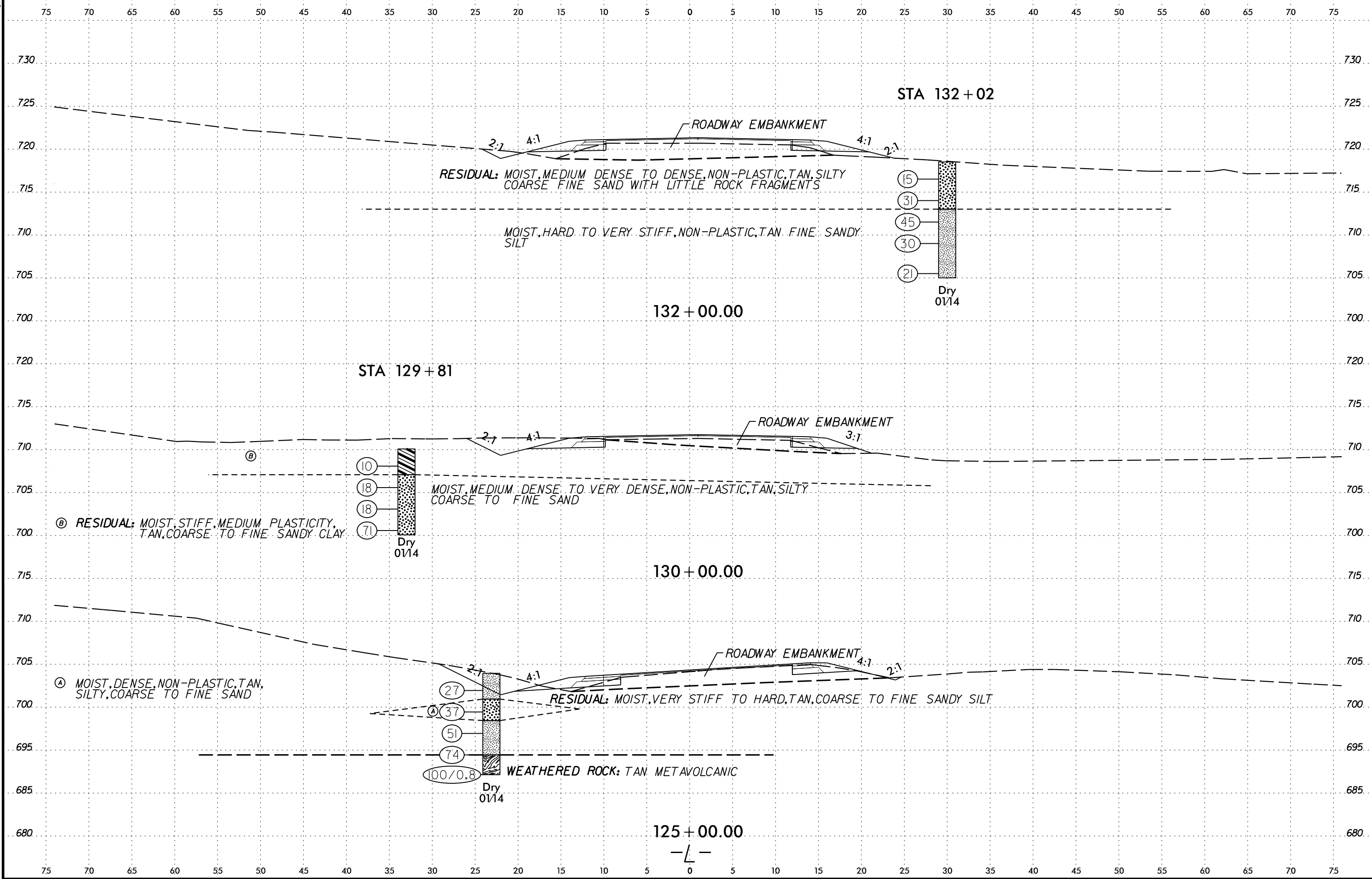
55 + 00.00

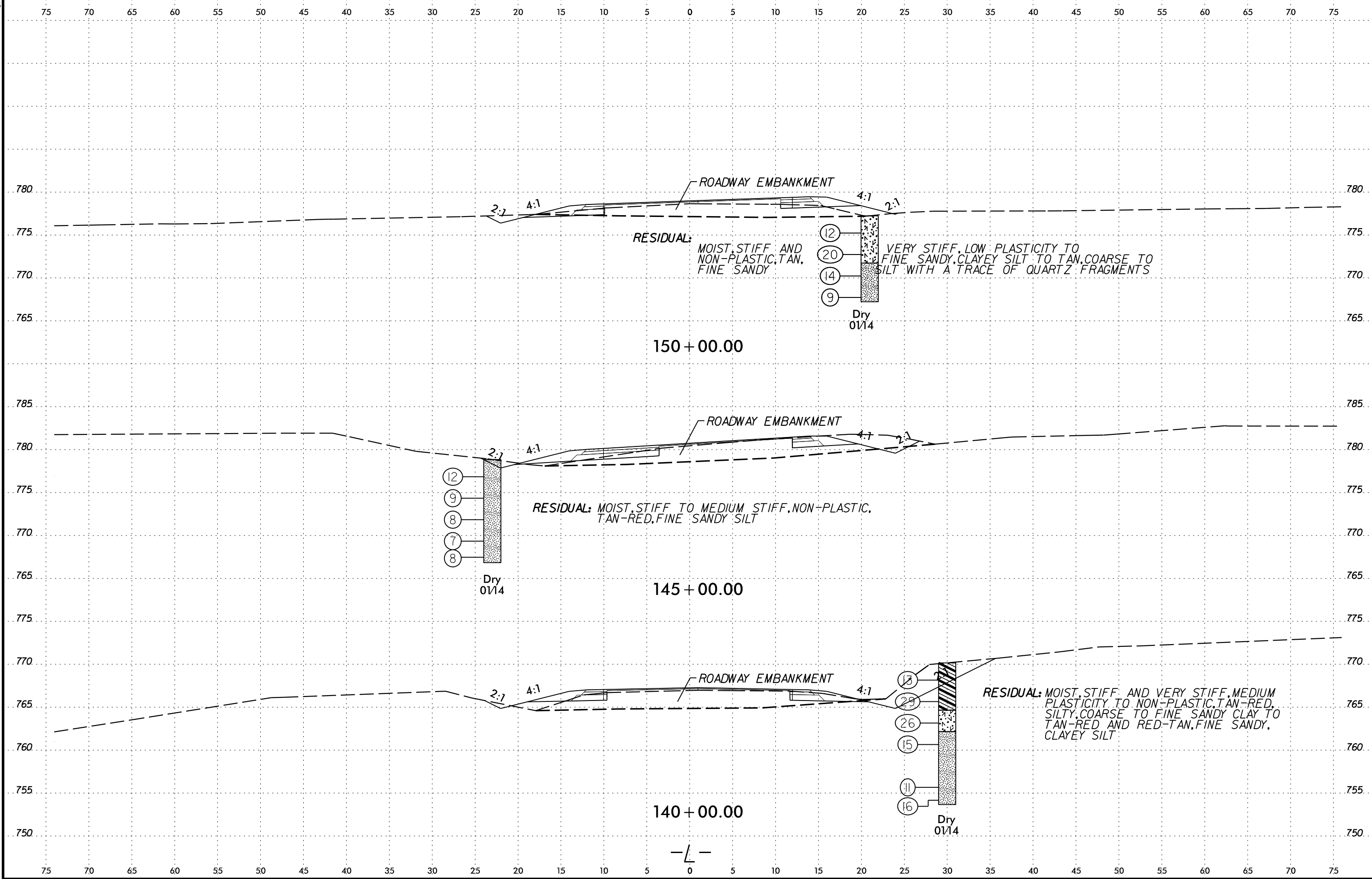
-L-

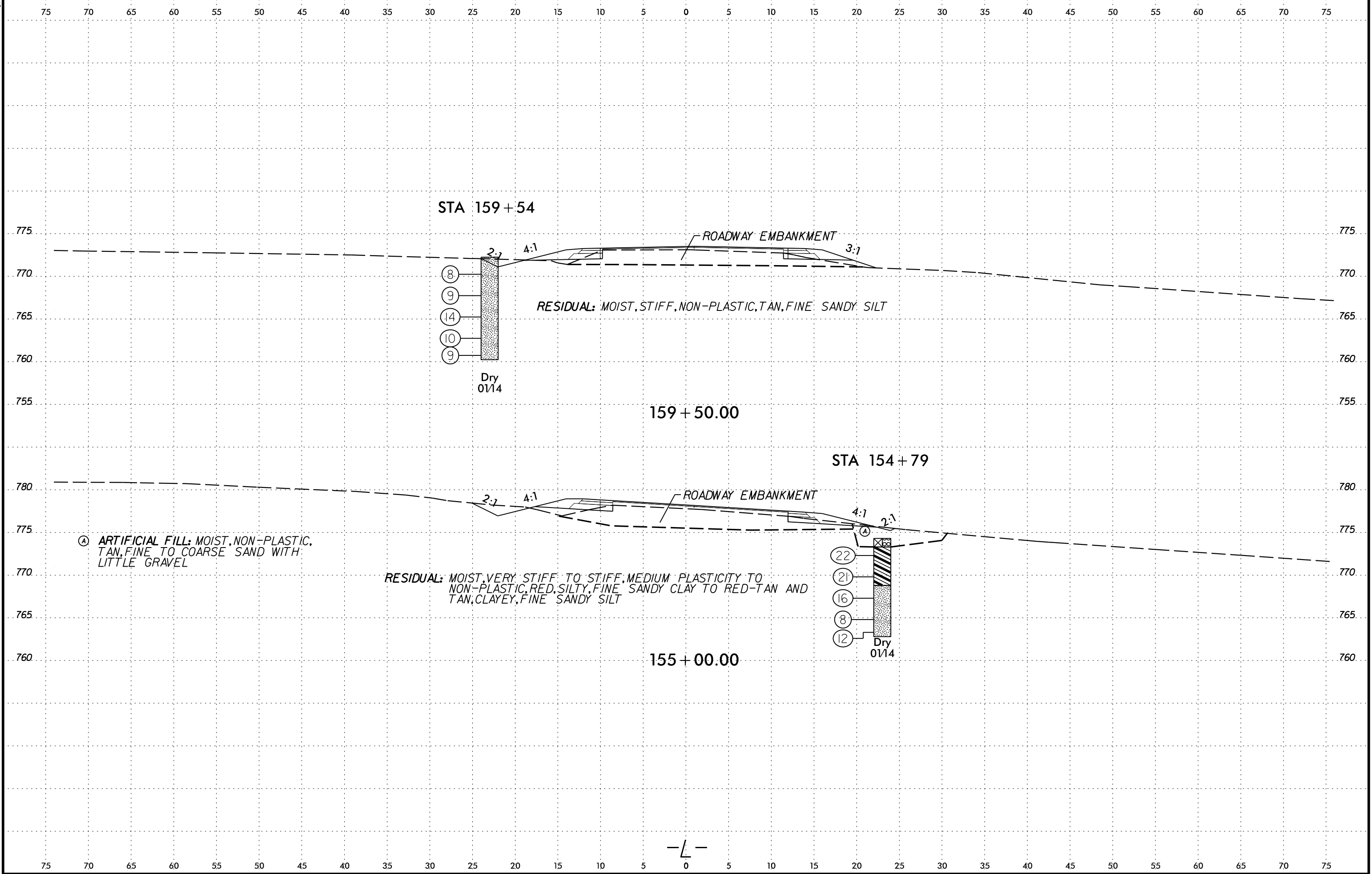


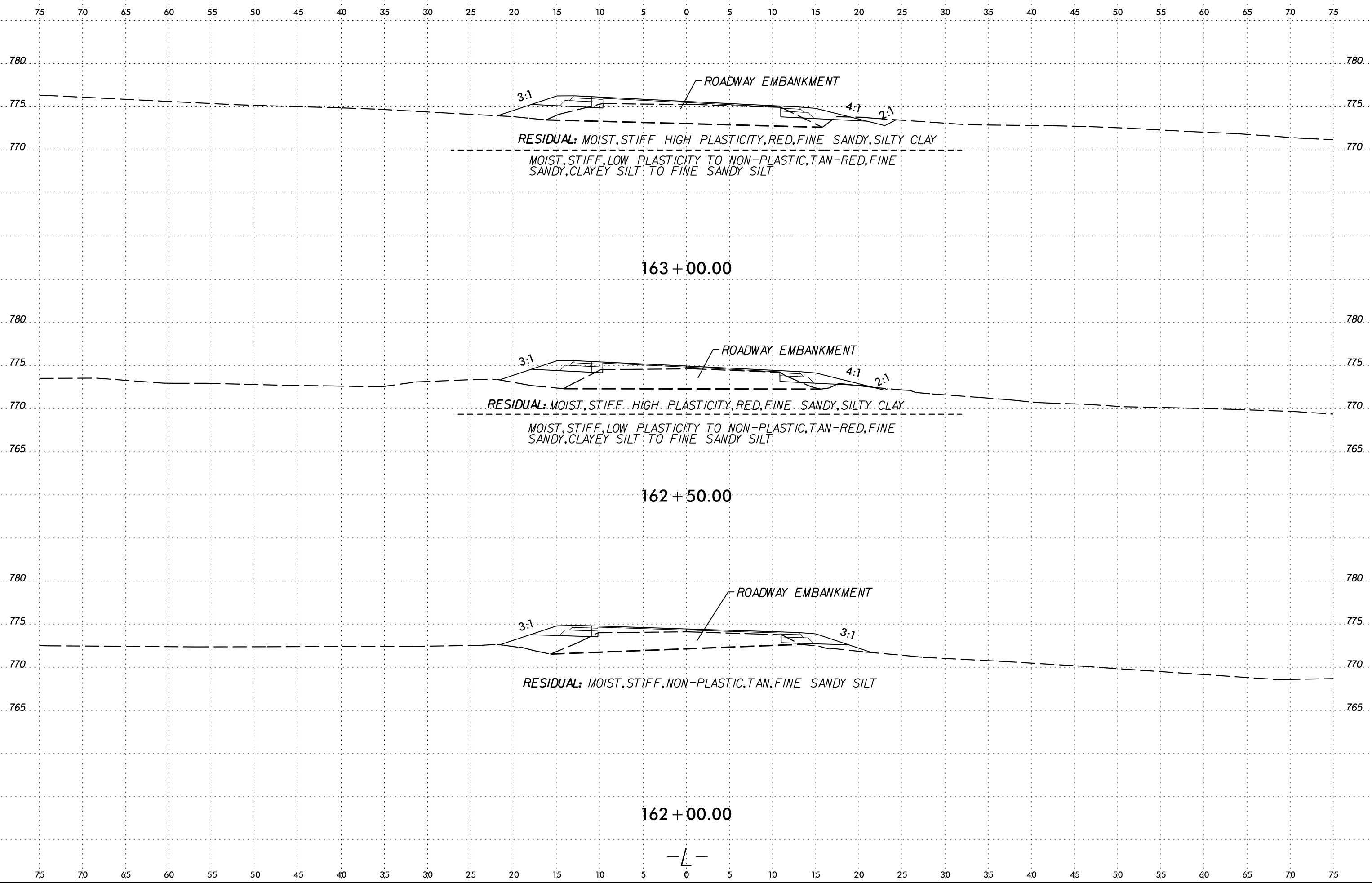


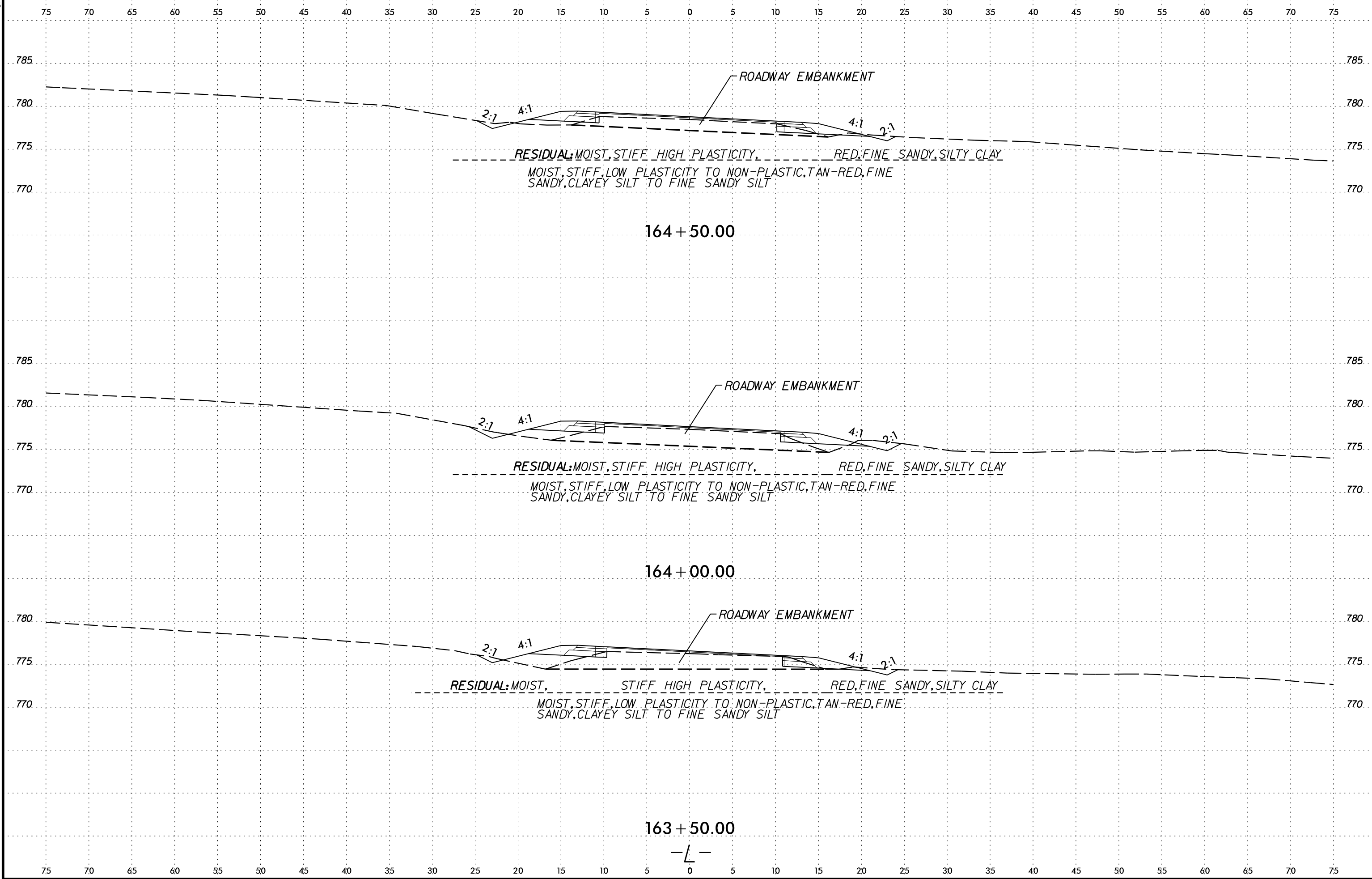


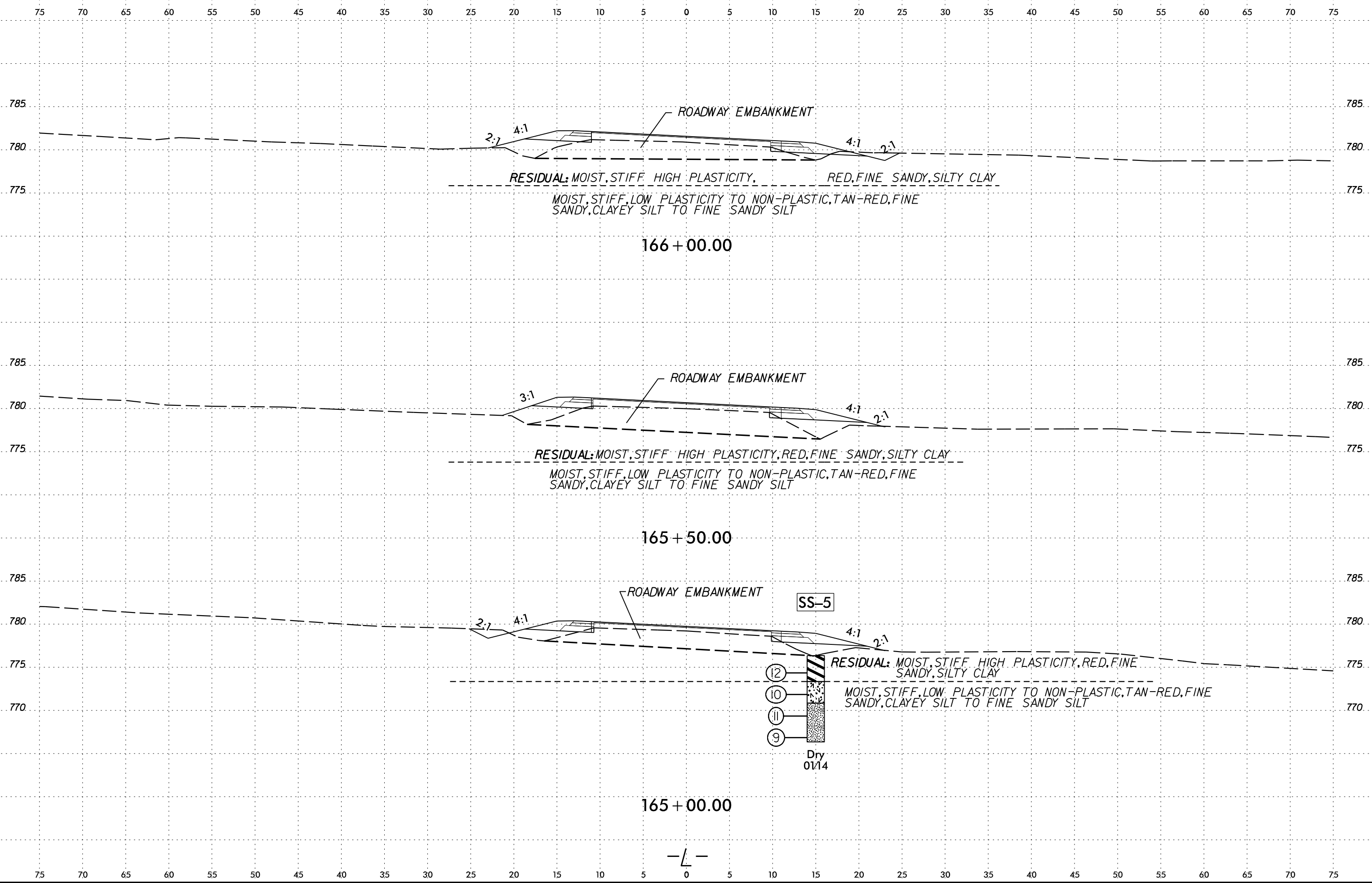


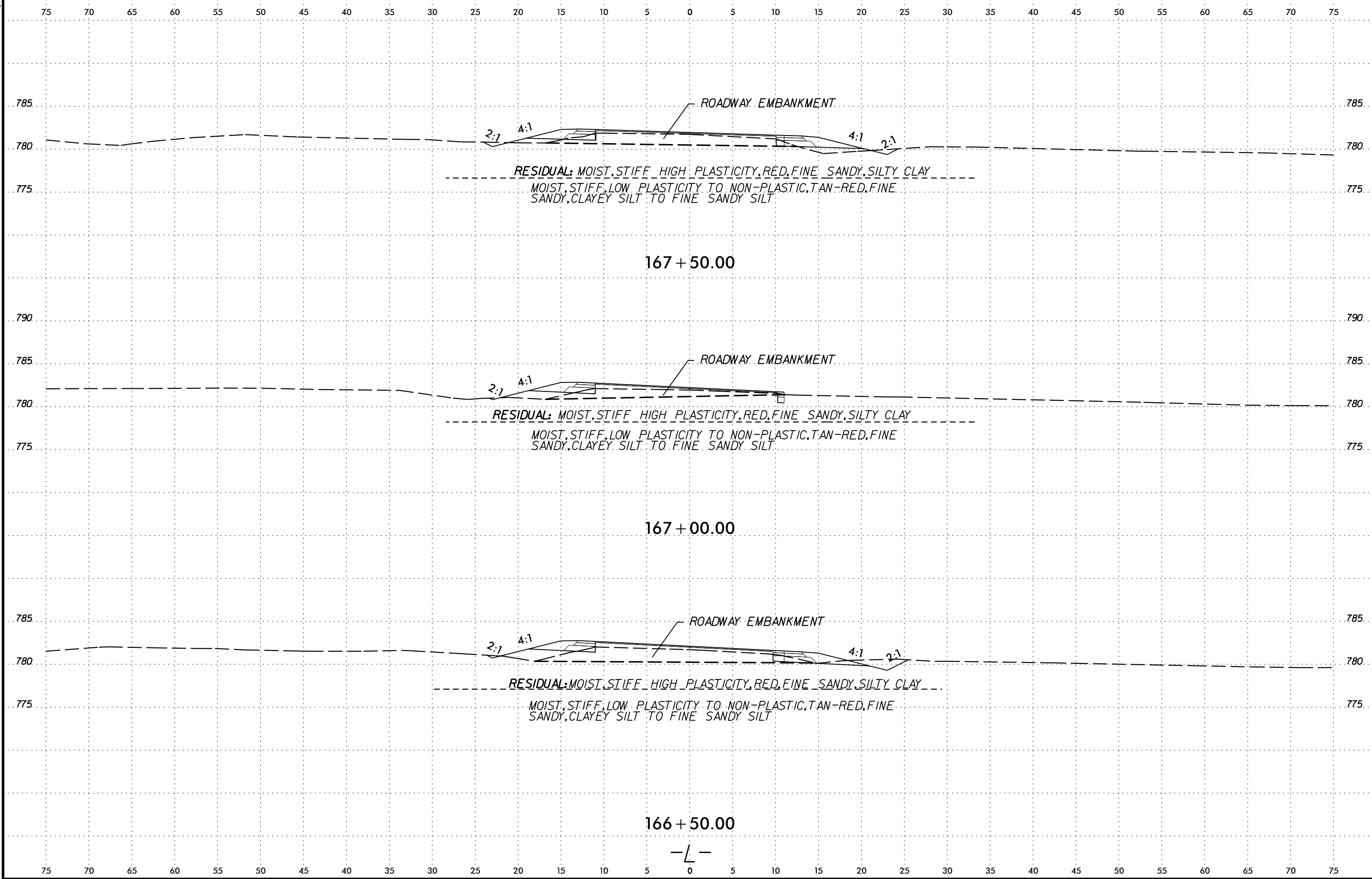


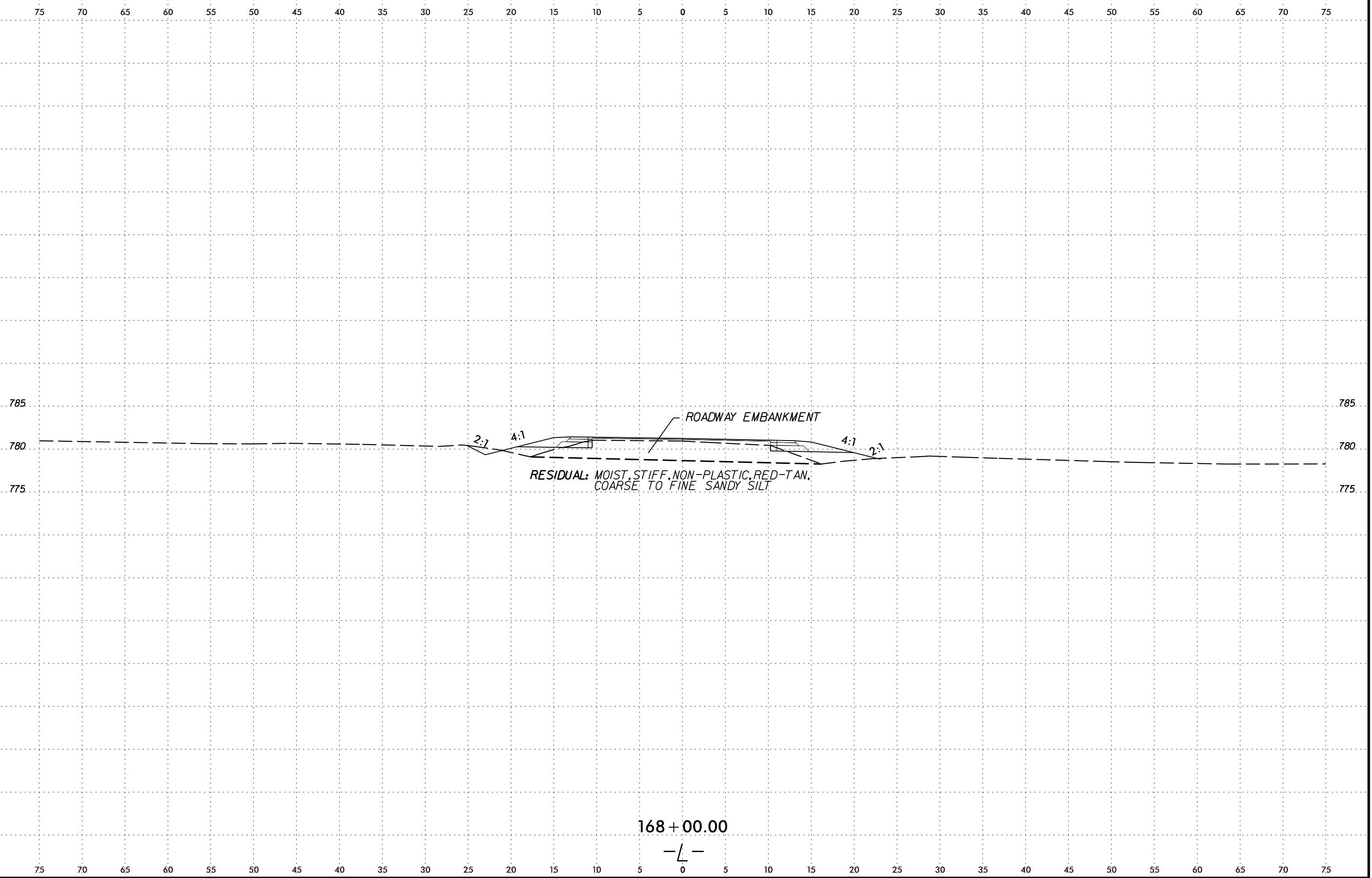


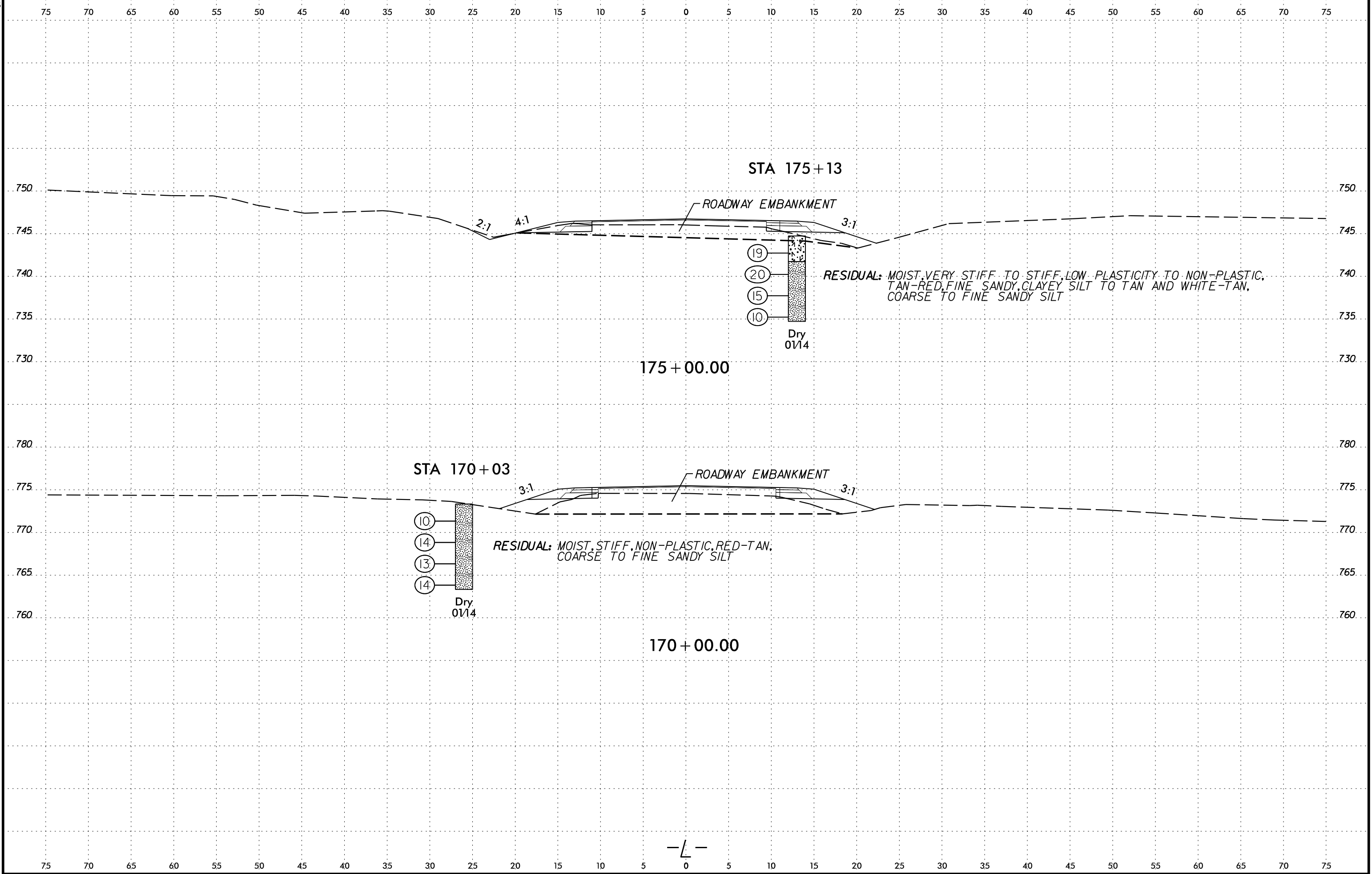


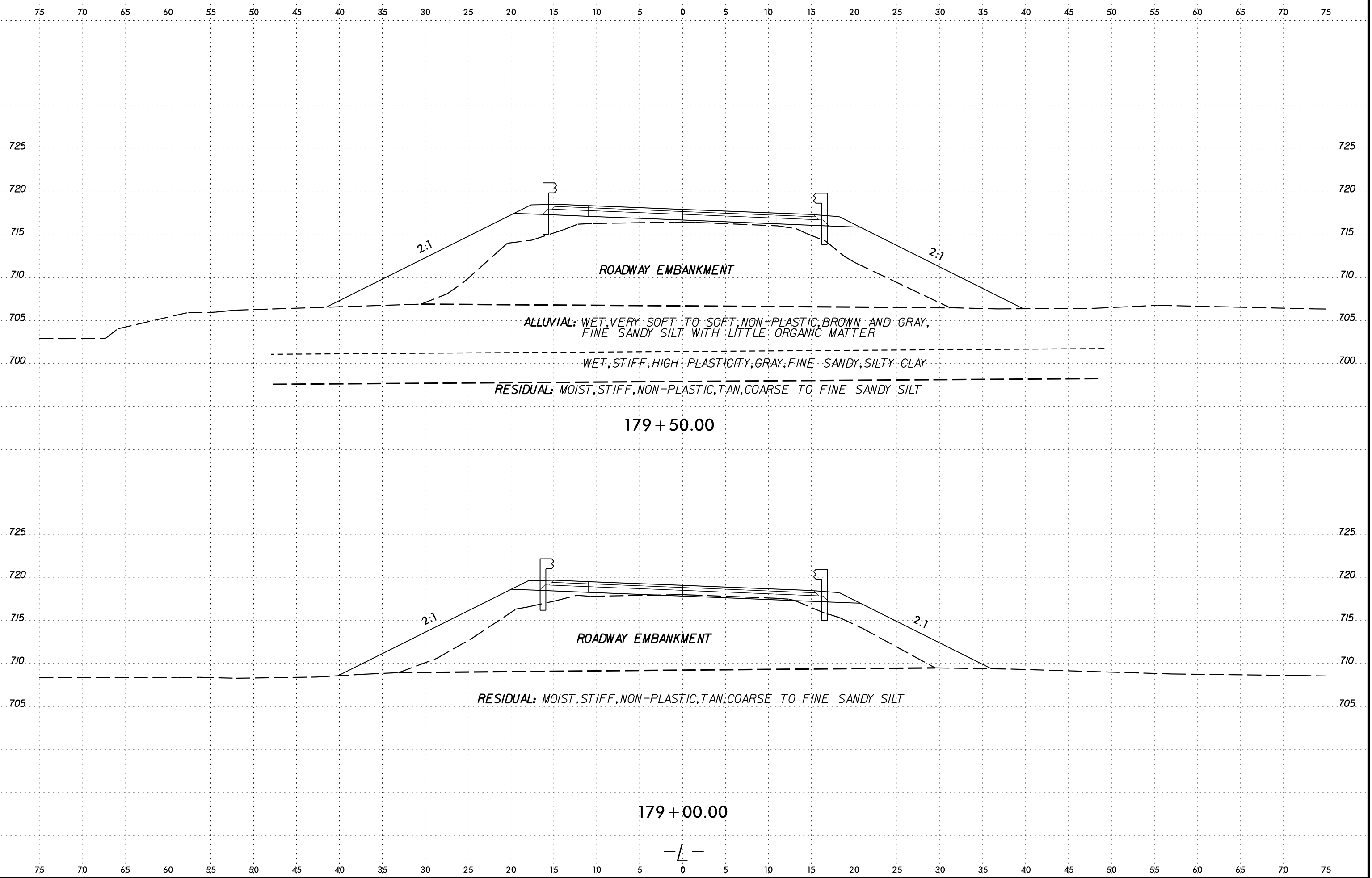












179 + 50.00

ROADWAY EMBANKMENT

ALLUVIAL: WET, VERY SOFT TO SOFT, NON-PLASTIC, BROWN AND GRAY, FINE SANDY SILT WITH LITTLE ORGANIC MATTER

WET, STIFF, HIGH PLASTICITY, GRAY, FINE SANDY, SILTY CLAY

RESIDUAL: MOIST, STIFF, NON-PLASTIC, TAN, COARSE TO FINE SANDY SILT

2:1

2:1

179 + 00.00

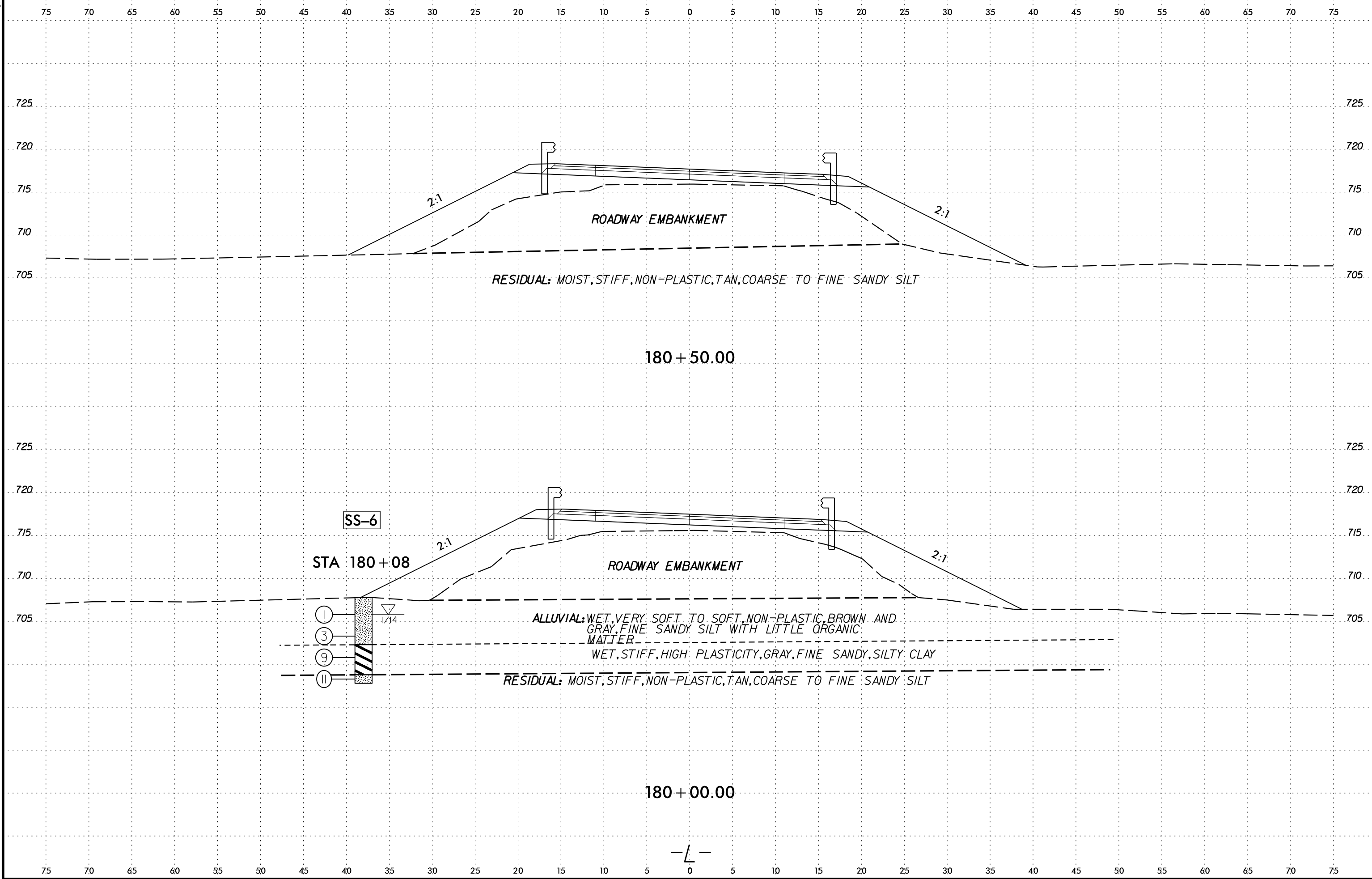
ROADWAY EMBANKMENT

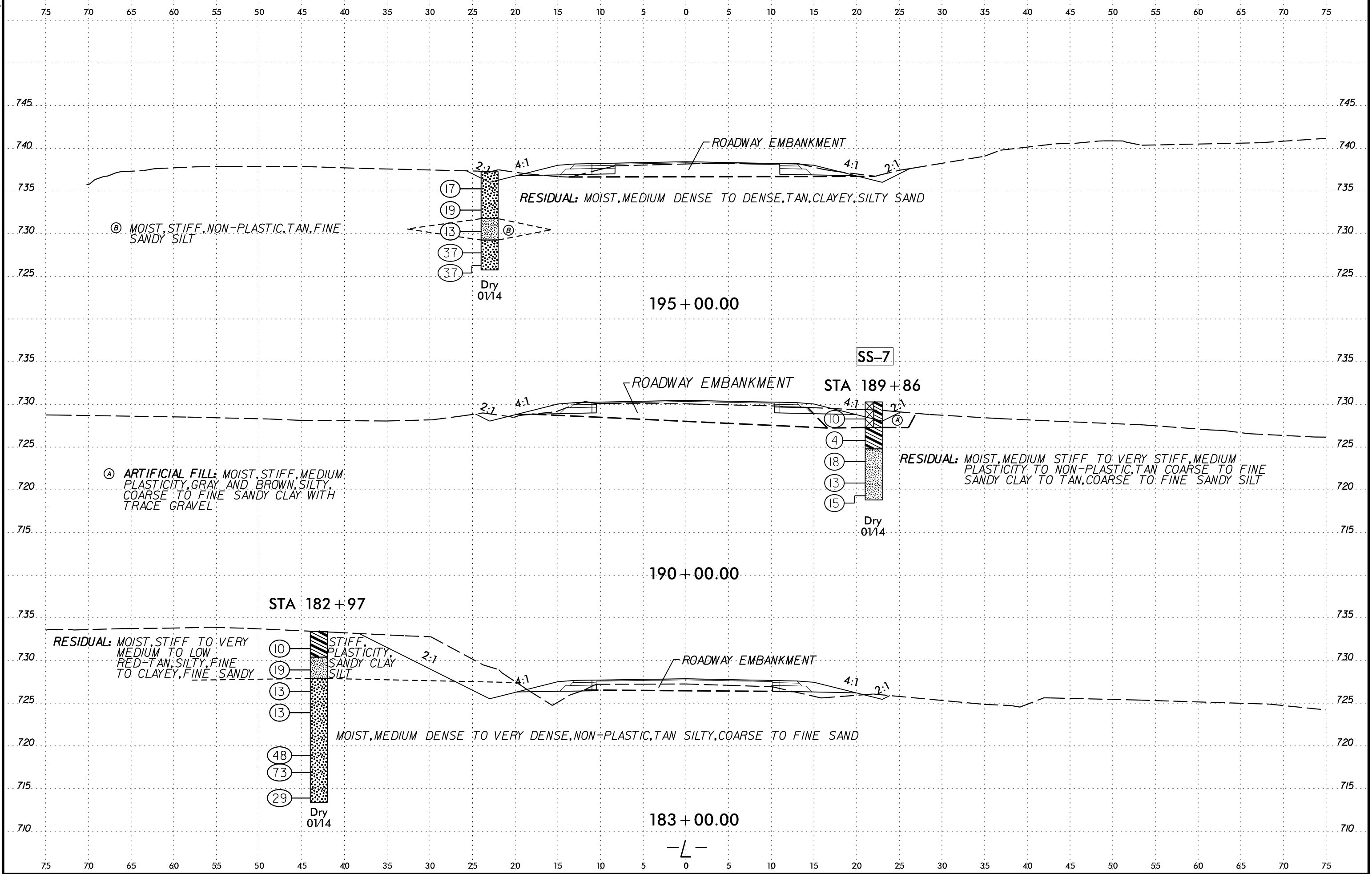
RESIDUAL: MOIST, STIFF, NON-PLASTIC, TAN, COARSE TO FINE SANDY SILT

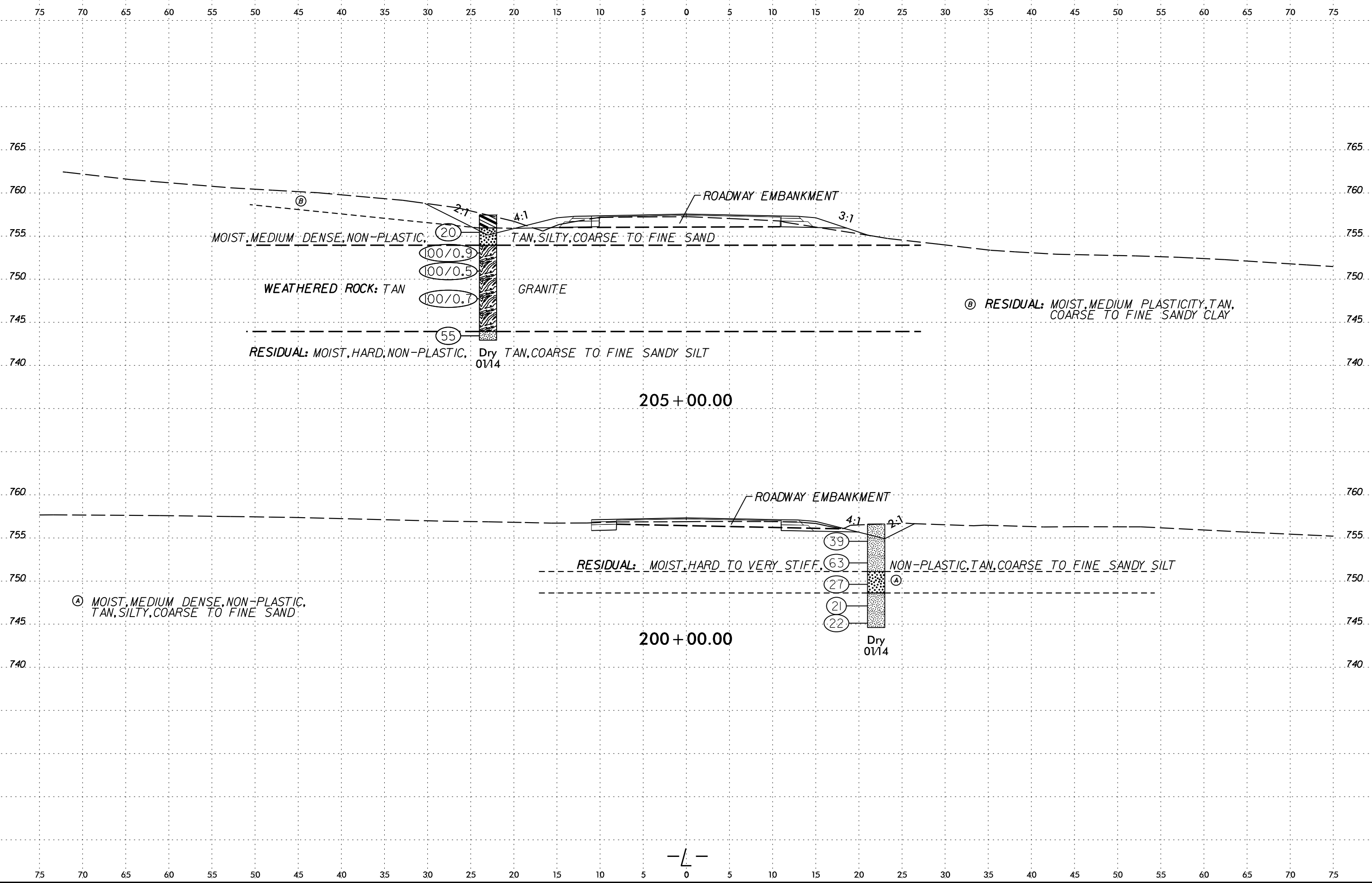
2:1

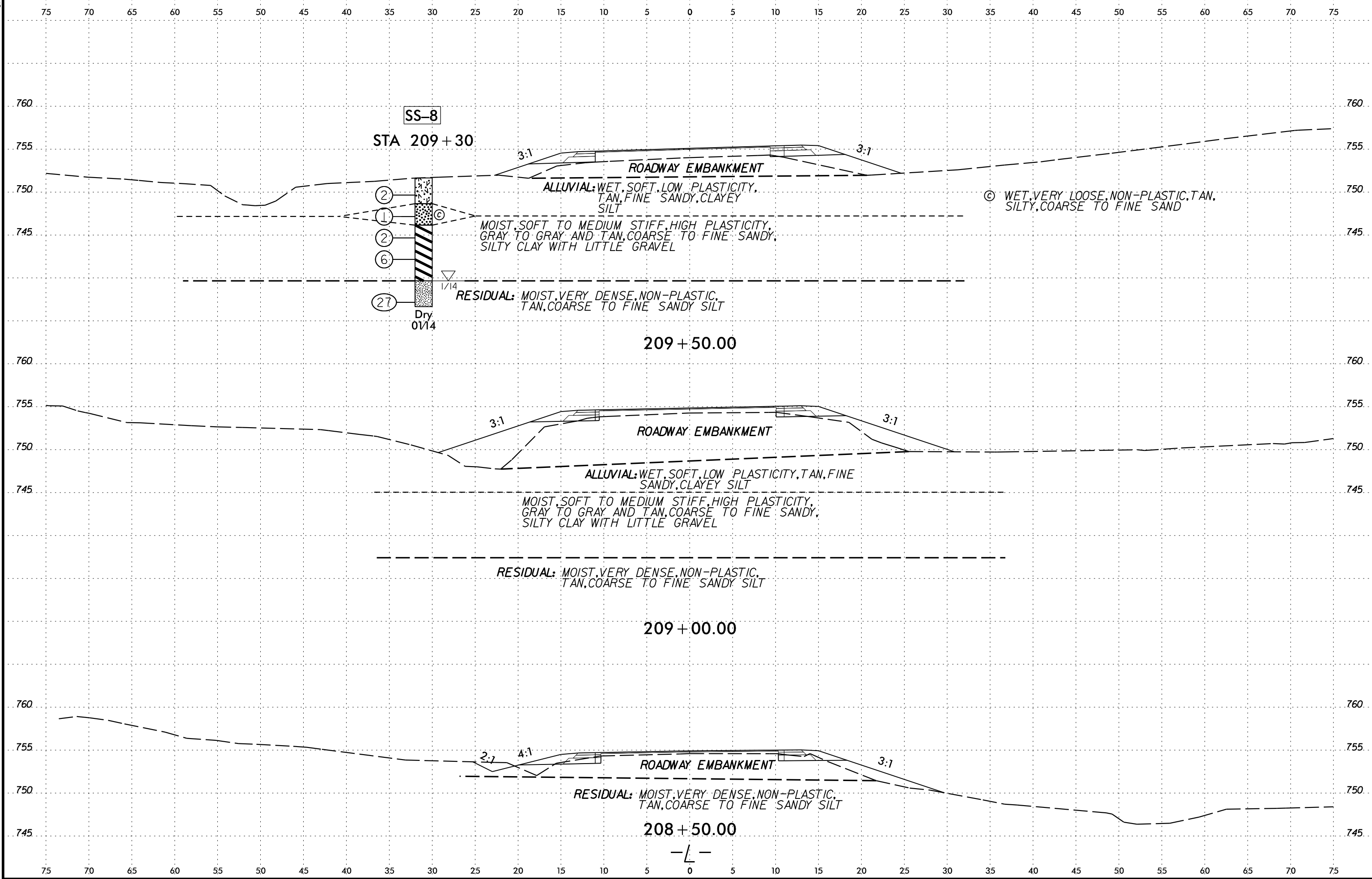
2:1

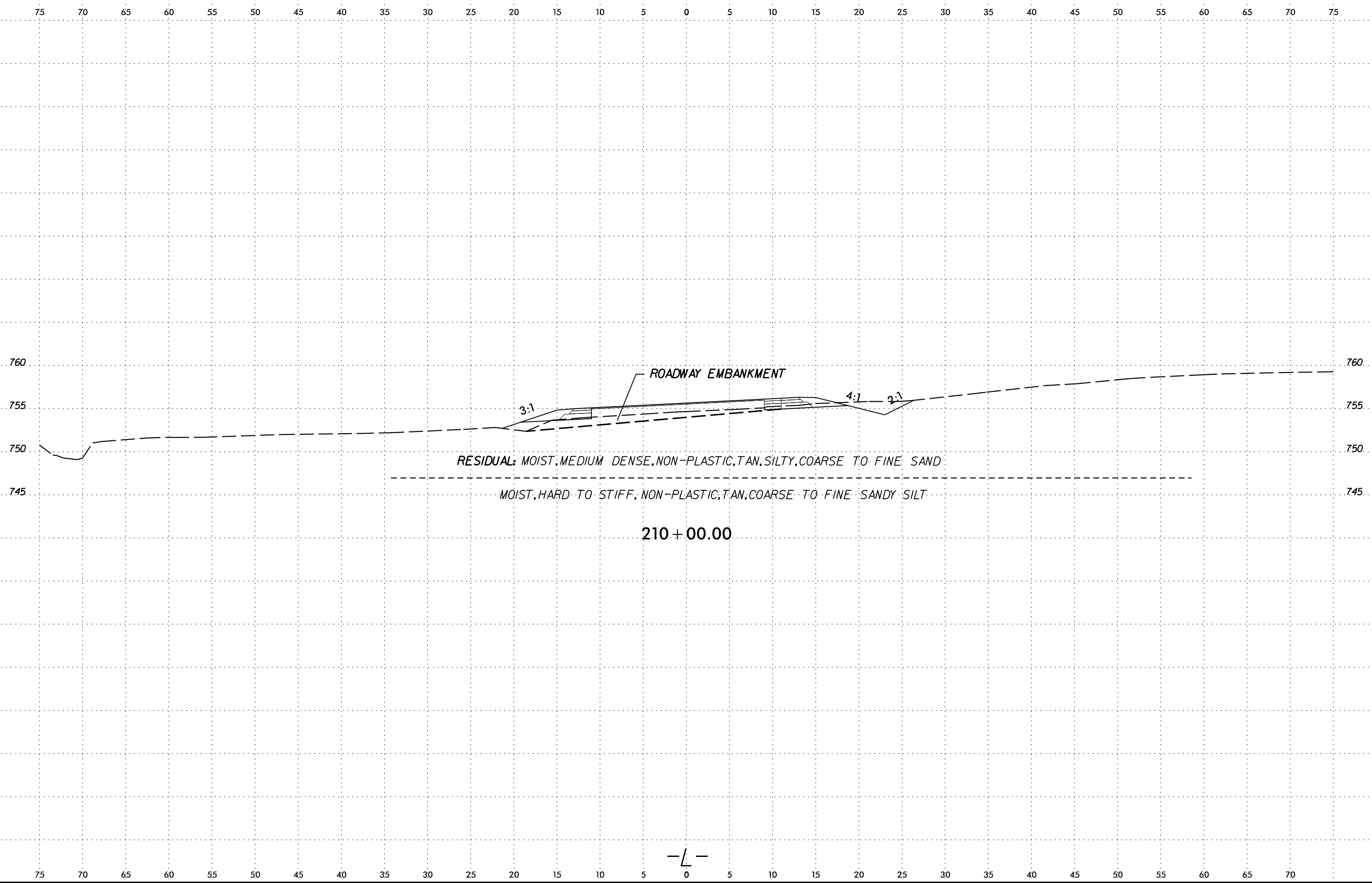




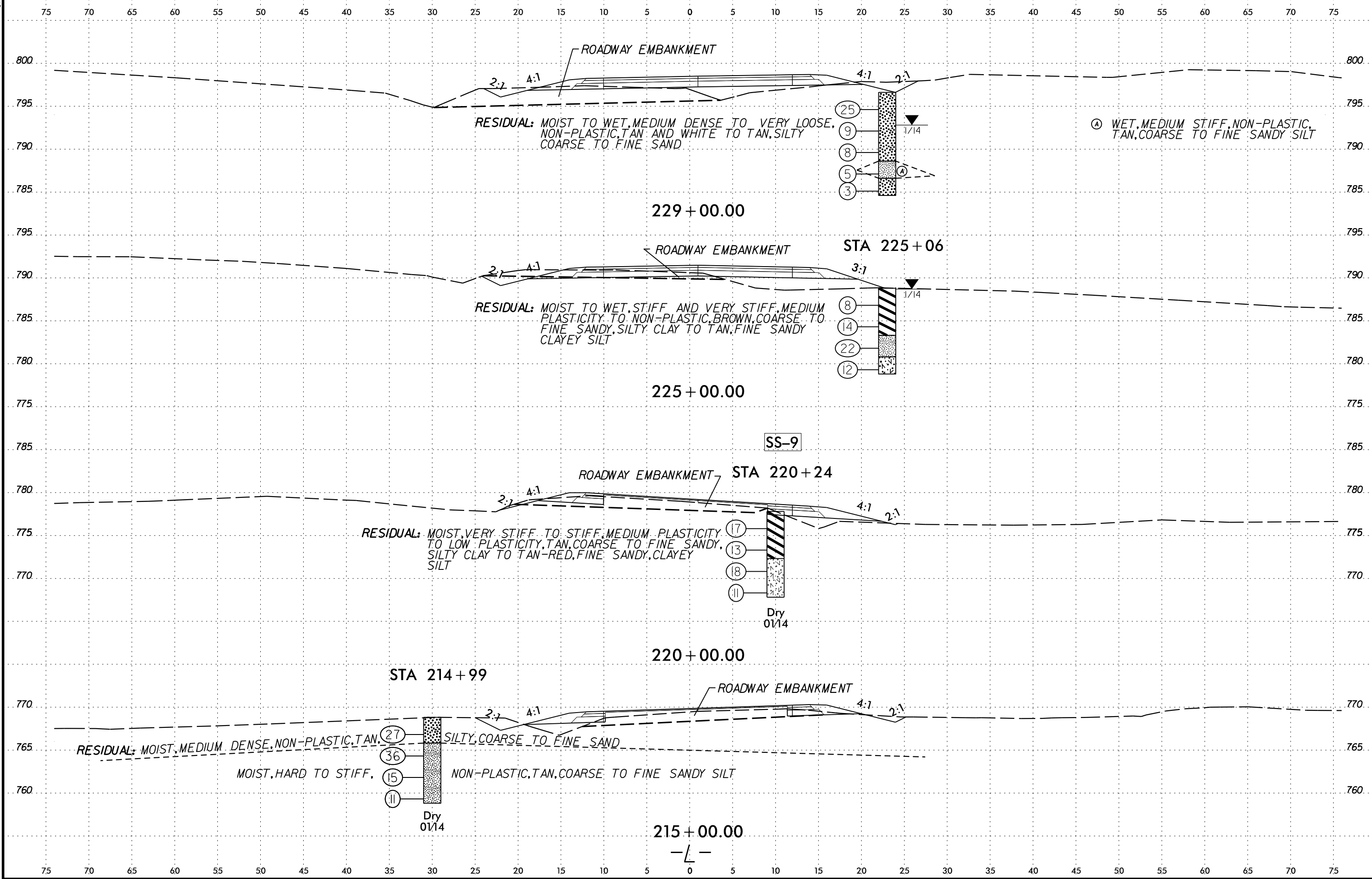


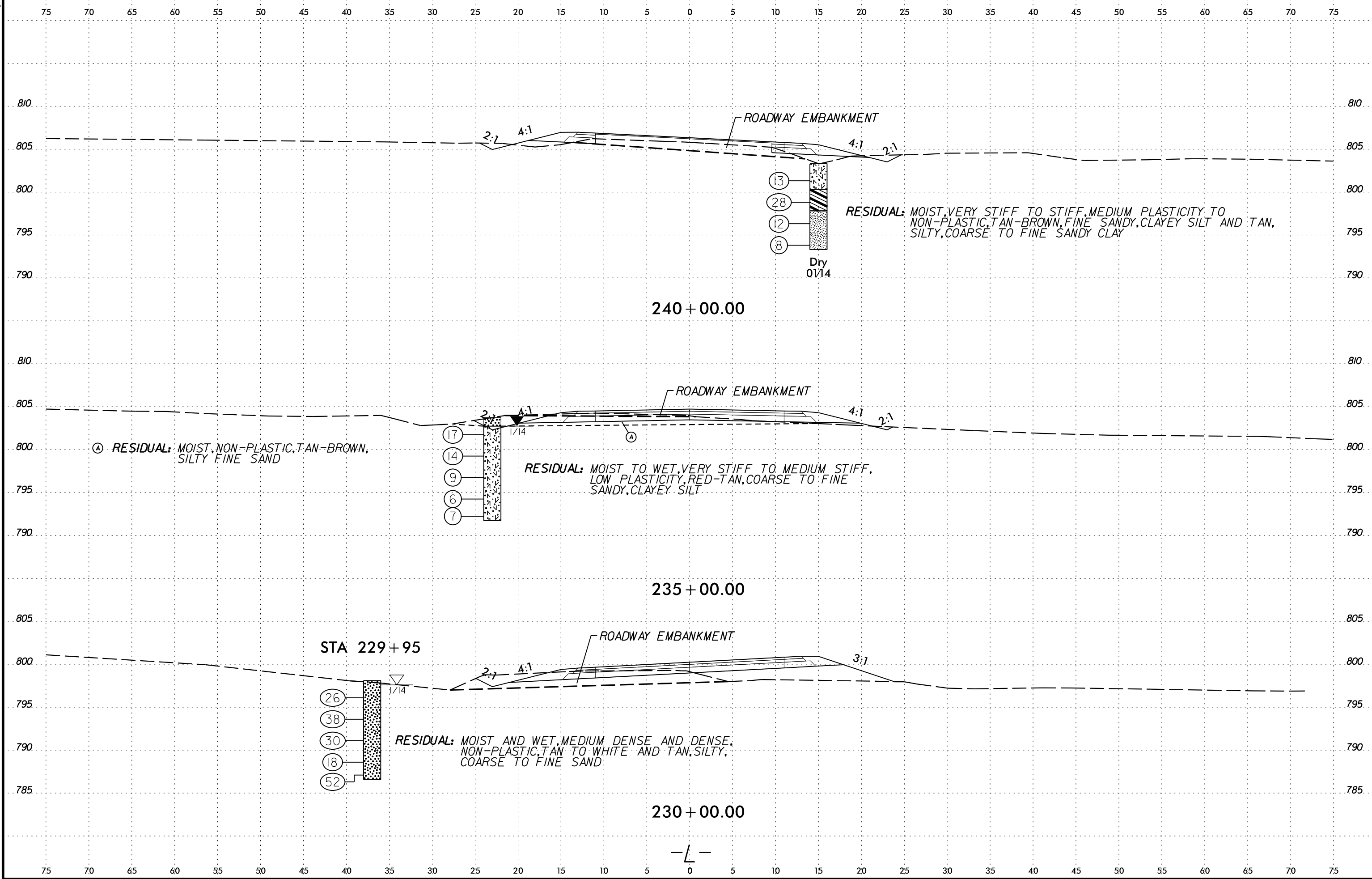


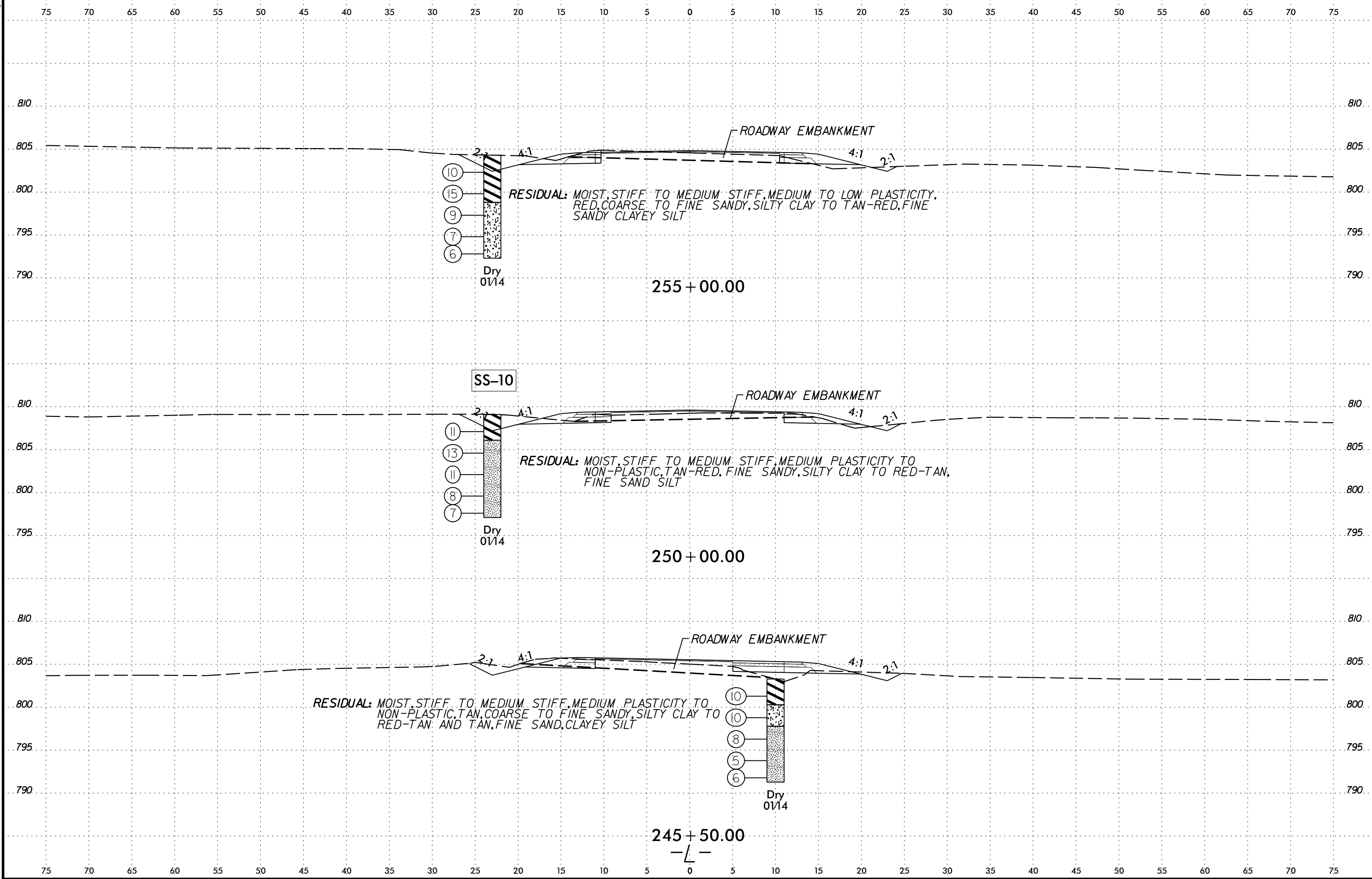


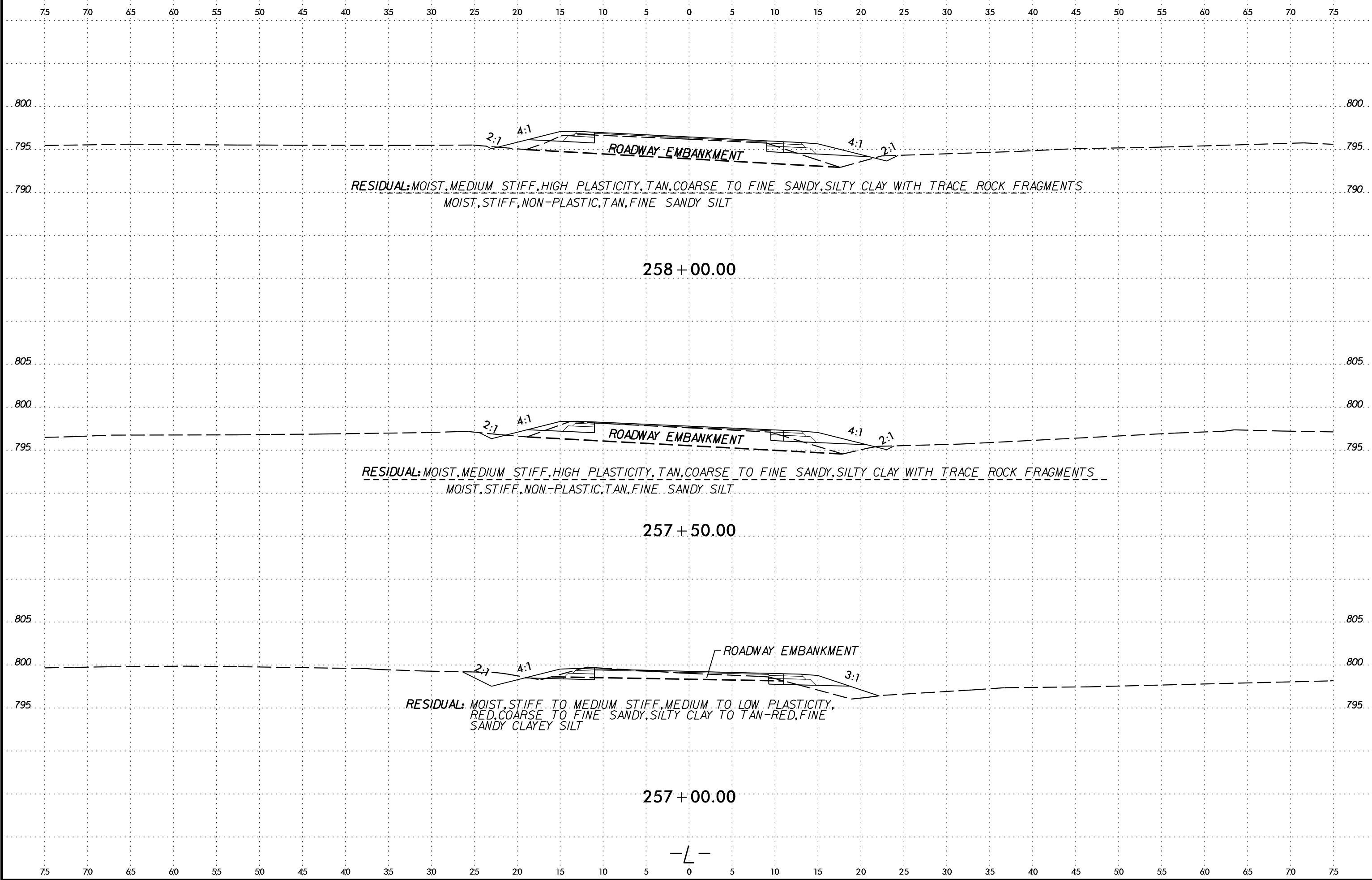


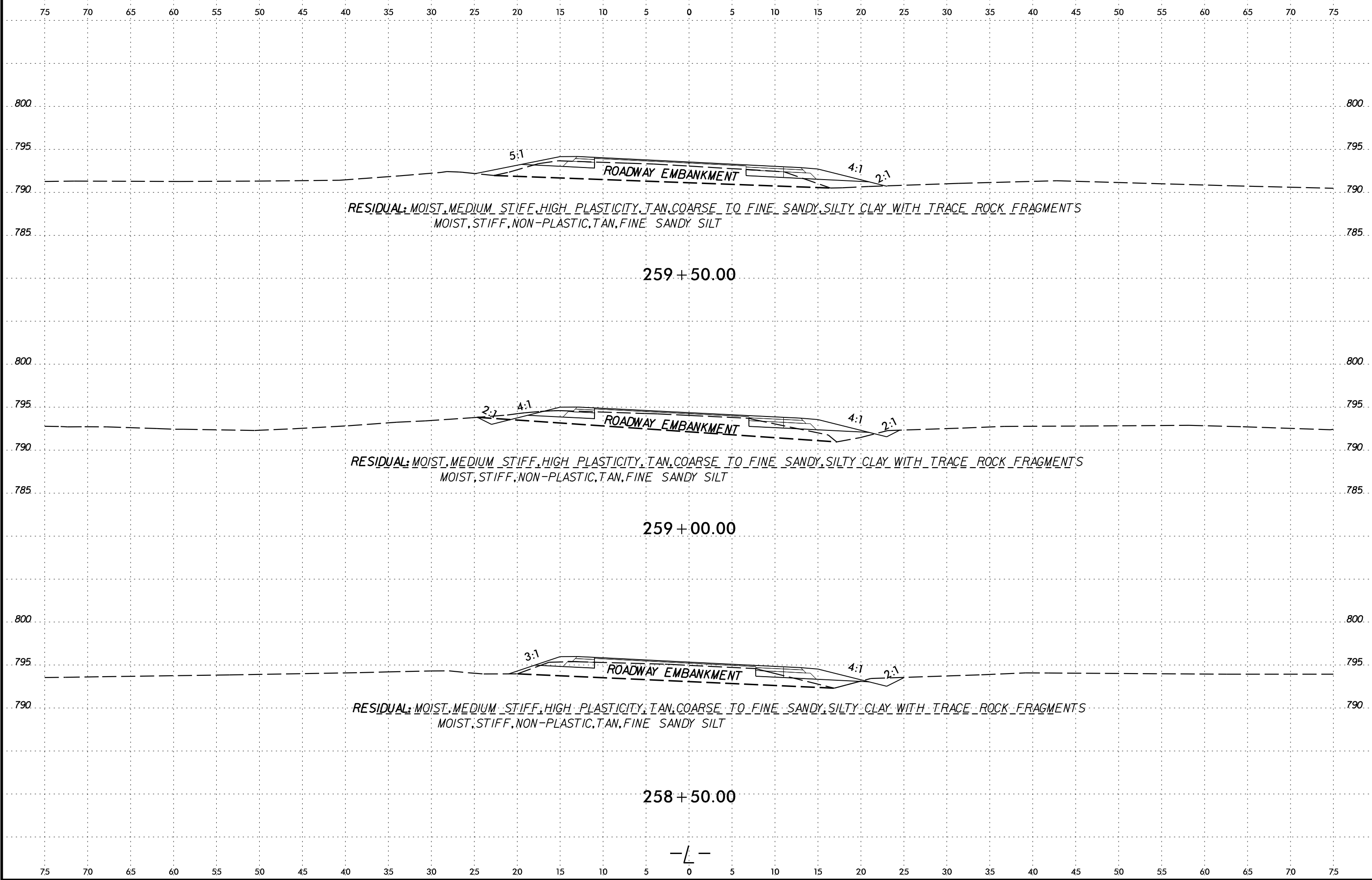
8/23/99

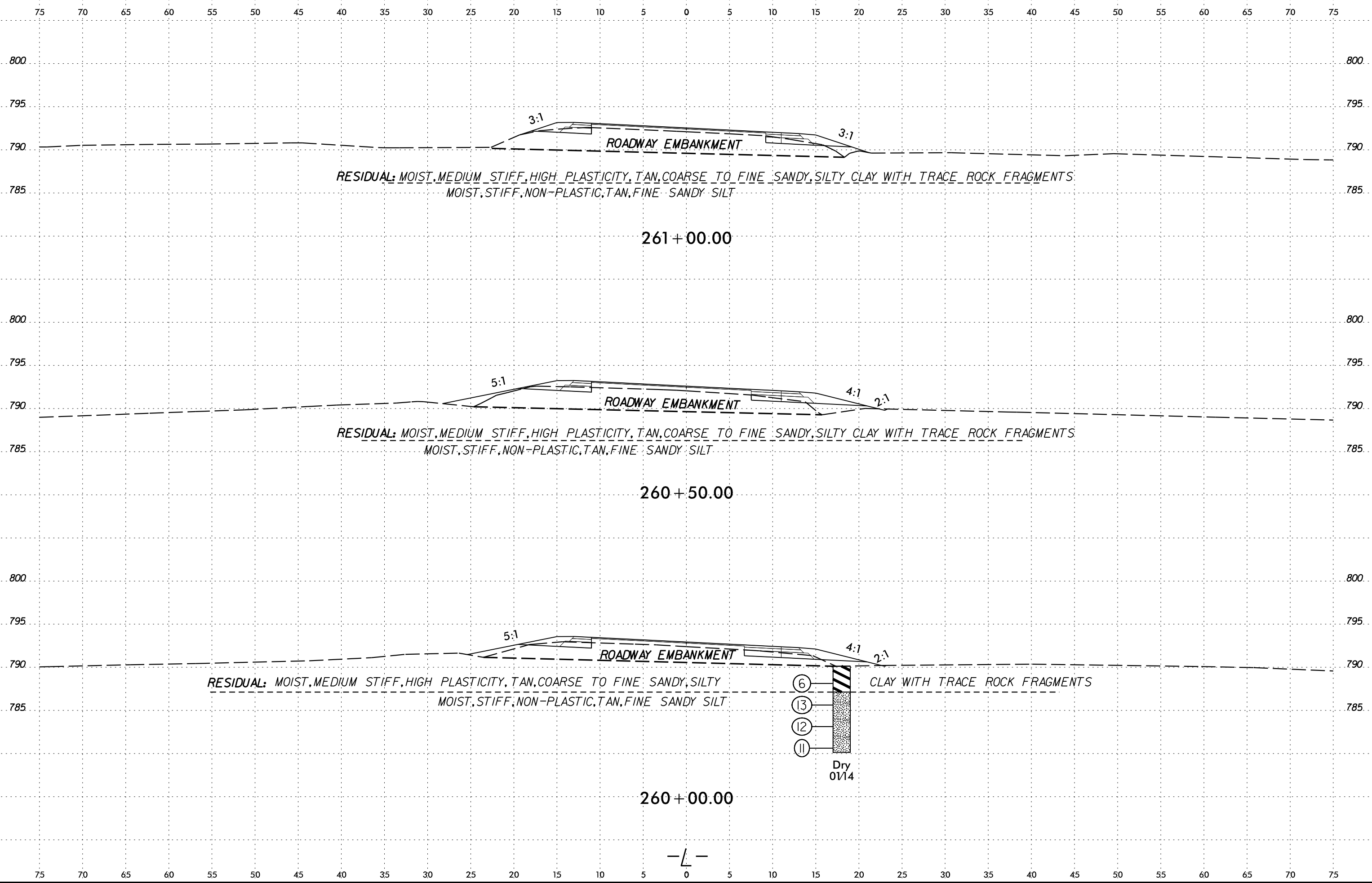


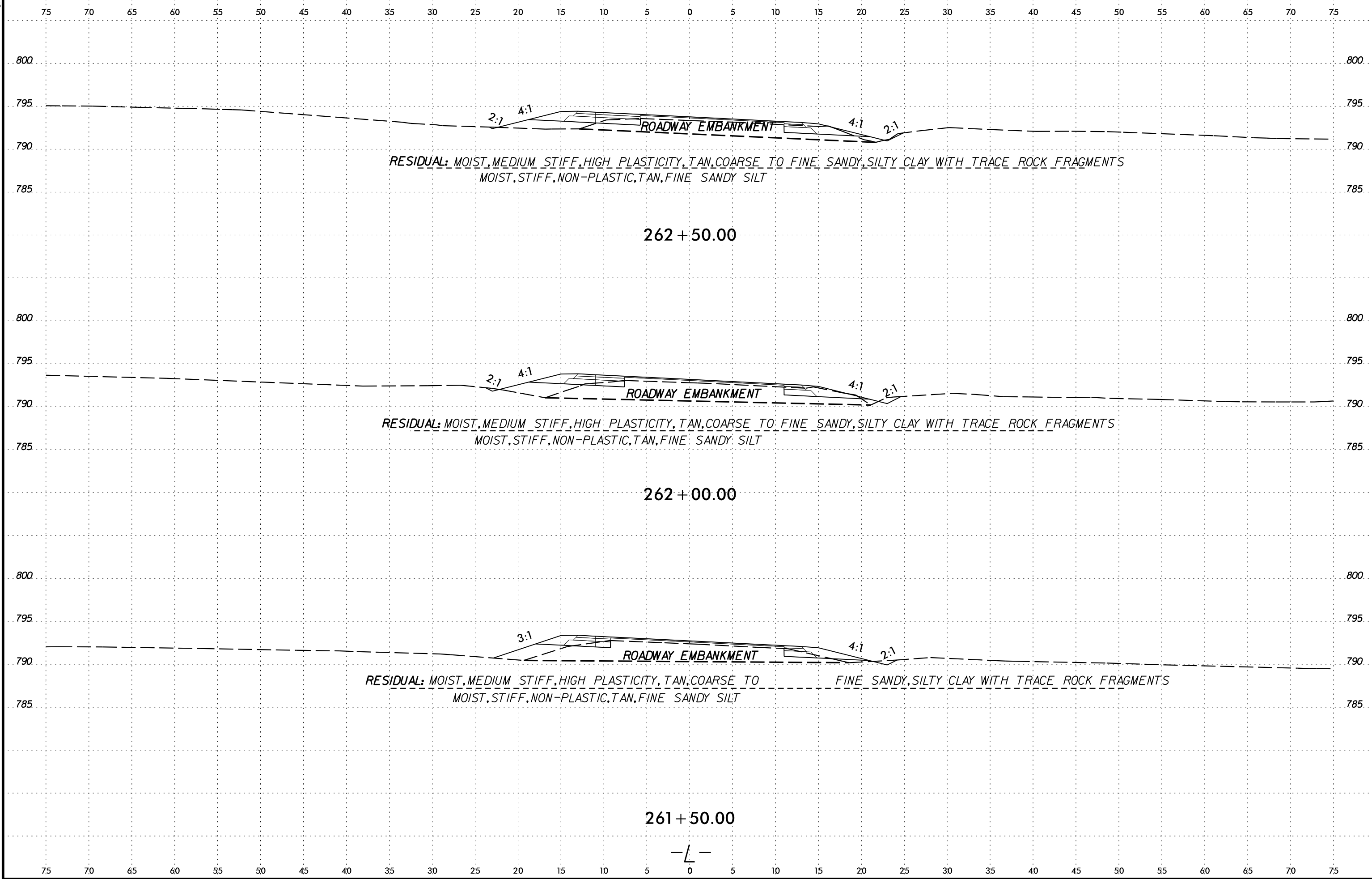


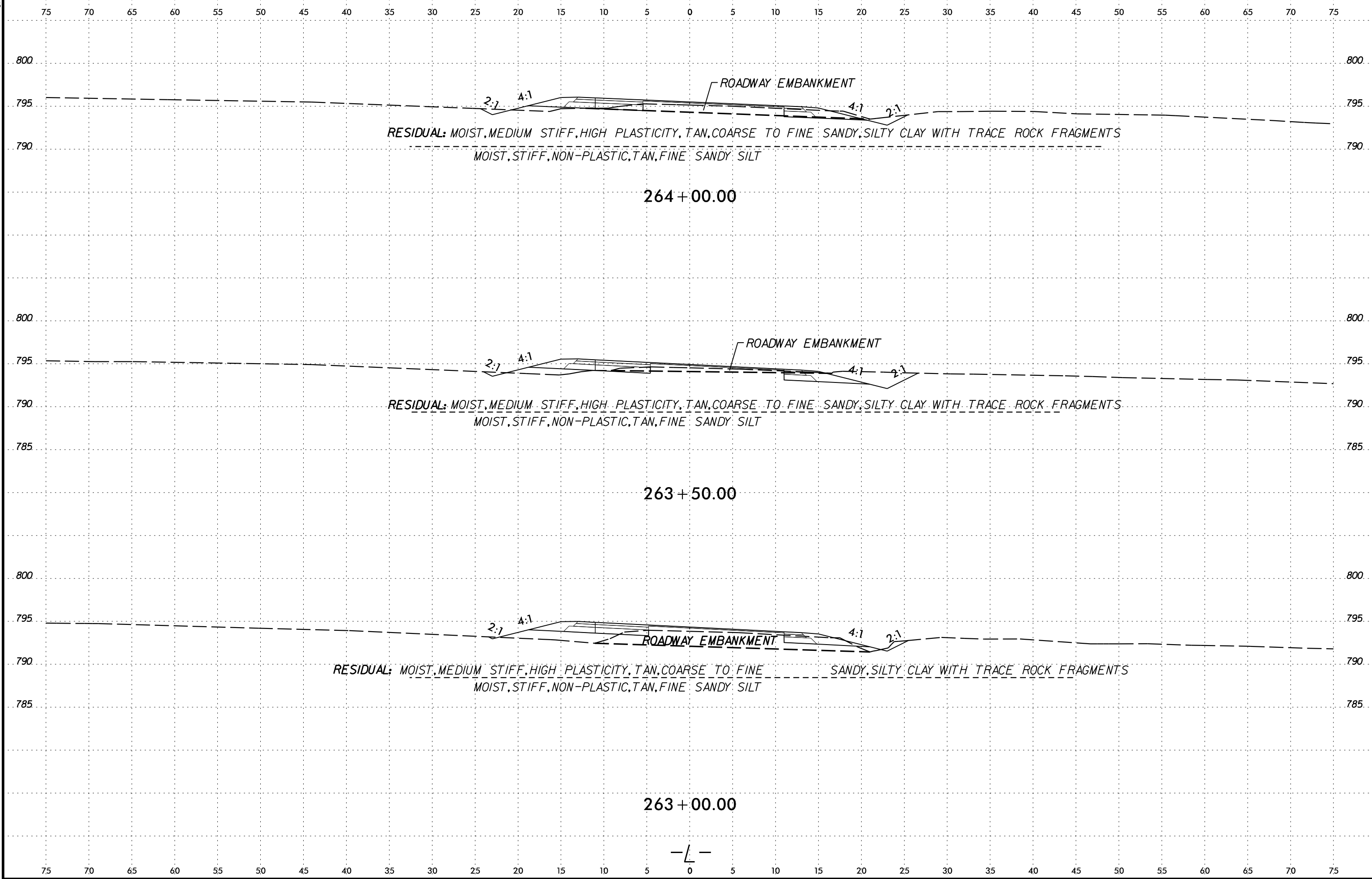


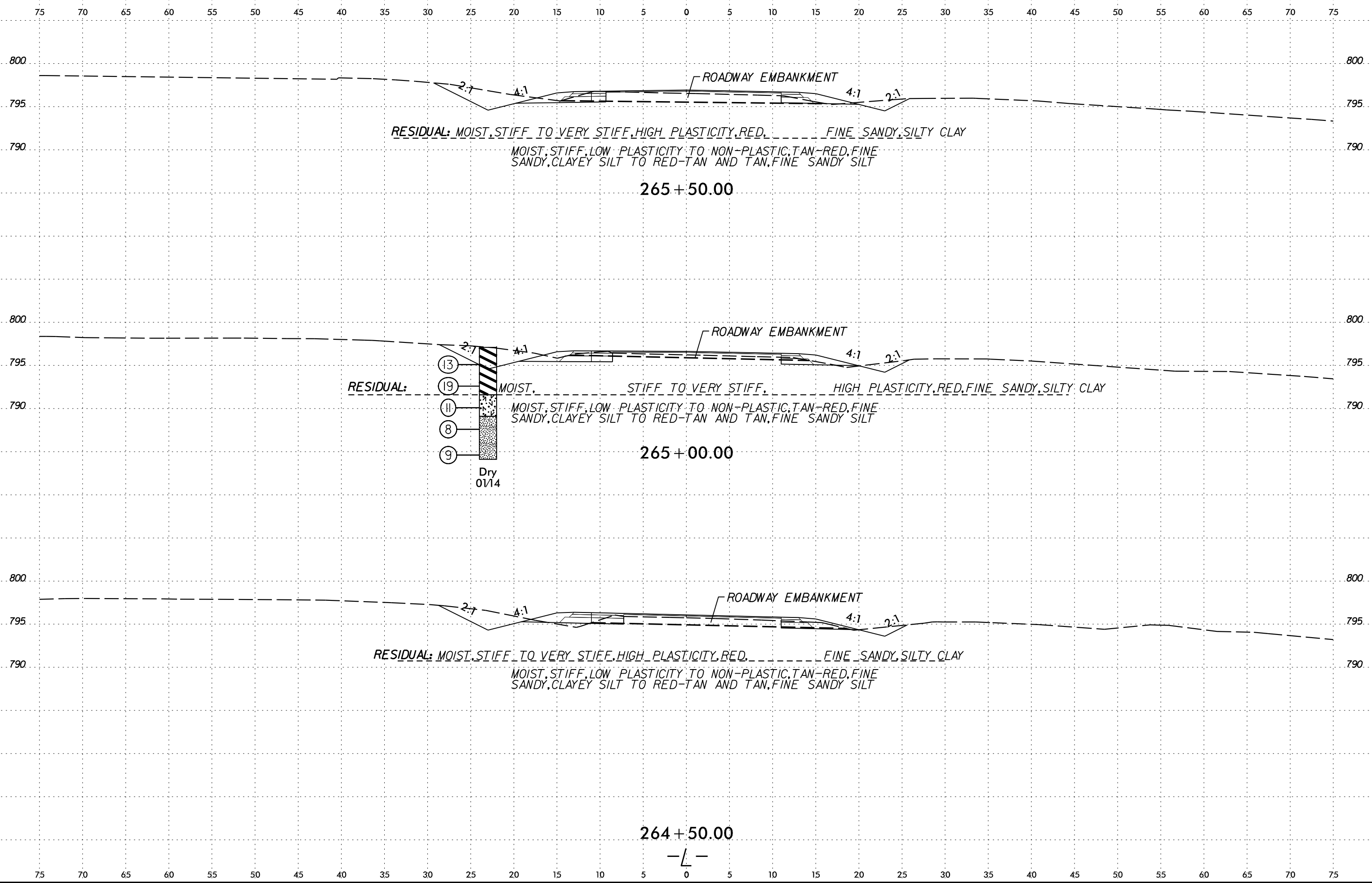


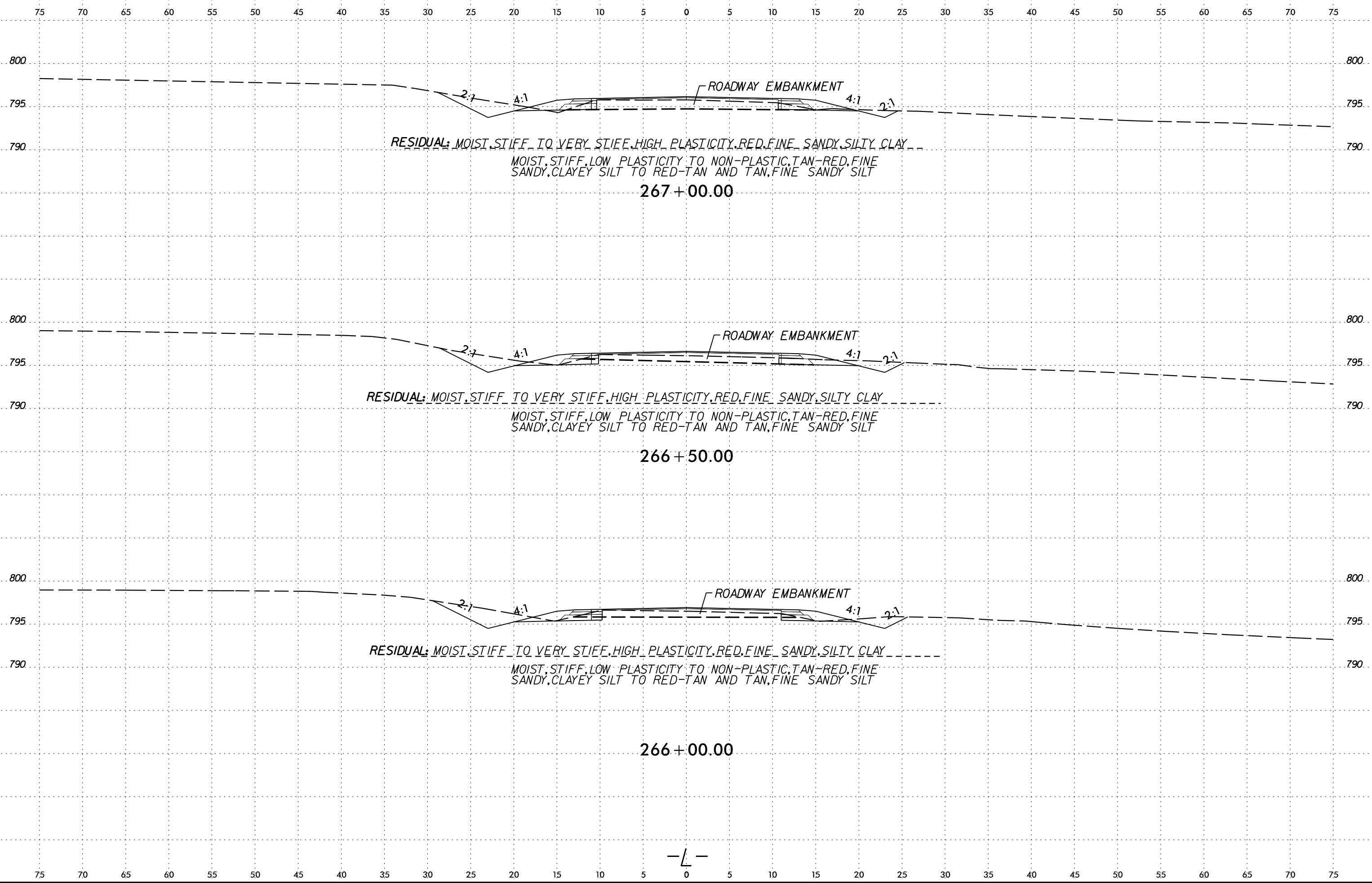


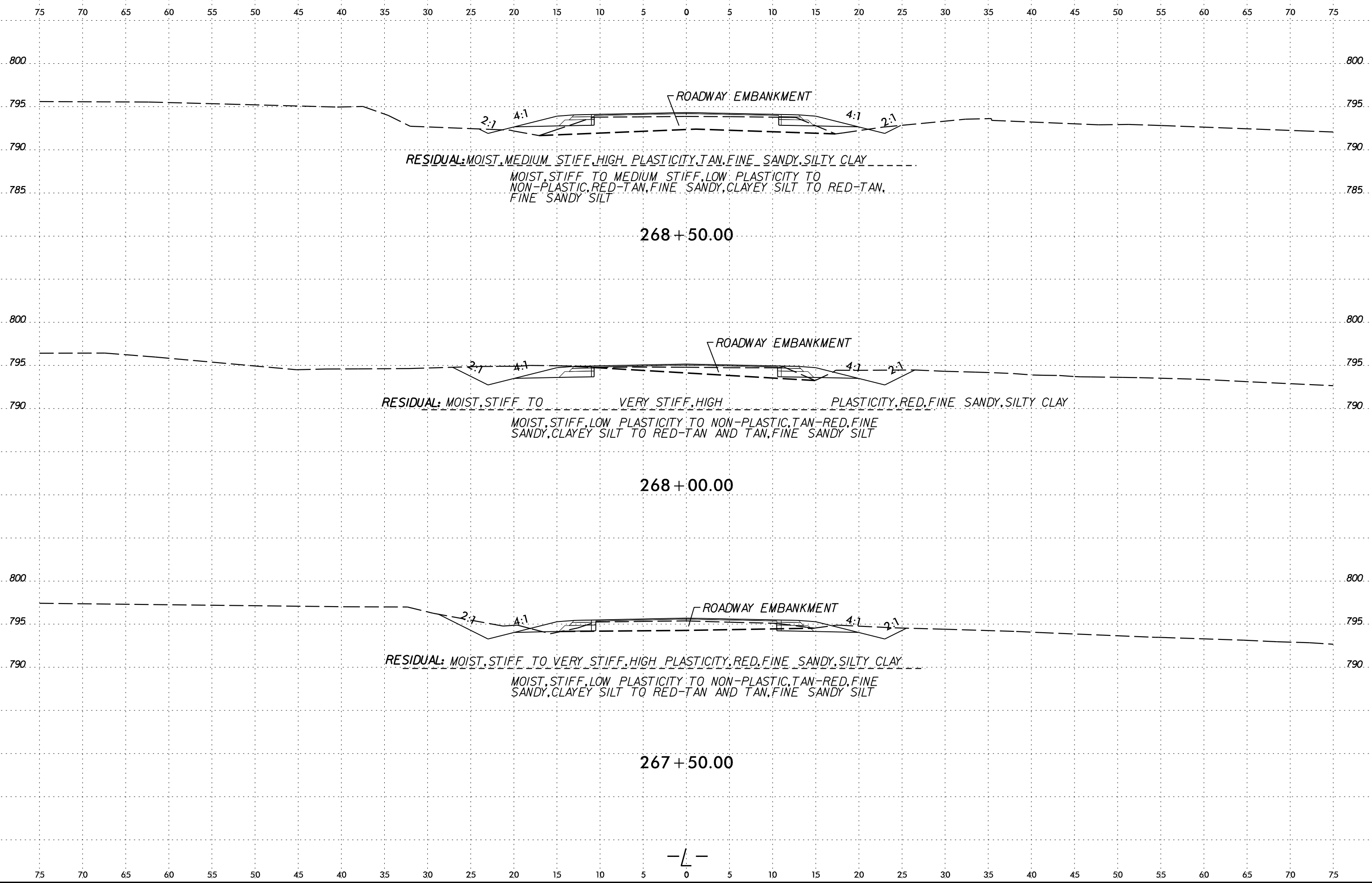


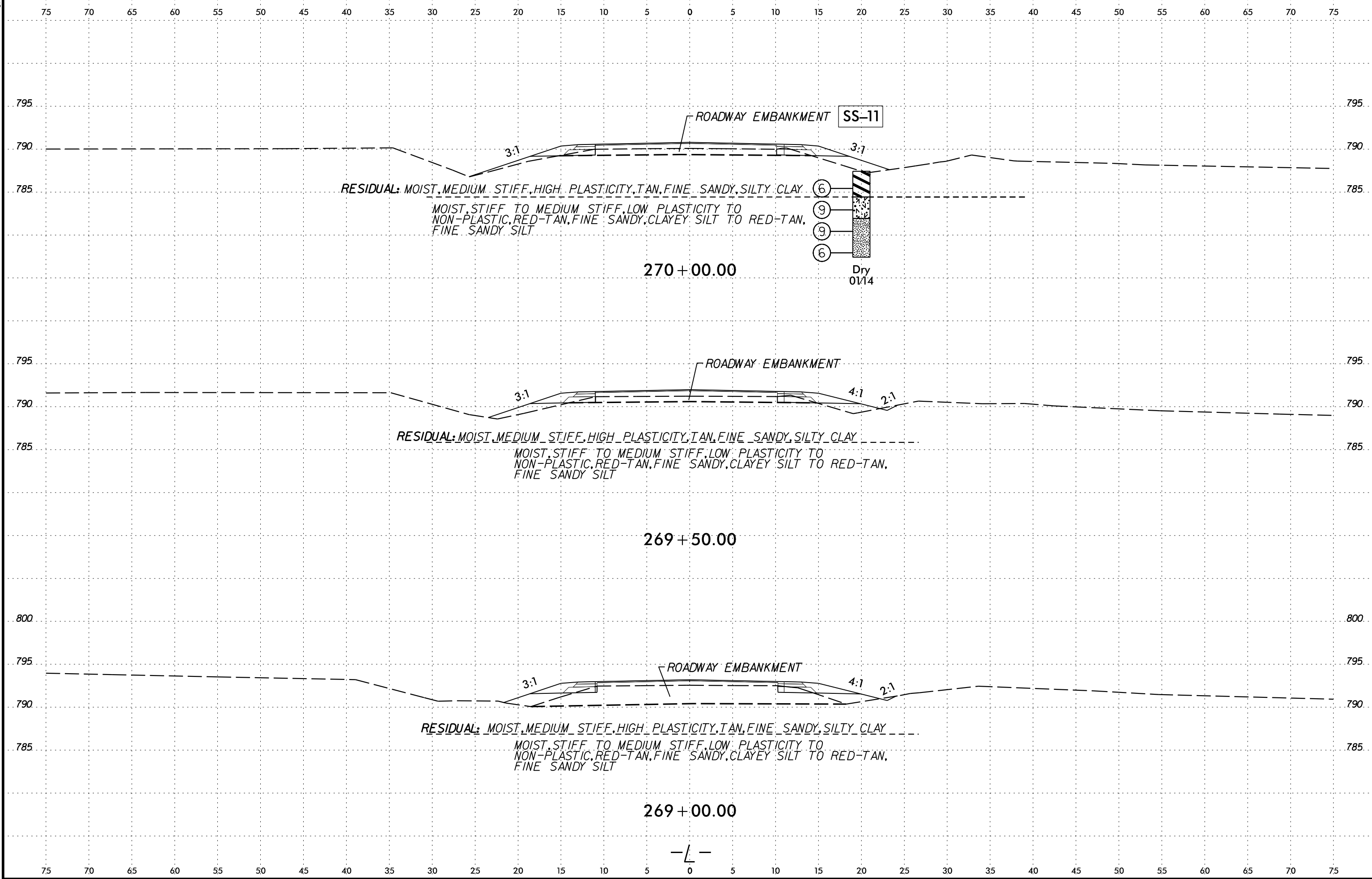










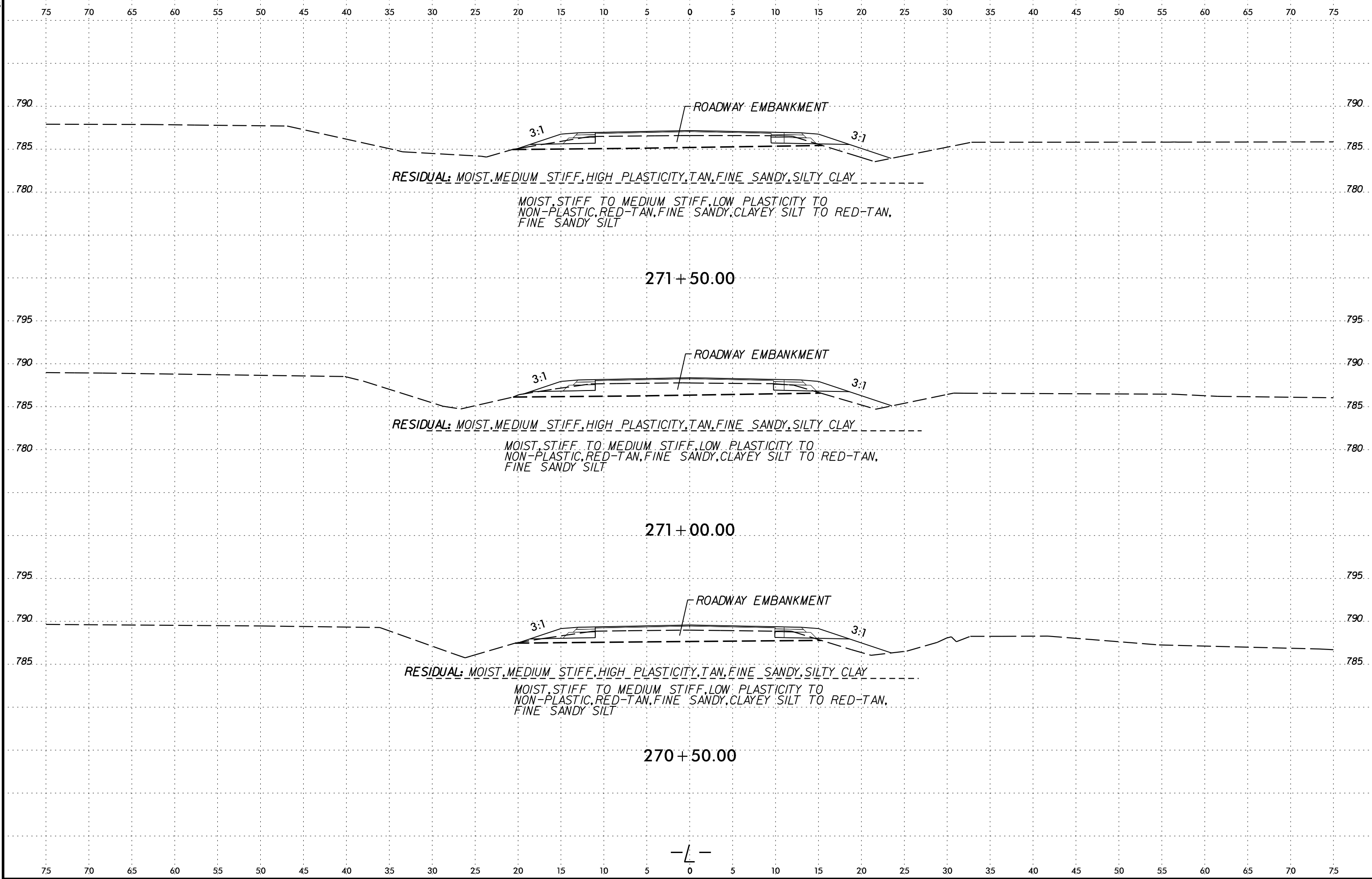


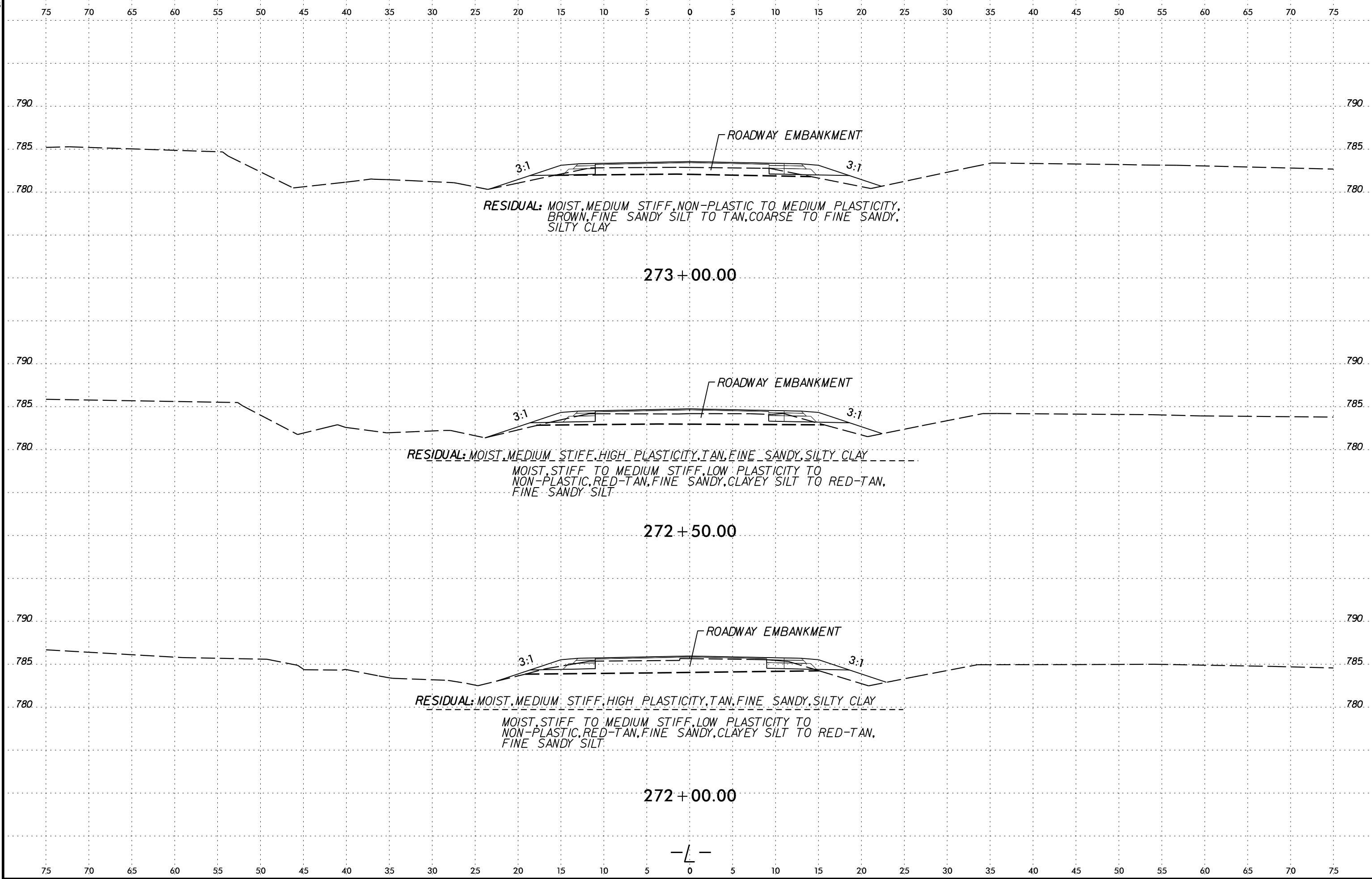
270 + 00.00

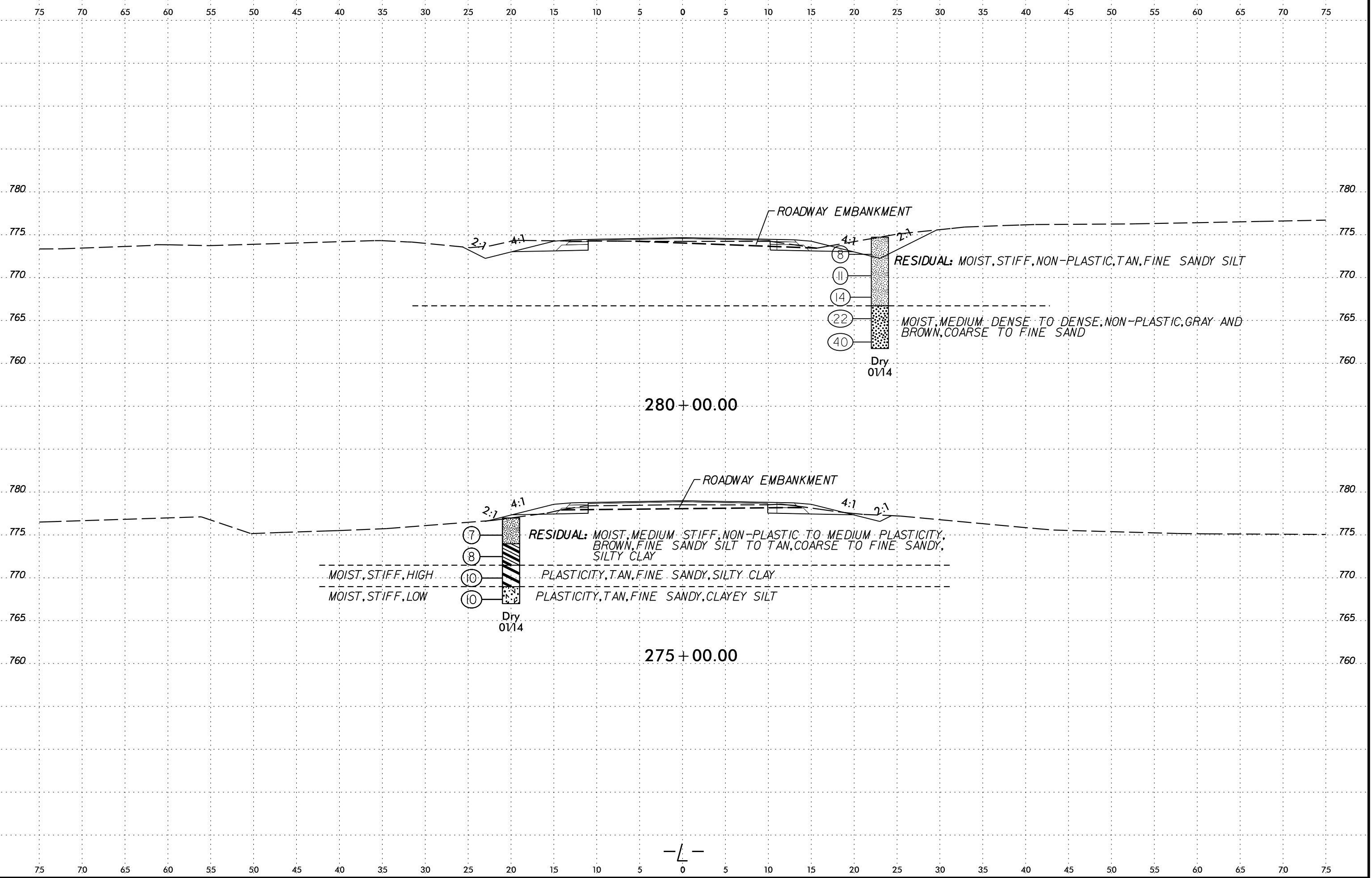
269 + 50.00

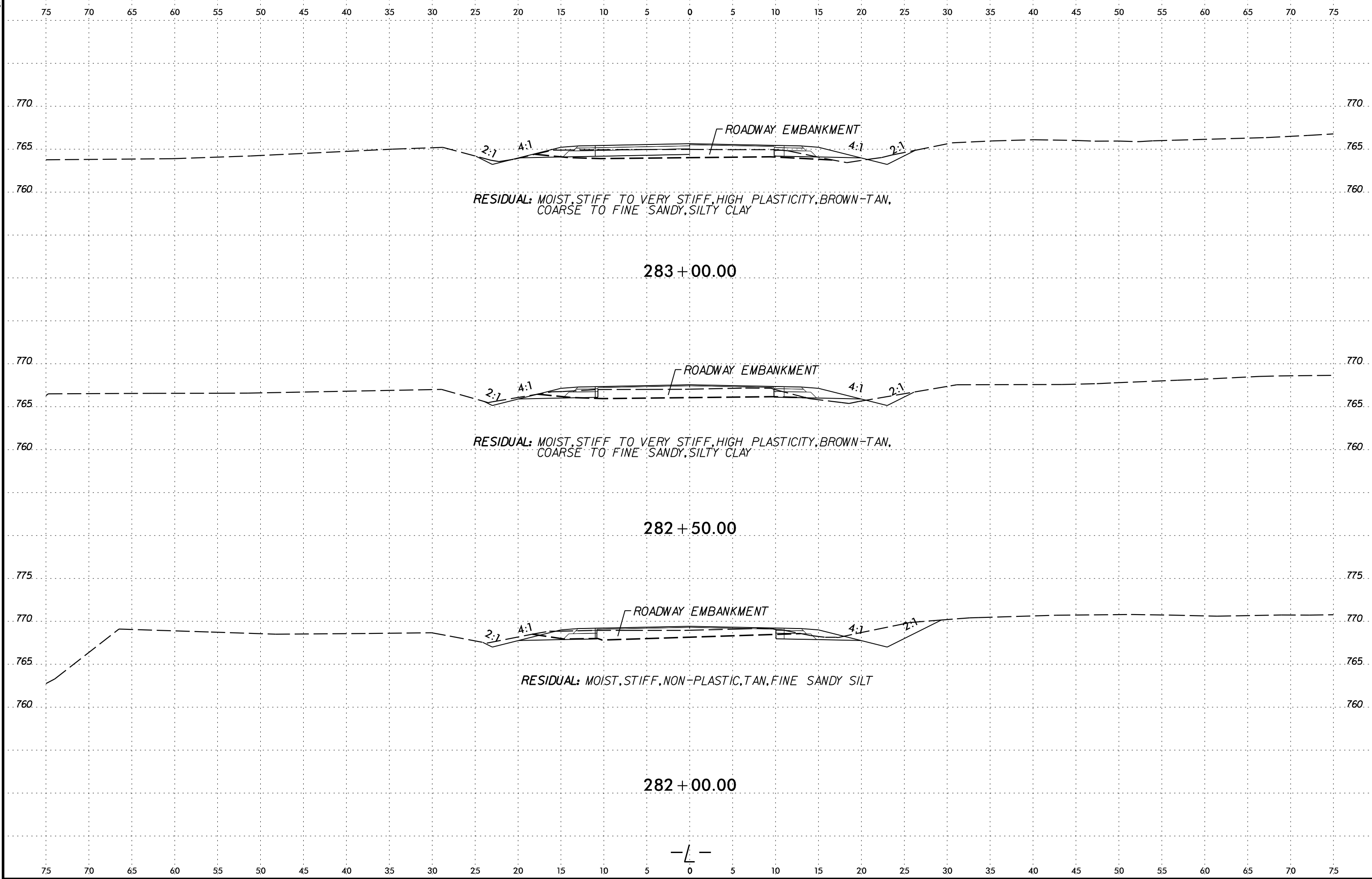
269 + 00.00

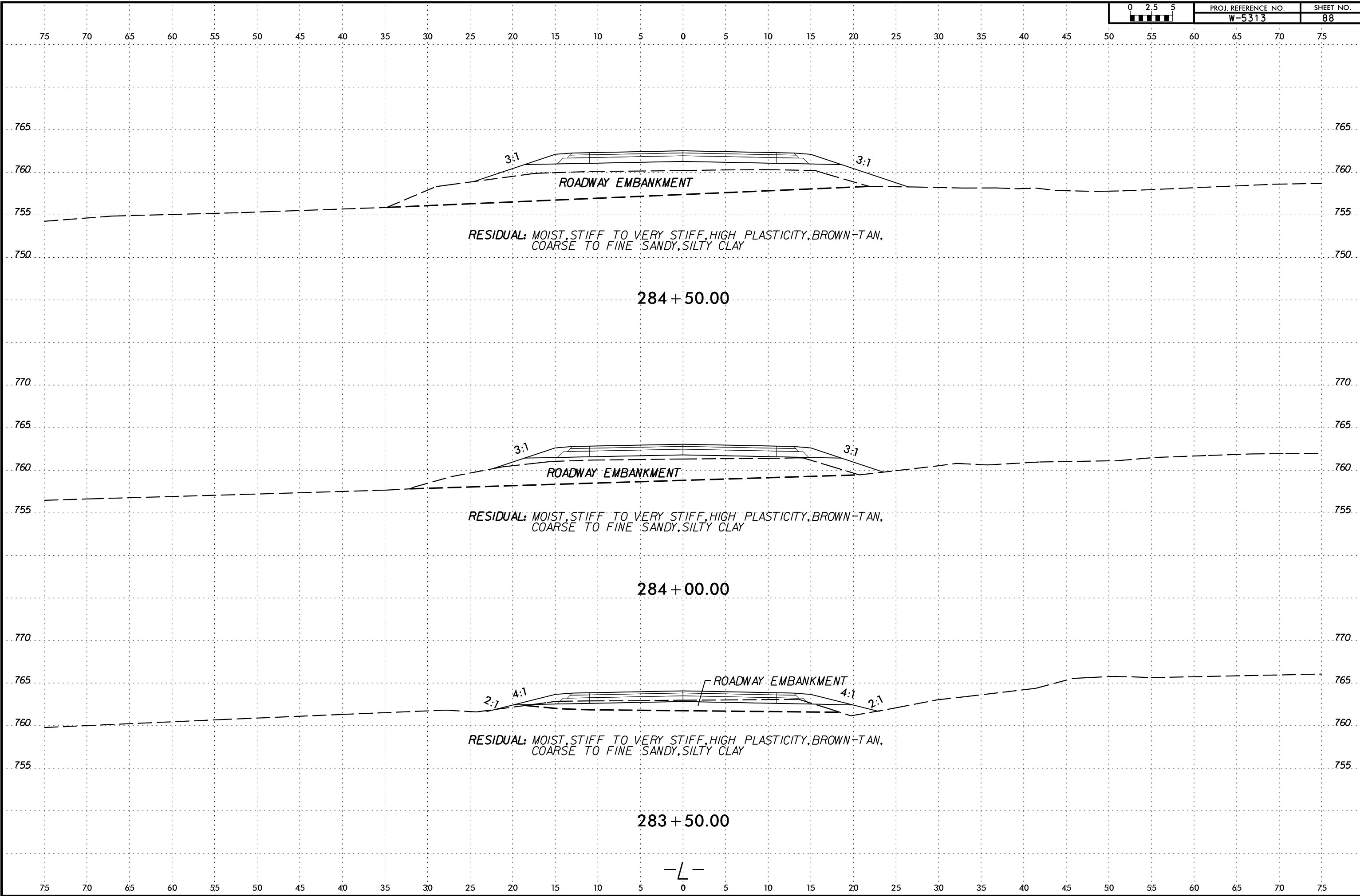
— 25 —

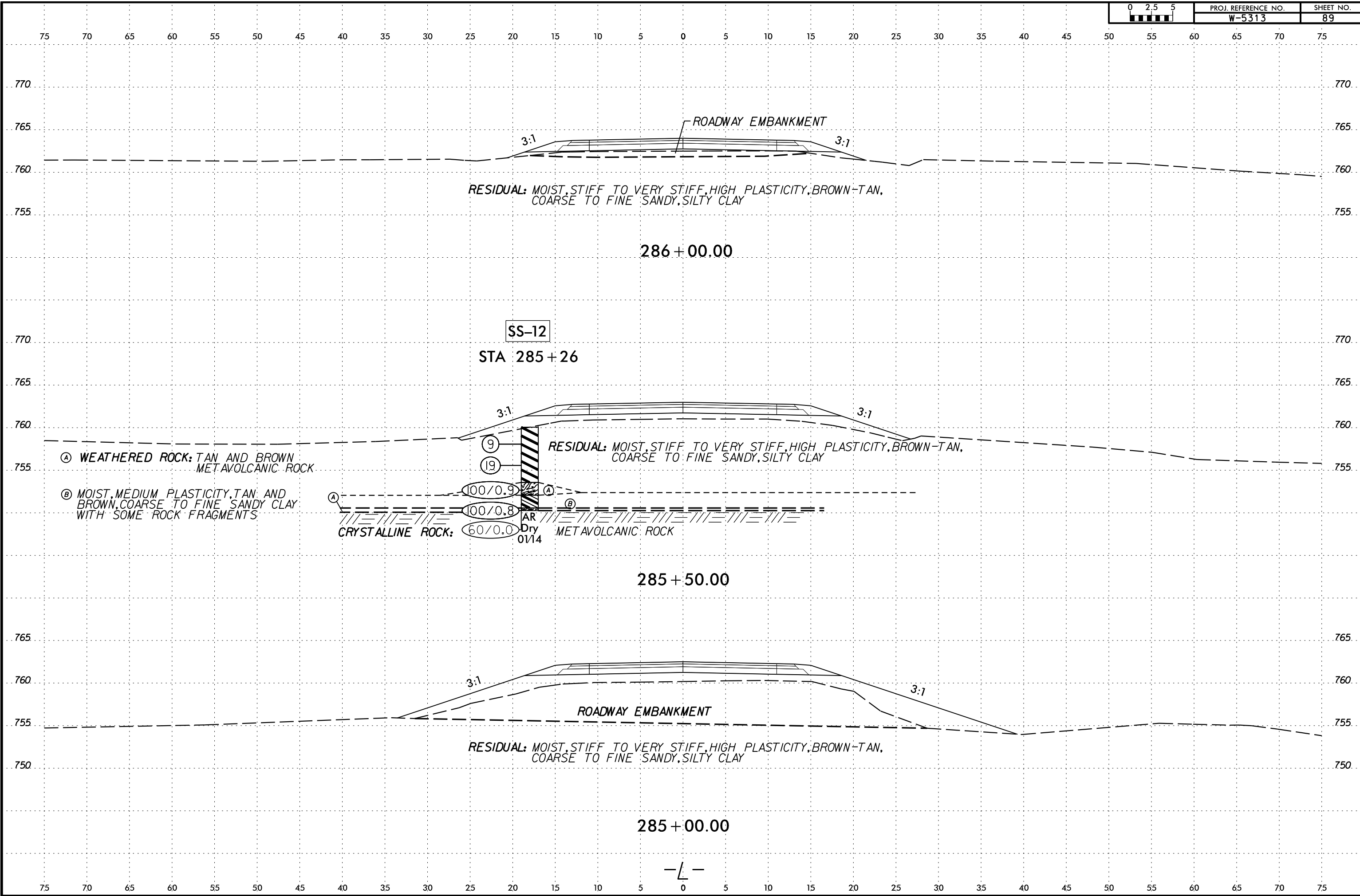


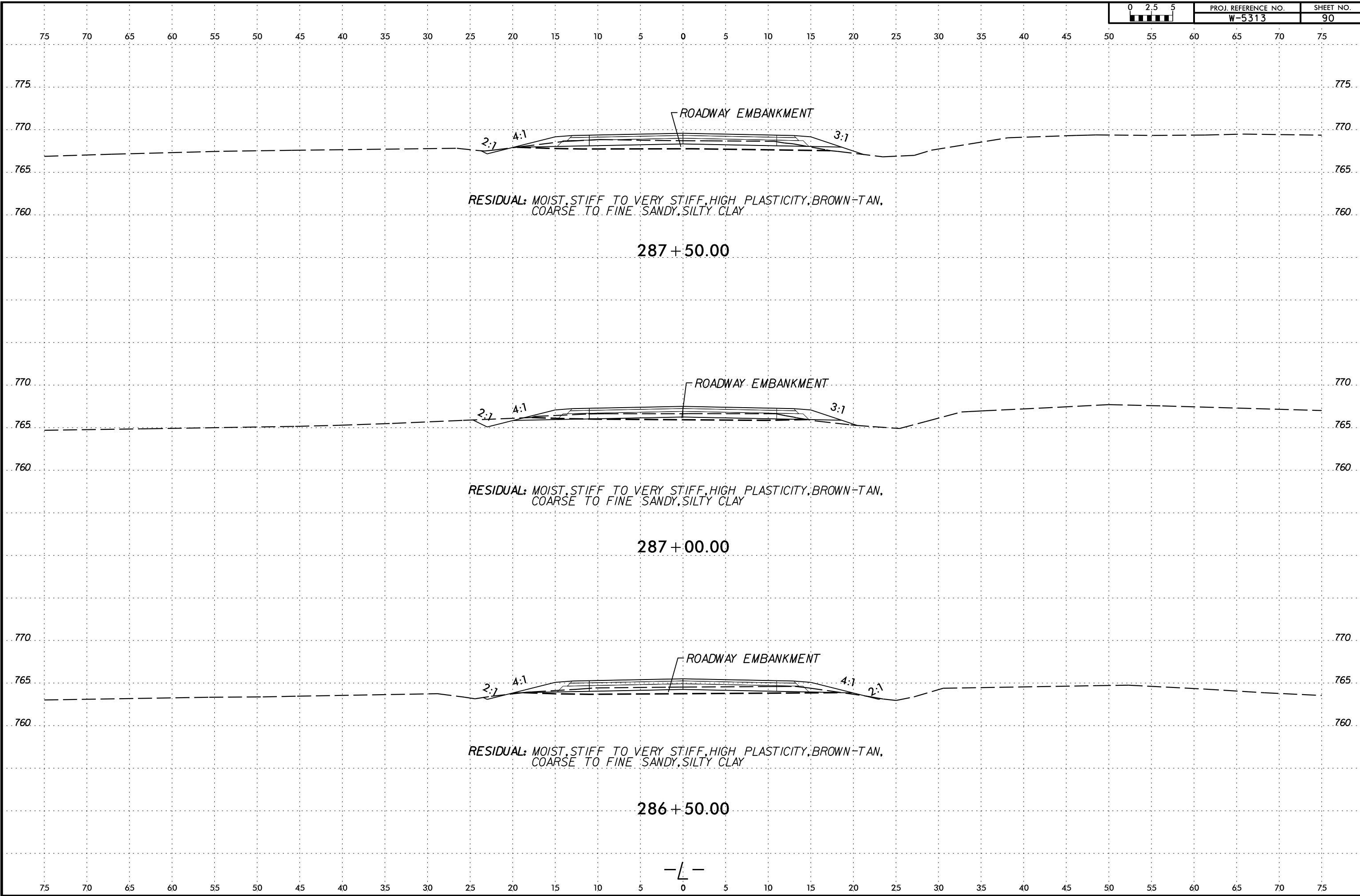


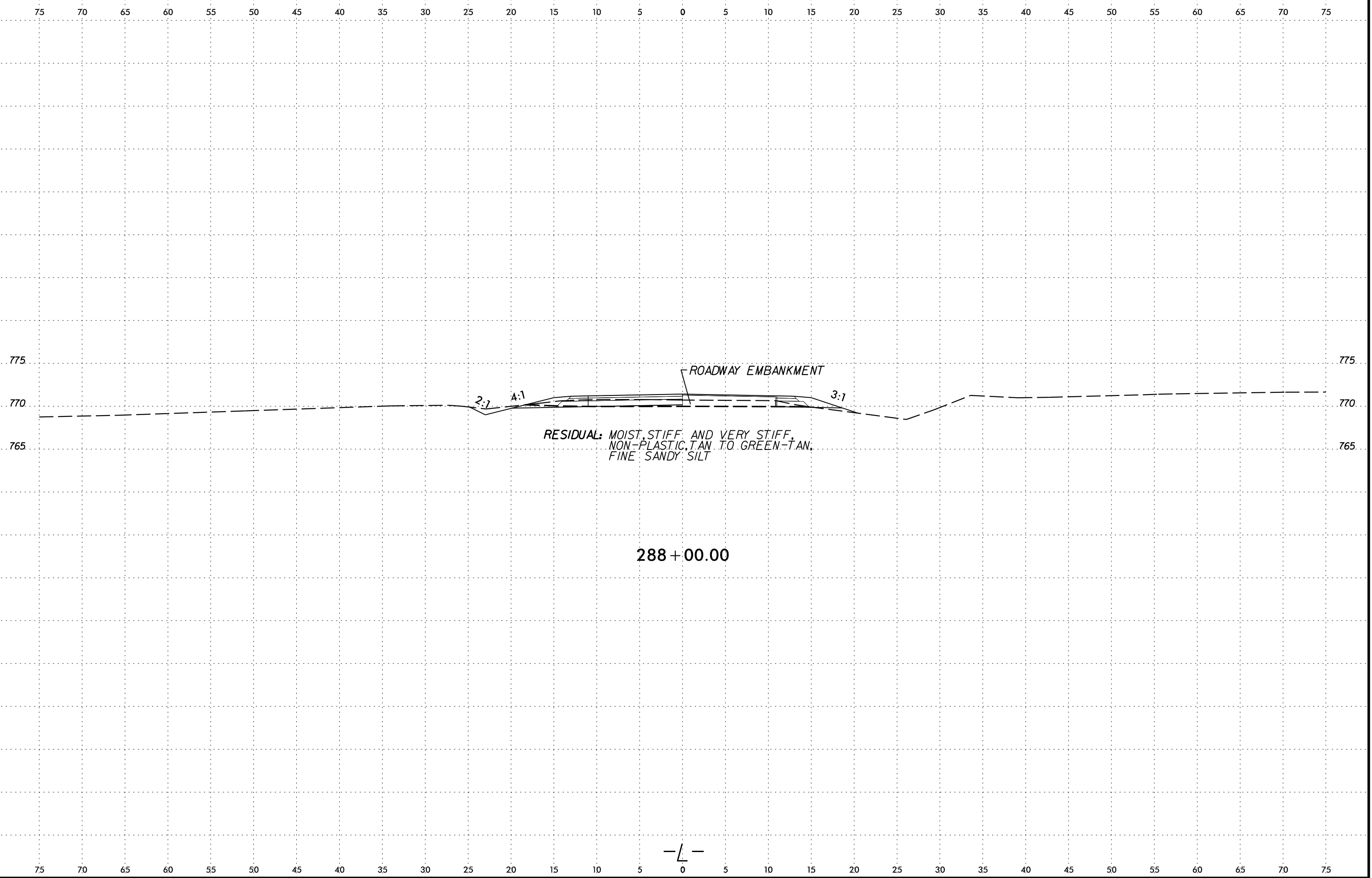








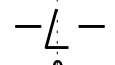


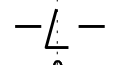
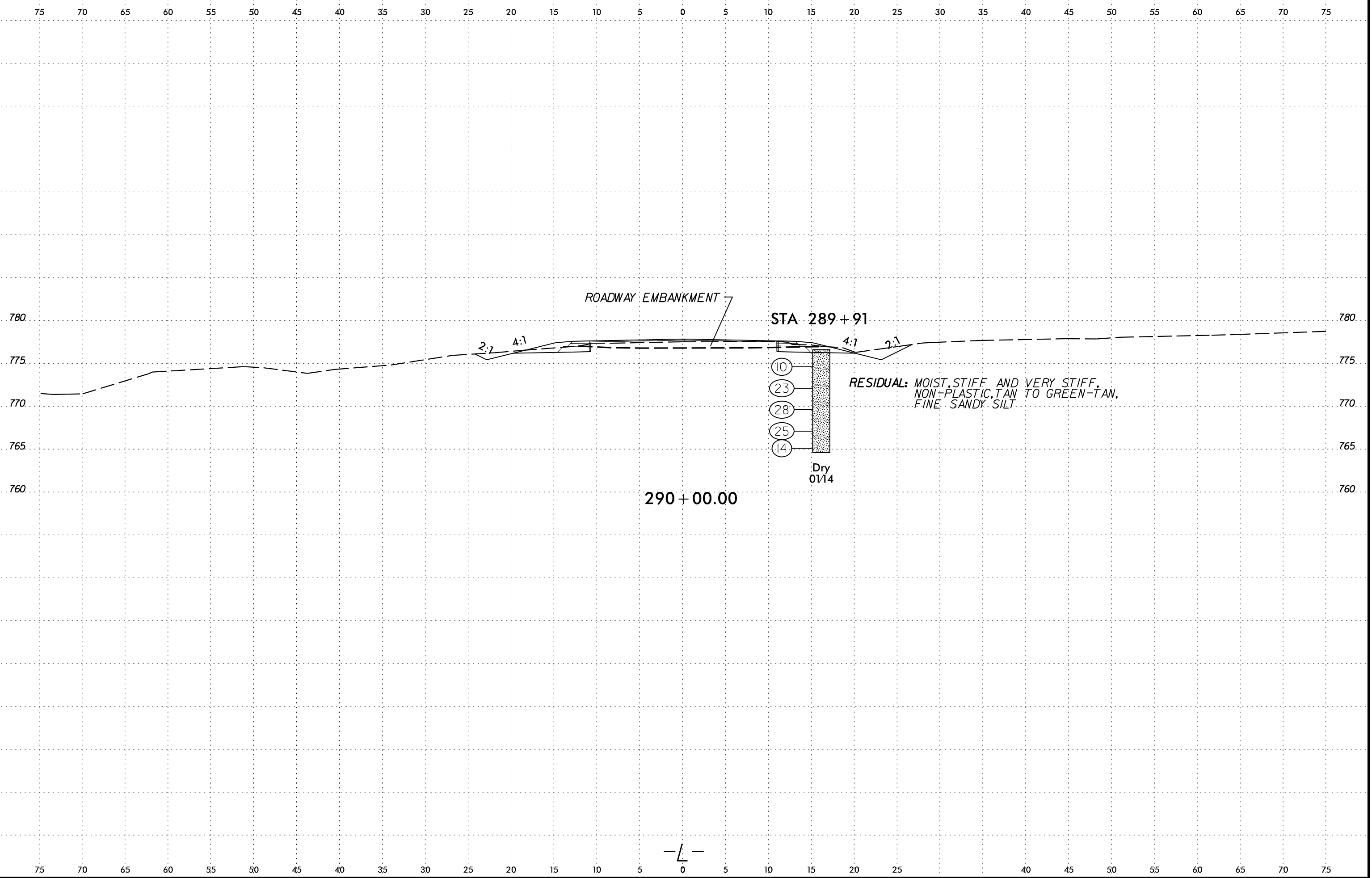


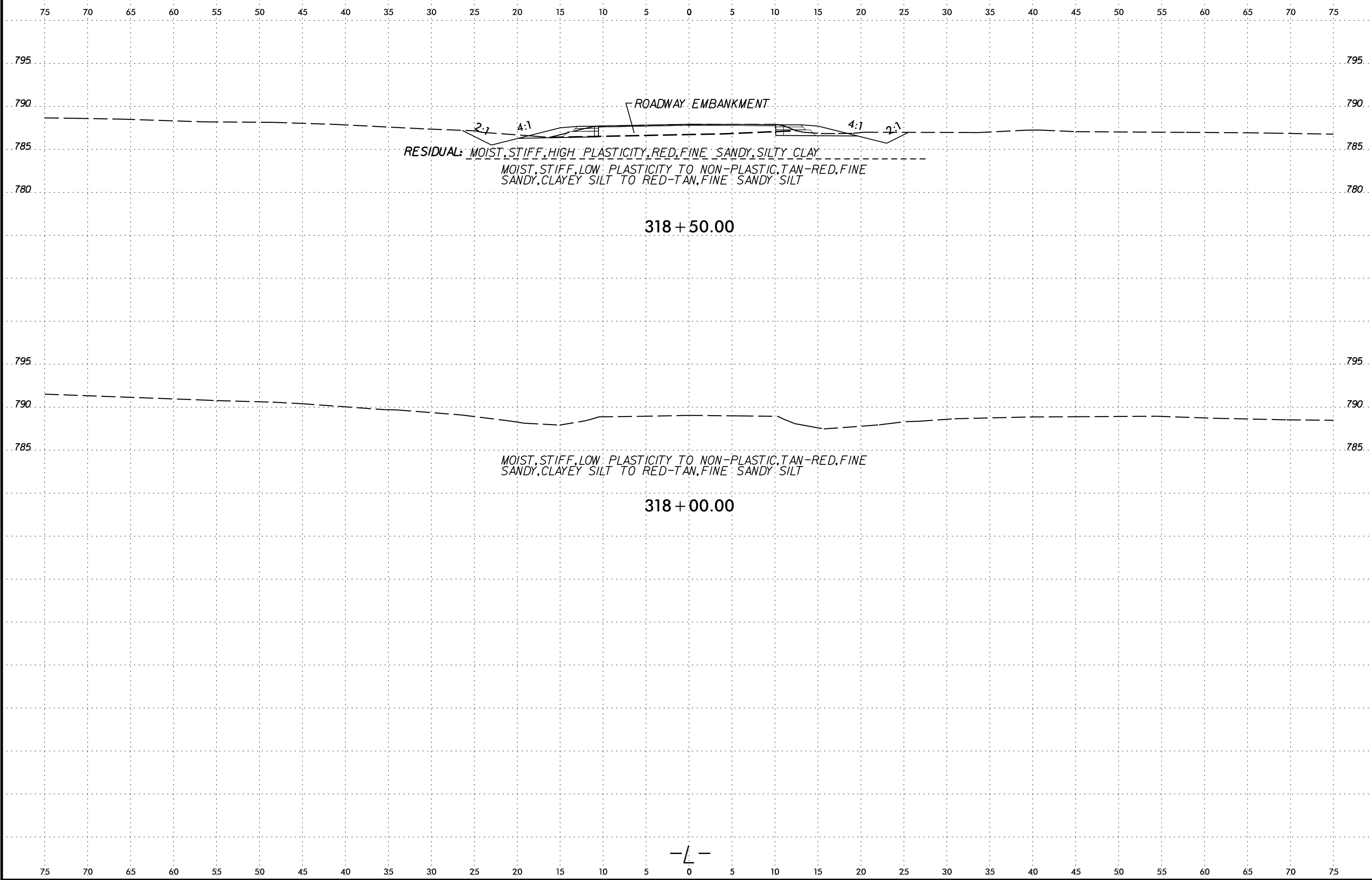
*RESIDUAL: MOIST, STIFF AND VERY STIFF,
NON-PLASTIC, TAN TO GREEN-TAN,
FINE SANDY SILT*

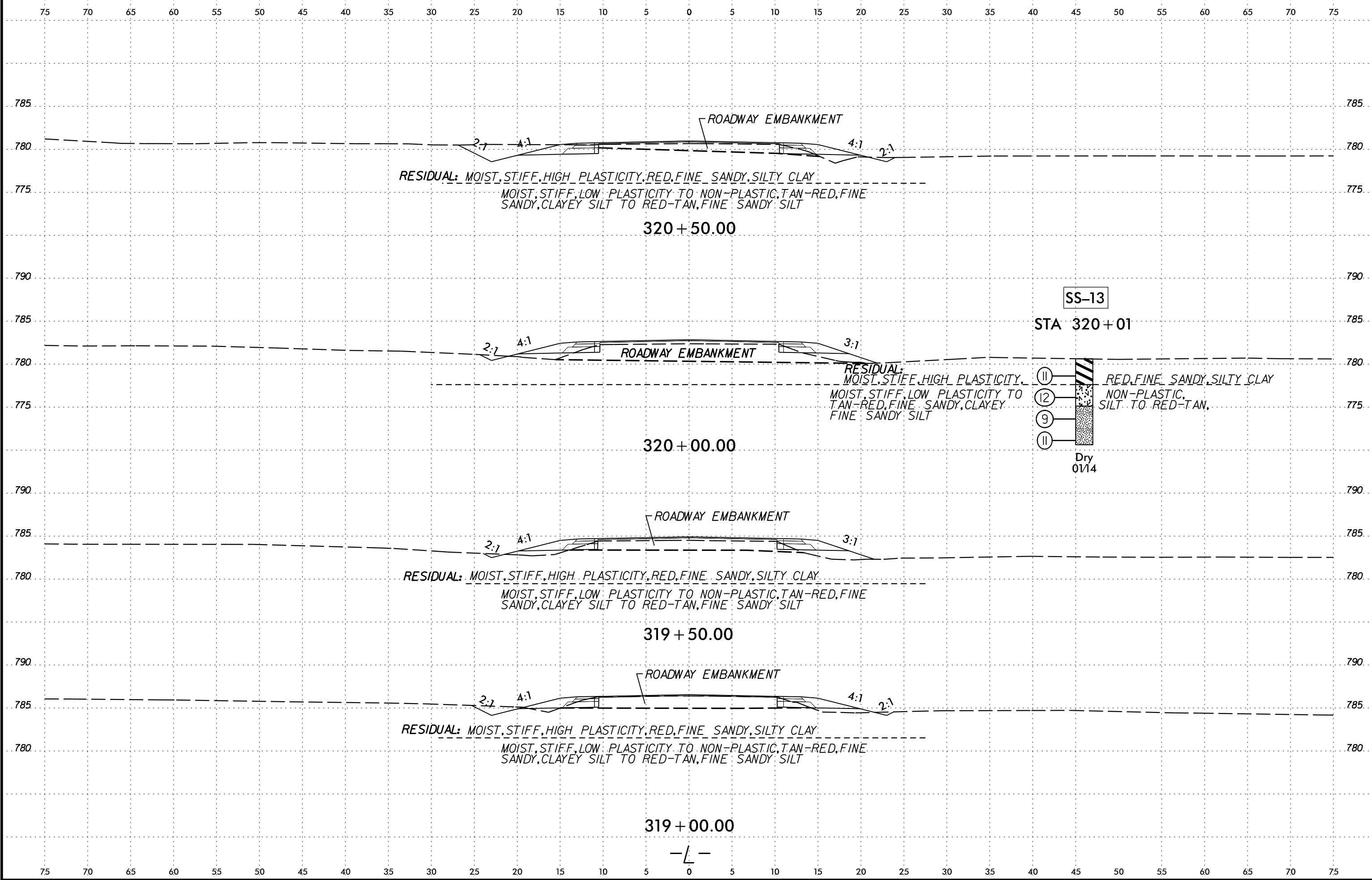
ROADWAY EMBANKMENT

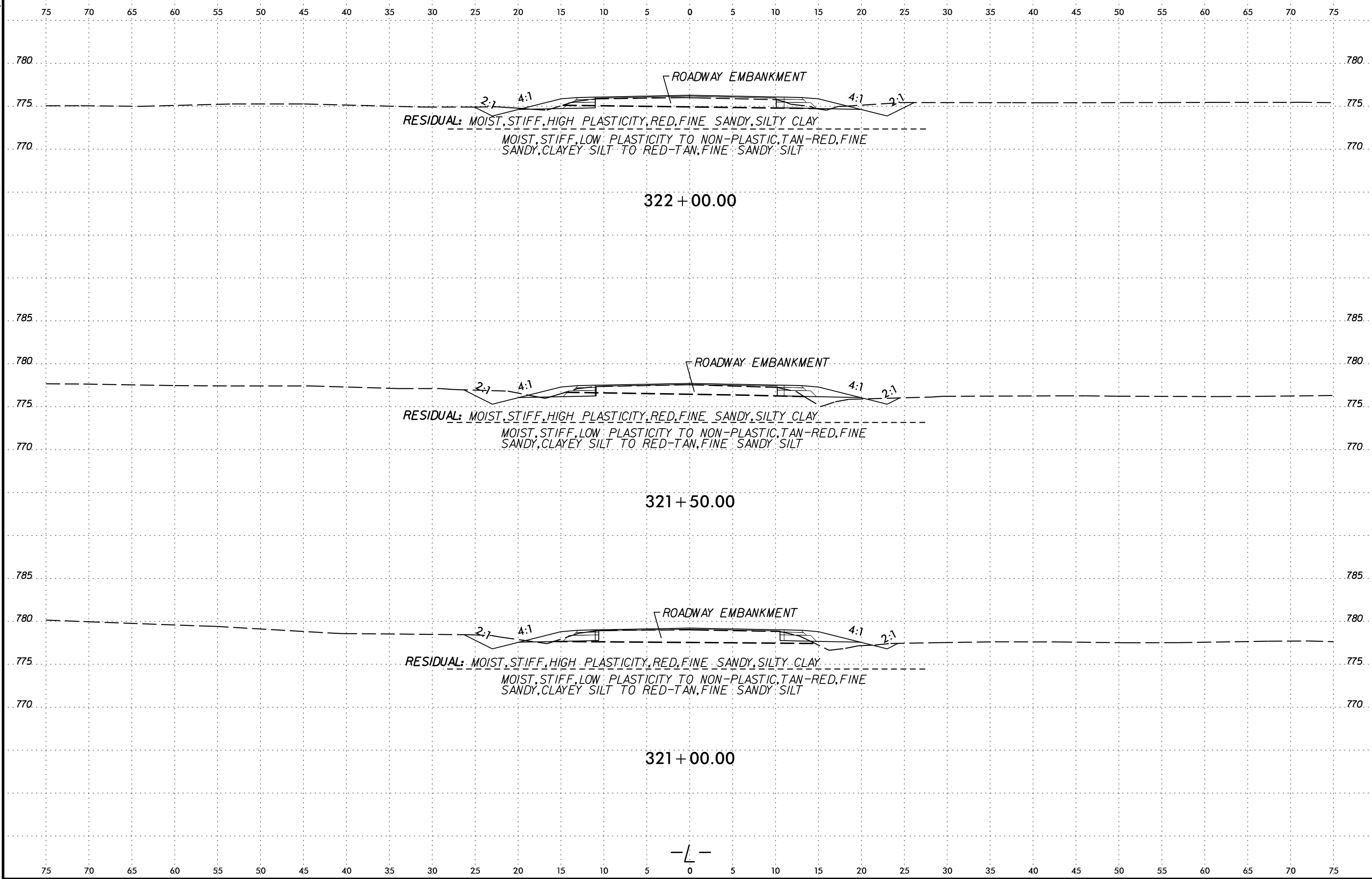
288 + 00.00

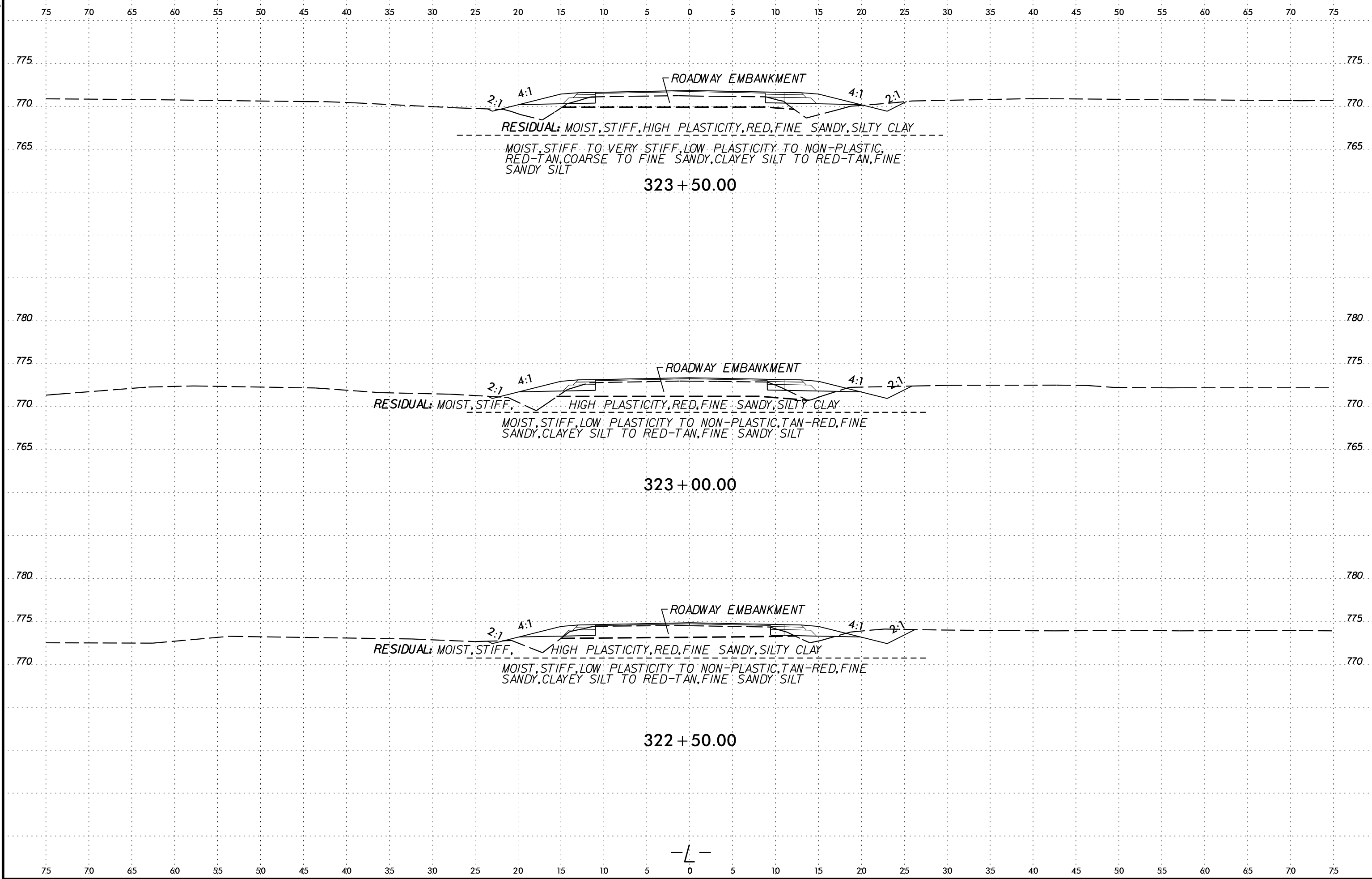


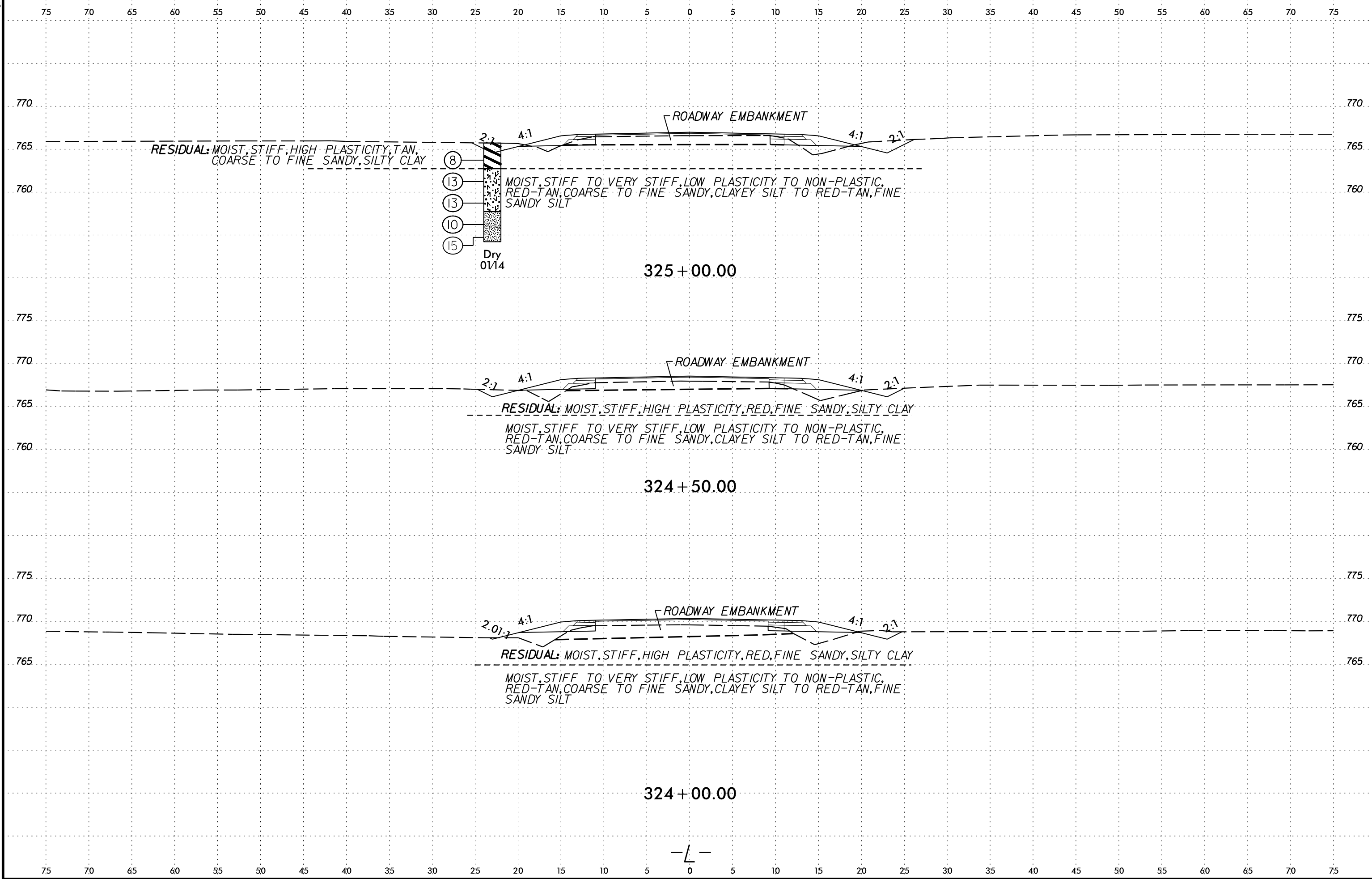












RESIDUAL: MOIST, STIFF, HIGH PLASTICITY, TAN,
COARSE TO FINE SANDY, SILTY CLAY

- (8)
- (13)
- (13)
- (10)
- (15)

MOIST, STIFF TO VERY STIFF, LOW PLASTICITY TO NON-PLASTIC,
RED-TAN, COARSE TO FINE SANDY, CLAYEY SILT TO RED-TAN, FINE
SANDY SILT

Dry
01/14

325 + 00.00

ROADWAY EMBANKMENT

RESIDUAL: MOIST, STIFF, HIGH PLASTICITY, RED, FINE SANDY, SILTY CLAY

MOIST, STIFF TO VERY STIFF, LOW PLASTICITY TO NON-PLASTIC,
RED-TAN, COARSE TO FINE SANDY, CLAYEY SILT TO RED-TAN, FINE
SANDY SILT

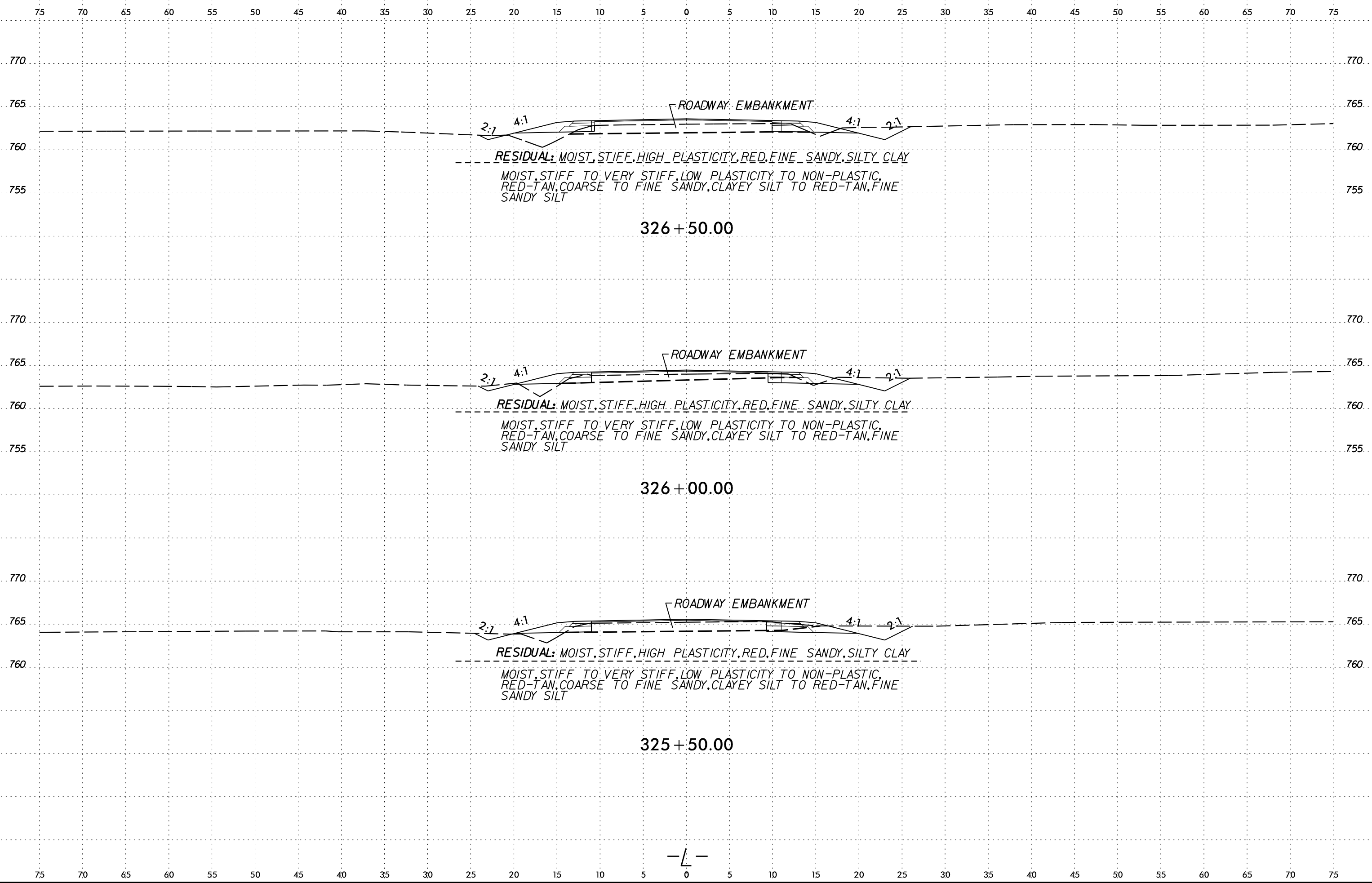
324 + 50.00

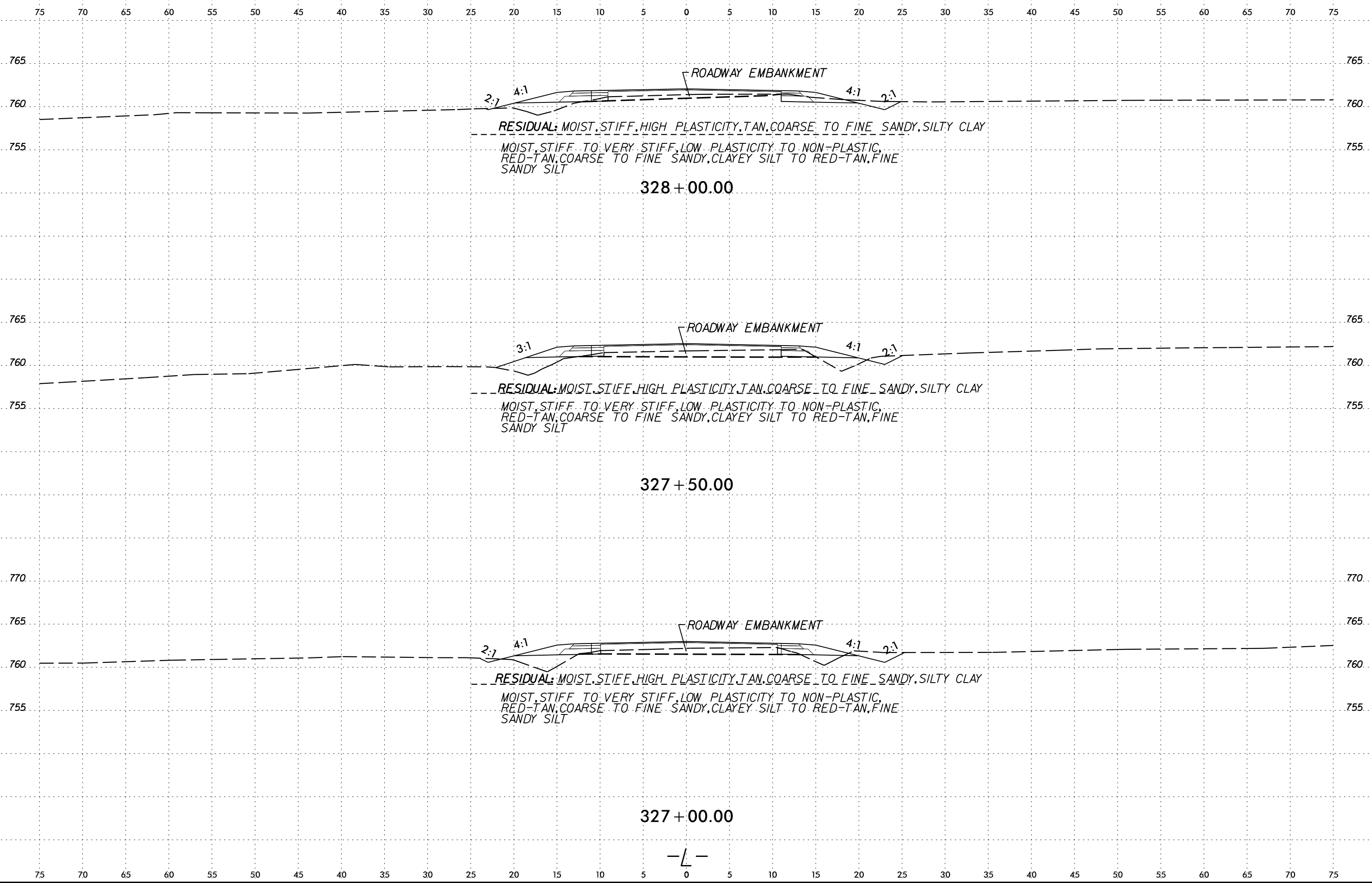
ROADWAY EMBANKMENT

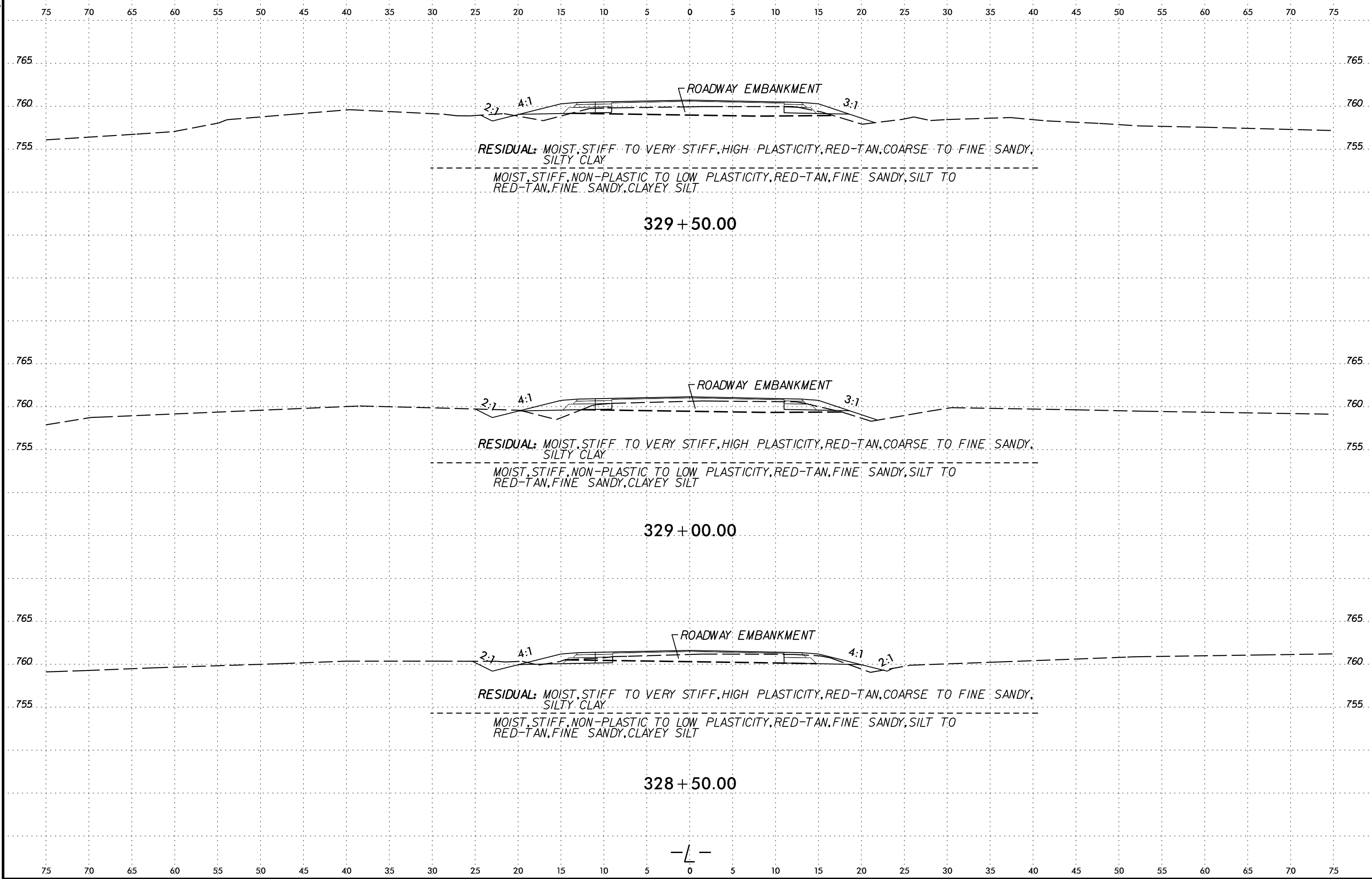
RESIDUAL: MOIST, STIFF, HIGH PLASTICITY, RED, FINE SANDY, SILTY CLAY

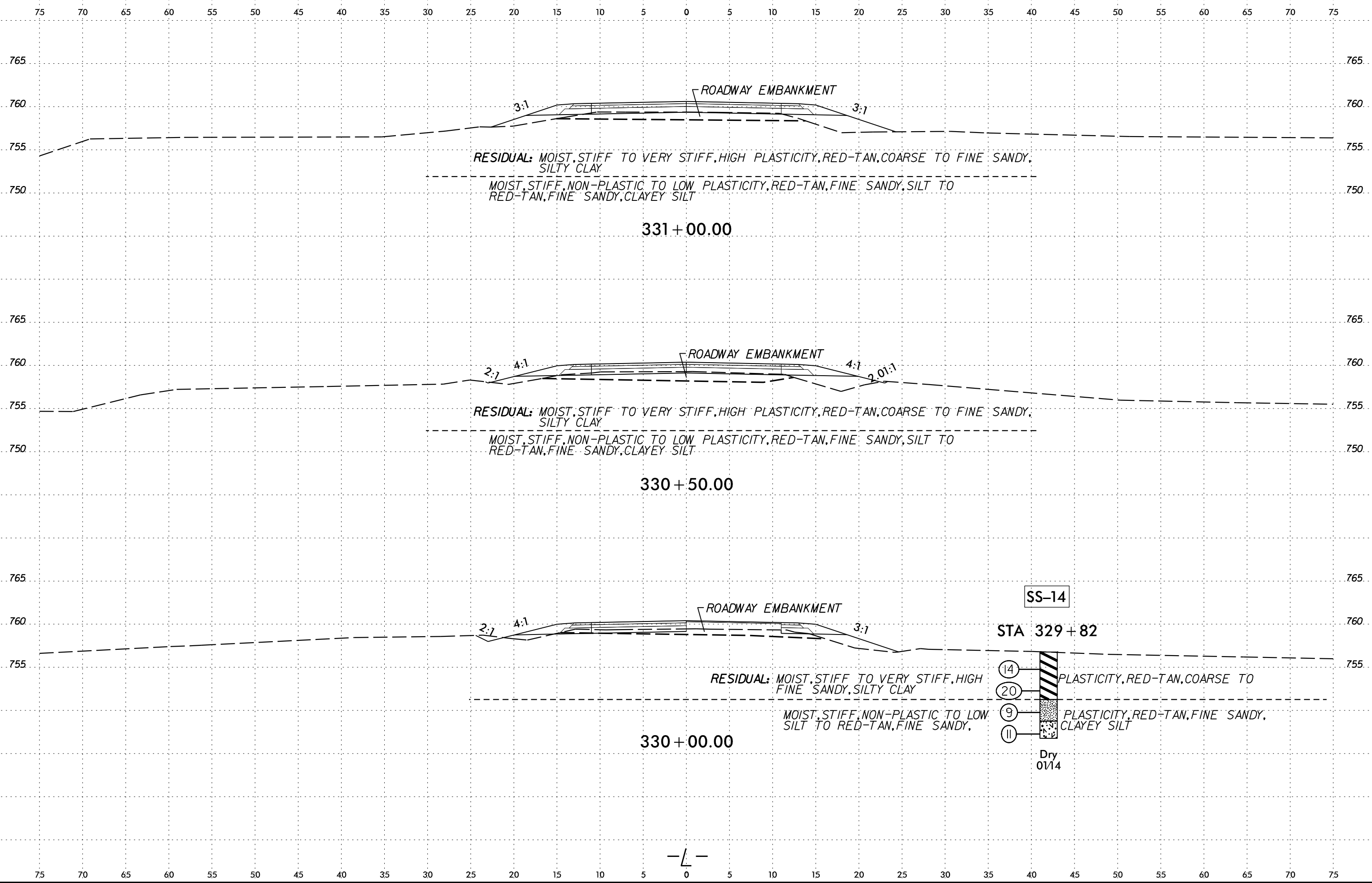
MOIST, STIFF TO VERY STIFF, LOW PLASTICITY TO NON-PLASTIC,
RED-TAN, COARSE TO FINE SANDY, CLAYEY SILT TO RED-TAN, FINE
SANDY SILT

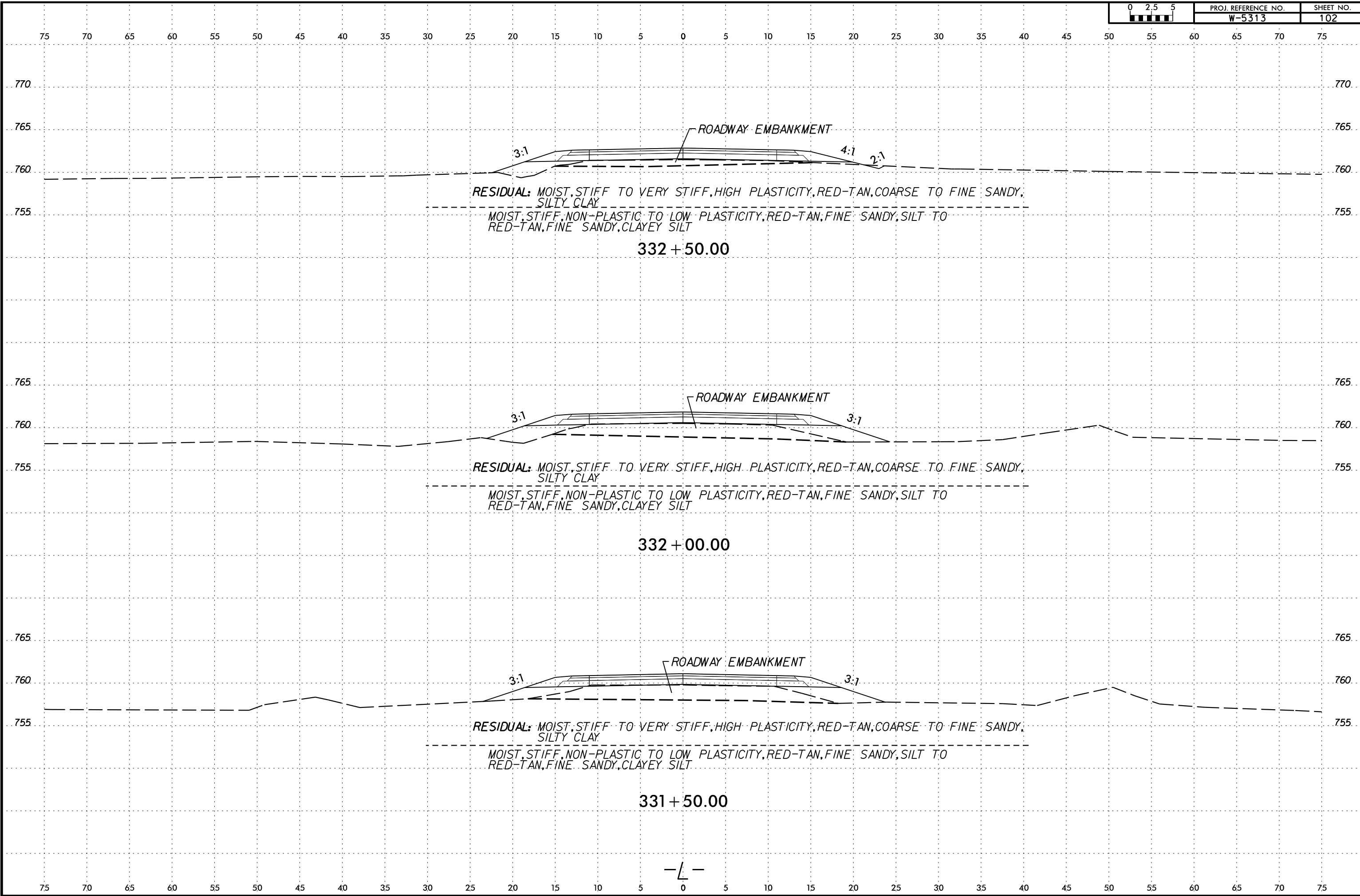
324 + 00.00

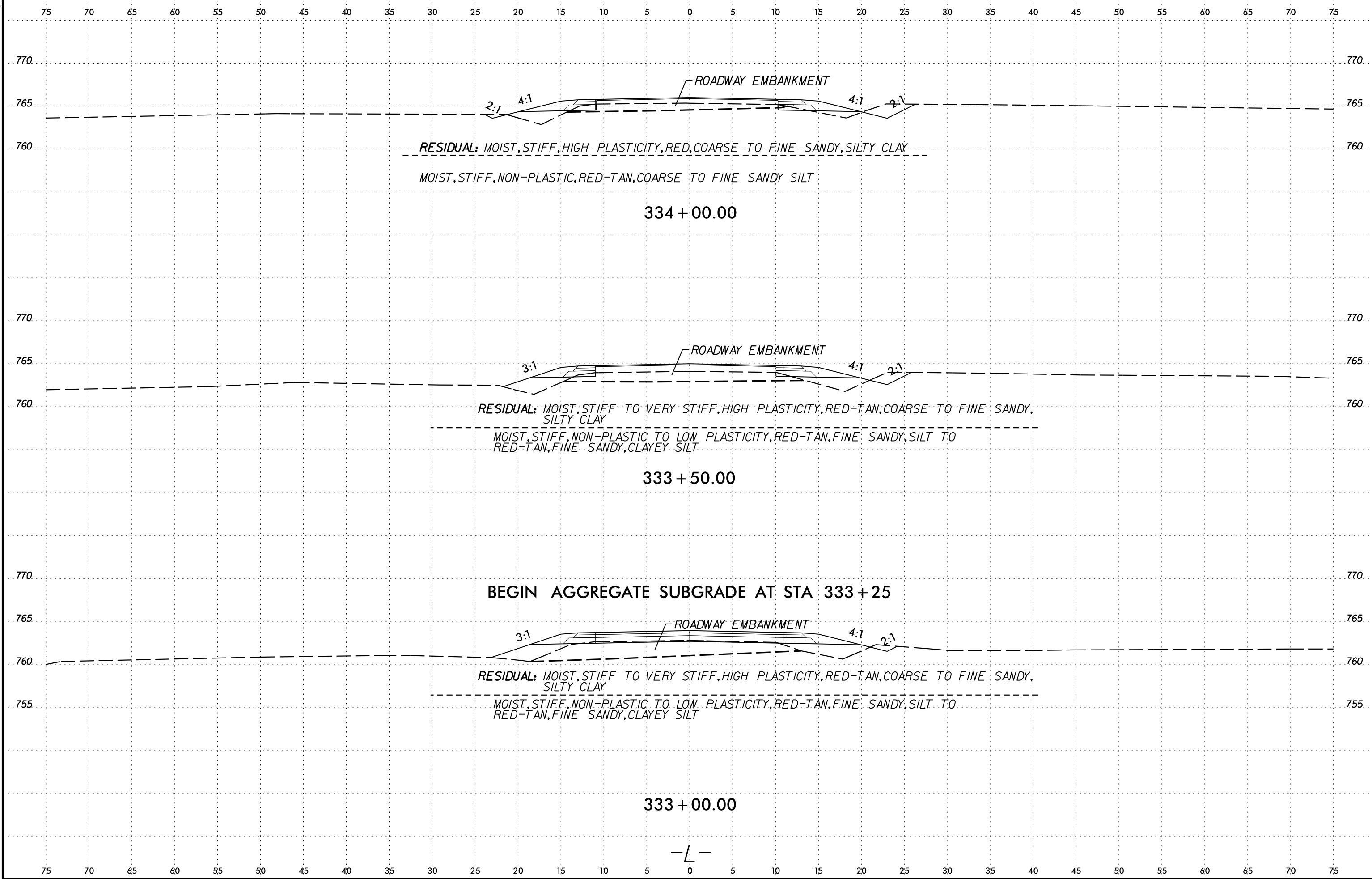












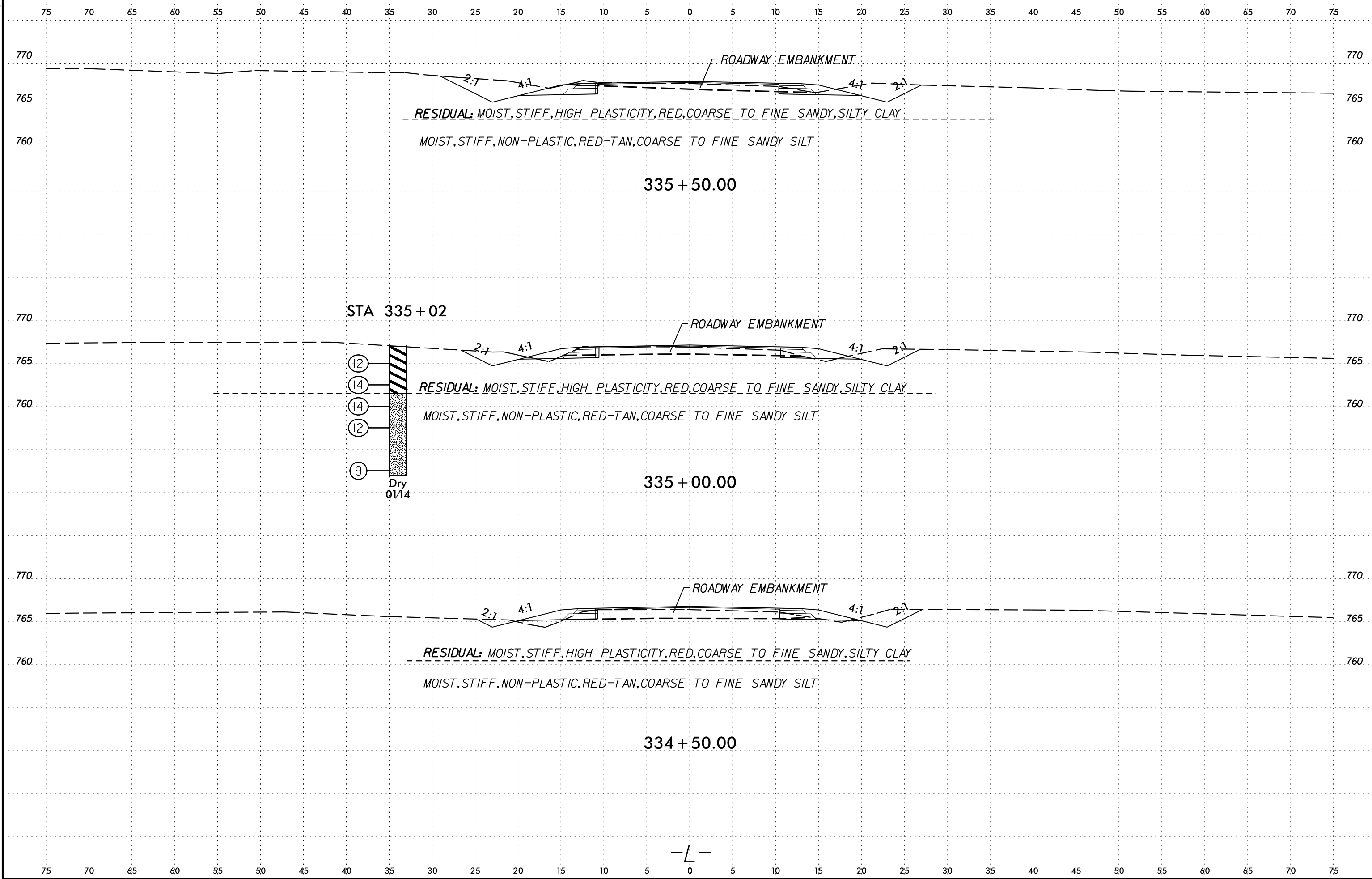
334 + 00.00

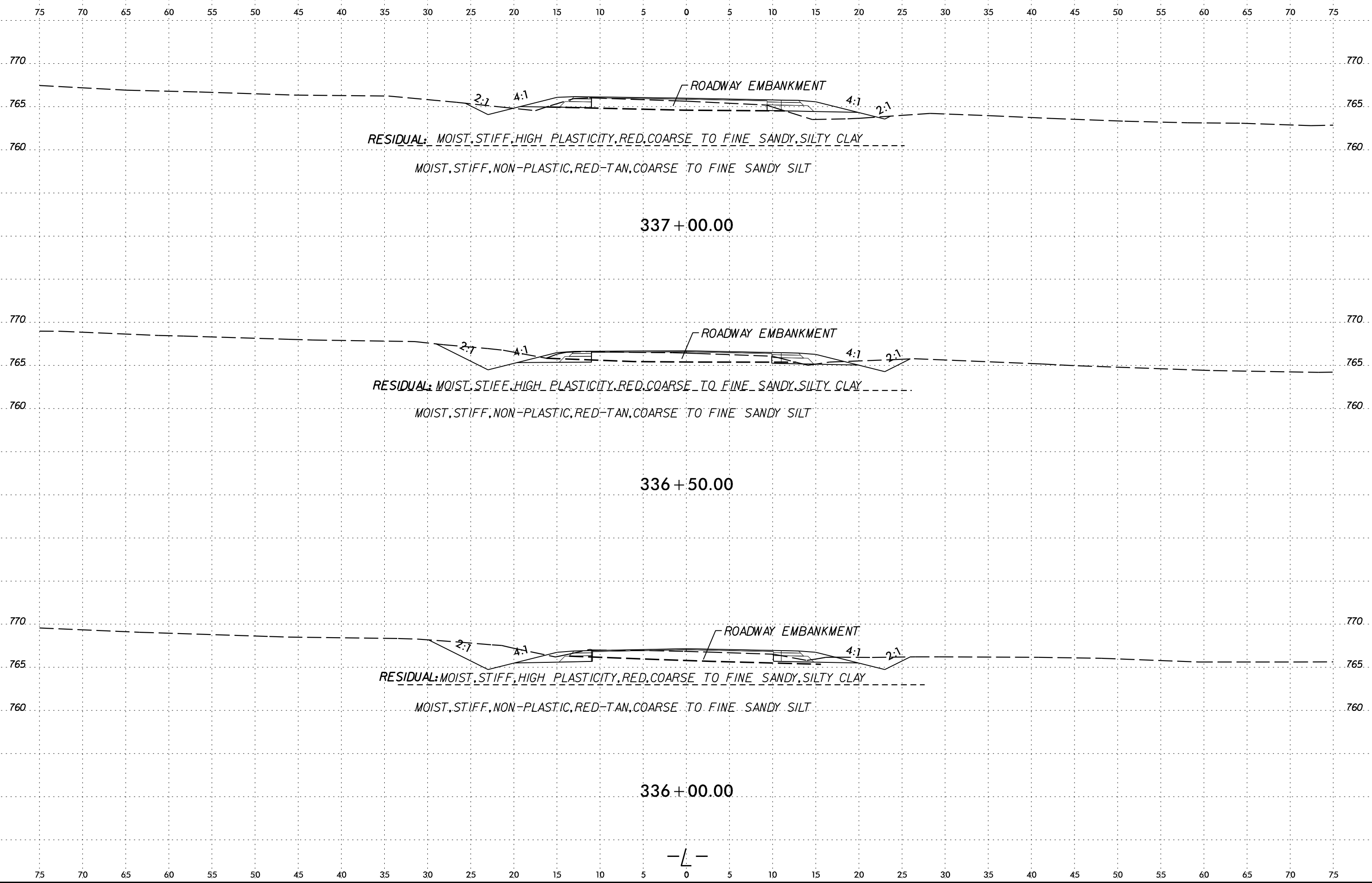
333 + 50.00

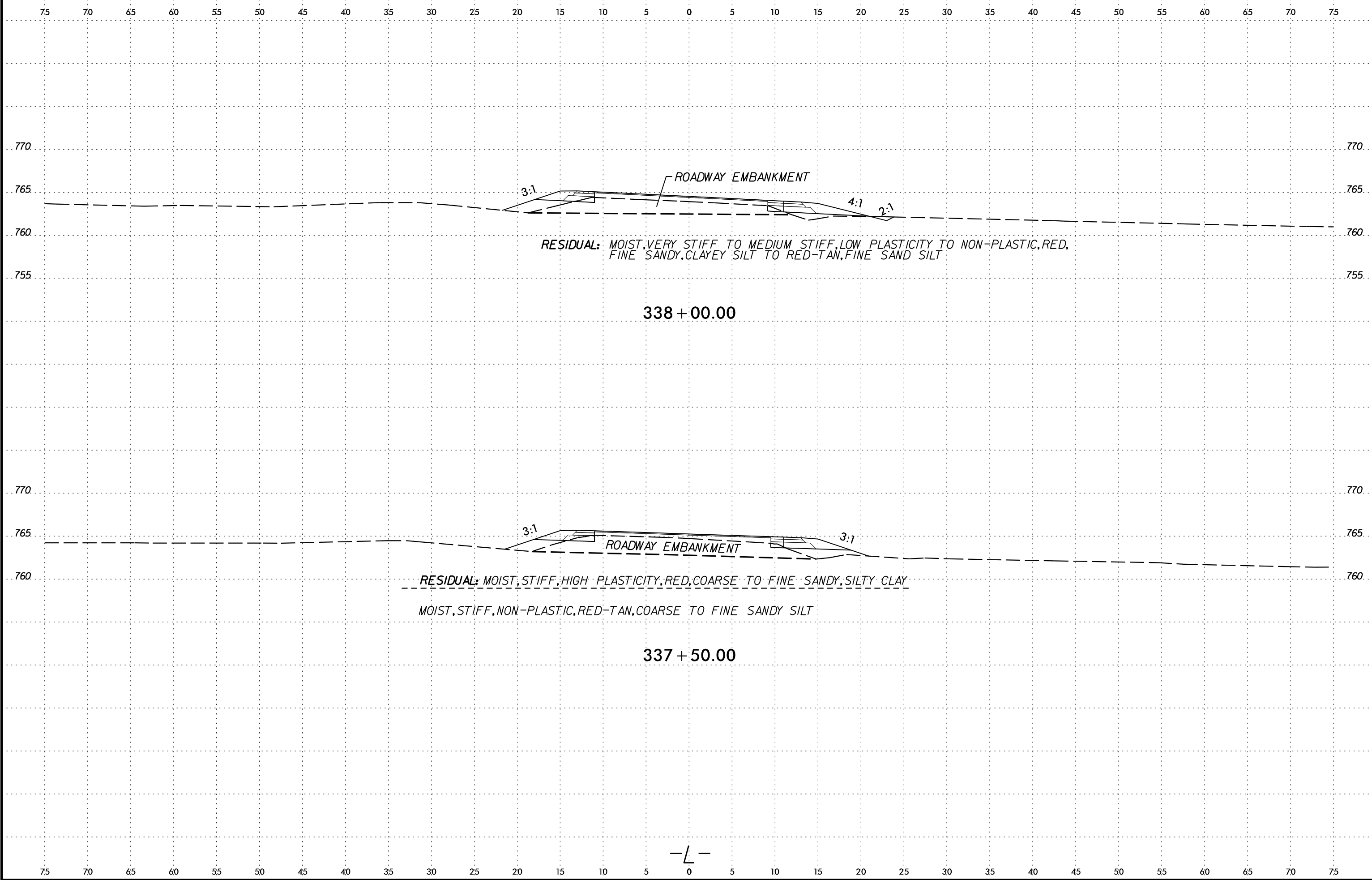
BEGIN AGGREGATE SUBGRADE AT STA 333 + 25

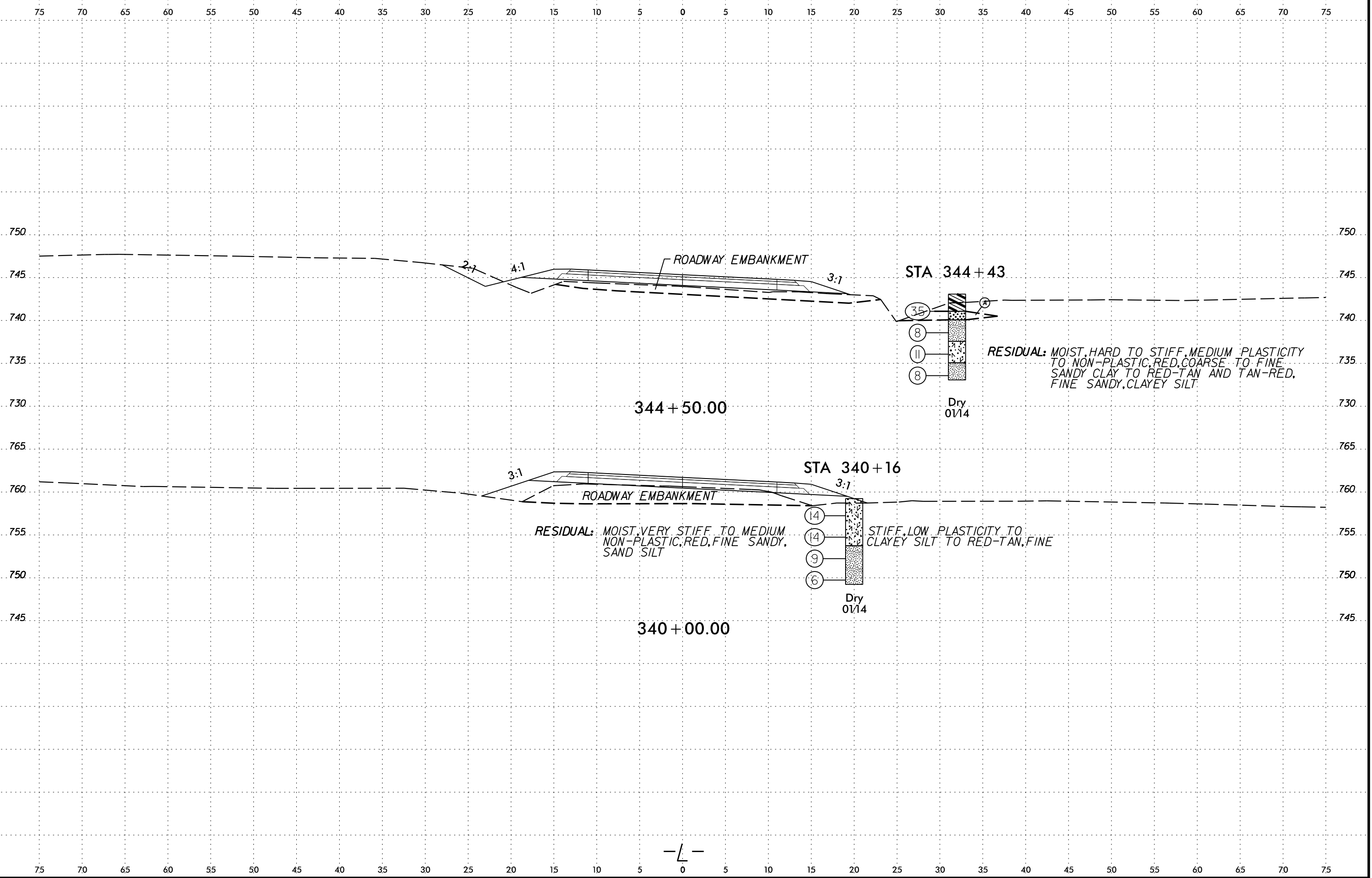
333 + 00.00

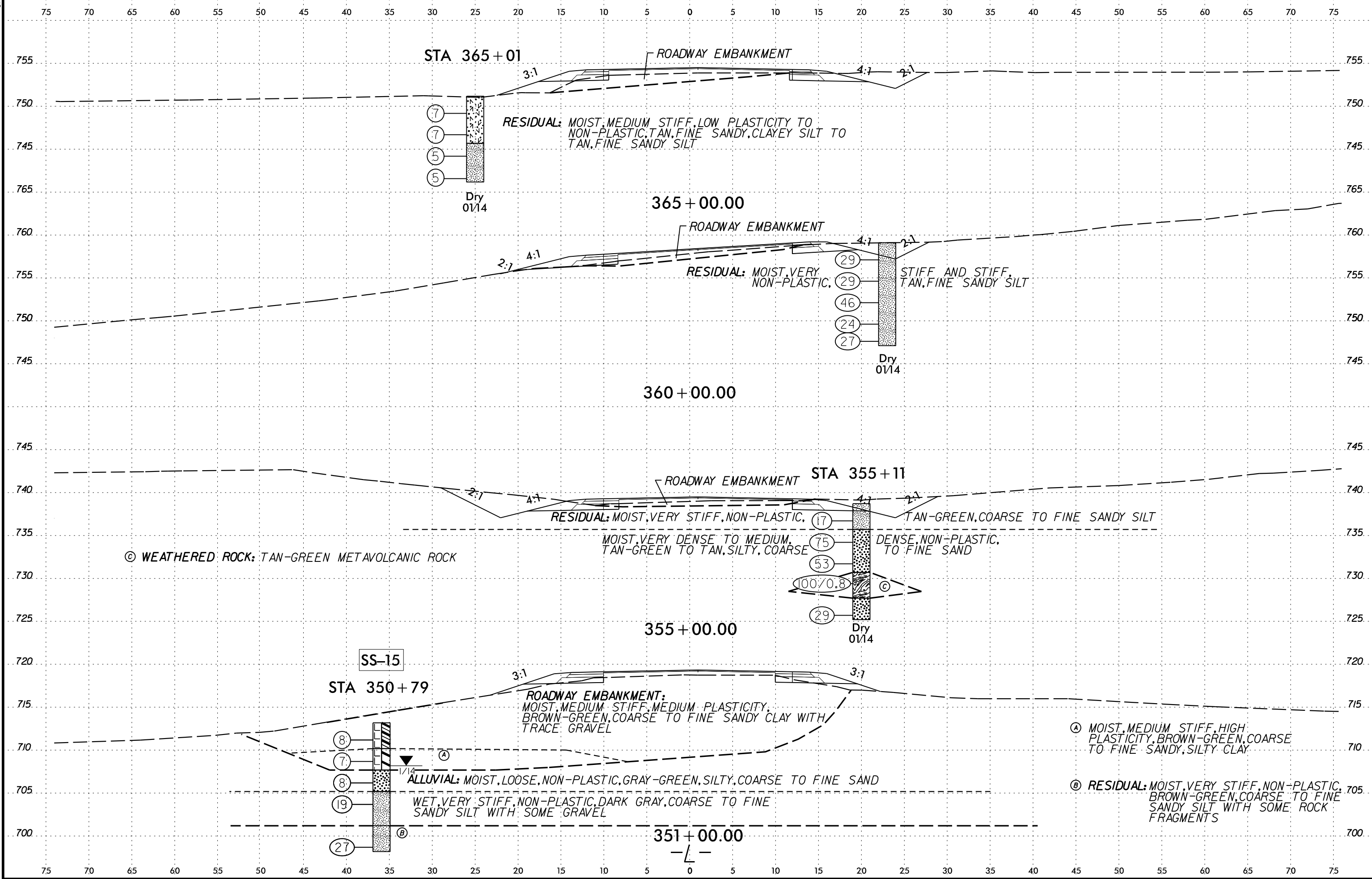


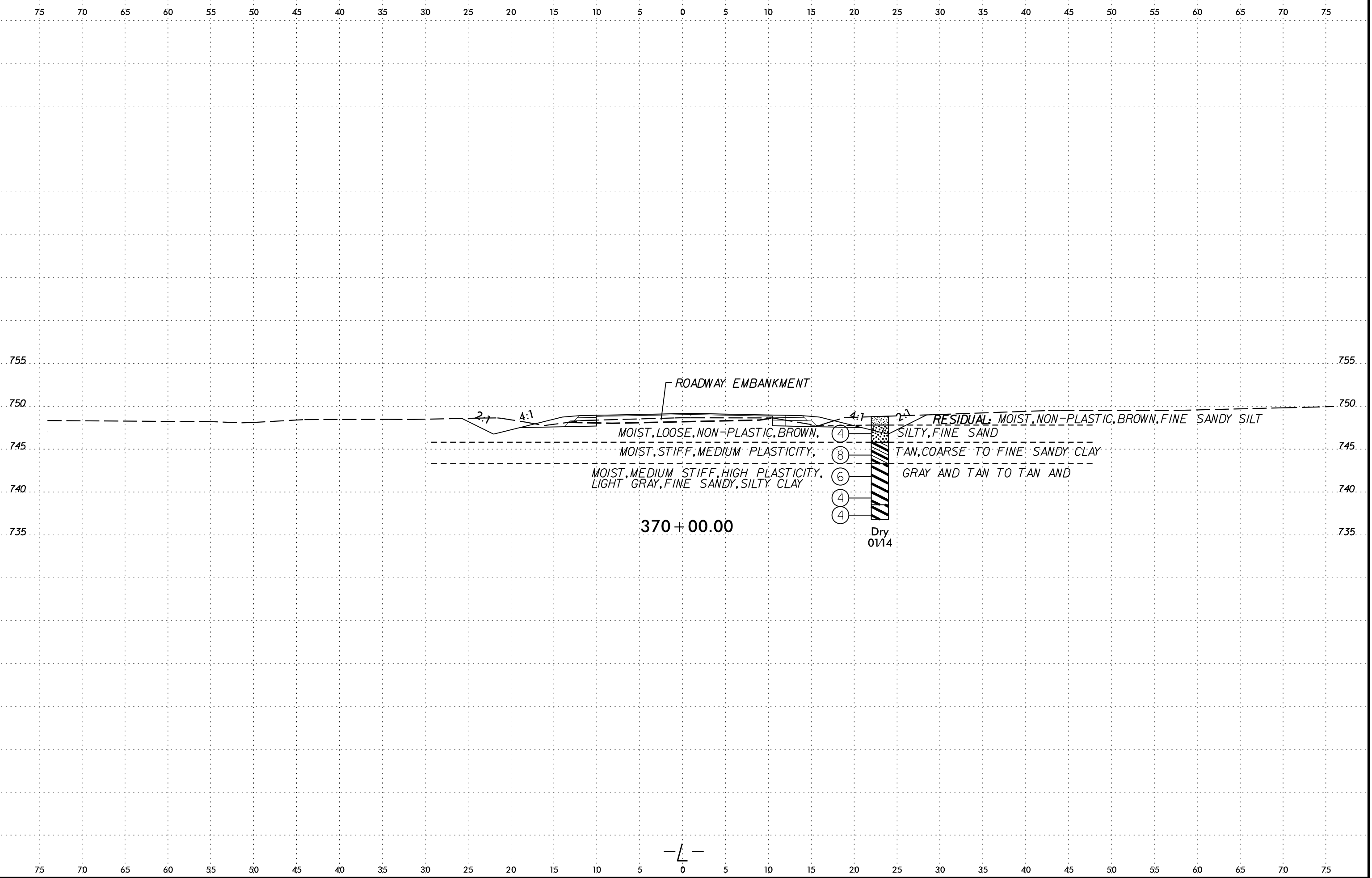


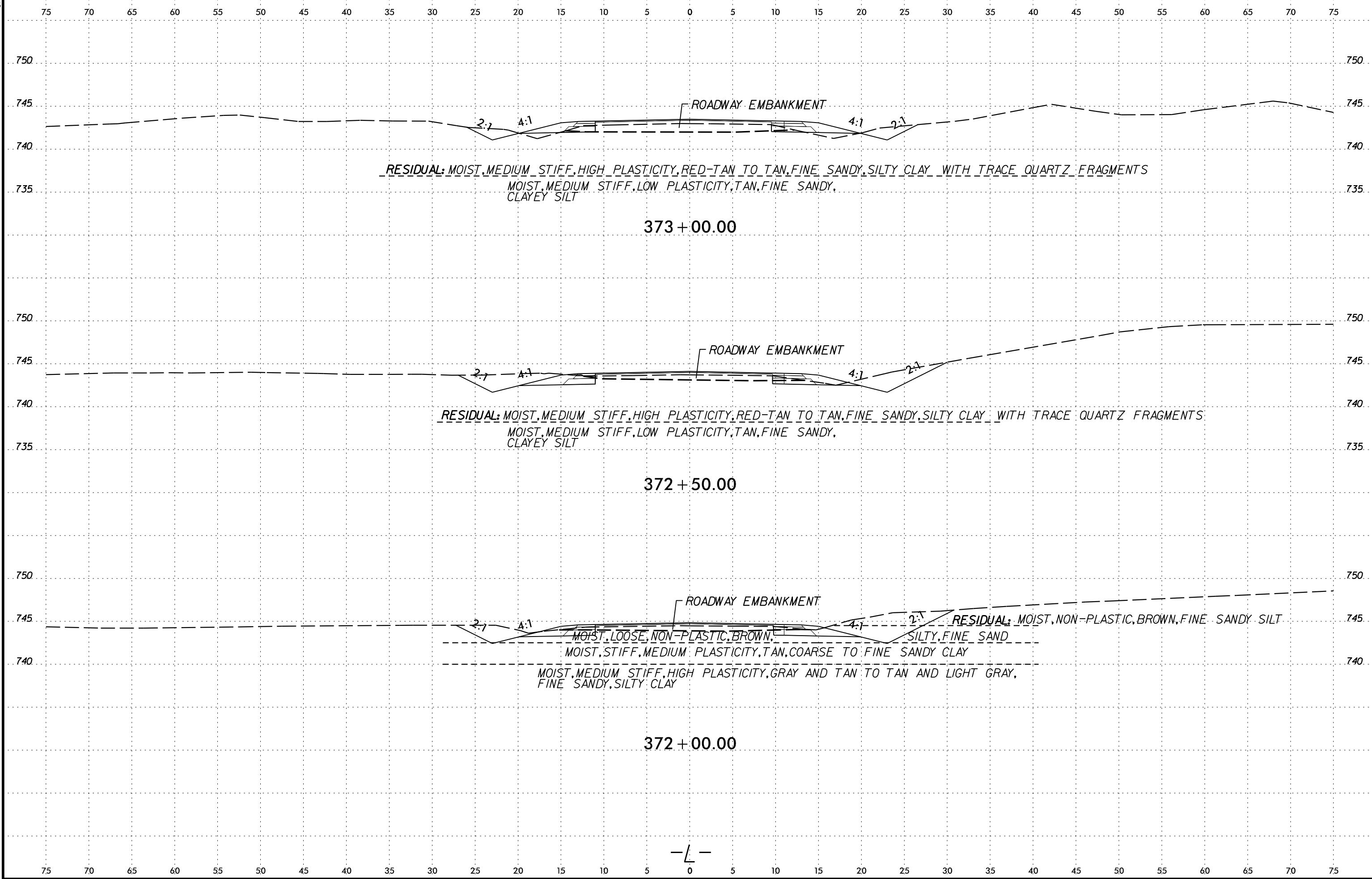


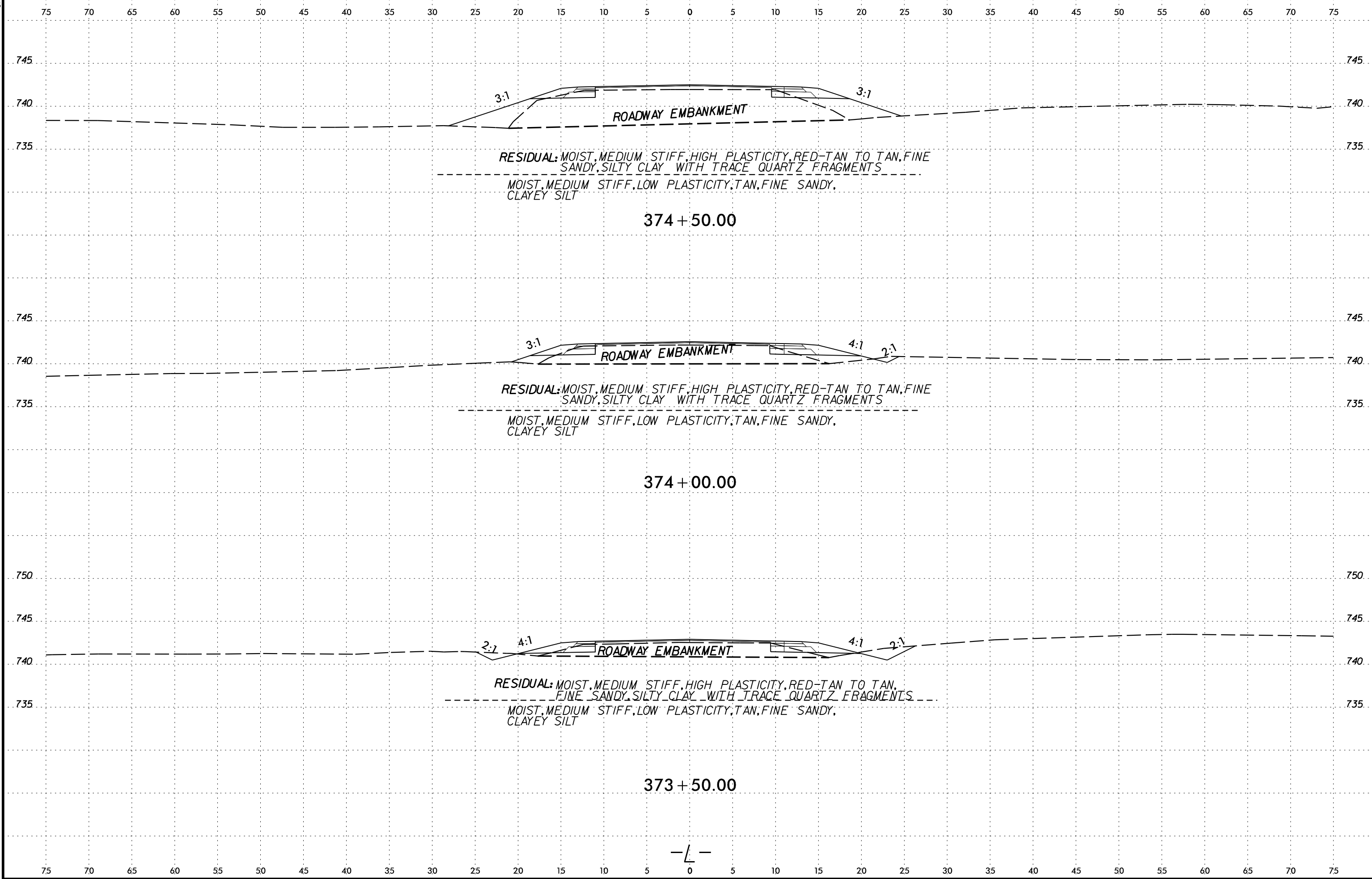


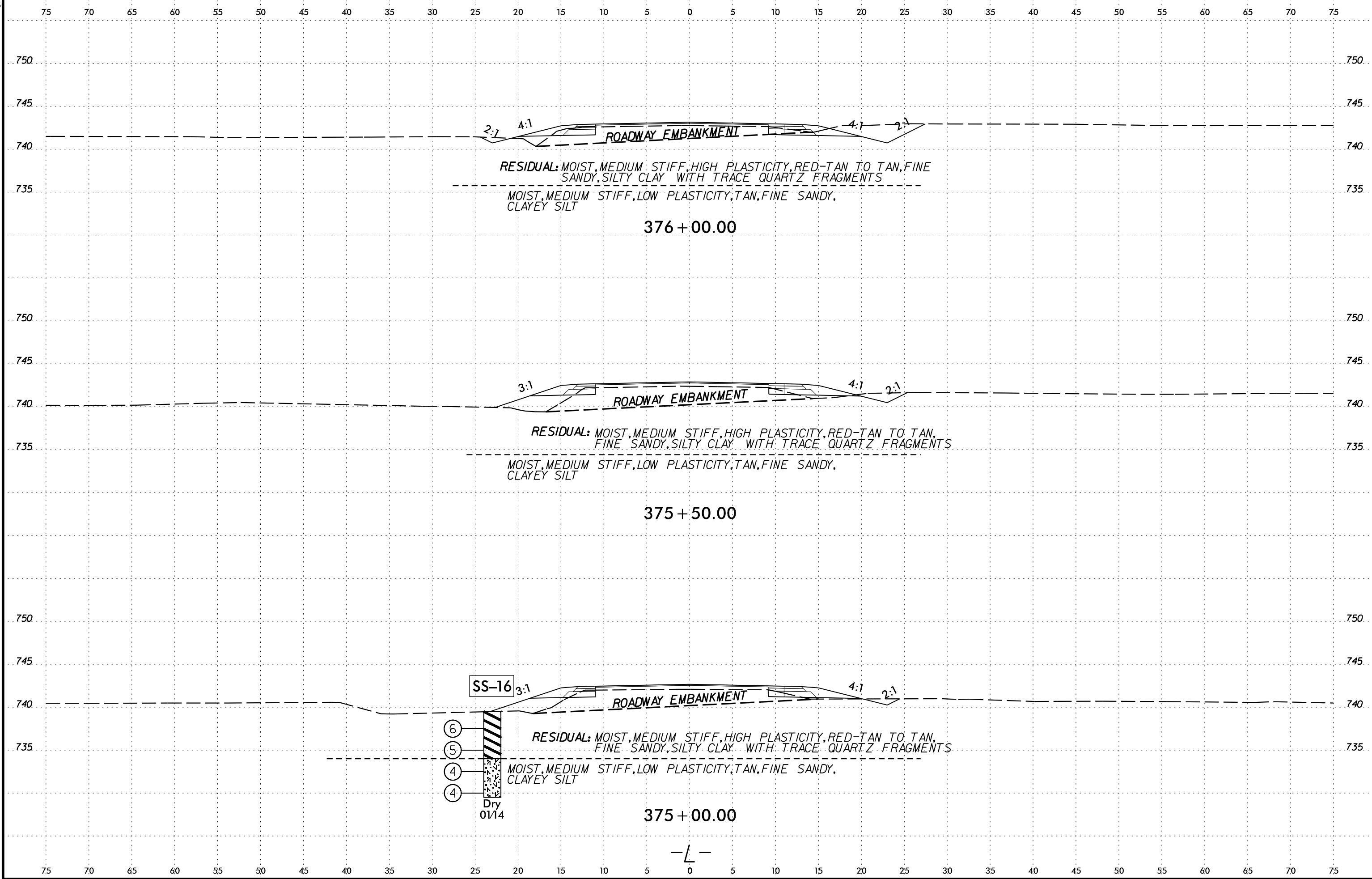


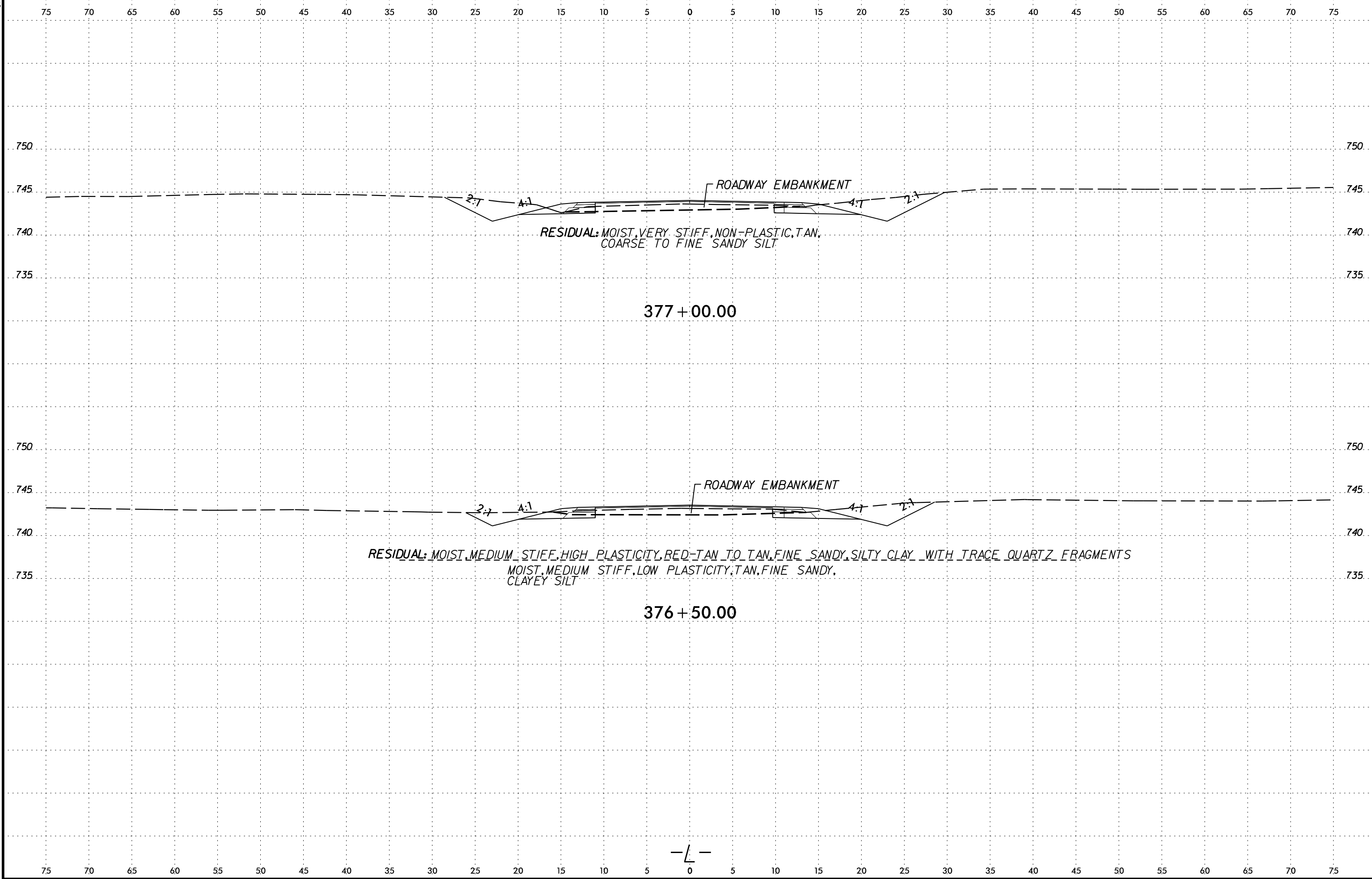


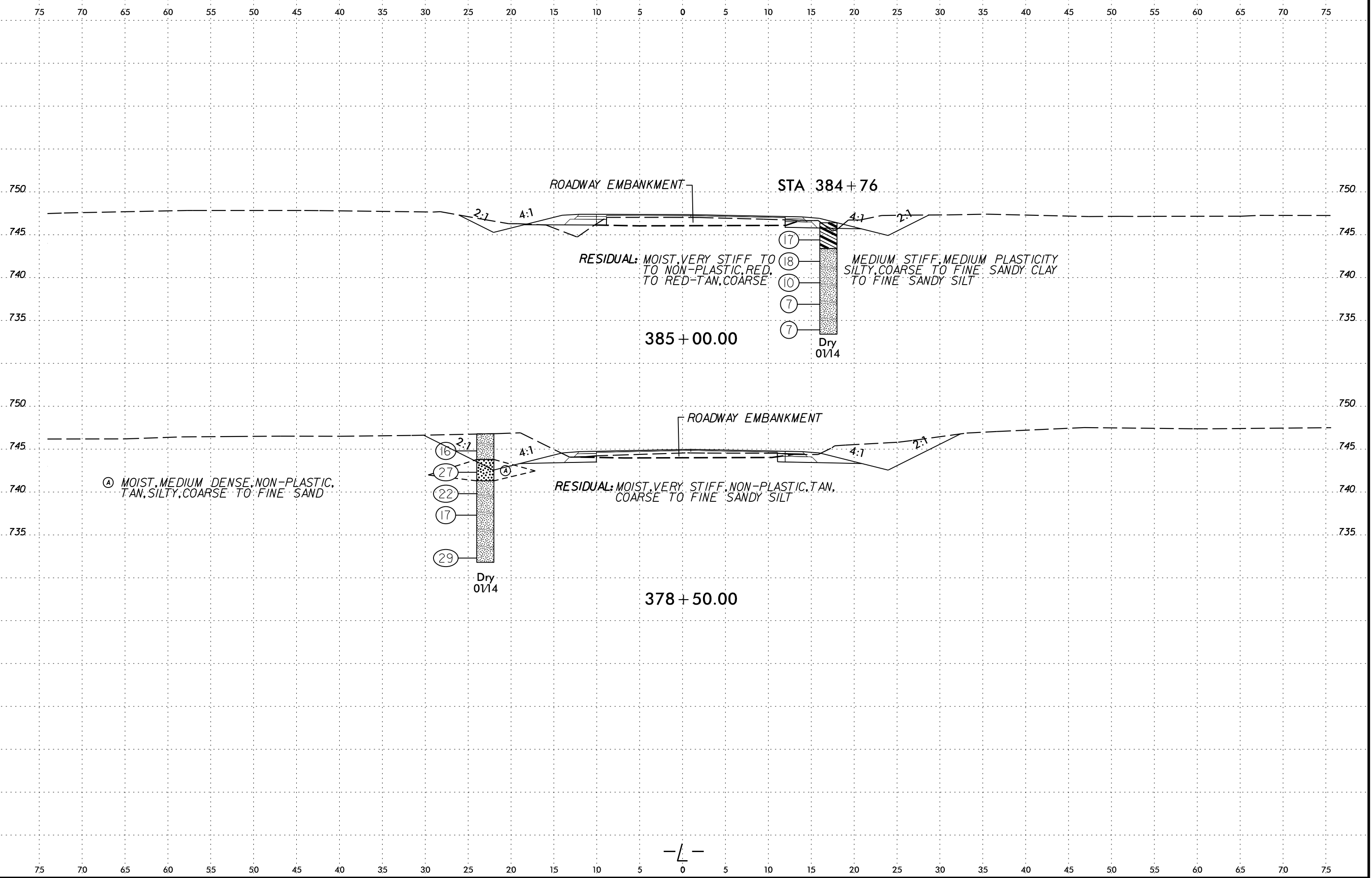


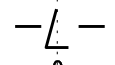
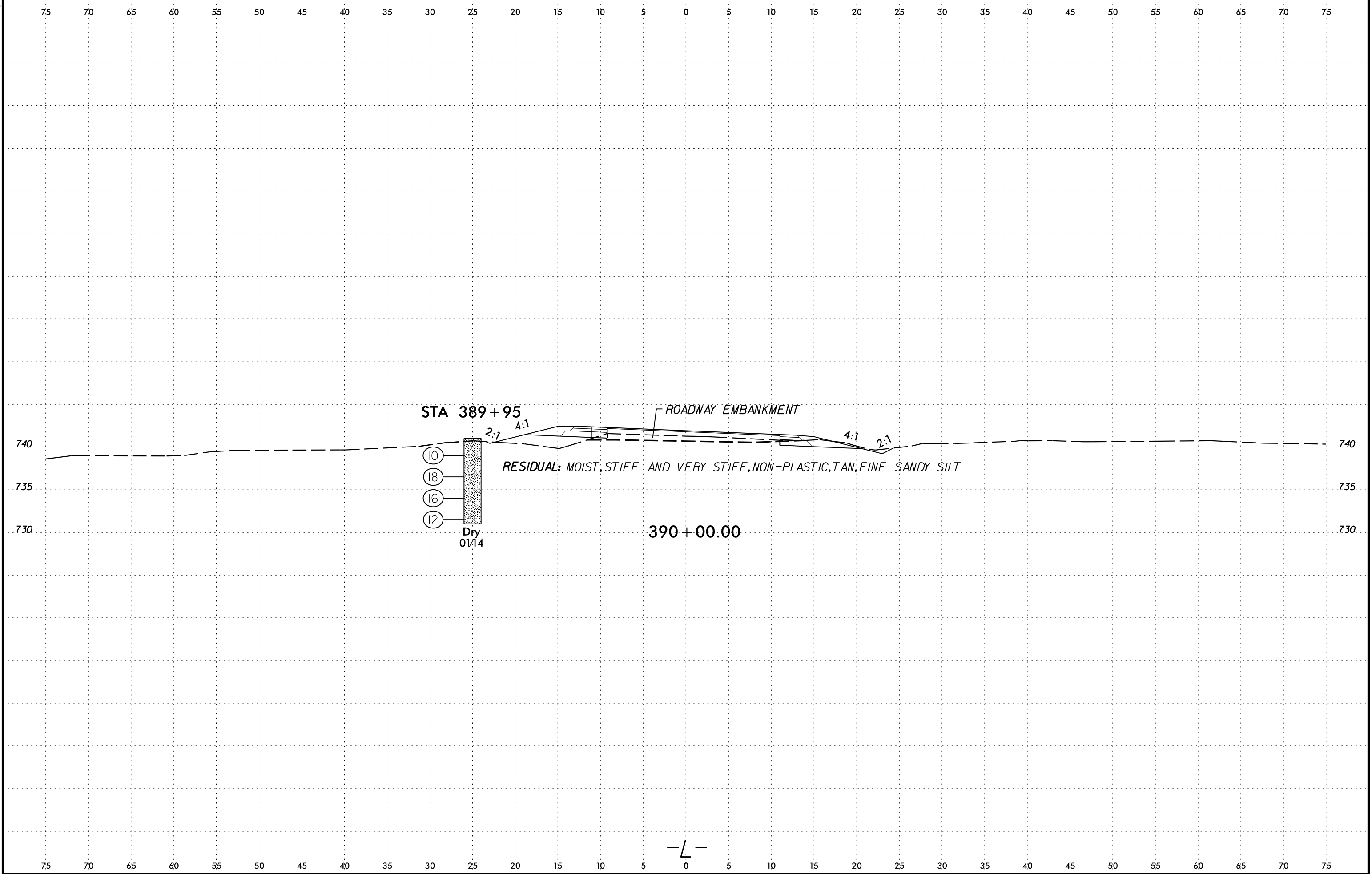


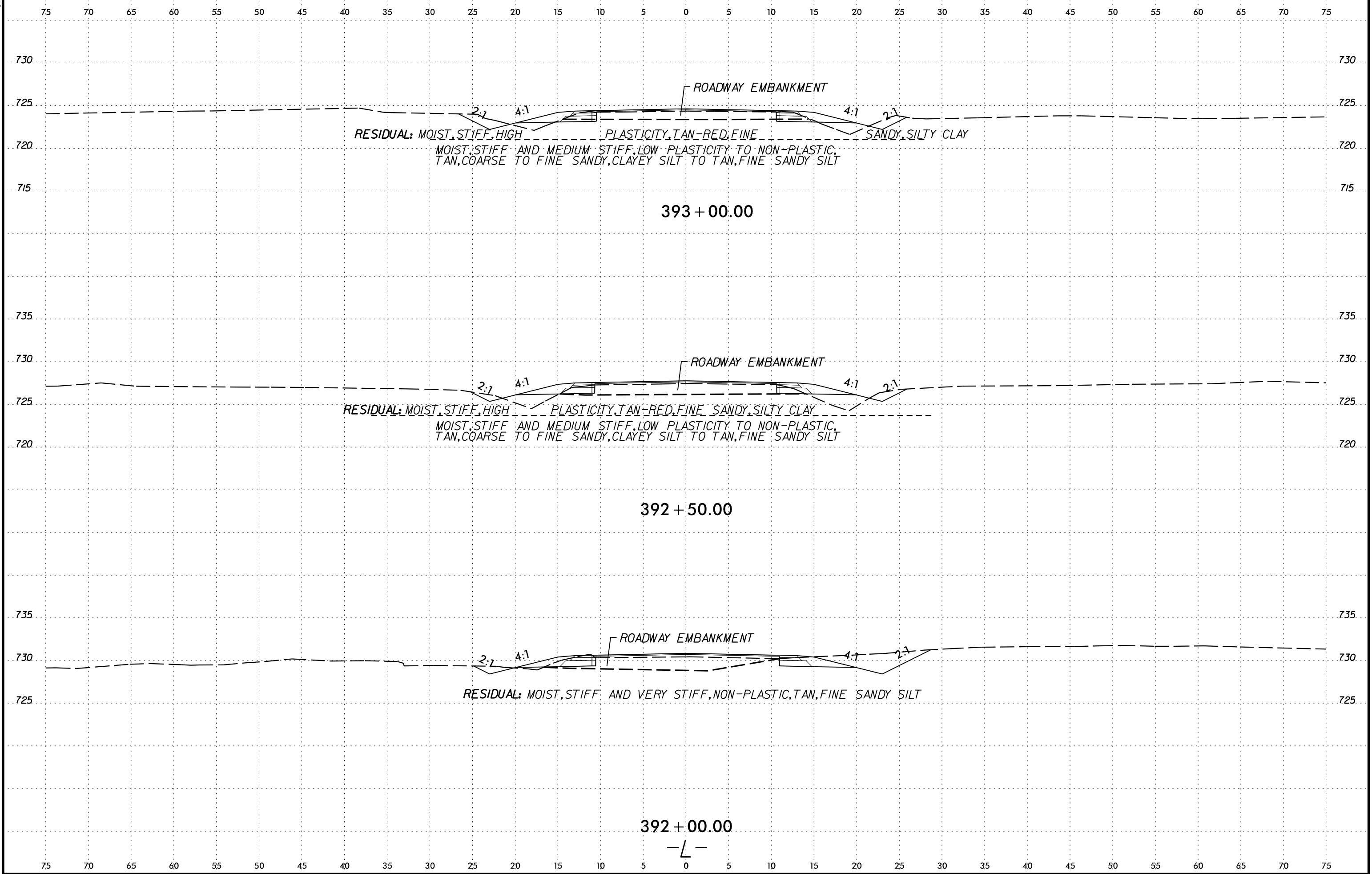


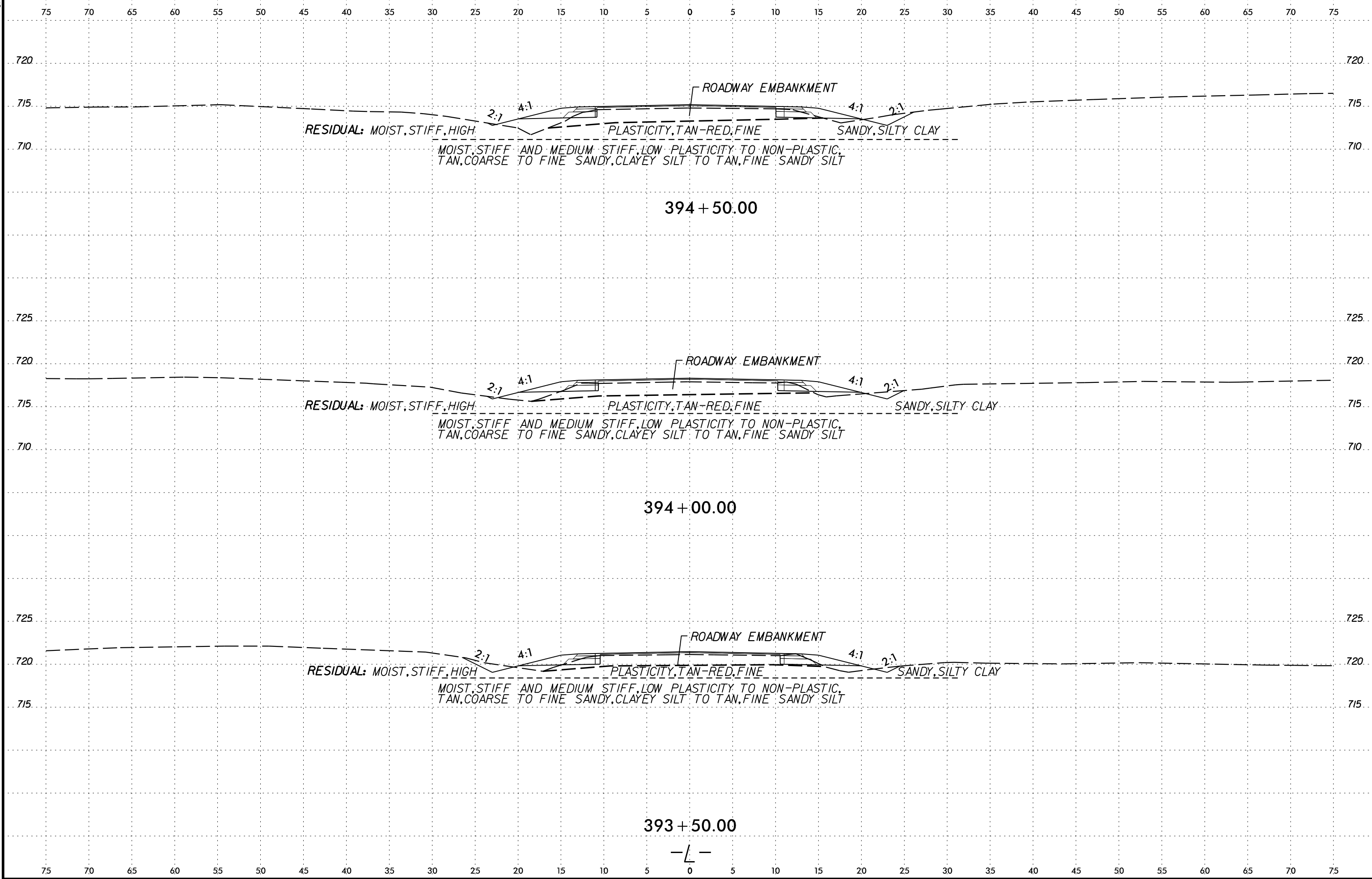


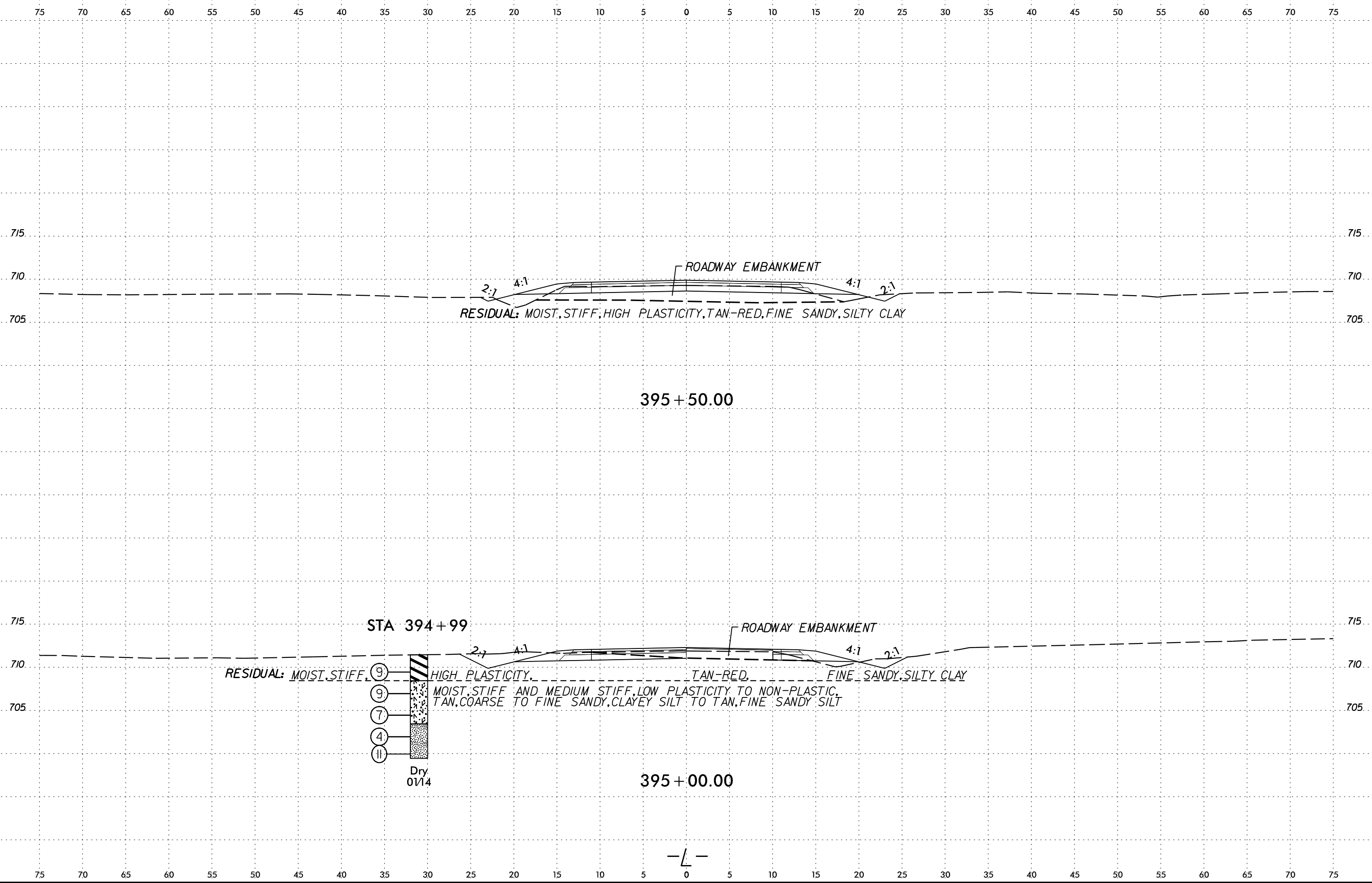












ROADWAY EMBANKMENT

RESIDUAL: MOIST, STIFF, HIGH PLASTICITY, TAN-RED, FINE SANDY, SILTY CLAY

395 + 50.00

STA 394 + 99

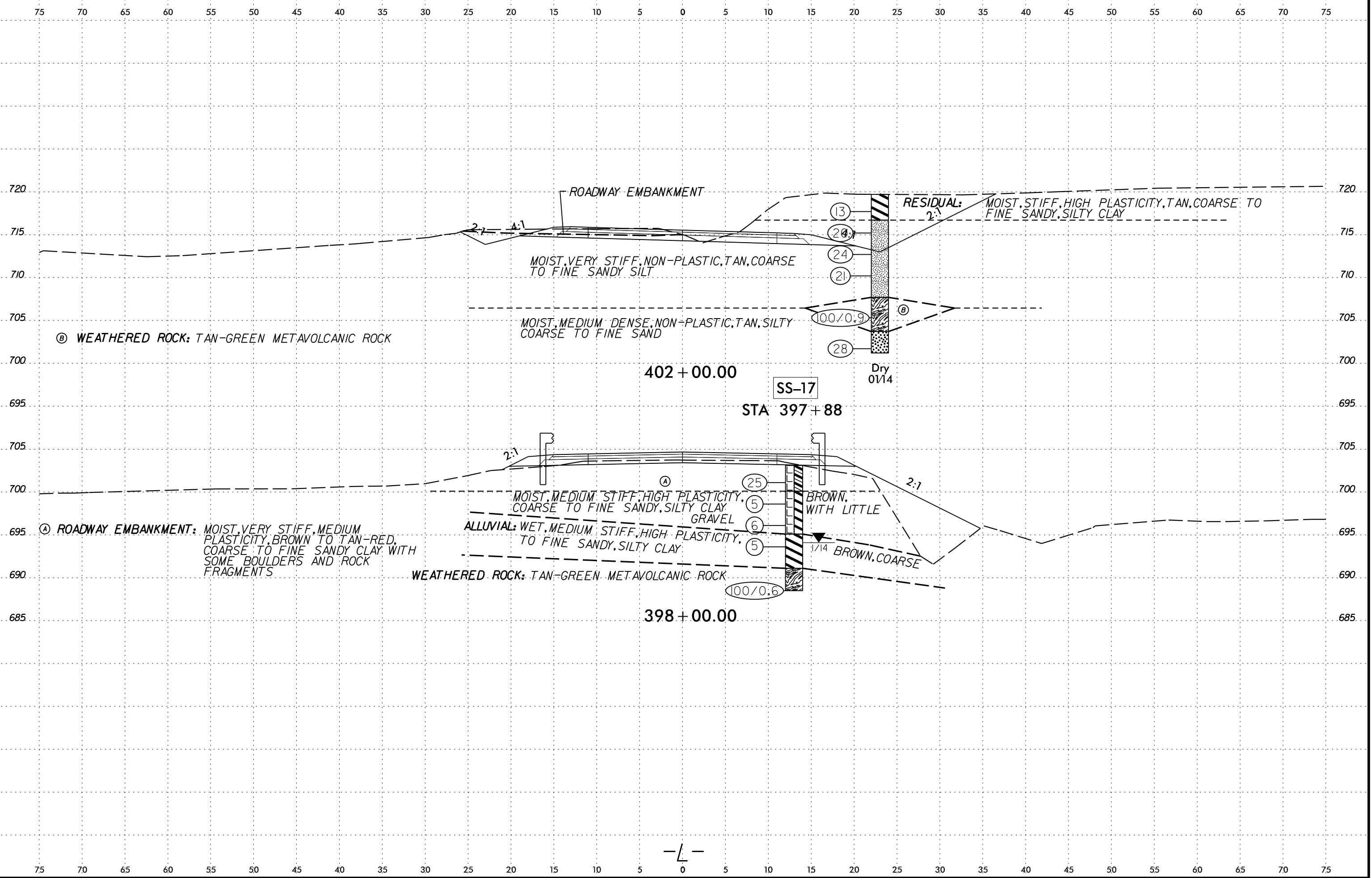
RESIDUAL: MOIST, STIFF, HIGH PLASTICITY, TAN-RED, FINE SANDY, SILTY CLAY

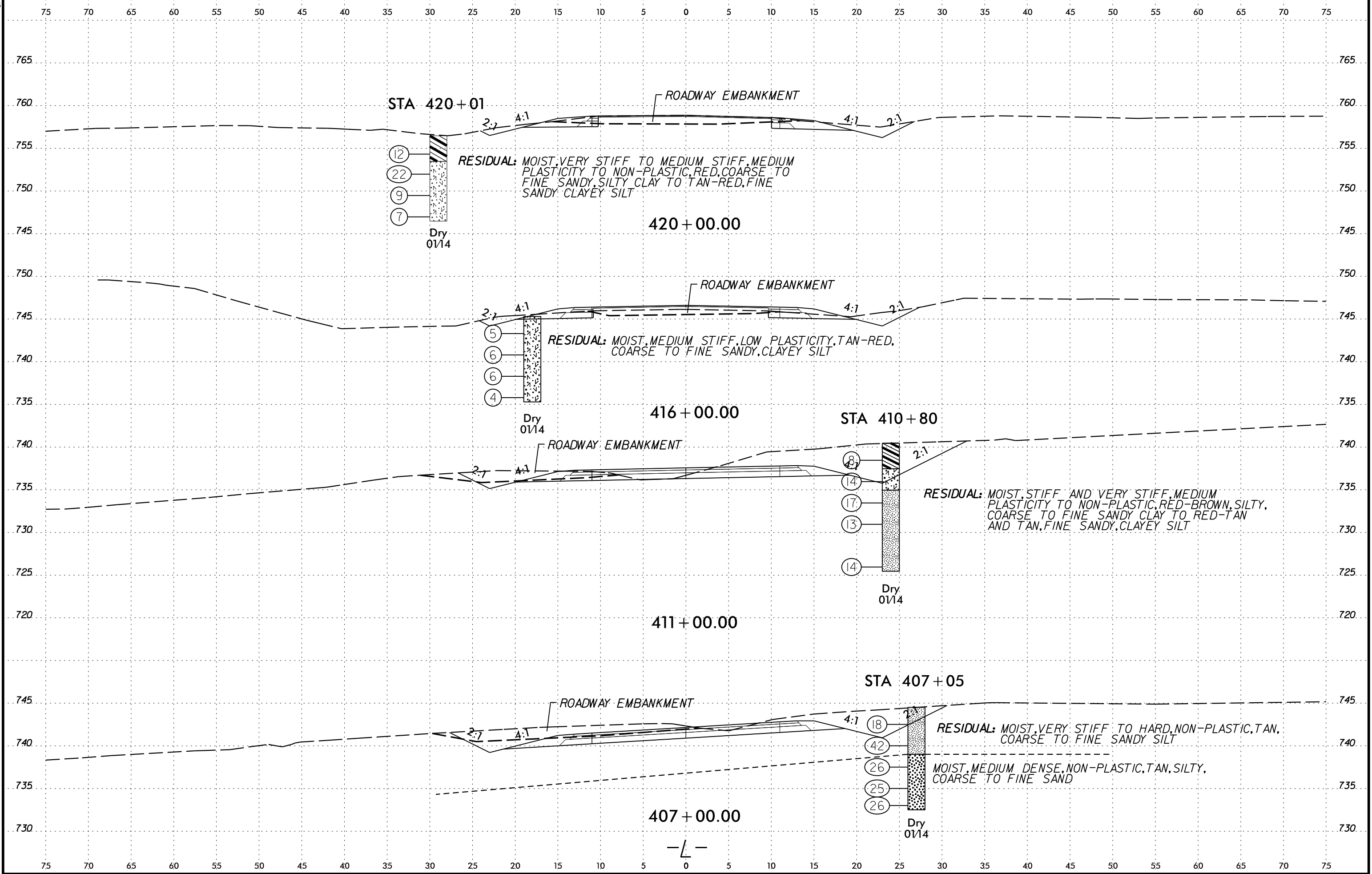
MOIST, STIFF AND MEDIUM STIFF, LOW PLASTICITY TO NON-PLASTIC, TAN, COARSE TO FINE SANDY, CLAYEY SILT TO TAN, FINE SANDY SILT

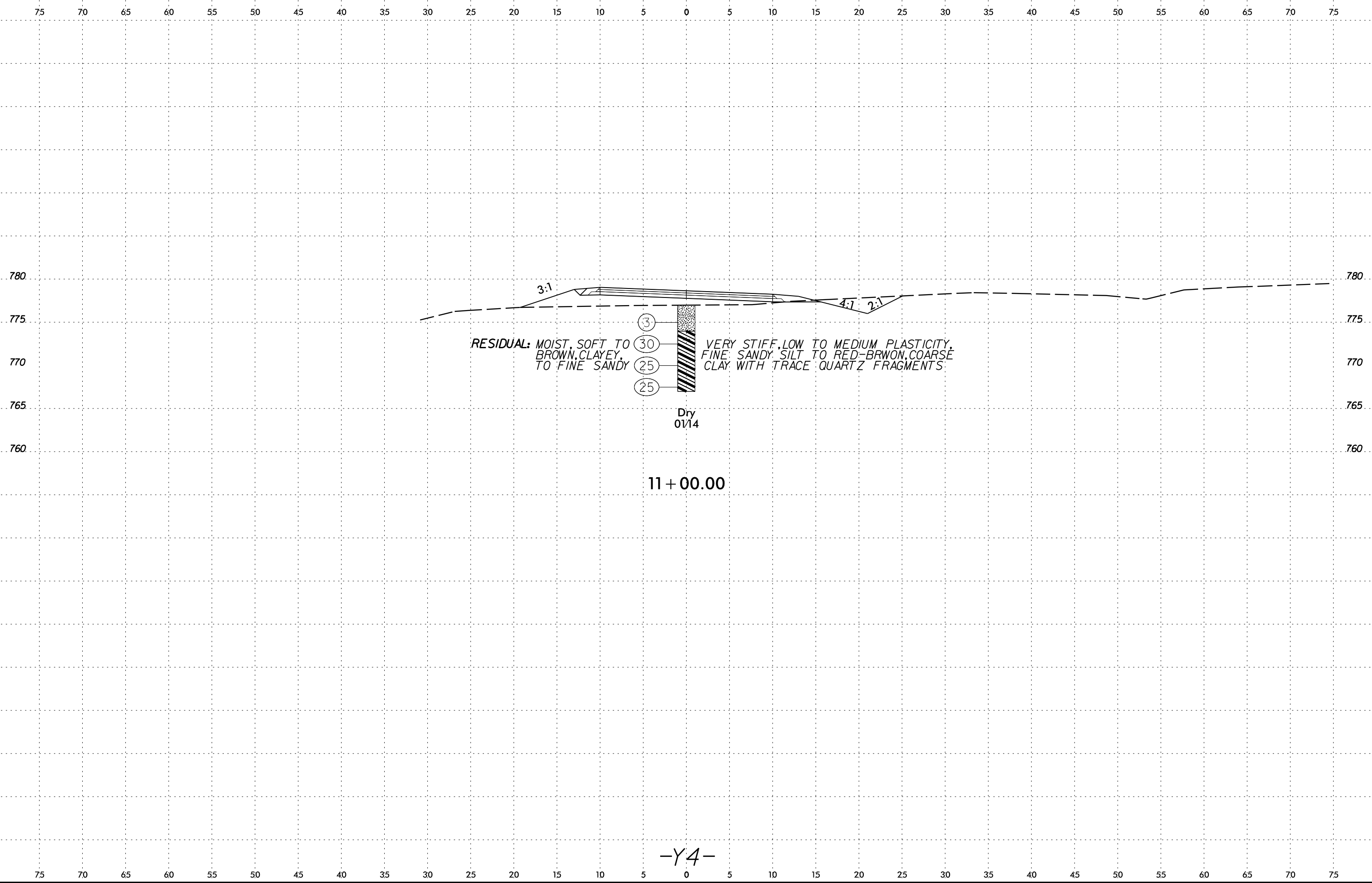
Dry 0/14

395 + 00.00

—/—





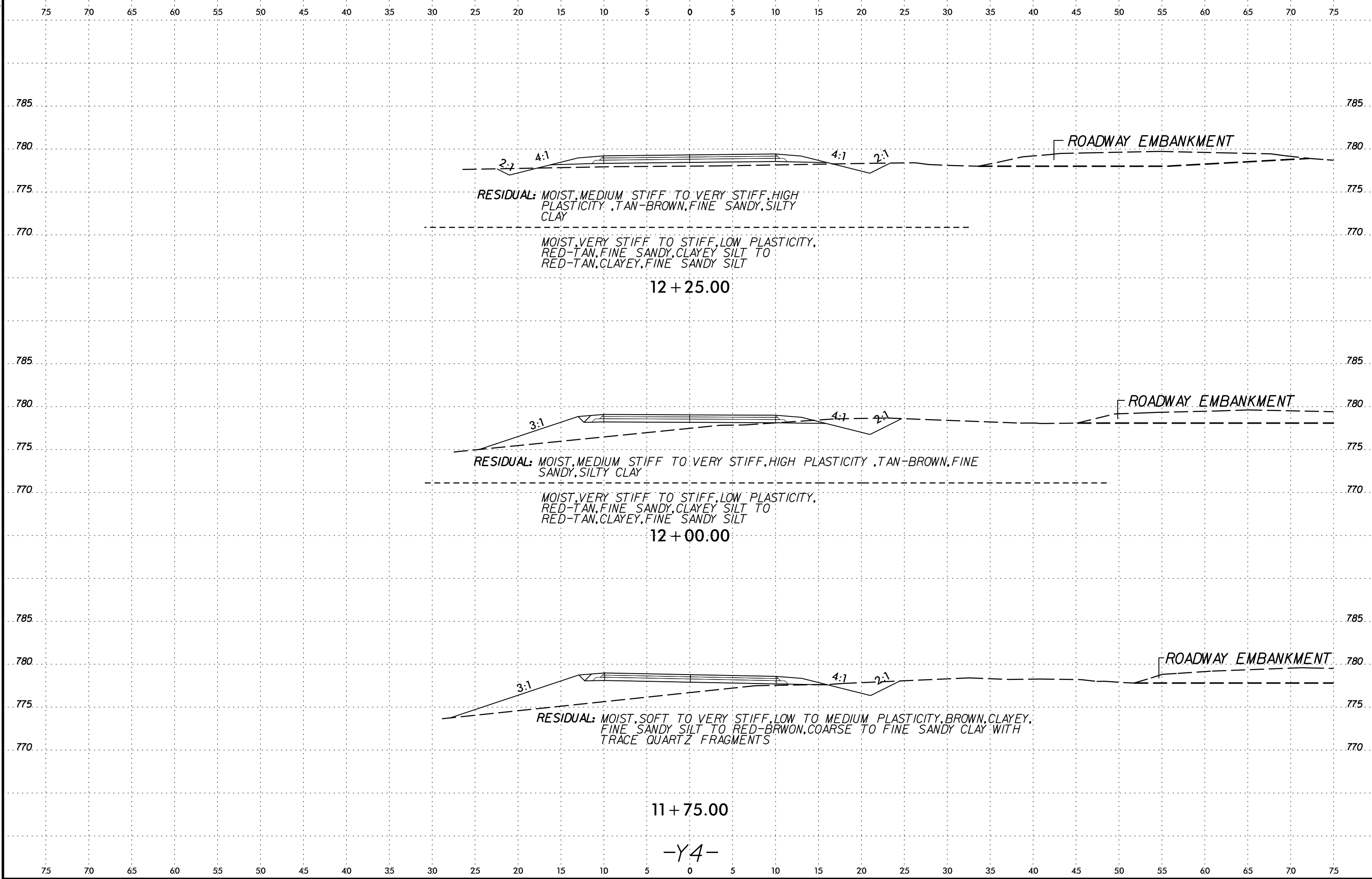


RESIDUAL: MOIST, SOFT TO BROWN, CLAYEY, TO FINE SANDY

VERY STIFF, LOW TO MEDIUM PLASTICITY, FINE SANDY SILT TO RED-BROWN, COARSE CLAY WITH TRACE QUARTZ FRAGMENTS

Dry 0/14

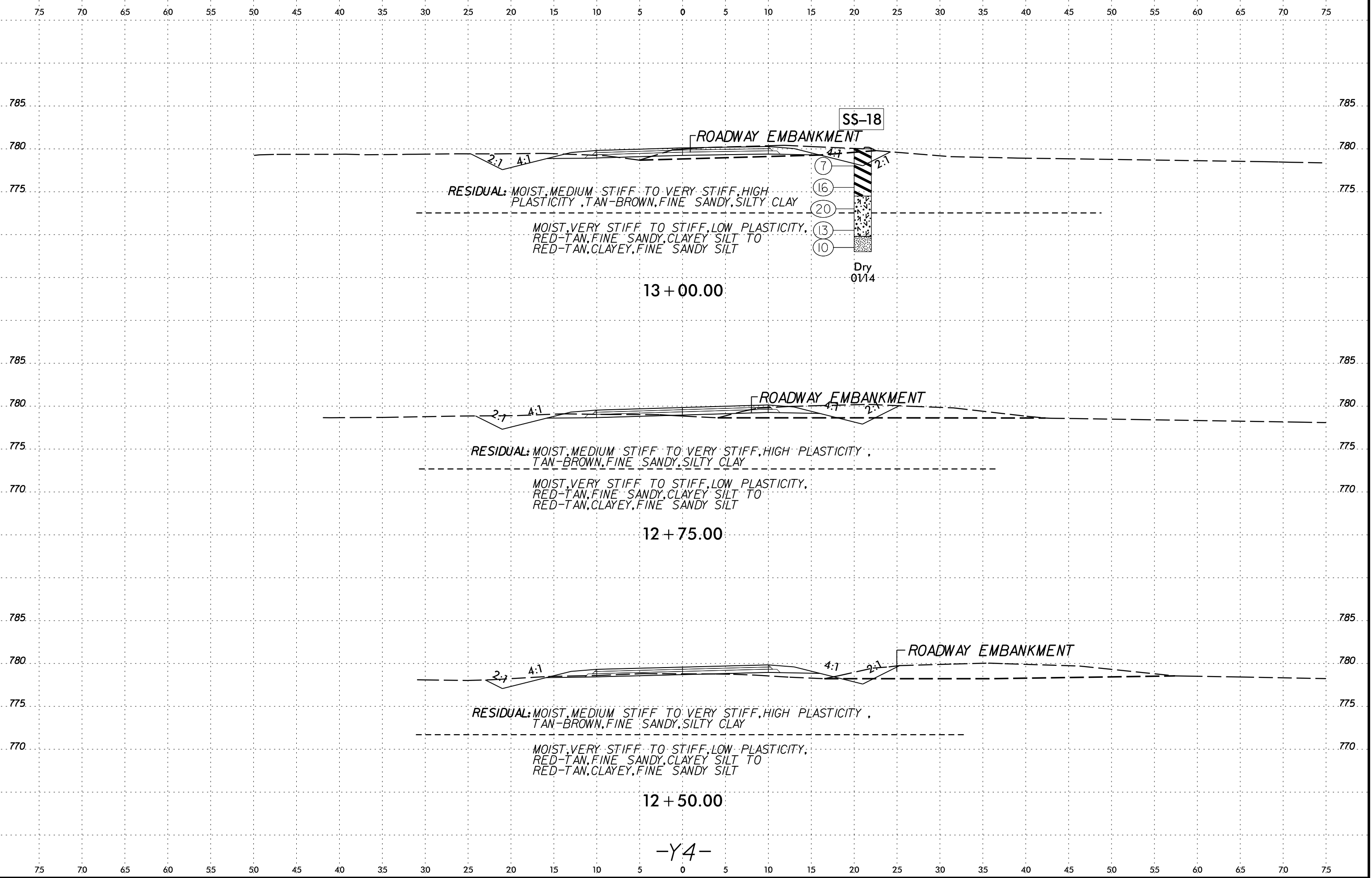
11 + 00.00



12 + 25.00

12 + 00.00

11 + 75.00

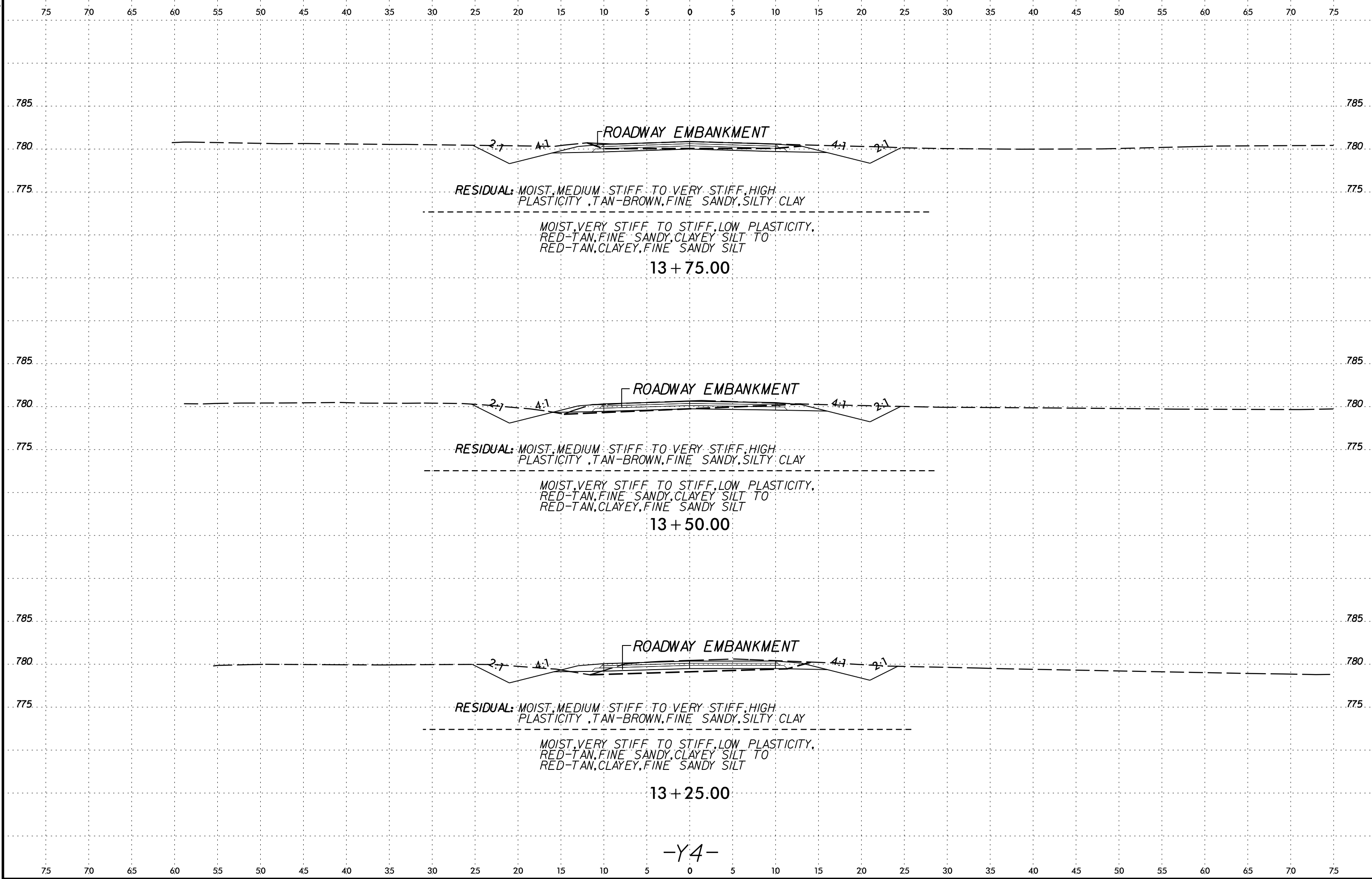


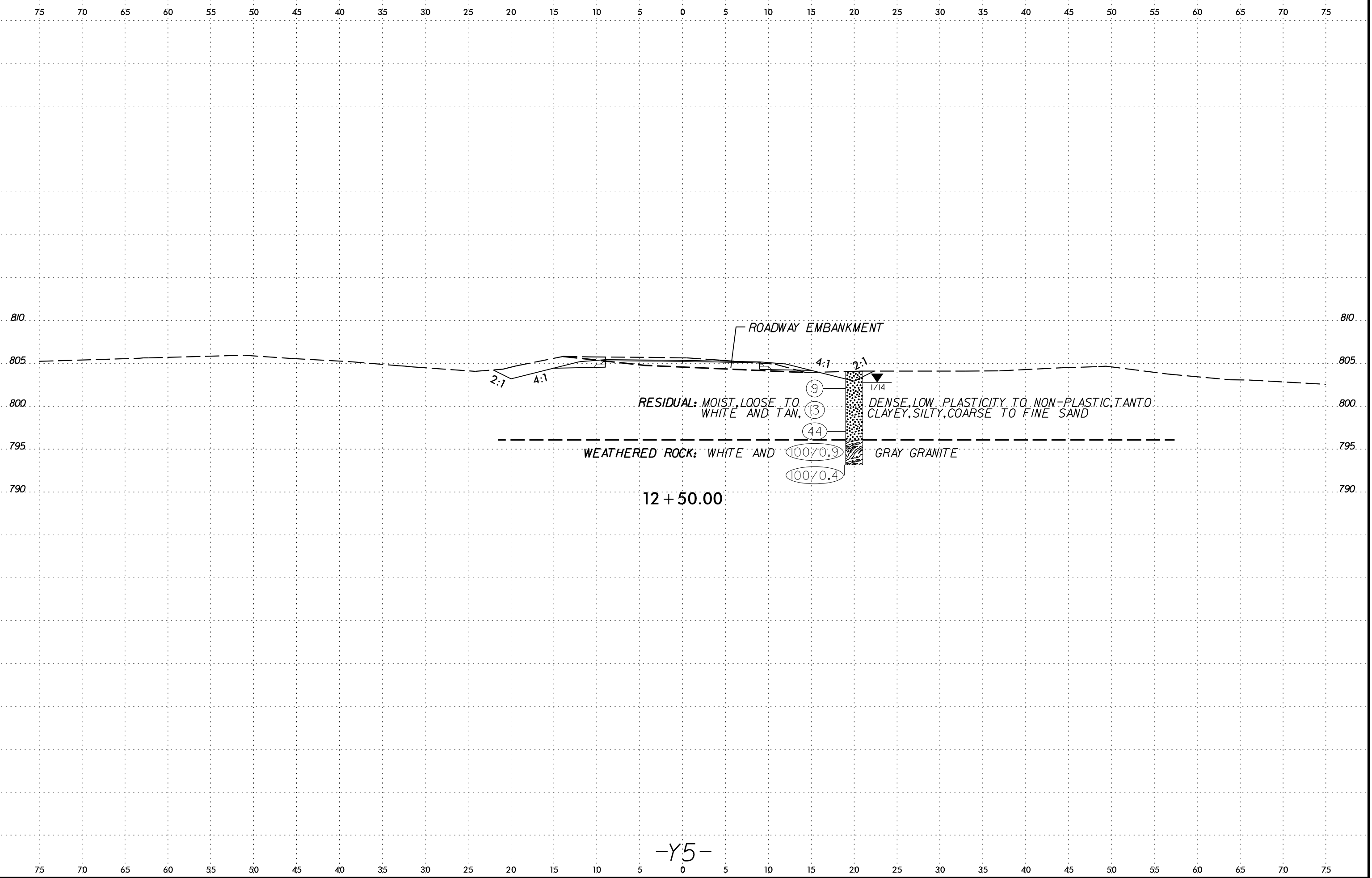
13 + 00.00

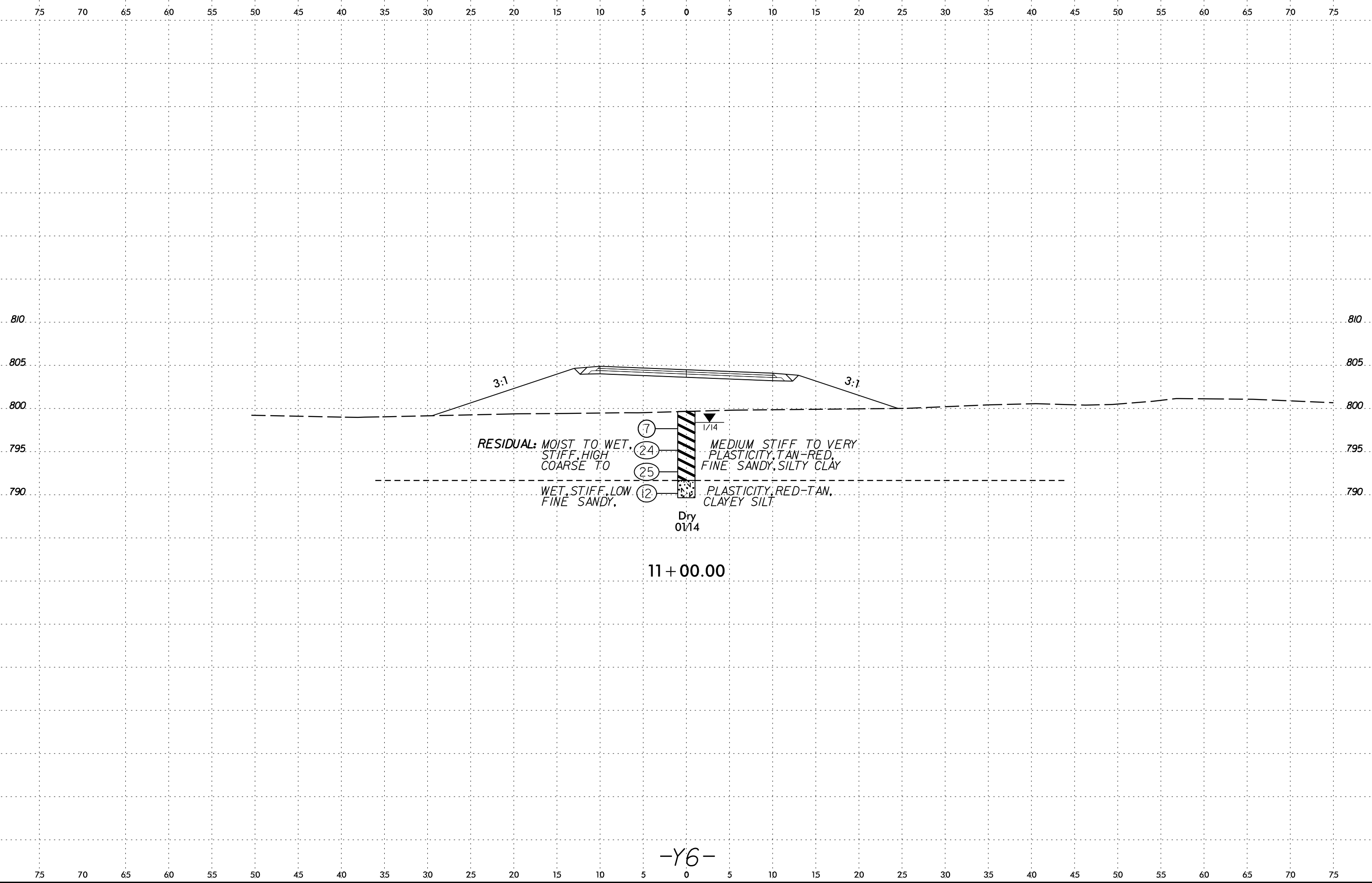
12 + 75.00

12 + 50.00

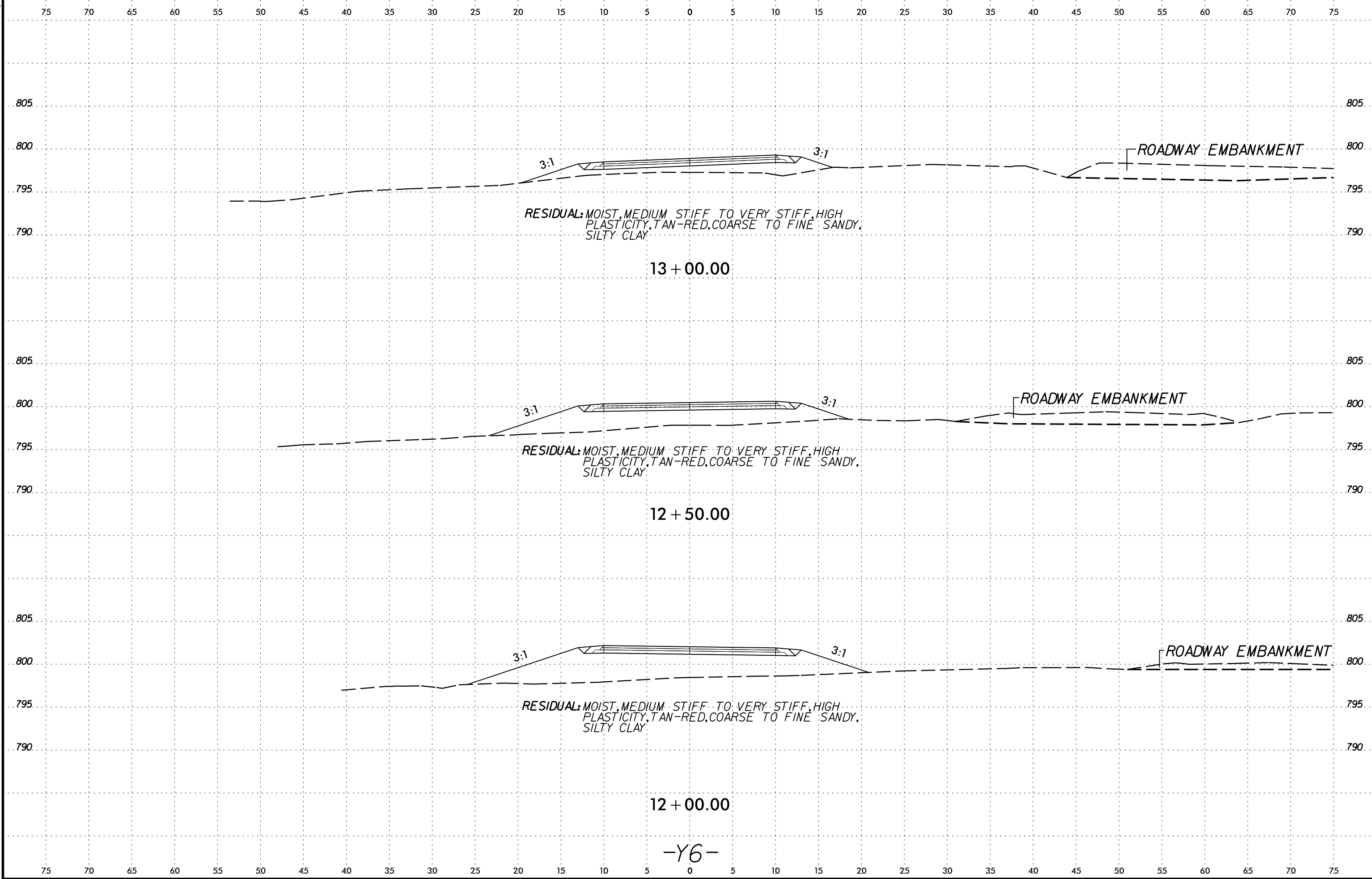
-Y4-

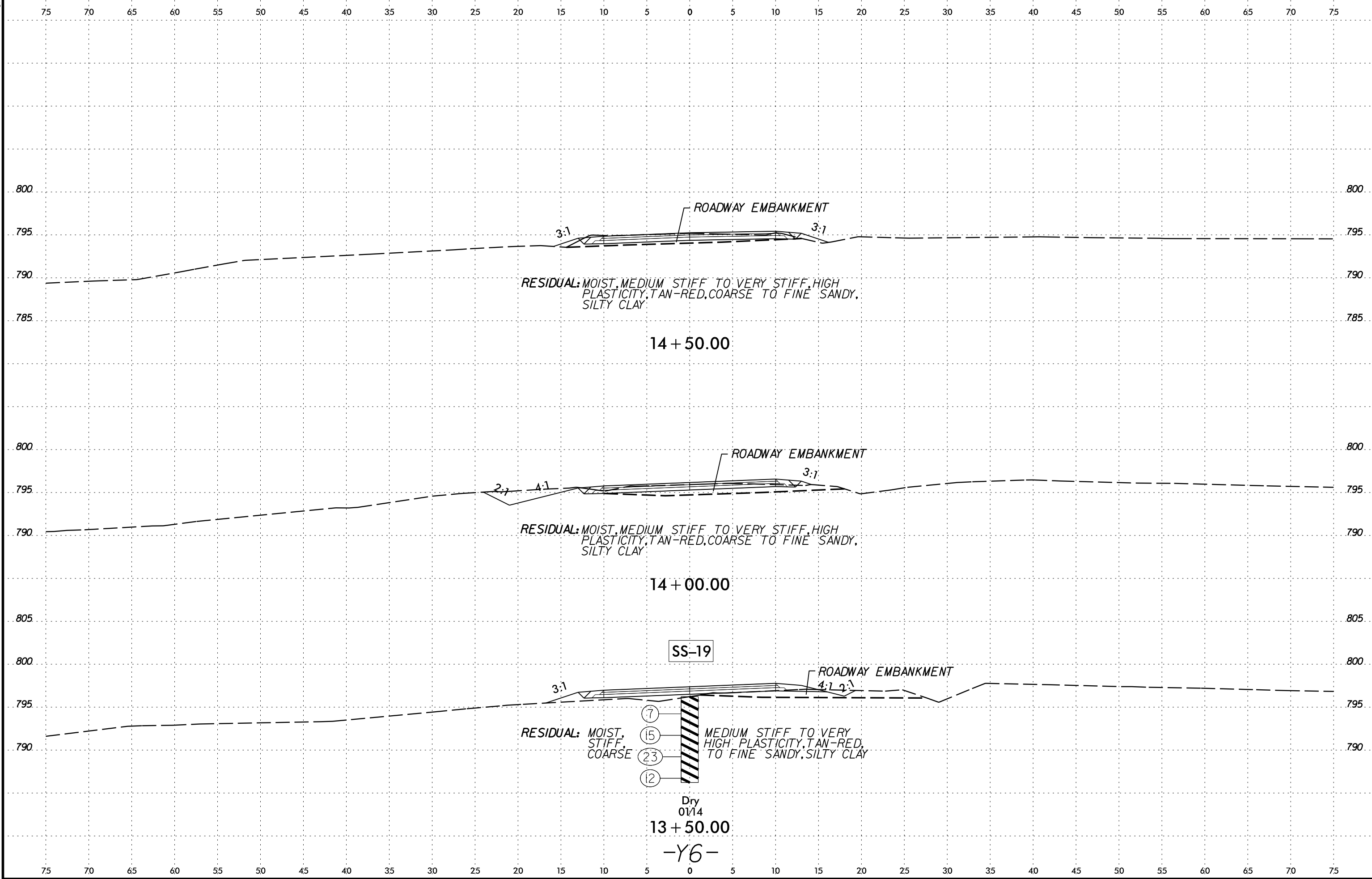


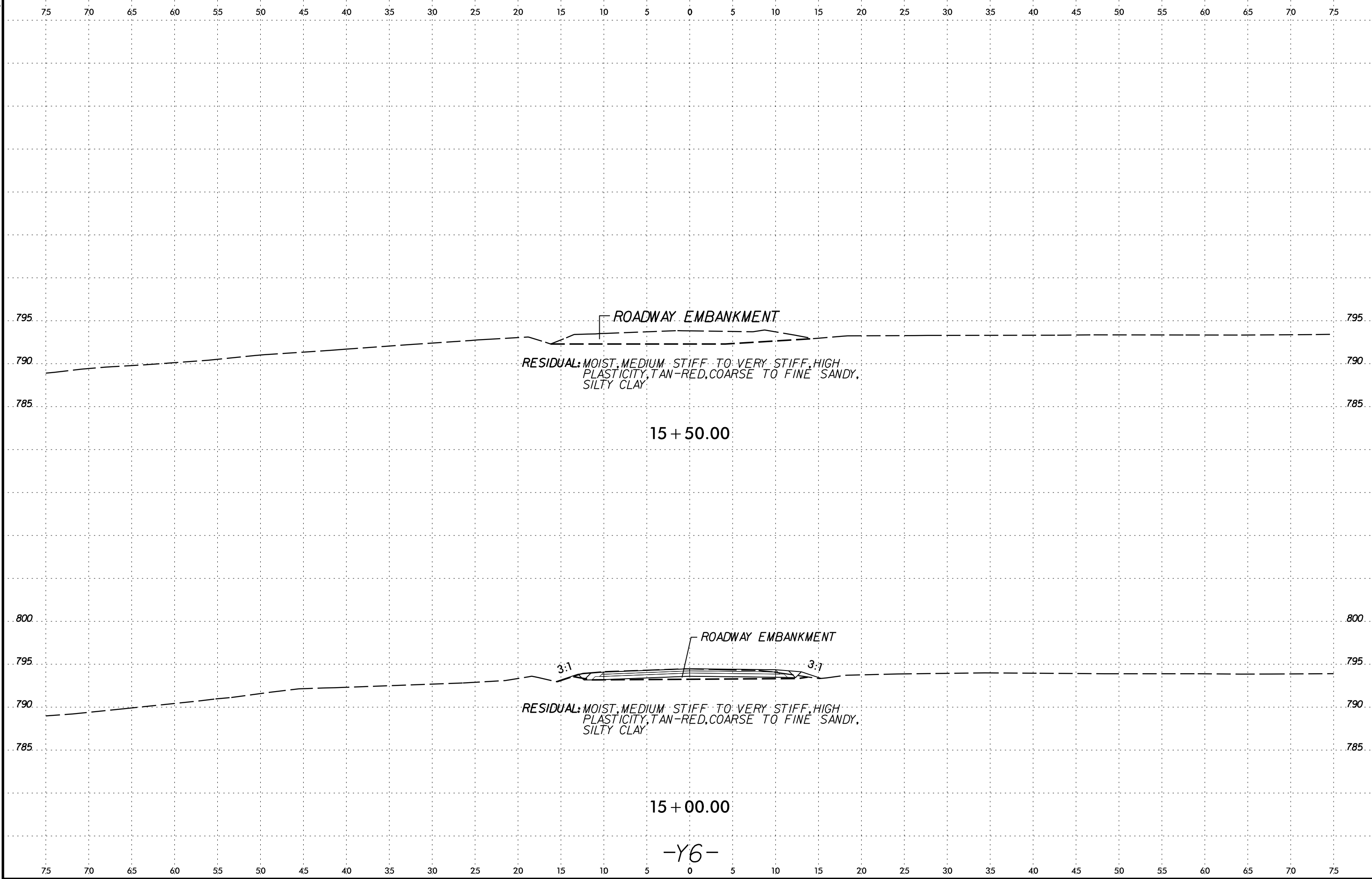




11 + 00.00







SUMMARY OF LABORATORY TEST DATA

PROJECT NO. 46136.1.1 (W-5313)
FA NO. STP-1221(15)
COUNTY: ROWAN
SR 1221 (OLD BEATTY FORD ROAD) FROM SR 1337 (LENTZ ROAD) TO SR 2335 (LOWER STONE CHURCH ROAD)

Sample No.	Boring Number	Station	Offset	Alignment	Sample Depth (ft.)	Natural Moisture Content (%)	AASHTO Class (Group Index)	N-Value (blows/ ft.)	Atterberg Limits			Gradation Results							
									L.L.	P.L.	P.I.	Pass #10 Sieve	Pass #40 Sieve	Pass #200 Sieve	Retained #270 Sieve	Coarse Sand (%)	Fine Sand (%)	Silt (%)	Clay (%)
SS-1	L_4000	40+00	18' RT	-L-	3.5-5.0	30.6	A-7-5(45)	27	75	37	38	100	99	95	7	1.2	5.9	14.7	78.2
SS-2	L_7050	70+50	36' RT	-L-	1.0-2.5	27.9	A-7-5(24)	11	56	34	22	100	98	90	12	2.7	9.6	23.4	64.2
SS-3	L_11000	110+00	23' RT	-L-	1.0-2.5	30.4	A-7-5(25)	11	59	35	24	100	99	85	18	4.0	14.1	26.1	55.9
SS-4	L_12048	120+48	26' RT	-L-	3.5-5.0	27.4	A-6(12)	5	34	17	17	99	97	82	23	3.8	19.4	25.3	51.5
SS-5	L_16500	165+00	15' RT	-L-	1.0-2.5	45.2	-	12	-	-	-	-	-	-	-	-	-	-	-
SS-6	L_18008	180+08	38' LT	-L-	6.0-7.5	41.7	A-7-5(60)	9	99	45	54	100	98	90	12	3.3	8.6	26.8	61.3
SS-7	L_18986	189+86	22' RT	-L-	1.0-2.5	23.0	A-6(6)	10	34	18	16	92	81	57	44	18.8	24.7	22.9	33.5
SS-8	L_20930	209+30	31' LT	-L-	1.0-2.5	27.7	-	2	-	-	-	-	-	-	-	-	-	-	-
SS-9	L_22024	220+24	10' RT	-L-	1.0-2.5	28.6	A-7-6(8)	17	50	24	26	84	65	46	48	28.6	18.9	14.2	38.3
SS-10	L_25000	250+00	23' LT	-L-	1.0-2.5	32.8	A-7-5(27)	11	61	38	23	100	100	91	11	1.9	9.2	36.9	52.0
SS-11	L_27000	270+00	20' RT	-L-	1.0-2.5	44.2	A-7-5(42)	6	76	41	35	100	99	93	8	2.3	5.6	27.6	64.5
SS-12	L_28526	285+26	18' LT	-L-	1.0-2.5	31.5	A-7-6(24)	9	55	27	28	95	91	79	21	7.5	13.5	24.7	54.3
SS-13	L_32001	320+01	46' LT	-L-	1.0-2.5	40.1	A-7-5(43)	11	74	36	38	100	99	91	11	1.6	9.6	26.9	61.9
SS-14	L_32982	329+82	42' RT	-L-	1.0-2.5	24.1	A-7-6(19)	14	51	22	29	97	87	70	30	15.0	15.0	2.8	67.2
SS-15	L_35079	350+79	37' LT	-L-	3.5-5.0	26.8	A-7-6(21)	7	51	22	29	96	94	73	30	5.7	24.1	35.0	35.2
SS-16	L_37500	375+00	23' LT	-L-	1.0-2.5	30.0	A-7-6(59)	6	93	25	68	98	96	80	21	7.1	13.6	16.7	62.6
SS-17	L_39788	397+88	13' RT	-L-	3.5-5.0	24.8	A-7-6(24)	5	59	27	32	95	89	73	27	11.9	14.8	33.7	39.6
SS-18	Y4_1300	13+00	21' RT	-Y4-	1.0-2.5	31.4	A-7-6(20)	7	56	28	28	99	91	72	30	12.6	17.7	14.7	55.0
SS-19	Y6_1350	13+50	CL	-Y6-	1.0-2.5	22.1	A-7-6(14)	7	44	18	26	96	85	62	39	18.4	20.3	18.6	42.8
SS-20	L_1986	19+86	15' LT	-L-	1.0-2.5	29.1	A-7-5(29)	14	59	34	25	100	99	94	9	1.2	7.4	31.5	59.9
SS-21	L_3000	30+00	15' LT	-L-	3.5-5.0	28.2	A-7-5(42)	22	73	37	36	100	99	94	9	1.4	7.3	21.6	69.7

SS = Split-Barrel Sample (ASTM-D-1586) ST = Shelby Tube (Undisturbed) Sample
 S = Grab Sample
 NP -- Non Plastic NA-- Non Applicable

Lab Technician: NCDOT Certification No.: 111-06-1203


 Rhonda Hudson