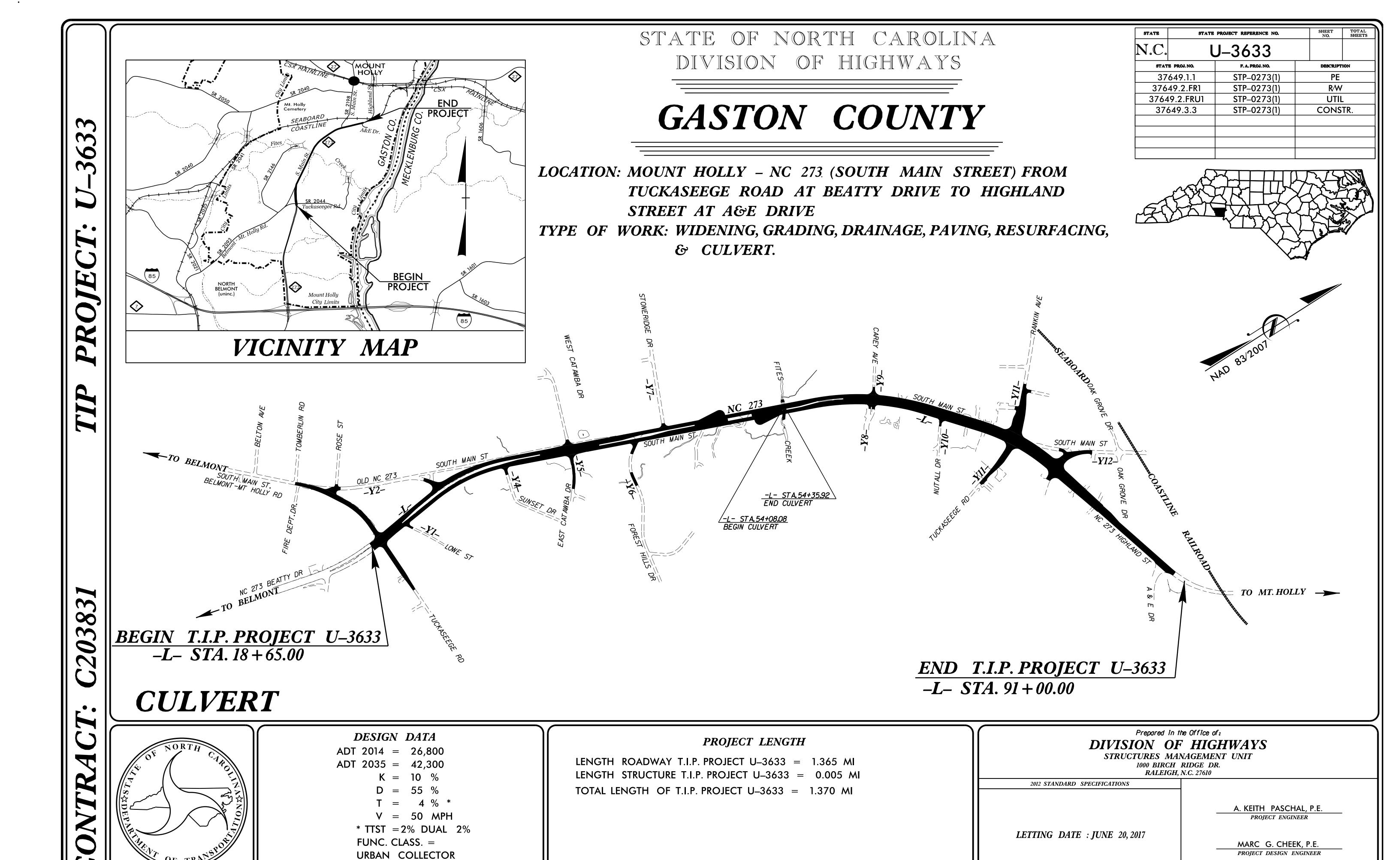
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REGIONAL TIER

ROADWAY DATA

GRADE POINT EL. @ STA. 54+22.00 -L- = 612.72 BED ELEVATION @ STA. 54+22.00 -L-= 587.2 ROADWAY SLOPES = 2 : 1

HYDRAULIC DATA

= 2262 C.F.S. DESIGN DISCHARGE = 50 YRS FREQUENCY OF DESIGN FLOOD DESIGN HIGH WATER ELEVATION = 599.1 DRAINAGE AREA = 3.9 SQ. MI. BASE DISCHARGE (Q100) = 2842 C.F.S. BASE HIGH WATER ELEVATION = 600.36

OVERTOPPING FLOOD DATA

CULVERT EXCAVATION

OVERTOPPING DISCHARGE = 6250 C.F.S. FREQUENCY OF OVERTOPPING FLOOD = > 500 YRS OVERTOPPING FLOOD ELEVATION = 612**.**54

TOTAL CULVERT QUANT	TITIES
CLASS A CONCRETE LEFT EXTENSION RIGHT EXTENSION TOTAL	121.6 C.Y. 324.8 C.Y. 446.4 C.Y.
REINFORCING STEEL LEFT EXTENSION RIGHT EXTENSION TOTAL	15,142 LBS. 45,119 LBS. 60,261 LBS.
FOUNDATION CONDITIONING MAT'L. LEFT EXTENSION	52 TONS

RIGHT EXTENSION 175 TONS 227 TONS TOTAL LUMP SUM

EXISTING CULVER 85'-0"± 56′-7″± 20'-0"± 20'-0"± 50'-0"± 4'-6"± 47'-0" 102'-0"

NOTES

ASSUMED LIVE LOAD HL-93 OR ALTERNATE LOADING.

DESIGN FILL: LEFT EXTENSION = 13.64 FEET, RIGHT EXTENSION = 13.61 FEET.

FOR OTHER DESIGN DATA AND NOTES, SEE STANDARD NOTES SHEET. 3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN CULVERT EXTENSIONS TO BE POURED IN THE FOLLOWING ORDER:

PHASE I

- 1. WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.
- 2. THE REMAINING PORTIONS OF PHASE I WALLS AND WING FULL HEIGHT.

PHASE II

- 3. FLOOR SLAB INCLUDING 4"OF VERTICAL WALL.
- 4. THE REMAINING PORTION OF PHASE II WALL FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALLS.

CONCRETE IN 72" Ø PIPE WINGS, FOOTINGS AND HEADWALL SHALL BE POURED IN THE FOLLOWING ORDER:

- 1. WINGS AND HEADWALL FOOTINGS UP TO CONSTRUCTION JOINT.
- 2. REMAINING PORTION OF WINGS AND HEADWALL FULL HEIGHT.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WALL AND BOTH FACES OF INTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

DOWELS SHALL BE USED TO CONNECT THE CULVERT EXTENSION TO THE EXISTING CULVERT AS SHOWN. FOR NOTE REGARDING SETTING OF DOWELS, SEE SHEET SN.

F8B6AD6DB2FC48F...

IF APPROVED BY THE ENGINEER, THE CONTRACTOR MAY USE THE EXISTING WINGS AS TEMPORARY SHORING FOR THE CONSTRUCTION OF THE CULVERT EXTENSIONS. IN THIS CASE, THE BOTTOM SLAB OF THE EXTENSION SHALL BE POURED AT LEAST 72 HOURS PRIOR TO CUTTING THE WINGS. THE WINGS MAY BE CUT EARLIER PROVIDED THE SLAB CONCRETE STRENGTH HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 1500 PSI.

FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS.

NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL. TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED IN THE BARREL, SPACED TO LIMIT THE POURS TO A MAXIMUM OF 70 FT. LOCATION OF JOINTS SHALL BE SUBJECT TO APPROVAL OF THE ENGINEER.

> HEREBY CERTIFY THESE PLANS ARE THE AS-BUILT PLANS

PROJECT NO. <u>U-3633</u> GASTON COUNTY 54+22.00 -L-

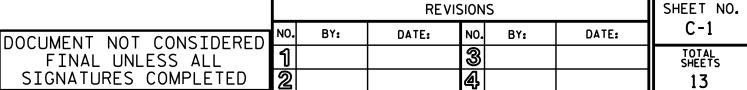
SHEET 1 OF 13

CULVERT No. C437

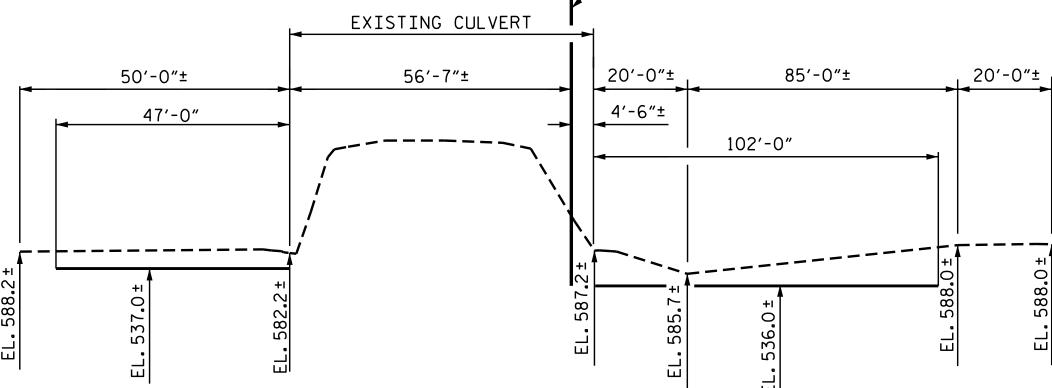
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

EXTENSION WITH

72"Ø PIPE



4/5/2017



PROFILE ALONG & CULVERT



N.D'AIUTO DATE : 7/30/16 DRAWN BY : H. A. LOCKLEAR __ DATE : <u>8/4/16</u> CHECKED BY : _

DESIGN ENGINEER OF RECORD: H.A.LOCKLEAR DATE: 11/16

29-MAR-2017 II:58 L:\Structures\FinalPlans\U3633_SD_CU_0I.dgn

BENCHMARK #5: RR SPIKE IN 22" Ø OAK,

169' RT. OF STA. 51+30.00 -L-, EL. 599.61

ALONG

90°-00′-00″

& CULVERT

-CLASS I RIP RAP

(TYP.) (ROADWAY

PAY ITEM & DETAIL)

NOTE: FOR UTILITY INFORMATION,

SEE UTILITY PLANS & SPECIAL

PROVISIONS.

EXISTING

NC 273

11'-0"

| ALONG

LOCATION SKETCH

-EXISTING TRIPLE

18' X 12' RCBC

EXTENSION

PROPOSED

GUARDRAIL

PAY ITEM)

(TYP.) (ROADWAY

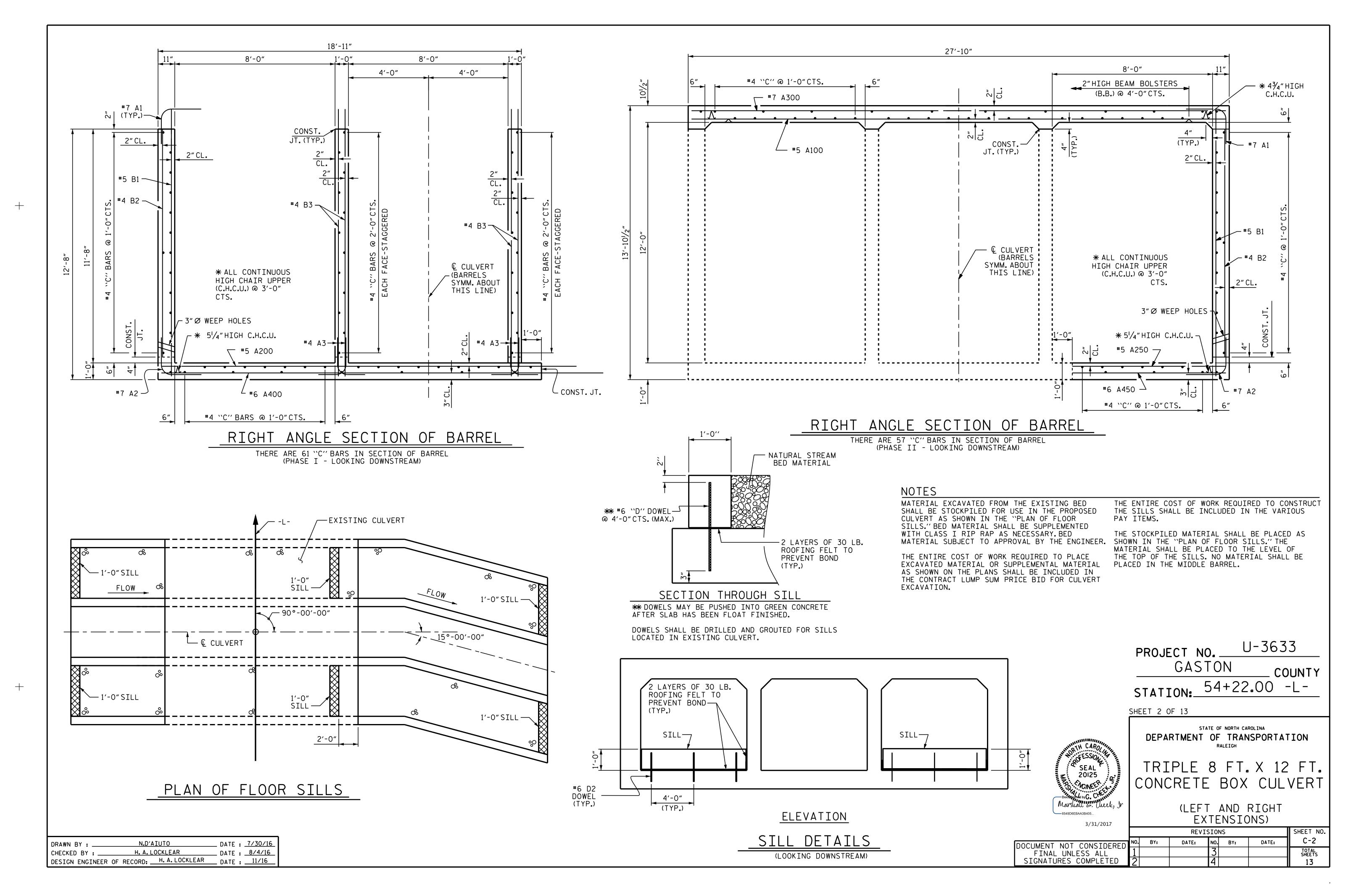
72"Ø PIPE

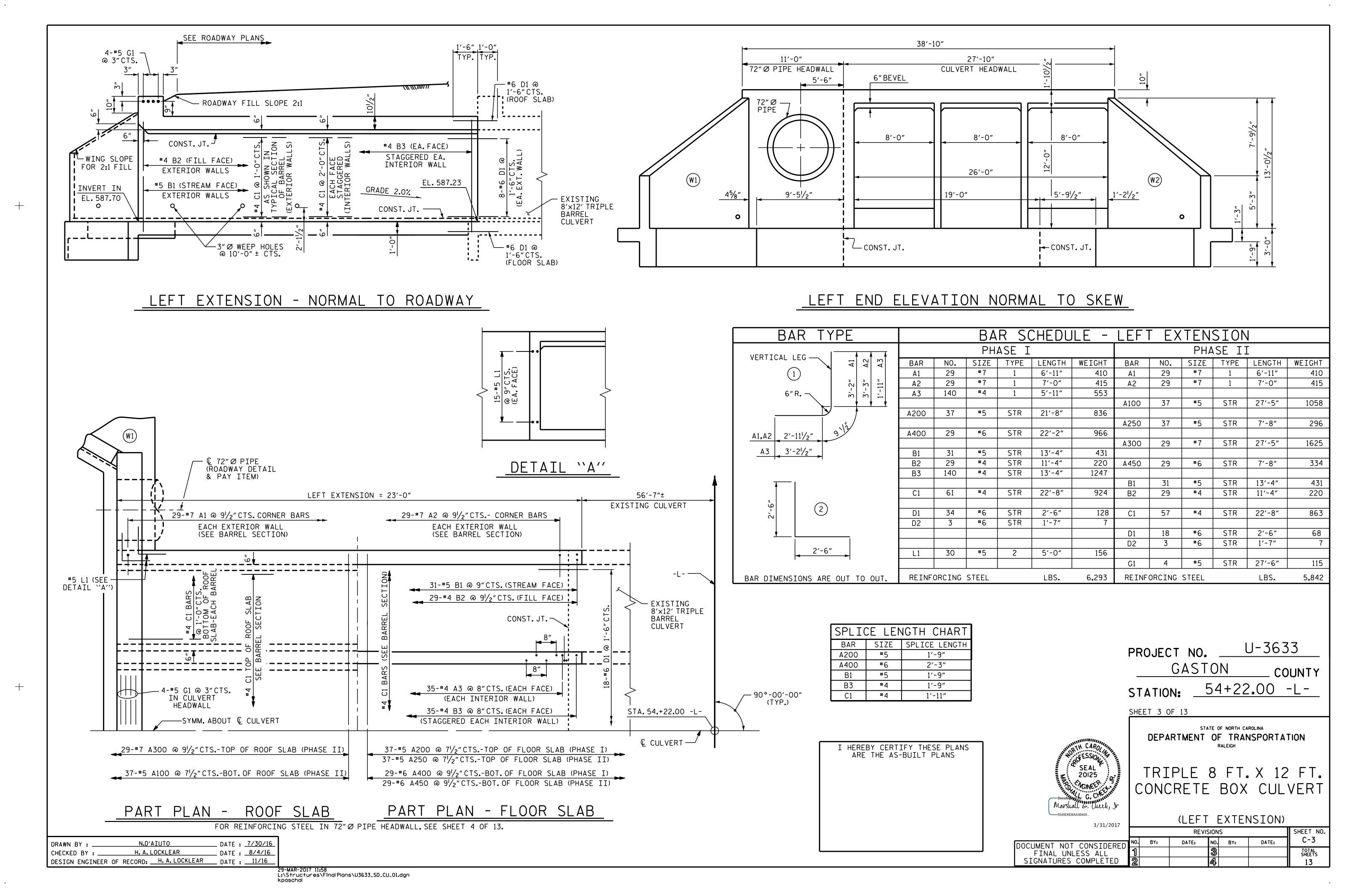
STA. 54+22.00 -L-

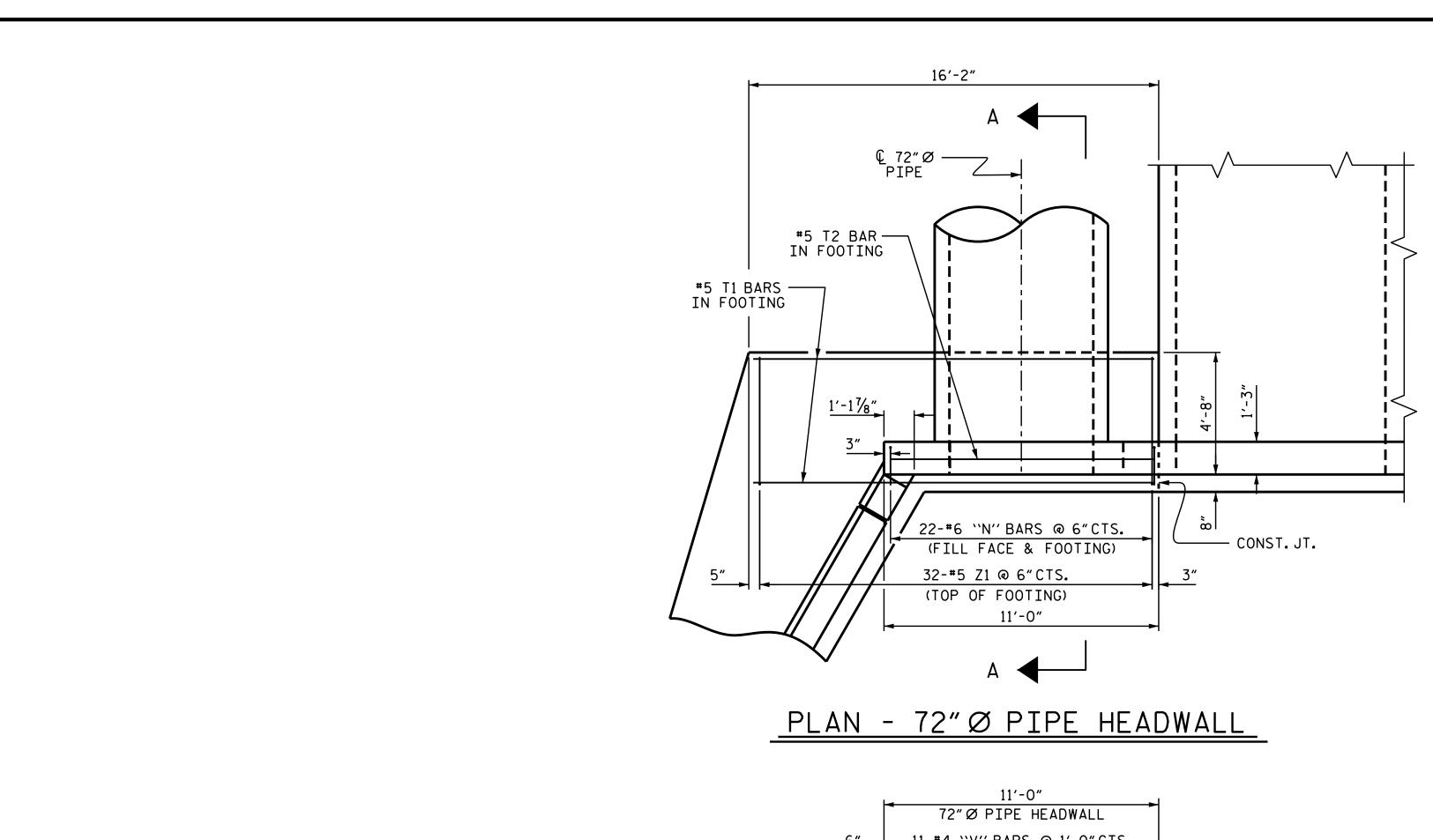
-(ROADWAY PAY ITEM)

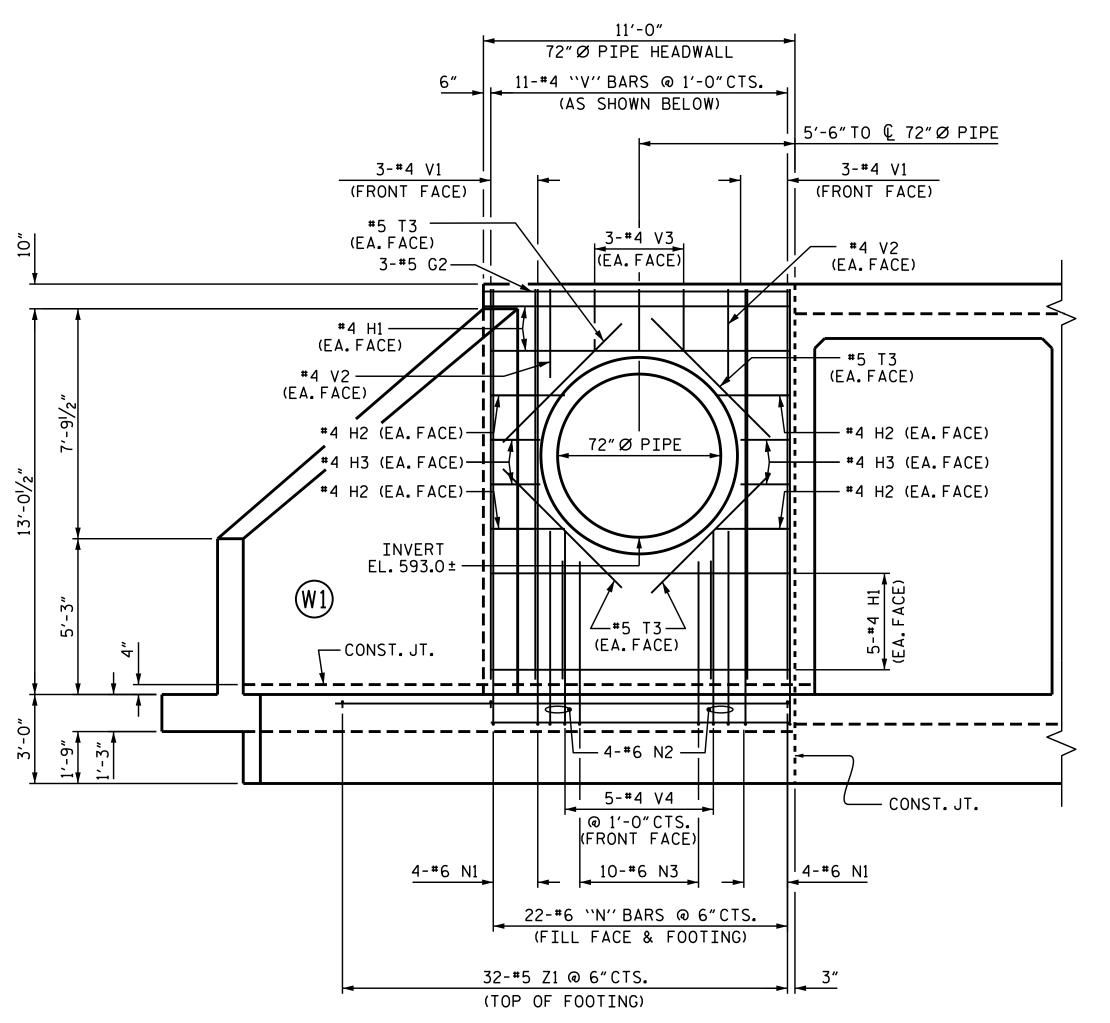
FITES CREEK

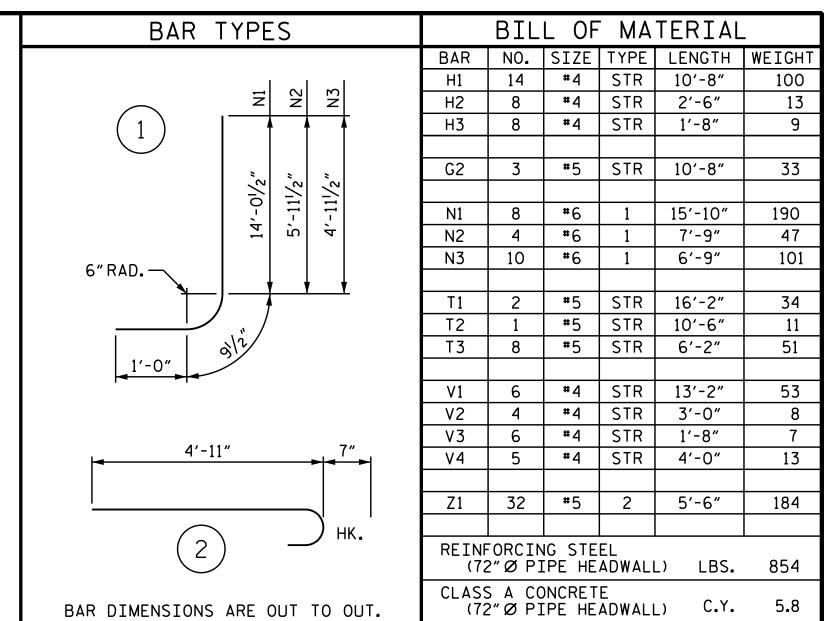
WOODS











LEFT	EXTEN	ISION	QUANT	ITIES	·
CLASS A CO	NCRETE				
PHASE I B	ARRELS @	1.998	CY/FT_	46.0	C.Y.
PHASE II I	BARREL @	1.651	CY/FT_	38.0	C.Y.
72″Ø PIPE	HEADWAL	<u> </u>		5.8	C.Y.
WING W1, E				16.6	
WING W2, E	TC			15.2	C.Y.
TOT	AL			121.6	C.Y.
REINFORCIN	G STEEL				
PHASE I B	ARRELS			6,293	LBS
PHASE II I	BARREL			5,842	LBS
72″Ø PIPE	HEADWAL	L		854	LBS
WING W1, E	TC			1,241	LBS
WING W2, E	TC			912	LBS
ТОТ	AL			15,142	LBS
FOUNDATION	CONDITI	ONING W	IAT'L	52	TONS
CULVERT EX	CAVATION	l		LUMP	SUM

PROJECT NO. U-3633 GASTON _ COUNTY STATION: 54+22.00 -L-

SHEET 4 OF 13

3-#5 G2 || || ||

└─ #5 T2

4'-8"

SECTION A-A

FRONT FACE

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

72" Ø PIPE HEADWALL

(LEFT EXTENSION)

3/31/2017 SHEET NO. REVISIONS C-4 DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED BY: TOTAL SHEETS

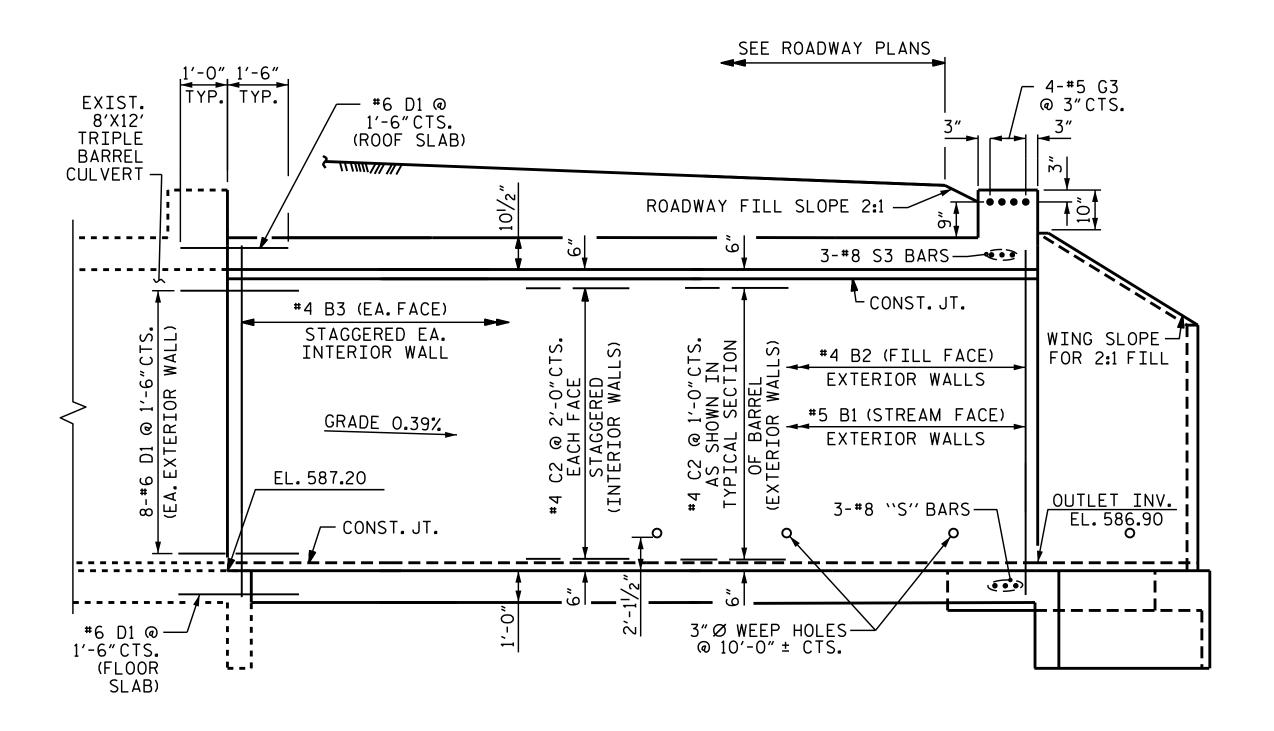
ELEVATION - 72" Ø PIPE HEADWALL

H.A. LOCKLEAR

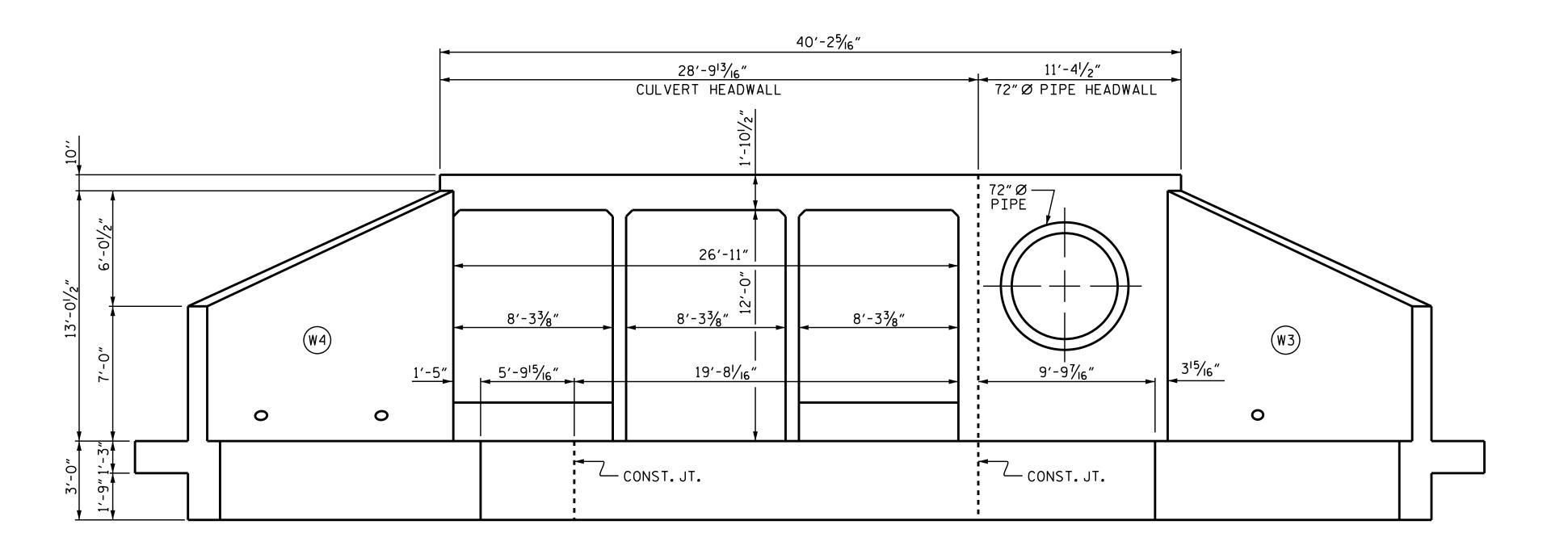
CHECKED BY: K.D.LAYNE DATE: 8/4/16
DESIGN ENGINEER OF RECORD: H.A.LOCKLEAR DATE: 11/16

DRAWN BY :

_ DATE : <u>3/31/16</u>



RIGHT EXTENSION - NORMAL TO ROADWAY



RIGHT END ELEVATION NORMAL TO SKEW

I HEREBY CERTIFY THESE PLANS ARE THE AS-BUILT PLANS

PROJECT NO. U-3633

GASTON COUNTY

STATION: 54+22.00 -L-

SHEET 5 OF 13

DEPARTMENT OF TRANSPORTATION
RALEIGH

TRIPLE 8 FT. X 12 FT

TRIPLE 8 FT. X 12 FT. CONCRETE BOX CULVERT

(RIGHT EXTENSION)

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2 A SHEET NO.

REVISIONS

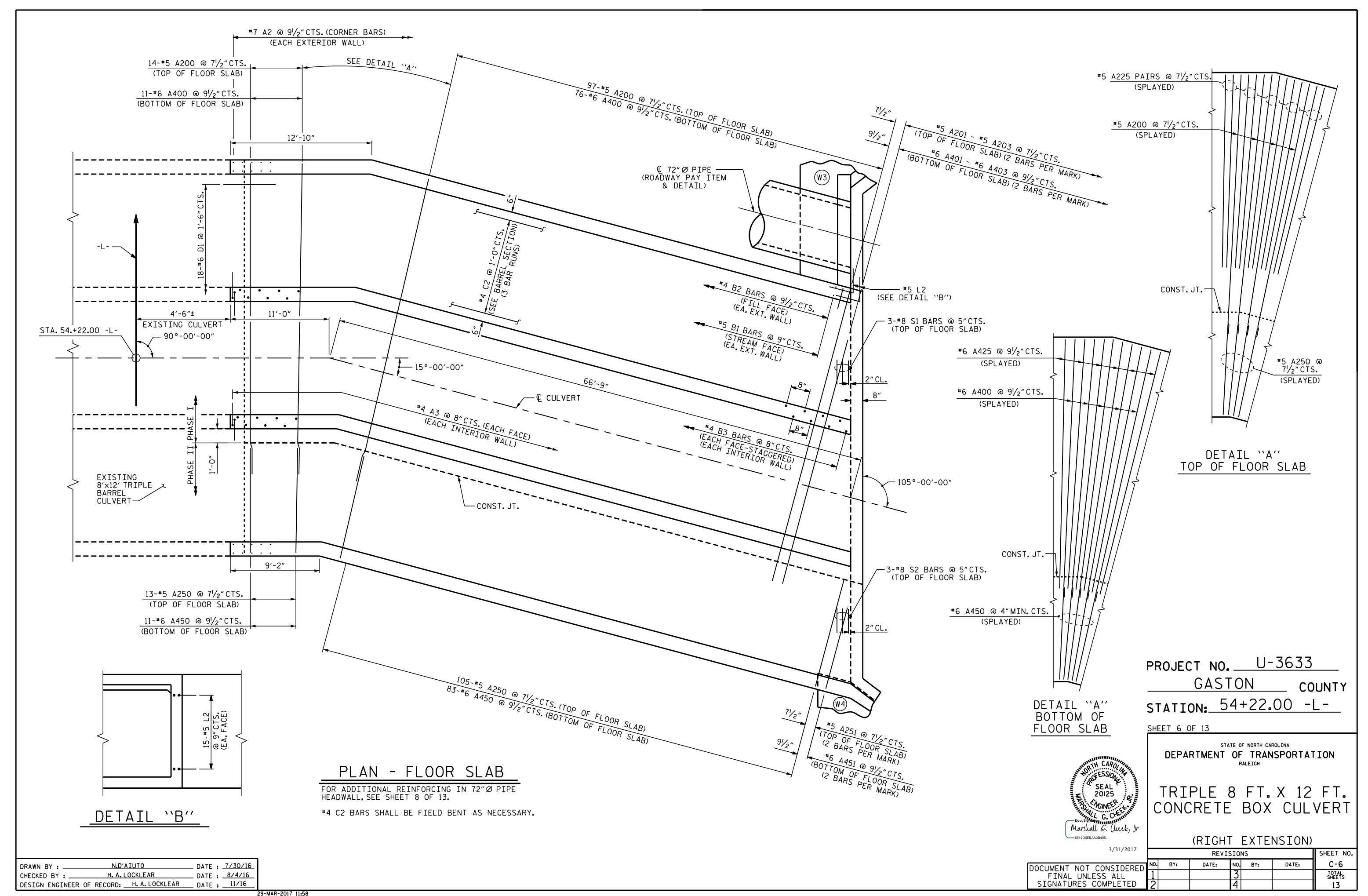
BY: DATE: NO. BY: DATE: C-5

TOTAL SHEETS

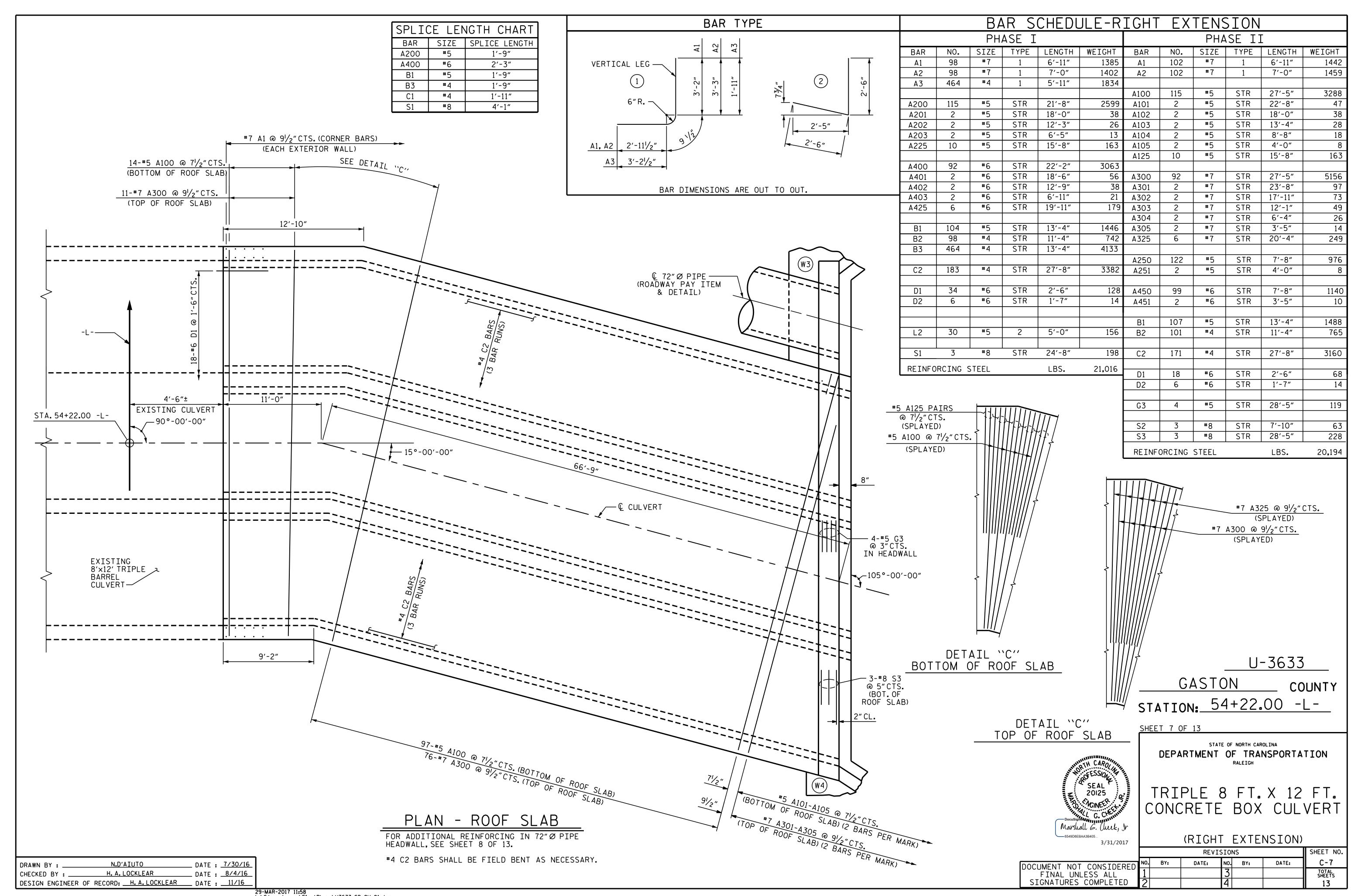
1 3 13

DRAWN BY: N.D'AIUTO DATE: 7/30/16
CHECKED BY: H.A.LOCKLEAR DATE: 8/4/16
DESIGN ENGINEER OF RECORD: H.A.LOCKLEAR DATE: 11/16

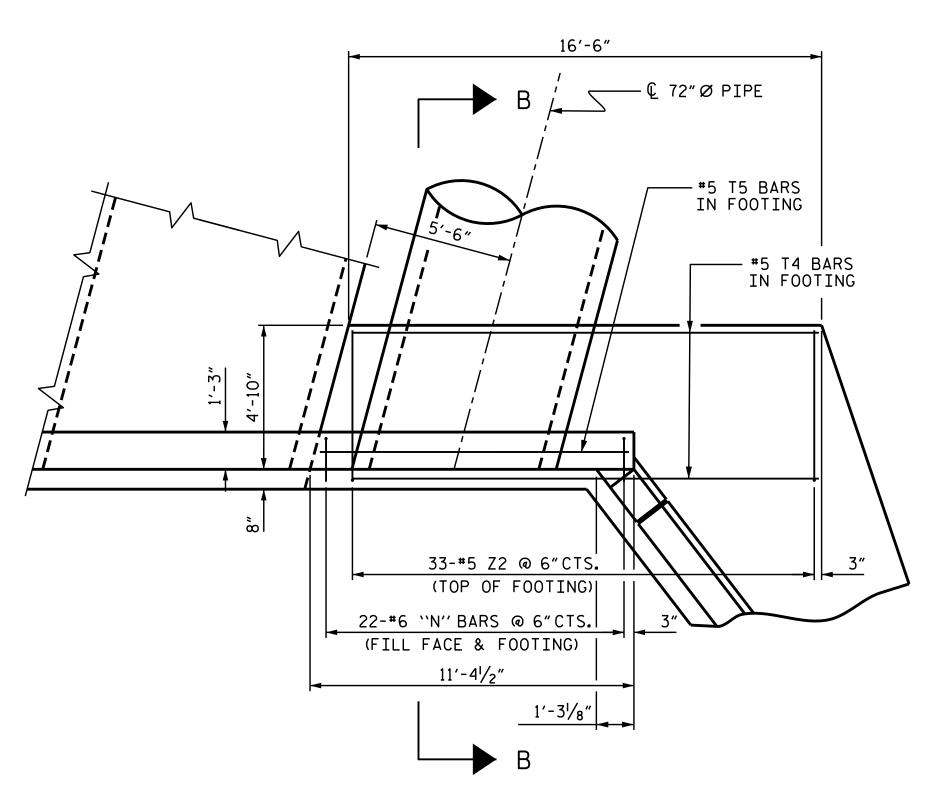
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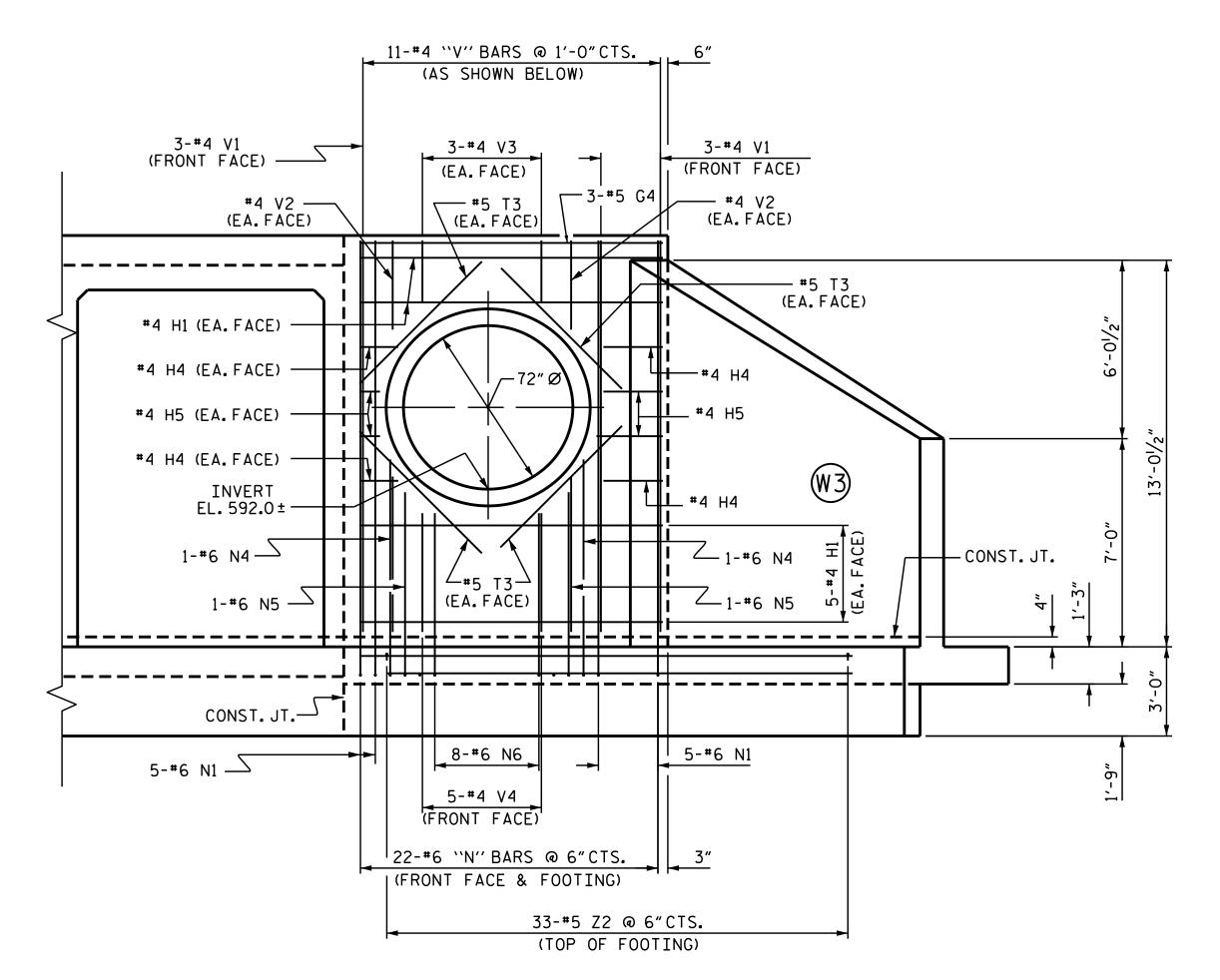
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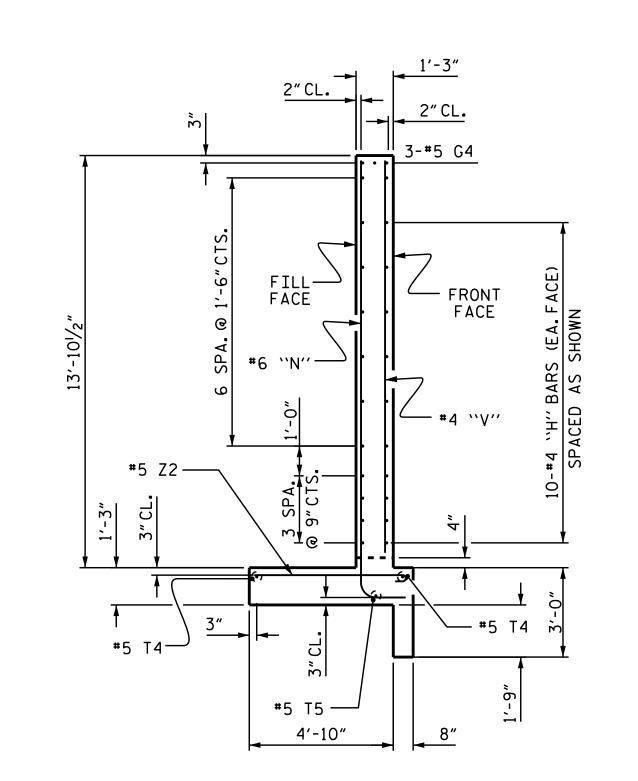
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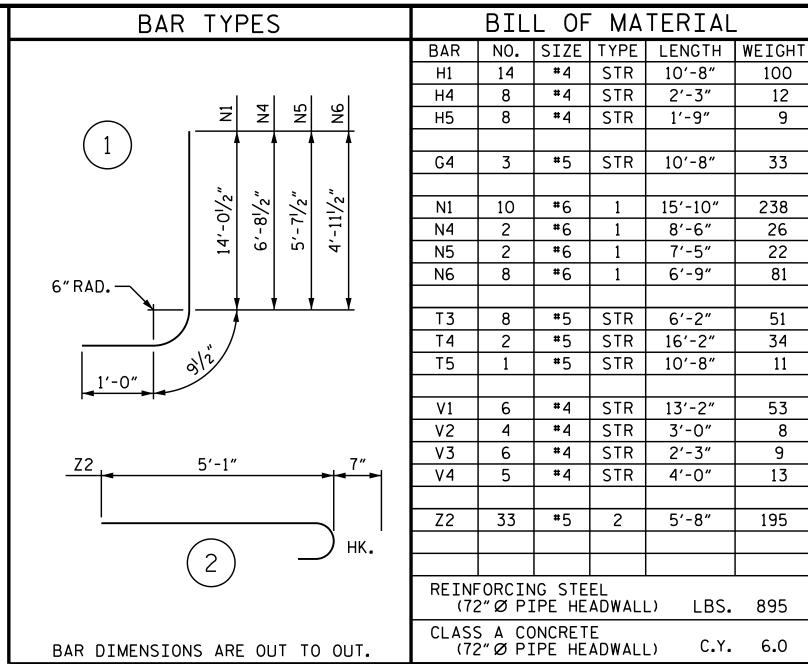
PLAN - 72"Ø PIPE HEADWALL



<u>ELEVATION - 72" Ø PIPE HEADWALL</u>



SECTION B-B



RIGHT EXTENSION QUANTIT	[ES
CLASS A CONCRETE	
PHASE I BARRELS @1.998CY/FT155.3	C.Y.
PHASE II BARREL @1.651CY/FT128.4	C.Y.
72"Ø PIPE HEADWALL6.0	C.Y.
WING W3, ETC15.7	C.Y.
WING W4, ETC	C.Y.
TOTAL324.8	C.Y.
REINFORCING STEEL	
PHASE I BARRELS21,016	LBS.
PHASE II BARREL 20,19	4_LBS.
72"Ø PIPE HEADWALL89	<u> </u>
WING W3, ETC. 1,392	2 LBS.
WING W4, ETC1,62	2_LBS.
TOTAL45,119	BS.
FOUNDATION CONDITIONING MAT'L. 17	<u>5</u> TONS
CULVERT EXCAVATION LUN	MP SUM

PROJECT NO. U-3633

GASTON COUNTY

STATION: 54+22.00 -L-

SHEET 8 OF 13

SEAL 20125

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

72"Ø PIPE HEADWALL

(RIGHT EXTENSION)

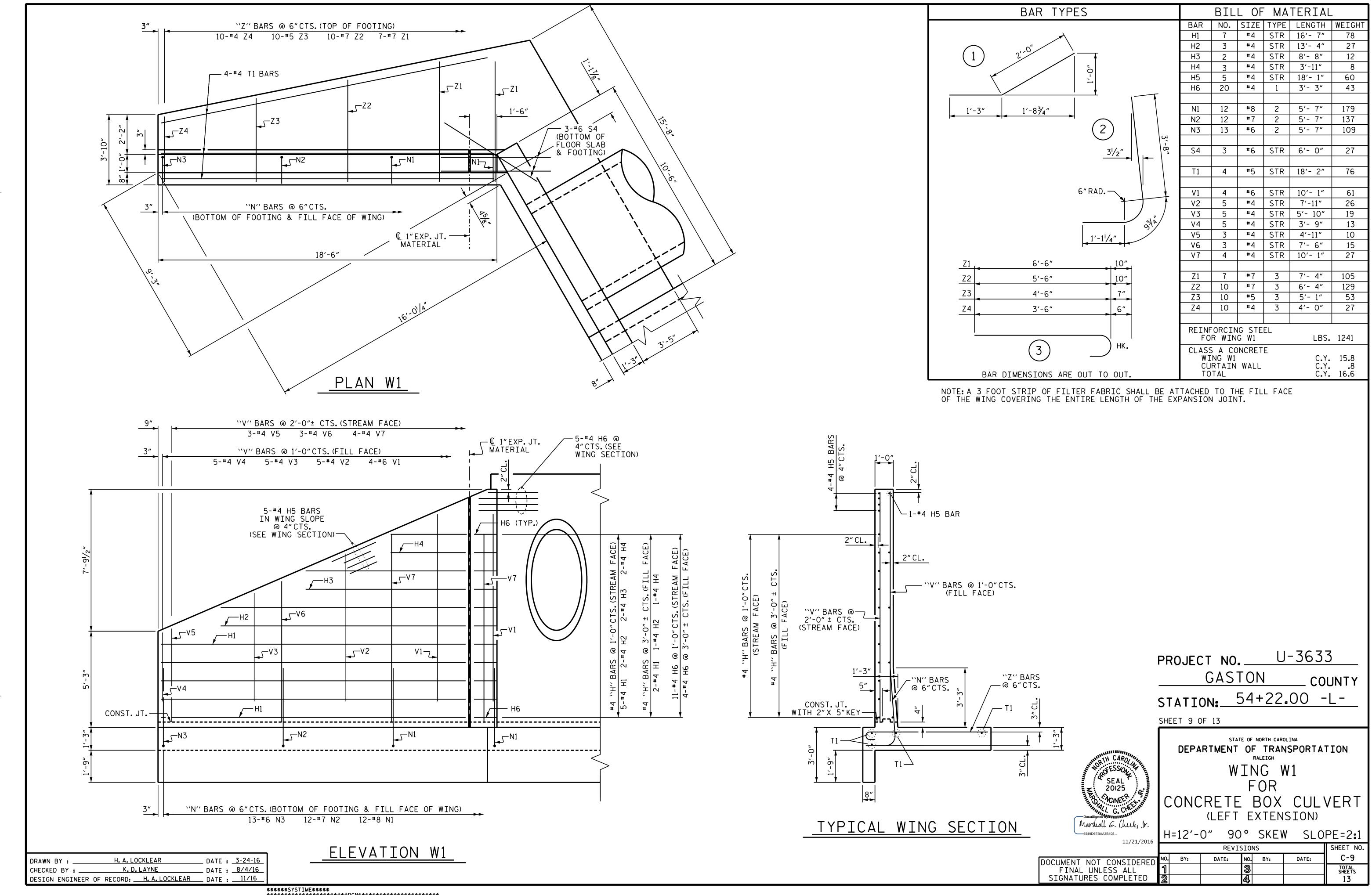
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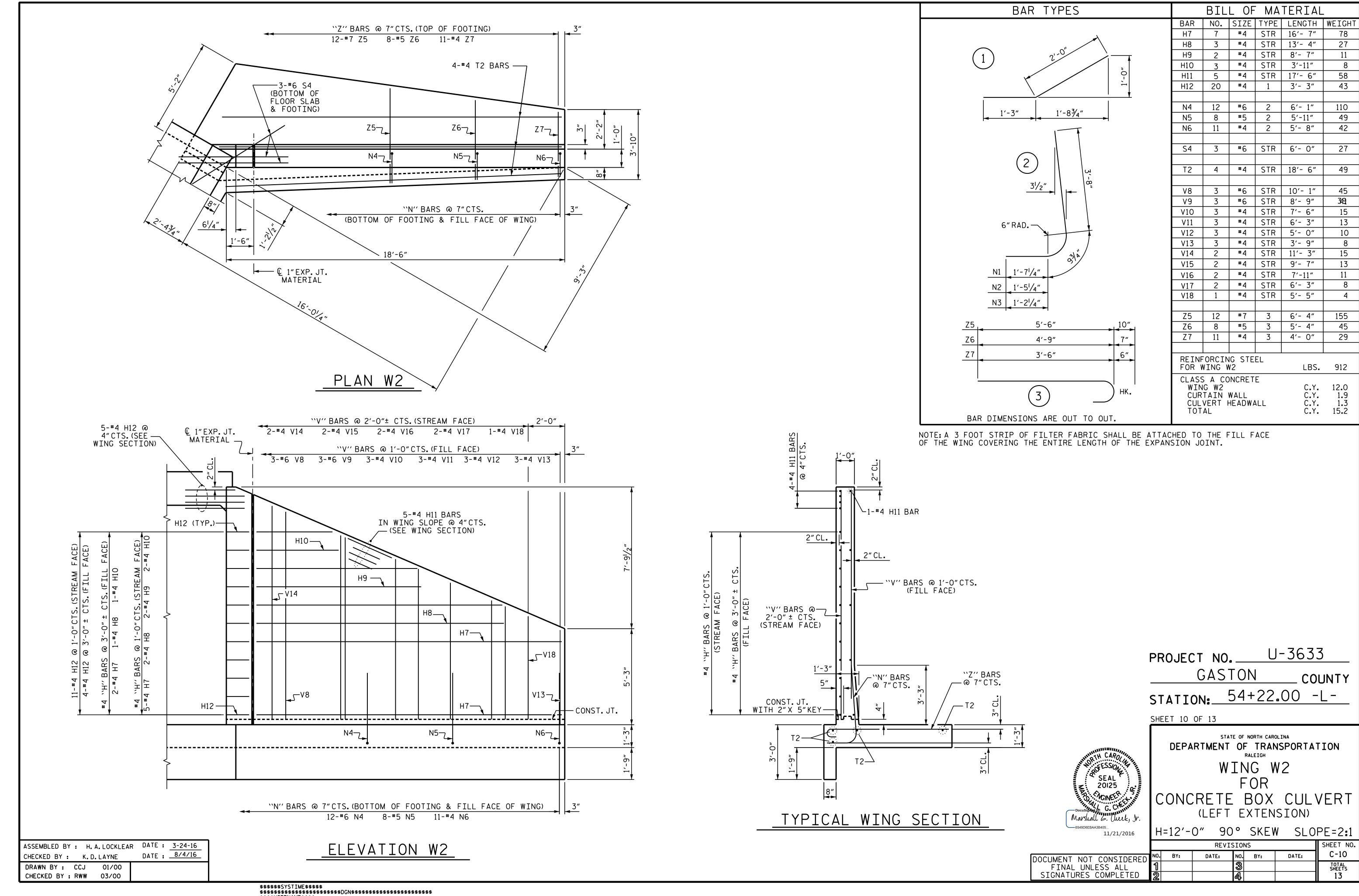
DRAWN BY: H.A. LOCKLEAR DATE: 3/31/16

CHECKED BY: K.D. LAYNE DATE: 8/4/16

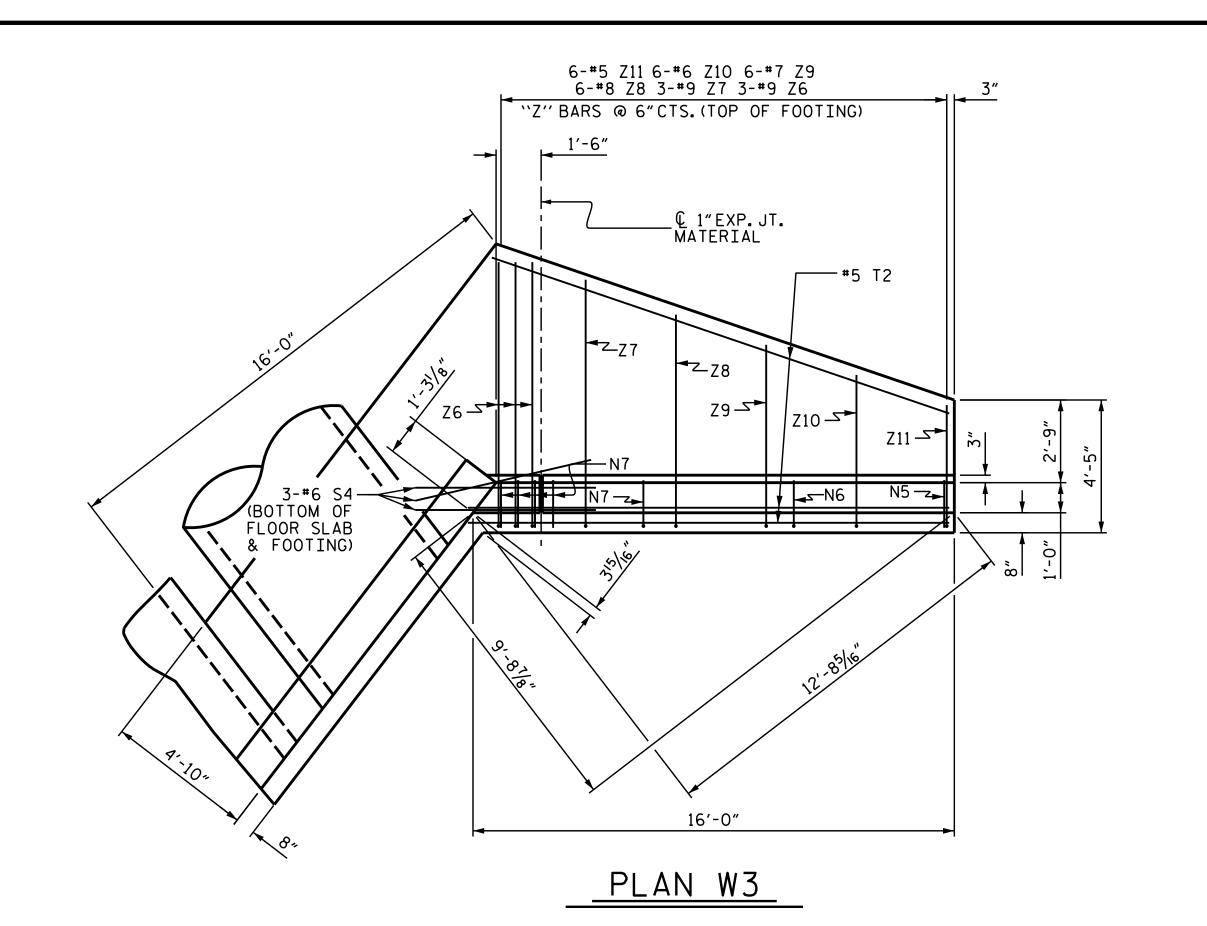
DESIGN ENGINEER OF RECORD: H.A. LOCKLEAR DATE: 11/16

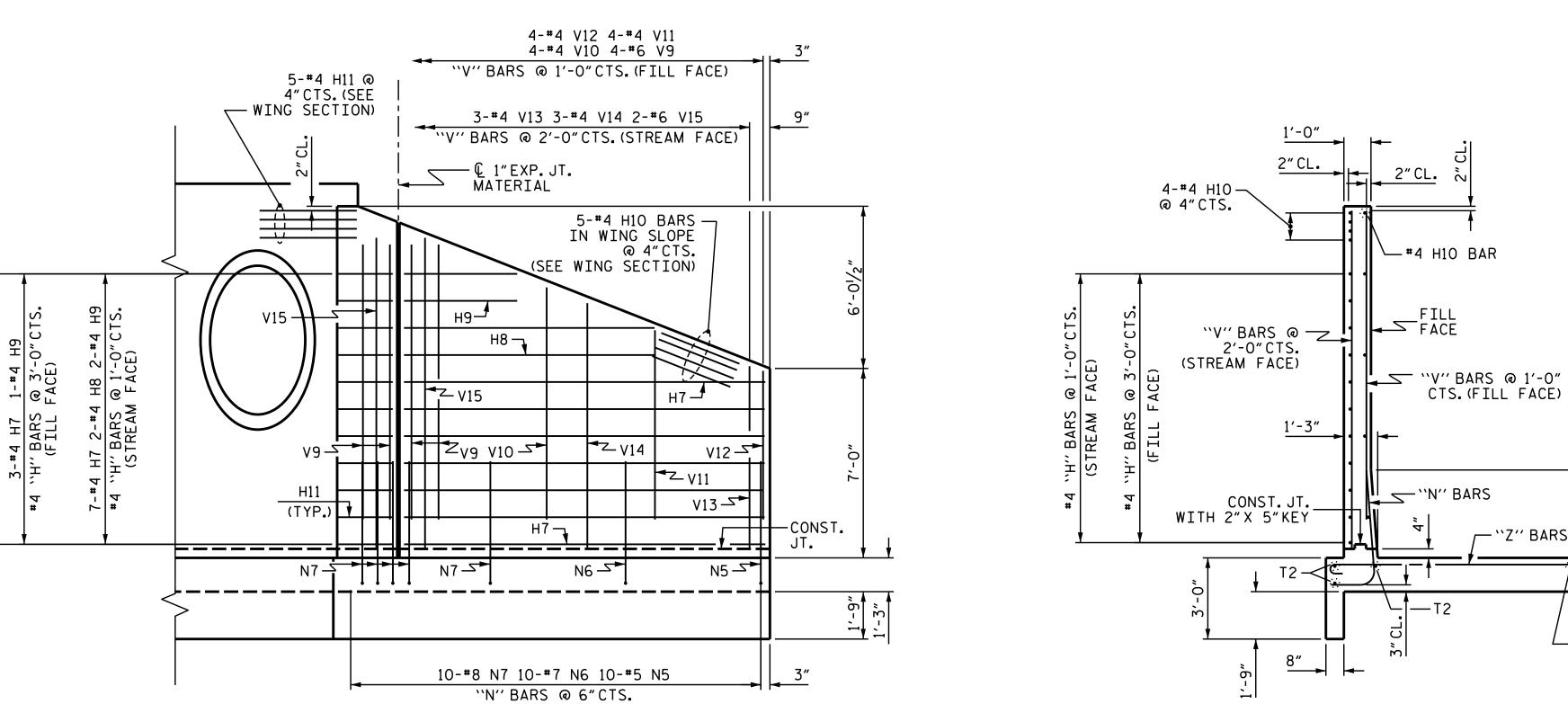
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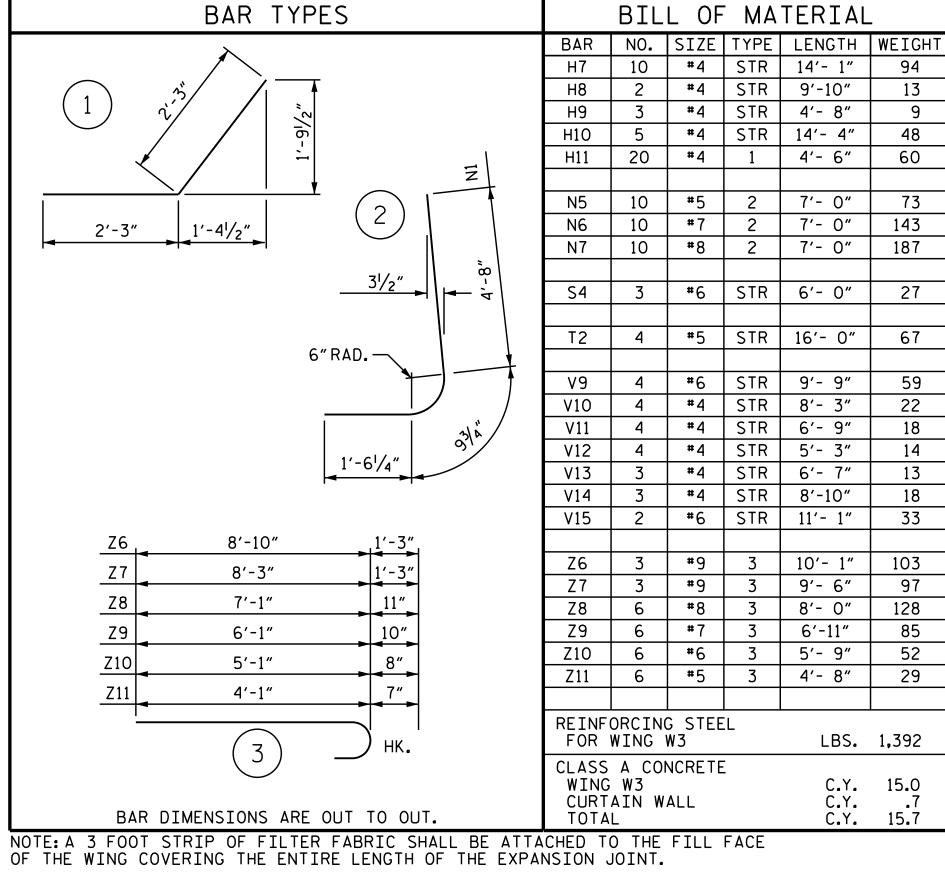


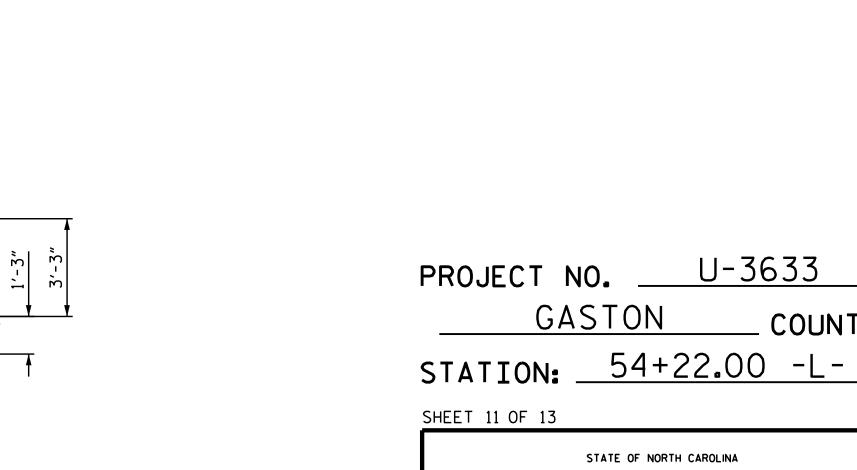
\$\$\$\$USERNAME\$\$\$\$











SEAL 20125 NOINEER

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

WING W3 FOR

U-3633

COUNTY

CONCRETE BOX CULVERT (RIGHT EXTENSION)

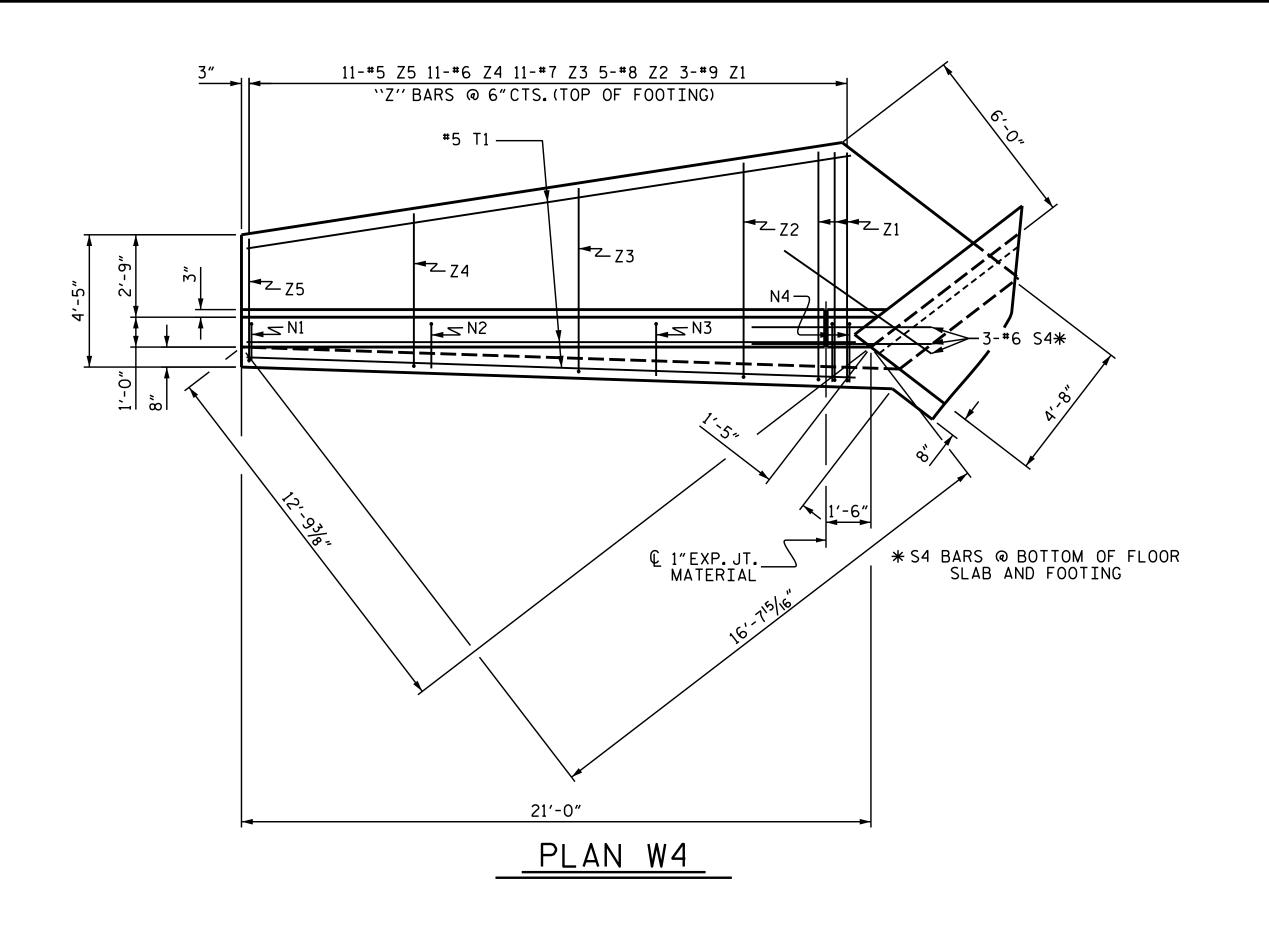
105° SKEW SLOPE=2:1 H=12'-0"

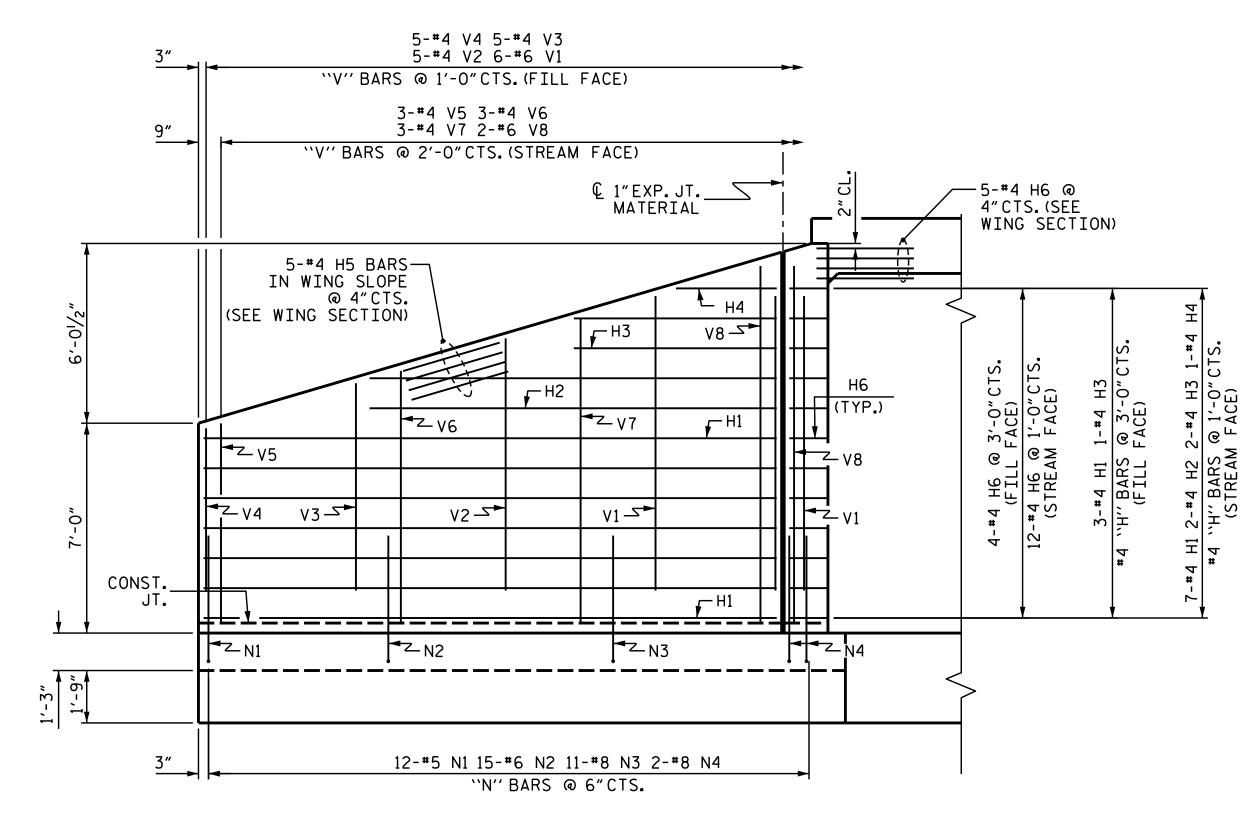
			SHEET NO.				
JMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	C-11
FINAL UNLESS ALL	[1]			3			TOTAL SHEETS
GNATURES COMPLETED	2			4			13

TYPICAL WING SECTION

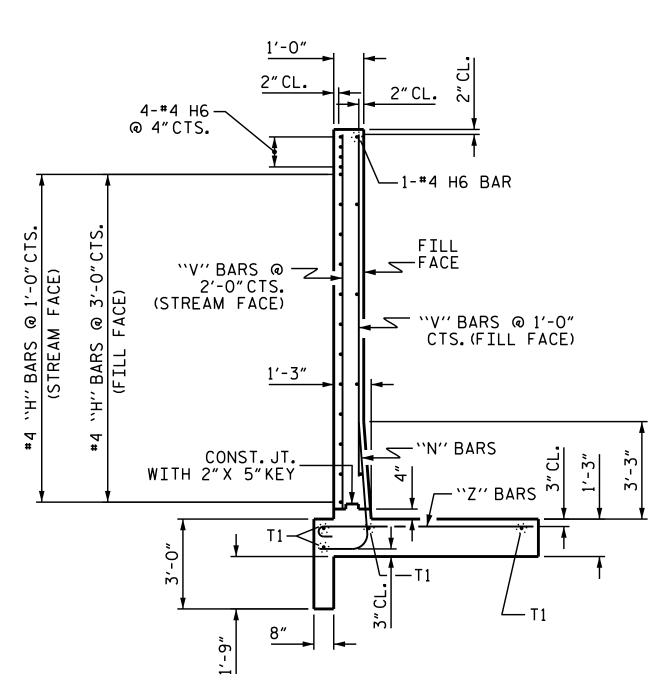
"Z" BARS

H.A. LOCKLEAR _ DATE : <u>3-31-16</u> DRAWN BY : CHECKED BY: K.D.LAYNE DATE: 8/4/16
DESIGN ENGINEER OF RECORD: H.A.LOCKLEAR DATE: 11/16









PROJECT NO. <u>U-3633</u> GASTON COUNTY STATION: 54+22.00 -L-

BILL OF MATERIAL

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

#4 STR 19'- 1"

#4 | STR | 3'- 2"

#4 | STR | 19'-11"

#6 | STR | 9'- 6"

#4 | STR | 6'- 8"

#4 | STR | 5'- 2"

3 | 8'- 1"

3 7'- 0"

3 | 5'- 9"

5 | #4 | STR | 8'- 1"

3 | #4 | STR | 6'- 7"

3 | #4 | STR | 8'- 4"

3 #4 STR 10'- 1"

2 | #6 | STR | 11'- 9"

5 | #8 |

11 | #6 |

REINFORCING STEEL FOR WING W4

CLASS A CONCRETE

CULVERT HEADWALL

CURTAIN WALL

WING W4

#7

Z5 | 11 | #5 | 3 | 4'- 8"

Z2

Z4

11"

10"

8"

7″

NOTE: A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

12 | #5 | 2 | 6'- 9"

1 3'- 3"

2 7'- 0"

2 7'- 3"

2 | 7'- 5"

#4

#6

#8

#8

S4 | 3 | #6 | STR | 6'- 0"

T1 | 4 | #5 | STR | 20'- 4"

21

13

67

46

158

213

40

27

27

22

13

35

108

95

54

LBS. 1,622

C.Y. 16.1 C.Y. 2.0 C.Y. 1.3 C.Y. 19.4

#4 | STR | 13'- 7" #4 | STR | 6'- 7"

SHEET 12 OF 13

SEAL 20125

BAR TYPES

1'-7"

31/2"

6" RAD. —

1'-61/4"

1'-91/4"

1'-111/4"

7′-8"

7'-2"

6'-2"

5′-1″

4'-1"

BAR DIMENSIONS ARE OUT TO OUT.

Ν3

Z3

Z4

1'-3"

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

WING W4 FOR

CONCRETE BOX CULVERT (RIGHT EXTENSION)

105° SKEW SLOPE=2:1 H=12'-0"

11/21/2016 **REVISIONS** SHEET NO. C-12 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS

TYPICAL WING SECTION

ASSEMBLED BY: H. A. LOCKLEAR DATE: 3-24-16
CHECKED BY: K. D. LAYNE DATE: 8/4/16 DRAWN BY: CCJ 01/00 CHECKED BY: RWW 03/00

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERT

										STRENGTH	TRENGTH I LIMIT STATE					
										MOMENT				SHEAR		
TEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.67		1.75	2.49	1	EXTERIOR WALL	6.47	1.67	1	EXTERIOR WALL	11.78	
DESIGN LOAD		HL-93 (OPERATING)	N/A		2.16		1.35	3.23	1	EXTERIOR WALL	6.47	2.16	1	EXTERIOR WALL	11.78	
RATING		HS-20 (INVENTORY)	36.000	2	1.70	61.08	1.75	2.49	1	EXTERIOR WALL	6.47	1.70	1	EXTERIOR WALL	11.78	
		HS-20 (OPERATING)	36.000		2.20	79 . 18	1.35	3.23	1	EXTERIOR WALL	6.47	2.20	1	EXTERIOR WALL	11.78	
		SNSH	13.500		2.16	29.10	1.40	3.10	1	EXTERIOR WALL	6.47	2.16	1	EXTERIOR WALL	11.78	
	 	SNGARBS2	20.000		2.15	43.10	1.40	3 . 10	1	EXTERIOR WALL	6.47	2.15	1	EXTERIOR WALL	11.78	
	ICL	SNAGRIS2	22.000		2.16	47.42	1.40	3 . 10	1	EXTERIOR WALL	6.47	2.16	1	EXTERIOR WALL	11.78	
	SLE VEHICLE (SV)	SNCOTTS3	27.250		2 . 11	57 . 58	1.40	3.10	1	EXTERIOR WALL	6.47	2.11	1	EXTERIOR WALL	11.78	
		SNAGGRS4	34 . 925		2.12	73.95	1.40	3.10	1	EXTERIOR WALL	6.47	2.12	1	EXTERIOR WALL	11.78	
	SINGL	SNS5A	35 . 550	3	2.11	75.02	1.40	3 . 10	1	EXTERIOR WALL	6.47	2.11	1	EXTERIOR WALL	11.78	
		SNS6A	39.950		2.11	84.33	1.40	3.10	1	EXTERIOR WALL	6.47	2.11	1	EXTERIOR WALL	11.78	
LEGAL LOAD		SNS7B	42.000		2 . 11	88.64	1.40	3.10	1	EXTERIOR WALL	6.47	2.11	1	EXTERIOR WALL	11.78	
RATING	ER	TNAGRIT3	33.000		2.12	69.92	1.40	3.10	1	EXTERIOR WALL	6.47	2.12	1	EXTERIOR WALL	11.78	
	RAII	TNT4A	33 . 075		2.12	70.10	1.40	3.10	1	EXTERIOR WALL	6.47	2.12	1	EXTERIOR WALL	11.78	
	SEMI-TRAILE	TNT6A	41.600		2.11	87.88	1.40	3.10	1	EXTERIOR WALL	6.47	2.11	1	EXTERIOR WALL	11.78	
	SEN ST)	TNT7A	42.000		2.12	88.84	1.40	3.10	1	EXTERIOR WALL	6.47	2.12	1	EXTERIOR WALL	11.78	
	CTOR (TTS)	TNT7B	42.000		2.11	88.73	1.40	3.10	1	EXTERIOR WALL	6.47	2.11	1	EXTERIOR WALL	11.78	
	TRAC	TNAGRIT4	43.000		2.12	91.10	1.40	3.10	1	EXTERIOR WALL	6.47	2.12	1	EXTERIOR WALL	11.78	
	TRUCK	TNAGT5A	45.000		2.11	95.17	1.40	3.10	1	EXTERIOR WALL	6.47	2.11	1	EXTERIOR WALL	11.78	
	TRL	TNAGT5B	45.000		2.11	95.03	1.40	3.10	1	EXTERIOR WALL	6.47	2.11	1	EXTERIOR WALL	11.78	

	8'-0" (TYP.)			
				,,0
1 2 3				12'-0" (TYP.)
l	BOX 1	BOX 2	BOX 3	J
		LRFR SUMMARY		

LOOKING DOWNSTREAM

DATE : 10/4/16 DATE : 10/4/16 ASSEMBLED BY : M.K. BEARD CHECKED BY : H.A. LOCKLEAR DRAWN BY: WMC 7/II CHECKED BY: GM 7/II

LOAD FACTORS

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
ЕН	1.35	0.90
ES	1.35	0.90
LS	1.75	
WA	1.00	

NOTE

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

PROJECT NO. U-3633

GASTON ___ COUNTY

STATION: 54+22.00 -L-

STATE OF NORTH CAROLINA

SHEET 13 OF 13

DEPARTMENT OF TRANSPORTATION

11/21/2016

STANDARD LRFR SUMMARY FOR REINFORCED CONCRETE BOX CULVERT (NON-INTERSTATE TRAFFIC)

REVISIONS C-13 DATE: NO. BY: DATE:

STD. NO. LRFR5

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ. IN.
- AASHTO M270 GRADE 50 -	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.

MATERIAL AND WORKMANSHIP:

EQUIVALENT FLUID PRESSURE OF EARTH

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

30 LBS. PER CU. FT.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH