REFERENCE: B-4830

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4830	1	8

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY WAKE

PROJECT DESCRIPTION BRIDGE NO. 20 ON -L- (NC 97) OVER MOCCASIN CREEK

CONTENTS

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CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1999 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CEMERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IMP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOL. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NDICATED IN THE SUBSURFACE OR INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

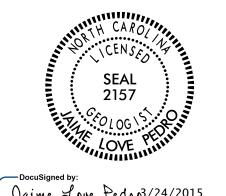
- NOTES:

 I. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

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_ <u>J</u>	I. R. SWARTLEY
_1	D. G. PINTER
INVESTIGATED BY	J. L. PEDRO
DRAWN BY <u>J. L</u>	. PEDRO
CHECKED BY N	. T. ROBERSON
SUBMITTED BY	N. T. ROBERSON
DATE MARCE	H 2015

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Jaime Love Pedro3/24/2015

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DATE

PROJECT REFERENCE NO.	SHEET NO.
B-4830	2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS (PAGE 1 OF 2)

(PAGE 1 OF 2)																	
	SOIL DESCRIPTION												GRADATION				
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 180 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:								WEATHE YIELD 206, AS GENERAL	LESS M DIS LY IN	THAN 100 586). SOIL CLUDE THE	<u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.						
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,										, ETC. FOR	EXAMPLE.	ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:					
VERY STIFF,GRAY,SILTY CLAY,MOIST WITH INTERBEDDED FINE SAND LAYERS,HIGHLY PLASTIC,A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION											ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.						
GENERAL	(GRANUL	AR MATERIALS	S		SILT	-CLAY	MATERIAL	<u> </u>		GANIC MATERI	IALS	MINERALOGICAL COMPOSITION				
CLASS. GROUP		≤ 35% A-3	PASSING *200	9) 4-2				SING #20 A-6	_	A-1, A-2 A-4, A-5			MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.				
CLASS.	A-1-a A-1-b		A-2-4 A-2-5	A-2-6		CODE INTO DE IN			7-5 -7-6	A-1, A-2 A-3 A-6, A-7			COMPRESSIBILITY				
SYMBOL				%	%		77.7						SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50				
% PASSING	50 MX									GRANULAR	SILT-	MUCK,	HIGHLY COMPRESSIBLE LL > 50 PERCENTAGE OF MATERIAL				
■40	30 MX 50 MX 1 15 MX 25 MX 1		35 MY 35 MY	(35 MY	35 MY	36 MN	36 MN	36 MN 3		SOILS	CLAY SOILS	PEAT	GRANULAR SILT - CLAY				
MATERIAL	13 14 23 14 1	ID PIA	33 FIX 33 FIX	1 33 FIA	33 HX	30 1414	30 1111	30 1414 3	1-114				TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%				
PASSING *40 LL	_	_	40 MX 41 MN	40 MY	41 MN	40 MY	41 MN	40 MY 4	MN	SOILS			LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%				
PI	6 MX		10 MX 10 MX			10 MX	10 MX	11 MN 1		LITTL MODE		HIGHLY ORGANIC	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE				
GROUP INDEX	0	0	0	4	MX	8 MX	12 MX	16 MX N	MX	amoun Orga		SOILS	GROUND WATER				
USUAL TYPES OF MAJOR	GRAVEL, AND	FINE SAND	SILTY O GRAVEL			SIL		CLAYE		MAT			✓ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING ✓ STATIC WATER LEVEL AFTER 24 HOURS ✓ MATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING ✓ MATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING ✓ MATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING ✓ MATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING ✓ MATER LEVEL AFTER 24 HOURS ✓ MATER LEVEL AFTER 25 HOURS ✓ MATER LEVEL AFTER				
MATERIALS GEN. RATING	SAND	0	5						_	FAIR TO			✓ PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA				
AS SUBGRADE	E	XCELL	ENT TO GOOD				FAIR TO	POOR		POOR	POOR	UNSUITABLE	SPRING OR SEEP				
	Р	I OF A	-7-5 SUBGROU							LL - 30			MISCELLANEOUS SYMBOLS				
			OMPACTNES		1	RANO	GE OF	STANDA	:D	RANG	E OF UNC	ONFINED	TT 25,405				
PRIMARY	SOIL TYPE	`	CONSISTE			PENETF	N-VA N-VA	RESIST LUE)	NCE	COMP	RESSIVE S (TONS/FT		WITH SOIL DESCRIPTION OF ROCK STRUCTURES				
GENERA			VERY LOC				4 T(SOIL SYMBOL SPT ONT TEST BORING SLOPE INDICATOR INSTALLATION				
GRANUL MATERI			MEDIUM DE	ENSE			10 T	30			N/A		ARTIFICIAL FILL (AF) OTHER AUCER PORING CONE PENETROMETER				
(NON-CO	HESIVE)		VERY DEN				3Ø T						THAN ROADWAY EMBANKMENT THOUGH BUNING TEST				
GENERA	LLY		VERY SO SOFT				(2 T				< 0.25 0.25 TO		— INFERRED SOIL BOUNDARY — CORE BORING ● SOUNDING ROD				
SILT-CI MATERI	_AY		MEDIUM S	TIFF			4 T	8 0			0.5 TO 1	.0	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE				
(COHES)			VERY ST				15 T	30			2 TO 4		↑ PIEZOMETER INSTALLATION SPT N-VALUE				
			HARD TEX	XTUR	E O	R GF	> RAIN	SIZE			> 4		RECOMMENDATION SYMBOLS				
U.S. STD. SI	EVE SIZE		4		10	40		60	200	270			UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE				
OPENING (M	M)		4.7	6 2	2.00	0.42			.075	0.053			SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF				
BOULDE (BLDR.		BBLE	GRAV (GR			COARS)		INE SAND		SILT SL.)	CLAY (CL.)	UNDERCUT ACCEPTABLE DEGRADABLE ROCK				
GRAIN M	1 305		75		2.0	(CSE. S).25	SD.)	0.05	0.005	<u> </u>	ABBREVIATIONS AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST				
SIZE IN			3				·			0.00	0.000		BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT				
			MOIST				LĄT	ION)F	TERMS			CPT - CONE PENETRATION TEST NP - NON PLASTIC $\dot{\gamma}_{ m d}$ - DRY UNIT WEIGHT				
	MOISTURE S TERBERG LIM		·		O MOIS			GUIDE F	OR F	IELD MOIS	STURE DES	CRIPTION	CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS				
					TURATI	ED -					WET, USU		DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON				
	LIQUID	LIMIT	. <u> </u>	(9	SAT.)			FROM B	ELOW	THE GRO	UND WATE	R TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK				
PLASTIC RANGE <				- WE	T - (W	SEMISOLID; REQUIRES DRYING TO							FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS " - MOISTURE CONTENT CBR - CALIFORNIA BEARING				
(PI) PL	PLASTIC	LIM	ΙТ					ATTAIN	OPTIN	MUM MOIS	TURE		HI HIGHLY V - VERY RATIO				
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE					T OR	NEAR OP	ISTURE	EQUIPMENT USED ON SUBJECT PROJECT									
SL T SHRINK													DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: CME-45C CLAY BITS AUTOMATIC MANUAL				
			- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE)	6* CONTINUOUS FLIGHT AUGER CODE CLZE				
PLASTICITY											CME-55 8* HOLLOW AUGERS CORE 512E:						
	PLASTICITY INDEX (PI) DRY STRENGTH							CME-550 HARD FACED FINGER BITS									
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT					VANE SHEAR TEST TUNGCARBIDE INSERTS HAND TOOLS:												
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH							MEDIUM	CASING W/ ADVANCER POST HOLE DIGGER									
		-				DLOR							PORTABLE HOIST TRICONE STIEL TEETH HAND AUGER TRICONE TUNG, CARB. SUINDING PRO				
DECCBIO	TIONS MAY I	NC: !"	חב רמי מפ	OB CO				C (TAN	DED '	/ELLOW_05	ייים אארס	=-CBAV\	CORE BIT SOUNDING ROD VANE SHEAR TEST				
	DDIFIERS SU												Think Shemy lest				

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS (PAGE 2 OF 2)

ROCK DESCRIPTION HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN I.FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.

ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: AQUIFER - A WATER BEARING FORMATION OR STRATA 115115 NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES 3 100 BLOWS PER FOOT IF TESTED. FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT FINE TO COARSE GRAIN IONEQUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.

FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.

COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC. CRYSTALLINE ROCK (CR) SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. NON-CRYSTALLINE ROCK (NCR) COASTAL PLAIN SEDIMENTARY ROCK WEATHERING ROCKS OR CUTS MASSIVE ROCK. **ERESH** ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HORIZONTAL. VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS II OF A CRYSTALLINE NATURE. (V SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO SLIGHT SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. 1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN MODERATE GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM, WITH FRESH ROCK. ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH MODERATELY SEVERE (MOD, SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT SEVERE REDUCED IN STRENOTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. (SEV.) IF TESTED. WOULD YIELD SPT N VALUES > 100 BPF ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVINIS STRATIM VERY SEVERE AN INTERVENING IMPERVIOUS STRATUM. (V SEV.) VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS RUN AND EXPRESSED AS A PERCENTAGE. ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED HARD TO DETACH HAND SPECIMEN. SLICKENSIDE - I MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. MEDILIM CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE HARD TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. POINT OF A GEOLOGIST'S PICK. CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. SOFT VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH

FRACTUR	E SPACING	BEDD1	ING
ERM	SPACING	TERM	THICKNESS
WIDE	MORE THAN 10 FEET	VERY THICKLY BEDDED	4 FEET
	3 TO 10 FEET	THICKLY BEDDED	1.5 - 4 FEET
RATELY CLOSE	1 TO 3 FEET	THINLY BEDDED	0.16 - 1.5 FEET
Ε	0.16 TO 1 FOOT	VERY THINLY BEDDED	0.03 - 0.16 FEET
CLOSE	LESS THAN 0.16 FEET	THICKLY LAMINATED THINLY LAMINATED	0.008 - 0.03 FEET < 0.008 FEET

OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY

SOFT

TE VERY WIDE MODE

CLOS VERY

FINGERNAIL.

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. RUBBING WITH FINGER FREES NUMEROUS GRAINS. GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. MODERATELY INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; INDURATED DIFFICULT TO BREAK WITH HAMMER. SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE: EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE

<u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

- A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

 $\underline{\mathsf{FLOAT}}$ - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

 $\underline{\mathsf{LEOGE}}$ - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE

<u>SAPROLITE (SAP.)</u> - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

<u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

- POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL. TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH	MARK:		
		ELEVATION:	FEET

NOTES:

DATE: 8-15-14



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

PAT MCCRORY
GOVERNOR
SECRETARY

March 24, 2015

STATE PROJECT: 38600.1.1 (B-4830) FEDEARL PROJECT: BRSTP-0097(34)

COUNTY: Wake

DESCRIPTION: Bridge No. 20 on -L- (NC 97) over Moccasin Creek

SUBJECT: Geotechnical Report – Inventory

The Geotechnical Engineering Unit has completed a subsurface investigation for this project and presents the following inventory. No plans, profiles, or cross-sections will be submitted for this roadway project.

Project Description

The project consists of the replacement of Bridge No. 20 on NC 97 at the Wake/Franklin county line over Moccasin Creek. The total length of the roadway portion of the project is 0.09 miles. The proposed grade will be raised just slightly. A site visit and literature review of surrounding projects was conducted during March of 2015.

Physiography & Geology

The project is located 2 miles east of the town of Zebulon in the rolling terrain of eastern Wake County. Geologically the site is characterized by sands, silts, and clays associated with the granitic rock of the Raleigh Belt.

Soil Properties

Soils encountered at the site are roadway embankment, alluvial, and residual soils. The soils consist of granular and cohesive materials.

Roadway embankment soils consist of orange-brown, medium stiff to stiff, moist, sandy silt and clay (A-4, A-6) and some loose, silty sand (A-2-4). This material varies in depth from 6.0 to 18.0 feet. Alluvial soils deposited by Moccasin Creek consist primarily of tan, brown, and gray, soft to stiff, moist to saturated, sandy and clayey silt (A-4 and A-5) with some loose to medium dense, silty sand. Residual soils consist of black, white, and brown, medium to very dense, moist, silty sand (A-2-4). Residual soils are derived from weathering of the underlying weathered and crystalline rock.

Rock Properties

Weathered and crystalline rock is approximately 25.0 to 35.0 feet below the ground surface and consists of gray, white, and, pink, severely weathered to fresh, hard to very hard, close to wide fracture spacing, metamorphosed granite. Crystalline rock is not anticipated to cause problems during construction.

Groundwater

The groundwater level is anticipated to be at elevations similar to Moccasin Creek. Seasonal fluctuations in the water table can be expected. Groundwater is not anticipated to cause problems during construction.

Respectfully submitted,

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3/24/2015

Jaime Love Pedro, LG Project Geological Engineer

JLP/NTR/jlp

TIP B-4830 COUNTY WAKE GEOLOGIST Swartley, J. R. WBS 38600.1.1 SITE DESCRIPTION BRIDGE NO. 20 ON -L- (NC 97) OVER MOCCASIN CREEK **GROUND WTR (ft) STATION** 14+00 OFFSET 35 ft LT ALIGNMENT -L-BORING NO. 1400LT 0 HR. N/A COLLAR ELEV. 215.3 ft TOTAL DEPTH 6.0 ft **NORTHING** 759,569 **EASTING** 2,218,933 24 HR. N/A DRILL RIG/HAMMER EFF./DATE N/A **DRILL METHOD** Hand Auger HAMMER TYPE N/A DRILLER Pinter, D. G. **START DATE** 03/23/15 **COMP. DATE** 03/23/15 SURFACE WATER DEPTH 0.5ft DRIVE **BLOW COUNT BLOWS PER FOOT** SAMP ELEV DEPTH **ELEV** 0 SOIL AND ROCK DESCRIPTION (ft) (ft) 100 0.5ft | 0.5ft | 0.5ft 25 50 75 NO. <u>M</u>OI (ft) G ELEV. (ft) DEPTH (ft) 220 WATER SURFACE (03/23/15) -215.3··· 215 M ALLUVIAL ORANGE AND TAN, SANDY SILT W 211.3 GRAY, SILTY SAND 210 Sat. 209.3 Boring Terminated at Elevation 209.3 ft ALLUVIAL (SILTY SAND) NCDOT BORE SINGLE B4830_GEO_BH.GPJ NC_DOT.GDT 3/24/15

TIP B-4830 COUNTY WAKE GEOLOGIST Swartley, J. R. WBS 38600.1.1 SITE DESCRIPTION BRIDGE NO. 20 ON -L- (NC 97) OVER MOCCASIN CREEK **GROUND WTR (ft) STATION** 14+00 OFFSET 35 ft RT ALIGNMENT -L-BORING NO. 1400RT 0 HR. 0.6 COLLAR ELEV. 215.1 ft TOTAL DEPTH 6.0 ft **NORTHING** 759,500 **EASTING** 2,218,946 24 HR. **FIAD** DRILL RIG/HAMMER EFF./DATE N/A **DRILL METHOD** Hand Auger HAMMER TYPE N/A DRILLER Pinter, D. G. **START DATE** 03/23/15 **COMP. DATE** 03/23/15 SURFACE WATER DEPTH N/A DRIVE **BLOW COUNT BLOWS PER FOOT** SAMP DEPTH **ELEV ELEV** 0 SOIL AND ROCK DESCRIPTION (ft) (ft) 100 0.5ft | 0.5ft | 0.5ft 25 50 NO. 75 (ft) MOI G ELEV. (ft) DEPTH (ft) 220 GROUND SURFACE 215.1 215 ALLUVIAL BROWN AND TAN, SANDY SILT W 211.1 GRAY, SILTY SAND 210 Sat. 209.1 Boring Terminated at Elevation 209.1 ft ALLUVIAL (SILTY SAND)

COUNTY WAKE **TIP** B-4830 WBS 38600.1.1 GEOLOGIST Swartley, J. R. SITE DESCRIPTION BRIDGE NO. 20 ON -L- (NC 97) OVER MOCCASIN CREEK **GROUND WTR (ft) STATION** 16+00 OFFSET 15 ft LT **BORING NO.** 1600_15LT ALIGNMENT 0 HR. Dry COLLAR ELEV. 225.1 ft TOTAL DEPTH 2.5 ft **NORTHING** 759,586 **EASTING** 2,219,133 24 HR. **FIAD** DRILL RIG/HAMMER EFF./DATE N/A DRILL METHOD Hand Auger HAMMER TYPE N/A DRILLER Pinter, D. G. **START DATE** 03/23/15 **COMP. DATE** 03/23/15 SURFACE WATER DEPTH N/A DRIVE **BLOW COUNT BLOWS PER FOOT** SAMP. ELEV DEPTH **ELEV** 0 SOIL AND ROCK DESCRIPTION (ft) (ft) 100 0.5ft | 0.5ft | 0.5ft 25 50 75 NO. (ft) MOI G ELEV. (ft) DEPTH (ft) 230 GROUND SURFACE 225 ROADWAY EMBANKMENT M ORANGE AND BROWN, SANDY CLAY WITH GRAVEL Boring Terminated by Auger Refusal at Elevation 222.6 ft ROADWAY EMBANKMENT (GRAVEL LAYER)

TIP B-4830 COUNTY WAKE GEOLOGIST Swartley, J. R. WBS 38600.1.1 SITE DESCRIPTION BRIDGE NO. 20 ON -L- (NC 97) OVER MOCCASIN CREEK GROUND WTR (ft) **STATION** 16+00 OFFSET 45 ft LT ALIGNMENT -L-BORING NO. 1600LT 0 HR. 0.5 COLLAR ELEV. 214.8 ft TOTAL DEPTH 6.0 ft **NORTHING** 759,615 **EASTING** 2,219,128 24 HR. **FIAD** DRILL RIG/HAMMER EFF./DATE N/A **DRILL METHOD** Hand Auger HAMMER TYPE N/A DRILLER Pinter, D. G. **START DATE** 03/23/15 **COMP. DATE** 03/23/15 SURFACE WATER DEPTH N/A DRIVE **BLOW COUNT BLOWS PER FOOT** SAMP DEPTH **ELEV** ELEV 0 SOIL AND ROCK DESCRIPTION (ft) (ft) 100 0.5ft | 0.5ft | 0.5ft 25 50 75 NO. (ft) G ELEV. (ft) DEPTH (ft) 215 **GROUND SURFACE** ALLUVIAL ORANGE AND TAN, CLAYEY SILT W 210.8 210 GRAY, SILTY SAND Sat. 208.8 Boring Terminated at Elevation 208.8 ft ALLUVIAL (SILTY SAND)

TIP B-4830 COUNTY WAKE WBS 38600.1.1 GEOLOGIST Swartley, J. R. SITE DESCRIPTION BRIDGE NO. 20 ON -L- (NC 97) OVER MOCCASIN CREEK **GROUND WTR (ft) STATION** 16+00 OFFSET 35 ft RT ALIGNMENT BORING NO. 1600RT 0 HR. 2.0 COLLAR ELEV. 215.4 ft TOTAL DEPTH 6.0 ft **NORTHING** 759,537 **EASTING** 2,219,142 24 HR. **FIAD** DRILL RIG/HAMMER EFF./DATE N/A **DRILL METHOD** Hand Auger HAMMER TYPE N/A DRILLER Pinter, D. G. **START DATE** 03/23/15 **COMP. DATE** 03/23/15 SURFACE WATER DEPTH N/A DRIVE **BLOW COUNT BLOWS PER FOOT** SAMP. ELEV DEPTH **ELEV** 0 SOIL AND ROCK DESCRIPTION (ft) (ft) 100 0.5ft | 0.5ft | 0.5ft 25 50 75 NO. (ft) MOI G ELEV. (ft) DEPTH (ft) 220 215.4 GROUND SURFACE 215 ALLUVIAL BROWN AND TAN, CLAYEY SILT W 211.4 GRAY, SILTY SAND 210 Sat. 209.4 Boring Terminated at Elevation 209.4 ft ALLUVIAL (SILTY SAND)