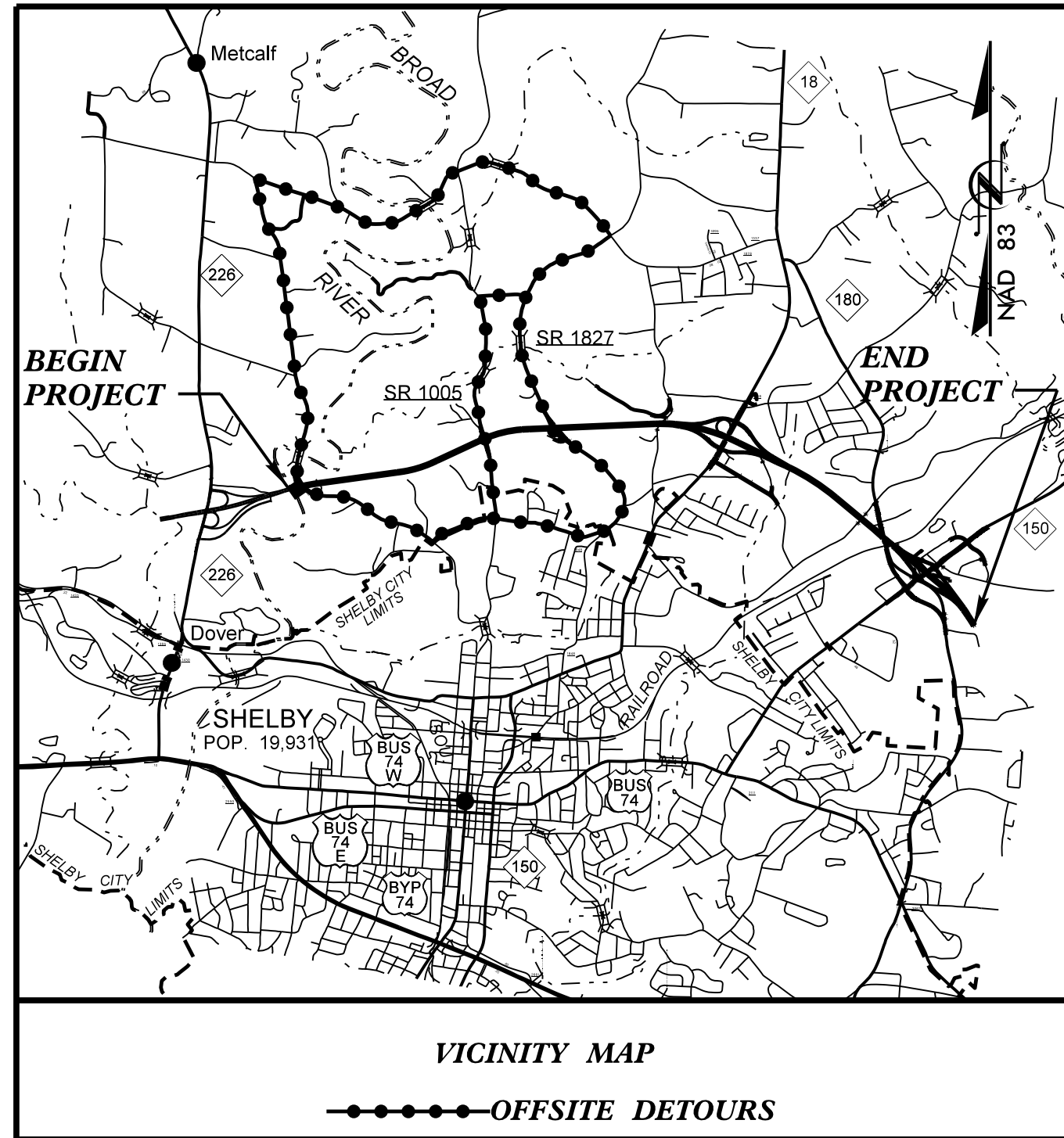


TIP PROJECT: R-2707C

CONTRACT: C203905

STRUCTURES

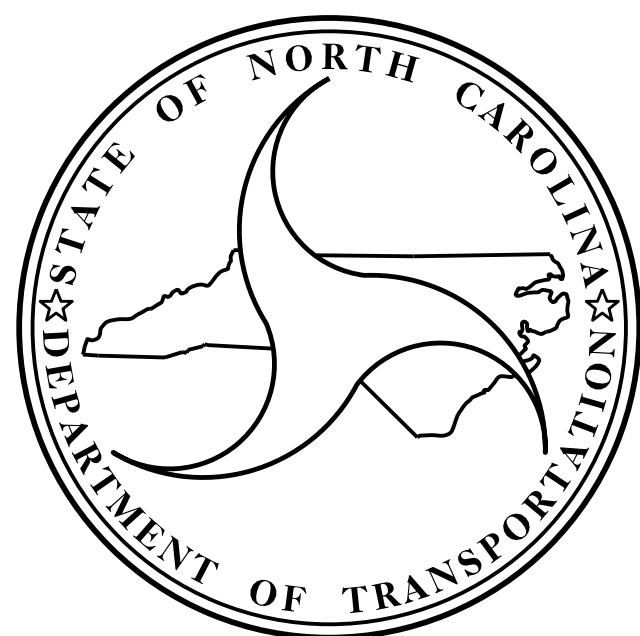
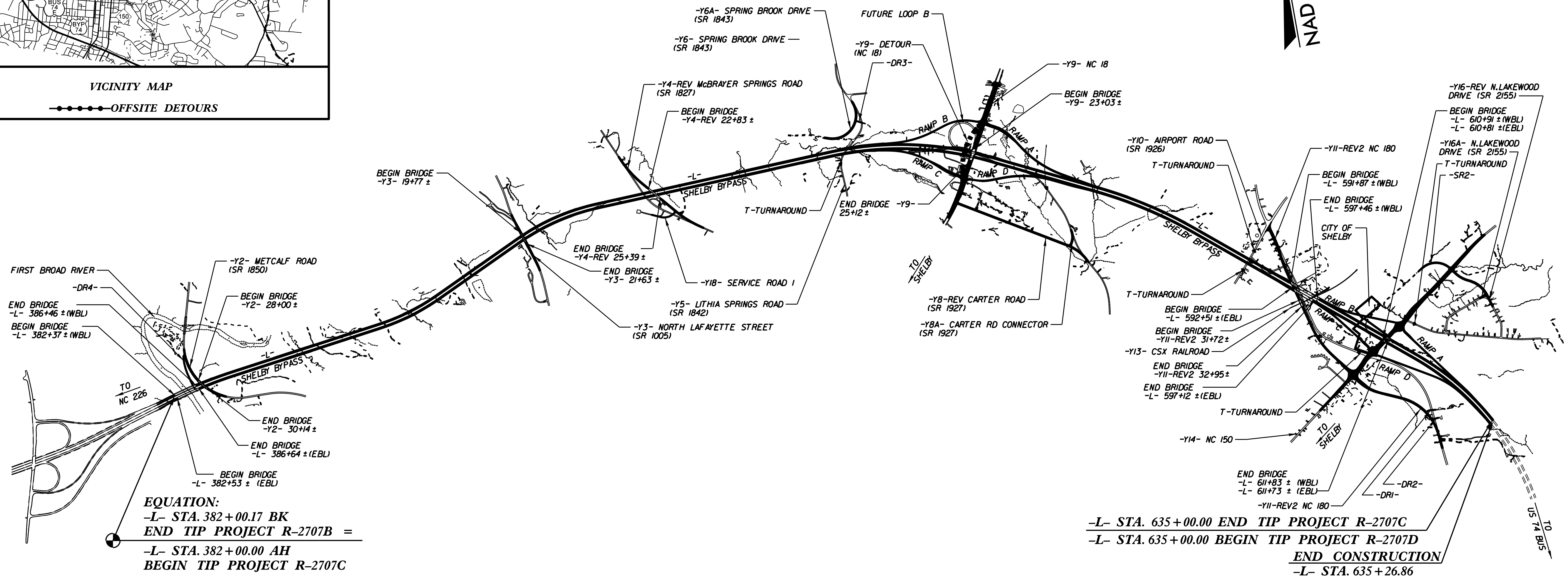


STATE OF NORTH CAROLINA
 DIVISION OF HIGHWAYS
CLEVELAND COUNTY

LOCATION: US 74 SHELBY BYPASS FROM EAST OF NC 226 TO EAST OF NC 150

TYPE OF WORK: GRADING, PAVING, DRAINAGE, SIGNING, RETAINING WALLS, CULVERTS, AND STRUCTURES

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-2707C	1	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
34497.1.4		P.E.	
34497.2.FS9	NHF-0074(152)	ROW	
34497.2.FSU4	NHF-0074(152)	UTILITY	
34497.3.6	NHF-0074(152)	CONST	



DESIGN DATA

ADT 2017	=	21,010
ADT 2037	=	35,110
K	=	11 %
D	=	55 %
T	=	14 % *
V	=	70 MPH
FUNC CLASS	=	FREWAY
(* TTST 9% DUAL 5%)		
STATEWIDE TIER		

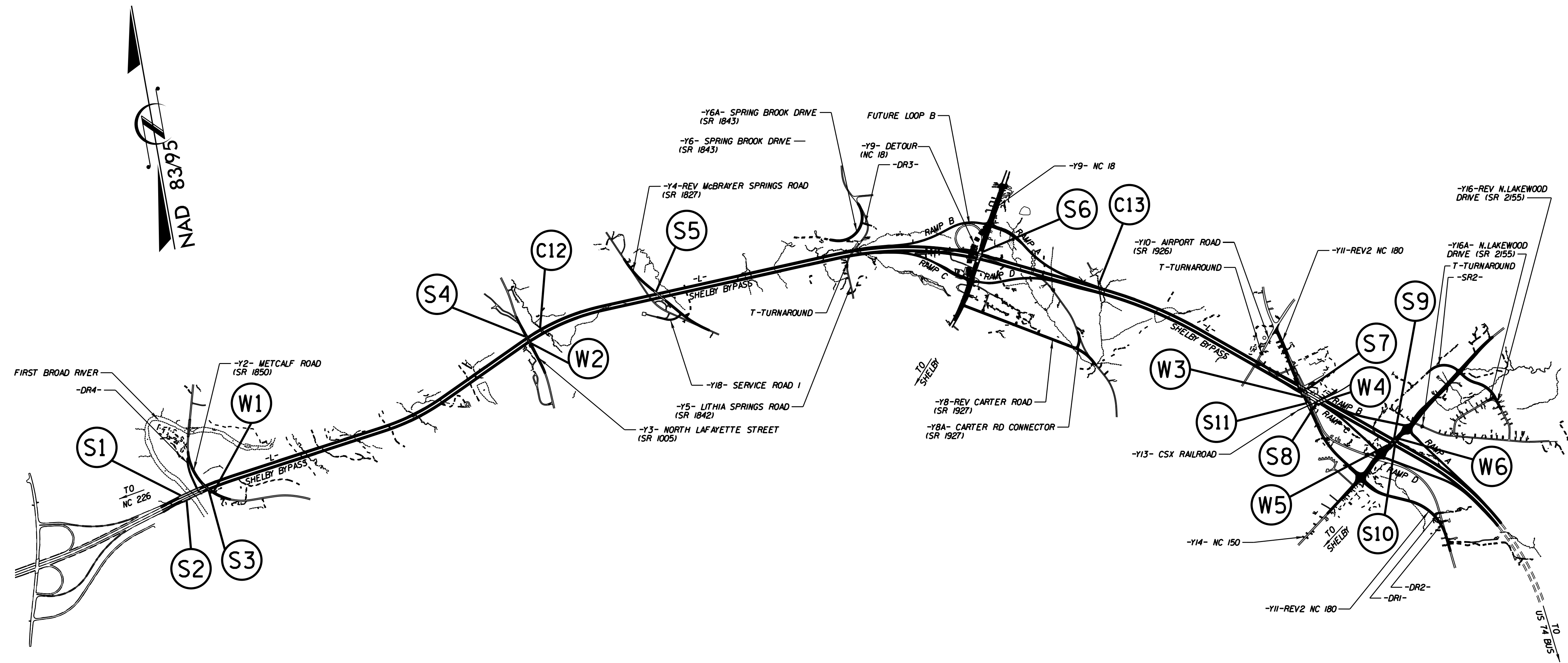
PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT R-2707C	=	4.591 MILES
LENGTH STRUCTURES TIP PROJECT R-2707C	=	0.201 MILES
LENGTH TIP PROJECT R-2707C	=	4.792 MILES
NOTE : PROJECT LENGTH BASED ON WBL		

Prepared in the Office of:
DIVISION OF HIGHWAYS
 STRUCTURES MANAGEMENT UNIT
 1000 BIRCH RIDGE DR.
 RALEIGH, N.C. 27610

2012 STANDARD SPECIFICATIONS

LETTING DATE :
 May 16, 2017



INDEX			
STR	STATION	DESCRIPTION	SHEETS
S1	384+50.00 -L- (LEFT LANE)	BRIDGE ON US 74/SHELBY BYPASS OVER FIRST BROAD RIVER (LEFT LANE)	S1-1 THRU S1-36
S2	384+50.00 -L- (RIGHT LANE)	BRIDGE ON US 74/SHELBY BYPASS OVER FIRST BROAD RIVER (RIGHT LANE)	S2-1 THRU S2-36
S3	29+07.16 -Y2-	BRIDGE ON SR 1850 OVER US 74/SHELBY BYPASS	S3-1 THRU S3-36
S4	20+70.23 -Y3-	BRIDGE ON SR 1005 OVER US 74/SHELBY BYPASS	S4-1 THRU S4-29
S5	24+05.50 -Y4-	BRIDGE ON SR 1827 OVER US 74/SHELBY BYPASS	S5-1 THRU S5-29
S6	24+07.99 -Y9-	BRIDGE ON NC 18 OVER US 74/SHELBY BYPASS	S6-1 THRU S6-37
S7	596+50.98 -L- (LEFT LANE)	BRIDGE ON US 74/SHELBY BYPASS OVER CSX RR & NORTH POST RD.(LEFT LANE)	S7-1 THRU S7-56

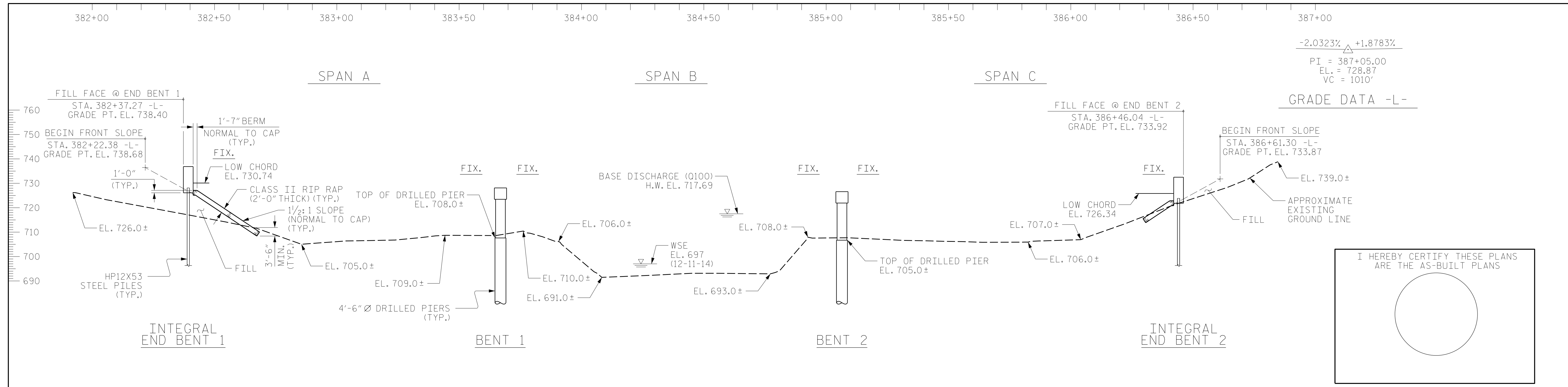
INDEX			
STR	STATION	DESCRIPTION	SHEETS
S8	596+50.98 -L- (RIGHT LANE)	BRIDGE ON US 74/SHELBY BYPASS OVER CSX RR & NORTH POST RD.(RIGHT LANE)	S8-1 THRU S8-44
S9	611+32.01 -L- (LEFT LANE)	BRIDGE ON US 74/SHELBY BYPASS OVER NC 150 (LEFT LANE)	S9-1 THRU S9-25
S10	611+32.01 -L- (RIGHT LANE)	BRIDGE ON US 74/SHELBY BYPASS OVER NC 150 (RIGHT LANE)	S10-1 THRU S10-25
S11	32+31.41 -Y11REV2-	BRIDGE ON NC 180 OVER CSX RR	S11-1 THRU S11-27
C12	453+07.00 -L-	SINGLE 8 FT. X 8 FT. RCBC	C12-01 THRU C12-05
C13	553+27.00 -L-	SINGLE 9 FT. X 7 FT. RCBC	C13-01 THRU C13-05
W1-W6	-	RETAINING WALLS	W-1 THRU W-9

PROJECT NO. R-2707C
CLEVELAND COUNTY

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

INDEX SHEET

REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

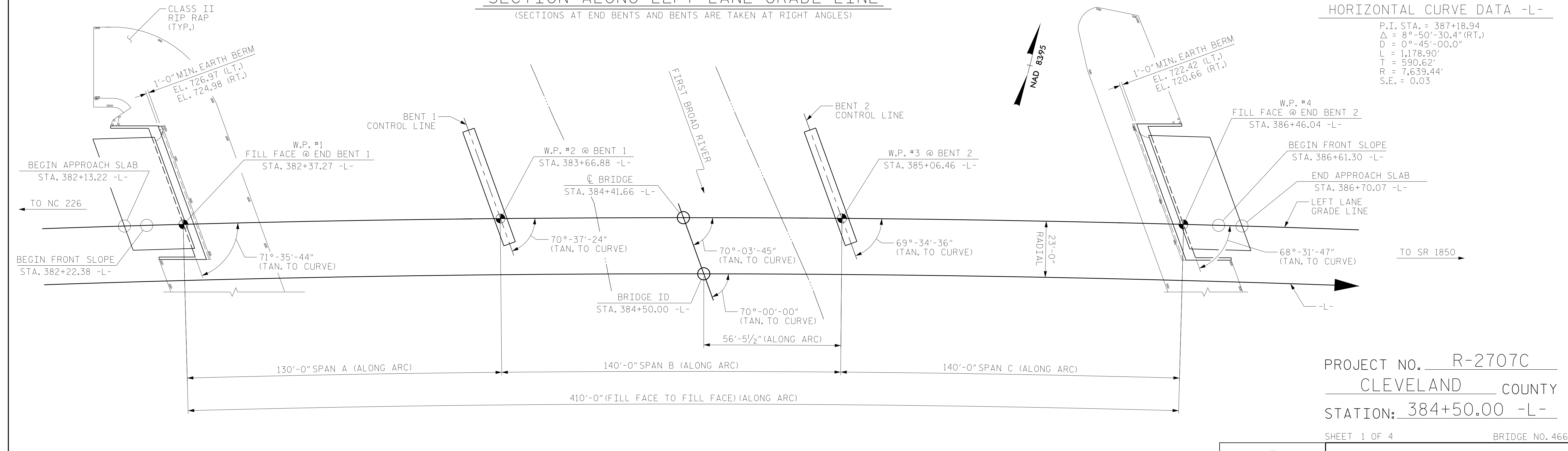


GRADE DATA -L-
 -2.0323% Δ $+1.8783\%$
 PI = 387+05.00
 EL. = 728.87
 VC = 1010'

I HEREBY CERTIFY THESE PLANS ARE THE AS-BUILT PLANS

SECTION ALONG LEFT LANE GRADE LINE
 (SECTIONS AT END BENTS AND BENTS ARE TAKEN AT RIGHT ANGLES)

HORIZONTAL CURVE DATA -L-
 P.I. STA. = 387+18.94
 Δ = 8°-50'-30.4" (RT.)
 D = 0°-45'-00.0"
 L = 1,178.90'
 T = 590.62'
 R = 7,639.44'
 S.E. = 0.03



PLAN ALONG LEFT LANE GRADE LINE
 (FOR CLARITY, PILES & COLUMNS ARE NOT SHOWN IN PLAN VIEW)

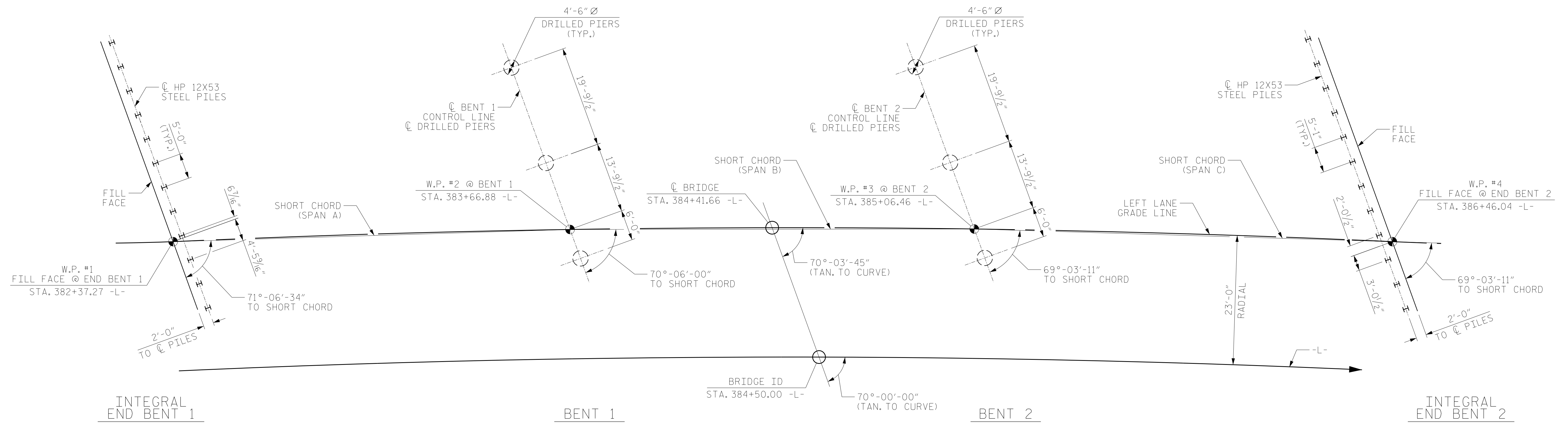
PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 1 OF 4 BRIDGE NO. 466

DRAWN BY : MAL DATE : 11/2016
 CHECKED BY : JMR DATE : 12/2016
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
GENERAL DRAWING LEFT LANE BRIDGE OVER FIRST BROAD RIVER ON US 74 SHELBY BYPASS BETWEEN NC 226 AND SR 1850					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S1-1
TOTAL SHEETS					36



FOUNDATION LAYOUT PLAN

DIMENSIONS LOCATING PILES AND DRILLED PIERS ARE SHOWN TO PILE AND DRILLED PIER CENTERLINE.

NOTES

- FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF STANDARD SPECIFICATIONS.
- PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.
- DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.
- FOR DRILLED PIERS, SEE SECTION 411 OF STANDARD SPECIFICATIONS.
- INSTALL DRILLED PIERS AT BENT NO.1 AND BENT NO.2 TO A TIP ELEVATION NO HIGHER THAN 670.6 FEET AND A PENETRATION OF 9 FEET INTO ROCK AS DEFINED BY ARTICLE 411-1 OF THE STANDARD SPECIFICATIONS.
- DRILLED PIERS AT BENT NO.1 AND BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 835 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 30 TSF.
- THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 AND BENT NO.2 IS 679 FEET. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.
- PERMANENT STEEL CASINGS ARE REQUIRED FOR DRILLED PIERS AT BENT NO.1 AND BENT NO.2. DO NOT EXTEND PERMANENT CASINGS BELOW ELEVATION 690 FT WITHOUT PRIOR APPROVAL FROM THE ENGINEER.
- CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL NEED TO DETERMINE THE NEED FOR CSL TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.
- DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.
- DRILLED-IN PILES ARE REQUIRED FOR INTEGRAL END BENT NO.2. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 713 FT. FILL THE BOTTOM 3 FT OF HOLES FOR PILE EXCAVATION WITH CONCRETE AND THE REST OF THE HOLES WITH CLASS II OR CLASS III SELECT MATERIAL THAT MEETS SECTION 1016 OF THE STANDARD SPECIFICATIONS FOR PILE EXCAVATION, SEE SECTION 450 OF THE SPECIFICATIONS.
- STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT NO.1 AND END BENT NO.2. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- DRILLED-IN PILES AT END BENT NO.2 THAT DO NOT HAVE A MINIMUM PENETRATION OF 3 FEET INTO ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS WILL BE DRIVEN TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

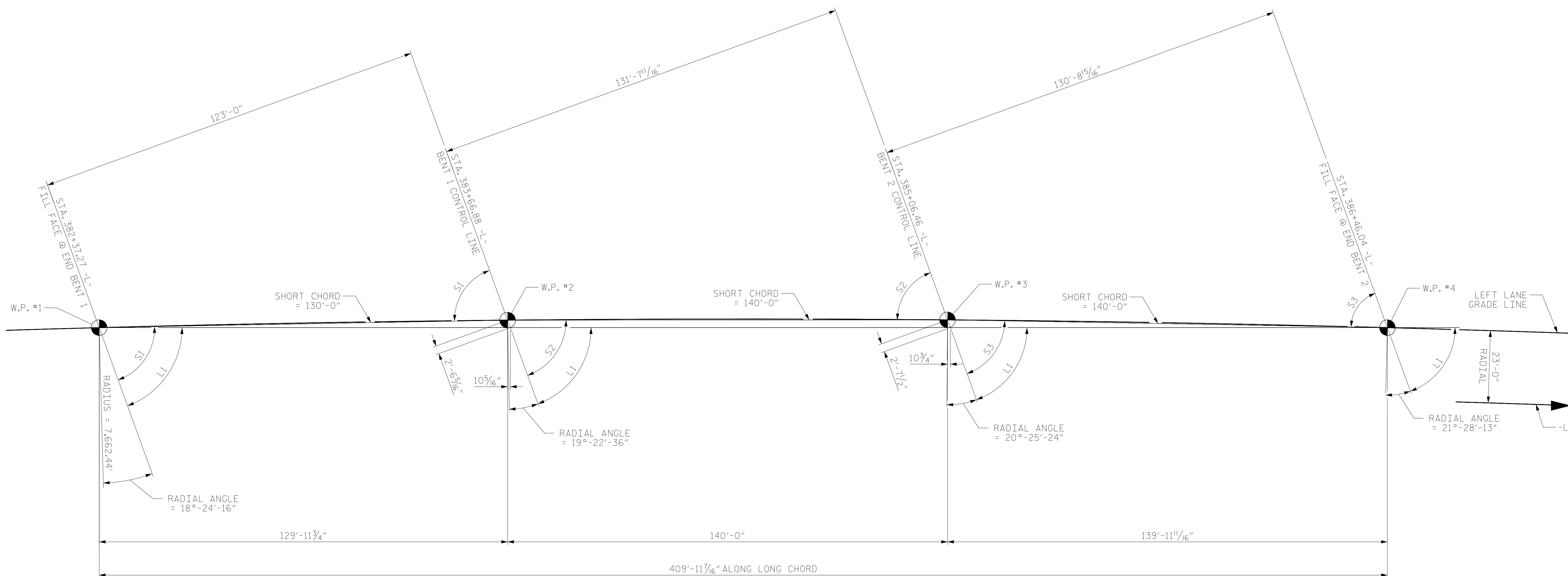
SHEET 2 OF 4

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
GENERAL DRAWING LEFT LANE BRIDGE OVER FIRST BROAD RIVER ON US 74 SHELBY BYPASS BETWEEN NC 226 AND SR 1850					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S1-2
TOTAL SHEETS					36

DRAWN BY :	MAL	DATE :	11/2016
CHECKED BY :	JMR	DATE :	12/2016
DESIGN ENGINEER OF RECORD:	MAL	DATE :	11/2016

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 SIGNATURES COMPLETED



LONG CHORD LAYOUT

NOTE: ALL BENTS ARE PARALLEL

ANGLES	
LONG CHORD	SHORT CHORD
L1 = 70°-03'-45"	S1 = 71°-06'-34"
	S2 = 70°-06'-00"
	S3 = 69°-03'-11"

HORIZONTAL CURVE DATA -L-

P.I. STA. = 387+18.94
 Δ = 8°-50'-30.4" (RT.)
 D = 0°-45'-00.0"
 L = 1,178.90'
 T = 590.62'
 R = 7,639.44'
 S.E. = 0.03

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 3 OF 4



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STATE OF NORTH CAROLINA
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 RALEIGH

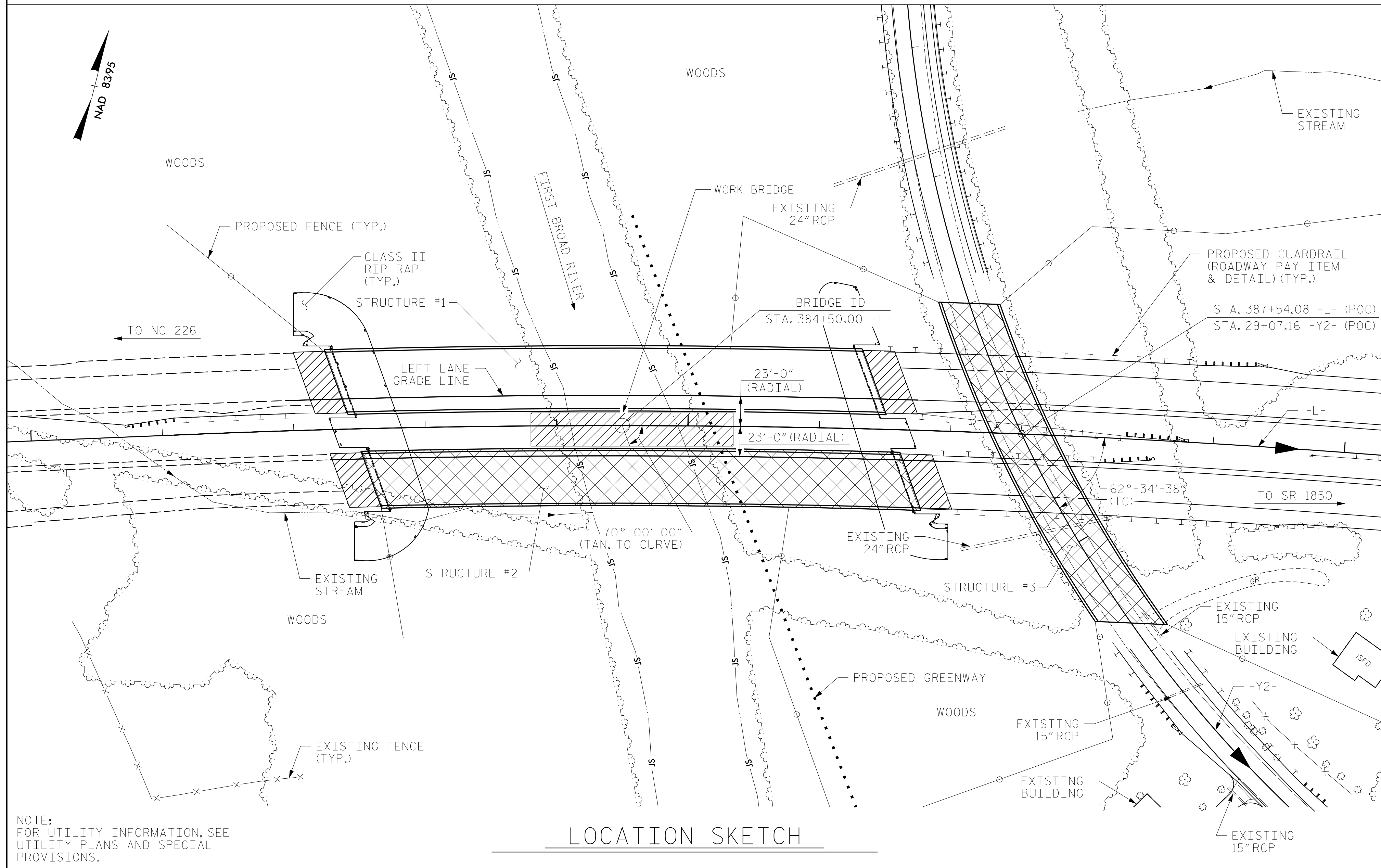
GENERAL DRAWING
 LEFT LANE BRIDGE OVER FIRST
 BROAD RIVER ON US 74
 SHELBY BYPASS BETWEEN
 NC 226 AND SR 1850

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-3
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : MAL DATE : 11/2016
 CHECKED BY : JMR DATE : 12/2016
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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 SIGNATURES COMPLETED

BENCH MARK #17: 8 INCH NAIL IN BASE OF 20 INCH SYCAMORE 93.83' LEFT OF STA. 385+52.11 -L-, EL. 707.47'



NOTE:
FOR UTILITY INFORMATION, SEE
UTILITY PLANS AND SPECIAL
PROVISIONS.

LOCATION SKETCH

NOTES:

- ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.
- THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- FOR PLACING LOAD ON STRUCTURES, SEE SPECIAL PROVISIONS.
- REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
- THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.
- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES."
- THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 AND NO.2 IS ELEVATION 679. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.
- FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
- THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE THE TEMPORARY ACCESS AT STATION 384+50.00 -L- FOR USE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE.
- FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS, SEE SPECIAL PROVISIONS.
- FOR 74" MODIFIED PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.

HYDRAULIC DATA

DESIGN DISCHARGE	= 20,200 CFS
FREQUENCY OF DESIGN FLOOD	= 50 YRS
DESIGN HIGH WATER ELEVATION	= 716.2
DRAINAGE AREA	= 226 SQ. MI.
BASE DISCHARGE (Q100)	= 23,400 CFS
BASE HIGH WATER ELEVATION	= 717.69

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE	= >31,700 CFS
FREQUENCY OF OVERTOPPING FLOOD	= >500 YRS+
OVERTOPPING FLOOD ELEVATION	= 733.80

TOTAL BILL OF MATERIALS

	CONSTRUCTION, MAINTENANCE & REMOVAL OF TEMP. ACCESS AT STA. 384+50.00 -L-	4'-6" Ø DRILLED PIER IN SOIL	4'-6" Ø DRILLED PIER NOT IN SOIL	PERMANENT STEEL CASING FOR 4'-6" DIA. DRILLED PIER	CSL TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	74" MODIFIED PRESTRESSED CONCRETE GIRDERS	PILE EXCAVATION IN SOIL	PILE EXCAVATION NOT IN SOIL
	LUMP SUM	LIN. FT.	LIN. FT.	LIN. FT.	EACH	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	APPROX. LBS.	NO. LIN. FT.	LIN. FT.	LIN. FT.
SUPERSTRUCTURE						20,101	19,672		LUMP SUM			15 2027.3		
END BENT No. 1								52.2		9,348				
BENT No. 1		85.1	27.0	54.0	1			69.8		21,459	4,702			
BENT No. 2		76.1	27.0	54.0	1			71.9		21,045	4,587			
END BENT No. 2								52.2		9,413			84	36
TOTAL	LUMP SUM	161.2	54.0	108.0	2	20,101	19,672	246.1	LUMP SUM	61,265	9,289	15 2027.3	84	36

	HP 12x53 STEEL PILES	STEEL PILE POINTS	PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES	CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS
	NO. LIN. FT.	EACH	EACH	LIN. FT.	TONS	SQ. YDS.	LUMP SUM
SUPERSTRUCTURE				816.3			LUMP SUM
END BENT No. 1	12 480	12	12		400	490	
BENT No. 1							
BENT No. 2							
END BENT No. 2	12 240	12	12		265	325	
TOTAL	24 720	24	24	816.3	665	815	LUMP SUM

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

SHEET 4 OF 4

DRAWN BY : MAL DATE : 11/2016
CHECKED BY : JMR DATE : 01/2017
DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING
LEFT LANE BRIDGE OVER FIRST
BROAD RIVER ON US 74
SHELBY BYPASS BETWEEN
NC 226 AND SR 1850

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-4
1			3			TOTAL SHEETS
2			4			36

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ_{DC}	γ_{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING Ⓝ	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					COMMENT NUMBER			
						MOMENT					SHEAR					MOMENT								
						LIVE-LOAD FACTORS (γ_{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FF)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FF)	LIVE-LOAD FACTORS (γ_{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FF)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	Ⓛ	1.34	--	1.75	0.89	1.53	B	EL	68.9	1.050	1.34	B	I	124.5	0.80	0.79	1.39	B	I	68.9		
	HL-93 (OPERATING)	N/A		1.77	--	1.35	0.89	1.98	B	EL	68.9	1.050	1.77	B	I	124.5	N/A	--	--	--	--	--		
	HS-20 (INVENTORY)	36.000	Ⓜ	1.99	71.640	1.75	0.89	2.32	B	EL	68.9	1.050	1.99	B	I	124.5	0.80	0.79	2.11	B	I	68.9		
	HS-20 (OPERATING)	36.000		2.62	94.320	1.35	0.89	3.01	B	EL	68.9	1.050	2.62	B	I	124.5	N/A	--	--	--	--	--		
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13,500		5.18	69.930	1.40	0.89	7.14	B	EL	68.9	1.050	6.56	B	I	124.5	0.80	0.79	5.18	B	I	68.9	
		SNGARBS2	20,000		3.67	73.400	1.40	0.89	5.06	B	EL	68.9	1.050	4.51	B	I	124.5	0.80	0.79	3.67	B	I	68.9	
		SNAGRIS2	22,000		3.40	74.800	1.40	0.89	4.69	B	EL	68.9	1.050	4.13	B	I	124.5	0.80	0.79	3.40	B	I	68.9	
		SNCOTTS3	27,250		2.57	70,033	1.40	0.89	3.55	B	EL	68.9	1.050	3.20	B	I	124.5	0.80	0.79	2.57	B	I	68.9	
		SNAGGRS4	34,925		2.08	72,644	1.40	0.89	2.86	B	EL	68.9	1.050	2.55	B	I	124.5	0.80	0.79	2.08	B	I	68.9	
		SNS5A	35,550		2.03	72,167	1.40	0.89	2.81	B	EL	68.9	1.050	2.55	B	I	124.5	0.80	0.79	2.03	B	I	68.9	
		SNS6A	39,950		1.84	73,508	1.40	0.89	2.53	B	EL	68.9	1.050	2.28	B	I	124.5	0.80	0.79	1.84	B	I	68.9	
	SNS7B	42,000		1.75	73,500	1.40	0.89	2.41	B	EL	68.9	1.050	2.20	B	I	124.5	0.80	0.79	1.75	B	I	68.9		
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33,000		2.23	73,590	1.40	0.89	3.08	B	EL	68.9	1.050	2.76	B	I	124.5	0.80	0.79	2.23	B	I	68.9	
		TNT4A	33,075		2.23	73,757	1.40	0.89	3.08	B	EL	68.9	1.050	2.73	B	I	124.5	0.80	0.79	2.23	B	I	68.9	
		TNT6A	41,600		1.80	74,880	1.40	0.89	2.48	B	EL	68.9	1.050	2.28	B	I	124.5	0.80	0.79	1.80	B	I	68.9	
		TNT7A	42,000		1.80	75,600	1.40	0.89	2.48	B	EL	68.9	1.050	2.25	B	I	124.5	0.80	0.79	1.80	B	I	68.9	
		TNT7B	42,000		1.84	77,280	1.40	0.89	2.54	B	EL	68.9	1.050	2.18	B	I	124.5	0.80	0.79	1.84	B	I	68.9	
		TNAGRIT4	43,000		1.76	75,680	1.40	0.89	2.43	B	EL	68.9	1.050	2.11	B	I	124.5	0.80	0.79	1.76	B	I	68.9	
TNAGT5A		45,000		1.67	75,150	1.40	0.89	2.31	B	EL	68.9	1.050	2.05	B	I	124.5	0.80	0.79	1.67	B	I	68.9		
TNAGT5B	45,000		Ⓝ	1.66	74,700	1.40	0.89	2.29	B	EL	68.9	1.050	2.01	B	I	124.5	0.80	0.79	1.66	B	I	68.9		

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.
ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

1. TRANSFORMING ALL PRESTRESSING TENDONS.
- 2.
- 3.
- 4.

Ⓝ CONTROLLING LOAD RATING

Ⓛ DESIGN LOAD RATING (HL-93)

Ⓜ DESIGN LOAD RATING (HS-20)

Ⓝ LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

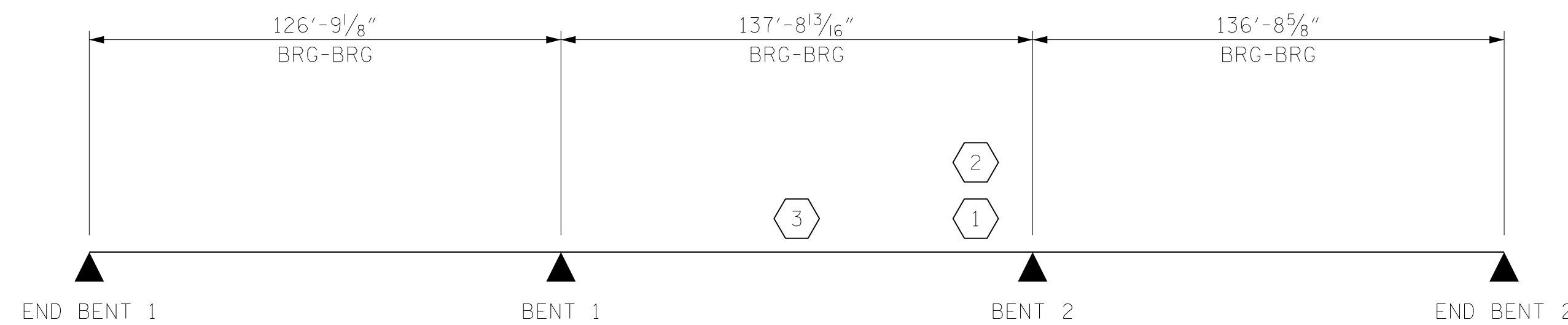
I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER - EXTERIOR RIGHT GIRDER

SECTION PROPERTIES			
SPAN B - INTERIOR			
	UNITS	NON-COMPOSITE	COMPOSITE
HEIGHT	IN	74	82.5
AREA	IN ²	885.1	1599.1
Ixx	IN ⁴	640172	1331347
Ycg	IN	36.56	55.17
SELF WT.	PLF	922.0	2037.6
EFF. WIDTH	IN	-	126.0

SECTION PROPERTIES PROVIDED AT MIDSPAN

SECTION PROPERTIES			
SPAN B - EXTERIOR			
	UNITS	NON-COMPOSITE	COMPOSITE
HEIGHT	IN	74	82.5
AREA	IN ²	885.1	1510.0
Ixx	IN ⁴	640172	1280580
Ycg	IN	36.56	53.81
SELF WT.	PLF	922.0	1898.4
EFF. WIDTH	IN	-	110.3

SECTION PROPERTIES PROVIDED AT MIDSPAN



PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
STANDARD LRFR SUMMARY FOR PRESTRESSED CONCRETE GIRDERS (NON-INTERSTATE TRAFFIC) LEFT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					SHEET NO. S1-5 TOTAL SHEETS 36

DRAWN BY : MAL DATE : 03/2017
 CHECKED BY : TLC DATE : 03/2017
 DESIGN ENGINEER OF RECORD: MAL DATE : 03/2017

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SIGNATURES COMPLETED

NOTES

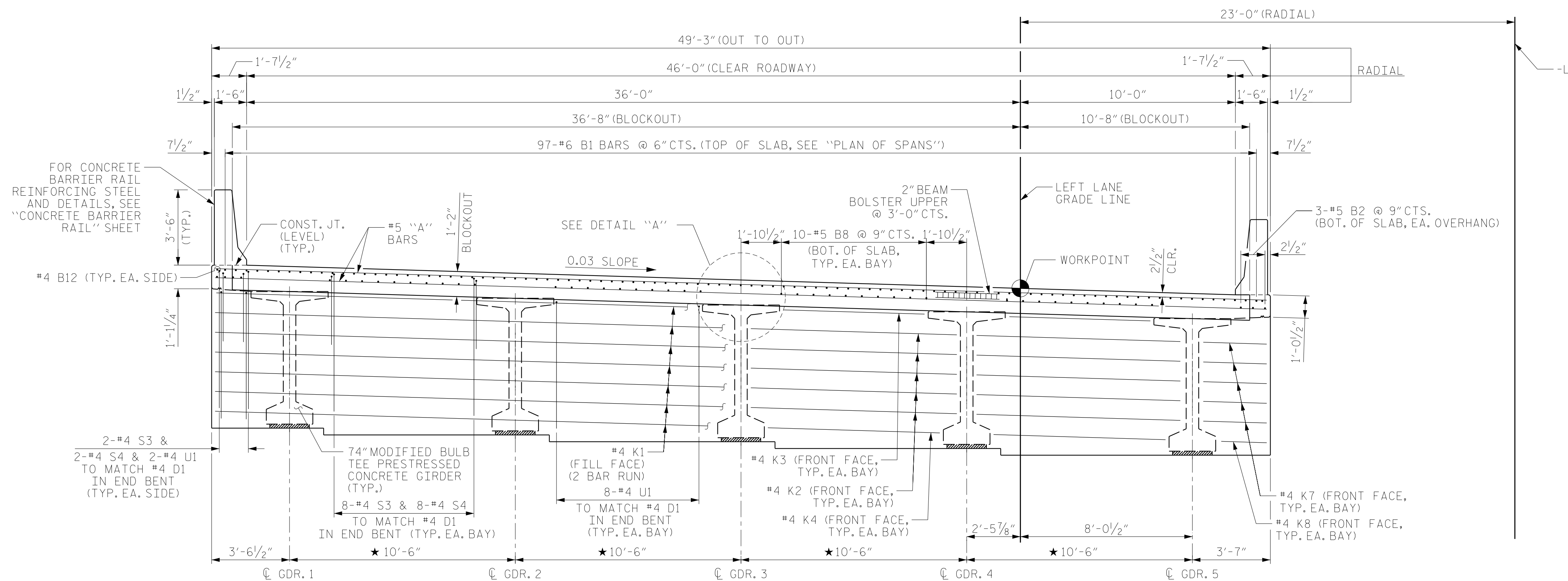
PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER @ 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK @ 4'-0" CTS. WITH A HEIGHT TO PROVIDE 2 1/2" CLEAR DISTANCE ABOVE THE FORMS.

LONGITUDINAL REINFORCING STEEL ("B" BARS) MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.

PREVIOUSLY CAST CONCRETE IN A CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.

CONCRETE BARRIER RAIL IN A CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL DECK SLAB CONCRETE IN THAT UNIT HAS BEEN CAST AND HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

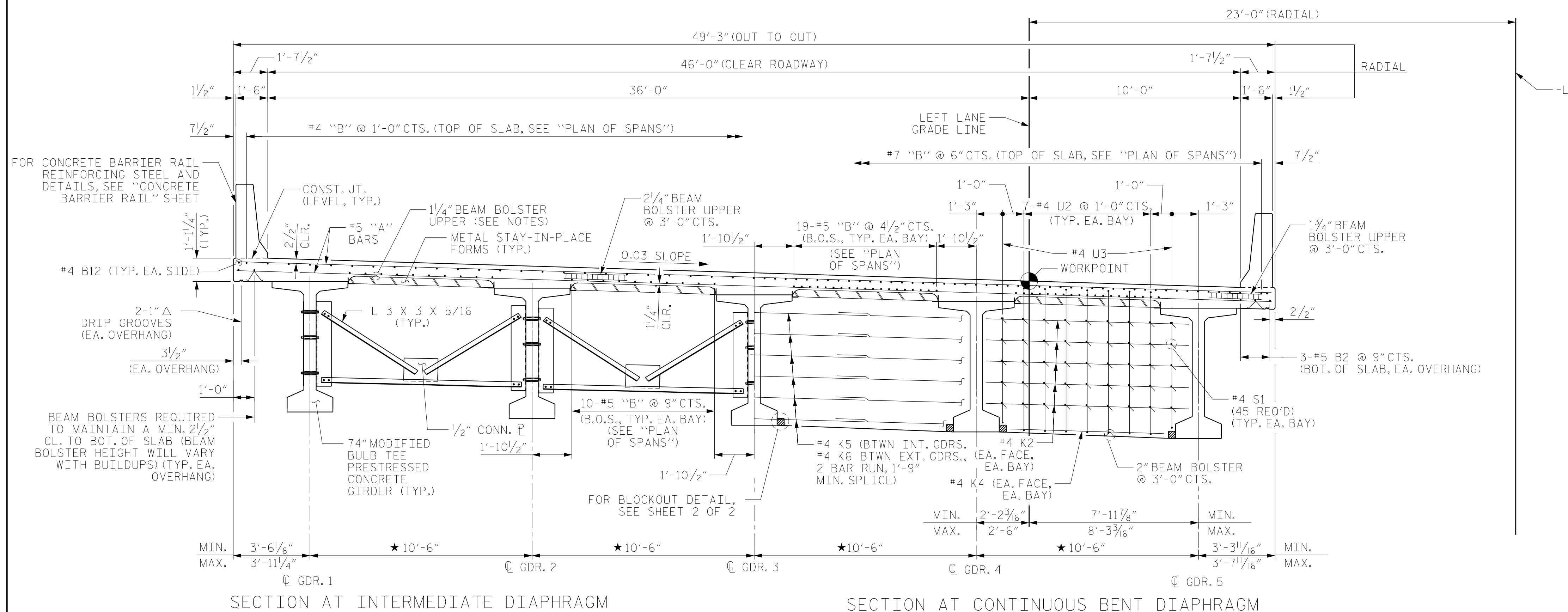
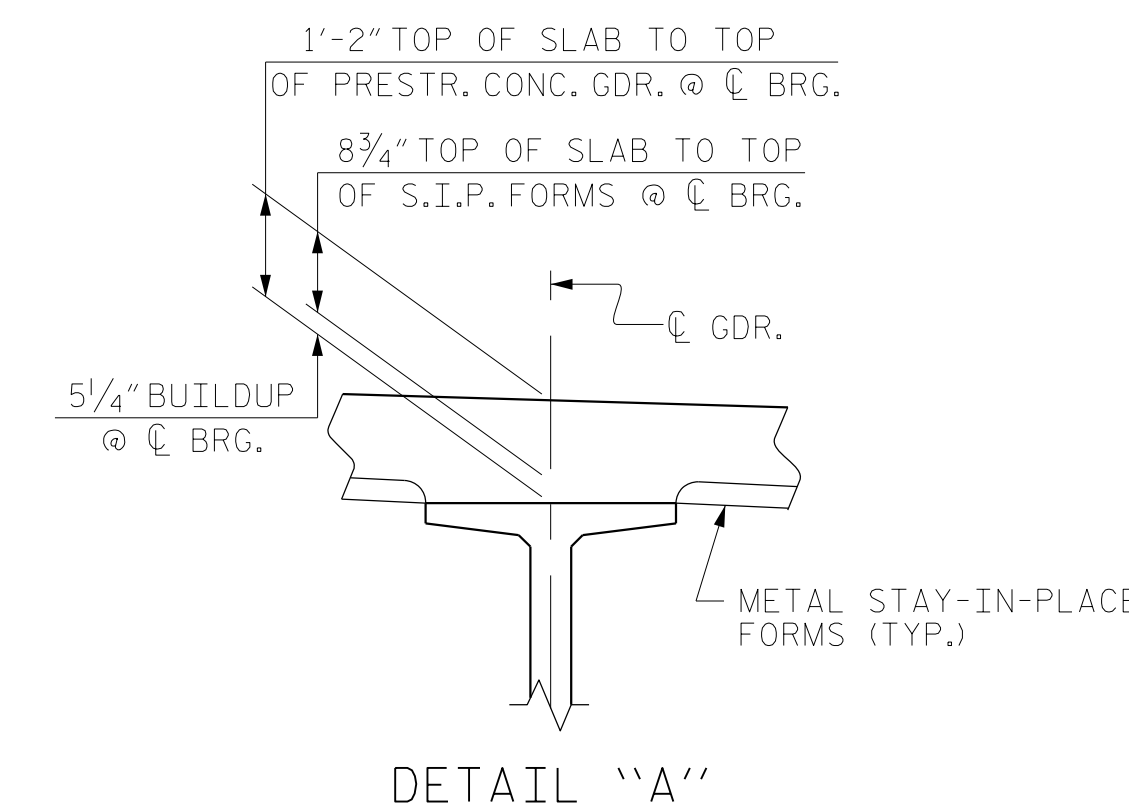
B.O.S. = BOTTOM OF SLAB



TYPICAL SECTION AT INTEGRAL END BENTS

END BENT 1 SHOWN, END BENT 2 SIMILAR

★ DIMENSIONS ARE PARALLEL TO SHORT CHORD OF RESPECTIVE SPAN



SECTION AT INTERMEDIATE DIAPHRAGM

SECTION AT CONTINUOUS BENT DIAPHRAGM

TYPICAL SECTION

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

SHEET 1 OF 2



RS&H Architects-Engineers-Planners, Inc.

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919-926-4100 FAX 919-846-9080

www.rsandh.com

North Carolina License No. 50737-54043-C&E

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
TYPICAL SECTION
LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-6
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : PDS DATE : 11/2016
CHECKED BY : JMR DATE : 11/2016
DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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NOTES

FOR SPLICE LENGTHS NOT SHOWN, REFER TO MINIMUM SPLICE LENGTH TABLE ON "SUPERSTRUCTURE BILL OF MATERIAL" SHEET 1 OF 2.

FOR END BENT DIAPHRAGM BARS AND BENT DIAPHRAGM BARS, SEE "TYPICAL SECTION DETAILS" SHEET 2 OF 2.

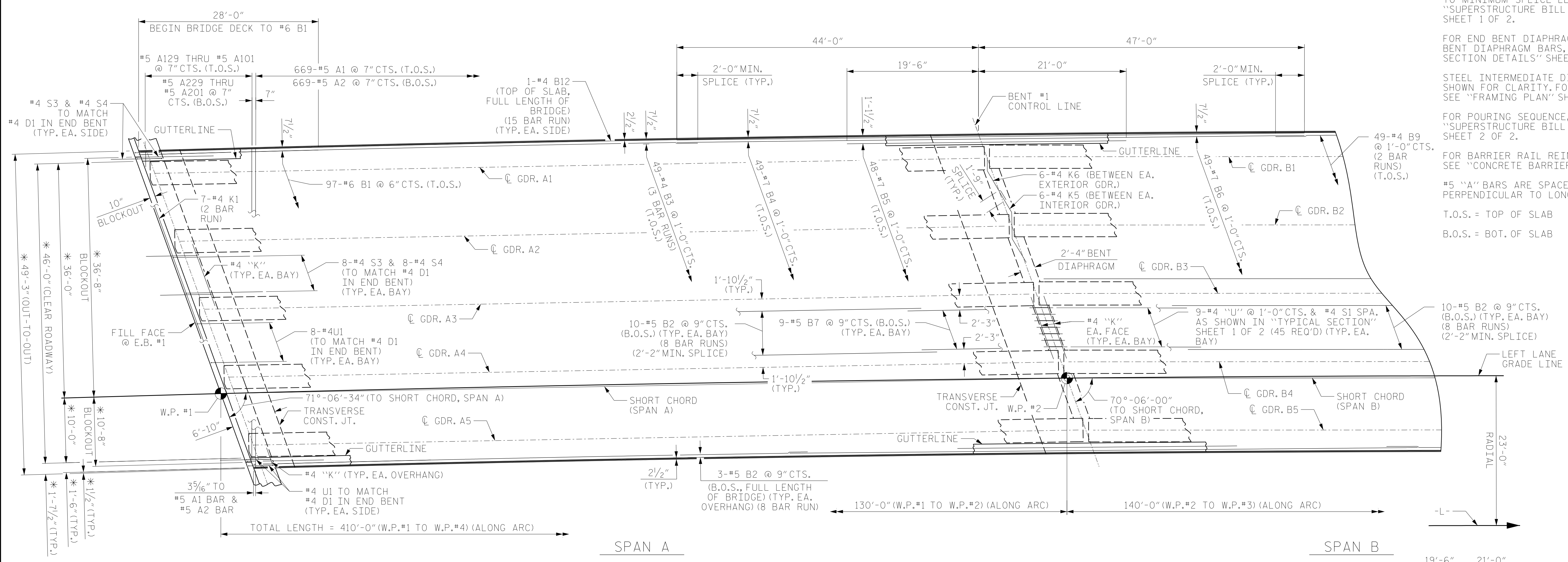
STEEL INTERMEDIATE DIAPHRAGMS NOT SHOWN FOR CLARITY, FOR LOCATIONS, SEE "FRAMING PLAN" SHEET.

FOR POURING SEQUENCE, SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET 2 OF 2.

FOR BARRIER RAIL REINFORCING STEEL, SEE "CONCRETE BARRIER RAIL" SHEET.

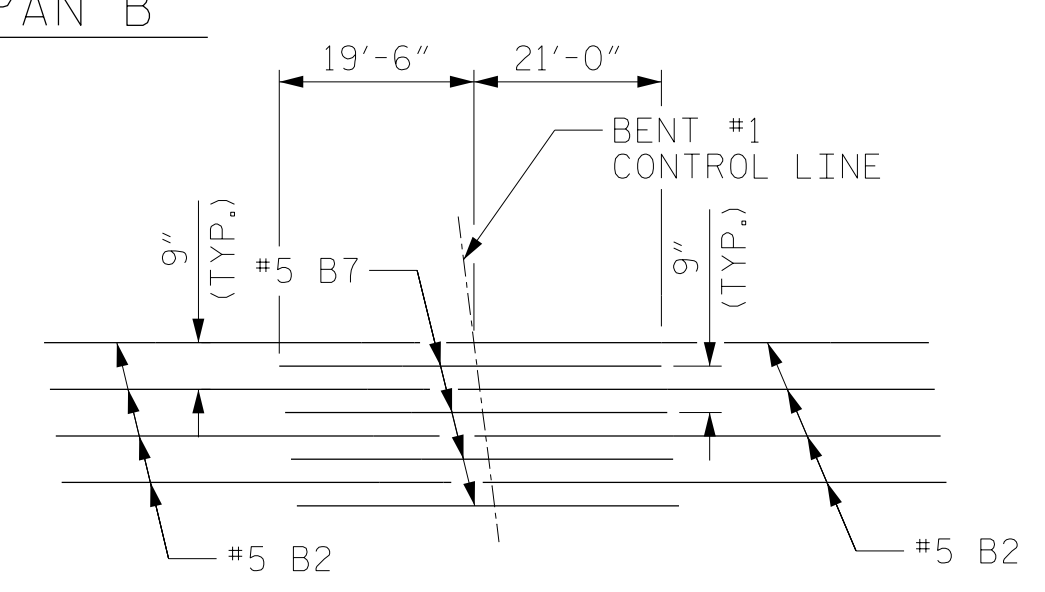
#5 "A" BARS ARE SPACED PERPENDICULAR TO LONG CHORD.

T.O.S. = TOP OF SLAB
B.O.S. = BOT. OF SLAB

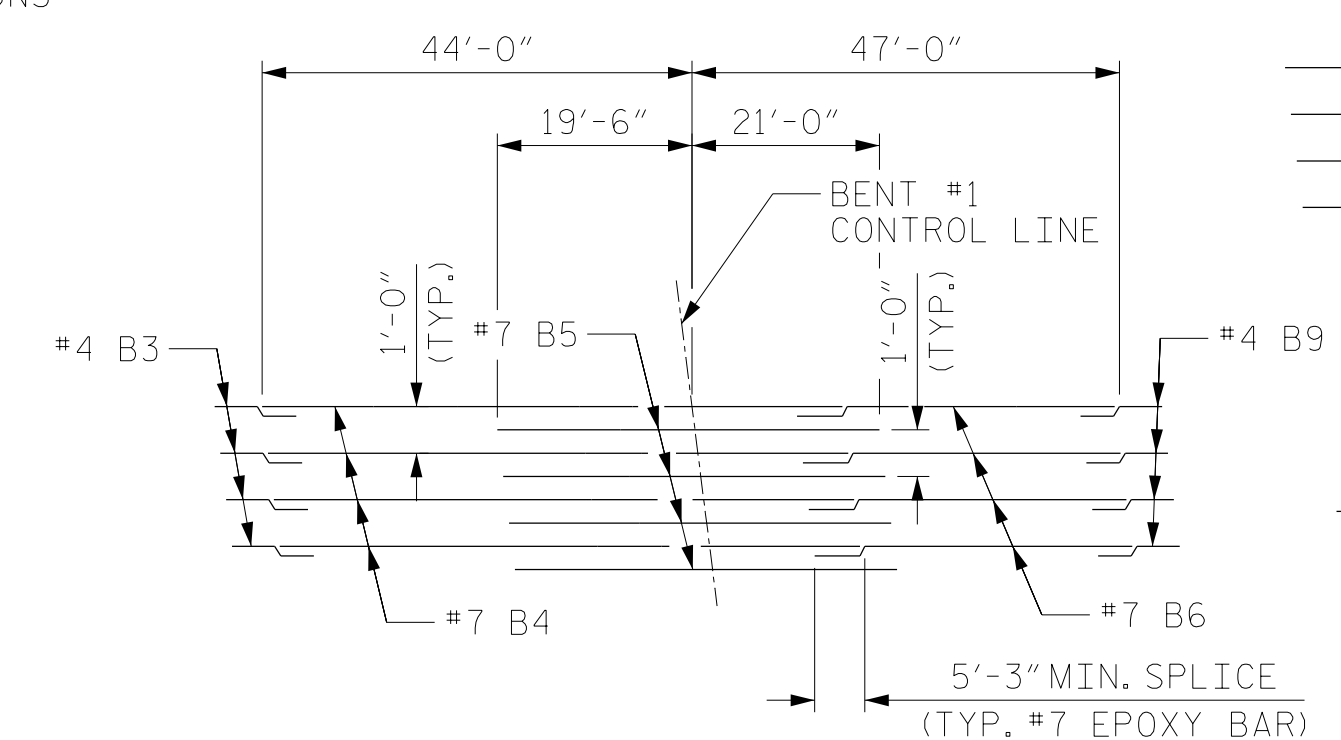


PARTIAL PLAN OF SPANS

* RADIAL DIMENSIONS



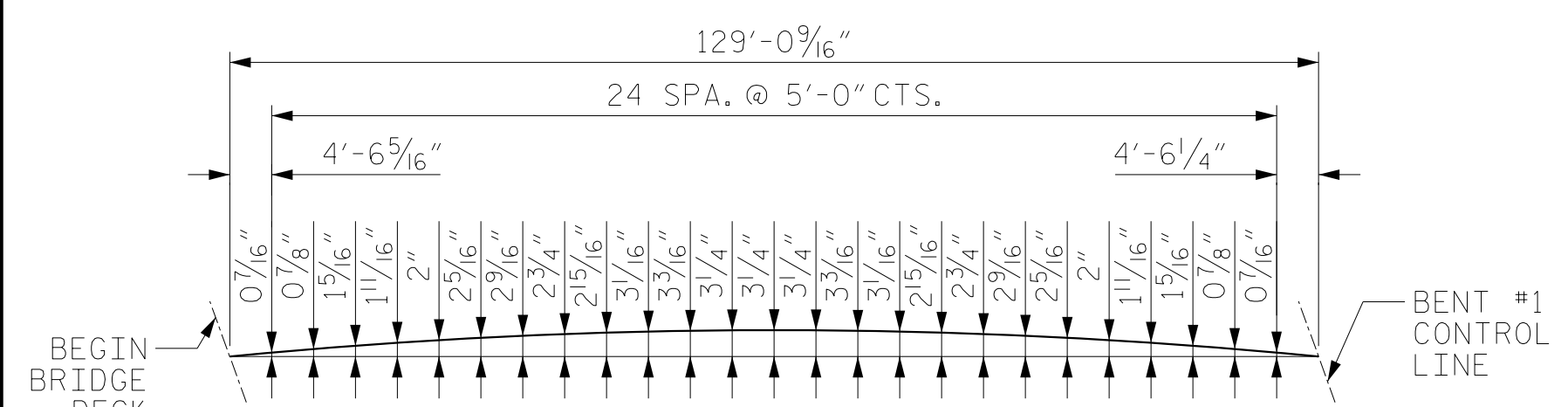
BOTTOM OF SLAB REINFORCING STEEL LAYOUT



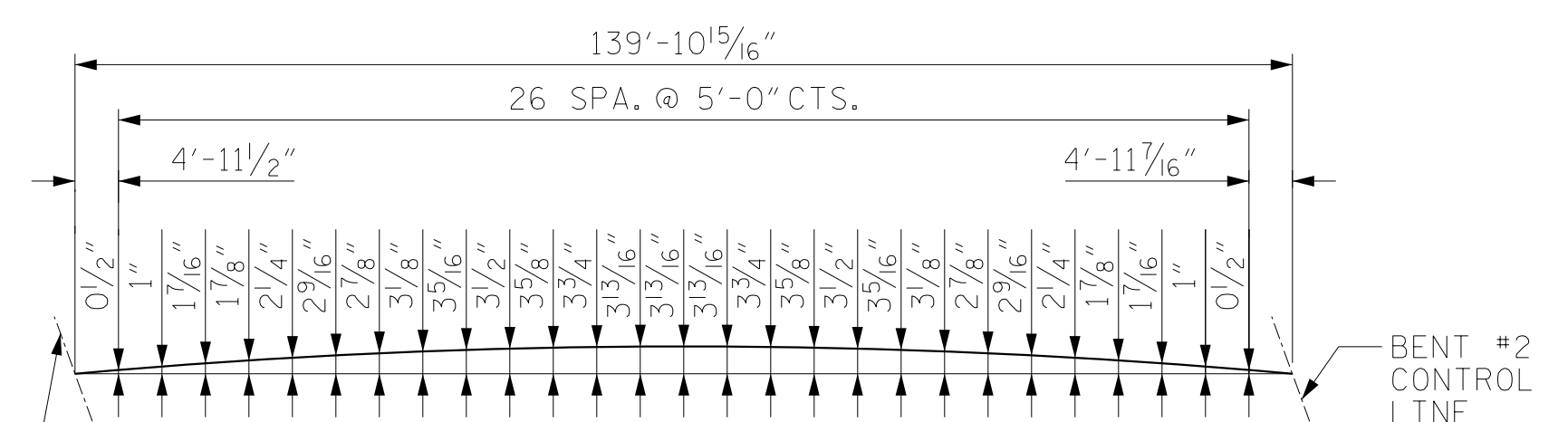
TOP OF SLAB REINFORCING STEEL LAYOUT

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

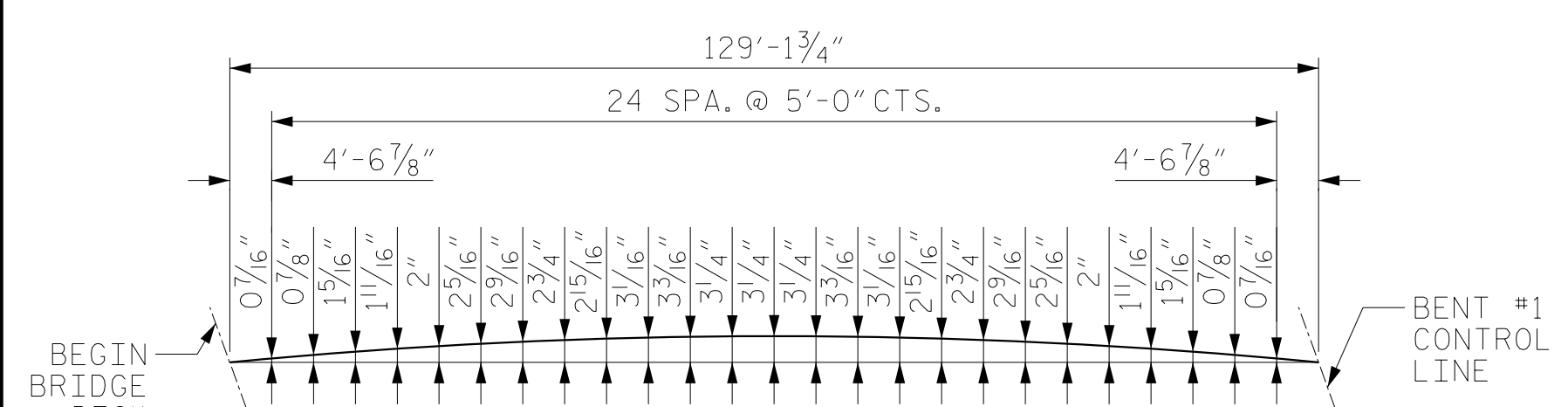
SHEET 1 OF 2



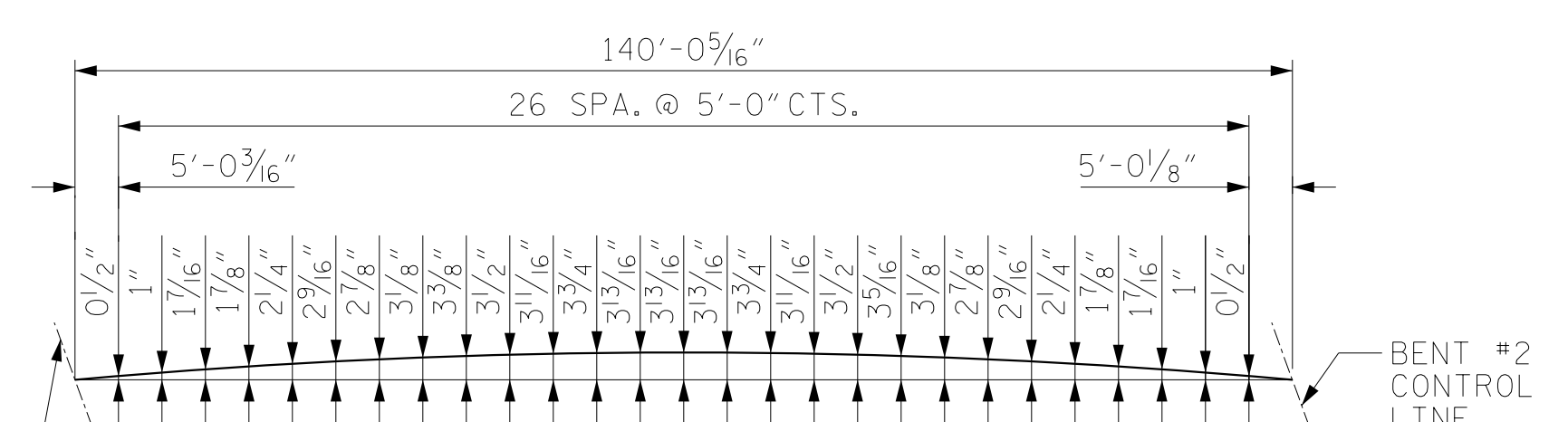
LEFT SIDE - SPAN A



LEFT SIDE - SPAN B



RIGHT SIDE - SPAN A



RIGHT SIDE - SPAN B

ARC OFFSETS

DRAWN BY: MAL DATE: 11/2016
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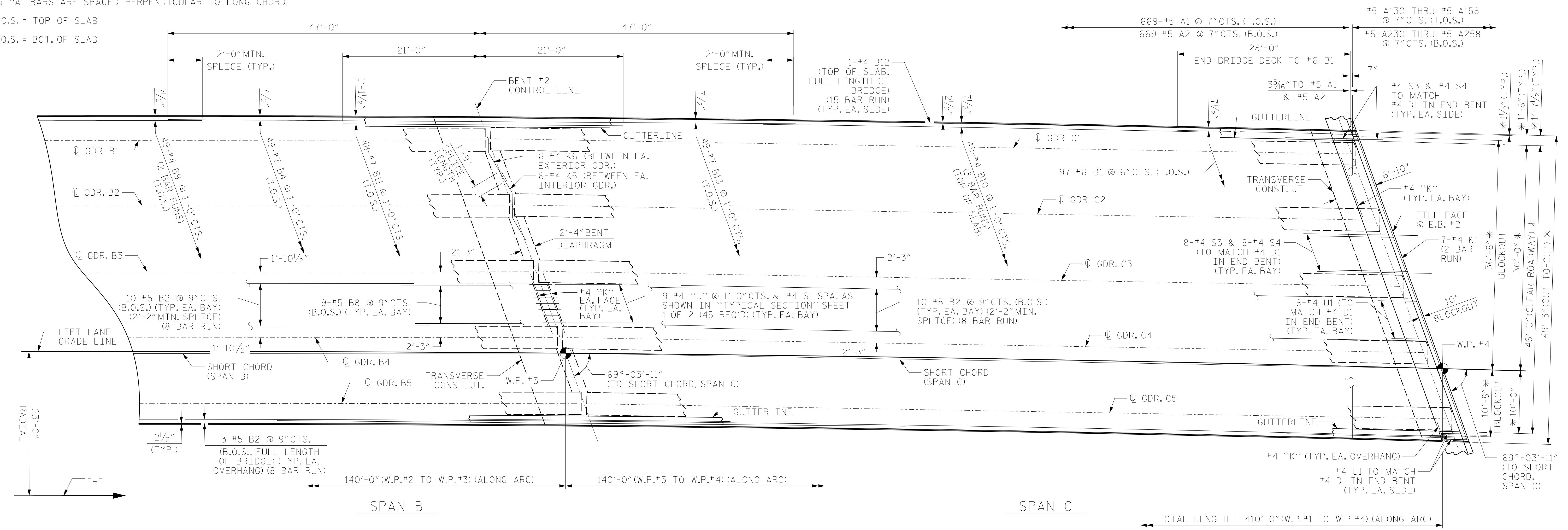
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8601 Six Forks Road, Suite 260
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www.rsandh.com
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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE PLAN OF SPANS LEFT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S1-8
TOTAL SHEETS					36

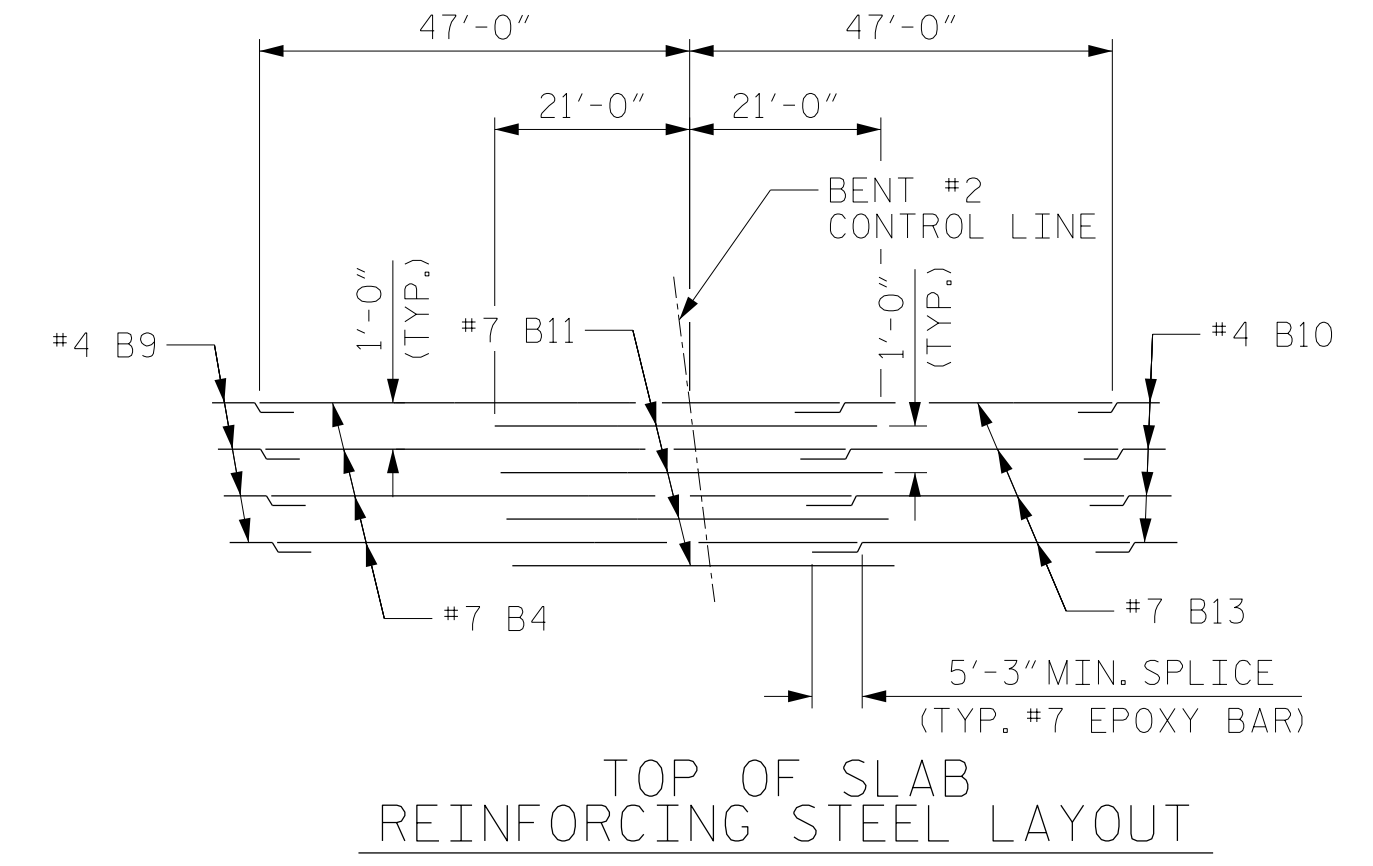
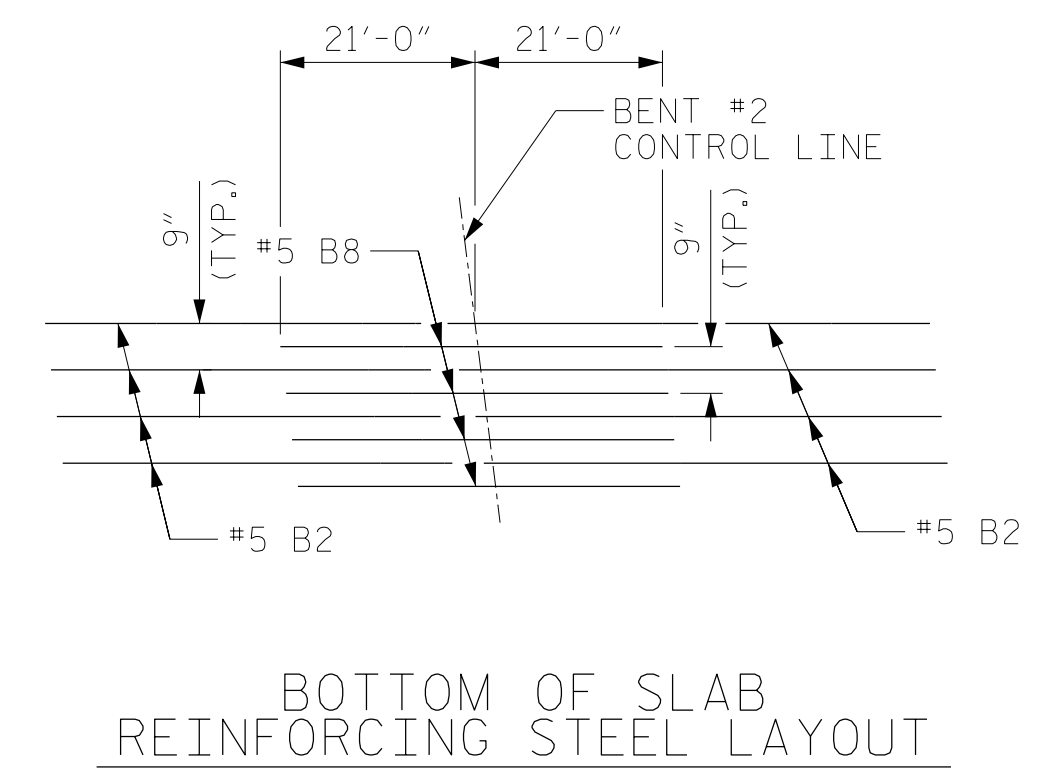
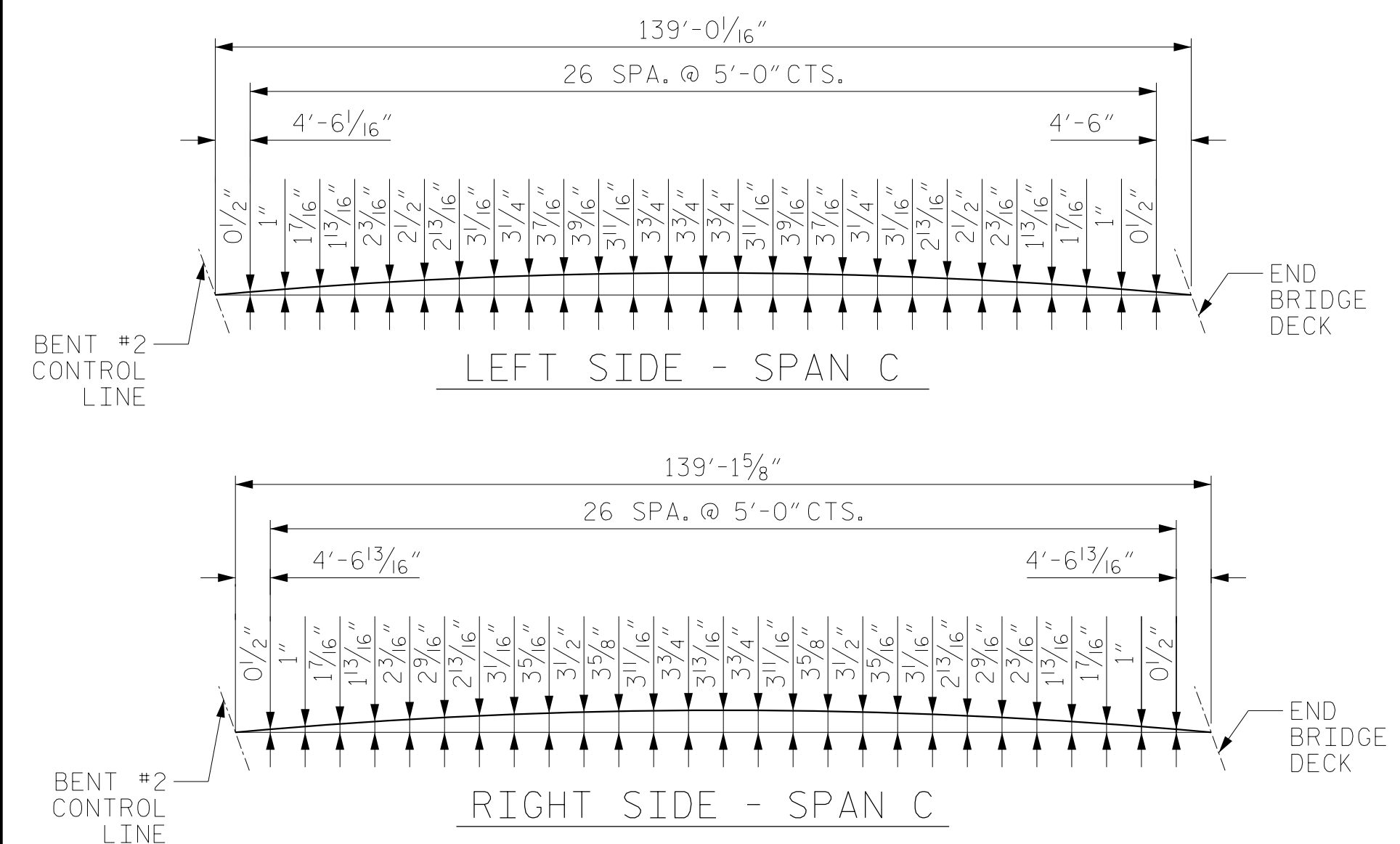
NOTES

FOR SPLICE LENGTHS NOT SHOWN, REFER TO MINIMUM SPLICE LENGTH TABLE ON "SUPERSTRUCTURE BILL OF MATERIAL" SHEET 1 OF 2.
FOR END BENT DIAPHRAGM BARS AND BENT DIAPHRAGM BARS, SEE "TYPICAL SECTION DETAILS" SHEET 2 OF 2.
STEEL INTERMEDIATE DIAPHRAGMS NOT SHOWN FOR CLARITY. FOR LOCATIONS, SEE "FRAMING PLAN" SHEET.
FOR POURING SEQUENCE, SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET 2 OF 2.
FOR BARRIER RAIL REINFORCING STEEL, SEE "CONCRETE BARRIER RAIL" SHEET.
#5 "A" BARS ARE SPACED PERPENDICULAR TO LONG CHORD.

T.O.S. = TOP OF SLAB
B.O.S. = BOT. OF SLAB



PARTIAL PLAN OF SPANS
*RADIAL DIMENSIONS



PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-
SHEET 2 OF 2



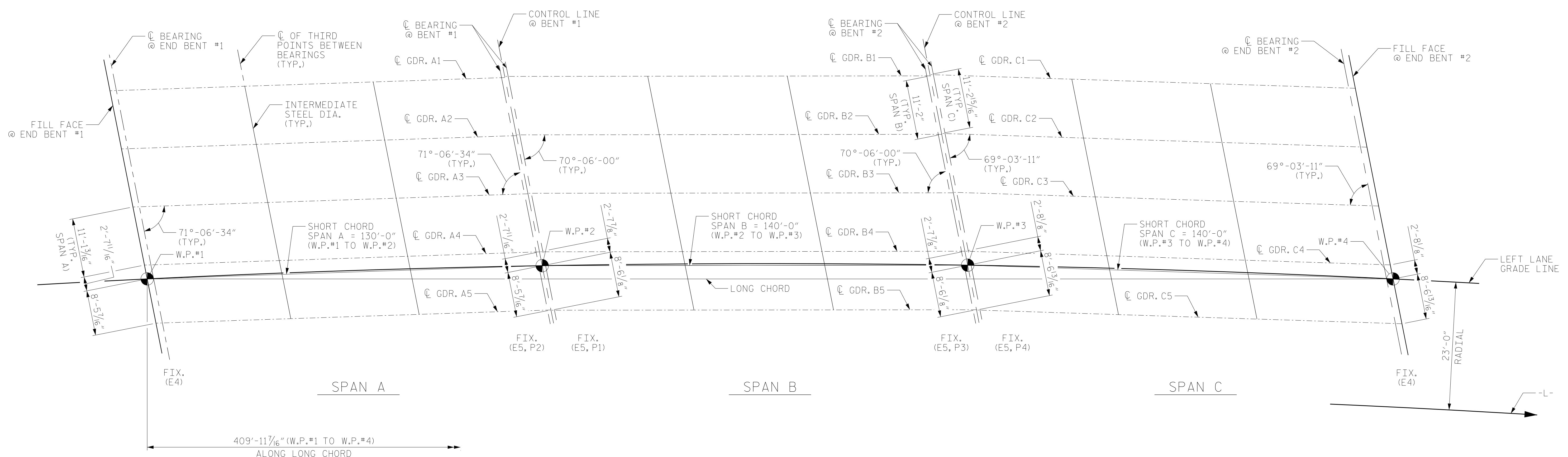
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
PLAN OF SPANS
LEFT LANE

DRAWN BY: MAL DATE: 11/2016
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DESIGN ENGINEER OF RECORD: MAL DATE: 11/2016

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REVISIONS
NO. BY: DATE: NO. BY: DATE:

REVISIONS			SHEET NO.		
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		



FRAMING PLAN

END BENT DIAPHRAGMS & BENT DIAPHRAGMS ARE NOT SHOWN
 DIMENSIONS TO INTERMEDIATE DIAPHRAGM LOCATIONS ARE SHOWN ON GIRDER SHEETS
 ALL DIMENSIONS ARE TO THE FILL FACE OR CONTROL LINE OF EACH BENT

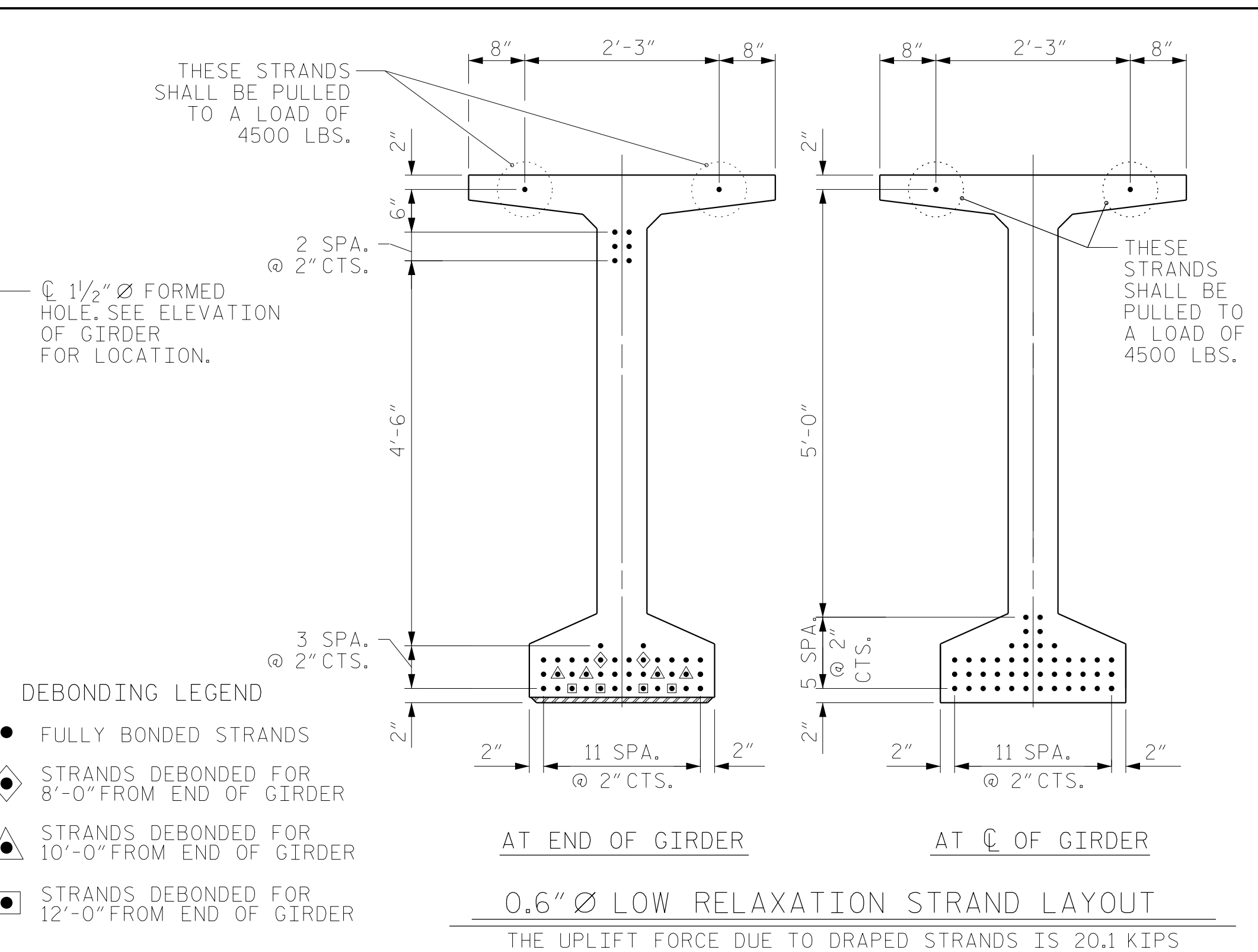
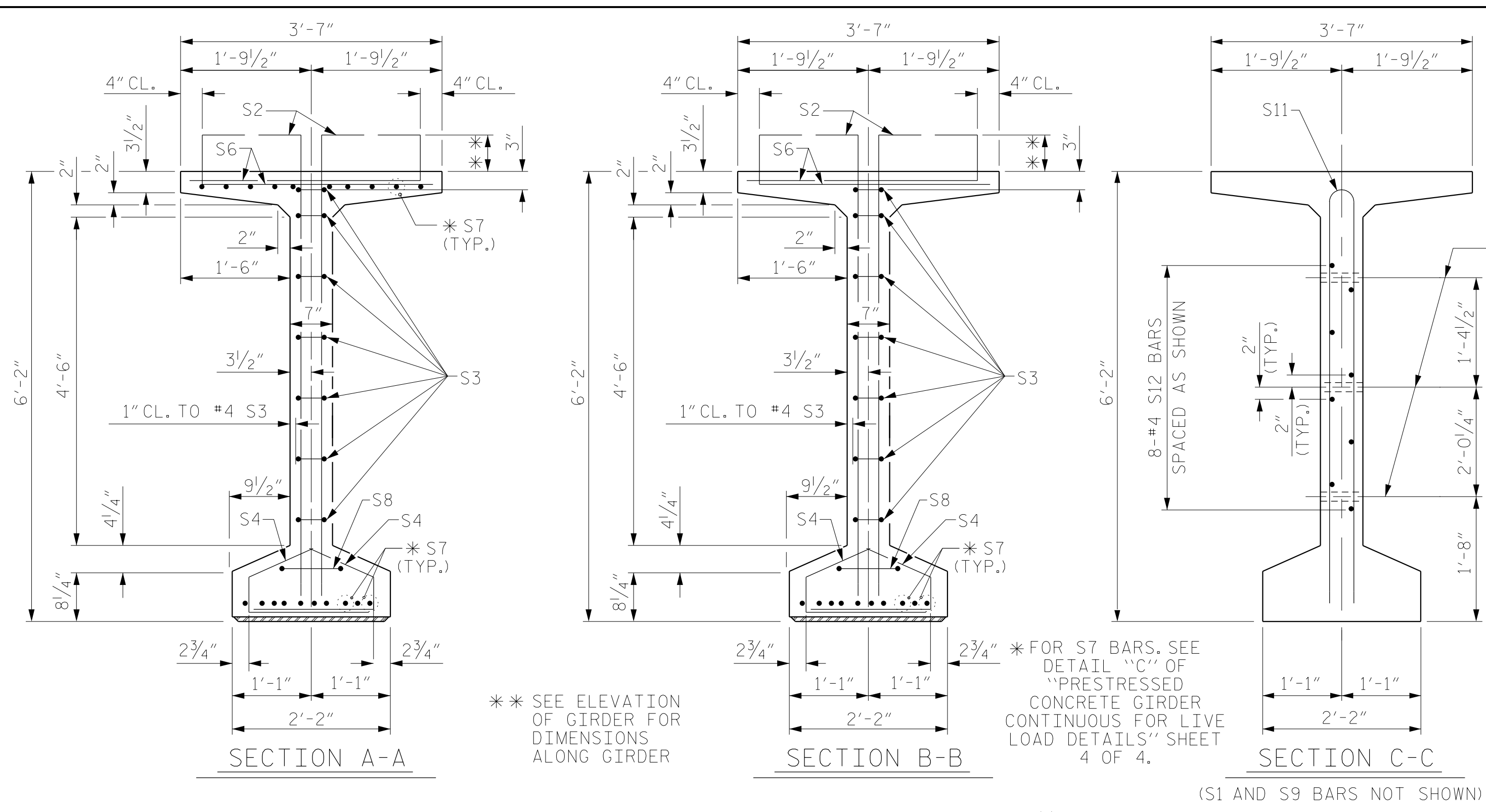
PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

DRAWN BY : MAL DATE : 11/2016
 CHECKED BY : JMR DATE : 11/2016
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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE FRAMING PLAN SPANS A, B & C LEFT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO. S1-10					TOTAL SHEETS 36



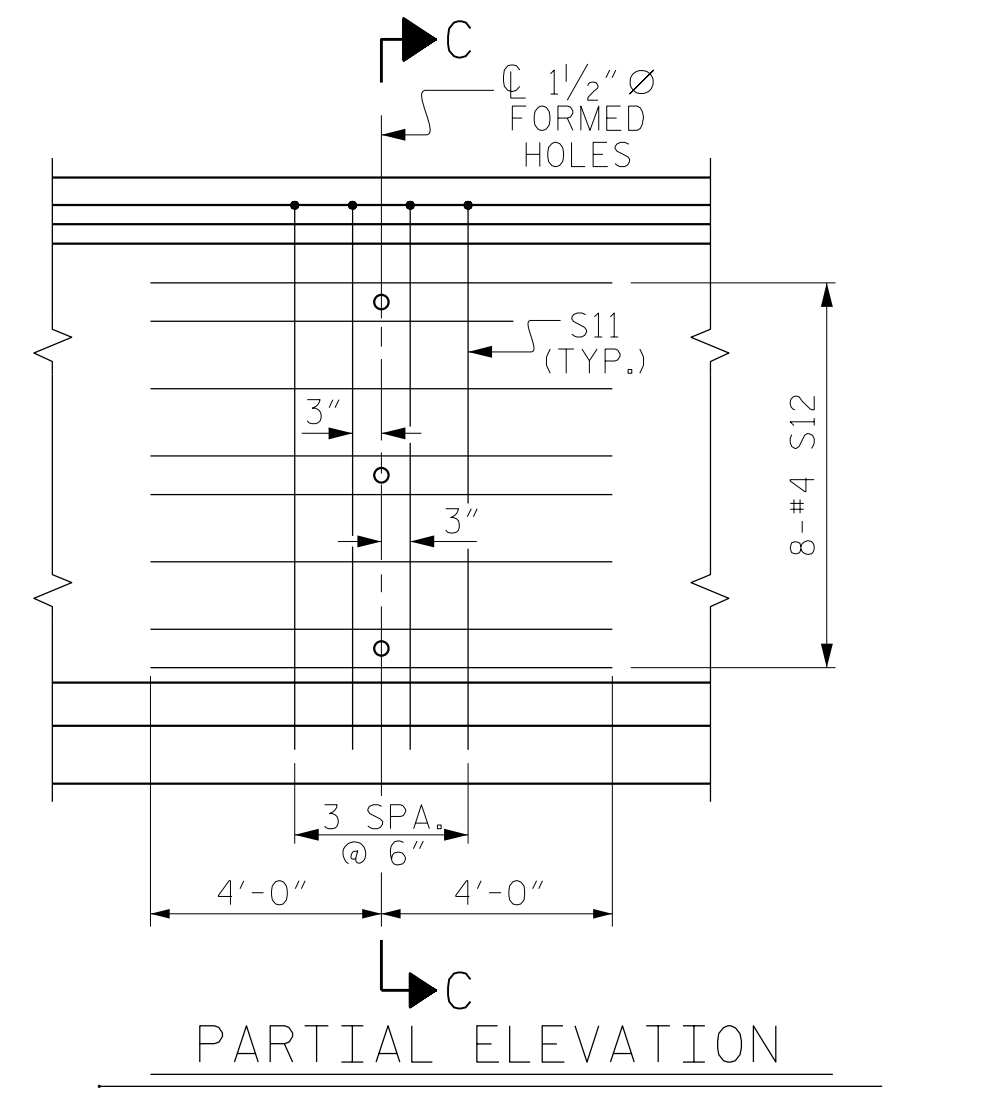
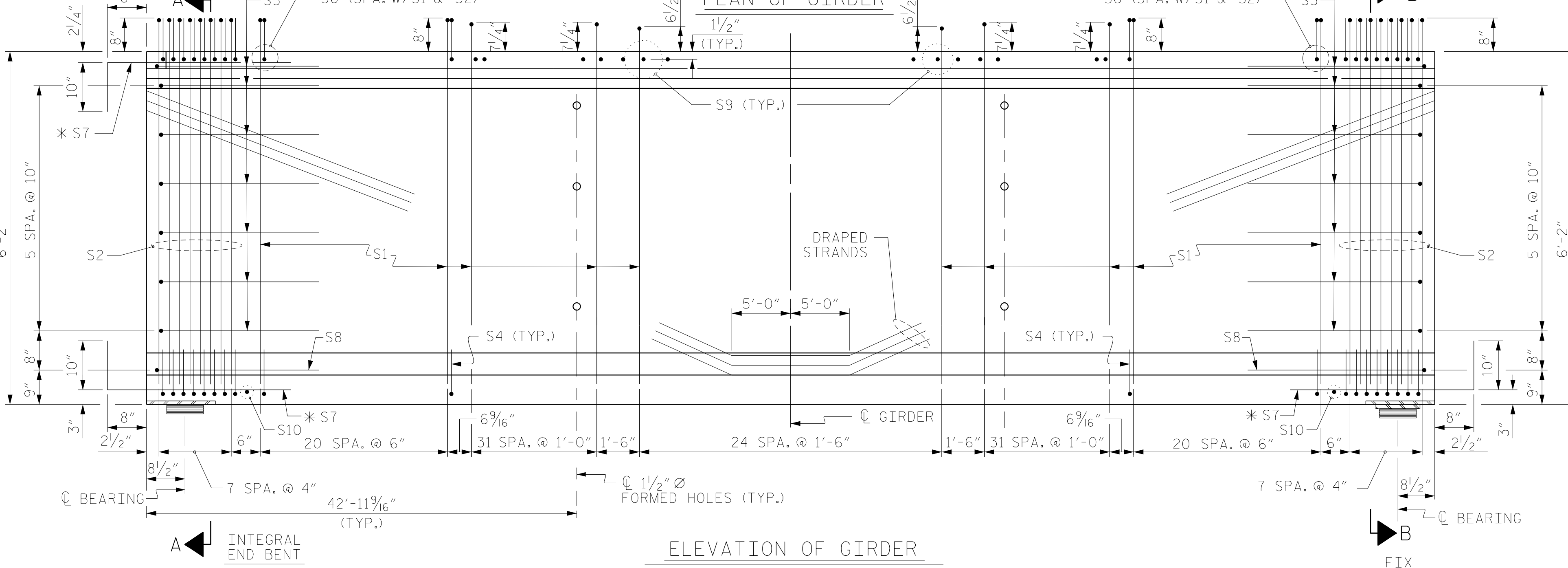
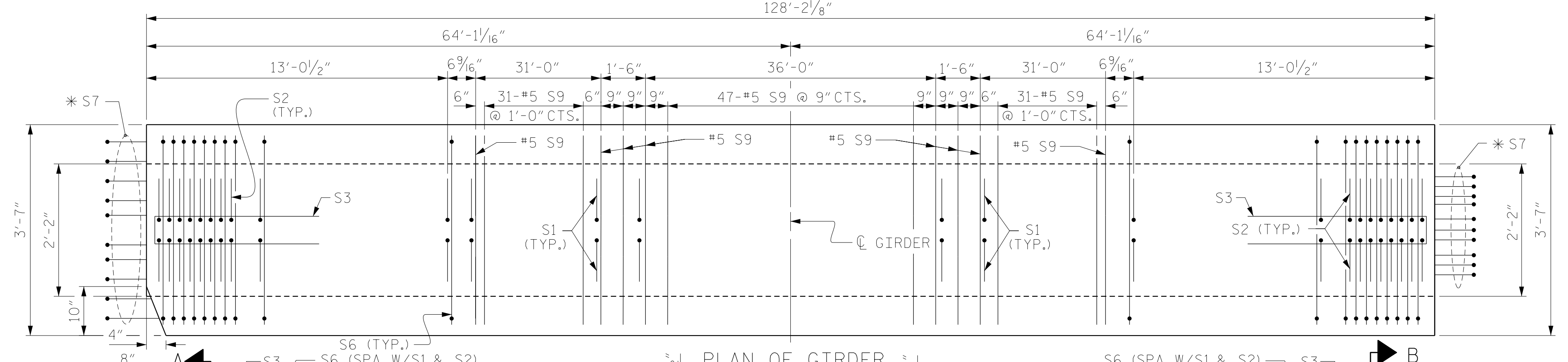
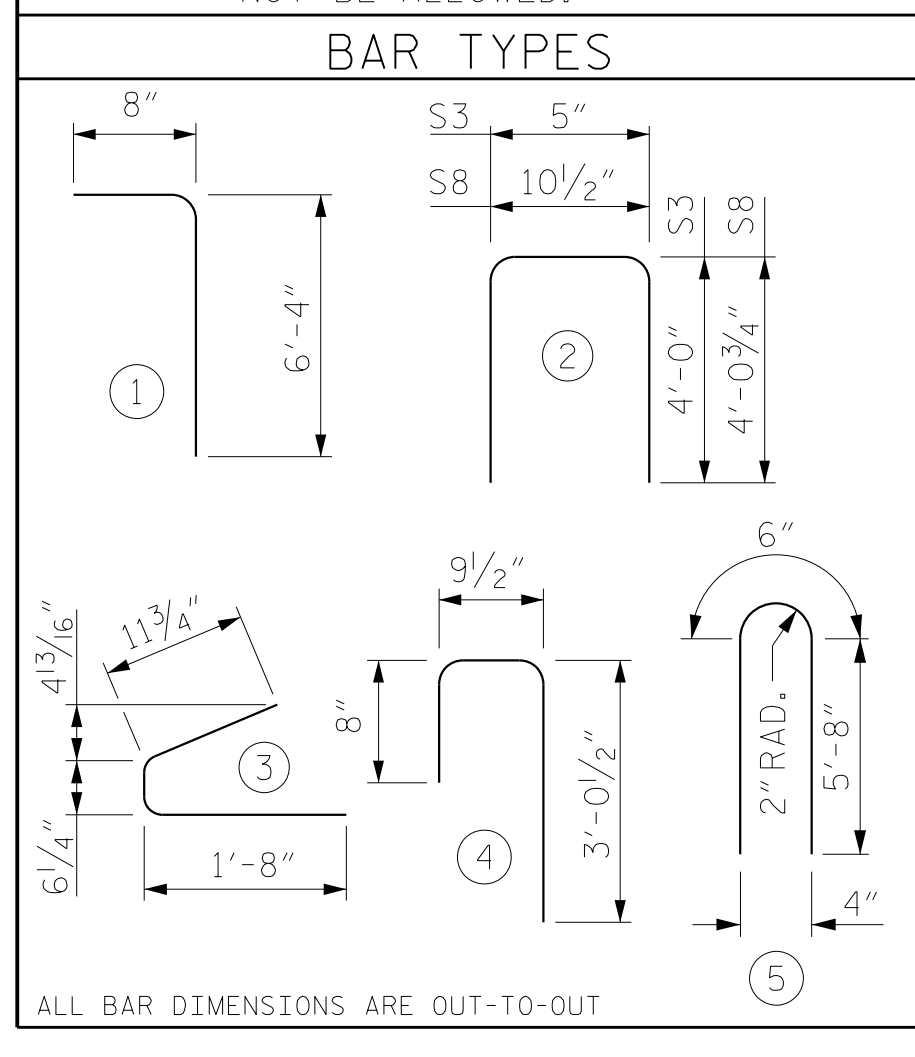
0.6" Ø L. R. GRADE 270 STRANDS

AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR

BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	262	#4	1	7'-0"	1225
S2	32	#5	1	7'-0"	234
S3	14	#4	2	8'-5"	79
S4	116	#4	3	3'-2"	245
S6	116	#5	4	4'-6"	544
*S7	30	#5	STR	3'-8"	115
S8	2	#5	2	9'-0"	19
S9	117	#5	STR	3'-3"	397
S10	2	#3	STR	1'-10"	1
S11	8	#5	5	11'-10"	99
S12	16	#4	STR	8'-0"	86

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



QUANTITIES FOR ONE GIRDER

REINFORCING STEEL	9000 PSI CONCRETE		0.6" Ø L.R. STRANDS
	LB.	C.Y.	No.
	3044	29.2	46

GIRDERS REQUIRED

NUMBER	LENGTH	TOTAL LENGTH
5	128'-2 1/8"	640'-10 5/8"

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 1 OF 4



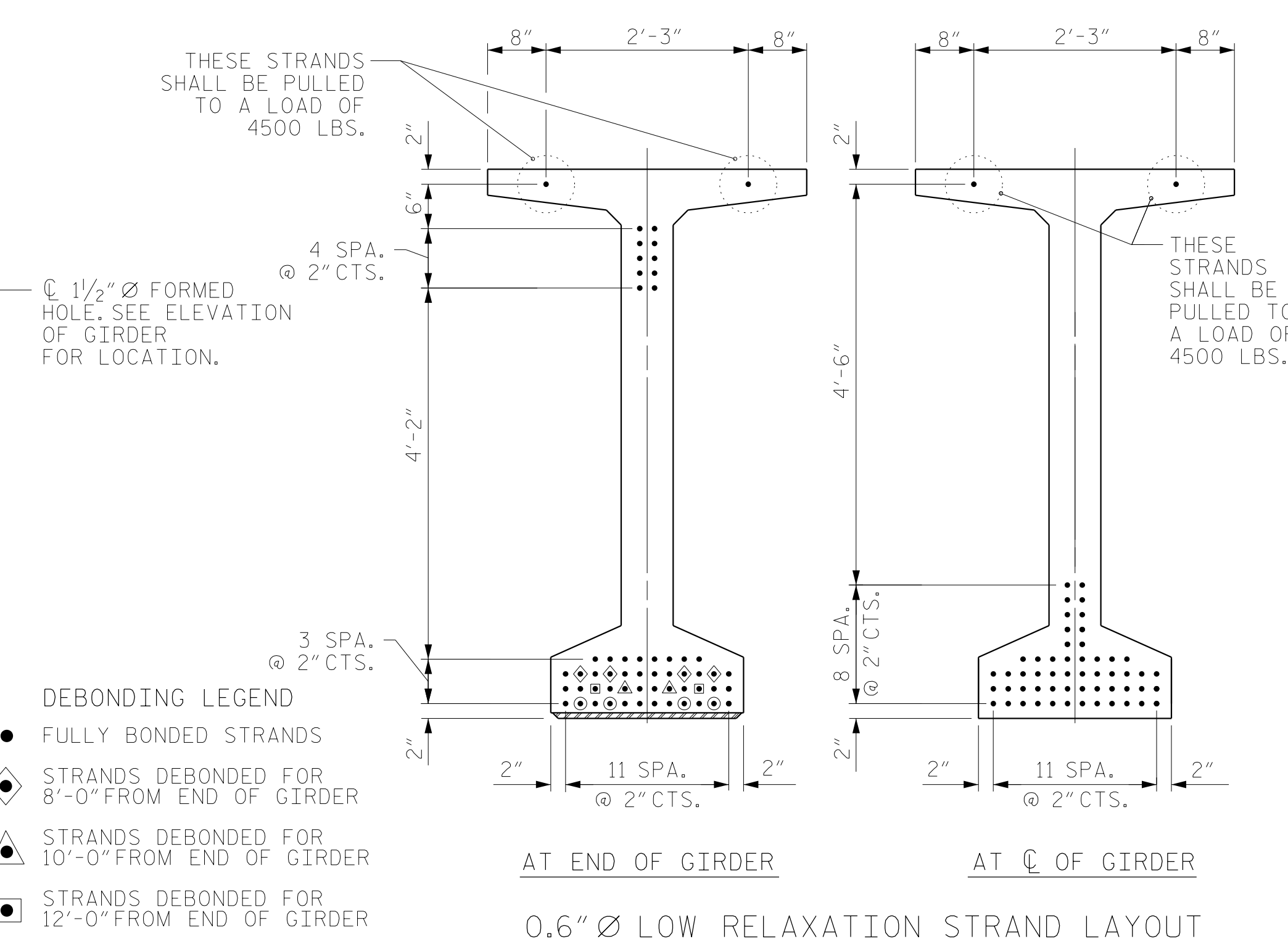
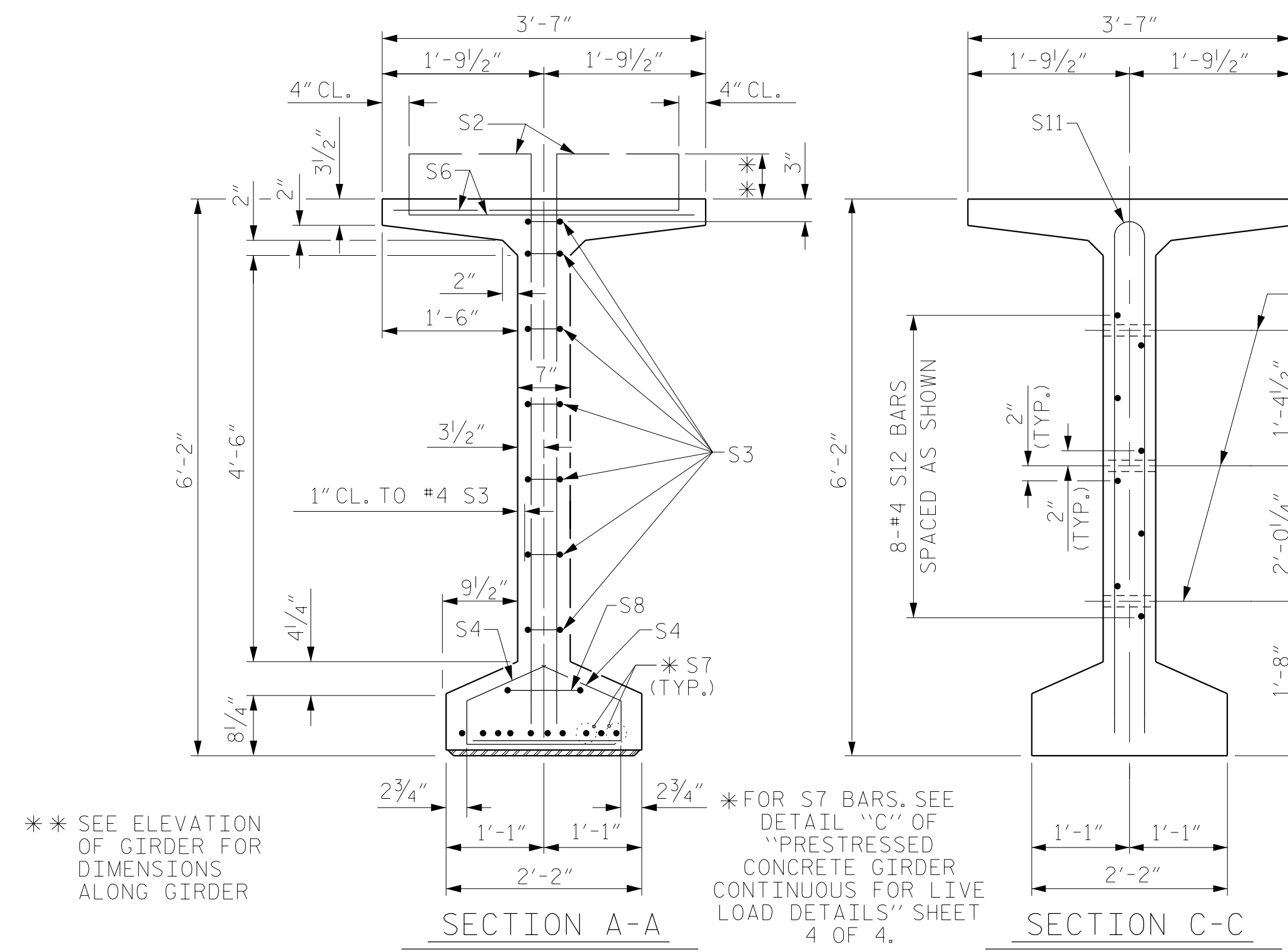
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 74" PRESTRESSED CONCRETE
 MODIFIED BULB TEE
 CONTINUOUS FOR LIVE LOAD
 SPAN A
 LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-11
1			3			TOTAL SHEETS
2			4			36

DRAWN BY: MAL DATE: 11/2016
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 DESIGN ENGINEER OF RECORD: MAL DATE: 11/2016

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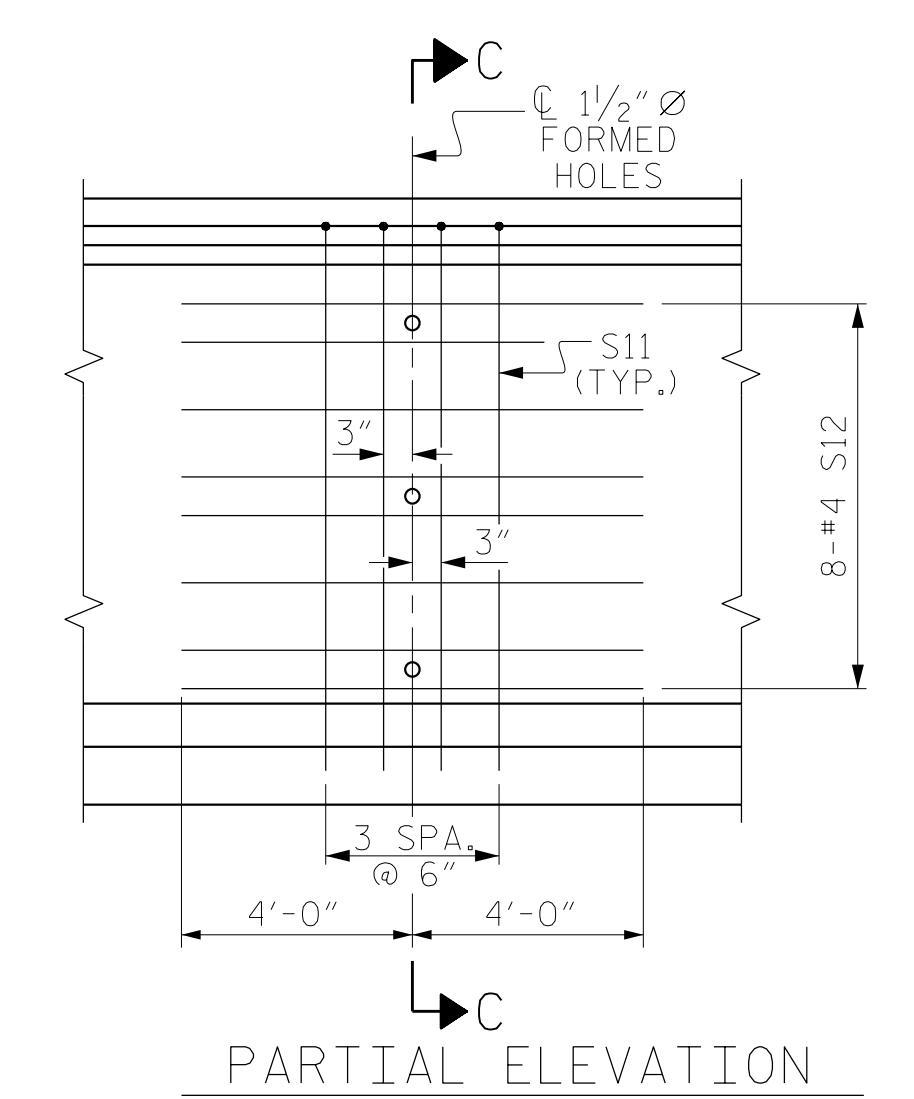
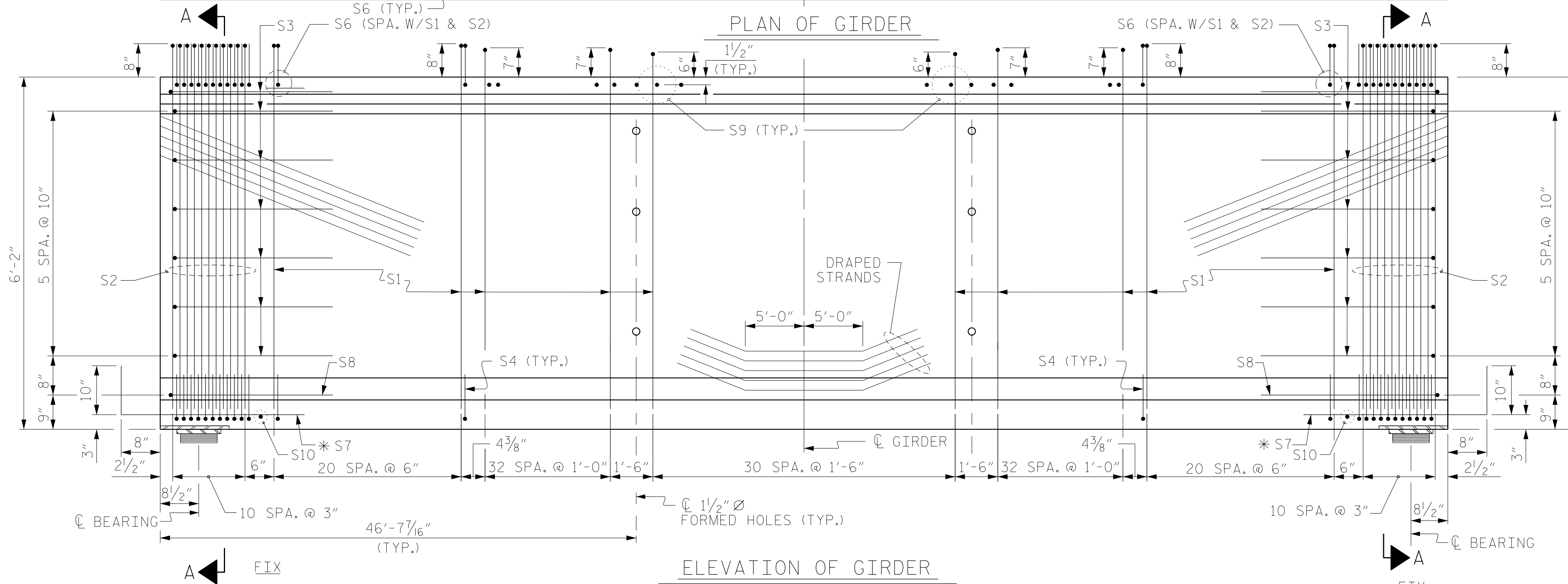
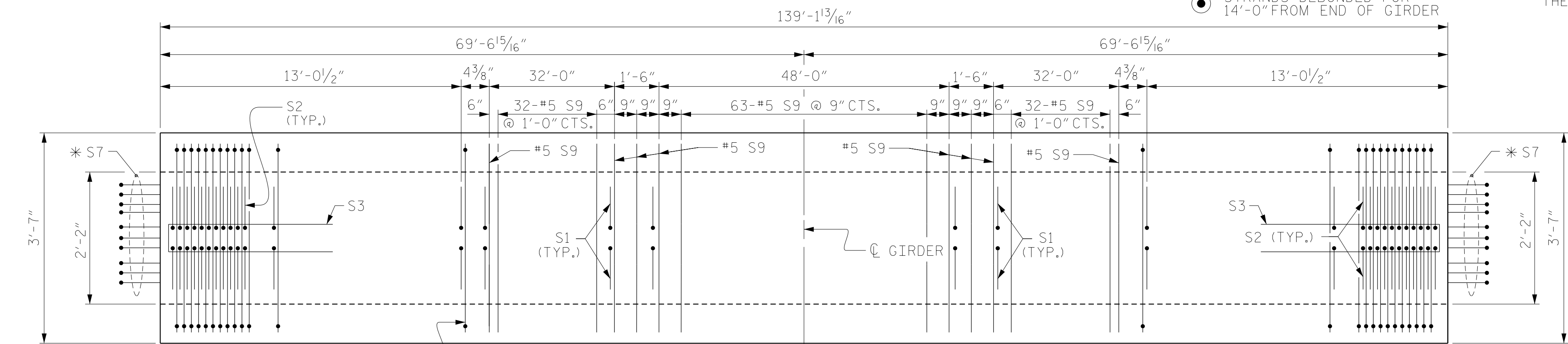


- DEBONDING LEGEND**
- FULLY BONDED STRANDS
 - ◊ STRANDS DEBONDED FOR 8'-0" FROM END OF GIRDER
 - ▲ STRANDS DEBONDED FOR 10'-0" FROM END OF GIRDER
 - ◻ STRANDS DEBONDED FOR 12'-0" FROM END OF GIRDER
 - STRANDS DEBONDED FOR 14'-0" FROM END OF GIRDER

** SEE ELEVATION OF GIRDER FOR DIMENSIONS ALONG GIRDER

* FOR S7 BARS. SEE DETAIL "C" OF "PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD DETAILS" SHEET 4 OF 4.

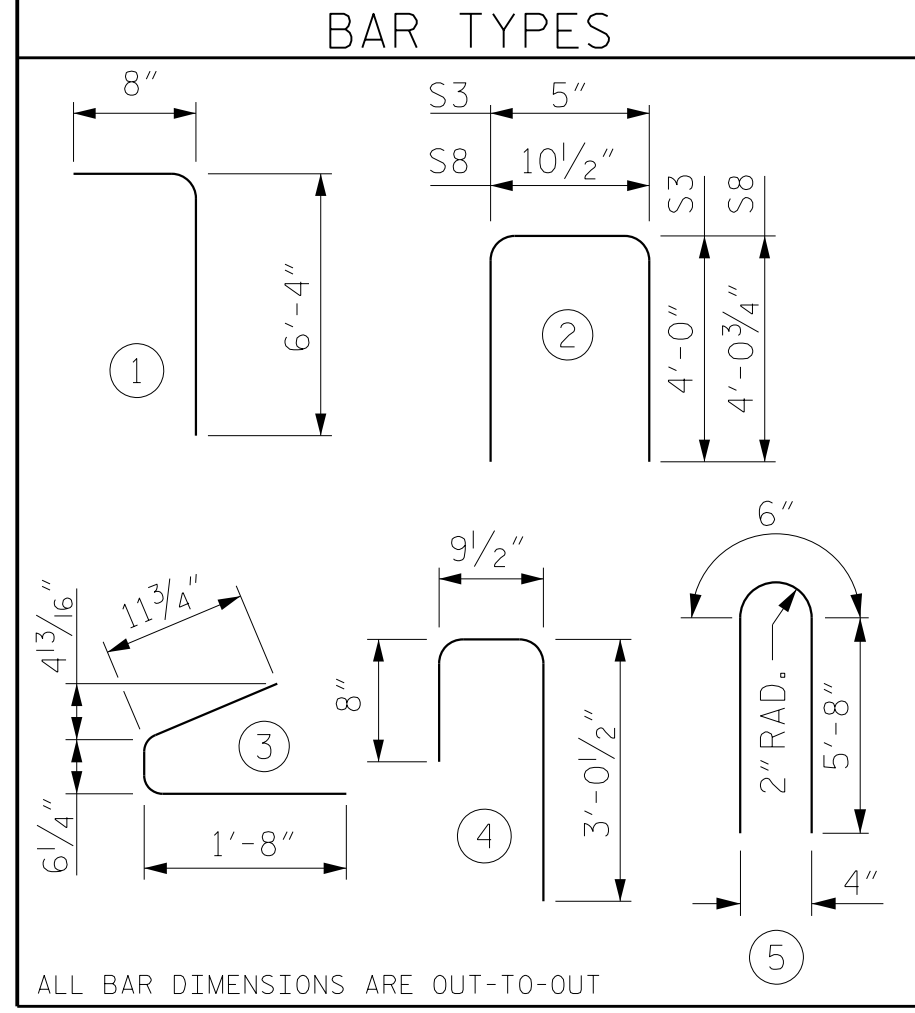
(S1 AND S9 BARS NOT SHOWN)



0.6" Ø L. R. GRADE 270 STRANDS		
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR						
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	
S1	278	#4	1	7'-0"	1300	
S2	44	#5	1	7'-0"	321	
S3	14	#4	2	8'-5"	79	
S4	116	#4	3	3'-2"	245	
S6	128	#5	4	4'-6"	601	
*S7	20	#5	STR	3'-8"	76	
S8	2	#5	2	9'-0"	19	
S9	135	#5	STR	3'-3"	458	
S10	2	#3	STR	1'-10"	1	
S11	8	#5	5	11'-10"	99	
S12	16	#4	STR	8'-0"	86	

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



QUANTITIES FOR ONE GIRDER			
REINFORCING STEEL	9000 PSI CONCRETE		0.6" Ø L.R. STRANDS
	LB.	C.Y.	No.
	3285	31.7	56

GIRDERS REQUIRED		
NUMBER	LENGTH	TOTAL LENGTH
5	139'-1 13/16"	695'-9 9/16"

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 2 OF 4

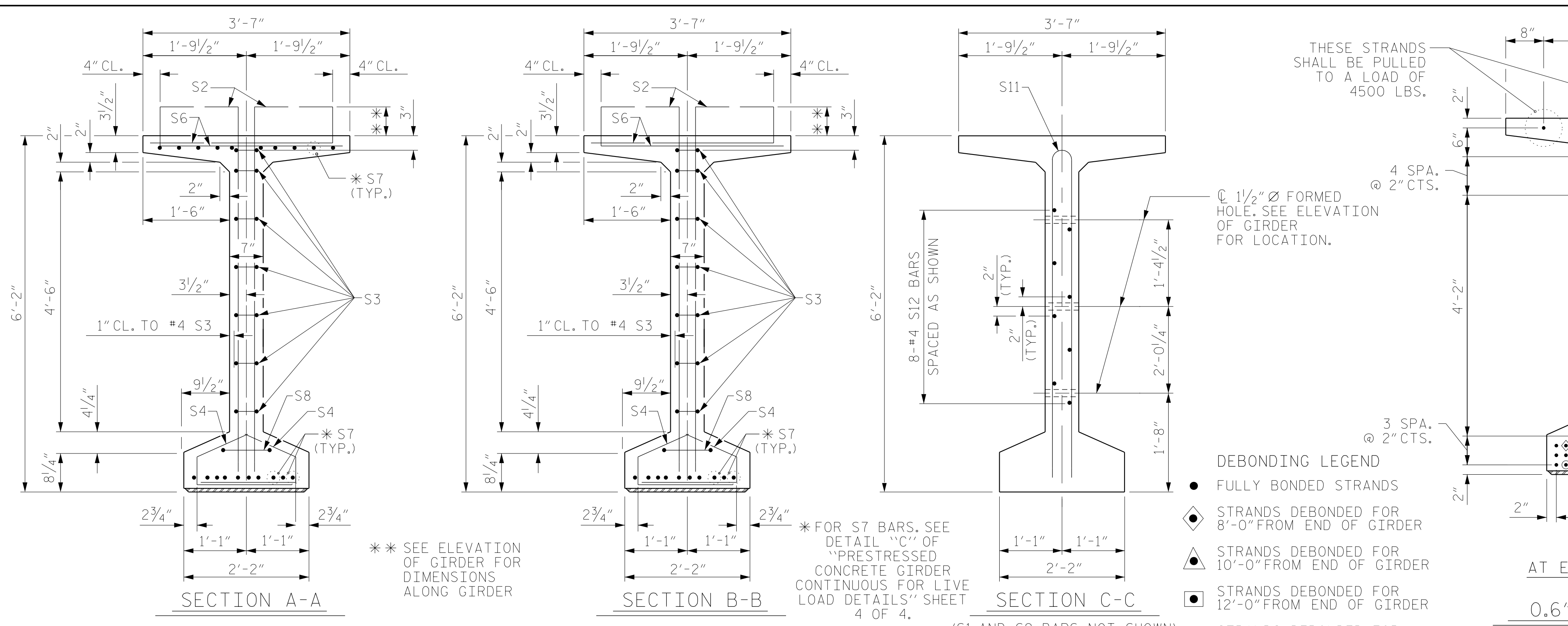


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 74" PRESTRESSED CONCRETE
 MODIFIED BULB TEE
 CONTINUOUS FOR LIVE LOAD
 SPAN B
 LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			S1-12
2			4			TOTAL SHEETS 36

DRAWN BY : MAL DATE : 11/2016
 CHECKED BY : JMR DATE : 11/2016
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

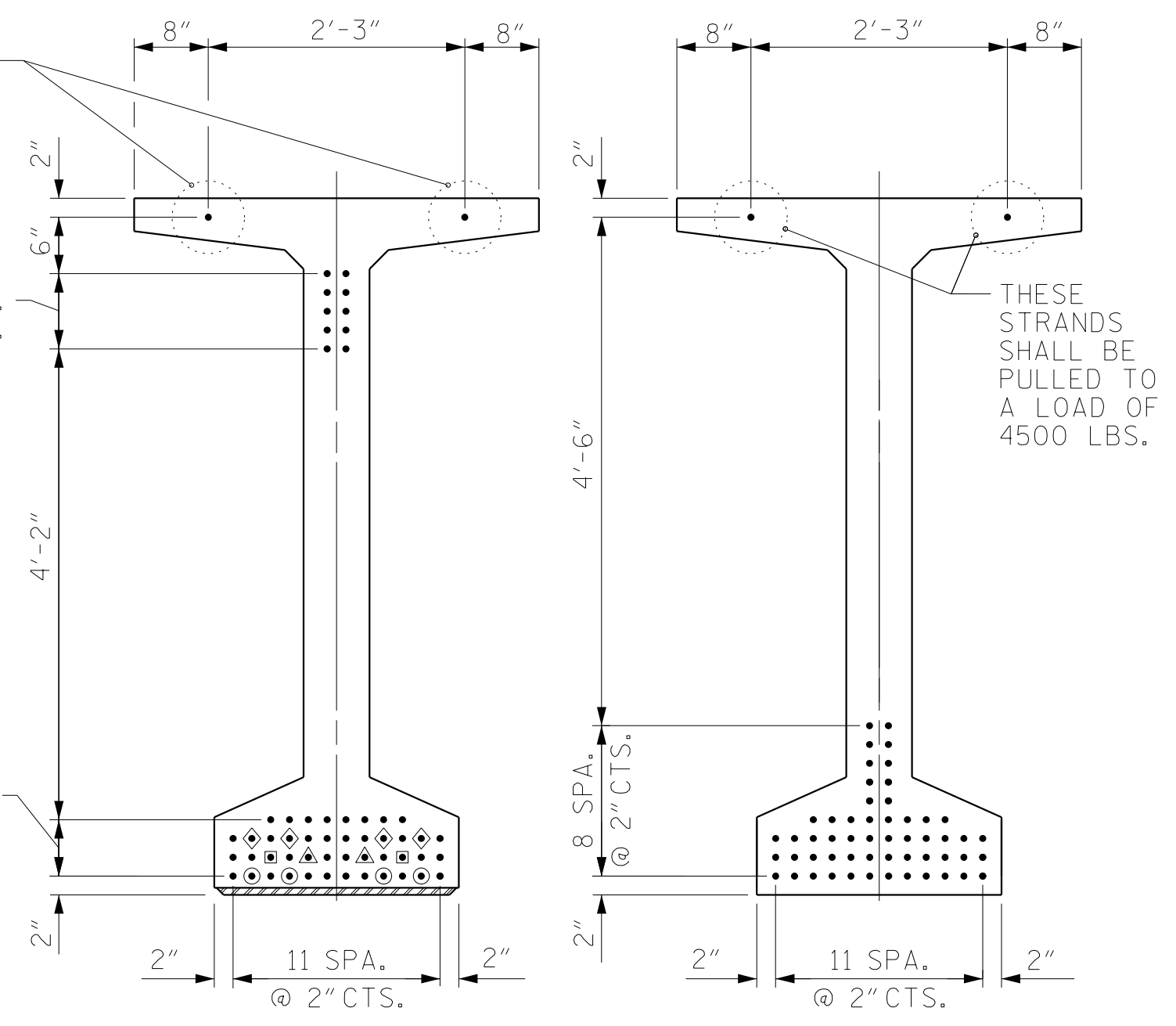
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THESE STRANDS SHALL BE PULLED TO A LOAD OF 4500 LBS.

1/2" Ø FORMED HOLE. SEE ELEVATION OF GIRDER FOR LOCATION.

- DEBONDING LEGEND**
- FULLY BONDED STRANDS
 - ◊ STRANDS DEBONDED FOR 8'-0" FROM END OF GIRDER
 - ▲ STRANDS DEBONDED FOR 10'-0" FROM END OF GIRDER
 - STRANDS DEBONDED FOR 12'-0" FROM END OF GIRDER
 - STRANDS DEBONDED FOR 14'-0" FROM END OF GIRDER

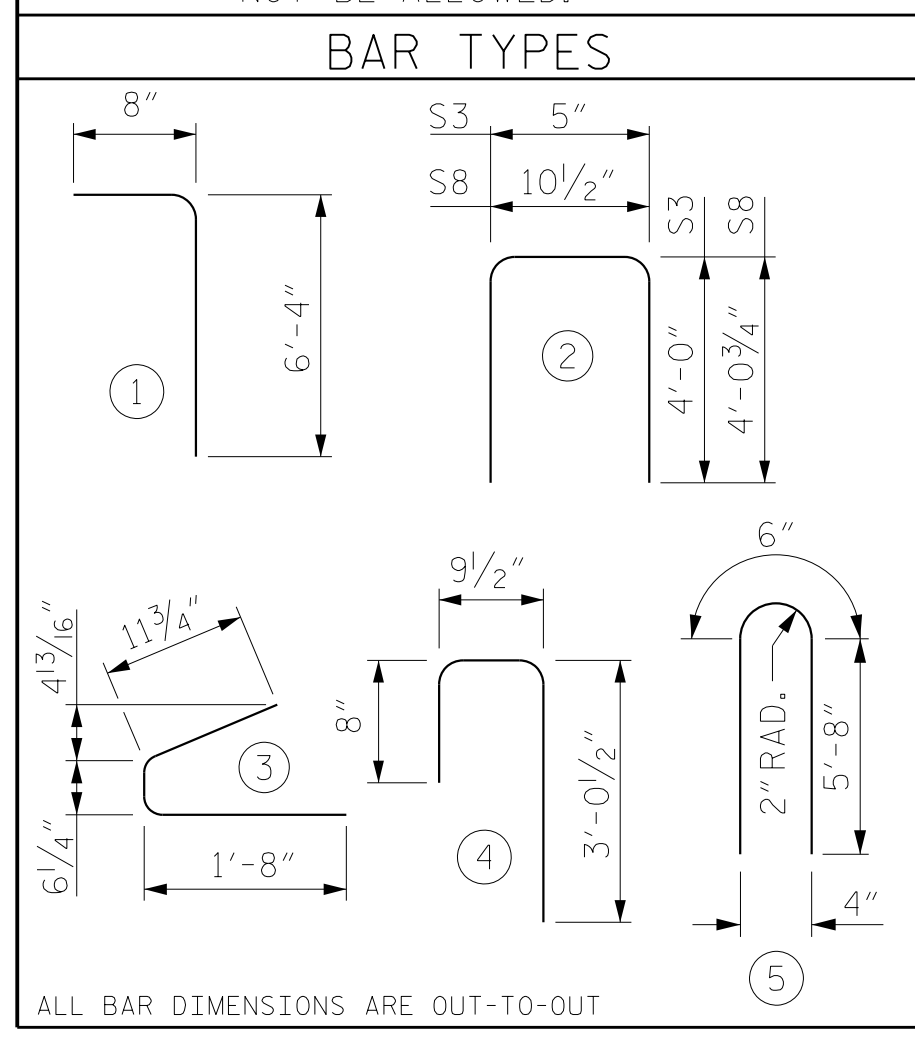


AT END OF GIRDER
AT C OF GIRDER
0.6" Ø LOW RELAXATION STRAND LAYOUT
THE UPLIFT FORCE DUE TO DRAPED STRANDS IS 27.4 KIPS (56 STRANDS REQUIRED)

0.6" Ø L. R. GRADE 270 STRANDS		
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	278	#4	1	7'-0"	1300
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S6	128	#5	4	4'-6"	601
*S7	30	#5	STR	3'-8"	115
S8	2	#5	2	9'-0"	19
S9	129	#5	STR	3'-3"	437
S10	2	#3	STR	1'-10"	1
S11	8	#5	5	11'-10"	99
S12	16	#4	STR	8'-0"	86

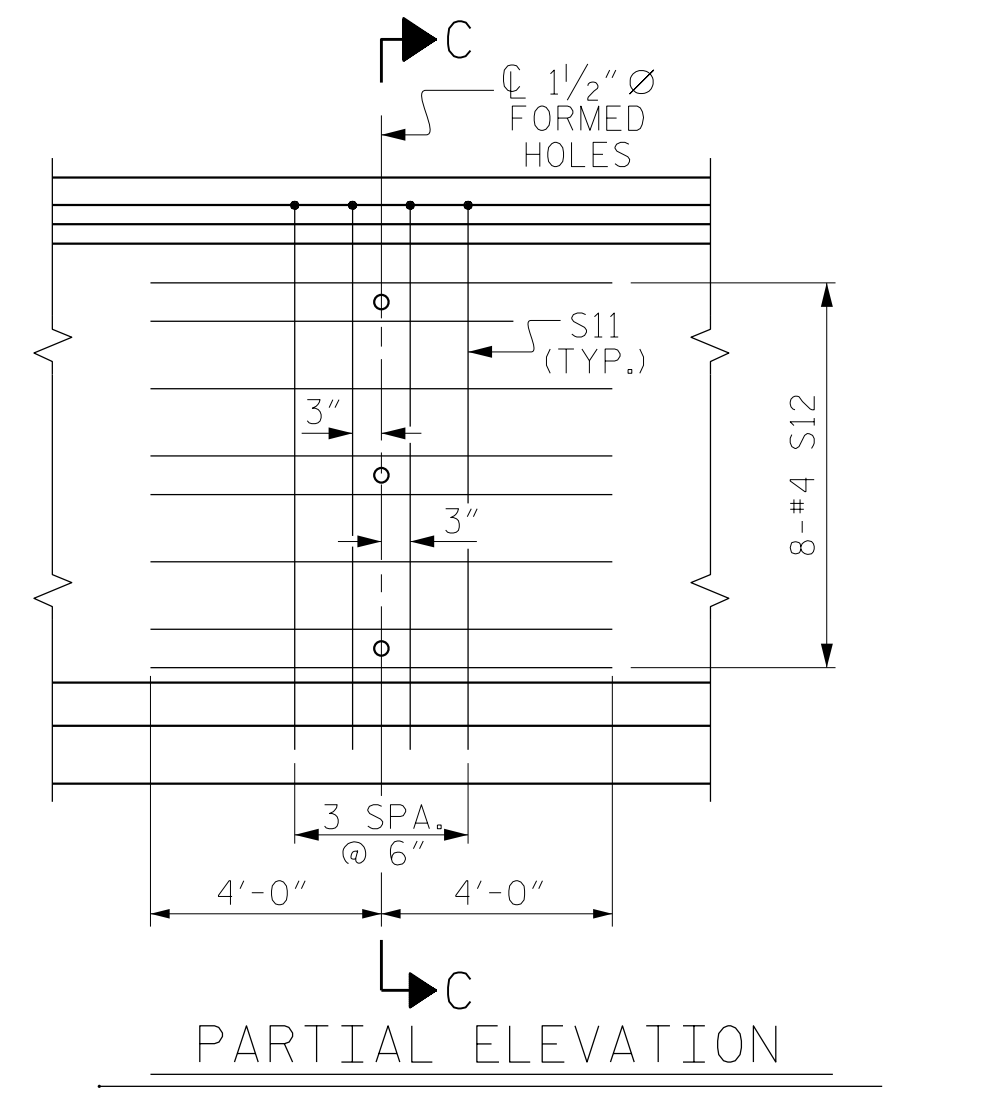
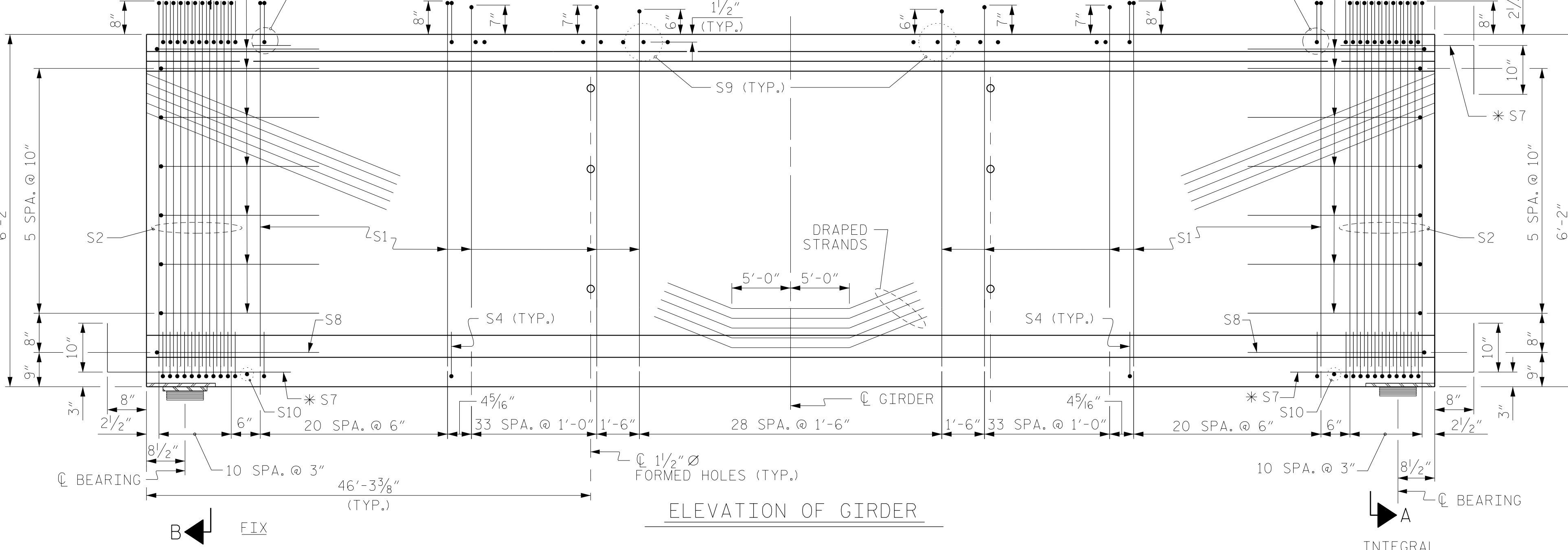
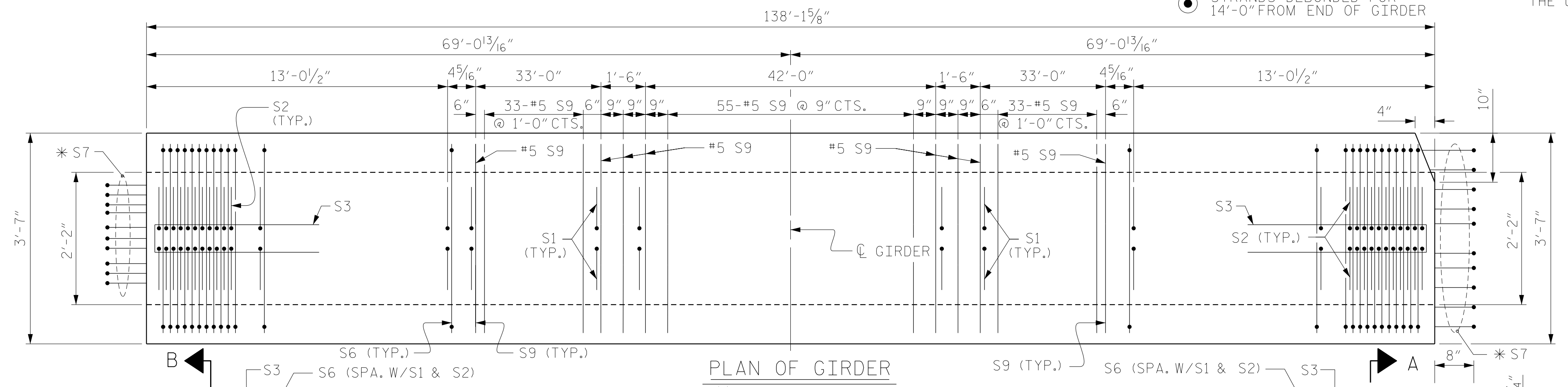
* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



ALL BAR DIMENSIONS ARE OUT-TO-OUT

QUANTITIES FOR ONE GIRDER			
REINFORCING STEEL	9000 PSI CONCRETE		0.6" Ø L.R. STRANDS
	LB.	C.Y.	No.
	3303	31.4	56

GIRDERS REQUIRED		
NUMBER	LENGTH	TOTAL LENGTH
5	138'-1 1/8"	690'-8 1/8"



SHOWING INTERMEDIATE STEEL DIAPHRAGM REINFORCING STEEL FOR GIRDER Nos. 1 THRU 5

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-
SHEET 3 OF 4



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
74" PRESTRESSED CONCRETE
MODIFIED BULB TEE
CONTINUOUS FOR LIVE LOAD
SPAN C
LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			S1-13
2			4			TOTAL SHEETS 36

DRAWN BY: MAL DATE: 11/2016
CHECKED BY: JMR DATE: 11/2016
DESIGN ENGINEER OF RECORD: MAL DATE: 11/2016

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NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. BEVEL EDGES OF PLATE "B-1" TO GIVE CLOSE FIT BUT NOT TIGHT FIT TO STEEL CASTING FORM.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 7200 PSI FOR SPAN A AND 7700 PSI FOR SPANS B AND C.

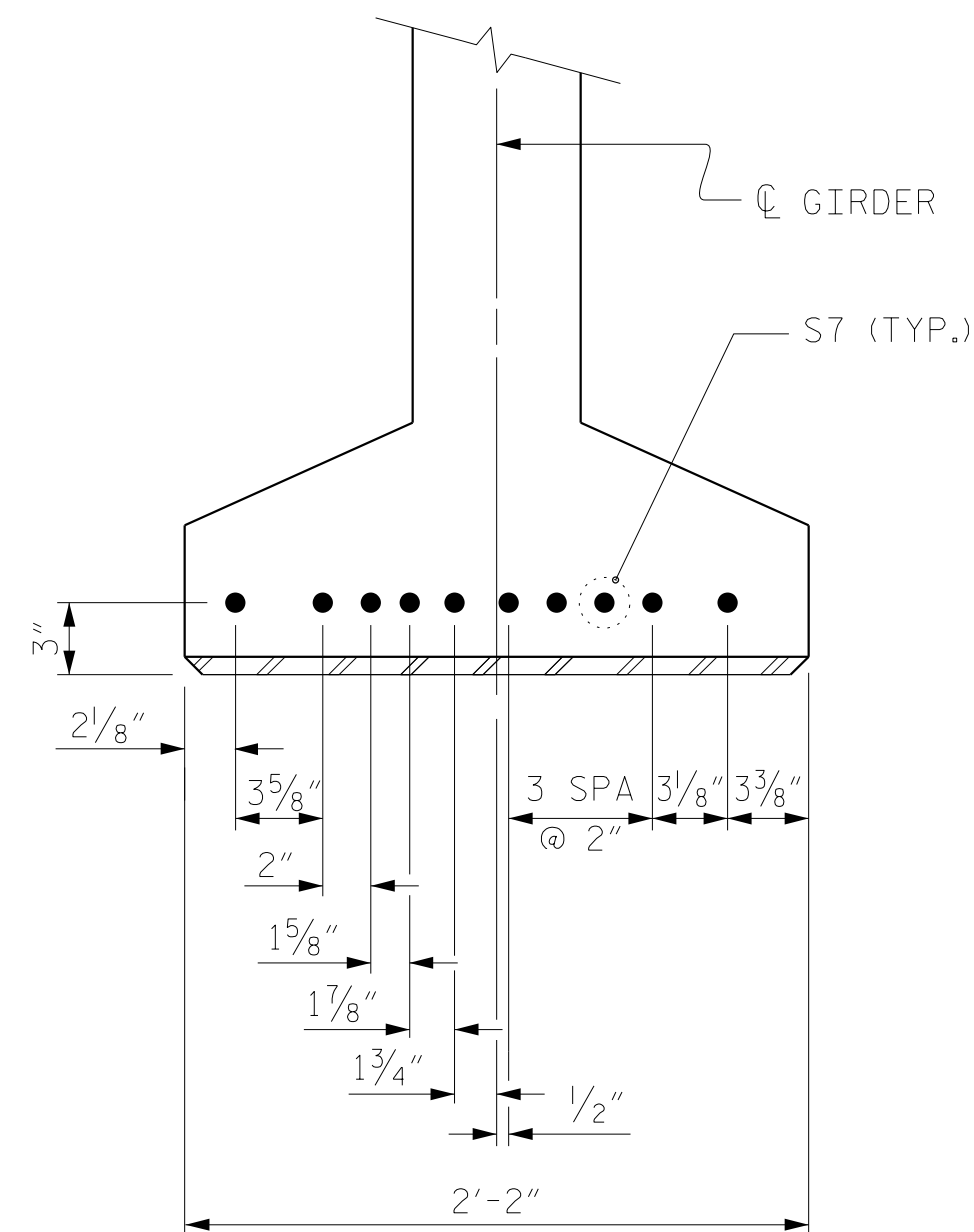
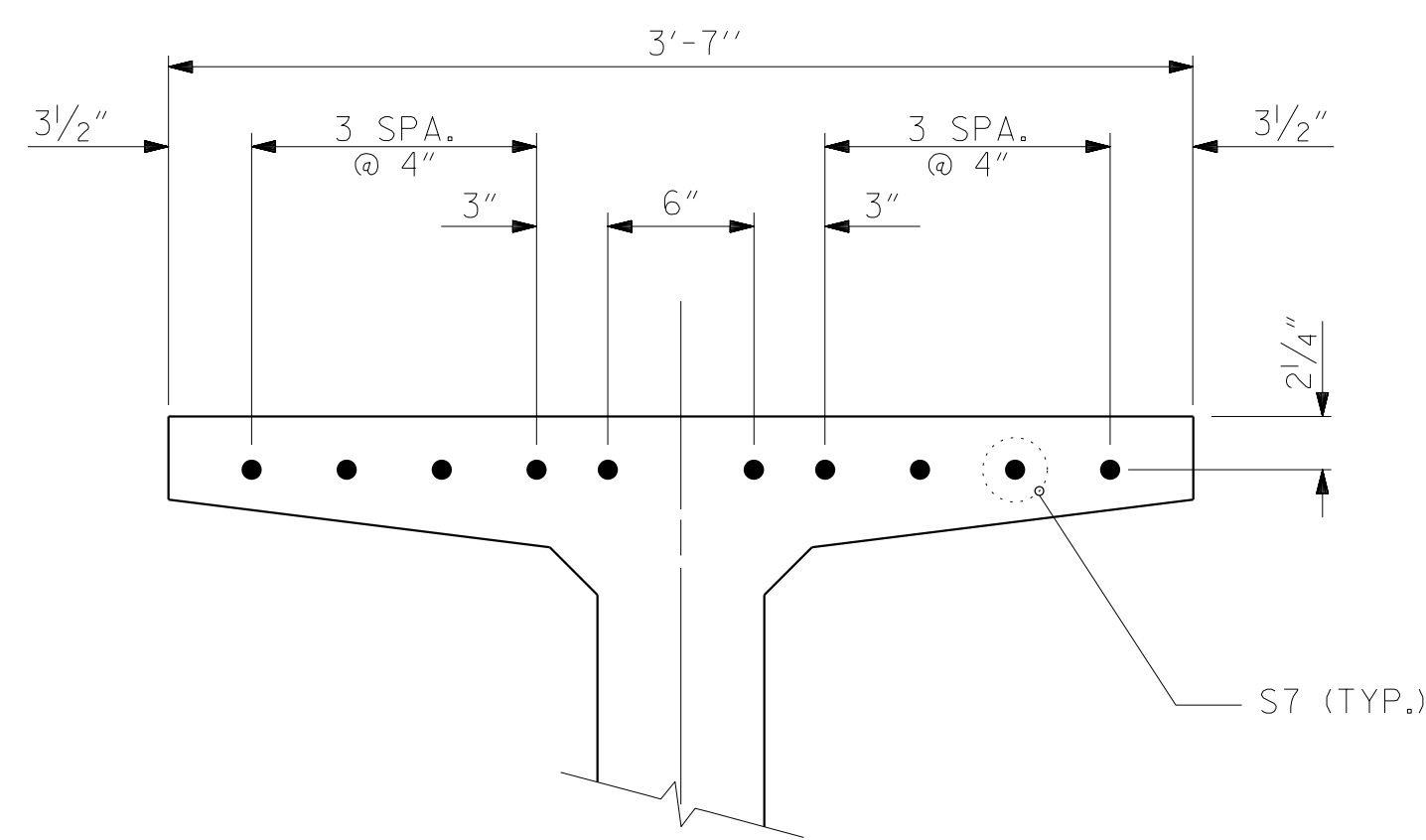
DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

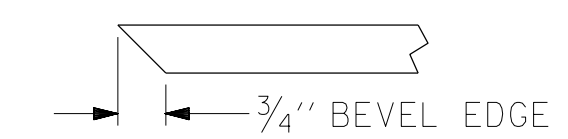
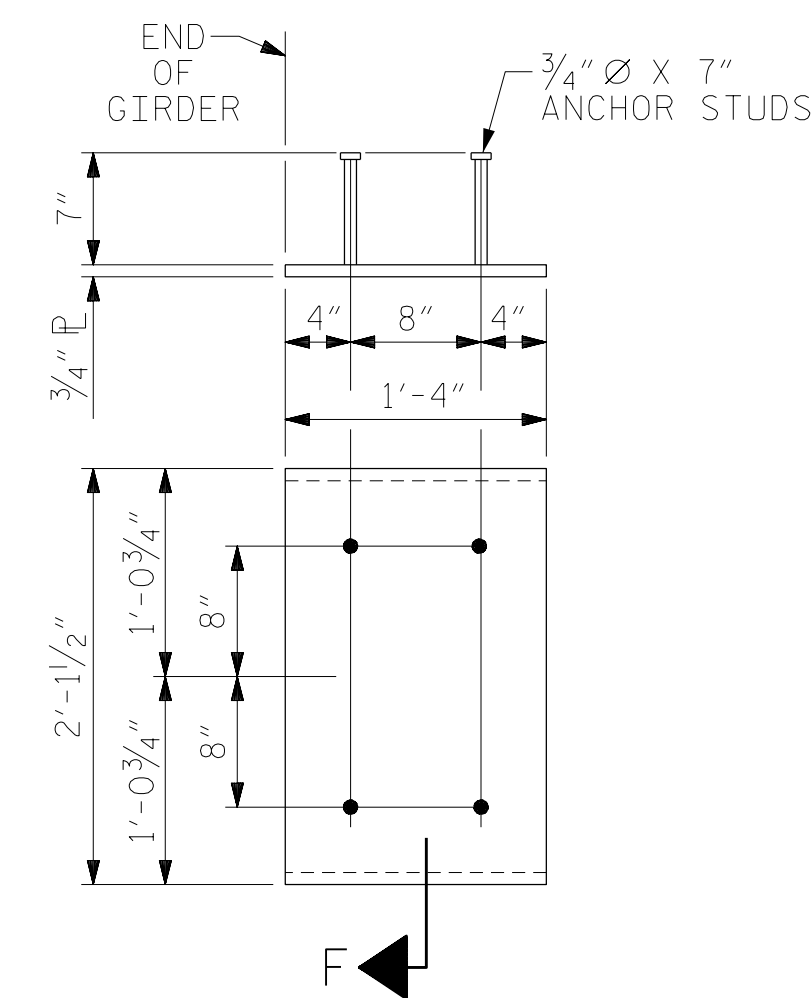
WHEN DRAPED STRANDS ARE DETAILED, THE LONGITUDINAL LOCATION OF THE HOLD DOWN DEVICES SHALL BE WITHIN 6" OF THE LOCATION SHOWN AND THE CENTER OF GRAVITY OF THE GROUP OF DRAPED STRANDS SHALL BE LOCATED WITHIN 1/2" OF THE THEORETICAL LOCATION SHOWN.

A 2" x 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 74" MODIFIED PRESTRESSED CONCRETE GIRDER.

FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.



DETAIL "C"



SECTION "F"
(SEE NOTES)

EMBEDDED PLATE "B-1" DETAILS FOR 74" MODIFIED PRESTRESSED CONCRETE GIRDER
(2 REQ'D PER GIRDER)

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

SHEET 4 OF 4

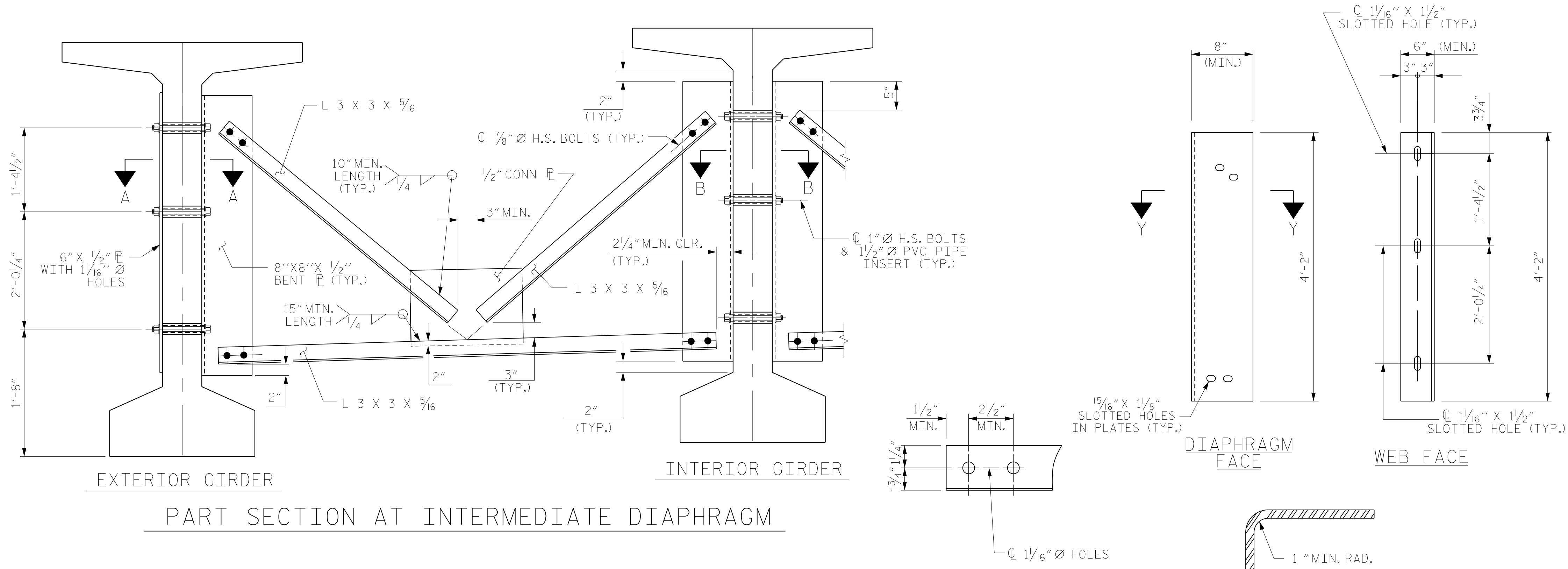
ASSEMBLED BY :	MAL	DATE :	11/2016
CHECKED BY :	JMR	DATE :	11/2016
DRAWN BY :	ELR 11/91	REV. 10/1/11	MAA/GM
CHECKED BY :	GRP 11/91	REV. 1/15	MAA/TMG
		REV. 2/15	MAA/TMG

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

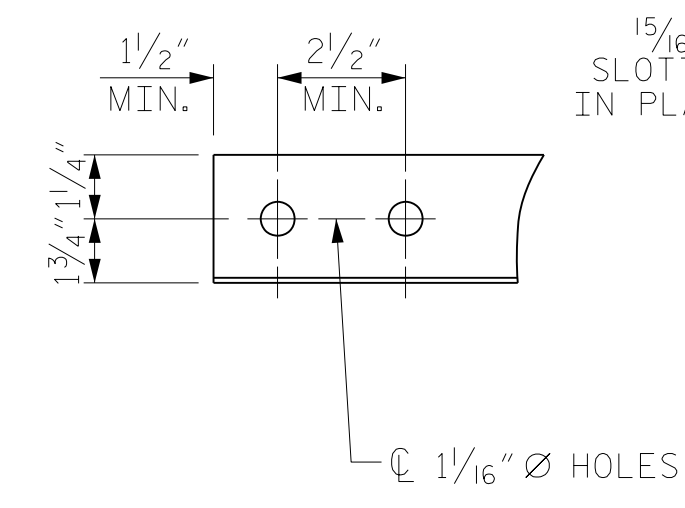
RS&H
RS&H Architects-Engineers-Planners, Inc.
8601 Six Forks Road, Suite 260
919-926-4100 FAX 919-846-9080
www.rsandh.com
North Carolina License No. 50737-F-0403-C-03

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
PRESTRESSED CONCRETE GIRDER
CONTINUOUS FOR LIVE LOAD
DETAILS
LEFT LANE

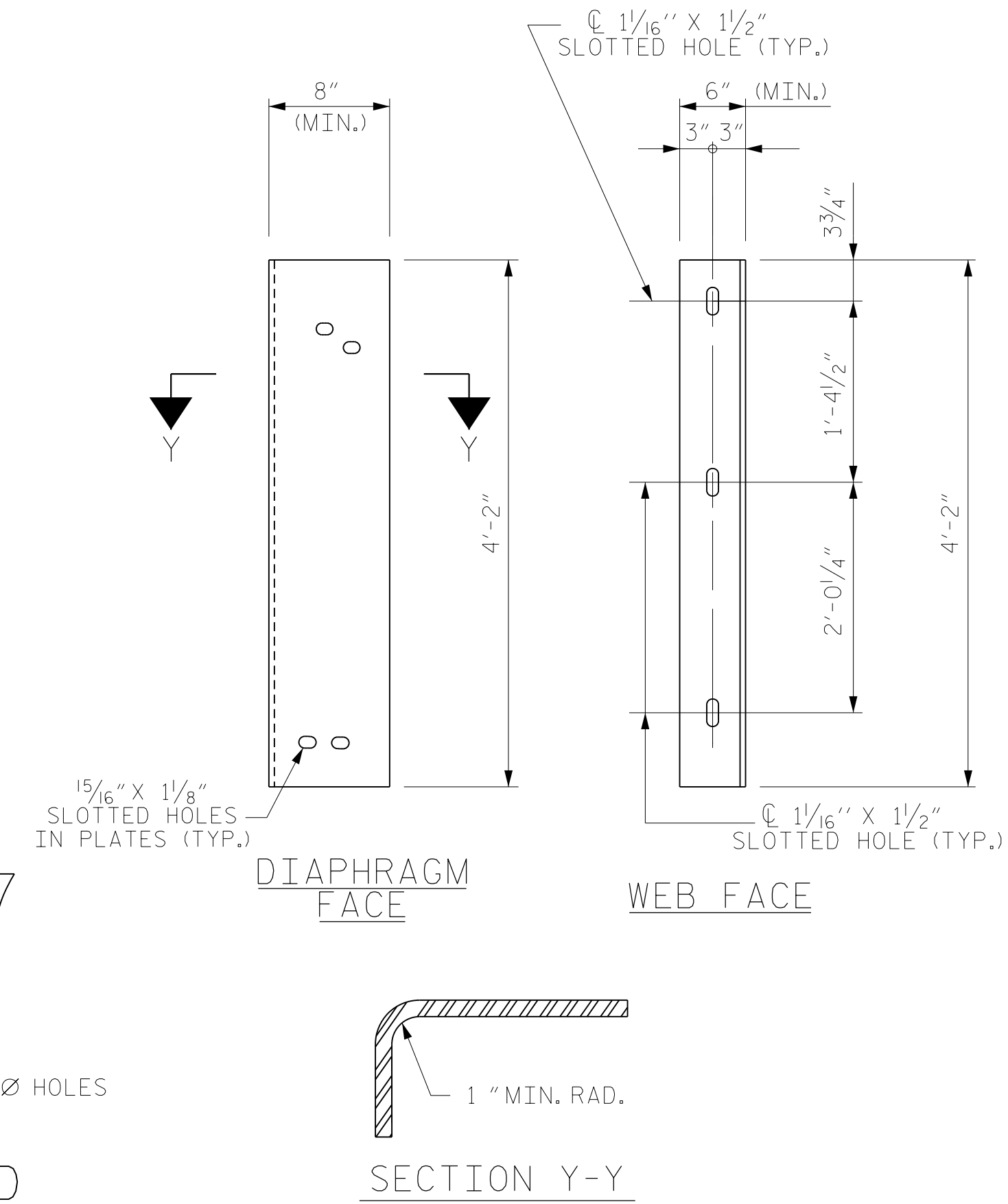
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-14
1			3			TOTAL SHEETS
2			4			36



PART SECTION AT INTERMEDIATE DIAPHRAGM



ANGLE END
(L 3 x 3 x 5/16)



CONNECTOR PLATE DETAIL

STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

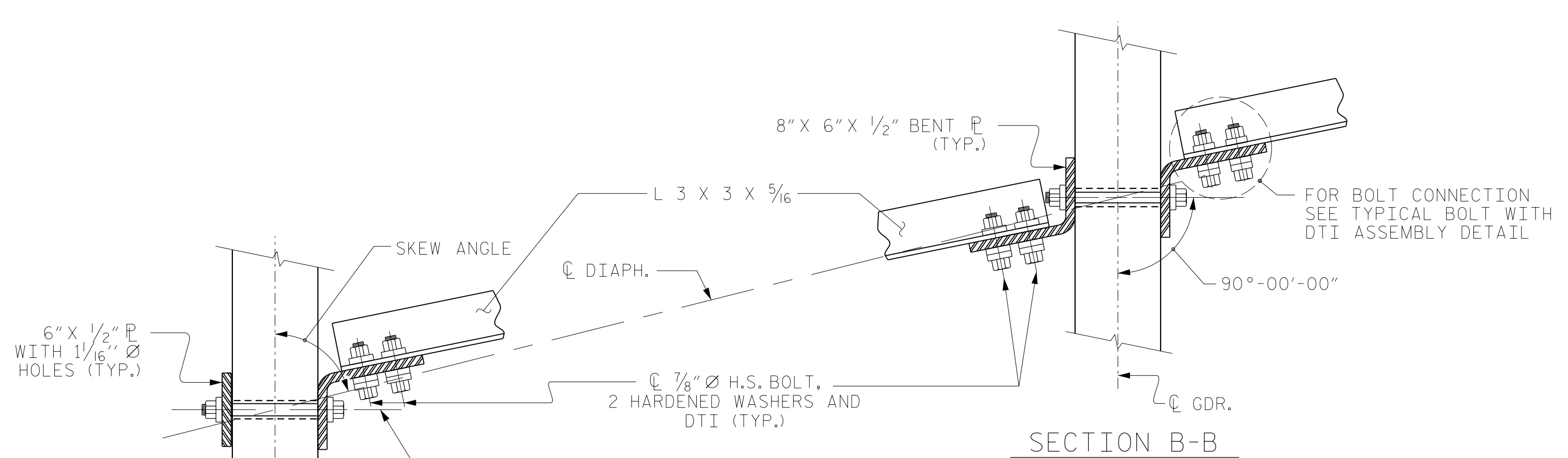
FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

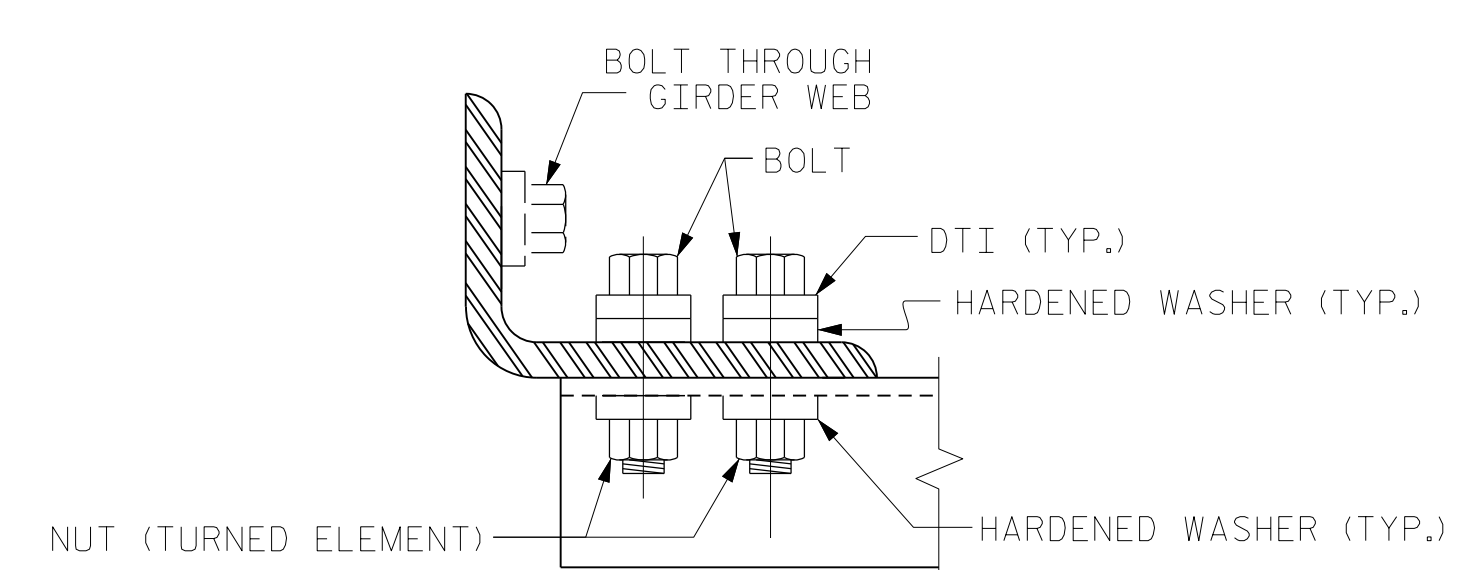
SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.



CONNECTION DETAILS



BOLT WITH DTI ASSEMBLY DETAIL

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

Professional Engineer Seal for Mark Robinson, State of North Carolina, License No. 50973, dated 3/9/2017.

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
INTERMEDIATE
STEEL DIAPHRAGMS
LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-15
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : MAL	DATE : 11/2016
CHECKED BY : JMR	DATE : 11/2016
DESIGN ENGINEER OF RECORD: MAL	DATE : 11/2016

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NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

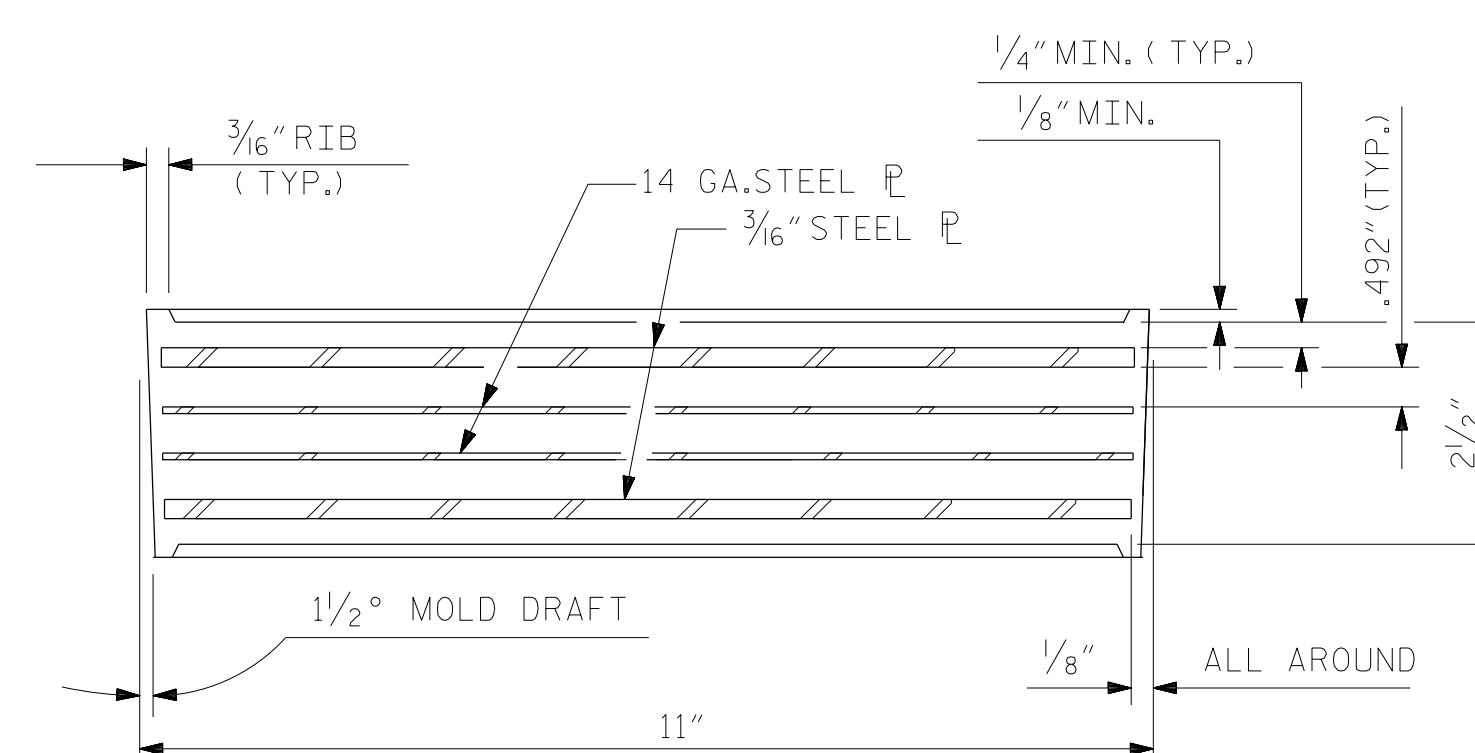
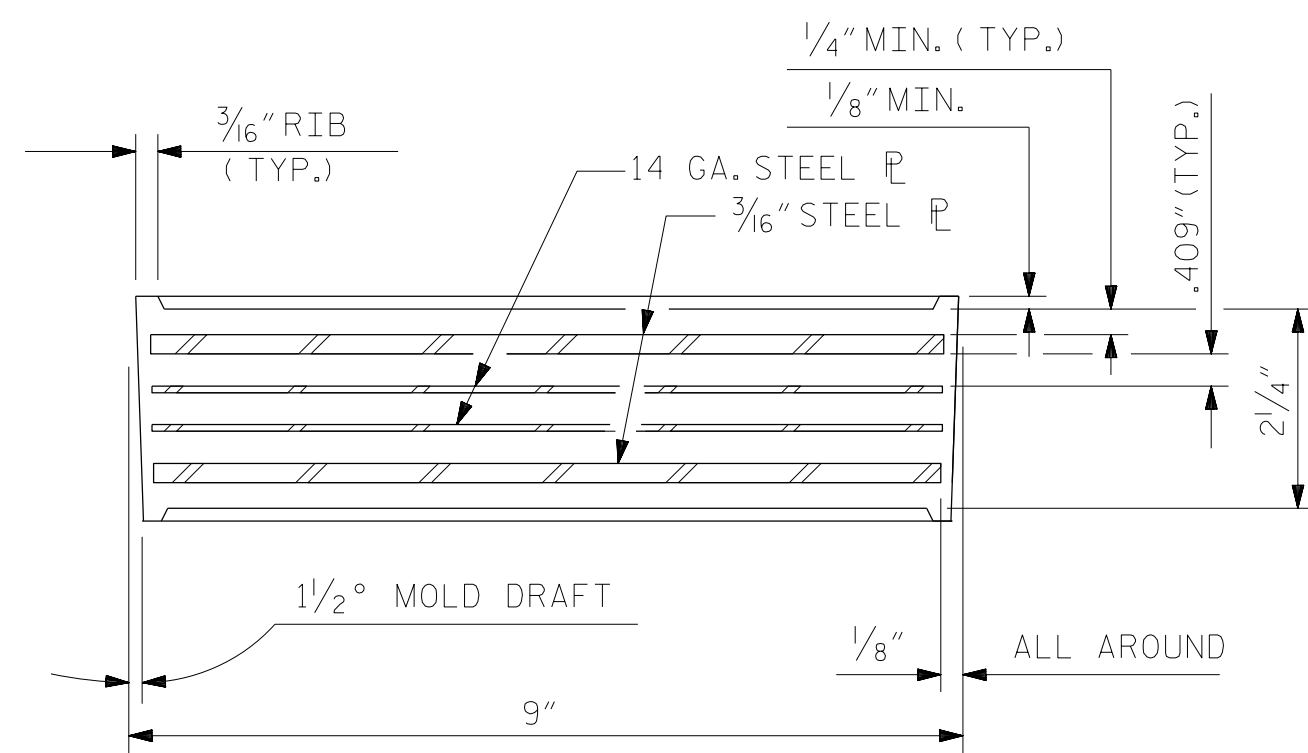
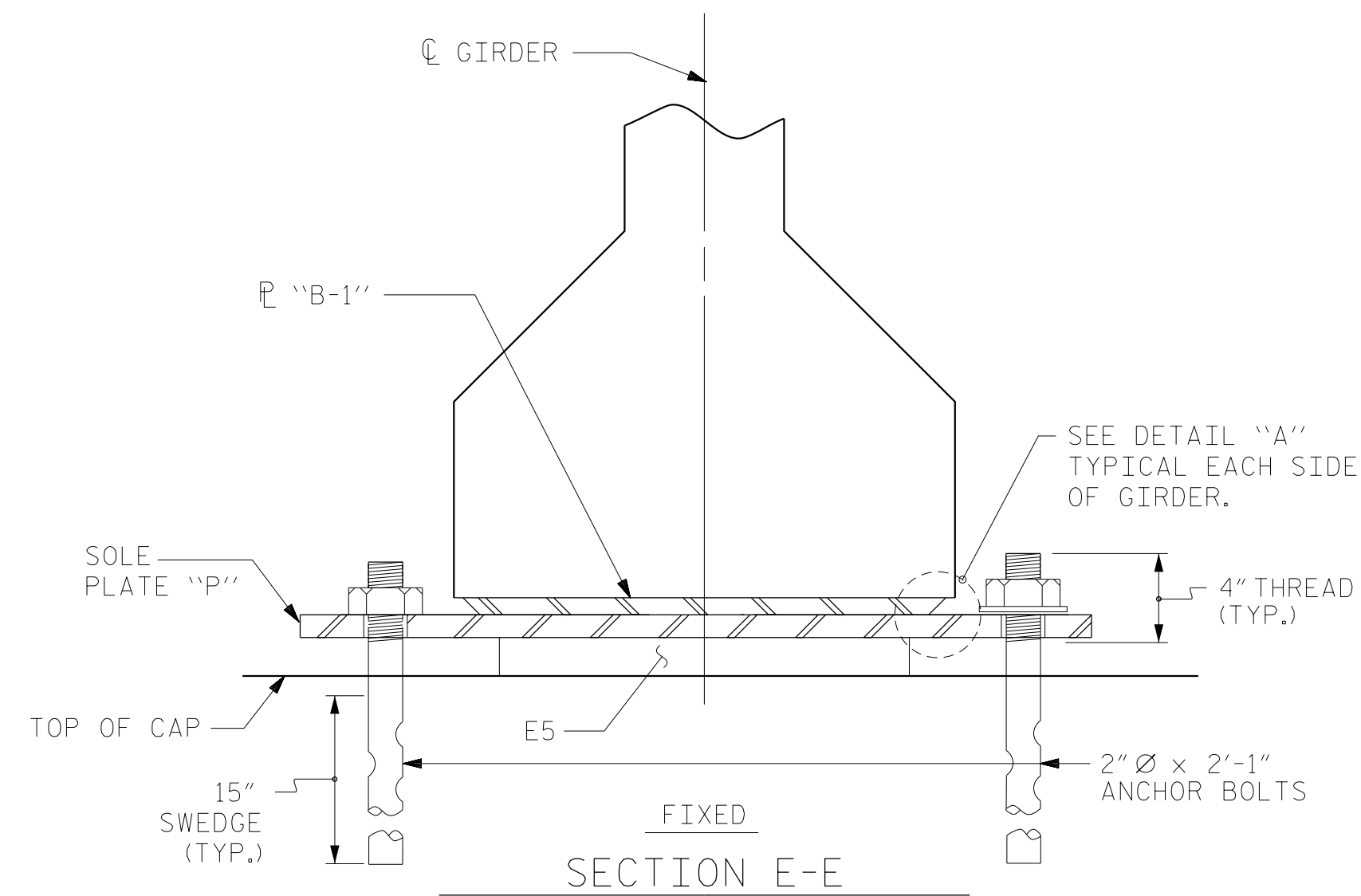
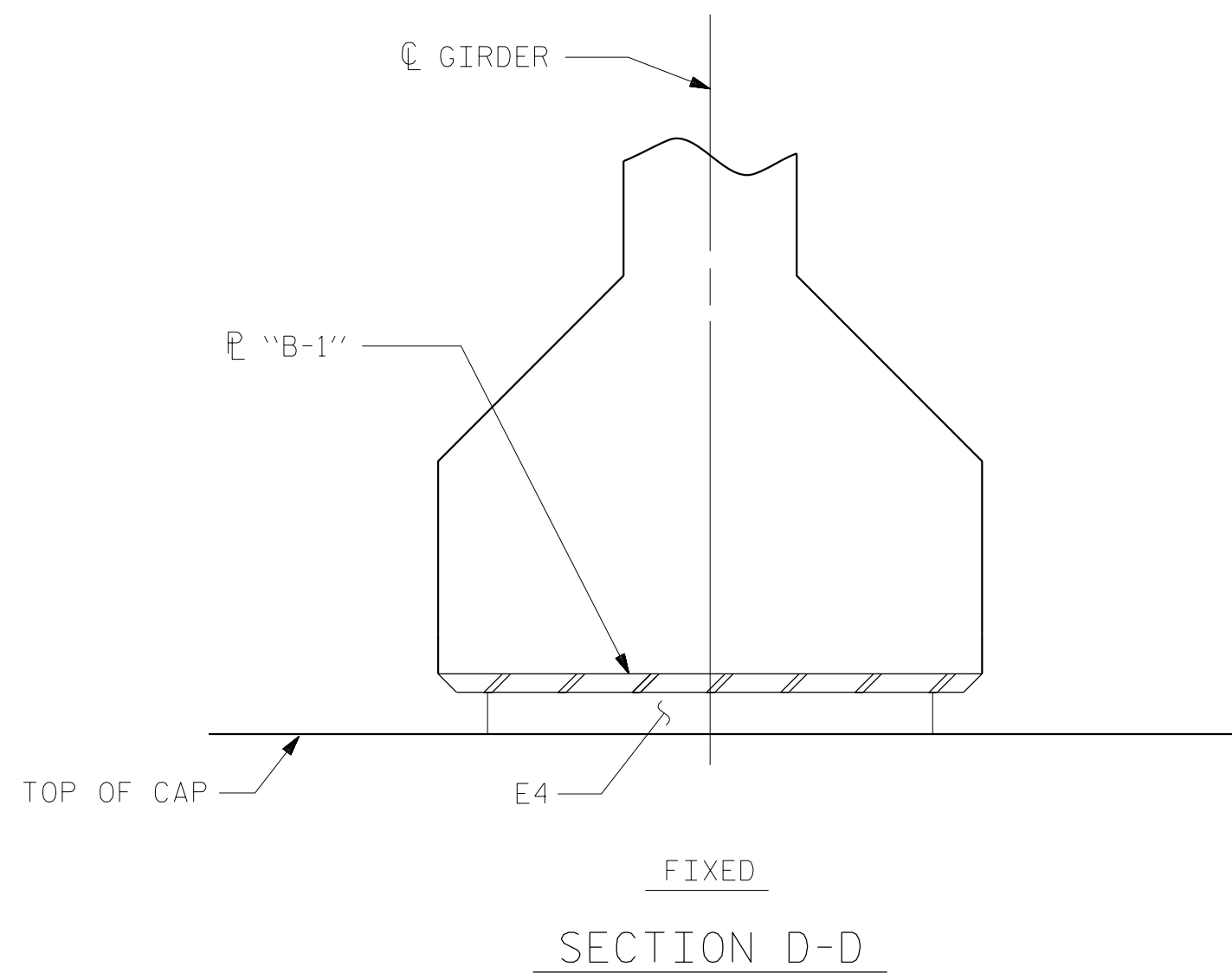
ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. NO SHOP DRAWINGS ARE REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

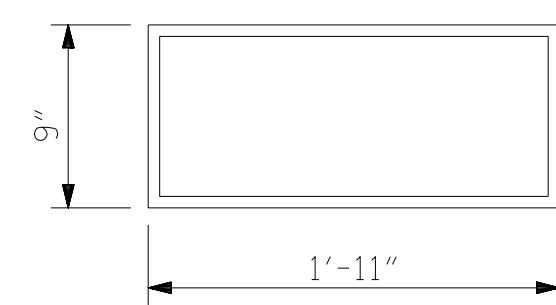
FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

ALL BEARING PLATES SHALL BE AASHTO M270 GRADE 36.



TYPICAL SECTION OF ELASTOMERIC BEARINGS

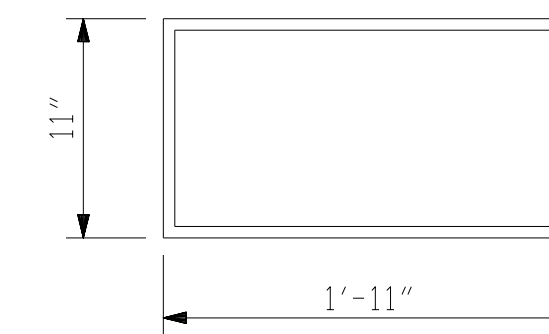
TYPICAL SECTION OF ELASTOMERIC BEARINGS



E4 (10 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

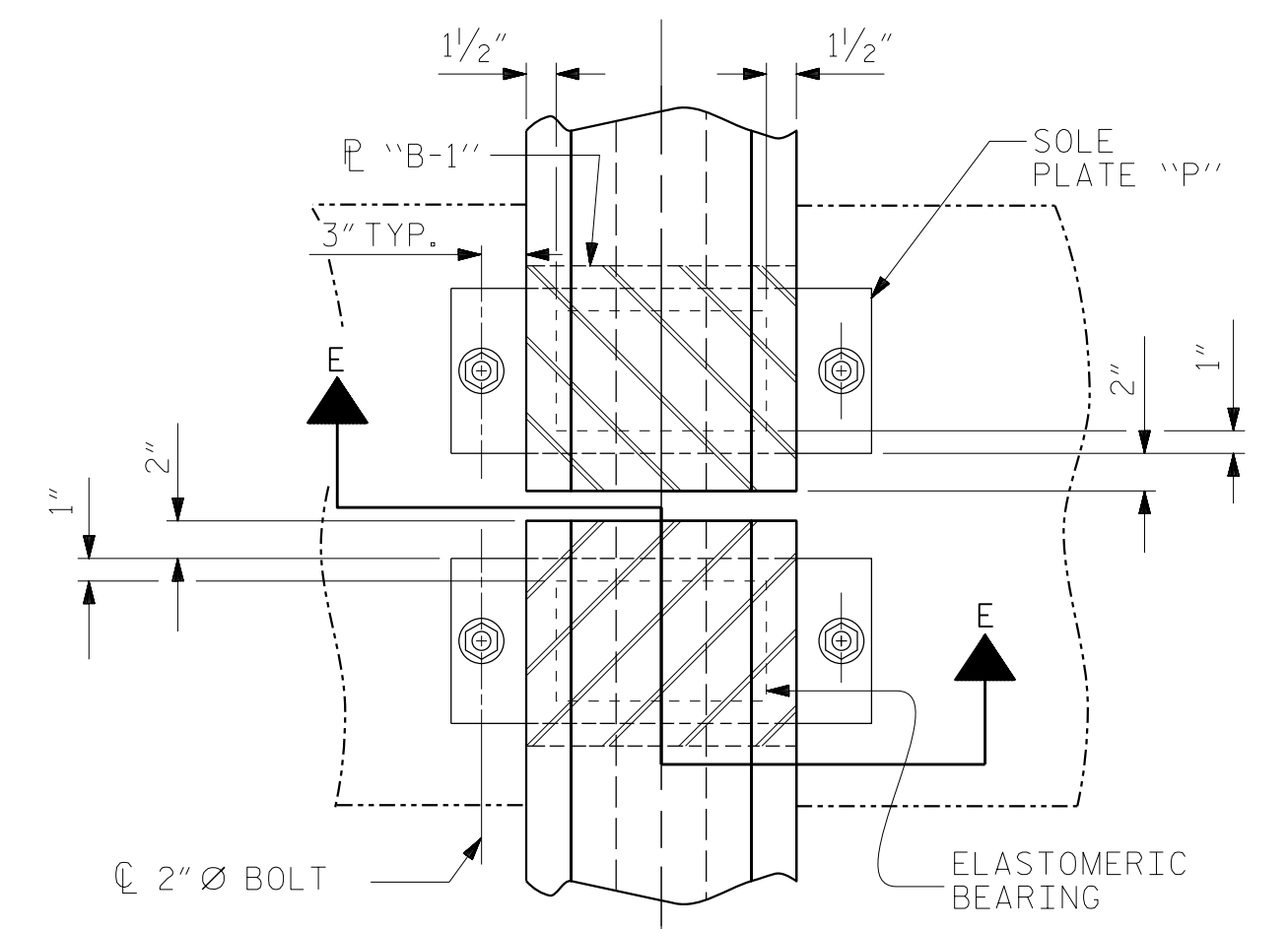
TYPE V



E5 (20 REQ'D)

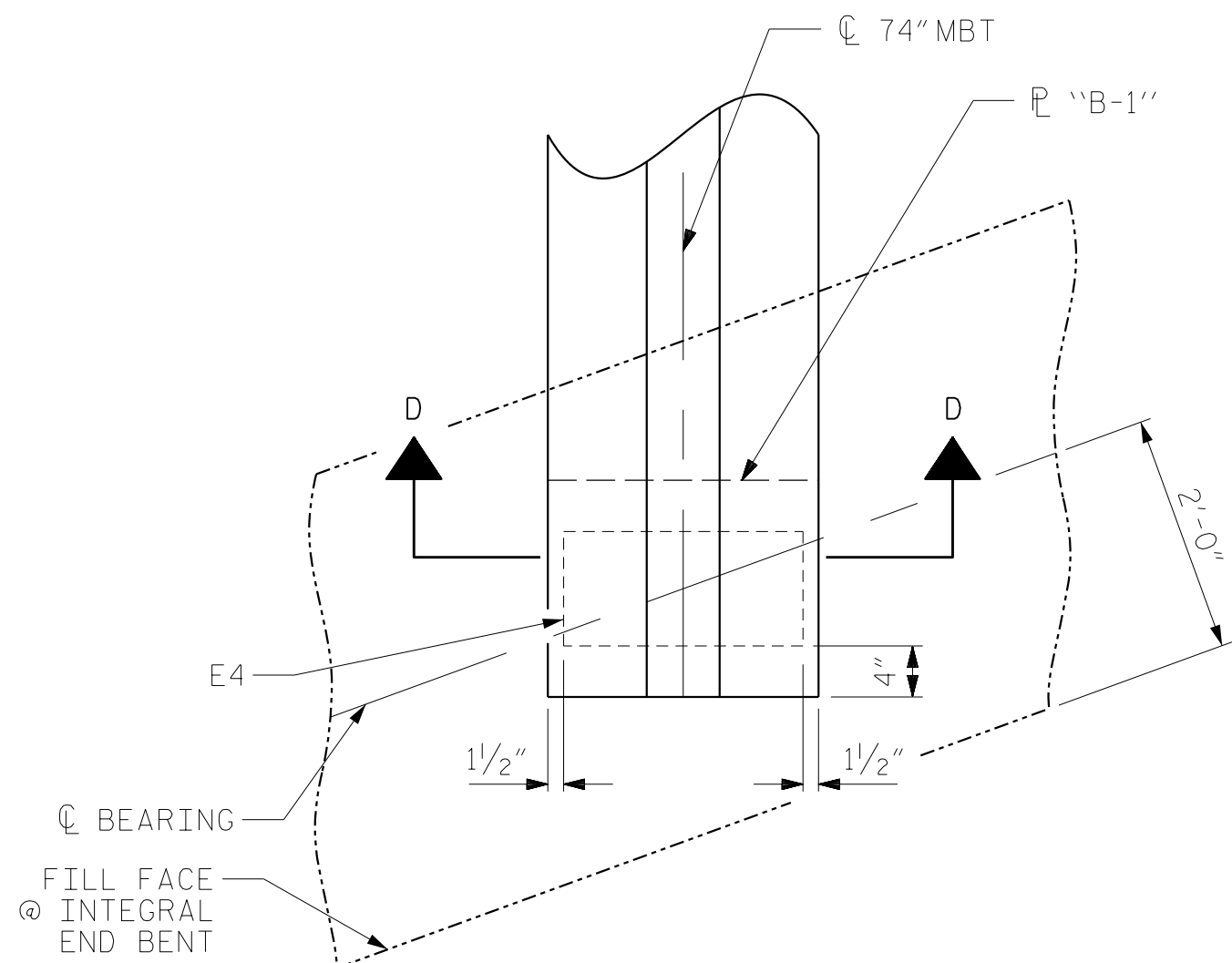
PLAN VIEW OF ELASTOMERIC BEARING

TYPE VI



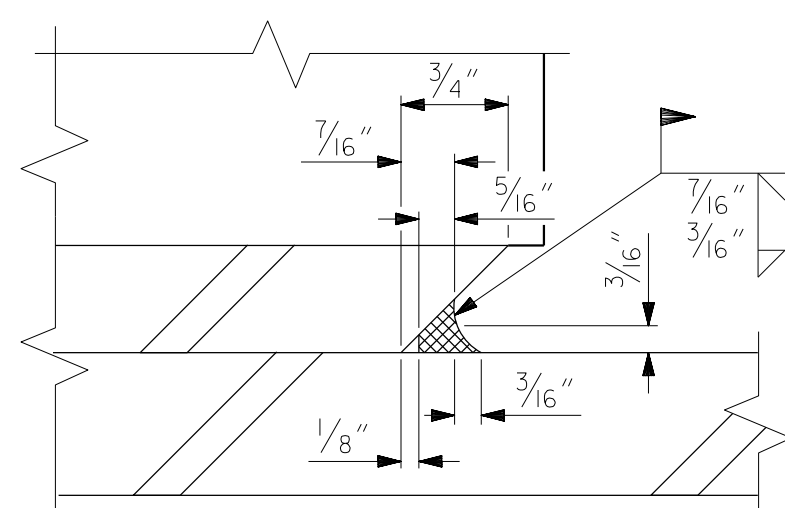
TYPICAL PLAN

(SHOWING CONTINUOUS BENT)

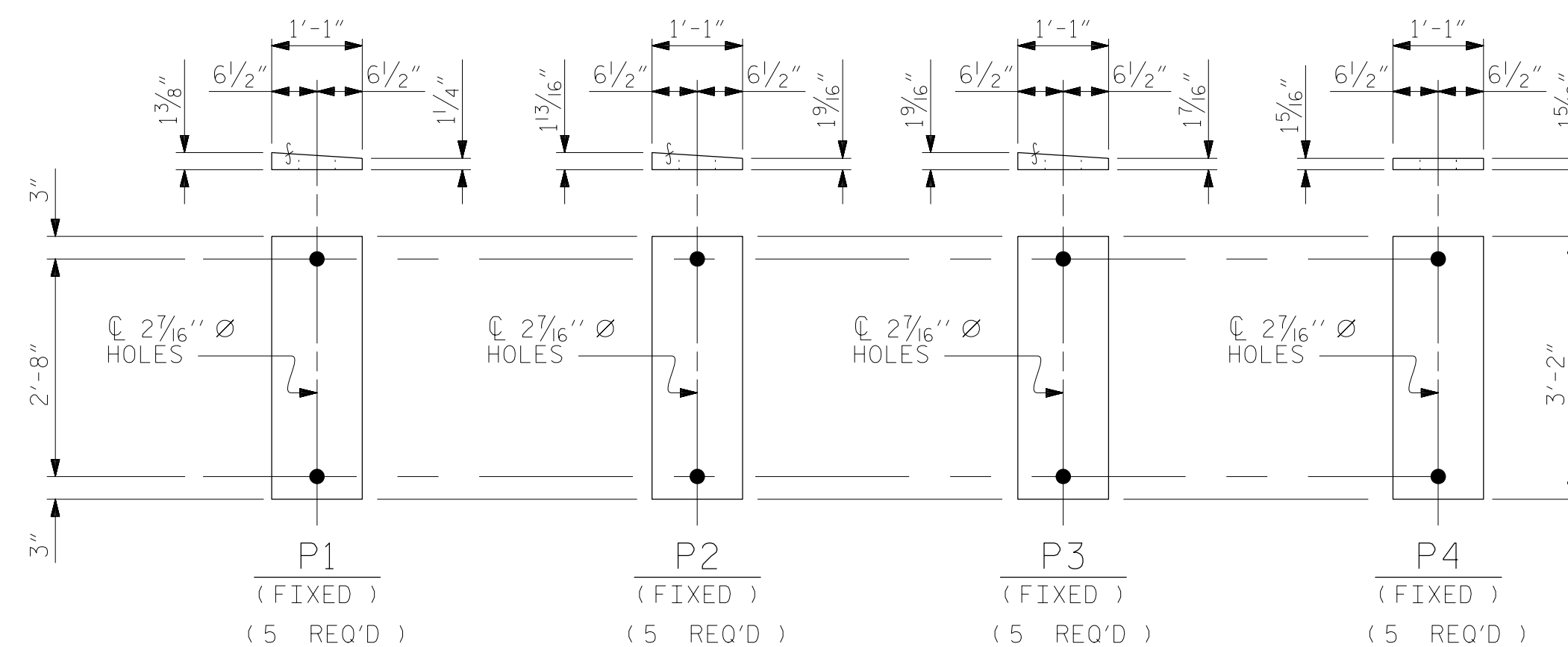


PLAN VIEW AT END BENTS

NOTE: BOTTOM FLANGE OF 74" MBT SHOWN, TOP FLANGE NOT SHOWN FOR CLARITY



DETAIL "A"



SOLE PLATE DETAILS ("P")

MAXIMUM ALLOWABLE SERVICE LOADS	
D.L.+L.L. (NO IMPACT)	
TYPE V	365 k
TYPE VI	420 k

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-



RS&H Architects-Engineers-Planners, Inc.
 8601 Six Forks Road, Suite 260
 919-926-4100 FAX 919-846-9080
 www.rsandh.com
 North Carolina License No. 30973-F-0403-C-03

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 ELASTOMERIC BEARING
 DETAILS
 LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-16
1			3			TOTAL SHEETS
2			4			36

DRAWN BY: MAL DATE: 11/2016
 CHECKED BY: JMR DATE: 12/2016
 DESIGN ENGINEER OF RECORD: MAL DATE: 11/2016

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DEAD LOAD DEFLECTION TABLE FOR GIRDERS

0.6" Ø LOW RELAXATION	SPAN A																				
	GIRDER 1 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.052	0.102	0.149	0.193	0.231	0.264	0.290	0.309	0.320	0.324	0.320	0.309	0.290	0.264	0.231	0.193	0.149	0.102	0.052	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.026	0.052	0.076	0.098	0.118	0.134	0.148	0.157	0.163	0.165	0.163	0.157	0.148	0.134	0.118	0.098	0.076	0.052	0.026	0.000
FINAL CAMBER ↑	0"	5/16"	5/8"	7/8"	1 1/8"	1 3/8"	1 9/16"	1 11/16"	1 13/16"	1 7/8"	1 15/16"	1 7/8"	1 13/16"	1 11/16"	1 9/16"	1 3/8"	1 1/8"	7/8"	5/8"	5/16"	0"
0.6" Ø LOW RELAXATION	GIRDER 2 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.052	0.102	0.149	0.193	0.231	0.264	0.290	0.309	0.320	0.324	0.320	0.309	0.290	0.264	0.231	0.193	0.149	0.102	0.052	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.029	0.057	0.084	0.109	0.130	0.149	0.163	0.174	0.181	0.183	0.181	0.174	0.163	0.149	0.130	0.109	0.084	0.057	0.029	0.000
FINAL CAMBER ↑	0"	1/4"	9/16"	13/16"	1"	1 3/16"	1 3/8"	1 1/2"	1 5/8"	1 11/16"	1 11/16"	1 11/16"	1 5/8"	1 1/2"	1 3/8"	1 3/16"	1"	13/16"	9/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 3 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.052	0.102	0.149	0.193	0.231	0.264	0.290	0.309	0.320	0.324	0.320	0.309	0.290	0.264	0.231	0.193	0.149	0.102	0.052	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.029	0.057	0.084	0.109	0.130	0.149	0.164	0.174	0.181	0.183	0.181	0.174	0.164	0.149	0.130	0.109	0.084	0.057	0.029	0.000
FINAL CAMBER ↑	0"	1/4"	9/16"	3/4"	1"	1 3/16"	1 3/8"	1 1/2"	1 5/8"	1 11/16"	1 11/16"	1 11/16"	1 5/8"	1 1/2"	1 3/8"	1 3/16"	1"	3/4"	9/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 4 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.052	0.102	0.149	0.193	0.231	0.264	0.290	0.309	0.320	0.324	0.320	0.309	0.290	0.264	0.231	0.193	0.149	0.102	0.052	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.029	0.057	0.084	0.109	0.130	0.149	0.164	0.174	0.181	0.183	0.181	0.174	0.164	0.149	0.130	0.109	0.084	0.057	0.029	0.000
FINAL CAMBER ↑	0"	1/4"	9/16"	3/4"	1"	1 3/16"	1 3/8"	1 1/2"	1 5/8"	1 11/16"	1 11/16"	1 11/16"	1 5/8"	1 1/2"	1 3/8"	1 3/16"	1"	3/4"	9/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 5 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.052	0.102	0.149	0.193	0.231	0.264	0.290	0.309	0.320	0.324	0.320	0.309	0.290	0.264	0.231	0.193	0.149	0.102	0.052	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.025	0.049	0.071	0.092	0.111	0.126	0.139	0.148	0.153	0.155	0.153	0.148	0.139	0.126	0.111	0.092	0.071	0.049	0.025	0.000
FINAL CAMBER ↑	0"	5/16"	5/8"	15/16"	1 3/16"	1 7/16"	1 5/8"	1 13/16"	1 15/16"	2"	2"	2"	1 15/16"	1 13/16"	1 5/8"	1 7/16"	1 3/16"	15/16"	5/8"	5/16"	0"
0.6" Ø LOW RELAXATION	SPAN B																				
	GIRDER 1 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.063	0.124	0.181	0.234	0.281	0.321	0.353	0.375	0.390	0.394	0.390	0.375	0.353	0.321	0.281	0.234	0.181	0.124	0.063	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.036	0.072	0.105	0.136	0.163	0.186	0.205	0.218	0.226	0.229	0.226	0.218	0.205	0.186	0.163	0.136	0.105	0.072	0.036	0.000
FINAL CAMBER ↑	0"	5/16"	5/8"	15/16"	1 3/16"	1 7/16"	1 5/8"	1 3/4"	1 7/8"	1 15/16"	2"	1 15/16"	1 7/8"	1 3/4"	1 5/8"	1 7/16"	1 3/16"	15/16"	5/8"	5/16"	0"
0.6" Ø LOW RELAXATION	GIRDER 2 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.063	0.124	0.181	0.234	0.281	0.321	0.353	0.375	0.390	0.394	0.390	0.375	0.353	0.321	0.281	0.234	0.181	0.124	0.063	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.040	0.079	0.116	0.150	0.180	0.206	0.226	0.241	0.250	0.253	0.250	0.241	0.226	0.206	0.180	0.150	0.116	0.079	0.040	0.000
FINAL CAMBER ↑	0"	1/4"	9/16"	3/4"	1"	1 3/16"	1 3/8"	1 1/2"	1 5/8"	1 11/16"	1 11/16"	1 11/16"	1 5/8"	1 1/2"	1 3/8"	1 3/16"	1"	3/4"	9/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 3 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.063	0.124	0.181	0.234	0.281	0.321	0.353	0.375	0.390	0.394	0.390	0.375	0.353	0.321	0.281	0.234	0.181	0.124	0.063	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.040	0.079	0.116	0.150	0.180	0.206	0.226	0.241	0.250	0.253	0.250	0.241	0.226	0.206	0.180	0.150	0.116	0.079	0.040	0.000
FINAL CAMBER ↑	0"	1/4"	9/16"	3/4"	1"	1 3/16"	1 3/8"	1 1/2"	1 5/8"	1 11/16"	1 11/16"	1 11/16"	1 5/8"	1 1/2"	1 3/8"	1 3/16"	1"	3/4"	9/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 4 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.063	0.124	0.181	0.234	0.281	0.321	0.353	0.375	0.390	0.394	0.390	0.375	0.353	0.321	0.281	0.234	0.181	0.124	0.063	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.040	0.079	0.116	0.150	0.180	0.206	0.226	0.241	0.250	0.253	0.250	0.241	0.226	0.206	0.180	0.150	0.116	0.079	0.040	0.000
FINAL CAMBER ↑	0"	1/4"	9/16"	3/4"	1"	1 3/16"	1 3/8"	1 1/2"	1 5/8"	1 11/16"	1 11/16"	1 11/16"	1 5/8"	1 1/2"	1 3/8"	1 3/16"	1"	3/4"	9/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 5 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.063	0.124	0.181	0.234	0.281	0.321	0.353	0.375	0.390	0.394	0.390	0.375	0.353	0.321	0.281	0.234	0.181	0.124	0.063	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.034	0.067	0.098	0.127	0.152	0.173	0.190	0.203	0.211	0.213	0.211	0.203	0.190	0.173	0.152	0.127	0.098	0.067	0.034	0.000
FINAL CAMBER ↑	0"	3/8"	11/16"	1"	1 5/16"	1 9/16"	1 3/4"	1 15/16"	2 1/16"	2 1/8"	2 3/16"	2 1/8"	2 1/16"	1 15/16"	1 3/4"	1 9/16"	1 5/16"	1"	11/16"	3/8"	0"

* INCLUDES FUTURE WEARING SURFACE IN SUPERIMPOSED DEAD LOAD.
ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT " FINAL CAMBER ", WHICH IS GIVEN IN INCHES (FRACTION FORM).

DRAWN BY : MAL DATE : 11/2016
CHECKED BY : JMR DATE : 12/2016
DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-
SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						SHEET NO. S1-17	
SUPERSTRUCTURE DEAD LOAD DEFLECTION SPANS A & B LEFT LANE						TOTAL SHEETS 36	
REVISIONS							
NO.	BY:	DATE:	NO.	BY:	DATE:		
1			3				
2			4				

DEAD LOAD DEFLECTION TABLE FOR GIRDERS

0.6" Ø LOW RELAXATION	SPAN C																				
	GIRDER 1 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.062	0.123	0.180	0.232	0.279	0.318	0.350	0.372	0.386	0.391	0.386	0.372	0.350	0.318	0.279	0.232	0.180	0.123	0.062	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.035	0.070	0.102	0.132	0.158	0.180	0.198	0.211	0.219	0.222	0.219	0.211	0.198	0.180	0.158	0.132	0.102	0.070	0.035	0.000
FINAL CAMBER ↑	0"	5/16"	5/8"	15/16"	1 3/16"	1 7/16"	1 5/8"	1 13/16"	1 15/16"	2"	2 1/16"	2"	1 15/16"	1 13/16"	1 5/8"	1 7/16"	1 3/16"	15/16"	5/8"	5/16"	0"
0.6" Ø LOW RELAXATION	GIRDER 2 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.062	0.123	0.180	0.232	0.279	0.318	0.350	0.372	0.386	0.391	0.386	0.372	0.350	0.318	0.279	0.232	0.180	0.123	0.062	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.039	0.077	0.113	0.146	0.175	0.199	0.219	0.234	0.242	0.245	0.242	0.234	0.219	0.199	0.175	0.146	0.113	0.077	0.039	0.000
FINAL CAMBER ↑	0"	1/4"	9/16"	13/16"	1 1/16"	1 1/4"	1 7/16"	1 9/16"	1 11/16"	1 3/4"	1 3/4"	1 3/4"	1 11/16"	1 9/16"	1 7/16"	1 1/4"	1 1/16"	13/16"	9/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 3 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.062	0.123	0.180	0.232	0.279	0.318	0.350	0.372	0.386	0.391	0.386	0.372	0.350	0.318	0.279	0.232	0.180	0.123	0.062	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.039	0.077	0.113	0.146	0.175	0.200	0.219	0.234	0.243	0.246	0.243	0.234	0.219	0.200	0.175	0.146	0.113	0.077	0.039	0.000
FINAL CAMBER ↑	0"	1/4"	9/16"	13/16"	1 1/16"	1 1/4"	1 7/16"	1 9/16"	1 11/16"	1 3/4"	1 3/4"	1 3/4"	1 11/16"	1 9/16"	1 7/16"	1 1/4"	1 1/16"	13/16"	9/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 4 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.062	0.123	0.180	0.232	0.279	0.318	0.350	0.372	0.386	0.391	0.386	0.372	0.350	0.318	0.279	0.232	0.180	0.123	0.062	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.039	0.077	0.113	0.146	0.175	0.200	0.219	0.234	0.243	0.246	0.243	0.234	0.219	0.200	0.175	0.146	0.113	0.077	0.039	0.000
FINAL CAMBER ↑	0"	1/4"	9/16"	13/16"	1 1/16"	1 1/4"	1 7/16"	1 9/16"	1 11/16"	1 3/4"	1 3/4"	1 3/4"	1 11/16"	1 9/16"	1 7/16"	1 1/4"	1 1/16"	13/16"	9/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 5 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.062	0.123	0.180	0.232	0.279	0.318	0.350	0.372	0.386	0.391	0.386	0.372	0.350	0.318	0.279	0.232	0.180	0.123	0.062	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.033	0.065	0.095	0.123	0.148	0.168	0.185	0.197	0.205	0.207	0.205	0.197	0.185	0.168	0.148	0.123	0.095	0.065	0.033	0.000
FINAL CAMBER ↑	0"	3/8"	11/16"	1"	1 5/16"	1 9/16"	1 13/16"	2"	2 1/8"	2 3/16"	2 3/16"	2 3/16"	2 1/8"	2"	1 13/16"	1 9/16"	1 5/16"	1"	11/16"	3/8"	0"

* INCLUDES FUTURE WEARING SURFACE IN SUPERIMPOSED DEAD LOAD.
ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT " FINAL CAMBER ", WHICH IS GIVEN IN INCHES (FRACTION FORM).

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

SHEET 2 OF 2

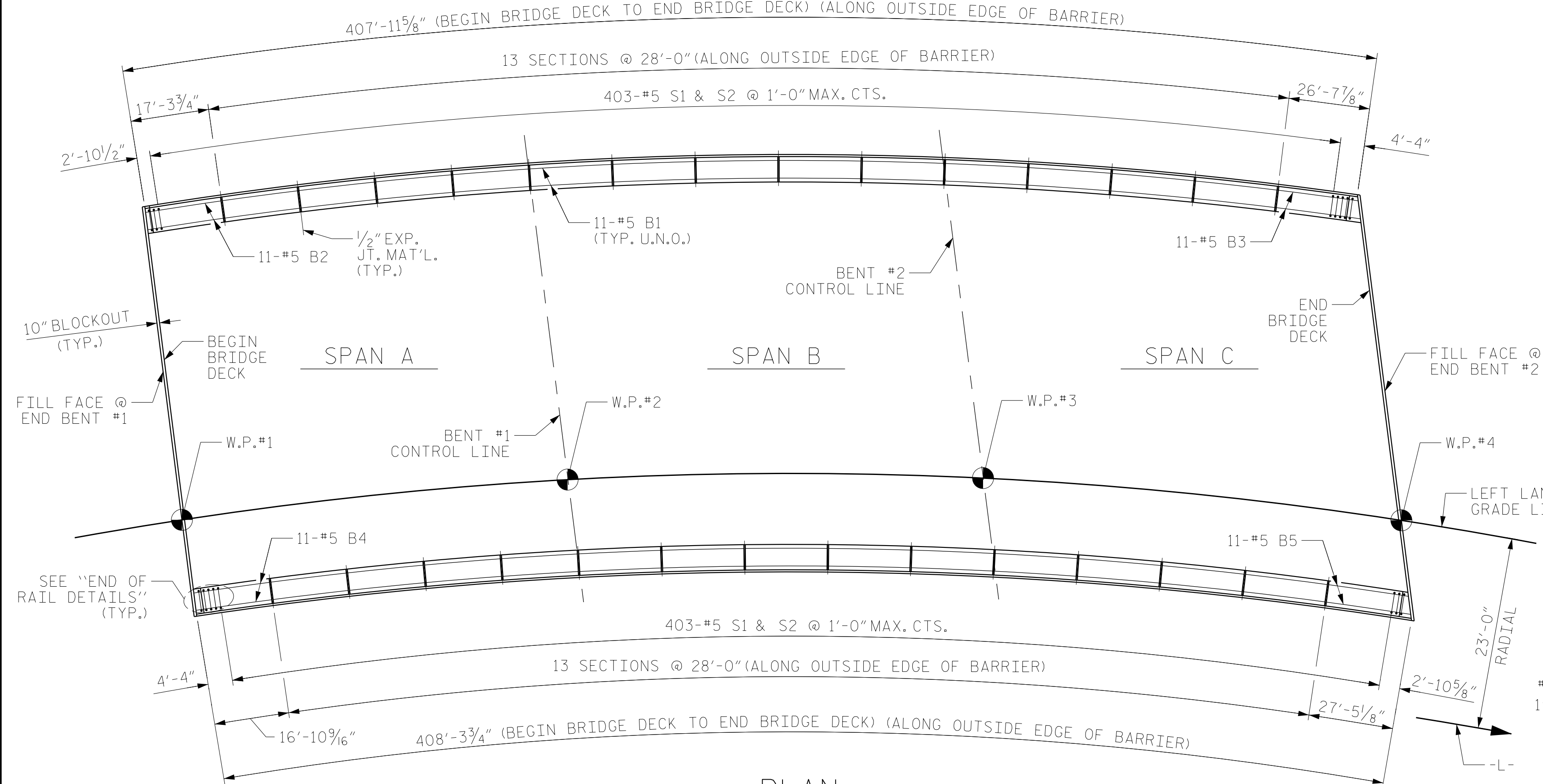


STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
DEAD LOAD DEFLECTION
SPAN C
LEFT LANE

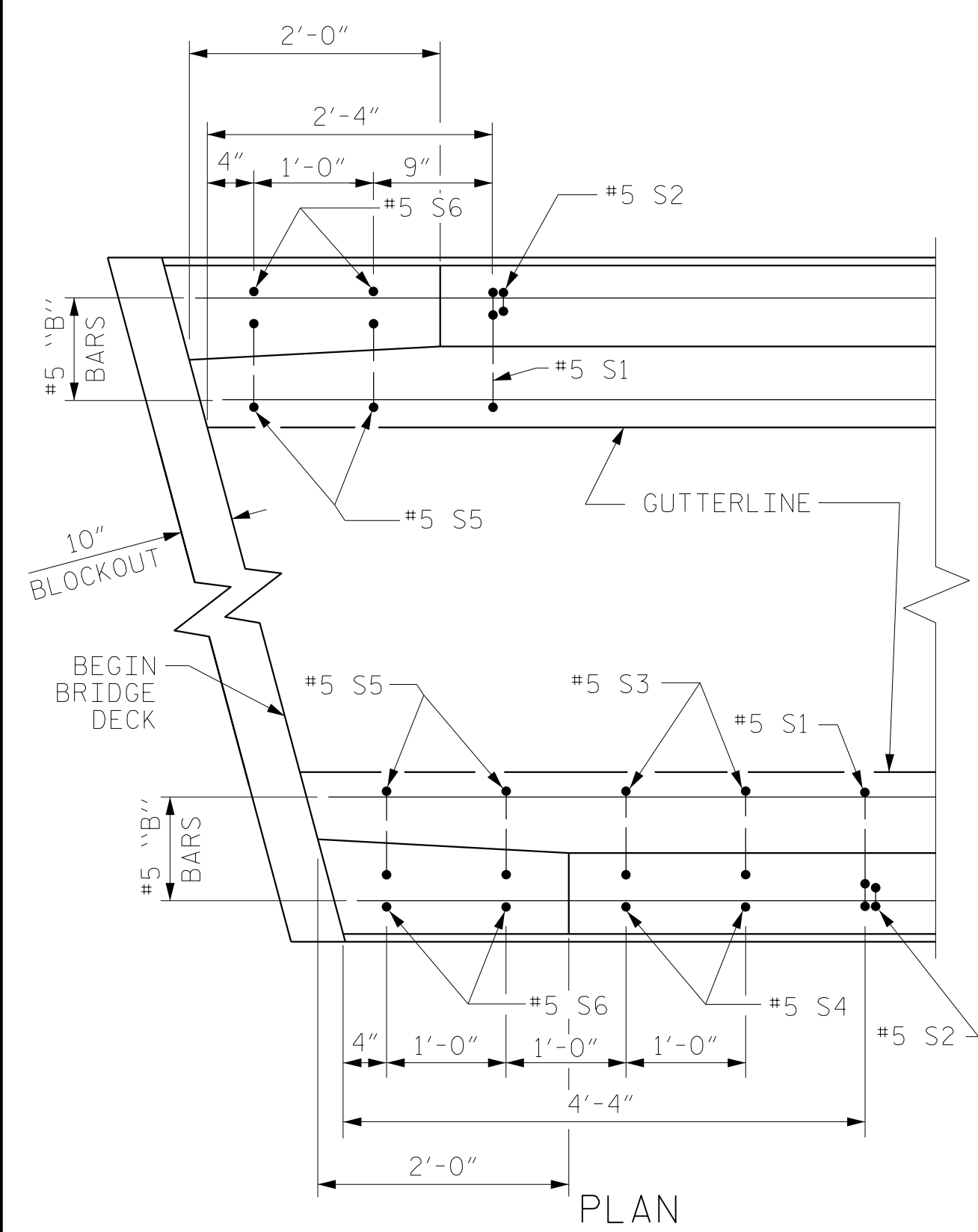
DRAWN BY : MAL DATE : 11/2016
CHECKED BY : JMR DATE : 12/2016
DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

DOCUMENT NOT CONSIDERED
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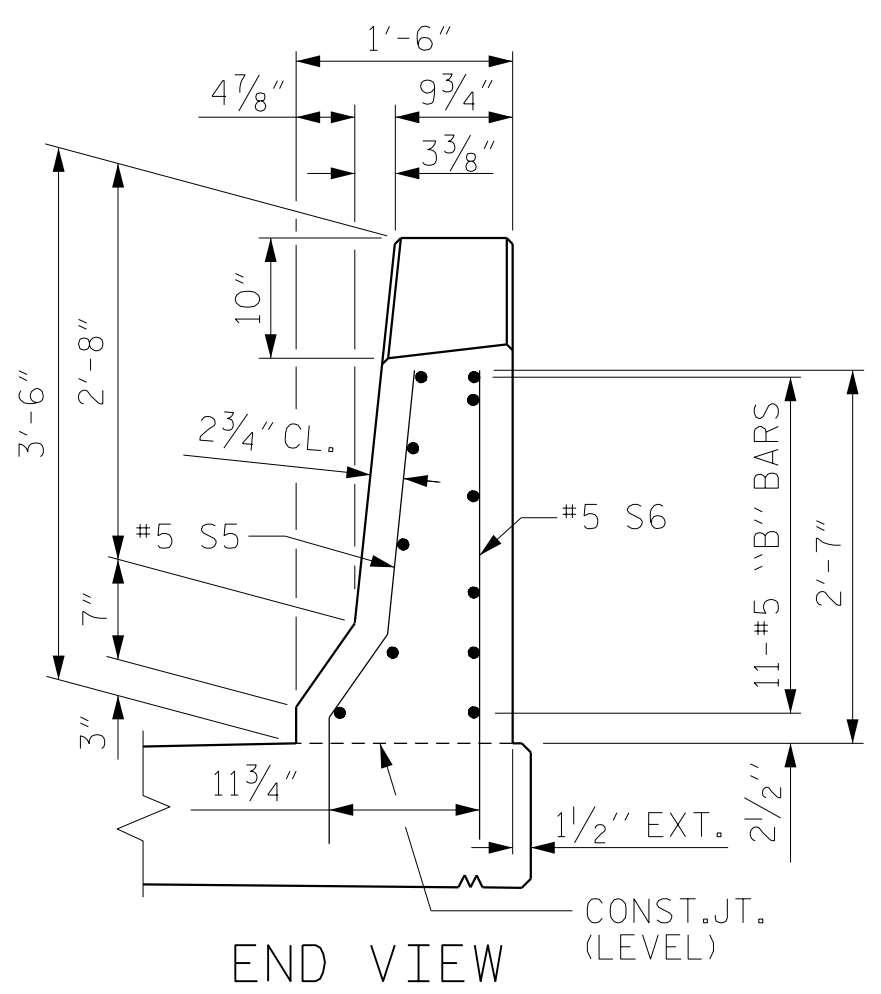
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-18
1			3			TOTAL SHEETS
2			4			36



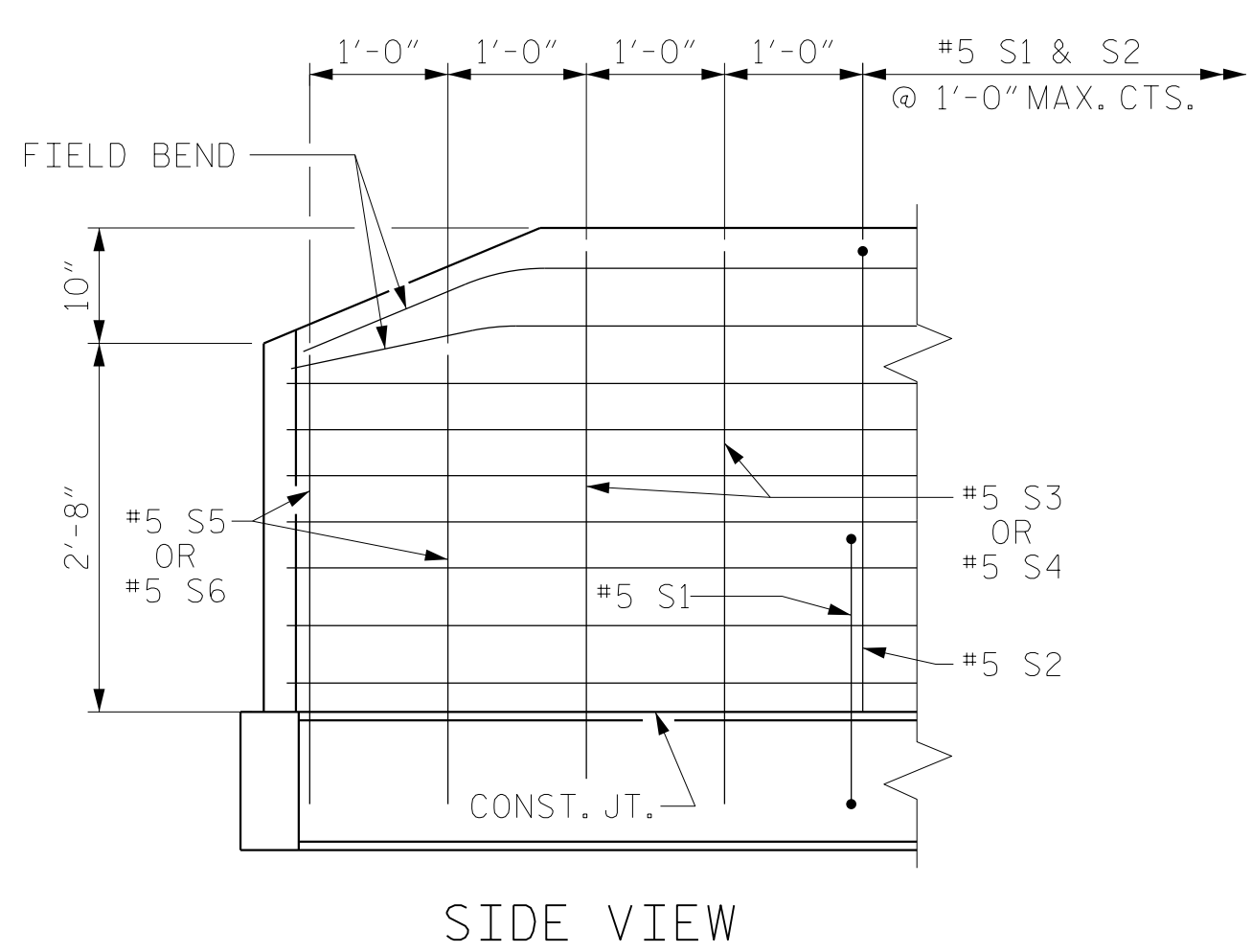
PLAN



PLAN



END VIEW



SIDE VIEW

END OF RAIL DETAILS

FOR ADHESIVE ANCHORING AT SAWED JOINTS

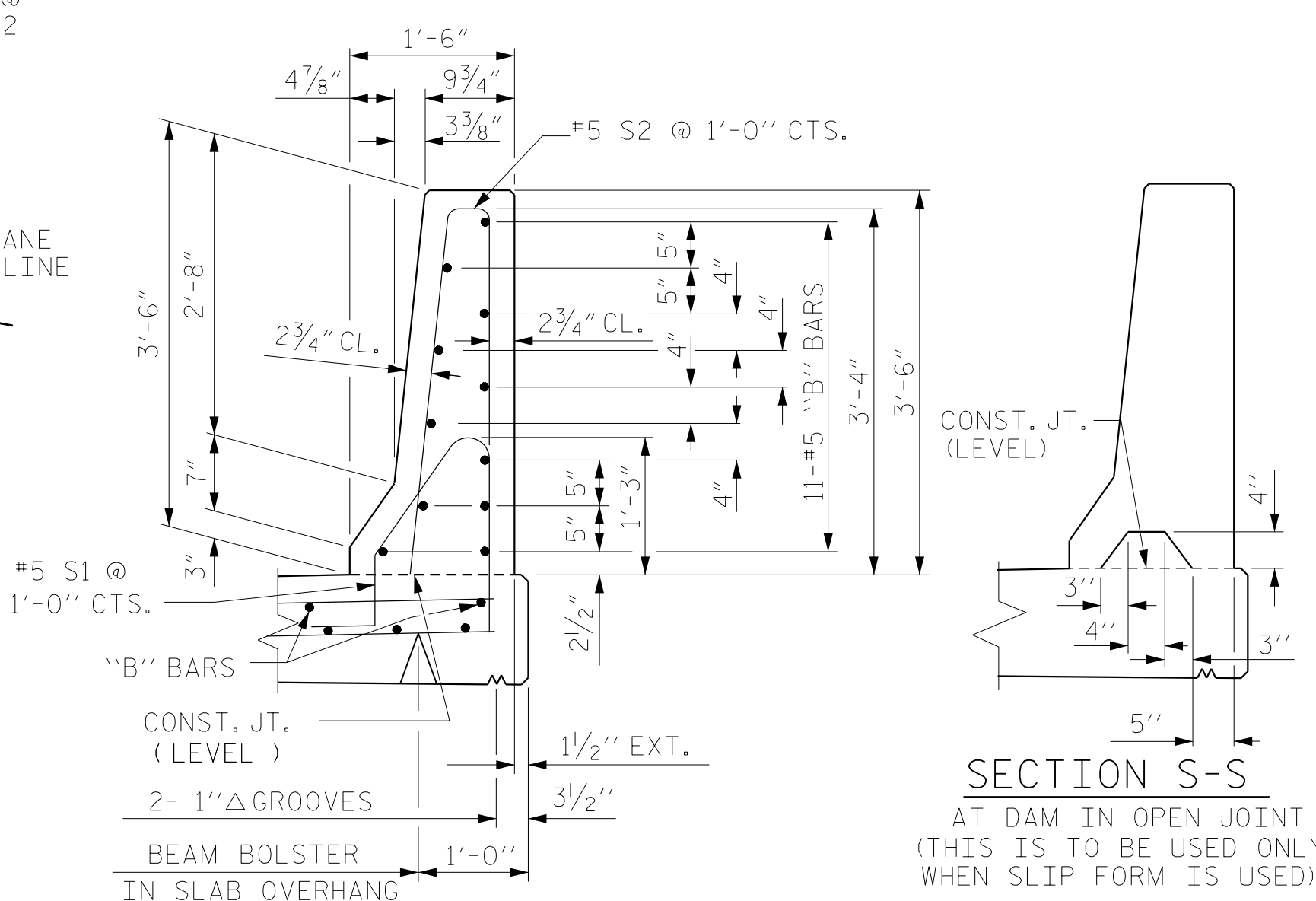
NOTES

THE BARRIER RAIL IN EACH SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

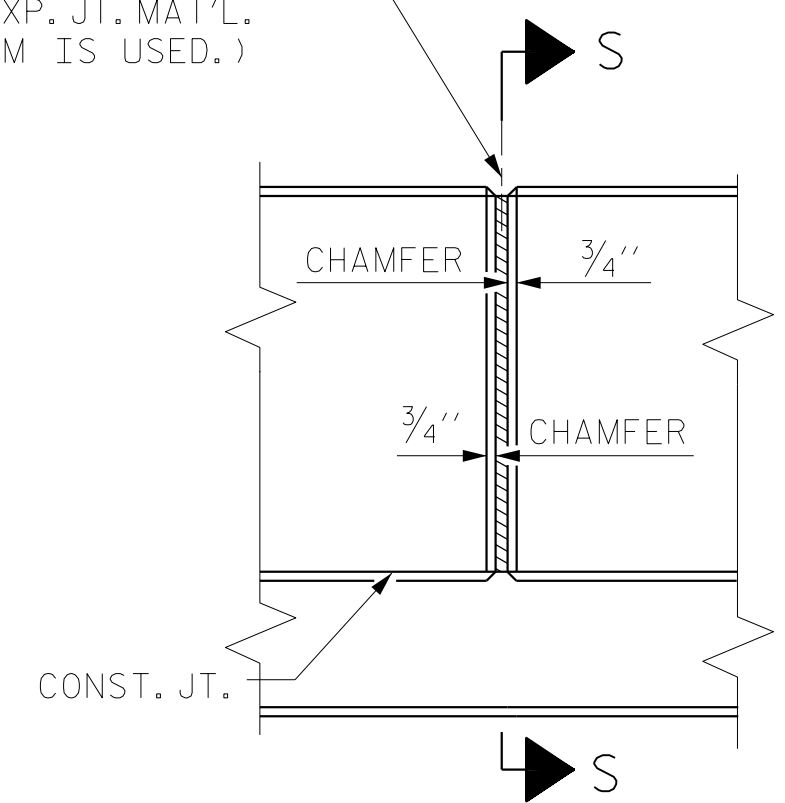
THE #5 S3, S4, S5 AND S6 BARS SHALL BE INSTALLED, USING AN ADHESIVE ANCHORING SYSTEM, AFTER SAWING THE JOINT. THE YIELD LOAD FOR THE #5 S3, S4, S5 AND S6 BARS IS 18.6 KIPS. FIELD TESTING FOR THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.



SECTION THRU RAIL

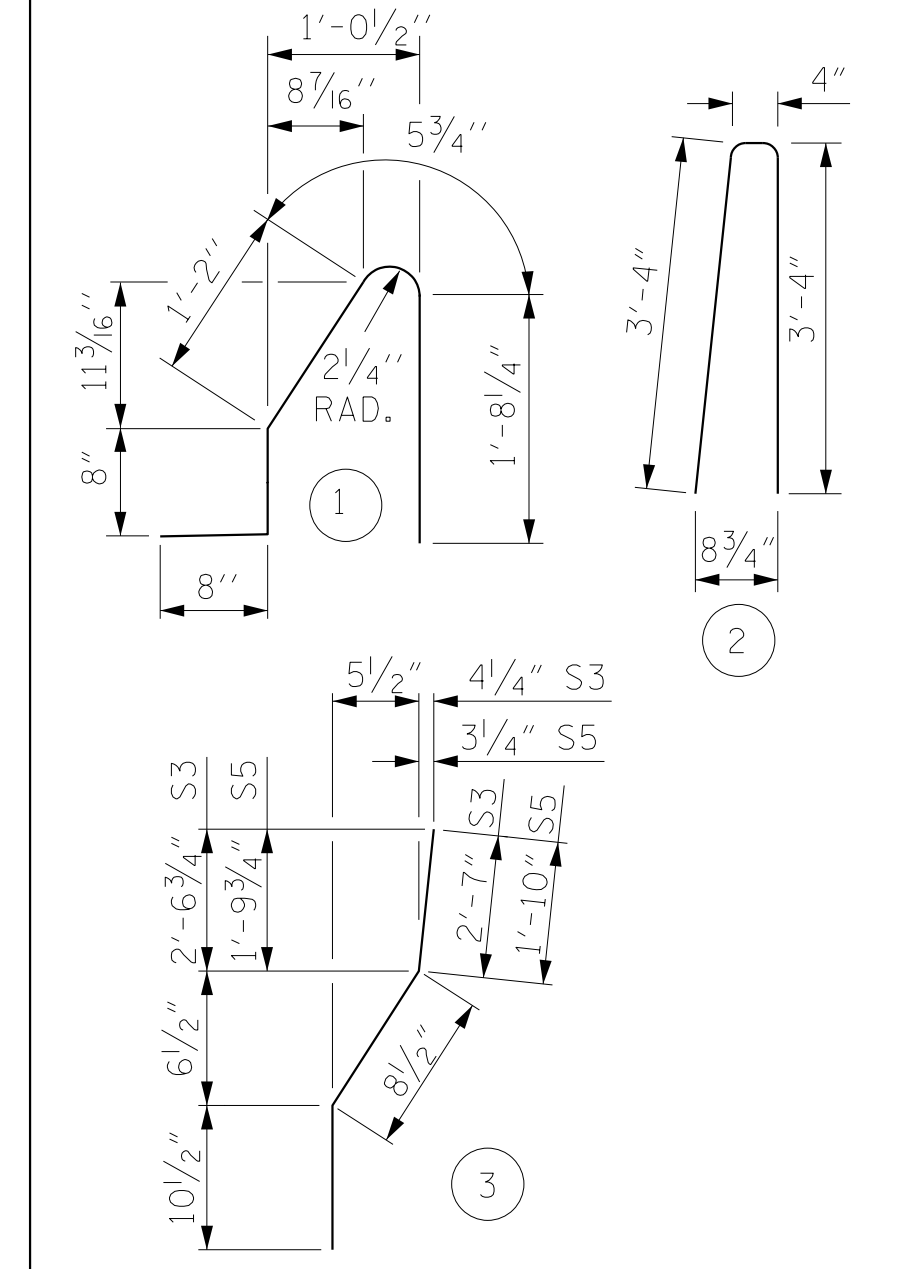
1/2" EXP. JT. MAT'L HELD IN PLACE WITH GALVANIZED NAILS. (NOTE: OMIT EXP. JT. MAT'L WHEN SLIP FORM IS USED.)



ELEVATION AT EXPANSION JOINTS

BARRIER RAIL DETAILS

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL

FOR CONCRETE BARRIER RAIL ONLY

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
*B1	286	#5	STR	27'-8"	8253
*B2	11	#5	STR	16'-5"	188
*B3	11	#5	STR	26'-3"	301
*B4	11	#5	STR	16'-6"	189
*B5	11	#5	STR	26'-5"	303
*S1	806	#5	1	4'-8"	3923
*S2	806	#5	2	7'-0"	5885
*S3	4	#5	3	4'-2"	17
*S4	4	#5	STR	4'-0"	17
*S5	8	#5	3	3'-5"	29
*S6	8	#5	STR	3'-3"	27

* EPOXY COATED REINFORCING STEEL 19,132 LBS.
 CLASS AA CONCRETE 111.0 CU. YDS.
 CONCRETE BARRIER RAIL 816.3 LIN. FT.

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 384+50.00 -L-



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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 CONCRETE BARRIER RAIL
 LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-19
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : MAL DATE : 11/2016
 CHECKED BY : JMR DATE : 11/2016
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

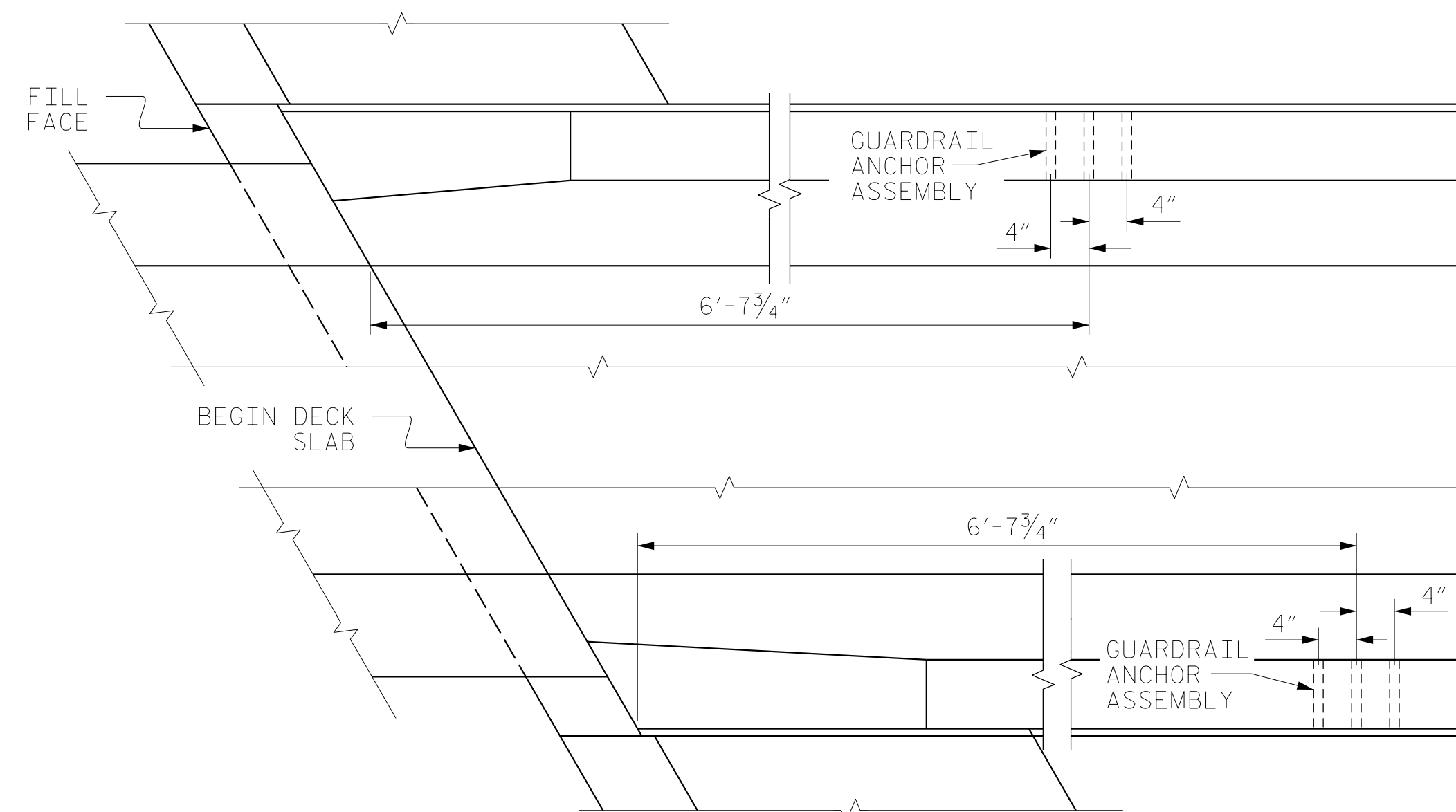
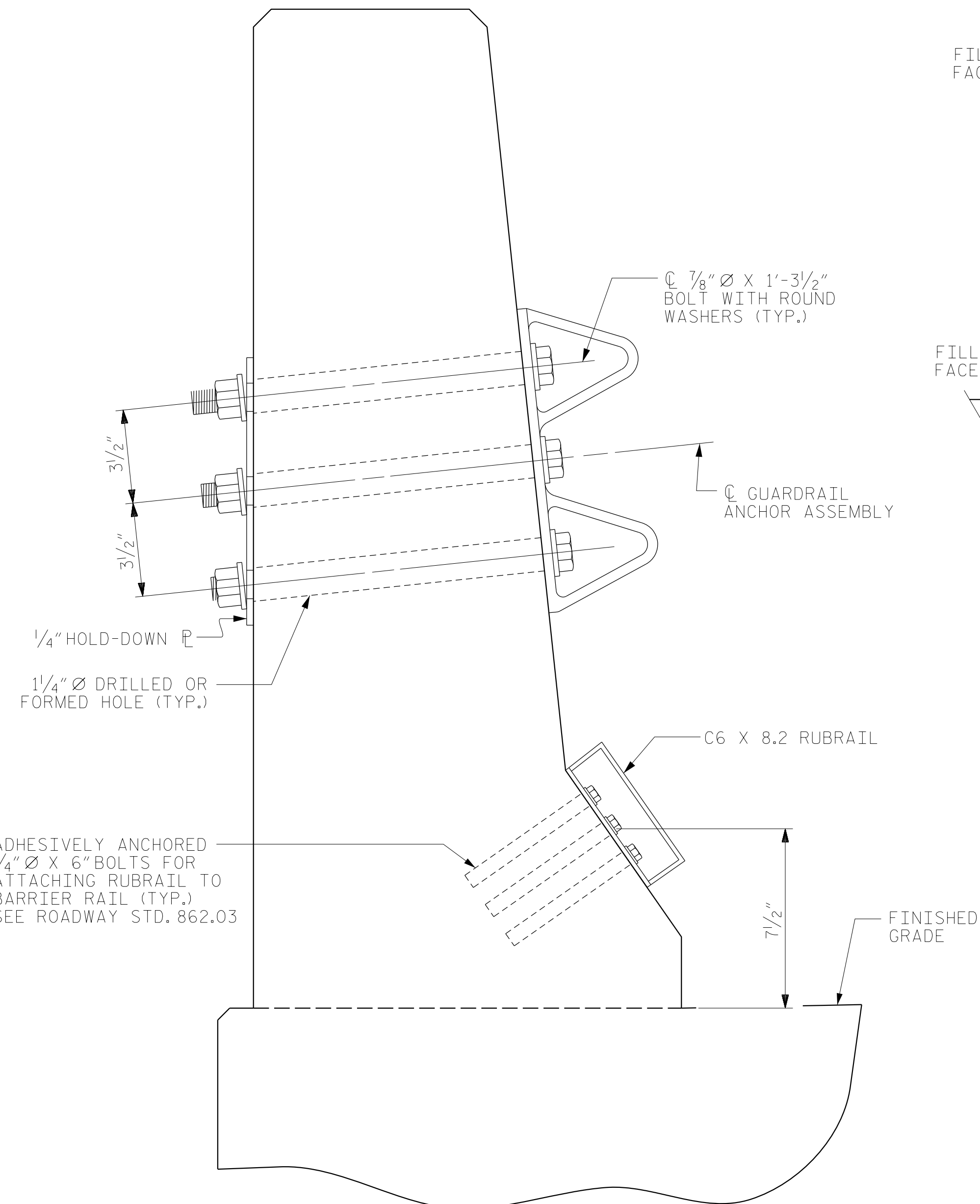
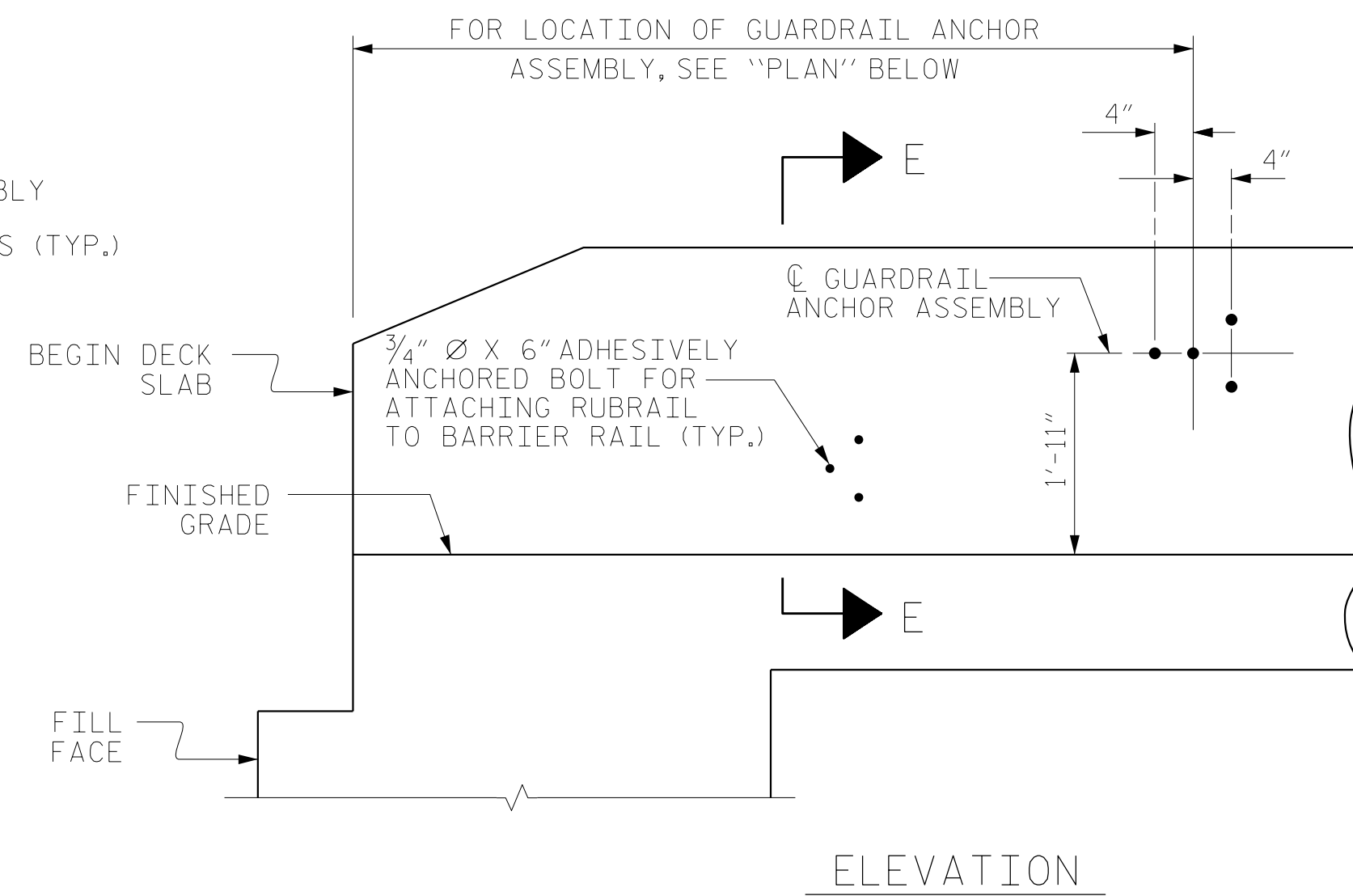
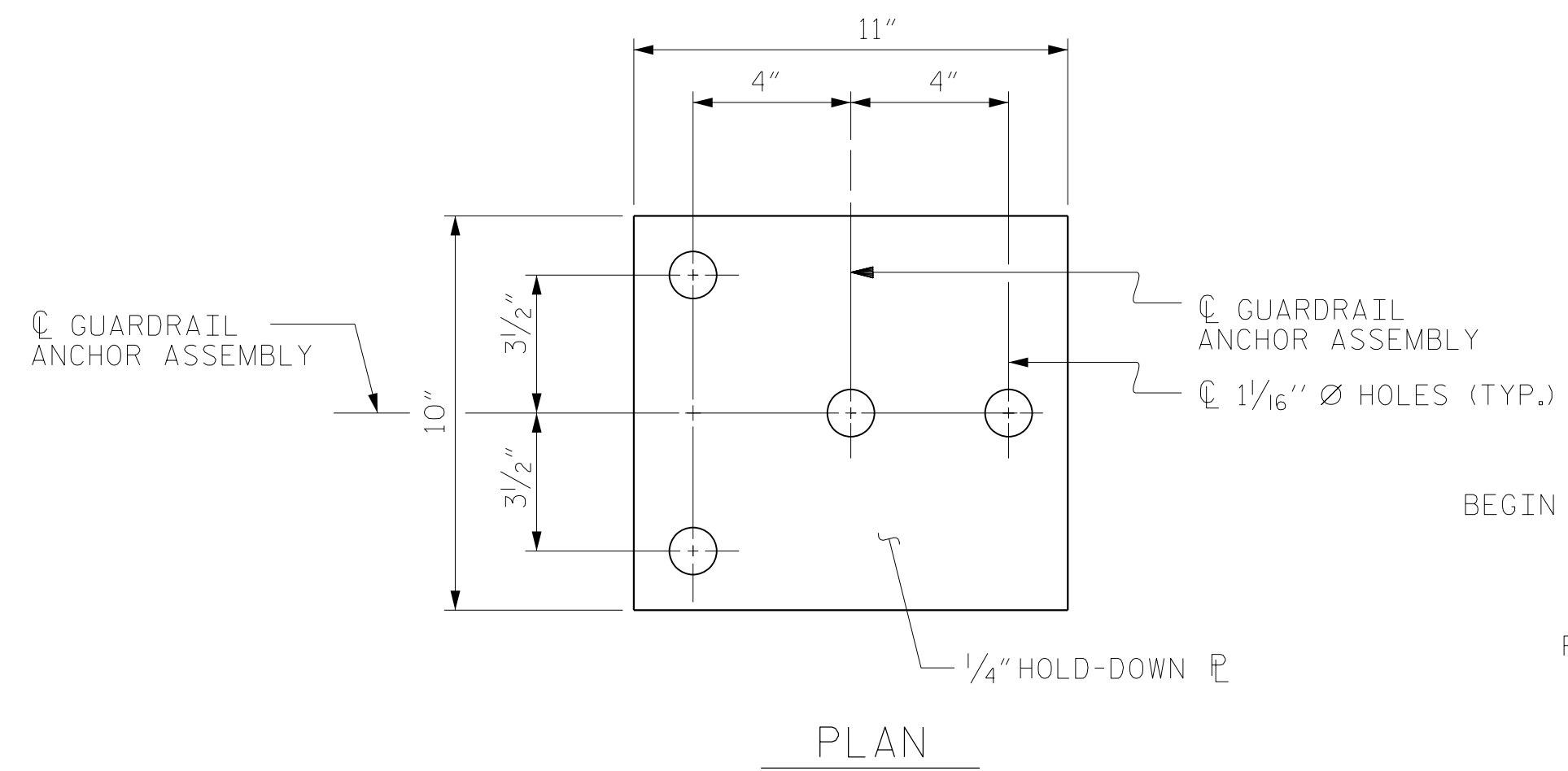
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

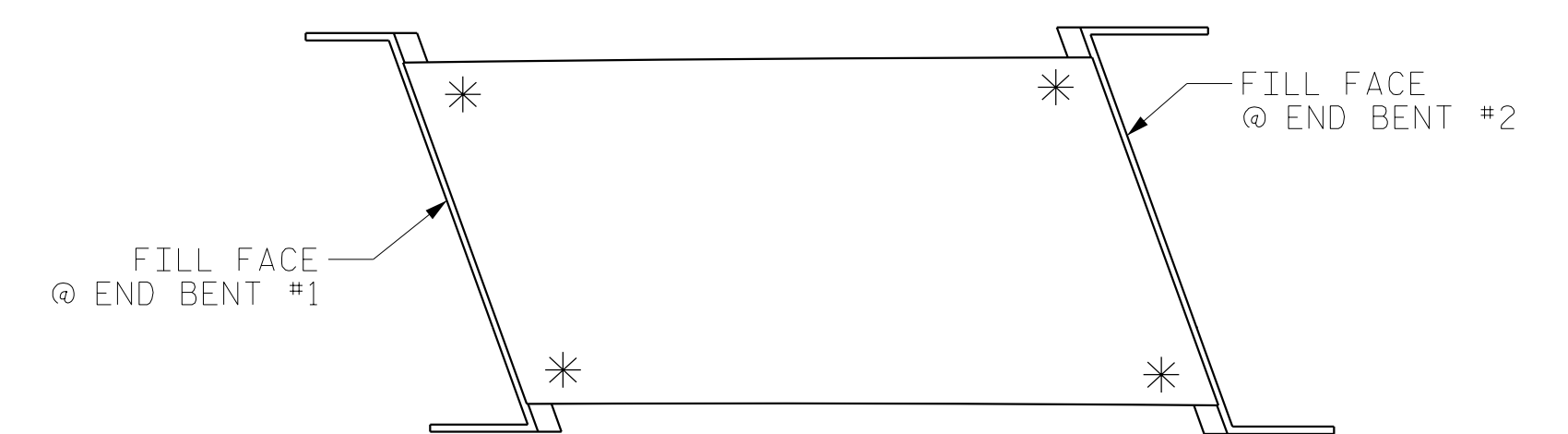
THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENTS

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-



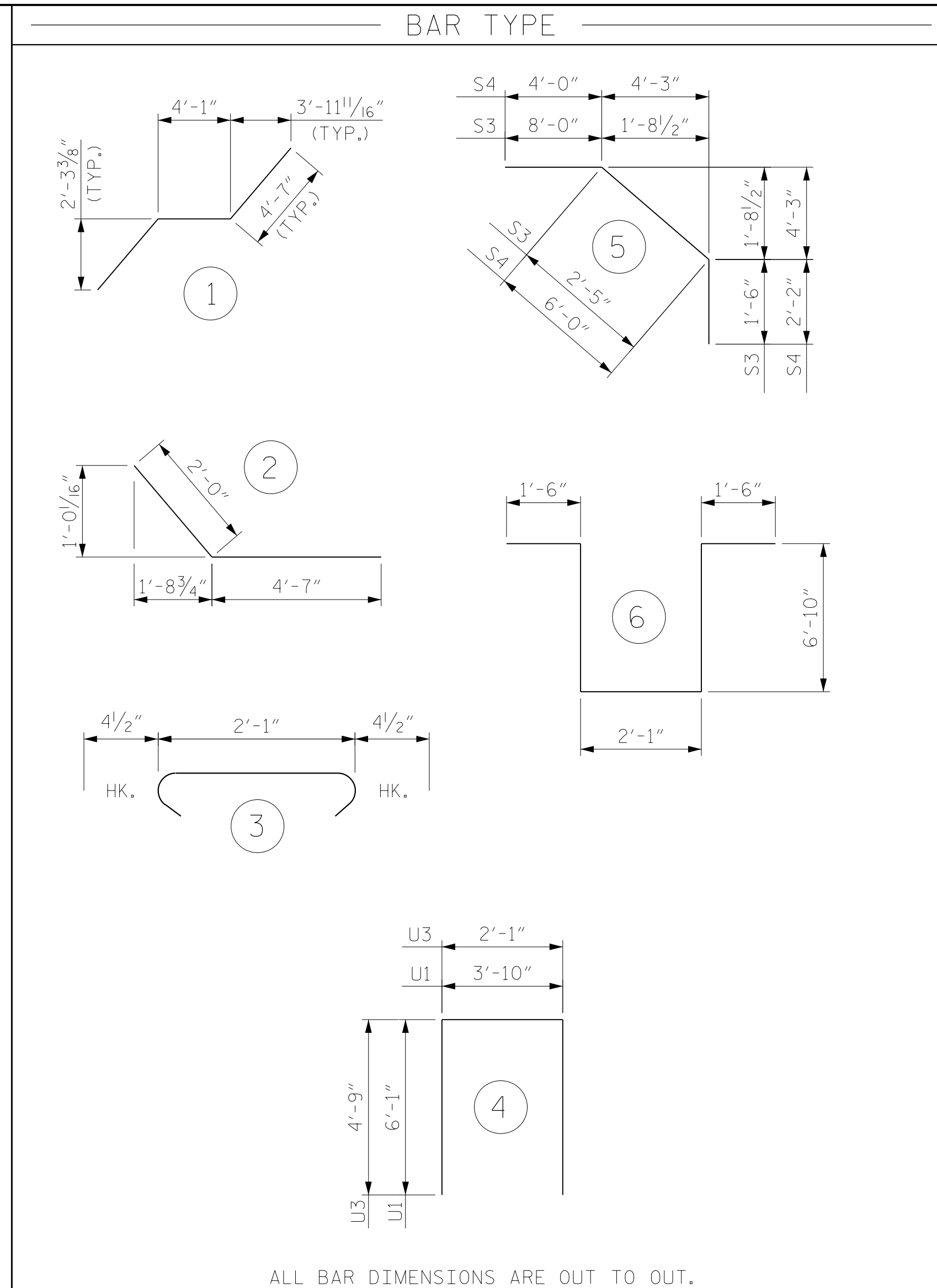
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 GUARDRAIL ANCHORAGE
 FOR BARRIER RAIL
 LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-20
1			3			TOTAL SHEETS
2			4			36

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ASSEMBLED BY : MAL	DATE : 11/2016
CHECKED BY : JMR	DATE : 11/2016
DRAWN BY : TLA 5/06	REV. 10/11/11 MAA/GM
CHECKED BY : GM 5/06	REV. 7/12 MAA/GM
	REV. 6/13 MAA/GM



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL																							
BAR NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR NO.	SIZE	TYPE	LENGTH	WEIGHT				
* A1	669	#5	STR	48'-11"	34132	* A144	1	#5	STR	25'-5"	27	A231	1	#5	STR	46'-2"	48	K4	24	#4	STR	8'-5"	135
A2	669	#5	STR	48'-11"	34132	* A145	1	#5	STR	23'-10"	25	A232	1	#5	STR	44'-7"	47	K5	36	#4	1	13'-3"	319
						* A146	1	#5	STR	22'-3"	23	A233	1	#5	STR	42'-11"	45	K6	24	#4	2	6'-7"	106
* A101	1	#5	STR	47'-9"	50	* A147	1	#5	STR	20'-8"	22	A234	1	#5	STR	41'-4"	43	K7	20	#4	STR	3'-1"	41
* A102	1	#5	STR	46'-1"	48	* A148	1	#5	STR	19'-1"	20	A235	1	#5	STR	39'-9"	41	K8	4	#4	STR	2'-3"	6
* A103	1	#5	STR	44'-6"	46	* A149	1	#5	STR	17'-6"	18	A236	1	#5	STR	38'-2"	40						
* A104	1	#5	STR	42'-10"	45	* A150	1	#5	STR	15'-10"	17	A237	1	#5	STR	36'-7"	38	S1	360	#4	3	2'-10"	681
* A105	1	#5	STR	41'-3"	43	* A151	1	#5	STR	14'-3"	15	A238	1	#5	STR	35'-0"	37	* S3	72	#4	5	11'-11"	573
* A106	1	#5	STR	39'-7"	41	* A152	1	#5	STR	12'-8"	13	A239	1	#5	STR	33'-5"	35	* S4	72	#4	5	12'-2"	585
* A107	1	#5	STR	38'-0"	40	* A153	1	#5	STR	11'-1"	12	A240	1	#5	STR	31'-10"	33						
* A108	1	#5	STR	36'-4"	38	* A154	1	#5	STR	9'-6"	10	A241	1	#5	STR	30'-2"	31	U1	72	#4	4	16'-0"	770
* A109	1	#5	STR	34'-9"	36	* A155	1	#5	STR	7'-11"	8	A242	1	#5	STR	28'-7"	30	U2	56	#4	6	18'-9"	701
* A110	1	#5	STR	33'-1"	35	* A156	1	#5	STR	6'-4"	7	A243	1	#5	STR	27'-0"	28	U3	16	#4	4	11'-7"	124
* A111	1	#5	STR	31'-6"	33	* A157	1	#5	STR	4'-9"	5	A244	1	#5	STR	25'-5"	27						
* A112	1	#5	STR	29'-10"	31	* A158	1	#5	STR	3'-2"	3	A245	1	#5	STR	23'-10"	25						
* A113	1	#5	STR	28'-3"	29						A246	1	#5	STR	22'-3"	23							
* A114	1	#5	STR	26'-7"	28	A201	1	#5	STR	47'-9"	50	A247	1	#5	STR	20'-8"	22						
* A115	1	#5	STR	25'-0"	26	A202	1	#5	STR	46'-1"	48	A248	1	#5	STR	19'-1"	20						
* A116	1	#5	STR	23'-4"	24	A203	1	#5	STR	44'-6"	46	A249	1	#5	STR	17'-6"	18						
* A117	1	#5	STR	21'-9"	23	A204	1	#5	STR	42'-10"	45	A250	1	#5	STR	15'-10"	17						
* A118	1	#5	STR	20'-1"	21	A205	1	#5	STR	41'-3"	43	A251	1	#5	STR	14'-3"	15						
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* A120	1	#5	STR	16'-10"	18	A207	1	#5	STR	38'-0"	40	A253	1	#5	STR	11'-1"	12						
* A121	1	#5	STR	15'-3"	16	A208	1	#5	STR	36'-4"	38	A254	1	#5	STR	9'-6"	10						
* A122	1	#5	STR	13'-7"	14	A209	1	#5	STR	34'-9"	36	A255	1	#5	STR	7'-11"	8						
* A123	1	#5	STR	12'-0"	13	A210	1	#5	STR	33'-1"	35	A256	1	#5	STR	6'-4"	7						
* A124	1	#5	STR	10'-4"	11	A211	1	#5	STR	31'-6"	33	A257	1	#5	STR	4'-9"	5						
* A125	1	#5	STR	8'-9"	9	A212	1	#5	STR	29'-10"	31	A258	1	#5	STR	3'-2"	3						
* A126	1	#5	STR	7'-1"	7	A213	1	#5	STR	28'-3"	29												
* A127	1	#5	STR	5'-6"	6	A214	1	#5	STR	26'-7"	28	* B1	194	#6	STR	27'-10"	8110						
* A128	1	#5	STR	3'-11"	4	A215	1	#5	STR	25'-0"	26	B2	368	#5	STR	52'-11"	20311						
* A129	1	#5	STR	2'-3"	2	A216	1	#5	STR	23'-4"	24	* B3	147	#4	STR	22'-0"	2160						
* A130	1	#5	STR	47'-8"	50	A217	1	#5	STR	21'-9"	23	* B4	98	#7	STR	60'-0"	12019						
* A131	1	#5	STR	46'-2"	48	A218	1	#5	STR	20'-1"	21	* B5	48	#7	STR	40'-6"	3974						
* A132	1	#5	STR	44'-7"	47	A219	1	#5	STR	18'-6"	19	* B6	49	#7	STR	36'-3"	3631						
* A133	1	#5	STR	42'-11"	45	A220	1	#5	STR	16'-10"	18	B7	36	#5	STR	40'-6"	1521						
* A134	1	#5	STR	41'-4"	43	A221	1	#5	STR	15'-3"	16	B8	36	#5	STR	42'-0"	1577						
* A135	1	#5	STR	39'-9"	41	A222	1	#5	STR	13'-7"	14	* B9	98	#4	STR	26'-0"	1702						
* A136	1	#5	STR	38'-2"	40	A223	1	#5	STR	12'-0"	13	* B10	147	#4	STR	24'-3"	2381						
* A137	1	#5	STR	36'-7"	38	A224	1	#5	STR	10'-4"	11	* B11	48	#7	STR	42'-0"	4121						
* A138	1	#5	STR	35'-0"	37	A225	1	#5	STR	8'-9"	9	* B12	30	#4	STR	29'-3"	586						
* A139	1	#5	STR	33'-5"	35	A226	1	#5	STR	7'-1"	7	* B13	49	#7	STR	39'-3"	3931						
* A140	1	#5	STR	31'-10"	33	A227	1	#5	STR	5'-6"	6												
* A141	1	#5	STR	30'-2"	31	A228	1	#5	STR	3'-11"	4	K1	28	#4	STR	28'-3"	528						
* A142	1	#5	STR	28'-7"	30	A229	1	#5	STR	2'-3"	2	K2	120	#4	STR	10'-1"	808						
* A143	1	#5	STR	27'-0"	28	A230	1	#5	STR	47'-8"	50	K3	8	#4	STR	6'-11"	37						

REINFORCING STEEL 63,324 LBS.
 *EPOXY COATED REINFORCING STEEL 79,432 LBS.

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS

BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET, AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER RAIL
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3'-5"
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"
#7	5'-3"	3'-6"			
#8	6'-10"	4'-7"			

GROOVING BRIDGE FLOORS

APPROACH SLABS	2,122 SQ.FT.
BRIDGE DECK	17,550 SQ.FT.
TOTAL	19,672 SQ.FT.

—SUPERSTRUCTURE BILL OF MATERIAL—

	CLASS AA CONCRETE (CU. YDS.)	REINFORCING STEEL (LBS.)	EPOXY COATED REINFORCING STEEL (LBS.)
POUR 1	194.3		
POUR 2	260.1		
POUR 3	260.1		
POUR 4	114.6		
TOTALS**	829.1	63,324 LBS.	79,432 LBS.

** QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

PROJECT NO. R-2707C
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SHEET 1 OF 2



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 DEPARTMENT OF TRANSPORTATION
 RALEIGH

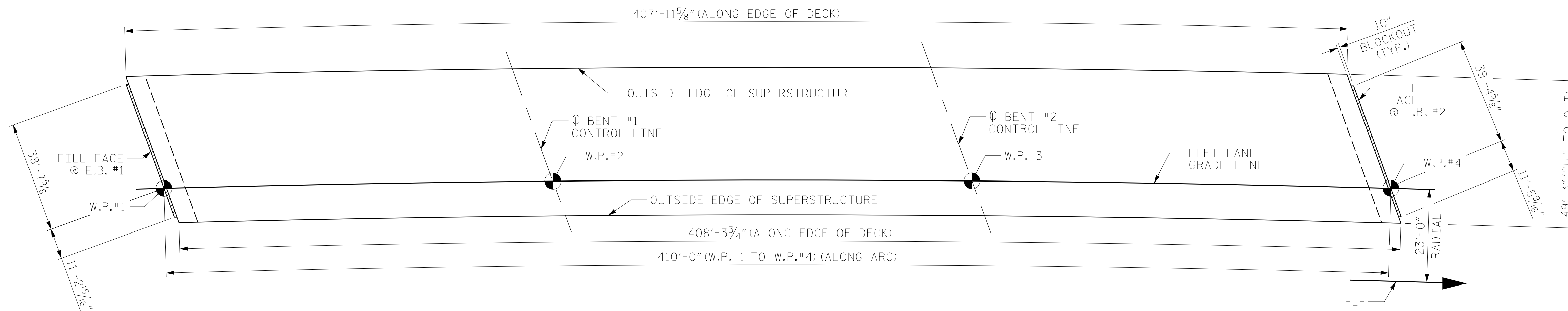
SUPERSTRUCTURE
 BILL OF MATERIAL

LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-21
1			3			TOTAL SHEETS
2			4			36

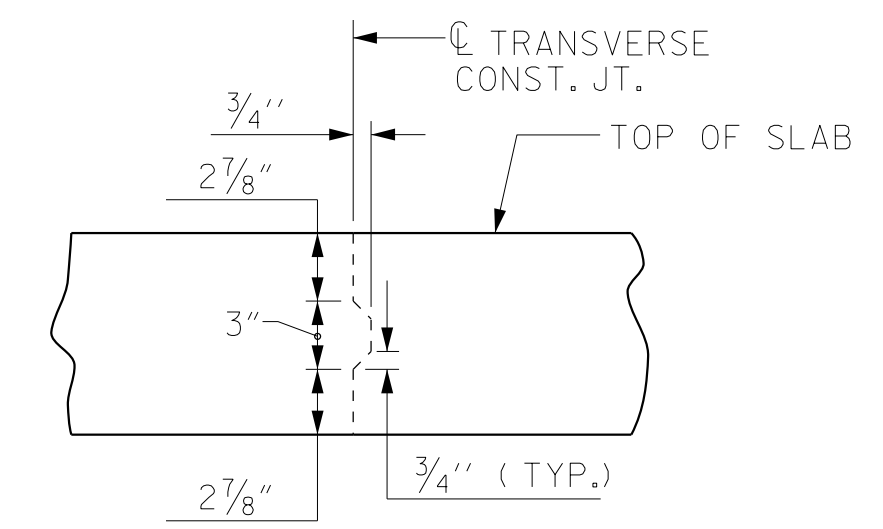
DRAWN BY : MAL DATE : 11/2016
 CHECKED BY : JMR DATE : 12/2016
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



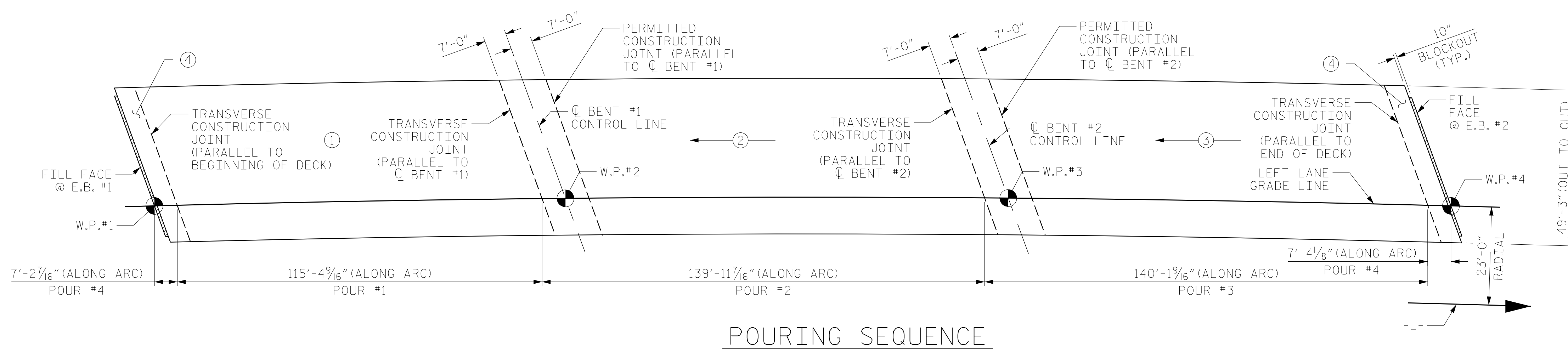
LAYOUT FOR COMPUTING
AREA OF REINFORCED CONCRETE DECK SLAB
(SQ. FT = 20,101)

NOTE: BENT & INTEGRAL END BENT
DIAPHRAGMS ARE PART OF
SUPERSTRUCTURE POUR.

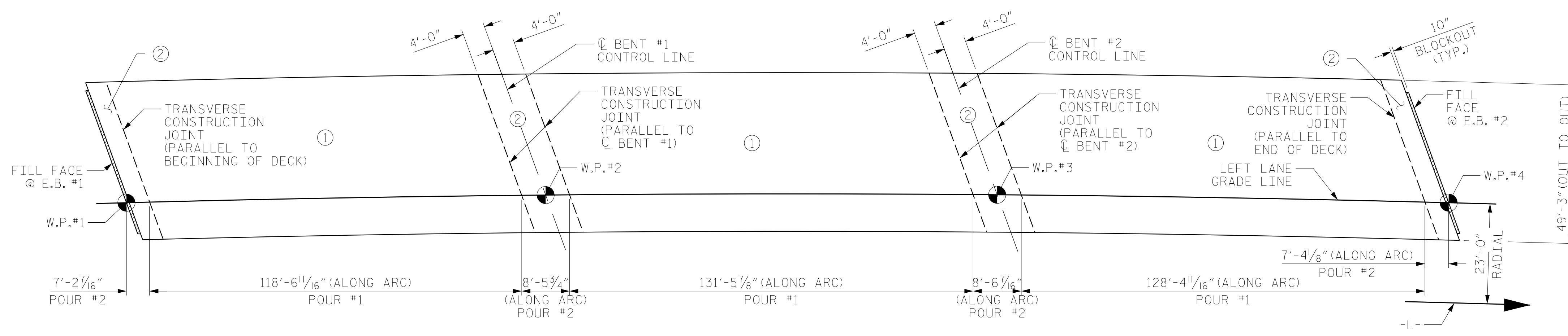


TRANSVERSE
CONSTRUCTION
JOINT DETAIL

NOTE: REINFORCING STEEL IN SLAB NOT
SHOWN. LONGITUDINAL REINFORCING
STEEL SHALL BE CONTINUOUS THRU
JOINT



POURING SEQUENCE



OPTIONAL POURING SEQUENCE

POUR 2 CAN NOT BE STARTED UNTIL BOTH ADJACENT ①POURS REACH A MINIMUM OF 3000 PSI

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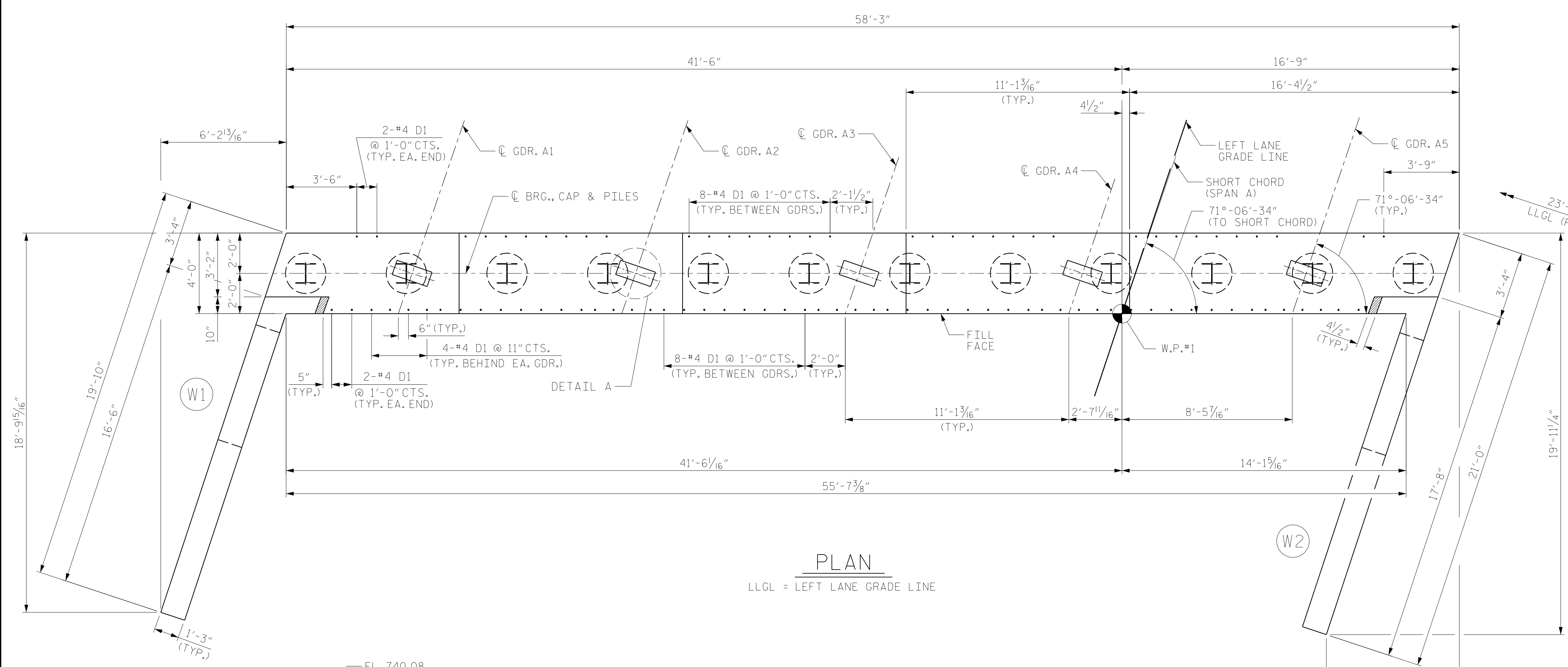
SHEET 2 OF 2

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						SHEET NO. S1-22	
SUPERSTRUCTURE BILL OF MATERIAL						TOTAL SHEETS 36	
LEFT LANE							
REVISIONS							
NO.	BY:	DATE:	NO.	BY:	DATE:		
1			3				
2			4				

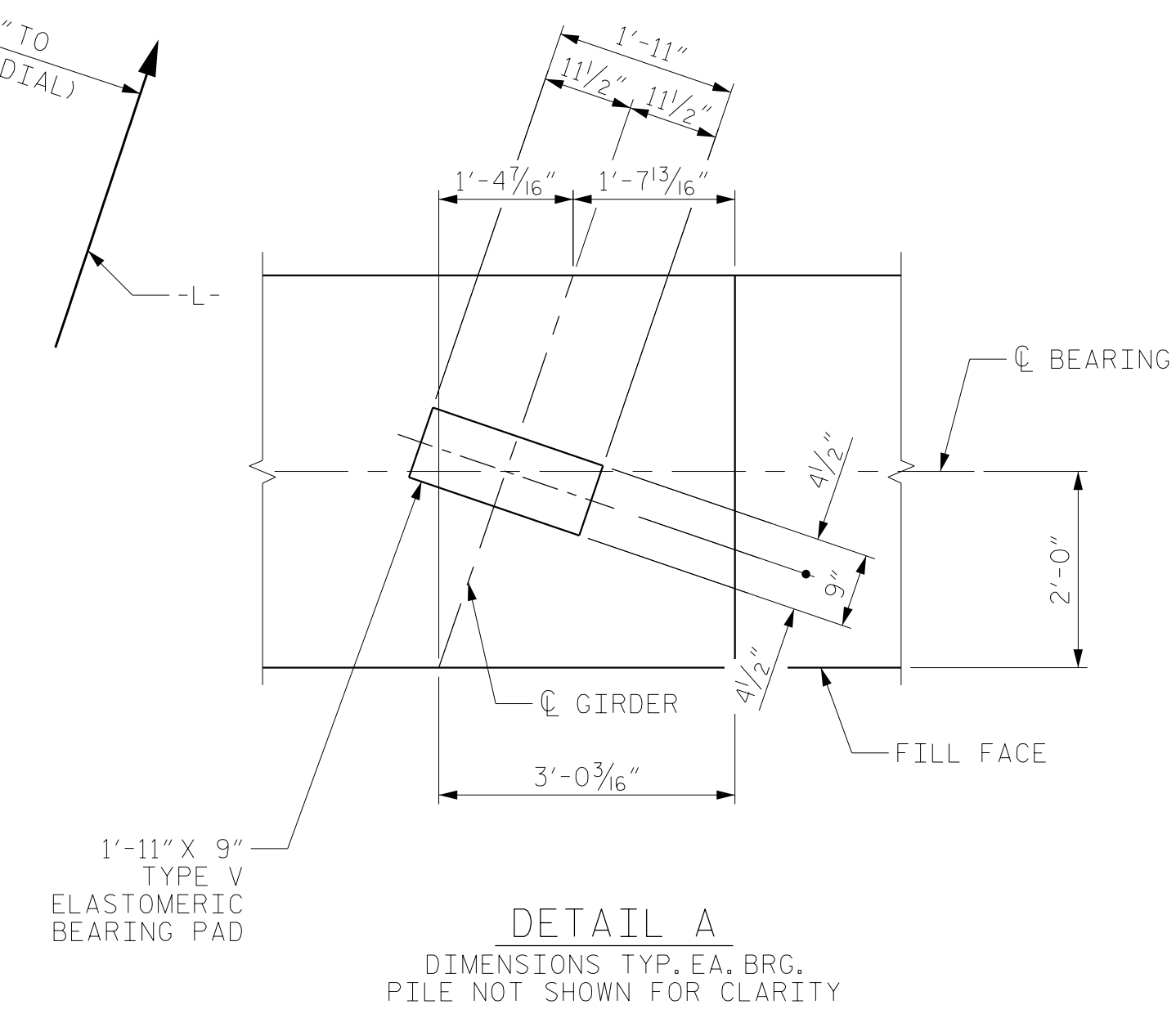
DRAWN BY : MAL DATE : 11/2016
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DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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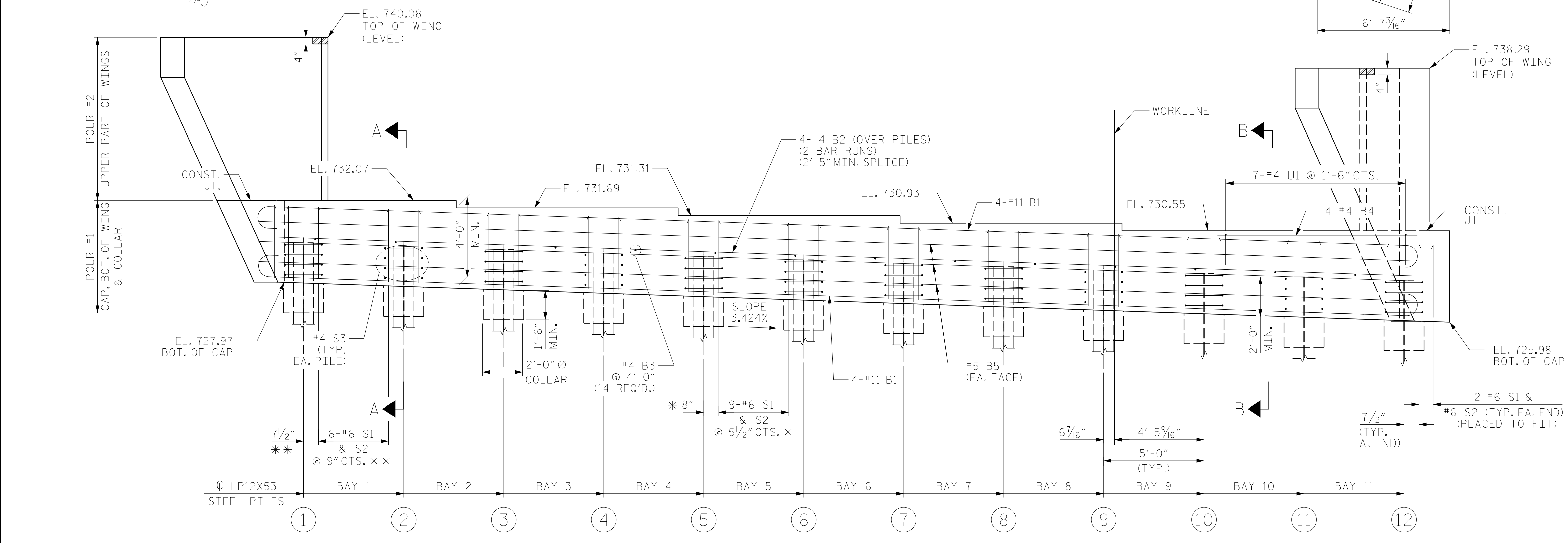
PLAN
LLGL = LEFT LANE GRADE LINE

NOTES:
 THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS. SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.
 FOR SECTION A-A AND SECTION B-B, SEE SHEET 4 OF 4.
 STIRRUPS MAY BE SHIFTED SLIGHTLY TO AVOID #4 DI BARS IN CAP.
 SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.
 THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE CONCRETE BARRIER IS CAST IF SLIP FORMING IS USED.



DETAIL A
DIMENSIONS TYP. EA. BRG. PILE NOT SHOWN FOR CLARITY

TOP OF PILE ELEVATIONS	
PILE	ELEVATION
1	729.96
2	729.79
3	729.62
4	729.45
5	729.27
6	729.10
7	728.93
8	728.76
9	728.59
10	728.42
11	728.25
12	728.08



ELEVATION
 * = TYPICAL BAY 4 THRU BAY 8
 ** = TYPICAL BAY 1 THRU BAY 3 AND BAY 9 THRU BAY 11

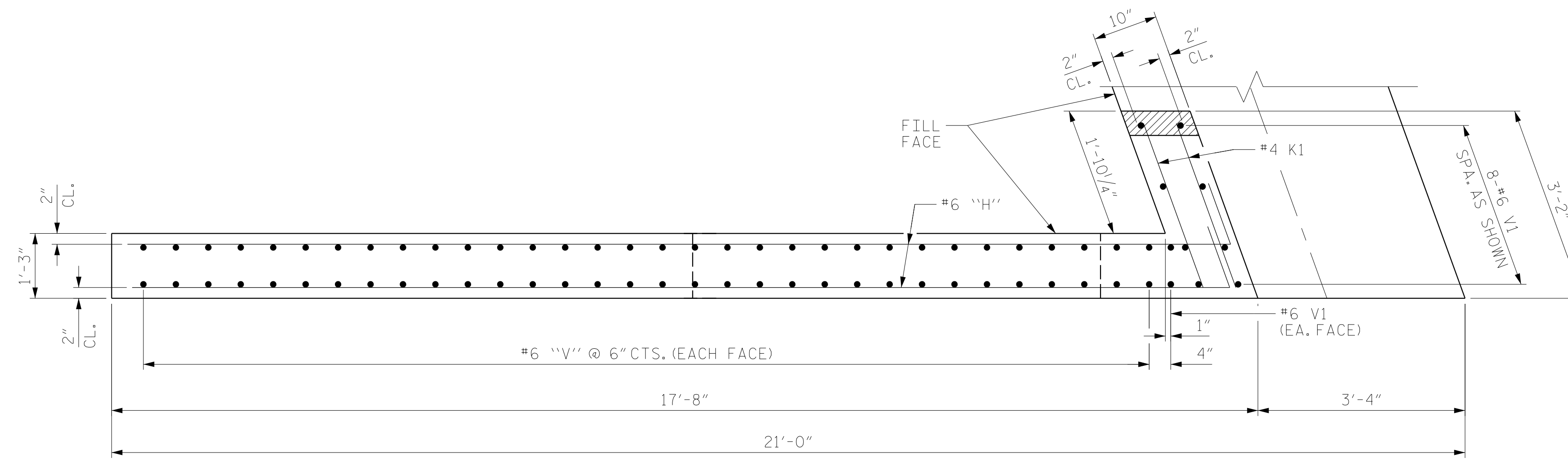
PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 1 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE					
INTEGRAL END BENT NO. 1					
LEFT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

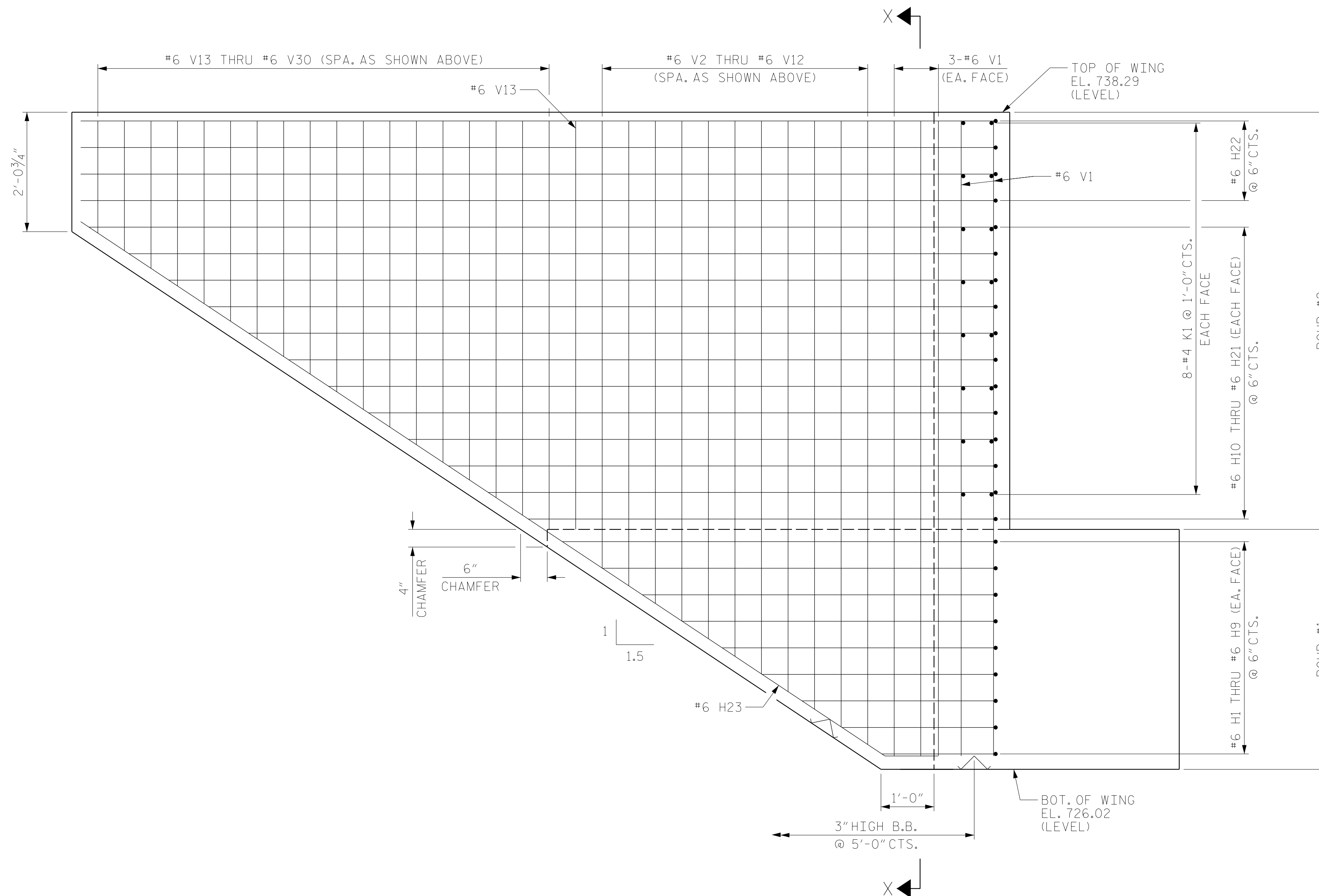
SHEET NO.	S1-23
TOTAL SHEETS	36

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 CHECKED BY: JMR DATE: 12/2016
 DESIGN ENGINEER OF RECORD: MAL DATE: 11/2016

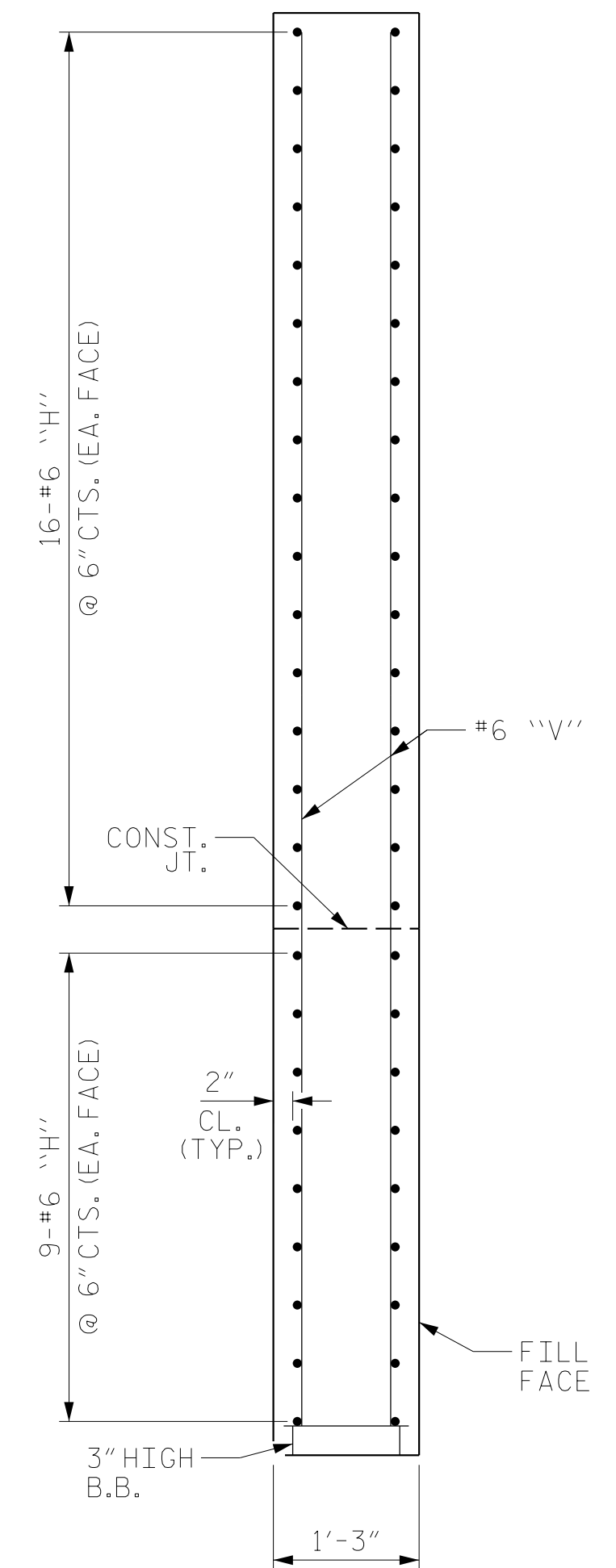
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PLAN OF WING - W2



ELEVATION OF WING - W2



SECTION X-X

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

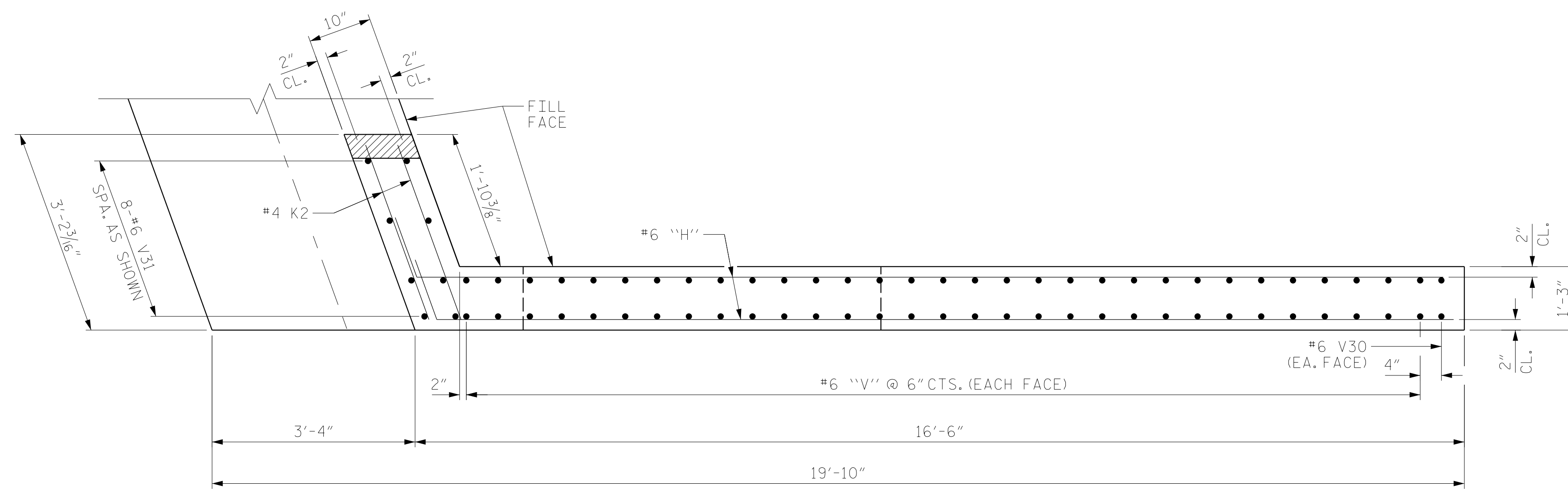
SHEET 2 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL
 END BENT NO. 1
 LEFT LANE

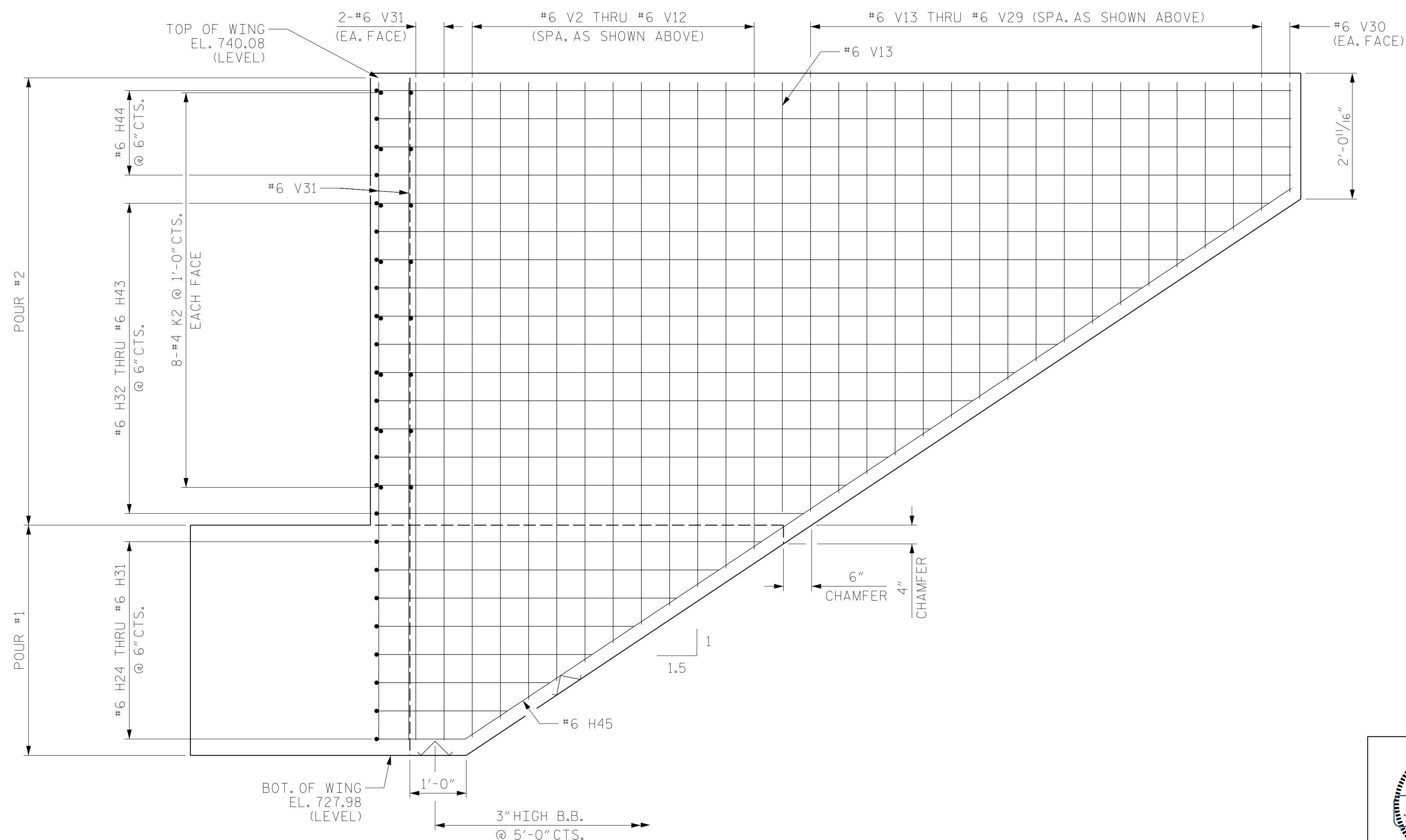
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-24
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : MAL DATE : 11/2016
 CHECKED BY : JMR DATE : 12/2016
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

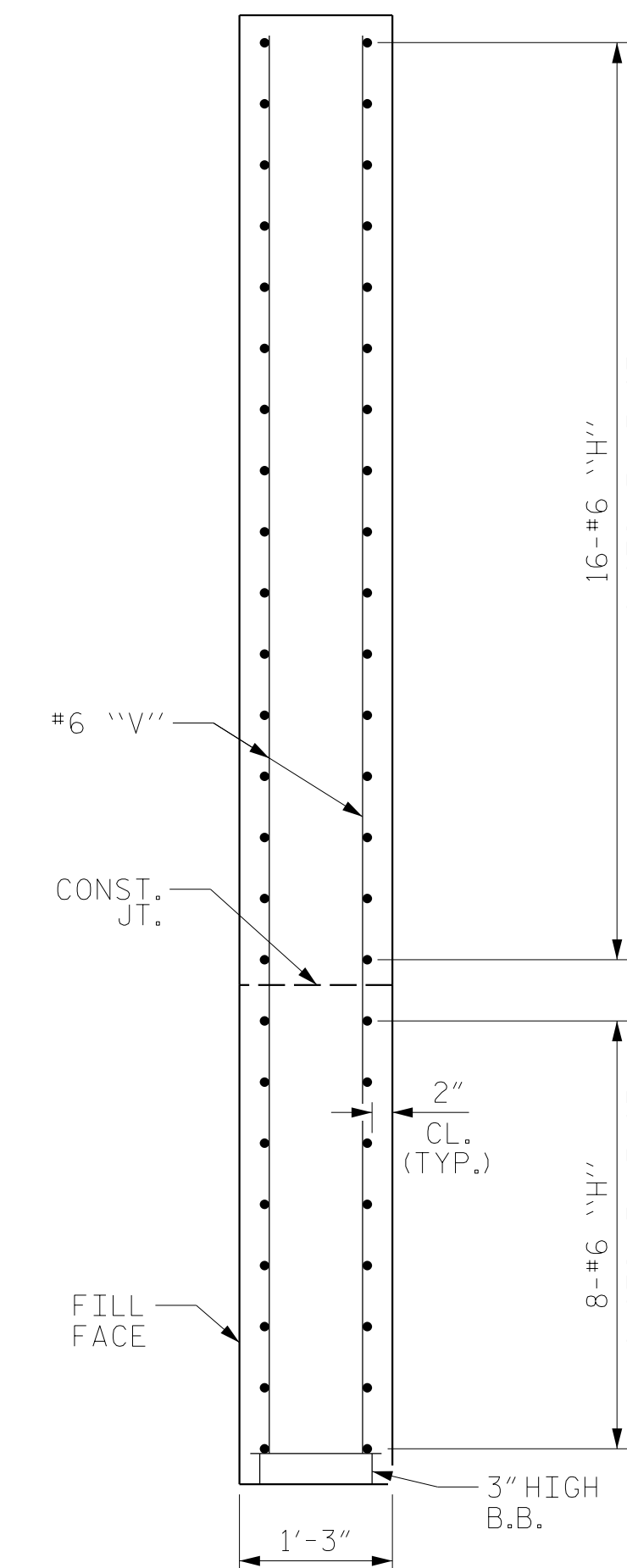
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PLAN OF WING - W1



ELEVATION OF WING - W1



SECTION Y-Y

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SHEET 3 OF 4



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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL
 END BENT NO. 1
 LEFT LANE

REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO.

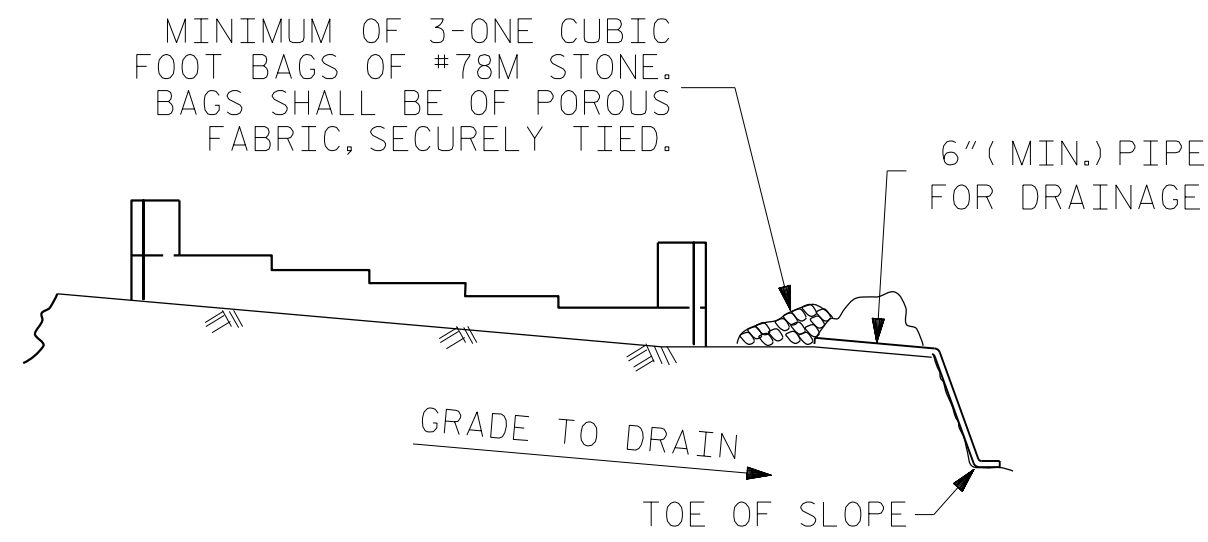
S1-25

TOTAL SHEETS

36

DRAWN BY : MAL DATE : 11/2016
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 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

DOCUMENT NOT CONSIDERED
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 SIGNATURES COMPLETED

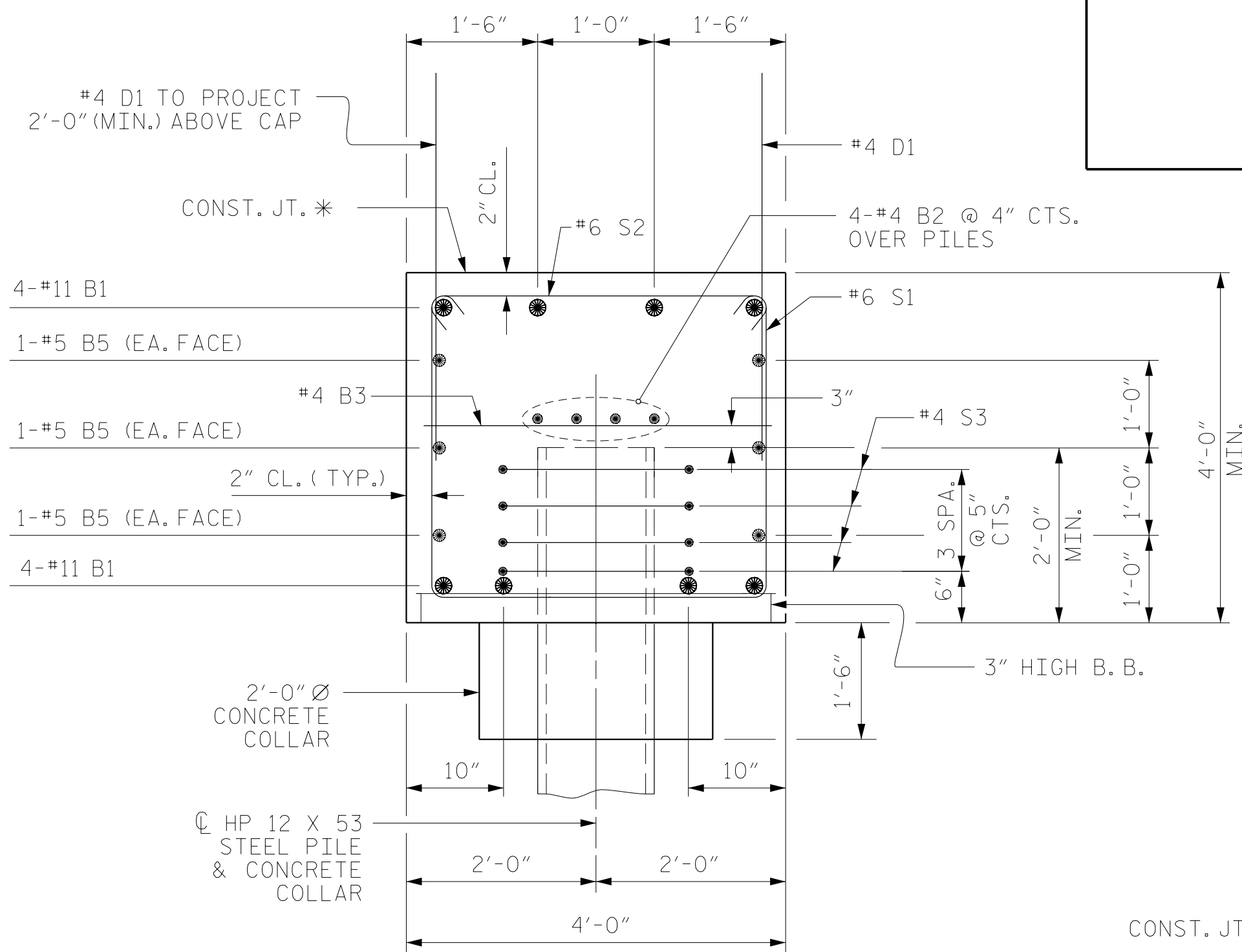


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

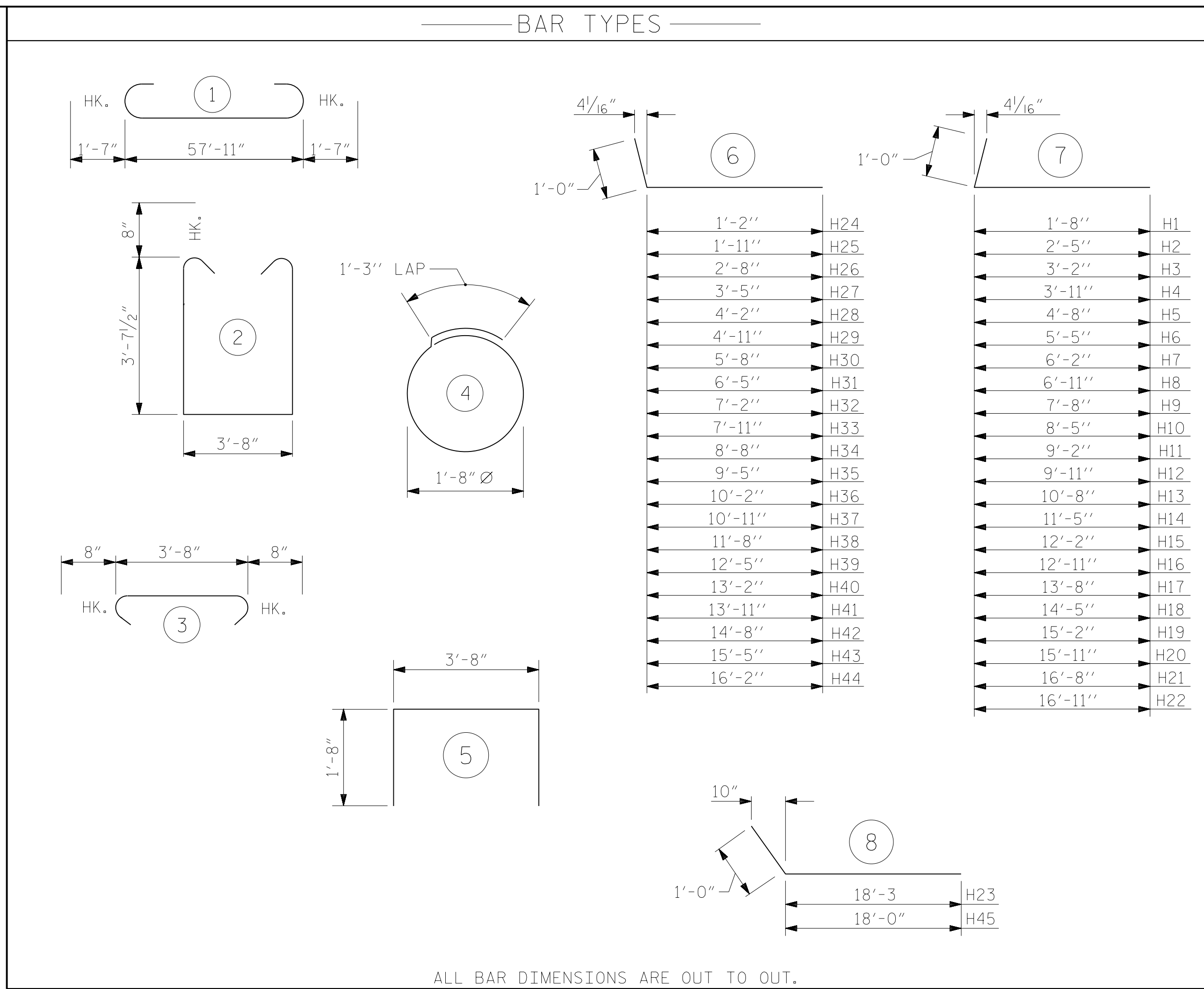
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

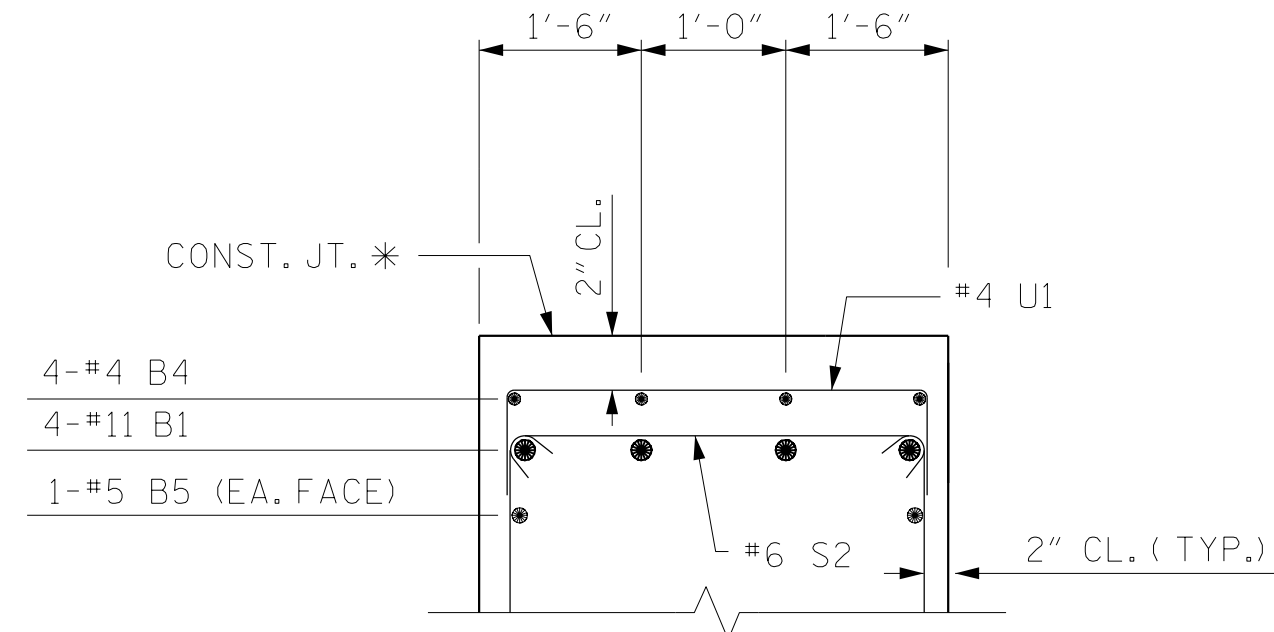


SECTION A-A

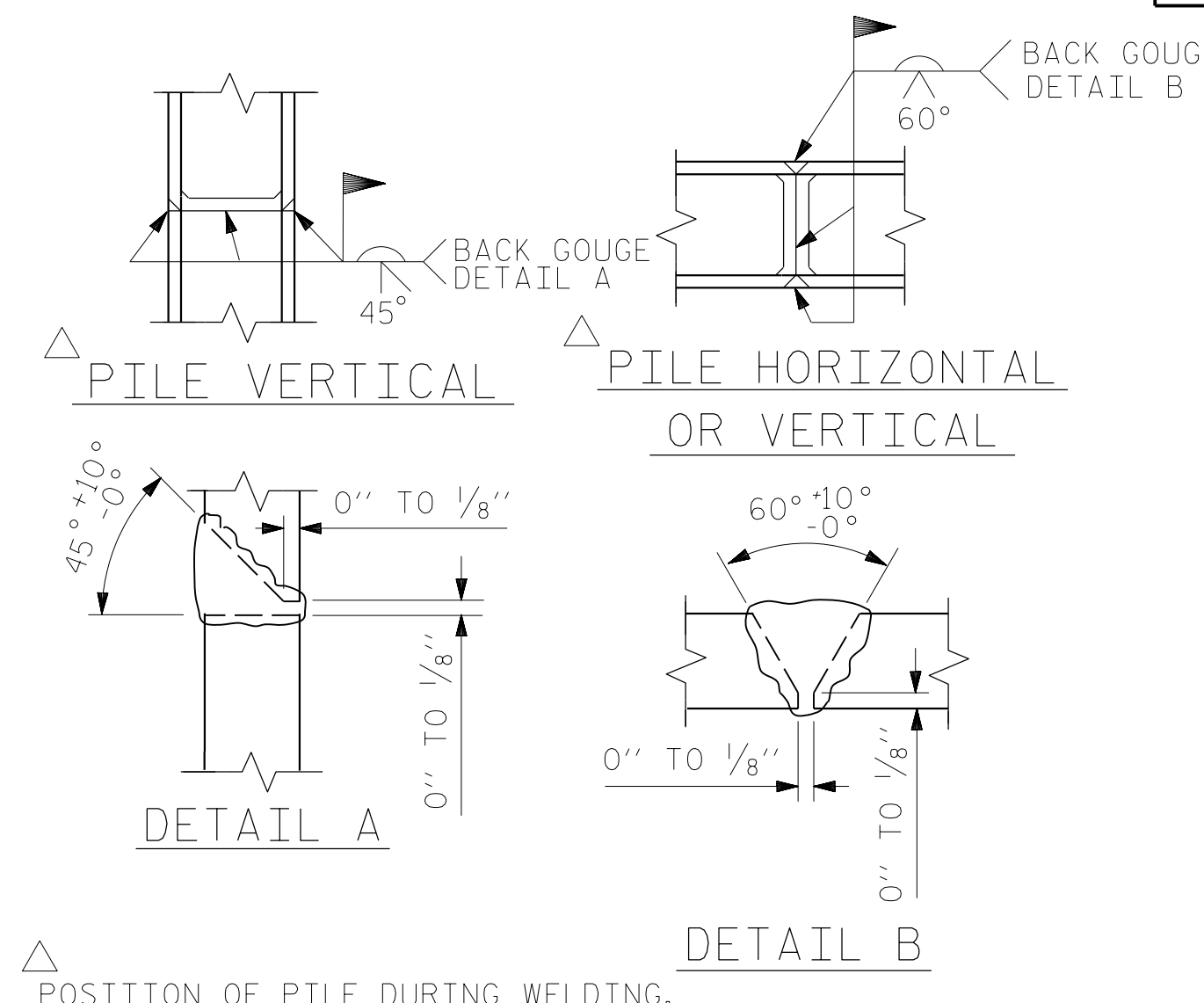
* THE TOP SURFACE OF THE END BENT CAP & WINGS, EXCLUDING THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4\".



ALL BAR DIMENSIONS ARE OUT TO OUT.



PARTIAL SECTION B-B



PILE SPLICE DETAILS

BILL OF MATERIAL

END BENT NO. 1

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#11	1	61'-11"	2596	H42	2	#6	6	15'-8"	47
B2	8	#4	STR	29'-0"	155	H43	2	#6	6	16'-5"	49
B3	14	#4	STR	3'-8"	34	H44	8	#6	6	17'-2"	206
B4	4	#4	STR	10'-0"	27	H45	2	#6	8	19'-0"	57
B5	6	#5	STR	57'-11"	362						
						K1	16	#4	STR	2'-8"	29
						K2	16	#4	STR	2'-8"	29
H1	2	#6	7	2'-8"	8	S1	85	#6	2	12'-3"	1564
H2	2	#6	7	3'-5"	10	S2	85	#6	3	5'-0"	638
H3	2	#6	7	4'-2"	13	S3	48	#4	4	6'-6"	208
H4	2	#6	7	4'-11"	15						
H5	2	#6	7	5'-8"	17	U1	7	#4	5	7'-0"	33
H6	2	#6	7	6'-5"	19						
H7	2	#6	7	7'-2"	22	V1	14	#6	STR	11'-11"	251
H8	2	#6	7	7'-11"	24	V2	4	#6	STR	11'-7"	70
H9	2	#6	7	8'-8"	26	V3	4	#6	STR	11'-3"	68
H10	2	#6	7	9'-5"	28	V4	4	#6	STR	10'-11"	66
H11	2	#6	7	10'-2"	31	V5	4	#6	STR	10'-7"	64
H12	2	#6	7	10'-11"	33	V6	4	#6	STR	10'-3"	62
H13	2	#6	7	11'-8"	35	V7	4	#6	STR	9'-11"	60
H14	2	#6	7	12'-5"	37	V8	4	#6	STR	9'-7"	58
H15	2	#6	7	13'-2"	40	V9	4	#6	STR	9'-3"	56
H16	2	#6	7	13'-11"	42	V10	4	#6	STR	8'-11"	54
H17	2	#6	7	14'-8"	44	V11	4	#6	STR	8'-7"	52
H18	2	#6	7	15'-5"	46	V12	4	#6	STR	8'-3"	50
H19	2	#6	7	16'-2"	49	V13	8	#6	STR	7'-7"	91
H20	2	#6	7	16'-11"	51	V14	4	#6	STR	7'-3"	44
H21	2	#6	7	17'-8"	53	V15	4	#6	STR	6'-11"	42
H22	8	#6	7	17'-11"	215	V16	4	#6	STR	6'-7"	40
H23	2	#6	8	19'-3"	58	V17	4	#6	STR	6'-3"	38
H24	2	#6	6	2'-2"	7	V18	4	#6	STR	5'-11"	36
H25	2	#6	6	2'-11"	9	V19	4	#6	STR	5'-7"	34
H26	2	#6	6	3'-8"	11	V20	4	#6	STR	5'-3"	32
H27	2	#6	6	4'-5"	13	V21	4	#6	STR	4'-11"	30
H28	2	#6	6	5'-2"	16	V22	4	#6	STR	4'-7"	28
H29	2	#6	6	5'-11"	18	V23	4	#6	STR	4'-3"	26
H30	2	#6	6	6'-8"	20	V24	4	#6	STR	3'-11"	24
H31	2	#6	6	7'-5"	22	V25	4	#6	STR	3'-7"	22
H32	2	#6	6	8'-2"	25	V26	4	#6	STR	3'-3"	20
H33	2	#6	6	8'-11"	27	V27	4	#6	STR	2'-11"	18
H34	2	#6	6	9'-8"	29	V28	4	#6	STR	2'-7"	16
H35	2	#6	6	10'-5"	31	V29	4	#6	STR	2'-3"	14
H36	2	#6	6	11'-2"	34	V30	4	#6	STR	2'-0"	12
H37	2	#6	6	11'-11"	36	V31	12	#6	STR	11'-8"	210
H38	2	#6	6	12'-8"	38						
H39	2	#6	6	13'-5"	40						
H40	2	#6	6	14'-2"	43						
H41	2	#6	6	14'-11"	45						

REINFORCING STEEL	9,348 LBS.
CLASS A CONCRETE	
POUR #1	
CAP, CONCRETE COLLARS & LOWER PART OF WINGS	40.9 C. Y.
POUR #2	
UPPER PART OF WINGS	11.3 C. Y.
TOTAL	52.2 C. Y.
HP 12 X 53 STEEL PILES	
NO. 12	480 LIN. FT.
STEEL PILE POINTS	NO.: 12

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CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 4 OF 4

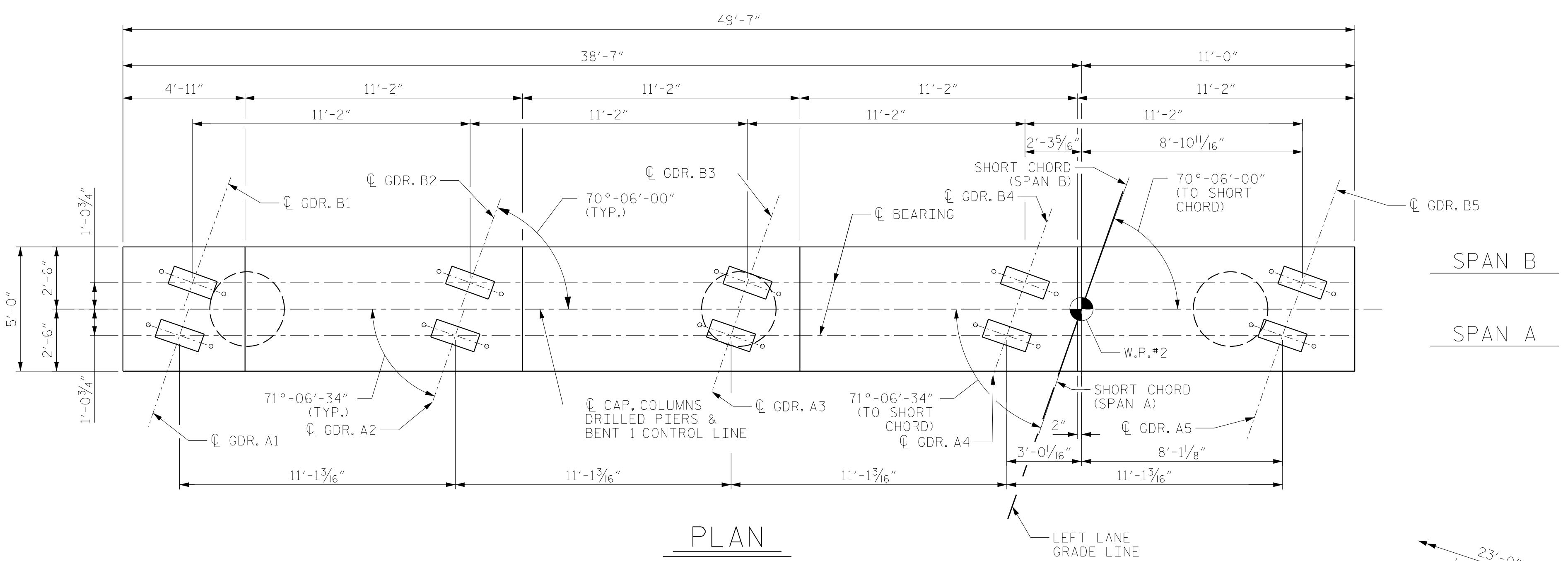
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL
 END BENT NO. 1
 LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-26
1			3			TOTAL SHEETS
2			4			36

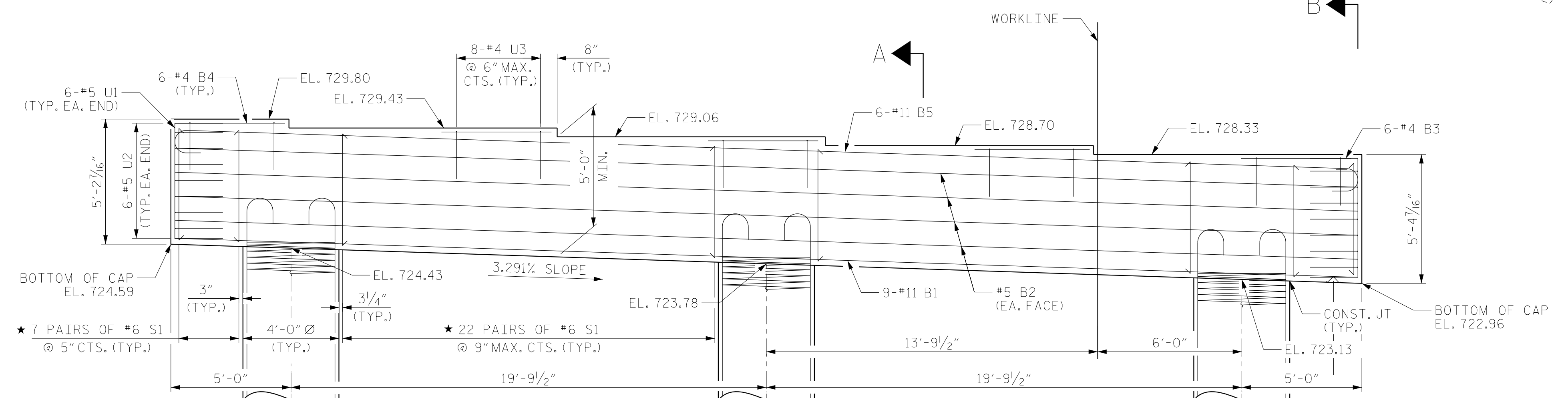
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CHECKED BY :	JMR	DATE :	12/2016
DESIGN ENGINEER OF RECORD:	MAL	DATE :	11/2016

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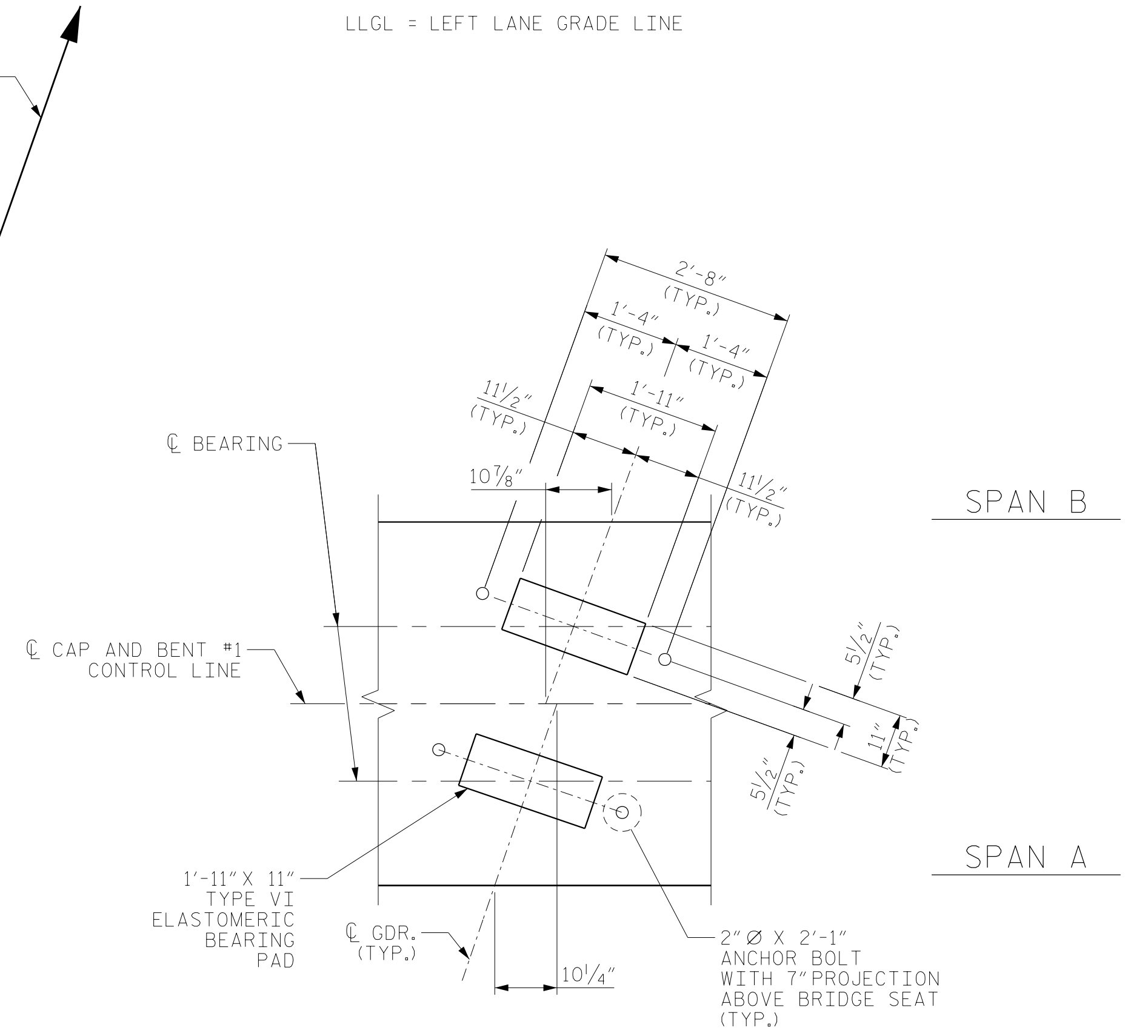
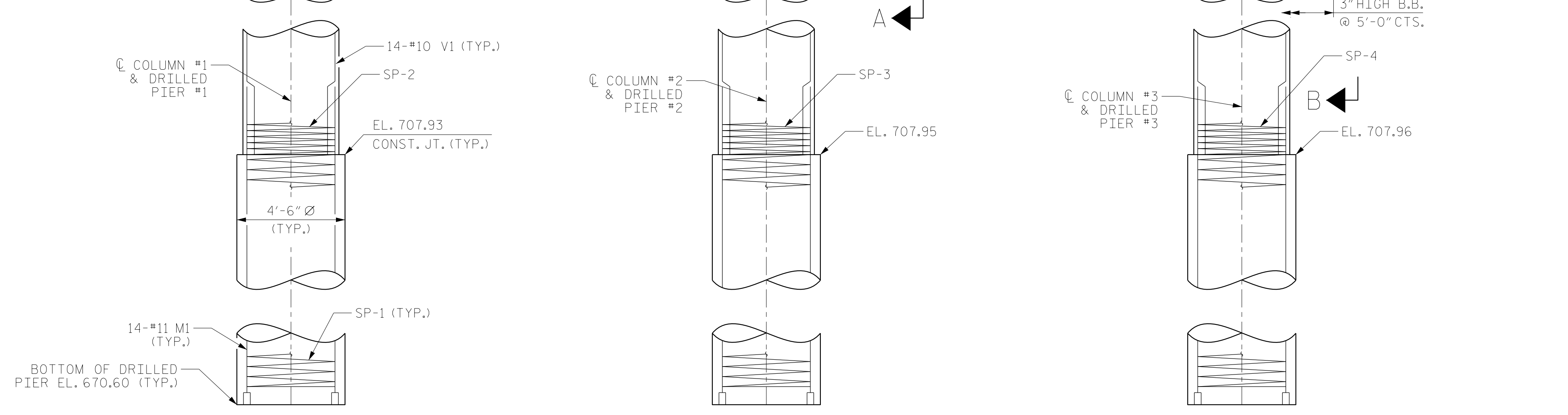
NOTES:
 FOR SECTION A-A AND VIEW B-B, SEE SHEET 2 OF 2.
 FOR REINFORCING BILL OF MATERIAL, SEE SHEET 2 OF 2.
 STIRRUPS AND U3 BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.
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 * INVERT ALTERNATE #6 S1 STIRRUP PAIRS.
 THE CONTRACTORS ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.
 THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT 1 FOOT BELOW THE GROUND LINE.
 LLGL = LEFT LANE GRADE LINE



PLAN



ELEVATION



DETAIL A

DIMENSIONS ARE TYPICAL FOR EACH GIRDER

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 1 OF 2



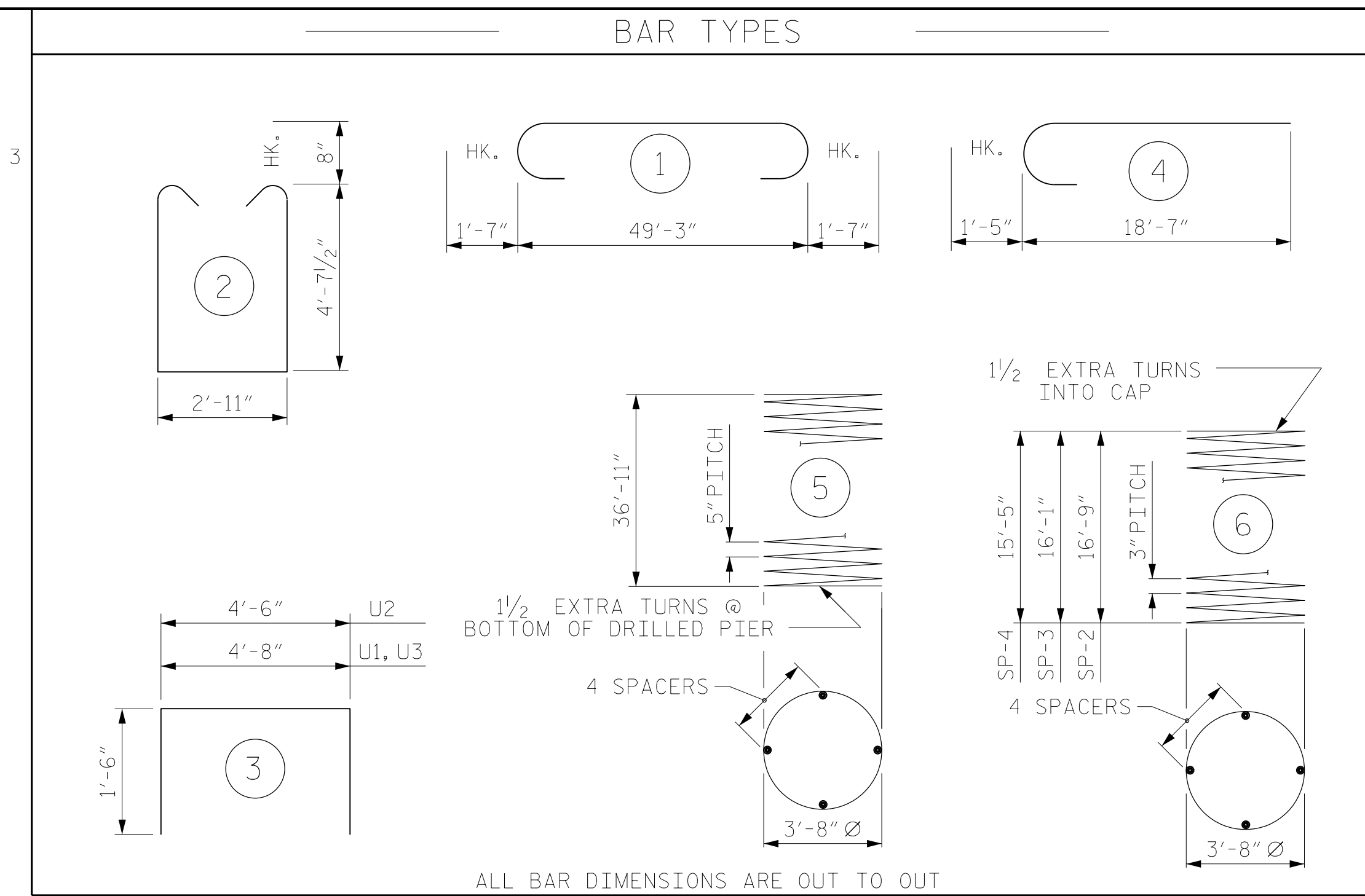
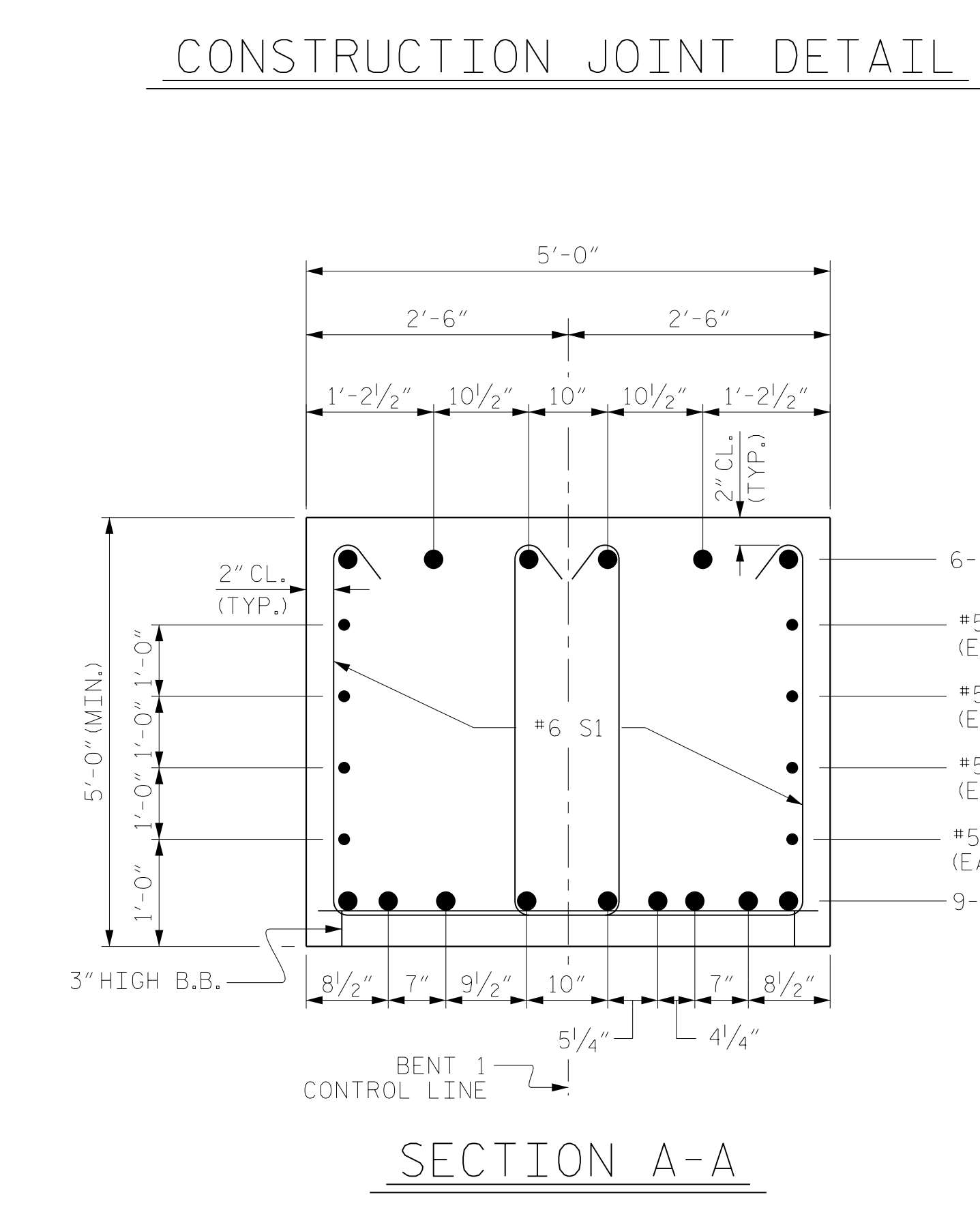
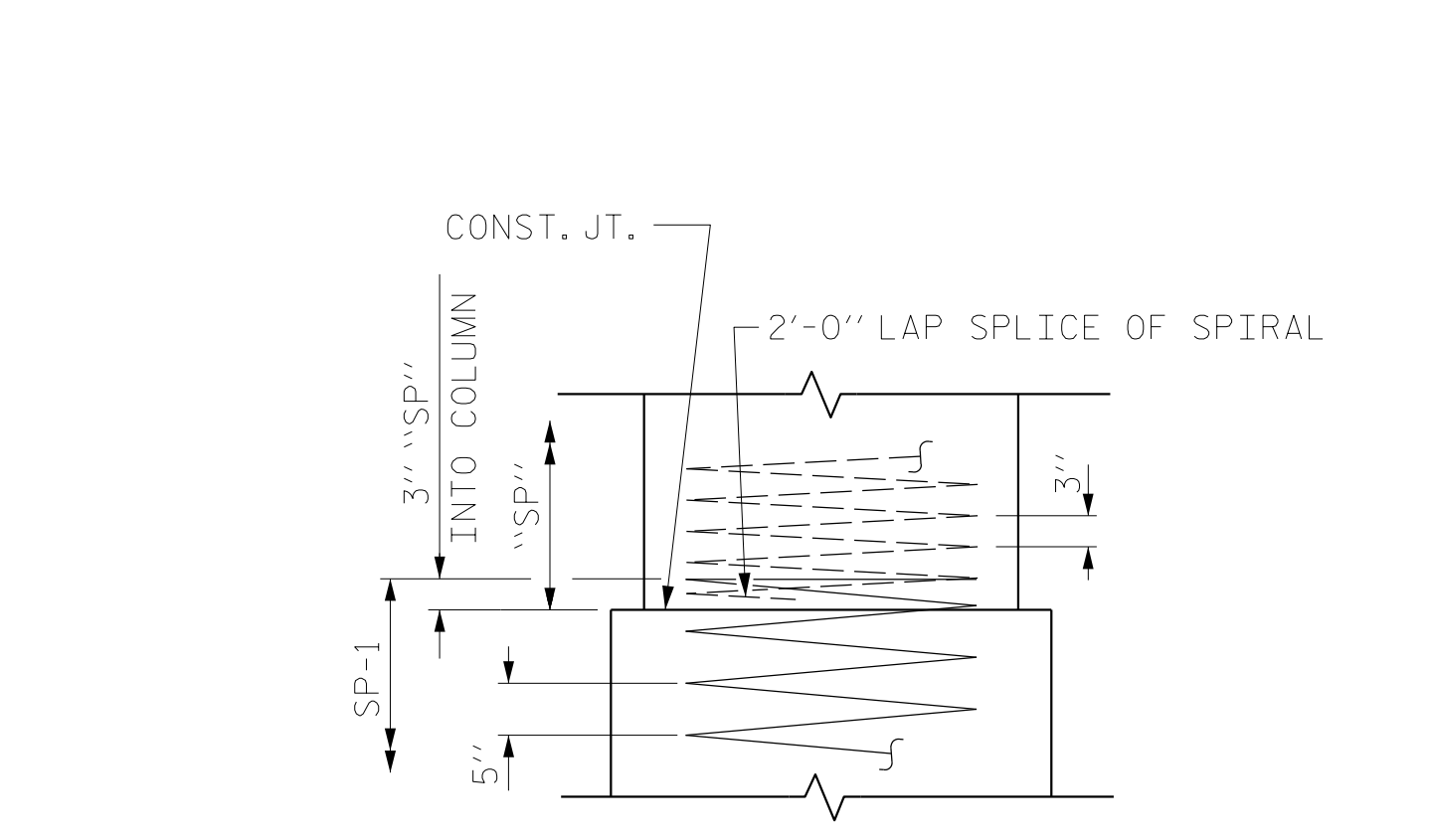
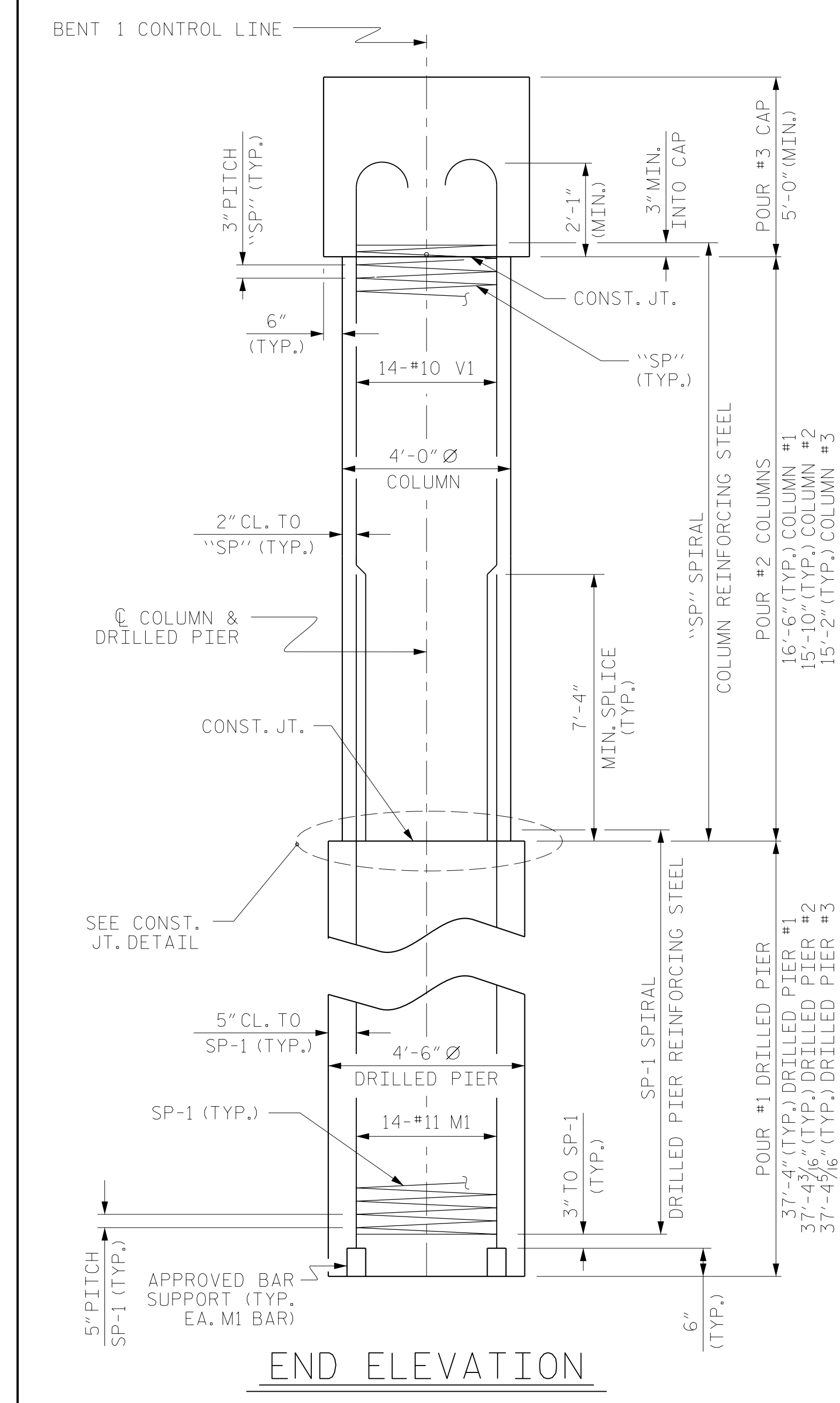
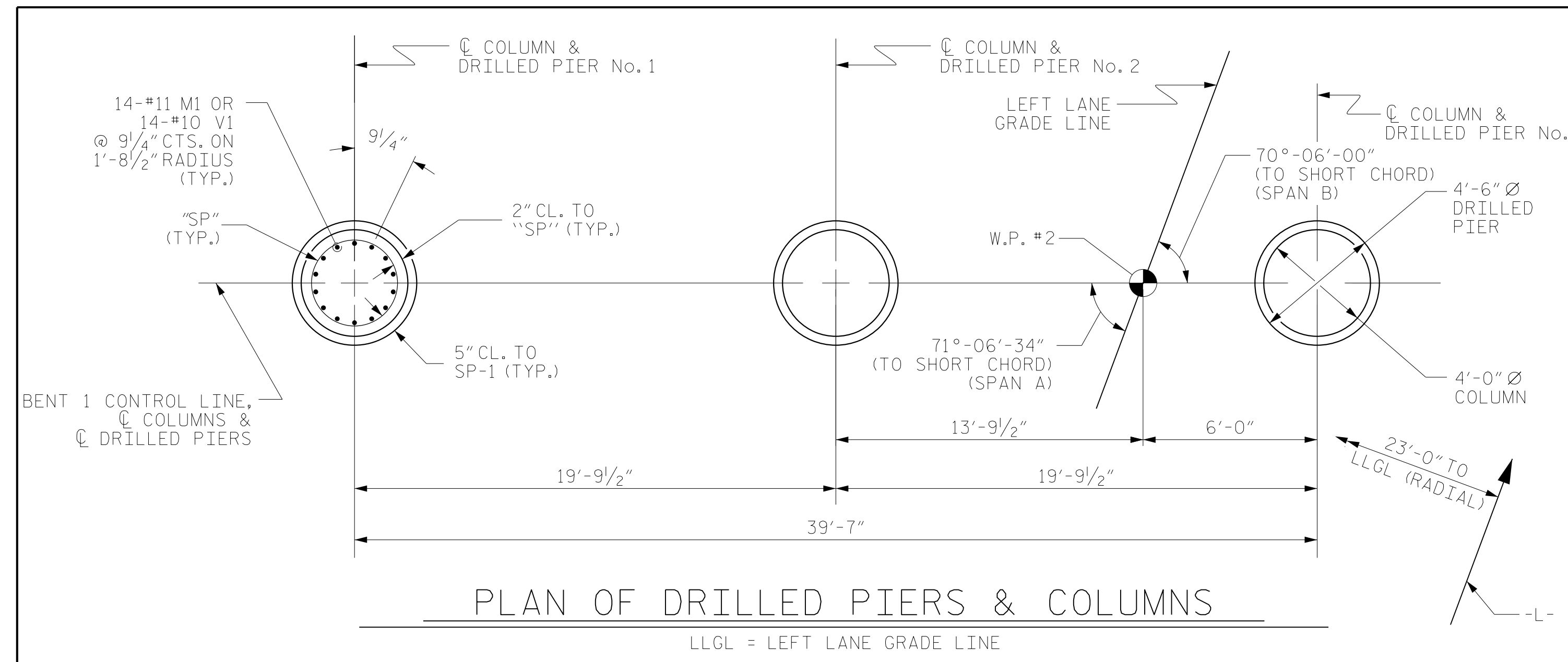
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 RALEIGH
 SUBSTRUCTURE
 BENT NO. 1
 LEFT LANE

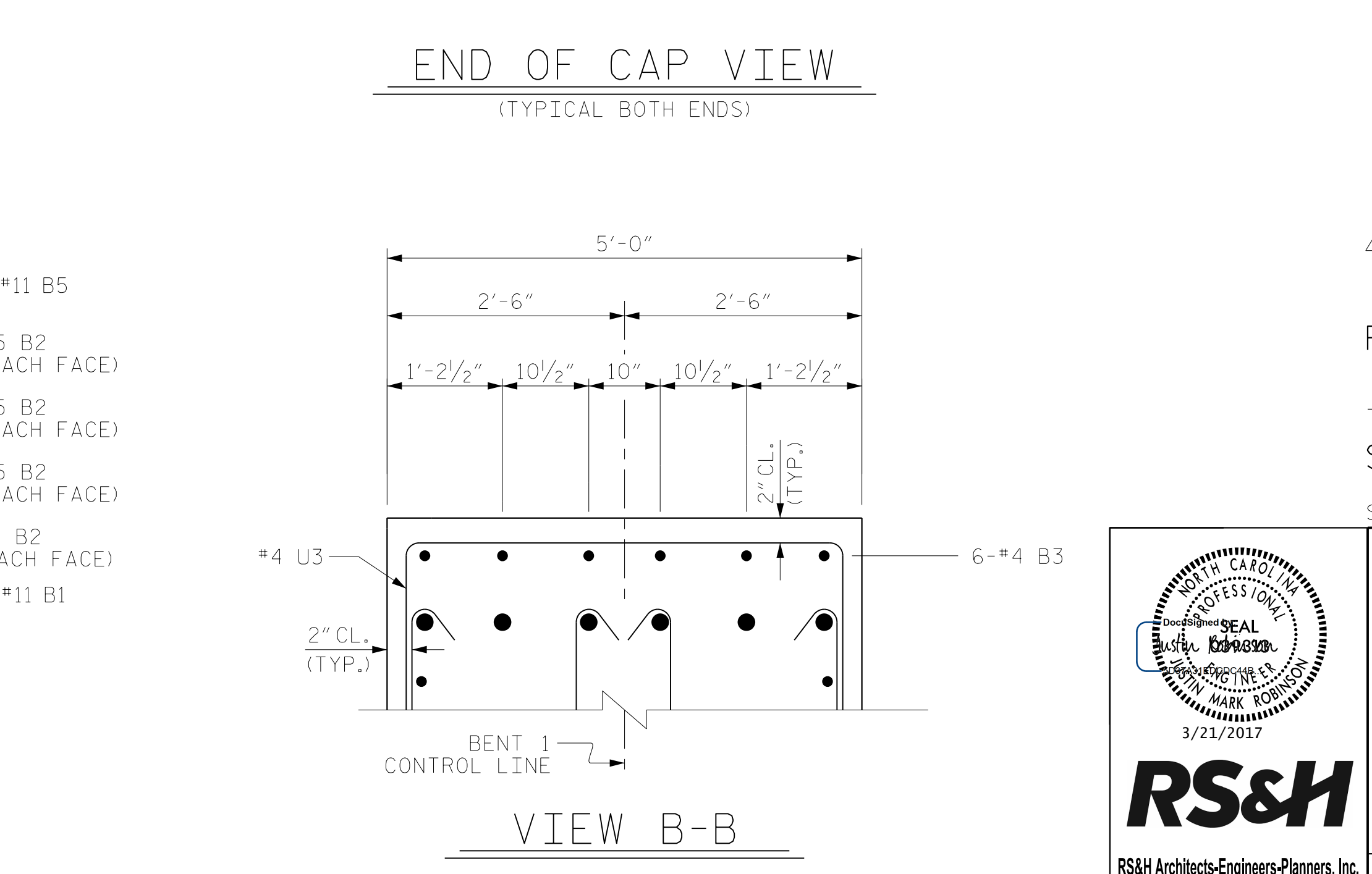
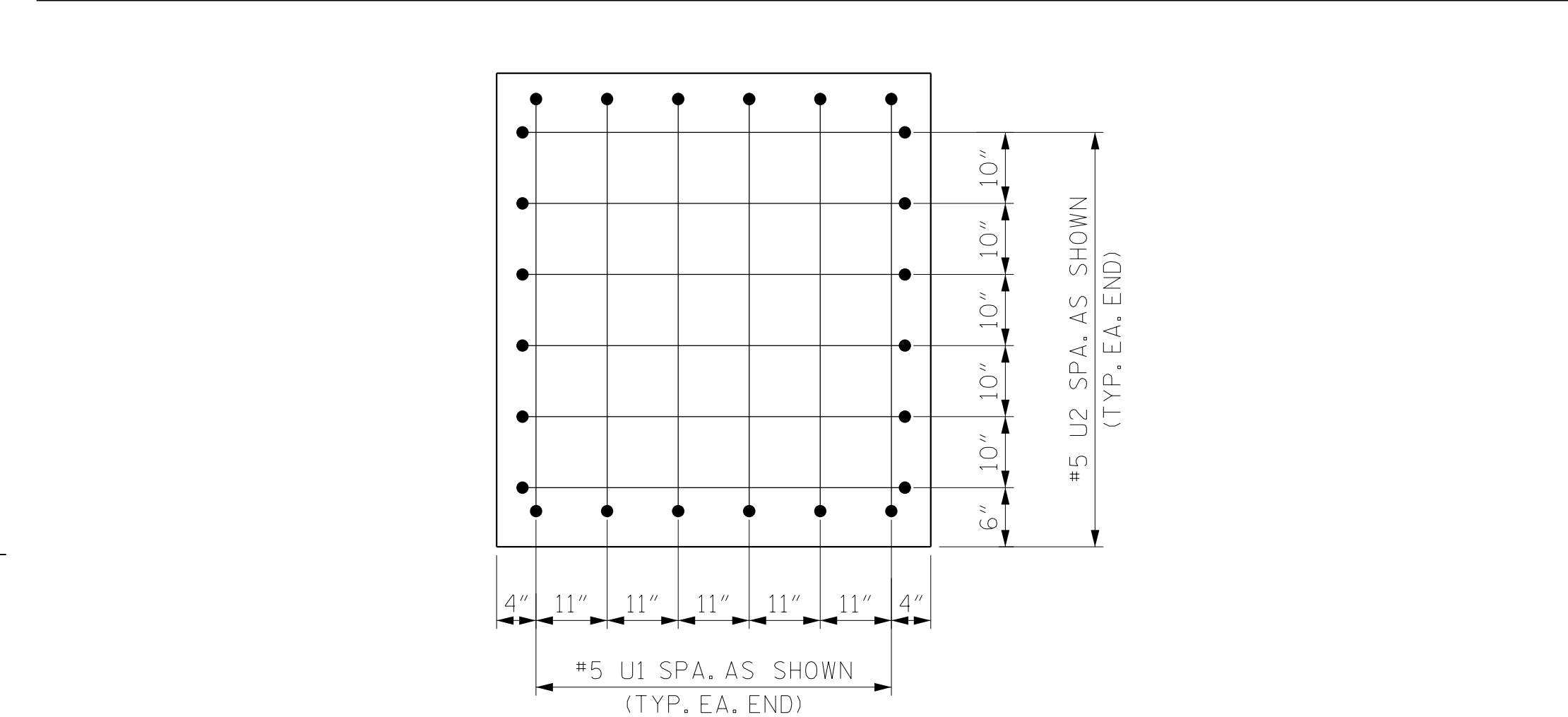
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-27
1			3			TOTAL SHEETS
2			4			36

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BILL OF MATERIAL FOR BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	9	#11	STR	49'-3"	2355
B2	8	#5	STR	49'-3"	411
B3	6	#4	STR	6'-0"	24
B4	30	#4	STR	4'-7"	92
B5	6	#11	1	52'-5"	1671
M1	42	#11	STR	47'-3"	10544
S1	116	#6	2	13'-6"	2352
U1	12	#5	3	7'-8"	96
U2	12	#5	3	7'-6"	94
U3	40	#4	3	7'-8"	205
V1	42	#10	4	20'-0"	3615
REINFORCING STEEL (FOR BENT 1)					21,459 LBS.
SP-1	3	**	5	1022'-9"	3200
SP-2	1	*	6	778'-1"	520
SP-3	1	*	6	751'-10"	502
SP-4	1	*	6	718'-5"	480
SPIRAL COLUMN REINFORCING STEEL (FOR BENT 1)					4,702 LBS.
** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR					
* THE "SP" SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR					
CLASS A CONCRETE BREAKDOWN (FOR BENT 1)					
POUR #2 (COLUMNS)					22.1 C.Y.
POUR #3 (CAP)					47.7 C.Y.
TOTAL CLASS A CONCRETE					69.8 C.Y.
DRILLED PIERS: (FOR BENT 1)					
DRILLED PIER CONCRETE POUR #1 (DRILLED PIERS)					66.0 C.Y.
4'-6" Ø DRILLED PIER NOT IN SOIL					27.0 LIN. FT.
4'-6" Ø DRILLED PIER IN SOIL					85.0 LIN. FT.
PERMANENT STEEL CASING FOR 4'-6" Ø DRILLED PIER					54 LIN. FT.
Δ CSL TUBES					466.0 LIN. FT.



Δ NO SEPARATE PAYMENT WILL BE MADE FOR CSL TUBES. CSL TUBES WILL BE INCLUDED IN THE UNIT BID PRICE FOR DRILLED PIERS.

PROJECT NO. R-2707C
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 STATION: 384+50.00 -L-

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3/21/2017
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REVISIONS

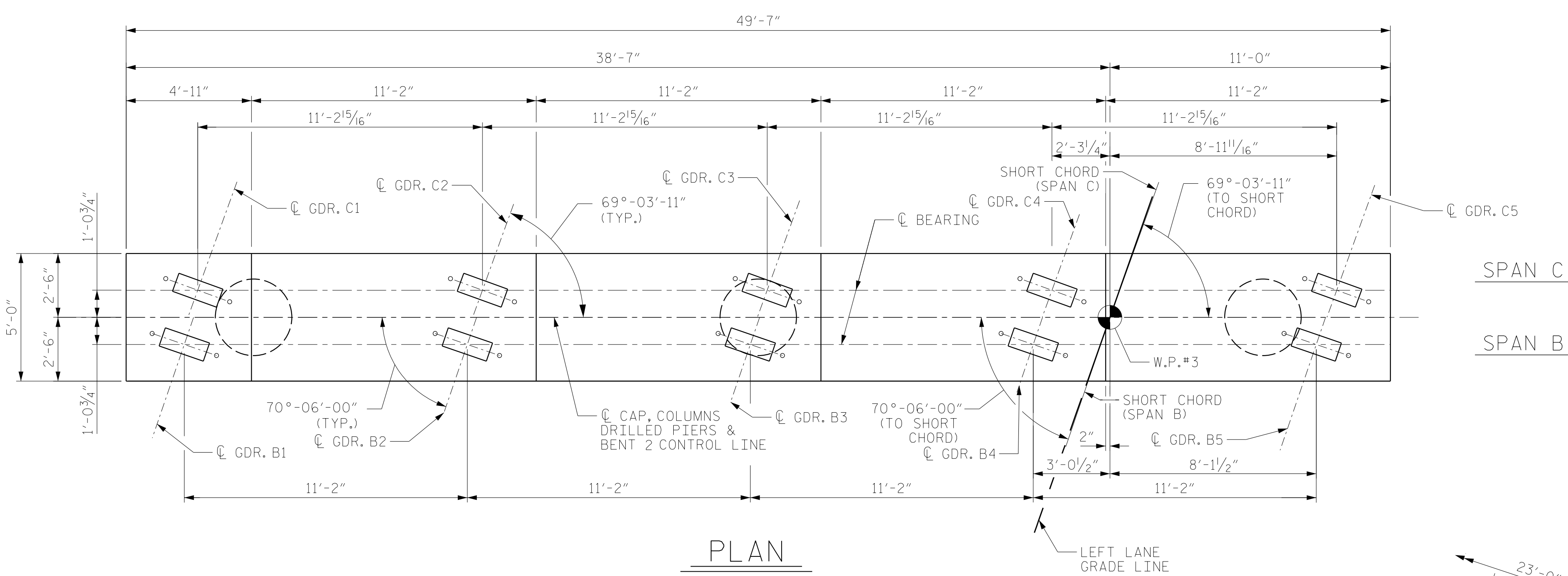
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2			4		

SHEET NO. S1-28
 TOTAL SHEETS 36

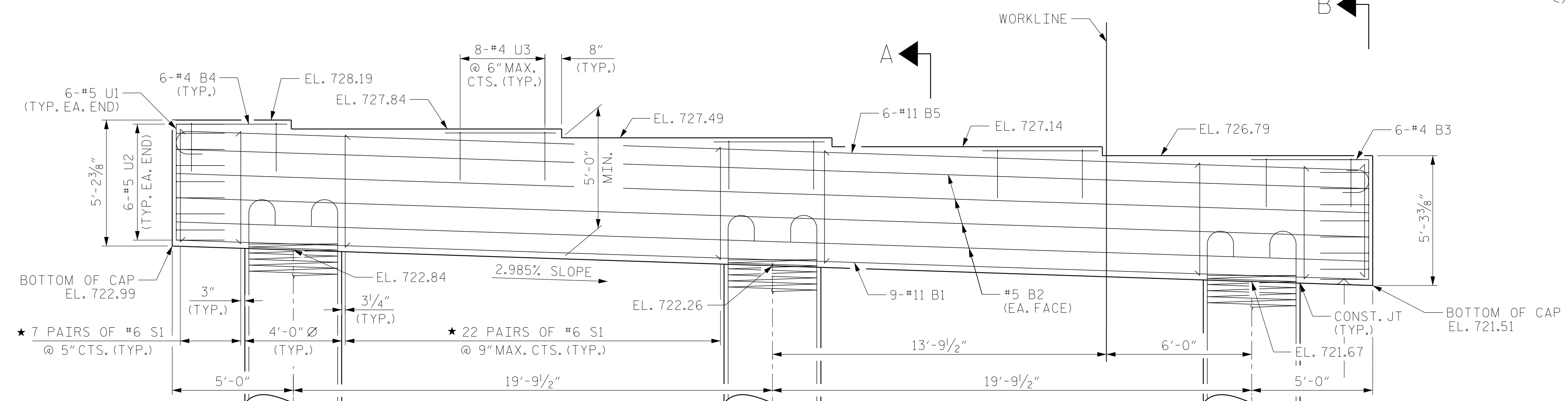


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 BENT NO. 1
 DETAILS
 LEFT LANE

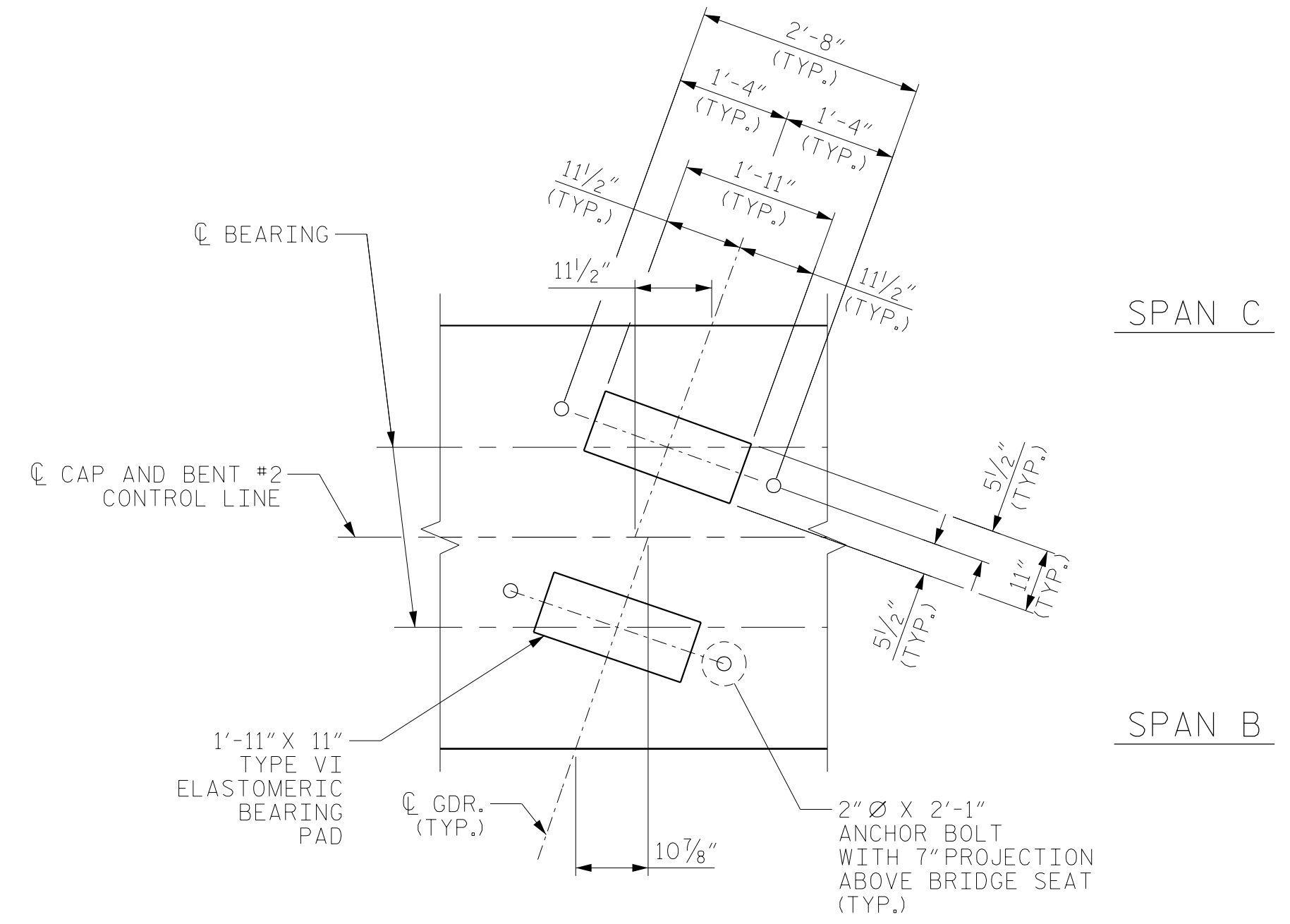
NOTES:
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 LLGL = LEFT LANE GRADE LINE



PLAN



ELEVATION



DETAIL A

DIMENSIONS ARE TYPICAL FOR EACH GIRDER

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CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 1 OF 2

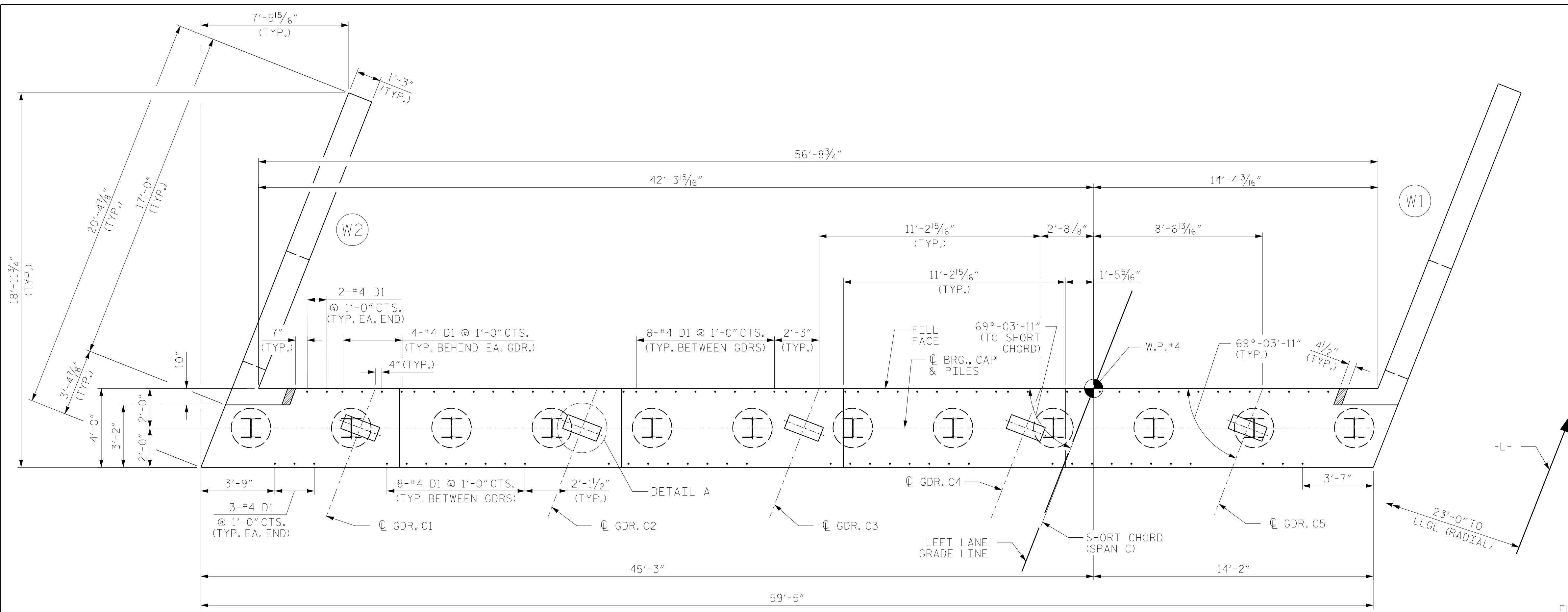


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
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 SUBSTRUCTURE
 BENT NO. 2
 LEFT LANE

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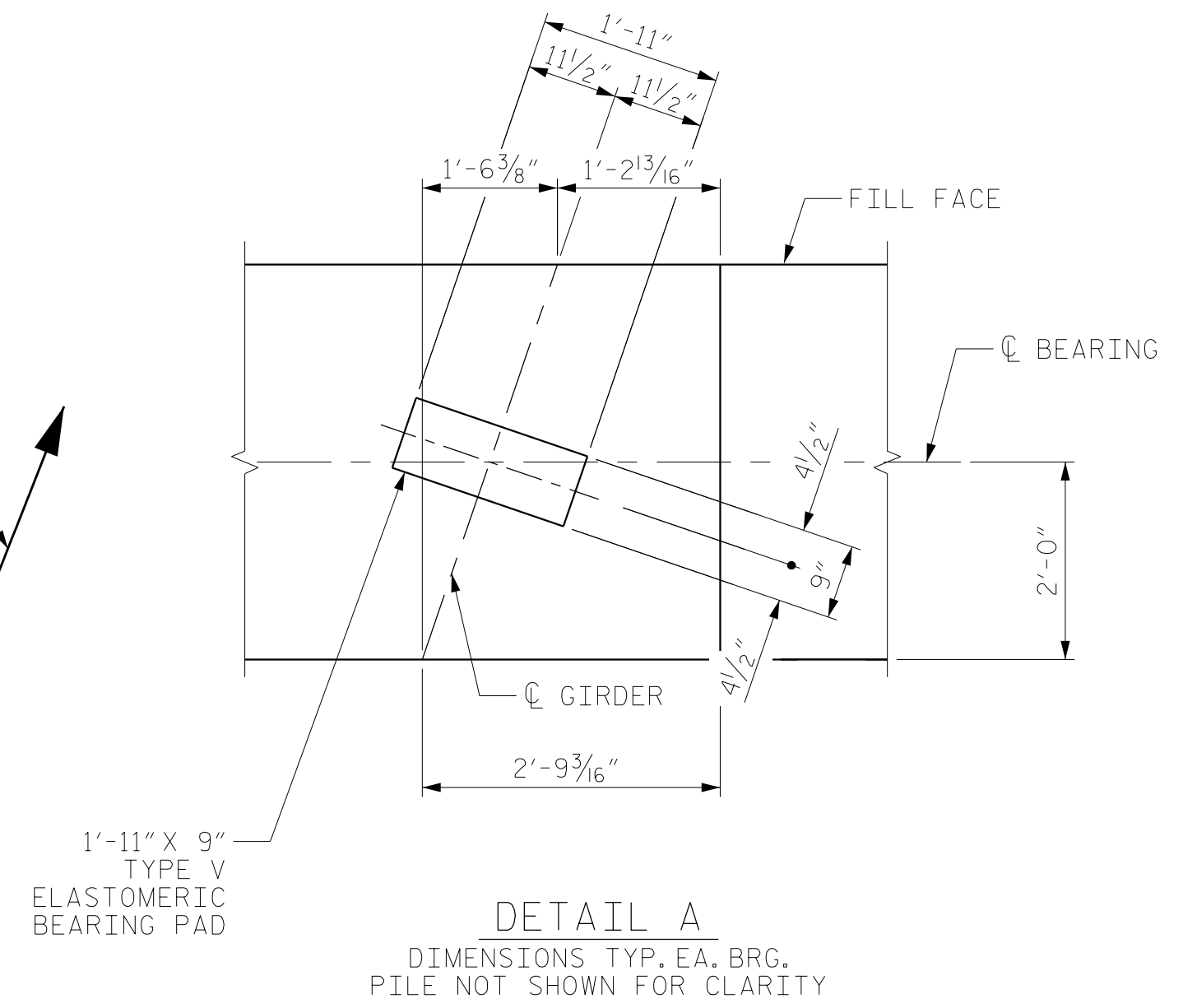
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-29
1			3			TOTAL SHEETS
2			4			36



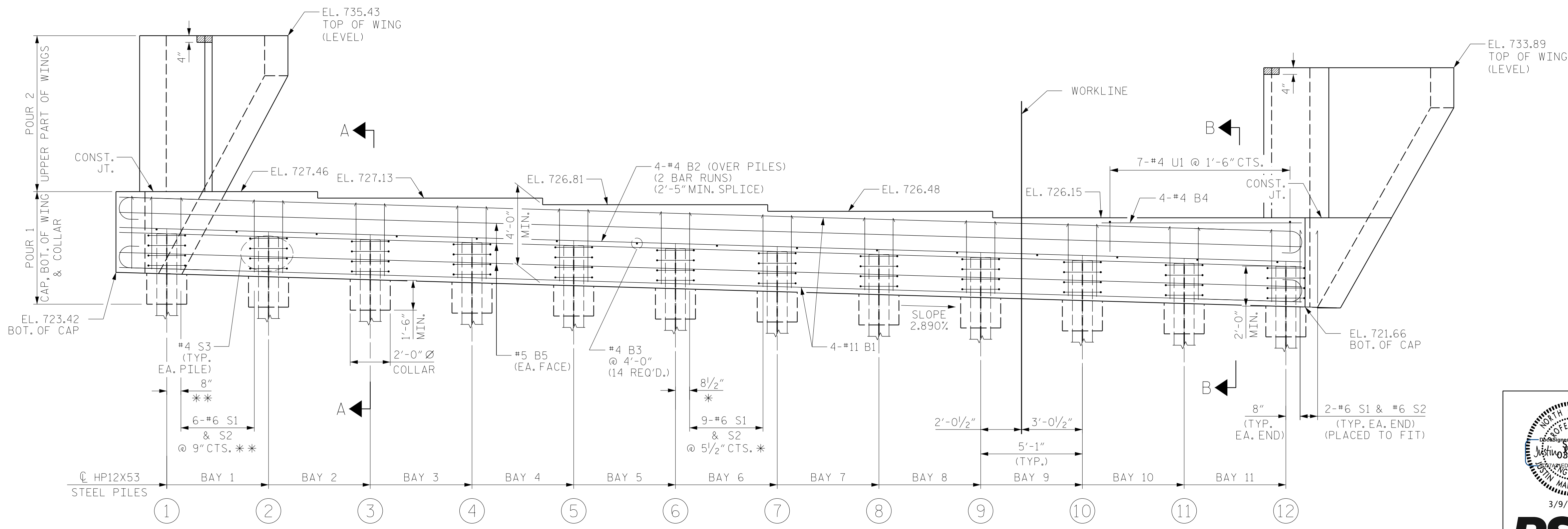
PLAN
LLGL = LEFT LANE GRADE LINE

NOTES:
 THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS. SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.
 FOR SECTION A-A AND SECTION B-B, SEE SHEET 4 OF 4.
 STIRRUPS MAY BE SHIFTED SLIGHTLY TO AVOID #4 DI BARS IN CAP.
 SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.
 THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE CONCRETE BARRIER IS CAST IF SLIP FORMING IS USED.



DETAIL A
DIMENSIONS TYP. EA. BRG. PILE NOT SHOWN FOR CLARITY

TOP OF PILE ELEVATIONS	
PILE	ELEVATION
1	725.37
2	725.22
3	725.07
4	724.93
5	724.78
6	724.63
7	724.48
8	724.33
9	724.18
10	724.04
11	723.89
12	723.74



ELEVATION

* = TYPICAL BAY 4 THRU BAY 8
 ** = TYPICAL BAY 1 THRU BAY 3 AND BAY 9 THRU BAY 11

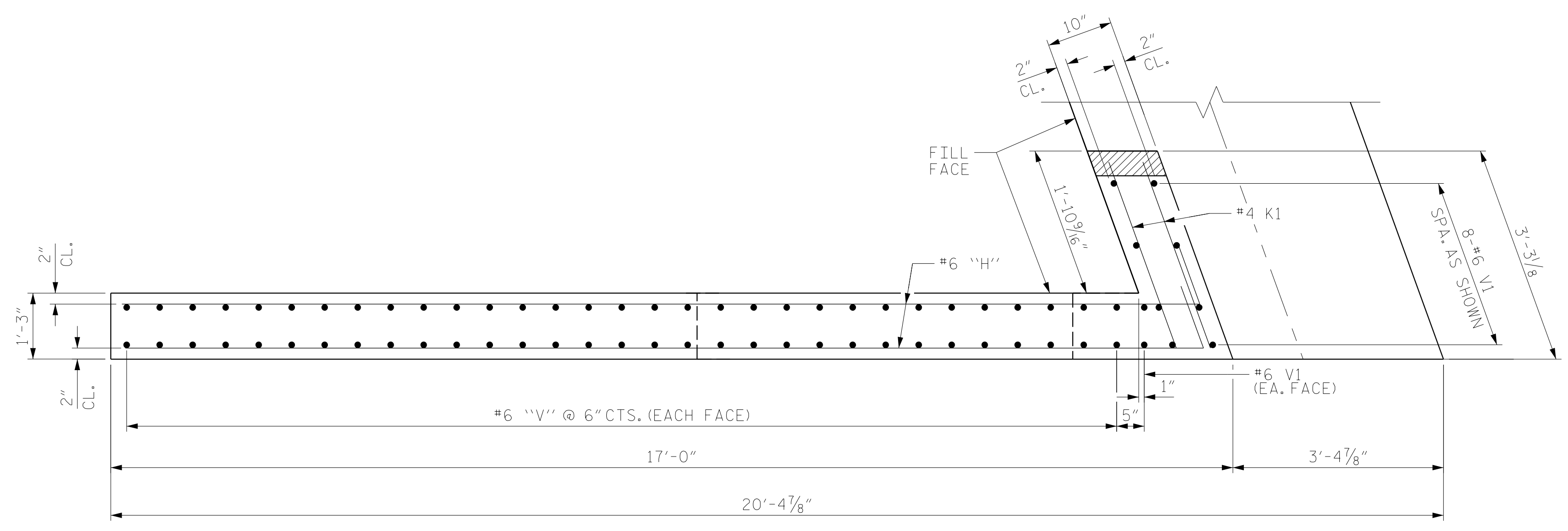
PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 1 OF 4



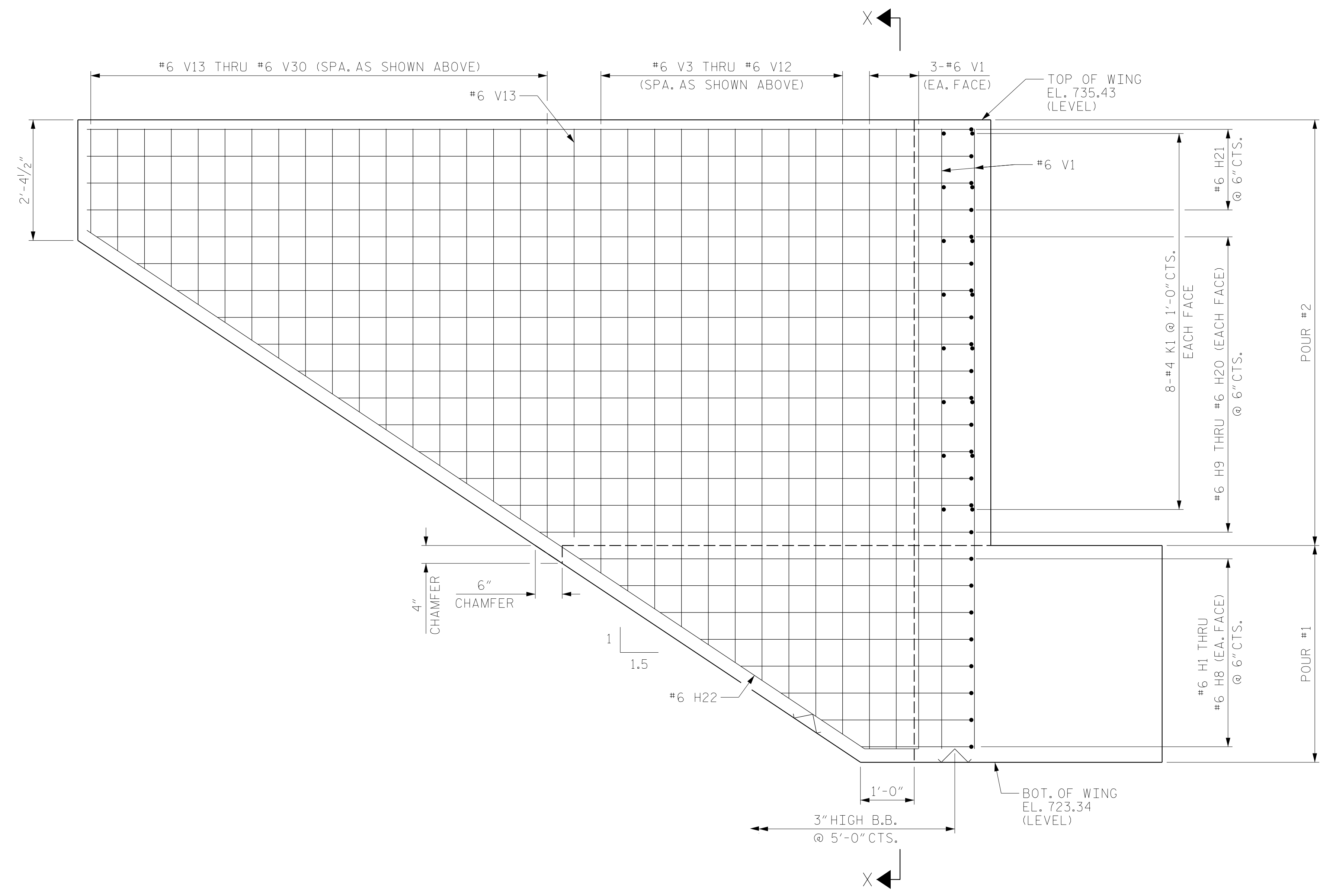
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE INTEGRAL END BENT NO. 2 LEFT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

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 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

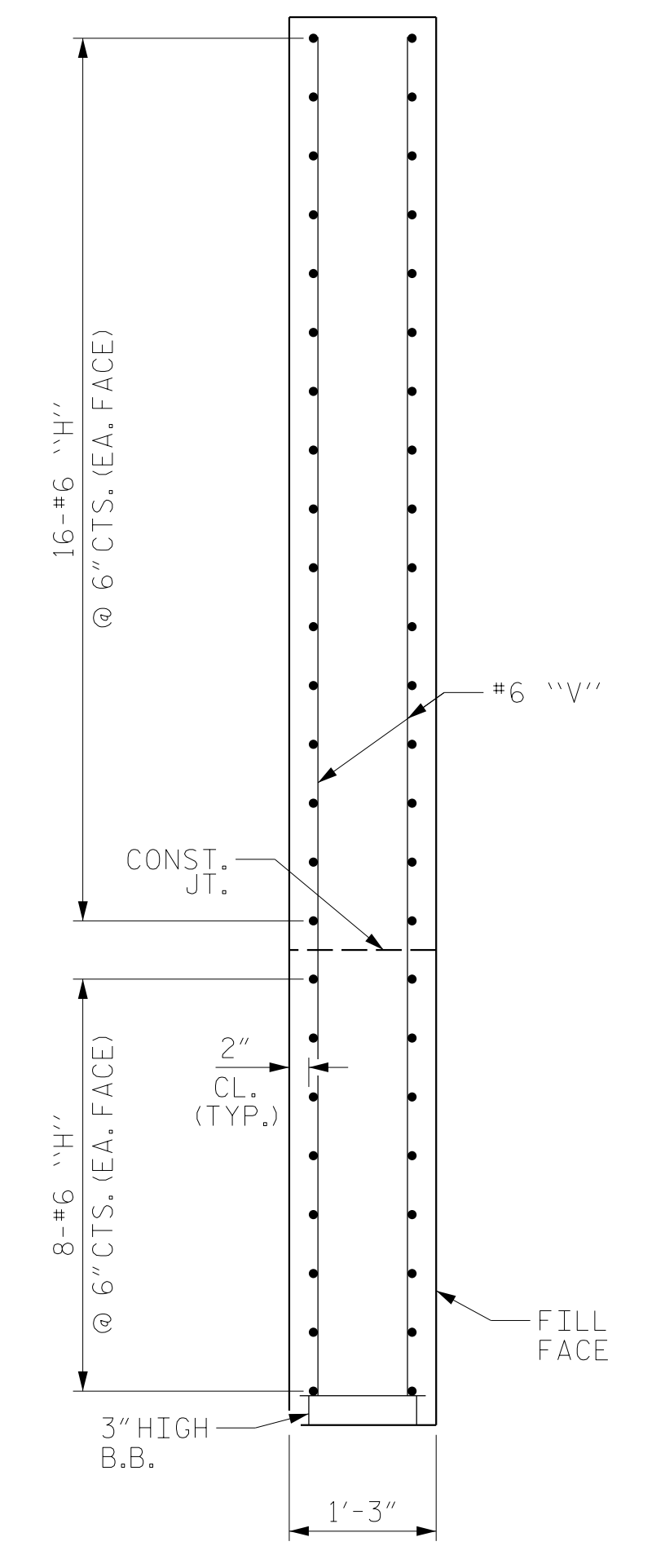
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PLAN OF WING - W2



ELEVATION OF WING - W2



SECTION X-X

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 2 OF 4

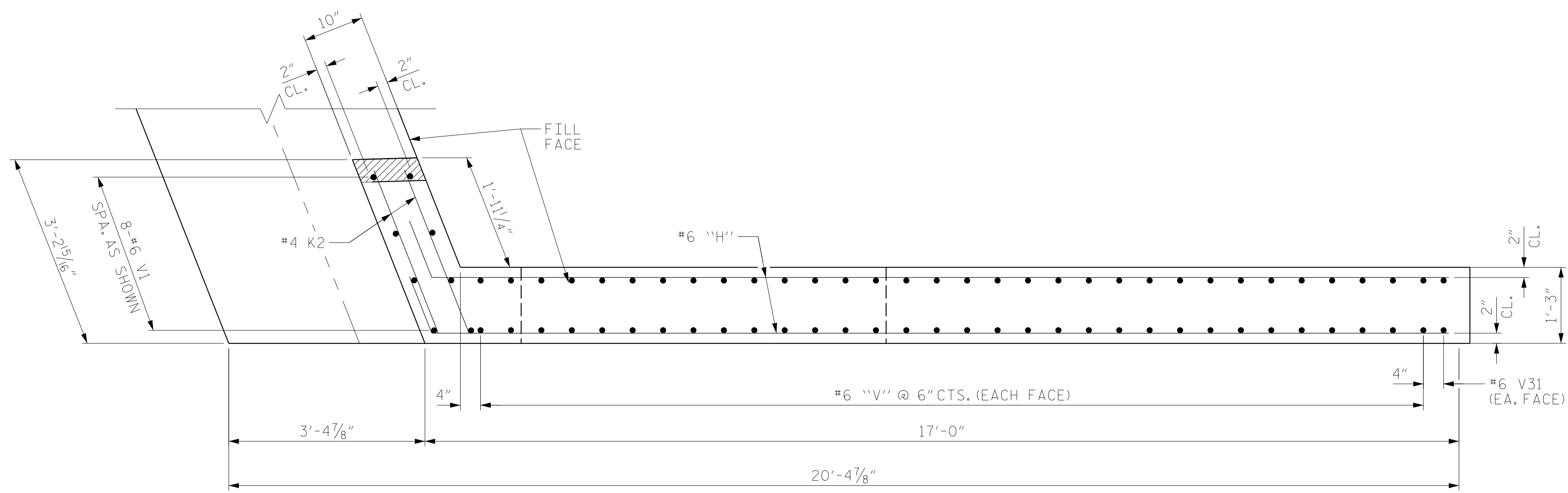


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL
 END BENT NO. 2
 LEFT LANE

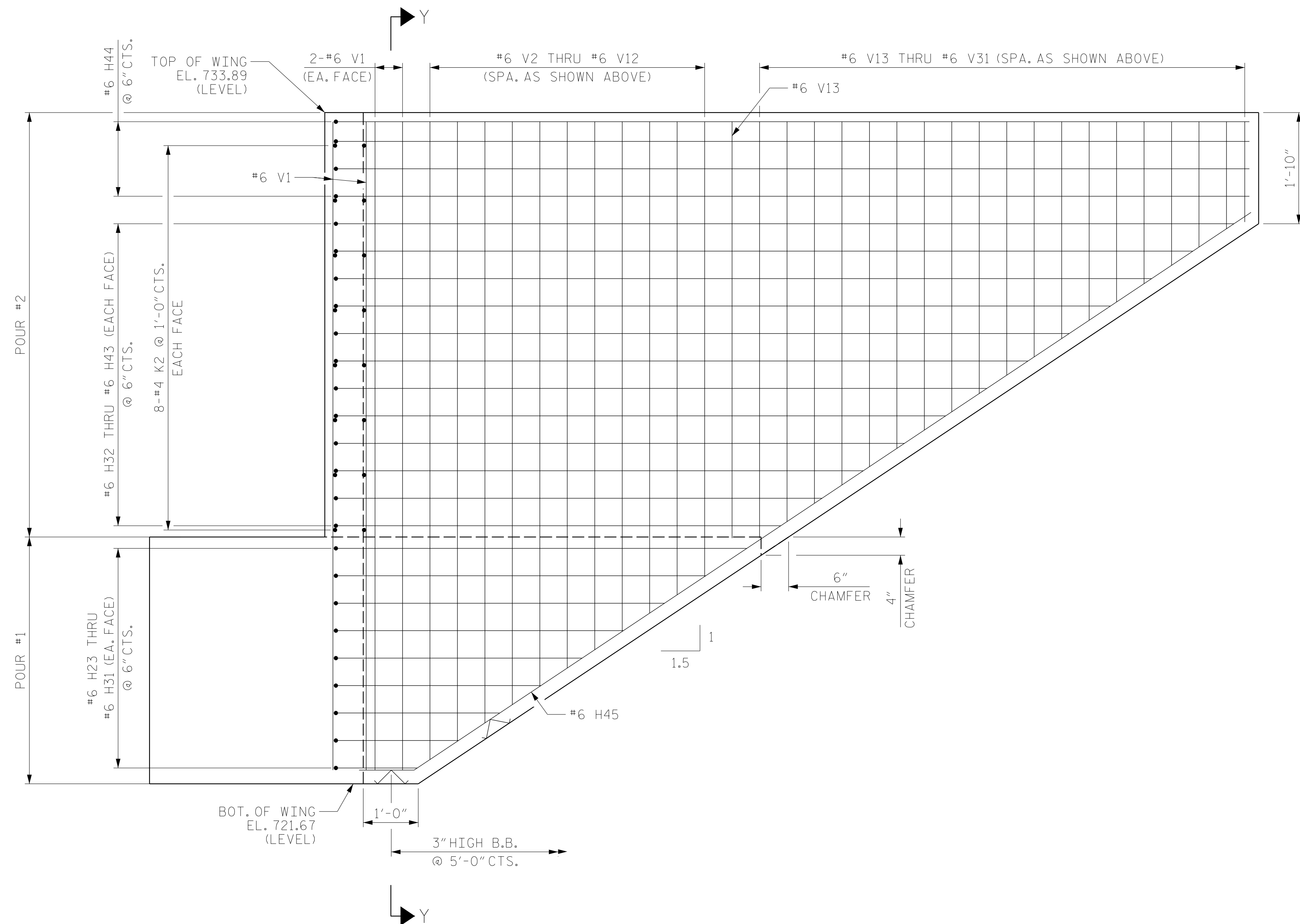
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NO.	BY:	DATE:	NO.	BY:	DATE:	S1-32
1			3			TOTAL SHEETS
2			4			36

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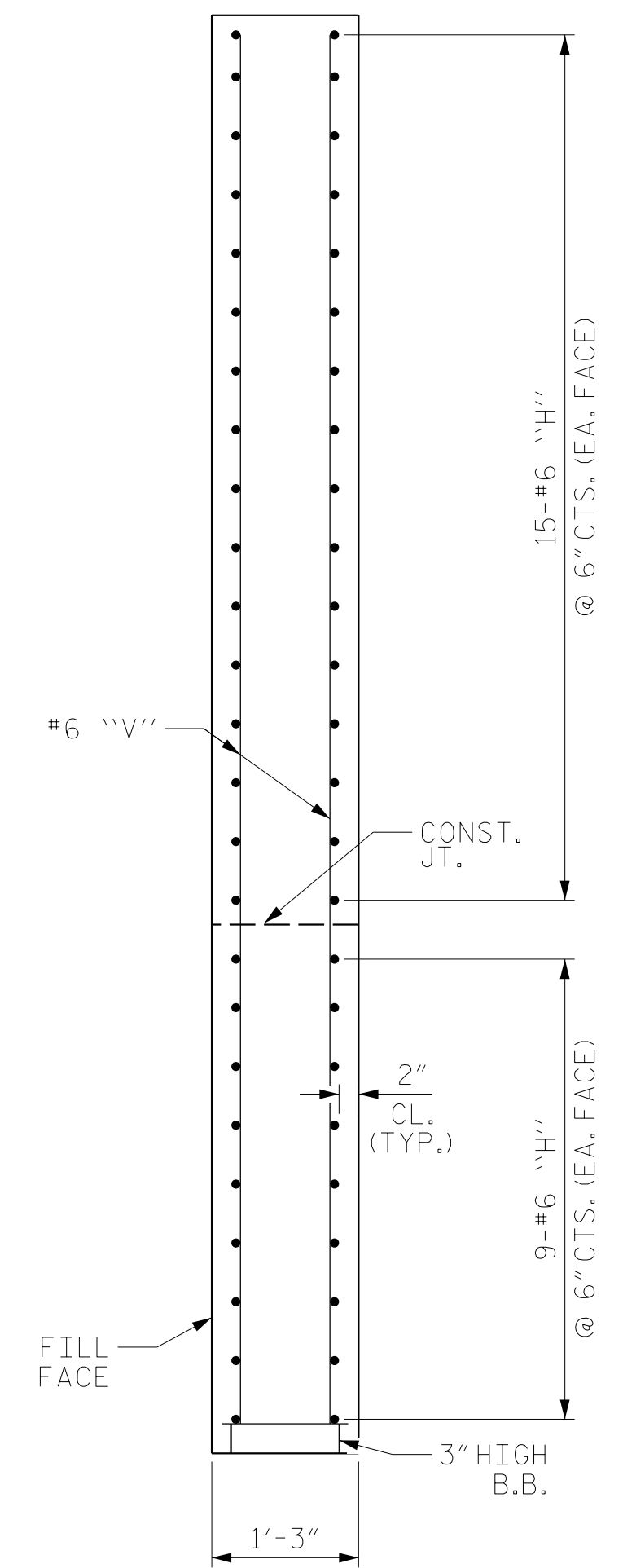
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PLAN OF WING - W1



ELEVATION OF WING - W1



SECTION Y-Y

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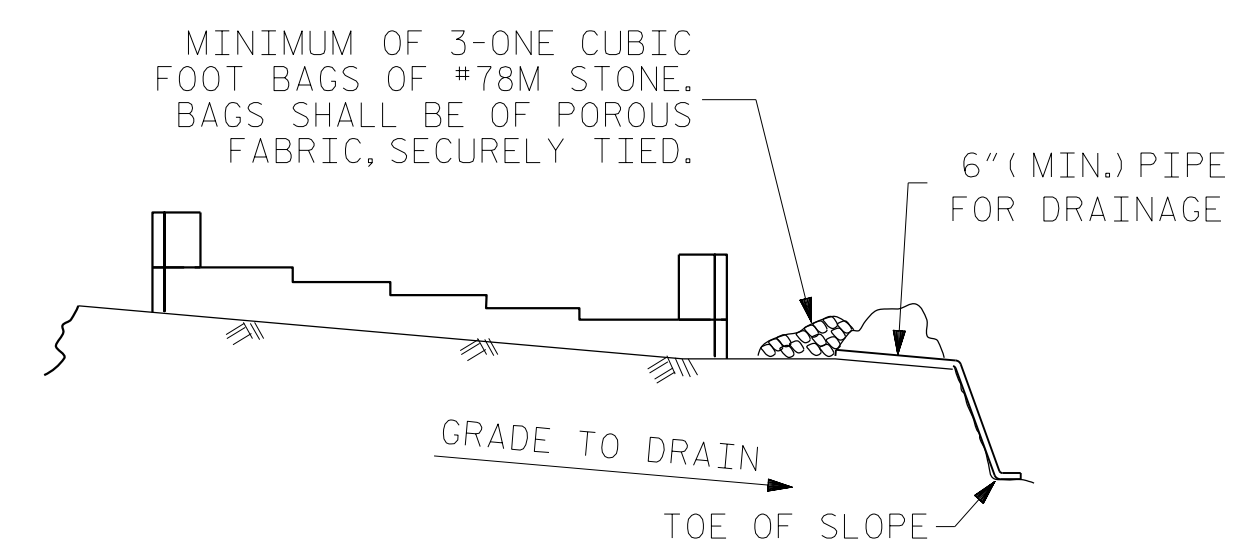
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL
 END BENT NO. 2
 LEFT LANE

REVISIONS						SHEET NO.
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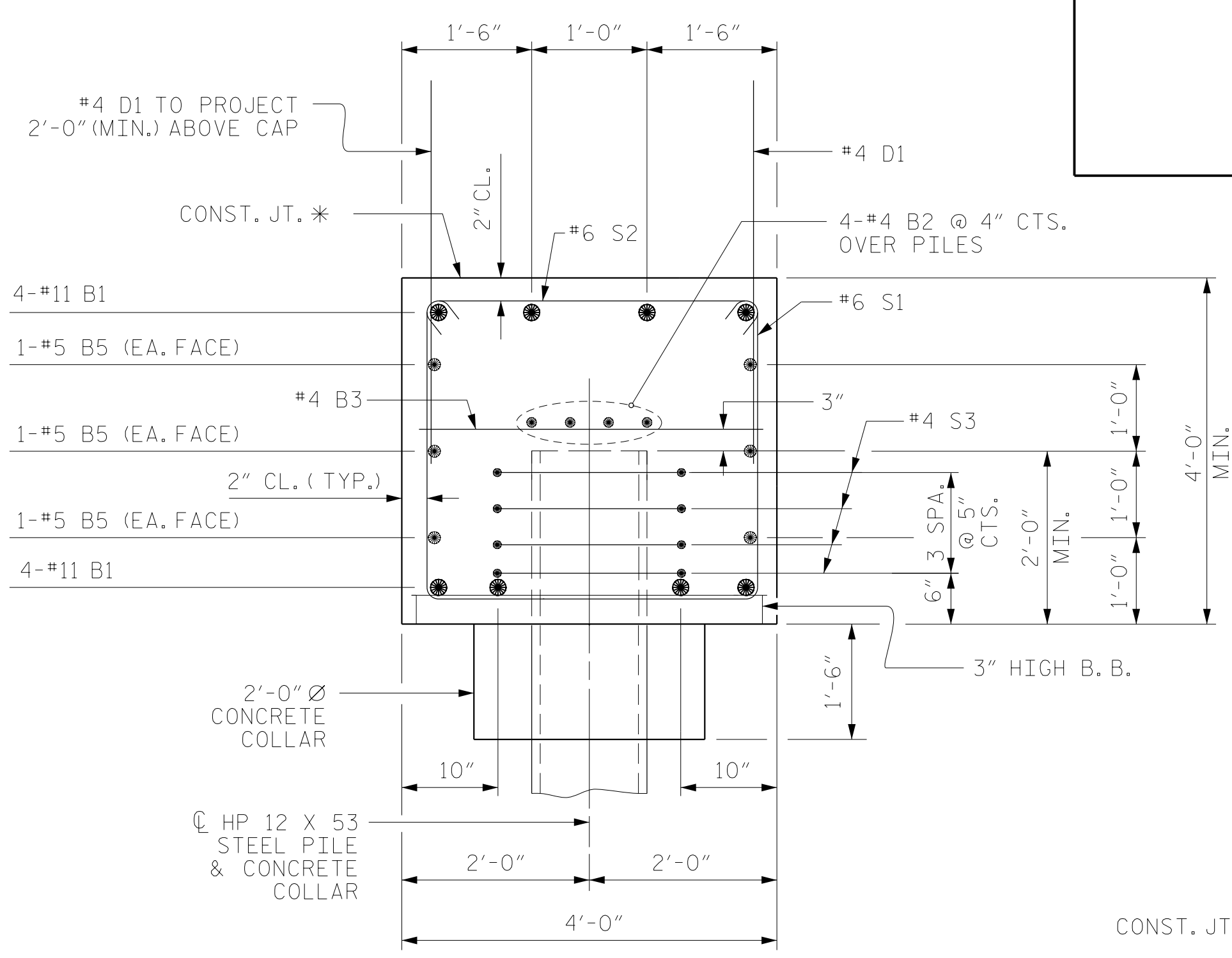


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

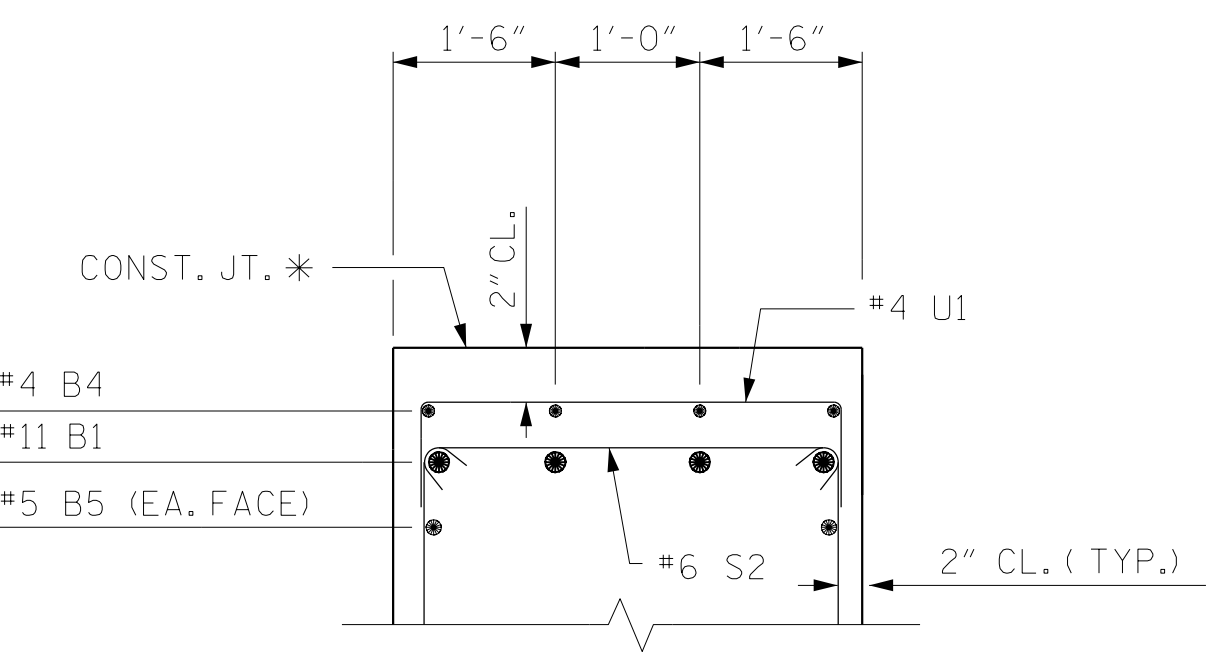
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



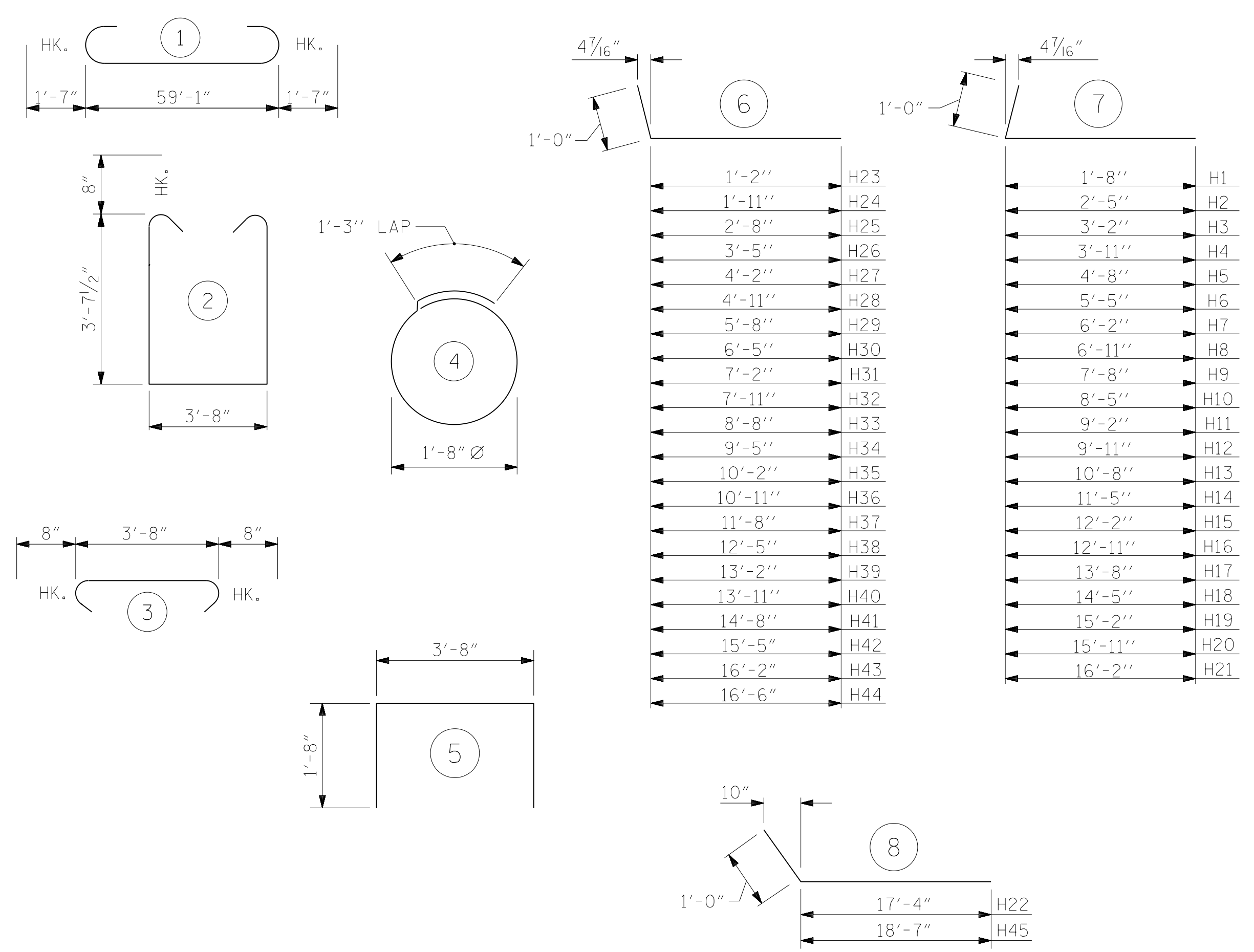
SECTION A-A

* THE TOP SURFACE OF THE END BENT CAP & WINGS, EXCLUDING THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".



PARTIAL SECTION B-B

BAR TYPES

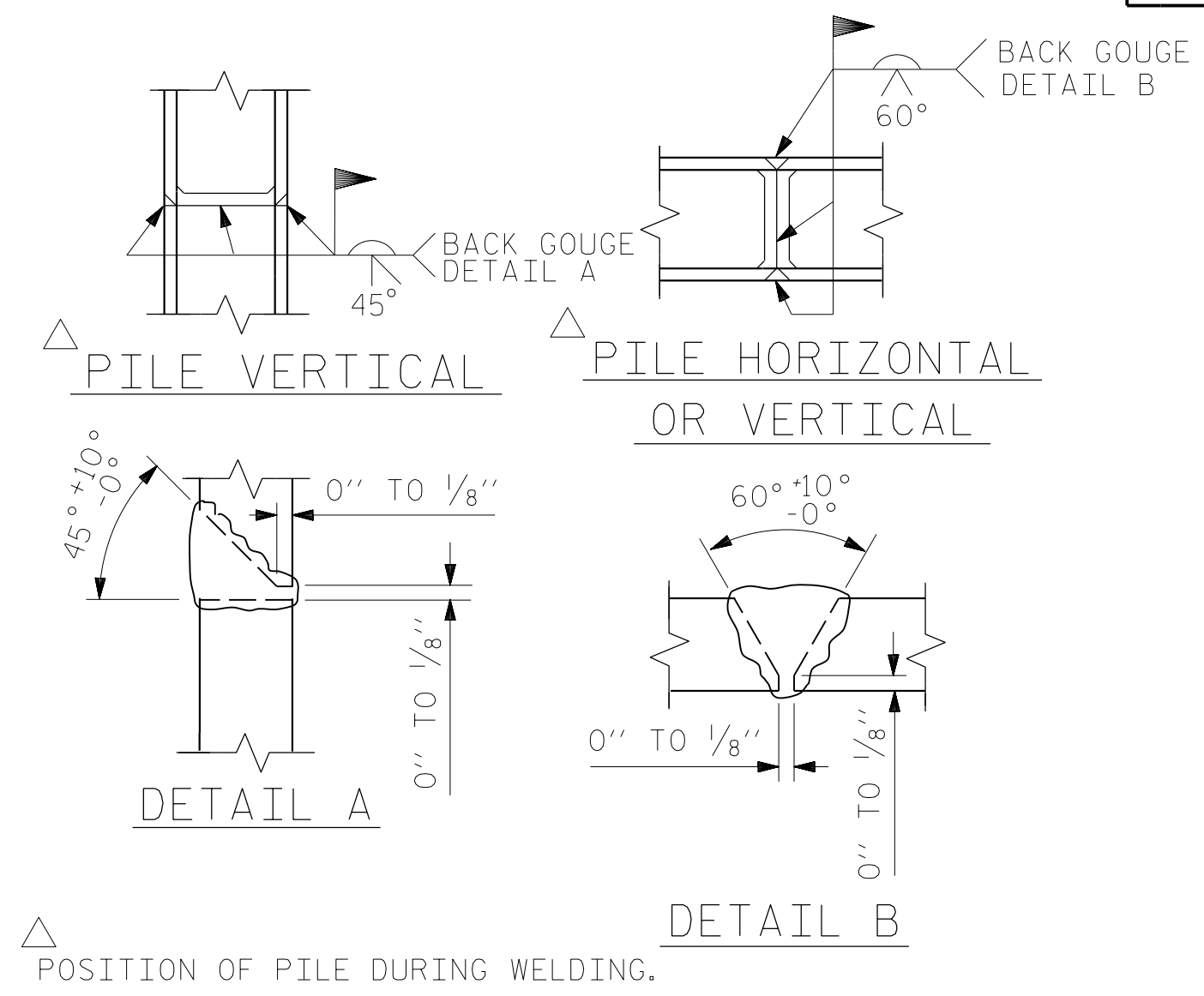


ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL

END BENT NO. 2

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#11	1	62'-3"	2646	H42	2	#6	6	16'-5"	49
B2	8	#4	STR	30'-0"	160	H43	2	#6	6	17'-2"	52
B3	14	#4	STR	3'-8"	34	H44	8	#6	6	17'-6"	210
B4	4	#4	STR	10'-0"	27	H45	2	#6	8	19'-7"	59
B5	6	#5	STR	59'-1"	370						
						K1	16	#4	STR	3'-0"	32
D1	94	#4	STR	4'-0"	251	K2	16	#4	STR	2'-9"	29
H1	2	#6	7	2'-8"	8	S1	85	#6	2	12'-3"	1564
H2	2	#6	7	3'-5"	10	S2	85	#6	3	5'-0"	638
H3	2	#6	7	4'-2"	13	S3	48	#4	4	6'-6"	208
H4	2	#6	7	4'-11"	15						
H5	2	#6	7	5'-8"	17	U1	7	#4	5	7'-0"	33
H6	2	#6	7	6'-5"	19						
H7	2	#6	7	7'-2"	22	V1	26	#6	STR	11'-8"	456
H8	2	#6	7	7'-11"	24	V2	4	#6	STR	11'-7"	70
H9	2	#6	7	8'-8"	26	V3	4	#6	STR	11'-3"	68
H10	2	#6	7	9'-5"	28	V4	4	#6	STR	10'-11"	66
H11	2	#6	7	10'-2"	31	V5	4	#6	STR	10'-7"	64
H12	2	#6	7	10'-11"	33	V6	4	#6	STR	10'-3"	62
H13	2	#6	7	11'-8"	35	V7	4	#6	STR	9'-11"	60
H14	2	#6	7	12'-5"	37	V8	4	#6	STR	9'-7"	58
H15	2	#6	7	13'-2"	40	V9	4	#6	STR	9'-3"	56
H16	2	#6	7	13'-11"	42	V10	4	#6	STR	8'-11"	54
H17	2	#6	7	14'-8"	44	V11	4	#6	STR	8'-7"	52
H18	2	#6	7	15'-5"	46	V12	4	#6	STR	8'-3"	50
H19	2	#6	7	16'-2"	49	V13	8	#6	STR	7'-7"	91
H20	2	#6	7	16'-11"	51	V14	4	#6	STR	7'-3"	44
H21	8	#6	7	17'-2"	206	V15	4	#6	STR	6'-11"	42
H22	2	#6	8	18'-4"	55	V16	4	#6	STR	6'-7"	40
H23	2	#6	6	2'-2"	7	V17	4	#6	STR	6'-3"	38
H24	2	#6	6	2'-11"	9	V18	4	#6	STR	5'-11"	36
H25	2	#6	6	3'-8"	11	V19	4	#6	STR	5'-7"	34
H26	2	#6	6	4'-5"	13	V20	4	#6	STR	5'-3"	32
H27	2	#6	6	5'-2"	16	V21	4	#6	STR	4'-11"	30
H28	2	#6	6	5'-11"	18	V22	4	#6	STR	4'-7"	28
H29	2	#6	6	6'-8"	20	V23	4	#6	STR	4'-3"	26
H30	2	#6	6	7'-5"	22	V24	4	#6	STR	3'-11"	24
H31	2	#6	6	8'-2"	25	V25	4	#6	STR	3'-7"	22
H32	2	#6	6	8'-11"	27	V26	4	#6	STR	3'-3"	20
H33	2	#6	6	9'-8"	29	V27	4	#6	STR	2'-11"	18
H34	2	#6	6	10'-5"	31	V28	4	#6	STR	2'-7"	16
H35	2	#6	6	11'-2"	34	V29	4	#6	STR	2'-3"	14
H36	2	#6	6	11'-11"	36	V30	4	#6	STR	1'-11"	12
H37	2	#6	6	12'-8"	38	V31	2	#6	STR	1'-10"	6
H38	2	#6	6	13'-5"	40						
H39	2	#6	6	14'-2"	43						
H40	2	#6	6	14'-11"	45						
H41	2	#6	6	15'-8"	47						
REINFORCING STEEL											9,413 LBS.
CLASS A CONCRETE											
POUR #1											
CAP, CONCRETE COLLARS & LOWER PART OF WINGS											41.4 C. Y.
POUR #2											
UPPER PART OF WINGS											11.3 C. Y.
TOTAL											52.2 C. Y.
HP 12 X 53 STEEL PILES											
NO. 12											240 LIN. FT.
STEEL PILE POINTS											NO.: 12



PILE SPLICE DETAILS

PROJECT NO. R-2707C
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 SHEET 4 OF 4

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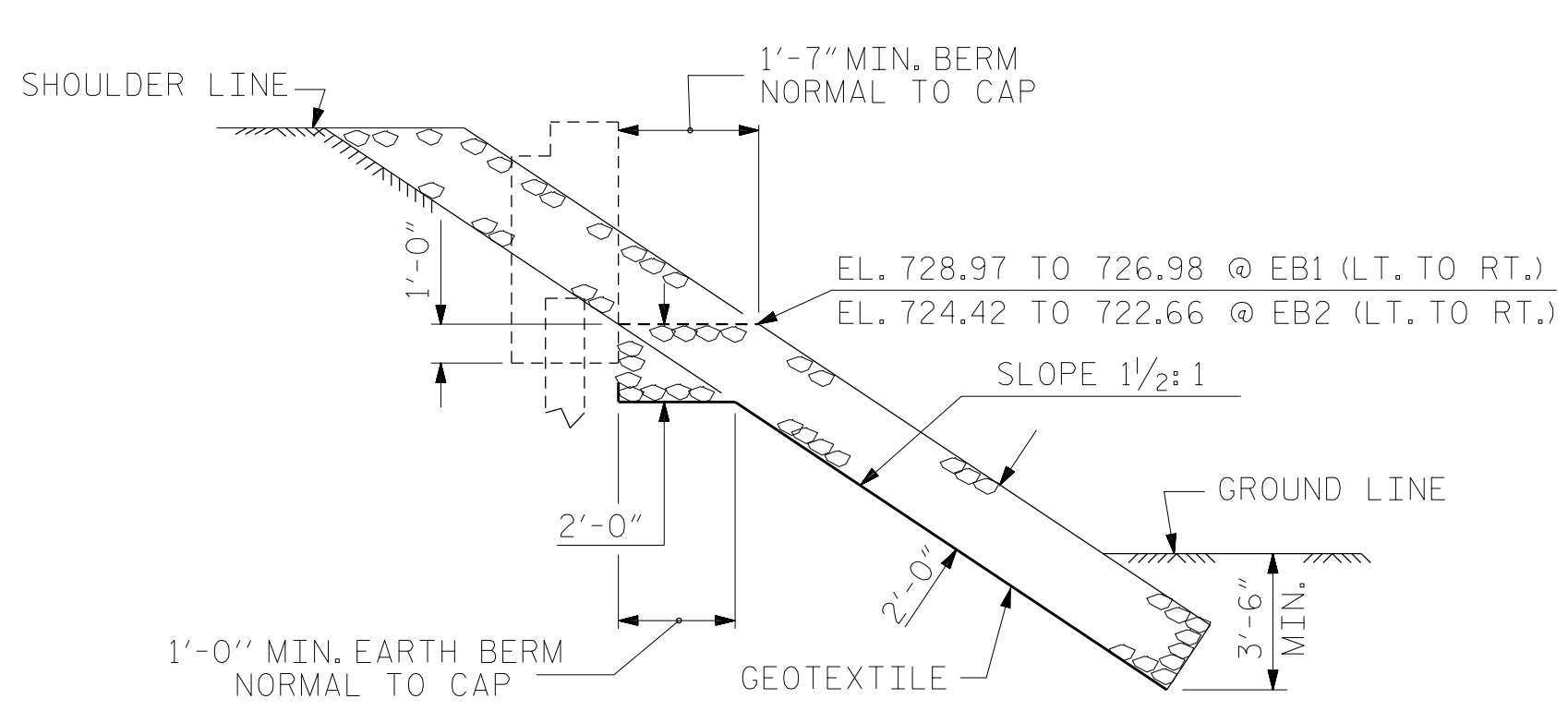
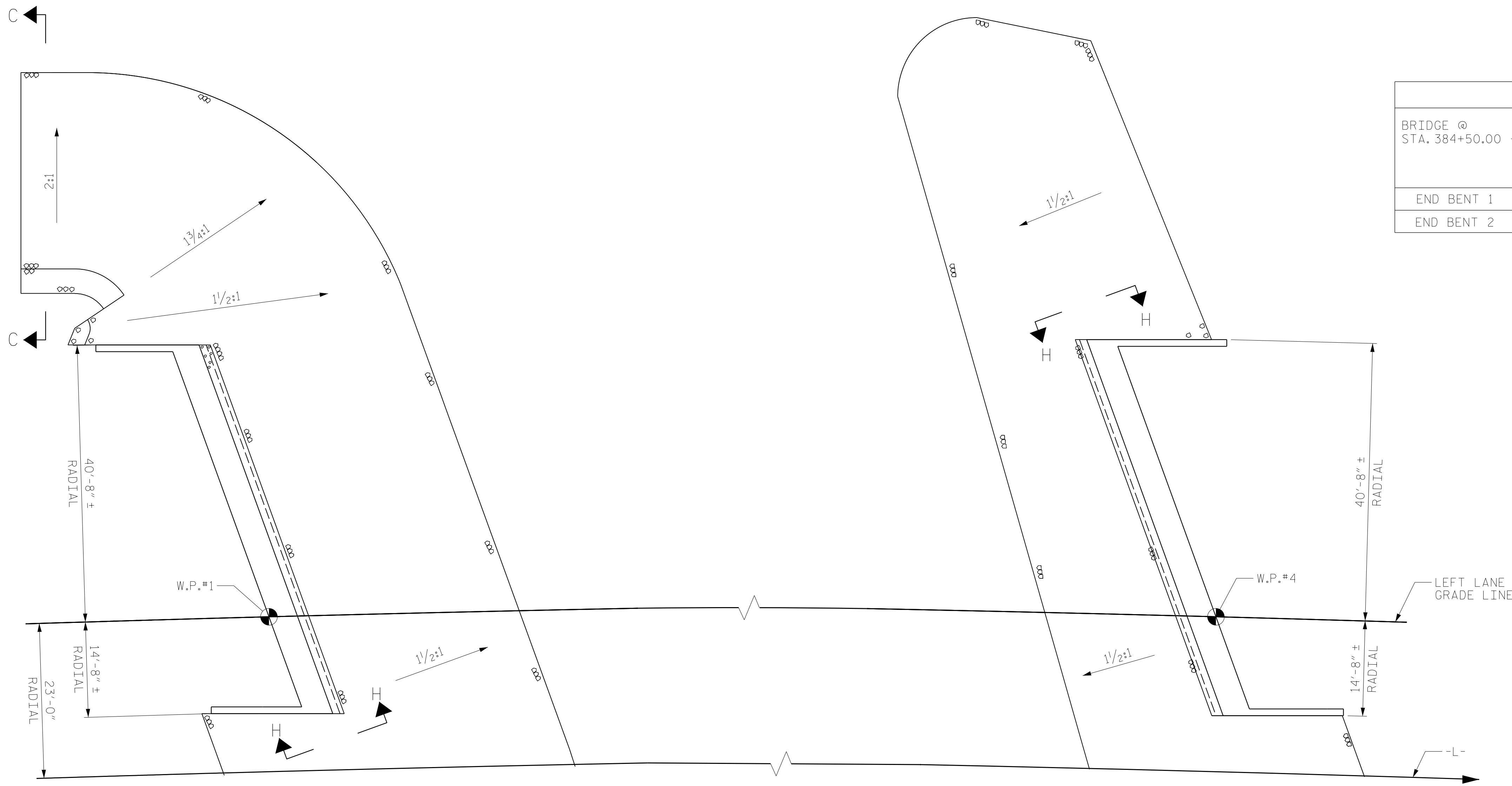
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-34
1			3			TOTAL SHEETS
2			4			36

DRAWN BY: MAL DATE: 11/2016
 CHECKED BY: JMR DATE: 12/2016
 DESIGN ENGINEER OF RECORD: MAL DATE: 11/2016

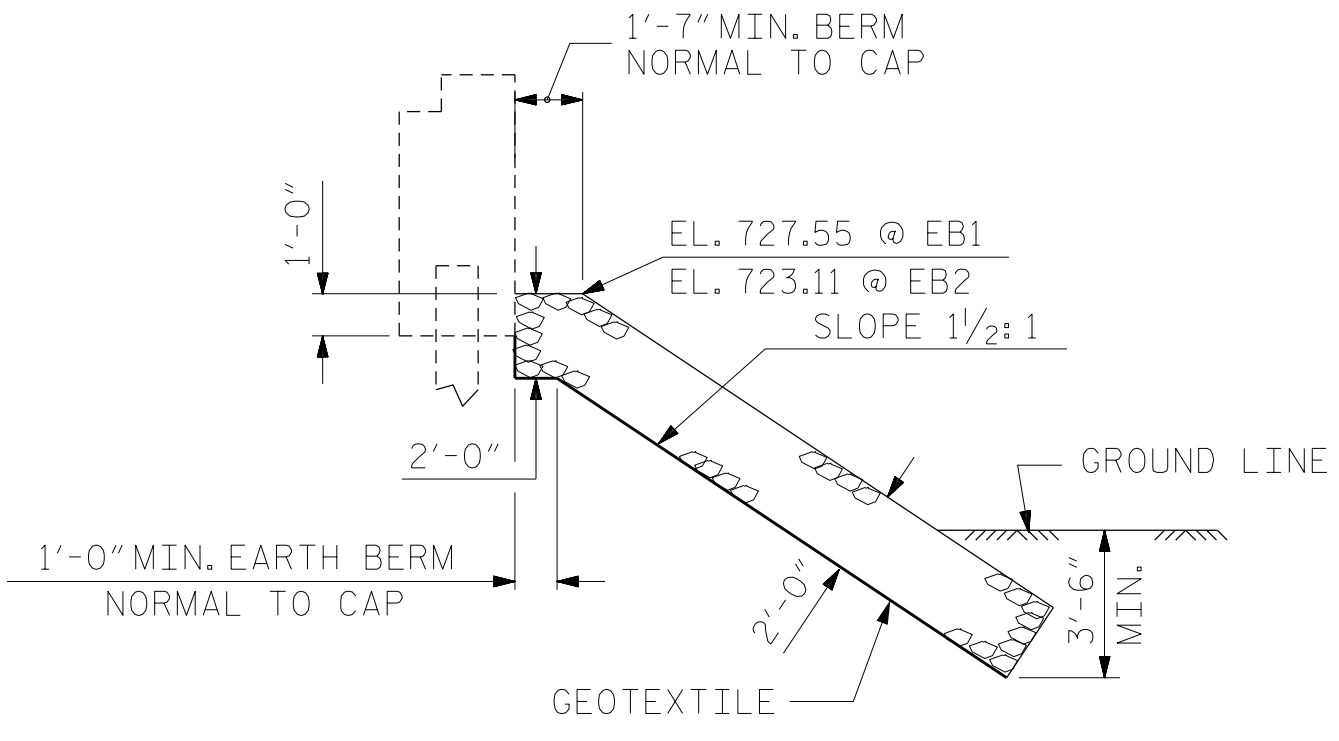
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NOTES
FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING.

ESTIMATED QUANTITIES		
BRIDGE @ STA. 384+50.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE
	TONS	SQUARE YARDS
END BENT 1	400	490
END BENT 2	265	325

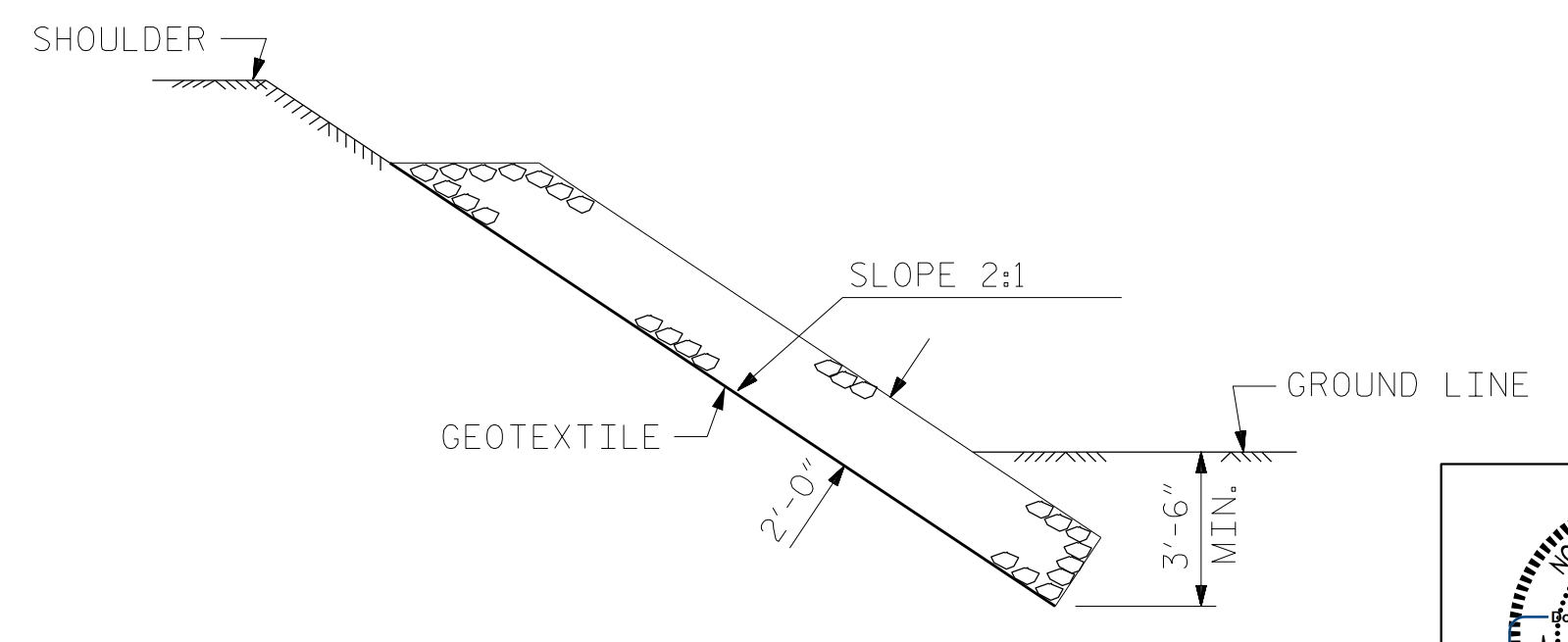


VIEW H-H



LEFT LANE GRADE LINE SECTION

BERM RIP RAPPED



SECTION C-C

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

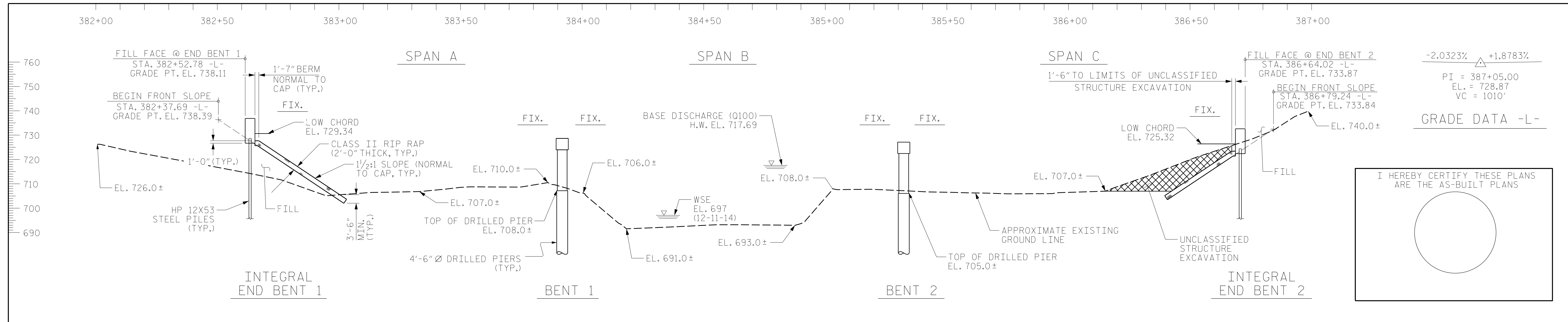


STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
RIP RAP DETAILS
LEFT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S1-35
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : MAL DATE : 11/2016
CHECKED BY : JMR DATE : 12/2016
DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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SECTION ALONG RIGHT LANE GRADE LINE

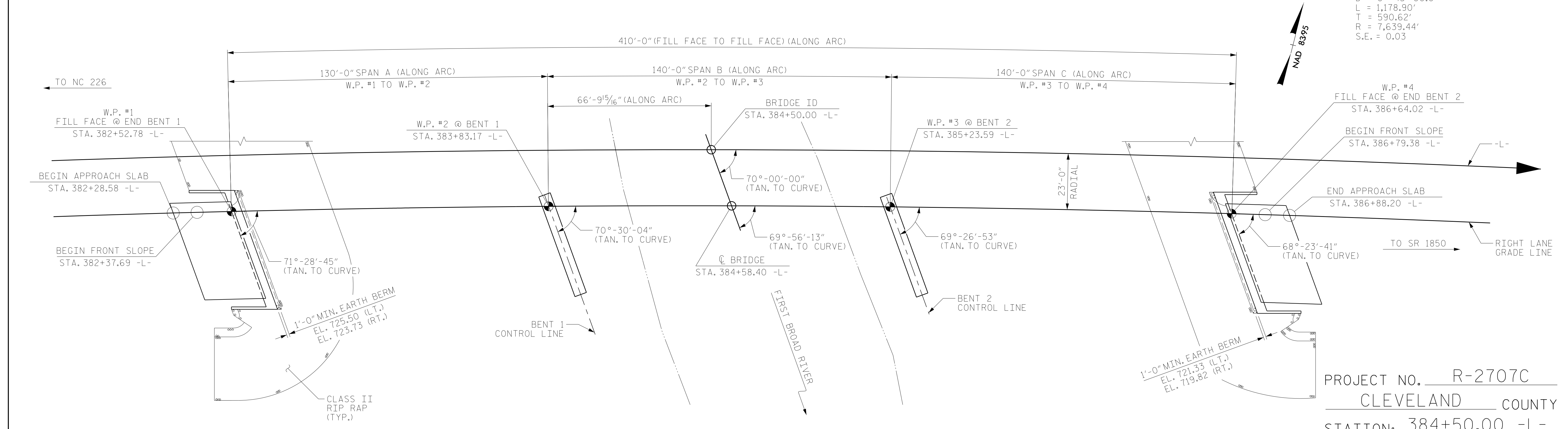
(SECTIONS AT END BENTS AND BENTS ARE TAKEN AT RIGHT ANGLES)

GRADE DATA -L-
 -2.0323% Δ +1.8783%
 PI = 387+05.00
 EL. = 728.87
 VC = 1010'

I HEREBY CERTIFY THESE PLANS ARE THE AS-BUILT PLANS

HORIZONTAL CURVE DATA -L-

P.I. STA. = 387+18.94
 Δ = 8°-50'-30.4"(RT.)
 D = 0°-45'-00.0"
 L = 1,178.90'
 T = 590.62'
 R = 7,639.44'
 S.E. = 0.03



PLAN ALONG RIGHT LANE GRADE LINE

(FOR CLARITY, PILES AND COLUMNS ARE NOT SHOWN IN PLAN VIEW)

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 1 OF 4 BRIDGE NO. 467

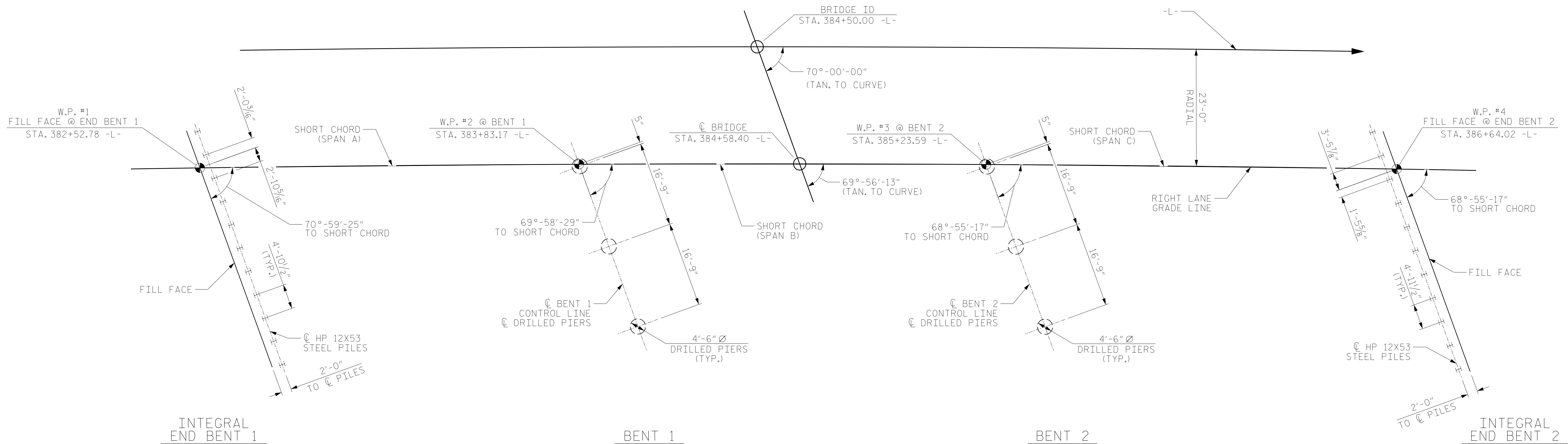
DRAWN BY : PDS DATE : 11/2016
 CHECKED BY : TLC DATE : 01/2017
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

3/8/2017
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 leblancm

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 www.rsandh.com
 North Carolina License No. 00737-F-0403-C-28

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
GENERAL DRAWING RIGHT LANE BRIDGE OVER FIRST BROAD RIVER ON US 74 SHELBY BYPASS BETWEEN NC 226 AND SR 1850					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO. S2-1					TOTAL SHEETS 36



FOUNDATION LAYOUT PLAN

DIMENSIONS LOCATING PILES AND DRILLED PIERS ARE SHOWN TO PILE AND DRILLED PIER CENTERLINE.

FOUNDATION NOTES

- FOR PILES, SEE GEOTECHNICAL SPECIAL PROVSIONS AND SECTION 450 OF STANDARD SPECIFICATIONS.
- PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.
- DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.
- FOR DRILLED PIERS, SEE SECTION 411 OF STANDARD SPECIFICATIONS.
- INSTALL DRILLED PIERS AT BENT NO.1 AND BENT NO.2 TO A TIP ELEVATION NO HIGHER THAN 670.6 FEET AND A PENETRATION OF 9 FEET INTO ROCK AS DEFINED BY ARTICLE 411-1 OF THE STANDARD SPECIFICATIONS.
- DRILLED PIERS AT BENT NO.1 AND BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 635 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 30 TSF.
- THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 AND BENT NO.2 IS 679 FEET. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.
- PERMANENT STEEL CASINGS ARE REQUIRED FOR DRILLED PIERS AT BENT NO.1 AND BENT NO.2. DO NOT EXTEND PERMANENT CASINGS BELOW ELEVATION 690 FT WITHOUT PRIOR APPROVAL FROM THE ENGINEER.
- CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL NEED TO DETERMINE THE NEED FOR CSL TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.
- DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.
- DRILLED-IN PILES ARE REQUIRED FOR INTEGRAL END BENT NO.2. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 713 FT. FILL THE BOTTOM 3 FT OF HOLES FOR PILE EXCAVATION WITH CONCRETE AND THE REST OF THE HOLES WITH CLASS II OR CLASS III SELECT MATERIAL THAT MEETS SECTION 1016 OF THE STANDARD SPECIFICATIONS FOR PILE EXCAVATION, SEE SECTION 450 OF THE SPECIFICATIONS.
- STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT NO.1 AND END BENT NO.2. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- DRILLED-IN PILES AT END BENT NO.2 THAT DO NOT HAVE A MINIMUM PENETRATION OF 3 FEET INTO ROCK AS DEFINED BY ARTICLE 411 OF THE STANDARD SPECIFICATIONS WILL BE DRIVEN TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

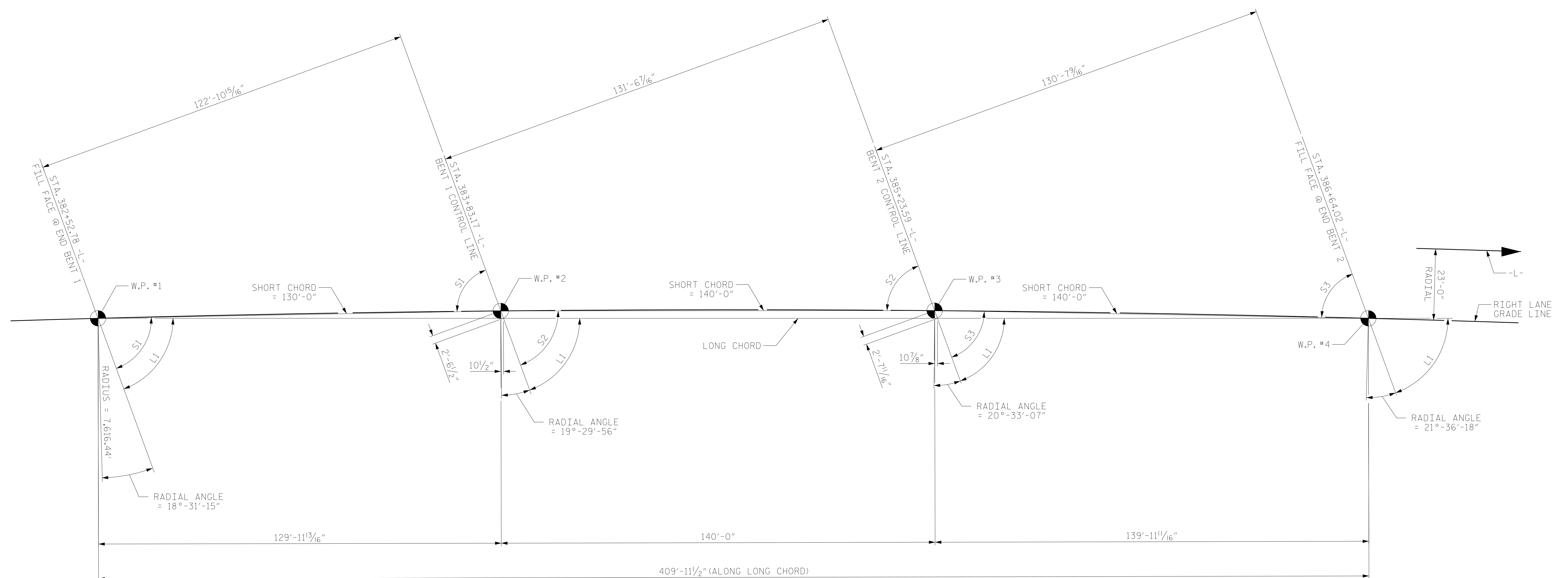
SHEET 2 OF 4



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
GENERAL DRAWING RIGHT LANE BRIDGE OVER FIRST BROAD RIVER ON US 74 SHELBY BYPASS BETWEEN NC 226 AND SR 1850					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S2-2
TOTAL SHEETS					36

DRAWN BY :	PDS	DATE :	11/2016
CHECKED BY :	TLC	DATE :	01/2017
DESIGN ENGINEER OF RECORD:	MAL	DATE :	11/2016

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LONG CHORD LAYOUT

NOTE: ALL BENTS ARE PARALLEL

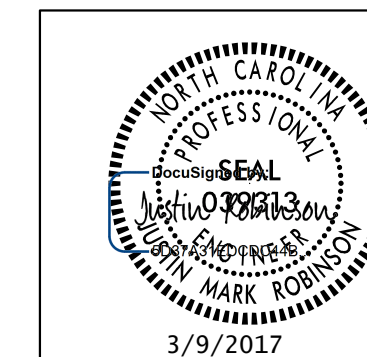
ANGLES	
LONG CHORD	SHORT CHORD
L1 = 69°-56'-13"	S1 = 70°-59'-25"
	S2 = 69°-58'-29"
	S3 = 68°-55'-17"

HORIZONTAL CURVE DATA -L-

P.I. STA. = 387+18.94
 Δ = 8°-50'-30.4" (RT.)
 D = 0°-45'-00.0"
 L = 1,178.90'
 T = 590.62'
 R = 7,639.44'
 S.E. = 0.03

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 3 OF 4



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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

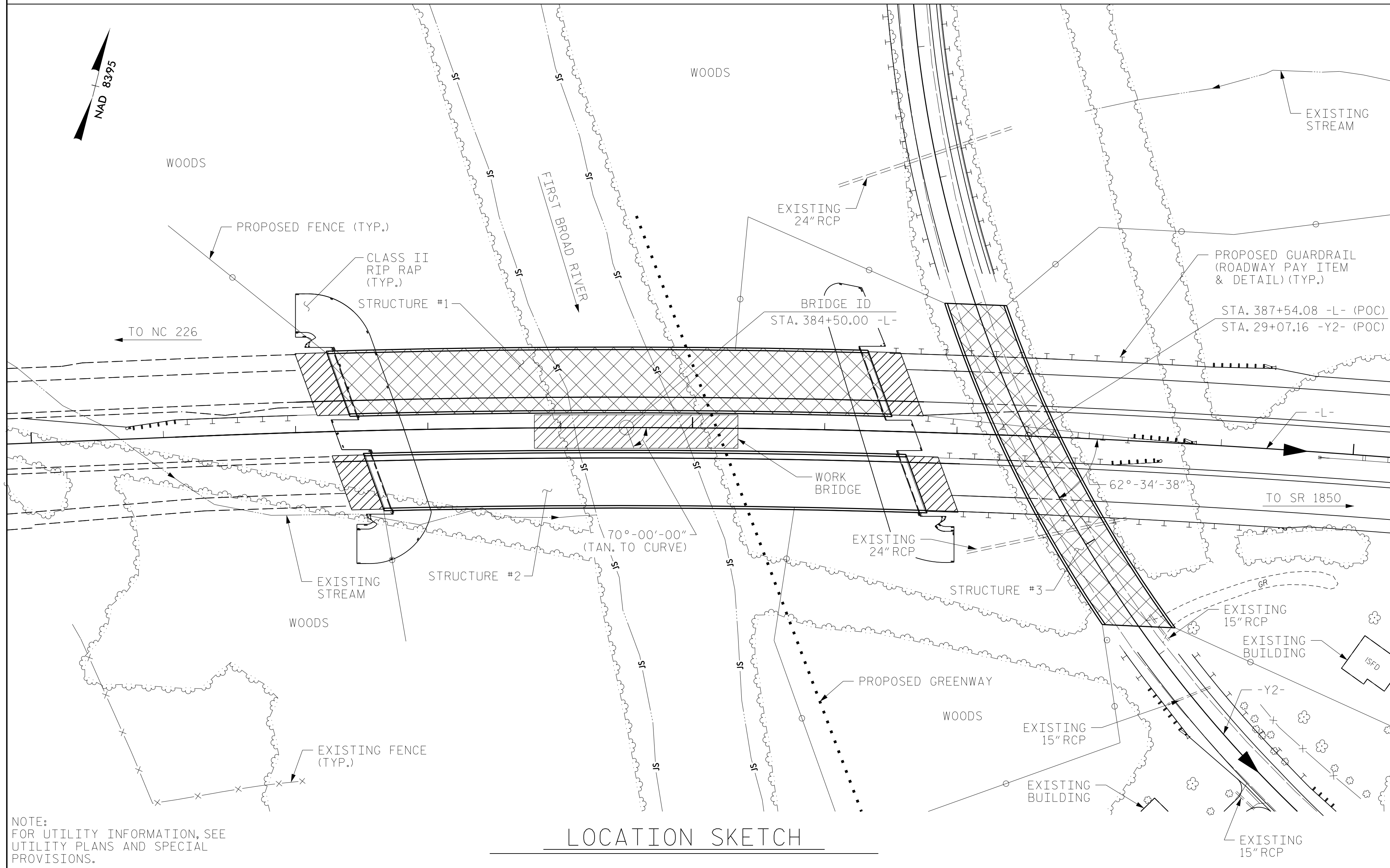
GENERAL DRAWING
 RIGHT LANE BRIDGE OVER FIRST
 BROAD RIVER ON US 74
 SHELBY BYPASS BETWEEN
 NC 226 AND SR 1850

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-3
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : PDS DATE : 11/2016
 CHECKED BY : TLC DATE : 01/2017
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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 FINAL UNLESS ALL
 SIGNATURES COMPLETED

BENCH MARK #17: 8 INCH NAIL IN BASE OF 20 INCH SYCAMORE 93.83' LEFT OF STA. 385+52.11 -L-, EL. 707.47'



NOTE:
FOR UTILITY INFORMATION, SEE
UTILITY PLANS AND SPECIAL
PROVISIONS.

LOCATION SKETCH

NOTES:

- ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.
- THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- FOR PLACING LOAD ON STRUCTURES, SEE SPECIAL PROVISIONS.
- REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
- THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.
- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES."
- THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 AND NO.2 IS ELEVATION 679. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.
- FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
- THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON GENERAL DRAWING SHEET 1 OF 4 SHALL BE EXCAVATED FOR A DISTANCE OF 23 FT LEFT AND 70 FT RIGHT OF RIGHT LANE GRADE LINE AT END BENT 2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.
- THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE THE TEMPORARY ACCESS AT STATION 384+50.00 -L- FOR USE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE.
- FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS, SEE SPECIAL PROVISIONS.
- FOR 74" MODIFIED PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.

HYDRAULIC DATA

DESIGN DISCHARGE	= 20,200 CFS
FREQUENCY OF DESIGN FLOOD	= 50 YRS
DESIGN HIGH WATER ELEVATION	= 716.2
DRAINAGE AREA	= 226 SQ. MI.
BASE DISCHARGE (Q100)	= 23,400 CFS
BASE HIGH WATER ELEVATION	= 717.69

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE	= >31,700 CFS
FREQUENCY OF OVERTOPPING FLOOD	= >500 YRS+
OVERTOPPING FLOOD ELEVATION	= 733.80

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

SHEET 4 OF 4

TOTAL BILL OF MATERIALS

	CONSTRUCTION, MAINTENANCE & REMOVAL OF TEMP. ACCESS AT STA. 384+50.00 -L- LUMP SUM	UNCLASSIFIED STRUCTURE EXCAVATION LUMP SUM	4'-6" Ø DRILLED PIER IN SOIL LIN. FT.	4'-6" Ø DRILLED PIER NOT IN SOIL LIN. FT.	PERMANENT STEEL CASING FOR 4'-6" DIA. DRILLED PIER LIN. FT.	CSL TESTING EACH	REINFORCED CONCRETE DECK SLAB SQ. FT.	GROOVING BRIDGE FLOORS SQ. FT.	CLASS A CONCRETE CU. YDS.	BRIDGE APPROACH SLABS LUMP SUM	REINFORCING STEEL LBS.	SPIRAL COLUMN REINFORCING STEEL APPROX. LBS.	74" MODIFIED PRESTRESSED CONCRETE GIRDERS NO.	PILE EXCAVATION IN SOIL LIN. FT.	PILE EXCAVATION NOT IN SOIL LIN. FT.
SUPERSTRUCTURE							17,661	16,827					12	1,621.85	
END BENT No. 1									47.5		8,647				
BENT No. 1			85.0	27.0	54.0				62.5		20,079	4,601			
BENT No. 2			76.0	27.0	54.0				64.1		19,678	4,478			
END BENT No. 2		LUMP SUM							47.2		8,619			77	33
TOTAL	LUMP SUM	LUMP SUM	161	54	108	2	17,661	16,827	221.3	LUMP SUM	57,023	9,079	12	1,621.85	77

	HP 12x53 STEEL PILES NO.	STEEL PILE POINTS LIN. FT.	PILE DRIVING EQUIPMENT SETUP FOR HP 12x53 STEEL PILES EACH	CONCRETE BARRIER RAIL LIN. FT.	RIP RAP CLASS II (2'-0" THICK) TONS	GEOTEXTILE FOR DRAINAGE SQ. YDS.	ELASTOMERIC BEARINGS LUMP SUM
SUPERSTRUCTURE				816.7			LUMP SUM
END BENT No. 1	11	440	11		500	550	
BENT No. 1							
BENT No. 2							
END BENT No. 2	11	165	11		450	500	
TOTAL	22	605	22	816.7	950	1,050	LUMP SUM

DRAWN BY : PDS DATE : 11/2016
CHECKED BY : TLC DATE : 01/2017
DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

4/6/2017
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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING
RIGHT LANE BRIDGE OVER FIRST BROAD RIVER ON US 74 SHELBY BYPASS BETWEEN NC 226 AND SR 1850

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-4
1			3			TOTAL SHEETS
2			4			36

RS&H Architects-Engineers-Planners, Inc.
6601 Six Forks Road, Suite 260
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LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ_{DC}	γ_{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING #	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					COMMENT NUMBER			
						MOMENT					SHEAR					MOMENT								
						LIVE-LOAD FACTORS (γ_{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	LIVE-LOAD FACTORS (γ_{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	①	1.07	--	1.75	0.95	1.27	A	EL	63.4	1.14	1.16	B	I	13.21	0.80	0.87	1.07	A	I	63.4		
	HL-93 (OPERATING)	N/A		1.54	--	1.35	0.95	1.64	A	EL	63.4	1.14	1.54	B	I	13.21	N/A	--	--	--	--	--		
	HS-20 (INVENTORY)	36.000	②	1.59	57,240	1.75	0.95	1.88	A	EL	63.4	1.14	1.73	B	I	13.21	0.80	0.87	1.59	A	I	63.4		
	HS-20 (OPERATING)	36.000		2.27	81,720	1.35	0.95	2.44	A	EL	63.4	1.14	2.27	B	I	124.53	N/A	--	--	--	--	--		
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13,500		3.89	52,515	1.40	0.95	5.73	A	EL	63.4	1.14	5.70	B	I	13.21	0.80	0.87	3.89	A	I	63.4	
		SNGARBS2	20,000		2.77	55,400	1.40	0.95	4.08	A	EL	63.4	1.14	3.91	B	I	124.53	0.80	0.87	2.77	A	I	63.4	
		SNAGRIS2	22,000		2.57	56,540	1.40	0.95	3.79	A	EL	63.4	1.14	3.58	B	I	13.21	0.80	0.87	2.57	A	I	63.4	
		SNCOTTS3	27,250		1.93	52,593	1.40	0.95	2.85	A	EL	63.4	1.14	2.78	B	I	124.53	0.80	0.87	1.93	A	I	63.4	
		SNAGGRS4	34,925		1.56	54,483	1.40	0.95	2.31	A	EL	63.4	1.14	2.22	B	I	13.21	0.80	0.87	1.56	A	I	63.4	
		SNS5A	35,550		1.53	54,392	1.40	0.95	2.26	A	EL	63.4	1.14	2.21	B	I	124.53	0.80	0.87	1.53	A	I	63.4	
		SNS6A	39,950		1.39	55,531	1.40	0.95	2.04	A	EL	63.4	1.14	1.98	B	I	124.53	0.80	0.87	1.39	A	I	63.4	
	SNS7B	42,000		1.32	55,440	1.40	0.95	1.95	A	EL	63.4	1.14	1.91	B	I	124.53	0.80	0.87	1.32	A	I	63.4		
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33,000		1.68	55,440	1.40	0.95	2.48	A	EL	63.4	1.14	2.40	B	I	13.21	0.80	0.87	1.68	A	I	63.4	
		TNT4A	33,075		1.69	55,897	1.40	0.95	2.49	A	EL	63.4	1.14	2.36	B	I	13.21	0.80	0.87	1.69	A	I	63.4	
		TNT6A	41,600		1.36	56,576	1.40	0.95	2.01	A	EL	63.4	1.14	1.98	B	I	13.21	0.80	0.87	1.36	A	I	63.4	
		TNT7A	42,000		1.36	57,120	1.40	0.95	2.00	A	EL	63.4	1.14	1.95	B	I	124.53	0.80	0.87	1.36	A	I	63.4	
		TNT7B	42,000		1.39	58,380	1.40	0.95	2.06	A	EL	63.4	1.14	1.89	B	I	13.21	0.80	0.87	1.39	A	I	63.4	
		TNAGRIT4	43,000		1.33	57,190	1.40	0.95	1.96	A	EL	63.4	1.14	1.83	B	I	124.53	0.80	0.87	1.33	A	I	63.4	
TNAGT5A		45,000		1.26	56,700	1.40	0.95	1.86	A	EL	63.4	1.14	1.78	B	I	13.21	0.80	0.87	1.26	A	I	63.4		
TNAGT5B	45,000	③	1.26	56,700	1.40	0.95	1.85	A	EL	63.4	1.14	1.74	B	I	124.53	0.80	0.87	1.26	A	I	63.4			

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.
ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

1. TRANSFORMING ALL PRESTRESSING TENDONS.
- 2.
- 3.
- 4.

CONTROLLING LOAD RATING
① DESIGN LOAD RATING (HL-93)
② DESIGN LOAD RATING (HS-20)
③ LEGAL LOAD RATING **
** SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER

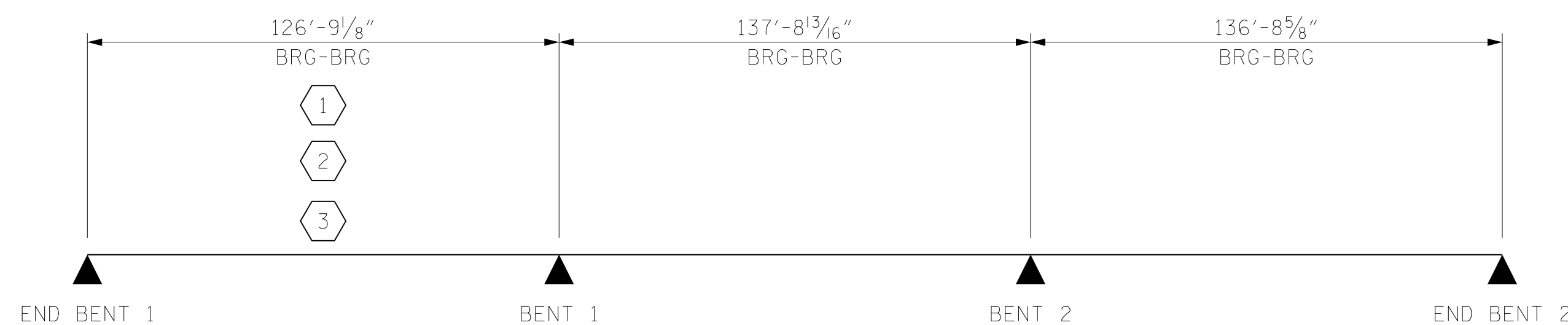
SECTION PROPERTIES			
SPAN A & B - INTERIOR			
	UNITS	NON-COMPOSITE	COMPOSITE
HEIGHT	IN	74	82.75
AREA	IN ²	885.1	1707.6
Ixx	IN ⁴	640172	1390848
Ycg	IN	36.56	56.70
SELF WT.	PLF	922.0	2207.1
EFF. WIDTH	IN	-	141.0

SECTION PROPERTIES PROVIDED AT MIDSPAN

SECTION PROPERTIES			
SPAN A - EXTERIOR			
	UNITS	NON-COMPOSITE	COMPOSITE
HEIGHT	IN	74	82.75
AREA	IN ²	885.1	1586.8
Ixx	IN ⁴	640172	1329038
Ycg	IN	36.56	55.05
SELF WT.	PLF	922.0	2018.5
EFF. WIDTH	IN	-	120.3

SECTION PROPERTIES PROVIDED AT MIDSPAN

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-



LRFR SUMMARY

DRAWN BY : PDS DATE : 03/2017
CHECKED BY : TLC DATE : 03/2017
DESIGN ENGINEER OF RECORD: MAL DATE : 03/2017

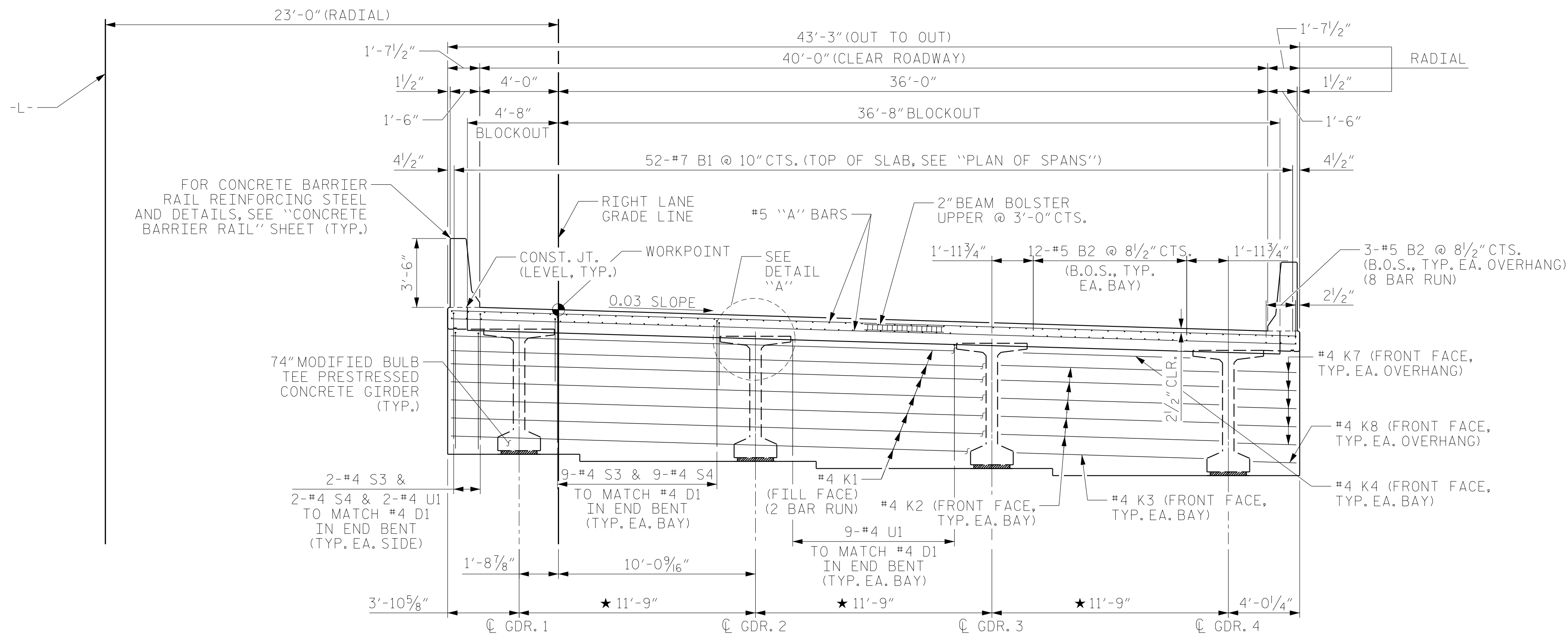
3/8/2017
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leblancm

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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
LRFR SUMMARY FOR
PRESTRESSED
CONCRETE GIRDERS
(NON-INTERSTATE TRAFFIC)
RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			S2-5
2			4			TOTAL SHEETS 36



TYPICAL SECTION AT INTEGRAL END BENTS

END BENT 1 SHOWN, END BENT 2 SIMILAR

* DIMENSIONS ARE PARALLEL TO SHORT CHORD OF RESPECTIVE SPAN

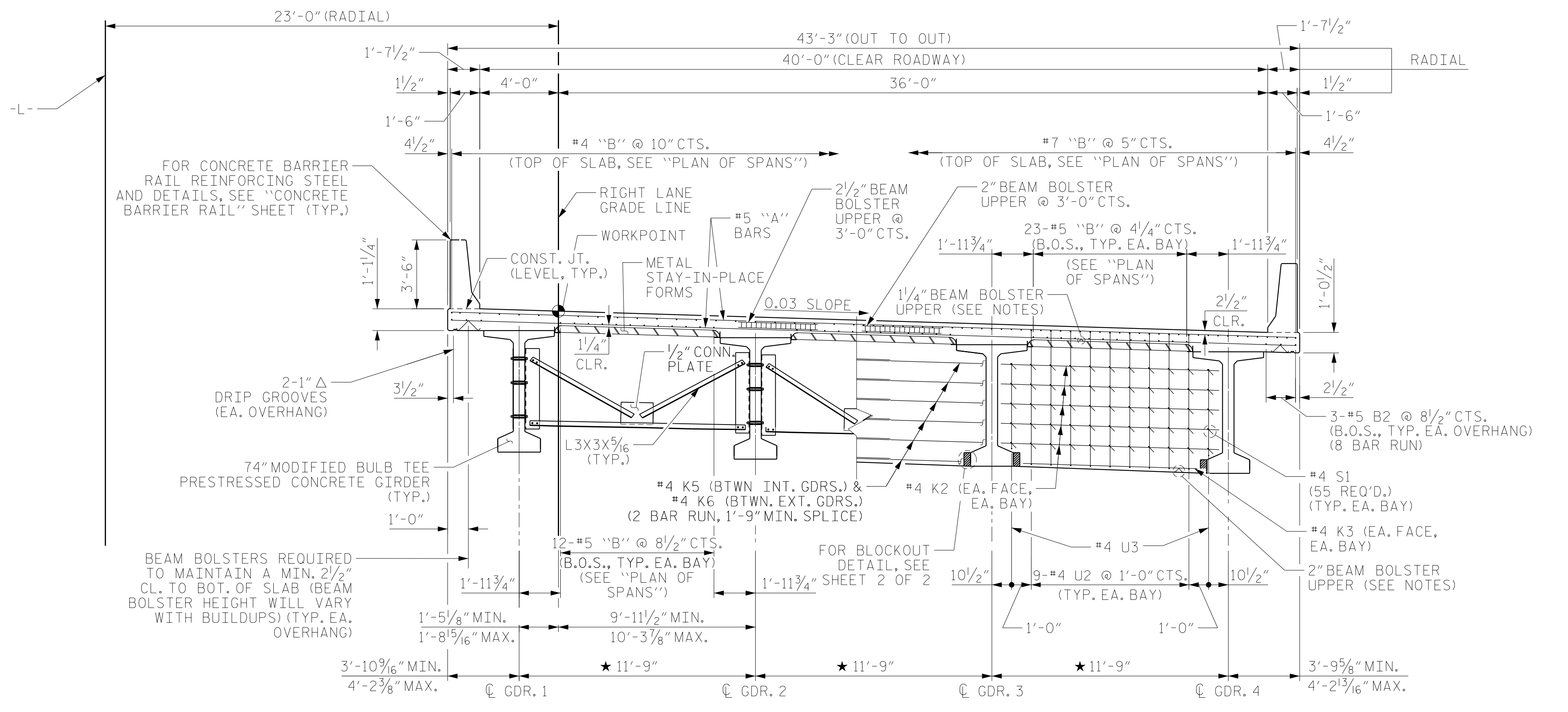
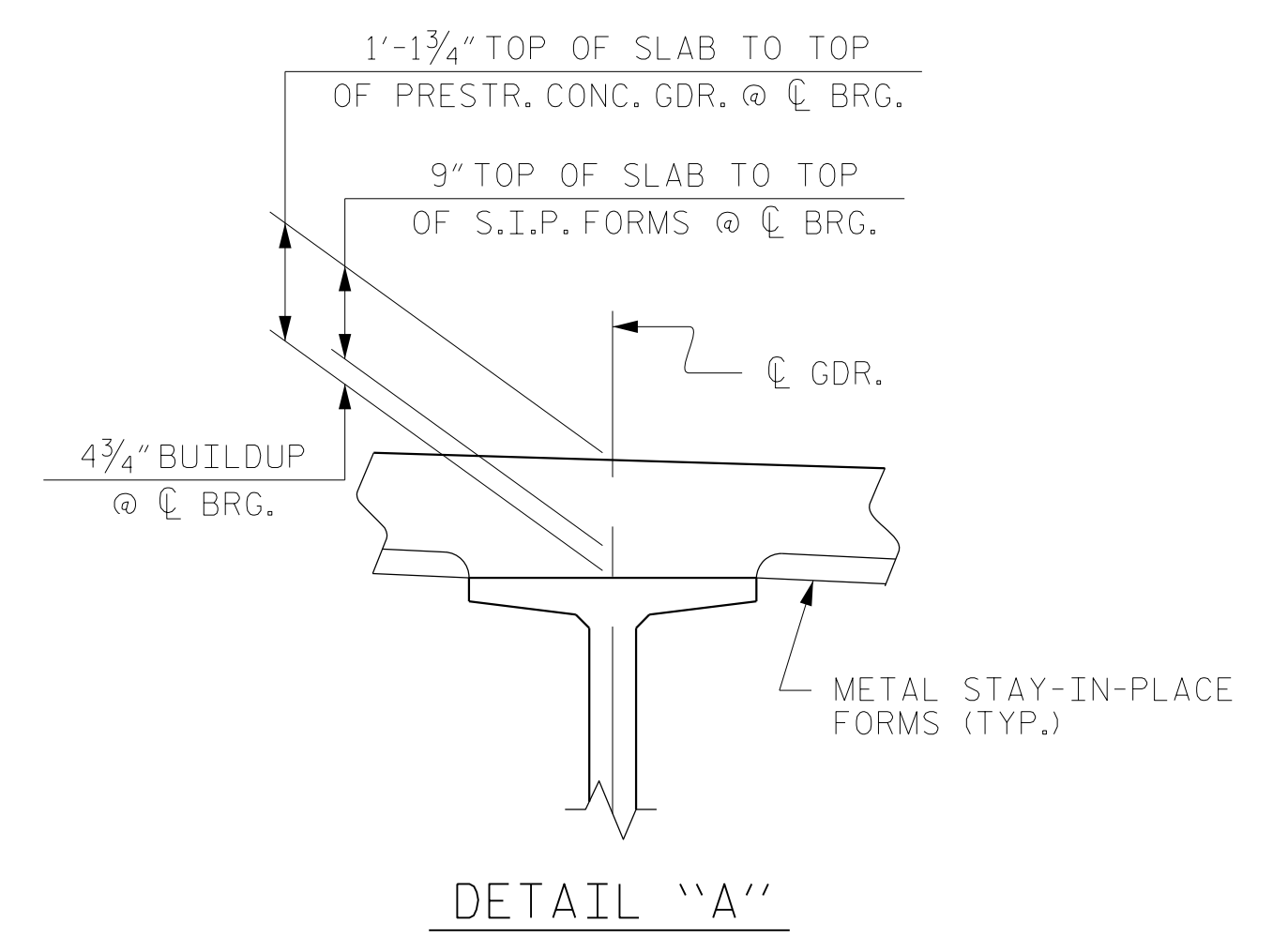
NOTES
 PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF 'A' BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) AT 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF 'A' BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.

LONGITUDINAL REINFORCING STEEL ('B' BARS) MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.

PREVIOUSLY CAST CONCRETE IN A CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.

CONCRETE BARRIER RAIL IN A CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL DECK SLAB CONCRETE IN THAT UNIT HAS BEEN CAST AND HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

B.O.S. = BOTTOM OF SLAB



SECTION AT INTERMEDIATE DIAPHRAGM SECTION AT CONTINUOUS BENT DIAPHRAGM

TYPICAL SECTION

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 1 OF 2

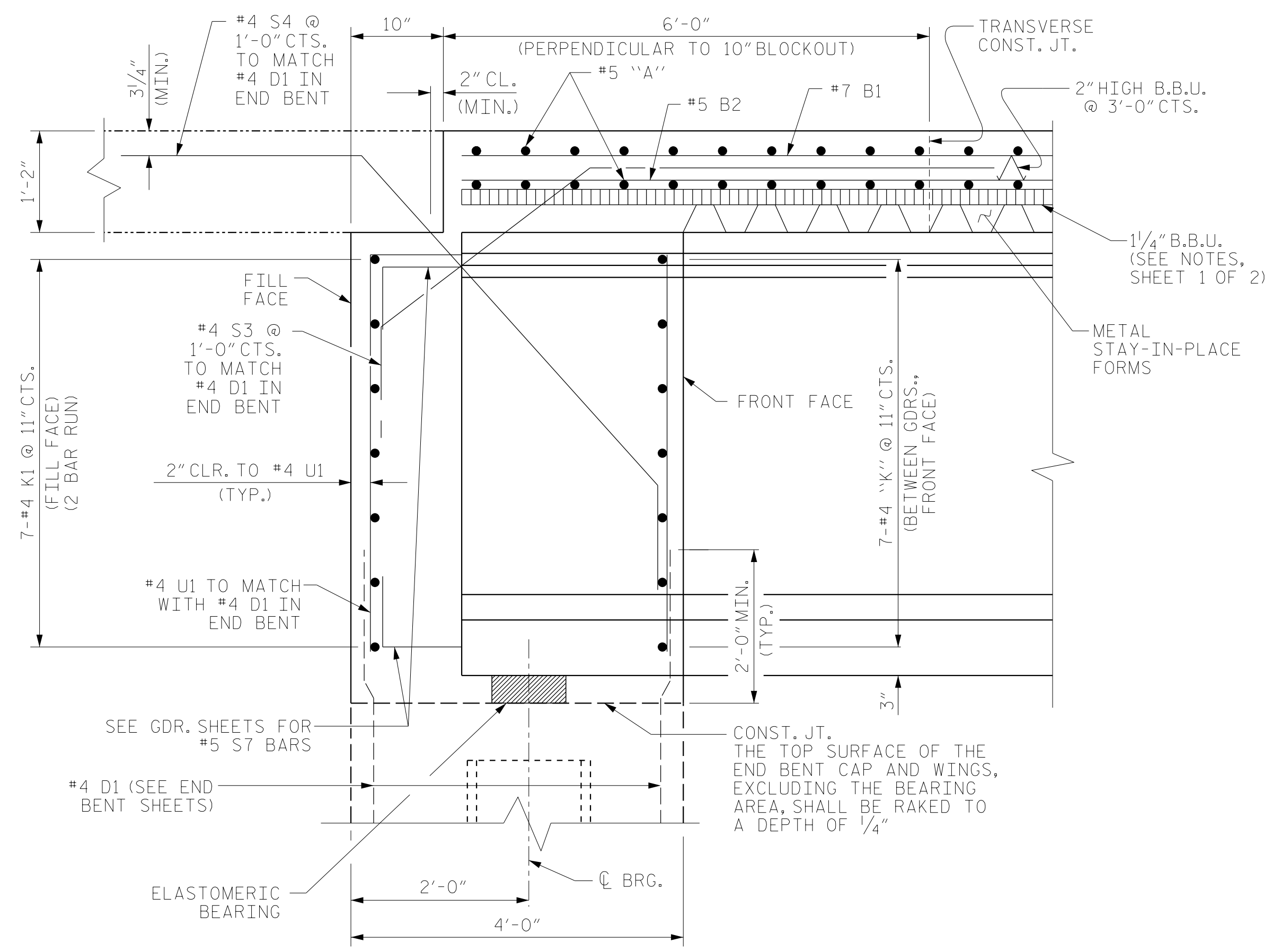


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 TYPICAL SECTION
 RIGHT LANE

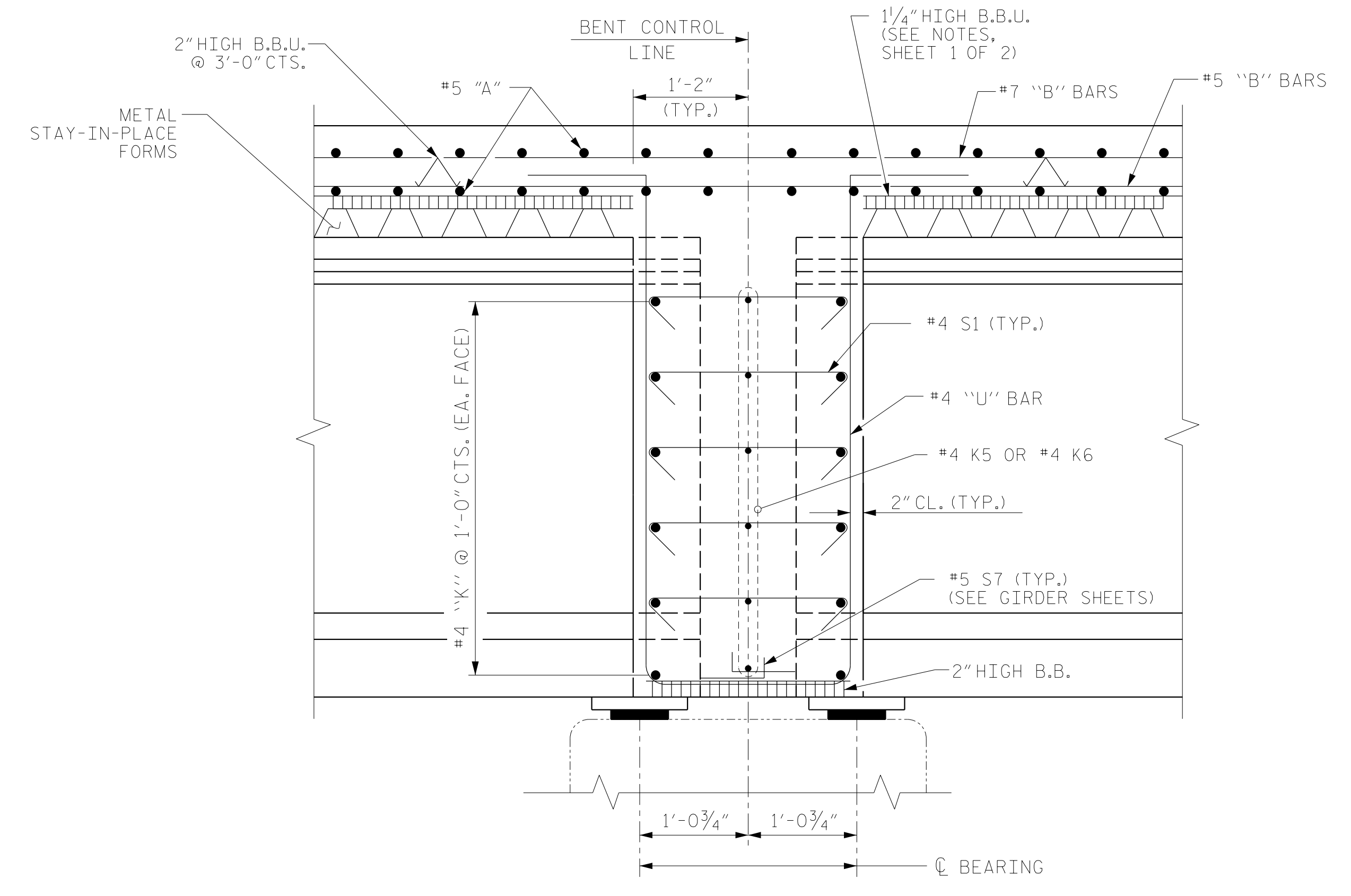
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-6
1			3			TOTAL SHEETS
2			4			36

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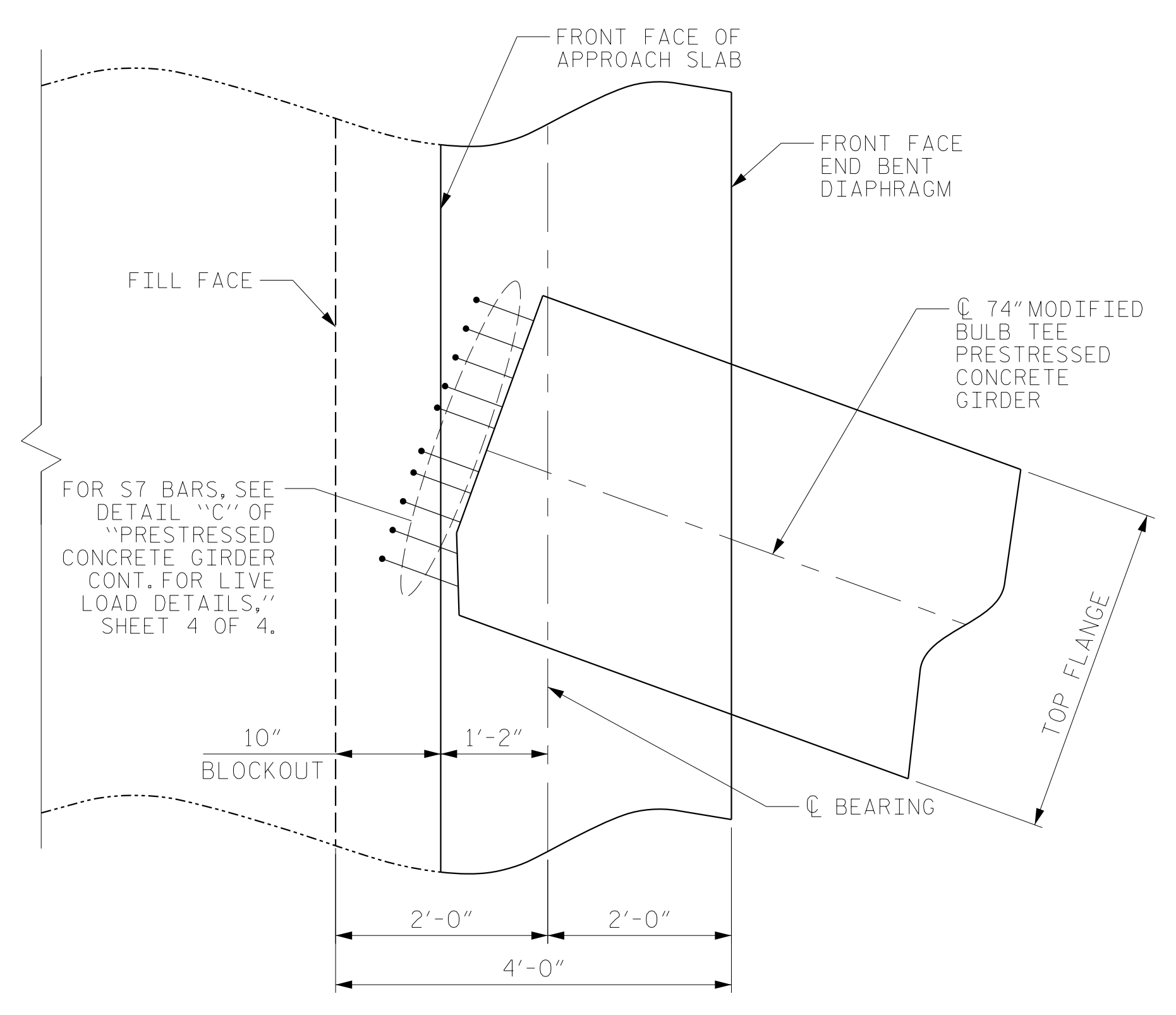
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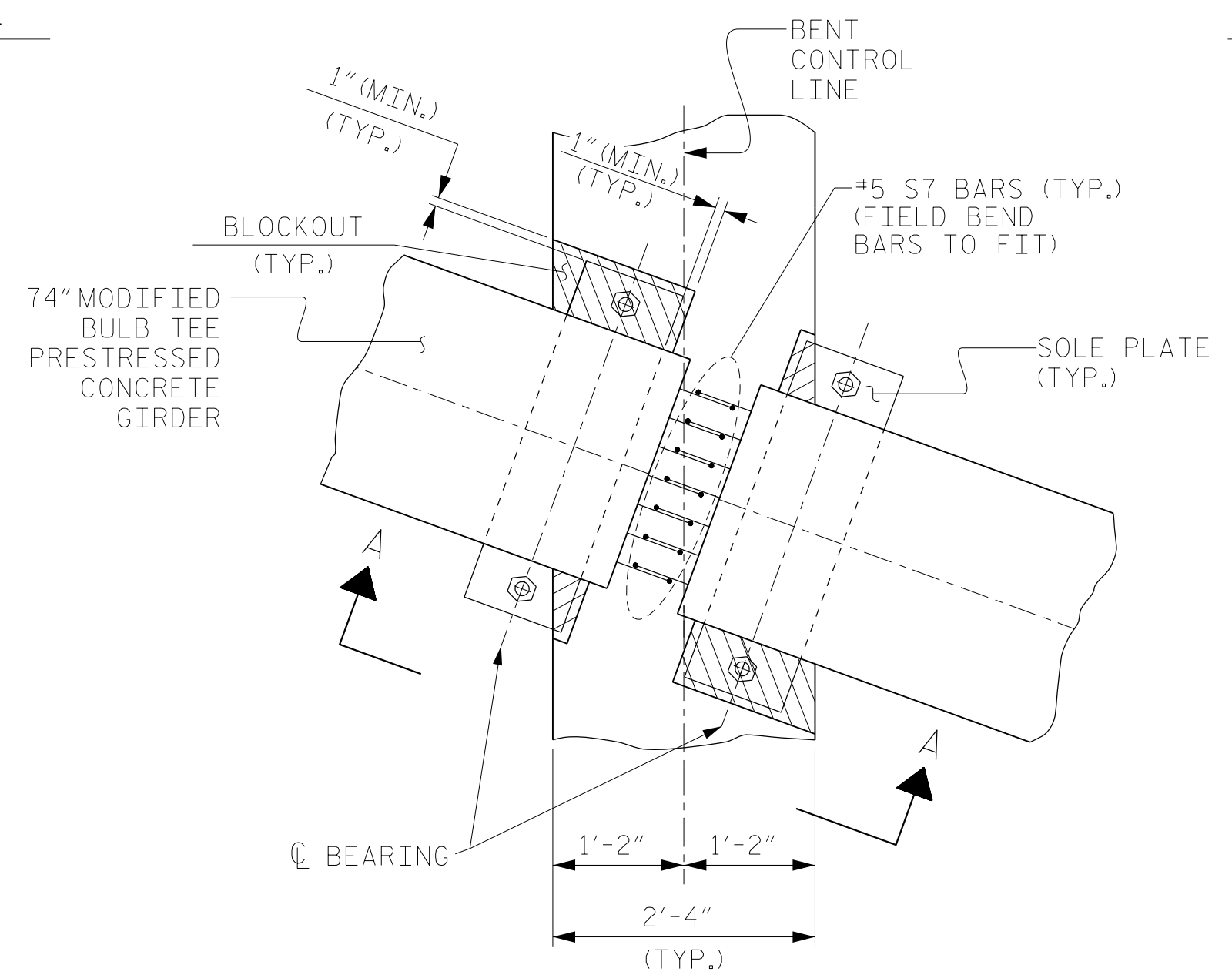
SECTION THRU INTEGRAL END BENT



SECTION THRU CONTINUOUS BENT DIAPHRAGM

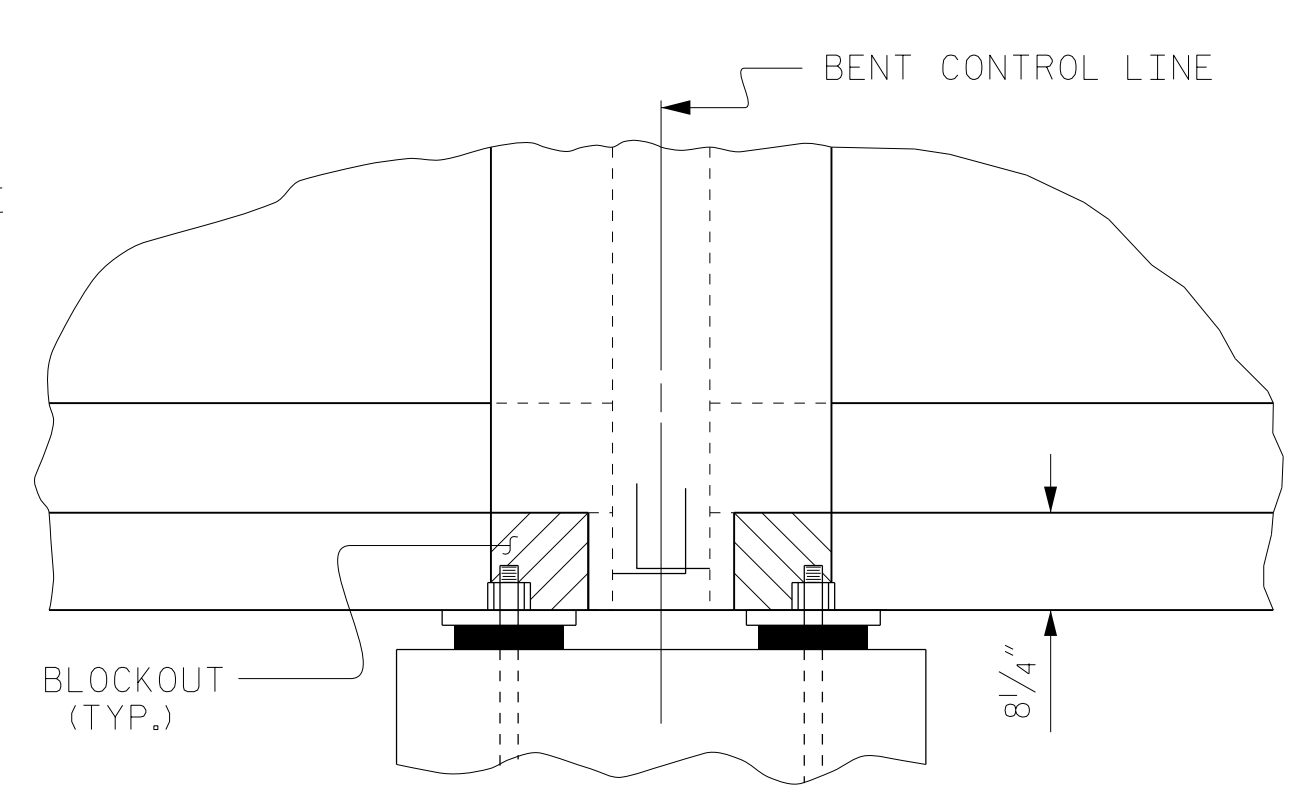


PLAN OF GIRDER @ INTEGRAL END BENT



PLAN VIEW

BENT DIAPHRAGM BLOCKOUT DETAIL



SECTION A-A

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 2 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 TYPICAL SECTION
 DETAILS
 RIGHT LANE

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DESIGN ENGINEER OF RECORD:	MAL	DATE :	11/2016

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 8601 Six Forks Road, Suite 260
 Raleigh, NC 27615
 919-926-4100 FAX 919-846-9080
 www.rsandh.com
 North Carolina License No. 50737-F-0403-1-C-01

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-7
1			3			TOTAL SHEETS
2			4			36

NOTES

FOR SPLICE LENGTHS NOT SHOWN, REFER TO MINIMUM SPLICE LENGTH TABLE ON "SUPERSTRUCTURE BILL OF MATERIAL" SHEET 1 OF 2.

FOR END BENT DIAPHRAGM BARS AND BENT DIAPHRAGM BARS, SEE "TYPICAL SECTION DETAILS" SHEET 2 OF 2.

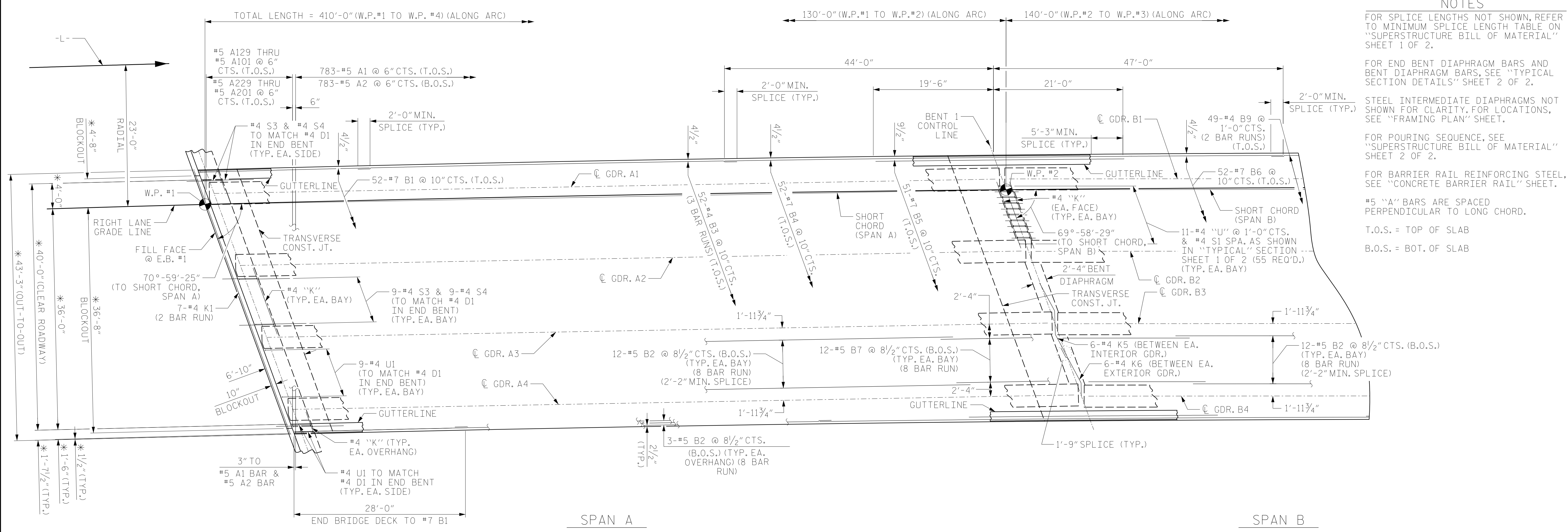
STEEL INTERMEDIATE DIAPHRAGMS NOT SHOWN FOR CLARITY, FOR LOCATIONS, SEE "FRAMING PLAN" SHEET.

FOR POURING SEQUENCE, SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET 2 OF 2.

FOR BARRIER RAIL REINFORCING STEEL, SEE "CONCRETE BARRIER RAIL" SHEET.

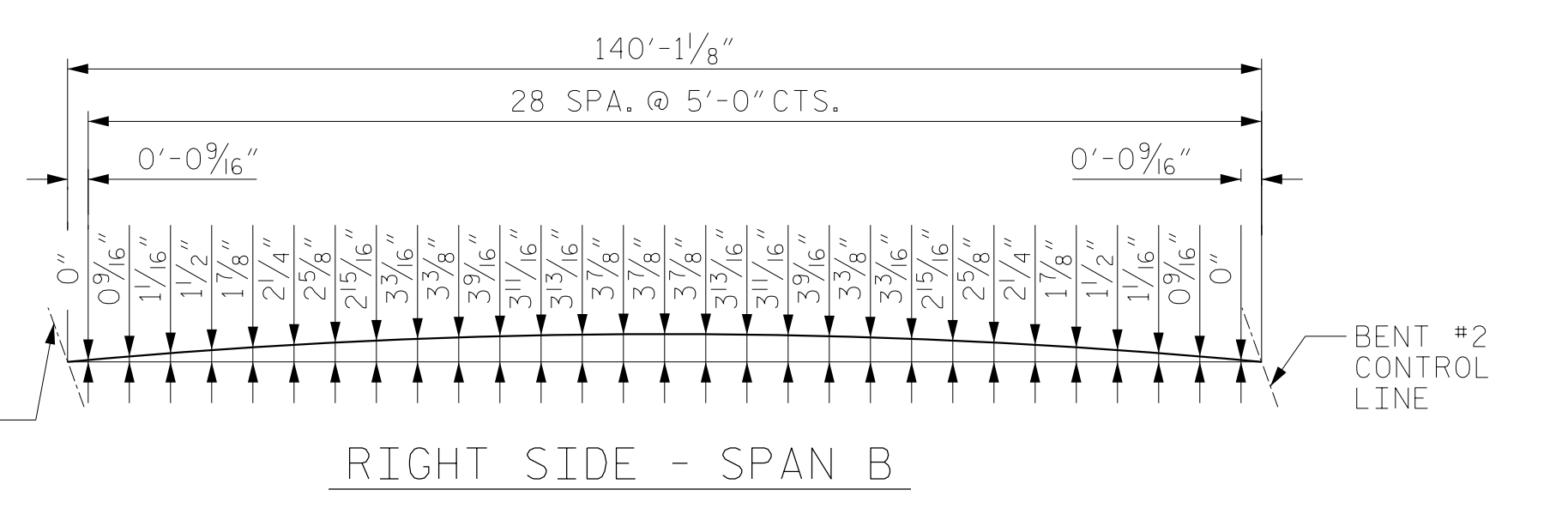
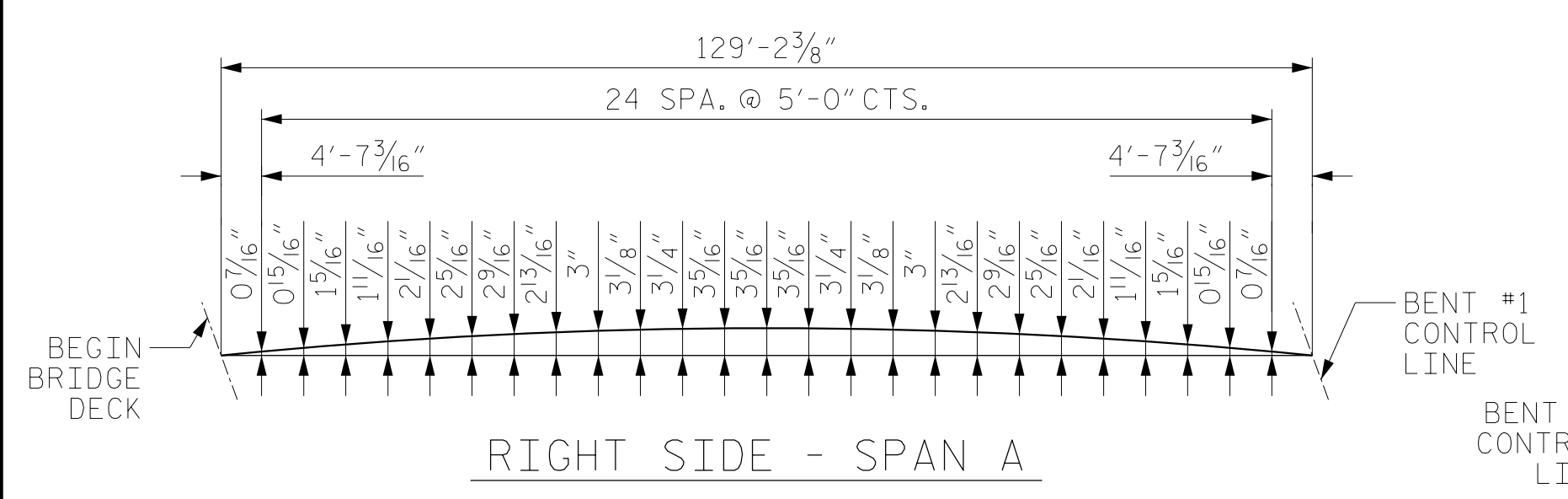
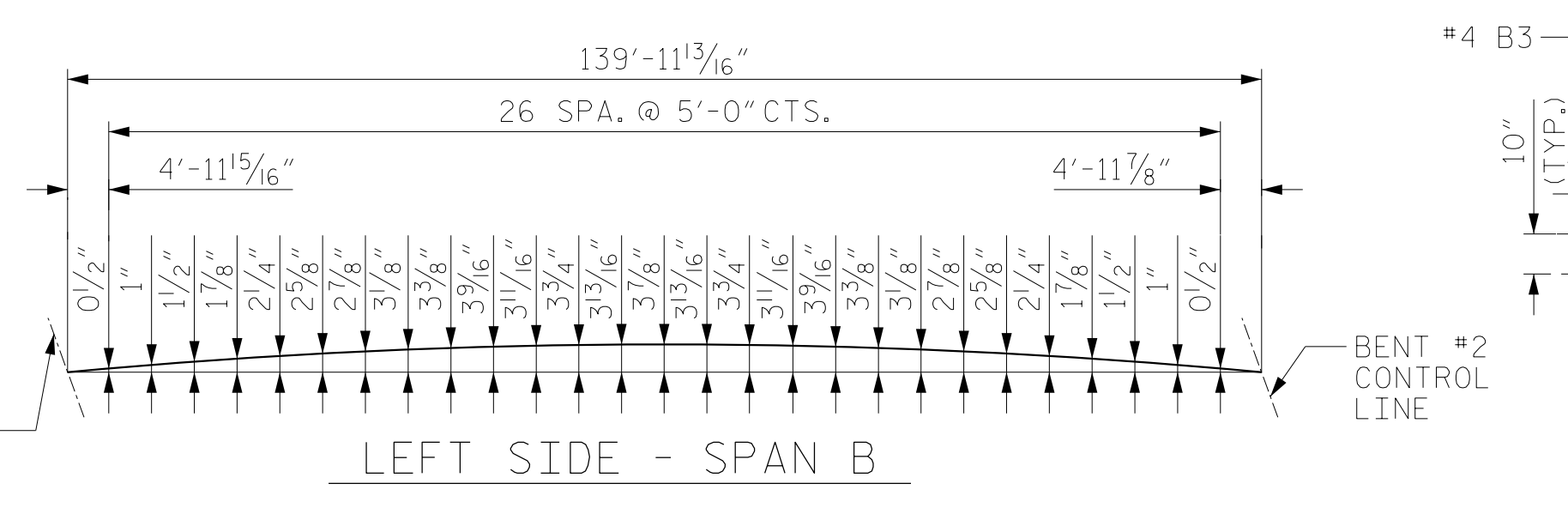
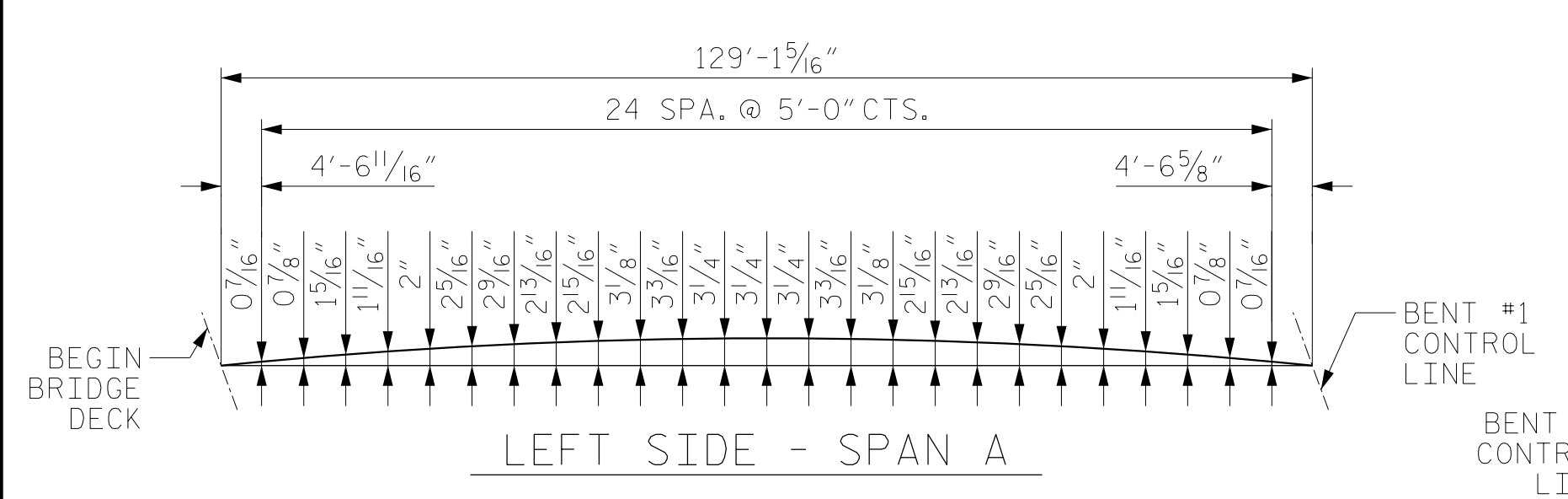
#5 "A" BARS ARE SPACED PERPENDICULAR TO LONG CHORD.

T.O.S. = TOP OF SLAB
B.O.S. = BOT. OF SLAB



PARTIAL PLAN OF SPANS

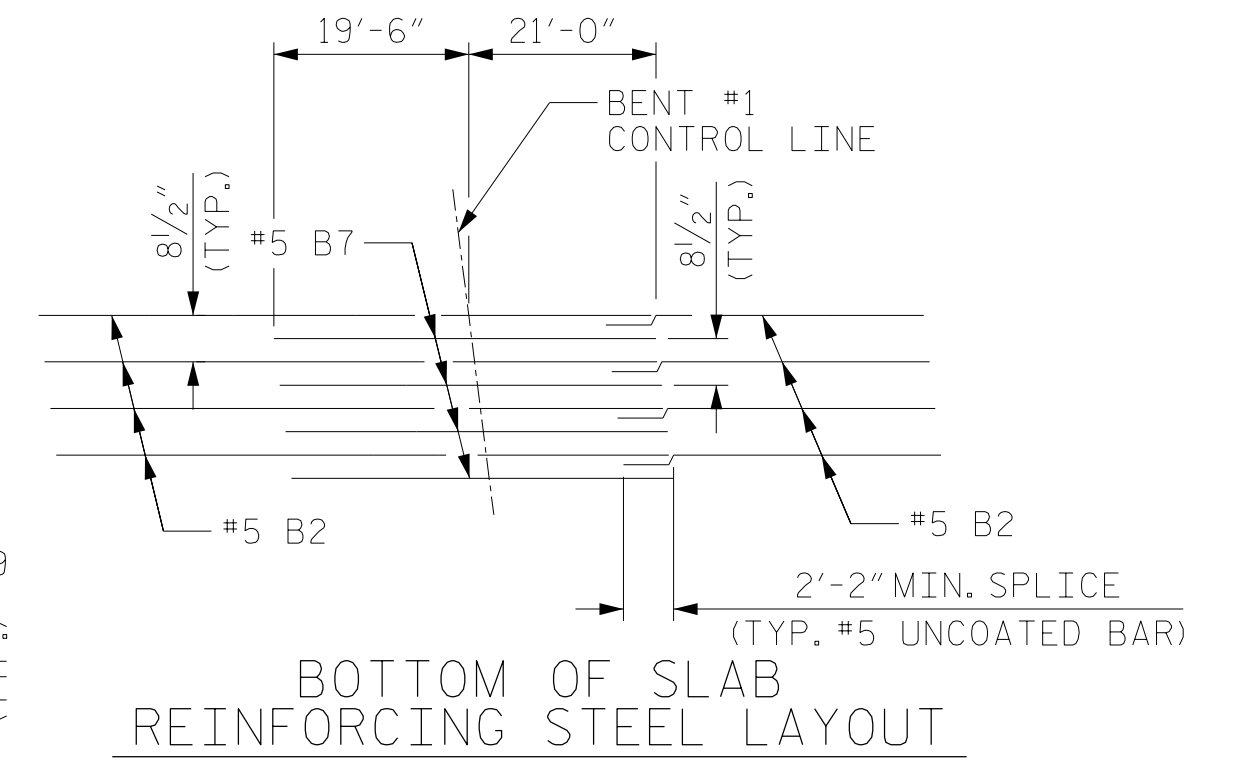
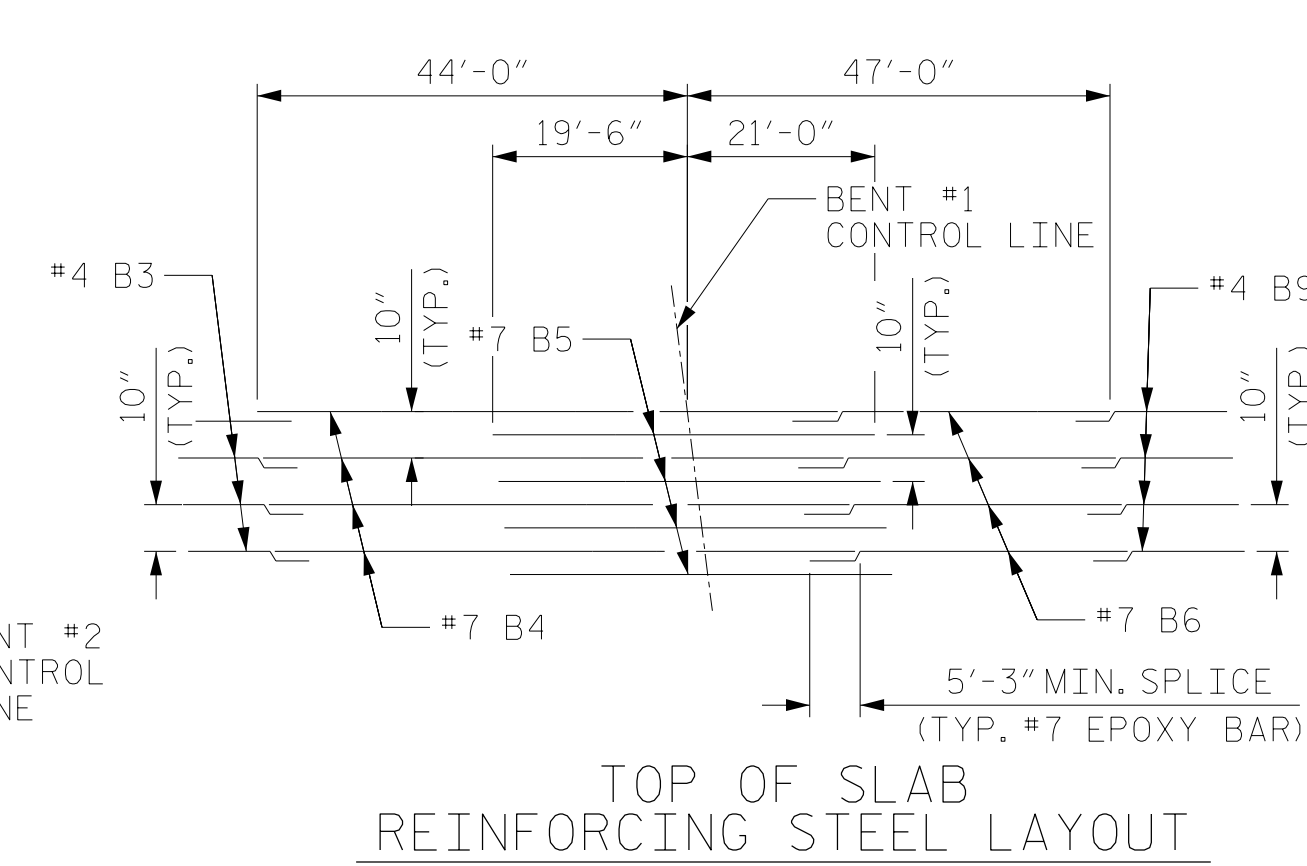
* RADIAL DIMENSIONS



ARC OFFSETS

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 DESIGN ENGINEER OF RECORD: MAL DATE : 12/2016

3/8/2017
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PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 1 OF 2

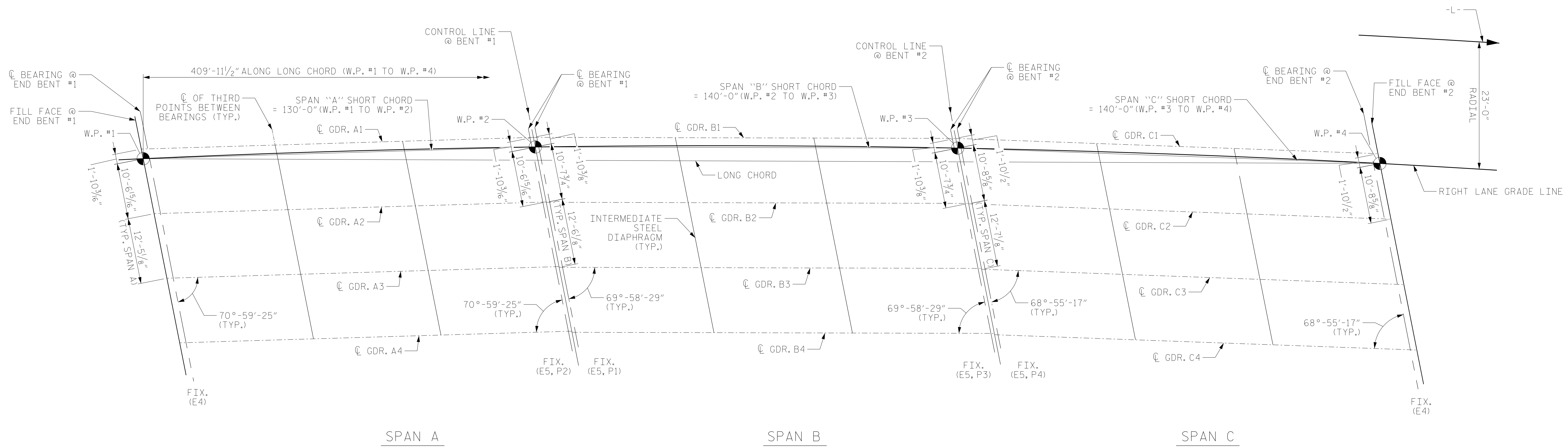


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 PLAN OF SPANS
 RIGHT LANE

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RS&H Architects-Engineers-Planners, Inc.
 8601 Six Forks Road, Suite 260
 Raleigh, NC 27615
 919-926-4100 FAX 919-846-9080
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-8
1			3			TOTAL SHEETS
2			4			36



FRAMING PLAN

END BENT DIAPHRAGMS & BENT DIAPHRAGMS ARE NOT SHOWN
 DIMENSIONS TO INTERMEDIATE DIAPHRAGM LOCATIONS ARE SHOWN ON GIRDER SHEETS
 ALL DIMENSIONS ARE TO THE FILL FACE OR CONTROL LINE OF EACH BENT

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

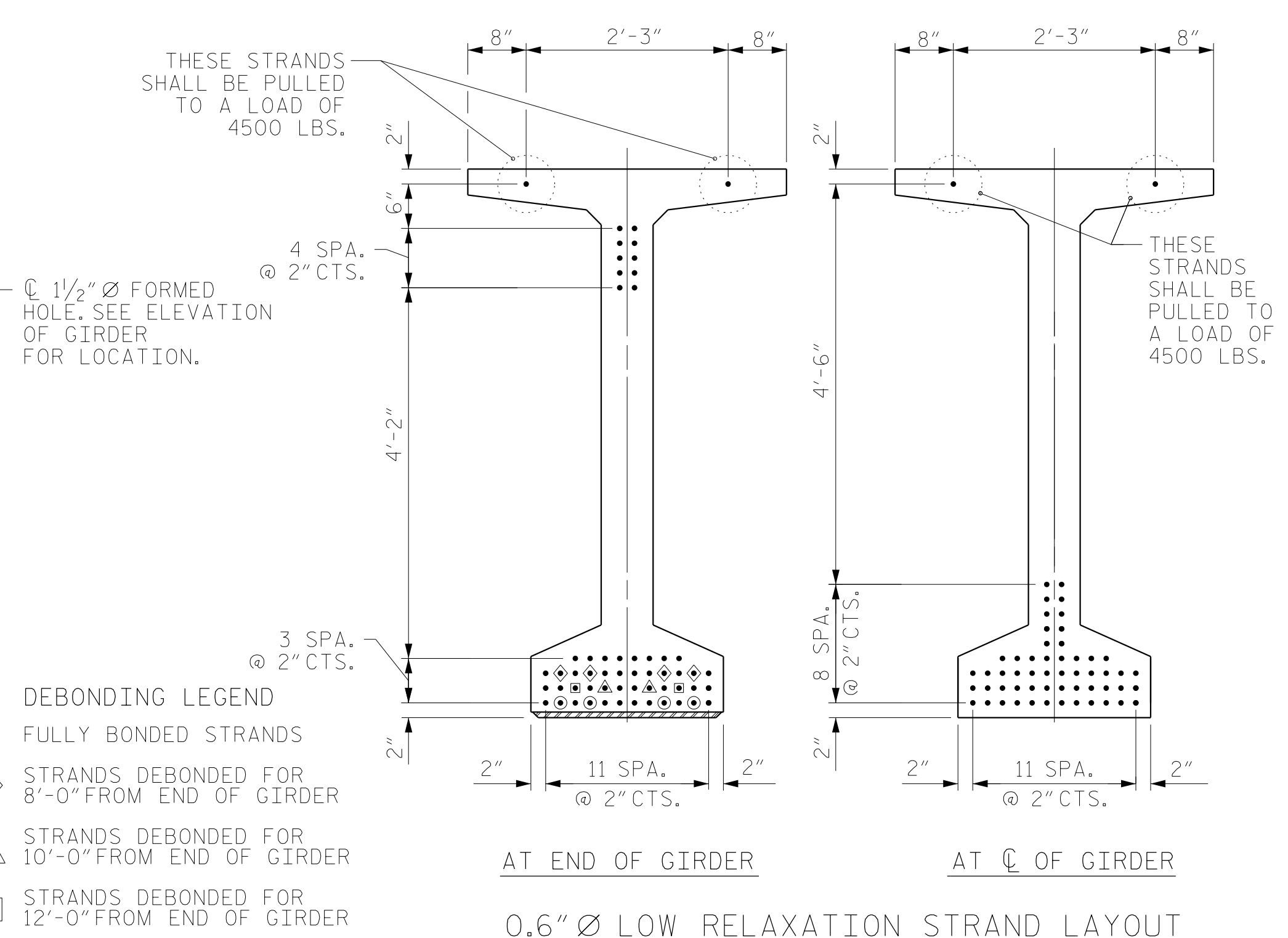
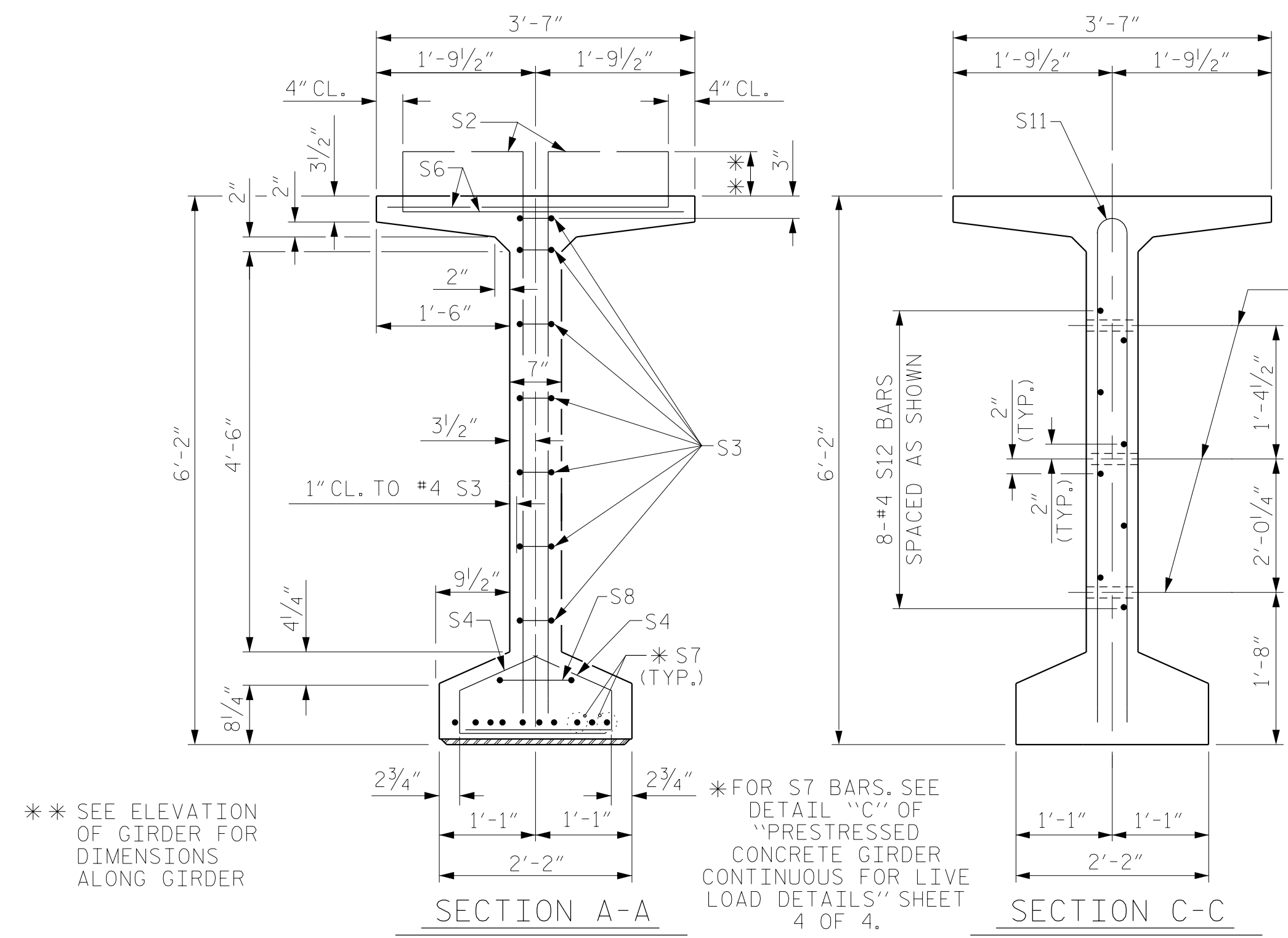
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 CHECKED BY : TLC DATE : 01/2017
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

3/8/2017
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 leblancm

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE FRAMING PLAN SPANS A, B & C RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S2-10
TOTAL SHEETS					36



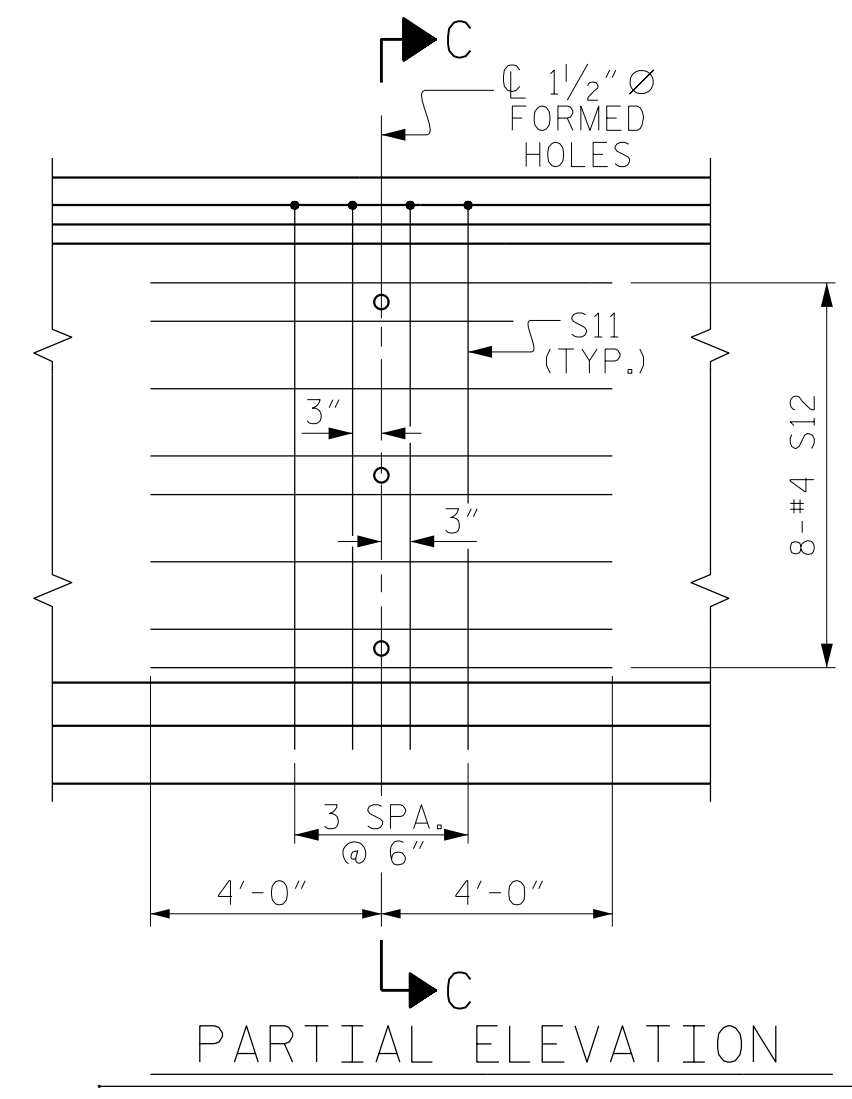
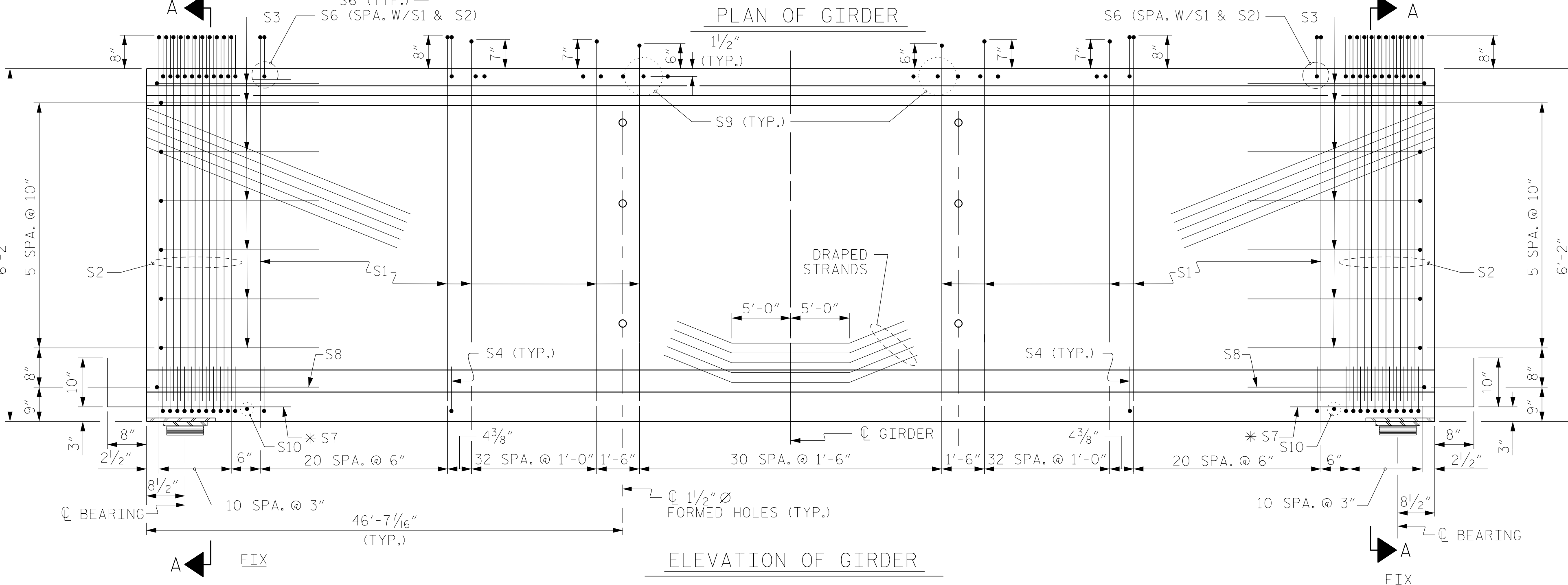
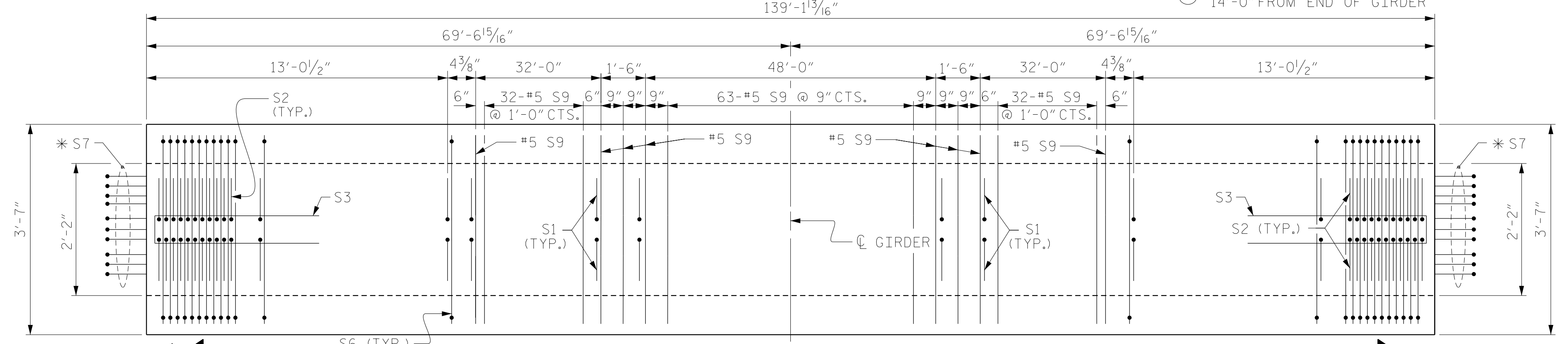
- DEBONDING LEGEND**
- FULLY BONDED STRANDS
 - ◊ STRANDS DEBONDED FOR 8'-0" FROM END OF GIRDER
 - ▲ STRANDS DEBONDED FOR 10'-0" FROM END OF GIRDER
 - ◻ STRANDS DEBONDED FOR 12'-0" FROM END OF GIRDER
 - STRANDS DEBONDED FOR 14'-0" FROM END OF GIRDER

** SEE ELEVATION OF GIRDER FOR DIMENSIONS ALONG GIRDER

* FOR S7 BARS. SEE DETAIL "C" OF "PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD DETAILS" SHEET 4 OF 4.

(S1 AND S9 BARS NOT SHOWN)

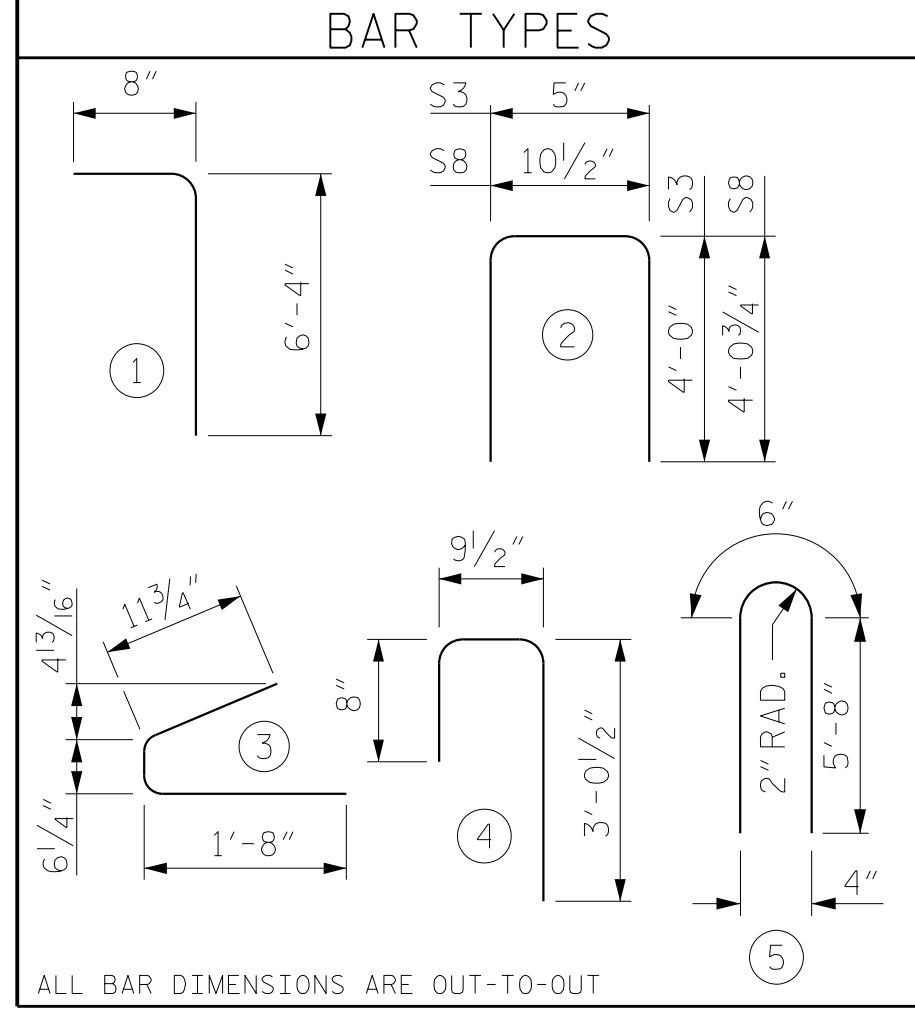
0.6" Ø LOW RELAXATION STRAND LAYOUT
THE UPLIFT FORCE DUE TO DRAPED STRANDS IS 27.2 KIPS (56 STRANDS REQUIRED)



0.6" Ø L. R. GRADE 270 STRANDS		
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	278	#4	1	7'-0"	1300
S2	44	#5	1	7'-0"	321
S3	14	#4	2	8'-5"	79
S4	116	#4	3	3'-2"	245
S6	128	#5	4	4'-6"	601
* S7	20	#5	STR	3'-8"	76
S8	2	#5	2	9'-0"	19
S9	135	#5	STR	3'-3"	458
S10	2	#3	STR	1'-10"	1
S11	8	#5	5	11'-10"	99
S12	16	#4	STR	8'-0"	86

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



QUANTITIES FOR ONE GIRDER			
REINFORCING STEEL	9000 PSI CONCRETE		0.6" Ø L.R. STRANDS
	LB.	C.Y.	No.
	3285	31.7	56

GIRDERS REQUIRED		
NUMBER	LENGTH	TOTAL LENGTH
4	139'-1 13/16"	556'-7 1/4"

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 2 OF 4

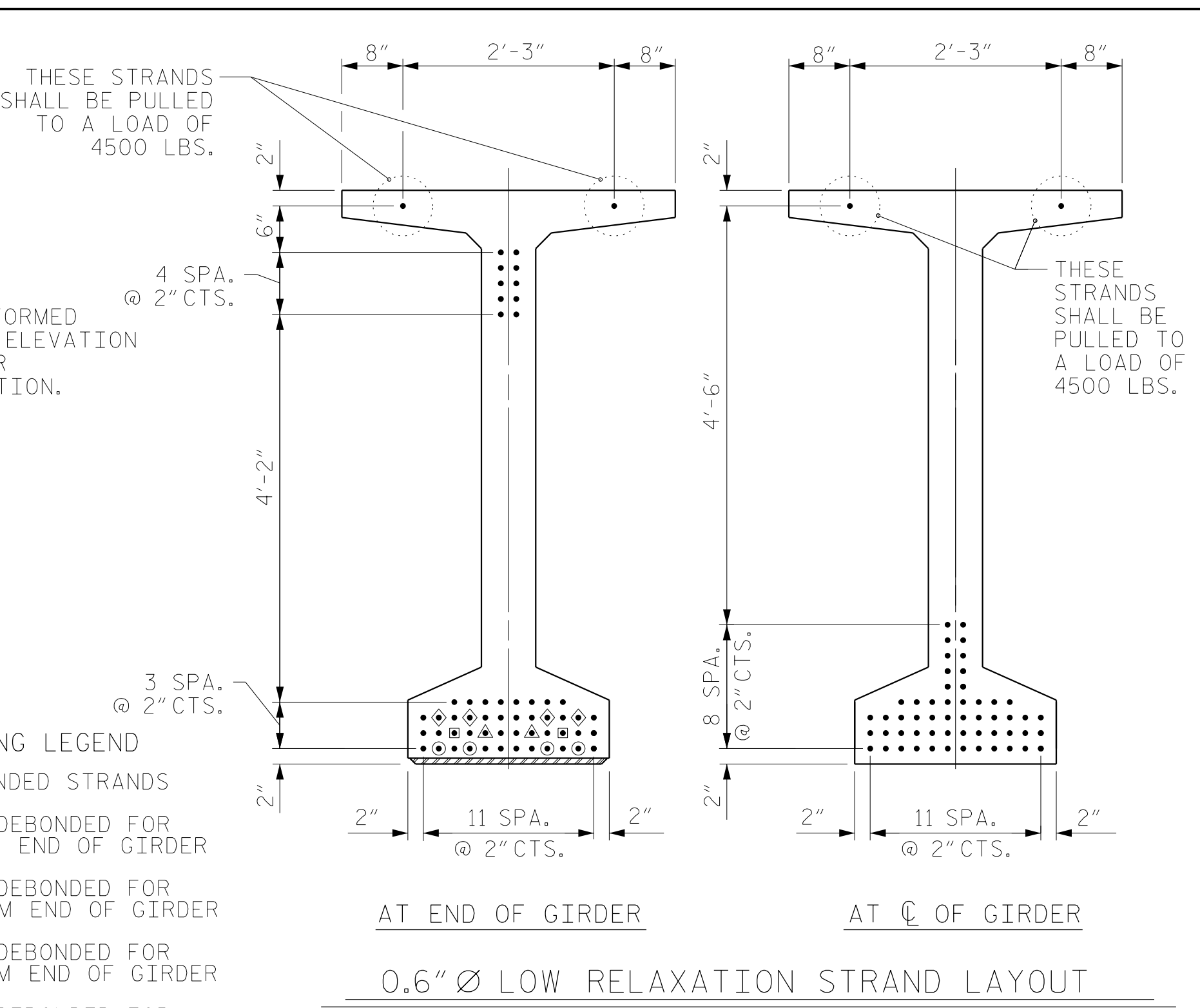
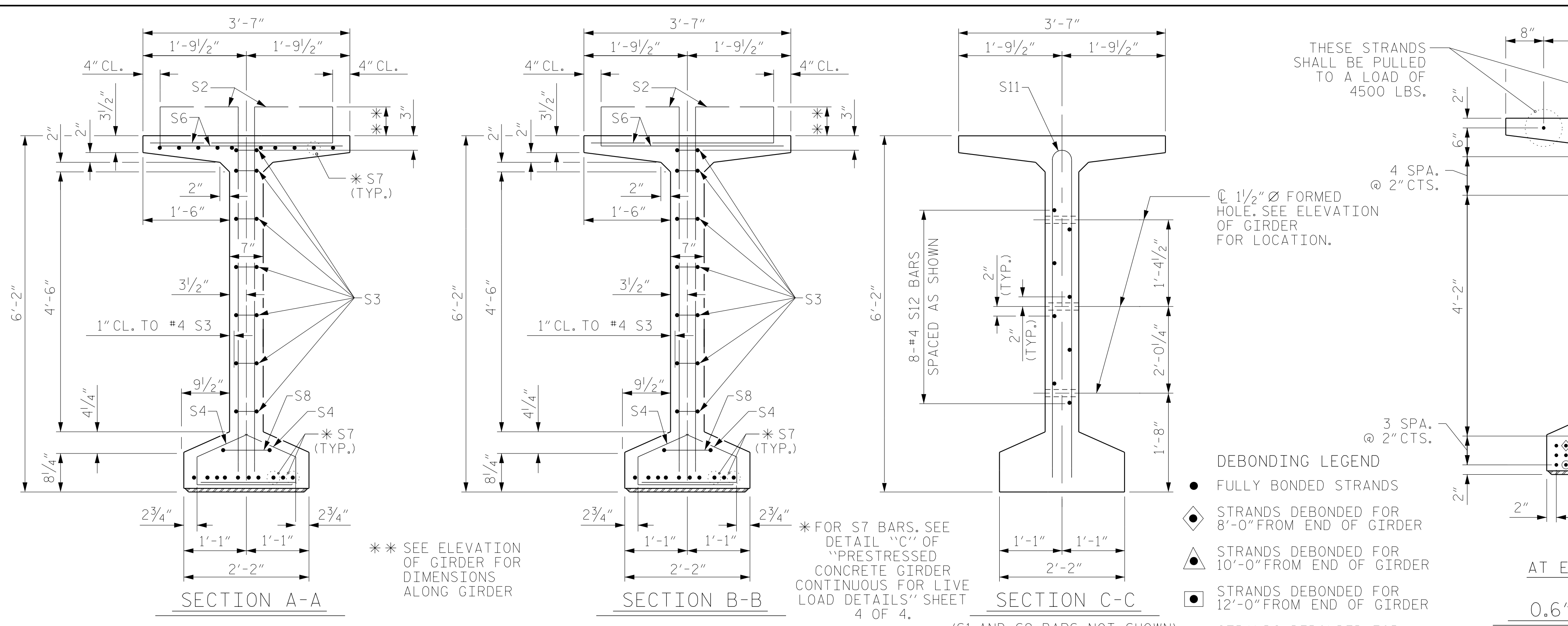


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 74" PRESTRESSED CONCRETE
 MODIFIED BULB TEE
 CONTINUOUS FOR LIVE LOAD
 SPAN B
 RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-12
1			3			TOTAL SHEETS
2			4			36

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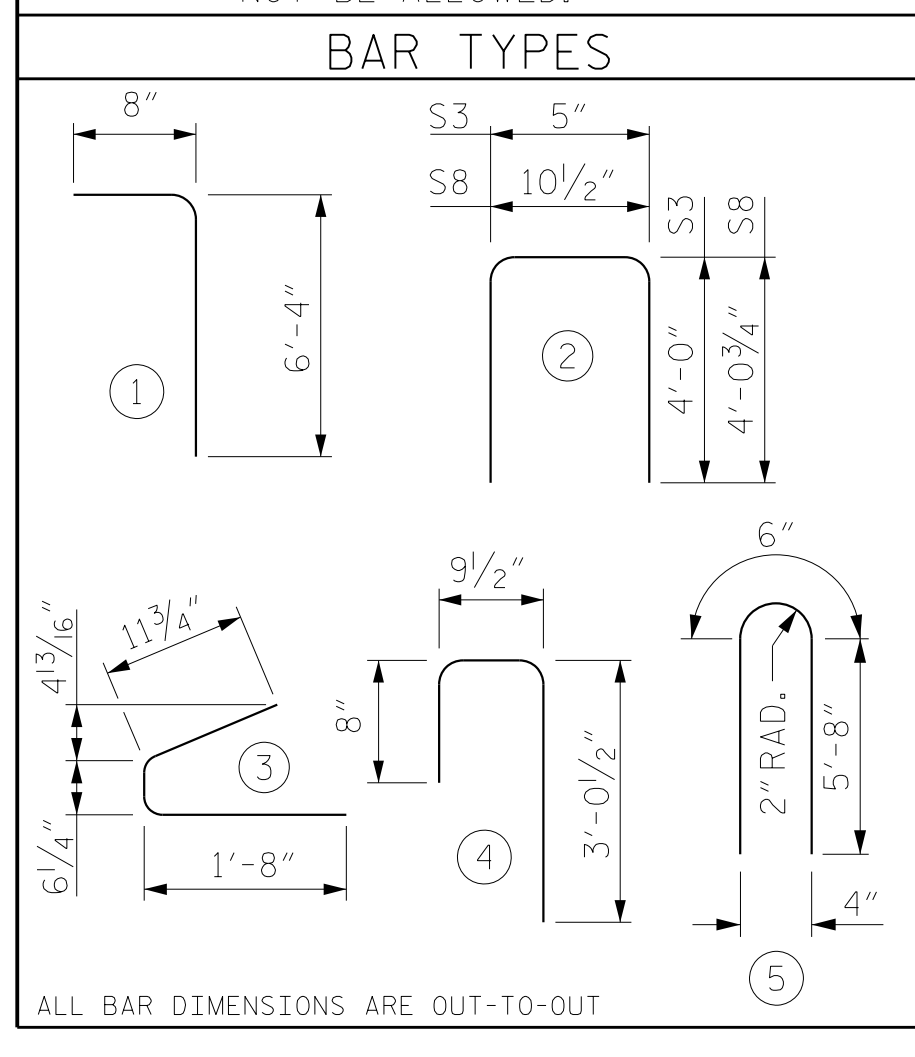
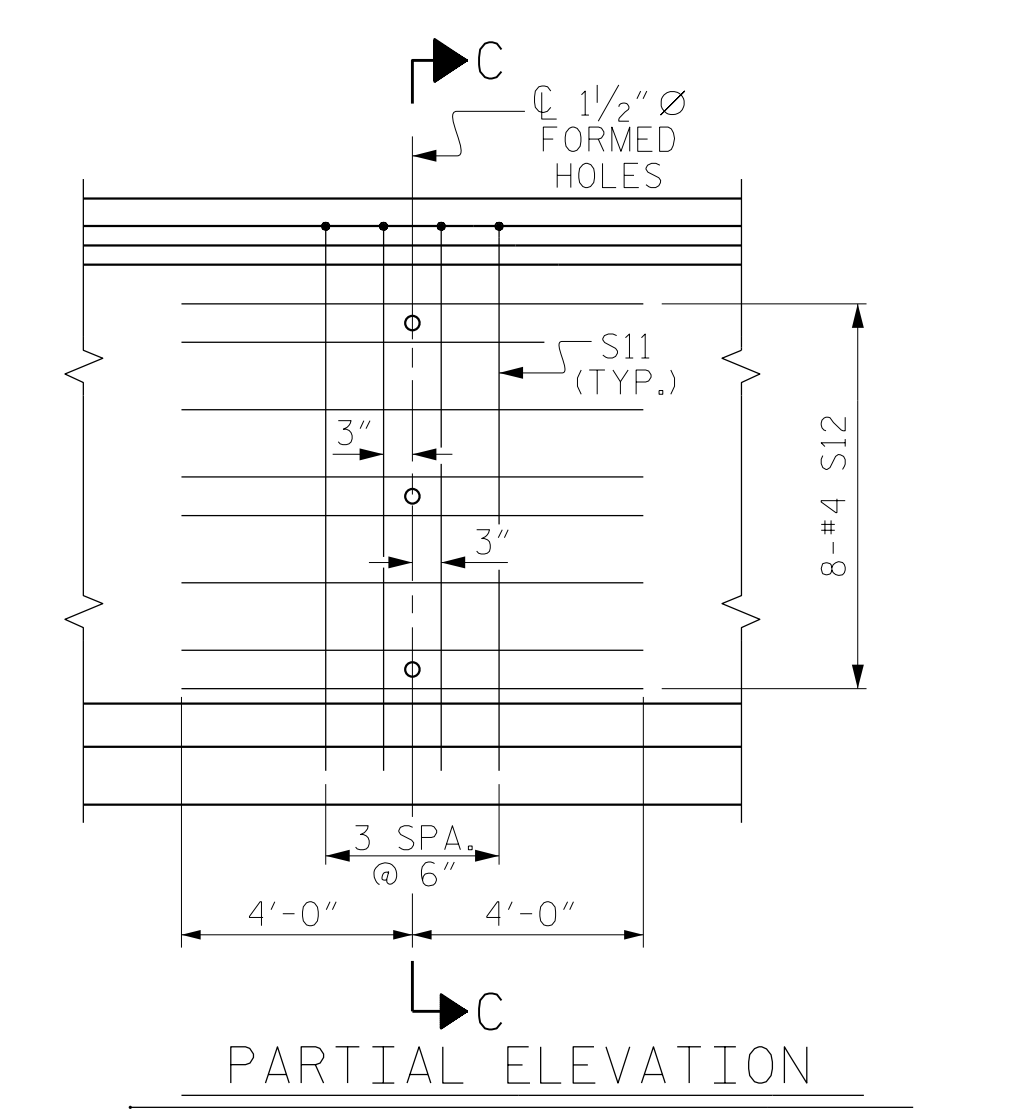
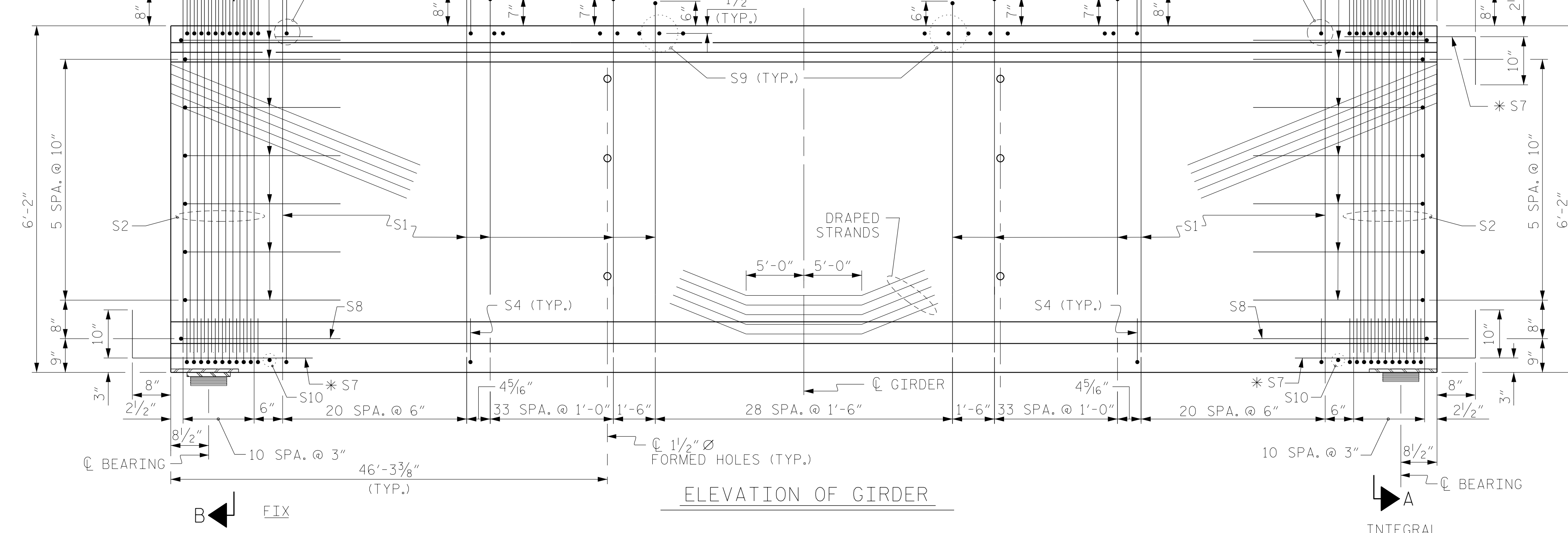
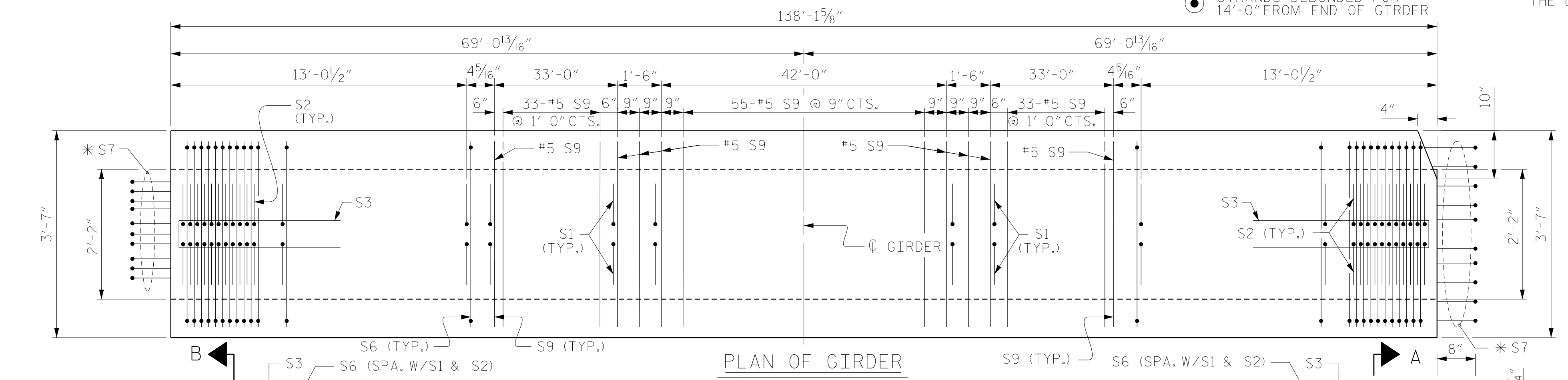
0.6" Ø L. R. GRADE 270 STRANDS

AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR

BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	278	#4	1	7'-0"	1300
S2	44	#5	1	7'-0"	321
S3	14	#4	2	8'-5"	79
S4	116	#4	3	3'-2"	245
S6	128	#5	4	4'-6"	601
*S7	30	#5	STR	3'-8"	115
S8	2	#5	2	9'-0"	19
S9	129	#5	STR	3'-3"	437
S10	2	#3	STR	1'-10"	1
S11	8	#5	5	11'-10"	99
S12	16	#4	STR	8'-0"	86

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



QUANTITIES FOR ONE GIRDER

REINFORCING STEEL	9000 PSI CONCRETE		0.6" Ø L.R. STRANDS
	LB.	C.Y.	No.
	3303	31.4	56

GIRDERS REQUIRED

NUMBER	LENGTH	TOTAL LENGTH
4	138'-1 5/8"	552'-6 1/2"

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 3 OF 4



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 74" PRESTRESSED CONCRETE
 MODIFIED BULB TEE
 CONTINUOUS FOR LIVE LOAD
 SPAN C
 RIGHT LANE

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1			3		
2			4		

SHEET NO. S2-13
 TOTAL SHEETS 36

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. BEVEL EDGES OF PLATE "B-1" TO GIVE CLOSE FIT BUT NOT TIGHT FIT TO STEEL CASTING FORM.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 7200 PSI FOR SPAN A AND 7700 PSI FOR SPANS B AND C.

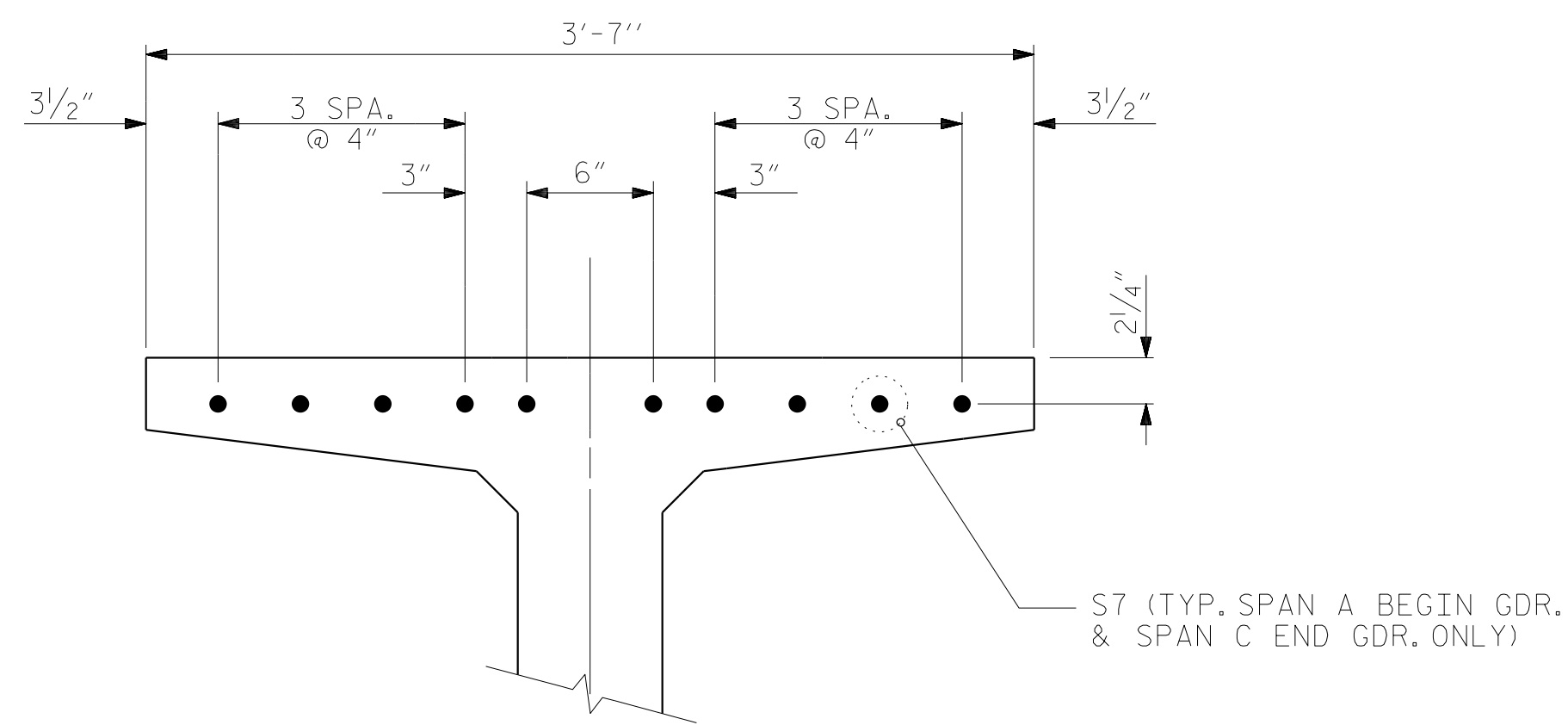
DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

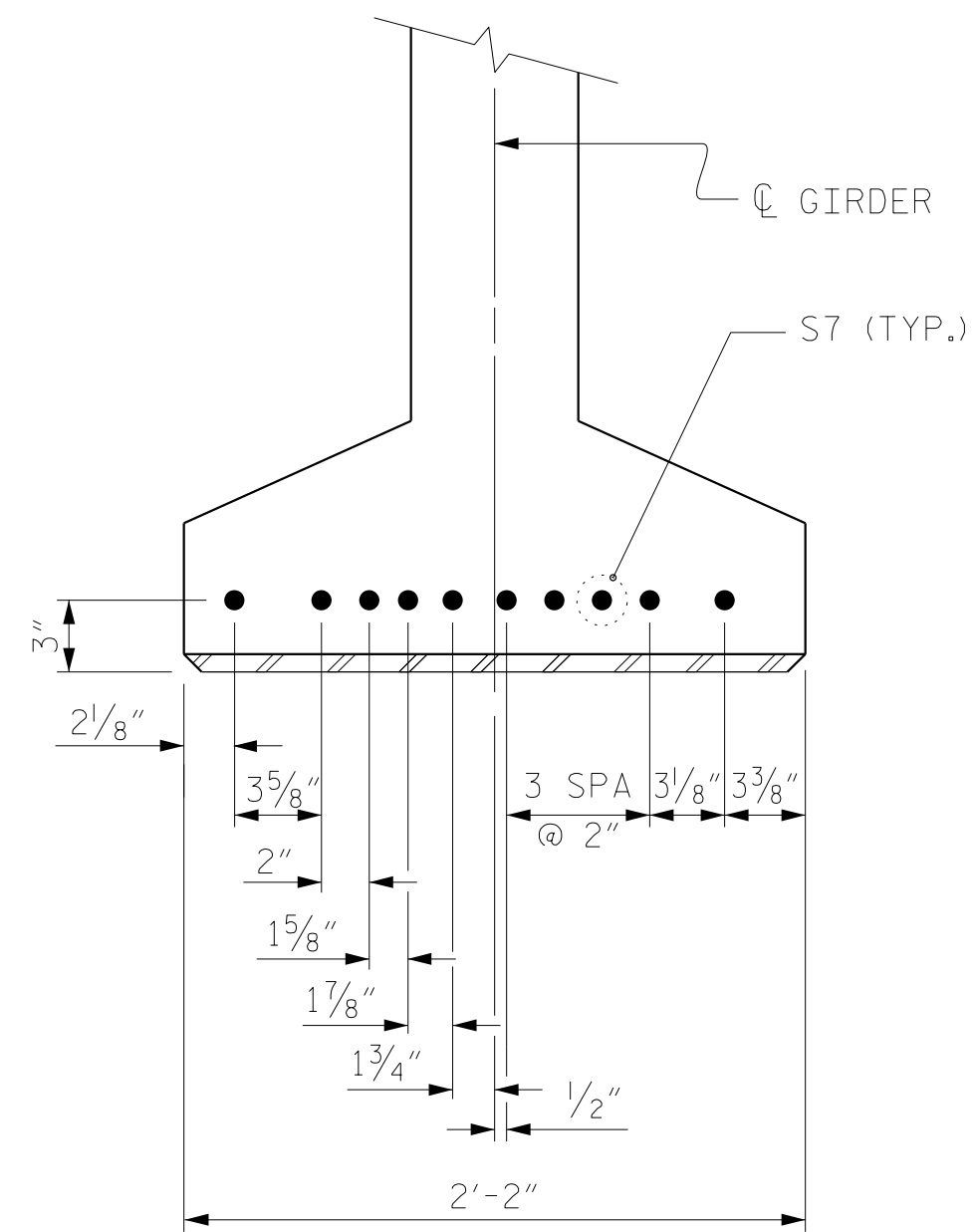
WHEN DRAPED STRANDS ARE DETAILED, THE LONGITUDINAL LOCATION OF THE HOLD DOWN DEVICES SHALL BE WITHIN 6" OF THE LOCATION SHOWN AND THE CENTER OF GRAVITY OF THE GROUP OF DRAPED STRANDS SHALL BE LOCATED WITHIN 1/2" OF THE THEORETICAL LOCATION SHOWN.

A 2" x 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 74" MODIFIED PRESTRESSED CONCRETE GIRDER.

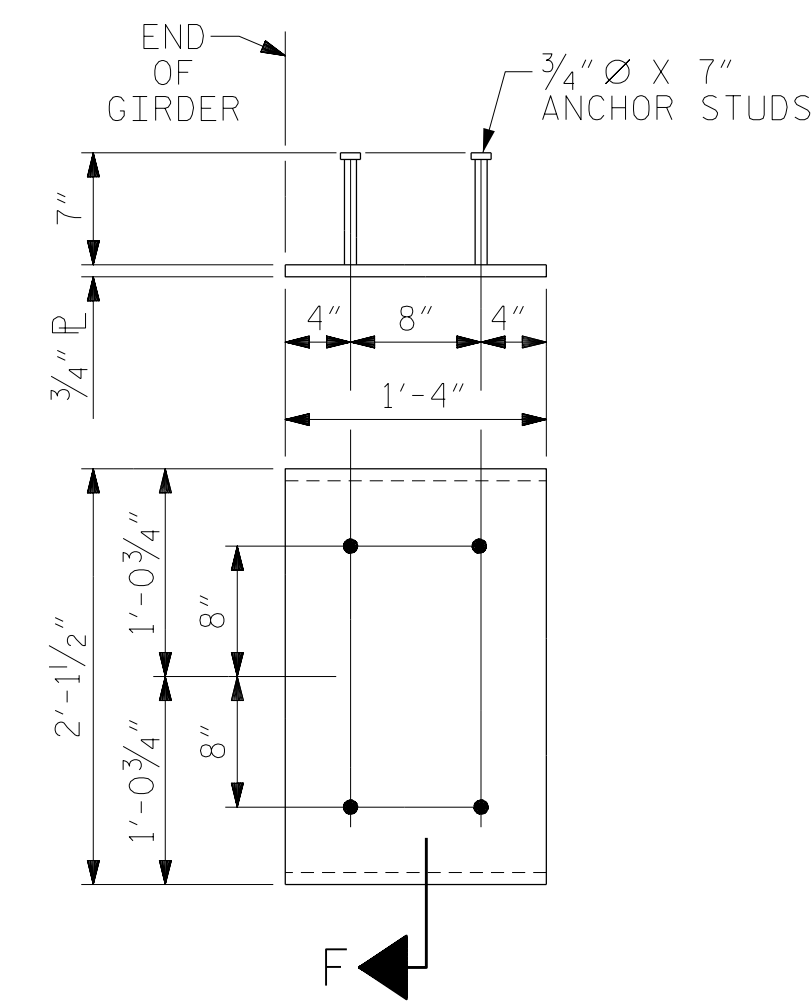
FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.



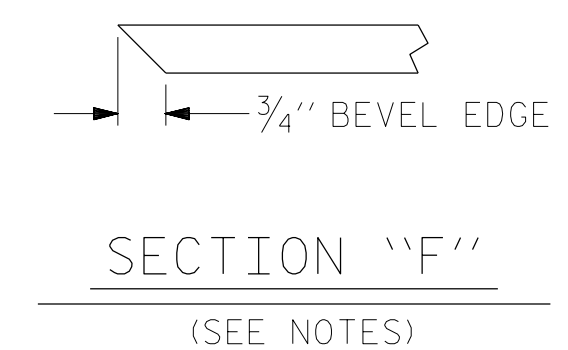
S7 (TYP. SPAN A BEGIN GDR. & SPAN C END GDR. ONLY)



DETAIL "C"



EMBEDDED PLATE "B-1" DETAILS FOR 74" MODIFIED PRESTRESSED CONCRETE GIRDER
(2 REQ'D PER GIRDER)



PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 4 OF 4

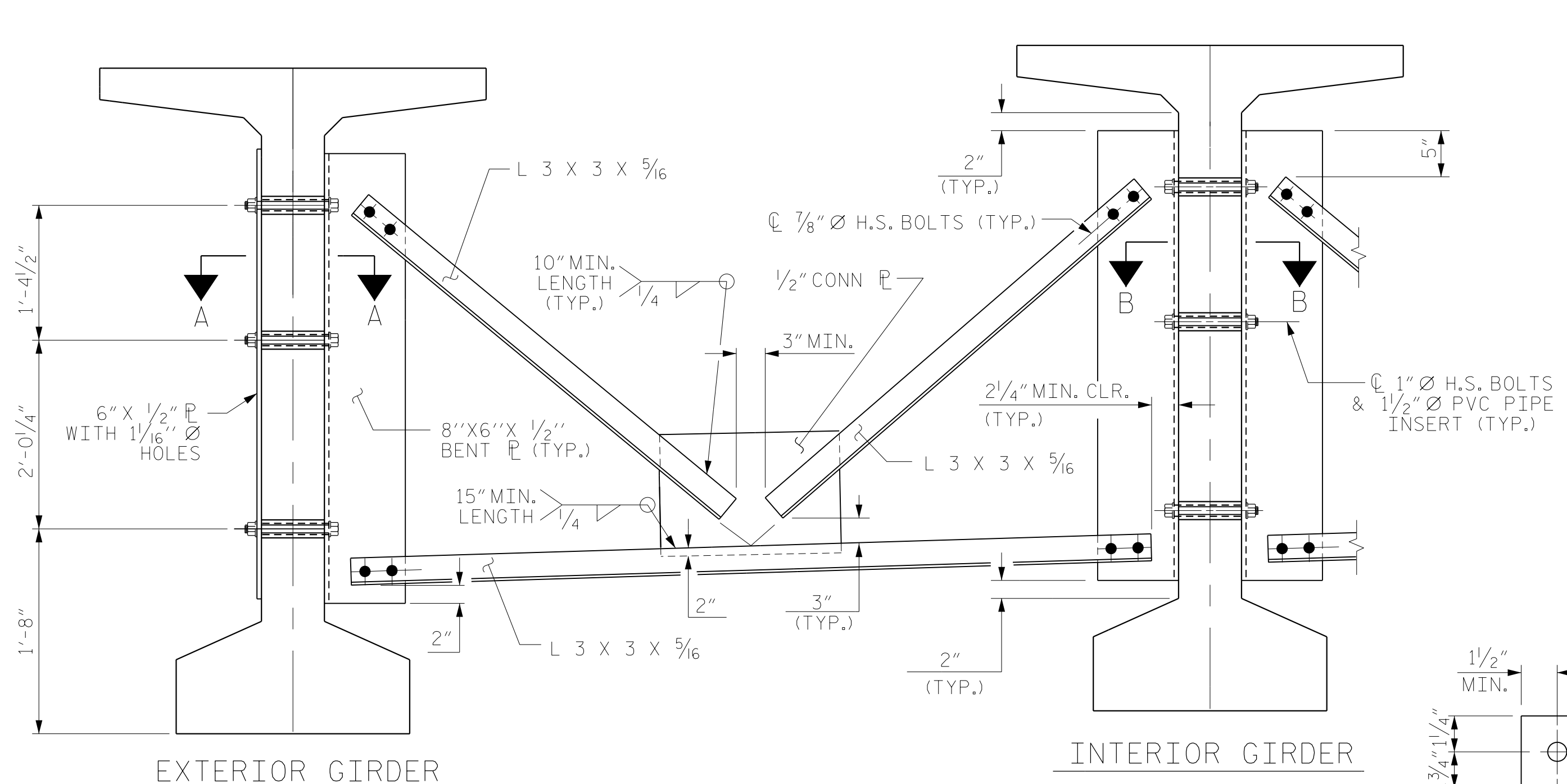


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 PRESTRESSED CONCRETE GIRDER
 CONTINUOUS FOR LIVE LOAD
 DETAILS
 RIGHT LANE

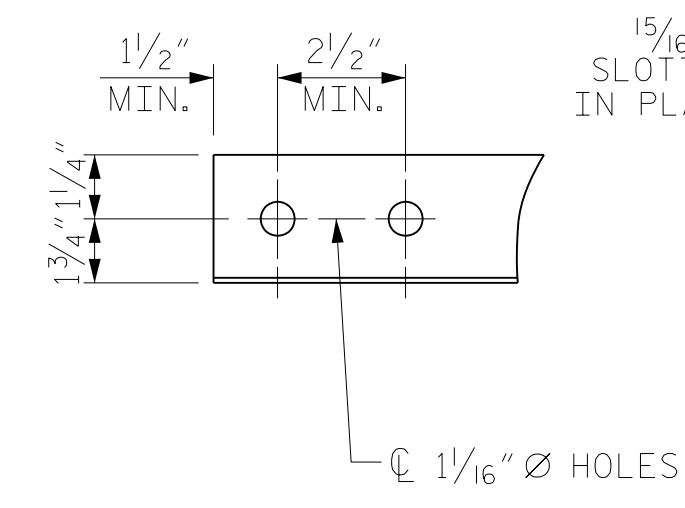
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			36
2			4			

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CHECKED BY :	GRP 11/91	REV. 1/15	MAA/TMG
		REV. 2/15	MAA/TMG

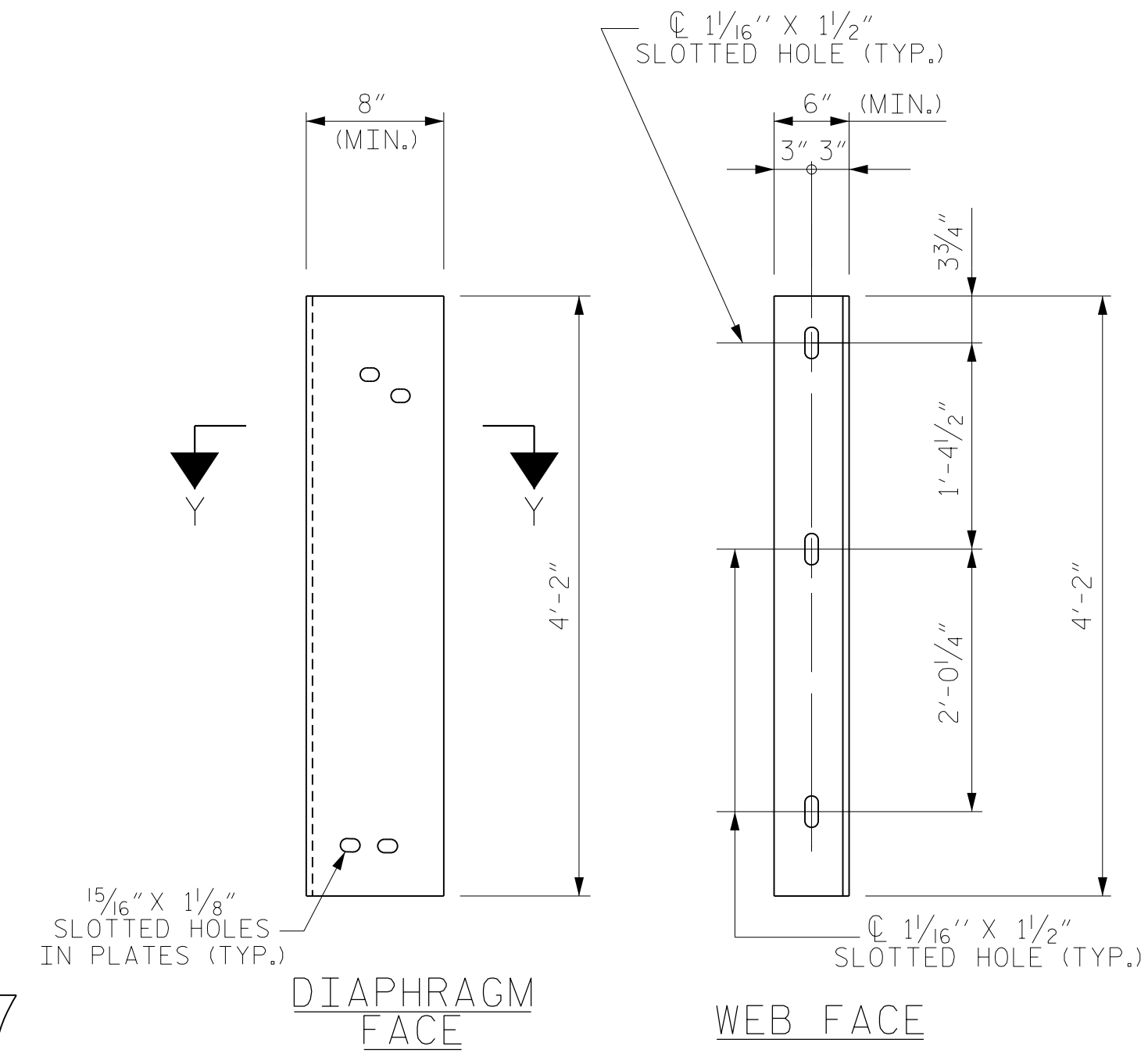
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PART SECTION AT INTERMEDIATE DIAPHRAGM



ANGLE END
(L 3 x 3 x 5/16)



CONNECTOR PLATE DETAIL

STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

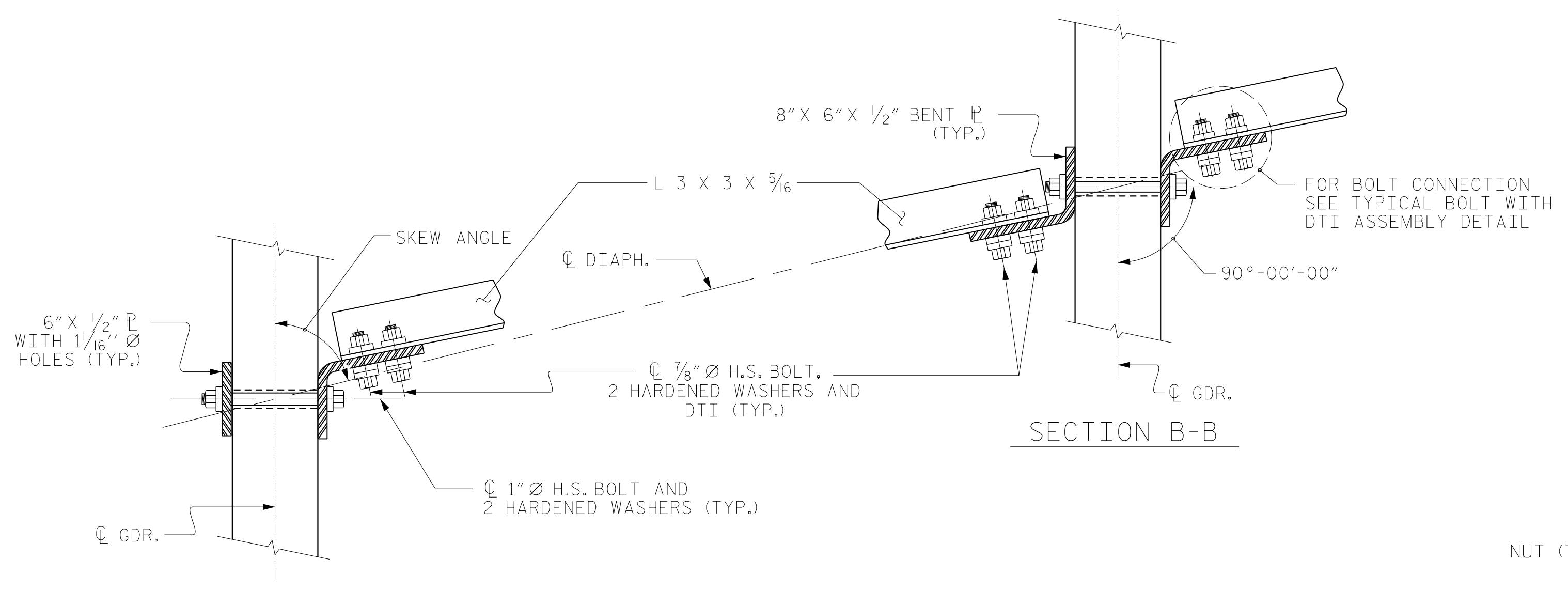
FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

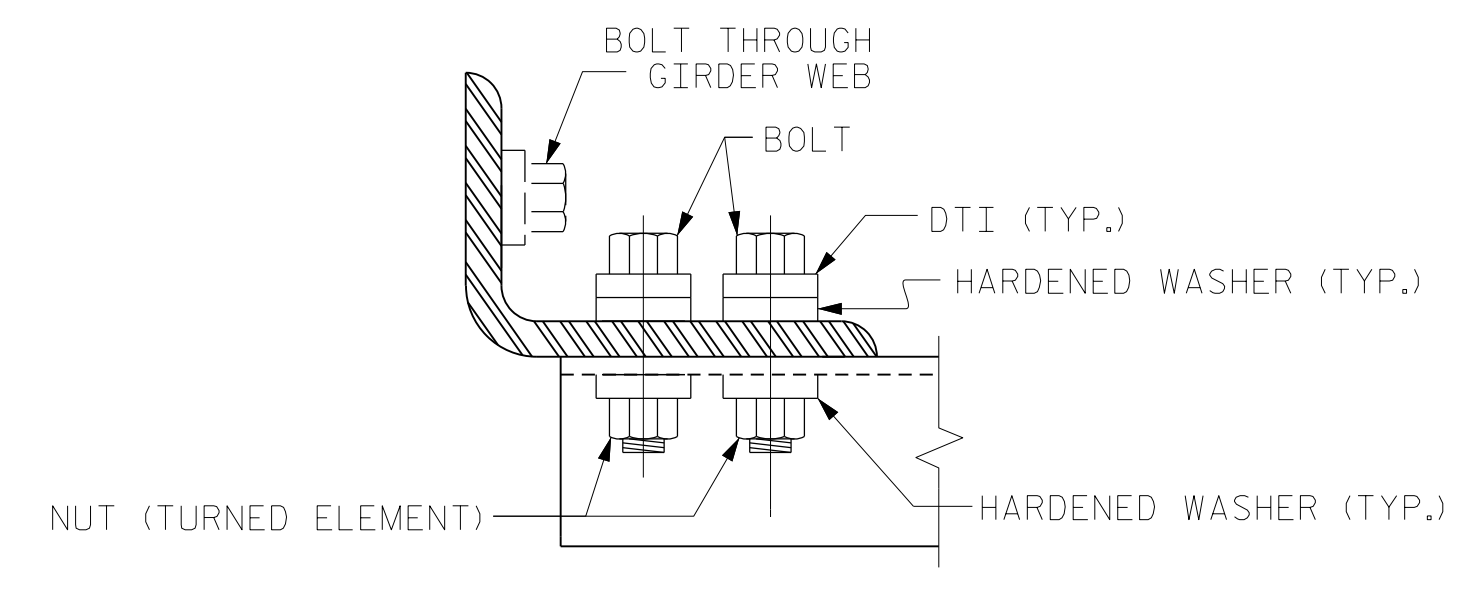
SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.



CONNECTION DETAILS



BOLT WITH DTI ASSEMBLY DETAIL

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

Professional Engineer Seal for RS&H Architects-Engineers-Planners, Inc. The seal is circular and contains the text: STATE OF NORTH CAROLINA, PROFESSIONAL ENGINEER, SEAL, 039213, 3/9/2017, MARK ROBERTSON, LICENSE NO. 10000, NORTH CAROLINA. Below the seal is the company name and address: RS&H Architects-Engineers-Planners, Inc., 8601 Six Forks Road, Suite 260, Raleigh, NC 27615, 919-926-4100 FAX 919-846-9080, www.rsandh.com, North Carolina License No. 50737-F-0403-C-02.

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
INTERMEDIATE
STEEL DIAPHRAGMS
RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-15
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : PDS	DATE : 11/2016
CHECKED BY : TLC	DATE : 01/2017
DESIGN ENGINEER OF RECORD: MAL	DATE : 11/2016

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

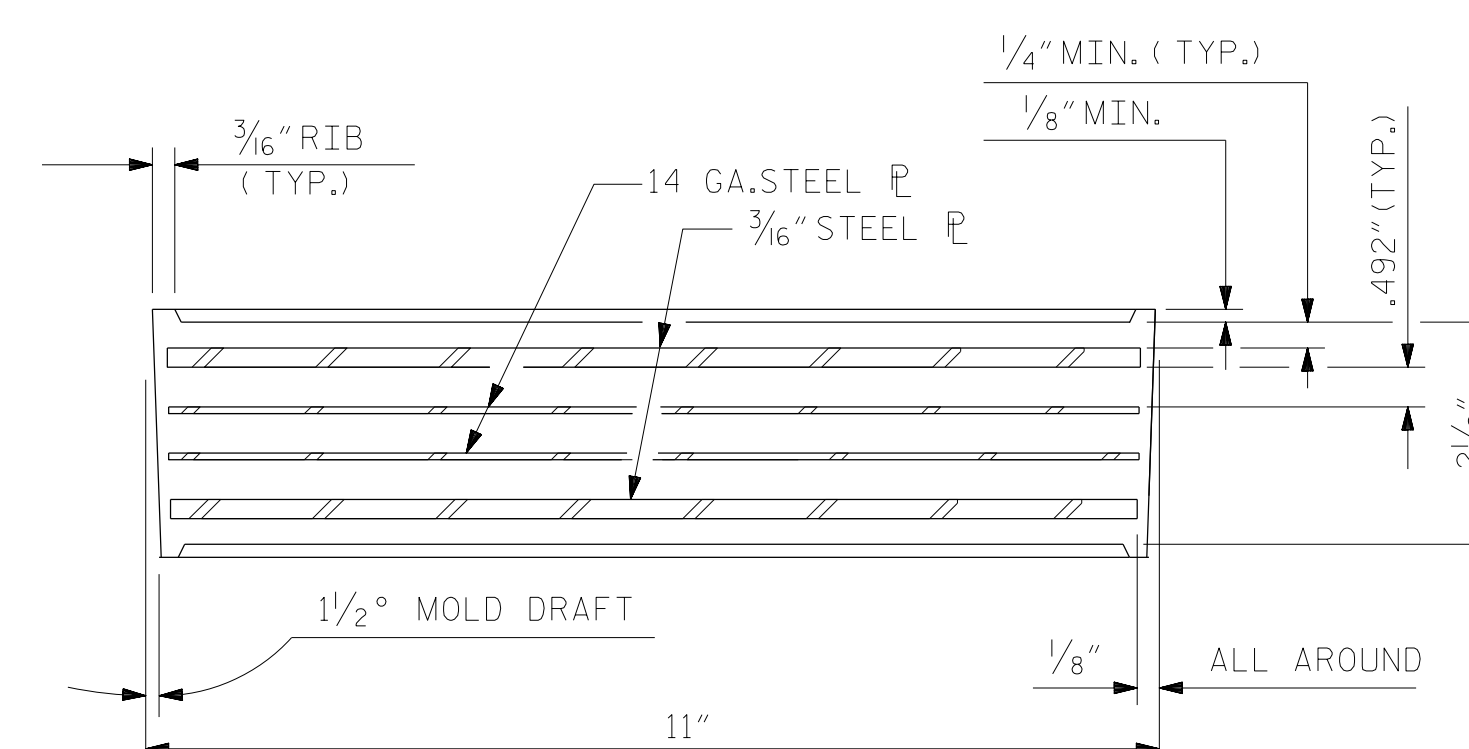
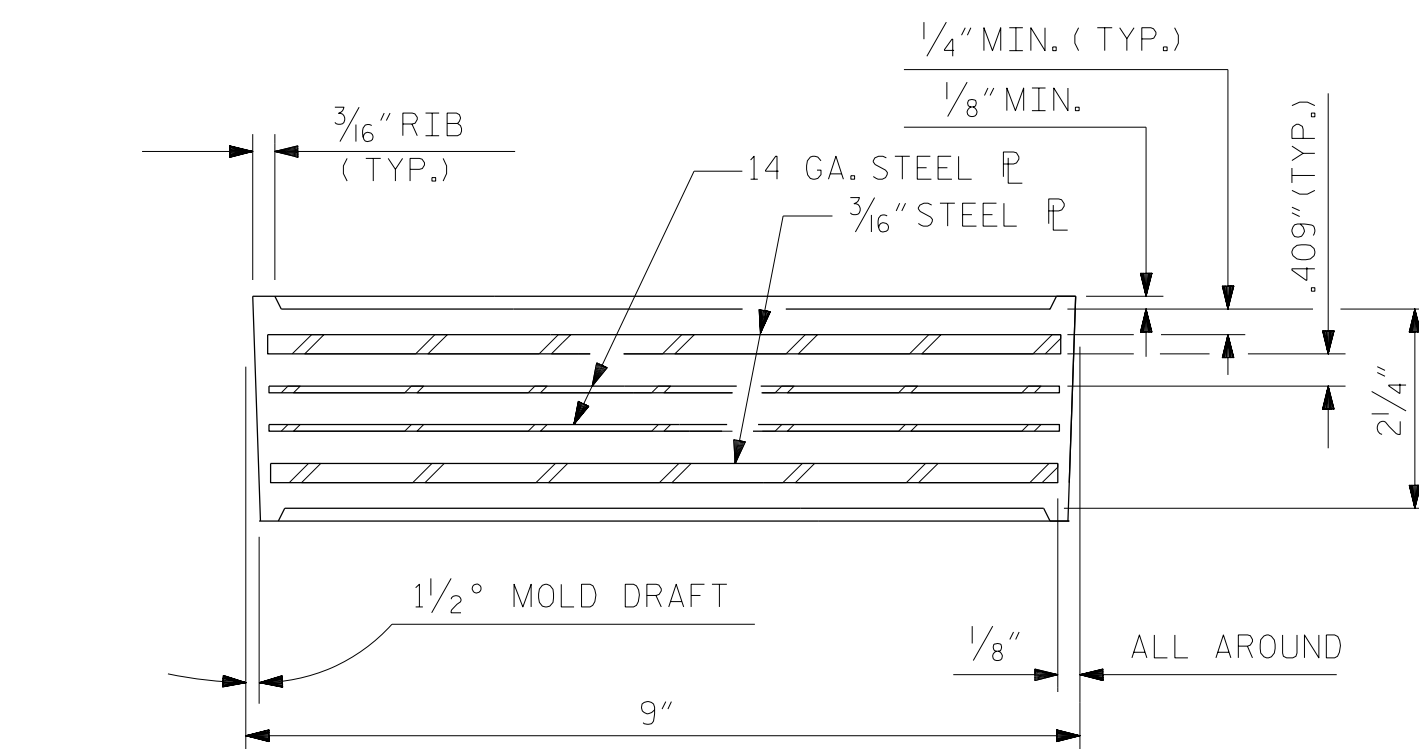
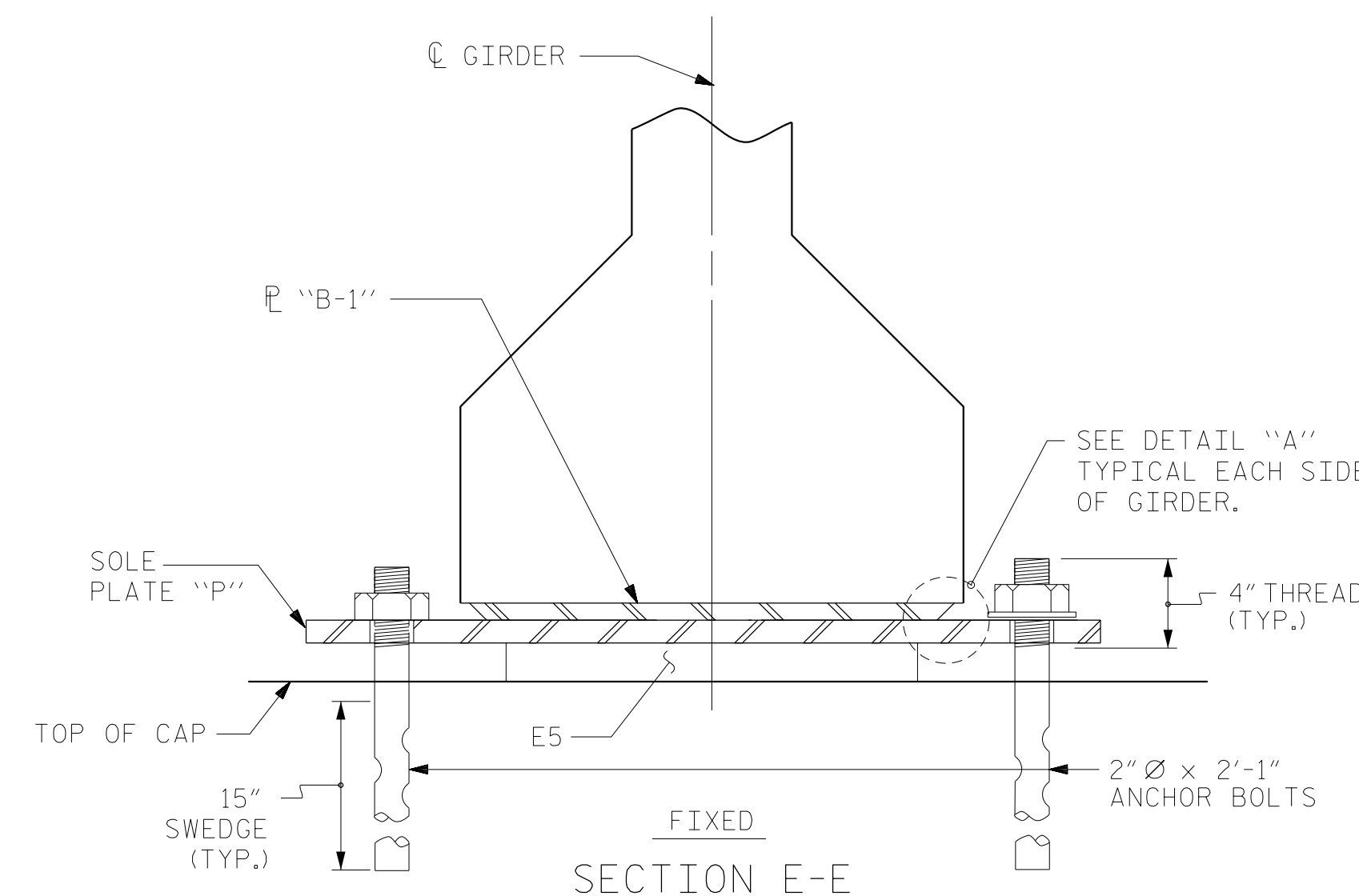
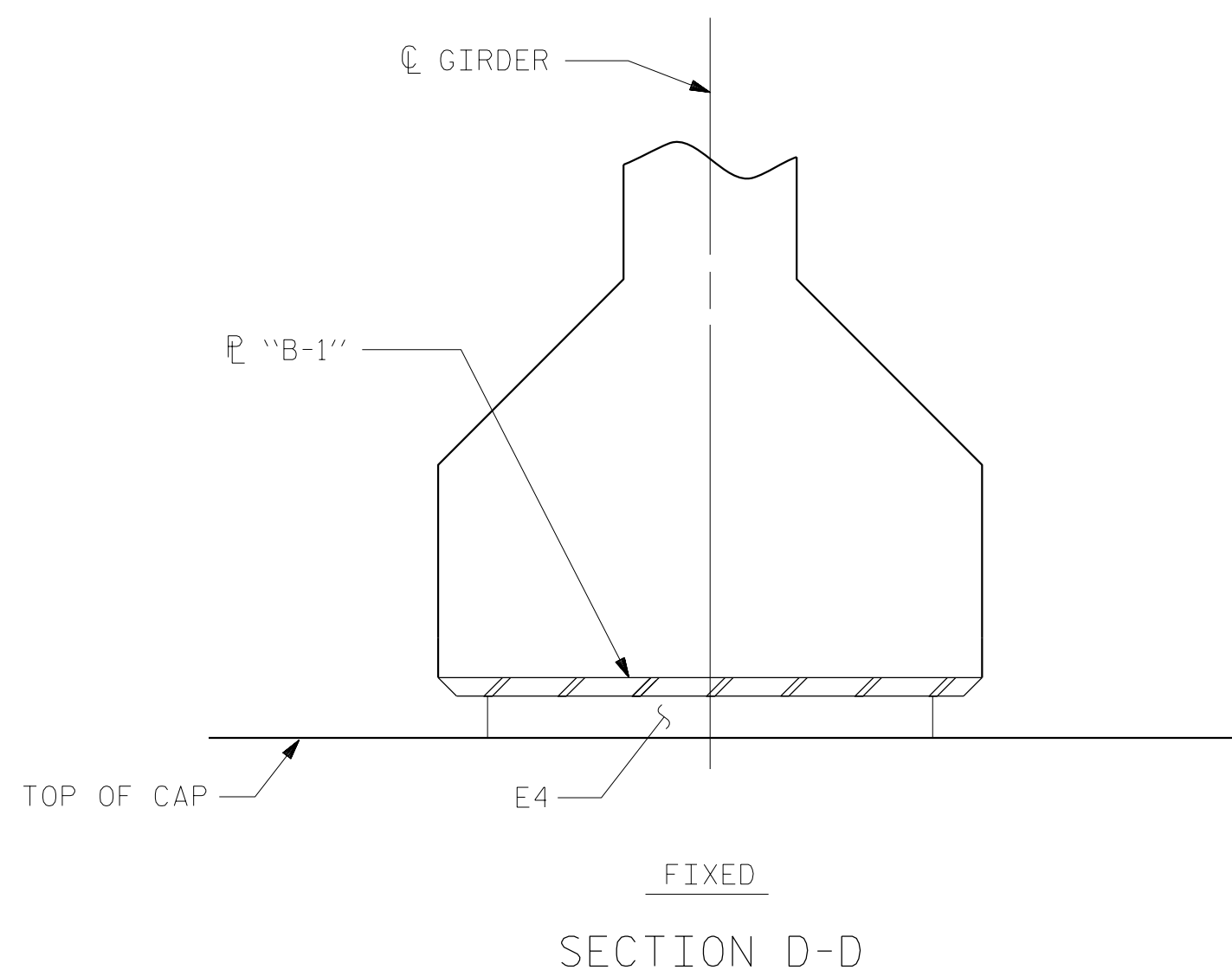
ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. NO SHOP DRAWINGS ARE REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

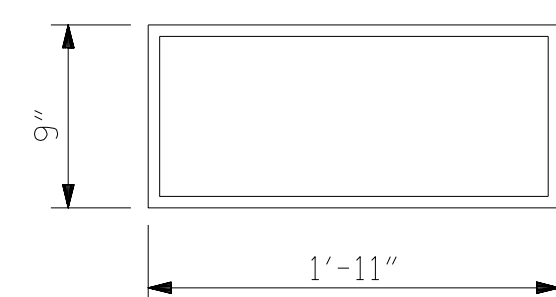
FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

ALL BEARING PLATES SHALL BE AASHTO M270 GRADE 36.



TYPICAL SECTION OF ELASTOMERIC BEARINGS

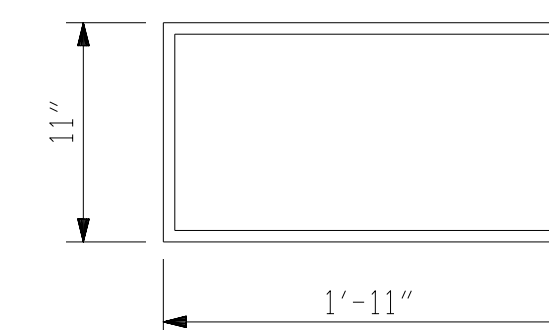
TYPICAL SECTION OF ELASTOMERIC BEARINGS



E4 (8 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

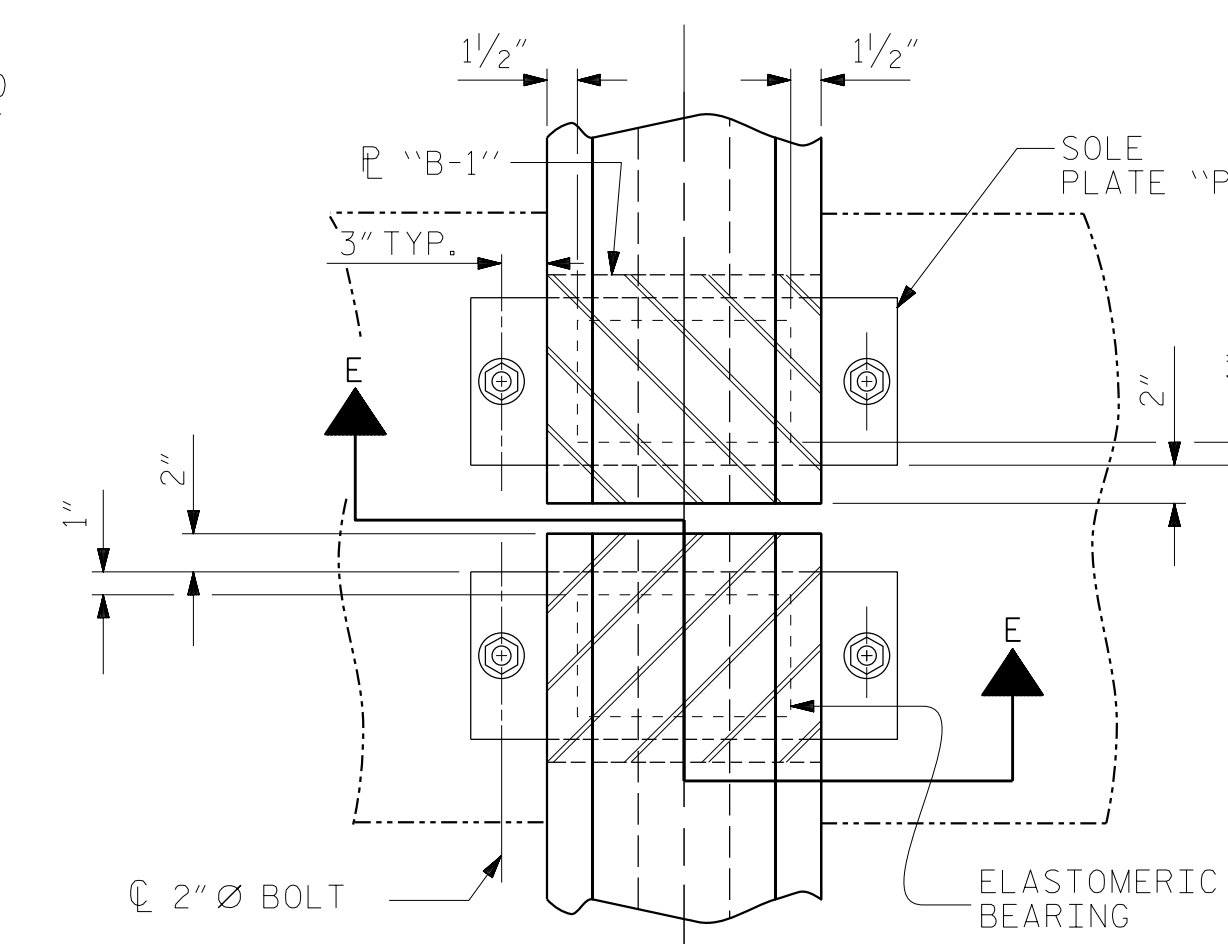
TYPE V



E5 (16 REQ'D)

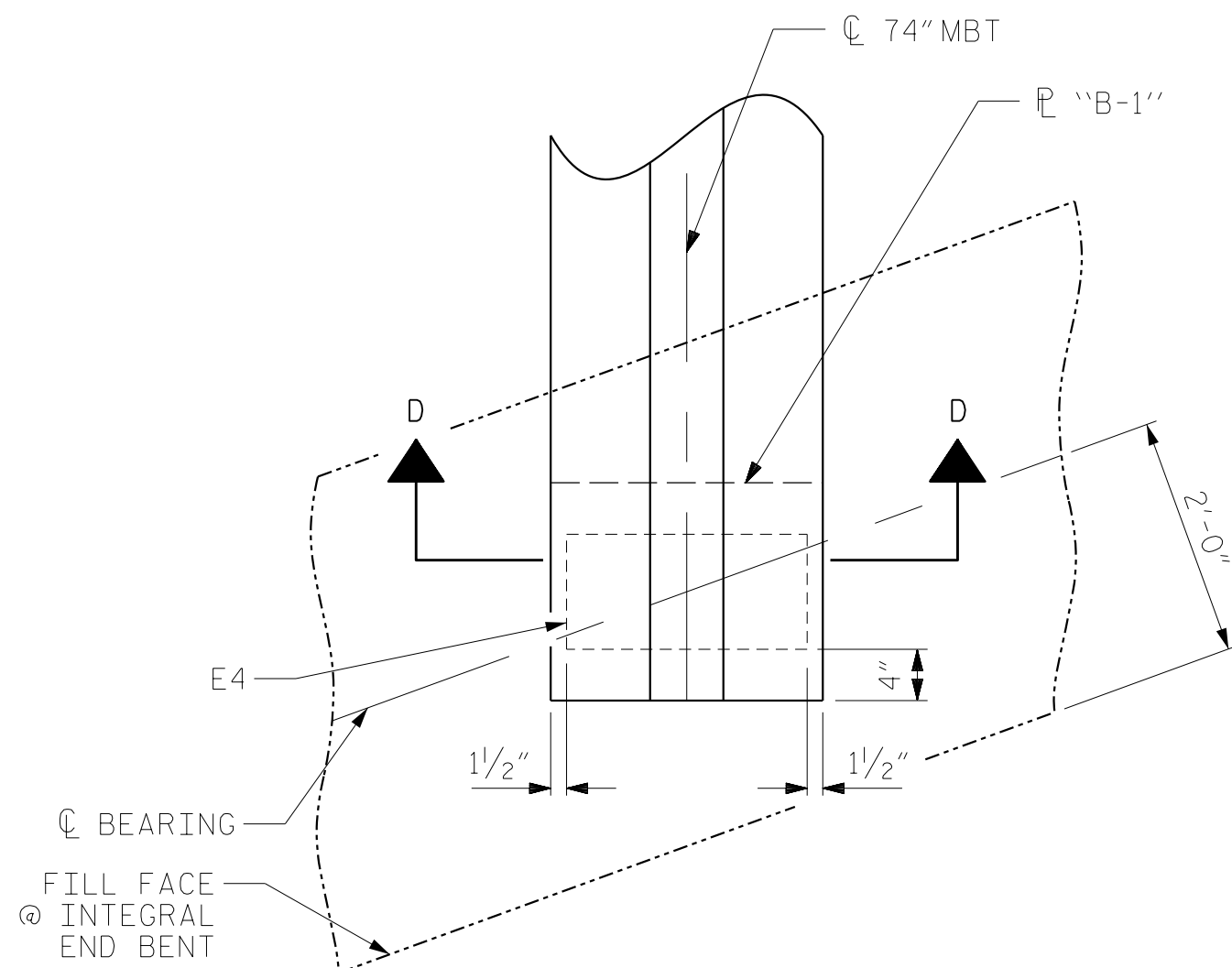
PLAN VIEW OF ELASTOMERIC BEARING

TYPE VI



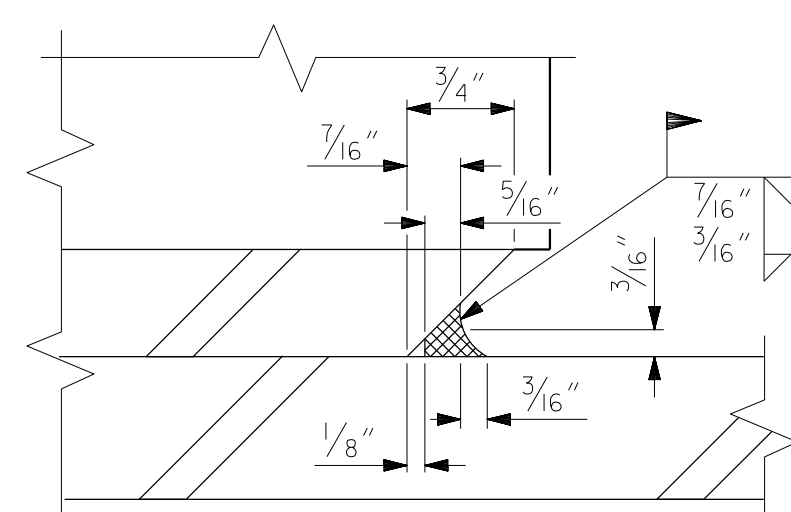
TYPICAL PLAN

(SHOWING CONTINUOUS BENT)

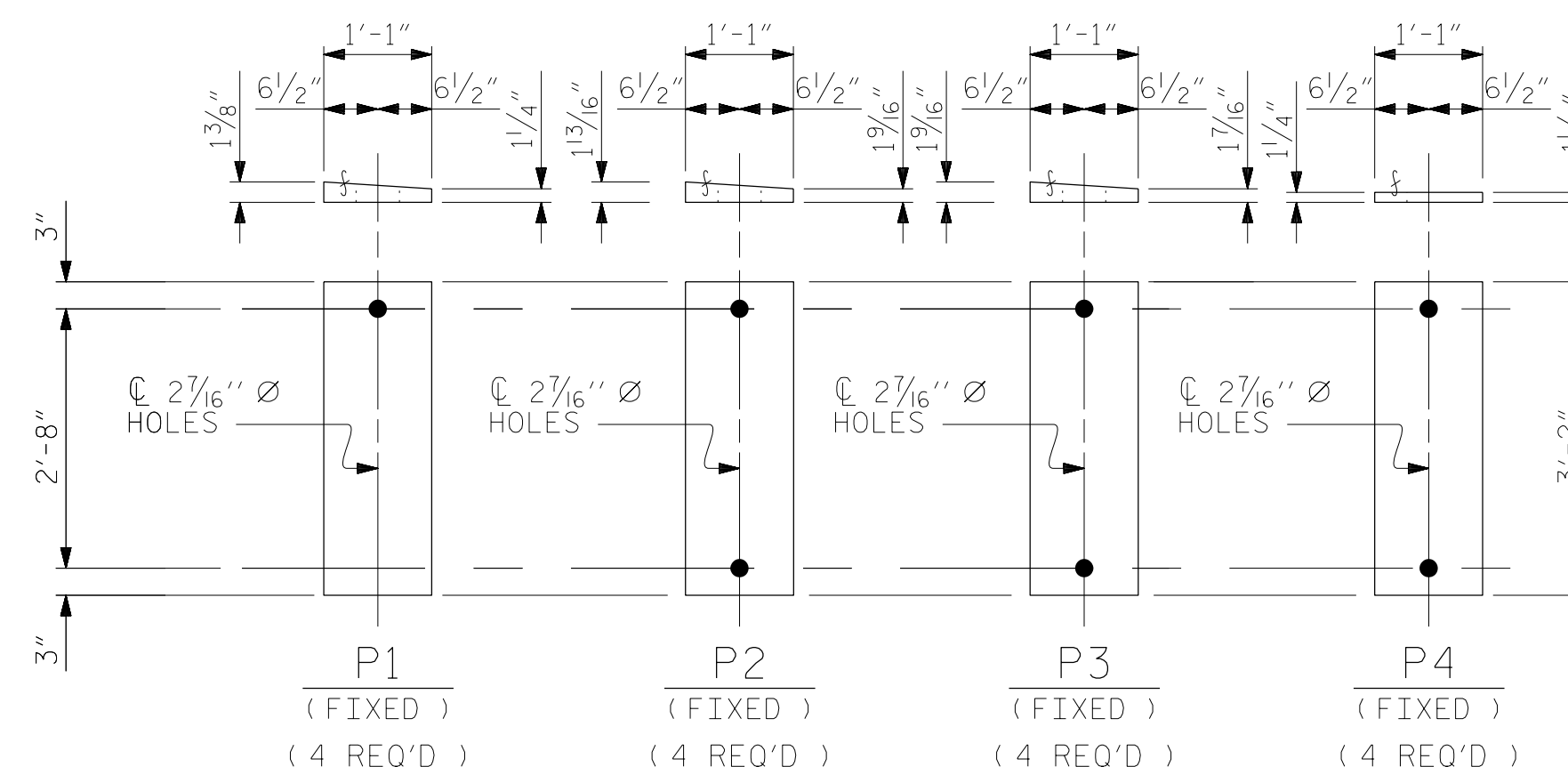


PLAN VIEW AT END BENTS

NOTE: BOTTOM FLANGE OF 74" MBT SHOWN, TOP FLANGE NOT SHOWN FOR CLARITY



DETAIL "A"



SOLE PLATE DETAILS ("P")

MAXIMUM ALLOWABLE SERVICE LOADS	
D.L.+L.L. (NO IMPACT)	
TYPE V	365 k
TYPE VI	420 k

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
ELASTOMERIC BEARING
DETAILS
RIGHT LANE

DRAWN BY : PDS DATE : 11/2016
CHECKED BY : TLC DATE : 01/2017
DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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8601 Six Forks Road, Suite 260
Raleigh, NC 27615
919-926-4100 FAX 919-846-9080
www.rsandh.com
North Carolina License No. 50737-0403-C&E

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-16
1			3			TOTAL SHEETS
2			4			36

DEAD LOAD DEFLECTION TABLE FOR GIRDERS

0.6" Ø LOW RELAXATION	SPAN A																				
	GIRDER 1 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.052	0.102	0.149	0.193	0.231	0.264	0.290	0.309	0.320	0.324	0.320	0.309	0.290	0.264	0.231	0.193	0.149	0.102	0.052	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.029	0.058	0.085	0.109	0.131	0.150	0.165	0.175	0.182	0.184	0.182	0.175	0.165	0.150	0.131	0.109	0.085	0.058	0.029	0.000
FINAL CAMBER ↑	0"	1/4"	1/2"	3/4"	1"	1 3/16"	1 3/8"	1 1/2"	1 5/8"	1 5/8"	1 11/16"	1 5/8"	1 5/8"	1 1/2"	1 3/8"	1 3/16"	1"	3/4"	1/2"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 2 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.052	0.102	0.149	0.193	0.231	0.264	0.290	0.309	0.320	0.324	0.320	0.309	0.290	0.264	0.231	0.193	0.149	0.102	0.052	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.032	0.064	0.094	0.121	0.146	0.166	0.183	0.195	0.202	0.204	0.202	0.195	0.183	0.166	0.146	0.121	0.094	0.064	0.032	0.000
FINAL CAMBER ↑	0"	1/4"	7/16"	11/16"	7/8"	1"	1 3/16"	1 5/16"	1 3/8"	1 7/16"	1 7/16"	1 7/16"	1 3/8"	1 5/16"	1 3/16"	1"	7/8"	11/16"	7/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 3 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.052	0.102	0.149	0.193	0.231	0.264	0.290	0.309	0.320	0.324	0.320	0.309	0.290	0.264	0.231	0.193	0.149	0.102	0.052	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.032	0.064	0.094	0.121	0.146	0.166	0.183	0.195	0.202	0.204	0.202	0.195	0.183	0.166	0.146	0.121	0.094	0.064	0.032	0.000
FINAL CAMBER ↑	0"	1/4"	7/16"	11/16"	7/8"	1"	1 3/16"	1 5/16"	1 3/8"	1 7/16"	1 7/16"	1 7/16"	1 3/8"	1 5/16"	1 3/16"	1"	7/8"	11/16"	7/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 4 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.052	0.102	0.149	0.193	0.231	0.264	0.290	0.309	0.320	0.324	0.320	0.309	0.290	0.264	0.231	0.193	0.149	0.102	0.052	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.028	0.056	0.082	0.105	0.127	0.144	0.159	0.169	0.175	0.178	0.175	0.169	0.159	0.144	0.127	0.105	0.082	0.056	0.028	0.000
FINAL CAMBER ↑	0"	5/16"	9/16"	13/16"	1 1/16"	1 1/4"	1 7/16"	1 9/16"	1 11/16"	1 3/4"	1 3/4"	1 3/4"	1 11/16"	1 9/16"	1 7/16"	1 1/4"	1 1/16"	13/16"	9/16"	5/16"	0"

0.6" Ø LOW RELAXATION	SPAN B																				
	GIRDER 1 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.063	0.124	0.181	0.234	0.281	0.321	0.353	0.376	0.390	0.394	0.390	0.376	0.353	0.321	0.281	0.234	0.181	0.124	0.063	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.041	0.080	0.118	0.152	0.182	0.208	0.229	0.244	0.253	0.256	0.253	0.244	0.229	0.208	0.182	0.152	0.118	0.080	0.041	0.000
FINAL CAMBER ↑	0"	1/4"	1/2"	3/4"	1"	1 3/16"	1 3/8"	1 1/2"	1 9/16"	1 5/8"	1 5/8"	1 5/8"	1 9/16"	1 1/2"	1 3/8"	1 3/16"	1"	3/4"	1/2"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 2 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.063	0.124	0.181	0.234	0.281	0.321	0.353	0.376	0.390	0.394	0.390	0.376	0.353	0.321	0.281	0.234	0.181	0.124	0.063	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.045	0.089	0.130	0.168	0.202	0.230	0.253	0.270	0.280	0.283	0.280	0.270	0.253	0.230	0.202	0.168	0.130	0.089	0.045	0.000
FINAL CAMBER ↑	0"	3/16"	7/16"	5/8"	13/16"	15/16"	1 1/16"	1 3/16"	1 1/4"	1 5/16"	1 5/16"	1 5/16"	1 1/4"	1 3/16"	1 1/16"	15/16"	13/16"	5/8"	7/16"	3/16"	0"
0.6" Ø LOW RELAXATION	GIRDER 3 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.063	0.124	0.181	0.234	0.281	0.321	0.353	0.376	0.390	0.394	0.390	0.376	0.353	0.321	0.281	0.234	0.181	0.124	0.063	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.045	0.089	0.130	0.168	0.202	0.230	0.253	0.270	0.280	0.283	0.280	0.270	0.253	0.230	0.202	0.168	0.130	0.089	0.045	0.000
FINAL CAMBER ↑	0"	3/16"	7/16"	5/8"	13/16"	15/16"	1 1/16"	1 3/16"	1 1/4"	1 5/16"	1 5/16"	1 5/16"	1 1/4"	1 3/16"	1 1/16"	15/16"	13/16"	5/8"	7/16"	3/16"	0"
0.6" Ø LOW RELAXATION	GIRDER 4 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.063	0.124	0.181	0.234	0.281	0.321	0.353	0.376	0.390	0.394	0.390	0.376	0.353	0.321	0.281	0.234	0.181	0.124	0.063	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.039	0.077	0.112	0.145	0.174	0.198	0.218	0.232	0.241	0.244	0.241	0.232	0.218	0.198	0.174	0.145	0.112	0.077	0.039	0.000
FINAL CAMBER ↑	0"	5/16"	9/16"	13/16"	1 1/16"	1 5/16"	1 1/2"	1 5/8"	1 3/4"	1 13/16"	1 13/16"	1 13/16"	1 3/4"	1 5/8"	1 1/2"	1 5/16"	1 1/16"	13/16"	9/16"	5/16"	0"

* INCLUDES FUTURE WEARING SURFACE IN SUPERIMPOSED DEAD LOAD.
ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT " FINAL CAMBER ", WHICH IS GIVEN IN INCHES (FRACTION FORM).

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

SHEET 1 OF 2

DRAWN BY : _____ PDS DATE : 11/2016
CHECKED BY : _____ TLC DATE : 01/2017
DESIGN ENGINEER OF RECORD: _____ MAL DATE : 11/2016

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						SHEET NO.
SUPERSTRUCTURE						S2-17
DEAD LOAD DEFLECTION						TOTAL SHEETS
SPANS A & B						36
RIGHT LANE						
REVISIONS						
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			
2			4			

DEAD LOAD DEFLECTION TABLE FOR GIRDERS

0.6" Ø LOW RELAXATION	SPAN C																				
	GIRDER 1 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.062	0.123	0.180	0.232	0.279	0.318	0.350	0.372	0.386	0.391	0.386	0.372	0.350	0.318	0.279	0.232	0.180	0.123	0.062	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.039	0.078	0.114	0.147	0.177	0.202	0.222	0.236	0.245	0.248	0.245	0.236	0.222	0.202	0.177	0.147	0.114	0.078	0.039	0.000
FINAL CAMBER ↑	0"	1/4"	9/16"	13/16"	1"	1 1/4"	1 3/8"	1 9/16"	1 5/8"	1 11/16"	1 11/16"	1 11/16"	1 5/8"	1 9/16"	1 3/8"	1 1/4"	1"	13/16"	9/16"	1/4"	0"
0.6" Ø LOW RELAXATION	GIRDER 2 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.062	0.123	0.180	0.232	0.279	0.318	0.350	0.372	0.386	0.391	0.386	0.372	0.350	0.318	0.279	0.232	0.180	0.123	0.062	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.044	0.086	0.126	0.163	0.196	0.223	0.246	0.262	0.272	0.275	0.272	0.262	0.246	0.223	0.196	0.163	0.126	0.086	0.044	0.000
FINAL CAMBER ↑	0"	3/16"	7/16"	5/8"	13/16"	1"	1 1/8"	1 1/4"	1 5/16"	1 3/8"	1 3/8"	1 3/8"	1 5/16"	1 1/4"	1 1/8"	1"	13/16"	5/8"	7/16"	3/16"	0"
0.6" Ø LOW RELAXATION	GIRDER 3 (INTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.062	0.123	0.180	0.232	0.279	0.318	0.350	0.372	0.386	0.391	0.386	0.372	0.350	0.318	0.279	0.232	0.180	0.123	0.062	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.044	0.086	0.126	0.163	0.196	0.223	0.246	0.262	0.272	0.275	0.272	0.262	0.246	0.223	0.196	0.163	0.126	0.086	0.044	0.000
FINAL CAMBER ↑	0"	3/16"	7/16"	5/8"	13/16"	1"	1 1/8"	1 1/4"	1 5/16"	1 3/8"	1 3/8"	1 3/8"	1 5/16"	1 1/4"	1 1/8"	1"	13/16"	5/8"	7/16"	3/16"	0"
0.6" Ø LOW RELAXATION	GIRDER 4 (EXTERIOR)																				
	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.062	0.123	0.180	0.232	0.279	0.318	0.350	0.372	0.386	0.391	0.386	0.372	0.350	0.318	0.279	0.232	0.180	0.123	0.062	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0.000	0.038	0.074	0.109	0.140	0.168	0.192	0.211	0.225	0.233	0.236	0.233	0.225	0.211	0.192	0.168	0.140	0.109	0.074	0.038	0.000
FINAL CAMBER ↑	0"	5/16"	9/16"	7/8"	1 1/8"	1 5/16"	1 1/2"	1 11/16"	1 3/4"	1 13/16"	1 7/8"	1 13/16"	1 3/4"	1 11/16"	1 1/2"	1 5/16"	1 1/8"	7/8"	9/16"	5/16"	0"

* INCLUDES FUTURE WEARING SURFACE IN SUPERIMPOSED DEAD LOAD.
ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT " FINAL CAMBER ", WHICH IS GIVEN IN INCHES (FRACTION FORM).

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

SHEET 2 OF 2

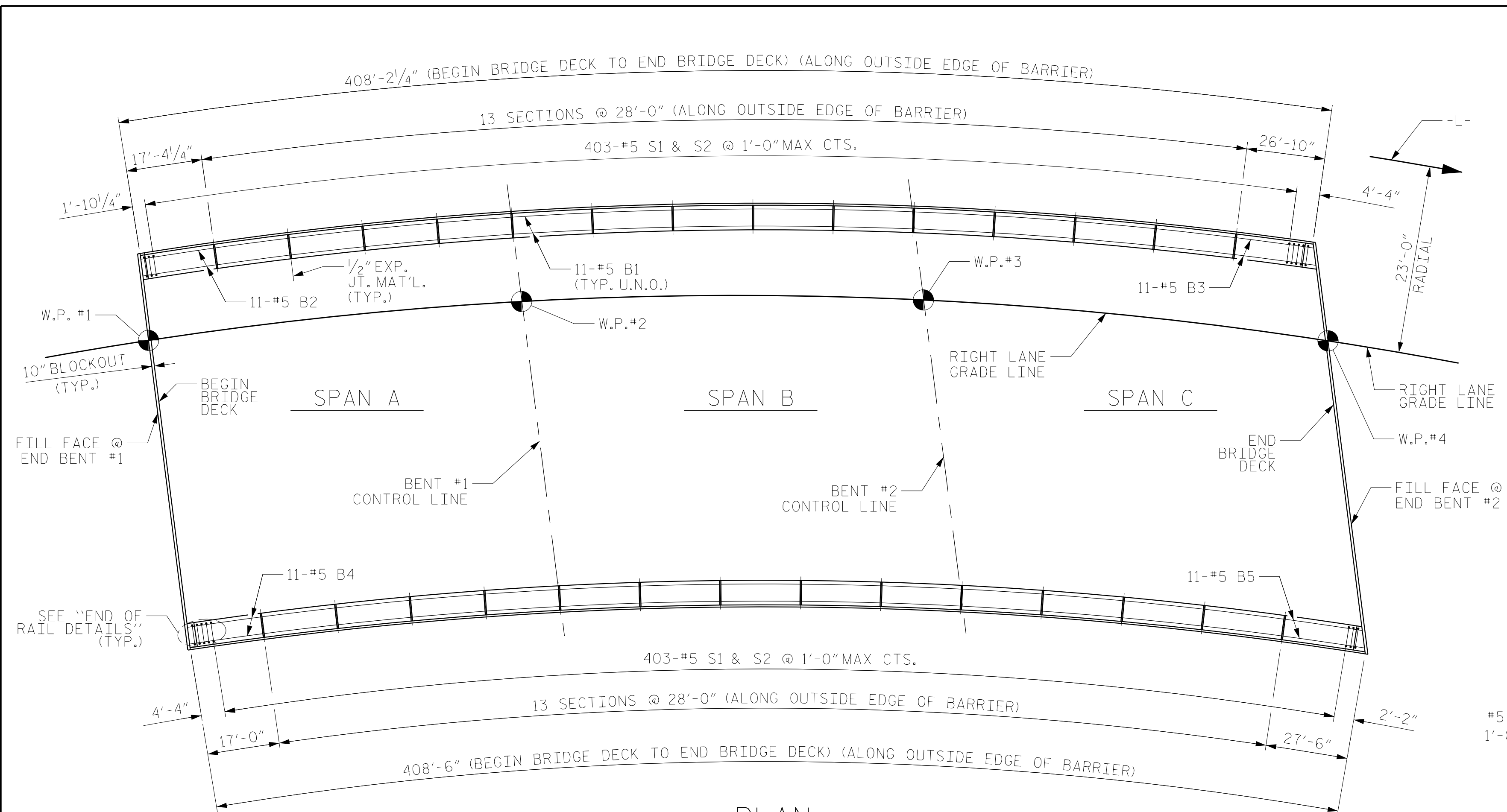


STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
DEAD LOAD DEFLECTION
SPAN C
RIGHT LANE

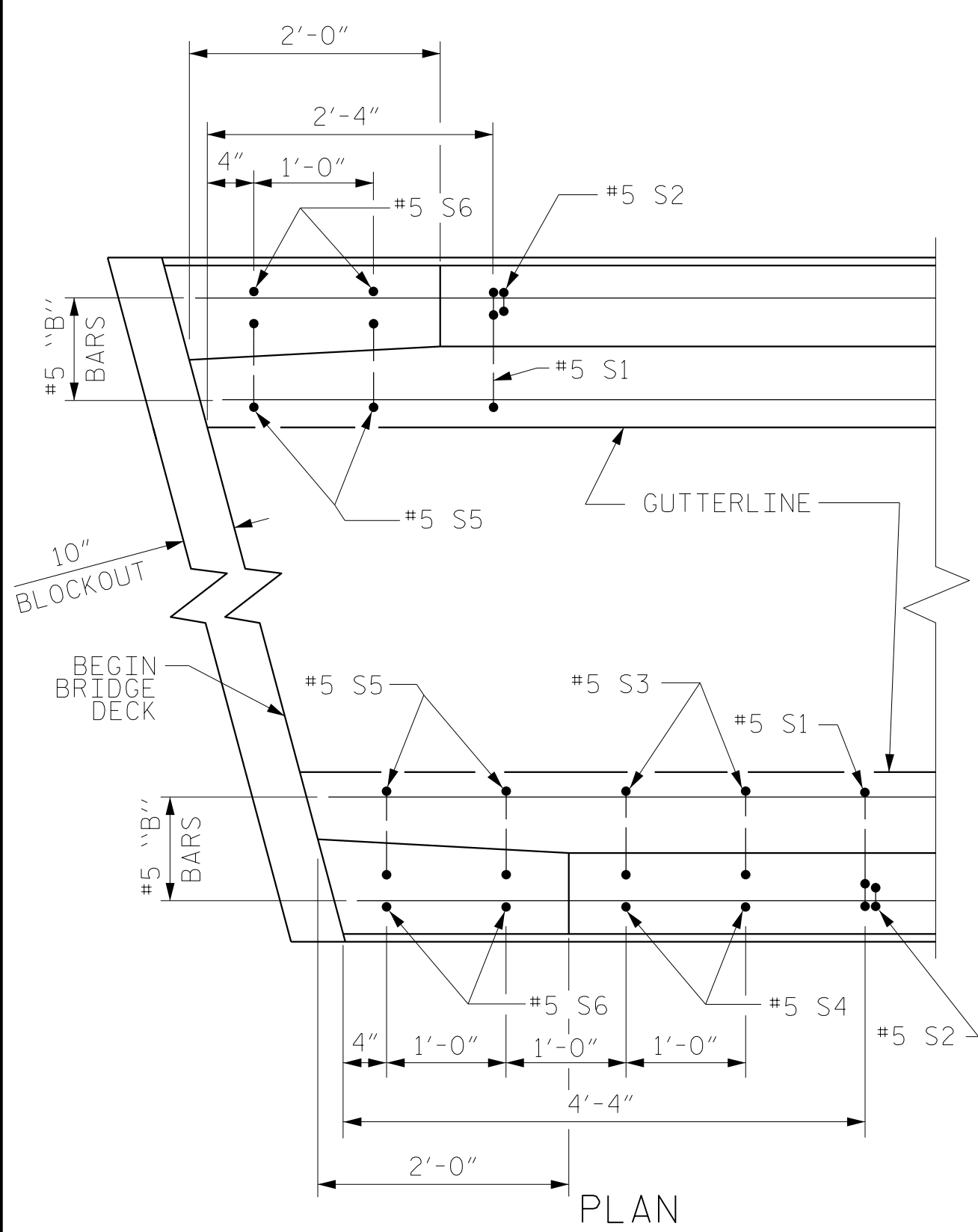
DRAWN BY : _____ PDS _____ DATE : 11/2016
CHECKED BY : _____ TLC _____ DATE : 01/2017
DESIGN ENGINEER OF RECORD: _____ MAL _____ DATE : 11/2016

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

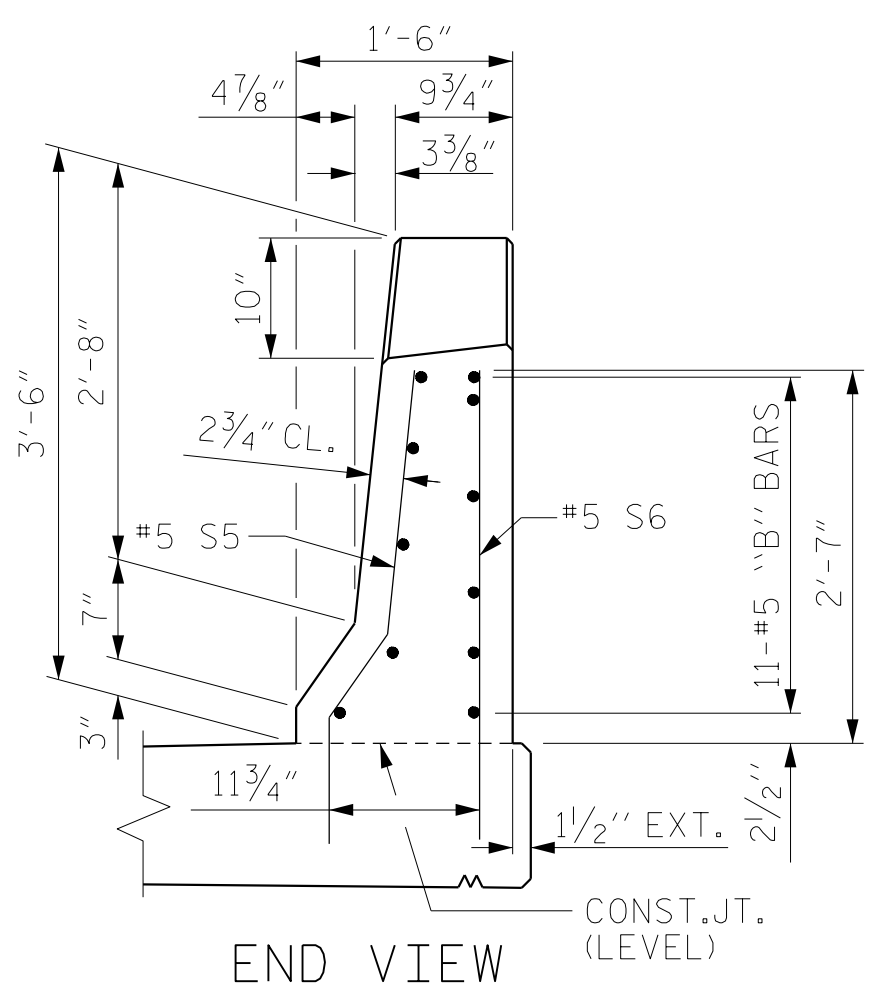
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-18
1			3			TOTAL SHEETS
2			4			36



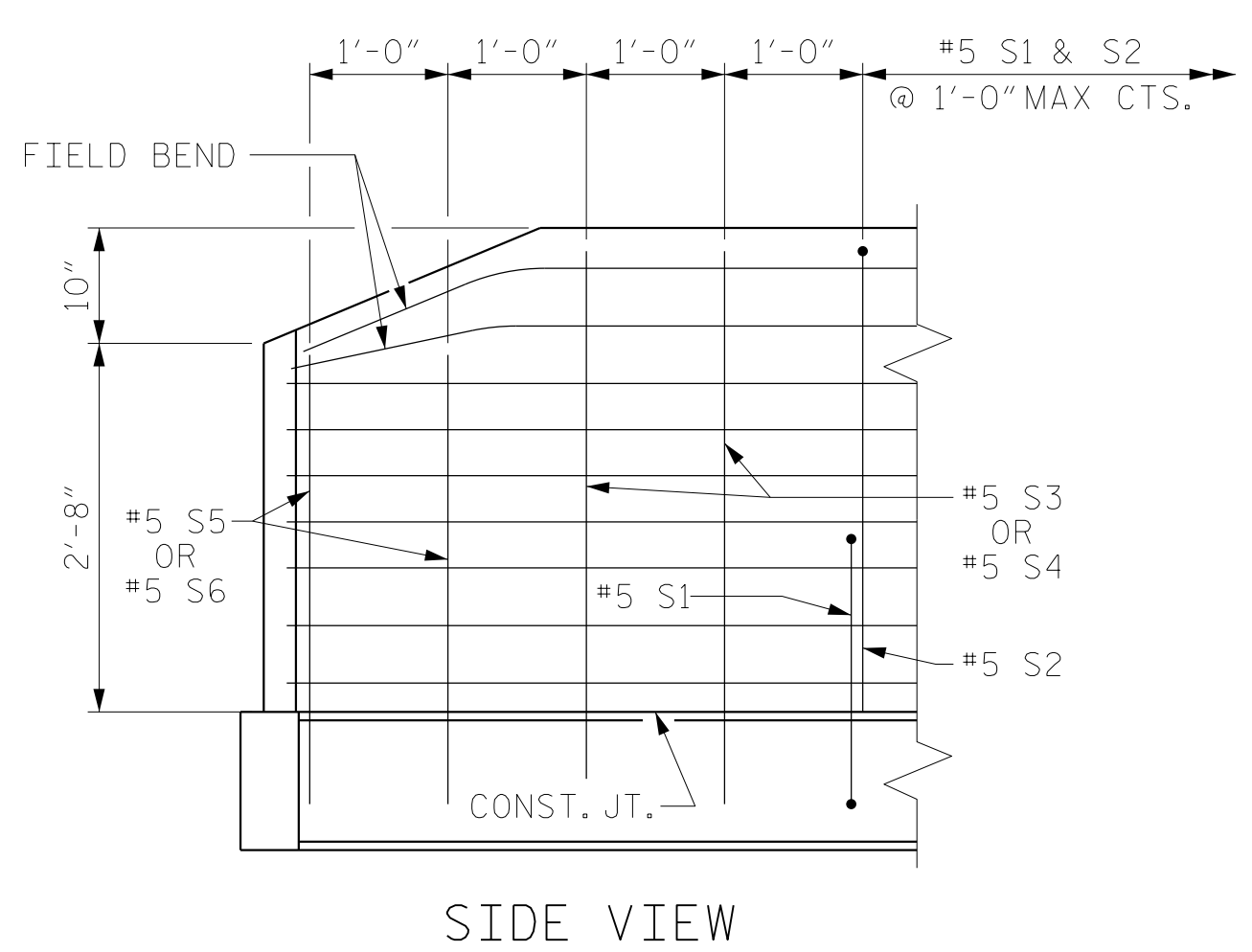
PLAN



PLAN



END VIEW



SIDE VIEW

END OF RAIL DETAILS FOR ADHESIVE ANCHORING AT SAWED JOINTS

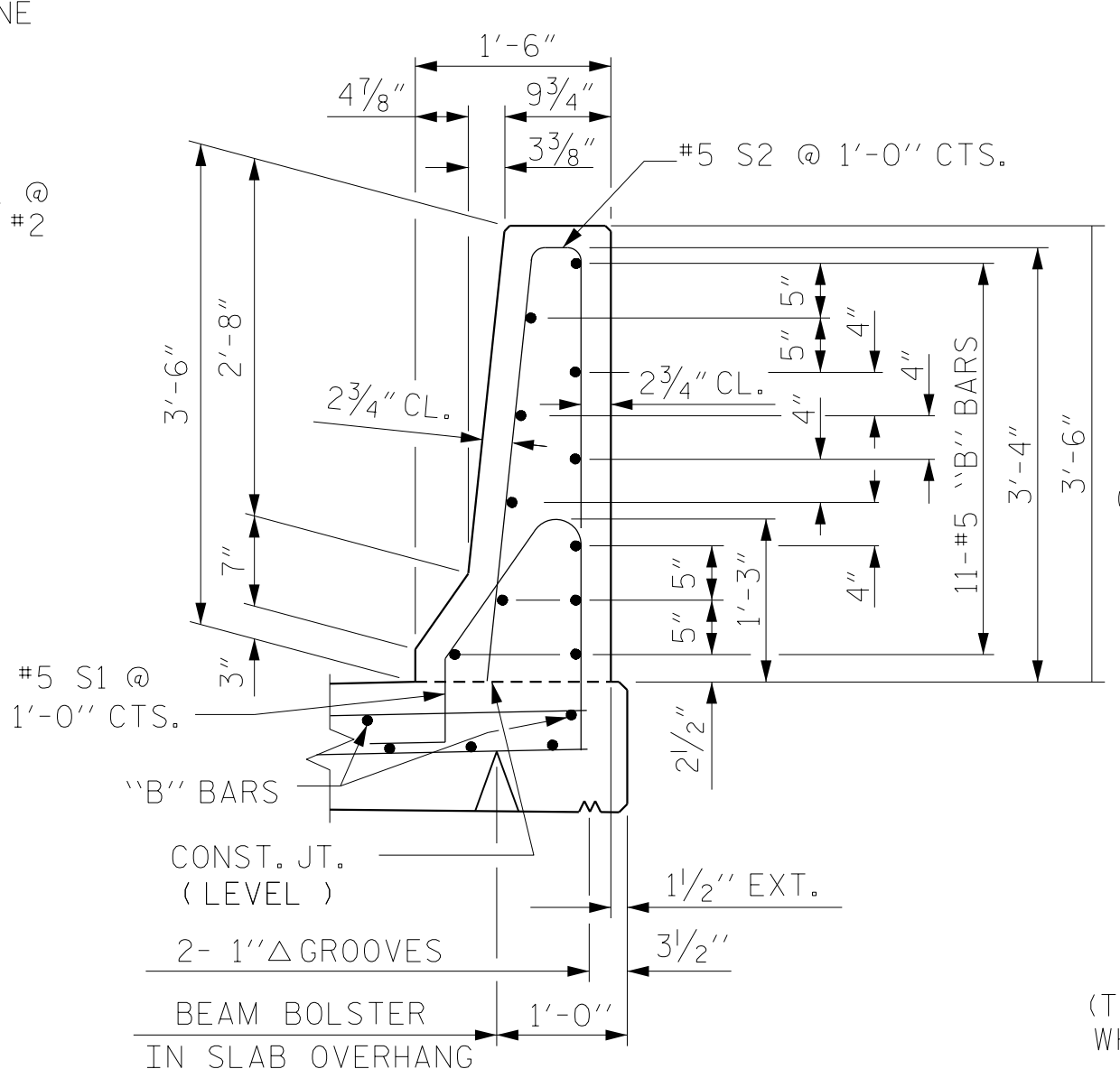
NOTES

CONCRETE BARRIER RAIL IN A CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL DECK SLAB CONCRETE IN THAT UNIT HAS BEEN CAST AND HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

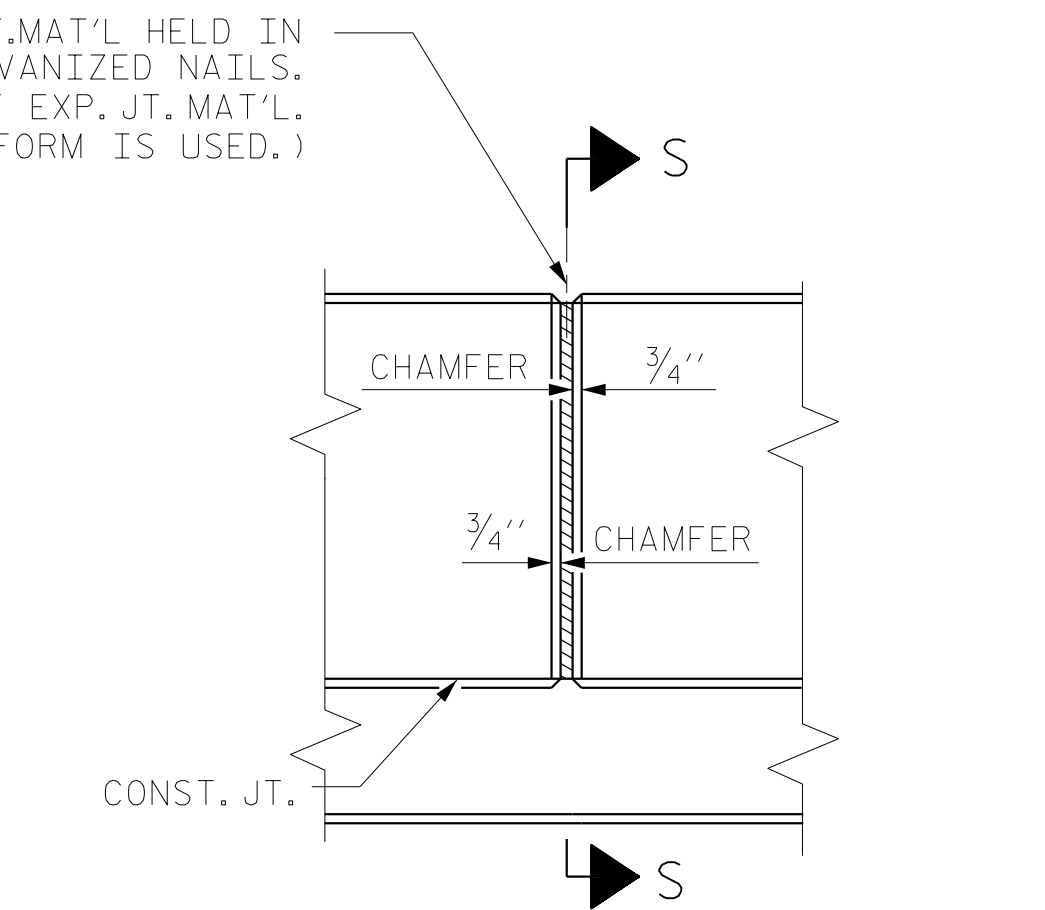
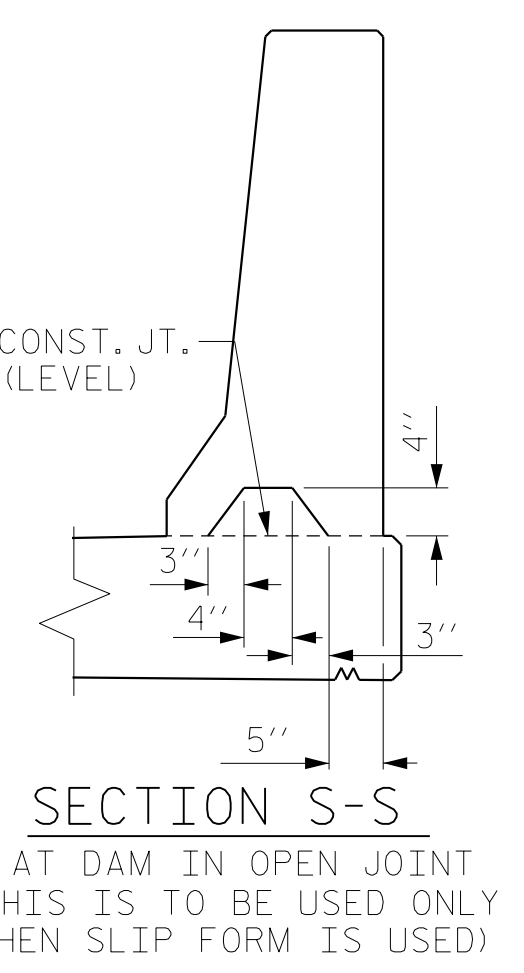
ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

THE #5 S3, S4, S5 AND S6 BARS SHALL BE INSTALLED, USING AN ADHESIVE ANCHORING SYSTEM, AFTER SAWING THE JOINT. THE YIELD LOAD FOR THE #5 S3, S4, S5 AND S6 BARS IS 18.6 KIPS. FIELD TESTING FOR THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

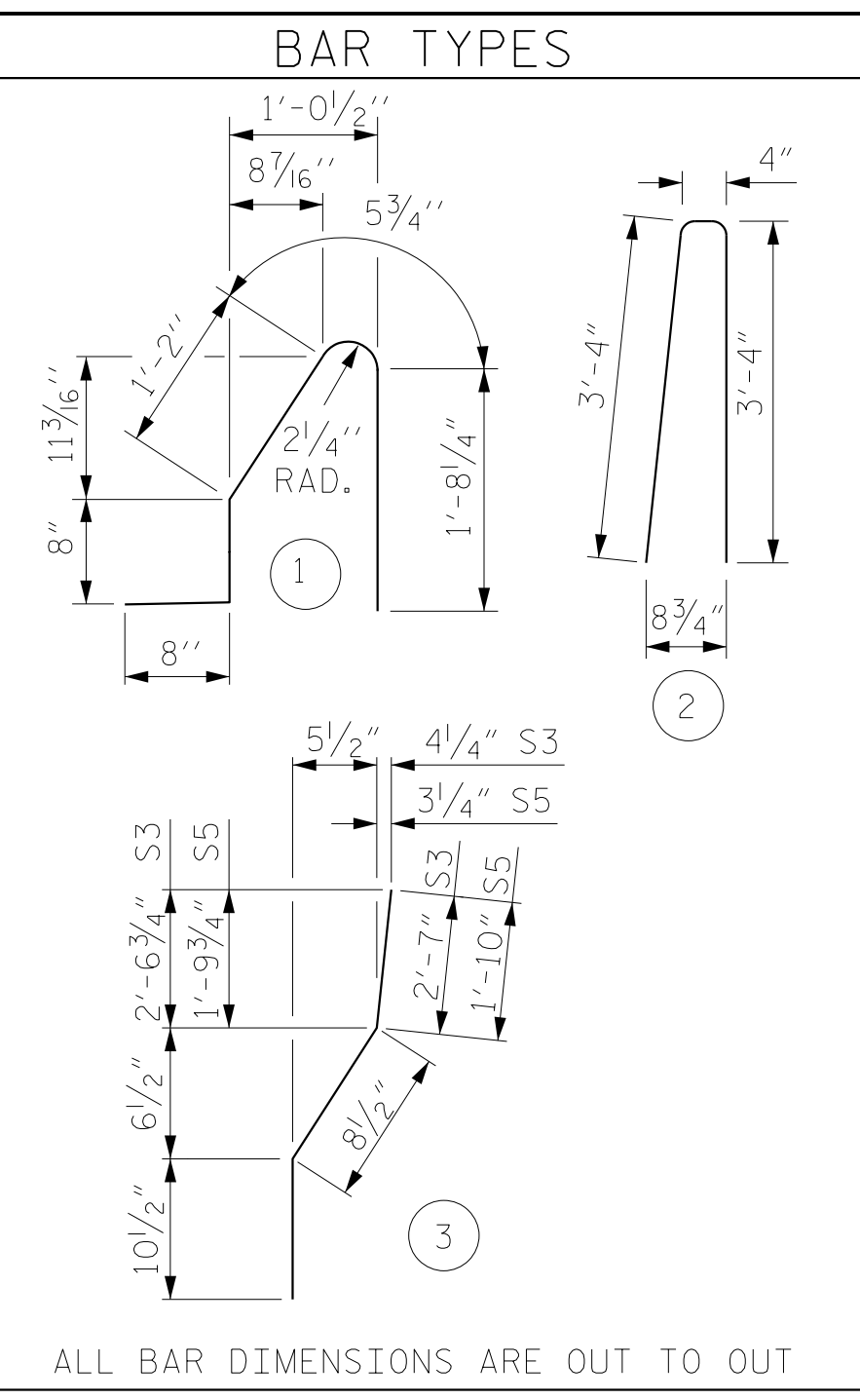
GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.



SECTION THRU RAIL



BARRIER RAIL DETAILS



ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL

FOR CONCRETE BARRIER RAIL ONLY

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
*B1	286	#5	STR	27'-8"	8253
*B2	11	#5	STR	17'-0"	195
*B3	11	#5	STR	26'-6"	304
*B4	11	#5	STR	16'-8"	191
*B5	11	#5	STR	27'-2"	312
*S1	806	#5	1	4'-8"	3923
*S2	806	#5	2	7'-0"	5885
*S3	4	#5	3	4'-2"	17
*S4	4	#5	STR	4'-0"	17
*S5	8	#5	3	3'-5"	29
*S6	8	#5	STR	3'-3"	27

* EPOXY COATED REINFORCING STEEL 19,153 LBS.

CLASS AA CONCRETE 111.1 CU. YDS.

CONCRETE BARRIER RAIL 816.7 LIN. FT.

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 384+50.00 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 CONCRETE BARRIER RAIL
 RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-19
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : PDS DATE : 11/2016
 CHECKED BY : TLC DATE : 01/2017
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

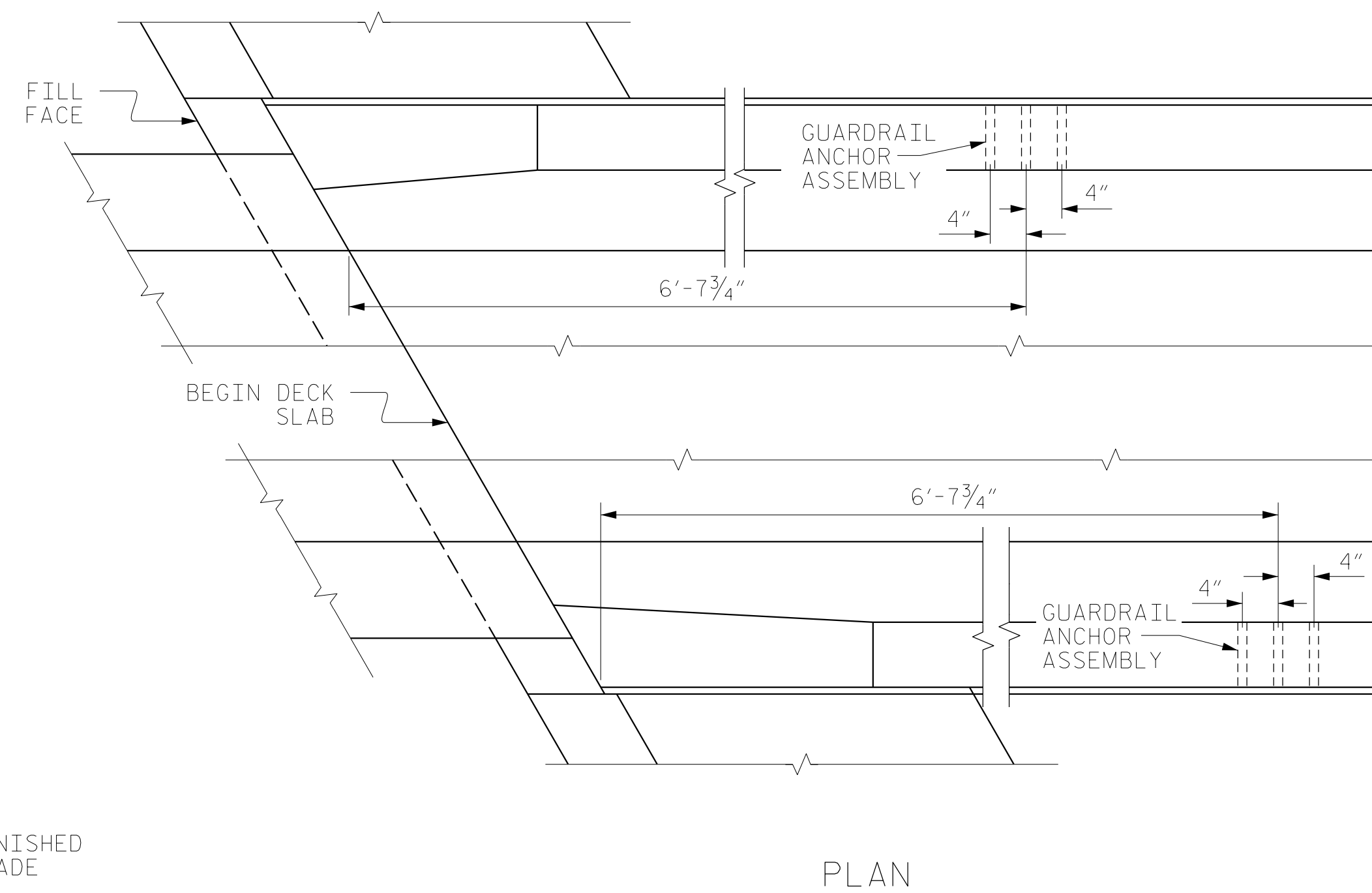
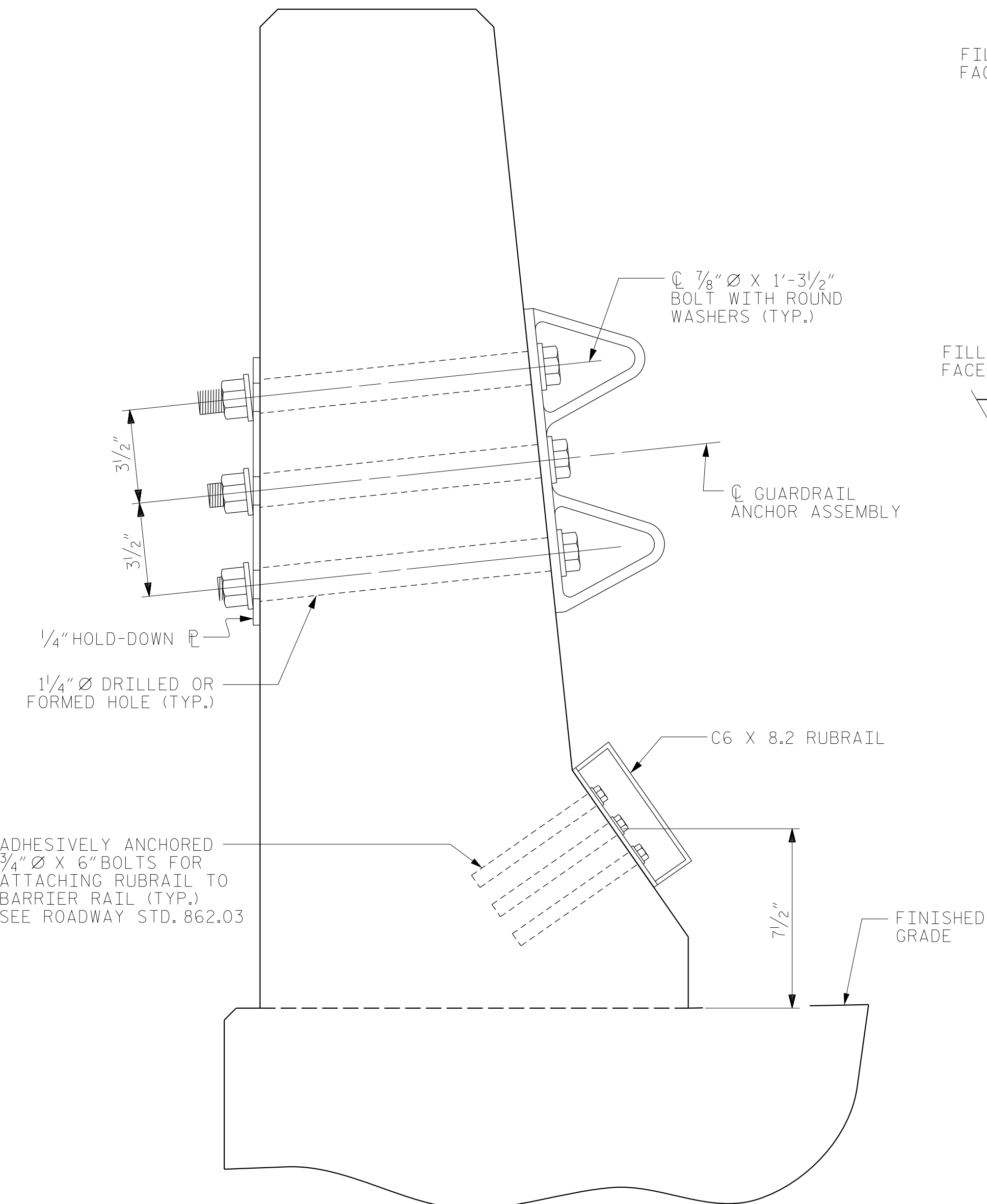
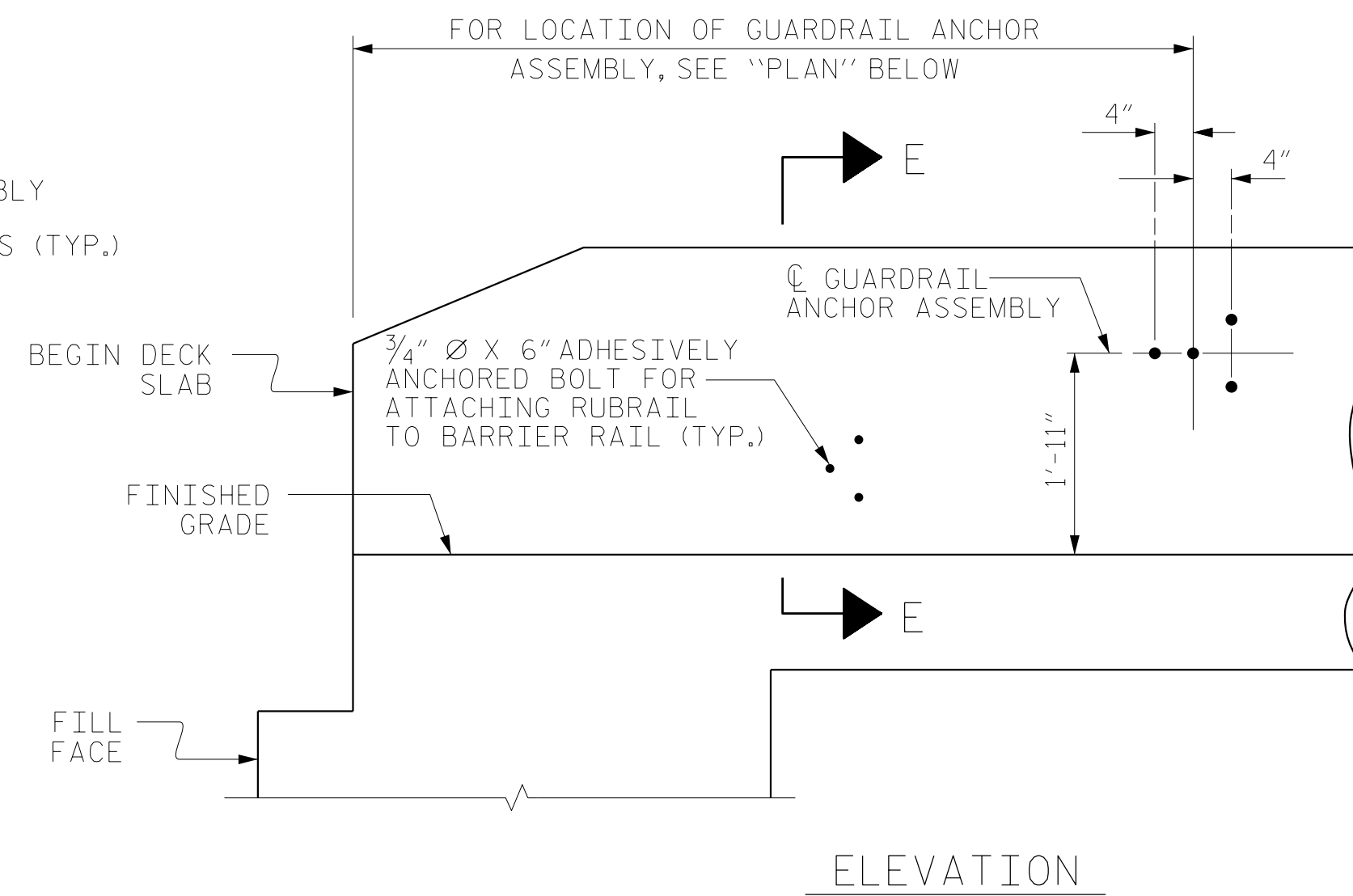
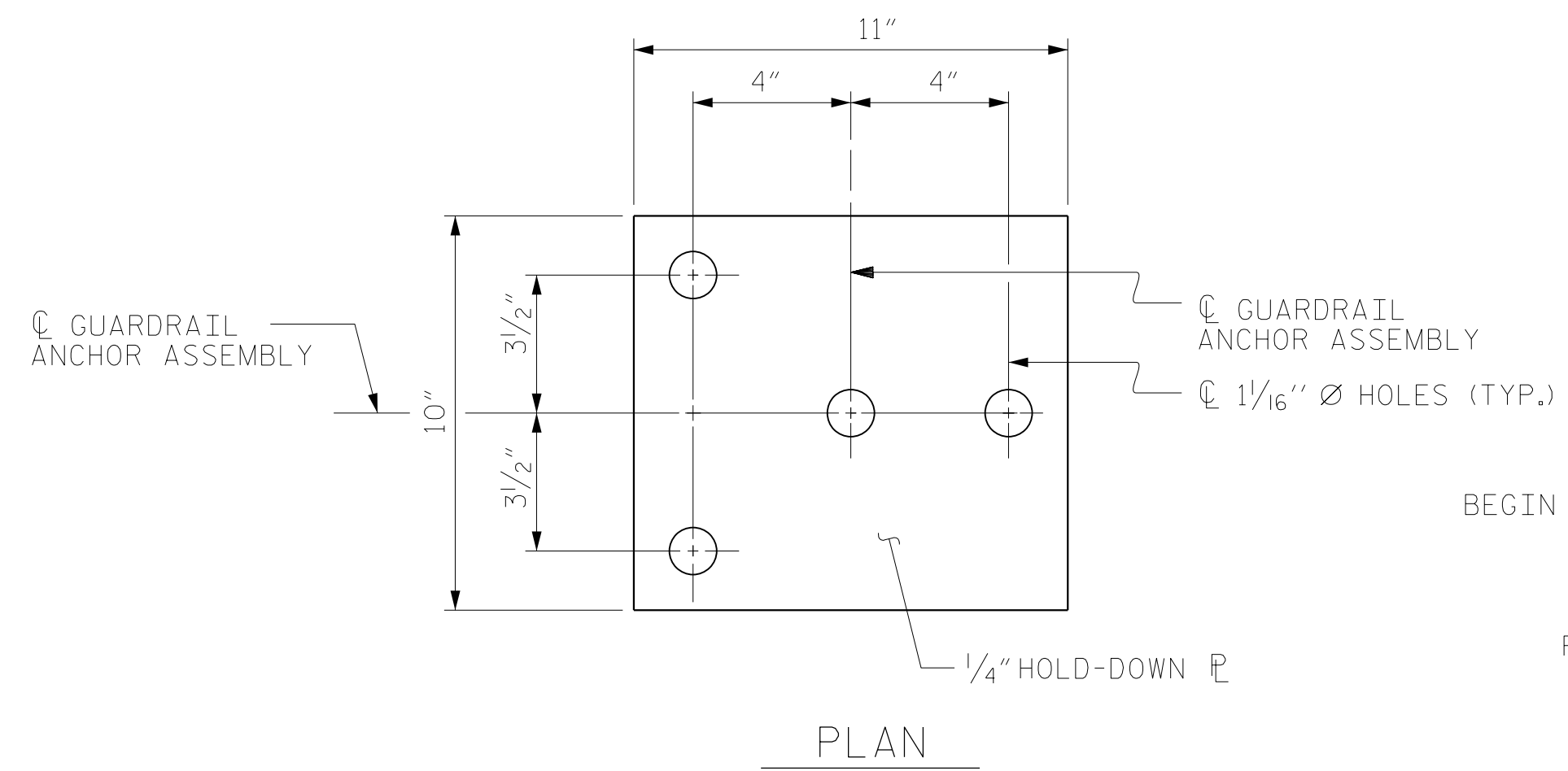
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

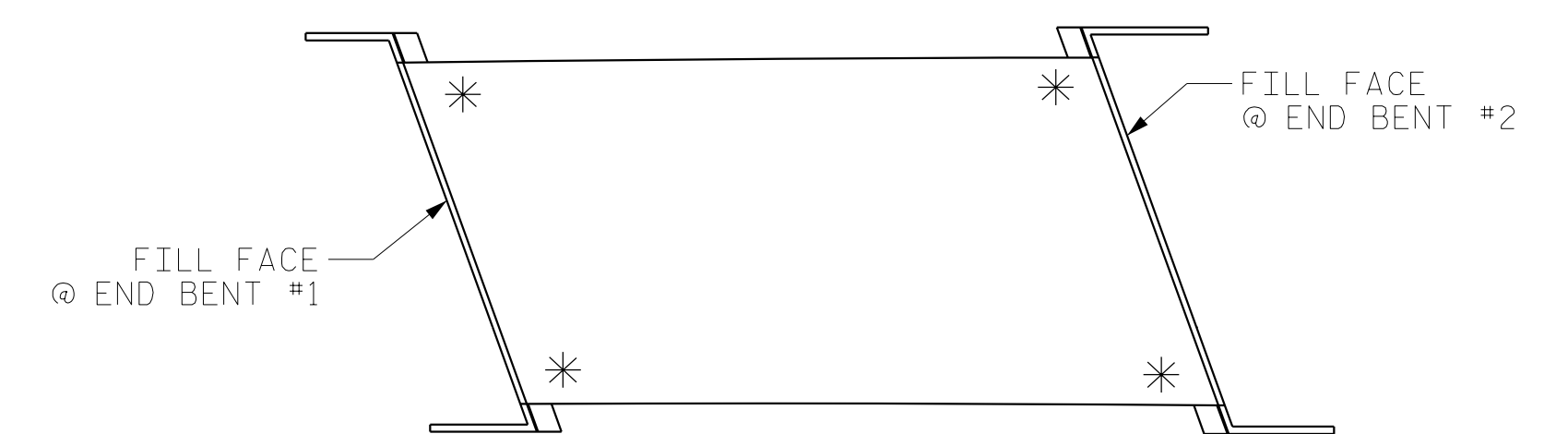
THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENTS

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

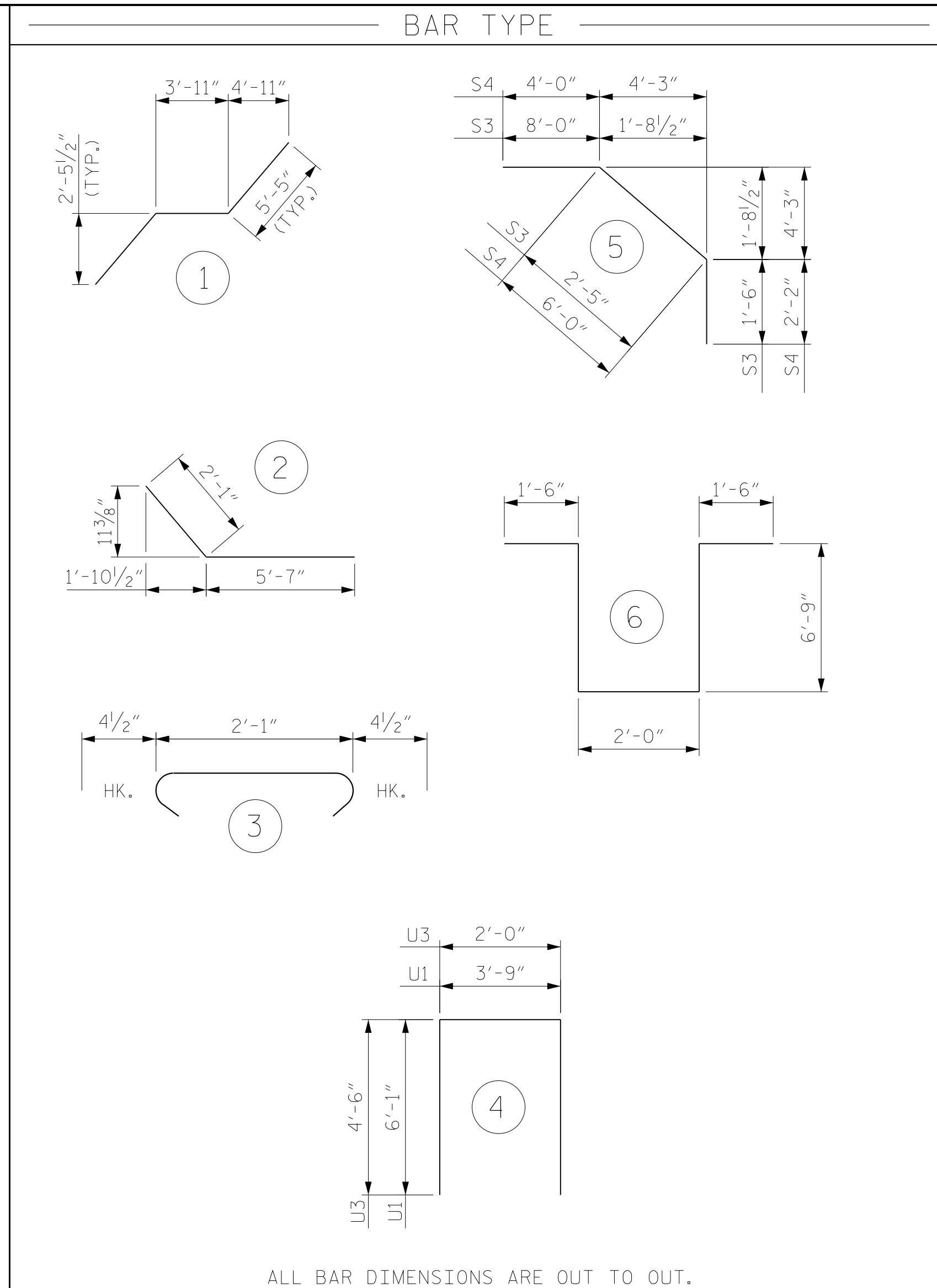


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 GUARDRAIL ANCHORAGE
 FOR BARRIER RAIL
 RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			36
2			4			

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

ASSEMBLED BY :	PDS	DATE :	11/2016
CHECKED BY :	TLC	DATE :	01/2017
DRAWN BY :	TLA 5/06	REV. 10/11/11	MAA/GM
CHECKED BY :	GM 5/06	REV. 7/12	MAA/GM
		REV. 6/13	MAA/GM



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL																							
BAR NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR NO.	SIZE	TYPE	LENGTH	WEIGHT				
* A1	783	#5	STR	42'-11"	35049	* A144	1	#5	STR	22'-10"	24	A230	1	#5	STR	41'-9"	44	K2	84	#4	STR	11'-6"	645
A2	783	#5	STR	42'-11"	35049	* A145	1	#5	STR	21'-6"	22	A231	1	#5	STR	40'-5"	42	K3	6	#4	STR	9'-2"	37
						* A146	1	#5	STR	20'-1"	21	A232	1	#5	STR	39'-1"	41	K4	18	#4	STR	8'-4"	100
* A101	1	#5	STR	41'-9"	44	* A147	1	#5	STR	18'-9"	20	A233	1	#5	STR	37'-9"	39	K5	24	#4	1	14'-9"	236
* A102	1	#5	STR	40'-5"	42	* A148	1	#5	STR	17'-5"	18	A234	1	#5	STR	36'-4"	38	K6	24	#4	2	7'-8"	123
* A103	1	#5	STR	39'-0"	41	* A149	1	#5	STR	16'-1"	17	A235	1	#5	STR	35'-0"	37	K7	20	#4	STR	3'-0"	40
* A104	1	#5	STR	37'-7"	39	* A150	1	#5	STR	14'-8"	15	A236	1	#5	STR	33'-8"	35	K8	4	#4	STR	2'-2"	6
* A105	1	#5	STR	36'-3"	38	* A151	1	#5	STR	13'-4"	14	A237	1	#5	STR	32'-4"	34						
* A106	1	#5	STR	34'-10"	36	* A152	1	#5	STR	12'-0"	13	A238	1	#5	STR	30'-11"	32	S1	330	#4	3	2'-9"	606
* A107	1	#5	STR	33'-6"	35	* A153	1	#5	STR	10'-7"	11	A239	1	#5	STR	29'-7"	31	* S3	64	#4	5	11'-11"	509
* A108	1	#5	STR	32'-1"	33	* A154	1	#5	STR	9'-3"	10	A240	1	#5	STR	28'-3"	29	* S4	64	#4	5	12'-2"	520
* A109	1	#5	STR	30'-8"	32	* A155	1	#5	STR	7'-11"	8	A241	1	#5	STR	26'-11"	28						
* A110	1	#5	STR	29'-4"	31	* A156	1	#5	STR	6'-7"	7	A242	1	#5	STR	25'-6"	27	U1	64	#4	4	15'-11"	680
* A111	1	#5	STR	27'-11"	29	* A157	1	#5	STR	5'-2"	5	A243	1	#5	STR	24'-2"	25	U2	54	#4	6	18'-6"	667
* A112	1	#5	STR	26'-7"	28	* A158	1	#5	STR	3'-10"	4	A244	1	#5	STR	22'-10"	24	U3	12	#4	4	11'-0"	88
* A113	1	#5	STR	25'-2"	26	* A159	1	#5	STR	2'-6"	3	A245	1	#5	STR	21'-6"	22						
* A114	1	#5	STR	23'-10"	25						A246	1	#5	STR	20'-1"	21							
* A115	1	#5	STR	22'-5"	23	A201	1	#5	STR	41'-9"	44	A247	1	#5	STR	18'-9"	20						
* A116	1	#5	STR	21'-0"	22	A202	1	#5	STR	40'-5"	42	A248	1	#5	STR	17'-5"	18						
* A117	1	#5	STR	19'-8"	21	A203	1	#5	STR	39'-0"	41	A249	1	#5	STR	16'-1"	17						
* A118	1	#5	STR	18'-3"	19	A204	1	#5	STR	37'-7"	39	A250	1	#5	STR	14'-8"	15						
* A119	1	#5	STR	16'-11"	18	A205	1	#5	STR	36'-3"	38	A251	1	#5	STR	13'-4"	14						
* A120	1	#5	STR	15'-6"	16	A206	1	#5	STR	34'-10"	36	A252	1	#5	STR	12'-0"	13						
* A121	1	#5	STR	14'-1"	15	A207	1	#5	STR	33'-6"	35	A253	1	#5	STR	10'-7"	11						
* A122	1	#5	STR	12'-9"	13	A208	1	#5	STR	32'-1"	33	A254	1	#5	STR	9'-3"	10						
* A123	1	#5	STR	11'-4"	12	A209	1	#5	STR	30'-8"	32	A255	1	#5	STR	7'-11"	8						
* A124	1	#5	STR	10'-0"	10	A210	1	#5	STR	29'-4"	31	A256	1	#5	STR	6'-7"	7						
* A125	1	#5	STR	8'-7"	9	A211	1	#5	STR	27'-11"	29	A257	1	#5	STR	5'-2"	5						
* A126	1	#5	STR	7'-3"	8	A212	1	#5	STR	26'-7"	28	A258	1	#5	STR	3'-10"	4						
* A127	1	#5	STR	5'-10"	6	A213	1	#5	STR	25'-2"	26	A259	1	#5	STR	2'-6"	3						
* A128	1	#5	STR	4'-5"	5	A214	1	#5	STR	23'-10"	25												
* A129	1	#5	STR	3'-1"	3	A215	1	#5	STR	22'-5"	23	* B1	104	#7	STR	27'-10"	5917						
* A130	1	#5	STR	41'-9"	44	A216	1	#5	STR	21'-0"	22	B2	336	#5	STR	53'-2"	18632						
* A131	1	#5	STR	40'-5"	42	A217	1	#5	STR	19'-8"	21	* B3	156	#4	STR	21'-10"	2275						
* A132	1	#5	STR	39'-1"	41	A218	1	#5	STR	18'-3"	19	* B4	104	#7	STR	60'-0"	12755						
* A133	1	#5	STR	37'-9"	39	A219	1	#5	STR	16'-11"	18	* B5	51	#7	STR	40'-6"	4222						
* A134	1	#5	STR	36'-4"	38	A220	1	#5	STR	15'-6"	16	* B6	52	#7	STR	36'-3"	3853						
* A135	1	#5	STR	35'-0"	37	A221	1	#5	STR	14'-1"	15	* B7	33	#5	STR	40'-6"	1394						
* A136	1	#5	STR	33'-8"	35	A222	1	#5	STR	12'-9"	13	* B8	33	#5	STR	42'-0"	1446						
* A137	1	#5	STR	32'-4"	34	A223	1	#5	STR	11'-4"	12	* B9	104	#4	STR	26'-1"	1812						
* A138	1	#5	STR	30'-11"	32	A224	1	#5	STR	10'-0"	10	* B10	52	#7	STR	60'-0"	6377						
* A139	1	#5	STR	29'-7"	31	A225	1	#5	STR	8'-7"	9	* B11	51	#7	STR	42'-0"	4378						
* A140	1	#5	STR	28'-3"	29	A226	1	#5	STR	7'-3"	8	* B12	52	#7	STR	39'-3"	4172						
* A141	1	#5	STR	26'-11"	28	A227	1	#5	STR	5'-10"	6	* B13	156	#4	STR	24'-2"	2518						
* A142	1	#5	STR	25'-6"	27	A228	1	#5	STR	4'-5"	5												
* A143	1	#5	STR	24'-2"	25	A229	1	#5	STR	3'-1"	3	K1	24	#4	STR	24'-5"	391						

REINFORCING STEEL 60,067 LBS.
 *EPOXY COATED REINFORCING STEEL 87,176 LBS.

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS

BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET, AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER RAIL
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3'-5"
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"
#7	5'-3"	3'-6"			
#8	6'-10"	4'-7"			

GROOVING BRIDGE FLOORS

APPROACH SLABS	1,734 SQ.FT.
BRIDGE DECK	15,093 SQ.FT.
TOTAL	16,827 SQ.FT.

—SUPERSTRUCTURE BILL OF MATERIAL—

	CLASS AA CONCRETE (CU. YDS.)	REINFORCING STEEL (LBS.)	EPOXY COATED REINFORCING STEEL (LBS.)
POUR 1	183.3		
POUR 2	241.4		
POUR 3	241.7		
POUR 4	119.7		
TOTALS**	786.1	60,067 LBS.	87,176 LBS.

**QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 1 OF 2

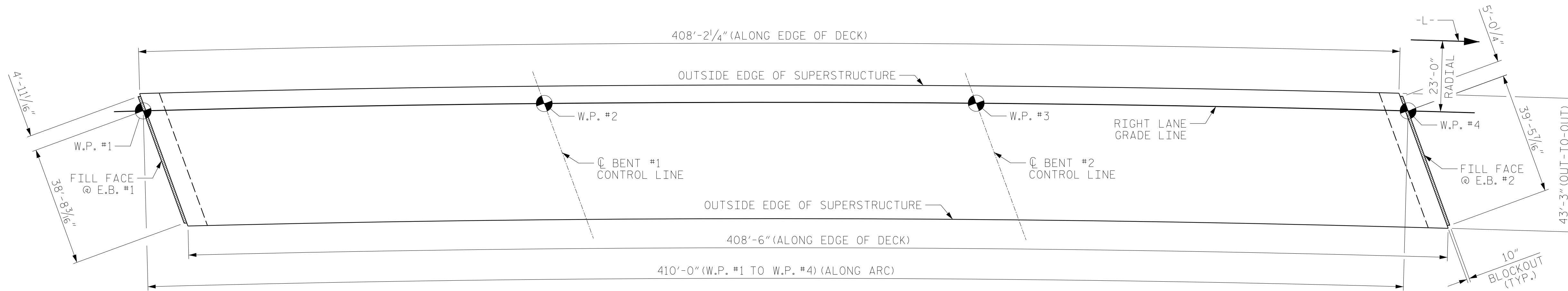


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 BILL OF MATERIAL
 RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			S2-21
2			4			TOTAL SHEETS 36

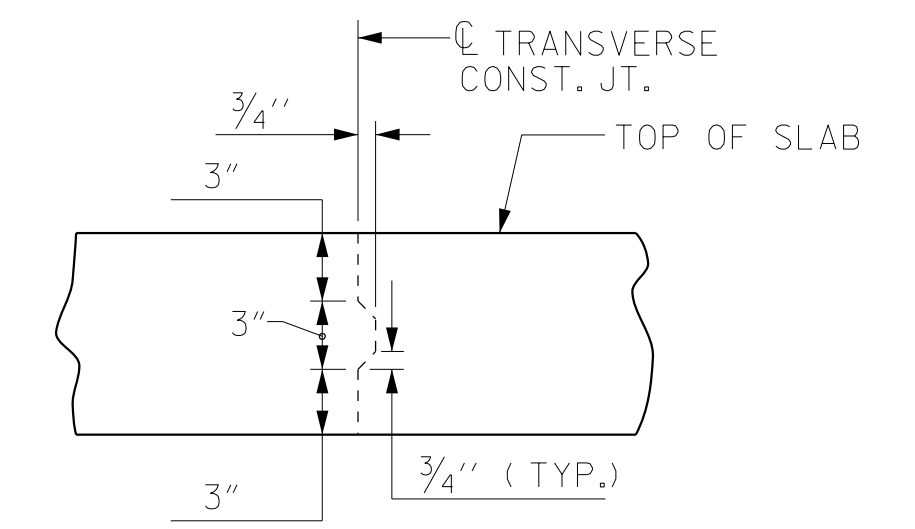
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 CHECKED BY : TLC DATE : 01/2017
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

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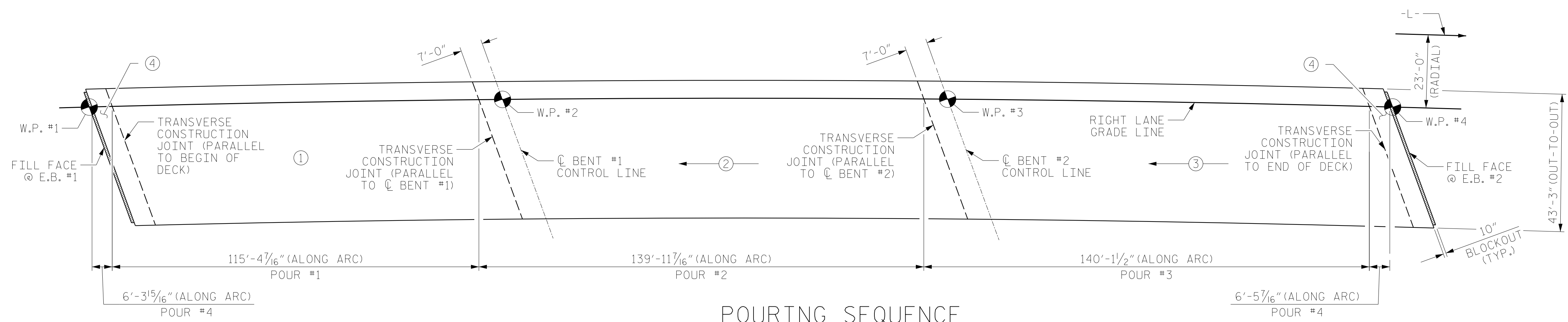
LAYOUT FOR COMPUTING
 AREA OF REINFORCED CONCRETE DECK SLAB
 (SQ. FT = 17,661)

NOTE: BENT & INTEGRAL END BENT DIAPHRAGMS ARE PART OF SUPERSTRUCTURE POUR.

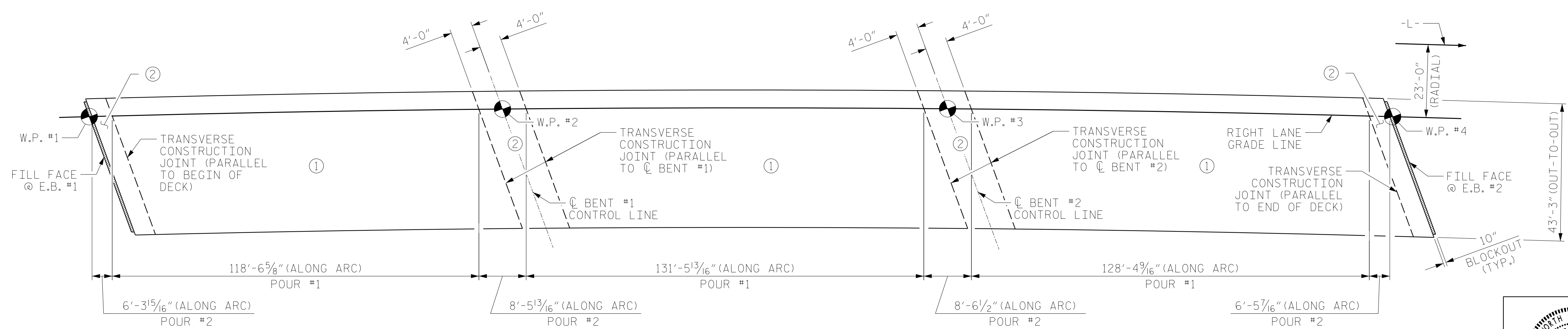


TRANSVERSE CONSTRUCTION JOINT DETAIL

NOTE: REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THRU JOINT



POURING SEQUENCE



OPTIONAL POURING SEQUENCE

POUR 2 CAN NOT BE STARTED UNTIL BOTH ADJACENT ①POURS REACH A MINIMUM OF 3000 PSI

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 2 OF 2



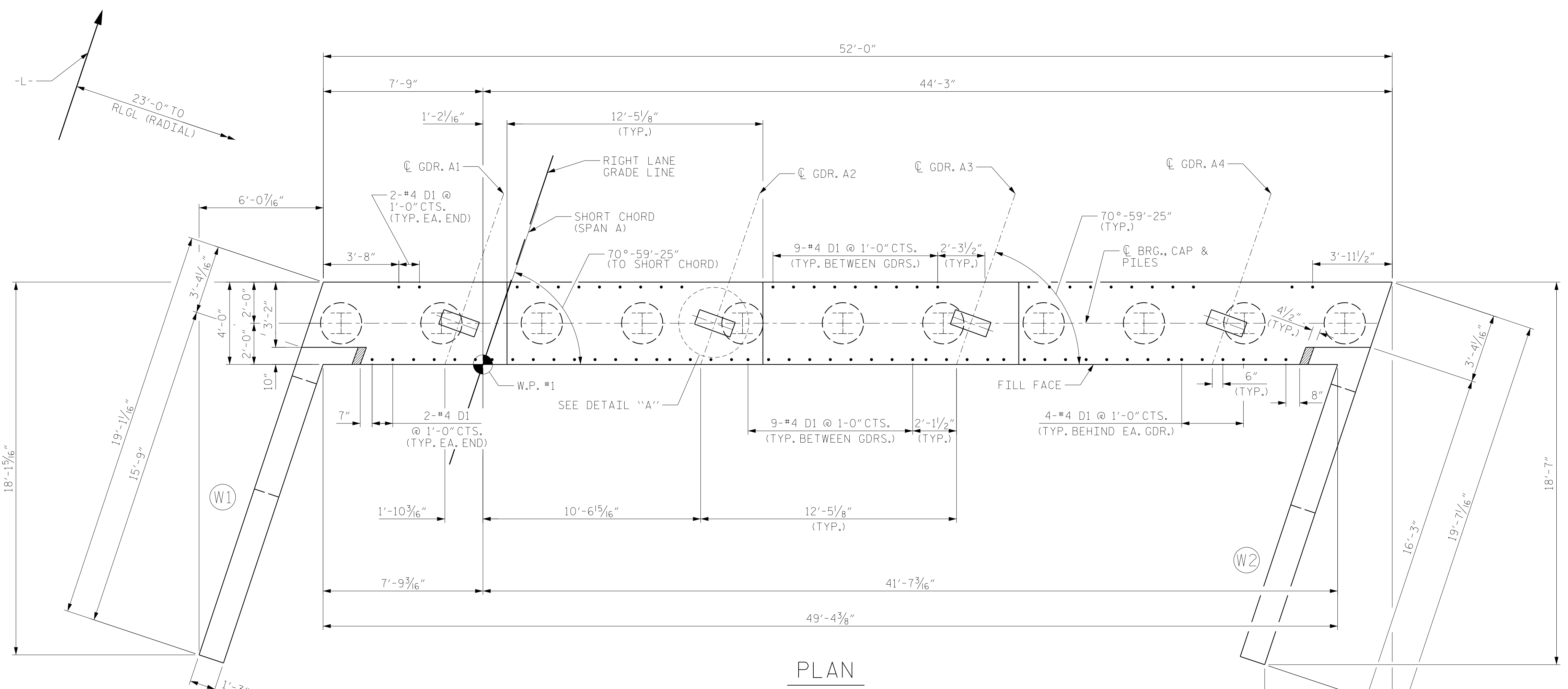
8601 Six Forks Road, Suite 260
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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 BILL OF MATERIAL
 RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-22
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : PDS DATE : 12/2016
 CHECKED BY : TLC DATE : 01/2017
 DESIGN ENGINEER OF RECORD: MAL DATE : 12/2016

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PLAN

NOTES

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

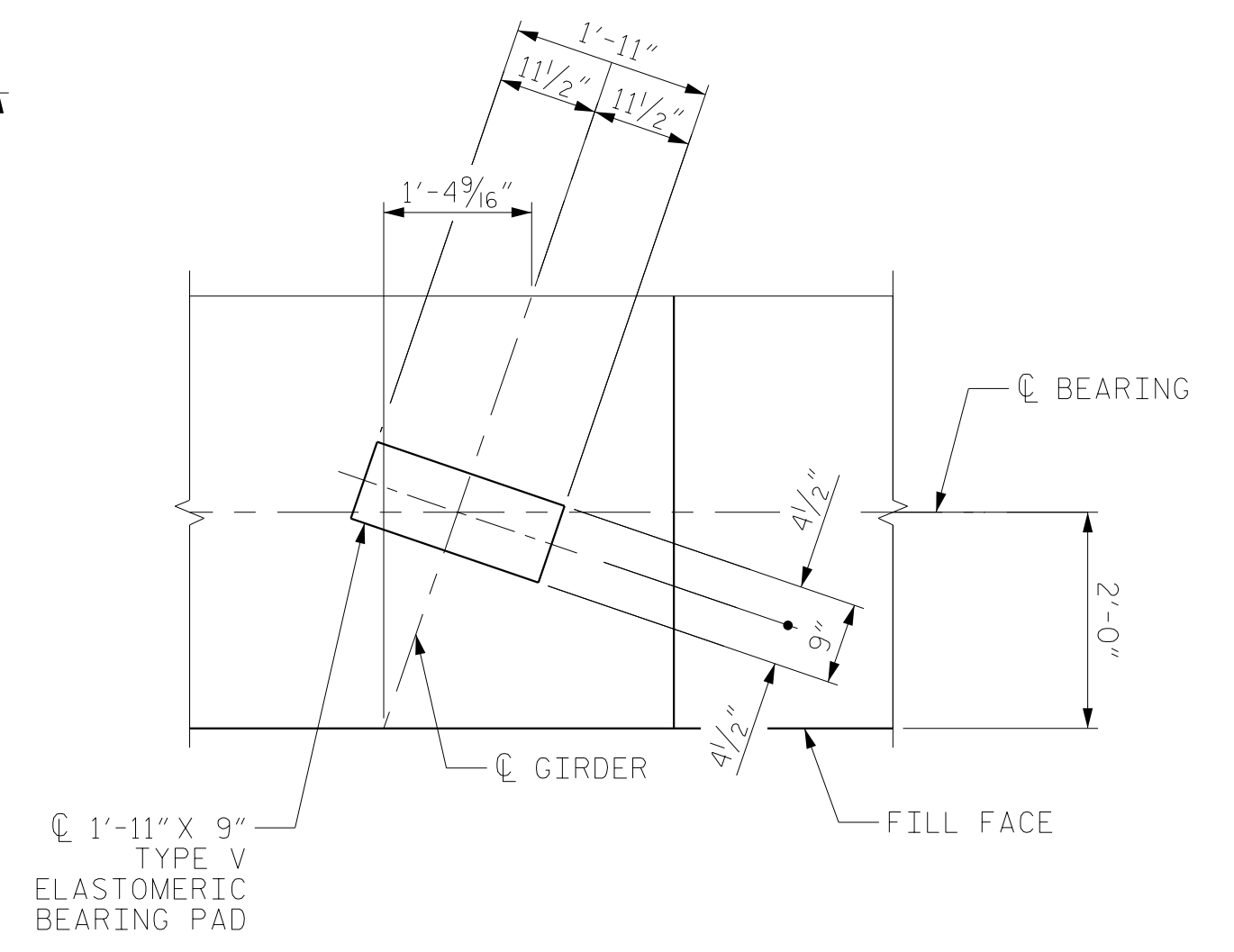
FOR SECTION A-A AND SECTION B-B, SEE SHEET 4 OF 4.

STIRRUPS MAY BE SHIFTED SLIGHTLY TO AVOID #4 D1 BARS IN CAP.

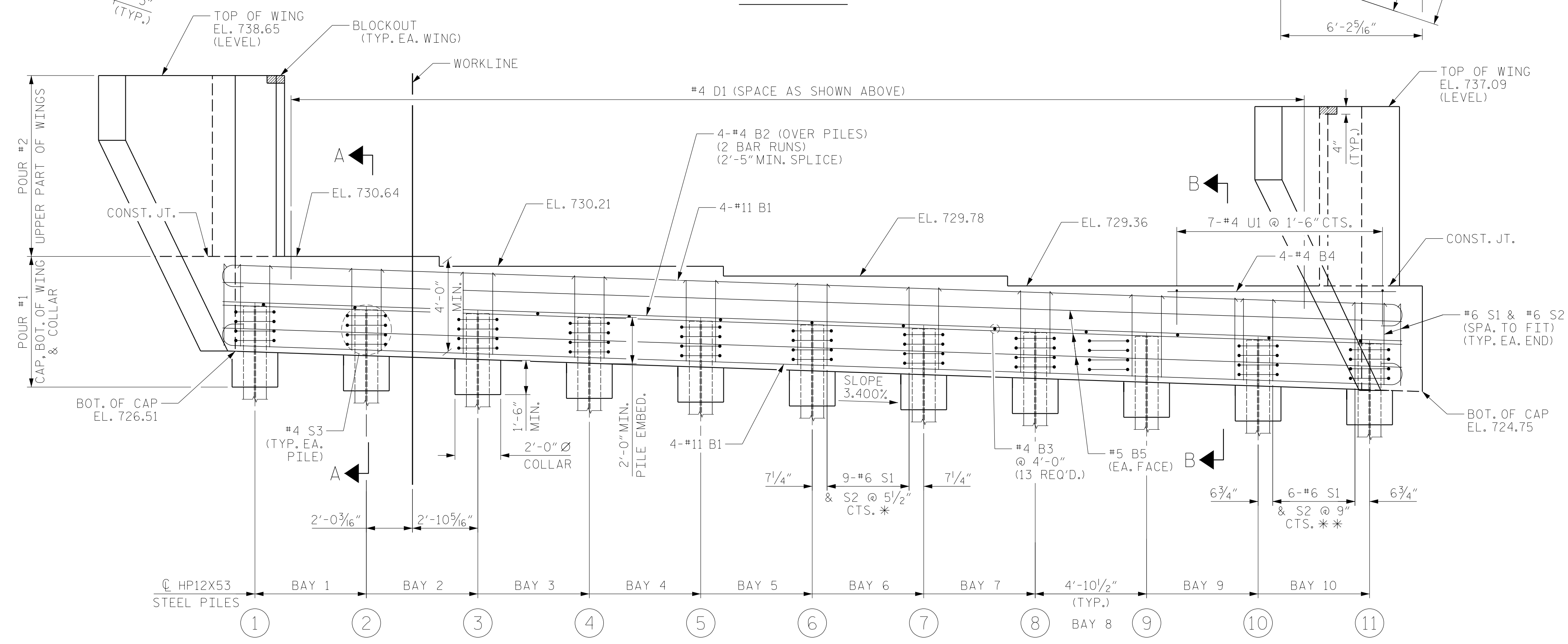
SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE CONCRETE BARRIER IS CAST IF SLIP FORMING IS USED.

RLGL = RIGHT LANE GRADE LINE



DETAIL "A"
DIMENSIONS TYP. EA. BRG. PILE NOT SHOWN FOR CLARITY



ELEVATION

TOP OF PILE ELEVATIONS	
PILE	ELEVATION
1	728.47
2	728.30
3	728.14
4	727.97
5	727.81
6	727.64
7	727.48
8	727.31
9	727.15
10	726.98
11	726.82

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-

SHEET 1 OF 4

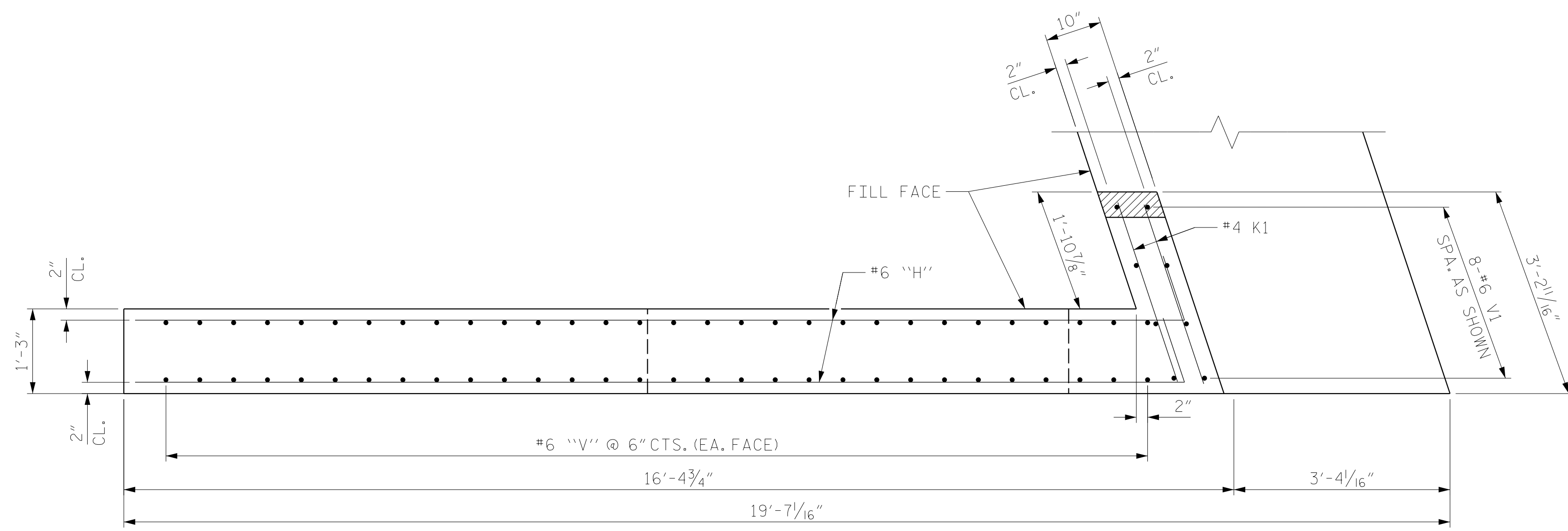


STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE INTEGRAL END BENT NO. 1 RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

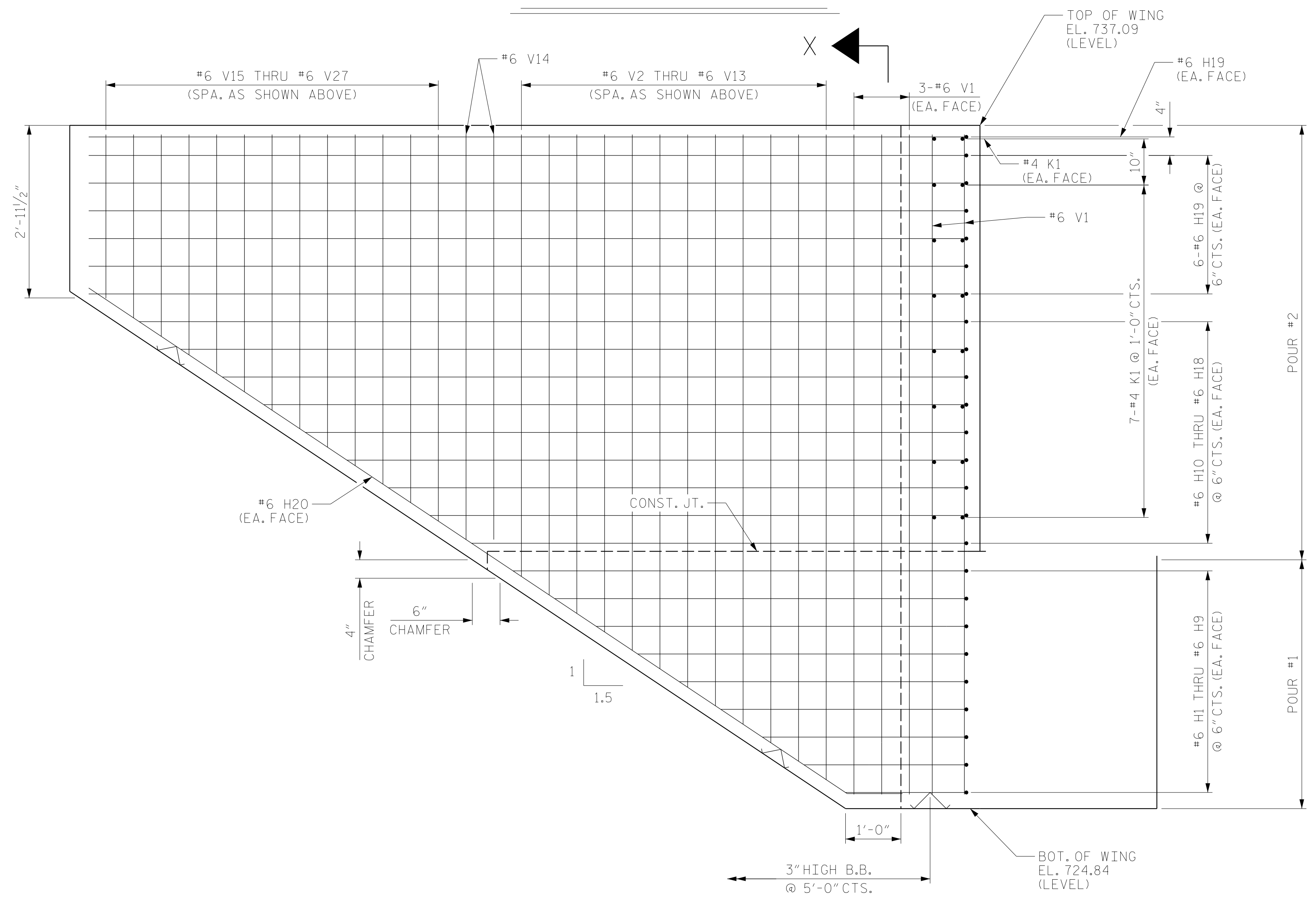
DRAWN BY : PDS DATE : 11/2016
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DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

* = TYPICAL BAY 4 THRU BAY 7
** = TYPICAL BAY 1 THRU BAY 3 AND BAY 8 THRU BAY 10

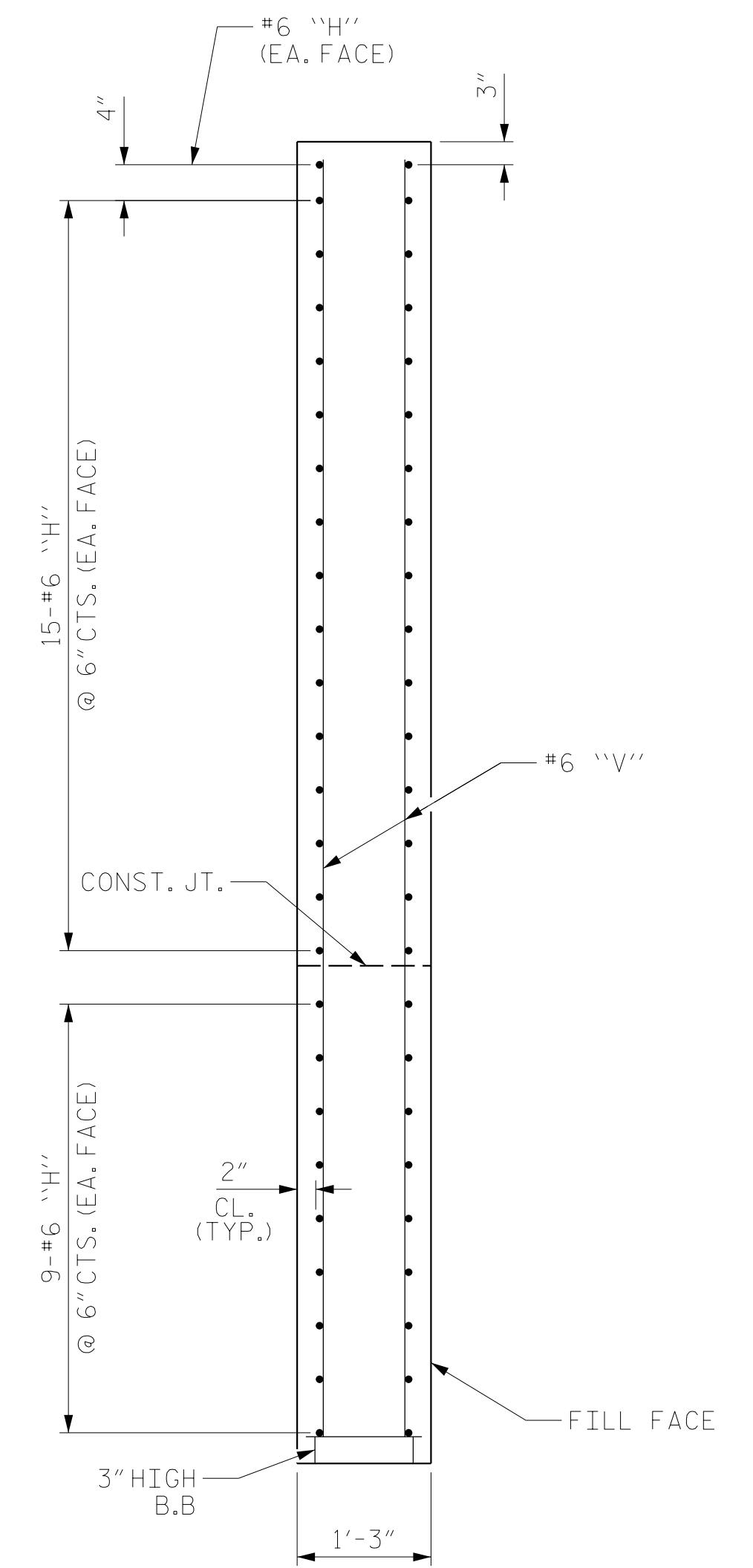
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PLAN OF WING - W2



ELEVATION OF WING - W2



SECTION X-X

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 2 OF 4

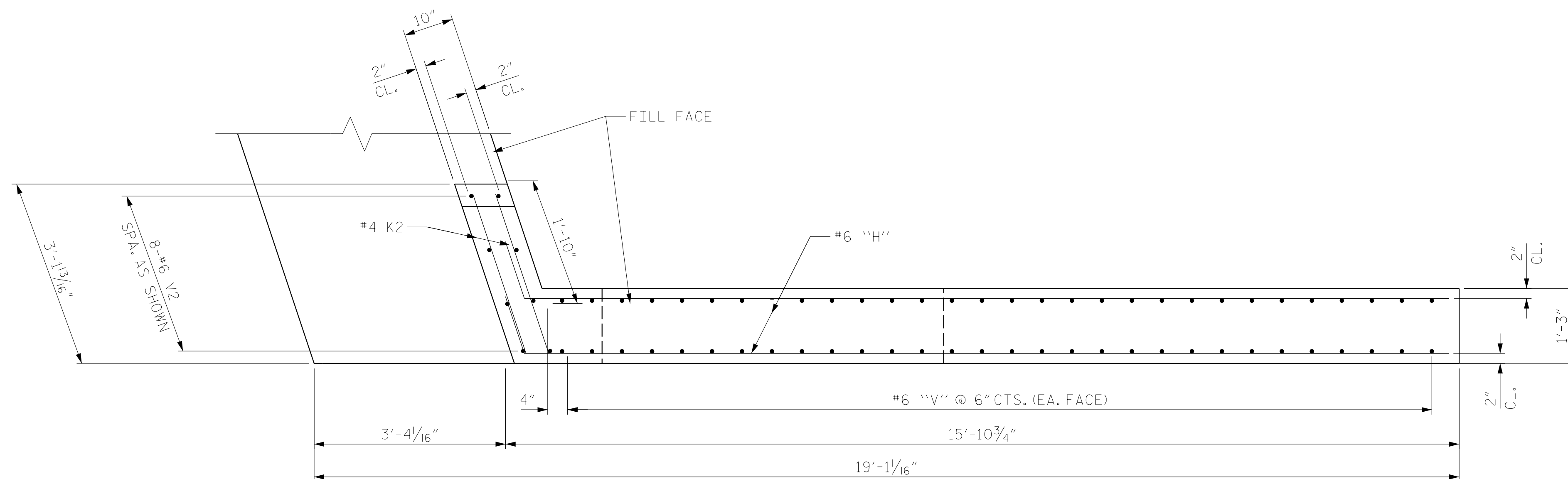


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 RALEIGH
 SUBSTRUCTURE
 INTEGRAL
 END BENT NO. 1
 RIGHT LANE

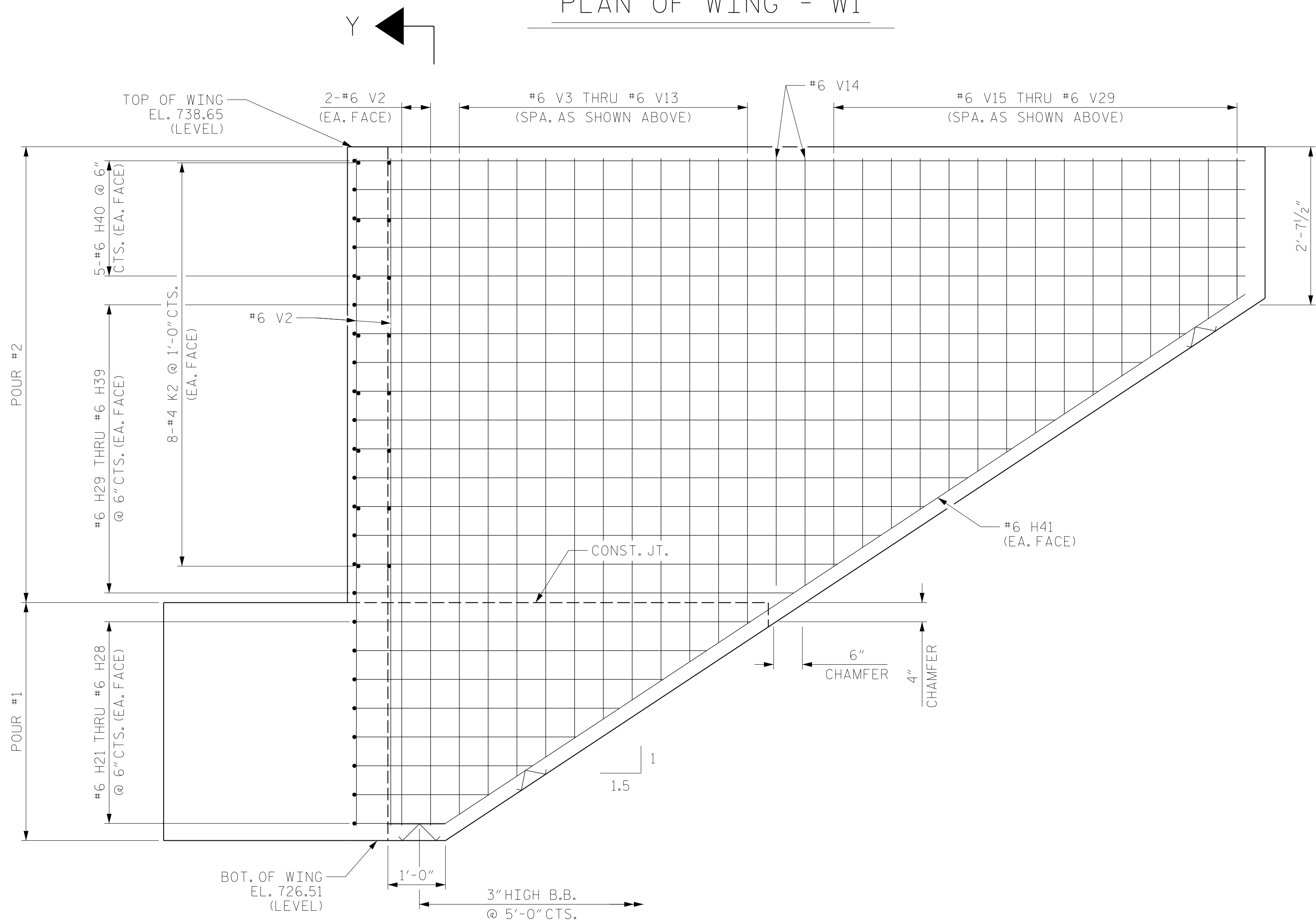
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-24
1			3			TOTAL SHEETS
2			4			36

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 CHECKED BY : TLC DATE : 01/2017
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

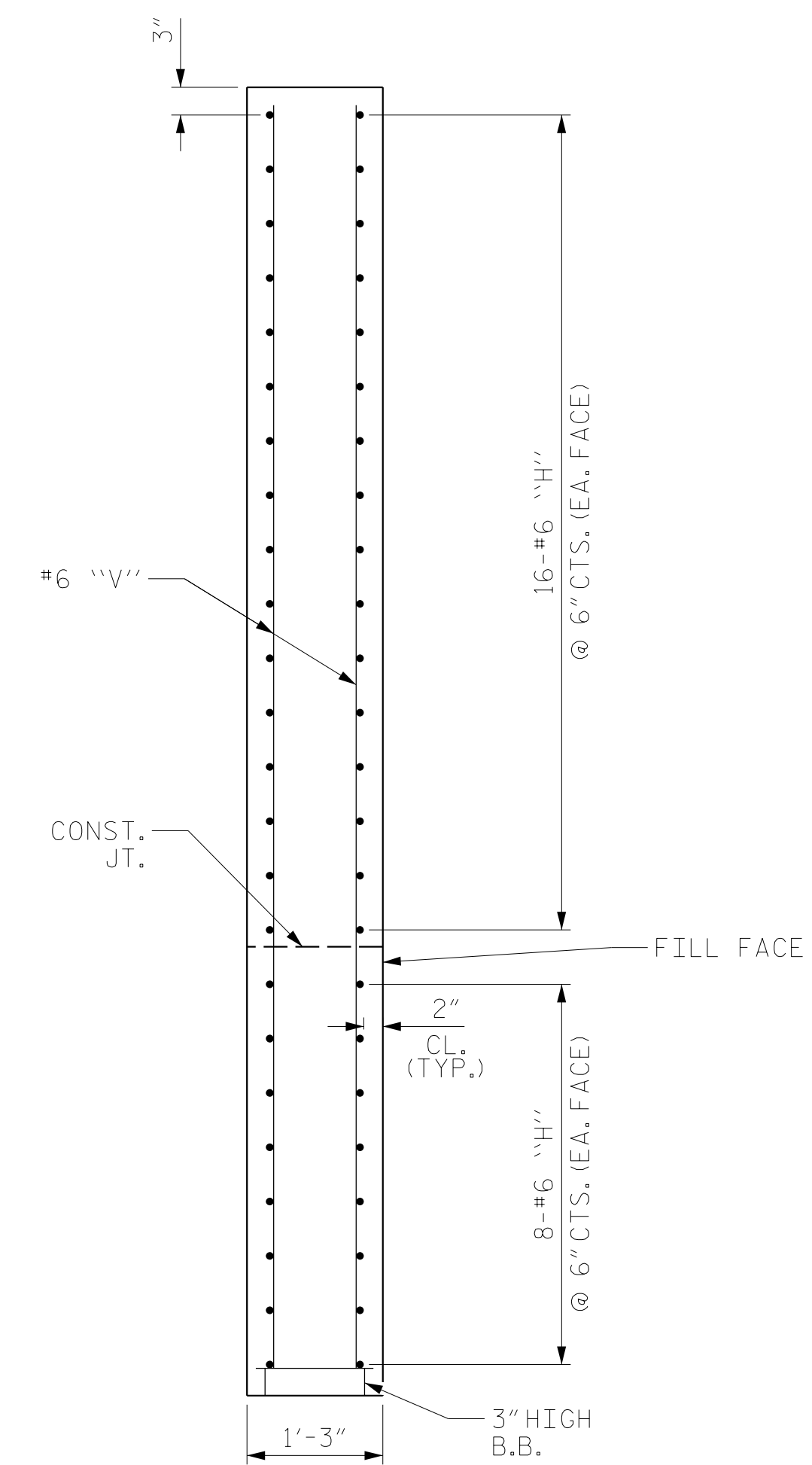
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PLAN OF WING - W1



ELEVATION OF WING - W1



SECTION Y-Y

PROJECT NO. R-2707C
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SHEET 3 OF 4

3/9/2017

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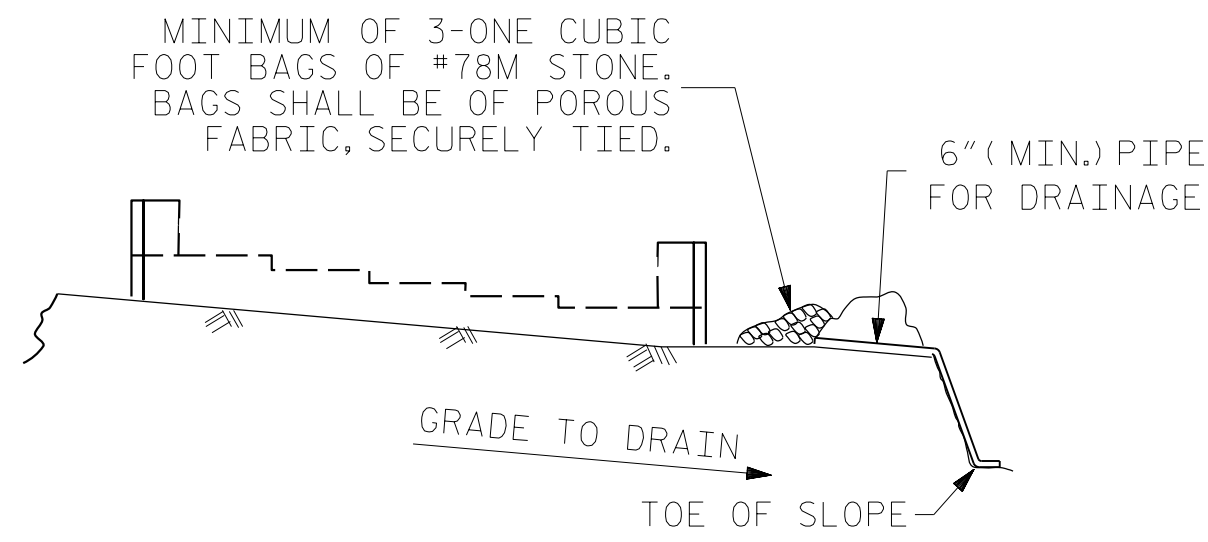
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL
 END BENT NO. 1
 RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-25
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : PDS DATE : 11/2016
 CHECKED BY : TLC DATE : 01/2017
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

DOCUMENT NOT CONSIDERED
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 SIGNATURES COMPLETED



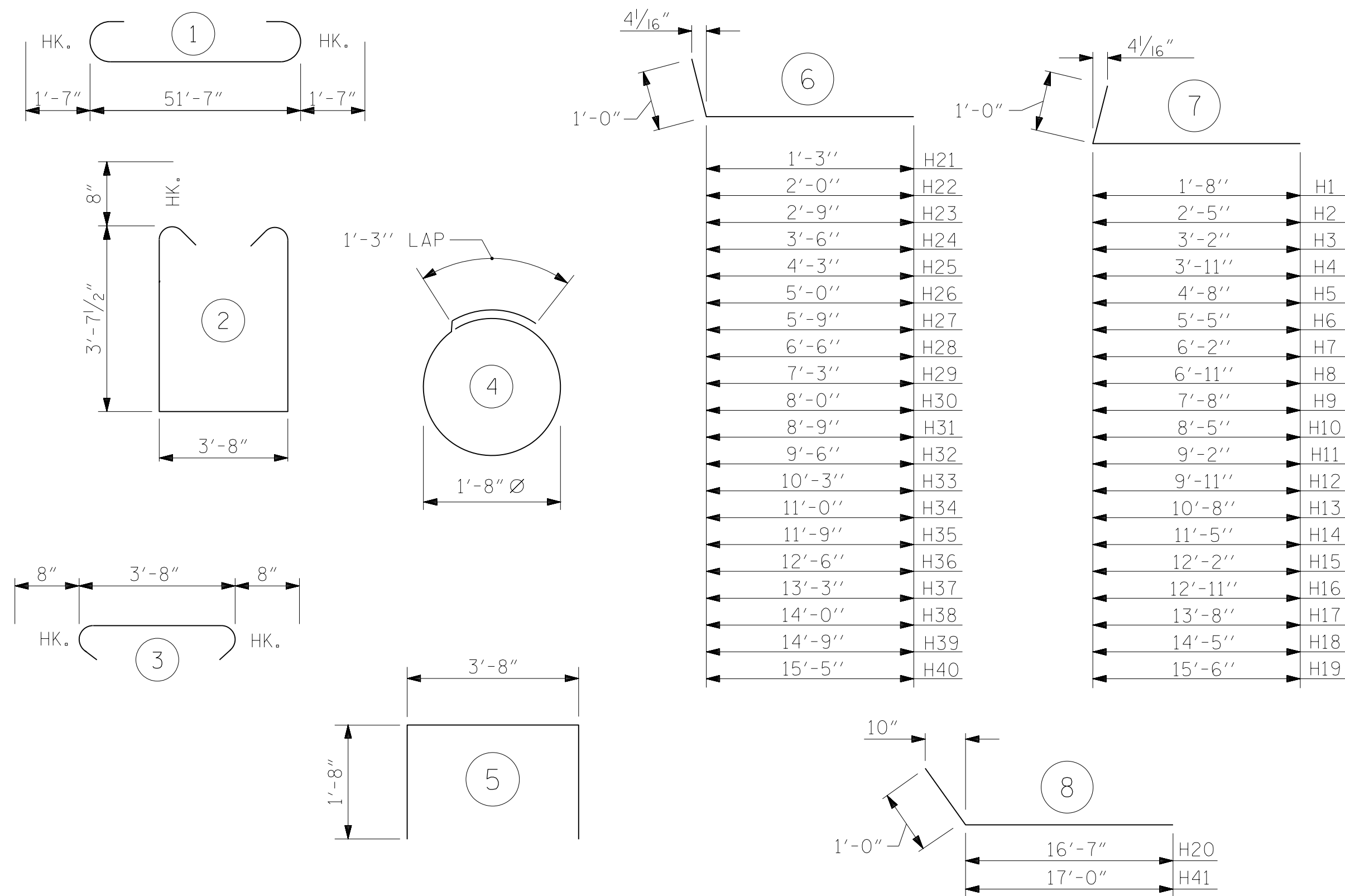
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

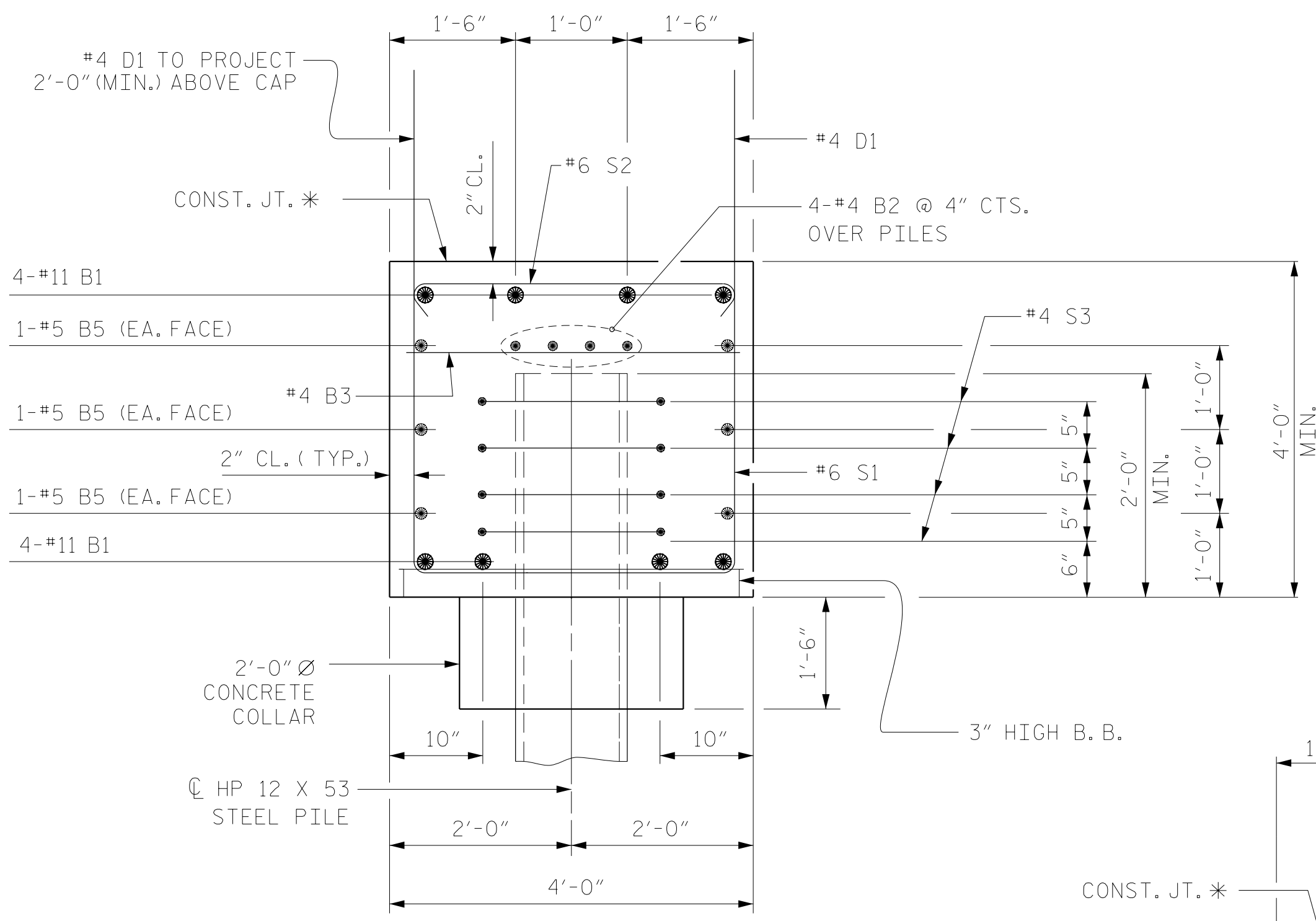
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

BAR TYPES

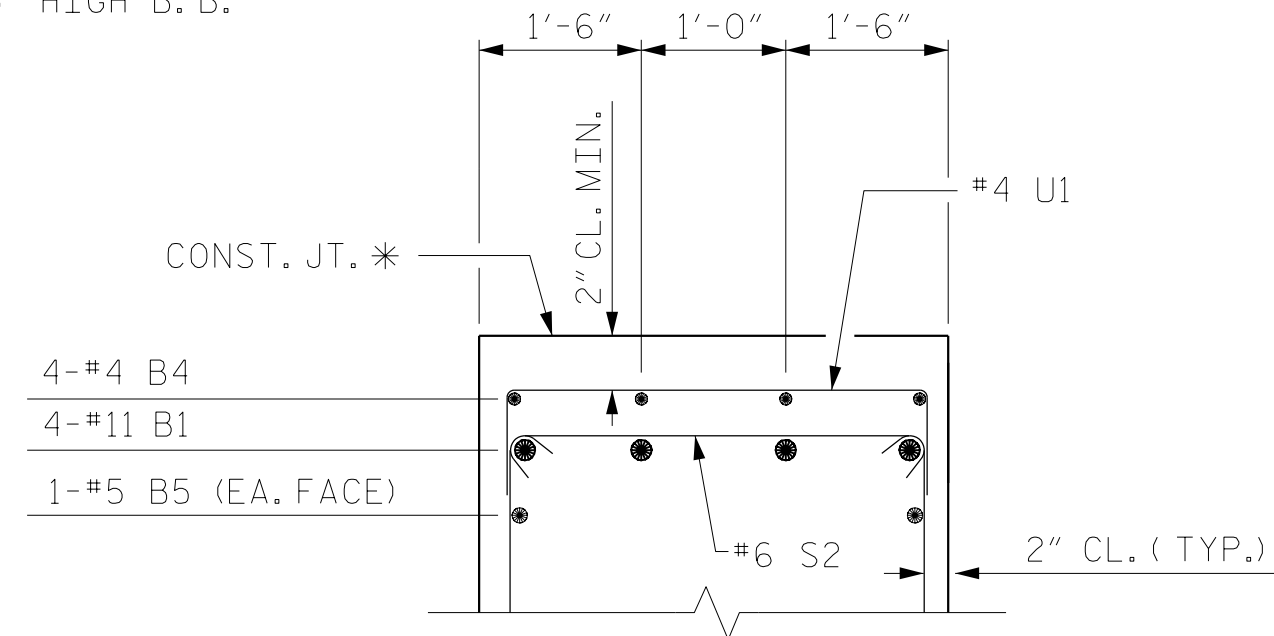


ALL BAR DIMENSIONS ARE OUT TO OUT.

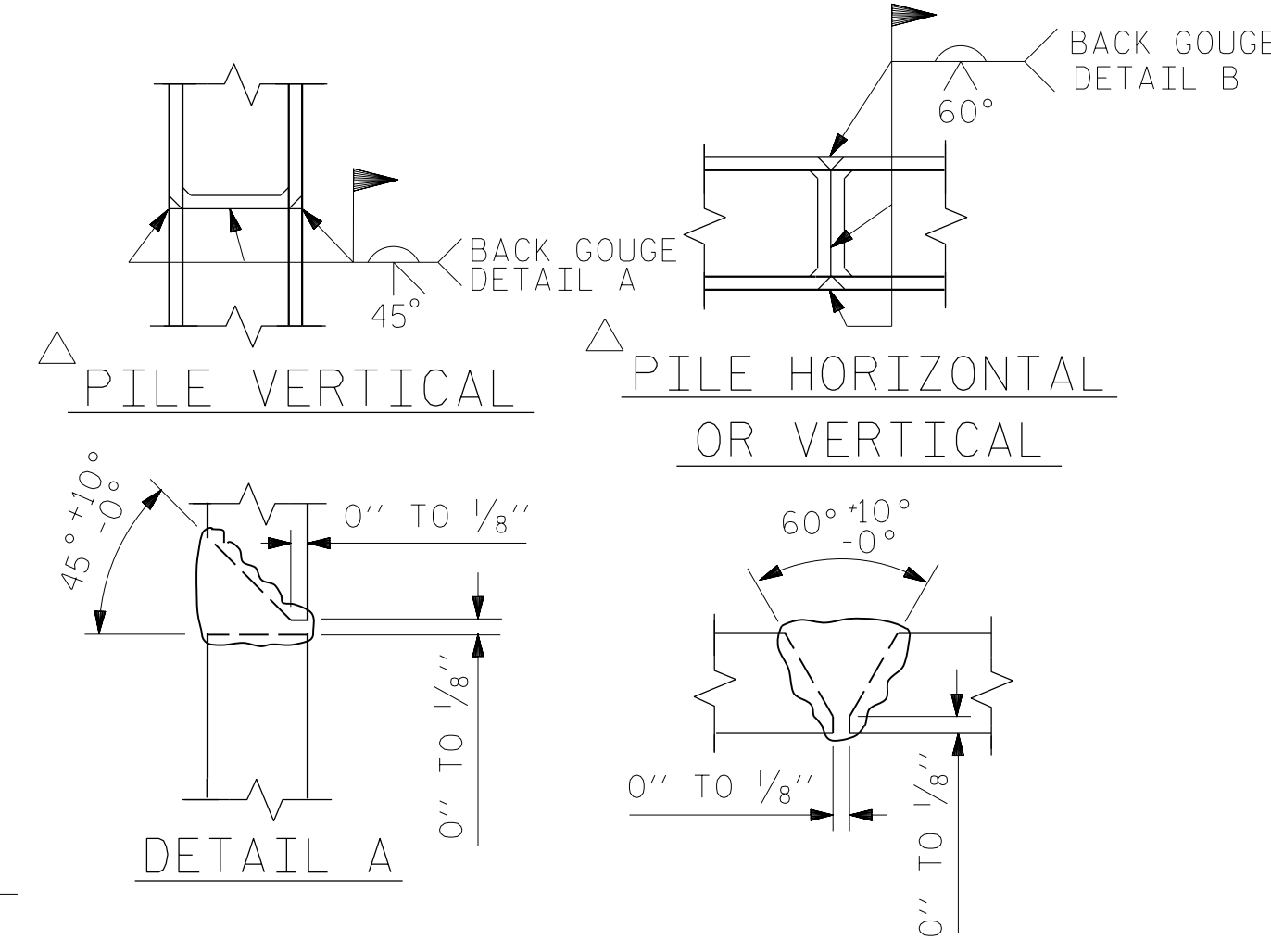


SECTION A-A

* THE TOP SURFACE OF THE END BENT CAP & WINGS, EXCLUDING THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4\".



PARTIAL SECTION B-B



△ POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS

BILL OF MATERIAL

END BENT NO. 1

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#11	1	54'-9"	2327	K1	16	#4	STR	2'-10"	30
B2	8	#4	STR	27'-1"	145	K2	16	#4	STR	2'-9"	29
B3	13	#4	STR	3'-8"	32						
B4	4	#4	STR	10'-0"	27	S1	76	#6	2	12'-3"	1398
B5	6	#5	STR	51'-7"	323	S2	76	#6	3	5'-0"	571
						S3	44	#4	4	6'-6"	191
D1	78	#4	STR	4'-0"	208	U1	7	#4	5	7'-0"	33
H1	2	#6	7	2'-8"	8	V1	14	#6	STR	11'-11"	251
H2	2	#6	7	3'-5"	10	V2	14	#6	STR	11'-8"	245
H3	2	#6	7	4'-2"	13	V3	4	#6	STR	11'-4"	68
H4	2	#6	7	4'-11"	15	V4	4	#6	STR	11'-0"	66
H5	2	#6	7	5'-5"	17	V5	4	#6	STR	10'-8"	64
H6	2	#6	7	6'-5"	19	V6	4	#6	STR	10'-4"	62
H7	2	#6	7	7'-2"	22	V7	4	#6	STR	10'-0"	60
H8	2	#6	7	7'-11"	24	V8	4	#6	STR	9'-8"	58
H9	2	#6	7	8'-8"	26	V9	4	#6	STR	9'-4"	56
H10	2	#6	7	9'-5"	28	V10	4	#6	STR	9'-0"	54
H11	2	#6	7	10'-2"	31	V11	4	#6	STR	8'-8"	52
H12	2	#6	7	10'-11"	33	V12	4	#6	STR	8'-4"	50
H13	2	#6	7	11'-8"	35	V13	4	#6	STR	8'-0"	48
H14	2	#6	7	12'-5"	37	V14	8	#6	STR	7'-4"	88
H15	2	#6	7	13'-2"	40	V15	4	#6	STR	7'-0"	42
H16	2	#6	7	13'-11"	42	V16	4	#6	STR	6'-8"	40
H17	2	#6	7	14'-8"	44	V17	4	#6	STR	6'-4"	38
H18	2	#6	7	15'-5"	46	V18	4	#6	STR	6'-0"	36
H19	14	#6	7	16'-6"	347	V19	4	#6	STR	5'-8"	34
H20	2	#6	8	17'-7"	53	V20	4	#6	STR	5'-4"	32
H21	2	#6	6	2'-3"	7	V21	4	#6	STR	5'-0"	30
H22	2	#6	6	3'-0"	9	V22	4	#6	STR	4'-8"	28
H23	2	#6	6	3'-9"	11	V23	4	#6	STR	4'-4"	26
H24	2	#6	6	4'-6"	14	V24	4	#6	STR	4'-0"	24
H25	2	#6	6	5'-3"	16	V25	4	#6	STR	3'-8"	22
H26	2	#6	6	6'-0"	18	V26	4	#6	STR	3'-4"	20
H27	2	#6	6	6'-9"	20	V27	4	#6	STR	2'-11"	18
H28	2	#6	6	7'-6"	23	V28	2	#6	STR	2'-8"	8
H29	2	#6	6	8'-3"	25	V29	2	#6	STR	2'-4"	7
H30	2	#6	6	9'-0"	27						
H31	2	#6	6	9'-9"	29						
H32	2	#6	6	10'-6"	32						
H33	2	#6	6	11'-3"	34						
H34	2	#6	6	12'-0"	36						
H35	2	#6	6	12'-9"	38						
H36	2	#6	6	13'-6"	41						
H37	2	#6	6	14'-3"	43						
H38	2	#6	6	15'-0"	45						
H39	2	#6	6	15'-9"	47						
H40	10	#6	6	16'-5"	247						
H41	2	#6	8	18'-0"	54						

REINFORCING STEEL	8,647 LBS.
CLASS A CONCRETE	
POUR #1	
CAP, CONCRETE COLLARS & LOWER PART OF WINGS	36.4 C. Y.
POUR #2	
UPPER PART OF WINGS	11.1 C. Y.
TOTAL	47.5 C. Y.
HP 12 X 53 STEEL PILES	
NO. 11	440 LIN. FT.
STEEL PILE POINTS	NO.: 11

PILE DRIVING EQUIPMENT SETUP NO. 11

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 4 OF 4

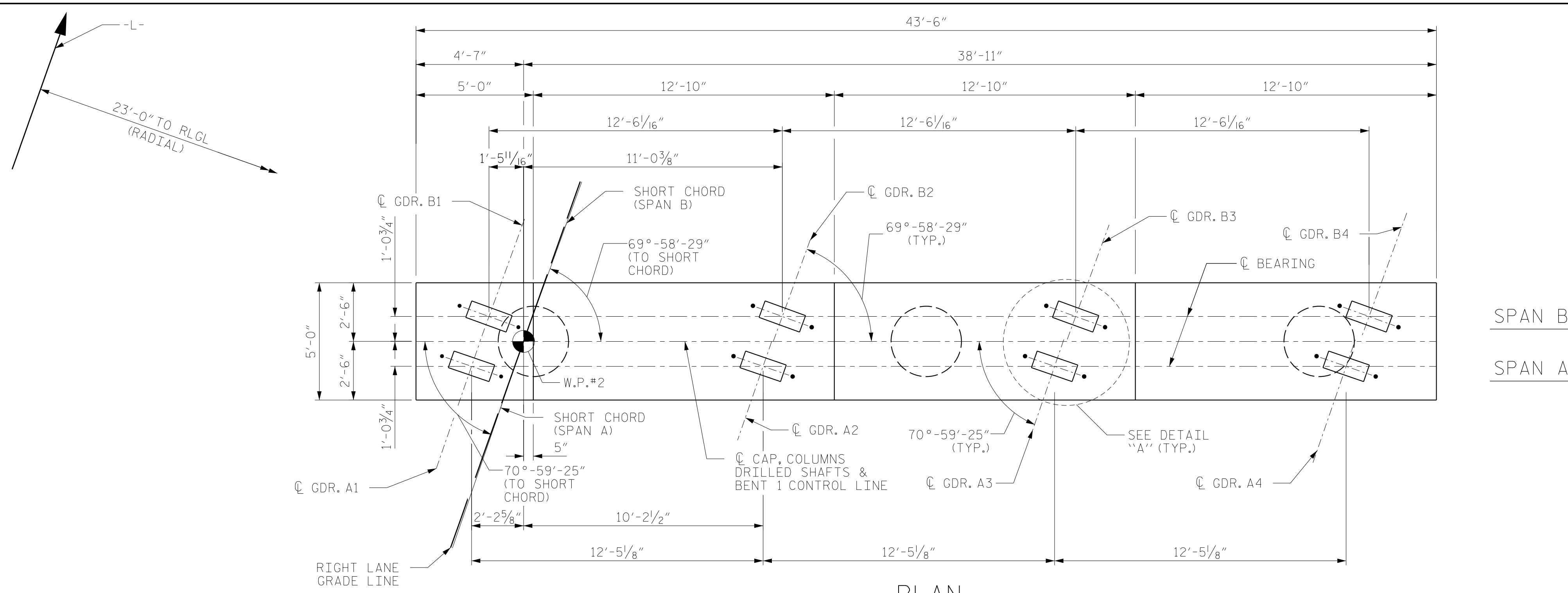


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 SUBSTRUCTURE
 INTEGRAL
 END BENT NO. 1
 RIGHT LANE

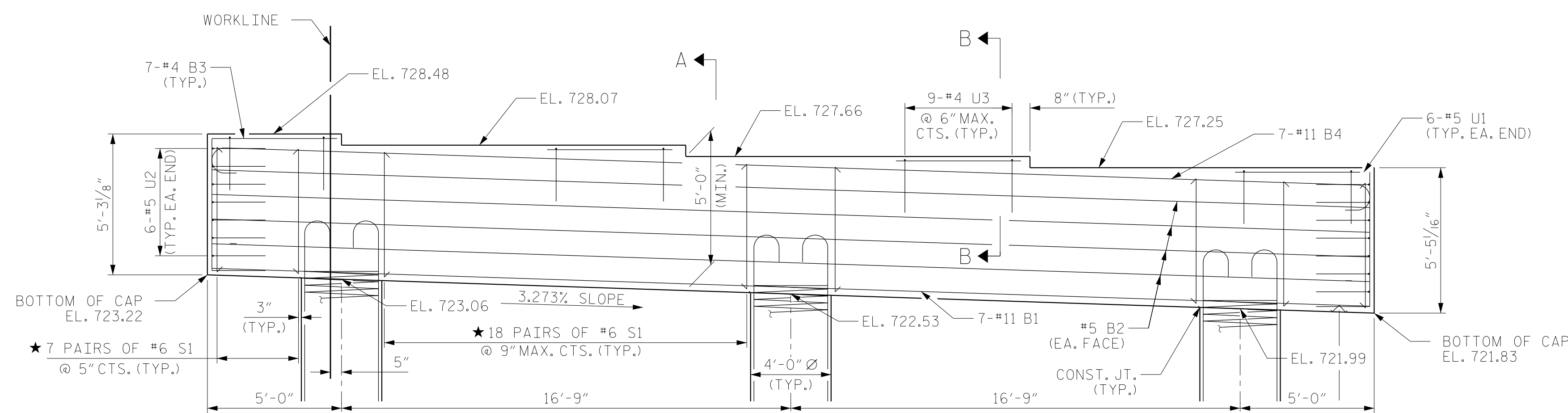
REVISIONS						SHEET NO.
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1			3			S2-26
2			4			TOTAL SHEETS 36

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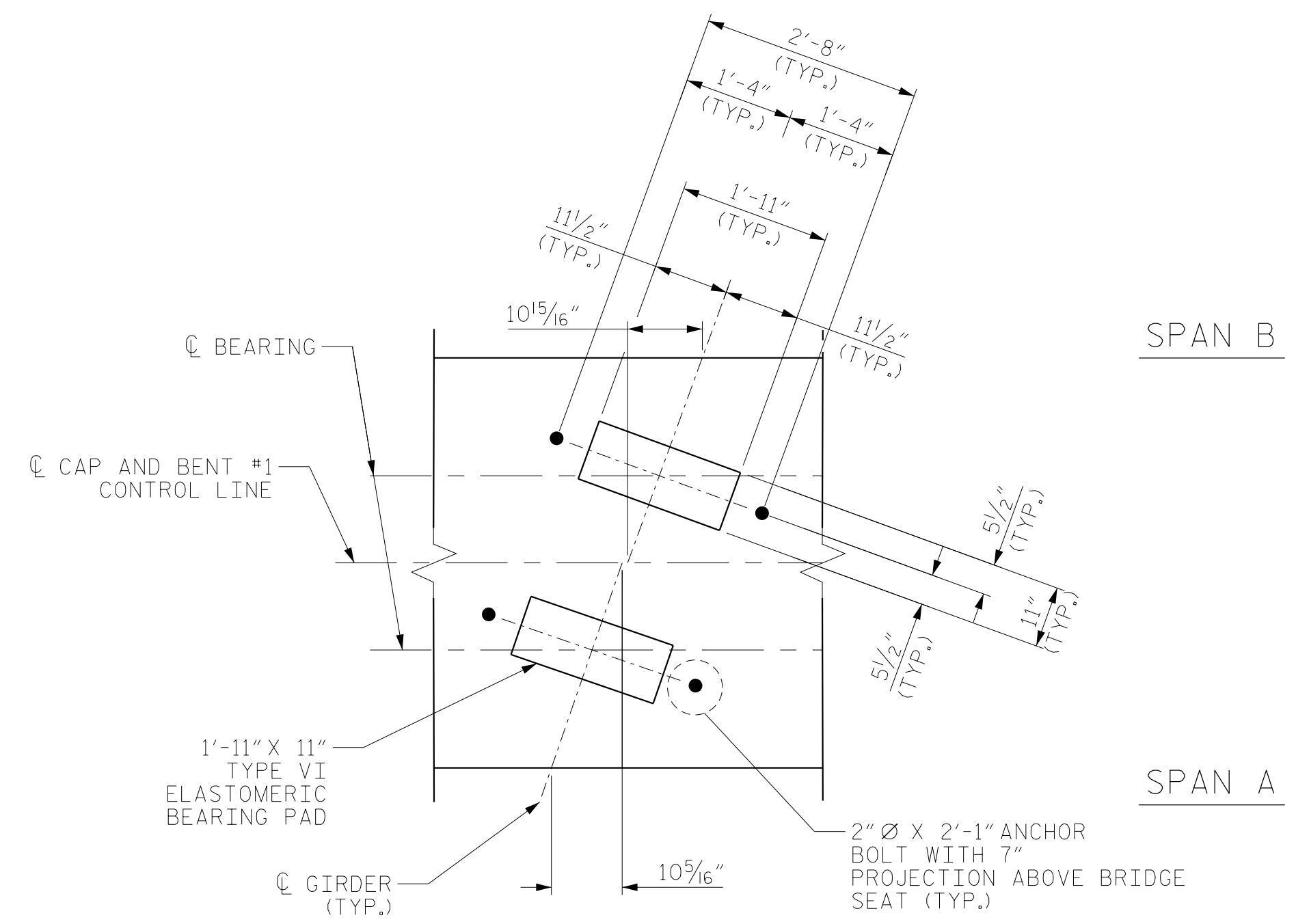
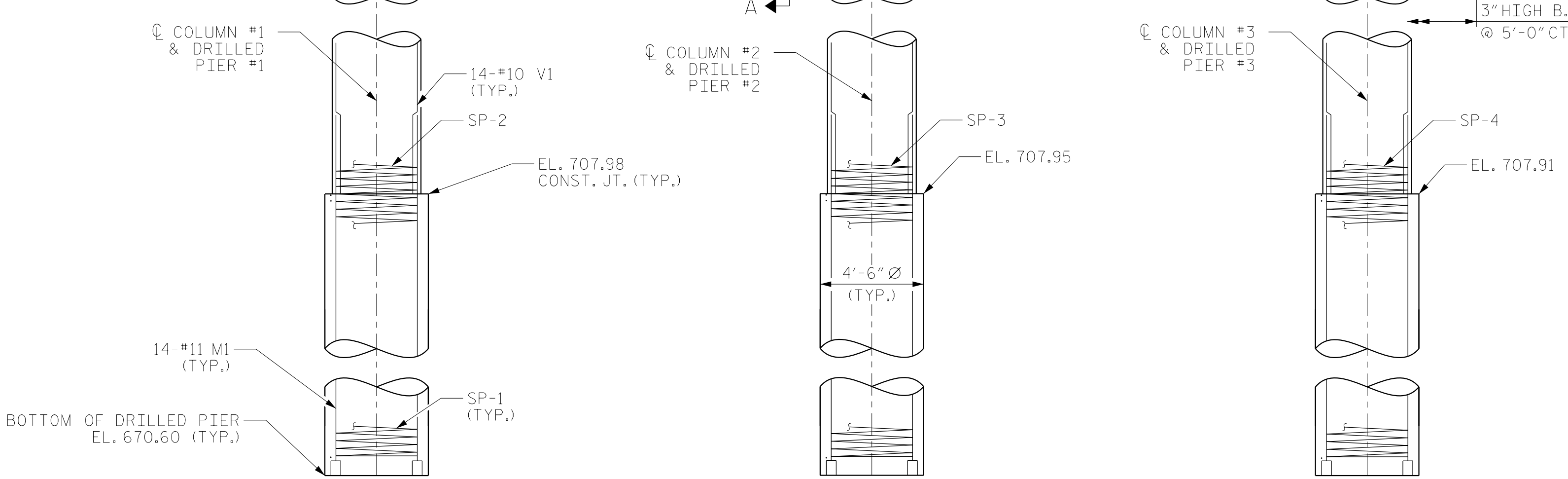
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PLAN



ELEVATION



DETAIL "A"
DIMENSIONS ARE TYPICAL FOR EACH GIRDER

NOTES

- FOR SECTION A-A AND VIEW B-B, SEE SHEET 2 OF 2.
- FOR REINFORCING STEEL BILL OF MATERIAL, SEE SHEET 2 OF 2.
- STIRRUPS AND UI BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.
- HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.
- ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL."
- ★ INVERT ALTERNATE #6 S1 STIRRUP PAIRS.
- THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.
- THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT 1 FOOT BELOW THE GROUND LINE.
- RLGL = RIGHT LANE GRADE LINE

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 1 OF 2



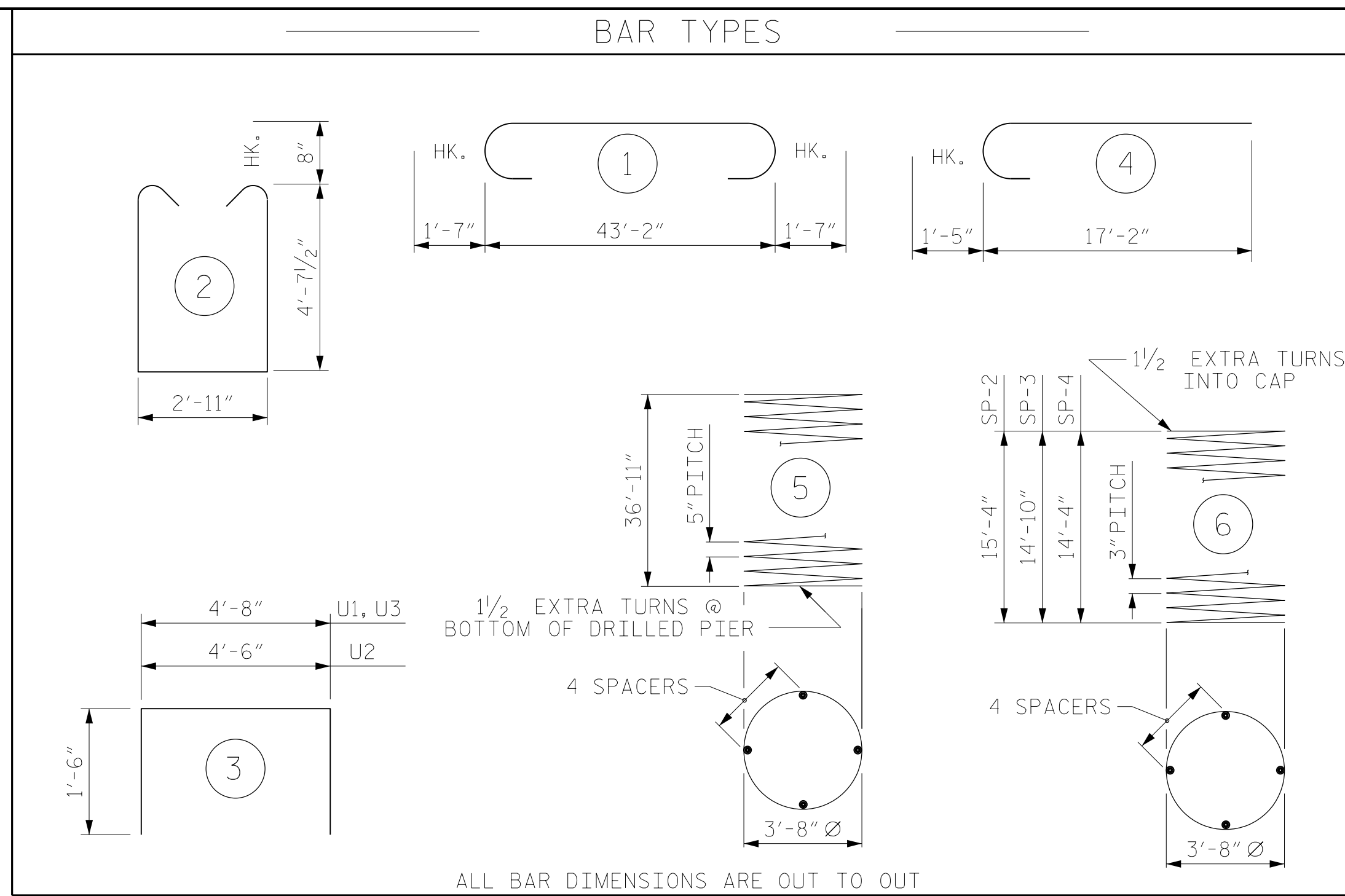
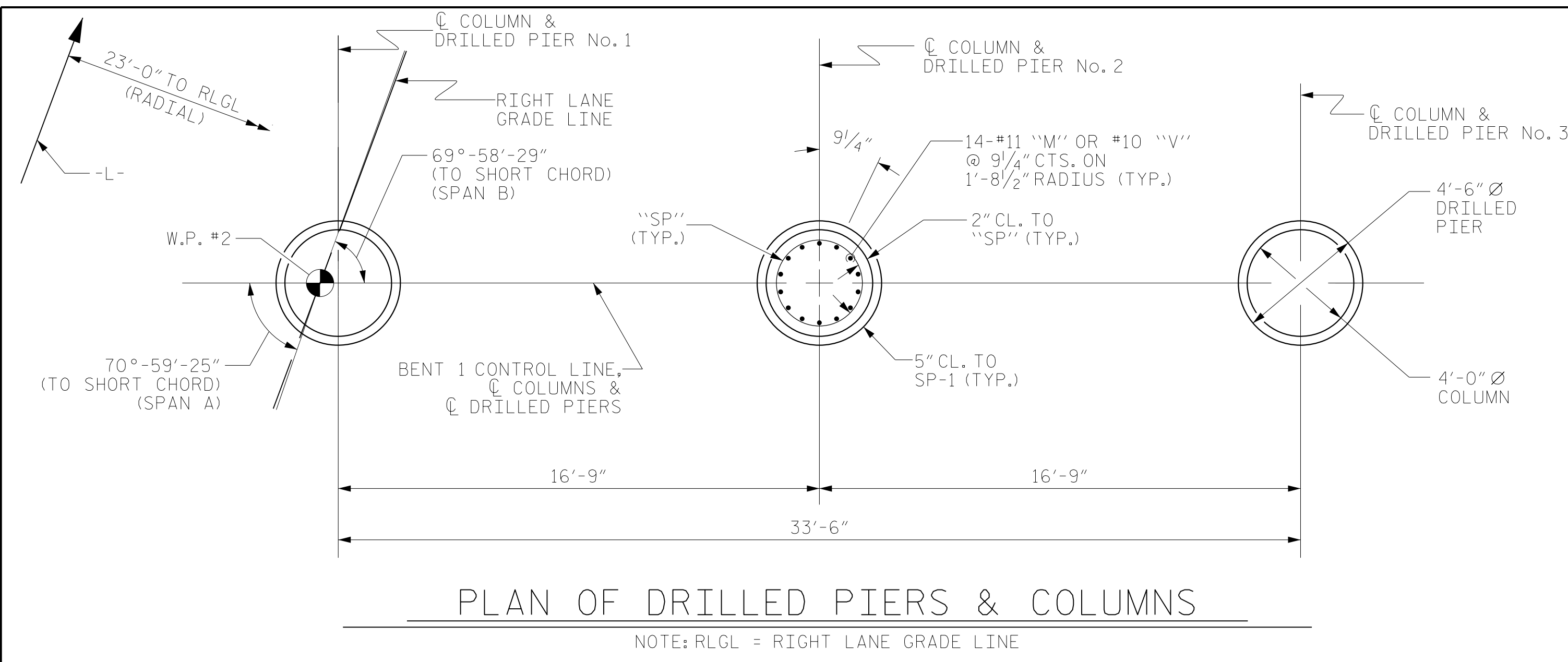
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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 BENT NO. 1
 RIGHT LANE

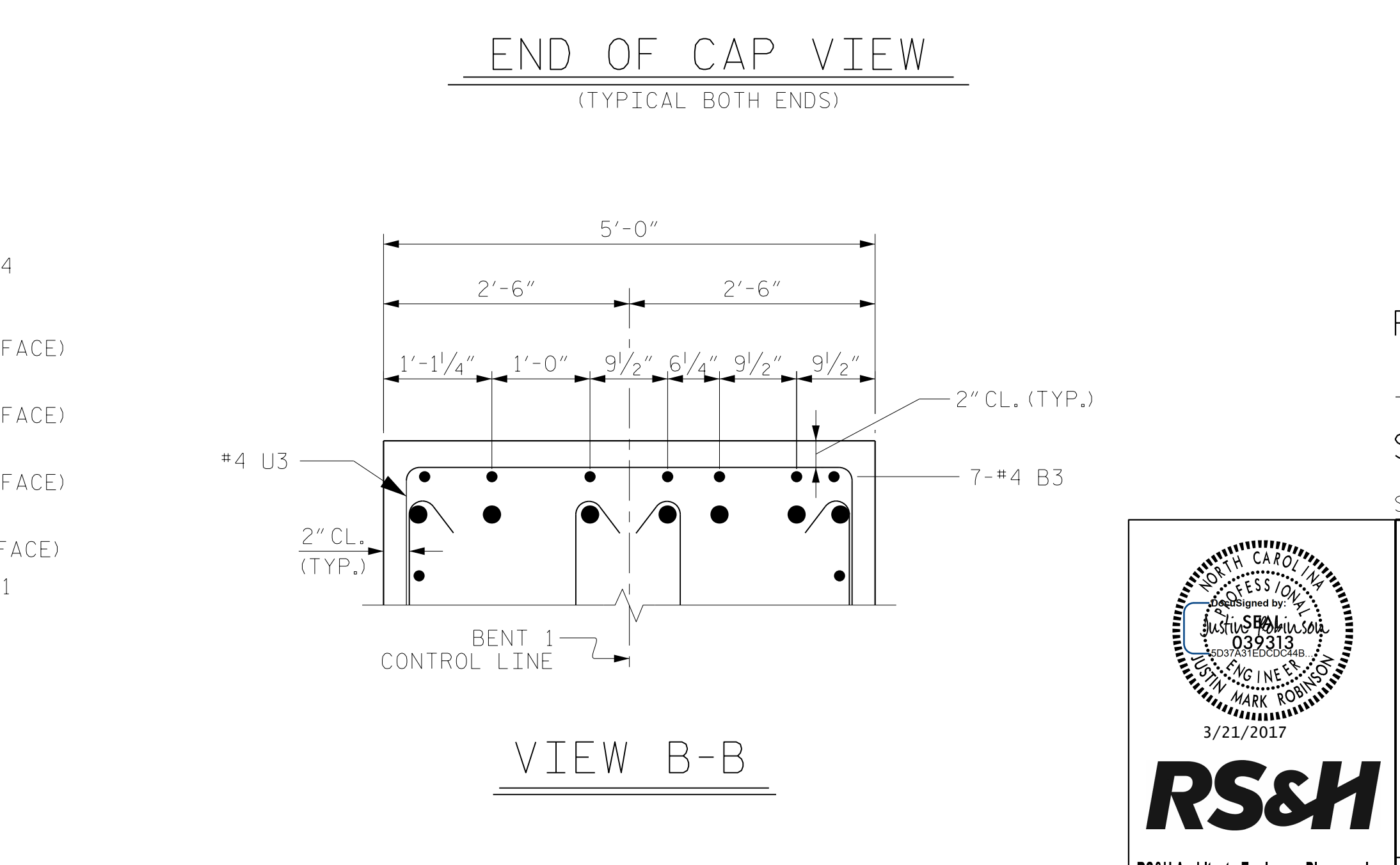
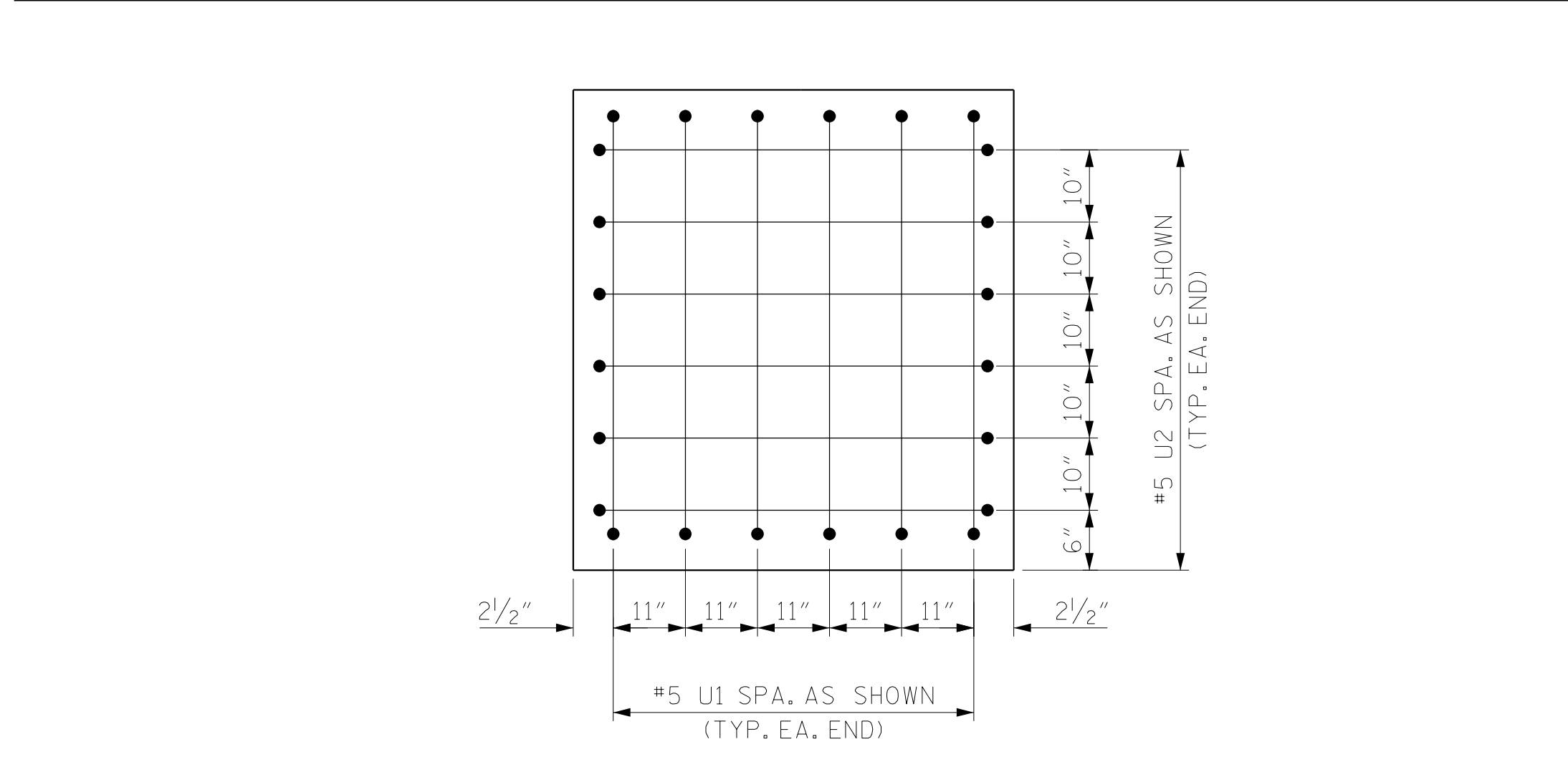
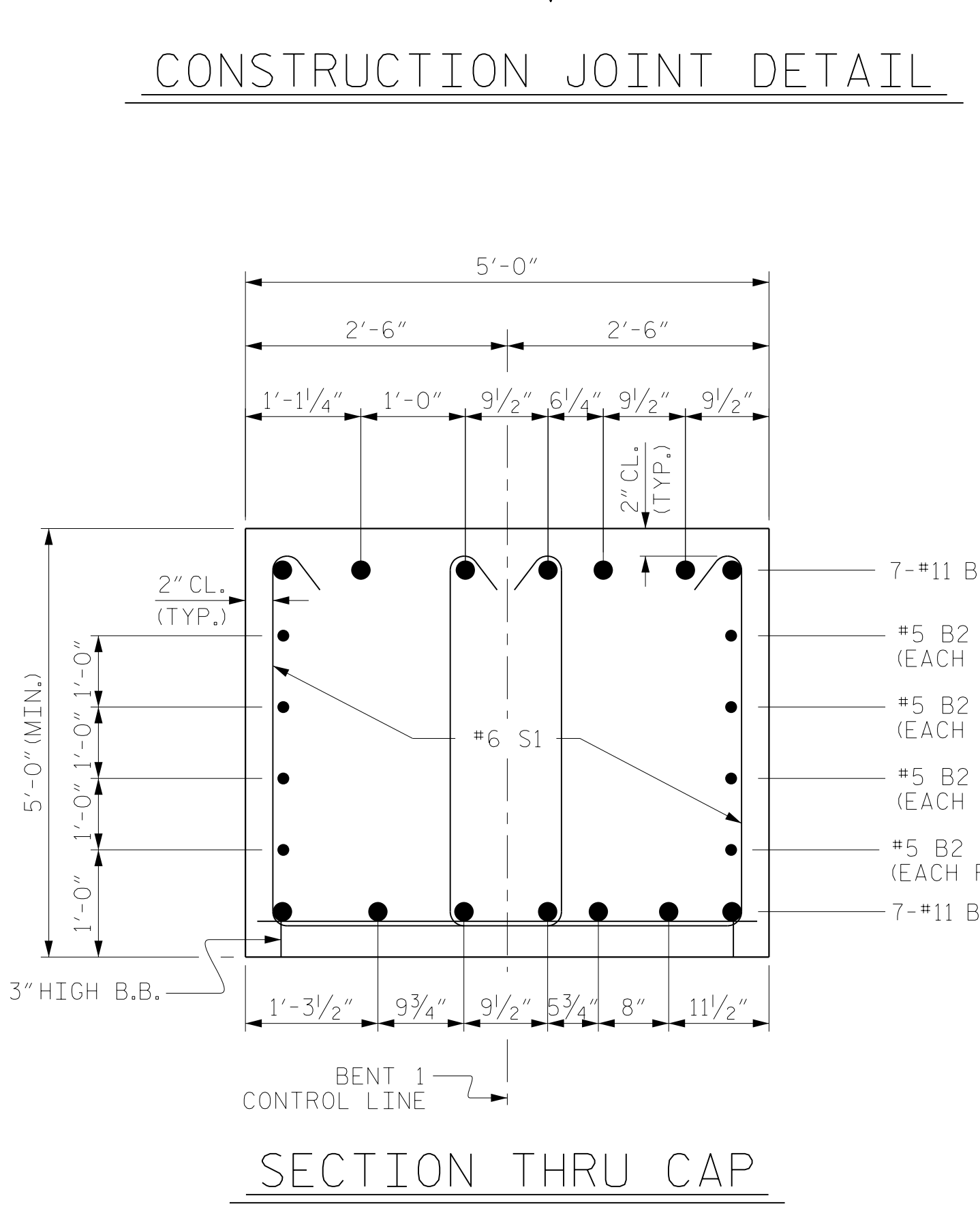
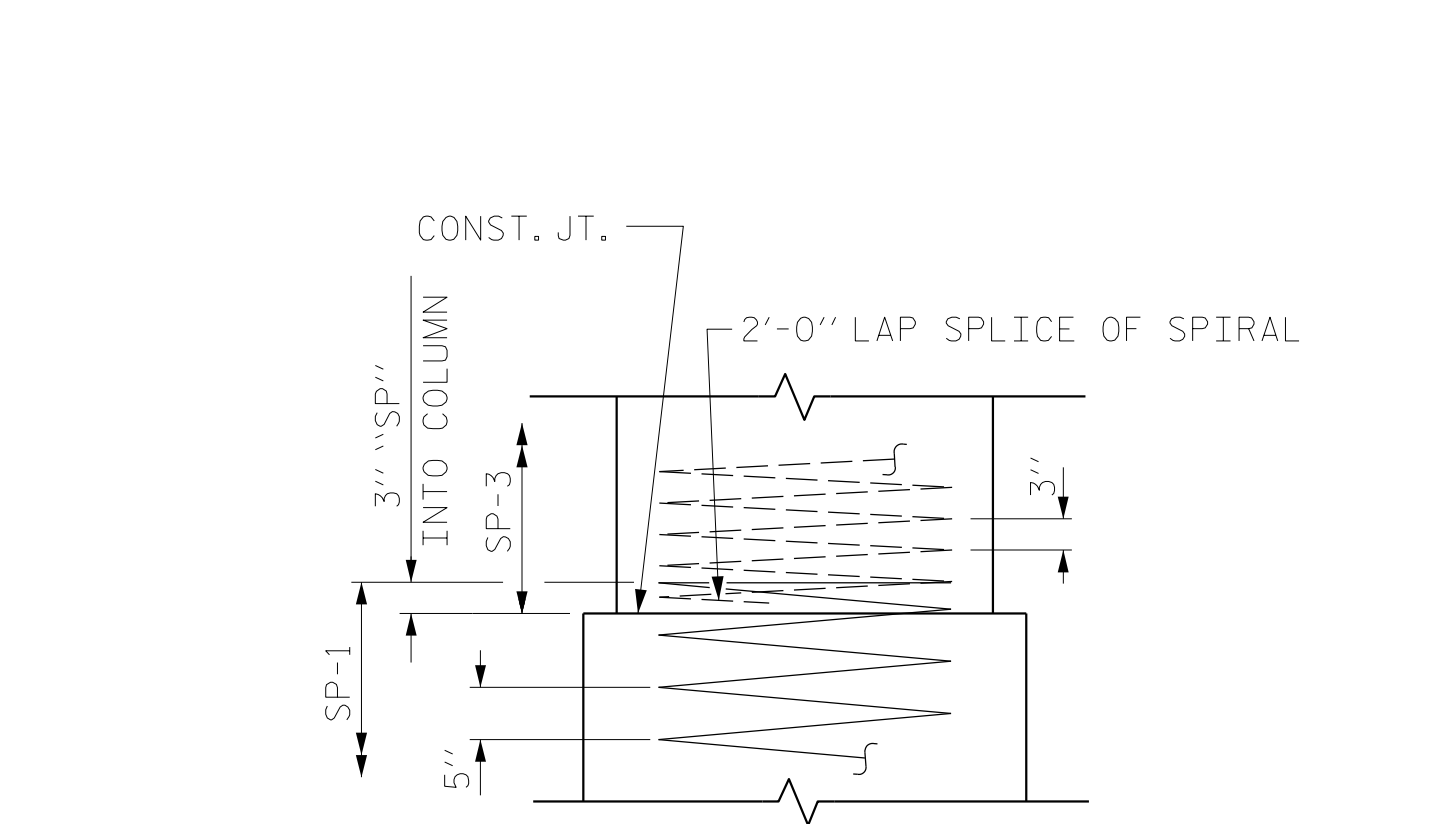
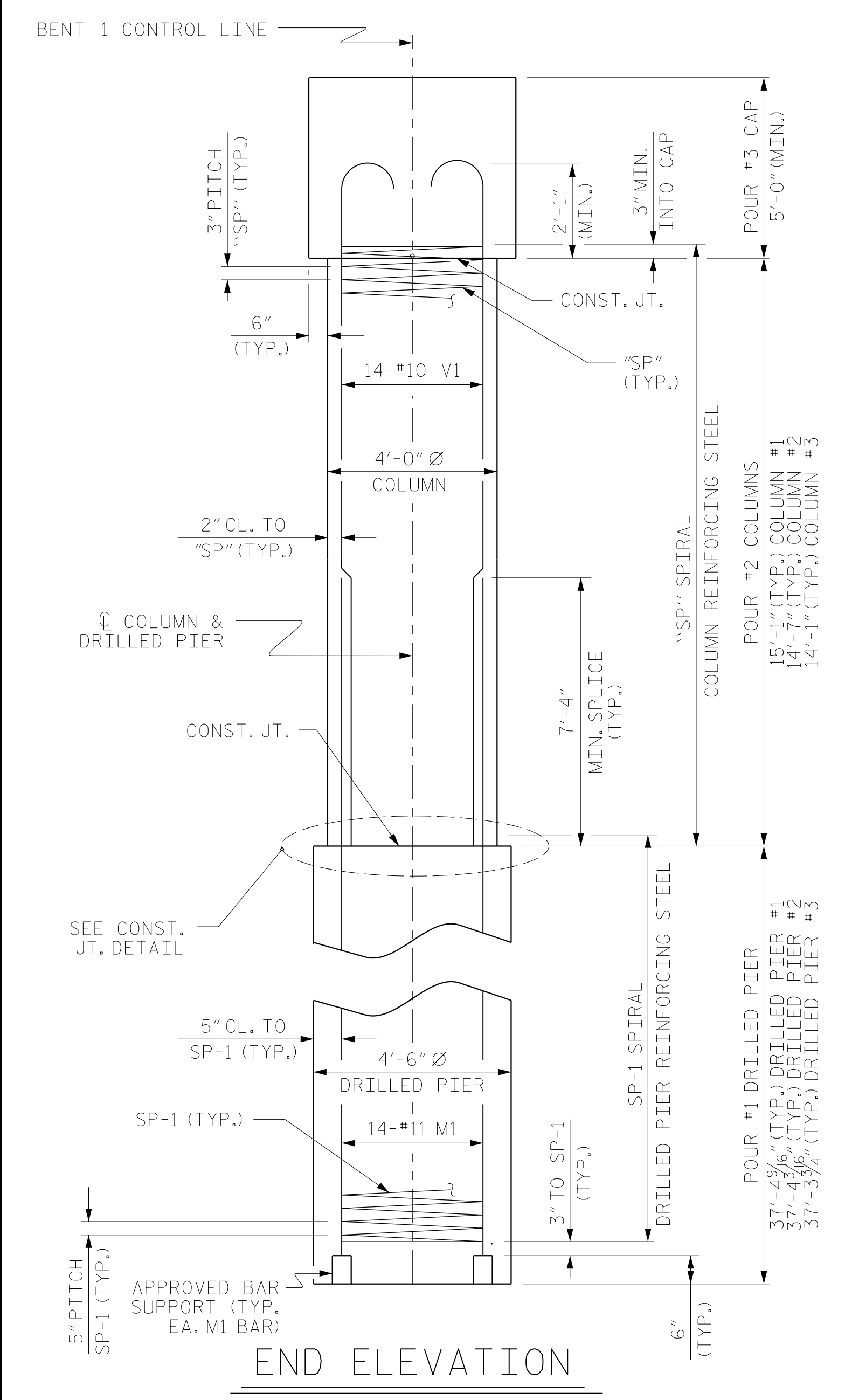
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-27
1			3			TOTAL SHEETS
2			4			36

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BILL OF MATERIAL FOR BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	7	#11	STR	43'-2"	1,605
B2	8	#5	STR	43'-2"	360
B3	28	#4	STR	4'-8"	87
B4	7	#11	1	46'-4"	1,723
M1	42	#11	STR	47'-3"	10,544
S1	100	#6	2	13'-6"	2,028
U1	12	#5	3	7'-8"	96
U2	12	#5	3	7'-6"	94
U3	36	#4	3	7'-8"	184
V1	42	#10	4	18'-7"	3,358
REINFORCING STEEL (FOR BENT 1)					20,079 LBS.
SP-1	3	**	5	1025'-7"	3,209
SP-2	1	*	6	717'-8"	479
SP-3	1	*	6	694'-11"	464
SP-4	1	*	6	672'-1"	449



SPIRAL COLUMN REINFORCING STEEL (FOR BENT 1)		4,601 LBS.
** THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR		
* THE SP-2, SP-3 & SP-4 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR		
CLASS A CONCRETE BREAKDOWN (FOR BENT 1)		
POUR #2 (COLUMNS)		20.4 C.Y.
POUR #3 (CAP)		42.1 C.Y.
TOTAL CLASS A CONCRETE		62.5 C.Y.
DRILLED PIERS: (FOR BENT 1)		
DRILLED PIER CONCRETE POUR #1 (DRILLED PIERS)		66.0 C.Y.
4'-6" Ø DRILLED PIER NOT IN SOIL		27.0 LIN. FT.
4'-6" Ø DRILLED PIER IN SOIL		85.0 LIN. FT.
PERMANENT STEEL CASING FOR 4'-6" Ø DRILLED PIERS		54 LIN. FT.
Δ CSL TUBES		467.0 LIN. FT.

Δ NO SEPARATE PAYMENT WILL BE MADE FOR CSL TUBES. CSL TUBES WILL BE INCLUDED IN THE UNIT BID PRICE FOR DRILLED PIERS.

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 2 OF 2



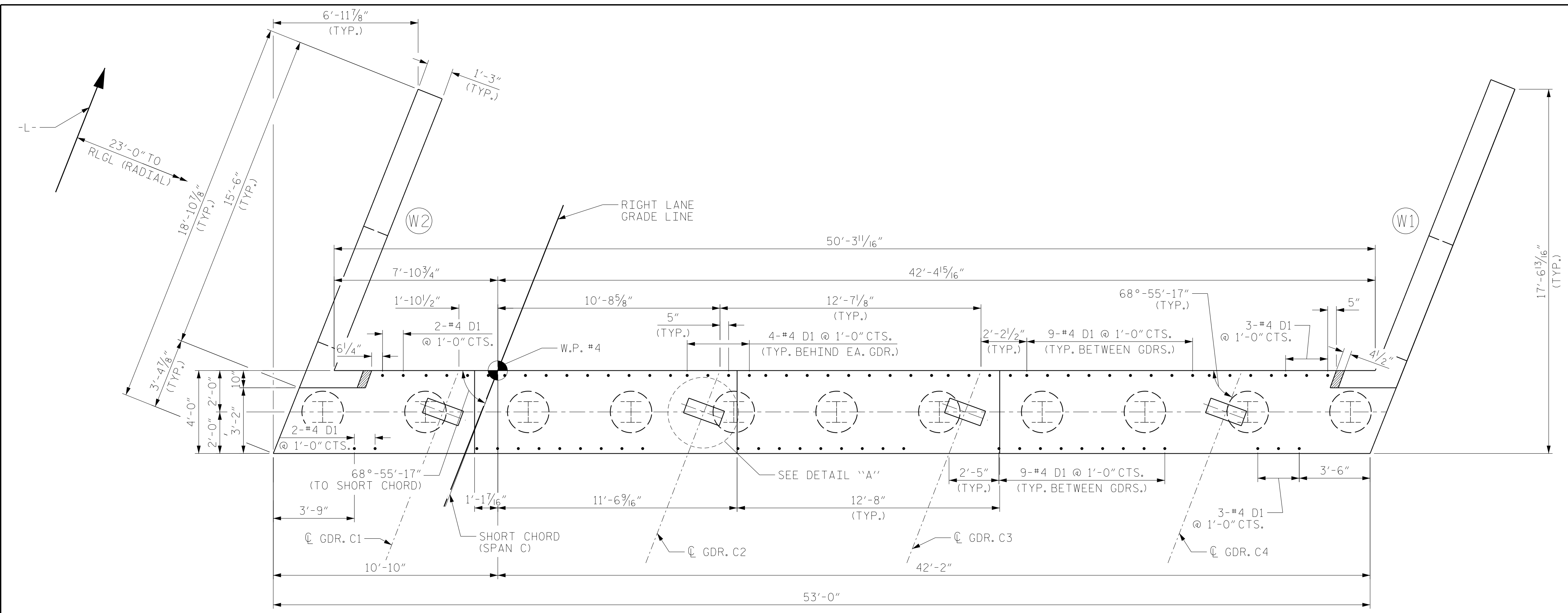
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE BENT NO. 1 DETAILS RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

DRAWN BY :	PDS	DATE :	11/2016
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DESIGN ENGINEER OF RECORD:	MAL	DATE :	11/2016

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SHEET NO.	S2-28
TOTAL SHEETS	36



PLAN

NOTES

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS. SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

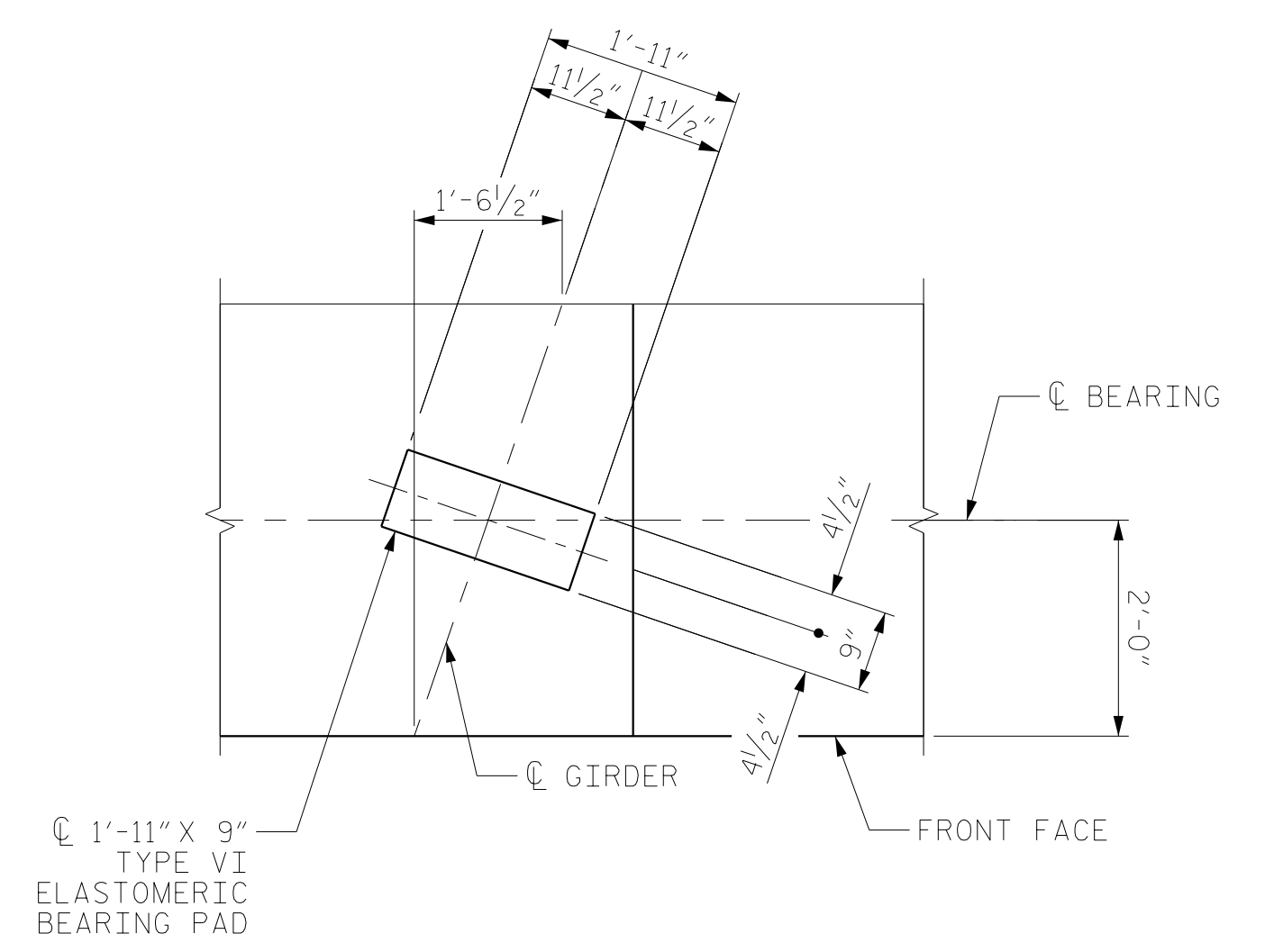
FOR SECTION A-A, SEE SHEET 4 OF 4.

STIRRUPS MAY BE SHIFTED SLIGHTLY TO AVOID #4 D1 BARS IN CAP.

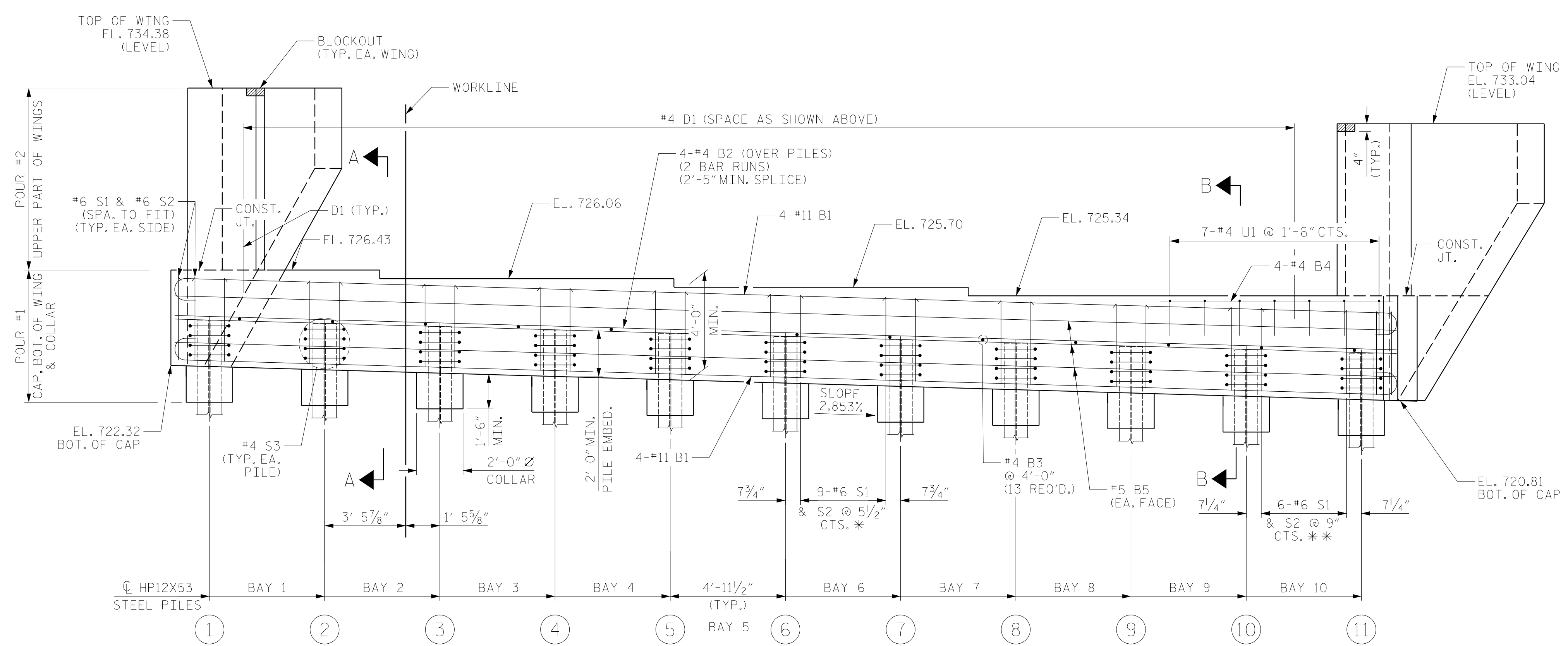
SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.

RLGL = RIGHT LANE GRADE LINE

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.



DETAIL "A-A"
DIMENSIONS TYP. EA. BRG. PILE NOT SHOWN FOR CLARITY



ELEVATION

TOP OF PILE ELEVATIONS	
PILE	ELEVATION
1	724.28
2	724.14
3	724.00
4	723.85
5	723.71
6	723.57
7	723.43
8	723.28
9	723.14
10	723.00
11	722.85

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 384+50.00 -L-
SHEET 1 OF 4



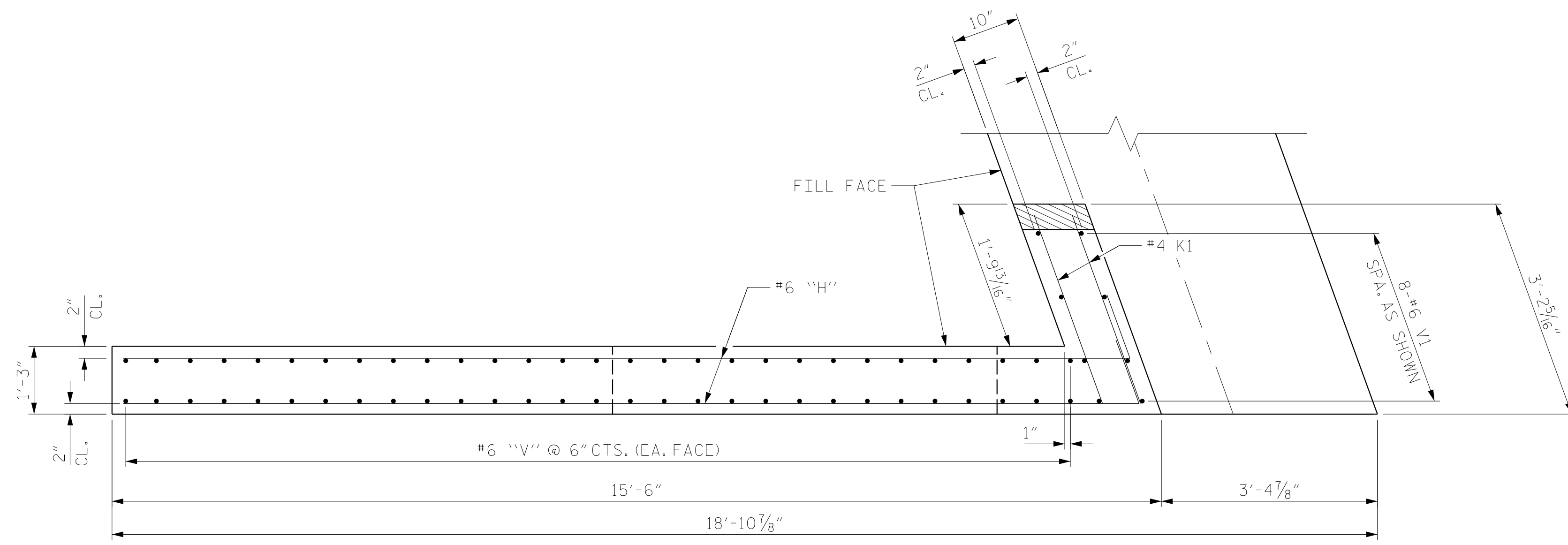
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE
INTEGRAL
END BENT NO. 2
RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-31
1			3			TOTAL SHEETS
2			4			36

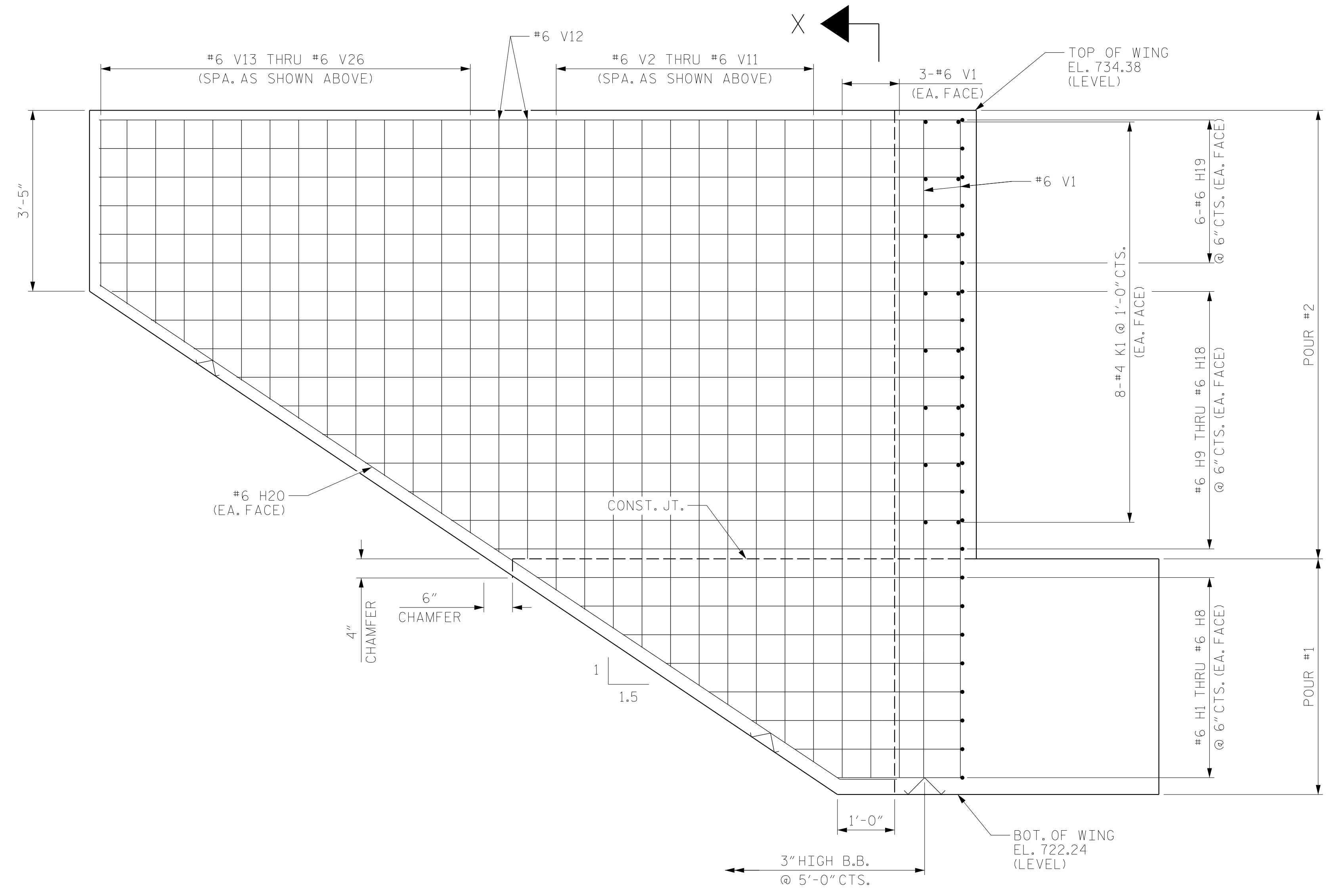
DRAWN BY : PDS DATE : 11/2016
CHECKED BY : TLC DATE : 01/2017
DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

* = TYPICAL BAY 4 THRU BAY 7
** = TYPICAL BAY 1 THRU BAY 3 AND BAY 8 THRU BAY 10

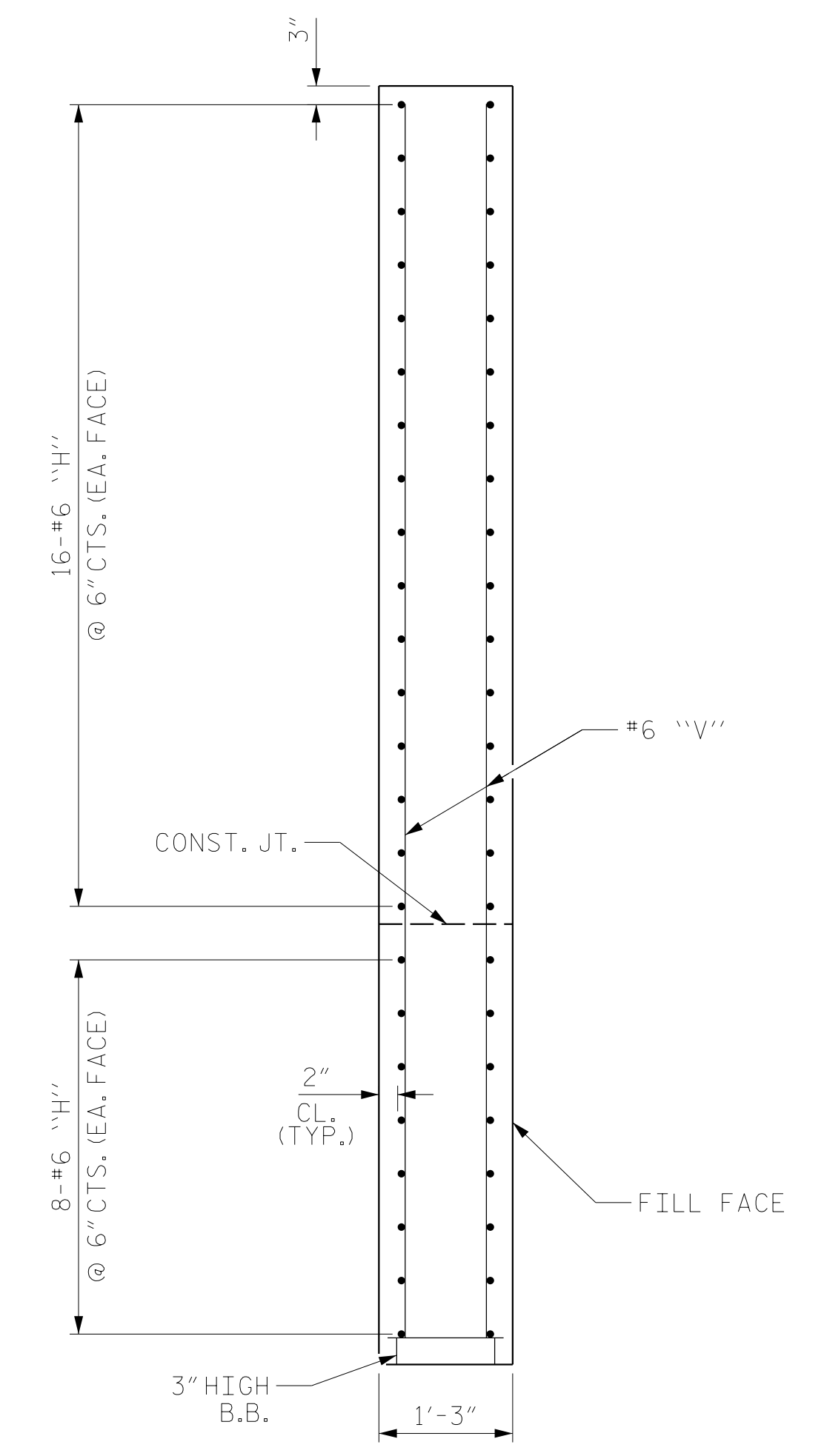
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



PLAN OF WING - W2



ELEVATION OF WING - W2



SECTION X-X

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-
 SHEET 2 OF 4

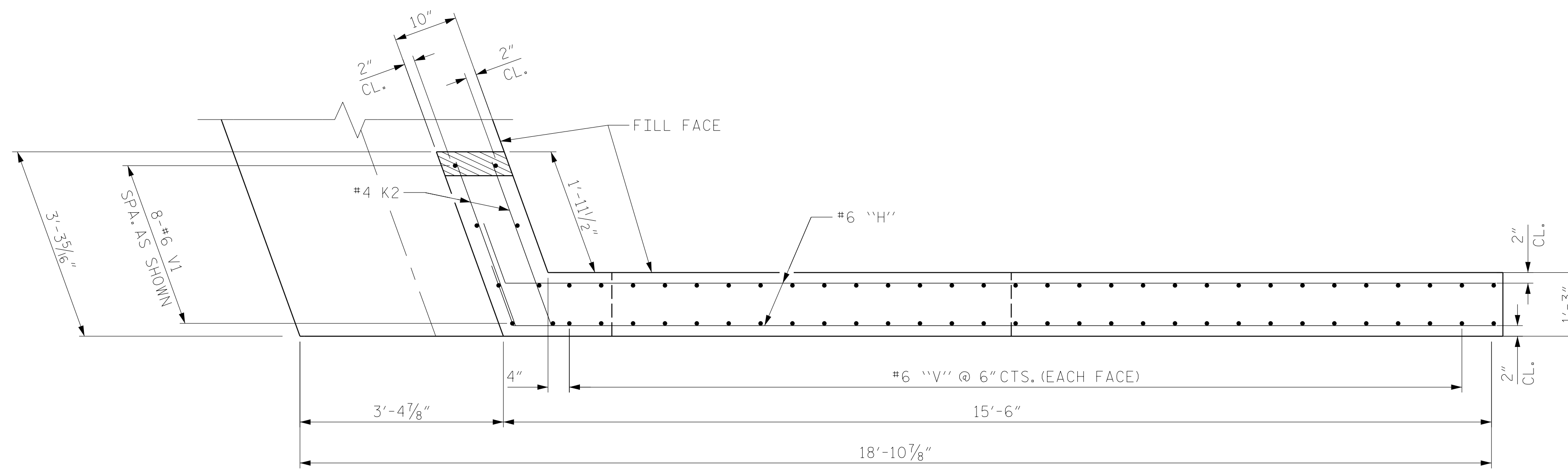


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL
 END BENT NO. 2
 RIGHT LANE

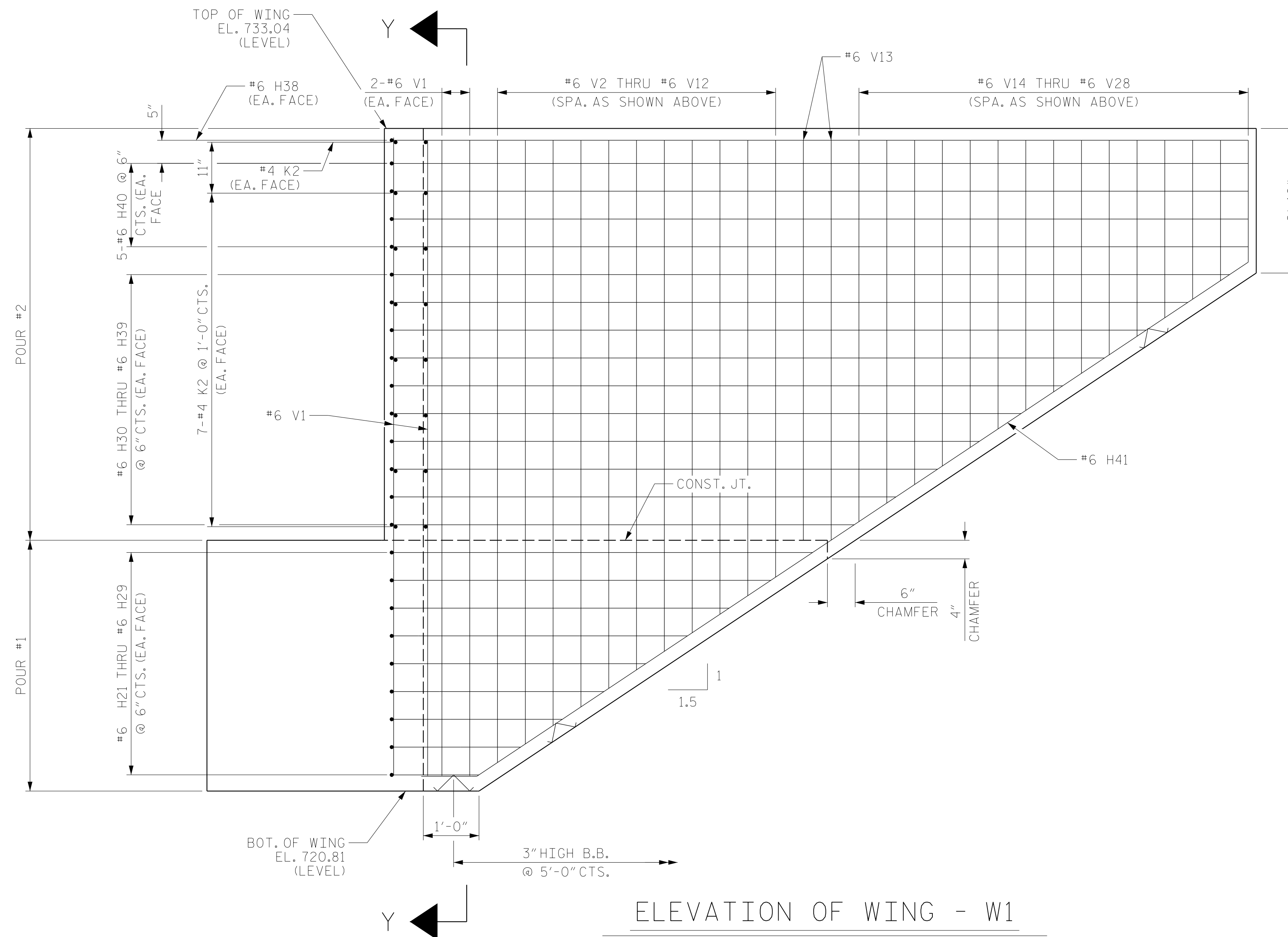
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			36
2			4			

DRAWN BY : PDS DATE : 11/2016
 CHECKED BY : TLC DATE : 01/2017
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

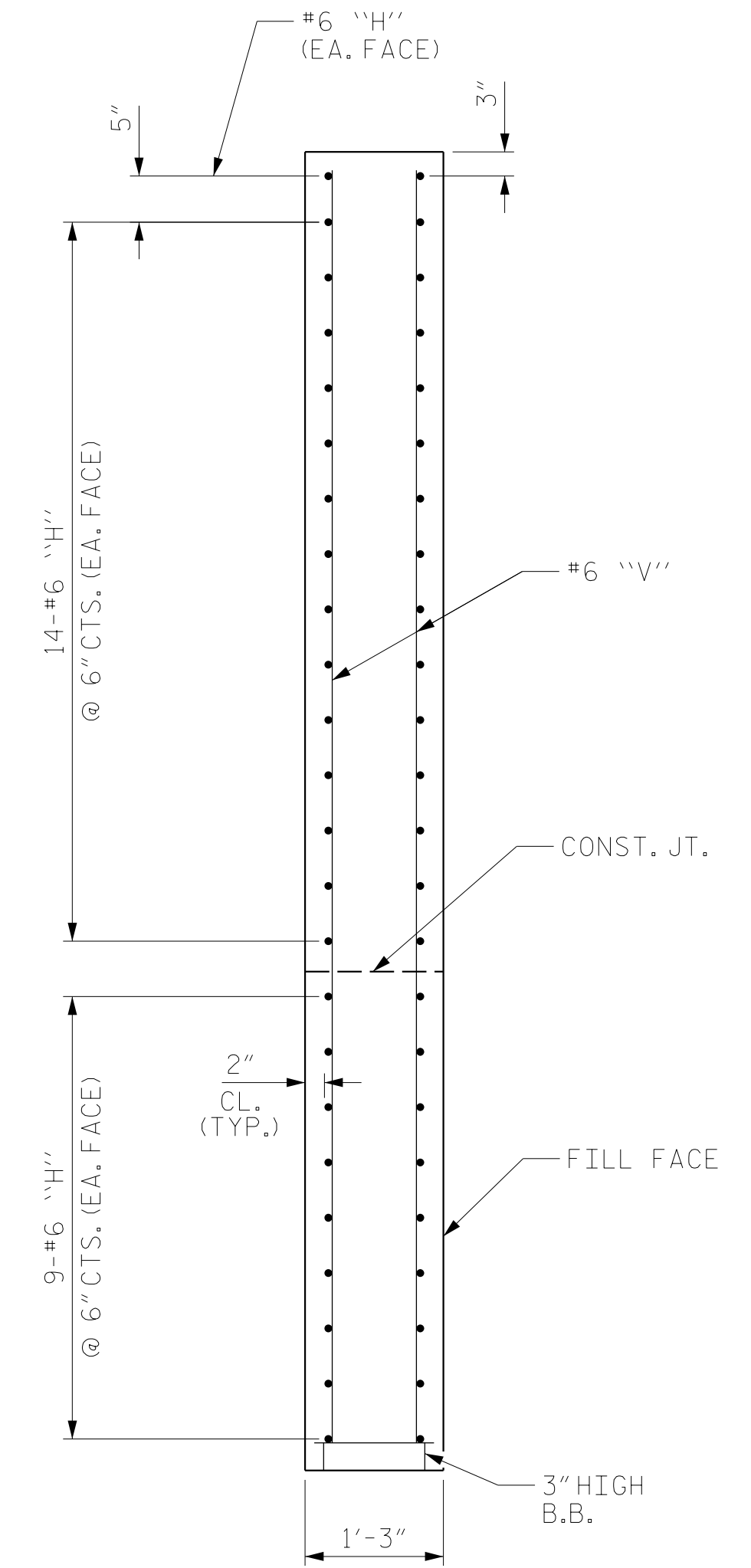
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PLAN OF WING - W1



ELEVATION OF WING - W1



SECTION Y-Y

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

SHEET 3 OF 4



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 INTEGRAL
 END BENT NO. 2
 RIGHT LANE

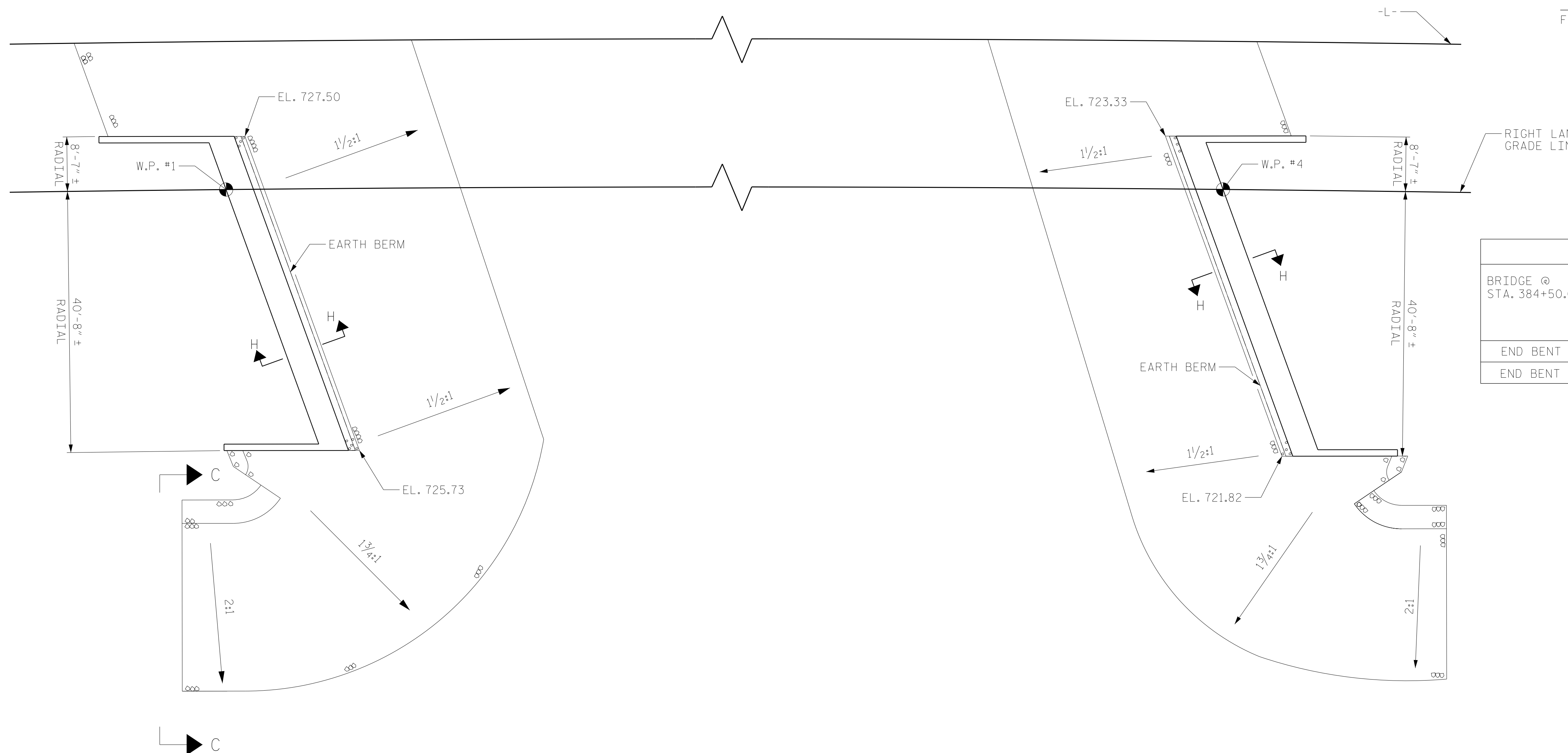
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			36
2			4			

DRAWN BY : PDS DATE : 11/2016
 CHECKED BY : TLC DATE : 01/2017
 DESIGN ENGINEER OF RECORD: MAL DATE : 11/2016

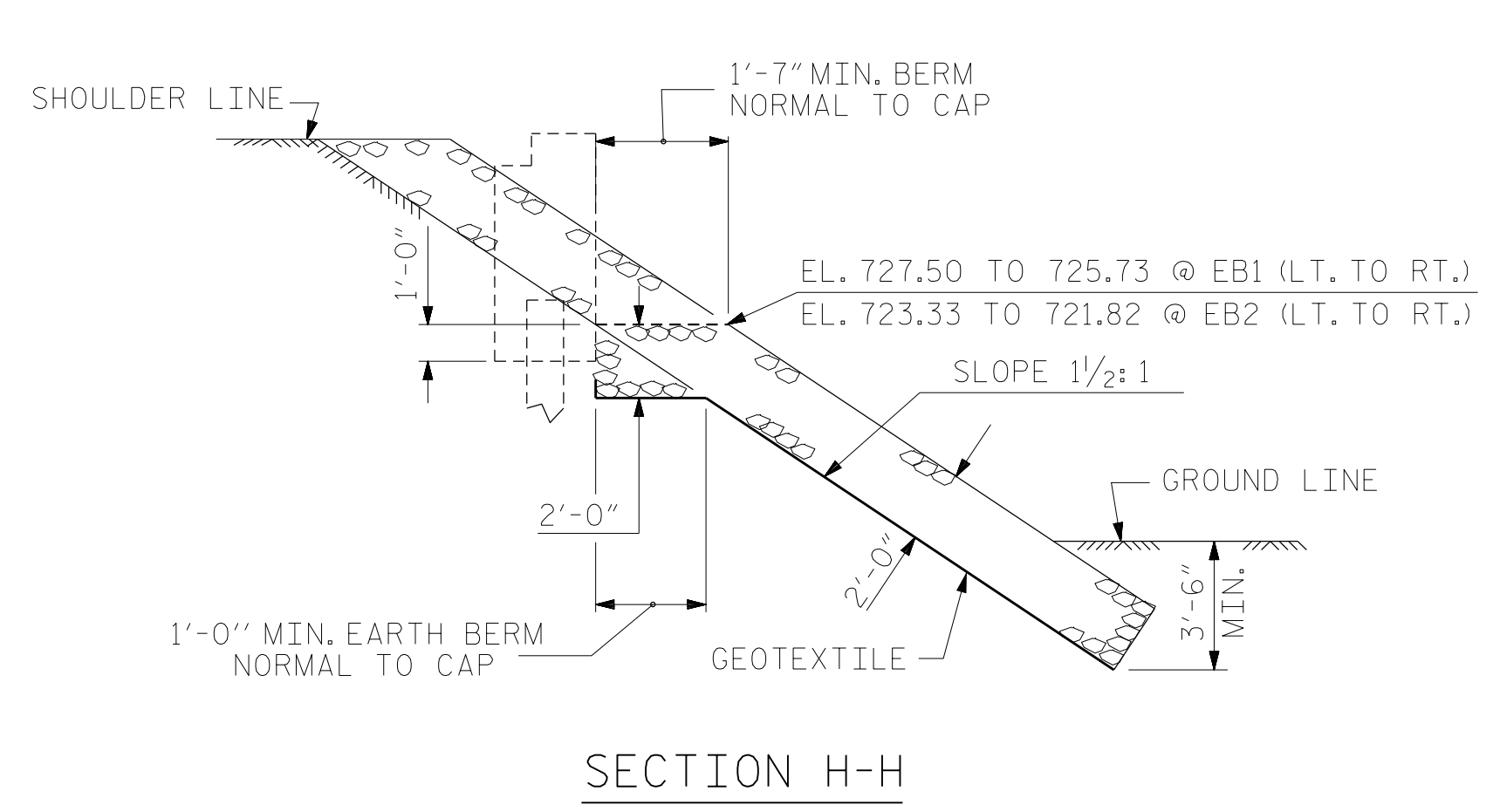
DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

NOTES

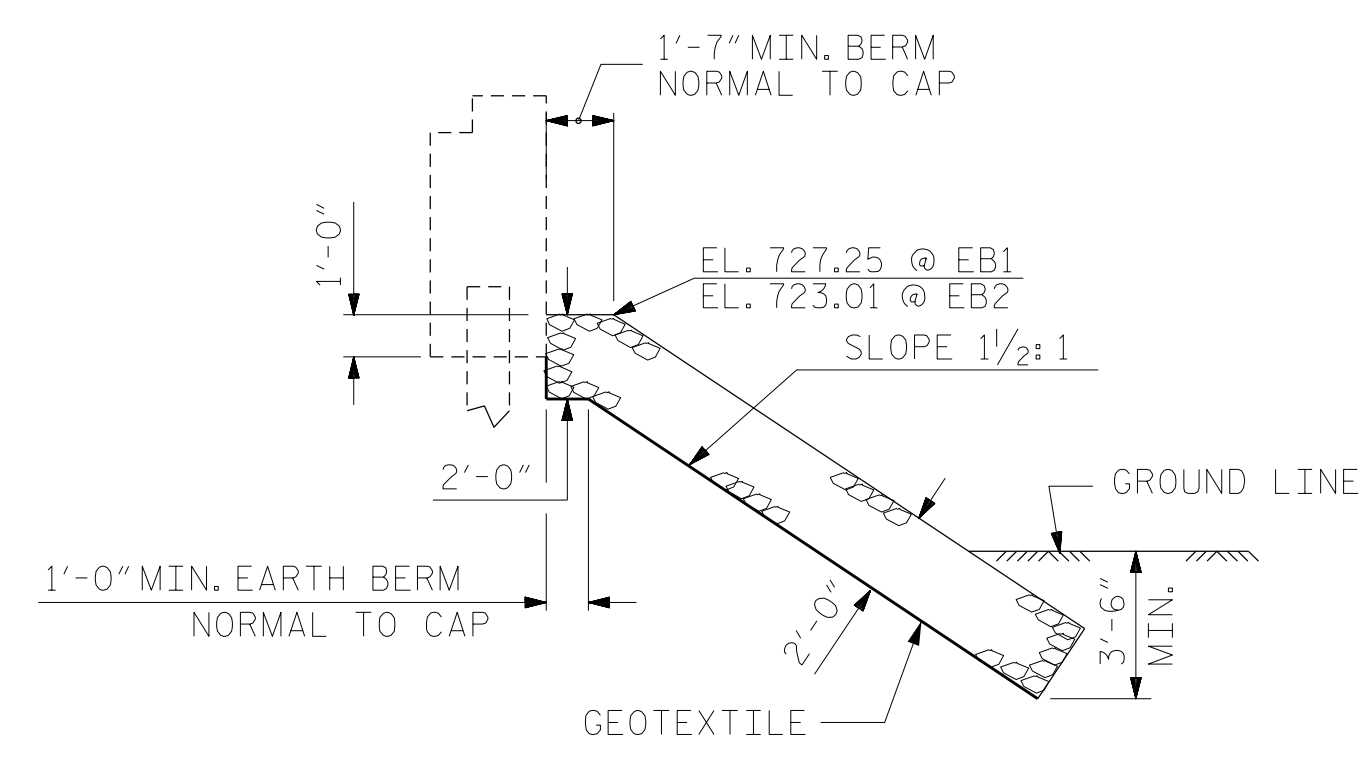
FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING.



ESTIMATED QUANTITIES		
BRIDGE @ STA. 384+50.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE
	TONS	SQUARE YARDS
END BENT 1	500	550
END BENT 2	450	500

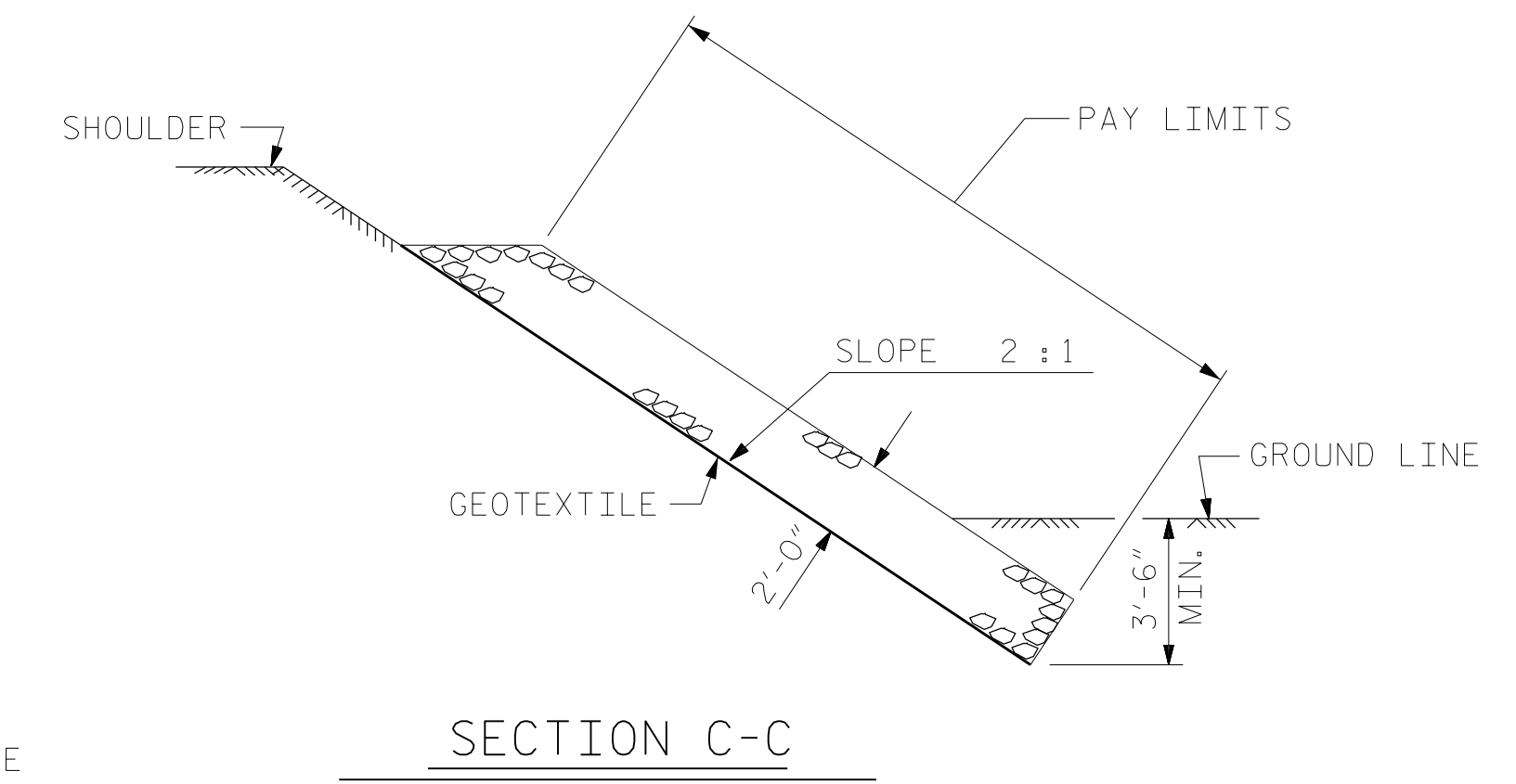


SECTION H-H



RIGHT LANE GRADE LINE SECTION

BERM RIP RAPPED



SECTION C-C

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 384+50.00 -L-

3/9/2017

RS&H

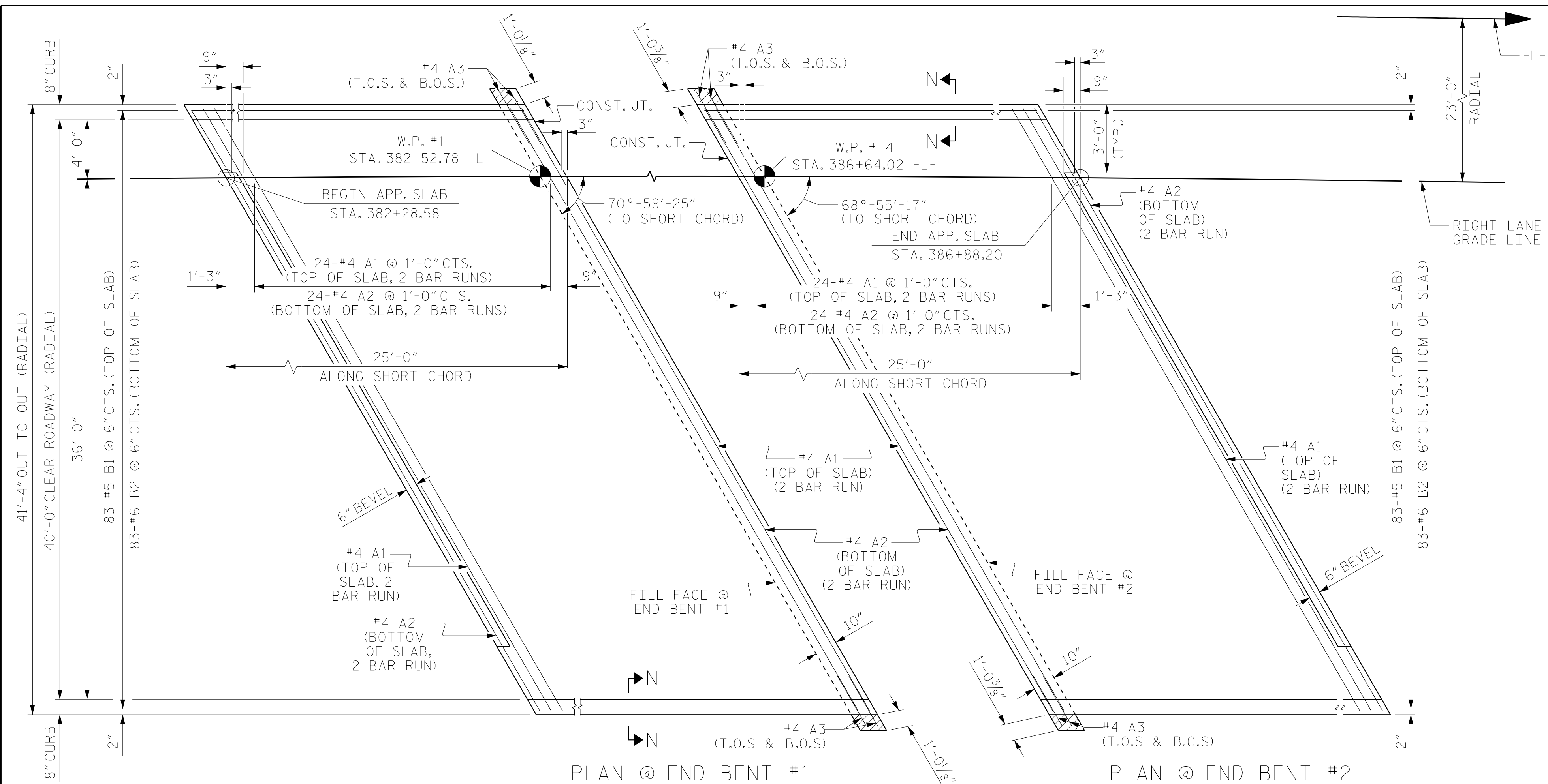
RS&H Architects-Engineers-Planners, Inc.
 8601 Six Forks Road, Suite 260
 Raleigh, NC 27615
 919-826-4100 FAX 919-846-9080
 www.rsandh.com
 North Carolina License No. 50737-F-0403-C-03

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
RIP RAP DETAILS					
RIGHT LANE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO.	S2-35
TOTAL SHEETS	36

DRAWN BY :	PDS	DATE :	11/2016
CHECKED BY :	TLC	DATE :	01/2017
DESIGN ENGINEER OF RECORD:	MAL	DATE :	11/2016

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PLAN @ END BENT #1 PLAN @ END BENT #2

DIMENSIONS SHOWN ARE TYPICAL FOR BOTH APPROACH SLABS
 CONCRETE TO BE FILLED IN DURING CONSTRUCTION OF APPROACH SLABS

NOTES

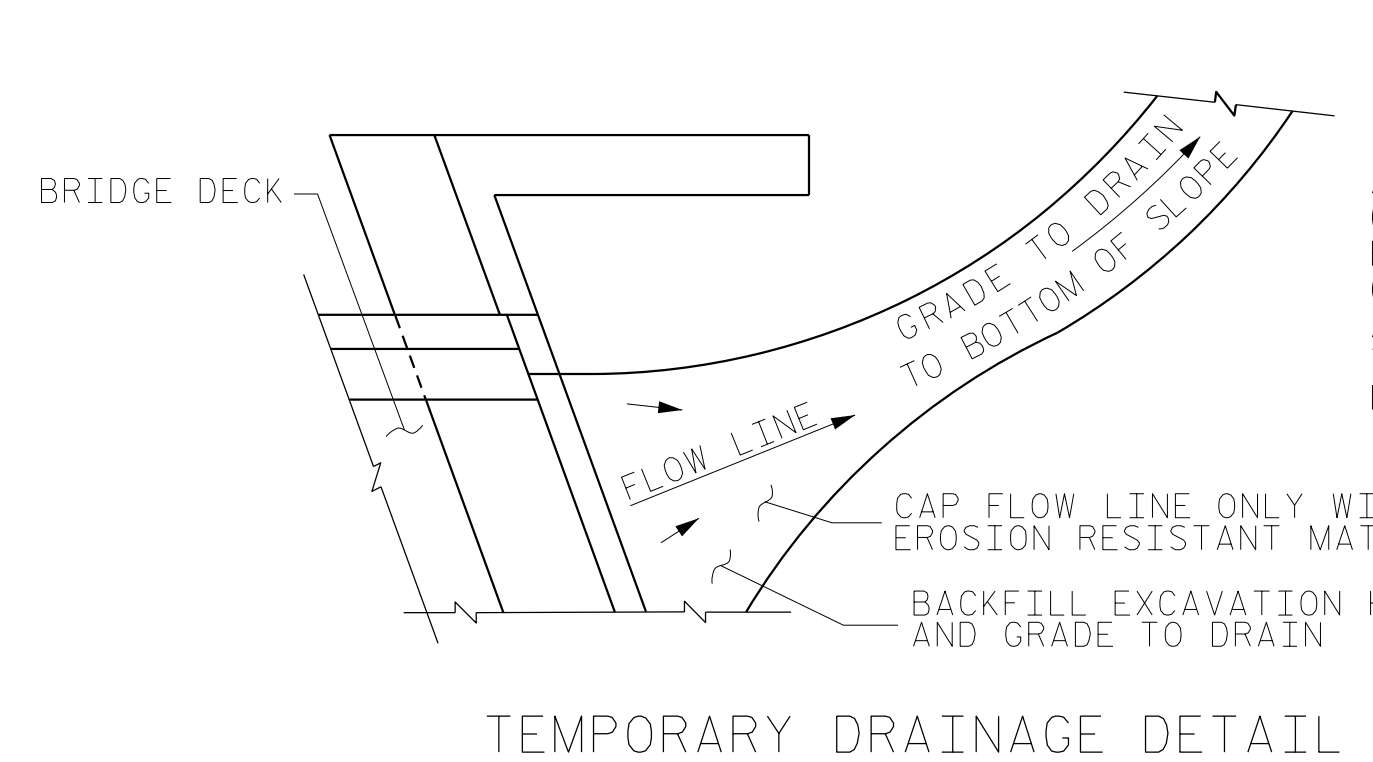
APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.
 FOR REINFORCED BRIDGE APPROACH FILL FABRIC WALL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, WELDED WIRE FORM, AND SELECT MATERIAL, SEE ROADWAY PLANS.
 AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.
 THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.
 ARC OFFSETS ARE NEGLIGIBLE AND THEREFORE NOT SHOWN.
 T.O.S. = TOP OF SLAB
 B.O.S. = BOTTOM OF SLAB

SPlice LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"
#5	2'-6"	2'-2"
#6	3'-10"	2'-7"

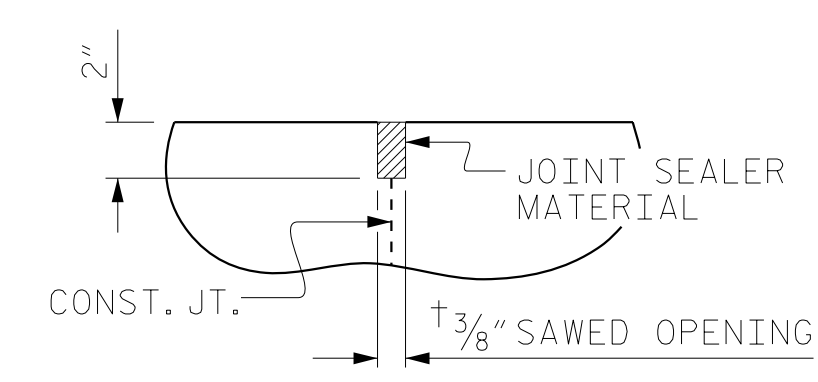
BILL OF MATERIAL					
APPROACH SLAB @ E.B. #1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	52	#4	STR	22'-9"	790
A2	52	#4	STR	22'-7"	784
* A3	8	#4	STR	3'-0"	16
REINFORCING STEEL					3,849 LBS.
* EPOXY COATED REINFORCING STEEL					2,891 LBS.
CLASS AA CONCRETE					45.5 C.Y.
APPROACH SLAB @ E.B. #2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	52	#4	STR	22'-9"	790
A2	52	#4	STR	22'-7"	784
* A3	8	#4	STR	3'-0"	16
REINFORCING STEEL					3,849 LBS.
* EPOXY COATED REINFORCING STEEL					2,891 LBS.
CLASS AA CONCRETE					45.5 C.Y.



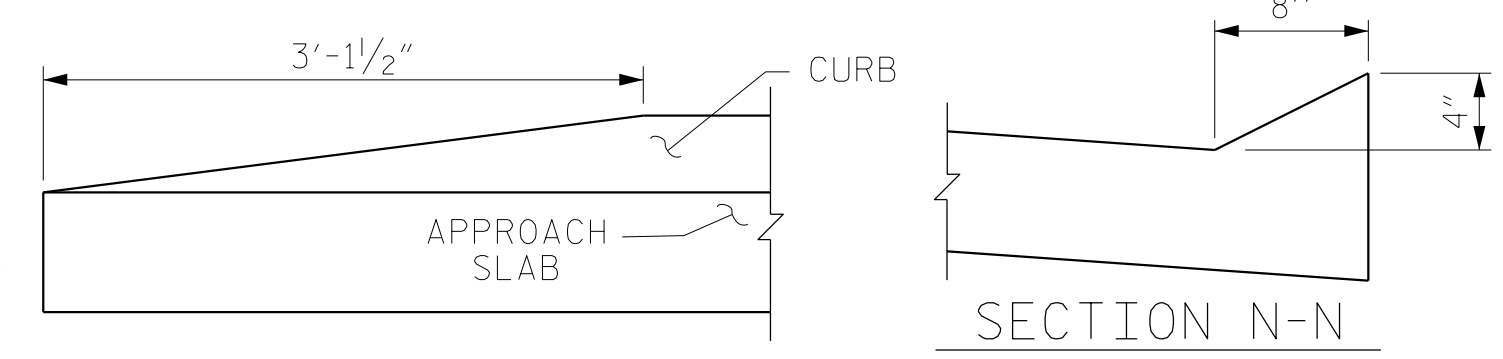
SECTION THRU SLAB



NOTE:
 IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.



DETAIL "A"



SECTION N-N

END OF CURB WITHOUT SHOULDER BERM GUTTER

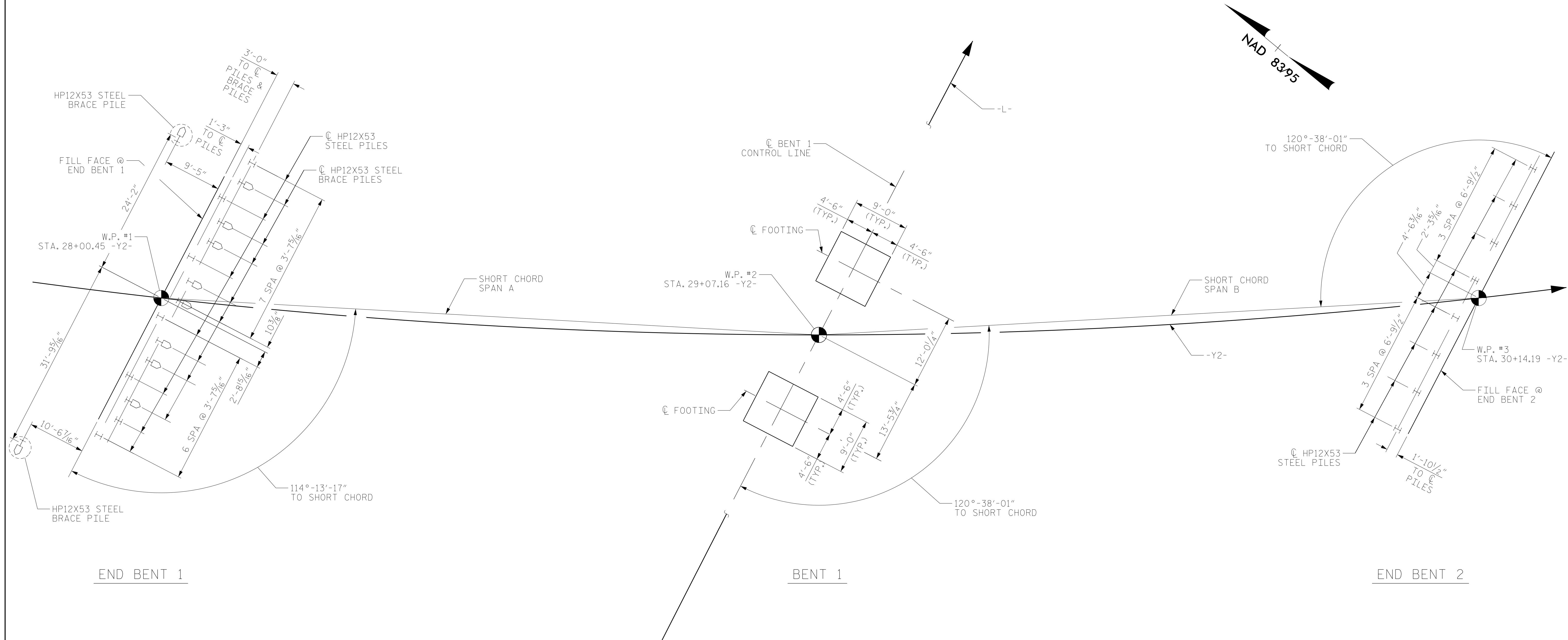
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 384+50.00 -L-

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 BRIDGE APPROACH SLAB
 FOR INTEGRAL ABUTMENT
 RIGHT LANE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S2-36
1			3			TOTAL SHEETS
2			4			36

ASSEMBLED BY :	PDS	DATE :	11/2016
CHECKED BY :	TLC	DATE :	01/2017
DRAWN BY :	TLA 10/05	REV. 10/1/11	MAA/GM
CHECKED BY :	GM 5/06	REV. 12/21/11	MAA/GM
		REV. 6/13	MAA/GM



NOTES:

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.

DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.

DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.

PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.

THE SPREAD FOOTING AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 36 TSF. CHECK FIELD CONDITIONS FOR THE REQUIRED RESISTANCE OF 80 TSF JUST BEFORE PLACING CONCRETE.

STEEL H-PILES POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT NO. 1. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

DRILLED-IN PILES ARE REQUIRED FOR END BENT NO.2. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 731.5 FT. FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

CONCRETE IS REQUIRED TO FILL HOLES FOR PILE EXCAVATION AT END BENT NO. 2.

FOUNDATION LAYOUT

DIMENSIONS TO PILES ARE SHOWN TO PILE CENTERLINE.
BRACE PILES AT END BENTS SHALL BE BATTERED AT 3:12.

DRAWN BY : MKO DATE : 12/2016
 CHECKED BY : JMR DATE : 12/2016
 DESIGN ENGINEER OF RECORD: MKO DATE : 12/2016

2/28/2017
 R:\Structures\Working DGN\403.003.R2707C.SMU.GD02.S3-2.dgn
 oconnor

DOCUMENT NOT CONSIDERED
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 SIGNATURES COMPLETED

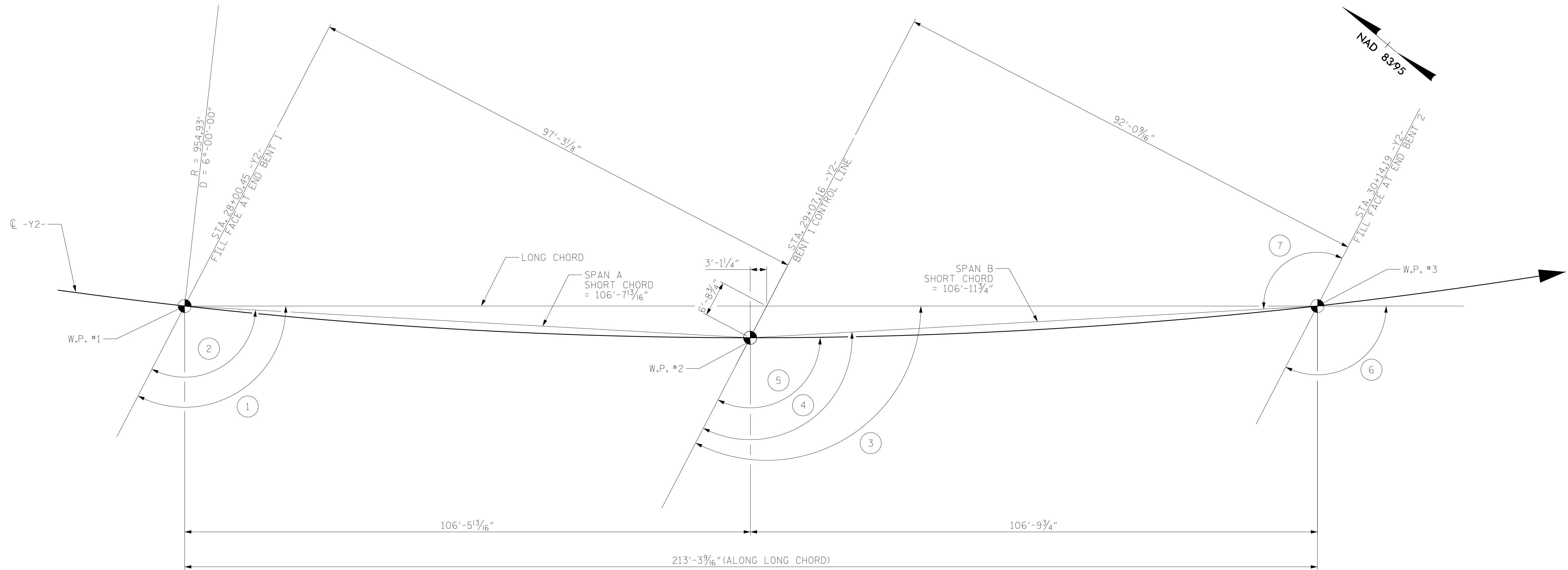


PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 2 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
GENERAL DRAWING
 BRIDGE OVER US 74
 (SHELBY BYPASS)
 ON SR 1850 BETWEEN
 SR 1344 AND SR 1827

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			S3-2
2			4			TOTAL SHEETS 36



ANGLES

- ① 117°-25'-57"
- ② 114°-13'-17"
- ③ 117°-25'-57"
- ④ 120°-38'-01"
- ⑤ 117°-25'-22" TC
- ⑥ 117°-25'-57"
- ⑦ 120°-38'-01"

HORIZONTAL CURVE DATA -Y2-

P.I. STA. = 31+27.15
 Δ = 96°-06'-18.7" (LT)
 D = 6°-00'-00.0"
 L = 1601.75'
 T = 1062.52'
 R = 954.93'
 SE = .06

LONG CHORD LAYOUT

NOTE: ALL BENTS ARE PARALLEL

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 3 OF 4



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

GENERAL DRAWING

BRIDGE OVER US 74
 (SHELBY BYPASS)
 ON SR 1850 BETWEEN
 SR 1344 AND SR 1827

REVISIONS

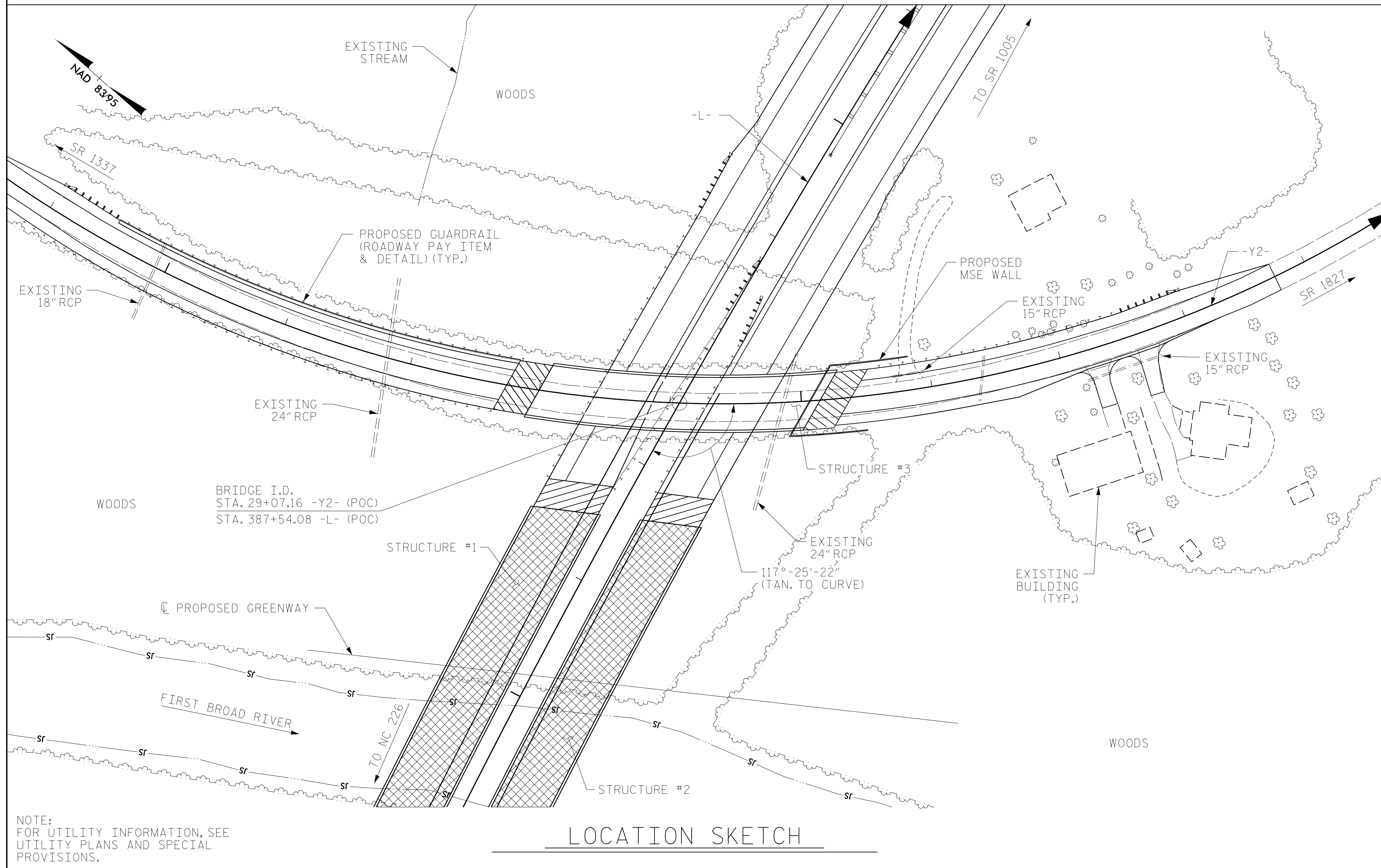
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO.	TOTAL SHEETS
S3-3	36

DRAWN BY : MKO DATE : 07/2016
 CHECKED BY : JMR DATE : 07/2016
 DESIGN ENGINEER OF RECORD: MKO DATE : 12/2016

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

BENCH MARK #18: 8 INCH NAIL IN BASE OF 20 INCH WHITE OAK 28.12' LEFT OF STA. 30+98.41 -Y2-, EL. 778.74'



NOTE:
FOR UTILITY INFORMATION, SEE
UTILITY PLANS AND SPECIAL
PROVISIONS.

NOTES:

- ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
- THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
- FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.
- FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
- THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

LOCATION SKETCH

TOTAL BILL OF MATERIAL

	FOUNDATION EXCAVATION FOR BENT 1	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	MODIFIED 72" PRESTRESSED CONCRETE GIRDERS	PILE EXCAVATION NOT IN SOIL	HP 12X53 STEEL PILES	STEEL PILE POINTS	PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES	VERTICAL CONCRETE BARRIER RAIL	4" SLOPE PROTECTION	ELASTOMERIC BEARINGS	FOAM JOINT SEALS
	LUMP SUM	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	LBS.	NO. LIN. FT.	LIN. FT.	NO. LIN. FT.	EACH	EA.	LIN. FT.	SQ. YDS.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE		8,927	9,522		LUMP SUM			10 1,043.38					422.9		LUMP SUM	LUMP SUM
END BENT 1				63.9		8,796				17 510	17	17		143		
BENT 1	LUMP SUM			85.5		13,408	1,307									
END BENT 2				44.1		7,226			80	8 240		8		175		
TOTAL	LUMP SUM	8,927	9,522	193.5	LUMP SUM	29,430	1,307	10 1,043.38	80	25 750	17	25	422.9	318	LUMP SUM	LUMP SUM

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-
 SHEET 4 OF 4



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

GENERAL DRAWING

BRIDGE OVER US 74
 (SHELBY BYPASS)
 ON SR 1850 BETWEEN
 SR 1344 AND SR 1827

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-4
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : MKO DATE : 12/2016
 CHECKED BY : JMR DATE : 01/2017
 DESIGN ENGINEER OF RECORD : MKO DATE : 12/2016

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ_{DC}	γ_{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE								SERVICE III LIMIT STATE								COMMENT NUMBER		
						MOMENT				SHEAR				MOMENT										
						LIVE-LOAD FACTORS (γ_{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FF)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FF)	LIVE-LOAD FACTORS (γ_{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION		DISTANCE FROM LEFT END OF SPAN (FF)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	①	1.12	-	1.75	0.99	1.15	A	ER	51.26	1.09	1.38	B	EL	30.75	0.80	0.99	1.12	A	ER	51.26		
	HL-93 (OPERATING)	N/A		1.49	-	1.35	0.99	1.49	A	ER	51.26	1.09	1.90	B	EL	103.44	N/A	-	-	-	-	-	-	
	HS-20 (INVENTORY)	36.000	②	1.57	56.52	1.75	0.99	1.61	A	ER	51.26	1.09	2.02	B	EL	103.44	0.80	0.99	1.57	A	ER	51.26		
	HS-20 (OPERATING)	36.000		2.09	75.24	1.35	0.99	2.09	A	ER	51.26	1.09	2.62	B	EL	103.44	N/A	-	-	-	-	-	-	
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500		3.74	50.49	1.40	0.99	4.8	A	ER	51.26	1.09	6.30	B	EL	103.44	0.80	0.99	3.74	A	ER	51.26	
		SNGARBS2	20.000		2.7	5.40	1.40	0.99	3.47	A	ER	51.26	1.09	4.39	B	EL	103.44	0.80	0.99	2.7	A	ER	51.26	
		SNAGRIS2	22.000		2.52	55.44	1.40	0.99	3.24	A	ER	51.26	1.09	4.04	B	EL	103.44	0.80	0.99	2.52	A	ER	51.26	
		SNCOTTS3	27.250		1.86	50.69	1.40	0.99	2.39	A	ER	51.26	1.09	3.14	B	EL	103.44	0.80	0.99	1.86	A	ER	51.26	
		SNAGGRS4	34.925		1.52	53.09	1.40	0.99	1.95	A	ER	51.26	1.09	2.54	B	EL	103.44	0.80	0.99	1.52	A	ER	51.26	
		SNS5A	35.550		1.49	52.97	1.40	0.99	1.91	A	ER	51.26	1.09	2.54	B	EL	103.44	0.80	0.99	1.49	A	ER	51.26	
		SNS6A	39.950		1.35	53.93	1.40	0.99	1.74	A	ER	51.26	1.09	2.29	B	EL	103.44	0.80	0.99	1.35	A	ER	51.26	
	SNS7B	42.000		1.29	54.18	1.40	0.99	1.65	A	ER	51.26	1.09	2.22	B	EL	103.44	0.80	0.99	1.29	A	ER	51.26		
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33.000		1.65	54.45	1.40	0.99	2.11	A	ER	51.26	1.09	2.75	B	EL	103.44	0.80	0.99	1.65	A	ER	51.26	
		TNT4A	33.075		1.65	54.57	1.40	0.99	2.12	A	ER	51.26	1.09	2.71	B	EL	103.44	0.80	0.99	1.65	A	ER	51.26	
		TNT6A	41.600		1.34	55.74	1.40	0.99	1.71	A	ER	51.26	1.09	2.31	B	EL	103.44	0.80	0.99	1.34	A	ER	51.26	
		TNT7A	42.000		1.34	56.28	1.40	0.99	1.71	A	ER	51.26	1.09	2.28	B	EL	103.44	0.80	0.99	1.34	A	ER	51.26	
		TNT7B	42.000		1.38	57.96	1.40	0.99	1.77	A	ER	51.26	1.09	2.20	B	EL	103.44	0.80	0.99	1.38	A	ER	51.26	
		TNAGRIT4	43.000		1.31	56.33	1.40	0.99	1.68	A	ER	51.26	1.09	2.13	B	EL	103.44	0.80	0.99	1.31	A	ER	51.26	
TNAGT5A		45.000		1.24	55.80	1.40	0.99	1.59	A	ER	51.26	1.09	2.08	B	EL	103.44	0.80	0.99	1.24	A	ER	51.26		
TNAGT5B	45.000	③	1.23	55.35	1.40	0.99	1.58	A	ER	51.26	1.09	2.03	B	EL	103.44	0.80	0.99	1.23	A	ER	51.26			

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.
ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- 1.
- 2.
- 3.
- 4.

⊕ CONTROLLING LOAD RATING

① DESIGN LOAD RATING (HL-93)

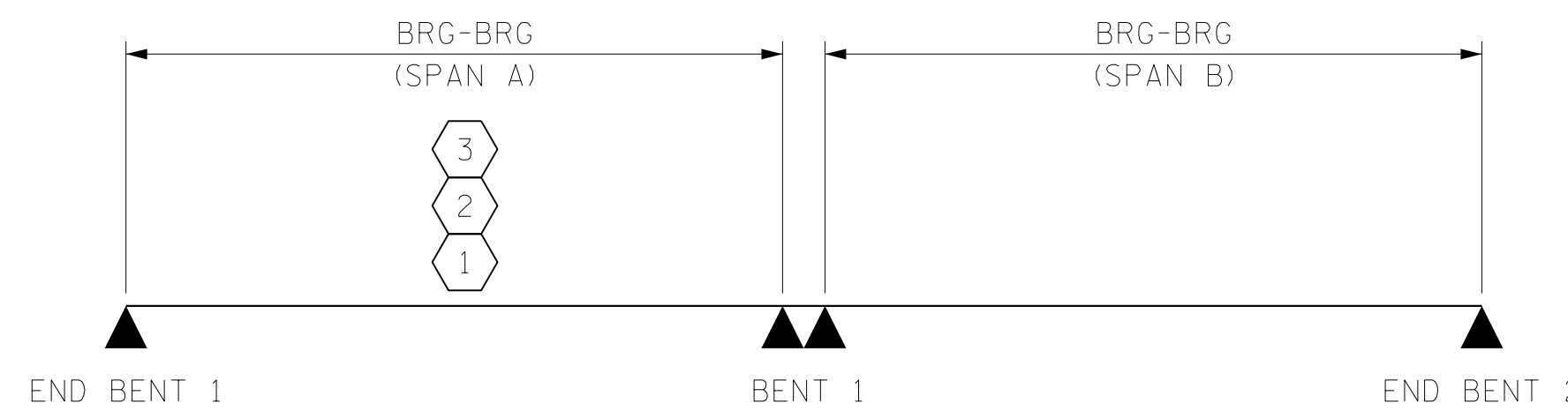
② DESIGN LOAD RATING (HS-20)

③ LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER - EXTERIOR RIGHT GIRDER



GIRDER	SPAN A	SPAN B
1	103'-4 1/4"	103'-6 7/16"
2	103'-1 7/8"	103'-2 5/16"
3	102'-11 9/16"	102'-10 9/16"
4	102'-9 5/16"	102'-6 1/2"
5	102'-7 3/16"	102'-2 13/16"

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

LRFR SUMMARY

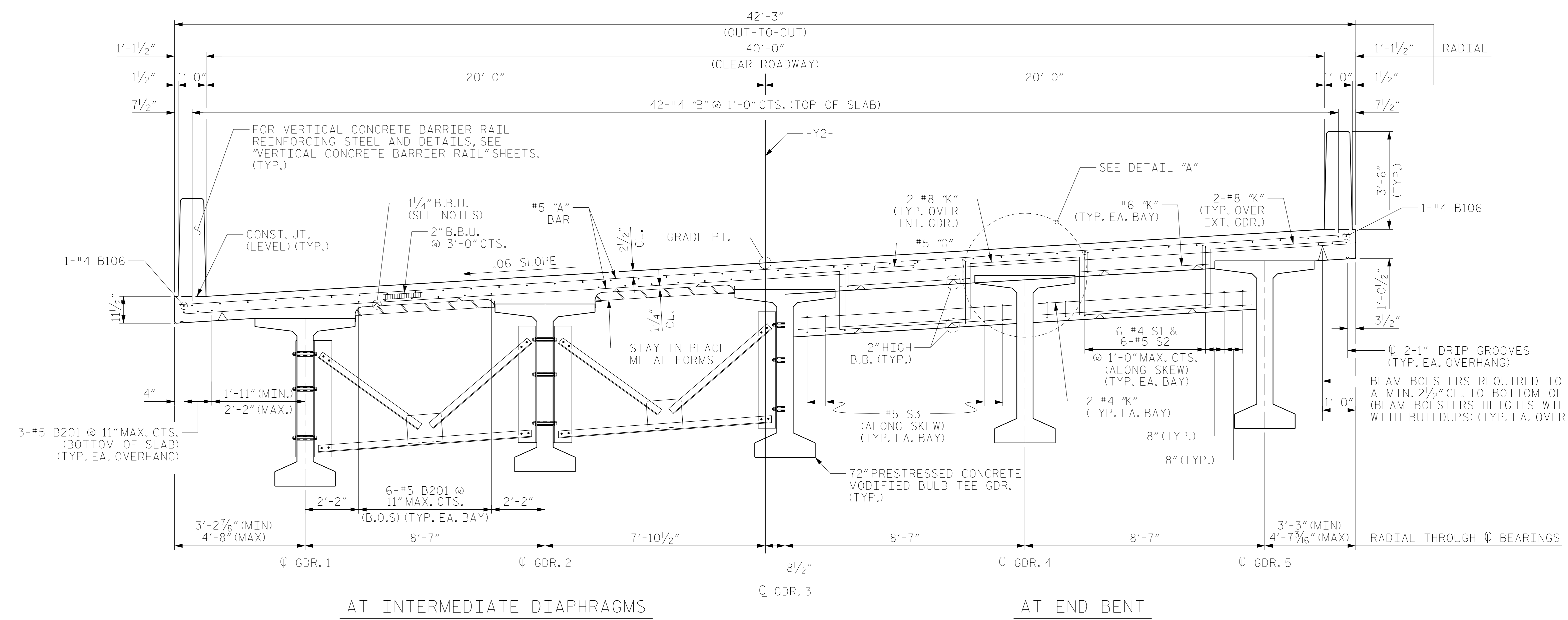
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CHECKED BY : TLC	DATE : 11/2016
DRAWN BY : MAA 1/08	REV. 11/2/08RR MAA/GM
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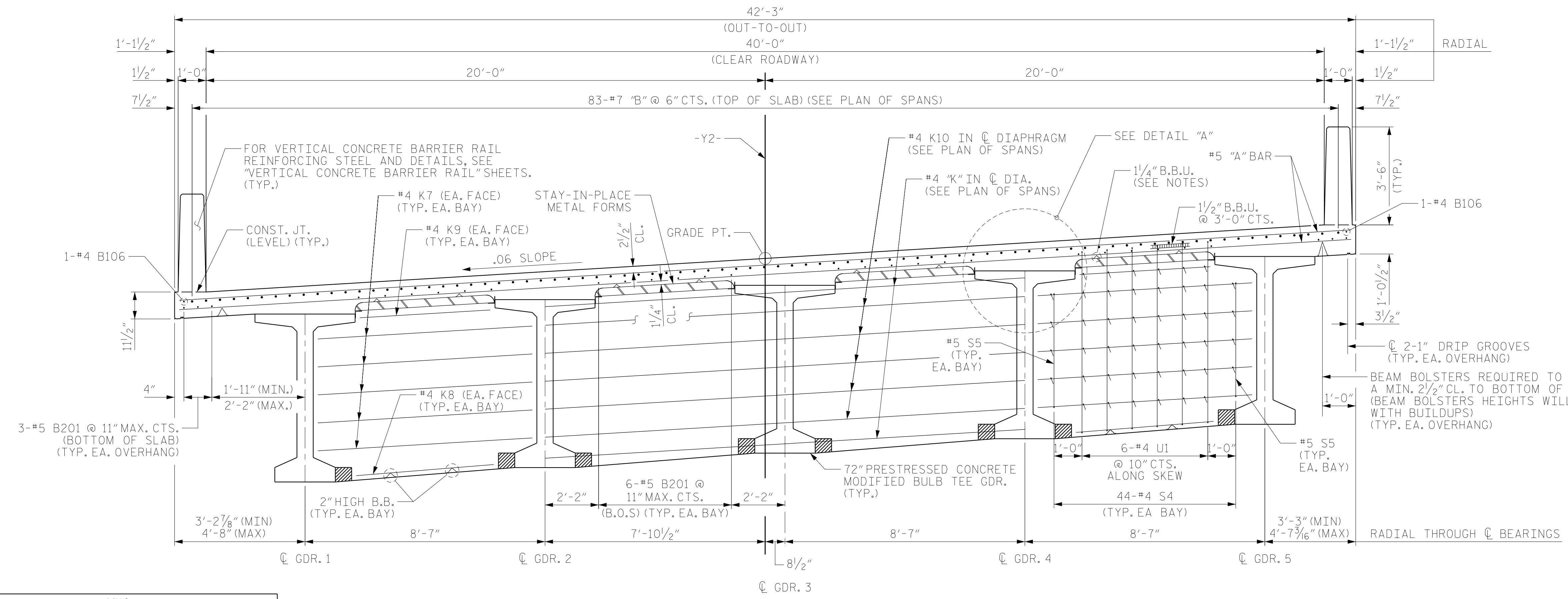
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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 LRFR SUMMARY FOR
 PRESTRESSED
 CONCRETE GIRDERS
 (NON-INTERSTATE TRAFFIC)

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-5
1			3			TOTAL SHEETS
2			4			36

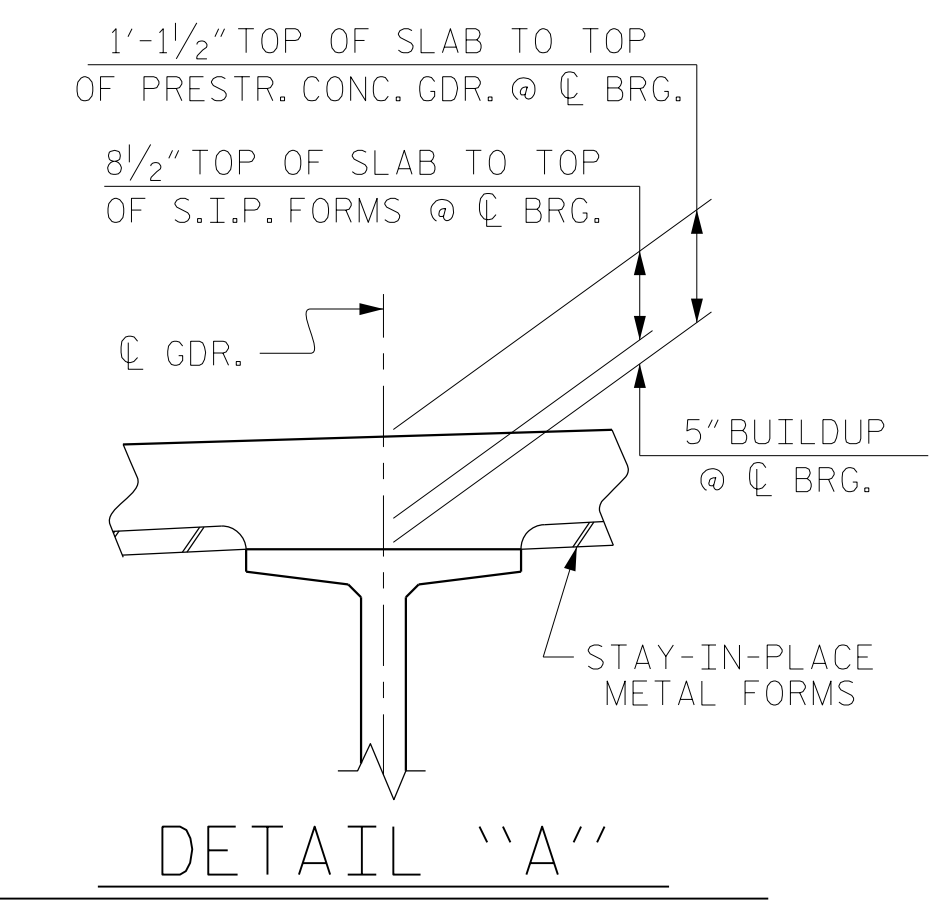


AT INTERMEDIATE DIAPHRAGMS
TYPICAL SECTION
 (B.O.S = BOTTOM OF SLAB)
 AT END BENT



TYPICAL SECTION AT BENT DIAPHRAGM

NOTES:
 PROVIDE 1/4" HIGH BEAM BOLSTERS UPPERS AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS, WHEN USING REMOVABLE FORMS. PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (CHCM) AT 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.
 LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.
 PREVIOUSLY CAST CONCRETE IN A CONTINUOUS UNIT SHALL HAVE ATTAINED MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.



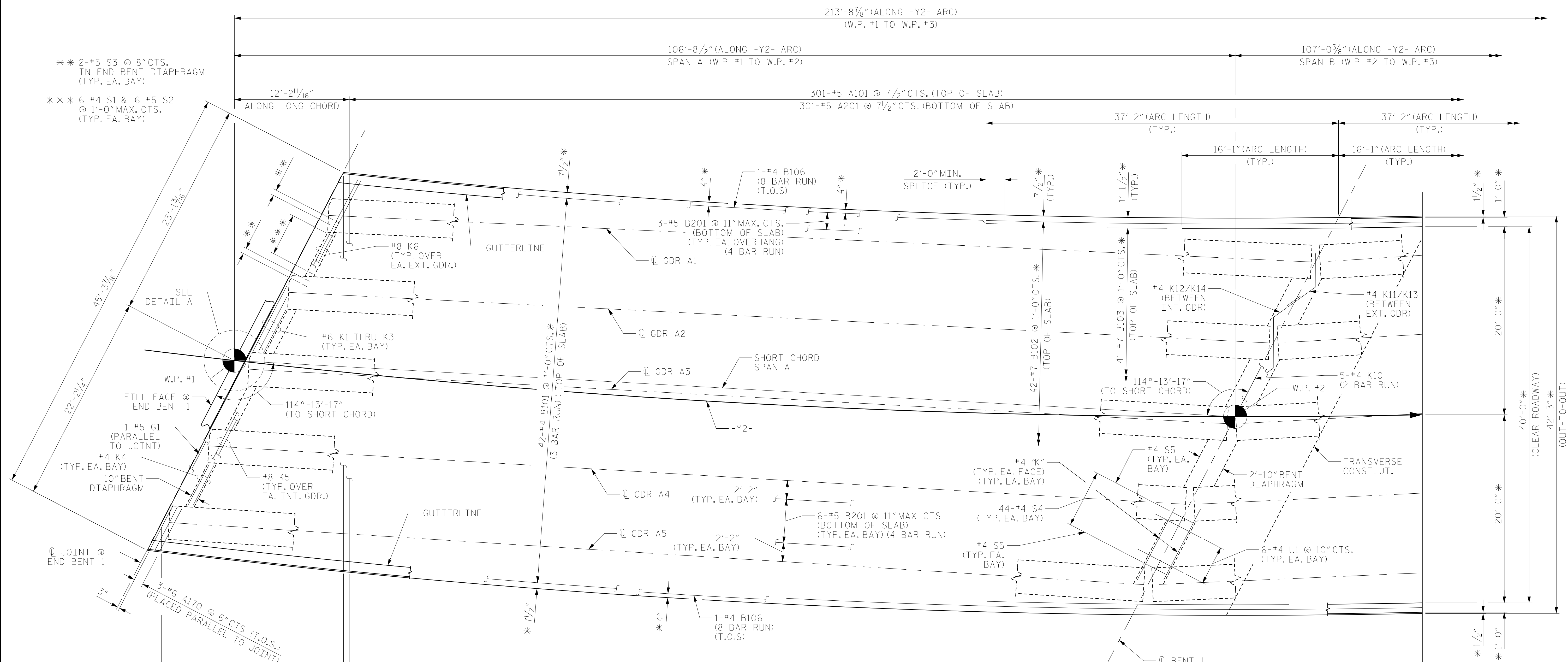
PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-
 SHEET 1 OF 2

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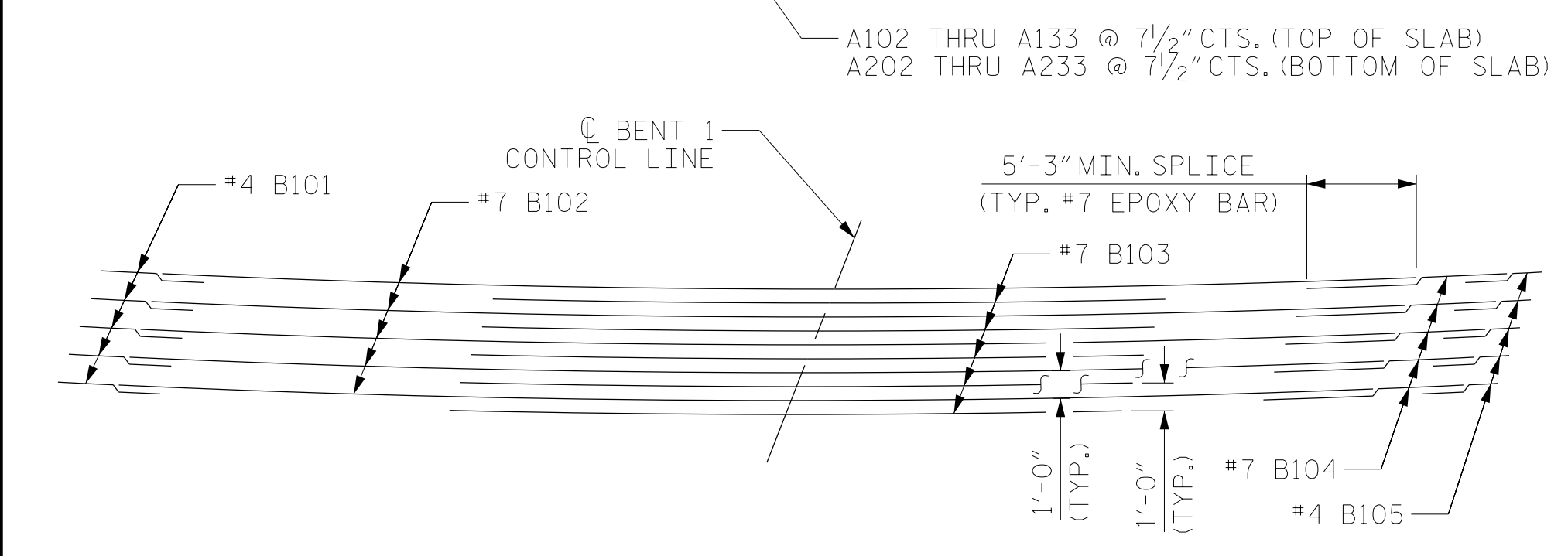
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE TYPICAL SECTION					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S3-6
TOTAL SHEETS					36

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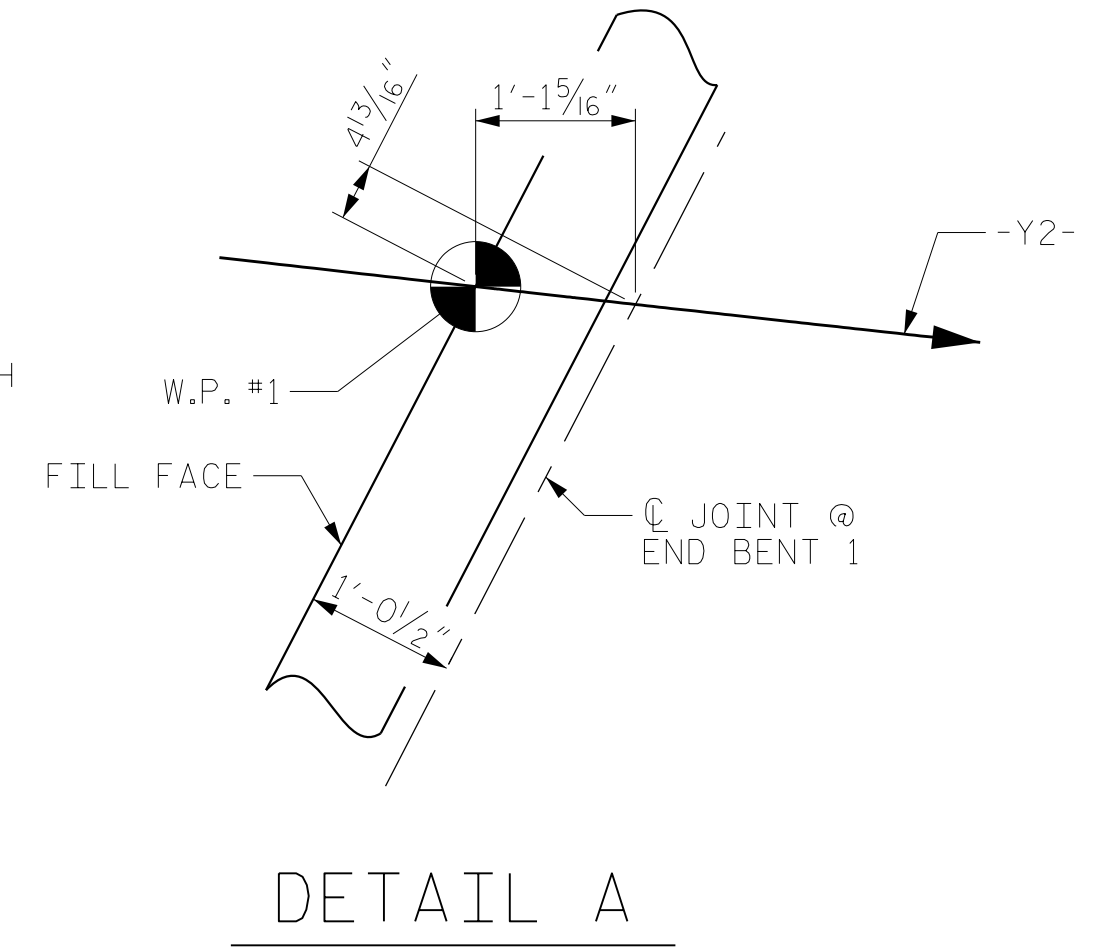
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PLAN OF SPAN A



TOP OF SLAB REINFORCING STEEL LAYOUT



DETAIL A

NOTES:
 FOR SPLICE LENGTHS NOT SHOWN, REFER TO MINIMUM SPLICE LENGTH TABLE ON "SUPERSTRUCTURE BILL OF MATERIAL" SHEET 1 OF 2.
 FOR END BENT DIAPHRAGM BARS AND BENT DIAPHRAGM BARS, SEE "TYPICAL SECTION" SHEET 2 OF 2.
 STEEL INTERMEDIATE DIAPHRAGMS NOT SHOWN FOR CLARITY. FOR LOCATIONS, SEE "FRAMING PLAN SPAN A" SHEET.
 FOR POURING SEQUENCE, SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET 2 OF 2.
 FOR BARRIER RAIL REINFORCING STEEL, SEE "VERTICAL CONCRETE BARRIER RAIL" SHEETS.
 #5 "A" BARS ARE SPACED PERPENDICULAR TO LONG CHORD.
 T.O.S = TOP OF SLAB

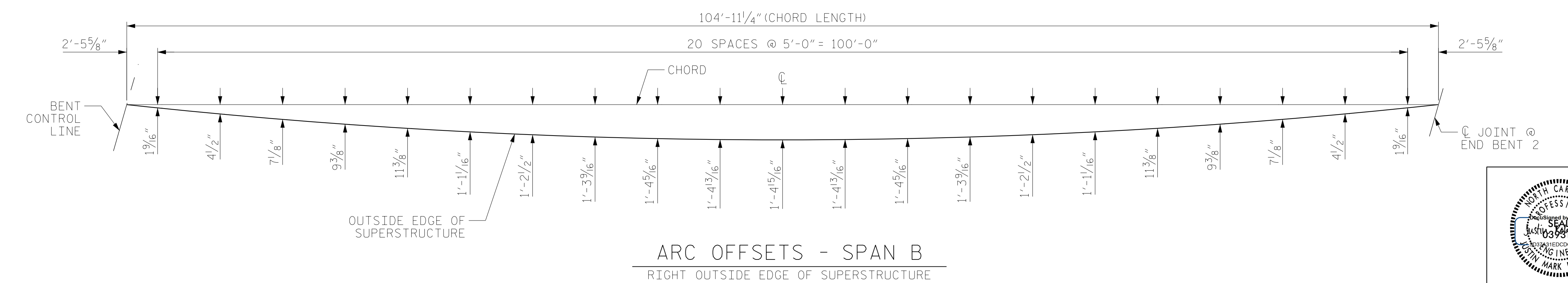
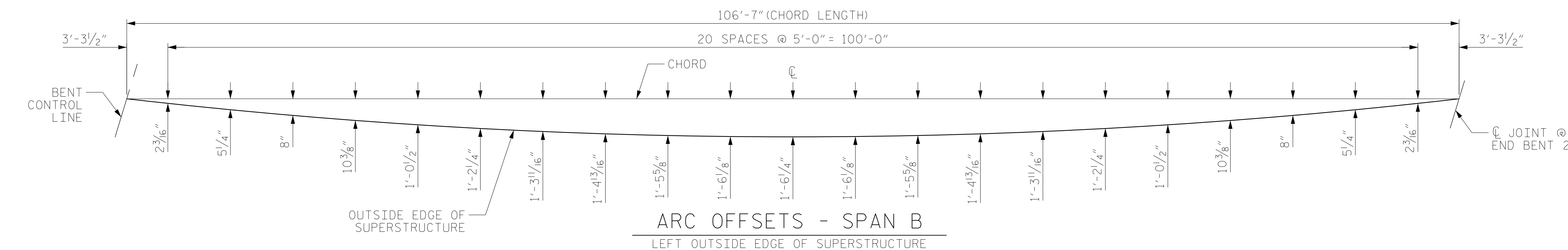
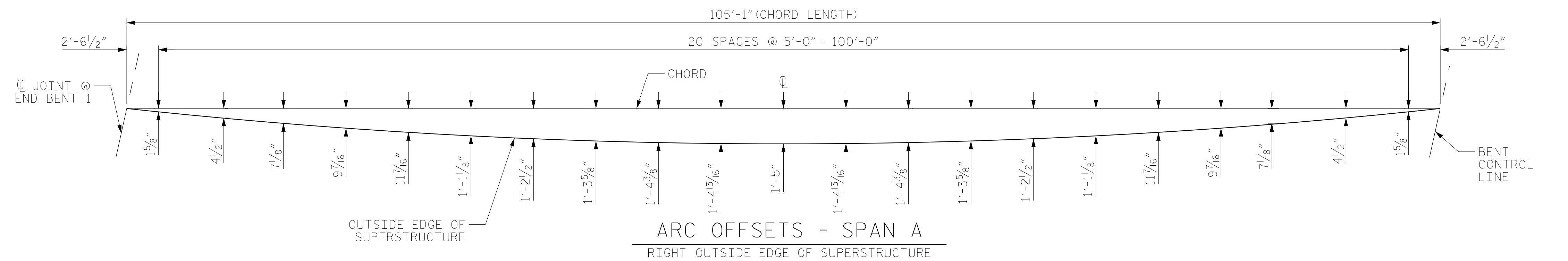
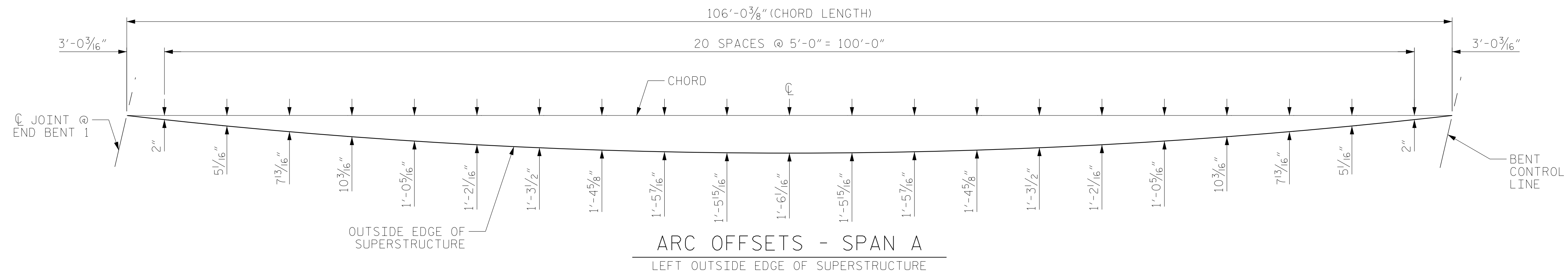
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CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-
 SHEET 1 OF 3



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE					
PLAN OF SPAN A					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S3-8
TOTAL SHEETS					36

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PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 3 OF 3



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

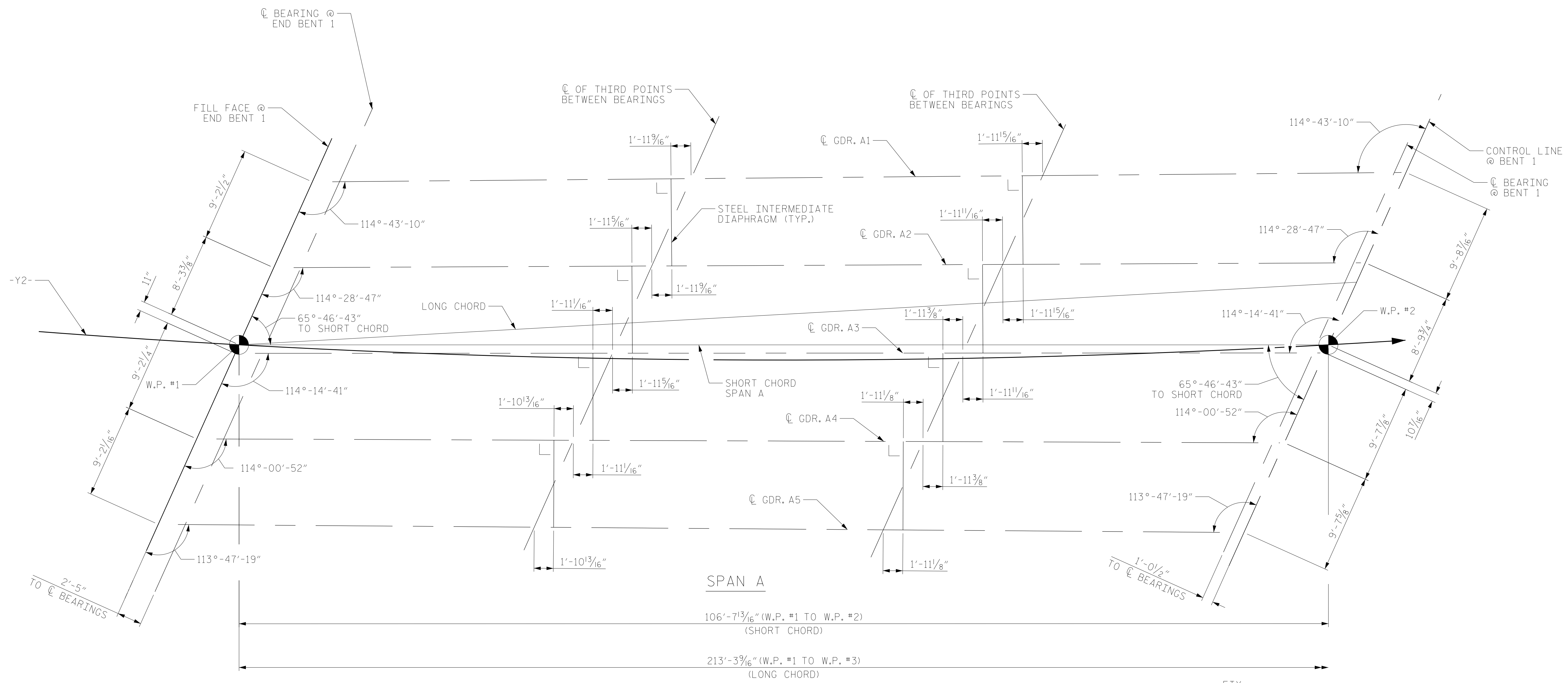
SUPERSTRUCTURE
 ARC OFFSETS

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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-10
1			3			TOTAL SHEETS
2			4			36



FRAMING PLAN

END BENT DIAPHRAGMS AND BENT DIAPHRAGMS ARE NOT SHOWN

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 1 OF 2

NOTES:

FOR DIMENSIONS TO STEEL INTERMEDIATE DIAPHRAGMS, SEE "PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD DETAILS," SHEET 3 OF 4.

FOR STEEL DIAPHRAGMS DETAILS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 72" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS."

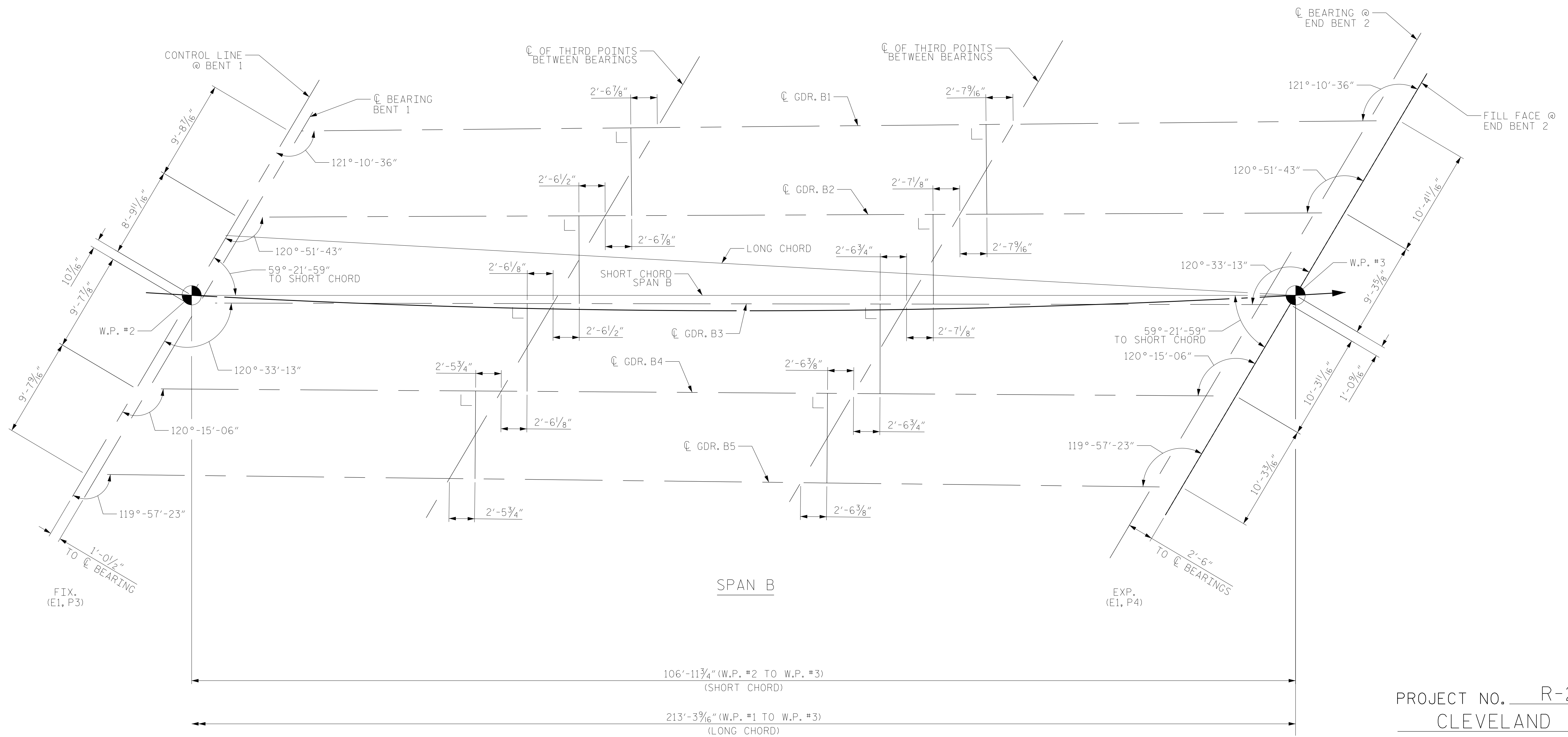
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DESIGN ENGINEER OF RECORD:	MKO	DATE :	12/2016

2/28/2017
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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE FRAMING PLAN SPAN A					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S3-11
TOTAL SHEETS					36



FRAMING PLAN

END BENT DIAPHRAGMS AND BENT DIAPHRAGMS ARE NOT SHOWN

NOTES

FOR DIMENSIONS TO STEEL INTERMEDIATE DIAPHRAGMS, SEE "PRESTRESSED CONCRETE FINDER CONTINUOUS FOR LIVE LOAD DETAILS," SHEET 3 OF 4.

FOR STEEL DISPHRAGM DETAILS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 72" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS."

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 2 OF 2

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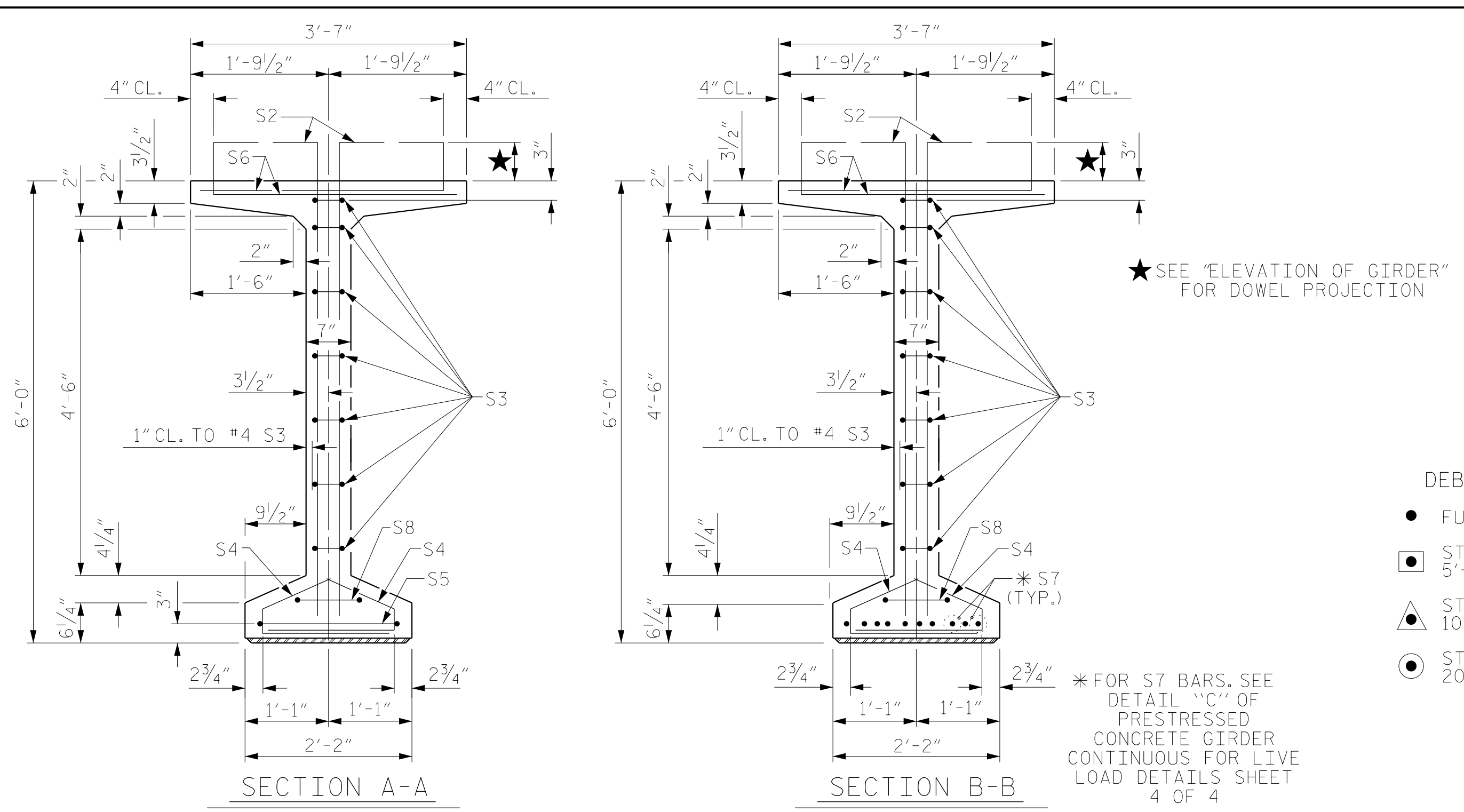
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 FRAMING PLAN
 SPAN B

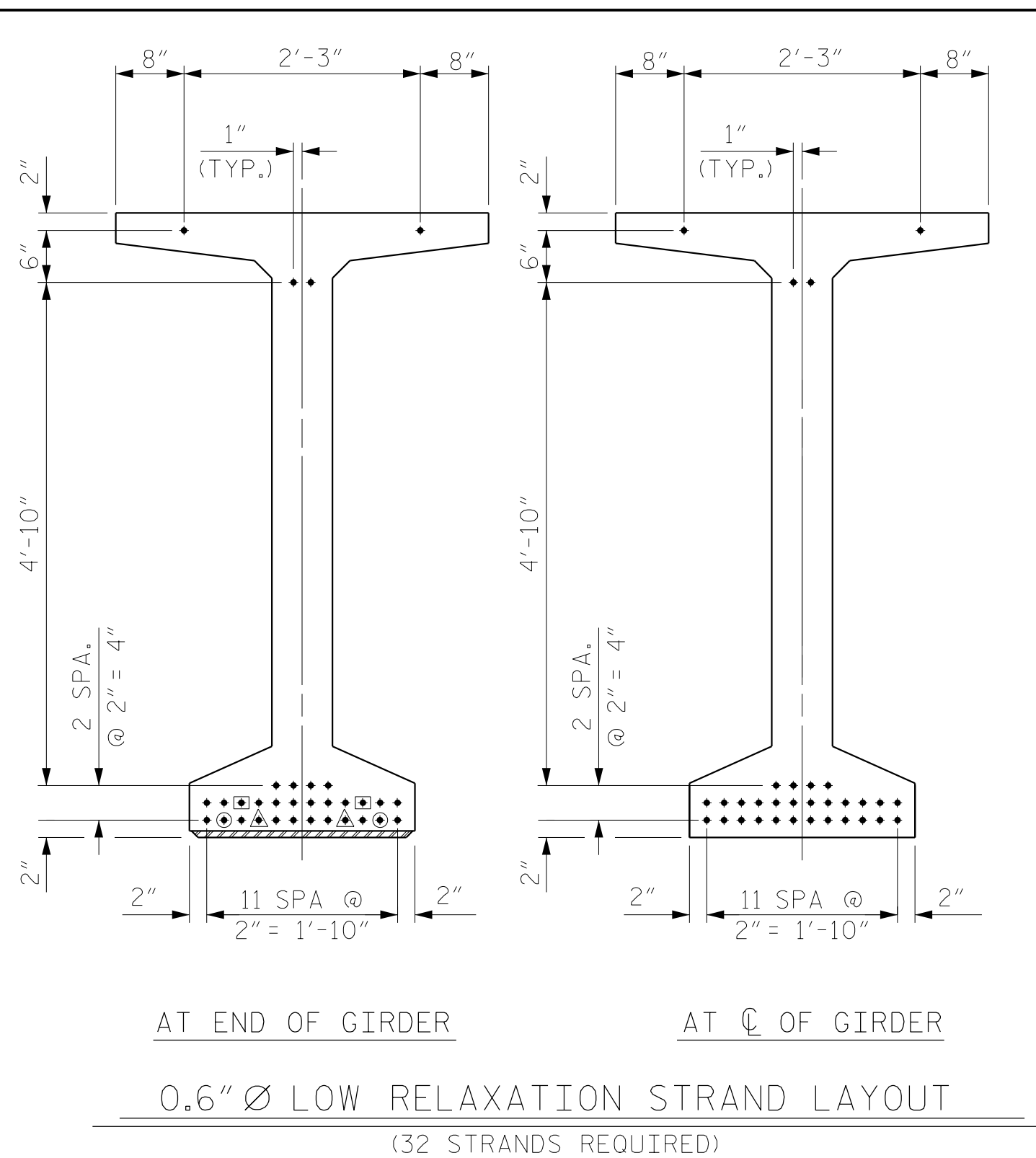
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1			3			36
2			4			



- DEBONDING LEGEND**
- FULLY BONDED STRANDS
 - STRANDS DEBONDED FOR 5'-0" FROM END OF GIRDER
 - ▲ STRANDS DEBONDED FOR 10'-0" FROM END OF GIRDER
 - STRANDS DEBONDED FOR 20'-0" FROM END OF GIRDER



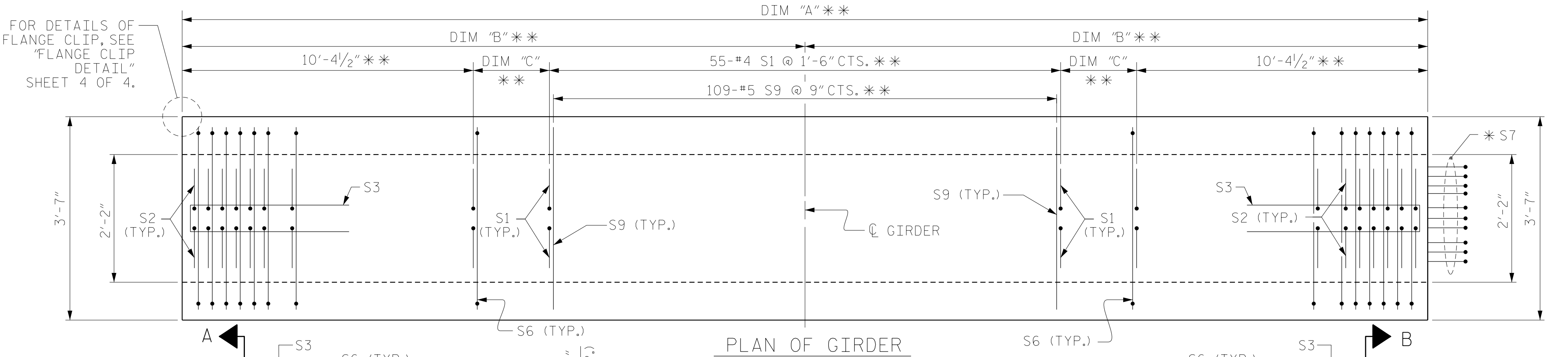
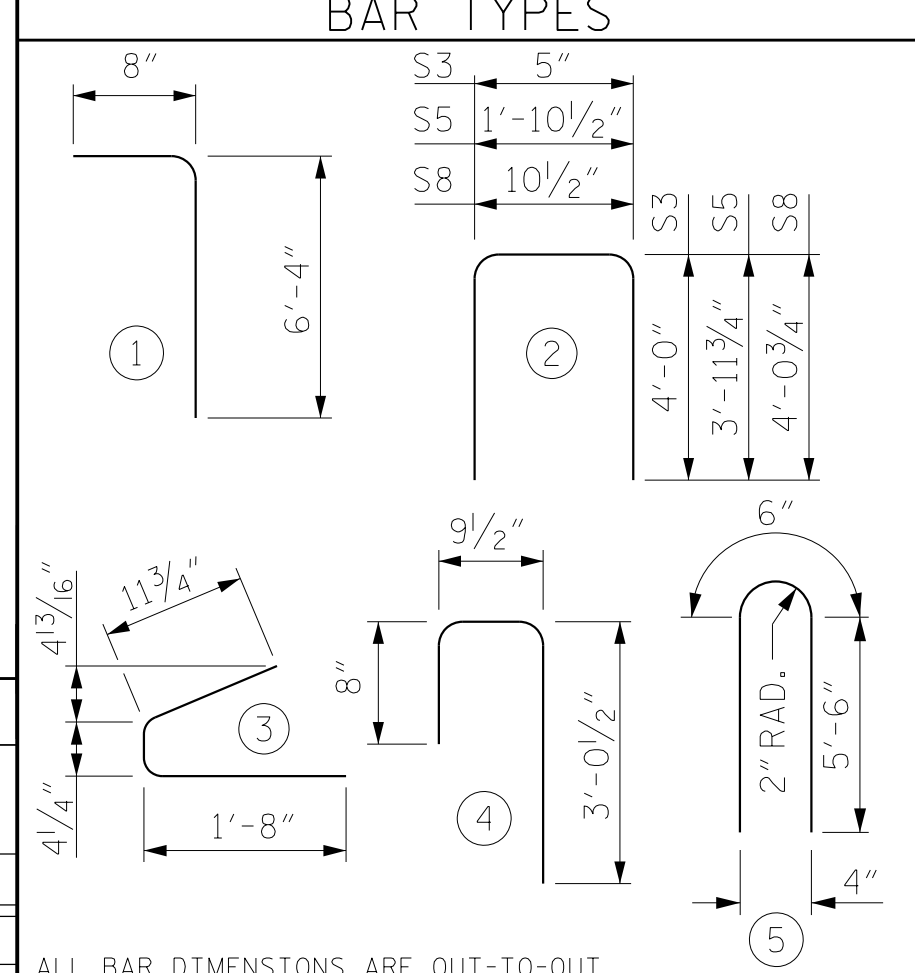
0.6" Ø L. R. GRADE 270 STRANDS

AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR

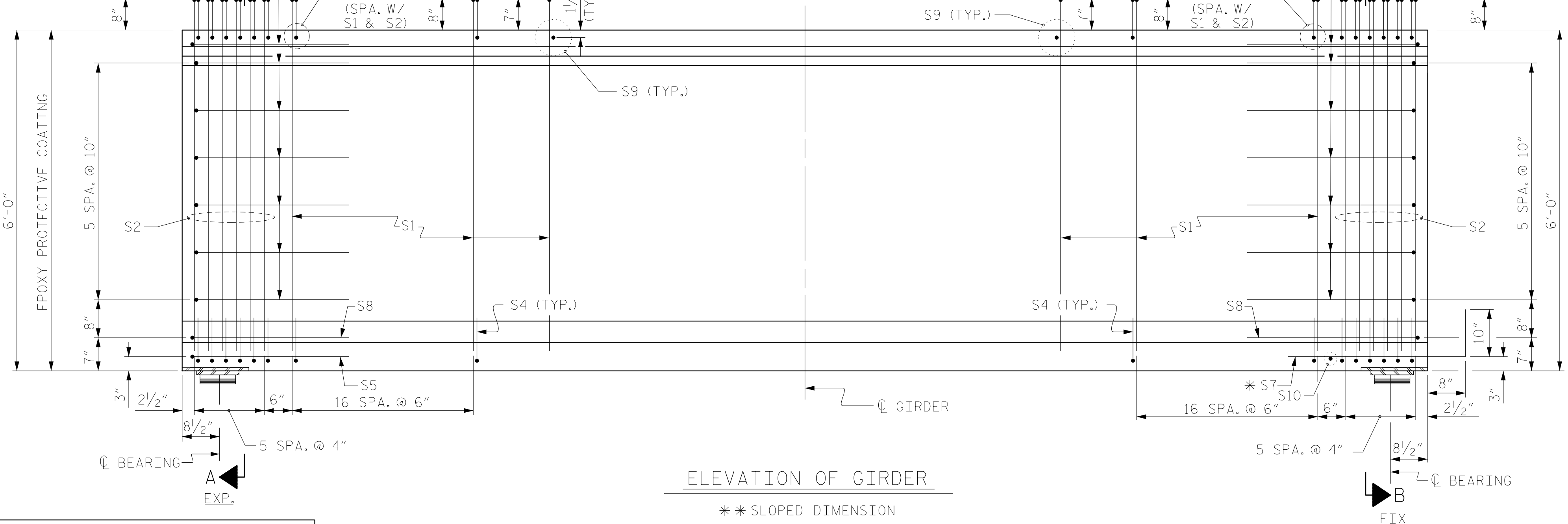
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	
S1	178	#4	1	7'-0"	832	
S2	24	#5	1	7'-0"	175	
S3	14	#4	2	8'-5"	79	
S4	92	#4	3	3'-0"	184	
S5	1	#5	2	9'-10"	10	
S6	92	#5	4	4'-6"	432	
* S7	10	#5	STR	3'-8"	38	
S8	2	#5	2	9'-0"	19	
S9	109	#5	STR	3'-3"	369	
S10	1	#3	STR	1'-10"	1	
S11	8	#5	5	11'-6"	96	
GDR A2, A3, A4	S11	16	#5	5	11'-6"	192
GDR A1 & A5	S12	16	#4	STR	8'-0"	86
GDR A2, A3, A4	S13	16	#4	STR	12'-0"	128

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



QUANTITIES FOR ONE GIRDER

	REINFORCING STEEL	8000 PSI CONCRETE	0.6" Ø L.R. STRANDS
	LB.	C.Y.	No.
GIRDER A1	2321	22.4	32
GIRDER A2	2459	22.4	32
GIRDER A3	2459	22.4	32
GIRDER A4	2459	22.3	32
GIRDER A5	2321	22.3	32



GIRDERS DIMENSION TABLE

GDR. #	DIM "A"	DIM "B"	DIM "C"
A1	104'-9 1/4"	52'-4 5/8"	1'-6 1/8"
A2	104'-6 7/8"	52'-3 1/8"	1'-4 5/16"
A3	104'-4 3/16"	52'-2 5/16"	1'-3 3/16"
A4	104'-2 3/16"	52'-1 3/16"	1'-2 1/16"
A5	104'-0 3/16"	52'-0 7/8"	1'-1 5/8"

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-
 SHEET 1 OF 4

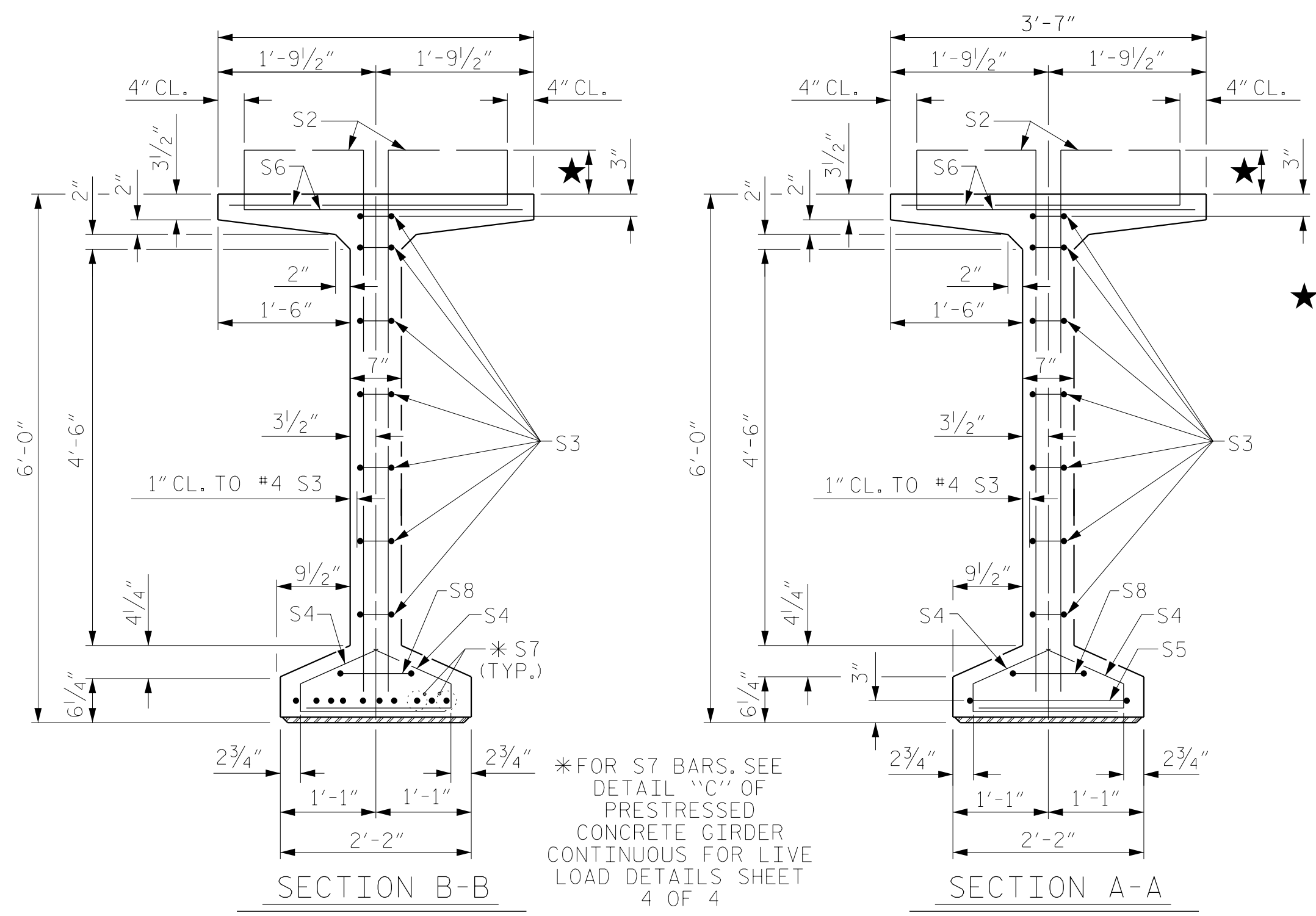


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 72" PRESTRESSED CONCRETE
 MODIFIED BULB TEE
 CONTINUOUS FOR LIVE LOAD
 SPAN A

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-13
1			3			TOTAL SHEETS
2			4			36

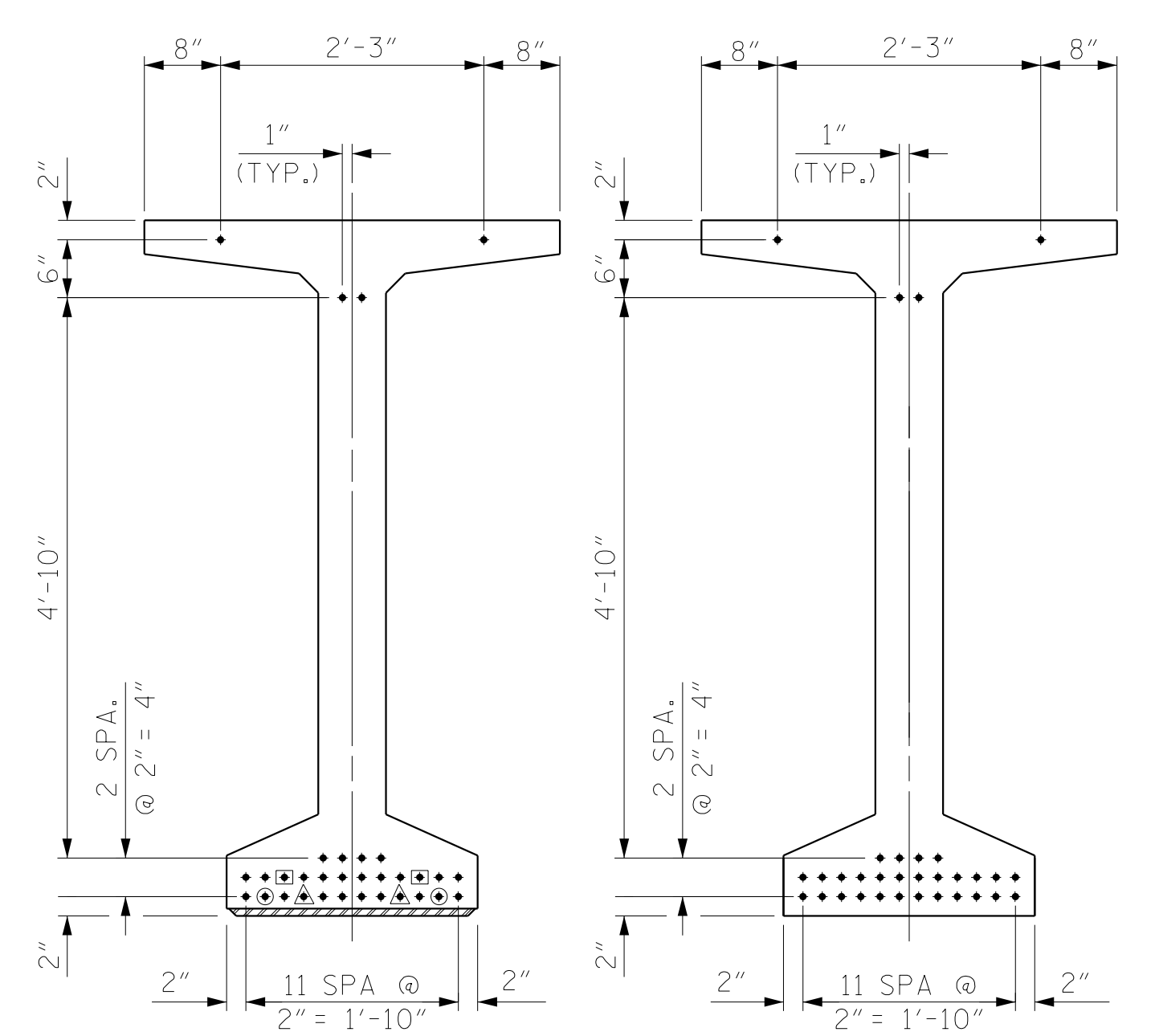
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★ SEE "ELEVATION OF GIRDER" FOR DOWEL PROJECTION

- DEBONDING LEGEND**
- FULLY BONDED STRANDS
 - STRANDS DEBONDED FOR 5'-0" FROM END OF GIRDER
 - ▲ STRANDS DEBONDED FOR 10'-0" FROM END OF GIRDER
 - STRANDS DEBONDED FOR 20'-0" FROM END OF GIRDER

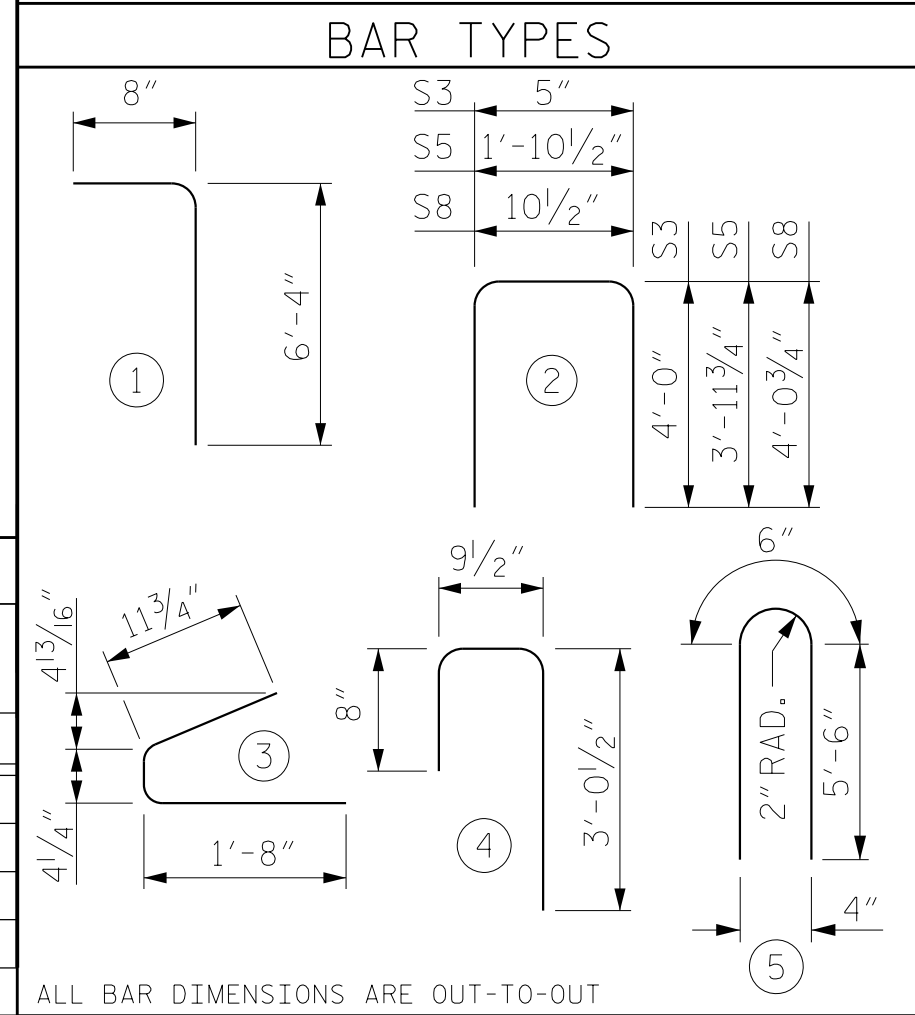


AT END OF GIRDER AT C OF GIRDER

0.6" Ø LOW RELAXATION STRAND LAYOUT (32 STRANDS REQUIRED)

REINFORCING STEEL FOR ONE GDR						
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	
GDR. B1	S1	180	#4	1	7'-0"	842
GDR. B2, B3, B4, B5	S1	178	#4	1	7'-0"	832
	S2	24	#5	1	7'-0"	175
	S3	14	#4	2	8'-5"	79
	S4	92	#4	3	3'-0"	184
	S5	1	#5	2	9'-10"	10
	S6	92	#5	4	4'-6"	432
	* S7	10	#5	STR	3'-8"	38
	S8	2	#5	2	9'-0"	19
GDR. B1	S9	111	#5	STR	3'-3"	376
GDR. B2, B3, B4, B5	S9	109	#5	STR	3'-3"	369
	S10	1	#3	STR	1'-10"	1
GDR. B1 & B5	S11	8	#5	5	11'-6"	96
GDR. B2, B3, B4	S11	16	#5	5	11'-6"	192
GDR. B1 & B5	S12	16	#4	STR	8'-0"	86
GDR. B2, B3, B4	S13	16	#4	STR	13'-3"	142

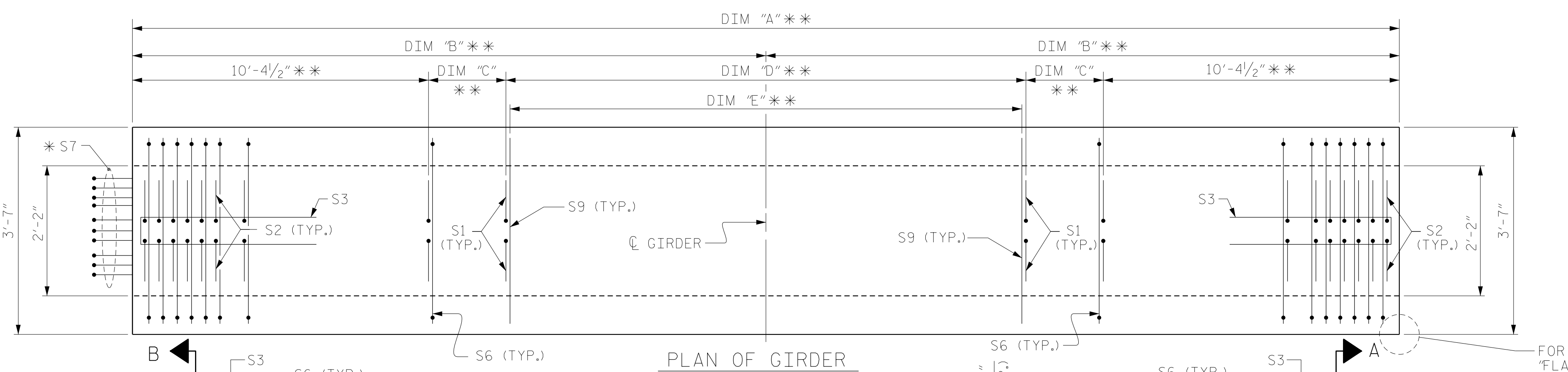
* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



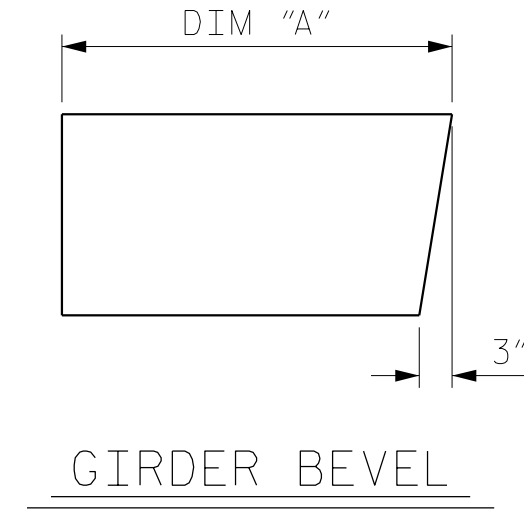
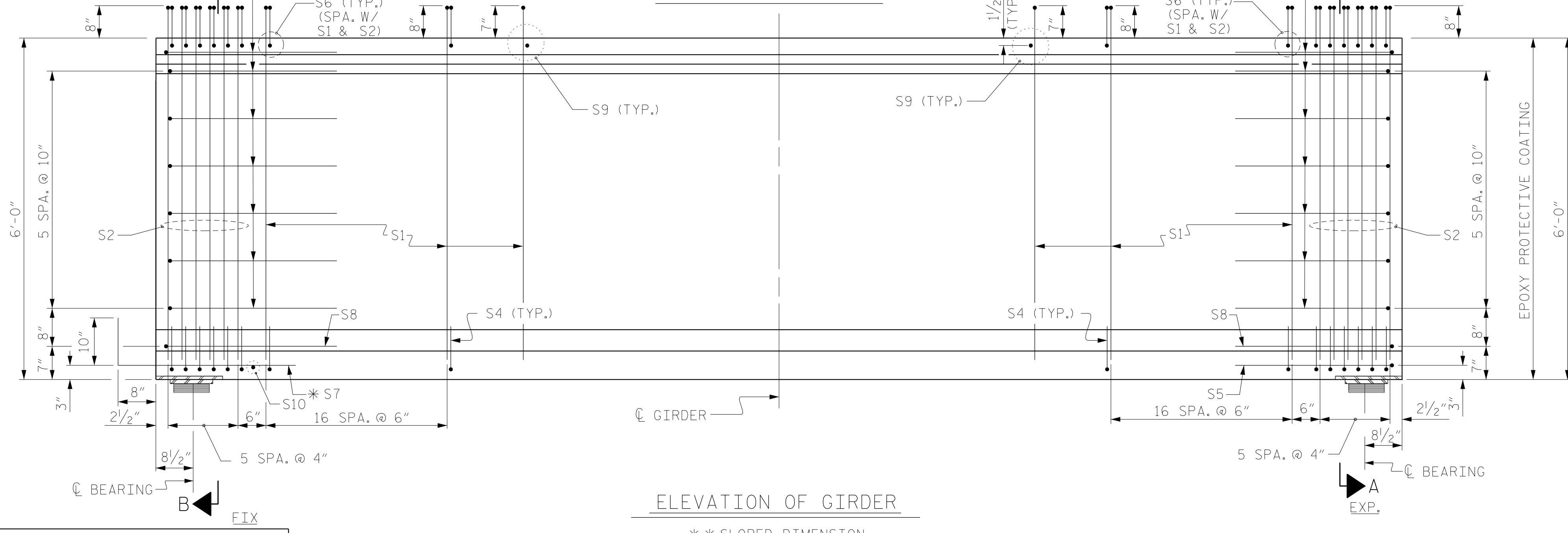
	QUANTITIES FOR ONE GIRDER		
	REINFORCING STEEL	8000 PSI CONCRETE	0.6" Ø L.R. STRANDS
	LB.	C.Y.	No.
GIRDER B1	2338	22.5	32
GIRDER B2	2473	22.4	32
GIRDER B3	2473	22.3	32
GIRDER B4	2473	22.3	32
GIRDER B5	2321	22.2	32

GIRDERS REQUIRED		
NUMBER	LENGTH	TOTAL LENGTH
5	SEE TABLE	521'-5 3/4"

GIRDER DIMENSION TABLE					
GDR. #	DIM "A"	DIM "B"	DIM "C"	DIM "D"	DIM "E"
B1	104'-11 7/16"	52'-5 3/4"	10 1/4"	56-#4 S1 @ 1'-6"CTS.	111-#5 S9 @ 9"CTS.
B2	104'-7 5/16"	52'-3 3/8"	1'-5 3/16"	55-#4 S1 @ 1'-6"CTS.	109-#5 S9 @ 9"CTS.
B3	104'-3 3/16"	52'-1 1/16"	1'-3 3/16"	55-#4 S1 @ 1'-6"CTS.	109-#5 S9 @ 9"CTS.
B4	103'-11 1/2"	51'-11 3/4"	1'-1 1/4"	55-#4 S1 @ 1'-6"CTS.	109-#5 S9 @ 9"CTS.
B5	103'-7 3/16"	51'-9 7/8"	11 7/16"	55-#4 S1 @ 1'-6"CTS.	109-#5 S9 @ 9"CTS.



FOR DETAILS OF FLANGE CLIP, SEE "FLANGE CLIP DETAIL" SHEET 4 OF 4.



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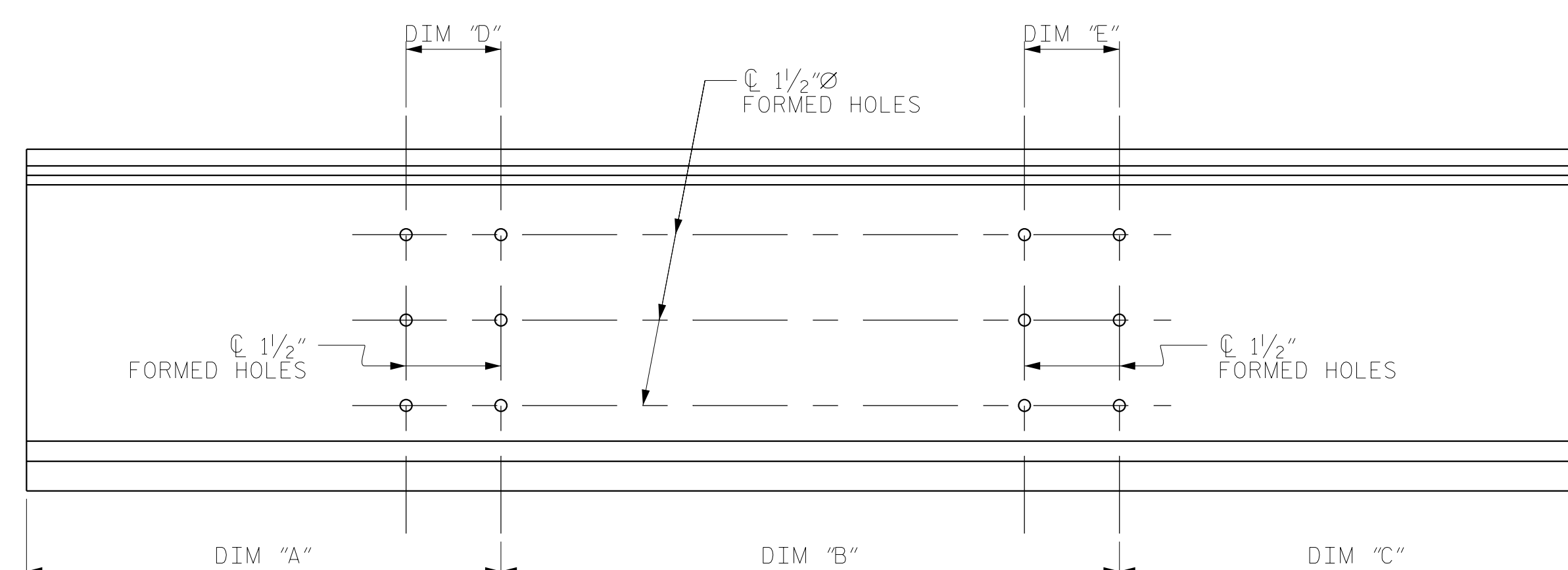
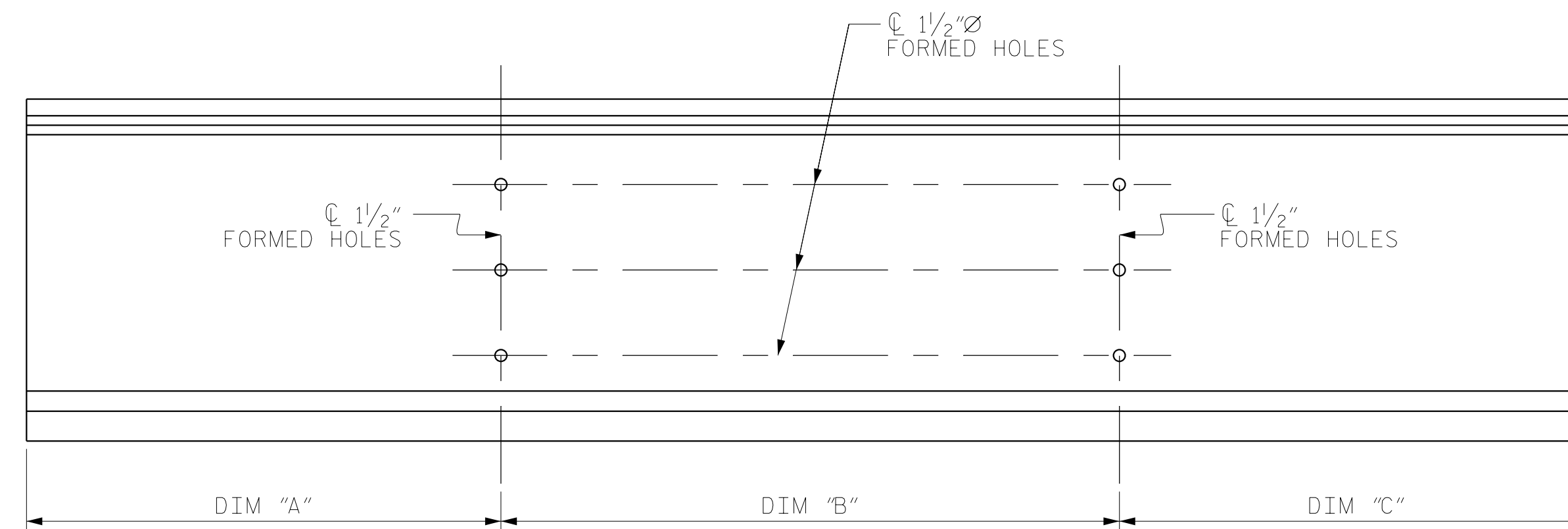
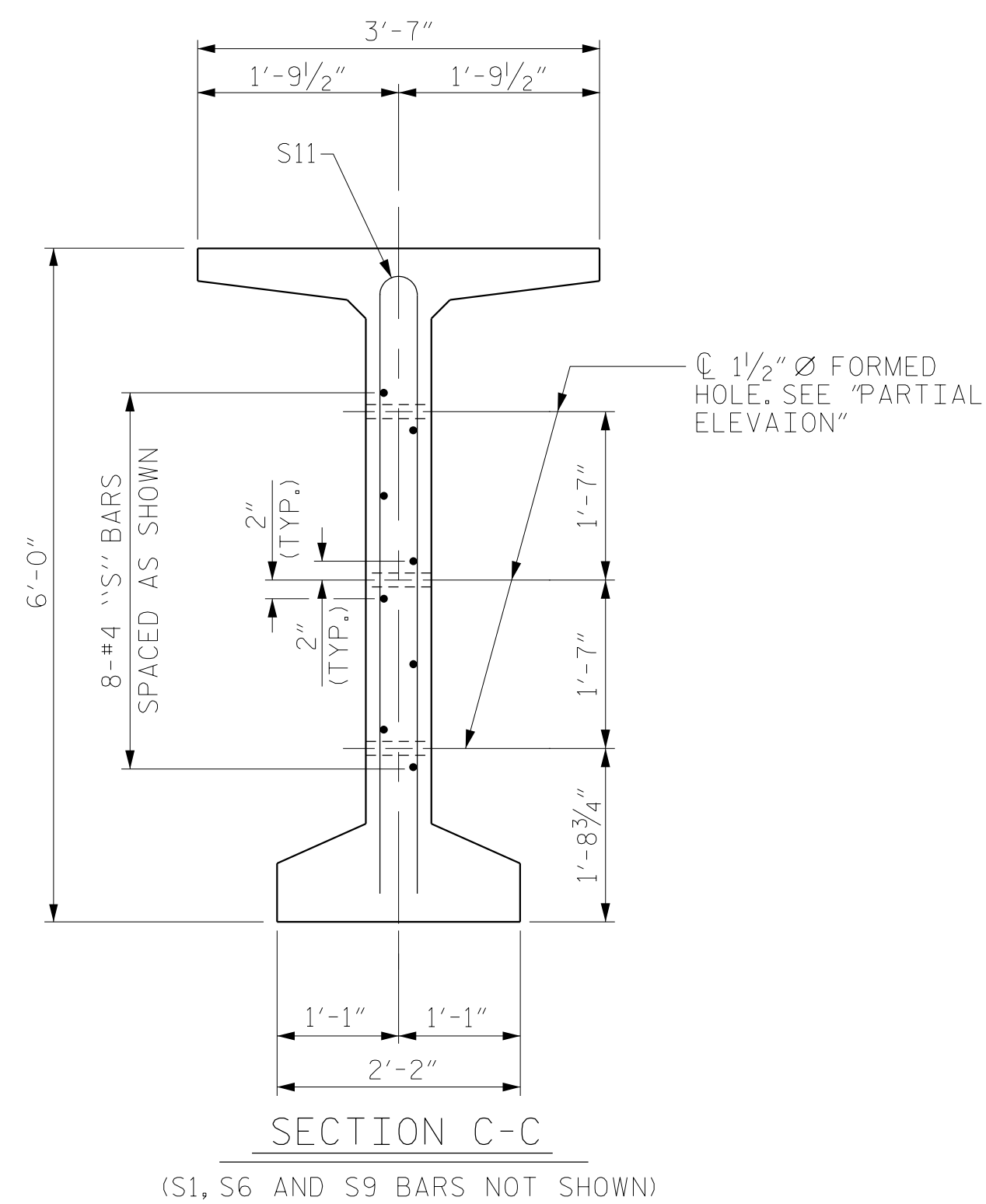
PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 29+07.16 -Y2-

SHEET 2 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
72" PRESTRESSED CONCRETE
MODIFIED BULB TEE
CONTINUOUS FOR LIVE LOAD
SPAN B

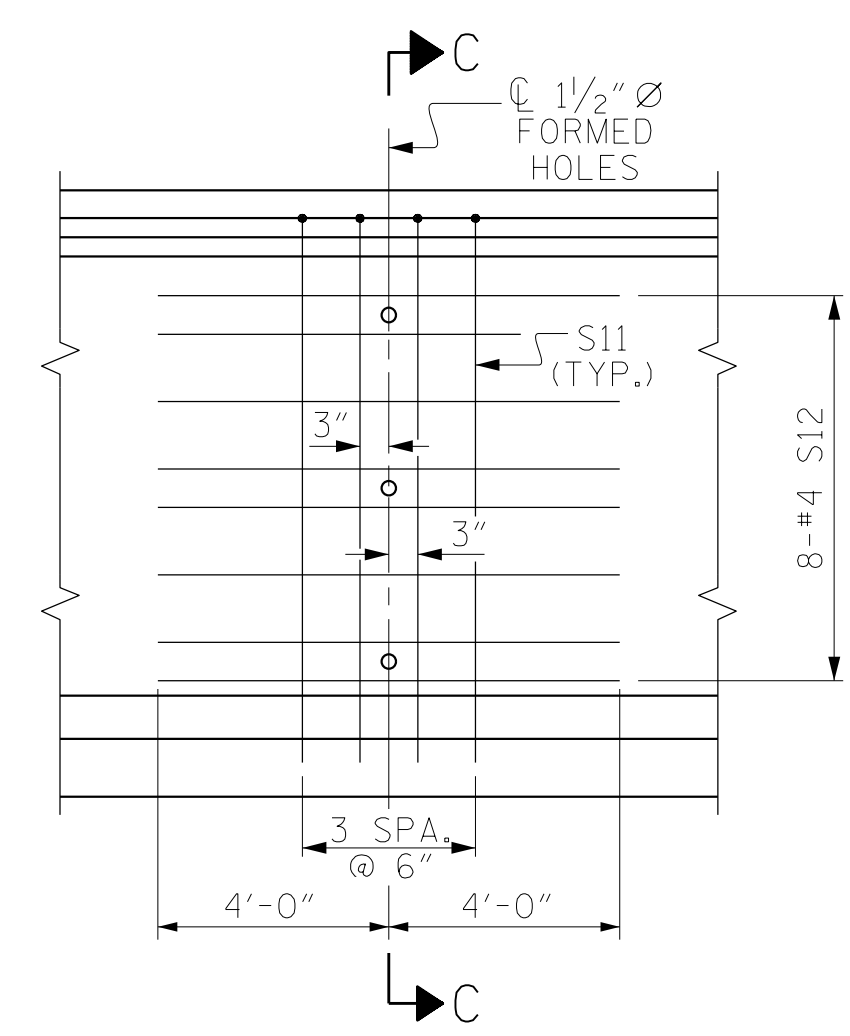
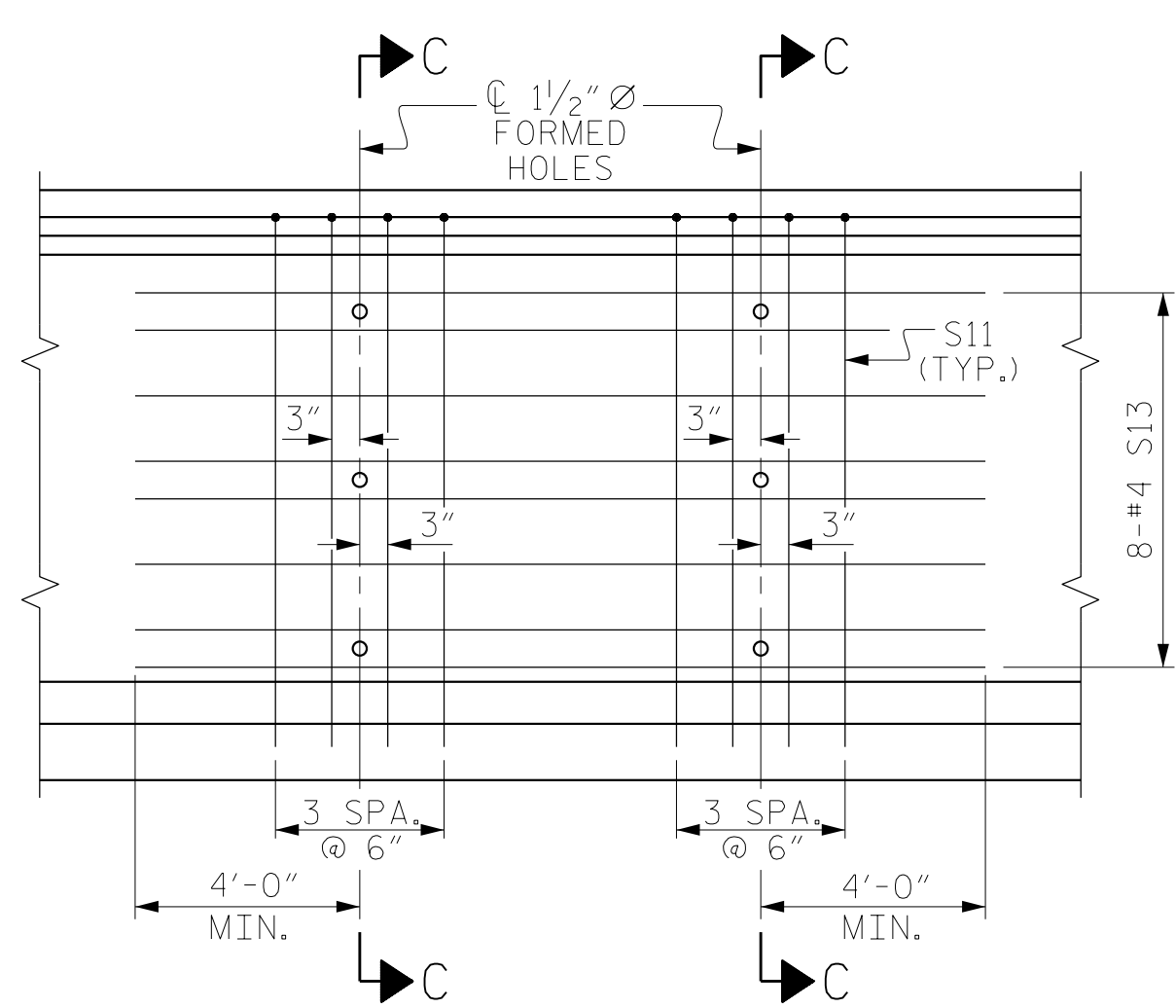
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-14
1			3			TOTAL SHEETS
2			4			36

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BOLT HOLE PLACEMENT

GDR. NO.	SPAN A					SPAN B				
	DIM "A"	DIM "B"	DIM "C"	DIM "D"	DIM "E"	DIM "A"	DIM "B"	DIM "C"	DIM "D"	DIM "E"
1	33'-2 1/2"	34'-4 1/16"	37'-2 1/16"	-	-	32'-7 15/16"	34'-5 1/8"	37'-10 3/8"	-	-
2	37'-0 13/16"	34'-4 1/16"	33'-1 3/8"	3'-10 3/16"	3'-11 5/8"	37'-8 1/4"	34'-5 1/8"	32'-5 15/16"	5'-1 5/16"	5'-2 1/16"
3	36'-11 13/16"	34'-3 7/8"	33'-0 7/8"	3'-10 9/16"	3'-11 1/16"	37'-6 9/16"	34'-3 3/4"	34'-5"	5'-0 9/16"	5'-1 7/8"
4	36'-10 13/16"	34'-3 1/8"	33'-0 3/8"	3'-9 13/16"	3'-10 3/16"	37'-4 15/16"	34'-2 1/2"	32'-4 1/16"	4'-11 1/8"	5'-1 1/8"
5	36'-9 1/8"	34'-2 1/16"	32'-11 1/8"	-	-	37'-3 3/8"	34'-1 1/4"	32'-3 1/4"	-	-



PARTIAL ELEVATION

SHOWING INTERMEDIATE STEEL DIAPHRAGM REINFORCING STEEL FOR GIRDERS

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 3 OF 4

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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 PRESTRESSED CONCRETE GIRDER
 CONTINUOUS FOR LIVE LOAD
 DETAILS

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-15
1			3			TOTAL SHEETS
2			4			36

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 DESIGN ENGINEER OF RECORD : MKO DATE : 12/2016

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NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

APPLY EPOXY PROTECTIVE COATING TO END OF GIRDER SURFACES INDICATED IN ELEVATION VIEW.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

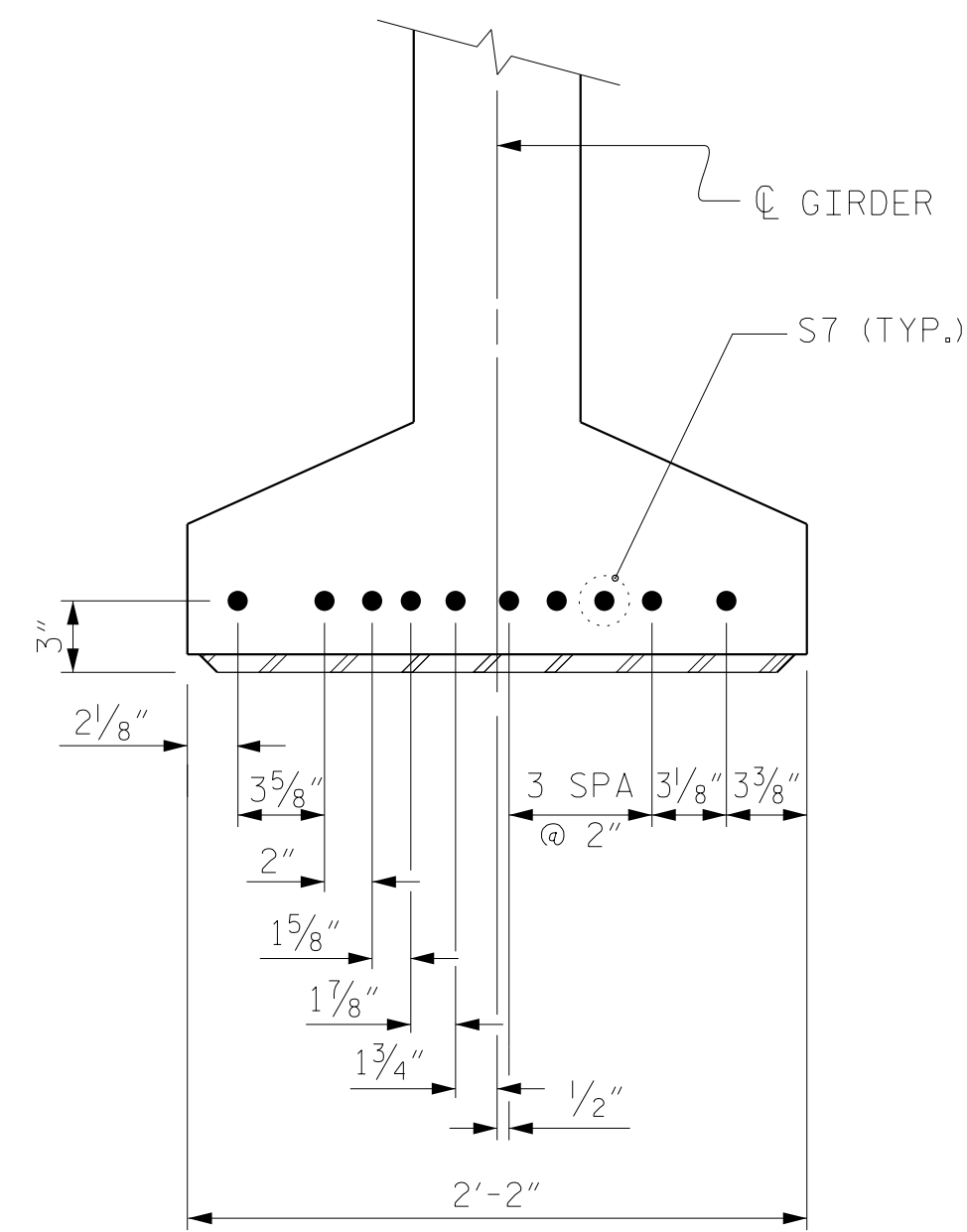
THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 5000 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

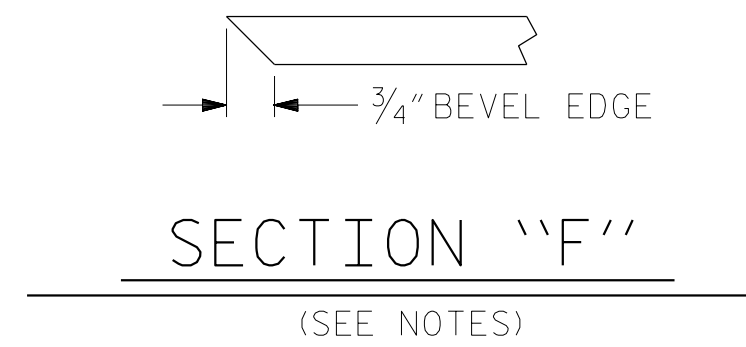
A 2" x 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 72" MODIFIED BULB TEE ONLY.

FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.



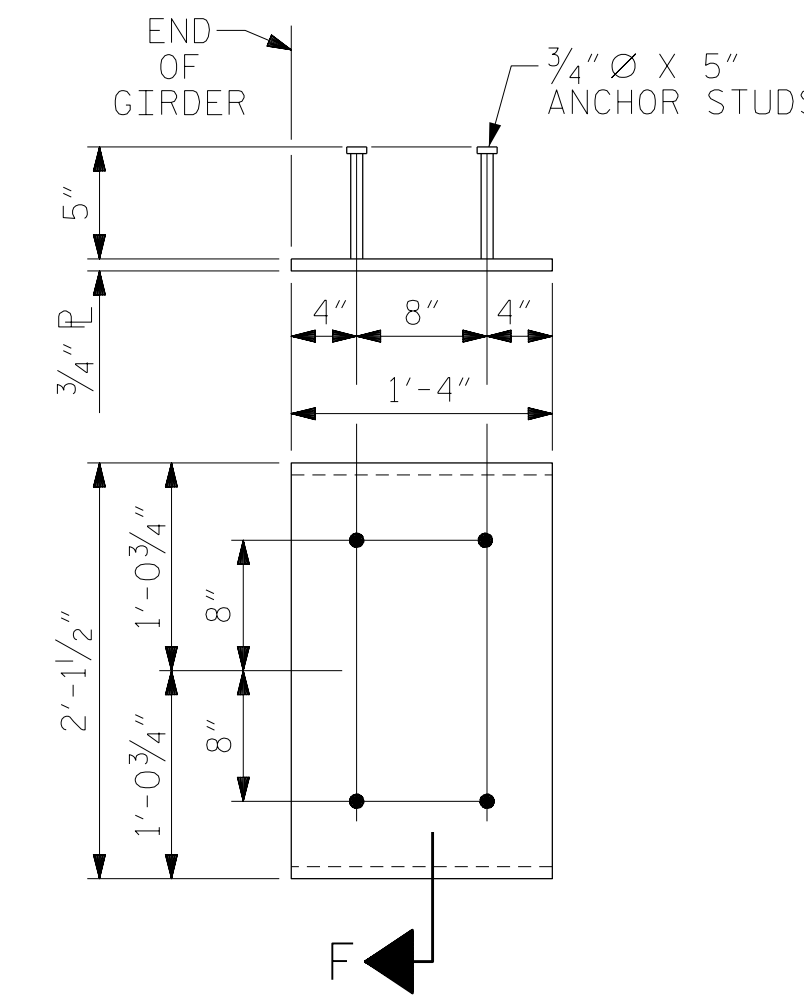
DETAIL "C"

(FOR 72" MODIFIED BULB TEES)



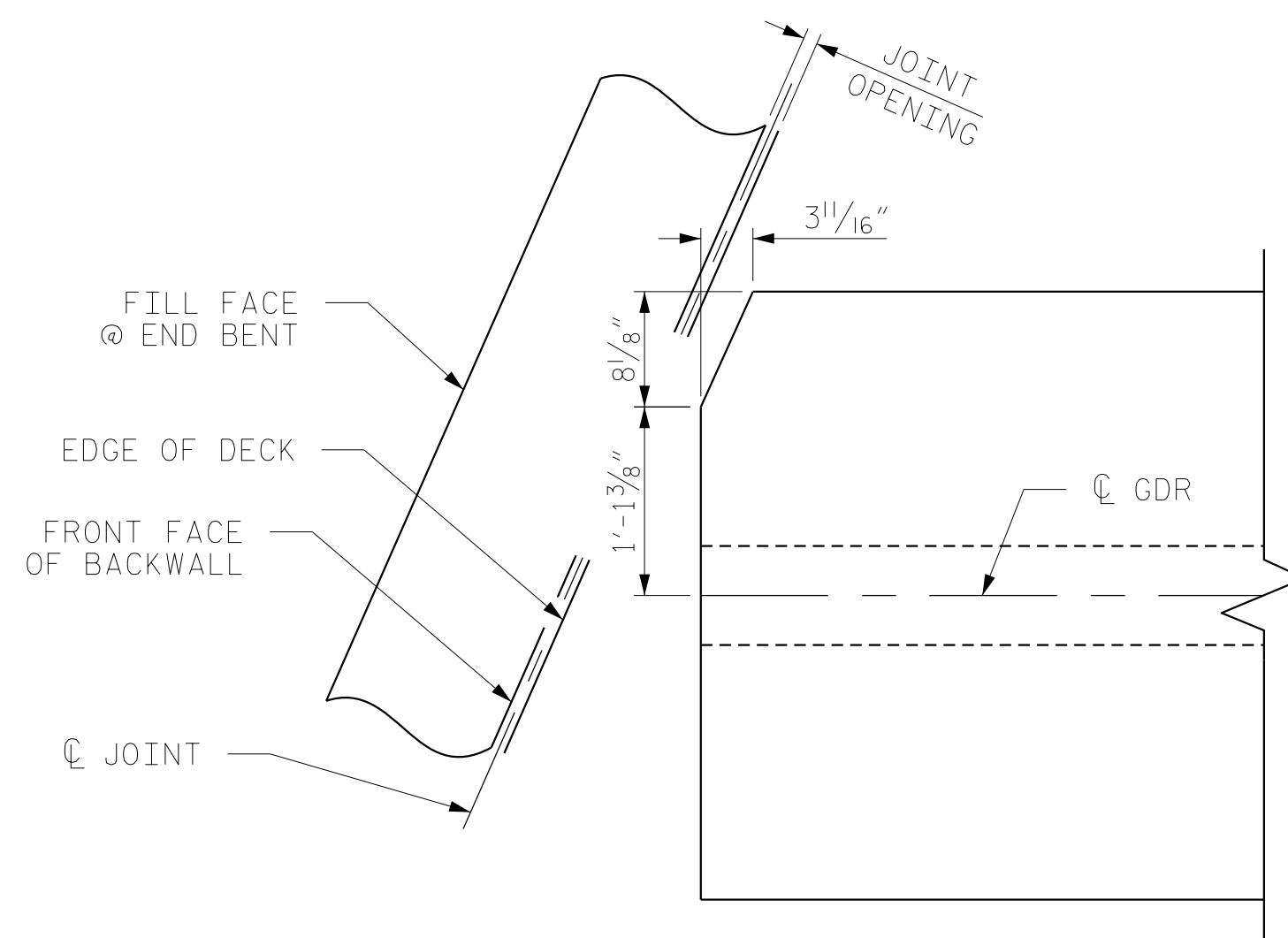
SECTION "F"

(SEE NOTES)

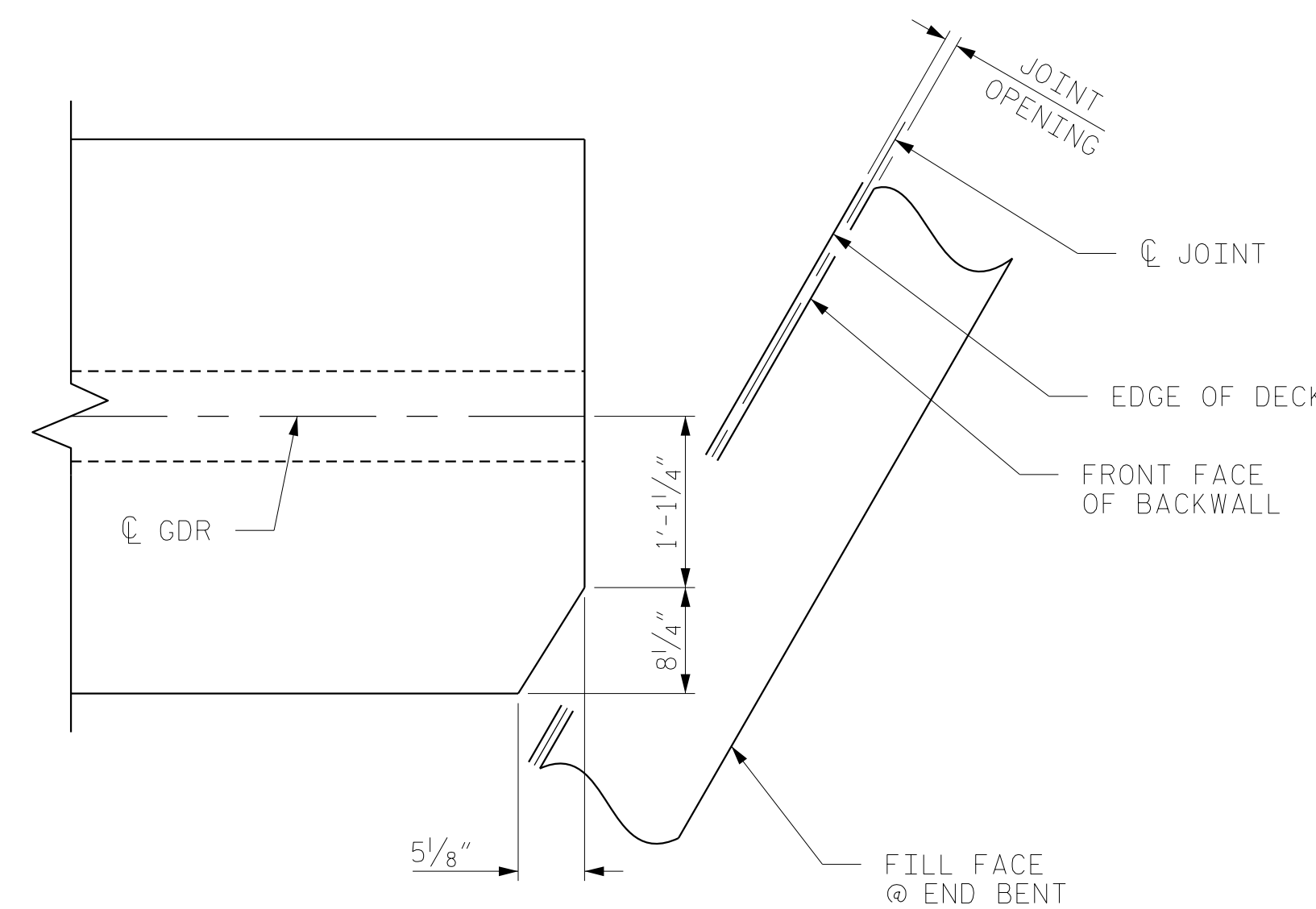


EMBEDDED PLATE "B-1" DETAILS FOR 72" MODIFIED BULB TEES

(2 REQ'D PER GIRDER)



END BENT 1



END BENT 2

FLANGE CLIP DETAIL

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 4 OF 4



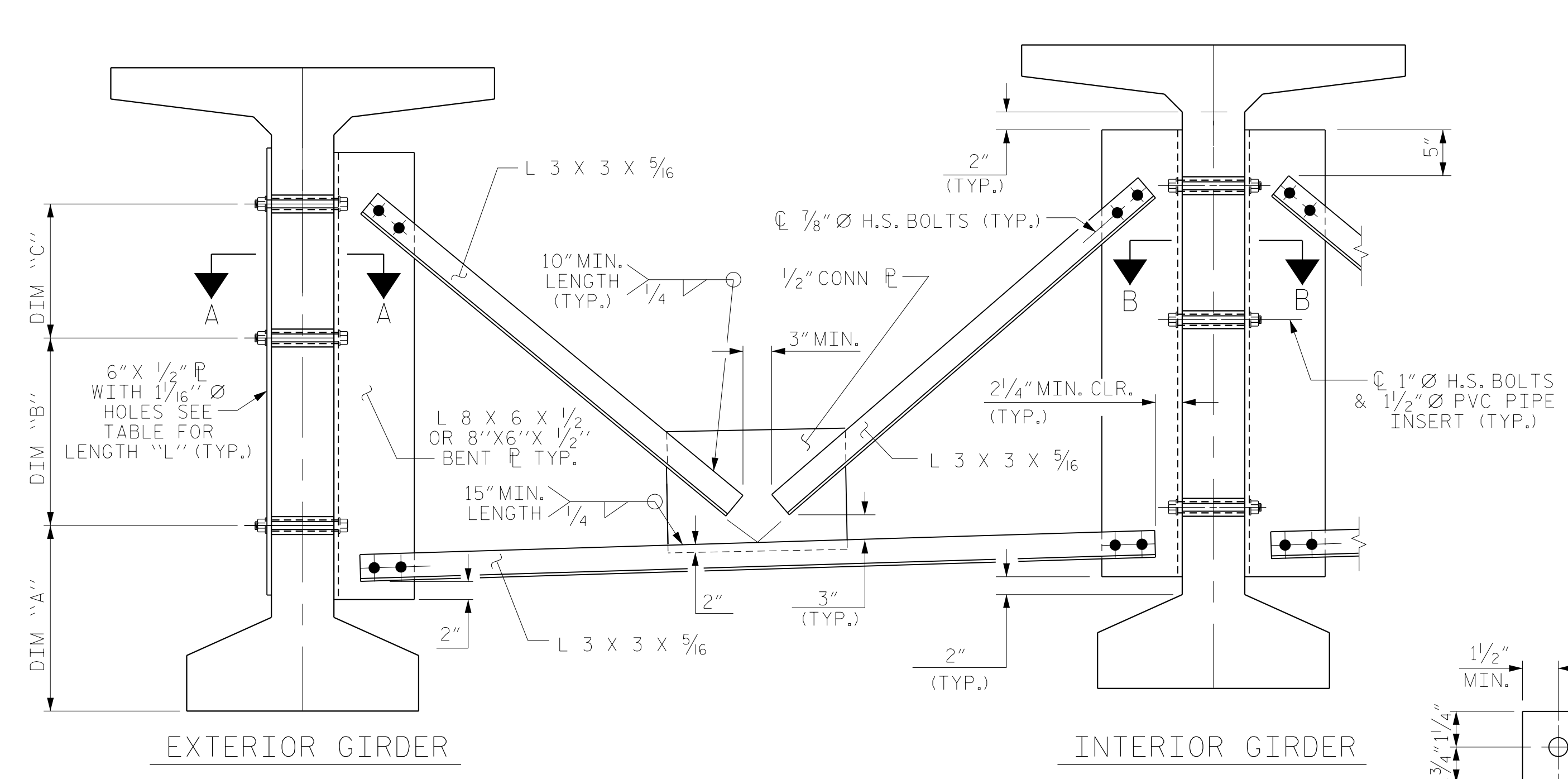
RS&H
 RS&H Architects-Engineers-Planners, Inc.
 8601 Six Forks Road, Suite 280
 Raleigh, NC 27615
 919-926-4100 FAX 919-846-9080
 www.rsandh.com
 North Carolina License No. 50737-5403-C-28

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 PRESTRESSED CONCRETE GIRDER
 CONTINUOUS FOR LIVE LOAD
 DETAILS

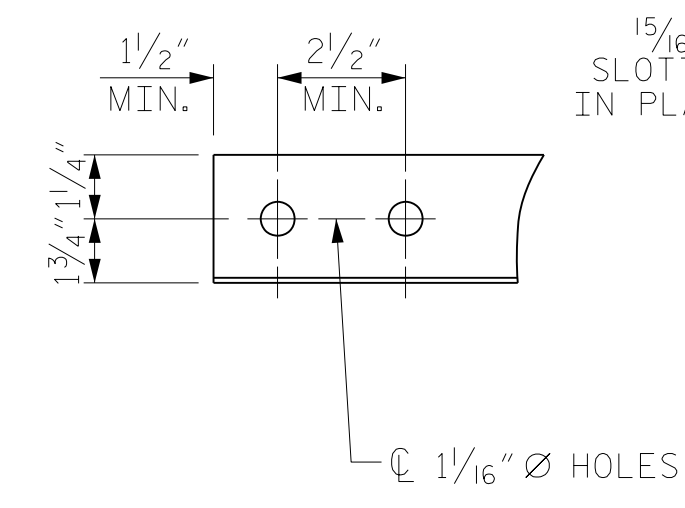
ASSEMBLED BY :	MKO	DATE :	11/2016
CHECKED BY :	JMR	DATE :	12/2016
DRAWN BY :	ELR 11/91	REV. 10/1/11	MAA/TMG
CHECKED BY :	GRP 11/91	REV. 1/15	MAA/TMG
		REV. 2/15	MAA/TMG

DOCUMENT NOT CONSIDERED
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 SIGNATURES COMPLETED

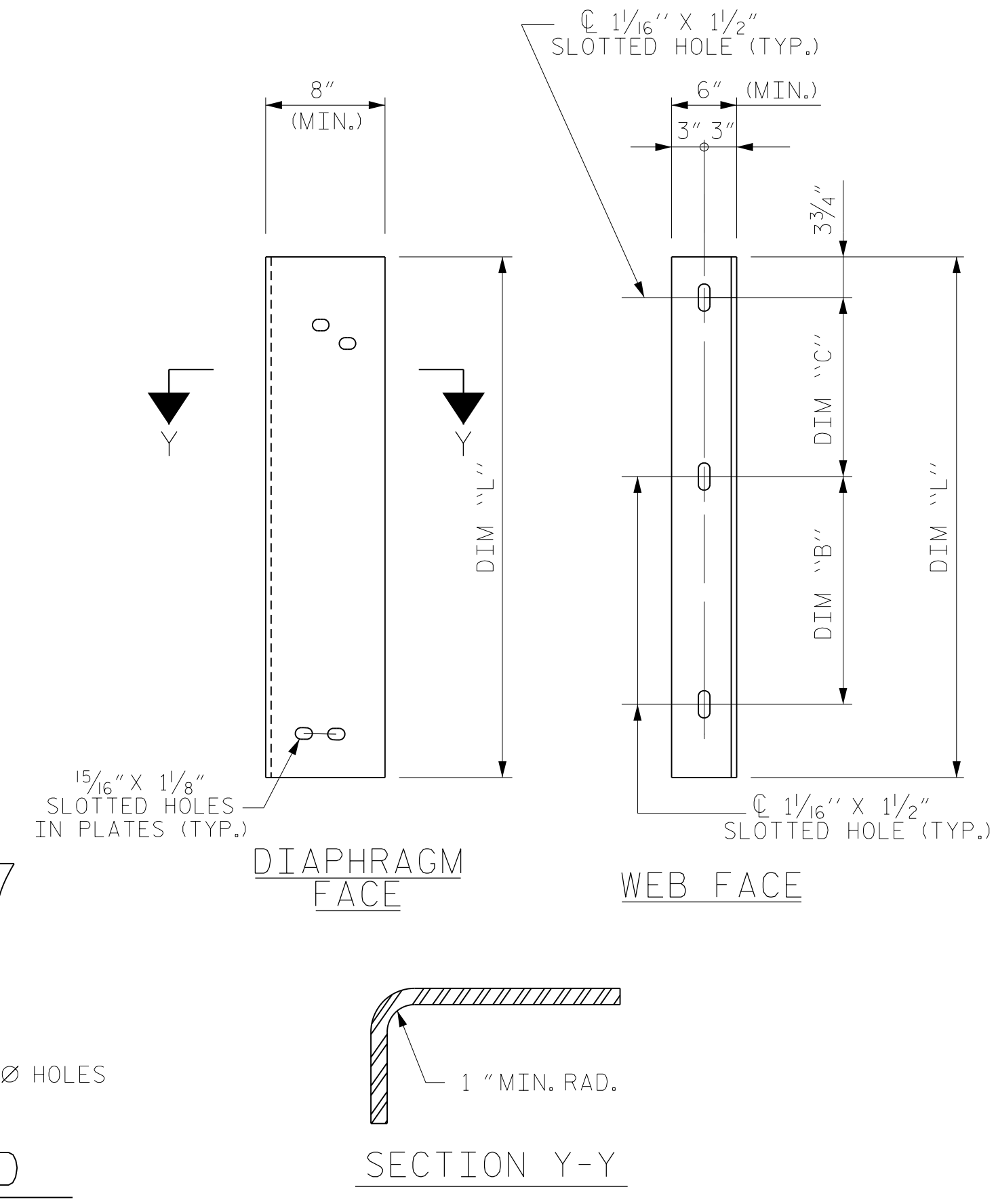
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			36
2			4			



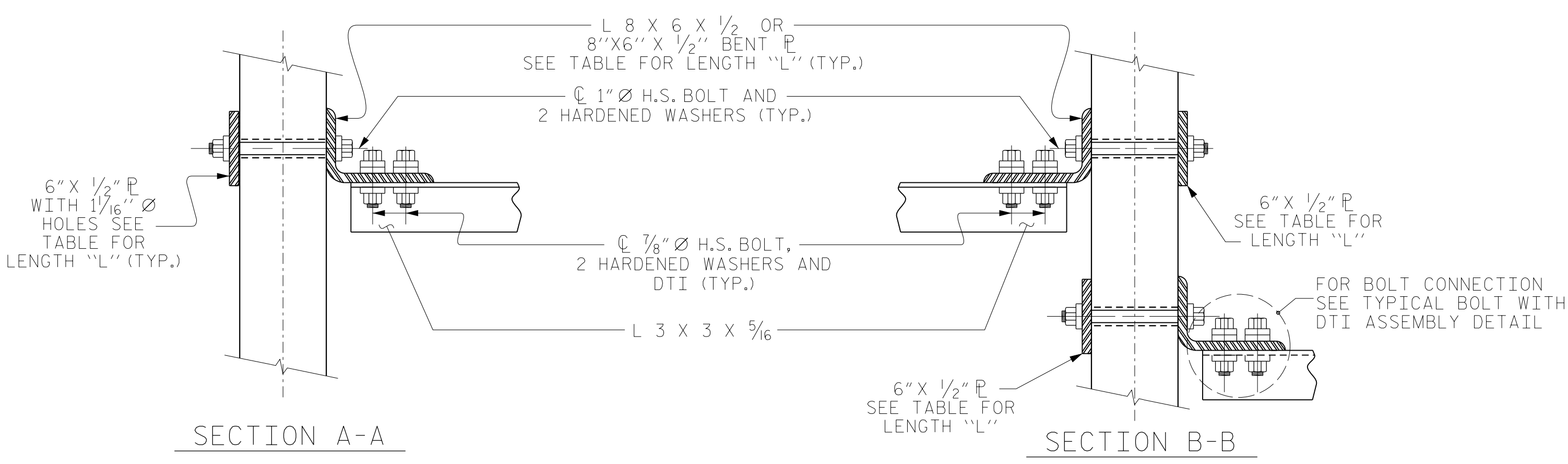
PART SECTION AT INTERMEDIATE DIAPHRAGM
(72" BULB TEE GIRDER)



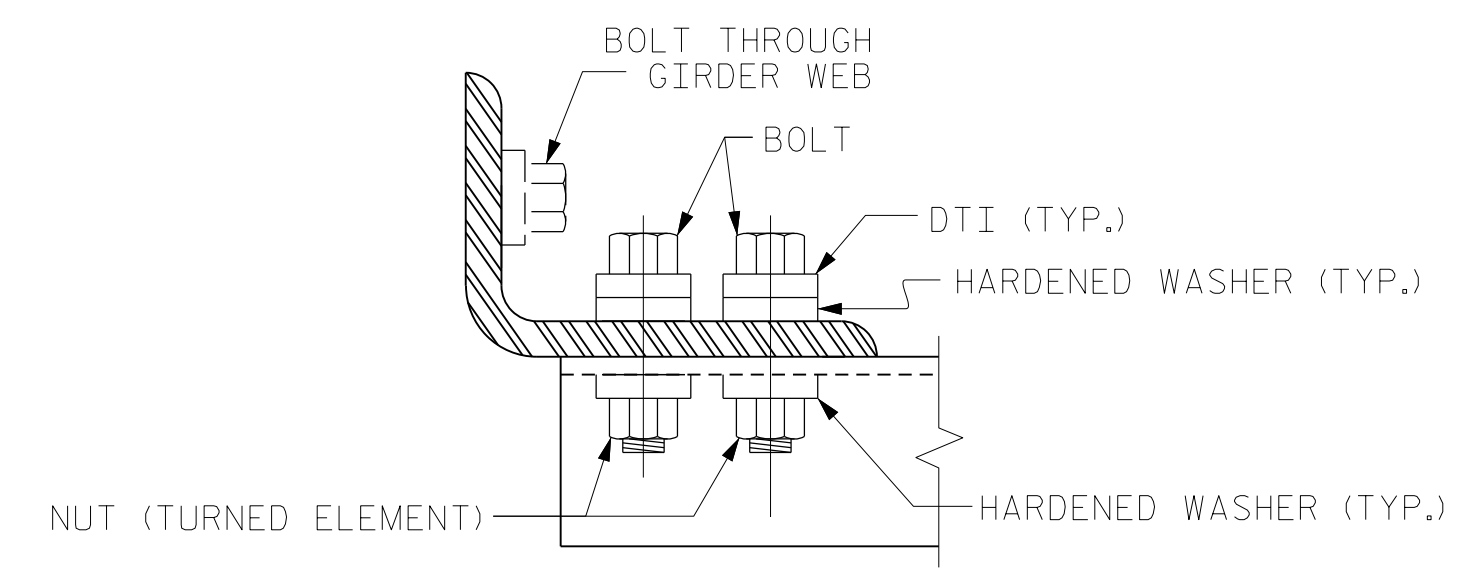
ANGLE END
(L 3 X 3 X 5/16)



CONNECTOR PLATE DETAIL



CONNECTION DETAILS



BOLT WITH DTI ASSEMBLY DETAIL

STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

TABLE

GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"	DIM "L"
72" BULB TEE	1'-8 3/4"	1'-7"	1'-7"	4'-2"

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

RS&H
 RS&H Architects-Engineers-Planners, Inc.
 8601 Six Forks Road, Suite 260
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 919-926-4100 FAX 919-846-9080
 www.rsandh.com
 North Carolina License Nos. 50737-F-0403-C-02

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						SHEET NO.
STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR 72" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS						S3-17
REVISIONS						TOTAL SHEETS
NO.	BY:	DATE:	NO.	BY:	DATE:	36
1			3			
2			4			

ASSEMBLED BY : MKO	DATE : 11/2016
CHECKED BY : JMR	DATE : 12/2016
DRAWN BY : RWW 11/09	ADDED 11/23/09R
CHECKED BY : GM 11/09	REV. 10/1/11 MAA/GM

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

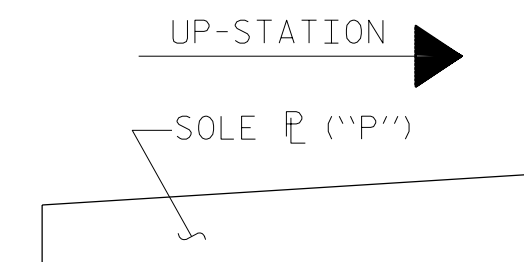
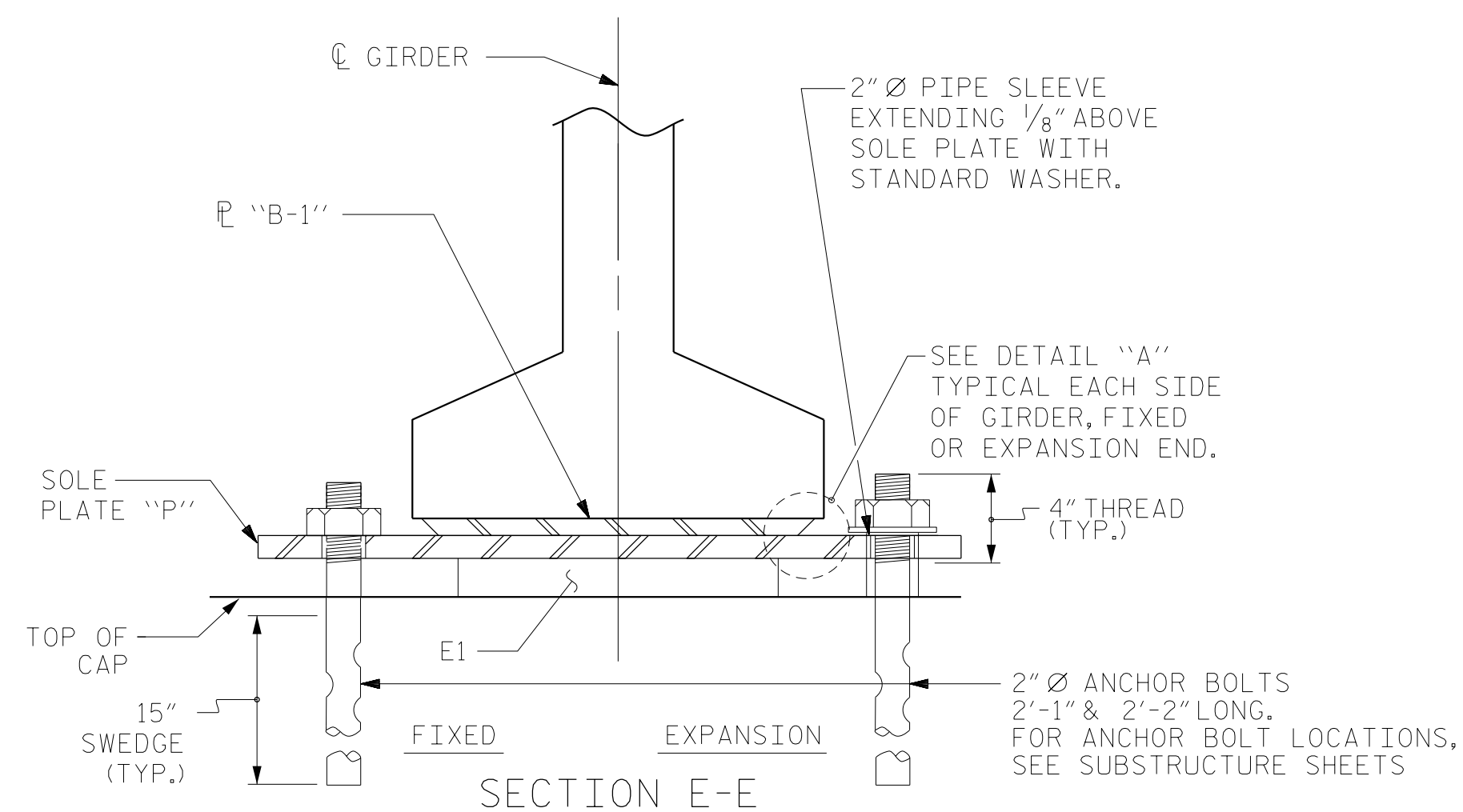
SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. NO SHOP DRAWINGS ARE REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

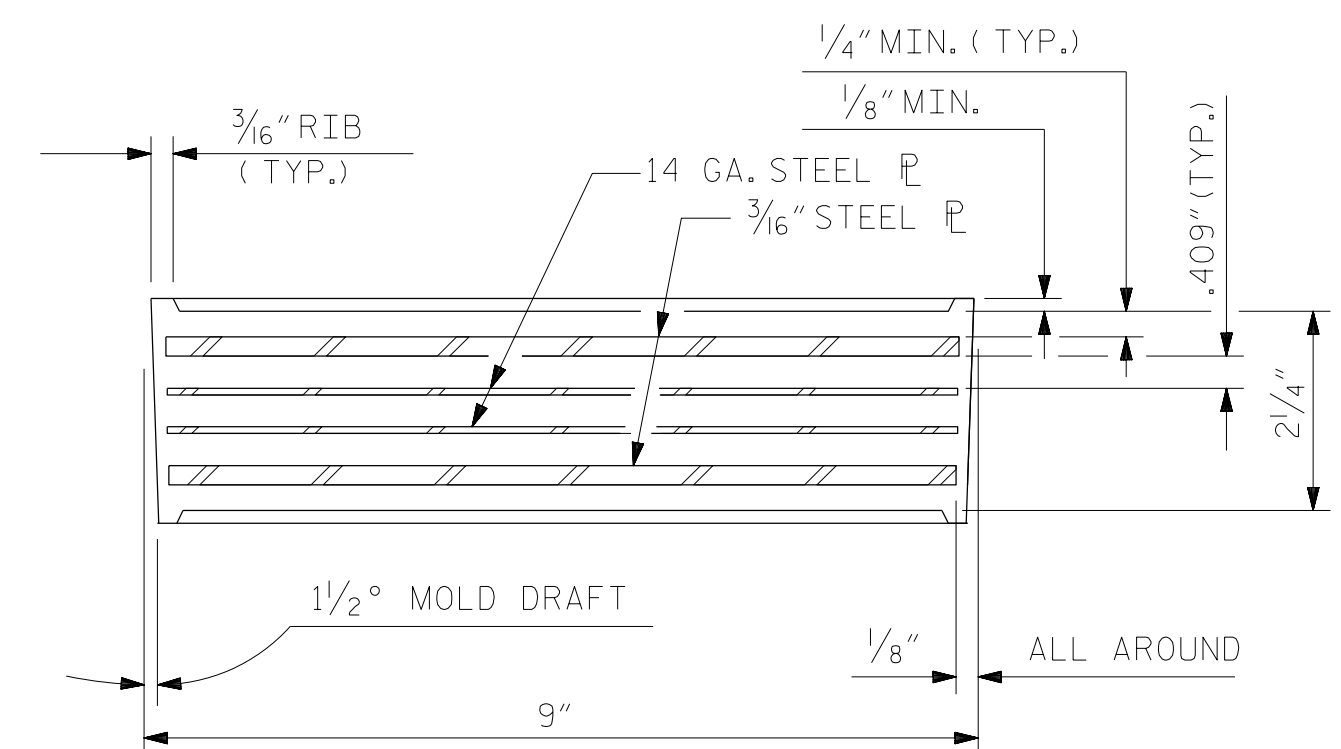
ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

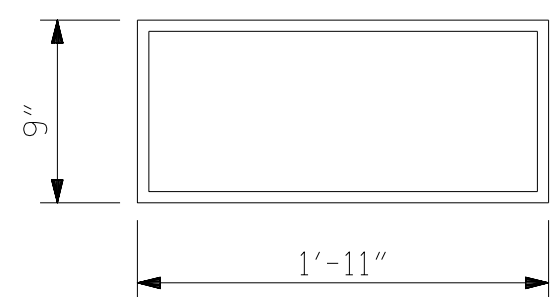
FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.



SOLE P PLACEMENT DETAIL



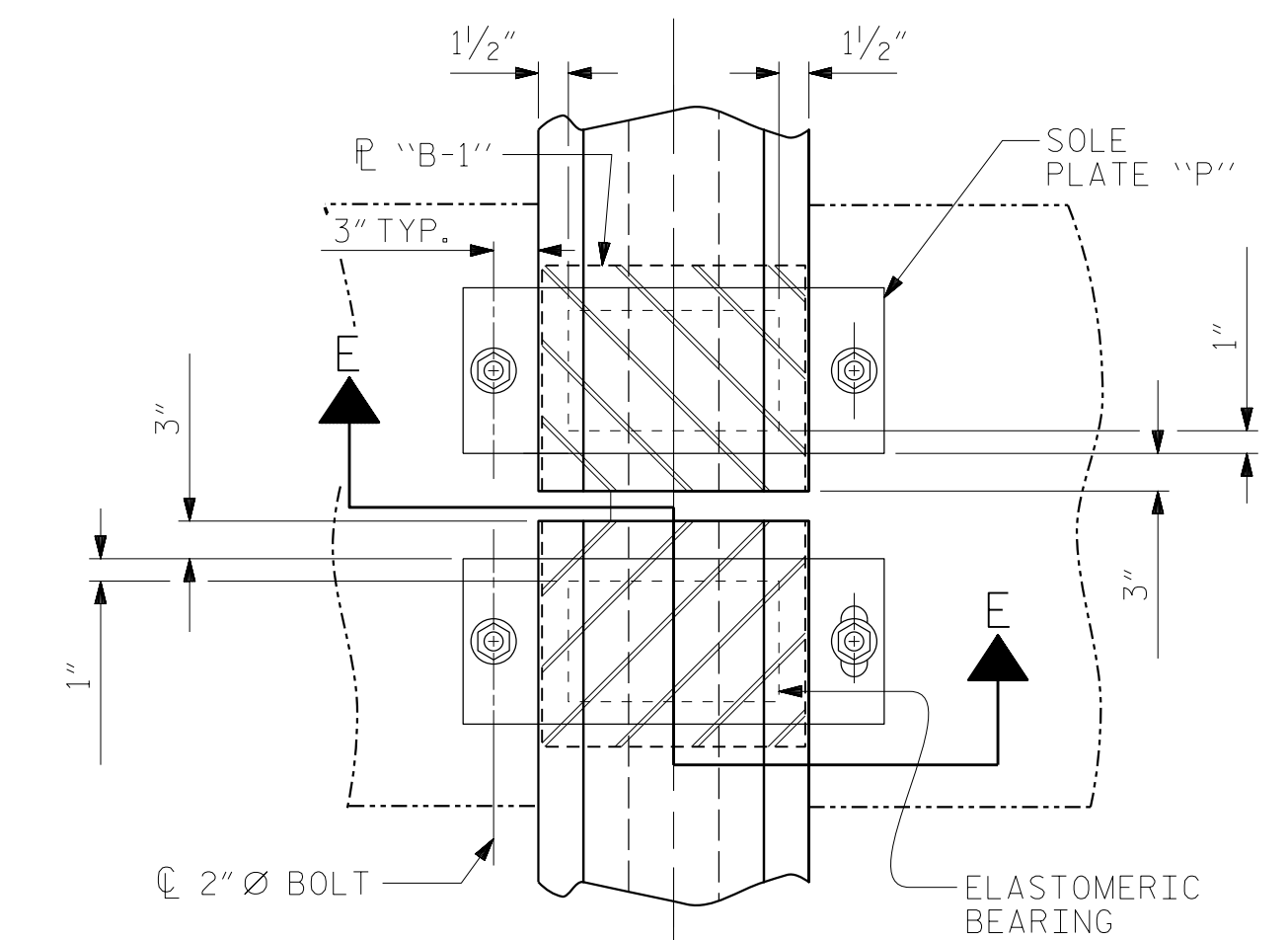
TYPICAL SECTION OF ELASTOMERIC BEARINGS



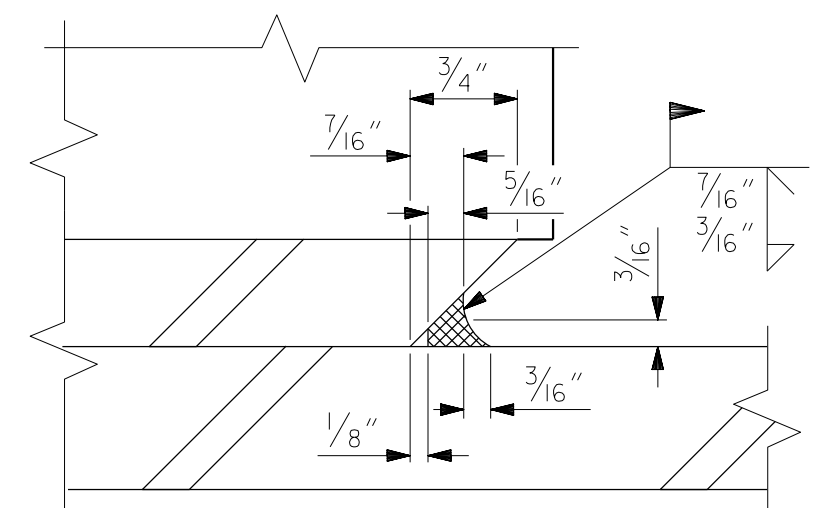
E1 (20 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

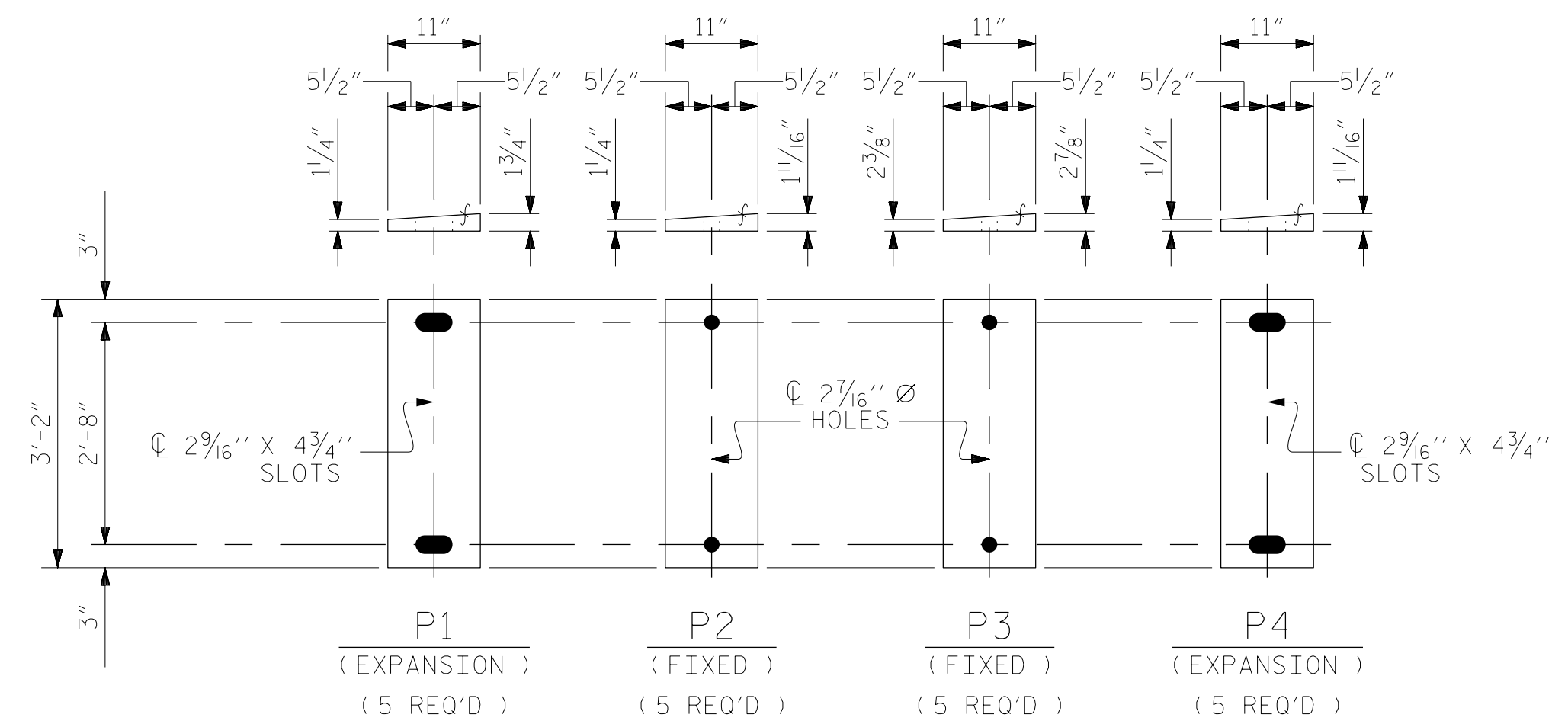
TYPE V



TYPICAL HALF-PLAN (SHOWING CONTINUOUS BENT) TYPICAL HALF-PLAN (SHOWING SIMPLE SPAN BENT)



DETAIL "A"



SOLE PLATE DETAILS ("P")

MAXIMUM ALLOWABLE SERVICE LOADS	
D.L.+L.L. (NO IMPACT)	
TYPE V	365 k

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

ASSEMBLED BY : MKO	DATE : 11/2016
CHECKED BY : JMR	DATE : 12/2016
DRAWN BY : EEM 2/97	REV. 10/1/11 MAA/GM
CHECKED BY : VAP 2/97	REV. 6/13 AAC/MAA
	REV. 1/15 MAA/TMG

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
STANDARD ELASTOMERIC BEARING DETAILS PRESTRESSED CONCRETE GIRDER SUPERSTRUCTURE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO. S3-18					TOTAL SHEETS 36

DEAD LOAD DEFLECTION TABLE FOR GIRDERS																						
0.6" Ø LOW RELAXATION	SPAN A																					
	GIRDER 1 (EXTERIOR)																					
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.031	0.062	0.091	0.117	0.140	0.160	0.176	0.187	0.195	0.197	0.195	0.187	0.176	0.160	0.140	0.117	0.091	0.062	0.031	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.012	0.025	0.036	0.046	0.056	0.063	0.070	0.074	0.077	0.078	0.077	0.074	0.070	0.063	0.056	0.046	0.036	0.025	0.012	0.000
FINAL CAMBER	↑	0	1/4"	7/16"	1 1/16"	7/8"	1"	1 3/16"	1 1/4"	1 3/8"	1 7/16"	1 7/16"	1 7/16"	1 3/8"	1 1/4"	1 3/16"	1"	7/8"	1 1/16"	7/16"	1/4"	0
0.6" Ø LOW RELAXATION	GIRDER 2 (INTERIOR)																					
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.031	0.062	0.090	0.117	0.140	0.160	0.176	0.187	0.194	0.197	0.194	0.187	0.176	0.160	0.140	0.117	0.090	0.062	0.031
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.013	0.025	0.036	0.047	0.056	0.064	0.070	0.075	0.078	0.079	0.078	0.075	0.070	0.064	0.056	0.047	0.036	0.025	0.013	0.000
FINAL CAMBER	↑	0	3/16"	7/16"	5/8"	13/16"	1"	1 1/8"	1 1/4"	1 3/8"	1 3/8"	1 7/16"	1 3/8"	1 3/8"	1 1/4"	1 1/8"	1"	13/16"	5/8"	7/16"	3/16"	0
0.6" Ø LOW RELAXATION	GIRDER 3 (INTERIOR)																					
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.031	0.062	0.090	0.117	0.140	0.160	0.175	0.187	0.194	0.196	0.194	0.187	0.175	0.160	0.140	0.117	0.090	0.062	0.031
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.012	0.025	0.036	0.046	0.056	0.064	0.070	0.074	0.077	0.078	0.077	0.074	0.070	0.064	0.056	0.046	0.036	0.025	0.012	0.000
FINAL CAMBER	↑	0	1/4"	7/16"	5/8"	7/8"	1"	1 1/8"	1 1/4"	1 3/8"	1 3/8"	1 7/16"	1 3/8"	1 3/8"	1 1/4"	1 1/8"	1"	7/8"	5/8"	7/16"	1/4"	0
0.6" Ø LOW RELAXATION	GIRDER 4 (INTERIOR)																					
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.031	0.062	0.090	0.116	0.140	0.159	0.175	0.187	0.194	0.196	0.194	0.187	0.175	0.159	0.140	0.116	0.090	0.062	0.031
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.012	0.024	0.036	0.046	0.055	0.063	0.069	0.074	0.077	0.078	0.077	0.074	0.069	0.063	0.055	0.046	0.036	0.024	0.012	0.000
FINAL CAMBER	↑	0	1/4"	7/16"	5/8"	13/16"	1"	1 1/8"	1 1/4"	1 3/8"	1 3/8"	1 7/16"	1 3/8"	1 3/8"	1 1/4"	1 1/8"	1"	13/16"	5/8"	7/16"	1/4"	0
0.6" Ø LOW RELAXATION	GIRDER 5 (EXTERIOR)																					
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.031	0.061	0.090	0.116	0.140	0.159	0.175	0.186	0.193	0.196	0.193	0.186	0.175	0.159	0.140	0.116	0.090	0.061	0.031
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.013	0.025	0.037	0.048	0.058	0.066	0.072	0.077	0.080	0.081	0.080	0.077	0.072	0.066	0.058	0.048	0.037	0.025	0.013	0.000
FINAL CAMBER	↑	0	3/16"	7/16"	5/8"	13/16"	1"	1 1/8"	1 1/4"	1 5/16"	1 3/8"	1 3/8"	1 3/8"	1 5/16"	1 1/4"	1 1/8"	1"	13/16"	5/8"	7/16"	3/16"	0

* INCLUDES SLAB, BUILDUPS, STAY-IN-PLACE FORMS, BARRIER RAIL AND FUTURE WEARING SURFACE.
 ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT " FINAL CAMBER ", WHICH IS GIVEN IN INCHES (FRACTION FORM).

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 1 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 DEAD LOAD DEFLECTION
 SPAN A

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-19
1			3			TOTAL SHEETS
2			4			36

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

DRAWN BY : MKO DATE : 12/2016
 CHECKED BY : JMR DATE : 12/2016
 DESIGN ENGINEER OF RECORD: MKO DATE : 12/2016

DEAD LOAD DEFLECTION TABLE FOR GIRDERS																						
0.6" Ø LOW RELAXATION	SPAN B																					
	GIRDER 1 (EXTERIOR)																					
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.031	0.062	0.091	0.117	0.141	0.160	0.176	0.188	0.195	0.197	0.195	0.188	0.176	0.160	0.141	0.117	0.091	0.062	0.031	0.000
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.013	0.025	0.036	0.047	0.056	0.064	0.070	0.075	0.078	0.079	0.078	0.075	0.070	0.064	0.056	0.047	0.036	0.025	0.013	0.000
FINAL CAMBER	↑	0	3/16"	7/16"	1 1/16"	1 3/16"	1"	1 1/8"	1 1/4"	1 3/8"	1 3/8"	1 7/16"	1 3/8"	1 3/8"	1 1/4"	1 1/8"	1"	1 3/16"	1 1/16"	7/16"	3/16"	0
0.6" Ø LOW RELAXATION	GIRDER 2 (INTERIOR)																					
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.031	0.062	0.090	0.117	0.140	0.160	0.176	0.187	0.194	0.197	0.194	0.187	0.176	0.160	0.140	0.117	0.090	0.062	0.031
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.013	0.025	0.036	0.047	0.056	0.064	0.071	0.075	0.078	0.079	0.078	0.075	0.071	0.064	0.056	0.047	0.036	0.025	0.013	0.000
FINAL CAMBER	↑	0	3/16"	7/16"	5/8"	1 3/16"	1"	1 1/8"	1 1/4"	1 3/8"	1 3/8"	1 7/16"	1 3/8"	1 3/8"	1 1/4"	1 1/8"	1"	1 3/16"	5/8"	7/16"	3/16"	0
0.6" Ø LOW RELAXATION	GIRDER 3 (INTERIOR)																					
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.031	0.062	0.090	0.116	0.140	0.159	0.175	0.187	0.194	0.196	0.194	0.187	0.175	0.159	0.140	0.116	0.090	0.062	0.031
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.012	0.024	0.036	0.046	0.056	0.063	0.070	0.074	0.077	0.078	0.077	0.074	0.070	0.063	0.056	0.046	0.036	0.024	0.012	0.000
FINAL CAMBER	↑	0	1/4"	7/16"	5/8"	1 3/16"	1"	1 1/8"	1 1/4"	1 3/8"	1 3/8"	1 7/16"	1 3/8"	1 3/8"	1 1/4"	1 1/8"	1"	1 3/16"	5/8"	7/16"	1/4"	0
0.6" Ø LOW RELAXATION	GIRDER 4 (INTERIOR)																					
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.031	0.061	0.090	0.116	0.139	0.159	0.175	0.186	0.193	0.196	0.193	0.186	0.175	0.159	0.139	0.116	0.090	0.061	0.031
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.012	0.024	0.035	0.046	0.055	0.063	0.069	0.073	0.076	0.077	0.076	0.073	0.069	0.063	0.055	0.046	0.035	0.024	0.012	0.000
FINAL CAMBER	↑	0	1/4"	7/16"	1 1/16"	1 3/16"	1"	1 1/8"	1 1/4"	1 3/8"	1 3/8"	1 7/16"	1 3/8"	1 3/8"	1 1/4"	1 1/8"	1"	1 3/16"	1 1/16"	7/16"	1/4"	0
0.6" Ø LOW RELAXATION	GIRDER 5 (EXTERIOR)																					
	TWENTIETH POINTS	0	.05	.10	.15	.20	.25	.30	.35	.40	.45	.50	.55	.60	.65	.70	.75	.80	.85	.90	.95	0
	CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.031	0.061	0.090	0.116	0.139	0.159	0.174	0.186	0.193	0.195	0.193	0.186	0.174	0.159	0.139	0.116	0.090	0.061	0.031
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0.000	0.013	0.025	0.037	0.047	0.057	0.065	0.071	0.076	0.079	0.080	0.079	0.076	0.071	0.065	0.057	0.047	0.037	0.025	0.013	0.000
FINAL CAMBER	↑	0	3/16"	7/16"	5/8"	1 3/16"	1"	1 1/8"	1 1/4"	1 5/16"	1 3/8"	1 3/8"	1 3/8"	1 5/16"	1 1/4"	1 1/8"	1"	1 3/16"	5/8"	7/16"	3/16"	0

* INCLUDES SLAB, BUILDUPS, STAY-IN-PLACE FORMS, BARRIERS AND FUTURE WEARING SURFACE
 ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT " FINAL CAMBER ", WHICH IS GIVEN IN INCHES (FRACTION FORM).

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

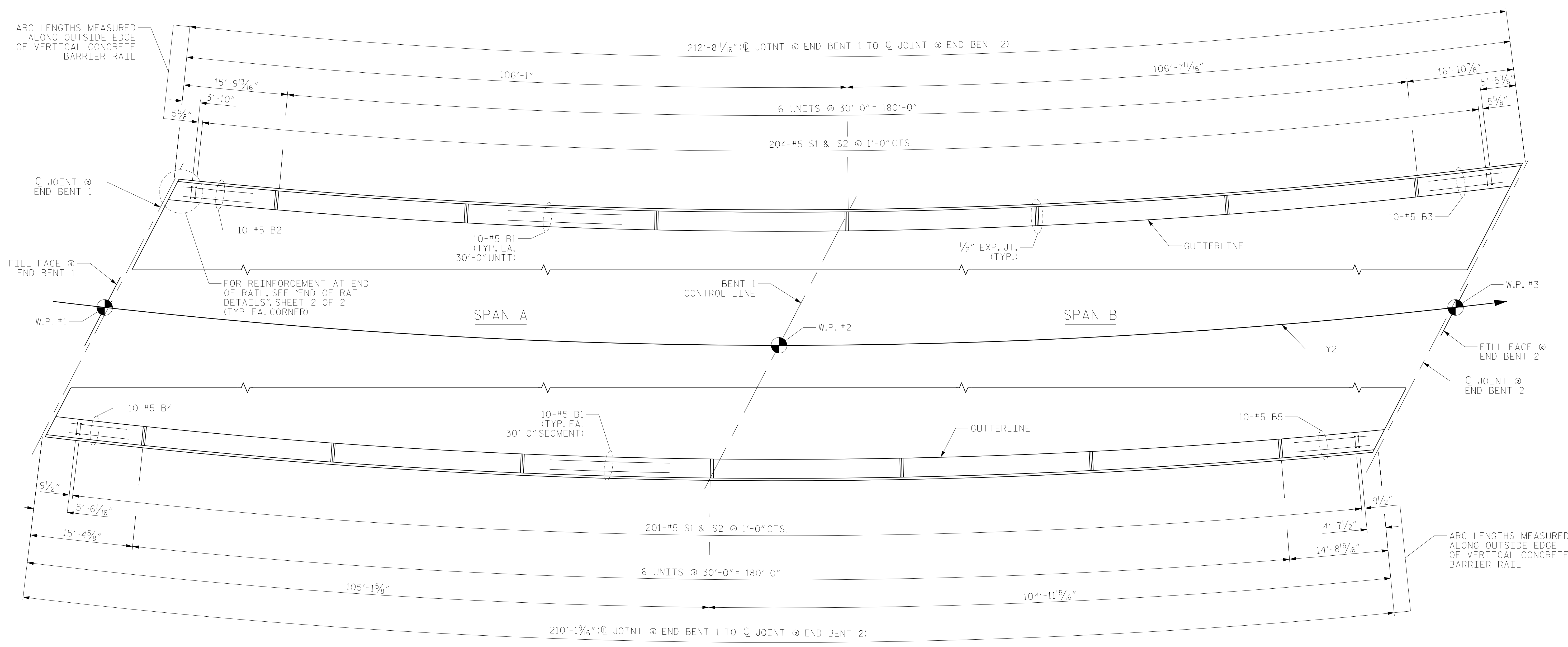
SHEET 2 OF 2



STATE OF NORTH CAROLINA					
DEPARTMENT OF TRANSPORTATION					
RALEIGH					
SUPERSTRUCTURE					
DEAD LOAD DEFLECTION					
SPAN B					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO.					S3-20
TOTAL SHEETS					36

DRAWN BY : MKO DATE : 12/2016
 CHECKED BY : JMR DATE : 12/2016
 DESIGN ENGINEER OF RECORD: MKO DATE : 12/2016

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PLAN OF BARRIER RAIL

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 1 OF 2

DRAWN BY : MKO DATE : 11/2016
 CHECKED BY : JMR DATE : 12/2016
 DESIGN ENGINEER OF RECORD : MKO DATE : 12/2016

2/29/2017
 R:\S\Structures\Working DGN\403.041_R2707C_SMU_BR21_53-21.dgn
 oconnor

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 RS&H Architects-Engineers-Planners, Inc.
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 Raleigh, NC 27615
 919-926-4100 FAX 919-846-9080
 www.rsandh.com
 North Carolina License No. 50737-F-0403-C-08

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 VERTICAL CONCRETE
 BARRIER RAIL

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-21
1			3			TOTAL SHEETS
2			4			36

NOTES

THE BARRIER RAIL IN EACH SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

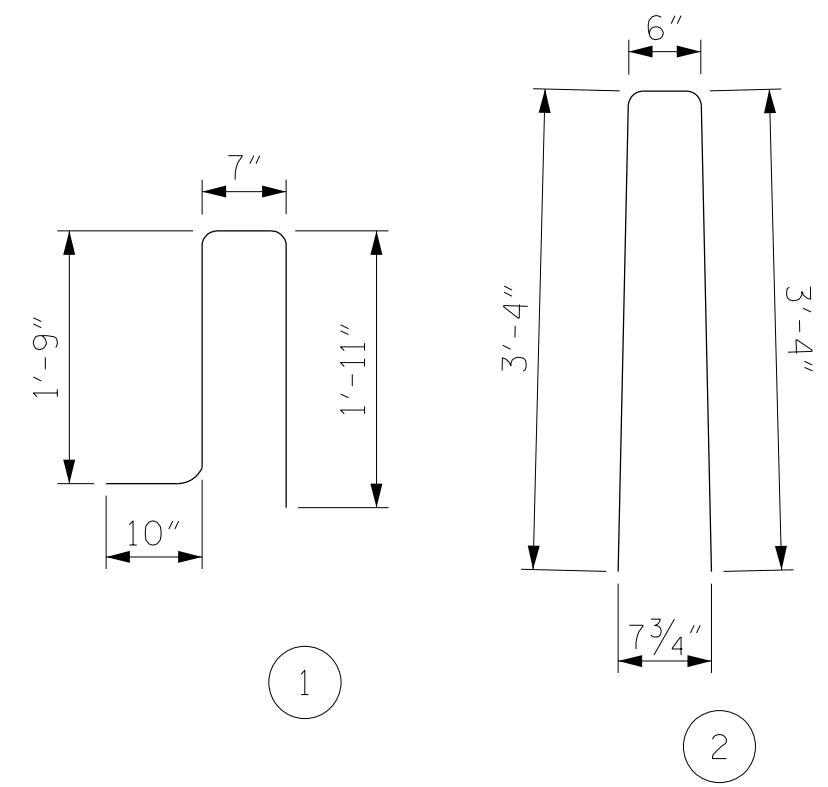
WHEN FOAM JOINT SEAL IS REQUIRED, THE JOINT IN THE DECK SHALL BE SAWED PRIOR TO THE CASTING OF VERTICAL CONCRETE BARRIER RAIL.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

THE #5 S3 & S4 BARS SHALL BE INSTALLED, USING AN ADHESIVE ANCHORING SYSTEM, AFTER SAWING THE JOINT. THE YIELD LOAD FOR THE #5 S3 & S4 BARS IS 18.6 KIPS. FIELD TESTING FOR THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS, THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

BAR TYPES

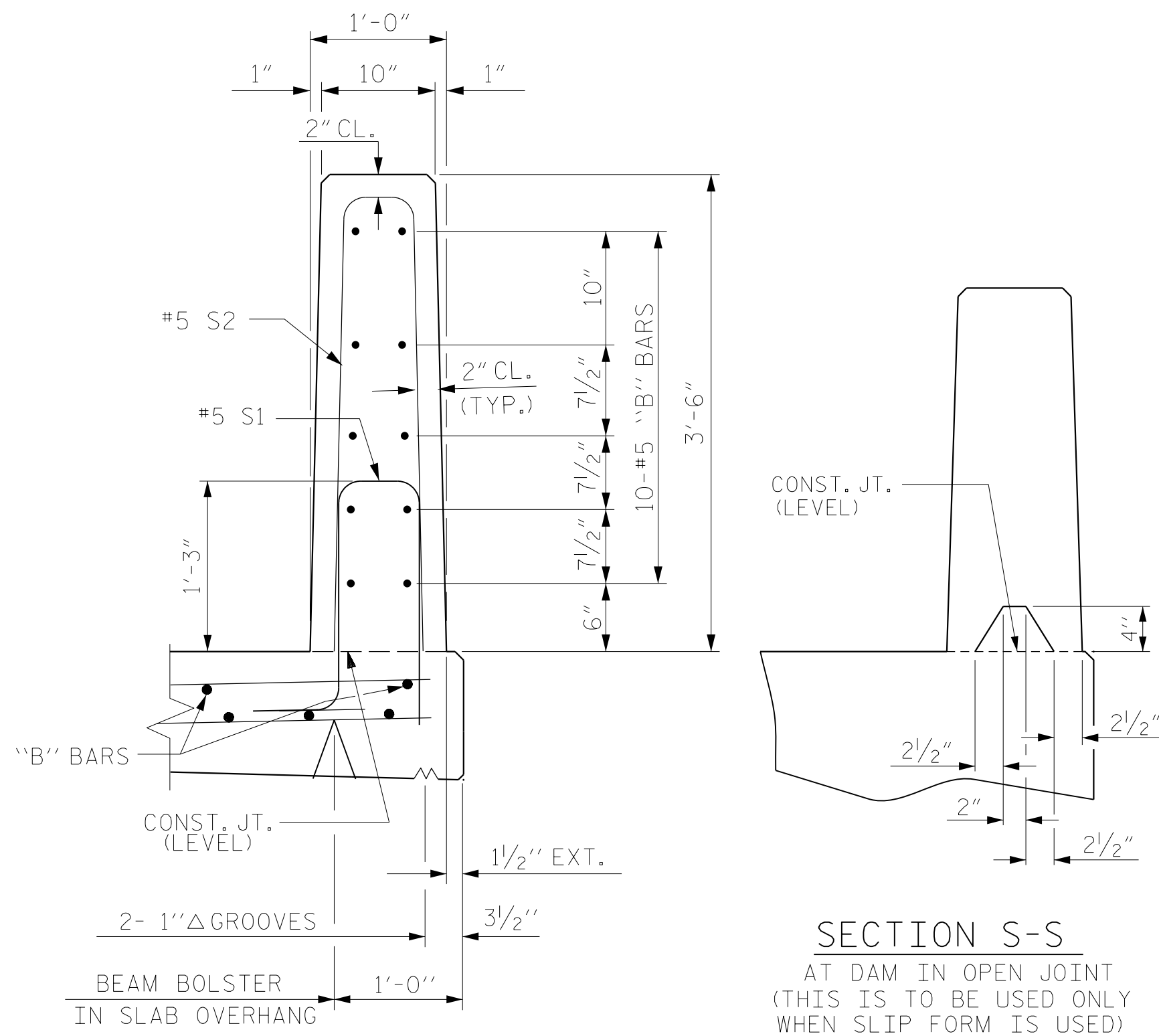


ALL BAR DIMENSIONS ARE OUT TO OUT

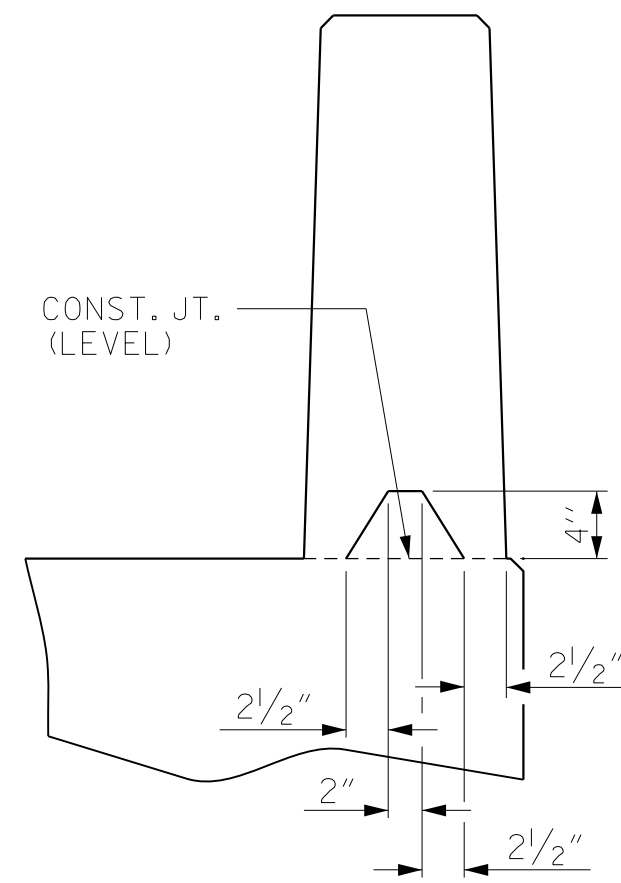
BILL OF MATERIAL

FOR VERTICAL CONCRETE BARRIER RAIL ONLY						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
* B1	120	#5	STR	29'-7"	3703	
* B2	10	#5	STR	15'-5"	161	
* B3	10	#5	STR	15'-11"	166	
* B4	10	#5	STR	14'-8"	153	
* B5	10	#5	STR	14'-5"	150	
* S1	415	#5	1	5'-1"	2200	
* S2	415	#5	2	7'-2"	3102	
* S3	16	#5	STR	4'-0"	67	
* S4	32	#5	STR	3'-6"	117	

* EPOXY COATED REINFORCING STEEL	9819 LBS.
CLASS AA CONCRETE	50.2 CU. YDS.
VERTICAL CONCRETE BARRIER RAIL	422.9 LIN. FT.

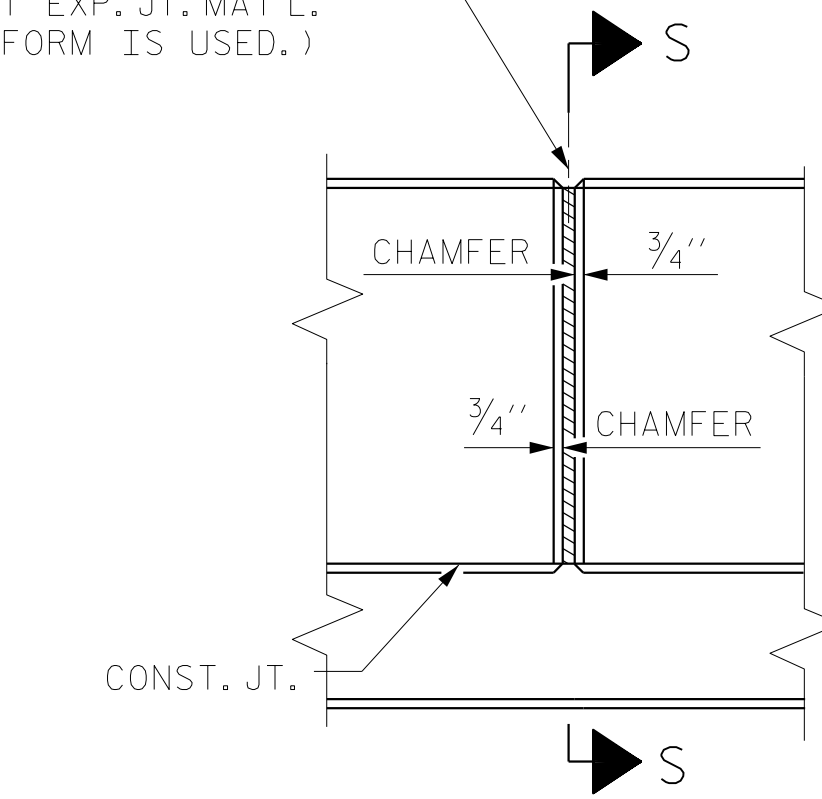


SECTION THRU RAIL



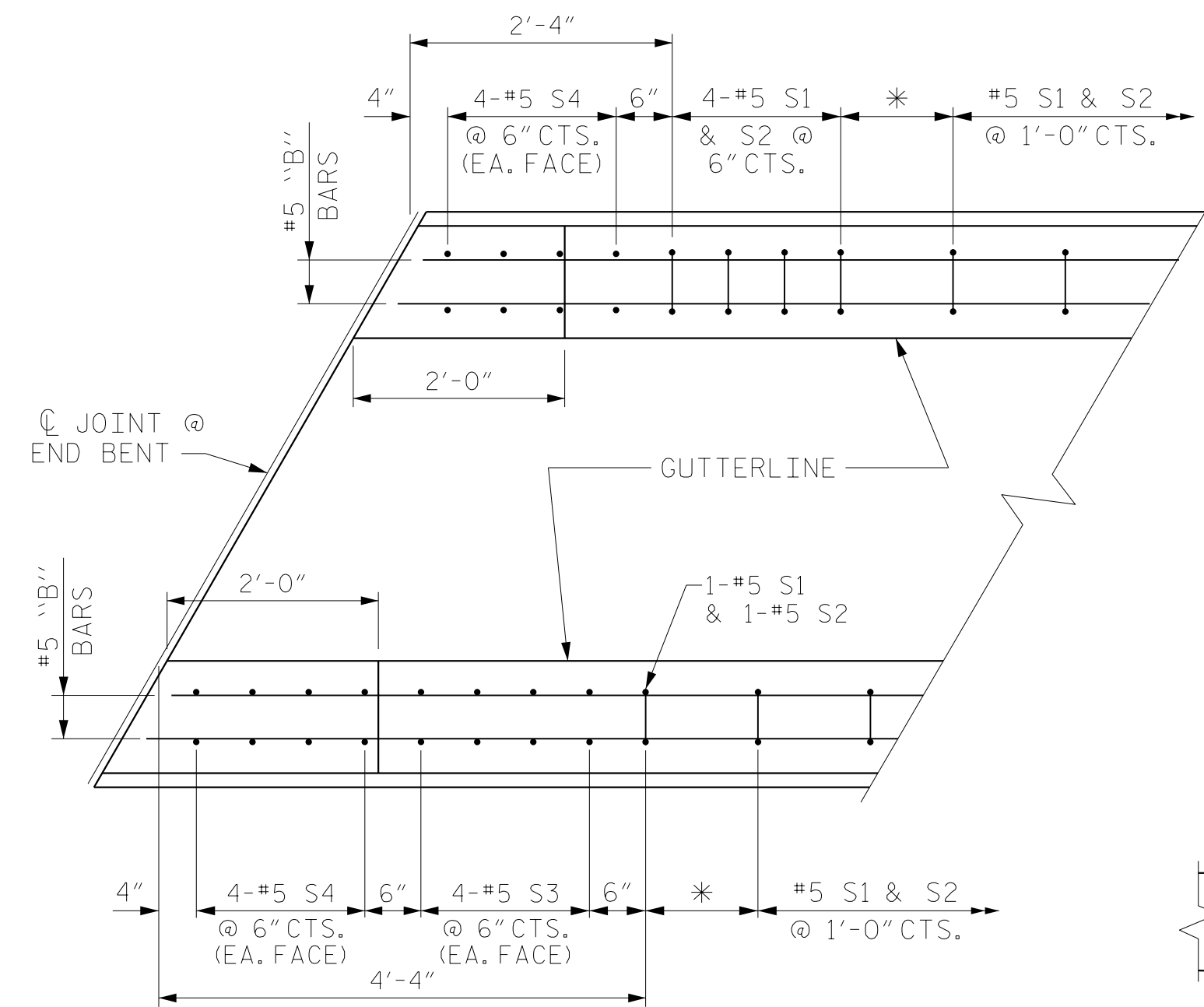
SECTION S-S
AT DAM IN OPEN JOINT
(THIS IS TO BE USED ONLY
WHEN SLIP FORM IS USED)

1/2" EXP. JT. MAT'L HELD IN PLACE WITH GALVANIZED NAILS.
(NOTE: OMIT EXP. JT. MAT'L. WHEN SLIP FORM IS USED.)



ELEVATION AT EXPANSION JOINTS

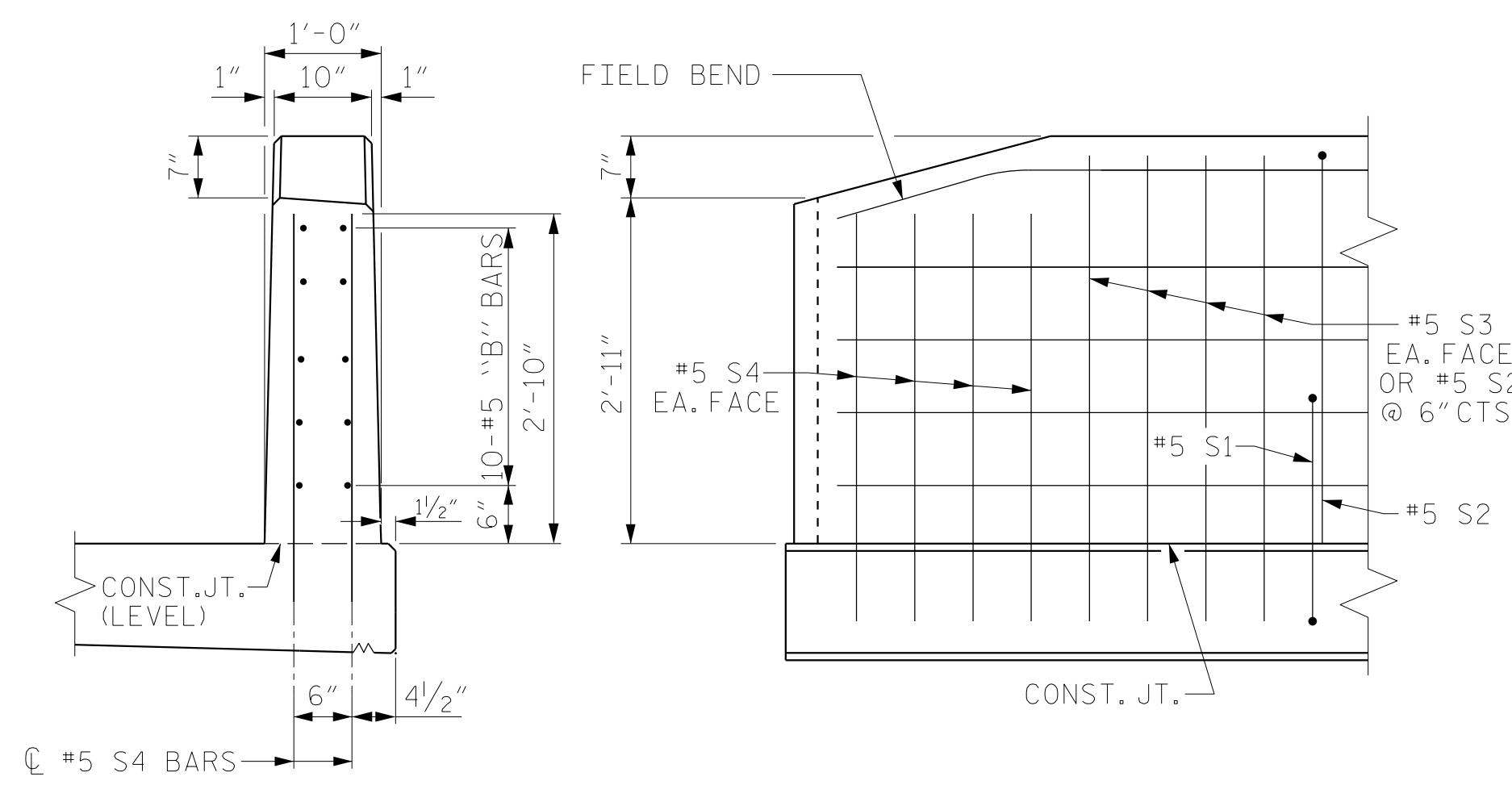
BARRIER RAIL DETAILS



PLAN

PLAN SHOWN TANGENT FOR CLARITY
DIMENSIONS SHOWN ARE ALONG ARC LENGTH

* FOR REINFORCEMENT SPACING, SEE "PLAN OF BARRIER RAIL" SHEET 1 OF 2.



END VIEW

SIDE VIEW

END OF RAIL DETAILS

FOR ADHESIVE ANCHORING AT SAWED JOINTS

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 29+07.16 -Y2-

SHEET 2 OF 2

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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
VERTICAL
CONCRETE
BARRIER RAIL

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-22
1			3			TOTAL SHEETS
2			4			36

DOCUMENT NOT CONSIDERED
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SIGNATURES COMPLETED

ASSEMBLED BY :	MKO	DATE :	11/2016
CHECKED BY :	JMR	DATE :	12/2016
DRAWN BY :	MAA 5/10	REV. 10/1/11	MAA/GM
CHECKED BY :	GM 5/10	REV. 12/5/11	MAA/GM
		REV. 6/13	MAA/GM

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 7/8" Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

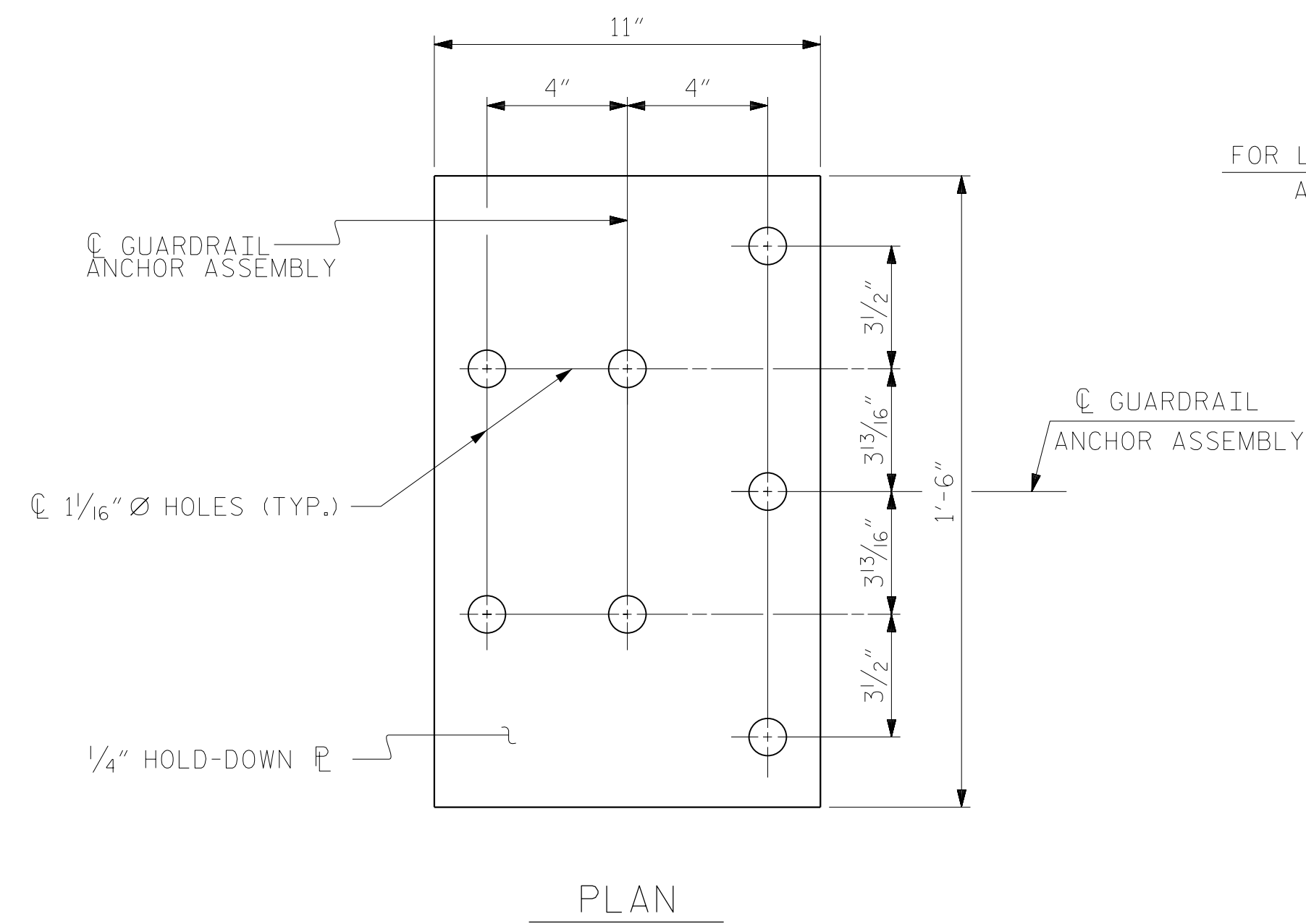
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

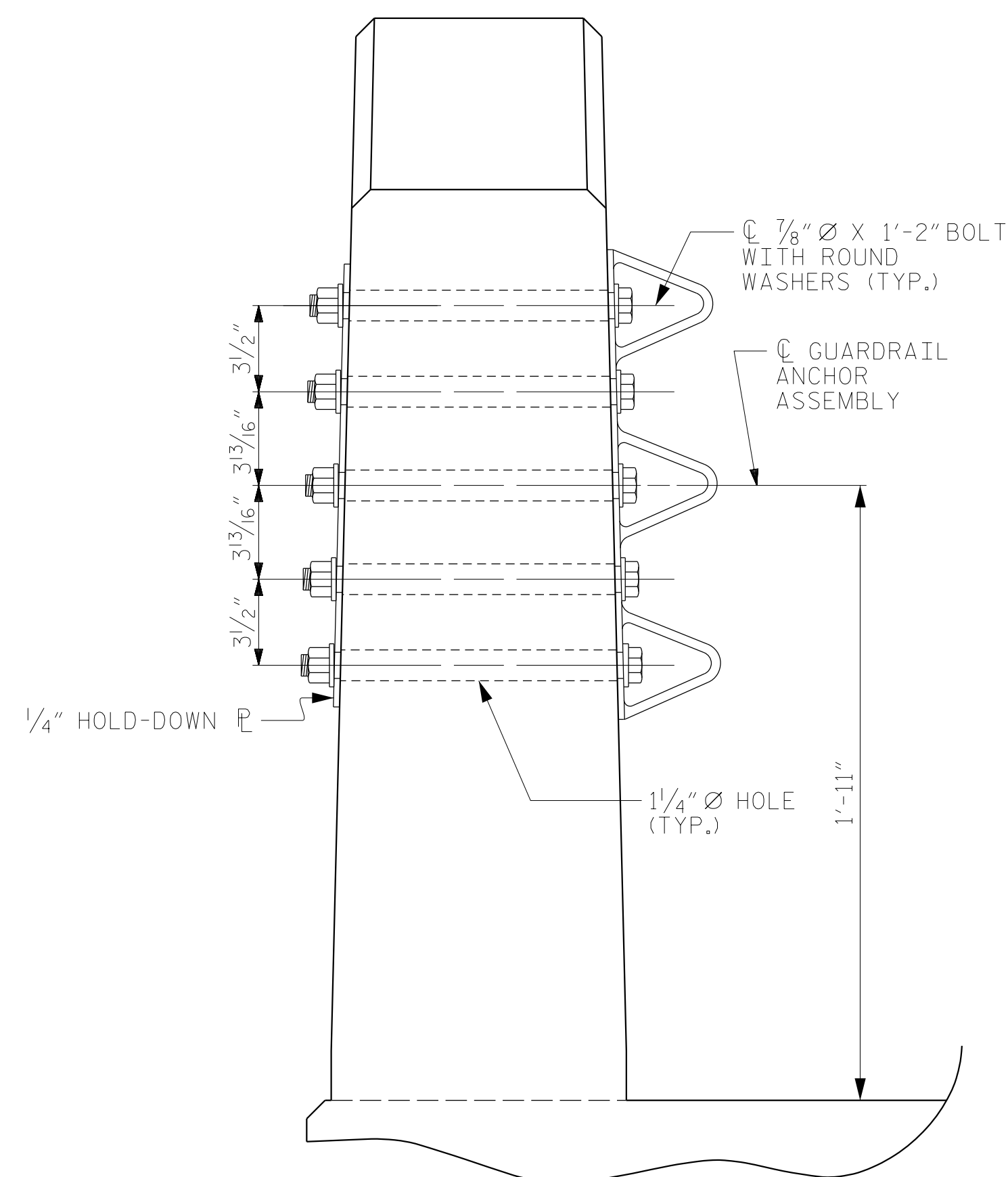
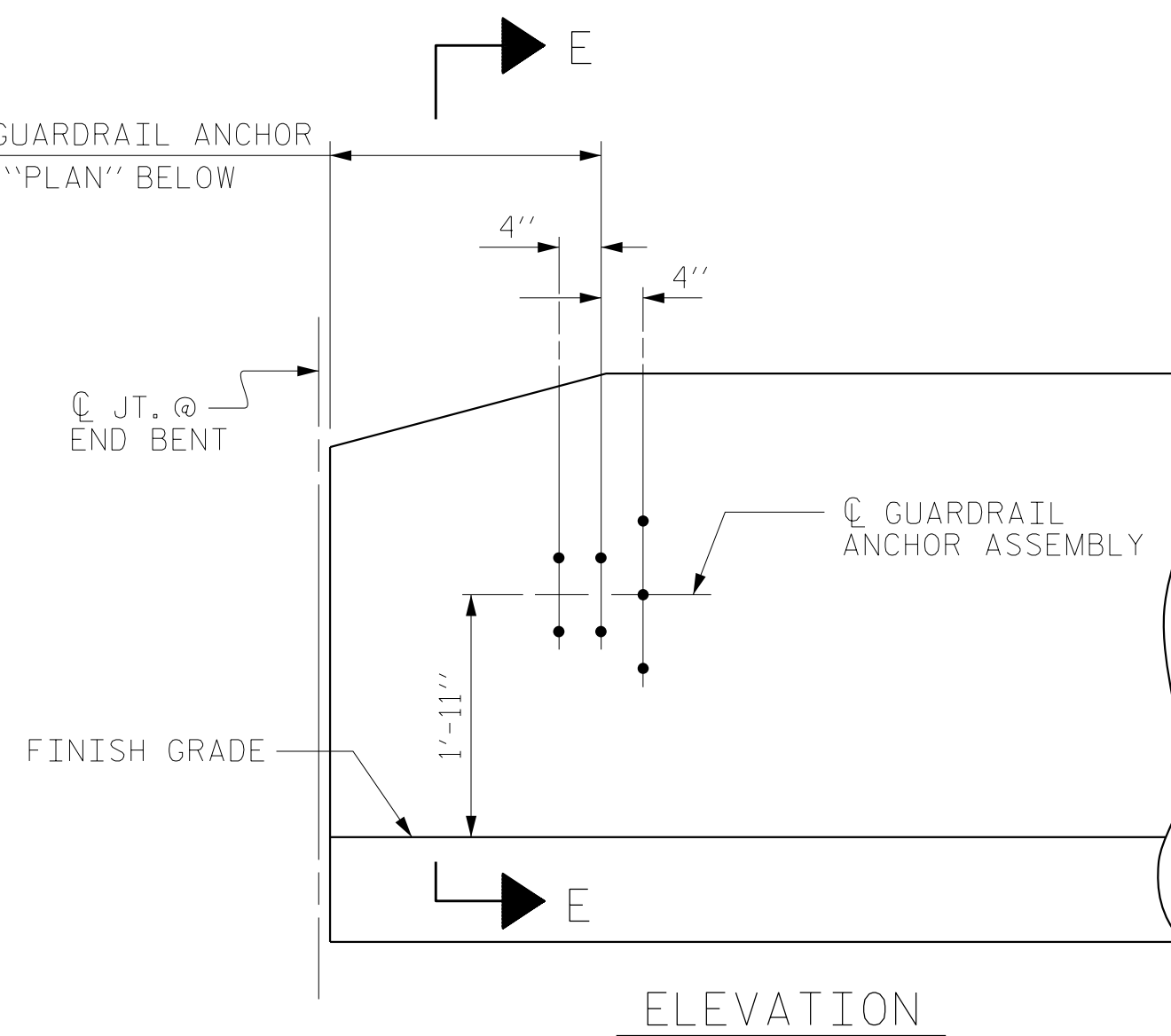
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

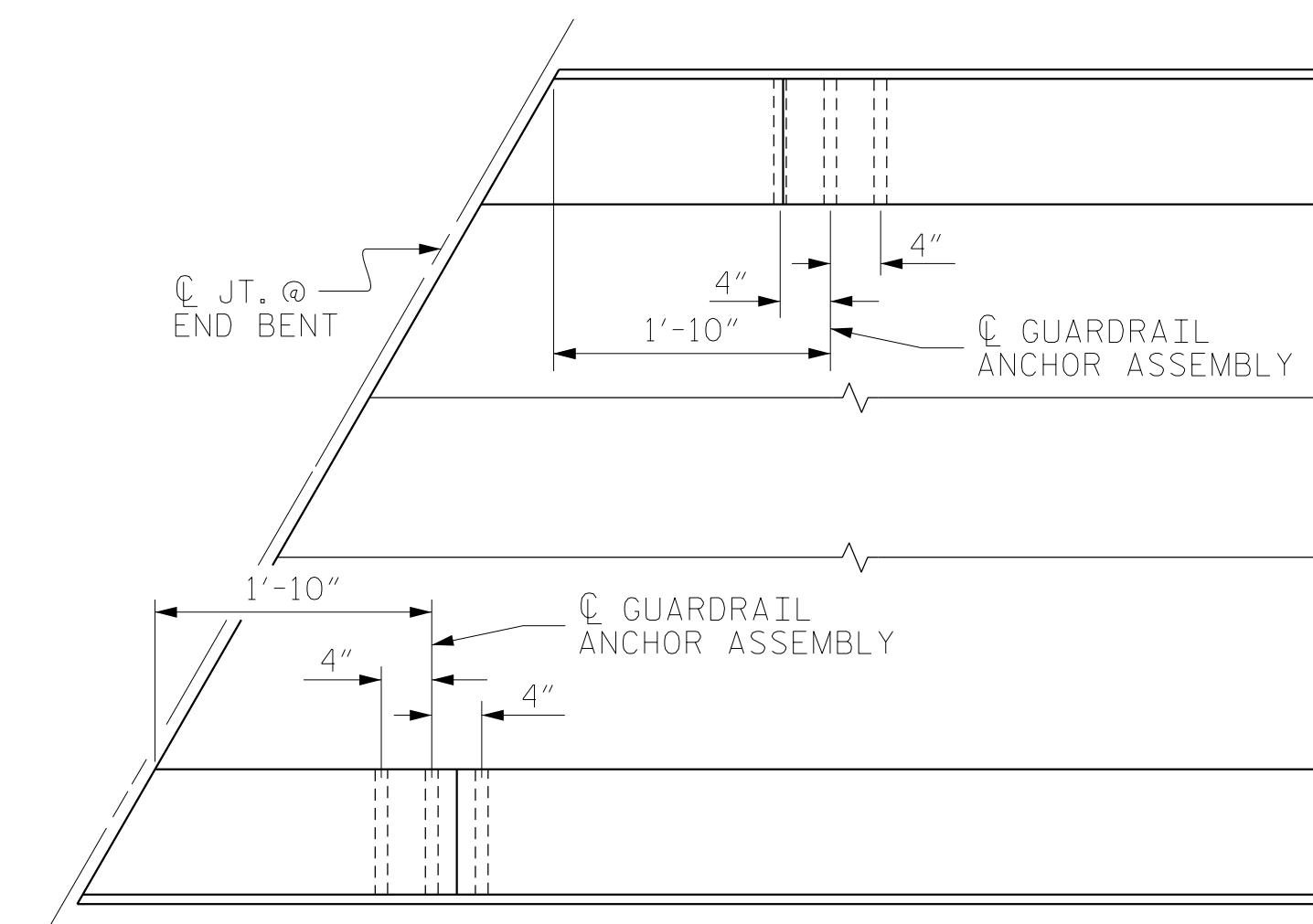
THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



FOR LOCATION OF GUARDRAIL ANCHOR ASSEMBLY, SEE "PLAN" BELOW

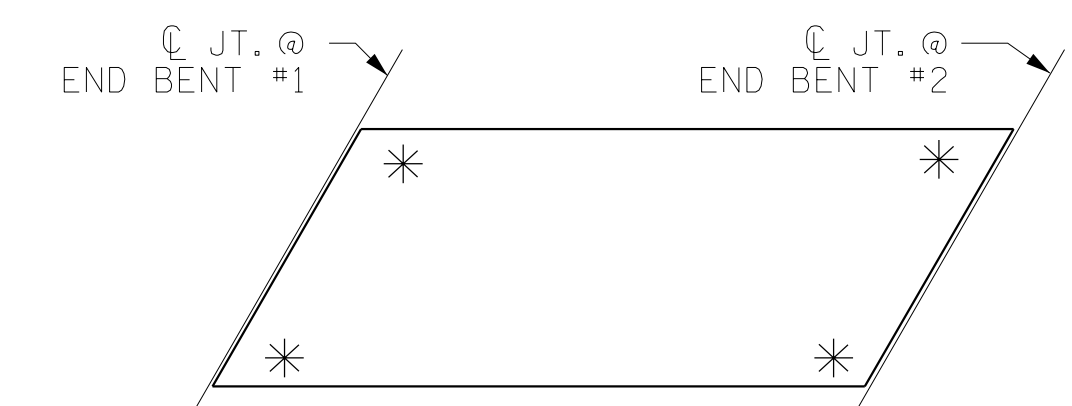


SECTION E-E
GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF ANCHORS FOR GUARDRAIL

END BENT 1 SHOWN, END BENT 2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-



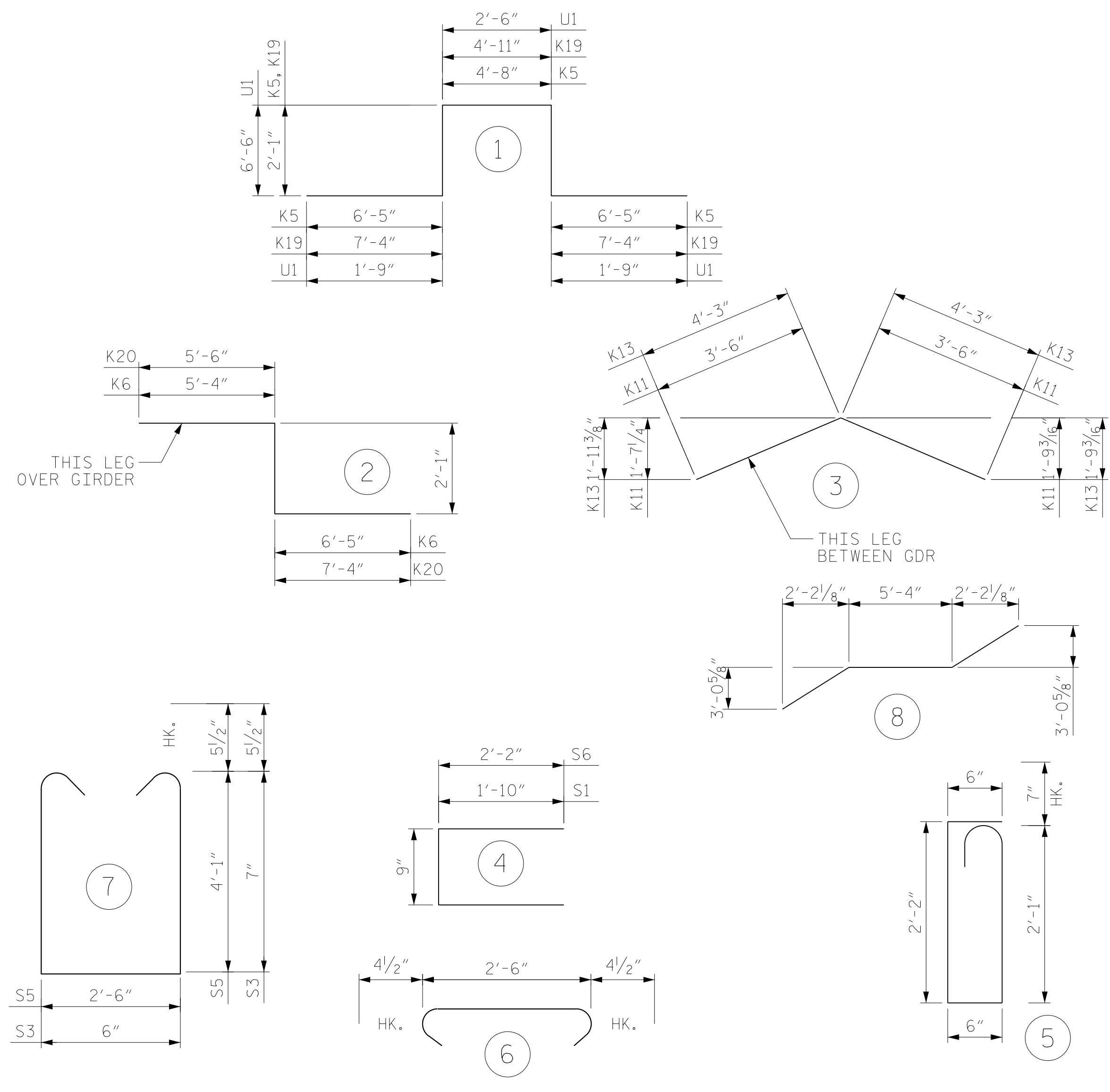
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 GUARDRAIL ANCHORAGE
 DETAILS
 FOR VERTICAL CONCRETE
 BARRIER RAIL

ASSEMBLED BY :	MKO	DATE :	12/2016
CHECKED BY :	JMR	DATE :	12/2016
DRAWN BY :	MAA 5/10	REV. 12/5/11	MAA/GM
CHECKED BY :	GM 5/10	REV. 6/13	MAA/GM
		REV. 1/15	MAA/TMG

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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-23
1			3			TOTAL SHEETS
2			4			36

BAR TYPE



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL

BAR NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR NO.	SIZE	TYPE	LENGTH	WEIGHT				
*A101	301	#5	STR	41'-11"	13159	*A150	1	#5	STR	23'-8"	25	A228	1	#5	STR	35'-7"	37	B201	120	#5	STR	54'-9"	6853
*A102	1	#5	STR	2'-6"	3	*A151	1	#5	STR	22'-7"	24	A229	1	#5	STR	36'-10"	38						
*A103	1	#5	STR	3'-9"	4	*A152	1	#5	STR	21'-5"	22	A230	1	#5	STR	38'-1"	40	* G1	1	#5	STR	44'-11"	47
*A104	1	#5	STR	5'-0"	5	*A153	1	#5	STR	20'-3"	21	A231	1	#5	STR	39'-4"	41	* G2	1	#5	STR	50'-5"	53
*A105	1	#5	STR	6'-4"	7	*A154	1	#5	STR	19'-2"	20	A232	1	#5	STR	40'-7"	42						
*A106	1	#5	STR	7'-7"	8	*A155	1	#5	STR	18'-0"	19	A233	1	#5	STR	41'-11"	44	* K1	4	#6	STR	7'-7"	46
*A107	1	#5	STR	8'-10"	9	*A156	1	#5	STR	16'-11"	18	A234	1	#5	STR	41'-11"	44	* K2	4	#6	STR	6'-5"	39
*A108	1	#5	STR	10'-2"	11	*A157	1	#5	STR	15'-9"	16	A235	1	#5	STR	40'-9"	43	* K3	4	#6	STR	5'-2"	31
*A109	1	#5	STR	11'-5"	12	*A158	1	#5	STR	14'-7"	15	A236	1	#5	STR	39'-7"	41	K4	8	#4	STR	8'-2"	44
*A110	1	#5	STR	12'-8"	13	*A159	1	#5	STR	13'-6"	14	A237	1	#5	STR	38'-6"	40	* K5	6	#8	1	21'-8"	347
*A111	1	#5	STR	13'-11"	15	*A160	1	#5	STR	12'-4"	13	A238	1	#5	STR	37'-4"	39	* K6	4	#8	2	13'-10"	148
*A112	1	#5	STR	15'-3"	16	*A161	1	#5	STR	11'-3"	12	A239	1	#5	STR	36'-2"	38	K7	40	#4	STR	8'-7"	229
*A113	1	#5	STR	16'-6"	17	*A162	1	#5	STR	10'-1"	11	A240	1	#5	STR	35'-1"	37	K8	8	#4	STR	6'-9"	36
*A114	1	#5	STR	17'-9"	19	*A163	1	#5	STR	9'-0"	9	A241	1	#5	STR	33'-11"	35	K9	8	#4	STR	5'-3"	28
*A115	1	#5	STR	19'-1"	20	*A164	1	#5	STR	7'-10"	8	A242	1	#5	STR	32'-9"	34	K10	10	#4	STR	20'-5"	136
*A116	1	#5	STR	20'-4"	21	*A165	1	#5	STR	6'-9"	7	A243	1	#5	STR	31'-8"	33	K11	2	#4	3	7'-0"	9
*A117	1	#5	STR	21'-7"	23	*A166	1	#5	STR	5'-7"	6	A244	1	#5	STR	30'-6"	32	K12	3	#4	8	12'-10"	26
*A118	1	#5	STR	22'-10"	24	*A167	1	#5	STR	4'-6"	5	A245	1	#5	STR	29'-4"	31	K13	2	#4	3	8'-6"	11
*A119	1	#5	STR	24'-2"	25	*A168	1	#5	STR	3'-4"	3	A246	1	#5	STR	28'-3"	29	K14	3	#4	8	12'-10"	26
*A120	1	#5	STR	25'-5"	27	*A169	1	#5	STR	2'-3"	2	A247	1	#5	STR	27'-1"	28	* K15	4	#6	STR	8'-8"	52
*A121	1	#5	STR	26'-8"	28	*A170	3	#6	STR	5'-0"	23	A248	1	#5	STR	25'-11"	27	* K16	4	#6	STR	7'-4"	44
*A122	1	#5	STR	27'-11"	29	*A171	3	#6	STR	7'-3"	33	A249	1	#5	STR	24'-10"	26	* K17	4	#6	STR	5'-11"	36
*A123	1	#5	STR	29'-3"	31	A201	301	#5	STR	41'-11"	13159	A250	1	#5	STR	23'-8"	25	K18	8	#4	STR	9'-2"	49
*A124	1	#5	STR	30'-6"	32	A202	1	#5	STR	2'-6"	3	A251	1	#5	STR	22'-7"	24	* K19	6	#8	1	23'-9"	380
*A125	1	#5	STR	31'-9"	33	A203	1	#5	STR	3'-9"	4	A252	1	#5	STR	21'-5"	22	* K20	4	#8	2	14'-11"	159
*A126	1	#5	STR	33'-0"	34	A204	1	#5	STR	5'-0"	5	A253	1	#5	STR	20'-3"	21						
*A127	1	#5	STR	34'-3"	36	A205	1	#5	STR	6'-4"	7	A254	1	#5	STR	19'-2"	20	* S1	24	#4	4	4'-5"	71
*A128	1	#5	STR	35'-7"	37	A206	1	#5	STR	7'-7"	8	A255	1	#5	STR	18'-0"	19	* S2	48	#5	5	5'-10"	292
*A129	1	#5	STR	36'-10"	38	A207	1	#5	STR	8'-10"	9	A256	1	#5	STR	16'-11"	18	* S3	32	#5	7	2'-7"	86
*A130	1	#5	STR	38'-1"	40	A208	1	#5	STR	10'-2"	11	A257	1	#5	STR	15'-9"	16	S4	176	#4	6	3'-3"	382
*A131	1	#5	STR	39'-4"	41	A209	1	#5	STR	11'-5"	12	A258	1	#5	STR	14'-7"	15	S5	8	#5	7	11'-7"	97
*A132	1	#5	STR	40'-7"	42	A210	1	#5	STR	12'-8"	13	A259	1	#5	STR	13'-6"	14	* S6	24	#4	4	5'-1"	81
*A133	1	#5	STR	41'-11"	44	A211	1	#5	STR	13'-11"	15	A260	1	#5	STR	12'-4"	13						
*A134	1	#5	STR	41'-11"	44	A212	1	#5	STR	15'-3"	16	A261	1	#5	STR	11'-3"	12	U1	24	#4	1	19'-0"	305
*A135	1	#5	STR	40'-9"	43	A213	1	#5	STR	16'-6"	17	A262	1	#5	STR	10'-1"	11						
*A136	1	#5	STR	39'-7"	41	A214	1	#5	STR	17'-9"	19	A263	1	#5	STR	9'-0"	9						
*A137	1	#5	STR	38'-6"	40	A215	1	#5	STR	19'-1"	20	A264	1	#5	STR	7'-10"	8						
*A138	1	#5	STR	37'-4"	39	A216	1	#5	STR	20'-4"	21	A265	1	#5	STR	6'-9"	7						
*A139	1	#5	STR	36'-2"	38	A217	1	#5	STR	21'-7"	23	A266	1	#5	STR	5'-7"	6						
*A140	1	#5	STR	35'-1"	37	A218	1	#5	STR	22'-10"	24	A267	1	#5	STR	4'-6"	5						
*A141	1	#5	STR	33'-11"	35	A219	1	#5	STR	24'-2"	25	A268	1	#5	STR	3'-4"	3						
*A142	1	#5	STR	32'-9"	34	A220	1	#5	STR	25'-5"	27	A269	1	#5	STR	2'-3"	2						
*A143	1	#5	STR	31'-8"	33	A221	1	#5	STR	26'-8"	28												
*A144	1	#5	STR	30'-6"	32	A222	1	#5	STR	27'-11"	29	*B101	126	#4	STR	24'-11"	2097						
*A145	1	#5	STR	29'-4"	31	A223	1	#5	STR	29'-3"	31	*B102	42	#7	STR	60'-0"	5151						
*A146	1	#5	STR	28'-3"	29	A224	1	#5	STR	30'-6"	32	*B103	41	#7	STR	32'-2"	2696						
*A147	1	#5	STR	27'-1"	28	A225	1	#5	STR	31'-9"	33	*B104	42	#7	STR	19'-7"	1681						
*A148	1	#5	STR	25'-11"	27	A226	1	#5	STR	33'-0"	34	*B105	126	#4	STR	25'-1"	2111						
*A149	1	#5	STR	24'-10"	26	A227	1	#5	STR	34'-3"	36	*B106	16	#4	STR	28'-4"	303						

REINFORCING STEEL 22961 LBS.
 *EPOXY COATED REINFORCING STEEL 30737 LBS.

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS

BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET, AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER RAIL
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3'-5"
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"
#7	5'-3"	3'-6"			
#8	6'-10"	4'-7"			

GROOVING BRIDGE FLOORS

APPROACH SLABS	1,757 SQ.FT.
BRIDGE DECK	7,765 SQ.FT.
TOTAL	9,522 SQ.FT.

—SUPERSTRUCTURE BILL OF MATERIAL—

	CLASS AA CONCRETE (CU. YDS.)	REINFORCING STEEL (LBS.)	EPOXY COATED REINFORCING STEEL (LBS.)
POUR 1	135.1		
POUR 2	185.9		
TOTALS**	321.0	22961 LBS.	30737 LBS.

**QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 1 OF 2

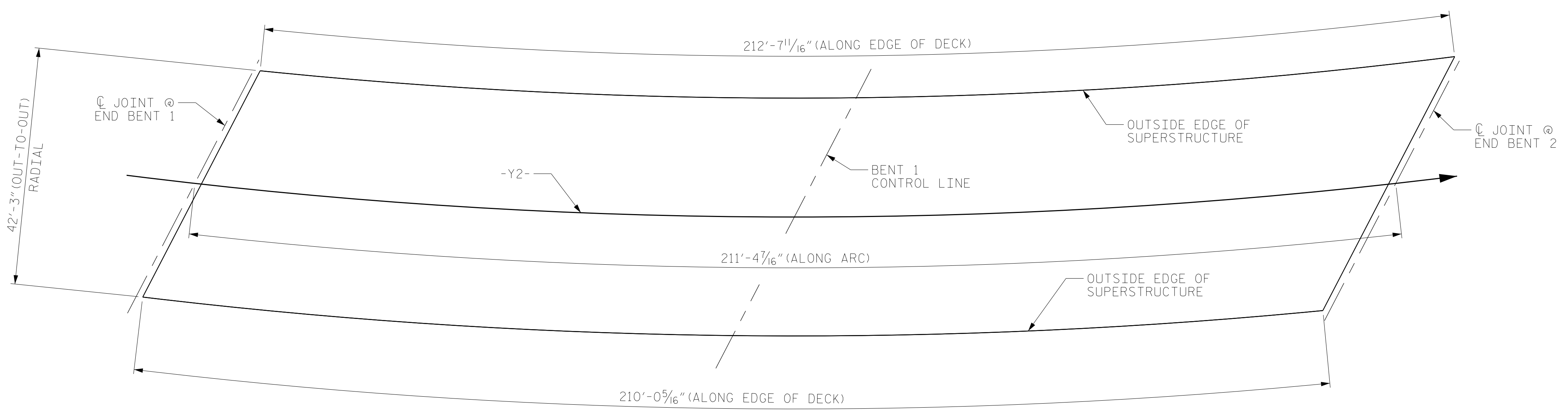


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 BILL OF MATERIAL

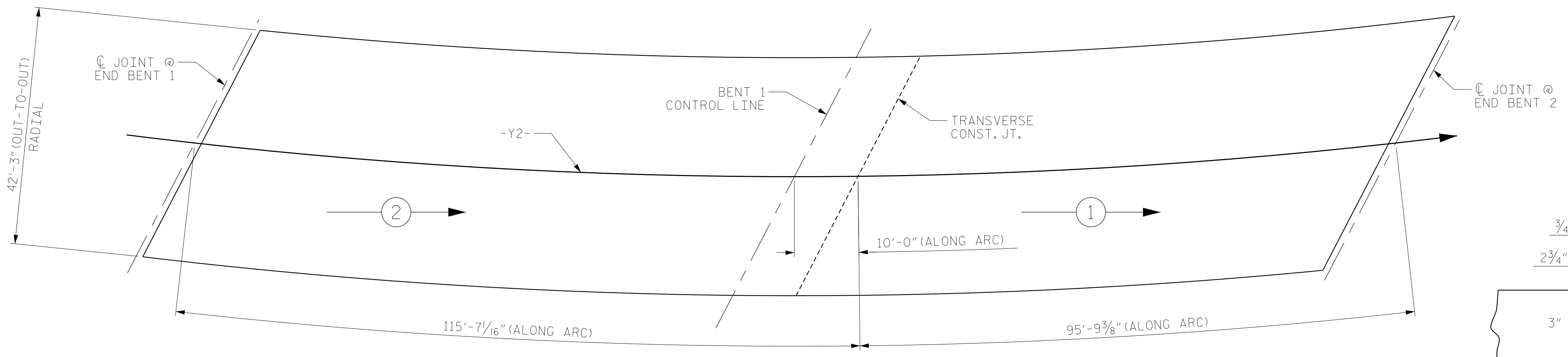
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NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 36
2			4			

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 DESIGN ENGINEER OF RECORD: MKO DATE: 12/2016

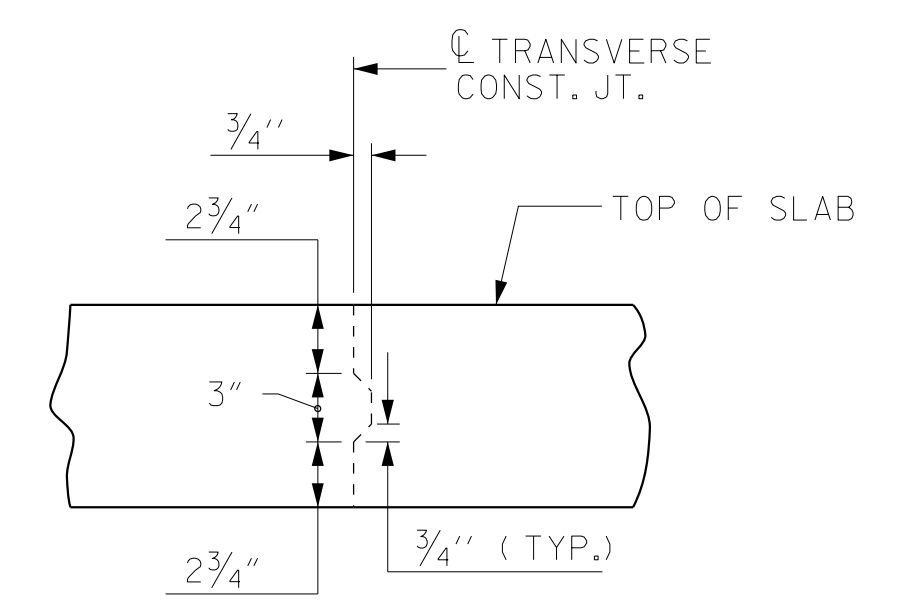
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



LAYOUT FOR COMPUTING
AREA OF REINFORCED CONCRETE DECK SLAB
(SQ. FT = 8,927)



POURING SEQUENCE

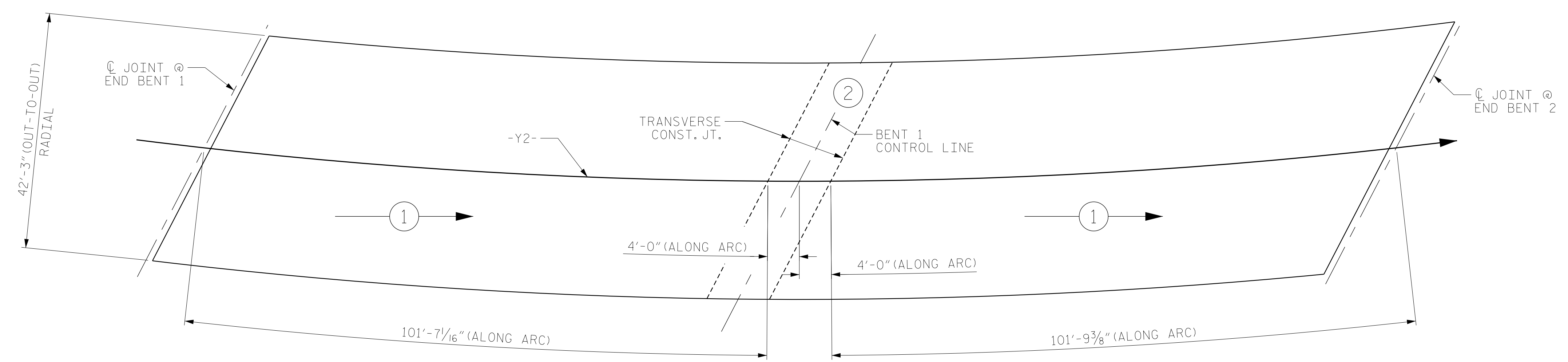


TRANSVERSE CONSTRUCTION JOINT DETAIL

NOTE: REINFORCING STEEL IN SLAB NOT SHOWN.
LONGITUDINAL REINFORCING STEEL SHALL BE
CONTINUOUS THRU JOINT

PROJECT NO. R-2707C
CLEVELAND COUNTY
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SHEET 2 OF 2



OPTIONAL POURING SEQUENCE

POUR ② CAN NOT BE STARTED UNTIL BOTH ADJACENT ① POURS REACH A MINIMUM OF 3000 PSI

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						SHEET NO.
SUPERSTRUCTURE BILL OF MATERIAL						S3-25
REVISIONS						TOTAL SHEETS
NO.	BY:	DATE:	NO.	BY:	DATE:	36
1			3			
2			4			

NOTES

STIRRUPS AND B3 BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

FOR PILE SPlice DETAILS, SEE END BENT DETAILS SHEET 3 OF 3.

THE TOP SURFACE AREAS OF THE END BENT CAP SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, EXCEPT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.

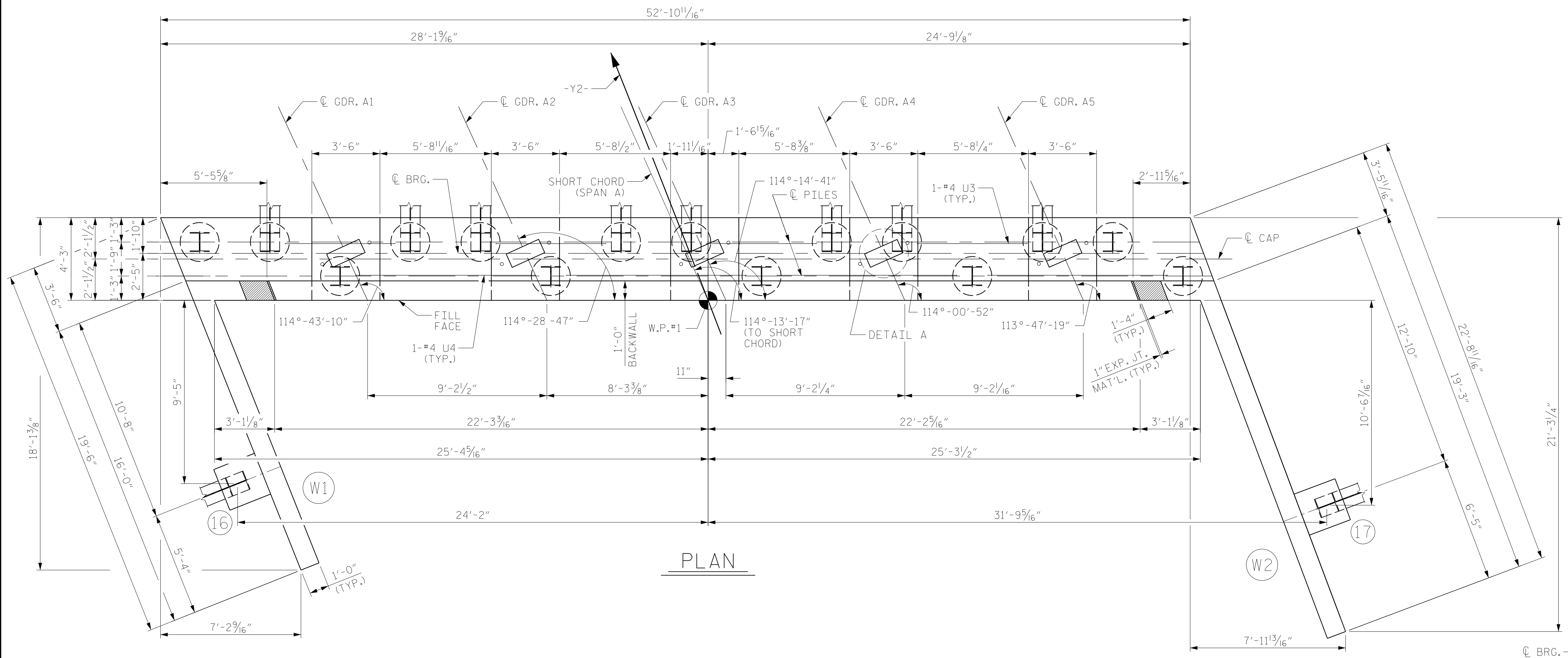
BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING.

THE TOP SURFACE OF THE END BENT CAP EXCEPT BRIDGE SEAT BUILDUPS SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE FRONT FACE AT A RATE OF 2%.

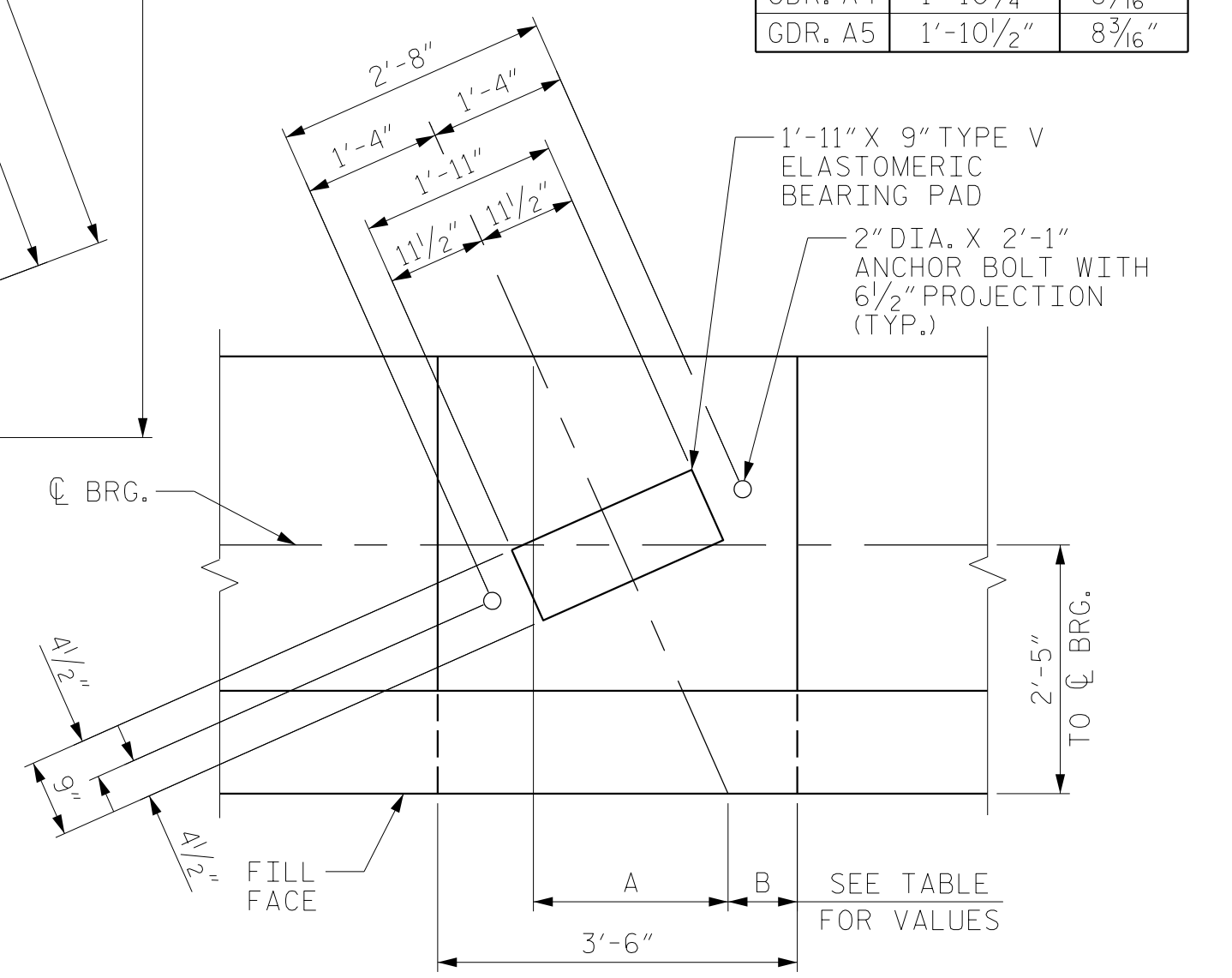
INSTALL THE 4" DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILL, SEE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE JOINT BETWEEN THE DECK AND THE APPROACH SLAB HAS BEEN SAWED AND THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

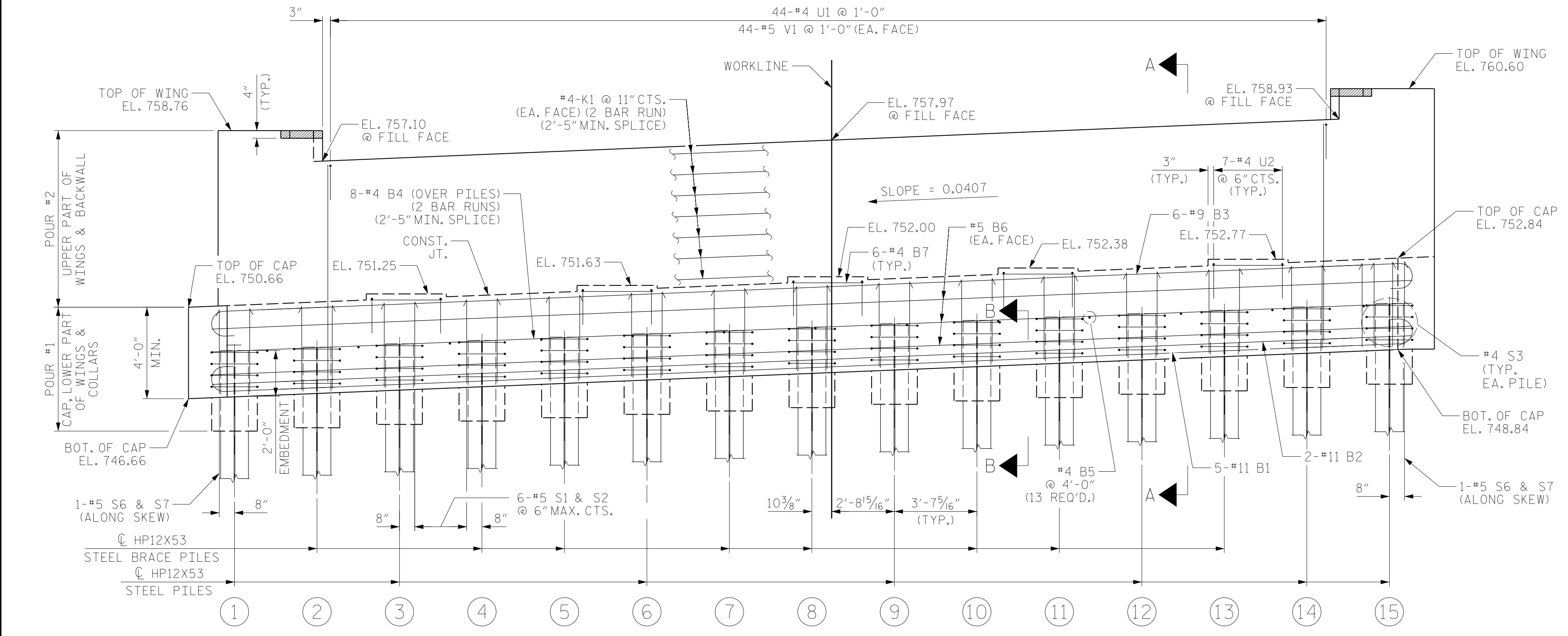
	A	B
GDR. A1	1'-11 1/2"	7 5/8"
GDR. A2	1'-11 1/4"	7 13/16"
GDR. A3	1'-10 5/16"	7 5/16"
GDR. A4	1'-10 3/4"	8 1/16"
GDR. A5	1'-10 1/2"	8 3/16"



PLAN



DETAIL A
DIMENSIONS TYP. EA. BRG.
PILE NOT SHOWN FOR CLARITY



ELEVATION

WINGS NOT SHOWN FOR CLARITY

TOP OF PILE ELEVATIONS

PILE	ELEVATION
1	748.77
2	748.92
3	749.07
4	749.21
5	749.36
6	749.51
7	749.65
8	749.80
9	749.95
10	750.09
11	750.24
12	750.39
13	750.53
14	750.68
15	750.83
16	748.73
17	750.86

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 29+07.16 -Y2-

SHEET 1 OF 3



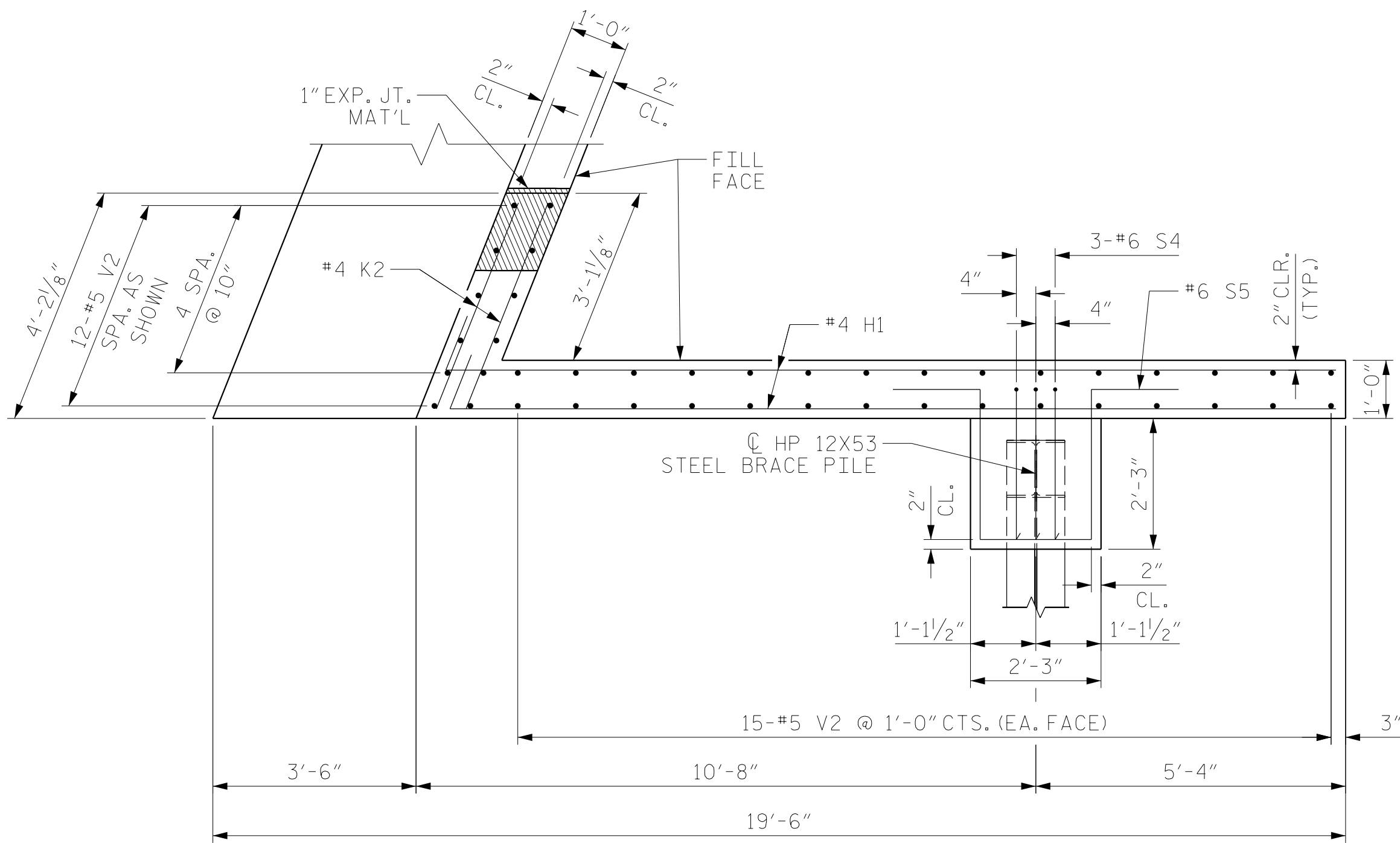
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8601 Six Forks Road, Suite 260
Raleigh, NC 27615
919-926-4100 FAX 919-846-9080
www.rsandh.com
North Carolina License No. 50737-F-0403-1-C-28

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE
END BENT NO. 1

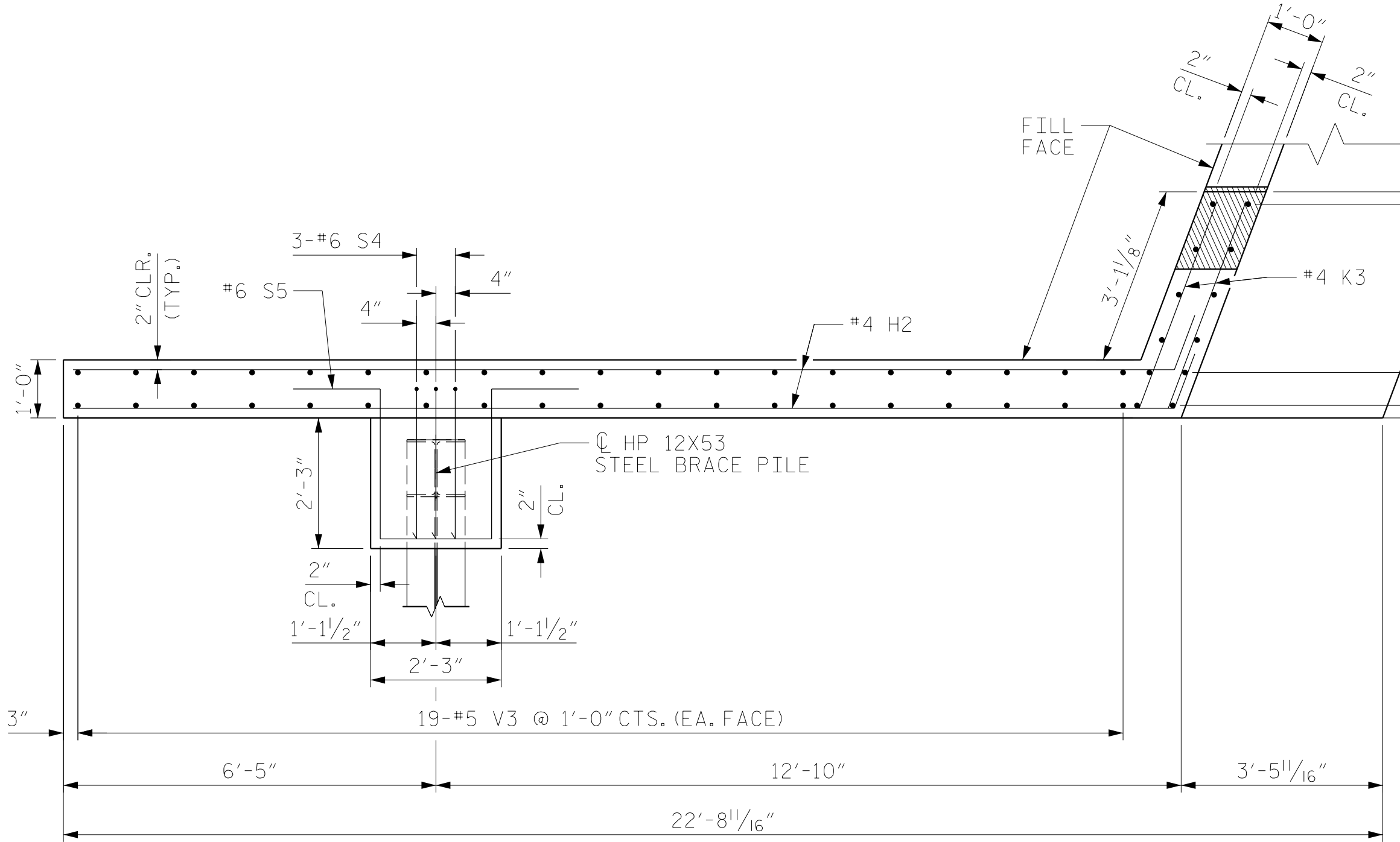
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NO.	BY:	DATE:	NO.	BY:	DATE:	S3-26
1			3			TOTAL SHEETS
2			4			36

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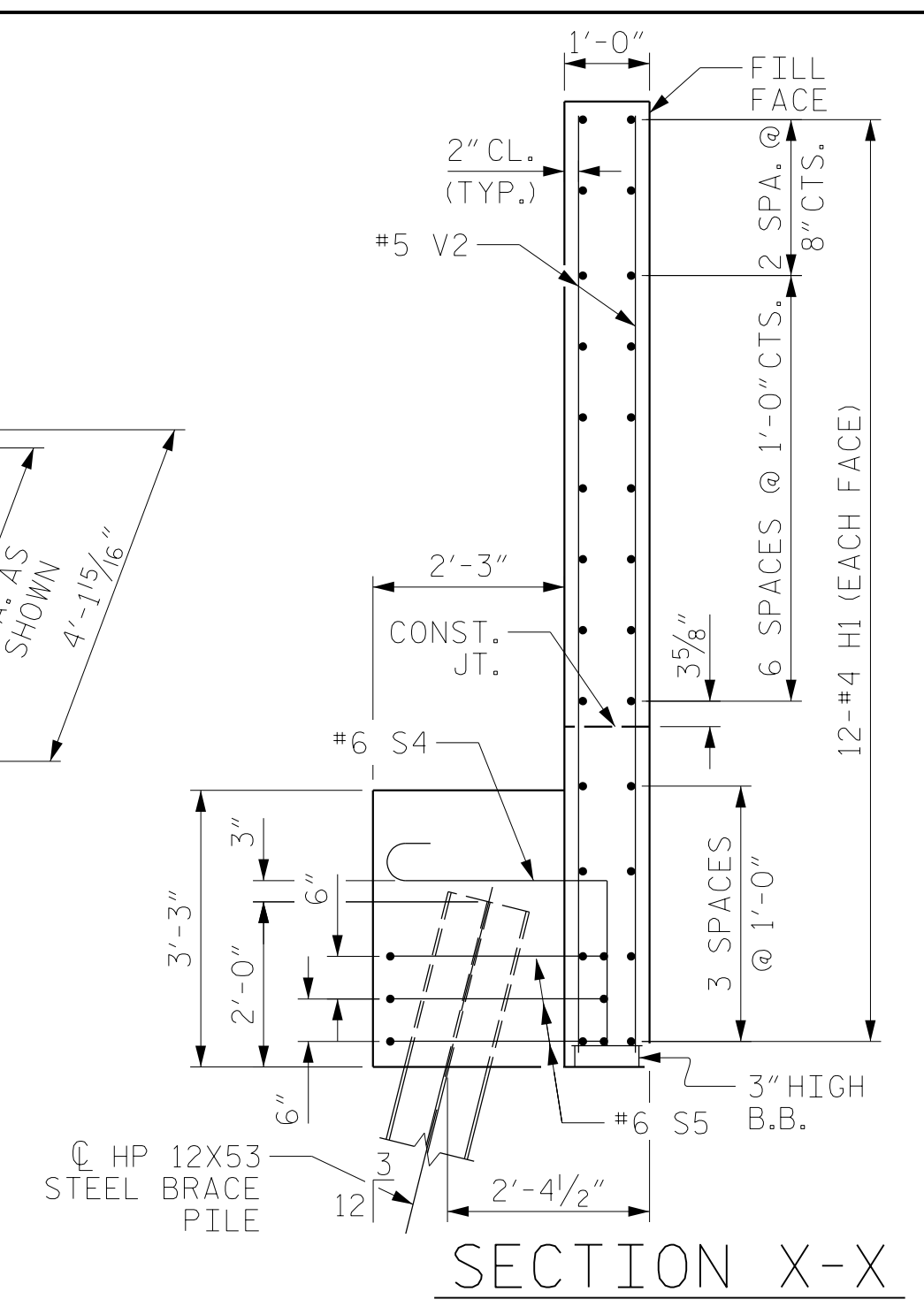
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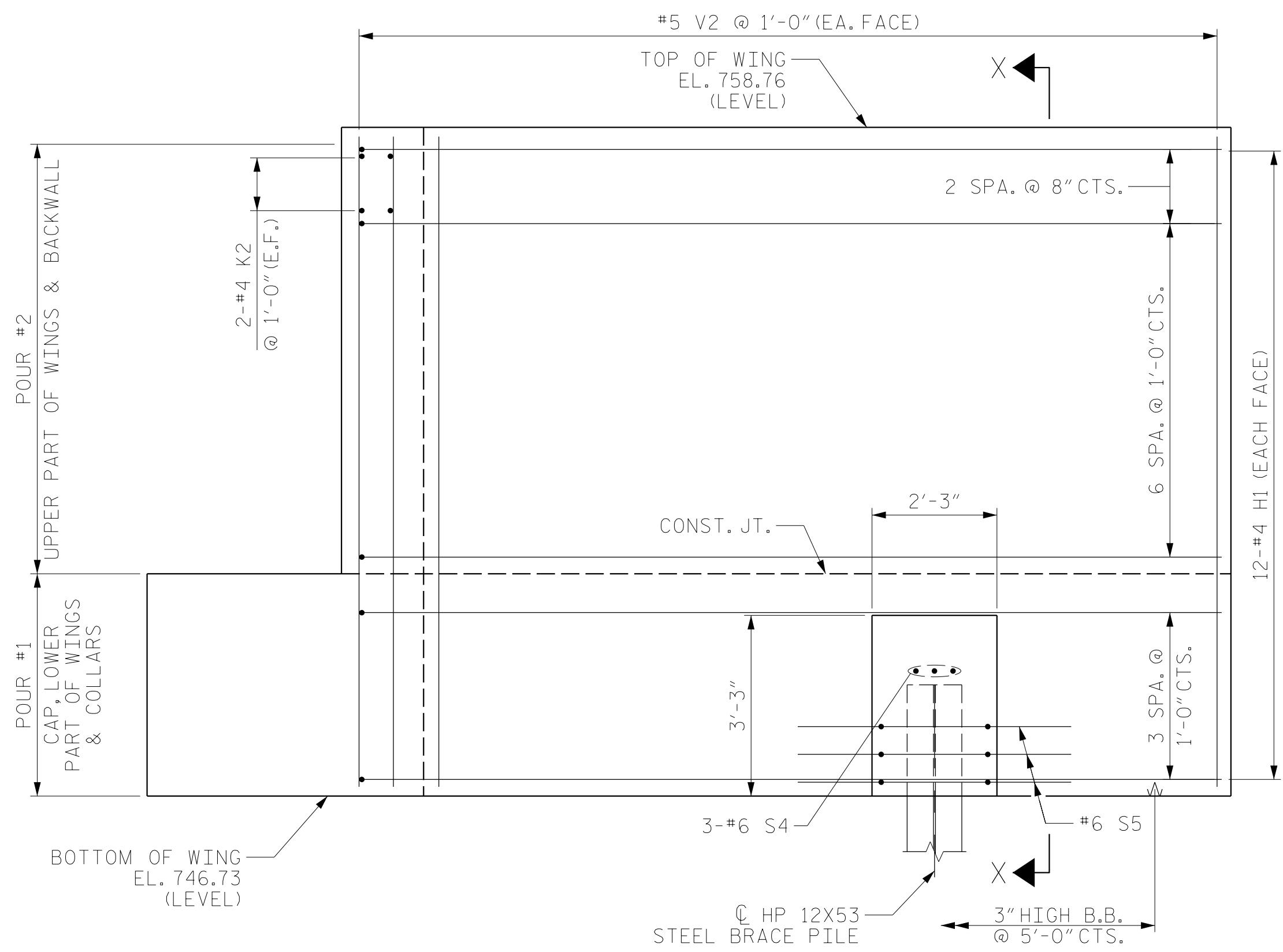
PLAN OF LEFT WING W1



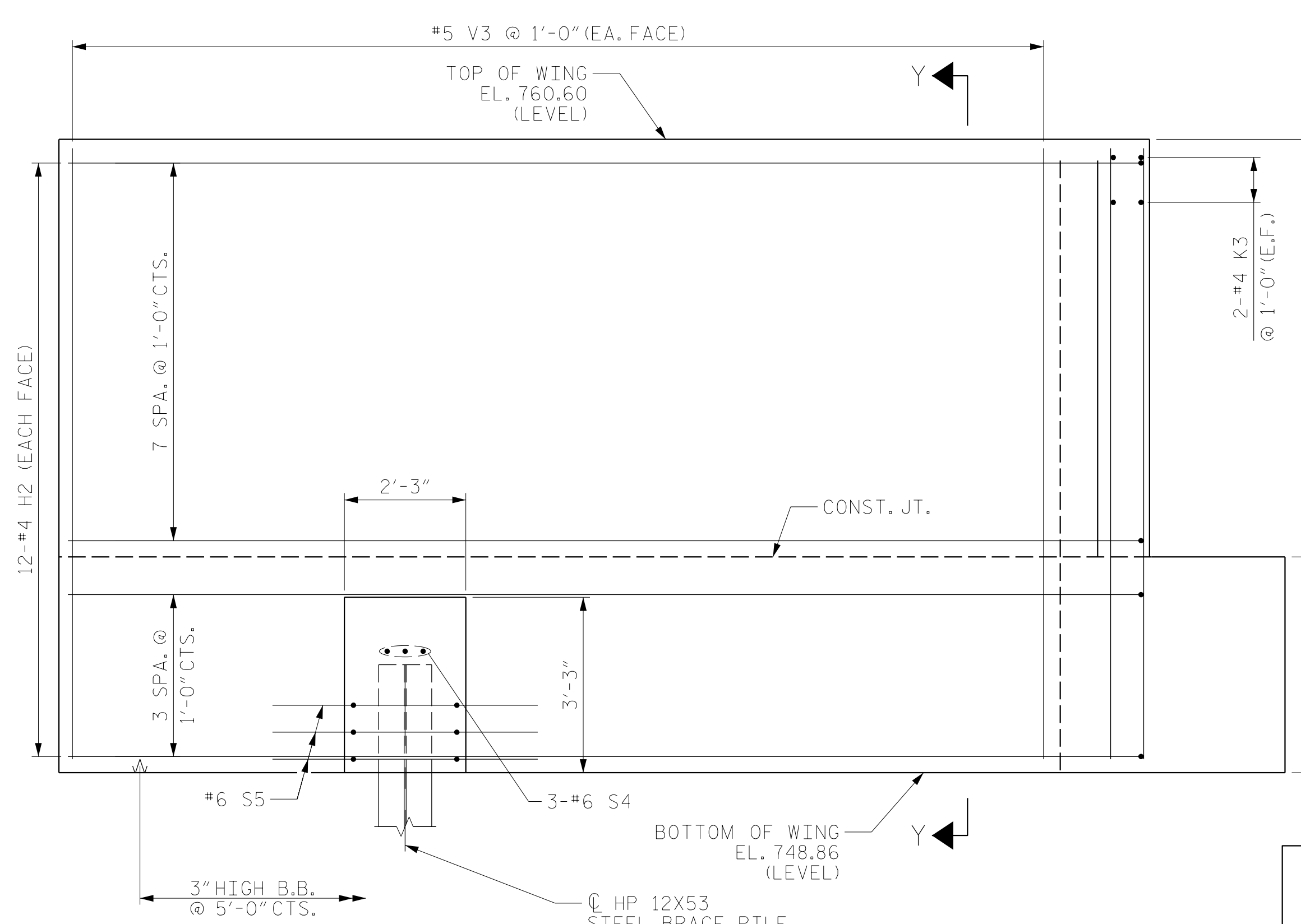
PLAN OF RIGHT WING W2



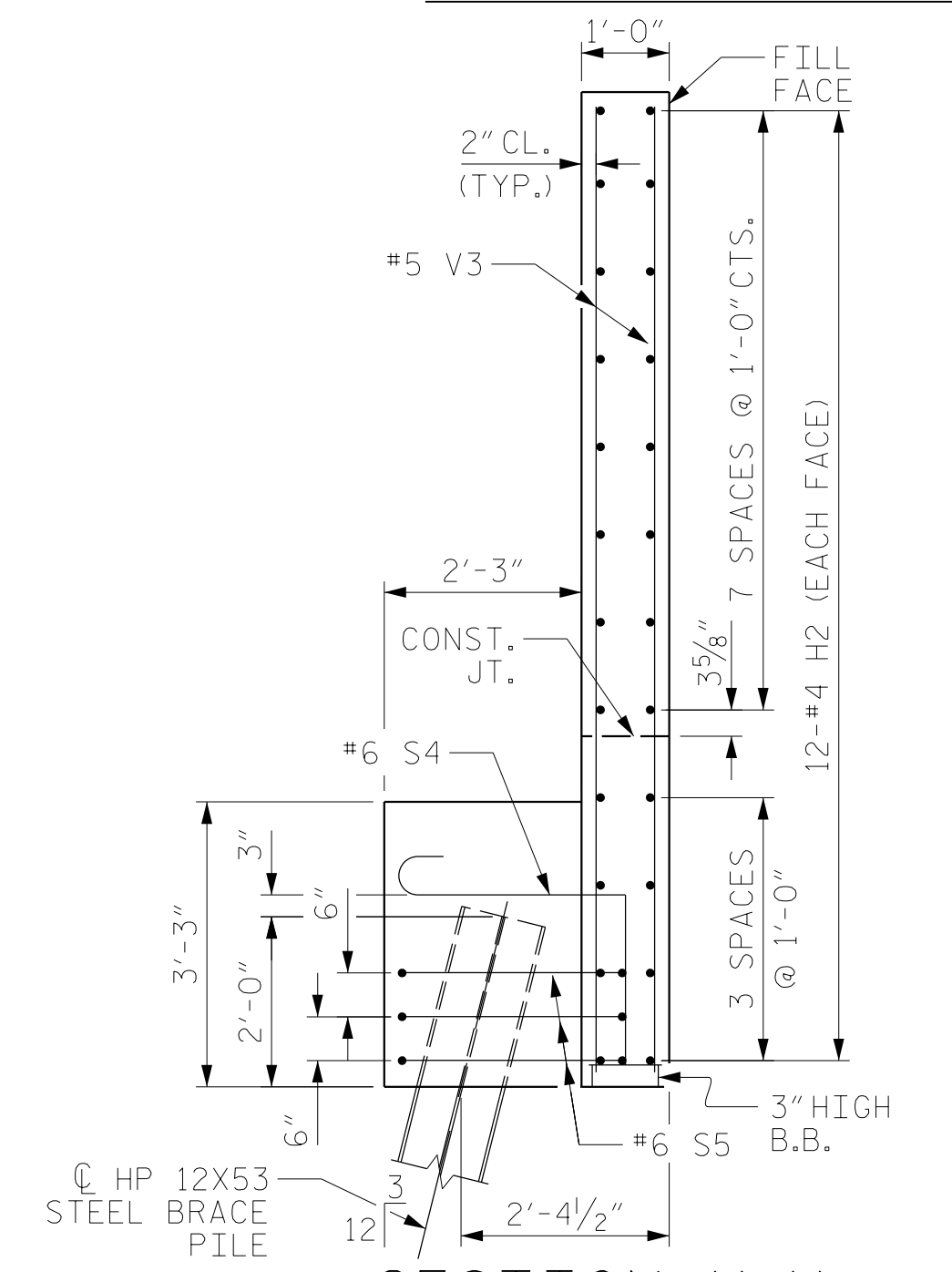
SECTION X-X



ELEVATION OF LEFT WING W1



ELEVATION OF RIGHT WING W2



SECTION Y-Y

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 2 OF 3

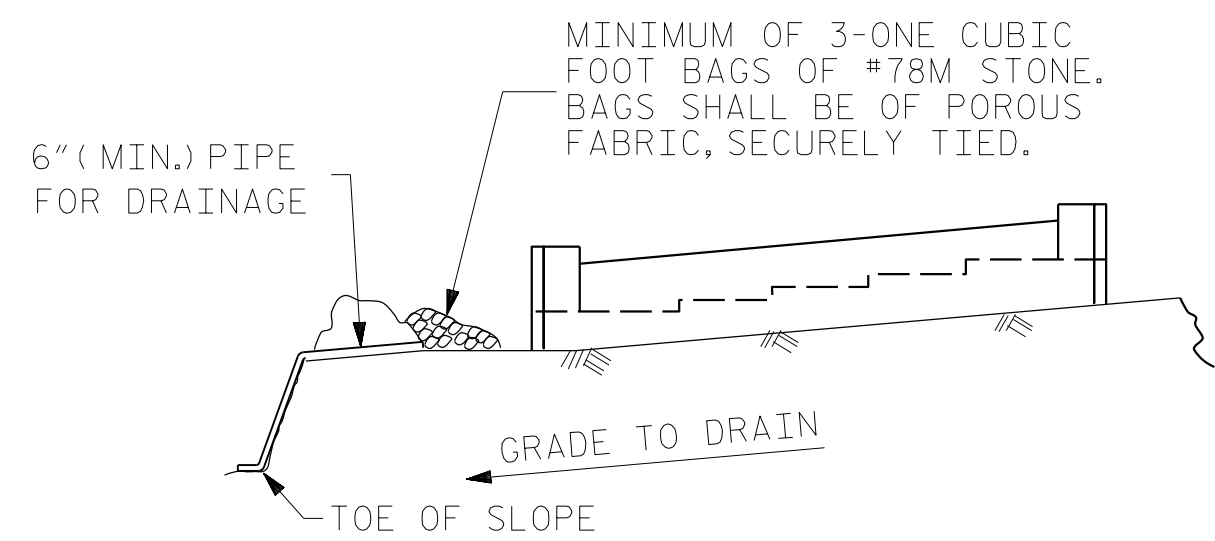


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 END BENT NO. 1

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 DESIGN ENGINEER OF RECORD: MKO DATE : 12/2016

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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-27
1			3			TOTAL SHEETS
2			4			36

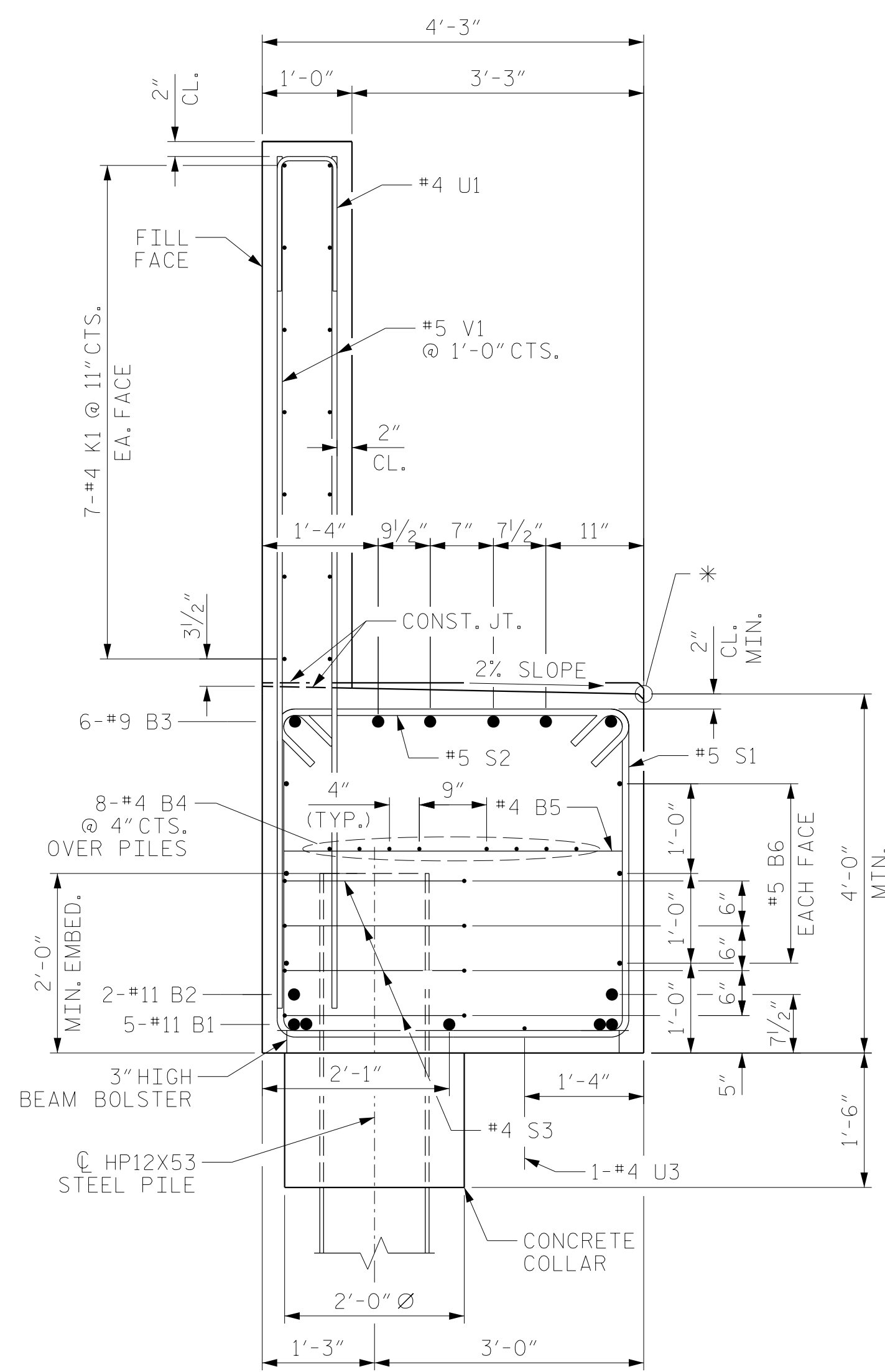


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

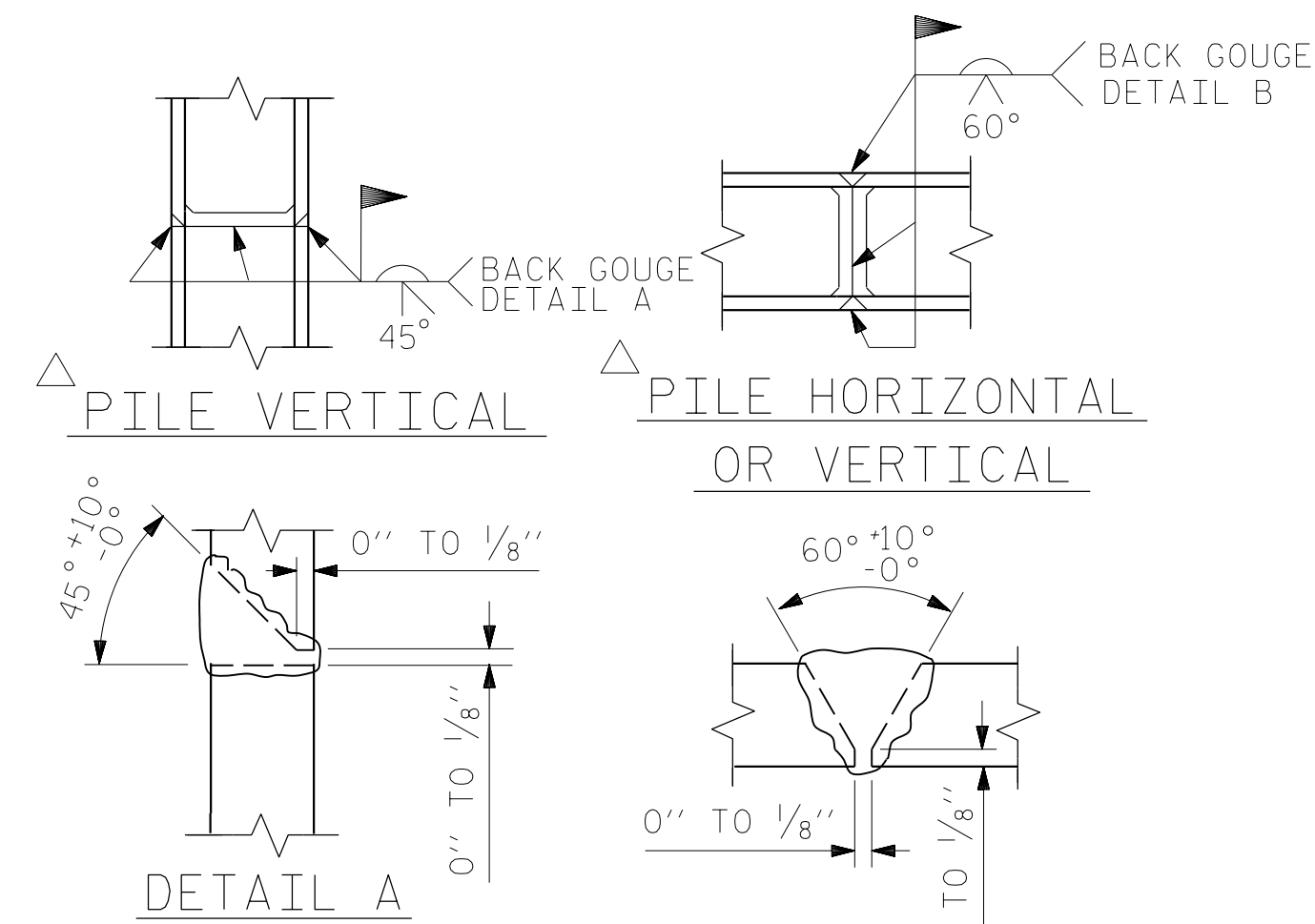
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



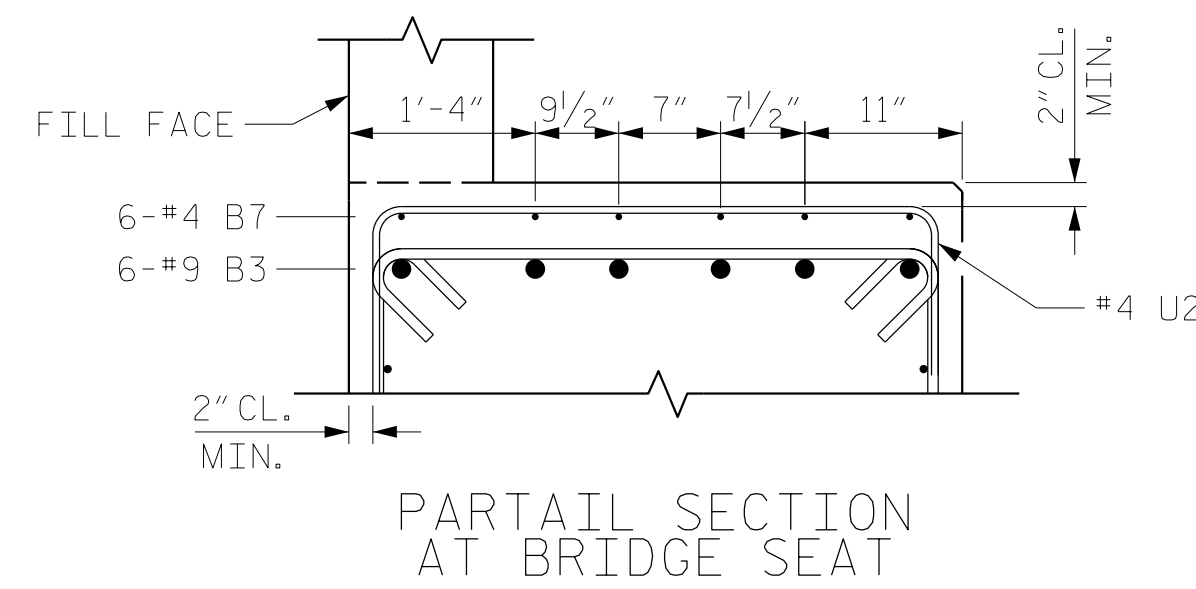
SECTION A-A

* ELEVATIONS BETWEEN BRIDGE SEAT BUILDUPS ARE SHOWN AT THIS POINT

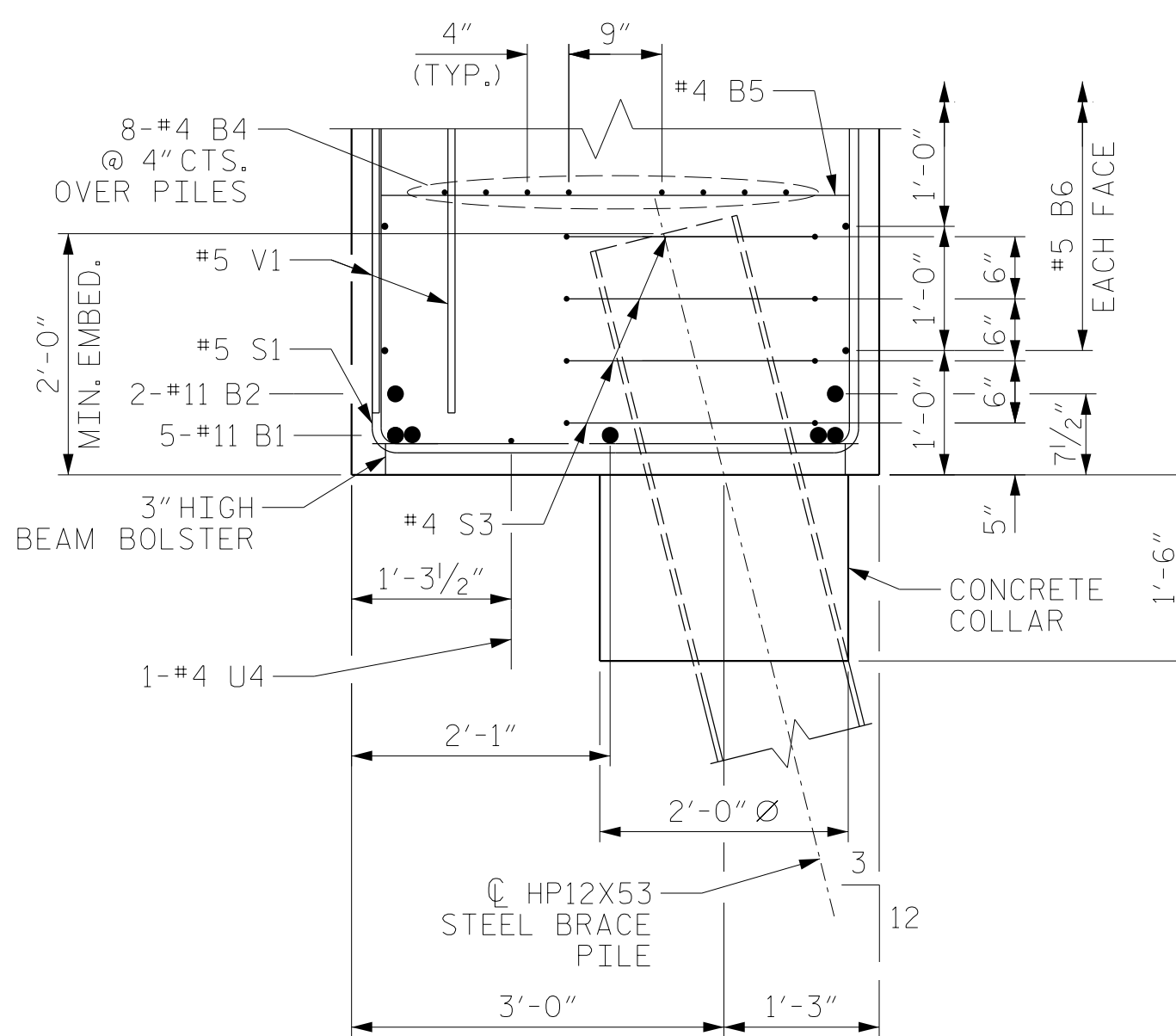


POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS

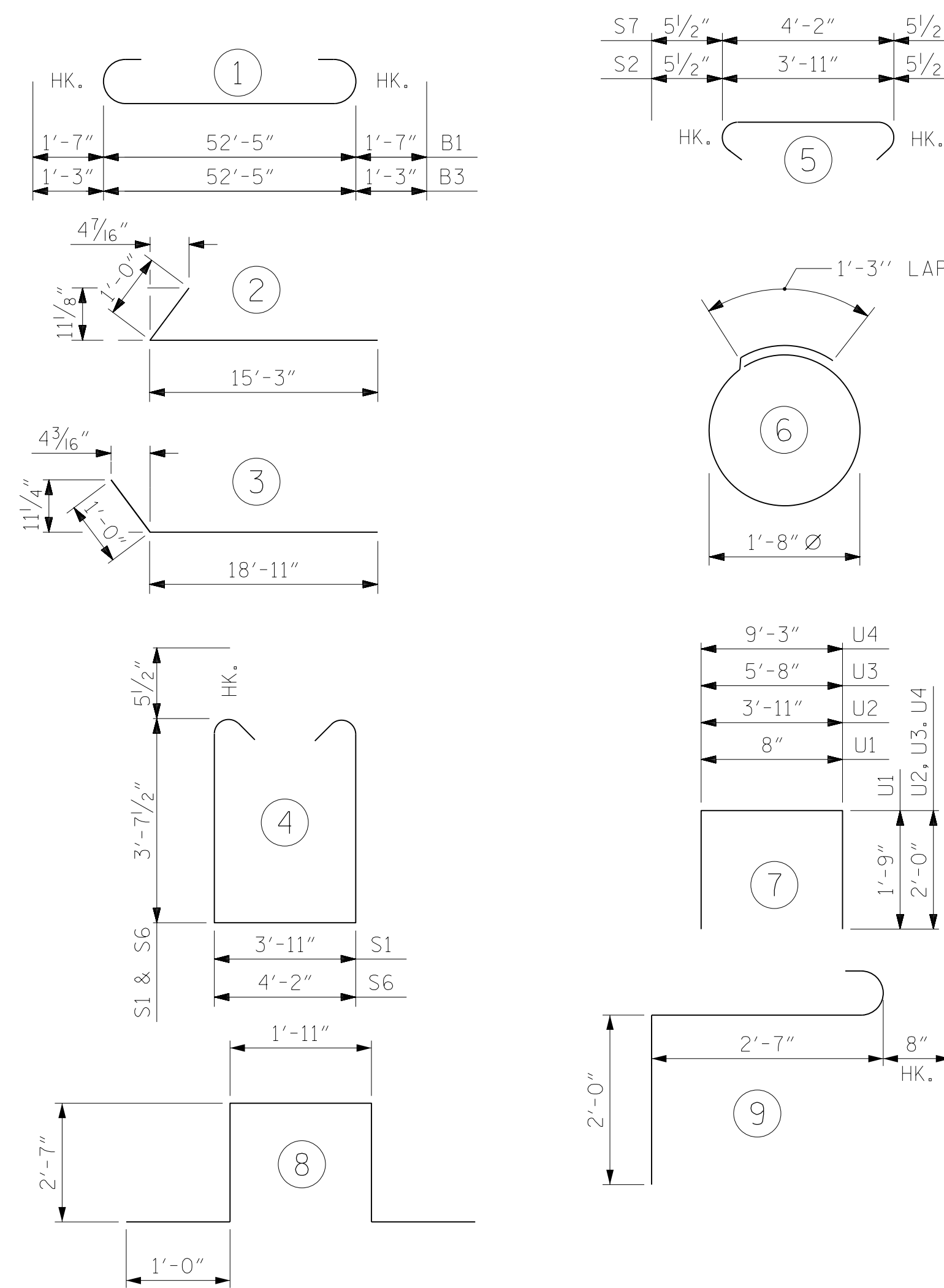


PARTIAL SECTION AT BRIDGE SEAT



SECTION B-B

BAR TYPE



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL

END BENT #1

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	5	#11	1	55'-7"	1477
B2	2	#11	STR	52'-5"	557
B3	6	#9	1	54'-11"	1120
B4	16	#4	STR	27'-5"	293
B5	13	#4	STR	3'-11"	34
B6	6	#5	STR	52'-5"	328
B7	30	#4	STR	3'-2"	63
H1	24	#4	2	16'-3"	261
H2	24	#4	3	19'-11"	319
K1	28	#4	STR	27'-5"	513
K2	4	#4	STR	3'-9"	10
K3	4	#4	STR	3'-9"	10
S1	84	#5	4	12'-1"	1059
S2	84	#5	5	4'-10"	423
S3	60	#4	6	6'-6"	261
S4	6	#6	9	5'-3"	47
S5	6	#6	8	9'-1"	82
S6	2	#5	4	12'-4"	26
S7	2	#5	5	5'-1"	11
U1	44	#4	7	4'-2"	122
U2	35	#4	7	7'-11"	185
U3	4	#4	7	9'-8"	26
U4	4	#4	7	13'-3"	35
V1	44	#5	STR	9'-6"	436
V2	42	#5	STR	11'-7"	507
V3	50	#5	STR	11'-4"	591

REINFORCING STEEL 8,796 LBS.

CLASS A CONCRETE BREAKDOWN

POUR #1	CAP, LOWER PART OF WING & COLLARS	41.7 C.Y.
POUR #2	UPPER PART OF WING & BACKWALL	22.2 C.Y.
TOTAL CLASS A CONCRETE		63.9 C.Y.

HP 12 X 53 STEEL PILES

NO: 17 LIN. FT. = 510

STEEL PILE POINTS NO. = 17

PILE DRIVING EQUIPMENT SETUP

HP 12 X 53 NO. = 17

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SHEET 3 OF 3



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE
END BENT NO. 1

REVISIONS

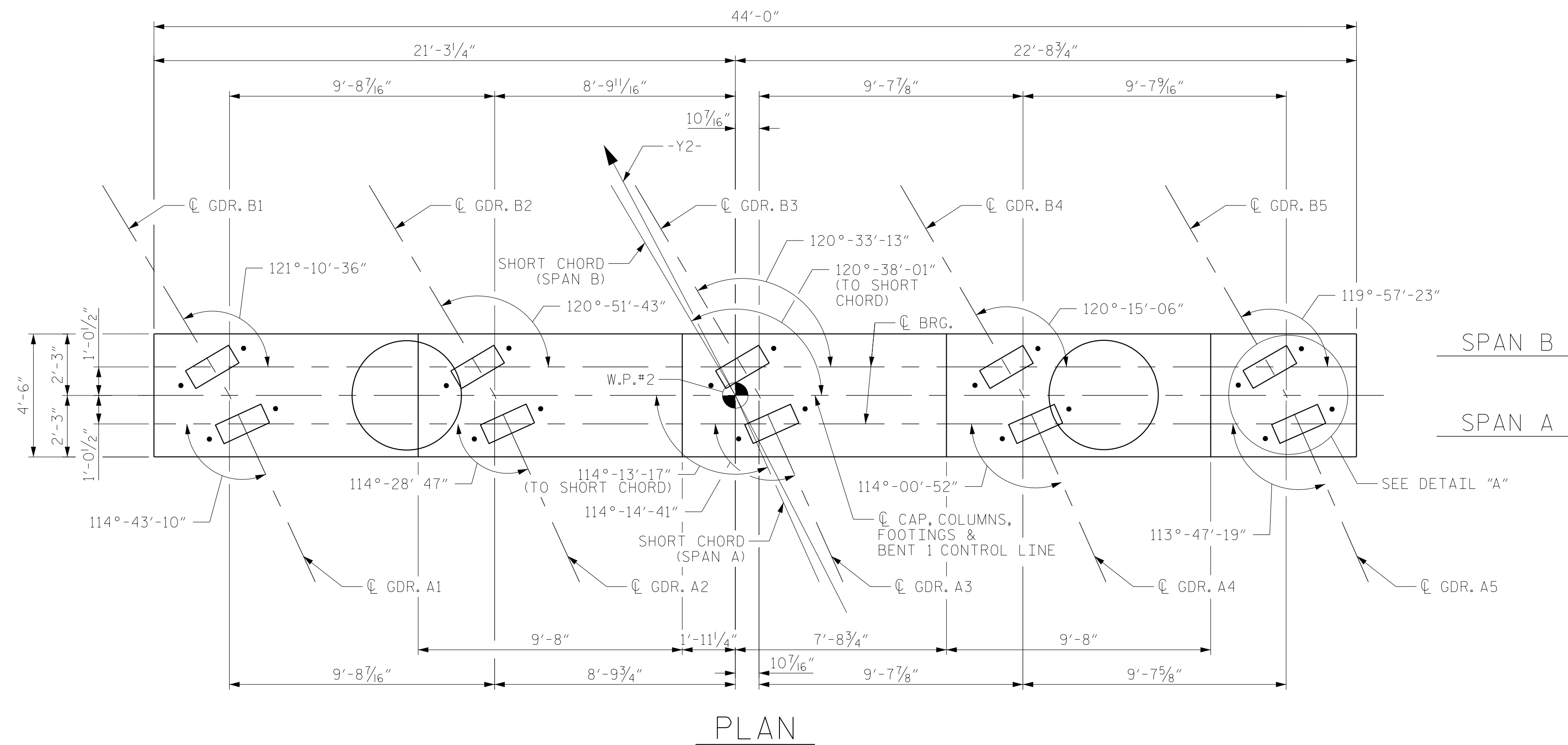
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SHEET NO.

S3-28
TOTAL SHEETS 36

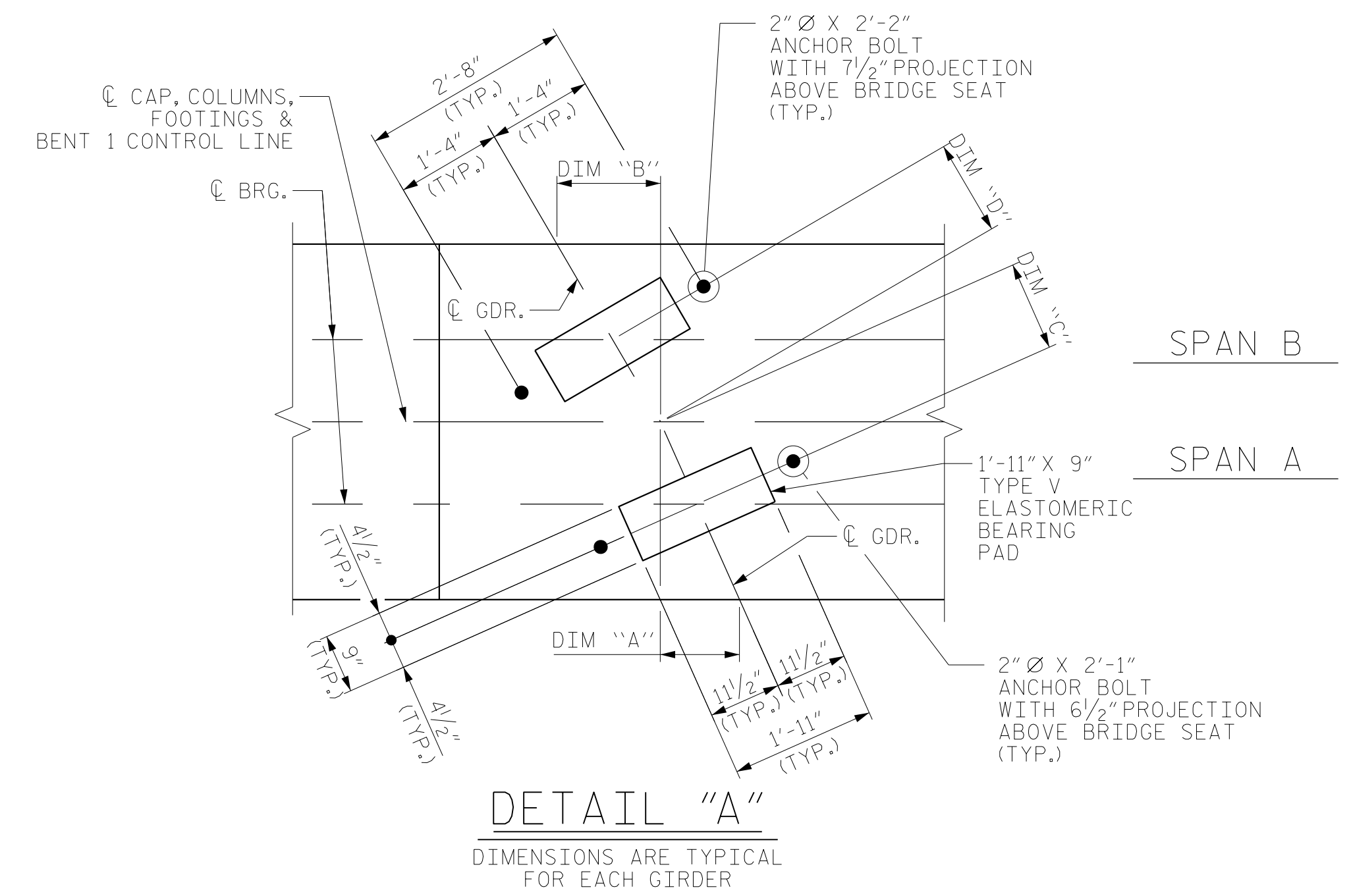
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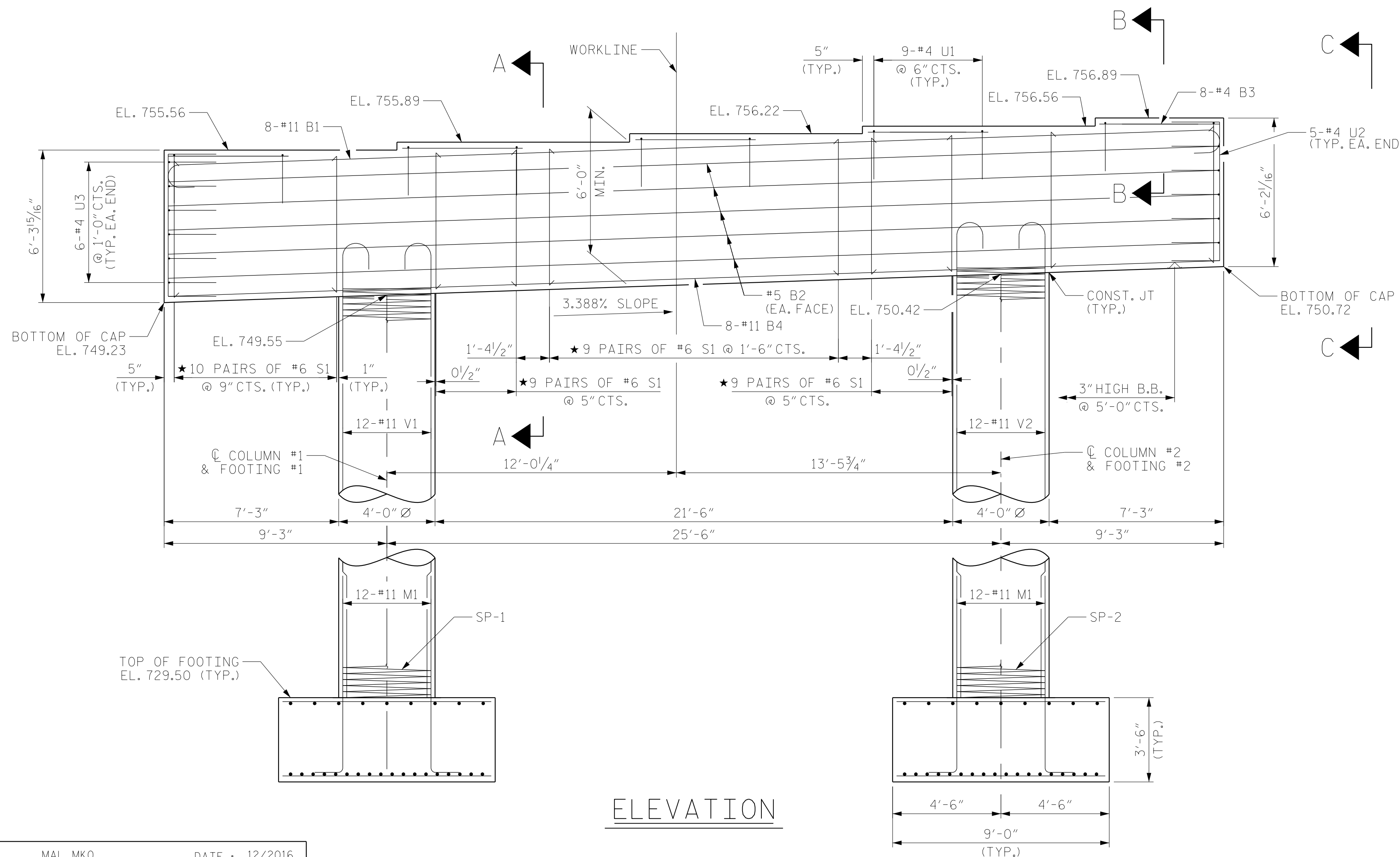


NOTES:

- FOR SECTION A-A AND SECTION B-B, SEE SHEET 2 OF 2.
- FOR REINFORCING BILL OF MATERIAL, SEE SHEET 2 OF 2.
- STIRRUPS AND "U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.
- HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.
- ★ INVERT ALTERNATE PAIRS OF #6 S1 STIRRUPS.



SPAN A			SPAN B		
GIRDER	DIM "A"	DIM "C"	GIRDER	DIM "B"	DIM "D"
A1	1'-0 7/16"	1'-1 3/4"	B1	1'-4 5/8"	1'-2 5/8"
A2	1'-0 5/16"	1'-1 3/4"	B2	1'-4 1/8"	1'-2 3/8"
A3	1'-0 3/16"	1'-1 1/16"	B3	1'-3 5/8"	1'-2 1/2"
A4	1'-0"	1'-1 1/16"	B4	1'-3 3/4"	1'-2 1/2"
A5	11 7/8"	1'-1 1/16"	B5	1'-3 3/8"	1'-2 1/8"



PROJECT NO. R-2707C
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 STATION: 29+07.16 -Y2-

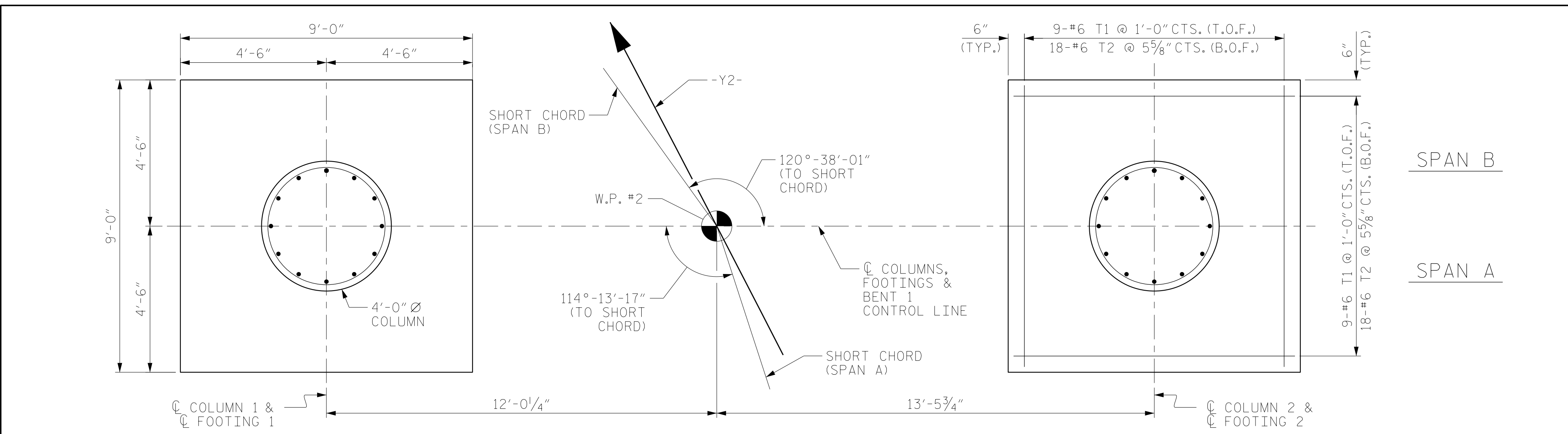
SHEET 1 OF 2



REVISIONS						SHEET NO.
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1			3			TOTAL SHEETS
2			4			36

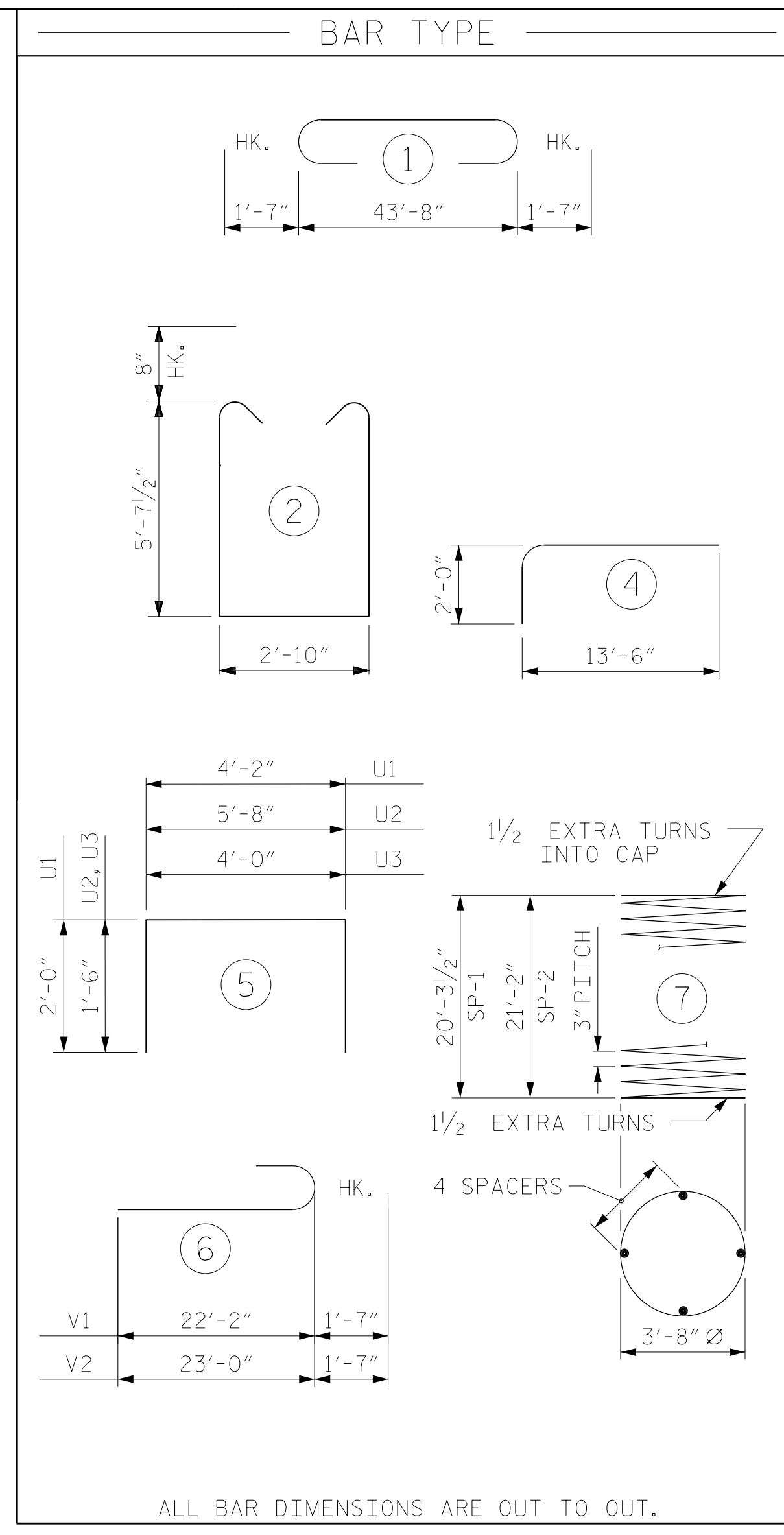
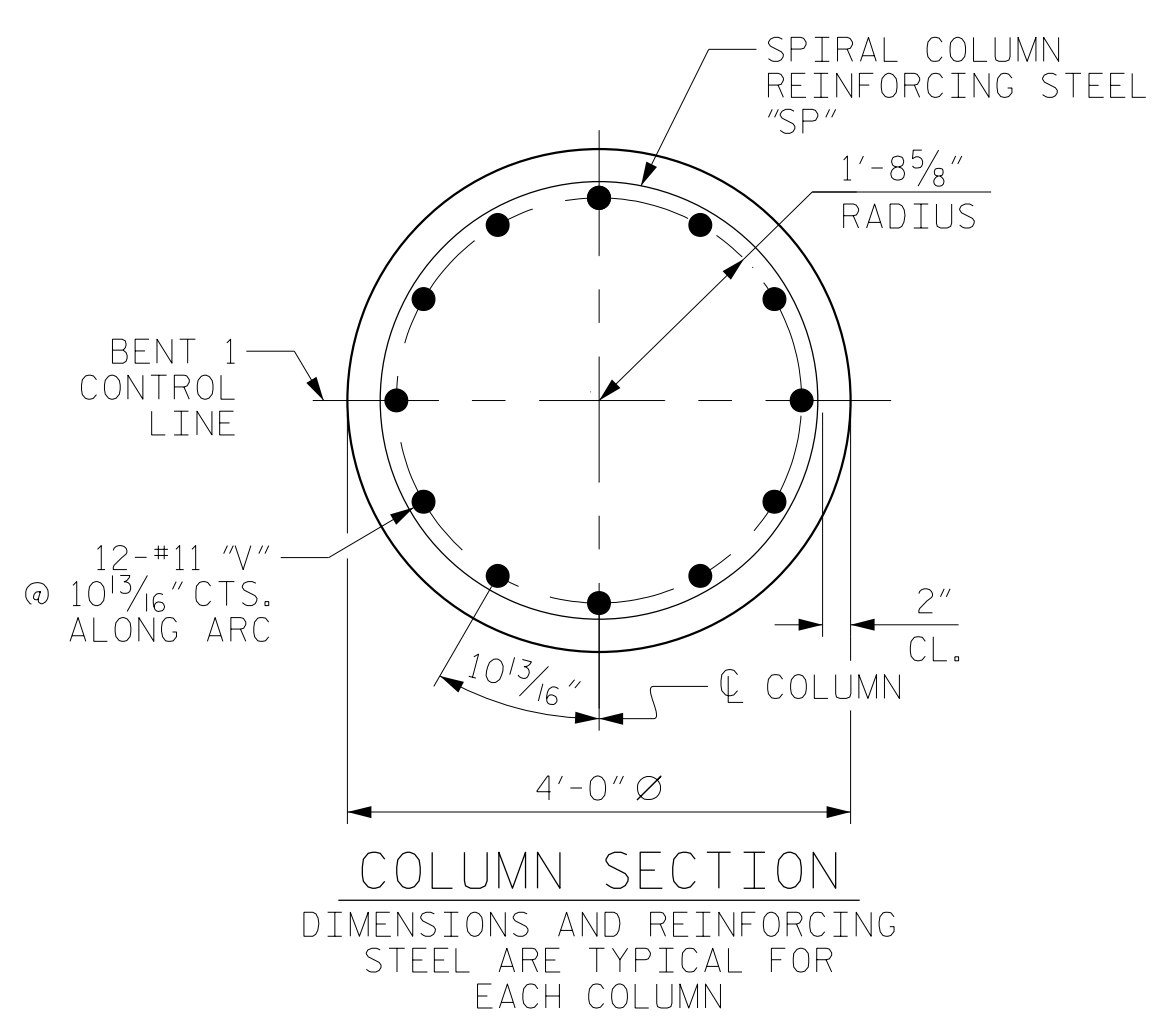
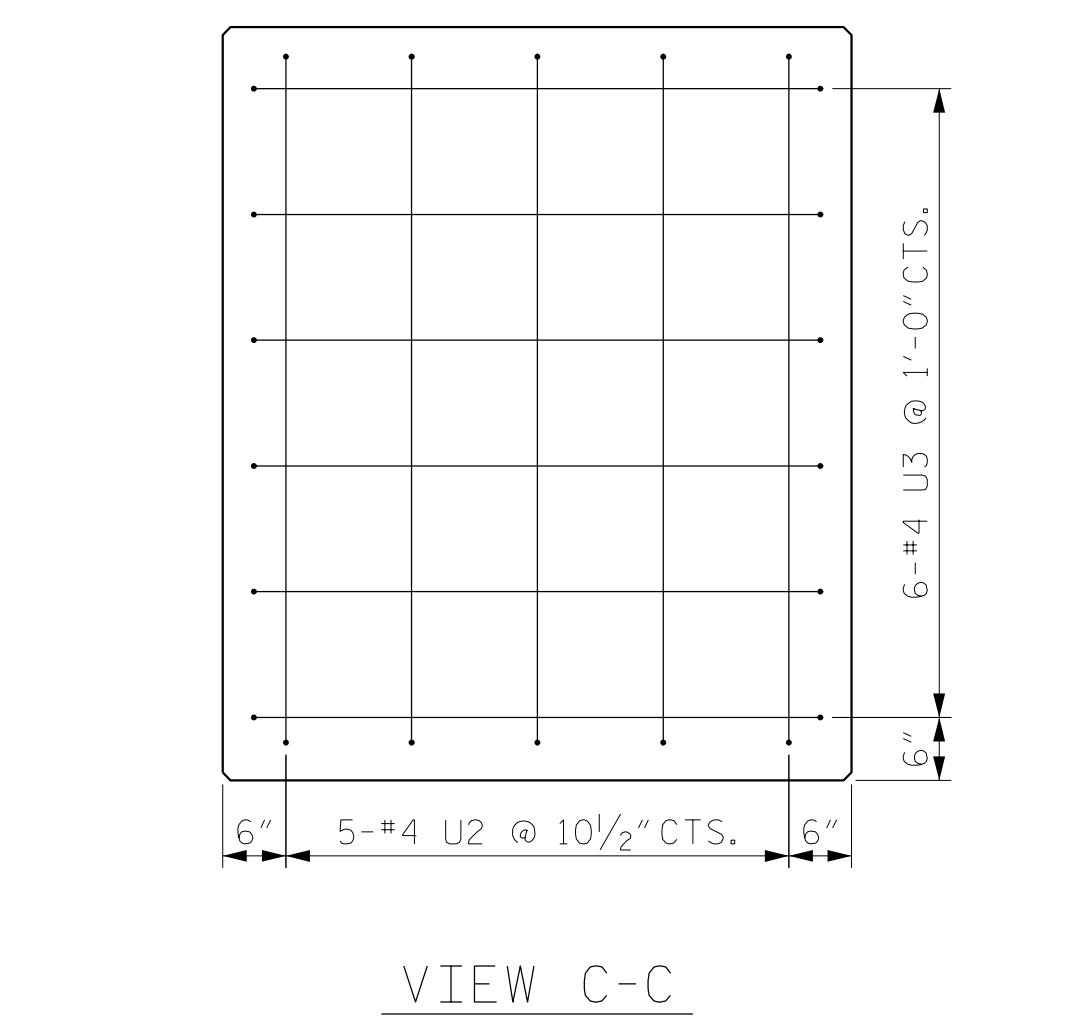
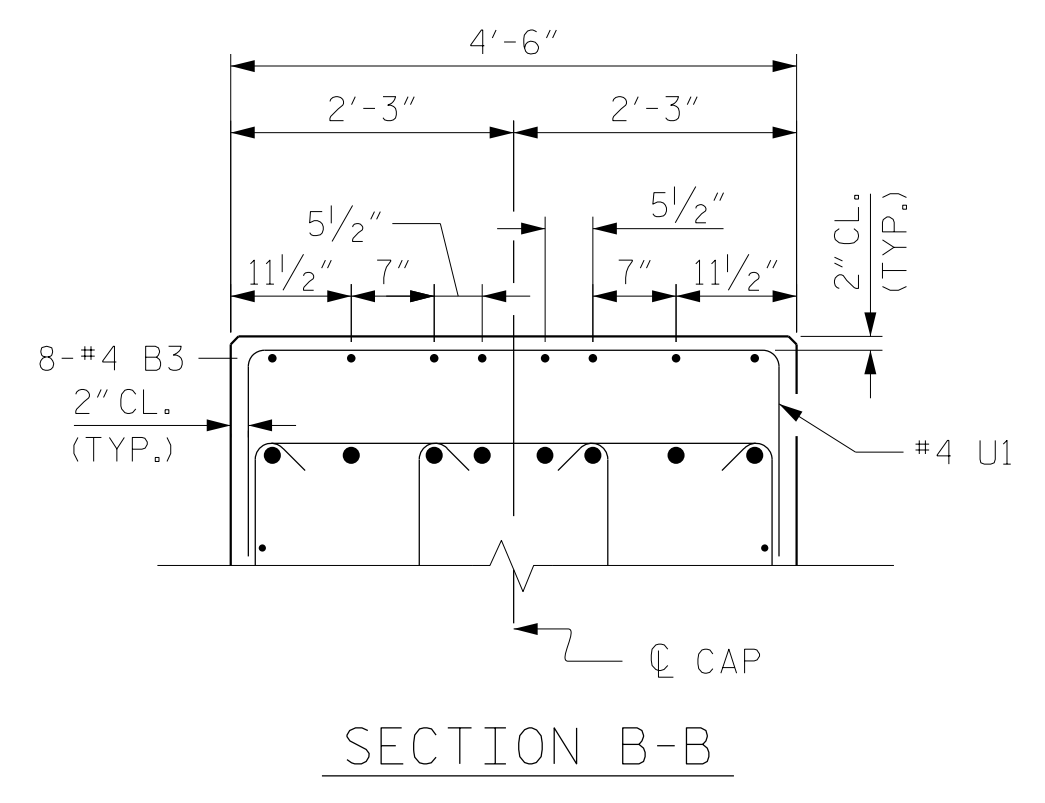
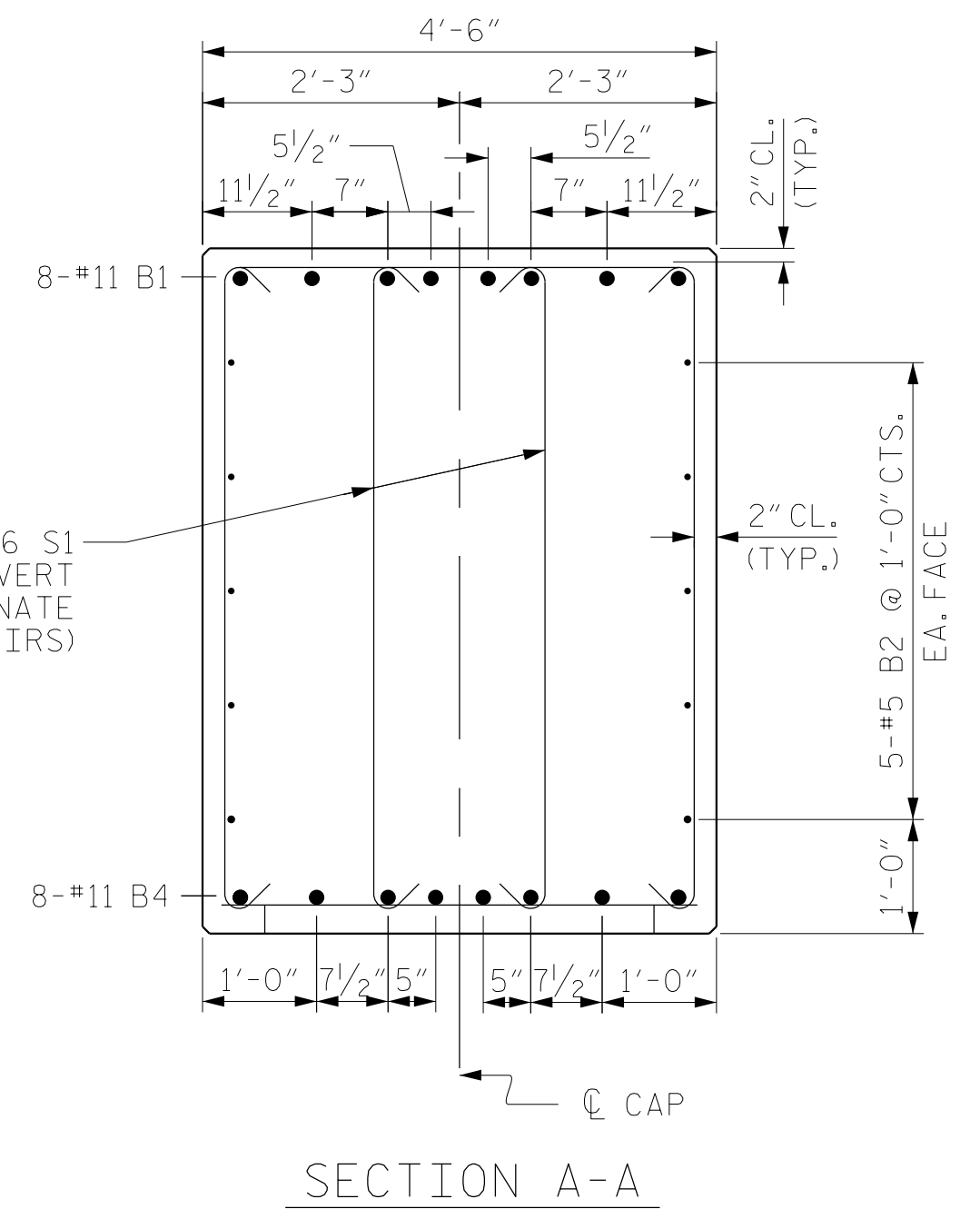
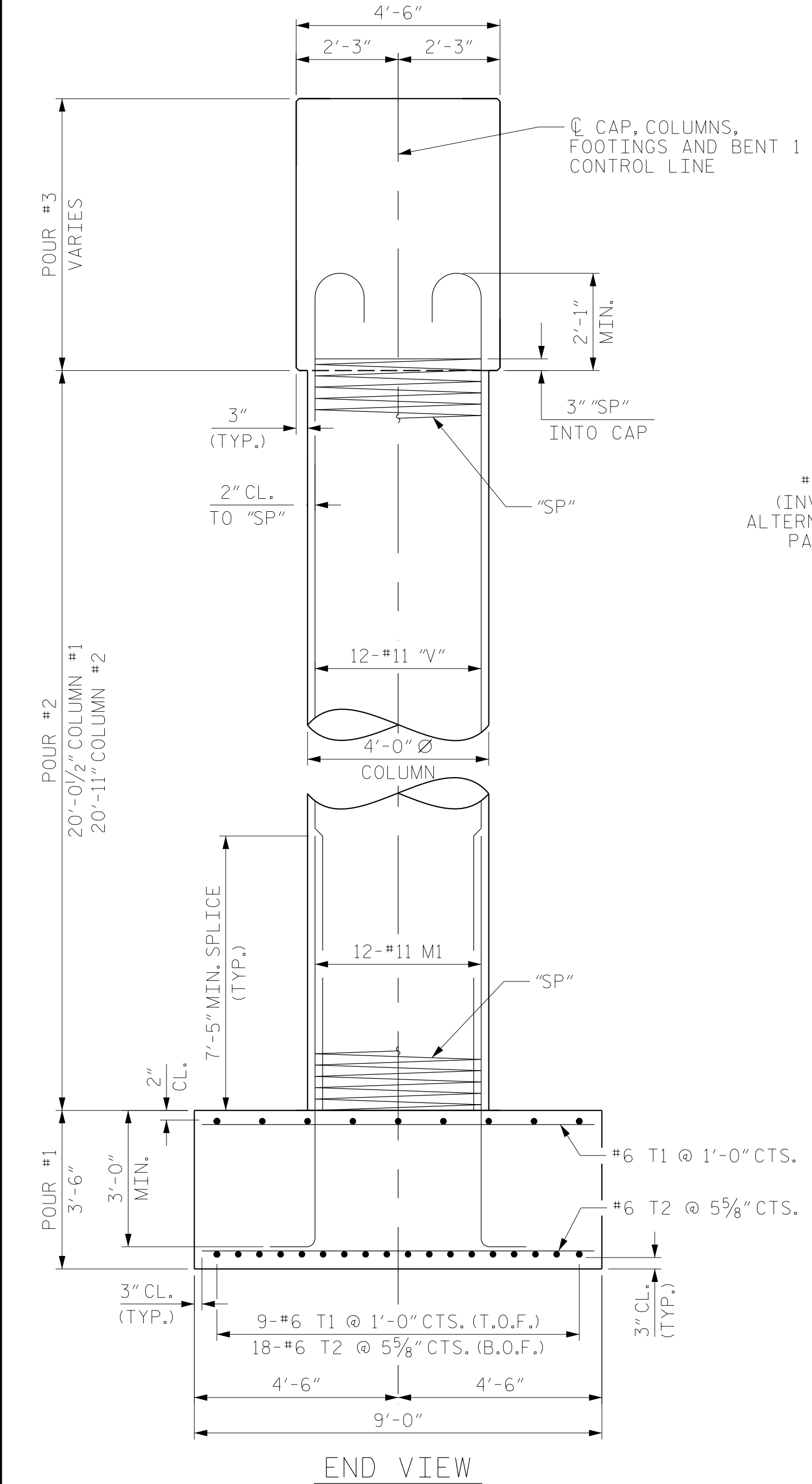
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PLAN OF FOOTINGS

ALL DIMENSIONS AND REINFORCING STEEL ARE TYPICAL FOR EACH FOOTING



BILL OF MATERIAL FOR BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#11		46'-10"	1991
B2	10	#5	STR	43'-8"	455
B3	40	#4	STR	5'-0"	134
B4	8	#11	STR	43'-8"	1856
M1	24	#11	4	15'-6"	1976
S1	94	#6	2	15'-5"	2177
T1	36	#6	STR	8'-6"	460
T2	72	#6	STR	8'-6"	919
U1	45	#4	5	8'-2"	245
U2	10	#4	5	8'-8"	58
U3	12	#4	5	7'-0"	56
V1	12	#11	6	23'-9"	1514
V2	12	#11	6	24'-7"	1567
REINFORCING STEEL					13,408 LBS.
SP-1	4	*	7	958'-9"	640
SP-2	4	*	7	998'-8"	667
SPIRAL COLUMN REINFORCING STEEL (FOR BENT 1)					1307 LBS.
* THE "SP" SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR					
CLASS A CONCRETE BREAKDOWN (FOR BENT 1)					
POUR #1 (FOOTINGS)					21.0 C.Y.
POUR #2 (COLUMNS)					19.1 C.Y.
POUR #3 (CAP)					45.4 C.Y.
TOTAL CLASS A CONCRETE					85.5 C.Y.
FOUNDATION EXCAVATION					LUMP SUM
FOR BENT					

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PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-
 SHEET 2 OF 2



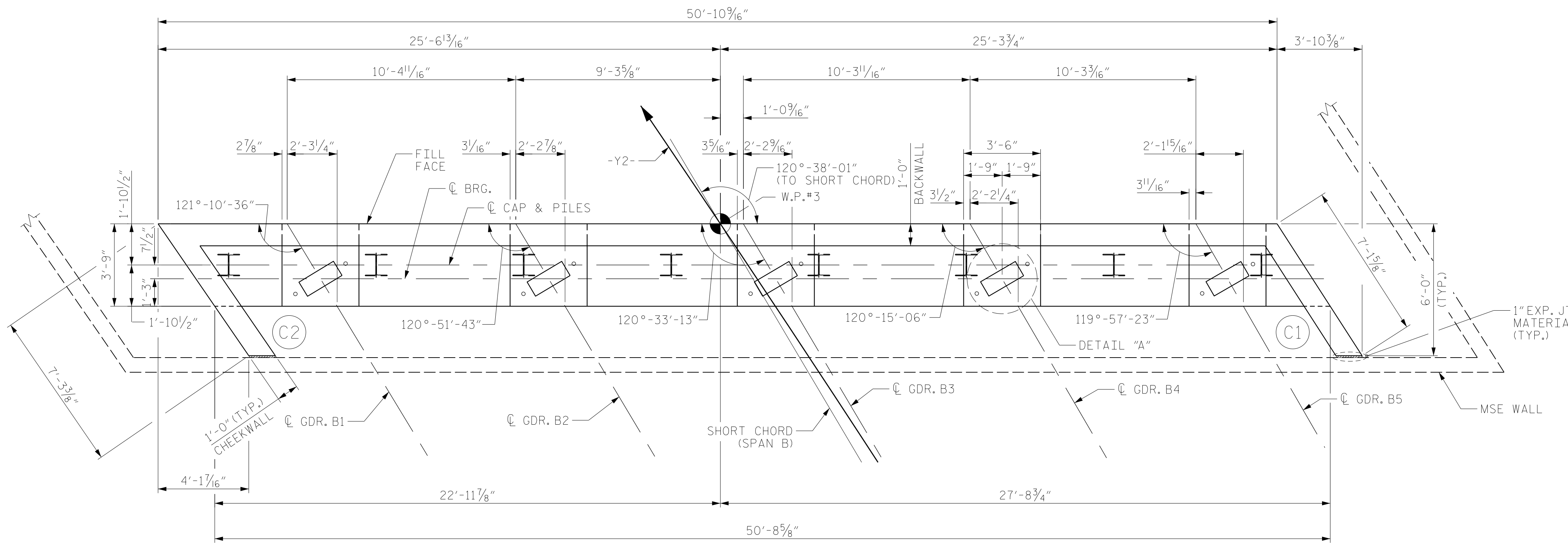
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SUBSTRUCTURE					
BENT NO. 1 DETAILS					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO. S3-30
 TOTAL SHEETS 36

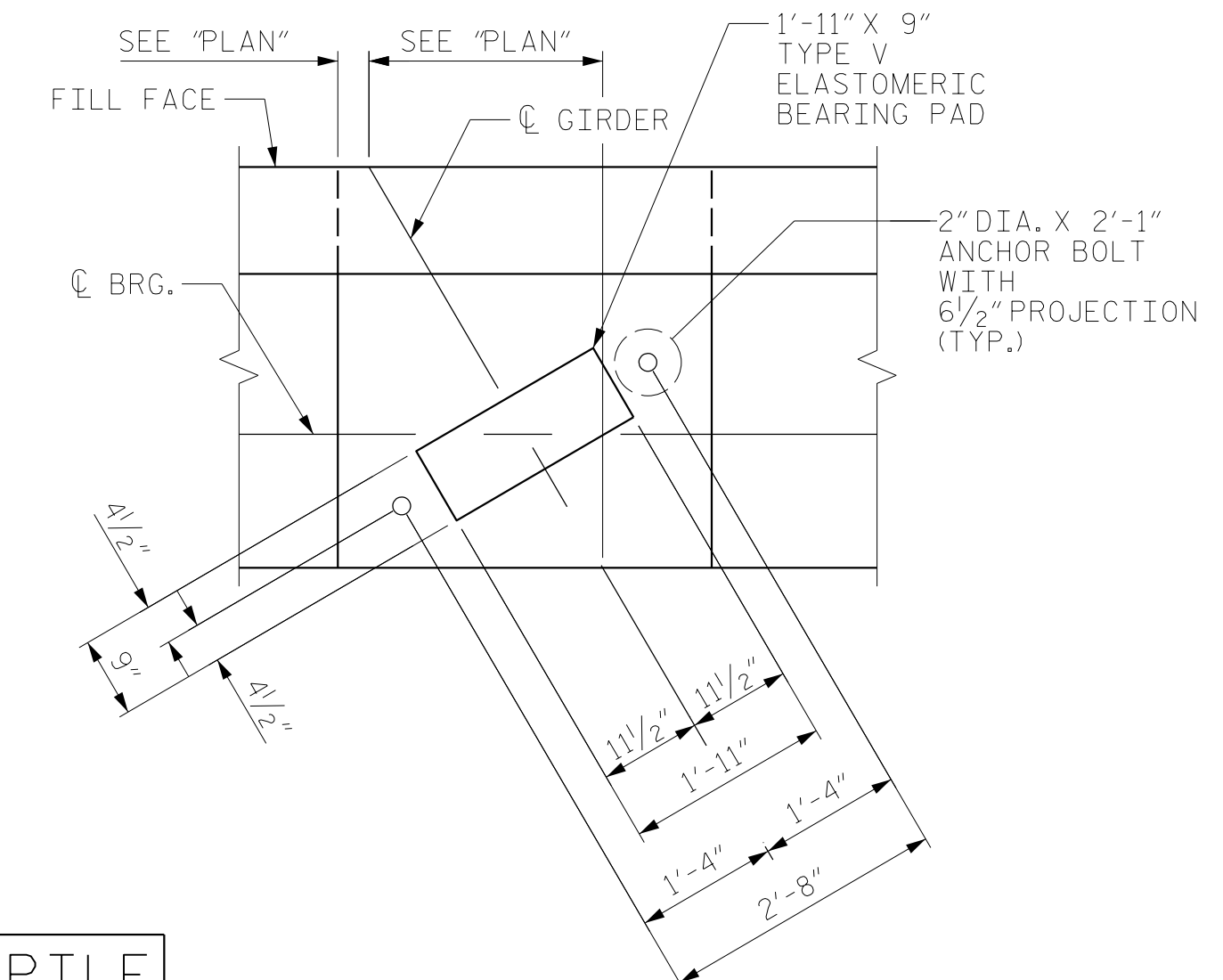
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NOTES

- STIRRUPS, B4 & U2 BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.
- FOR PILE SPLICE DETAILS, SEE END BENT DETAILS SHEET 3 OF 3.
- THE TOP SURFACE AREAS OF THE END BENT CAP SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, EXCEPT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.
- BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING.
- THE TOP SURFACE OF THE END BENT CAP EXCEPT BRIDGE SEAT BUILDUPS SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE FRONT FACE AT A RATE OF 2%.



PLAN



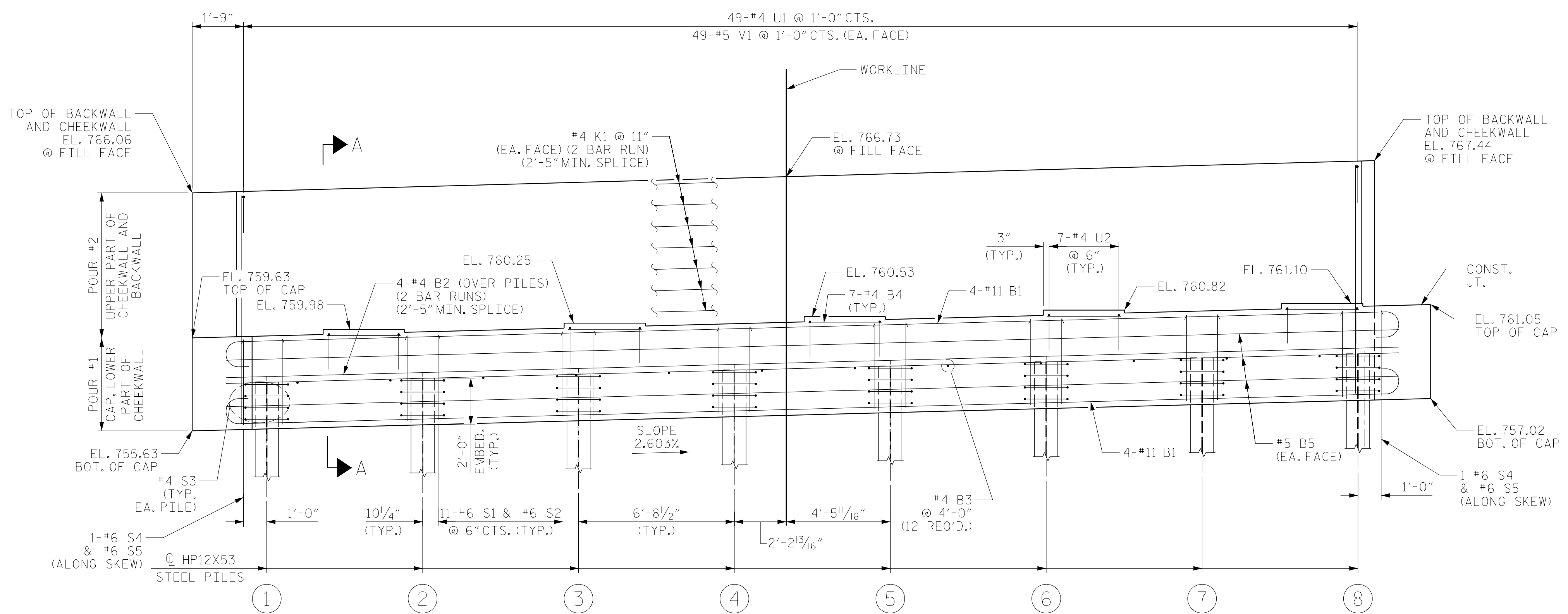
DETAIL "A"

DIMENSIONS TYP. EA. GDR. PILES NOT SHOWN FOR CLARITY

TOP OF PILE ELEVATIONS	
PILE	ELEVATION
1	757.71
2	757.89
3	758.06
4	758.24
5	758.41
6	758.59
7	758.76
8	758.94

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

SHEET 1 OF 3



ELEVATION

CHEEKWALLS NOT SHOWN FOR CLARITY

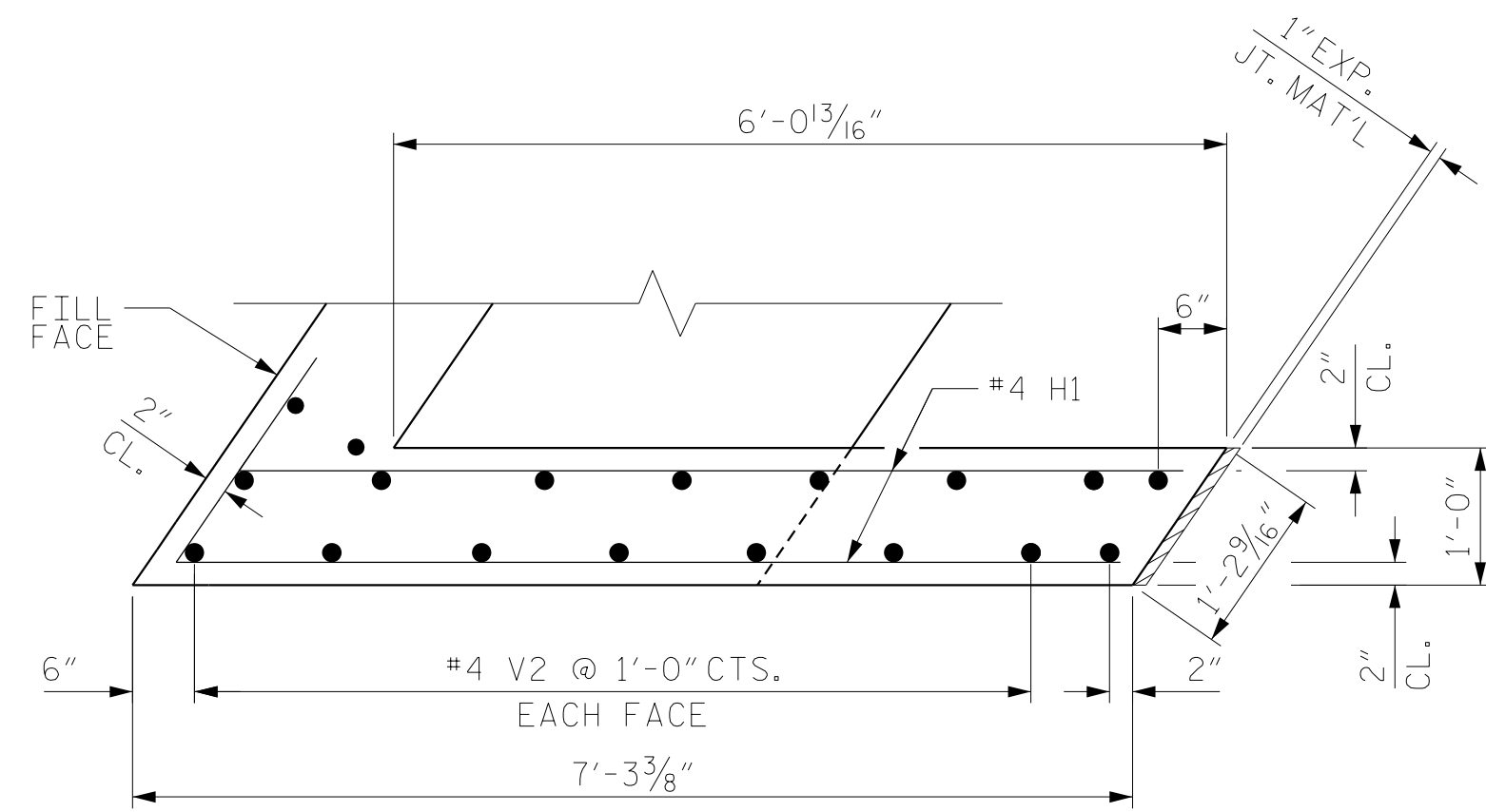


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 North Carolina License No. 50737-F-0403-C-03

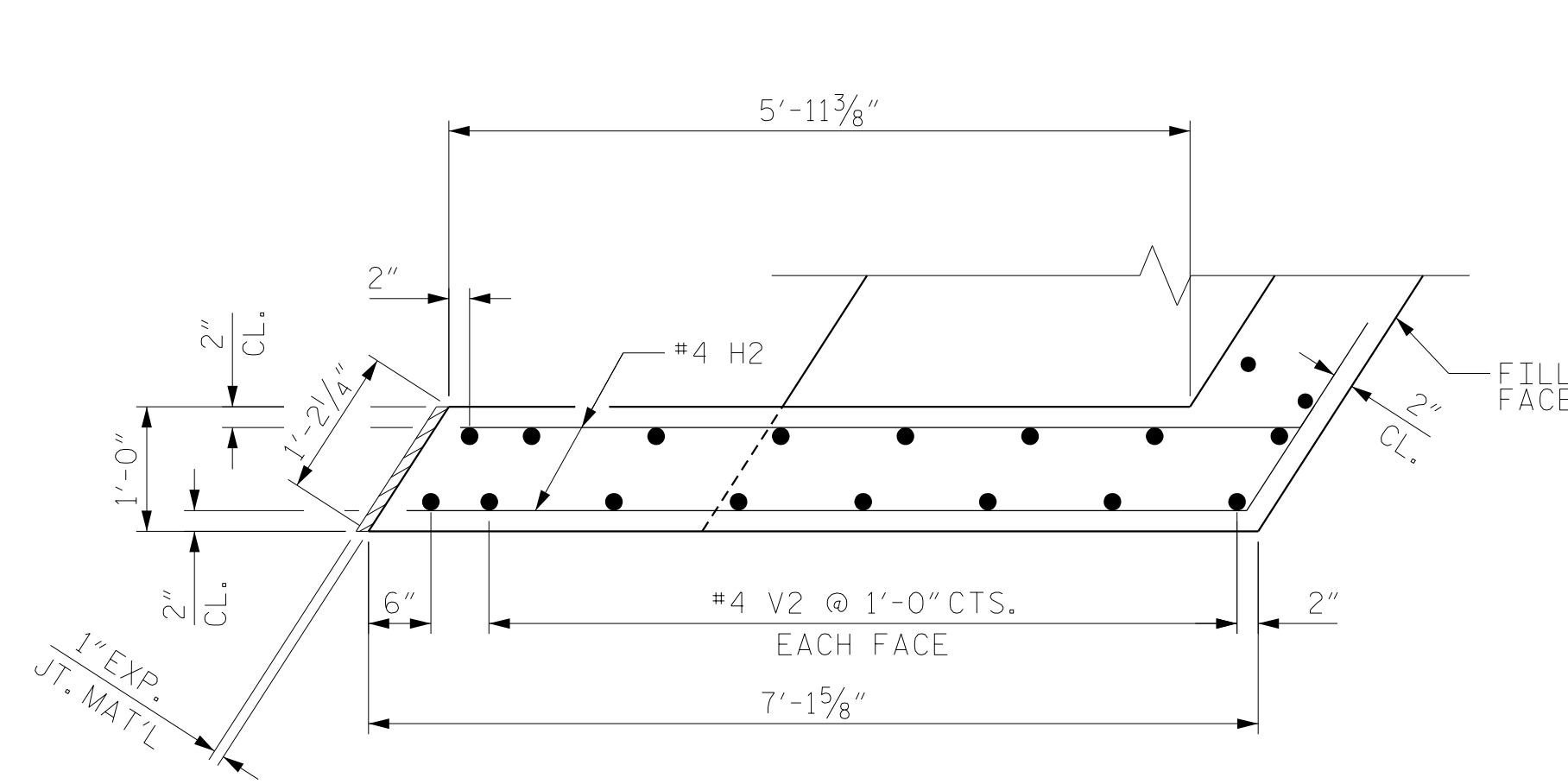
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-31
1			3			TOTAL SHEETS
2			4			36

DRAWN BY : MAL, MKO DATE : 12/2016
 CHECKED BY : JMR DATE : 12/2016
 DESIGN ENGINEER OF RECORD: MKO DATE : 12/2016

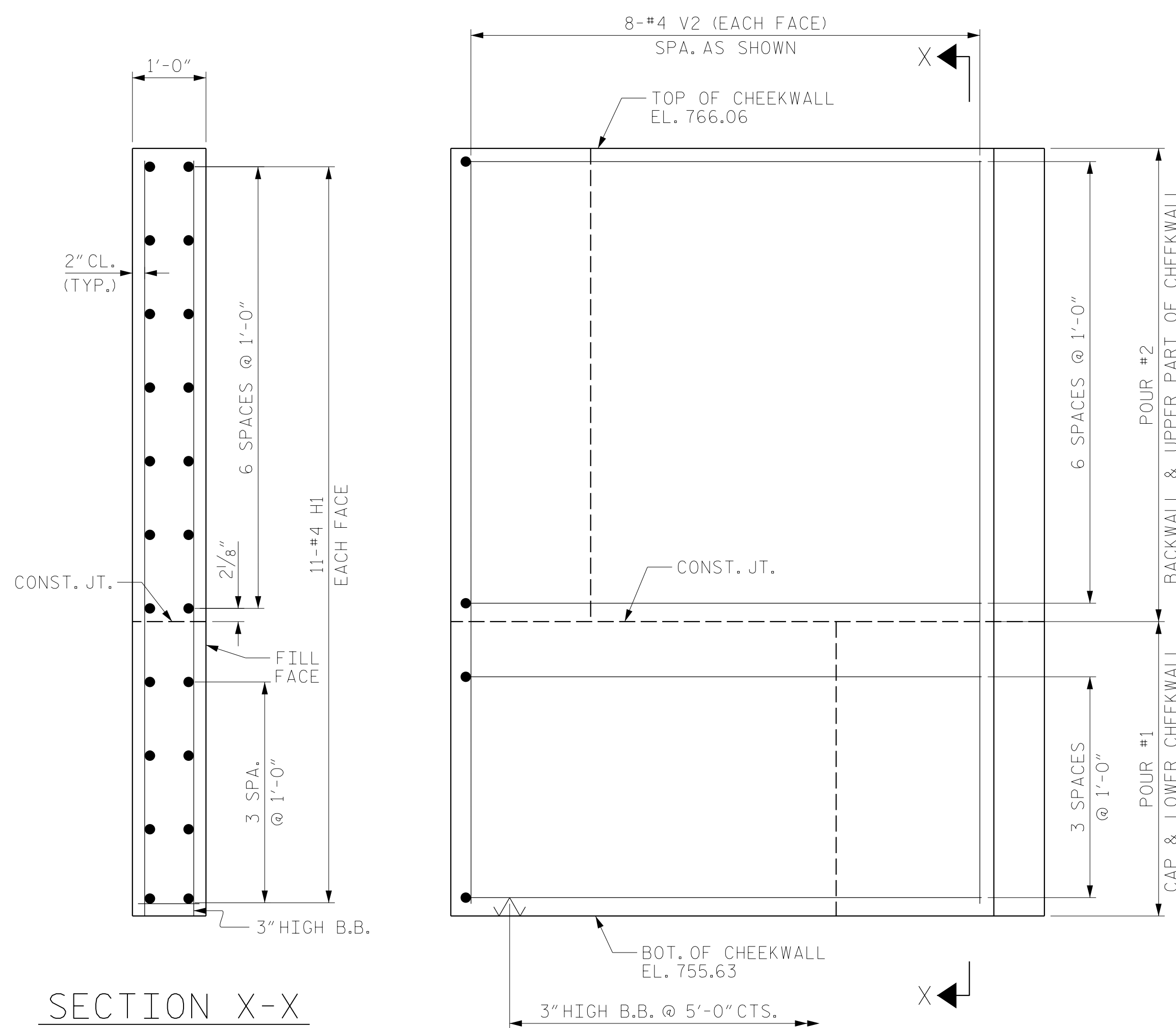
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



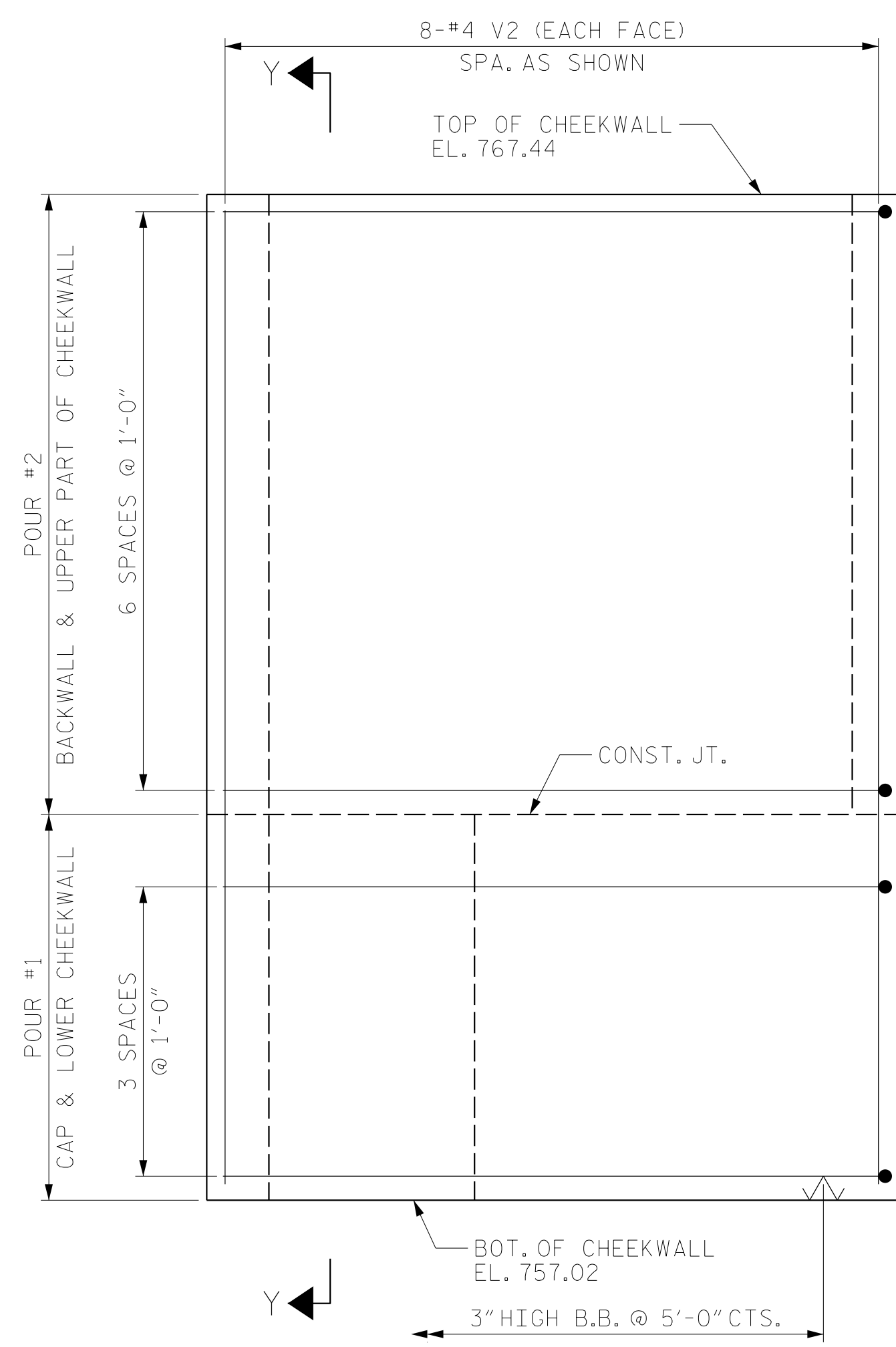
PLAN OF LEFT CHEEKWALL C2



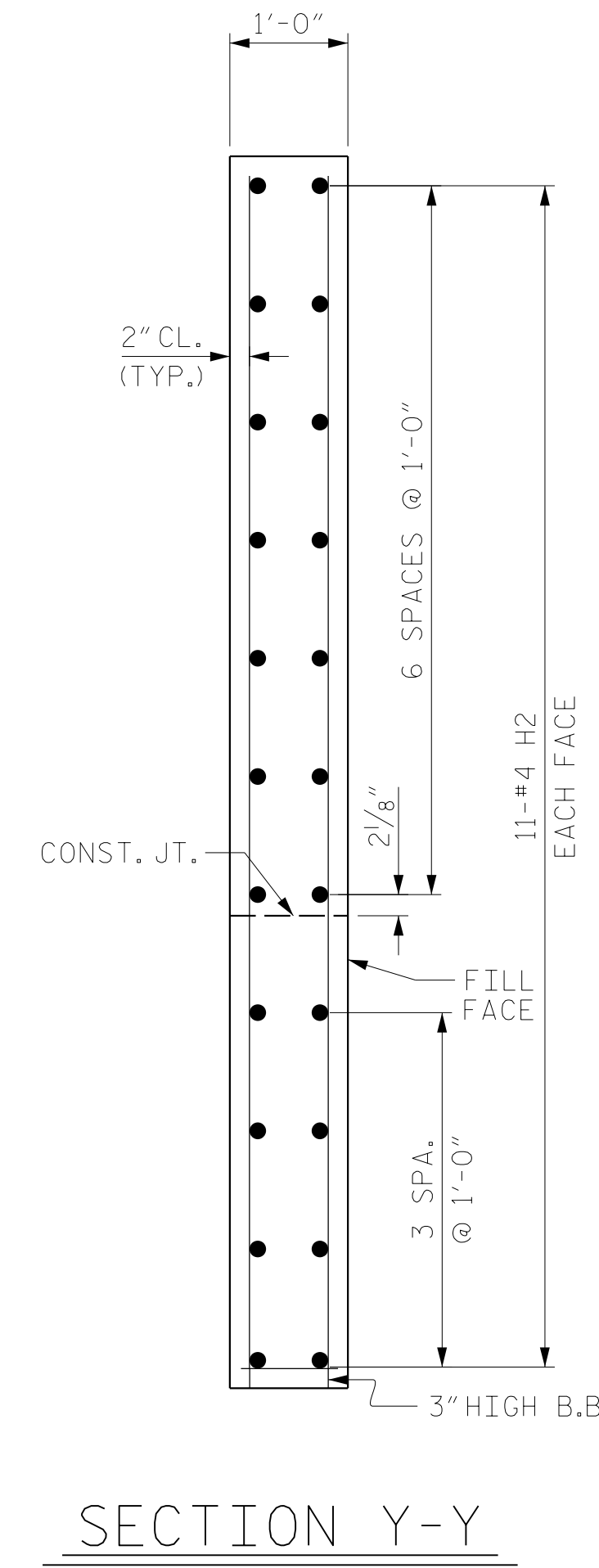
PLAN OF RIGHT CHEEKWALL C1



ELEVATION OF LEFT CHEEKWALL C2



ELEVATION OF RIGHT CHEEKWALL C1



SECTION Y-Y

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 29+07.16 -Y2-

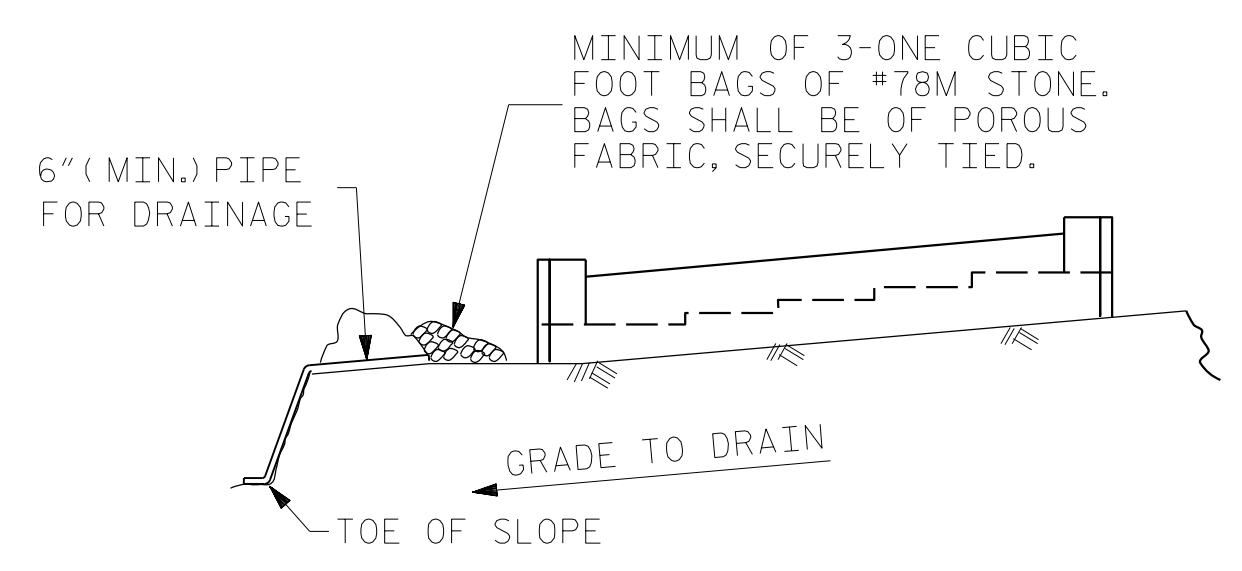
SHEET 2 OF 3



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE END BENT NO. 2					
SHEET NO. S3-32					
TOTAL SHEETS 36					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

DRAWN BY : MAL, MKO DATE : 12/2016
 CHECKED BY : JMR DATE : 12/2016
 DESIGN ENGINEER OF RECORD: MKO DATE : 12/2016

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 SIGNATURES COMPLETED

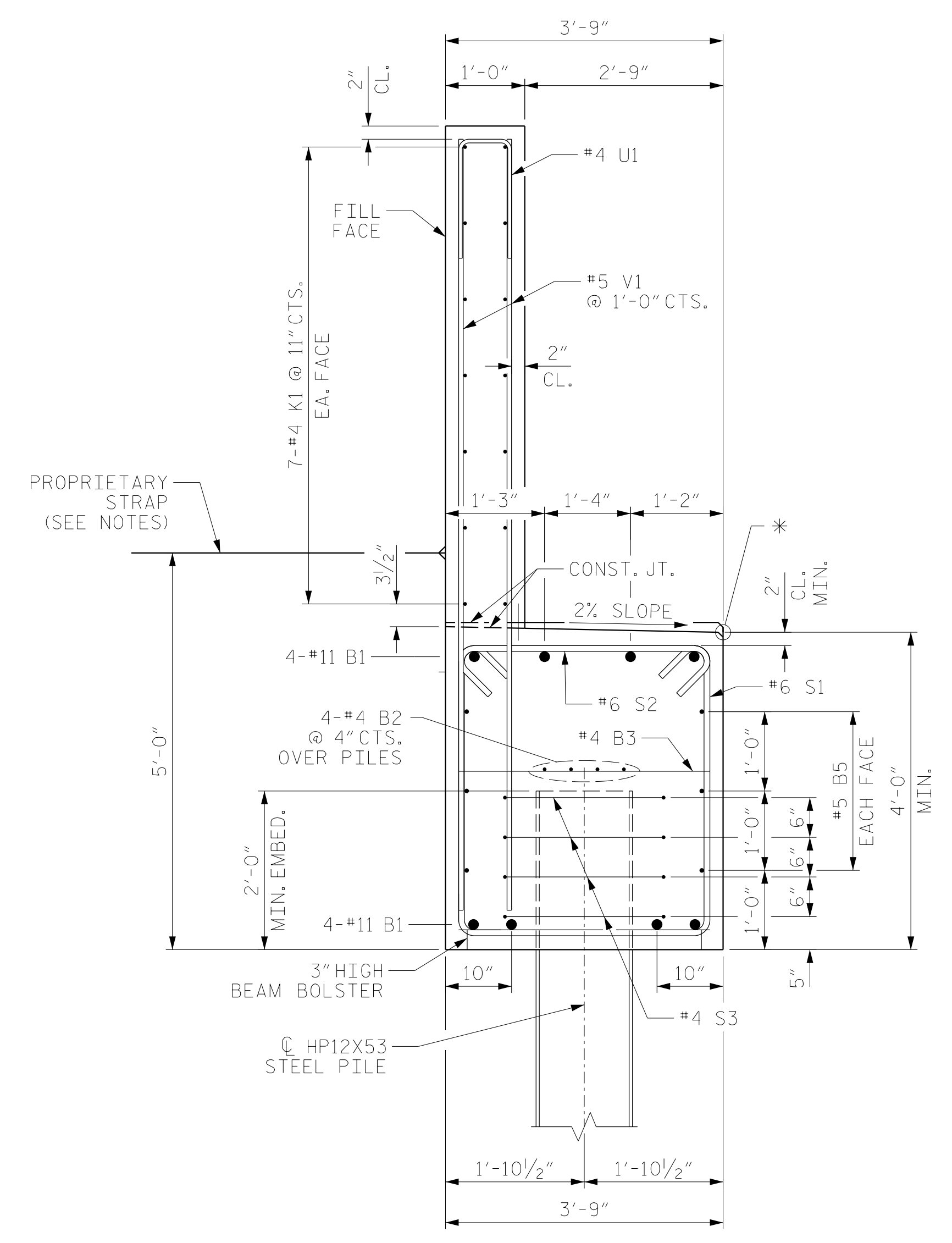


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

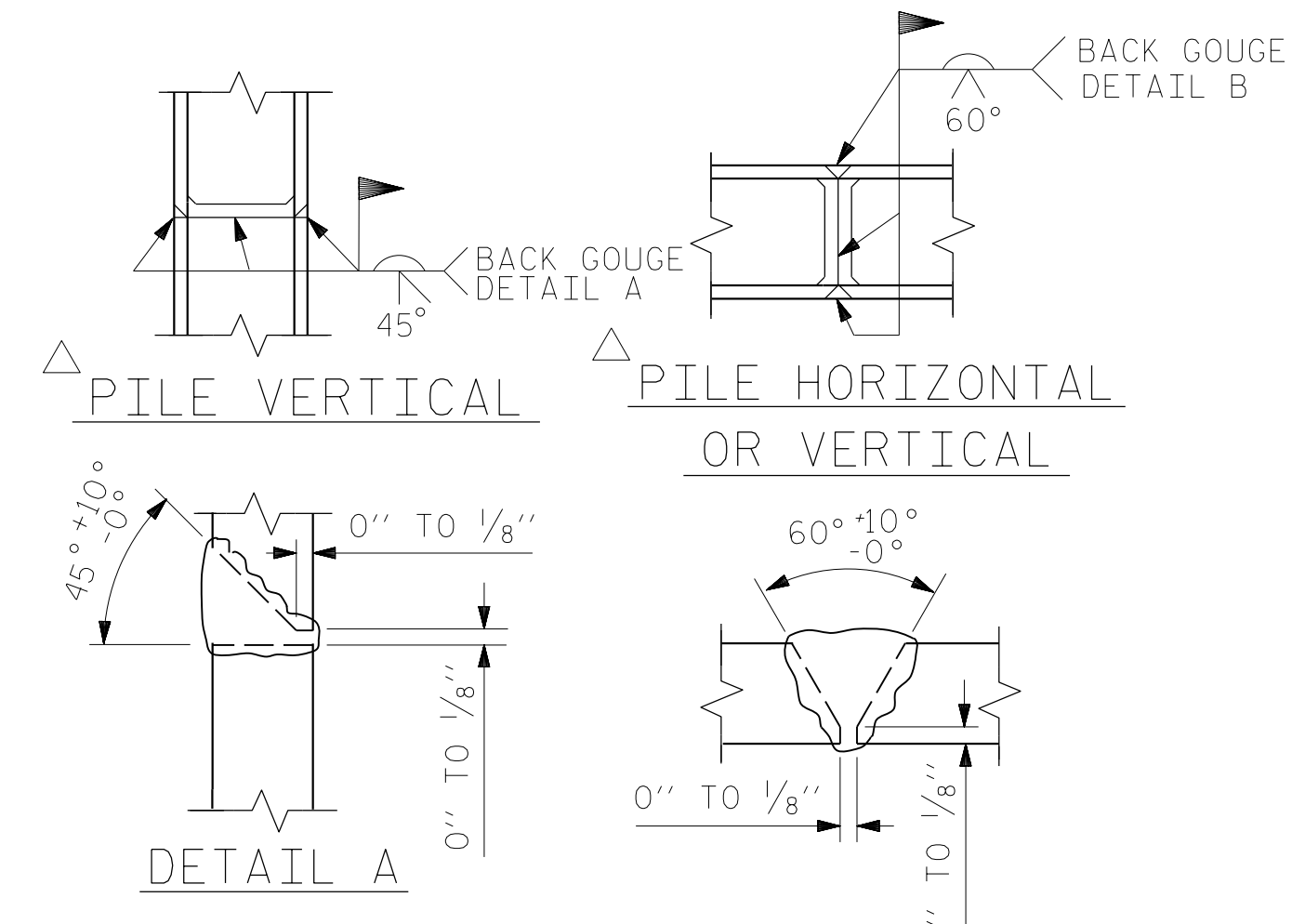
TEMPORARY DRAINAGE AT END BENT



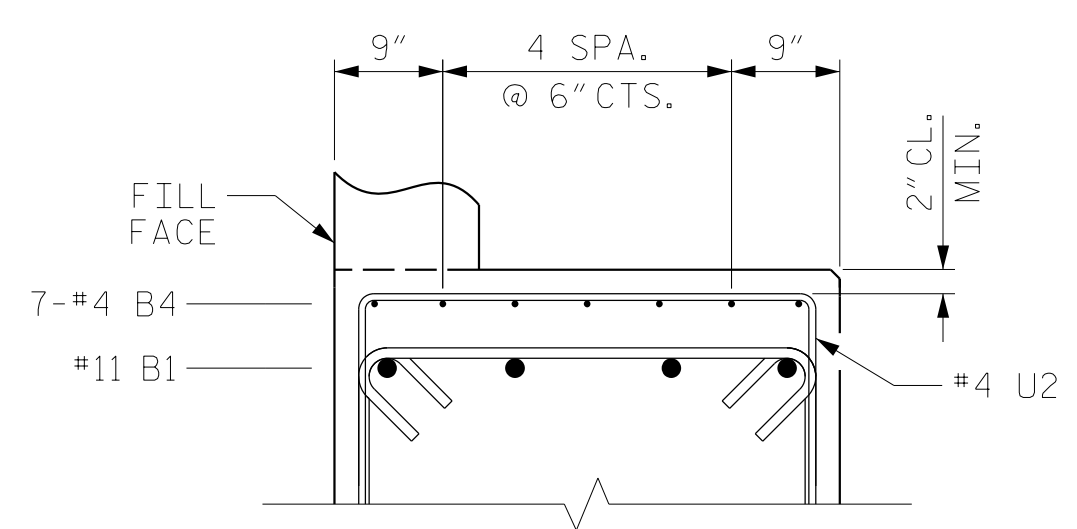
SECTION A-A
*ELEVATIONS BETWEEN BRIDGE SEAT BUILDUPS ARE SHOWN AT THIS POINT

NOTES

THE CONTRACTOR SHALL DESIGN, FURNISH AND INSTALL, A TIE BACK SYSTEM CAPABLE OF RESISTING THE FOLLOWING HORIZONTAL FORCES: 9.5 K/FT UNDER STRENGTH LOADING DEMAND AND 6.6 K/FT UNDER SERVICE LOADING DEMAND. THE CONTRACTOR MAY CHANGE THE STRAP LOCATION SHOWN ABOVE AS LONG AS THE FORCE IS RECALCULATED FOR THE NEW LOCATION. THE TIE BACK SYSTEM SHALL BE SUBMITTED FOR APPROVAL BY THE ENGINEER IN THE FORM OF SHOP DRAWINGS.

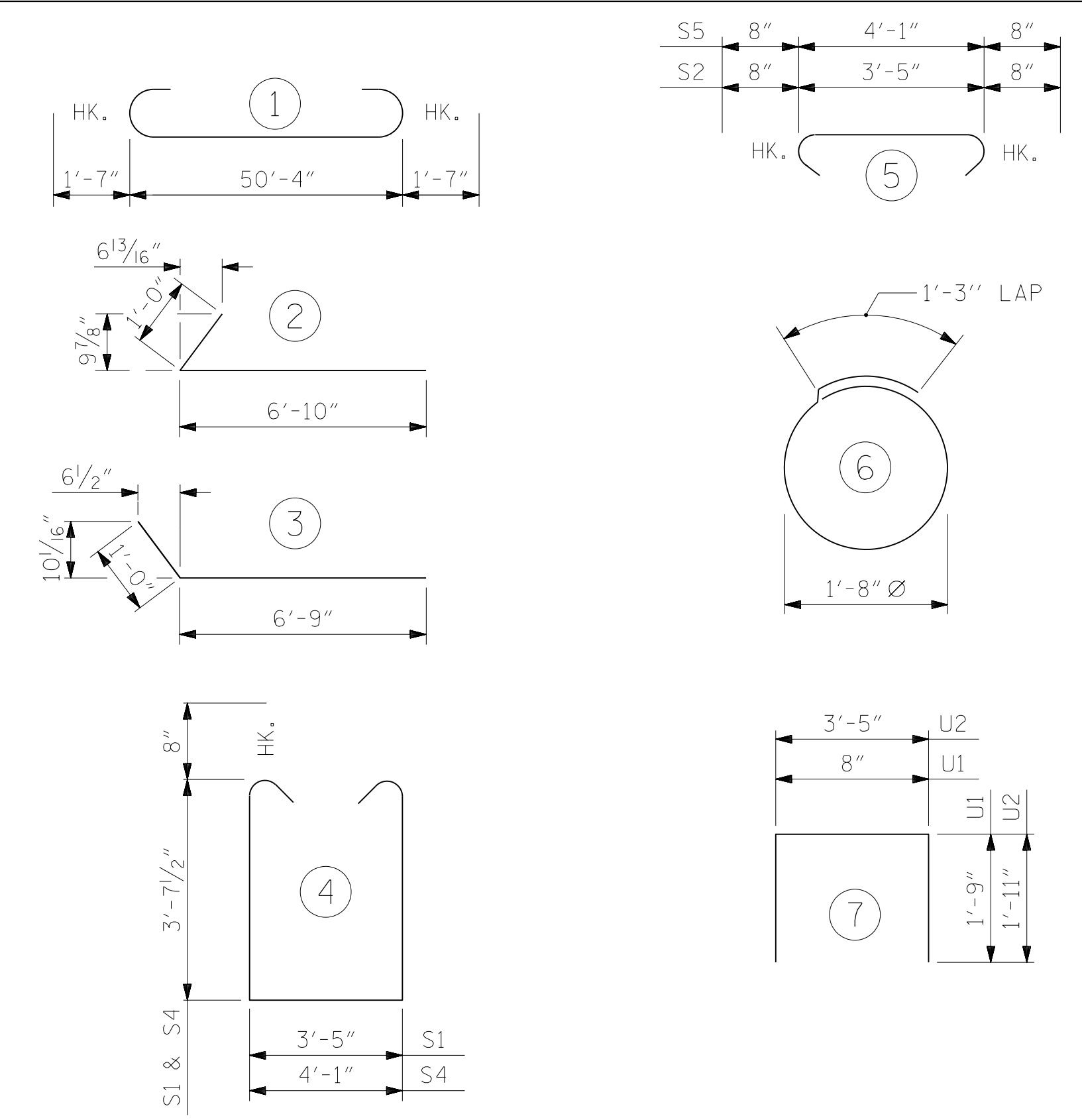


POSITION OF PILE DURING WELDING.
PILE SPLICE DETAILS



SECTION THRU BRIDGE SEAT

BAR TYPE



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL

END BENT #2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#11		53'-6"	2274
B2	8	#4	STR	26'-1"	139
B3	12	#4	STR	3'-5"	27
B4	35	#4	STR	3'-2"	74
B5	6	#5	STR	50'-4"	315
H1	22	#4		7'-10"	115
H2	22	#4		7'-9"	114
K1	28	#4	STR	26'-6"	496
S1	77	#6		12'-0"	1388
S2	77	#6		4'-9"	549
S3	32	#4		6'-6"	139
S4	2	#6		12'-8"	38
S5	2	#6		5'-5"	16
U1	49	#4		4'-2"	136
U2	35	#4		7'-3"	170
V1	98	#5	STR	10'-0"	1022
V2	32	#4	STR	10'-0"	214
REINFORCING STEEL					7,226 LBS.
CLASS A CONCRETE BREAKDOWN					
POUR #1 CAP & LOWER PART OF CHEEKWALL					29.5 C.Y.
POUR #2 UPPER PART OF CHEEKWALL & BACKWALL					14.6 C.Y.
TOTAL CLASS A CONCRETE					44.1 C.Y.
HP 12 X 53 STEEL PILES					
NO: 8					LIN. FT.= 240
PILE DRIVING EQUIPMENT SETUP					
HP 12 X 53					No. 8

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 29+07.16 -Y2-

SHEET 3 OF 3

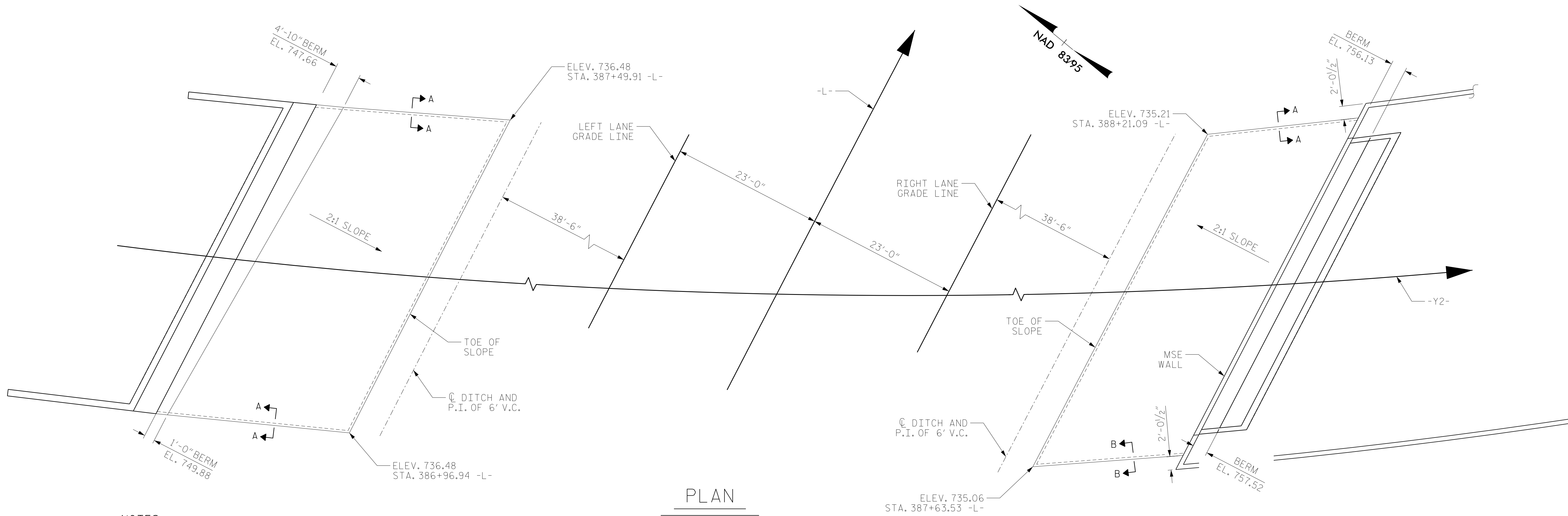
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE
END BENT NO. 2

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			S3-33
2			4			TOTAL SHEETS 36

DRAWN BY : MAL, MKO DATE : 12/2016
CHECKED BY : JMR DATE : 12/2016
DESIGN ENGINEER OF RECORD: MKO DATE : 12/2016

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PLAN

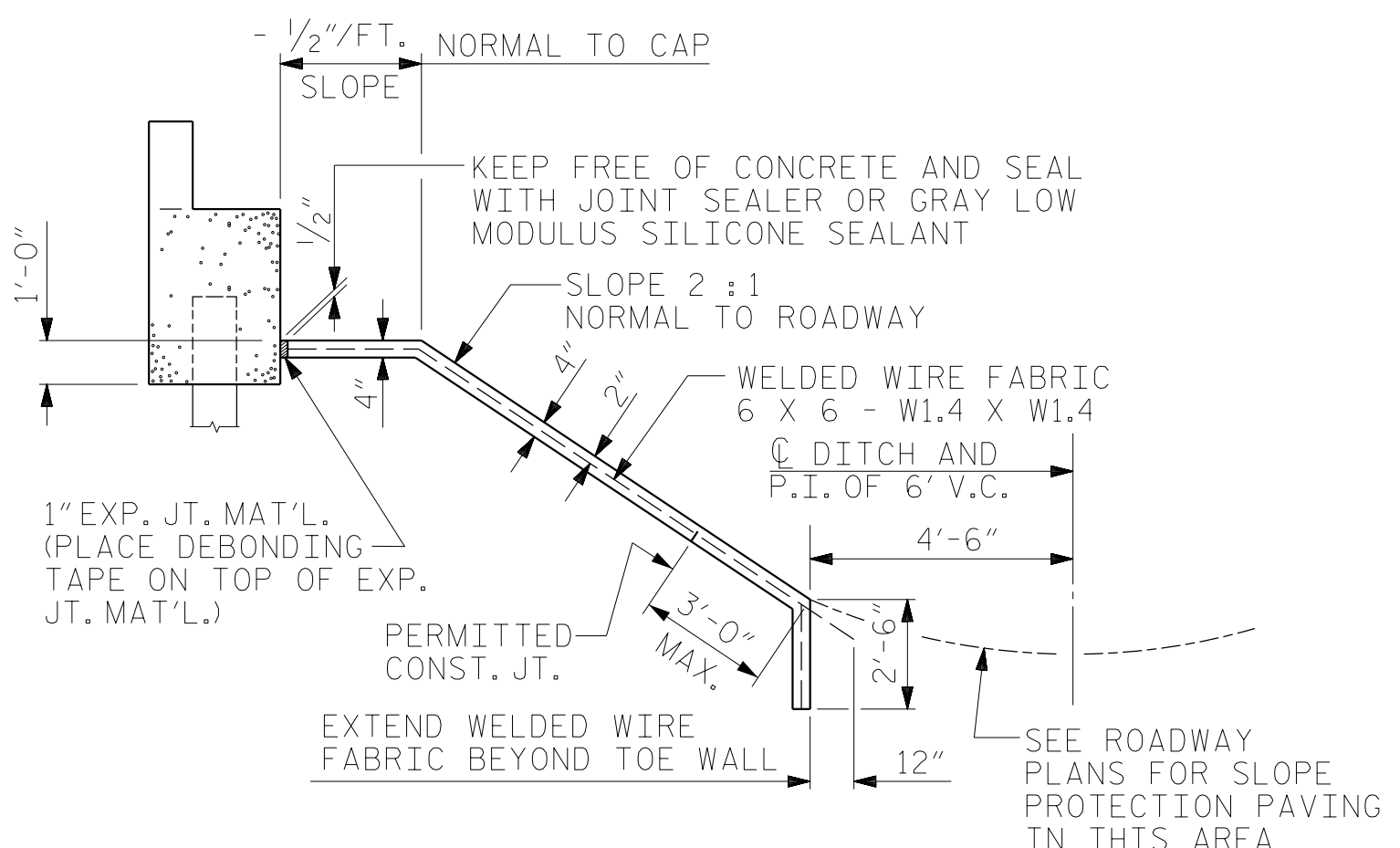
NOTES

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS. FOR BERM WIDTH, SEE GENERAL DRAWING.

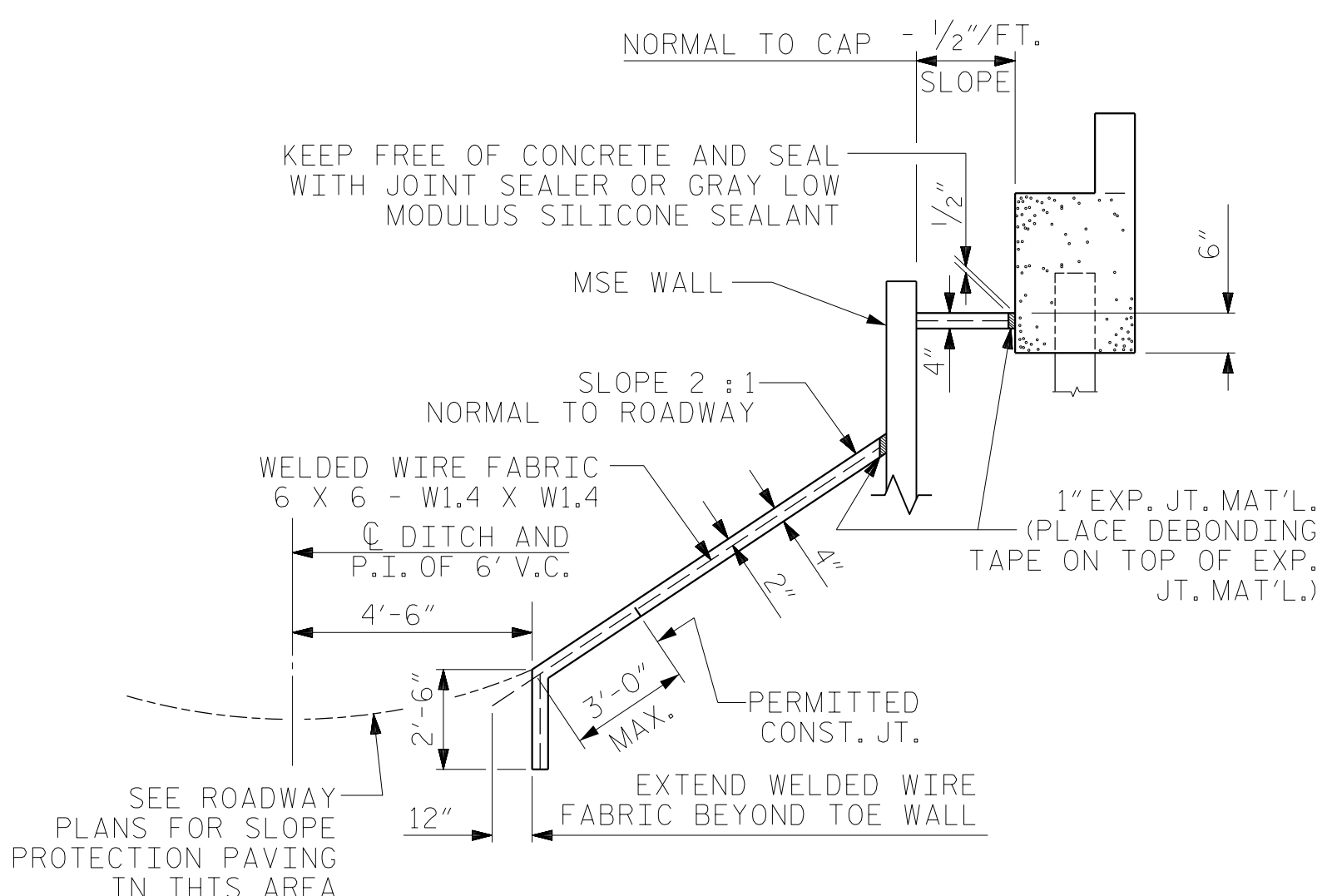
SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0" LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA. 29+07.16 -Y2-	4" INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX. L.F.
END BENT 1	143	292
END BENT 2	175	332

* QUANTITY SHOWN IS BASED ON 5' POURS.



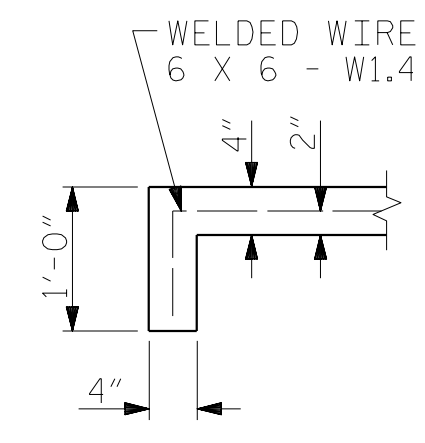
SECTION A-A SECTION ALONG C ROADWAY WHEN FILL CATCHES IN DITCH
END BENT 1



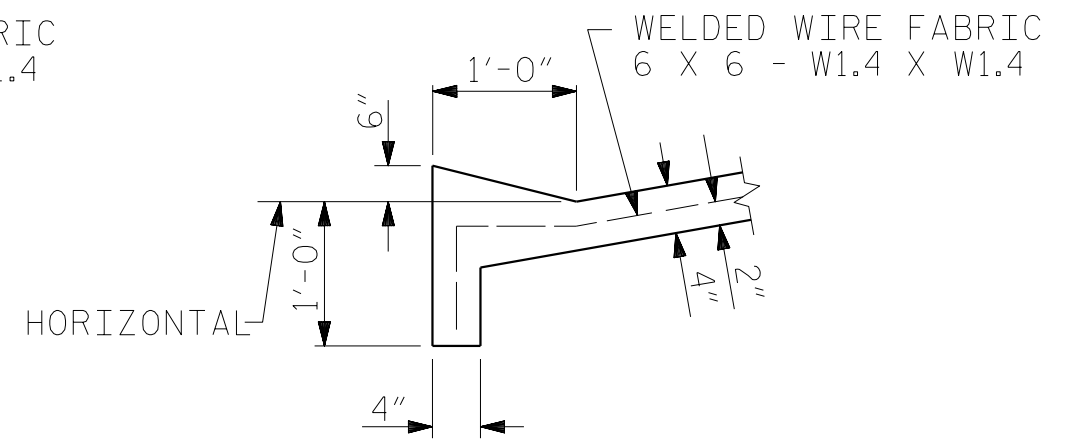
SECTION B-B SECTION ALONG C ROADWAY WHEN FILL CATCHES IN DITCH
END BENT 2

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
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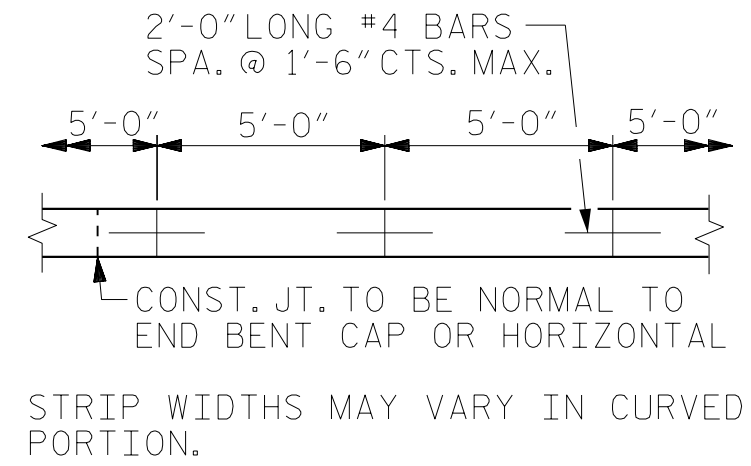
PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 29+07.16 -Y2-



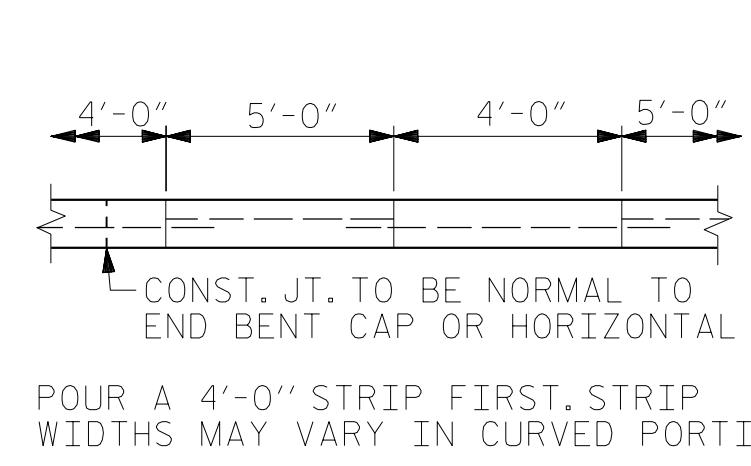
SECTION A-A



SECTION B-B



POURING DETAIL



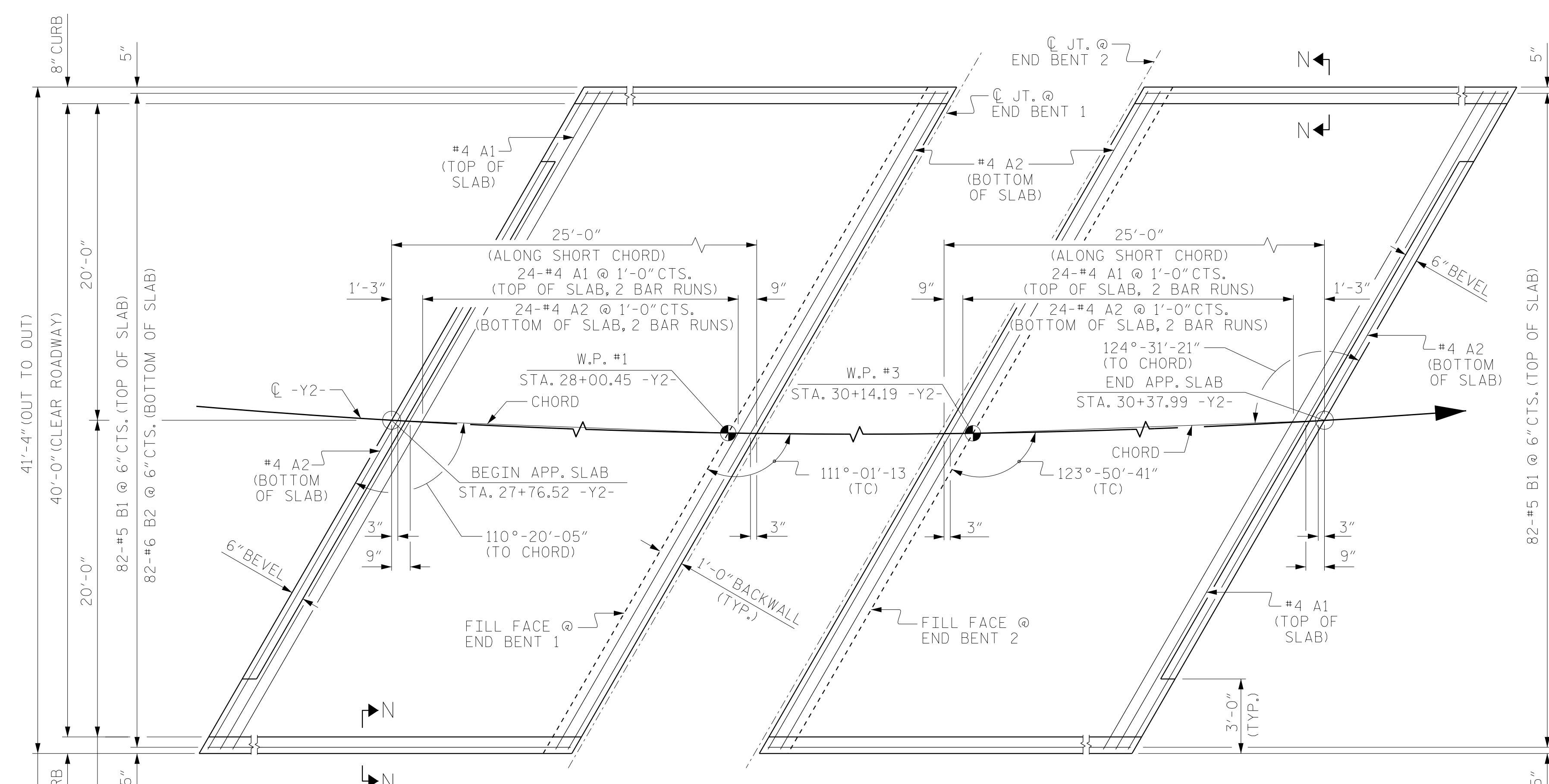
OPTIONAL POURING DETAIL

DRAWN BY : MKO DATE : 12/2016
CHECKED BY : JMR DATE : 12/2016
DESIGN ENGINEER OF RECORD: MKO DATE : 12/2016

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SLOPE PROTECTION
DETAILS

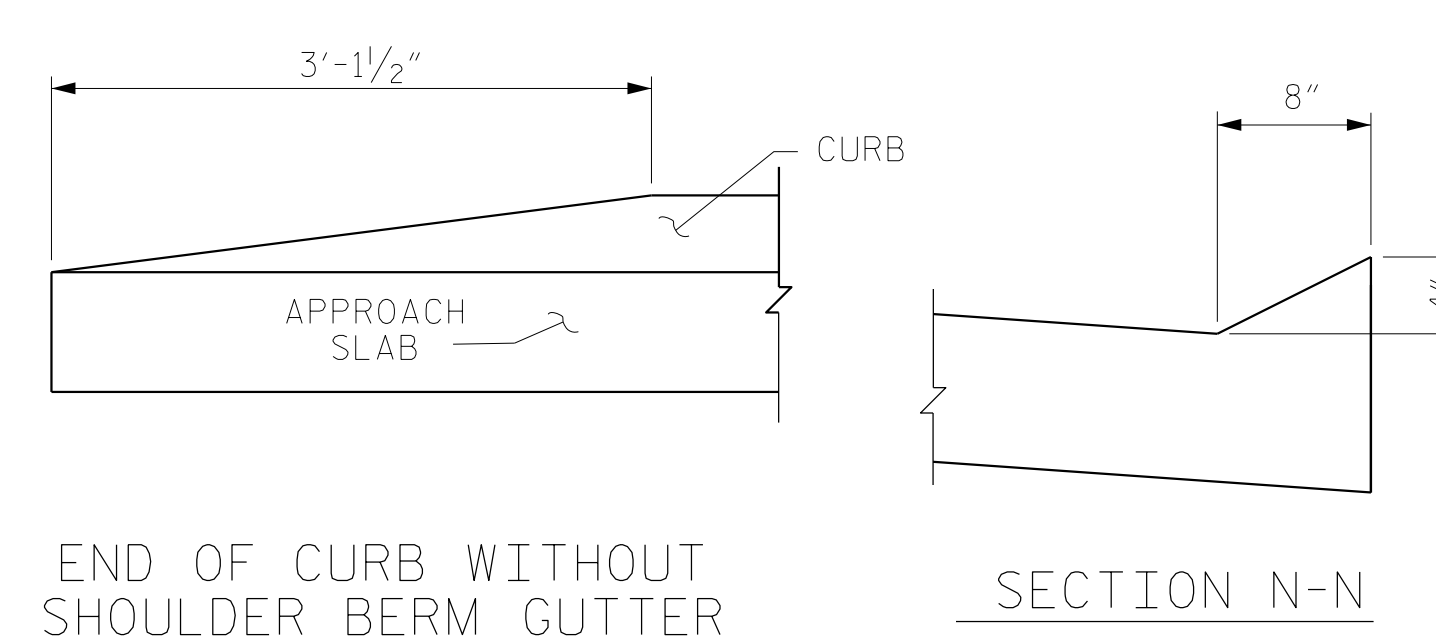
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-34
1			3			TOTAL SHEETS
2			4			36



PLAN @ END BENT 1 PLAN @ END BENT 2
DIMENSIONS SHOWN ARE TYPICAL FOR BOTH APPROACH SLABS

NOTES

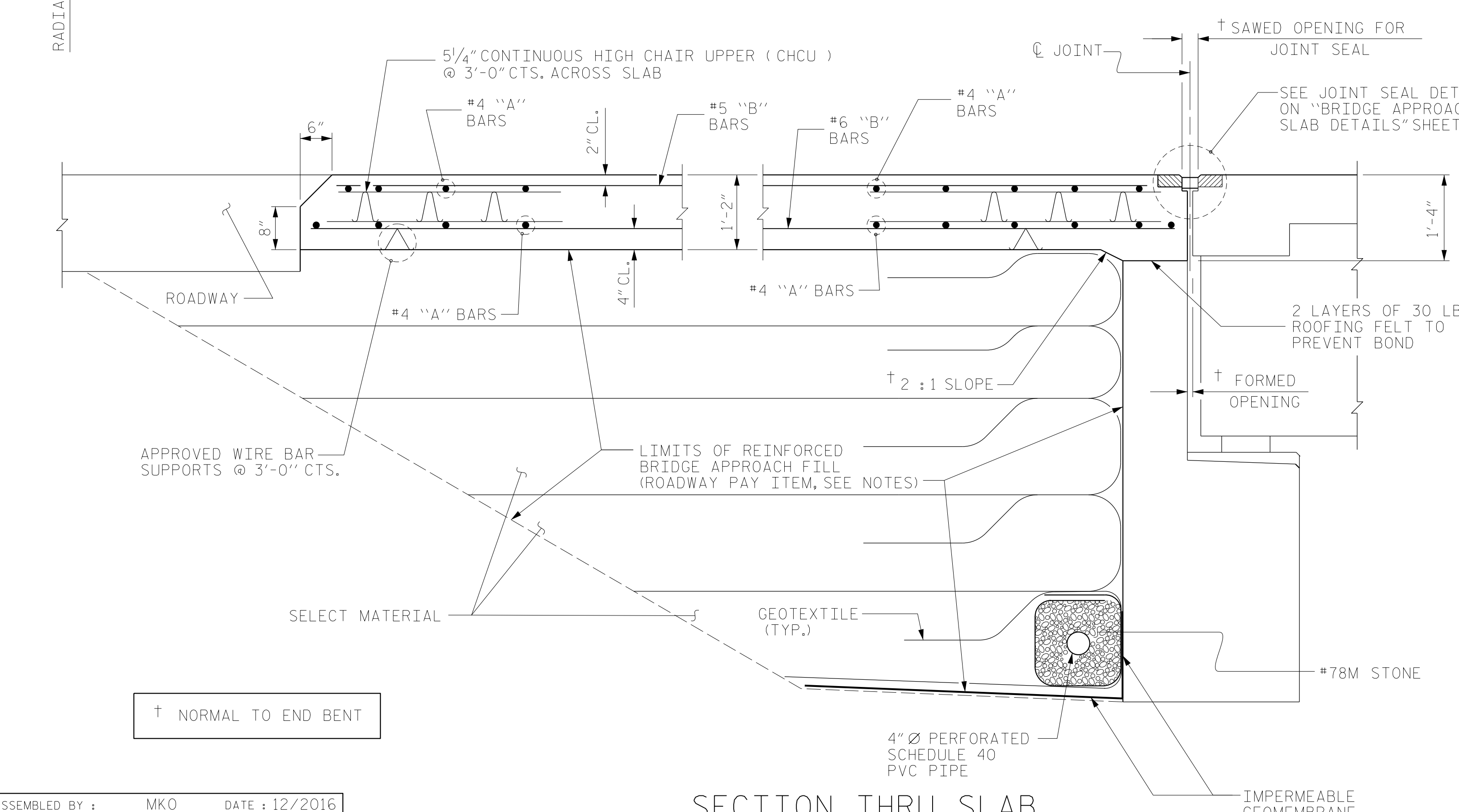
- APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.
- FOR REINFORCED BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.
- AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.
- THE JOINT SHALL BE SAWS PRIOR TO THE CASTING OF THE BARRIER RAIL OR PARAPET AND END POST.
- "A" BARS TO BE PLACED PARALLEL TO SKEW AND SPACED ALONG SHORT CHORD.
- "B" BARS TO BE PLACED PARALLEL TO SHORT CHORD.
- WITH FOAM JOINT SEAL
- FOR FOAM JOINT SEALS, SEE SPECIAL PROVISIONS.
- THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE FOAM JOINT SEAL SHALL BE 2".
- FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.



CURB DETAILS

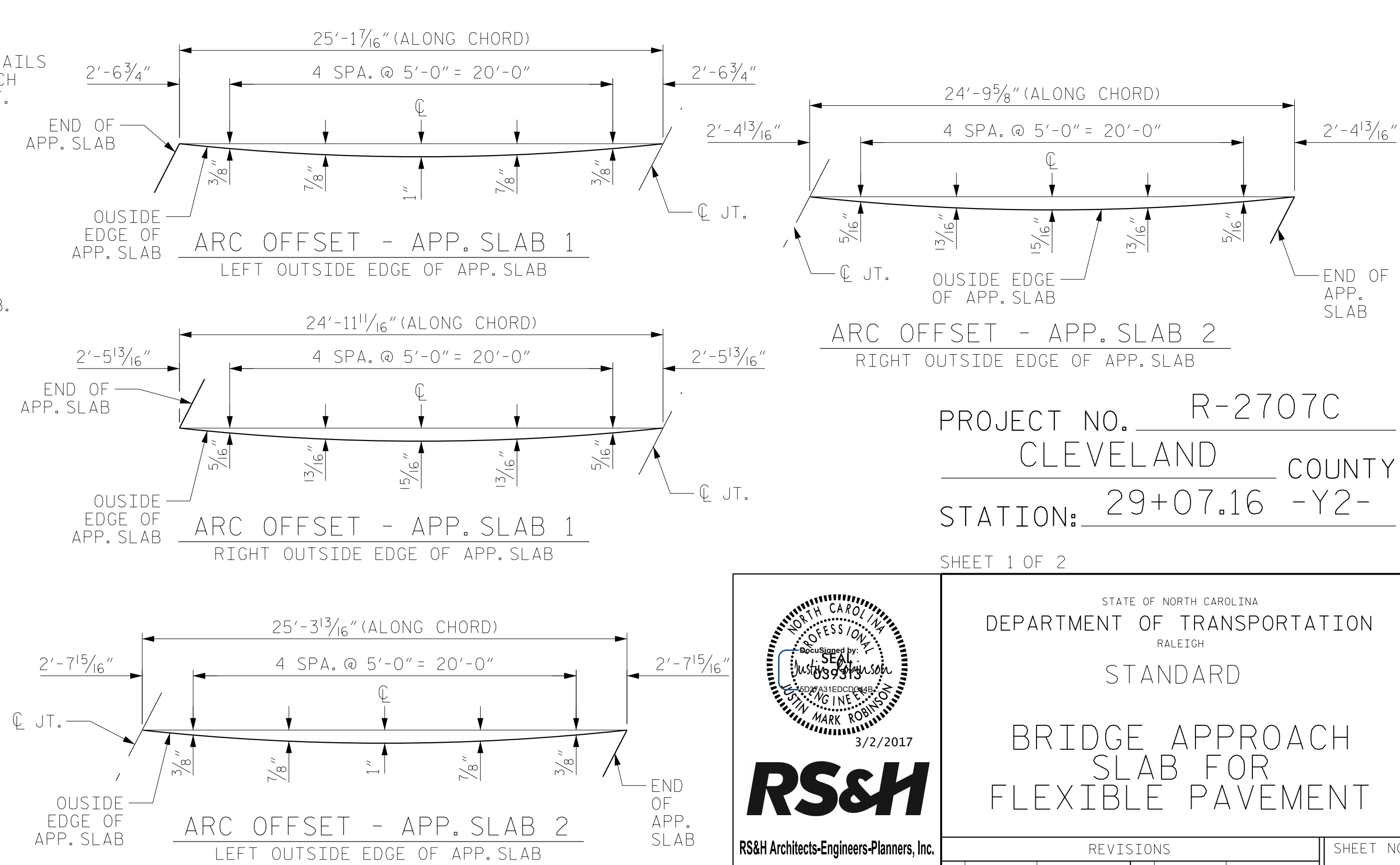
BILL OF MATERIAL					
APPROACH SLAB AT EB #1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	50	#4	STR	23'-0"	768
A2	52	#4	STR	22'-11"	796
* B1	82	#5	STR	24'-0"	2053
B2	82	#6	STR	24'-5"	3007
REINFORCING STEEL				LBS.	3803
* EPOXY COATED REINFORCING STEEL				LBS.	2821
CLASS AA CONCRETE				C. Y.	44.7
APPROACH SLAB AT EB #2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	50	#4	STR	26'-2"	874
A2	52	#4	STR	26'-0"	903
* B1	82	#5	STR	24'-0"	2053
B2	82	#6	STR	24'-5"	3007
REINFORCING STEEL				LBS.	3910
* EPOXY COATED REINFORCING STEEL				LBS.	2927
CLASS AA CONCRETE				C. Y.	44.7

SPLICE LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"
#5	2'-6"	2'-2"
#6	3'-10"	2'-7"



SECTION THRU SLAB

END BENT 1 SHOWN. FOR SECTION AT END BENT 2, SEE SHEET 2 OF 2.



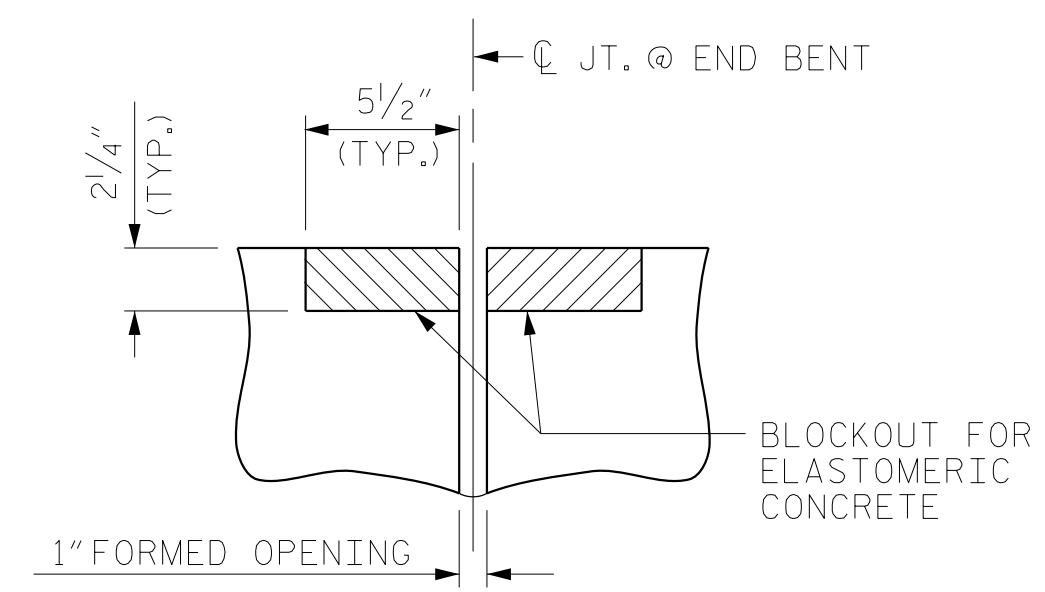
PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 29+07.16 -Y2-

SHEET 1 OF 2

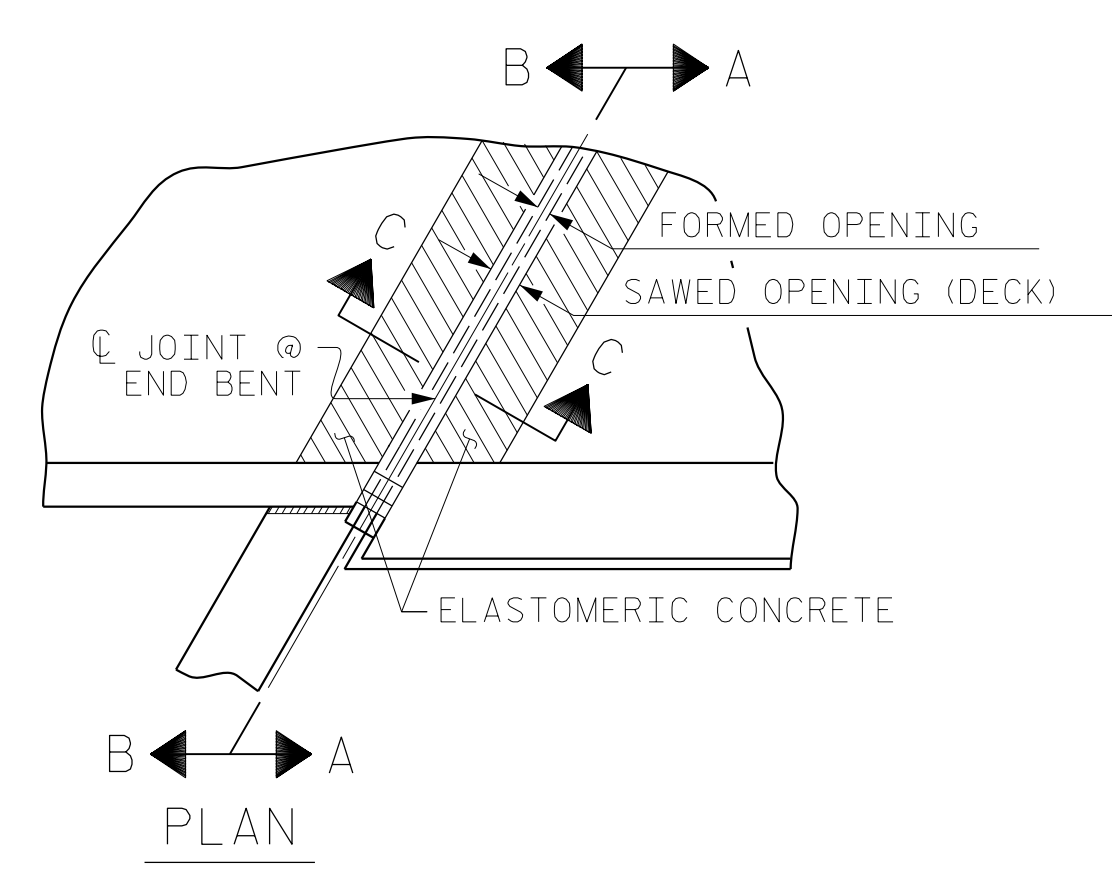
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
STANDARD					
BRIDGE APPROACH SLAB FOR FLEXIBLE PAVEMENT					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

ASSEMBLED BY :	MKO	DATE :	12/2016
CHECKED BY :	JMR	DATE :	12/2016
DRAWN BY :	EEM 3/95	REV. 10/11/11	MAA/GM
CHECKED BY :	VAP 3/95	REV. 12/21/11	MAA/GM
		REV. 6/13	MAA/GM

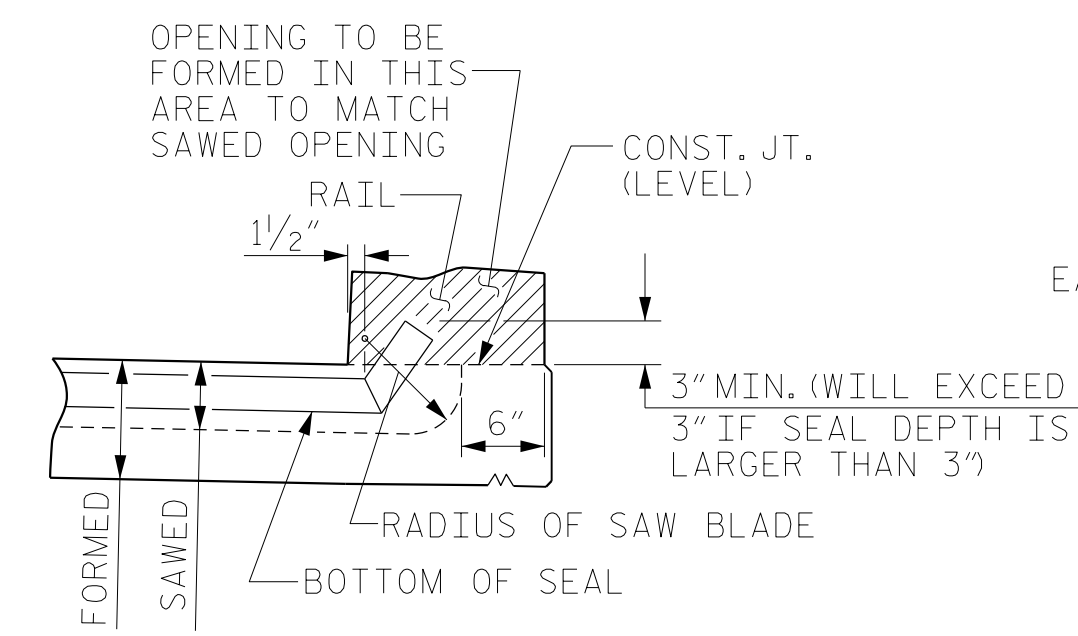
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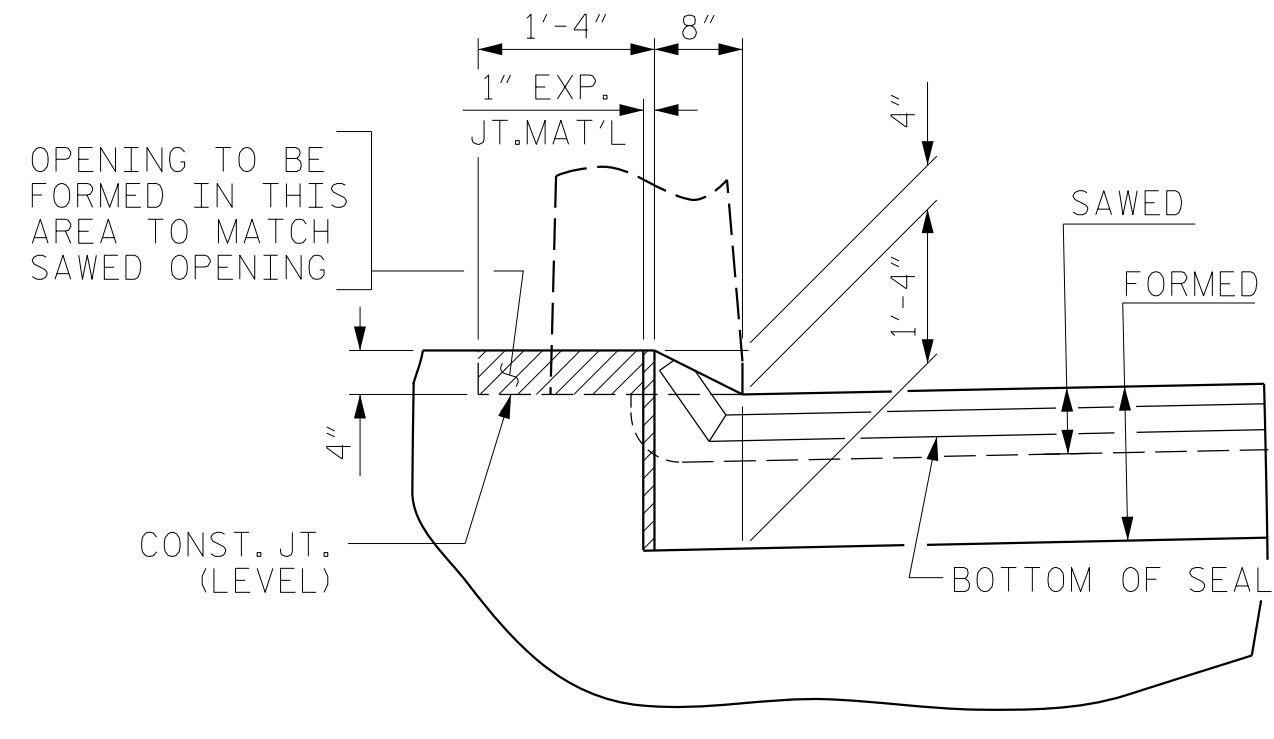
SECTION C-C
FOAM JOINT SEAL
(PRE-SAWED ELASTOMERIC
CONCRETE DIMENSIONS)



PLAN



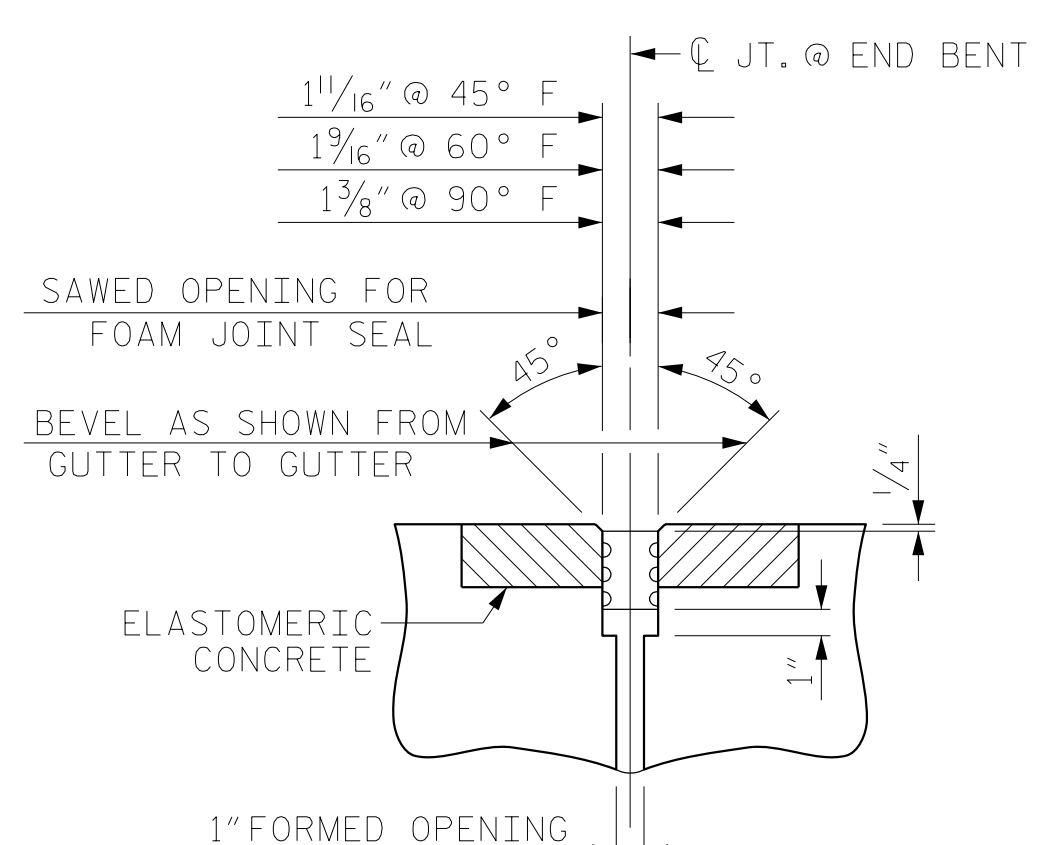
SECTION A-A



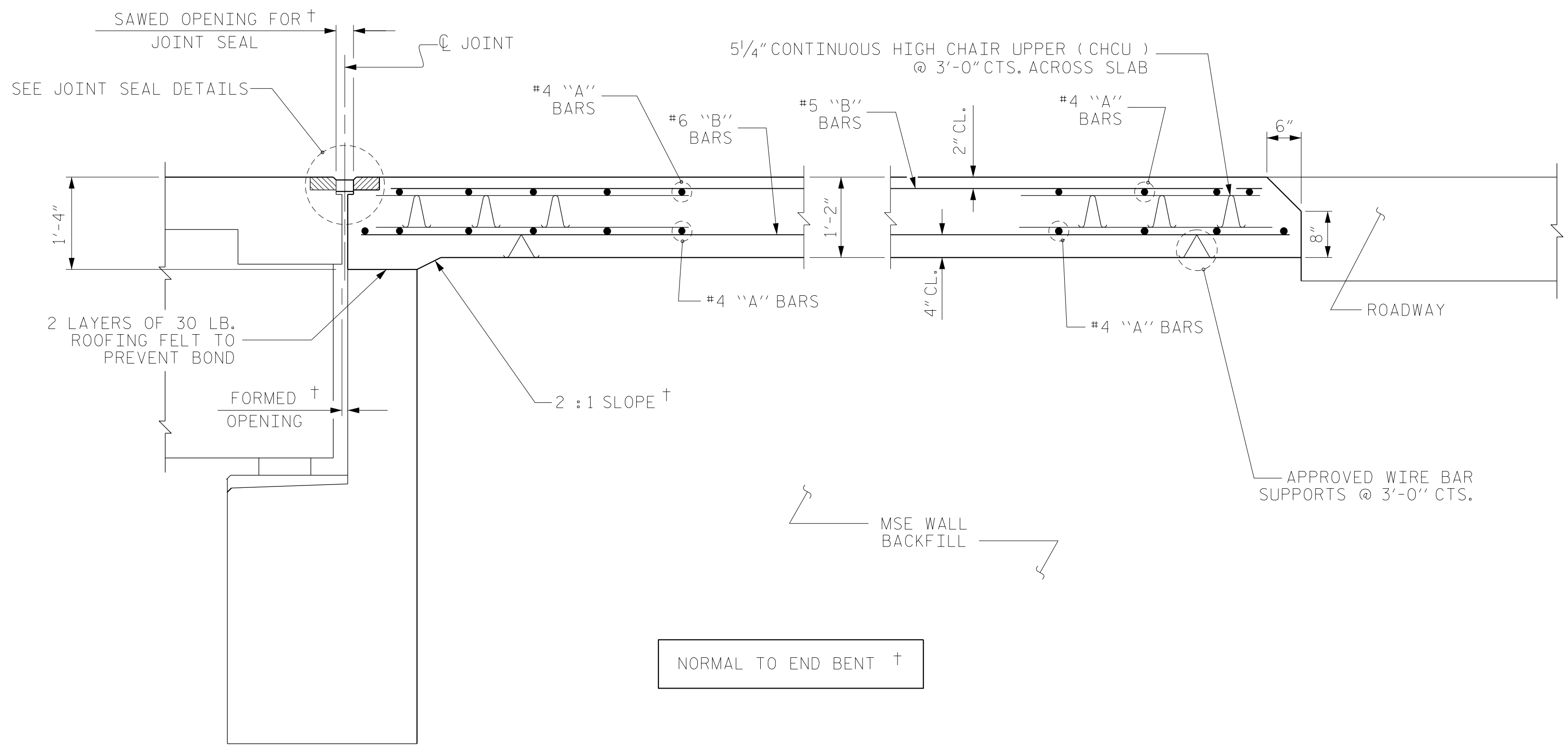
SECTION B-B

JOINT SEAL DETAILS @ END BENT

FOAM JOINT SEAL TO BE CUT, HEAT WELDED AND TURNED UP PARALLEL TO SLOPED FACE OF THE BARRIER RAIL.
THE JOINT SHALL BE SAWED PRIOR TO THE CASTING OF THE BARRIER RAIL.



SECTION C-C
FOAM JOINT SEAL
(EXPANSION)

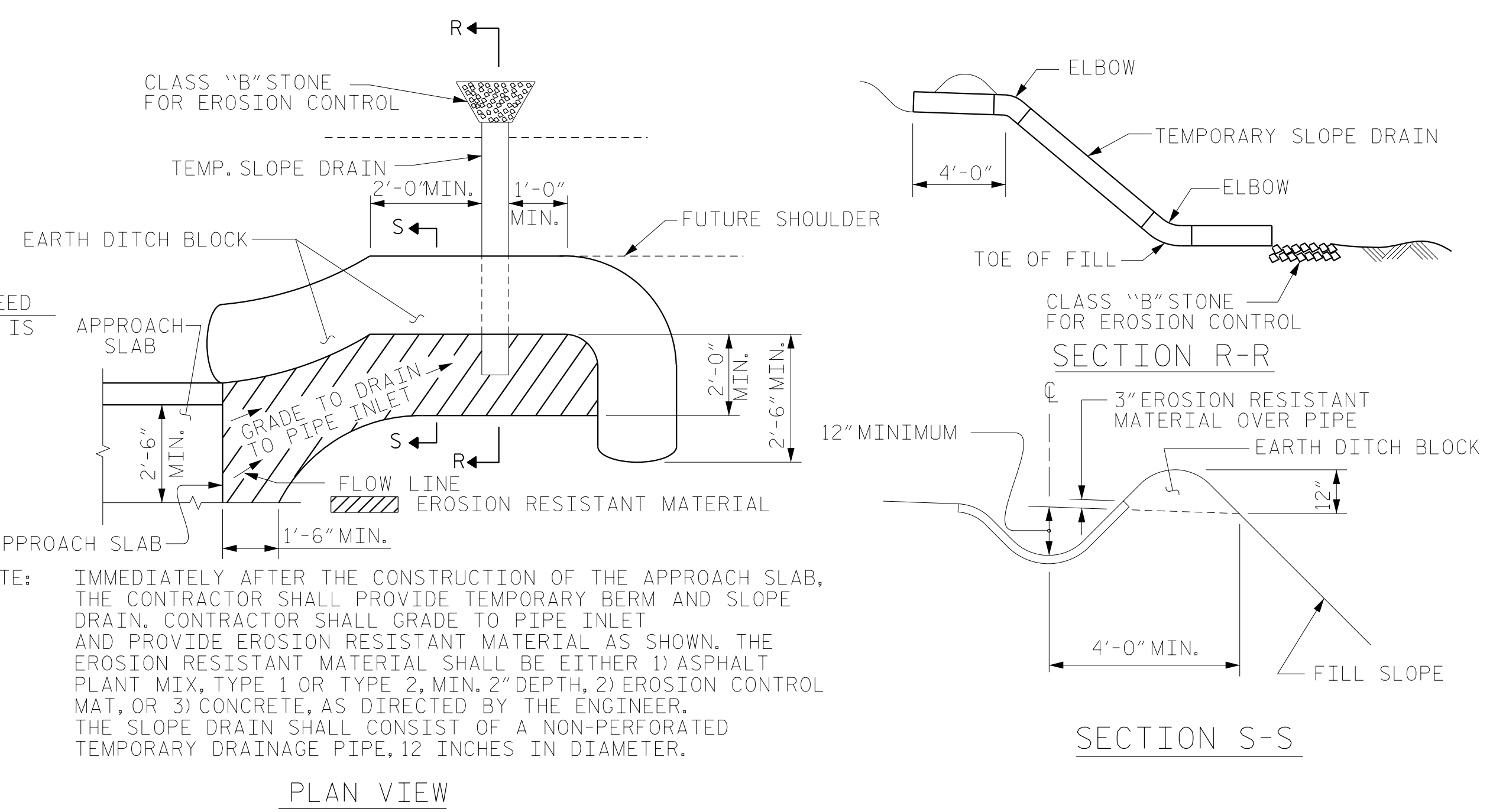


SECTION THRU SLAB

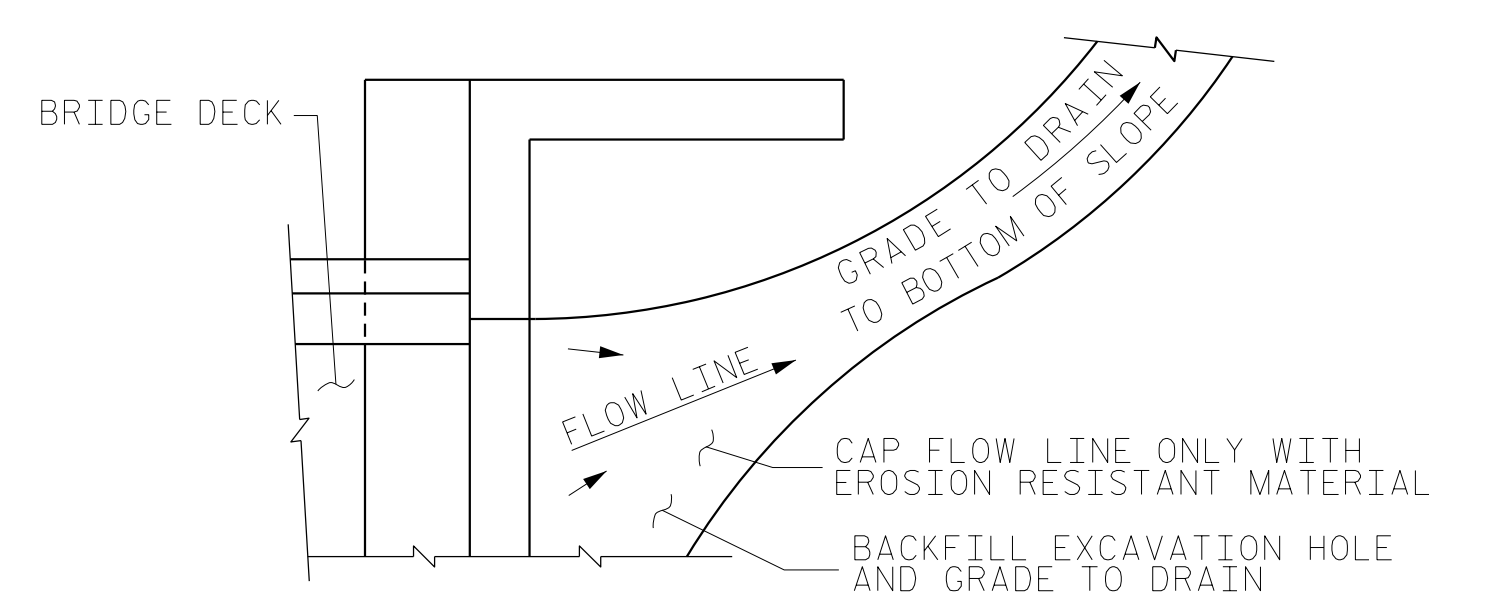
END BENT 2 SHOWN.

ELASTOMERIC CONCRETE	
END BENT NO.	ELASTOMERIC CONCRETE * (CU, FT.)
1	7.4
2	8.3
TOTAL	15.7

* BASED ON THE MINIMUM BLOCKOUT SHOWN.



SECTION S-S



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 29+07.16 -Y2-

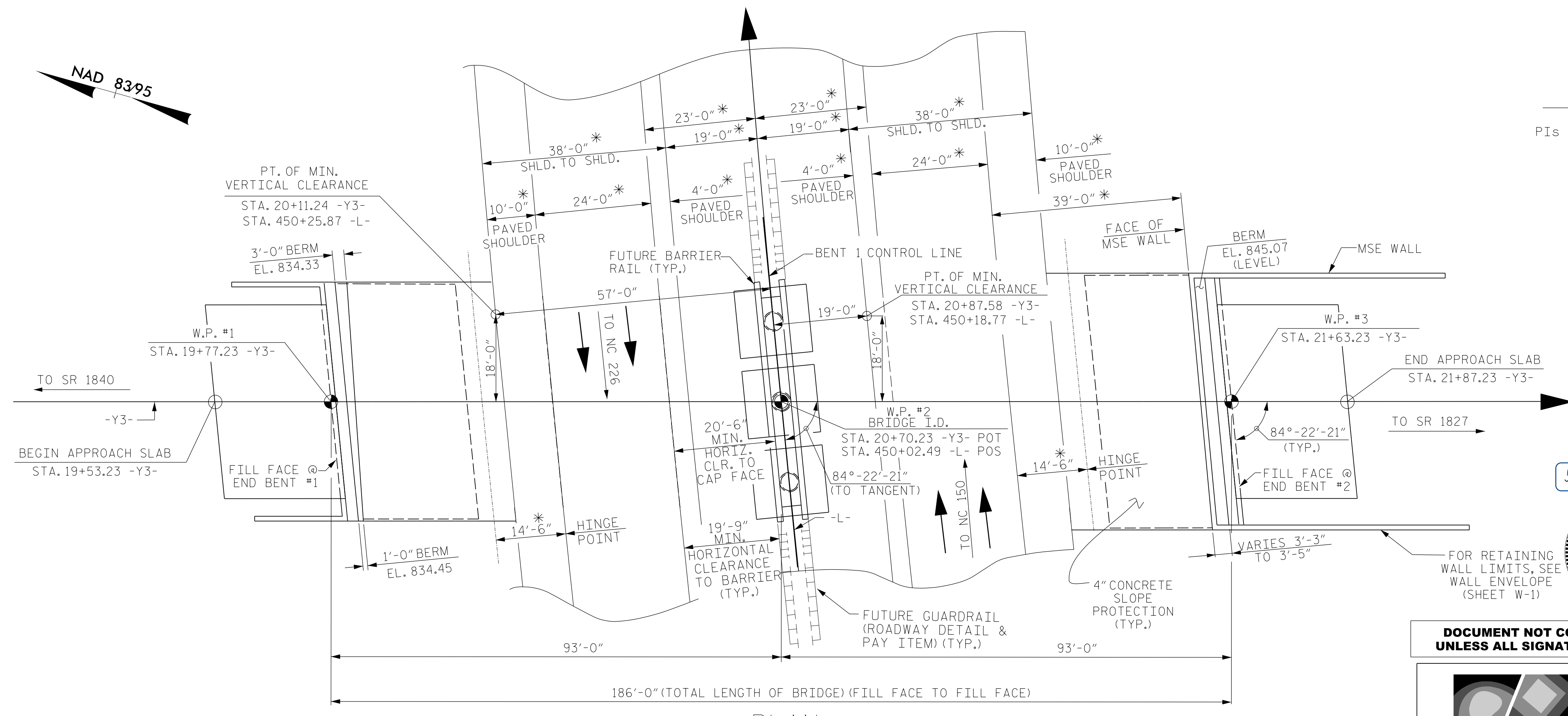
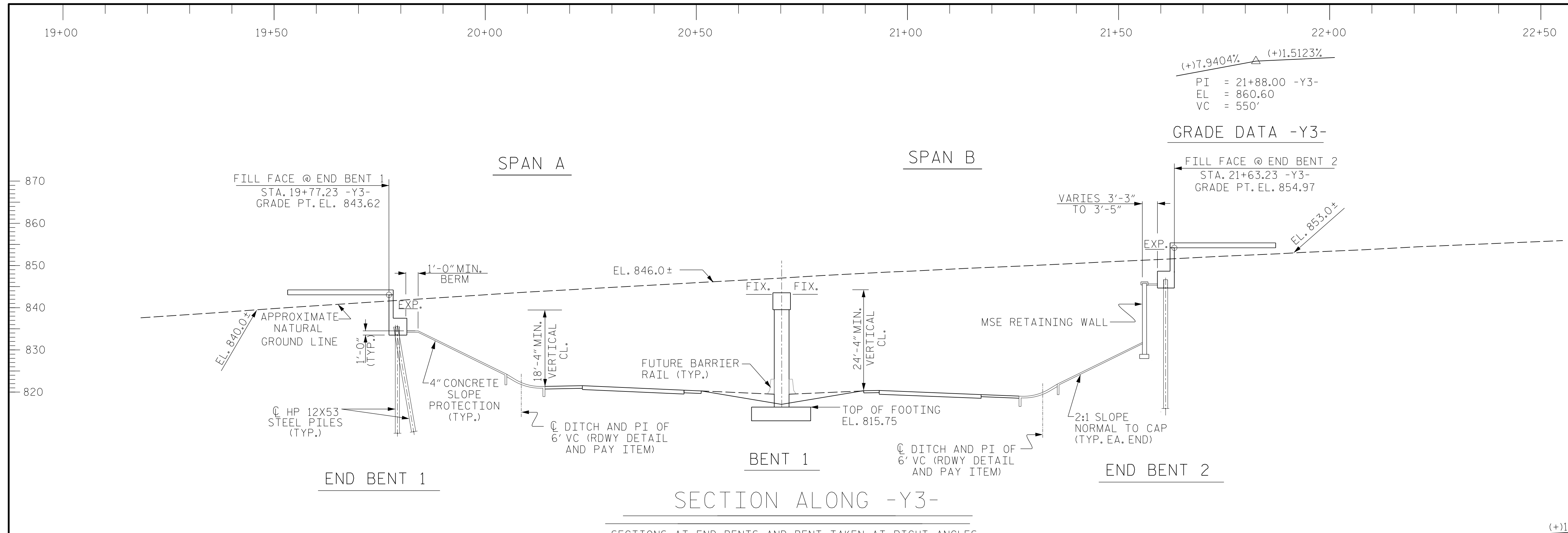
SHEET 2 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
APPROACH SLAB
DETAILS

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S3-36
1			3			TOTAL SHEETS
2			4			36

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ASSEMBLED BY :	MKO	DATE :	12/2016
CHECKED BY :	JMR	DATE :	12/2016
DRAWN BY :	FCJ	11/88	REV. 10/1/11 MAA/GM
CHECKED BY :	ARB	11/88	REV. 7/12 MAA/GM
			REV. 6/13 MAA/GM

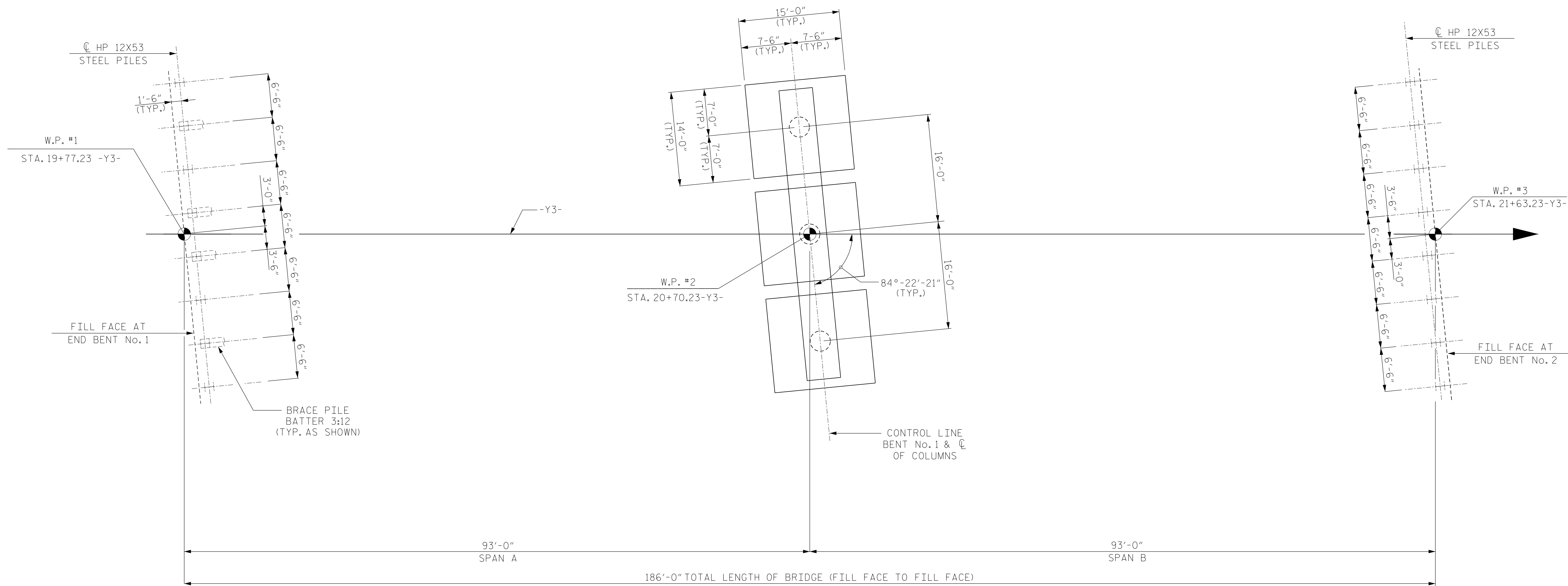


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STEWART
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421 Fayetteville St,
Suite 400
Raleigh, NC 27601
T 919.380.8750
www.stewartinc.com

3/24/2017
R_2707C-SITE 3
404_001_R2707C-SMU_GD1_S4-1.dgn
USER:jeffloftus

DRAWN BY: H.ASSFOURA DATE: 08/16
CHECKED BY: J.LOFTUS DATE: 11/16
DESIGN ENGINEER OF RECORD: J.LOFTUS DATE: 01/17



FOUNDATION LAYOUT

ALL BENTS ARE PARALLEL
 DIMENSIONS LOCATING PILES ARE SHOWN
 TO PILE CENTERLINE AT BOTTOM OF CAP

FOUNDATION NOTES

- FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.
- PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.
- DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.
- PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 100 TONS PER PILE.
- DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 170 TONS PER PILE.
- THE SPREAD FOOTINGS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 6 TSF. CHECK FIELD CONDITIONS FOR THE REQUIRED RESISTANCE OF 13.5 TSF JUST BEFORE PLACING CONCRETE.

SPECIAL NOTE

- FOR BENT 1 (LT), UNDERCUT TO ELEVATION 808 SHOULD BE ANTICIPATED. ENGINEER SHALL VERIFY FOOTING ARE BEARING ON WEATHERED ROCK OR ROCK. ANY UNDERCUT SHALL BE BACKFILLED WITH A MINIMUM CLASS B CONCRETE.

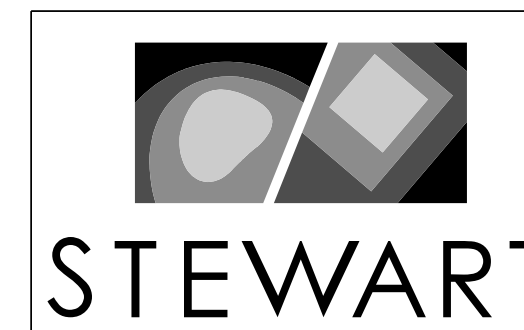
PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT

SHEET 2 OF 4



DocuSigned by:
Jeff Loftus
 2/6/2017

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 421 Fayetteville St,
 Suite 400
 Raleigh, NC 27601
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 www.stewartinc.com



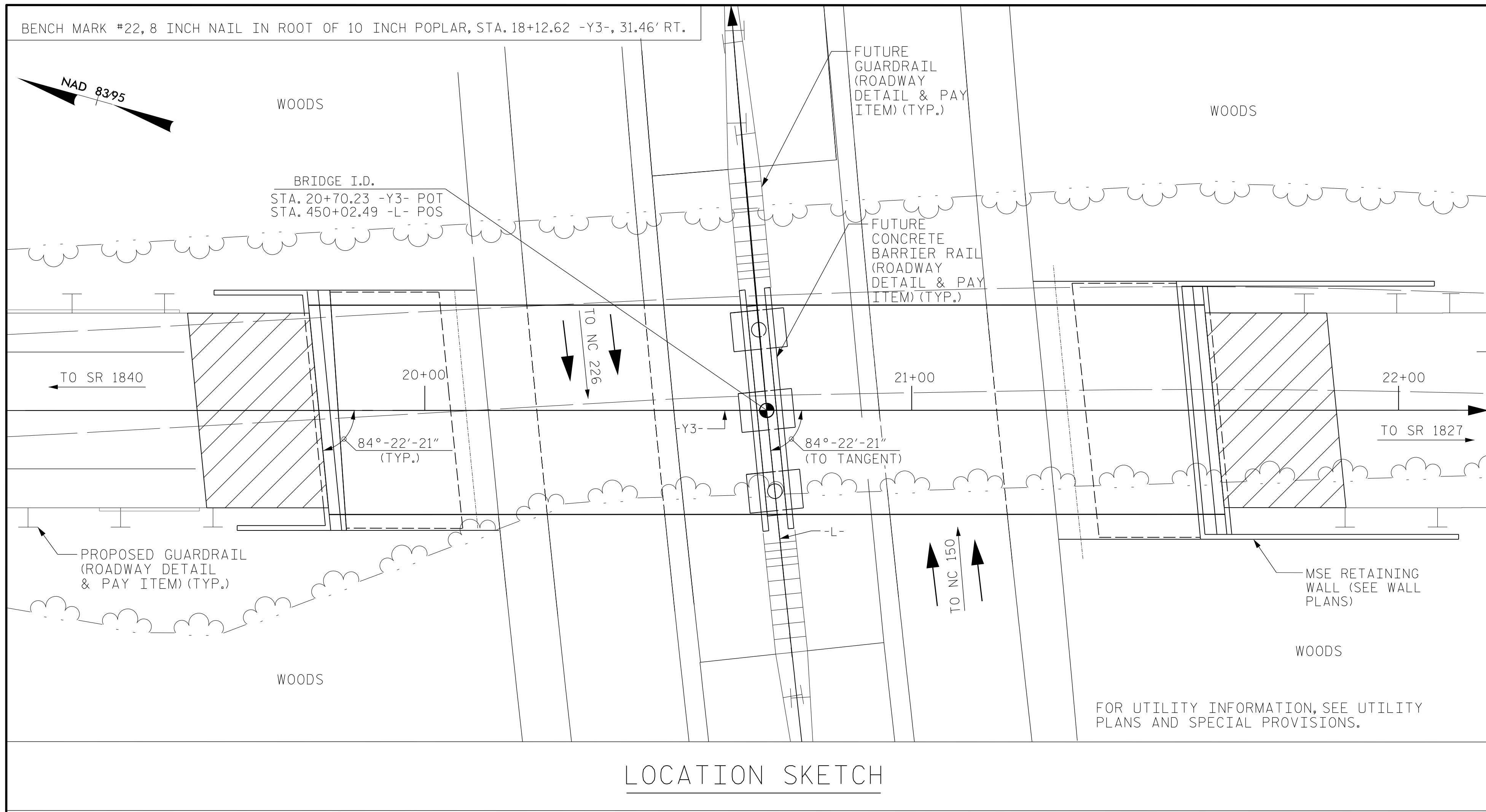
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
FOUNDATION LAYOUT					
SHEET NO. S4-2					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS 29

DRAWN BY: <u>H.ASSFOURA</u>	DATE: <u>08/16</u>
CHECKED BY: <u>J.LOFTUS</u>	DATE: <u>11/16</u>
DESIGN ENGINEER OF RECORD: <u>J.LOFTUS</u>	DATE: <u>01/17</u>

STR. #4

R 2707C-SITE 3

2/6/2017
 ...\\404_003_R2707C_SMU_FL2_S4-2.dgn
 USERdefault



1. ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING
2. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS
3. THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
4. THE ELEVATION(S) AND CLEARANCE(S) SHOWN ON THE PLANS AT THE POINT(S) OF MINIMUM VERTICAL CLEARANCE ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE ELEVATION(S) ON THE EXISTING PAVEMENT AND CHECK THE CLEARANCE. REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.
5. REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
6. FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
7. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
8. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
9. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
10. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
11. FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS
12. FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
13. NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
14. THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE FOAM JOINT SEAL SHALL BE 2" AT END BENTS 1 & 2.
15. FOR FOAM JOINT SEALS, SEE SPECIAL PROVISIONS.
16. FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.
17. FOR FOUNDATION NOTES, SEE SHEET S4-2.
18. THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

LOCATION SKETCH

TOTAL BILL OF MATERIAL																
	FOUNDATION EXCAVATION FOR BENT 1	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOOR	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	54" PRESTRESSED CONCRETE GIRDERS	PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES	HP 12X53 STEEL PILES	CONCRETE BARRIER RAIL	4" SLOPE PROTECTION	ELASTOMERIC BEARINGS	FOAM JOINT SEAL		
	LUMP SUM	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS	LBS	NO. LIN. FT.	EACH	No. LIN. FT.	LIN. FT.	SQ. YDS.	LUMP SUM	LUMP SUM		
SUPERSTRUCTURE		7,954	8,558		LUMP SUM			8 729'-0"			367.81		LUMP SUM	LUMP SUM		
END BENT No. 1				43.8		6,720			8	8 200		168				
BENT No. 1	LUMP SUM			131.9		24,444	1,599									
END BENT No. 2				35.1		5,138			8	8 380		175				
TOTAL	LUMP SUM	7,954	8,558	210.8	LUMP SUM	36,302	1,599	8 729'-0"	16	16 580	367.81	343	LUMP SUM	LUMP SUM		

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT



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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

GENERAL DRAWING
 FOR BRIDGE OVER
 US 74/SHELBY BYPASS
 ON SR 1005 (N LAFAYETTE ST.)
 BETWEEN SR 1840 AND SR 1827

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-3
1			3			TOTAL SHEETS
2			4			29

DRAWN BY: H.ASSFOURA DATE: 08/16
 CHECKED BY: J.LOFTUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: J.LOFTUS DATE: 01/17

R-2707C-SITE 3
 3/28/2017
 \\404_005_R2707C_SMU_LS3_S4-3.dgn
 USER:default

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					COMMENT NUMBER			
						LIVE-LOAD FACTORS (γ _{LL})	MOMENT					SHEAR					LIVE-LOAD FACTORS (γ _{LL})	MOMENT						
							DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FH)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FH)		DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FH)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	①	1.36	--	1.75	0.950	1.99	A	E	44.8	1.118	1.46	B	I	8.40	0.80	0.874	1.36	A	I	44.8		
	HL-93 (OPERATING)	N/A		1.89	--	1.35	0.950	2.54	A	E	44.8	1.118	1.89	B	I	8.40	N/A	--	--	--	--	--		
	HS-20 (INVENTORY)	36.000	②	1.84	66.24	1.75	0.950	2.64	A	E	44.8	1.118	1.97	B	I	35.7	0.80	0.874	1.84	A	I	44.8		
	HS-20 (OPERATING)	36.000		2.50	90.00	1.35	0.950	3.43	A	E	44.8	1.118	2.50	B	I	35.7	N/A	--	--	--	--	--		
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH		3.39	45.76	1.40	0.950	7.41	A	E	44.8	1.118	5.04	A	I	54.0	0.80	0.874	3.39	B	I	44.8		
		SNGARBS2	20.000		2.49	49.80	1.40	0.950	5.39	B	E	54.0	1.118	3.78	A	I	54.0	0.80	0.874	2.49	A	I	44.8	
		SNAGRIS2	22.000		2.33	51.26	1.40	0.950	5.06	B	E	54.0	1.118	3.56	A	I	54.0	0.80	0.874	2.33	A	I	44.8	
		SNCOTTS3	27.250		1.69	46.05	1.40	0.950	3.66	A	E	44.8	1.118	2.68	A	I	54.0	0.80	0.874	1.69	A	I	44.8	
		SNAGGRS4	34.925		1.40	48.89	1.40	0.950	3.04	B	E	54.0	1.118	2.21	A	I	54.0	0.80	0.874	1.40	A	I	44.8	
		SNS5A	35.550		1.38	49.05	1.40	0.950	2.98	A	E	35.7	1.118	2.26	A	I	54.0	0.80	0.874	1.38	A	I	44.8	
		SNS6A	39.950		1.25	49.93	1.40	0.950	2.72	B	E	54.0	1.118	2.09	A	I	54.0	0.80	0.874	1.25	A	I	44.8	
	SNS7B	42.000		1.19	49.98	1.40	0.950	2.59	A	E	44.8	1.118	2.08	A	I	54.0	0.80	0.874	1.19	A	I	44.8		
	TRUCK TRACTOR SEMI-TRAILER (TST)	TNAGRIT3	33.000		1.54	50.82	1.40	0.950	3.34	B	E	54.0	1.118	2.40	B	I	35.7	0.80	0.874	1.54	A	I	44.8	
		TNT4A	33.075		1.53	50.60	1.40	0.950	3.31	B	E	54.0	1.118	2.57	A	I	54.0	0.80	0.874	1.53	A	I	44.8	
		TNT6A	41.600		1.25	52.00	1.40	0.950	2.71	A	E	44.8	1.118	2.18	A	I	81.3	0.80	0.874	1.25	A	I	44.8	
		TNT7A	42.000		1.25	52.50	1.40	0.950	2.71	A	E	35.7	1.118	2.14	A	I	54.0	0.80	0.874	1.25	A	I	44.8	
		TNT7B	42.000		1.28	53.76	1.40	0.950	2.76	B	E	54.0	1.118	2.02	A	I	54.0	0.80	0.874	1.28	A	I	44.8	
		TNAGRIT4	43.000		1.23	52.89	1.40	0.950	2.66	B	E	54.0	1.118	2.02	B	I	35.7	0.80	0.874	1.23	A	I	44.8	
TNAGT5A		45.000		1.16	52.20	1.40	0.950	2.53	B	E	54.0	1.118	1.97	A	I	81.3	0.80	0.874	1.16	A	I	44.8		
TNAGT5B	45.000		③	1.15	51.75	1.40	0.950	2.49	A	E	44.8	1.118	1.87	A	I	54.0	0.80	0.874	1.15	A	I	44.8		

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ _{DC}	γ _{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

⊕ CONTROLLING LOAD RATING

① DESIGN LOAD RATING (HL-93)

② DESIGN LOAD RATING (HS-20)

③ LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

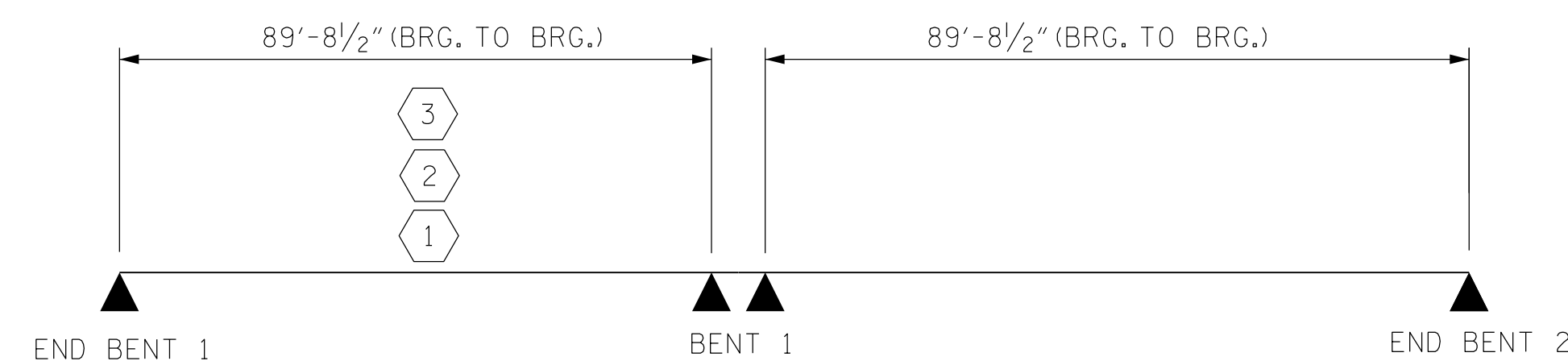
GIRDER LOCATION

I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER - EXTERIOR RIGHT GIRDER

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.



LRFR SUMMARY

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT

SHEET 4 OF 4



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 2/6/2017

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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 LRFR SUMMARY FOR
 PRESTRESSED
 CONCRETE GIRDERS
 (NON-INTERSTATE TRAFFIC)

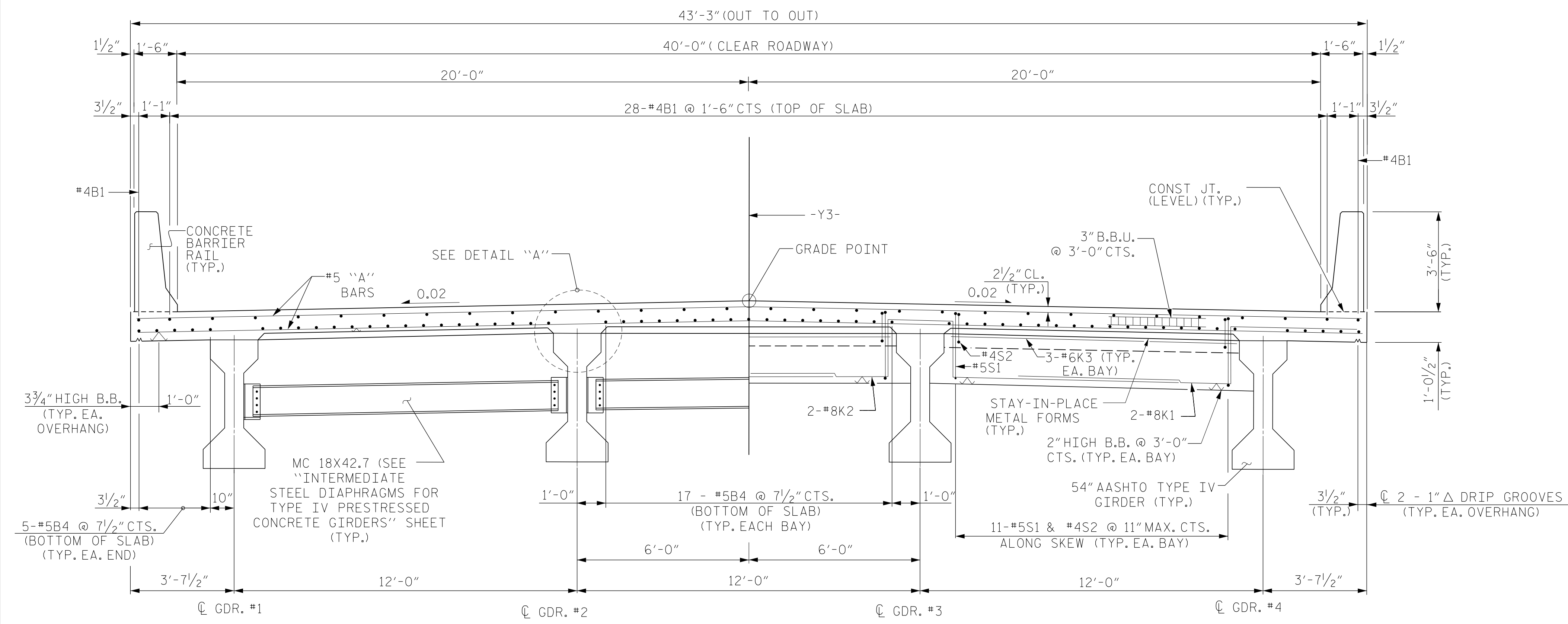
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-4
1			3			TOTAL SHEETS
2			4			29

STR.#4

DRAWN BY: H.ASSFOURA DATE: 08/16
 CHECKED BY: J.LOFIUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: J.LOFIUS DATE: 01/17

2/6/2017
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 USERdefault

R 2707C-SITE 3

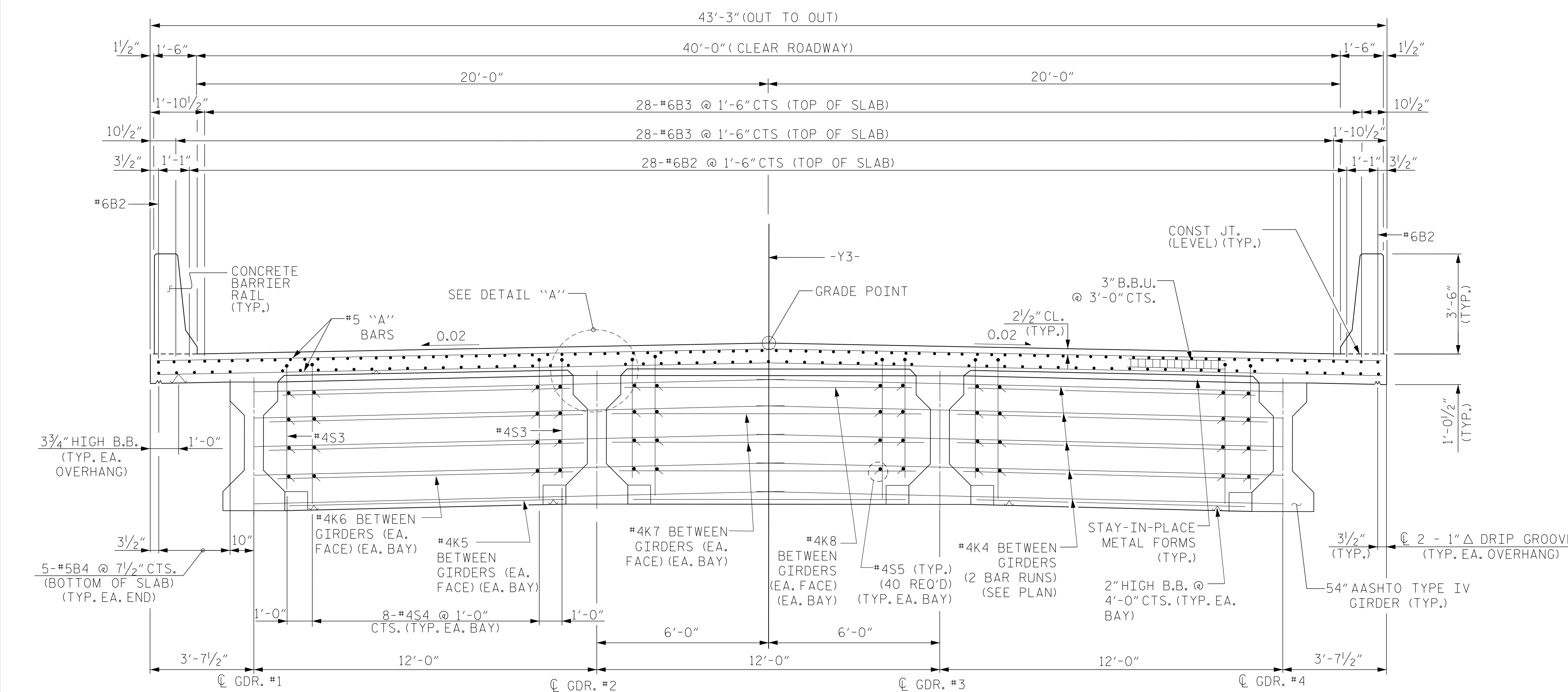


TYPICAL SECTION

SHOWING INTERMEDIATE DIAPHRAGM

TYPICAL SECTION

SHOWING END-BENT DIAPHRAGMS

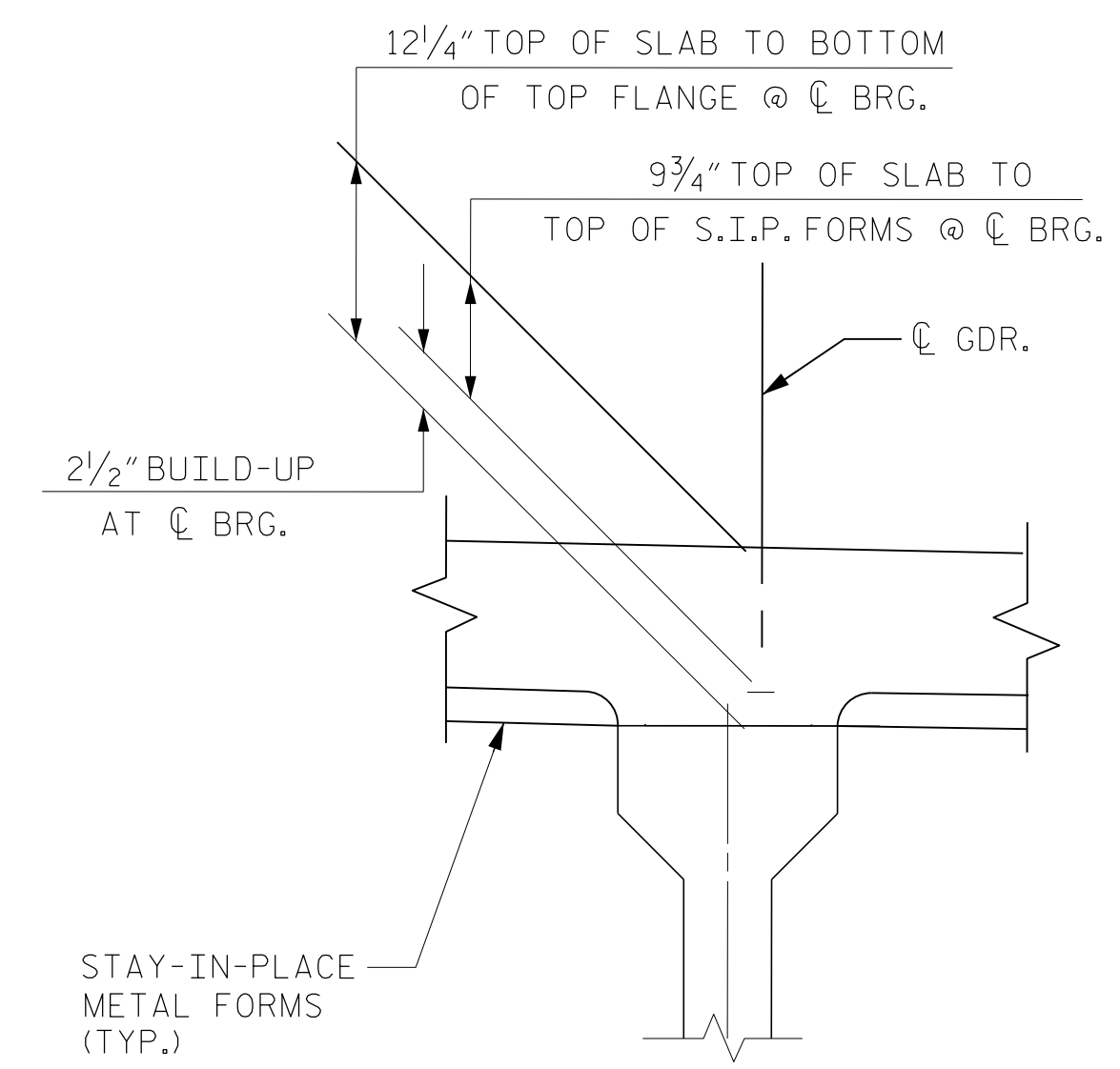


TYPICAL SECTION

SHOWING INTERIOR BENT DIAPHRAGMS

NOTES:

1. PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF 'A' BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF 'A' BARS A CLEAR DISTANCE OF 2/2" ABOVE THE TOP OF THE REMOVABLE FORM.
2. LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.
3. PREVIOUSLY CAST CONCRETE IN A CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.



DETAIL "A"

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT

SHEET 1 OF 2



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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 TYPICAL SECTION

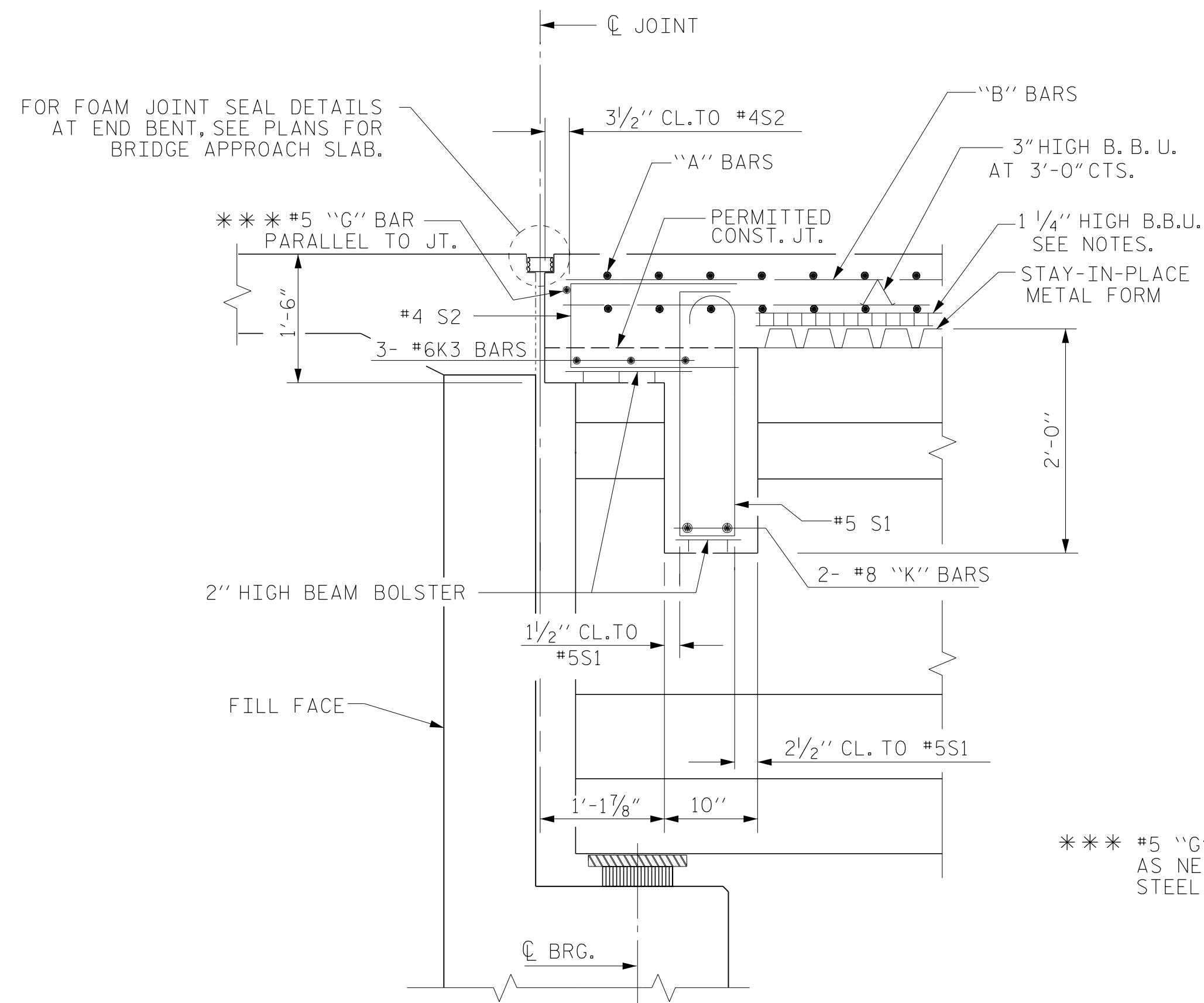
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NO.	BY:	DATE:	NO.	BY:	DATE:	S4-5
1			3			TOTAL SHEETS
2			4			29



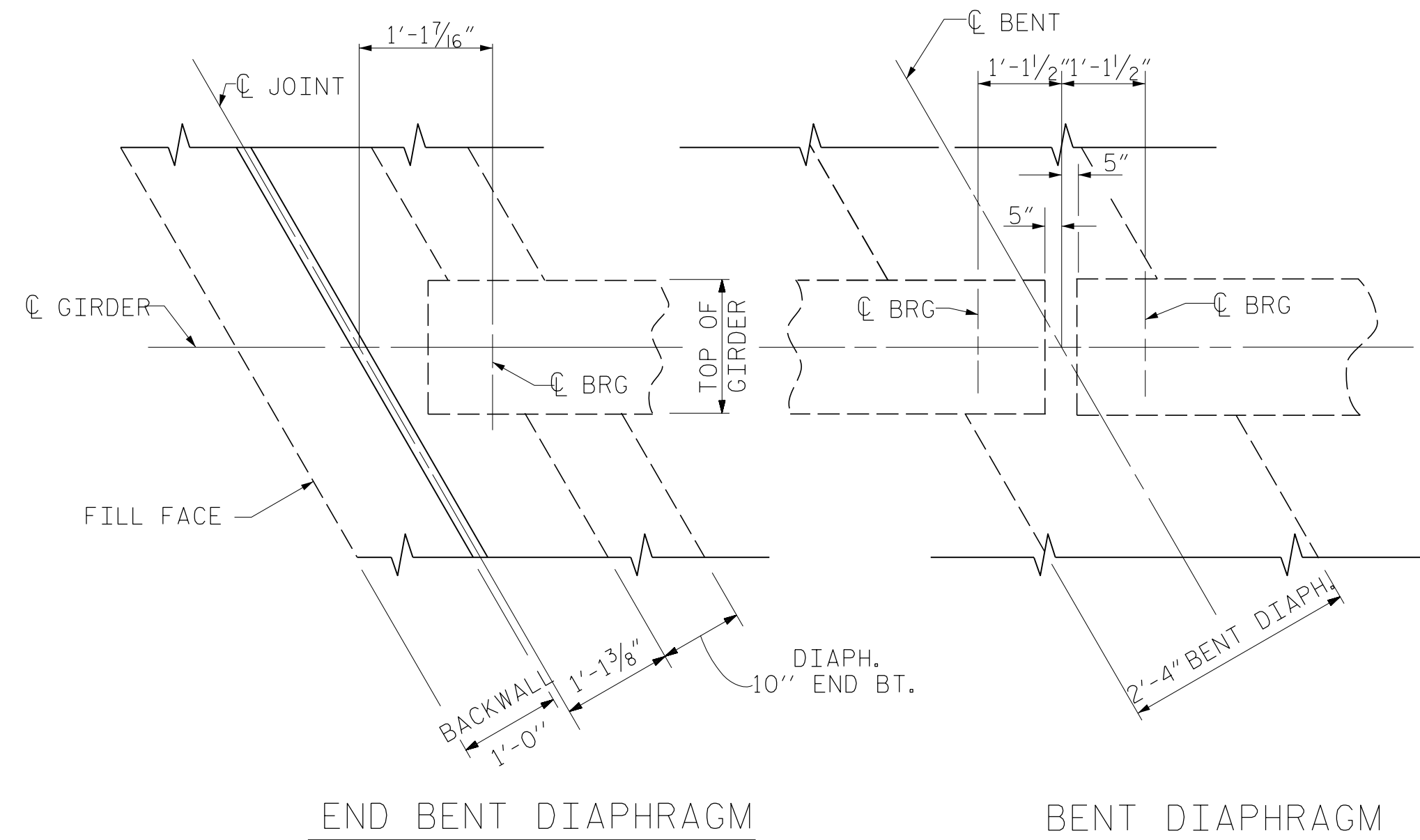
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DRAWN BY: H.ASSFOURA DATE: 07/16
 CHECKED BY: J.L.OFTUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: J.L.OFTUS DATE: 01/17

R 2707C-SITE 3
 2/6/2017
 \\404-009-R2707C-SMU-TS1-S4-5.dgn
 USER:dfault



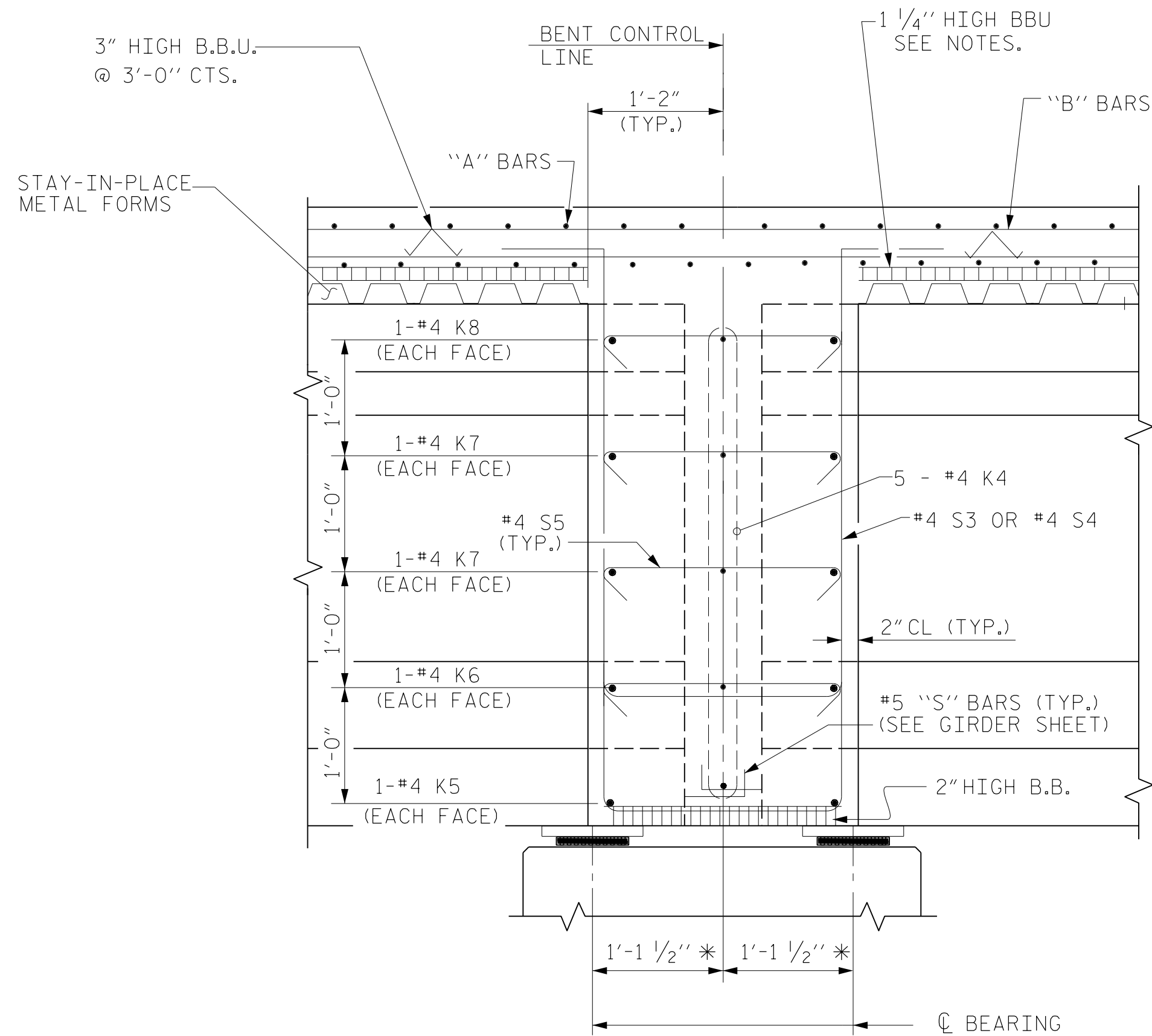
SECTION @ END BENT DIAPHRAGM



END BENT DIAPHRAGM

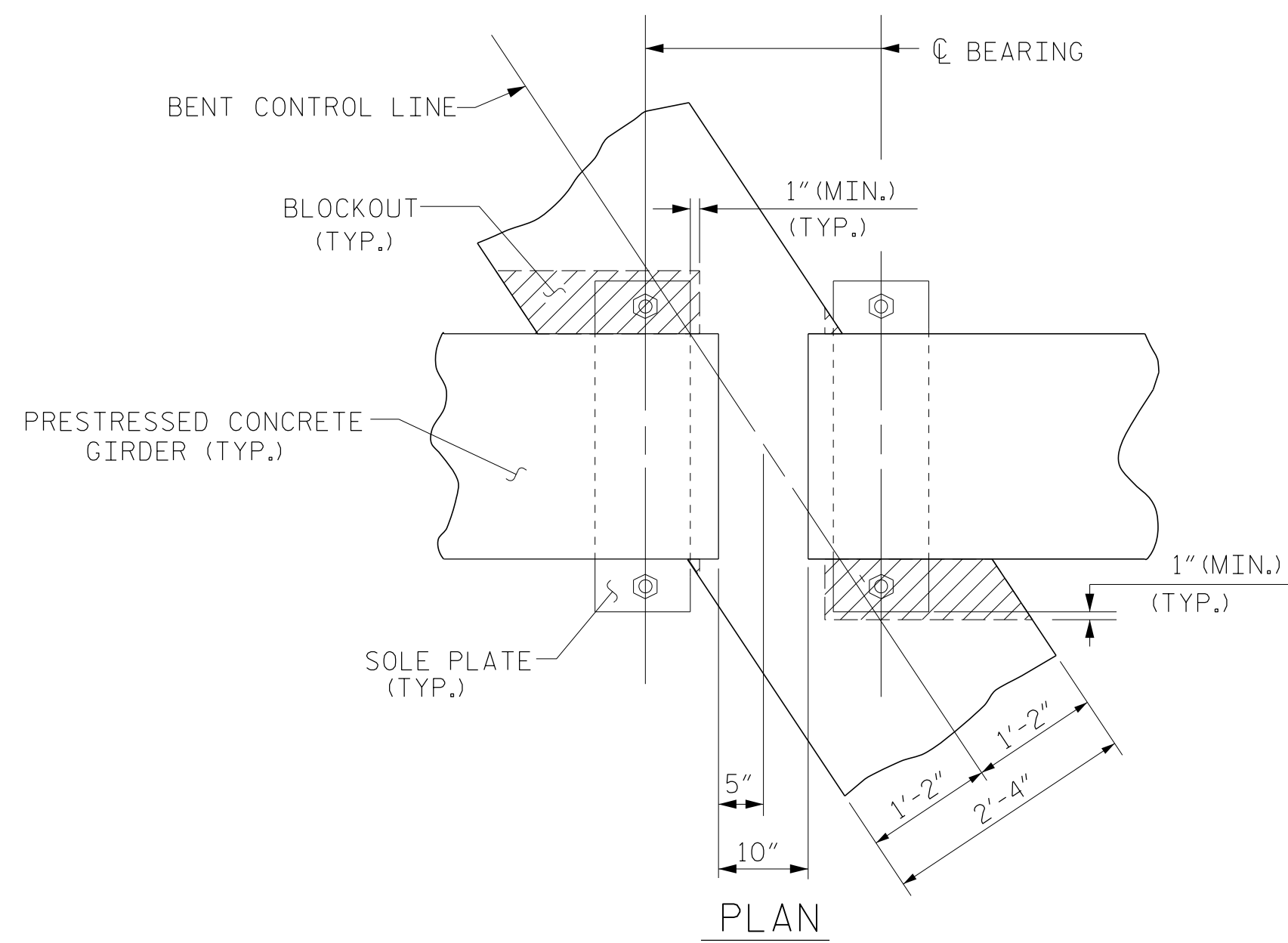
BENT DIAPHRAGM

PLAN

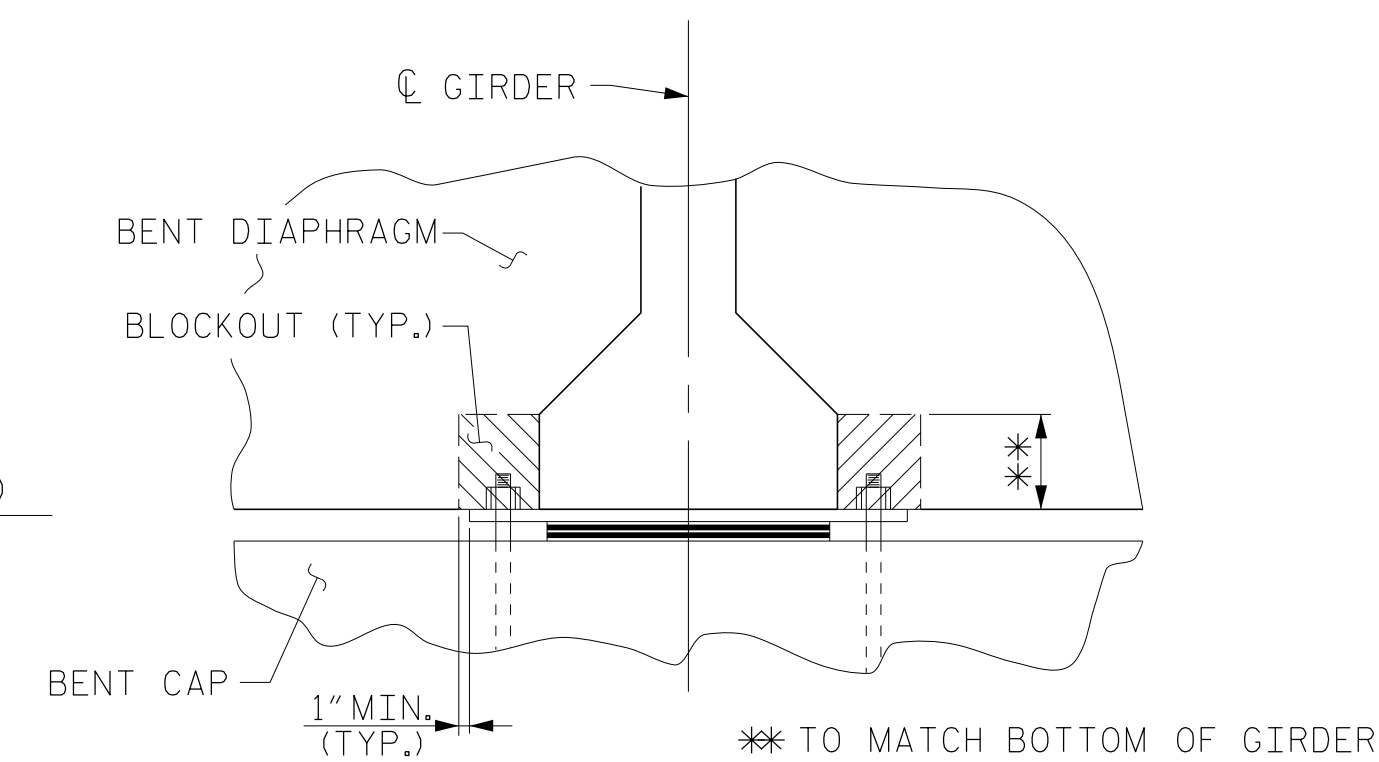


SECTION @ BENT DIAPHRAGM

* MEASURED ALONG GIRDER



BENT DIAPHRAGM BLOCK-OUT DETAIL



SECTION

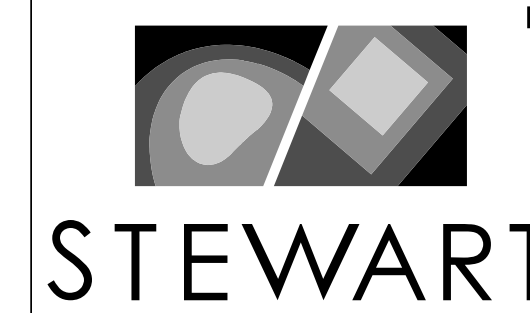
PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT

SHEET 2 OF 2



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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

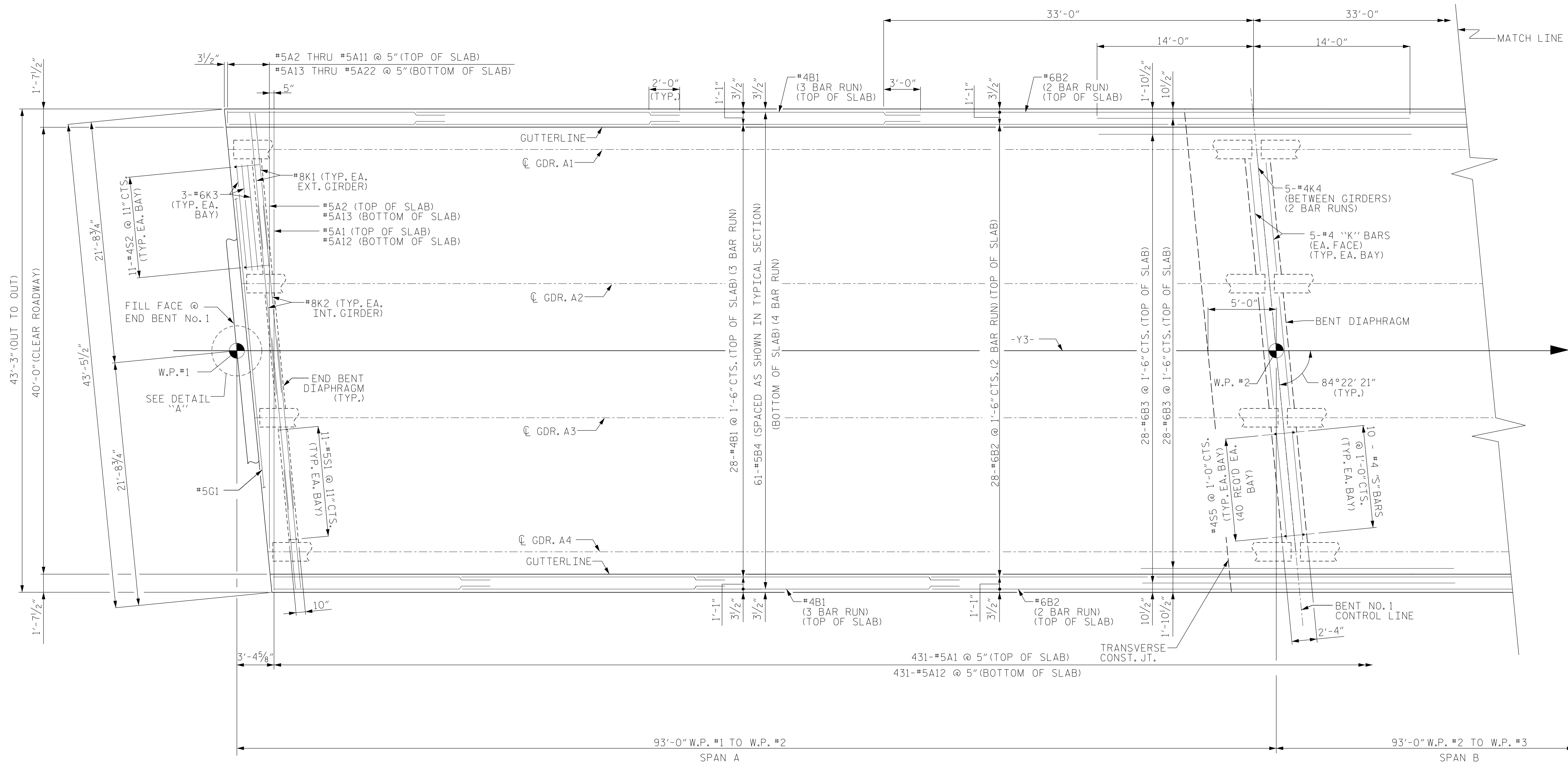
SUPERSTRUCTURE
 TYPICAL SECTION DETAILS

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-6
1			3			TOTAL SHEETS
2			4			29

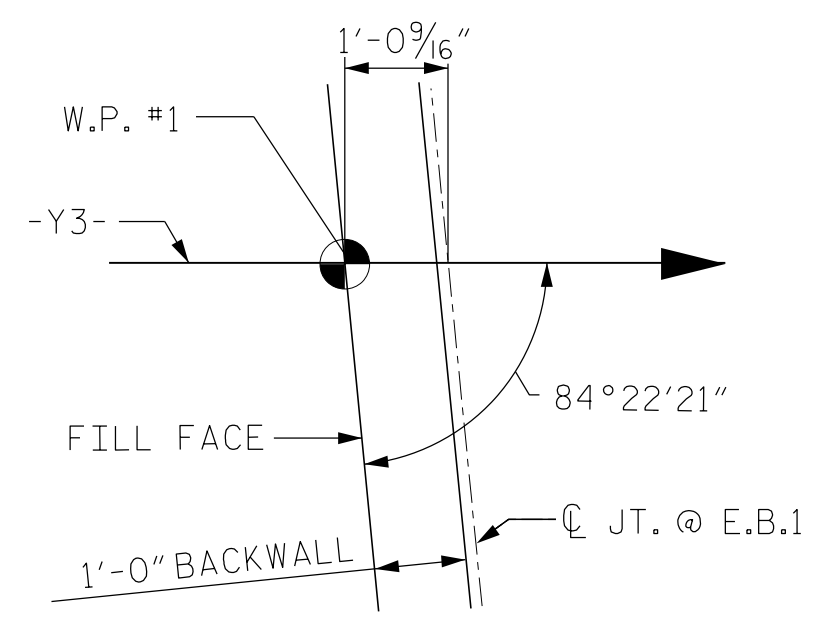
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DRAWN BY: H.ASSFOURA DATE: 07/16
 CHECKED BY: J.LOFTUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: J.LOFTUS DATE: 01/17

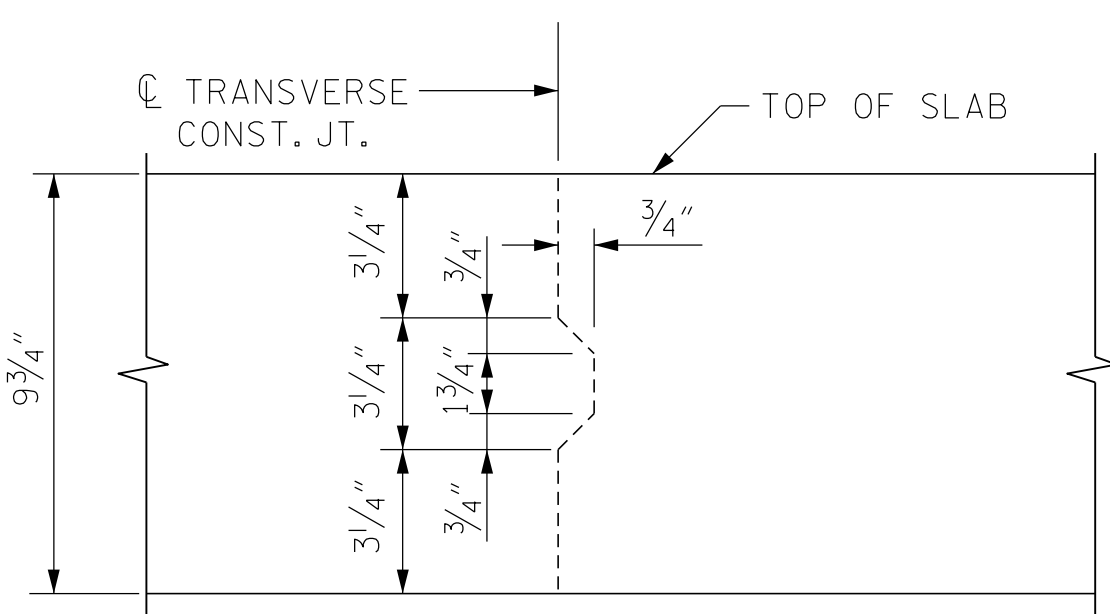
R 2707C-SITE 3
 2/6/2017
 \\404-011-R2707C-SMU-TS2-S4-6.dgn
 USER:jeffloftus



PLAN OF SPAN A & PARTIAL SPAN B



DETAIL "A"



TRANSVERSE CONSTRUCTION JOINT DETAIL

NOTE: REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL STEEL SHALL BE CONTINUOUS THROUGH JOINT.

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT
 SHEET 1 OF 2



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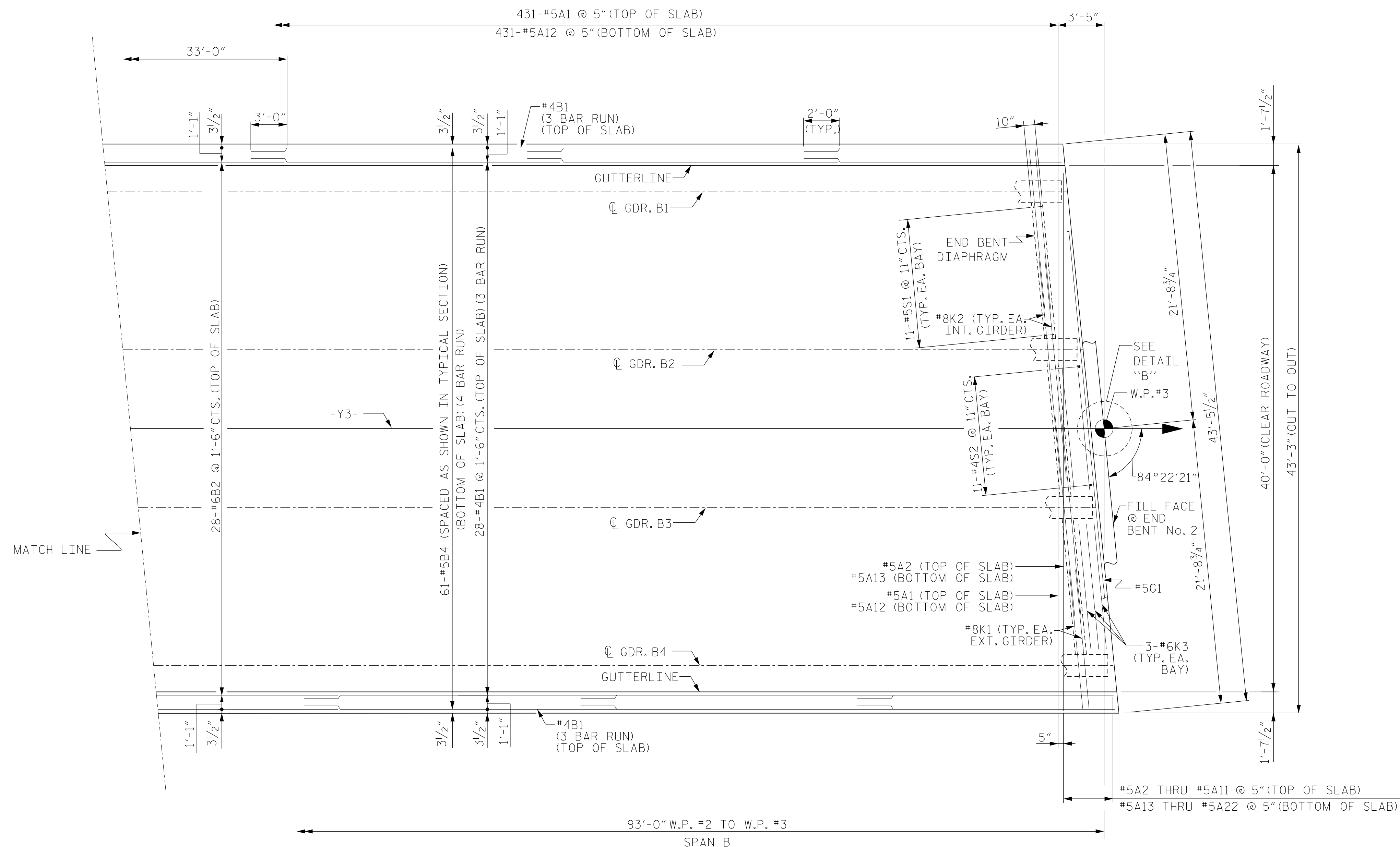
REVISIONS						SHEET NO. S4-7
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 29
2			4			

STR. #4

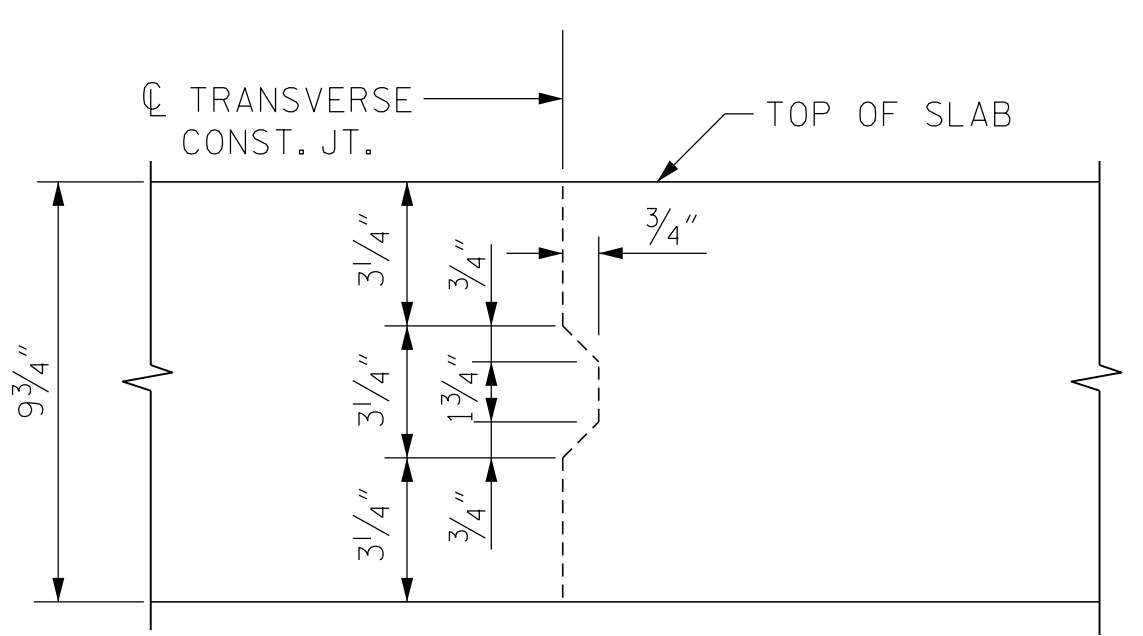
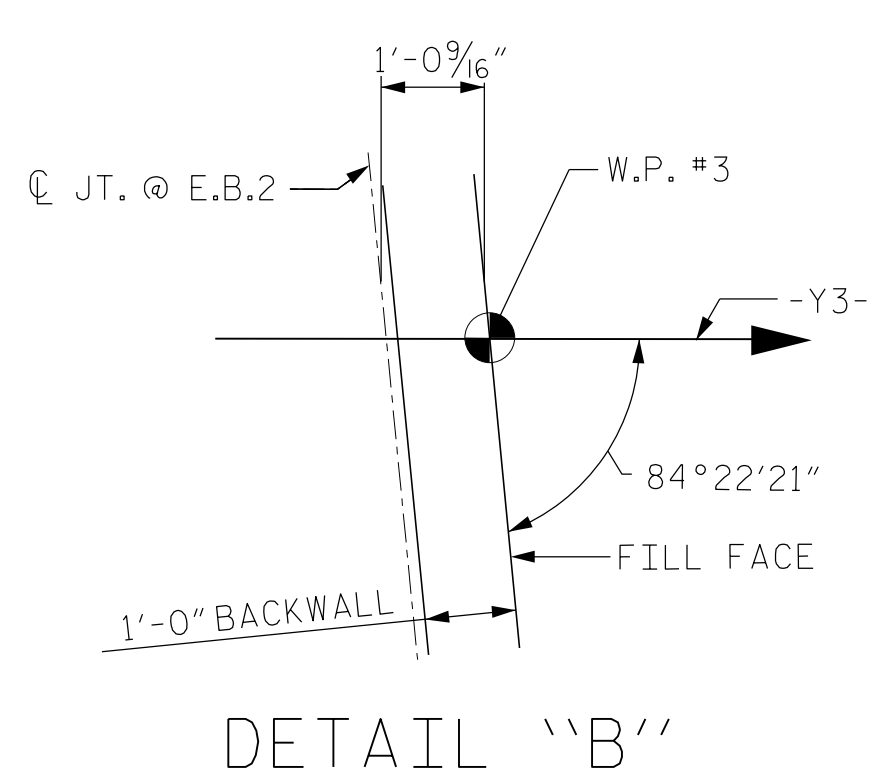
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DRAWN BY: HASSFOURA DATE: 07/16
 CHECKED BY: JLOFTUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: JLOFTUS DATE: 01/17



PARTIAL PLAN OF SPAN B



NOTE: REINFORCING STEEL IN SLAB NOT SHOWN.
LONGITUDINAL STEEL SHALL BE CONTINUOUS THROUGH JOINT.

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 20+70.23-Y3- POT

SHEET 2 OF 2



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NO.	BY:	DATE:	NO.	BY:	DATE:	S4-8
1			3			TOTAL SHEETS
2			4			29

STR. #4

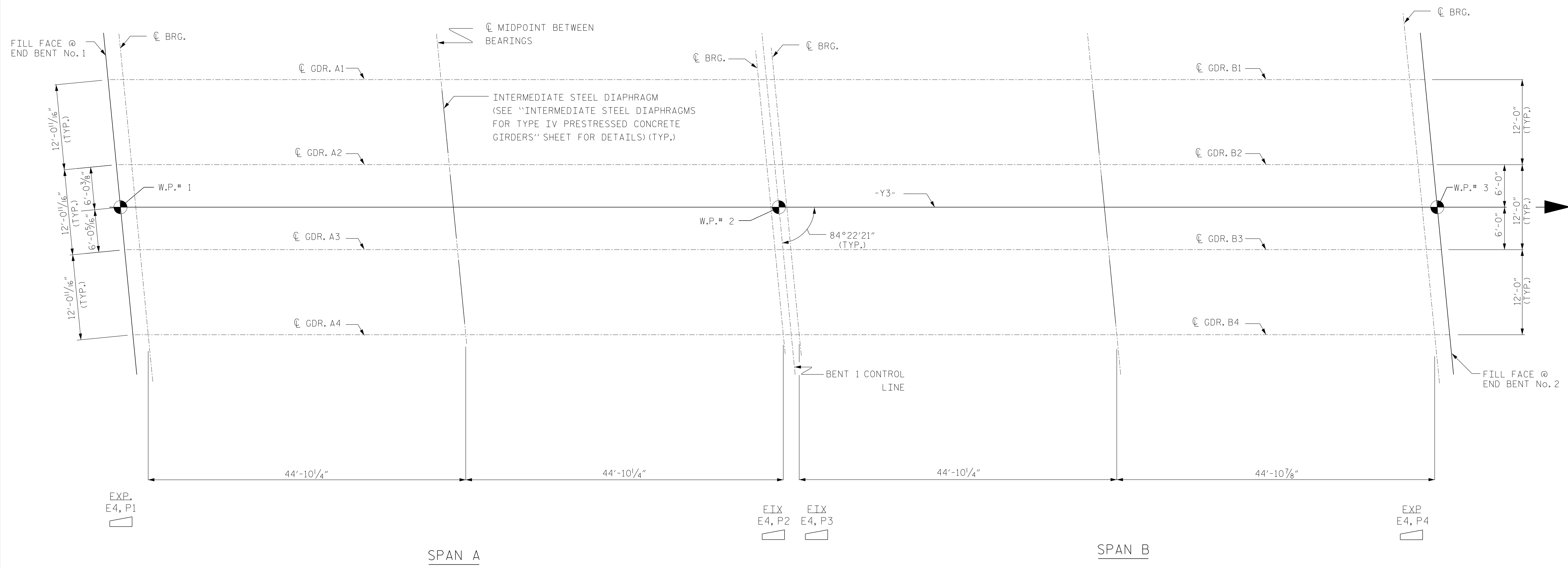
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DESIGN ENGINEER OF RECORD: JLOFTUS DATE: 01/17

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2/6/2017
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R 2707C-SITE 3

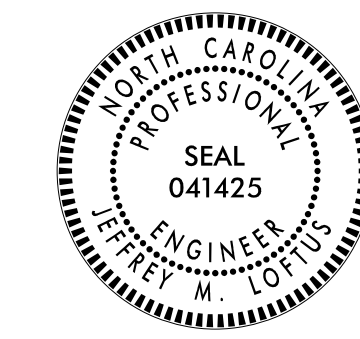
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CHECKED BY: JLOFTUS DATE: 11/16
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FRAMING PLAN

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 20+70.23-Y3- POT



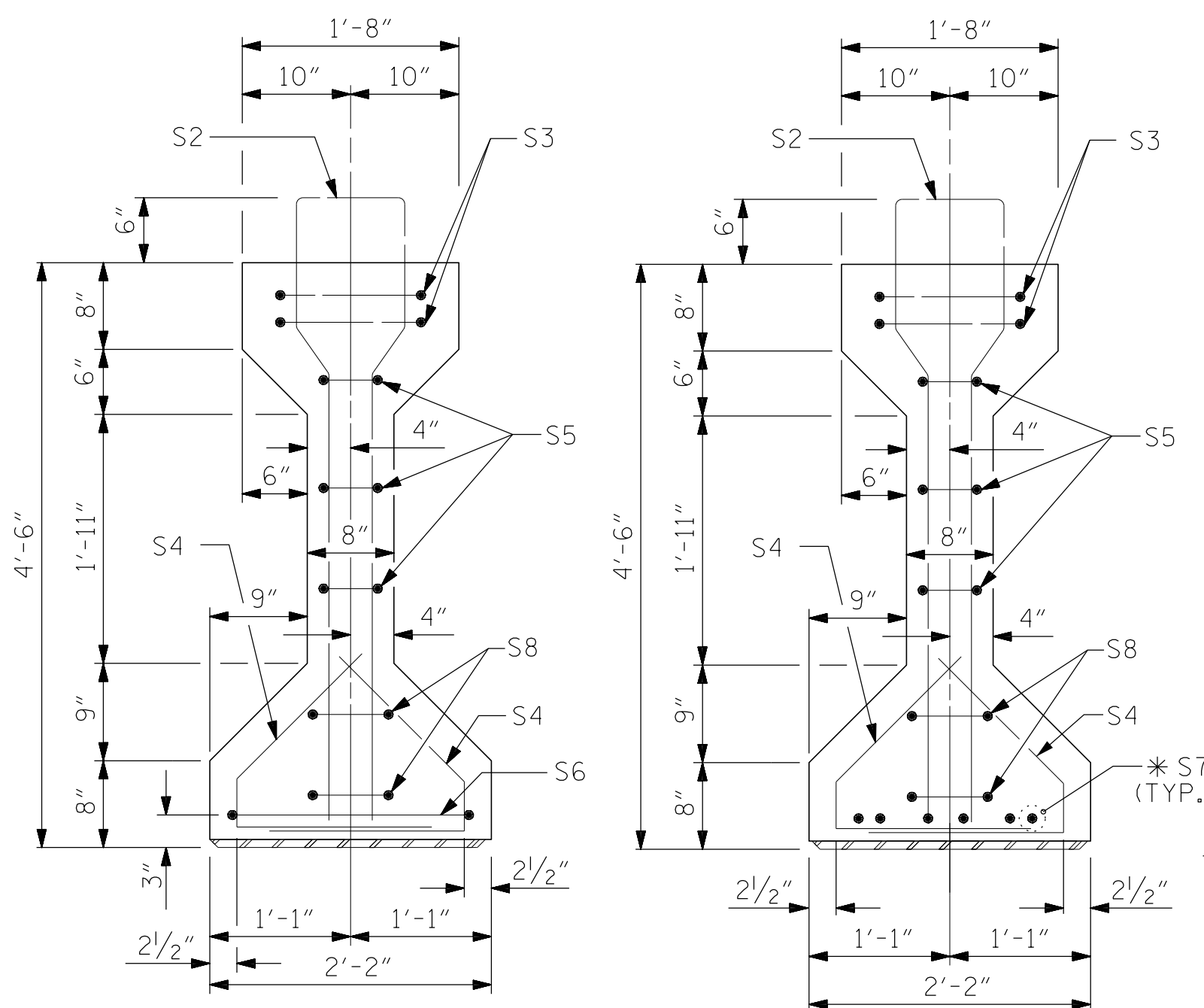
DocuSigned by:
Jeff Loftus 2/6/2017
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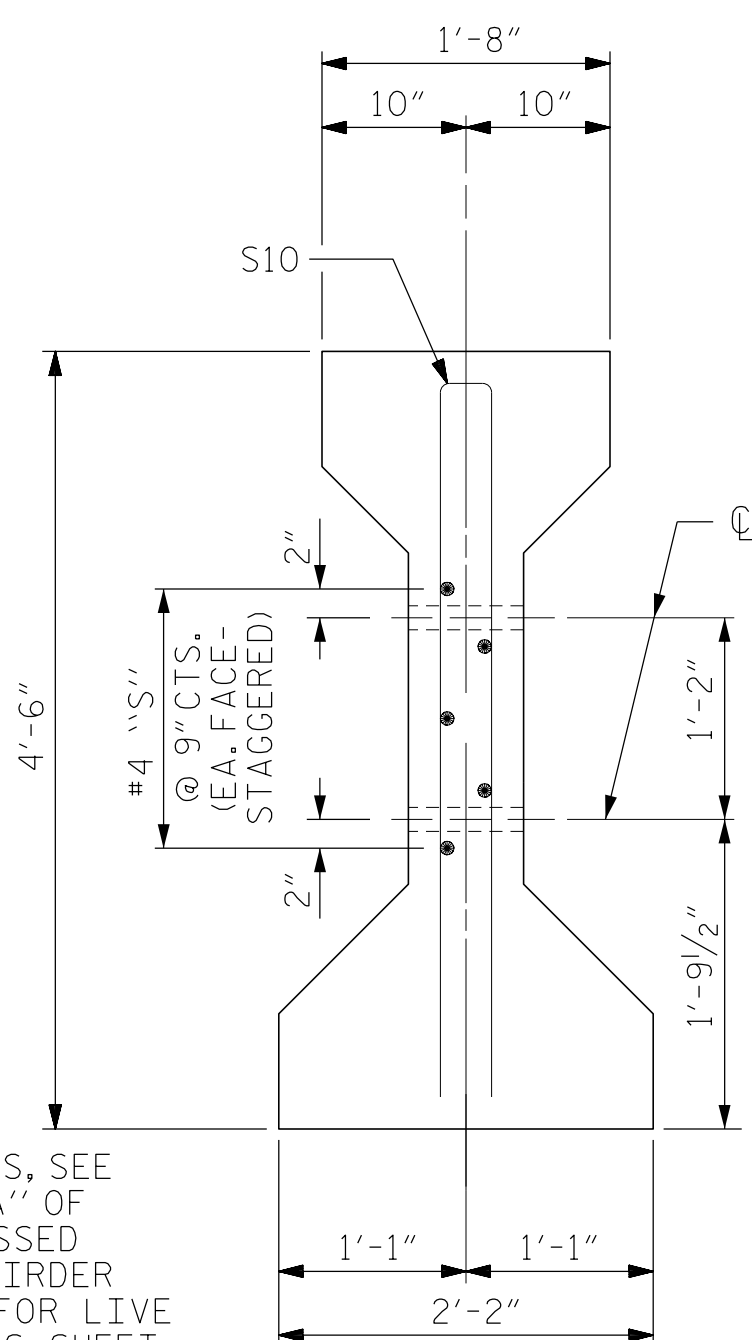
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE FRAMING PLAN					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO. S4-9					TOTAL SHEETS 29

STR. #4

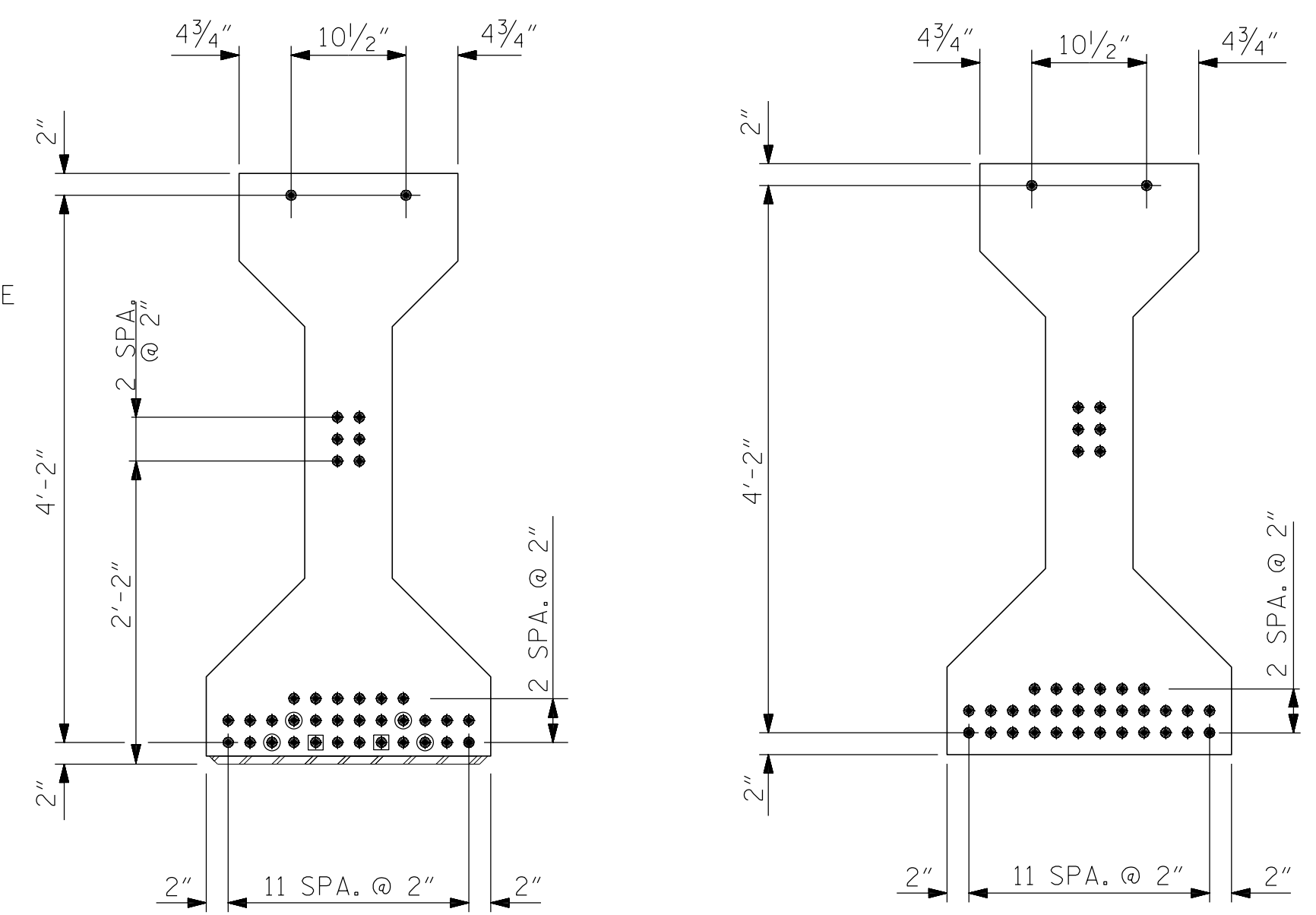


SECTION A-A

SECTION B-B



SECTION C-C
(S1 BARS NOT SHOWN)

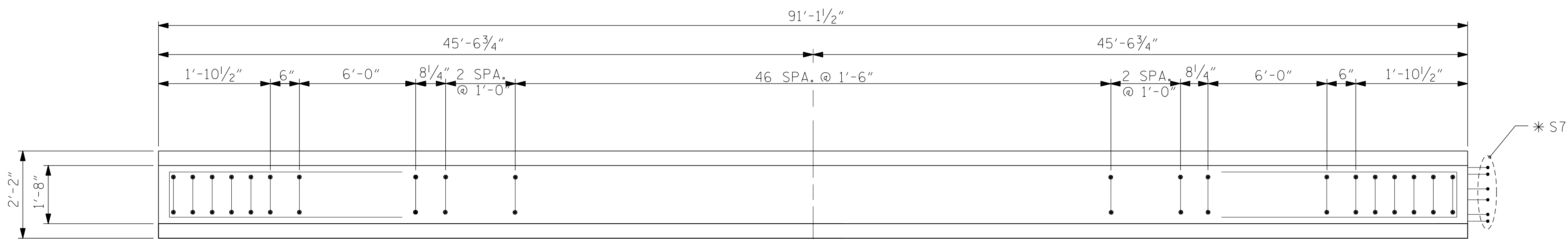


AT END OF GIRDER

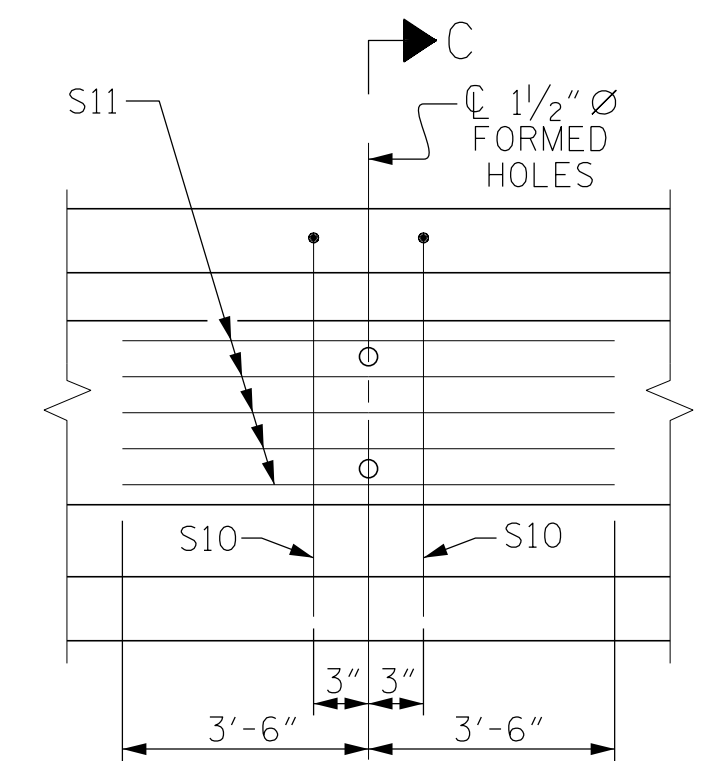
AT C OF GIRDER

0.6" Ø LOW RELAXATION STRAND LAYOUT

- FULLY BONDED STRANDS
- STRANDS DEBONDED FOR 8'-0" FROM END OF GIRDER
- STRANDS DEBONDED FOR 4'-0" FROM END OF GIRDER

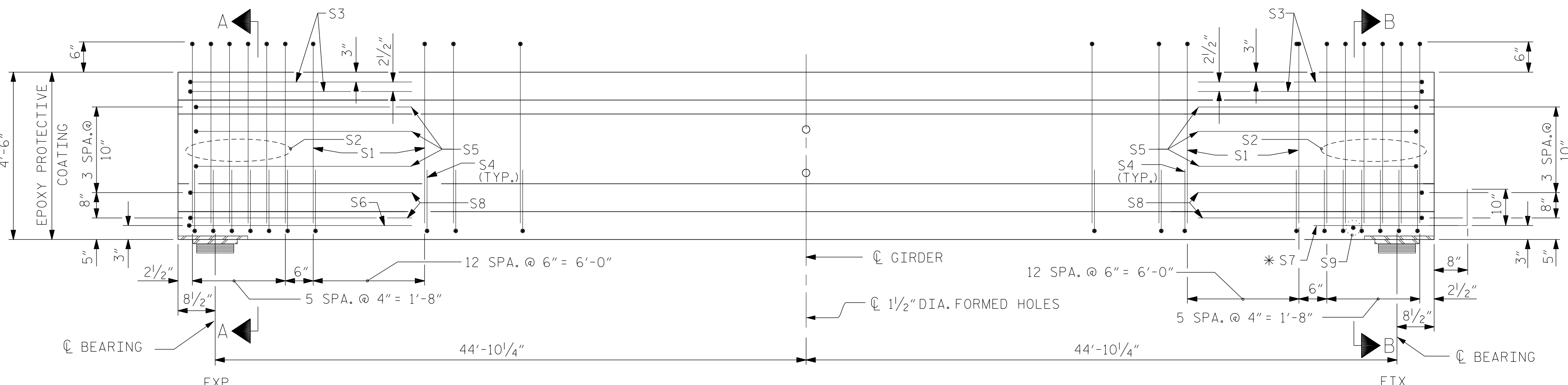


PLAN OF GIRDER



PARTIAL ELEVATION

SHOWING INTERMEDIATE DIAPHRAGM REINFORCING STEEL FOR GIRDER Nos. 1 THRU 4



ELEVATION OF GIRDER

(SEE PARTIAL ELEVATION FOR ADDITIONAL "S" BARS)

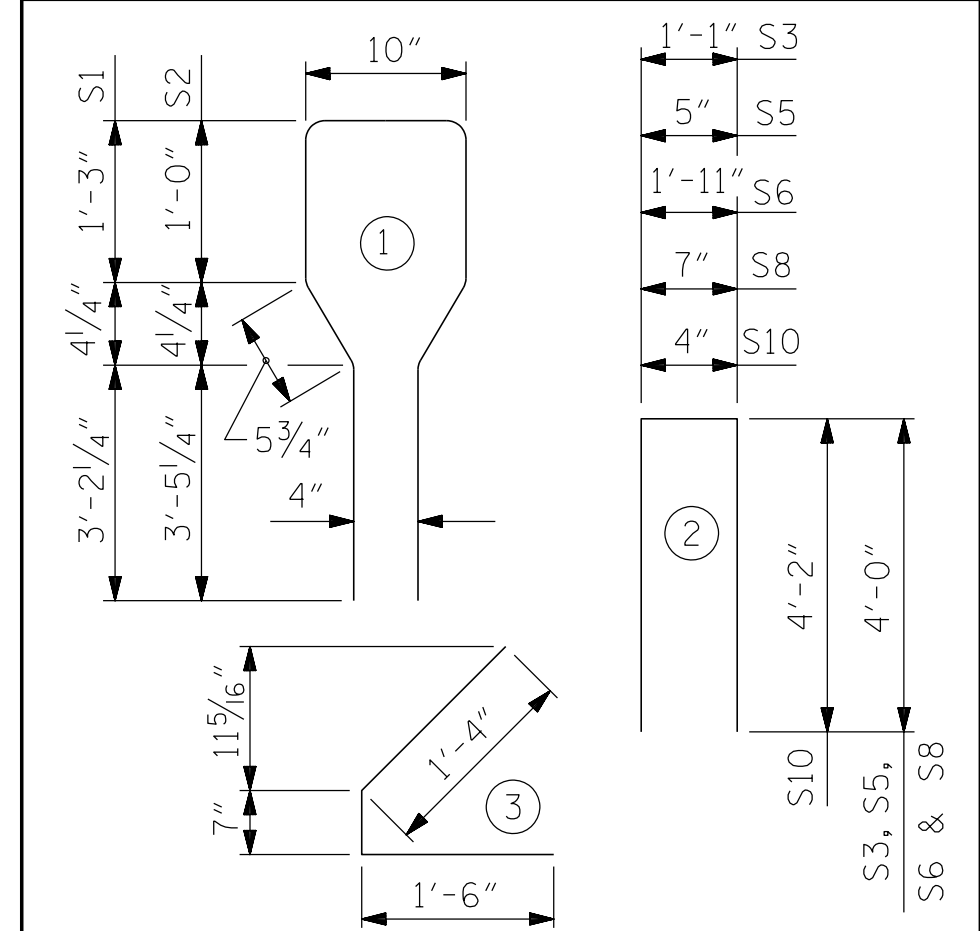
0.6" Ø L. R. GRADE 270 STRANDS		
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GIRDER					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	77	#5	1	10'-8"	857
S2	12	#6	1	10'-8"	192
S3	4	#4	2	9'-1"	24
S4	88	#4	3	3'-5"	201
S5	6	#4	2	8'-5"	34
S6	1	#4	2	9'-11"	7
* S7	6	#5	STR	3'-8"	23
S8	4	#4	2	8'-7"	23
S9	1	#3	STR	1'-10"	1
S10	2	#5	2	8'-8"	18
S11	5	#4	STR	7'-0"	24

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.

BAR TYPES

ALL BAR DIMENSIONS ARE OUT-TO-OUT



QUANTITIES FOR ONE GIRDER

	REINFORCING STEEL	8,000 PSI CONCRETE	0.6" Ø L. R. STRANDS
	LB.	C.Y.	No.
EXTERIOR GIRDER	1404	18.5	38
INTERIOR GIRDER	1404	18.5	38

GIRDERS REQUIRED

NUMBER	LENGTH	TOTAL LENGTH
4	91'-1 1/2"	364'-6"

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 20+70.23-Y3- POT

SHEET 1 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
AASHTO TYPE IV
PRESTRESSED CONCRETE GIRDER
CONTINUOUS FOR LIVE LOAD
SPAN A



DocuSigned by:
Jeff Capus
2/6/2017

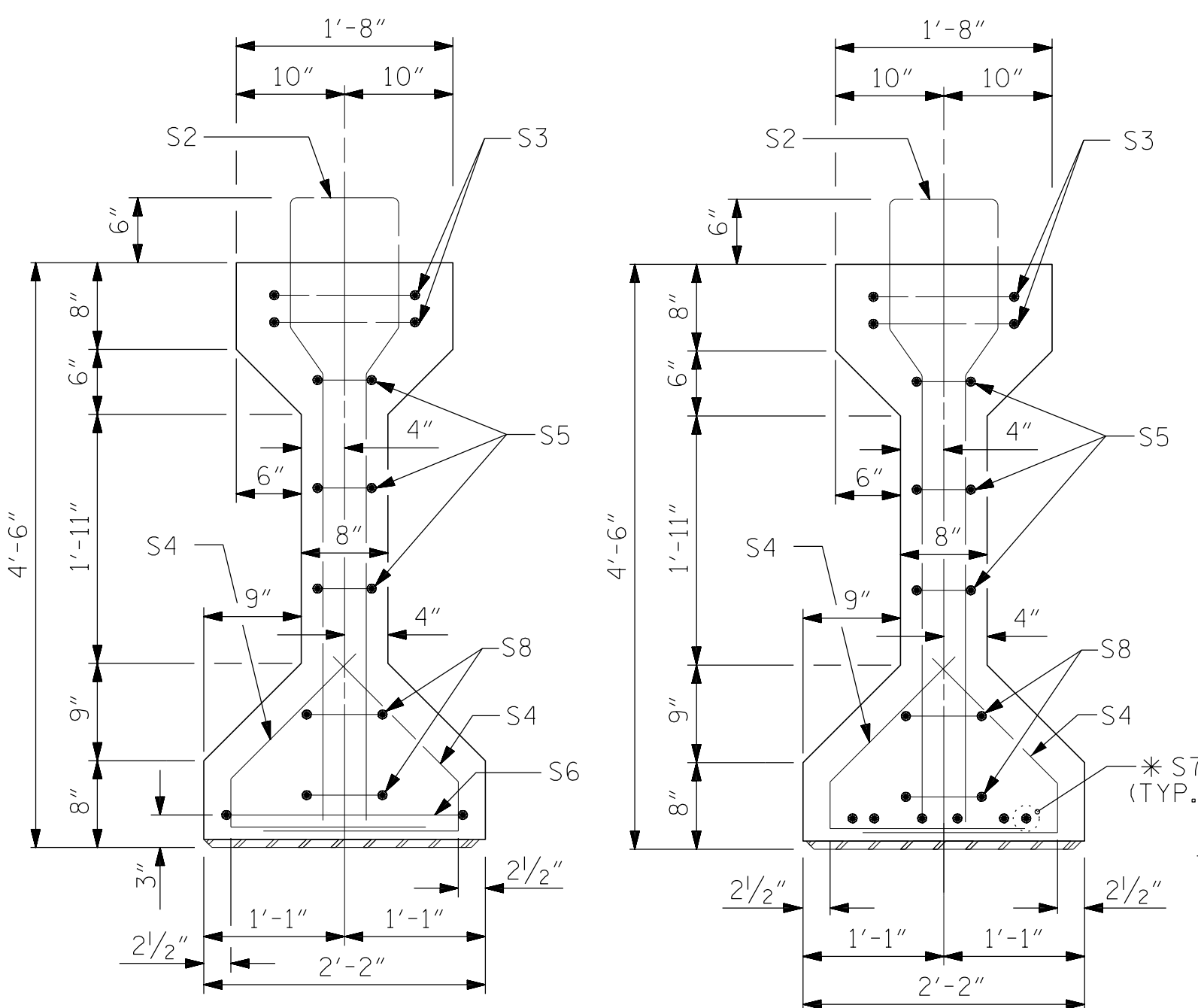
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REVISIONS						SHEET NO. S4-10
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 29
2			4			

STR. #4
STD. NO. PCG6

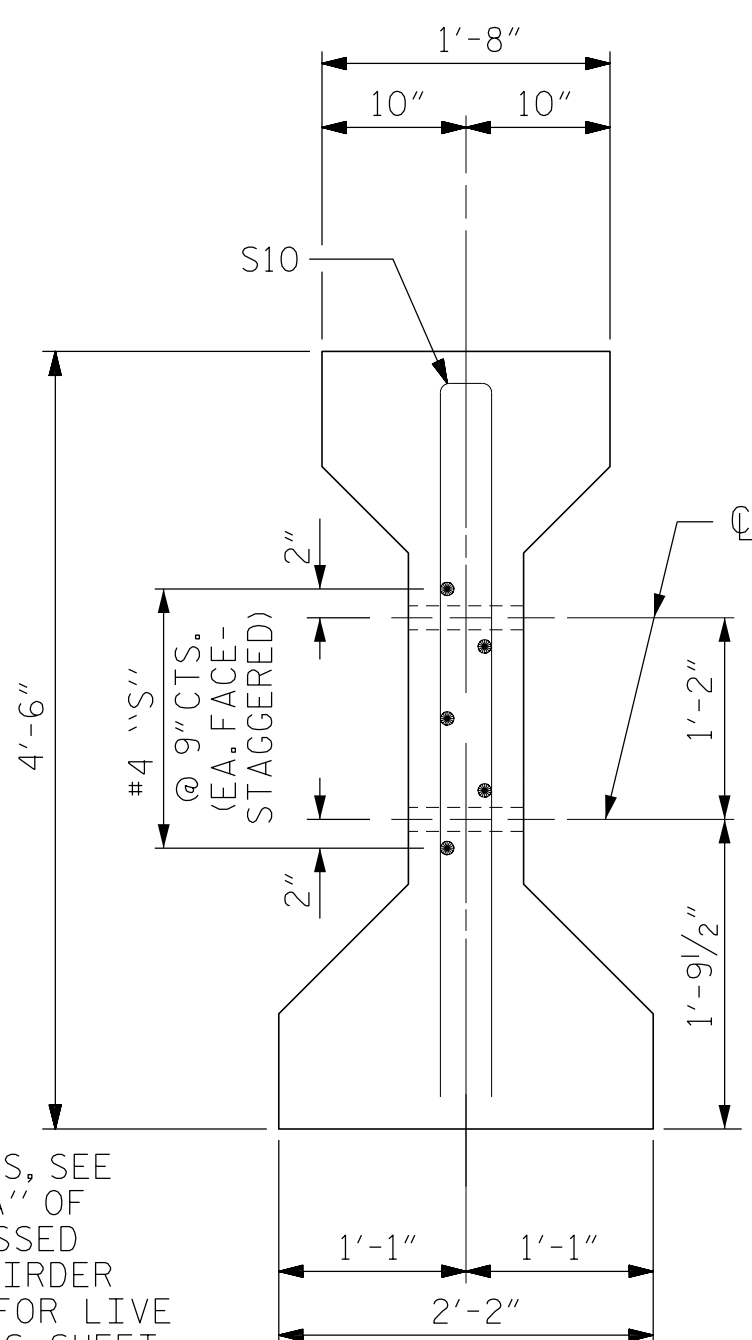
DRAWN BY: H.ASSFOURA	DATE: 07/16
CHECKED BY: J.LOFIUS	DATE: 11/16
DESIGN ENGINEER OF RECORD: J.LOFIUS	DATE: 01/17

R 2707C-SITE 3
 2/6/2017
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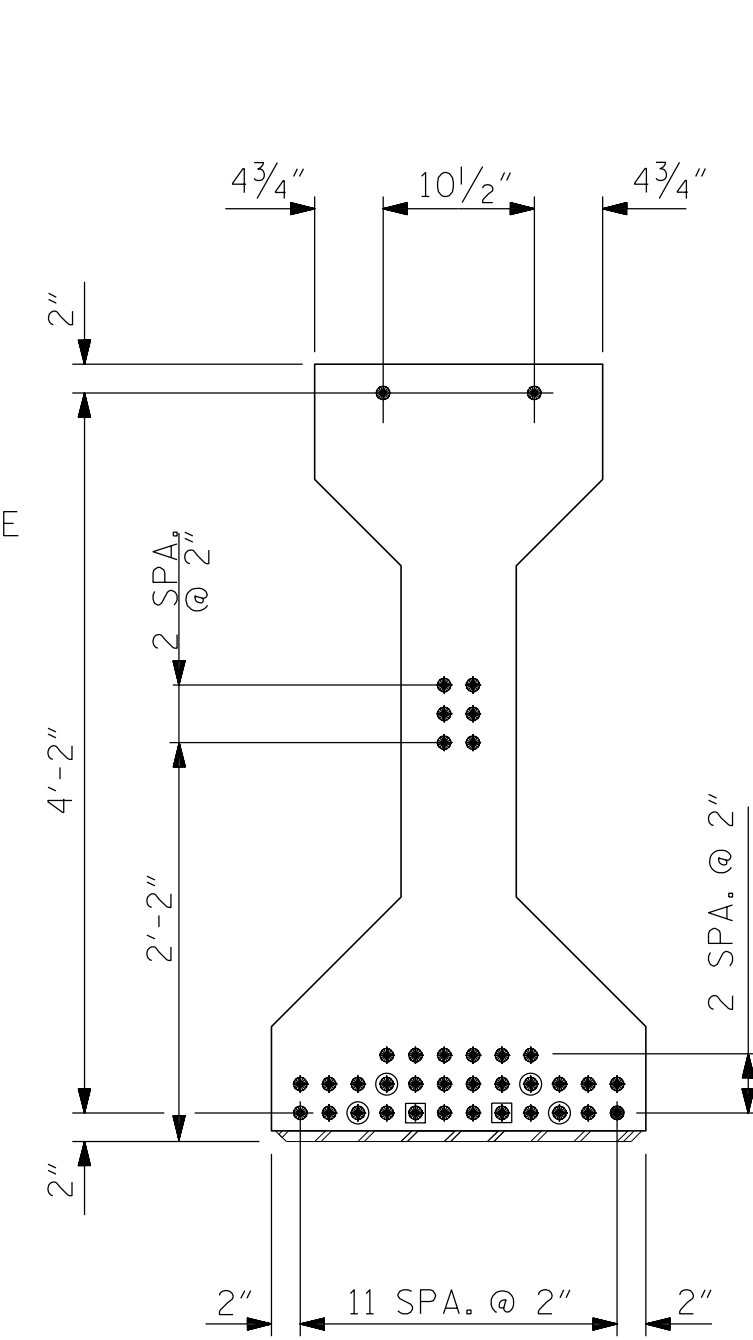


SECTION A-A

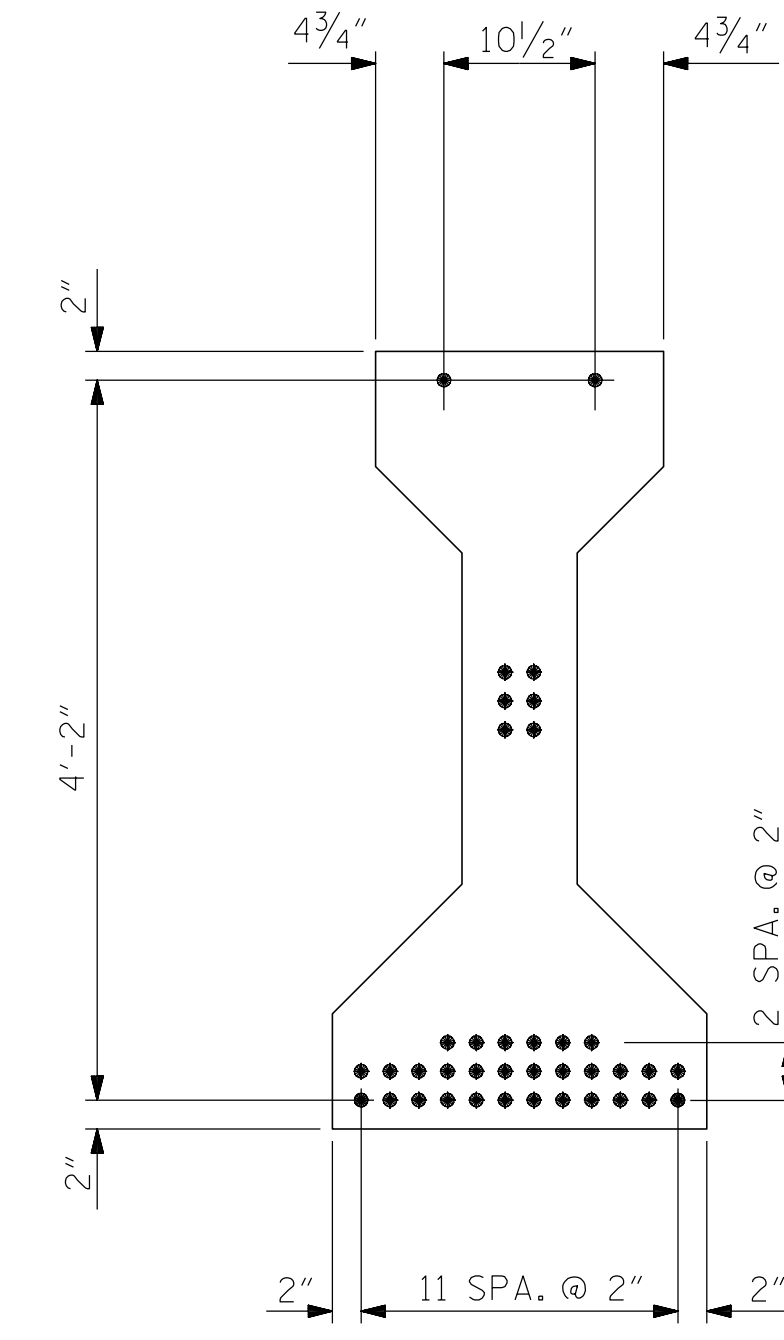
SECTION B-B



SECTION C-C
(S1 BARS NOT SHOWN)



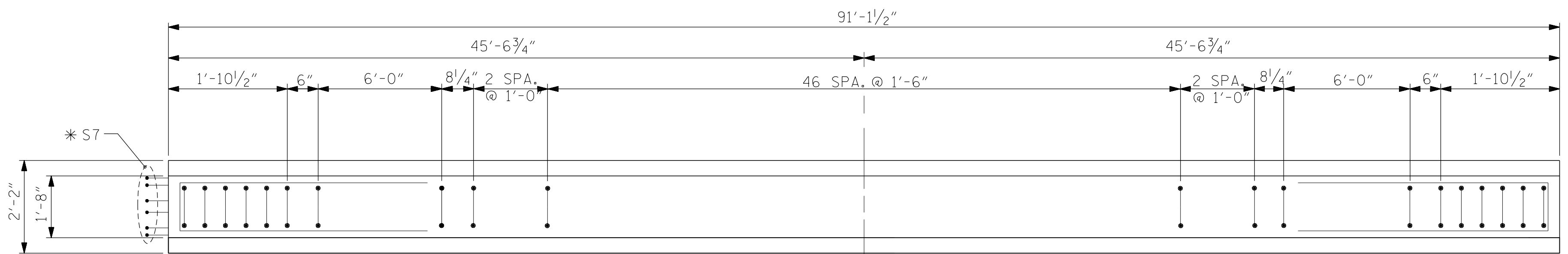
AT END OF GIRDER



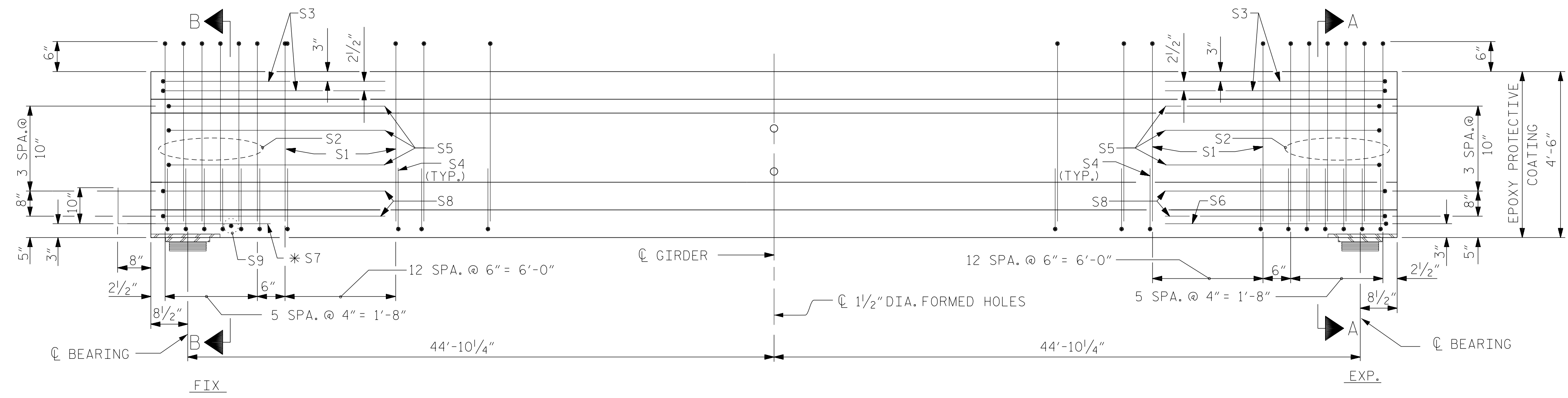
AT C OF GIRDER

0.6" Ø LOW RELAXATION STRAND LAYOUT

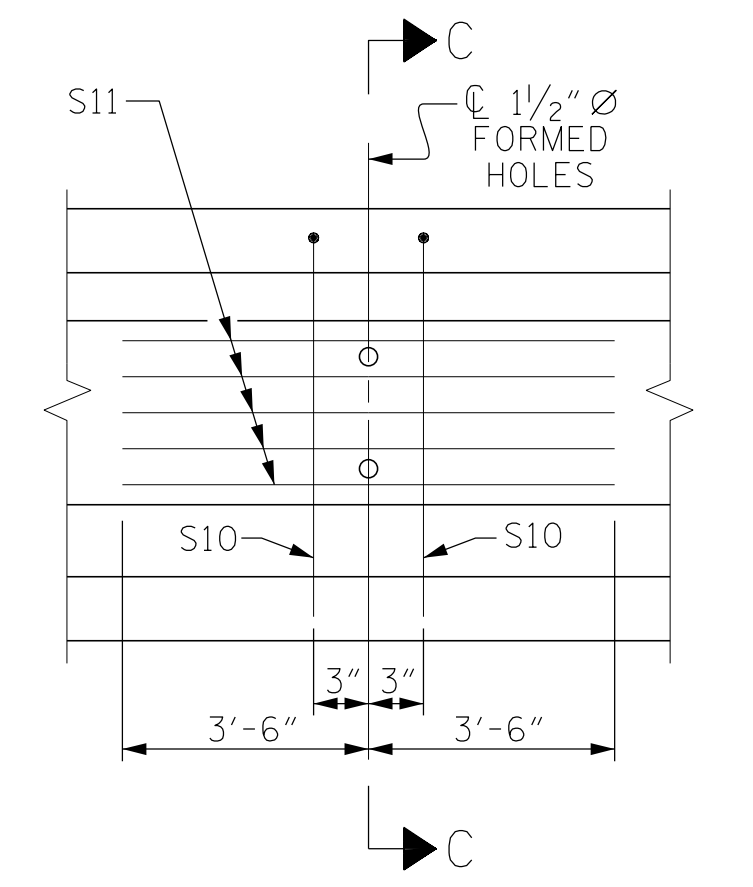
- FULLY BONDED STRANDS
- STRANDS DEBONDED FOR 8'-0" FROM END OF GIRDER
- STRANDS DEBONDED FOR 4'-0" FROM END OF GIRDER



PLAN OF GIRDER



ELEVATION OF GIRDER
(SEE PARTIAL ELEVATION FOR ADDITIONAL "S" BARS)



PARTIAL ELEVATION

SHOWING INTERMEDIATE DIAPHRAGM REINFORCING STEEL FOR GIRDER Nos. 1 THRU 4

0.6" Ø L. R. GRADE 270 STRANDS

AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

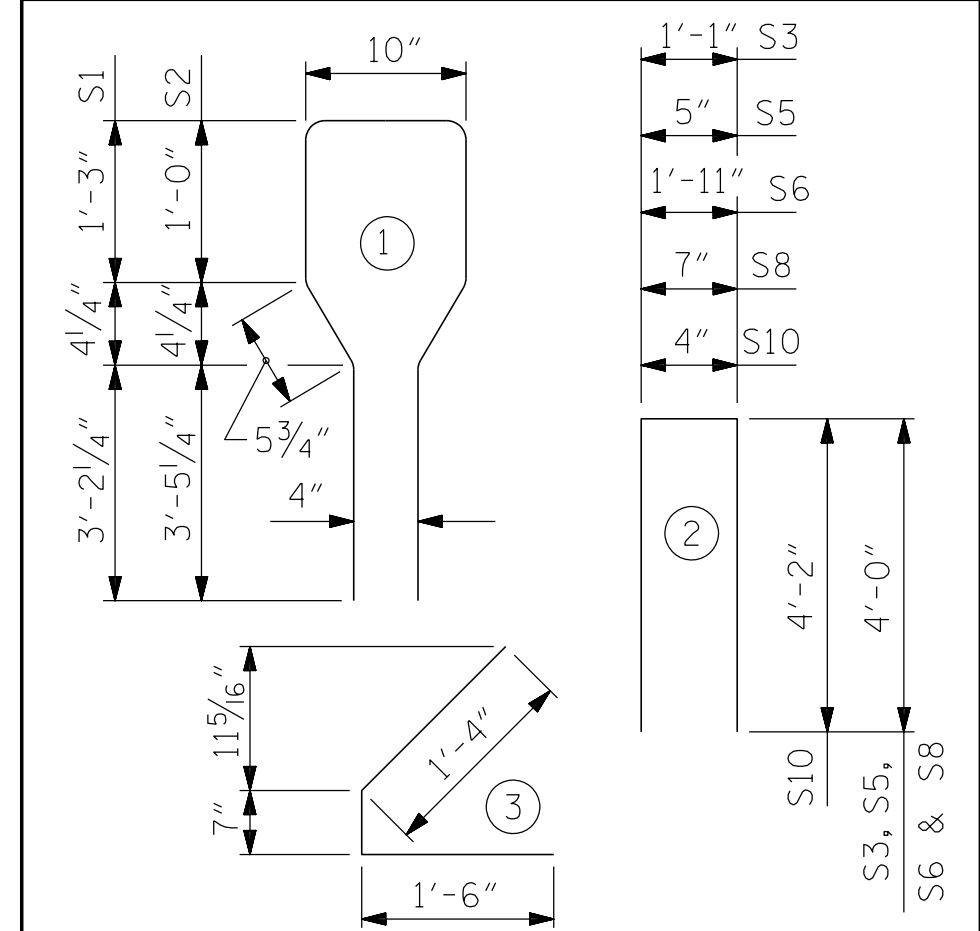
REINFORCING STEEL FOR ONE GIRDER

BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	77	#5	1	10'-8"	857
S2	12	#6	1	10'-8"	192
S3	4	#4	2	9'-1"	24
S4	88	#4	3	3'-5"	201
S5	6	#4	2	8'-5"	34
S6	1	#4	2	9'-11"	7
*S7	6	#5	STR	3'-8"	23
S8	4	#4	2	8'-7"	23
S9	1	#3	STR	1'-10"	1
S10	2	#5	2	8'-8"	18
S11	5	#4	STR	7'-0"	24

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.

BAR TYPES

ALL BAR DIMENSIONS ARE OUT-TO-OUT



QUANTITIES FOR ONE GIRDER

	REINFORCING STEEL LB.	8,000 PSI CONCRETE C.Y.	0.6" Ø L. R. STRANDS No.
EXTERIOR GIRDER	1404	18.5	38
INTERIOR GIRDER	1404	18.5	38

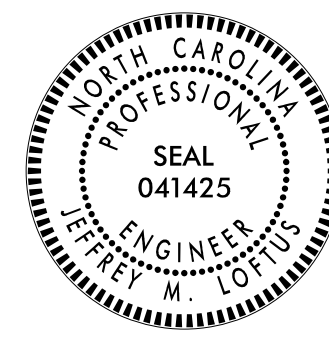
GIRDERS REQUIRED

NUMBER	LENGTH	TOTAL LENGTH
4	91'-1 1/2"	364'-6"

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 20+70.23-Y3- POT

SHEET 2 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
AASHTO TYPE IV
PRESTRESSED CONCRETE GIRDER
CONTINUOUS FOR LIVE LOAD
SPAN B



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REVISIONS						SHEET NO.
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1			3			TOTAL SHEETS 29
2			4			

STR. #4
STD. NO. PCG6

DRAWN BY: H.ASSFOURA	DATE: 07/16
CHECKED BY: J.LOFIUS	DATE: 11/16
DESIGN ENGINEER OF RECORD: J.LOFIUS	DATE: 01/17

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2/6/2017
\\404-021-R2707C-SMU-PCG2-S4-11.dgn
USER:deFault

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

APPLY EPOXY PROTECTIVE COATING TO END OF GIRDER SURFACES INDICATED IN ELEVATION VIEW.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

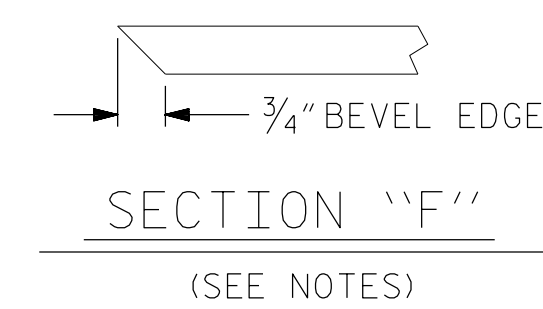
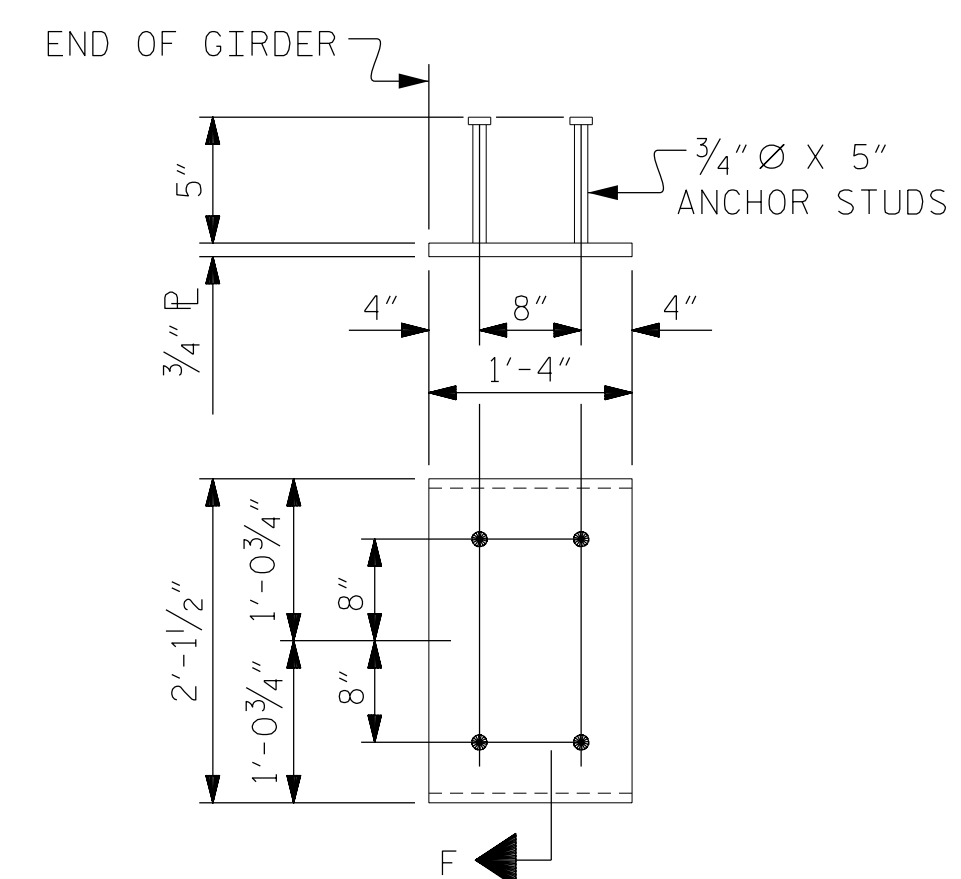
AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6000 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

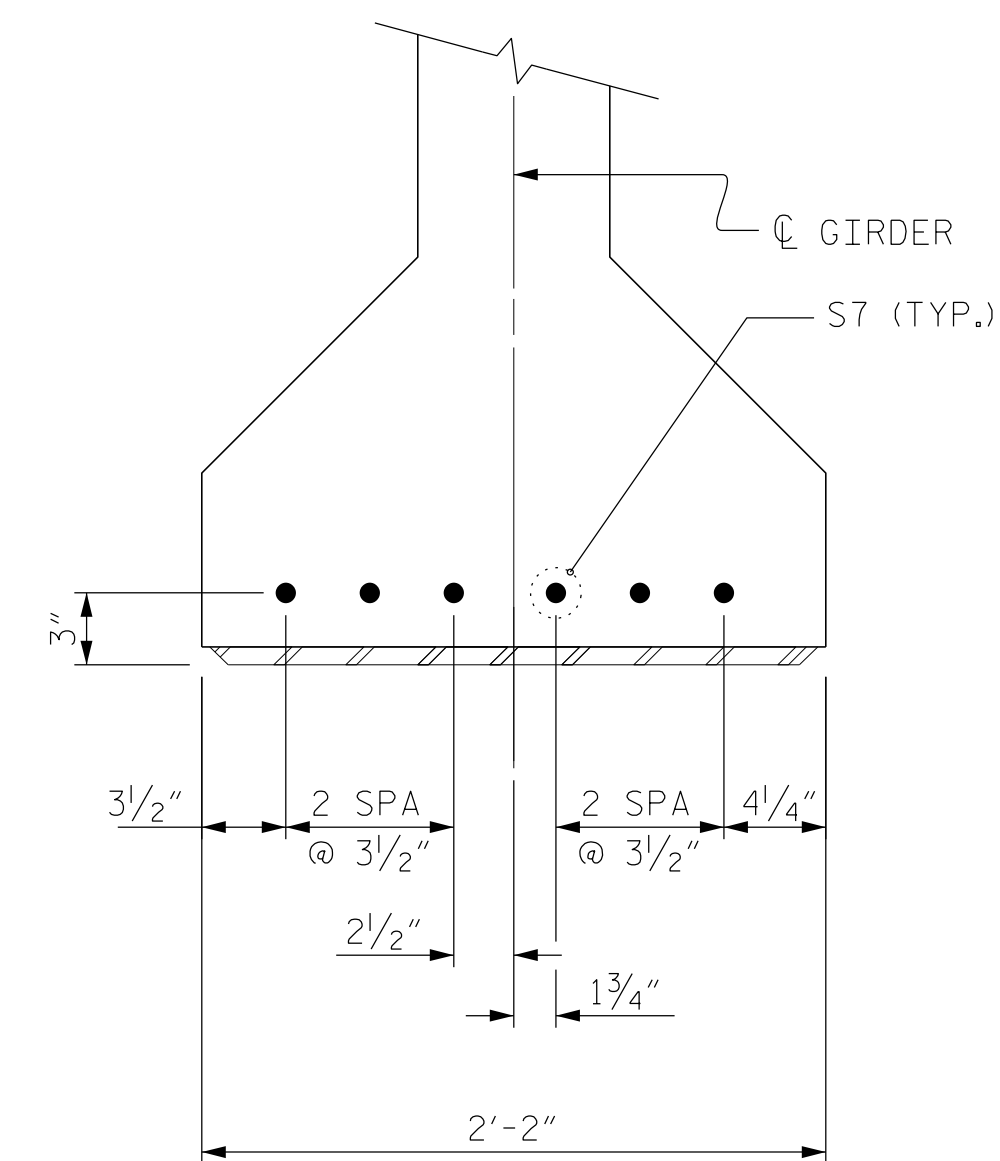
THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.



EMBEDDED PLATE "B-1" DETAILS

TWO EMBEDDED PLATES "B-1" ARE REQUIRED FOR EACH GIRDER.



DETAIL "A"

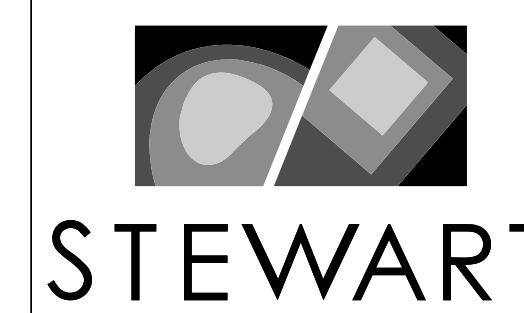
PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT

SHEET 3 OF 3



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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

PRESTRESSED CONCRETE
 GIRDER CONTINUOUS FOR
 LIVE LOAD DETAILS

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			29
2			4			

STR. #4

DRAWN BY: H.ASSFOURA DATE: 07/16
 CHECKED BY: J.LOFTUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: J.LOFTUS DATE: 01/17

2/6/2017

\\404_023-R2707C-SMU-PC03_S4-12.dgn
 USER: jloftus

BRIDGE SITE 3

STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE CHANNEL MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, CHANNELS, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

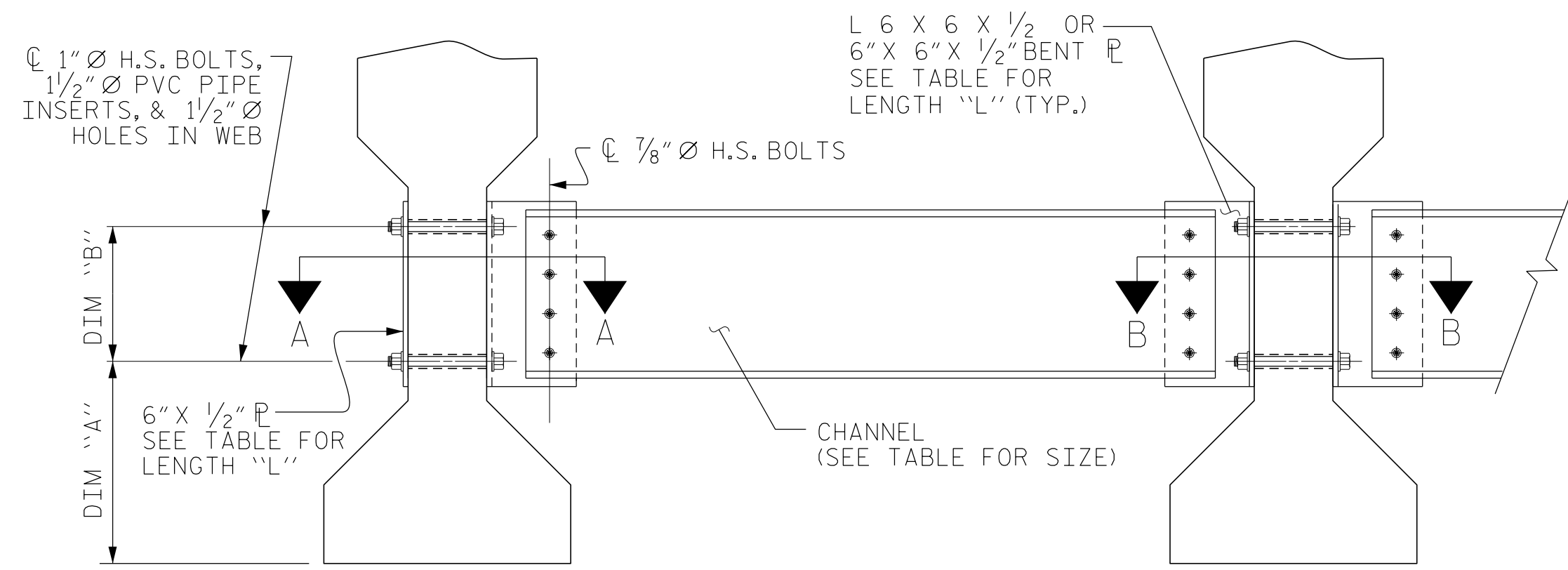
FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

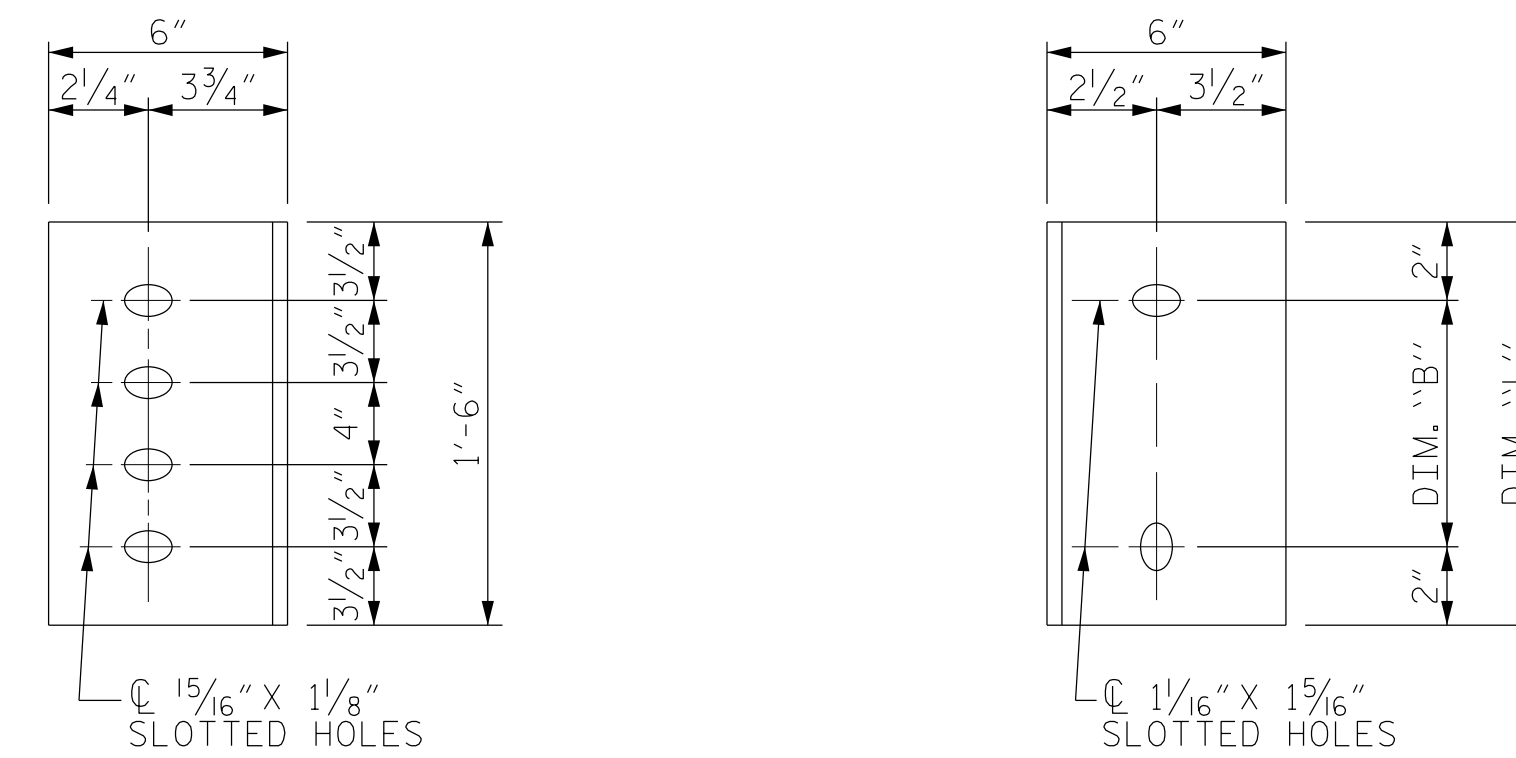
IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.



EXTERIOR GIRDER INTERIOR GIRDER

PART SECTION AT INTERMEDIATE DIAPHRAGM



DIAPHRAGM FACE WEB FACE

CONNECTOR PLATE DETAILS

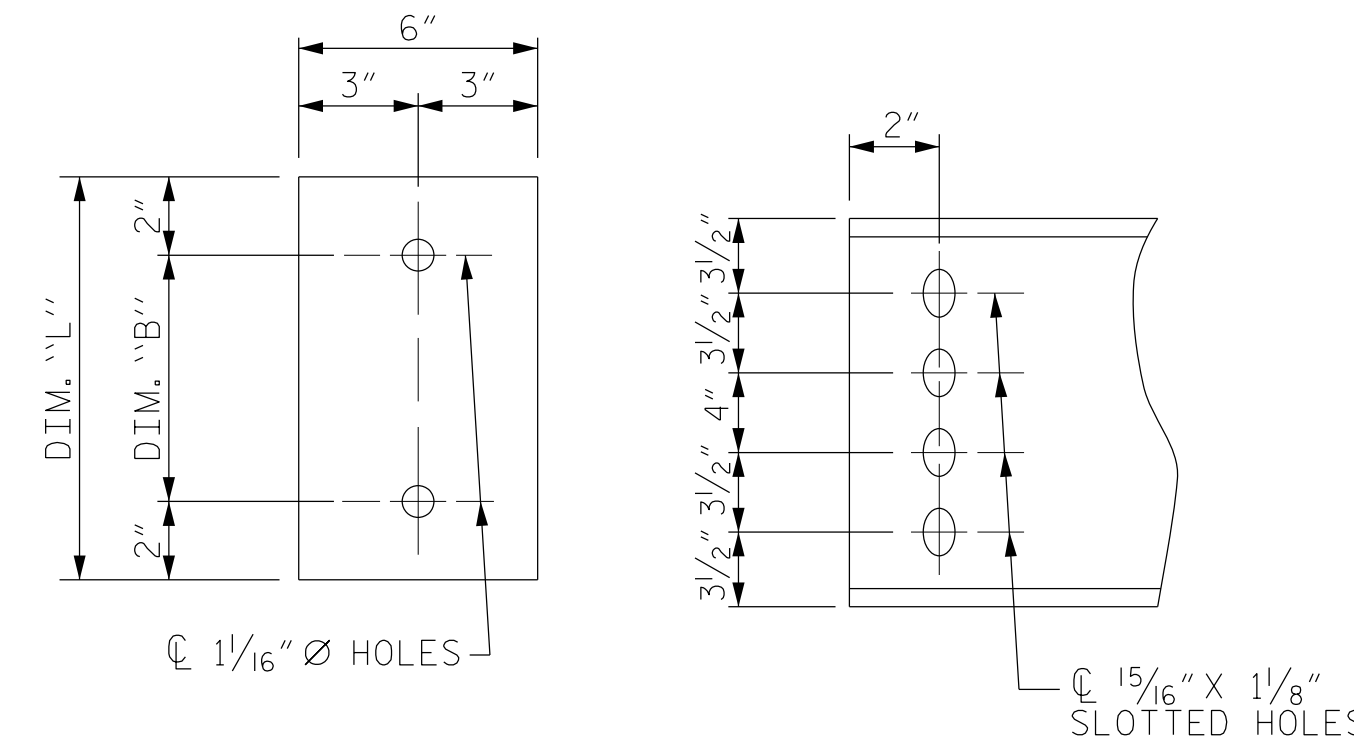
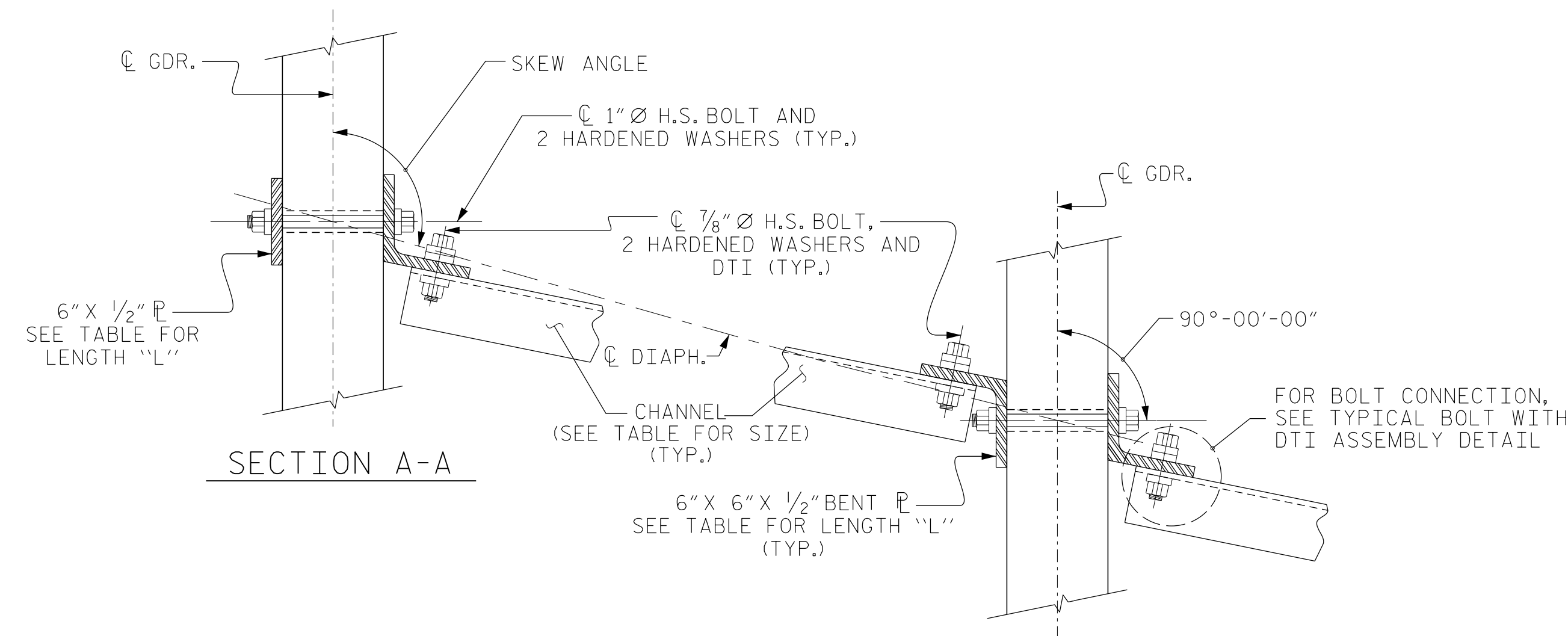
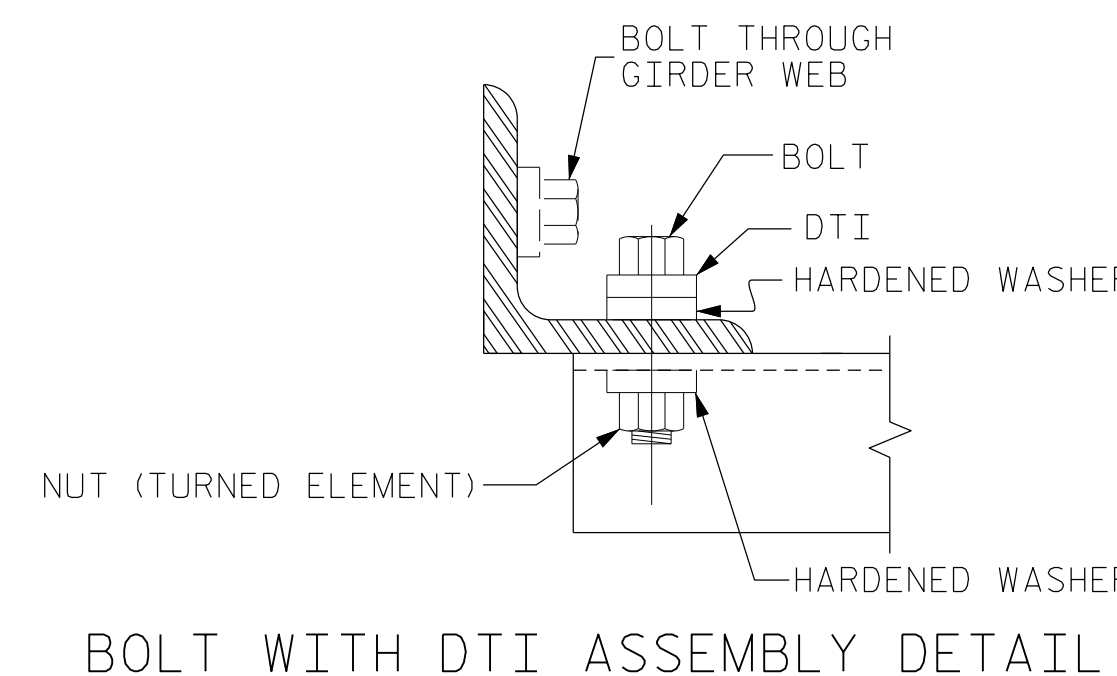


PLATE DETAILS CHANNEL END



CONNECTION DETAILS

(90° < SKEW < 110° SHOWN
70° < SKEW < 90° SIM.)



BOLT WITH DTI ASSEMBLY DETAIL

GIRDER TYPE	CHANNEL SIZE	DIM "A"	DIM "B"	DIM "L"
IV	MC 18 x 42.7	1'-9 1/2"	1'-2"	1'-6"

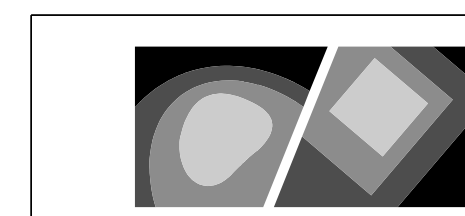
TABLE

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 20+70.23-Y3- POT



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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH				
STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR TYPE II, III, & IV PRESTRESSED CONCRETE GIRDERS				
REVISIONS				SHEET NO.
NO.	BY:	DATE:	NO.	DATE:
1			3	
2			4	
				S4-13
				TOTAL SHEETS 29

STR. #4

DRAWN BY: H.ASSFOURA DATE: 07/16
CHECKED BY: J.LOFIUS DATE: 11/16
DESIGN ENGINEER OF RECORD: J.LOFIUS DATE: 01/17

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NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

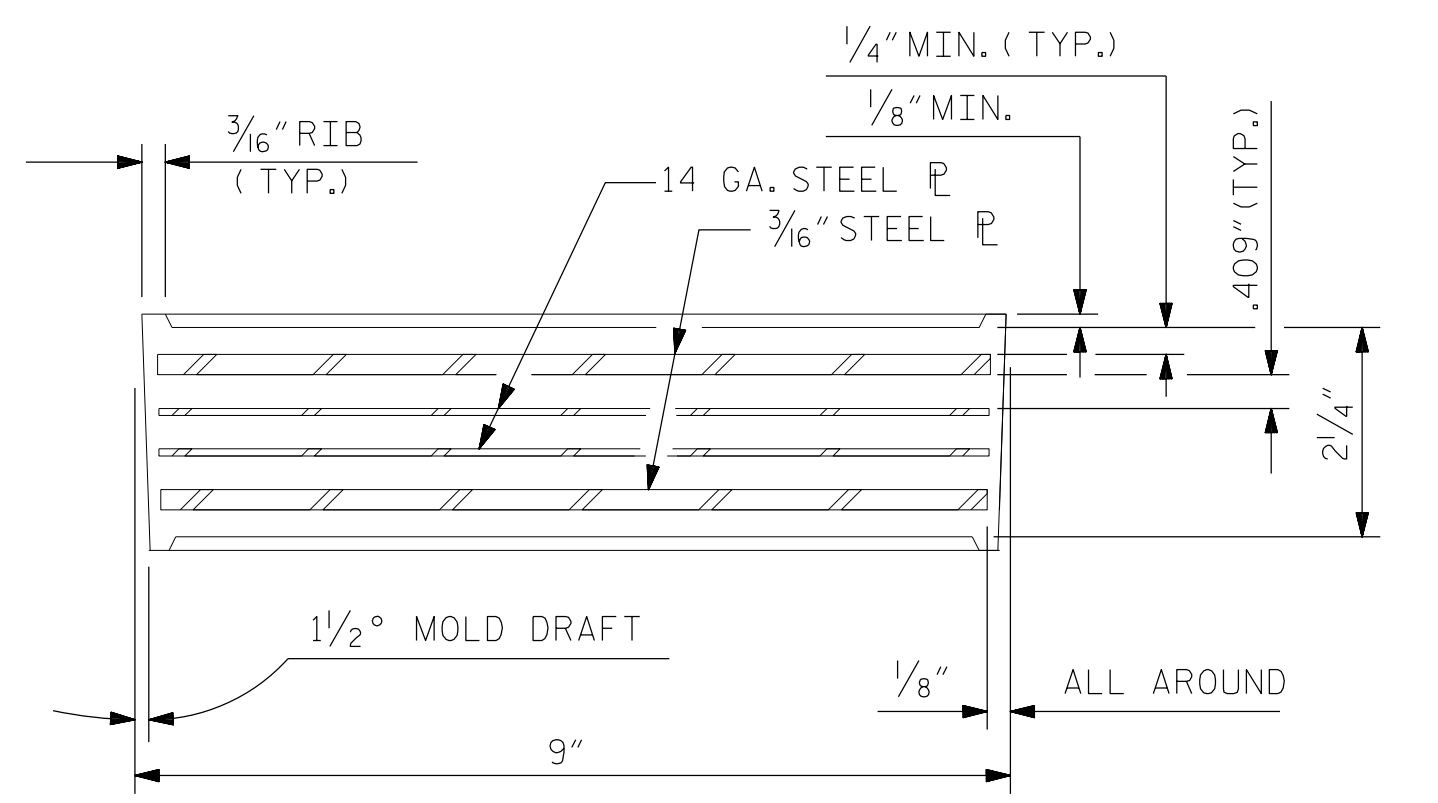
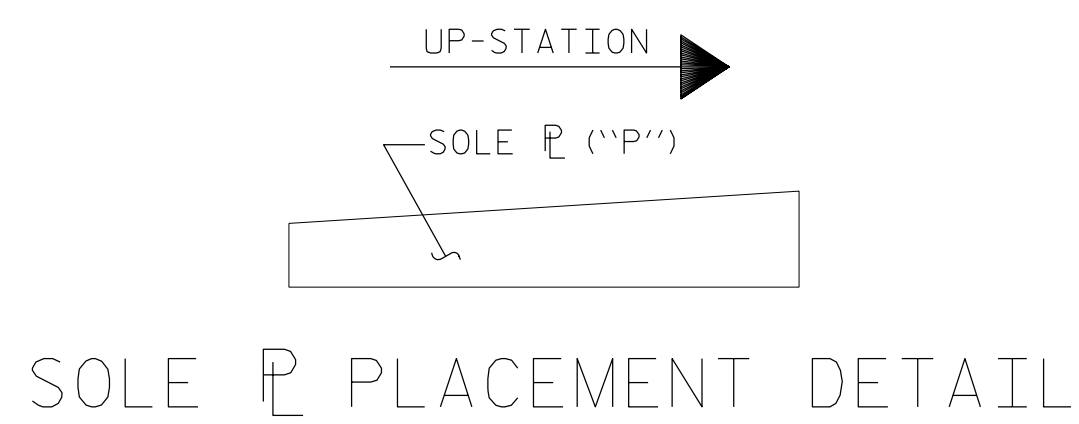
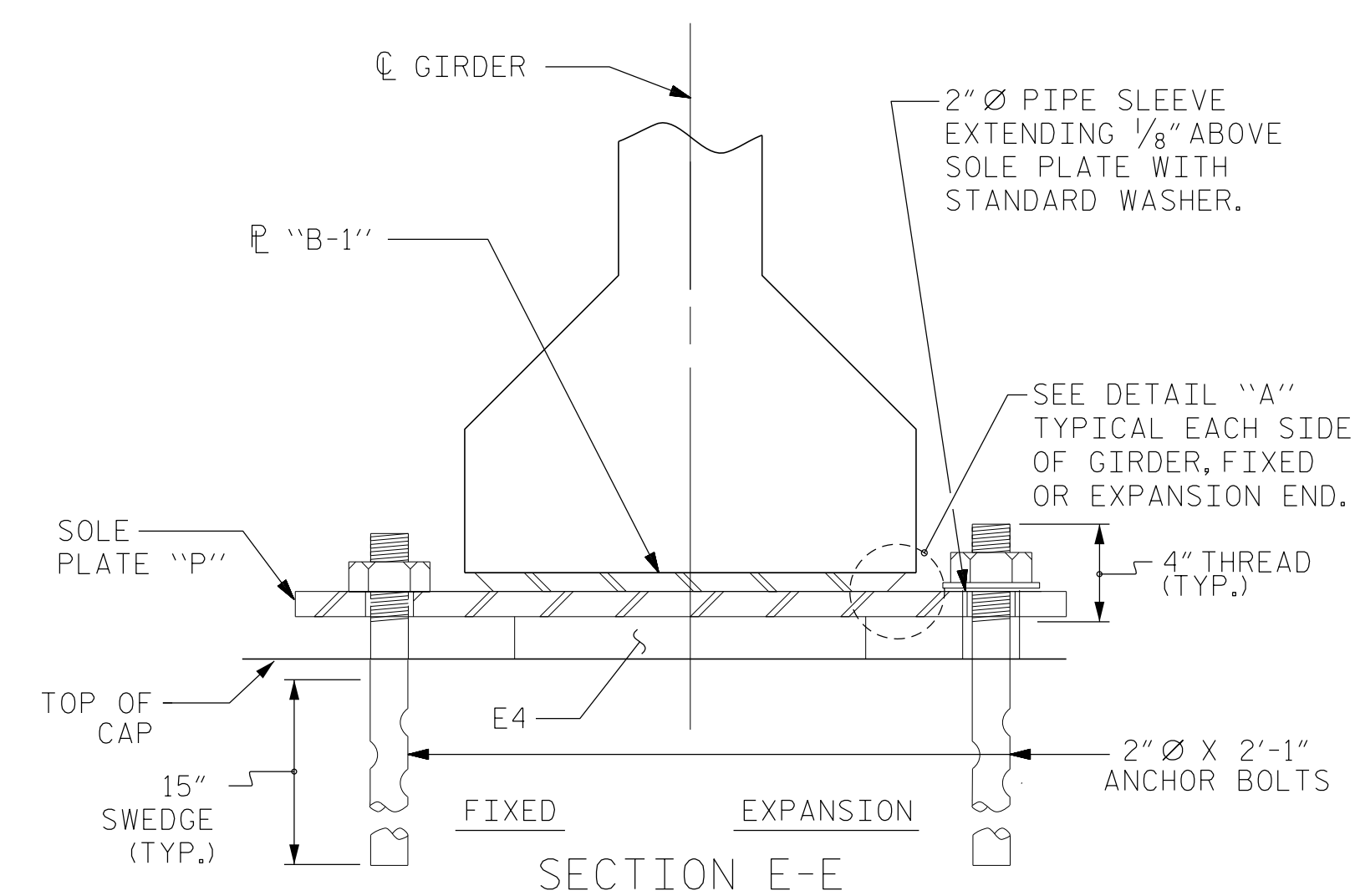
ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. NO SHOP DRAWINGS ARE REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

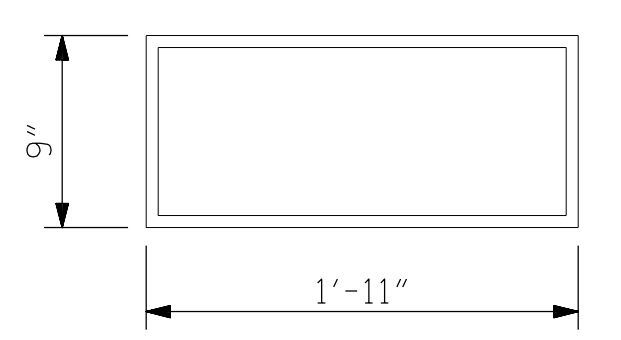
THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

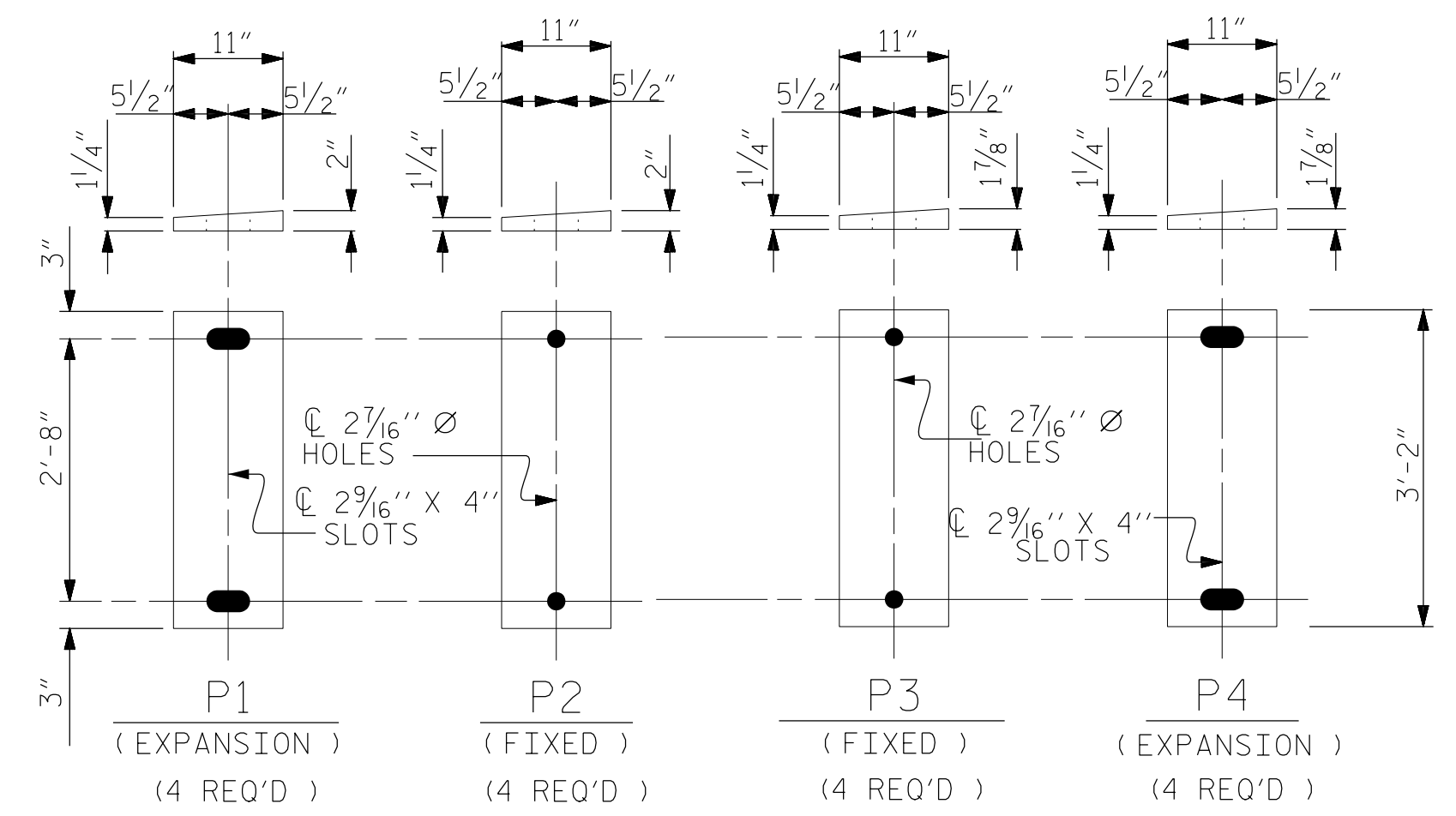
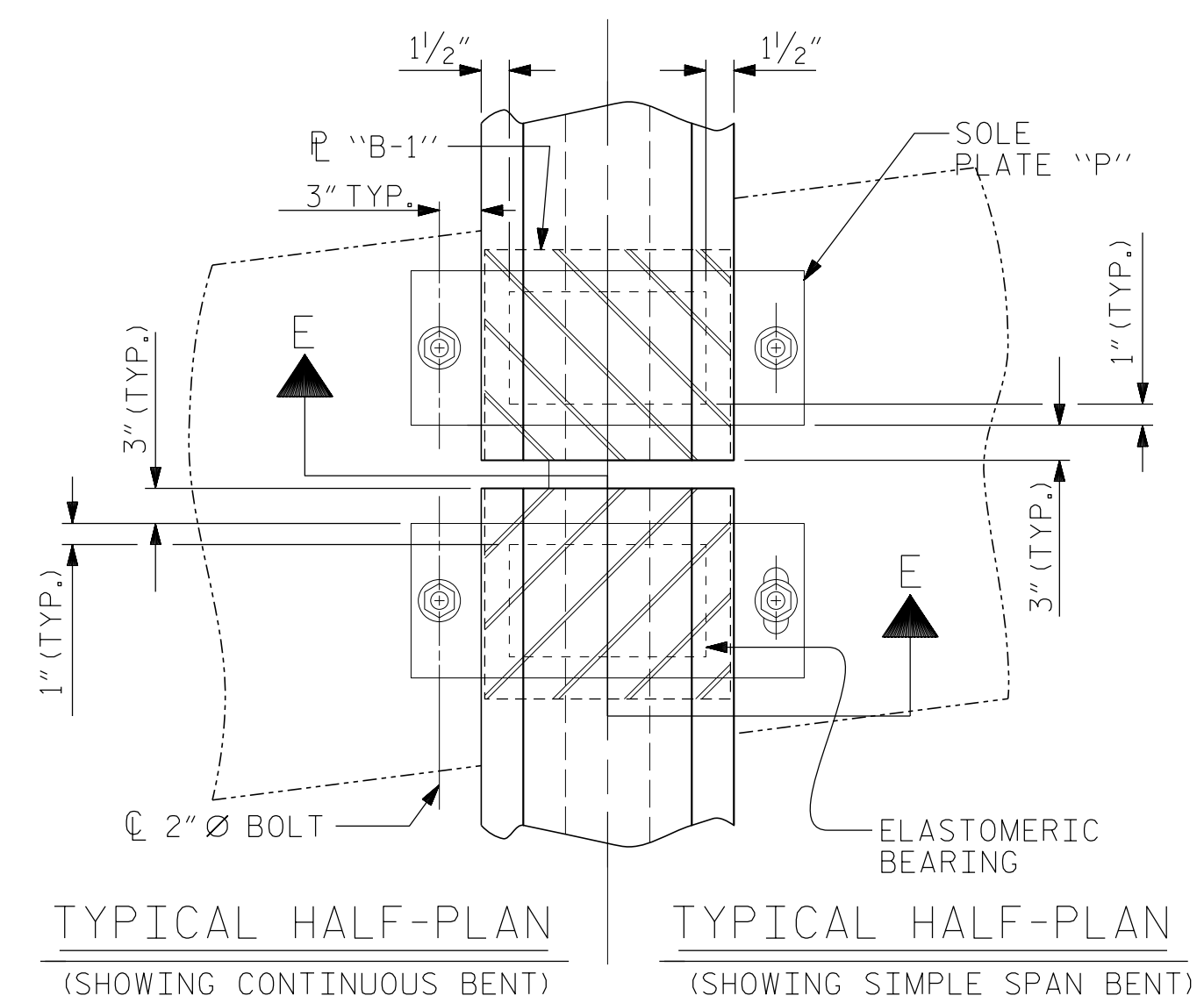
MAXIMUM ALLOWABLE SERVICE LOADS	
D.L.+L.L. (NO IMPACT)	
TYPE V	365 k



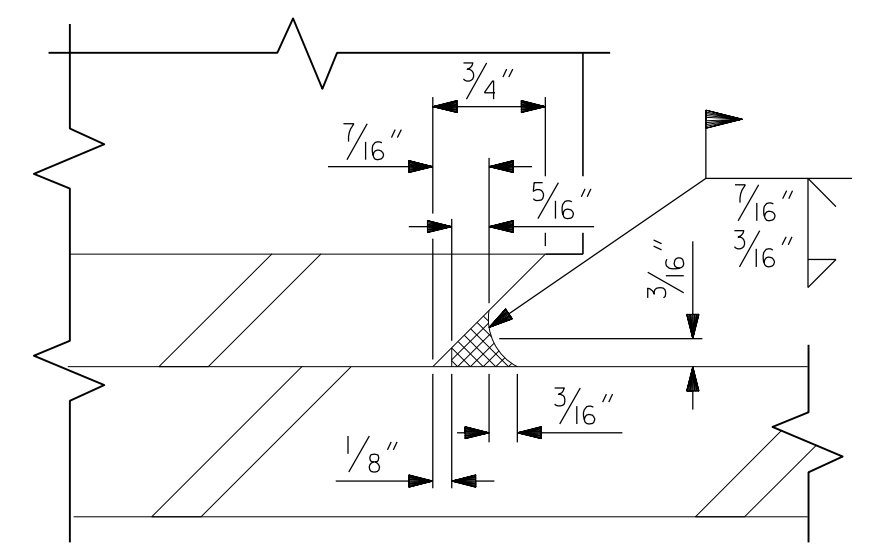
TYPICAL SECTION OF ELASTOMERIC BEARINGS



E4 (16 REQ'D)
PLAN VIEW OF ELASTOMERIC BEARING
TYPE V

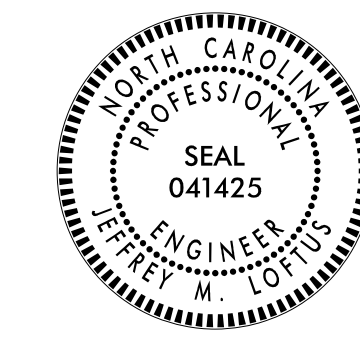


SOLE PLATE DETAILS ("P")



DETAIL "A"

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 20+70.23-Y3- POT



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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
ELASTOMERIC BEARING
DETAILS

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-14
1			3			TOTAL SHEETS
2			4			29

STR. #4

DRAWN BY: H.ASSFOURA DATE: 07/16
CHECKED BY: J.LOFIUS DATE: 11/16
DESIGN ENGINEER OF RECORD: J.LOFIUS DATE: 01/17

R 2707C-SITE 3
2/6/2017
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USER:dfault

DEAD LOAD DEFLECTION TABLE - SPAN A													
0.6 Ø LOW RELAXATION													
TENTH POINTS													
GIRDER		☉ BRG.	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	☉ BRG.	
A1	CAMBER (GIRDER ALONE IN PLACE)	↑	0	0.059	0.111	0.152	0.177	0.186	0.177	0.152	0.111	0.059	0
	* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0	0.039	0.077	0.106	0.124	0.130	0.123	0.103	0.074	0.037	0
	FINAL CAMBER	↑	0	1/4"	7/16"	9/16"	5/8"	11/16"	5/8"	9/16"	7/16"	1/4"	0
A2	CAMBER (GIRDER ALONE IN PLACE)	↑	0	0.059	0.111	0.152	0.177	0.186	0.177	0.152	0.111	0.059	0
	* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0	0.047	0.092	0.127	0.149	0.156	0.148	0.125	0.089	0.045	0
	FINAL CAMBER	↑	0	1/8"	1/4"	5/16"	5/16"	3/8"	3/8"	5/16"	1/4"	3/16"	0
A3	CAMBER (GIRDER ALONE IN PLACE)	↑	0	0.059	0.111	0.152	0.177	0.186	0.177	0.152	0.111	0.059	0
	* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0	0.047	0.092	0.127	0.149	0.156	0.148	0.125	0.089	0.045	0
	FINAL CAMBER	↑	0	1/8"	1/4"	5/16"	5/16"	3/8"	3/8"	5/16"	1/4"	3/16"	0
A4	CAMBER (GIRDER ALONE IN PLACE)	↑	0	0.059	0.111	0.152	0.177	0.186	0.177	0.152	0.111	0.059	0
	* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0	0.039	0.077	0.106	0.124	0.130	0.123	0.103	0.074	0.037	0
	FINAL CAMBER	↑	0	1/4"	7/16"	9/16"	5/8"	11/16"	5/8"	9/16"	7/16"	1/4"	0

DEAD LOAD DEFLECTION TABLE - SPAN B													
0.6 Ø LOW RELAXATION													
TENTH POINTS													
GIRDER		☉ BRG.	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	☉ BRG.	
B1	CAMBER (GIRDER ALONE IN PLACE)	↑	0	0.059	0.111	0.152	0.177	0.186	0.177	0.152	0.111	0.059	0
	* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0	0.036	0.072	0.101	0.120	0.127	0.121	0.104	0.075	0.038	0
	FINAL CAMBER	↑	0	1/4"	7/16"	5/8"	11/16"	11/16"	11/16"	9/16"	7/16"	1/4"	0
B2	CAMBER (GIRDER ALONE IN PLACE)	↑	0	0.059	0.111	0.152	0.177	0.186	0.177	0.152	0.111	0.059	0
	* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0	0.045	0.089	0.125	0.148	0.156	0.149	0.127	0.092	0.047	0
	FINAL CAMBER	↑	0	3/16"	1/4"	5/16"	3/8"	3/8"	5/16"	5/16"	1/4"	1/8"	0
B3	CAMBER (GIRDER ALONE IN PLACE)	↑	0	0.059	0.111	0.152	0.177	0.186	0.177	0.152	0.111	0.059	0
	* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0	0.045	0.089	0.125	0.148	0.156	0.149	0.127	0.092	0.047	0
	FINAL CAMBER	↑	0	3/16"	1/4"	5/16"	3/8"	3/8"	5/16"	5/16"	1/4"	1/8"	0
B4	CAMBER (GIRDER ALONE IN PLACE)	↑	0	0.059	0.111	0.152	0.177	0.186	0.177	0.152	0.111	0.059	0
	* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0	0.036	0.072	0.101	0.120	0.127	0.121	0.104	0.075	0.038	0
	FINAL CAMBER	↑	0	1/4"	7/16"	5/8"	11/16"	11/16"	11/16"	9/16"	7/16"	1/4"	0

* INCLUDES FUTURE WEARING SURFACE

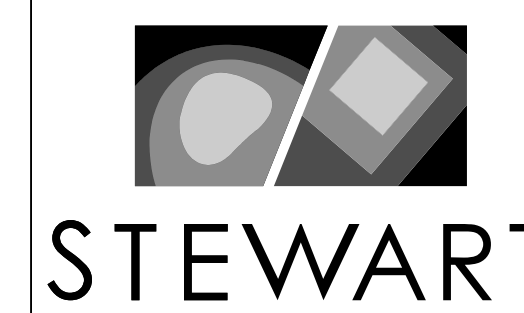
ALL VALUES ARE GIVEN IN FEET (DECIMAL FORMAT), EXCEPT "FINAL CAMBER" WHICH IS GIVEN IN INCHES

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT



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 FES1DC2E879449...

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
DEAD LOAD DEFLECTION FOR PRESTRESSED CONCRETE GIRDER					
REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS 29

DRAWN BY: H.ASSFOURA DATE: 07/16
 CHECKED BY: J.LOFTUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: J.LOFTUS DATE: 01/17

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 7/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

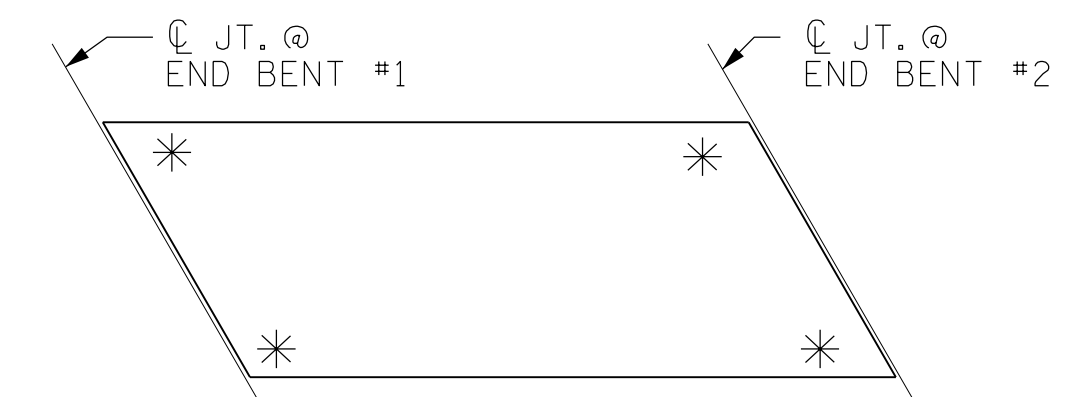
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



SKETCH SHOWING POINTS OF ATTACHMENTS

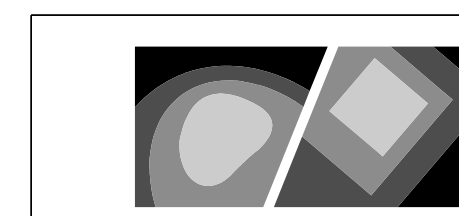
* DENOTES GUARDRAIL ANCHOR ASSEMBLY

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 STATION: 20+70.23-Y3- POT



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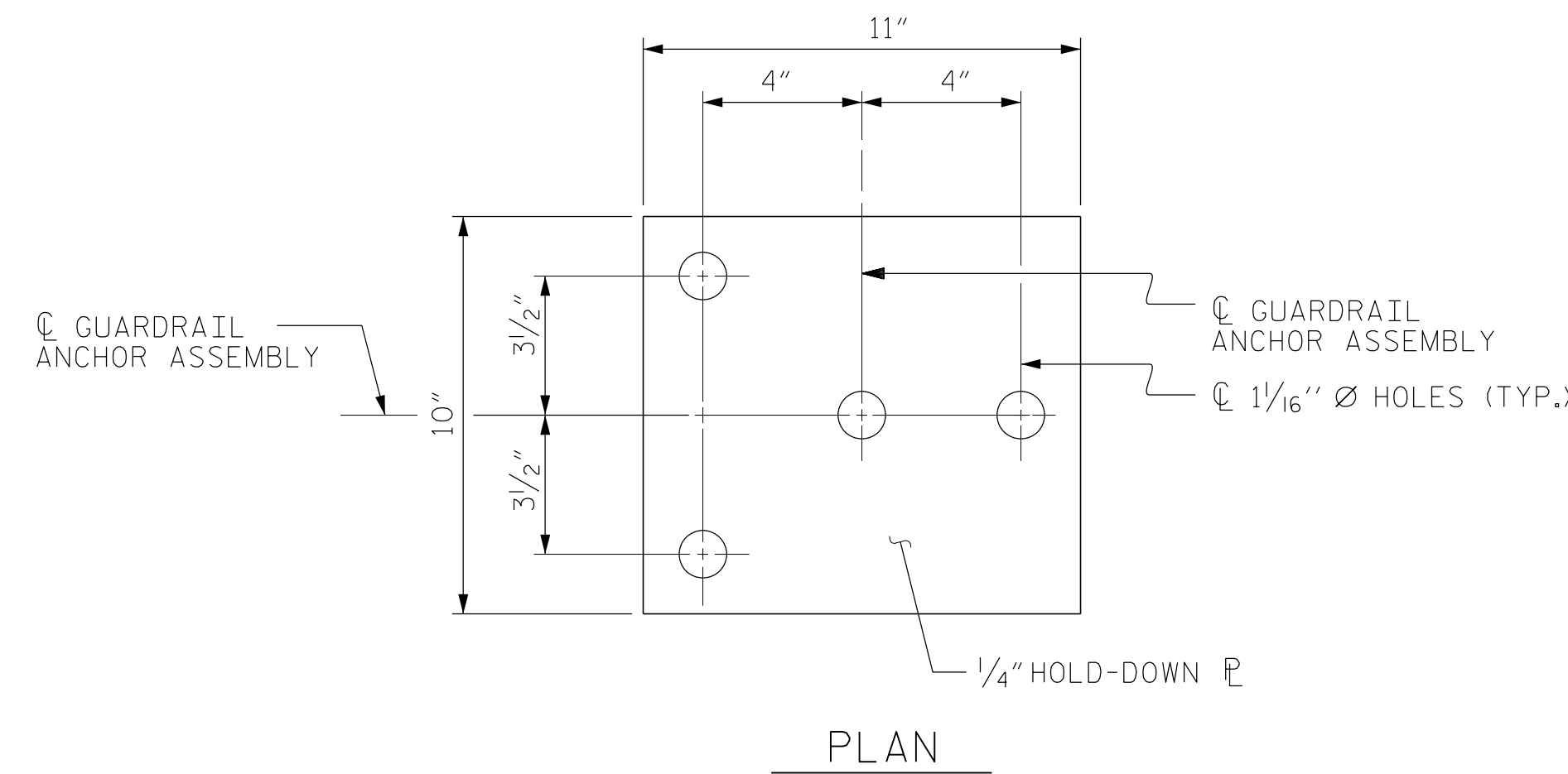


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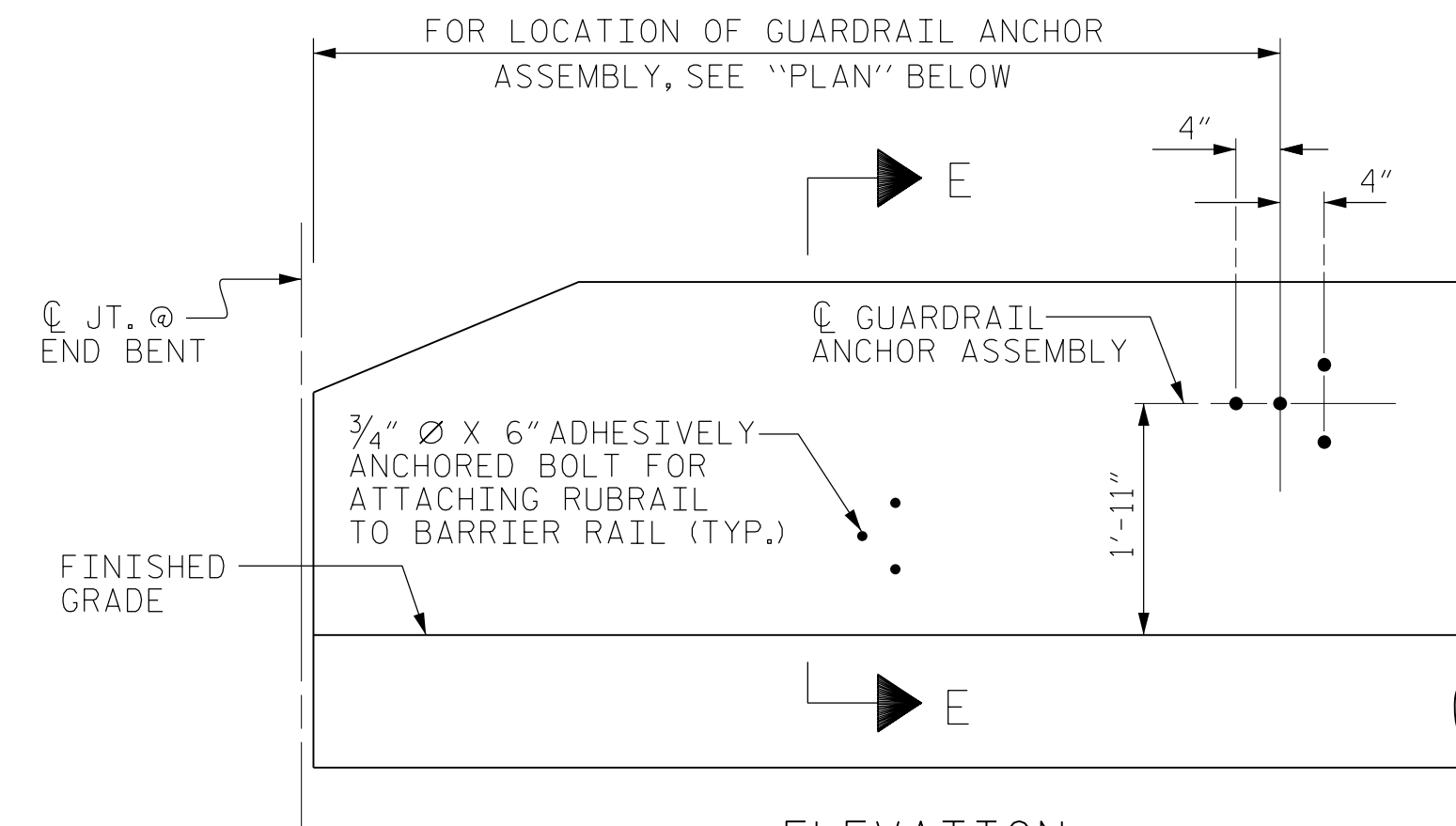
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 GUARDRAIL ANCHORAGE
 FOR BARRIER RAIL

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS
1			3			29
2			4			

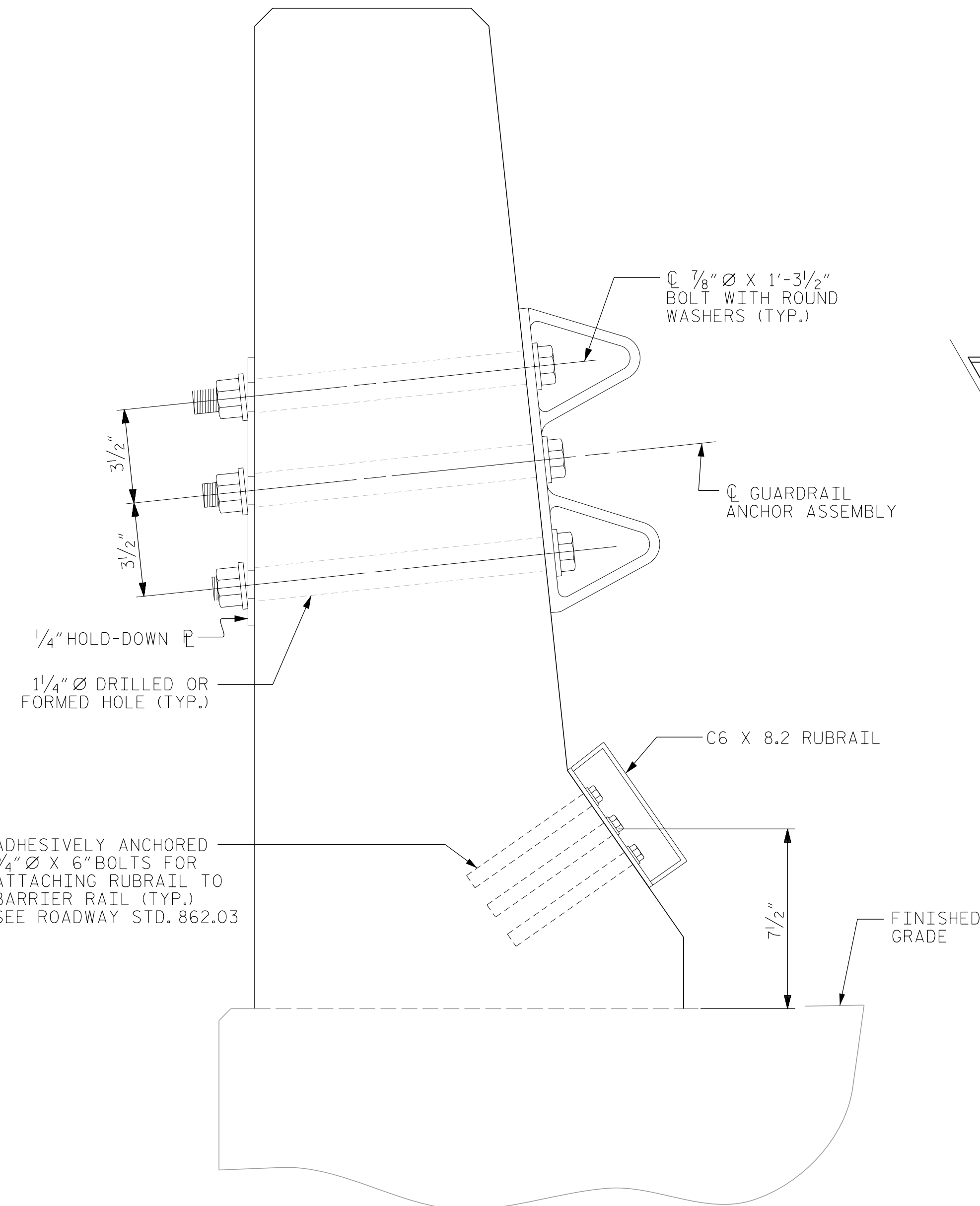
STR. #4



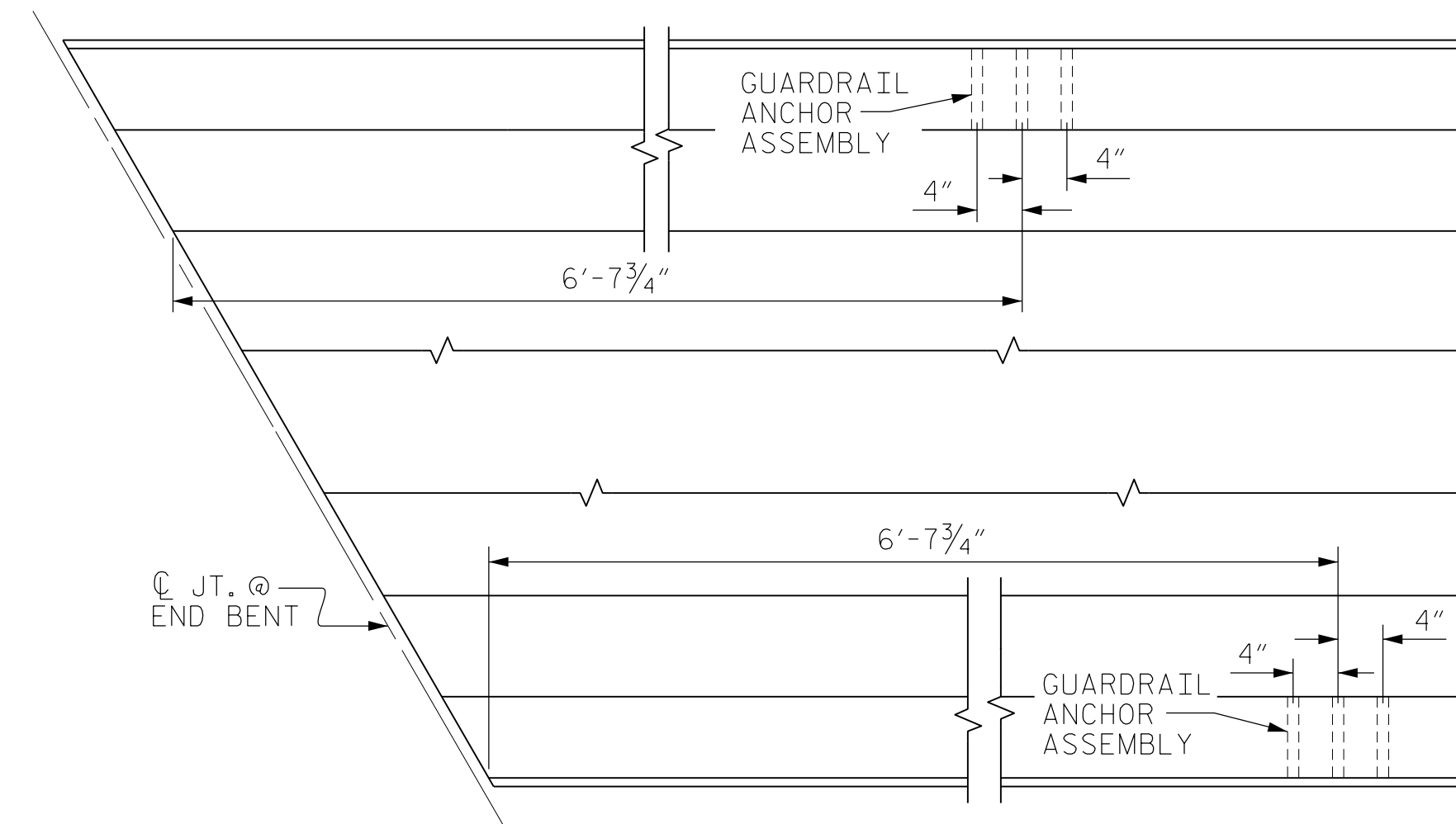
PLAN



ELEVATION



SECTION E-E
 GUARDRAIL ANCHOR ASSEMBLY DETAILS



PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

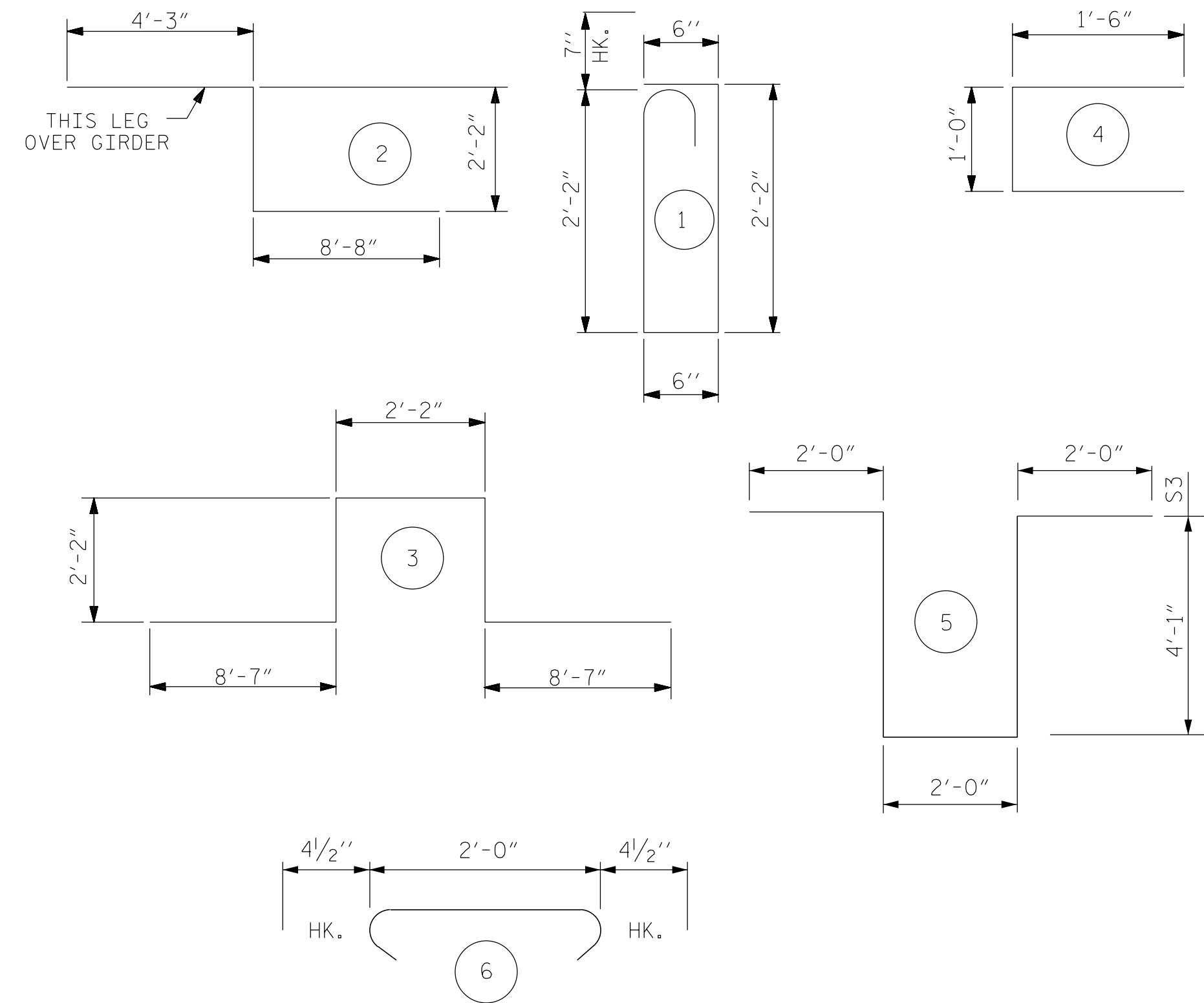
END BENT #1 SHOWN, END BENT #2 SIMILAR.

DRAWN BY: H.ASSFOURA DATE: 07/16
 CHECKED BY: J.LOFIUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: J.LOFIUS DATE: 01/17

2/6/2017
 \\404-033-R2707C-SMU-GRA1-S4-17.dgn
 USER:deFault

R 2707C-SITE 3

BAR TYPES

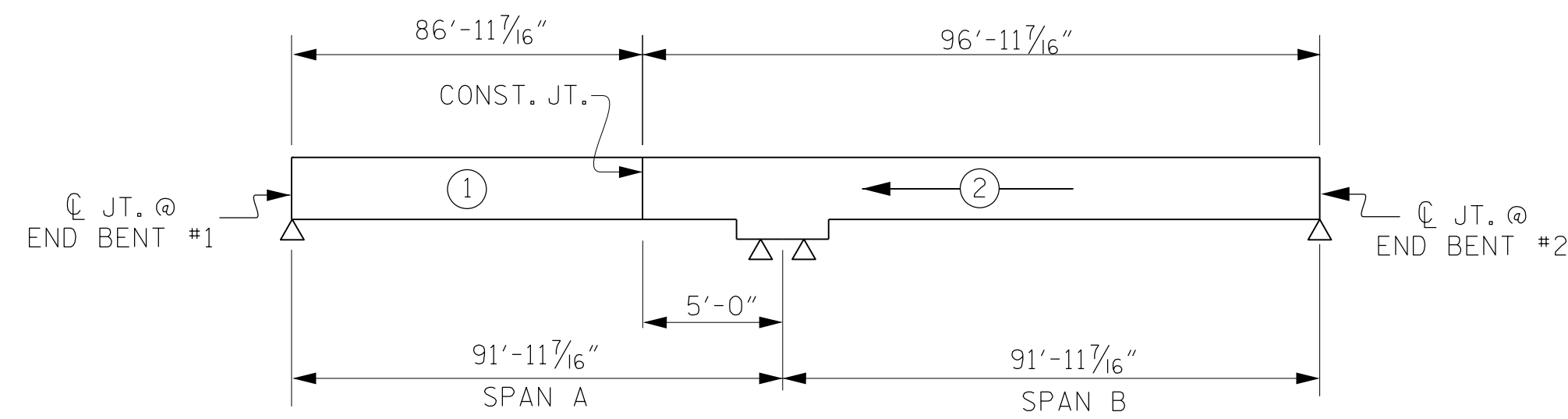


ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL

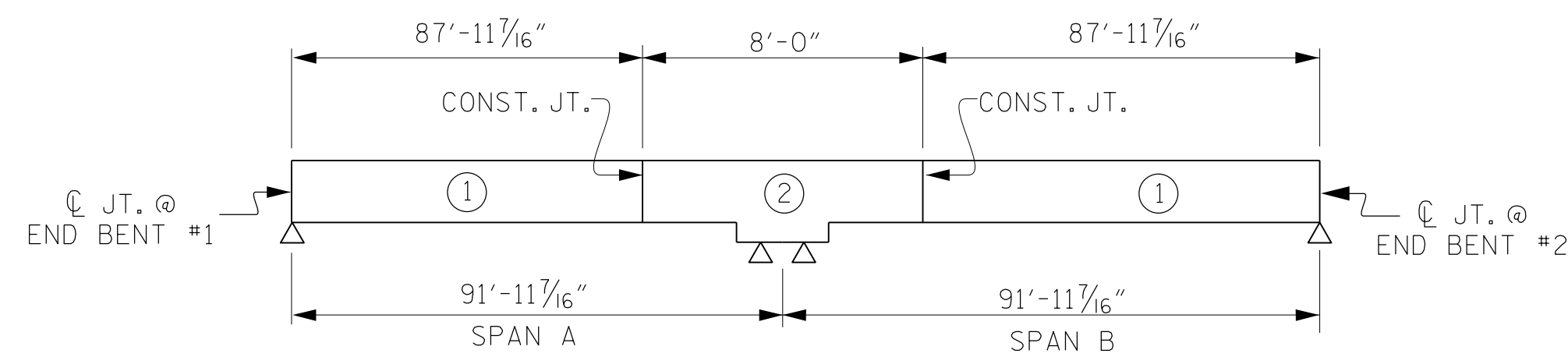
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	431	#5	STR	42'-11"	19,292	* B1	180	#4	STR	21'-11"	2,635
* A2	2	#5	STR	39'-1"	82	* B2	60	#6	STR	34'-6"	3,109
* A3	2	#5	STR	34'-11"	73	* B3	56	#6	STR	28'-0"	2,355
* A4	2	#5	STR	30'-8"	64	* B4	244	#5	STR	47'-6"	12,088
* A5	2	#5	STR	26'-5"	55						
* A6	2	#5	STR	22'-3"	46	* G1	2	#5	STR	43'-1"	90
* A7	2	#5	STR	18'-0"	38						
* A8	2	#5	STR	13'-9"	29	* K1	8	#8	2	15'-1"	322
* A9	2	#5	STR	9'-6"	20	* K2	8	#8	3	23'-8"	506
* A10	2	#5	STR	5'-4"	11	* K3	18	#6	STR	9'-11"	268
* A11	2	#5	STR	1'-1"	2	* K4	10	#4	STR	19'-1"	127
						* K5	6	#4	STR	9'-6"	38
A12	431	#5	STR	42'-11"	19,292	* K6	6	#4	STR	10'-6"	42
A13	2	#5	STR	39'-1"	82	* K7	12	#4	STR	11'-0"	88
A14	2	#5	STR	34'-11"	73	* K8	6	#4	STR	10'-0"	40
A15	2	#5	STR	30'-8"	64						
A16	2	#5	STR	26'-5"	55	* S1	66	#5	1	5'-11"	407
A17	2	#5	STR	22'-3"	46	* S2	66	#4	4	4'-0"	176
A18	2	#5	STR	18'-0"	38	* S3	6	#4	5	14'-2"	57
A19	2	#5	STR	13'-9"	29	* S4	24	#4	5	15'-10"	254
A20	2	#5	STR	9'-6"	20	* S5	120	#4	6	2'-9"	220
A21	2	#5	STR	5'-4"	11						
A22	2	#5	STR	1'-1"	2						

REINFORCING STEEL 32,666 LBS.
*EPOXY REINFORCING STEEL 29,580 LBS.



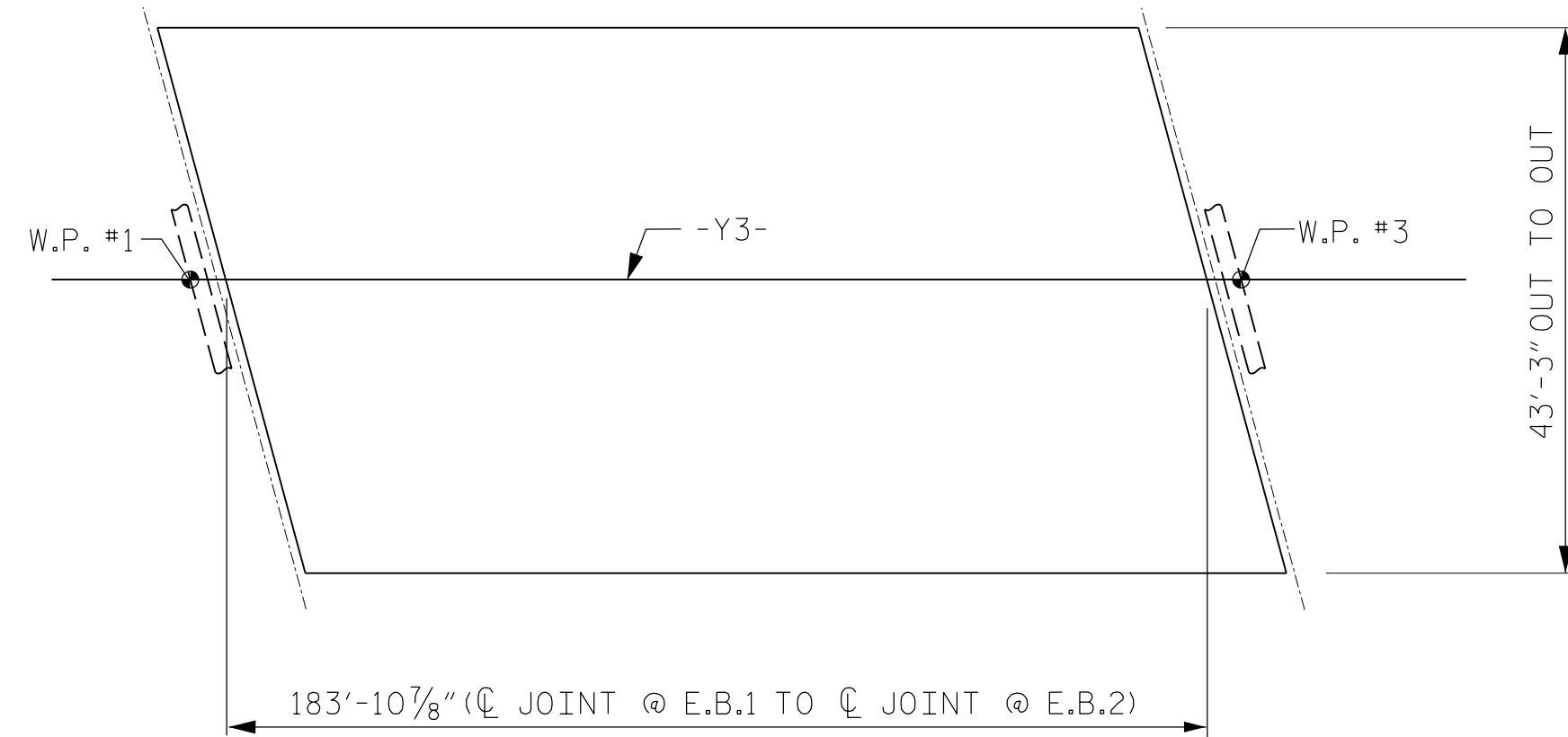
POURING SEQUENCE

⊕ = INDICATES POUR NUMBER AND DIRECTION OF POUR



OPTIONAL POURING SEQUENCE

⊕ = INDICATES POUR NUMBER



LAYOUT OF COMPUTED AREA REINFORCED CONCRETE DECK SLAB (SQ.FT = 7,954)

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS

BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET, AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER RAIL
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3'-5"
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"
#7	5'-3"	3'-6"			
#8	6'-10"	4'-7"			

GROOVING BRIDGE FLOORS

APPROACH SLABS	1,769	SQ.FT.
BRIDGE DECK	6,789	SQ.FT.
TOTAL	8,558	SQ.FT.

SUPERSTRUCTURE BILL OF MATERIAL

	CLASS AA CONCRETE (CU. YDS.)	REINFORCING STEEL (LBS.)	EPOXY COATED REINFORCING STEEL (LBS.)
POUR 1	126.0		
POUR 2	140.5		
TOTALS**	266.5	32,666	29,580

**QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

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CLEVELAND COUNTY
STATION: 20+70.23-Y3- POT



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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
BILL OF MATERIAL

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			S4-18
2			4			TOTAL SHEETS 29

STR. #4

DRAWN BY: H.ASSFOURA DATE: 07/16
CHECKED BY: J.LOFIUS DATE: 11/16
DESIGN ENGINEER OF RECORD: J.LOFIUS DATE: 01/17

R 2707C-SITE 3
2/6/2017
\\404-035-R2707C-SMU-B0M1-S4-18.dgn
USERdefault

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

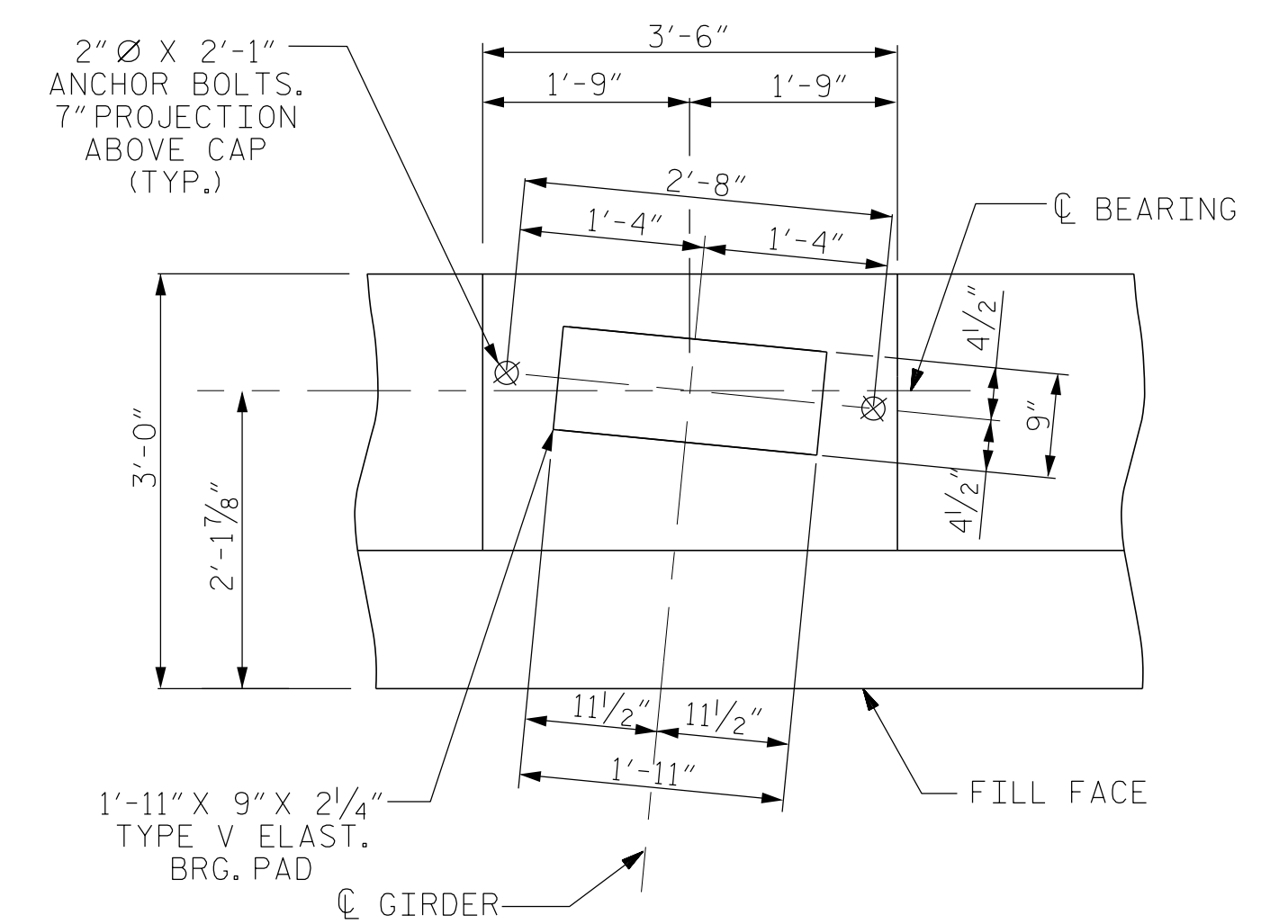
FOR BEARING DETAILS, SEE ELASTOMERIC BEARING DETAILS SHEET.

FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.

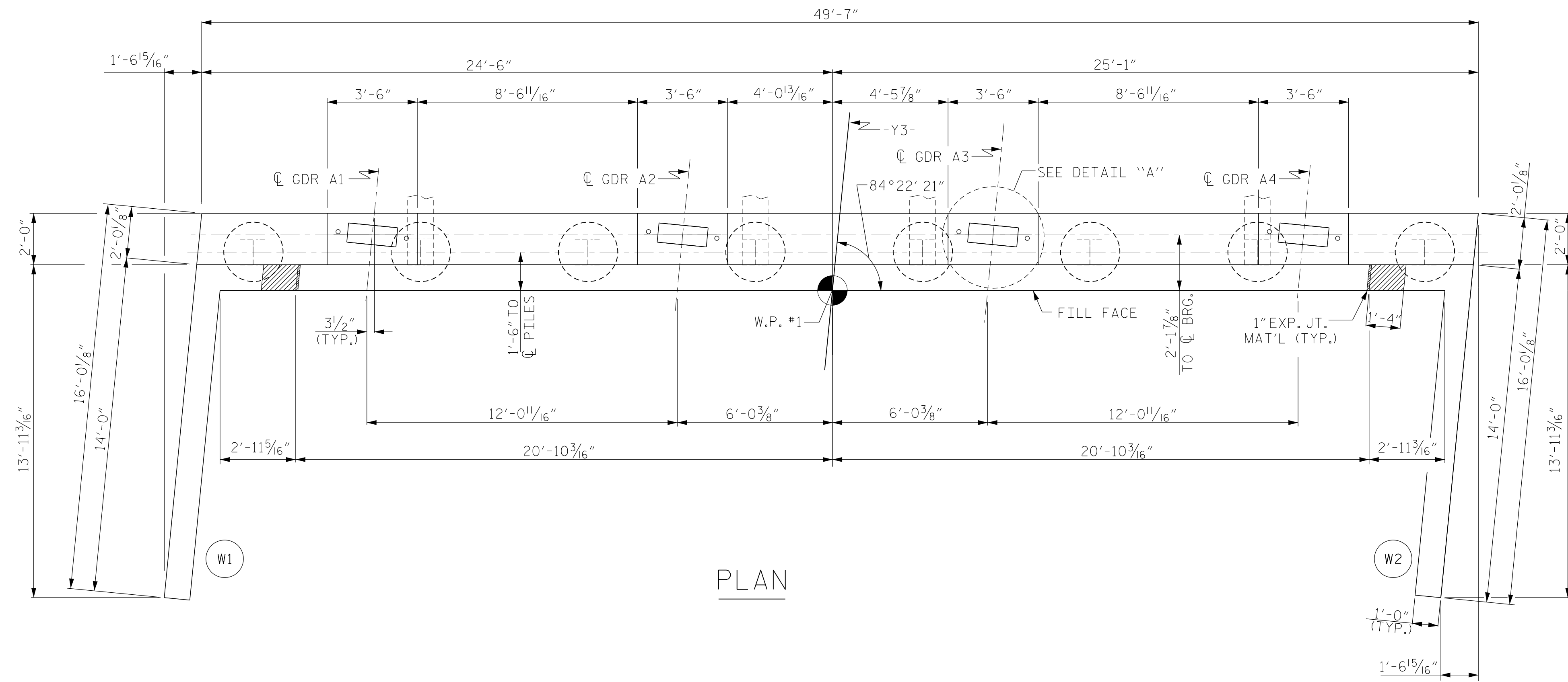
BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING.

THE TOP SURFACE AREAS OF THE END BENT CAPS SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.

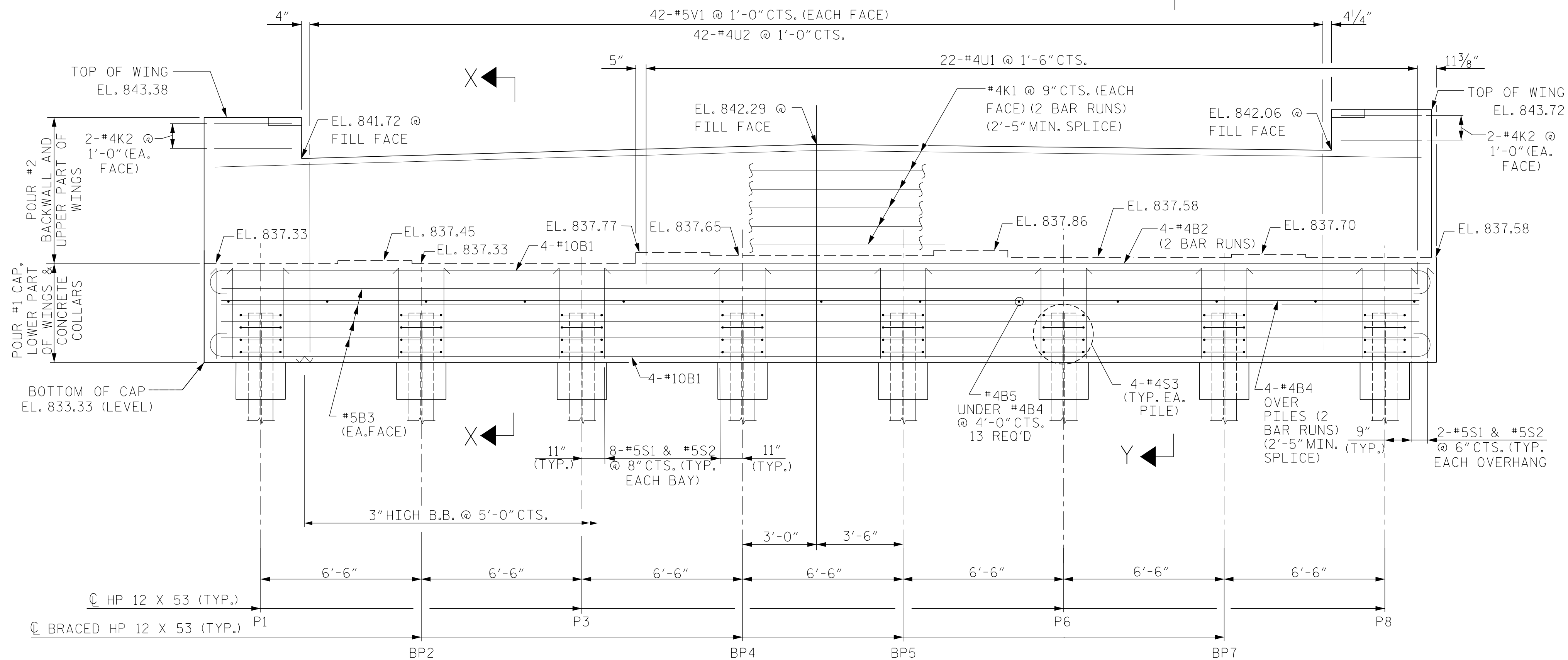
INSTALL THE 4" DIA. DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.



DETAIL "A"



PLAN



ELEVATION

WINGS NOT SHOWN FOR CLARITY.
 FOR SECTIONS X-X & Y-Y, SEE SHEET 3 OF 3.
 CONCRETE COLLARS FOR STEEL PILES NOT SHOWN IN PLAN VIEW FOR CLARITY.
 SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL", END BENT No.1 SHEET 3 OF 3.

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT

SHEET 1 OF 3



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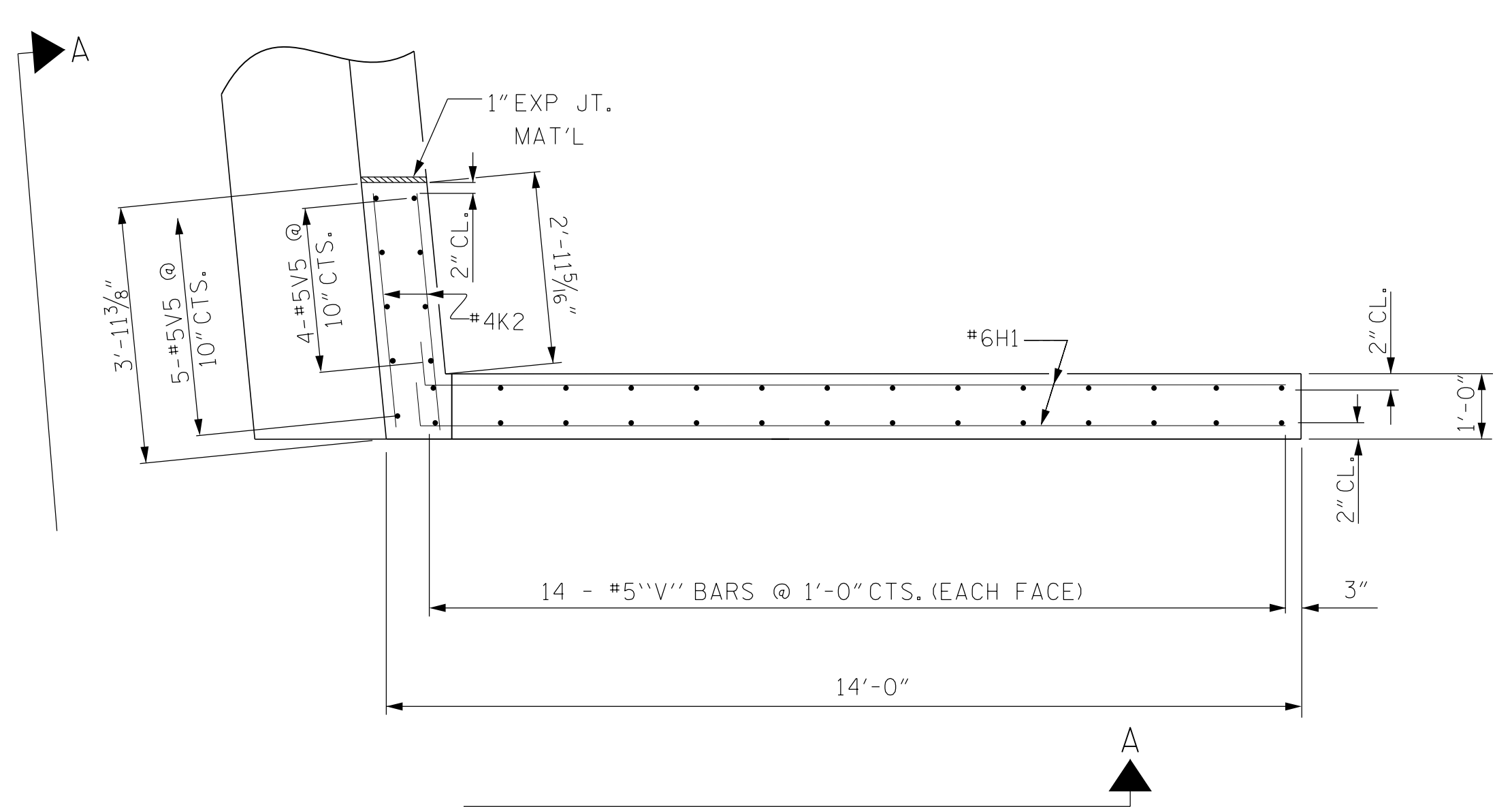
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 END BENT NO. 1

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-19
1			3			TOTAL SHEETS
2			4			29

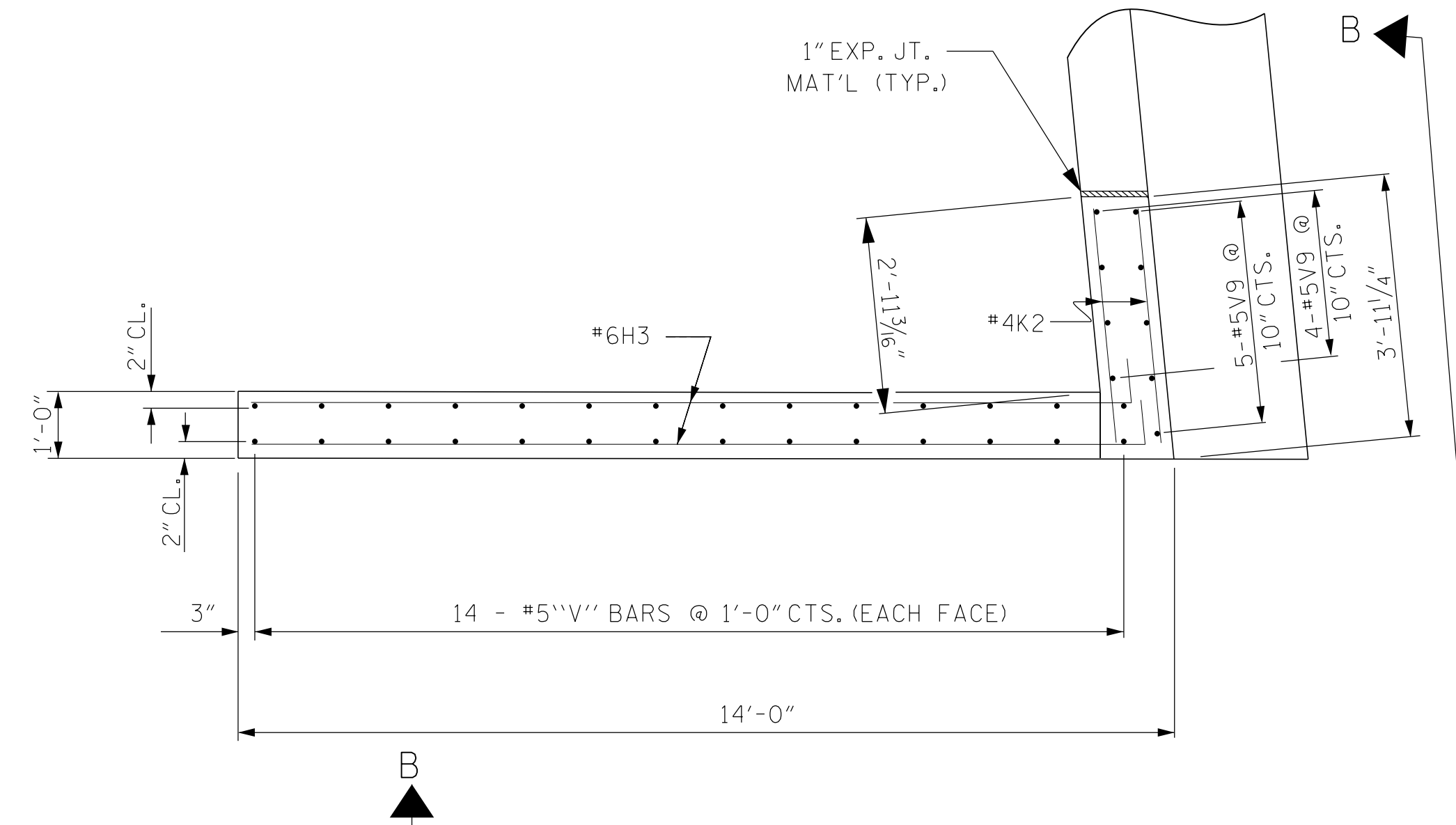
STR. #4

DRAWN BY: HASSFOURA DATE: 08/16
 CHECKED BY: JLOFTUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: JLOFTUS DATE: 01/17

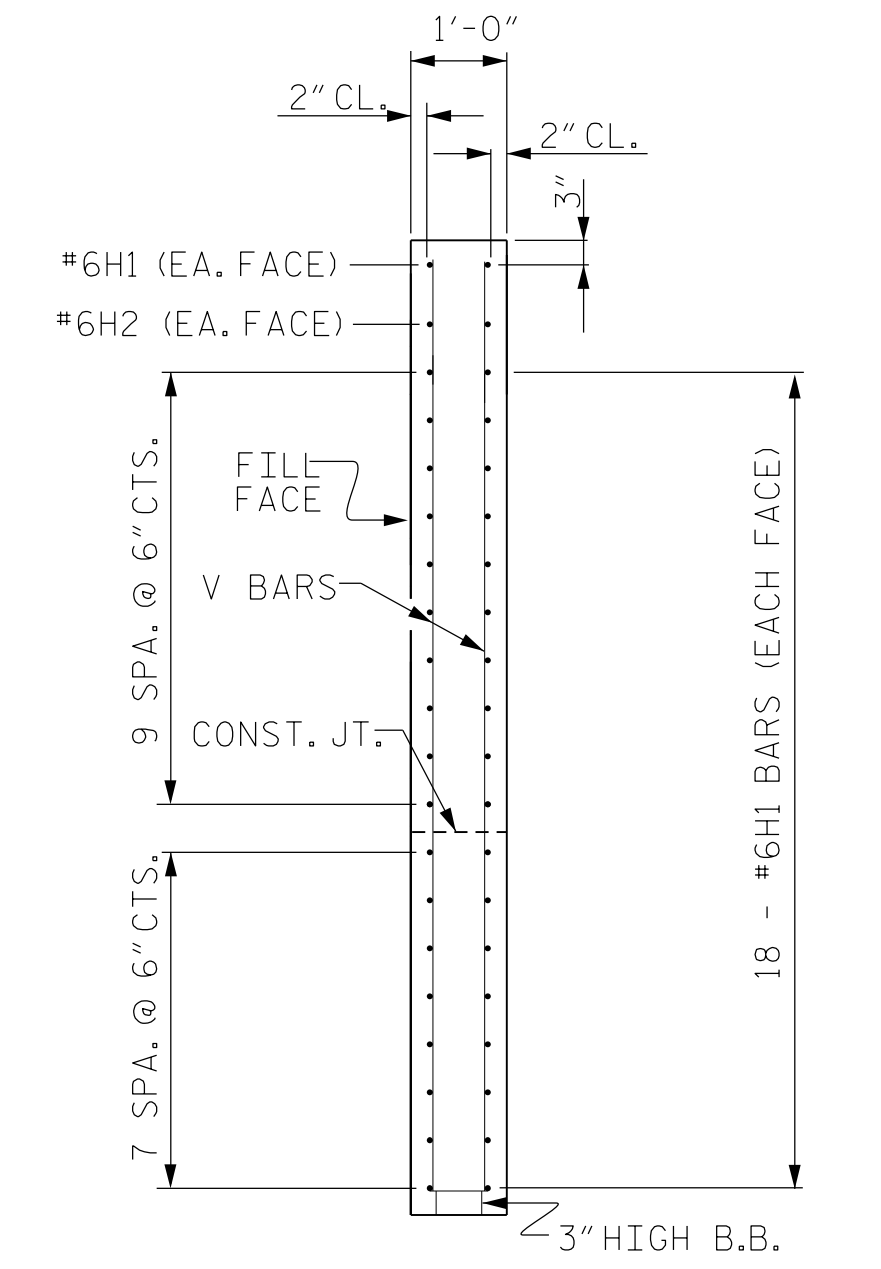
R 2707C-SITE 3
 2/6/2017
 \\404-037-R2707C-SMU-1EB1-S4-19.dgn
 USER:JLofTus



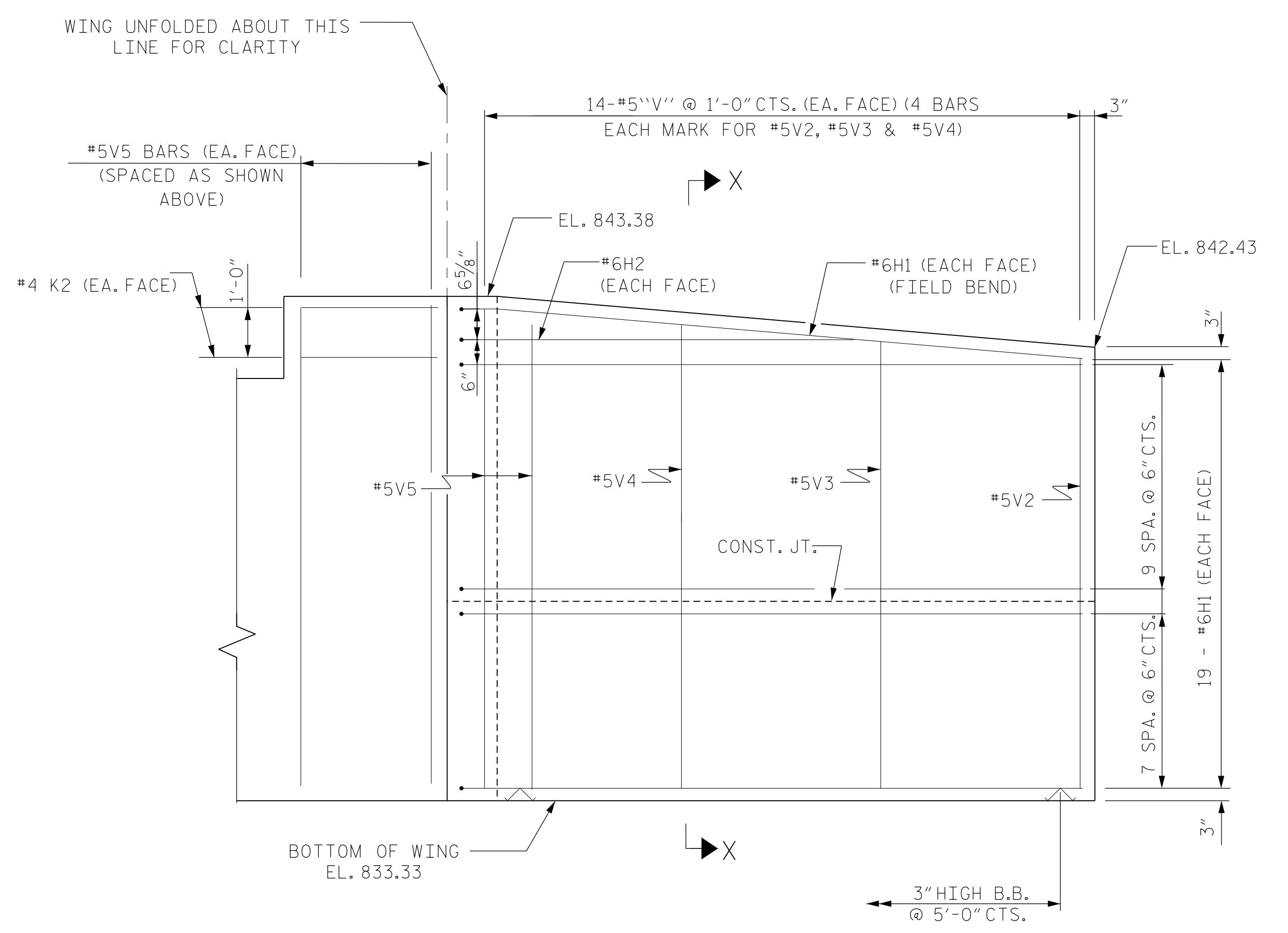
PLAN OF WING (W1)



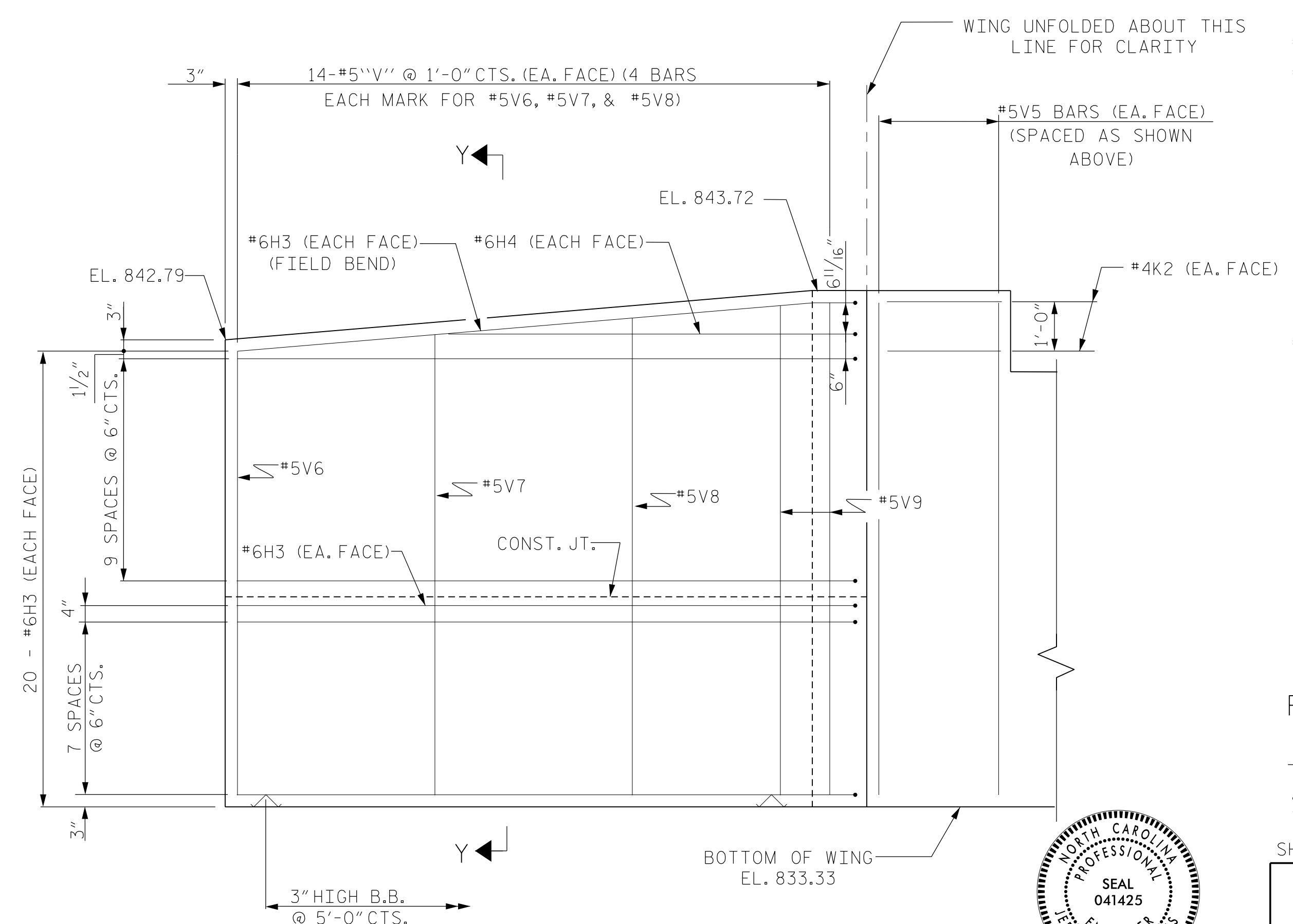
PLAN OF WING (W2)



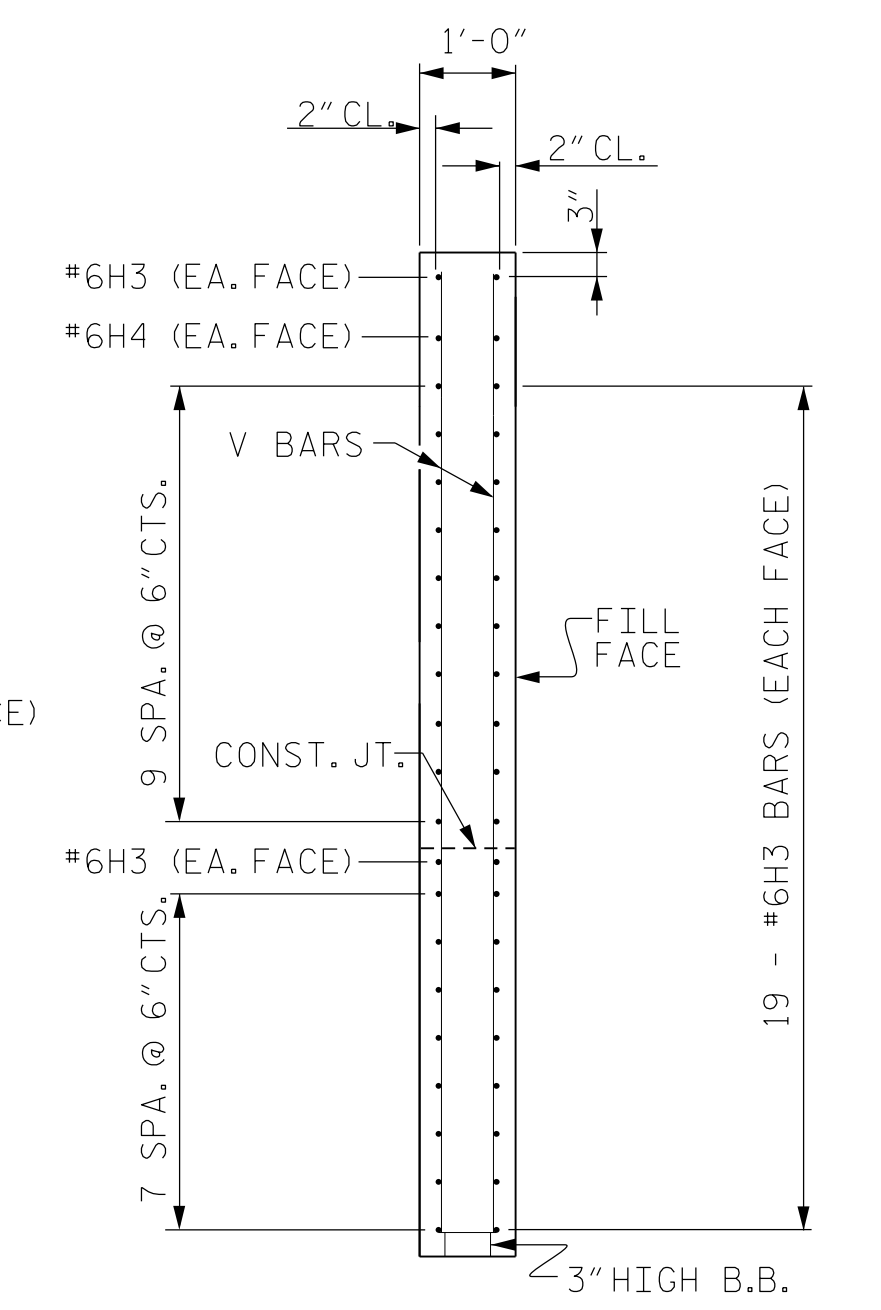
SECTION X-X



VIEW A-A



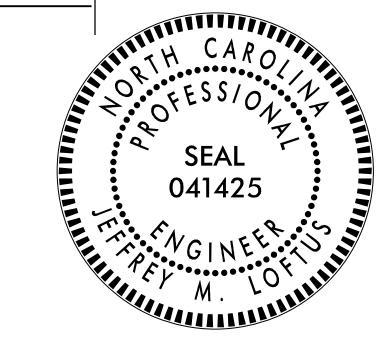
VIEW B-B



SECTION Y-Y

PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT

SHEET 3 OF 3



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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 END BENT NO.1
 WING DETAILS

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S4-20
1			3			TOTAL SHEETS
2			4			29

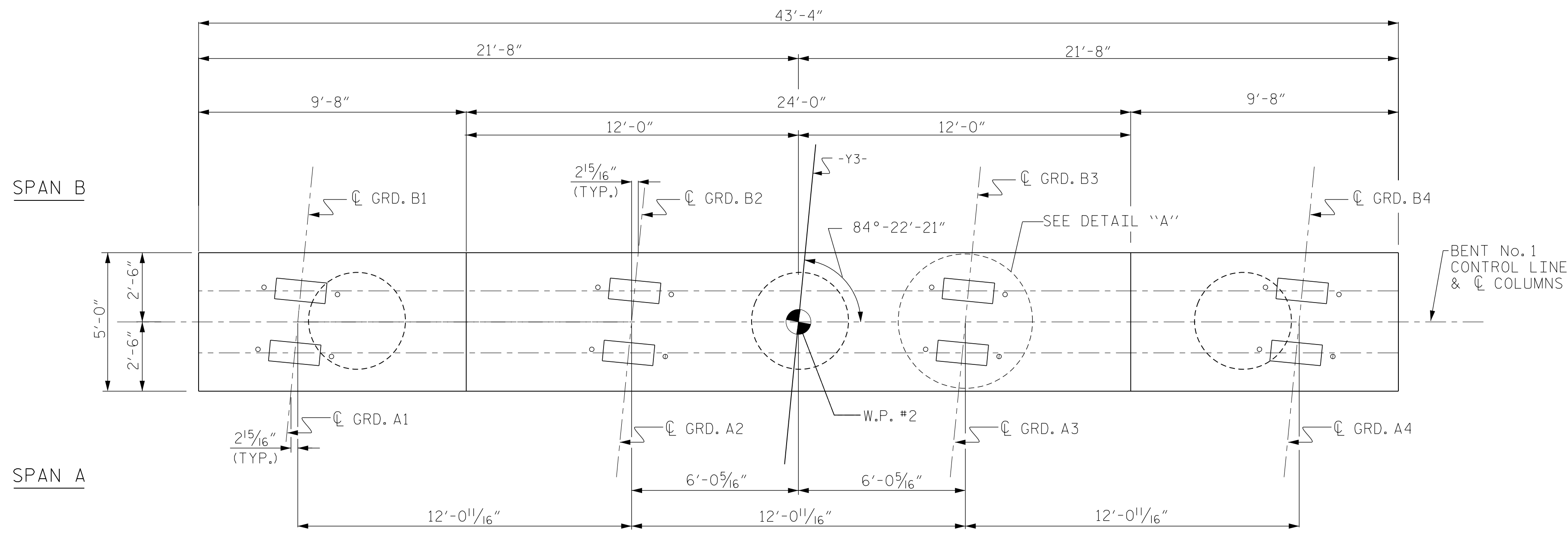
STR. #4

DRAWN BY: H.ASSFOURA DATE: 08/16
 CHECKED BY: J.LOFIUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: J.LOFIUS DATE: 01/17

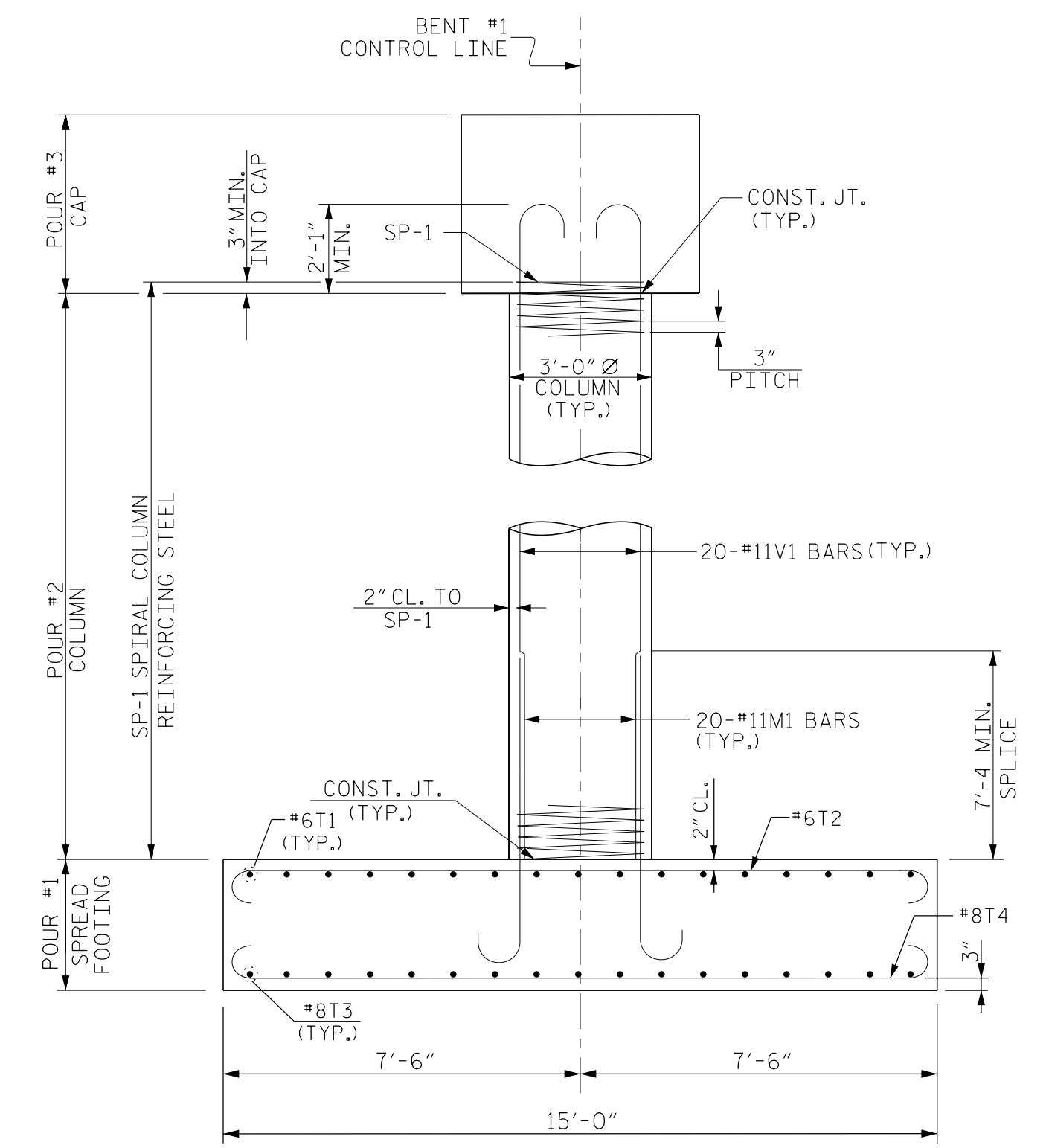
R 2707C-SITE 3
 2/6/2017
 \\V04-039-R2707C-SMU-1EB2-S4-20.dgn
 USER:JeffCapus

NOTES

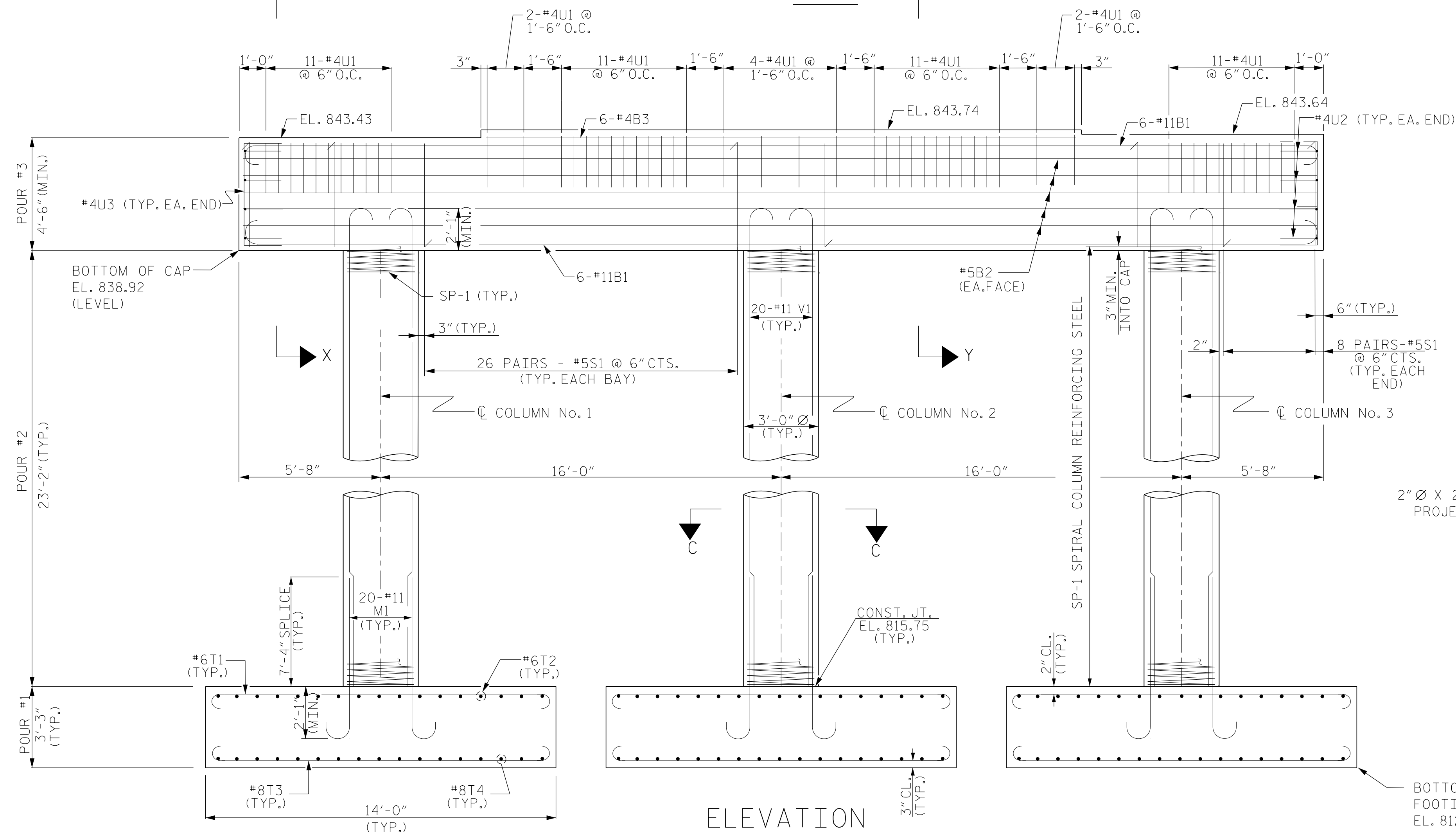
STIRRUPS TO BE PLACED VERTICALLY AND INVERTED ALTERNATELY.
 STIRRUPS AND "U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.
 HOOKS ON "V" AND "M" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.



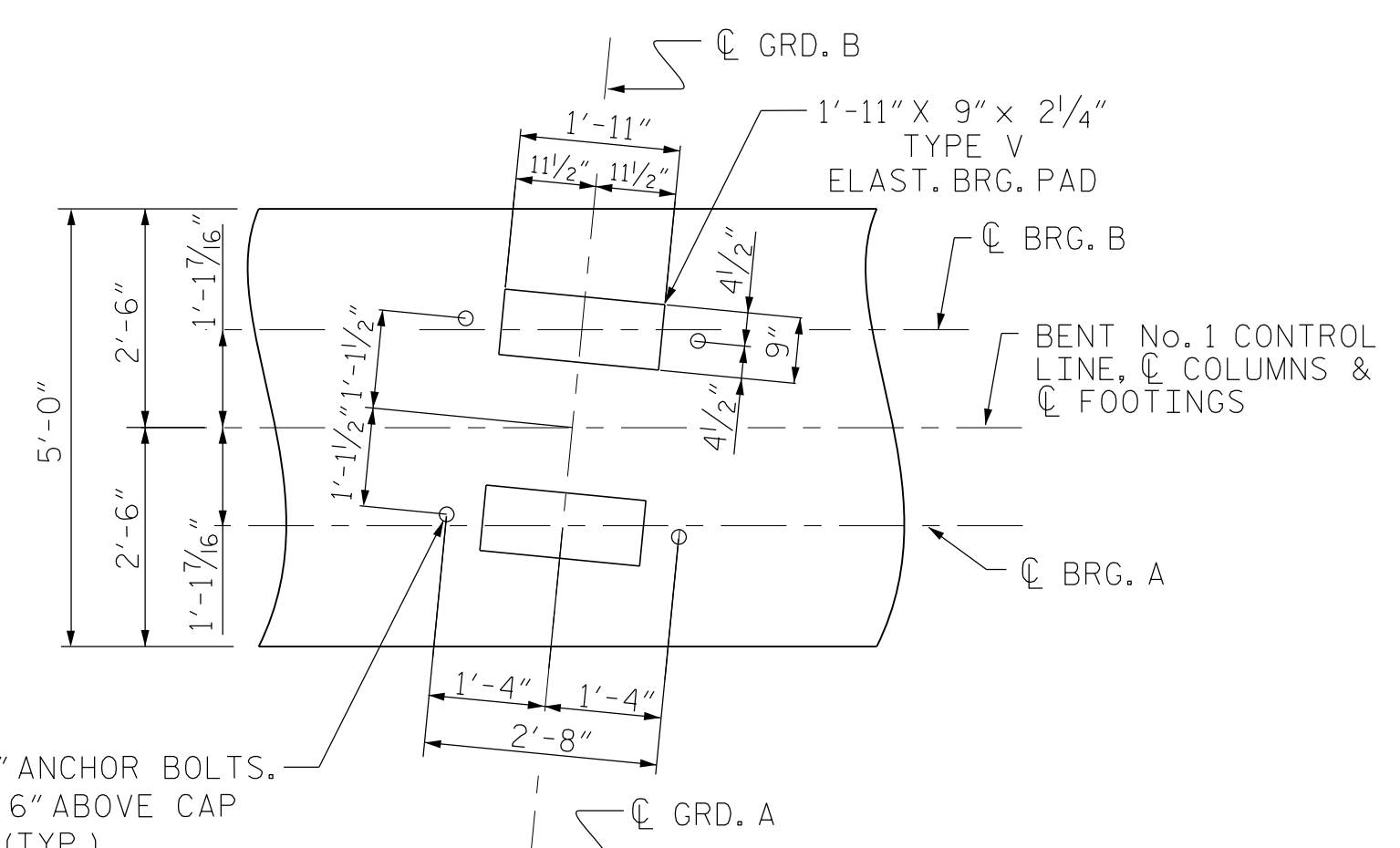
PLAN



END ELEVATION



ELEVATION

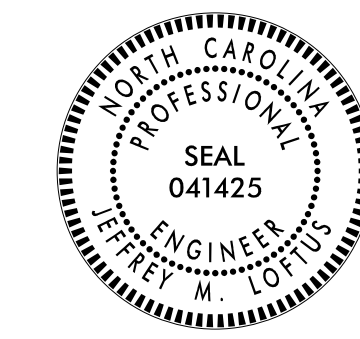


DETAIL "A"

(DIMENSIONS ARE TYP. FOR EACH GIRDER)
 (COLUMNS NOT SHOWN FOR CLARITY)

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT

SHEET 1 OF 2



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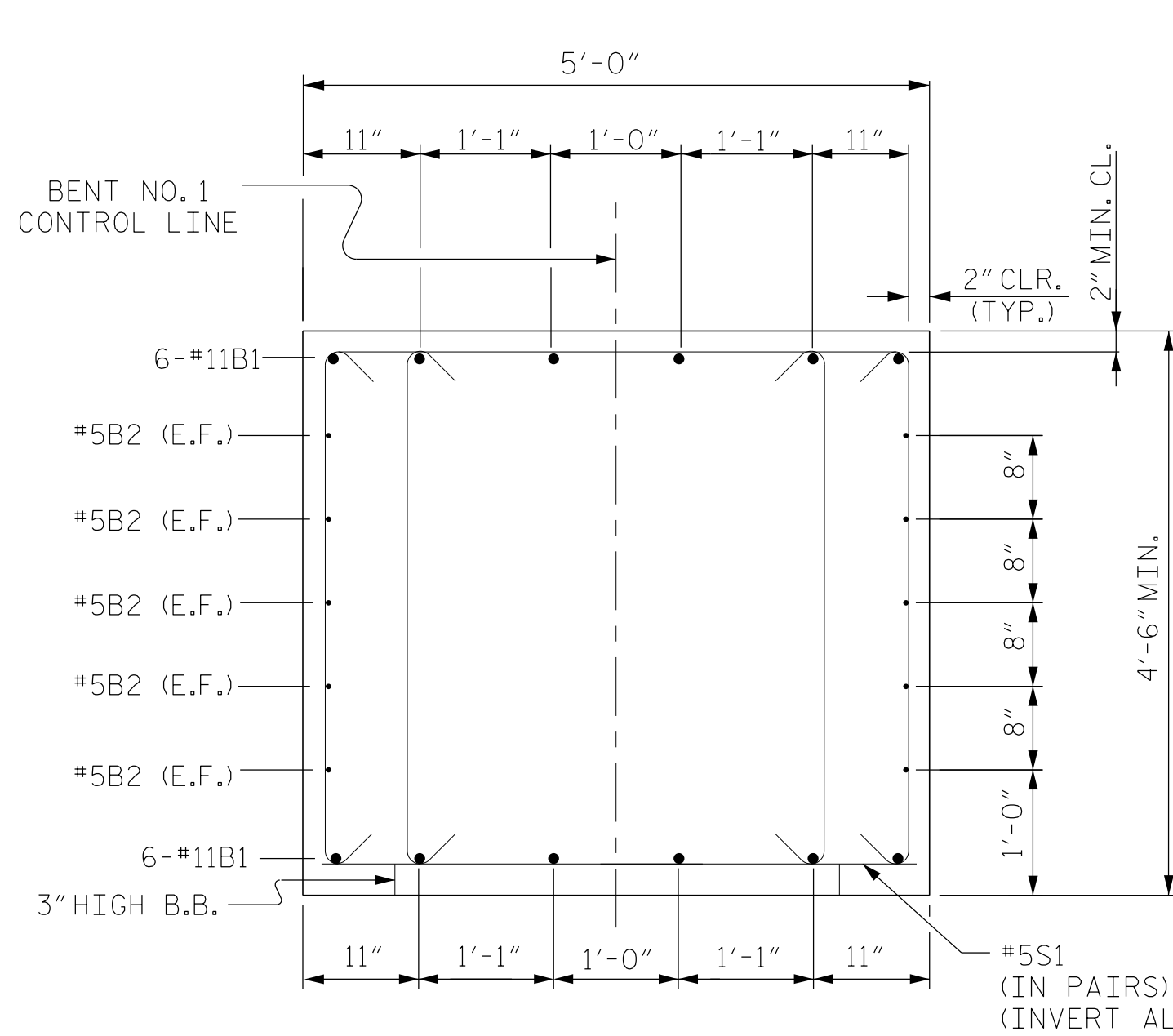


STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE BENT NO. 1					
SHEET NO. S4-22					
TOTAL SHEETS 29					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

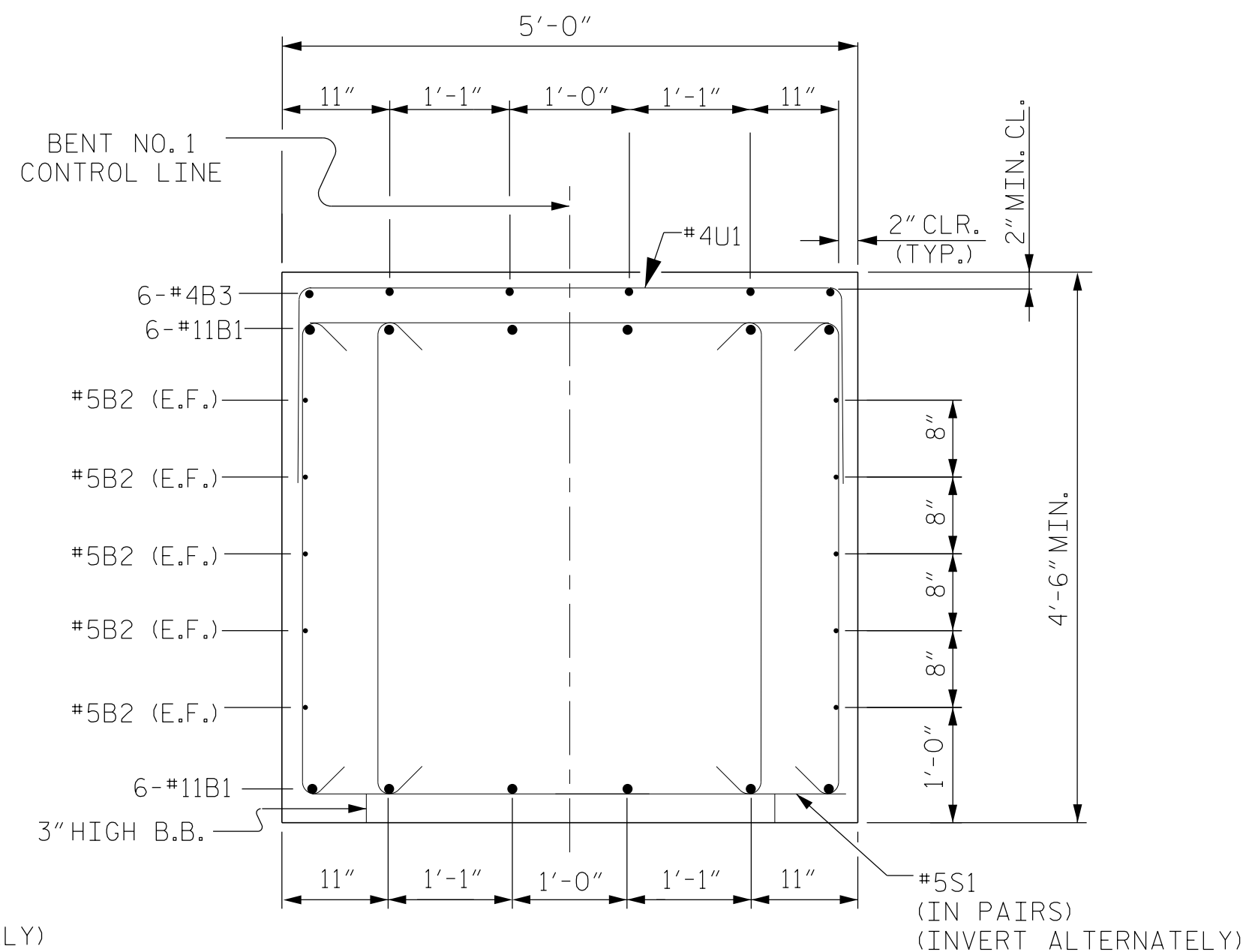
STR. #4

DRAWN BY: H.ASSFOURA DATE: 10/16
 CHECKED BY: J.LOFIUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: J.LOFIUS DATE: 01/17

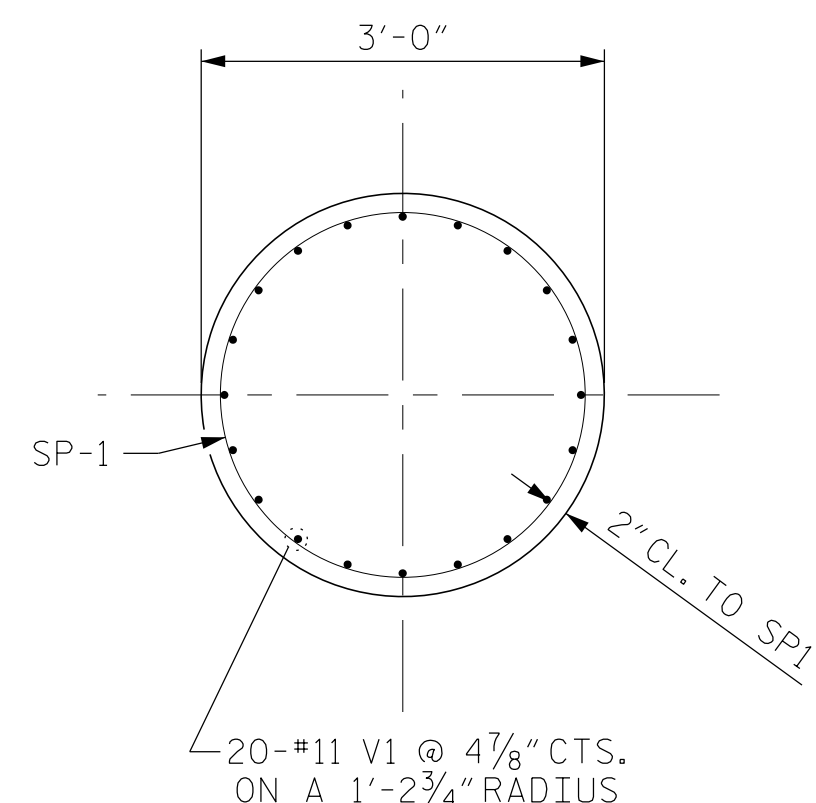
R-2707C-SITE 3
 2/6/2017
 \\404-043-R2707C-SMU-BENT1-S4-22.dgn
 USER:deFault



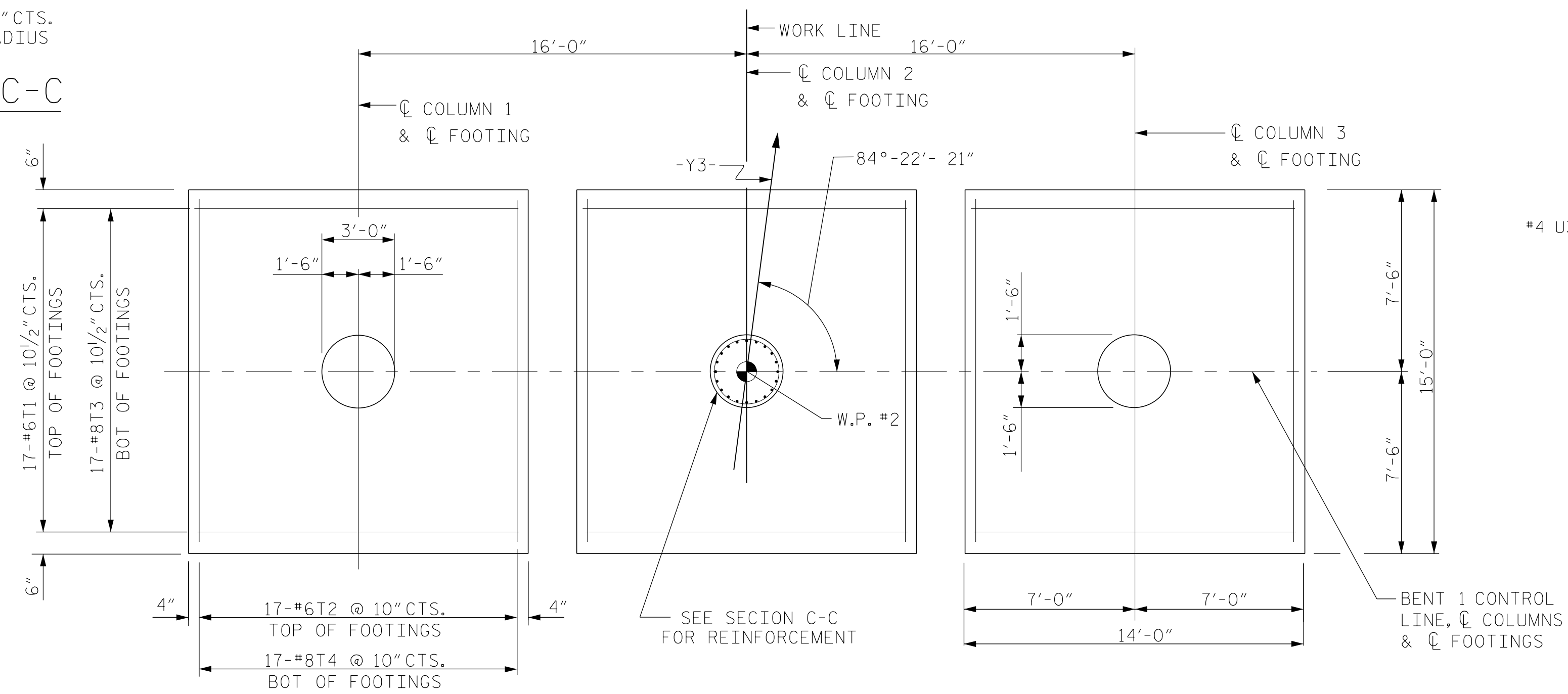
SECTION X-X



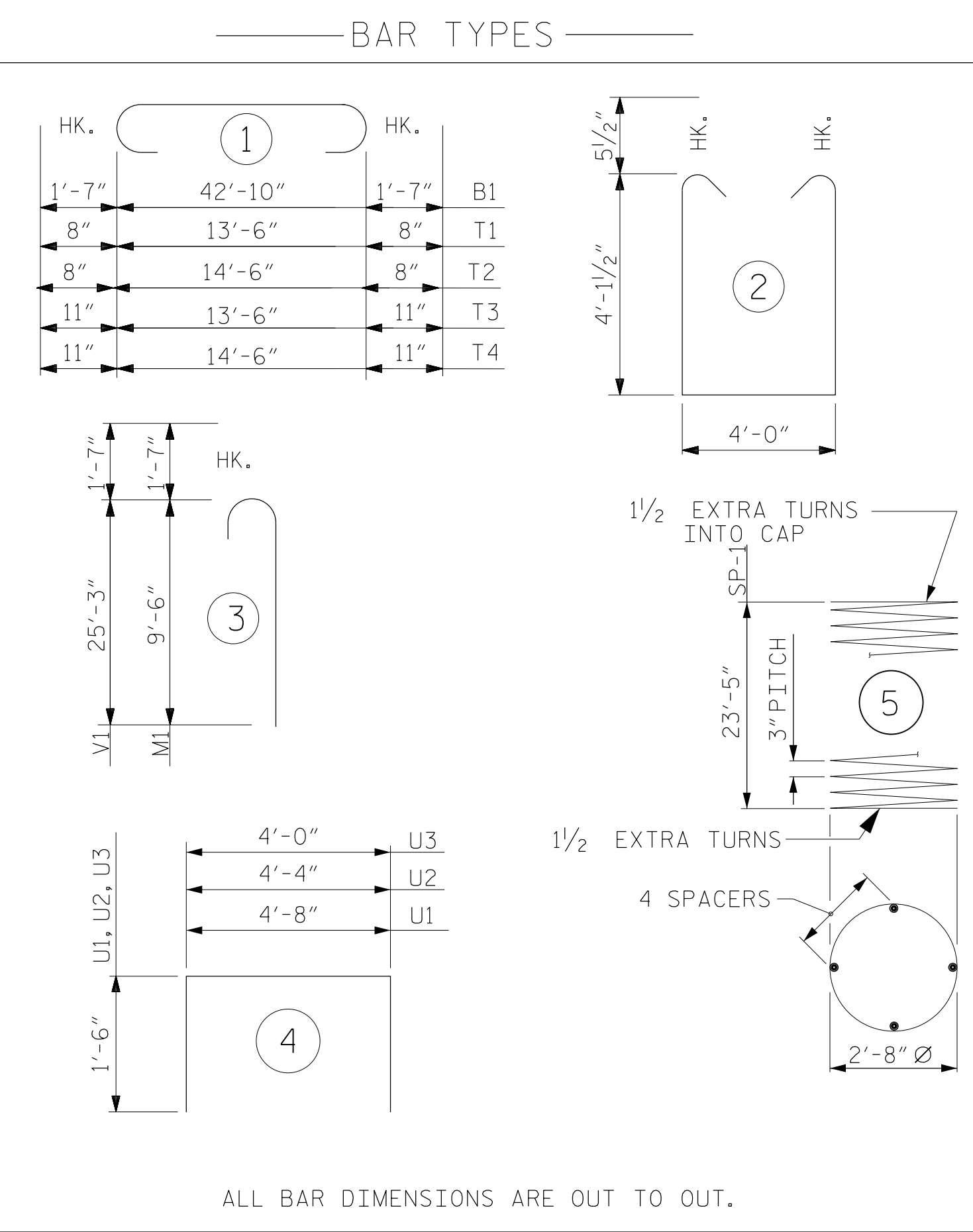
SECTION Y-Y



SECTION C-C

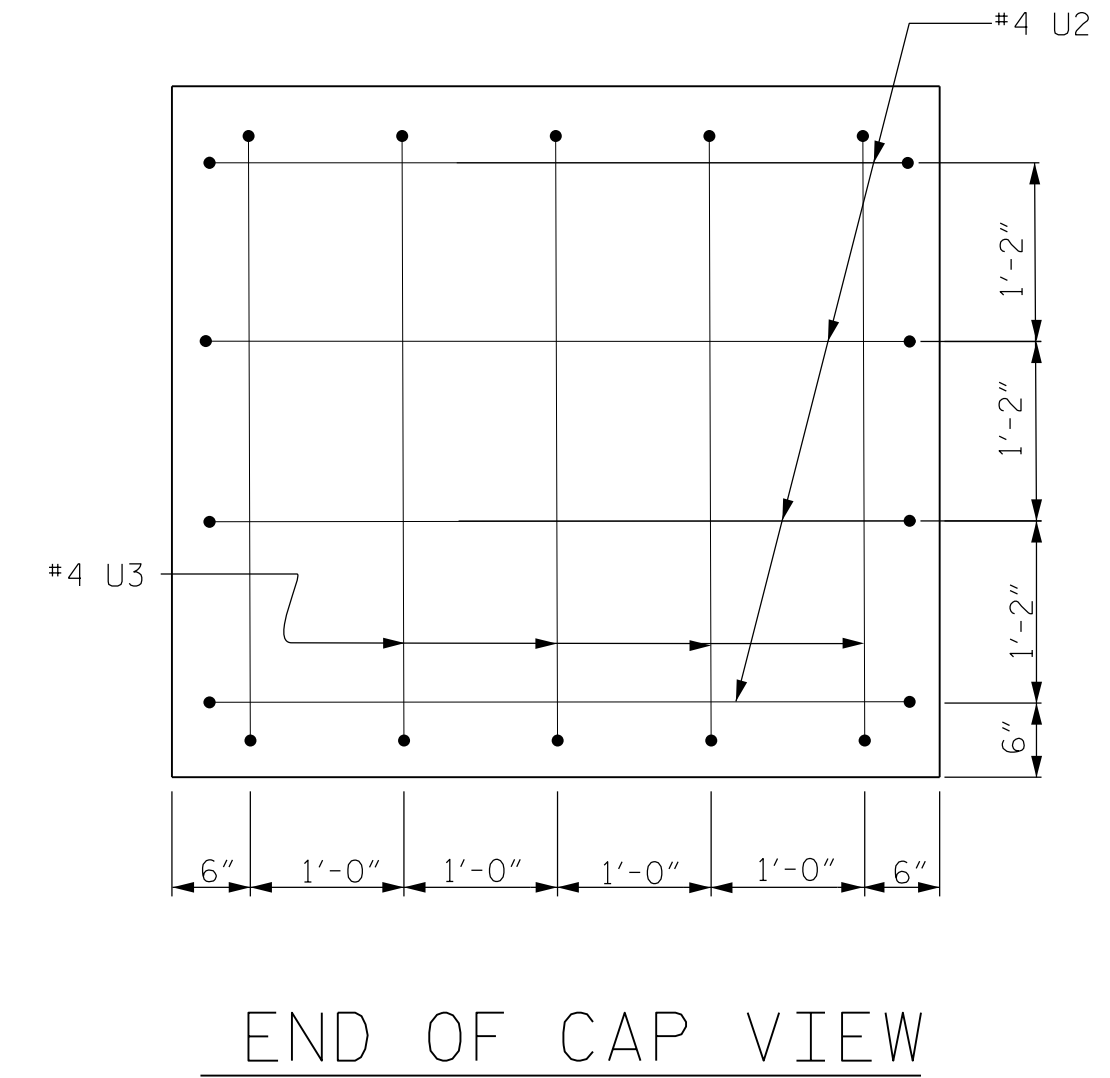


PLAN OF FOOTINGS



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL FOR BENT NO. 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	12	#11		46'-0"	2,933
B2	10	#5	STR	43'-0"	448
B3	6	#4	STR	23'-7"	95
M1	60	#11	3	11'-1"	3,533
S1	136	#5	2	13'-2"	1,868
T1	51	#6	1	14'-10"	1,136
T2	51	#6	1	15'-10"	1,213
T3	51	#8	1	15'-4"	2,088
T4	51	#8	1	16'-4"	2,224
U1	52	#4	4	7'-8"	266
U2	8	#4	4	7'-4"	39
U3	10	#4	4	7'-0"	47
V1	60	#11	3	26'-10"	8,554
REINFORCING STEEL (BENT NO. 1)					24,444 LBS.
SP-1	3	*	5	797'-8"	1,599
SPIRAL COLUMN REINFORCING STEEL (BENT NO. 1)					1,599 LBS.
* THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR					
CLASS A CONCRETE BREAKDOWN (FOR ONE BENT)					
POUR #1 (FOOTINGS)					75.8 C.Y.
POUR #2 (COLUMNS)					18.2 C.Y.
POUR #3 (CAP)					37.9 C.Y.
TOTAL CLASS A CONCRETE					131.9 C.Y.
FOUNDATION EXCAVATION					LUMP SUM



END OF CAP VIEW



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PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 20+70.23-Y3- POT
SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE BENT NO. 1					
REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					S4-23
					TOTAL SHEETS 29

STR. #4

R 2707C-SITE 3

2/6/2017
...\\04_045_R2707C_SMU_BENT1_S4-23.dgn
USER:JeffCapus

DRAWN BY: H.ASSFOURA DATE: 10/16
CHECKED BY: J.LOFIUS DATE: 11/16
DESIGN ENGINEER OF RECORD: J.LOFIUS DATE: 01/17

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

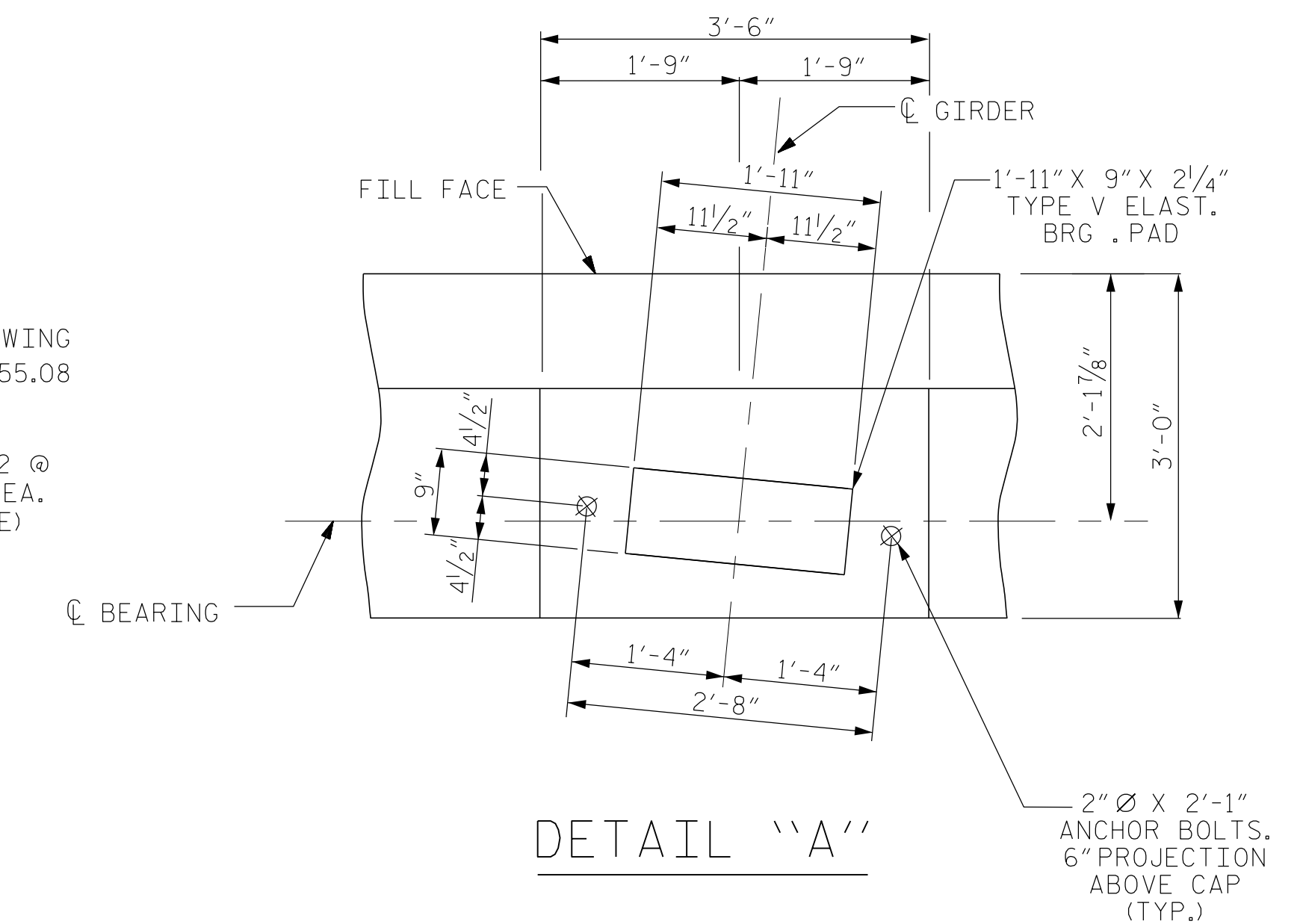
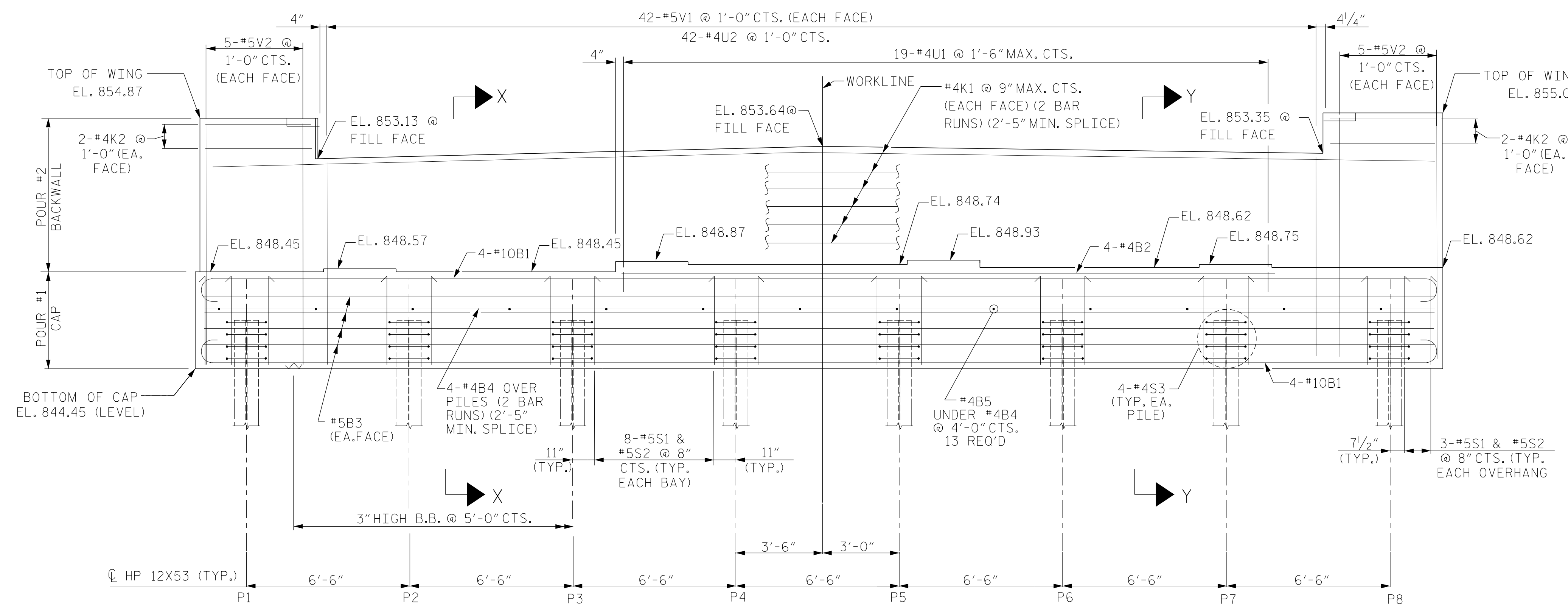
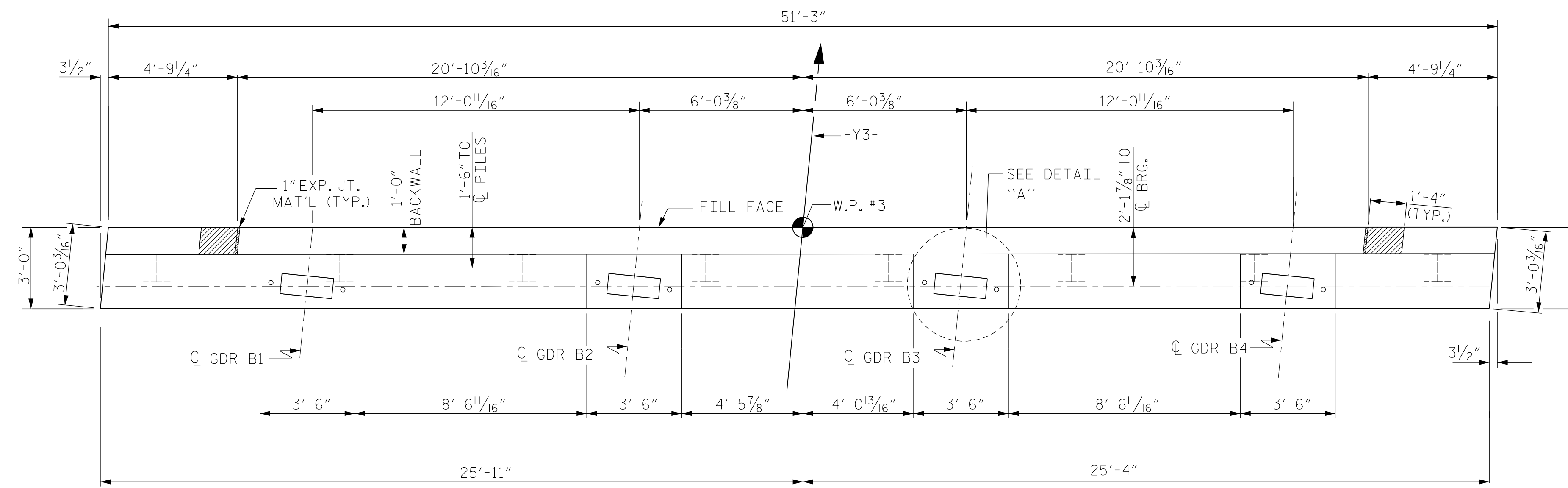
THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR BEARING DETAILS, SEE ELASTOMERIC BEARING DETAILS SHEET.

FOR PILE SPLICE DETAILS, SEE SHEET 2 OF 2.

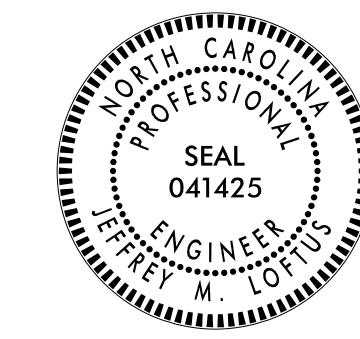
BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING.

THE TOP SURFACE AREAS OF THE END BENT CAPS SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.



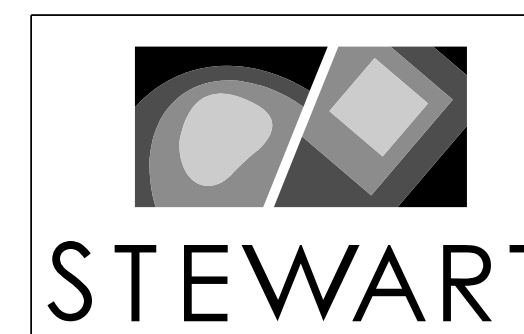
PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT

SHEET 1 OF 2



DocuSigned by:
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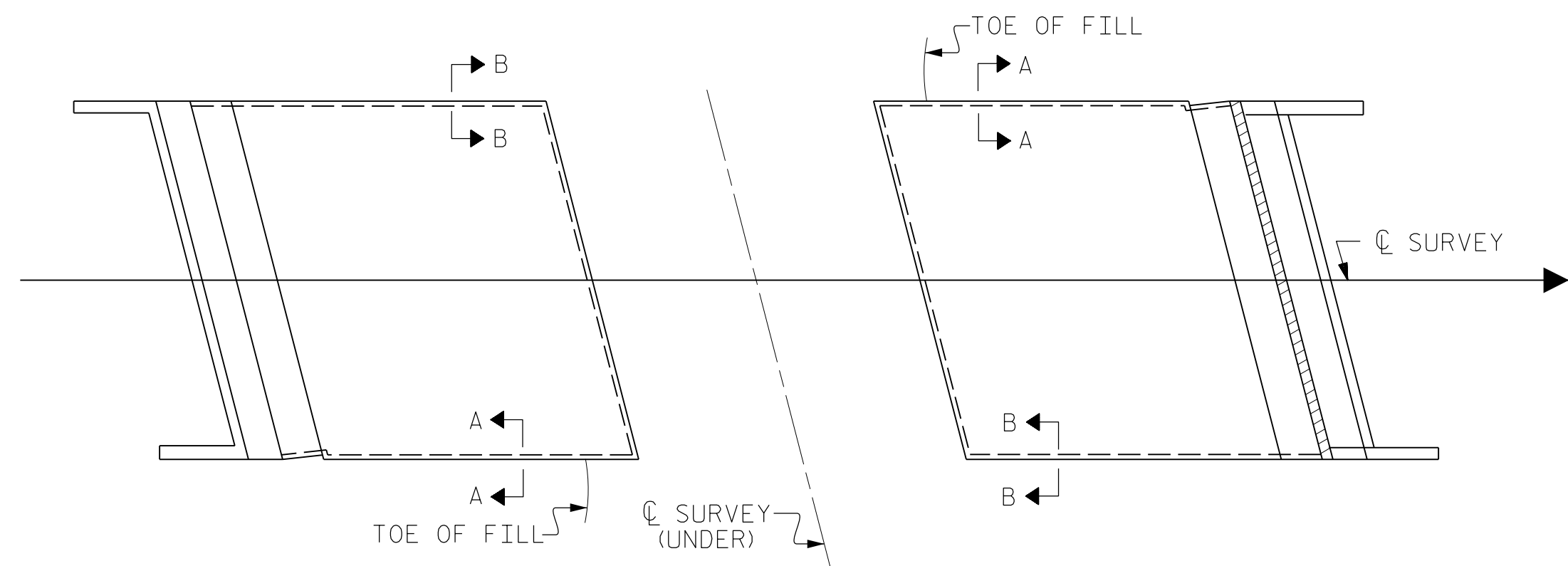


STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE END BENT NO. 2					
SHEET NO. S4-24					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS 29

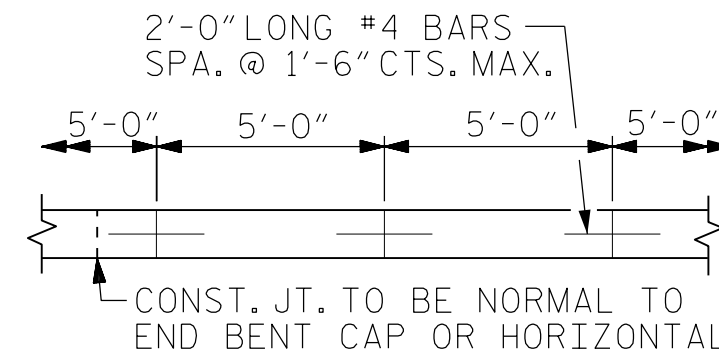
STR. #4

DRAWN BY: HASSFOURA DATE: 09/16
 CHECKED BY: JLOFTUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: JLOFTUS DATE: 01/17

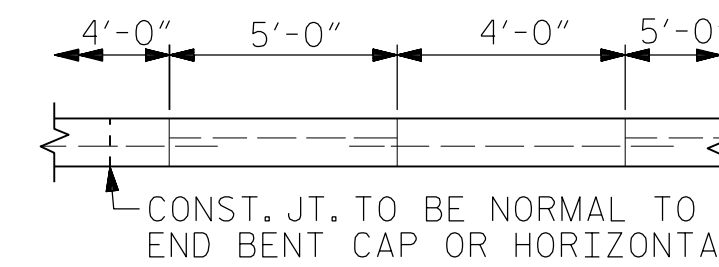
R-2707C-SITE 3
 2/6/2017
 \\V04-047-R2707C-SMU-2EB1-S4-24.dgn
 USER:deFault



PLAN



POURING DETAIL



OPTIONAL POURING DETAIL

GENERAL NOTES

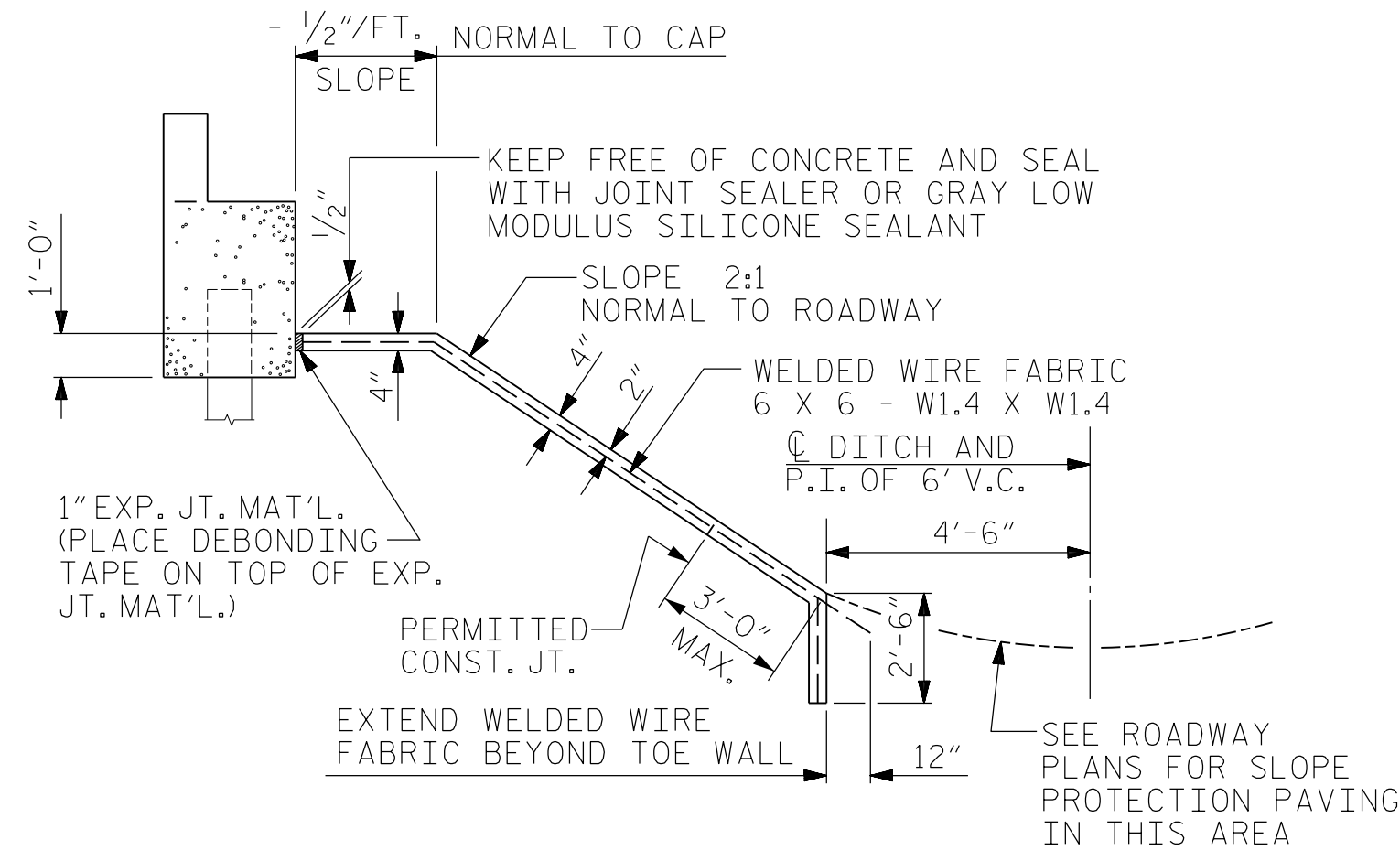
SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS. THE CONTRACTOR, AT HIS OPTION, MAY USE ALTERNATE "B" ONLY FOR HIGHWAY OVER HIGHWAY GRADE SEPARATIONS WITH 2:1 END BENT SLOPE IN RURAL, UNPOPULATED AREAS. STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS. FOR BERM WIDTH, SEE GENERAL DRAWING.

ALTERNATE "A"

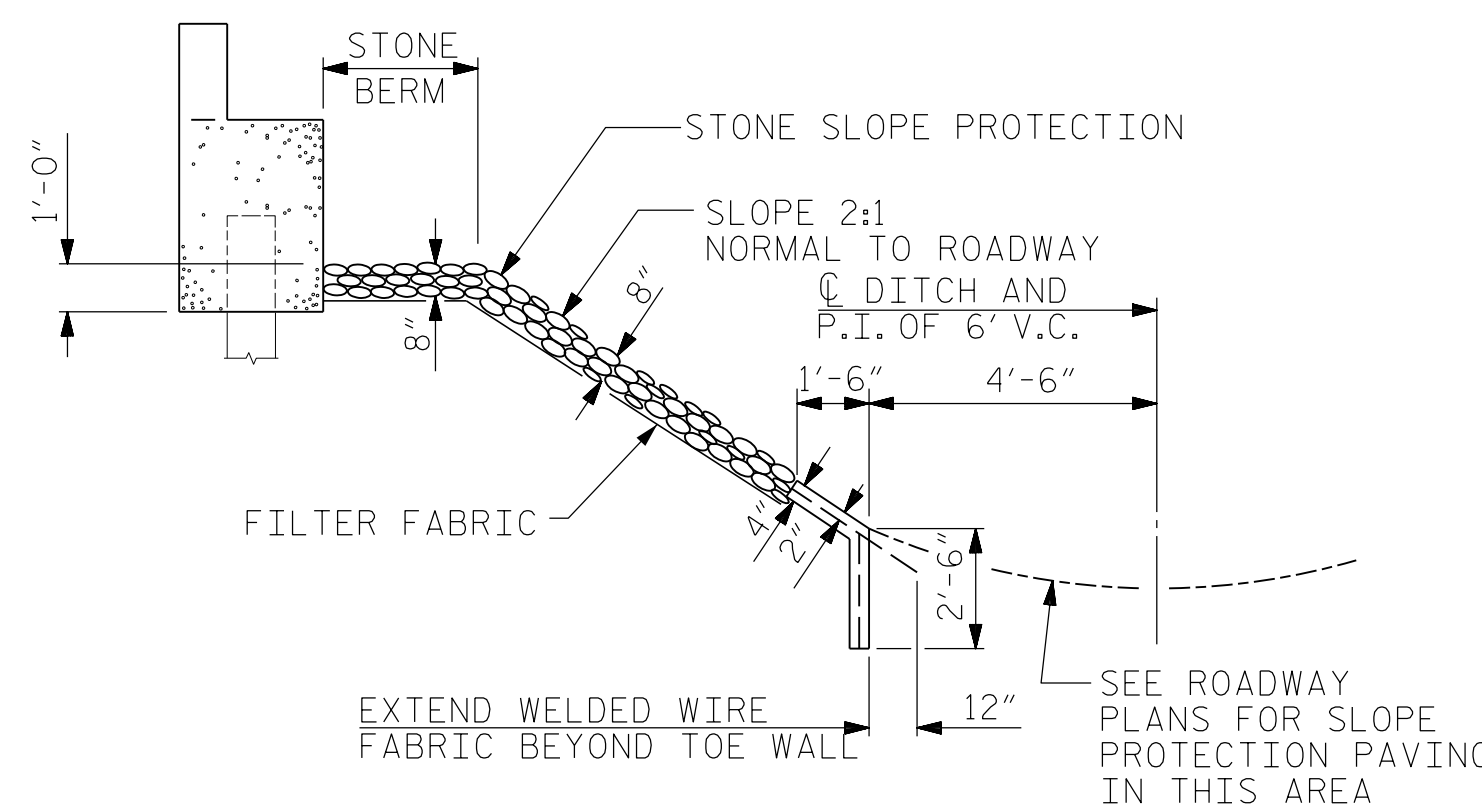
ALTERNATE "A" SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0" LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

ALTERNATE "B"

ALTERNATE "B" SHALL CONSIST OF A COMBINATION CONCRETE SLAB AND STONE SLOPE PROTECTION. THE CONCRETE PORTIONS SHALL CONSIST OF PAVED STRIPS ALONG THE DITCH AS SHOWN IN THE DETAILS. FILTER FABRIC AND 8" OF STONE SHALL BE PLACED OVER THE REMAINING AREA SHOWN ON THE PLANS TO BE COVERED WITH SLOPE PROTECTION. CONCRETE SHALL BE CLASS "B". THE COST OF THE CONCRETE, FILTER FABRIC, STONE AND WELDED WIRE FABRIC 6 X 6 - W1.4 X W1.4, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION. SUBGRADING, STONE TYPE, STONE SIZING, AND HERBICIDE PROTECTION, SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THE HERBICIDE TYPE SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO APPLICATION.



SECTION A-A $\text{\textcircled{C}}$ ROADWAY WHEN FILL CATCHES IN DITCH



SECTION B-B $\text{\textcircled{C}}$ ROADWAY WHEN FILL CATCHES IN DITCH

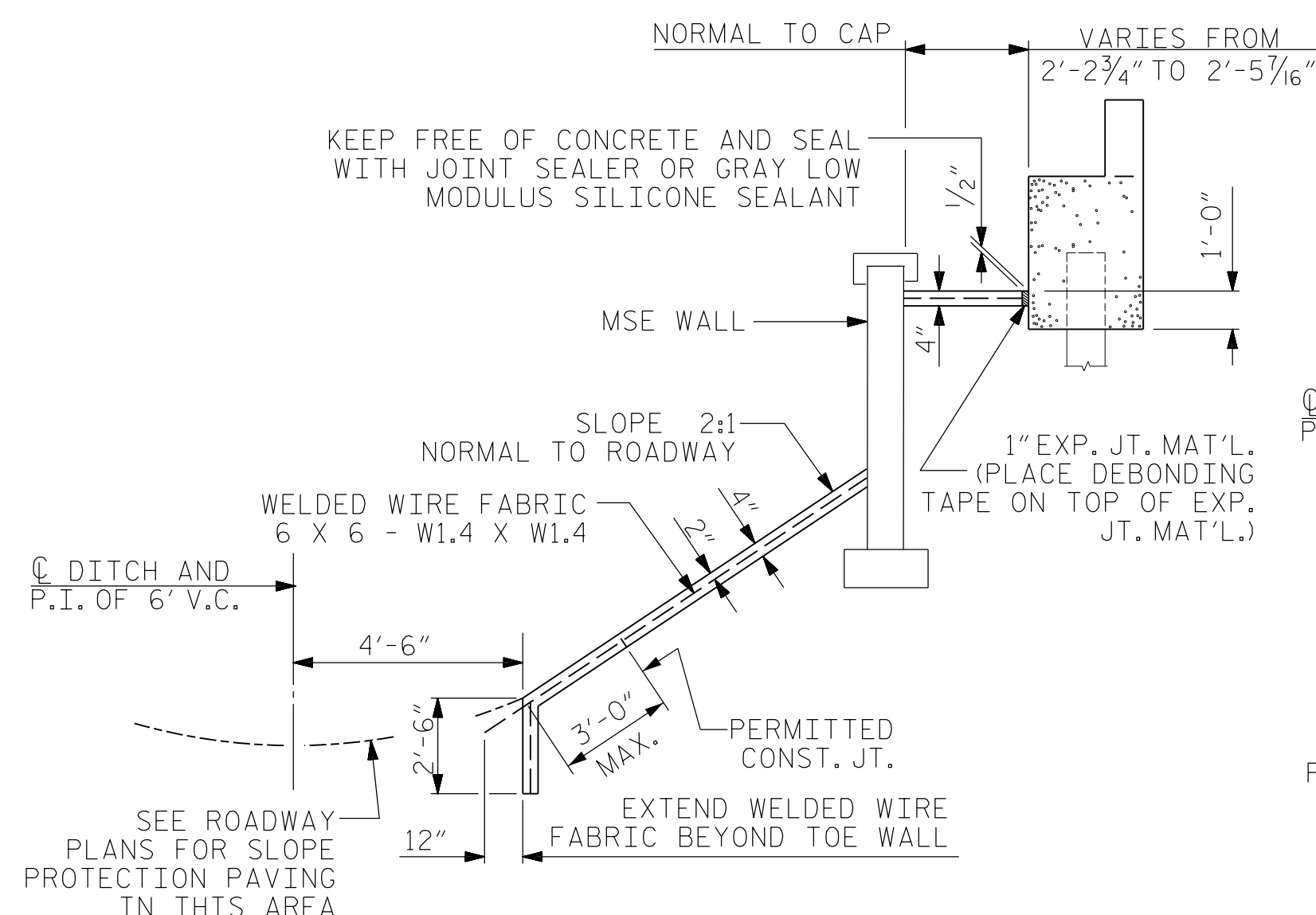
BRIDGE @ STA. 20+70.23 -Y3-	4" INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX. L.F.
END BENT 1	168	371
END BENT 2	175	350

* QUANTITY SHOWN IS BASED ON 5' POURS.

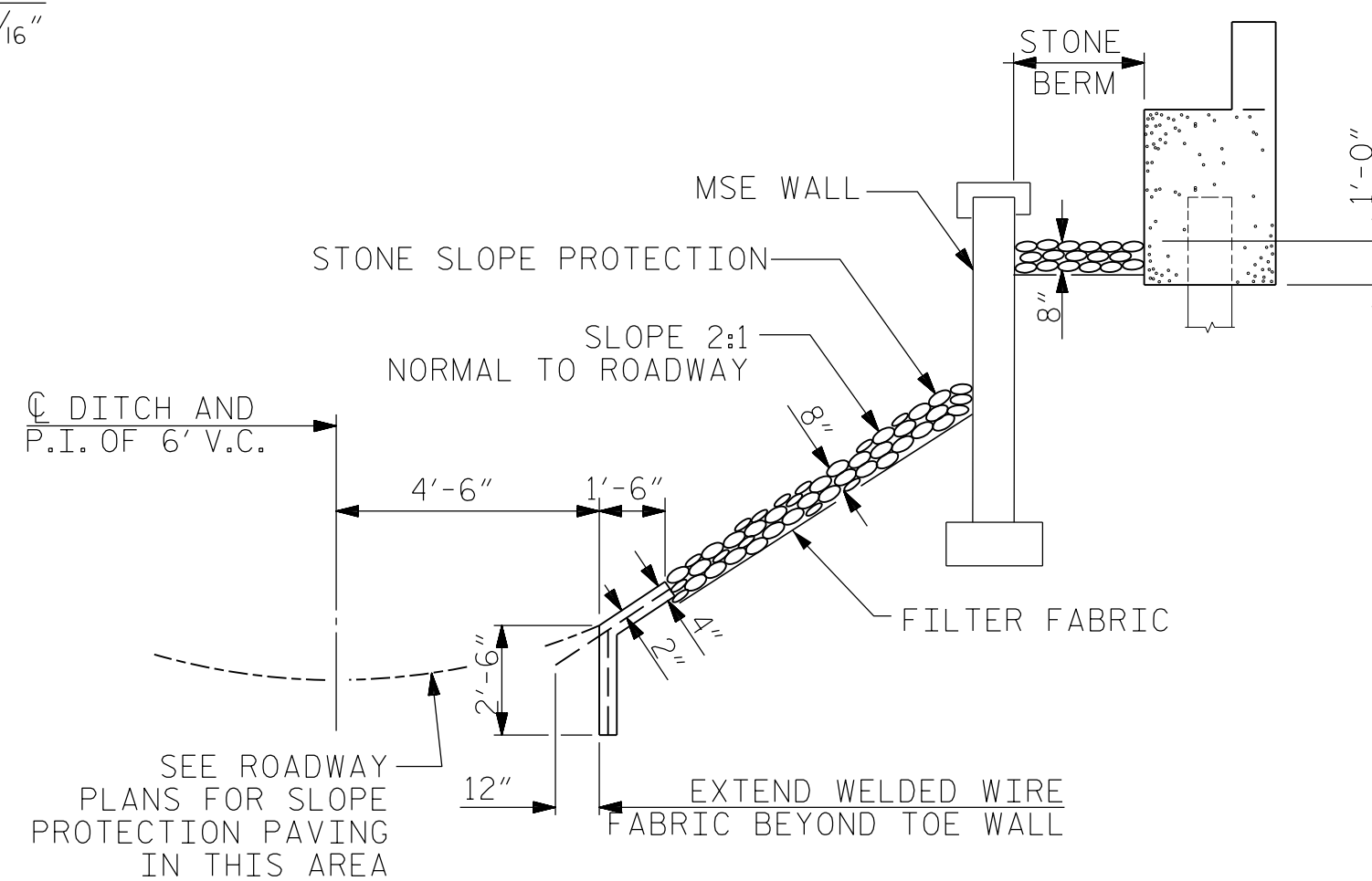
DETAILS FOR ALTERNATE "A"

DETAILS FOR ALTERNATE "B"

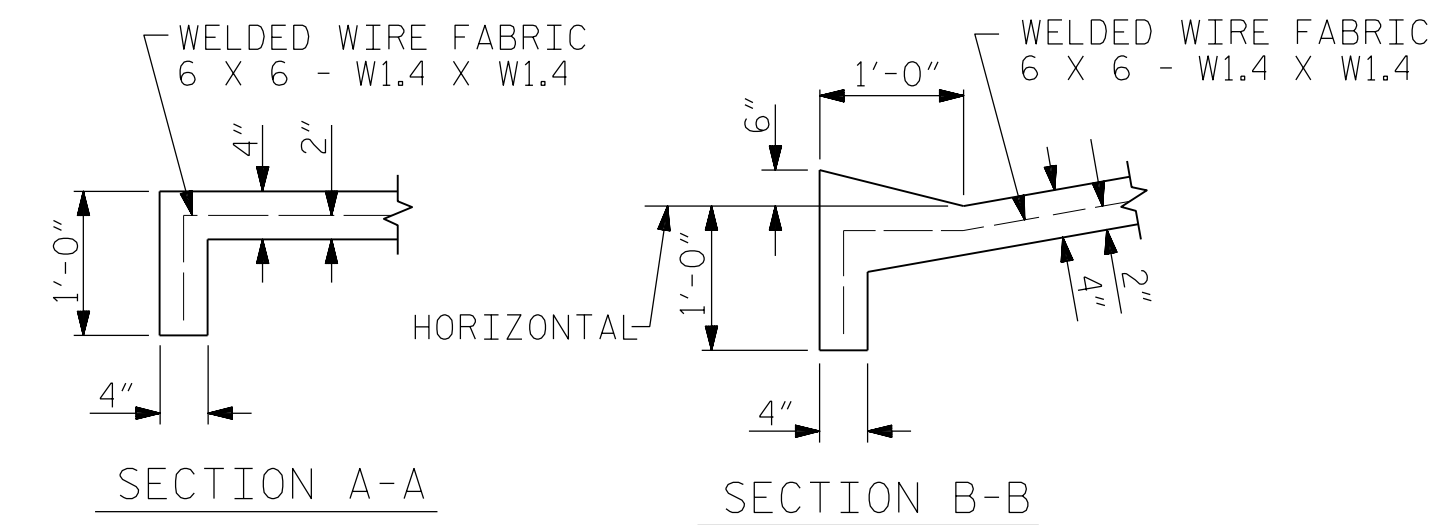
END BENT NO. 1



SECTION A-A $\text{\textcircled{C}}$ ROADWAY WHEN FILL CATCHES IN DITCH

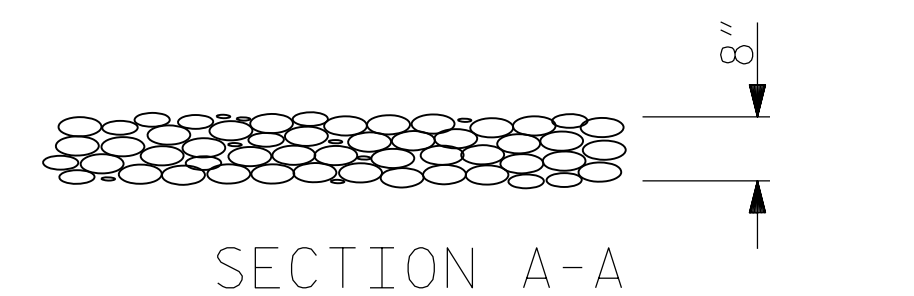


SECTION B-B $\text{\textcircled{C}}$ ROADWAY WHEN FILL CATCHES IN DITCH

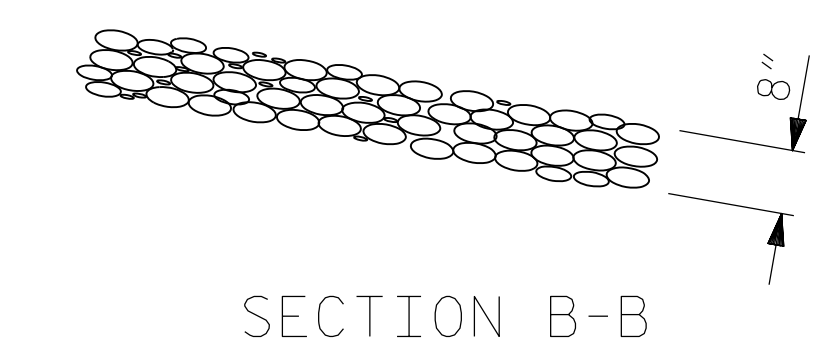


SECTION A-A

SECTION B-B



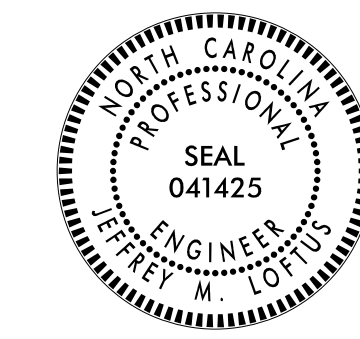
SECTION A-A



SECTION B-B

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 20+70.23-Y3- POT

SHEET 1 OF 2



DocuSigned by: Jeff Loftus 2/6/2017

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SLOPE PROTECTION DETAILS					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO. S4-26
TOTAL SHEETS 29

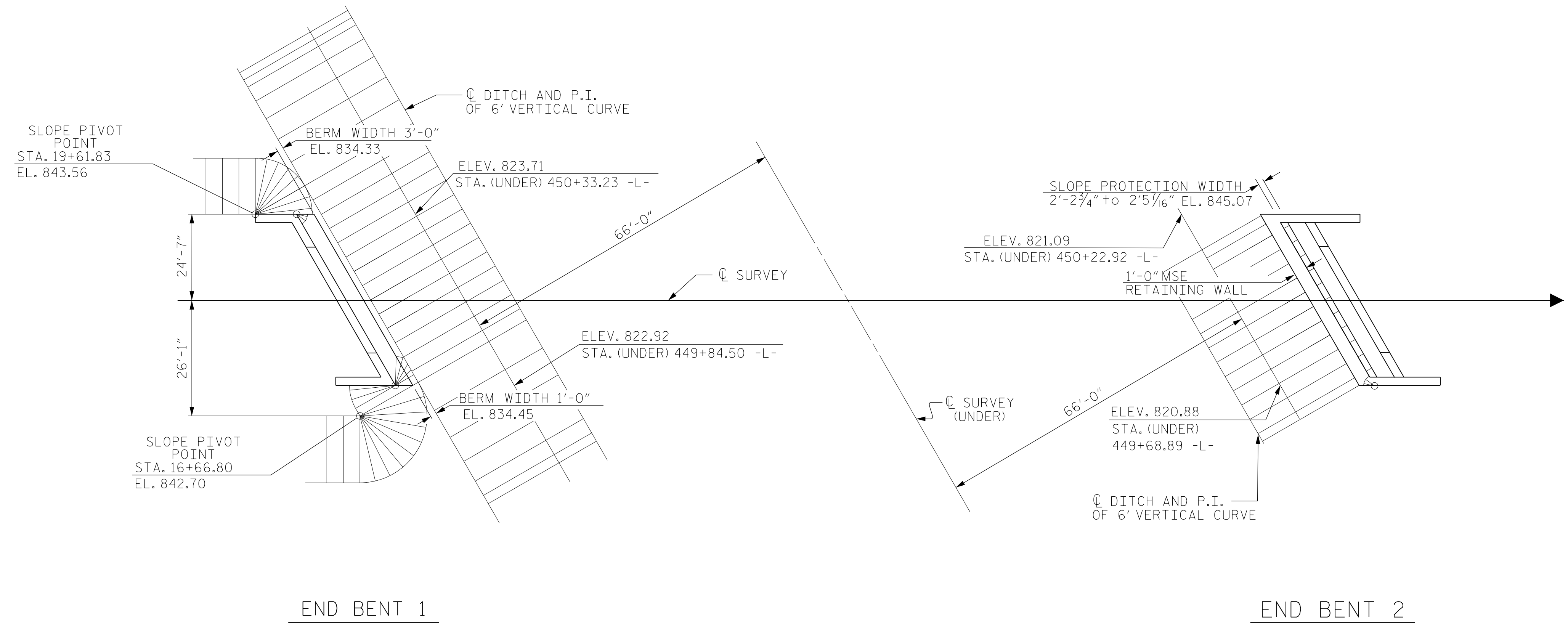
STR. #4

DRAWN BY: HASSFOURA DATE: 11/16
CHECKED BY: JLOFTUS DATE: 11/16
DESIGN ENGINEER OF RECORD: JLOFTUS DATE: 01/17

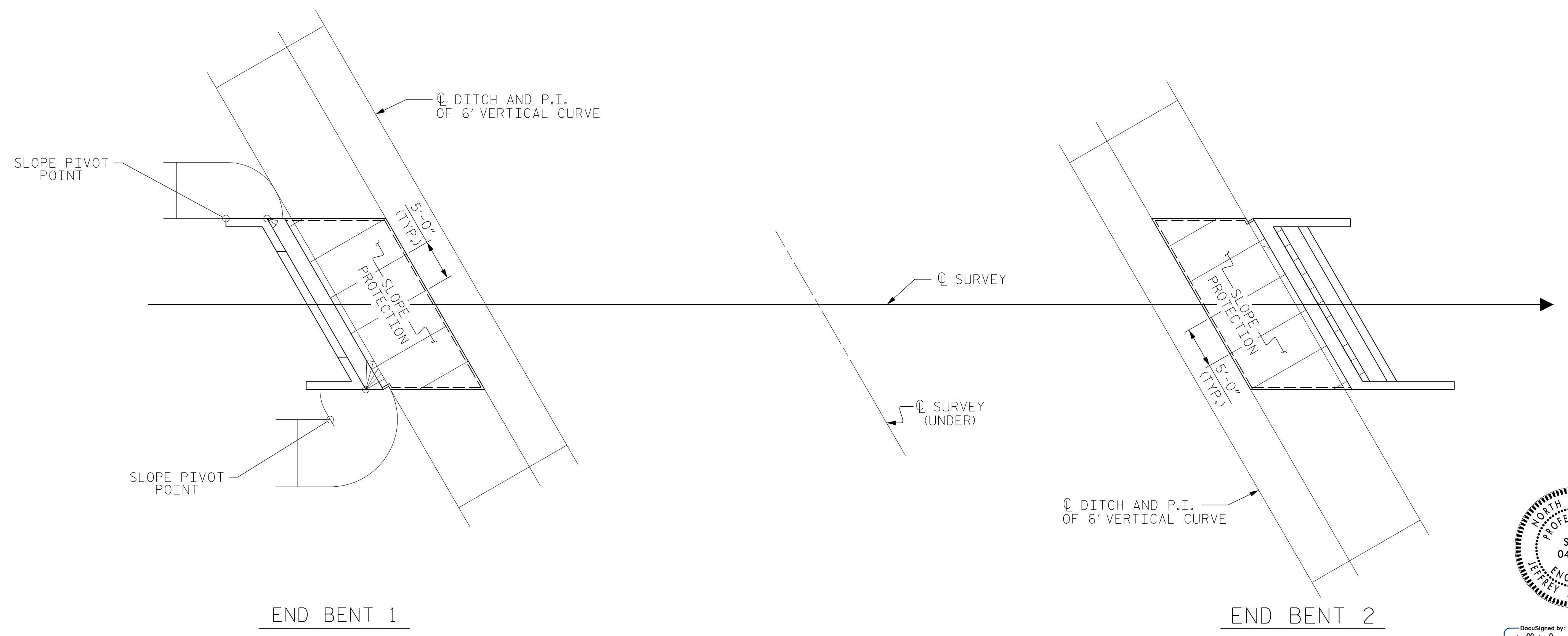
END BENT NO. 2

R 2707C-SITE 3

2/6/2017
\\404-051-R2707C-SMU-SP01-S4-26.dgn
USER:jeffloftus



PLAN - GRADING



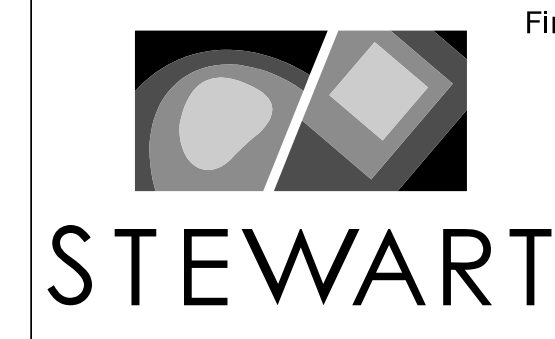
PLAN - CONCRETE PLACEMENT

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 20+70.23-Y3- POT
 SHEET 2 OF 2



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 Jeff Capus 2/6/2017

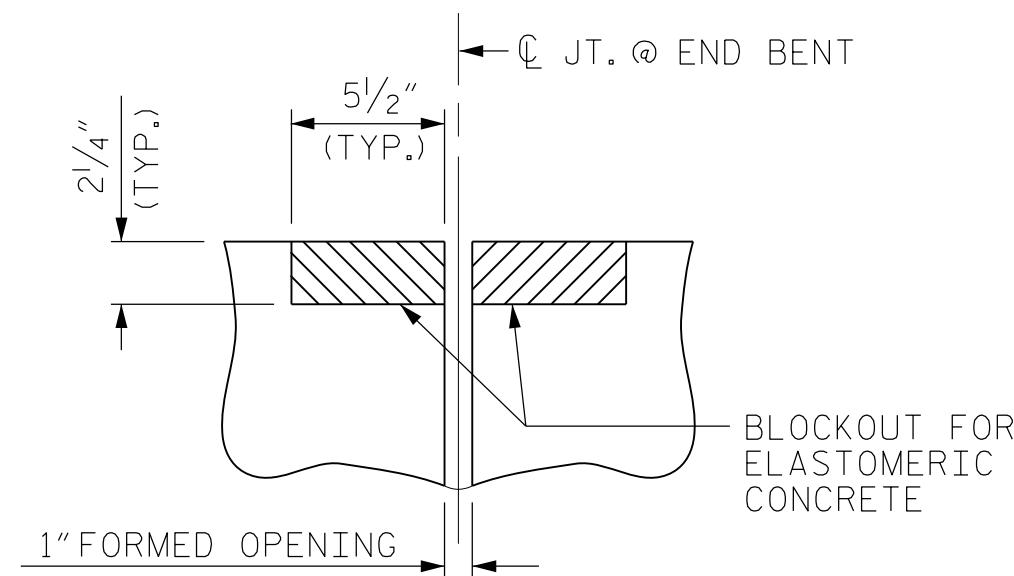
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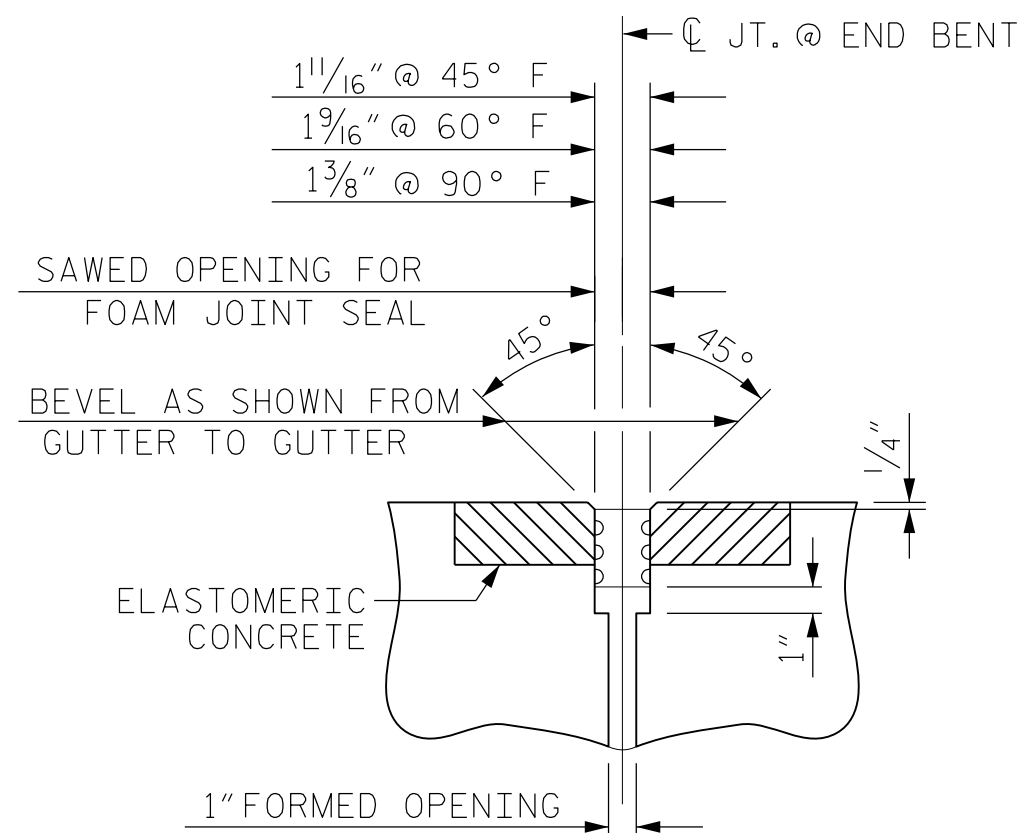
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SLOPE PROTECTION DETAILS					
REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
TOTAL SHEETS					29

DRAWN BY: <u>H.ASSFOURA</u>	DATE: <u>11/16</u>
CHECKED BY: <u>J.LOFIUS</u>	DATE: <u>11/16</u>
DESIGN ENGINEER OF RECORD: <u>J.LOFIUS</u>	DATE: <u>01/17</u>

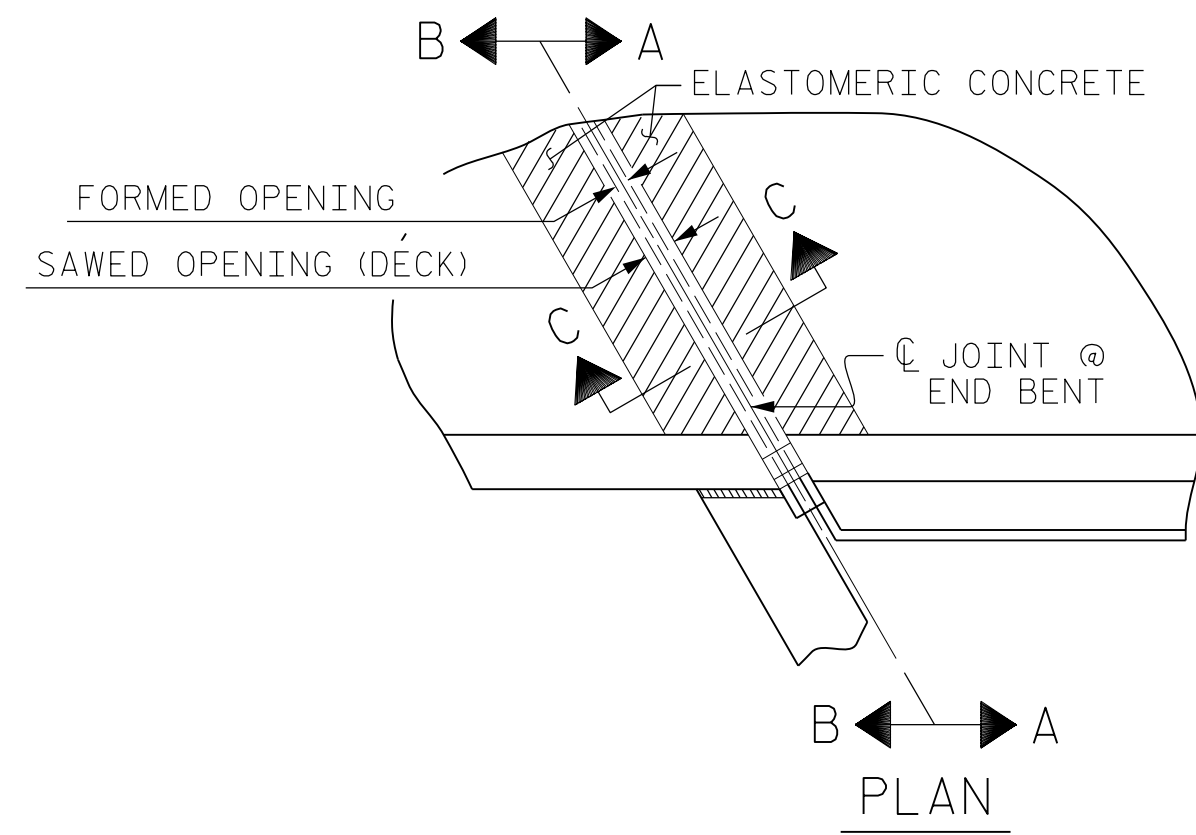
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 2/6/2017
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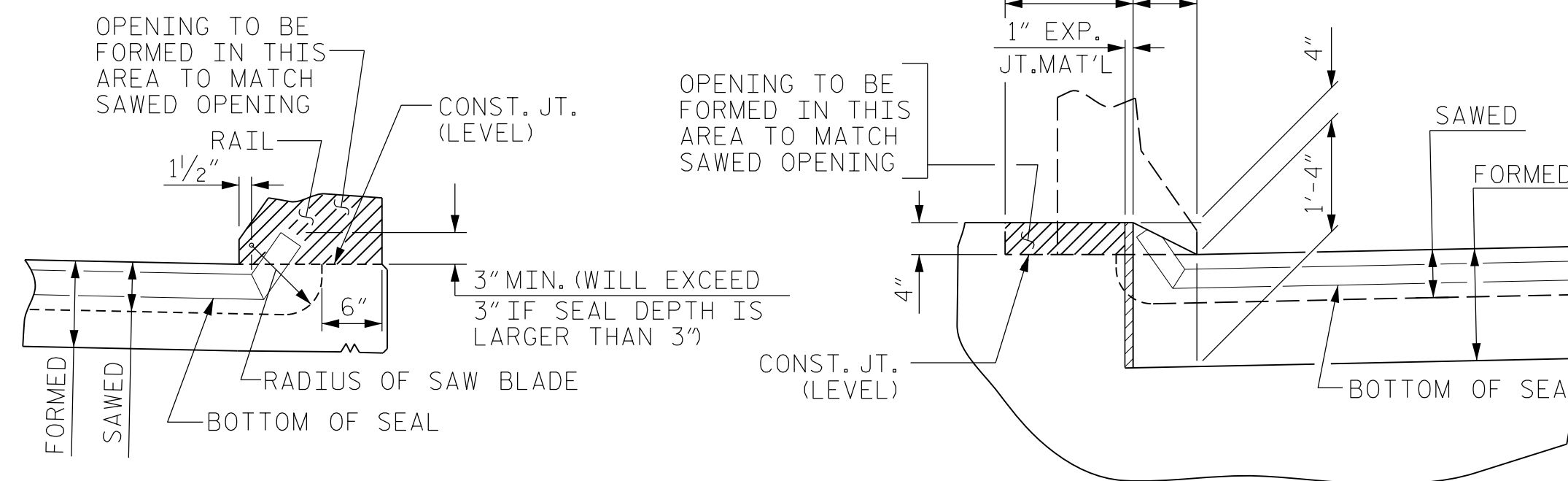
SECTION C-C
FOAM JOINT SEAL
(PRE-SAWED ELASTOMERIC
CONCRETE DIMENSIONS)



SECTION C-C
FOAM JOINT SEAL
(EXPANSION)



PLAN



SECTION A-A

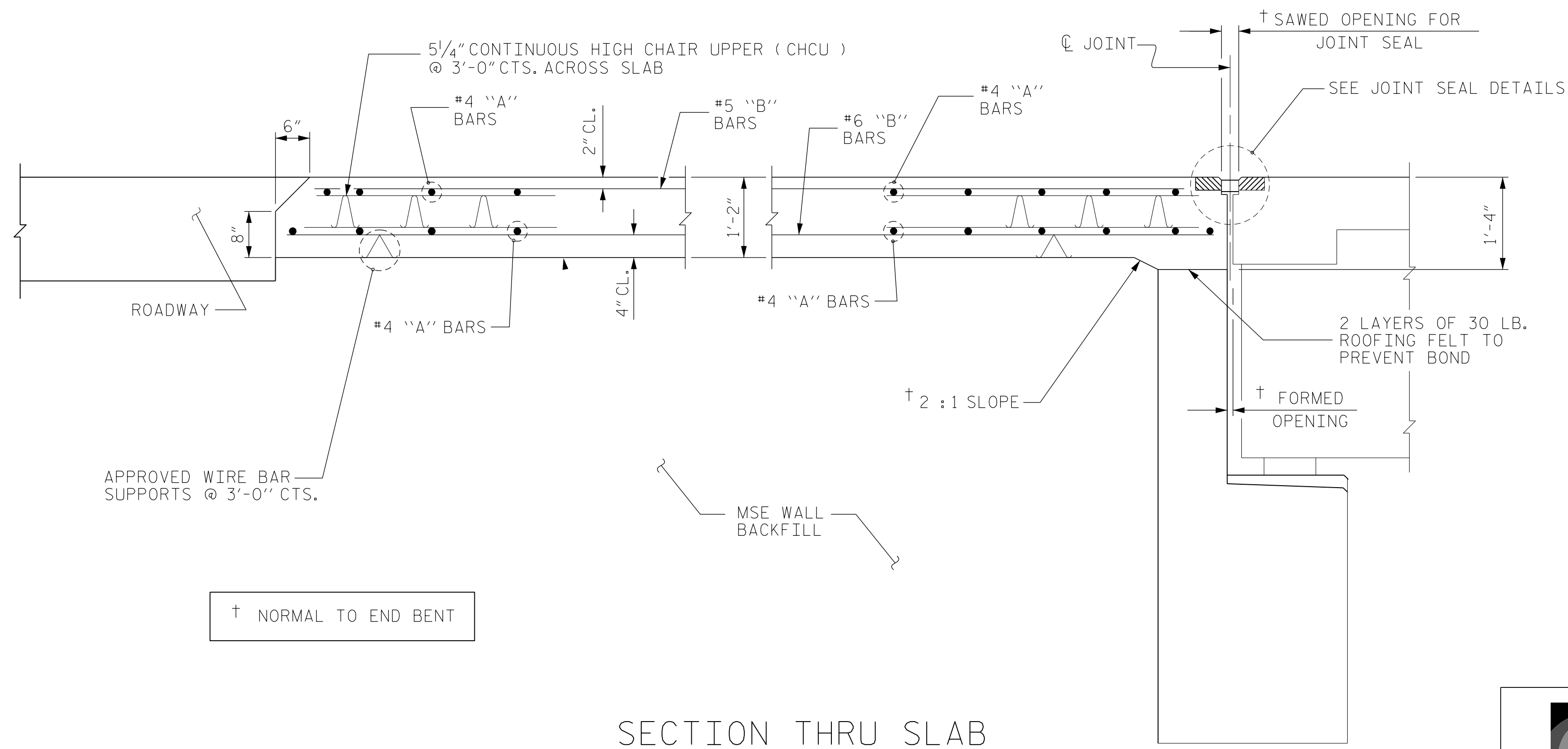
SECTION B-B

JOINT SEAL DETAILS @ END BENT

FOAM JOINT SEAL TO BE CUT, HEAT WELDED AND TURNED UP PARALLEL TO SLOPED FACE OF THE BARRIER RAIL.
THE JOINT SHALL BE SAWED PRIOR TO THE CASTING OF THE BARRIER RAIL.

ELASTOMERIC CONCRETE	
END BENT NO.	ELASTOMERIC CONCRETE * (CU. FT.)
1	6.9
2	6.9
TOTAL	13.8

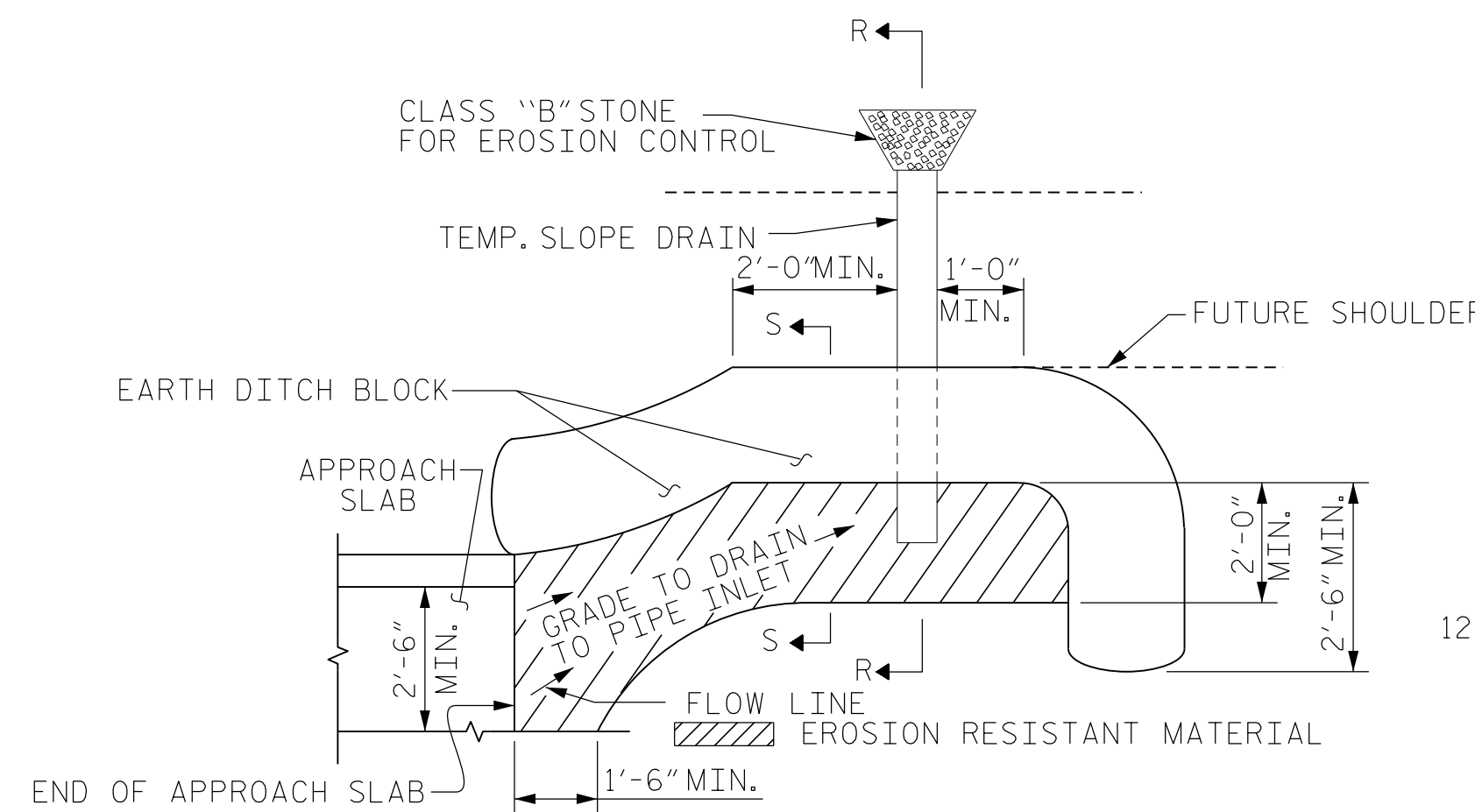
* BASED ON THE MINIMUM BLOCKOUT SHOWN.



SECTION THRU SLAB

@ END BENT #2

NOTE: FOR MSE RETAINING WALLS, SEE GEOTECHNICAL SPECIAL PROVISIONS

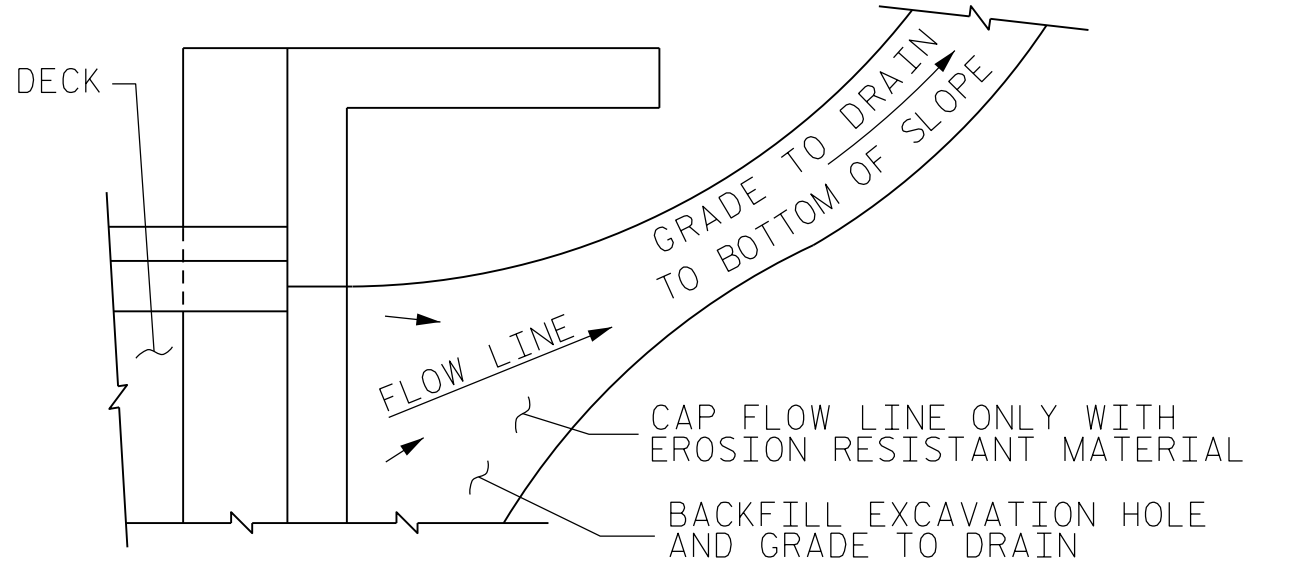


PLAN VIEW

NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. R-2707C
CLEVELAND COUNTY
STATION: 20+70.23-Y3- POT

SHEET 2 OF 2



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2/6/2017

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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
BRIDGE APPROACH
SLAB DETAILS

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			S4-29
2			4			TOTAL SHEETS 29

STR. #4

STD. NO. BAS4

R 2707C-SITE 3

2/6/2017

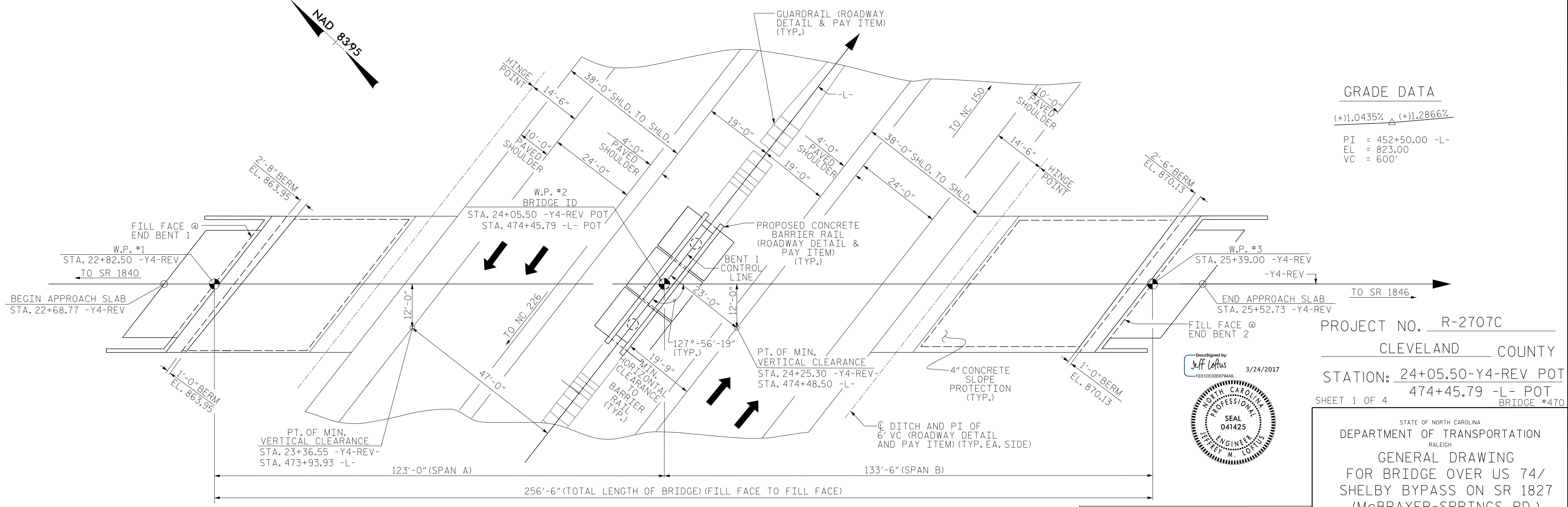
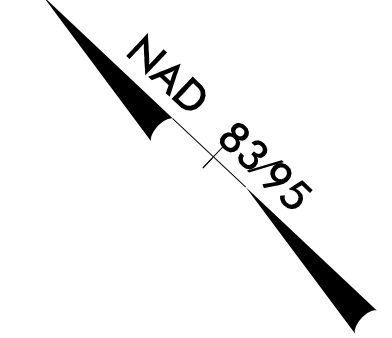
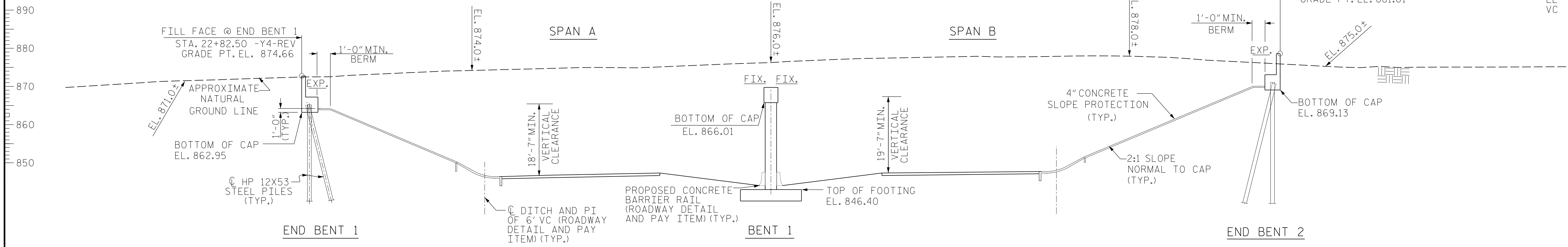
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DRAWN BY: H.ASSFOURA DATE: 07/16
CHECKED BY: J.LOFIUS DATE: 11/16
DESIGN ENGINEER OF RECORD: J.LOFIUS DATE: 01/17

22+50 23+00 23+50 24+00 24+50 25+00 25+50 26+00

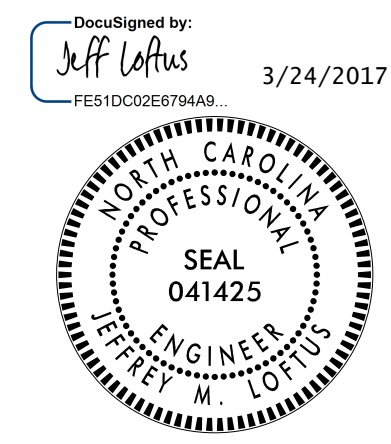
GRADE DATA

(+)-2.4775% (+)-1.6804%
 PI = 27+00.00 -Y4-REV-
 EL = 885.00
 VC = 200'



GRADE DATA

(+)-1.0435% (+)-1.2866%
 PI = 452+50.00 -L-
 EL = 823.00
 VC = 600'



PROJECT NO. R-2707C
 CLEVELAND COUNTY
 STATION: 24+05.50-Y4-REV POT
 474+45.79 -L- POT
 SHEET 1 OF 4 BRIDGE #470

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 GENERAL DRAWING
 FOR BRIDGE OVER US 74/
 SHELBY BYPASS ON SR 1827
 (MCBRAYER-SPRINGS RD.)
 BETWEEN SR 1840 AND SR 1846

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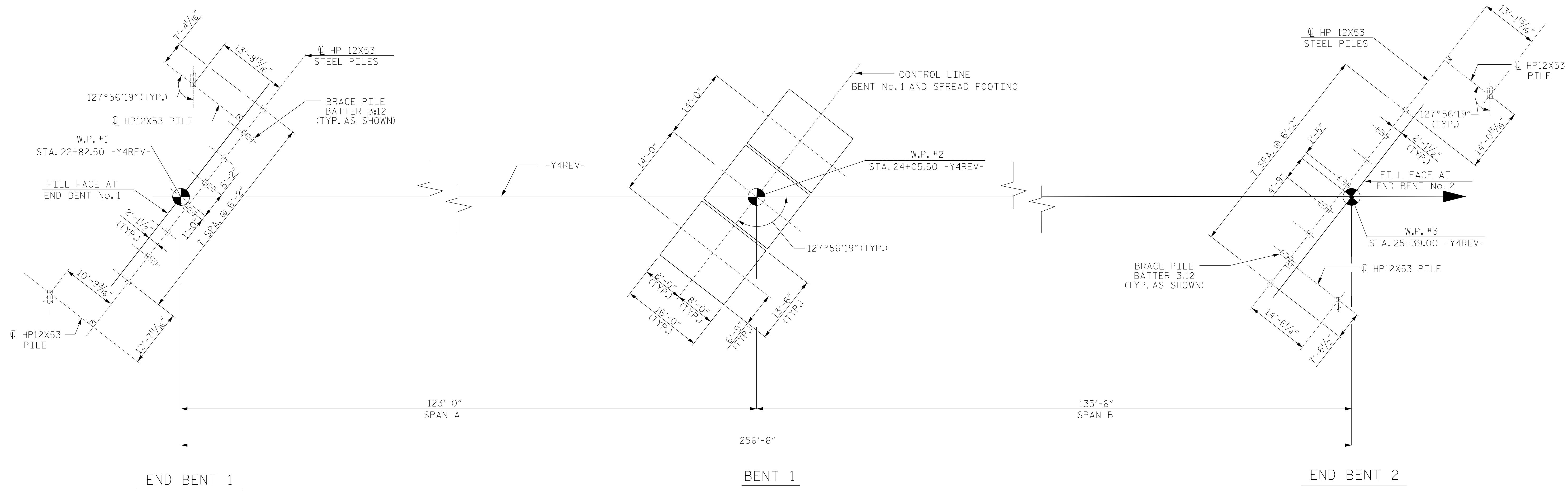
DRAWN BY: H.ASSFOURA DATE: 02/16
 CHECKED BY: J.LOFTUS DATE: 11/16
 DESIGN ENGINEER OF RECORD: J.LOFTUS DATE: 01/17

REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S5-1	
1			3			TOTAL SHEETS	
2			4			29	

STR. #5

R 2707C.5

3/24/2017
 \\V05-001-R2707C-SMU-GD01-S5-1.dgn
 USER:jeffloftus



FOUNDATION LAYOUT

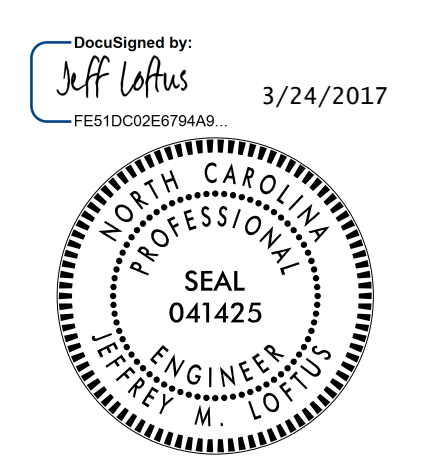
DIMENSIONS LOCATING PILES ARE SHOWN TO PILE CENTERLINE AT BOTTOM OF CAP

NOTES

- FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.
- PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.
- PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 100 TONS PER PILE.
- DRIVE PILES AT END BENT NO.1 AND END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.
- IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 40 TO 50 FT.-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT NO.1 AND END BENT NO.2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING EQUIPMENT IN ACCORDANCE WITH THE PILE PROVISION.
- THE SPREAD FOOTINGS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 3 TSF. CHECK FIELD CONDITIONS FOR THE REQUIRED RESISTANCE OF 6.6 TSF JUST BEFORE PLACING CONCRETE.
- FOOTING EXCAVATIONS AT BENT NO.1 WILL EXTEND INTO MATERIAL THAT DETERIORATES WHEN EXPOSED TO THE ELEMENTS. CHECK FIELD CONDITIONS TO THE REQUIRED RESISTANCE AND PLACE CONCRETE IMMEDIATELY AFTER THE EXCAVATION IS COMPLETED.

PROJECT NO. R-2707C
CLEVELAND COUNTY
 STATION: 24+05.50-Y4-REV POT

SHEET 2 OF 4



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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
FOUNDATION LAYOUT					
REVISIONS				SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
TOTAL SHEETS					29

DRAWN BY: <u>H.ASSFOURA</u>	DATE: <u>03/16</u>
CHECKED BY: <u>J.LOFTUS</u>	DATE: <u>11/16</u>
DESIGN ENGINEER OF RECORD: <u>J.LOFTUS</u>	DATE: <u>01/17</u>

STR. #5

R 2707C-5
 3/24/2017
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