5166 Ö REFERENCE

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

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STRUCTURE SUBSURFACE INVESTIGATION

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STATE PROJECT REFERENCE NO. B-5166

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1999 707-680. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR CUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS FOR ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

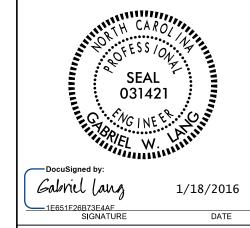
- TES:
 THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT
 OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS
 OR CONTRACT FOR THE PROJECT.
 BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
 FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
 CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

S. CROCKETT G. LANG J. BARE M. WITMORE Z. AGHAZADEH DRAWN BY S. CROCKETT

PERSONNEL

SUBMITTED BY AECOM DATE __DECEMBER, 2015

CHECKED BY Z. AGHAZADEH



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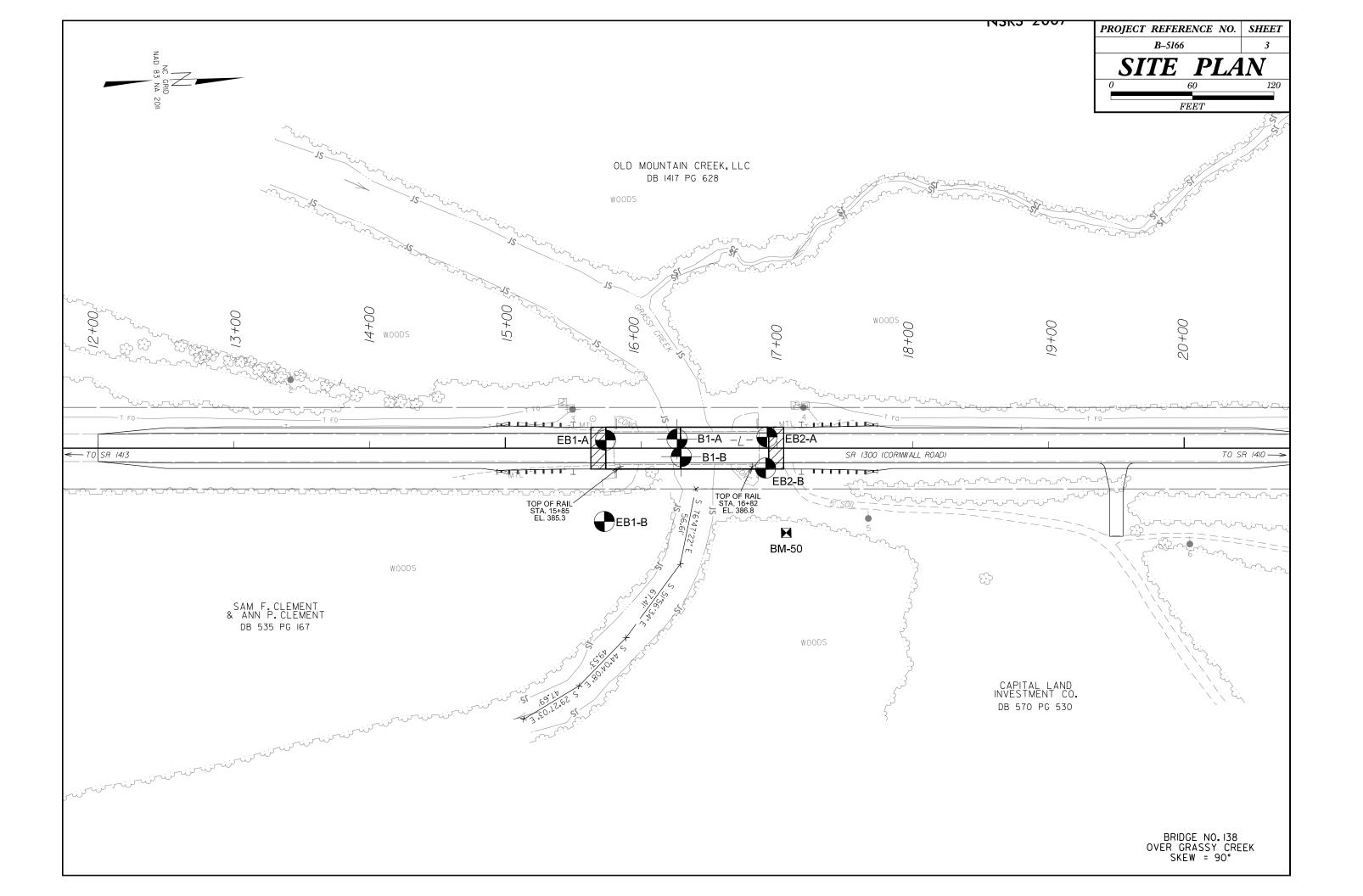
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

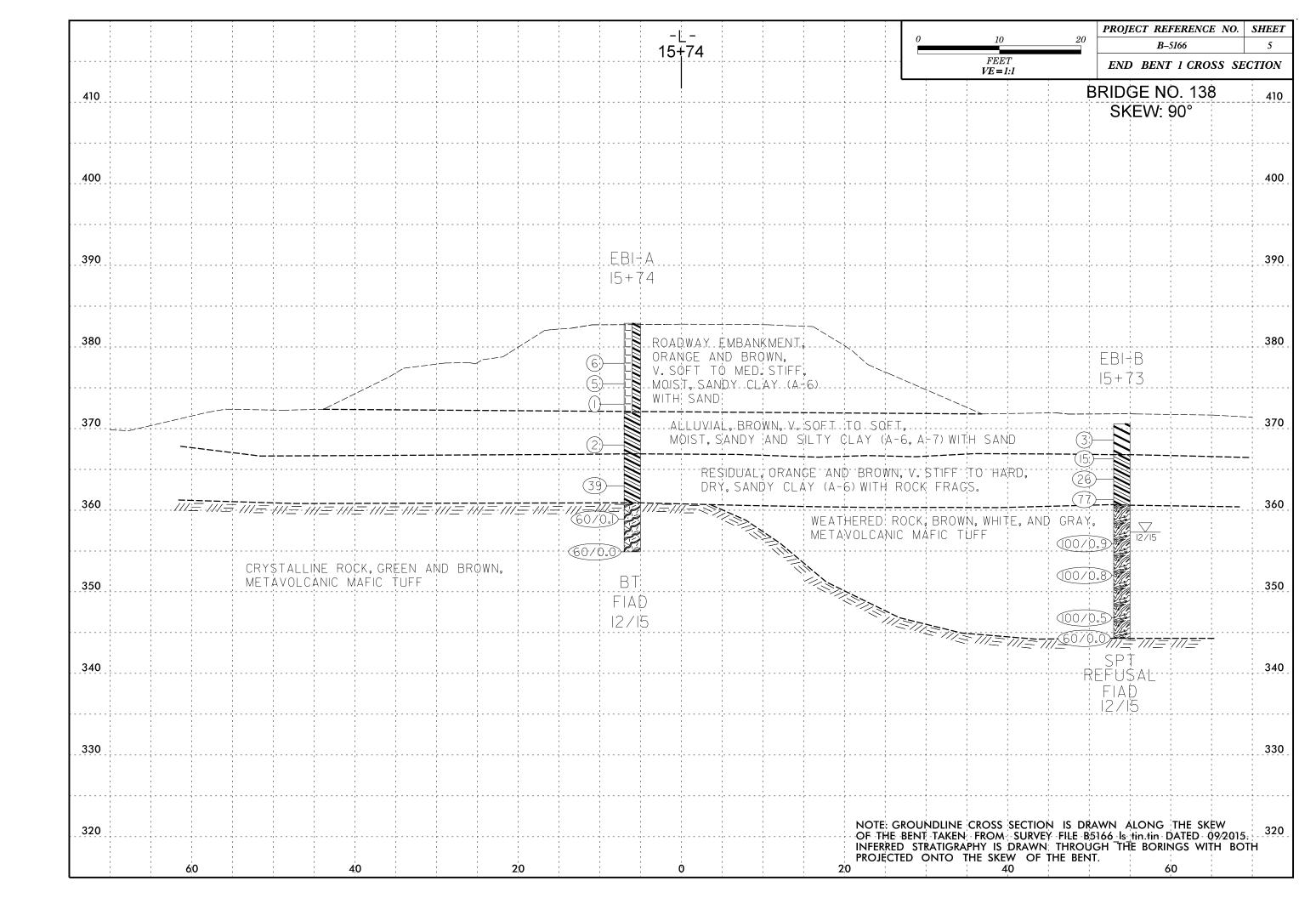
SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

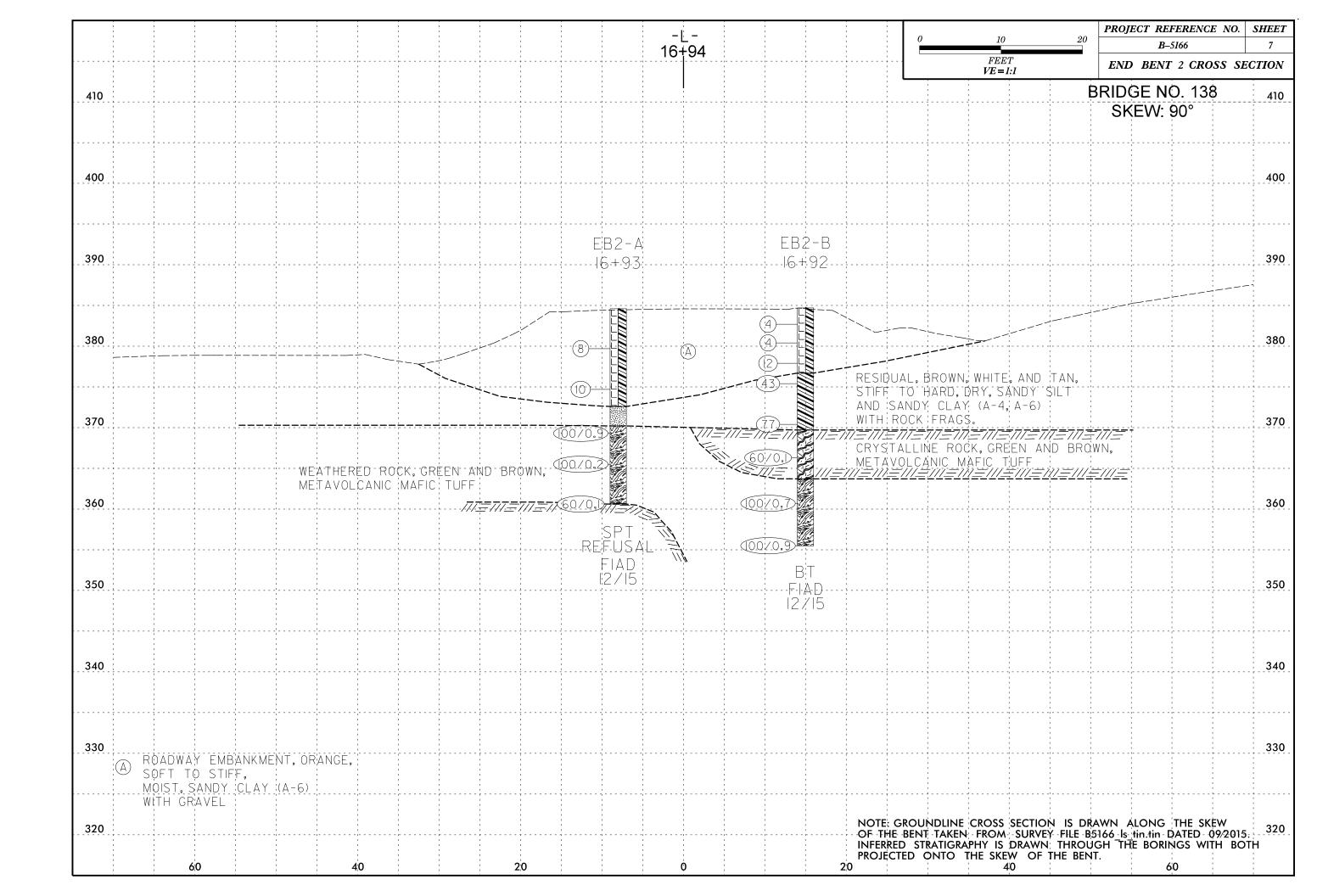
COLUMBIA DE CODIDETION	CDADATION	Tooly proprietion	TEDMO AND DESTRICTIONS
SOIL DESCRIPTION SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN	GRADATION WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	ROCK DESCRIPTION HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED	TERMS AND DEFINITIONS
BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD VIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	WEATHERED WISS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERALOGICAL COMPOSITION	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
ULASS. (\$\(\sigma\) 35/ PASSING "200) (\$\(\sigma\) 35/ PASSING "200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-0 A-1-b A-2-4 A-2-5 A-2-6 A-2-7 B-2-6 A-2-7 A-4, A-5 A-6 A-7 A-1, A-2 A-1,	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
000000000000000000000000000000000000000	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
7. PASSING SILT-GRANULAR SILT-GRANULAR CLAY MUCK,	PERCENTAGE OF MATERIAL	(CP) SHELL BEDS, ETC.	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
#40 38 MX 58 MX 51 MN FEAT FEAT FOR FEAT FEA	GRANULAR SILT - CLAY	- WEATHERING	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
PASSING *40 SOILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	HORIZONTAL.
LL — — 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 41 MN 41 MN LITTLE OR	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	<u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
CROUP INDEX 8 8 8 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF ORGANIC	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USIAL TYPES STONE FRACS ORGANIC		(SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL, AND CAND CAND CAND COLOR SOLIC COLIC	▼ STATIC WATER LEVEL AFTER 24 HOURS	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SANU	✓ PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN (MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
GEN, RATING AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	il	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP	WITH FRESH ROCK. MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.
COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED	POADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (N-VALUE) (TONS/FT ²)	ROADWAY EMBANKMENT (RE) DIP & DIP DIRECTION WITH SOIL DESCRIPTION OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4	SOIL SYMBOL SPIT MIT TEST BORING SLOPE INDICATOR INSTALLATION	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR MEDIUM DENSE 10 TO 30 N/A	M 1	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS
MATERIAL (NON-COLESIVE) DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERT DENSE / DW	M	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
VERY SOFT < 2 < 0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.5	INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL STIFF 8 TO 15 1 TO 2	TTTTT ALLUVIAL SOIL BOUNDARY A PIEZOMETER SPT N-VALUE	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4	INSTALLATION	ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SAND SILT CLAY	UNDERCOT ESS ACCEPTABLE DEGRADABLE ROCK	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(LSE, SU,) (F SU,)	ABBREVIATIONS	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN 12 3	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	BY MODERATE BLOWS. MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
	CL CLAY MOD MODERATELY 7- UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE CHURC FOR FIELD MOISTURE PERCENTATION	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{ m d}$ - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS) OBSCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u>	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
(SAT.) FROM BELOW THE GROUND WATER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC LIQUID LIMIT	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNALL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE - WET - (W) SEMISOLID; REQUIRES DRYING TO	FRAGS FRAGMENTS ω - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: BM-50, RR SPIKE IN 14" BLACK OAK
(PI) PL PLASTIC LIMITATTAIN OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	N: 991163 E: 2098814
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: 391.21 FEET
SL _ SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	
- DRY - (U) ATTAIN OPTIMUM MOISTURE	CME-55 6° CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	WOU - WEICHT OF HAMMED
PLASTICITY	X 8* HOLLOW AUGERS	INDURATION	WOH = WEIGHT OF HAMMER FIAD = FILLED IMMEDIATELY AFTER DRILLING
PLASTICITY INDEX (PI) DRY STRENGTH	CME-550 HARD FACED FINGER BITS X-N Q	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	TOP OF RAIL EL SOUTH END OF BRIDGE (EB-I): 385.3
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST UNGCARBIDE INSERTS	RUBBING WITH FINGER FREES NUMEROUS GRAINS; FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	TOP OF RAIL EL NORTH END OF BRDIGE (EB-21): 386.8
MODERATELY PLASTIC 16-25 MEDIUM	X CASING X W/ ADVANCER POST HOLE DIGGER	CRAINC CAN DE CERADATED EDOM CAMBLE WITH CIFEL BRODE.	
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	MODERATELY INDURATED MODERATELY INDURATED BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE 'TUNG,-CARB, COUNDING DOD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE:	
	1 171 11=50	DIFFICULT TO BREAK WITH HAMMER.	I and the second
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN BED YELLOW-RROWN BLUE-CRAY)	X D-50 SOUNDING ROD VANE SHEAR TEST	SILVIOUS VO BIESIK WITH THE VIEW	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	CORE BIT VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14

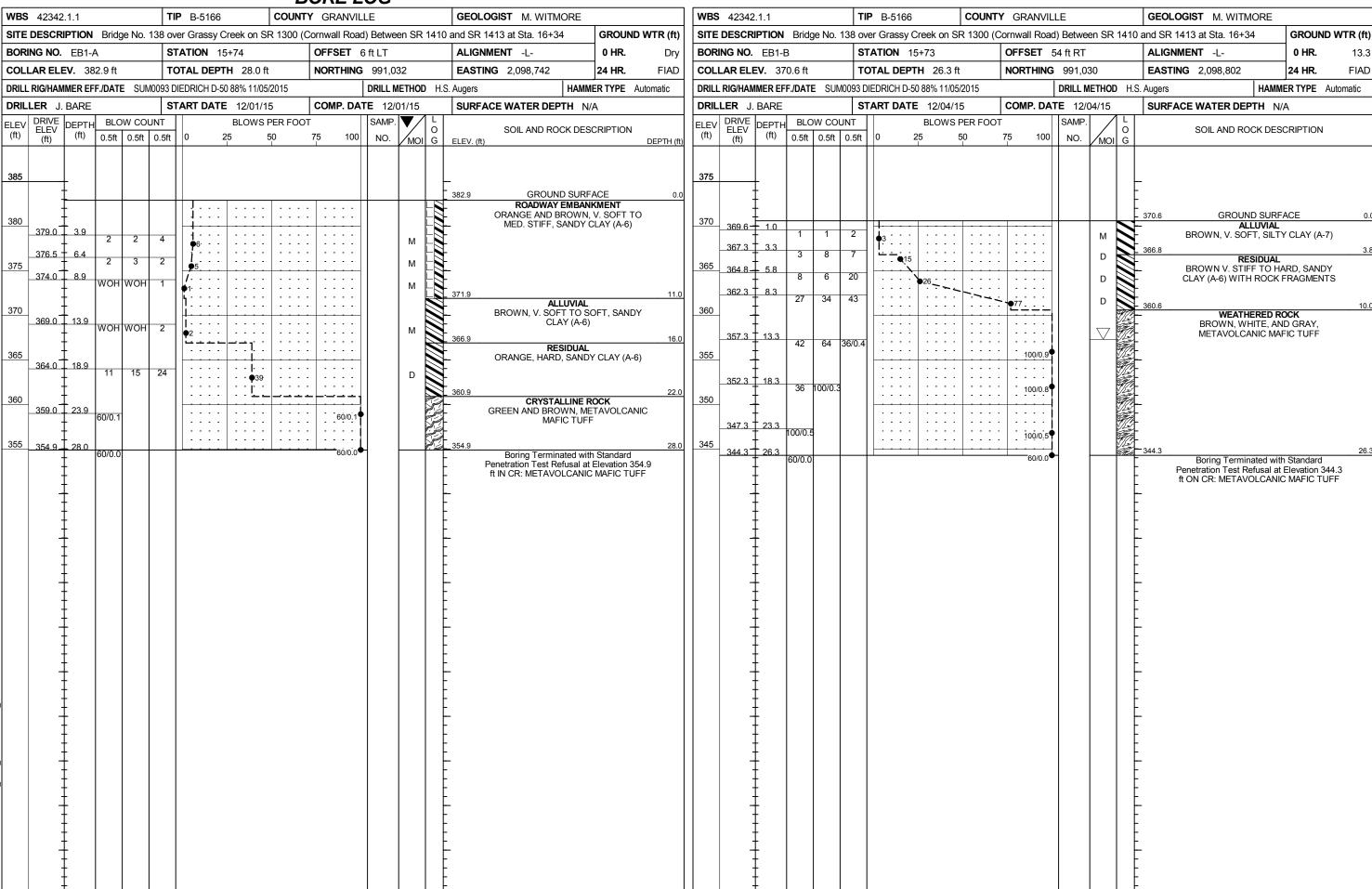


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	ALLUVIAL. E	; Brown. v	.SOFT TO SOFT,										1	1 1 1		1 1 1 1		
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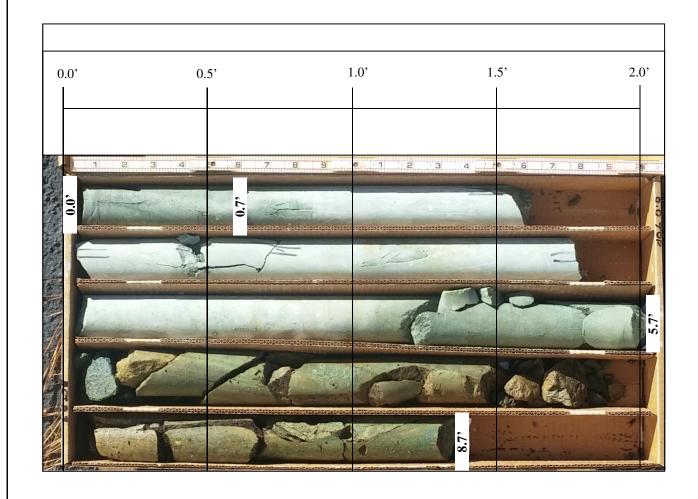


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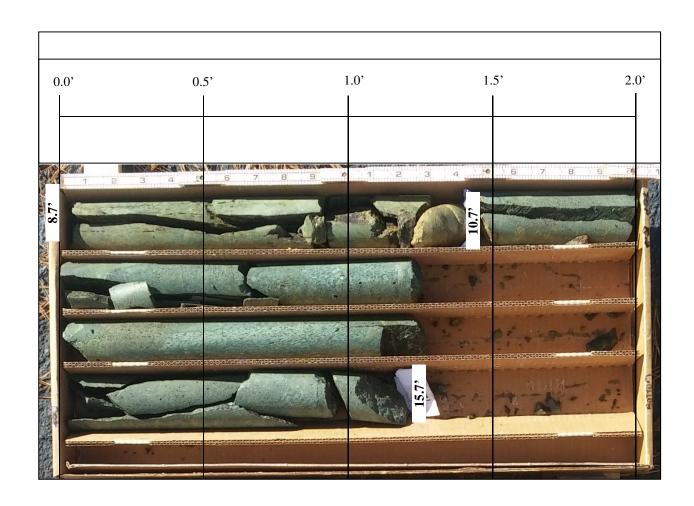




		BORE LOG					
WBS 42342.1.1	TIP B-5166 COUNT	TY GRANVILLE	GEOLOGIST M. WITMORE	WBS 42342.1.1	TIP B-5166 COUN	TY GRANVILLE	GEOLOGIST M. WITMORE
SITE DESCRIPTION Bridge No. 1	38 over Grassy Creek on SR 1300	(Cornwall Road) Between SR 141	0 and SR 1413 at Sta. 16+34 GROUND WT	R (ft) SITE DESCRIPTION Bridge No. 1	38 over Grassy Creek on SR 1300	(Cornwall Road) Between SR 141	0 and SR 1413 at Sta. 16+34 GROUND WTR (ft)
BORING NO. B1-A	STATION 16+27	OFFSET 7 ft LT	ALIGNMENT -L- 0 HR.	N/A BORING NO . B1-A	STATION 16+27	OFFSET 7 ft LT	ALIGNMENT -L- 0 HR. N/A
COLLAR ELEV. 363.2 ft	TOTAL DEPTH 20.7 ft	NORTHING 991,085	EASTING 2,098,743 24 HR .	N/A COLLAR ELEV. 363.2 ft	TOTAL DEPTH 20.7 ft	NORTHING 991,085	EASTING 2,098,743 24 HR. N/A
DRILL RIG/HAMMER EFF./DATE SUM	10093 DIEDRICH D-50 88% 11/05/2015	DRILL METHOD N	W Casing w/ Core HAMMER TYPE Autor	atic DRILL RIG/HAMMER EFF./DATE SUM	M0093 DIEDRICH D-50 88% 11/05/2015	DRILL METHOD N	IW Casing w/ Core HAMMER TYPE Automatic
DRILLER J. BARE	START DATE 12/03/15	COMP. DATE 12/03/15	SURFACE WATER DEPTH 2.7ft	DRILLER J. BARE	START DATE 12/03/15	COMP. DATE 12/03/15	SURFACE WATER DEPTH 2.7ft
ELEV DRIVE DEPTH BLOW COUN			SOIL AND ROCK DESCRIPTION	CORE SIZE NQ	TOTAL RUN 20.7 ft		
(ft) ELEV (ft) 0.5ft 0.5ft (0.5ft 0 25 50	75 100 NO. MOI G	ELEV. (ft)	PTH (ft) ELEV RUN DEPTH RUN DRILL RATE	I REC. I ROD I SAIVIP. I REC. I ROI		DESCRIPTION AND REMARKS
			WATER SURFACE (12/03/15)	(ft) (ft) (it) (Min/ft)	(ft) (ft) NO. (ft) (ft) %	G _{ELEV. (ft)}	DEPTH (ft)
365				363.2 363.2 0.0 0.7 5.56/0.7	7 (0.6) (0.6) (19.1)(12.0	8) 7-2	Ground Surface CRYSTALLINE ROCK
			GROUND SURFACE CRYSTALLINE ROCK	0.0 363.2 0.0 0.7 5:56/0.7 4:56/1.0 4:37/1.0 4:37/1.0	7 (0.6) (0.6) (19.1) (12.1) (1		TO MOD. WEATHERED, HARD, V. CLOSELY TO FRACTURED, METAVOLCANIC MAFIC TUFF
360			GREEN, FRESH TO MOD. WEATHERED, HARD, V. CLOSELY TO CLOSELY	4:18/1.0	5 (4.8) (4.0) 5 96% 80%	CLOSLETT	RMR=57
 			FRACTURED, METAVOLCANIC MAFIC TUFF	357.5 + 5.7 1:32/1.0 + 5.0 2:32/1.0	0 (4.2) (0.0)		(CLASS III - FAIR ROCK, TYPE E)
			- REC=92%	355 † 2:35/1.0 2:51/1.0	0 84% 0%		
355			- RQD=61% - RMR=57	3:42/1.0 352.5 + 10.7 3:11/1.0			
		-	- (CLASS III - FAIR ROCK, TYPE E)		0 (4.5) (3.0) 0 90% 60%		
350			- -	2:57/1.0 3:18/1.0			
			 - -	347.5 + 15.7 2:54/1.0 + 5.0 3:15/1.0	0 (5.0) (5.0)	CLOSELY F	
		-	-	+ 3:23/1.0	0 100% 100%		
345			<u>-</u>	3:21/1.0 342.5 + 20.7 3:12/1.0	RS-1		20.7
			- 342.5	20.7		Boring Terminated	at Elevation 342.5 ft IN CR: METAVOLCANIC MAFIC TUFF
			Boring Terminated at Elevation 342.5 ft IN CR: METAVOLCANIC MAFIC TUFF			ROCK OUTCRO	OP AT GROUND SURFACE. BEGAN CORING AT
			ROCK OUTCROP AT GROUND				GROUND SURFACE.
			SURFACE. BEGAN CORING AT GROUND SURFACE.				
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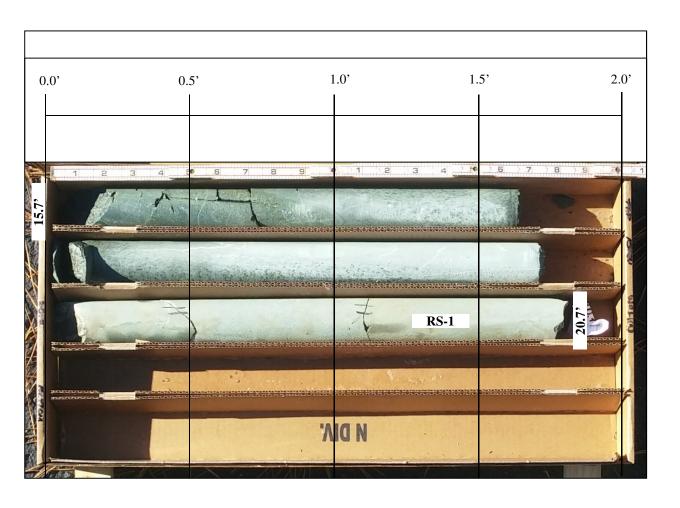
B1-A, Box 1 of 3, 0.0 to 8.7 feet.



B1-A, Box 2 of 3, 8.7 to 15.7 feet.

BRIDGE NO. 138 ON SR 1300 OVER GRASSY CREEK GRANVILLE COUNTY, NORTH CAROLINA WBS NO.: 42342.1.1, TIP NO.: B-5166



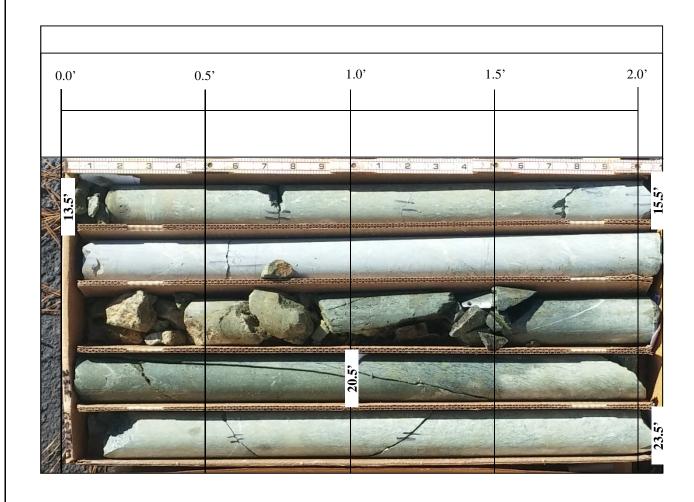


B1-A, Box 3 of 3, 15.7 to 20.7 feet.

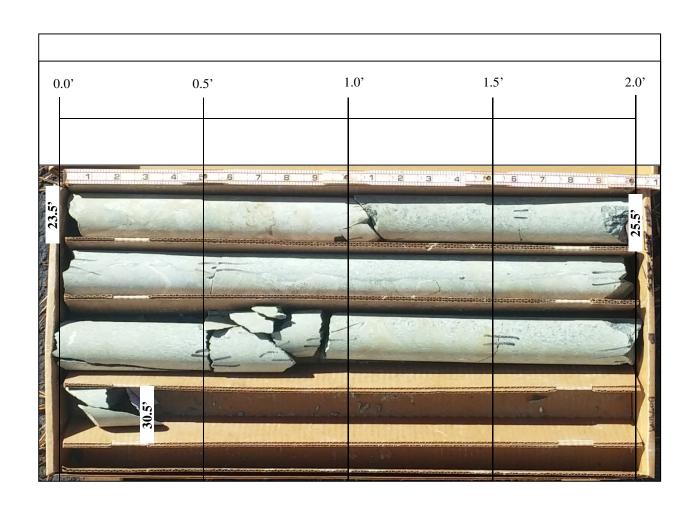
BRIDGE NO. 138 ON SR 1300 OVER GRASSY CREEK GRANVILLE COUNTY, NORTH CAROLINA WBS NO.: 42342.1.1, TIP NO.: B-5166



		BORE LOG	<u> </u>					
WBS 42342.1.1		ITY GRANVILLE	GEOLOGIST M. WITMORE	_	WBS 42342.1.1		TY GRANVILLE	GEOLOGIST M. WITMORE
SITE DESCRIPTION Bridge No. 1	<u> </u>	<u>'</u>	0 and SR 1413 at Sta. 16+34	GROUND WTR (ft)	SITE DESCRIPTION Bridge No. 13	38 over Grassy Creek on SR 1300	`	1410 and SR 1413 at Sta. 16+34 GROUND WTR (ft)
BORING NO. B1-B	STATION 16+30	OFFSET 6 ft RT	ALIGNMENT -L-	0 HR . N/A	BORING NO. B1-B	STATION 16+30	OFFSET 6 ft RT	ALIGNMENT -L- 0 HR. N/A
COLLAR ELEV. 362.8 ft	TOTAL DEPTH 35.5 ft	NORTHING 991,088	EASTING 2,098,756	24 HR. N/A	COLLAR ELEV. 362.8 ft	TOTAL DEPTH 35.5 ft	NORTHING 991,088	EASTING 2,098,756 24 HR. N/A
DRILL RIG/HAMMER EFF./DATE SUM	//0093 DIEDRICH D-50 88% 11/05/2015	DRILL METHOD N	W Casing W/SPT & Core HAMN	MER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE SUM	0093 DIEDRICH D-50 88% 11/05/2015	DRILL METHOD	NW Casing W/SPT & Core HAMMER TYPE Automatic
DRILLER J. BARE	START DATE 12/02/15	COMP. DATE 12/03/15	SURFACE WATER DEPTH 3	3.2ft	DRILLER J. BARE	START DATE 12/02/15	COMP. DATE 12/03/15	SURFACE WATER DEPTH 3.2ft
ELEV DRIVE DEPTH BLOW COU		0	SOIL AND ROCK DES	SCRIPTION	CORE SIZE NQ	TOTAL RUN 22.0 ft		
(ft) (ft) (ft) 0.5ft 0.5ft	0.5ft 0 25 50	75 100 NO. MOI G	WATER SURFACE	DEPTH (ft)	ELEV RUN ELEV (ft) DEPTH RUN (ft) RATE (Min/ft)	(ii) (ii) (iii) (iii) (iii)	C C C C C C C C C C C C C C C C C C C	DESCRIPTION AND REMARKS DEPTH (ft Begin Coring @ 13.5 ft
361.8 1.0			362.8 GROUND SURI		349.3 + 13.5 2.0 4:51/1.0 347.3 + 15.5 3:48/1.0 + 5.0 4:46/1.0	(2.0) (1.8) (21.2) (19.1) (19.		CRYSTALLINE ROCK 13.5 SH TO MOD. SEV. WEATHERED, HARD, CLOSELY TO DSELY FRACTURED, METAVOLCANIC MAFIC TUFF
360 359.3 3.5 5 6	4	M	BROWN, GREEN, AND G V. STIFF, SILTY CLAY (A- FRAGMENT	RAY, STIFF TO -7) WITH ROCK	+ 20/1.0 2:14/1.0 2:02/1.0	100% 62%	MOD. CLC	RMR=57 (CLASS III - FAIR ROCK, TYPE E)
356.8 1 6.0	16		-		342.3 + 20.5 3:16/1.0 5.0 3:48/1.0 4:05/1.0	(5.0) (5.0) (100%		,
354.3 T 8.5	11 220		- -		3:59/1.0 4:10/1.0 337.3 + 25.5 5:35/1.0 5.0 4:28/1.0	(4.3) (4.3) 86% 86%		
350 349.3 13.5 60/0.1			350.8 CRYSTALLINE I	12.0 ROCK C MAFIC TUFF 1 13.5	T 3:43/1.0 3:20/1.0			
345			CRYSTALLINE I GREEN, FRESH TO WEATHERED, HARD, (ROCK MOD. SEV. CLOSELY TO	332.3 + 30.5 3:08/1.0	(4.9) (4.9) 98% 98%	327.3	
			MOD. CLOSELY FRA	AFIC TUFF	327.3 + 35.5 4:05/1.0			35.5 ated at Elevation 327.3 ft IN CR: METAVOLCANIC MAFIC
340 +			REC=96% RQD=87% RMR=57 (CLASS III - FAIR ROC				[TUFF
335		RS-2	- (012400 III - 1 AII (11000	JN, 111 L L)			-	
330			<u>-</u> -				[
			Boring Terminated at Elev CR: METAVOLCANIC I	ration 327.3 ft IN MAFIC TUFF			<u> </u>	
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12/27/16								
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BBDG.							-	
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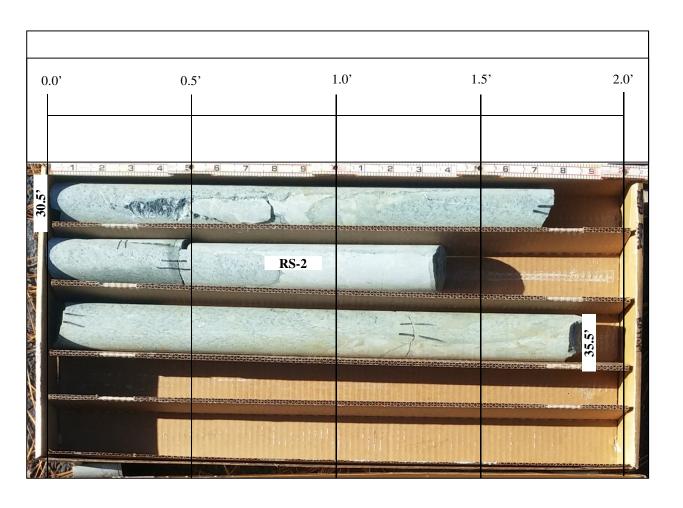
B1-B, Box 1 of 3, 13.5 to 23.5 feet.



B1-B, Box 2 of 3, 23.5 to 30.5 feet.

BRIDGE NO. 138 ON SR 1300 OVER GRASSY CREEK GRANVILLE COUNTY, NORTH CAROLINA WBS NO.: 42342.1.1, TIP NO.: B-5166

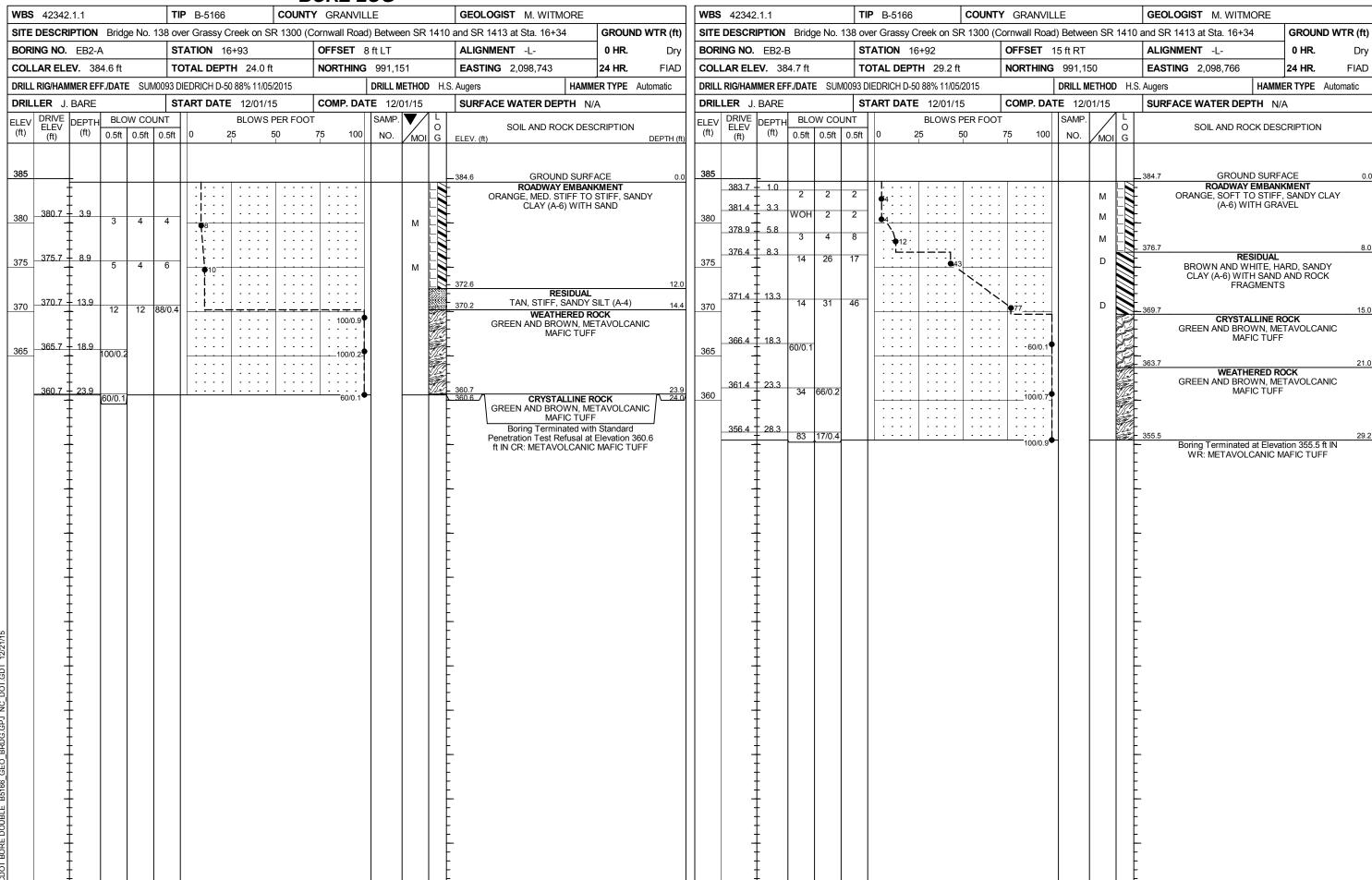




B1-B, Box 3 of 3, 30.5 to 35.5 feet.

BRIDGE NO. 138 ON SR 1300 OVER GRASSY CREEK GRANVILLE COUNTY, NORTH CAROLINA WBS NO.: 42342.1.1, TIP NO.: B-5166







UNCONFINED COMPRESSIVE STRENGTH of INTACT ROCK CORE SPECIMENS

ASTM D 7012-14 Method C

(This method does not report Strain Rate or Deformation) Sample Prep and Conformance Verification: ASTM D4543-08

Client: **AECOM**

Client Reference:

Project No.:

Lab ID:

B-5166 Bridge over Grassy Creek 60447478

Boring No.: Depth (ft):

Boring No.: B1-B

B-5166 Bridge over Grassy Creek 60447478

32.8-33.7

Sample No.: RS-2

Lab ID: 2015-648-001-002

SPECIMEN LENGTH (in)

2015-648-001

Reading 1:

Reading 2:

Reading 3:

Average Length:

2015-648-001-001

4.03

4.03

4.03 4.03

SPECIMEN DIAMETER (in): Reading 1:

Sample No.: RS-1

B1-A

Moisture Condition: As Received-Unpreserved

20.0-20.7

Reading 2: 1.99 Average Diameter: 1.99

Area (in²): 3.11

Length/Diameter: 2.03

Total Load (lb): 27,780

138

1.99

Uniaxial Compressive Strength (psi): 8,920

Fracture Type: Cone & Split

Rate of Loading (lb/sec): 3:21.38

Time to Break (min:sec):

Deviation from Straitness (2):

Axial: Pass

Top: Pass

Bottom: Pass





ROCK CORE

Notes:

1) Moisture conditions at time of the test are:

As Received-Unpreserved

- 2) Deviation from straightness, Procedure A of ASTM D 4543-08 Pass/Fail criteria: gap < 0.02 = Pass, gap > 0.02 = Fail
- 3) Temperature is laboratory room temperature.
- 4) Geotechnics Equipment Used: G788 Compression Tester.
- G1122 Digital Calipers, G1380 Dial Gauge,
- G1557 Straight Edge, G1571 Feeler Gauge,
- G1633 V-Block, G1634 Rock Saw, G1635 Grinder.

Tested By: JAC 12/9/15 Date: Checked By: KC 12/9/15 Date:

DCN: CT45A; Revision No. 2e; Revision Date: 08/25/15

544 Braddock Avenue • East Pittsburgh, PA 15112 • Phone (412) 823-7600 • Fax (412) 823-8999 • www.geotechnics.net



UNCONFINED COMPRESSIVE STRENGTH of INTACT ROCK CORE SPECIMENS

ASTM D 7012-14 Method C

(This method does not report Strain Rate or Deformation) Sample Prep and Conformance Verification: ASTM D4543-08

Client: **AECOM**

Client Reference:

Project No.:

Reading 1:

Reading 2:

Reading 3:

Average Length:

Depth (ft):

2015-648-001

Moisture Condition: As Received-Unpreserved

SPECIMEN LENGTH (in)

3.98

3.99

3.99

3.99

SPECIMEN DIAMETER (in): Reading 1: 1.99

> Reading 2: 1.99 Average Diameter: 1.99

> > Area (in²): 3.11

Length/Diameter: 2.00

Total Load (lb): 33,770

Uniaxial Compressive Strength (psi): 10,840

Fracture Type: Cone & Split

Rate of Loading (lb/sec): 146 3:51.94

Time to Break (min:sec): Deviation from Straitness (2):

Axial: Pass

Top: Pass

Bottom: Pass



DCN: CT45A; Revision No.: 2e; Revision Date: 08/25/15

Physical

Properties:

ROCK CORE

Notes:

1) Moisture conditions at time of the test are:

As Received-Unpreserved

- 2) Deviation from straightness, Procedure A of ASTM D 4543-08 Pass/Fail criteria: gap < 0.02 = Pass, gap > 0.02 = Fail
- 3) Temperature is laboratory room temperature.
- 4) Geotechnics Equipment Used: G788 Compression Tester,
- G1122 Digital Calipers, G1380 Dial Gauge, G1557 Straight Edge, G1571 Feeler Gauge,

G1633 V-Block, G1634 Rock Saw, G1635 Grinder.

12/9/15 Tested By: Date:

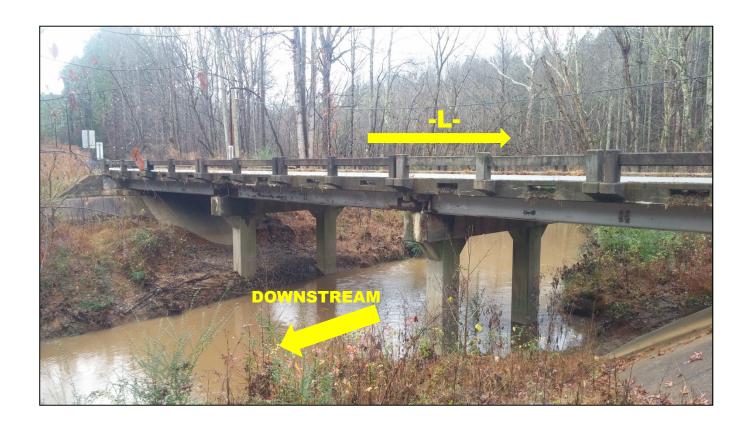
Checked By:

KC

Date:

12/9/15

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GRASSY CREEK, LOOKING UPSTREAM TOWARDS EXISTING BRIDGE.



-L-, LOOKING DOWNSTATION FROM STA. 17+00.

SITE PHOTOGRAPHS

BRIDGE NO. 138 ON SR 1300 (CORNWALL RD) OVER GRASSY CREEK WBS NO.: 42342.1.1, TIP NO.: B-5166

