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NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

SOIL TEST RESULTS

5000

LINE	STATION	PLAN	PROFILE	XSECT
- Y	11+00.00 to 45+00.00	4-6	10 - 11	
-Y2-	10+50.00 to 26+53.73	8,9	11 - 12	16 - 30
-Y3-	10+00.00 to 23+79.29	4,8	12	31 - 38
-RPA-	10+00.00 to 24+41.71	7,6,5	13	39 - 46
-RPD-	10+00.00 to 35+90.81	7,6,5,4	13 - 14	47 - 66
-LPB-	10+00.00 to 16+99.28	5	14	
-TRAIL-	14+50 to 31+58.83	6,5	15	
IL TEST RESUL	TS	67		

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 41153.1.1 (I-5000) _ F.A. PROJ. *IMF-85-1(113)1*7 COUNTY **GASTON** PROJECT DESCRIPTION <u>I-85 /US 321 INTERCHANGE GEOMETRIC</u> SAFETY IMPROVEMENTS

> **INVENTORY** (REVISED)

STATE STATE PROJECT REFERENCE NO. 67 N.C. I-5000 1 STATE PROJ.NO. IMF-85-1(113)17

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORNIC LOOS, ROCK CORES, AND SOL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN ARLEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRAINSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1999 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS. NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A CENERAL SQL AND ROCK STRAIA DESCRIPTIONS AND INDICATED BOUNDARES ARE BASED OIL A COTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRAIA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UN-PLACETEST DATA CAN BE RELIED ON ONLY TO THE DECREE OF RELIGABILITY INNERSETH IN THE STRANDARD TEST METHOD. THE OBSERVED WOTER LEVELS OR SOL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC CONDITIONS INCLUDING

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSUPFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FIRML DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OF ACCURACY OF THE INVESTIGATION MADE, NOR THE MITTERPRETATIONS MADE, OR DIFFORM OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INSOPENDENT USBURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATEFY MASSLE AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY BEASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AY THE SITE OFFERING FROM THESE INFORMATION. THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS

J.K. STICKNEY
C.L. SMITH
JP ROGERS
S&ME
J.P. ROGERS
C.B. LITTLE
C.B. LITTLE
JUNE 2014

PERSONNEL



NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS. SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

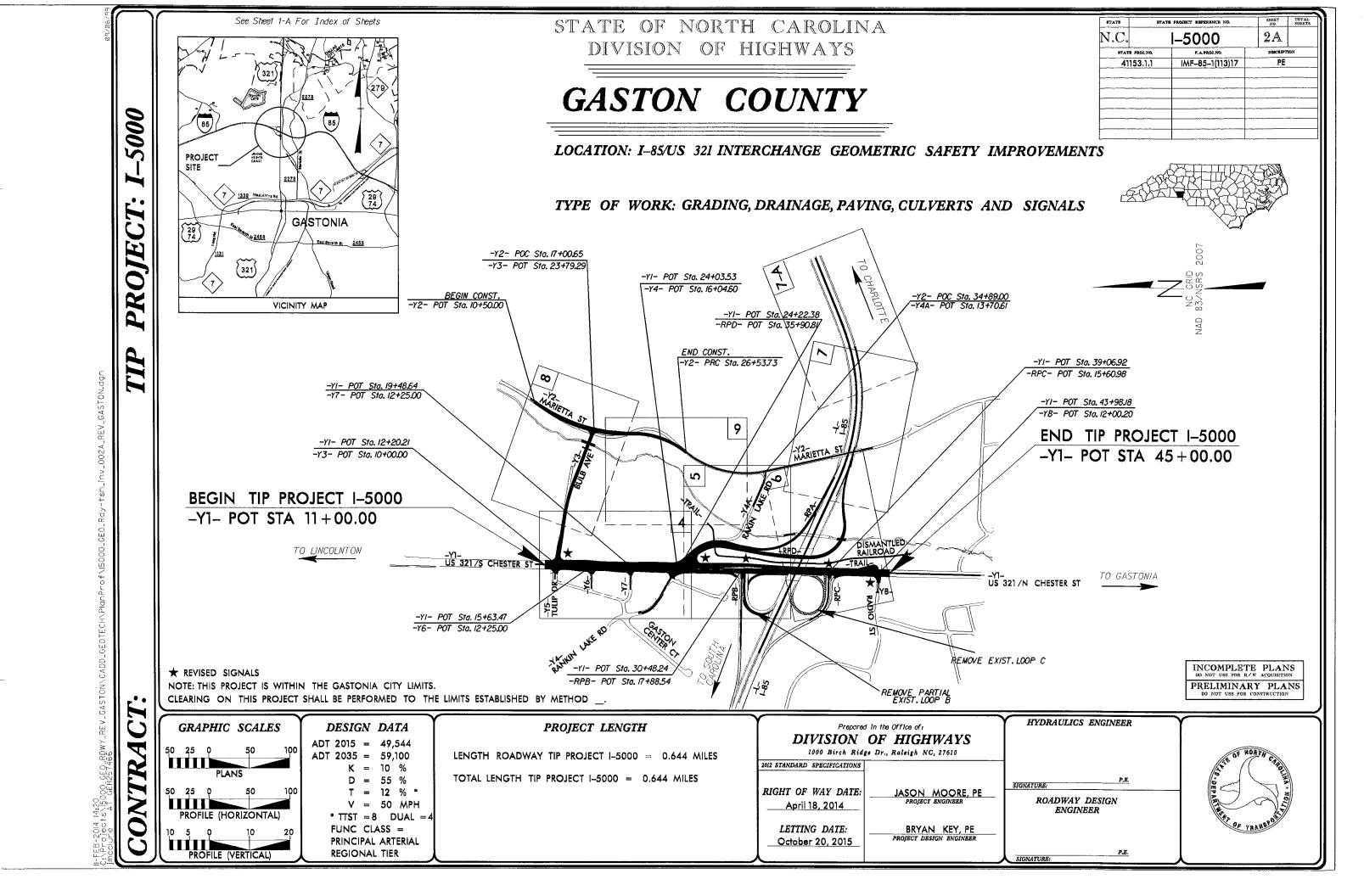
007	DECORPTATION								
SOIL DESCRIPTION		VE	GRADATION CONTRACTOR AND STREET OF THE PROPERTY OF THE PROPERT		ROCK DESCRIPTION			TERMS AND DEFINITIONS	
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS		ils <u>uni</u>	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO		HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.			ALLUYIUM (ALLUY.) - SDILS THAT HAVE BEEN TRANSPORTED BY WATER.	
THAT CAN BE PENETRATED WITH A CONTINUOUS 100 BLOWS PER FOOT ACCORDING TO STANDARD	PENETRATION TEST (AASHTO T206, ASTM D-1586). S	POO	DORLY GRADED) AP-GRADED - INDICATES A MIXTI	TURE OF UNIFORM PARTICUES OF TWO OR M	AUBE 612E6			SAMPLER EQUAL TO OR LESS THAN &1 FOOT PER 60 BLOWS.	ADUIFER - A WATER BEARING FORMATION OR STRATA.
CLASSIFICATION IS BASED ON THE AASHTO SYST	TEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUE		GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS		OF WEATHERED	IL PLAIN MATERIAL, THE TRANSITIO	ON BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.	
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC, EXAMPLE:			THE ANGUL ARITY OR POLINONESS	S OF SOIL GRAINS IS DESIGNATED BY THE T	TERMS, ANGULAR	RDCK MATERIAL	LS ARE TYPICALLY DIVIDED AS FOLL	_OWS:	ARCILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS,
	WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6		SUBANGULAR, SUBROUNDED, OR RO		TEMOS MISSOCIAL	WEATHERED		LAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100	OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
		_		MINERALOGICAL COMPOSITION)N	ROCK (WR)	DEDNO TEN 1001		ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL
GENERAL GRANULAR MATERIALS	D AASHTO CLASSIFICATION SILT-CLAY MATERIALS	MINI		Z, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE US		CRYSTALLINE	FINE TO COARSE	E GRAIN IGNEOUS AND METAMORPHIC ROCK THAT PT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE,	AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE,
CLASS. (\$\leq 35\% PASSING *200)	(> 35% PASSING #200) ORGANIC MA		HENEVER THEY ARE CONSIDERED		SED IN DESCRIPTIONS	ROCK (CR)	GNEISS, GABBRO,		CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 COMPRESSIBILITY		NON-CRYSTALLINE		GRAIN METAMORPHIC AND NON-COASTAL PLAIN	COLLUYJUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM				
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-	6-7-5 A-3 A-6		SLIGHTLY COMPRESSIBL		LESS THAN 31	ROCK (NCR)	SEDIMENTANT NO	DCK THAT WOULD YEILD SPT REFUSAL IF TESTED, ROCK TYPE LITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
SYMBOL 000000000000000000000000000000000000	A-7-6		MODERATELY COMPRESS	SSIBLE LIQUID LIMIT I	EQUAL TO 31-50	CDASTAL PLAIN	COASTAL PLAIN S	SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL
0000000000			HIGHLY COMPRESSIBLE		GREATER THAN 50	SEDIMENTARY ROC	SPT REFUSAL. RO	ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
% PASSING	SIL			PERCENTAGE OF MATERIAL		100.7		ATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
* 10 58 MX * 40 39 MX 58 MX 51 MN	GRANULAR CLA SDILS SOILS	MUCK, PEAT		GRANULAR SILT - CLAY SOILS SOILS	OTHER MATERIAL				ROCKS OR CUTS MASSIVE ROCK.
* 200 15 MX 25 MX 10 MX 35 MX 35 MX 35 H	1X 35 MX 36 MN 36 MN 36 MN 36 MN 501L5 SOIL	TRA	RACE OF ORGANIC MATTER	2 - 3½ 3 - 5½ TRAC	ACE 1 - 10%		ICK FREBH.CRYSTALS BRIGHT,FEW JO MMER IF CRYSTALLINE.	OINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
LIQUID LIMIT 40 MY 41 MN 40 M	12 41 MM 42 M2 41 MM 42 M2 41 MM			3 - 5% 5 - 12% LITT		1		NED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN.	HORIZONTAL.
ו פון ווו זרן אוו פדן	AX 41 MN 48 MX 41 MN 48 MX 41 MN SOILS WITH 11 MN 18 MX 18 MX 11 MN 11 MN LITTLE OR	1 Luic		5 - 10% 12 - 20% SOMI >10% >20% H1GH				CE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
	4 MX 8 MX 12 MX 16 MX No MX MODERATE	DRGANIC TIL			TET SOX HIND HOUTE	OF.	A CRYSTALLINE NATURE.		
	AMOUNTS OF	10000		GROUND WATER	~			NED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STONE FRAGS, FINE SILTY OR CL.		1 1 .		EL IN BORE HOLE IMMEDIATELY AFTER DE	RILLING			AY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR . CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND	SAND SOILS SOILS MATTER		STATIC WAT	ITER LEVEL AFTER 24 HOURS				DISCOLORATION AND WEATHERING EFFECTS. IN	
GEN, RATING	FAIR TO		VPW PERCHED WA	ATER, SATURATED ZONE, OR WATER BEARIN	ATAGES DA			RE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
AS A EXCELLENT TO GOOD SUBGRADE	FAIR TO POOR POOR PO	UNSULTABLE	_	ATEN, SHIONHIED ZONE, ON WHICH BEHALD	NO STORIA	DUI	ILL SOUND UNDER HAMMER BLOWS AND	ND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY
	L - 30 ; P1 OF A-7-6 SUBGROUP 1S > LL -	_ (O-∭ SPRING OR S	SEEP			TH FRESH ROCK.		THE STREAM.
	ENCY OR DENSENESS	'		MISCELLANEOUS SYMBOLS				D OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL DW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN
	DANCE OF CTANDARD PANCE OF UN	NFINED .						DGIST'S PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	THE FIELD.
PRIMARY SOIL TYPE COMPACTNESS OF CONSISTENCY	PENETRATION RESISTENCE COMPRESSIVE		ROADWAY EMBANKME		NG TEST BORING W/ CORE		TESTED, WOULD YIELD SPT REFUSAL		JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
	(N-VALUE) (TONS/I)	WITH SOIL DESCRIPT		_			D OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
GENERALLY VERY LOOSE	(4		FOIL SYMBOL	AUGER BORING	SPT N-VALUE			ANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME	ITS LATERAL EXTENT,
BRANULAR MEDIUM DENCE	4 TO 10 10 TO 30 N/	 	ARTIFICIAL FILL (AF	AEL OTHER	OFF OFF PERION		TENT. SOME FRAGMENTS OF STRONG TESTED, YIELDS SPT N VALUES > 10		LENS - A BODY OF SDIL OR ROCK THAT THINS DUT IN ONE OR MORE DIRECTIONS.
MATERIAL DENSE	30 TO 50		THAN ROADWAY EMBI		REF SPT REFUSAL	1		D OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN
VERY DENSE	>50	1	INFERRED SOIL BOU	HV	(1)			TO SOIL STATUS, WITH DNLY FRAGMENTS OF STRONG ROCK	SDILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY SOFT	<2 <0.2		INFERRED SUIL BOOK		_			OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
GENERALLY SOFT SILT-CLAY MEDIUM STIFF	2 TO 4 0.25 TO 4 TO 8 P.5 TO		INFERRED ROCK LIN	NE A PIEZOMETER INSTALLATION		1		RIC REMAIN. IF TESTED, YIELDS SPT N YALUES < 100 BPF	INTERVENING IMPERVIOUS STRATUM.
SILT-CLAY MEDIUM STIFF MATERIAL STIFF	4 10 8 8 TO 15 1 TO		Trad ALLUVIAL SOIL BOU		20			NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
(COHESIVE) VERY STIFF	15 TO 30 2 TO		A PAR ALEDAINE SOIL BOOK	INSTALLATION	п		ATTERED CONCENTRATIONS. DUARTZ M SO AN EXAMPLE.	MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
HARD	>30 >4	25/0	DIP & DIP DIRECTIO	ION OF _				HARDNESS	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
TEXTUR	RE OR GRAIN SIZE		ROCK STRUCTURES	CONE PENETROME	EIER IESI		-		1
HE CID CIEVE CITE	10 40 60 200 270			SOUNDING ROD				SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 OPENING (MM) 4.76	2,00 0.42 0.25 0.075 0.053						EVERAL HARD BLOWS OF THE GEOLOG		SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
				ABBREVIATIONS			AN BE SCRATCHED BY KNIFE OR PICK D DETACH HAND SPECIMEN.	K ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL
BOULDER COBBLE GRAVEL	COARSE FINE SILT		AR - AUGER REFUSAL	MED MEDIUM	VST - VANE SHEAR TEST	1			TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
(BLDR.) (COB.) (GR.)	(CSE. SD.) (F SD.) (SL.)		BT - BORING TERMINATED	MICA MICACEDUS	WEA WEATHERED				
GRAIN MM 305 75	2.0 0.25 0.05						YCAVATED BY HARD BLOW OF A GEOL	CK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR
SIZE IN. 12 3		രാ പ	CL CLAY CPT - CONE PENETRATION TE	MDD MODERATELY			XCAVATED BY HARD BLOW OF A GEOL Y MODERATE BLOWS.	CR. GUUGES OR GROUVES ID 0.25 INCHES DEEP CAN BE DLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	SLIP PLANE,
		, ,	CPT - CONE PENETRATION TE CSE COARSE	MDD MODERATELY	7 - UNIT WEIGHT 76 - DRY UNIT WEIGHT	MEDIUM C	Y MODERATE BLOWS. AN BE GROOVED OR GOUGED 0.05 INC	DLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED ICHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SOIL MOISTURE	- CORRELATION OF TERMS		CPT - CONE PENETRATION TE CSE COARSE DMT - DILATOMETER TEST	MOD, - MODERATELY EST NP - NON PLASTIC ORG, - ORGANIC PMT - PRESSUREMETER TEST	√g− DRY UNIT WEIGHT SAMPLE ABBREVIATIONS	MEDIUM C HARD C	LY MODERATE BLOWS. CAN BE GRODVED OR GOUGED 0.05 INC CAN BE EXCAVATED IN SMALL CHIPS	DLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH
	- CORRELATION OF TERMS	0	CPT - CONE PENETRATION TE CSE COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION	MOD MODERATELY REST NP - NON PLASTIC ORG ORGANIC PMT - PRESSUREMETER TEST N TEST SAP SAPROLITIC	√g- DRY UNIT WEIGHT SAMPLE ABBREVIATIONS S - BULK	B MEDIUM C HARD C P	LY MODERATE BLOWS. CAN BE GRODVED OR GOUGED 0.05 INC CAN BE EXCAVATED IN SMALL CHIPS TO POINT OF A GEOLOGIST'S PICK.	DLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED CHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SOIL MOISTURE BCALE FIE		ESCRIPTION 6	CPT - CONE PENETRATION TE CSE COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION • - VOID RATIO	MOD MODERATELY 'EST NP - NON PLASTIC ORG ORGANIC PMT - PRESSUREMETER TEST N TEST SAP SAPROLITIC SD SAND, SANDY	√g - DRY UNIT WEIGHT SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON	MEDIUM CI HARD CI PI SOFT CI	Y MODERATE BLOWS. AN BE GROOVED OR GOUGED 0.05 INC. AN BE EXCAVATED IN SMALL CHIPS. OINT OF A GEOLOGIST'S PICK. AN BE GROVED OR GOUGED READILY	DLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED ICHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF) OF A 140 LB. HAMMER FACLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO DR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
SOIL MOISTURE SCALE FIE (ATTERBERG LIMITS) DE	LD MOISTURE GUIDE FOR FIELD MOISTURE	ESCRIPTION 6	CPT - CONE PENETRATION TE CSE COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION	MOD MODERATELY REST NP - NON PLASTIC ORG ORGANIC PMT - PRESSUREMETER TEST N TEST SAP SAPROLITIC	√g- DRY UNIT WEIGHT SAMPLE ABBREVIATIONS S - BULK	MEDIUM C. HARD C. PI SOFT C.	Y MODERATE BLOWS. AN BE GROOVED OR GOUGED 0.05 INC. AN BE EXCAVATED IN SMALL CHIPS. OINT OF A GEOLOGIST'S PICK. AN BE GROVED OR GOUGED READILY	DLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED NCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH DUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO DR LESS THAN 8.1 FOOT PER 60 BLOWS. STRATA COBE RECOVERY (SBEC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
SOIL MOISTURE SCALE FIE (ATTERBERG LIMITS) DE	LD MOISTURE GUIDE FOR FIELD MOISTURE	ESCRIPTION 6 FOULLY FOULTH TABLE F	CPT - CONE PENETRATION TE CSE COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION e - VOID RATIO F - FINE FOSS FOSSILIFEROUS FRAC FRACTURED, FRACTURE	MOD MODERATELY TEST NP - NON PLASTIC ORG ORGANIC PMT - PRESSUREMETER TEST N TEST SAP SAPROLITIC SD SAND, SANDY SL SILT, SILTY SLI SLIGHTLY TCR - TRICONE REFUSAL	SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL	MEDIUM CI HARD C. PI SOFT CI FI VERY CA	IT MODERATE BLOWS. AN BE GROUVED OR GOUGED 0.05 INC. AND BE EXCAVATED IN SMALL CHIPS - DINT OF A GEOLOGIST'S PICK. AN BE GROVED OR GOUGED READILY. RECES CAN BE BROKEN BY FINGER BY. AN BE CARVED WITH KNIFE. CAN BE	DLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED ICHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PRESSURE. EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPDON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO DR LESS THAN 8.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK DUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESIGNATION (SROD) -
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SOIL MOISTURE BCALE (ATTERBERG LIMITS) LL LIOUID LIMIT PLASTIC (P1) PL PLASTIC LIMIT	GUIDE FOR FIELD MOISTURE SATURATED - USUALLY LIQUID; VERY WET FROM BELOW THE GROUND - WET - (W) GUIDE FOR FIELD MOISTURE SEMISOLID; REQUIRES DRYIF ATTAIN OPTIMUM MOISTURE	SUALLY FOR TABLE	CPT - CONE PENETRATION TE CSE COARSE DMT - OILATOMETER TEST DPT - DYNAMIC PENETRATION e - VOID RATIO F - FINE FOSS FOSSILIFEROUS FRAC FRACTURED, FRACTURE FRAGS FRAGMENTS HI HIGHLY EQUIF	MOD MODERATELY NP - NON PLASTIC ORG ORGANIC PMT - PRESSUREMETER TEST N TEST SAP SAPROLITIC SD SAND, SANDY SL SILT, SILTY SLI SLIGHTLY RES TCR - TRICONE REFUSAL # - MOISTURE CONTENT V - VERY PMENT USED ON SUBJECT P ADVANCING TOOLS: CLAY BITS	A- DRY UNIT WEIGHT SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO PROJECT	MEDIUM C. HARD C. HARD PH SOFT C. FI VERY C, SDFT OF FRAC	IT MODERATE BLOWS. IAAN BE GRODVED OR GOUGED 0.05 INC. IAAN BE EXCAVATED IN SMALL CHIPS: IDINT OF A GEOLOGIST'S PICK. IAAN BE GROVED OR GOUGED READILY IMPORT CHIPS TO SEVERAL INCHES IN SIECES CAN BE BROKEN BY FINGER PI AN BE CARVED WITH KNIFE. CAN BE R MORE IN THICKNESS CAN BE BROKE INGERNAIL. CTURE SPACING MORE THAN 10 FEET 3 TO 10 FEET	DLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED KCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PRESSURE. EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH KEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY BEDDING TERM THICKNESS VERY THICKLY BEDDED 1.5 - 4 FEET THINKLY BEDDED 1.6 - 1.5 FEET	SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF) OF A 148 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPDON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO DR LESS THAN 8.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO DR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. IOPSDIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
SOIL MOISTURE BCALE (ATTERBERG LIMITS) LL LIOUID LIMIT PLASTIC (PI) PL OM. OPTIMUM MOISTURE SL SHRINKAGE LIMIT	GUIDE FOR FIELD MOISTURE SCRIPTION SATURATED - USUALLY LIQUID; VERY WET (SAT.) FROM BELOW THE GROUND WET - (W) SEMISOLID; REQUIRES DRYIN ATTAIN OPTIMUM MOISTURE MOIST - (M) SOLID: AT OR NEAR OPTIM REQUIRES ADDITIONAL WAT	ESCRIPTION BUALLY TER TABLE TD I MOISTURE	CPT - CONE PENETRATION TE CSE COARSE DMT - OILATOMETER TEST DPT - DYNAMIC PENETRATION e - VOID RATIO F - FINE FOSS FOSSILIFEROUS FRAC FRACTURED, FRACTURE FRAGS FRAGMENTS HI HIGHLY EQUIF DRILL UNITS: MOBILE B-	MOD MODERATELY NP - NON PLASTIC ORG ORGANIC PMT - PRESSUREMETER TEST N TEST SAP SAPROLITIC SD SAND, SANDY SL SILT, SILTY SLI SLIGHTLY RES TCR - TRICONE REFUSAL # - MOISTURE CONTENT V - VERY PMENT USED ON SUBJECT P ADVANCING TDOLS:	SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SPLIT SPOON ST - SPLIT SPOON ST - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO PROJECT HAMMER TYPE:	MEDIUM C. HARD C. HARD C. PI SOFT C. FI SOFT GI SOFT GI SOFT GI FRAC IERM VERY MIDE MIDE MODERATELY CLOSE	IN MODERATE BLOWS. AN BE EXCAVATED IN SMALL CHIPS - CHAN BE EXCAVATED IN SMALL CHIPS - CHAN BE EXCAVATED IN SMALL CHIPS - CHAN BE GROVED OR GOUGED READILY ROM CHIPS TO SEVERAL INCHES IN SIECES CAN BE BROKEN BY FINGER PI AN BE CARVED WITH KNIFE. CAN BE ROMER IN THICKNESS CAN BE BROKE INGERNAIL. CTURE SPACING SPACING MORE THAN 10 FEET 3 TO 10 FEET CLOSE 1 TO 3 FEET 0.16 TO 1 FEET	DLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED NCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PRESSURE. EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH KEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY BEODING TERM VERY THICKLY BEODED VERY THICKLY BEODED VERY THICKLY BEODED VERY THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.40 - 1.6 FEET	SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH DUTSIDE DIANCTER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 8.1 FOOT PER 60 BLOWS. STRATA COBE RECOVERT (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK DUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. IOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. BENCH MARK: ELEVATION: FT.
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REVISED 09/23/09

SHEET NO.

PROJECT REFERENCE NO.

4||53.|.| (|-5000)







STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

Pat McCrory
GOVERNOR

Anthony Tata SECRETARY

March 6, 2014

STATE PROJECT:

41153.1.1 (I-5000)

FEDERAL PROJECT:

IMF-85-1(113)17

COUNTY:

Gaston

DESCRIPTION:

I-85/US 321 Interchange Geometric Safety Improvements

SUBJECT:

Geotechnical Report – Inventory

Due to a design revision on Ramp D, this report supersedes the report written on September 12, 2013.

PROJECT DESCRIPTION

This project is located in south-central Gaston County in the City of Gastonia. In addition to the existing I-85/US 321 Interchange, the proposed project includes upgrades to Bulb Ave. (Y3) and Marietta Street (Y2). A slight relocation and extension to a Greenway Trail (TRAIL) that runs through the project corridor has also been included. In addition, several culverts and culvert extensions have been proposed over the creek that runs parallel to US 321 thru the project corridor. The culvert at –Y3- and Hardin creek was originally investigated as a bridge crossing in 2006. Two of the borings from that investigation have been included in this project. The following alignments were investigated:

11+00.00 to 45+00.00 (0.66 miles
10+50.00 to 26+53.73 (0.30 miles
10+00.00 to 23+79.29 (0.26 miles
10+00.00 to 24+41.71 (0.27 miles)
10+00.00 to 35+90.81 (0.49 miles)
14+50.00 to 31+58.83 (0.32 miles)

The total length of lines investigated is 2.30 miles. The original NCDOT field investigation of the new interchange was conducted in April and May of 2013. After receiving the revised plans in December 2013, S&ME was contracted to return to the site to perform additional Landfill borings.

MAILING ADDRESS:

NC DEPARTMENT OF TRANSPORTATION GEOTECHNICAL ENGINEERING UNIT 1589 MAIL SERVICE CENTER RALEIGH NC 27699-1589 TELEPHONE: 919-707-6850 FAX: 919-250-4237

WEBSITE: www.ncdot.gov/doh/preconstruct/highway/geotech

LOCATION:
CENTURY CENTER COMPLEX
ENTRANCE B-2
1020 BIRCH RIDGE DRIVE
RALEIGH NC

All borings performed during the NCDOT phase of the investigation were conducted with a CME-550X drill machine with an automatic hammer. Standard Penetration Tests were performed utilizing hollow stem augers with carbide insert teeth. Forty-two soil samples were submitted to the Materials and Tests Unit for laboratory analysis.

AREAS OF SPECIAL GEOTECHNICAL INTEREST

Artificial Fill: An old landfill was encountered within the project corridor under two proposed alignments:

<u>Line</u>	Station(s)	<u>Offset</u>
-Ramp A-	13+77 to 17+55	Right.
-Ramp D-	14+08 to 17+65	Left and Right

Alignment Ramp D crosses the edge of the largest landfill which is south of I-85 and east of US 321. Ramp A crosses a smaller landfill which is north of I-85 and east of US 321. It is unclear at this time whether or not either landfill extends underneath existing I-85.

After initially finding these landfill materials and roughly determining their extent, a private engineering firm (S&ME) was called in to do a detailed study of the two areas. In addition to a geotechnical study of the landfill areas, S&ME's investigation included a geophysical and environmental assessment. This report will include their geotechnical findings while the other reports are available from the Geotechnical Engineering Unit's Geoenvironmental branch.

This was an unregulated landfill that was active from 1930 until the late 1950's. The deepest landfill materials encountered extended up to 19.5' below the ground surface in a boring performed around Ramp A. Trace amounts of wood debris, brick, glass, gravel, and micaceous soil were encountered in the borings performed in these two areas. Based on the environmental work done in the area, arsenic, benzene and asbestos were found in both landfill sites. Please refer to the attached plan views, profiles, and cross-sections for a complete graphical depiction of these areas.

The artificial fill encountered on the –TRAIL- alignment (stas. 22+50 to 29+30) is not associated with the landfill. This fill was built to accommodate development adjacent to existing US 321N. Two businesses and a parking area for both currently occupy this property.

Crystalline Rock: Crystalline rock was encountered within 10' of grade at the following locations:

<u>Line</u>	Station(s)		
-Y2-	10+50 to 20+50		
-Y3-	16+50 to 20+50		

Please refer to the attached plan views, profiles, and cross-sections for a complete graphical depiction of these alignments. Rock outcrops are visible on the surface at or near Stations 15+00 to 17+00 -Y2- and in the creek bed at approximately Station 22+40 -Y3-.

Alluvial Soils: This project falls within the Long Creek basin with Highland Creek serving as the primary drainage outlet. Highland Creek parallels US 321 on the eastside and flows south to north. Alluvial materials encountered in its floodplain are up to 19' thick and consist primarily of sandy silts (A-4), silty sands (A-2-4), and coarse sands (A-3) with gravel. In addition, a six-foot lens of stiff,

sandy clay (A-7-5) with gravel was encountered on the east side of Hardin Creek at the proposed Ramp D. Groundwater, where encountered, was between elevations 690' and 700'. Alluvial deposits are thicker on the eastern side of the creek. Maximum fill heights over these deposits are approximately 25' and are associated with Ramp A.

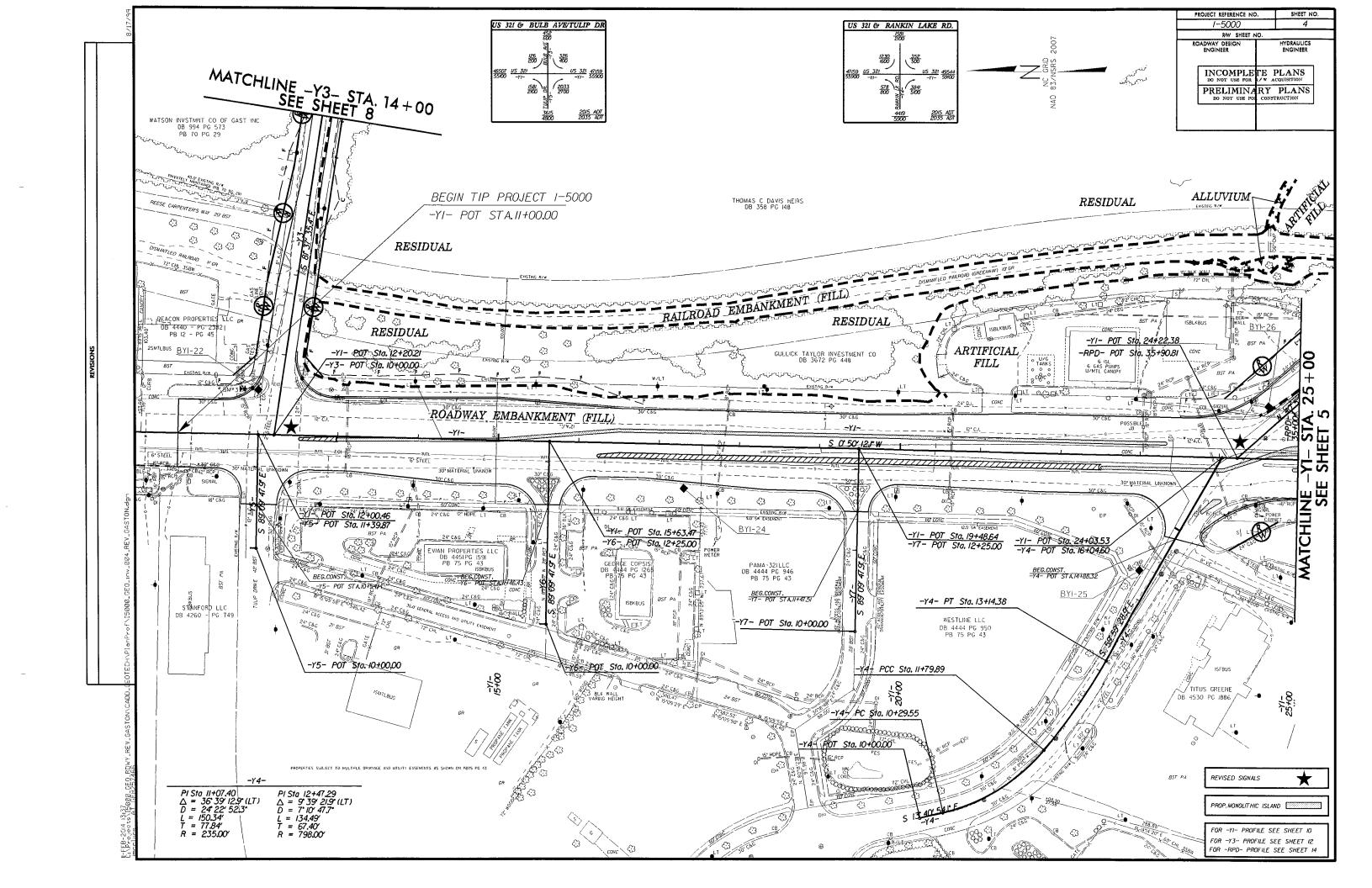
SOIL PROPERTIES

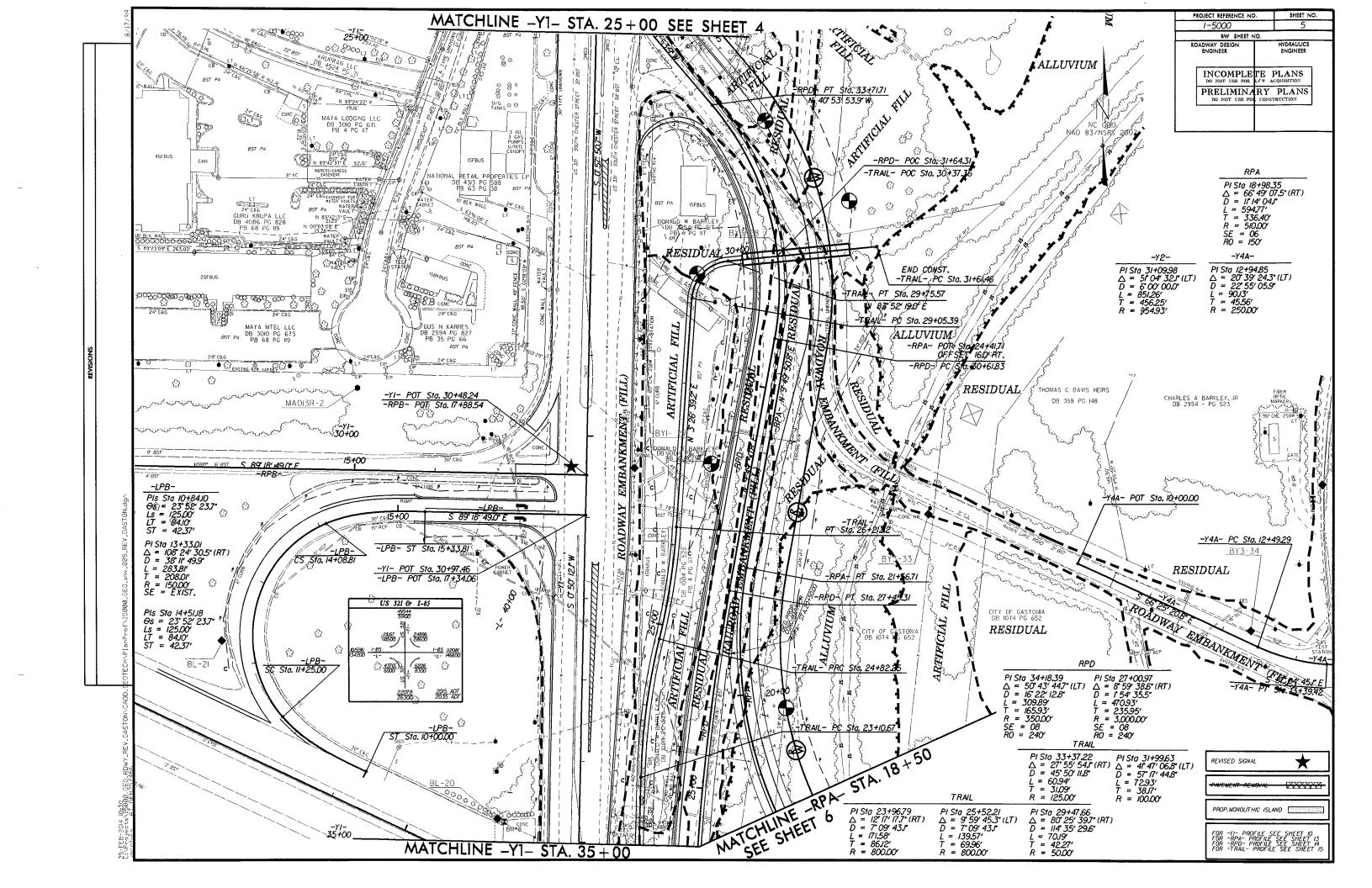
Residual Soils

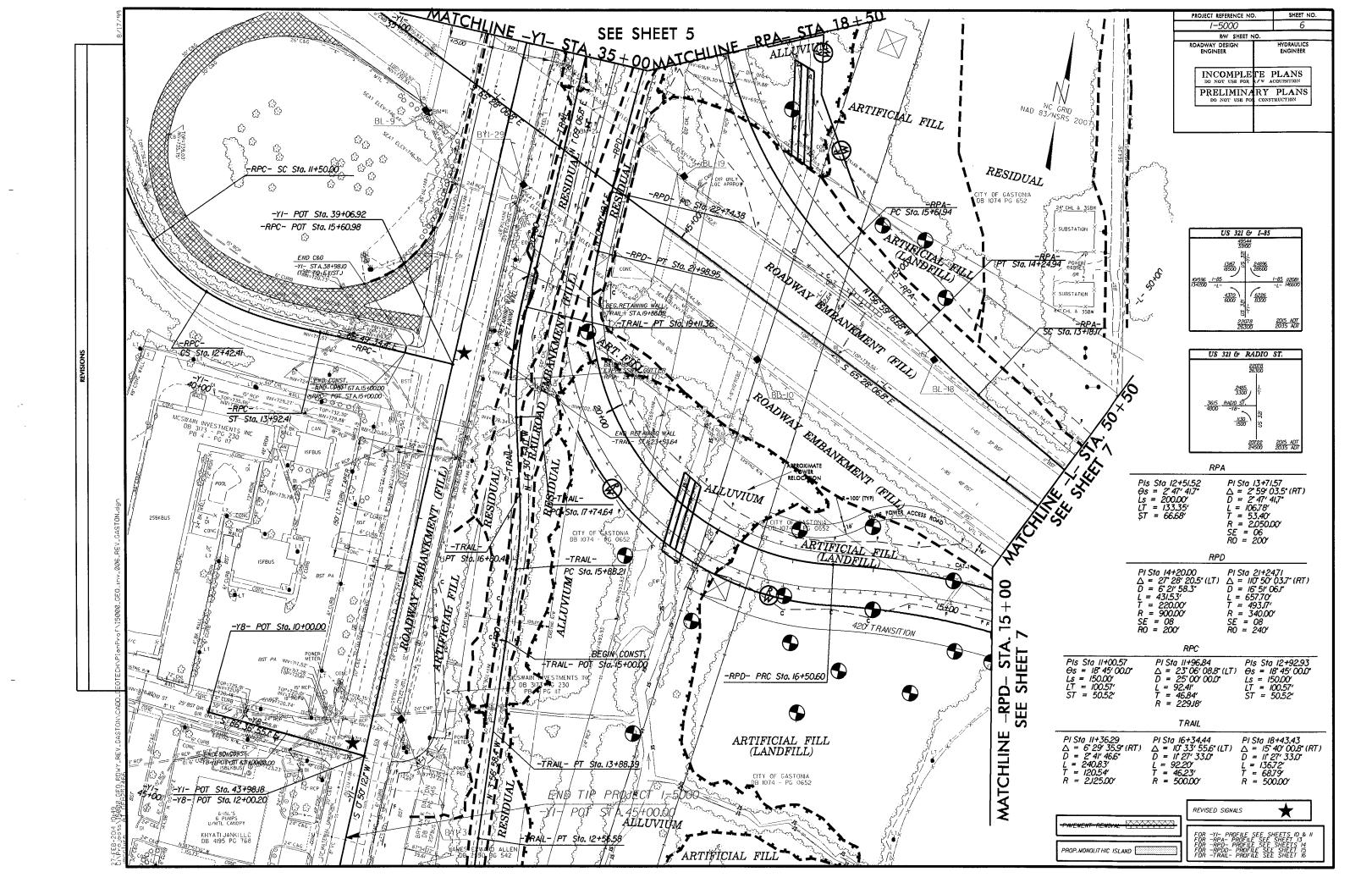
All residual soils on the project are derived from the intrusive, quartz-rich (PPmg) rocks encountered within the project corridor. Crystalline and severely weathered crystalline rock was encountered in the cut sections between Stations 10+50 to 20+50 –Y2-and 18+50 to 20+50 –Y3-. The dominant residual soil type encountered is sandy clay (A-7-6, A-6) and silty sand (A-2-4). Sandy silt (A-4) is also present within the project corridor, but in lesser concentrations.

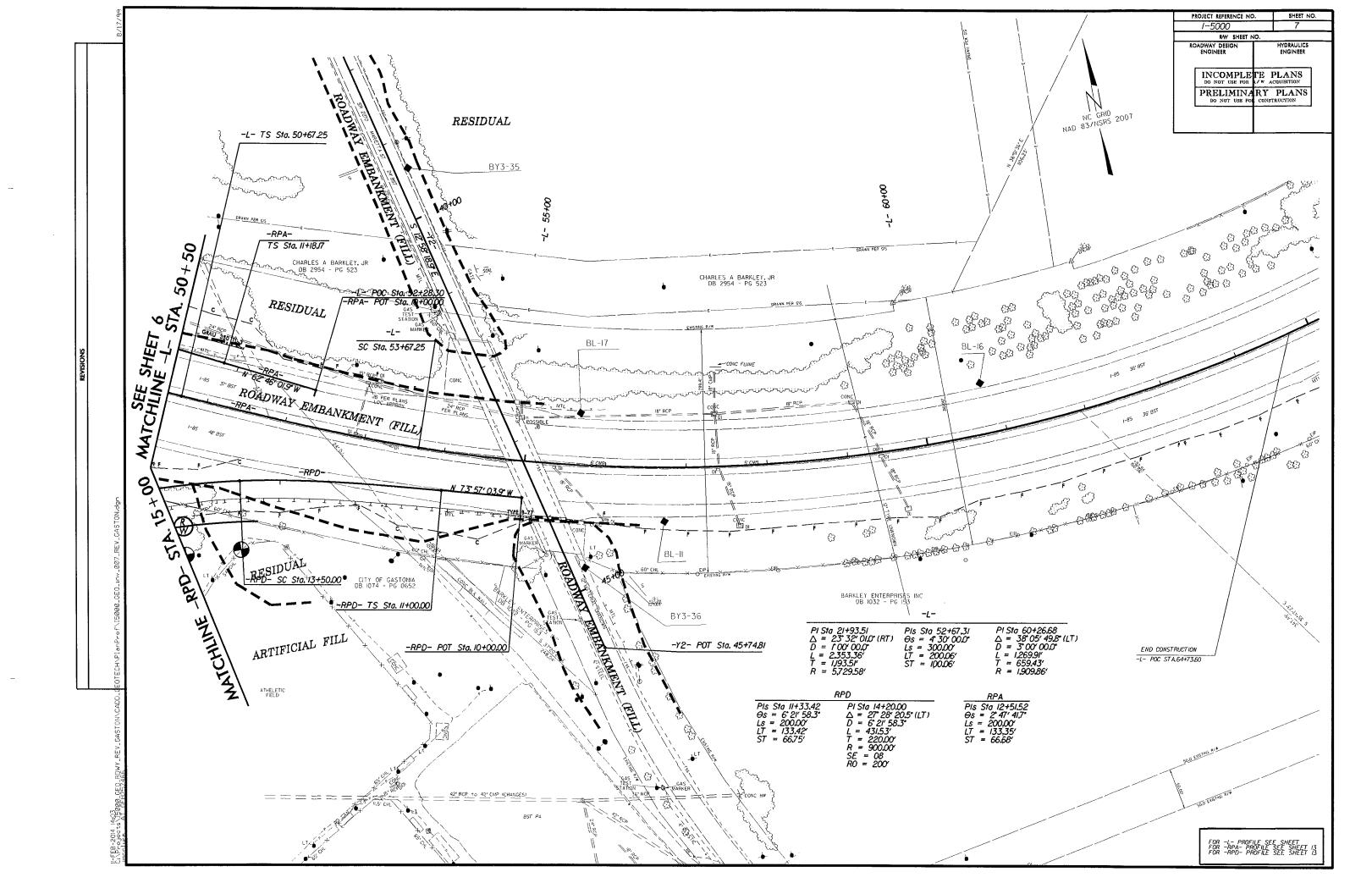
Respectfully submitted,

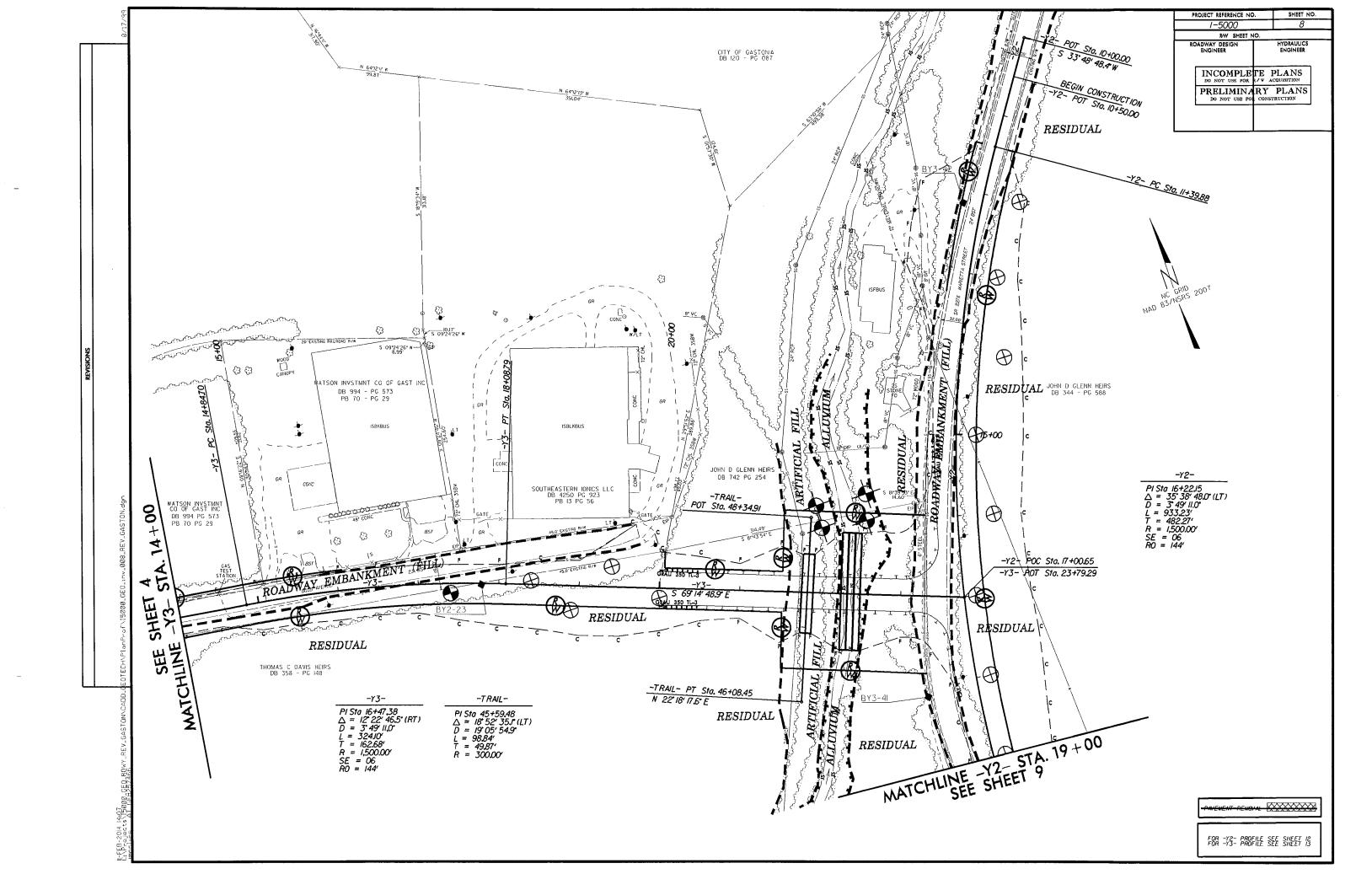
John P. Rogers Project Geological Engineer 3B

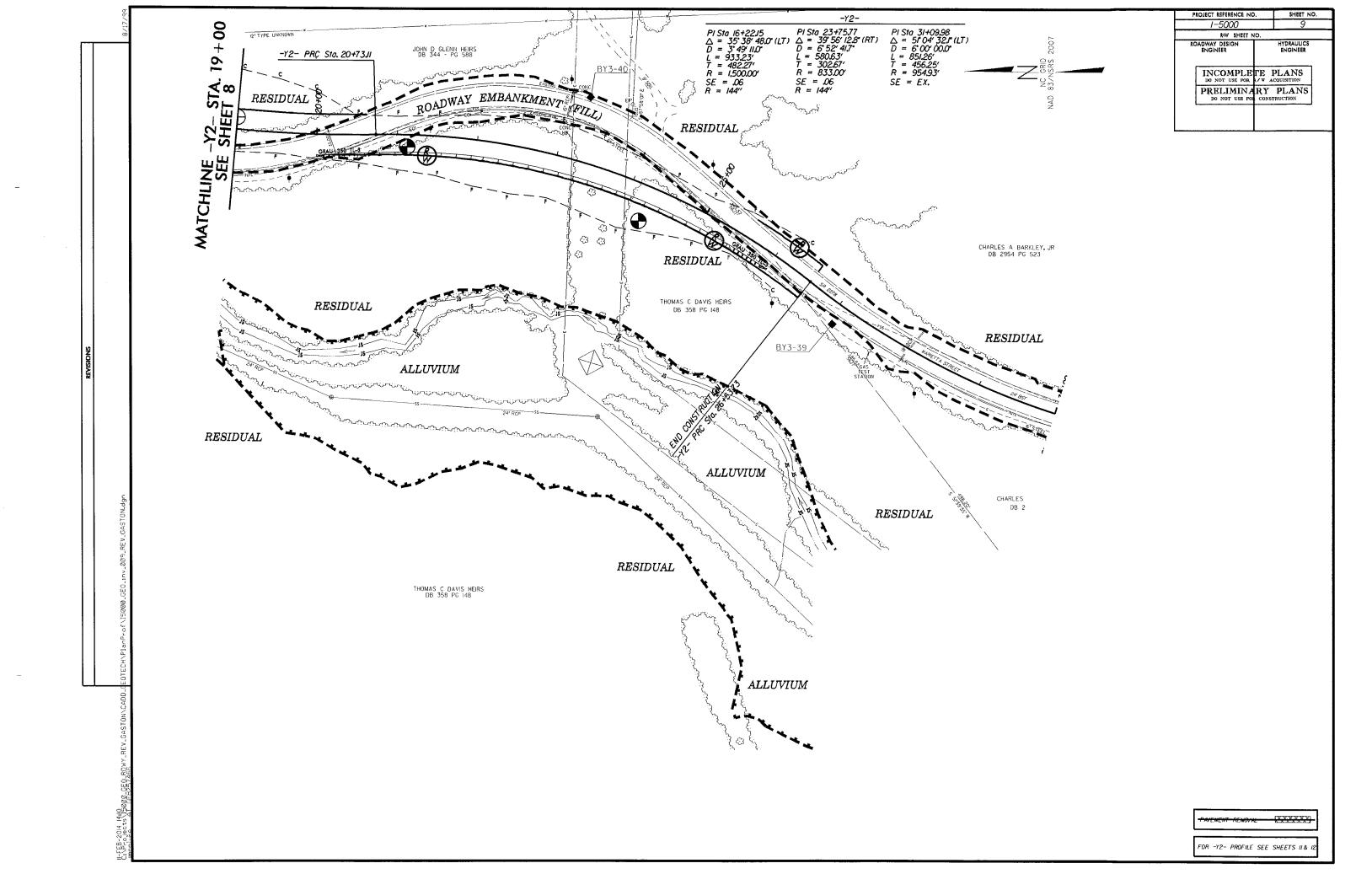


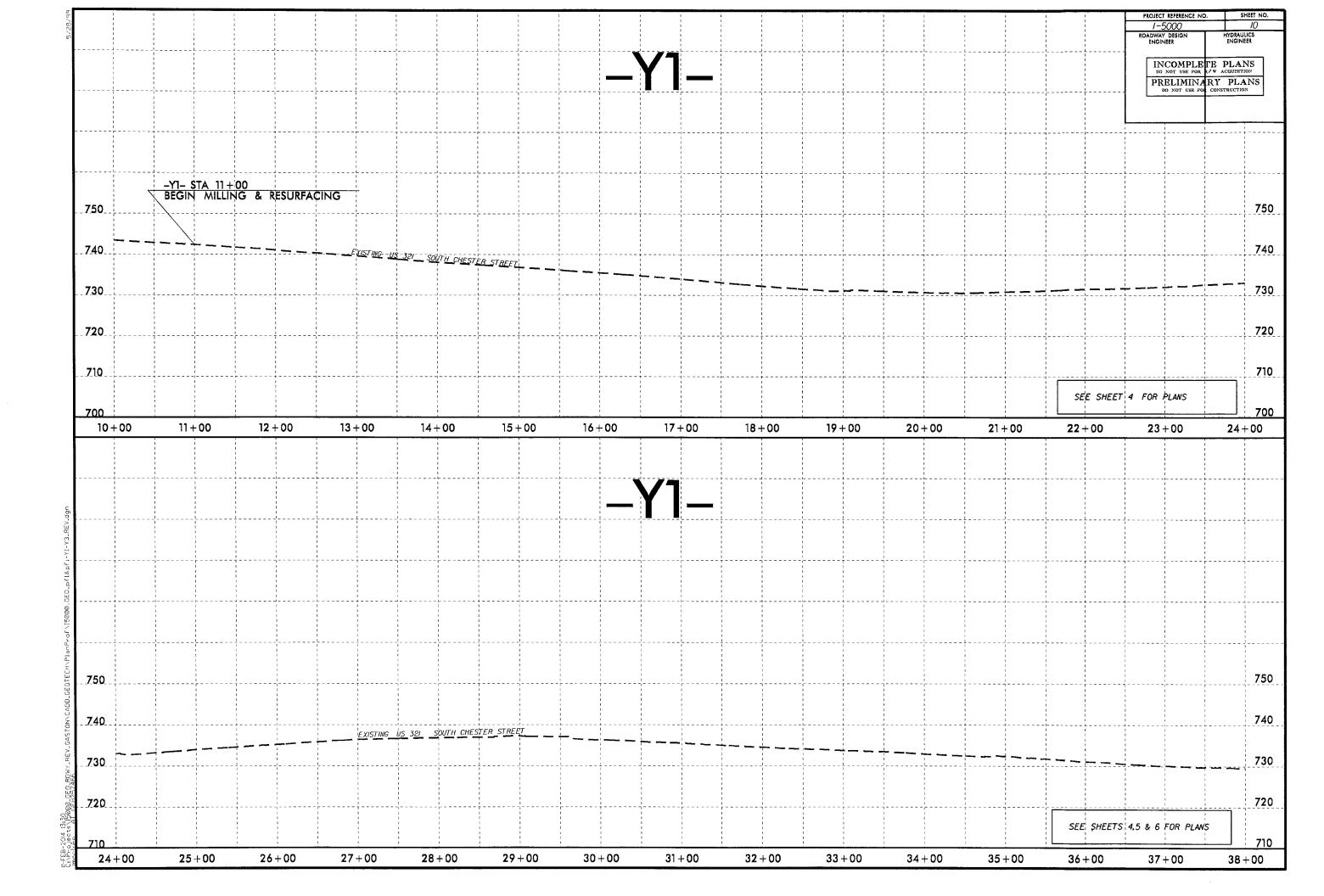


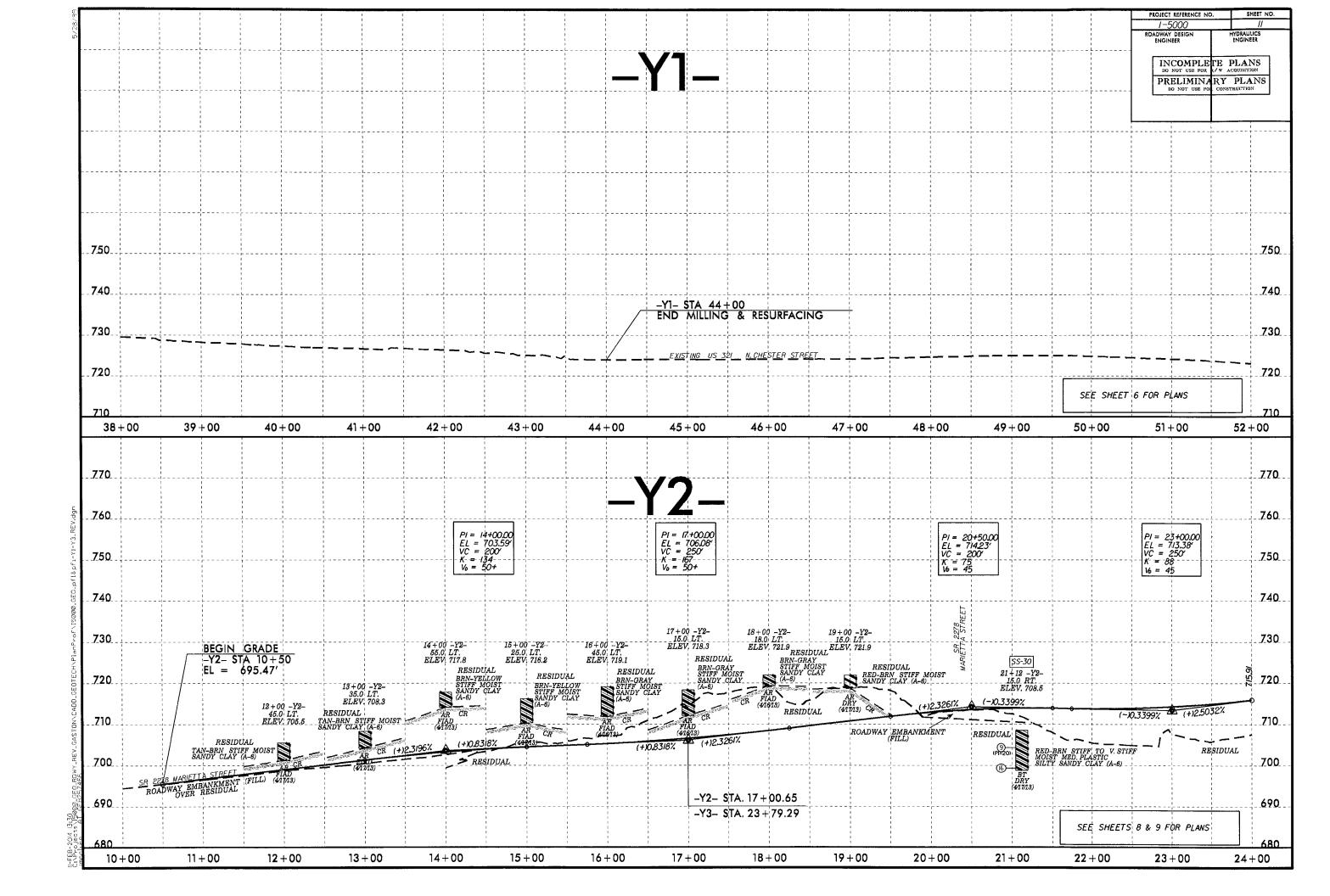


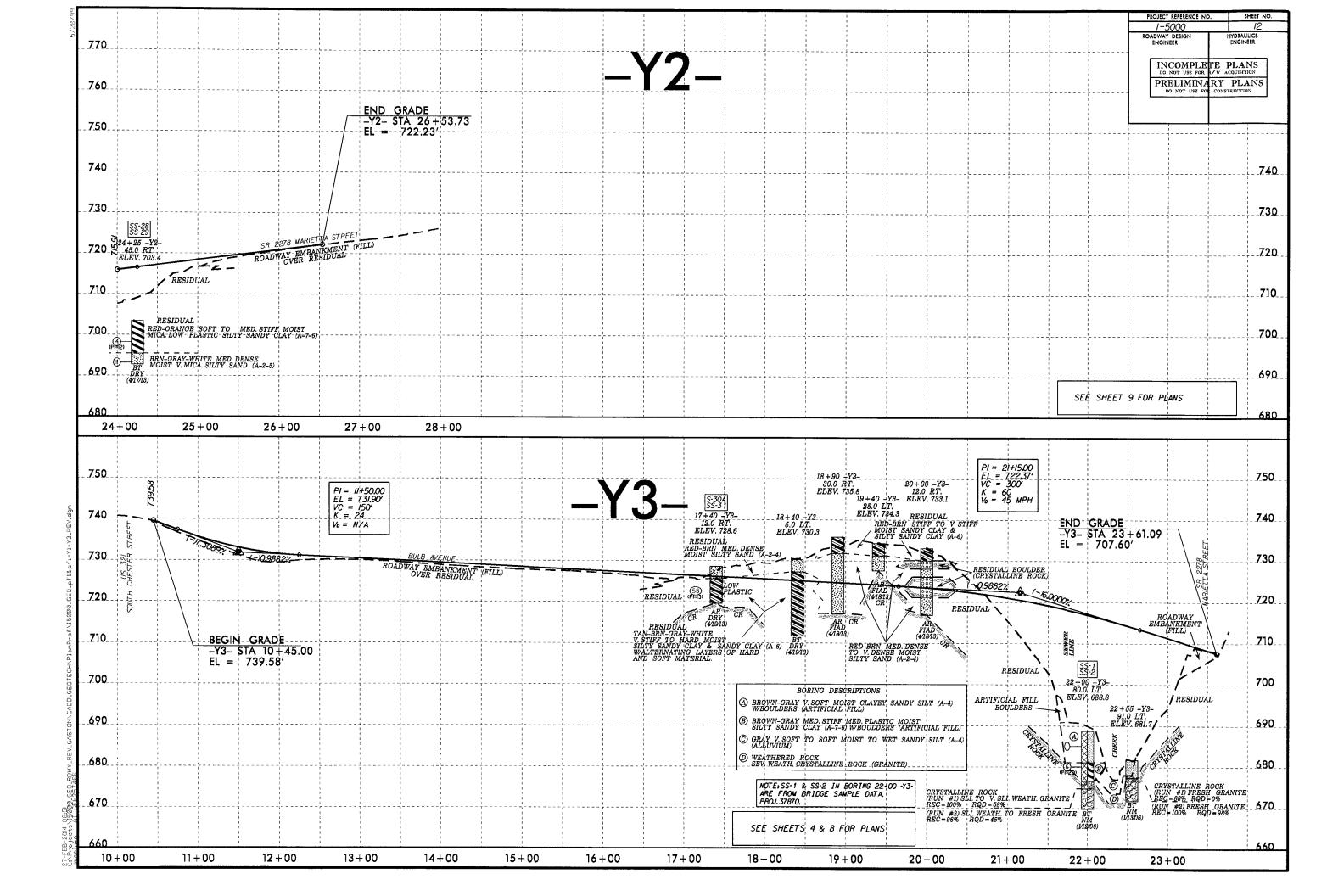


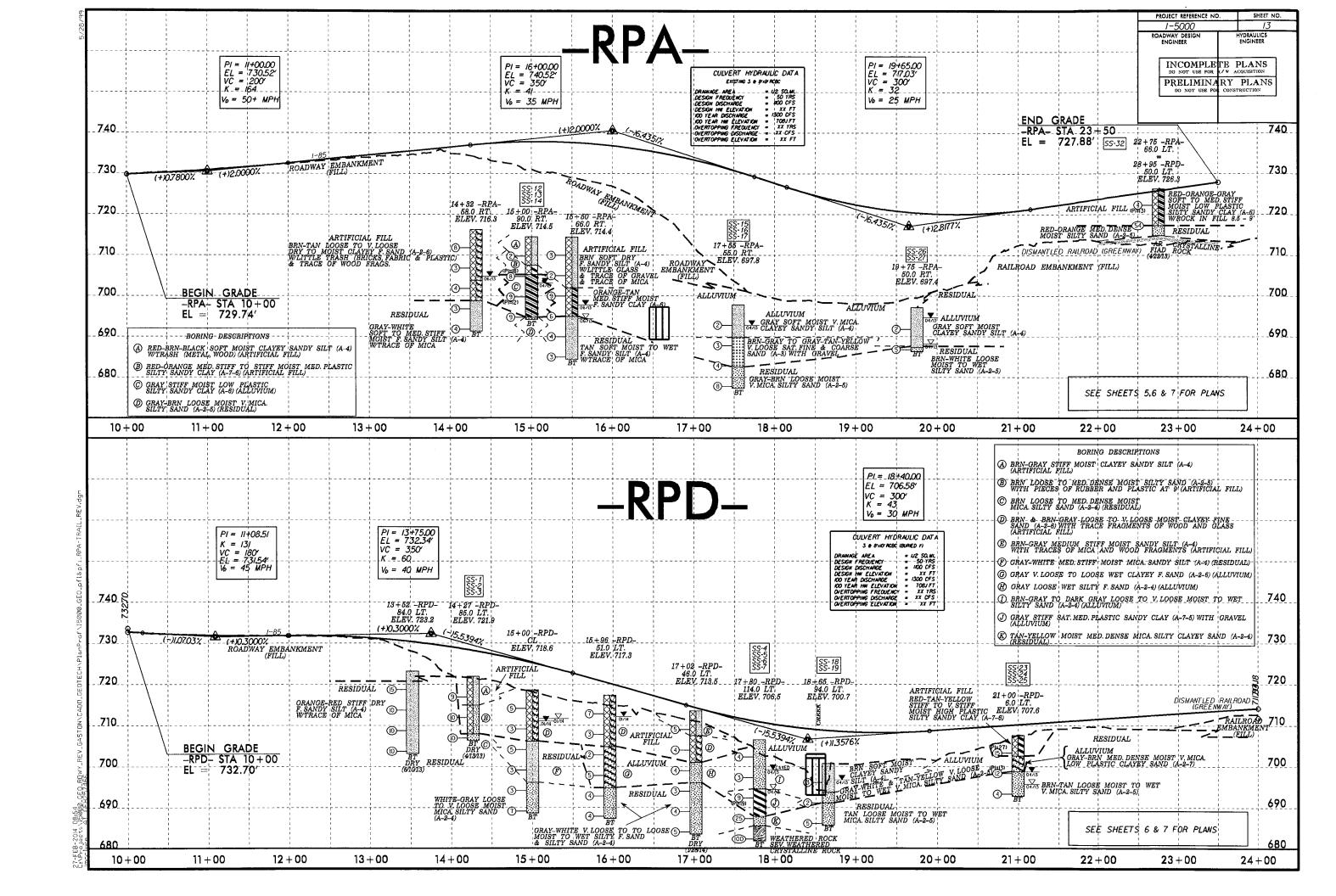


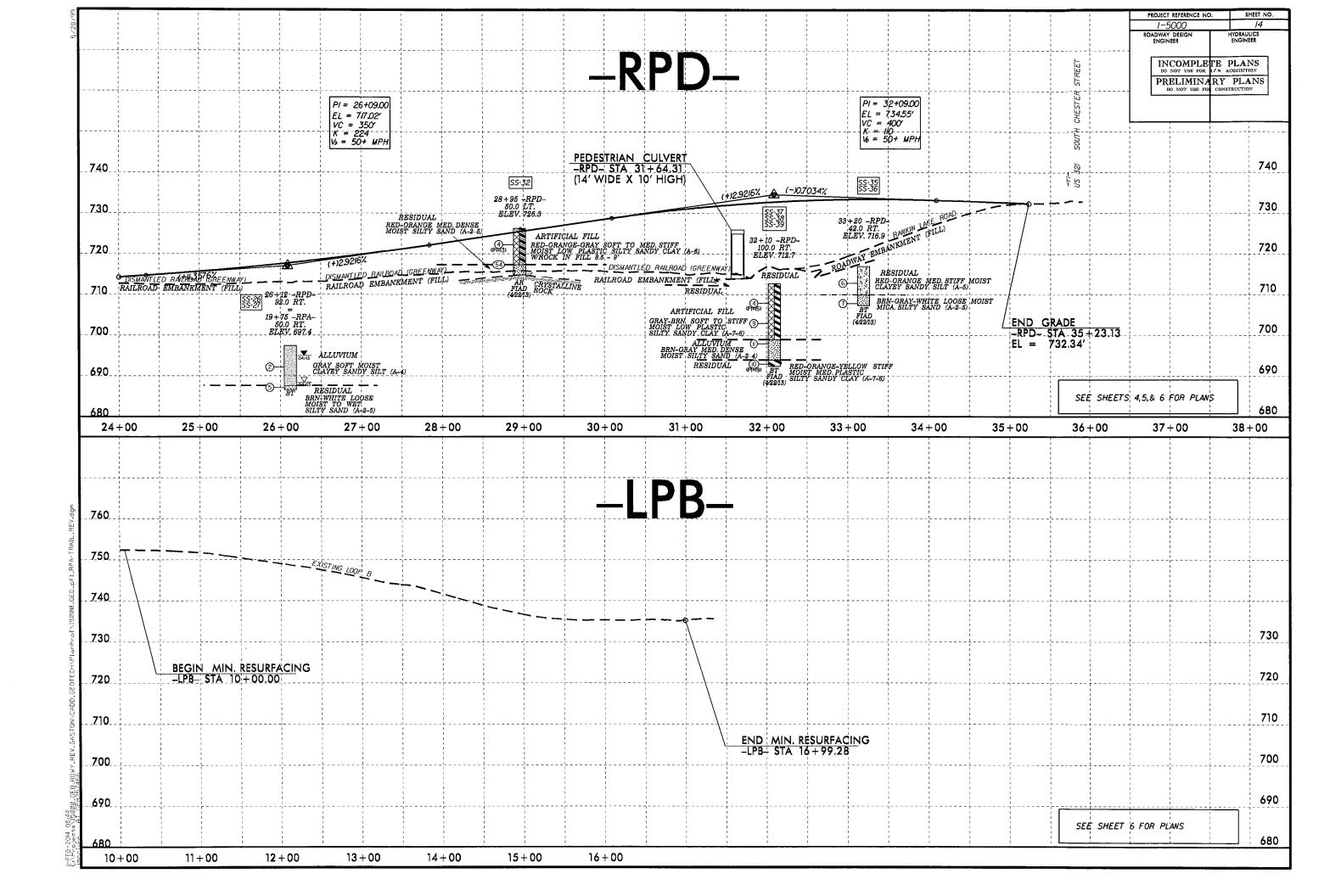


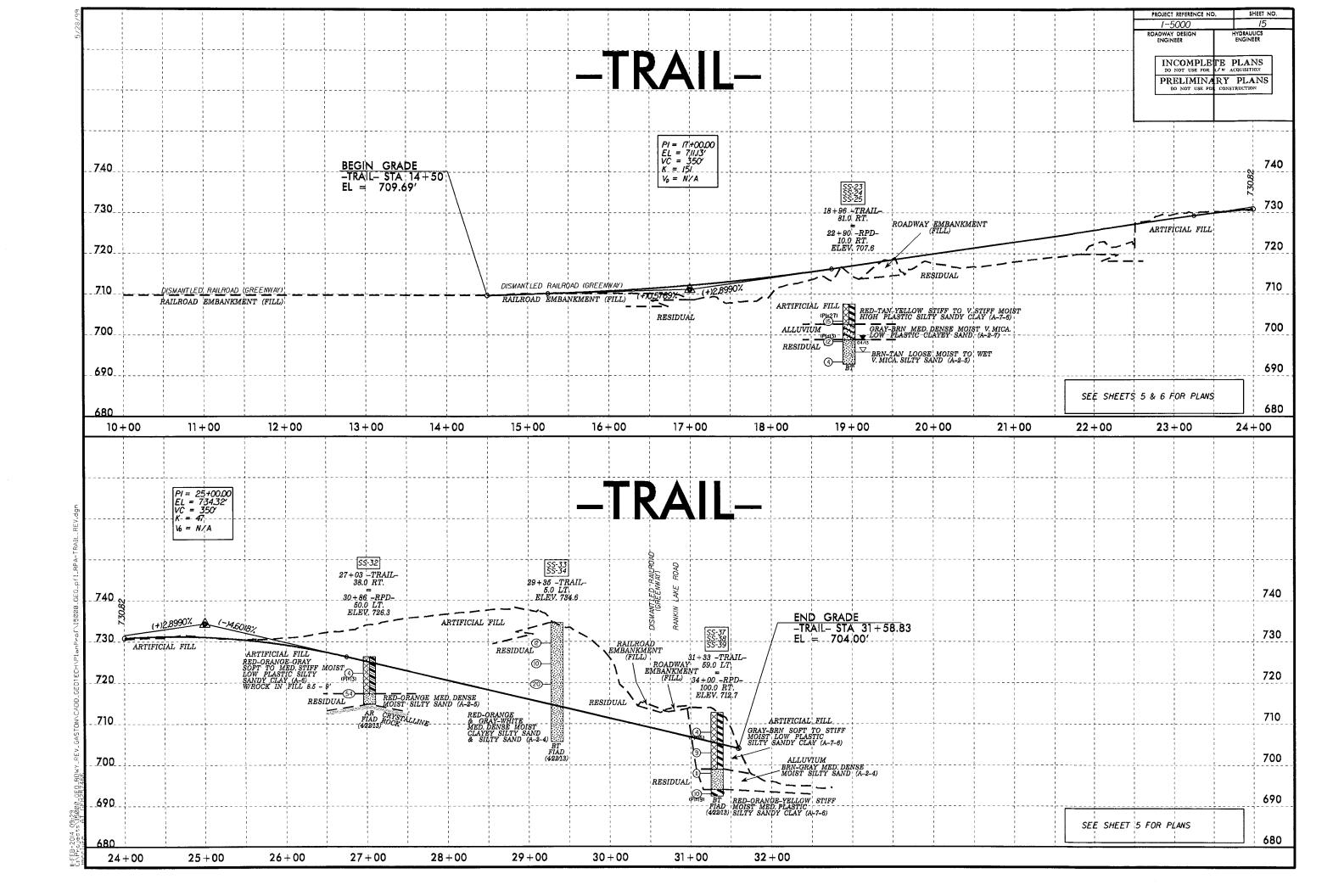


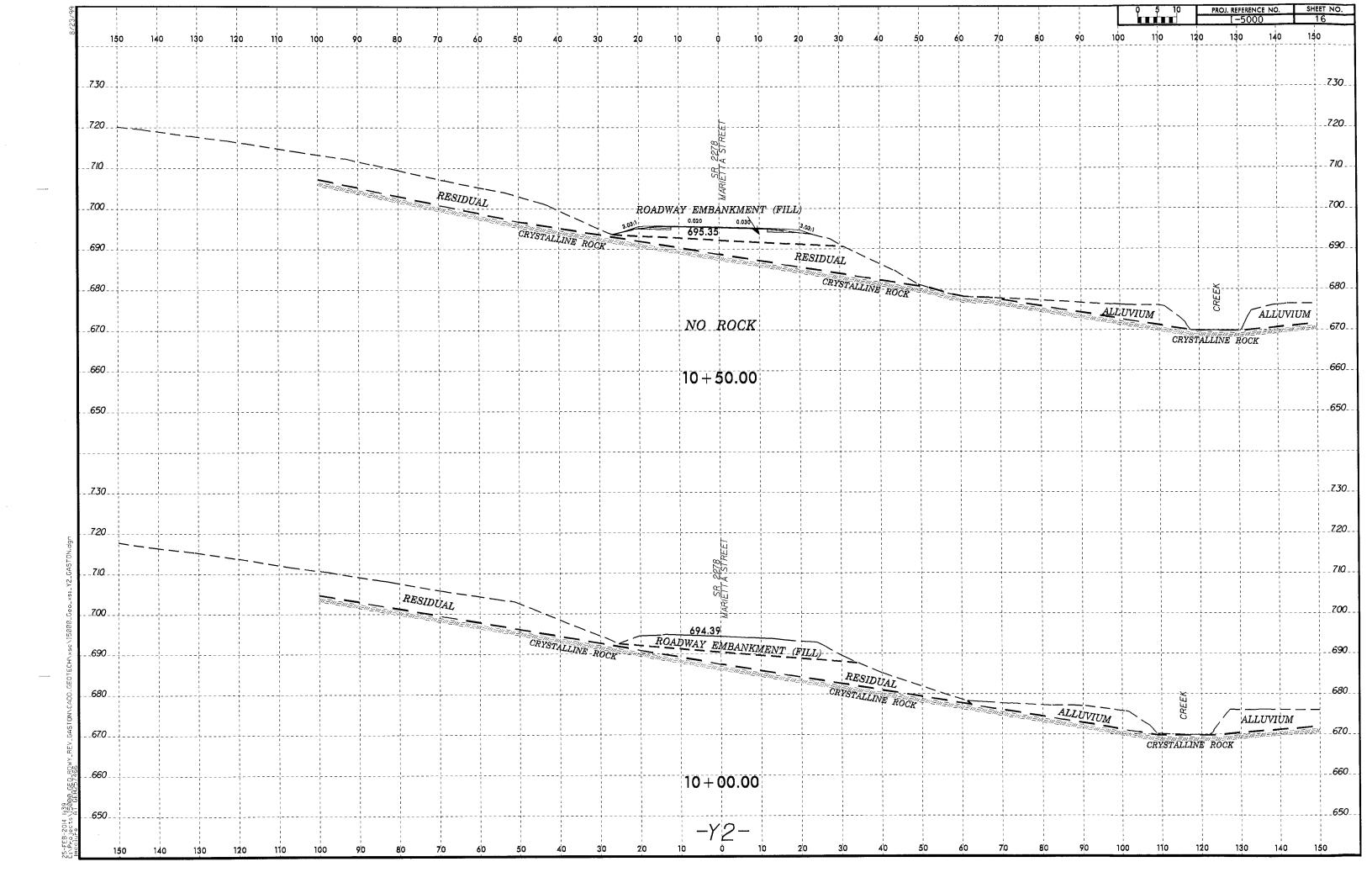


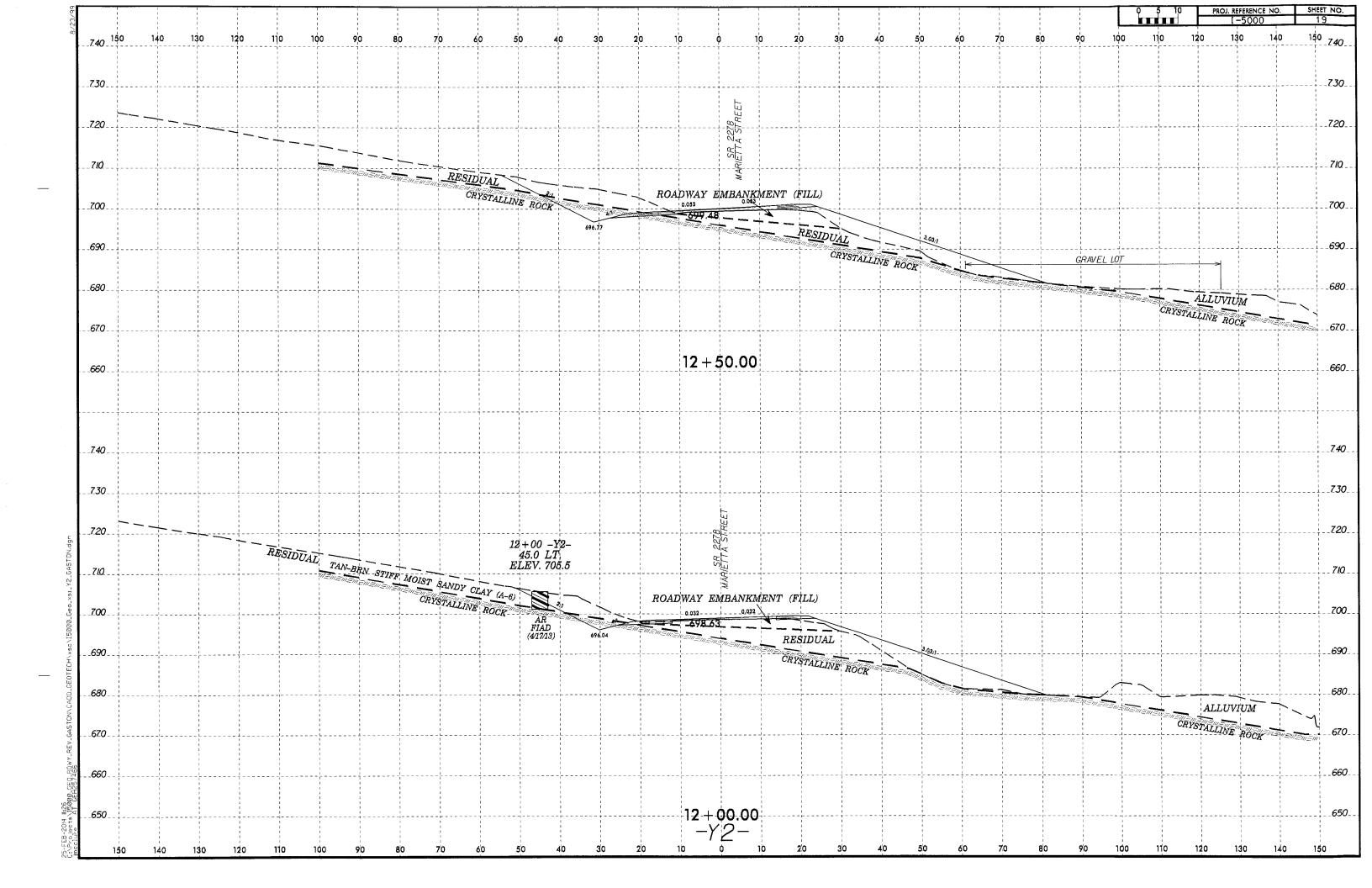


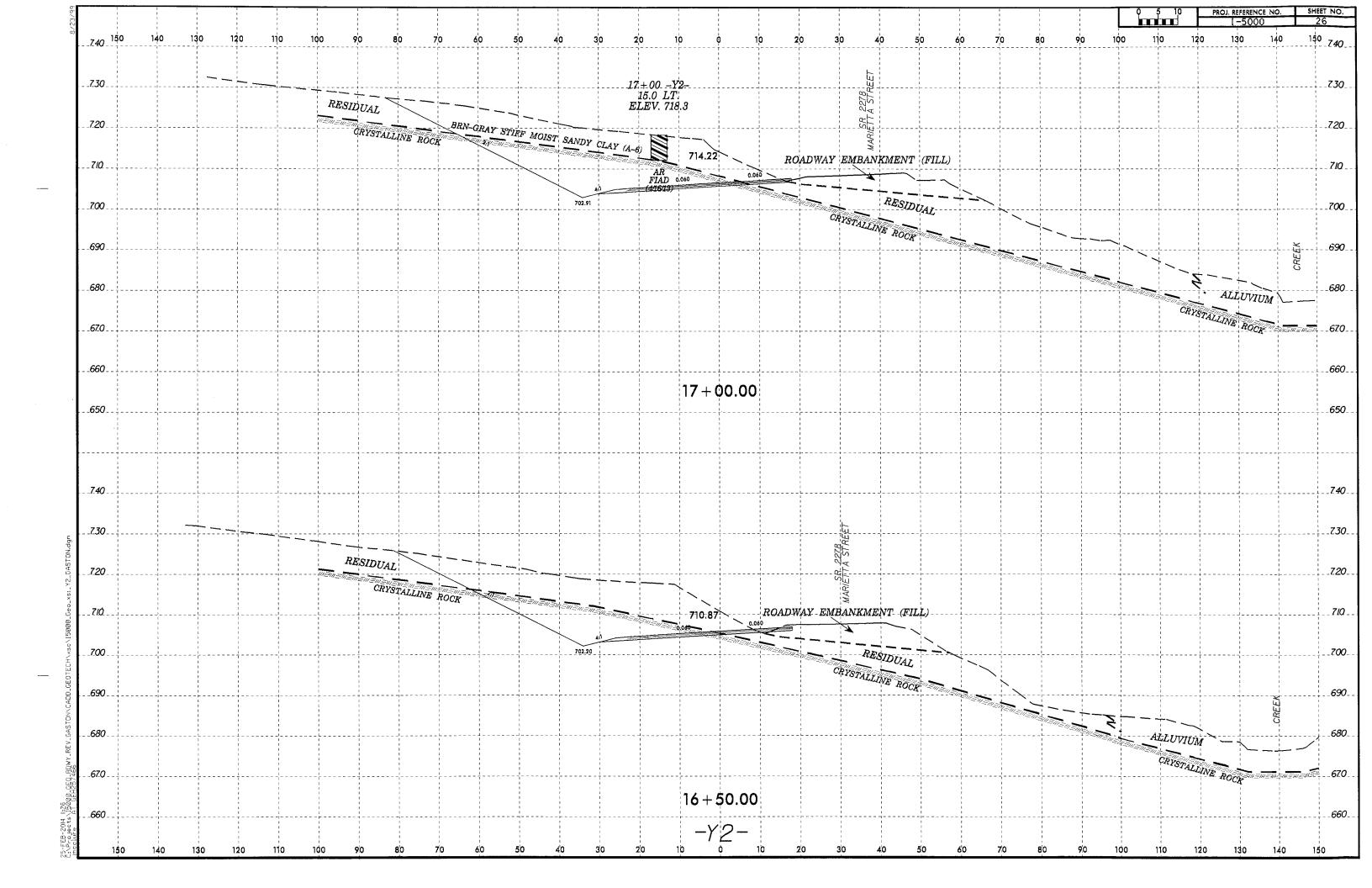


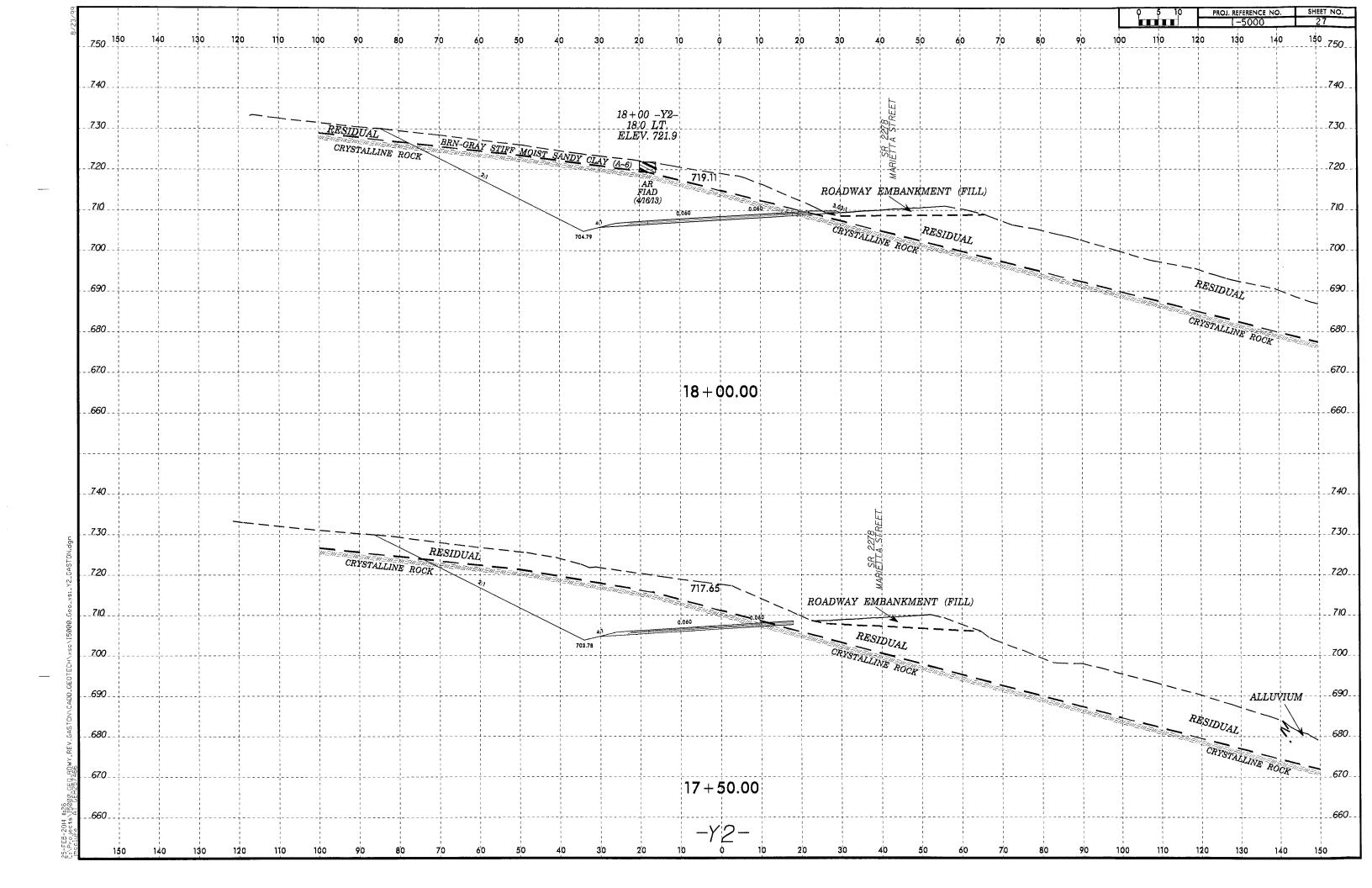


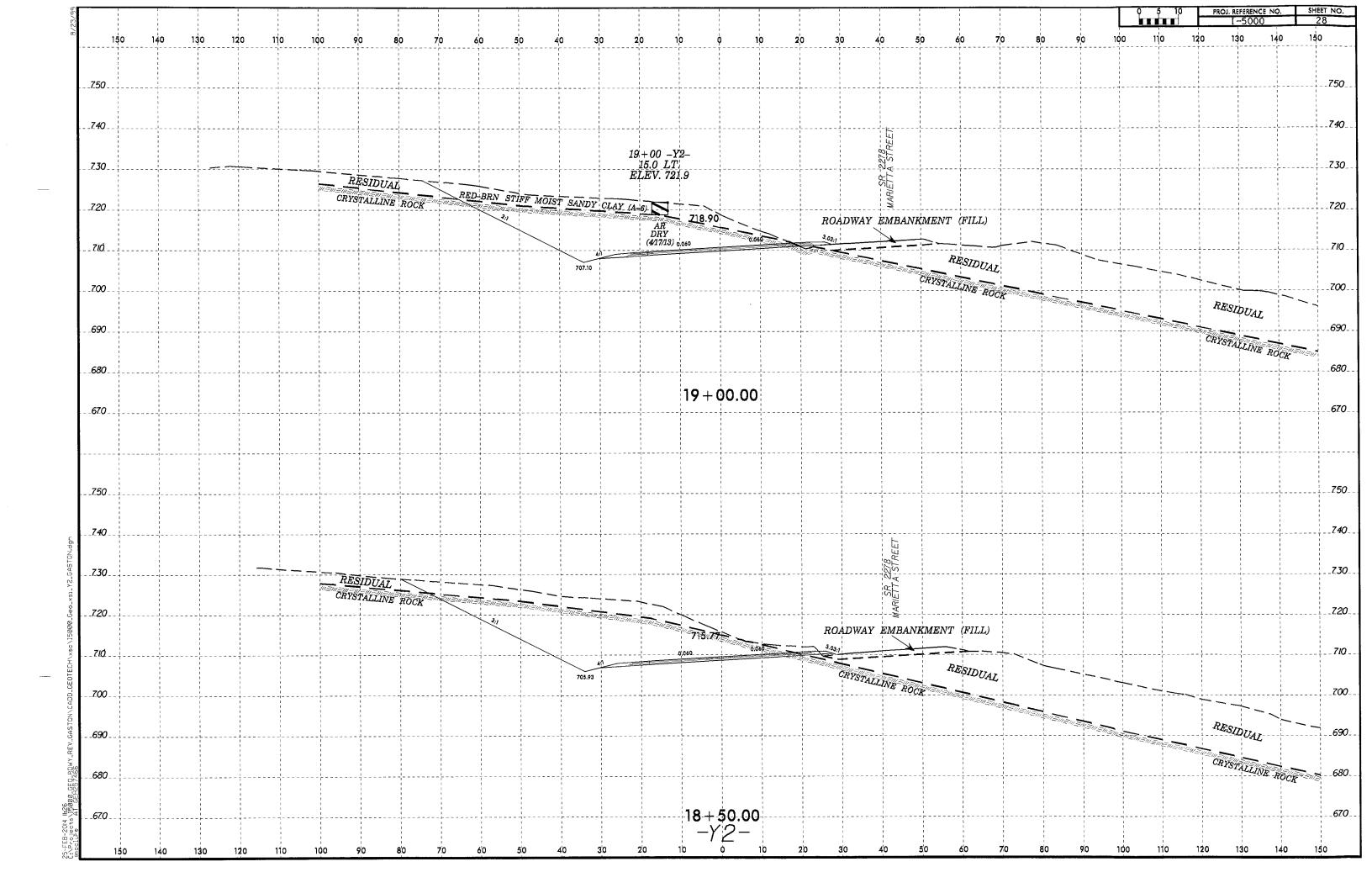


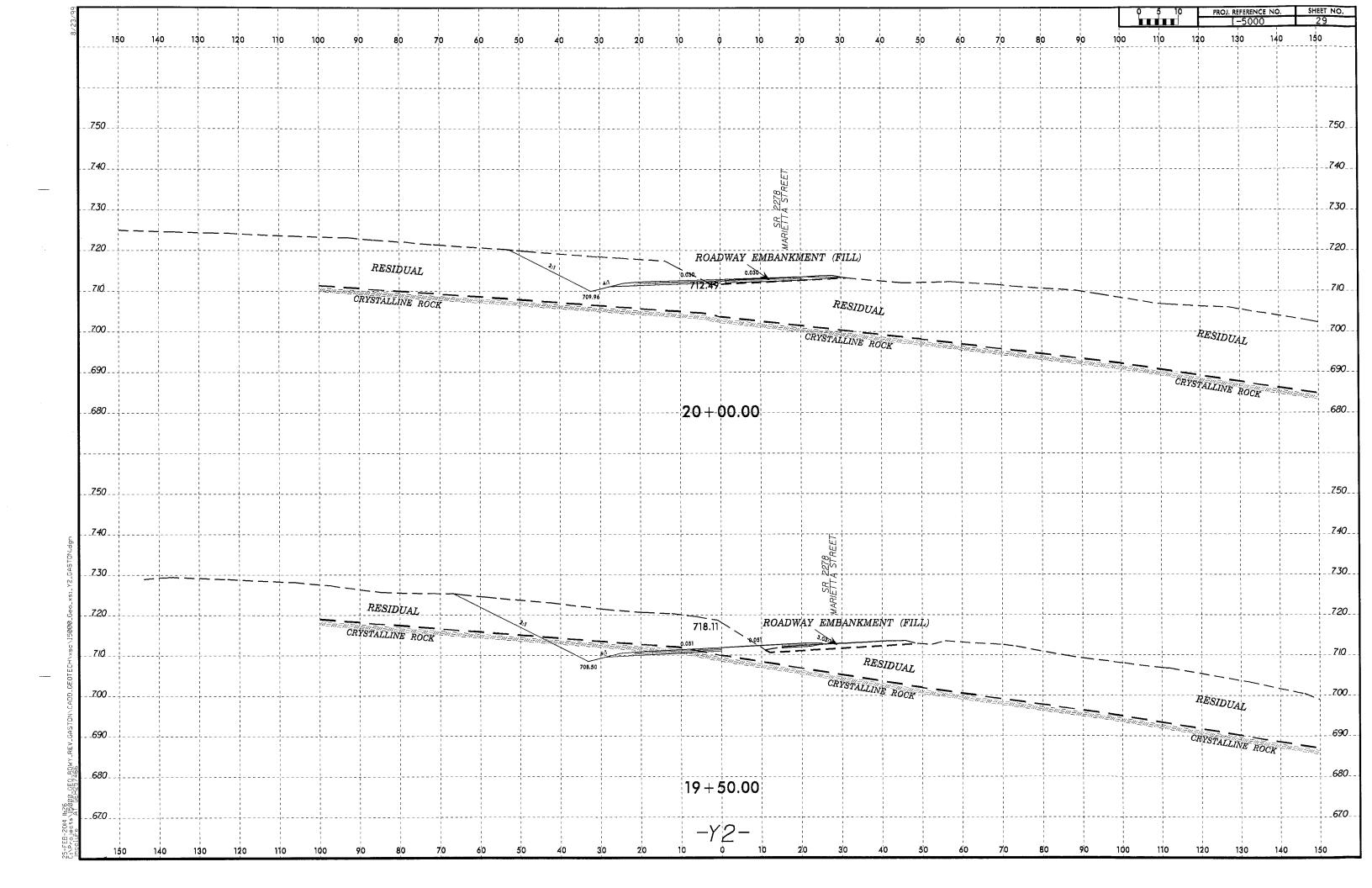


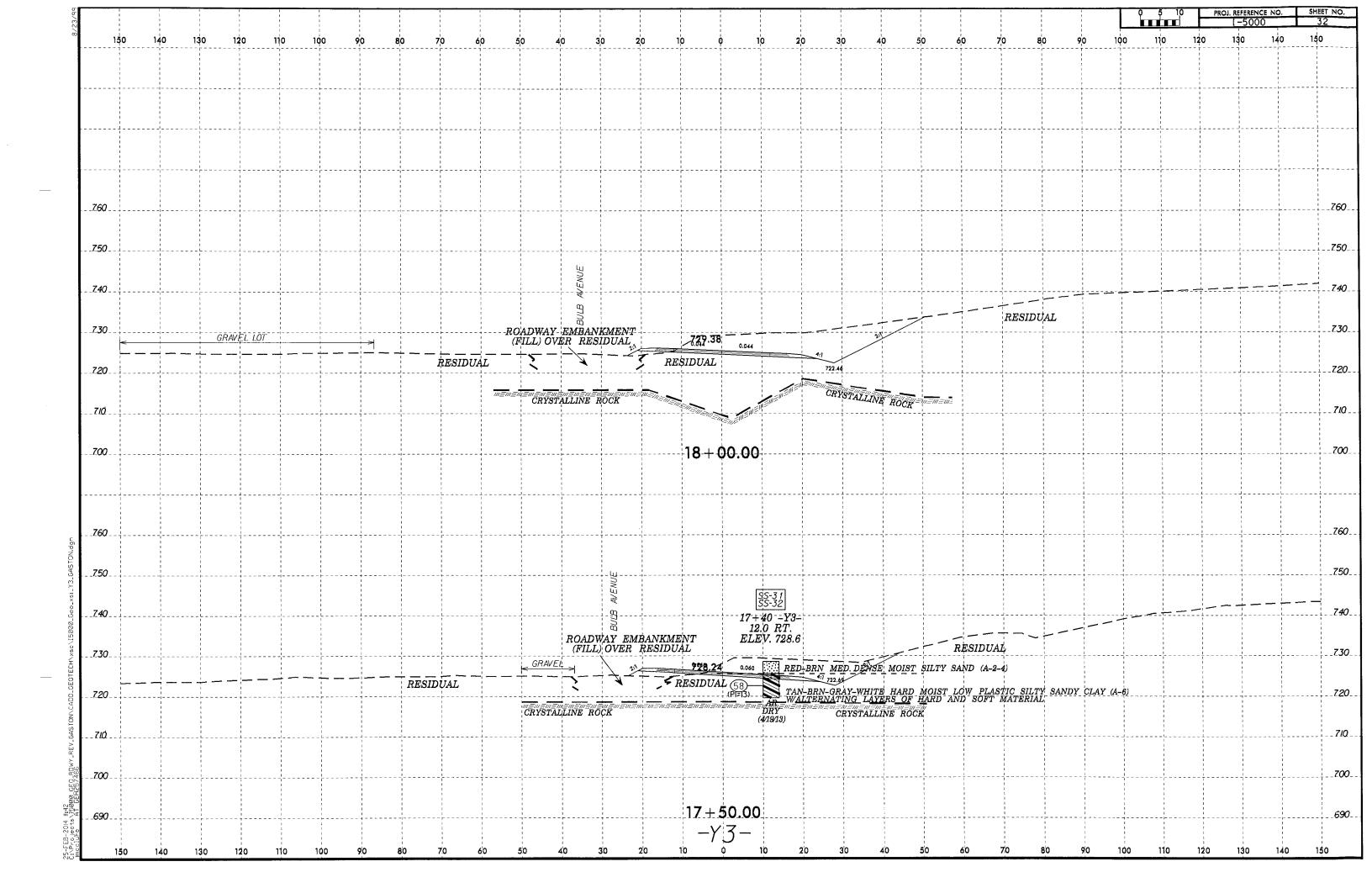


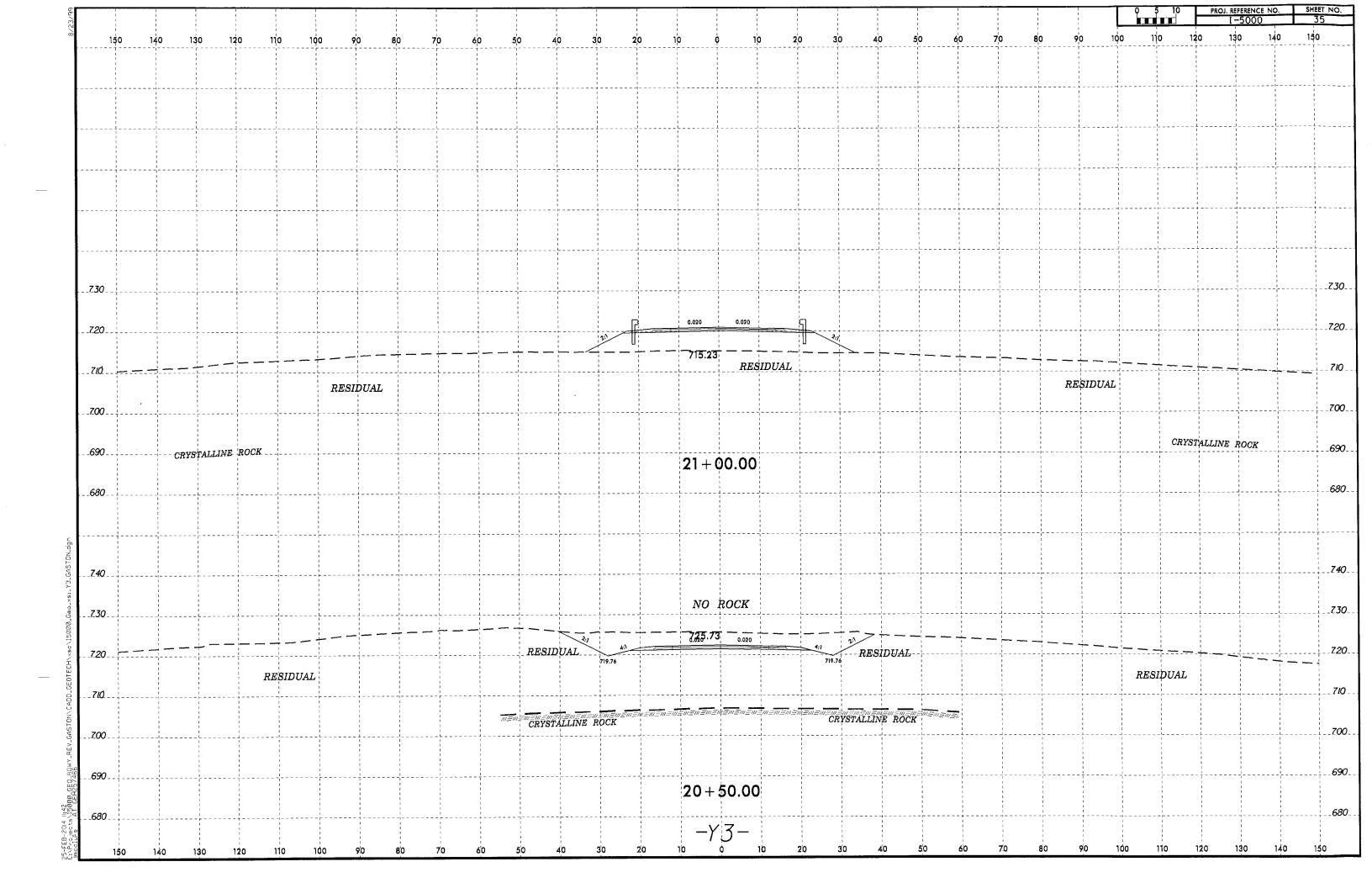


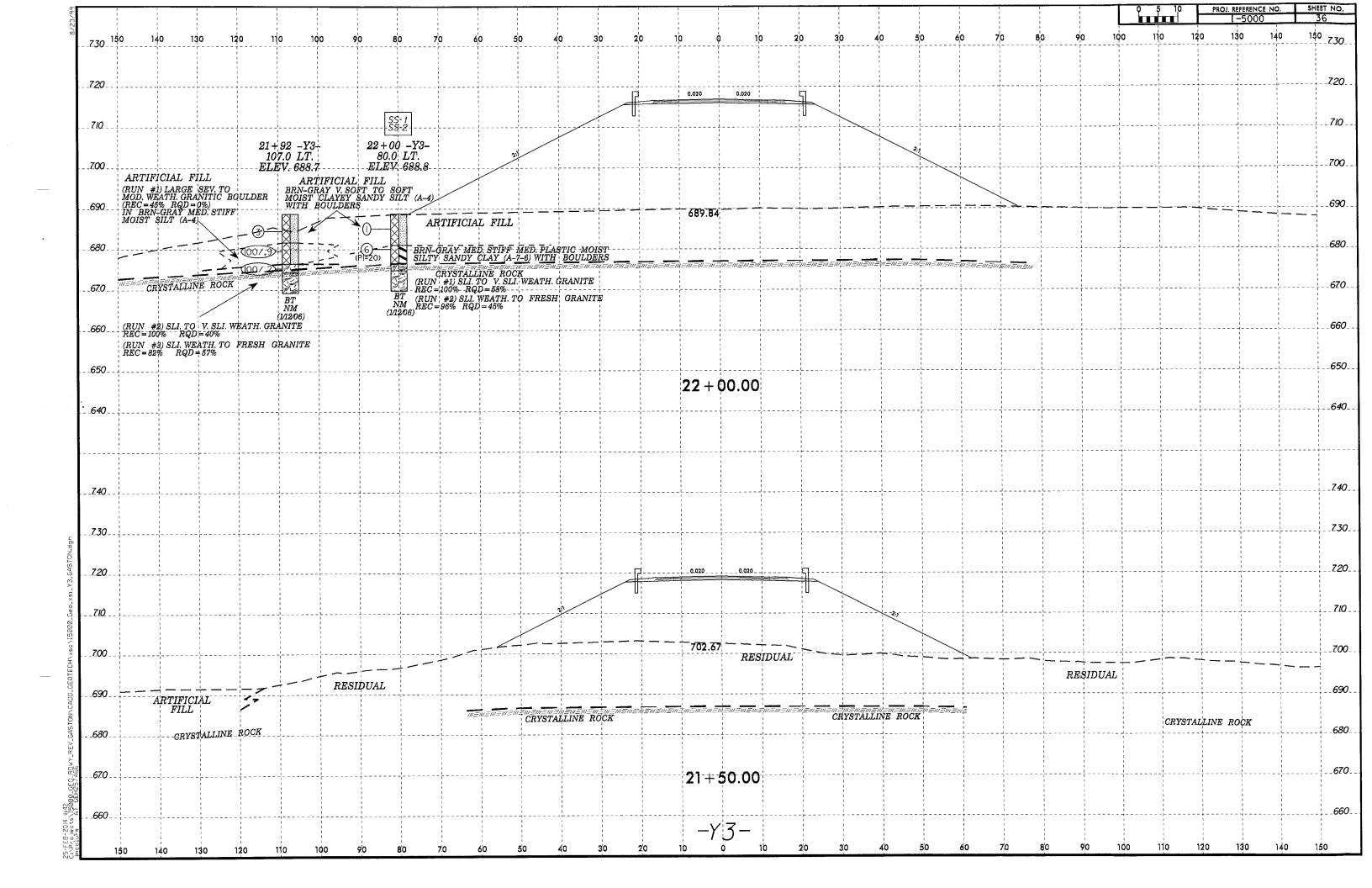


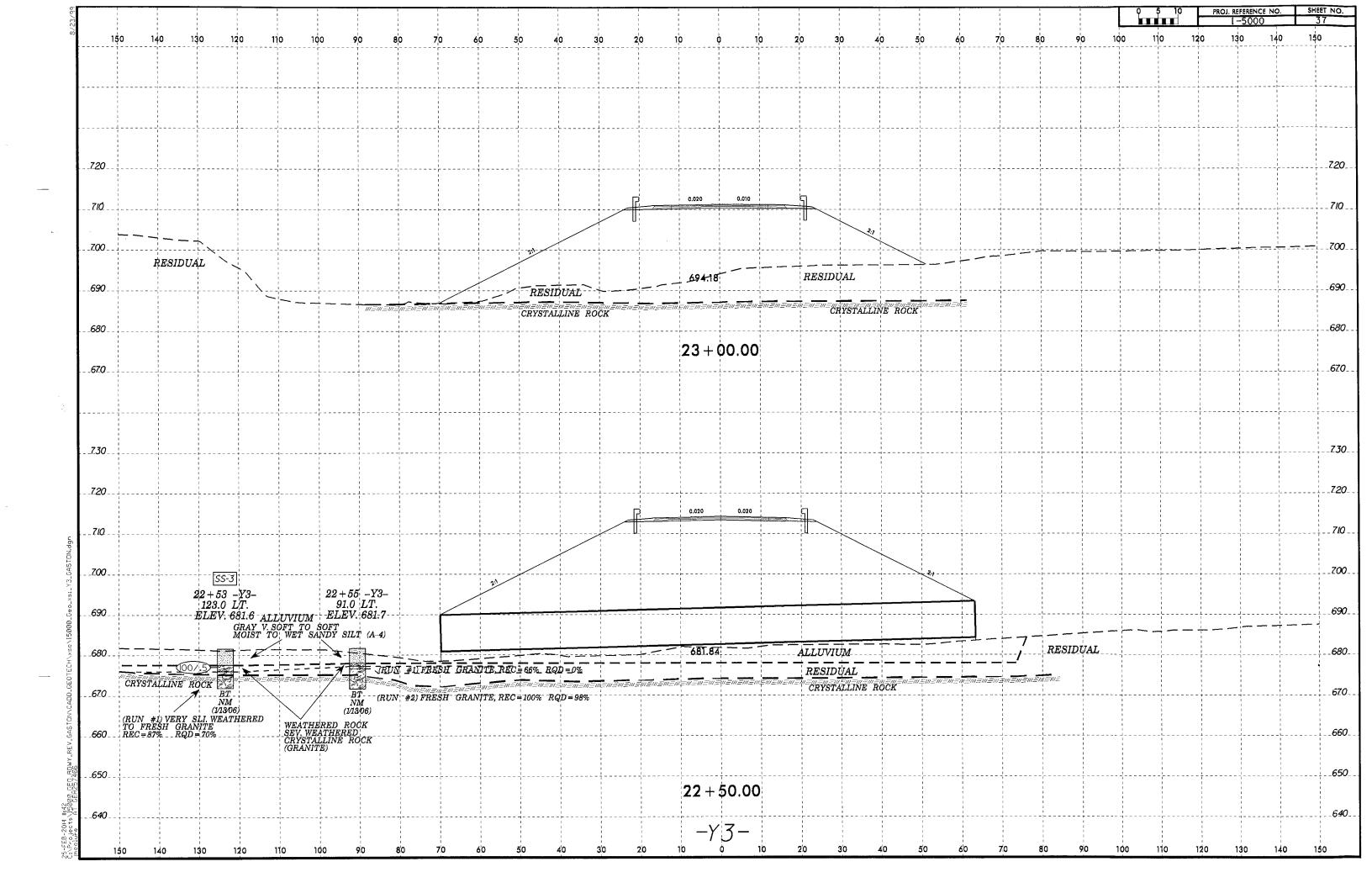


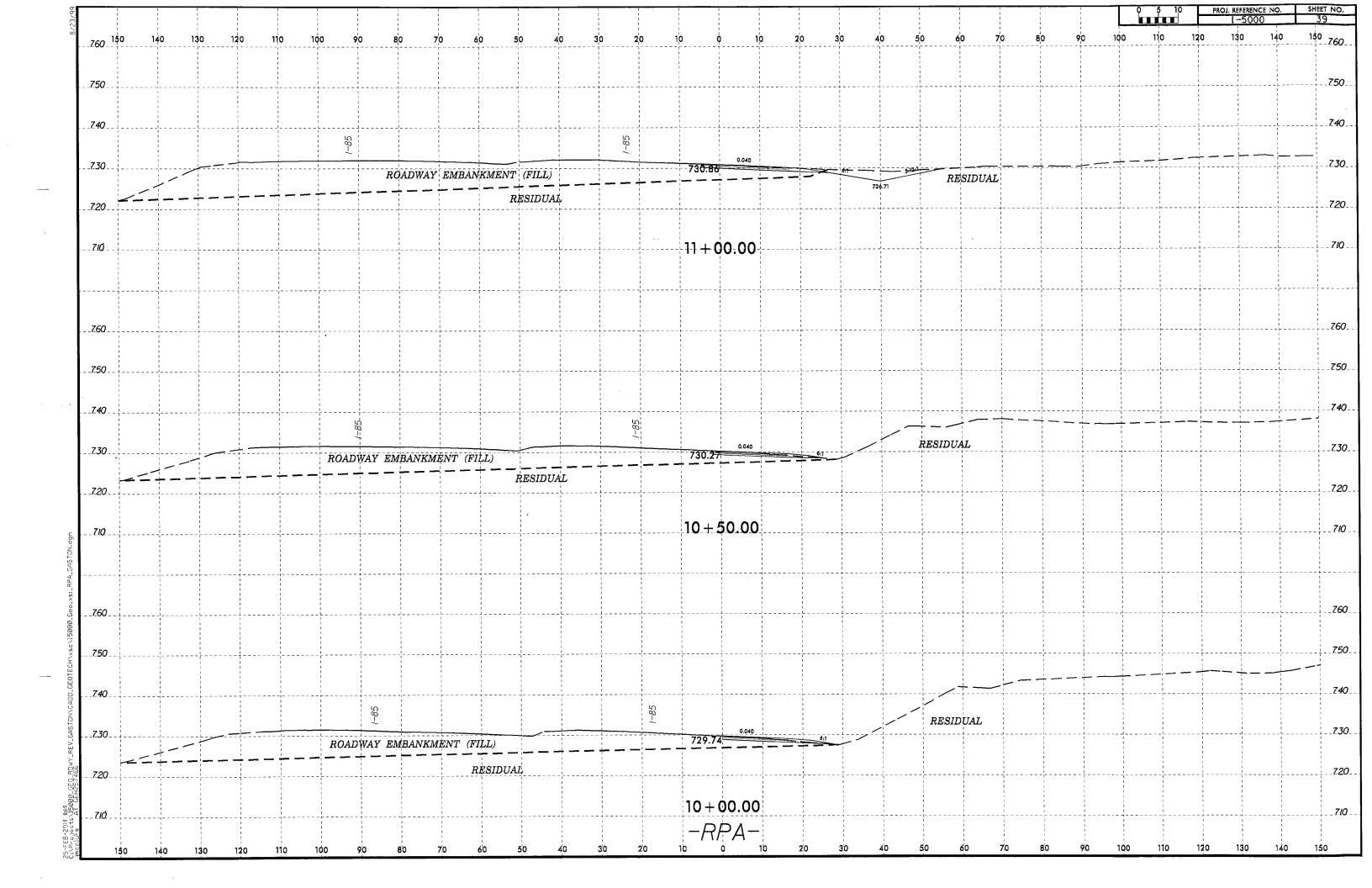


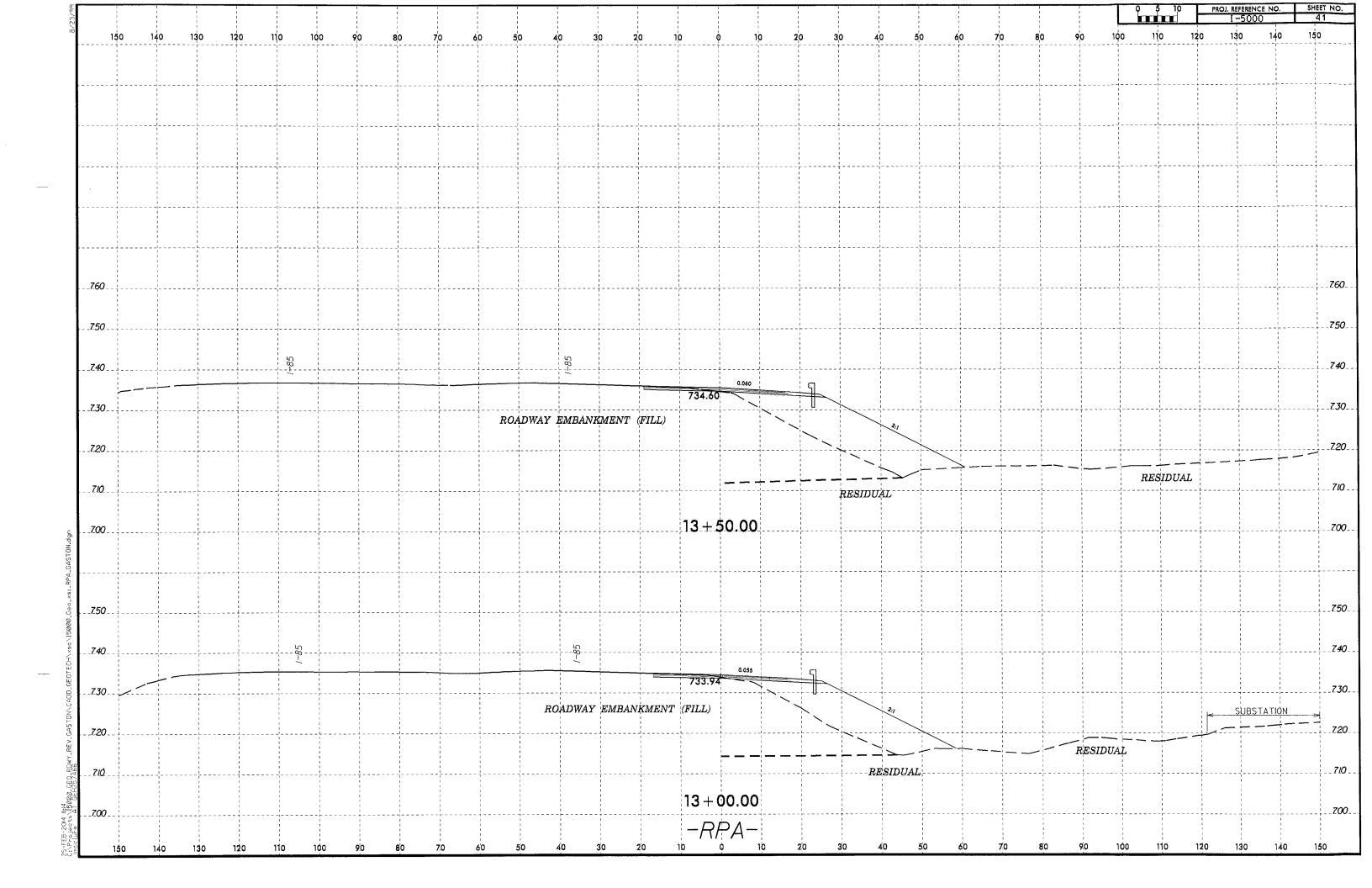


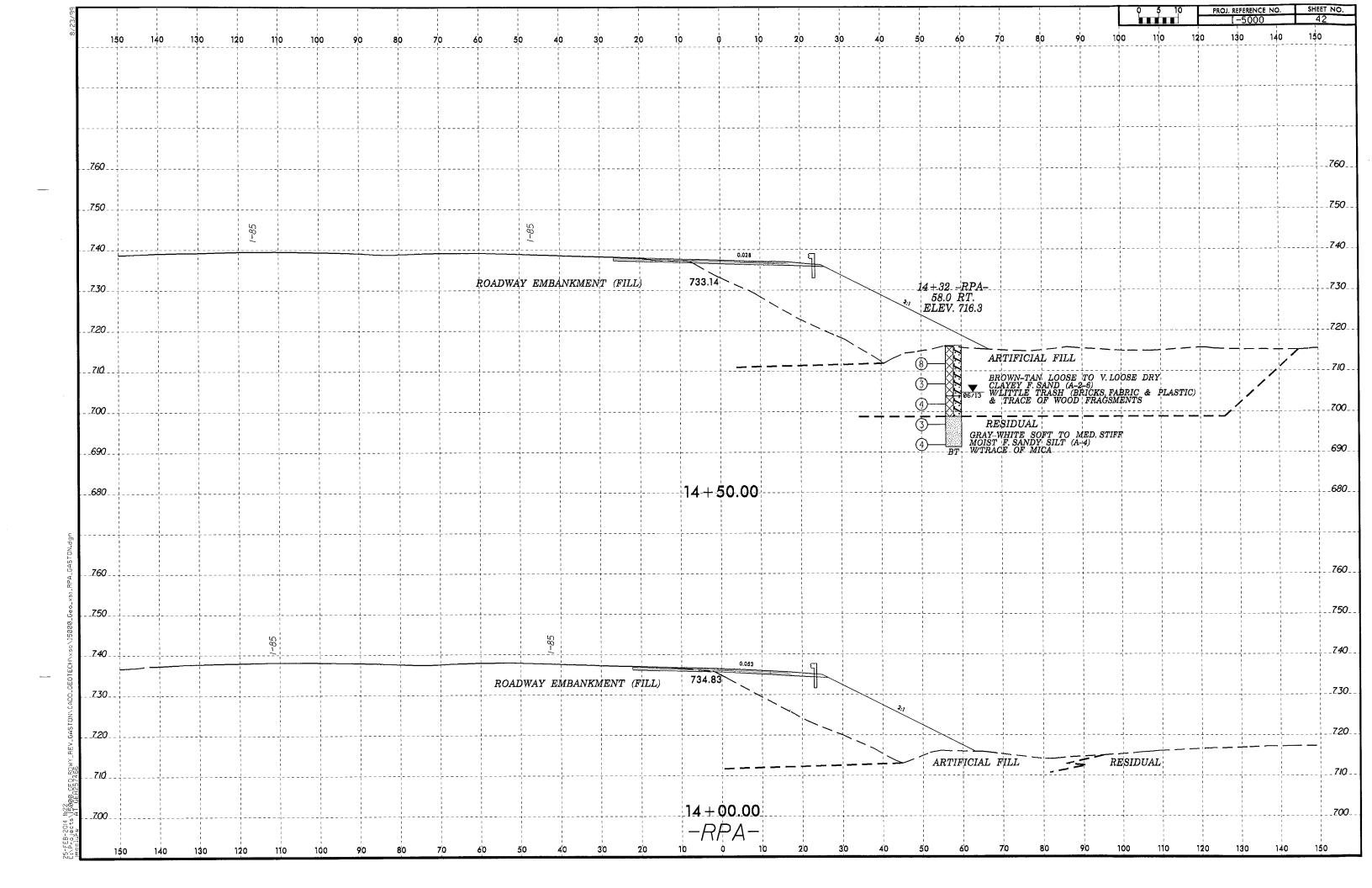


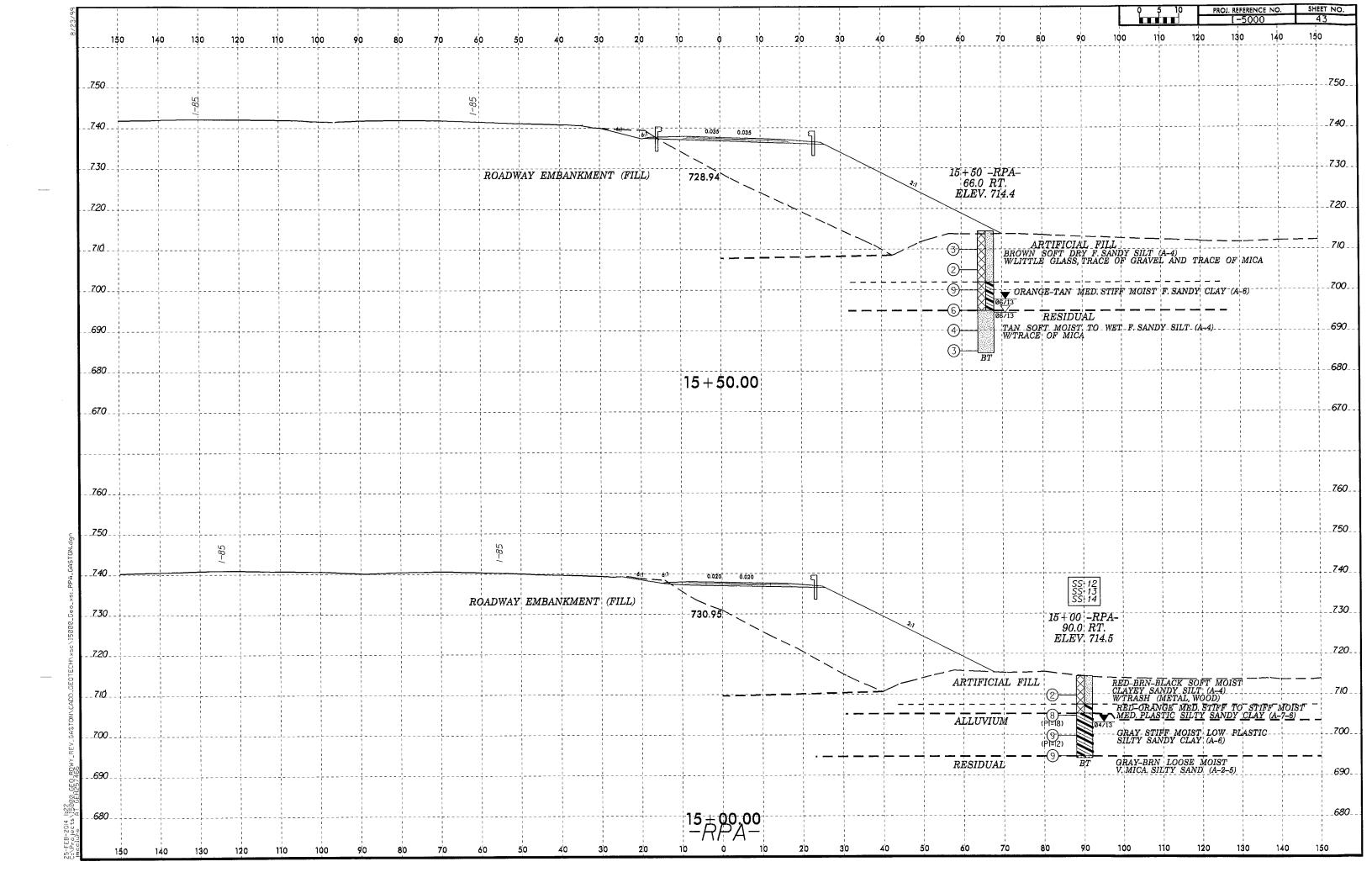


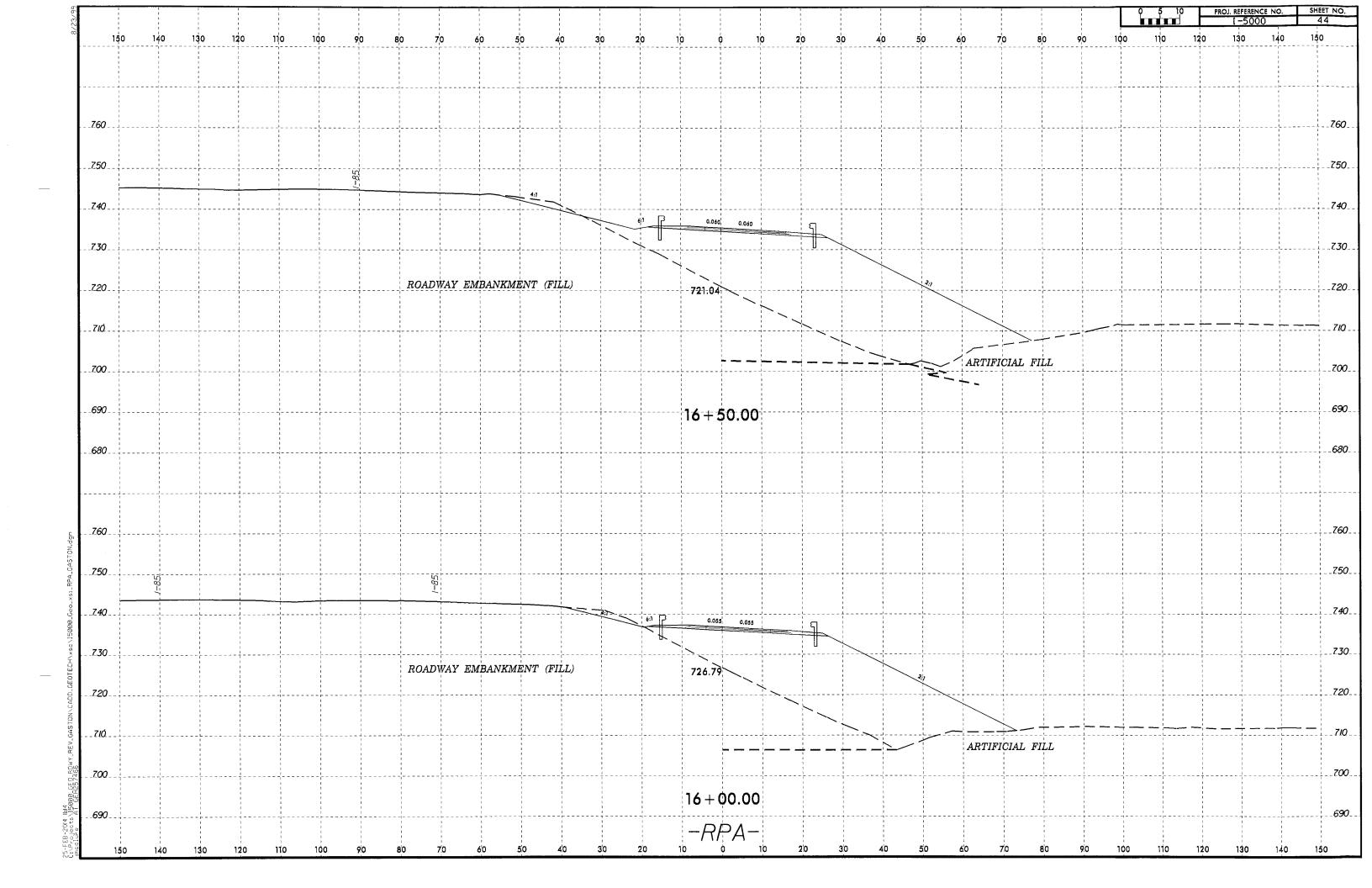


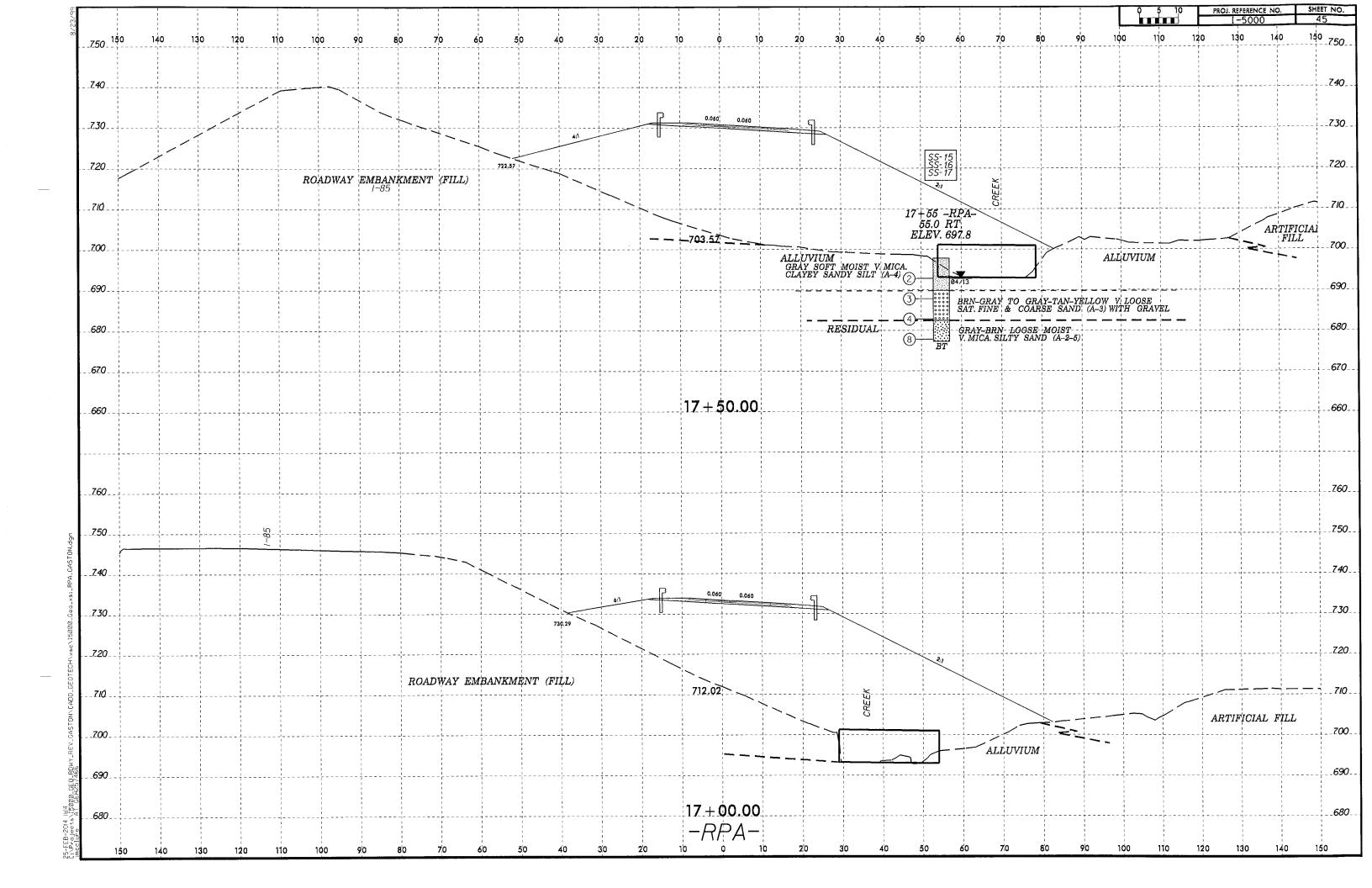


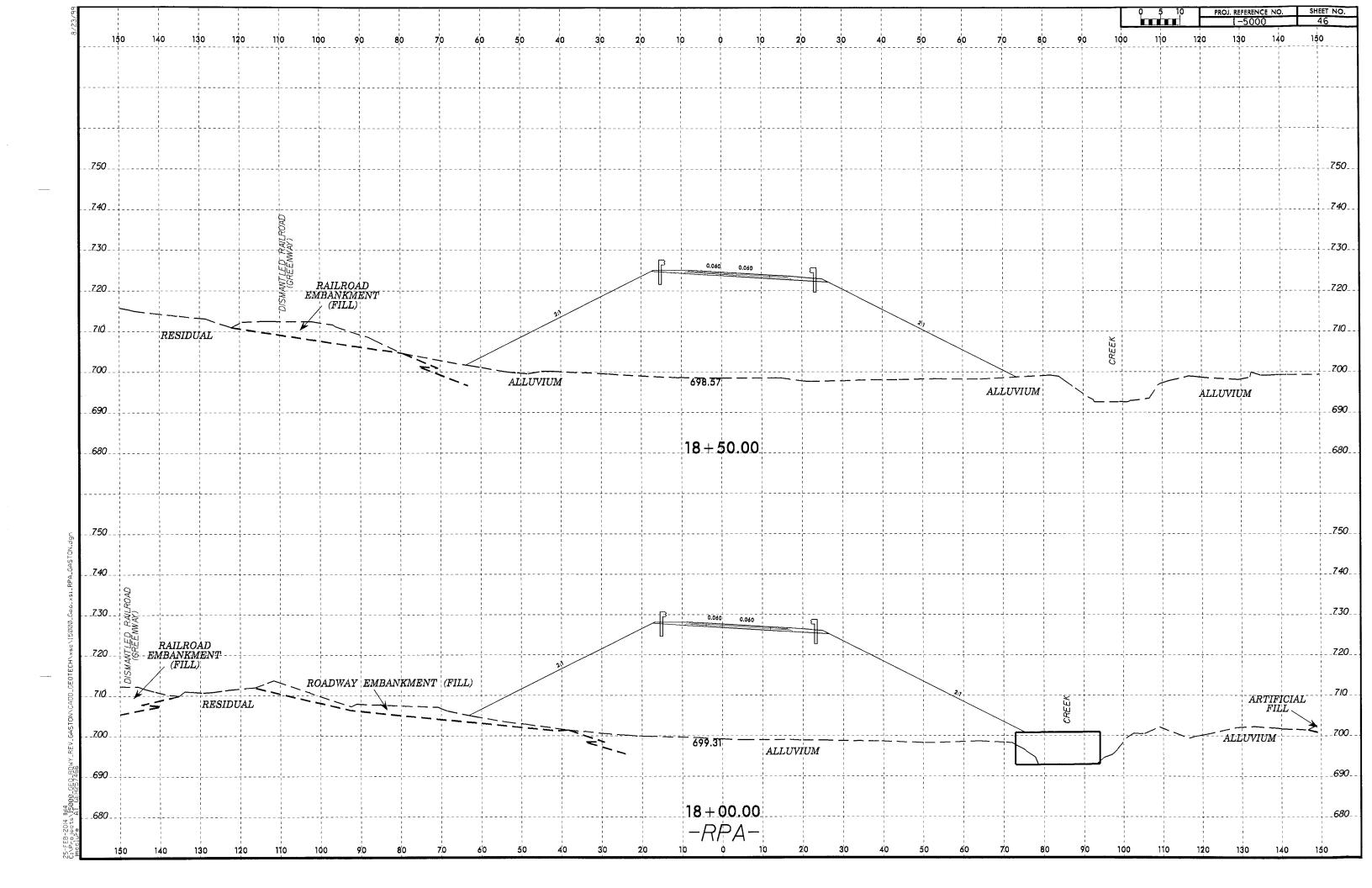


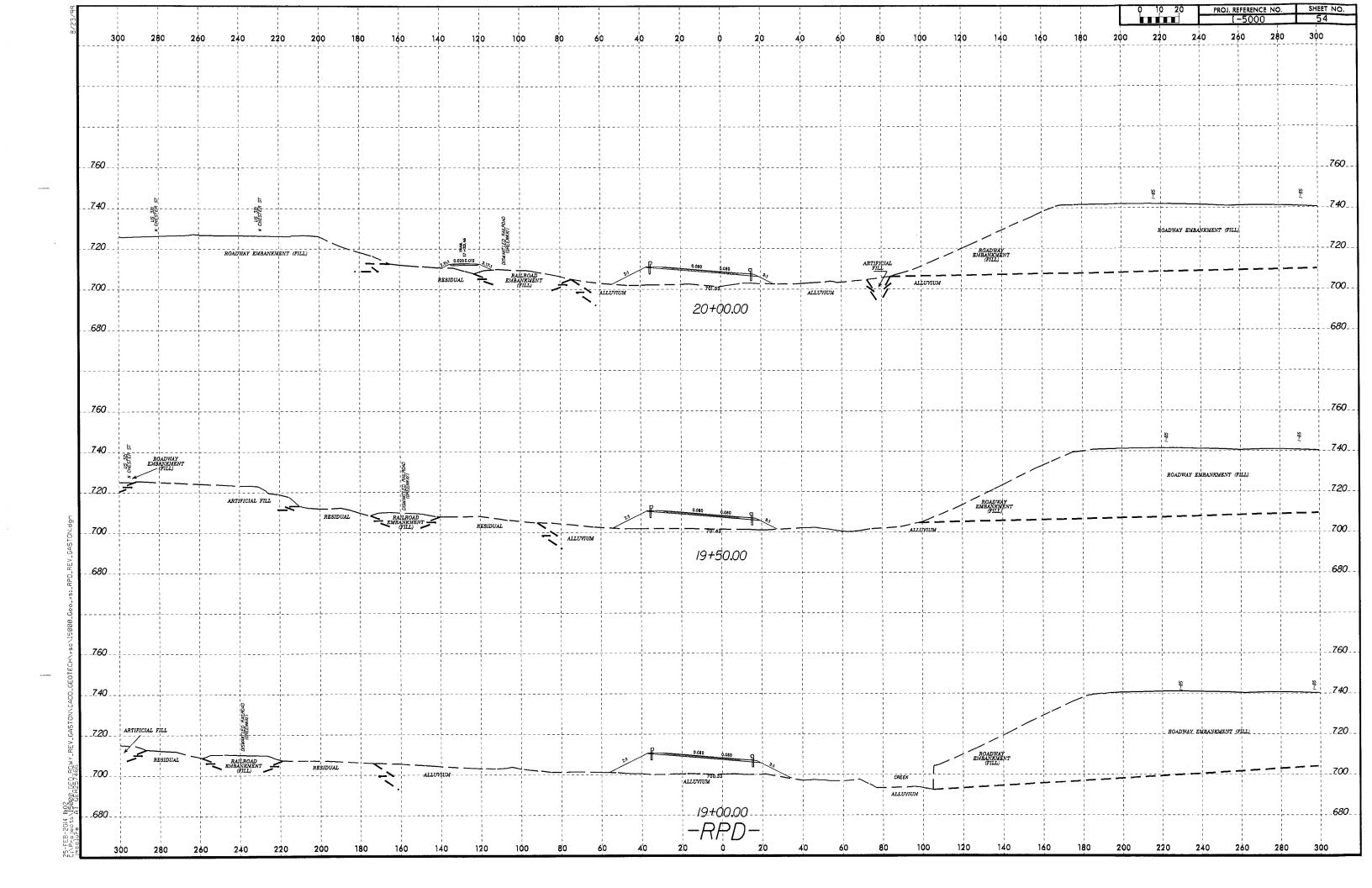


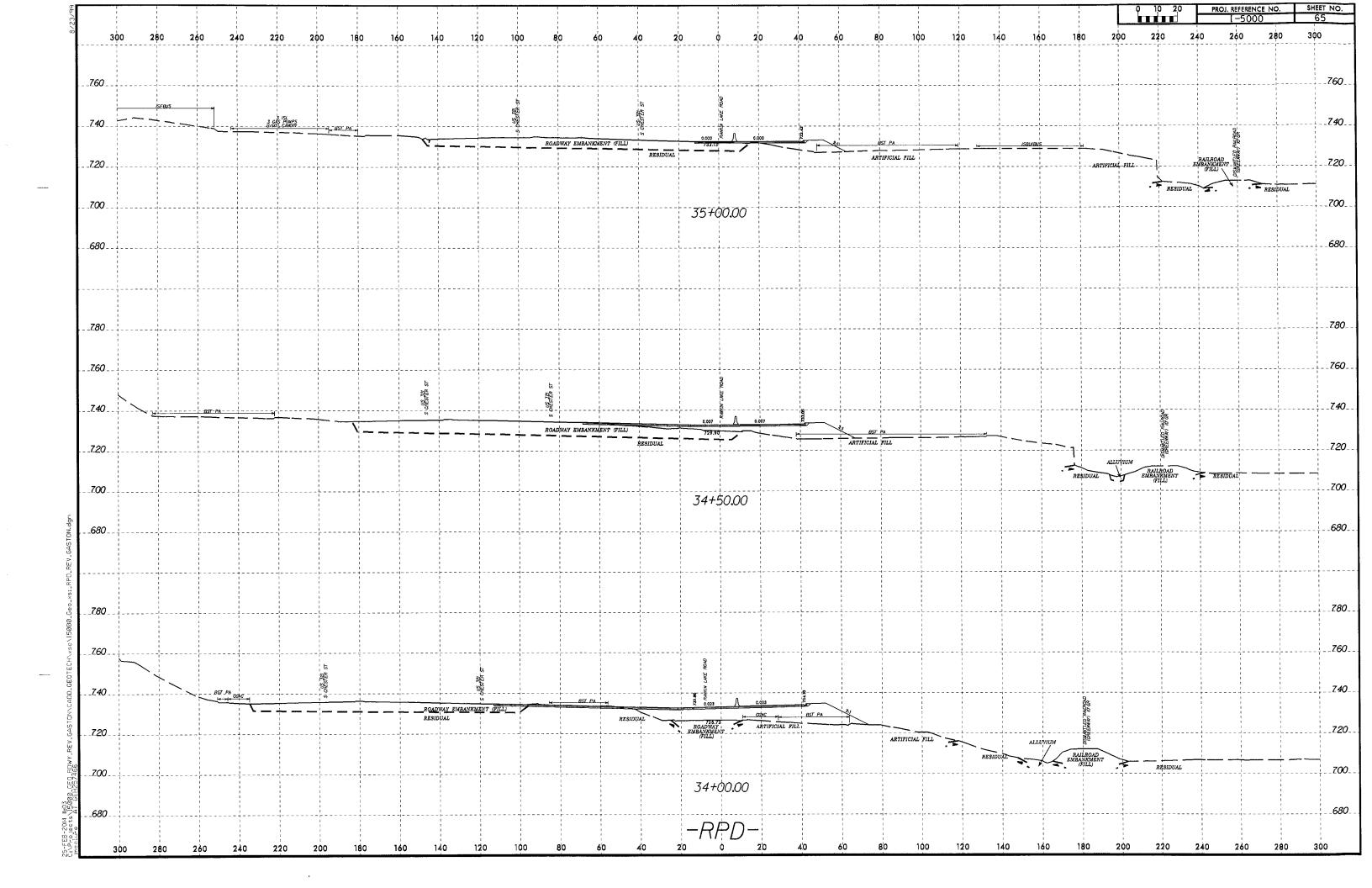












		·	S	OIL 7	TE.	ST	RE	SUI	LTS						. <u></u>	
SAMPLE			DEPTH	AASHTO		Γ.		% BY V	VEIGHT		% PAS	SING (S	IEVES)	%	%	Line or
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40		MOISTURE	ORGANIC	Boring ID
SS-1	85 LT.	14+27	4.5-5.5	A-4(0)	40	4	17.5	54.6	9.6	18.3	100	91	36	-	-	RPD
SS-2	85 LT.	14+27	9.5-10.5	A-2-5(0)	47	NP	28.3	50.7	12.8	8.1	97	83	26	•	-	RPD
SS-3	85 LT.	14+27	14.5-15.5	A-2-4(0)	30	NP	30.9	52.2	12.8	4.1	96	82	24	-	-	RPD
SS-4	114 LT.	17+80	3.7-4.7	A-2-4(0)	34	NP	30.3	49.9	13.6	6.1	95	83	25	-	-	RPD
SS-5	114 LT.	17+80	8.7-9.7	A-2-4(0)	30	NP	49.3	33.8	8.8	8.1	84	57	20	-	-	RPD
SS-6	114 LT.	17+80	13.7-14.7	A-7-5(3)	49	19	29.3	32.4	9.8	28.5	93	77	40	-	-	RPD
SS-7	114 LT.	17+80	18.7-19.7	A-2-4(0)	37	1	28.7	42.4	10.6	18.3	91	78	30	-		RPD
SS-8	129 LT.	16+35	4.0-5.0	A-2-5(0)	43	NP	50.9	33.4	9.6	6.1	97	69	19	•	•	RPD
SS-9	129 LT.	16+35	14.0-15.0	A-2-5(0)	42	NP	24.6	55.8	13.4	6.1	98	88	26	-	-	RPD
SS-10	129 LT.	16+35	19.0-20.0	A-2-5(0)	57	NP	26.1	57.6	12.2	4.1	100	91	21	-		RPD
SS-11	129 LT.	16+35	24.0-25.0	A-2-5(0)	49	NP	39.5	37.3	15.1	8.1	98	75	28	-	-	RPD
SS-12	90 RT.	15+00	4.3-5.3	A-4(0)	35	9	35.8	26.5	17.3	20.4	84	66	36	-	-	RPA
SS-13	90 RT.	15+00	9.3-10.3	A-7-6(8)	44	18	28.5	17.7	11.0	42.8	98	82	56	-	-	RPA
SS-14	90 RT.	15+00	14.3-15.3	A-6(3)	34	12	32.4	23.8	13.2	30.5	100	82	48	•	-	RPA
SS-15	55 RT.	17+55	4.5-5.5	A-4(0)	31	2	34.4	25.9	19.3	20.4	95	75	42	-	- 1	RPA
SS-16	55 RT.	17+55	9.5-10.5	A-3(0)	29	NP	66.0	27.3	2.6	4.1	93	64	8	-	-	RPA
SS-17	55 RT.	17+55	19.5-20.5	A-2-5(0)	49	NP	46.6	33.4	13.8	6.1	91	62	23	-	-	RPA
SS-18	94 LT.	18+65	4.1-5.1	A-4(0)	27	6	22.6	38.7	18.3	20.4	100	89	45	-	-	RPD
SS-19	94 LT.	18+65	9.1-10.1	A-2-5(0)	47	NP	44.8	29.5	17.5	8.1	88	60	28	-	-	RPD
SS-20	269 LT.	17+82	4.5-5.5	A-4(0)	25	2	16.5	53.8	13.4	16.3	99	91	37	-	-	RPD
SS-21	269 LT.	17+82	9.5-10.5	A-2-4(0)	26	NP	43.0	41.8	7.1	8.1	91	70	18	-		RPD
SS-22	269 LT.	17+82	14.5-15.5	A-2-4(0)	28	NP	54.2	32.2	9.6	4.1	92	59	17	-	•	RPD
SS-23	6 LT.	21+00	3.8-4.8	A-7-6(16)	56	27	24.1	16.7	18.9	40.2	100	85	63	-		RPD
SS-24	6 LT.	21+00	8.8-9.8	A-2-7(0)	45	13	47.7	24.3	9.9	18.1	100	69	31	-	•	RPD
SS-25	6 LT.	21+00	13.8-14.8	A-2-5(0)	53	NP	45.9	30.6	17.5	6.0	97	65	28	•		RPD
SS-26	50 RT.	19+75	4.8-5.8	A-4(0)	21	3	46.7	19.7	15.5	18.1	100	67	37	-	-	RPA
SS-27	50 RT.	19+75	9.8-10.8	A-2-5(0)	48	NP	48.3	24.9	18.7	8.0	94	60	31	-	-	RPA
SS-28	45 RT.	24+25	4.7-5.7	A-7-6(2)	41	12	40.4	23.3	14.1	22.1	100	72	40	-	-	Y2
SS-29	45 RT.	24+25	9.7-10.7	A-2-5(0)	47	NP	40.8	28.8	22.3	8.0	99	72	34	-	•	Y2
SS-30	15 RT.	21+12	3.7-4.7	A-6(7)	40	20	30.0	22.7	15.1	32.2	100	85	51	-	•	Y2
S-30A	12 RT.	17+40	0.0-3.0	A-4(4)	35	10	24.7	25.4	23.7	26.2	100	87	56	-	-	Y3
SS-31	12 RT.	17+40	5.5-6.5	A-2-4(0)	28	NP	43.3	34.2	18.5	4.0	99	75	28			Y3
SS-32	50 LT.	28+95	3.5-4.5	A-6(2)	35	13	31.4	22.1	18.3	28.2	85	67	43	-	-	RPD
SS-33	5 LT.	29+35	4.5-5.5	A-2-4(0)	35	5	45.5	27.4	17.1	10.1	98	67	31		-	TRAIL
SS-34	5 LT.	29+35	9.5-10.5	A-2-4(0)	33	NP	53.7	23.1	15.1	8.0	97	59	26	-	-	TRAIL
SS-35	42 RT.	33+20	3.6-4.6	A-5(5)	43	10	25.8	20.7	27.4	26.2	100	82	60	-	-	RPD
SS-36	42 RT.	33+20	8.6-9.6	A-2-5(0)	53	9	42.1	30.6	19.3	8.0	95	66	31	-	•	RPD
SS-37	100 RT.	32+10	4.3-5.3	A-7-6(5)	42	15	31.4	19.3	21.1	28.2	99	78	52			RPD
SS-38	100 RT.	32+10	14.3-15.3	A-2-4(0)	27	NP	57.5	26.2	10.3	6.0	94	58	19	-	-	RPD
SS-39	100 RT.	32+10	19.3-20.3	A-7-6(6)	46	19	37.0	18.7	14.1	30.2	100	75	48		-	RPD
SAMPLES FROM PROJECT: 37870, BULB AVENUE EXTENSION OVER LONG CREEK																
SS-1	15 RT.	22+10	3.2-4.2	A-4(0)	30	7	36.6	26.5	22.7	14.2	95	73	40			<u> </u>
SS-1 SS-2	15 RT.	22+10	8.2-9.2	A-7-6(7)	42	20	28.3	26.1	23.3	22.3	100	84	51		<u> </u>	L
\$\$-2 \$\$-3	15 KT.	22+10	0.0-3.7		25	3	32.6	30.8	28.5	8.1	100	82	42	<u> </u>	-	i
33-3	1/ 1.1.		0.0-3.7	A-4(0)	L 23		32.0	30.0	20.0	0.1	100	υZ	4 <u>/</u>	I	l	L L

SHEET 67

SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

LINE **STATION PLAN PROFILE** 0+00 - 37+50 BORING LOGS SHEETS 9-20 SITE PHOTOS SHEETS 21-23

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY GASTON

PROJECT DESCRIPTION <u>I-85/US</u> 321 INTERCHANGE GEOMETRIC SAFETY IMPROVEMENTS SPECIAL: STREAM AGGRADATION INVESTIGATION INVENTORY

STATE PROJECT REFERENCE NO. 23 41153.1.1

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSES OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARBIOLS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR MSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6550. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (INP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NIDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE TRUTH ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HINSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

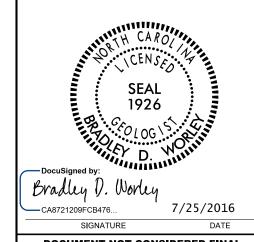
 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

B. WORLEY, PG A. STEWART J. BARE D. SUTTON INVESTIGATED BY <u>B. WORLEY</u>, PG DRAWN BY B. WORLEY, PG Summit Design and

SUBMITTED BY Engineering Services PLLC

PERSONNEL



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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REFERENC

PROJECT REFERENCE NO.	SHEET NO.
41153.1.1	2

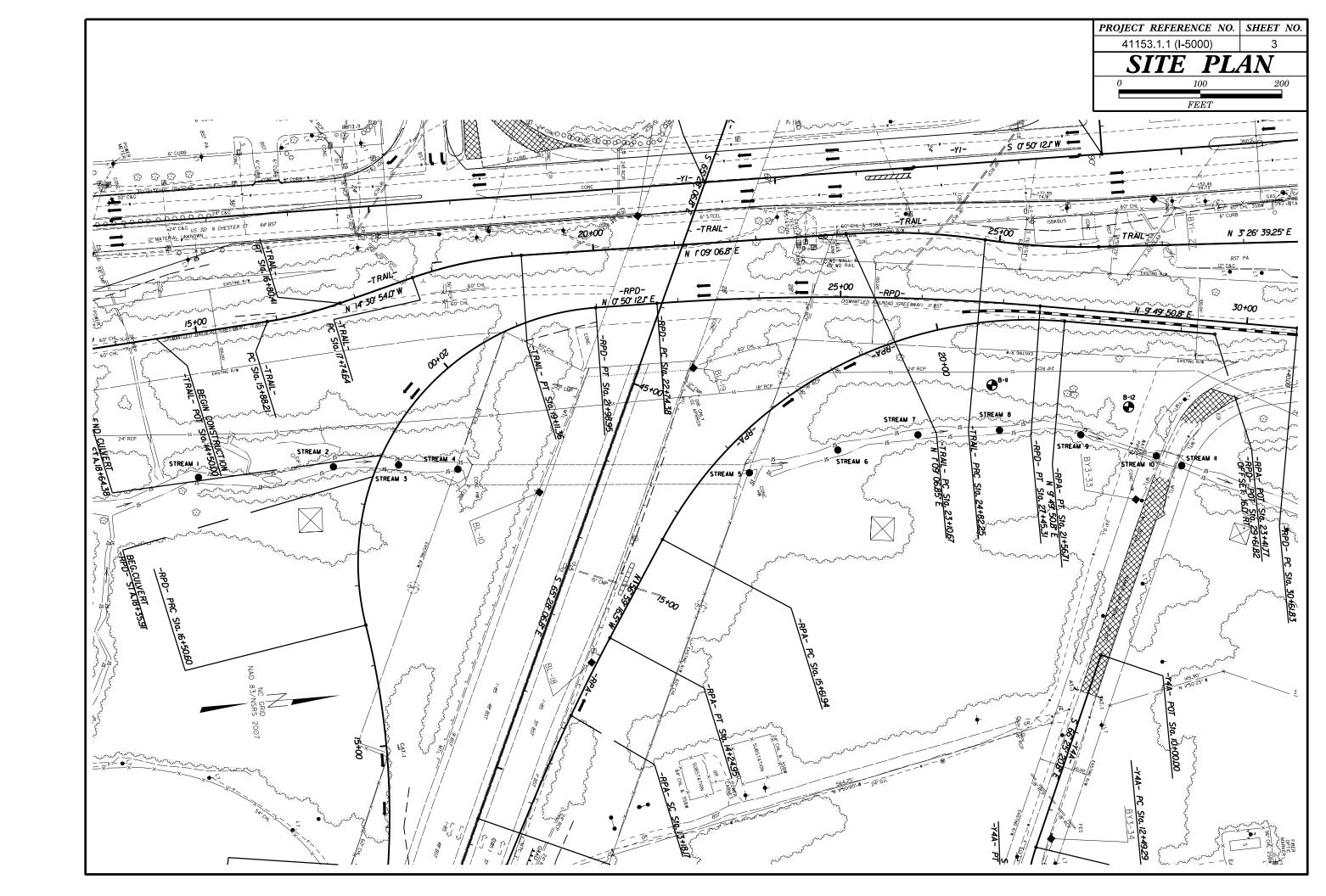
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

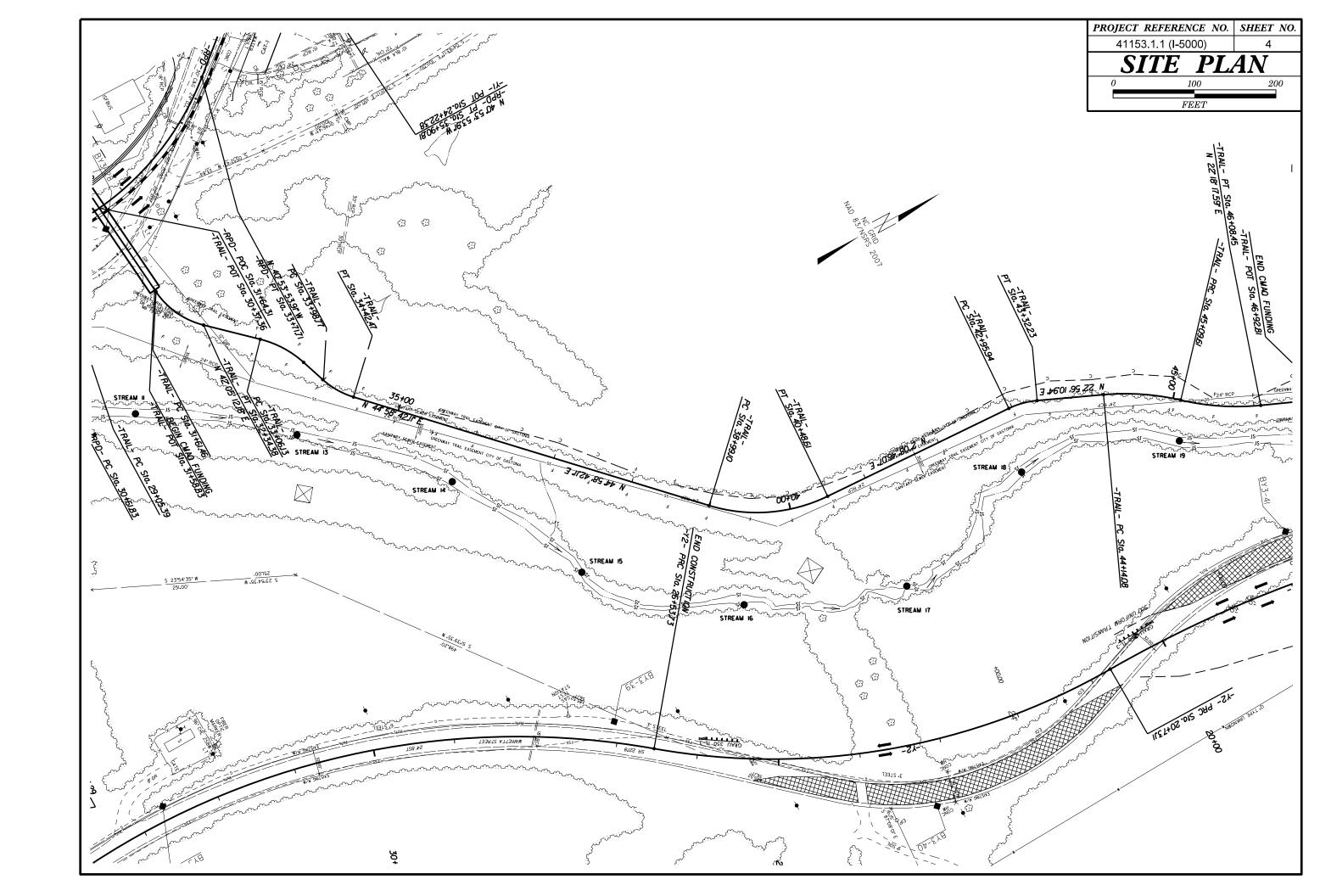
GEOTECHNICAL ENGINEERING UNIT

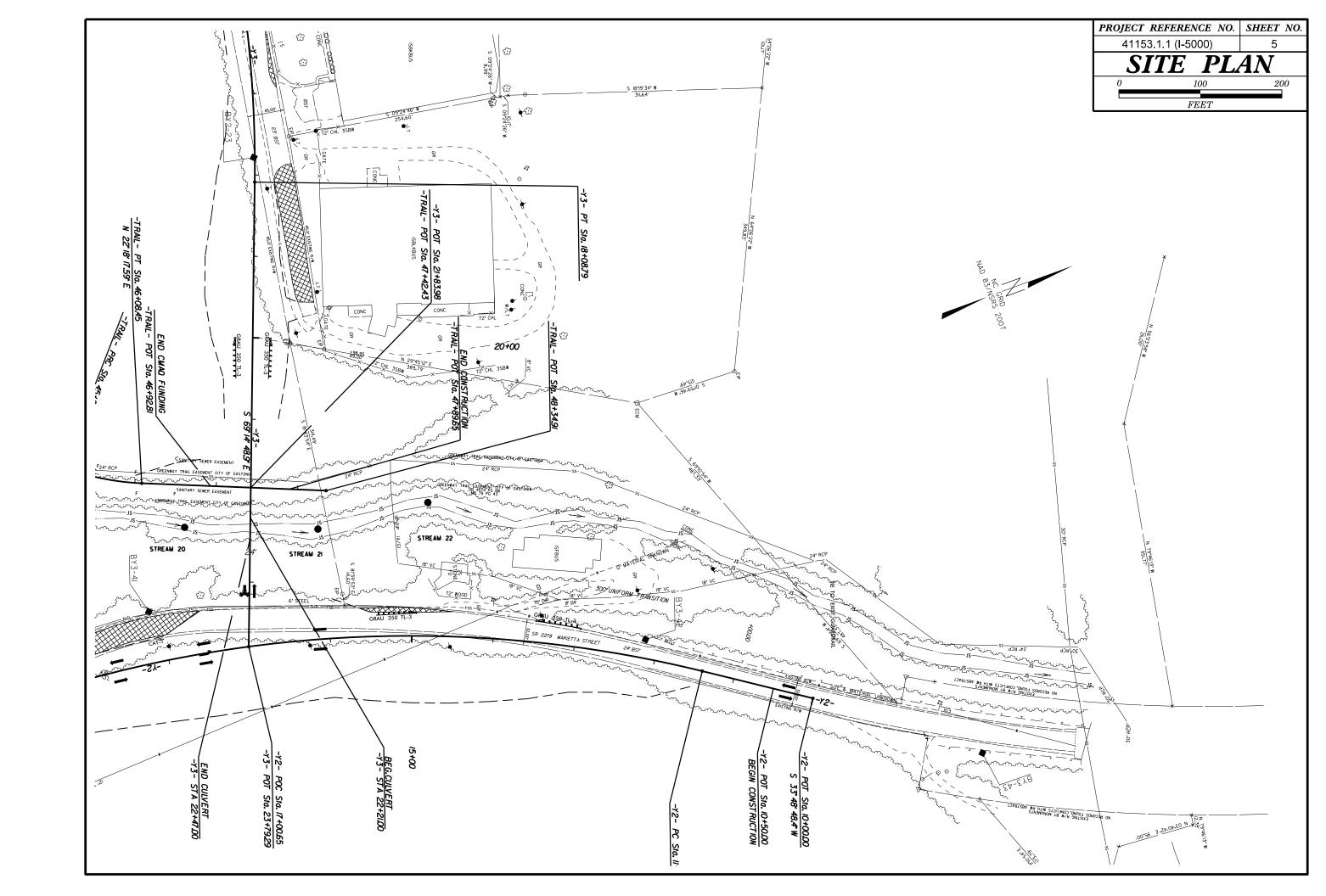
SUBSURFACE INVESTIGATION

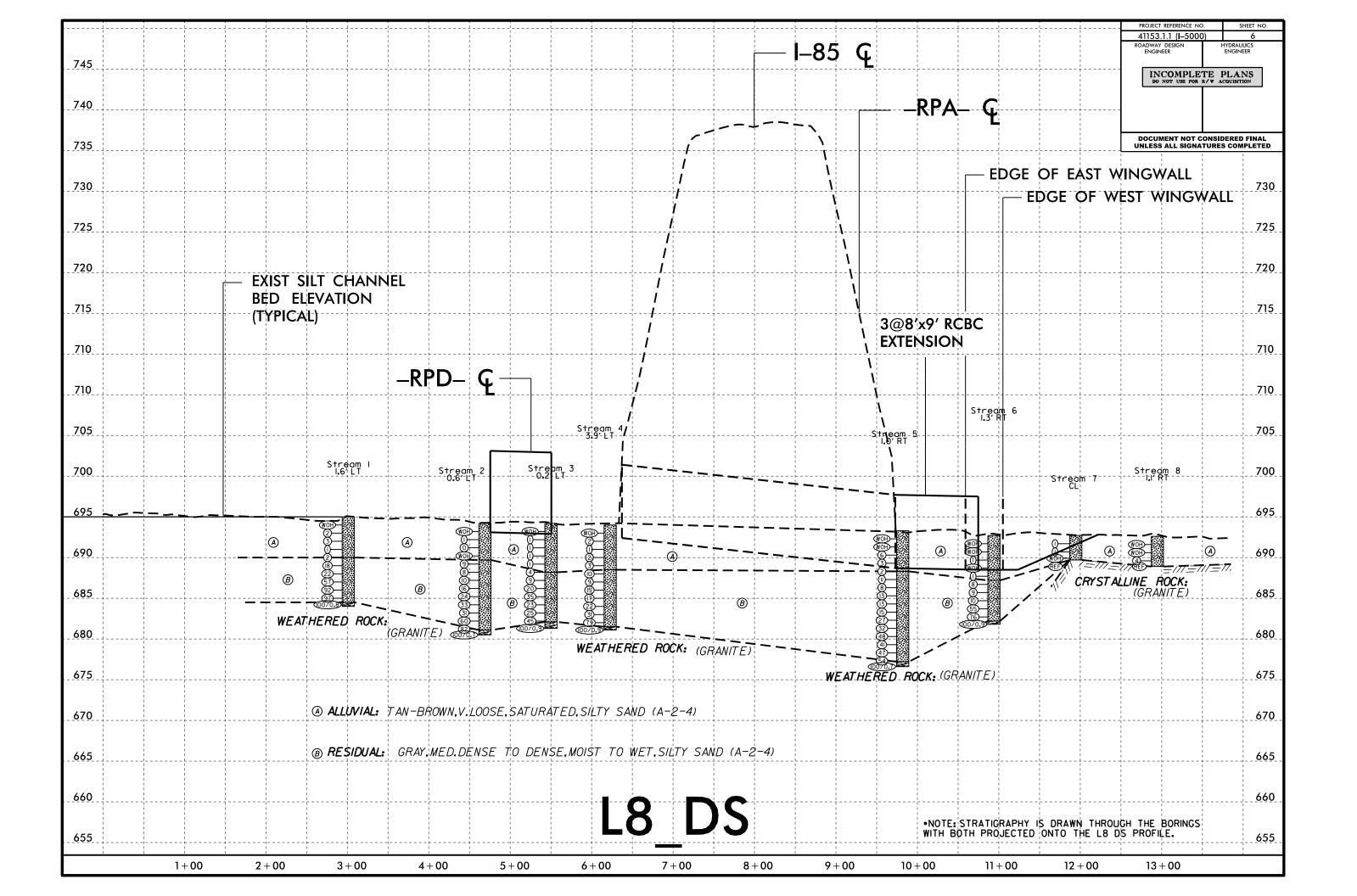
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

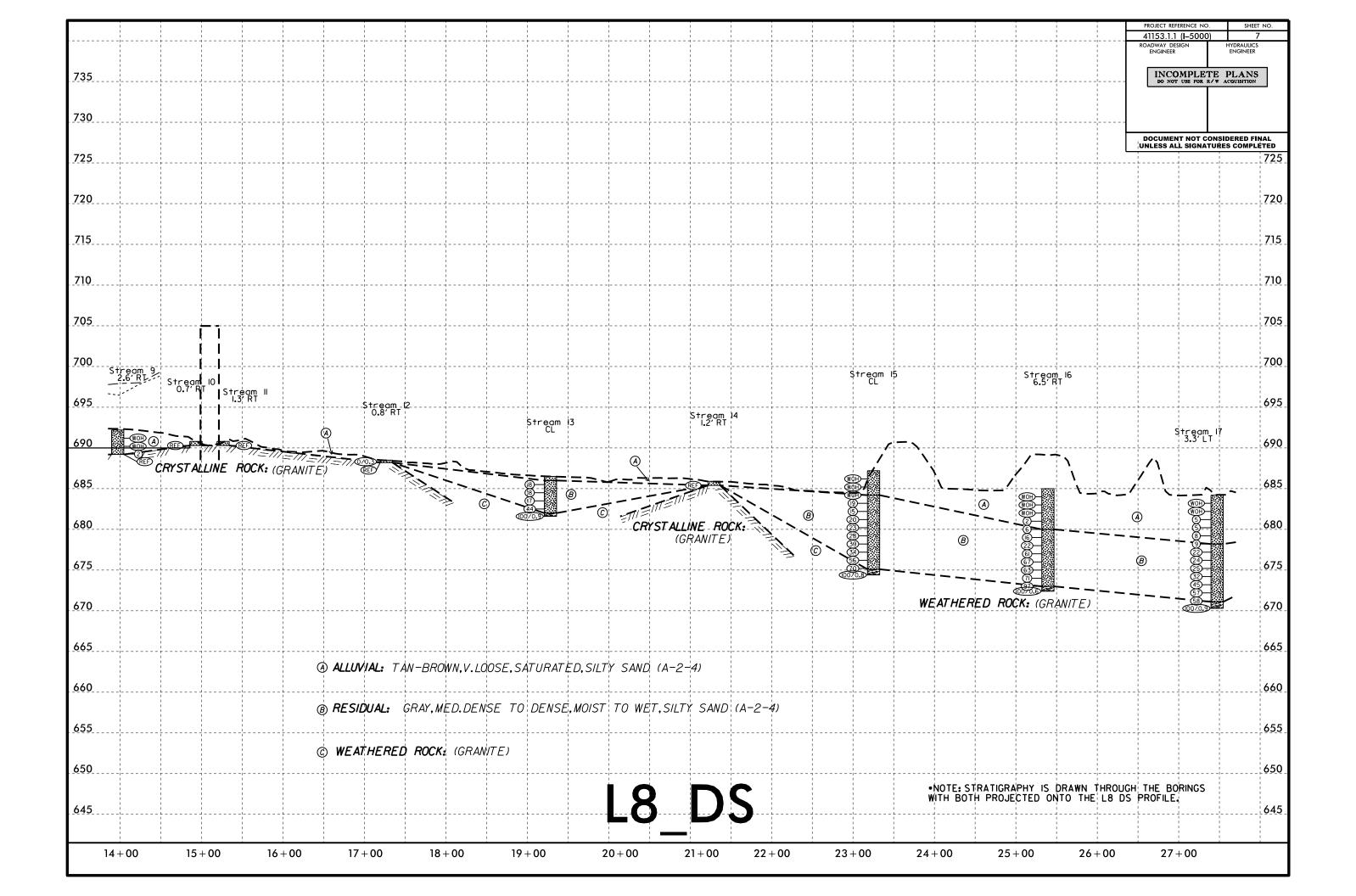
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND VIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PERTRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN Ø.I FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA, ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANDULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
	MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (WR) 100 BLOWS PER FOOT IF TESTED. CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GMEISS, GABBRO, SCHIST, ETC.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 G-6 A-7 A-1, A-2 A-4, A-5 A-6 A-7 A-1, A-2 A-4, A-5 A-6, A-7	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LL < 31	NON-CRYSTALLINE ROCK (NCR) FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
2. PASSING 19 GRANII AR SILT-	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50 PERCENTAGE OF MATERIAL	COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SEDIMENTARY ROCK SPT REFUSAL ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED (CP) SHELL BEDS, ETC.	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*40 30 MX 58 MX 51 MN 1 35 MX 35 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN SOILS SOILS SOILS	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	WEATHERING FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL PASSING *40 LL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	HAMMER IF CRYSTALLINE. VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN MODERATE GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF ORGANIC USUAL TYPES STONE FRAGS.	GROUND WATER	OF A CRYSTALLINE NATURE. SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI). I INCH. DPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL, AND SAND GRAVEL AND SAND SOILS SOILS	▼ STATIC WATER LEVEL AFTER <u>24</u> HOURS	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
CEN. RATING EXCELLENT TO GOOD FAIR TO POOR FAIR TO POOR POOR UNSUITABLE	E PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS RANGE OF STANDARD RANGE OF UNCONFINED DENMETATION RECISTENCE OF COMPRESSIVE STOPPINGTU	MISCELLANEOUS SYMBOLS The roadway embankment (re) 25/02/5 DIP & DIP DIRECTION	MODERATELY ALC NOW EXCEPT ODARTZ DISCOUNTED OR STAINED. IN GRANITOID HOURS, ALC FELDSPARS DULC SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL	FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
CONSISTENCY (N-VALUE) (TONS/FT ²) GENERALLY VERY LOOSE 4 TO 10	WITH SOIL DESCRIPTION → OF ROCK STRUCTURES → SOIL SYMBOL → SPT Det OMT TEST BORING INSTALLATION INSTALLATION	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SQLL. IN GRANLTOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LEDGE - HOLDE THE RIDGE OF PROJECTION OF NOTE WHOSE INTERNETS IS SHILL COMPANED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR GRANULAR MATERIAL MEDIUM DENSE 10 T0 30 N/A OENSE 00 T0 50 VERY DENSE 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING COME PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED SOIL BOUNDARY	(V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD > 30 > 4	######################################	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053 COARSE FINE 0.17 COARSE	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE SHALLOW UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE UNCLASSIFIED EXCAVATION - ACCEPTABLE DECAPADABLE ROCK UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - EXCEPTABLE DECAPADABLE ROCK SHALLOW UNDERCUT UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - EXCEPTABLE DECAPADABLE ROCK UNDERCUT UN	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SAND (CDB.) (GR.) (CSE. SD.) (F SD.) (SL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN. 12 3 SOIL MOISTURE - CORRELATION OF TERMS	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERD CL CLAY MOD MODERATELY 7 - UNIT WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC 7 - DRY UNIT WEIGHT	BY MODERATE BLOWS. MEDIUM CAN BE GROOVED OR COUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REDUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC ORG ORGANIC ONT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS DPT - DYNAMIC PENETRATION TEST SAP. SAPROLITIC S - BULK	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE LL LIQUID LIMIT	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID: REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRACS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FINGERNAIL. FRACTURE SPACING BEDDING	TOPSOL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. BENCH MARK: See Notes
(P) PL PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT	EQUIPMENT USED ON SUBJECT PROJECT DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: CME-45C CLAY BITS X AUTOMATIC MANUAL	WIDE MUNE THAN 10 FEET VERT THICKET BEDDED 4 FEET	ELEVATION: FEET NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	G* CONTINUOUS FLIGHT AUGER CORE SIZE:	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	Boring collar elevations derived using Geopak and the TIN file (15000_ls_tin (2).tin)
PLASTICITY INDEX (DI) DRY CTREATH	X 6° HOLLOW AUGERS	INDURATION FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	REF = SOUNDING ROD REFUSAL
PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	CME-550 HARD FACED FINGER BITS TUNGCARBIDE INSERTS HAND TOOLS:	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS: FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC	CASING W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
COLOR	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER TRICONE TUNGCARB. SOUNDING ROD	BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE: DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY), MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	CORE BIT VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1

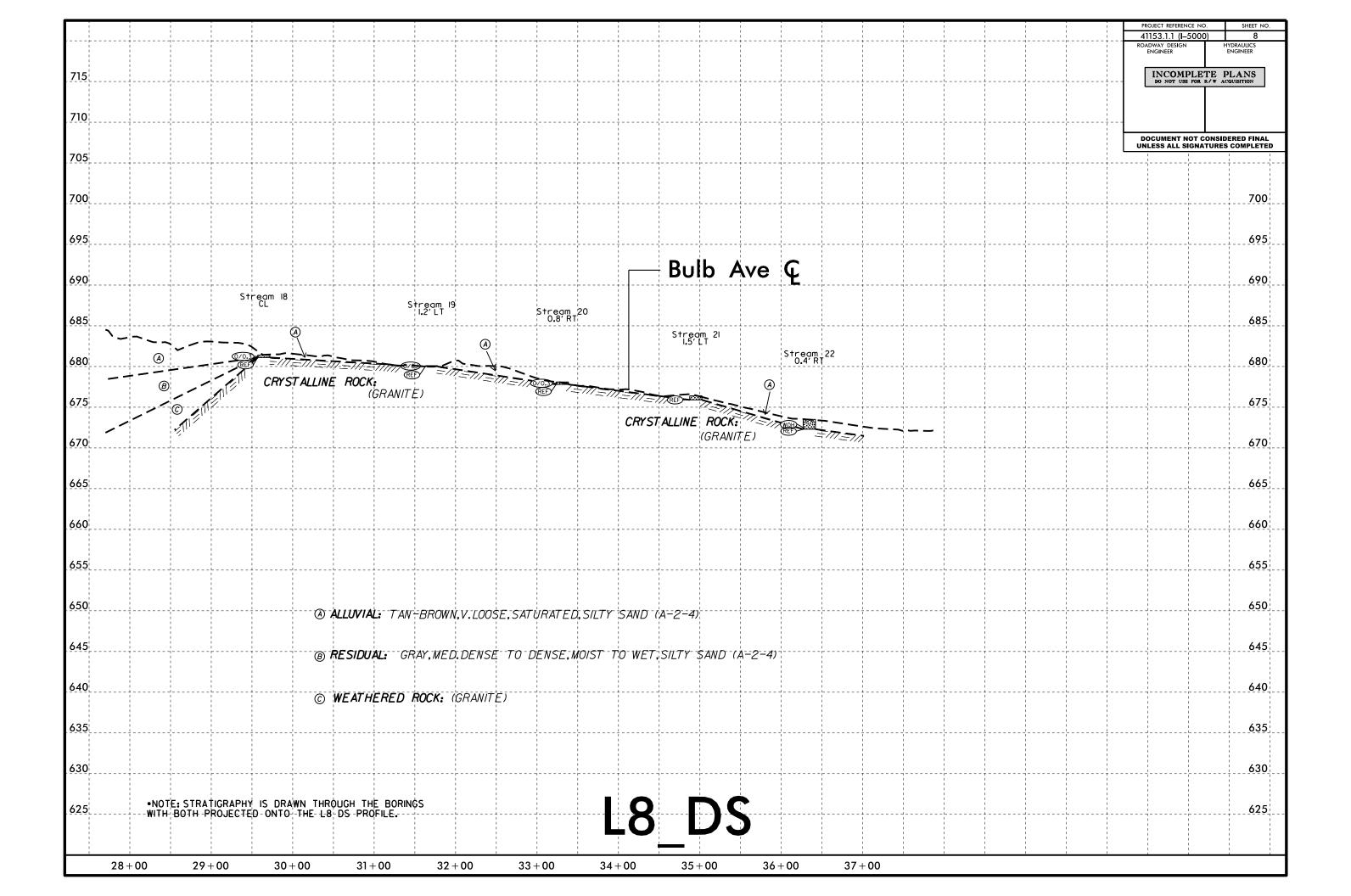


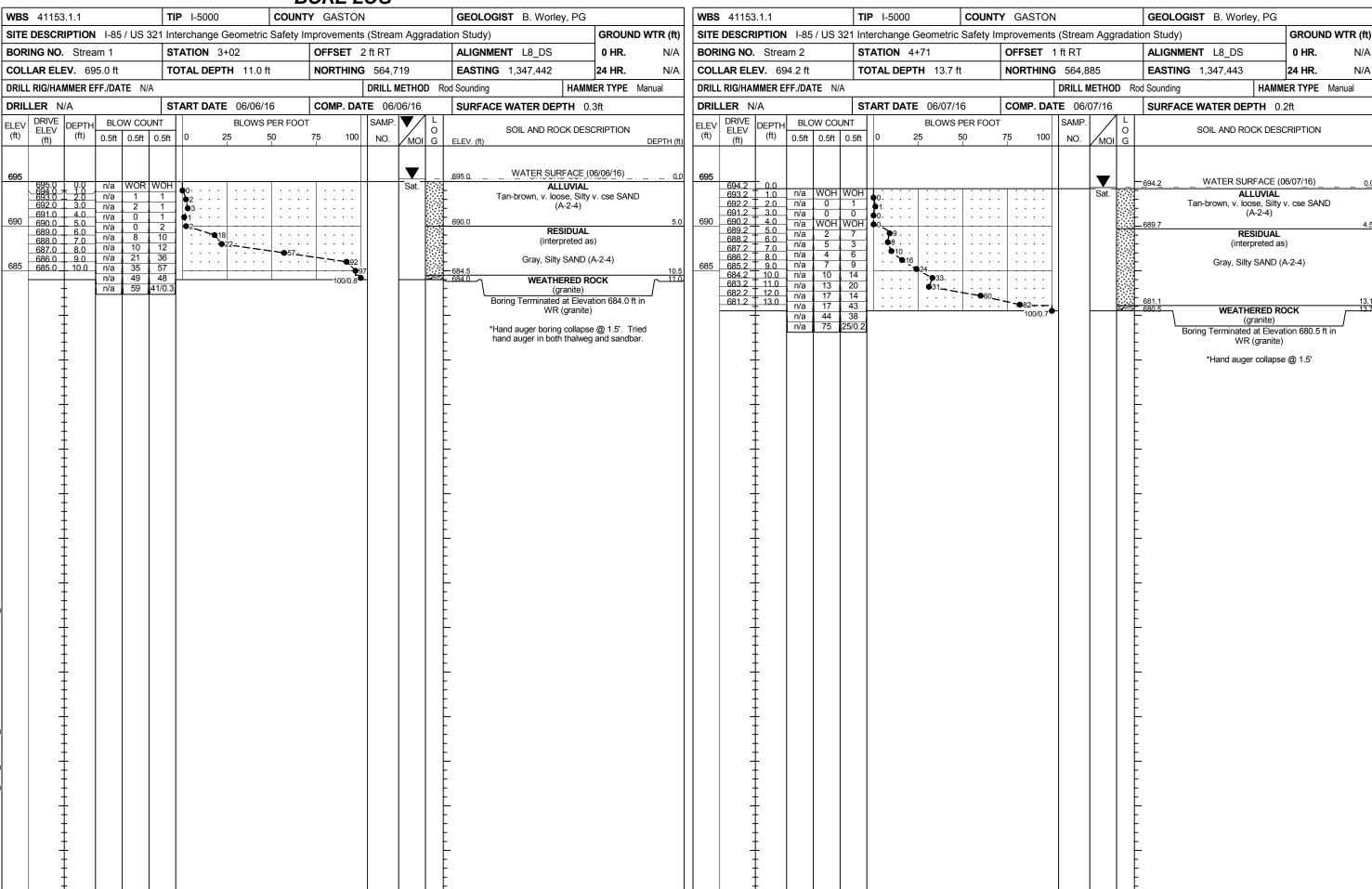


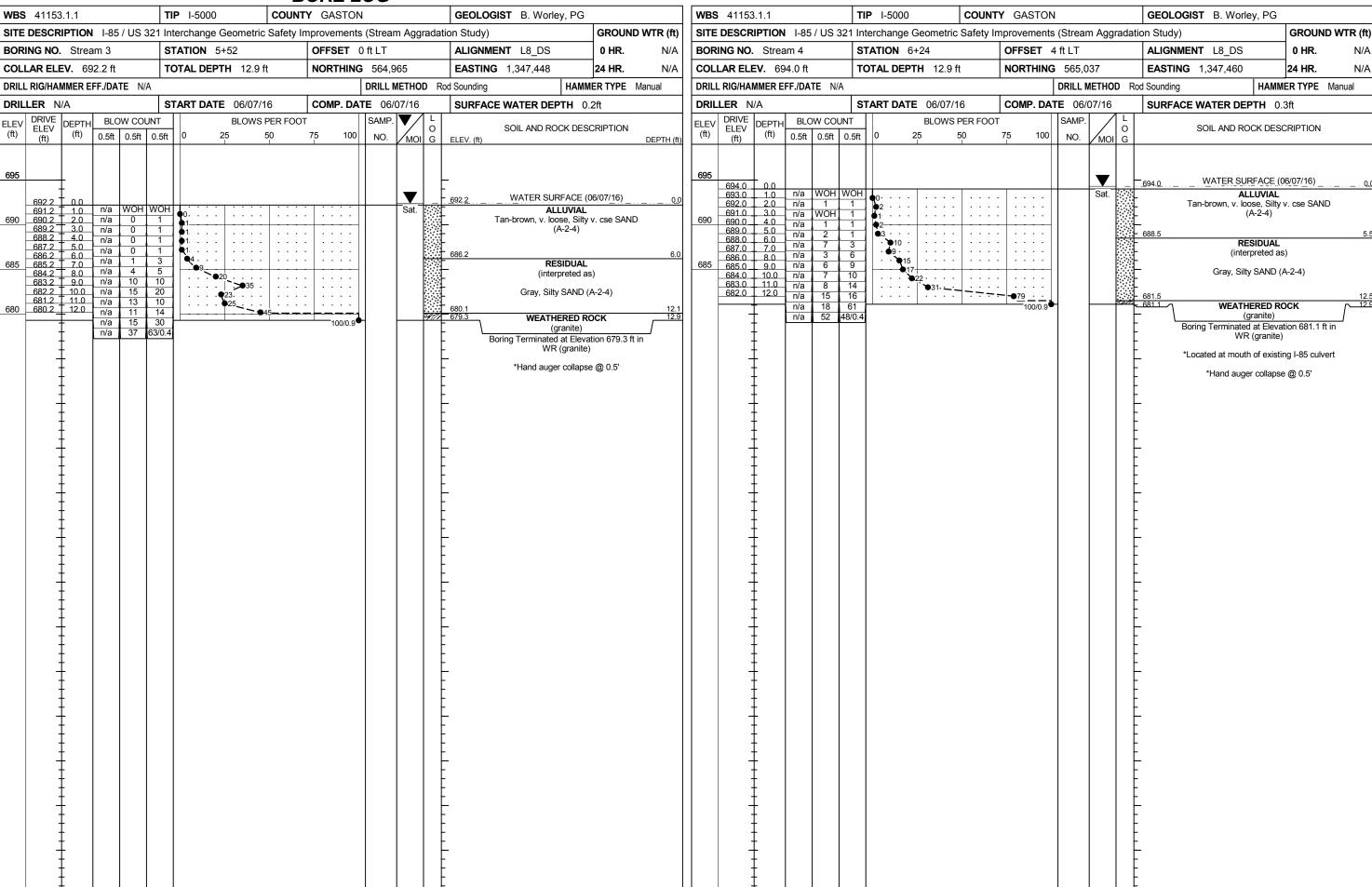


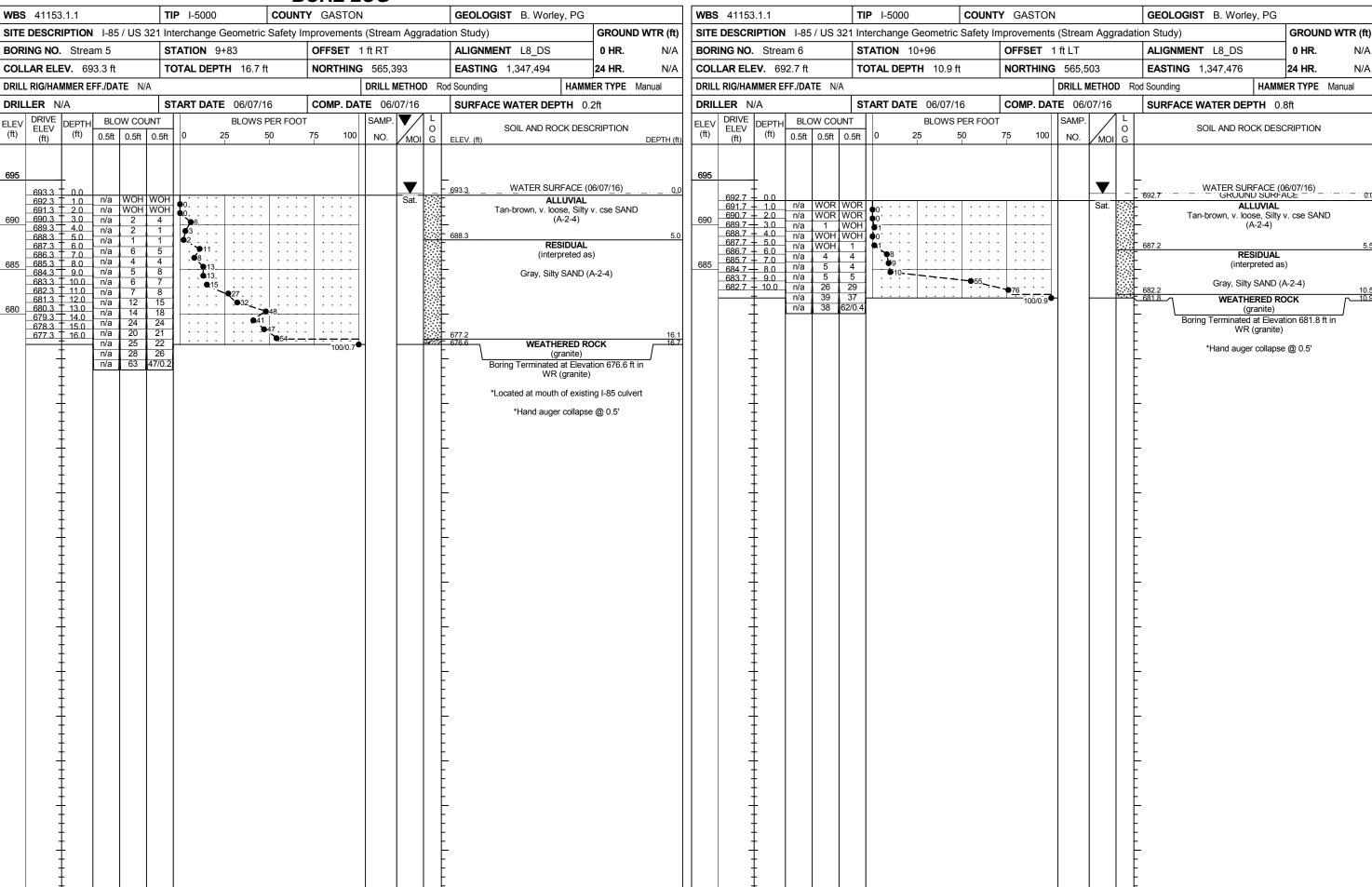


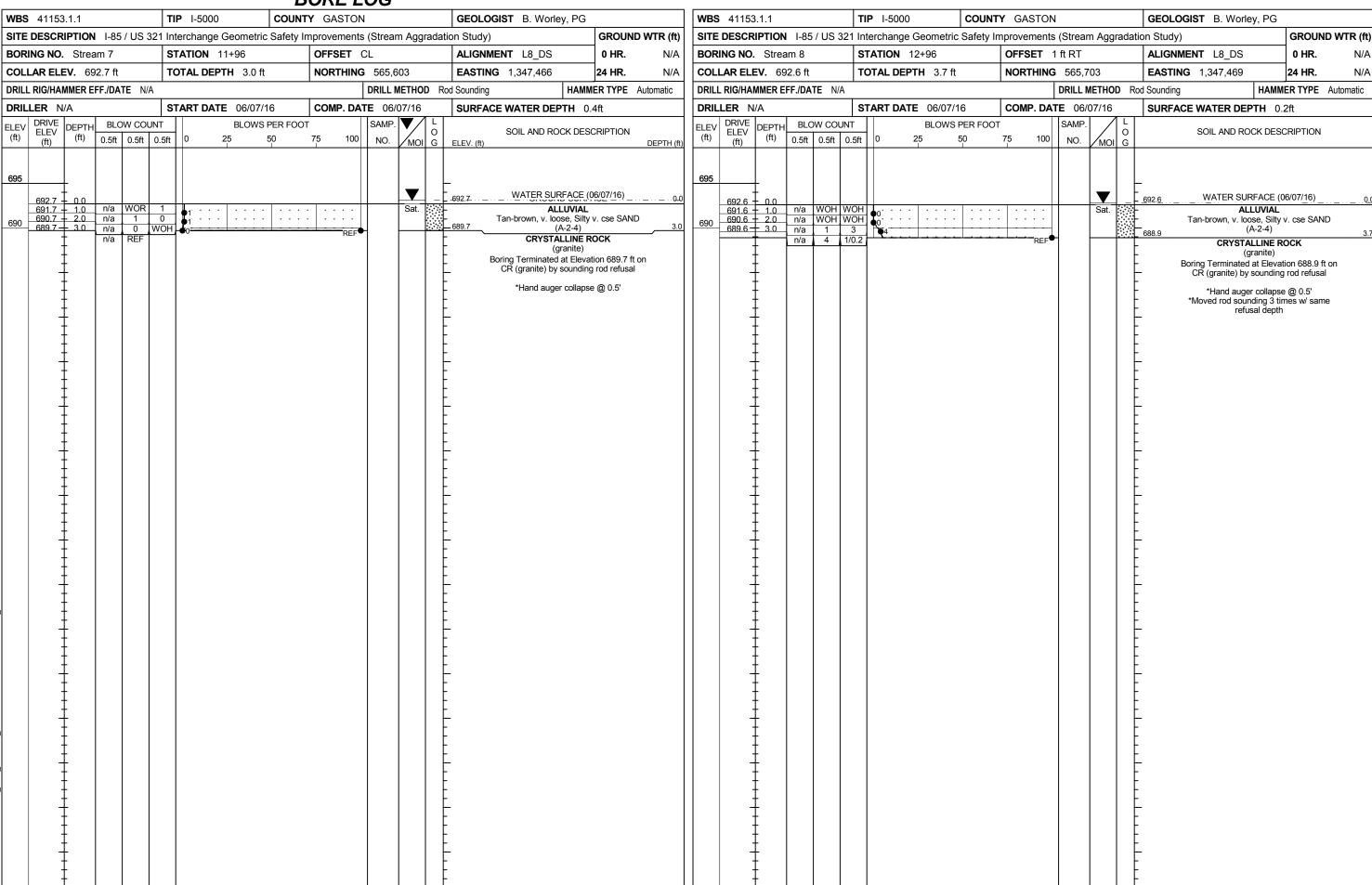


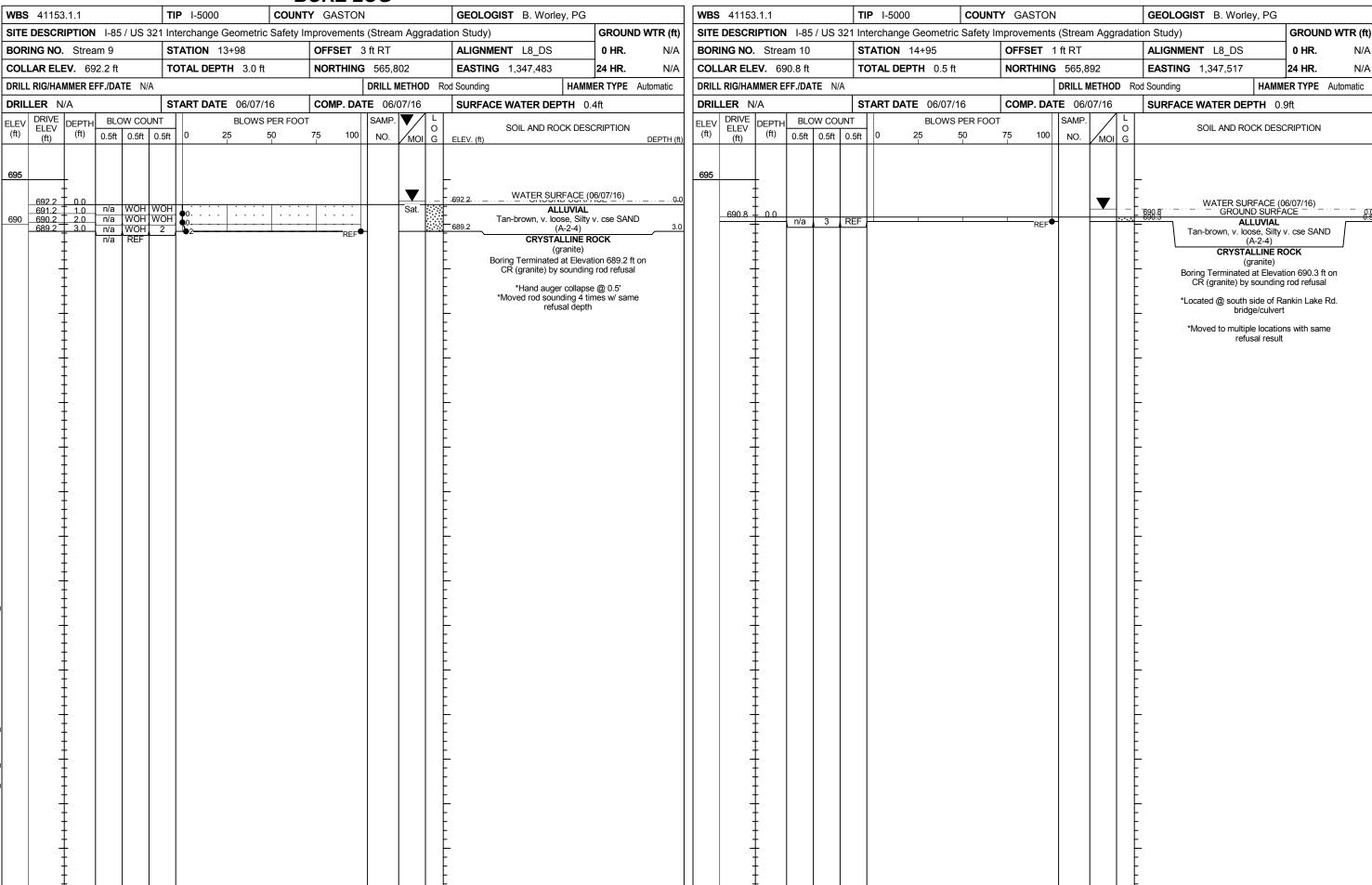












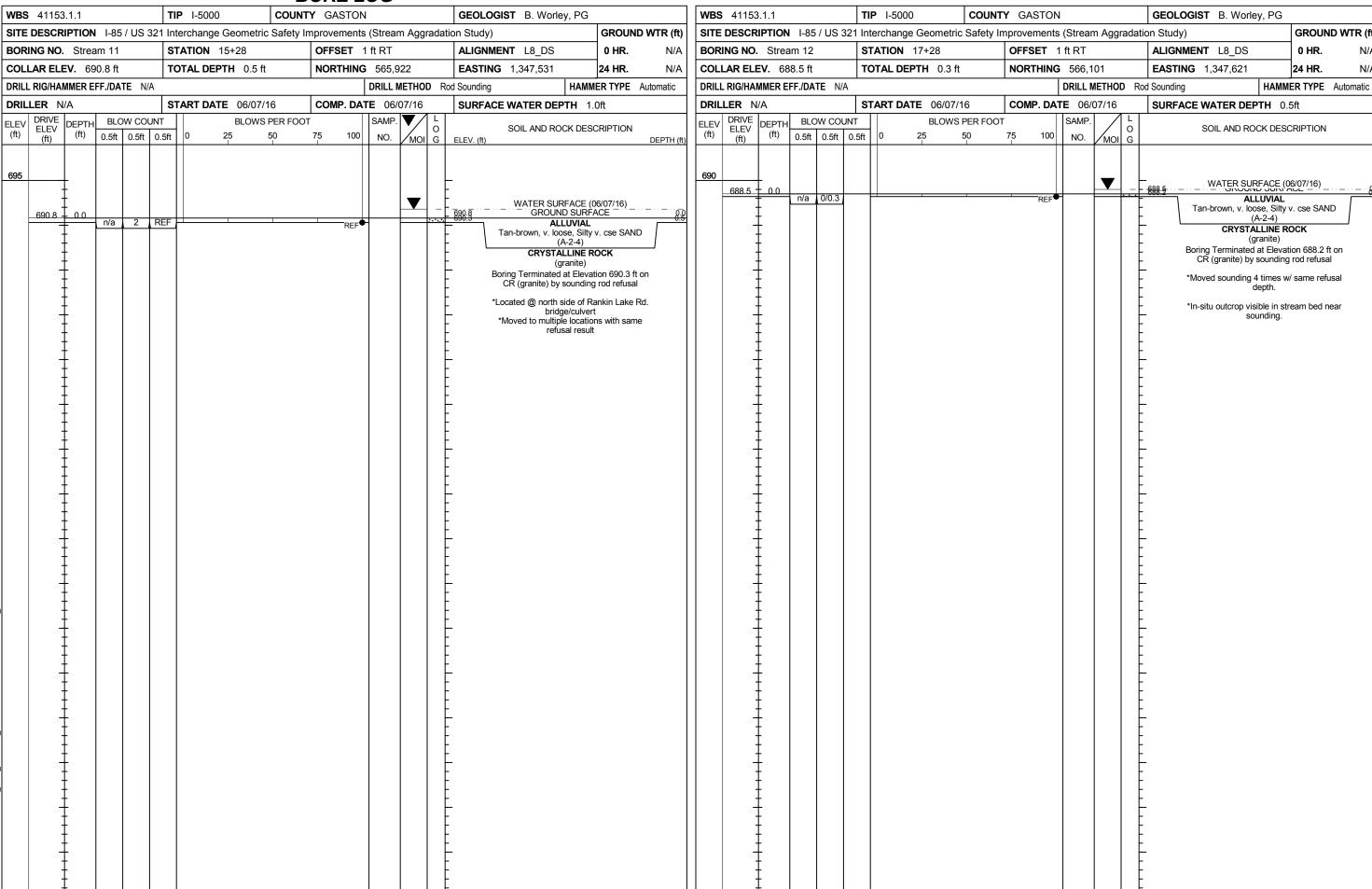
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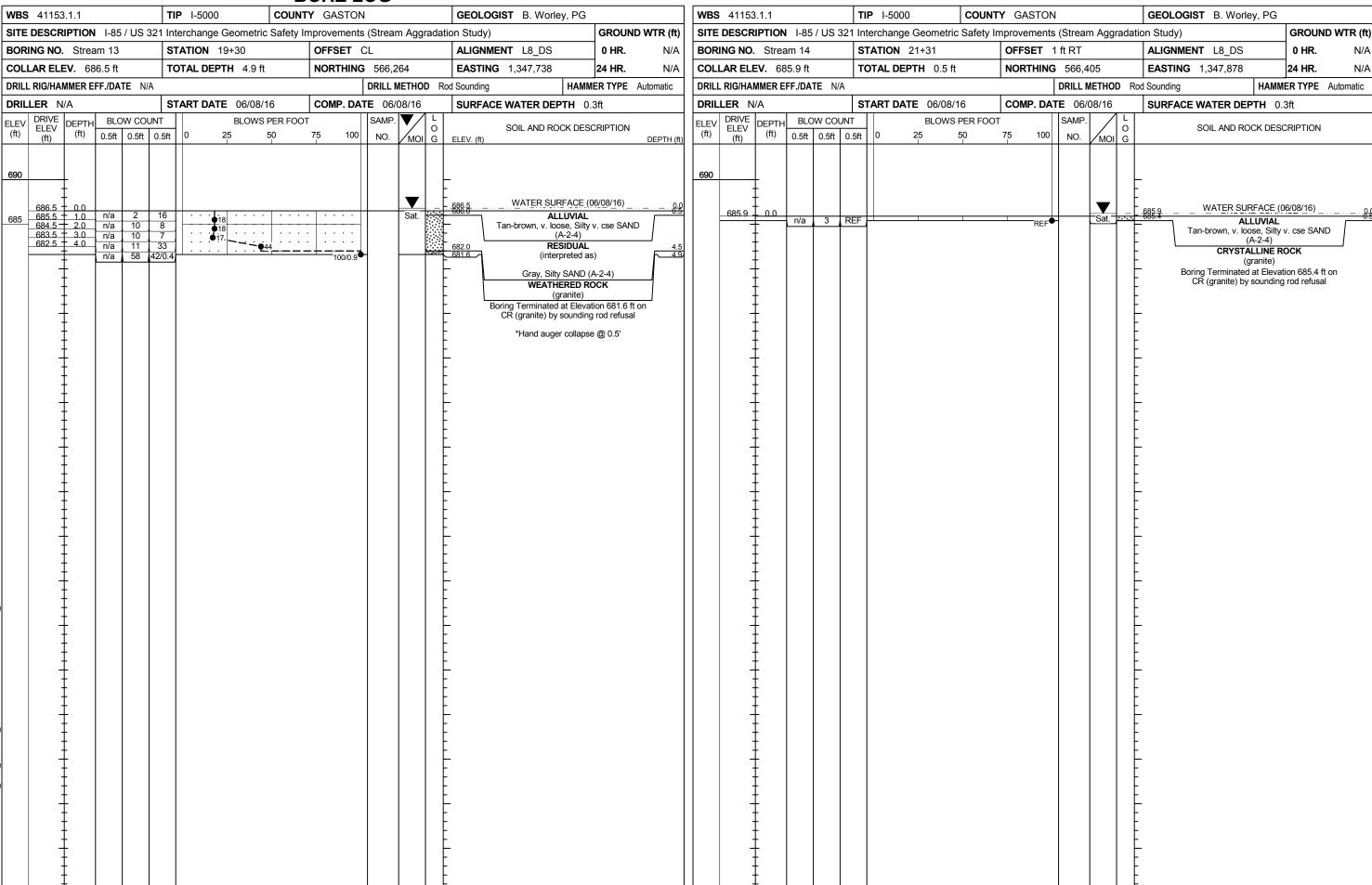
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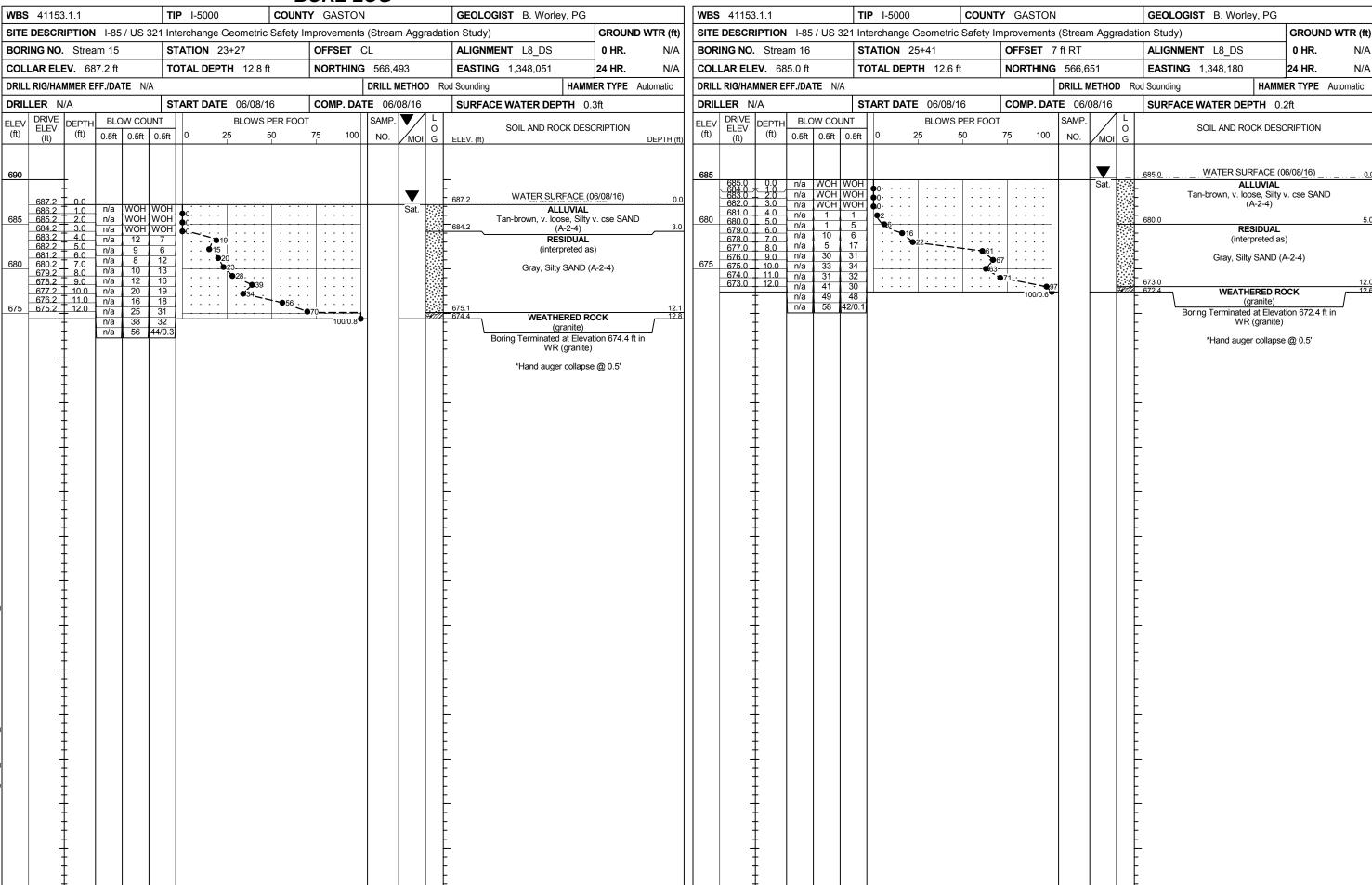
GROUND WTR (ft)

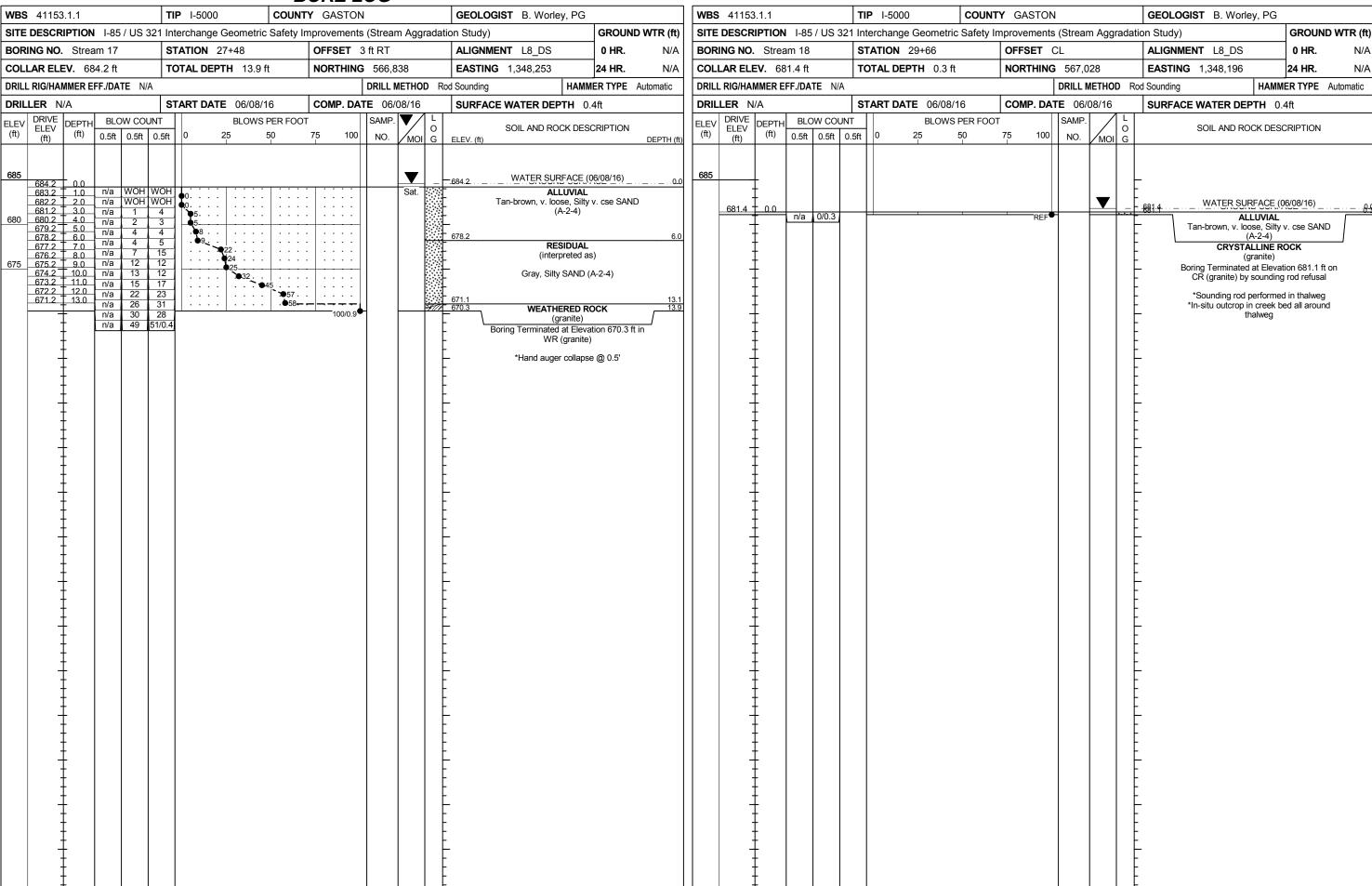
0 HR.

24 HR.





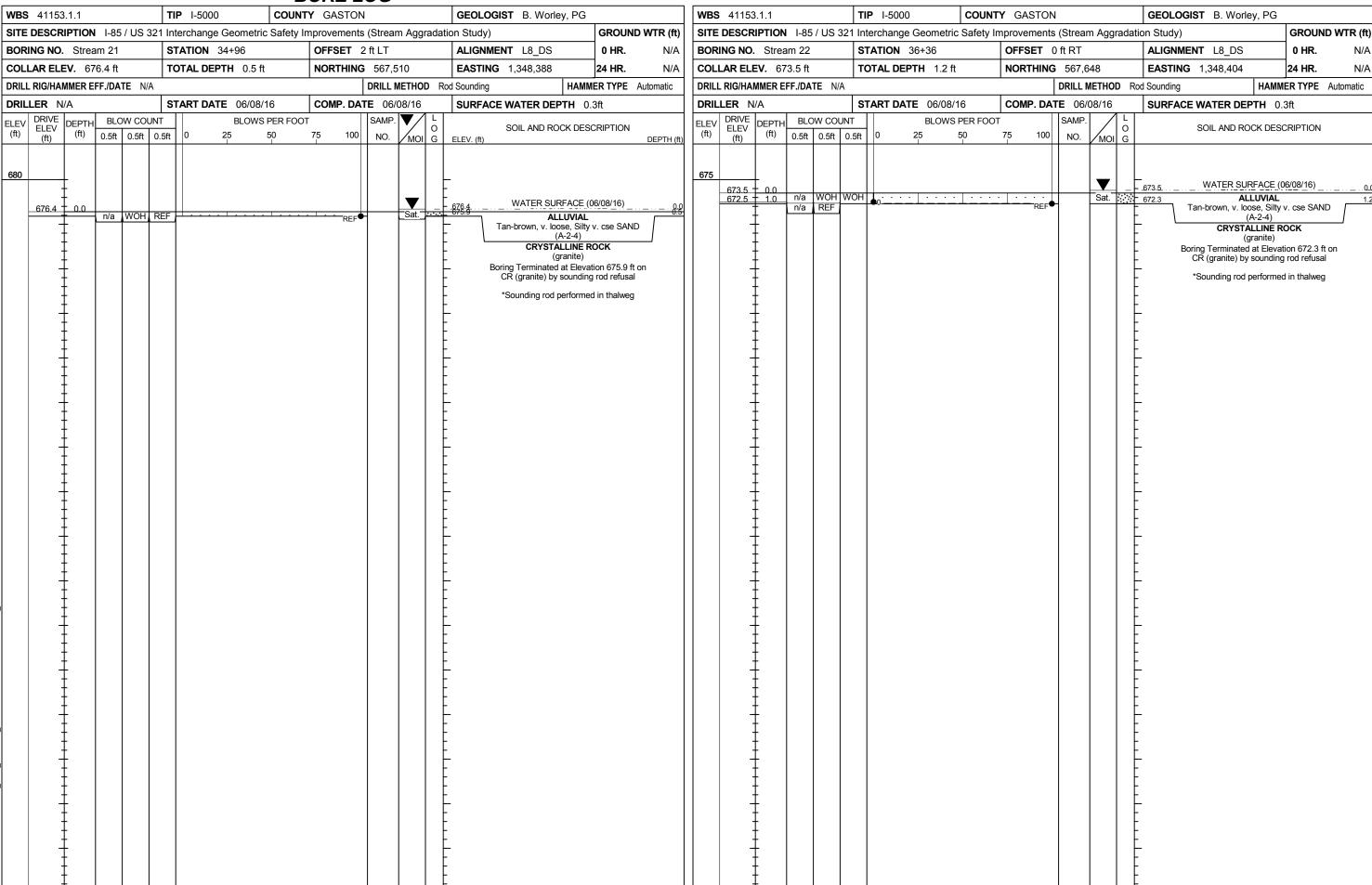




BORE LOG					<u> </u>				
WBS 41153.1.1	TIP I-5000 COUN	TY GASTON	GEOLOGIST B. Worley, PG	WBS 41153.1.1	TIP I-5000 COUN	TY GASTON	GEOLOGIST B. Worley, PG		
SITE DESCRIPTION I-85 / US 32	1 Interchange Geometric Safety	mprovements (Stream Aggradati	on Study) GROUND WTR (ft)	SITE DESCRIPTION I-85 / US 321	1 Interchange Geometric Safety	Improvements (Stream Aggrada	tion Study) GROUND WTR (ft)		
BORING NO. Stream 19 STATION 31+72		OFFSET 1 ft LT	ALIGNMENT L8_DS 0 HR. N/A	BORING NO. Stream 20	STATION 33+32	OFFSET 1 ft RT	ALIGNMENT L8_DS 0 HR. N/A		
COLLAR ELEV. 680.1 ft	TOTAL DEPTH 0.1 ft	NORTHING 567,217	EASTING 1,348,253 24 HR. N/A	COLLAR ELEV. 678.1 ft	TOTAL DEPTH 0.3 ft	NORTHING 567,357	EASTING 1,348,330 24 HR . N/A		
DRILL RIG/HAMMER EFF./DATE N/A		DRILL METHOD Ro	d Sounding HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE N/A		DRILL METHOD R	tod Sounding HAMMER TYPE Automatic		
DRILLER N/A START DATE 06/08/16		COMP. DATE 06/08/16			DRILLER N/A START DATE 06/08/16		SURFACE WATER DEPTH 0.2ft		
ELEV DRIVE DEPTH BLOW COUN		T SAMP.		ELEV DRIVE DEPTH BLOW COUNT	ļ.,,	COMP. DATE 06/08/16 SAMP. L	-		
(ft) ELEV (ft) 0.5ft 0.5ft 0.5ft 0		75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)	(ft) (ft) (ft) 0.5ft 0.5ft 0.	.5ft 0 25 50	75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION		
685				680					
				678.1 0.0			- ₆₇₈₋₁ WATER SURFACE (06/08/16) 8.0		
				n/a 0/0.3		REF	ALLUVIAL		
680 680.1 0.0 n/a 0/0.1		REF -	688.4 WATER SURFACE (06/08/16) 0.0	‡			Tan-brown, v. loose, Silty v. cse SAND (A-2-4)		
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			Tan-brown, v. loose, Silty v. cse SAND				CRYSTALLINE ROCK (granite)		
			(A-2-4) CRYSTALLINE ROCK				Boring Terminated at Elevation 677.8 ft on CR (granite) by sounding rod refusal		
			granite) Boring Terminated at Elevation 680.0 ft on				*Sounding rod performed in thalweg		
			CR (granite) by sounding rod refusal				*In-situ outcrop in creek bed all around		
			*Sounding rod performed in thalweg				thalweg		
			*In-situ outcrop in creek bed all around thalweg						
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NCDV +		‡							

N/A

N/A



BORE LOG							1	İ						
WBS 41153.1.1	TIP I-5000 COUNTY GASTON		GEOLOGIST Worley, B.		WBS 41153.1.1				COUNTY GASTO			GEOLOGIST Worley, B.		
SITE DESCRIPTION I-85 / US 321 Interchange Geometric Safety Improvements (Slope Boring		· · · · · · · · · · · · · · · · · · ·						 	Improvements (Slope Boring)				ΓR (ft)	
BORING NO. B-11 STATION N/A		OFFSET N/A	ALIGNMENT N/A	0 HR. 7.3	BORING NO. B-12			STATION N/A	OFFSET	OFFSET N/A		ALIGNMENT N/A		9.0
COLLAR ELEV. 697.9 ft TOTAL DEPTH 11.9 ft			EASTING 1,347,419	24 HR. 7.7								1,347,469		
DRILL RIG/HAMMER EFF./DATE SUM0093 DIEDRICH D-50 88% 11/05/2015		DRILL METHOD H	H.S. Augers HAMMER TYPE Aut		DRILL RIG/HAMMER EFF./DATE SUM		TE SUM009	93 DIEDRICH D-50 88% 11/0	05/2015	DRILL METHOD	HOD H.S. Augers		MMER TYPE Autor	matic
DRILLER Bare, J.	START DATE 05/25/16	COMP. DATE 05/25/16	SURFACE WATER DEPTH	I/A	DRILLER B			START DATE 05/25/1		ATE 05/25/16	SURFACE	WATER DEPTH	N/A	
DRILL RIG/HAMMER EFF./DATE SU	START DATE 05/25/16	COMP. DATE 05/25/16 OT 75 100 NO. MOI G M M M M M M M M M M M M M M M M M M	S. Augers HAMI SURFACE WATER DEPTH N SOIL AND ROCK DES	MER TYPE Automatic N/A SCRIPTION DEPTH (ft) FACE 0.0 ILL SAND (A-2-6) 0, 7.5') 7.5 ROCK 11.9') 11.9 ROCK ation 686.0 ft On	DRILLER B	MMER EFF./DA Bare, J. DEPTH BL (ft) 0.5ft	OW COUNT 0.5ft 0.5ft 2 4 1 2 2 4 1 2 2 4 1 2 2 4 1 2 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 1 2 4 4 1 2 4 4 4 4 4 4 4 4 4	START DATE 05/25/10 BLOWS F 0 25 5	05/2015	ATE 05/25/16 SAMP.	H.S. Augers SURFACE CO G -699.5 -699.5 -688.0 -688.0	HA	RFACE FILL ND (A-2-6) 11.5') ROCK 10 (12.9') vation 687.6 ft On (Granite) ROCK	8.2 matic 0.0
NCDOT BORE DOUBLE 15000_GEO_SPECIAL_STREAMAGGRADATION.GPJ NC_DOT.GDT 6/24/16														

SITE PHOTOGRAPHS

Stream Aggradation Study



STREAM 1: View Facing North



STREAM 5: View Facing South



STREAM 2: View Facing North





STREAM 3: View Facing North



STREAM 7: View Facing North



STREAM 4: View Facing North



STREAM 8: View Facing North

SITE PHOTOGRAPHS

Stream Aggradation Study



STREAM 9: View Facing North



STREAM 13: View Facing North



STREAM 10: View Facing South



STREAM 14: View Facing North



STREAM 11: View From Rankin Lake Rd.



STREAM 15: View Facing North



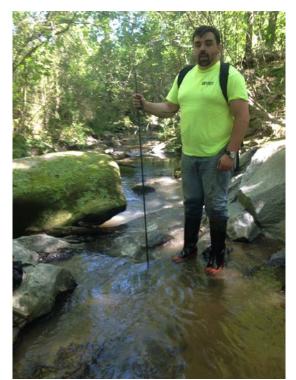
STREAM 12: View Facing North



STREAM 16: View Facing South

SITE PHOTOGRAPHS

Stream Aggradation Study



STREAM 17: View Facing North



STREAM 21: View Facing North



STREAM 18: View Facing North





STREAM 19: View Facing North



STREAM 20: View Facing North