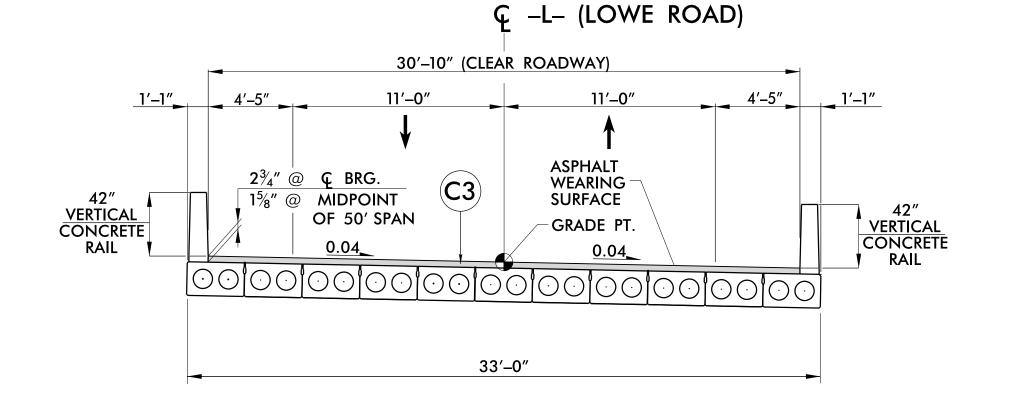
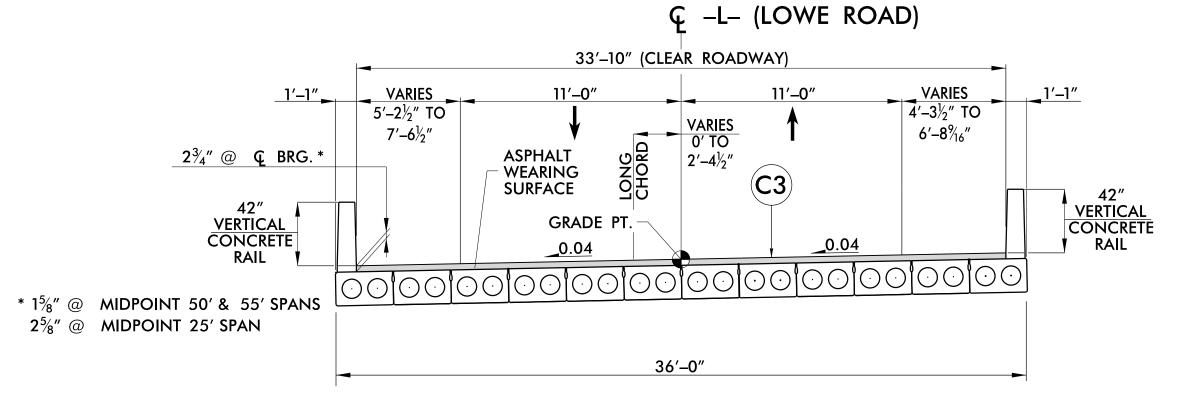
						PROJECT REFERENCE NO.	SHEET NO.
						B-5333	2A-2
FINAL PAVEMENT SCHEDULE						ROADWAY DESIGN ENGINEER CARO	PAVEMENT DESIGN ENGINEER
ITEM	DESCRIPTION	ITEM	DESCRIPTION	ITEM	DESCRIPTION	OFESS /ON NA	poolisigned by: SEAL Clark Stoarisan
C 1	PROPOSED APPROXIMATE 1.25" ASPHALT CONC. SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 137.50 LBS. PER SQ. YARD.	E2	PROP. APPROX. 4.5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 513 LBS. PER SQ. YARD.	T1	AGGREGATE SHOULDER BORROW	Docusioned by: SEAL Mori Document of the control	Clark 2 Bossison
C2	PROPOSED APPROXIMATE 2.5" ASPHALT CONC. SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 137.50 LBS. PER SQ. YARD IN EACH OF TWO LAYERS.	E3	PROP. VAR. DEPTH ASPHALT CONC. BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH, TO BE PLACED IN LAYERS NOT GREATER THAN 5.5" IN DEPTH OR LESS THAN 3" IN DEPTH.	U	EXISTING PAVEMENT		2/7/2017 MORRIEN
C3	PROP. VAR. DEPTH ASPHALT CONC. SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YARD PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 1.5" IN DEPTH.	R1	SHOULDER BERM GUTTER	V	MILLING (SEE MILLING DETAIL ON THIS SHEET)	DOCUMENT NOT COUNTESS ALL SIGNATES Dewberr	URES COMPLETED 2610 WYCLIFF ROAD
El	PROP. APPROX. 4.0" ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YARD.	T	EARTH MATERIAL	W	WEDGING (SEE WEDGE DETAIL ON THIS SHEET)	N P/	NC COA No. F-0929 C DEPARTMENT OF TRANSPORTATION AVEMENT MANAGEMENT UNIT 193 MAIL SERVICE CENTER ALEIGH, NC 27699–1593

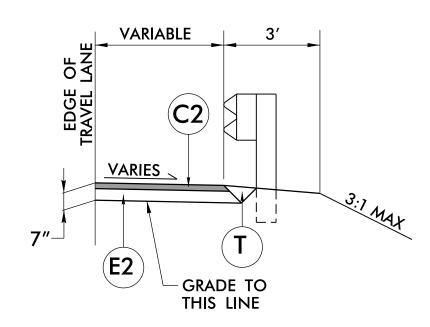
NOTE: PAVEMENT EDGES ARE 1:1 UNLESS OTHERWISE NOTED.



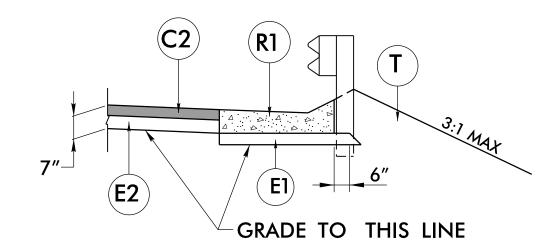
TYPICAL SECTION ON STRUCTURE (BRIDGE 173) -L- STA. 20+30.62 (BEGIN BRIDGE) TO STA. 22+83.37 (END BRIDGE)



TYPICAL SECTION ON STRUCTURE (BRIDGE 174) -L- STA. 36+62.19 (BEGIN BRIDGE) TO STA. 37+94.81 (END BRIDGE)



FULL DEPTH PAVED SHOULDER DETAIL NOT TO SCALE



DETAIL SHOWING SHOULDER BERM GUTTER (SBG) ON TOP OF SUBGRADE

-L- STA. 20+00.00 TO -L- STA. 20+19.64 - RT.

-L- STA. 22 + 94.58 TO STA. 23 + 14.00 - RT.

-L- STA. 36+30.00 TO -L- STA. 36+49.93 - LT.