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STATE	STA1	E PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.		1	35	
STAT	E PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	ION
53	008.1.1	NHPP-0040(028)	P.E.	
53	008.3.1	NHPP-0040(028)	CONS	5T.



STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# DAVIE COUNTY

LOCATION: DAVIE COUNTY

BRIDGE #61 ON I-40 EBL OVER DUTCHMANS CREEK BRIDGE #62 ON I-40 WBL OVER DUTCHMANS CREEK BRIDGE #74 ON I-40 EBL OVER CEDAR CREEK BRIDGE #75 ON I-40 WBL OVER CEDAR CREEK

TYPE OF WORK: BRIDGE PRESERVATION- HYDRO-DEMOLITION, SCARIFICATION, LATEX MODIFIED CONCRETE-VERY EARLY STRENGTH, PAINTING EXISTING BEARING PLATES, JOINT DEMOLITION, SUBSTRUCTURE REPAIRS USING SHOTCRETE AND EPOXY RESIN INJECTION.

# INDEX OF SHEETS

SHEET NO.	DESCRIPTION
1	TITLE SHEET
14	INDEX OF SHEETS
S-1 THRU S-16	STRUCTURAL PLANS (
S–17 THRU S–33	STRUCTURAL PLANS (
S-34	JOINT DETAILS (BRID
<b>S</b> –35	SUBSTRUCTURE REPA
SN	STANDARD NOTES

NO: CONTRACT

STATE	STAT	E PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.		1A	35	
STAT	E PROJ. NO.	DESCRIPT	ION	
53	008.1.1	NHPP_0040(028)	P.E.	
53	008.3.1	NHPP-0040(028)	CONS	T.

(BRIDGES NO. 61 & 62) (BRIDGES NO. 74 & 75) GES NO. 61, 62, 74 & 75) NIR DETAILS (BRIDGES NO. 61, 62, 74 & 75)



\*\* THE QUANTITY OF LATEX MODIFIED CONCRETE OVERLAY-VES INCLUDES THE 4"OVERLAP BETWEEN OVERLAYS.

DRAWN BY :	J.D. HAWK	DATE :	<u>11/15</u>
Checked By :	W. SMITH	DATE :	1/16

FOR CONTROL OF TRAFFIC AND LIMITS ON PHASING OF CONSTRUCTION, SEE NCDOT STANDARD DRAWINGS 1101.02 SHEETS 3, 4, 8, 9 AND 10. FOR BRIDGE JOINT DEMOLITION, SEE SPECIAL PROVISIONS. FOR LATEX MODIFIED CONCRETE OVERLAY - VERY EARLY STRENGTH, SEE SPECIAL PROVISIONS. FOR FOAM JOINT SEALS, SEE SPECIAL PROVISIONS. FOR ELASTOMERIC CONCRETE, SEE PROVISIONS. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS. FOR VOLUMETRIC MIXER, SEE SPECIAL PROVISIONS. FOR CONCRETE FOR DECK REPAIRS, SEE SPECIAL PROVISIONS. CLEANING AND PAINTING OF EXISTING BEARING PLATES WITH HRCSA, SEE SPECIAL PROVISIONS.

ON GE	ELASTOMERIC CONCRETE						
	CU.FT.						
	20.0				Τ·	-5765	
	20.0	Г Г	RUJEU			0100	
	40.0	-		DAV.	LL	CO	UNTY
		E	SRIDGE	E NO F 2	61	<u> </u>	2
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I HEREBY CERTIFY THAT THIS STRUCTURE(S) WAS REHABILITATED ACCORDING TO THESE PLANS OR

DATE

AS NOTED THEREIN.

RESIDENT ENGINEER



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REPAIR QUANTITY TABLE								
END DENIT 1		QUANTITIES						
END BENT I	ESTI	MA	TE	ACTUAL				
SHOTCRETE REPAIRS	AREA SF	V	OLUME CF	AREA SF	VOLUME CF			
CAP (VERTICAL FACE)	0.0		0.0					
CAP (HORIZONTAL, CORNER)	0.0		0.0					
EPOXY RESIN INJECTION	1		LN. FT		LN. FT			
САР			0.0					
EPOXY COATING	AREA SF							
САР	58.5							
END BENT 2		QUANTITIES						
	ESTI	MATE		ACTUAL				
SHOTCRETE REPAIRS	AREA SF	۷	OLUME CF	AREA SF				
CAP (VERTICAL FACE)	0.0		0.0					
CAP (HORIZONTAL, CORNER)	0.0		0.0					
EPOXY RESIN INJECTION	l		LN. FT		LN. FT			
САР			10.0					
EPOXY COATING	AREA SF							
CAP	58.5	]						

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN.OF 1"BEHIND REBAR AND MIN.2"CL TO SAWCUT.SEE REPAIR DETAILS.

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.								
CONTRACTOR SHALL SAW CUT TO A MINIMUM DEPTH OF $V_2^{\prime\prime}$ BUT REINFORCING STEEL SHALL NOT BE DAMAGED.								
CONTRACTOR SHALL REMOVE SURFACE CONCRETE TO VERIFY THAT SAWCUT DEPTH WILL NOT DAMAGE EXISTING REINFORCING STEEL.								
FOR SHOTCRETE REPAIRS, SEE	SPECIAL	PROVISION	s.					
FOR EPOXY RESIN INJECTION	,SEE SPEC	IAL PROVI	SI	DNS.				
FOR EPOXY COATING, SEE SPE	CIAL PROV	ISIONS.						
FOR REPAIR DETAILS, SEE TY	PICAL CAP	AND COLU	MN	REPAIR	:			
PAIRS	PROJE(	DAVI	 [ E	I :	<u>-5765</u> CC	D UNTY		
INJECTION								
DocuSigned by	DEPA	STAT	E OF					
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REPAIR QUAN	ΓΙΤ`	Υ ΤΑ	BLE			
DEDATOS DENIT 1		QUANTITIES				
REFAIRS DENT I	EST:	ΕΜΑΤΕ	AC	TUAL		
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF		
CAP (VERTICAL FACE)	163.0	41.0				
CAP (HORIZONTAL, CORNER)	0	0				
COLUMN (HORIZONTAL FACE)		0				
EPOXY RESIN INJECTI	ON	LN. FT	l	_N. - T		
CAP		0				
COLUMN		0				
EPOXY COATING		SO. FT		50. T		
TOP OF CAP		92.0				

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE.IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE. CONTRACTOR SHALL SAW CUT TO A MINIMUM DEPTH OF  $^{\prime}\!/_{2}{}^{\prime\prime}$  BUT REINFORCING STEEL SHALL NOT BE DAMAGED. FOR REPAIR DETAILS, SEE <code>``TYPICAL CAP AND COLUMN REPAIR DETAILS''</code> SHEET. FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS. FOR EPOXY COATING, SEE SPECIAL PROVISIONS. FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

- SHOTCRETE REPAIRS

ERI - EPOXY RESIN INJECTION

	PROJEC	T NO. <u>DAV</u> NO.:_	I 	<u>-5765</u> cc 61	<u>)</u> UNTY	
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
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REPAIR QUAN	ΓIΤ	Υ ΤΑ	BLE	
DEDATOS DENT 2		QUANT	ITIES	
REFAIRS DENI Z	EST:	ΙΜΑΤΕ	AC	TUAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP (VERTICAL FACE)	1.0	0.5		
CAP (HORIZONTAL, CORNER)	21.0	5.5		
COLUMN (HORIZONTAL FACE)		12.0		
EPOXY RESIN INJECTI	ON	LN. FT	l	_N. - T
CAP		0		
COLUMN	0			
EPOXY COATING		SQ. FT		50. T
TOP OF CAP		92.0		

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE.IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE. CONTRACTOR SHALL SAW CUT TO A MINIMUM DEPTH OF  $^{\prime}\!/_{2}{}^{\prime\prime}$  BUT REINFORCING STEEL SHALL NOT BE DAMAGED. FOR REPAIR DETAILS, SEE <code>``TYPICAL CAP AND COLUMN REPAIR DETAILS''</code> SHEET. FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS. FOR EPOXY COATING, SEE SPECIAL PROVISIONS. FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

- SHOTCRETE REPAIRS ERI - EPOXY RESIN INJECTION

	PROJEC	T NO. DAV: NO.: _	 I E	<u>I-57</u> 61	65 cc	<u>)</u> DUNTY
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
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REPAIR QL	JANTI	Т	Y T A	ABLE	
END DENT 1		QUANT	ITIES		
END BENT I	ESTI	MA	TE	ACT	UAL
SHOTCRETE REPAIRS	AREA SF	V	OLUME CF	AREA SF	VOLUME CF
CAP (VERTICAL FACE)	0.0		0.0		
CAP (HORIZONTAL, CORNER)	0.0		0.0		
EPOXY RESIN INJECTION	EPOXY RESIN INJECTION				LN. FT
CAP			3.0		
EPOXY COATING	AREA SF				
САР	58.5				
END BENT 2	QUANTITIES				
	ESTI	MATE ACT			UAL
SHOTCRETE REPAIRS	AREA SF	V	OLUME CF	AREA SF	
CAP (VERTICAL FACE)	0.0		0.0		
CAP (HORIZONTAL, CORNER)	0.0		0.0		
EPOXY RESIN INJECTION			LN. FT		LN. FT
САР			12.0		
EPOXY COATING	AREA SF				
САР	58.5				

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.

contractor shall saw cut to a minimum depth of  ${\rm V_2}^{\prime\prime}$  but reinforcing steel shall not be damaged.

CONTRACTOR SHALL REMOVE SURFACE CONCRETE TO VERIFY THAT SAWCUT DEPTH WILL NOT DAMAGE EXISTING REINFORCING STEEL.

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

FOR REPAIR DETAILS, SEE TYPICAL CAP AND COLUMN REPAIR DETAILS SHEET.

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REPAIR QUAN	ΓIΤ	Υ ΤΑ	BLE				
DEDATOS DENIT 1		QUANTITIES					
REFAIRS DENT I	EST:	ΙΜΑΤΕ	AC	TUAL			
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF			
CAP (VERTICAL FACE)	24.0	6.0					
CAP (HORIZONTAL, CORNER)	0	0					
COLUMN (HORIZONTAL FACE)	0	0					
EPOXY RESIN INJECTI	ON	LN. FT	LN. FT				
CAP		2.0					
COLUMN		0					
EPOXY COATING	SQ. FT	SQ. FT					
TOP OF CAP		92.0					

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE.IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE. CONTRACTOR SHALL SAW CUT TO A MINIMUM DEPTH OF  $^{\prime}\!/_{2}{}^{\prime\prime}$  BUT REINFORCING STEEL SHALL NOT BE DAMAGED. FOR REPAIR DETAILS, SEE <code>``TYPICAL CAP AND COLUMN REPAIR DETAILS''</code> SHEET. FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS. FOR EPOXY COATING, SEE SPECIAL PROVISIONS. FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

- SHOTCRETE REPAIRS

ERI - EPOXY RESIN INJECTION

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REPAIR QUAN	ΓIΤ	Υ ΤΑ	BLE				
DEDATOS DENT 2		QUANTITIES					
REFAIRS DENI Z	EST:	ΙΜΑΤΕ	AC	TUAL			
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF			
CAP (VERTICAL FACE)	6.0	1.5					
CAP (HORIZONTAL, CORNER)	18.0	4.5					
COLUMN (HORIZONTAL FACE)	0	0					
EPOXY RESIN INJECTI	ЛС	LN. FT	LN. FT				
CAP		5.0					
COLUMN		0					
EPOXY COATING	SQ. FT	SQ. FT					
TOP OF CAP		92.0					

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE.IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE. CONTRACTOR SHALL SAW CUT TO A MINIMUM DEPTH OF  $^{\prime}\!/_{2}{}^{\prime\prime}$  But reinforcing steel shall not be damaged. FOR REPAIR DETAILS, SEE <code>``TYPICAL CAP AND COLUMN REPAIR DETAILS''</code> SHEET. FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS. FOR EPOXY COATING, SEE SPECIAL PROVISIONS. FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

- SHOTCRETE REPAIRS ERI - EPOXY RESIN INJECTION

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	TOTAL BILL OF MATERIAL																		
BR	RIDGE NO.	GROOVING BRIDGE FLOORS	POLLUTION CONTROL	*CLASS II SURFACE PREPARATION	*CLASS III SURFACE PREPARATION	** LATEX MODIFIED CONCRETE OVERLAY- VES	PLACING & FINISHING OF LATEX MODIFIED CONCRETE OVERLAY-VES	SHOTCRETE REPAIR	CLEANING AND PAINTING EXISTING BEARING PLATES W/HRCSA	EPOXY RESIN INJECTION	FOAM JOINT SEALS	PAINTING CONTAINMENT FOR BRIDGE #	* VOLUMETRIC MIXER	* CONCRETE FOR DECK REPAIR	BRIDGE JOINT DEMOLITION	EPOXY COATING	SCARIFYING BRIDGE DECK	HYDRO- DEMOLITION OF BRIDGE DECK	EL (
		SQ.FT.	LUMP SUM	SQ.YDS.	SQ.YDS.	C.Y.	SQ.YDS.	CU.FT.	EA.	LN.FT.	LUMP SUM	LUMP SUM	LUMP SUM	CU.FT.	SQ.FT.	SQ.FT.	SQ.YDS.	SQ.YDS.	
	74	5426	LUMP SUM	3.0	3.0	31.0	653	4.0	36	77.0	LUMP SUM	LUMP SUM	LUMP SUM	1.0	83	337	653	653	
	75	5426	LUMP SUM	3.0	3.0	31.0	653	9.0	36	6.0	LUMP SUM	LUMP SUM	LUMP SUM	1.0	83	337	653	653	
Т	OTAL	10,852	LUMP SUM	6.0	6.0	62.0	1,306	13.0	72	83.0	LUMP SUM	LUMP SUM	LUMP SUM	2.0	166	674	1,306	1,306	

\* CLASS II AND CLASS III SURFACE PREPARATION, VOLUMETRIC MIXER AND CONCRETE FOR DECK REPAIR ARE NOT ANTICIPATED. TOKEN PAY ITEMS ARE INDICATED FOR PRICING PURPOSES, IN CASE UNANTICIPATED CLASS III SURFACE PREPARATION AREAS ARE ENCOUNTERED.

\*\* THE OUANTITY OF LATEX MODIFIED CONCRETE OVERLAY - VES INCLUDES THE 4"OVERLAP BETWEEN OVERLAYS.

DRAWN BY :J.D. HAWK	DATE :	<u>11/15</u>
CHECKED BY :W. SMITH	DATE :	11/16

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DN SKETCH ION, ONLY. IER SOURCES,	FOR CONTROL OF TRAFFIC AND LIMITS ON PHASING OF CONSTRUCTION, SEE NCDOT STANDARD DRAWINGS 1101.02 SHEETS 3, 4, 8, 9 AND 10.					
AREA, AND Sary to	FOR BRIDGE JOINT DEMOLITION, SEE SPECIAL PROVISIONS.					
TTON ADE EDOM	FOR EPOXY MORTAR REPAIRS, SEE SPECIAL PROVISIONS.					
RACTOR SHALL	FOR LATEX MODIFIED CONCRETE OVERLAY - VERY EARLY STRENGTH, SEE SPECIAL PROVISIONS.					
MENSIONS AND	FOR FOAM JOINT SEALS, SEE SPECIAL PROVISIONS.					
Y TO FOLLOW	FOR ELASTOMERIC CONCRETE, SEE PROVISIONS.					
ND DISPOSE OF	FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.					
SPECIAL	FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.					
L BE SEALED	FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.					
LUN OF BRIDGE	FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.					
RIATE	FOR VOLUMETRIC MIXER, SEE SPECIAL PROVISIONS.					
ACTIVE TRAVEL	FOR CONCRETE FOR DECK REPAIRS, SEE SPECIAL PROVISIONS.					
D OF HANDLING	FOR CLEANING AND PAINTING OF EXISTING BEARING PLATES WITH HRCSA,SEE SPECIAL PROVISIONS.					
OVERLAYS E OR EDGE OF						
OLITION OF TION, AND (ERLAY NS.						

_ASTOMERIC CONCRETE	EPOXY MORTAR REPAIRS	F	PRO.IFC	Τ ΝΟ	I	-5765	1				
CU.FT.	SQ.FT.		NOULC								
21.0	3.4	-	DAVIECOUNTY								
21.0	2.6	E	BRIDGE NO. <u>74 &amp; 75</u>								
42.0	6.0	s	SHEET 1 O	F 2							
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DATE

RESIDENT ENGINEER









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TEX MODIFIED CONCRETE Y-VES								
.DECK RO DEMOLITION	PROJEC	DAY DAY NO	] VIE 74	<u>-5765</u> cc <u>&amp;</u> 75	5 DUNTY			
DocuSigned by: AD500F1D97/49462L- 18565 M. SHUTTING 12/9/2016	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE SURFACE PREPARATION							
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	SPAN A	and A	PPR. S	SLAB QI	JANTIT	IES			
		- 105	ES	TIMATE	AC	TUAL			
	PREPARATION	ACE	1.0	SQ. YDS.					
	CLASS III SUP PREPARATION	RFACE	1.0	SQ. YDS.					
$\setminus$	BRIDGE JOINT DEMOLITION		21.	0 SQ.FT.					
	SCARIFYING BRIDGE DECK		177.0	SQ.YDS.					
	HYDRO-DEMOLI OF BRIDGE DEC	FION CK	177.0	SQ.YDS.					
	SCARIFYING APPROACH SLAE	3	63.0	63.0 SQ. YDS.					
	HYDRO-DEMOLI OF APPROACH S	TION SLAB	63.0	SQ. YDS.					
A1'-5" (JO	PAYMENT FO PREPARATIO ADDITIONAL HYDRO-DEMOU "OVERLAY SL CLASS II O ANTICIPATE PRICING PUI OR III SUR	R CLASS N IS BA DEMOLI ITION IRFACE F R III S D.A TOK RPOSES, FACE PR	II & C SED ON TION RE OF THE E PREPARAT URFACE F EN AMOU IN CASE EPARATIO	LASS III THE SOUAR QUIRED FO BRIDGE DEC ION″SPECI PREPARATI NT IS INE UNANTICI DN AREAS	SURFACE E FEET OF LLOWING CK. SEE CAL PROVI ON IS NO DICATED F PATED CL ARE ENCO	SIONS. T OR ASS II JNTERED.			
		- E	RRIDGE JU CLASS II PREPARAT GCARIFIC HYDRO-DE	DINT DEMC & III SU ION ATION & MOLITION	URF ACE				
ION ILAR CK SP	AN B PF	ROJEC	T NO. DAVI	I IE	<u>-5765</u> co 74	5			
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REPAIR QL	JANTI	Т	Y T A	ABLE			
	QUANTITIES						
END BENT I	ESTI	MA	TE	ACTUAL			
SHOTCRETE REPAIRS	AREA SF	VOLUME CF		AREA SF	VOLUME CF		
CAP (VERTICAL FACE)	0.0		0.0				
CAP (HORIZONTAL, CORNER)	0.0		0.0				
EPOXY RESIN INJECTION			LN. FT		LN. FT		
CAP			74.0				
EPOXY COATING	AREA SF						
САР	62.5						
END BENT 2	QUANTITIES						
	ESTIMATE			ACTUAL			
SHOTCRETE REPAIRS	AREA SF	V	OLUME CF	AREA SF			
CAP (VERTICAL FACE)	0.0		0.0				
CAP (HORIZONTAL, CORNER)	0.0		0.0				
EPOXY RESIN INJECTION			LN. FT		LN. FT		
САР			3.0				
EPOXY COATING	AREA SF						
САР	62.5						

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1"BEHIND REBAR AND MIN. 2"CL TO SAWCUT. SEE REPAIR DETAILS.

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE. CONTRACTOR SHALL SAW CUT TO A MINIMUM DEPTH OF  $\frac{1}{2}$ " BUT REINFORCING STEEL SHALL NOT BE DAMAGED. CONTRACTOR SHALL REMOVE SURFACE CONCRETE TO VERIFY THAT SAWCUT DEPTH WILL NOT DAMAGE EXISTING REINFORCING STEEL. FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS. FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

FOR REPAIR DETAILS, SEE TYPICAL CAP AND COLUMN REPAIR DETAILS SHEET.

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REPAIR QUAN	ΓΙΤ`	Υ ΤΑ	BLE	
DEDATOS DENIT 1	QUANTITIES			
REFAIRS DENT I	EST:	ΕΜΑΤΕ	AC	TUAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP (VERTICAL FACE)	0	0		
CAP (HORIZONTAL, CORNER)	0	0		
COLUMN (HORIZONTAL FACE)	0	0		
EPOXY RESIN INJECTI	ЛС	LN. FT	L	_N. - T
CAP		0		
COLUMN	0			
EPOXY COATING		SQ. FT		50. - T
TOP OF CAP		106.0		

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE.IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.

CONTRACTOR SHALL SAW CUT TO A MINIMUM DEPTH OF  $^{\prime}\!/_{2}{}^{\prime\prime}$  BUT REINFORCING STEEL SHALL NOT BE DAMAGED.

FOR REPAIR DETAILS, SEE <code>``TYPICAL CAP AND COLUMN REPAIR DETAILS''</code> SHEET.

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

- SHOTCRETE REPAIRS ERI - EPOXY RESIN INJECTION

NO REPAIRS NOTED DURING INSPECTION BY STRUCTURES MANAGEMENT UNIT. THE CONTRACTOR AND ENGINEER SHALL INSPECT THE BENT PRIOR TO BEGINNING WORK.

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REPAIR QUANTITY TABLE								
DEDATOS DENT 2		QUANTITIES						
REFAIRS DENI Z	EST:	ΙΜΑΤΕ	AC	TUAL				
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF				
CAP (VERTICAL FACE)	5.0	1.25						
CAP (HORIZONTAL, CORNER)	11.0	2.75						
COLUMN (HORIZONTAL FACE)	0	0						
EPOXY RESIN INJECTI	ON	LN. FT	L	_N. - T				
CAP		0						
COLUMN		0						
EPOXY COATING		SQ. FT	0,1	50. T				
TOP OF CAP		106.0						

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE.IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE. CONTRACTOR SHALL SAW CUT TO A MINIMUM DEPTH OF  $^{\prime}\!/_{2}{}^{\prime\prime}$  BUT REINFORCING STEEL SHALL NOT BE DAMAGED. FOR REPAIR DETAILS, SEE <code>``TYPICAL CAP AND COLUMN REPAIR DETAILS''</code> SHEET. FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS. FOR EPOXY COATING, SEE SPECIAL PROVISIONS. FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

- SHOTCRETE REPAIRS ERI - EPOXY RESIN INJECTION

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	PREPARATIC		1.0	D SQ.YDS.		
	CLASS III PREPARATIC	SURFACE )N	1.0	) SQ.YDS.		
$\setminus$	BRIDGE JOI DEMOLITION	NT I	21	.0 SQ.FT.		
	SCARIFYING BRIDGE DEC	; К	177.0	D SQ. YDS.		
	HYDRO-DEMO OF BRIDGE	DECK	177.0	D SQ.YDS.		
	SCARIFYING APPROACH S	LAB	63.0	D SQ. YDS.		
	HYDRO-DEMO OF APPROAC	LITION CH SLAB	63.0	) SQ.YDS.		
	PAYMENT PREPARA ADDITIO HYDRO-DE "OVERLAY	FOR CLAS TION IS B NAL DEMOL MOLITION SURFACE	S II & ASED ON ITION RE OF THE PREPARA	CLASS III THE SQUAR QUIRED FC BRIDGE DE TION" SPEC	SURFACE E FEET O DLLOWING CK. SEE IAL PROVI	F ISIONS.
41'-5" (JOINT	CLASS II ANTICIP PRICING OR III	I OR III ATED.A TO PURPOSES SURFACE P	SURFACE KEN AMOU ,IN CASE REPARATI	PREPARATI JNT IS INU UNANTICI ON AREAS	ON IS NO DICATED F PATED CL ARE ENCO	)T FOR ASS II DUNTERED.
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REPAIR QUANTITY TABLE						
			QUANT	ITIES		
END BENT I	ESTI	MA	TE	ACT	UAL	
SHOTCRETE REPAIRS	AREA SF	V	OLUME CF	AREA SF	VOLUME CF	
CAP (VERTICAL FACE)	0.0		0.0			
CAP (HORIZONTAL, CORNER)	0.0		0.0			
EPOXY RESIN INJECTION			LN. FT		LN. FT	
CAP			4.0			
EPOXY COATING	AREA SF	AREA				
CAP	62.5					
END BENT 2	QUANTITIES					
	ESTI	MA	TE	ACTUAL		
SHOTCRETE REPAIRS	AREA SF	V	OLUME CF	AREA SF		
CAP (VERTICAL FACE)	7.0		2.0			
CAP (HORIZONTAL, CORNER)	0.0		0.0			
EPOXY RESIN INJECTION	l		LN. FT		LN. FT	
САР	_		0.0			
EPOXY COATING	AREA SF					
CAP	62.5					

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1"BEHIND REBAR AND MIN.2"CL TO SAWCUT.SEE REPAIR DETAILS.

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE.

Contractor shall saw cut to a minimum depth of  $\prime\!/_2{}''$  but reinforcing steel shall not be damaged.

CONTRACTOR SHALL REMOVE SURFACE CONCRETE TO VERIFY THAT SAWCUT DEPTH WILL NOT DAMAGE EXISTING REINFORCING STEEL.

FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS.

FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

FOR REPAIR DETAILS, SEE TYPICAL CAP AND COLUMN REPAIR DETAILS SHEET.

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REPAIR QUAN	ΓΙΤ`	Y TA	BLE		
		QUANTITIES			
REFAIRS DENT I	EST:	ΕΜΑΤΕ	AC	TUAL	
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF	
CAP (VERTICAL FACE)	0	0			
CAP (HORIZONTAL, CORNER)	0	0			
COLUMN (HORIZONTAL FACE)	1.0	0.5			
EPOXY RESIN INJECTI	ON	LN. FT	L	_N. - T	
CAP		0			
COLUMN		0			
EPOXY COATING		SQ. FT		50. T	
TOP OF CAP		106.0			

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE.IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE. CONTRACTOR SHALL SAW CUT TO A MINIMUM DEPTH OF  $^{\prime}\!/_{2}{}^{\prime\prime}$  But reinforcing steel shall not be damaged. FOR REPAIR DETAILS, SEE <code>``TYPICAL CAP AND COLUMN REPAIR DETAILS''</code> SHEET. FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS. FOR EPOXY COATING, SEE SPECIAL PROVISIONS. FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

- SHOTCRETE REPAIRS ERI - EPOXY RESIN INJECTION

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REPAIR QUANTITY TABLE								
DEDATOS DENT 2								
REFAIRS DENI Z	EST:	ΙΜΑΤΕ	AC	TUAL				
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF				
CAP (VERTICAL FACE)	3.0	1.0						
CAP (HORIZONTAL, CORNER)	19.0	5.5						
COLUMN (HORIZONTAL FACE)	0	0						
EPOXY RESIN INJECTI	ON	LN. FT	LN. FT					
CAP		2.0						
COLUMN		0						
EPOXY COATING		SQ. FT		50. T				
TOP OF CAP		106.0						

## NOTES:

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE.IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE INSPECTOR OR ENGINEER THE CONTRACTOR SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ADJUST THE ACTUAL QUANTITIES ENTERED INTO THE REPAIR QUANTITY TABLE. CONTRACTOR SHALL SAW CUT TO A MINIMUM DEPTH OF  $^{\prime}\!/_{2}{}^{\prime\prime}$  BUT REINFORCING STEEL SHALL NOT BE DAMAGED. FOR REPAIR DETAILS, SEE ``TYPICAL CAP AND COLUMN REPAIR DETAILS' SHEET. FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS. FOR EPOXY COATING, SEE SPECIAL PROVISIONS. FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

- SHOTCRETE REPAIRS

ERI - EPOXY RESIN INJECTION

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#### NOTES:

CONTRACTOR SHALL FIELD VERIFY THE EXISTING FORMED OPENING PRIOR TO OBTAINING JOINT MATERIAL.

IF THE EMBEDDED PORTION OF THE EXISTING WATERSTOP IS EXPOSED DURING REMOVAL OF UNSOUND CONCRETE OR IF UNSOUND CONCRETE IS REMOVED TO WITHIN 2"OF THE WATERSTOP, THE ENTIRE WATERSTOP SHALL BE REMOVED.

HYDRO-DEMOLITION OR EXCAVATION OF CONCRETE AT THE EXISTING JOINT SHALL RESULT IN THE BOTTOM OF THE EXCAVATION BEING REASONABLY FLAT AND LEVEL, TO PROVIDE SUFFICIENT SUBSTRATE FOR PLACEMENT AND SUPPORT OF ELASTOMERIC CONCRETE.

FOR FOAM JOINT SEALS, SEE SPECIAL PROVISIONS.

FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.

RETAIN ALL EXISTING REINFORCING STEEL.CLEAN AND REPAIR AS NEEDED.

THE WIDTH OF THE UNCOMPRESSED FOAM JOINT MATERIAL SHALL BE 2".





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# NOTE

CONTRACTOR SHALL SAW CUT TO A MINIMUM DEPTH OF  $^{\prime\prime}\!\!/_{2}{}^{\prime\prime}$  But reinforcing steel shall not be damaged. CONTRACTOR SHALL REMOVE SURFACE CONCRETE TO VERIFY THAT SAWCUT DEPTH WILL NOT DAMAGE EXISTING REINFORCING STEEL. CONTRACTOR SHALL SAW CUT THE REPAIR AREAS SO THAT THE CORNERS ARE SQUARE AS INDICATED ON THE DETAILS. CONCRETE REPAIRS MAY BE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER. FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIONS. FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

\* IF CONFINEMENT STEEL IS NOT PRESENT, THEN REPAIR LENGTH SHALL NOT EXCEED 10 FEET.

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## DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
GRADE 60 CONCRETE IN COMPRESSION	24,000 LBS.PER SO.IN. 1,200 LBS.PER SO.IN.
CONCRETE IN COMPRESSION -	24,000 LBS.PER SO.IN. 1,200 LBS.PER SO.IN. SEE A.A.S.H.T.O.
GRADE GRADE GO - -   CONCRETE IN COMPRESSION - <	24,000 LBS.PER SO.IN. 1,200 LBS.PER SO.IN. SEE A.A.S.H.T.O.
GRADE GONCRETE IN COMPRESSION - <td>24,000 LBS. PER SO. IN. 1,200 LBS. PER SO. IN. SEE A.A.S.H.T.O. 1,800 LBS. PER SO. IN.</td>	24,000 LBS. PER SO. IN. 1,200 LBS. PER SO. IN. SEE A.A.S.H.T.O. 1,800 LBS. PER SO. IN.
GRADE 60 -<	24,000 LBS. PER SO. IN. 1,200 LBS. PER SO. IN. SEE A.A.S.H.T.O. 1,800 LBS. PER SO. IN. 375 LBS. PER SO. IN.

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

#### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUIMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REOUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

#### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12"INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

## STANDARD NOTES

#### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER. DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

#### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT

IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OF ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

#### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8"Ø SHEAR STUDS FOR THE %4"Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0". EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SUFFACES WHICH BEAR ON OTHER SURFACES,ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

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#### SPECIAL NOTES:

SPECIFICATIONS ARTICLE 105-4.

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#### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING, CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL BE OCCEPTED CEPTITEED MILL DEPORTS APE PEOLUPED FOR METAL PAILS AND POSTS NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE

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