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88 80

GN DISCHARGE JENCY OF DESIGN FLOOD GN HIGH WATER ELEVATION DISCHARGE (Q100) HIGH WATER ELEVATION	1500 CFS 50 YR. 613.7 FT. 1700 CFS 614.50 FT.
OVERTOPPING FLOO TOPPING DISCHARGE JENCY OF OVERTOPPING FLOOD TOPPING FLOOD ELEVATION SHOULDER POINT @ 124+19 -L-	<u>)D DATA</u> 2700+ CFS 500+ YR. * 627.08 FT.
AGE I STRUCTURE QU	JANTITIES
SS A CONCRETE RREL @3.35CY/FT LS ET WINGS TOTAL	<u>234.5</u> C.Y. <u>0.7</u> C.Y. <u>13.9</u> C.Y. <u>249.1</u> C.Y.
NFORCING STEEL REL ET WINGS TOTAL	<u>41979</u> LBS. <u>734</u> LBS. <u>42713</u> LBS.
VERT EXCAVATION	LUMP SUM
STAGE II & I. STRUCTURE QUANT	LI ITIES
SS A CONCRETE REL @3.35_CY/FT LS ET WINGS TOTAL	<u>280.6</u> C.Y. <u>0.7</u> C.Y. <u>13.9</u> C.Y. <u>295.2</u> C.Y.
NFORCING STEEL REL ET WINGS	<u>51206</u> LBS. <u>734</u> LBS.
	E1040 LBS
INDATION CONDITIONING MATERIAL	51940 LBS. 196 TONS LUMP SUM
INDATION CONDITIONING MATERIAL	51940 LBS. 196 TONS LUMP SUM
INDATION CONDITIONING MATERIAL VERT EXCAVATION OTAL STRUCTURE QUA SS A CONCRETE RREL @ 3.35 CY/FT LS ET WINGS	51940 LBS. 196 TONS LUMP SUM NTITIES 515.1 C.Y. 1.4 C.Y. 27.8 C.Y.

1468 LBS.

94653 LBS.

- EL.607.0±

EL.606.7±

- EL.608.4±

EL.607.3±

EL.607.2± -

359 TONS

LUMP SUM

EL.611.7±

NOTES:

HEADWALLS. STAGE II 1. WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF BARREL 3 VERTICAL WALLS AND CURTAIN WALLS TO CONSTRUCTION JOINTS. STAGE III 1. WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF BARRELS 1 & 2 VERTICAL WALLS AND CURTAIN WALLS TO CONSTRUCTION JOINTS. 3. ROOF SLAB FOR ALL BARRELS AND HEAD WALLS. 4. CONSTRUCTION OF SILLS IN BARREL 1. U-4910A PROJECT NO. CABARRUS COUNTY 124+11.00 -L-STATION: STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION - EL. 612.6± RALEIGH SR 1445 (DERITA ROAD) OVER UT TO ROCKY RIVER TRIPLE 10 FT.X 8 FT. CONCRETE BOX CULVERT 90° SKEW

STAGE I 1. WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS. 2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND 2. THE REMAINING PORTION OF BARREL 3 WALLS AND WINGS FULL HEIGHT. 2. THE REMAINING PORTIONS OF BARRELS 2&3 WALLS AND WING FULL HEIGHT.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN EL.612.7±

ASSUMED LIVE LOAD ----- HL-93 OR ALTERNATE LOADING. DESTGN ETLL----- 12.2 ET. THE EXISTING STRUCTURE CONSISTING OF 3 - 73" x 55" CSPA LOCATED AT THE PROPOSED STRUCTURE LOCATION SHALL BE REMOVED. THE EXISTING STRUCTURE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SEE SHEET NO. TMP-02B FOR CULVERT CONSTRUCTION SEQUENCE. 3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS. CONCRETE IN CULVERTS TO BE POURED IN THE FOLLOWING ORDER: THAT TT WILL PROPERLY TAKE CARE OF THE FILL. DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET. STEEL IN THE BOTTOM SLAB MAY BE SPLICED AT THE PERMITTED CONSTRUCTION JOINT AT THE CONTRACTOR'S OPTION. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY CONTRACTOR. TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED IN THE BARREL, SPACED TO LIMIT THE POURS TO A MAXIMUM OF 70 FEET. LOCATION OF JOINTS SHALL BE SUBJECT TO APPROVAL OF THE ENGINEER. AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WALL AND BOTH FACES OF INTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR. AT THE CONTRACTOR'S OPTION HE MAY SUBMIT TO THE ENGINEER FOR APPROVAL, DESIGN AND DETAIL DRAWINGS FOR A PRECAST REINFORCED CONCRETE BOX CULVERT IN LIEU OF THE CAST-IN-PLACE DESIGN. FOR OPTIONAL PRECAST REINFORCED BOX CULVERT, SEE SPECIAL PROVISIONS. THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED. AND FOR PROJECT REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF TEH SIZE AND LENGTH OF THE SAMPLE PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS. EXCAVATE 1.0 FEET BELOW THE BARREL AND FOOTINGS AND REPLACE WITH FOUNDATION CONDITIONING MATERIAL. CONSTRUCT THE REINFORCED BOX CULVERT AT STA. 124+11 WITH 3" OF CAMBER TO ACCOUNT FOR ANTICIPATED SETTLEMENT. BACKFILL WITH SELECT MATERIAL, CLASS VI MEETING THE REQUIREMENTS OF SECTION 1016 OF THE STANDARD SPECIFICATIONS. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS. FOR CULVERT DIVERSION DETAILS, SEE EROSION CONTROL PLANS. FOR MAINTENANCE OF TRAFFIC. SEE TRAFFIC CONTROL PLANS. FOR LIMITS OF TEMPORARY SHORING, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING, SEE ROADWAY PLANS. FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN. - EL. 611.8±

OCUMENT NOT CO FINAL UNLESS SIGNATURES COMP

F.A. PROJECT NO. STPDA-1445(008)

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	C-01
	•	•				TOTAL SHEETS
						7

	AECOM TECHNICAL SERVICES, INC. 701 CORPORATE CENTER DRIVE, SUITE 475 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342
NSIDERED S ALL MPI FTFD	11/30/2016 TH CAROL NORTH CAROL NORTH CAROL SEAL 030474 John C: MCINEER. John C: MCINEER. A2FDE142C82F4AB

	LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS															
										STRENG	TH I LIMI	T STATE				
									MON	MENT			SH	EAR		R
	LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W x RF	LIVE-LOAD FACTORS (_{YLL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBE
DESIGN		HL-93 (INVENTORY)	N/A	2	1.16		1.75	2.31	2	Top Slab	5.00	1.16	2	Top Slab	0.83	
		HL-93 (OPERATING)	N/A		1.50		1.35	2.95	2	Top Slab	5.00	1.50	2	Top Slab	0.83	
		HS-20 (INVENTORY)	36.000	1	1.16	41.76	1.75	2.17	2	Top Slab	5.00	1.16	2	Top Slab	0.83	
		HS-20 (OPERATING)	36.000		1.50	54.00	1.35	2.81	2	Top Slab	5.00	1.50	2	Top Slab	0.83	
		SNSH	13.500		1.95	26.33	1.40	3.07	2	Top Slab	5.00	1.95	2	Top Slab	0.83	
	(SV	SNGARBS2	20.000		3.11	62.20	1.40	3.07	2	Top Slab	5.00	1.70	2	Top Slab	0.83	
	Ш	SNAGRIS2	22.000		1.74	38.28	1.40	3.12	2	Top Slab	5.00	1.74	2	Top Slab	0.83	
		SNCOTTS3	27.250	3	1.40	38.15	1.40	2.83	2	Top Slab	5.00	1.40	2	Top Slab	0.83	
		SNAGGRS4	34.925		1.66	57.98	1.40	2.98	2	Top Slab	5.00	1.66	2	Top Slab	0.83	
	GLE	SNS5A	35.550		1.49	52.97	1.40	2.97	2	Top Slab	5.00	1.49	2	Top Slab	0.83	
	SIN	SNS6A	39.950		1.48	59.13	1.40	2.95	2	Top Slab	5.00	1.48	2	Top Slab	0.83	
		SNS7B	42.000		2.11	88.62	1.40	2.99	2	Top Slab	5.00	1.48	2	Top Slab	0.83	
	<u> </u>	TNAGRIT3	33.000		1.64	54.12	1.40	3.13	2	Top Slab	5.00	1.64	2	Top Slab	0.83	
) EV	TNT4A	33.075		1.64	54.24	1.40	2.98	2	Top Slab	5.00	1.64	2	Top Slab	0.83	
	JR S TST	TNT6A	41.600		1.57	65.31	1.40	2.98	2	Top Slab	5.00	1.57	2	Top Slab	0.83	
	CTC R (T	TNT7A	42.000		1.60	67.20	1.40	2.95	2	Top Slab	5.00	1.60	2	Top Slab	0.83	
	-RA	TNT7B	42.000		1.92	80.64	1.40	2.92	2	Top Slab	5.00	1.92	2	Top Slab	0.83	
	TRA TRA	TNAGRIT4	43.000		1.76	75.68	1.40	3.09	2	Top Slab	5.00	1.76	2	Top Slab	0.83	
		TNAGT5A	45.000		1.80	81.00	1.40	2.88	2	Top Slab	5.00	1.80	2	Top Slab	0.83	
	⊢	TNAGT5B	45.000		1.59	71.55	1.40	2.90	2	Top Slab	5.00	1.59	2	Top Slab	0.83	



DATE: 11/30/2016 TIME: 9:43:09 AM

ASSEMBLED BY : NKB CHECKED BY : JCM		DATE : DATE :	07/16 07/16
DRAWN BY : WMC CHECKED BY : GM	7/ 7/	REV. 10/1/11	MAA/GN

LOAD FACTORS:

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	
WA	1.00	

DESIGN LOAD RATING FACTORS

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

COMMENTS:

1.	
2.	

- 3.
- 4.

(#) CONTROLLING LOAD RATIN
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
<pre>3 LEGAL LOAD RATING **</pre>
** SEE CHART FOR VEHICLE TYPE





NOTE: STAGE II WILL NOT INCLUDE ROOF SLAB.

-3#8S2 @ 5″ CTS. (Bottom of Roof SLAB)

— SYMM. ABOUT 🖳 CULVERT

— W1, TYP.

- 3#8S4 @ 5″ CTS. (TOP OF FLOOR SLAB)

SIAGE IL		PROJEC	CT NO. CABAR N:	l RUS 124+1	J-49104 CO .1.00 -l	4 UNTY
6 @ 5″ (TOP OF	AECOM TECHNICAL SERVICES, INC. 701 CORPORATE CENTER DRIVE, SUITE 475 RALEIGH, NC 27607 (919) 854-6200 AECOM License No. F-0342 11/30/2016 11/30/2016	DEPA [TR] CON	RTMENT O BARREL PLE 10 ICRETE 90 9	OF NORTH CA F TRAN RALEIGH STA D FT. BOX P SKI	NSPORTAT	ΓΙΟΝ Ο FT. ERT
R SLAB) ONSIDERED	SEAL 030474 Docusigned/py: C. MORRIS John C. MORRIS A2FDE142C82F4AB	NO. BY: 1	REVISIO DATE: NO 3	NS . BY:	DATE:	SHEET NO. C-03 TOTAL SHEETS 7

STD.NO.CB13







BARREL 3	BARREL 2	
	STAGE I (LOOKING UPSTREAM)	

ASSEMBLED BY :NKB CHECKED BY :JCM	_ DATE : <u>07/16</u> _ DATE : <u>07/16</u>	SPECIAL
DRAWN BY :JOEL JOHNSON CHECKED BY :GARY BROOME	_ DATE : <u>MAR.1971</u> _ DATE : <u>MAR.1971</u>	STANDARD





STAGE II & III

U-4910A PROJECT NO. ____ CABARRUS COUNTY 124+11.00 -L-STATION: STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION AECOM TECHNICAL SERVICES, INC. RALEIGH AECOM TECHNICAL SERVICES, INC. 701 CORPORATE CENTER DRIVE, SUITE 475 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342 BARREL STANDARD TRIPLE 10 FT. X 8 FT. 11/30/2016 CONCRETE BOX CULVERT ATH CARO FESSION 90° SKEW SEAL 030474 SHEET NO. REVISIONS C-04 NO. BY: DATE: DATE: BY: John C. Mg TOTAL SHEETS A2FDE142C82F4AB..



(LOOKING UPSTREAM) DENOTES STAGE II NOTE: FOR ADDITIONAL STAGING INFORMATION, SEE TRAFFIC CONTROL PLANS SHEET TMP-02B. DOCUMENT NOT CONSIDERED FINAL UNLESS ALL

CONSTRUCTION SEQUENCE

STD.NO.CB13



2016 AM 11/30/2 9:44:41 1 M M M

NG STEEL SCHEDULE STAGE I						
E	TYPE	LENGTH	WEIGHT			
	(1)	6'-1"	1130			
	(1)	7'-1"	2958			
	1	5'-4"	990			
	STR	32′-8″	4736			
	STR	32′-8″	4736			
	STR	32′-8″	6820			
	STR	32′-8″	6820			
	STR	9'-9"	912			
	STR	7'-2″	1331			
	STR	9′-9″	1811			
	STR	24'-3"	8456			
	STR	2'-7"	16			
	STR	32'-8"	136			
	STR	32′-8″	604			
	STR	32′-8″	523			
ING STEEL 41979						

SPLICE	LENGTH CHART				
BAR SIZE	SPLICE LENGTH				
#4	1'-9″				
#5	2'-2"				
#6	2'-9"				
#7	3'-9″				
#8	4'-11"				



BAR TYPE BAR DIMENSIONS ARE OUT TO OUT

NOTE: SEE SHEET C-01 FOR COMPLETE SUMMARY OF QUANTITIES

U-4910A PROJECT NO. CABARRUS COUNTY 124+11.00 -L-STATION: STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION SR 1445 (DERITA ROAD) OVER UT TO ROCKY RIVER TRIPLE 10 FT.X 8 FT. CONCRETE BOX CULVERT 90° SKEW

		SHEET NO.				
N0.	BY:	DATE:	NO.	BY:	DATE:	C-05
	•	•				TOTAL SHEETS
						7

θE	II	& III	DOLL
E	TYPE	LENGTH	WEIGHT
	1	6'-1"	1358
	1	7'-1″	3554
		5′-4″	1190
	STR	32′-8″	5690
	STR	9′-7″	1669
	STR	25′-4″	4413
	STR	32'-8″	8194
	STR	9'-7"	2404
	STR	25'-10"	6480
	STR	9′-9″	1094
	STR	7'-2″	1600
	STR	9′-5″	2102
	STR	29'-0"	10112
	STR	2'-7"	16
	STR	32′-8″	136
	STR	32′-8″	294
	STR	25'-4"	203
	STR	25'-10"	233
	STR	28'-0"	224
	STR	9'-7"	86
	STR	9'-7"	154

ING	ST	EEL

	AECOM TECHNICAL SERVICES, INC. 701 CORPORATE CENTER DRIVE, SUITE 475 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342
SIDERED ALL PLETED	11/30/2016 TH CAROL NORTH CAROL SEAL 030474 Docusignet/Apr: John C. MORRISON A2FDE142C82F4AB

51206

11/30/2016 9:44:54 AM

DATE: TIME:



NOTE:

BED MATERIAL PLACED BETWEEN SILLS IN THE CULVERT SHALL PROVIDE A CONTINUOUS LOW FLOW CHANNEL BETWEEN THE LOWER SILLS. THE MATERIAL SHALL BE NATURAL STONE WITH A GRADATION SIZE SIMILAR TO THAT OF CLASS B RIP RAP. STONES LARGER THAN 9 INCHES SHALL NOT BE PLACED WITHIN THE LOW FLOW CHANNEL. BED MATERIAL IS SUBJECT TO APPROVAL BY THE ENGINEER.

		PROJEC	<mark>f no</mark> . Cabar	U RUS	-4910A	
		STATIO	N: <u>1</u>	24+11.0	CC	
AECOM TH 701 CORPORA (919) 854-620 AECO 1	COM CCHNICAL SERVICES, INC. TE CENTER DRIVE, SUITE 475 ALEIGH, NC 27607 WWW.aecom.com M License No. F-0342 1/30/2016 TH CAROL SEAL 030474	DEPART SI T C(STA F MENT R 1445 OVER RIPLE DNCRET 9	TE OF NORTH CA OF TRAN (DERIT ROCKY 10 FT. E BOX	ROLINA ISPORTAT A ROAD RIVER X 8 FT. CULVER W	Г ІО N) Г
Docu\$ignet	C. MORRES.	NO. BY:	REVIS date:	NO. BY:	DATE:	SHEET NO. C-06
ERED John C. 1 A2FDE142C8; ED	1977114700 ·····	•••				total sheets 7



11/30/2016 9:45:06 AM DATE: TIME:

	BILL OF MATERIAL										
STAGE I							ST,	AGE	II a	& III	
2	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	12	#4	STR	10'-10"	87	H1	12	#4	STR	10'-10"	87
	4	#4	STR	7′-8″	21	H2	4	#4	STR	7′-8″	21
	4	#4	STR	4'-1"	11	H3	4	#4	STR	4'-1"	11
	24	#4	1	3'-3"	52	H4	24	#4	1	3'-3"	52
	4	#4	STR	11′-9″	32	H5	4	#4	STR	11'-9″	32
	4	#5	2	10'-2"	43	N1	4	#5	2	10'-2"	43
	6	#5	2	9'-2"	58	N2	6	#5	2	9'-2"	58
	6	#4	2	7'-11″	32	N3	6	#4	2	7'-11″	32
	6	#4	2	6′-7″	27	N4	6	#4	2	6'-7"	27
	6	#4	2	5′-4″	22	N5	6	#4	2	5′-4″	22
	6	#6	STR	6'-0"	54	S1	6	#6	STR	6'-0"	54
	6	#5	STR	12′-9″	80	Τ1	6	#5	STR	12'-9"	80
	4	#4	STR	8'-1"	22	V1	4	#4	STR	8'-1"	22
	6	#4	STR	7'-1"	29	V2	6	#4	STR	7'-1"	29
	6	#4	STR	5'-10"	24	٧3	6	#4	STR	5'-10"	24
	6	#4	STR	4'-7"	19	V4	6	#4	STR	4'-7"	19
	6	#4	STR	3'-4"	14	V5	6	#4	STR	3'-4"	14
	4	#5	3	6'-0"	25	Z1	4	#5	3	6'-0"	25
	6	#5	3	5′-5″	34	Z2	6	#5	3	5′-5″	34
	6	#4	3	4'-7"	19	Z3	6	#4	3	4'-7"	19
	6	#4	3	3'-10"	16	Z4	6	#4	3	3'-10"	16
	6	#4	3	3'-1"	13	Z5	6	#4	3	3'-1"	13
NFORCING STEEL 2 WINGS 734 LBS			REINFORCING STEEL FOR 2 WINGS 734 LBS					34 LBS			
SS 2 1 1	SS A CONCRETE 2 WINGS 10.7 CY 1 HEADWALLS 1.3 CY 1 END CURTAIN WALLS 1.9 CY TOTAL 13.9 CY				0.7 CY 1.3 CY 1.9 CY 3.9 CY	CLASS 2 1 1	A CO WING HEADV END (NCRETE S VALLS CURTAI	N WAL Tot	10 LS AL 13	D.7 CY 1.3 CY 1.9 CY 3.9 CY

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

		PROJEC	CT NO. CABA ON:	 1	ן US 24+1	J-49104 CC 1.00 -	4)UNTY
	AECOM TECHNICAL SERVICES, INC. 701 CORPORATE CENTER DRIVE, SUITE 475 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342 11/30/2016 11/30/2016 SEAL 030474	DEP/ S CONC H = 8'	artment TAND CRETE '-0" 9	OF OF F C O O O O O	NORTH CAR TRAI ALEIGH RD OR BOX SKE	NSPORTA WING CUL SLOPE	TION S VERT = 2:1
DERED L ETED	John C. MORRSON A2FDE 142C82F4AB	№. вү: 1 2	REVIS	510NS №. 3 4	BY:	DATE:	SHEET NO. C-07 TOTAL SHEETS 7

STD.NO.CW9008

DESIGN DATA:

+

+

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SO.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES. ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION. SETTLEMENT. ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS,AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER

SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED.CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

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