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This file or an individual page shall not be considered a certified document.

♠ PROJECT ≤

VICINITY MAP

NOT TO SCALE

OFFSITE DETOUR -

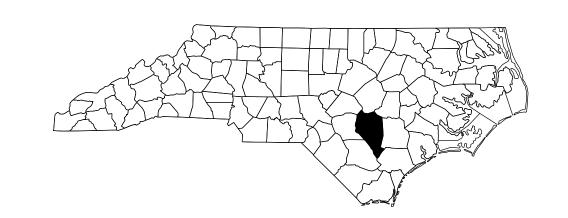
00

SAMPSON COUNTY

LOCATION: BRIDGES NO. 102, 103, AND 104 OVER LITTLE COHARIE CREEK ON SR 1233 (AUTRYVILLE RD.)

TYPE OF WORK: GRADING, PAVING, DRAINAGE, AND STRUCTURES

STATE	STATE	NO.		SHEETS		
N.C.		B-4814				
STATE PRO	J. NO.	P. A. PROJ. NO.		DESCR	LIPTION	
38584.	1.2.	BRZ-1233 (6)		P	PE	
38584	.2.1	BRZ-1233 (6)	R/\	W &	UTII	ITIES
38584	.3.1	BRZ-1233 (6)		CO	NST	•
				<u> </u>		



END TIP PROJECT B-4814 STA. 23 + 00.00 -L-BEGIN TIP PROJECT B-4814 \ STA. 11 + 50.00 -L-BRIDGE #102 END BRIDGE STA. 18 + 12.13 -L-BRIDGE #104 STA. 17 + 29.88 -L-TO SR 1002 TO SALEMBURG -> (DUNN RD) BEGIN BRIDGE LEND BRIDGE STA. 14+02.88 -L-STA. 20 + 73.83 -L-STA. 14 + 75.13 -L-END BRIDGE STA. 21 + 56.17 -L-

DESIGN DATA

ADT 2016 = 1,960

ADT 2036 = 2,740

K = 12 %

D = 55 %

T = 7 % *

V = 60 MPH

* TTST = 2% + DUAL 5%

FUNCTIONAL CLASSIFICATION = MINOR COLLECTOR SUBREGIONAL TIER

PROJECT LENGTH

LENGTH OF ROADWAY PROJECT B-4814 = 0.173 MI

LENGTH OF STRUCTURE PROJECT B-4814 = 0.045 MI

0.218 MI LENGTH OF TOTAL PROJECT B-4814

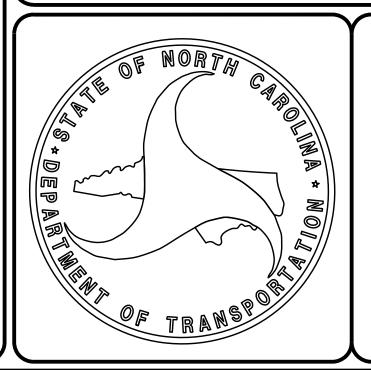
Prepared in the Office of:

DIVISION OF HIGHWAYS 1000 Birch Ridge Dr., Raleigh NC, 27610

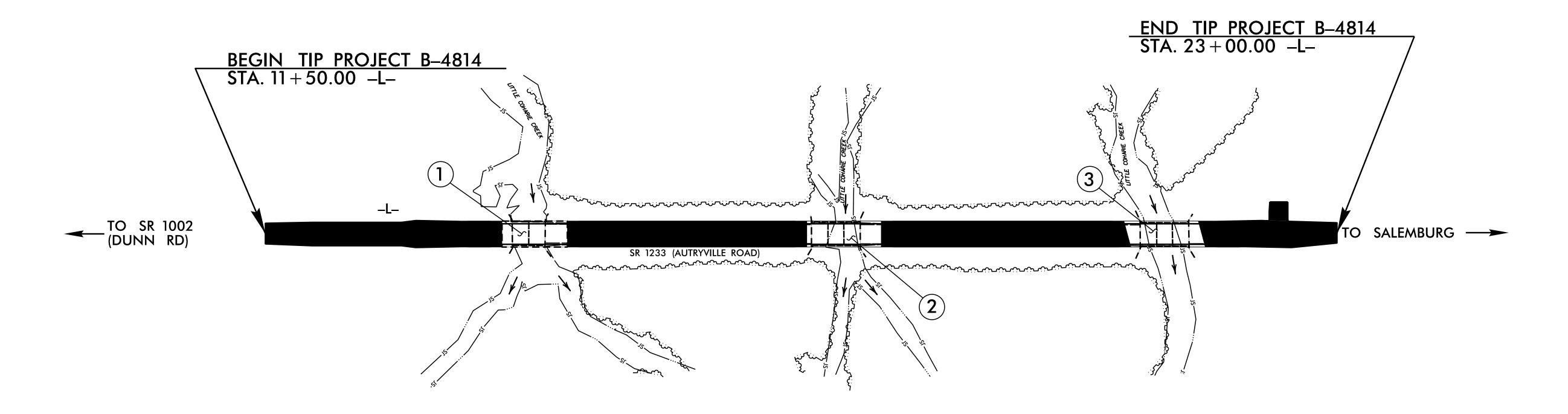
2012 STANDARD SPECIFICATIONS

LETTING DATE: **DECEMBER 20, 2016**

MARC G. CHEEK P.E. PROJECT DESIGN ENGINEER







INDEX							
STR. No.	STATION	DESCRIPTION	SHEET NUMBERS				
1	14+39.00 -L-	BRIDGE NO.102 OVER LITTLE COHARIE CREEK ON SR 1233 BETWEEN SR 1406 & SR 1412	S1-1 THRU S1-14				
2	17+71 . 00 -L-	BRIDGE NO.103 OVER LITTLE COHARIE CREEK ON SR 1233 BETWEEN SR 1406 & SR 1412	S2-1 THRU S2-16				
3	21+15 . 00 -L-	BRIDGE NO.104 OVER LITTLE COHARIE CREEK ON SR 1233 BETWEEN SR 1406 & SR 1412	S3-1 THRU S3-16				

PROJECT NO. B-4814

SAMPSON COUNTY

STATION:

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

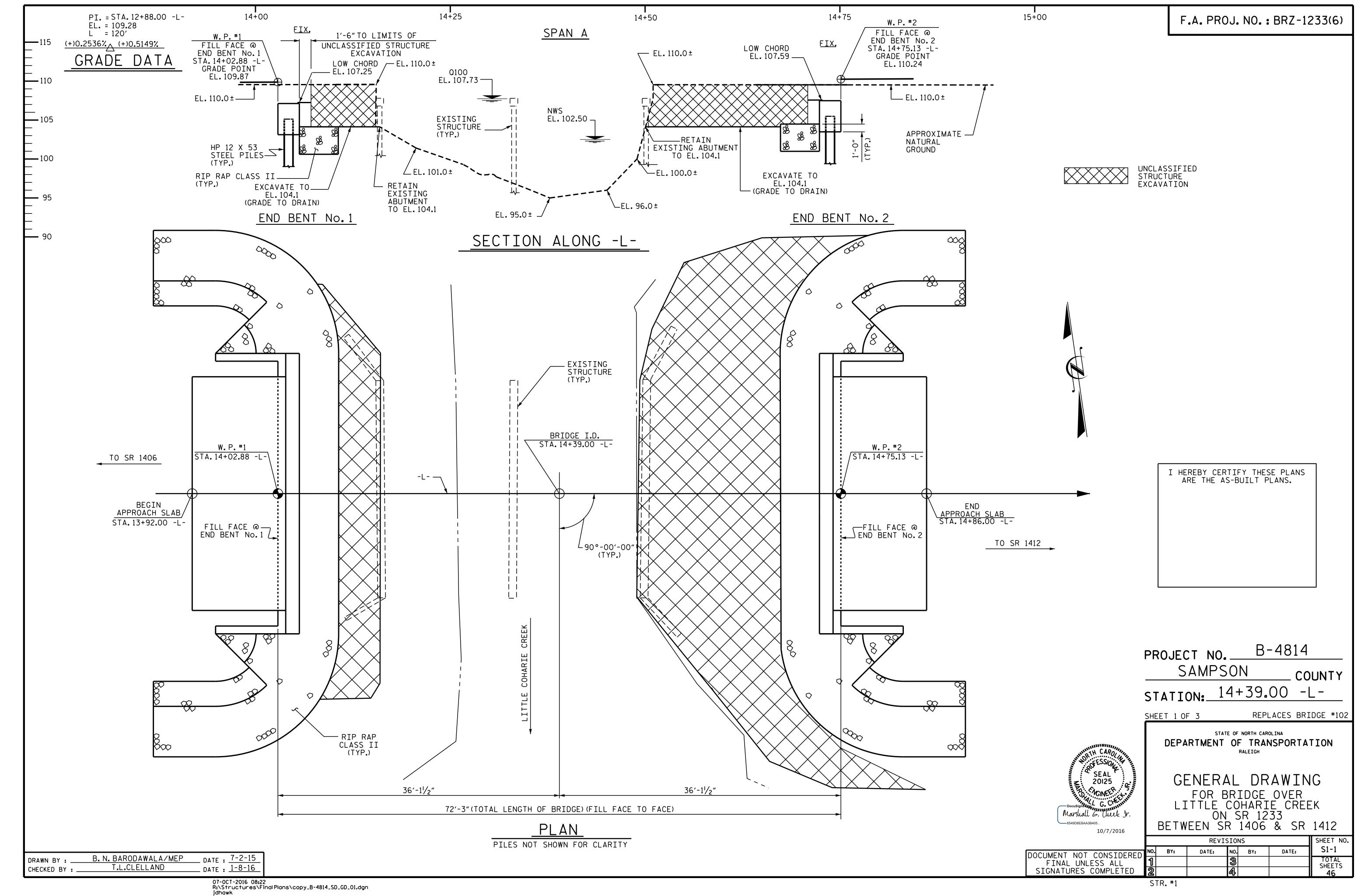
RALEIGH

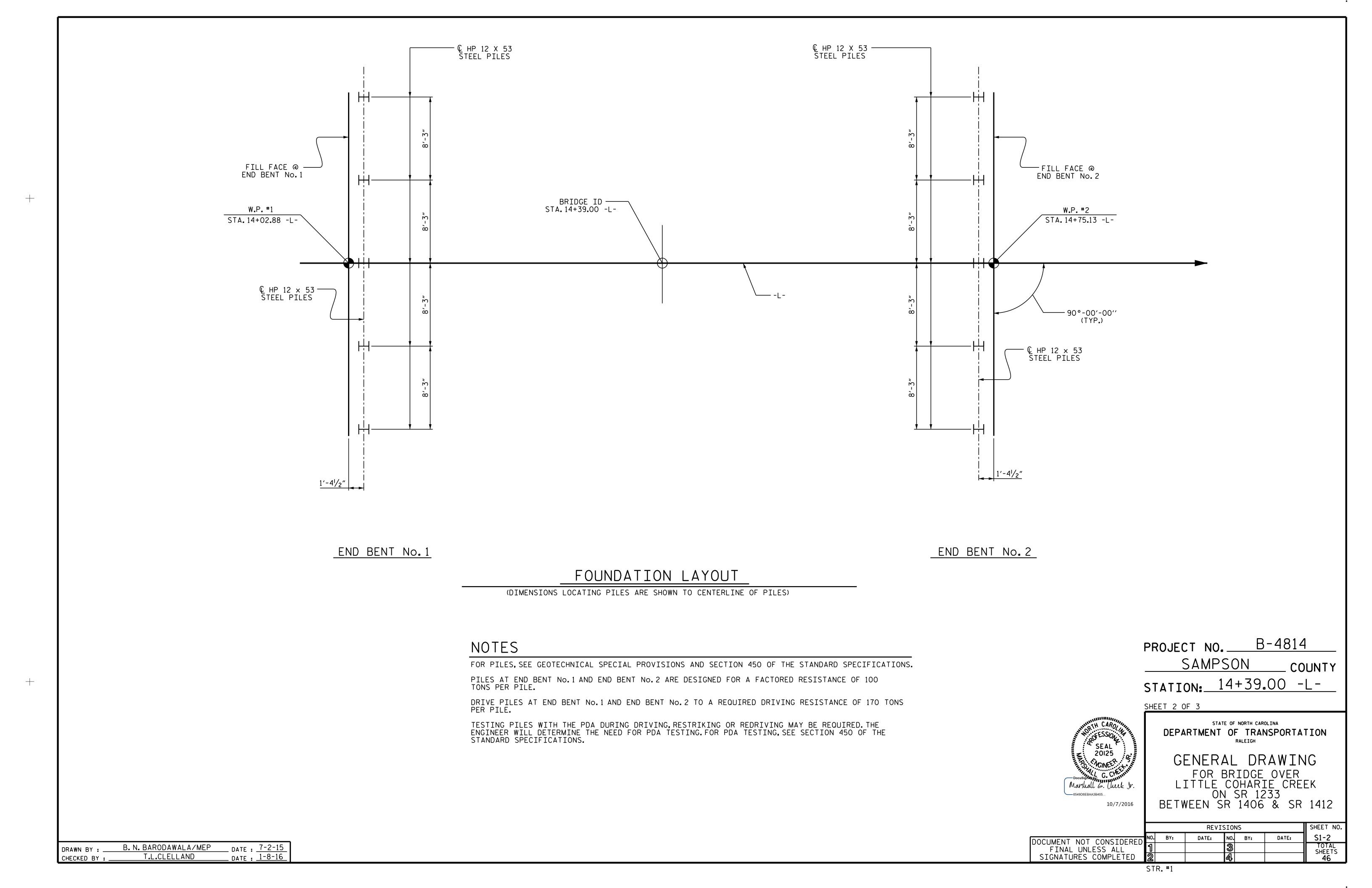
STRUCTURE INDEX

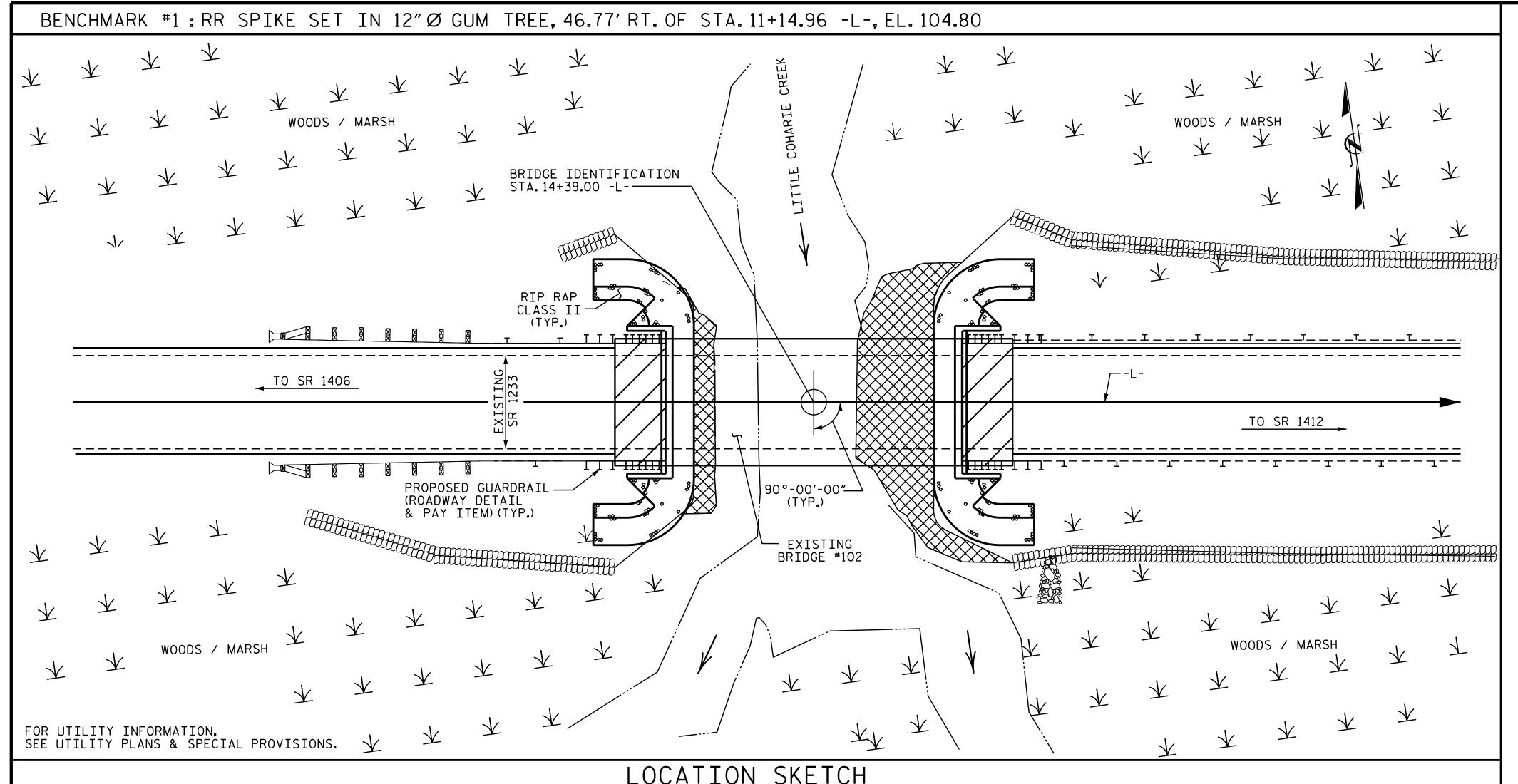
	REVISIONS							
NO.	BY:	DATE:	NO.	BY:	DATE:			
ኅ			3					
2			4					

 DRAWN BY :
 M. POOLE
 DATE :
 7-2016

 CHECKED BY :
 H. T. BARBOUR
 DATE :
 7-2016







HYDRAULIC DATA

DESIGN DISCHARGE = 3300 C.F.S. FREQUENCY OF DESIGN FLOOD = 25 YRS. DESIGN HIGH WATER ELEVATION = 106.70 DRAINAGE AREA = 72.8 SQ. MI. = 4900 C.F.S.

BASE DISCHARGE (Q100) BASE HIGH WATER ELEVATION = 107.73

OVERTOPPING DATA

OVERTOPPING DISCHARGE = 6000 C.F.S. FREQUENCY OF OVERTOPPING FLOOD = 200+ YRS. OVERTOPPING FLOOD ELEVATION = 108.60 @ STA. 9+95.00 -L-

TOTAL BILL OF MATERIAL **INCLASSIFIED** GEOTEXTIL REMOVAL OF CLASS A CONCRETE **ELASTOMERI** REINFORCING HP 12 x 53 RIP RAP PRESTRESSED PILE CONCRETE **ASBESTOS** STRUCTURE **APPROACH** EXISTING FOR STEEL PILES REDRIVES TESTIN(CONCRETE BEARINGS **ASSESSMENT** BARRIER CLASS II STEEL SLABS EXCAVATION DRAINAGE STRUCTURE CORED SLABS RAIL LUMP SUN LUMP SUM EACH CU. YDS. LBS. NO. LIN. FT EACH SQ.YDS. LUMP SUM LIN.FT. LUMP SUM LIN.FT. LUMP SUM TONS LUMP SUM 700.00 LUMP SUM LUMP SUN 140.25 SUPERSTRUCTURE LUMP SUM 20.2 2449 325 132 147 END BENT No.1 END BENT No. 2 LUMP SUM 20.2 325 132 147 2449 LUMP SUM LUMP SUM LUMP SUM 10 700.00 LUMP SUM 4898 LUMP SUM 140.25 264 294 650 TOTAL

NOTES:

ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

THE EXISTING STRUCTURE CONSISTS OF 2 SPANS (1 @ 17'-9", 1 @ 17'-10") WITH A CLEAR ROADWAY WIDTH OF 24'-0" AND A REINFORCED CONCRETE DECK ON 19 LINES OF 6" X 13 1/2" TIMBER JOISTS. END BENTS AND BENTS CONSIST OF TIMBER CAPS AND TIMBER PILES. THE EXISTING STRUCTURE, LOCATED AT THE SITE OF THE PROPOSED BRIDGE SHALL BE REMOVED.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 25 FT EACH SIDE OF CENTERLINE ROADWAY AT END BENT No.1 AND FOR A DISTANCE OF 35 FT. LEFT AND 40 FT. RIGHT OF THE CENTERLINE ROADWAY AT END BENT No. 2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES. SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, PRECAST CONCRETE END BENT CAPS MAY BE SUBSTITUTED IN PLACE OF THE CAST-IN-PLACE CAPS. THE CONTRACTOR SHALL COORDINATE WITH RESIDENT ENGINEER TO RECEIVE REVISED PLANS AND DETAILS FROM THE STRUCTURES MANAGEMENT UNIT. THE REDESIGN AND ANY ADDITIONAL MATERIALS NEEDED WILL BE AT NO ADDITIONAL COST TO THE CONTRACTOR.

> B-4814 PROJECT NO. ____ SAMPSON COUNTY STATION: 14+39.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING FOR BRIDGE OVER LITTLE COHARIE CREEK ON SR 1233

SHEETS 46

10/7/2016

FESSION.

20125

CHOINEER Marshall G. Check Ir BETWEEN SR 1406 & SR 1412 SHEET NO REVISIONS S1-3 DATE: BY:

DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

_ DATE : <u>7-2-15</u>

DATE : 1-8-16

B. N. BARODAWALA

T.L.CLELLAND

DRAWN BY

CHECKED BY : _

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	NGTH	I LIN	MIT ST	ГАТЕ				SE	RVICE	III	LIMI	ГЅТА	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVELOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	<u> </u>	1.006		1.75	0.273	1.03	70′	EL	34.5	0.507	1 . 32	70′	EL	6.9	0.80	0.273	1.01	70′	EL	34.5	
DESIGN	-	HL-93(0pr)	N/A		1.341		1.35	0.273	1.34	70′	EL	34.5	0.507	1.72	70′	EL	6.9	N/A						
LOAD RATING		HS-20(Inv)	36.000	(2)	1.306	47.020	1.75	0.273	1.34	70′	EL	34.5	0.507	1.65	70′	EL	6.9	0.80	0.273	1.31	70′	EL	34.5	
	1	HS-20(0pr)	36.000		1.740	62.640		0.273	1.74	70′	EL	34.5	0.507	2.14	70′	EL	6.9	N/A						
		SNSH	13.500		2.917	39.379	1.40	0.273	3.75	70′	EL	34.5	0.507	4.87	70′	EL	6.9	0.80	0.273	2.92	70′	EL	34.5	
	щ	SNGARBS2	20.000		2.187	43.741	1.40	0.273	2.81	70′	EL	34.5	0.507	3.47	70′	EL	6.9	0.80	0.273	2.19	70′	EL	34.5	
	HICL	SNAGRIS2	22.000		2.077	45.690	1.40	0.273	2.67	70′	EL	34.5	0.507	3.23	70′	EL	6.9	0.80	0.273	2.08	70′	EL	34.5	
	> < 	SNCOTTS3	27.250		1.452	39 . 565	1.40	0.273	1.87	70′	EL	34.5	0.507	2.43	70′	EL	6.9	0.80	0.273	1.45	70′	EL	34.5	
	IGLE (SV)	SNAGGRS4	34.925		1.218			0.273	1.57		EL	34.5	0.507	2.03	70′	EL	6.9	0.80	0.273	1.22	70′	EL	34.5	
	SIN	SNS5A	35.550		1.191	42.346	1.40	0.273	1.53	70′	EL	34.5	0.507	2.06	70′	EL	6.9	0.80	0.273	1.19	70′	EL	34.5	
		SNS6A	39.950		1.095	43.747	1.40	0.273	1.41	70′	EL	34.5	0.507	1.88	70′	EL	6.9	0.80	0.273	1.10	70′	EL	34.5	
LEGAL	~	SNS7B	42.000		1.043	43.801	1.40	0.273	1.34	70′	EL	34.5	0.507	1.85	70′	EL	6.9	0.80	0.273	1.04	70′	EL	34.5	
LOAD RATING	ILER	TNAGRIT3	33.000		1.336	44.087	1.40	0.273	1.72	70′	EL	34.5	0.507	2.23	70′	EL 	6.9	0.80	0.273	1.34	70′	EL	34.5	
	-TRA	TNT4A	33.075		1.342	44.401	1.40	0.273	1.72	70′	EL .	34.5	0.507	2.17	70′	EL	6.9	0.80	0.273	1.34	70′	EL	34.5	
	EMI	TNT6A	41.600		1.100	45.746		0.273	1.41	70′	EL	34.5	0.507	1.98	70′	EL	6.9	0.80	0.273	1.10	70′	EL	34.5	
	ER S	TNT7A	42.000		1.106	46.462	1.40	0.273	1.42	70′	EL	34.5	0.507	1.94	70′	EL	6.9	0.80	0.273	1.11	70′	EL	34.5	
	AILER (TTST	TNT7B	42.000		1.147	48.180	1.40	0.273	1.47	70′	EL	34.5	0.507	1.8	70′	EL	6.9	0.80	0.273	1.15	70′	EL	34.5	
	A ∃ R	TNAGRIT4	43.000		1.089	46.838	1.40	0.273	1.4	70′	EL	34.5	0.507	1.74	70′	EL	6.9	0.80	0.273	1.09	70′	EL	34.5	
	RUC	TNAGT5A	45.000	 /-\	1.026	46.175	1.40	0.273	1.32	70′	EL	34.5	0.507	1.74	70′	EL	6.9	0.80	0.273	1.03	70′	EL	34.5	
	–	TNAGT5B	45.000	<u> </u>	1.013	45 . 579	1.40	0.273	1.3	70′	EL	34.5	0.507	1.66	70′	EL	6.9	0.80	0.273	1.01	70′	EL	34.5	

LOAD FACTORS:

	DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
	LOAD RATING FACTORS	STRENGTH I	1.25	1.50
		SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

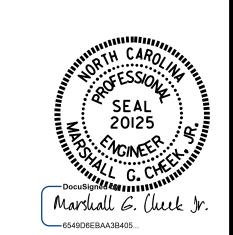
EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-4814

SAMPSON COUNTY

STATION: 14+39.00 -L-



DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

LRFR SUMMARY FOR

70' CORED SLAB UNIT

90°-00'-00" SKEW

(NON-INTERSTATE TRAFFIC)

REVISIONS SHEET

TOTAL SIGNATURES COMPLETED

10/7/2016

REVISIONS

REVISIONS

SHEET NO. BY: DATE: NO. BY: DATE: S1-4

SIGNATURES COMPLETED

2

46

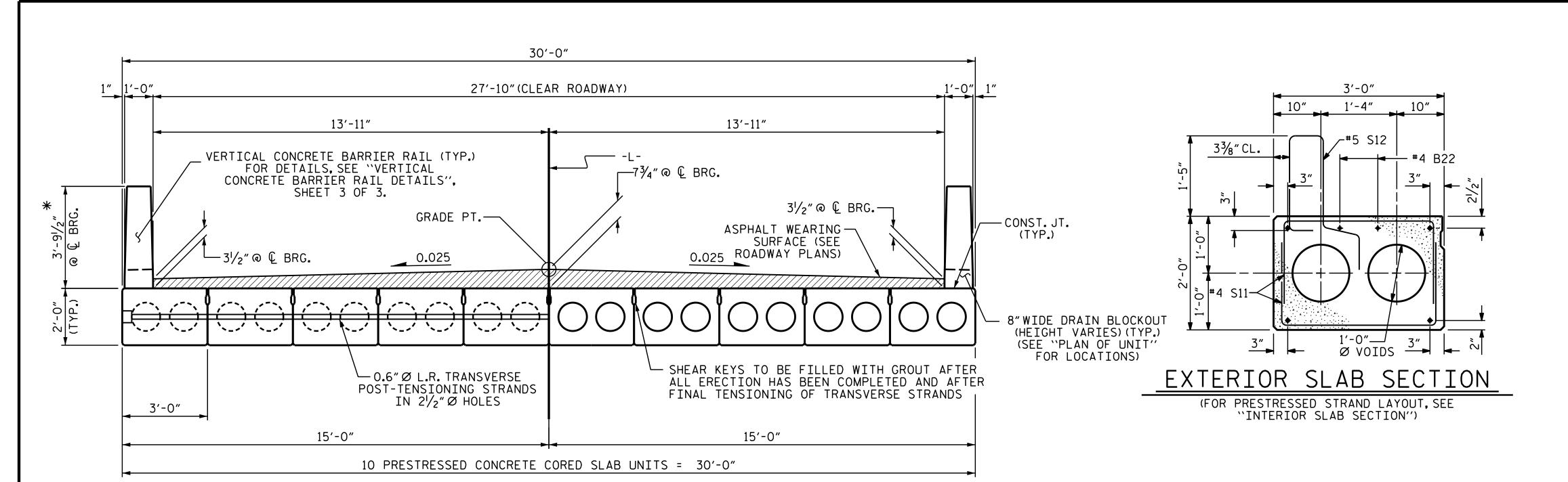
2 3

LRFR SUMMARY

FOR SPAN A

ASSEMBLED BY: R. CAREATHERS/MP DATE: 5/29/15 CHECKED BY: T.L.CLELLAND DATE: 1/2016

DRAWN BY: CVC 6/IO
CHECKED BY: DNS 6/IO

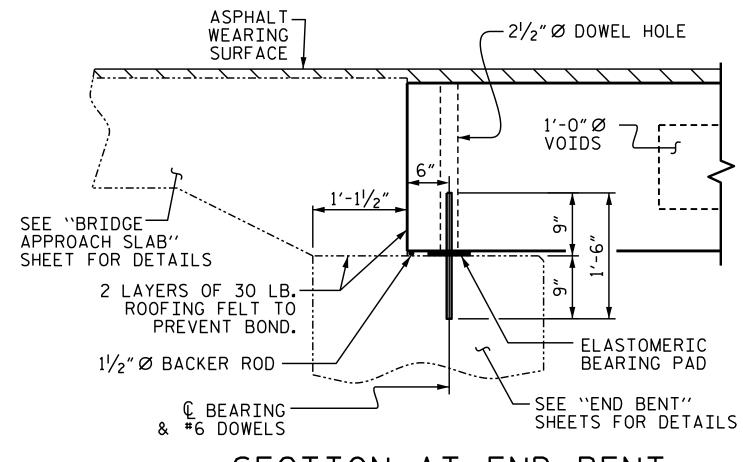


HALF SECTION AT INTERMEDIATE DIAPHRAGMS

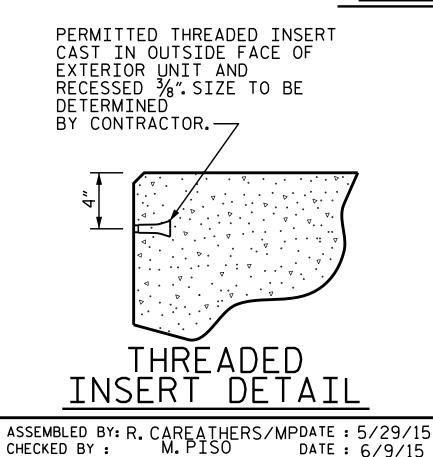
HALF SECTION THROUGH VOIDS TYPICAL SECTION

* - THE MAXIMUM VERTICAL CONCRETE BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE VERTICAL CONCRETE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE "VERTICAL CONCRETE BARRIER RAIL DETAILS", SHEET 3 OF 3.

FIXED END



SECTION AT END BENT



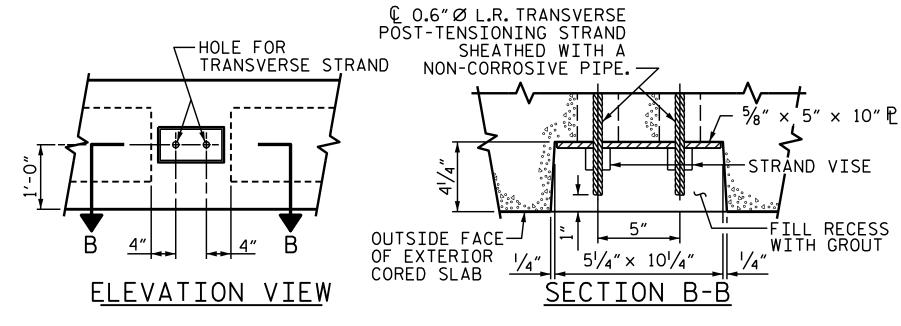
DRAWN BY : MAA 6/10

CHECKED BY : MKT 7/10

DATE: 6/9/15

MAA/TMG

REV. 8/14



GROUTED RECESS AT END OF POST-TENSIONED STRAND CORED SLABS

-2 SPA. @ 2"CTS. └6 SPA. └─2 SPA. @ 2"CTS. @ 2"CTS. 2 SPA. — @ 2"CTS. INTERIOR SLAB SECTION (28 STRANDS REQUIRED) 0.6'' Ø LOW

3'-0"

1'-4"

11" 4" 4"

1'-6"

ï VOIDS

1'-6"

#4 B22—

3'-0"

81/2" 91/2" 91/2" 81/2"

1'-2" 4" 4" 1'-2"

END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS

AND LOCATION OF DOWEL HOLES.

(STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB

UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

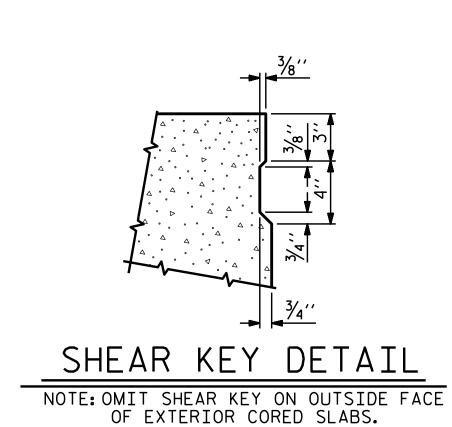
— © 2½″Ø \ DOWEL HOLES

-#5 S15

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 12'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

RELAXATION STRAND LAYOUT

DEBONDING LEGEND



PROJECT NO. B-4814 SAMPSON COUNTY STATION: 14+39.00 -L-

SEAL 3 20125 VIGINEER L Marshall G. Check Ir.

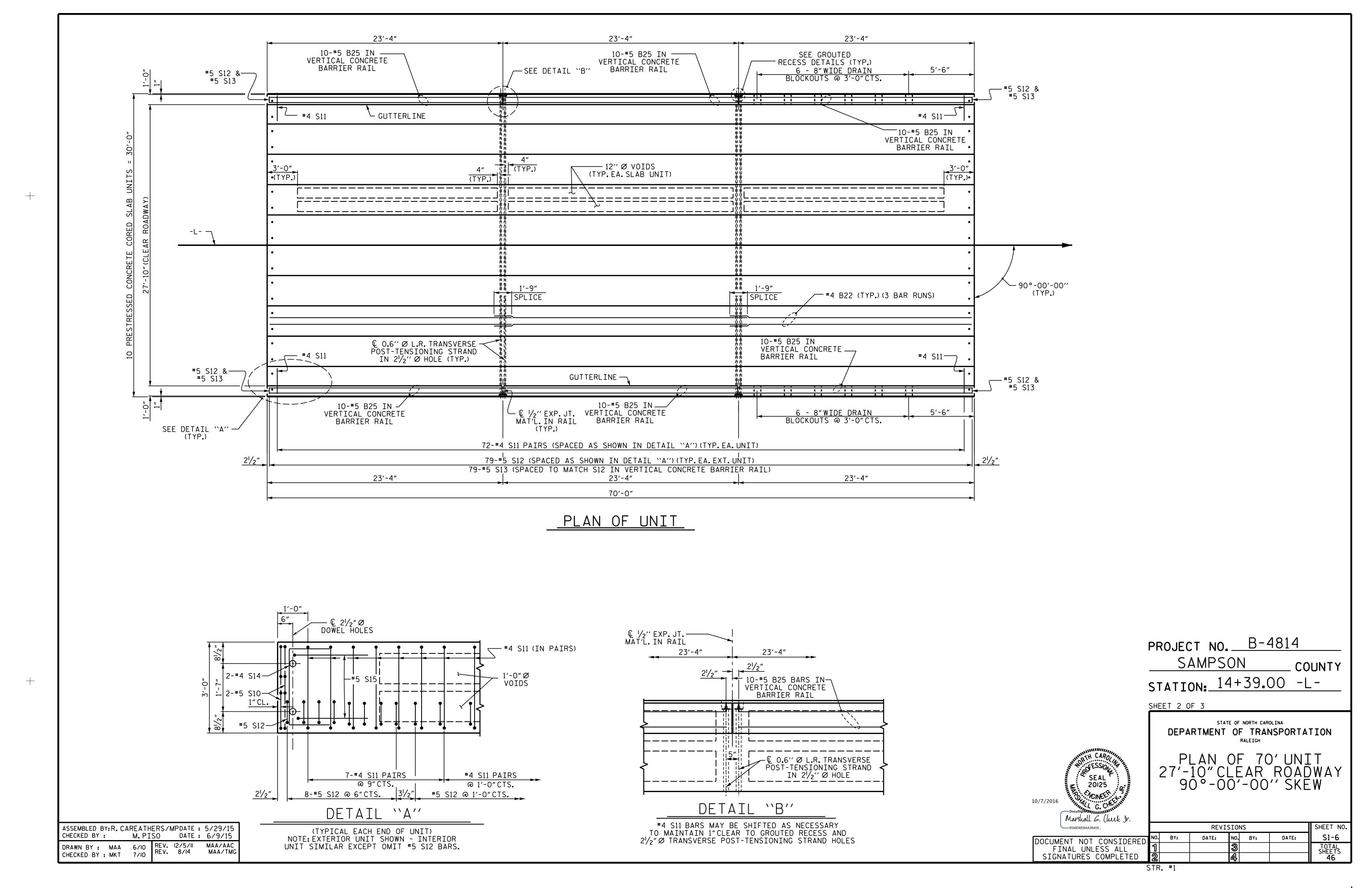
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

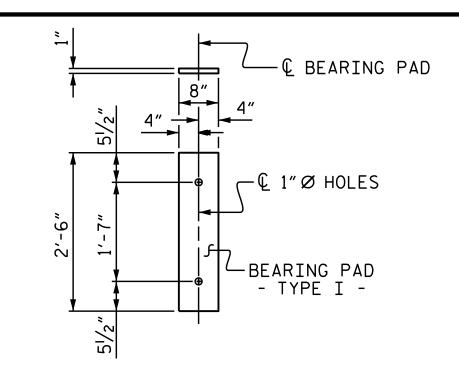
3'-0'' X 2'-0'' PRESTRESSÉD CONCRETE CORED SLAB UNIT

10/7/2016 SHEET NO. **REVISIONS** S1-5 DATE: DATE: BY: DOCUMENT NOT CONSIDERED TOTAL SHEETS FINAL UNLESS ALL SIGNATURES COMPLETED 46 STR. #1

SHEET 1 OF 3

\$\$\$\$\$\$\$SYSTIME\$\$\$\$ \$\$\$\$USERNAME\$\$\$\$





FIXED END (TYPE I - 20 REQ'D)

ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

CORED SLABS REQUIRED							
	NUMBER	LENGTH	TOTAL LENGTH				
70'UNIT							
EXTERIOR C.S.	2	70'-0"	140'-0"				
INTERIOR C.S.	8	70'-0"	560′-0″				
TOTAL	10		700′-0″				

ASSEMBLED BY: R. CAREATHERS/MP DATE: 5/29/15

DATE : 6/9/15

M. PISO

CHECKED BY :

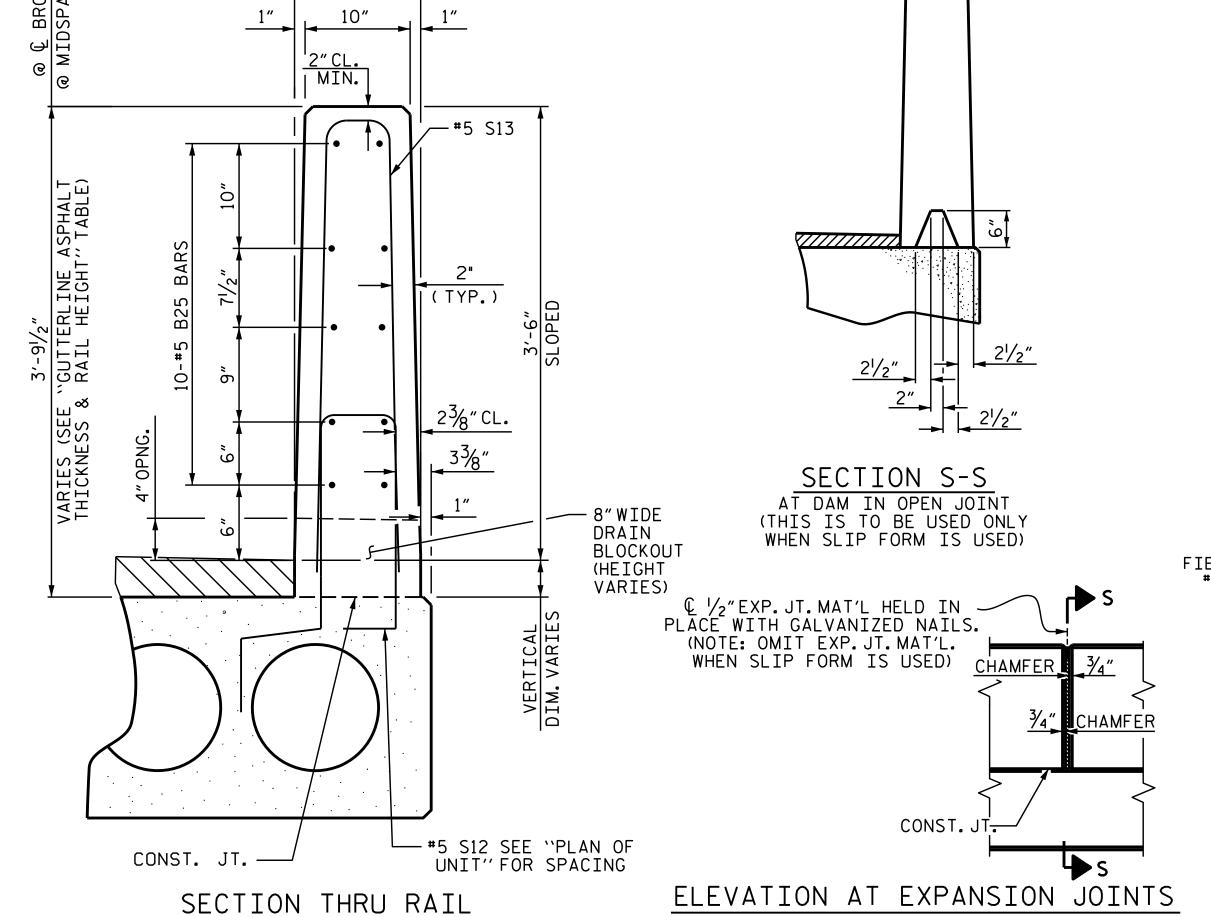
DRAWN BY: MAA 6/10 CHECKED BY : MKT 7/10

BILL OF MATERIAL FOR ONE 70' CORED SLAB UNIT EXTERIOR UNIT INTERIOR UNIT BAR | NUMBER | SIZE | TYPE | LENGTH | WEIGHT LENGTH | WEIGHT B22 | 6 #4 | STR | 24'-6" 24′-6″ 98 98 4'-9" S10 | #5 40 4′-9″ 40 S11 | 144 561 5′-10″ 561 #4 5′-10″ ***** S12 | 79 #5 5′-8″ 467 S14 | 4 #4 5′-7″ 15 5′-7″ 15 S15 7′-1″ 30 #5 4 7′-1″ 30 744 744 REINFORCING STEEL LBS. * EPOXY COATED REINFORCING STEEL 467 11.8 11.8 7000 P.S.I. CONCRETE CU. YDS. 0.6" Ø L.R. STRANDS No. 28 28 I DEAD LOAD DEFLECTION AND CAMBER

DEND LOND DELECTION IN	15 07 11115 21 1
	3'-0" × 2'-0"
70'CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	21/4″ ╽
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	3⁄4″ ♦
FINAL CAMBER	11/2"
** INCLUDES FUTURE WEARING SURF	ACE

GUTTERLINE ASP	HALT THICKNESS &	RAIL HEIGHT
	ASPHALT OVERLAY THICKNE @ MID-SPAN	SS RAIL HEIGHT @ MID-SPAN
70' UNITS	2"	3′-8″

1'-0"



NOTES

BAR TYPES

7¾"

S15 1'-81/2"

S14 2'-7"

S10 1'-9"

END VIEW

2'-8"

ALL BAR DIMENSIONS ARE OUT TO OUT

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2^{1}/2$ " Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

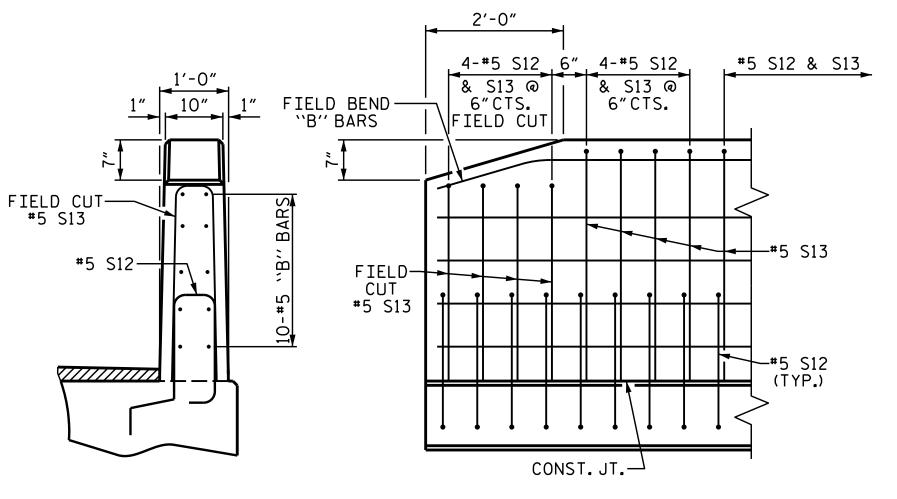
APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL BAR | BARS PER PAIR OF EXTERIOR UNITS | TOTAL NO. | SIZE | TYPE | LENGTH | WEIGHT 70' UNIT **∗**B25 60 60 #5 STR 22'-11" 158 ***** S13 158 2 7'-2" * EPOXY COATED REINFORCING STEEL 2615 LBS. CLASS AA CONCRETE 18.1 CU.YDS TOTAL VERTICAL CONCRETE BARRIER RAI LN. FT 140.25

SIDE VIEW



END OF RAIL DETAILS

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1"CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 4" X 8". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE CORED SLAB UNIT TO THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR CORED SLAB UNITS THAT REQUIRE DRAINS IN THE BARRIER RAIL.

CONCRETE	RELEASE	STRENGTH
UNIT		PSI
70' UNITS		5500
CDADE 27	CTDANID	

GRADE 270 S	IRANUS
	0.6"Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950

B-4814 PROJECT NO. ___ SAMPSON COUNTY STATION: 14+39.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 2'-0" PRESTRESSED CONCRETE CORED SLAB UNIT

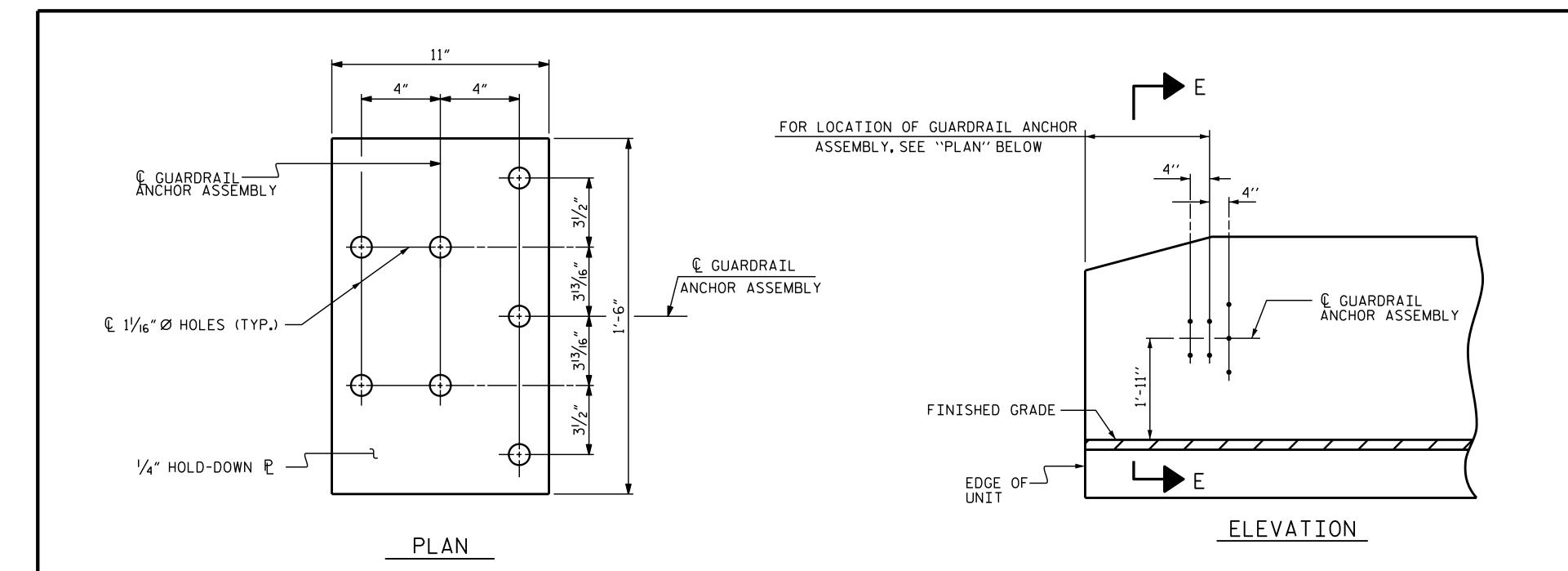
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SHEET NO. **REVISIONS** S1-7 DATE: DATE: NO. BY: TOTAL SHEETS 46

VERTICAL CONCRETE

BARRIER RAIL DETAILS

STR. #1



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

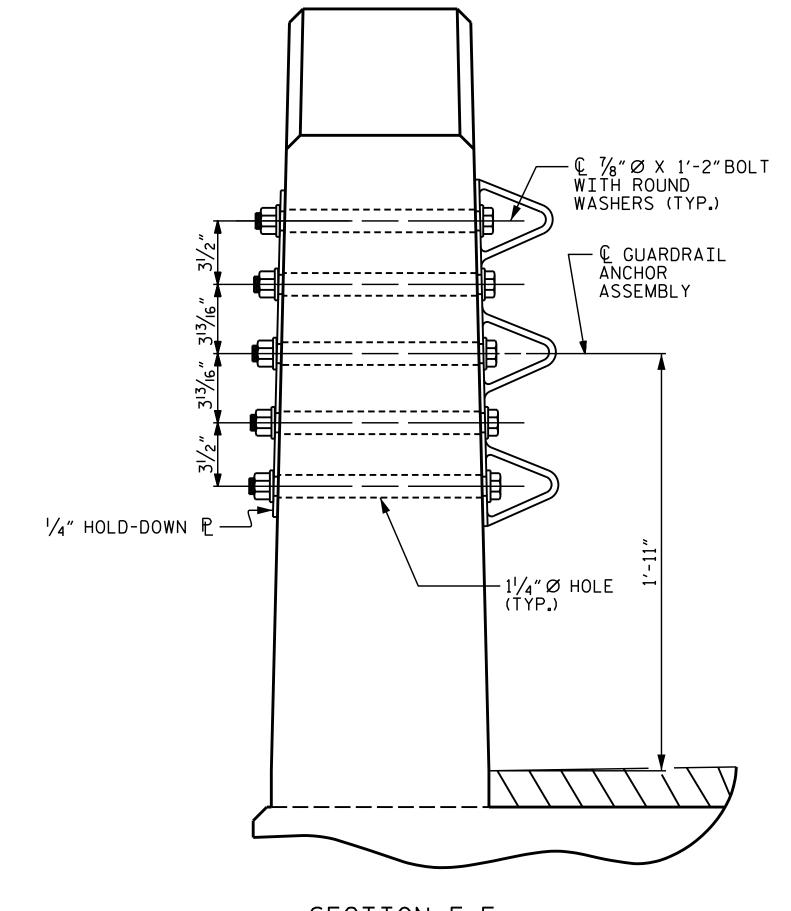
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT. SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

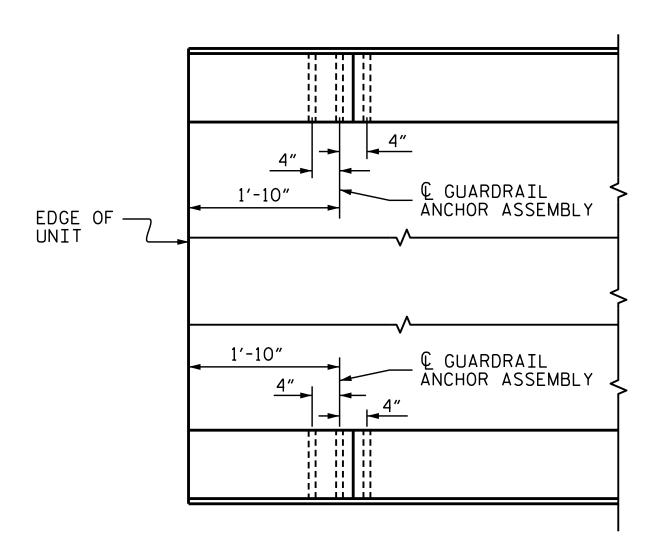
THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SECTION E-E

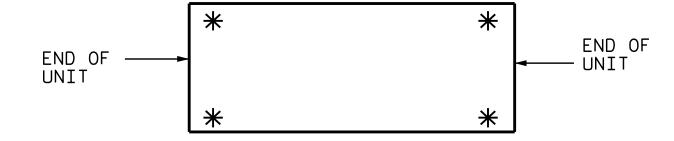
GUARDRAIL ANCHOR ASSEMBLY DETAILS



PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

END BENT No.1 SHOWN, END BENT No.2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-4814

SAMPSON COUNTY

STATION: 14+39.00 -L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

PALETCH

GUARDRAIL ANCHORAGE
DETAILS
FOR VERTICAL CONCRETE
BARRIER RAIL

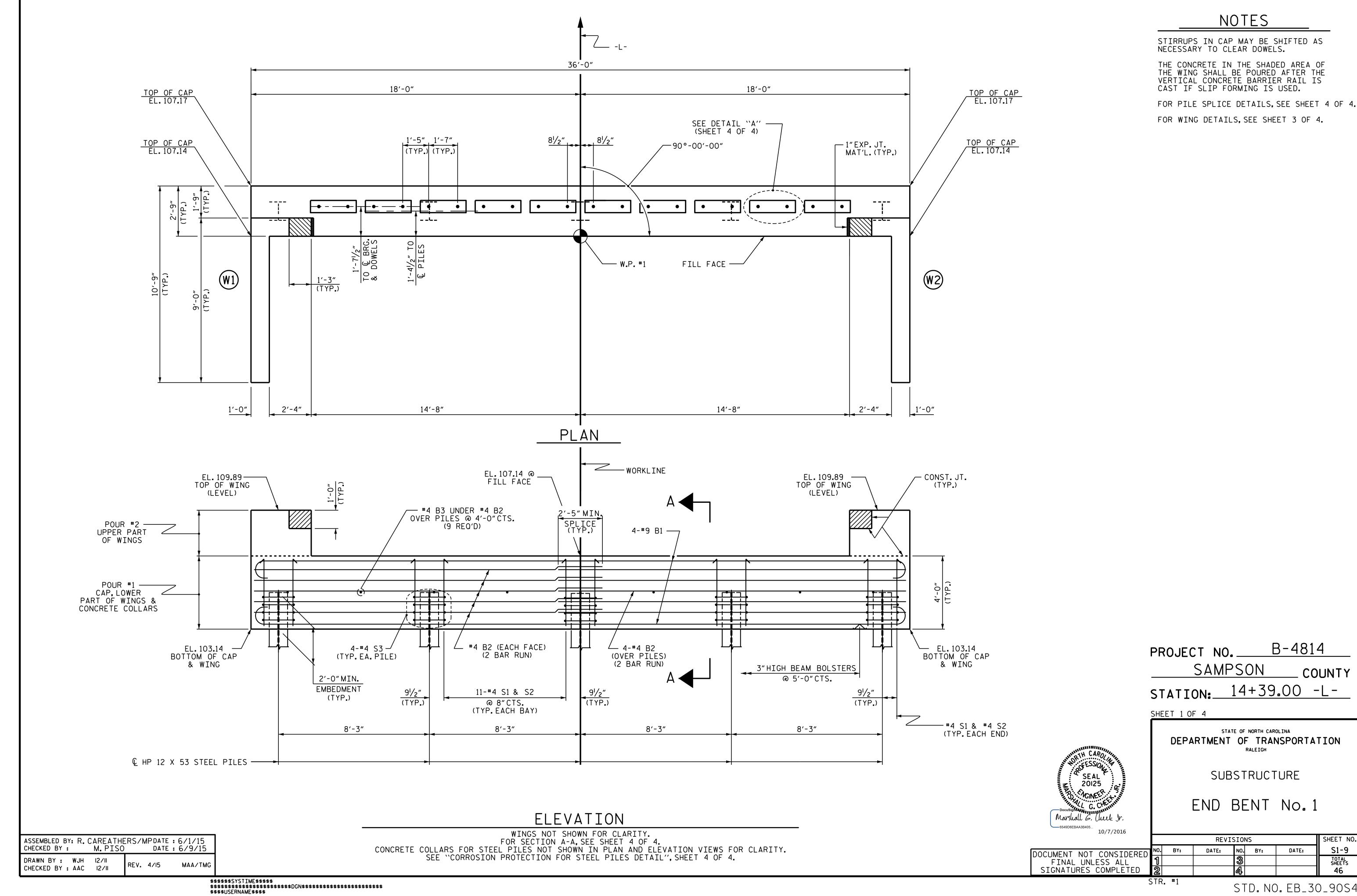
REVISIONS SHEET NO.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2 46

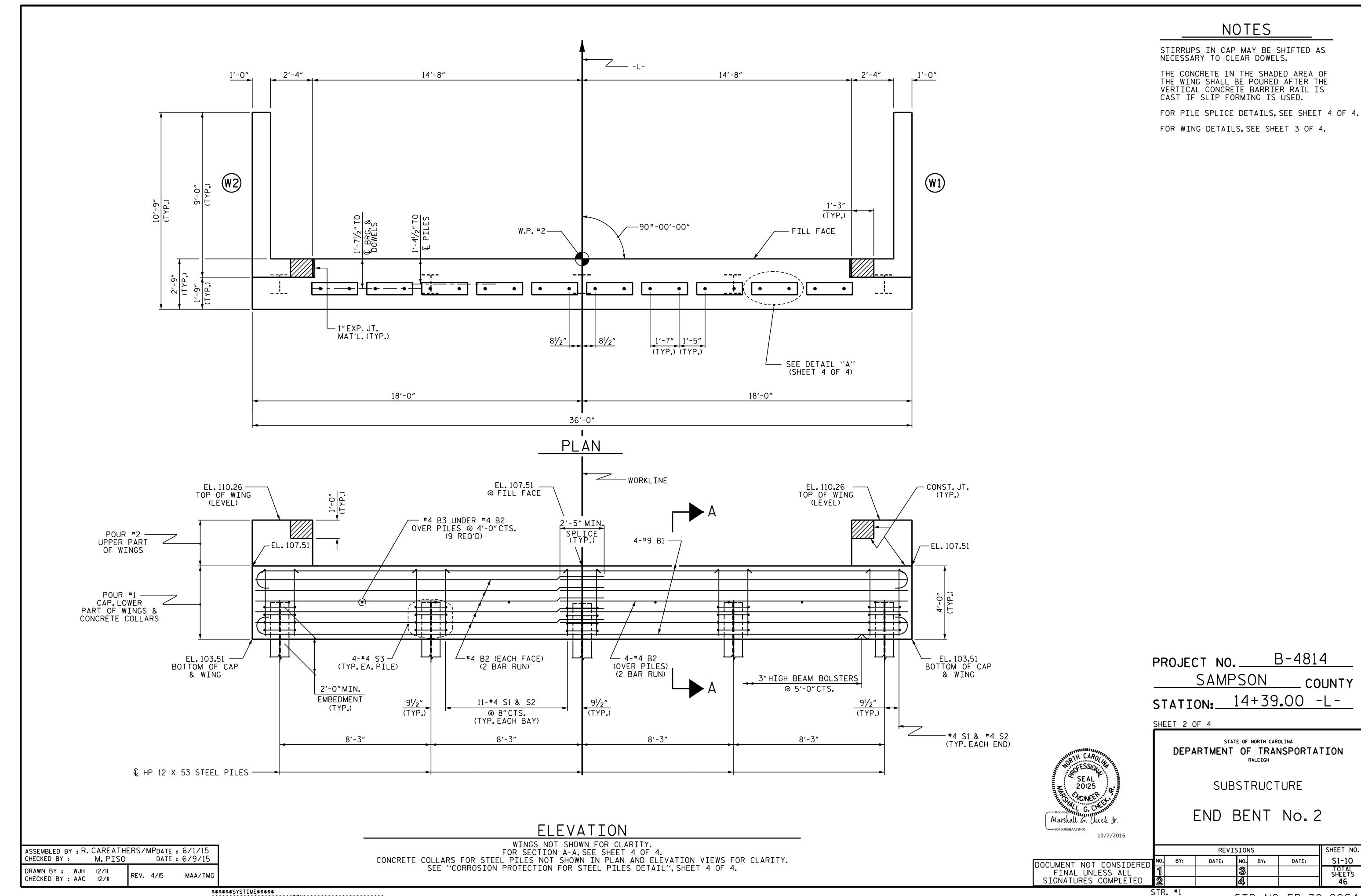
ASSEMBLED BY: R.CAREATHERS/MP DATE: 6/3/15
CHECKED BY: M. PISO DATE: 6/9/15

DRAWN BY: MAA 5/10
CHECKED BY: GM 5/10
REV. 12/5/II
REV. 6/13
REV. 1/15
MAA/GM
REV. 1/15

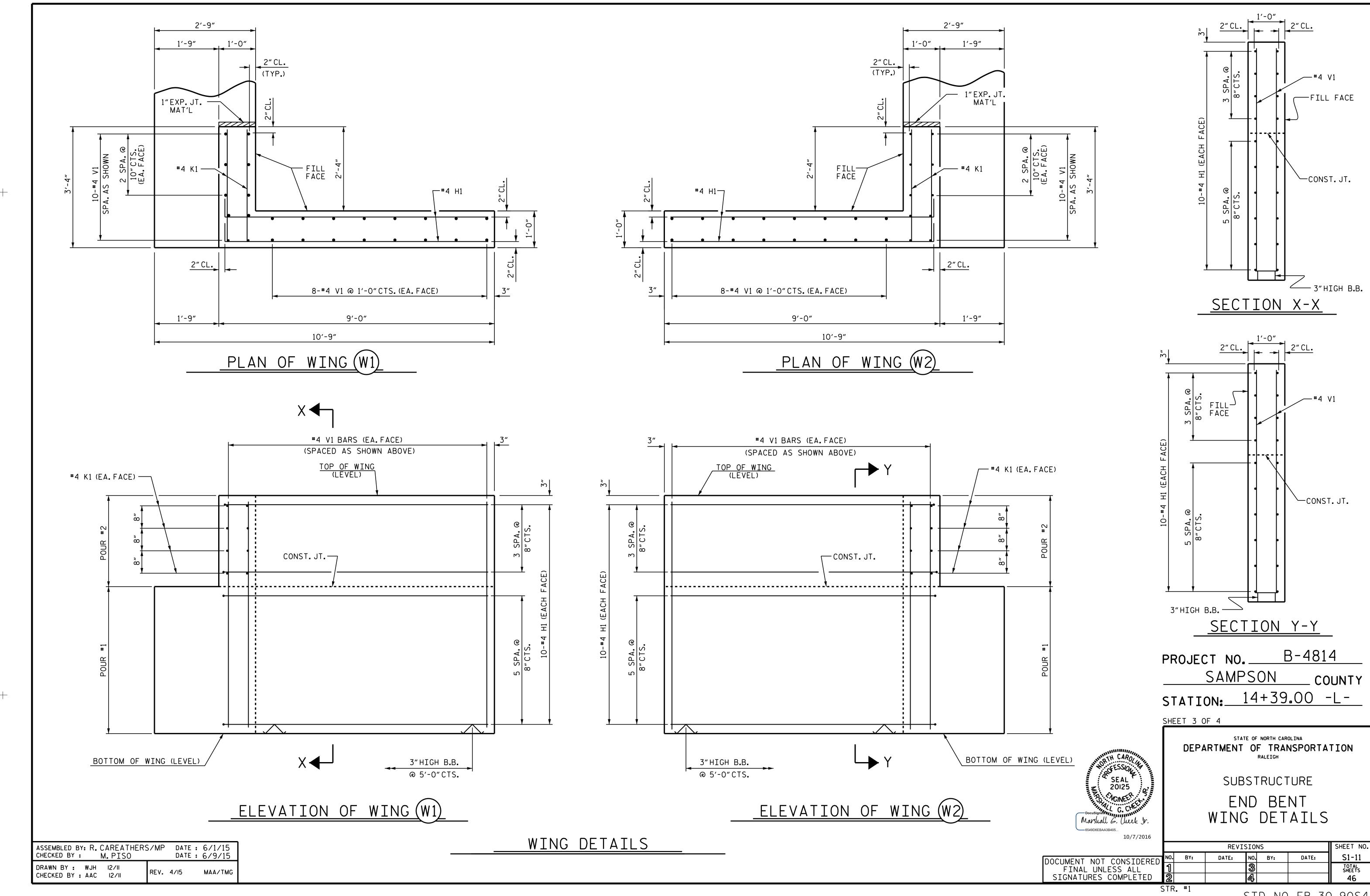
STR. #1 (SHT 1)

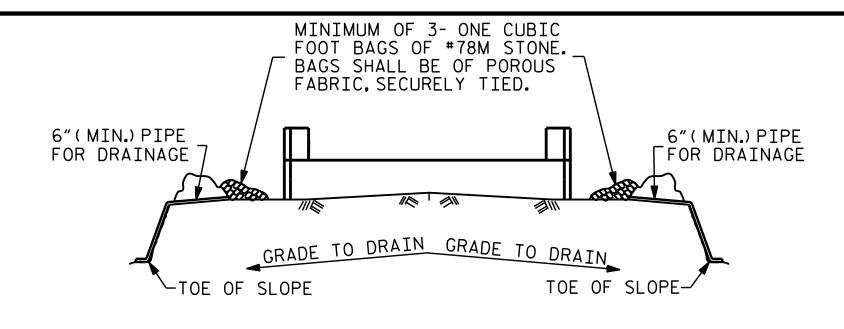


STD. NO. EB_30_90S4



STD.NO.EB_30_90S4



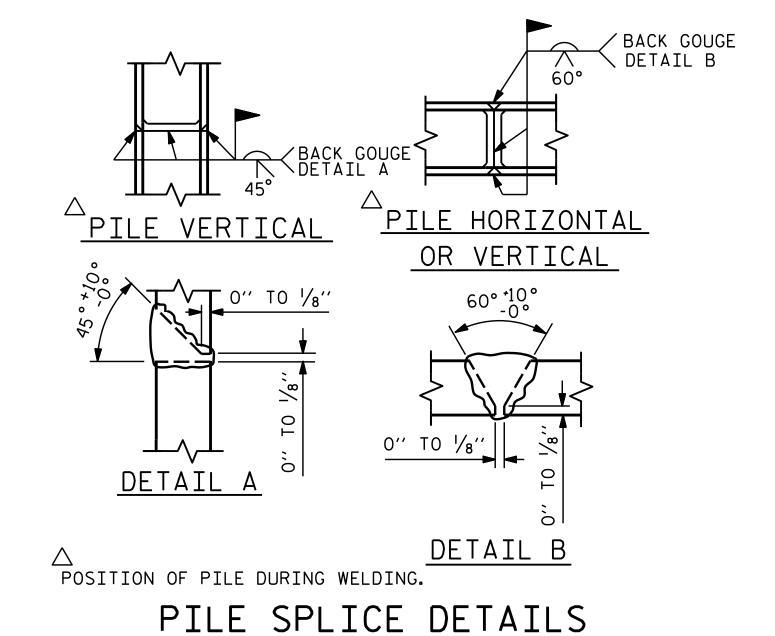


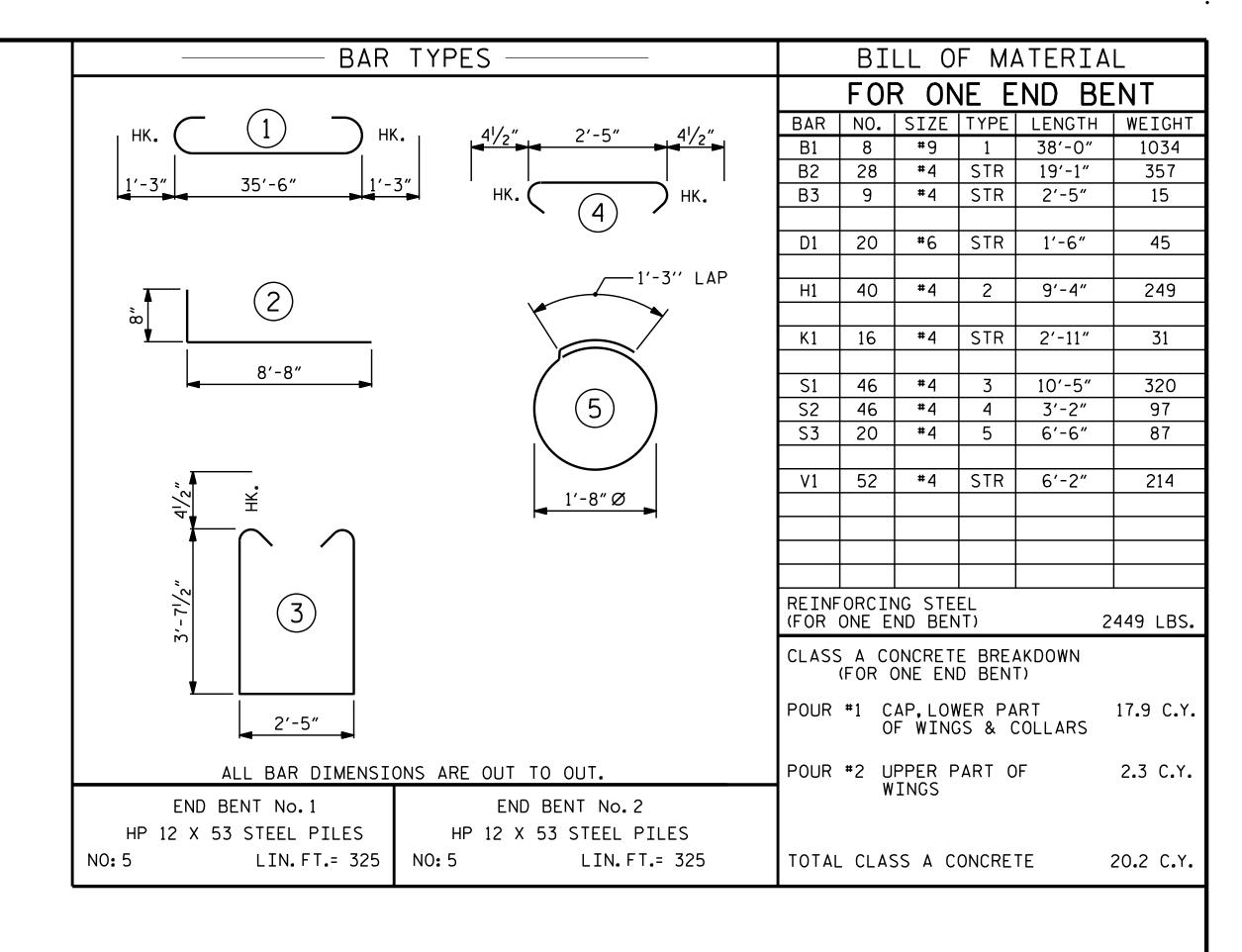
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

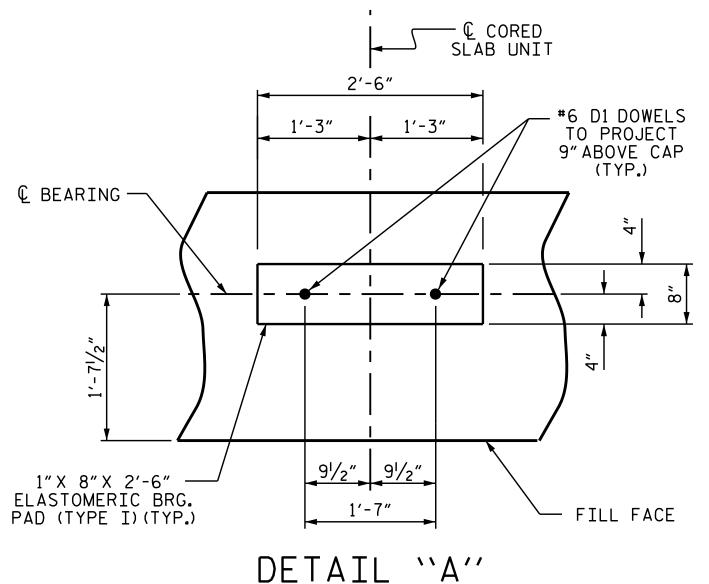
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

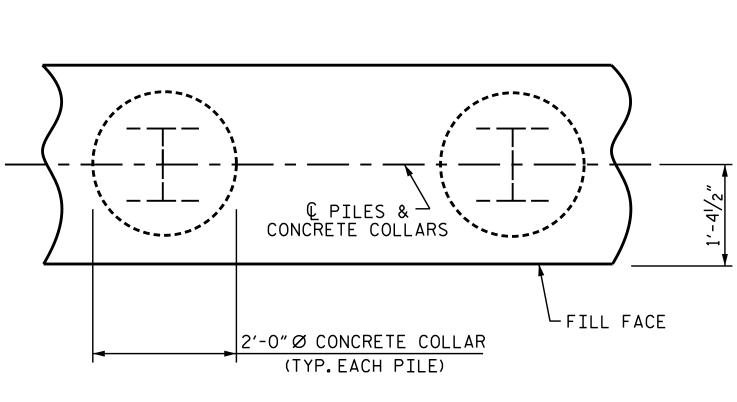
TEMPORARY DRAINAGE AT END BENT





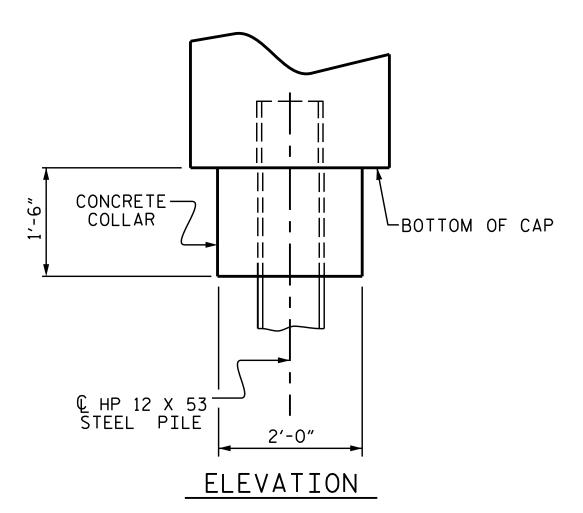


(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



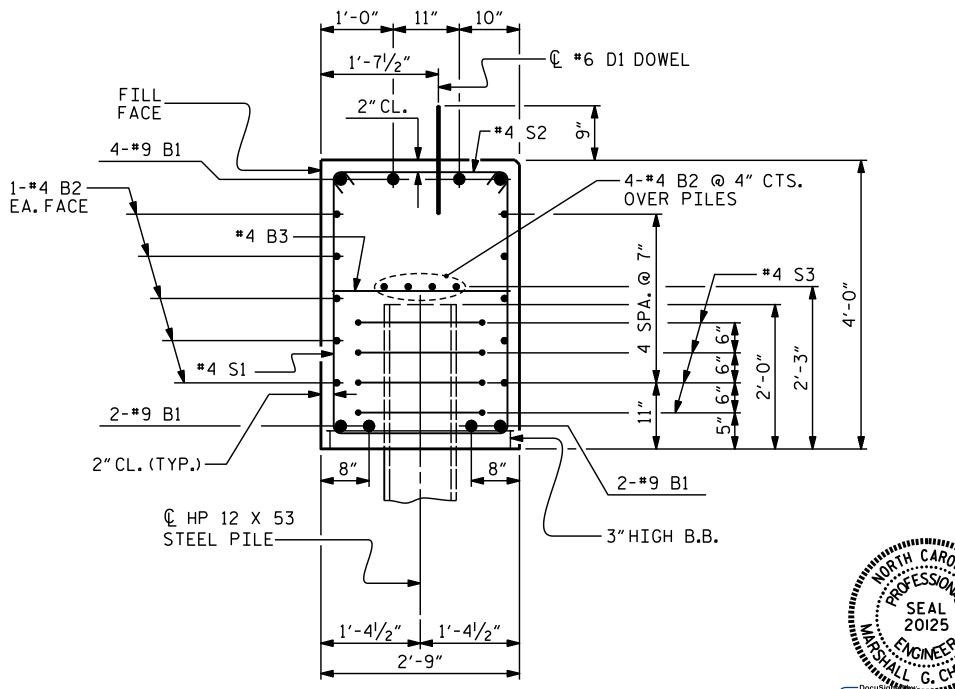
PLAN CORROSION PROTECTION FOR STEEL PILES DETAIL (END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

ASSEMBLED BY: R. CAREATHERS/MP DATE: 6/1/15 CHECKED BY: M. PISO DATE: 6/9/15 DRAWN BY : WJH 12/11 CHECKED BY : AAC 12/11



SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")



B-4814 PROJECT NO._ SAMPSON COUNTY 14+39.00 -L-

STATION:_

SHEET 4 OF 4

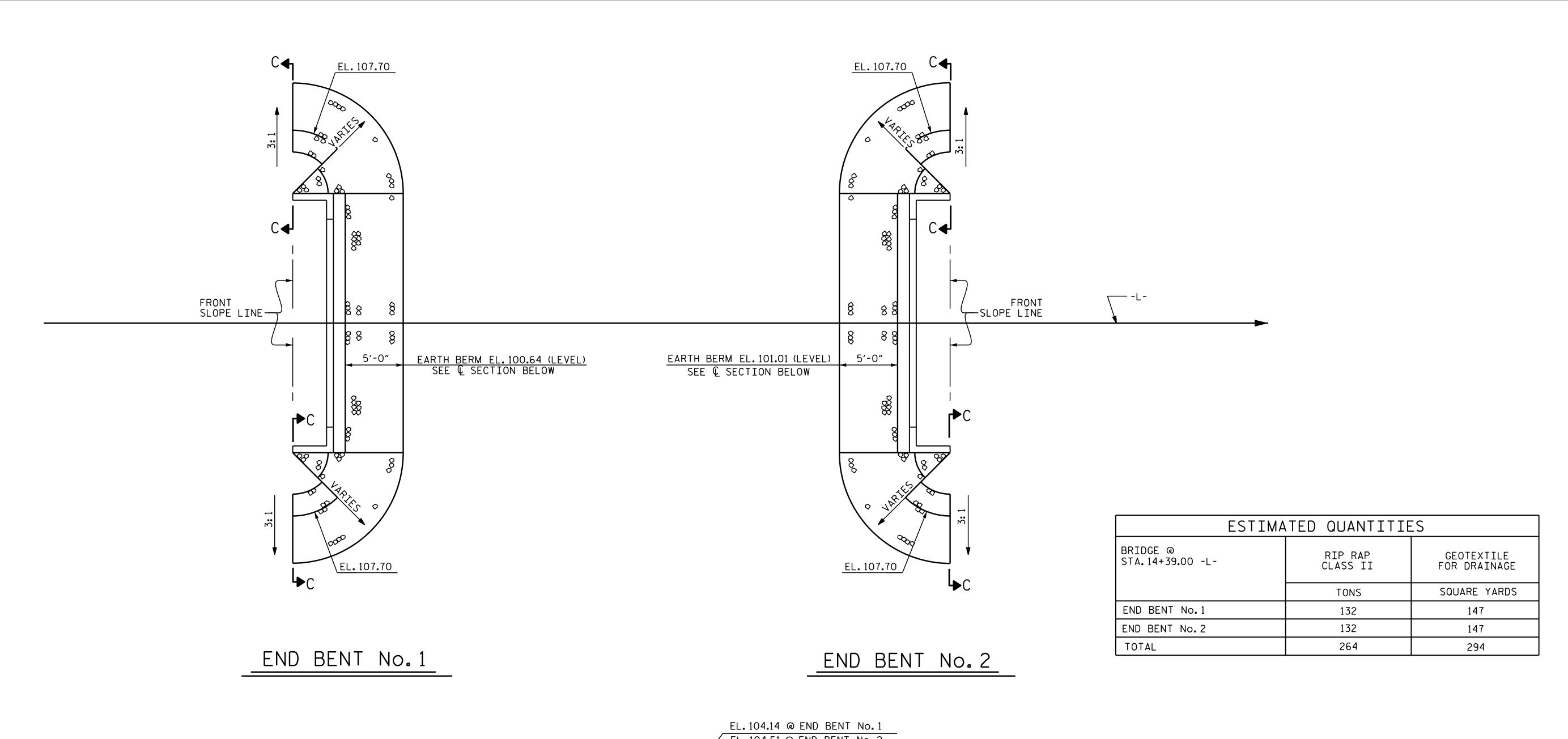
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

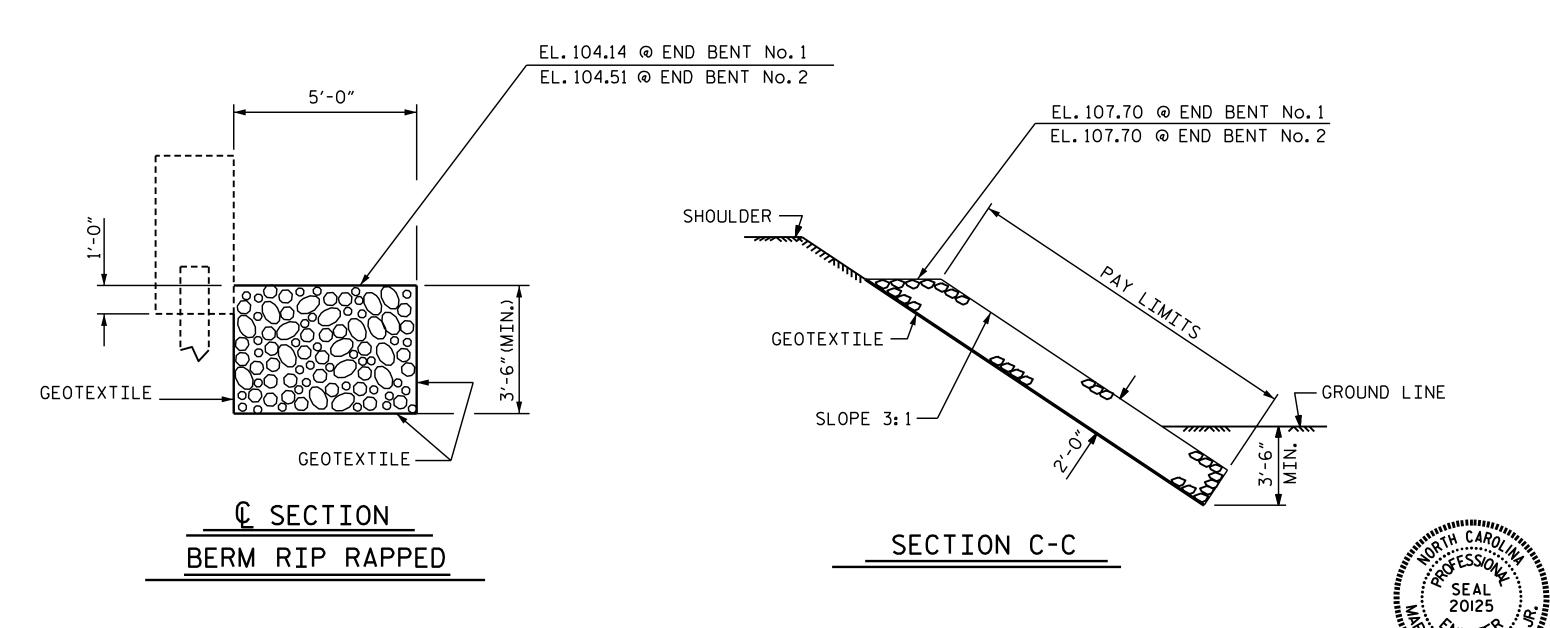
SUBSTRUCTURE

END BENT No.1 & 2 DETAILS

10/7/2016 **REVISIONS** SHEET NO S1-12 NO. BY: DATE: DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS 46

STR. #1





PROJECT NO. B-4814

SAMPSON COUNTY

STATION: 14+39.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

-RIP RAP DETAILS-

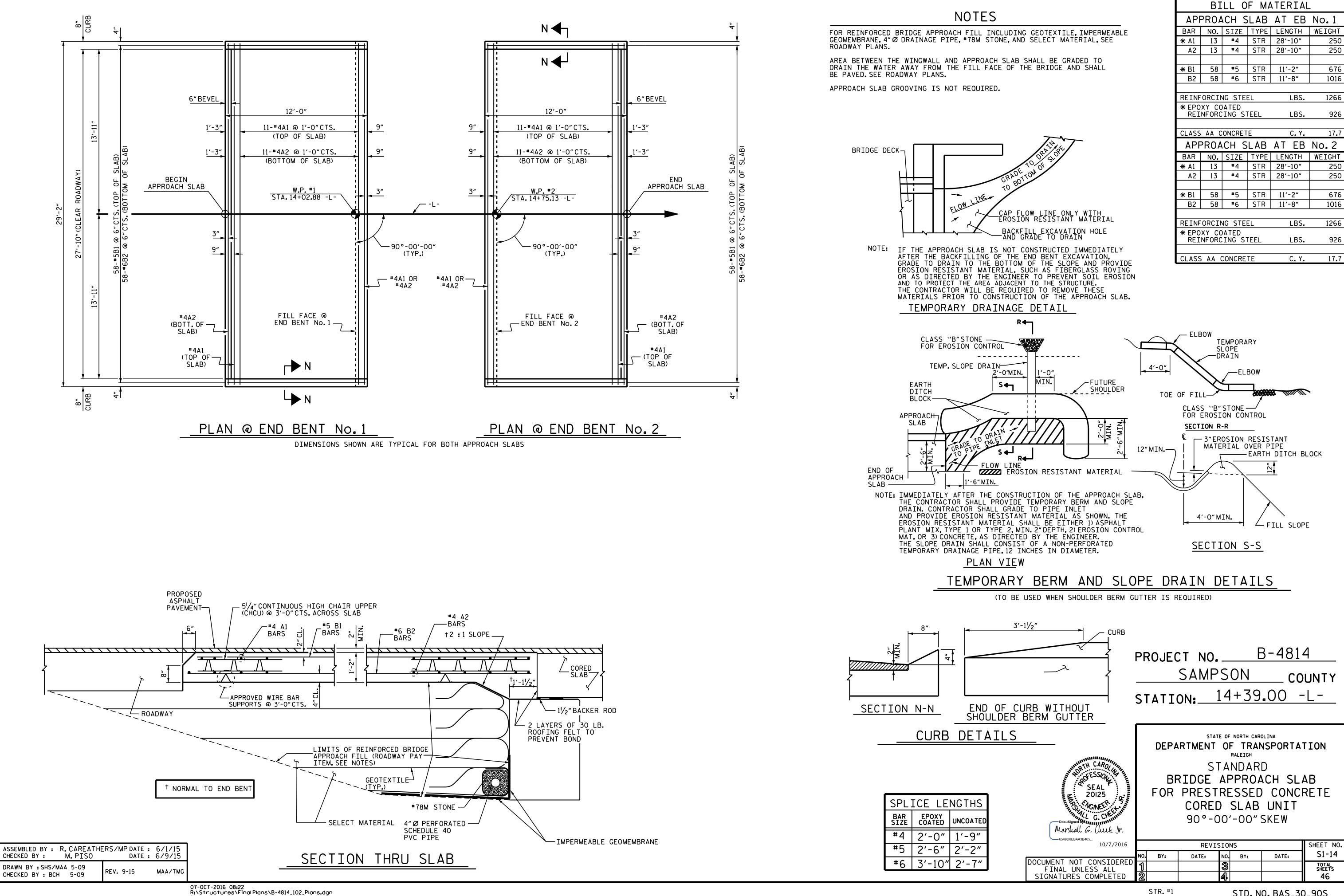
REVISIONS SHEET NO.

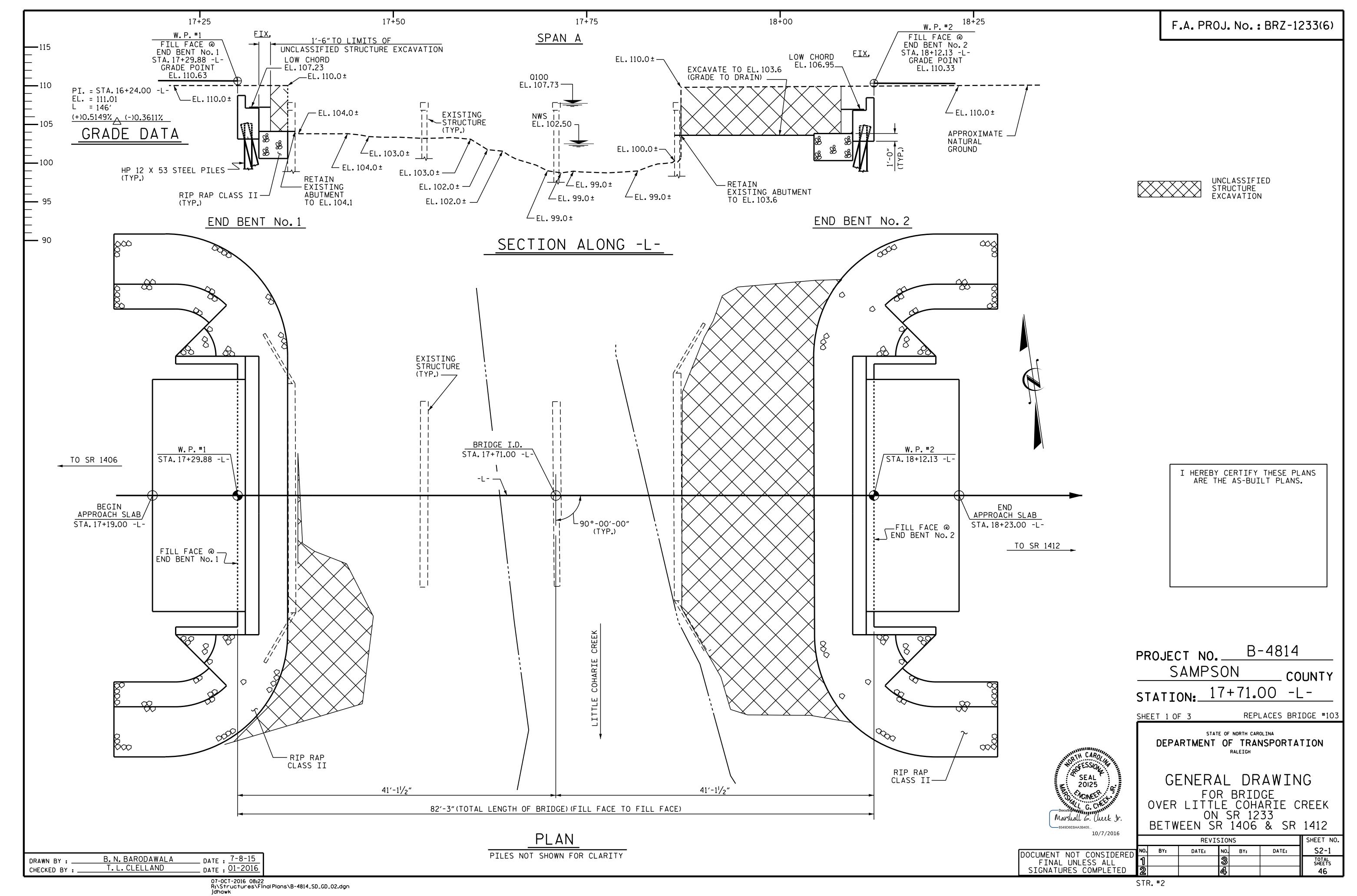
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2 4 46

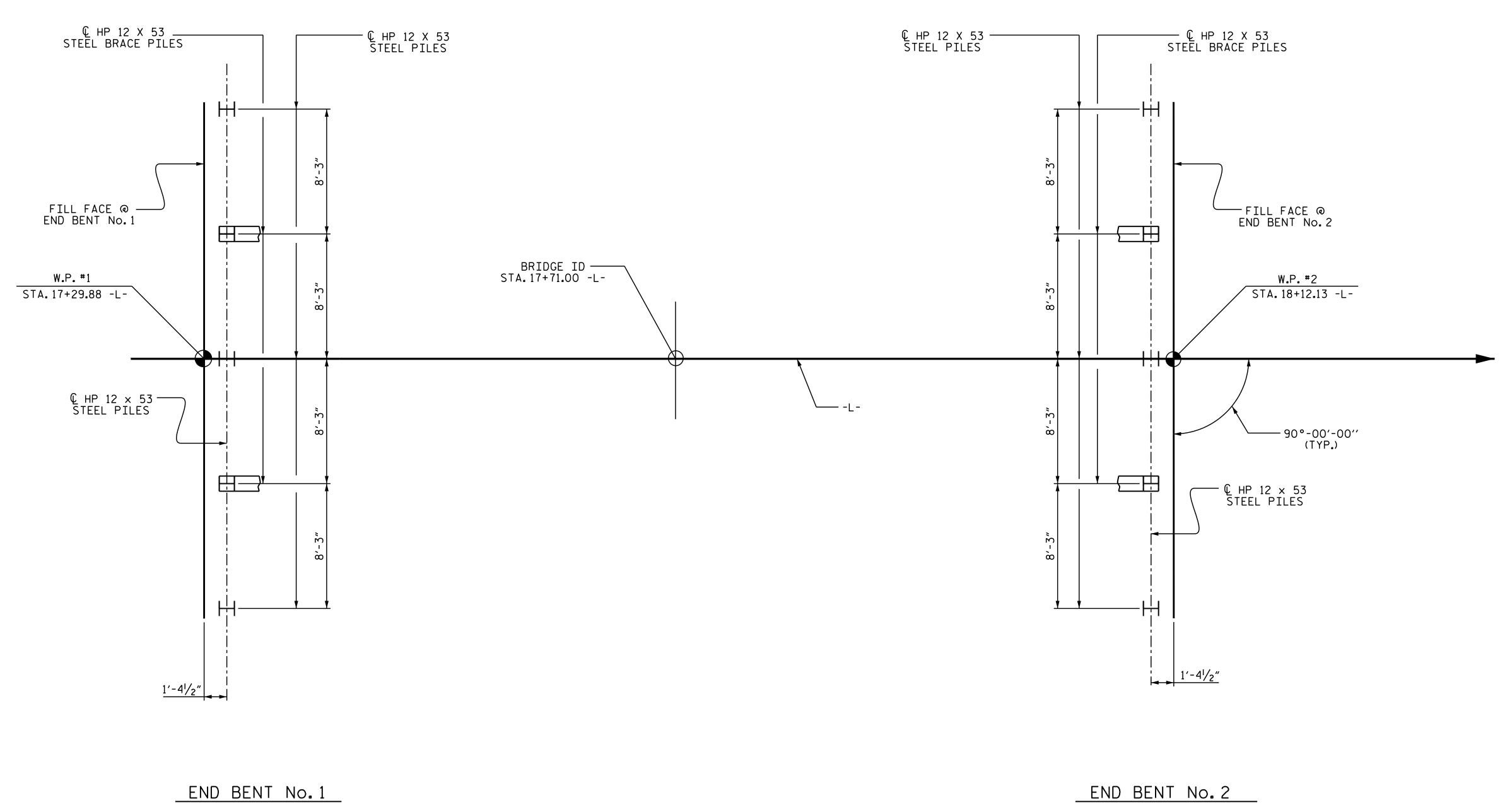
STR. #1

10/7/2016

ASSEMBLED BY: R. CAREATHERS/MP DATE: 6/1/15 CHECKED BY: M. PISO DATE: 6/9/15







FOUNDATION LAYOUT

(DIMENSIONS LOCATING PILES ARE SHOWN TO CENTERLINE OF PILES) (BRACE PILES AT END BENTS ARE BATTERED @ 3:12)

NOTES

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT No.1 AND END BENT No.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE.

DRIVE PILES AT END BENT No.1 AND END BENT No.2 TO A REQUIRED DRIVING RESISTANCE OF 200 TONS PER PILE.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 50,000 TO 60,000 FT-LBS.PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT No.1 AND END BENT No.2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

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SHEET 2 OF 3

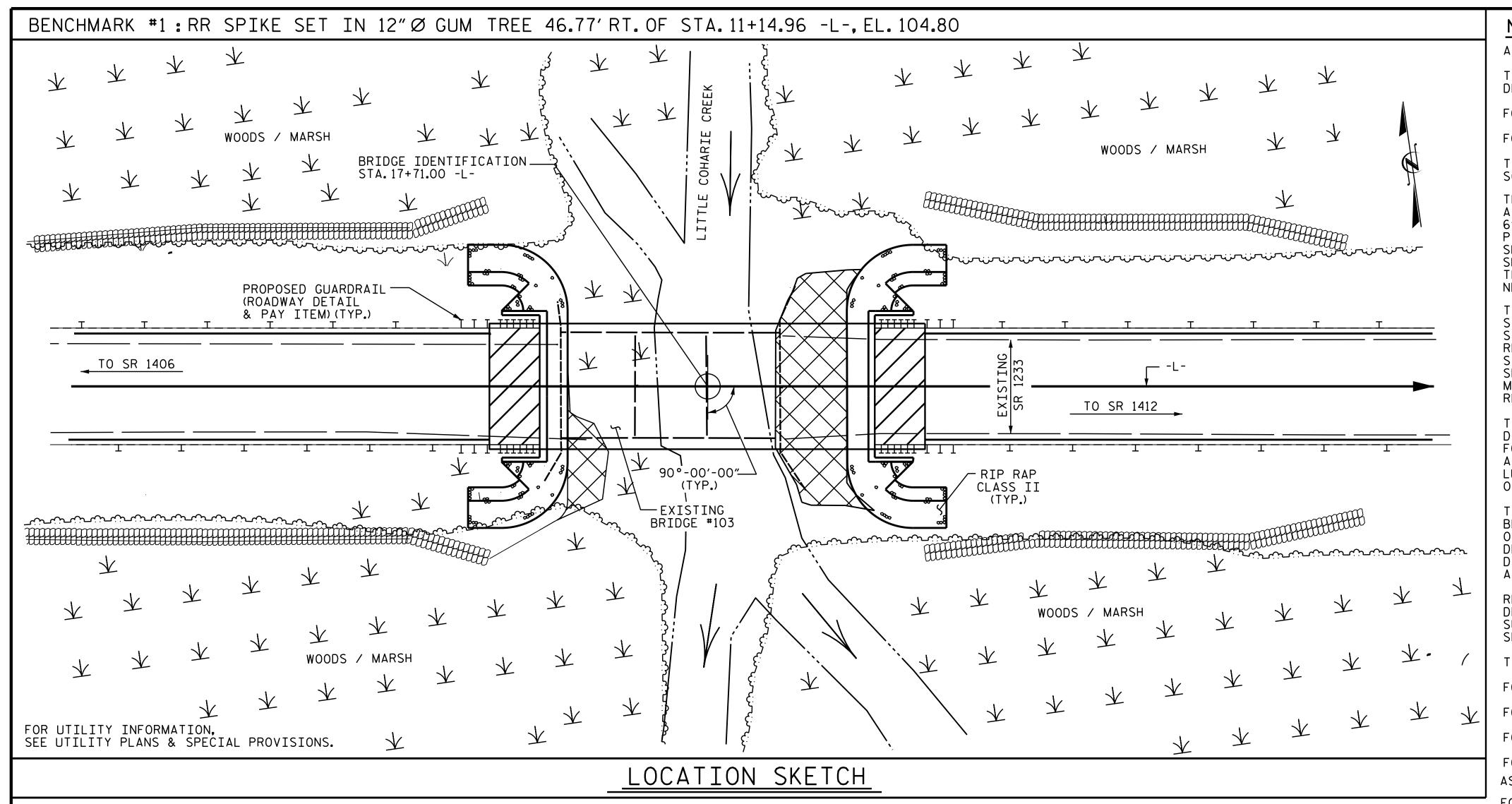
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING FOR BRIDGE OVER LITTLE COHARIE CREEK ON SR 1233 BETWEEN SR 1406 & SR 1412

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6549D6EBAA3B405				
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REVISIONS SHEET NO. S2-2 DATE: NO. BY: DATE: TOTAL SHEETS 46

DRAWN BY :	B.N.BARODAWALA/MP	DATE: 7-8-15
CHECKED BY :	T.L.CLELLAND	DATE : 01-2016



HYDRAULIC DATA

DESIGN DISCHARGE = 3300 C.F.S.
FREQUENCY OF DESIGN FLOOD = 25 YRS.
DESIGN HIGH WATER ELEVATION = 106.70
DRAINAGE AREA = 72.8 SQ. MI.

BASE DISCHARGE (Q100) = 4900 C.F.S. BASE HIGH WATER ELEVATION = 107.73

OVERTOPPING DATA

OVERTOPPING DISCHARGE = 6000 C.F.S. FREQUENCY OF OVERTOPPING FLOOD = 200+ YRS. OVERTOPPING FLOOD ELEVATION = 108.60

					— Т	OTAL B	IL	L OF	MATER	RIAL—			•			
	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP STE	12 × 53 EL PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PI	'-0" x 2'-9" RESTRESSED CONCRETE BOX BEAMS	ASBESTOS ASSESSMENT
	LUMP SUM	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	EACH	LIN.FT.	TONS	SQ.YDS.	LUMP SUM	NO.	LIN.FT.	LUMP SUM
SUPERSTRUCTURE					LUMP SUM					160.00			LUMP SUM	10	800.00	LUMP SUM
END BENT No.1			LUMP SUM	23.7		3342	5	350	3		115	128				
END BENT No. 2			LUMP SUM	23.7		3342	5	350	3		132	147				
TOTAL	LUMP SUM	1	LUMP SUM	47.4	LUMP SUM	6684	10	700	6	160.00	247	275	LUMP SUM	10	800.00	LUMP SUM

B. N. BARODAWALA/MP

T.L.CLELLAND

DATE: 7-8-15

O1-2016

DRAWN BY

CHECKED BY :

07-0CT-2016 08:22 R:\Structures\FinalPlans\B-4814_SD_GD_02.dgn idbawk

NOTES:

ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES".

THE EXISTING STRUCTURE CONSISTING OF 3 SPANS (1 @ 17'-9",1 @ 17'-2") (1 @ 17'-8") WITH A CLEAR ROADWAY WIDTH OF 24'+0" AND A REINFORCED CONCRETE DECK ON 19 LINES OF 6' X 13'/2" TIMBER JOISTS AND END BENTS AND BENTS CONSIST OF TIMBER CAPS AND TIMBER PILES. THE EXISTING STRUCTURE, LOCATED AT THE SITE OF THE PROPOSED BRIDGE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT.RIGHT OF THE CENTERLINE ROADWAY AT END BENT No.1 AND FOR A DISTANCE OF 30 FT.EACH SIDE OF THE CENTERLINE ROADWAY AT END BENT No.2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITES, SEE SPECIAL PROVISIONS.

PROJECT NO. B-4814

SAMPSON COUNTY

STATION: 17+71.00 -L-

SEAL 20125

Shall G. Change Shall G. Check Jr. DGEBAA3B405... 10/7/2016 SHEET 3 OF 3

DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

FOR BRIDGE OVER
LITTLE COHARIE CREEK
ON SR 1233
BETWEEN SR 1406 & SR 1412

REVISIONS

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REVISIONS

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STR.#2

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

									SIRE	NGIH	I LIN	11 ST	AIL				SE	RVICE	I I I	$\Gamma T M T$	I STA	IL	
									MOMENT					SHEAR						MOMENT			
LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVELOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	COMMENT NUMBER
	HL-93(Inv)	N/A	<u>\(1 \)</u>	1.155		1.75	0.273	1.72	А	EL	39 . 25	0.502	1 . 51	А	EL	7.85	0.80	0.273	1.15	А	EL	39.25	
DESIGN	HL-93(0pr)	N/A		1.958		1.35	0.273	2.23	Α	EL	39.25	0.502	1.96	Α	EL	7.85	N/A						
LOAD RATING	HS-20(Inv)	36.000	(2)	1.533	55.181	1.75	0.273	2.28	Α	EL	39.25	0.502	1.91	Α	EL	7.85	0.80	0.273	1.53	Α	EL	39.25	
<u> </u>	HS-20(0pr)	36.000		2.473	89.021	1.35	0.273	2.96	А	EL	39.25	0.502	2.47	А	EL	7.85	N/A						
	SNSH	13.500		3.509	47.376	1.40	0.273	6 . 53	А	EL	39.25	0.502	5.73	А	EL	7.85	0.80	0.273	3 . 51	А	EL	39 . 25	
Т	SNGARBS2	20.000		2.594	51.880	1.40	0.273	4 . 82	А	EL_	39.25	0.502	4.06	А	EL	7.85	0.80	0.273	2.59	А	EL	39 . 25	
HICL	SNAGRIS2	22.000		2.448	53.850	1.40	0.273	4.55	А	EL_	39.25	0.502	3.76	А	EL	7.85	0.80	0.273	2.45	А	EL	39 . 25	
V) VEHIC	SNCOTTS3	27.250		1.746	47.571	1.40	0.273	3.25	Α .	EL 	39.25	0.502	2.86	A .	EL	7.85	0.80	0.273	1.75	Α .	EL	39.25	
INGLE	SNAGGRS4	34.925		1.451	50.667	1.40	0.273	2.70	A .	EL 	39.25	0.502	2.36	A .	EL -	7.85	0.80	0.273	1.45	Α	EL	39.25	
Sir	SNS5A	35.550		1.419	50.453	1.40	0.273	2.64	A	EL	39.25	0.502	2.38	A	EL	7.85	0.80	0.273	1.42	Α .	EL	39.25	
	SNS6A	39.950		1.299	51.885	1.40	0.273	2.42	A	EL	39.25	0.502	2.17	A	EL	7.85	0.80	0.273	1.30	Α	EL	39.25	
LEGAL	SNS7B	42.000		1.237	51.941	1.40	0.273	2.30	A	EL	39.25	0.502	2.13	A A	EL	7.85	0.80	0.273	1.24	Α	EL	39.25	
RATING	TNAGRIT3 TNT4A	33.000		1.583	52.231	1.40	0.273	2.94 2.96	A A	EL	39.25	0.502	2 . 59	A A	EL	7.85 7.85	0.80	0.273	1.58	Α	EL	39.25	
-TRA	TNT6A	33 . 075		1.589 1.296	52 . 550 53 . 907	1.40	0.273	2.96	A A	EL EL	39 . 25	0 . 502	2.25	A A	EL EL	7.85	0.80	0.273 0.273	1.59 1.30	Α	EL EL	39 . 25	
SEMI	TNT7A	42.000		1.301	54.625	1.40	0.273	2.42	٨	EL	39.25	0.502	2.23	Α Λ	EL	7.85	0.80	0.273	1.30	А А	EL	39.25	
l	TNT7B	42.000		1.341	56.333	1.40	0.273	2.49	Δ	EL	39.25	0.502	2.08	Δ	EL	7.85	0.80	0.273	1.34	Α	EL	39.25	
RAILE	TNAGRIT4	43.000		1.279	55.001	1.40	0.273	2.38	Δ	EL	39.25	0.502	2.02	Δ	EL	7.85	0.80	0.273	1.28	A	EL	39.25	
I F	TNAGT5A	45.000		1.207	54.337	1.40	0.273	2.25	Δ	EL	39.25	0.502	2.00	Δ	EL	7.85	0.80	0.273	1.21	A	EL	39.25	
TRUCK	TNAGT5B	45.000	3	1.194	53.739	1.40	0.273	2.22	Δ	EL	39.25	0.502	1.92	Δ	EL	7.85	0.80	0.273	1.19	Δ	EL	39.25	

LOAD FACTORS:



STRENGTH I 1.25 1.50 SERVICE III 1.00 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-4814 SAMPSON ___ COUNTY STATION: 17+71.00 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> > STANDARD

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

LRFR SUMMARY FOR 80'BOX BEAM UNIT 90°-00'-00"SKEW (NON-INTERSTATE TRAFFIC)

REVISIONS SHEET NO. S2-4 DATE: NO. BY:

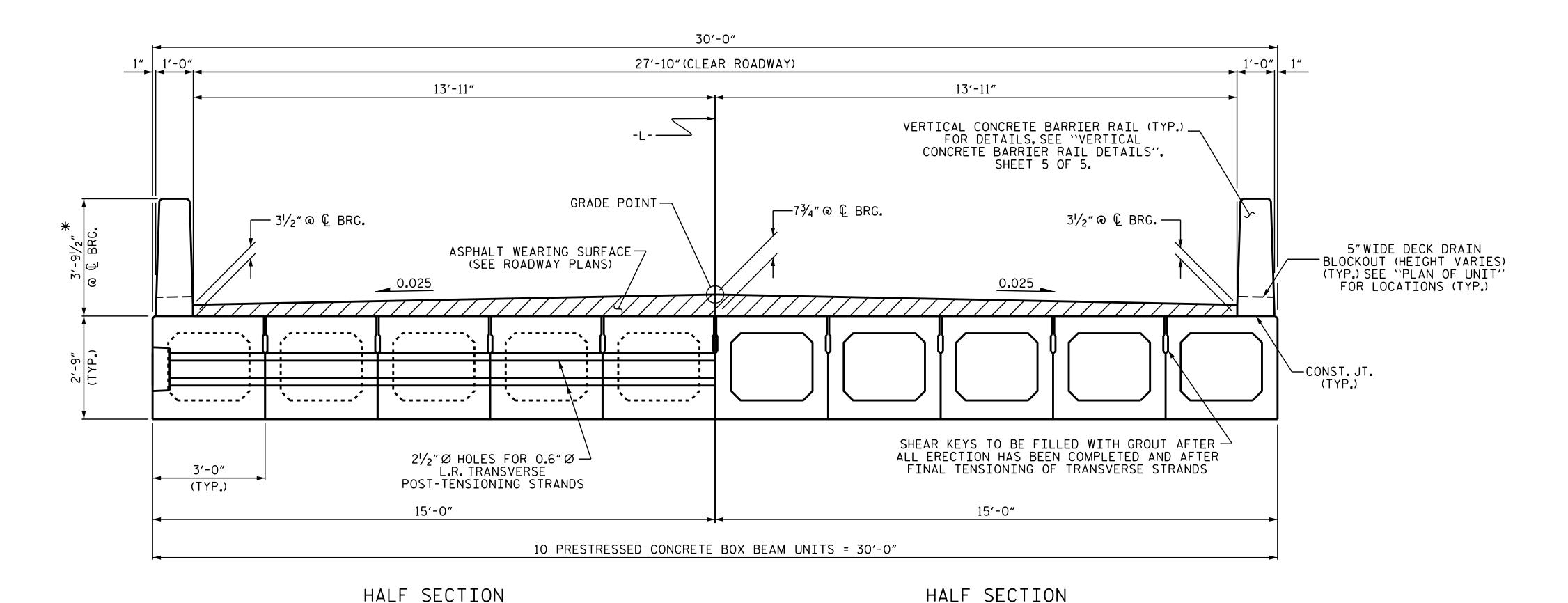
STD. NO. 33LRFR1_90S_80L

LRFR SUMMARY

FOR SPAN A

ASSEMBLED BY: R. CAREATHERS/MP DATE: 6/3/15 CHECKED BY: M. PISO DATE: 6/17/15

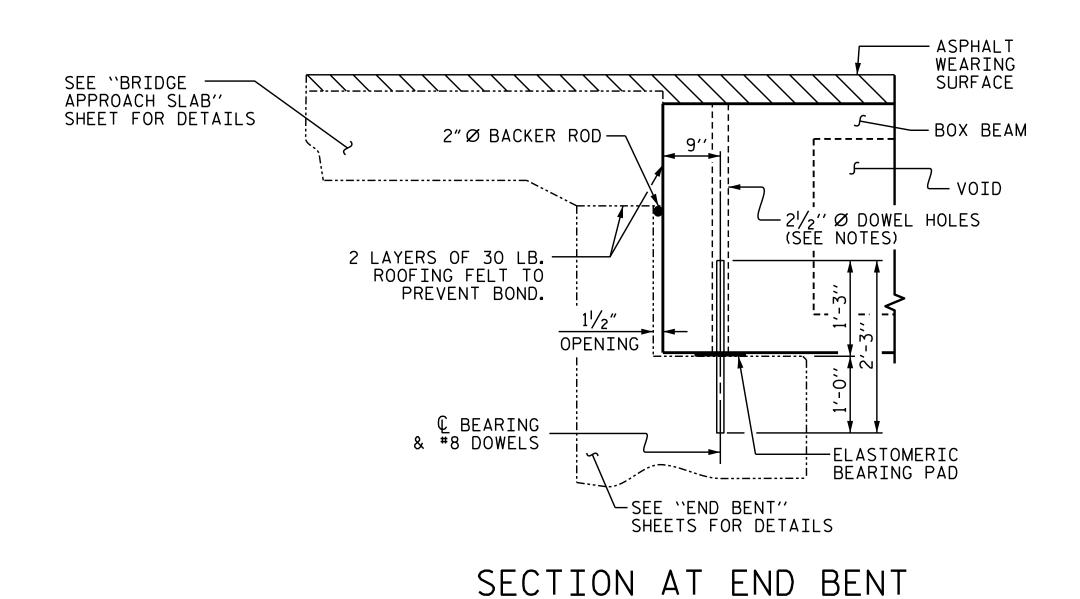
DRAWN BY: TMG II/II CHECKED BY : AAC II/II



TYPICAL SECTION

* THE MAXIMUM VERTICAL CONCRETE BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE VERTICAL CONCRETE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE VERTICAL CONCRETE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE.FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION DETAILS", SHEET 5 OF 5.

FIXED END



AT INTERMEDIATE DIAPHRAGMS

ASSEMBLED BY : R. CAREATHERS/MP DATE : 6/3/15

DATE: 6/17/15

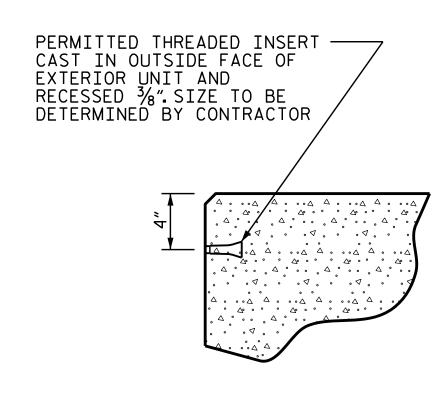
REV. 8/14

MAA/TMG

CHECKED BY : M. PISO

DRAWN BY : DGE 8/II

CHECKED BY : TMG II/II



THROUGH VOIDS

THREADED INSERT DETAIL

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ \emptyset DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6000 PSI.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 5"X 4". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE BOX BEAM UNIT TO THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR BOX BEAM UNITS THAT REQUIRE DRAINS IN THE VERTICAL CONCRETE BARRIER RAIL.

SHEET 1 OF 5

PROJECT NO. B-4814 SAMPSON COUNTY STATION: 17+71.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

SEAL 20125

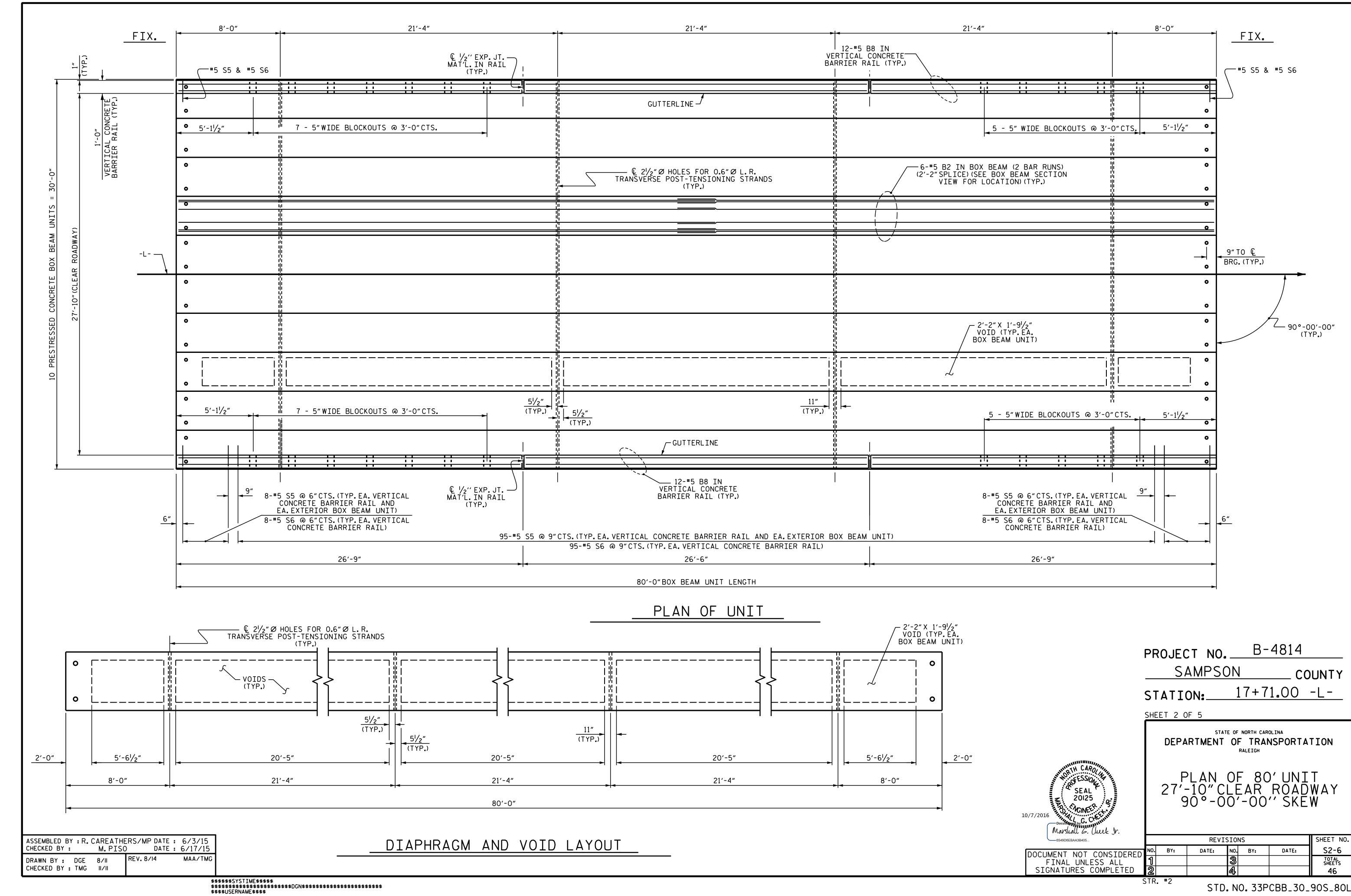
Marshall G. Check Ir.

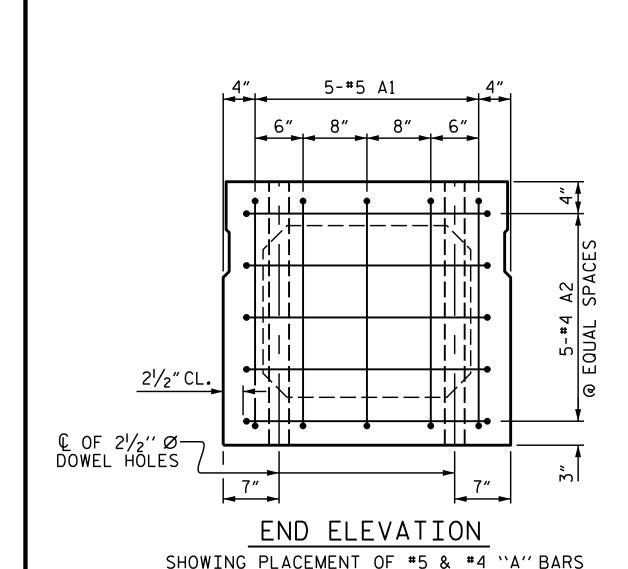
OCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

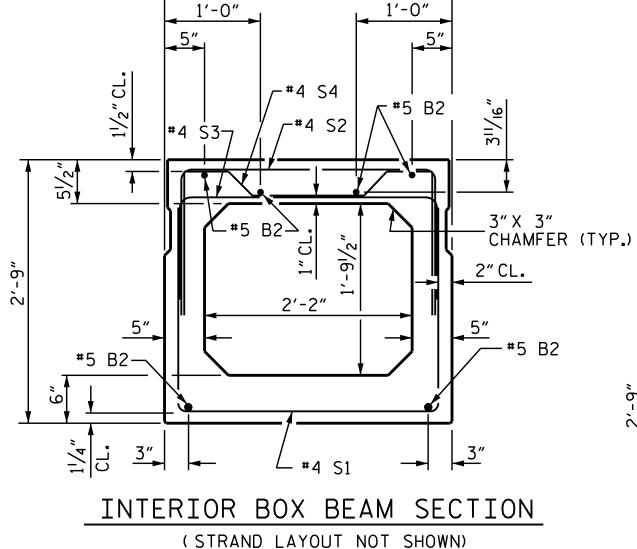
STANDARD 3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT

SHEET NO. **REVISIONS** S2-5 NO. BY: DATE: BY: DATE: TOTAL SHEETS 46

STD. NO. STD.33PCBB1_30







CL. #5 S5 — #4 S2¬ #4 S47 #4 S37 - #5 B2 — CHAMFER (TYP.) 2"CL. 2'-2" #5 B2→ ∕− **#**5 B2

EXTERIOR BOX BEAM SECTION

(STRAND LAYOUT NOT SHOWN)

3'-0"

2 SPA @

0.6" Ø LOW RELAXATION STRAND LAYOUT

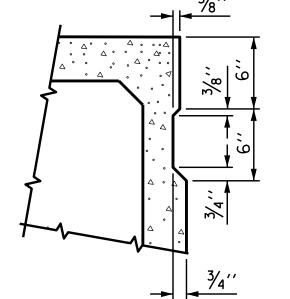
TYPICAL STRAND LOCATION (24 STRANDS REQUIRED)

STRANDS DEBONDED FOR 4'-O"FROM END OF UNIT

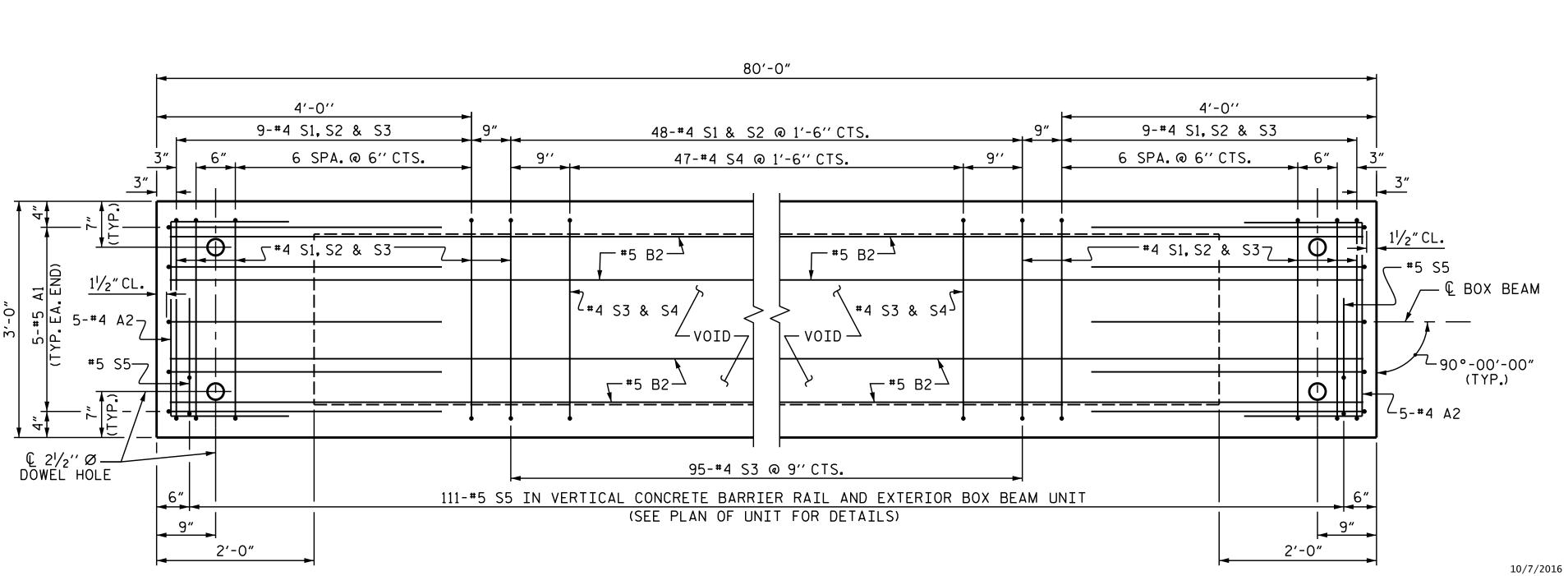
STRANDS DEBONDED FOR 10'-0" FROM END OF UNIT

OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE BOX BEAM UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL COST.

BOND SHALL BE BROKEN ON STRANDS AS SHOWN FOR THE SPECIFIED LENGTH FROM EACH END OF THE BOX BEAM. SEE STANDARD SPECIFICATIONS ARTICLE 1078-7.



GRADE 270	STRANDS
	0.6"Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43 , 950



PROJECT NO. B-4814 SAMPSON COUNTY 17+71.00 -L-STATION:

SHEET 3 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT

REVISIONS SHEET NO. S2-7 DATE: DATE: BY: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL TOTAL SHEETS SIGNATURES COMPLETED STR. #2

STD. NO. 33PCBB4_90S_80L

PLAN OF BOX BEAM

EXTERIOR UNIT SHOWN, INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S5 BARS.

FOR LOCATION OF DIAPHRAGMS, SEE "PLAN OF UNIT".

FOR THREADED INSERTS, SEE "THREADED INSERT DETAIL". FOR REINFORCING STEEL IN DIAPHRAGMS, SEE "DOUBLE DIAPHRAGM DETAILS".

SHOWING PLACEMENT OF #5 & #4 "A" BARS AND LOCATION OF DOWEL HOLES. (INTERIOR BOX BEAM SECTION SHOWN-EXTERIOR SECTION SIMILAR EXCEPT SHEAR KEY LOCATION. STRAND LAYOUT NOT SHOWN.)

SHEAR KEY DETAIL NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR BOX BEAMS.

70

127

512

49

14

250

183

695

LBS.

14.2 CU. YDS.

LBS.

6′-8″

5'-7"

40'-11"

6′-2"

2'-7"

7′-6″

5′-8″

4′-10″

5′-10″

--

No. 24

70

512

49

14

250

183

--

1901 LBS.

14.1 CU. YDS.

127

ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL FOR ONE BOX BEAM SECTION

BAR | NUMBER | SIZE | TYPE | LENGTH | WEIGHT | LENGTH | WEIGHT

5′-7"

6′-2"

2′-7″

5′-8"

4′-10″

5′-10″

6′-0″

No. 24

#5 | STR | 40'-11"

STR

4

BAR TYPES

1'-6"

2

1'-6"

3'-6"

1'-0"

#5

#4

#4

#4

#4

#4

#5

* EPOXY COATED REINF. STEEL 695

10

K1

S2

S3 S4

* S5 | 111

REINFORCING STEEL

8000 P.S.I. CONCRETE

0.6" Ø L.R. STRANDS

K2

10"

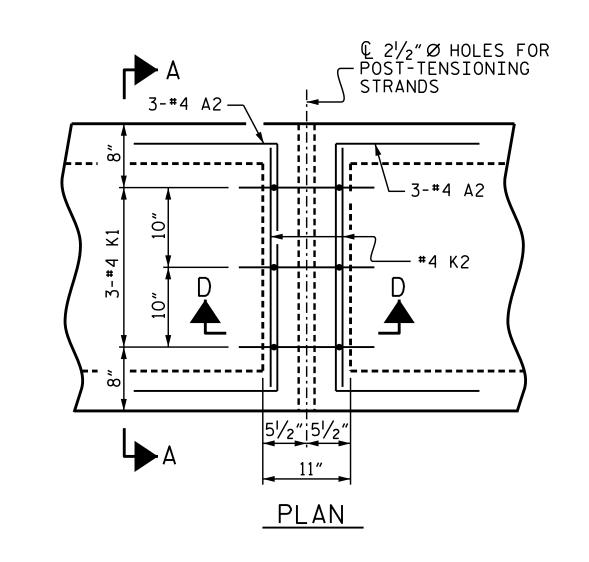
THIS LEG AT

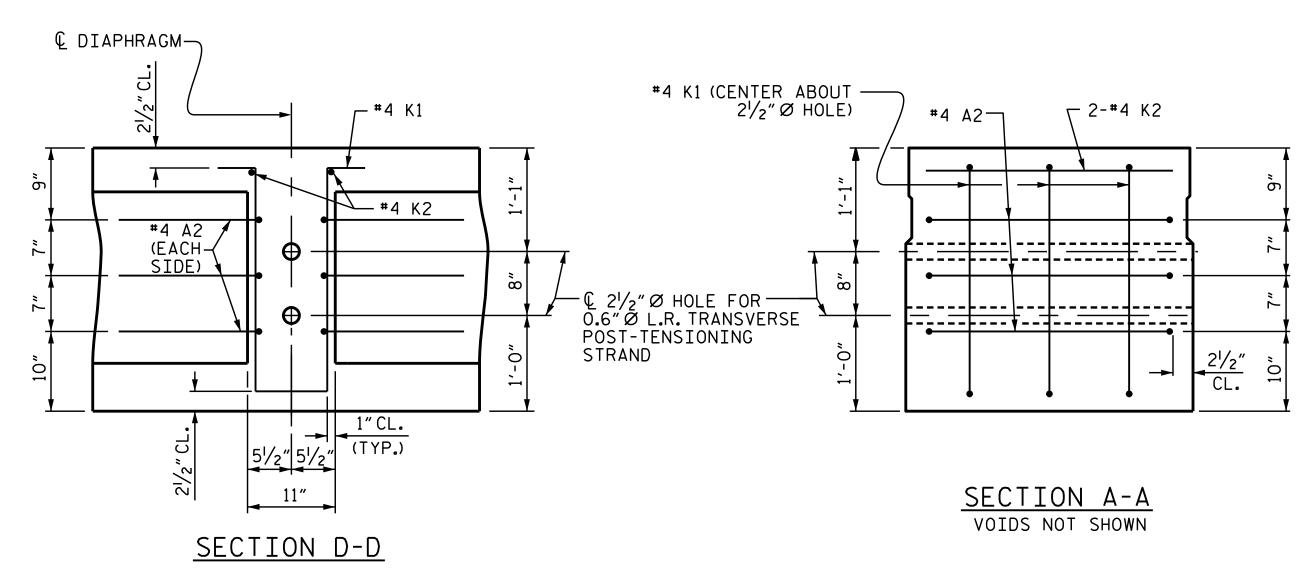
S1 S2 S3

DEBONDING LEGEND

FULLY BONDED STRANDS

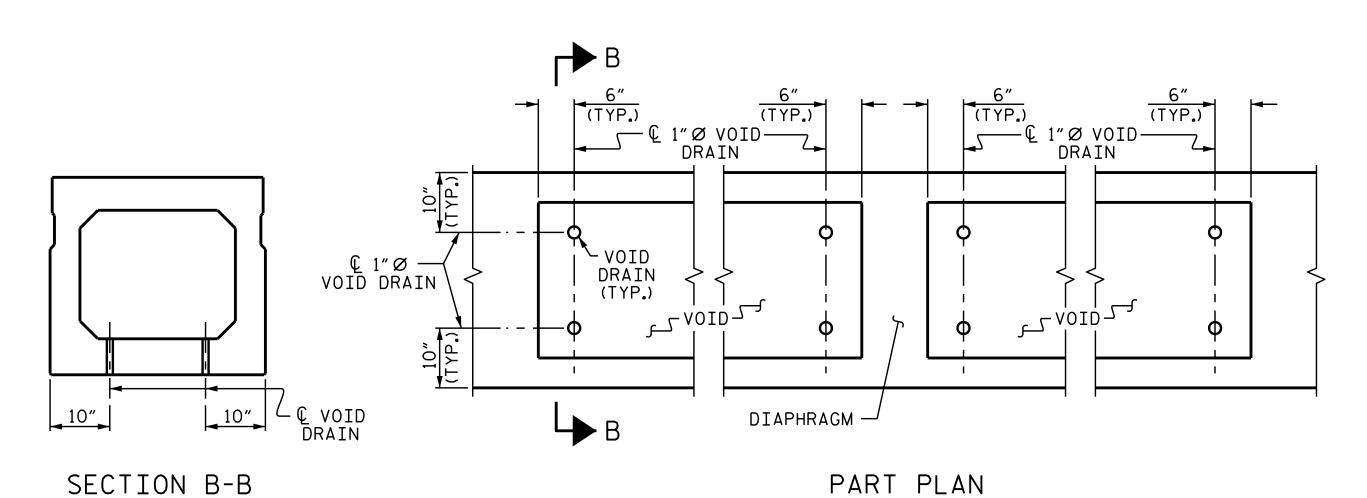
ASSEMBLED BY : R. CAREATHERS/MP DATE : 6/3/15 CHECKED BY: M. PISO DATE: 6/17/15 REV. 9/14 MAA/TMG DRAWN BY : DGE IO/II CHECKED BY : TMG II/II





DOUBLE DIAPHRAGM DETAILS

#4 "S" BARS NOT SHOWN. #4 "S" BARS MAY BE SHIFTED SLIGHTLY TO CLEAR $2\frac{1}{2}$ " Ø HOLE.

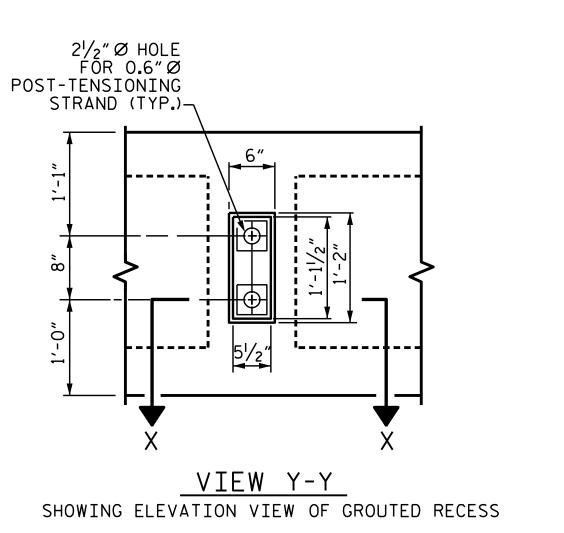


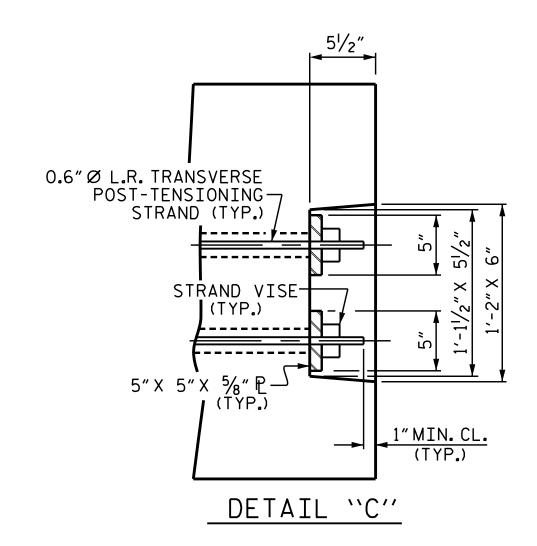
VOID DRAIN DETAILS

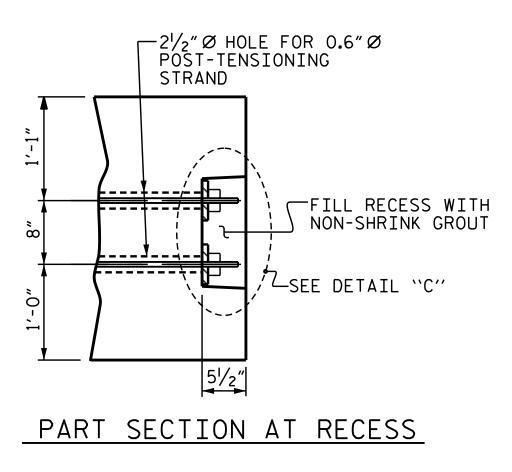
(DIMENSIONS SHOWN ARE TYPICAL FOR EACH VOID)

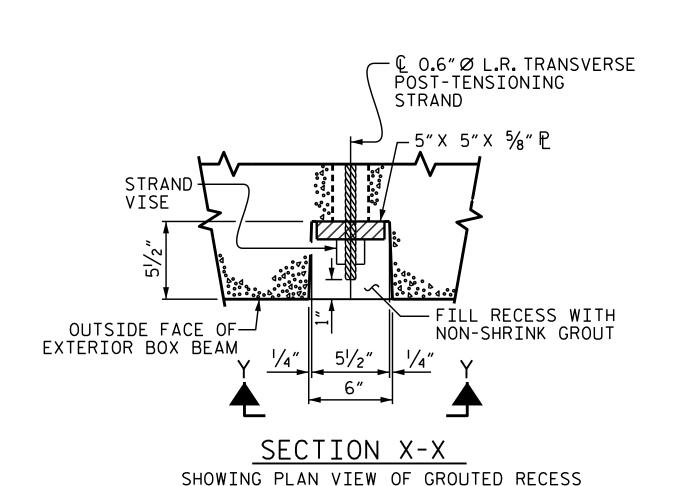
DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-9"
80'BOX BEAM UNIT	0.6″∅ L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1¾″ ∤
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD***	1/2″ ♦
FINAL CAMBER	11/4″ ∮
JAK THELLINES CHITHDE WEADTHE SHOP	. A C E

** INCLUDES FUTURE WEARING SURFACE









GROUTED RECESS DETAIL AT END OF POST-TENSIONED STRANDS OF EXTERIOR BOX BEAM

> PROJECT NO. B-4814 SAMPSON COUNTY STATION: 17+71.00 -L-

SHEET 4 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 2'-9"

PRESTRESSED CONCRETE BOX BEAM UNIT

REVISIONS DATE: NO. BY: STR. #2

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

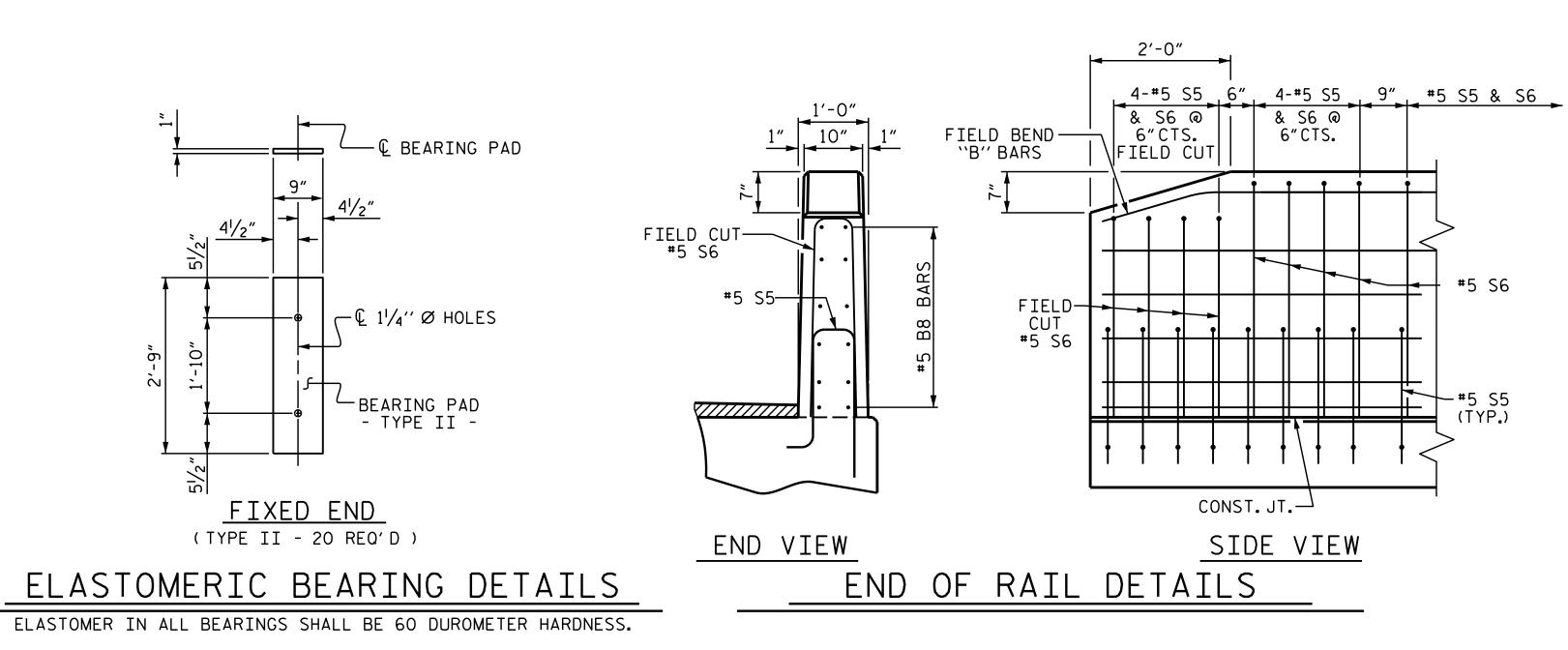
STD.NO.33PCBB5_90S

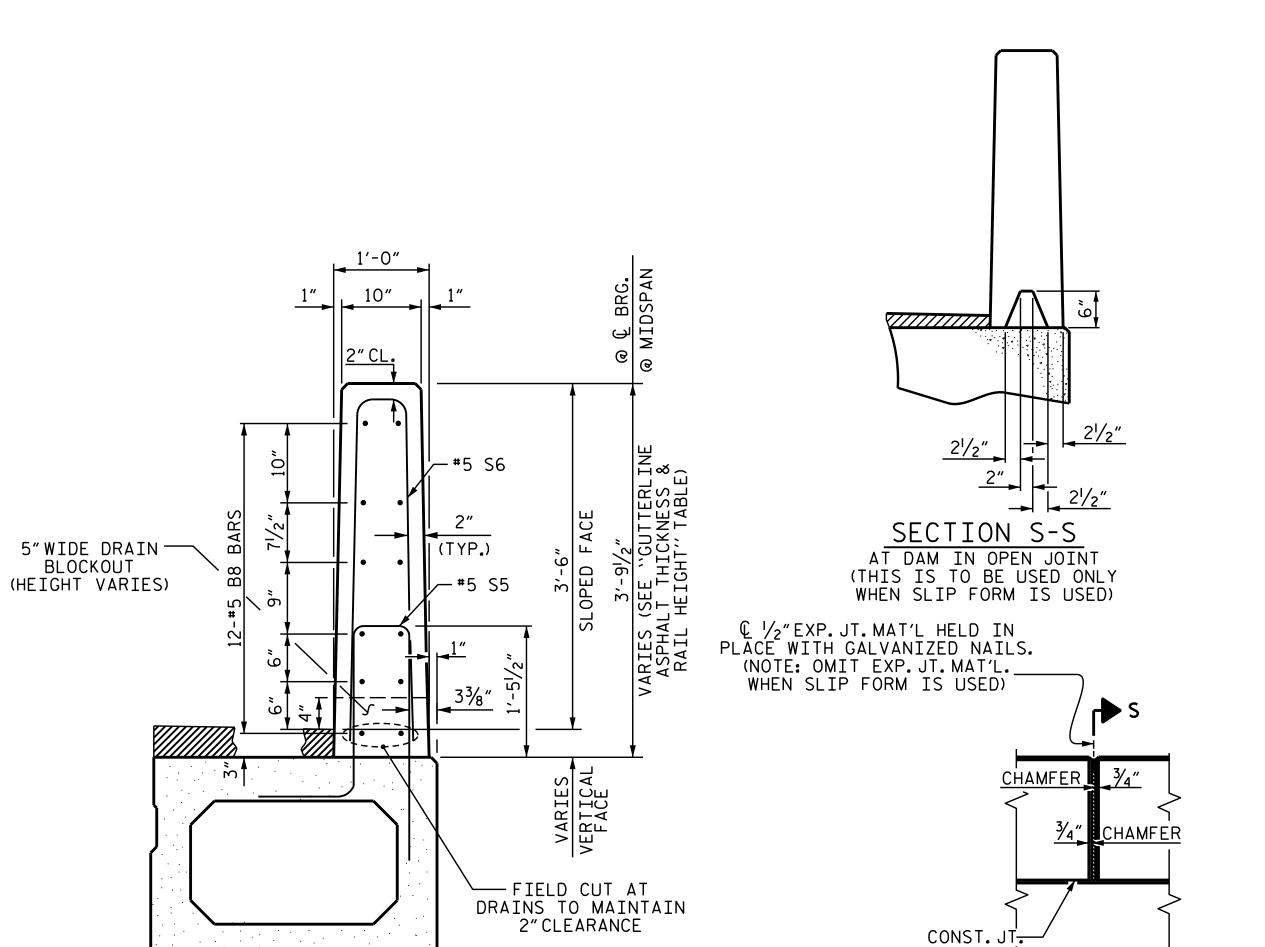
SHEET NO.

S2-8

TOTAL SHEETS

ASSEMBLED BY: R. CAREATHERS/MP DATE: 6/3/15 CHECKED BY: M. PISO DATE: 6/17/15 REV. 8/14 MAA/TMG DRAWN BY : DGE 10/11 CHECKED BY : TMG II/II



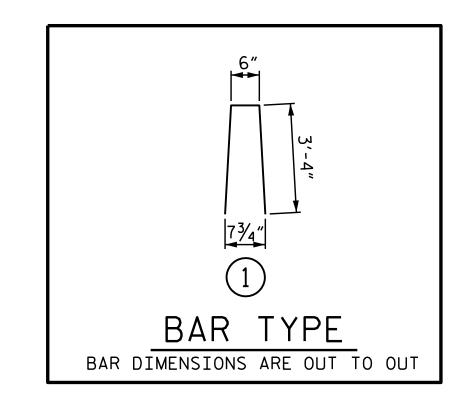


VERTICAL CONCRETE BARRIER RAIL DETAILS

ELEVATION AT EXPANSION JOINTS

ASSEMBLED BY : R. CAREATHERS/MP DATE : 6/3/15 CHECKED BY : M. PISO DATE : 6/17/15 REV. 4/15 DRAWN BY: DGE 10/11 CHECKED BY: TMG 11/11 MAA/TMG

SECTION THRU RAIL



BI	LL OF MATERIAL FOR VERTICAL CONCRE	TE B	ARR:	IER R	RAIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
	80' UNIT				
∗ B8	72	#5	STR	26'-3"	1971
* S6	222	#5	1	7′-2″	1659
* EPOX	Y COATED REINFORCING STEEL		LBS.		3630
CLASS	AA CONCRETE		CU.YDS.	-	20.7
TOTAL	VERTICAL CONCRETE BARRIER RAIL		LN.FT.		160.00

GUTTERLINE ASPH	ALT THICKNESS 8	RAIL HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
80' UNITS	21/4"	3′-81/4″

BOX BEA	M UN	NITS RE	QUIRED
	NUMBER	LENGTH	TOTAL LENGTH
EXTERIOR B.B.	2	80'-0"	160'-0"
INTERIOR B.B.	8	80'-0"	640'-0"
TOTAL	10		800′-0″

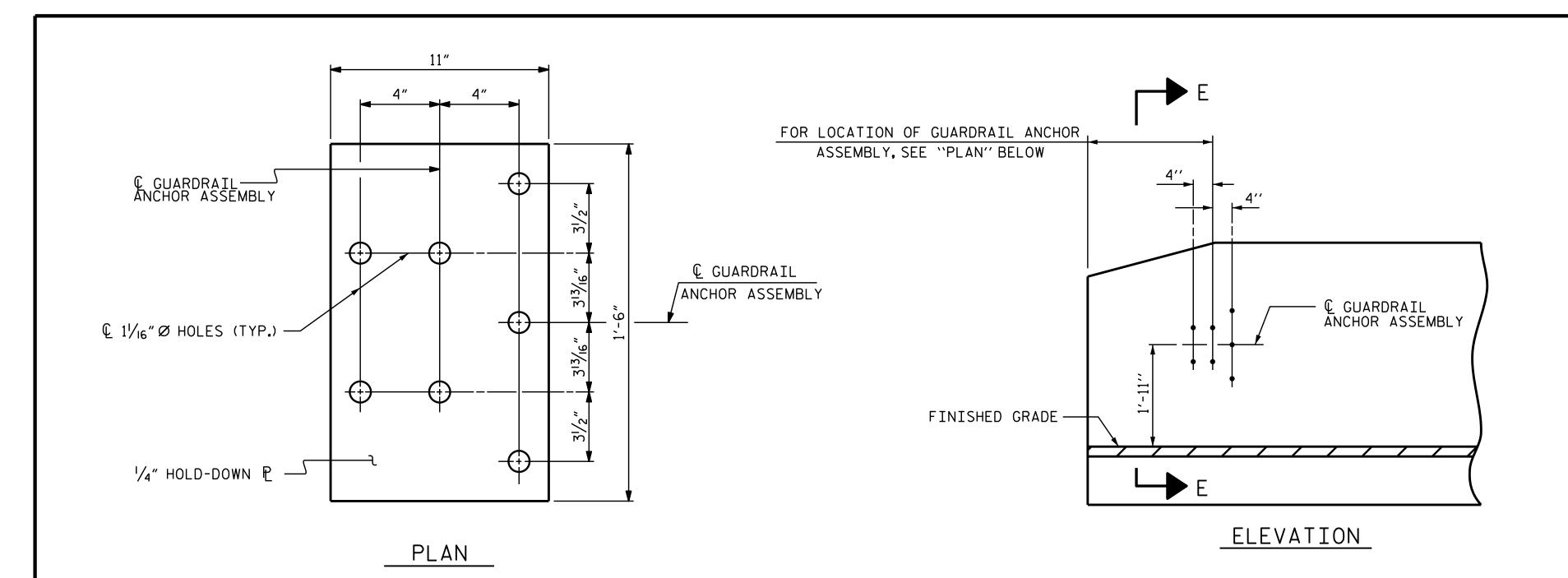
PROJECT NO. B-4814 SAMPSON COUNTY STATION: 17+71.00 -L-

SHEET 5 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT

A CANALI SO CANALI							
Marshalt Einthick Jr.			REVI	SION	IS		SHEET NO
6549D6EBAA3B405	NO.	BY:	DATE:	NO.	BY:	DATE:	S2-9
CUMENT NOT CONSIDERED FINAL UNLESS ALL	1			13			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			46
						•	



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

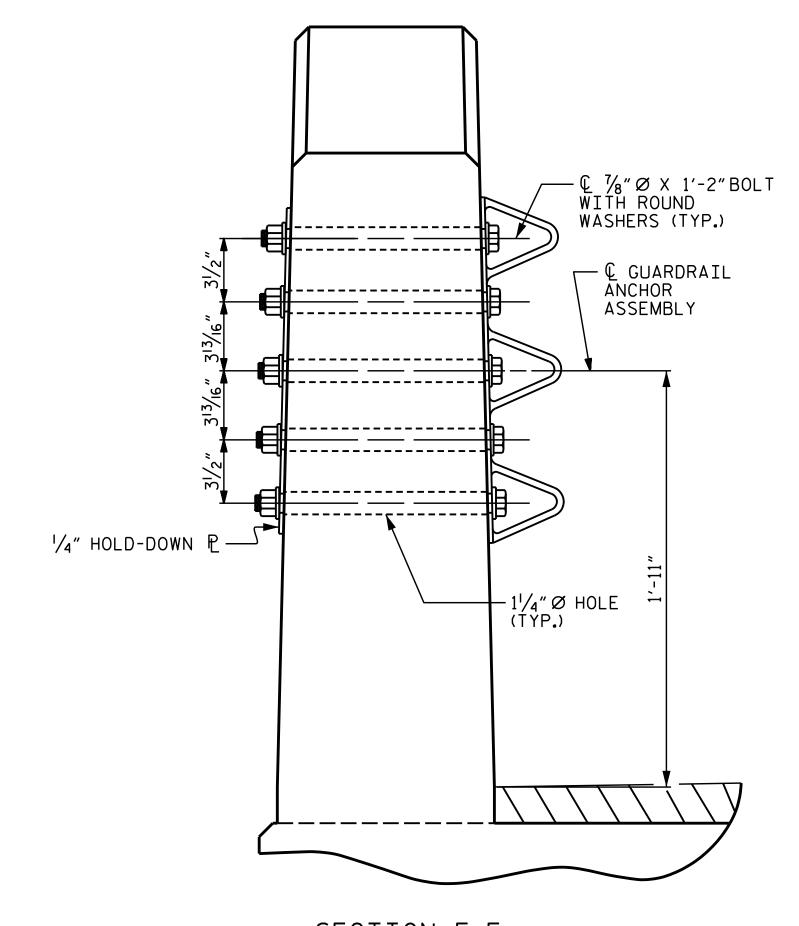
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

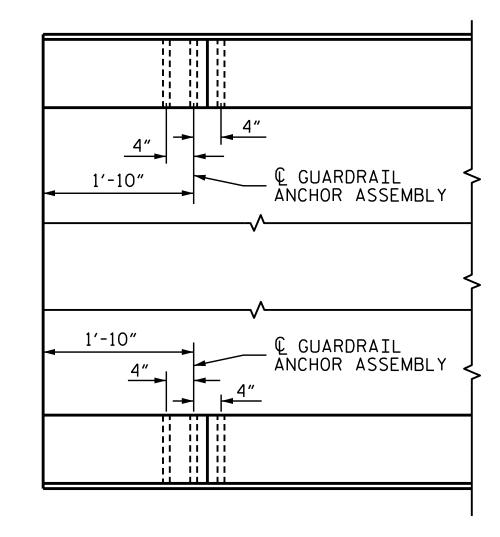
THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SECTION E-E

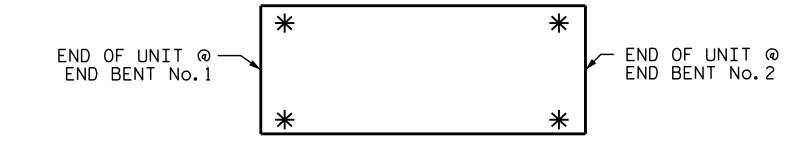
GUARDRAIL ANCHOR ASSEMBLY DETAILS



PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

END BENT No.1 SHOWN, END BENT No.2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-4814

SAMPSON COUNTY

STATION: 17+71.00 -L-



DEPARTMENT OF TRANSPORTATION

STANDARD

GUARDRAIL ANCHORAGE

DETAILS

FOR VERTICAL CONCRETE

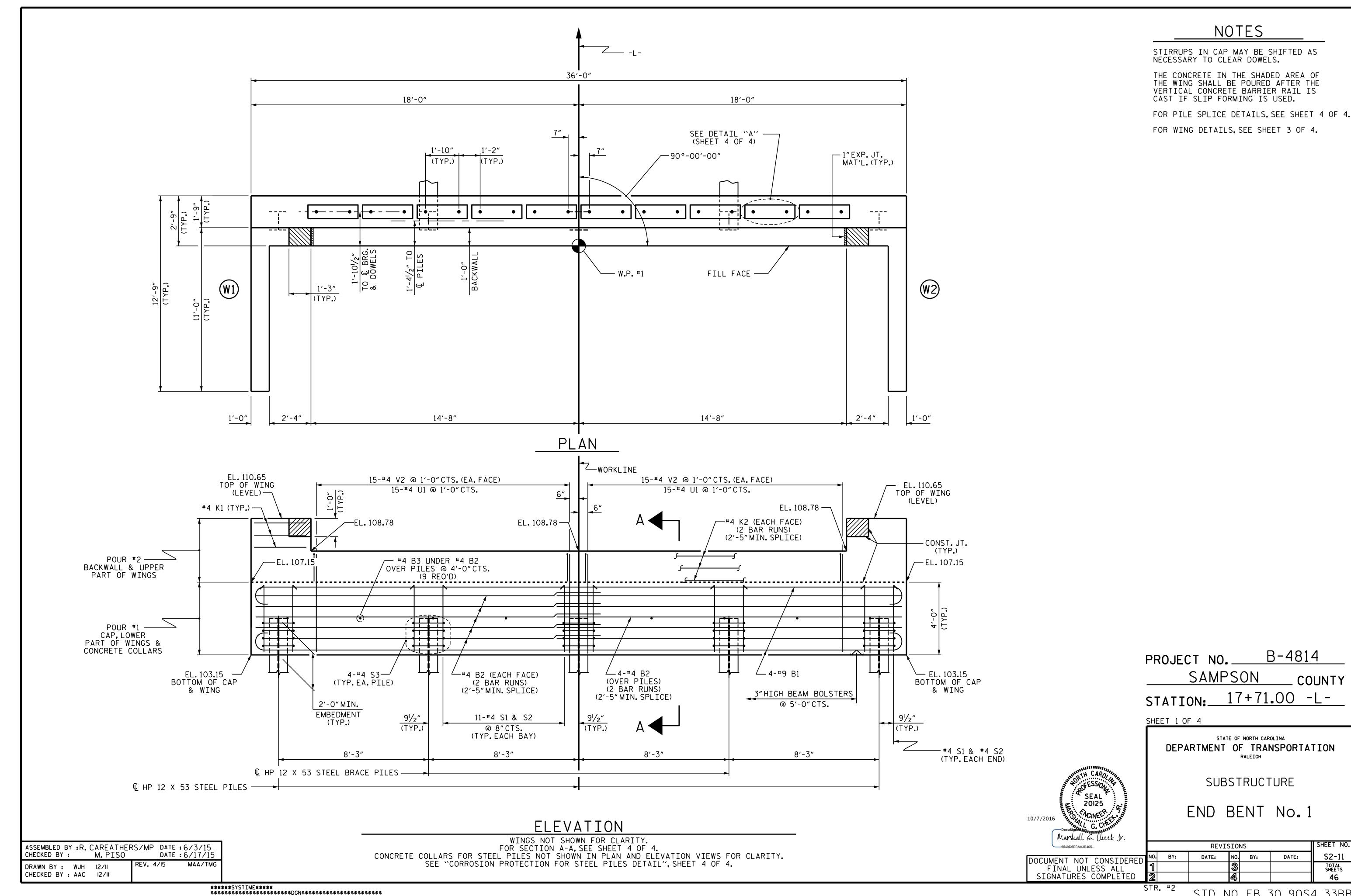
BARRIER RAIL

—6549D6EBAA3B405			REVIS	SIO	NS		SHEET NO.
CUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S2-10
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			46

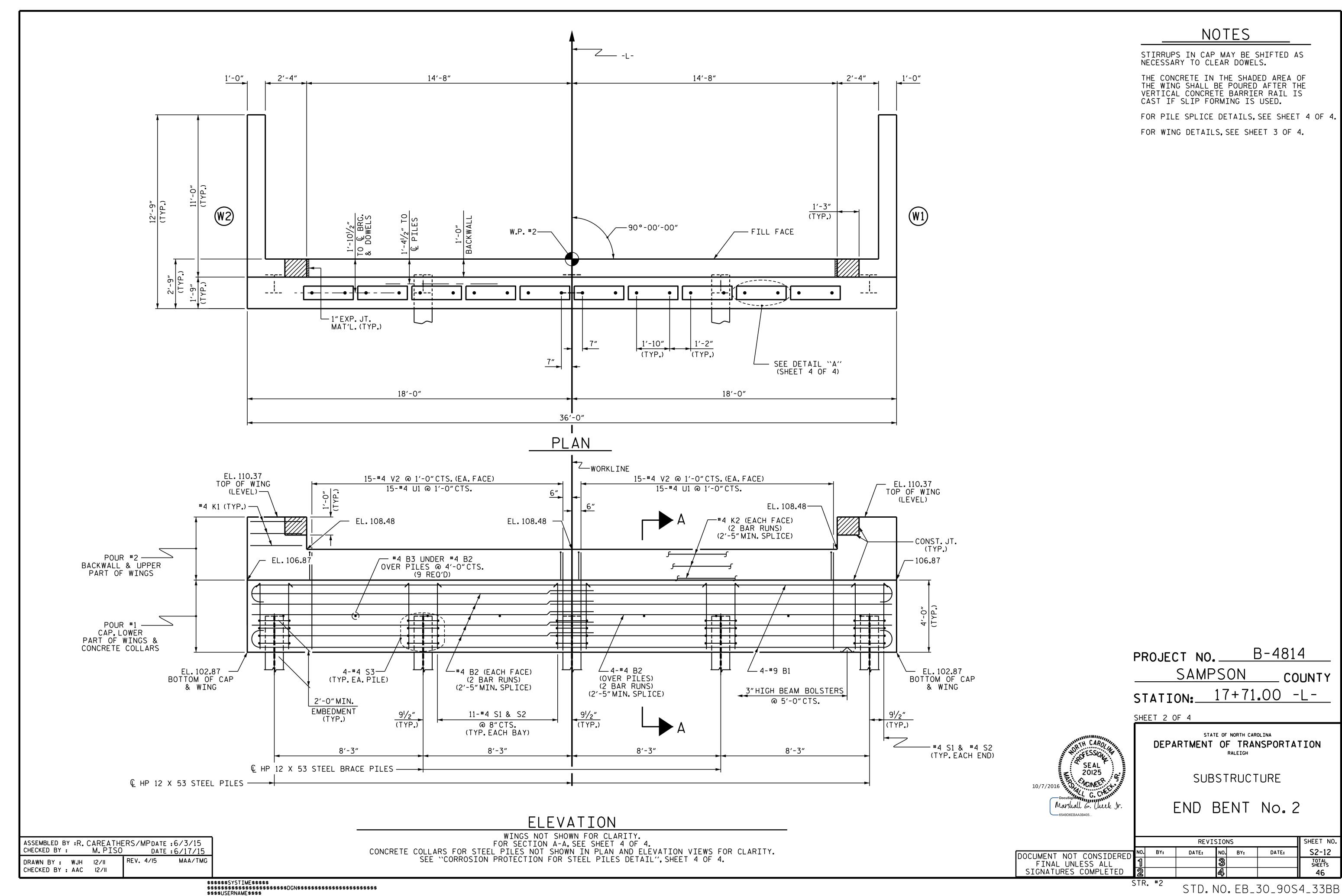
ASSEMBLED BY: R. CAREATHERS/MP DATE: 6/5/15 CHECKED BY: M. PISO DATE: 6/17/15

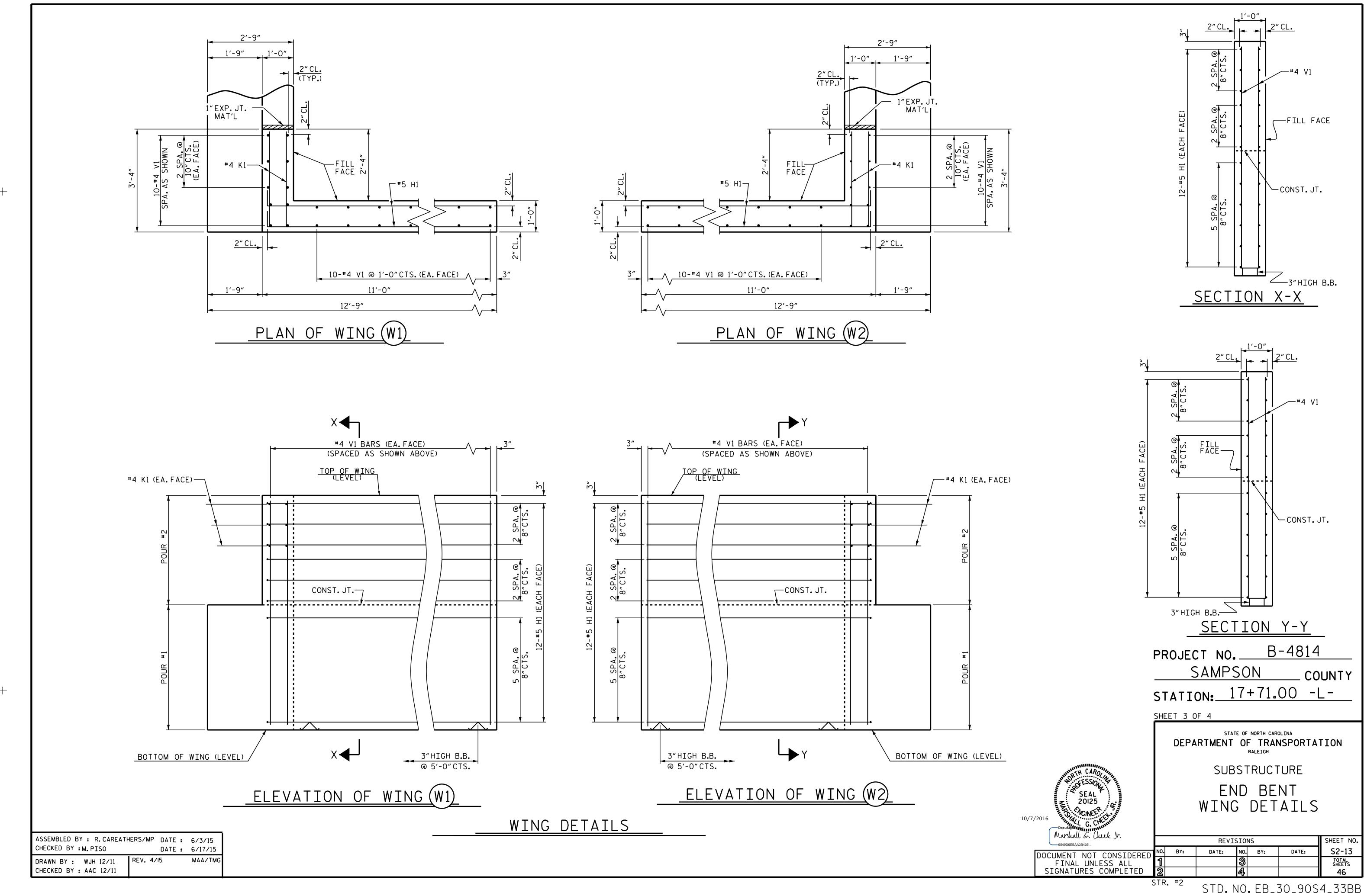
DRAWN BY: MAA 5/10 REV. 12/5/II MAA/GM REV. 6/13 MAA/GM REV. 1/15 MAA/TMG

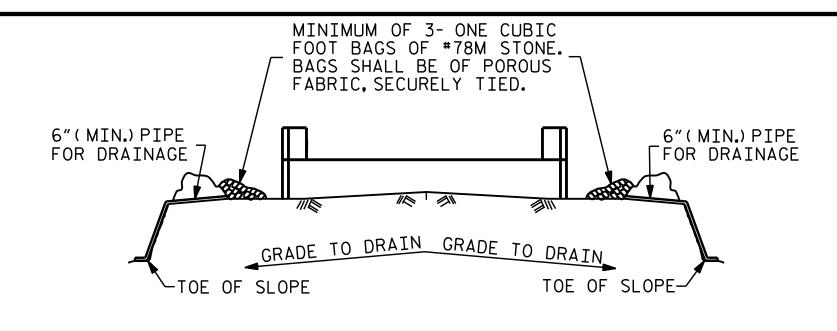
STR. #2 (SHT 1) STD. NO. GRA3



STD. NO. EB_30_90S4_33BB





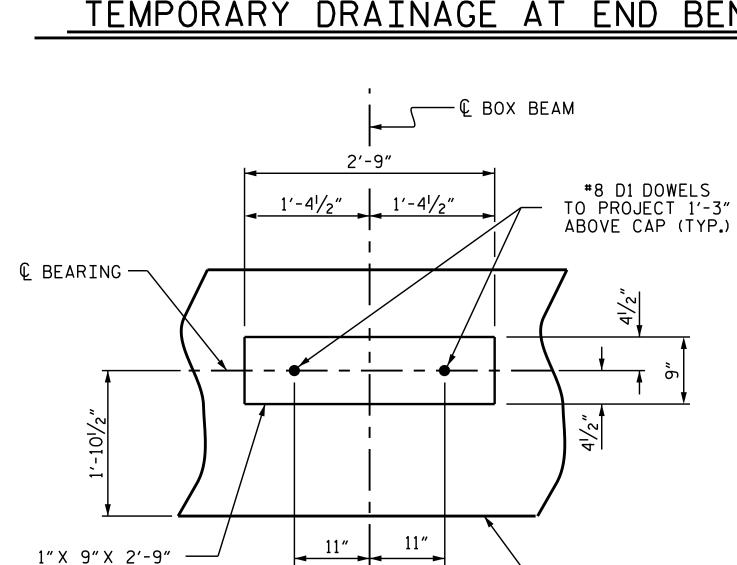


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

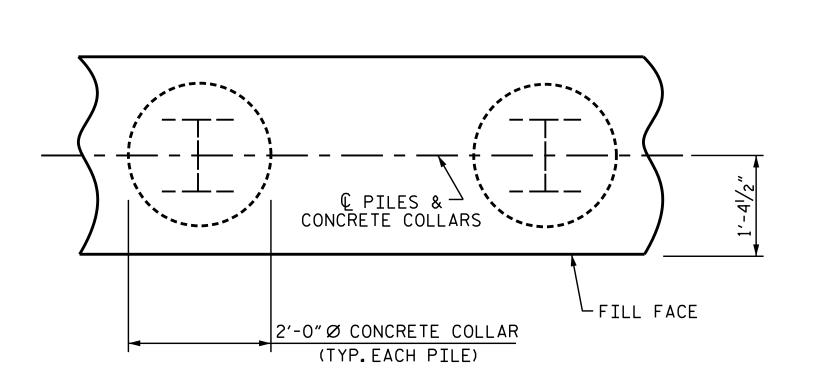
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



DETAIL "A" (END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

1'-10"



ELEVATION PLAN CORROSION PROTECTION FOR STEEL PILES DETAIL

CONCRETE — COLLAR

© HP 12 X 53 TEEL PILE

2'-0"

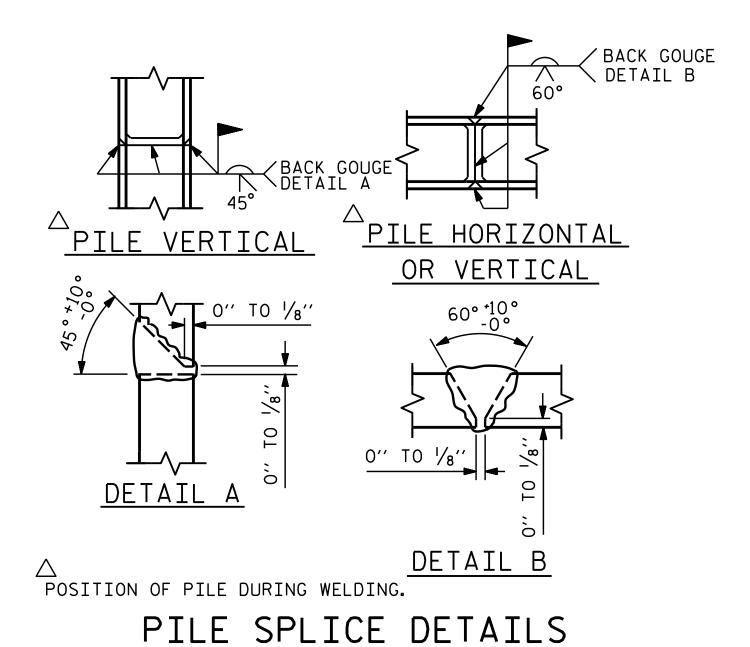
BOTTOM OF CAP

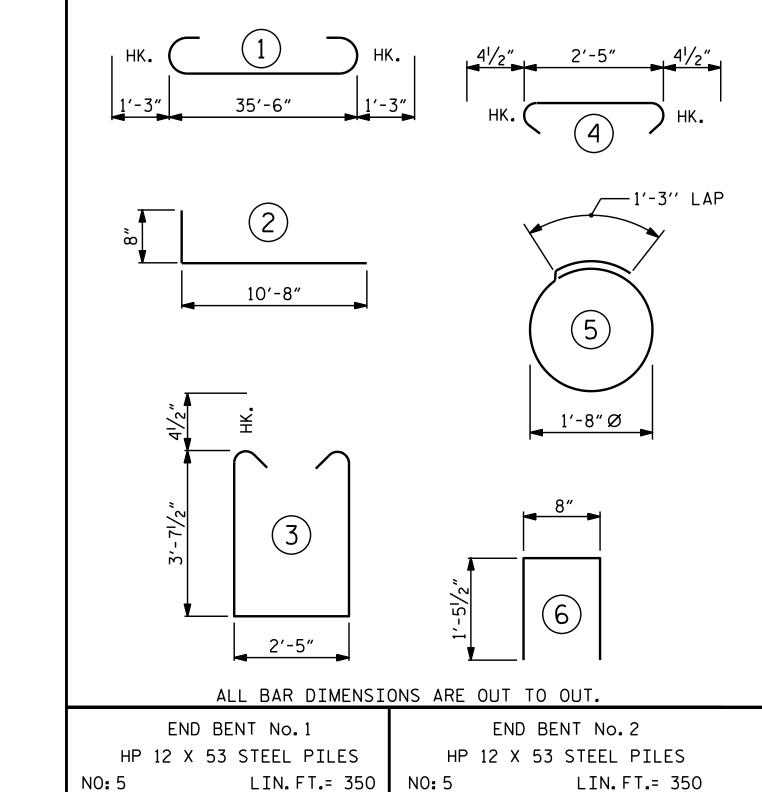
- FILL FACE

(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

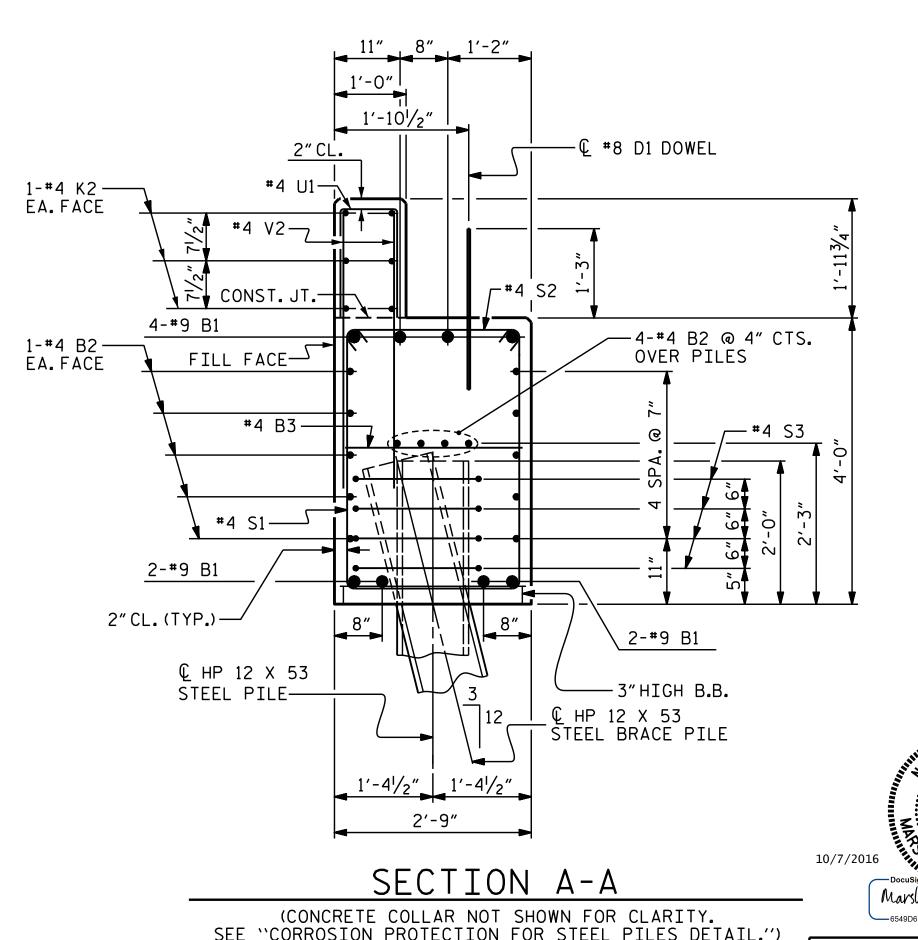
ASSEMBLED BY:R. CAREATHERS/MPDATE: 6/3/15 CHECKED BY: M. PISO DATE: 6/17/15 REV. 8/14 MAA/TMG DRAWN BY: WJH 12/11 CHECKED BY : AAC 12/11

ELASTOMERIC BRG. PAD (TYPE II) (TYP.)





BAR TYPES



B-4814 PROJECT NO.____ SAMPSON COUNTY 17+71.00 -L-STATION: SHEET 4 OF 4

BILL OF MATERIAL

FOR ONE END BENT

#4 | STR | 2'-5"

38'-0"

19′-1″

11'-4"

10′-5″

3′-2″

6′-6"

3'-7"

1034

357

15

120

567

23

153

320

97

87

72

287

210

3342 LBS.

18.5 C.Y.

5.2 C.Y.

23.7 C.Y.

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

#9 | 1

#4 STR

D1 | 20 | #8 | STR | 2'-3"

K1 | 12 | #4 | STR | 2'-11"

K2 | 12 | #4 | STR | 19'-1"

48 | #5 | 2

S1 | 46 | #4 | 3

S3 | 20 | #4 | 5 |

U1 | 30 | #4 | 6

REINFORCING STEEL

(FOR ONE END BENT)

46 | #4 | 4

V1 | 60 | #4 | STR | 7'-2"

V2 | 60 | #4 | STR | 5'-3"

CLASS A CONCRETE BREAKDOWN

(FOR ONE END BENT)

OF WINGS & COLLARS

POUR #1 CAP, LOWER PART

POUR #2 BACKWALL & UPPER

PART OF WINGS

B1

B2 |

В3

S2

28

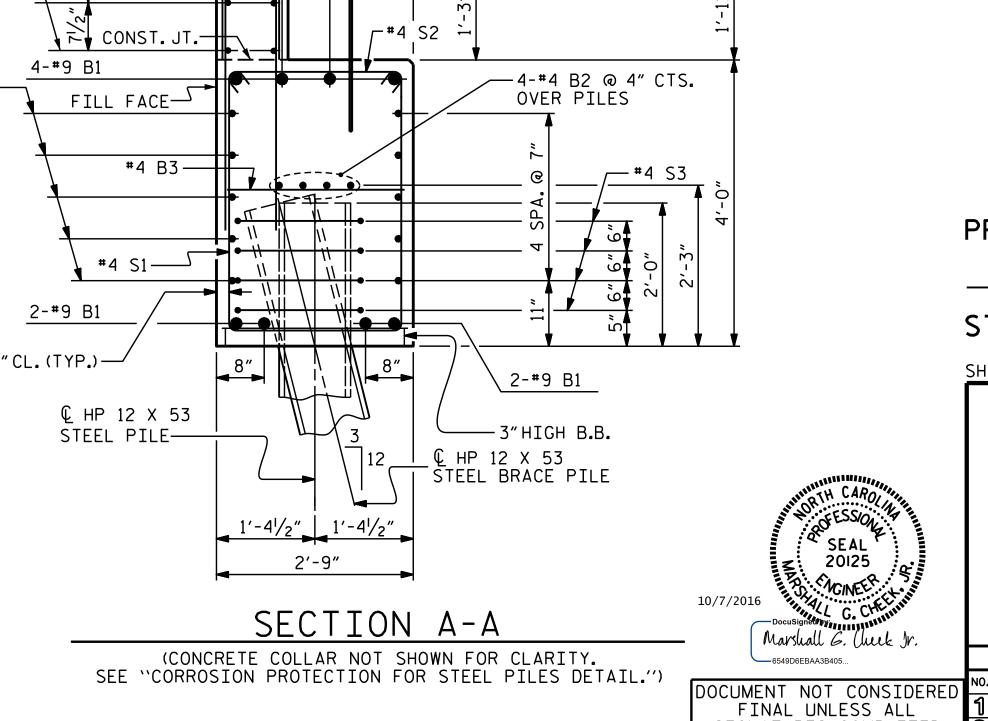
9

DEPARTMENT OF TRANSPORTATION

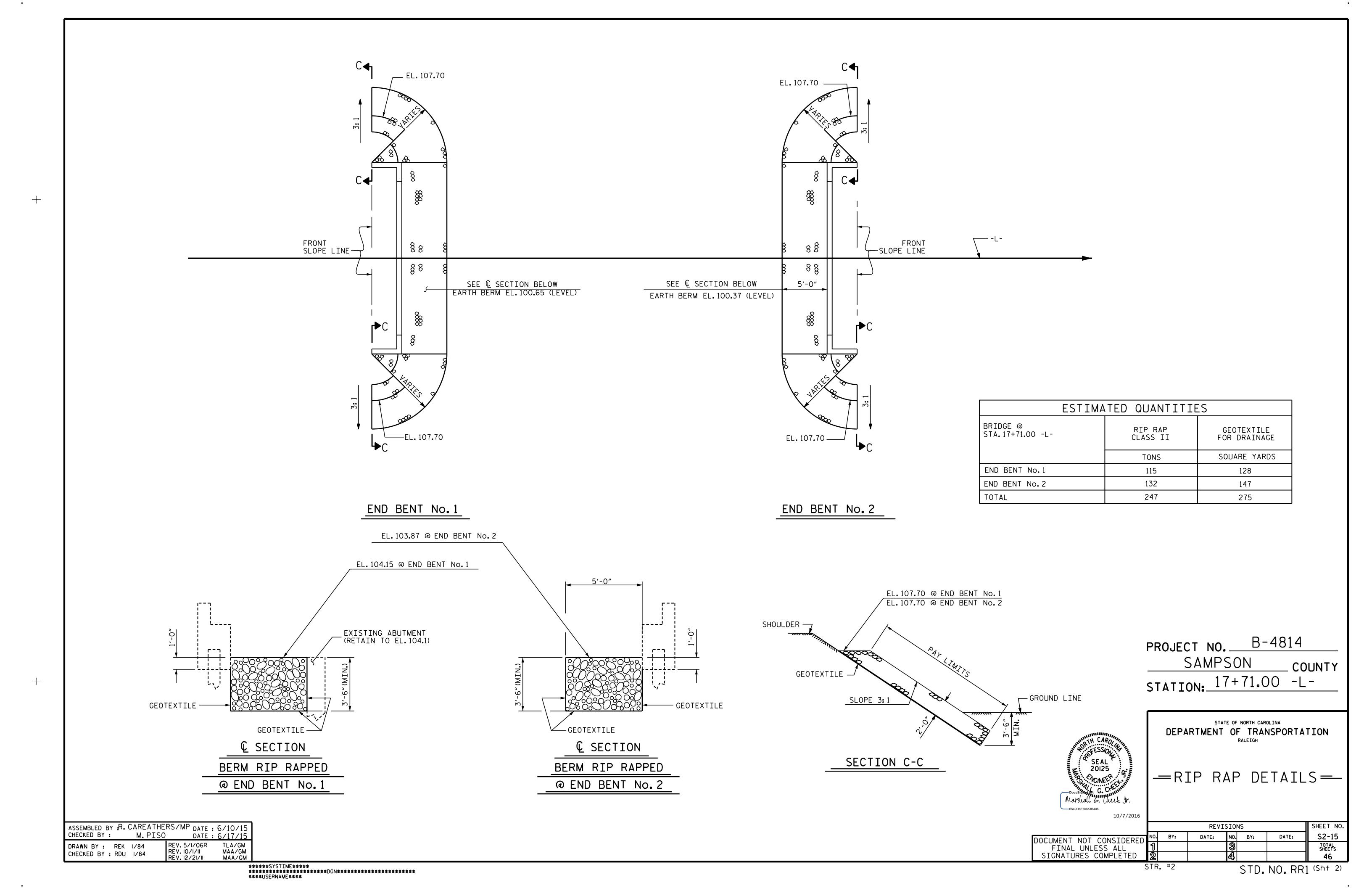
SUBSTRUCTURE

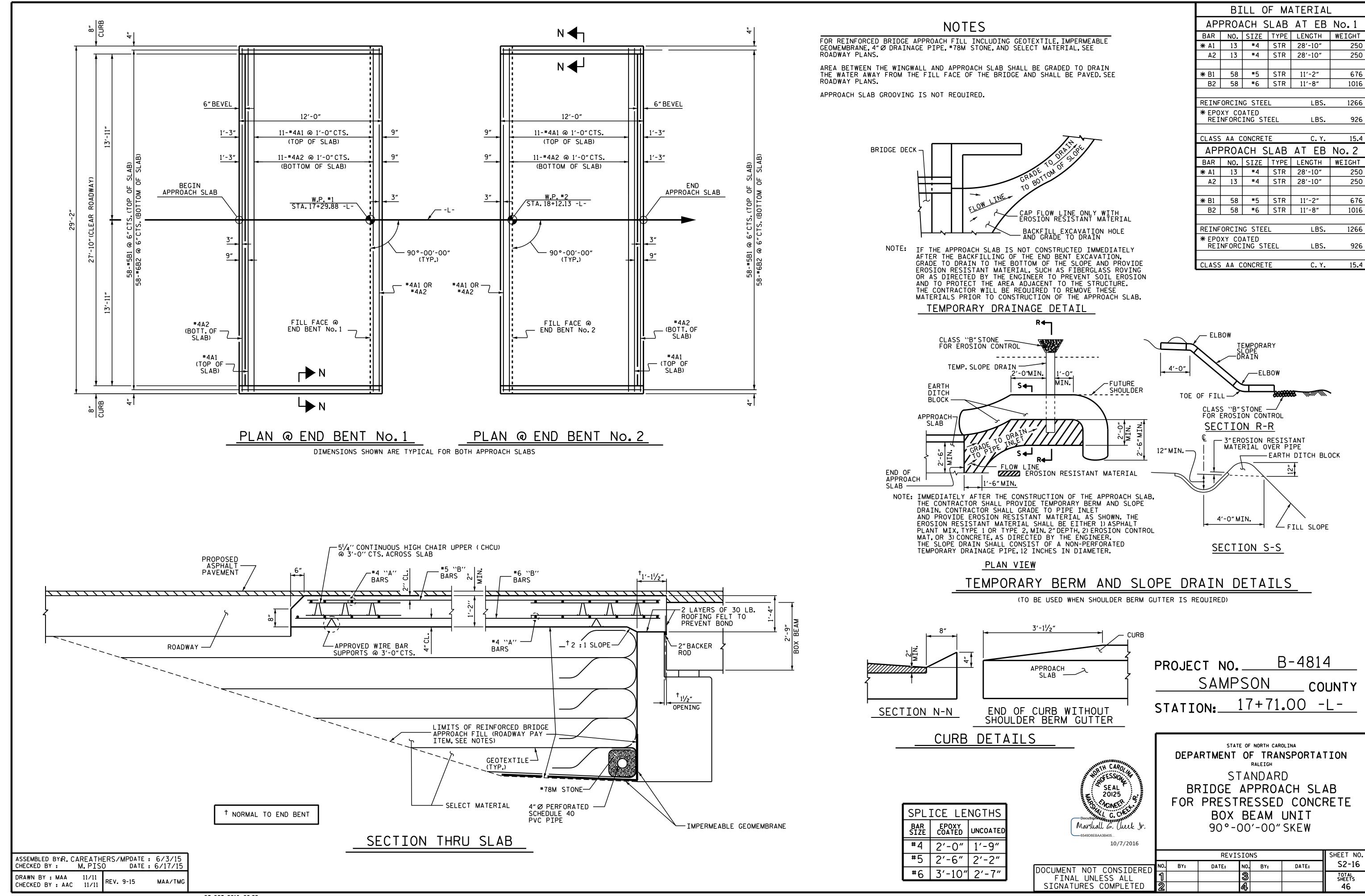
END BENT No.1 & 2 DETAILS

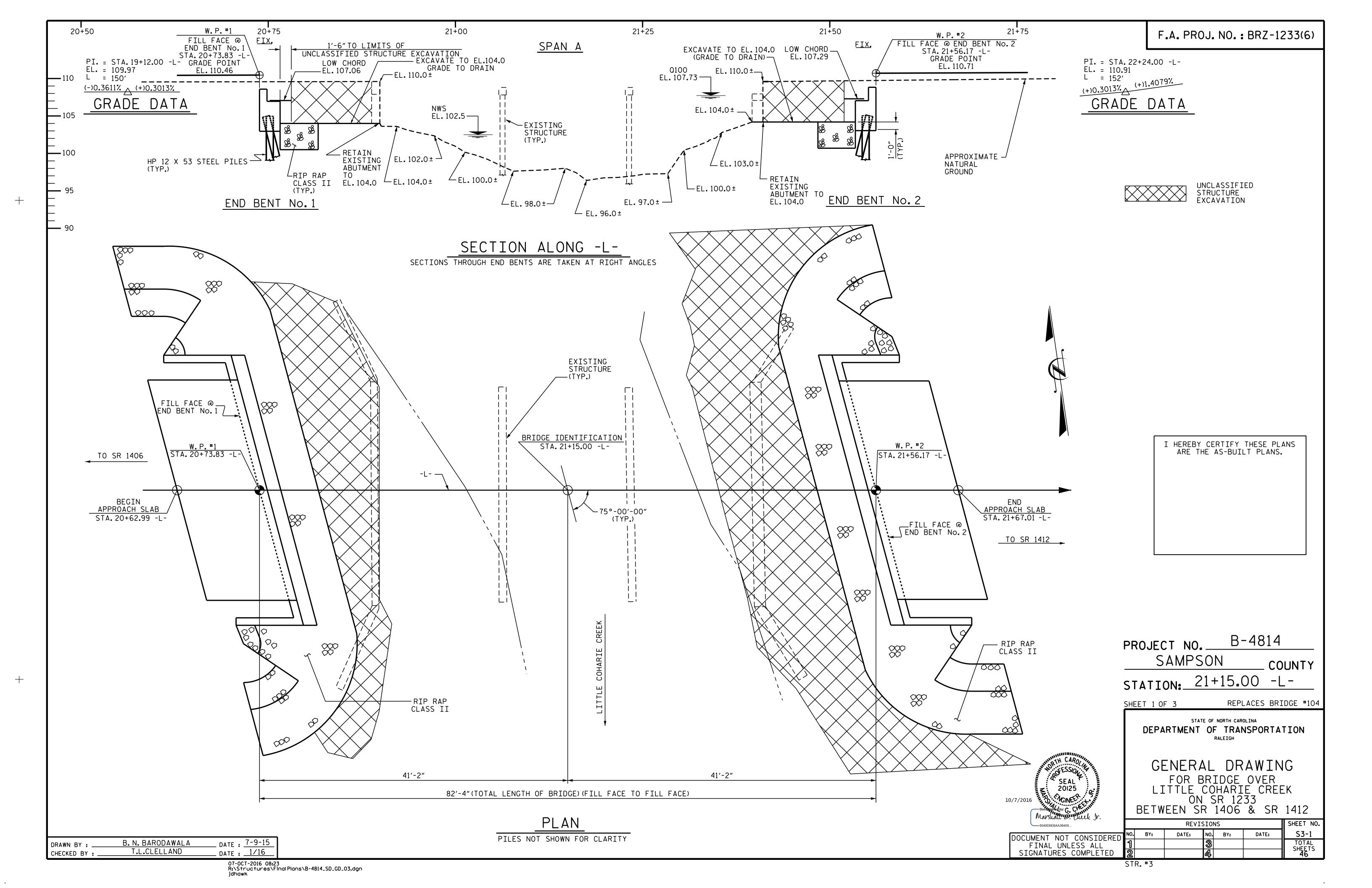
REVISIONS SHEET NO. S2-14 NO. BY: DATE: DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS

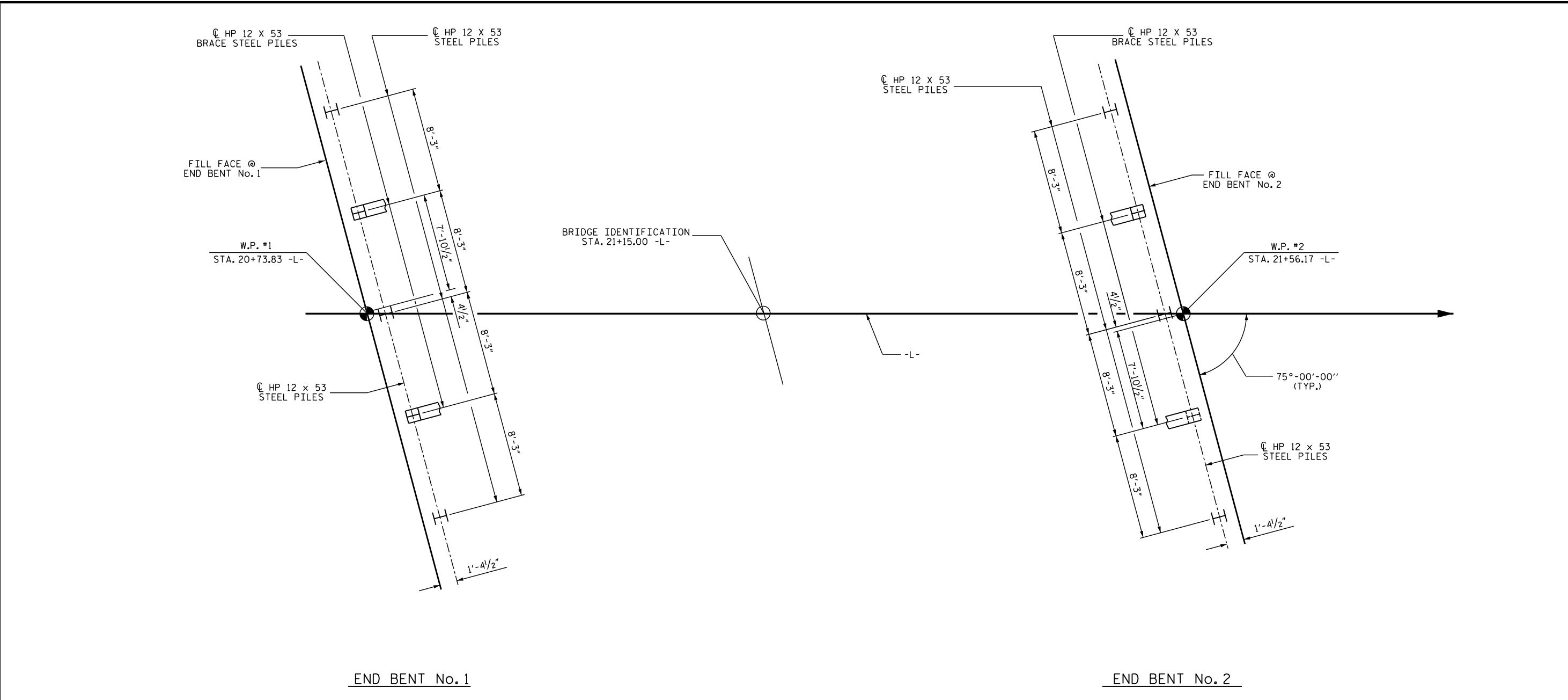


STD. NO. EB_30_90S4_33BB









FOUNDATION LAYOUT

(BRACE PILES AT END BENTS ARE BATTERED 3:12)

NOTES

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT No.1 AND END BENT No.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE.

DRIVE PILES AT END BENT No.1 AND END BENT No.2 TO A REQUIRED DRIVING RESISTANCE OF 200 TONS PER PILE.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 60,000 TO 80,000 FT-LBS.PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT No.1 AND END BENT No.2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

PROJECT NO. B-4814

SAMPSON COUNTY

STATION: 21+15.00 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING
FOR BRIDGE OVER
LITTLE COHARIE CREEK
ON SR 1233
BETWEEN SR 1406 & SR 1412

SEAL 20125

NOINEER

Marshall E. Junk Jr.

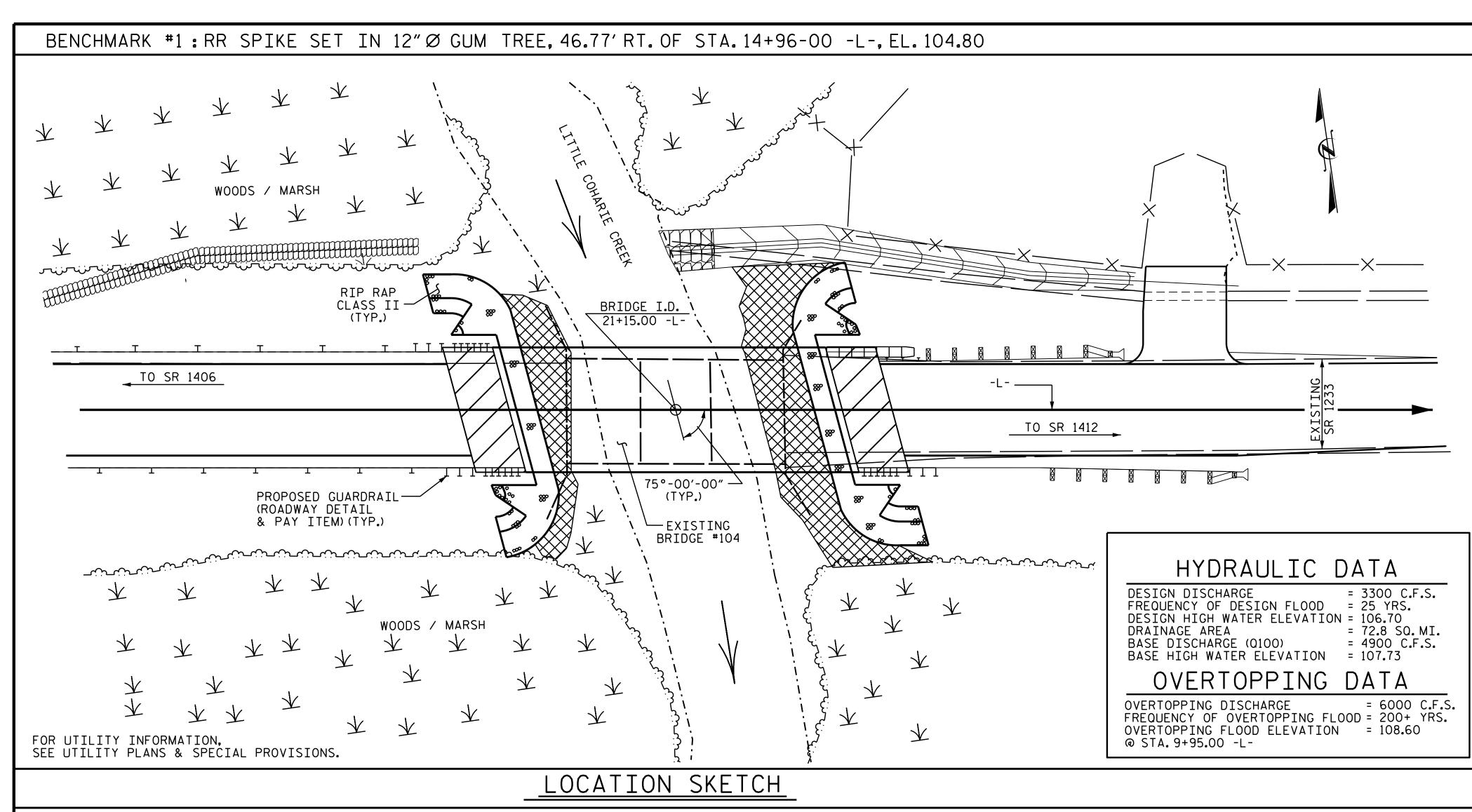
6549D6EBAA3B405...

DOCUMENT NOT CONSIDERED	NO.
FINAL UNLESS ALL	1
SIGNATURES COMPLETED	2

10/7/2016

			REVI	SION	1S		SHEET NO.
7	NO.	BY:	DATE:	NO.	BY:	DATE:	S3-2
۱	1			3			TOTAL SHEETS
	2			4			46

DRAWN BY: B. N. BARODAWALA DATE: 7-8-15
CHECKED BY: T.L.CLELLAND DATE: 1/16



NOTES:

ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES".

THE EXISTING STRUCTURE CONSISTING OF 3 SPANS (1 @ 17'-10", 1 @ 17'-1", 1 @ 17'-9") WITH A CLEAR ROADWAY WIDTH OF 24'+0" AND A REINFORCED CONCRETE DECK ON 19 LINES OF 6'X 131/2"TIMBER JOISTS AND END BENTS AND BENTS CONSIST OF TIMBER CAPS AND TIMBER PILES. THE EXISTING STRUCTURE, LOCATED AT THE SITE OF THE PROPOSED BRIDGE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT LEFT AND 35 FT RIGHT OF THE CENTERLINE ROADWAY AT END BENT No. 1 AND FOR A DISTANCE OF 35 FT. EACH SIDE OF THE CENTERLINE ROADWAY AT END BENT No 2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITES, SEE SPECIAL PROVISIONS.

						TOTAL	В	ILL (OF MAT	ERIAL						
	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP STEE	12 × 53 L PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE C	O" × 2'-9" STRESSED ONCRETE IX BEAMS	ASBESTOS ASSESSMENT
	LUMP SUM	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	EACH	LIN.FT.	TONS	SQ.YDS.	LUMP SUM	NO.	LIN.FT.	LUMP SUM
SUPERSTRUCTURE					LUMP SUM					160.00			LUMP SUM	10	800.00	LUMP SUM
END BENT No.1			LUMP SUM	24.4		3430	5	400	3		162	180				
END BENT No. 2			LUMP SUM	24.4		3430	5	375	3		162	180				
TOTAL	LUMP SUM	1	LUMP SUM	48.8	LUMP SUM	6860	10	775	6	160.00	324	360	LUMP SUM	10	800.00	LUMP SUM

B-4814 PROJECT NO.____ SAMPSON _ COUNTY STATION: 21+15.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING FOR BRIDGE OVER LITTLE COHARIE CREEK ON SR 1233 BETWEEN SR 1406 & SR 1412

Can Carlotte						
Marshall in Philip Jr.	REVISIONS					
DOCUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	
FINAL UNLESS ALL	1			3		
SIGNATURES COMPLETED	2			<u>a</u> ,		

B. N. BARODAWALA _ DATE : <u>7-8-15</u> DRAWN BY T.L.CLELLAND DATE : 1/16 CHECKED BY:

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE SHEAR **MOMENT** MOMENT DISTRIBUTION FACTORS (DF) FR OF MINIMUM RATING F/ 1.179 1.75 0.268 39.224 0.584 1.27 7.845 0.80 0.268 1.18 39.224 N/A EL HL-93(Inv)EL 1.644 1.35 2.27 0.584 1.64 7.845 N/A EL HL-93(0pr) 0.268 39.224 N/A --DESIGN $\langle 2 \rangle$ 36.000 1.564 LOAD 56.305 0.268 2.33 39.224 0.584 1.60 7.845 0.80 0.268 1.56 39.224 HS-20(Inv) EL EL RATING 36.000 2.077 74.771 1.35 0.268 3.02 39.224 0.584 2.08 7.845 HS-20(0pr) EL N/A 0.584 13.500 3.580 48.335 0.268 6.66 39.224 4.81 7.845 0.80 0.268 3.58 39.224 SNSH 1.40 EL EL 20.000 52.933 0.268 4.92 39.224 0.584 3.41 7.845 0.80 0.268 2.65 SNGARBS2 2.647 EL EL 1.40 39.224 22.000 2.498 0.268 39.224 0.584 3.16 7.845 0.80 0.268 2.50 39.224 SNAGRIS2 54.946 4.64 EL EL 27.250 1.781 0.268 39.224 0.584 2.40 7.845 0.80 0.268 1.78 SNCOTTS3 1.40 3.31 EL EL 39.224 0.584 34.925 1.480 51.695 0.268 2.75 39.224 1.98 0.80 0.268 SNAGGRS4 1.40 EL EL 7.845 1.48 39.224 0.268 35.550 51.477 1.40 2.69 EL 39.224 0.584 2.00 7.845 0.80 0.268 1.45 39.224 SNS5A 1.448 EL EL 39.950 0.584 1.82 1.325 52.939 0.268 39.224 7.845 0.80 0.268 1.33 39.224 SNS6A 2.46 EL EL 1.262 52.996 0.268 2.35 39.224 0.584 0.80 0.268 1.26 SNS7B 42.000 EL 1.79 7.845 39.224 1.40 LEGAL LOAD 33.000 53.292 0.268 3.00 39.224 0.584 0.80 0.268 2.17 1.61 39.224 TNAGRIT3 1.615 1.40 EL 7.845 RATING 33.075 1.621 53.618 0.268 39.224 0.584 2.12 7.845 0.80 0.268 1.62 TNT4A 1.40 3.01 EL 39.224 41.600 1.322 55.003 0.268 EL 39.224 0.584 1.89 7.845 0.80 0.268 1.32 39.224 TNT6A 2.46 EL 1.327 0.268 2.47 39.224 0.584 1.86 0.80 0.268 1.33 TNT7A 42.000 55.736 EL EL 7.845 39.224 1.40 EL 42.000 1.369 2.54 39.224 0.584 57.481 EL TNT7B 1.40 0.268 1.75 EL 7.845 0.80 0.268 1.37 39.224 43.000 1.305 56.120 0.268 2.43 39.224 0.584 1.69 7.845 0.80 EL 0.268 1.31 39.224 TNAGRIT4 1.40 EL 0.584 1.68 45.000 55.443 0.268 39.224 7.845 0.80 0.268 1.23 39.224 TNAGT5A

EL 39.224 0.584 1.61

7.845 0.80 0.268

LOAD FACTORS:

	DESIGN LOAD RATING FACTORS	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
		STRENGTH I	1.25	1.50
		SERVICE III	1.00	1.00

NOTES:

EL **39.224**

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

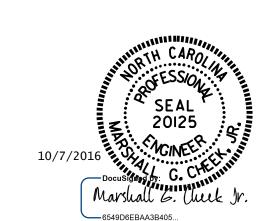
- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- $\langle 3 \rangle$ LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER

B-4814 SAMPSON __ COUNTY



PROJECT NO. ____ STATION: 21+15.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD LRFR SUMMARY FOR 80' BOX BEAM UNIT 75°-00'-00" SKEW (NON-INTERSTATE TRAFFIC)

SHEET NO. REVISIONS S3-4 DATE: NO. BY: DATE: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

0.268

2.27

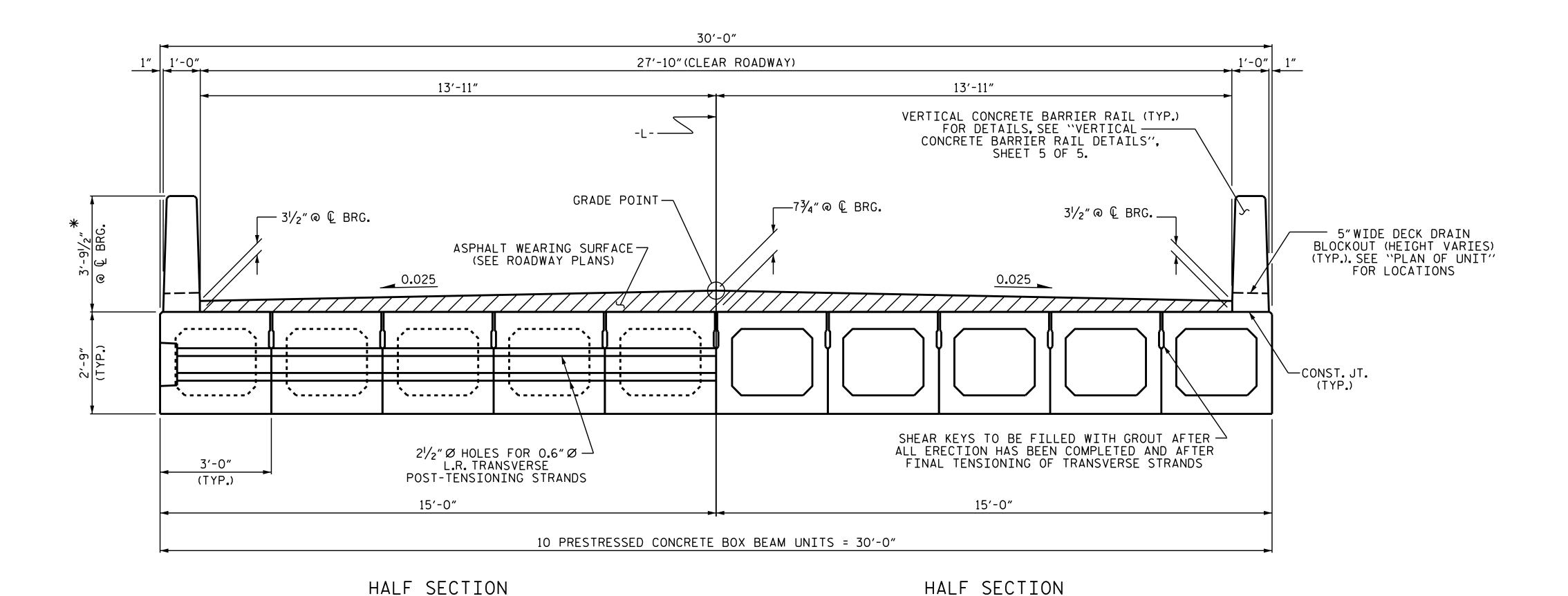
1.218 54.832 1.40

LRFR SUMMARY FOR SPAN A

ASSEMBLED BY: R. CAREATHERS/MP DATE: 6/3/15 CHECKED BY: M. PISO DATE: 6/25/15

TNAGT5B

DRAWN BY : TMG II/II CHECKED BY : AAC II/II

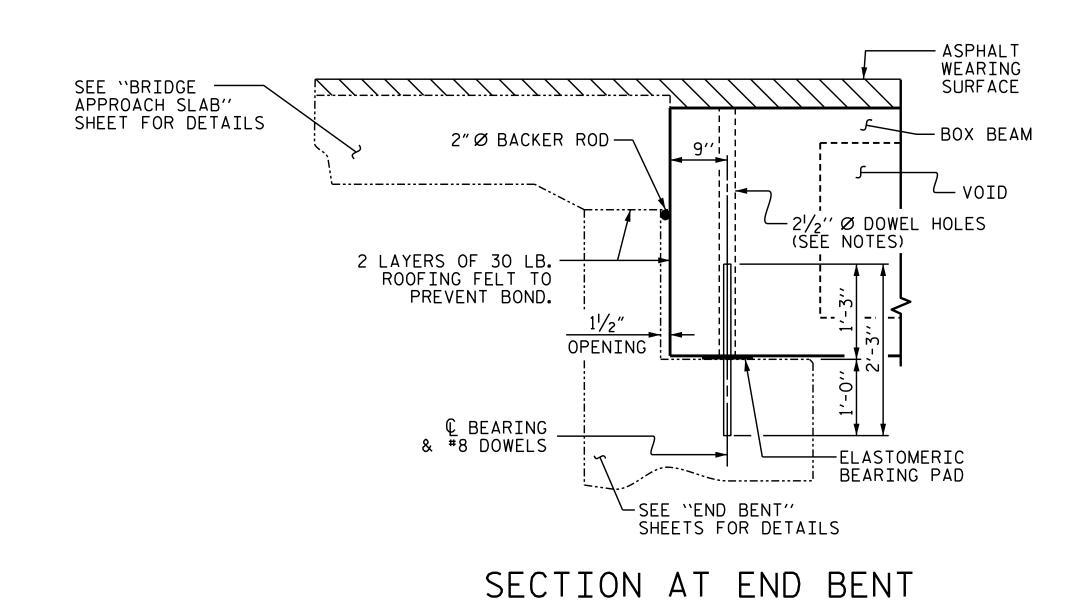


THROUGH VOIDS

TYPICAL SECTION

* THE MAXIMUM VERTICAL CONCRETE BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE VERTICAL CONCRETE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE VERTICAL CONCRETE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

FIXED END



AT INTERMEDIATE DIAPHRAGMS

PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF EXTERIOR UNIT AND RECESSED 3/8". SIZE TO BE DETERMINED BY CONTRACTOR.

THREADED INSERT DETAIL

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6000 PSI.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 5" X 4". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE BOX BEAM UNIT TO THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR BOX BEAM UNITS THAT REQUIRE DRAINS IN THE VERTICAL CONCRETE BARRIER RAIL.

> B-4814 PROJECT NO. SAMPSON COUNTY 21+15.00 -L-STATION:

SHEET 1 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

PRESTRESSED CONCRETE BOX BEAM UNIT 75°-00'-00" SKEW

SHEET NO **REVISIONS** S3-5 DATE: BY: DATE: BY: TOTAL SHEETS FINAL UNLESS ALL

DOCUMENT NOT CONSIDERE

20125

SUCINEER

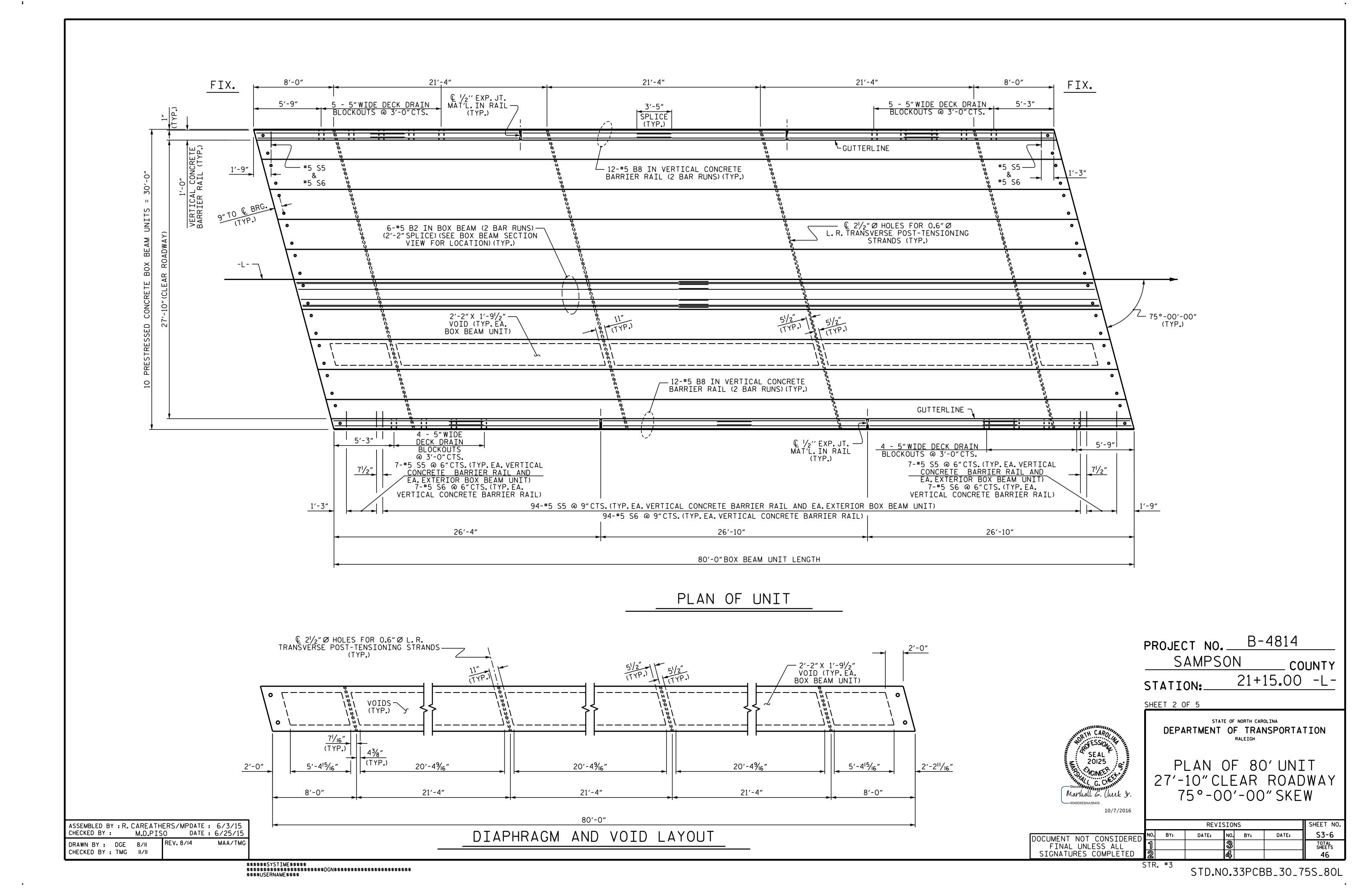
10/7/2016

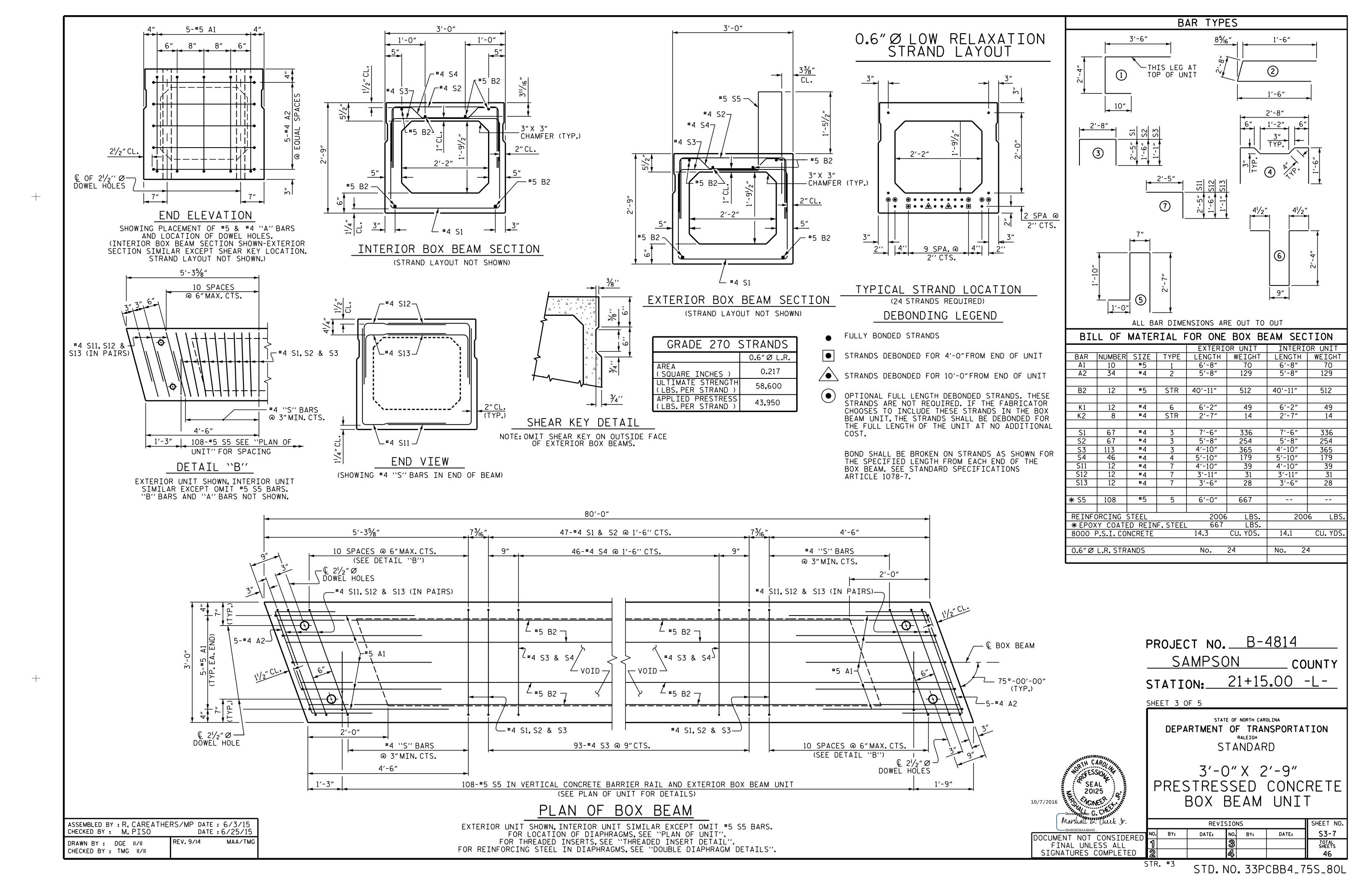
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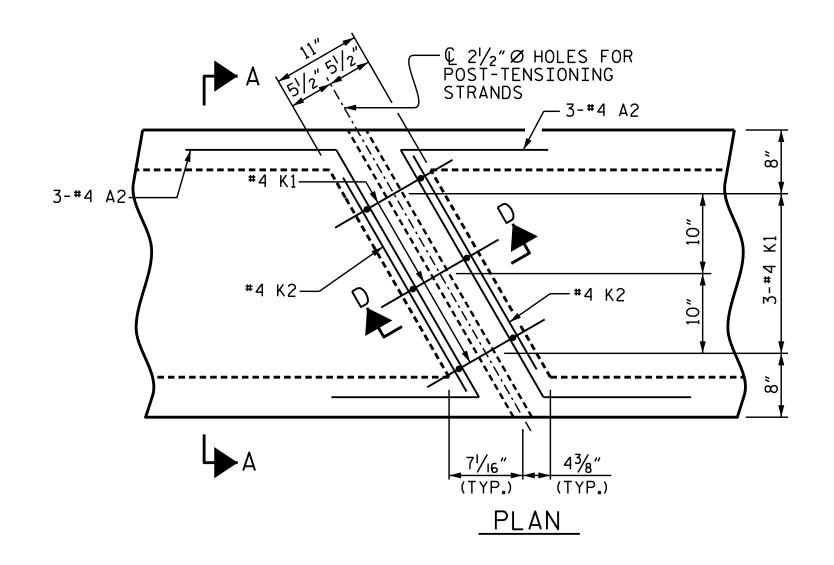
STR. #3 STD. NO. STD.33PCBB1_30

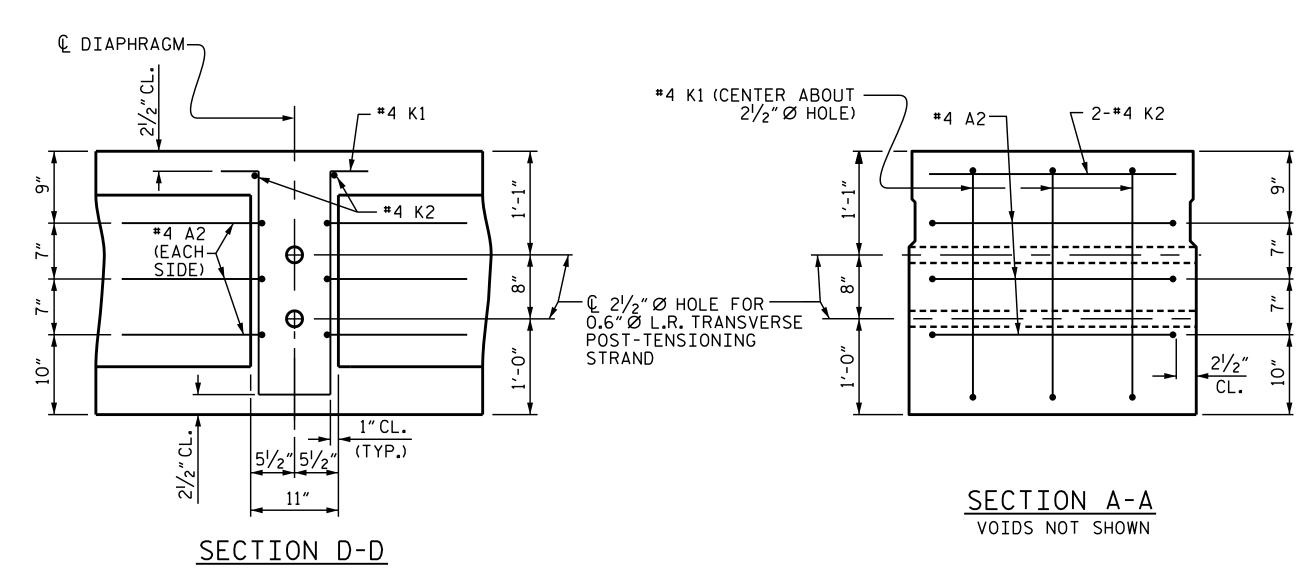
CHECKED BY: M. PISO DATE: 6/25/15 DRAWN BY : DGE 8/II REV. 8/14 MAA/TMG CHECKED BY : TMG II/II

ASSEMBLED BY: R. CAREATHERS/MP DATE: 6/3/15



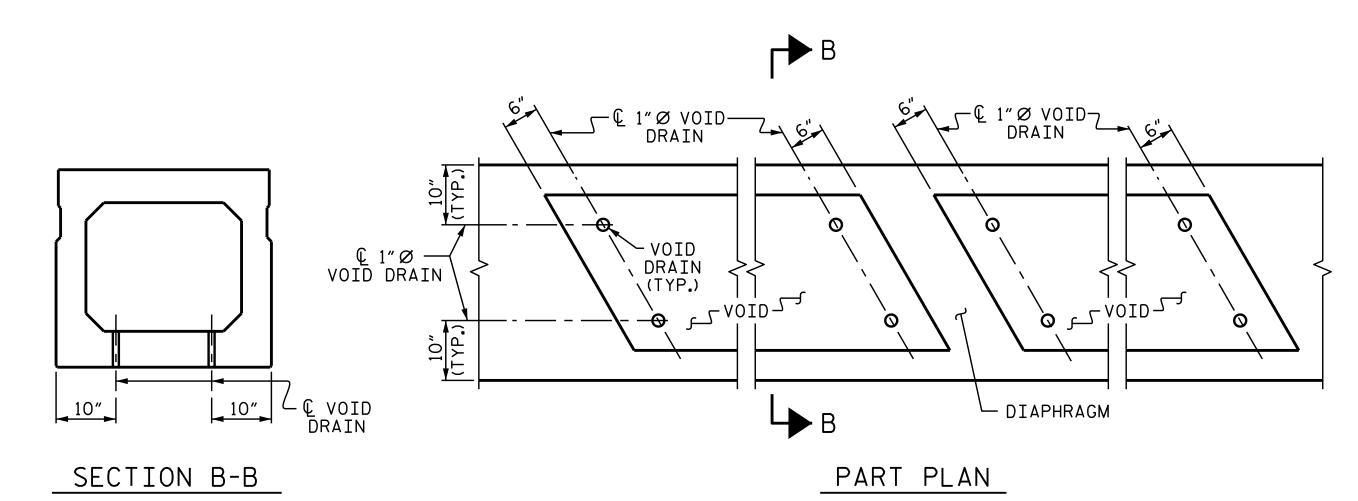






DOUBLE DIAPHRAGM DETAILS

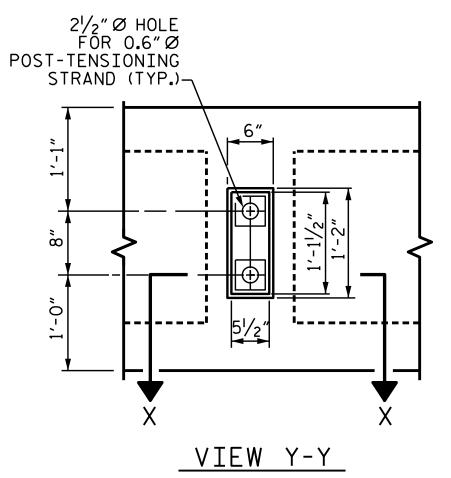
#4 ``S'' BARS NOT SHOWN. #4 ``S'' BARS MAY BE SHIFTED SLIGHTLY TO CLEAR $2^{1}/_{2}$ " Ø HOLE.



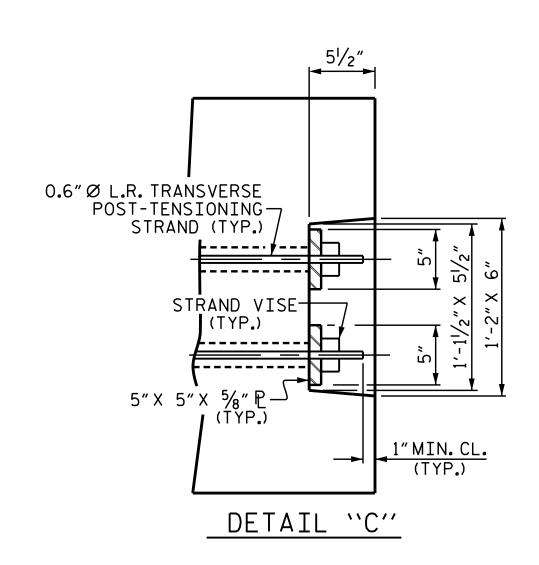
VOID DRAIN DETAILS

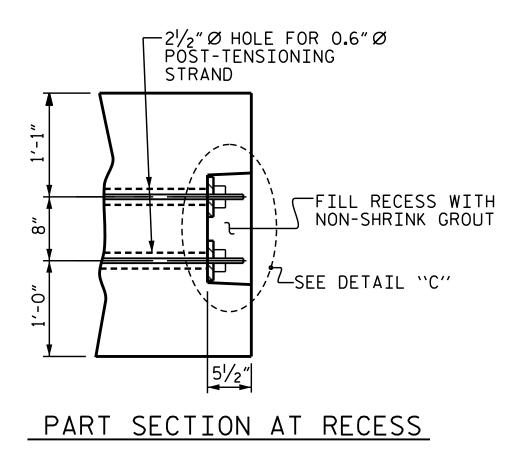
(DIMENSIONS SHOWN ARE TYPICAL FOR EACH VOID)

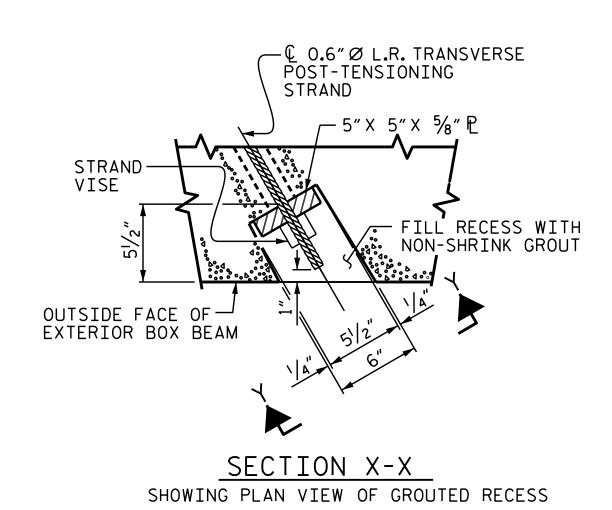
ASSEMBLED BY :R. CAREATHERS/MP DATE : 6/3/15 CHECKED BY : M.D.PISO DATE : 6/25/15 REV. 8/14 MAA/TMG DRAWN BY : DGE II/II CHECKED BY : TMG II/II



SHOWING ELEVATION VIEW OF GROUTED RECESS







GROUTED RECESS DETAIL AT END OF POST-TENSIONED STRANDS OF EXTERIOR BOX BEAM

									_
	EAD	LOAD	DEFL	ECTION	1A 1	ND	CAME	3ER)
						3'	-0"×	2′-9	"
	8	30' BOX	BEAM (TINL		(0.6″Ø STRAI		
СА	MBER	(SLAB	ALONE	IN PLAC	CE)		13/4"	· •	
DE SU	FLECT JPERIM	ION DUI IPOSED	E TO DEAD LO	DAD**			1/2"	+	,
FI	INAL C	AMBER					11/4"	· •	

** INCLUDES FUTURE WEARING SURFACE

PROJECT NO. B-4814 SAMPSON COUNTY 21+15.00 -L-STATION:

SEAL 20125

NOINELL LINE

DEPARTMENT OF TRANSPORTATION
RALEIGH STANDARD

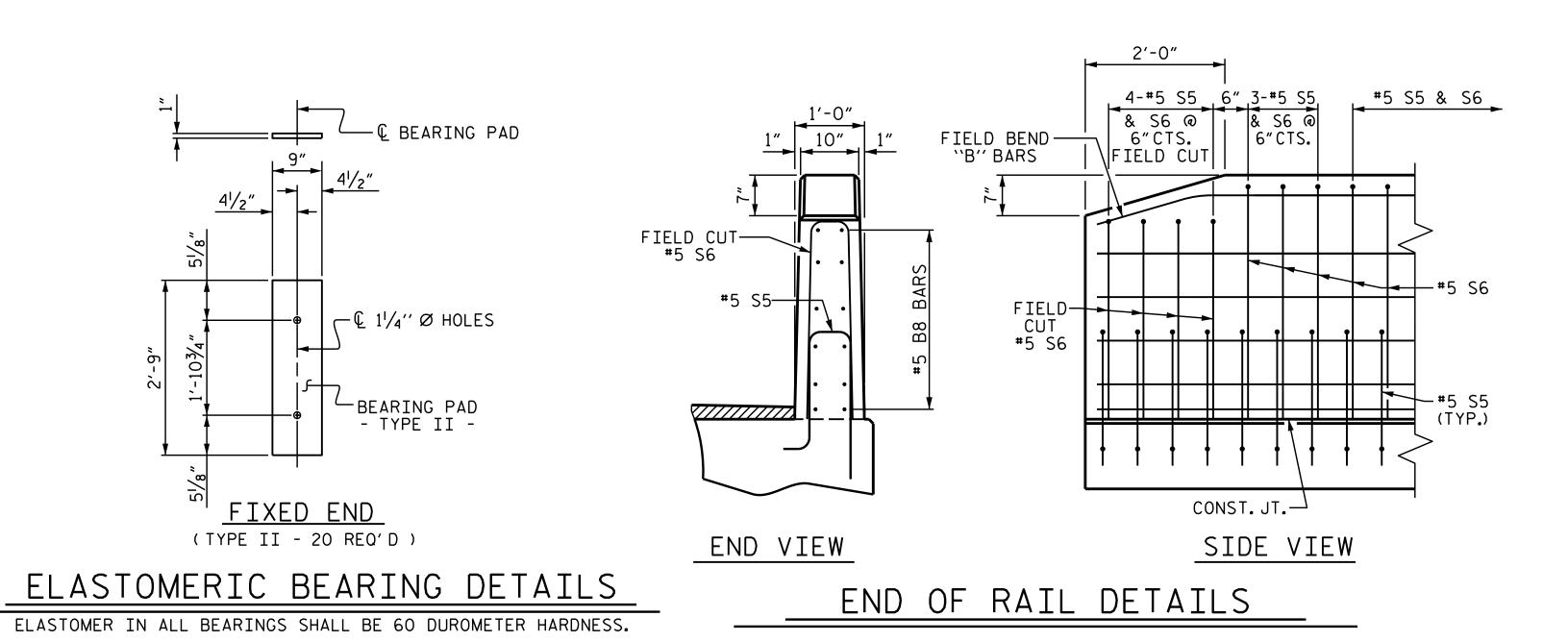
SHEET 4 OF 5

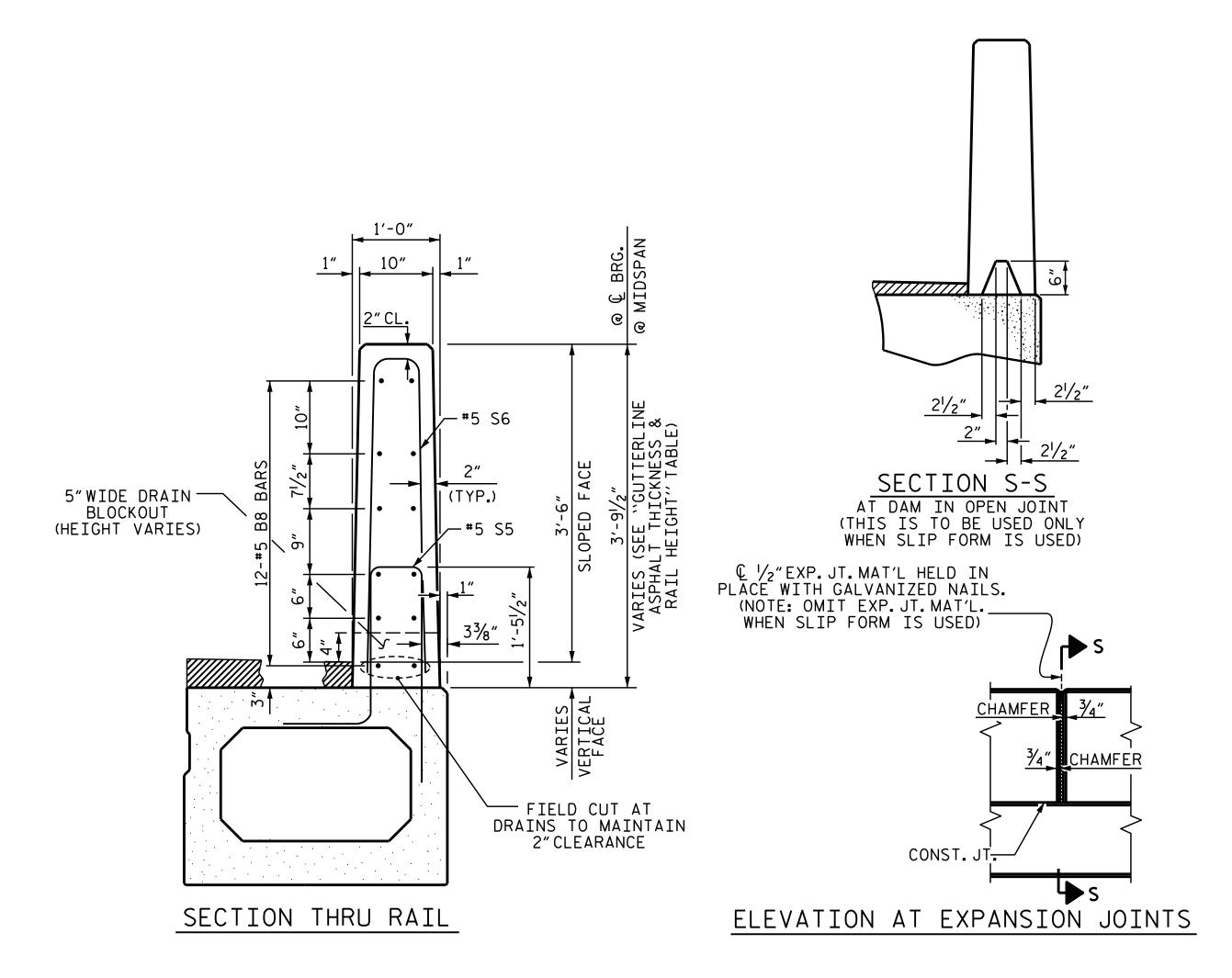
3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT

STATE OF NORTH CAROLINA

10/7/2016

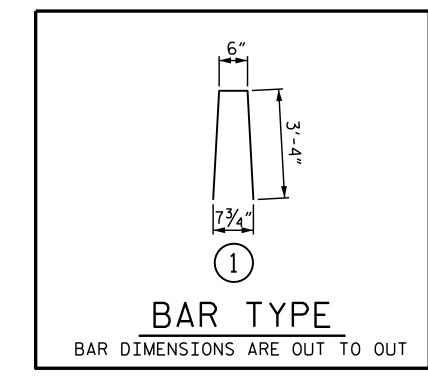
		REVISIONS					
OCUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S3-
FINAL UNLESS ALL	1			3			TOTAL SHEET
SIGNATURES COMPLETED	2			4			46
	STR.	#3		<u> </u>	TD 110	77000	





VERTICAL CONCRETE BARRIER RAIL DETAILS

ASSEMBLED BY : R. CAREATHERS/MPDATE : 6/3/15 CHECKED BY : M.D.PISO DATE : 6/25/15 MAA/TMG REV. 4/15 DRAWN BY : DGE 10/11 CHECKED BY : TMG II/II



BII	L OF MATERIAL FOR VERTICAL CONCRE	TE B	ARR:	IER R	RAIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
	80' UNIT				
∗ B8	144	#5	STR	14'-11"	2240
* S6	216	#5	1	7′-2″	1615
* EPOX	Y COATED REINFORCING STEEL		LBS.		3855
CLASS	AA CONCRETE		CU.YDS.	1	20.7
TOTAL	VERTICAL CONCRETE BARRIER RAIL		LN.FT.		160.00

GUTTERLINE ASPHA	ALT THICKNESS &	RAIL HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
80'UNITS	21/4"	3'-81/4"

BOX BEAM UNITS REQUIRED						
	NUMBER	LENGTH	TOTAL LENGTH			
EXTERIOR B.B.	2	80'-0"	160'-0"			
INTERIOR B.B.	8	80′-0″	640'-0"			
TOTAL	10		800′-0"			

PROJECT NO. B-4814 SAMPSON __ COUNTY STATION: 21+15.00 -L-

SHEET 5 OF 5

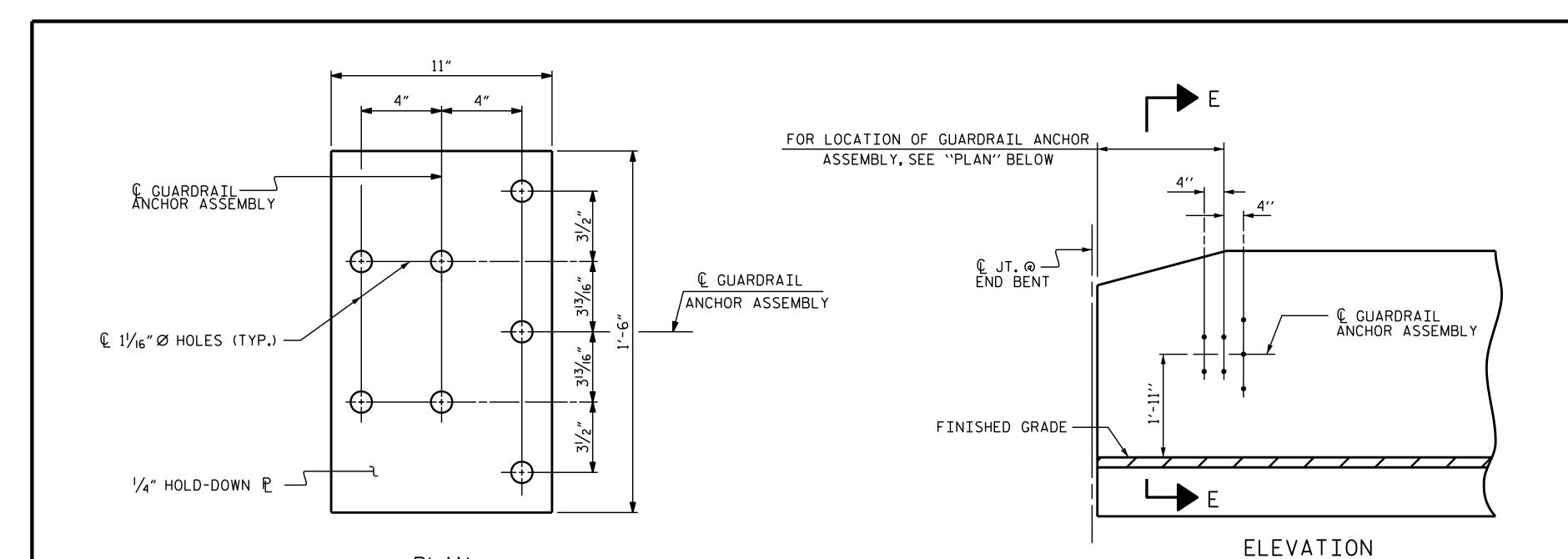
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT

DOCU

10/7/2016

			SHEET NO.				
JMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S3-9
FINAL UNLESS ALL	1			3			TOTAL SHEETS
GNATURES COMPLETED	2			4			46



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE \(\frac{7}{8}'' \) \(\Omega \) GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

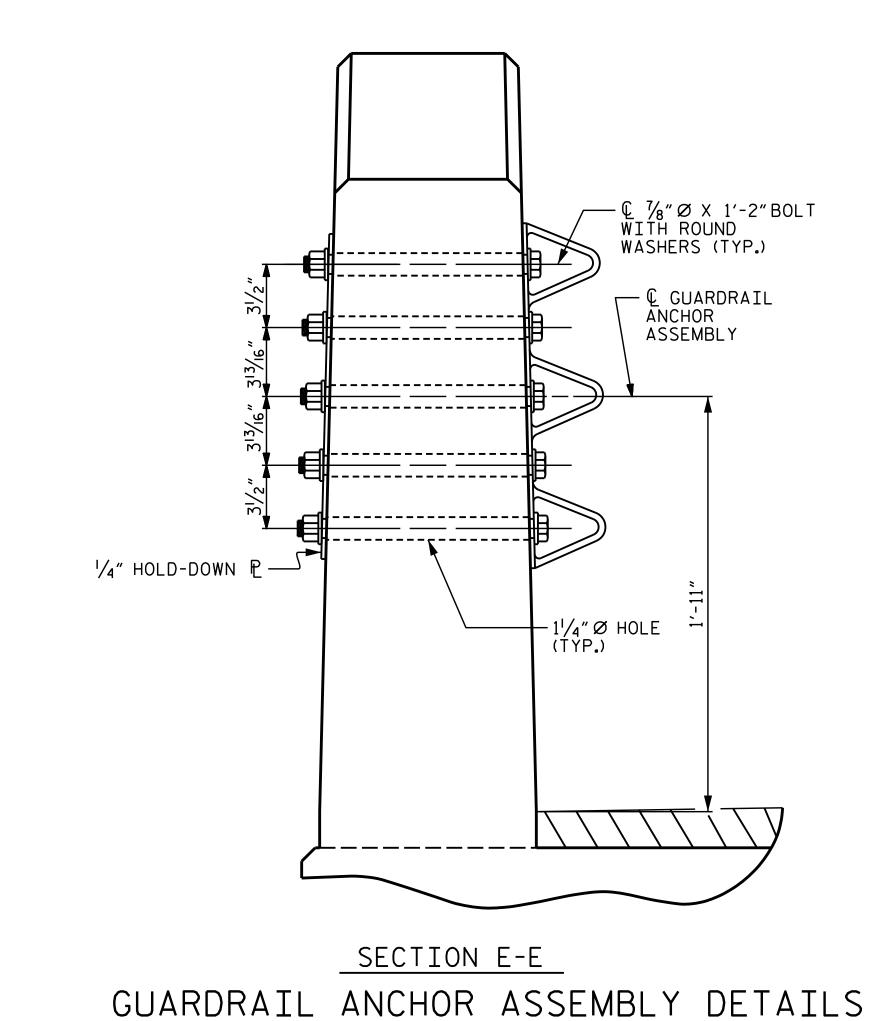
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

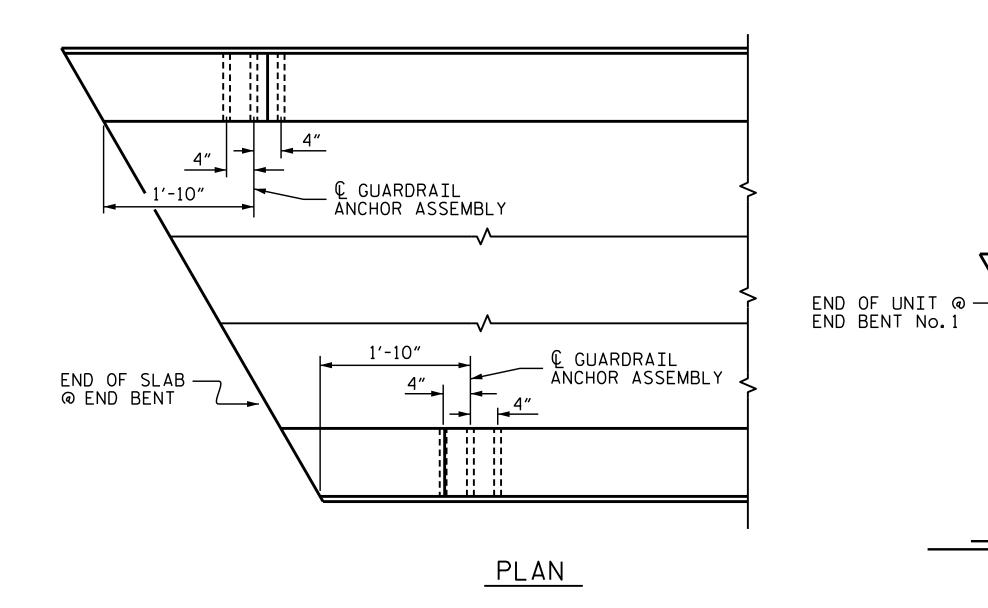


PLAN

ASSEMBLED BY: R. CAREATHERS/MP DATE: 6/3/15 CHECKED BY: M.D.PISO DATE: 6/25/15

DRAWN BY: MAA 5/10 CHECKED BY : GM 5/10 REV. 12/5/II REV. 6/13 REV. 1/15

MAA/GM MAA/GM MAA/TMG



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

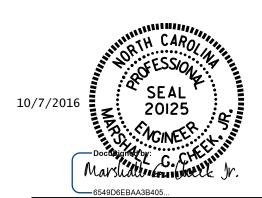
LOCATION OF ANCHORS FOR GUARDRAIL

END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR.

PROJECT NO. B-4814 SAMPSON _ COUNTY STATION: 21+15.00 -L-

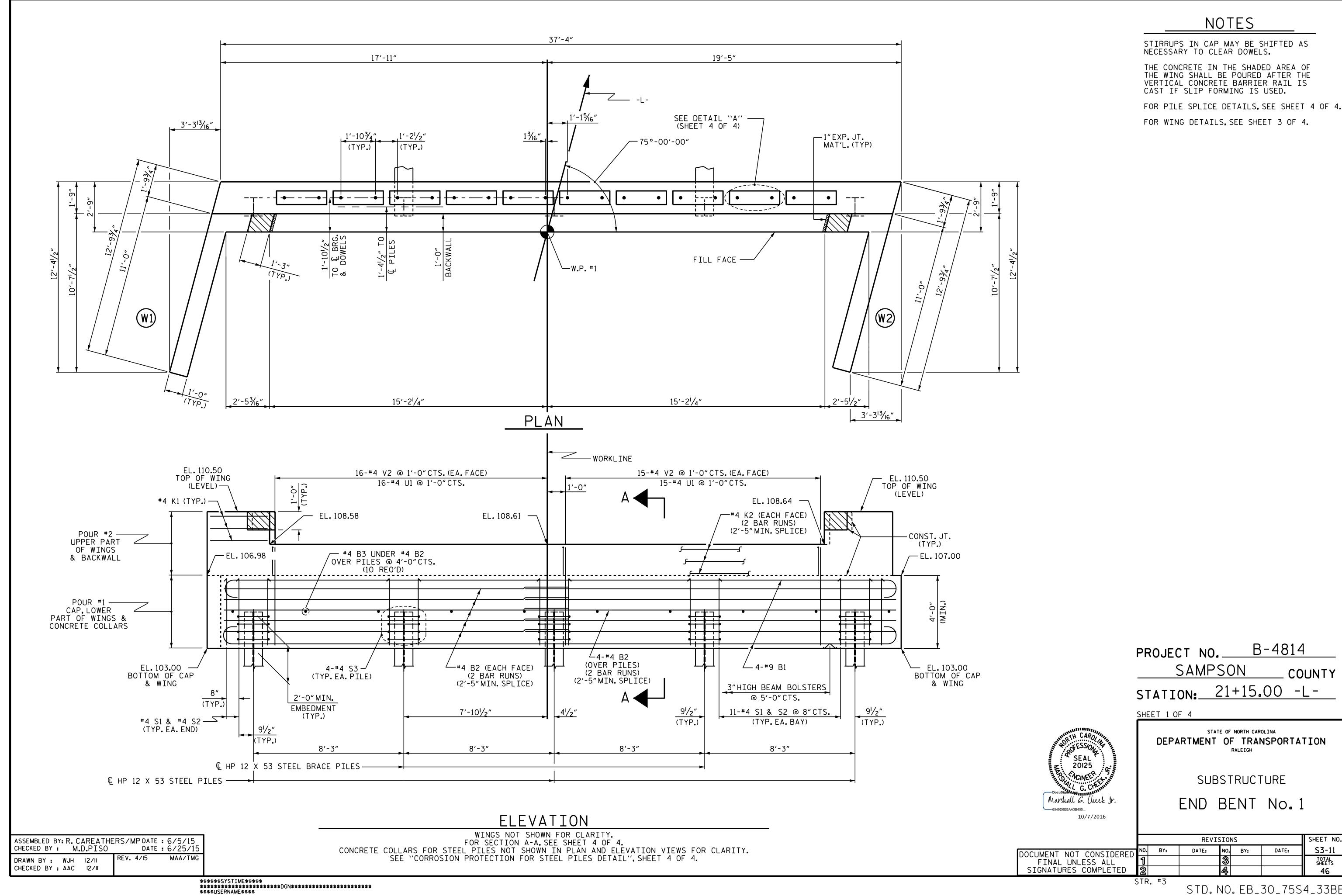
_ END OF UNIT @

END BENT No. 2

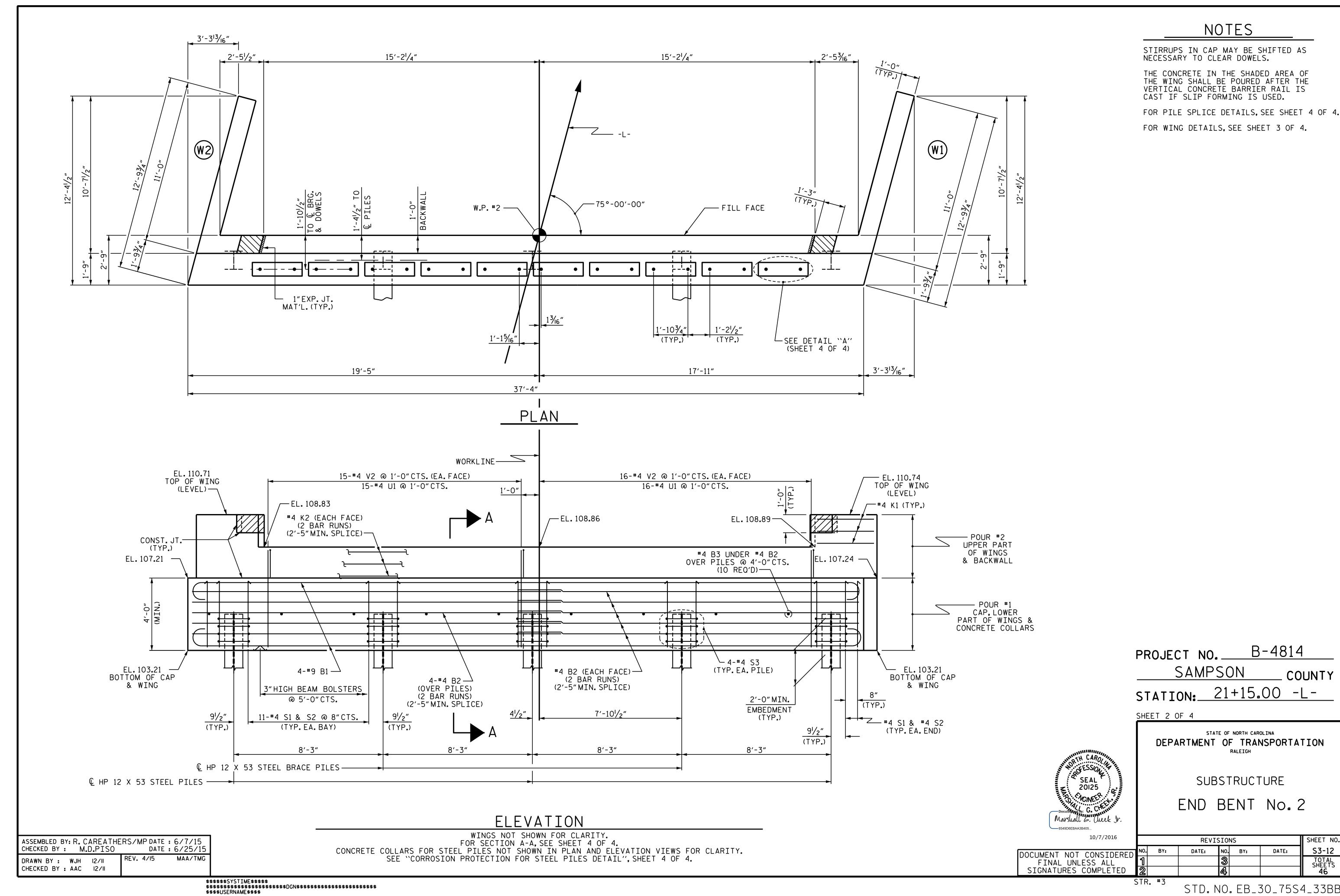


STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE BARRIER RAIL

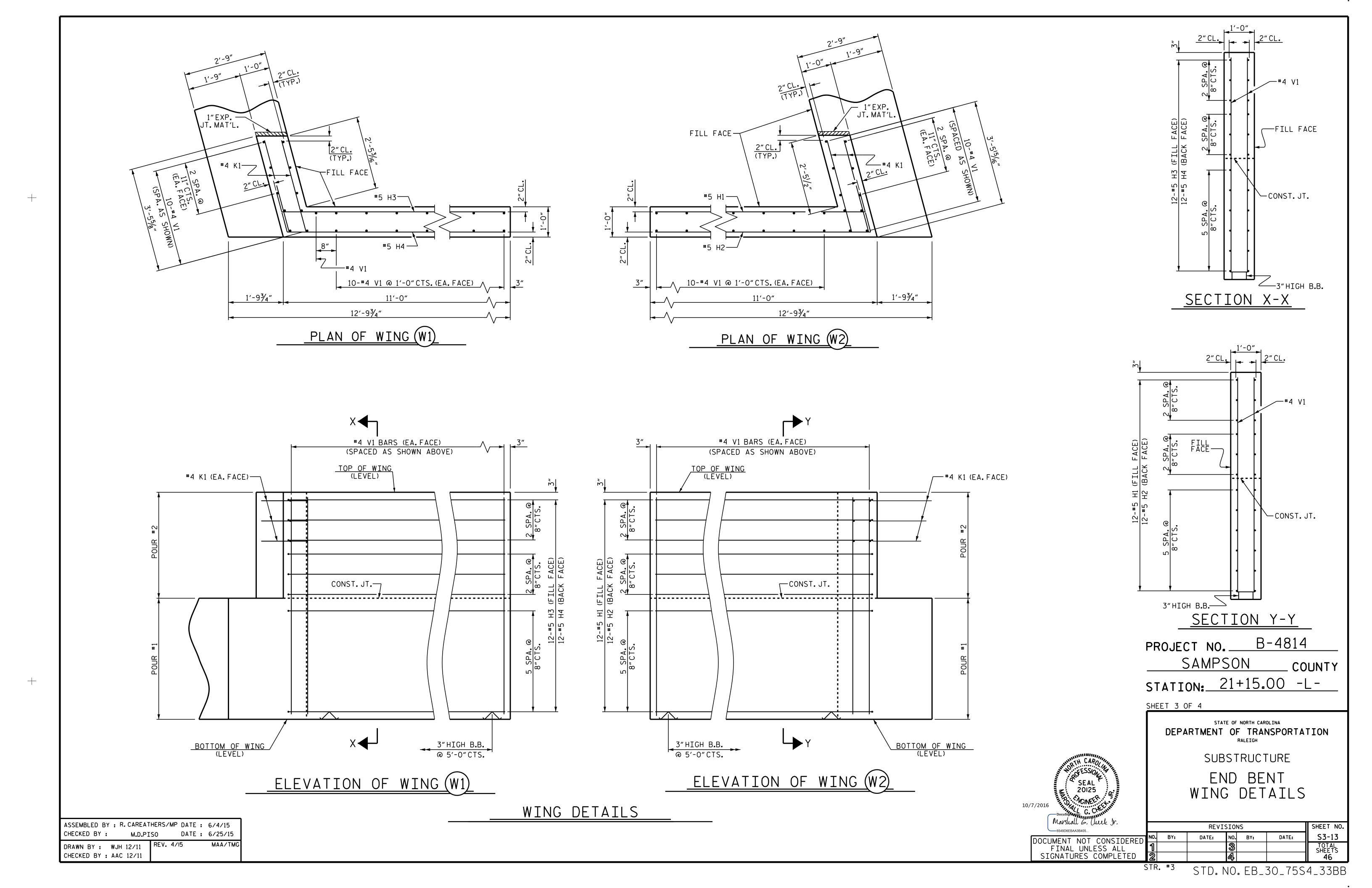
Marshallenenihoette Jr.			REVIS	SIO	NS		SHEET N
UMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S3-10
FINAL UNLESS ALL	1			3			TOTAL SHEETS
IGNATURES COMPLETED	2			4			46

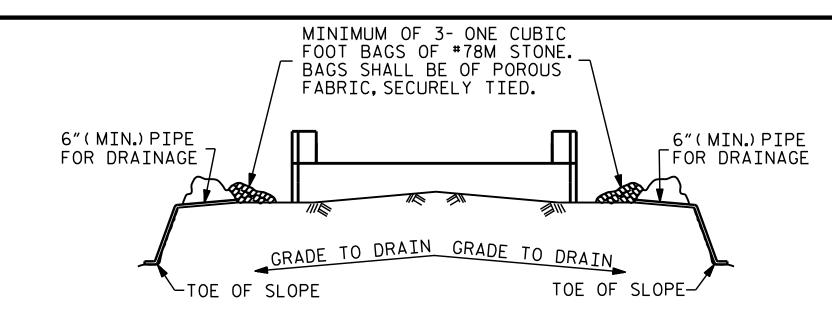


STD. NO. EB_30_75S4_33BB



STD. NO. EB_30_75S4_33BB



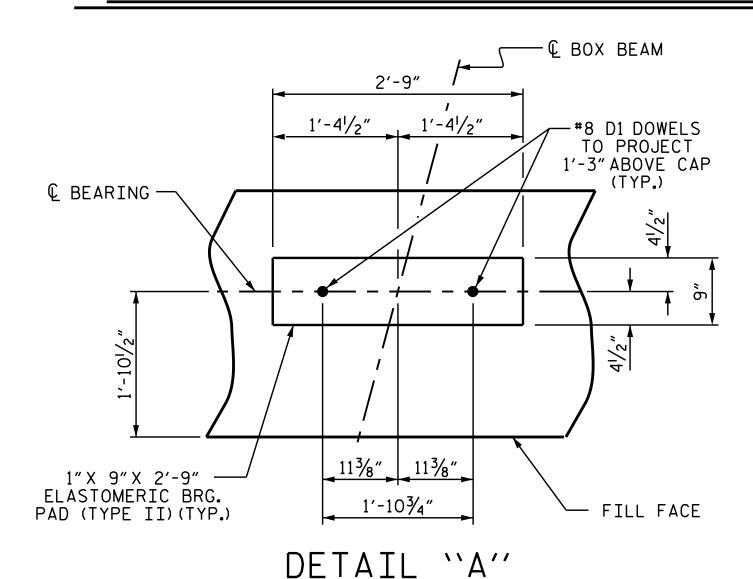


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



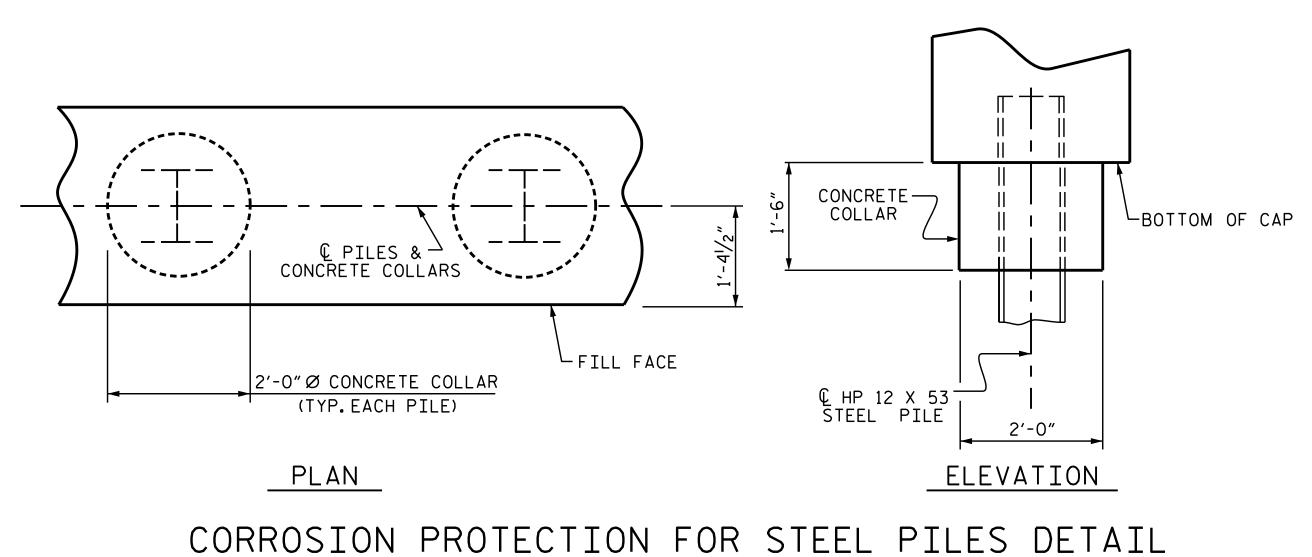
(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

ASSEMBLED BY: R. CAREATHERS/MP DATE: 6/4/15 CHECKED BY: M.D.PISO DATE: 6/25/15

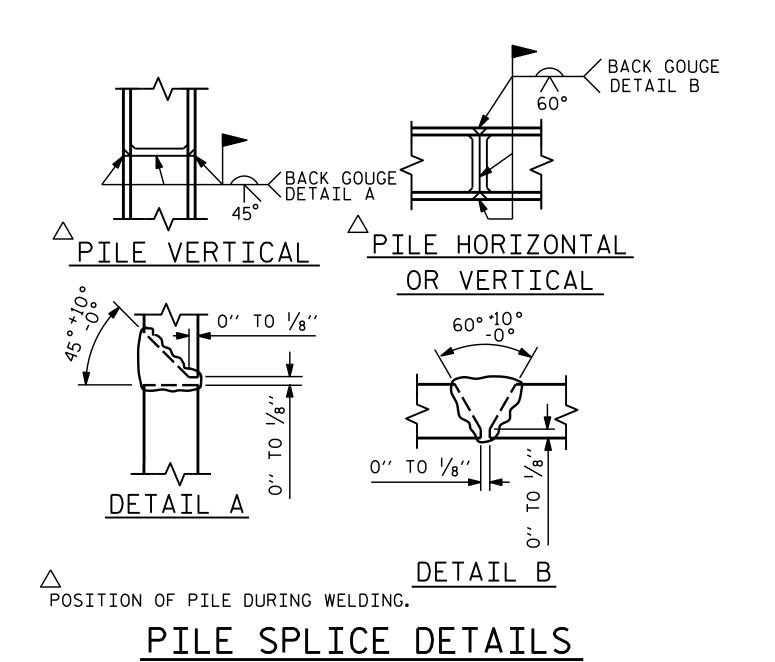
DRAWN BY : WJH 12/11 CHECKED BY : AAC 12/11 REV. 8/14

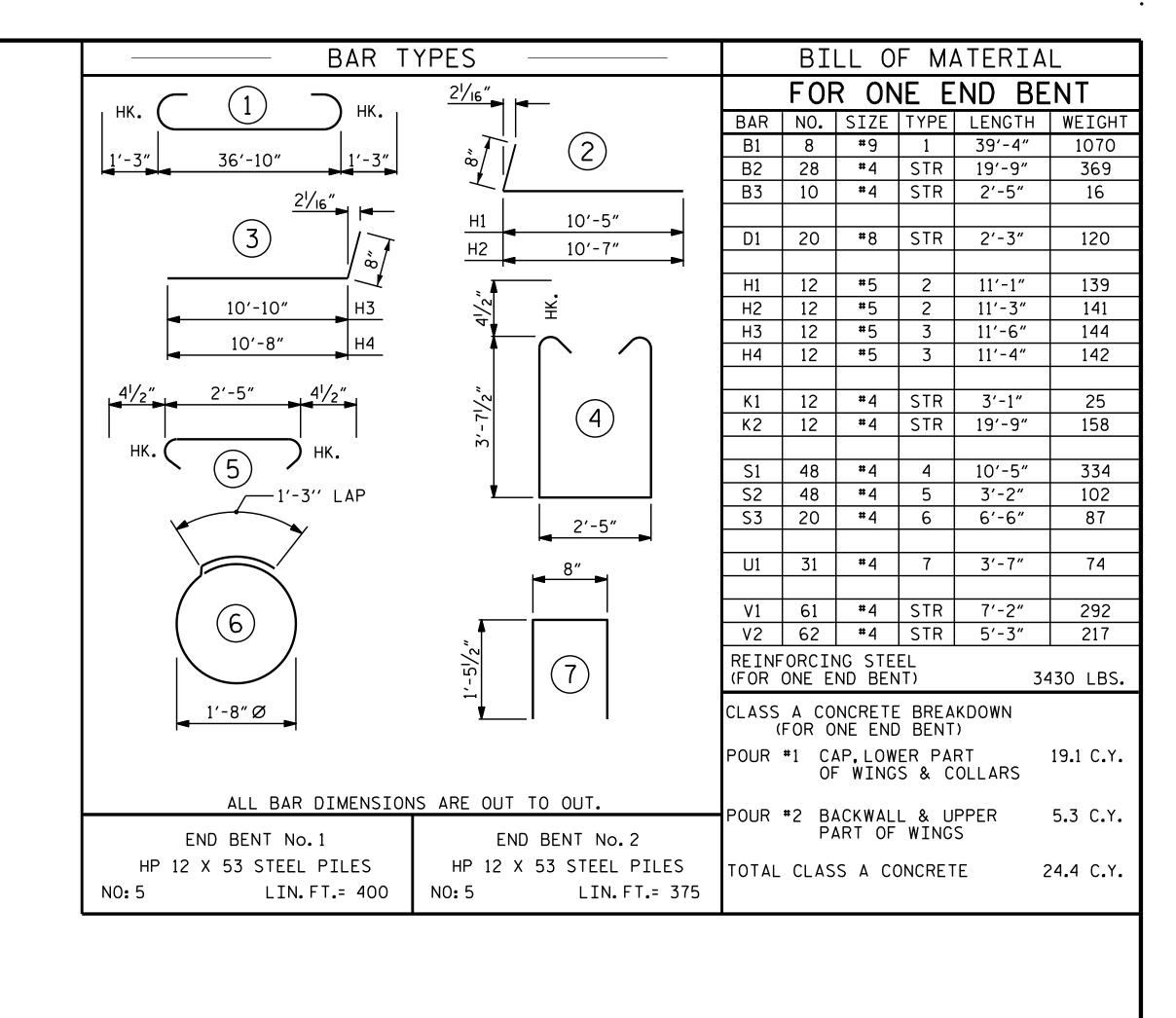
DATE: 6/25/15

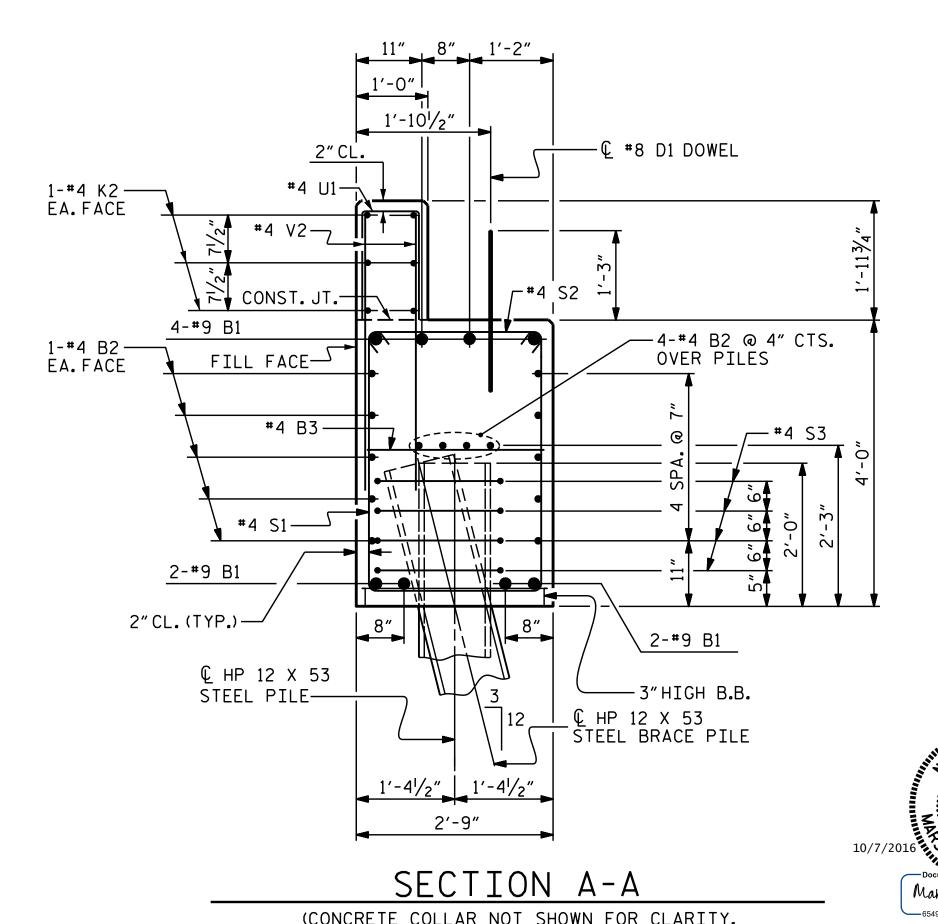
MAA/TMG



(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)







(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

B-4814 PROJECT NO. __ SAMPSON COUNTY 21+15.00 -L-STATION:_

SHEET 4 OF 4

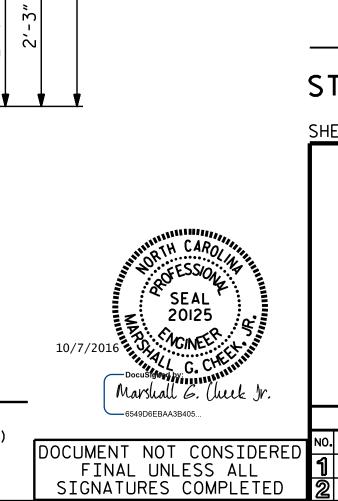
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

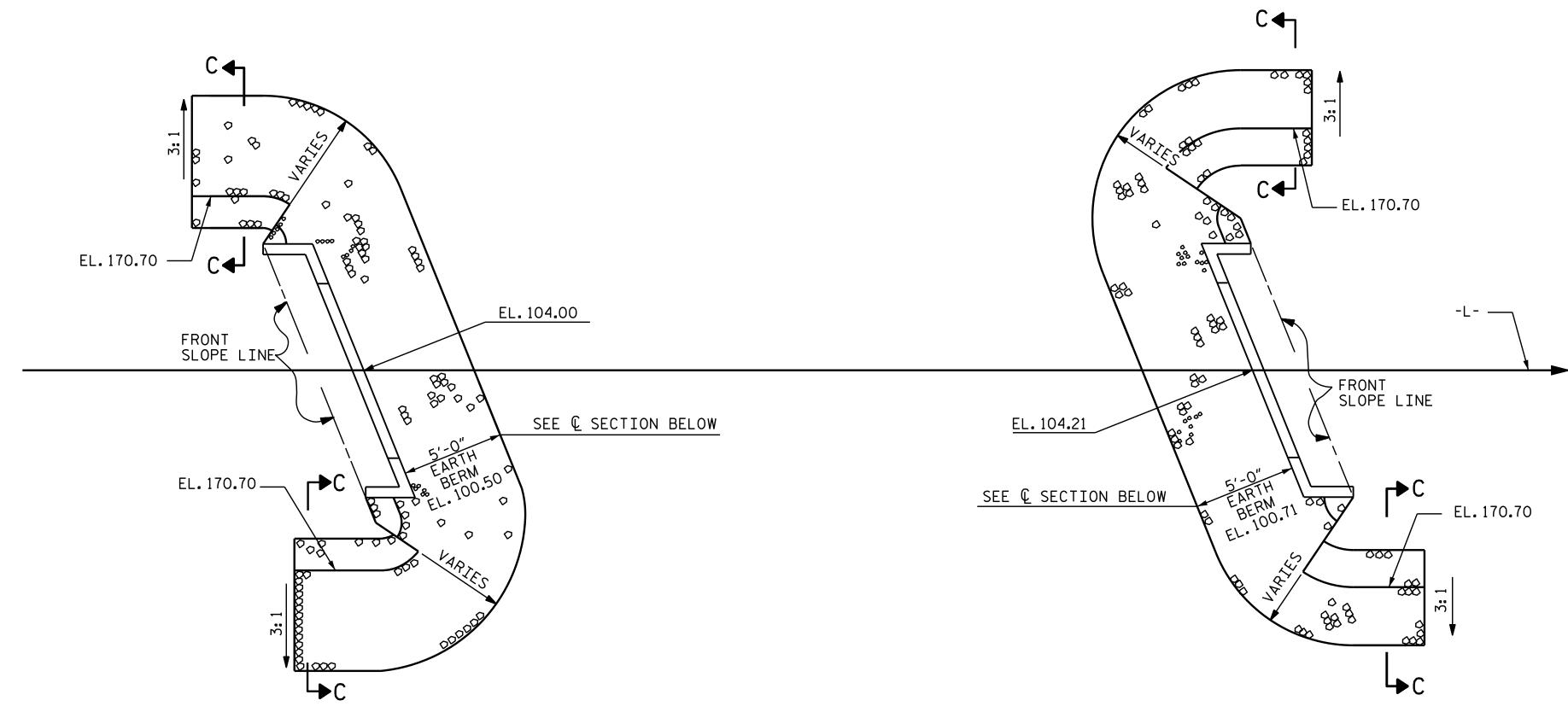
SUBSTRUCTURE

END BENT No.1 & 2 DETAILS

		SHEET NO.					
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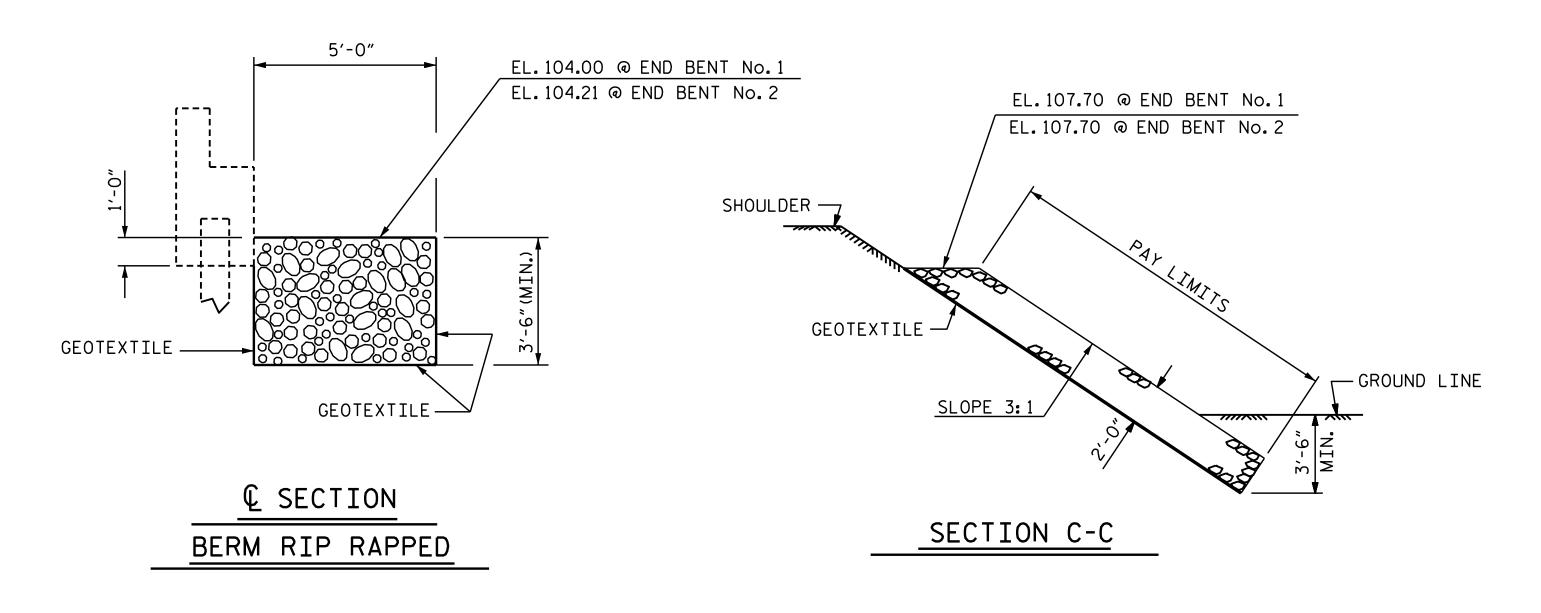
STD. NO. EB_30_75S4_33BB





END BENT No. 2

ESTIMATED QUANTITIES							
BRIDGE @ STA. 21+15.00 -L-	RIP RAP CLASS II	GEOTEXTILE FOR DRAINAGE					
	TONS	SQUARE YARDS					
END BENT No.1	162	180					
END BENT No. 2	162	180					
TOTAL	324	360					



PROJECT NO. B-4814

SAMPSON COUNTY

STATION: 21+15.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALETCH

-RIP RAP DETAILS-

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

REVISIONS

REVISIONS

SHEET NO.

BY:

DATE:

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BY:

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TOTAL SHEETS

46

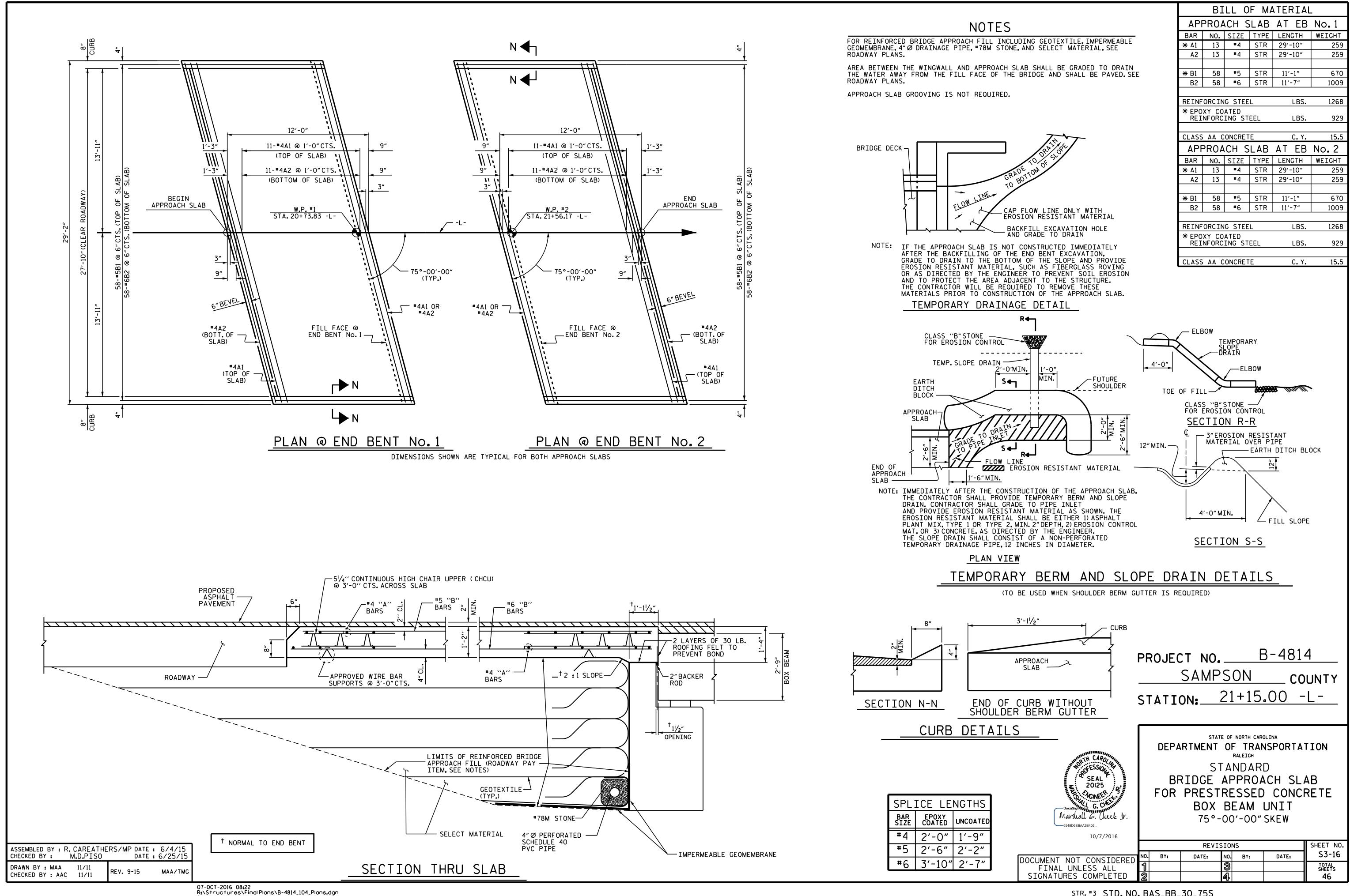
ASSEMBLED BY: R. CAREATHERS/MP DATE: 6/4/15 CHECKED BY: M.D.PISO DATE:6/25/15

DRAWN BY: REK 1/84 REV. 5/1/06R REV. 10/1/11 MAA/GM REV. 12/21/11 MAA/GM

STR. #3

STD. NO. RR1 (Sht 1)

END BENT No. 1



STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) LIVE LOAD ---- SEE PLANS IMPACT ALLOWANCE STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W -27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER - - - -375 LBS. PER SQ. IN.

MATERIAL AND WORKMANSHIP:

EQUIVALENT FLUID PRESSURE OF EARTH

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

30 LBS. PER CU. FT.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.
SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH