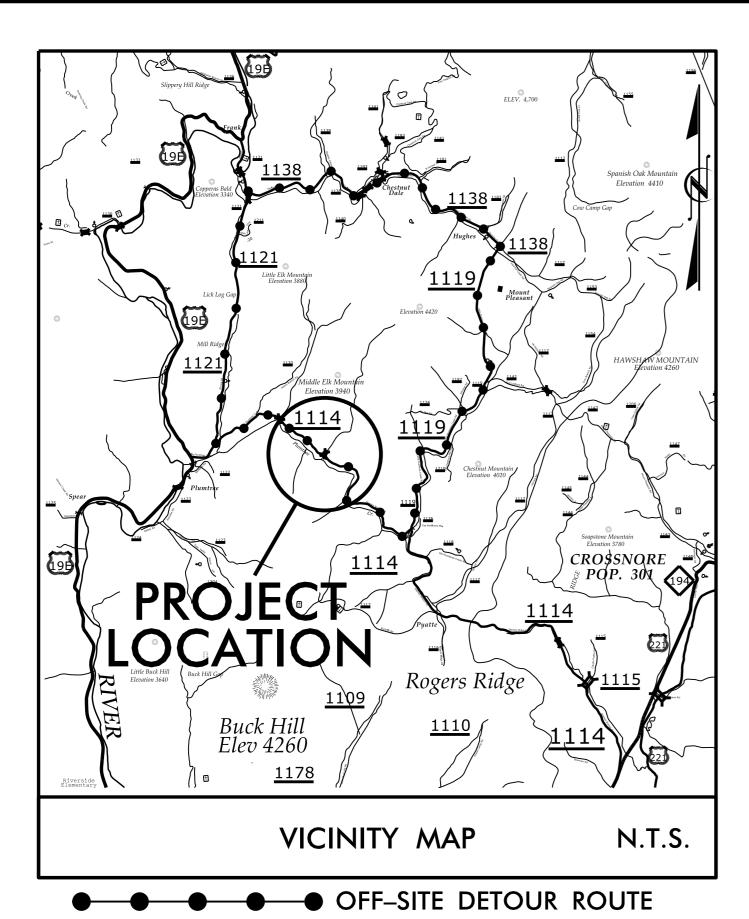
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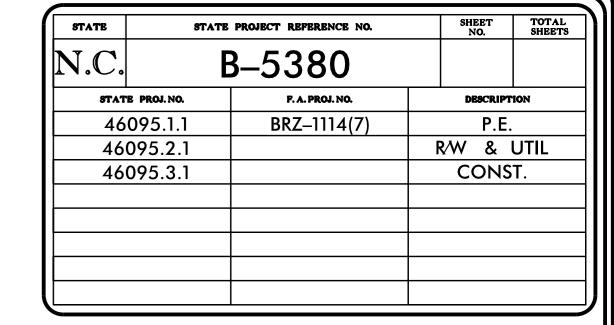
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

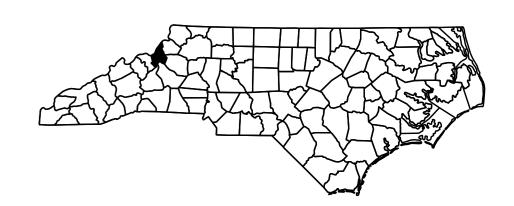
AVERY COUNTY

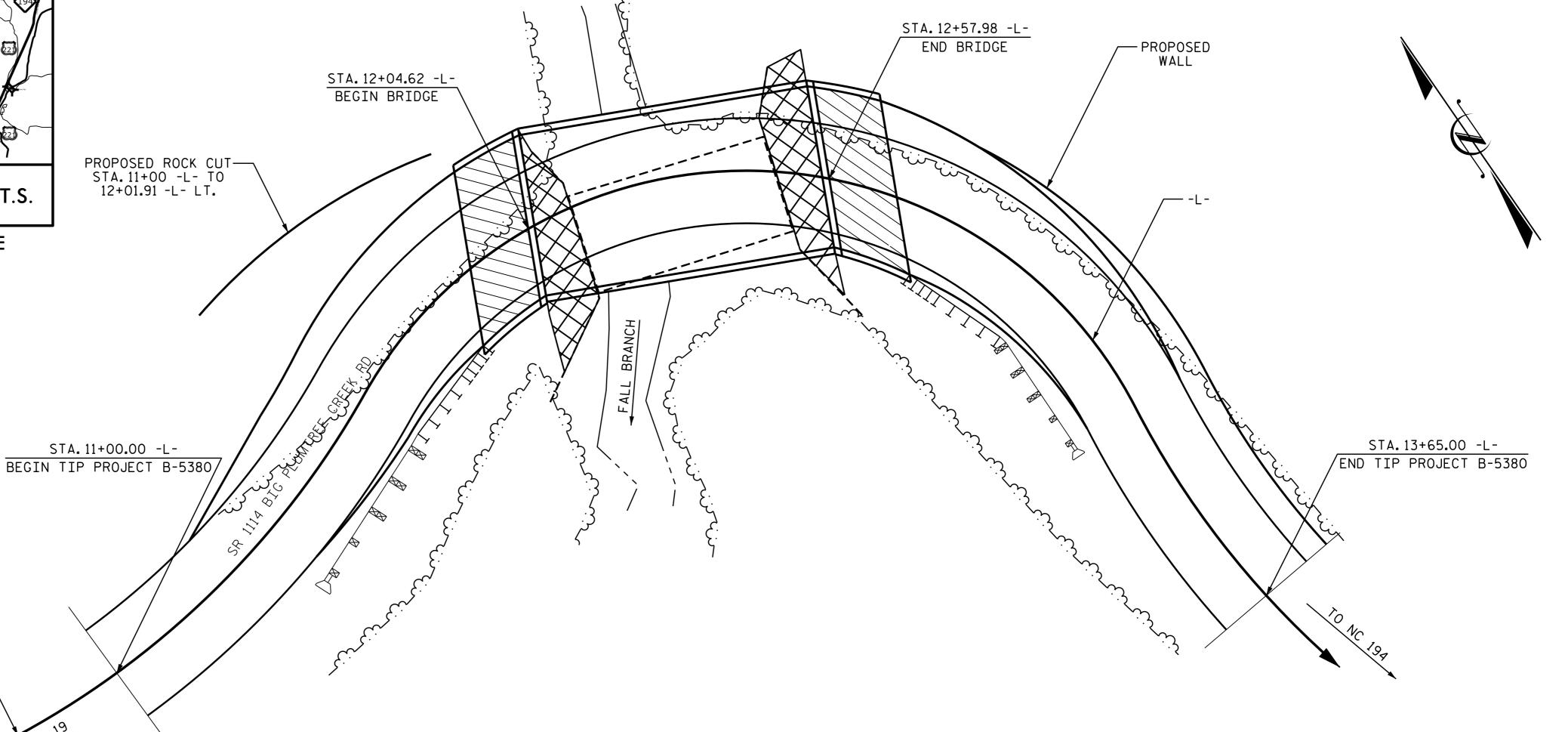
LOCATION: REPLACE BRIDGE NO. 141 ON SR 1114 OVER

FALL BRANCH

TYPE OF WORK: GRADING, DRAINAGE, PAVING, BRIDGE







NORTH CAROLINA AND

DESIGN DATA

STA. 10+80.00 -L-BEGIN CONST.

STRUCTURES

ADT 2016 = 215 ADT 2036 = 286 K = 12 % D = 55 % T = 19 % * V = 25 MPH

* TTST = 1% DUAL 18% FUNC CLASS =

LOCAL RURAL SUB-REGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-5380 = 0.040 MILES
LENGTH STRUCTURE TIP PROJECT B-5380 = 0.010 MILES

TOTAL LENGTH TIP PROJECT B-5380 = 0.050 MILES

Prepared in the Office of:

DIVISION OF HIGHWAYS

STRUCTURES MANAGEMENT UNIT
1000 BIRCH RIDGE DR.

RALEIGH, N.C. 27610

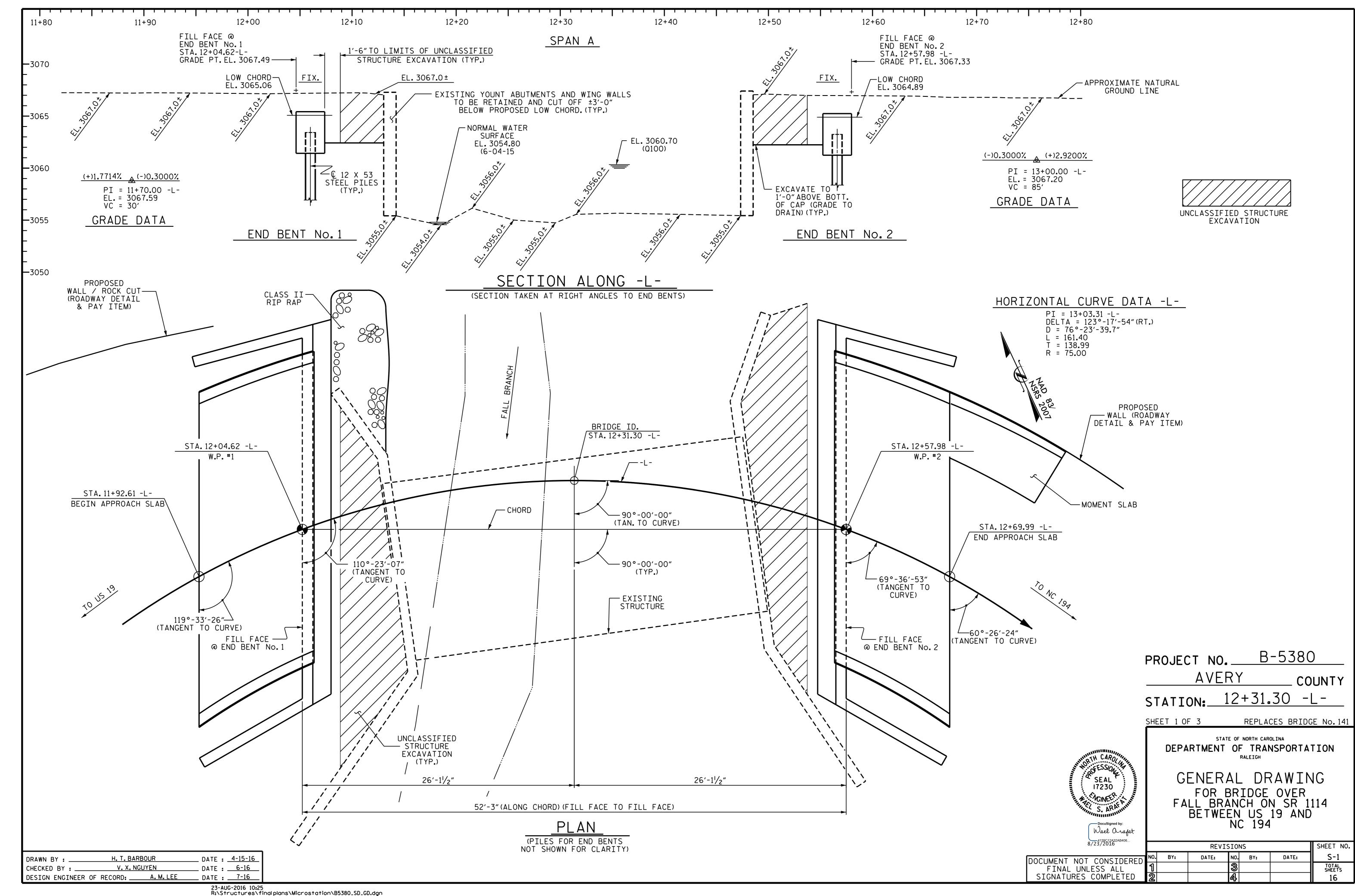
2012 STANDARD SPECIFICATIONS

LETTING DATE:

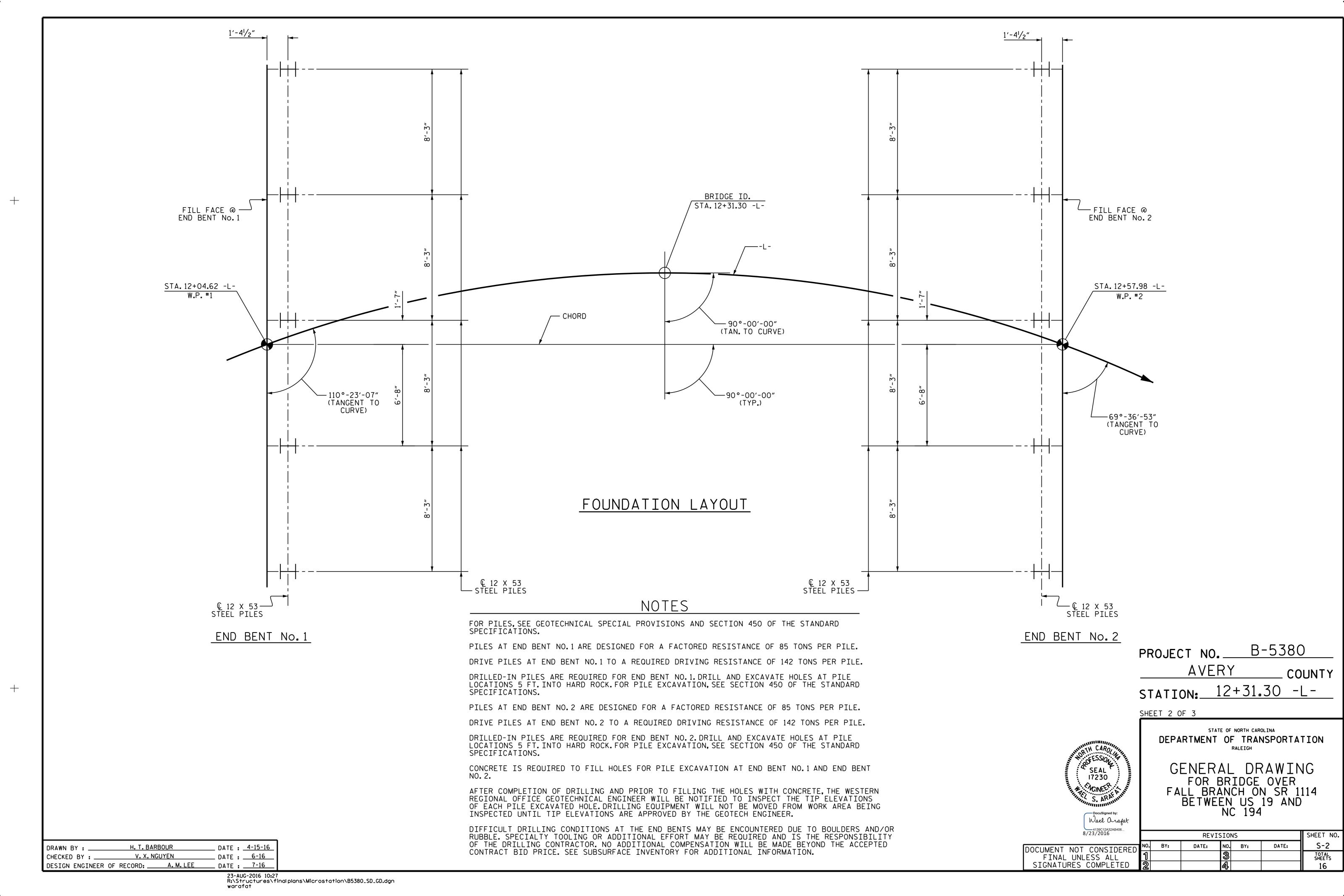
OCTOBER 18, 2016

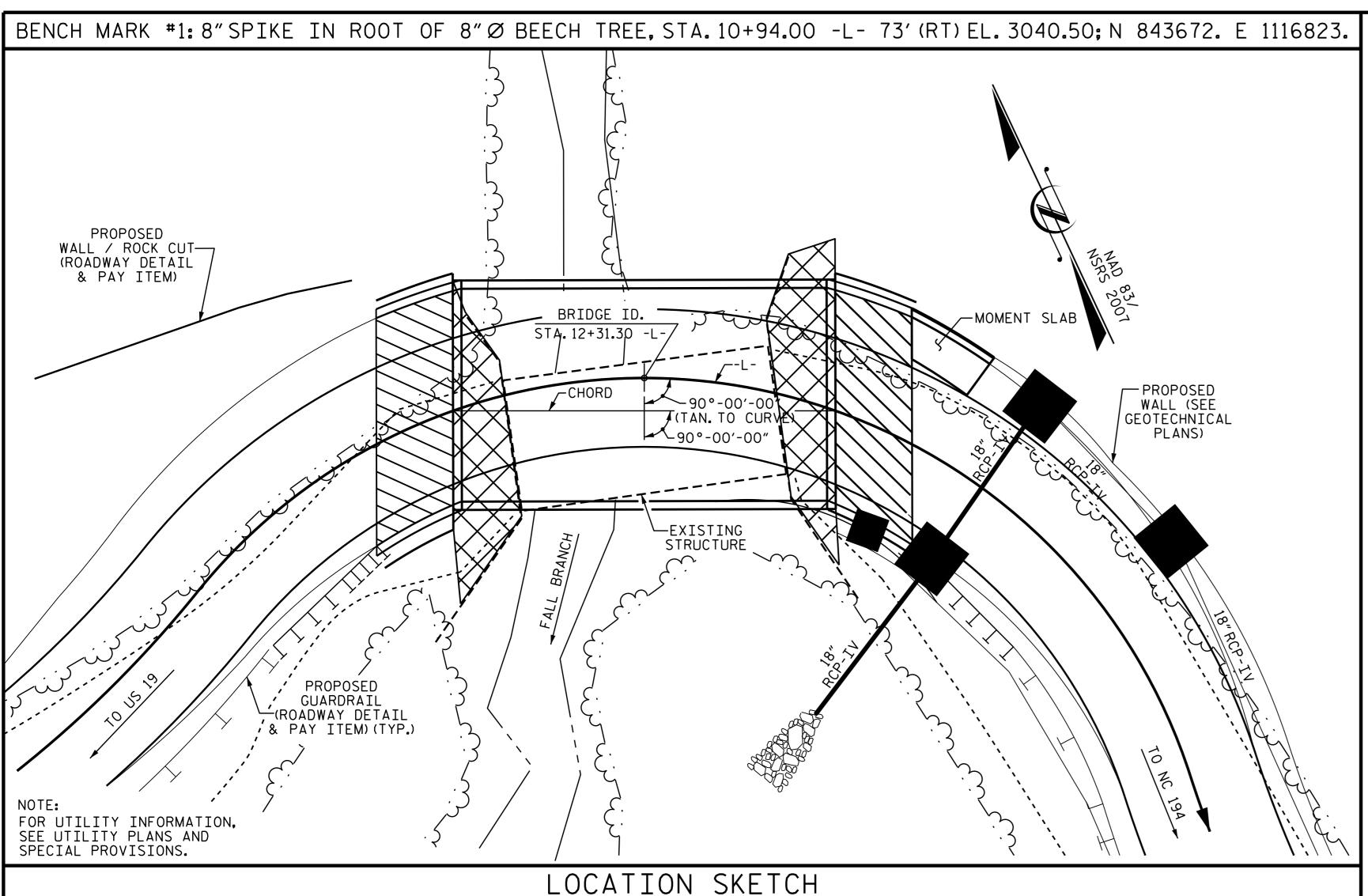
D.R. CALHOUN, PE
PROJECT ENGINEER

W.S. ARAFAT, PE
PROJECT DESIGN ENGINEER



23-AUG-2016 10:25
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warafat





ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

- FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

IN AS MUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE ORFEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 12+31.30 -L-."

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 20 FT.EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF ONE SPAN AT 37.0'; WITH A CLEAR ROADWAY WIDTH OF 15.92', WITH A TIMBER DECK ON STEEL FLOOR BEAMS ON YOUNT MASONRY ABUTMENTS AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

FOR SALVAGE, PACKAGING AND DELIVERY OF ALL EXISTING STEEL I-BEAMS, INTERNAL BRACING, DIAPHRAGMS AND BEARING PLATES, SEE SPECIAL PROVISIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR ASBESTOS ASSEMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

FOR MOMENT SLAB, SEE SPECIAL PROVISIONS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

						— тот.	AL BILL	. 0	F MA	TERIAL							_
	REMOVAL OF EXISTING STRUCTURE	PILE EXCAVATION IN SOIL	PILE EXCAVATION NOT IN SOIL	UNCLASSFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP STE	12 X 53 EL PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'- PR (-0" X 1"-9" EESTRESSED CONCRETE CORED SLABS	ASBESTOS ASSESSMENT	VERTICAL CONCRETE BARRIER RAIL WITH MOMENT SLAB
	LUMP SUM	LIN.FT.	LIN.FT.	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.	LUMP SUM	LIN.FT.
SUPERSTRUCTURE	LUMP SUM			LUMP SUM		LUMP SUM				153 . 60			LUMP SUM	10	500	LUMP SUM	12.20
END BENT NO. 1		55.0	25.0		20.2		2459	5	60.0		11	12					
END BENT NO. 2		20.0	25.0		20.2		2459	5	80.0								
TOTAL	LUMP SUM	75.0	50.0	LUMP SUM	40.4	LUMP SUM	4918	10	140.0	153.60	11	12	LUMP SUM	10	500	LUMP SUM	12.20

HYDRAULIC DATA

DESIGN DISCHARGE ______ 280 CFS FREQUENCY OF DESIGN FLOOD ____ 25 YEARS DESIGN HIGH WATER ELEVATION ____ 3060.20 DRAINAGE AREA _____ 0.62 SQ. MI. BASE DISCHARGE(Q100) _____ 410 CFS BASE HIGH WATER ELEVATION ____ 3060.70

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE_____N/A
FREQUENCY OF OVERTOPPING FLOOD__500 (+) YR.
OVERTOPPING FLOOD ELEVATION____3067.30

SEAL 17230

NOINEER

Docusigned by:

Wael Orafat

DEPARTMENT OF TRANSPORTATION
RALEIGH

PROJECT NO. B-5380

STATION: 12+31.30 -L-

COUNTY

AVERY

SHEET 3 OF 3

GENERAL DRAWING FOR BRIDGE OVER FALL BRANCH ON SR 1114 BETWEEN US 19 AND NC 194

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

REVISIONS
OT CONSIDERED NO. BY: DATE: NO. BY: DATE: S-3
UNLESS ALL
S COMPLETED 2 4 16

H. T. BARBOUR DATE: 4-14-16

V. X. NGUYEN DATE: 6-16

DRAWN BY :

CHECKED BY:

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	NGTH	I LIN	MIT ST	ГАТЕ				SE	SERVICE III LIMIT STATE					
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.037		1.75	0.283	1.83	30′	EL	14.5	0.574	1.04	30′	EL	1.45	0.80	0.283	1.58	30′	EL	14.5	
DESIGN		HL-93(0pr)	N/A		1.344		1.35	0.283	2.38	30′	EL	14.5	0.574	1.34	30′	EL	1.45	N/A						
LOAD RATING		HS-20(Inv)	36.000	<u>2</u>	1.183	42 . 587	1.75	0.283	2 . 53	30′	EL	11.6	0.574	1.18	30′	EL	1.45	0.80	0.283	2.20	30′	EL	11.6	
MATINO		HS-20(0pr)	36.000		1.533	55 . 205	1.35	0.283	3.28	30′	EL	11.6	0.574	1.53	30′	EL	1.45	N/A						
		SNSH	13.500		2.895	39.081	1.40	0.283	5.18	30'	EL	14.5	0.574	2.89	30′	EL	1.45	0.80	0.283	3 . 56	30′	EL	14.5	
		SNGARBS2	20.000		2.240	44.792	1.40	0.283	4.53	30′	EL	11.6	0.574	2.24	30′	EL	1.45	0.80	0.283	3 . 15	30′	EL	11.6	
		SNAGRIS2	22.000		2.157	47.463	1.40	0.283	4.6	30′	EL	11.6	0.574	2.16	30′	EL	1.45	0.80	0.283	3.20	30′	EL	11.6	
		SNCOTTS3	27.250		1.462	39.849	1.40	0.283	2.6	30′	EL	14.5	0.574	1.46	30′	EL	1.45	0.80	0.283	1.79	30′	EL	14.5	
		SNAGGRS4	34.925		1.346	46.999	1.40	0.283	2.5	30'	EL	14.5	0.574	1.35	30′	EL	1.45	0.80	0.283	1.72	30′	EL	14.5	
		SNS5A	35 . 550		1.427	50.733	1.40	0.283	2.42	30′	EL	14.5	0 . 574	1.43	30′	EL	1.45	0.80	0.283	1.67	30′	EL	14.5	
		SNS6A	39.950		1.341	53 . 59	1.40	0.283	2.29	30′	EL	14.5	0.574	1.34	30′	EL	1.45	0.80	0.283	1.58	30′	EL	14.5	
LEGAL		SNS7B	42.000		1.369	57 . 505	1.40	0.283	2 . 23	30′	EL	14.5	0 . 574	1.37	30′	EL	1.45	0.80	0.283	1 . 53	30′	EL	14.5	
LOAD RATING		TNAGRIT3	33.000		1.593	52.58	1.40	0.283	2.97	30′	EL	14.5	0.574	1.59	30′	EL	1.45	0.80	0.283	2.04	30′	EL	14.5	
NAT 1110		TNT4A	33.075		1.483	49.043	1.40	0.283	2.82	30′	EL	14.5	0 . 574	1.48	30′	EL	1.45	0.80	0.283	1.94	30′	EL	14.5	
		TNT6A	41.600		1.433	59 . 622	1.40	0.283	2 . 56	30′	EL	14.5	0.574	1.43	30′	EL	1.45	0.80	0.283	1.76	30′	EL	14.5	
	IST	TNT7A	42.000		1.363	57.264	1.40	0.283	2.64	30′	EL	14.5	0.574	1.36	30′	EL	1.45	0.80	0.283	1.82	30′	EL	14.5	
		TNT7B	42.000		1.331	55 . 915	1.40	0.283	2.49	30′	EL	14.5	0.574	1.33	30′	EL	1.45	0.80	0.283	1.72	30′	EL	14.5	
		TNAGRIT4	43.000		1.287	55.356	1.40	0.283	2.58	30′	EL	14.5	0.574	1.29	30′	EL	1.45	0.80	0.283	1.78	30′	EL	14.5	
		TNAGT5A	45.000		1.381	62.151	1.40	0.283	2.5	30′	EL	14.5	0.574	1.38	30′	EL	1.45	0.80	0.283	1.72	30′	EL	14.5	
		TNAGT5B	45.000	3	1.212	54.54	1.40	0.283	2.41	30′	EL	11.6	0.574	1.21	30′	EL	1.45	0.80	0.283	1.66	30′	EL	11.6	

LOAD FACTORS:

	DESIGN LOAD RATING	LIMIT STATE	$\gamma_{\sf DC}$	$\gamma_{\sf DW}$
		STRENGTH I	1.25	1.50
	FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

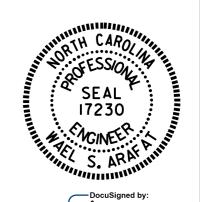
GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-5380 AVERY COUNTY STATION: 12+31.30 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD LRFR SUMMARY FOR 30' CORED SLAB UNIT 90° SKEW

Wael Orafat

(NON-INTERSTATE TRAFFIC)

8/23/2016		
OCUMENT NOT CONSIDERED	NO.	BY:
FINAL UNLESS ALL	1	
SIGNATURES COMPLETED	2	

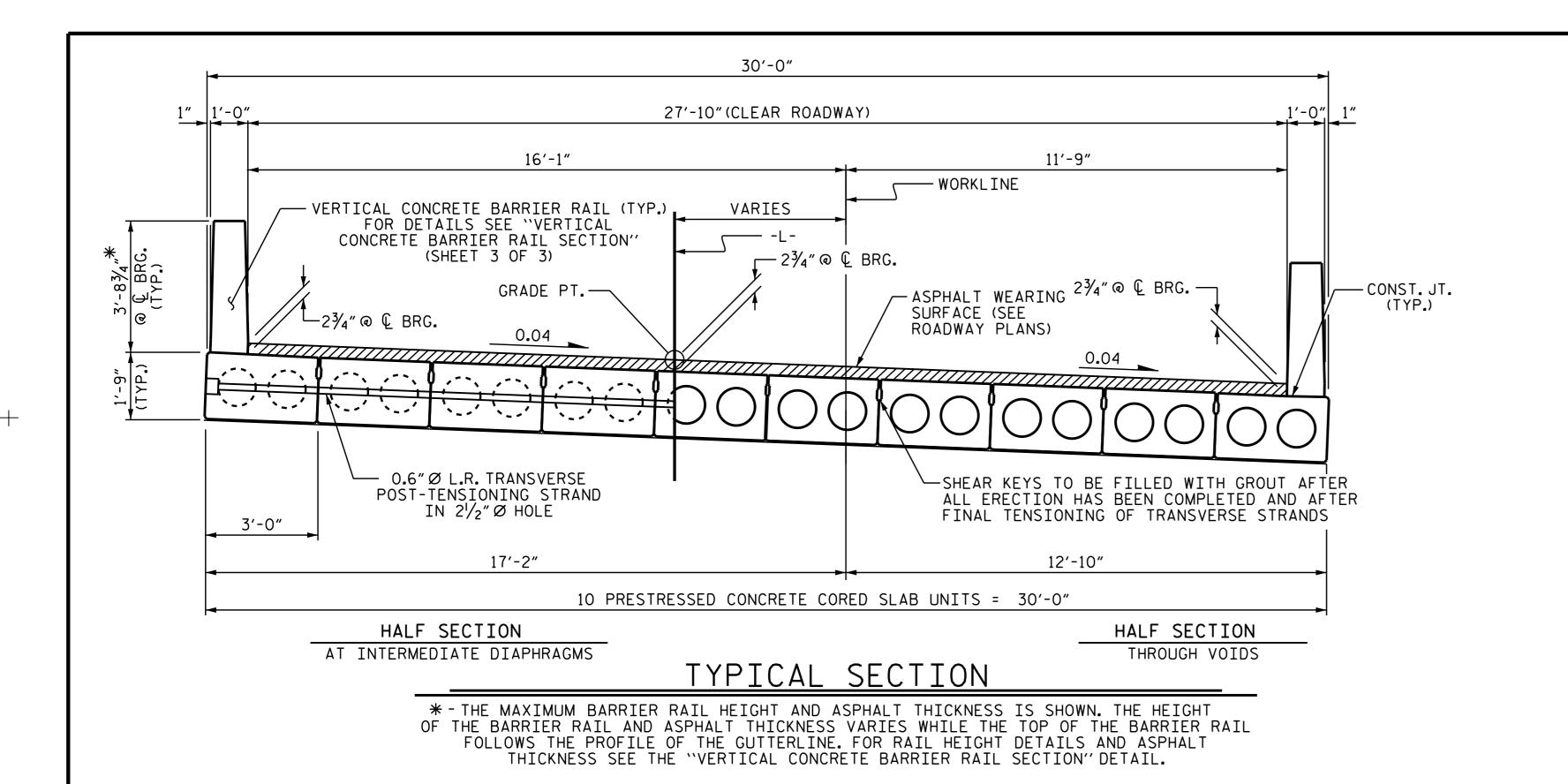
REVISIONS SHEET NO. DATE: NO. BY: DATE:

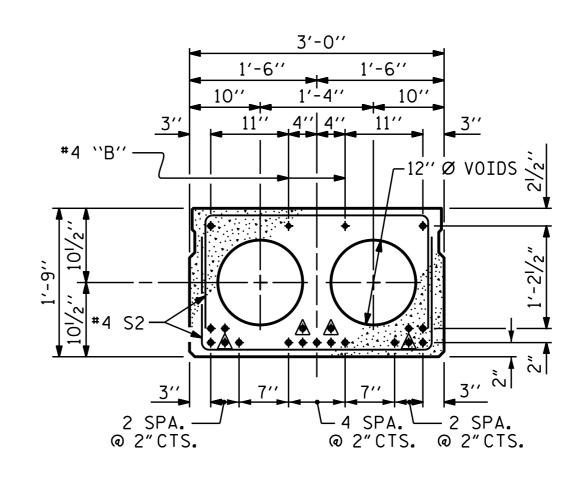
LRFR SUMMARY

FOR SPAN 'A'

DESIGN ENGINEER OF RECORD: A.M.LEE DATE : 7-16 ASSEMBLED BY: H. T. BARBOUR DATE: 4-14-16 CHECKED BY: V. X. NGUYEN DATE: 6-16 DRAWN BY: CVC 6/10 CHECKED BY: DNS 6/10

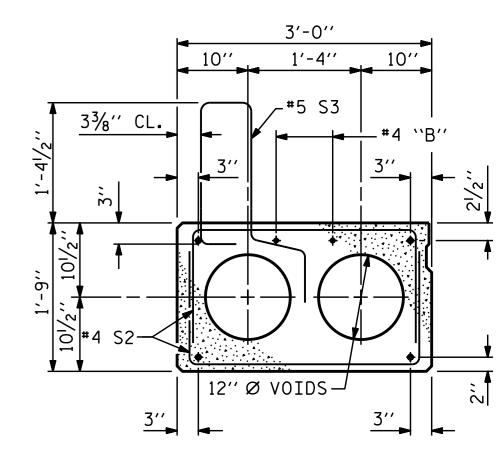
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INTERIOR SLAB SECTION (19 STRANDS REQUIRED)

0.6" Ø LOW RELAXATION STRAND LAYOUT

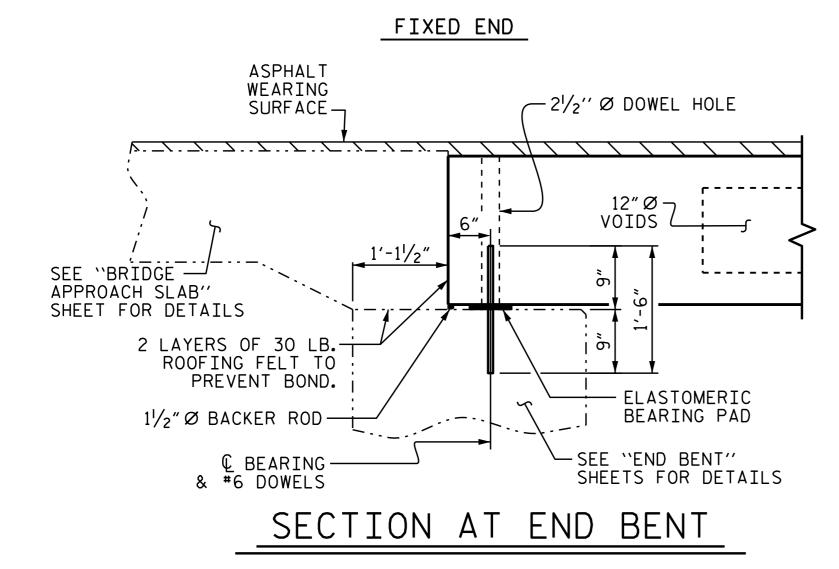


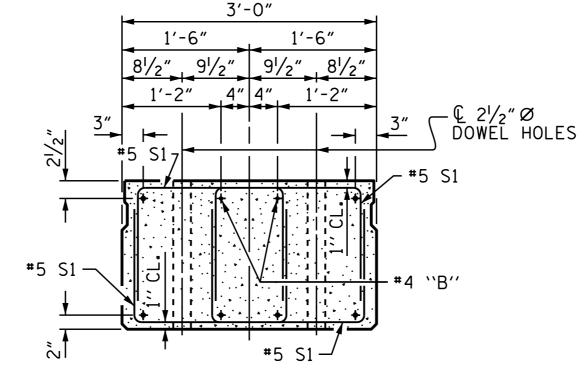
EXT. SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-O" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND





END ELEVATION

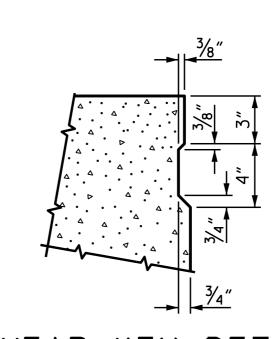
SHOWING PLACEMENT OF DOUBLE STIRRUPS
AND LOCATION OF DOWEL HOLES.
(STRAND LAYOUT NOT SHOWN.) INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

%" X 5" X 5" ₽

-STRAND VISE

—FILL RECESS

WITH GROUT



SHEAR KEY DETAIL NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.

PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF EXTERIOR UNIT AND RECESSED 3/8". SIZE TO BE DETERMINED BY CONTRACTOR. THREADED INSERT DETAIL

> PROJECT NO. B-5380 **AVERY** COUNTY

STATION: 12+31.30-L-

SHEET 1 OF 3

BY:

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW

NO. BY:

REVISIONS

DATE:

SHEET NO.

S-5

TOTAL SHEETS

DATE:

SEAL 17230 S NOINEER

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Wael Orafat

8/23/2016

GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS

OUTSIDE FACE — OF EXTERIOR 1/2 CORED SLAB

€ 0.6" Ø L.R. TRANSVERSE POST-TENSIONING STRAND

SECTION B-B

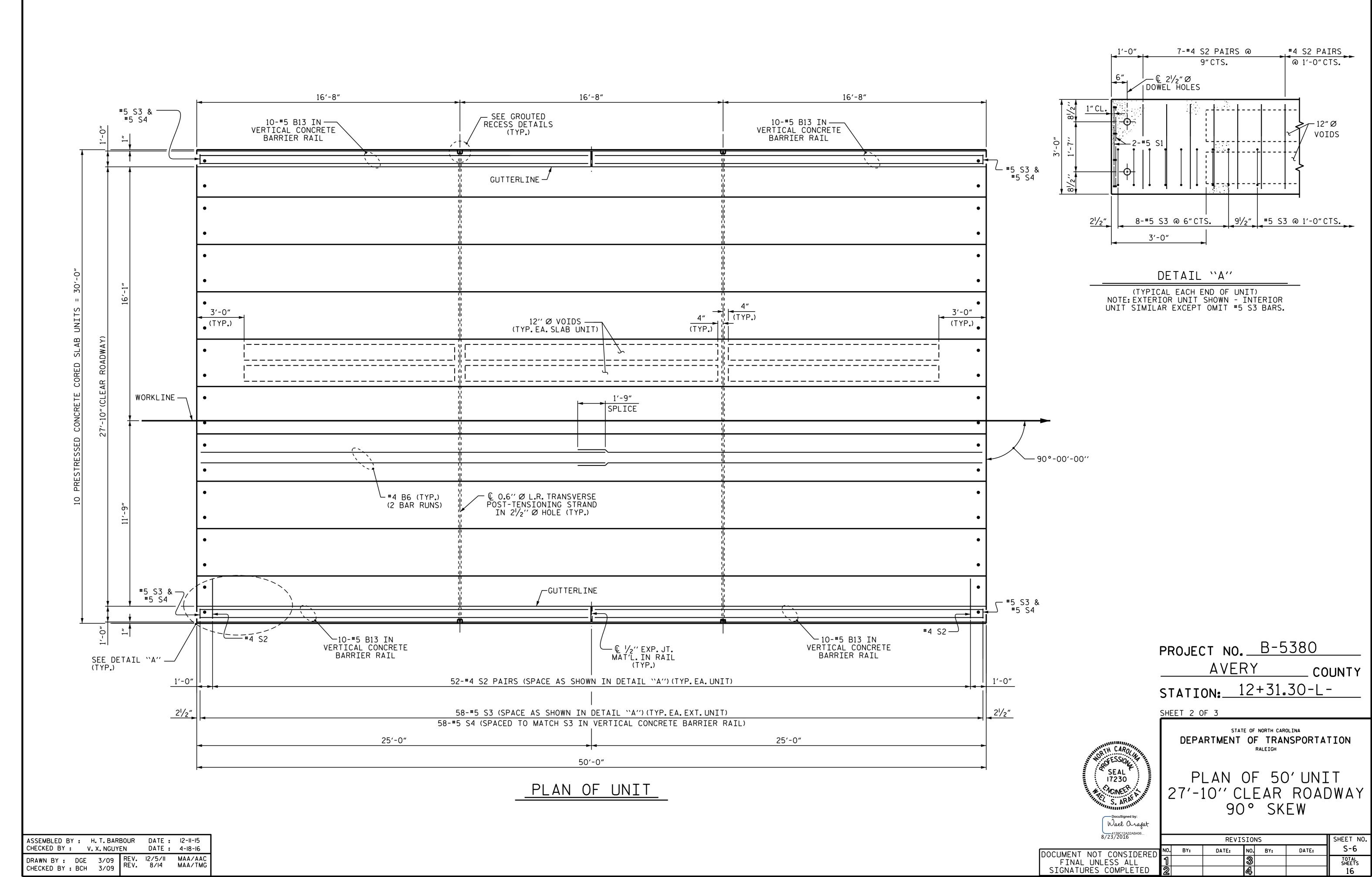
SHEATHED WITH A -

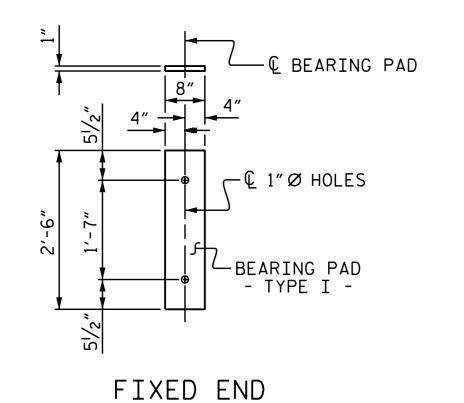
NON-CORROSIVE PIPE.

-HOLE FOR TRANSVERSE STRAND

ELEVATION VIEW

ASSEMBLED BY: H. T. BARBOUR DATE: II-IO-I5 V. X. NGUYEN DATE: 4-18-16 CHECKED BY : DRAWN BY: DGE 5/09 CHECKED BY: BCH 6/09 MAA/TMG REV. 8/14





ELASTOMERIC BEARING DETAILS

10"

— #5 S4

2"CL.MIN.

CONST.JT. —

DATE : 4-18-16

MAA/TMC

ASSEMBLED BY: H. T. BARBOUR DATE: 12-11-15

V. X. NGUYEN

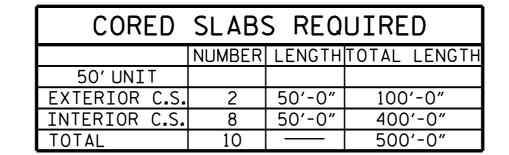
CHECKED BY :

DRAWN BY: DGE 5/09

CHECKED BY : BCH 6/09

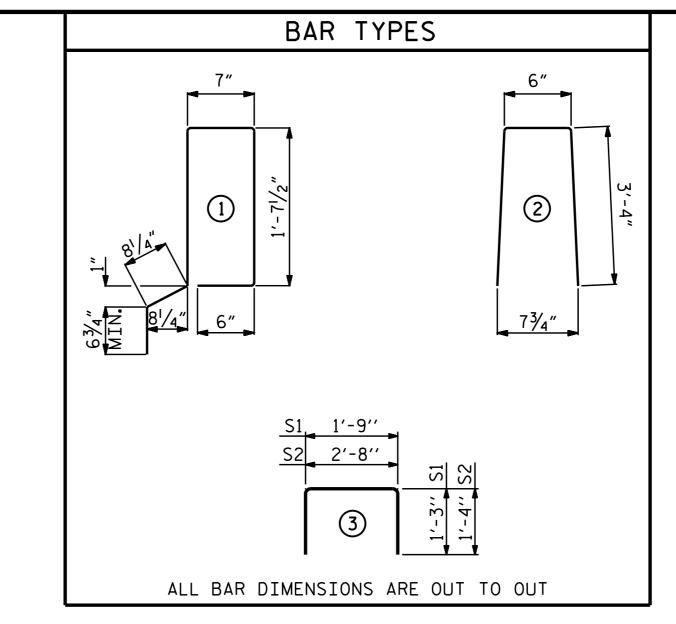
(TYPE I - 20 REQ'D)

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.



DEAD LOAD DEFLECTION AND	ND CAMBER
	3'-0" × 1'-9"
50' CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	11/2"
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	3/8″ ♦
FINAL CAMBER	11/8″ 🛉

** INCLUDES FUTURE WEARING SURFACE



50' CORED SLAB UNIT EXTERIOR UNIT INTERIOR UNIT |NUMBER | SIZE | TYPE | LENGTH | WEIGHT LENGTH | WEIGHT В6 #4 | STR I 25′-9″ 25'-9" 4 69 69 4'-3" 35 4'-3" 35 S2 #4 5′-4″ 371 5'-4" 371 104 #5 5′-7" * S3 58 338 475 475 REINFORCING STEEL LBS. * EPOXY COATED REINFORCING STEEL 338

BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL								
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT		
	50' UNIT							
∗ B13	40	40	#5	STR	24'-7"	1026		
* S4	116	116	#5	2	7'-2"	867		
* EPOXY COATED REINFORCING STEEL LBS. 1893								
CLASS AA CONCRETE CU.YDS. 19.5								
△ TOTAL VERTICAL CONCRETE BARRIER RAIL LN.FT. 153.6								

△ INCLUDES VERTICAL CONCRETE BARRIER RAIL ON APPROACH SLABS.

#5 S4-

10" 1"

#5 S3—

BILL OF MATERIAL FOR ONE 6500 P.S.I. CONCRETE CU. YDS. 7.1 7.1 19 19 0.6" Ø L.R. STRANDS No.

· · · -					
RIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	40	#5	STR	24'-7"	1026
	116	#5	2	7′-2″	867
EEL			LBS.		1893
			CU.YDS.		19.5
RIER RAIL			LN.FT.		153.6
ADDIED DATI	ON ADDDOACH	CLADC			

#5 S3 & S4

8-#5 S3 & S4 @ 6"CTS.

GUTTERLINE ASPHALT THICKNESS & RAIL HEIGHT ASPHALT OVERLAY THICKNESS RAIL HEIGHT @ MID-SPAN @ MID-SPAN

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE

REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD

GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST

PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS,

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE

JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT

CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS.

STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN

BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO

SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK

THE PRICE BID FOR THE PRECAST UNITS.

STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE

825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION

LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE

SPECIFICATIONS.

PRESTRESSED CONCRETE CORED SLABS.

TENSIONING OF THE STRANDS.

SHALL BE EPOXY COATED.

FEET IN LENGTH.

"CONCRETE RELEASE STRENGTH" TABLE.

FILLED WITH NON-SHRINK GROUT.

50' UNITS 3'-75/8" 15/8"

CONCRETE RELEASE STRENGTH PSI UNIT 50′ 4900

0.6" Ø L.R

B-5380 PROJECT NO. _ **AVERY** COUNTY STATION: 12+31.30-L-

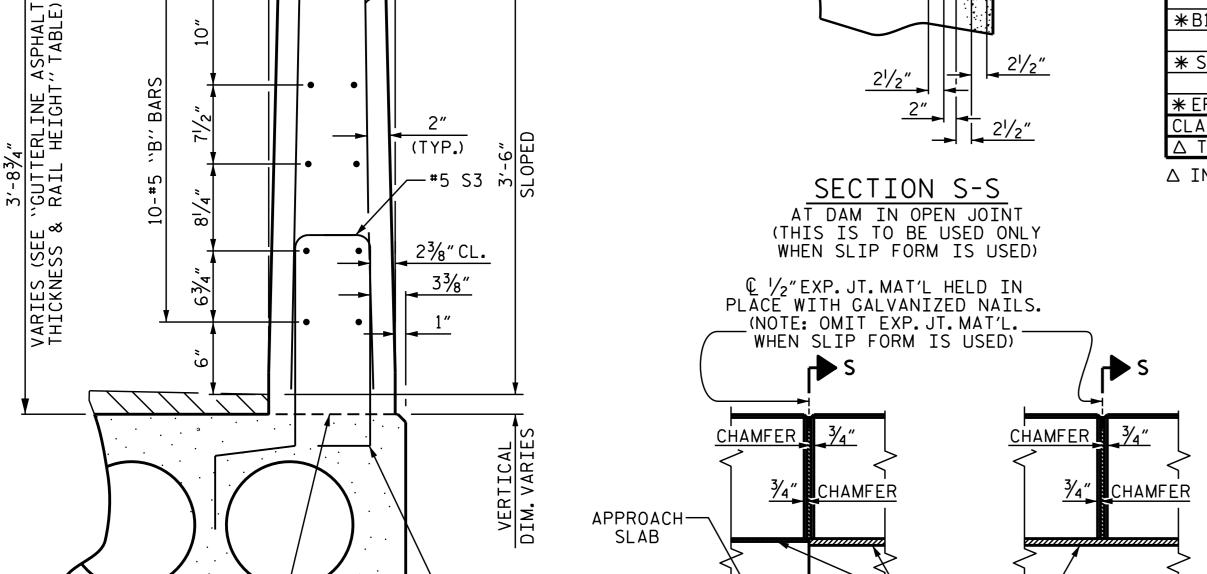
SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD 3'-0'' X 1'-9''

PRESTRESSÉD CONCRETE CORED SLAB UNIT 90° SKEW

06							
00			REVIS	SION	S		SHEET N
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LIVED	1			3			TOTAL SHEETS
ED	2			4			16



#5 S3 (SEE "PLAN OF UNIT" FOR SPACING)

VERTICAL CONCRETE BARRIER RAIL SECTION

-#5 S3 (TYP.) CONST. JT. END VIEW SIDE VIEW END OF RAIL DETAILS

0.217 (SQUARE INCHES) ULTIMATE STRENGT (LBS.PER STRAND 58,600 APPLIED PRESTRESS 43,950 (LBS.PER STRAND

GRADE 270 STRANDS

Wael Orafat 4139C12A32AB4

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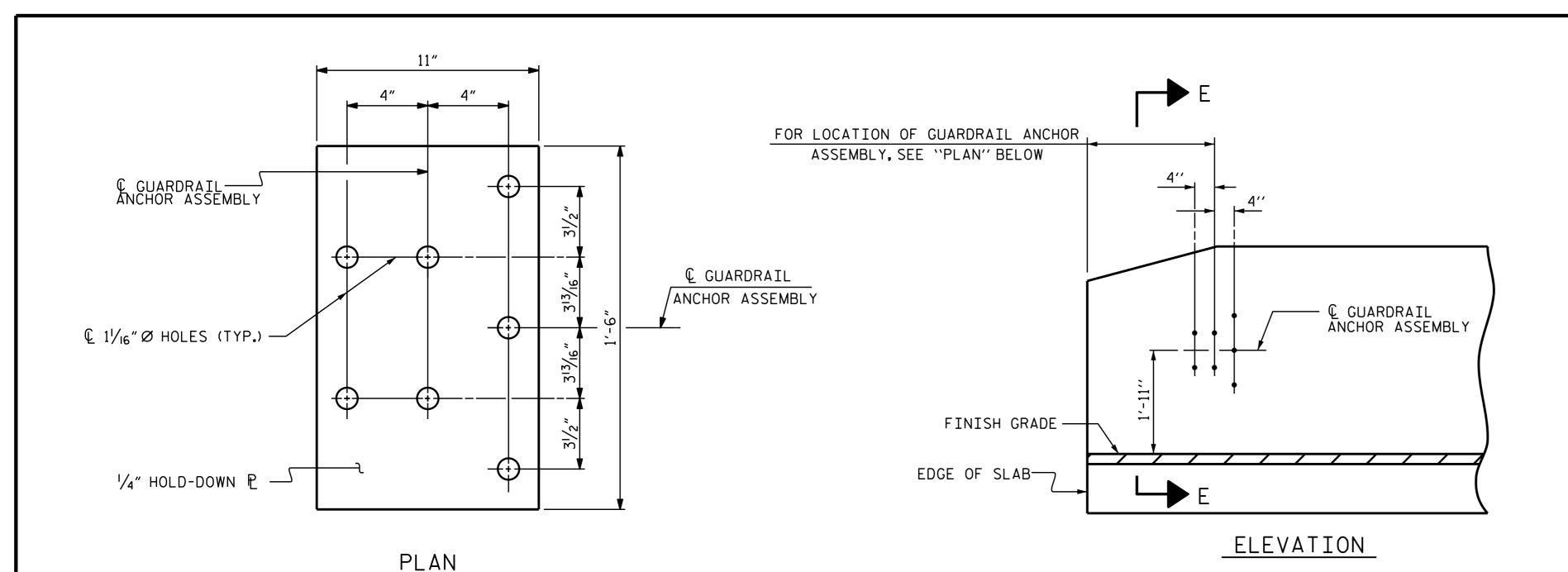
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ELEVATION AT EXPANSION JOINTS

STD. NO. 21" PCS3_30_90S



THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

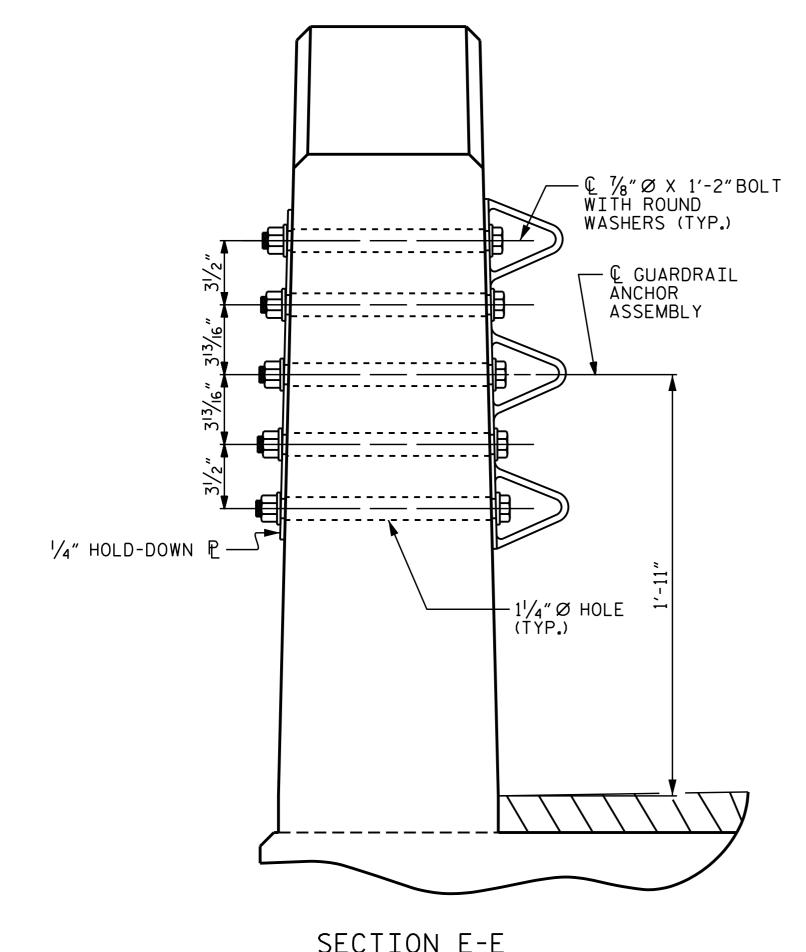
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SECTION E-E GUARDRAIL ANCHOR ASSEMBLY DETAILS

ASSEMBLED BY: H.T.BARBOUR CHECKED BY: V.X.NGUYEN

DRAWN BY: MAA 5/10 REV. 12/5/II REV. 6/13 REV. 1/15

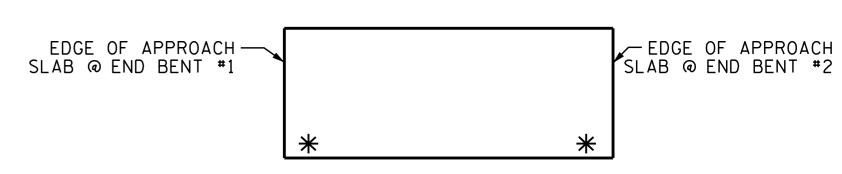
DATE : 12-11-15 DATE : 4-18-16

MAA/GM MAA/GM MAA/TMG

 GUARDRAIL
ANCHOR ASSEMBLY 1'-10" EDGE OF SLAB € GUARDRAIL ANCHOR ASSEMBLY PLAN LOCATION OF

ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

B-5380 PROJECT NO. ___ AVERY COUNTY 12+31.30-L-STATION:_



DEPARTMENT OF TRANSPORTATION STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE

STATE OF NORTH CAROLINA

SHEET NO.

S-8

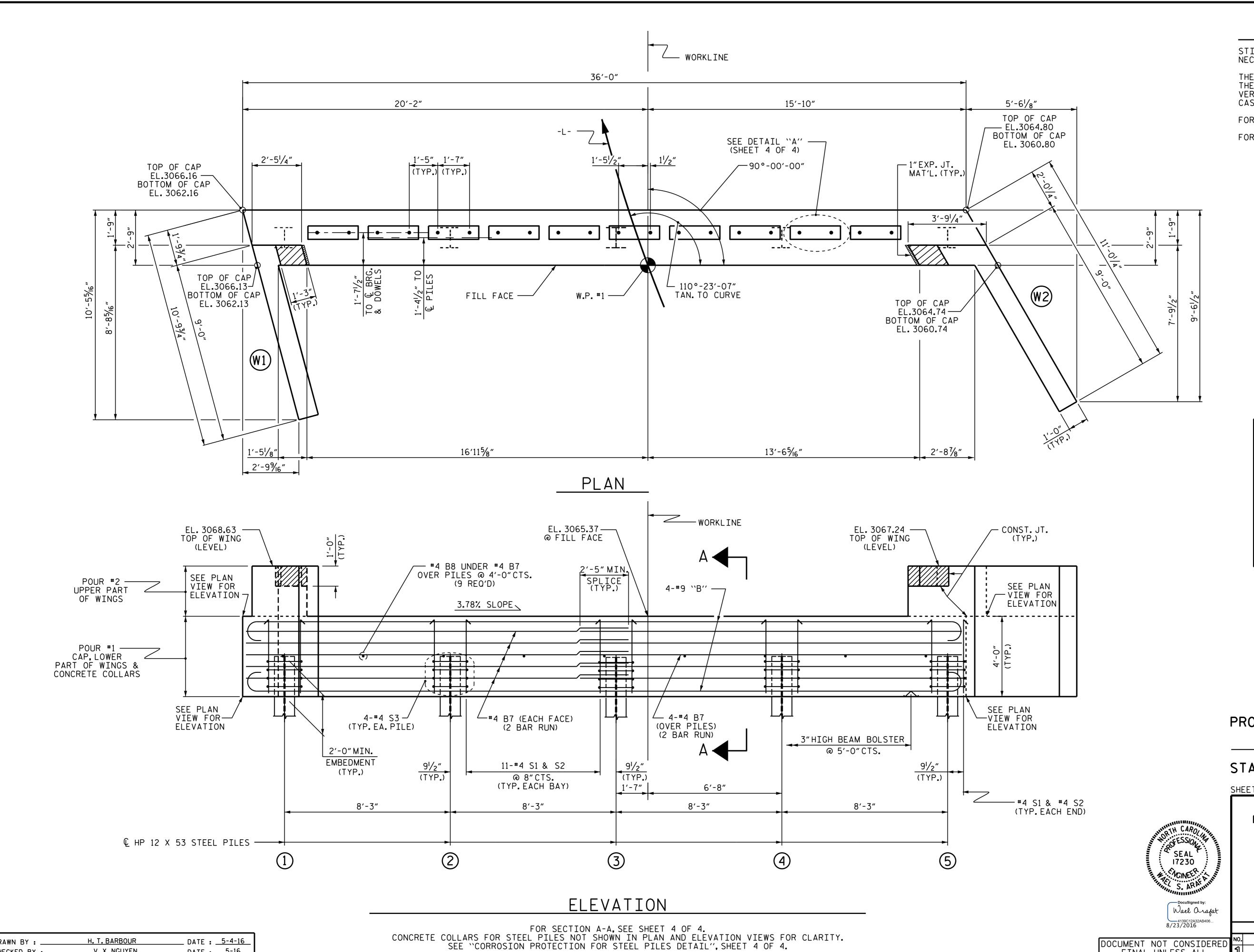
Wael Orafat

BARRIER RAIL 4139C12A32AB406... 8/23/2016 **REVISIONS** BY: DATE:

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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STD. NO. GRA3



STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

TOP OF PILE ELEVATIONS								
1	EL 3064.08							
2	EL. 3063.77							
3	EL. 3063.46							
4	EL. 3063.15							
5	EL. 3062.83							

PROJECT NO. B-5380

AVERY COUNTY

STATION: 12+31.30 -L-

SHEET 1 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE

END BENT No. 1

REVISIONS SHEET NO. S-9 NO. BY: DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED DATE: TOTAL SHEETS

_ DATE : <u>5-4-16</u>

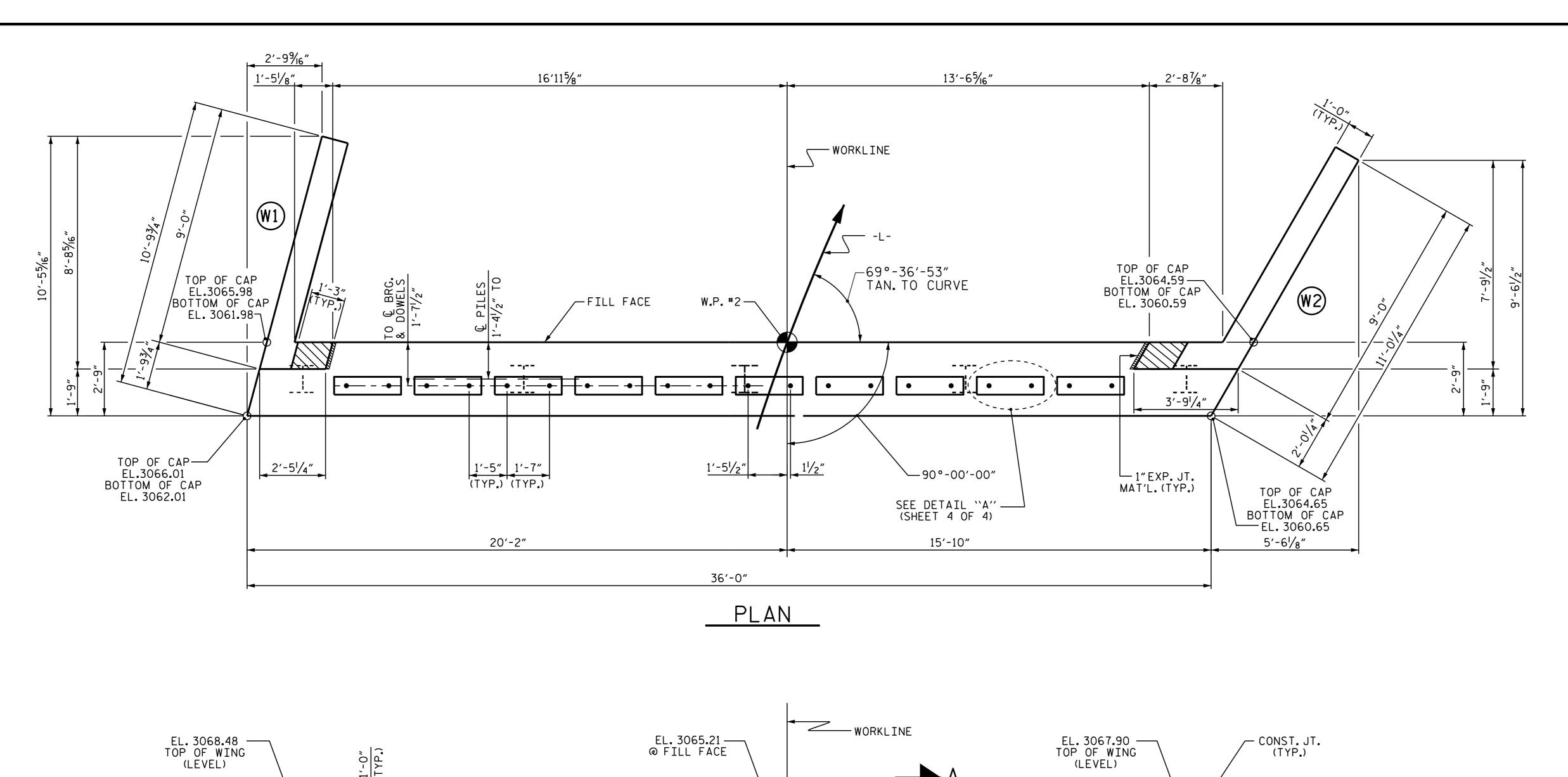
H. T. BARBOUR

DESIGN ENGINEER OF RECORD: A.M.LEE

V. X. NGUYEN

DRAWN BY :

CHECKED BY : _



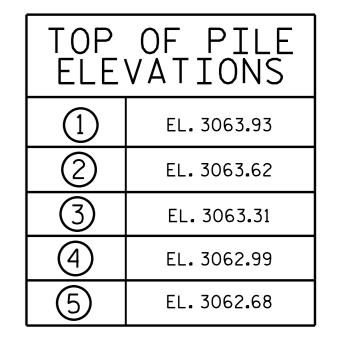


STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.



PROJECT NO. B-5380 AVERY COUNTY

STATION: 12+31.30 -L-

SHEET 2 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

END BENT No. 2

SHEET NO.

S-10

DATE:

Wael Orafat 4139C12A32AB406... 8/23/2016 REVISIONS NO. BY: DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

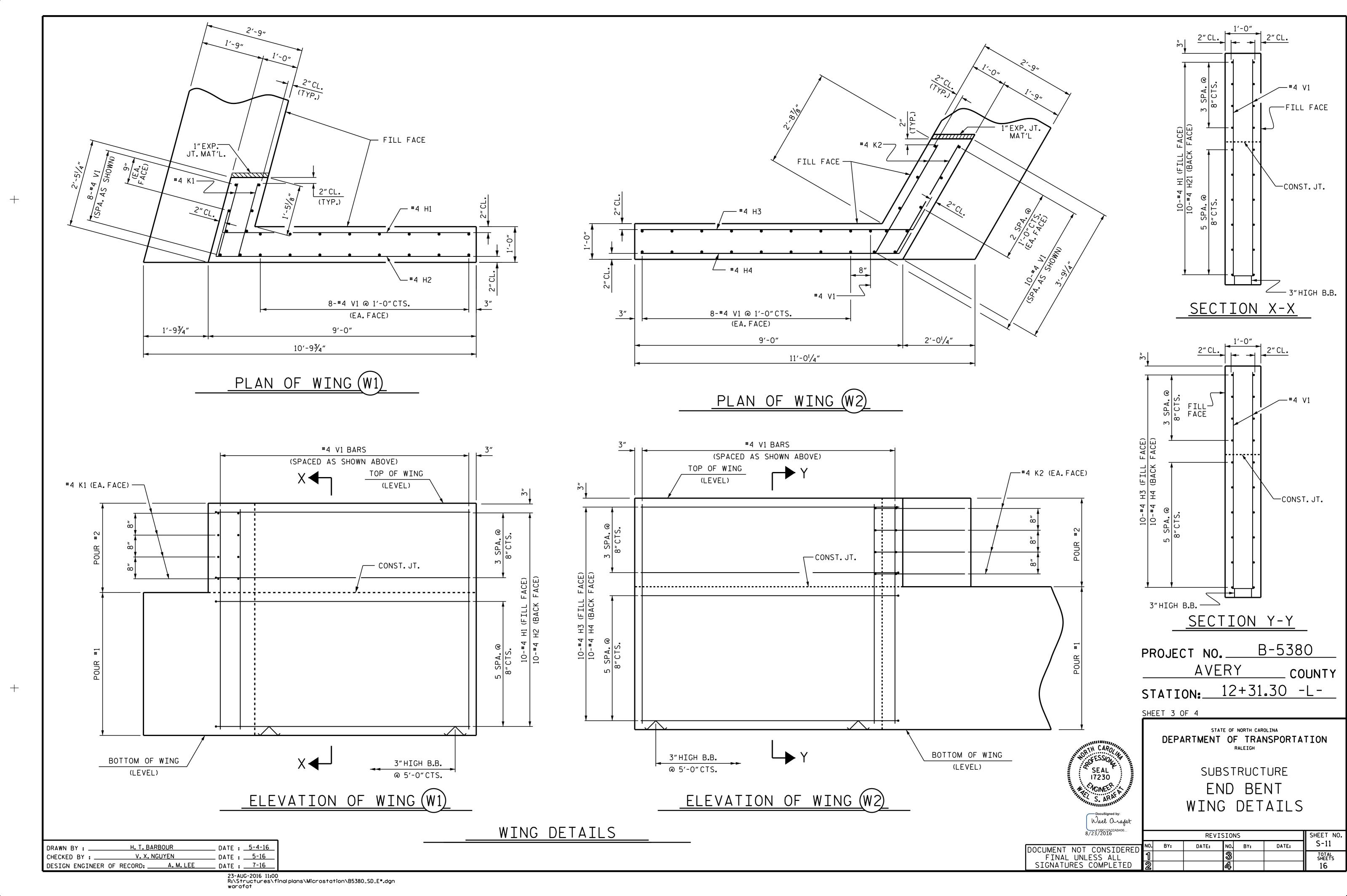
TOP OF (LEV	WING (ab)		@ FILL FACE	TOP O	WING (TYP.)
UDDED DADT VIEW	PLAN / FOR ATION 7	#4 B8 UNDER #4 B7 OVER PILES @ 4'-0"CTS. (9 REQ'D) 3.78% SLOPE	2'-5" MIN. SPLICE (TYP.) 4-#9 "B" -	A 7	SEE PLAN I VIEW FOR I ELEVATIONI
POUR #1 CAP, LOWER PART OF WINGS & CONCRETE COLLARS					4'-0" (TYP.)
SEE PLA VIEW F ELEVAT	AN OR 4-#	9 ¹ / ₂ " 14 S3 - #4 B7 (EACH (2 BAR II)	4-#4 B7 (OVER PILES) (2 BAR RUN) & S2) .	SEE PLAN VIEW FOR ELEVATION
	8′-3″	(TYP. EACI	H BAY) 1'-7" 6'-	8′-3″	#4 S1 & #4 S2 (TYP. EACH END)
© HP 12 X 53 STEE	IL PILES 1	2	3	4	5
			ELEVATION		

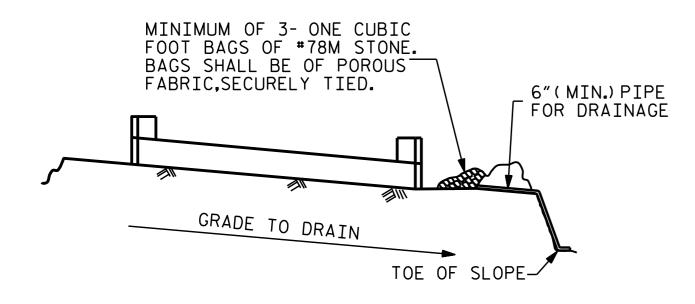
__ DATE : <u>5-4-16</u> V. X. NGUYEN DESIGN ENGINEER OF RECORD: A.M.LEE

H. T. BARBOUR

DRAWN BY :

FOR SECTION A-A, SEE SHEET 4 OF 4.
CONCRETE COLLARS FOR STEEL PILES NOT SHOWN IN PLAN AND ELEVATION VIEWS FOR CLARITY.
SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL", SHEET 4 OF 4.



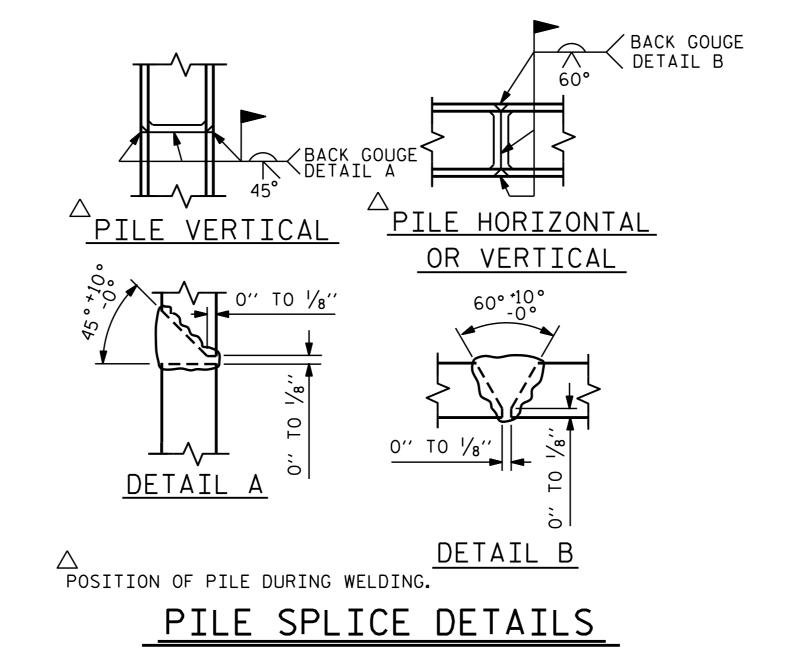


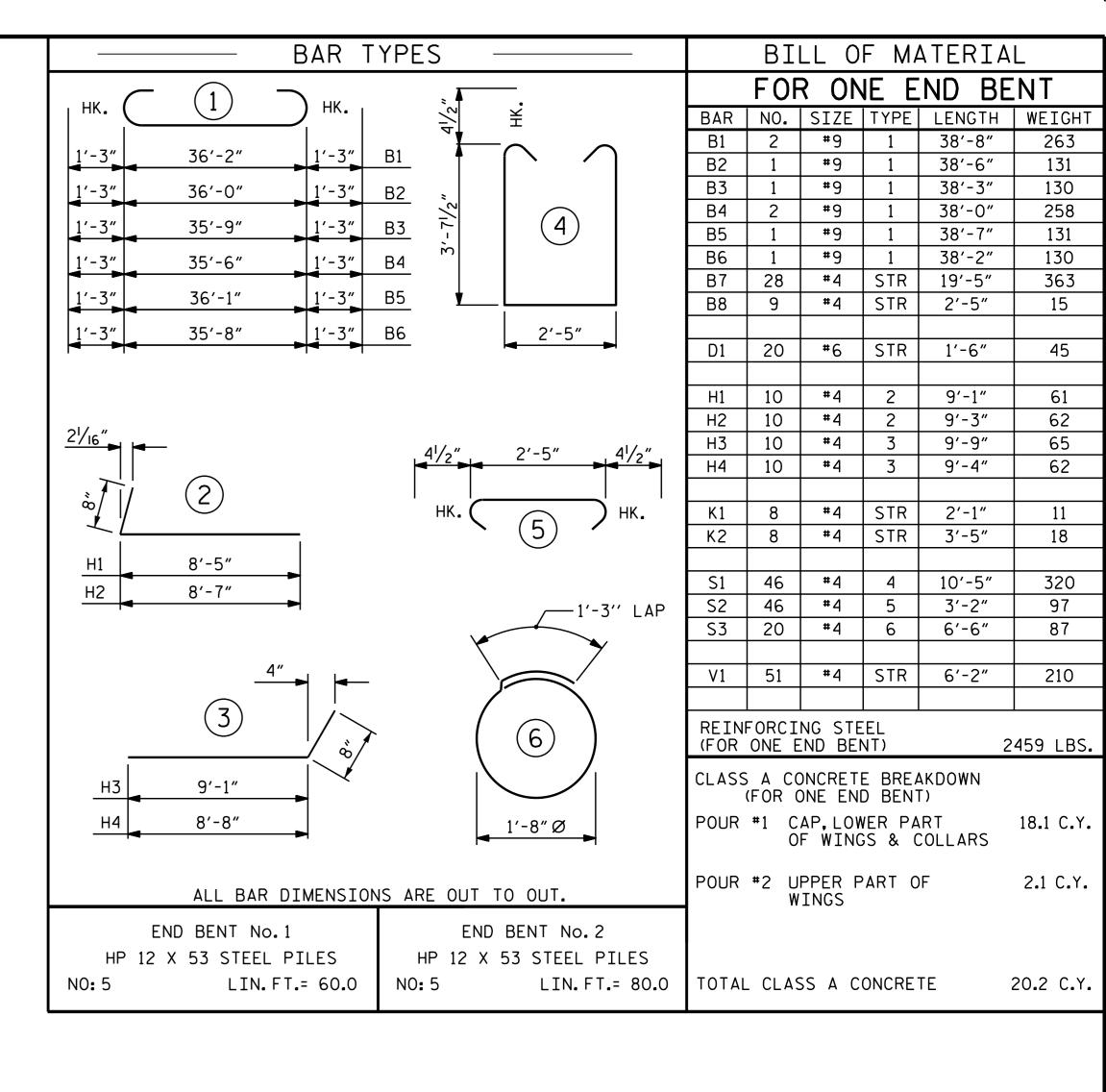
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

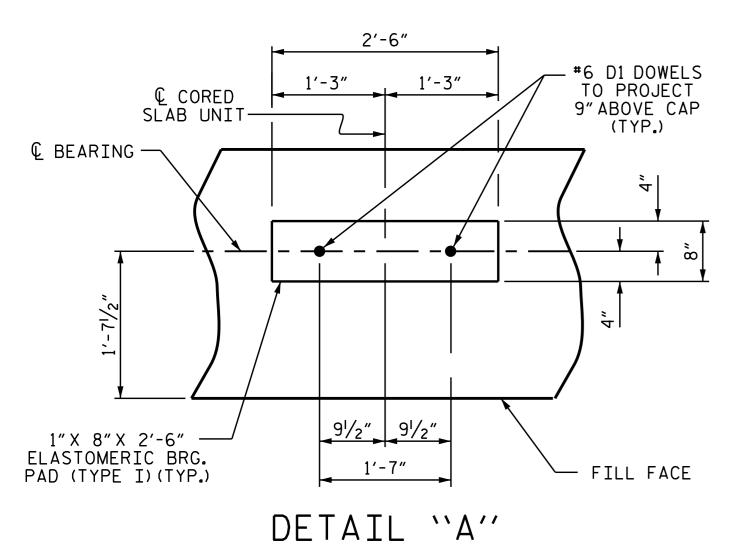
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

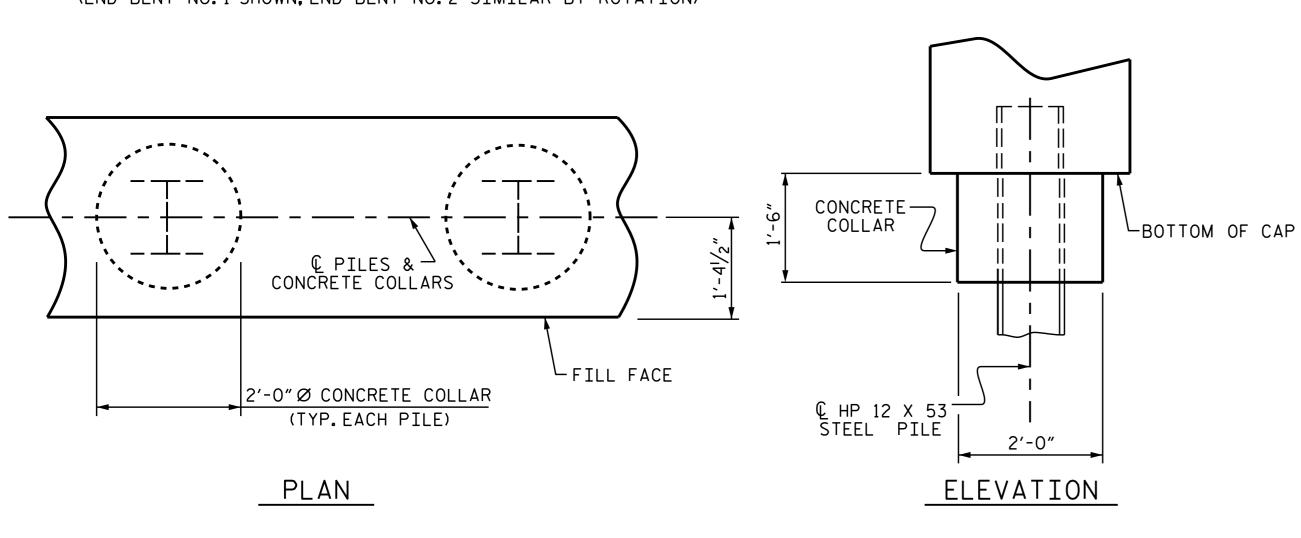
TEMPORARY DRAINAGE AT END BENT







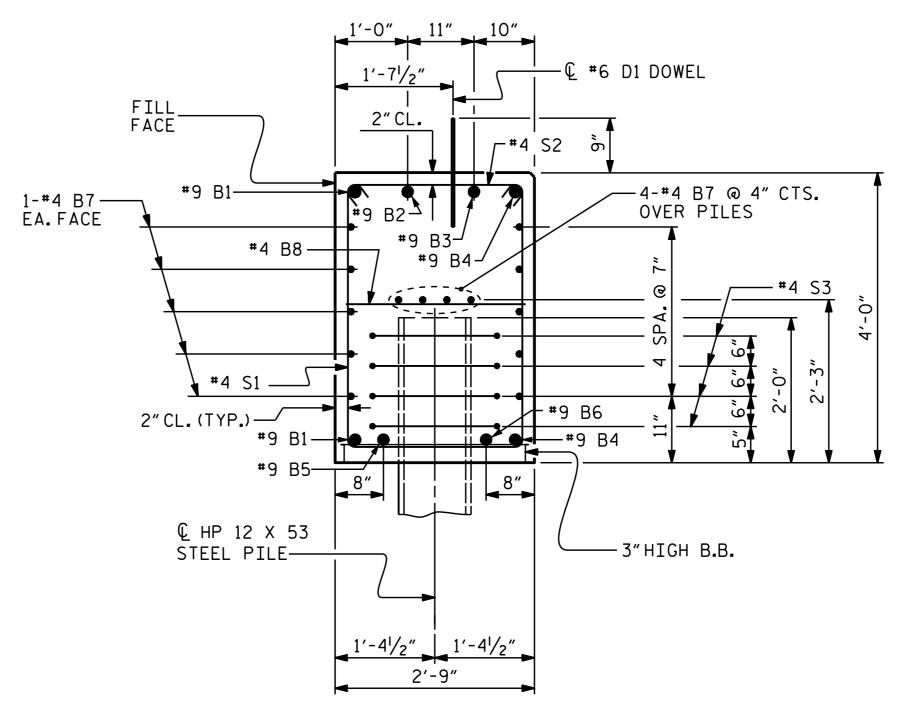
(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)



CORROSION PROTECTION FOR STEEL PILES DETAIL

(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

DRAWN BY :	RAWN BY :H.T.BARBOUR			5-5-16
CHECKED BY :	V. X. N	GUYEN	DATE :	5-16
DESIGN ENGINEER	OF RECORD:	A. M. LEE	DATE :	7-16



SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

PROJECT NO. B-5380

AVERY COUNTY

STATION: 12+31.30 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE

END BENT No.1 & 2 DETAILS

REVISIONS

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REVISIONS

REVISIONS

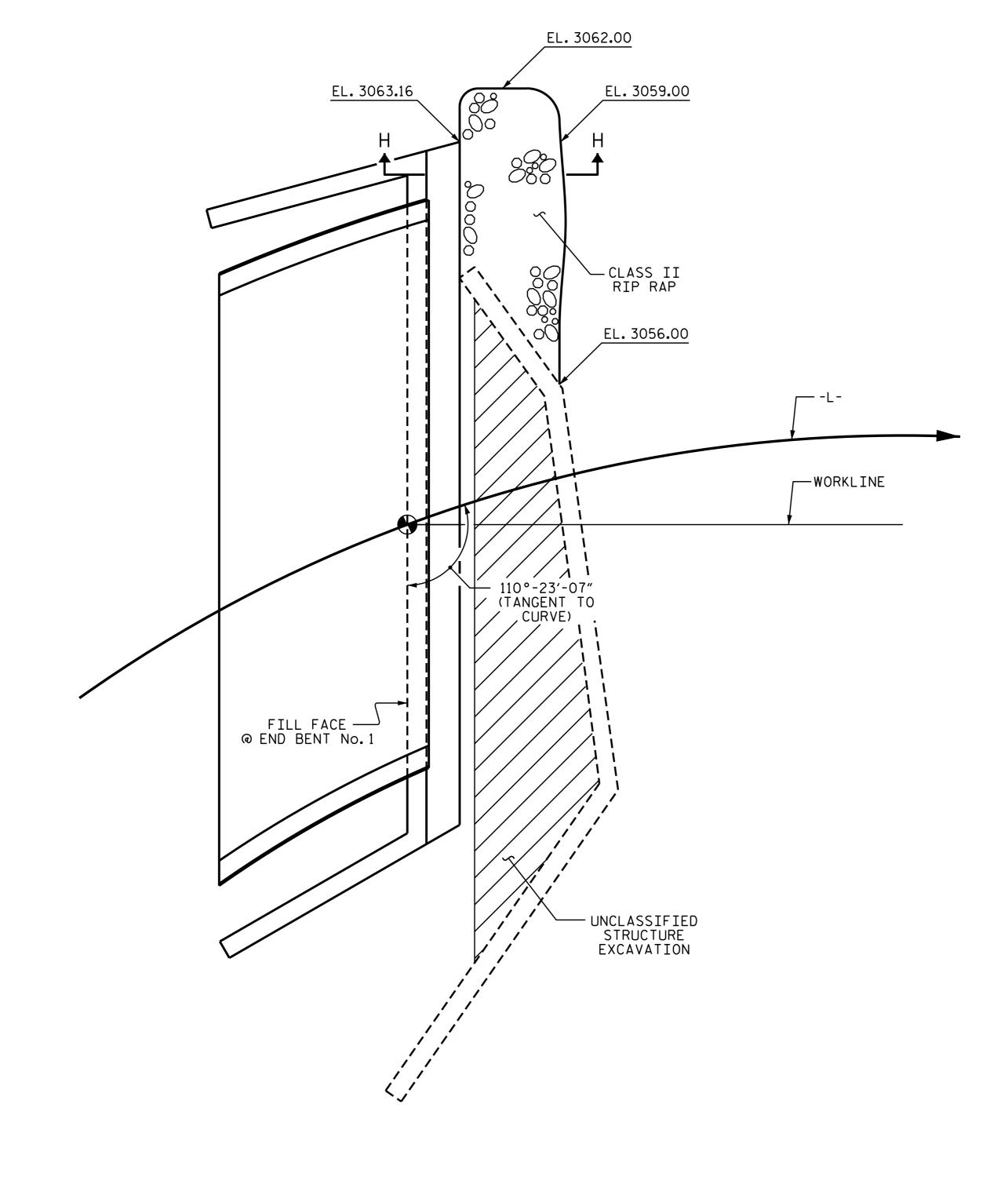
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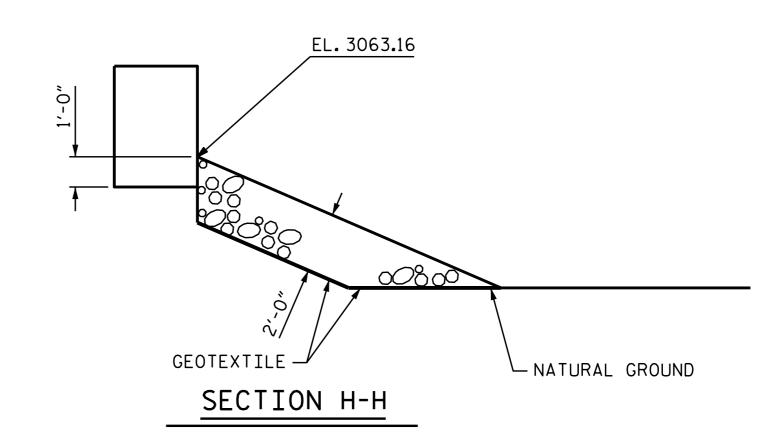
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PLAN @ END BENT No.1

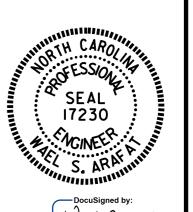
ESTIMATED QUANTITIES								
BRIDGE @ STA.12+31.30 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE						
	TONS	SQUARE YARDS						
END BENT 1	11.0	12.0						



PROJECT NO. B-5380

AVERY COUNTY

STATION: 12+31.30 -L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

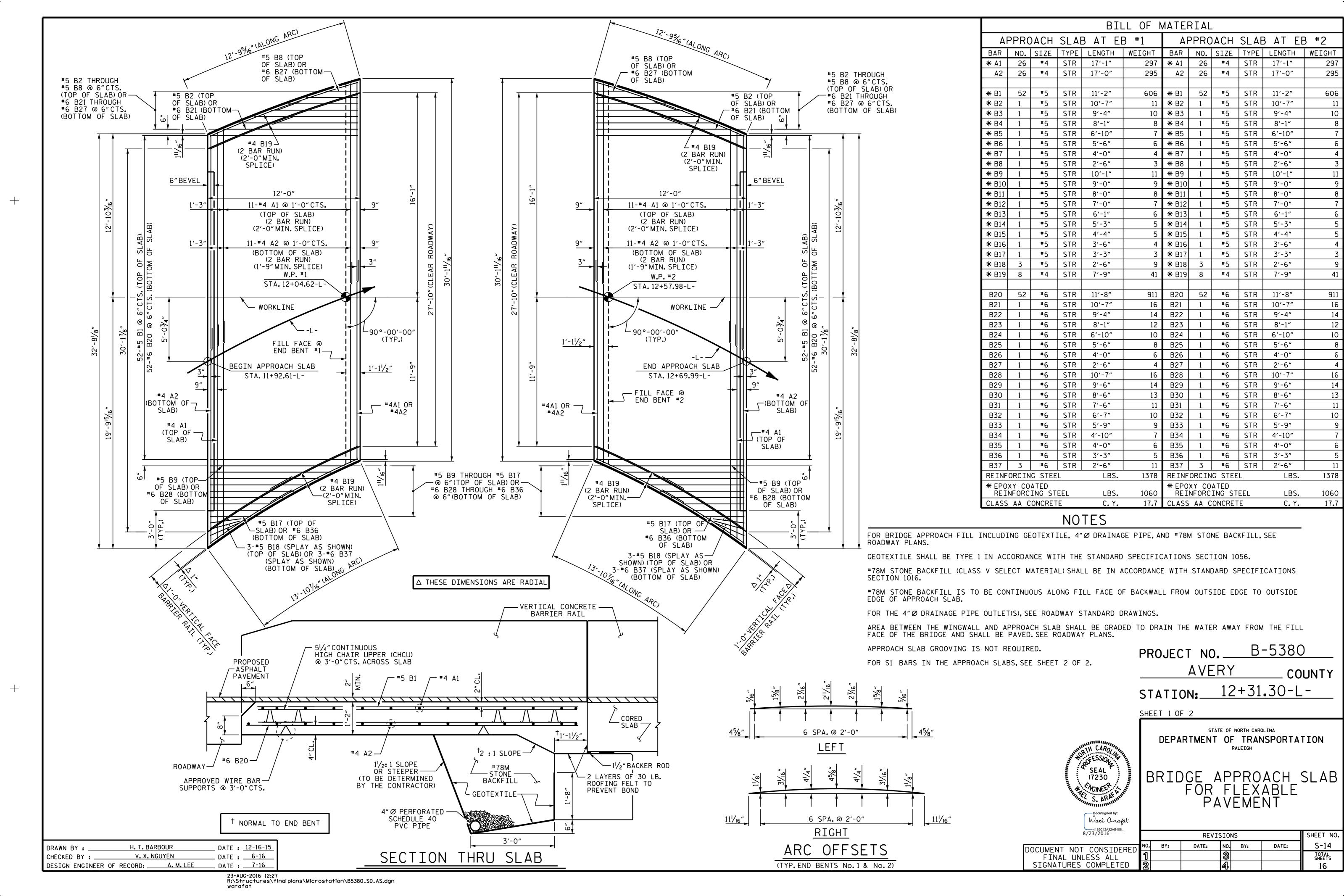
RALEIGH

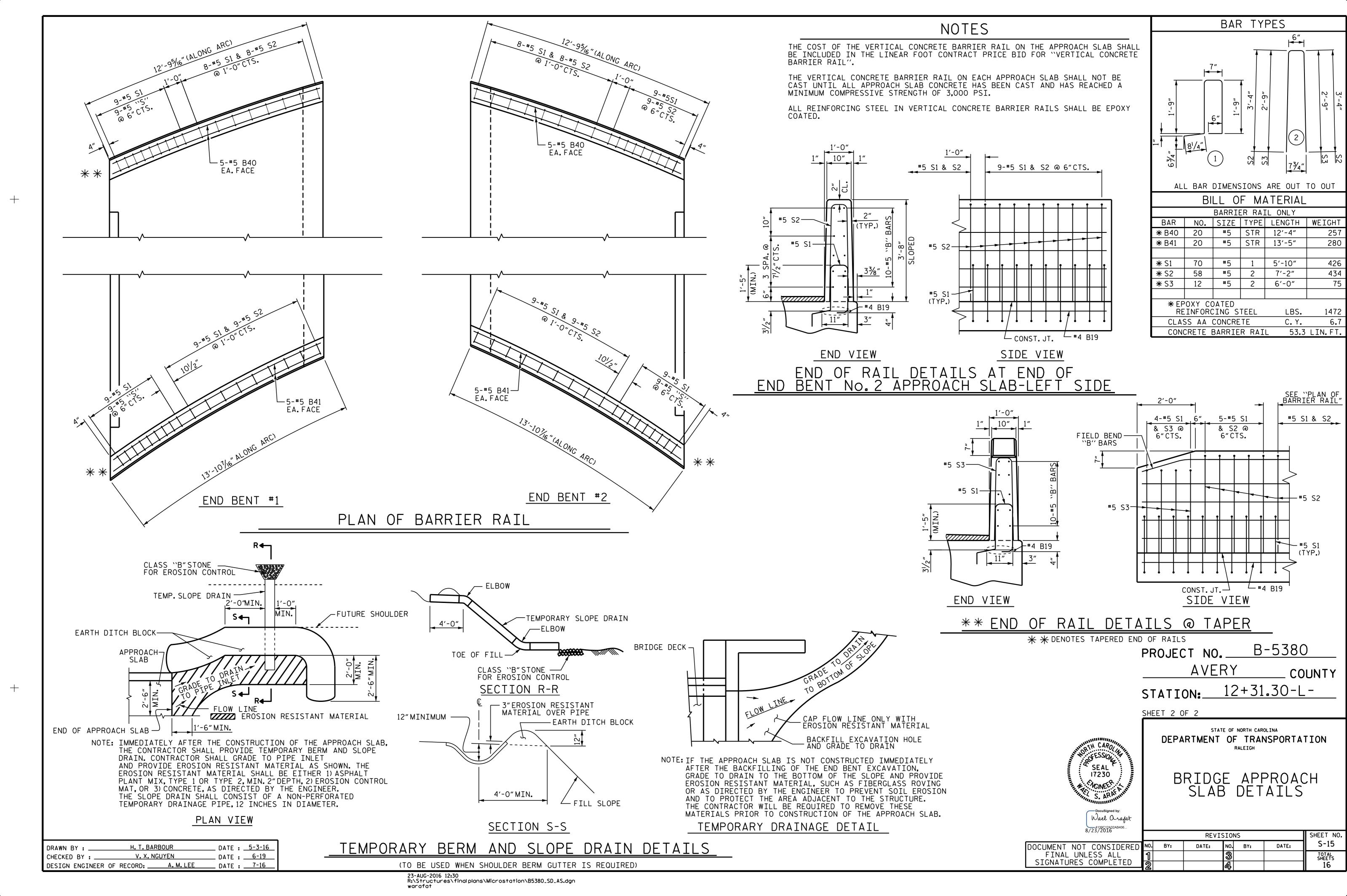
RIP RAP

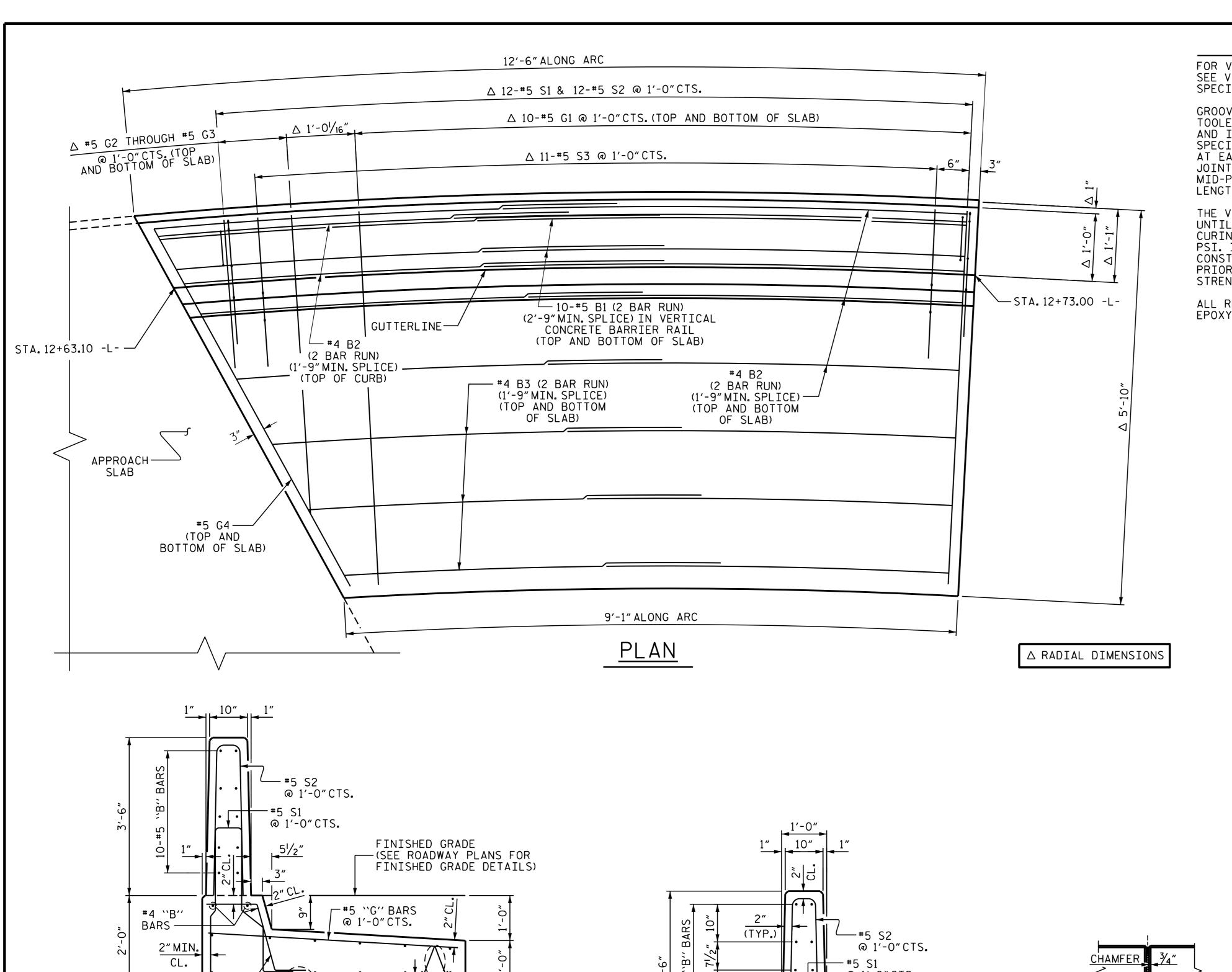
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Wael Orafat
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SIGNATURES COMPLETED	2			4			16

DRAWN BY: H. T. BARBOUR DATE: 5-20-15
CHECKED BY: V. X. NGUYEN DATE: 7-16







FOR VERTICAL CONCRETE BARRIER RAIL WITH MOMENT SLAB, SEE VERTICAL CONCRETE BARRIER RAIL WITH MOMENT SLAB SPECIAL PROVISION.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED SURFACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MID-POINT OF BARRIER RAIL SEGMENTS LESS THAN 20' IN LENGTH.

THE VERTICAL CONCRETE BARRIER RAIL SHALL NOT BE CAST UNTIL THE MOMENT SLAB HAS ATTAINED AN AGE OF THREE CURING DAYS OR A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI. IN ADDITION, NO FILL MATERIAL, ASPHALT, OR CONSTRUCTION EQUIPMENT IS ALLOWED ON THE MOMENT SLAB PRIOR TO SATISFYING THE MINIMUM CONCRETE CURING AND STRENGTH REQUIREMENTS.

ALL REINFORCING STEEL IN THE BARRIER RAIL SHALL BE EPOXY COATED.

> VERTICAL CONCRETE BARRIER RAIL WITH MOMENT SLAB PAY LENGTH = 12.2 LIN FT

B-5380 PROJECT NO. ___ AVERY COUNTY STATION: 12+31.30 -L-

VERTICAL CONCRETE BARRIER 12.2 LIN FT

BAR TYPES

8¹/₄"

155

75

47

115

9

5

13

100

90

302 LB

345 LB

1.4 CY

3.1 CY

SHEET NO. S-16

DATE:

ALL BAR DIMENSIONS ARE OUT TO OUT

FOR ONE SECTION OF VERTICAL CONCRETE

#5

#5

#5

#5

#5

#5

#4 | STR |

NO.

20

16

12

20

2

REINFORCING STEEL

REINFORCING STEEL CLASS AA CONCRETE

VERTICAL CONCRETE BARRIER RAIL

RAIL WITH MOMENT SLAB

CLASS A CONCRETE

MOMENT SLAB

* EPOXY COATED

B2

G4

S3

BARRIER RAIL WITH MOMENT SLAB

#4 STR 5′-10″

#5 | STR | 5'-6"

#5 | STR | 4'-4"

STR

STR

SIZE TYPE | LENGTH | WEIGHT

7′-5″

7′-0″

2'-3"

6′-0″

8'-0"

7′-2″

3'-4"

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

VERTICAL CONCRETE BARRIER RAIL WITH MOMENT SLAB

8/23/2016 REVISIONS BY: DATE: NO. BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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Wael Orafat

1" 10" 1" SYMB STATE OF THE ST

VERTICAL CONCRETE BARRIER RAIL SECTION

VERTICAL CONCRETE BARRIER RAIL WITH MOMENT SLAB

Δ 6-#4 "B" BARS TOP & BOTTOM SPA. AS SHOWN Δ 5′-10″

[#5 ``G'' BARS — 리 @ 1'-0" CTS. ※

△ 5-#4 "B" BARS

TOP & BOTTOM @ 1'-0"CTS.

LAPPROVED WIRE BAR

SUPPORTS @ 3'-0"CTS.

H. T. BARBOUR _ DATE : <u>6-7-16</u> DRAWN BY __ DATE : ____7-16__ V. X. NGUYEN CHECKED BY : _ DESIGN ENGINEER OF RECORD: A.M.LEE DATE: 7-16

#5 S3---

@ 1'-0"CTS. ALT. AT 6"

W/ #5 S2

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