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SEE SHEET 3 FOR PLAN SHEET LAYOUT STATE OF NORTH CAROLINA AT TIME OF INVESTIGATION DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS CONTENTS** GEOTECHNICAL ENGINEERING UNIT <u>LINE</u> **PROFILE STATION** <u>PLAN</u> **ROADWAY** 12+20 TO 36+50 04-05 06-07 SUBSURFACE INVESTIGATION COUNTY _COLUMBUS PROJECT DESCRIPTION CONSTRUCT OVERPASS OF SR 1574 (OLD US 74) OVER US 74 CROSS SECTIONS <u>LINE</u> **STATION SHEETS** INVENTORY 12+20 TO 12+50 80 13+00 TO 14+00 09 REFEREN 14+50 15+00 TO 15+50 11 12 13 16+00 TO 16+50 17+00 TO 17+50 18+00 14 15 16 17 19+00 20+00 18 19 20+50 21+00 20 21 22 23 24 25 26 27 28 29 30 31 21+50 22+00 22+50 23+00 23+50 25+00 25+50 26+00 26+50 27+00 TO 27+50 32 33 34 35 36 37 38 39 40 29+00 29+50 TO 30+00 30+50 31+00 TO 31+50 32+00 TO 32+50 33+00 TO 34+00 34+50 TO 35+00 35+50 TO 36+00 36+50 3 4

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	W-5518	01	41

CAUTION NOTICE

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SIGNATURE

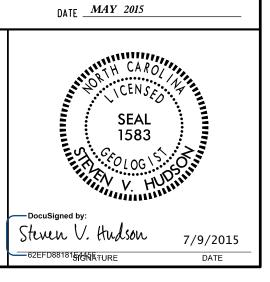
DATE

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 OR CONTRACT FOR THE PROJECT.
 BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
 FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
 CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

JACOB C. WESSELL, PE SHAWN MCGUIRE MICHAEL D. MASON D. T. CHALMERS, CWC INVESTIGATED BY _ CATLIN DRAWN BY STEVEN HUDSON, LG CHECKED BY JACOB C. WESSELL, PE SUBMITTED BY STEVEN HUDSON, LG

PERSONNEL

STEVEN HUDSON, LG



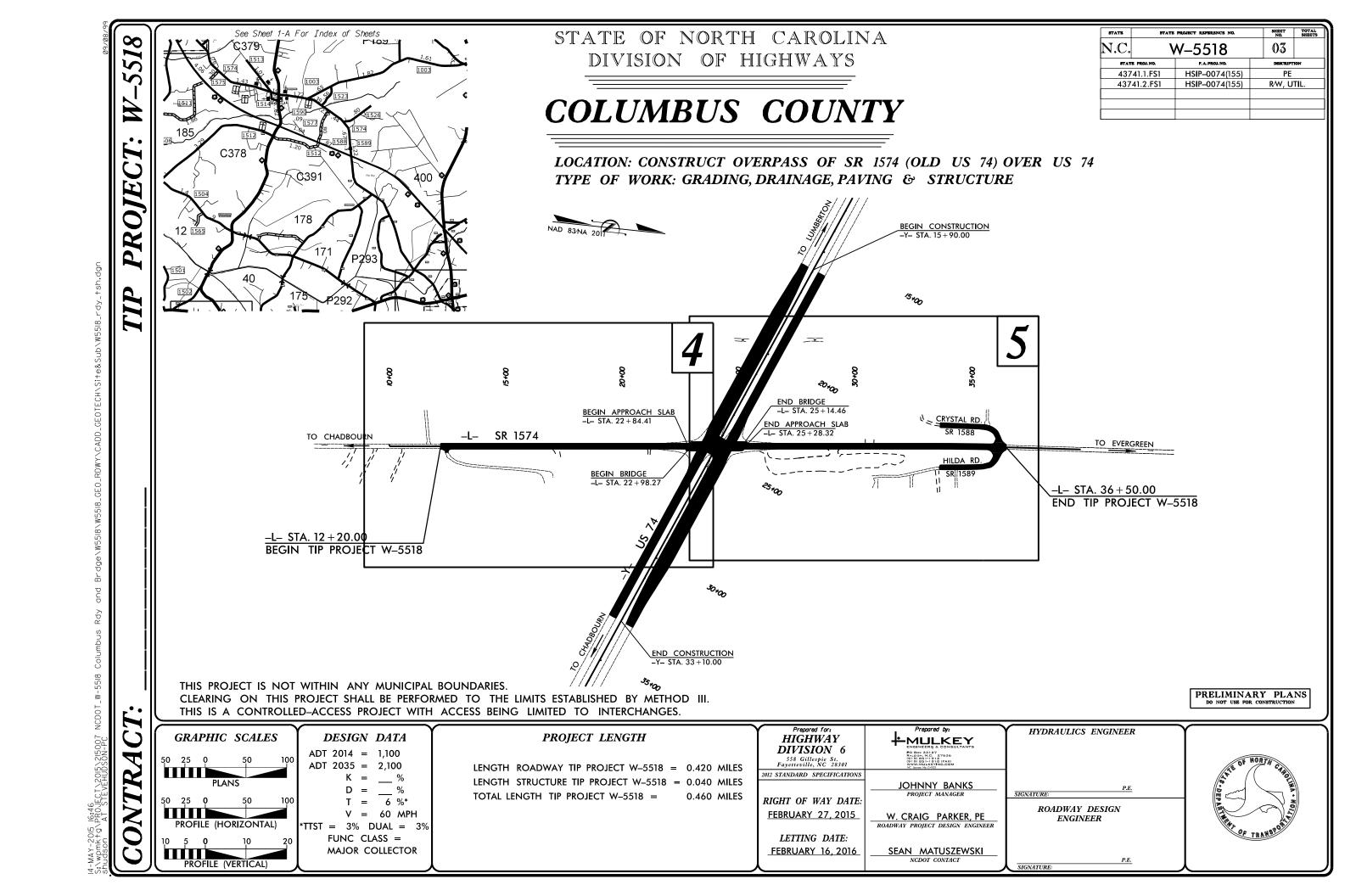
W-5518 02

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS								
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.								
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.								
IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.								
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF GRAY SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.								
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED VILVE NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT								
CENERAL CRAMIII AR MATERIALS SILT-CLAY MATERIALS	MINERALOGICAL COMPOSITION	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND								
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	CRYSTALLINE ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.									
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	NON-CRYSTALLINE - FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.								
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-3 A-6, A-7	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LL < 31	ROCK (NCR) SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.								
SYMBOL 000000000000000000000000000000000000	MODERATELY COMPRESSIBLE LL = 31 - 5Ø	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED								
7. PASSING GRANULAR SILT- MUCK,	HIGHLY COMPRESSIBLE LL > 50 PERCENTAGE OF MATERIAL	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.								
*40 30 MX 50 MX 51 MN S 1 MN S		WEATHERING	<u>DIKE</u> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.								
*2000 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE								
MATERIAL PASSING *40	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	HORIZONTAL.								
LL 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 501L5 WITH	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE								
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN MODERATE ORGANIC	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE GROUND WATER	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH, FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE								
GROUP INDEX W W A MX 8 MX 12 MX 16 MX NU MX AMUUNIS UF SOILS	GROOMS WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.								
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.								
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM								
GEN. RATING EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE		(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.								
AS SUBGRADE LACELLEUR 10 00000 FRITT 10 0000 POOR 1001 01001 2001	→ SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE								
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.								
COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED		(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.								
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (N-VALUE) (TONS/FT ²)	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION WITH SOIL DESCRIPTION OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO								
VERY LOOSE 44	SPT C CLOPE INDICATOR	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT. <u>LENS</u> - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.								
GENERALLY LOOSE 4 TO 10	SOIL SYMBOL SOIL SYMBOL SOIL SYMBOL SOIL SYMBOL SING SLOPE INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS								
MATERIAL MEDIUM DENSE 10 10 30 N/A	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.								
(NON-COHESIVE) VERY DENSE > 50	THE THOP WAS EMBRICATED TO TEST	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE								
VERY SOFT < 2 < 0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.5	— INFERRED SOIL BOUNDARY — CORE BORING SOUNDING ROD	(V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u>	OF AN INTERVENING IMPERVIOUS STRATUM.								
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF								
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	A ALLUMAN CON POINTARY A PIEZOMETER	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE								
HARD > 30 > 4	INSTALLATION SPIN-VALUE	ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE.								
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.								
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND								
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	SHALLOW STEEL OF STEEL OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.								
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	UNDERCUT SHALLOW ACCEPTABLE DEGRADABLE ROCK EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN. MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT								
(BLDR.) (COB.) (GR.) (GSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	OR SLIP PLANE.								
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF								
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB.HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL								
SOIL MOISTURE - CORRELATION OF TERMS	$oldsymbol{\bot}$ CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{\!\scriptscriptstyle d}$ - DRY UNIT WEIGHT	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.								
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION (ATTERBERG LIMITS) DESCRIPTION	CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.								
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED I								
(SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.								
PLASTIC PLOUID LIMIT	─ FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.								
RANGE - WET - (W) SEMISULIDE REJURIES DRYTING TO	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS w - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: SURVEY CONDUCTED WITH TRIMBLE 5800 GPS AND								
(PI) PL PLASTIC LIMIT	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	TRIMBLE TSC2 DATA COLLECTOR.								
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: _ FEET								
SL _ SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED Ø.16 - 1.5 FEET	NOTES:								
- DRY - (D) REQUIRES ADDITIONAL WATER TO	CME-45C CLAY BITS X AUTOMATIC MANUAL	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET									
ATTAIN UPTIMUM MUISTURE	G*CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	Project Datum = North American Datum 1983 (Conus) Zone = North Carolina 3200								
PLASTICITY	X 8" HOLLOW AUGERSB	INDURATION	Coordinate System = North Carolina State Plane 1983 Project Datum = North American Datum 1983 (Conus) Zone = North Carolina 3200 Geoid Model = GEOIDO3 (Conus) Vertical Datum = NGVD88 All Units = US Feet								
PLASTICITY INDEX (PI) DRY STRENGTH	X CME-550 X HARD FACED FINGER BITS	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. RUBBING WITH FINGER FREES NUMEROUS GRAINS:	All Units = US Feet								
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST UNGCARBIDE INSERTS	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	U.C.P. = UNDIVIDED COASTAL PLAIN								
MODERATELY PLASTIC 16-25 MEDIUM	CASING W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;									
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST X TRICONE 27/8 STEEL TEETH X HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.									
COLOR	X CME-45B TRICONE TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.									
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT VANE SHEAR TEST	CHARD HAMMED BLOWS DECLURED TO BREAK SAMPLE.									
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14								



MAY 2015

STATE PROJECT: 43741.1.FS1, W-5518

F.A PROJECT: HSIP-0074(155)

COUNTY: Columbus

DESCRIPTION: Construct Overpass of SR 1574 (Old US 74) over US 74

SUBJECT: Geotechnical Inventory Report

PROJECT DESCRIPTION

The project consists of constructing an overpass structure with associated embankment and roadway encroachment at the existing intersection of SR 1574 (Old US 74) (-L-) and US 74 (-Y-) located approximately 2.5 miles southeast of the town of Evergreen in Columbus County. The project begins approximately 2,400 feet south of US 74 and extends northward for approximately 3,600 feet.

A geotechnical investigation was conducted by CATLIN Engineers and Scientists (CATLIN) in February and March 2015. Thirty-five borings were advanced utilizing an ATV mounted CME-550 drill machine with an automatic hammer; a track mounted CME-45B drill machine with an automatic hammer; and a hand auger. Standard Penetration Tests were performed at selected locations and additional borings were advanced using hand augers. Representative soil samples were collected for visual classification in the field and selected samples were submitted for laboratory analysis by CATLIN Geotechnical Laboratory located in Wilmington, North Carolina.

The following alignments were investigated. Plan sheets, subsurface profiles, and selected cross sections of the alignment are included in this report.

<u>Line</u> <u>Station (±)</u>

-L- 12+20 to 36+50

AREAS OF SPECIAL GEOTECHNICAL INTEREST

1) <u>GROUNDWATER</u>: The entire project was found to exhibit a high water table, seasonal high groundwater, or the potential for groundwater related construction problems.

- COHESIVE SOILS: Moderately to highly plastic (PI>20) clays and silts which have the potential to cause embankment/subgrade and or slope stability problems during construction were encountered along the entire project.
- ORGANIC SOILS: A thin (0.1 to 0.6 feet thick) lens of sand with little organics (5.4%) was encountered at an elevation of approximately 76 to 80 feet in most borings that were advanced below the contact between the Undifferentiated Coastal Plain and Duplin Formation. This lens was identified at the following sections on the project:

<u>Line</u> <u>Station (±)</u>
-L- 19+00 to 26+50

4) <u>WATER WELLS</u>: No water wells were identified within the proposed construction limits. Potable water is supplied to residences in the vicinity by a public water supply.

Water supply wells may be present along the project corridor that were not detected.

PHYSIOGRAPHY AND GEOLOGY

The project is located in the flat to gently rolling terrain of the North Carolina Coastal Plain physiographic province. Land surface elevations measured at the boring locations range from approximately 104 feet to 108 feet with an average elevation of 105.6 feet. Land use along the project corridor consists of homes, farmland and woods. Geologically, the project is located within the limits of the Duplin Formation (often undivided with the Yorktown Formation) underlain by the Peedee Formation which are Tertiary and Cretaceous age, respectively. Coastal Plain sediments were deposited in a number of different environments including but not limited to, off-shore marine, near-shore marine, lagoonal, and deltaic. The eustatic rise and fall of sea level has resulted in numerous sedimentary packages of transgressive and regressive sequences deposited throughout the Coastal Plain. The dominant geological structural feature within the surrounding area of the subject site is the Cape Fear arch. Brown Mill Branch and Dunn Swamp are the closest named water bodies and are located approximately 4,000 feet southwest and 4,200 feet southeast of the project, respectively. The project is drained by manmade ditches and is located within the Cape Fear River Basin.

GROUNDWATER

Groundwater data was collected from open boreholes, where possible, during the field investigation conducted during February and March 2015. According to available data, rainfall in Whiteville, North Carolina located approximately 10 miles east southeast of the project, rainfall was reported as follows:

MONTH (2015)	RECORDED RAINFALL (inches)	AVERAGE RAINFALL (inches)
January	2.36	3.82
February	4.62	3.27
March	4.58	3.86

Measured groundwater elevations ranged from elevation 97.7 feet to 106 feet with an average elevation of 103.2 feet. Depth to groundwater measurements ranged from 0.1 feet to 8.5 feet (below existing land surface), with an average depth to water of 2.1 feet.

SOIL PORPERTIES

Soils encountered at the project site include roadway embankment, undivided coastal plain, and coastal plain sediments that include the Duplin and Peedee Formations.

Roadway Embankment soils are present along the existing SR 1574 and US 74 to an approximate average elevation of 103 feet. These soils consist of orange, gray, and brown, dry to saturated, soft to stiff, fine grained sandy silt (A-4) to loose to medium dense, silty and clayey, fine sand (A-2-4, A-2-6).

Undivided coastal plain material is present beneath the roadway embankment to an average approximate elevation of 77 feet and consists of gray, orange, tan, red, and yellow, dry to saturated, very soft to stiff, moderately to highly plastic silty clay (A-7) and sandy to clayey silt (A-4) interbedded with, and overlain by, gray, orange, tan, yellow, and red, saturated, very loose to medium dense, silty to clayey, fine sand (A-2-4, A-2-6) and gray to yellow orange, soft, slightly plastic clayey silt with sand (A-4). Laboratory analysis of six samples collected within the silt and clay material reported natural moisture contents ranging from 19% to 38% with an average moisture content of 27%. Gray to tan, saturated, very loose to loose, silty to clayey, fine grained sand (A-2-4, A-2-6) lie at the base of the undivided coastal plain material. A basal lens (approximately 0.1 feet to 0.6 feet thick) of sand with little organic material (organic content = 3.8%) was identified beneath the very loose to loose sand rather consistently across the project.

Duplin Formation materials consisting of loose to dense, brown, tan, and gray, moist to saturated, fine to coarse grained sand and tan, loose, saturated, clayey sand with silt (A-3, A-2-7) occur beneath the undivided coastal plain material to an approximate average elevation of 69 feet. Approximately four to 16 feet of moist to wet, very soft to medium stiff, moderately to slightly plastic sandy clay to sandy silt (A-7-6, A-4) was encountered beneath the upper sands of the Duplin Formation. Laboratory analysis of the clay material revealed liquid limits ranging from 42 to 46 and a moisture content of 61%. Dark gray, fine sand to silty or clayey, fine sand with some limestone and shell fragments (A-3, A-2-4, A-2-6) extend from below the

silt and clay to an approximate average elevation of 44 feet. The sandy material is reported as moist to saturated with a very loose to medium dense consistency.

Highly plastic, very stiff to hard, dark gray, gray, and olive gray, fine grained sandy clay and some slightly plastic clayey silt (A-7-6, A-4) of the Peedee Formation underlie the Duplin Formation to an average elevation of approximately 20 feet. Laboratory analysis of clay samples collected from within the stratum revealed liquid limits of 52 to 64 and plasticity indexes of 28 to 38. Green gray to gray, very dense saturated fine to coarse sand and clayey sand (A-3, A-2-6) was identified in five of the six borings advanced below 20 feet elevation. Deep borings were terminated at elevations ranging from 12.6 feet to 7.8 feet in either saturated, very dense, green gray to gray, fine sand to clayey, fine sand (A-3, A-2-7) or highly plastic (PI=56, LL=88), gray to olive gray, very stiff, moist, calcareous silty clay with mica (A-7-5).

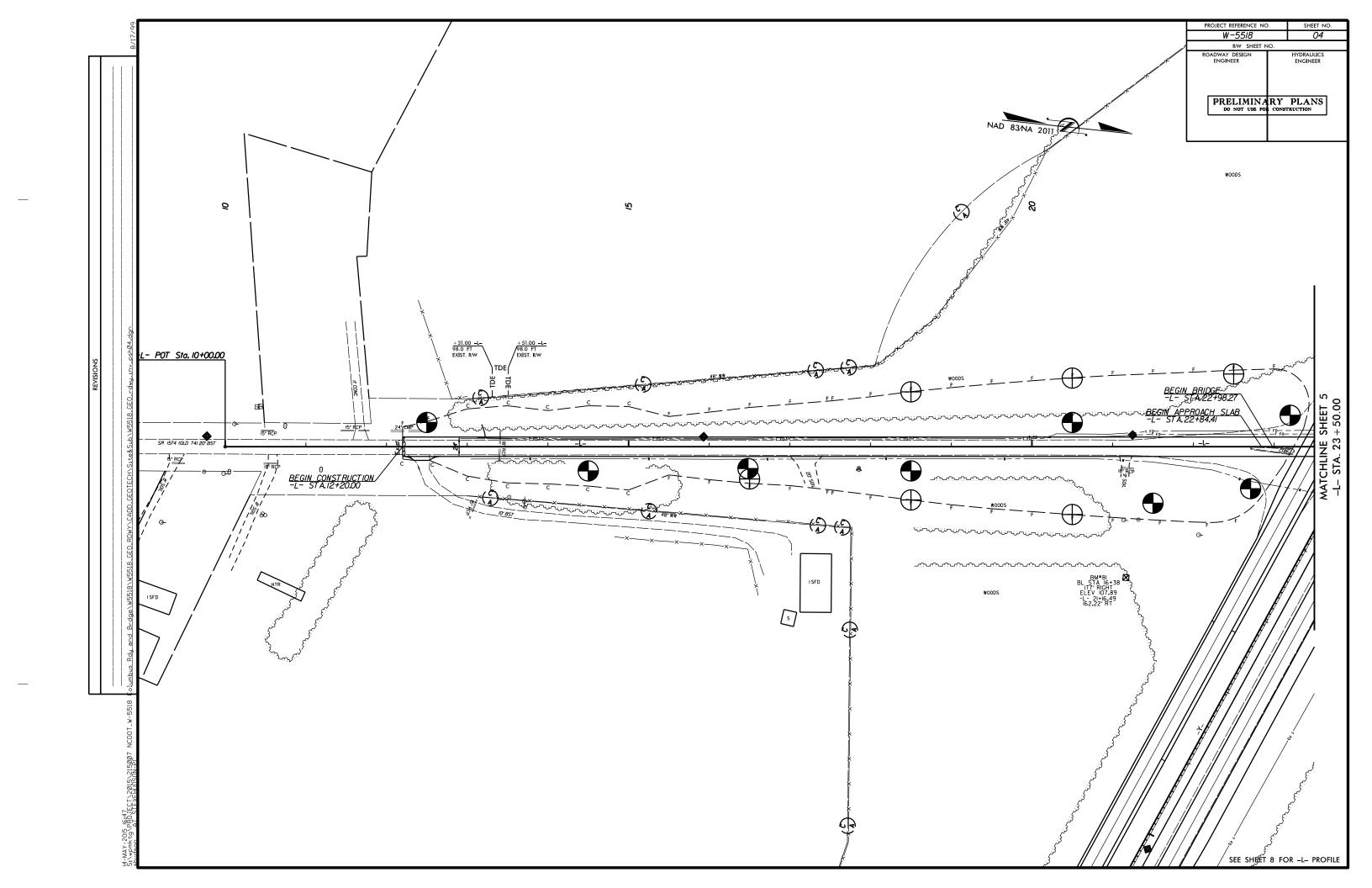
UNDISTURBED SAMPLES

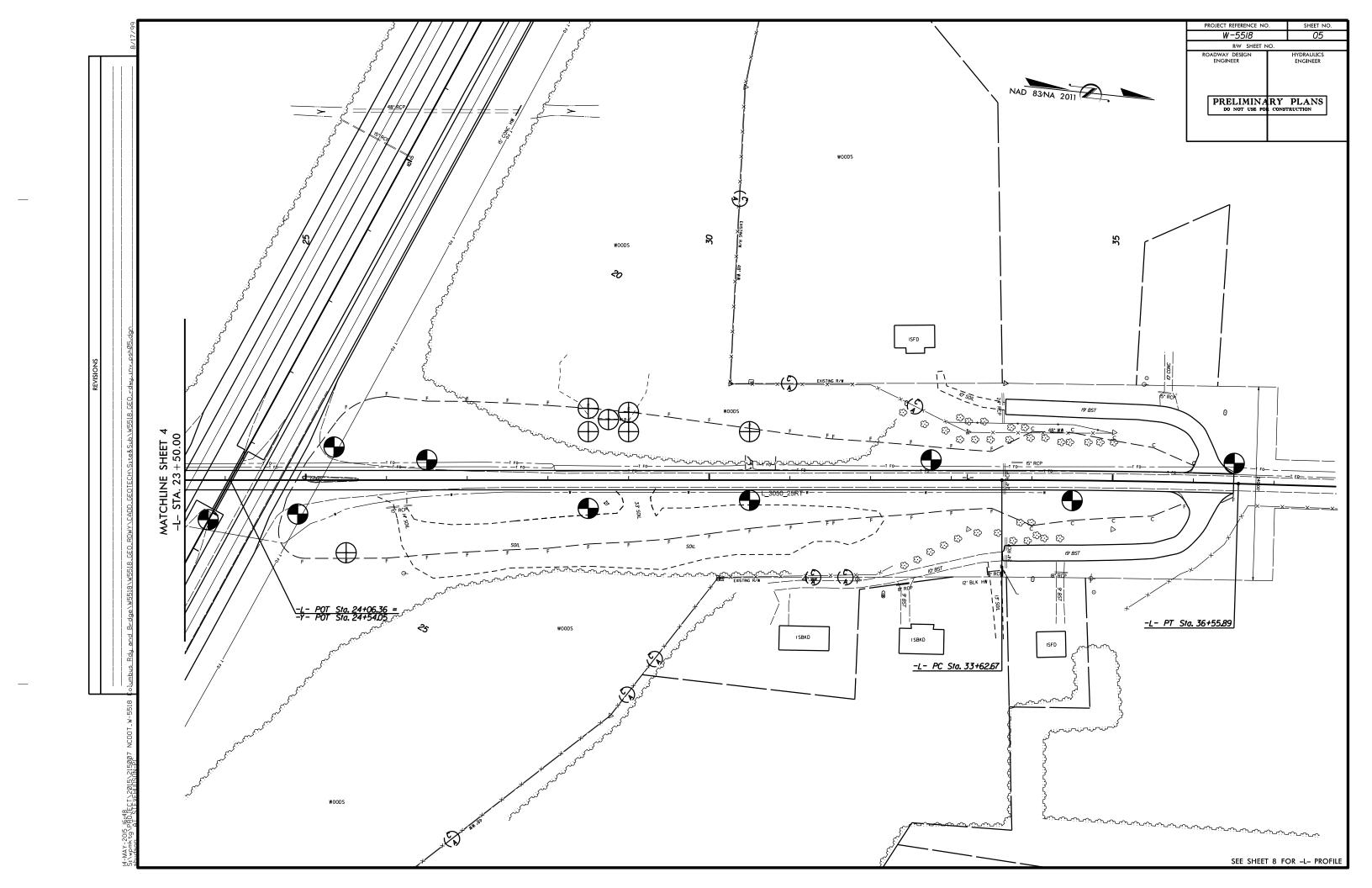
Undisturbed thin wall Shelby tube samples were collected at the following locations and submitted to Geotechnics Geotechnical and Geosynthetic Testing located in Raleigh, North Carolina.

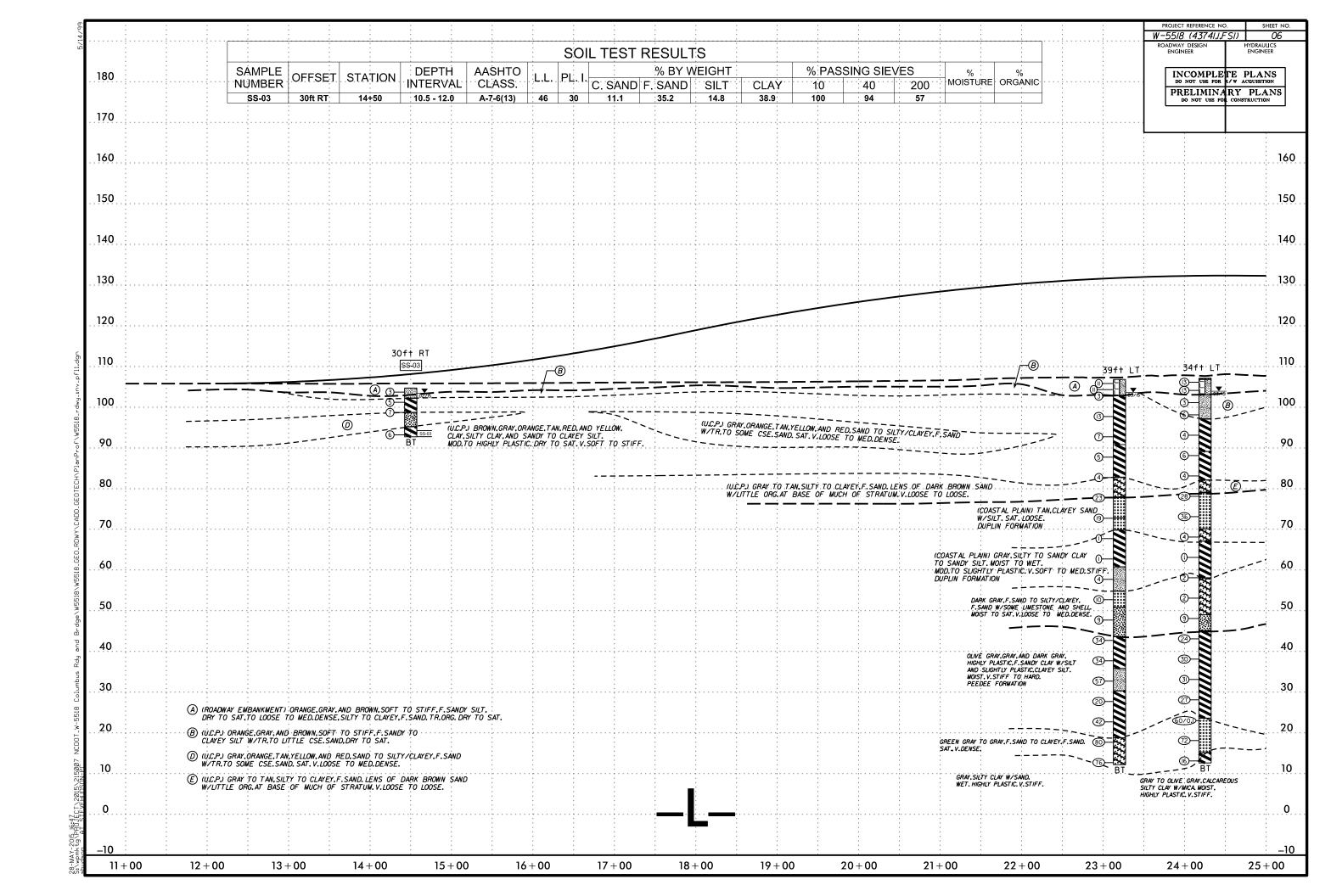
Sample No.	Line / Station / Offset	Depth (ft.)	<u>Test</u>
ST-01	-L- / 26+50 / 26ft LT	9.0 – 11.5	Specific Gravity Consolidation
ST-02	-L- / 22+71 / 53ft RT	1.5 – 4.0	Specific Gravity Consolidation
ST-03	-L- / 22+71 / 53ft RT	8.0 – 10.5	Specific Gravity Tri-axial Consolidation
ST-04	-L- / 16+48 / 28ft RT	9.0 – 11.5	Specific Gravity Consolidation

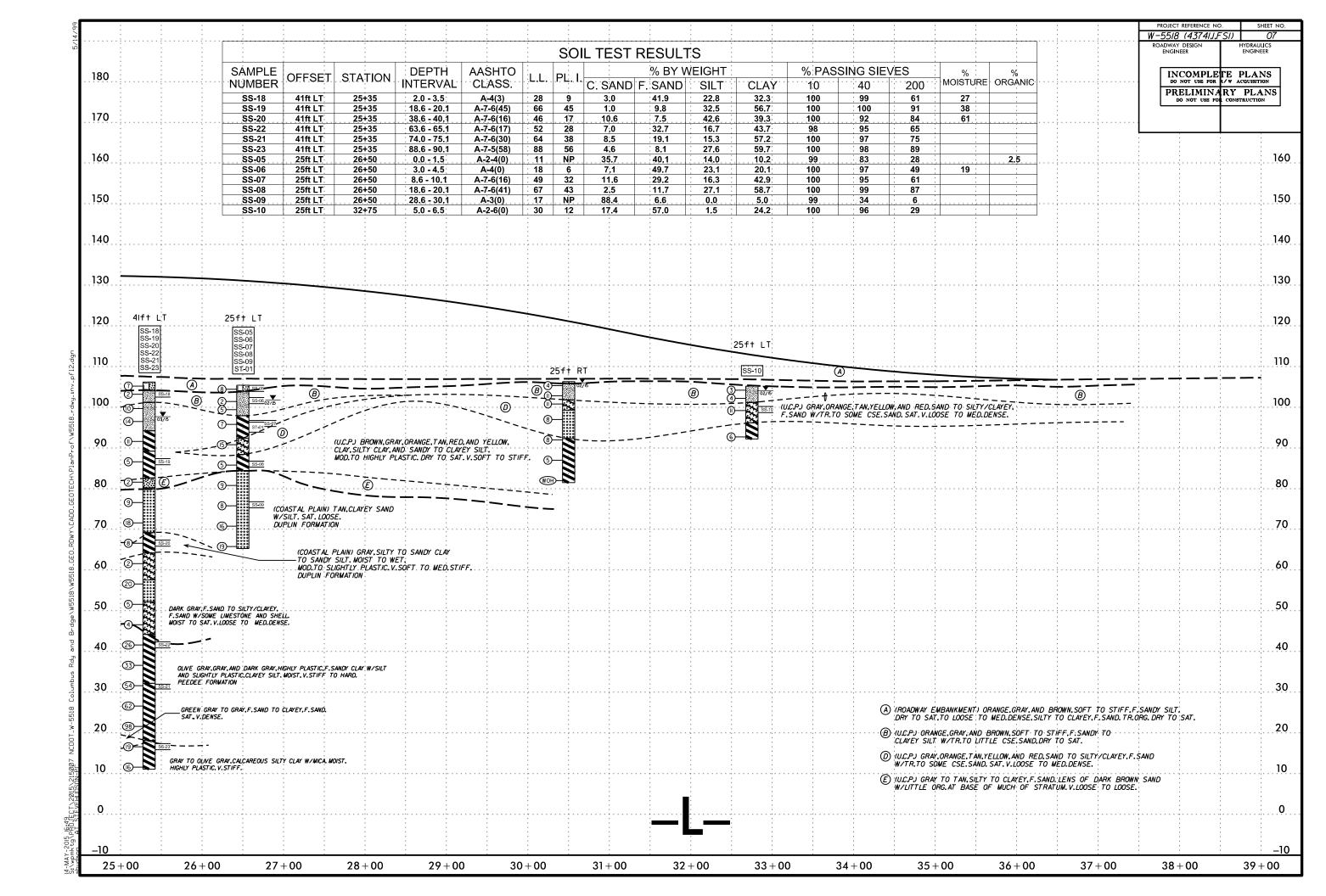
Prepared by,

Steven V. Hudson, L.G. Project Geologist









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