

PROJECT: B-2500A
CONTRACT: C203171

NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

LINE	STATION	PLAN	PROFILE
-L-	3083+00 TO 3207+37	4-14	4-14
-DET-	3076+53 TO 3215+10	15-25	15-25
-XI-	3157+01 TO 3161+57	26	26
-X2-	3179+58 TO 3185+13	27	27
-X3-	3159+00 TO 3163+51	26,28	26,28
-X4-	3178+46 TO 3182+19	27,29	27,29
-X5-	3159+00 TO 3162+37	28	28

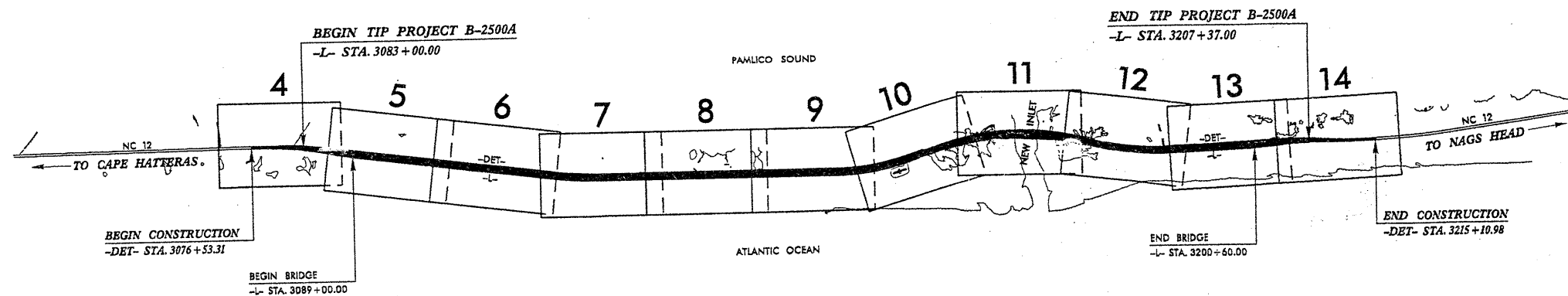
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

**ROADWAY
SUBSURFACE INVESTIGATION**

PROJ. REFERENCE NO. 13201.1028025 F.A. PROJ. _____
COUNTY DARE
PROJECT DESCRIPTION NC HWY 12 OVER PEA ISLAND BREACH

INVENTORY

CROSS SECTIONS	STATION	SHEET
-L-	3087+00	30
-L-	3090+00	30
-L-	3095+00	31
-L-	3100+00	31
-L-	3105+00	32
-L-	3110+00	32
-L-	3112+50 TO 3116+50	33-37
-L-	3120+00	37
-L-	3125+00	38
-L-	3130+00	38
-L-	3135+50 TO 3136+00	39



STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-2500A	1	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
32635.1.6		P.E.	
32635.2.2	BRNH-0012(55)	UTIL.	
32635.3.6	BRNH-0012(55)	CONST.	

CAUTION NOTICE

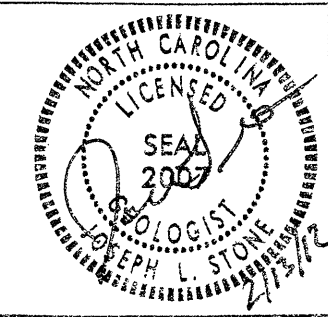
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PERSONNEL
J.R. SWARTLEY
J.M. EDMONDSON

INVESTIGATED BY J.L. STONE
CHECKED BY D.N. ARGENBRIGHT
SUBMITTED BY D.N. ARGENBRIGHT
DATE FEBRUARY 2012



DRAWN BY: C.P. TURNER

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

PROJECT REFERENCE NO.
13201.028025

SHEET NO.
2

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

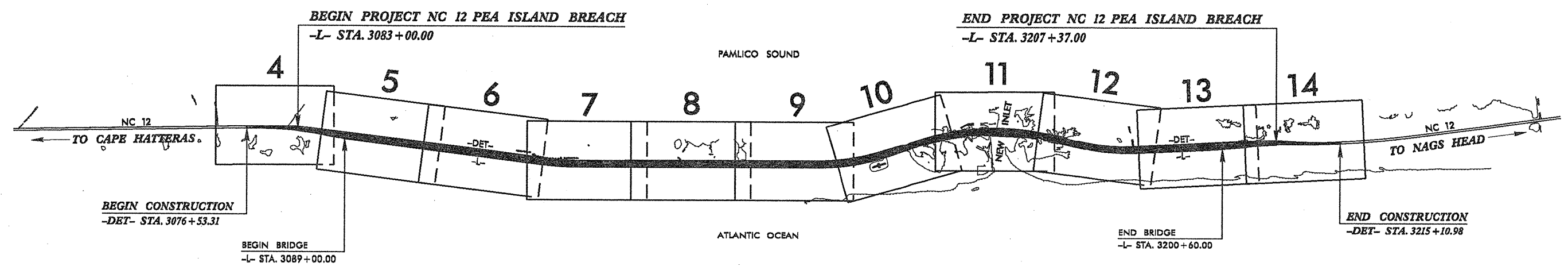
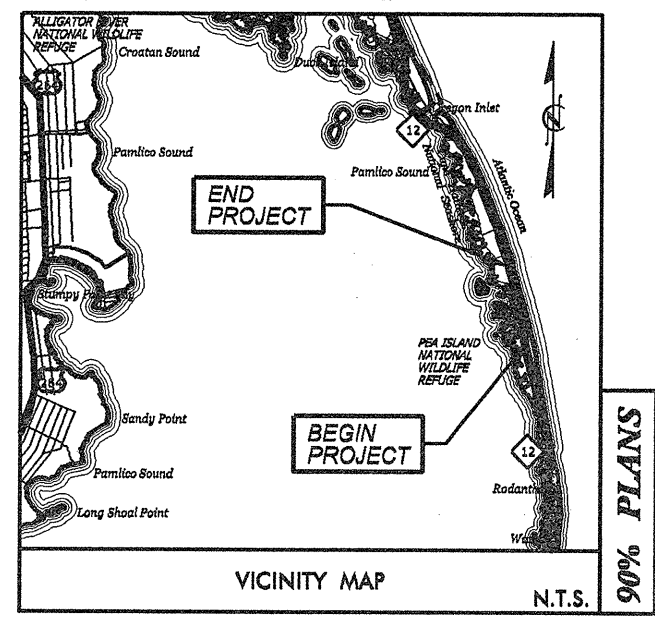
SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS	
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY-SILT CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6</i>		WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.		HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:		ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN REPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SCREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.	
SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS GROUP CLASS. A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-3 A-4, A-5 A-6, A-7 SYMBOLOGY: [Diagrams showing soil textures for various groups] % PASSING: [Diagrams showing sieve analysis curves for groups A-1 to A-7] LIQUID LIMIT PLASTIC INDEX: [Diagrams showing LL and PI relationships for groups] GROUP INDEX: [Diagrams showing GI values for groups] USUAL TYPES OF MAJOR MATERIALS: [Diagrams showing soil types like stone frags, fine sand, silty clay, etc.] GEN. RATING AS A SUBGRADE: EXCELLENT TO GOOD, FAIR TO POOR, FAIR TO POOR, POOR, UNSUITABLE PI OF A-7-5 SUBGROUP IS <= LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30		MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50 PERCENTAGE OF MATERIAL ORGANIC MATERIAL GRANULAR SILT-CLAY OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE GROUND WATER WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP		WEATHERED ROCK (WR) CRYSTALLINE ROCK (CR) NON-CRYSTALLINE ROCK (NCR) COASTAL PLAIN SEDIMENTARY ROCK (CP) WEATHERING FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SLI.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL. SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF. VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF. COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.			
CONSISTENCY OR DENSENESS PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²) GENERALLY GRANULAR MATERIAL (NON-COHESIVE) VERY LOOSE, LOOSE, MEDIUM DENSE, DENSE, VERY DENSE GENERALLY SILT-CLAY MATERIAL (COHESIVE) VERY SOFT, SOFT, MEDIUM STIFF, STIFF, VERY STIFF, HARD		MISCELLANEOUS SYMBOLS ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES TEST BORING W/ CORE SPT N-VALUE SPT REFUSAL AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION CONE PENETROMETER TEST SOUNDING ROD		ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.			
TEXTURE OR GRAIN SIZE U.S. STD. SIEVE SIZE OPENING (MM) 4, 10, 40, 60, 200, 270 COEFFICIENTS: 4.76, 2.00, 0.42, 0.25, 0.075, 0.053 BOULDER (BLDR.), COBBLE (COB.), GRAVEL (GR.), COARSE SAND (CSE. SD.), FINE SAND (F SD.), SILT (SL.), CLAY (CL.) GRAIN SIZE MM: 305, 75, 2.0, 0.25, 0.05, 0.005		ABBREVIATIONS AR - AUGER REFUSAL, BT - BORING TERMINATED, CL - CLAY, CPT - CONE PENETRATION TEST, CSE - COARSE, DMT - DILATOMETER TEST, DPT - DYNAMIC PENETRATION TEST, F - FINE, FOSS. - FOSSILIFEROUS, FRAC. - FRACTURED, FRAGMENTS, HI. - HIGHLY, MED. - MEDIUM, MICA - MICAEOUS, MOD. - MODERATELY, NP - NON PLASTIC, ORG. - ORGANIC, PMT - PRESSUREMETER TEST, SAP. - SAPROLITIC, SD. - SAND, SANDY, SL. - SILT, SILTY, SLI. - SLIGHTLY, TCR - TRICONE REFUSAL, # - MOISTURE CONTENT, V - VERY, VST - VANE SHEAR TEST, WEA. - WEATHERED, UNIT WEIGHT, DRY UNIT WEIGHT, SAMPLE ABBREVIATIONS: S - BULK, SS - SPLIT SPOON, ST - SHELBY TUBE, RS - ROCK, RT - RECOMPACTED TRIAXIAL, CBR - CALIFORNIA BEARING RATIO		FRACATURE SPACING TERM SPACING: VERY WIDE, WIDE, MODERATELY CLOSE, CLOSE, VERY CLOSE TERM THICKNESS: VERY THICKLY BEDDED, THICKLY BEDDED, THINLY BEDDED, VERY THINLY BEDDED, THICKLY LAMINATED, THINLY LAMINATED			
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION LL - LIQUID LIMIT, PL - PLASTIC LIMIT, OM - OPTIMUM MOISTURE, SL - SHRINKAGE LIMIT - SATURATED - (SAT.) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE		EQUIPMENT USED ON SUBJECT PROJECT DRILL UNITS: MOBILE B-, BK-51, CME-45C, CME-55B, PORTABLE HOIST ADVANCING TOOLS: CLAY BITS, 6" CONTINUOUS FLIGHT AUGER, 8" HOLLOW AUGERS, HARD FACED FINGER BITS, TUNG-CARBIDE INSERTS, CASING W/ ADVANCER, TRICONE STEEL TEETH, TRICONE TUNG-CARB., CORE BIT HAMMER TYPE: AUTOMATIC, MANUAL CORE SIZE: B, N, H HAND TOOLS: POST HOLE DIGGER, HAND AUGER, SOUNDING ROD, VANE SHEAR TEST		INDURATION FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.			
PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH NONPLASTIC 0-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH		COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		BENCH MARK: ELEVATION: FT. NOTES: APPROXIMATE LIMIT OF ORGANIC SOILS			

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	13201.1028025	2A	59
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
13201.1028025		P.E.	

STATE OF NORTH CAROLINA
 DIVISION OF HIGHWAYS
DARE COUNTY

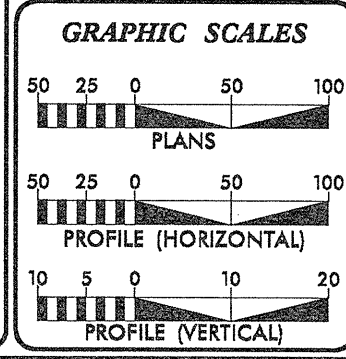
LOCATION: NC HWY 12 OVER PEA ISLAND BREACH
 TYPE OF WORK: GRADING, PAVING, DRAINAGE & STRUCTURE

See Sheet 1-A For Index of Sheets
 See Sheet 1-B For Standard Symbology Sheet



PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONTRACT: PROJECT: NC 12 PEA ISLAND BREACH



DESIGN DATA

ADT 2012 =	7,300
ADT 2032 =	10,900
DHV =	N/A
D =	N/A
T =	6%
V =	60 MPH
FUNC. CLASSIFICATION:	
COLLECTOR	
T = 1% TTST+5% DUALS	
REGIONAL TIER	

PROJECT LENGTH

LENGTH OF ROADWAY TIP PROJECT =	0.241 MILES
LENGTH OF STRUCTURE TIP PROJECT =	2.114 MILES
TOTAL LENGTH OF TIP PROJECT =	2.355 MILES

NCDOT CONTACT: DOUG TAYLOR, PE
 Asst. Unit Head, Roadway Design

PLANS PREPARED FOR THE NCDOT BY:
STV/RALPH WHITEHEAD ASSOCIATES, INC.
 1000 West Morehead St., Ste. 200, Charlotte NC, 28208
 NC License Number F-0991

2012 STANDARD SPECIFICATIONS

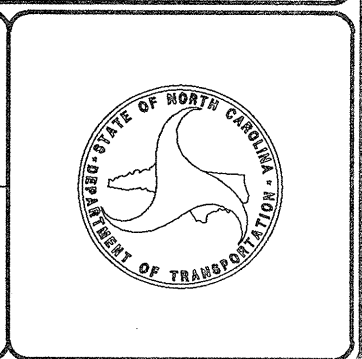
RIGHT OF WAY DATE:	N/A
LETTING DATE:	AUGUST 14, 2012

HYDRAULICS ENGINEER

SIGNATURE: _____ PE

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ PE



90% PLANS
 REVISED



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

BEVERLY EAVES PERDUE
GOVERNOR

EUGENE A. CONTI, JR.
SECRETARY

February 13, 2012

STATE PROJECT: 13201.1028025
F.A. PROJECT: N/A
COUNTY: Dare
DESCRIPTION: NC 12 over Pea Island Breach

SUBJECT: Geotechnical Inventory Report

Project Description

This project begins along NC 12 approximately 3.5 miles north of Rodanthe in Dare County, and extends northward approximately 2.6 miles. This geotechnical investigation was confined to the areas of proposed construction.

Fieldwork was conducted in January and February of 2012. Hand auger borings were completed at various offsets along the project corridor. Representative soil samples were collected for visual classification in the field and for laboratory analysis by the Materials and Tests Unit.

The following alignments were investigated. Subsurface profiles and selected cross sections of these alignments are included in this report.

<u>Line</u>	<u>Station(±)</u>
-L-	3083+00 to 3207+37
-DET-	3076+53 to 3215+10
-X1-	3157+01 to 3161+57
-X2-	3179+58 to 3185+13
-X3-	3159+00 to 3163+51
-X4-	3178+46 to 3182+19
-X5-	3159+00 to 3162+37

Areas of Special Geotechnical Interest

- 1) The entire project was found to exhibit seasonal high ground water.
- 2) The following section contains organic soils associated with marsh deposits found within the project area. These soils have the potential to cause embankment/subgrade and or slope stability problems during construction.

<u>Line</u>	<u>Station(±)</u>
-DET-	3112+85 to 3116+23

- 3) The following section contains organic soils found within an existing ditch running parallel to NC 12. These soils have the potential to cause embankment/subgrade and or slope stability problems during construction.

<u>Line</u>	<u>Station(±)</u>
-DET-	3087+27 to 3135+70

Physiography and Geology

This project corridor is located within the Coastal Plain Physiographic Province. Topography along the project is nearly flat to gently sloping. Natural ground elevations ranged from -8± below sea level in the channel of New Inlet to 7± feet above sea level.

Surficial soils in this area are generally classified as undivided coastal plain sediments.

Ground Water

Ground water data was collected in January and February of 2012, during a time of normal precipitation. Ground water elevations ranged from sea level to 4± feet above sea level.

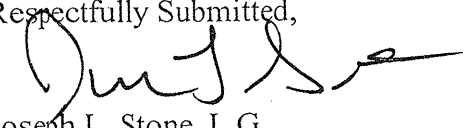
Soils

Soils encountered within this project area have been divided into two categories, undivided coastal plain soils, and roadway embankment.

Soils classified as undivided coastal plain are comprised of 6 or more feet of loose to dense sand (A-3), and less than 1 foot of soft brown muck.

Roadway embankment was encountered along NC 12. It is comprised of up to 8± feet of dense sand (A-3).

Respectfully Submitted,

A handwritten signature in black ink, appearing to read 'J. Stone', with a long horizontal flourish extending to the right.

Joseph L. Stone, L.G.
Project Engineering Geologist

PROJECT: B-2500A

COUNTY: Dare

Volumes in Cubic Yards

DATE: 1/16/13

COMPILED BY: STV

SHEET 1 OF 3 SHEETS

STATION	STATION	EXCAVATION					EMBANKMENT				BORROW	WASTE			
		TOTAL UNCLASS.	ROCK	UNDERCUT	UNSUIT. UNCLASS.	SUITABLE UNCLASS.	TOTAL	ROCK	EARTH	EMBANK. +30%		ROCK	SUITABLE	UNSUIT.	TOTAL
Temporary Widening															
-DET- 3113+00.00	-DET- 3121+93.40	75				75	46		46	60			15		15
-DET- 3144+93.40	-DET- 3158+27.42	163				163	309		309	402	239				
-DET- 3160+34.57	-DET- 3162+86.51	21				21	23		23	30	9				
-DET- 3184+25.56	-DET- 3195+16.97	258				258	19		19	25			233		233
	SUBTOTAL	517				517	397		397	517	248		248		248
Temporary Detour															
-DET- 3076+53.31	-DET- 3106+53.31	27				27	4,004		4,004	5,206	5,179				
-DET- 3106+53.31	-DET- 3136+46.71	133		52	50	83	2,703		2,703	3,514	3,431			102	102
-DET- 3136+46.71	-DET- 3166+34.87	387				387	2,287		2,287	2,974	2,587				
-DET- 3166+34.87	-DET- 3169+12.70 Begin Bridge	29				29	187		187	244	215				
-X1- 3157+01.57	-X1- 3161+57.48	91				91	109		109	142	51				
	SUBTOTAL	667		52	50	617	9,290		9,290	12,080	11,463			102	102
Temporary Detour															
-DET- 3175+74.74 End Bridge	-DET- 3205+68.69	937				937	1,436		1,436	1,867	930				
-DET- 3205+68.69	-DET- 3214+79.47	444				444	240		240	312			132		132
-X2- 3179+58.32	-X2- 3185+13.93	143				143	277		277	361	218				
	SUBTOTAL	1,524				1,524	1,953		1,953	2,540	1,148		132		132
Cross Over															
-X3- 3159+00.00	-X3- 3163+51.20	225				225	214		214	279	54				
-X4- 3178+46.41	-X4- 3182+19.84	245				245	108		108	141			104		104
	SUBTOTAL	470				470	322		322	420	54		104		104
Cross Over															
-X5- 3159+00.00	-X5- 3162+41.31	176				176	53		53	69			107		107
	SUBTOTAL	176				176	53		53	69			107		107
Permanent Road															
-L- 3083+00.00	-L- 3089+00.00 Begin Bridge	28				28	2,340		2,340	3,042	3,014				
	SUBTOTAL	28				28	2,340		2,340	3,042	3,014				
Permanent Road															
-L- 3200+60.00 End Bridge	-L- 3207+37.00	7				7	2,685		2,685	3,491	3,484				
	SUBTOTAL	7				7	2,685		2,685	3,491	3,484				
	SHEET TOTAL	3,389		52	50	3,339	17,040		17,040	22,159	19,411		591	102	693

NOTE: EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.

PROJECT: B-2500A

COUNTY: Dare

Volumes in Cubic Yards

DATE: 1/16/13

COMPILED BY: STV

SHEET 2 OF 3 SHEETS

STATION	STATION	EXCAVATION					EMBANKMENT				BORROW	WASTE			
		TOTAL UNCLASS.	ROCK	UNDERCUT	UNSUIT. UNCLASS.	SUITABLE UNCLASS.	TOTAL	ROCK	EARTH	EMBANK. +30%		ROCK	SUITABLE	UNSUIT.	TOTAL
Temporary Widening Removal															
-DET- 3113+00.00	-DET- 3121+93.40	60				60	58		58	75	15				
-DET- 3144+93.40	-DET- 3158+27.42	402				402	125		125	163			239	239	
-DET- 3160+34.57	-DET- 3162+86.51	30				30	16		16	21			9	9	
-DET- 3184+25.56	-DET- 3195+16.97	25				25	198		198	258	233				
	SUBTOTAL	517				517	398		398	517	248		248	248	
Temporary Detour Removal															
-DET- 3076+53.31	-DET- 3106+53.31	5,206				5,206	21		21	27			5,179	5,179	
-DET- 3106+53.31	-DET- 3136+46.71	3,514				3,514	64		64	83			3,431	3,431	
-DET- 3136+46.71	-DET- 3166+34.87	2,974				2,974	298		298	387			2,587	2,587	
-DET- 3166+34.87	-DET- 3169+12.70 Begin Bridge	244				244	22		22	29			215	215	
-X1- 3157+01.57	-X1- 3161+57.48	142				142	70		70	91			51	51	
	SUBTOTAL	12,080				12,080	475		475	617			11,463	11,463	
Temporary Detour Removal															
-DET- 3175+74.74 End Bridge	-DET- 3205+68.69	1,867				1,867	721		721	937			930	930	
-DET- 3205+68.69	-DET- 3214+79.47	312				312	342		342	444	132				
-X2- 3179+58.32	-X2- 3185+13.93	361				361	110		110	143			218	218	
	SUBTOTAL	2,540				2,540	1,172		1,172	1,524	132		1,148	1,148	
Cross Over Removal															
-X3- 3159+00.00	-X3- 3163+51.20	279				279	173		173	225			54	54	
-X4- 3178+46.41	-X4- 3182+19.84	141				141	188		188	245	104				
	SUBTOTAL	420				420	362		362	470	104		54	54	
Cross Over															
-X5- 3159+00.00	-X5- 3162+41.31	69				69	135		135	176	107				
	SUBTOTAL	69				69	135		135	176	107				
	SHEET TOTAL	15,626				15,626	2,542		2,542	3,304	591		12,913	12,913	

NOTE: EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.

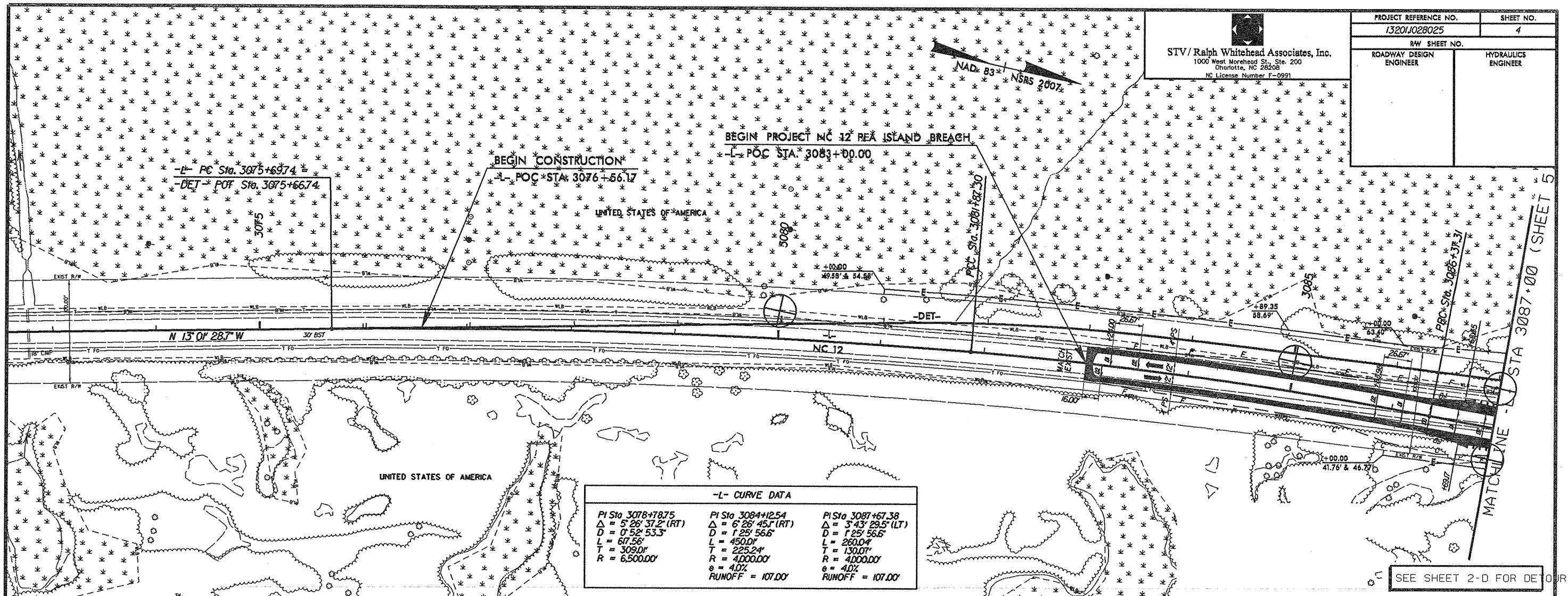
30

PROJECT: B-2500A COUNTY: Dare DATE: 1/16/13 Volumes in Cubic Yards COMPILED BY: STV SHEET 3 OF 3 SHEETS

STATION	STATION	EXCAVATION					EMBANKMENT				BORROW	WASTE			
		TOTAL UNCLASS.	ROCK	UNDERCUT	UNSUIT. UNCLASS.	SUITABLE UNCLASS.	TOTAL	ROCK	EARTH	EMBANK. +30%		ROCK	SUITABLE	UNSUIT.	TOTAL
	TOTAL FROM SHEET 1	3,389		52	50	3,339	17,040		17,040	22,159	19,411		591	102	693
	TOTAL FROM SHEET 2	15,626				15,626	2,542		2,542	3,304	591		12,913		12,913
	TOTAL	19,015		52	50	18,965	19,582		19,582	25,463	20,002		13,504	102	13,606
	MATERIAL FOR SHOULDER CONSTRUCTION LOSS DUE TO CLEARING & GRUBBING						1,362		1,362	1,771	1,771				
	ADDITIONAL UNDERCUT			2,600			2,600		2,600	3,380	3,380			2,600	2,600
	ROCK WASTE TO REPLACE BORROW														
	ADJUST FOR ROCK WASTE														
	WASTE IN LIEU OF BORROW										-1,182		-1,182		-1,182
	PROJECT TOTAL	19,015		2,652	50	18,965	23,544		23,544	30,614	23,971		12,322	2,702	15,024
	EST. 5% TO REPLACE TOP SOIL ON BORROW PIT										1,199				
	GRAND TOTAL	19,015		2,652	50	18,965	23,544		23,544	30,614	25,170		12,322	2,702	15,024
	SAY	19,100									25,200				

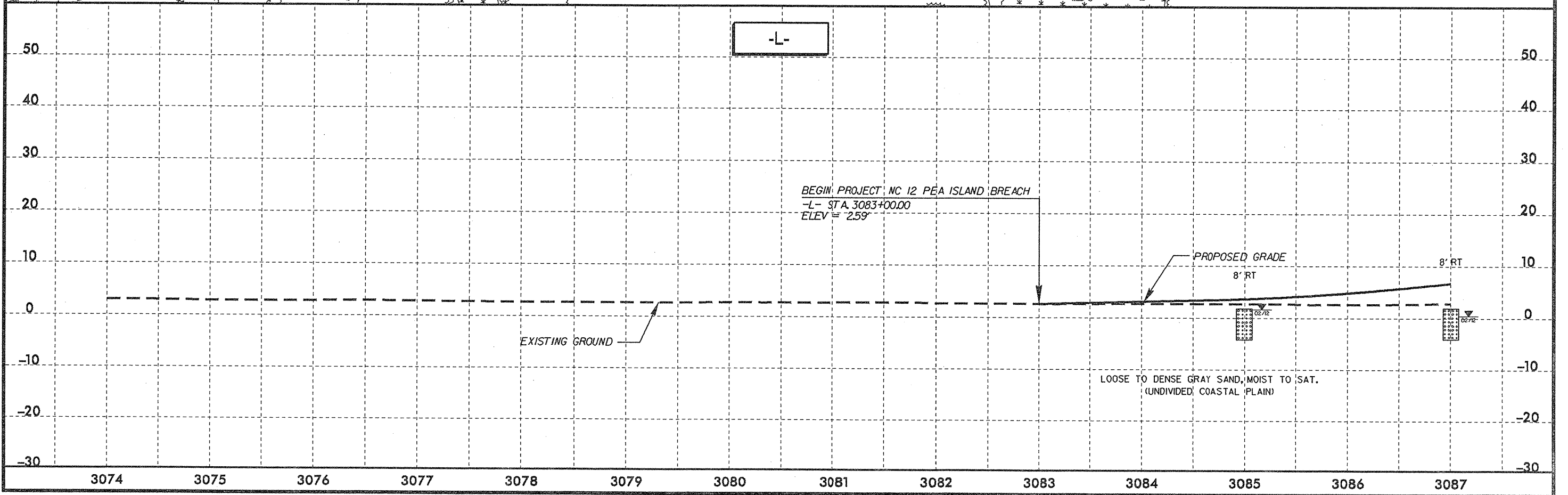
NOTE: EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.

STV/Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991



-L- CURVE DATA

PI Sta 3078+18.75 Δ = 5° 28' 37.2" (RT) D = 0' 52' 53.3" L = 67.56' T = 309.01' R = 6,500.00'	PI Sta 3084+12.54 Δ = 6° 26' 45.1" (RT) D = 1' 25' 56.6" L = 450.01' T = 225.24' R = 4,000.00' e = 4.0% RUNOFF = 107.00'	PI Sta 3087+67.38 Δ = 3° 43' 29.5" (LT) D = 1' 25' 56.6" L = 260.04' T = 130.01' R = 4,000.00' e = 4.0% RUNOFF = 107.00'
--	---	---



\$DATE\$
\$FILE\$

MATCHLINE - STA 3087+00 (SHEET 5)

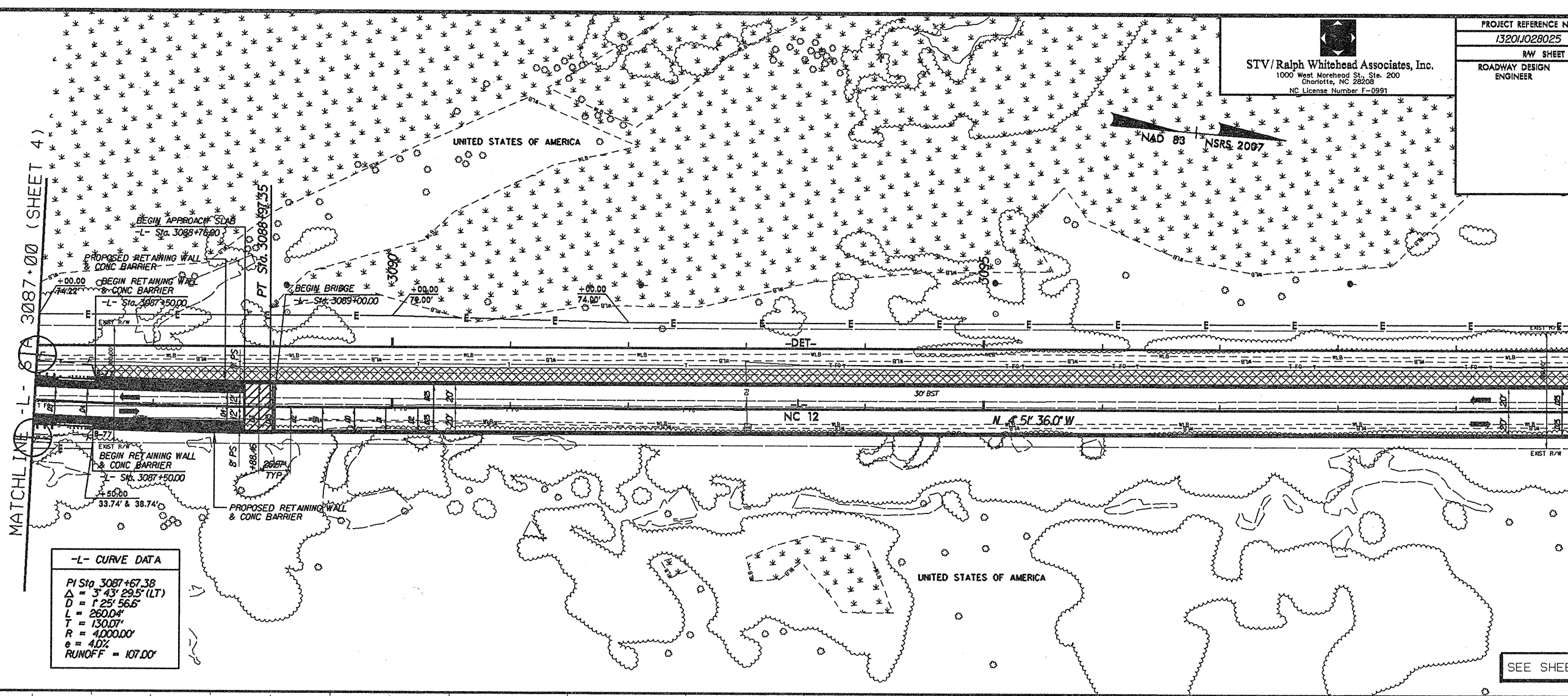
SEE SHEET 2-D FOR DETAIL

STV/Ralph Whitehead Associates, Inc.
 1000 West Morehead St., Ste. 200
 Charlotte, NC 28208
 NC License Number F-0991

PROJECT REFERENCE NO. 13201J028025	SHEET NO. 5
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

MATCHLINE -L- STA 3087+00 (SHEET 4)

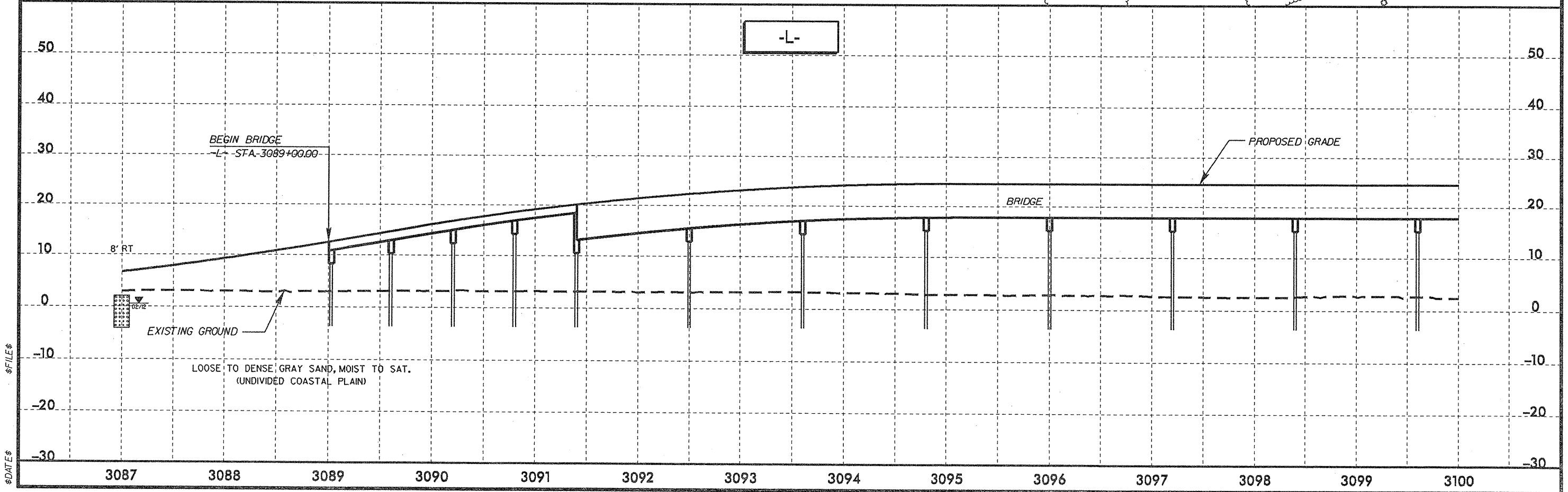
MATCHLINE -L- STA 3100+00 (SHEET 6)



-L- CURVE DATA

PI Sta 3087+67.38
$\Delta = 3^\circ 43' 29.5''$ (LT)
$D = 1^\circ 25' 56.6''$
$L = 260.04'$
$T = 130.02'$
$R = 4000.00'$
$e = 4.0\%$
$RUNOFF = 107.00'$

SEE SHEET 2-E FOR DETOUR

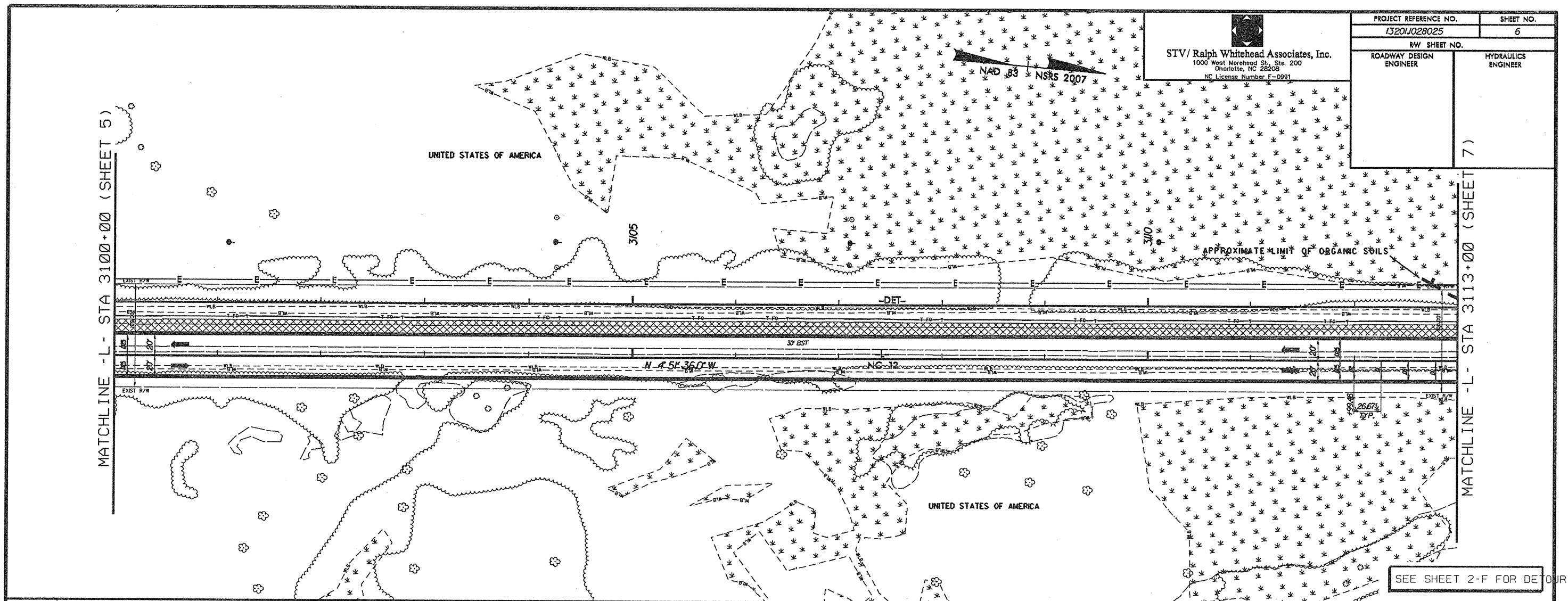


PROJECT REFERENCE NO.		SHEET NO.	
13201028025		6	
RAW SHEET NO.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	

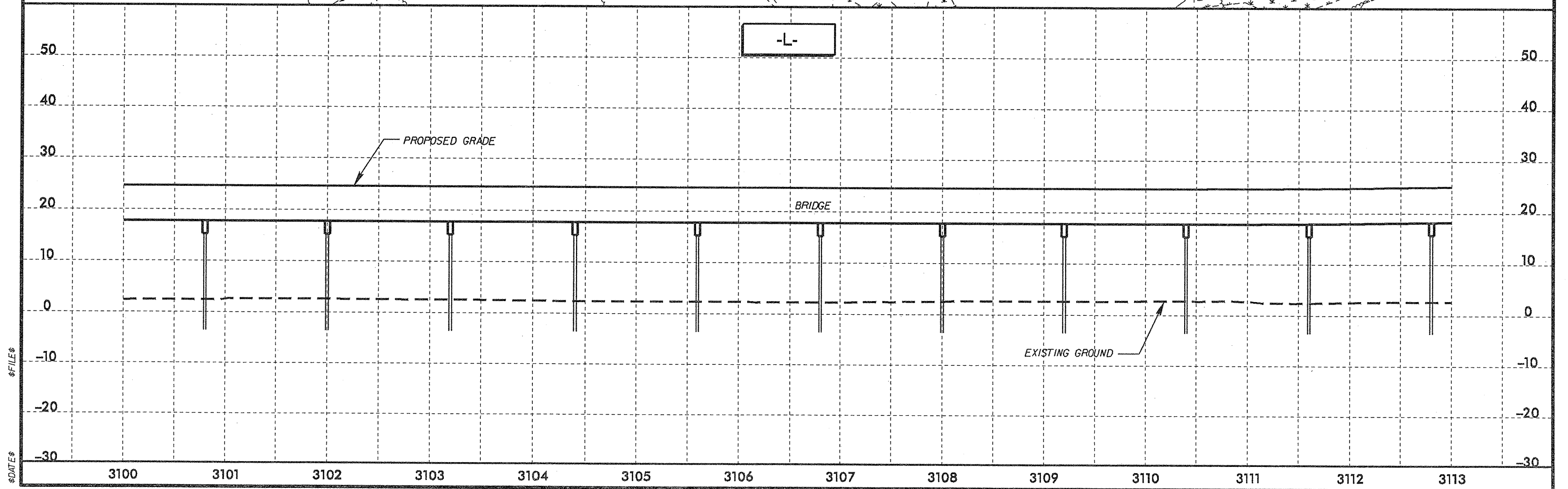
STV/Ralph Whitehead Associates, Inc.
 1000 West Morehead St., Ste. 200
 Charlotte, NC 28208
 NC License Number F-0991

MATCHLINE - L - STA 3100+00 (SHEET 5)

MATCHLINE - L - STA 3113+00 (SHEET 7)

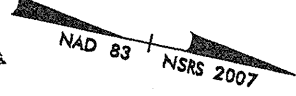


SEE SHEET 2-F FOR DETAIL



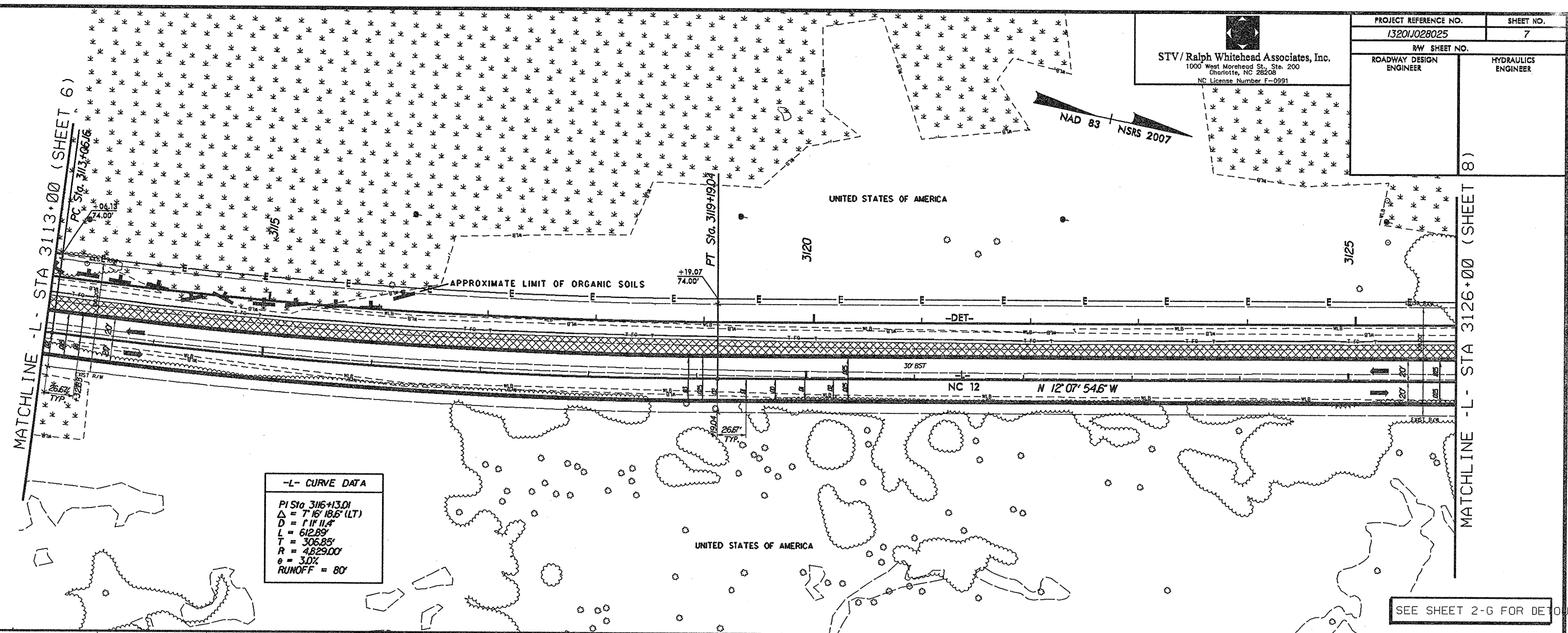
PROJECT REFERENCE NO. 13201028025	SHEET NO. 7
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

STV/Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number 7-0991



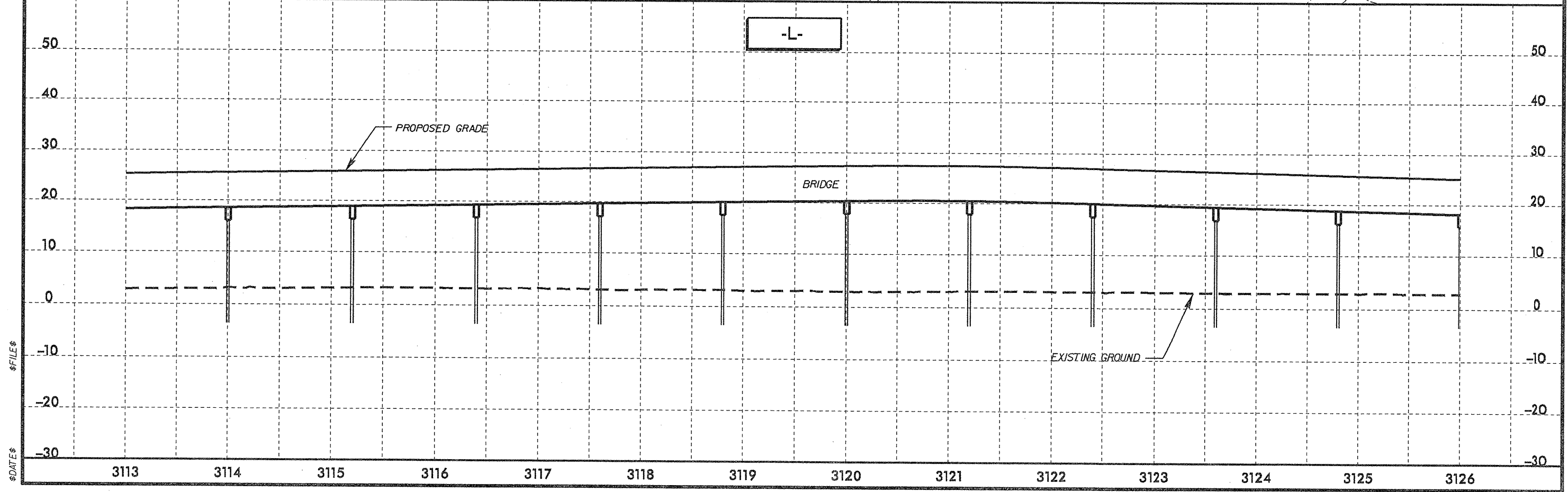
MATCHLINE -L- STA 3113+00 (SHEET 6)

MATCHLINE -L- STA 3126+00 (SHEET 8)



-L- CURVE DATA	
PI Sta	3116+13.01
Δ	7° 16' 18.5\" (LT)
D	111' 11.4\"
L	612.89'
T	306.85'
R	4829.00'
e	3.0%
RUNOFF	= 80'

SEE SHEET 2-G FOR DETAIL



\$DATE\$

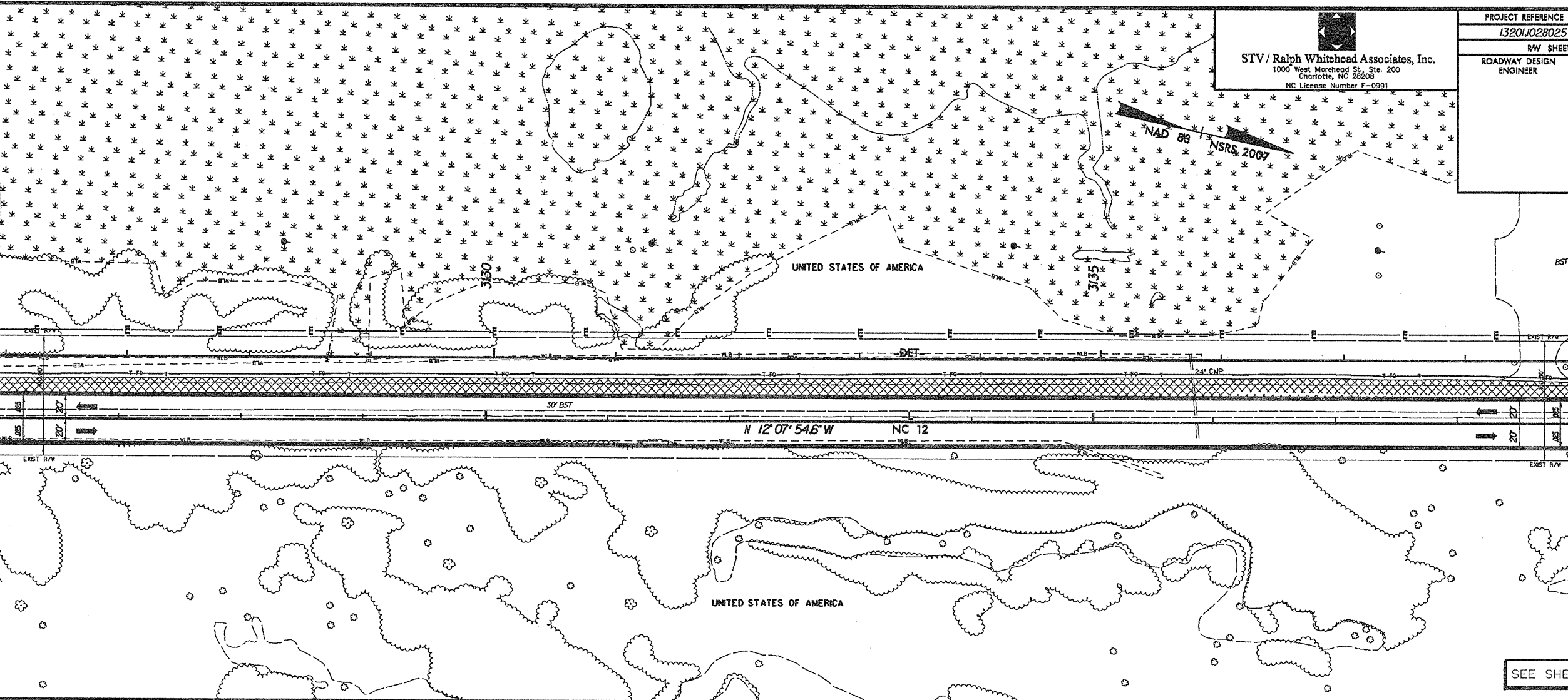
\$FILE\$

PROJECT REFERENCE NO. 13201028025	SHEET NO. 8
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

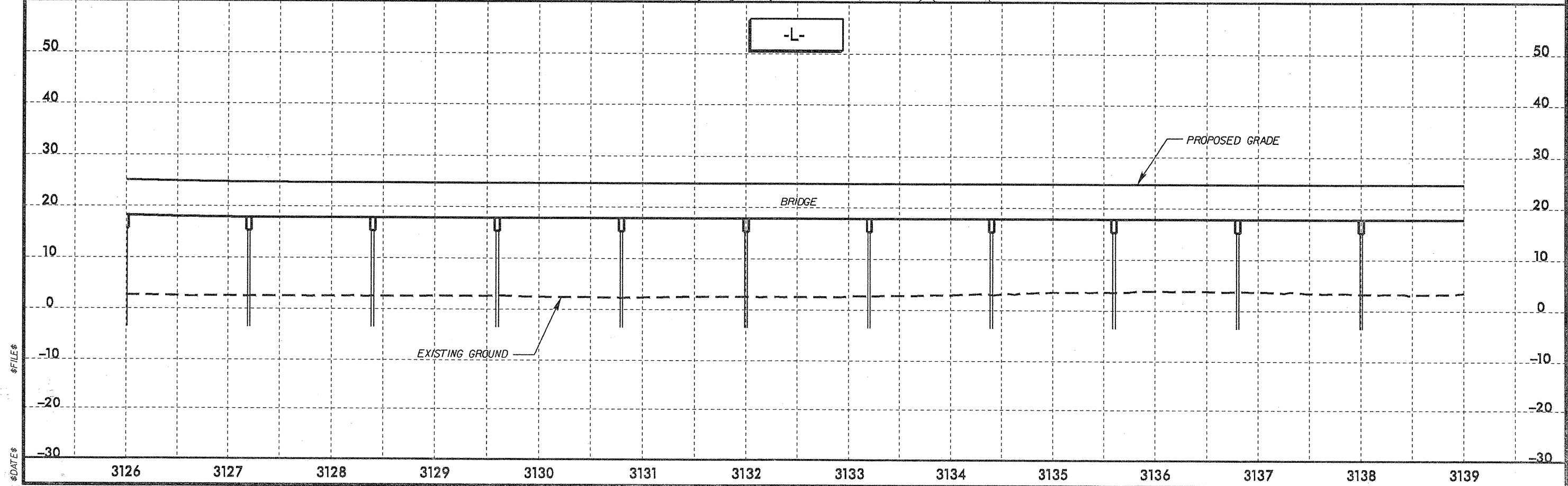
STV/Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991

MATCHLINE - L - STA 3126+00 (SHEET 7)

MATCHLINE - L - STA 3139+00 (SHEET 9)



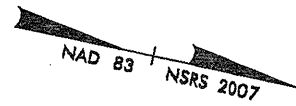
SEE SHEET 2-H FOR DETAIL



PROJECT REFERENCE NO. 13201028025	SHEET NO. 9
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

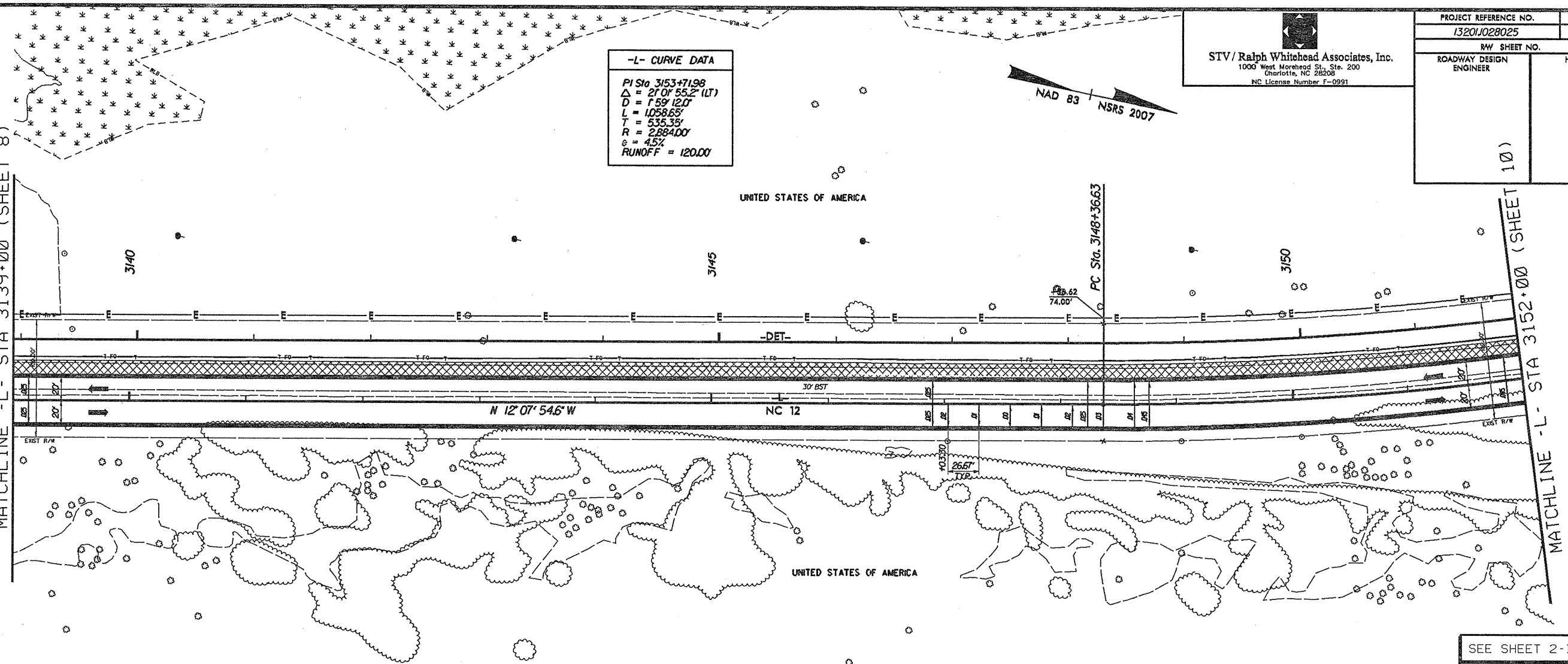
STV / Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28203
NC License Number F-0991

-L- CURVE DATA
 PI Sta 3153+71.98
 $\Delta = 27^{\circ} 07' 55.2" (LT)$
 $D = 159' 12.0"$
 $L = 1058.65'$
 $T = 535.35'$
 $R = 2884.00'$
 $e = 4.5\%$
 RUNOFF = 120.00'

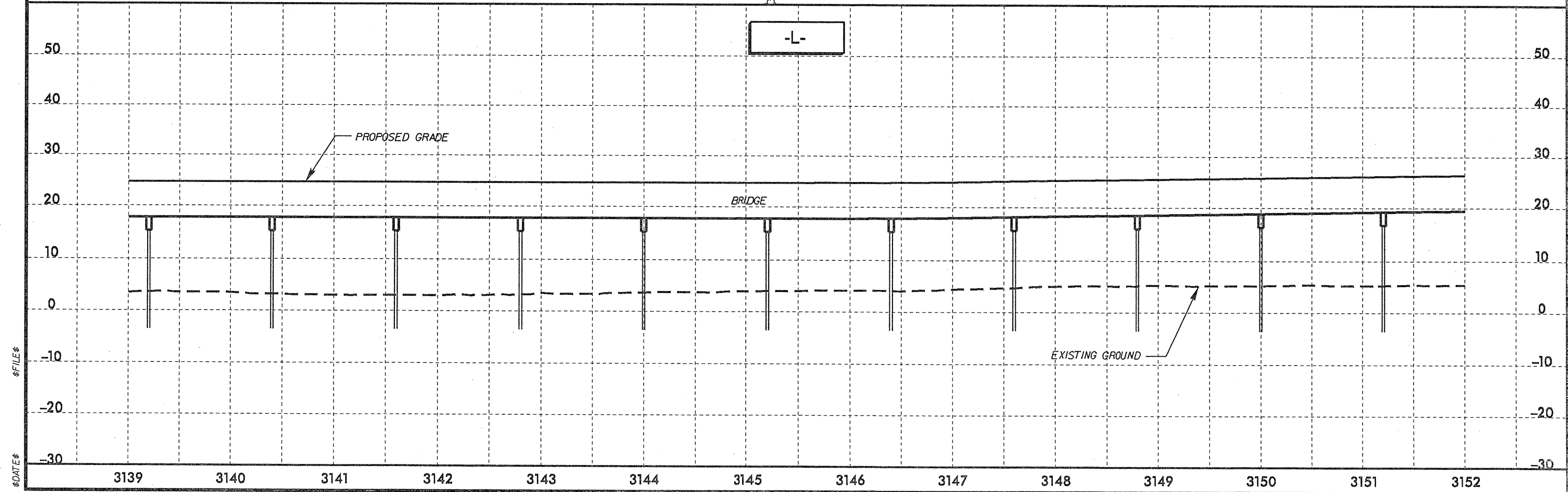


MATCHLINE -L- STA 3139+00 (SHEET 8)

MATCHLINE -L- STA 3152+00 (SHEET 10)



SEE SHEET 2-I FOR DETAIL

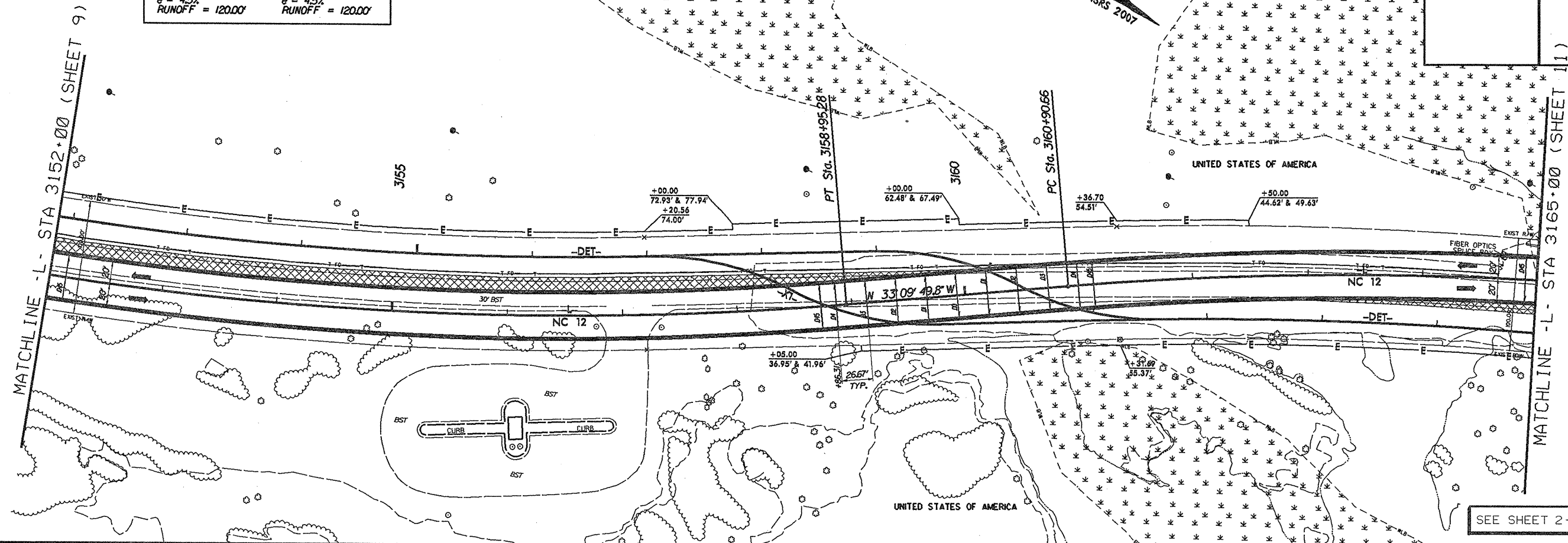


-L- CURVE DATA

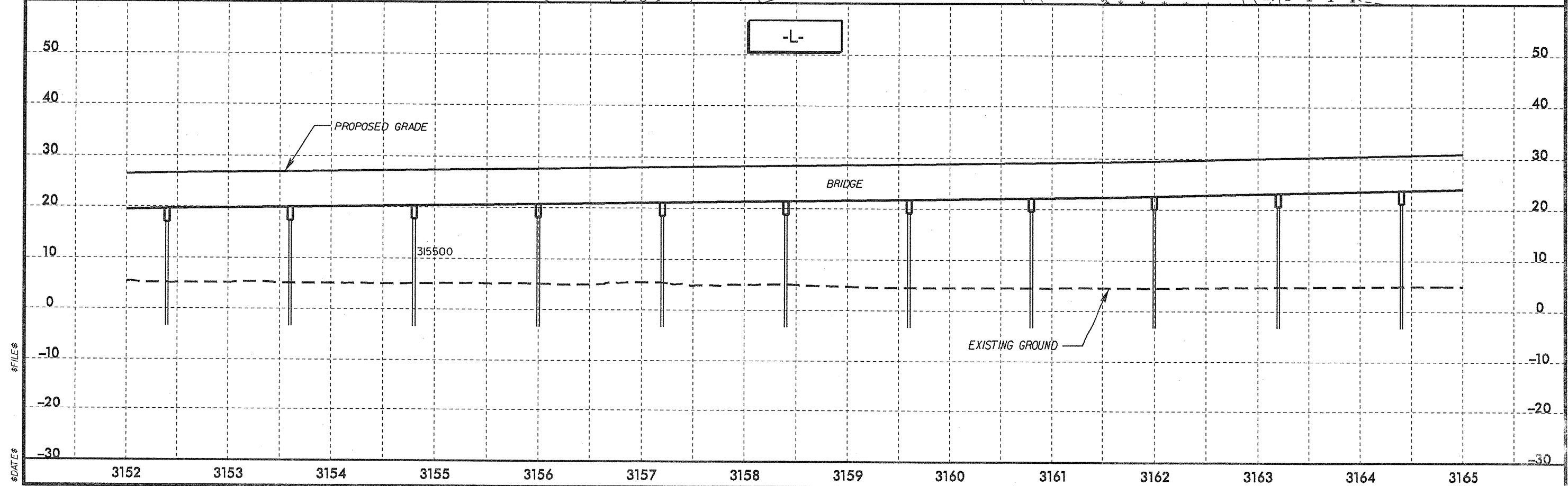
PI Sta 3153+71.96	PI Sta 3170+92.51
$\Delta = 21^{\circ} 0' 55.2" (LT)$	$\Delta = 38^{\circ} 18' 45.6" (RT)$
$D = 1^{\circ} 59' 12.0"$	$D = 1^{\circ} 59' 12.0"$
$L = 1058.65'$	$L = 1928.48'$
$T = 535.35'$	$T = 1001.85'$
$R = 2884.00'$	$R = 2884.00'$
$e = 4.5\%$	$e = 4.5\%$
RUNOFF = 120.00'	RUNOFF = 120.00'

PROJECT REFERENCE NO. 1320J028025		SHEET NO. 10	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	

STV / Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991



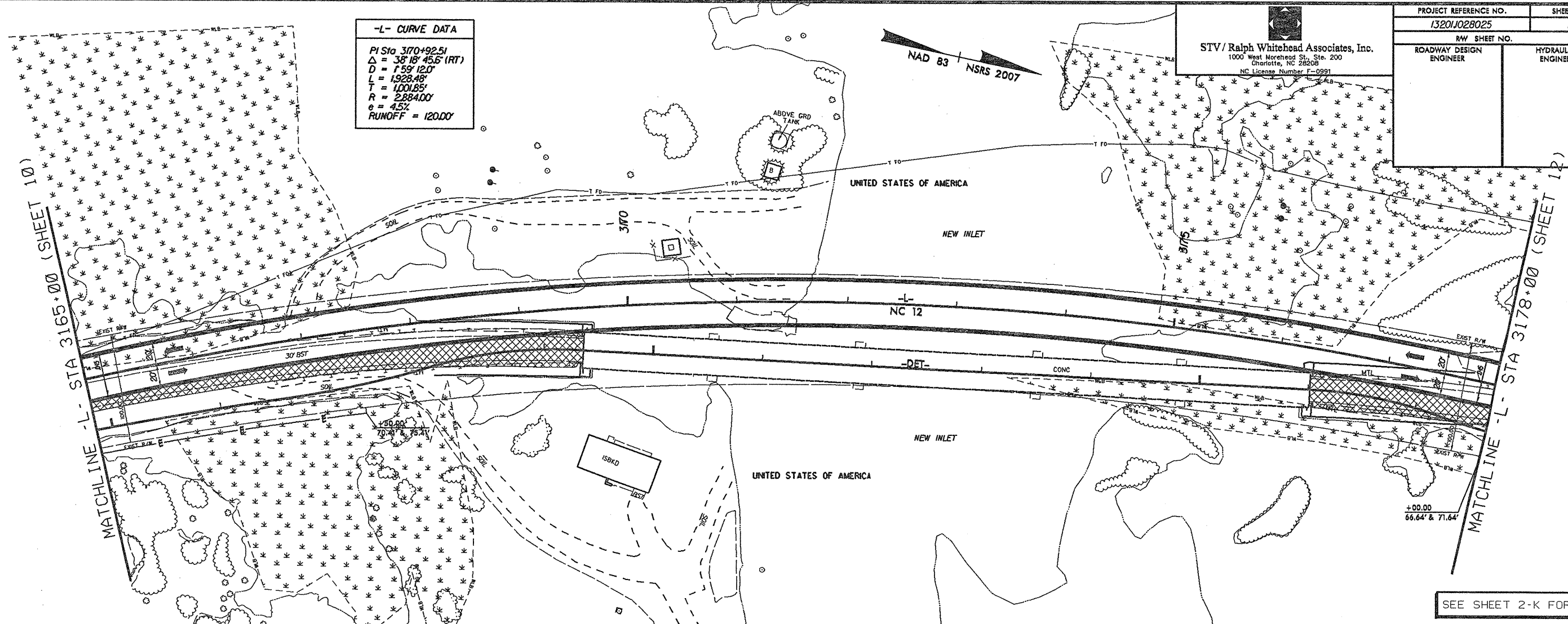
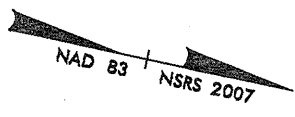
SEE SHEET 2-J FOR DETAIL



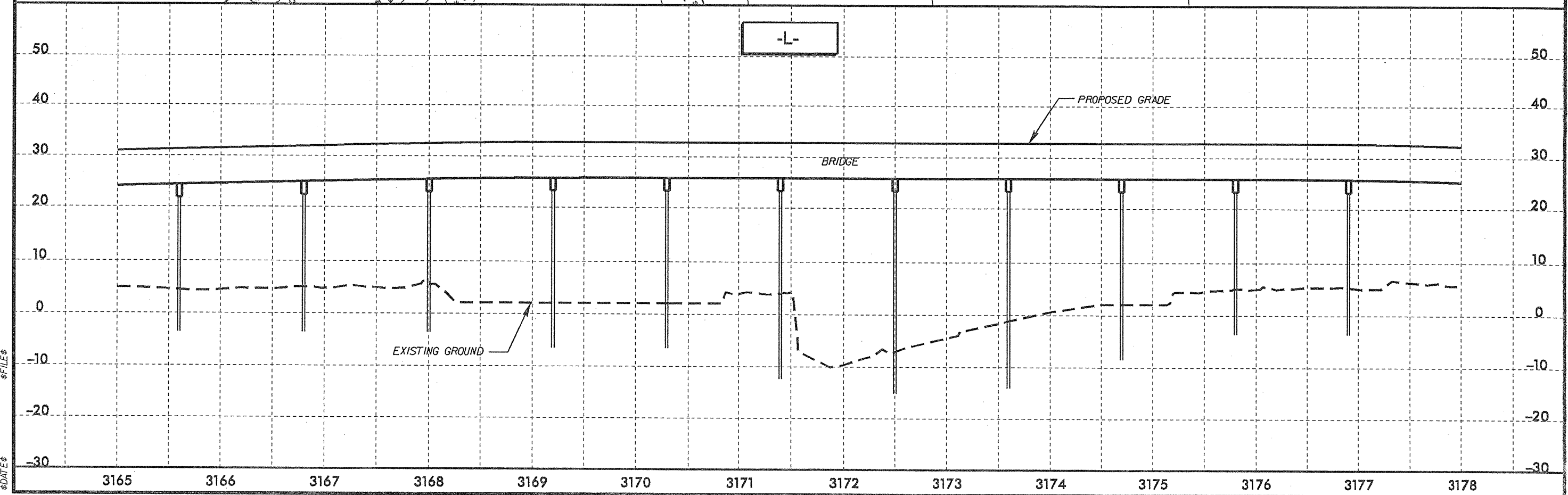
PROJECT REFERENCE NO.		SHEET NO.	
13201028025		II	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	

STV/ Ralph Whitehead Associates, Inc.
 1000 West Morehead St., Ste. 200
 Charlotte, NC 28208
 NC License Number F-0991

-L- CURVE DATA
 PI Sta 3170+92.51
 $\Delta = 38^{\circ} 18' 45.6" (RT)$
 $D = 1^{\circ} 59' 12.0"$
 $L = 1928.48'$
 $T = 1001.85'$
 $R = 2884.00'$
 $e = 4.5\%$
 RUNOFF = 120.00'



SEE SHEET 2-K FOR DE



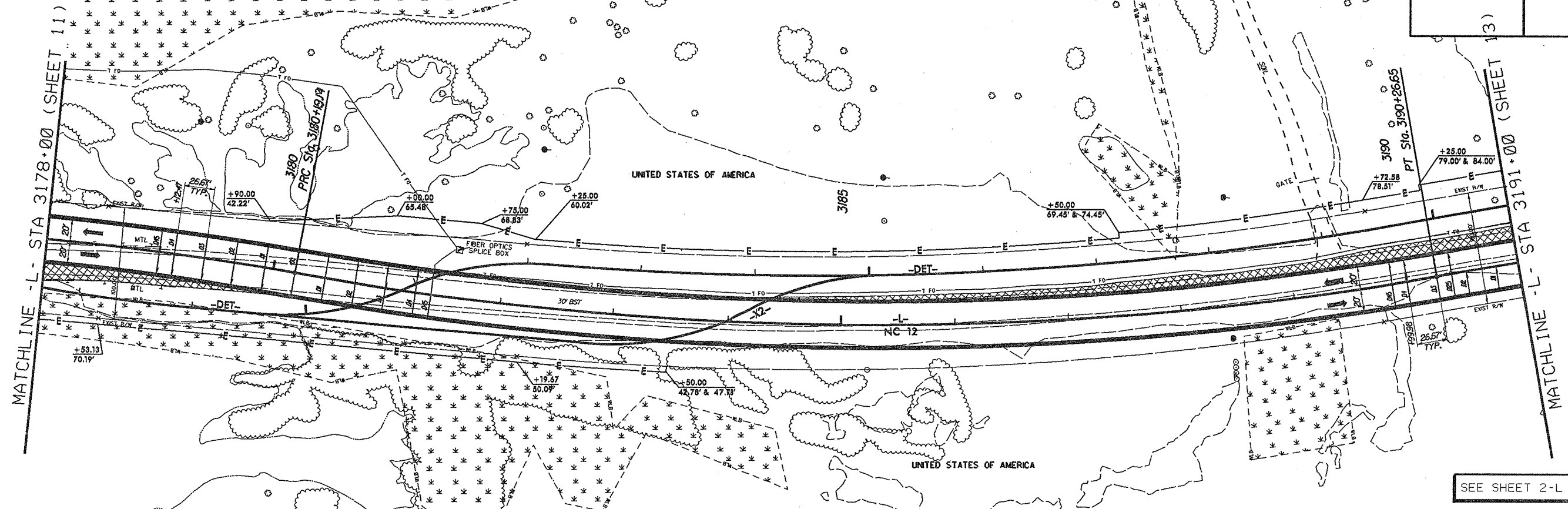
\$DATE\$

-L- CURVE DATA	
PI Sta 3170+92.51	PI Sta 3185+28.08
$\Delta = 37^{\circ}18'45.6"$ (RT)	$\Delta = 20^{\circ}00'57.9"$ (LT)
$D = 1^{\circ}59'12.0"$	$D = 1^{\circ}59'12.0"$
$L = 1,928.48'$	$L = 1,007.52'$
$T = 1,001.85'$	$T = 508.94'$
$R = 2,884.00'$	$R = 2,884.00'$
$e = 4.5\%$	$e = 4.5\%$
RUNOFF = 120.00'	RUNOFF = 120.00'

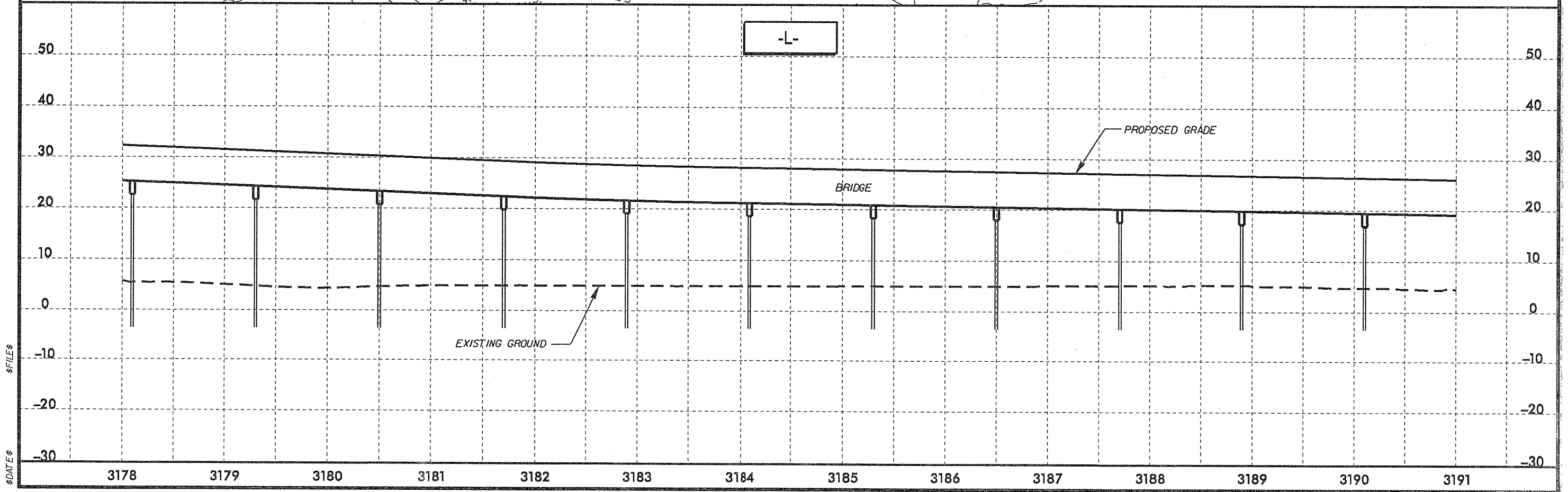
STV/Ralph Whitehead Associates, Inc.
 1000 West Morehead St., Ste. 200
 Charlotte, NC 28208
 NC License Number F-0991

PROJECT REFERENCE NO. 13201028025	SHEET NO. 12
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

NAD 83 | NSRS 2007



SEE SHEET 2-L FOR DETAIL



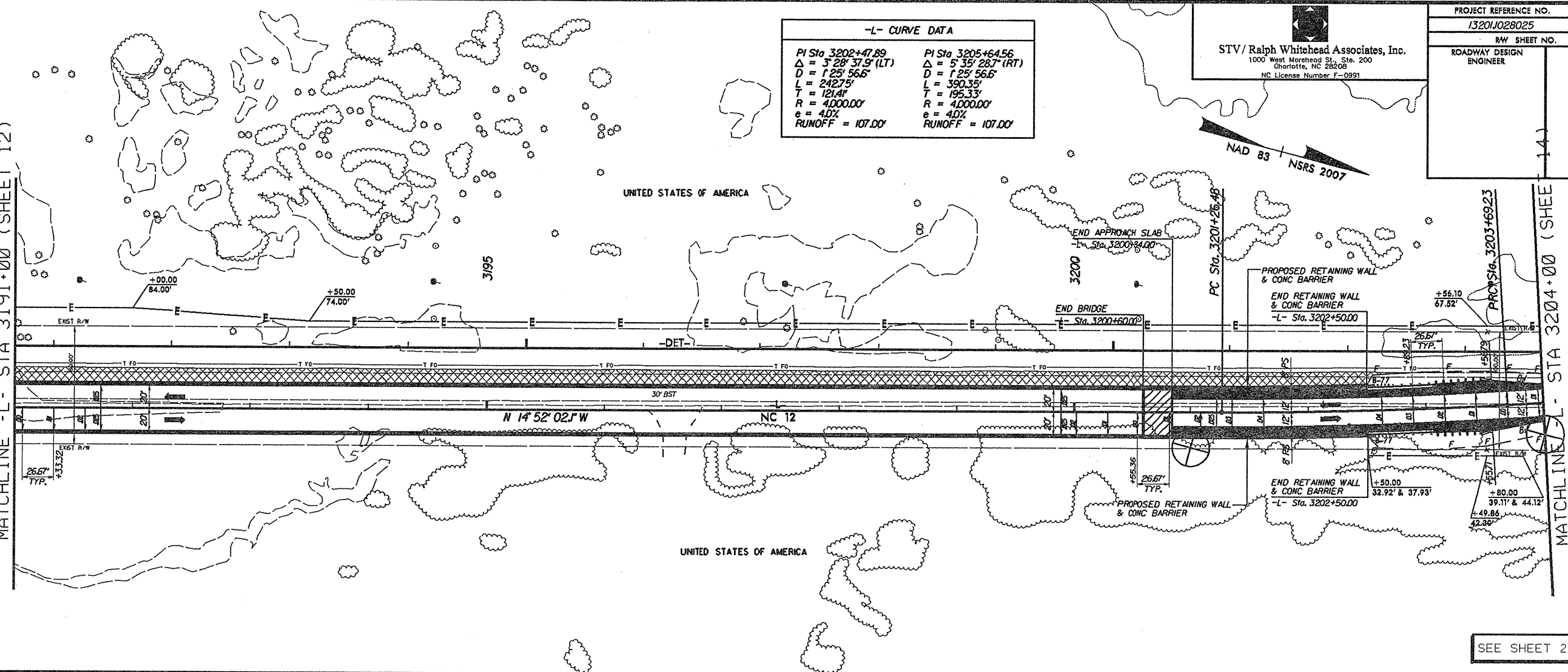
PROJECT REFERENCE NO. 1320J028025	SHEET NO. 13
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

STV / Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991

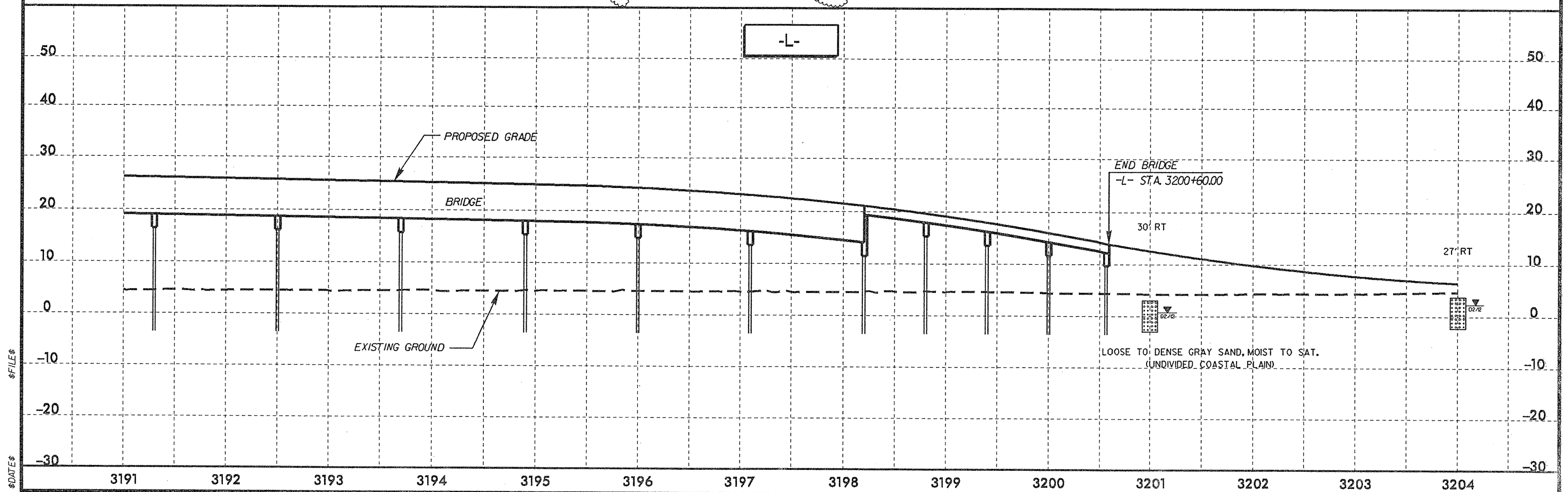
-L- CURVE DATA	
PI Sta 3202+47.89	PI Sta 3205+64.56
$\Delta = 3^{\circ} 28' 37.9"$ (LT)	$\Delta = 5^{\circ} 35' 28.7"$ (RT)
$D = 1^{\circ} 25' 56.6"$	$D = 1^{\circ} 25' 56.6"$
$L = 242.75'$	$L = 390.35'$
$T = 121.41'$	$T = 195.33'$
$R = 4000.00'$	$R = 4000.00'$
$e = 4.0\%$	$e = 4.0\%$
RUNOFF = 107.00'	RUNOFF = 107.00'

MATCHLINE -L- STA 3191+00 (SHEET 12)

MATCHLINE -L- STA 3204+00 (SHEET 14)



SEE SHEET 2-M FOR DETAILS

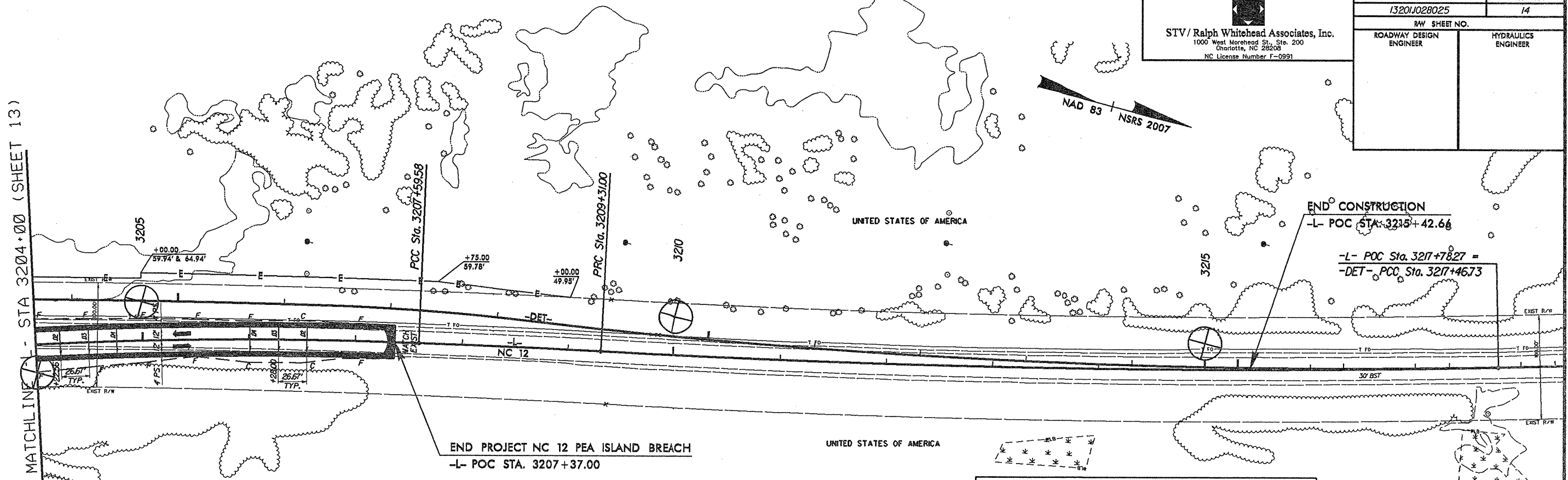


\$DATE\$

STV / Ralph Whitehead Associates, Inc.
 1000 West Morehead St., Sta. 200
 Charlotte, NC 28208
 NC License Number F-0991

PROJECT REFERENCE NO. 13201028025	SHEET NO. 14
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

MATCHLINE - STA 3204+00 (SHEET 13)



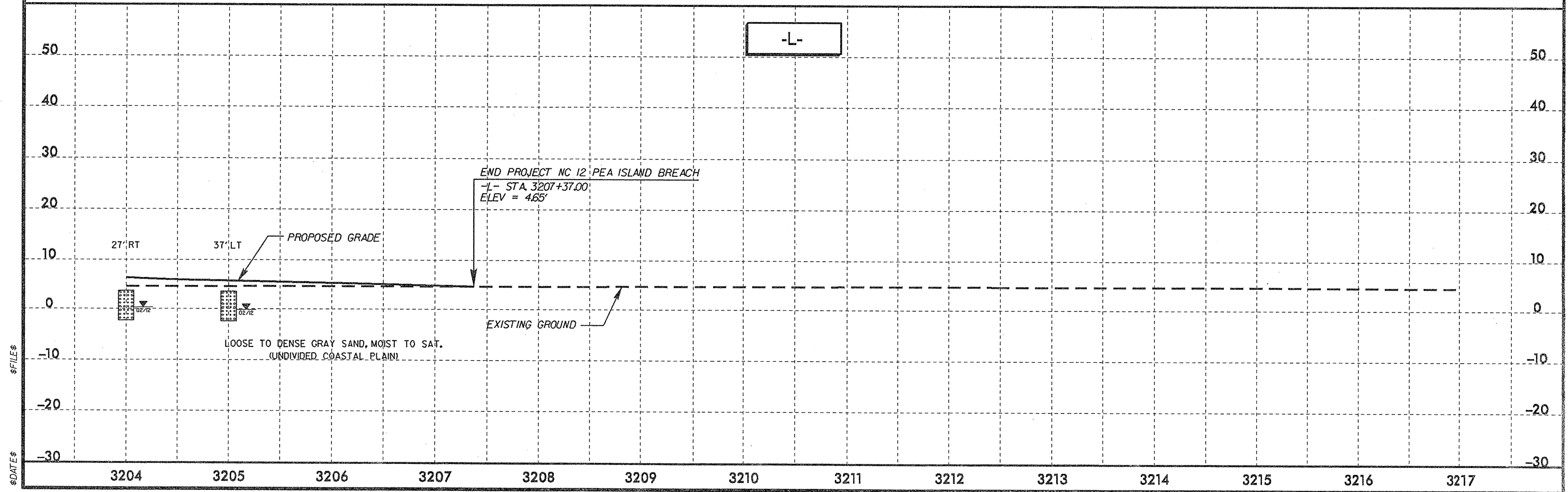
END PROJECT NC 12 PEA ISLAND BREACH
 -L- POC STA. 3207+37.00

END CONSTRUCTION
 -L- POC STA. 3215+42.66
 -L- POC Sta. 3217+78.27 =
 -DET- POC Sta. 3217+46.73

-L- CURVE DATA

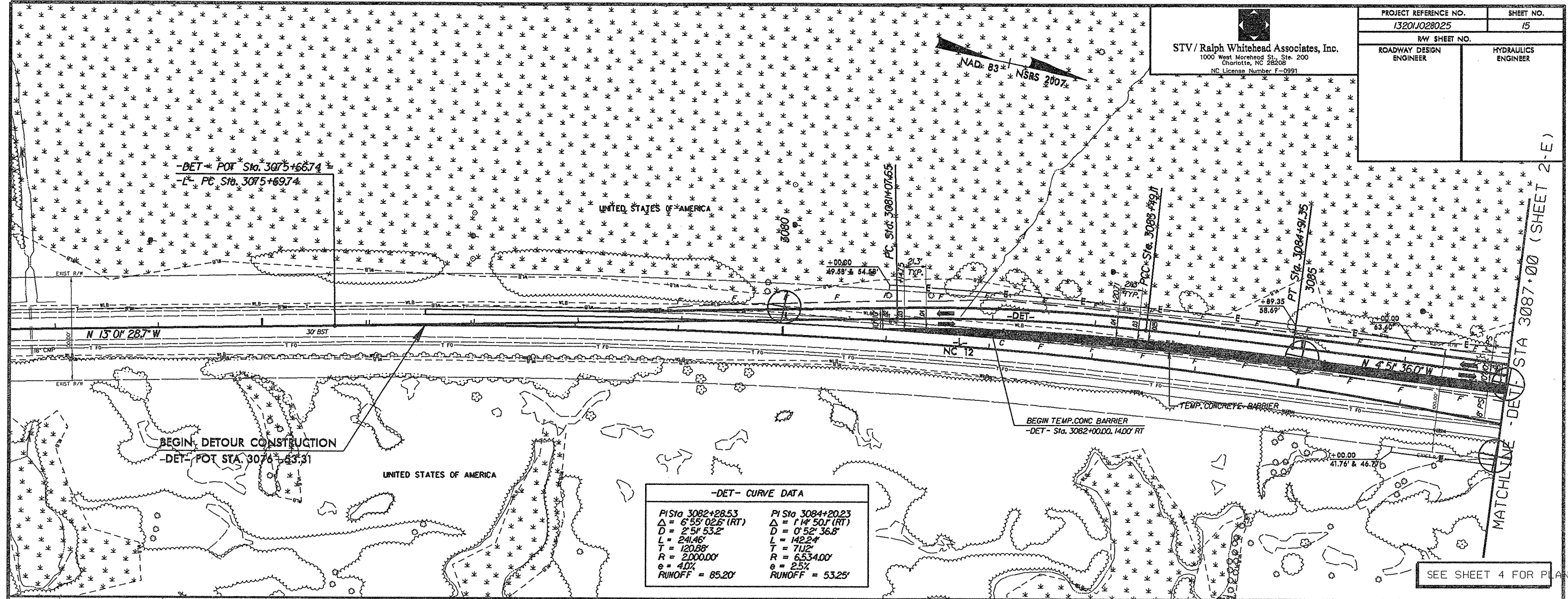
PI Sta 3205+64.56 Δ = 5° 35' 28.7" (RT) D = 1° 25' 56.6" L = 390.35' T = 195.33' R = 4,000.00' e = 4.0% RUNOFF = 107.00'	PI Sta 3208+45.29 Δ = 0° 51' 25.3" (RT) D = 0° 29' 59.9" L = 171.42' T = 85.71' R = 11,460.00'	PI Sta 3217+33.43 Δ = 8° 00' 38.4" (LT) D = 0° 29' 59.9" L = 1602.25' T = 802.43' R = 11,460.00'
---	---	---

SEE SHEET 2-N FOR DETOUR

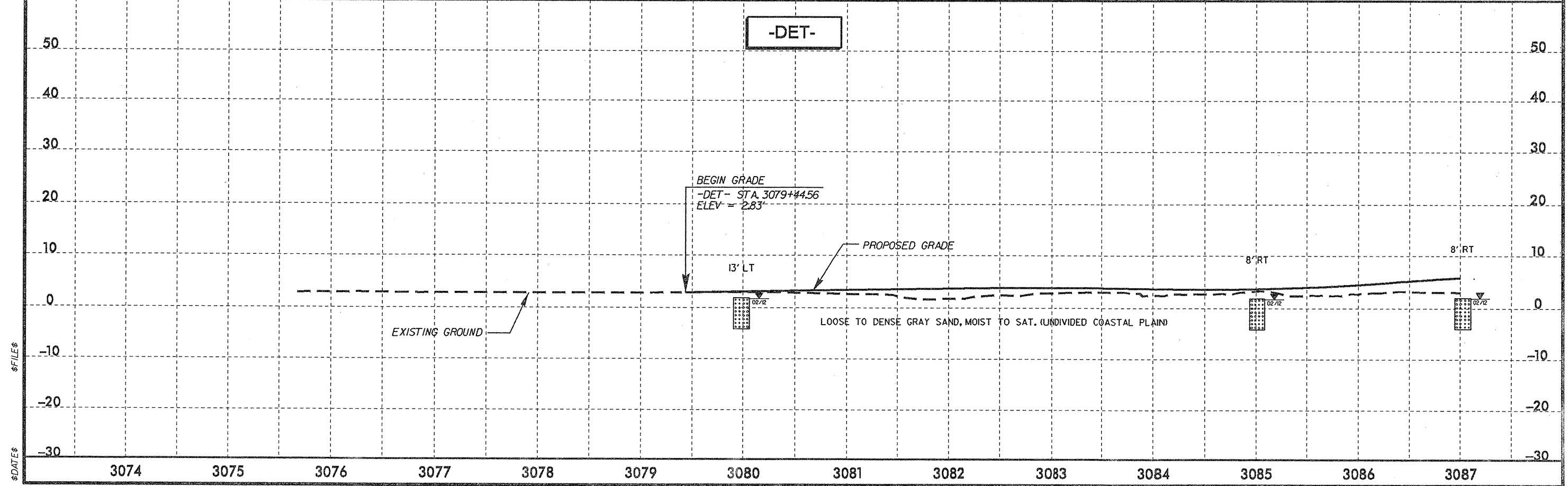


\$FILE\$
\$DATE\$

STV/Ralph Whitehead Associates, Inc.
 1000 West Morehead St., Ste. 200
 Charlotte, NC 28208
 NC License Number F-0991



SEE SHEET 4 FOR PLAN

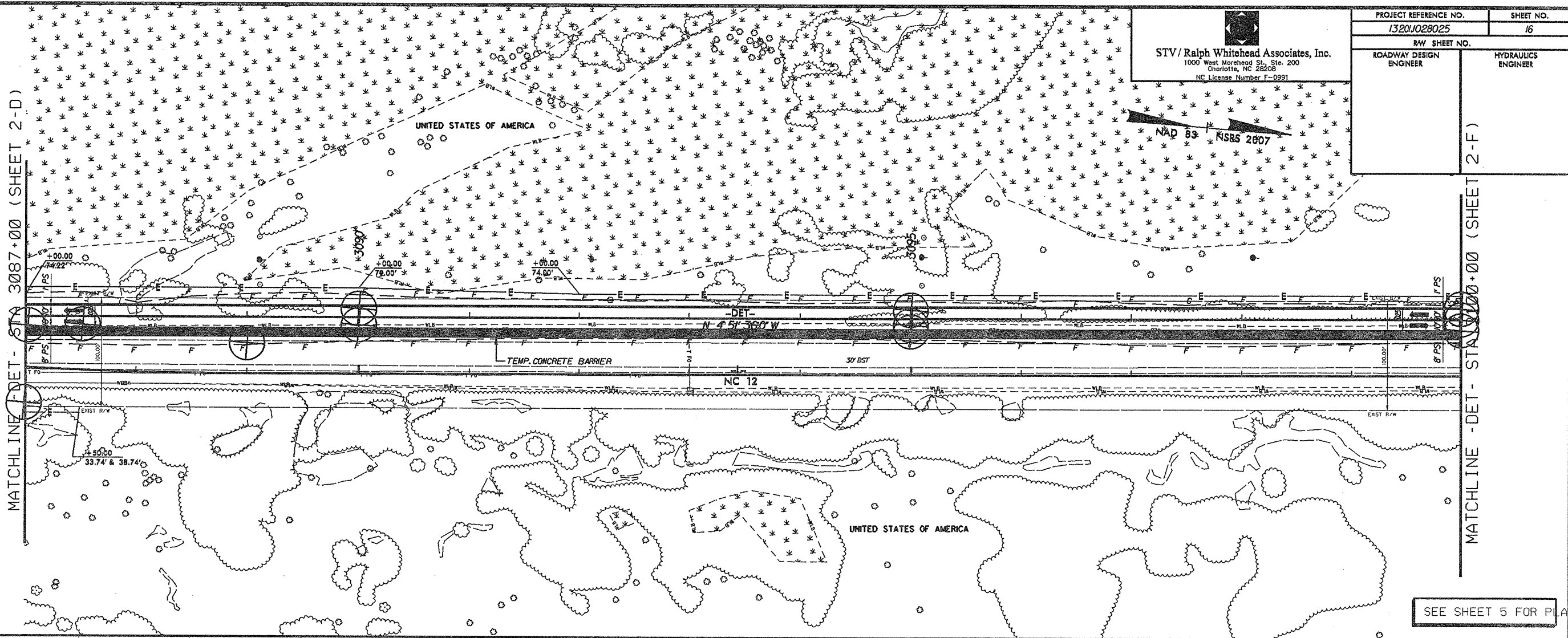


STV/Ralph Whitehead Associates, Inc.
 1000 West Morehead St., Ste. 200
 Charlotte, NC 28208
 NC License Number F-0991

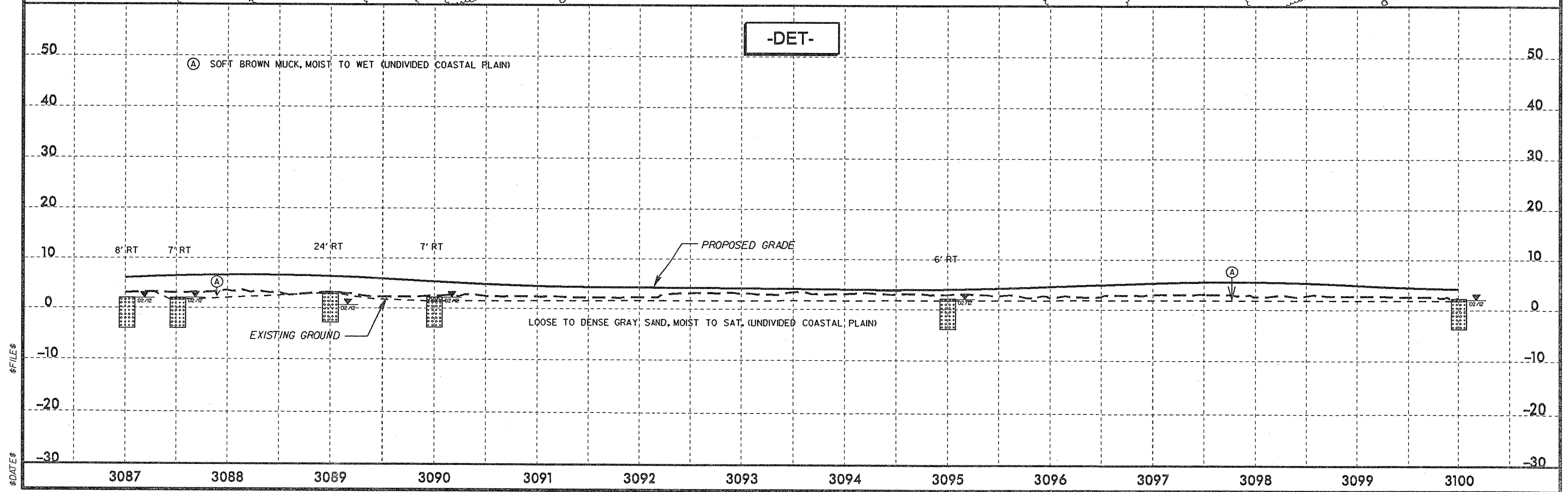
PROJECT REFERENCE NO. 13201028025	SHEET NO. 16
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

MATCHLINE - DET - STA 3087+00 (SHEET 2-D)

MATCHLINE - DET - STA 3100+00 (SHEET 2-F)

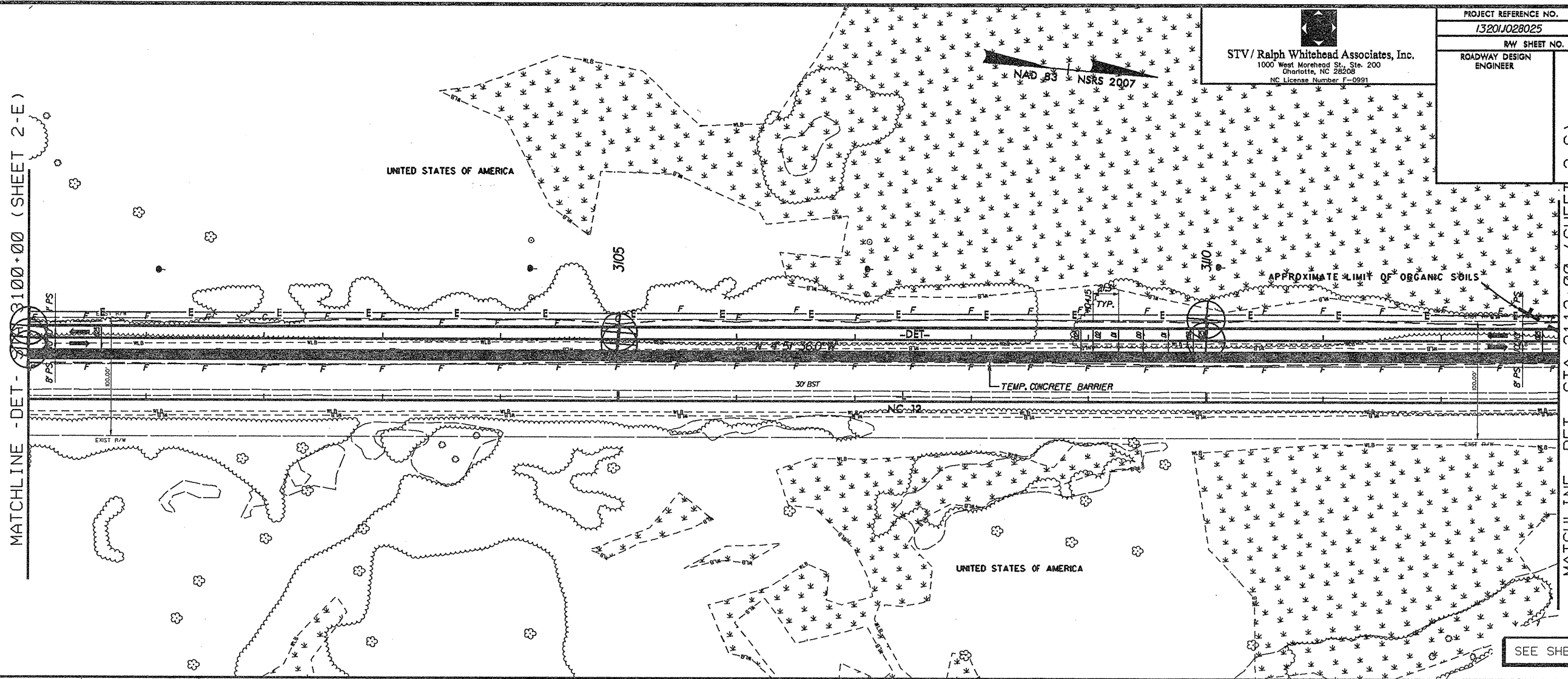


SEE SHEET 5 FOR PLAN

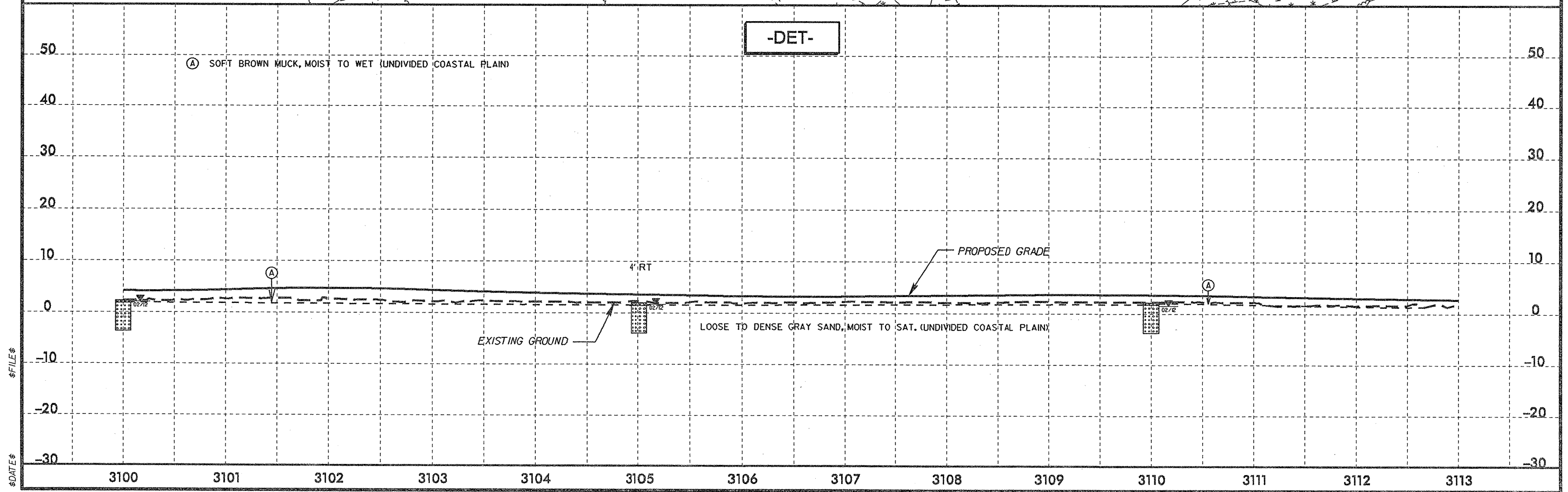


PROJECT REFERENCE NO.	SHEET NO.
13201028025	17
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

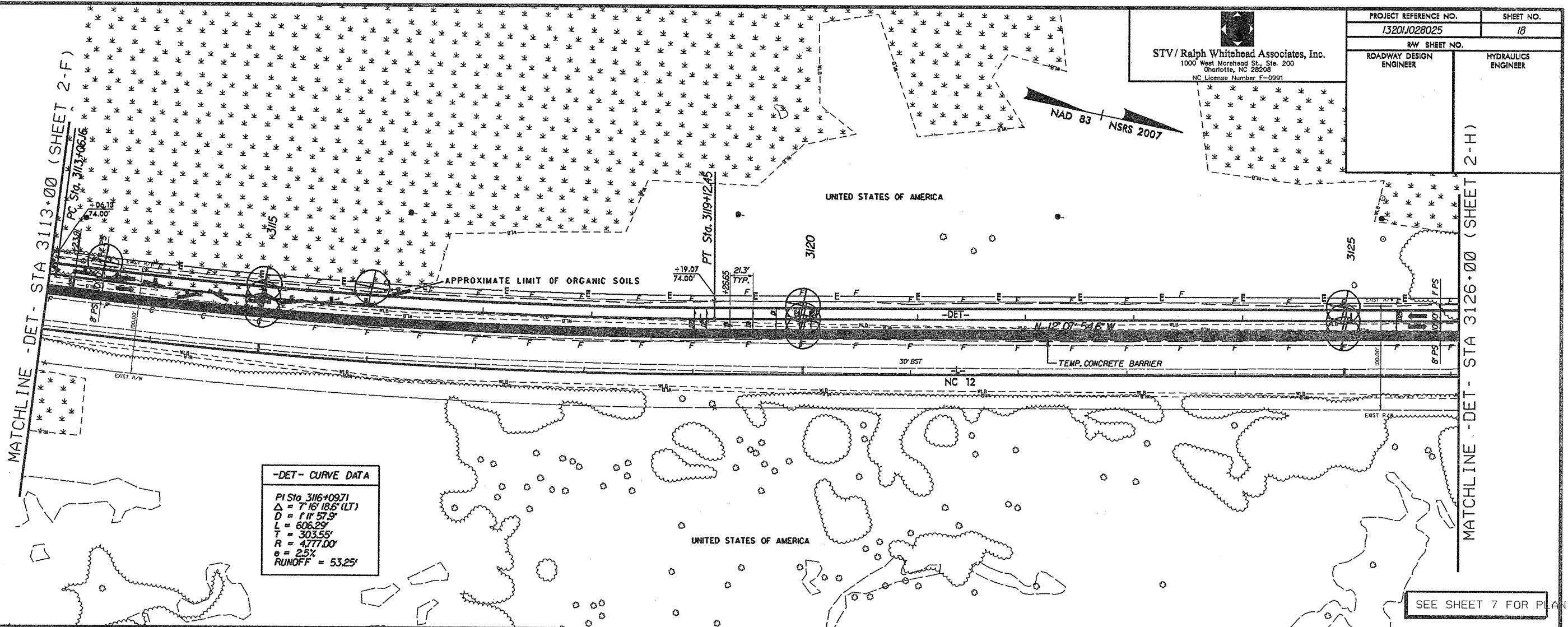
STV/Ralph Whitehead Associates, Inc.
 1000 West Morehead St., Ste. 200
 Charlotte, NC 28208
 NC License Number F-0991



SEE SHEET 6 FOR PLAN

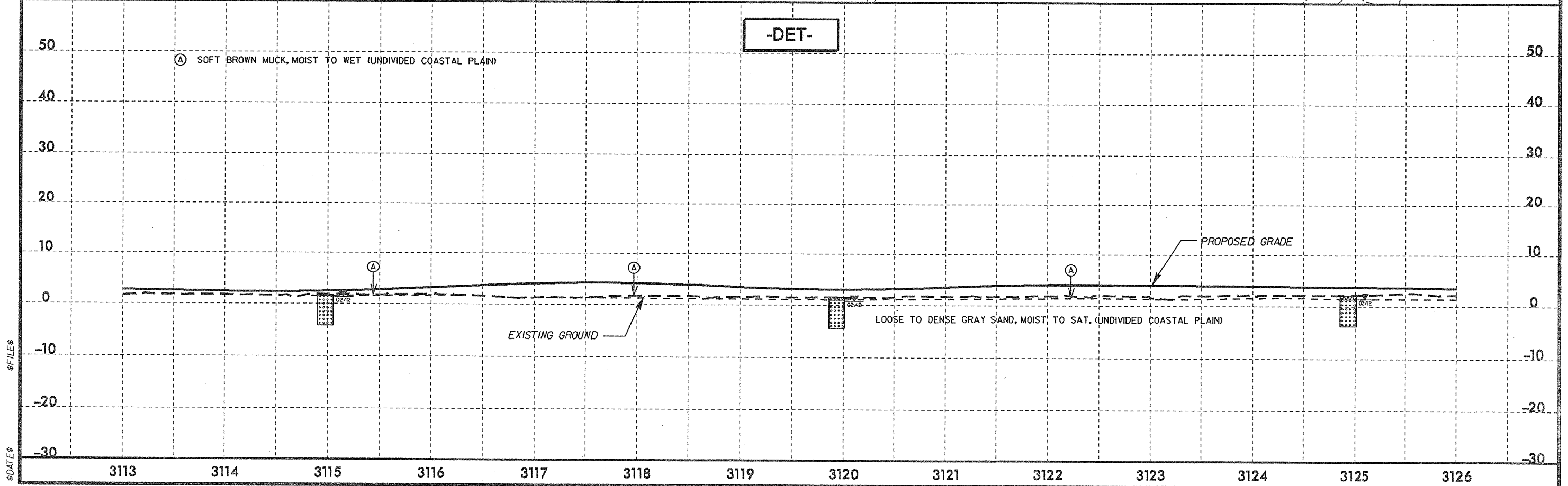


STV / Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991



-DET- CURVE DATA	
PI Sta	3116+09.71
Δ	7' 16" 18.6" (LT)
D	1' 11" 57.9"
L	606.29'
T	303.55'
R	4777.00'
e	2.5%
RUNOFF	53.25'

SEE SHEET 7 FOR PLAN



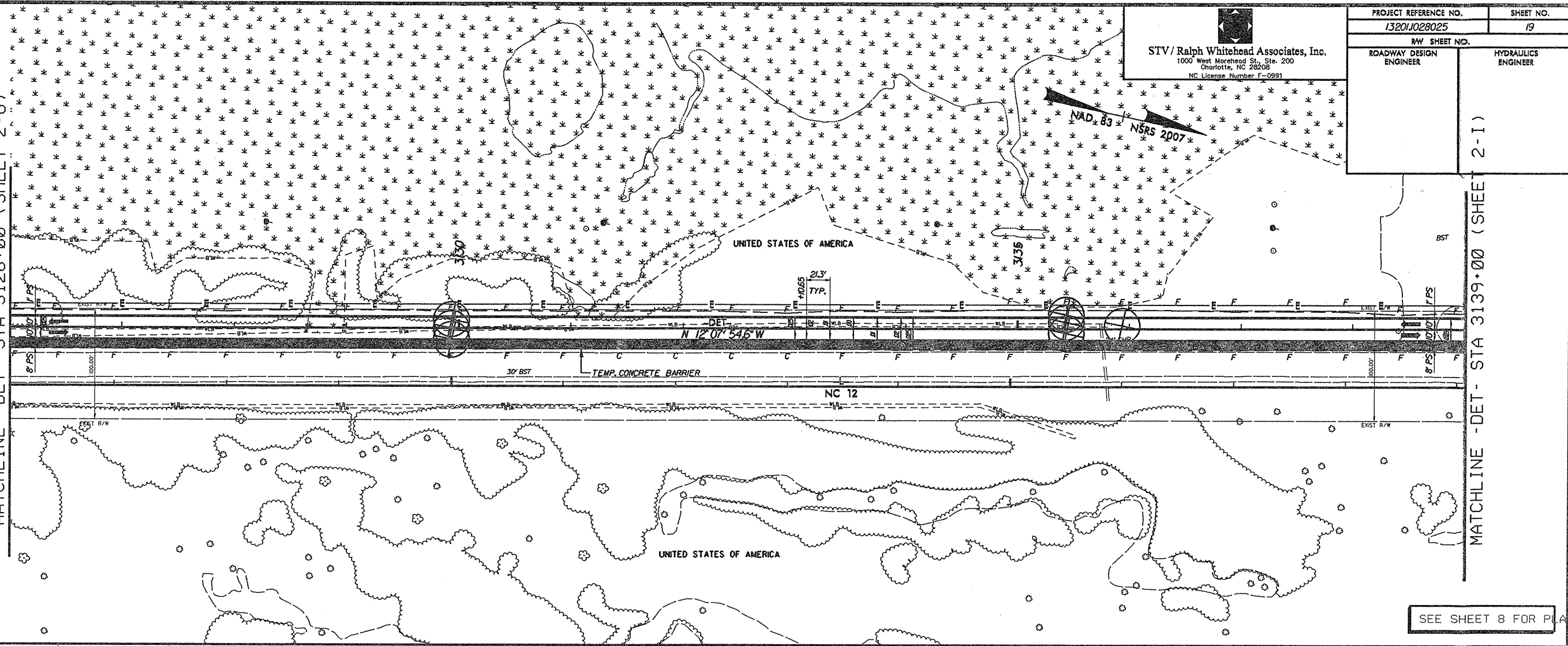
\$FILES\$
\$DATES\$

PROJECT REFERENCE NO.	SHEET NO.
13201028025	19
R/W SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	

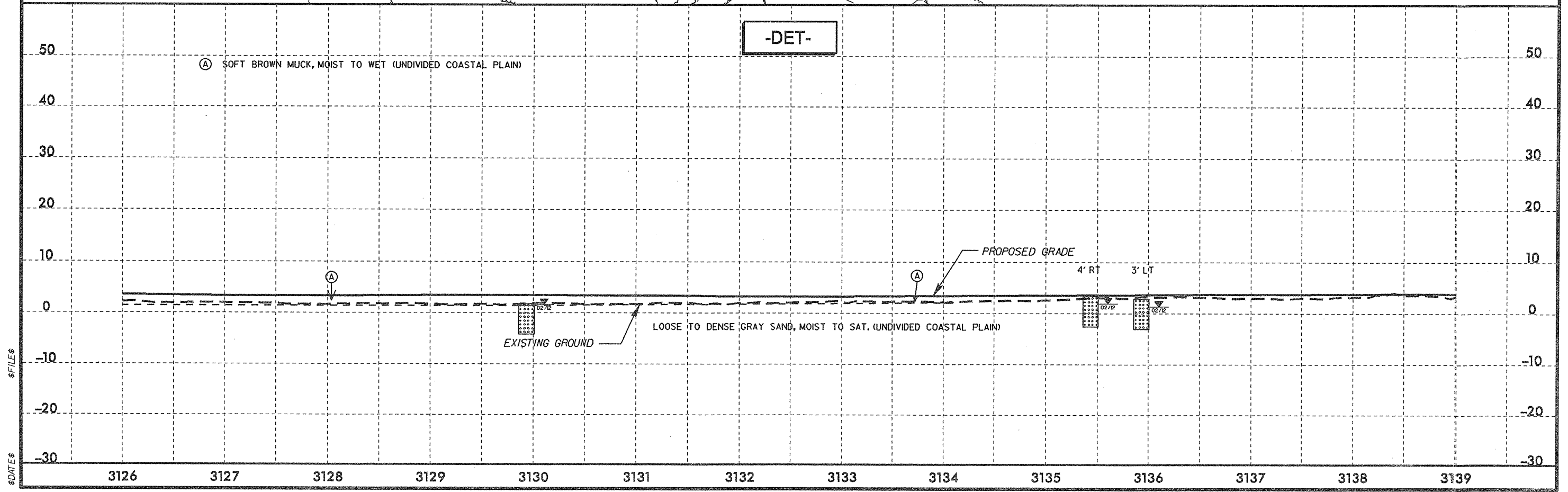
STV/Ralph Whitehead Associates, Inc.
 1000 West Morehead St., Ste. 200
 Charlotte, NC 28208
 NC License Number F-0991

MATCHLINE -DET- STA 3126+00 (SHEET 2-G)

MATCHLINE -DET- STA 3139+00 (SHEET 2-I)



SEE SHEET 8 FOR PLAN



\$FILES
\$DATES

PROJECT REFERENCE NO. 13201028025	SHEET NO. 20
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

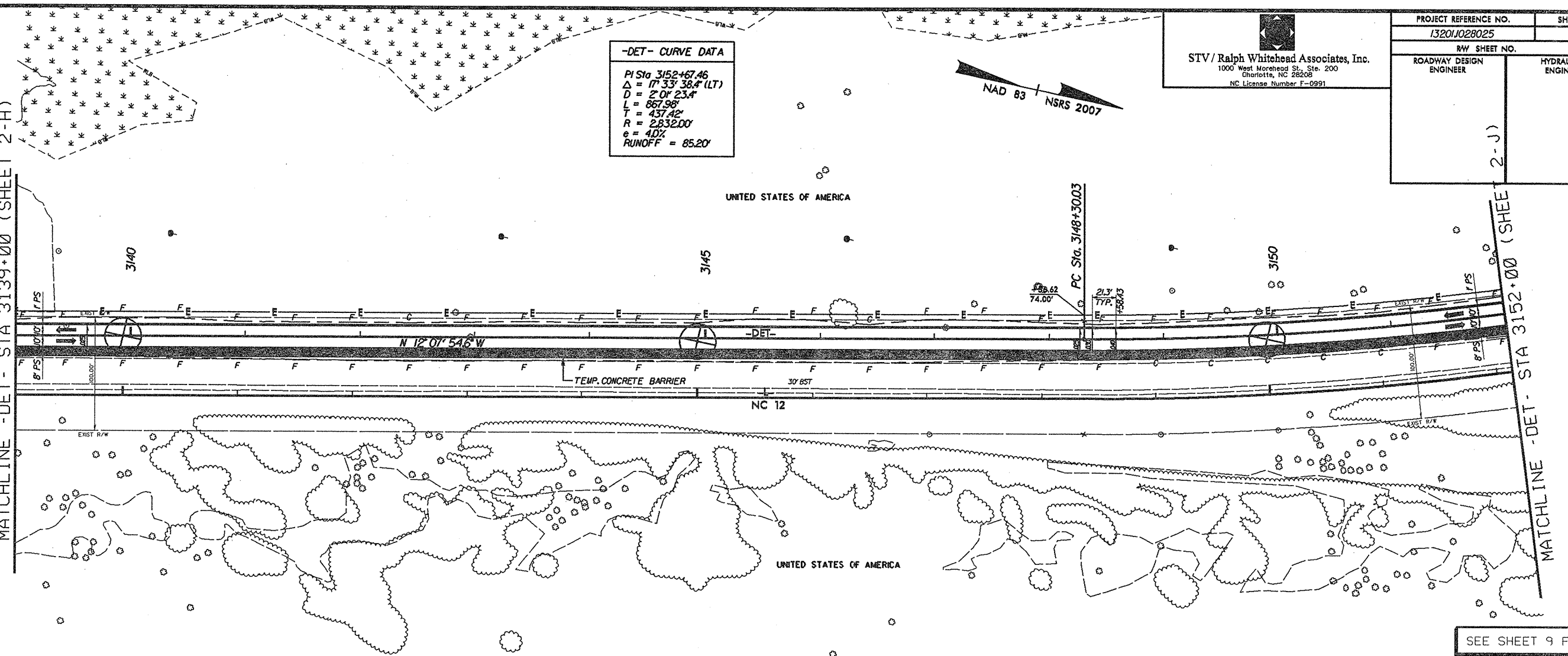
STV / Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991

-DET- CURVE DATA
 PI Sta 3152+67.46
 $\Delta = 17^{\circ} 33' 38.4" (LT)$
 $D = 2^{\circ} 07' 23.4"$
 $L = 867.98'$
 $T = 437.42'$
 $R = 2832.00'$
 $e = 4.0\%$
 RUNOFF = 85.20'

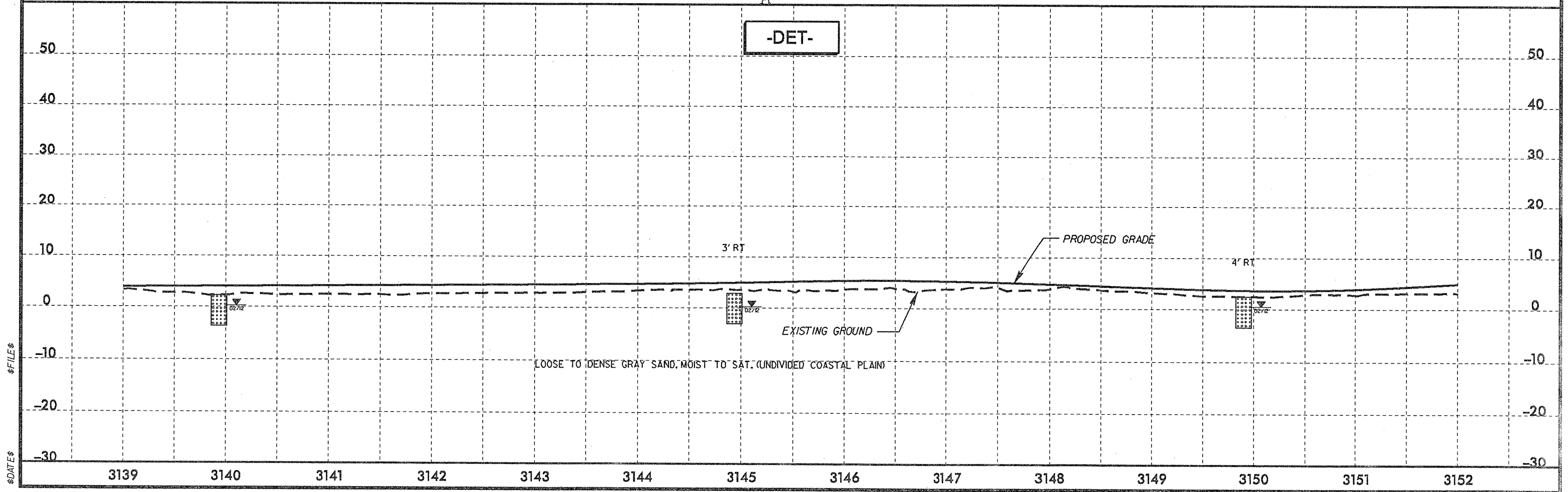


MATCHLINE -DET- STA 3139+00 (SHEET 2-H)

MATCHLINE -DET- STA 3152+00 (SHEET 2-J)



SEE SHEET 9 FOR PLAN



-DET- CURVE DATA

PI Sta 3152+67.46 Δ = 17° 33' 38.4" (LT) D = 2° 0' 23.4" L = 867.96' T = 437.42' R = 2832.00' e = 4.0% RUNOFF = 85.20'	PI Sta 3159+66.56 Δ = 29° 49' 07.6" (RT) D = 22° 55' 05.9" L = 130.11' T = 66.56' R = 250.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3160+95.24 Δ = 29° 12' 15.4" (LT) D = 22° 55' 05.9" L = 127.43' T = 65.13' R = 250.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3164+05.24 Δ = 8° 44' 28.2" (RT) D = 1° 46' 04.2" L = 494.45' T = 247.71' R = 3241.00' e = 2.5% RUNOFF = 39.33'
---	---	---	---

-XI- CURVE DATA

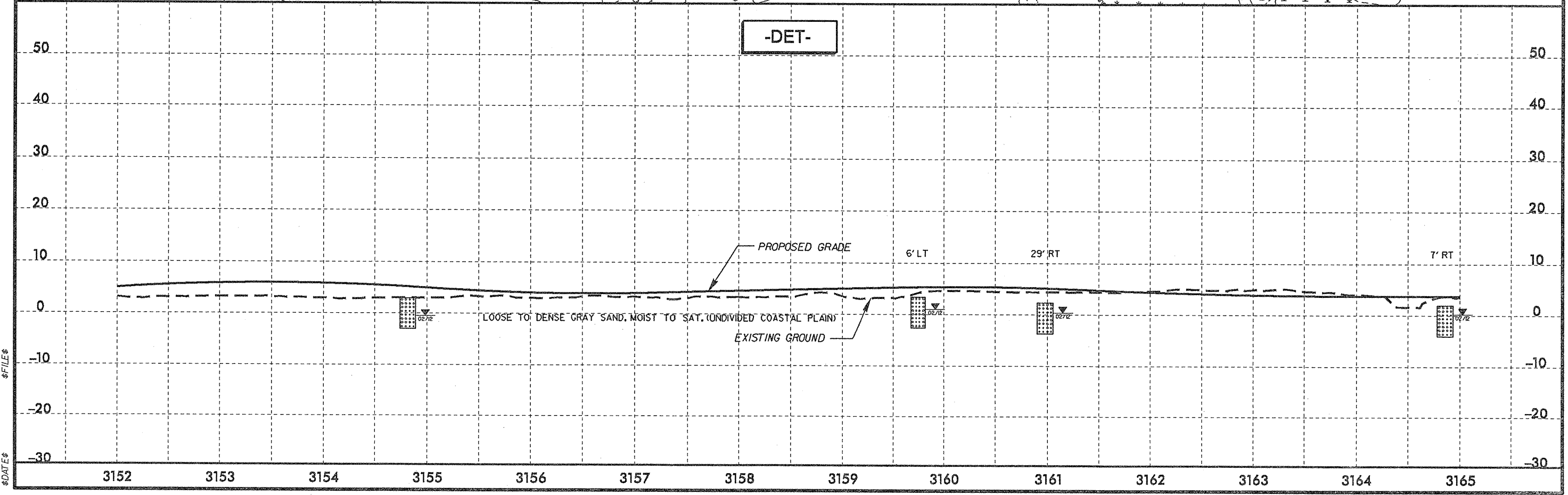
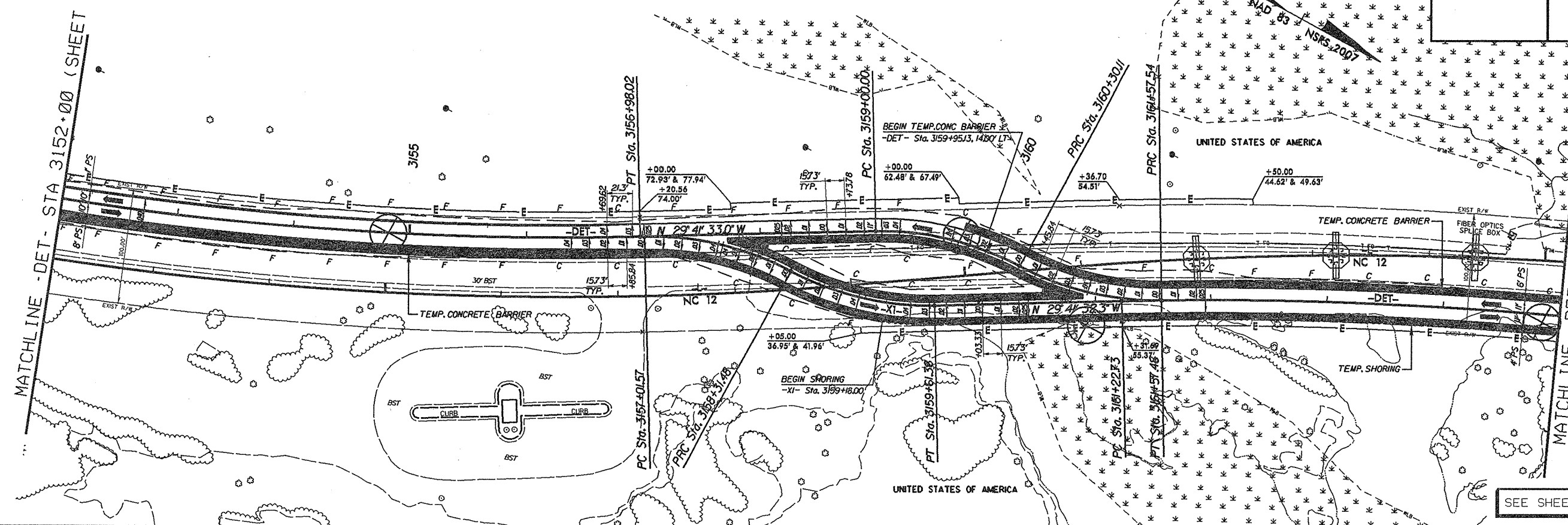
PI Sta 3157+68.03 Δ = 29° 46' 20.5" (RT) D = 22° 55' 05.9" L = 129.91' T = 66.46' R = 250.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3158+97.93 Δ = 29° 46' 19.7" (LT) D = 22° 55' 05.9" L = 129.91' T = 66.45' R = 250.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3161+40.11 Δ = 0° 36' 51.5" (RT) D = 1° 46' 04.2" L = 347.5' T = 17.37' R = 3241.00' e = 2.5% RUNOFF = 39.33'
---	---	---

PROJECT REFERENCE NO. 13201028025	SHEET NO. 21
R/W SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	

STV/ Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991

MATCHLINE -DET- STA 3152+00 (SHEET 2-I)

MATCHLINE -DET- STA 3165+00 (SHEET 2-K)



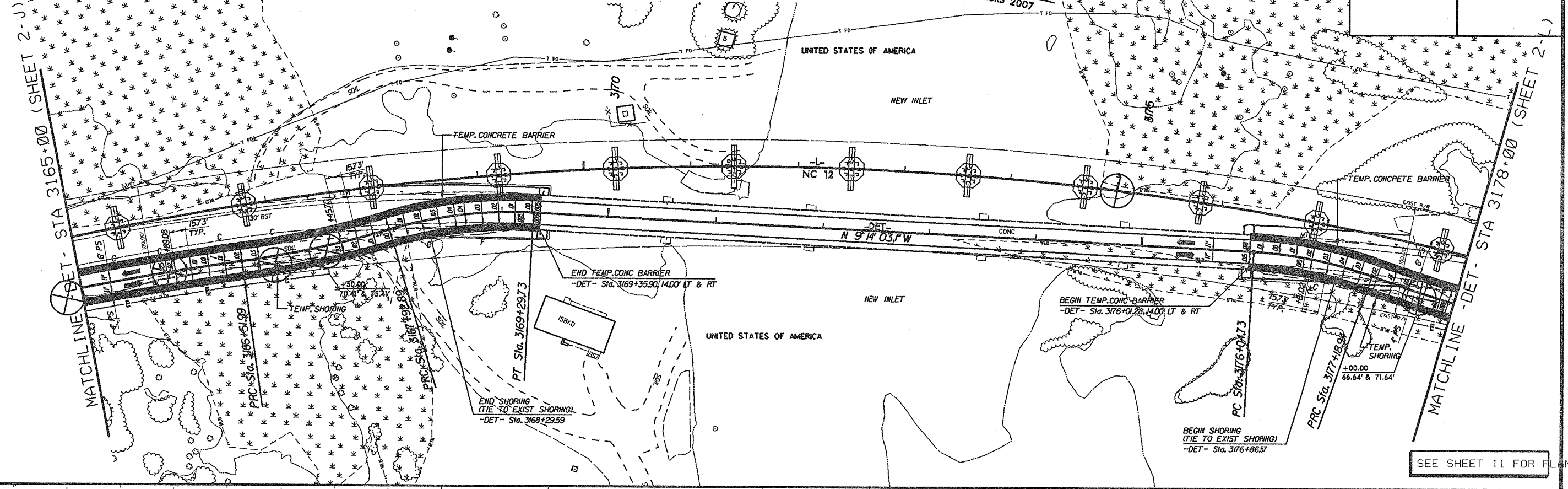
\$FILES\$
\$DATES\$

-DET- CURVE DATA

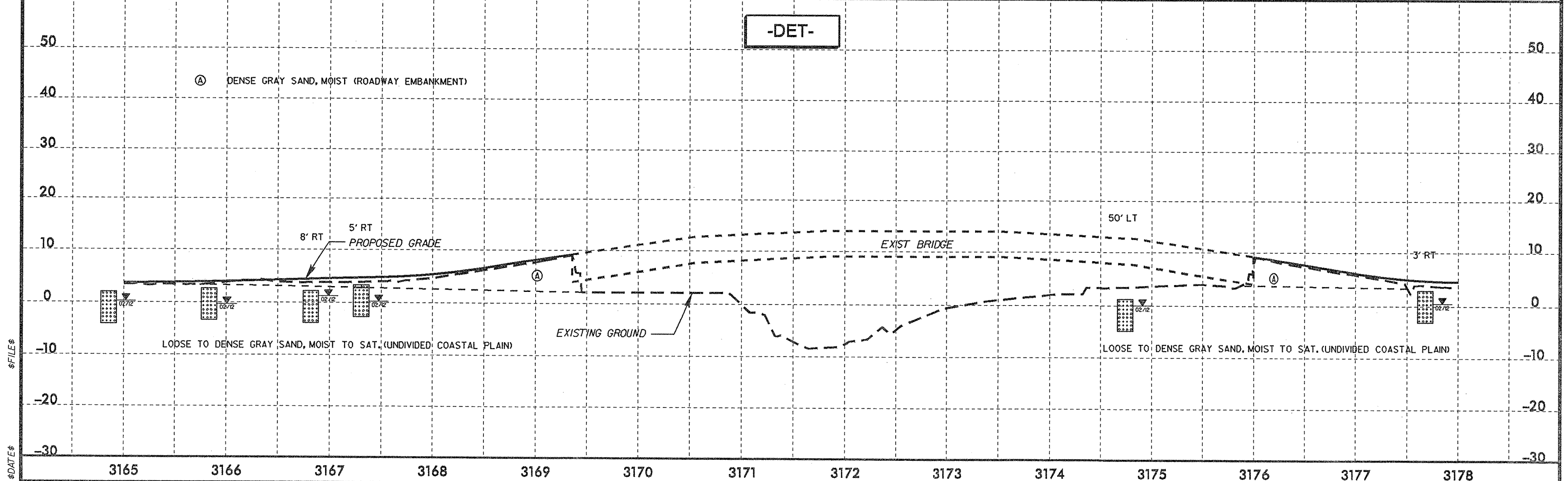
PI Sta 3164+05.24 Δ = 8° 44' 28.2" (RT) D = 1' 46' 04.2" L = 494.45' T = 247.71' R = 3,241.00' e = 2.5% RUNOFF = 39.33'	PI Sta 3167+22.57 Δ = 8° 29' 52.9" (LT) D = 6' 01' 52.7" L = 140.90' T = 70.58' R = 950.00' e = 3.0% RUNOFF = 47.19'	PI Sta 3168+61.99 Δ = 19° 36' 02.3" (RT) D = 14' 19' 26.2" L = 136.84' T = 69.09' R = 400.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3176+62.23 Δ = 15° 21' 34.5" (RT) D = 14' 19' 26.2" L = 114.21' T = 57.50' R = 400.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3178+10.07 Δ = 6° 57' 10.3" (LT) D = 3' 49' 11.0" L = 182.03' T = 91.2' R = 1,500.00' e = 2.5% RUNOFF = 39.33'
--	---	---	---	--

PROJECT REFERENCE NO. 1320J028025	SHEET NO. 22
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

STV / Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991



SEE SHEET 11 FOR PLAN



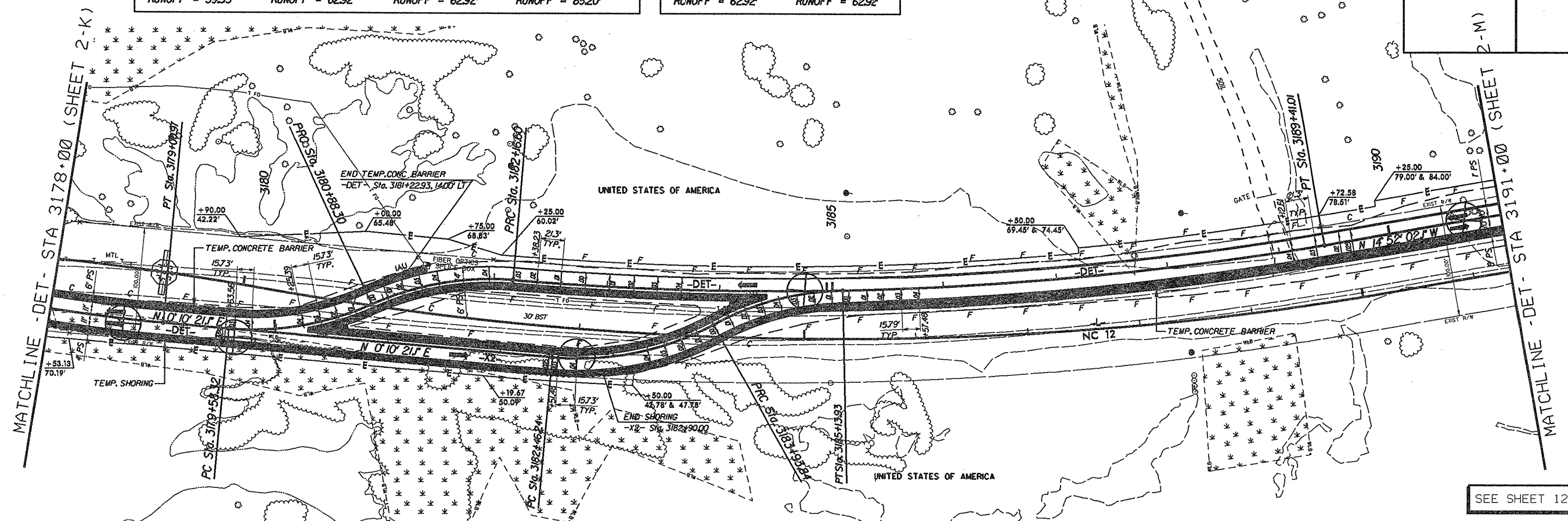
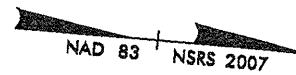
\$FILE \$
\$DATE \$

-DET- CURVE DATA			
PI Sta 3178+10.07 Δ = 6° 51' 10.3" (LT) D = 3° 49' 11.0" L = 182.03' T = 91.2' R = 1500.00' e = 2.5% RUNOFF = 39.33'	PI Sta 3180+24.82 Δ = 2° 47' 20.0" (LT) D = 22° 55' 05.9" L = 128.30' T = 66.49' R = 250.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3181+53.90 Δ = 2° 24' 17.5" (RT) D = 22° 55' 05.9" L = 128.30' T = 66.49' R = 250.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3185+80.79 Δ = 14° 39' 21.0" (LT) D = 2° 01' 23.4" L = 724.40' T = 364.19' R = 2832.00' e = 4.0% RUNOFF = 85.20'

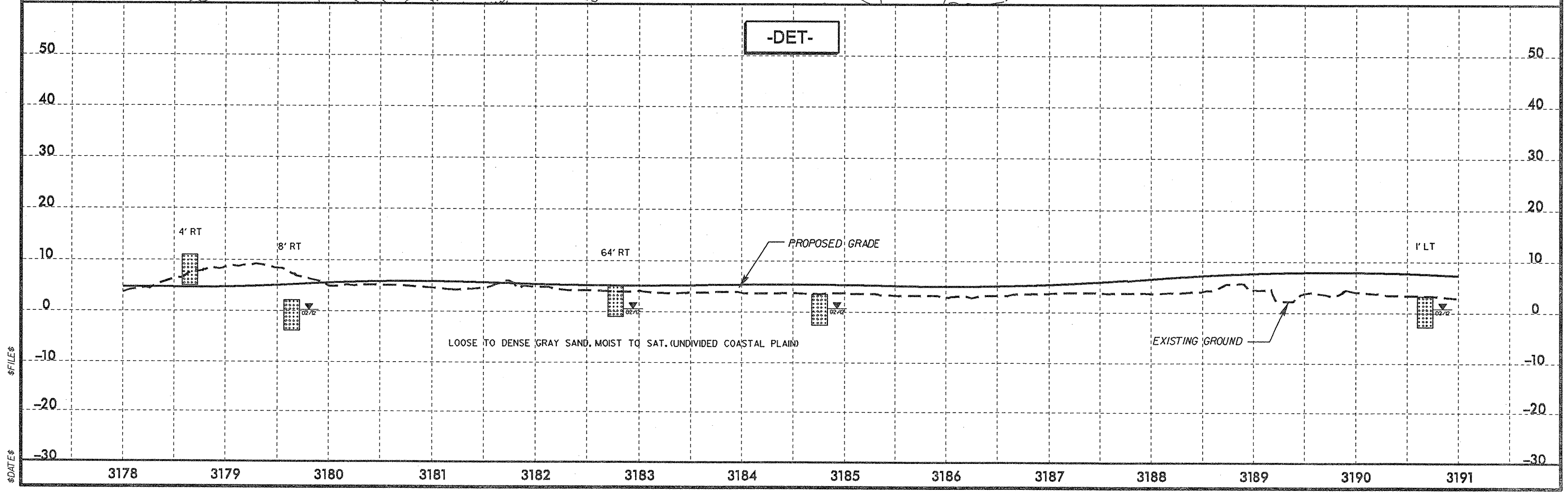
-X2- CURVE DATA	
PI Sta 3183+22.26 Δ = 3° 49' 43.7" (LT) D = 22° 55' 05.9" L = 147.61' T = 76.02' R = 250.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3184+55.07 Δ = 27° 31' 22.1" (RT) D = 22° 55' 05.9" L = 120.09' T = 61.23' R = 250.00' e = 4.0% RUNOFF = 62.92'

STV / Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991

PROJECT REFERENCE NO. 13201028025	SHEET NO. 23
R/W SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	



SEE SHEET 12 FOR PLAN



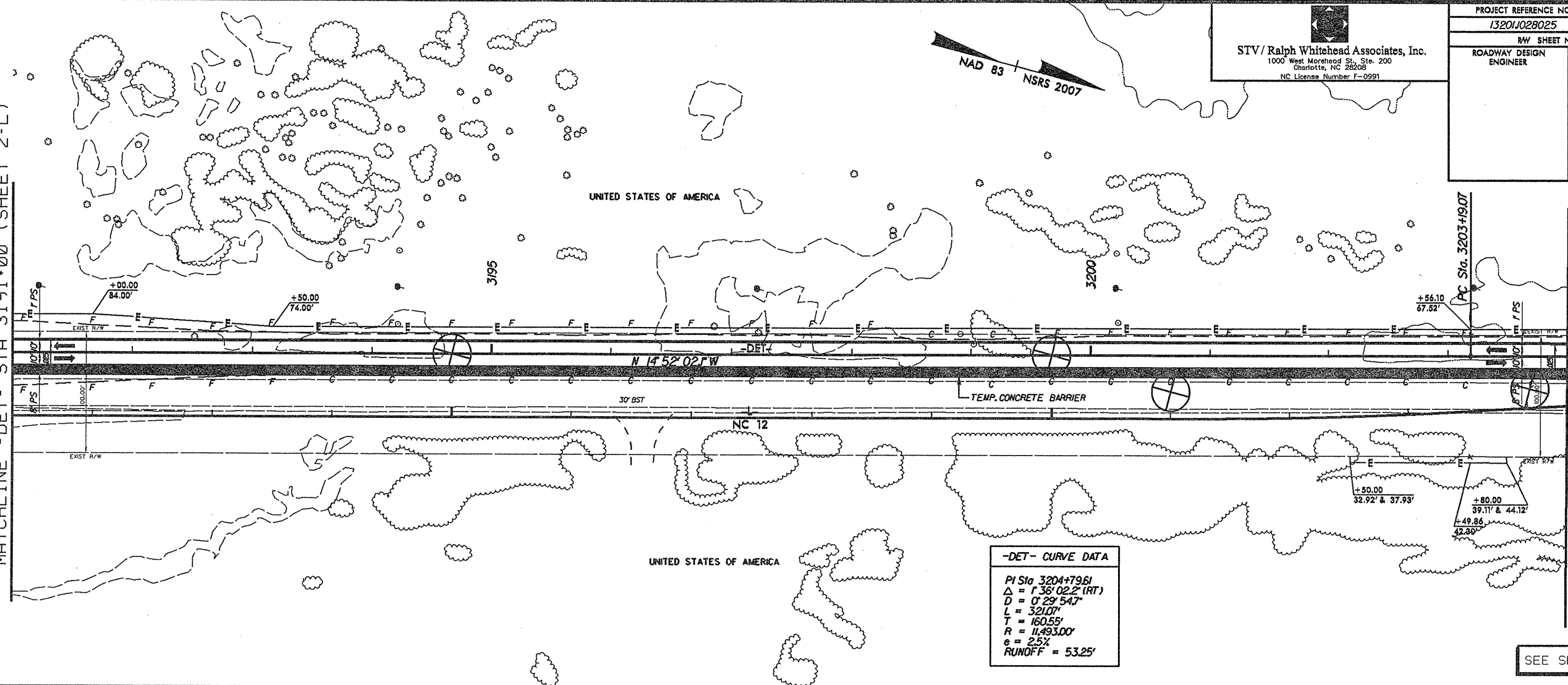
\$FILES \$DATE\$

STV/Ralph Whitehead Associates, Inc.
 1000 West Morehead St., Ste. 200
 Charlotte, NC 28208
 NC License Number F-0991

PROJECT REFERENCE NO. 13201028025	SHEET NO. 24
HW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

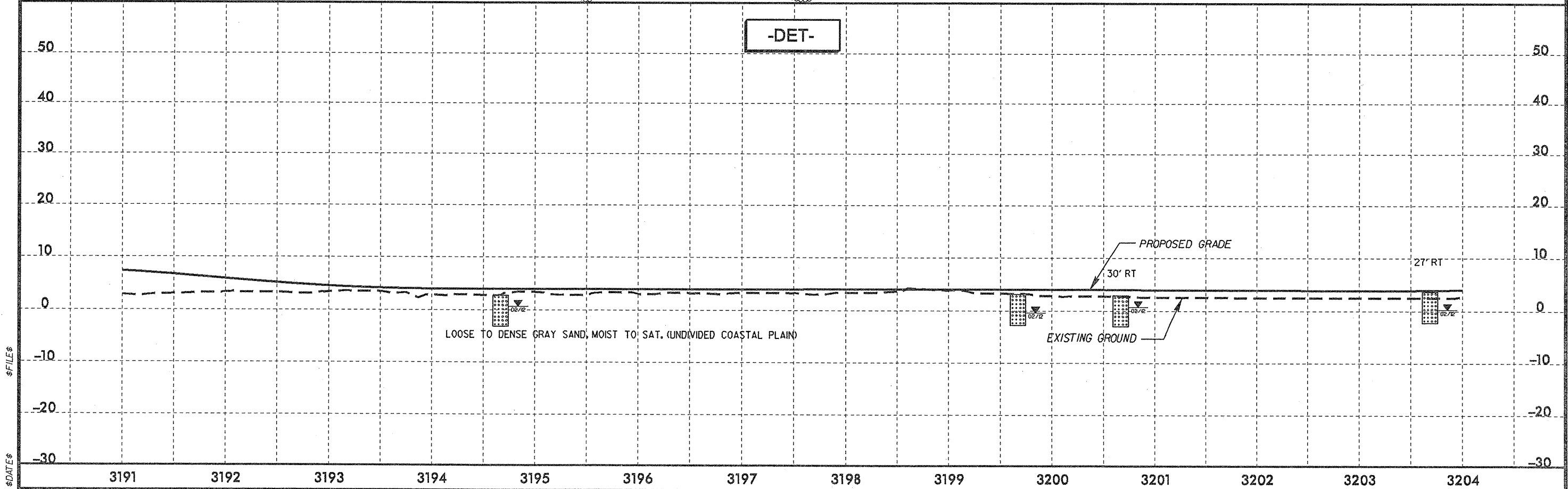
MATCHLINE -DET- STA 3191+00 (SHEET 2-L)

MATCHLINE -DET- STA 3204+00 (SHEET 2-N)



-DET- CURVE DATA
 PI Sta 3204+79.61
 $\Delta = 1.36^{\circ}02'2''$ (RT)
 $D = 0^{\circ}29'54''$
 $L = 321.07'$
 $T = 160.55'$
 $R = 11,493.00'$
 $e = 2.5\%$
 RUNOFF = 53.25'

SEE SHEET 13 FOR PLAN



LOOSE TO DENSE GRAY SAND, MOIST TO SAT. (UNDIVIDED COASTAL PLAIN)

PROPOSED GRADE
 30' RT
 EXISTING GROUND
 27' RT

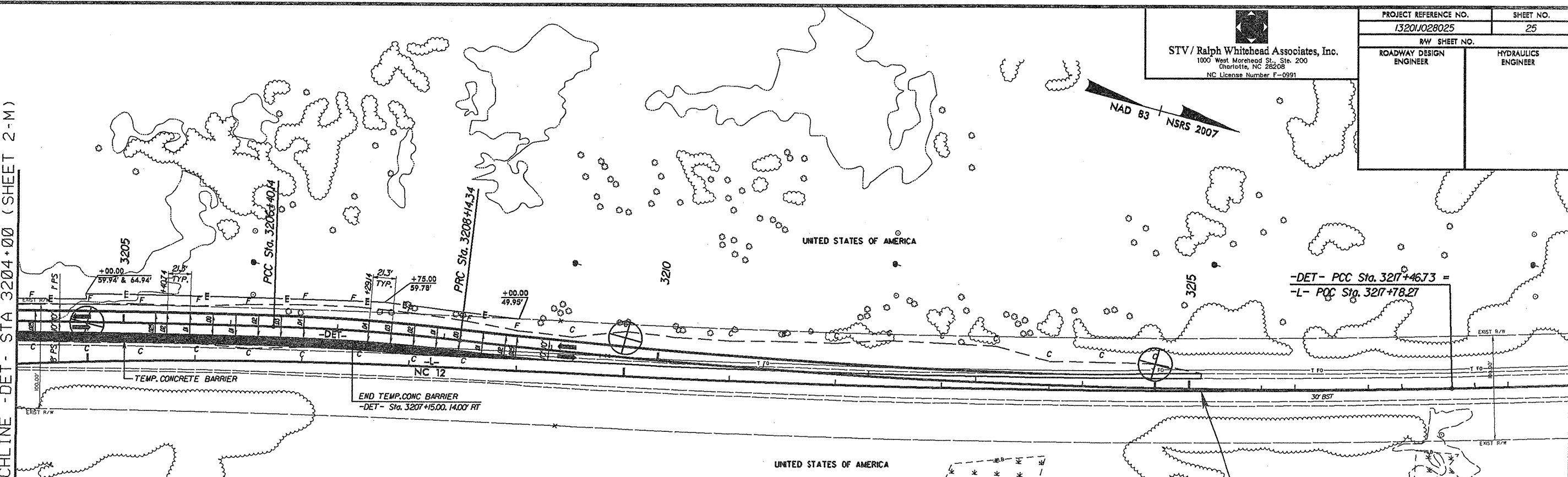
-DET-

STV / Ralph Whitehead Associates, Inc.
 1000 West Morehead St., Ste. 200
 Charlotte, NC 28208
 NC License Number F-0991

PROJECT REFERENCE NO. 13201028025	SHEET NO. 25
RAW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



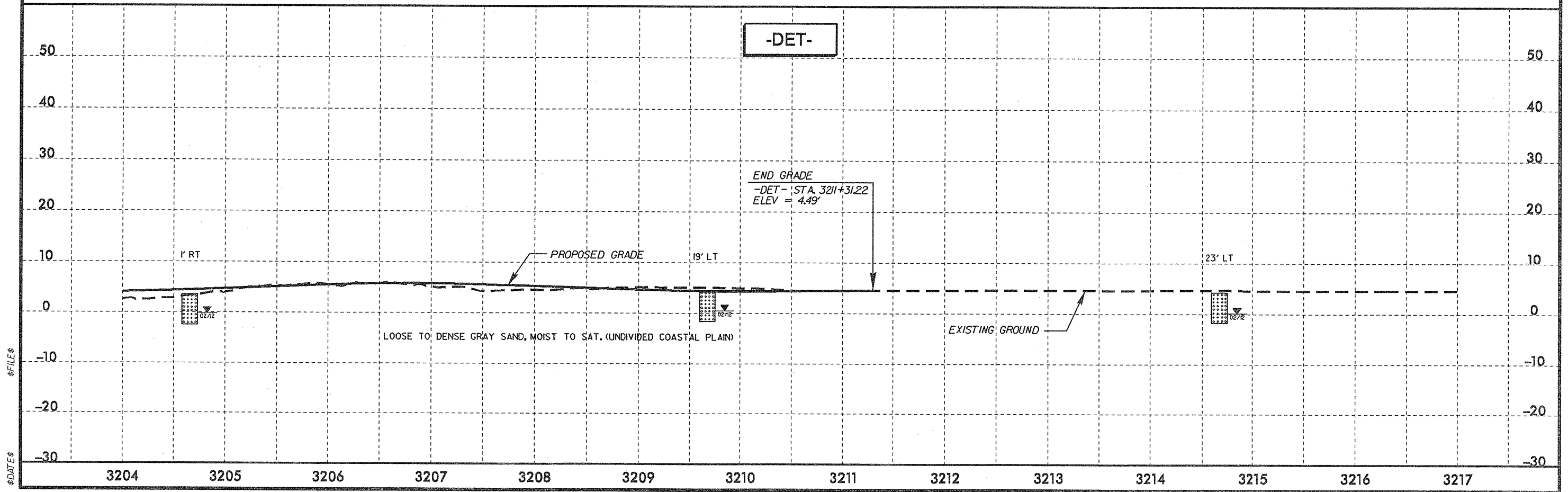
MATCHLINE - DET - STA 3204+00 (SHEET 2-M)



-DET- CURVE DATA

PI Sta 3204+79.61 Δ = 1° 35' 02.2" (RT) D = 0° 29' 54.7" L = 321.07' T = 160.55' R = 11,493.00' e = 2.5% RUNOFF = 53.25'	PI Sta 3207+27.30 Δ = 4° 59' 26.2" (RT) D = 2° 51' 53.2" L = 174.21' T = 87.16' R = 2,000.00' e = 4.0% RUNOFF = 85.20'	PI Sta 3212+81.27 Δ = 7° 51' 22.1" (LT) D = 0° 30' 33.3" L = 932.39' T = 466.92' R = 6,800.00' e = 2.5% RUNOFF = 53.25'	PI Sta 3221+24.35 Δ = 3° 46' 28.5" (LT) D = 0° 29' 59.9" L = 754.97' T = 377.62' R = 11,460.00' e = 2.5% RUNOFF = 53.25'
---	---	--	---

SEE SHEET 14 FOR PLAN



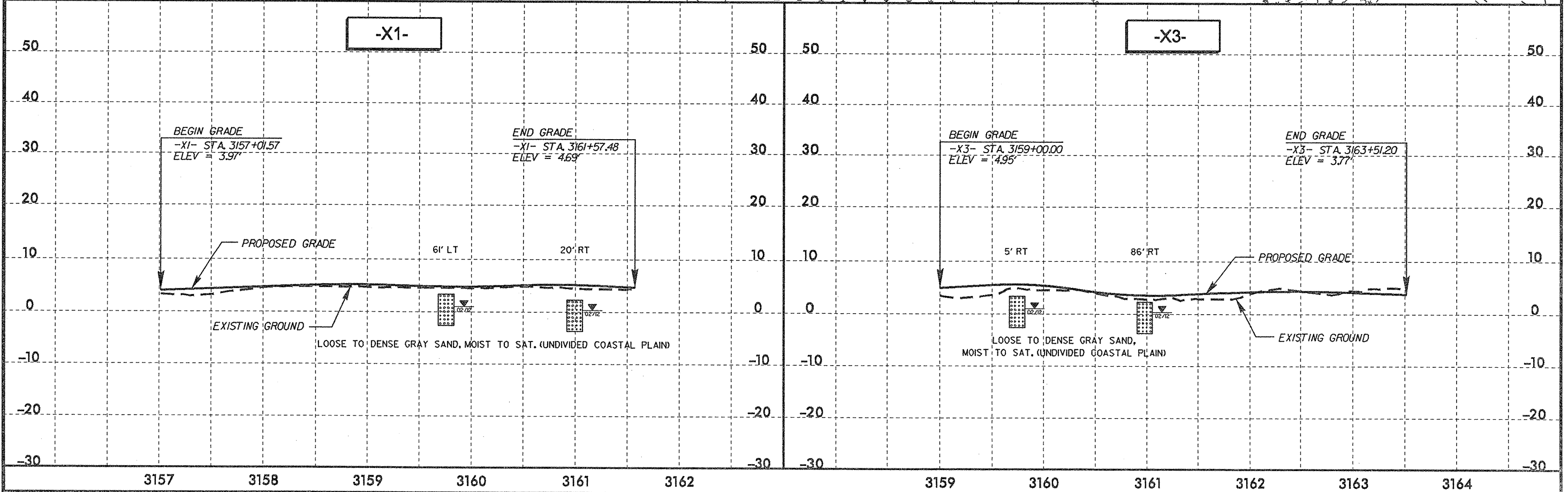
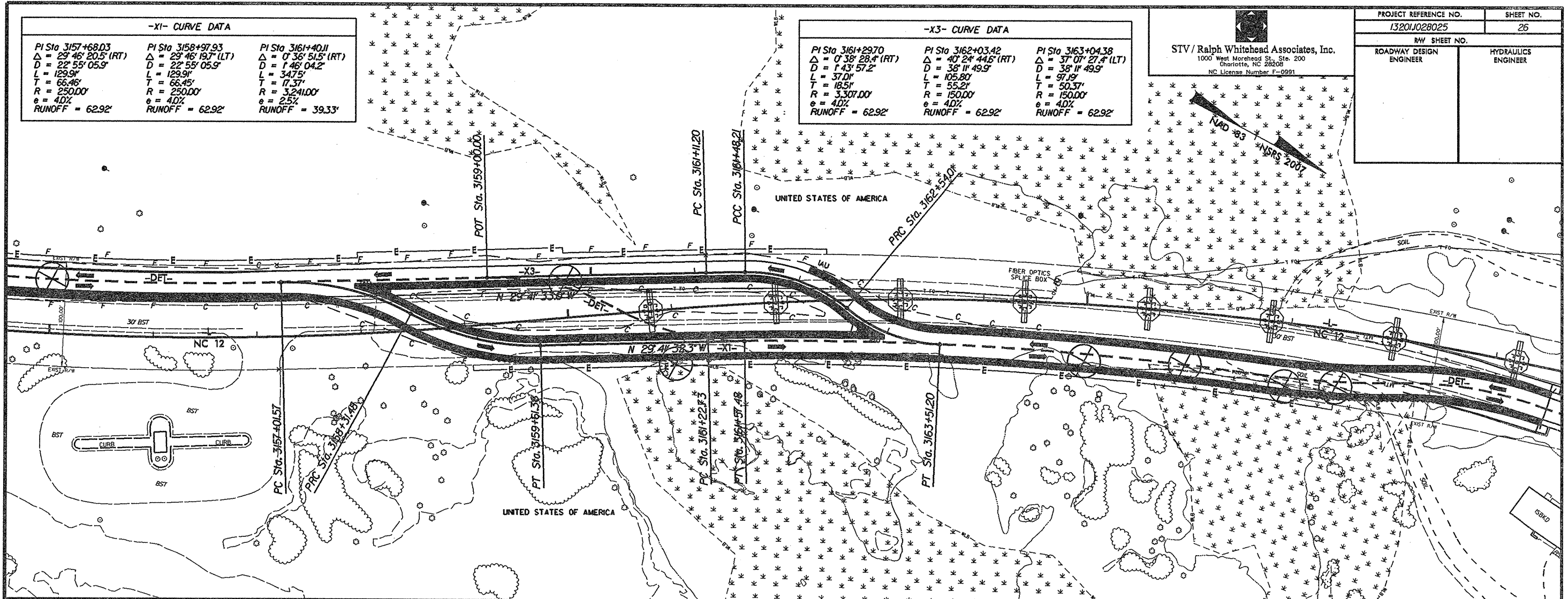
SIV / Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991

-X1- CURVE DATA

PI Sta 3157+68.03 Δ = 29° 46' 20.5" (RT) D = 22° 55' 05.9" L = 129.9' T = 66.46' R = 250.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3158+97.93 Δ = 29° 46' 19.7" (LT) D = 22° 55' 05.9" L = 129.9' T = 66.45' R = 250.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3161+40.11 Δ = 0° 36' 51.5" (RT) D = 1° 46' 04.2" L = 34.75' T = 17.37' R = 3,241.00' e = 2.5% RUNOFF = 39.33'
--	--	--

-X3- CURVE DATA

PI Sta 3161+29.70 Δ = 0° 38' 28.4" (RT) D = 1° 43' 57.2" L = 37.0' T = 18.51' R = 3,307.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3162+03.42 Δ = 40° 24' 44.6" (RT) D = 38° 11' 49.9" L = 105.80' T = 55.21' R = 150.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3163+04.38 Δ = 37° 07' 27.4" (LT) D = 38° 11' 49.9" L = 97.19' T = 50.37' R = 150.00' e = 4.0% RUNOFF = 62.92'
---	---	--



\$DATE\$

PROJECT REFERENCE NO. 1320J028025	SHEET NO. 27
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

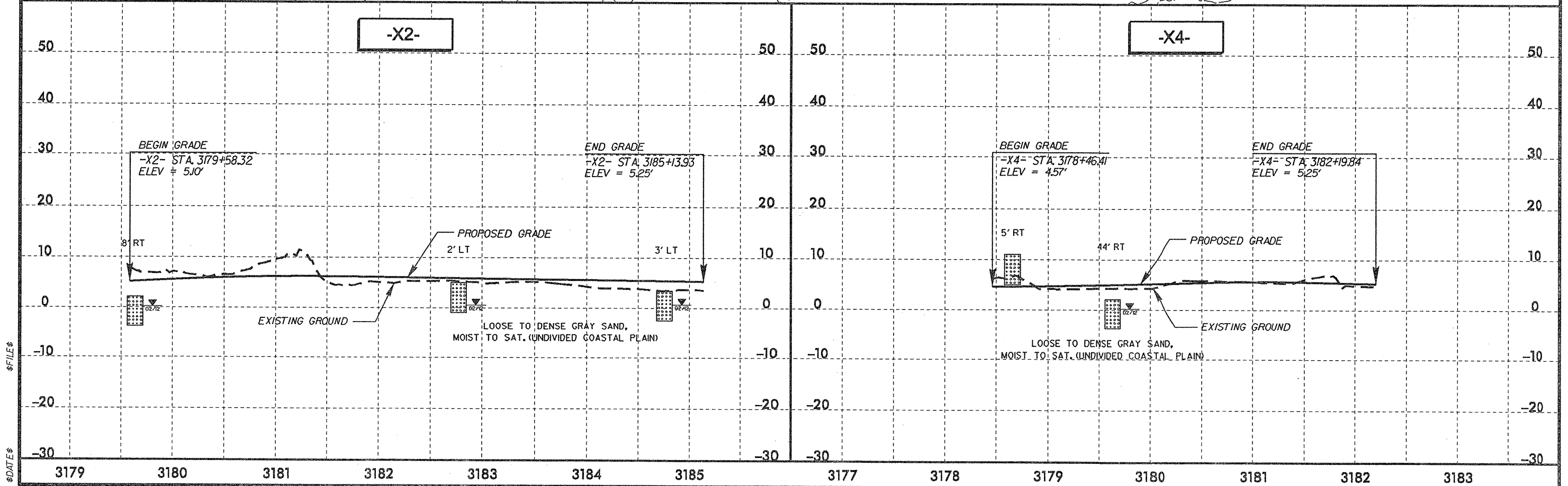
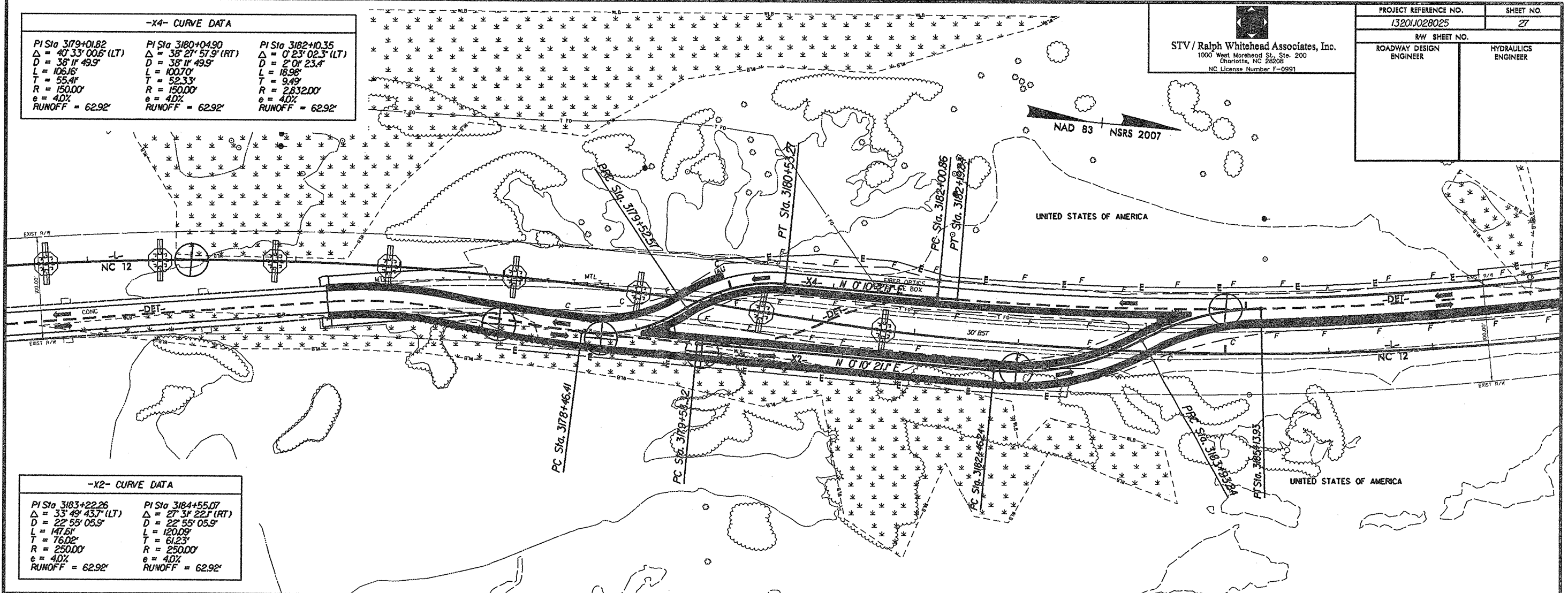
STV / Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991

-X4- CURVE DATA

PI Sta 3179+01.82 Δ = 40° 33' 00.6" (LT) D = 38° 11' 49.5" L = 106.16' T = 55.41' R = 150.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3180+04.90 Δ = 38° 27' 57.9" (RT) D = 38° 11' 49.5" L = 100.70' T = 52.33' R = 150.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3182+00.35 Δ = 0° 23' 02.3" (LT) D = 2° 01' 23.4" L = 18.98' T = 9.49' R = 2,832.00' e = 4.0% RUNOFF = 62.92'
---	---	---

-X2- CURVE DATA

PI Sta 3183+22.26 Δ = 33° 49' 43.7" (LT) D = 22° 55' 05.9" L = 147.61' T = 76.02' R = 250.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3184+55.07 Δ = 27° 31' 22.1" (RT) D = 22° 55' 05.9" L = 120.09' T = 61.23' R = 250.00' e = 4.0% RUNOFF = 62.92'
---	---



\$DATE\$ \$FILE\$

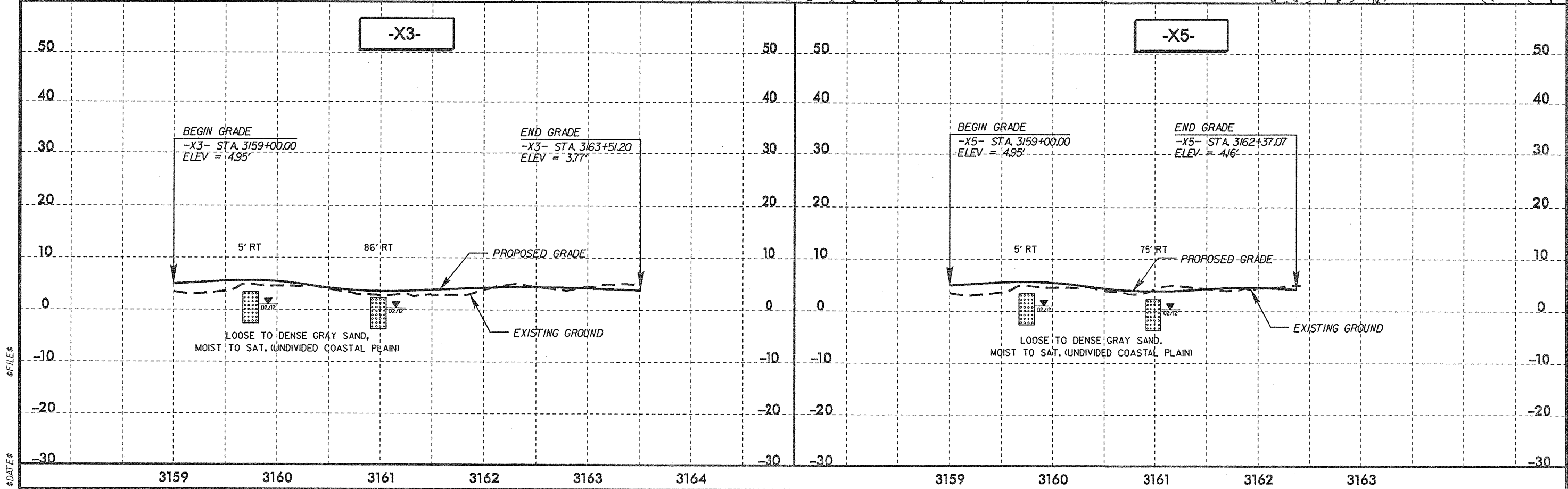
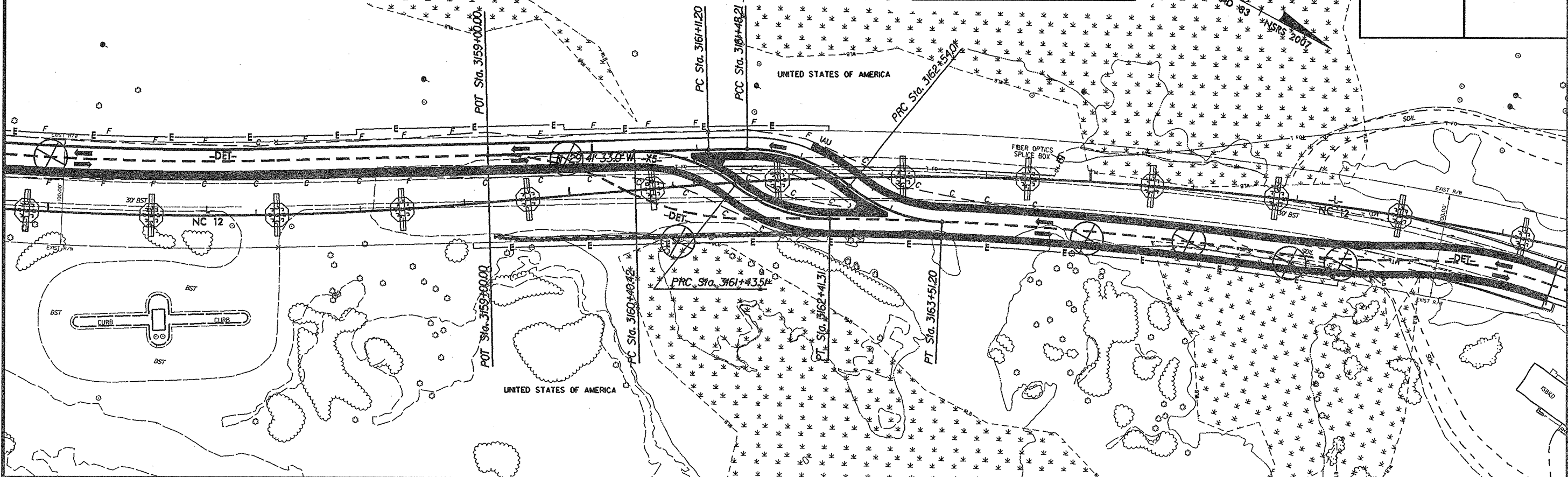
STV / Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991

-X3- CURVE DATA

PI Sta 3161+29.70 Δ = 0° 38' 28.4" (RT) D = 1' 43' 57.2" L = 37.0' T = 18.5' R = 3,307.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3162+03.42 Δ = 40° 24' 44.6" (RT) D = 38' 11' 49.9" L = 105.80' T = 55.2' R = 150.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3163+04.38 Δ = 37° 07' 27.4" (LT) D = 38' 11' 49.9" L = 97.19' T = 50.37' R = 150.00' e = 4.0% RUNOFF = 62.92'
--	--	--

-X5- CURVE DATA

PI Sta 3160+94.10 Δ = 39° 22' 40.4" (RT) D = 38' 11' 49.9" L = 103.08' T = 53.68' R = 150.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3161+94.22 Δ = 37° 21' 26.2" (LT) D = 38' 11' 49.9" L = 97.80' T = 50.71' R = 150.00' e = 4.0% RUNOFF = 62.92'
---	--



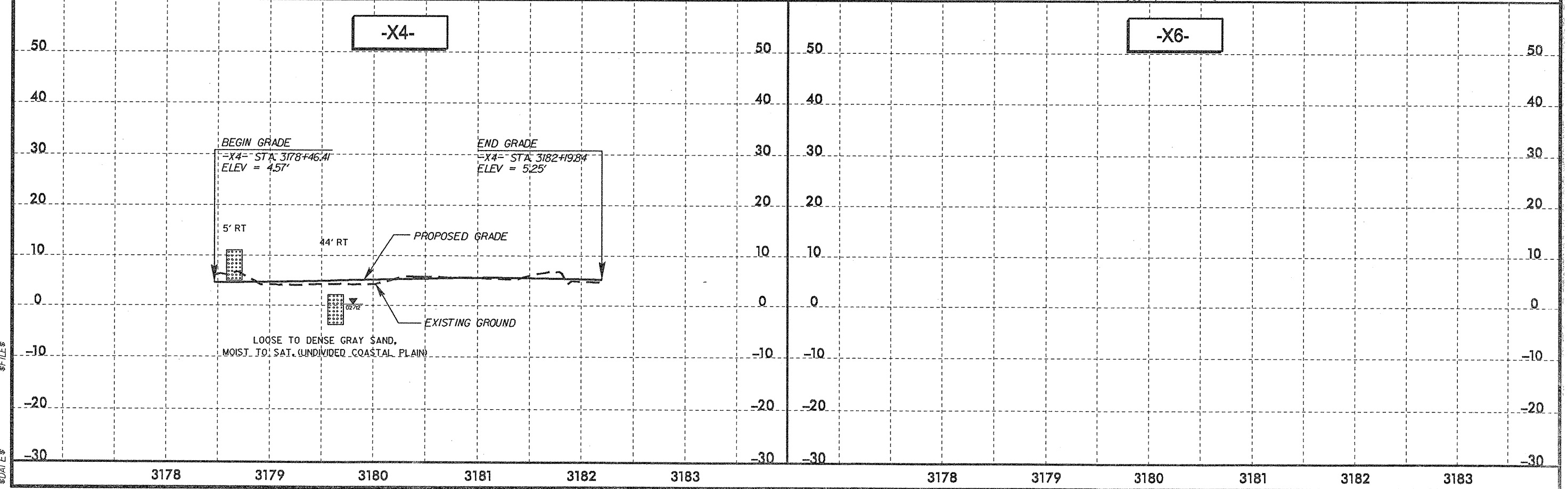
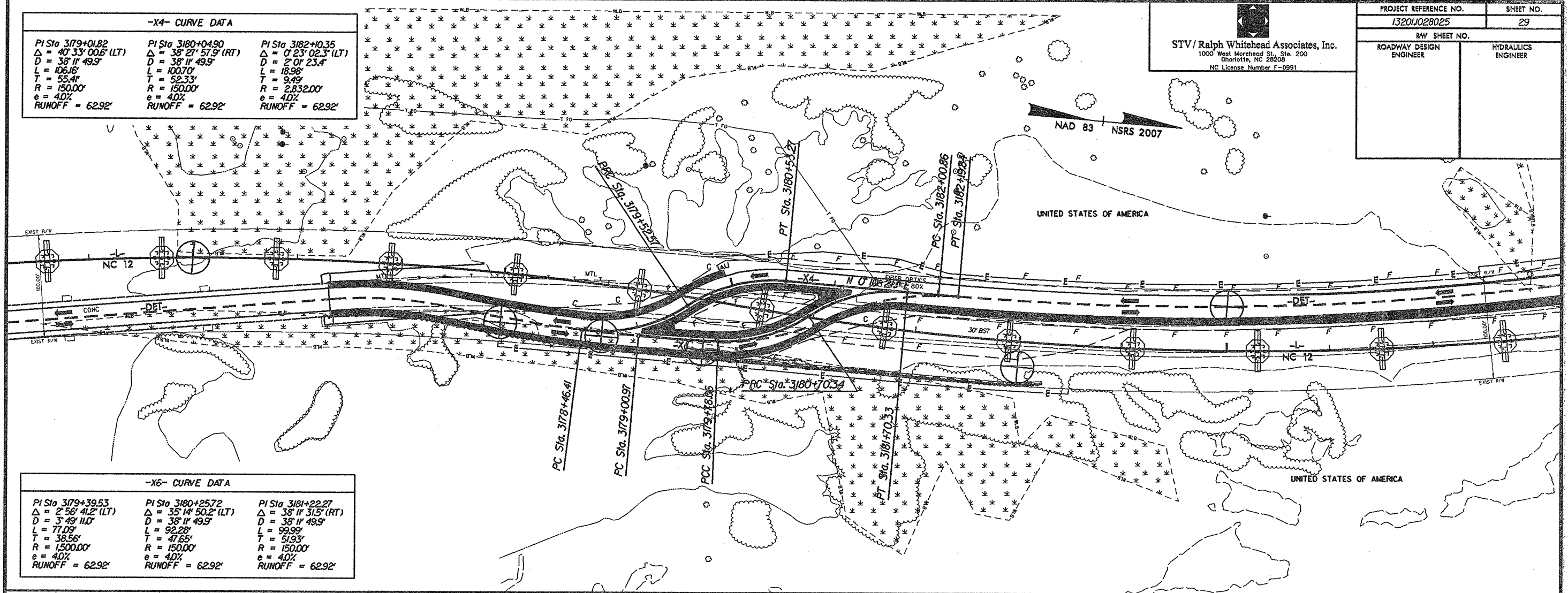
STV / Ralph Whitehead Associates, Inc.
1000 West Morehead St., Ste. 200
Charlotte, NC 28208
NC License Number F-0991

-X4- CURVE DATA

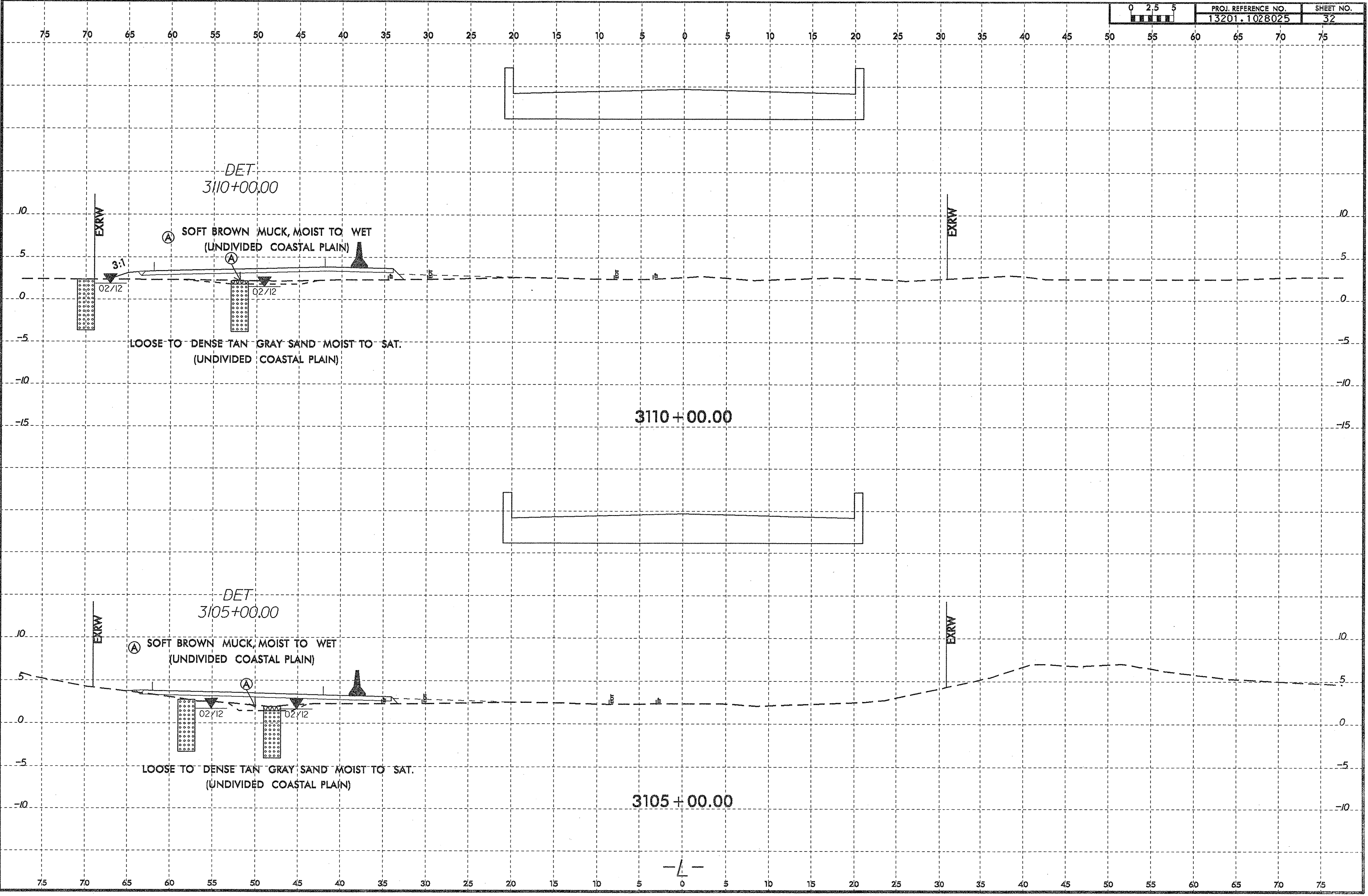
PI Sta 3179+01.82 Δ = 40° 33' 00.6" (LT) D = 38' 11" 49.9" L = 106.16' T = 55.41' R = 150.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3180+04.90 Δ = 38° 27' 57.9" (RT) D = 38' 11" 49.9" L = 100.70' T = 52.33' R = 150.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3182+10.35 Δ = 0° 23' 02.3" (LT) D = 2' 01" 23.4" L = 18.98' T = 9.49' R = 2,832.00' e = 4.0% RUNOFF = 62.92'
---	---	---

-X6- CURVE DATA

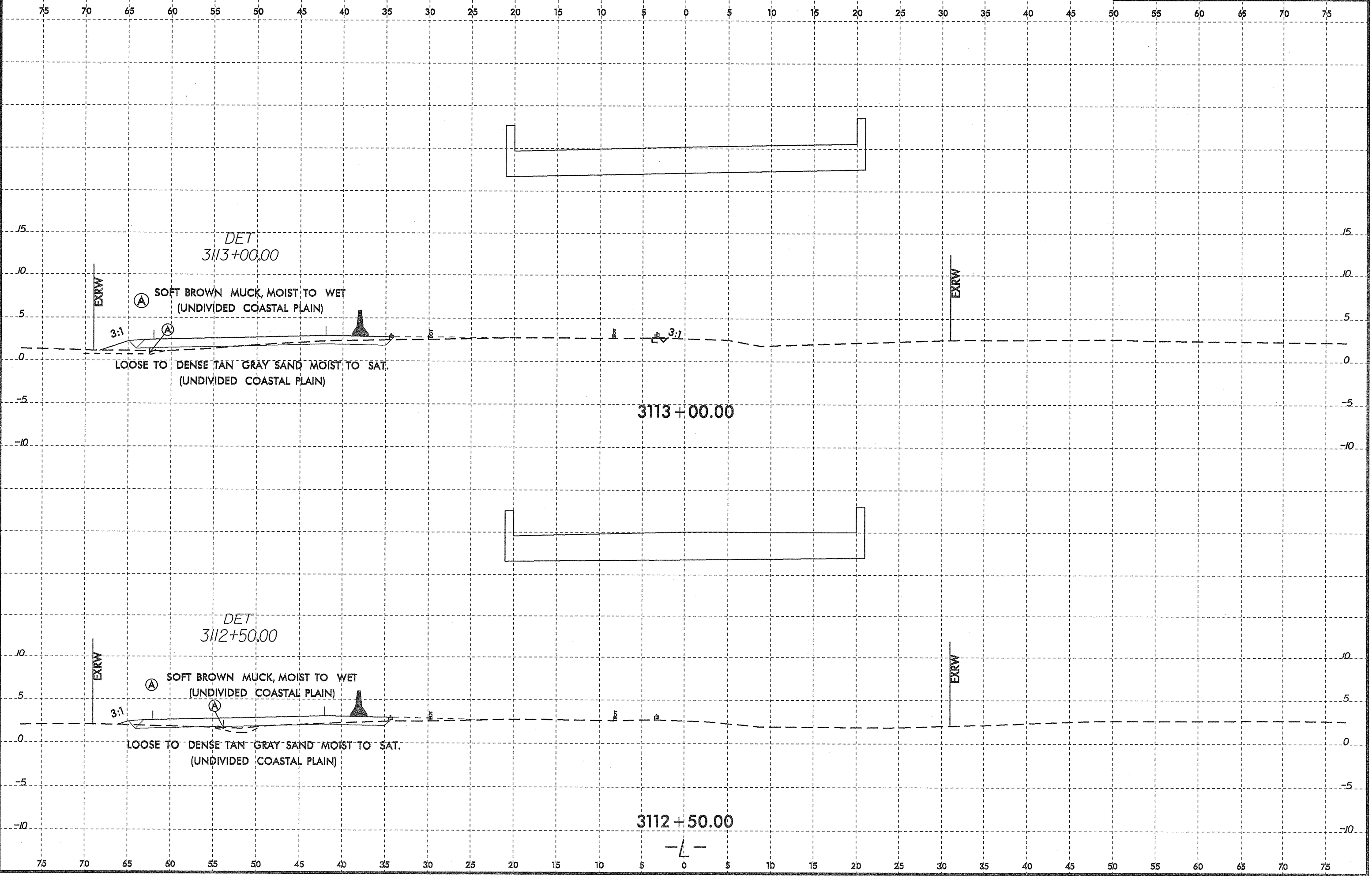
PI Sta 3179+39.53 Δ = 2° 56' 41.2" (LT) D = 3' 49' 11.0" L = 77.09' T = 38.56' R = 1500.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3180+25.72 Δ = 35° 14' 50.2" (LT) D = 38' 11" 49.9" L = 92.28' T = 47.65' R = 150.00' e = 4.0% RUNOFF = 62.92'	PI Sta 3181+22.27 Δ = 38° 11' 31.5" (RT) D = 38' 11" 49.9" L = 99.99' T = 51.93' R = 150.00' e = 4.0% RUNOFF = 62.92'
---	--	--



8/23/99
D:\FEB-2012 09:04
L:\FERD\GR\REV\1\16_105319station\TIP\B2500_GEO_RDWY\CADD_GEOTECH\XAC\B2500_GEO_RDWY_L.XSI.dgn
DATE: 02/25/16
DRAWN BY: [redacted]

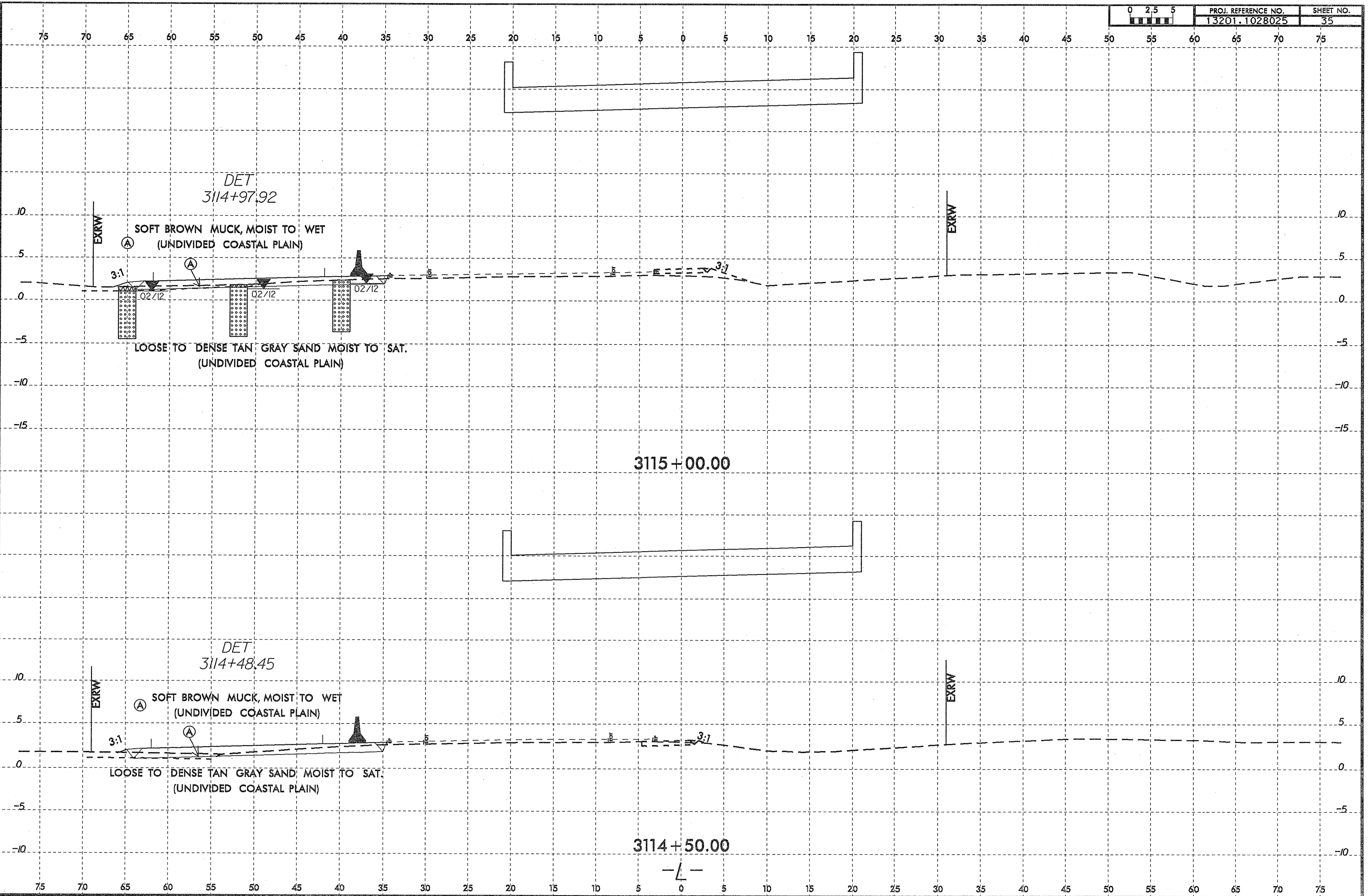


8/23/99



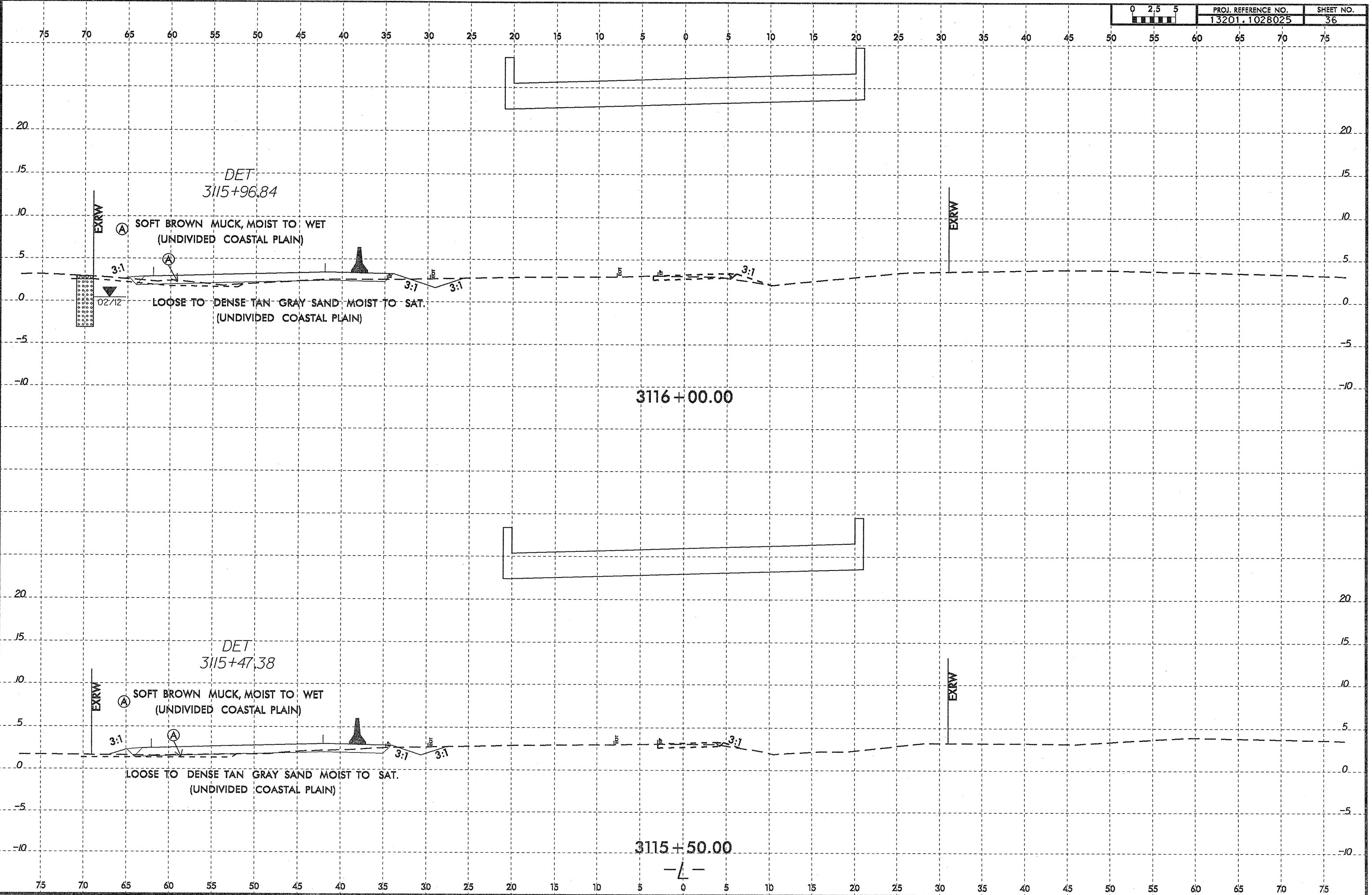
13-FEB-2012 09:04
 L:\FRO-Green\116...
 L:\FRO-Green\116...
 L:\FRO-Green\116...

8/23/99



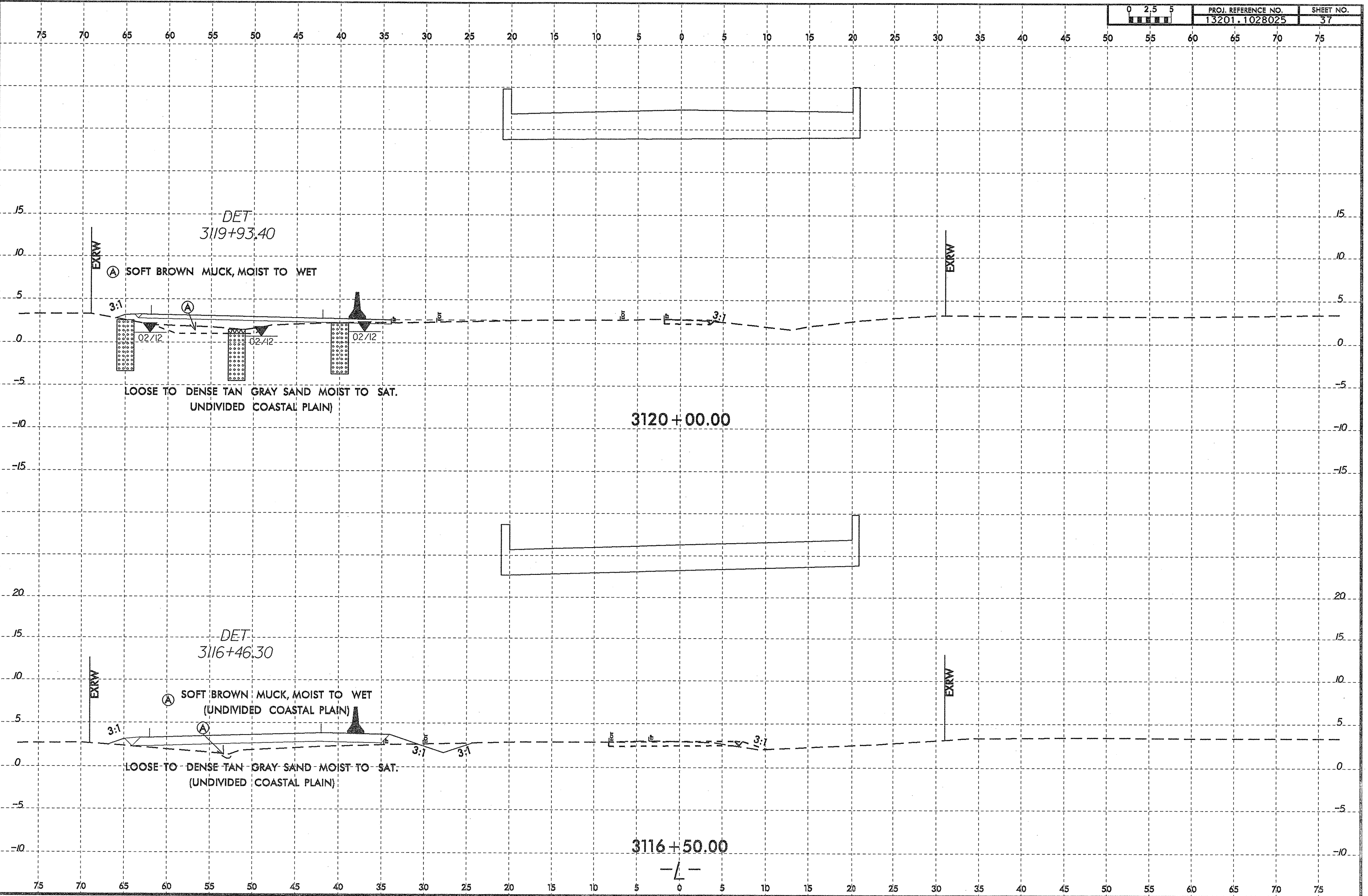
13-FEB-2012 09:05
 C:\PROGRA~1\Autocad\MapServer\GIS\G...
 13-FEB-2012 09:05
 C:\PROGRA~1\Autocad\MapServer\GIS\G...

8/23/99



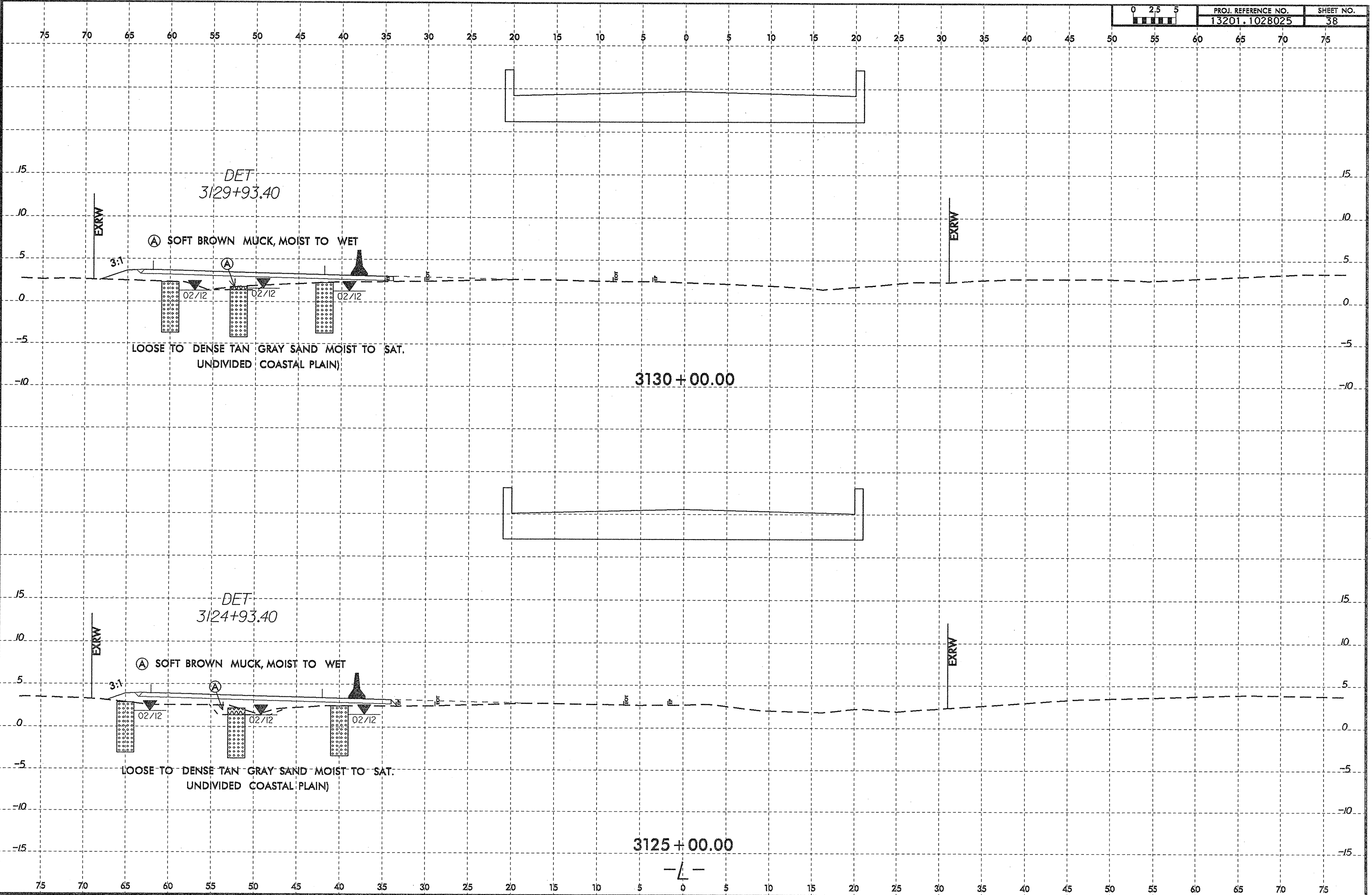
I3-FEB-2012 09:06
 C:\VRO\Greevy\UG\02366\13201.1028025\13201.1028025.dgn

8/23/99



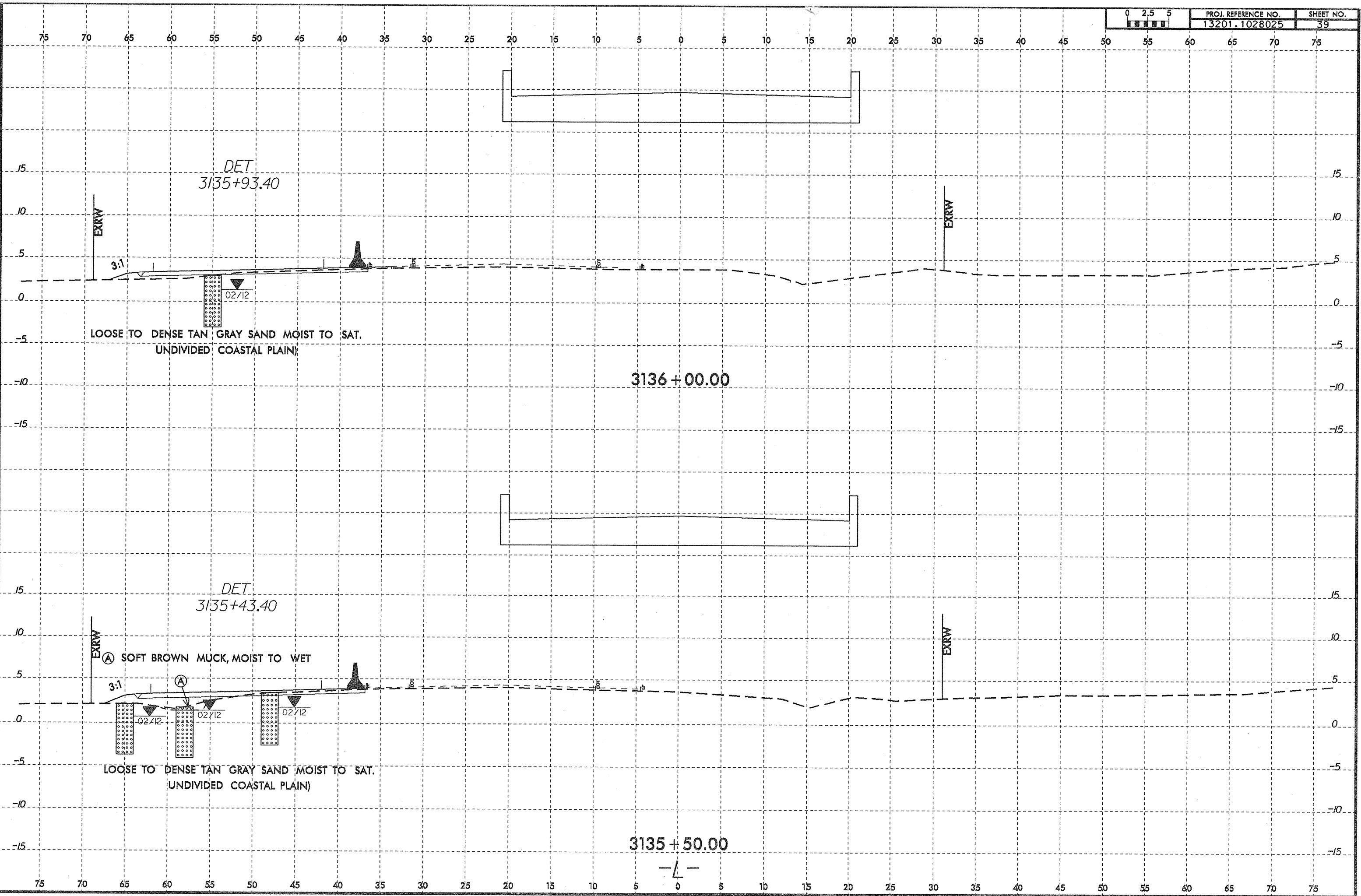
I:\FEED-2012 08\05
 L:\FEED\2012 08\05\Investigation\IP\B2500.GEO_RDWY\CADD_GEO\TECH\XSC\B2500.GEO_RDWY_L.XA.dgn
 11/16/12
 G. Turner

8/23/99



I:\FEB-2002_09:05
 C:\VERD\Green\116\202500\13201.1028025.dgn
 13-FEB-2002 09:05
 C:\VERD\Green\116\202500\13201.1028025.dgn

8/23/99



13-FEB-2012 09:05
 C:\Users\rdy\Documents\13201.1028025\13201.1028025.dgn