

NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	43684.3.1 (B-5551)	1	17
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
8B.108314.1		P.E.	
8B.108314.1		RW & UTIL.	
43684.3.1	BRSTP-0501(29)	CONST.	

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LINE	STATION	PLAN	PROFILE	XSECT
-L-	10+80.00 TO 29+95.00	4-5	6-7	8-16

BORING RB2(1) SHEET 17

ROADWAY  
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 43684.3.1 (B-5551) F.A. PROJ. BRSTP-0501(29)  
COUNTY SCOTLAND  
PROJECT DESCRIPTION ROADWAY APPROACHES TO BRIDGE  
NO. 18 OVER LEITH'S CREEK ON US 501 (-L-)

INVENTORY

CAUTION NOTICE

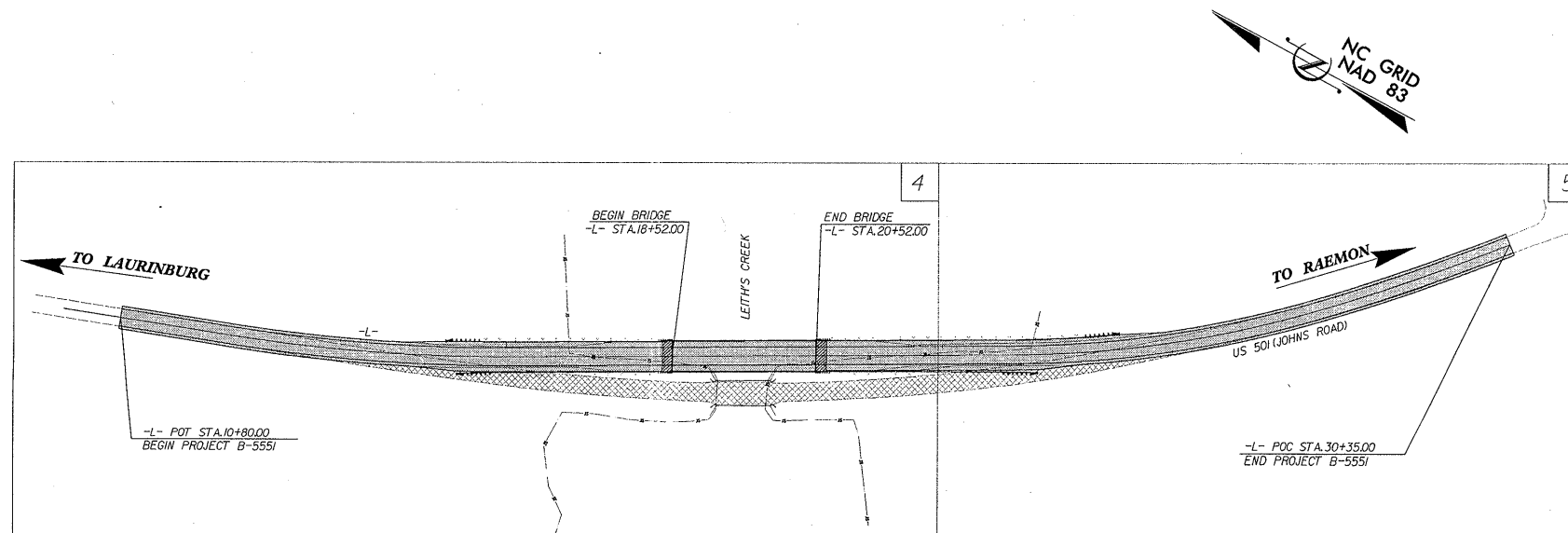
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

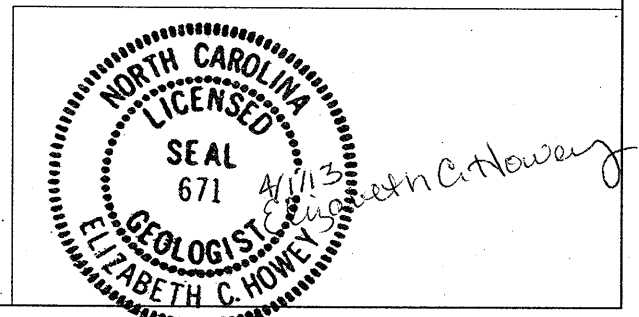
ID: B-5551

CONTRACT: C203311



- PERSONNEL
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  - B. DANIEL

INVESTIGATED BY HDR, Inc.  
CHECKED BY E. HOWEY  
SUBMITTED BY HDR, Inc.  
DATE 4/2013



DRAWN BY: M. BATTEN

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.


NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION**  
**DIVISION OF HIGHWAYS**  
**GEOTECHNICAL ENGINEERING UNIT**

PROJECT REFERENCE NO. 43684.3.(B-555)  
 SHEET NO. 2

**SUBSURFACE INVESTIGATION**

**SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

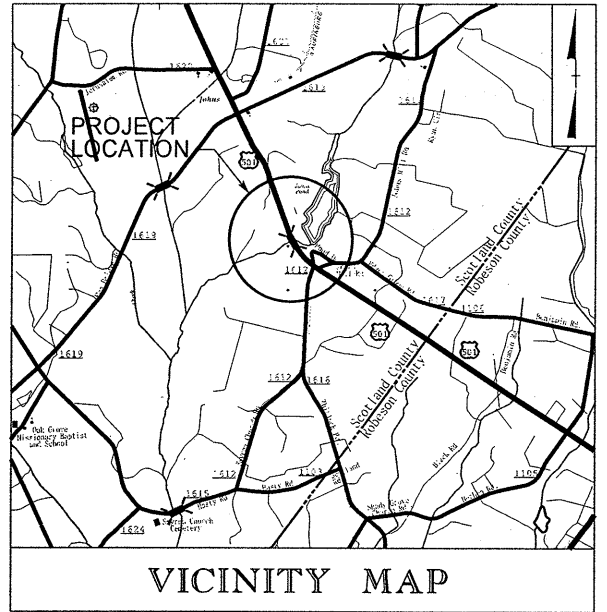
SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS			
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T296, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>		WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.		HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 60 BLOWS PER FOOT IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: 		ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRODUCED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.			
SOIL LEGEND AND AASHTO CLASSIFICATION		MINERALOGICAL COMPOSITION		WEATHERING					
GENERAL CLASS. GRANULAR MATERIALS (≤ 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS		MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.		WEATHERED ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.					
GROUP CLASS. A-1-a, A-1-b, A-2-4, A-2-5, A-2-6, A-2-7, A-4, A-5, A-6, A-7, A-7-5, A-7-6		SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50		CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.					
SYMBOL		PERCENTAGE OF MATERIAL		NON-CRYSTALLINE ROCK (NCR) FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.					
% PASSING #10, #40, #200		ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL		COASTAL PLAIN SEDIMENTARY ROCK (CP) COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.					
LIQUID LIMIT PLASTIC INDEX		TRACE OF ORGANIC MATTER 2-3% 3-5% TRACE LITTLE ORGANIC MATTER 3-5% 5-12% LITTLE MODERATELY ORGANIC 5-10% 12-20% SOME HIGHLY ORGANIC >10% >20% HIGHLY 20-35% 35% AND ABOVE		FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.					
GROUP INDEX		GROUND WATER		VERY SLIGHT (V SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.					
USUAL TYPES OF MAJOR MATERIALS		WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP		SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.					
GEN. RATING AS A SUBGRADE		MISCELLANEOUS SYMBOLS		MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.					
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30		ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD		SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT REFUSAL > 100 BPF					
CONSISTENCY OR DENSENESS		SPT DPT VST TEST BORING SAMPLE DESIGNATIONS S - BULK SAMPLE SS - SPLIT SPOON SAMPLE ST - SHELBY TUBE SAMPLE RS - ROCK SAMPLE RT - RECOMPACTED TRIAXIAL SAMPLE CBR - CALIFORNIA BEARING RATIO SAMPLE		VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF					
TEXTURE OR GRAIN SIZE		ABBREVIATIONS		COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.					
U.S. STD. SIEVE SIZE OPENING (MM)		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST WEA. - WEATHERED γ - UNIT WEIGHT γ <sub>d</sub> - DRY UNIT WEIGHT		ROCK HARDNESS		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROUDED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROUDED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.			
BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE. SD.) FINE SAND (F SD.) SILT (SL.) CLAY (CL.)		EQUIPMENT USED ON SUBJECT PROJECT		FRACTURE SPACING		BEDDING			
GRAIN SIZE MM 305, 150, 75, 2.0, 0.25, 0.075, 0.005		DRILL UNITS: MOBILE B-51, BK-51, CME-45C, CME-550, PORTABLE HOIST, DIEDRICH D-25		ADVANCING TOOLS: 3-7/8" & 2-7/8" DRAG BIT, 6" CONTINUOUS FLIGHT AUGER, 8" HOLLOW AUGERS, HARD FACED FINGER BITS, TUNG-CARBIDE INSERTS, CASING w/ ADVANCER, TRICONE STEEL TEETH, TRICONE TUNG-CARB., CORE BIT		TERM SPACING: VERY WIDE MORE THAN 10 FEET, WIDE 3 TO 10 FEET, MODERATELY CLOSE 1 TO 3 FEET, CLOSE 0.16 TO 1 FEET, VERY CLOSE LESS THAN 0.16 FEET		TERM THICKNESS: VERY THICKLY BEDDED > 4 FEET, THICKLY BEDDED 1.5 - 4 FEET, THINLY BEDDED 0.16 - 1.5 FEET, VERY THINLY BEDDED 0.03 - 0.16 FEET, THICKLY LAMINATED 0.003 - 0.03 FEET, THINLY LAMINATED < 0.003 FEET	
SOIL MOISTURE - CORRELATION OF TERMS				INDURATION					
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION				FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.					
LL LIQUID LIMIT PLASTIC RANGE (PI) PL PLASTIC LIMIT OM OPTIMUM MOISTURE SL SHRINKAGE LIMIT				FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.					
PLASTICITY									
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY									
COLOR									
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS, LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.									

09/08/13

PROJECT: B-5551

CONTRACT: DH00102

See Sheet 1-A For Index of Sheets  
See Sheet 1-B For Conventional symbols  
See Sheet 1-C For Survey Control Sheet

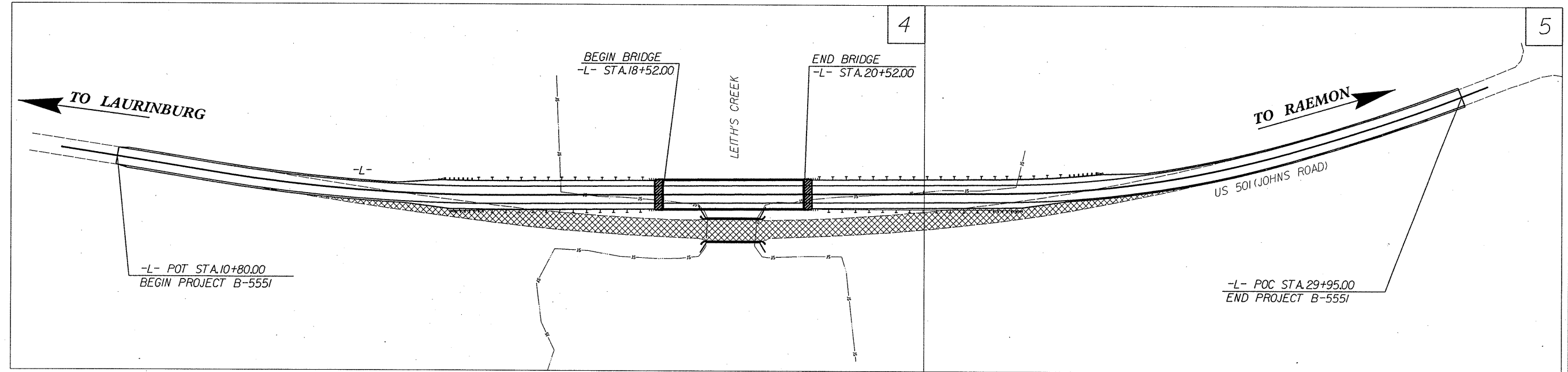


# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS SCOTLAND COUNTY

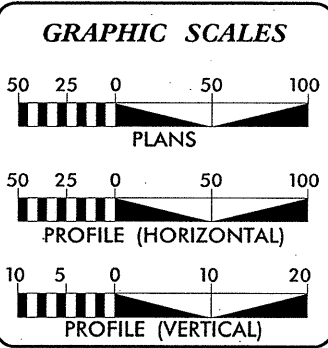
LOCATION: BRIDGE NO.18 OVER LEITH'S CREEK ON  
US 501 (JOHNS ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, STRUCTURE

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5551	2A	17
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
8B.108314.1		PE	
8B.108314.1		RW & UTIL.	
43684.3.1		CONST.	



PRELIMINARY PLANS  
DO NOT USE FOR CONSTRUCTION



**DESIGN DATA**

ADT 2013 =	4810
D =	50 %
T =	8 % *
V =	60 MPH
FUNC CLASS =	MINOR ARTERIAL
* TTST 4%	DUAL 4%

**PROJECT LENGTH**

LENGTH OF ROADWAY PROJECT B-5551 =	0.325 MI
LENGTH OF STRUCTURE PROJECT B-5551 =	0.038 MI
TOTAL LENGTH OF PROJECT B-5551 =	0.363 MI

Prepared in the Office of:  
**SEPI**  
ENGINEERING & CONSTRUCTION  
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FOR THE NORTH CAROLINA DEPT. OF TRANSPORTATION  
2012 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:  
SEPTEMBER 2012

LETTING DATE:  
JUNE 18, 2013

STEVE SCOTT, PE  
PROJECT ENGINEER

AGNIESZKA NAU, PE  
ROADWAY PROJECT DESIGN ENGINEER

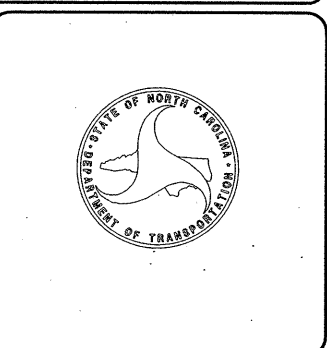
TIM WELCH, PE  
NCDOT CONTACT

HYDRAULICS ENGINEER

\_\_\_\_\_  
SIGNATURE: P.E.

ROADWAY DESIGN ENGINEER

\_\_\_\_\_  
SIGNATURE: P.E.



\$\$\$\$\$SYTIME\$\$\$\$\$  
\$\$\$\$\$DGN\$\$\$\$\$  
\$\$\$\$\$USERNAME\$\$\$\$\$



ONE COMPANY *Many Solutions*

February 21, 2013

STATE PROJECT: 43684.3.1 (B-5551)  
 FEDERAL PROJECT: BRSTP-0501 (29)  
 COUNTY: Scotland  
 DESCRIPTION: Bridge No. 18 over Leith's Creek on US 501  
 SUBJECT: Geotechnical Report – Inventory

**Project Description**

This project involves the realigned roadway approaches for the replacement of the existing bridge on US 501 over Leith's Creek. The total project length is 0.363 mile with the roadway approaches comprising 0.325 mile of that total. The existing and proposed roadway realignment each contain 2 lanes.

**Project Investigation**

A geotechnical investigation was performed in January, 2013. A Diedrich D-25 drill was mounted on a large marsh buggy to access the boring locations in the swampy area where the water depth was up to approximately 5 feet at the time of our investigation. The drill was equipped with an automatic hammer with a measured hammer efficiency of 75% (7-16-2012). Casing with 2-7/8" and 3-7/8" drag bits were used to advance the borings. Standard Penetration Tests (SPT) were performed at frequent intervals in the borings advanced along the roadway approaches.

The following alignment was investigated:

<u>Line</u>	<u>Stations</u>
-L-	10+80 to 29+95

**Areas of Special Geotechnical Interest**

- 1) Muck: Muck is present at the ground surface from approximately -L- Station 17+00 to 24+00.
- 2) Very soft alluvial clay: This material is anticipated to be present either at the ground surface or beneath the muck from approximate -L- Station 16+00 to 17+50 and 20+50 to 24+00.
- 3) Surface/Groundwater: Water is present above the ground surface across much of the site; depths ranged up to approximately 5 feet. All of the borings encountered water at or above the ground surface.
- 4) Downed trees: Downed trees cover much of the ground surface in the floodplain area.

**Physiography and Geology**

The project is located in an area of Cretaceous Age Coastal Plain Deposits. More specifically, the area is mapped as the Middendorf Formation which is described as gray to pale gray with an orange cast sand, sandstone and mudstone. The area around the site is generally rural and the Coastal Plain topography is relatively flat. Existing ground elevations across the site range from nearly 165 feet near the beginning of the project to approximately 143 feet in the floodplain. Leith's Creek is part of a large swampy/wetland area, apparently flooded by beaver activity. The creek flow is to the west.

**Soil Properties**

Soils encountered at the site were derived from two origins: alluvial deposits from Leith's Creek underlain by the above described Coastal Plain deposits of the Middendorf Formation. Existing roadway embankment is present along current US 501. No borings were advanced through the existing embankment.

The surficial alluvial soils consist of very soft, saturated black muck through much of the floodplain along the proposed alignment. The muck was generally 2 to 3.5 feet thick in borings advanced along the roadway approaches. Other alluvial soil encountered consists of very loose to medium dense silty fine to coarse sand (A-3, A-2-4) or very soft to medium stiff silty fine sandy clay (A-6, A-7-5). Many of the alluvial samples recovered contained organics and wood fragments. The total thickness of the alluvial layer ranged from 8 to 15.5 feet in borings advanced across the site.

The Middendorf Formation was encountered beneath the alluvial deposits to boring terminations and consists of very loose to dense silty sand (A-2-4), stiff fine sandy silt (A-4), and very stiff to hard silty fine sandy clay (A-6, A-7-6).

Nearly all of the recovered samples were wet to saturated.

**Groundwater**

Water was encountered at or above the ground surface at all of the boring locations at the site. As mentioned above, the proposed alignment crosses a swampy area with water depths ranging from at the surface to 5 feet deep. Per the provided Hydraulics Report, the normal water surface for Leith's Creek is located at elevation 147.5 feet.

Samples

Ten undisturbed Shelby tube samples were attempted in the roadway and bridge end bent borings along the alignment. However, upon opening the recovered samples, many were not favorable for testing due to roots or organics encountered throughout the tube or due to the fact that the sample consisted of sand rather than the clay layer that was targeted to sample. The samples below were the only tubes that recovered the soft clay and were testable:

<u>Sample No.</u>	<u>Location</u>	<u>Depth</u>	<u>Test</u>
ST-2	-L- Sta. 22+87, 7' LT	13.5 – 15.5	Consolidation
ST-3	-L- Sta. 16+50, 14' LT	8.0 – 10.0	CU Triaxial

Respectfully Submitted,

*Elizabeth C. Howey*  
 Elizabeth C. Howey, LG, PE  
 Senior Geotechnical Project Manager

EARTHWORK BALANCE SHEET  
Volumes in Cubic Yards

PROJECT: B-5551

COUNTY: Scotland

DATE: 6/3/2013

COMPILED BY: SEPI Engineering

SHEET 1 OF 1 SHEETS

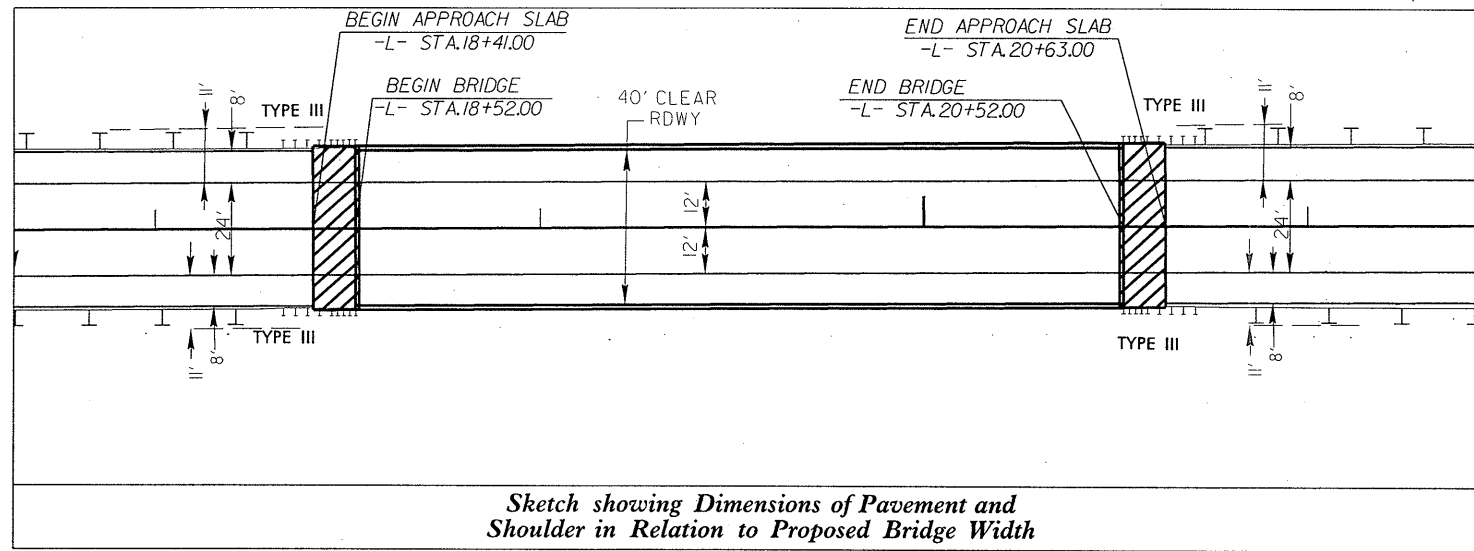
STATION	STATION	EXCAVATION					EMBANKMENT				BORROW	WASTE			
		TOTAL UNCLASS.	ROCK	UNDERCUT	UNSUIT. UNCLASS.	SUITABLE UNCLASS.	TOTAL	ROCK	EARTH	EMBANK. +25%		ROCK	SUITABLE	UNSUIT.	TOTAL
10+80.00	30+35	745		2,217		745	10,877		10,877	13,596	12,851			2,217	2,217
	<b>SUBTOTAL</b>	745		2,217		745	10,877		10,877	13,596	12,851			2,217	2,217
EXISTING ROADBED REMOVAL STA. 15+00.00 TO 24+50.00 (AFTER NEW BRIDGE)		9,790				9,790								9,790	9,790
	<b>SUBTOTAL</b>	9,790				9,790								9,790	9,790
	<b>SUBTOTAL</b>														
	<b>SUBTOTAL</b>														
	<b>SUBTOTAL</b>														
	<b>TOTAL</b>	10,535		2,217		10,535	10,877		10,877	13,596	12,851			9,790	12,007
LOSS DUE TO CLEARING & GRUBBING															
ADDITIONAL UNDERCUT (GEOTECHNICAL UNIT)				100			100		100	125	125			100	100
SELECT GRANULAR MATERIAL IN LIEU OF BORROW										-6,625	-6,625				
ROCK WASTE TO REPLACE BORROW															
WASTE IN LIEU OF BORROW															
	<b>PROJECT TOTAL</b>	10,535		2,317		10,535	10,977		10,977	7,096	6,351			9,790	12,107
EST. 5% TO REPLACE TOP SOIL ON BORROW PIT															
	<b>GRAND TOTAL</b>	10,535		2,317		10,535	10,977		10,977	7,096	6,351			9,790	12,107
	<b>SAY</b>														

NOTE: EARTHWORK QUANTITIES ARE CALCULATED BY SEPI ENGINEERING AND CONSTRUCTION. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT. PER GEOTECH RECOMMENDATION, ESTIMATED 5,300 CY OF SELECT GRANULAR MATERIAL, CLASS III WILL BE REQUIRED



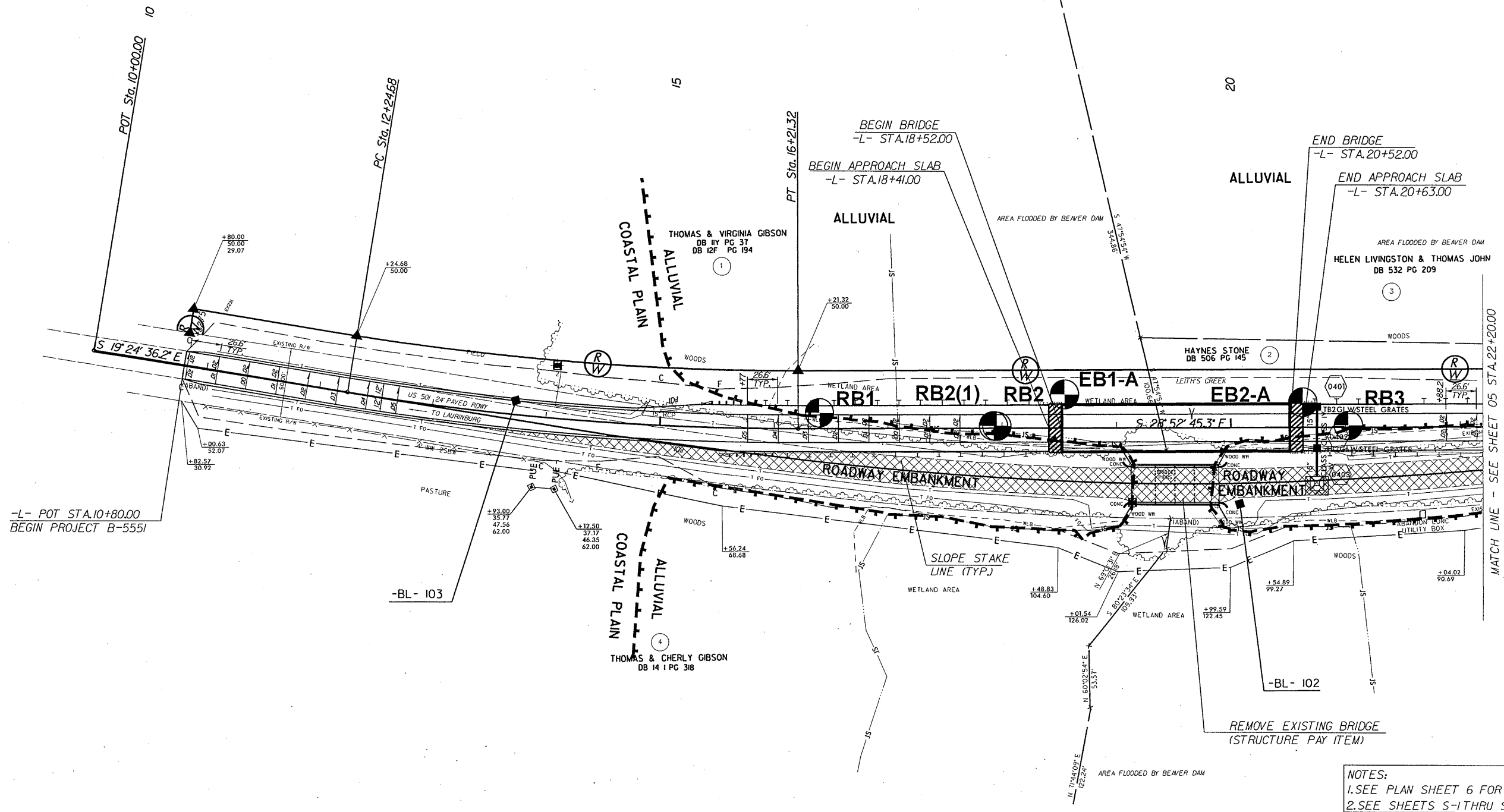
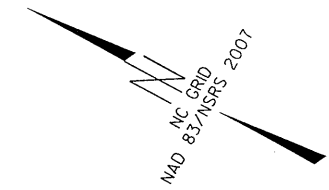
5/14/99

PROJECT REFERENCE NO. B-5551	SHEET NO. 4
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



Sketch showing Dimensions of Pavement and Shoulder in Relation to Proposed Bridge Width

-L-  
 PI Sta 14+23.45  
 $\Delta = 9' 28'' 09.1'' (LT)$   
 $D = 2' 23'' 14.4''$   
 $L = 396.64'$   
 $T = 198.77'$   
 $R = 2,400.00'$

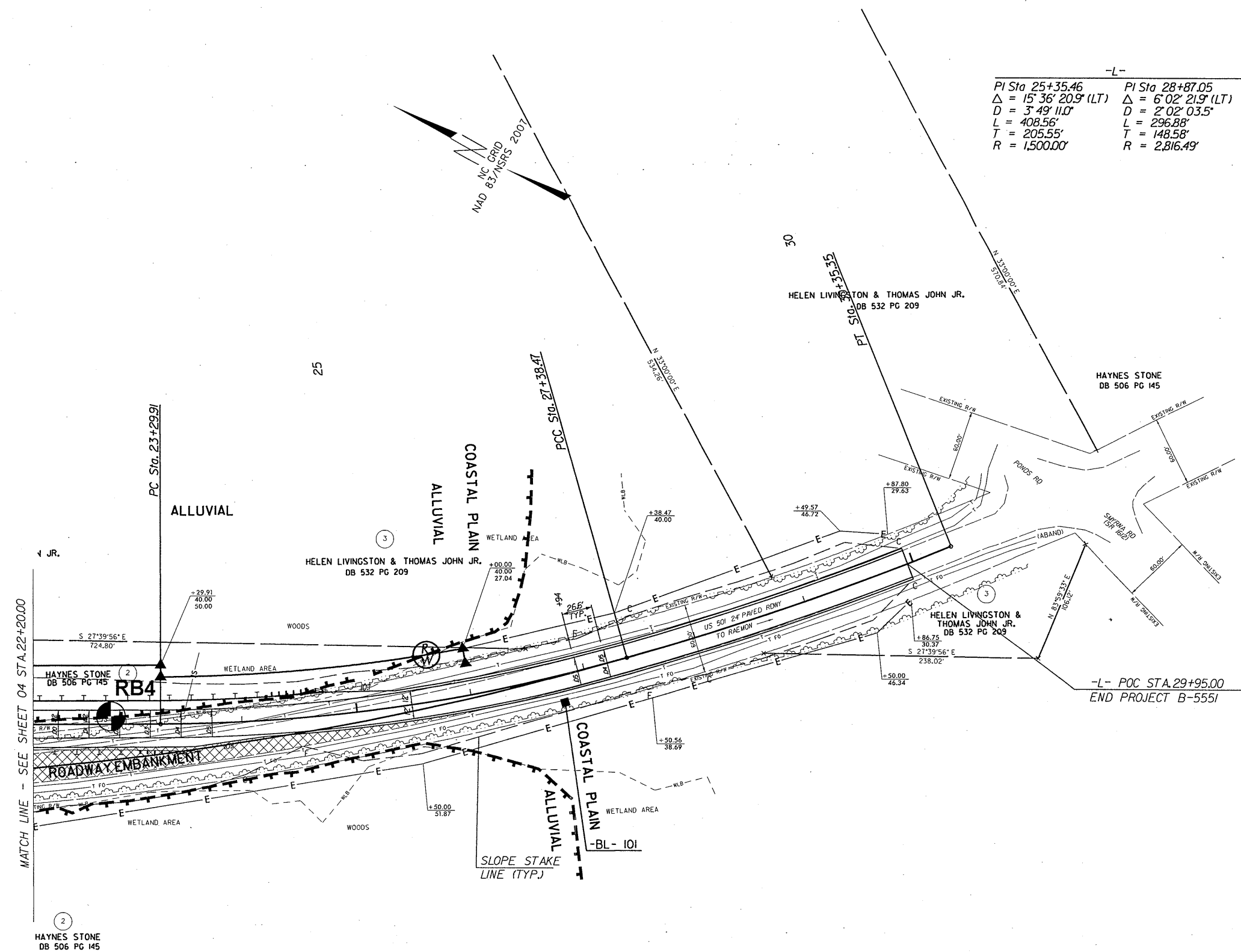


NOTES:  
 1. SEE PLAN SHEET 6 FOR PROFILE  
 2. SEE SHEETS S-1 THRU S-4 FOR STRUCTURES PLANS

4/1/2013  
1:57:56 PM  
Inventory Plan Sheet 1

5/14/99

PROJECT REFERENCE NO. <b>B-555I</b>	SHEET NO. <b>5</b>
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



MATCH LINE - SEE SHEET 04 STA. 22+20.00

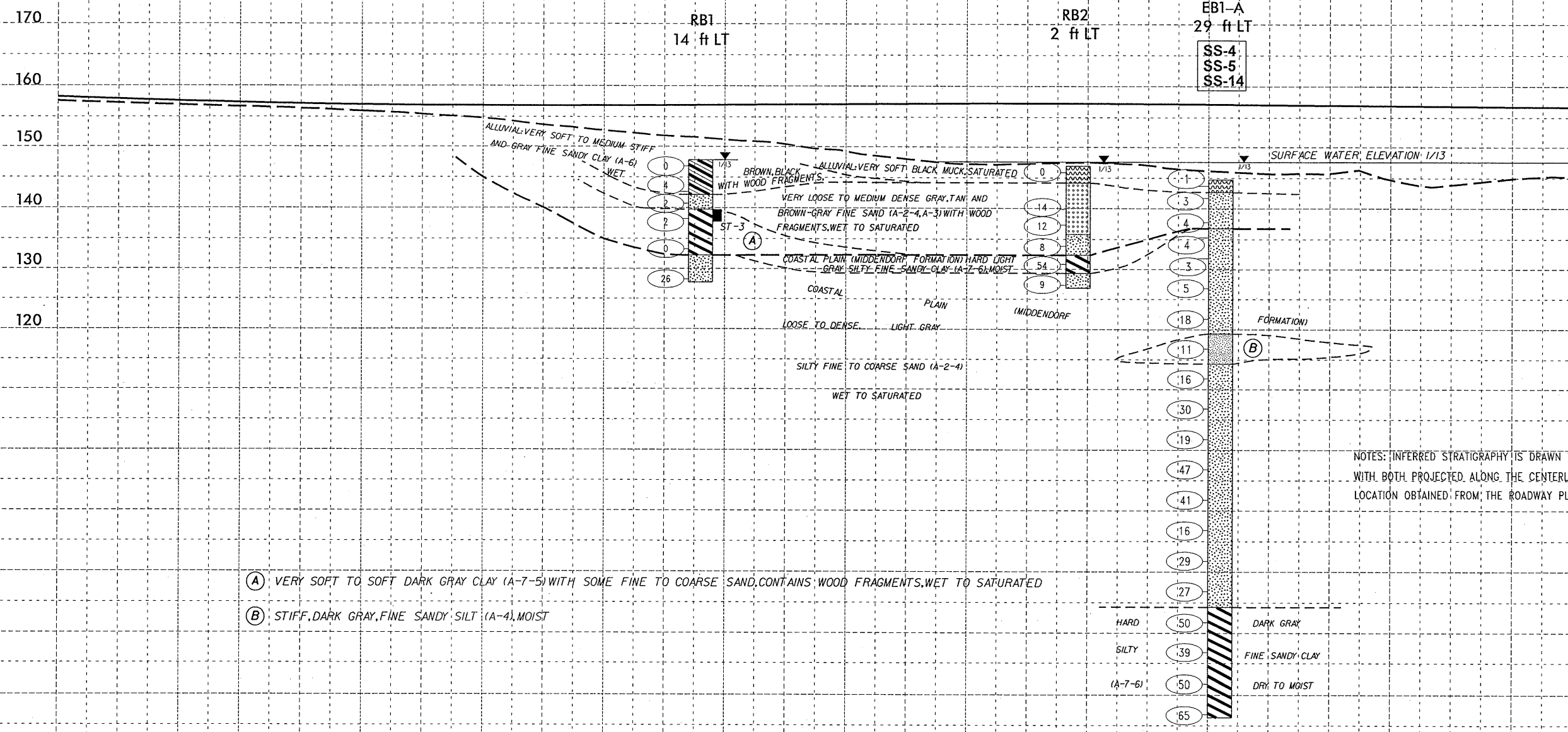
-L- POC STA. 29+95.00  
END PROJECT B-555I

4/7/2013  
Inventory Plan Sheet 2

NOTE: SEE PLAN SHEET 6 FOR PROFILE



PROJECT REFERENCE NO. 43684.31 (B-555)	SHEET NO. 6
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

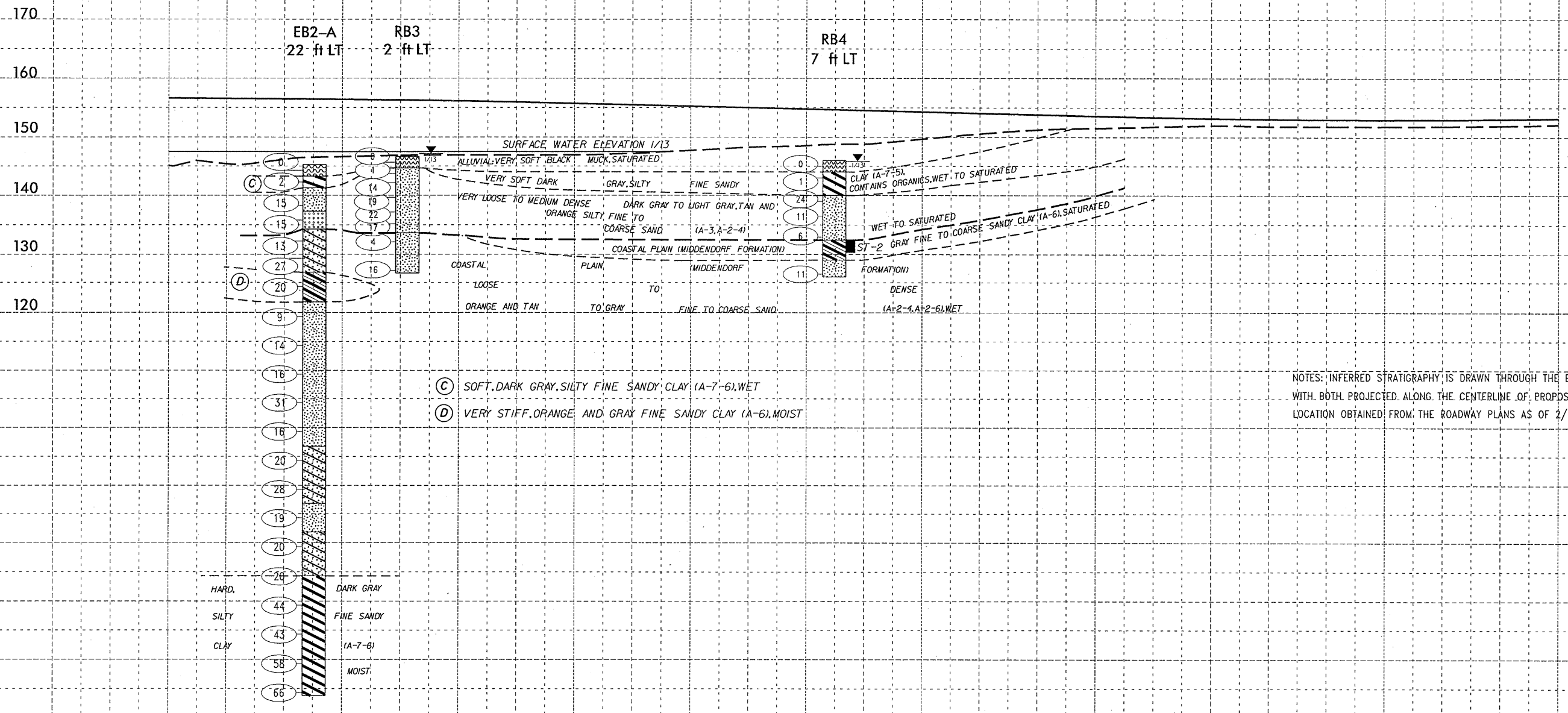


NOTES: INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ALONG THE CENTERLINE OF PROPOSED F-L PROFILE LOCATION OBTAINED FROM THE ROADWAY PLANS AS OF 2/12/13

- (A) VERY SOFT TO SOFT DARK GRAY CLAY (A-7-5) WITH SOME FINE TO COARSE SAND, CONTAINS WOOD FRAGMENTS, WET TO SATURATED
- (B) STIFF, DARK GRAY, FINE SANDY SILT (A-4), MOIST

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-3	14 LT	16+40	8.0-10.0	A-7-5 (5)	86	55	4.6	15.4	18.6	61.4	99.8	97.8	82.5	84.5	-
SS-4	29 LT	18+55	8.5-10.0	A-2-4 (0)	25	NP	39.2	40.9	2.6	17.3	99.8	90.9	20.7	26.5	-
SS-5	29 LT	18+55	12.0-13.5	A-2-4 (0)	23	5	78.3	9.1	1.2	11.4	91.4	33.8	12.2	18.6	-
SS-14	29 LT	18+55	57.0-58.5	A-2-4 (0)	33	NP	28.2	47.5	6.2	18.1	99.2	89.6	25.7	27.1	-

NOTE: FOR PLAN VIEW SEE SHEET 4 & 5

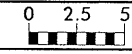


NOTES: INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ALONG THE CENTERLINE OF PROPOSED L-L PROFILE LOCATION OBTAINED FROM THE ROADWAY PLANS AS OF 2/12/13

**SOIL TEST RESULTS**

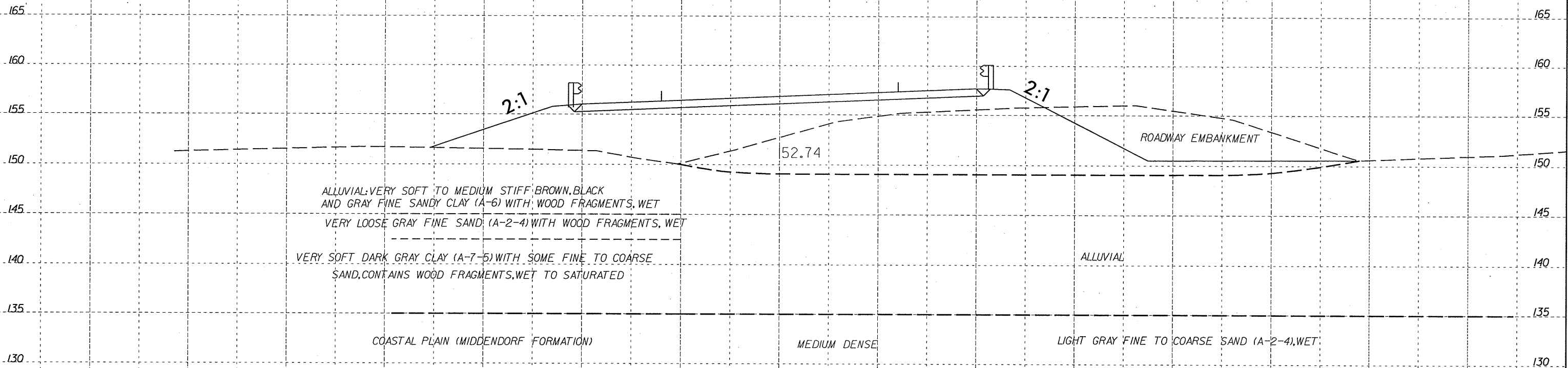
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-2	7 LT	22+87	13.5-15.5	A-6 (3)	38	20	24.4	39.5	7.6	28.5	99.8	83.3	40.2	42.2	-

8/23/99



PROJ. REFERENCE NO.	SHEET NO.
B-5551	8

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75



ALLUVIAL: VERY SOFT TO MEDIUM STIFF BROWN, BLACK  
AND GRAY FINE SANDY CLAY (A-6) WITH WOOD FRAGMENTS, WET  
VERY LOOSE GRAY FINE SAND (A-2-4) WITH WOOD FRAGMENTS, WET  
VERY SOFT DARK GRAY CLAY (A-7-5) WITH SOME FINE TO COARSE  
SAND, CONTAINS WOOD FRAGMENTS, WET TO SATURATED

COASTAL PLAIN (MIDDENDORF FORMATION)

MEDIUM DENSE

LIGHT GRAY FINE TO COARSE SAND (A-2-4), WET

ROADWAY EMBANKMENT

52.74

2:1

2:1

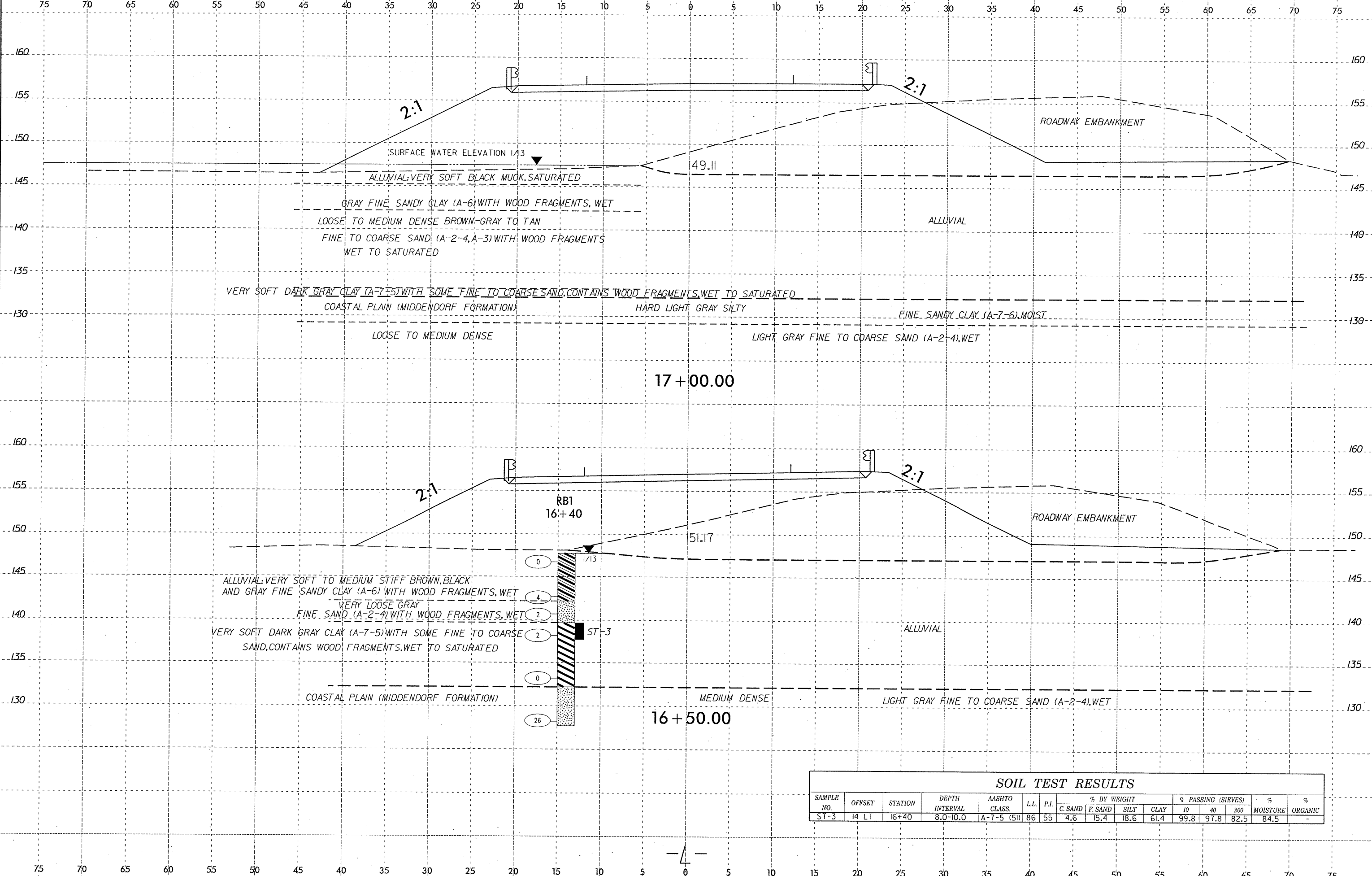
16 + 00.00

-4-

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

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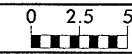
8/23/99



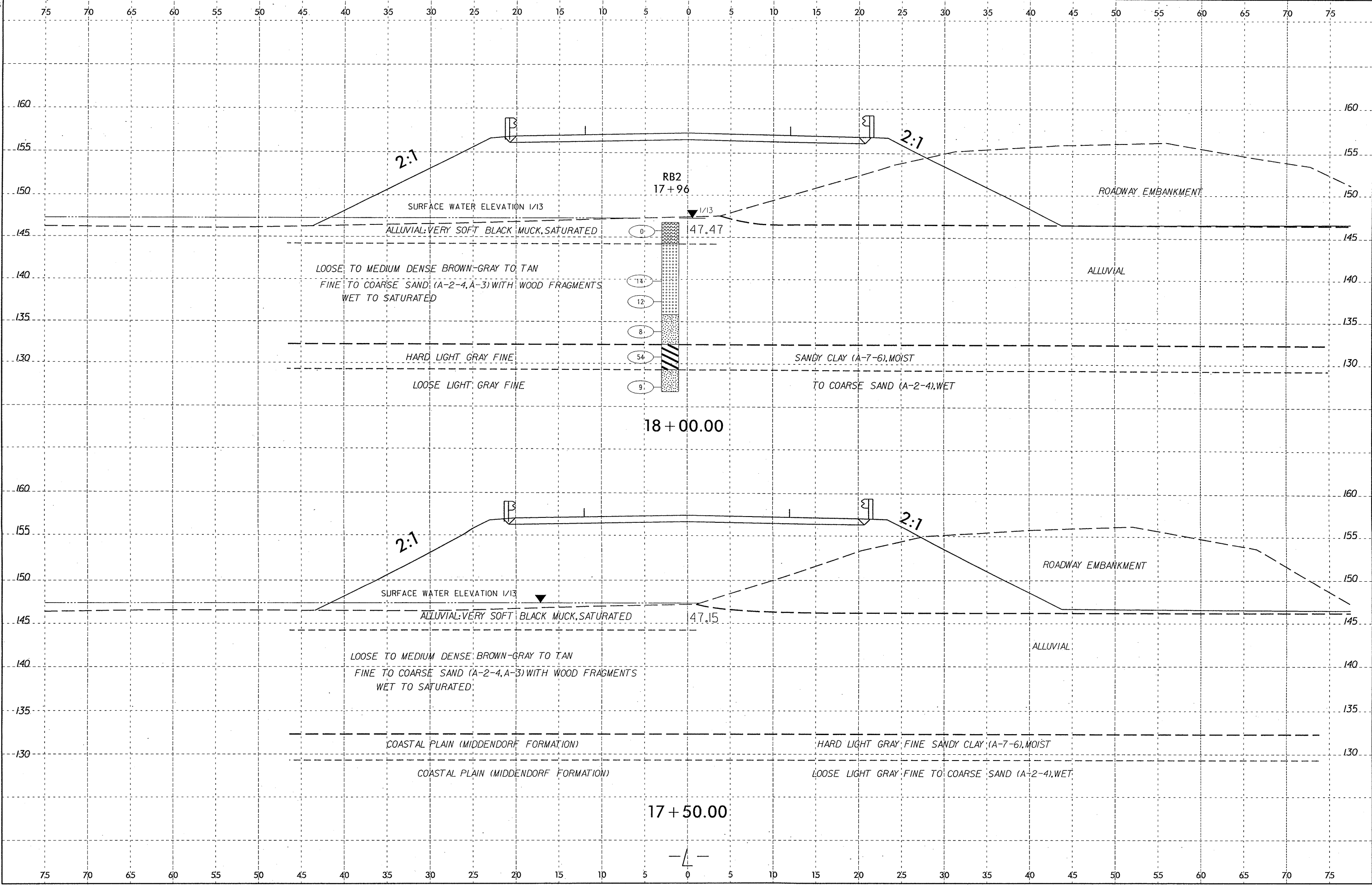
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-3	14 LT	16+40	8.0-10.0	A-7-5 (SI)	86	55	4.6	15.4	18.6	61.4	99.8	97.8	82.5	84.5	-

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8/23/99



PROJ. REFERENCE NO.	SHEET NO.
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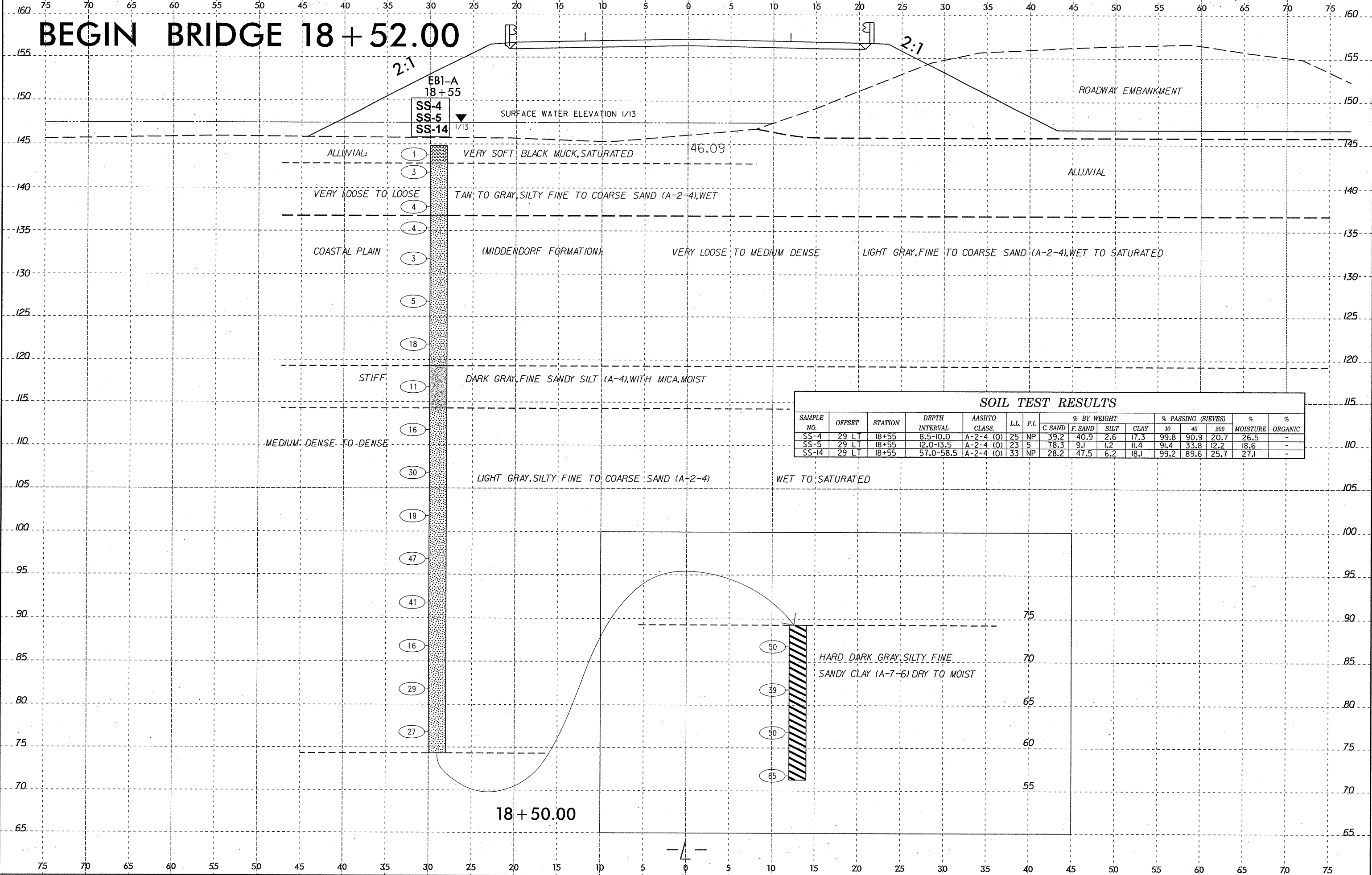


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8/23/99



# BEGIN BRIDGE 18+52.00



EB1-A  
18+55  
SS-4  
SS-5  
SS-14

SURFACE WATER ELEVATION 1/13

ALLUVIAL: (1) VERY SOFT BLACK MUCK, SATURATED

VERY LOOSE TO LOOSE TAN TO GRAY SILTY FINE TO COARSE SAND (A-2-4), WET

COASTAL PLAIN (MIDDENDORF FORMATION): (3) VERY LOOSE TO MEDIUM DENSE LIGHT GRAY, FINE TO COARSE SAND (A-2-4), WET TO SATURATED

STIFF (11) DARK GRAY, FINE SANDY SILT (A-4), WITH MICA, MOIST

MEDIUM-DENSE TO DENSE (16) LIGHT GRAY, SILTY FINE TO COARSE SAND (A-2-4) WET TO SATURATED

(30) LIGHT GRAY, SILTY FINE TO COARSE SAND (A-2-4) WET TO SATURATED

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-4	29 LT	18+55	8.5-10.0	A-2-4 (O)	25	NP	39.2	40.9	2.6	17.3	99.8	90.9	20.7	26.5	-
SS-5	29 LT	18+55	12.0-13.5	A-2-4 (O)	23	5	78.3	9.1	1.2	11.4	91.4	33.8	12.2	18.6	-
SS-14	29 LT	18+55	57.0-58.5	A-2-4 (O)	33	NP	28.2	47.5	6.2	18.1	99.2	89.6	25.7	27.1	-

(30) HARD DARK GRAY, SILTY FINE SANDY CLAY (A-7-6) DRY TO MOIST

18+50.00

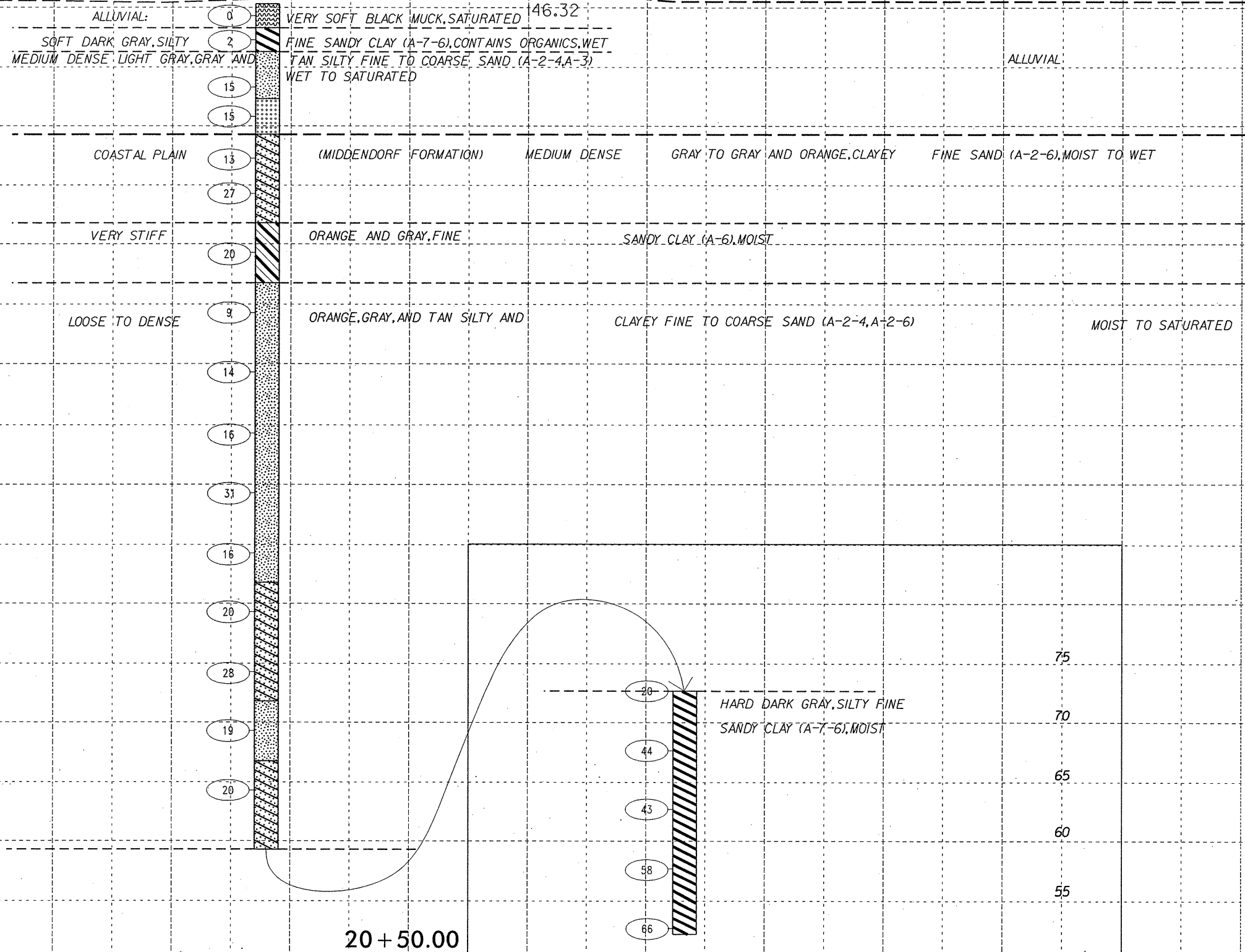
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# END BRIDGE 20 + 52.00

EB2-A  
 20 + 63

SURFACE WATER ELEVATION 1/13

ROADWAY EMBANKMENT

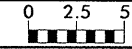


20 + 50.00

8/23/99  
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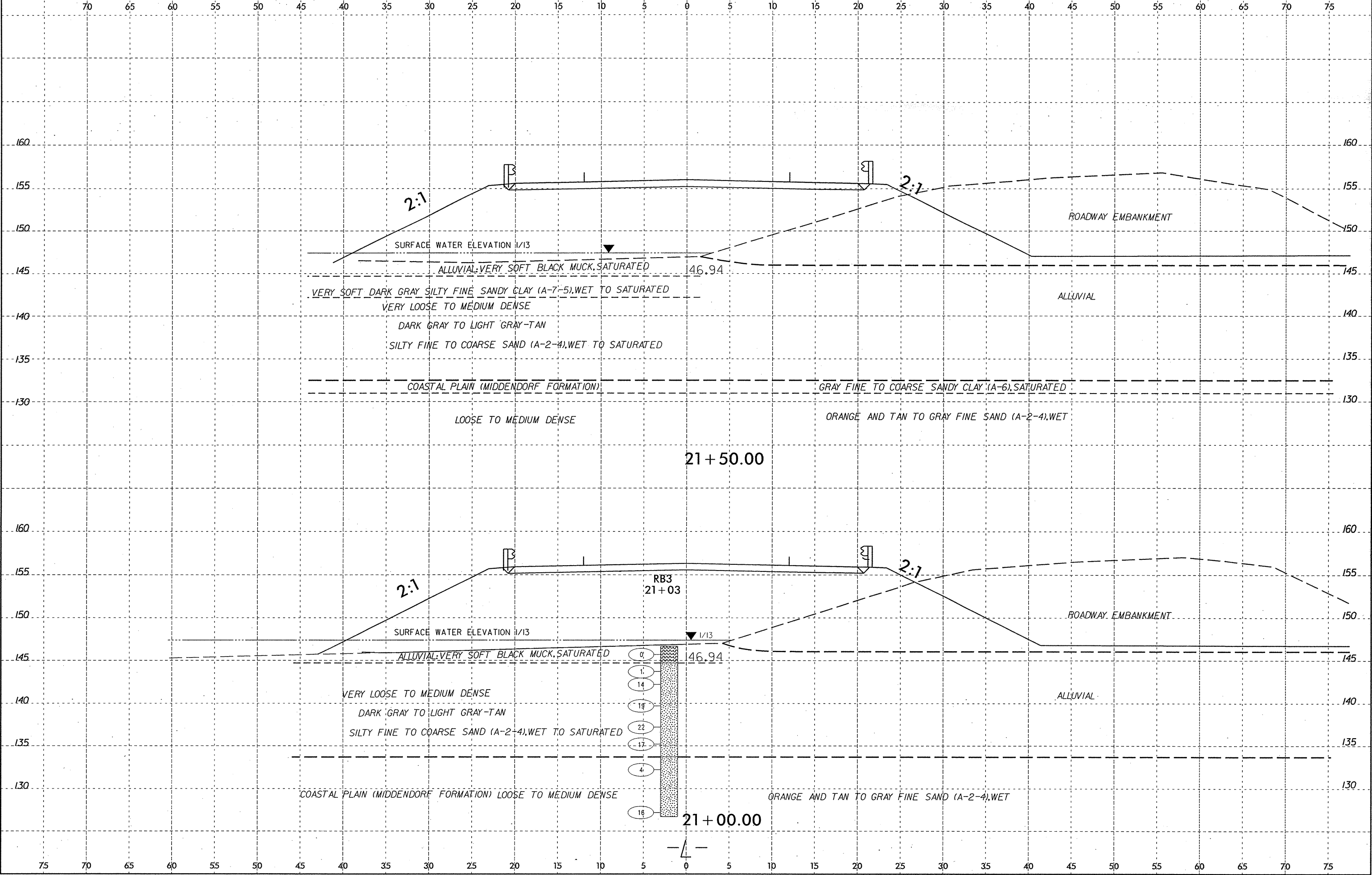


8/22/99



PROJ. REFERENCE NO.  
B-5551

SHEET NO.  
13



2:1

2:1

ROADWAY EMBANKMENT

SURFACE WATER ELEVATION 1/13

ALLUVIAL VERY SOFT BLACK MUCK, SATURATED 46.94

VERY SOFT DARK GRAY SILTY FINE SANDY CLAY (A-7-5), WET TO SATURATED  
VERY LOOSE TO MEDIUM DENSE  
DARK GRAY TO LIGHT GRAY-TAN  
SILTY FINE TO COARSE SAND (A-2-4), WET TO SATURATED

ALLUVIAL

COASTAL PLAIN (MIDDENDORF FORMATION)

GRAY FINE TO COARSE SANDY CLAY (A-6), SATURATED

LOOSE TO MEDIUM DENSE

ORANGE AND TAN TO GRAY FINE SAND (A-2-4), WET

21 + 50.00

2:1

2:1

ROADWAY EMBANKMENT

SURFACE WATER ELEVATION 1/13

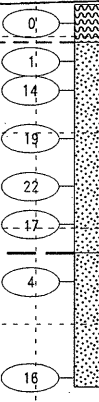
ALLUVIAL VERY SOFT BLACK MUCK, SATURATED 46.94

VERY LOOSE TO MEDIUM DENSE  
DARK GRAY TO LIGHT GRAY-TAN  
SILTY FINE TO COARSE SAND (A-2-4), WET TO SATURATED

ALLUVIAL

COASTAL PLAIN (MIDDENDORF FORMATION) LOOSE TO MEDIUM DENSE

ORANGE AND TAN TO GRAY FINE SAND (A-2-4), WET

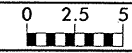


RB3  
21 + 03

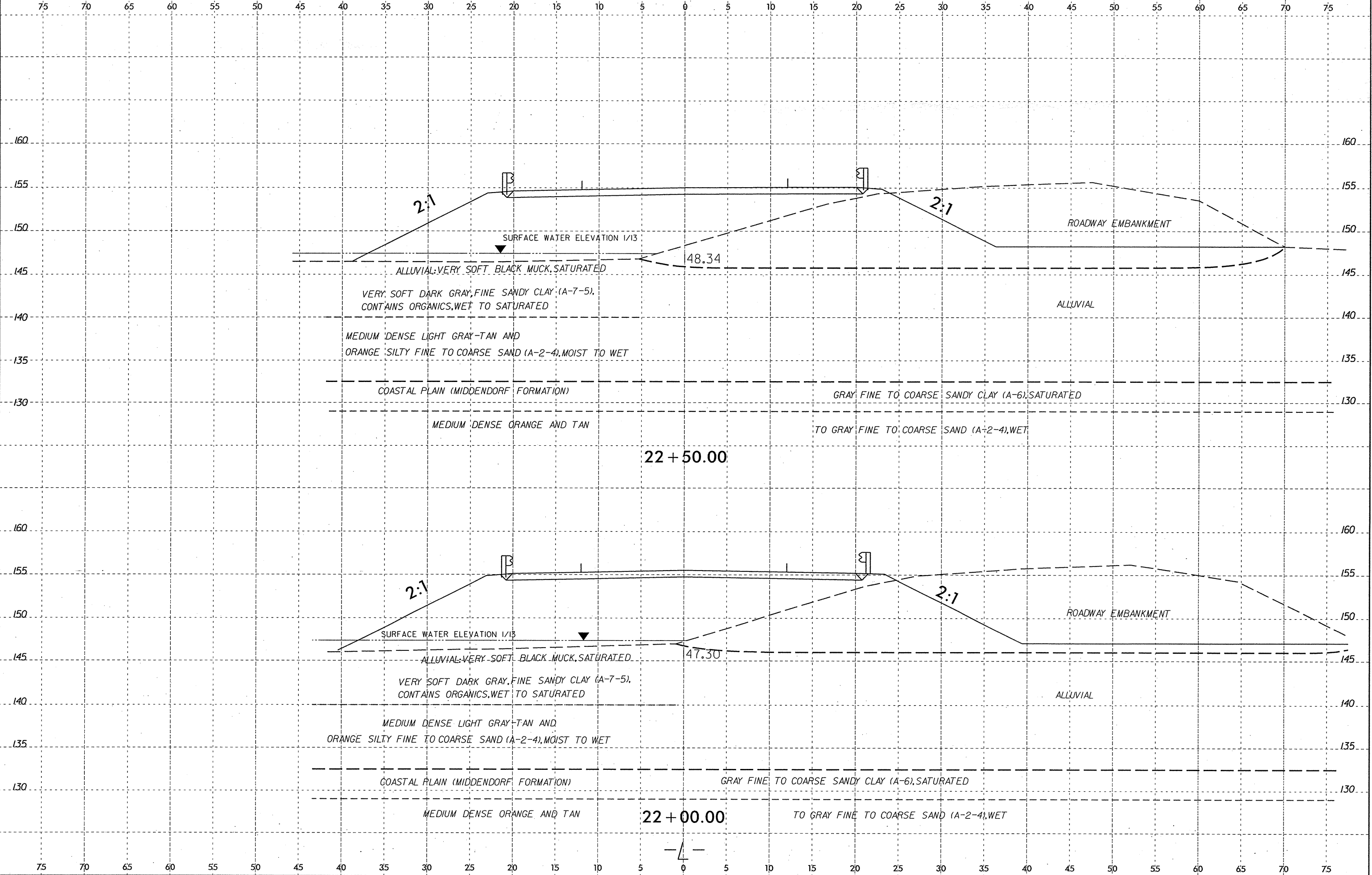
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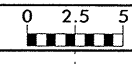


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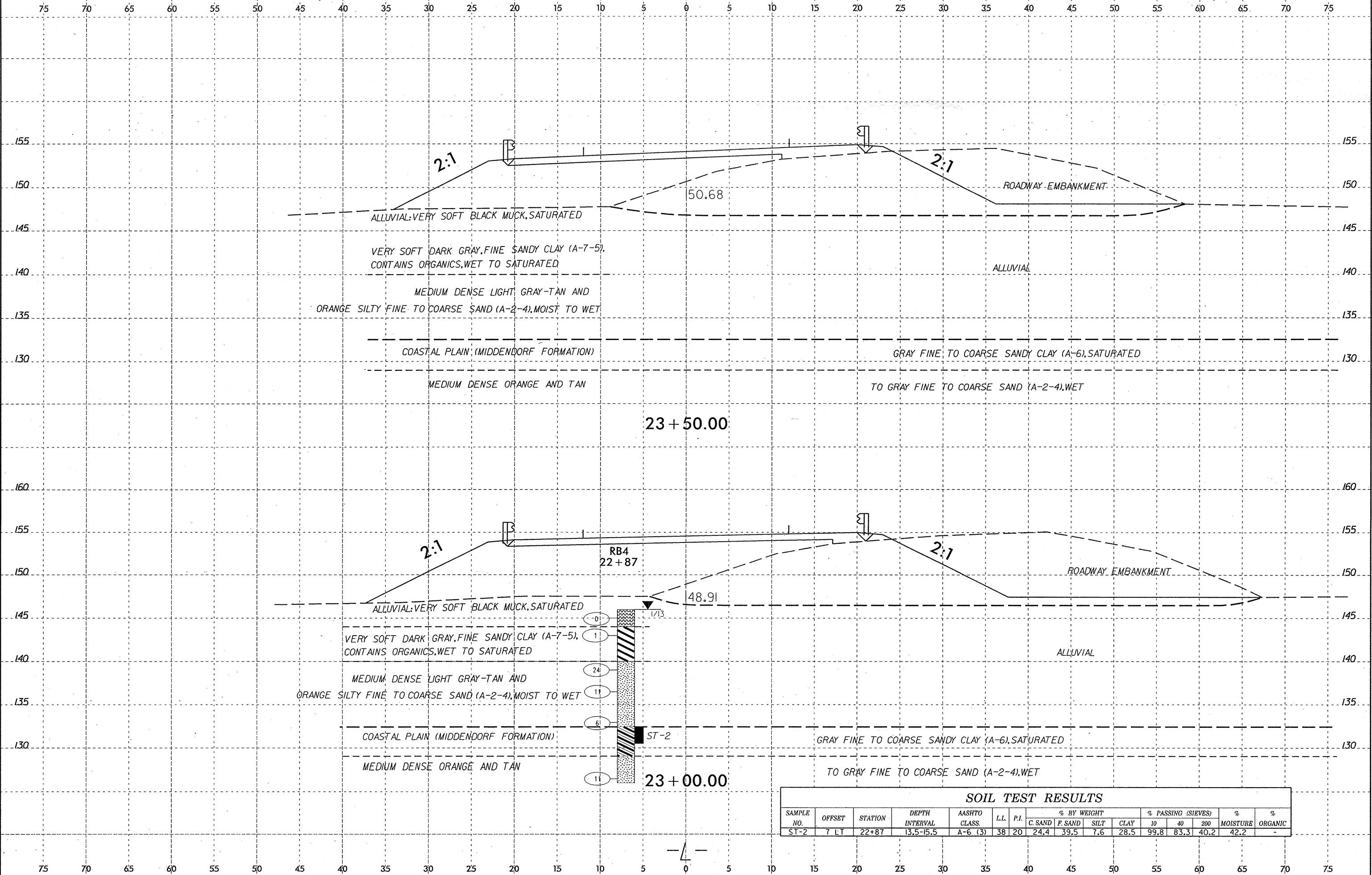


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8/23/99



PROJ. REFERENCE NO. B-5551 SHEET NO. 15



23 + 50.00

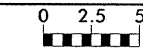
RB4  
22 + 87

23 + 00.00

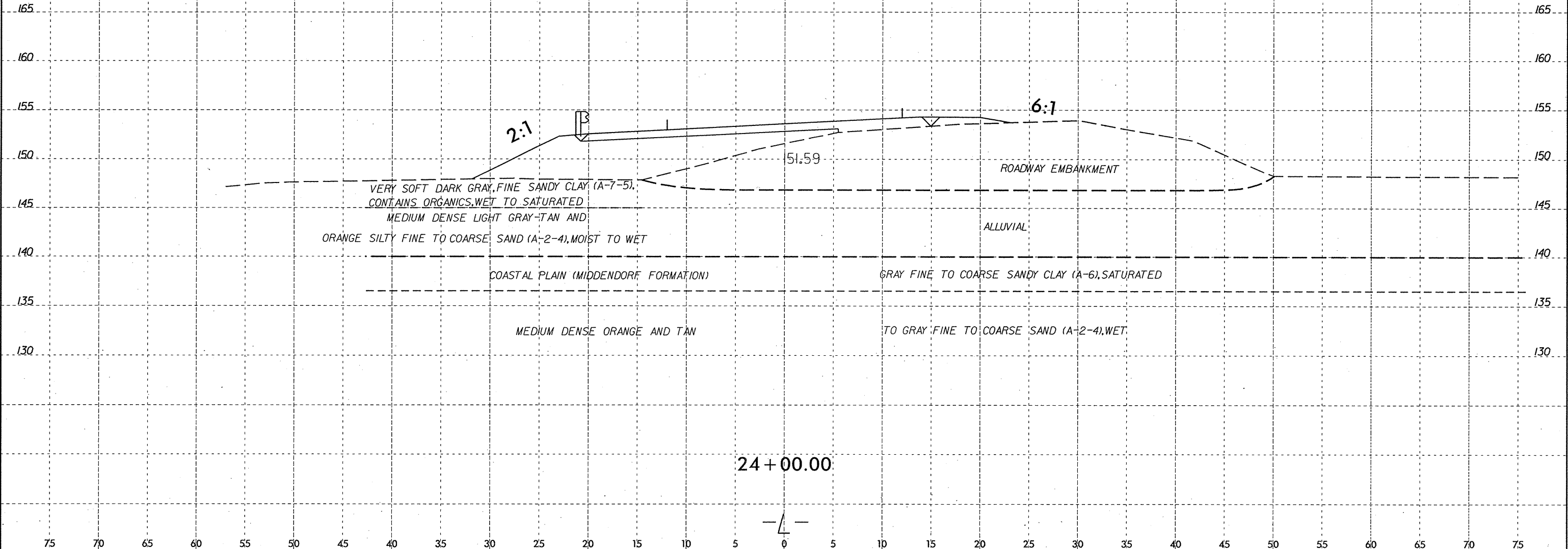
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-2	7 LT	22+87	13.5-15.5	A-6 (3)	38	20	24.4	39.5	7.6	28.5	99.8	83.3	40.2	42.2	-

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75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75



VERY SOFT DARK GRAY FINE SANDY CLAY (A-7-5), CONTAINS ORGANICS, WET TO SATURATED  
MEDIUM DENSE LIGHT GRAY-TAN AND ORANGE SILTY FINE TO COARSE SAND (A-2-4), MOIST TO WET

COASTAL PLAIN (MIDDENDORF FORMATION)

MEDIUM DENSE ORANGE AND TAN

GRAY FINE TO COARSE SANDY CLAY (A-6), SATURATED

TO GRAY FINE TO COARSE SAND (A-2-4), WET

ROADWAY EMBANKMENT

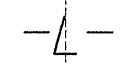
ALLUVIAL

2:1

6:1

51.59

24 + 00.00



WBS 43684.3.1		TIP B-5551		COUNTY SCOTLAND		GEOLOGIST Batten, M.								
SITE DESCRIPTION Bridge No. 18 over Leith's Creek on US 501								GROUND WTR (ft)						
BORING NO.	RB2(1)	STATION	17+93	OFFSET	2 ft LT	ALIGNMENT	-L-							
COLLAR ELEV.	146.8 ft	TOTAL DEPTH	5.0 ft	NORTHING	341,506	EASTING	1,869,013							
DRILL RIG/HAMMER EFF./DATE MAD25152 Diedrich D-25 75% 7/16/2012				DRILL METHOD		Mud Rotary 3-7/8" Drag Bit								
DRILLER		START DATE		COMP. DATE		SURFACE WATER DEPTH								
Fowler, B.		01/23/13		01/23/13		0.5ft								
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
150														
												▼		146.8 GROUND SURFACE 0.0
145	144.8	2.0												ALLUVIAL Black MUCK
	143.3	3.5	1	WOH	WOH							W		143.3 3.5
			3	4	6	10						W		141.8 Light gray-brown, fine SAND (A-3) 5.0
														Boring Terminated at Elevation 141.8 ft in SAND
														<p>NOTES:</p> <p>1) Offset boring taken for RB2-A because SPT's could not be properly performed at 1.5' and 3.5' in depth after Shelby tubes were pushed.</p> <p>Note water surface at 0.5ft above ground surface</p>