NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RAIL DIVISION
GEOTECHNICAL ENGINEERING UNIT

CONTENTS

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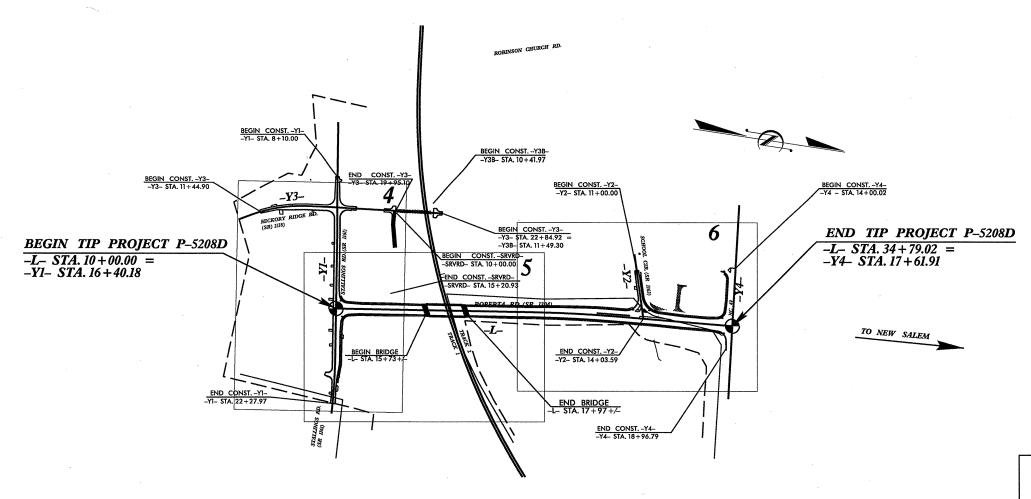
LINE	STATION	PLAN	PROFILE	XSEC
-L-	10+00 - 34+79	4-6	7-9	13-16
-YI-	8+10 - 22+28	4	10-11	17-18
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-Y3-	11+45 - 17+50	4	12	
-Y4-	14+00 - 18+97	6	-	19-21

ROADWAY SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. P-5208D F.A. PROJ. COUNTY CABARRUS

PROJECT DESCRIPTION ROBERTA ROAD EXTENSION OVER NS/NCRR FROM STALLINGS RD. (SR 1161) TO NC 49

INVENTORY





	N.C.		P-5208D		1	25
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CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, CEOTECHNICAL ENGINEERING UNIT AT 1919 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORNING LOSS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORNOS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE, THE LABORATORY SAMPLE DATA AND THE IN SITU UN-PLACE) TEST DATA CAN BE RELED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH THE ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELAMMARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BUDDING AND CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BUDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY OF SATISFY SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS TO BE ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PERSONNEL
C. V. NORVILLE
J. R. HAMM

T. E. EVANS

INVESTIGATED BY T.E.E / J.R.H.

HECKED BY <u>C. V. NORVILLE</u>

SUBMITTED BY FALCON ENG.

DATE DECEMBER, 2012

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT
OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS,
SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

DRAWN BY: T. E. EVANS

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS SOIL DESCRIPTION GRADATION ROCK DESCRIPTION TERMS AND DEFINITIONS SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 1200 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. AQUIFER - A WATER BEARING FORMATION OR STRATA. OORLY GRADED) A<u>P-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONI CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANDULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND ANGULARITY OF GRAINS ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS. THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. WEATHERED SUBANGULAR, SUBROUNDED, OR ROUNDED. VERY STIFF, GRAY, SILTY CLAY, WOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL ROCK (WR) MINERALOGICAL COMPOSITION SOIL LEGEND AND AASHTO CLASSIFICATION FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. GROUND SURFACE. GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (≤ 35% PASSING *200 (> 35% PASSING *200) GNEISS, GABBRO, SCHIST, ETC. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED, ROCK TYPE COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY CRAVITY ON SLOPE OR AT BOTTOM A-1 A-3 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 A-6, A-7 CRUIP A-2 CLASS. -1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.

CDASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD. LIQUID LIMIT EQUAL TO 31-50 LIQUID LIMIT GREATER THAN 50 COASTAL PLAIN SEDIMENTARY ROCK CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SYMBOL UNSTAL PLAIN SELIMENTS CEMENTED INTO TOCK, BUT FIRST TO.

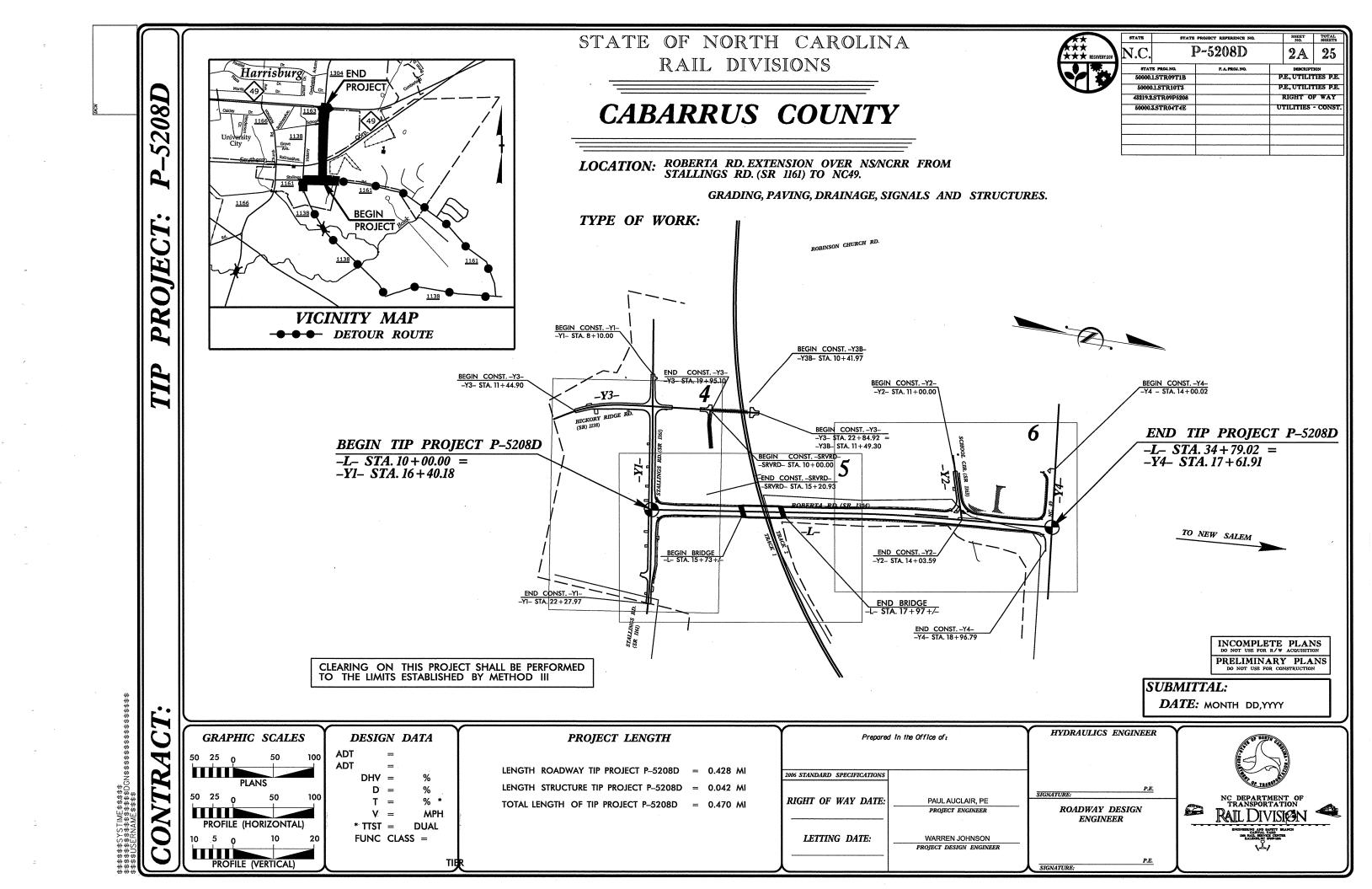
SPT. REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED

SHELL BEDS, ETC. PERCENTAGE OF MATERIAL PASSIN DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACEN SII T-WEATHERING MUCK. ROCKS OR CUTS MASSIVE ROCK. ORGANIC MATERIAL OTHER MATERIAL SOILS SOILS SOILS ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE SOILS FRESH * 200 RACE OF ORGANIC MATTER 2 - 37 3 - 5% TRACE LITTLE HAMMER IF CRYSTALLINE. 10 - 20% ונאז ו חזונמז ו MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 M ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, MODERATELY ORGANIC 5 - 10% 12 - 20% DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF LASTIC INDEX CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF √P | 19 MX | 19 MX | 11 MN | 11 MN | 10 MX | 19 MX | 11 MN | 11 MN HIGHLY ORGANIC V SLI.) LITTLE OR >20% 35% AND ABOVE MODERATE GROUP INDEX 0 4 MX 8 MX 12 MX 16 MX No MX GROUND WATE ORGANI FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO AMOUNTS OF SOILS SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE LISUAL TYPES STONE FRAGS. 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING SILTY OR CLAYEY ORGANIC (SLI.) FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MATTER GRAVEL AND SAND SOILS SOILS STATIC WATER LEVEL AFTER 24 HOURS MATERIALS SAND SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. GEN, RATING **∇**P₩ GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA (LCOM) EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABL DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. OW-SPRING OR SEEP PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30 ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH CONSISTENCY OR DENSENESS MISCELLANEOUS SYMBOLS FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN SEVERE AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. THE FIELD. COMPACTNESS OR TEST BORING ROADWAY EMBANKMENT (RE) PRIMARY SOIL TYPE DPT DMT TEST BORING PENETRATION RESISTENCE COMPRESSIVE STRENGTH (TONS/FT²) IF TESTED, WOULD YIELD SPT REFUSAL JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. W/ CORE WITH SOIL DESCRIPTION (N-VALUE) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED ROCK FARRIC CLEAR AND EVIDENT RUT REDUCED <u>LEDGE</u> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. VERY LOOSE AUGER BORING SPT N-VALUE SOIL SYMBOL 4 TO 10 MEDIUM DENSE 10 TO 30 LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. ARTIFICIAL FILL (AF) OTHER IF TESTED, YIELDS SPT N VALUES > 100 BPF MATERIAL CORE BORING (REF)- SPT REFUSAL MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN (NON-COHESIVE) THAN ROADWAY EMBANKMEN ERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT
Y SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK VERY DENSE >50 SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. MONITORING WELL INFERRED SOIL BOUNDARY VERY SOFT REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR <u>PERCHED WATER</u> - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN <0.25 VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF GENERALLY 0.25 TO 0.50 0.5 TO 1.0 INFERRED ROCK LINE Δ MEDIUM STIFF SILT-CLAY 4 TO 8 INSTALLATION ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE DNLY IN SMALL AND SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS COMPLETE RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. 8 TO 15 SLOPE INDICATOR INSTALLATION 1 TO 2 ALLUVIAL SOIL BOUNDARY ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND REPORTS OF THE PROPERTY OF THE ROCK OF THE RUN AND REPORTS OF THE ROCK OF THE VERY STIFF (COHESIVE) 15 TO 30 2 TO 4 DIP & DIP DIRECTION OF ROCK STRUCTURES ROCK HARDNESS EXPRESSED AS A PERCENTAGE. CONE PENETROMETER TEST TEXTURE OR GRAIN SI SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES VERY HARD SOUNDING ROD J.S. STD. SIEVE SIZE 40 0.42 60 0.25 200 0.075 SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND 4.76 DPENING (MM) 2.00 CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED ABBREVIATIONS RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL COARSE FINE TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. VST - VANE SHEAR TEST BOULDER COBBLE GRAVEL SILT AR - AUGER REFUSAL MED. - MEDIUM CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. (COB.) (SL.) (CL.) MODERATELY SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. (GR.) BT - BORING TERMINATED MICA. - MICACEOUS WEA. - WEATHERED (BLDR.) MOD. - MODERATELY Y - UNIT WEIGHT 0.05 GRAIN MM 305 75 2.0 0.25 0.005 7 DRY UNIT WEIGHT NP - NON PLASTIC STANDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPT) - NUMBER OF BLOWS (N OR BPF) OF SIZE IN. 12 CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CSE. - COARSE ORG. - ORGANIC MEDIUM A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS SOIL MOISTURE - CORRELATION OF TERMS DPT - DYNAMIC PENETRATION TEST SAP. - SAPROLITIC S - BILLY SOIL MOISTURE SCALE HAN 0.1 FOOT PER 60 BLOWS. VOID RATIO SS - SPLIT SPOOM CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS SOFT (ATTERBERG LIMITS) STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. - FINE SL. - SILT, SILTY ST - SHELBY TUBE - FOSSILIFEROUS USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY FRAC. - FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH (SAT.) FRAGS. - FRAGMENTS w - MOISTURE CONTENT CBR - CALIFORNIA BEARING OTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE LIQUID LIMI OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. RATIO HI. - HIGHLY V - VERY SEMISOLID: REQUIRES DRYING TO TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. RANGE - WET - (W) EQUIPMENT USED ON SUBJECT PROJECT FRACTURE SPACING ATTAIN OPTIMUM MOISTURE PLASTIC LIMIT TERM THICKNESS HAMMER TYPE: BENCH MARK: DRILL UNITS: VERY THICKLY BEDDED > 4 FEET VERY WIDE MORE THAN 10 FEET - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTUR AUTOMATIC MANUAL 1.5 - 4 FEET OPTIMUM MOISTURE THICKLY BEDDED CLAY BITS 3 TO 10 FEET MOBILE B-**ELEVATION:** THINLY BEDDED VERY THINLY BEDDED 0.16 - 1.5 FFFT L SHRINKAGE LIMIT MODERATELY CLOSE 1 TO 3 FEET 6 CONTINUOUS FLIGHT AUGER REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE CORE SIZE: THICKLY LAMINATED 0.008 - 0.03 FEET - DRY - (D) BK-51 VERY CLOSE LESS THAN 0.16 FEET < 0.008 FEET FIAD - FILLED-IN AFTER DRILLING 8" HOLLOW AUGERS -в___ INDURATION PLASTICIT HARD FACED FINGER BITS CME-45C _____ FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. PLASTICITY INDEX (PI) DRY STRENGTH TUNG.-CARBIDE INSERTS CME-550 NONPLASTIC 0-5 VERY LOW RUBBING WITH FINGER FREES NUMEROUS GRAINS: FRIABLE CASING W/ ADVANCER SLIGHT GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. 6-15 HAND TOOLS MED, PLASTICITY 16-25 MEDIUM PORTABLE HOIST TRICONE___ GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. POST HOLE DIGGER MODERATELY INDURATED TRICONE HAND AUGER ▼ TRI0055 CME-55 GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE: INDURATED SOUNDING ROD CORE BIT DIFFICULT TO BREAK WITH HAMMER. DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY) VANE SHEAR TEST TRI9435 CME-55 MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE: EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.

PROJECT REFERENCE NO.

P-5208D

SHEET NO.





Roadway Subsurface Investigation Report

Inventory

Roberta Rd (SR 1304) Grade Separation over NS/NCRR from NC-49 to Stallings Road (SR 1161) Cabarrus County, North Carolina

Prepared for:

Michael Baker Engineering, Inc. 8000 Regency Parkway, Suite 600 Cary, NC 27518

Submitted by:

Falcon Engineering, Inc. 1210 Trinity Road, Suite 110 Raleigh, North Carolina 27607 (919) 871-0800 www.falconengineers.com

Falcon Project Number | G11027.00

December 11, 2012

PREFACE

This roadway subsurface investigation was conducted in February 2012 in general accordance with our proposal number F2011-055, dated September 6, 2011. The recommendations provided in this report are based solely on our site reconnaissance, soil test borings and laboratory test data, engineering evaluation of these data, and generally accepted soil and foundation engineering practices and principles.

A total of fifteen (15) Standard Penetration Test (SPT) borings were drilled for the new roadway alignments. Additional borings were drilled for the bridge structure and are included in a separate Structure Subsurface Investigation Report. The end bent borings have been utilized in this report since they provide additional pertinent subsurface information relating to approach embankments. All borings were drilled using either a CME-55 all-terrain-vehicle (ATV) mounted drill rig, or a CME-55, truck-mounted drill rig, both equipped with 2 ¼-inch inside diameter hollow-stem augers and an automatic hammer. Representative soil samples, collected with a split-barrel sampler, were selected for laboratory testing to verify visual field classifications.

Falcon appreciates the opportunity to have provided our geotechnical engineering services for the above referenced project. If you have any questions concerning the contents of this report or need additional information, please do not hesitate to contact our office.

FALCON ENGINEERING, INC.

Report Prepared By:

Report Reviewed By:

Jeremy Hamm, El Geotechnical Designer Christopher V. Norville, PE Director of Geotechnical Services



WBS:

50000.1.STR09T1B

TIP:

P-5208D

COUNTY:

Cabarrus

DESCRIPTION:

Roberta Rd. Extension over NS/NCRR Stallings Rd. (SR 1161)

to NC 49

SUBJECT:

Roadway Subsurface Investigation – Inventory

PROJECT DESCRIPTION

The Roberta Road Extension Project consists of the following:

- Extension of Roberta Road from the existing intersection with NC 49 southward to Stallings Road in the vicinity of the existing intersection with Martin Street (gravel residential drive).
- A new four bent, three span bridge carrying the new Roberta Road alignment over Norfolk Southern (NS) / North Carolina Railroad (NCRR) corridor.
- Widening of NC 49 in the vicinity of the intersection with Roberta Road to accommodate a new right turn lane to Roberta Road.
- Realigning School Circle to intersect the new Roberta Road extension with turning lanes.
- Realigning Stallings Road and Hickory Ridge Road slightly to accommodate proposed elevated grades along the Roberta Road extension.
- Closing of the existing at-grade crossing along NS/NCRR at Hickory Ridge Road and Robinson Church Road.

Construction of the project will follow the construction of new rail bed and track paralleling the existing tracks to the north along the NS/NCRR Corridor. This work will require cuts on the order of 8-10 feet. Three bents of the proposed bridge will be located north of the existing railroad tracks and one to the south. The Roberta Road Extension will predominantly traverse an existing agricultural field north of the railroad. To the south of the railroad, the alignment will traverse an open grassy field and residential area (Martin Street). The entire Roberta Road extension will be constructed on fills ranging in height from a few feet to nearly 30 feet (in the vicinity of the end bent 1 approach). In order to accommodate the grades required to span over the railroad tracks, fills approaching 6 feet will be required at the intersection with Stallings Road where Roberta Road ends. The fills taper down in either direction along Stallings Road to meet existing grades.

The following alignments, totaling approximately 4,341 feet (.82 miles) were investigated. Subsurface profiles and cross sections of these alignments are included in this report.

Sheet 3a P-5208D

<u>Line</u>	<u>Station</u>
Roberta Road (-L-)	10+00 – 34+79
Stallings Road (-Y1-)	09+20 - 22+28
Hickory Ridge Road (-Y3-)	11+45 – 17+00

Subsurface profiles and cross sections showing the existing and proposed grades along Roberta Road, Stallings Road, and Hickory Ridge Road are included in this report on pages 10 through 17. Boring logs are included on pages 18 through 27.

AREAS OF SPECIAL GEOTECHNICAL INTEREST

The following areas contained topsoil and/or rootmat exceeding four (4) inches in thickness:

<u>Station</u>	<u>Offset</u>
-L- 15+72 (End Bent 1)	16 ft LT
-L- 15+83 (End Bent 1)	24 ft RT

Large rootballs and thick rootmat exceeding four inches should be expected in other areas throughout the site, particularly areas which are wooded or were minimally disturbed during previous clearing/grading operations. Additionally, stripping and grubbing within the agricultural lands may expose buried organic materials which will need to be removed prior to placement of fills.

The following areas contained wet soils near the ground surface which may also be encountered elsewhere.

<u>Station</u>	<u>Offset</u>
-L- 14+00	14 ft RT
-L- 18+09	38 ft RT
-L- 22+00	14 ft RT
-L- 28+00	2 ft RT
-L- 32+00	CL

The majority of the boring locations contained clayey soils with medium to high plasticity (A-7) near the ground surface. These soils degrade rapidly when exposed to water and may not adequately support construction equipment or fill placement.

The following areas contained medium to high plasticity soils near the ground surface which may also be encountered elsewhere.



Station	<u>Offset</u>
-Y1- 20+01	8 ft RT
-L- 12+00	14 ft RT
-L- 14+00	14 ft RT
-L- 15+72	2 ft LT
-L- 15+82	38 ft RT
-L- 17+96	2 ft LT
-L- 18+09	38 ft RT
-L-22+00	14 ft RT
-L- 24+00	12 ft RT
-L- 26+00	7 ft RT
-L- 28+00	2 ft RT
-L- 30+00	CL
-L- 32+00	CL

PHYSIOGRAPHY AND GEOLOGY

The project site is located in the Piedmont physiographic province of North Carolina.

Existing site topography is gently rolling, and the majority of the site consists of either agricultural fields or grassy areas, with narrow wooded corridors bordering the NS/NCRR corridor, and School Circle, and ditch traversing the agricultural field approximately perpendicular to the alignment.

Adjacent to NS/NCRR to the north, approximately 8 to 10 feet of cut will be performed required to accommodate rail corridor improvements including additional parallel railroad bed and track. We understand this work will performed separately prior to construction of the new roadway and grade separation.

Since the extension crosses primarily through agricultural lands, the presence of pockets containing organic soils should be anticipated during clearing and grubbing. In addition, the presence of loose/soft surficial soils should be anticipated in this area as a result of agricultural operations.

Steep sloped ditches, on the order of 5 to 10 feet deep, are present along existing right-of-ways (Hickory Ridge Road, Stallings Road, and School Circle, NS/NCRR) and bordering the agricultural field.

According to the **Geologic Map of North Carolina** (1985), the proposed site is located within the Charlotte Belt region of the Western Piedmont of North Carolina. Specifically,

Sheet 3b P-5208D

bedrock at the site is noted to consist of metamorphosed quartz diorite (PzZq), which is consistent with our findings.

SOIL PROPERTIES

In general, the subsurface soil conditions encountered across the site were relatively consistent; roadway embankment fills, residual soils underlain by and interlayered with weathered and crystalline metadiorite rock.

Based on the borings drilled on the existing roadway (R-1 through R-6), paved roadway sections consisted of 5.0 to 7.0 inches of bituminous concrete over 2.5 to 5.0 inches of aggregate base course.

Roadway Embankment soils encountered in borings R-1 and R-2 consisted of tan and brown, stiff, silts and clays (A-4, A-6) with gravel and little to trace amounts of organics.

Residual soils were encountered in all borings at the surface or below fills, consisting of brown, tan and green, medium stiff to hard, clays and silts (A-4, A-5, A-6, A-7), and loose to very dense sands (A-2-4), with rock fragments, weathered rock layers, and some with trace to little amounts of mica. The majority of residual soils were noted to be saprolitic. In general, the surficial clay layer throughout the project site is noted to be moderately to highly plastic clay (A-7) that could potentially be expansive.

Weathered rock was encountered in some borings underlying residual soils or as layers within residual soils and extending to boring termination or auger refusal depths. Weathered rock materials consist of diorite metamorphosed to varying degrees (Metadiorite). Auger refusal, indicating the potential presence of crystalline rock, was encountered in some borings at elevations ranging from approximately 550 to 597 feet, NAVD.

GROUNDWATER PROPERTIES

Groundwater levels were measured at the time of boring completion, and in most cases after at least 24 hours. Borings near or within existing roadways were backfilled immediately after completion due to safety considerations. Groundwater was observed directly in many borings, ranging in depth from approximately 5 to 27 feet below existing grades (elevations ranging from 575 to 604 feet, NAVD). Wet soils and cave-ins were observed in many borings as well, potentially indicating the present of groundwater near the caved-in elevation. Detailed groundwater measurement data are included in the boring logs.

In general, shallow groundwater, including potentially perched water and wet fine grained soils were encountered near the existing ground surface at both termini of the project. Overall, surficial site soils appear to exhibit poor permeability and slow infiltration of stormwater.

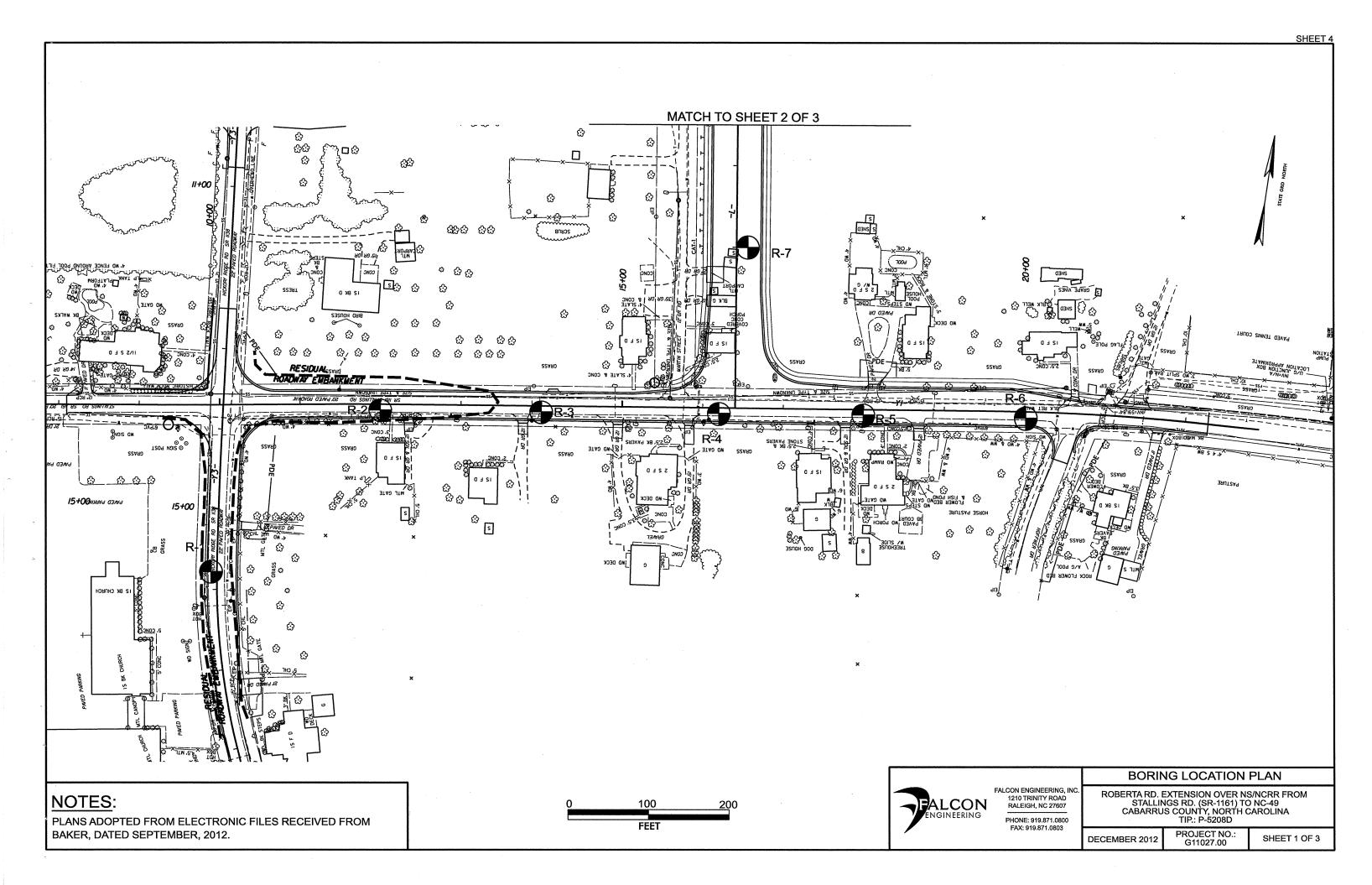


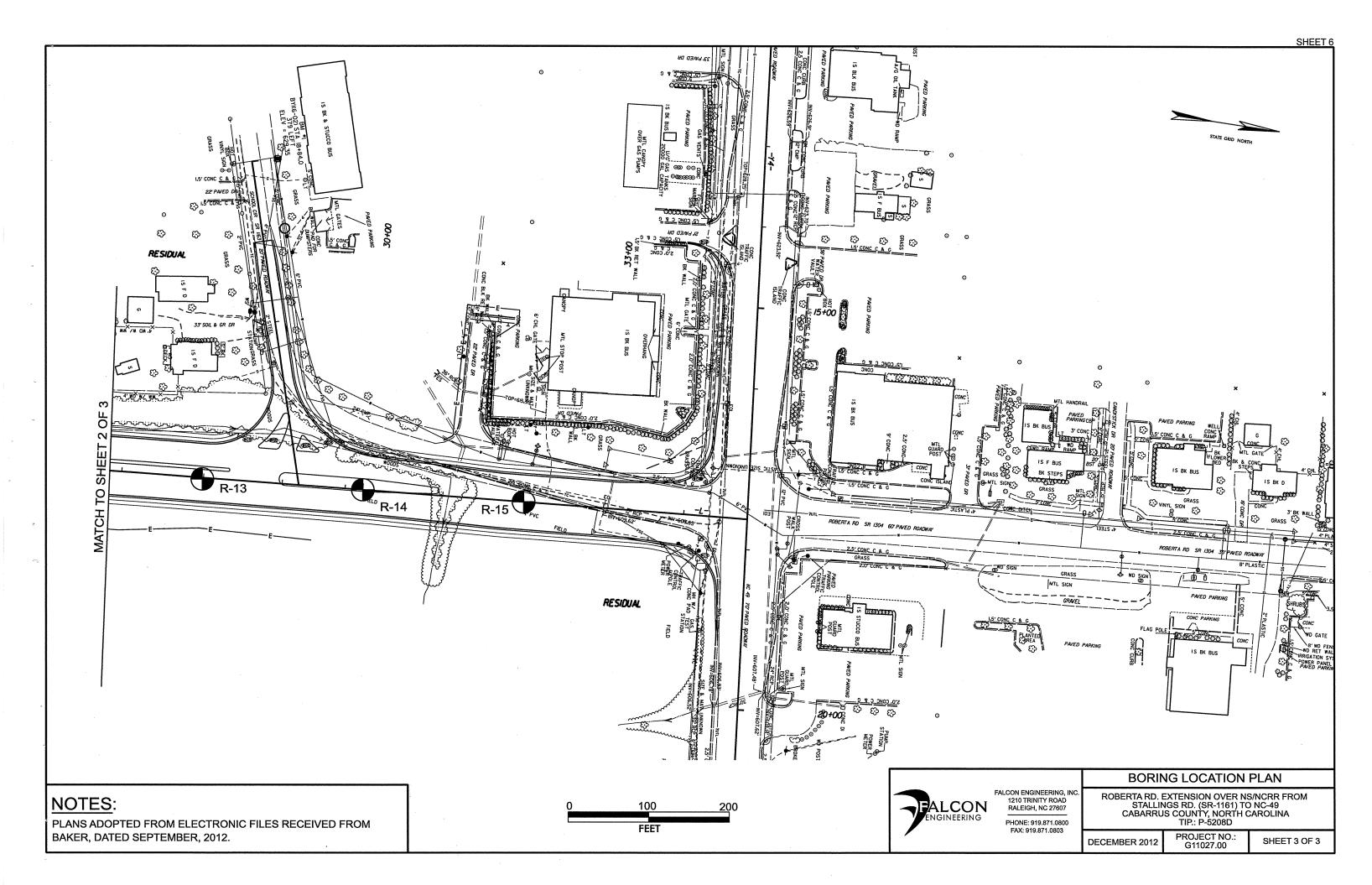
Volumes in Cubic Yards

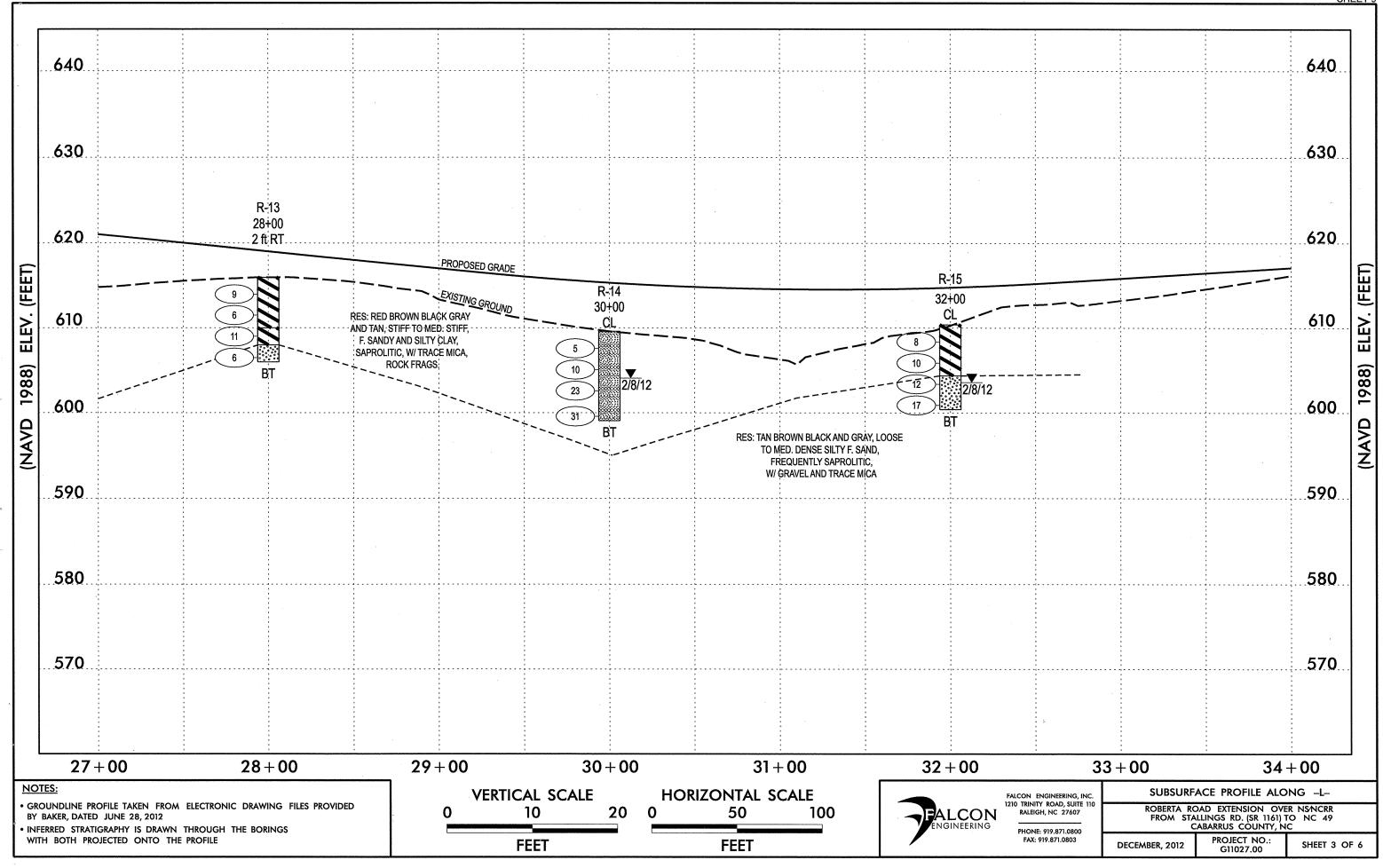
		EXCAVATION			EMBANKMENT				WASTE			
STATION	STATION	TOTAL	UNDERCUT	UNSUIT.	SUITABLE	TOTAL	EARTH	EMBANK.	BORROW	SUITABLE	UNSUIT.	TOTAL
		UNCLASS.		UNCLASS.	UNCLASS.			+20%				
L Sta. 10+50.00	L Sta. 15+69.41(Begin Bridge					50,490	50,490	60,588	60,588			
Y1 Sta. 8+10.00	Y1 Sta. 22+27.97	633	348		633	6,914	6,914	8,297	7,664		348	348
Y3 Sta. 11+45.00	Y3 Sta. 15+95.78	151			151	120	120	. 144		7		7
Y3 Sta. 16+50.00	Y3 Sta. 19+85.10	35			35	137	137	164	129			
-SRVRD- Sta. 10+25.00	-SRVRD- Sta. 12+37.36					203	203	244	244			
	SUBTOTAL # 1	819	348		819	57,864	57,864	69,437	68,625	7	348	355
L Sta. 17+99.41(End Bridge)	L Sta. 34+50.00	48	1,348		48	57,507	57,507	69,008	68,960		1,348	1,348
Y2 Sta. 11+00.00	Y2 Sta. 13+50.00	95	1,510		95	1,075	1,075	1,290	1,195			
Y3 Sta. 22+24.19	Y3 Sta. 22+86.37					-,-,-						
Y4 Sta 14+50.00	Y4 Sta. 17+08.33	240	224		240	11	11	13		227	224	451
	SUBTOTAL # 2	383	1,572		383	58,593	58,593	70,312	70,155	227	1,572	1,799
					- 4 0.75	1.127	1 127	1 264		53,311	1,460	54,771
*M1 Sta. 10409+19.40	M1 Sta. 10439+00.00	54,675	1,460		54,675	1,137	1,137	1,364		00,011	1,400	
	SUBTOTAL#3	54,675	1,460		54,675	1,137	1,137	1,364		53,311	1,460	54,771
*M1 Sta. 10439.+00.00	M1 Sta. 10440+00.00	1,135	75		1,135					1,135	75	1,210
	SUBTOTAL # 4	1,135	75		1,135					1,135	75	1,210
	TOTAL	57,012	3,455		57,012	117,594	117,594	141,113	138,780	54,680	3,455	58,135
*ADDITIONAL UNDERCU	7		1,965								1,965	1,965
SHOULDER MATERIAL	1					860	860	1,032	1,032			
BORROW TO REPLACE UN WASTE IN LIEU OF BORR						5,420	5,420	6,504	6,504 - 54,680	- 54,680		-54,680
PROJECT TOTAL	J. Ow	57,012	5,420		57,012	123,874	123,874	148,649	91,636		5,420	5,420
ECT 50/ TO BEDI ACE TO	D COULON BODBOW DIT								4,582			
EST. 5% TO REPLACE TOP	SOIL ON BORKOW FIT											
GRAND TOTAL		57,012	5,420						96,218			
SAY		57,100	5,500						96,300			·
		to a sale of the property of the second seco										
										,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		

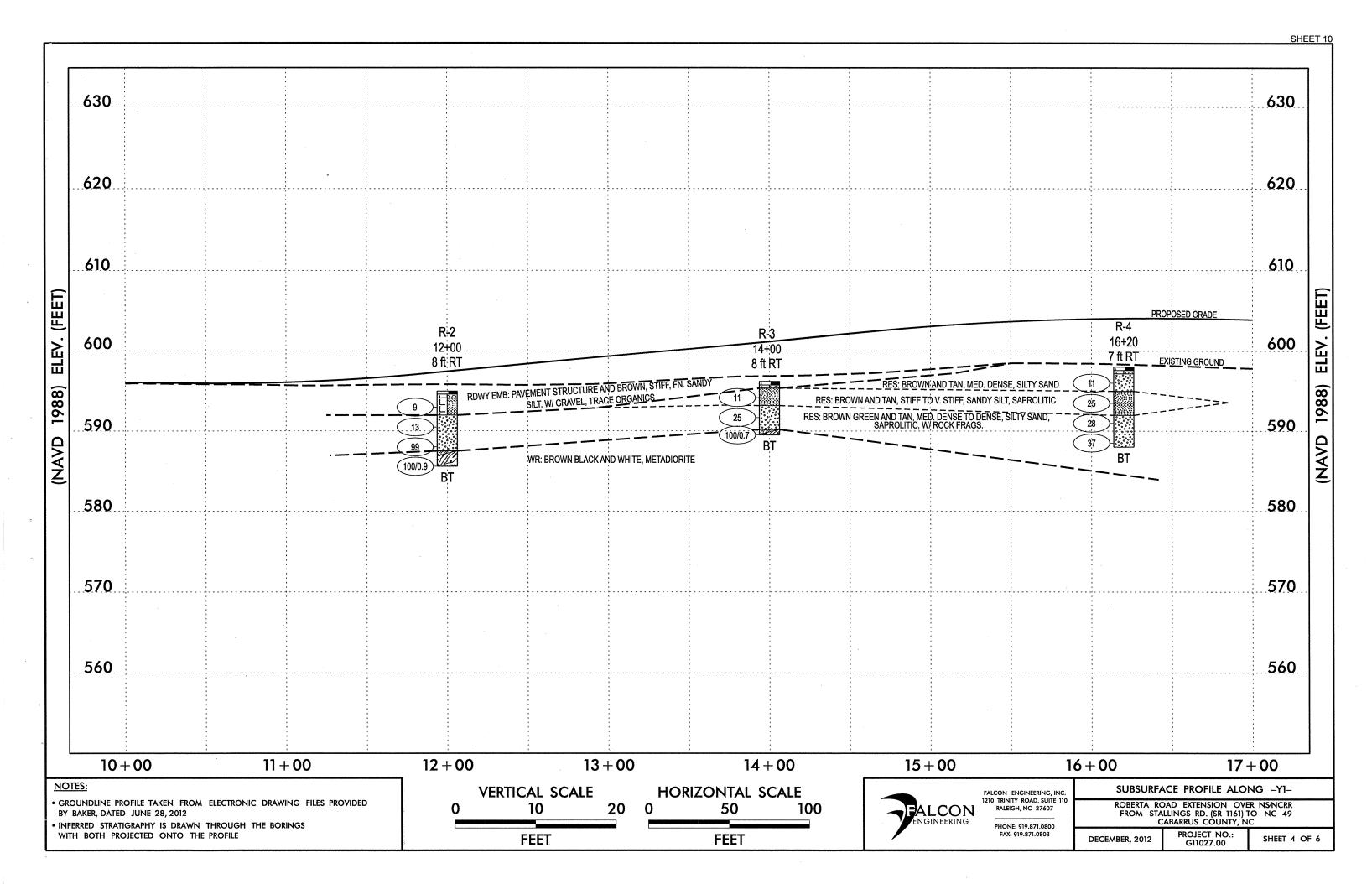
*DATA FROM ROADBED PLANS AND CONTINGENCY

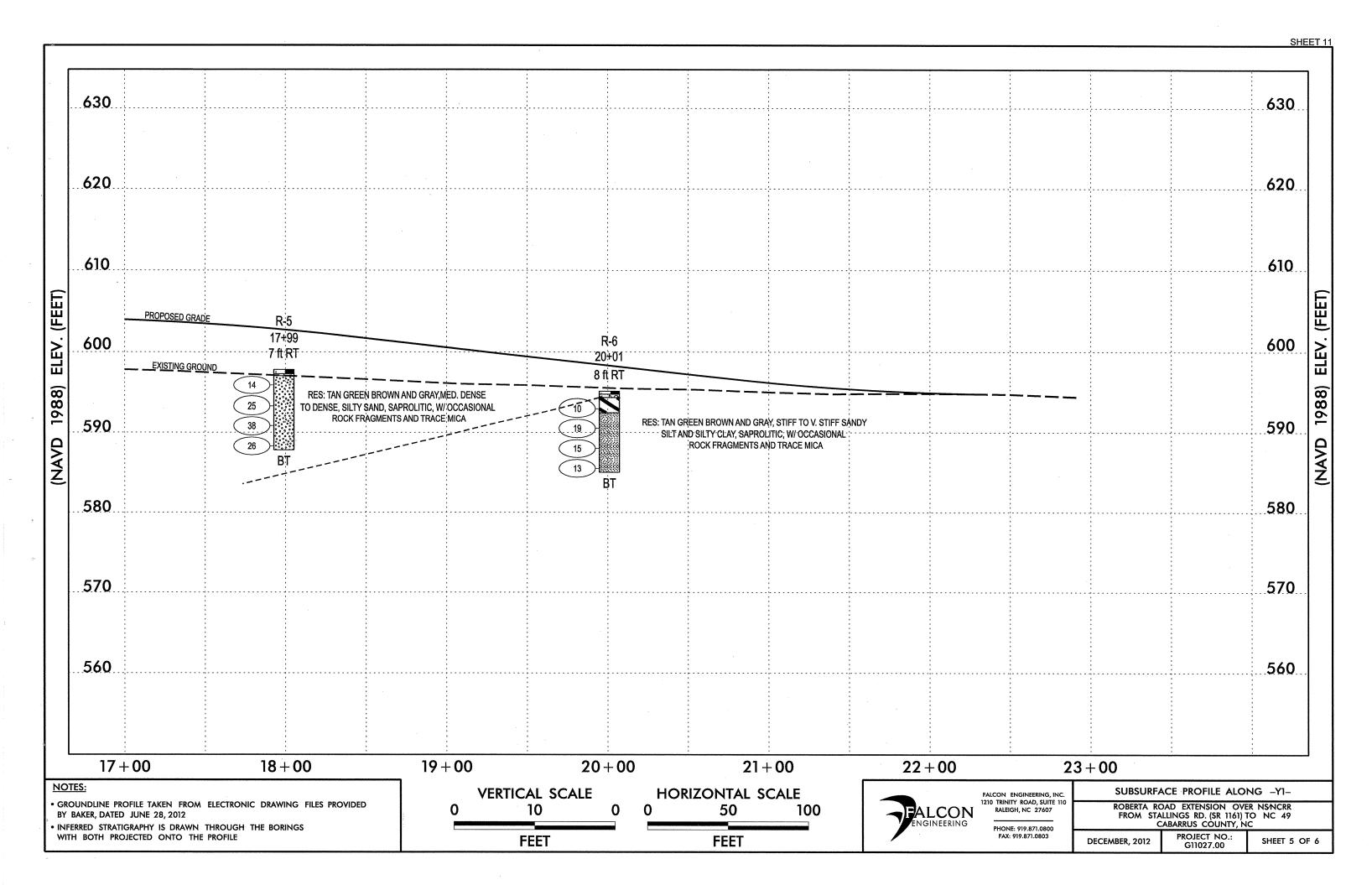
EST. DDE = 2340 CY
EST. SHALLOW UNDERCUT 1,750 CY
EST.SHALLOW UNDERCUT BY STATIONS 813 CY
TOTAL SHALLOW UNDERCUT 2563 CY
CLASS IV SUBGRADE STABILIZATION 200 TONS

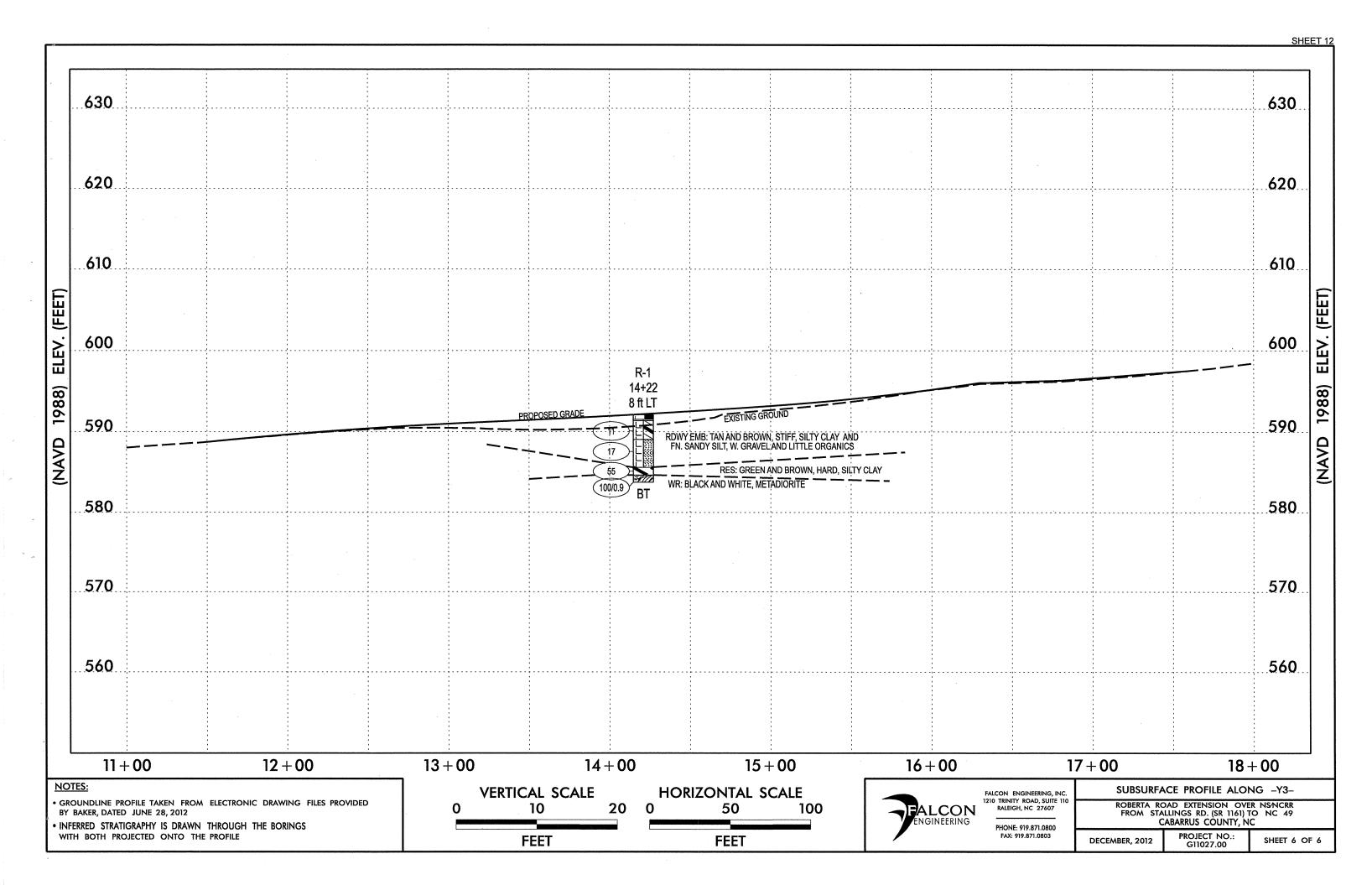




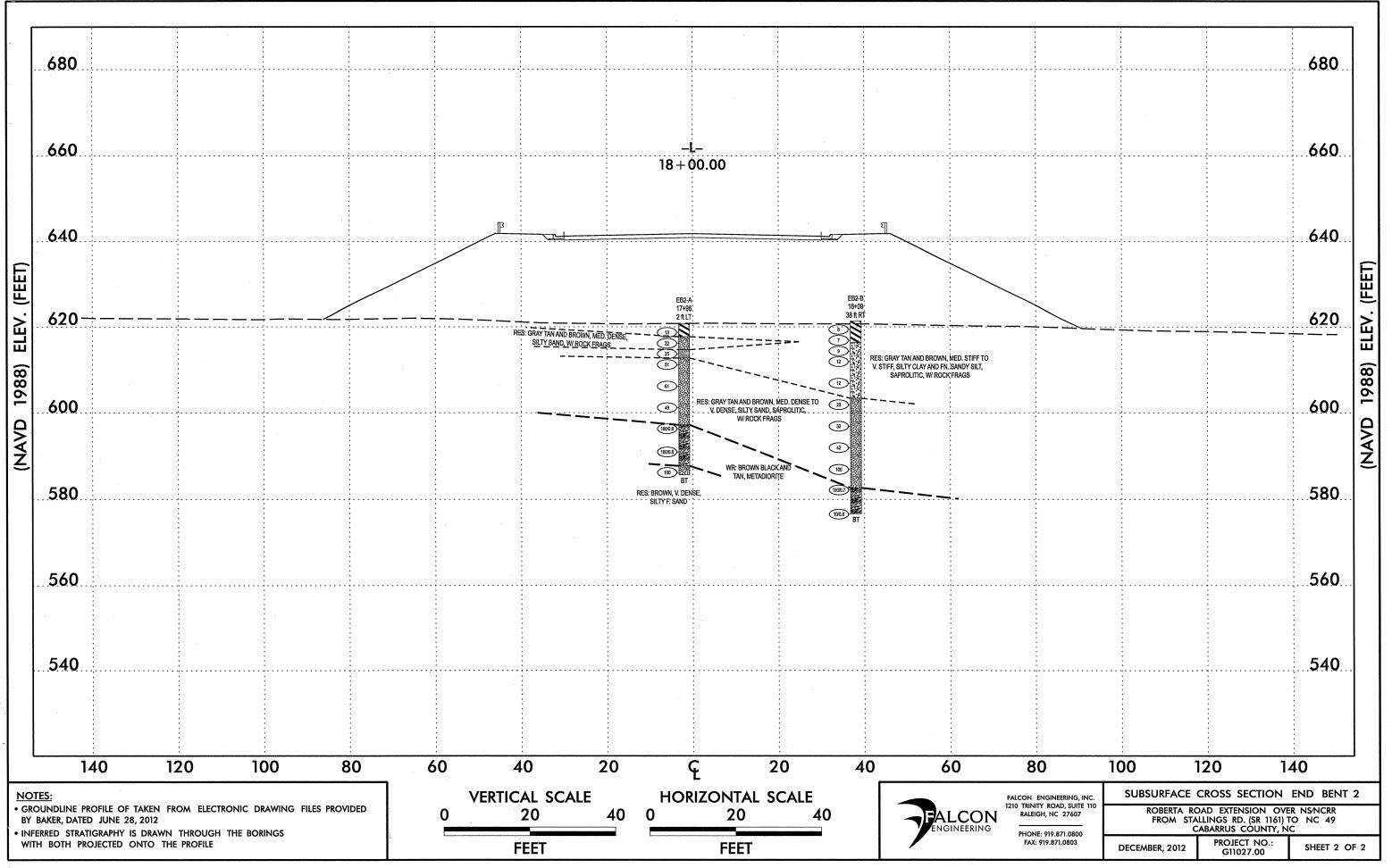


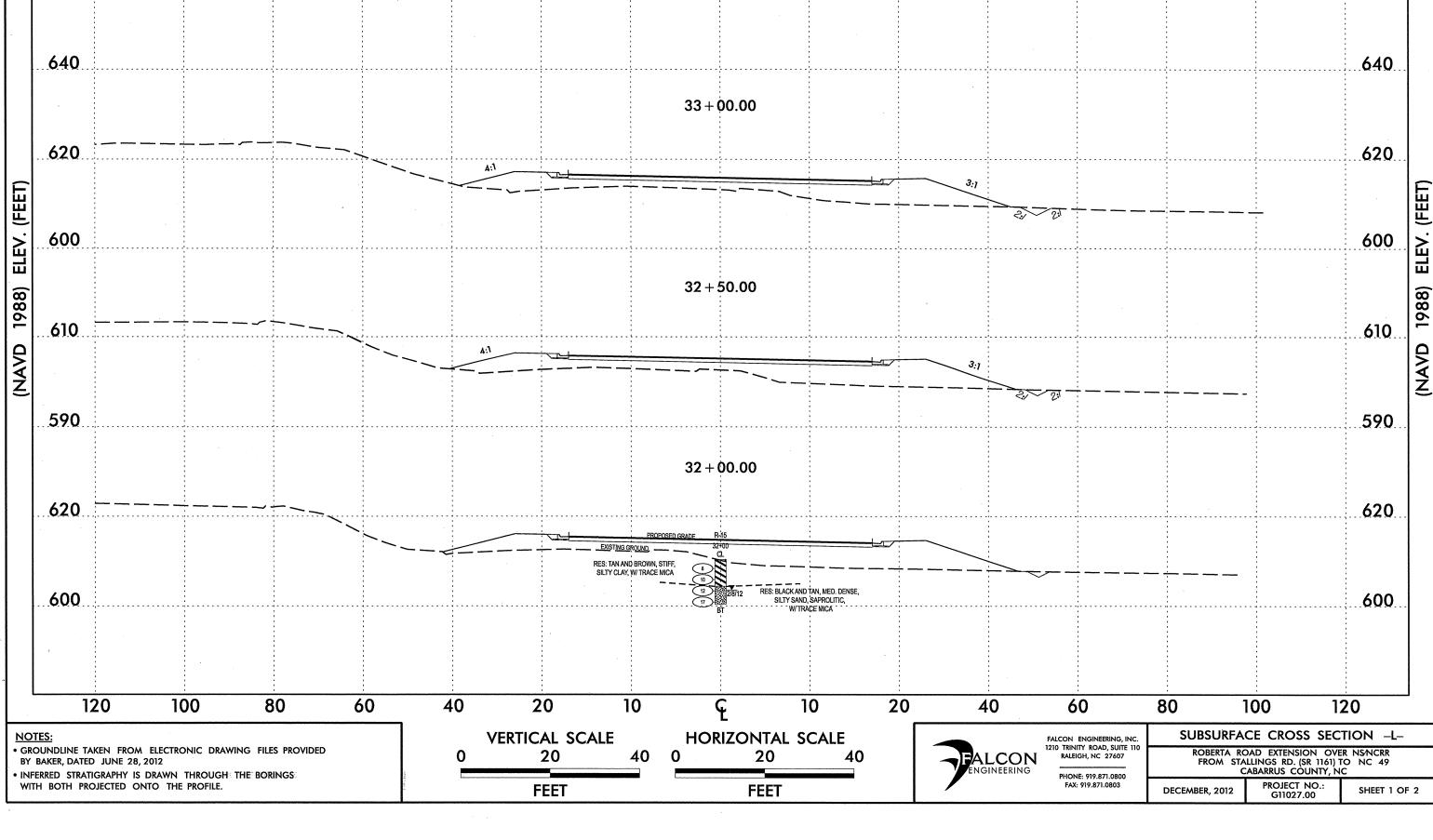


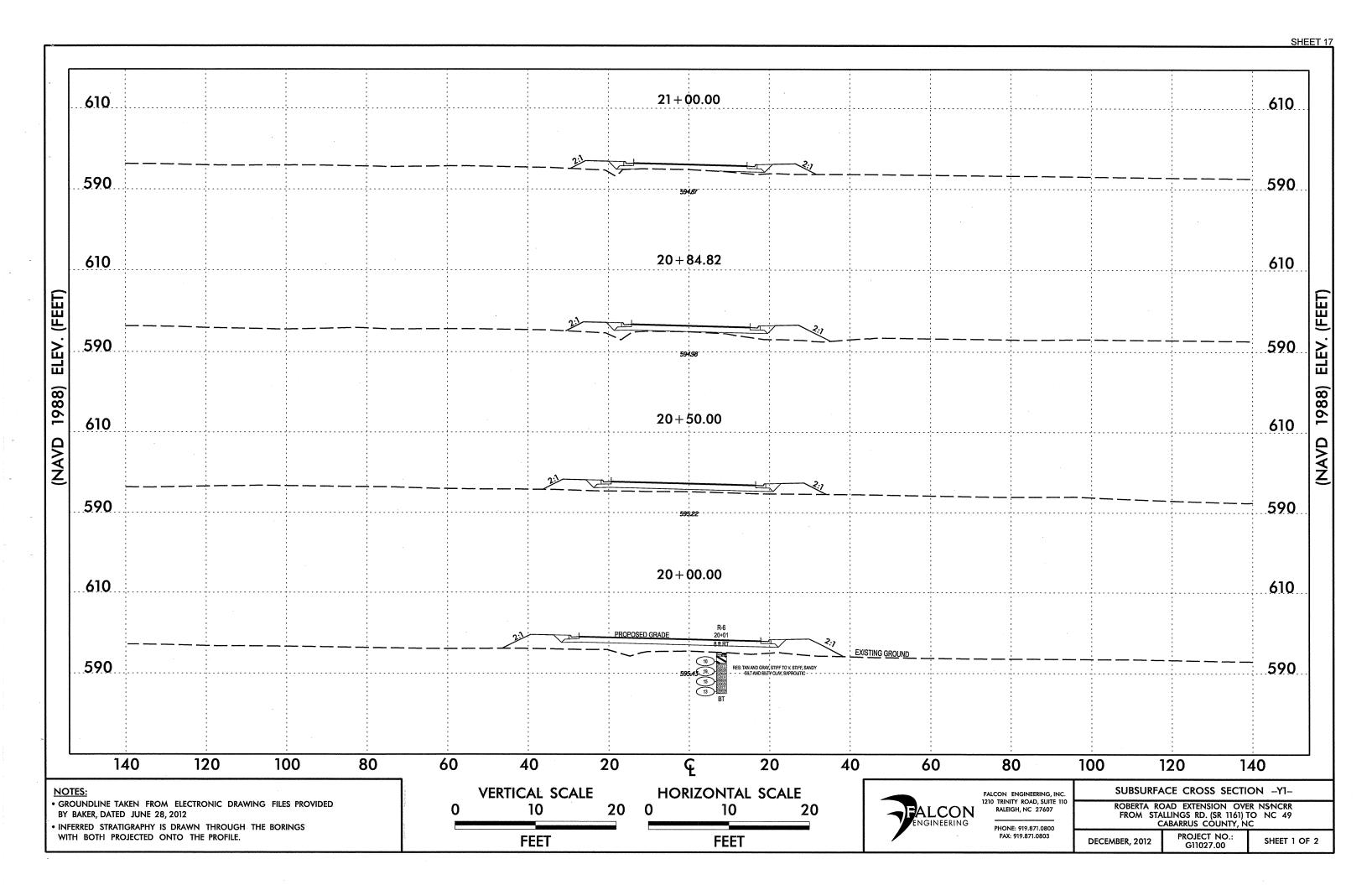


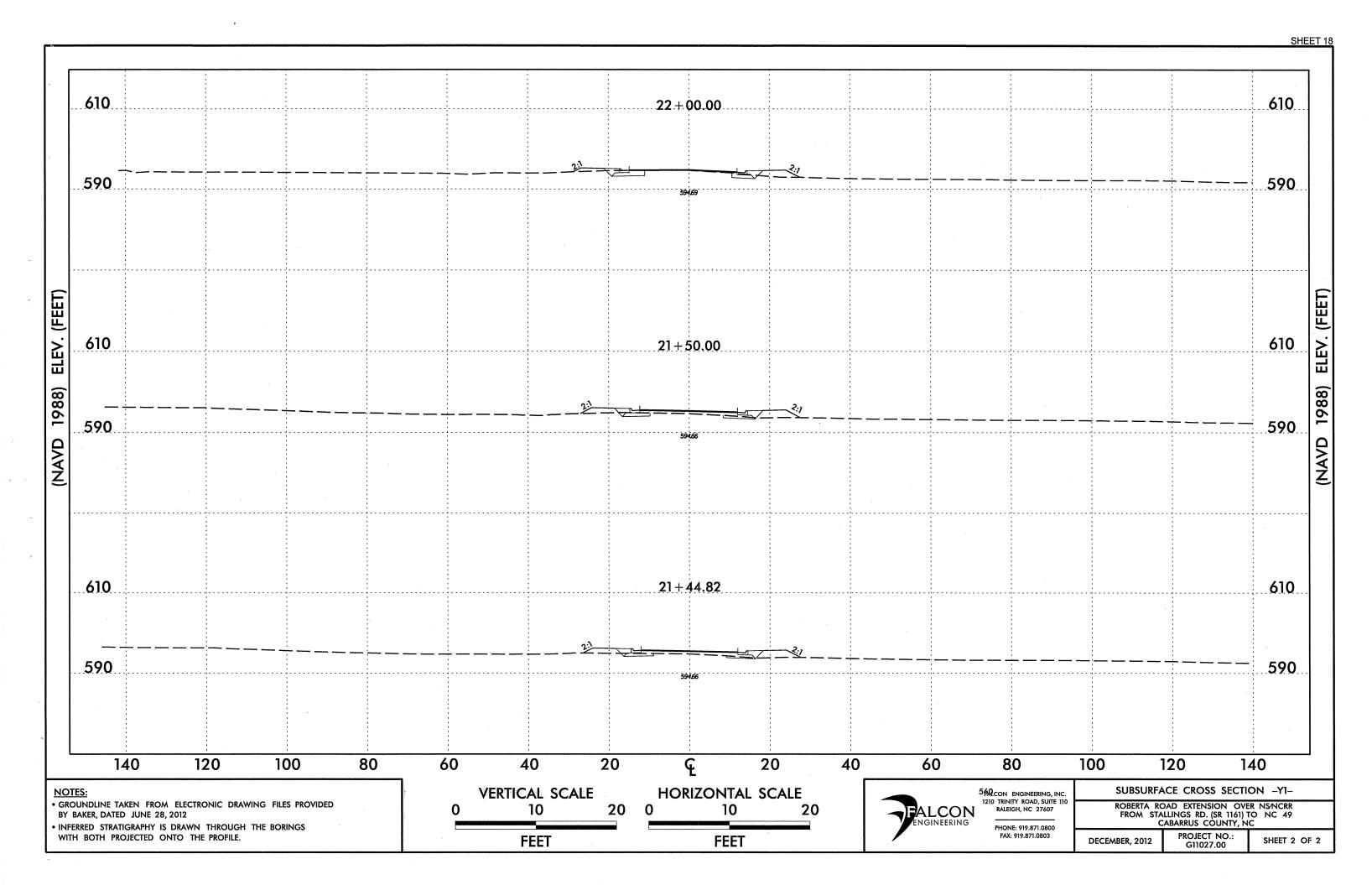


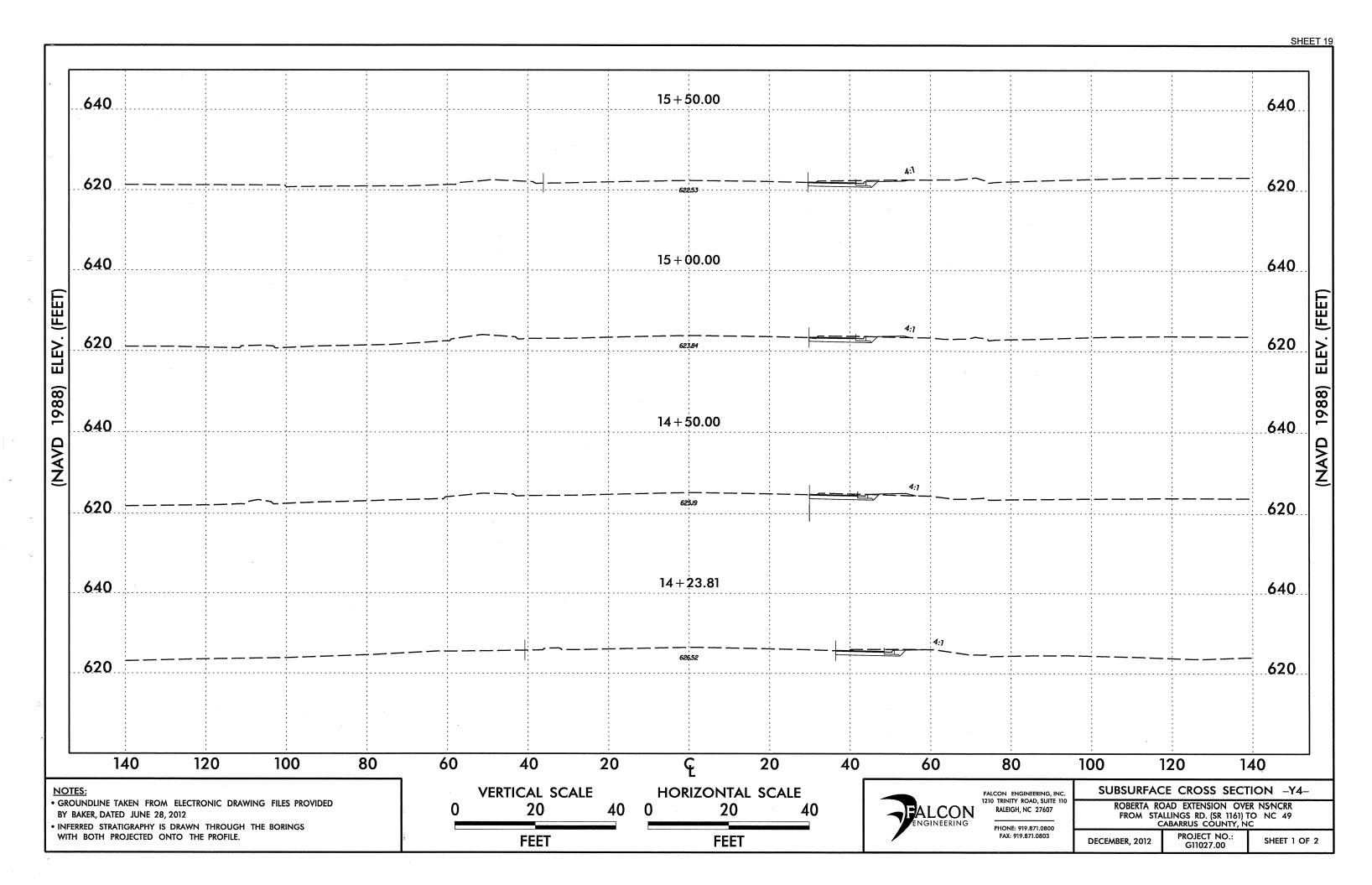


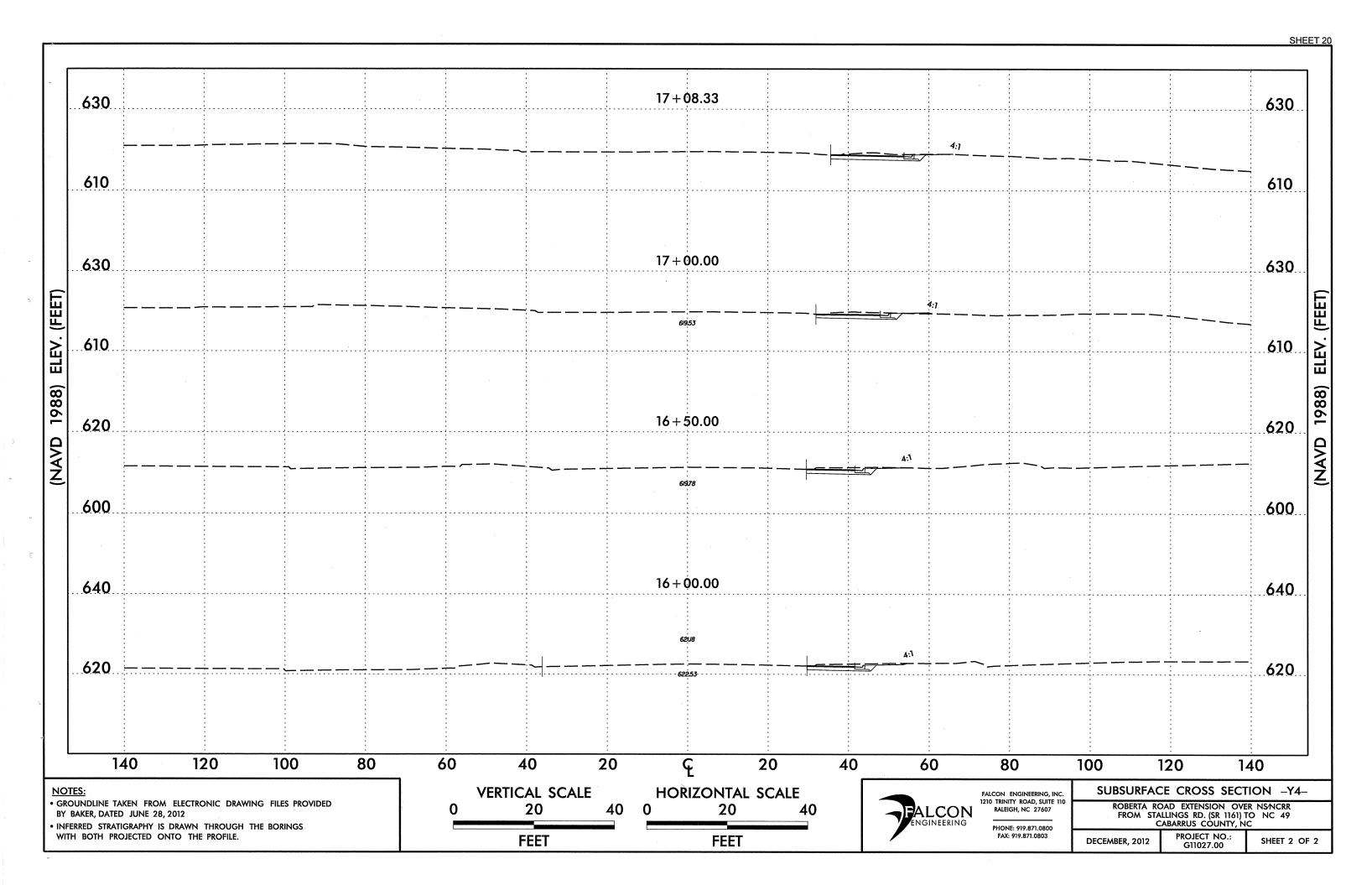












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SHEET 21
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FALCON

1210 TRINITY ROAD, SUITE 110, RALEIGH, NORTH CAROLINA 27607

AASHTO SOIL CLASSIFICATION AND GRADATION SHEET

ROBERTA RD. EXTENSION OVER NS/NCRR FROM

STALLINGS RD. (SR 1161) TO NC 49

WBS NO.: 50000.1.STR09T1B, TIP NO.: P-5208D

CABARRUS COUNTY, NORTH CAROLINA

FALCON	ENGIN	EERING	G. INC.	PROJEC	Γ NO: G11	027.00
			7			

			ENGINEE	KING, INC	C. PROJEC	I NO: G11	<u> 127.00</u>	************		
BOR	BORING SAMPLE		TOTAL SAMPLE			Atterberg Limit Test Results			Natural Moisture	
AAS	HTO Classifica	ation	PE	RCENT PAS	SING	Auerbe	Autoreig Liniit 1651 Nesults		Content	
STATION	OFFSET (FEET)	DEPTH (FEET)	#10	#40	#200	ĿĽ	PL	PI	%	
R-(SS-1								
	A-4	γ	98	88	59	39	31	8	25.0	
-Y1- 20+01	8 ft RT	6.0 - 7.5					<u> </u>			
R-		SS-2								
1	A-4		95	75	37	34	28	6	21.6	
-L- 12+00	14 ft RT	3.5 - 5.0					<u> </u>			
R-	A-4	SS-3	99		44	20		ND	40.0	
1 42:00		0.5.400	33	88	44	26	0	NP	18.9	
-L- 12+00 R-I	14 ft RT	8.5 - 10.0 SS-4		 						
17-0	A-7-6	33-4	96	87	72	74	27	47	20.0	
-L- 14+00	14 ft RT	1.0 - 2.5	30	01	12	/4	27	47	30.9	
R-1		SS-5			 					
	A-7-5	1 00-0	96	80	42	44	33	-11	27.1	
-L- 14+00	14 ft RT	6.0 - 7.5]	1			"	''		
R-1		SS-6								
	A-2-4		96	79	33	30	0	NP	20.0	
-L- 14+00	14 ft RT	13.5 - 15.0								
R-	9	SS-7								
	A-6	•	99	91	49	33	21	12	23.2	
-L- 20+00	14 ft RT	1.0 - 2.5								
R-1	0 .	SS-8								
	A-7-5		100	99	88	85	40	45	49.4	
-L- 22+00	14 ft RT	1.0 - 2.5								
R-1	0	SS-9	100							
	A-7-5			94	64	53	32	21	43.6	
-L- 22+00	14 ft RT	6.0 - 7.5			<u> </u>					
R-1		SS-10								
	A-2-4	T	100 84	35	31	0	NP	20.5		
-L- 22+00	14 ft RT	13.5 - 15.0		<u> </u>						
R-1	1 A-6	SS-11	۸,	0.5	40					
1 24:00		1 05 50	95	85	46	35	21	14	21.9	
-L- 24+00 R-1	14 ft RT	3.5 - 5.0		ļ			<u> </u>			
K-1	A-4	SS-12	100	90	00 50	,,	١ .		20.4	
-L- 26+00	14 ft RT	6.0 - 7.5	100	90	50	37	0	NP	32.4	
R-1		SS-13					<u> </u>			
	A-4	1	100	91	37	26	0	ND	24.1	
-L- 26+00	14 ft RT	13.5 - 15.0	100	"	"	20	"	NP	24.1	
R-1		SS-14		 			 			
	A-7-5	<u> </u>	100	97	76	69	41	27	47.8	
-L- 28+00	2 ft RT	1.0 - 2.5								
R-1	3	SS-15								
	A-7-6		77	66	40	43	29	14	27.6	
-L- 28+00	2 ft RT	6.0 - 7.5			<u> </u>					
R-14 SS-16										
	A-4		76	59	44	26	18	8	18.6	
-L- 30+00	CL	1.0 - 2.5								
R-1		SS-17								
	A-7-5	T	92	87	81	89	31	58	37.8	
-L- 32+00	CL	1.0 - 2.5			<u> </u>		<u> </u>			
R-1		SS-18					_			
1 00 55 T	A-2-4	T	93	56	29	42	0	NP	32.9	
-L- 32+00	CL	6.0 - 7.5	L	L			L	L		