NOTE: SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

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<u>LINE</u>

4901

STATION 16+00 - 32+00

<u>PLAN</u> <u>PROFILE</u> <u>XSECT</u> 9-II 12-I5

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RAIL DIVISION
GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. C-4901C

F.A. PROJ._

COUNTY **DAVIDSON**

PROJECT DESCRIPTION TURNER ROAD (SR 2005) GRADE

SEPARATION OVER NORFOLK SOUTHERN RAILROAD

"BOWERS TO LAKE"

INVENTORY



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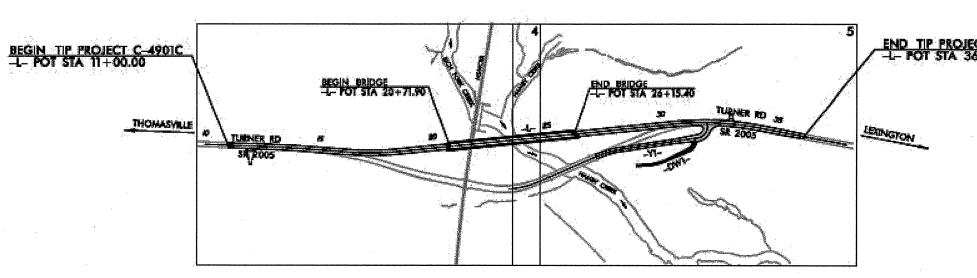
CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING, LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, CEOTECHNICAL ENDINERRING UNIT AT (1919 250-4088. NETHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOOS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORNIOS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU ON-PLACEITEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS AND VARY CONSIDERABLY WITH THE ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT, THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS ANDE, OR PORTION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED, THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HINSELF AS TO CONDITIONS. TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OF FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.





PERSONNEL
C. V. NORVILLE

M. BAHIRADHAN

J. R. HAMM

T. E. EVANS

INVESTIGATED BY T. E. EVANS

CHECKED BY M. BAHIRADHAN

SUBMITTED BY_ FALCON

DATE APRIL 6, 2012

SEAL 036072

04/06/2012

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

	SOIL AND ROC	K LEGEND, TERM	s, symbols,	AND ABBREVI	IATIONS	
SOIL DESCRIPTION	GRADATION				DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AGSHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANDULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FR UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE POORLY GRADED GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MO ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TI SUBANGULAR, SUBROUNDED, OR ROUNDED.	DRE SIZES.	ROCK LINE INDICA SPT REFUSAL IS F IN NON-COASTAL P OF WEATHERED RO	ES THE LEVEL AT WHICH NON- ENETRATION BY A SPLIT SPOON LAIN MATERIAL. THE TRANSITI CK. RE TYPICALLY DIVIDED AS FOL	IT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. I SAMPLER EQUAL TO OR LESS THAN Ø.1 FOOT PER 60 BLOWS. ON BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE LOWS: ILAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100	ALLUYIUM (ALLUY.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA, ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
VERY STIFF, GRAY, SUTY CLAY, MOST WITH INTERBEDEDED FINE SAND LATERS, HIGHLY PLASTIC, A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION	N	ROCK (WR)	BLOWS PER FOO	DT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC. ARE US WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.		CRYSTALLINE ROCK (CR)	WOULD YIELD S GNEISS, GABBRO	E GRAIN IGNEOUS AND METAMORPHIC ROCK THAT PT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, , SCHIST, ETC. E GRAIN METAMORPHIC AND NON-COASTAL PLAIN	GROUND SURFACE. <u>CALCAREOUS (CALC.)</u> - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP (A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 A-6 A-7 A-1, A-2 A-4, A-5 A-6, A-7 A-1, A-1-b A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-4, A-5 A-6, A-7	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT L	LESS THAN 31	NON-CRYSTALLINE ROCK (NCR)	SEDIMENTARY R	OCK THAT WOULD YEILD SPT REFUSAL IF TESTED, ROCK TYPE	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 0000800000	MODERATELY COMPRESSIBLE LIQUID LIMIT E	EQUAL TO 31-50 GREATER THAN 50	COASTAL PLAIN SEDIMENTARY ROCK (CP)	COASTAL PLAIN	SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
Z PASSING SILT- GRANULAR CLAY	ORGANIC MATERIAL GRANULAR SILT - CLAY	OTHER MATERIAL		WE.	ATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
- 40 39 MX 550 MX 51 MN - 2000 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN	SOILS SOIL			RESH, CRYSTALS BRIGHT, FEW J R IF CRYSTALLINE.	OINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
CLOUID LIMIT	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME HIGHLY ORGANIC >10% >20% HIGH	20 - 35%	(V SLI.) CRYSTA		NED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	<u>DIP DIRECTION (DIP AZIMUTH) -</u> THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 8 8 4 MX 8 MX 12 MX 16 MX No MX MODERATE ORGAN USUAL TYPES STORE FRAGS. EINE SILTY CLAYEY ORGANIC ORGA	C GROUND WATER WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DR	Still INC	SLIGHT ROCK (ENERALLY FRESH, JOINTS STAIR	NED AND DISCOLORATION EXTENDS INTO ROCK UP TO AY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO DNE ANOTHER PARALLEL TO THE FRACTURE.
OSUBLITYES THE TRIBUS FINE SILTY OR CLAYEY SILTY CLAYEY ORGANIC OF MAJOR GRAYEL AND SAND GRAYEL AND SAND SOILS SOILS MATTER	STATIC WATER LEVEL AFTER 24 HOURS	iii iii	CRYST	ALS ARE DULL AND DISCOLORED	. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. DISCOLORATION AND WEATHERING EFFECTS. IN	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
GEN. RATING AS A EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITA	LE VPW PERCHED WATER, SATURATED ZONE, OR WATER BEARING	G STRATA	(MOD.) GRANIT	OID ROCKS, MOST FELDSPARS AF OUND UNDER HAMMER BLOWS AF	RE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS NO SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
SUBGRADE 10511	SPRING OR SEEP		1	RESH ROCK. ICK EXCEPT QUARTZ DISCOLORE	D OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM,
CONSISTENCY OR DENSENESS RANGE OF STANDARD RANGE OF UNCONFINED	MISCELLANEOUS SYMBOLS	TEGY DODING	(MOD. SEV.) AND CA	N BE EXCAVATED WITH A GEOLI	DW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH OGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (I-VALUE) (TONS/F12)	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION ROADWAY EMBANKMENT (RE) OPT DMT VST PMT VST PMT	TEST BORING W/ CORE	-	<u>TED, WOULD YIELD SPT REFUSAL</u> NCK EXCEPT QUARTZ DISCOLORE	D OR STAINED ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
GENERALLY VERY LOOSE 4 TO 10	SOIL SYMBOL AUGER BORING	SPT N-VALUE	(SEV.) IN STR		ANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
MATERIAL MEDIUM DENSE 10 TO 30	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY MONITORING WELL	REF — SPT REFUSAL	VERY SEVERE ALL RO (V SEV.) THE MAREMAIN	TED, YIELDS SPT N VALUES > 1 ICK EXCEPT QUARTZ DISCOLORE INSS IS EFFECTIVELY REDUCED TO ING. SAPROLITE IS AN EXAMPLE	00 BPF D OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK TO FROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTILED (MOT) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
GENERALLY SOFT 2 TO 4 0.25 TO 0.50	INFERRED ROCK LINE A PIEZOMETER INSTALLATION		ı		RIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL, AND	INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD 230 34	SLOPE INDICATOR INSTALLATION	₹	SCATTE		MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND
TEXTURE OR GRAIN SIZE	25/025 DIP & DIP DIRECTION OF ROCK STRUCTURES CONE PENETROME	TER TEST		ROCK	HARDNESS	EXPRESSED AS A PERCENTAGE.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	SOUNDING ROD			IT BE SCRATCHED BY KNIFE OR AL HARD BLOWS OF THE GEOLO	SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES GIST'S PICK.	SAPPOLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM)	ABBRE VIATIONS AR - AUGER REFUSAL MED MEDIUM	VST - VANE SHEAR TEST	TO DE	TACH HAND SPECIMEN.	CK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.) GRAIN MM 305 75 2.0 0.25 0.05 0.005	CL CLAY CPT - CONE PENETRATION TEST NP - NON PLASTIC	WEA WEATHERED 7 - UNIT WEIGHT 7 - DRY UNIT WEIGHT	HARD EXCAN		CK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE DLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE CHURC FOR FIFLD MOISTURE	CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC	SAMPLE ABBREVIATIONS S - BULK	HARD CAN E		ICHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
(ATTERBERG LIMITS) DESCRIPTION GOIDE FOR FIELD MOISTORE DESCRIPTION	0	SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK	FROM		BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PRESSURE.	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABL	5040 5040TURES 5040TURES 708 TOTOGUE DESUGAL	RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO	VERY CAN E	E CARVED WITH KNIFE. CAN BE IRE IN THICKNESS CAN BE BROK	EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH (EN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID: REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE PID ACTIC LIMIT PLASTIC LI	EQUIPMENT USED ON SUBJECT PR			JRE SPACING	BEDDING	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
PLL PLASTIC LIMIT	DRILL UNITS: ADVANCING TOOLS:	HAMMER TYPE:	TERM	SPACING MORE THAN 10 FEET	TERM THICKNESS VERY THICKLY BEDDED > 4 FEET	BENCH MARK:
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT	MOBILE B- LLAY BITS	X AUTOMATIC MANUAL	VERY WIDE WIDE MODERATELY CLO	3 TO 10 FEET SE 1 TO 3 FEET	THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET	ELEVATION: FT.
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	6 CONTINUOUS FLIGHT AUGER X 8 HOLLOW AUGERS	CORE SIZE:	CLOSE VERY CLOSE	0.16 TO 1 FEET LESS THAN 0.16 FEET	THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	NOTES: FIAD - FILLED-IN AFTER DRILLING
PLASTICITY	CME-45C HARD FACED FINGER BITS		FOR SEDIMENTARY PO		OURATION WING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	, ROCK OUTCROP
PLASTICITY INDEX (PI) DRY STRENGTH NONPLASTIC 0-5 VERY LOW	TUNGCARBIDE INSERTS X CME-55	н	FRIABLE	RUBBING	WITH FINGER FREES NUMEROUS GRAINS;	\
LOW PLASTICITY 6-15 SLIGHT MED. PLASTICITY 16-25 MEDIUM	CASING W/ ADVANCER PORTABLE HOIST X TRICONE 3 STEEL TEETH	HAND TOOLS:	1	co	BLOW BY HAMMER DISINTEGRATES SAMPLE. CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
HIGH PLASTICITY 26 OR MORE HIGH COLOR	TOTOGRAF . TIME CARD	POST HOLE DIGGER HAND AUGER	MODERATEL	BREAKS	EASILY WHEN HIT WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT	SOUNDING ROD	INDURATED		ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; ILT TO BREAK WITH HAMMER.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		VANE SHEAR TEST	EXTREMELY		HAMMER BLOWS REQUIRED TO BREAK SAMPLE; BREAKS ACROSS GRAINS.	

PROJECT REFERENCE NO. C-490IC

SHEET NO.

2

490I IE PR

STATE OF NORTH CAROLINA RAIL DIVISION

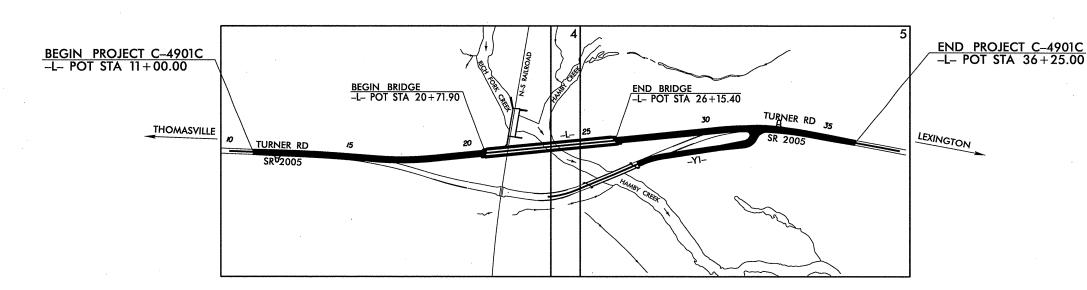
SHEET NO. 3 22 C-4901C STATE PROJ.NO. 49010.1.STR07T1B

DAVIDSON COUNTY

LOCATION: TURNER RD (SR 2005) GRADE SEPARATION OVER NORFOLK SOUTHERN RAILROAD "BOWERS TO LAKE"

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE





INCOMPLETE PLANS PRELIMINARY PLANS

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III THIS PROJECT IS NOT WITHIN ANY MUNICIPAL BOUNDARIES

SUBMITTAL: STRUCTURE RECOMMENDATIONS DATE: OCT. 14, 2011



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	PROF	ILE (H	ORIZONTA	AL)	
10	-	^	10	20	

DESIGN DATA

ADT 2013 = 1700 ADT 2033 = 2500DHV = 9%

PROP. DETOUR - - - -

D = 60%T = 15%*

V = 60 MPH* TTST = 2% DUAL 13% FUNC CLASS = LOCAL

NCDOT CONTACT: SUBREGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY STATE PROJECT C-4901C = 0.375 MILES LENGTH STRUCTURES STATE PROJECT C-4901C = 0.103 MILES

TOTAL LENGTH STATE PROJECT C-4901C = 0.478 MILES

SANDRA STEPNEY, PE

Prepared in the Office of: NC FIRM LICENSE No: F-0342 70|Corporate Center Drive, Suite 475 Raleigh, NC 27607 (919) 854-6200 - (919) 854-6259(FAX)

FOR THE NORTH CAROLINA DEPT. OF TRANSPORTATION 2012 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: APRIL 13, 2012

> CLAUDETTE M.K. ROQUE, PE LETTING DATE: APRIL 16, 2013

HYDRAULICS ENGINEER

ROADWAY DESIGN

ENGINEER



LEN HILL, PE PROJECT ENGINEER

Roadway Subsurface Investigation Report

Inventory

Turner Rd (SR 2005) Grade Separation over Norfolk Southern Railroad "Bowers to Lake" Davidson County, North Carolina

Prepared for:

AECOM 701 Corporate Center Drive, Suite 475 Raleigh, NC 27607

Submitted by:

Falcon Engineering, Inc. 1210 Trinity Road, Suite 110 Raleigh, North Carolina 27607 (919) 871-0800 www.falconengineers.com

Falcon Project Number | G11019.00

April 6, 2012

PREFACE

This roadway subsurface investigation was conducted between January 19, 2012 and March 9, 2012 in general accordance with our proposal number F2011-051, dated March 25, 2011. The recommendations rendered in this report are based solely on our site reconnaissance, performance of soil test borings, laboratory test results, engineering evaluation of the data gathered, and generally accepted soil and foundation engineering practices and principles.

Falcon appreciates the opportunity to have provided our geotechnical engineering services for the above referenced project. If you have any questions concerning the contents of this report or need additional information, please do not hesitate to contact our office.

FALCON ENGINEERING, INC.

Report Prepared By:

Report Reviewed By:

Jeremy Hamm, El

Geotechnical Designer

Mahalingam Bahiradhan, PE Senior Geotechnical Project Manager



WBS:

49010.1.STR07T1B

TIP:

C-4901C

COUNTY:

Davidson

DESCRIPTION:

Turner Rd (SR 2005) Grade Separation over

Norfolk Southern Railroad "Bowers to Lake"

SUBJECT:

Roadway Subsurface Investigation – Inventory

PROJECT DESCRIPTION

The project site is located adjacent the existing at-grade crossing between Turner Road (SR 2005) and Norfolk Southern Railroad (NSR) near Lexington, North Carolina. The project involves the realignment of Turner Road to the east and a new bridge structure crossing both Hamby Creek and NSR with six (6) spans. The new tracks will be constructed south of the existing tracks, along with a proposed parallel siding. The existing bridge structure and southern approach will remain in place to provide access to a private property west of Turner Road. The northern approach will be modified, and the existing at-grade crossing will be removed up to the northern intersection with the realigned roadway.

A total of six (6) Standard Penetration Test (SPT) borings were drilled for the new roadway alignment. Additional borings were drilled for the bridge structure and are included in the separate Structure Subsurface Investigation Report. The end bent borings have been utilized in this report since they provide additional pertinent subsurface information relating to approach embankments. All borings were drilled using a CME-55 all-terrain-vehicle (ATV) mounted drill rig, or a CME-850, rubber-tracked vehicle mounted drill rig equipped with 2 1/4-inch inside diameter hollow-stem augers and an automatic hammer. Representative soil samples, collected with a split-barrel sampler, were selected for laboratory testing to verify visual field classifications.

The following alignment, totaling approximately 1,600 feet (.30 miles) was investigated. Subsurface profiles and cross sections of these alignments are included in this report.

Line

Station

Turner Road (-L-)

16+00 - 32+00

SHEET 5

AREAS OF SPECIAL GEOTECHNICAL INTEREST

The following areas contained topsoil and/or rootmat exceeding four (4) inches in thickness:

<u>Station</u>	<u>Offset</u>
-L- 19+79	87 ft LT
-L- 20+68	17 ft RT
-L- 20+77	17 ft LT
-L- 27+87	35 ft LT

Large rootballs and thick rootmat exceeding four inches should be expected in other areas throughout the site, particularly areas which are wooded or were minimally disturbed during previous clearing/grading operations. Additionally, stripping and grubbing within the existing floodplain may expose buried organic materials which will need to be removed prior to placement of fills.

The following areas contained soft/loose soils near the ground surface which may not adequately support construction equipment:

Station	<u>Offset</u>
-L- 25+75	15 ft R1
-L- 27+87	35 ft LT

The following areas contained wet soils near the ground surface which may not adequately support construction equipment:

<u>Station</u>		<u>Offset</u>
-L- 20+68		17 ft LT
-L- 25+84		30 ft LT
-L- 19+61		42 ft RT

The following areas contained clayey/silty soils with medium to high plasticity (A-7) near the ground surface. These soils degrade rapidly when exposed to water and may not adequately support construction equipment or fill placement.

<u>Station</u>	<u>Offset</u>
-L- 20+68	17 ft LT
-I - 19+61	42 ft RT



Shallow groundwater (less than 6 feet from existing grades) was encountered at the following locations:

<u>Station</u> <u>Offset</u> -L- 25+84 30 ft LT

The new embankments will cross existing water and natural gas easements. Considerations concerning potential damage to the pipes as a result of settlement of the surrounding soils are contained within the Structure Subsurface Investigation Report submitted separately.

PHYSIOGRAPHY AND GEOLOGY

According to the **Geologic Map of North Carolina** (1985), the proposed site is located within the Charlotte Belt region of the western piedmont of North Carolina. Specifically, bedrock at the site is noted to consist of metamorphosed granitic rock **(CZg)**, consisting of megacrystic, well foliated rocks which locally contain hornblende.

Topographically, the site contains rolling natural hills representative of the western piedmont. Steep embankments are also present, associated with the existing bridge over Hamby Creek, existing railroad embankment, and the previous roadway embankment south of Hamby Creek. Total elevation change between the Hamby Creek floodplain and surrounding hills approaches 100 feet.

North of NSR, the end bent 1 approach is located partially within a wooded area. An existing overhead power and natural gas easement runs parallel to existing Turner Road and traverses the proposed alignment. The extremities of a previously clear-cut field lie just left of centerline in this area. This area slopes downward away from the alignment and a significant amount of fill will be placed within it. The fill slopes will come within approximately 27 feet of the NSR centerline near the western end of an existing concrete arch railroad bridge.

South of Hamby Creek, new roadway embankments will be constructed predominantly within a naturally rolling cattle field with a previously constructed, grass covered roadway embankment following roughly parallel to the existing Turner Road. This embankment approaches heights of 15 feet. New fills will be placed partially on top of this existing embankment. Underground water and natural gas lines appear to be installed along and/or within this embankment. The northeastern extremity of these fills will be placed within a small wooded area. Two small rock outcrops are located left of centerline, at approximately station 27+00, 135 and 170 feet left. Fills will not be placed within these areas. Surface drainage in the area is provided predominantly by naturally sloping ground and ditches along the existing roadway and railroad.

SHEET 6

SOIL PROPERTIES

In general, the subsurface soil conditions encountered across the site were relatively consistent; residual soils underlain by and interlayered with weathered metamorphosed granitic rock. South of Hamby Creek, alluvial soils were encountered overlying residual soils and/or weathered rock. Roadway embankment soils were encountered near ground surface on both sides of the project.

Roadway Embankment soils were encountered in all end bent 2 borings, as well as borings R-2 and R-4, and consisted of medium stiff to hard silts and clays (A-4, A-7-5) and very loose to medium dense silty sand (A-2-4) with gravel and sparse trace organics.

Alluvial soils were encountered in end bent 2 borings only, consisting of loose to medium dense sands with gravel (A-1-b, A-2-4) and soft to stiff, fine sandy silt (A-4).

Residual soils were encountered in most borings near the surface or beneath roadway embankment and alluvial soils, consisting of medium stiff to very hard, clays and silts (A-4, A-7), and medium dense to very dense, silty (A-2-4), with rock fragments, weathered rock layers, and varying amounts of mica. Some residual soils were noted to be saprolitic.

Weathered rock was encountered in most of the borings underlying residual soils or immediately beneath alluvial soils, and extending to boring termination or auger refusal depths. Weathered rock materials consist of metamorphosed granitic rock. Auger refusal, indicating the potential presence of crystalline rock, was encountered in some borings at elevations ranging from approximately 620 to 669 feet, NAVD.

Near surface soils north of Hamby Creek consist of predominantly fine-grained residual soils. South of Hamby Creek, these materials transition to sandy and silty alluvial soils upstation of end bent 2. Existing railway and roadway embankment soils are known to exist nearby within the project site, although they were not investigated within the scope of this report.

GROUNDWATER PROPERTIES

Groundwater levels were measured at the time of boring completion, and in most cases after at least 24 hours. Groundwater was encountered in boring R-3 and in most of the end bent borings, at elevations between 641 and 648 feet, NAVD. Specific groundwater information for each boring can be found in the boring logs in this report.

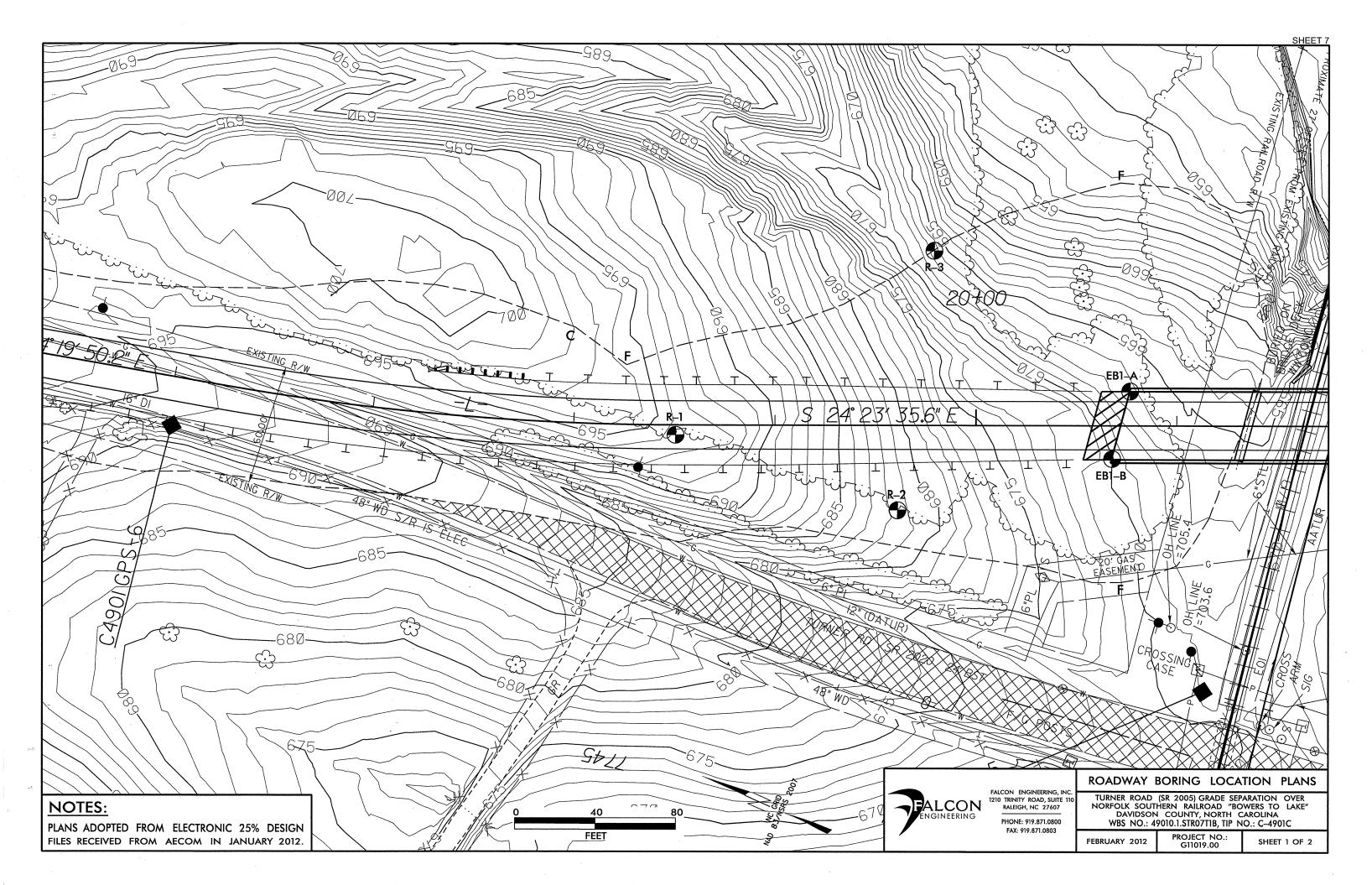
In general, the site topography allows for reasonable surface drainage, however, depressed areas within the floodplain of Hamby Creek drain poorly and appear to hold water for a significant portion of the year.

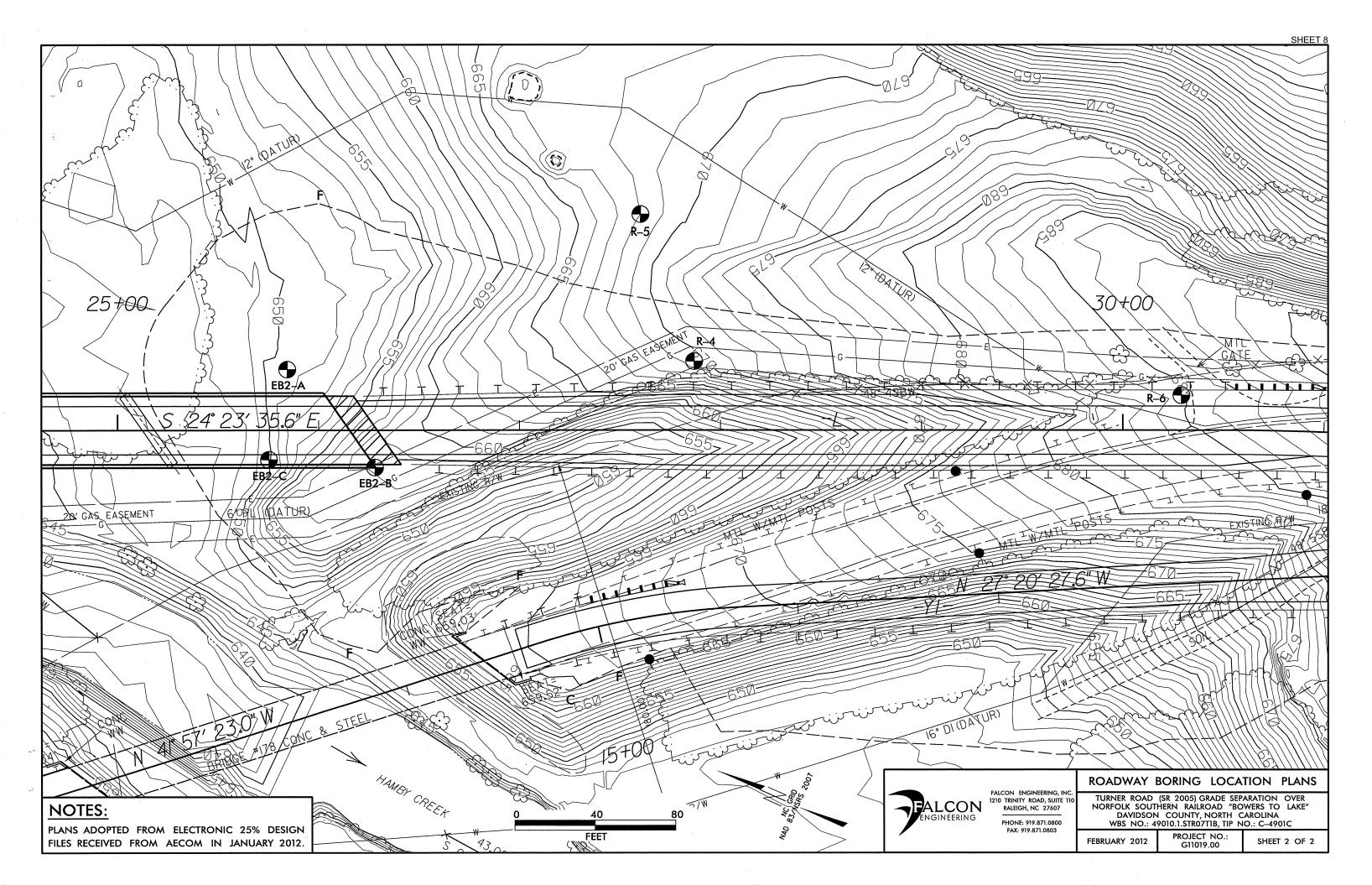


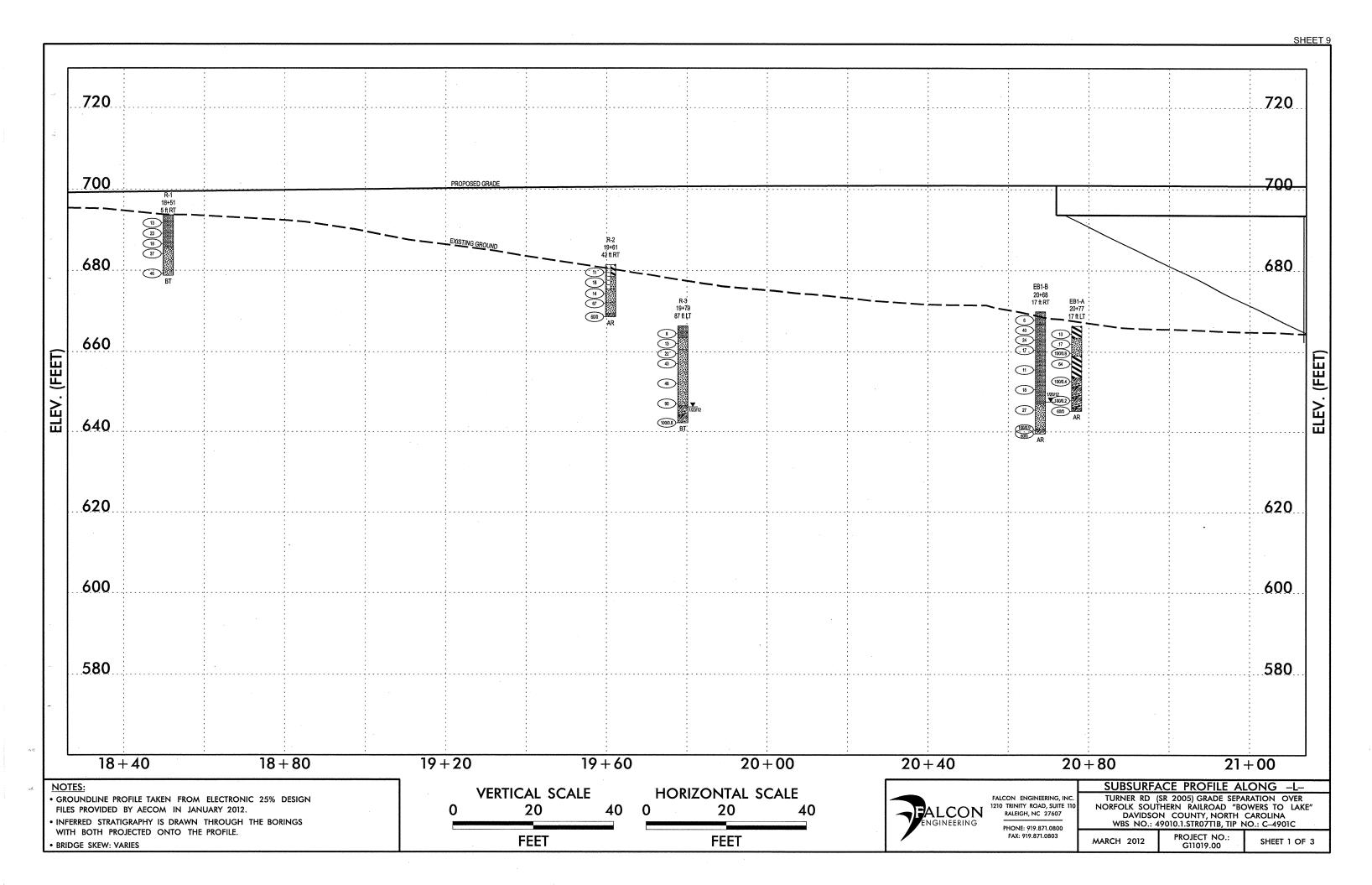
EARTHWORK BALANCE SHEET

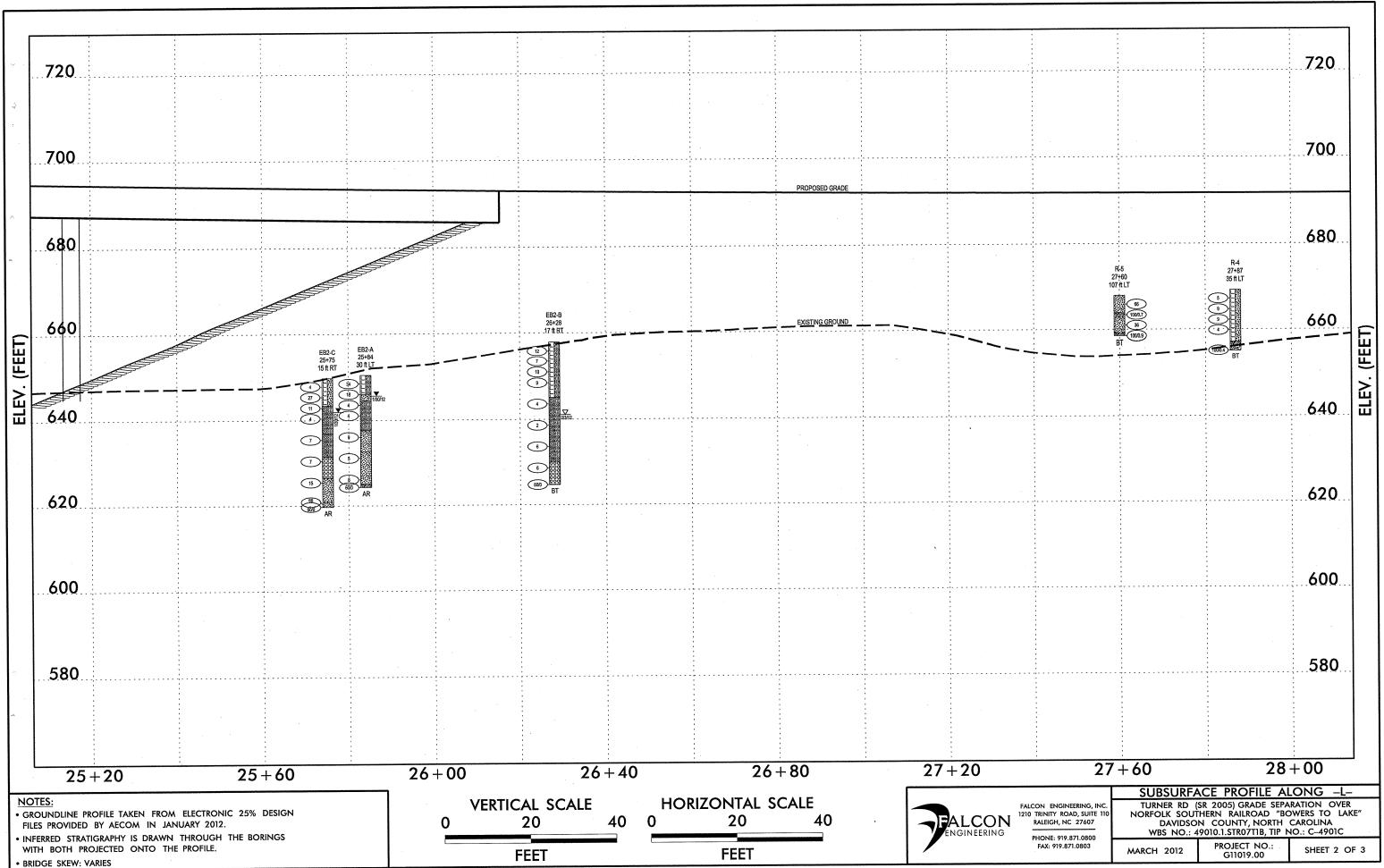
Volumes in Cubic Yards

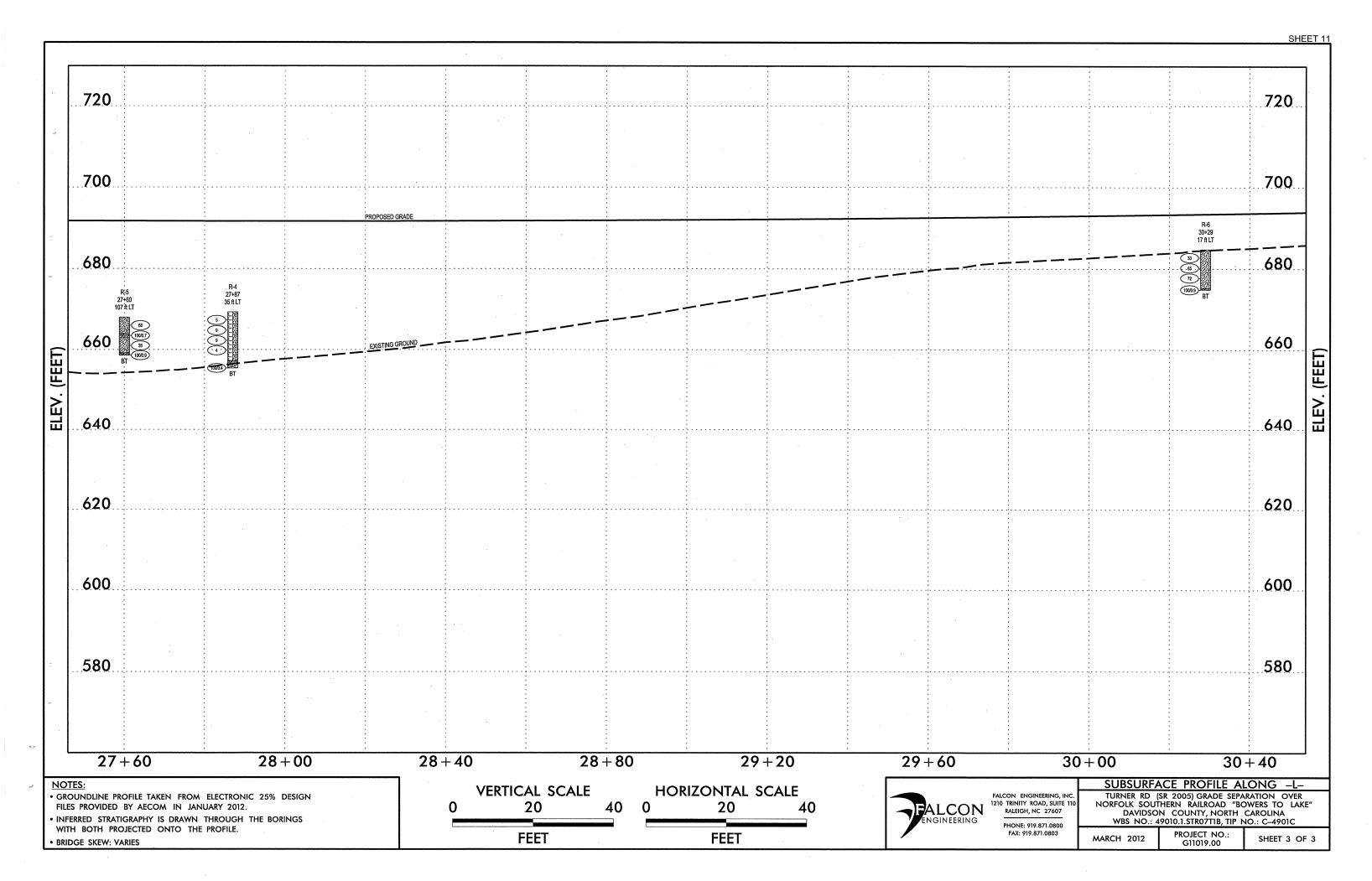
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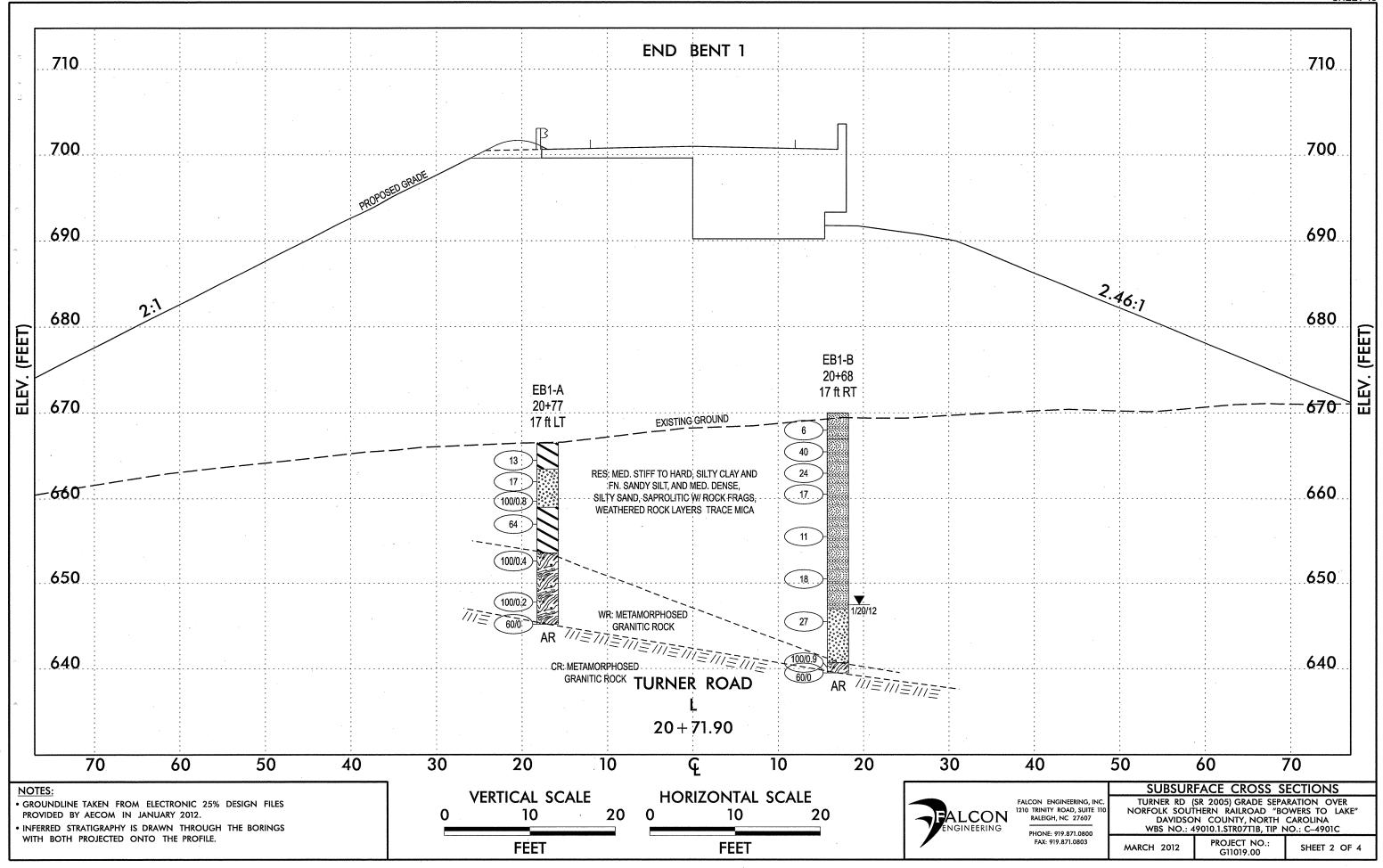


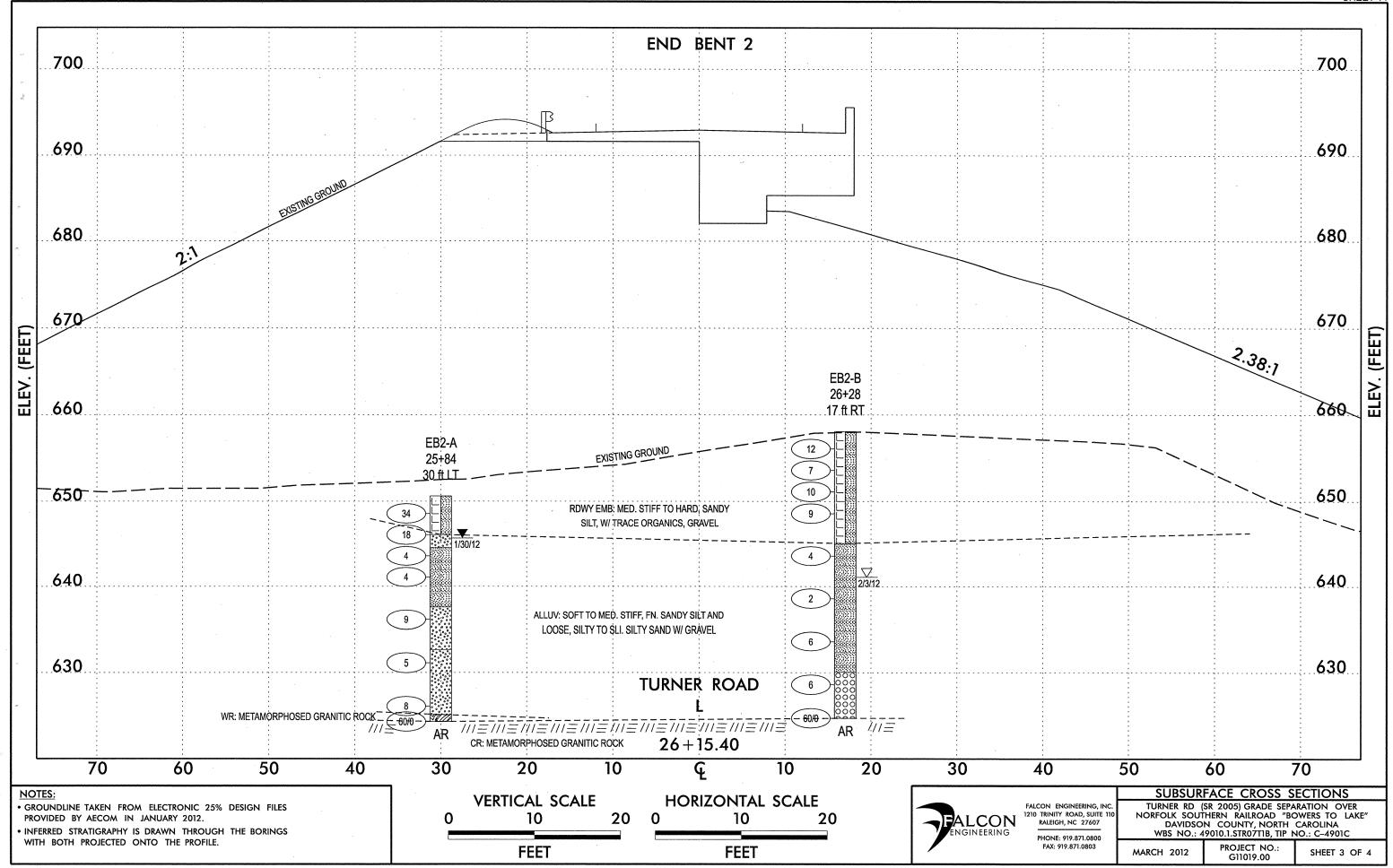


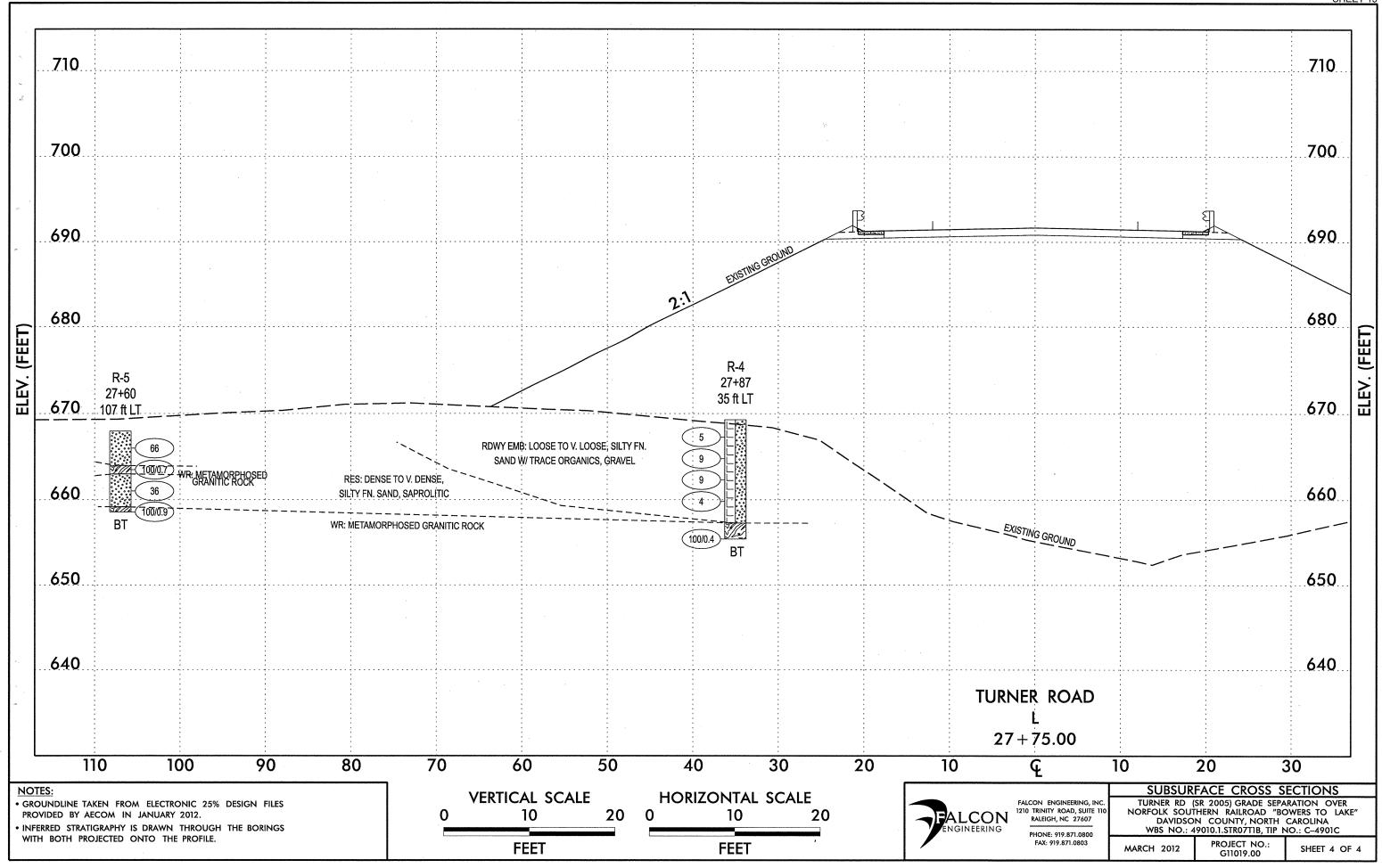












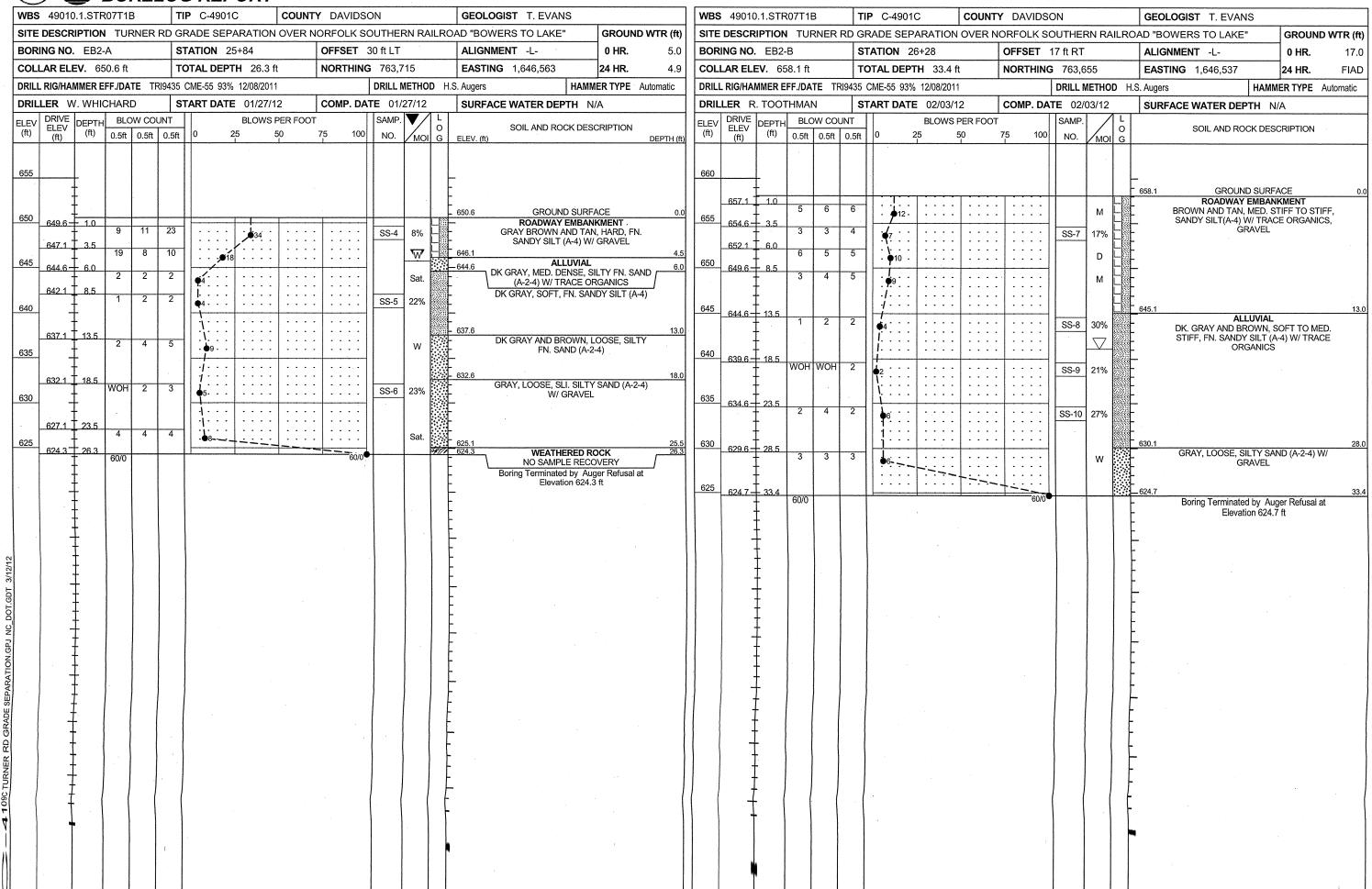
NCDOT GEOTECHNICAL ENGINEERING UNIT BORELOG REPORT

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NCDOT GEOTECHNICAL ENGINEERING UNIT BORELOG REPORT

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R W. WHICHARD START DATE 01/19/12 COMP. DATE 01/19/12 SURFACE WATER DPTH N/A SURFACE WATER DPTH N/A	LLAR ELEV. 666.5 ft	TOTAL DEPTH 21.3 ft	NORTHING 764,172	EASTING 1,646,341	24 HR . Dry	COLLAR ELEV. 67	'0.0 ft	TOTAL DEPTH 30.5 ft	NORTHIN	G 764,166	EASTING 1,646,307	24 HR. 2
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66.5 10 4 5 8 13	DRIVE DEPTH BLOW COU (ft) (ft) 0.5ft 0.5ft		-0 75 400	O SOIL AND ROCK DESC		1 (11)	0.5ft 0.5ft 0			/ 0	SOIL AND ROCK	DESCRIPTION
65.5 1.0 4 5 8 6 13.3												
83.0 3.5 5 7 10 80.5 8.0 28 72/0.3 83.0 3.5 5 7 10 80.5 8.0 8.5 9 30 34 83.0 3.5 600.2 83.0	005 5				「MAT 0.0	666.5 + 3.5					TAN AND BROWN, 667.0 SANDY SI	MED. STIFF, FN. LT (A-4)
TAN AND BROWN, MED, DENSE, SILTY SSO, 85, 0 85,	663.0 3.5	8	SS-1 24%	TAN AND BROWN, STIFF, (A-7-6) W/ ROCK FRAGS,	WEATHERED3.0	6640 7 60		13			 HARD, FN. SANDY SI 	LT (A-4) W/ ROCK
58.0 8.5 9 30 34 M TAN, V. HARD, SILTY CLAY (A-7) W GRAVEL LAYERS 53.0 13.5 100/0.2	660.5 = 6.0 28 72/0.3		†	TAN AND BROWN, MED. I SAND (A-2-4) W/ ROCI WEATHERED ROCK LAY	DENSE, SILTY K FRAGS,		6 8			М	- - - -	
53.0 13.5 100/0.4 100/0.4 100/0.4 100/0.4 100/0.4 100/0.4 100/0.4 100/0.4 100/0.4 100/0.4 100/0.4 100/0.2 100	658.0 8.5 9 30	34		TAN, V. HARD, SILTY CL		1 1 1000	3 4	::/:: :::: :		SS-3 25%	- - -	
48.0 18.5 100/0.2 100/	653.0 13.5		100/0.4	WEATHERED RO DARK GRAY AND V	OCK VHITE,	651.5 + 18.5		: : ; ; : : : :			- - -	
45.1 21.4 60/0 Boring Terminated by Auger Refusal at Elevation 645.2 ft 645.2 21.5 645.2 21.3 645.2 64	648.0 18.5			METAMORPHOSED (GRANITE	650	6 7	₹18			- - -	
Elevation 645.2 ft 641.5 28.5 21 42 58/0.4	645 1 7 21 4		· · · · · · · · <u> </u>	Boring Terminated by Aug	er Refusal at		9 11	16			GRAY TAN AND BLA SILTY FN. SAND (A-2-4	4) W/ TRACE MICA,
- 100/0.98	+			Elevation 645.2	ft · ·	640 +	21 42 58			D	- - 640.7	
	Ŧ			-		639.5 + 30.5	60/0		100/0.9 60/0	\$ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	BROWN GRAY METAMORPHOS	Y AND TAN, SED GRANITE
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Dry

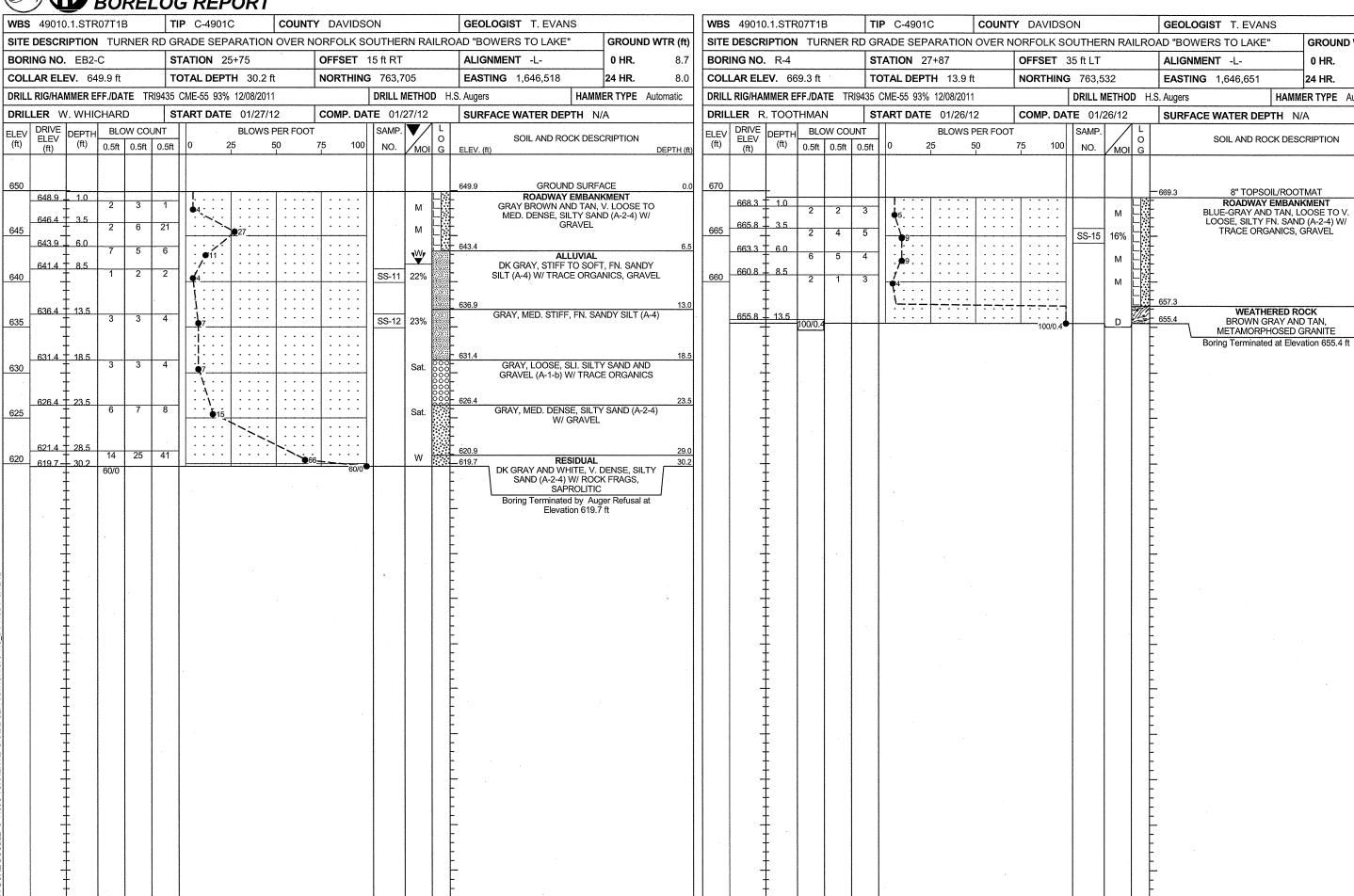
Dry

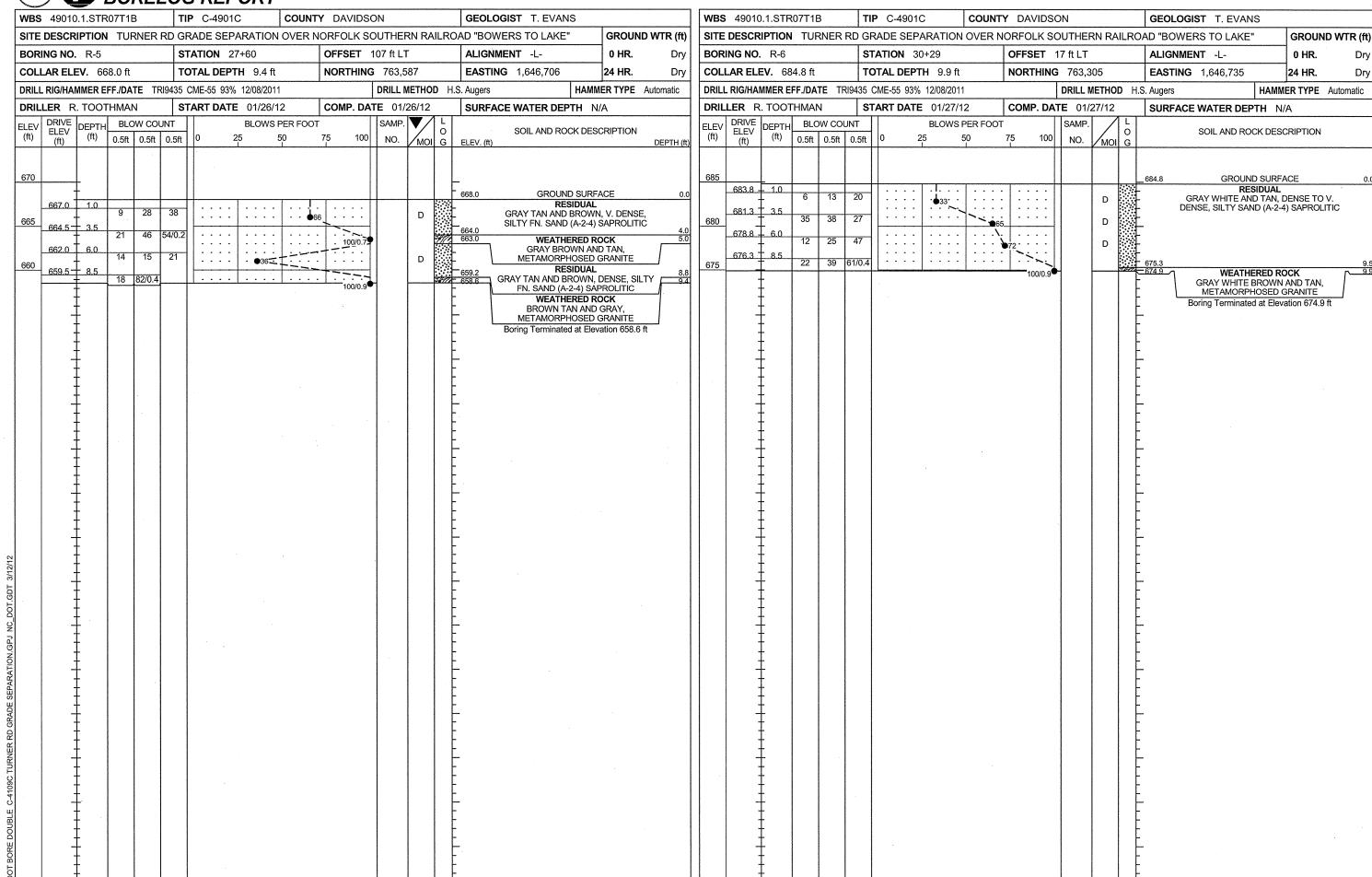
GROUND WTR (ft)

0 HR.

24 HR.

HAMMER TYPE Automatic





FALCON

1210 TRINITY ROAD, SUITE 110, RALEIGH, NORTH CAROLINA 27607

SHEET 22

AASHTO SOIL CLASSIFICATION AND GRADATION SHEET

TURNER ROAD (SR 2005) GRADE SEPARATION OVER NORFOLK SOUTHERN RAILROAD "BOWERS TO LAKE"

WBS NO.: 49010.1.STR07T1B, TIP NO.: C-4901C

DAVIDSON COUNTY, NORTH CAROLINA FALCON ENGINEERING, INC. PROJECT NO: G11019.00

BOR	ING	SAMPLE	то	TAL SAM	PLE	Atte	Natural Moisture		
AASI	HTO Classific	eation	PEI	RCENT PAS	SING		Content		
STATION	OFFSET (FEET)	DEPTH (FEET)	#10	#40	#200	LL	PL	PI	%
EB1	-А	SS-1							
	A-7-6		99	85	63	49	24	25	24.5
20+68	17 ft LT	1.0 - 2.5							
EB1		SS-2							
	A-4		97	80	53	22	16	6	13.8
20+77	17 ft RT	1.0 - 2.5				<u> </u>	<u></u>		
EB1		SS-3							
00.77	A-4		99	88	58	41	NP	NP	24.6
20+77	17 ft RT	13.5 - 15.0			ļ		<u> </u>	<u> </u>	
EB2		SS-4	91	60	20		ND	l ND	٠.
05.04	A-4	1000	91	68	36	20	NP	NP	8.3
25+84 EB2	30 ft LT	1.0 - 2.0					<u> </u>	<u> </u>	
EDZ	A-4	SS-5	100	96	44	19	ND	l ND	~ -
25+84	30 ft LT	8.5 - 10.0	100	90	44	19	NP	NP	21.5
EB2		SS-6							
EDZ	A-2-4	33-0	100	97	33	20	NP	NP	22.5
25+84 30 ft LT EB2-B A-4 26+28 17 ft RT		13.5 - 15.0	100	"	33	20	145	INF	22.5
		SS-7					 	1	
		00-7	95	78	40	25	NP	NP	16.9
		3.5 - 5.0	••		"	-~	'''	'*'	10.5
EB2		SS-8							
	A-4		100	100	92	32	23	9	30.1
26+28	17 ft RT	13.5 - 15.0		100					00.1
EB2		SS-9							
	A-4		100	98	48	20	NP	NP	20.5
26+28	17 ft RT	18.5 - 20.0							
EB2	-В	SS-10							
	A-4	-1	100	99	87	28	21	7	27.2
26+28	17 ft RT	23.5 - 25.0							
EB2		SS-11							
	A-4		100	99	50	16	NP	NP	21.9
25+75	15 ft RT	8.5 - 10.0							
EB2		SS-12	400	400			١		
06.75	A-4	40.5.45.6	100	100	41	20	NP	NP	23.2
25+75	15 ft RT	13.5 - 15.0							
R-1		SS-13	99	0.4	_ F0	25	l ND	No.	444
A-4 18+51 5 ft RT		1.0 - 2.5	55	84	50	35	NP	NP	14.1
R-2		SS-14							
17.77	33-14	100	94	85	74	39	35	33.0	
A-7-5 19+61 42 ft RT 1.0 - 2.5			100	34	00	′4	39	30	აა.∪
R-4 SS-15						<u> </u>			
A-2-4			82	60	34	27	NP	NP	16.4
27+87	35 ft LT	3.5 - 5.0					'''	'*'	.0.7
		V.V V.V		L	L	L	1	L	<u> </u>